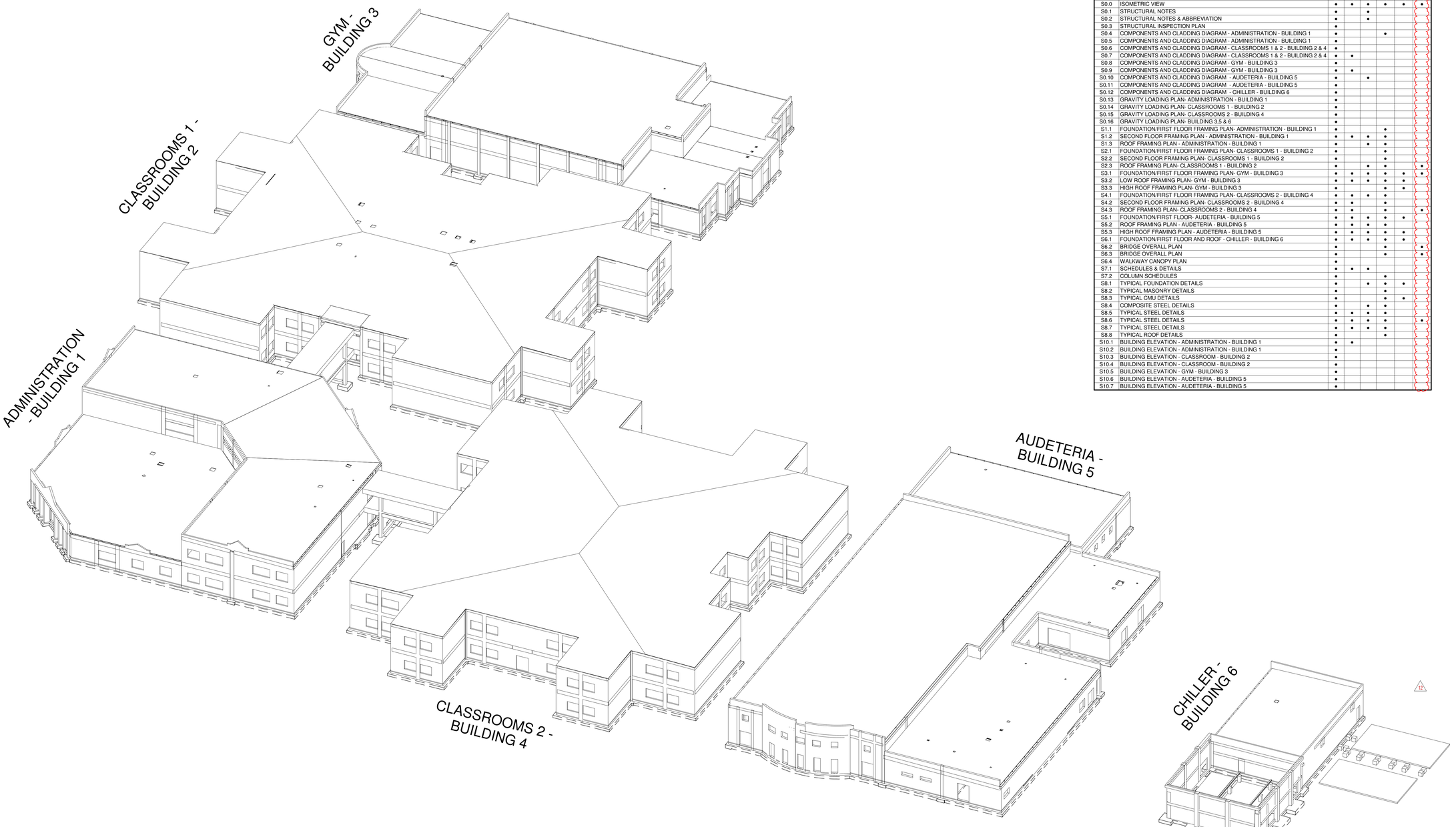


1 S0.0 ISOMETRIC VIEW

ISOMETRIC VIEW IS A GRAPHICAL REPRESENTATION ONLY AND SHALL NOT BE USED FOR CONSTRUCTION PURPOSES

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CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
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SIGNATURE:

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Certificate of Authorization No. 35826

TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

	REVISION NAME	DATE
4	ADDENDUM 4	01.27.2025
8	ASI #8	06.19.2025
11	ASI #11	09.29.2025
12	ASI #14	12.05.2025
13	ASI #17	03.06.2026

FOR BID:

DRAWING TITLE:

ISOMETRIC VIEW

COMMISSION NUMBER:

24FTL018

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S0.0

THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"

3"=1'-0" 1 1/2"=1'-0" 1"=1'-0" 3/4"=1'-0" 1/2"=1'-0" 1/4"=1'-0" 1/8"=1'-0" 1/16"=1'-0"

STRUCTURAL NOTES

STRUCTURAL NOTES:

A. ELECTRONIC VERSIONS OF STRUCTURAL DRAWINGS ARE THE SOLE, COPYRIGHTED PROPERTY OF TRC WORLDWIDE ENGINEERING. ELECTRONIC VERSIONS OF DRAWINGS ARE NOT TO BE USED OR TRANSFERRED WITHOUT THE EXPRESS, WRITTEN PERMISSION OF TRC WORLDWIDE ENGINEERING.

1010 GENERAL:

A. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR DEPRESSIONS, DIMENSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

B. DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.

C. WHERE DRAWINGS AND SPECIFICATIONS ARE IN CONFLICT, THE MORE STRINGENT RESTRICTIONS AND REQUIREMENTS SHALL GOVERN.

D. PLAN NOTES, DETAILS AND SECTIONS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES. TYPICAL DETAILS AND SECTIONS NOT CUT ON PLANS SHALL APPLY UNLESS NOTED OTHERWISE.

E. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE SAFETY OF THE BUILDING AND IT'S COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIE DOWNS. CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT CONSTRUCTION COMPLIES WITH OSHA REGULATION INCLUDING DESIGN OF CONNECTIONS OF MEMBERS THAT WILL NOT BE FULLY COMPLETED AT THE TIME OF INSTALLATION.

F. TOWER CRANES ARE A CONTRACTOR RESPONSIBILITY AND SHALL BE DESIGNED FOR APPLICABLE CRITERIA INCLUDING BUT NOT LIMITED TO INSURANCE REQUIREMENTS AND APPLICABLE BUILDING CODES AT THE PROJECT LOCATION. THE CONTRACTOR SHALL SUBMIT A CRANE LAYOUT, SHOP DRAWINGS, CALCULATIONS AND REACTIONS FOR BOTH FOUNDATIONS AND CRANE TIE-IN LOCATIONS, SIGNED AND SEALED BY A FLORIDA LICENSED PROFESSIONAL ENGINEER. THE ENGINEER OF RECORD SHALL BE RETAINED BY THE CONTRACTOR TO EVALUATE CRANE TIE-IN FORCES ON THE BUILDING STRUCTURE AND TO PROVIDE CRANE FOUNDATION DESIGN.

1011 CONTRACTOR PROPOSED CHANGES AND SUBSTITUTIONS:

A. PROPOSED CHANGES OR SUBSTITUTIONS TO STRUCTURAL DETAILS OR PLANS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD (EOR) FOR REVIEW AND APPROVAL. SUBMITTALS SHALL CONTAIN FULL DOCUMENTATION OF CHANGES OR SUBSTITUTIONS WITH SUPPORTING, SEALED CALCULATIONS (WHERE APPLICABLE). THE REVIEW OF CHANGES AND SUBSTITUTIONS, RE-ANALYSIS AND/OR RE-DRAFTING TO INCORPORATE CHANGES OR SUBSTITUTIONS INTO CONTRACT DOCUMENTS ARE ADDITIONAL SERVICES FOR EOR. EOR IS NOT RESPONSIBLE FOR DETERMINING THE COST EFFECTIVENESS OF PROPOSED CHANGES.

1012 CONTRACTOR REQUIRED REMEDIAL WORK:

A. DESIGN OF REMEDIAL WORK RELATED TO CONSTRUCTION ERRORS, INSTALLATIONS NOT IN CONFORMANCE WITH CONTRACT DOCUMENTS, OR IN ANY WAY BROUGHT ABOUT BY ACTIVITIES OF THE CONTRACTOR, IS NOT WITHIN THE SCOPE OF CONSTRUCTION ADMINISTRATION SERVICES PROVIDED BY TRC WORLDWIDE ENGINEERING. THE CONTRACTOR SHALL CARRY IN THEIR BASE BID THE COSTS FOR ENGINEERING WORK ASSOCIATED TRC PROVIDING ADDITIONAL SERVICES IN REACTION TO NON-CONFORMING WORK.

1060 DESIGN AND CONSTRUCTION STANDARDS:

A. THE STRUCTURAL SYSTEM FOR THIS BUILDING HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING CODES AND REFERENCED STANDARDS:

(FBC 2023)	FLORIDA BUILDING CODE 8th EDITION BY THE INTERNATIONAL CODE COUNCIL, INC
(ADM1-2020)	2020 EDITION OF THE ALUMINUM DESIGN MANUAL
(ACI 318-19)	2019 EDITION OF AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
(AISC 15 ED)	FIFTEENTH EDITION OF THE STEEL CONSTRUCTION MANUAL BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION
(AISI S100-16)	NORTH AMERICAN SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, WITH SUPPLEMENT 2 (2020)
(ASCE 7-22)	MINIMUM DESIGN LOADS AND ASSOCIATED CRITERIA FOR BUILDINGS AND OTHER STRUCTURES BY THE AMERICAN SOCIETY OF CIVIL ENGINEERS
(NDS-18)	2018 EDITION OF NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION WITH 2018 SUPPLEMENT BY AMERICAN FOREST & PAPER ASSOCIATION
(PCI)	PCI DESIGN HANDBOOK – PRECAST AND PRESTRESSED CONCRETE 8TH EDITION
(PTI)	POST-TENSION MANUAL 6TH EDITION, POST TENSION INSTITUTE
(SDI)	STEEL DECK INSTITUTE ROOF DECK DESIGN MANUALL (2ND EDITION), FLOOR DECK DESIGN MANUAL (2ND EDITION), AND DIAPHRAGM DESIGN MANUAL (4TH EDITION)
(SJI-100-15)	STANDARD SPECIFICATIONS FOR K-SERIES, LH-SERIES, AND DLH SERIES OPEN WEB STEEL JOISTS AND FOR JOIST GIRDERS
(TMS 402-2016)	BUILDING CODE FOR MASONRY STRUCTURES

B. CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH THE ABOVE REFERENCED CODES AND STANDARDS AND THE FOLLOWING:

(ACI 117-10)	SPECIFICATIONS FOR TOLERANCES FOR CONCRETE CONSTRUCTION MATERIALS
(ACI 301-16)	SPECIFICATIONS FOR TOLERANCES FOR STRUCTURAL CONCRETE CONSTRUCTION MATERIALS
(ACI 304R-00)	RECOMMENDED PRACTICES FOR MEASURING, MIXING, TRANSPORTING, AND PLACING CONCRETE.
(ACI 305R-10)	HOT WEATHER CONCRETING
(ACI 306R-16)	COLD WEATHER CONCRETING
(ACI 309R-05)	GUIDE FOR CONSOLIDATION OF CONCRETE
(ACI 315-18)	DETAILS AND DETAILING OF CONCRETE REINFORCING
(MNL-15 (16))	FIELD REFERENCE MANUAL: SPECIFICATIONS FOR STRUCTURAL CONCRETE (ACI 301-16) WITH SELECTED ACI AND ASTM REFERENCES
(AWS D1.1-20)	STRUCTURAL WELDING CODE - STEEL
(AWS D1.4-17)	STRUCTURAL WELDING CODE - REINFORCING STEEL
(TPI/WTCA BCSI-18 (W/ 2020 UPDATES))	GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING, RESTRAINING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES

FLOOD LOADS:

A. FEMA ZONE CATEGORIZATION: X (SHADED) FLOOD DESIGN NOT REQUIRED.

1061 DESIGN LOADS:

A. THE STRUCTURAL SYSTEM FOR THIS BUILDING HAS BEEN DESIGNED IN ACCORDANCE WITH THE 2023 FLORIDA BUILDING CODE, 8TH EDITION.

B. FOR SUPERIMPOSED DEAD LOADS AND LIVE LOADS SEE SHEETS:

WIND LOADS	ENHANCED HURRICANE PROTECTION AREA (EHPA) - AUDETERIA, COLD STORAGE	OTHER EDUCATIONAL BUILDINGS - CLASS ROOM, ADMIN, GYM AND CHILLER PLANT	NON EDUCATIONAL STRUCTURES
RISK CATEGORY	IV	III	II
ULT HURRICANE WIND SPEED (3 SECOND GUST)	184 MPH	166 MPH	155 MPH
ASD WIND SPEED (3 SECOND GUST)	143 MPH	129 MPH	120 MPH
TORNADO PLAN AREA			
TORNADO WIND SPEED	76 MPH	50 MPH	NA
WIND EXPOSURE CATEGORY	C	C	C
DIRECTIONALITY FACTOR Cd	1	0.85	0.85
BUILDING CLASSIFICATION	PARTIALLY ENCLOSED	ENCLOSED	ENCLOSED
INTERNAL PRESSURE COEFFICIENTS	-0.55 / +0.55	-0.18 / +0.18	-0.18 / +0.18
TORNADO INTERNAL PRESSURE COEFFICIENTS			
VELOCITY PRESSURE	82 PSF	68 PSF	-

WIND LOAD NOTES:
1. SEE BUILDING'S ROOF UPLIFT PLAN AND WALL WIND LOAD TABLES FOR APPLICABLE PRESSURES.

8

1070 SPECIAL INSPECTIONS:

A. THE AUDETERIA BUILDING - BUILDING 5, IS CLASSIFIED AS A "THRESHOLD BUILDING". SPECIAL INSPECTION OF THE CONSTRUCTION IS REQUIRED BY THE STATE OF FLORIDA IN ACCORDANCE WITH CHAPTER 553 OF THE FLORIDA STATUTES. CONSTRUCTION SHALL BE INSPECTED IN ACCORDANCE WITH THE STRUCTURAL INSPECTION PLAN.

1120 SHOP DRAWING REVIEW:

A. SHOP DRAWING SUBMITTALS ARE REQUIRED FOR ALL COMPONENTS SHOWN ON THESE STRUCTURAL CONTRACT DOCUMENTS INCLUDING, BUT NOT LIMITED TO:

a. CONCRETE MIXES
b. CONCRETE AND MASONRY REINFORCING
c. STRUCTURAL STEEL AND CONNECTIONS
d. STEEL JOIST
e. STEEL DECK
f. LIGHT GAGE FRAMING
g. WOOD ROOF TRUSS FRAMING
h. NEW PLYWOOD
i. ALL FASTENERS, ANCHORS, BOLTS, EPOXY ADHESIVES
j. WOOD FRAMING CONNECTORS
k. WOOD FRAMING
l. PRESSURE TREATED MATERIAL
m. ALUMINUM SHOP DRAWINGS
n. WELDER QUALIFICATIONS/CERTIFICATIONS FOR STEEL AND ALUMINUM

B. SHOP DRAWINGS SHALL PROVIDE ACCURATE, DETAILED DIMENSIONAL INFORMATION AS WELL AS COMPLETE SHOP AND FIELD ERECTION DETAILS NOT SHOWN ON CONTRACT DOCUMENTS NECESSARY FOR FABRICATION AND INSTALLATION OF COMPONENT.

C. SHOP DRAWINGS SHALL BE REVIEWED AND APPROVED BY THE CONTRACTOR'S FIELD ENGINEER PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. DRAWINGS SUBMITTED WITHOUT REVIEW WILL BE RETURNED UNCHECKED.

D. SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC.

E. SHOP DRAWING SUBMITTALS SHALL BE SUBMITTED IN ELECTRONIC PDF FILE FORMAT ONLY.

F. SHOP DRAWINGS SHALL NOT CONTAIN DETAILS COPIED OR REPRODUCED FROM THE CONTRACT DOCUMENTS. REPRODUCTION OF THE CONTRACT DOCUMENTS WILL RESULT IN A REJECTION OF THE SHOP DRAWINGS

G. ELECTRONIC VERSIONS OF STRUCTURAL CONTRACT DOCUMENTS ARE THE SOLE, COPYRIGHTED PROPERTY OF TRC WORLDWIDE ENGINEERING. ELECTRONIC VERSIONS OF DRAWINGS ARE NOT TO BE USED OR TRANSFERRED WITHOUT THE EXPRESS, WRITTEN PERMISSION OF TRC WORLDWIDE ENGINEERING. USERS WILL SIGN A RELEASE AND REIMBURSE TRC WORLDWIDE ENGINEERING. FOR EXPENSES INCURRED IN PREPARING AND TRANSMITTING ELECTRONIC DRAWINGS AT THE RATE TO BE DETERMINED UPON REQUEST.

H. THE CONTRACT DOCUMENTS SHALL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED IN WRITING BY THE ENGINEER.

I. CHANGES AND ADDITIONS MADE ON RE-SUBMITTALS SHALL BE CLEARLY FLAGGED AND NOTED. THE PURPOSE OF THE RE-SUBMITTALS SHALL BE CLEARLY NOTED ON THE LETTER OF TRANSMITTAL. ENGINEER REVIEW WILL BE LIMITED TO THOSE ITEMS CAUSING THE RE-SUBMITTAL.

J. ELECTRONIC SUBMITTALS OF SHOP DRAWINGS WILL ONLY BE ACCEPTED AND REVIEWED PENDING ABOVE CONDITIONS ARE MET. TRC WORLDWIDE ENGINEERING. WILL ELECTRONICALLY RETURN SUBMITTAL IN PDF FORMAT AND WILL NOT BE RESPONSIBLE FOR PRINTING MULTIPLE COPIES FOR DISTRIBUTION.

1121 SHOP DRAWINGS FOR SPECIALTY ENGINEERED PRODUCTS:

A. THE FOLLOWING SYSTEMS AND COMPONENTS AS A MINIMUM REQUIRE FABRICATION AND ERECTION DRAWINGS PREPARED BY A DELEGATED ENGINEER. DELEGATED ENGINEER SHALL POSSESS CURRENT PROFESSIONAL LICENSURE IN THE JURISDICTION OF THE PROJECT. THE SPECIALTY/DELEGATE ENGINEER SHALL POSSESS AND MAINTAIN A MINIMUM OF $1,000,000 LIMIT OF PROFESSIONAL LIABILITY INSURANCE.

a. COLD FORMED STEEL FRAMING
b. ALUMINUM WALL SYSTEMS AND STORM PANELS
c. GLAZED CURTAIN WALLS AND STOREFRONT
d. PREFABRICATED STEEL STAIRS AND RAILINGS
e. ARCHITECTURAL PRECAST CONCRETE ELEMENTS
f. STRUCTURAL PRECAST SYSTEMS
g. GLASS FIBER REINFORCED CONCRETE PANEL SYSTEMS
h. ENGINEERED ROOF TOP EQUIPMENT ATTACHMENT DRAWINGS AND CALCULATIONS
i. OPEN WEB STEEL JOISTS
j. STRUCTURAL STEEL CONNECTIONS REQUIRING ENGINEERING
k. TILT-WALL ERECTION DRAWINGS
l. COLD FORMED STEEL TRUSS SYSTEMS
m. CANOPY ROOFS
n. ALUMINUM CANOPIES
o. PRE-ENGINEERED WOOD FLOOR AND ROOF TRUSSES
p. POST-TENSION REINFORCING STRANDS
q. CONCRETE FORMWORK AND LAYOUT DRAWINGS
r. VIBRO-REPLACEMENT STONE COLUMN SOIL IMPROVEMENT
s. SHORING SYSTEMS

B. SUBMITTALS SHALL CLEARLY IDENTIFY THE SPECIFIC PROJECT AND APPLICABLE CODES, LIST THE DESIGN CRITERIA, AND SHOW ALL DETAILS AND PLANS NECESSARY FOR PROPER FABRICATION AND INSTALLATION. CALCULATIONS AND SHOP DRAWINGS SHALL IDENTIFY SPECIFIC PRODUCT UTILIZED. GENERIC PRODUCTS WILL NOT BE ACCEPTED.

1121 SHOP DRAWINGS FOR SPECIALTY ENGINEERED PRODUCTS (CONT):

C. SHOP DRAWINGS AND CALCULATIONS SHALL BE PREPARED UNDER THE DIRECT SUPERVISION AND CONTROL OF THE DELEGATED ENGINEER. SHOP DRAWINGS AND CALCULATIONS REQUIRE THE IMPRESSED SEAL (OR ELECTRONICALLY SIGNED AND SEALED PER FLORIDA RULES), DATE AND SIGNATURE OF THE DELEGATED ENGINEER.

D. REVIEW BY THE STRUCTURAL ENGINEER OF RECORD OF SUBMITTALS IS LIMITED TO VERIFYING THE FOLLOWING:

a. THAT THE SPECIFIED STRUCTURAL SUBMITTALS HAVE BEEN FURNISHED.
b. THAT THE STRUCTURAL SUBMITTALS HAVE BEEN SIGNED AND SEALED BY THE DELEGATED ENGINEER.
c. THAT THE DELEGATED ENGINEER HAS UNDERSTOOD THE DESIGN INTENT AND HAS USED THE SPECIFIED STRUCTURAL CRITERIA. (NO DETAILED CHECK OF CALCULATIONS WILL BE MADE).
d. THAT THE CONFIGURATION SET FORTH IN THE STRUCTURAL SUBMITTALS IS CONSISTENT WITH THE CONTRACT DOCUMENTS. (NO DETAILED CHECK OF DIMENSIONS OR QUANTITIES WILL BE MADE).

E. SUBMITTALS NOT MEETING THE ABOVE CRITERIA WILL NOT BE REVIEWED.

1122 RFI REVIEW:

A. A CONTRACTOR'S REQUEST(S) FOR INFORMATION (RFI's) SHALL STATE CONTRACTOR'S SUGGESTION(S) FOR RESOLUTION AND COST IMPLICATIONS FOR SUGGESTION(S). ENGINEER OF RECORD IS NOT RESPONSIBLE FOR DETERMINING COST OR COST EFFECTIVENESS OF RFI RESPONSES.

2011 FOUNDATIONS:

A. SEE THE FOLLOWING REPORT FOR COMPLETE GEOTECHNICAL RECOMMENDATIONS AND INSTALLATION PROCEDURES:

REPORT NO: UES PROJECT NO. 3330.2400207.0000
PREPARED BY: UES - 607 NW COMMODITY COVE, PORT ST LUCIE FL 34986
TITLED: GEOTECH EXPLORAATION - WESTERN GROVE K8 SCHOOL. SW POWERLINE RD AND SW TRADITION PKWY PORT ST. LUCIE, FL
DATED: NOVEMBER 15, 2024

THIS REPORT SHALL BE CONSIDERED PART OF THE CONTRACT DOCUMENTS.

B. NO APPROVAL OR VERIFICATION OF RECOMMENDATIONS MADE WITHIN THE ABOVE NOTED GEOTECHNICAL REPORT IS IMPLIED THROUGH REFERENCE OR USE BY TRC.

C. A GEOTECHNICAL ENGINEER, LICENSED WITHIN THE JURISDICTION OF THE PROJECT, SHALL VERIFY IN THE FIELD THAT ALL SITE PREPARATION FILL OPERATIONS, BEARING CONDITIONS, FOUNDATION TESTING AND INSTALLATION COMPLY WITH THE SOILS REPORT.

D. AS PROVIDED BY ABOVE GEOTECHNICAL REPORT, FOUNDATION DESIGN FOR SHALLOW FOUNDATIONS SYSTEMS ARE BASED UPON THE FOLLOWING:

a. ALLOWABLE NET SOIL BEARING PRESSURE 6,000 PSF OVER SOIL TREATED W/ VIBRO-REPLACEMENT TECHNIQUE FOR ADMINISTRATION BLDG, TWO CLASSROOM BLDGS AND THE BRIDGES
b. ALLOWABLE NET SOIL BEARING PRESSURE 2,500 PSF OVER SOIL TREATED WITH GROUND IMPROVEMENT TECHNIQUES FOR THE GYMNASIUM, AUDITERIA, CHILLER ROOM AND YARD ETC.

SEE GEOTECH REPORT FORSITE PREPARATION, BOTH TECHNIQUES ABOVE AND OTHER INFORMATION.

E. FOUNDATION WALLS ARE DESIGNED FOR THE FOLLOWING DESIGN LOADS:

a. SOIL WEIGHT:..... ### PCF
b. AT-REST PRESSURE: (BRACED AT TOP)..... ## PCF
c. ACTIVE PRESSURE:..... ## PCF
d. PASSIVE PRESSURE:..... ### PCF
e. FRICTION COEFFICIENT:.....###

F. SUBGRADE PREPARATION AND VAPOR RETARDER INSTALLATION FOR SLAB-ON-GRADE SHALL BE PERFORMED IN ACCORDANCE WITH PROJECT GEOTECHNICAL REPORT.

G. CONCRETE FOR FOOTINGS SHALL BE PLACED IMMEDIATELY AFTER FINAL INSPECTION AND ACCEPTANCE BY THE GEOTECHNICAL ENGINEER. IN NO CASE SHALL FOOTING EXCAVATIONS BE ALLOWED TO STAND OPEN OVERNIGHT OR DURING RAIN.

2011 FOUNDATIONS (CONT):

H. FOUNDATION WALLS WITHOUT CANTILEVERED FOOTINGS SHALL NOT BE BACKFILLED UNTIL SHORED OR PERMANENTLY SUPPORTED AT THE TOP OF WALL.

I. BACKFILLING OF WALLS AND PIERS SHALL BE PLACED SUCH THAT SYMMETRICAL LOADING SHALL BE MAINTAINED ON BOTH SIDES. WHERE DESIGN CONDITIONS REQUIRE BACKFILLING EACH SIDE TO UNEQUAL HEIGHTS, WALLS OR PIERS SHALL BE FIRMLY SHORED ON POSITION, AND SHORES SHALL REMAIN UNTIL FLOORS OR OTHER PERMANENT BRACING ELEMENTS ARE PLACED AND PROPERLY SET TO PROVIDE FULL SUPPORT.

J. GRADE SHALL BE SUCH THAT THICKNESS OF FOUNDATION, SLAB ON GRADE, ETC. IS NOT REDUCED BY MORE THAN 5% OF THAT SHOWN ON DRAWINGS.

2350 VIBRO REPLACEMENT STONE COLUMN FOUNDATION CONSTRUCTION NOTES:

A. VIBRO REPLACEMENT STONE COLUMN FOUNDATIONS ARE AS DESIGNED BY GEOPIER FOUNDATION COMPANY, OR EQUAL, IN ACCORDANCE WITH REFERENCED GEOTECH REPORT.

B. LAYOUT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR. ELEMENTS SHALL BE INSTALLED IN THE FIELD WITHIN 6 INCHES OF LOCATION SHOWN ON PLAN.

C. DESIGN SHALL BE CONFIRMED BY A FULL-SCALE LOAD TEST PERFORMED IN A LOCATION AGREED UPON BY THE DESIGNER AND THE OWNER'S REPRESENTATIVE.

D. A QUALIFIED, FULL-TIME QUALITY CONTROL (QC) REPRESENTATIVE PROVIDED BY THE FOUNDATION INSTALLER SHALL BE RESPONSIBLE FOR INSTALLATION OF THE ELEMENTS IN ACCORDANCE WITH THE DESIGN, AND SHALL REPORT ALL FOUNDATION REMEDIATION CONSTRUCTION ACTIVITIES TO THE DESIGNER. IF AUTHORIZED BY THE OWNER, THE QC REPRESENTATIVE SHALL COORDINATE QC ACTIVITIES WITH TESTING AGENCY HIRED BY THE OWNER. UNDER NO CIRCUMSTANCE SHALL THE TESTING AGENCY DIRECT VIBRO REPLACEMENT STONE COLUMN ELEMENT INSTALLATION PROCEDURES.

E. ELEMENTS SHALL BE ACCEPTED BASED ON THE FOLLOWING CRITERIA UNLESS OTHERWISE APPROVED IN WRITING BY THE OWNER.

1. DRILL DEPTH SHALL BE WITHIN 3 INCHES OR DEEPER THAN THE DEPTHS SHOWN ON PLAN.
2. AVERAGE COMPACTED LIFT THICKNESS DURING EACH DAY'S PRODUCTION SHALL BE APPROXIMATELY 12 INCHES.
3. ELEMENT AGGREGATE MEASURED DURING BOTTOM STABILIZATION SHALL BE WITHIN 150% OF THE VALUE ACHIEVED IN THE LOAD TEST.
4. ELEMENT AGGREGATE COMPACTION SHALL BE RECORDED PERIODICALLY THROUGHOUT THE DAY. THE AVERAGE BLOW COUNT RECORDED USING A DYNAMIC CONE PENETROMETER IN GENERAL ACCORDANCE WITH ASTM STP #399) SHALL BE GREATER THAN 15 BLOWS FOR 1.75" PENETRATION. NO MORE THAN 10% OF TEST CONDUCTED EACH DAY SHALL FALL BELOW 15 BLOWS FOR 1.75" NOTE: USE OF DYNAMIC CONE PENETROMETER IS NOT APPROPRIATE FOR OPEN GRADE AGGREGATE SUCH AS #57 STONE.
5. ELEMENT AGGREGATE SHALL BE THE SAME AS USED IN A SUCCESSFUL LOAD TEST, UNLESS OTHERWISE APPROVED IN WRITING BY THE DESIGNER (TYPICAL AGGREGATE CONSISTS OF CLEAN 3/4" MINUS GRAVEL FOR THE BOTTOM BULB, AND GRADED 3/4" MINUS CRUSHED STONE FOR THE SHAFT).

F. WHEN OBSTRUCTIONS ARE ENCOUNTERED THAT CANNOT BE REMOVED WITH CONVENTIONAL INSTALLATION EQUIPMENT, THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING THE OBSTRUCTIONS. IF THE GENERAL CONTRACTOR DOES NOT DO SO IN A TIMELY MANNER THAT DOES NOT INTERRUPT FOUNDATION REMEDIATION PRODUCTION, THE INSTALLER MAY REMOVE THE OBSTRUCTION(S) AND SHALL BE REIMBURSED FOR COSTS INCURRED, INCLUDING LABOR, EQUIPMENT, AND MATERIALS.

G. IF CAVE-INS OCCUR ON TOP OF A LIFT OF AGGREGATE SUCH THAT THE VOLUME OF CAVED SOILS IS GREATER THAN APPROXIMATELY 10% OF THE VOLUME OF THE AGGREGATE IN THE LIFT, THEN THE AGGREGATE SHALL BE CONSIDERED CONTAMINATED, AND SHALL BE REMOVED AND REPLACED WITH UNCONTAMINATED AGGREGATE.

H. STONE COLUMN FOOTING BOTTOMS SHALL BE PREPARED IN STRICT ACCORDANCE WITH THE FOLLOWING:

a. OVER-EXCAVATION BELOW THE BOTTOM OF THE FOOTING SHALL BE LIMITED TO 3 INCHES. THIS INCLUDES LIMITING THE TEETH FROM EXCAVATORS FROM OVER-EXCAVATION BEYOND 3 INCHES BELOW THE PLAN FOOTING ELEVATION.
b. FOOTING EXCAVATIONS SHALL BE MADE SHORTLY BEFORE CONCRETE IS PLACED IN THE FOOTING EXCAVATION. IF THE FOOTING CONCRETE IS NOT PLACED ON THE SAME DAY, IT SHOULD BE PLACED THE FOLLOWING CALENDAR DAY. IF IMMEDIATE CONCRETE PLACEMENT IS NOT POSSIBLE, A "MUD-MAT" CONSISTING OF A 3-INCH MINIMUM THICKNESS OF LEAN CONCRETE MAY BE PLACED OVER CUT GROUND SURFACE AFTER FOOTING EXCAVATION.
c. PRIOR TO PLACING CONCRETE OR A MUD-MAT, THE TOP OF THE EXCAVATED SOIL AND STONE COLUMN ELEMENTS SHALL BE COMPACTED WITH A STANDARD, HAND-OPERATED IMPACT COMPACTOR (WHACKER-PACKER OR EQUAL).VIBRATING PLATE TYPE COMPACTORS SHOULD NOT BE USED FOR ELEMENTS INSTALLED IN PREDOMINANTLY FINE-GRAINED SOILS.COMPACTION SHALL BE PERFORMED OVER THE ENTIRE FOOTING BOTTOM TO COMPACT ANY LOOSE SURFACE SOILS OR SURFACE STONE COLUMN AGGREGATE.
d. THE TESTING AGENCY SHALL INSPECT EACH COMPACTED FOOTING BOTTOM AND APPROVE IT IN WRITING ON THE SAME DAY THAT THE CONCRETE IS POURED IN THE FOOTING EXCAVATION. THE APPROVAL SHALL STATE THAT THE FOOTING BOTTOM AND EXPOSED STONE COLUMN AGGREGATE HAS BEEN REASONABLY DENSIFIED WITH A WHACKER-PACKER (OR EQUAL) THE TESTING AGENCY SHALL PROVIDE COPIES OF THE FOOTING INSPECTION REPORTS TO THE DESIGNER.
e. IN THE EVENT THAT FOOTING-BOTTOM PREPARATIONS, AS DESCRIBED ABOVE, ARE NOT PERFORMED OR DOCUMENTED IN THE ACCORDANCE WITH THIS SECTION, ANY WRITTEN OR IMPLIED WARRANTY WITH RESPECT TO VIBRO REPLACEMENT STONE COLUMN FOUNDATION PERFORMANCE CAN BE CONSIDERED VOID.

I. VIBRO REPLACEMENT STONE COLUMN ELEMENTS NOT MEETING THE REQUIREMENTS DEFINED IN THE DESIGN AND LOAD TEST SHALL BE RE-DRILLED AND TAMPED TO MEET PROJECT REQUIREMENTS UNLESS OTHERWISE APPROVED IN WRITING BY THE DESIGNER.

J. FOOTING ELEVATIONS ARE THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE REPORTED IN WRITING TO THE INSTALLER'S QC REPRESENTATIVE PRIOR TO INSTALLING GEOPIER ELEMENTS.

K. UTILITIES' LOCATIONS ARE THE RESPONSIBILITY OF THE CONTRACTOR. THE DESIGNER SHOULD BE NOTIFIED OF ANY CONFLICTS WITH VIBRO REPLACEMENT STONE COLUMN ELEMENT LOCATIONS SHOWN ON THE PLANS.

L. ELEMENTS ARE LOCATED AT THE INTERSECTION OF REFERENCE GRID LINES OR AT THE CENTERLINE OF STRIP FOOTINGS UNLESS DIMENSIONED OTHERWISE.

M. ALL VIBRO REPLACEMENT STONE COLUMN ELEMENTS HAVE A DIAMETER OF 30 INCHES, UNLESS OTHERWISE NOTED. SHAFT LENGTHS BELOW BOTTOM OF FOOTING ARE AS SHOWN ON DESIGN SPREADSHEETS UNLESS OTHERWISE AGREED UPON BY THE DESIGNER AND THE OWNER'S REPRESENTATIVE.

N. AFTER COMPLETION OF STONE COLUMN INSTALLATIONS, THE GENERAL CONTRACTOR IS RESPONSIBLE FOR PROTECTION OF THE WORK. THIS INCLUDES, BUT IS NOT LIMITED TO, PROPER SITE DRAINAGE TO PREVENT PONDING OF WATER ABOVE ELEMENTS AND APPROPRIATE CONTROL AND COORDINATION OF EARTHWORK AND SUBSEQUENT DRILLING ACTIVITIES TO PREVENT DAMAGE TO INSTALLED STONE COLUMN ELEMENTS.

3101 FORMWORK AND SHORING (CONCRETE SLABS AND BEAMS):

A. NO STRUCTURAL CONCRETE SHALL BE STRIPPED UNTIL IT HAS REACHED AT LEAST TWO-THIRDS OF THE 28 DAY DESIGN STRENGTH (& ALL TENDONS STRESSED FOR PT SLABS). A MINIMUM OF 3 STORIES OF SHORING AND (OR) RESHORING SHALL BE USED WHICH SHALL CONSIST OF ONE COMPLETE SET OF VERTICAL SHORES AND TWO SETS OF VERTICAL SHORES THAT COMPRISE AT LEAST 50% OF A COMPLETE SET.

B. DRAWINGS FOR SHORING AND RESHORING SHALL BE PREPARED BY AN ENGINEER LICENSED WITHIN THE JURISDICTION OF THE PROJECT.

C. DESIGN, ERECTION AND REMOVAL OF ALL FORMWORK, SHORES AND RESHORES SHALL MEET REQUIREMENTS SET FORTH IN ACI STANDARDS 347 AND 301.

D. SUBMIT SIGNED & SEALED SHORING DRAWINGS INCLUDING POUR SEQUENCE AND CALCULATIONS, WHERE NECESSARY, TO DEMONSTRATE THAT THE POUR SEQUENCE AND SHORING/RE-SHORING METHODS DO NOT OVERSTRESS THE STRUCTURE. THIS ANALYSIS SHALL INCLUDE STRESSES CAUSED BY SHRINKAGE OF STRUCTURAL SLAB. PROVIDE LOCATION AND DETAILS OF POUR STRIPS IF REQUIRED TO REDUCE SHRINKAGE AND RESTRAINT CRACKS.

E. SHORING INSPECTIONS SHALL BE PERFORMED BY THE SHORING ENGINEER.

F. UNLESS ARCHITECT SPECIFIES OTHERWISE, CONSTRUCT FORMWORK SO CONCRETE SURFACES CONFORM TO THE TOLERANCE LIMITS OF ACI 117 [STANDARD SPECIFICATIONS FOR TOLERANCES FOR CONCRETE CONSTRUCTION MATERIALS]. THE CLASS OF SERVICE FOR OFFSET BETWEEN ADJACENT PIECES OF FORMWORK FACING MATERIAL SHALL BE CLASS B FOR SURFACES PERMANENTLY EXPOSED TO PUBLIC VIEW AND CLASS D FOR SURFACES THAT WILL BE PERMANENTLY CONCEALED.

3103 PLUMBING SLEEVES AND EMBEDDED CONDUITS:

A. LOCATION DRAWINGS FOR ALL SLEEVES AND BLOCKOUTS IN THE CONCRETE SHALL BE SUBMITTED FOR APPROVAL BY THE STRUCTURAL ENGINEER PRIOR TO PLACEMENT.

B. All CONDUIT, SLEEVES, AND PIPES EMBEDDED IN OR PASSING THRU CONCRETE SHALL CONFORM TO SECTION 20.7 OF ACI 318 AND THE FOLLOWING:

a. SLEEVES AND PIPES SHALL BE PLACED SO THAT REINFORCING STEEL CAN BE PLACED WITH THE SPECIFIED COVER AND CLEAR DISTANCE BETWEEN BARS.

b. MINIMUM SLEEVE SPACING SHALL BE THREE DIAMETERS CENTER TO CENTER OF THE LARGER SLEEVE OR 6" CLEAR BETWEEN SLEEVES, WHICHEVER IS GREATER.

c. SLEEVES OR GROUPS OF SLEEVES 16 INCH IN DIAMETER AND LARGER SHALL BE TREATED AS A SLAB OPENING AND REINFORCED PER TYPICAL OPENING REINFORCING DETAILS.

3103 PLUMBING SLEEVES AND EMBEDDED CONDUITS (CONT):

d. CONDUIT AND PIPES PLACED WITHIN SLABS, BEAMS, WALLS AND TOPPING OVER SLABS SHALL OCCUPY ONLY THE MIDDLE ONE THIRD OF THE MEMBER DEPTH OR THICKNESS. MAXIMUM CONDUIT O.D. FOR SINGLE CONDUITS OR SUM OF O.D.'S FOR MULTIPLE CONDUITS THAT CROSS SHALL BE NO LARGER THAN ONE THIRD THE MEMBER DEPTH. PARALLEL CONDUITS SHALL BE SPACED WITH A MINIMUM OF 3 DIAMETERS CLEAR. CONDUITS SHALL BE A MINIMUM OF ONE DIAMETER AWAY FROM AND SHALL NOT INTERFERE WITH OR DISPLAC

STRUCTURAL NOTES

3307 PENETRATIONS:

A. NO PENETRATIONS SHALL BE MADE IN ANY STRUCTURAL MEMBERS OTHER THAN THOSE SPECIFICALLY DESIGNATED ON THE STRUCTURAL DRAWINGS WITHOUT PREVIOUS APPROVAL OF THE ENGINEER. CONTRACTOR SHALL SUBMIT A PENETRATION PLAN FOR APPROVAL INDICATING ANY PENETRATIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS PRIOR TO CONCRETE PLACEMENT.

4203 MASONRY WALLS:

A. MASONRY UNITS SHALL MEET ASTM C90 FOR HOLLOW LOAD BEARING TYPE MASONRY WITH UNIT STRENGTH OF 2000 PSI ON THE NET AREA (f'm = 2000 PSI). MORTAR SHALL BE TYPE "M" WHERE IN CONTACT WITH EARTH AND TYPE "M" OR "S" ELSEWHERE AND MEET ASTM C270. GROUT SHALL BE 2000 PSI MINIMUM COMPRESSIVE STRENGTH AND MEET ASTM C476. WHERE MINIMUM DIMENSION OF ANY VERTICAL, CONTINUOUS CELL IS 3 INCHES OR LESS, USE FINE GROUT, OTHERWISE USE COARSE (PEA GRAVEL) GROUT.

B. MASONRY WALLS SHALL BE LAID IN RUNNING BOND PATTERN UNLESS NOTED OTHERWISE. INTERSECTING WALLS SHALL BE INTERLOCKING WITH RUNNING BOND UNLESS NOTED OTHERWISE. BLOCK CELLS SHALL BE GROUT FILLED WITH VERTICAL REINFORCING BARS, SAME SIZE BARS AS REINFORCING, AT CORNERS, INTERSECTIONS, EACH SIDE OF OPENINGS OVER 4 FEET WIDE, AND AS SHOWN ON THE PLANS. DOWELS SHALL BE USED TO PROVIDE CONTINUITY INTO THE STRUCTURE ABOVE AND/OR BELOW, UNLESS NOTED OTHERWISE. AT VERTICAL BAR TERMINATION, HOOK INTO FOOTING AND BOND BEAM, TIE BEAM, OR SLAB AT TOP OF WALL. USE METAL LATH, MORTAR, OR SPECIAL UNITS TO CONFINE CONCRETE AND GROUT TO AREA REQUIRED. ALL CELLS BELOW GRADE ARE TO BE GROUTED SOLID.

C. PROVIDE 9 GAUGE GALVANIZED HORIZONTAL JOINT REINFORCING (HOHMANN & BARNARD 220 LADDER MESH OR ENGINEER APPROVED SUBSTITUTION) AT ALTERNATE BLOCK COURSES 16" OC VERTICALLY.

D. CONTROL JOINTS SHALL BE PROVIDED IN CONCRETE MASONRY CONSTRUCTION AT LOCATIONS INDICATED ON DRAWINGS. HORIZONTAL WALL REINFORCING SHALL BE STOPPED EACH SIDE OF CONTROL JOINTS. SEE ARCHITECTURAL DRAWINGS FOR SEALANT REQUIREMENTS AT CONTROL JOINTS.

E. SUBMIT PROPOSED GROUT MIX DESIGN FOR REVIEW PRIOR TO USE. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. GROUT SLUMP SHALL BE BETWEEN 9 AND 11 INCHES. USE OF SUPERPLASTICIZER IS PROHIBITED.

F. CELLS TO BE GROUT FILLED SHALL HAVE VERTICAL ALIGNMENT SUFFICIENT TO MAINTAIN A CLEAR, UNOBSTRUCTED, CONTINUOUS VERTICAL GROUT SPACE. CLEANOUT OPENINGS SHALL BE PROVIDED AT THE BOTTOM OF CELLS TO BE GROUT FILLED IN EACH POUR IN EXCESS OF 5 FEET IN HEIGHT. ANY OVERHANGING MORTAR OR OTHER OBSTRUCTION OR DEBRIS SHALL BE REMOVED FROM THE INSIDES OF SUCH CELL WALLS. THE CLEANOUTS SHALL BE SEALED BEFORE GROUTING, AFTER INSPECTION.

G. VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 192 BAR DIAMETERS. CELLS CONTAINING REINFORCEMENT SHALL BE FILLED SOLIDLY WITH GROUT. GROUT SHALL BE CONSOLIDATED AT TIME OF PLACING BY VIBRATING AND RECONSOLIDATED LATER BY VIBRATING BEFORE PLASTICITY IS LOST.

H. PLACE GROUT IN LIFTS NOT EXCEEDING 5 FT. MINIMUM CELL DIMENSION SHALL BE IN ACCORDANCE WITH TMS 402 (3" x 3" FOR COARSE GROUT). MAXIMUM POUR HEIGHT SHALL BE LIMITED TO 12 FT.

I. WHEN THE GROUTING IS STOPPED FOR ONE HOUR OR LONGER, HORIZONTAL CONSTRUCTION JOINTS SHALL BE MADE BY STOPPING THE POUR OF GROUT NOT LESS THAN 1-1/2 INCH BELOW THE TOP OF THE UPPERMOST UNIT GROUTED.

J. PROVIDE BRACING FOR TOP OF ALL INTERIOR CMU WALLS, WHERE CONNECTION DETAILS ARE NOT SHOWN PROVIDE A 1" GAP BETWEEN WALL AND BOTTOM OF STRUCTURE. BRACE TOP OF WALL WITH 14 GA BENT PLATE 4x4x0'-6" LONG AT 4'-0" OC. PROVIDE (3) 1/4" HILTI KWIK CON II SCREWS TO WALL AND (4) #10 SCREWS TO BOTTOM OF STRUCTURE (OR EQUIVALENT CAPACITY CONNECTION). CONNECTION TO ALLOW FOR 1" DEFLECTION OF STRUCTURE THROUGH USE OF SLOTTED HOLES.

K. CONCRETE MASONRY QUALITY CONTROL:

a. WORK IN PROGRESS SHALL BE INSPECTED FOR CONFORMANCE WITH SPECIFIED MATERIALS AND THAT WORKMANSHIP AND CONSTRUCTION IS IN COMPLIANCE WITH PLANS, SPECIFICATIONS, AND INDUSTRY STANDARDS.

b. STRENGTH DETERMINATION SHALL BE MADE USING UNIT STRENGTH OR PRISM TESTS IN ACCORDANCE WITH TMS 402.

4204 HIGH STRENGTH MASONRY:

A. MASONRY UNITS SHALL MEET ASTM C-90 FOR HOLLOW LOAD BEARING TYPE MASONRY. MASONRY UNITS INDICATED AS HIGH STRENGTH SHALL HAVE A UNIT STRENGTH OF 3750 PSI ON THE NET AREA (f'm = 2500 PSI). PROVIDE COLORED DYE IN HIGH STRENGTH MASONRY UNITS TO DISTINGUISH THEM FROM OTHER MASONRY UNITS.

B. MORTAR SHALL BE TYPE "M" OR "S" AND MEET ASTM C-270. GROUT SHALL BE 2500 PSI MINIMUM COMPRESSIVE STRENGTH AND MEET ASTM C-476. SAMPLE AND TEST GROUT IN ACCORDANCE WITH ASTM C1019.

4205 TIE BEAMS:

A. BEAMS WITH THE PREFIX "TB" SHALL BE OF CONCRETE, POURED AFTER THE BLOCK WALLS BELOW ARE IN PLACE. REINFORCING SHALL BE CONTINUOUS THROUGH TIE BEAMS WITH MINIMUM LAP SPLICES OF 48 BAR DIAMETERS UNLESS INDICATED OTHERWISE ON DRAWINGS, AND BENT BARS AT CORNERS. USE METAL LATH, MORTAR, OR SPECIAL UNITS TO CONFINE CONCRETE TO AREA REQUIRED, IN ACCORDANCE WITH TMS 402 (SOLID METAL OR FELT CAVITY CAPS ARE PROHIBITED). TIE BEAM CONCRETE SHALL NOT BE USED FOR CELL FILL IN MASONRY WALLS.

4207 LINTELS:

A. UNLESS NOTED OTHERWISE, MASONRY OPENINGS LESS THAN '* FEET SHALL BE SPANNED WITH 8"x16" CONCRETE LINTELS WITH (2) #5 REINFORCING BARS TOP AND BOTTOM. MASONRY OPENINGS LESS THAN '* FEET SHALL BE SPANNED WITH 8"x12" CONCRETE LINTELS WITH (2) #5 REINFORCING BARS TOP AND BOTTOM. MASONRY OPENINGS LESS THAN "' FEET SHALL BE SPANNED WITH 8"x8" CONCRETE LINTELS WITH (2) #5 REINFORCING BARS BOTTOM. LINTELS SHALL BEAR A MINIMUM OF 8" AT EACH END.

5101 STRUCTURAL STEEL:

A. CONTRACTOR SHALL MAKE ALLOWANCE FOR SUPPLYING AND ERECTING ADDITIONAL STEEL IN ADDITION TO THAT SHOWN ON PLAN, WITHIN SECTIONS, DETAILS OR SCHEDULES TO BE USED IN PLACE AT THE DISCRETION OF THE STRUCTURAL ENGINEER AS FOLLOWS:

a. (3) TONS OF STRUCTURAL STEEL
b. (2) TONS OF STEEL BAR JOIST
c. (5) TONS OF MISCELLANEOUS STEEL, ASSUME 50% ARE HSS MEMBERS

B. ALL STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING UNO ON THE STRUCTURAL DRAWINGS:

a. WIDE FLANGE SHAPES: ASTM A572 OR ASTM A992 (Fy = 50 KSI)
b. CHANNELS, ANGLES, PLATES, BARS: ASTM A36 (Fy = 36 KSI)
c. RECTANGULAR TUBES (HSS):ASTM A500 GRADE B, (Fy = 46 KSI)
d. STRUCTURAL PIPE: ASTM A53 GRADE B (Fy = 35 KSI)
e. ROUND TUBES (HSS): ASTM A500 GRADE C (Fy = 46 KSI)

C. ALL STRUCTURAL BOLTS (INCLUDING WASHERS AND NUTS) SHALL CONFORM TO THE REQUIREMENTS OF ASTM A 325 OR A490. ALL BOLTS SHALL BE TIGHTENED TO THE SNUG TIGHT CONDITION UNO BOLTING OF STRUCTURAL STEEL SHALL CONFORM TO THE PROVISIONS OF RCSC "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A 325 AND A490 BOLTS."

D. ANCHOR RODS TO BE ASTM F1554 GRADE 36 UNLESS NOTED OTHERWISE.

E. WELDER QUALIFICATIONS: QUALIFY WELDING PROCESSES AND WELDING OPERATORS IN ACCORDANCE WITH AWS "STANDARD QUALIFICATION PROCEDURE" PER AWS D1.1. OPERATORS SHALL CARRY PROOF OF QUALIFICATIONS ON THEIR PERSONS.

F. TEST REPORTS: (2) COPIES, PLUS THE NUMBER CONTRACTOR WANTS RETURNED, OF STEEL PRODUCER'S REPORT OF MILL ANALYSIS AND TENSILE AND BEND TESTS FOR STRUCTURAL STEEL MADE NO MORE THAN (60) DAYS BEFORE SHIPMENT.

G. CERTIFICATES: TESTING LABORATORY'S CERTIFICATE THAT:

a. STRUCTURAL STEEL HAS BEEN FURNISHED AND INSTALLED IN ACCORDANCE WITH CONTRACT DOCUMENTS.

b. THE ON-SITE INSPECTIONS HAVE BEEN CONDUCTED AND INSTALLED IN ACCORDANCE WITH THE FIELD QUALITY CONTROL BELOW.

H. TESTING LABORATORY SHALL INSPECT CONNECTIONS IN ACCORDANCE WITH REFERENCES AS FOLLOWS:

a. WELDED CONNECTIONS: INSPECT ALL COMPLETE PENETRATION WELDS AND ALL BUTT WELDS MADE BY FABRICATOR. VISUALLY INSPECT 50% MINIMUM OF FIELD WELDS. SHOULD ANY WELDS FAIL, 100% SHALL BE INSPECTED.

b. BOLTED CONNECTORS: INSPECT AT LEAST 10% OF ALL HIGH STRENGTH BOLTS WHICH ARE WELL SCATTERED THROUGHOUT THE STRUCTURE. IF LESS THAN 95% OF THE BOLTS MEET DESIGN TENSION OR IF ANY BOLT IS LESS THAN 85% OF DESIGN TENSION, THEN ALL BOLTS SHALL BE REWORKED. INSPECT 50% OF ALL REWORKED BOLTS, REPEAT THIS PROCESS UNTIL THE ABOVE REQUIREMENTS ARE MET. LOAD INDICATOR WASHERS MAY BE USED TO TEST 100% OF ALL HIGH STRENGTH BOLTS.

c. VISUALLY INSPECT ALL STEEL DECK ATTACHMENT.

d. INSPECT A MINIMUM OF 20% OF SHEAR CONNECTORS.

e. FOR COMPOSITE BEAMS, AT THE BEGINNING OF EACH WORK SHIFT, (2) STUDS SHALL BE BENT TO 15 DEGREES. STUDS THAT FAIL THIS TEST SHALL BE REPLACED. DO NOT BEND TESTED STUDS BACK TO STRAIGHT.

f. FULL PENETRATION GROOVE WELDS SHALL BE INSPECTED BY ULTRASONIC TESTING. TWENTY-FIVE PERCENT OF THE WELDS SHALL BE INSPECTED AT RANDOM UNLESS NOTED OTHERWISE. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

5101 STRUCTURAL STEEL (CONT):

I. COPIES OF TEST RESULTS AND INSPECTION REPORTS SHALL BE SENT DIRECTLY TO THE ENGINEER PRIOR TO STEEL BEING ENCLOSED BY FINISH MATERIALS. STEEL DECK INSTALLATION MUST BE INSPECTED PRIOR TO VISIT BY STRUCTURAL ENGINEER IN ADVANCE OF SCHEDULED CONCRETE DECK POUR.

J. PRE-CONCRETE & PRE-STEEL ERECTION CONFERENCES SHALL BE HELD BY THE CONTRACTOR WITH SUBCONTRACTORS, TESTING LAB PERSONNEL, ARCHITECT, AND ENGINEERS. THESE CONFERENCES SHALL BE HELD WELL IN ADVANCE OF CONSTRUCTION TO ENSURE PROPER INTERPRETATION OF DESIGN INTENT. STEEL ERECTOR SHALL FIELD VERIFY CORRECTNESS OF FOUNDATION, ANCHOR RODS, OR OTHER EXISTING WORK AFFECTING THE STEEL BEFORE STARTING ERECTION.

K. STRUCTURAL STEEL DETAILING, FABRICATION, AND ERECTION TO BE IN ACCORDANCE WITH THE 15th EDITION OF THE "STEEL CONSTRUCTION MANUAL" OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION. SHOP DRAWINGS SHALL GIVE COMPLETE WELDING INFORMATION, BOTH SHOP AND FIELD, USING AWS SYMBOLS. WELDING ELECTRODES SHALL CONFORM TO AWS A5.1 OR A5.5 E-70XX (LOW-HYDROGEN FOR SMAW WELDING) ALL WELDING PROCEDURES SHALL BE LOW-HYDROGEN PROCESSES. ELECTRODES SHALL BE STORED AFTER OPENING TO MAINTAIN HYDROGEN CONTENTS. ALL CONNECTIONS, EXCEPT THOSE INDICATED ON THE DRAWINGS AS WELDED CONNECTIONS, ARE TO BE MADE USING 3/4" OR 7/8" DIAMETER ASTM A325 BOLTS.

L. SUBMIT SHOP DRAWINGS, SIGNED AND SEALED BY AN ENGINEER LICENSED WITHIN THE JURISDICTION OF THE PROJECT, FOR FABRICATION AND ERECTION OF ALL STEEL MEMBERS IN ACCORDANCE WITH AISC STANDARDS NOTED ABOVE. DETAILER SHALL ASSUME EQUAL BEAM SPACING BETWEEN COLUMN LINES (OR BETWEEN BEAMS THAT ARE SPECIFICALLY LOCATED ON THE DRAWINGS) UNLESS NOTED OTHERWISE ON THE DRAWINGS BY WAY OF SPECIFIC DIMENSIONS.

M. FABRICATOR SHALL DESIGN ALL CONNECTIONS NOT SPECIFICALLY DETAILED ON DRAWINGS. CONNECTIONS SHALL BE DESIGNED BY AN ENGINEER EMPLOYED BY THE CONTRACTOR OR THEIR SUB CONTRACTOR, REGISTERED IN THE JURISDICTION OF THE PROJECT, AND SEALED CALCULATIONS SHALL BE SUBMITTED TO EOR FOR REVIEW AND APPROVAL REGARDLESS OF PROVISION TO THE CONTRARY IN AISC CODE OF STANDARD PRACTICE FOR BUILDINGS AND BRIDGES, ALL CONNECTIONS DESIGNED BY FABRICATOR SHALL BE THEIR RESPONSIBILITY AND REVIEW OF SHOP DRAWINGS BY THE ENGINEER SHALL NOT RELIEVE FABRICATOR OF THEIR RESPONSIBILITY. STEEL FABRICATOR SHALL VERIFY THAT CONNECTION DETAILS SHOWN ON THEIR DRAWINGS CORRESPOND WITH THE REQUIREMENTS OF CONNECTION CALCULATIONS AND CONNECTIONS COMPLETELY DETAILED IN THE CONTRACT DOCUMENTS SHALL NOT BE ALTERED WITHOUT WRITTEN APPROVAL BY THE ENGINEER OF RECORD.

N. UNLESS OTHERWISE NOTED, ALL BEAM CONNECTIONS SHALL BE STANDARD FRAMED OR SEATED CONNECTIONS AS SHOWN IN PART 9 OF THE AISC STEEL CONSTRUCTION MANUAL (13th EDITION). BEAM CONNECTIONS TO TUBE COLUMNS SHALL BE THRU-PLATE TYPE UNLESS SHOWN OTHERWISE. UNLESS GREATER REACTIONS ARE NOTED ON THE DRAWINGS, CONNECTIONS SHALL DEVELOP AT LEAST ONE-HALF OF THE TOTAL UNIFORM LOAD CAPACITY OF THE BEAM. CONNECTIONS SHALL BE DESIGNED AS BEARING-TYPE CONNECTIONS WITH THREADS IN THE SHEAR PLANE, UNLESS OTHERWISE NOTED. IN NO CASE SHALL THE LENGTH OF FRAMED CONNECTIONS BE LESS THAN ONE-HALF THE "T" DISTANCE OF THE BEAM WEB.

O. DESIGN OF MOMENT CONNECTIONS BY DELEGATED ENGINEER SHALL CONSIDER REACTIONS ON WEB OF COLUMN MEMBER AND DESIGN DOUBLER PLATES AS REQUIRED.

P. CONNECTIONS FOR BEAMS WHICH CANNOT CONFORM TO THE TYPICAL CONNECTION DETAILS SHALL BE IN ACCORDANCE WITH THE FOLLOWING:

a. UNLESS OTHERWISE NOTED, ALL BEAM CONNECTIONS SHALL BE STANDARD FRAMED OR SEATED CONNECTIONS AS SHOWN IN PART 9 OF THE AISC STEEL CONSTRUCTION MANUAL (15th EDITION).

b. WHERE BEAM REACTIONS ARE NOT SHOWN ON THE DRAWINGS, CONNECTIONS SHALL BE DETAILED FOR THE MAXIMUM UNIFORM LOAD WHICH THE BEAM WILL SUPPORT (AS SIMPLE BEAM) FOR THE SPAN SHOWN ON DRAWING. MINIMUM BEAM REACTION TO BE USED = 15 KIPS (UNFACTORED).

c. WHERE BEAM REACTIONS ARE SHOWN ON THE DRAWINGS, THE CONNECTIONS SHALL BE DESIGNED BY A DELEGATE ENGINEER TO DEVELOP THE REACTIONS SHOWN.

d. WHERE CONNECTIONS ARE SUBJECT TO ECCENTRICITY, SUCH ECCENTRICITY SHALL BE TAKEN INTO ACCOUNT WHEN DETAILING THE CONNECTION. WELDED CONNECTIONS SHALL BE DETAILED TO TAKE INTO CONSIDERATION THE ECCENTRICITIES OF INDIVIDUAL MEMBERS.

e. MINIMUM NUMBER OF BOLTS PER CONNECTION SHALL BE BASED ON NOMINAL BEAM DEPTH AS FOLLOWS: 2 FOR BEAMS UP TO 10',3 FOR BEAMS 12" AND 14", 4 FOR BEAMS 16" AND 18", 5 FOR BEAMS 21", 6 FOR BEAMS 24", 7 FOR BEAMS 27" OR 30", 9 FOR BEAMS 33",10 FOR BEAMS 36", 12 FOR BEAM 40" OR 44". GIRDER CONNECTIONS SHALL EMPLOY DOUBLE GUSSET PLATES. NON-LABELED CONNECTIONS SHOWN IN DETAILS ARE FOR GENERAL ARRANGEMENT ONLY.

f. THE FOLLOWING BOLTED CONNECTIONS SHALL BE TIGHTENED TO THE MINIMUM BOLT PRETENSION VALUES INDICATED IN TABLE J3.1 OF THE 15th EDITION OF THE AISC STEEL CONSTRUCTION MANUAL:

- MOMENT CONNECTION FLANGE PLATES
- BRACED FRAME CONNECTIONS
- HANGERS
- BOLTS IN TENSION
- ALL SLIP-CRITICAL BOLTS
- ALL CONNECTIONS SUBJECT TO VIBRATION
- ALL ASTM A490 BOLTS
- TIGHTENING SHALL BE DONE BY ANY OF THE FOLLOWING METHODS: DIRECT TENSION INDICATORS; CALIBRATED WRENCH; TURN-OF-NUT METHOD.
- ALL OTHER BOLTED CONNECTIONS MAY BE TIGHTENED ONLY TO THE SNUG-TIGHT CONDITION. THE SNUG-TIGHT CONDITION IS DEFINED AS THE TIGHTNESS OBTAINED BY EITHER A FEW IMPACTS OF AN IMPACT WRENCH, OR THE FULL EFFORT OF A WORKER WITH A SPUD WRENCH THAT BRINGS THE PLIES INTO FIRM CONTACT.

Q. MINIMUM WELD SIZE SHALL BE 3/16" UNLESS OTHERWISE NOTED.

R. GUSSET PLATES SHALL BE 3/8" MINIMUM THICKNESS. UNLESS NOTED OTHERWISE

S. THE GENERAL CONTRACTOR SHALL VERIFY THE REQUIRED CAMBER IN THE FIELD PRIOR TO ERECTION OF EACH MEMBER.

T. SPLICING OF STRUCTURAL STEEL MEMBERS IS PROHIBITED WITHOUT PRIOR APPROVAL OF THE ENGINEER AS TO LOCATION AND TYPE OF SPLICE TO BE MADE. ANY MEMBER HAVING A SPLICE NOT SHOWN AND DETAILED ON SHOP DRAWINGS WILL BE REJECTED.

U. STRUCTURAL STEEL SHALL BE PUNCHED FOR WOOD BLOCKING AND NAILERS IN ACCORDANCE WITH ARCHITECTURAL AND STRUCTURAL DETAILS.

V. THIS STRUCTURE IS NOT DESIGNED FOR EACH COLUMN LINE BENT TO RESIST LATERAL FORCES FROM WIND OR SEISMIC LOADS (UNLESS NOTED OTHERWISE ON PLAN). THIS STRUCTURE DEPENDS ON THE DIAPHRAGM AND BRACING MEMBERS SHOWN. THE CONTRACTOR IS TO PROVIDE LATERAL BRACING IN EACH DIRECTION DURING THE ERECTION PHASE. SUCH BRACING SHALL REMAIN IN PLACE UNTIL ALL DIAPHRAGM AND WIND BRACING ELEMENTS ARE IN PLACE IN THEIR ENTIRETY AND HAVE BEEN APPROVED BY THE STRUCTURAL ENGINEER.

W. SHEAR STUD CONNECTORS SHALL BE FABRICATED AND INSTALLED IN ACCORDANCE WITH AWS D1.1 "STRUCTURAL WELDING CODE", SECTION 7 - STUD WELDING. WHERE STUDS ARE PART OF A COMPOSITE FLOOR SYSTEM, STUD WELDING SHALL BE BY AUTOMATIC WELDING EQUIPMENT ONLY. HAND WELDING OF STUDS IS NOT ALLOWED. STUDS SHALL BE TYPE "B", HEADED STUDS HAVING A MINIMUM TENSILE STRENGTH OF 60,000 psi, AND SHALL BE OF LENGTH AND DIAMETER SHOWN ON STRUCTURAL DRAWINGS.

X. ALL STRUCTURAL STEEL TO RECEIVE SPRAY ON FIRE PROOFING OR TO BE IN CONTACT WITH FRESH CONCRETE SHALL BE BARE, WITHOUT COATING. ALL STRUCTURAL STEEL TO BE EXPOSED TO WEATHER SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A123-17. STEEL COLUMNS BELOW GRADE SHALL PAINTED WITH 2 COATS OF ASPHALTIC PAINT AND ENCASED IN A MINIMUM OF 4" OF CONCRETE. DO NOT PAINT TOP FLANGES OF BEAMS TO RECEIVE SHEAR STUDS. ALL OTHER STRUCTURAL STEEL NOT LISTED ABOVE SHALL RECEIVE ONE (1) SHOP COAT OF PAINT AFTER CLEANING OF ALL RUST, SCALE, AND FOREIGN MATTER. SURFACES SHALL BE DRY AND CLEAN AT TIME OF PAINT APPLICATION. SEE SPECIFICATIONS FOR REQUIREMENTS, INCLUDING TOUCH-UP OF CONNECTIONS IN PRIMED OR GALVANIZED STRUCTURAL STEEL FOLLOWING APPROVAL OF TESTING AGENCY.

Y. SUSPENSION FROM ROOF STRUCTURE:

a. SUBCONTRACTORS INSTALLING CONDUIT, PIPING, OR EQUIPMENT SUSPENDED FROM THE STRUCTURE SHALL ATTEND A PRECONSTRUCTION MEETING.
b. ATTACHMENT TO METAL DECK IS PROHIBITED.
c. PIPE HANGERS SHALL BE ATTACHED TO BOTTOM FLANGES OF WIDE FLANGE BEAMS, AND CHANNELS WITH APPROVED "BEAM CLAMPS" AND "CHANNEL CLAMPS".
d. ALL SINGLE OR MULTIPLE TIER CABLE TRAYS, PIPE RACKS OR GROUPS OF DUCTS SHALL BE SUPPORTED FROM EACH BEAM WHERE SUCH A SYSTEM CROSSES OR AT 8'-0" OC MAX. WHERE SUCH SYSTEMS ARE PARALLEL TO BEAMS.
e. INDIVIDUAL PIPES TO 6" IN DIAMETER SHALL BE SUPPORTED FROM ALL BEAMS WHEN PIPES ARE PERPENDICULAR TO BEAMS AND AT 10'-0" OC MAXIMUM WHEN PIPES ARE PARALLEL TO BEAMS.
f. HANGERS SHALL BE ADDED AT ALL LOCATIONS WHERE VALVES OR FITTINGS OCCUR.

5101 STRUCTURAL STEEL (CONT):

g. ALL CONTRACTORS AND SUBCONTRACTORS SUSPENDING LOADS FROM STRUCTURE SUCH AS ELECTRICAL, MECHANICAL, PLUMBING, FIRE PROTECTION, AND PROCESS CONTRACTORS ARE TO PROVIDE ALL RIGGING REQUIRED FOR COMPLETE INSTALLATION AND FURNISH DRAWINGS SHOWING NECESSARY POINTS OF SUPPORT, REACTIONS AND SUPPLEMENTARY BRACING. PROVIDE REQUIRED SUPPORTS AND HANGERS FOR PIPING, CONDUIT AND EQUIPMENT, SO THAT ALL COMBINED LOADING WILL NOT EXCEED ALLOWABLE LOADINGS OF STRUCTURE AS SHOWN ON THE STRUCTURAL DRAWINGS. SUBMITTAL OF A BID SHALL BE DEEMED A REPRESENTATION THAT THE CONTRACTOR SUBMITTING SUCH A BID HAS ATTAINED ALLOWABLE LOADINGS AND HAS INCLUDED IN THEIR ESTIMATES THE COSTS ASSOCIATED IN FURNISHING REQUIRED SUPPORTS. REQUIRED SUPPORTS SHALL BE COORDINATED WITH OTHER TRADES AND PROPERLY INSTALLED MEETING SPECIFICATIONS OF REPRESENTATIVE ITEMS SUPPORTED. ANY EXPENSE RESULTING FROM IMPROPER LOCATION OR INSTALLATION OF ANCHOR BOLTS, SLEEVES, OPENINGS, INSERTS, HANGERS, RACKS OR SUPPORTS TO BE PAID FOR BY THE CONTRACTOR RESPONSIBLE FOR THAT PORTION OF THE WORK.

Z. SHEAR CONNECTORS SHALL BE AS SHOWN ON PLANS OR DETAILS WELDED DIRECTLY TO BEAM OR THRU METAL DECK. DO NOT WELD TO CLOSURE PLATES.

AA. SLOTTED HOLES FOR BEAM END CONNECTIONS ARE NOT ALLOWED FOR BEAMS ALIGNED ALONG A COLUMN LINE OR WITHIN BRACED FRAMES.

5201 STEEL JOISTS:

A. SHALL BE THE SIZE AND SPACING AS SHOWN ON THE STRUCTURAL DRAWINGS AND SHALL BE DESIGNED, FABRICATED, INSTALLED AND BRIDGED IN ACCORDANCE WITH THE STEEL JOIST INSTITUTE SPECIFICATIONS. ENDS OF BRIDGING LINES TERMINATING AT WALLS OR BEAMS SHALL BE ANCHORED THERETO AT TOP AND BOTTOM CHORDS. MINIMUM JOIST BRIDGING TERMINATION CONNECTIONS TO MASONRY SHALL BE L3 x 3 x 1/4 x 3" LONG WITH (1) 1/2" DIAMETER ANCHOR BOLT OR 1/4 x 4 x 4 WITH (1) 3/4" DIAMETER BOLT. BRIDGING LINES SHALL BE WELDED OR BOLTED AT POINTS OF CONTACT. WELD SHALL NOT DAMAGE THE JOIST. CROSS BRIDGING SHALL BE WELDED OR BOLTED AT ITS CENTER POINT.

B. K-SERIES JOISTS SHALL BEAR A MINIMUM OF 2 1/2" ON STEEL BEAMS AND 4" ON CONCRETE BEAMS. JOIST BEARING PLATES TO BE MINIMUM 3/8" x 4" x 7 1/2" WITH (2) 1/2" DIAMETER x 5" SHEAR STUD CONNECTORS. BEARING PLATES FOR BACK TO BACK SINGLE JOISTS SHALL BE MINIMUM 3/8" x 7 1/2" x 7 1/2" WITH (4) 1/2" DIAMETER x 5" SHEAR STUD CONNECTORS. BEARING PLATES SHALL BE CAST INTEGRALLY WITH THE CONCRETE BEAM. WELD JOISTS TO BEARING PLATE WITH A MINIMUM OF (2) 1/4" FILLET WELDS, UNLESS NOTED OTHERWISE. BACK TO BACK JOISTS SHALL BE OFFSET IF CONCRETE BEAM IS LESS THAN 12" NOMINAL WIDTH OR STEEL BEAM IS LESS THAN 8" WIDE.

D. SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWING SUBMITTAL SHALL INCLUDE LAYOUT, COMPONENT DESIGNATION, BRIDGING, AND PERTINENT SECTIONS AND DETAILS. SUBMITTALS FOR JOISTS, OTHER THAN STANDARD SJI CATALOG SELECTIONS WHICH HAVE BEEN CHECKED BY SJI, SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A REGISTERED PROFESSIONAL ENGINEER WITHIN THE JURISDICTION OF THE PROJECT.

E. JOIST BRIDGING SHALL CONFORM TO THE LATEST SJI SPECIFICATIONS AND/OR OSHA REQUIREMENTS. FIELD WELD BRIDGING AT ENDS AND INTERSECTIONS. ALL JOISTS FORTY (40) FEET AND LONGER REQUIRE A ROW OF BOLTED CROSS BRIDGING TO BE IN PLACE BEFORE SLACKENING OF HOISTING LINES.

F. ALL JOISTS SHALL BE PROPERLY ANCHORED AT BEARINGS BY EITHER BOLTING OR FIELD WELDING PER TYPICAL DETAILS.

G. JOIST BRIDGING AND CONNECTIONS SHALL BE COMPLETELY INSTALLED PRIOR TO PLACING ANY CONSTRUCTION LOADS ON THE JOISTS. CONSTRUCTION LOADING SHALL NOT EXCEED THE JOIST DESIGN LOADS.

H. ALL JOISTS SHALL BE SHOP PAINTED IN ACCORDANCE WITH SJI REQUIREMENTS.

I. THE JOIST MANUFACTURER SHALL SUBMIT CALCULATIONS FOR ALL SPECIAL JOISTS TO THE A/E FOR APPROVAL PRIOR TO FABRICATION. THESE CALCULATIONS SHALL BEAR THE SIGNED AND DATED SEAL OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE PROJECT IS LOCATED.

J. THE JOIST MANUFACTURER SHALL BE A SJI CERTIFIED SHOP AND MAINTAIN APPROVED FABRICATION PROCEDURES AS REQUIRED PER SJI REQUIREMENTS.

5202 JOIST BEARING:

A. WHERE STEEL BEAMS SUPPORT JOISTS FRAMING FROM ONE SIDE ONLY, OR WHERE JOISTS FROM ONE SIDE ARE 30% LONGER THAN THE JOISTS ON THE OPPOSITE OF THE BEAM, JOISTS SHALL BEAR FULL WIDTH OF THE STEEL BEAM OR 5 1/2", WHICH EVER IS LESS.

5203 NET UPLIFT DESIGN:

A. STEEL ROOF JOISTS SHALL BE DESIGNED TO WITHSTAND NET UPLIFT INDICATED ON DRAWINGS. IN ADDITION TO THE STANDARD SJI BOTTOM CHORD BRIDGING, WHICH INCLUDES THE FIRST END PANELS, THE JOIST MANUFACTURER SHALL PROVIDE DESIGN CALCULATIONS FOR UPLIFT, EITHER CONFIRMING THE SJI BRIDGING REQUIREMENT OR PROVIDING A DESIGN ADEQUATE FOR THE UPLIFT. THIS SUBMITTAL SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A REGISTERED PROFESSIONAL ENGINEER WITHIN THE JURISDICTION OF THE PROJECT.
8 DEAD LOAD TO RESIST UPLIFT SHALL NOT BE MORE THAN 5 PSF.

5212 STEEL ROOF DECK:

A. ROOF DECK SHALL BE A GALVANIZED (G90) WIDE RIB STEEL ROOF DECK (WR) OF THE DEPTH AND GAGE AS NOTED ON PLANS AND SHALL CONFORM TO PROVISIONS OF THE STEEL DECK INSTITUTE (SDI) SPECIFICATIONS FOR NON-COMPOSITE STEEL FORM DECK. ATTACHMENT PATTERN SHALL BE AS SPECIFIED ON PLANS.

B. ALL DETAILING, FABRICATION, AND ERECTION SHALL COMPLY WITH ALL APPLICABLE OSHA REGULATIONS INCLUDING ADDITIONAL CONNECTORS, PLATES, HOLES, WELDING, BOLTS, NOT DEPICTED ON THESE DRAWINGS.

C. THE DECK HAS BEEN DESIGNED TO ACT AS A SHEAR DIAPHRAGM. THE DECK PANELS ACT AS BRACING FOR THE BUILDING AND REMOVING ANY OF THE PANELS IS PROHIBITED UNLESS SEPARATE BRACING IS DESIGNED AND PROVIDED. DECK FASTENERS USED AROUND FRAMED OPENINGS SHALL BE THE SAME AS, AND THEIR SPACING EQUAL TO OR CLOSER THAN, THE SPACING USED TO ATTACH THE DECK TO THE FRAME.

D. PROVIDE ALL REQUIRED ACCESSORIES FOR A COMPLETE JOB INCLUDING DECK SUPPORT ANGLES AT COLUMNS AND DECK SPAN CHANGES, COVER PLATES, CLOSURES AND SUMP PANS.

5213 DECK BEND INSTALLATION:

A. ROOF AND FLOOR STEEL DECK SHALL BE OF TYPE SHOWN ON THE CONTRACT DOCUMENTS, AND SHALL BE INSTALLED IN THREE SPAN LENGTH MINIMUM. IF DECK IS INSTALLED IN SINGLE SPAN LENGTHS OR DOUBLE SPAN LENGTHS, THE DECK SHALL BE INSTALLED IN STRICT CONFORMANCE WITH THE STEEL DECK INSTITUTE RECOMMENDATIONS INCLUDING ANY REQUIRED SHORING. SUBMIT SHOP DRAWINGS AND ERECTION PLAN LOCATING EACH SECTION OF DECK AND INCLUDE LOCATION OF ALL SUPPORTS, TYPE AND LOCATION OF ACCESSORIES, DETAILS OF FASTENING, REINFORCEMENT TO BE PROVIDED, DECK PROPERTIES, AND OTHER PERTINENT INFORMATION.

a. CUT DECK SO END JOINTS WILL OCCUR ON SUPPORTING MEMBERS AND OVERLAP A MINIMUM OF 2", UNLESS NOTED OTHERWISE.

b. MINI

STRUCTURAL INSPECTION PLAN FOR AUDETERIA STRUCTURE ONLY

SCOPE OF WORK
THIS STRUCTURAL INSPECTION PLAN IS INTENDED AS GUIDANCE FOR THE SPECIAL INSPECTOR OF THRESHOLD BUILDINGS SO THE STRUCTURAL COMPONENTS CAN BE INSPECTED FOR GENERAL COMPLIANCE WITH THE CONTRACT DOCUMENTS. THE SPECIAL INSPECTOR'S LOCAL AND STATE STATUTORY DUTIES ARE NOT MODIFIED BY THIS INSPECTION PLAN. LOCAL AND STATE STATUTORY REQUIREMENTS FOR SPECIAL INSPECTORS ARE TO SUPERSEDE ANY REQUIREMENTS OF THE INSPECTION PLAN THAT ARE IN CONFLICT. FURTHER, THE CONTRACTOR'S CONTRACTUAL AND STATUTORY OBLIGATIONS AND DUTIES ARE NOT MODIFIED BY THE SPECIAL INSPECTOR'S PERFORMANCE OF THEIR DUTIES. THE CONTRACTOR HAS THE SOLE RESPONSIBILITY FOR REPAIR OR REPLACEMENT OF ANY PART OR PORTION OF THE STRUCTURE NOT CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THE SPECIAL INSPECTOR DOES NOT REPLACE THE DUTIES OF THE BUILDING OFFICIAL'S INSPECTOR, THE QUALITY CONTROL PERSONNEL OF THE CONTRACTOR OR THE DUTIES OF THE OWNER'S CONSTRUCTION MATERIALS TESTING AGENCY.

THE SPECIAL INSPECTOR IS OBLIGATED TO BOTH THE OWNER AND THE BUILDING OFFICIAL FOR OBSERVING THAT THE WORK IS EXECUTED IN GENERAL ACCORDANCE WITH THE CONTRACT DOCUMENTS. THE CONTRACT DOCUMENTS ARE DEFINED AS THE APPROVED, PERMITTED PLANS AND SPECIFICATIONS, AND ALL ADDENDA. INSPECTIONS SHALL BE CONDUCTED IN SUCH A MANNER SO AS NOT TO REASONABLY DELAY PROGRESS OF THE PROJECT.

1. **QUALIFICATIONS**
THE SPECIAL INSPECTOR SHALL BE REGISTERED AS REQUIRED BY CURRENT STATUTE. FUTURE REQUIREMENTS, IF ENACTED BY LAW, WILL ALSO BE APPLICABLE TO THIS PLAN. THE SPECIAL INSPECTOR MAY SEND A FULL-TIME EMPLOYEE AS THEIR AUTHORIZED REPRESENTATIVE TO THE PROJECT. THAT PERSON SHALL POSSESS A MINIMUM OF 5 YEARS VERIFIABLE EXPERIENCE AND KNOWLEDGE IN THE STRUCTURAL SYSTEMS BEING USED AND BE A REGISTERED PROFESSIONAL ENGINEER, GRADUATE CIVIL OR STRUCTURAL ENGINEER WHO POSSESSES THE E.I CERTIFICATE, LICENSED GENERAL CONTRACTOR OR A FLORIDA CERTIFIED BUILDING INSPECTOR. DOCUMENTATION OF SUCH EXPERIENCE SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD (SEOR) FOR APPROVAL PRIOR TO COMMENCING WORK. SPECIFICALLY EXCLUDED FROM ACCEPTANCE BY THIS PLAN SHALL BE THE PERFORMANCE OF THRESHOLD INSPECTIONS BY THE SAME INDIVIDUAL WHO PERFORMS CONCRETE TESTING.

2. **DUTIES**
THE SPECIAL INSPECTOR SHALL MAINTAIN A RECORD OF THE PROGRESS, WORKING CONDITIONS, ALL OBSERVATIONS GIVEN TO THE CONTRACTOR AND DEVIATIONS OF THE STRUCTURAL COMPONENTS OR THE OVERALL INTENT OF THE OFFICIAL CONTRACT DOCUMENTS. HE IS RESPONSIBLE TO POSSESS A THOROUGH KNOWLEDGE OF THE SPECIFICATIONS, APPROPRIATE PORTIONS OF THE GOVERNING CODE AND THE EXERCISE OF GOOD JUDGMENT. THE SPECIAL INSPECTOR'S FIELD REPORTS SHALL BE IN WRITING AND A SEPARATE REPORT SHALL BE SUBMITTED FOR EACH WORKING DAY. A JOB SITE LOG WILL BE MAINTAINED INDICATING THE DATE OF SITE VISITS, THE INSPECTOR'S NAME AND BRIEF DESCRIPTION OF THE DAY'S PRIMARY ACTIVITIES. THE ACTUAL WRITTEN FIELD REPORT WILL BE SUBMITTED IN A TIMELY MANNER AND WITHIN A PERIOD AS DETERMINED AND APPROVED BY THE SEOR. EACH REPORT SHALL CONTAIN AT A MINIMUM THE PROJECT NAME AND LOCATION, BUILDING PERMIT NUMBER, NAME OF KEY INDIVIDUALS PRESENT, DATE, CONTRACTOR'S NAME, NAME OF INDIVIDUAL PREPARING THE REPORT, OBSERVATIONS, DEVIATIONS FROM CONTRACT DOCUMENTS, AND CHANGES MADE IN THE FIELD DURING A PARTICULAR OBSERVATION.

THE REPORT MAY CONSIST OF ANY OR ALL OF THE FOLLOWING:
1. REPORT OF EACH DAY'S INSPECTION
2. AREA INSPECTED
3. SPECIAL RECORDS BY OTHERS (MILL TESTS, CONCRETE TESTS)
4. RECORD OF PLACING AND CURING CONCRETE
5. CHANGES MADE IN THE FIELD DURING A PARTICULAR OBSERVATION
6. MINIMUM (5) PHOTOGRAPHS PER REPORT

DAILY FIELD REPORTS SHALL BE SIGNED BY THE DULY AUTHORIZED REPRESENTATIVE AND SHALL BE SUBMITTED TO THE BUILDING OFFICIAL UNDER A SIGNED AND SEALED COVER LETTER BY THE SPECIAL INSPECTOR. IN ADDITION, THE FINAL LETTER IS TO BE SIGNED AND SEALED BY THE SPECIAL INSPECTOR.

3. **REPORTING INFORMATION AND OBSERVATIONS**
IT IS THE DUTY OF THE SPECIAL INSPECTOR TO NOTIFY THE CONTRACTOR, BUILDING OFFICIAL, ARCHITECT/ENGINEER OF RECORD, AND THE OWNER OF THE FOLLOWING:

A. THE USE OF MATERIALS, EQUIPMENT OR WORKMANSHIP, WHICH DO NOT CONFORM TO THE OFFICIAL CONTRACT DOCUMENTS OR WHICH MAY CAUSE IMPROPER CONSTRUCTION.

B. WORK WHICH IS NOT BEING DONE IN ACCORDANCE WITH THE APPROVED OFFICIAL CONTRACT DOCUMENTS.

C. POTENTIAL RECOMMENDATIONS FOR REMOVAL OR REPAIR OF FAULTY CONSTRUCTION OR CONSTRUCTION WHICH IS PERFORMED WITHOUT INSPECTION AND NOT CAPABLE OF BEING INSPECTED OR TESTED IN PLACE. FINAL DECISIONS REGARDING REMOVAL OR REPAIRS WILL BE MADE BY OTHERS.

4. **REQUIREMENTS OF THE OWNER**
A. THE OWNER SHALL ARRANGE FOR ALL NECESSARY CONSTRUCTION RECORDS TO BE FURNISHED TO THE SPECIAL INSPECTOR IN A TIMELY MANNER. SUCH RECORDS SHALL CONSIST OF, BUT MAY NOT BE LIMITED TO: CONCRETE CYLINDER TEST REPORTS, SOIL DENSITY TEST RECORDS, MILL RECORDS, SHOP DRAWINGS, ETC.

B. THE OWNER SHALL INSURE THAT THE CONTRACTOR PROVIDES A SHORING AND RE-SHORING PLAN TO THE BUILDING DEPARTMENT THAT IS SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE OF FLORIDA. SUCH REGISTERED ENGINEER MAY REPRESENT THE FORM SUPPLIER.

5. **REQUIREMENTS OF THE CONTRACTOR**
THE CONTRACTOR SHALL ADVISE THE SPECIAL INSPECTOR IN ADVANCE OF CONSTRUCTION SCHEDULES AND PLANNED OPERATIONS IN ORDER TO ASSURE TIMELY AND APPROPRIATE INSPECTIONS AND OBSERVATIONS. A MINIMUM OF 24 HOURS NOTICE SHALL BE GIVEN FOR ALL REQUESTED INSPECTIONS.

6. **RESPONSIBILITY**
THE SPECIAL INSPECTOR DOES NOT SURROGATE THE BUILDING OFFICIAL'S OR THE ARCHITECT/ENGINEER OF RECORD'S RESPONSIBILITIES. FURTHER, IT IS NOT INTENDED THAT THE CONTRACTOR'S CONTRACTUAL OR STATUTORY OBLIGATIONS ARE IN ANY WAY RELIEVED OR FOREGONE BY THE PRESENCE OF THE SPECIAL INSPECTOR. THE CONTRACTOR HAS THE SOLE RESPONSIBILITY, BOTH CONTRACTUALLY AND OTHERWISE, FOR ANY DEVIATIONS FROM THE OFFICIAL CONTRACT DOCUMENTS AND FOR ALL CONSTRUCTION QUALITY ASSURANCE AND QUALITY CONTROL. IT IS UNDERSTOOD THAT THE CONTRACTOR IS THE ONLY ENTITY RESPONSIBLE FOR ALL DIRECTON, SUPERVISION, MANAGEMENT AND PLANNING OF THE CONSTRUCTION PROCESS AND FOR ALL MEANS AND METHODS. THE CONTRACTOR IS THE ONLY ENTITY RESPONSIBLE FOR PROVIDING WARRANTEES AND GUARANTEES TO THE OWNER.

IT IS ALSO UNDERSTOOD THAT THE SPECIAL INSPECTOR IS CONTRACTUALLY NOT OBLIGATED TO PERFORM 100% INSPECTIONS OF ALL STRUCTURAL ITEMS NOR ARE 100% INSPECTIONS REQUIRED BY THIS INSPECTION PLAN. THE SPECIAL INSPECTOR'S ROLE IS TO SPOT CHECK THE CONSTRUCTION OF THE STRUCTURAL ELEMENTS AS INDICATED BELOW. IT SHALL BE THE RESPONSIBILITY OF THE SPECIAL INSPECTOR TO OBSERVE THE PLACEMENT OF STRUCTURAL COMPONENTS ONLY, AND TO REPORT TO THE CONCERNED PARTIES THAT THE STRUCTURE IS BEING BUILT IN GENERAL CONFORMANCE WITH THE STRUCTURAL DOCUMENTS, AND IF NOT, THE LOCATION AND DESCRIPTIONS OF THOSE VARIATIONS. AT THE COMPLETION OF THE STRUCTURAL WORK, THE SPECIAL INSPECTOR SHALL STATE THAT "TO THE BEST OF MY KNOWLEDGE AND BELIEF, THE ABOVE DESCRIBED CONSTRUCTION OF STRUCTURAL LOAD BEARING COMPONENTS GENERALLY COMPLIES WITH THE PERMITTED DOCUMENTS, AND THE SHORING AND RE-SHORING CONFORMS WITH THE SHORING AND RESHORING PLANS SUBMITTED TO THE ENFORCING AGENCY".

7. **LIMITATIONS**
THE FOLLOWING INSPECTION PLAN IS OUTLINED AS A MINIMUM ONLY AND MAY BE SUPPLEMENTED BY SPECIAL REQUIREMENTS AS DEEMED NECESSARY DURING THE COURSE OF THE WORK. THE CONTRACTOR'S PERFORMANCE AND CAPABILITY MAY ONLY INCREASE THE SPECIAL INSPECTOR INVOLVEMENT IN ANY PROJECT. THIS DOCUMENT WILL OUTLINE THE PLAN ONLY AS WELL AS DELINEATE SPECIFIC SUBMISSION REQUIREMENTS. THE ACTUAL DETAILS OF SPECIAL INSPECTOR CHECKLISTS WILL NOT BE ELABORATED UPON, BUT RATHER A CLEAR DEFINITION OF RESPONSIBILITIES, SCHEDULED VISITS, AND INTENT WILL BE GIVEN.

THE SPECIAL INSPECTOR IS TO PROVIDE SERVICES ONLY WITH REGARD TO THE STRUCTURAL FRAME OF THE BUILDING INCLUDING FOUNDATION, PRIMARY AND SECONDARY FRAMING SYSTEMS, CONNECTIONS ONLY OF GLASS AND CURTAIN WALL SYSTEMS TO A BUILDING STRUCTURAL COMPONENT, VARIOUS STRUCTURAL SYSTEMS DESIGNED BY SPECIALTY ENGINEERS AND ITEMS INCLUDED IN THE INSPECTION PLAN. THIS DOES NOT INCLUDE INSPECTION OF ANY SAFETY PROVISIONS AS REQUIRED BY OSHA OR OTHER SAFETY STANDARDS WHICH APPLY DURING THE CONSTRUCTION PERIOD, NOR DOES IT APPLY TO ELEMENTS SUCH AS GLASS, METAL OR WOODEN RAILINGS, FIRE PROTECTION, ROOFING, MECHANICAL/ELECTRICAL SYSTEMS, ARCHITECTURAL COMPONENTS, SITE WORK OR OTHER ELEMENTS NOT CONTRIBUTING TO THE CAPACITY OF THE STRUCTURAL BUILDING FRAME.

SINCE THE INSPECTOR DOES NOT VERIFY THAT THE OFFICIAL CONTRACT DOCUMENTS ARE IN THEMSELVES IN COMPLIANCE WITH THE CURRENT EDITION OF THE BUILDING CODE, STATEMENTS ISSUED WILL REFER TO COMPLETED CONSTRUCTION BEING IN GENERAL ACCORDANCE WITH THE OFFICIAL CONTRACT DOCUMENTS RATHER THAN THE APPROPRIATE BUILDING CODE.

STRUCTURAL INSPECTION PLAN
1. **FOLLOWING IS A DESCRIPTION OF THE STRUCTURAL SYSTEM EMPLOYED FOR THIS PROJECT:**

1. FOUNDATION: CAST-IN-PLACE (CIP) CONCRETE FOOTINGS, COLUMN SPREAD FOOTINGS AND THICKENED SLAB FOOTINGS.
2. GROUND FLOOR: CIP CONCRETE SLAB ON COMPACTED FILL.
3. STRUCTURE: REINFORCED MASONRY WALLS WITH PRECAST CONCRETE PLANK FLOORS AND PRE-ENGINEERED WOOD TRUSSES AT ROOF.

2. CONSTRUCTION SHALL GENERALLY CONFORM TO THE APPROVED PERMITTED DOCUMENTS AS SUBMITTED TO THE BUILDING DEPARTMENT.

3. STRUCTURAL DRAWINGS, THE APPLICABLE SPECIFICATIONS AND DRAWINGS SUBMITTED BY SPECIALTY ENGINEERS, INCLUDING SHORING AND RE-SHORING PLANS, SHALL GOVERN THE REQUIREMENTS OF THIS WORK.

4. **SUBSURFACE AND FOUNDATIONS**

A. SOILS INVESTIGATION
THE SPECIAL INSPECTOR SHALL OBTAIN A COPY OF THE FINAL PROJECT SOILS REPORT, SIGNED AND SEALED BY A PROFESSIONAL ENGINEER, AND MAINTAIN SAID COPY IN THE SPECIAL INSPECTOR'S FILE. SPECIAL INSPECTOR SHALL CONFIRM THAT NOTED REGARDING FOUNDATION PREPARATION AND INSTALLATION CONTAINED WITHIN SUCH SOILS REPORT AND THE CONTRACT DOCUMENTS ARE IN AGREEMENT WITH THE CONSTRUCTION.

B. BACKFILL AND COMPACTION
A QUALIFIED SOILS CONSULTANT SHALL BE RETAINED TO MONITOR BACKFILL AND COMPACTION OPERATIONS AND PLACEMENT OF SLAB ON GRADE SUB-BASES. THE SOILS CONSULTANT'S REPORTS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA REPORTS AND SHALL BE SUBMITTED TO THE SPECIAL INSPECTOR. AT THE CONCLUSION OF FOUNDATION CONSTRUCTION, A REPORT SHALL BE SUBMITTED TO THE SPECIAL INSPECTOR, SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER EMPLOYED BY THE GEOTECHNICAL ENGINEER, STATING THAT THE GEOTECHNICAL ENGINEER, OR THEIR REPRESENTATIVE, HAS WITNESSED INSTALLATION OF THE FOUNDATION PREPARATION, THAT THE REQUIRED BEARING STRATA AND COMPACTION UNDER EACH FOOTING HAD BEEN OBTAINED, AND THAT THE FOUNDATION SOILS WERE CONSTRUCTED IN ACCORDANCE WITH THE REQUIREMENTS OF THE GEOTECHNICAL REPORT, INCLUDING THE SPECIFIED DESIGN CAPACITIES.

C. FOUNDATIONS:
1. THE SPECIAL INSPECTOR SHALL:
   a. REVIEW THE PROCEDURES AND FIELD METHODS UTILIZED BY THE FULL TIME SITE REPRESENTATIVE OF THE GEOTECHNICAL ENGINEER AND TESTING LABORATORY.
   b. MAKE PERIODIC VISITS TO THE JOBSITE TO REVIEW THE INSTALLATION OF THE FOUNDATIONS, VERIFY REINFORCEMENT AND COVER IN FOOTINGS.
   c. VERIFY WALL AND COLUMN DOWELS, EMBEDMENT LENGTHS, CONFIGURATION AND EXTENSIONS TO PROVIDE PROPER LAPS ABOVE.
   d. VERIFY FOUNDATION DIMENSIONS.

D. SPECIAL CONDITIONS/COMPONENTS
1. THE SPECIAL INSPECTOR SHALL:
   a. VERIFY THAT DOWELS FOR VERTICAL WALL REINFORCEMENT AND COLUMNS ARE IN PLACE AND ARE IN ACCORDANCE WITH THE STRUCTURAL DRAWINGS.
   b. VERIFY THAT AN AS-BUILT SURVEY HAS BEEN PERFORMED AND SUBMITTED TO THE ENGINEER OF RECORD OF THE AS-BUILT PILING LOCATIONS.
   c. VERIFY THAT THE VARIOUS PROFESSIONALS INVOLVED IN THE PERMANENT DRAINAGE SYSTEM HAVE PERFORMED THEIR INSPECTIONS PRIOR TO PLACING SOIL ABOVE THE SYSTEM.
   d. VERIFY THAT THE REQUIREMENTS INDICATED ON THE CONTRACT DOCUMENTS AS TO THE STAGING OF BACKFILLING BEHIND BELOW-GRADE WALLS HAS BEEN FOLLOWED.

5. **CONCRETE**

A. FORMS:
1. THE SHORING SPECIALTY ENGINEER SHALL PROVIDE SIGNED AND SEALED SHOP DRAWINGS INDICATING THE TYPE, SIZE AND LAYOUT OF THE FORMWORK AND SHORING. THEN THE SPECIALTY ENGINEER SHALL INSPECT THE SHORING AND FORMWORK AND PROVIDE A STATEMENT THAT THE SHORING IS IN ACCORDANCE WITH THE SIGNED AND SEALED SHOP DRAWINGS. THIS REQUIREMENT APPLIES TO EACH LEVEL OF THE BUILDING. THE SPECIAL NSPECTOR SHALL RECEIVE A COPY OF THIS CERTIFICATION AND THEN WILL MAKE A CURSORY INSPECTION OF SHORING TO CONFIRM GENERAL COMPLIANCE WITH SIGNED AND SEALED SHORING DRAWINGS.

2. FORM TIES AND SPACERS SHALL BE STRONG ENOUGH NOT TO BEND IF WORKMEN CLIMB ON THEM. WIRE TIES ARE PERMITTED ONLY ON LIGHT WORK.

3. BEFORE CONCRETE IS PLACED, FORM SURFACES SHALL BE WETTED, OILED OR COATED WITH SATISFACTORY MATERIALS. IF OILING IS REQUIRED BY SPECIFICATIONS, THE OIL SHOULD BE APPLIED BEFORE REINFORCING STEEL IS PLACED TO AVOID CONTACT WITH THE REINFORCING STEEL.

4. FOREIGN MATERIAL SUCH AS CHIPS, BLOCKS, SAWDUST AND DRIED MORTAR SHALL BE REMOVED BEFORE THE PLACEMENT OF CONCRETE. AIR AND WATER SHALL BE UTILIZED FOR THIS OPERATION.

5. SLEEVES REQUIRED FOR CMU WALL REINFORCING AND/OR WALL REINFORCING DOWELS SHALL BE CHECKED FOR GENERAL CONFORMANCE

B. JOINTS:
1. THE SPECIAL INSPECTOR SHALL:
   a. CHECK THE LOCATION JOINTS, SUCH AS EXPANSION, CONTRACTION AND CONSTRUCTION JOINTS AS SHOWN IN CONTRACT DRAWINGS OR AS OTHERWISE APPROVED.
   b. VERIFY THE PREPARATION OF JOINTS AS REQUIRED BY CONTRACT DRAWINGS AND/OR SPECIFICATIONS, SUCH AS WET SAND_BLASTING, ROUGHENING, WETTING, ETC.
   c. CHECK THE LOCATION OF BULKHEADS IN STRUCTURAL MEMBERS SUCH AS BEAMS OR SLABS.
   d. VERIFY THAT EXPANSION JOINTS ARE FREE FROM DEBRIS OR IRREGULARITIES WHICH WOULD INTERFERE WITH FREE MOVEMENT.
   e. CHECK IF FILLER HAS BEEN INSTALLED AND SECURELY FASTENED IN EXPANSION JOINTS IN ACCORDANCE WITH SPECIFICATIONS AND/OR MANUFACTURER'S RECOMMENDATIONS.
   f. CHECK THAT CONTRACTION JOINTS HAVE BEEN COATED WITH THE SPECIFIED APPROVED MATERIAL NECESSARY TO BREAK BOND.

C. OPENINGS AND SLEEVES:
1. THE SPECIAL INSPECTOR SHALL:
   a. REPORT ALL SLAB OPENINGS LARGER THAN 12" (OR AS INDICATED ON THE STRUCTURAL DRAWINGS) AND THOSE NOT SHOWN ON STRUCTURAL DRAWINGS.
   b. VERIFY PLACEMENT OF ADDITIONAL REINFORCING AROUND OPENINGS. NO SLEEVES OR OPENINGS WILL BE PERMITTED IN BEAMS WITHOUT PRIOR APPROVAL OF THE SEOR.
   c. CHECK THE LOCATION OF OPENINGS
   d. IF ADDITIONAL OPENINGS WHICH ARE NOT SHOWN ON THE PLANS ARE REQUIRED, THE PROPOSED OPENINGS SHALL BE APPROVED BY THE SEOR AND, WITH THE EXCEPTION OF VERY SMALL OPENINGS, SHALL BE PROTECTED BY SHORT DIAGONAL REINFORCING STEEL BARS, AS SPECIFIED ON THE STRUCTURAL DRAWINGS.

D. CONCRETE PLACEMENT:
1. THE SPECIAL INSPECTOR SHALL:
   a. BE PRESENT ON THE SITE FOR CONCRETE POURS. IT IS INCUMBENT ON THE GENERAL CONTRACTOR TO PROPERLY NOTIFY THE INSPECTOR OF UPCOMING CONCRETE POURS.
   b. VERIFY THAT THE CONTRACTOR IS PROVIDING THE NECESSARY CONSOLIDATION AND PLACING TECHNIQUES.
   c. REVIEW ASPECTS OF TRANSPORTING, HANDLING AND PLACEMENT OF CONCRETE TO AVOID ADVERSELY AFFECTING CONCRETE MIX.
   d. VERIFY THAT AREAS REQUIRING SPECIFIC WATER/CEMENT RATIOS ARE PROVIDED WITH THE PROPER CONCRETE MIX.
   e. VERIFY THAT ANY CONCRETE MIX WHICH HAS WATER ADDED AT THE SITE HAS BEEN BROUGHT TO THE ATTENTION OF THE CONTRACTOR AND THAT A SET OF TEST CYLINDERS IS TAKEN FROM THE MIX AFTER WATER IS ADDED. NO WATER SHALL BE ADDED TO ANY MIX WHICH HAS A SPECIFIC WATER/CEMENT RATIO REQUIREMENT UNLESS APPROVED BY THE SEOR.
   f. VERIFY THAT THE CONCRETE TESTING LAB IS PRESENT ON THE SITE.
   g. VERIFY THAT NO CONCRETE IS PLACED WHICH HAS AGED PAST THE LIMITATIONS OF THE CONTRACT DOCUMENTS.
   h. REPORT THE ADDITION OF WATER TO THE CONCRETE MIX IN THE TRUCK. THE ADDITION OF WATER BEYOND THE LIMITS GIVEN ON THE APPROVED MIX DESIGN IS NOT PERMITTED.
   i. DOCUMENT SPECIFICALLY WHERE ANY CONCRETE IS PLACED THAT DOES NOT MEET THESE REQUIREMENTS.
   j. RECORD REJECTED BATCHES CAREFULLY, WITH REASONS FOR REJECTION, AND REPORT IT PROMPTLY TO THE SEOR.
   k. PERIODICALLY CHECK TRANSIT MIX DELIVERY SLIPS TO CONFIRM CORRECT MIX AND TO DETERMINE ELAPSED TIME BETWEEN BATCHING AND DEPOSITING OF CONCRETE.
   l. VERIFY THAT SLUMP IS MEASURED AT THE PLACE OF DEPOSIT FOR EACH BATCH. EMPHASIZE SLUMP MEASUREMENTS WHEN MIX CONSISTENCY IS IN DOUBT.
   m. VERIFY THAT CONCRETE TESTING TECHNICIAN TAKES CONCRETE TEST CYLINDERS IN ACCORDANCE WITH CONTRACT SPECIFICATIONS AND REVIEW RESULTS OF STRENGTH TESTS FOR COMPLIANCE WITH REQUIREMENTS OF DRAWINGS.
   n. CONFIRM THAT THE CONTRACTOR USES MECHANICAL VIBRATORS TO CONSOLIDATE THE FRESH CONCRETE AND PAY PARTICULAR ATTENTION TO CONSOLIDATION BEHIND EMBEDDED ANCHORAGES, TENDON BEARING PLATES, AND IN BEAM-COLUMN JOINTS.
   o. CHECK THAT CURING METHODS HAVE BEEN EXECUTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
   p. VISUALLY CHECK THE UNIFORMITY OF FRESHLY MIXED CONCRETE.
   q. CHECK FOR HONEYCOMBING AND ROCK POCKETS IN CAST CONCRETE AFTER THE REMOVAL OF FORMS. DURING REPAIRS, VERIFY THAT ADEQUATE MATERIAL HAS BEEN CUT BACK TO EXPOSE SOLID MATERIAL AND THAT ALL LOOSE MATERIAL HAS BEEN REMOVED. LARGE REPAIR AREAS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER.
   r. CHECK THE REQUIREMENTS FOR THE TREATMENT OF AREAS CONTAINING DEFECTIVE CONCRETE.

E. REINFORCEMENT - (MILD STEEL):
1. THE SPECIAL INSPECTOR SHALL:
   a. SPOT CHECK THAT REINFORCEMENT IS IN PLACE IN GENERAL ACCORDANCE WITH THE STRUCTURAL DRAWINGS. IN THE EVENT OF ANY CONFLICT BETWEEN THE CONTRACT DOCUMENTS AND SUPPLIER'S SHOP DRAWINGS, NOTIFY THE SEOR IMMEDIATELY.
   b. PRIOR TO THE PLACEMENT OF CONCRETE, SPOT CHECK THE FOLLOWING ITEMS:
      1. REINFORCEMENT PLACEMENT FOR SIZE, BENDING, GRADE, LENGTHS OF SPLICES, AND BAR_TO_BAR SPACING.
      2. REINFORCEMENT HAS BEEN CLEANED OF LOOSE, FLAKY, RUST AND SCALE, GREASE OR OTHER FOREIGN MATERIALS WHICH WOULD REDUCE OR PREVENT BOND.
      3. REINFORCEMENT IS GENERALLY TIED AND SUPPORTED SECURELY SO THAT DISPLACEMENT WILL NOT OCCUR DURING CONCRETE PLACEMENT.
      4. CONCRETE COVER OF REINFORCEMENT IN PLACE PER THE PROJECT DRAWINGS AND SPECIFICATIONS.
      5. LOCATION AND LENGTH OF SPLICES OF REINFORCEMENT SHALL BE CHECKED AND SPLICES SHALL NOT BE MADE WITHOUT THE APPROVAL OF THE SEOR.

F. EMBEDDED FIXTURES:
1. UNLESS OTHERWISE PROVIDED OR APPROVED, ANCHOR BOLTS, NSERTS, PIPE SLEEVES, PIPES, CONDUITS, WIRING, FLASHINGS, INSTRUMENTS, AND OTHER EMBEDDED FIXTURES AND MECHANICAL EQUIPMENT SHALL BE FIXED FIRMLY IN CORRECT POSITION BEFORE CONCRETE IS PLACED.

2. IF EMBEDDED ITEMS ARE IN CONFLICT WITH EACH OTHER OR WITH REINFORCING STEEL BARS, THE RELOCATION OF THESE ITEMS AND/OR CUTTING, BENDING, DISPLACEMENT OR OMISSION OF STEEL REINFORCING BARS SHALL NOT BE ALLOWED EXCEPT AS APPROVED BY THE ENGINEER.

6. **STRUCTURAL STEEL**

A. THE SPECIAL INSPECTOR SHALL:

a. REVIEW THE APPROVED SHOP DRAWINGS.
b. VERIFY STEEL SIZES AND GRADES AND STEEL GIRDER DESIGNATIONS WITH CONTRACT DRAWINGS.
c. VERIFY THAT STEEL MEMBERS ARE CORRECTLY LOCATED.
d. AT BOLTED CONNECTIONS, VERIFY TYPE OF BOLT, SIZE, WASHER, AND METHOD OF TIGHTENING. CONFIRM THAT BOLT TIGHTENING HAS BEEN COMPLETED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS AND STEEL SHOP DRAWINGS.
e. VERIFY THAT WELDED CONNECTIONS ARE PERFORMED BY CERTIFIED WELDERS. DETERMINE AND RECORD THE TYPE OF WELDING ELECTRODES. CONFIRM THAT WELDERS ARE PERFORMING WELDS IN POSITIONS THAT THEY HAVE BEEN CERTIFIED TO PERFORM.
f. REVIEW DOCUMENTATION FROM THE INDEPENDENT STEEL TESTING LAB VERIFYING THE FOLLOWING, IF APPROPRIATE:
   - TESTING IS COMPLETED PER PROJECT CONTRACT DOCUMENTS AND VERIFIED IN THE FIELD
   - INSPECTION OF HIGH-STRENGTH BOLTING
   - INSPECTION OF WELDING
   - TESTING OF STUD CONNECTIONS TO BEAMS, ANGLES AND PLATES.
g. ALL DISCREPANCIES SHALL BE RECORDED AND REPORTED TO THE SEOR.

7. **NON-SHRINK GROUT UNDER STEEL BASE PLATES**

A. THE SPECIAL INSPECTOR SHALL PERFORM THE FOLLOWING:
a. COMPRESSIVE STRENGTH TESTS PER ASTM C109.
b. NUMBER OF TESTS: ONE TEST FOR EACH TEN BAGS OF GROUT USED OR MINIMUM OF ONE TEST FOR EACH DAY OF GROUTING.
c. CUBE SIZE: 2 INCH x 2 INCH
d. TEST SCHEDULE: ONE CUBE AT 3 DAYS, TWO CUBES AT 7 DAYS, 3 CUBES AT 28 DAYS.

1. **SPECIALTY ENGINEERED ITEMS**

A. THE SPECIAL INSPECTOR SHALL:

a. VERIFY THAT WORK IS BASED ON SIGNED, SEALED AND APPROVED DRAWINGS.
b. VERIFY THAT WORK IS IN ACCORDANCE WITH BOTH THE SPECIFIC REQUIREMENTS OF THE SHOP DRAWINGS PREPARED BY THE SPECIALTY ENGINEER AND THE SCHEMATIC REQUIREMENTS OF THE CONTRACT DOCUMENTS. NOTIFY THE SEOR IMMEDIATELY OF ANY DISCREPANCIES.
c. EXTERIOR CURTAIN WALL/WINDOW SYSTEMS SHALL BE INSPECTED AS TO STRUCTURAL FASTENING REQUIREMENTS ONLY.

2. **OTHER MISCELLANEOUS INSPECTION**

A. THE SPECIAL INSPECTOR SHALL:

a. INSPECT FOR HOLES DRILLED IN SLABS, WALLS, COLUMNS OR ANY OTHER STRUCTURAL COMPONENT WITHOUT PRIOR APPROVAL.
b. CHECK FOR EXPOSED REINFORCING STEEL BARS AT THE FACE OF REINFORCED CONCRETE MEMBERS.
c. MAINTAIN A CORRECTION LIST INDICATING THE AREAS WHERE DISCREPANCIES WERE NOTED AND THE STATUS OF REPAIRS.
d. PROVIDE A COPY OF THE DAILY INSPECTION REPORT TO THE ARCHITECT/ENGINEER, OWNER, GENERAL CONTRACTOR AND OTHERS AS REQUESTED BY THE OWNER.
e. VISUALLY INSPECT CONCRETE SURFACES FOR CRACKS AND NOTIFY THE SEOR OF STRUCTURAL CRACKS OTHER THAN SHRINKAGE CRACKS.

3. **SUBMISSIONS**

A. GENERAL: VARIOUS SUBMISSIONS TO THE SPECIAL INSPECTOR WILL BE REQUIRED AS LISTED BELOW. SUBMISSIONS SHALL BE REVIEWED BY THE CONTRACTOR AND THE ARCHITECT/ENGINEER OF RECORD PRIOR TO TRANSMITTAL TO THE SPECIAL INSPECTOR. SUBMIT ONE COPY FOR THE SPECIAL INSPECTOR'S USE AND RECORD. NOT ALL ITEMS LISTED BELOW WILL BE APPLICABLE FOR ALL PROJECTS. RECEIPT OF SUBMISSIONS BY THE SPECIAL INSPECTOR DOES NOT RELIEVE THE CONTRACTOR OR SUBCONTRACTORS OF THEIR OBLIGATIONS FOR PERFORMANCE.

B. CONCRETE MIX DESIGN: CONCRETE MIX DESIGNS FOR PROPOSED STRENGTHS AND QUALITIES SHALL BE FURNISHED TO THE SPECIAL INSPECTOR. THESE DESIGNS SHALL BE PREPARED IN ACCORDANCE WITH ACI RECOMMENDATIONS AND REQUIREMENTS OF THE CONTRACT DOCUMENTS AND SUPPORTED BY ACTUAL TEST DATA.

C. REINFORCING STEEL: SHOP DRAWINGS AND BAR LISTS WILL BE FURNISHED TO THE SPECIAL INSPECTOR FOR REINFORCING STEEL TO BE LISED IN STRUCTURAL COMPONENTS. THESE DRAWINGS SHALL BE PREPARED IN ACCORDANCE WITH ACI AND CRSI RECOMMENDATIONS AND SHALL CLEARLY SHOW BAR SIZES, DIMENSIONS, BENDS AND GRADES. MILL REPORTS FOR REINFORCING STEEL SHALL BE INCORPORATED INTO THE WORK AND SHALL BE FURNISHED TO THE SPECIAL INSPECTOR.

D. COLD FORMED STEEL SHOP DRAWINGS (IF COLD FORM STEEL MEMBERS ARE UTILIZED FOR EXTERIOR STRUCTURAL MEMBERS): SIGNED AND SEALED SHOP DRAWINGS FROM DELEGATED SPECIALTY ENGINEER SHALL BE FURNISHED TO SPECIAL INSPECTOR.

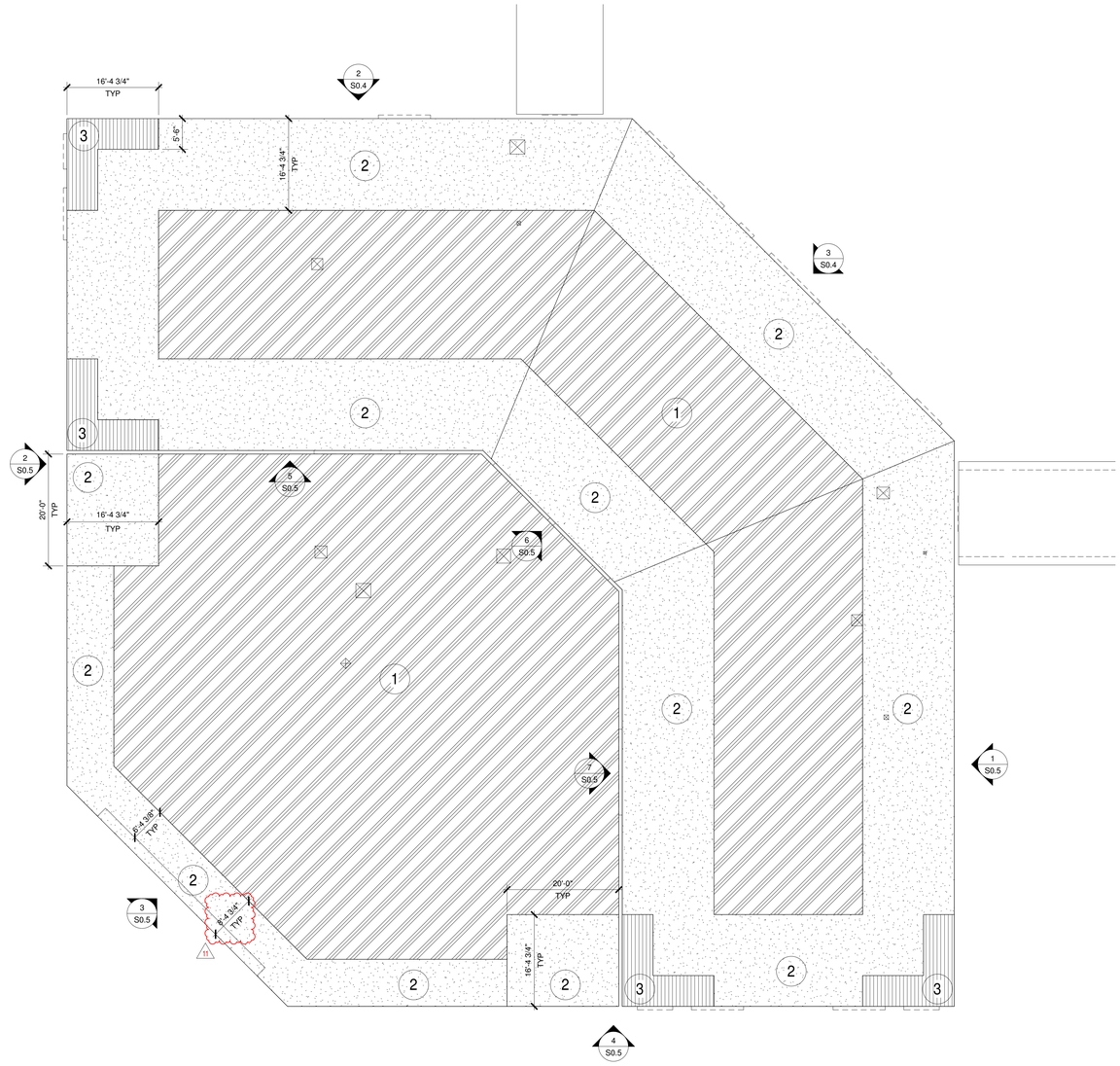
E. MISCELLANEOUS METALS: SHOP DRAWINGS FOR STRUCTURAL METALS, SUCH AS EMBEDDED ANCHORS, CONNECTION PLATES, HANGERS, ETC., SHALL BE FURNISHED TO THE SPECIAL INSPECTOR PRIOR TO INSTALLATION. THIS WILL INCLUDE STRUCTURAL ITEMS ONLY AS INTERPRETED FROM THE STRUCTURAL DOCUMENTS.

F. STRUCTURAL STEEL: SHOP AND ERECTION DRAWINGS FOR STRUCTURAL STEEL COMPONENTS SHALL BE FURNISHED TO THE SPECIAL INSPECTOR SHOWING MEMBER SIZES, CONFIGURATIONS AND DIMENSIONS, CONNECTION DETAILS, AND MATERIAL GRADES. MILL REPORTS FOR STEEL TO BE INCORPORATED INTO THE WORK WILL BE FURNISHED.

G. MASONRY UNITS: THE SPECIAL INSPECTOR SHALL BE PROVIDED WITH SPECIFICATIONS FOR MASONRY UNITS TO BE USED THAT CLEARLY SHOW APPLICABLE DATA INCLUDING UNIT GRADES.

H. MORTAR DESIGN MIXES: THE SPECIAL INSPECTOR SHALL BE SUPPLIED WITH DESIGN MIXES FOR SPECIFIED TYPES OF MORTAR TO BE USED IN THE PROJECT. THESE DESIGN MIXES ARE TO BE SUPPORTED WITH PERTINENT TEST DATA.

I. MANUFACTURED WOOD: SHOP DRAWINGS FOR PRE-ENGINEERED TRUSSES AND STRUCTURAL WOOD COMPONENTS SHALL BE FURNISHED TO THE SPECIAL INSPECTOR SHOWING ALL MEMBER SIZES, CONFIGURATIONS, DIMENSIONS, CONNECTIONS AND BRACING AS REQUIRED



1 S0.4 COMPONENTS AND CLADDING DIAGRAM- ADMINISTRATION - BUILDING 1
3/32" = 1'-0"

COMPONENT & CLADDING NOMINAL WIND PRESSURES (ASD) - ADMINISTRATION
(PER ASCE 7-22)

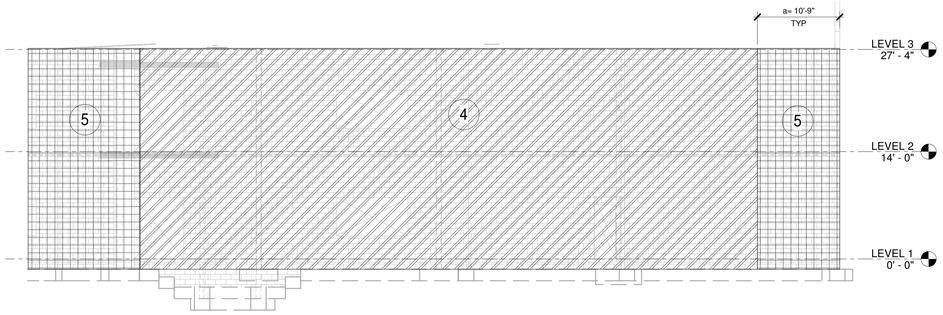
ULTIMATE WIND SPEED, Vult	166 mph	RISK CATEGORY:	III	EDGE DISTANCE, a (ft)	10.9
NOMINAL WIND SPEED, Vasd	129 mph	EXPOSURE CATEGORY:	C	ROOF SLOPE (DEGREES)	0.25/12
		DIRECTIONALITY FACTOR, Kd =	0.85		

COMPONENT LOCATION		POSITIVE PRESSURES (psf)						NEGATIVE PRESSURES (psf)					
EFFECTIVE AREA, Ae (ft²)		10	20	50	100	200	500	10	20	50	100	200	500
ROOFS	ZONE 1	16.6	15.6	14.2	13.2	13.2	13.2	-65.1	-60.8	-55.1	-50.8	-46.5	-40.8
	ZONE 1'	16.6	15.6	14.2	13.2	13.2	13.2	-37.4	-37.4	-37.4	-37.4	-32.2	-25.3
	ZONE 2	16.6	15.6	14.2	13.2	13.2	13.2	-85.8	-80.3	-73.0	-67.5	-62.0	-54.7
	ZONE 3	16.6	15.6	14.2	13.2	13.2	13.2	-117.0	-105.9	-91.4	-80.3	-69.3	-54.7
	OVERHANG ZONE 1	16.6	15.6	14.2	13.2	13.2	13.2	-58.8	-57.8	-56.4	-55.4	-46.4	-34.6
	OVERHANG ZONE 1'	16.6	15.6	14.2	13.2	13.2	13.2	-58.8	-57.8	-56.4	-55.4	-46.4	-34.6
	OVERHANG ZONE 2	16.6	15.6	14.2	13.2	13.2	13.2	-79.6	-72.2	-62.5	-55.2	-47.8	-38.1
	OVERHANG ZONE 3	16.6	15.6	14.2	13.2	13.2	13.2	-110.8	-97.9	-80.9	-68.0	-55.1	-38.1
WALLS	ZONE 4	37.4	35.7	33.5	31.9	30.2	28.0	-40.5	-38.8	-36.6	-35.0	-33.3	-31.1
	ZONE 5	37.4	35.7	33.5	31.9	30.2	28.0	-49.8	-46.5	-42.1	-38.8	-35.5	-31.1
		CASE A						CASE B					
PARAPETS	INTERIOR ZONE	113.1	105.8	96.1	88.8	81.5	71.8	-66.8	-63.4	-59.0	-55.6	-52.2	-47.7
	CORNER ZONE	145.0	132.0	114.8	101.9	88.9	71.8	-76.4	-71.3	-64.6	-59.5	-54.4	-47.7

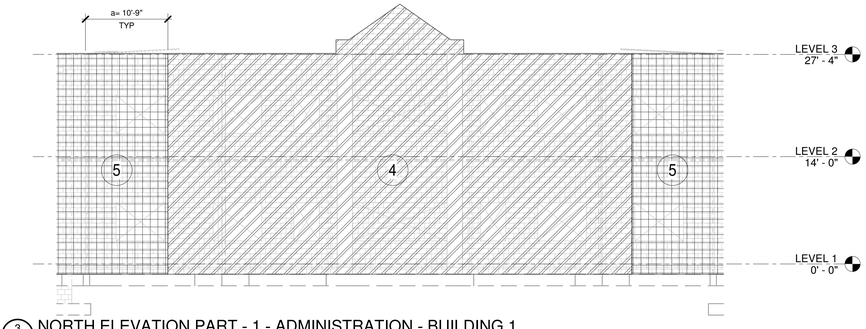
NOTES:

1. "Ae" INDICATES EFFECTIVE AREA AS DEFINED BY SECTION 26.2 OF ASCE 7.
2. PRESSURE VALUES IN ABOVE TABLE ARE BASED ON THE PARAMETERS LISTED AT THE TOP OF THE TABLE.
3. PRESSURES VALUES IN THE ABOVE TABLE ARE FOR ENCLOSED BUILDING CONDITION WITH INTERNAL PRESSURE COEFFICIENT OF +/-0.18
4. GLAZED OPENINGS SHALL BE PROTECTED IN ACCORDANCE WITH SECTION 26.10.3 OF ASCE 7.
5. POSITIVE OR NEGATIVE DESIGN PRESSURES SHALL NOT BE TAKEN LESS THAN 16PSF (ULTIMATE VALUE) OR 10PSF (NOMINAL VALUE).
6. POSITIVE WIND PRESSURES ACT TOWARD THE SURFACE, AND NEGATIVE PRESSURES ACT AWAY FROM THE SURFACE.
7. PRESSURES IN TABLE ARE ALLOWABLE PRESSURES. NO FURTHER REDUCTIONS PERMITTED.

LEGEND	
PATTERN	ZONE
	1
	1'
	2
	3

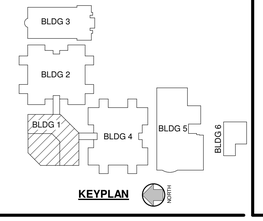


2 S0.4 EAST ELEVATION - ADMINISTRATION - BUILDING 1
1/8" = 1'-0"



3 S0.4 NORTH ELEVATION PART - 1 - ADMINISTRATION - BUILDING 1
1/8" = 1'-0"

NOTE:
THE PLANS AND ELEVATIONS SHOWN HERE ARE FOR THE PURPOSE OF SHOWING WIND PRESSURE ZONES ONLY. FOR STRUCUTURAL MEMBERS SEE APPROPRIATE PLANS, SCHEDULES AND OTHER DETAILS.



KEYPLAN

CODE REVIEW:

CERTIFICATE:

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONADO AR0012456
STEVEN G. SIEGEL AR99546
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CONSULTANTS:

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Certificate of Authorization No. 35826

TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

Δ	REVISION NAME	DATE
11	ASI #11	09.29.2025

FOR BID:

DRAWING TITLE:

COMPONENTS AND CLADDING DIAGRAM - ADMINISTRATION - BUILDING 1

COMMISSION NUMBER:
24FTL018

DO NOT SCALE THE DRAWINGS

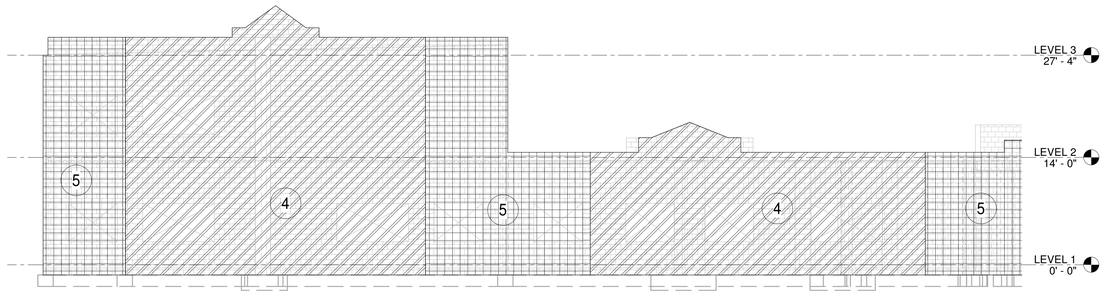
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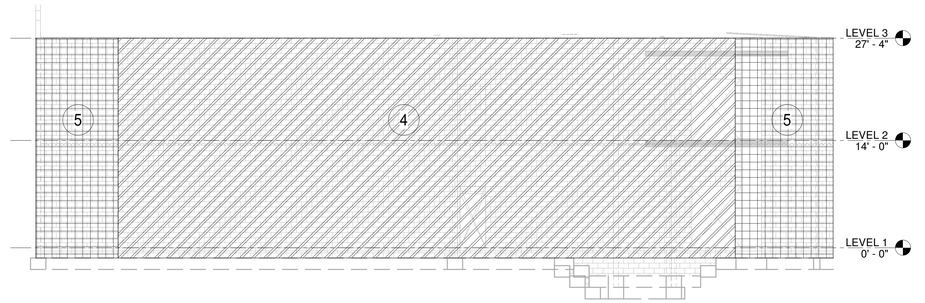
THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"

3"=1'-0" 1 1/2"=1'-0" 1"=1'-0" 3/4"=1'-0" 1/2"=1'-0" 1/4"=1'-0" 1/8"=1'-0" 1/16"=1'-0"

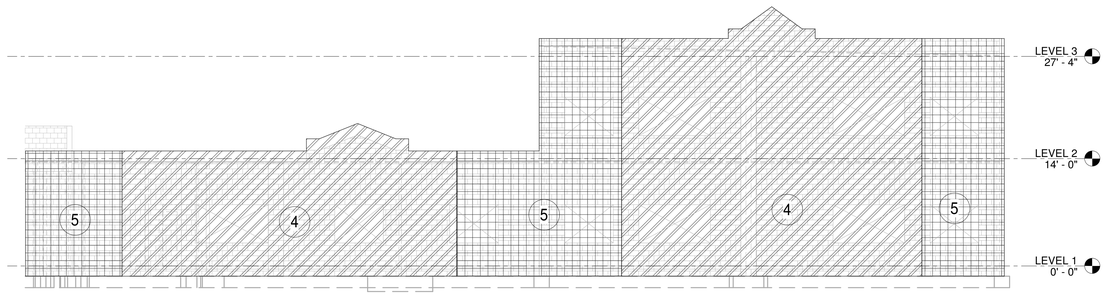
SPIEZLE ARCHITECTURAL GROUP, INC. COPYRIGHT © 2024
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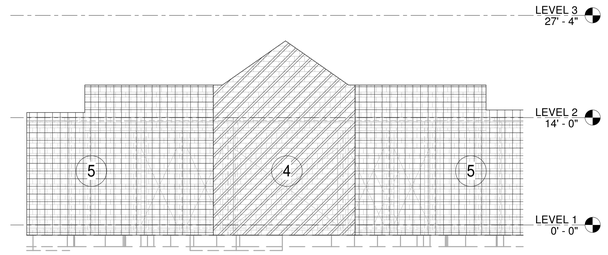
2 S0.5 SOUTH ELEVATION - ADMINISTRATION - BUILDING 1
1/8" = 1'-0"



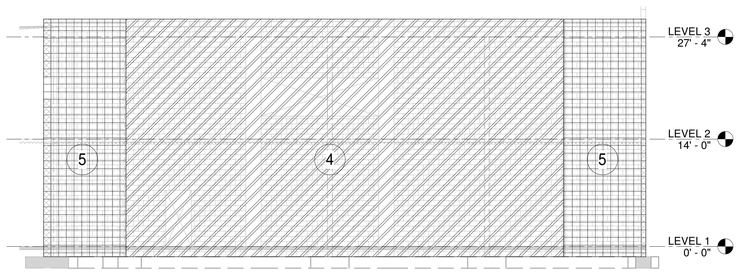
1 S0.5 NORTH ELEVATION - ADMINISTRATION - BUILDING 1
1/8" = 1'-0"



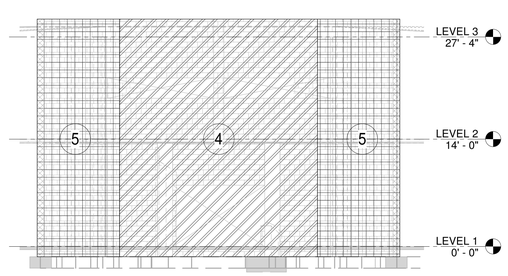
4 S0.5 WEST ELEVATION - ADMINISTRATION - BUILDING 1
1/8" = 1'-0"



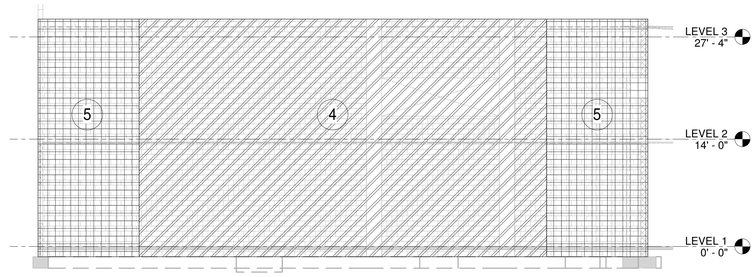
3 S0.5 SOUTH ELEVATION PART - 1 - ADMINISTRATION - BUILDING 1
1/8" = 1'-0"



7 S0.5 SOUTH ELEVATION PART - 1 - ADMINISTRATION - BUILDING 1
1/8" = 1'-0"

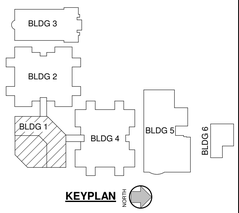


6 S0.5 SOUTH ELEVATION PART - 2 - ADMINISTRATION - BUILDING 1
1/8" = 1'-0"



5 S0.5 WEST ELEVATION PART - 1 - ADMINISTRATION - BUILDING 1
1/8" = 1'-0"

NOTE:
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KEYPLAN

CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONADO AR0012456
STEVEN G. SIEGEL AR99546
JOHN F. WRIGHT AR107341

SEAL:

CN=Nitin R Ranadive, dnQualifier=A01410D00000018DC34549AA00048BD39, O=Florida, C=US
Nitin Ranadive, P.E., No.81459
TRC Worldwide Engineering, dba Jenkins and Charland, INC.
2025.02.12 10:49:09-05'00'

NITIN RANADIVE LICENSE No. 81459 STATE OF FLORIDA PROFESSIONAL ENGINEER

This item has been electronically signed and sealed by Nitin Ranadive, PE on the date adjacent to the seal.

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

CONSULTANTS:

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TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
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FOR

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9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

	REVISION NAME	DATE
1		

FOR BID:

DRAWING TITLE:

COMPONENTS AND CLADDING DIAGRAM - ADMINISTRATION - BUILDING 1

COMMISSION NUMBER:
24FTL018

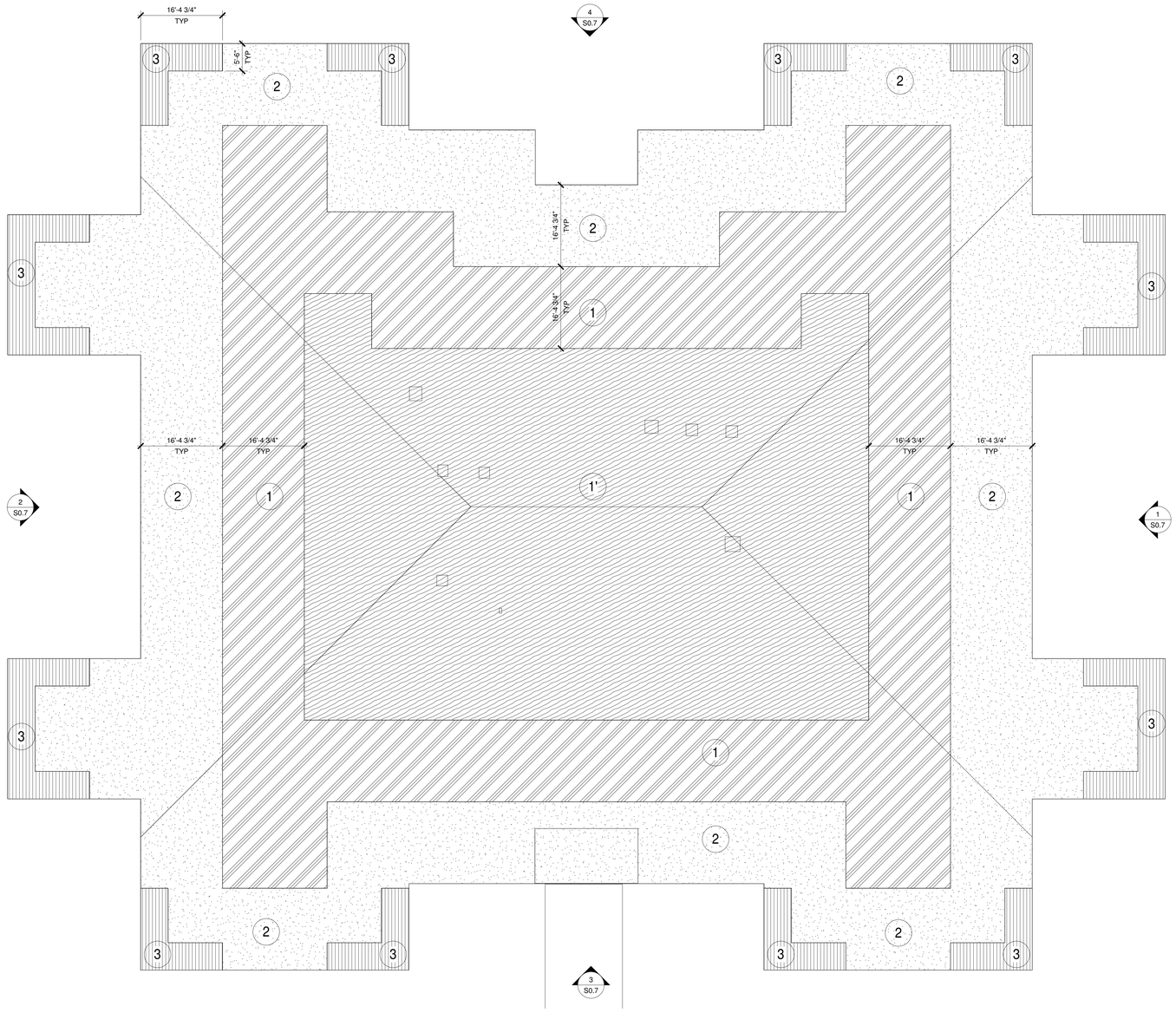
DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S0.5

THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"

3/4"=1'-0" 1 1/2"=1'-0" 1"=1'-0" 3/4"=1'-0" 1/2"=1'-0" 1/4"=1'-0" 1/8"=1'-0" 1/16"=1'-0"



1 S0.6 COMPONENTS AND CLADDING DIAGRAM - CLASSROOMS 1 & 2 - BUILDING 2 & 4
3/32" = 1'-0"

COMPONENT & CLADDING NOMINAL WIND PRESSURES (ASD) - CLASSROOM (PER ASCE 7-22)

ULTIMATE WIND SPEED, Vult	166 mph	RISK CATEGORY:	III	EDGE DISTANCE, a (ft)	10.8
NOMINAL WIND SPEED, Vasd	128.6 mph	EXPOSURE CATEGORY:	C	ROOF SLOPE (DEGREES)	0.25/12
		DIRECTIONALITY FACTOR, Kd =	0.85		

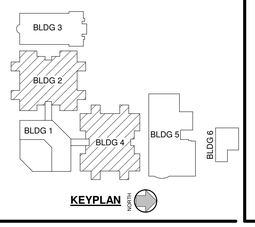
COMPONENT LOCATION		POSITIVE PRESSURES (psf)						NEGATIVE PRESSURES (psf)					
EFFECTIVE AREA, Ae (ft²)		10	20	50	100	200	500	10	20	50	100	200	500
ROOFS	ZONE 1	16.6	15.5	14.2	13.1	13.1	13.1	-65.0	-60.7	-55.0	-50.7	-46.4	-40.8
	ZONE 1'	16.6	15.5	14.2	13.1	13.1	13.1	-37.3	-37.3	-37.3	-37.3	-32.1	-25.2
	ZONE 2	16.6	15.5	14.2	13.1	13.1	13.1	-85.7	-80.2	-72.9	-67.4	-61.9	-54.6
	ZONE 3	16.6	15.5	14.2	13.1	13.1	13.1	-116.8	-105.8	-91.2	-80.2	-69.2	-54.6
	OVERHANG ZONE 1	16.6	15.5	14.2	13.1	13.1	13.1	-58.7	-57.7	-56.3	-55.3	-46.4	-34.6
	OVERHANG ZONE 1'	16.6	15.5	14.2	13.1	13.1	13.1	-58.7	-57.7	-56.3	-55.3	-46.4	-34.6
	OVERHANG ZONE 2	16.6	15.5	14.2	13.1	13.1	13.1	-79.6	-72.1	-62.4	-55.1	-47.7	-38.0
	OVERHANG ZONE 3	16.6	15.5	14.2	13.1	13.1	13.1	-110.8	-97.7	-80.7	-67.9	-55.0	-38.0
WALLS	ZONE 4	37.3	35.7	33.5	31.8	30.2	28.0	-40.4	-38.8	-36.6	-34.9	-33.3	-31.1
	ZONE 5	37.3	35.7	33.5	31.8	30.2	28.0	-49.8	-46.5	-42.1	-38.8	-35.5	-31.1
		CASE A						CASE B					
PARAPETS	INTERIOR ZONE	113.0	105.6	96.0	88.6	81.3	71.7	-66.7	-63.3	-58.9	-55.5	-52.1	-47.7
	CORNER ZONE	144.7	131.8	114.7	101.7	88.8	71.7	-76.2	-71.2	-64.5	-59.4	-54.3	-47.7

NOTES:

1. "Ae" INDICATES EFFECTIVE AREA AS DEFINED BY SECTION 26.2 OF ASCE 7.
2. PRESSURE VALUES IN ABOVE TABLE ARE BASED ON THE PARAMETERS LISTED AT THE TOP OF THE TABLE.
3. PRESSURES VALUES IN THE ABOVE TABLE ARE FOR ENCLOSED BUILDING CONDITION WITH INTERNAL PRESSURE COEFFICIENT OF +/-0.18
4. GLAZED OPENINGS SHALL BE PROTECTED IN ACCORDANCE WITH SECTION 26.10.3 OF ASCE 7.
5. POSITIVE OR NEGATIVE DESIGN PRESSURES SHALL NOT BE TAKEN LESS THAN 16PSF (ULTIMATE VALUE) OR 10PSF (NOMINAL VALUE).
6. POSITIVE WIND PRESSURES ACT TOWARD THE SURFACE, AND NEGATIVE PRESSURES ACT AWAY FROM THE SURFACE.
7. PRESSURES IN TABLE ARE ALLOWABLE PRESSURES. NO FURTHER REDUCTIONS PERMITTED.

LEGEND	
PATTERN	ZONE
	1
	1'
	2
	3

NOTE:
THE PLANS AND ELEVATIONS SHOWN HERE ARE FOR THE PURPOSE OF SHOWING WIND PRESSURE ZONES ONLY. FOR STRUCUTURAL MEMBERS SEE APPROPRIATE PLANS, SCHEDULES AND OTHER DETAILS.



KEYPLAN

CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONADO AR0012456
STEVEN G. SIEGEL AR99546
JOHN F. WRIGHT AR107361

SEAL:

CN=Nitin R Ranadive, dnQualifier=A01410D0000018DC34549AA0004BD39,O=Florida, C=US
Nitin Ranadive, P.E., No.81459
TRC Worldwide Engineering, dba Jenkins and Charland, INC.
2025.02.12 10:49:35-05'00'

NITIN RANADIVE LICENSE No. 81459 STATE OF FLORIDA PROFESSIONAL ENGINEER

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CONSULTANTS:

TRC
WORLDWIDE ENGINEERING
1230 N. University Dr.
PLANTATION, FL 33322
PHONE: (954) 484-7777
FAX: (954) 484-7834

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TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

	REVISION NAME	DATE
1		

FOR BID:

DRAWING TITLE:

COMPONENTS AND CLADDING DIAGRAM - CLASSROOMS 1 & 2 - BUILDING 2 & 4

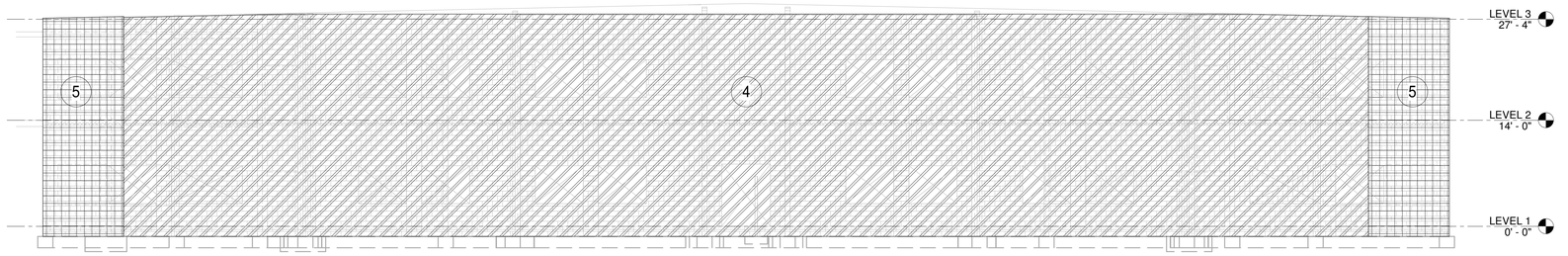
COMMISSION NUMBER:
24FTL018

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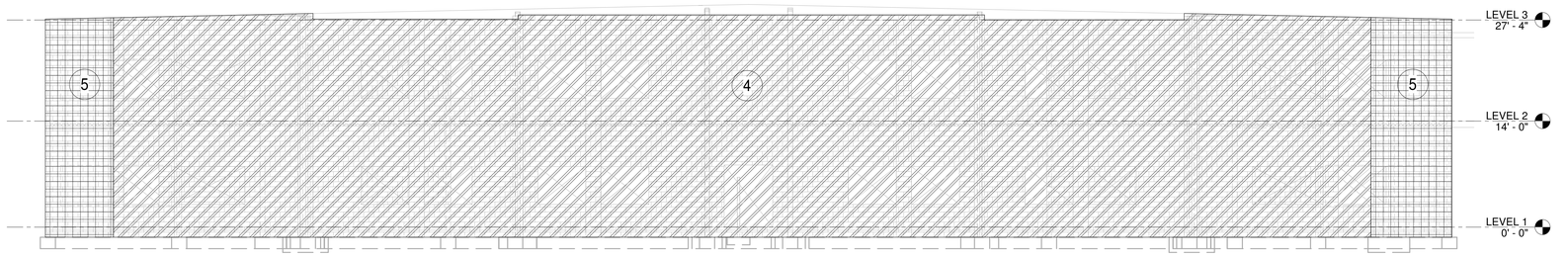
DRAWING NUMBER:
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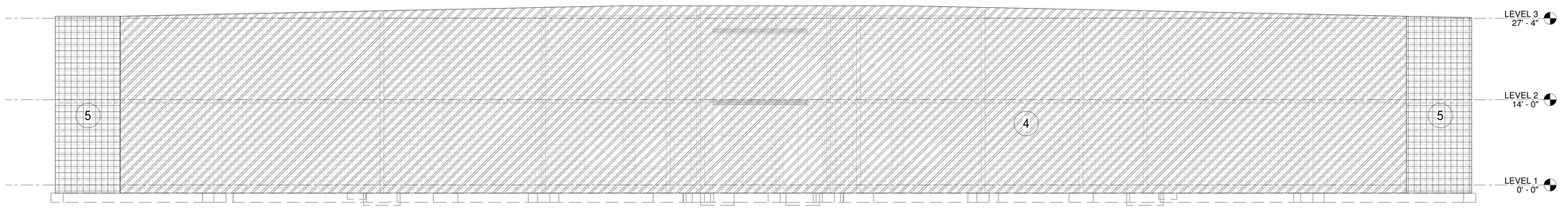
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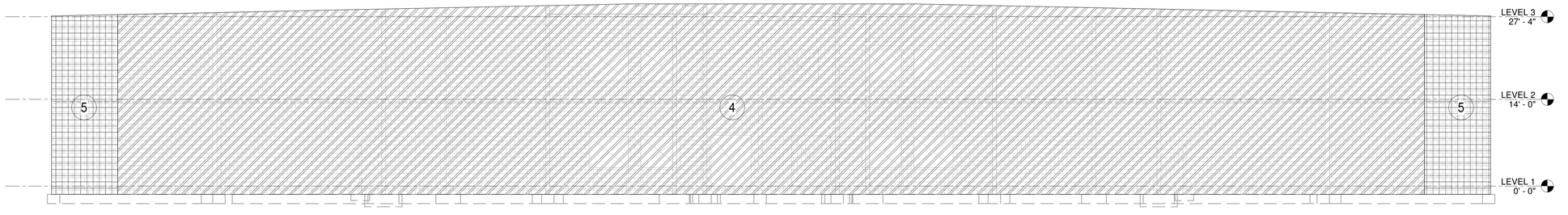
1 S0.7 NORTH ELEVATION - CLASSROOMS 1 - BUILDING 2
1/8" = 1'-0"



2 S0.7 SOUTH ELEVATION - CLASSROOMS 1 - BUILDING 2
1/8" = 1'-0"

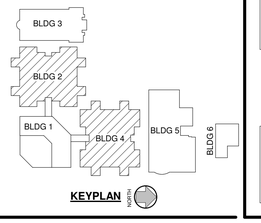


3 S0.7 EAST ELEVATION - CLASSROOMS 1 - BUILDING 2
1/8" = 1'-0"



4 S0.7 WEST ELEVATION - CLASSROOMS 1 - BUILDING 2
1/8" = 1'-0"

NOTE:
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KEYPLAN NOT TO SCALE

CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONADO AR0012456
STEVEN G. SIEGEL AR99546
JOHN F. WRIGHT AR107341

SEAL:

CN=Nitin R Ranadive, dnQualifier=A01410D000008 18DC34549AA00048D39 C=Florida, C=US Nitin Ranadive, P.E., No.81459 TRC Worldwide Engineering, dba Jenkins and Charland, INC. 2025.02.12 10:50:04-05'00'

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CONSULTANTS:

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WORLDWIDE ENGINEERING
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PLANTATION, FL 33322
PHONE: (954) 484-7777
FAX: (954) 484-7834

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TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

	REVISION NAME	DATE
4	ADDENDUM 4	01.27.2025

FOR BID:

DRAWING TITLE:

COMPONENTS AND CLADDING DIAGRAM - CLASSROOMS 1 & 2 - BUILDING 2 & 4

COMMISSION NUMBER:
24FTL018

DO NOT SCALE THE DRAWINGS

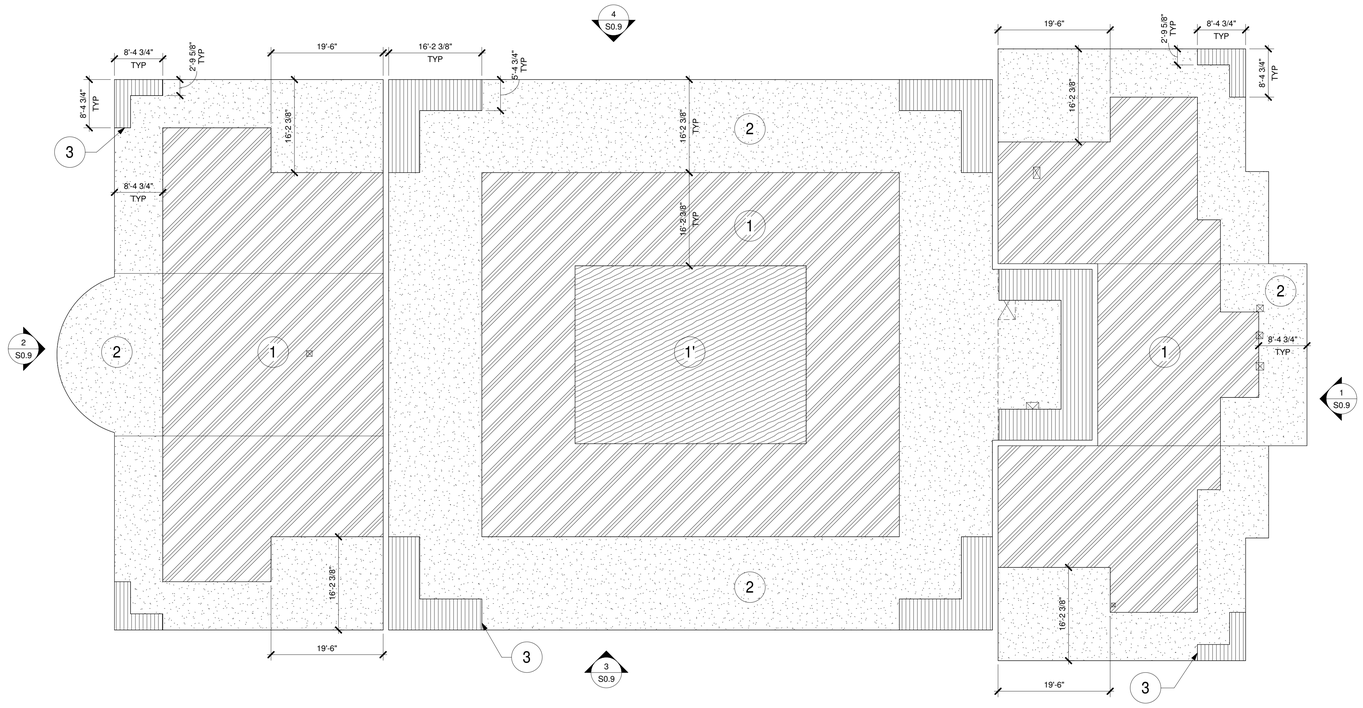
DRAWING NUMBER:

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THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"

3"=1'-0" 1 1/2"=1'-0" 1"=1'-0" 3/4"=1'-0" 1/2"=1'-0" 1/4"=1'-0" 1/8"=1'-0" 1/16"=1'-0"

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1 S0.8 COMPONENTS AND CLADDING DIAGRAM - GYM - BUILDING 3
3/32" = 1'-0"

LEGEND	
PATTERN	ZONE
	1
	1'
	2
	3

COMPONENT & CLADDING NOMINAL WIND PRESSURES (ASD) - GYM (PER ASCE 7-22)

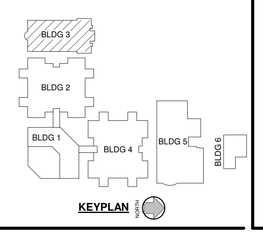
ULTIMATE WIND SPEED, Vult	166 mph	RISK CATEGORY:	III	EDGE DISTANCE, a (ft)	10.8
NOMINAL WIND SPEED, Vasd	129 mph	EXPOSURE CATEGORY:	C	ROOF SLOPE (DEGREES)	0.25/12
		DIRECTIONALITY FACTOR, Kd =	0.85		

COMPONENT LOCATION		POSITIVE PRESSURES (psf)						NEGATIVE PRESSURES (psf)					
EFFECTIVE AREA, Ae (ft²)		10	20	50	100	200	500	10	20	50	100	200	500
ROOFS	ZONE 1	16.6	15.5	14.2	13.1	13.1	13.1	-64.9	-60.6	-55.0	-50.7	-46.4	-40.7
	ZONE 1'	16.6	15.5	14.2	13.1	13.1	13.1	-37.3	-37.3	-37.3	-37.3	-32.1	-25.2
	ZONE 2	16.6	15.5	14.2	13.1	13.1	13.1	-85.6	-80.1	-72.8	-67.3	-61.8	-54.5
	ZONE 3	16.6	15.5	14.2	13.1	13.1	13.1	-116.7	-105.7	-91.1	-80.1	-69.1	-54.5
	OVERHANG ZONE 1	16.6	15.5	14.2	13.1	13.1	13.1	-58.7	-57.7	-56.3	-55.2	-46.3	-34.5
	OVERHANG ZONE 1'	16.6	15.5	14.2	13.1	13.1	13.1	-58.7	-57.7	-56.3	-55.2	-46.3	-34.5
	OVERHANG ZONE 2	16.6	15.5	14.2	13.1	13.1	13.1	-79.4	-72.2	-62.4	-55.0	-47.7	-38.0
	OVERHANG ZONE 3	16.6	15.5	14.2	13.1	13.1	13.1	-110.5	-97.6	-80.7	-67.8	-55.0	-38.0
WALLS	ZONE 4	37.3	35.7	33.5	31.8	30.1	28.0	-40.4	-38.8	-36.6	-34.9	-33.3	-31.1
	ZONE 5	37.3	35.7	33.5	31.8	30.1	28.0	-49.7	-46.5	-42.1	-38.7	-35.4	-31.1

NOTES:

1. "Ae" INDICATES EFFECTIVE AREA AS DEFINED BY SECTION 26.2 OF ASCE 7.
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7. PRESSURES IN TABLE ARE ALLOWABLE PRESSURES. NO FURTHER REDUCTIONS PERMITTED.

NOTE:
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KEYPLAN

CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONOIO AR0012456
STEVEN G. SIEGEL AR99546
JOHN F. WYNGHT AR107341

SEAL:

CN=Nitin R Ranadive, dnQualifier=A01410D000003918DC34549AA00048BD39, O=Florida, C=US
Nitin Ranadive, P.E., No.81459
TRC Worldwide Engineering, dba Jenkins and Charland, INC.
2025.02.12 10:50:27-05'00'

NITIN RANADIVE LICENSE No. 81459 STATE OF FLORIDA PROFESSIONAL ENGINEER

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CONSULTANTS:

TRC
WORLDWIDE ENGINEERING
1230 N. University Dr.
PLANTATION, FL 33322
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FAX: (954) 484-7834

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TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

	REVISION NAME	DATE

FOR BID:

DRAWING TITLE:

COMPONENTS AND CLADDING DIAGRAM - GYM - BUILDING 3

COMMISSION NUMBER:
24FTL018

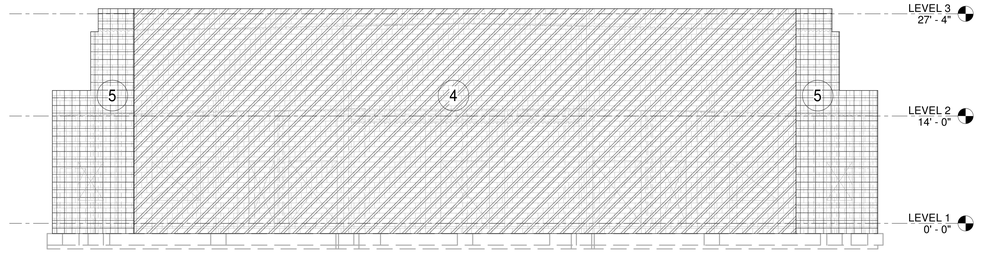
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DRAWING NUMBER:

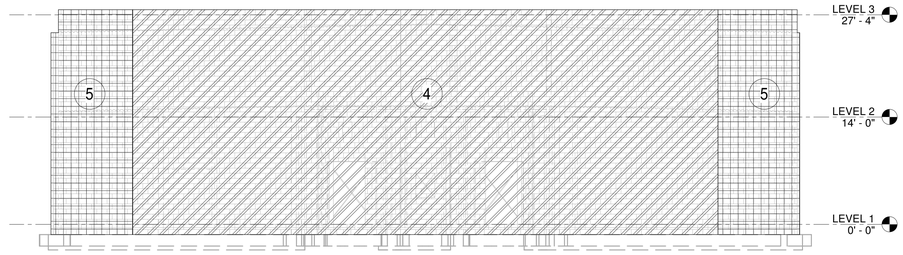
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THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"

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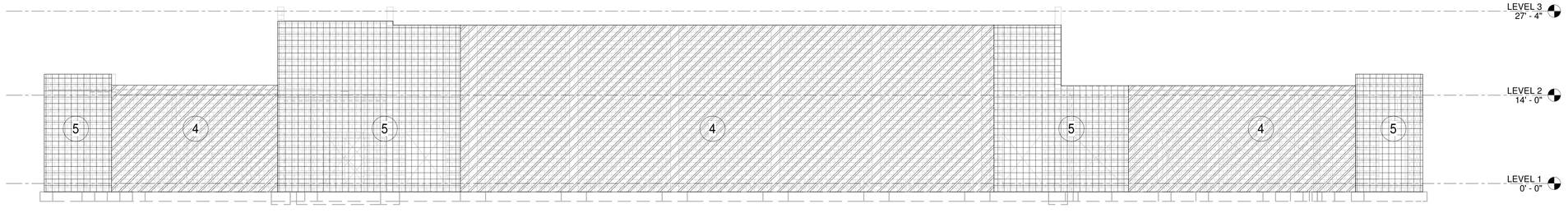


1 S0.9 NORTH ELEVATION - GYM - BUILDING 3
1/8" = 1'-0"

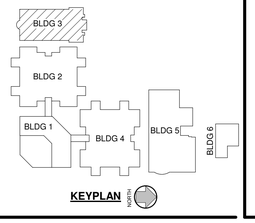


2 S0.9 SOUTH ELEVATION - GYM - BUILDING 3
1/8" = 1'-0"

3 S0.9 EAST ELEVATION - GYM - BUILDING 3
1/8" = 1'-0"



4 S0.9 WEST ELEVATION - GYM - BUILDING 3
1/8" = 1'-0"



KEYPLAN

NOTE:
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CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONADO AR0012456
STEVEN G. SIEGEL AR99546
JOHN F. WRIGHT AR107341

SEAL:

CN=Nitin R Ranadive, dnQualifier=A01410D00000 18DC34549AA0004BD39, O=Florida, C=US
Nitin Ranadive, P.E., No.81459
TRC Worldwide Engineering, dba Jenkins and Charland, INC.
2025.02.12 10:50:47-05'00'

NITIN RANADIVE
LICENSE
No. 81459
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

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CONSULTANTS:

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WORLDWIDE ENGINEERING

1230 N. University Dr.
PLANTATION, FL 33322
PHONE: (954) 484-7777
FAX: (954) 484-7834

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FILE No. 24FTL018

PERMIT SET 2-13-2025

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9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

Δ	REVISION NAME	DATE
4	ADDENDUM 4	01.27.2025

FOR BID:

DRAWING TITLE:

COMPONENTS AND CLADDING DIAGRAM - GYM - BUILDING 3

COMMISSION NUMBER:
24FTL018

DO NOT SCALE THE DRAWINGS

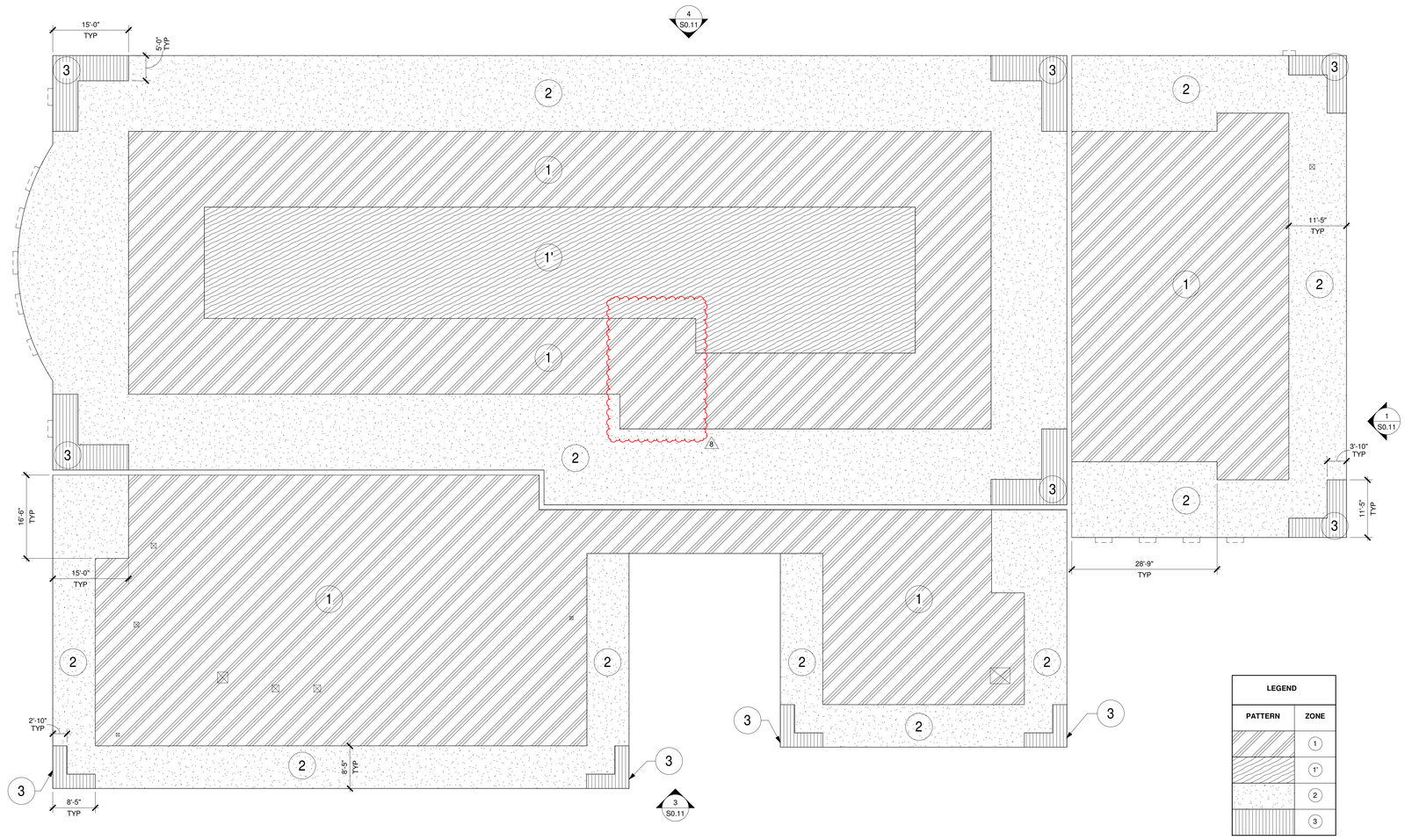
DRAWING NUMBER:

S0.9

THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"

3"=1'-0" 1 1/2"=1'-0" 1"=1'-0" 3/4"=1'-0" 1/2"=1'-0" 1/4"=1'-0" 1/8"=1'-0" 1/16"=1'-0"

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1 COMPONENTS AND CLADDING DIAGRAM - AUDETERIA - BUILDING 5

S0.10 3/32" = 1'-0"

COMPONENT & CLADDING NOMINAL WIND PRESSURES (ASD) - AUDETERIA (PER ASCE 7-22)

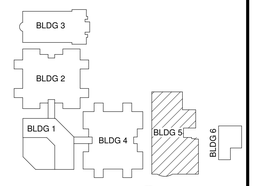
ULTIMATE WIND SPEED, Vult	184 mph	RISK CATEGORY:	IV	EDGE DISTANCE, a (ft)	10.0		
NOMINAL WIND SPEED, Vasd	142.5 mph	EXPOSURE CATEGORY:	C	ROOF SLOPE (DEGREES)	0.25/12		
		DIRECTIONALITY FACTOR, Kd =	1				

COMPONENT LOCATION		POSITIVE PRESSURES (psf)						NEGATIVE PRESSURES (psf)					
EFFECTIVE AREA, Ae (ft²)		10	20	50	100	200	500	10	20	50	100	200	500
ROOFS	ZONE 1	41.8	40.3	38.3	36.8	36.8	36.8	-110.5	-104.4	-96.4	-90.3	-84.2	-76.1
	ZONE 1'	41.8	40.3	38.3	36.8	36.8	36.8	-71.2	-71.2	-71.2	-71.2	-63.8	-54.1
	ZONE 2	41.8	40.3	38.3	36.8	36.8	36.8	-140.0	-132.2	-121.8	-114.0	-106.2	-95.8
	ZONE 3	41.8	40.3	38.3	36.8	36.8	36.8	-184.2	-168.6	-147.8	-132.2	-116.5	-95.8
	OVERHANG ZONE 1	41.8	40.3	38.3	36.8	36.8	36.8	-83.5	-82.0	-80.1	-78.6	-65.9	-49.1
	OVERHANG ZONE 1'	41.8	40.3	38.3	36.8	36.8	36.8	-83.5	-82.0	-80.1	-78.6	-65.9	-49.1
	OVERHANG ZONE 2	41.8	40.3	38.3	36.8	36.8	36.8	-113.0	-102.5	-88.7	-78.3	-67.8	-54.0
	OVERHANG ZONE 3	41.8	40.3	38.3	36.8	36.8	36.8	-157.2	-138.9	-114.8	-96.5	-78.2	-54.0
WALLS	ZONE 4	71.2	68.9	65.8	63.4	61.1	58.0	-75.7	-73.3	-70.2	-67.8	-65.5	-62.4
	ZONE 5	71.2	68.9	65.8	63.4	61.1	58.0	-88.9	-84.2	-78.0	-73.3	-68.6	-62.4
		CASE A						CASE B					
PARAPETS	INTERIOR ZONE	160.9	105.5	136.7	126.3	115.8	102.1	-95.0	-90.2	-83.8	-79.0	-74.2	-67.9
	CORNER ZONE	206.1	187.7	163.3	144.9	126.4	102.1	-108.6	-101.4	-91.8	-84.6	-77.4	-67.9

NOTES:

1. "Ae" INDICATES EFFECTIVE AREA AS DEFINED BY SECTION 26.2 OF ASCE 7.
2. PRESSURE VALUES IN ABOVE TABLE ARE BASED ON THE PARAMETERS LISTED AT THE TOP OF THE TABLE.
3. PRESSURES VALUES IN THE ABOVE TABLE ARE FOR PARTIALLY ENCLOSED BUILDING CONDITION WITH INTERNAL PRESSURE COEFFICIENT OF +/-0.55
4. GLAZED OPENINGS SHALL BE PROTECTED IN ACCORDANCE WITH SECTION 26.10.3 OF ASCE 7.
5. POSITIVE OR NEGATIVE DESIGN PRESSURES SHALL NOT BE TAKEN LESS THAN 16PSF (ULTIMATE VALUE) OR 10PSF (NOMINAL VALUE).
6. POSITIVE WIND PRESSURES ACT TOWARD THE SURFACE, AND NEGATIVE PRESSURES ACT AWAY FROM THE SURFACE.
7. PRESSURES IN TABLE ARE ALLOWABLE PRESSURES. NO FURTHER REDUCTIONS PERMITTED.

NOTE:
THE PLANS AND ELEVATIONS SHOWN HERE ARE FOR THE PURPOSE OF SHOWING WIND PRESSURE ZONES ONLY. FOR STRUCUTURAL MEMBERS SEE APPROPRIATE PLANS, SCHEDULES AND OTHER DETAILS.



KEYPLAN

CODE REVIEW:

CERTIFICATE:

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:

ANTHONY DONADO AR0012456
STEVEN G. SIEGEL AR99546
JOHN F. WRIGHT AR107381

SEAL:

CONSULTANTS:

TRC WORLDWIDE ENGINEERING
1230 N. University Dr.
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PHONE: (954) 484-7777
FAX: (954) 484-7834

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Certificate of Authorization No. 35826

TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

ASI #8_06/19/2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

	REVISION NAME	DATE
8	ASI #8	06.19.2025

FOR BID:

DRAWING TITLE:

COMPONENTS AND CLADDING DIAGRAM - AUDETERIA - BUILDING 5

COMMISSION NUMBER:

24FTL018

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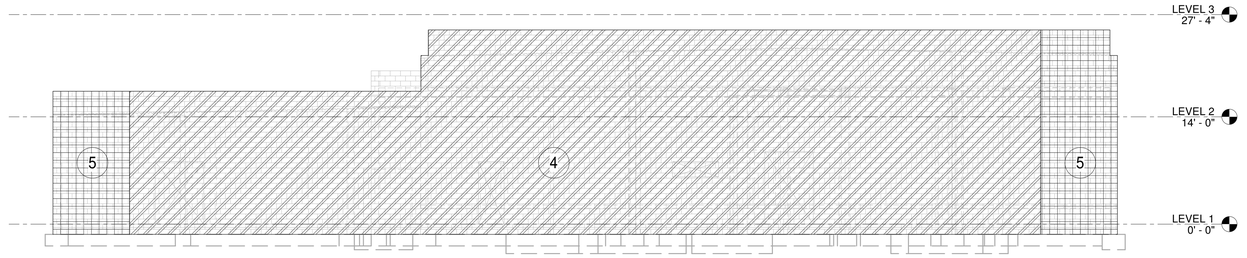
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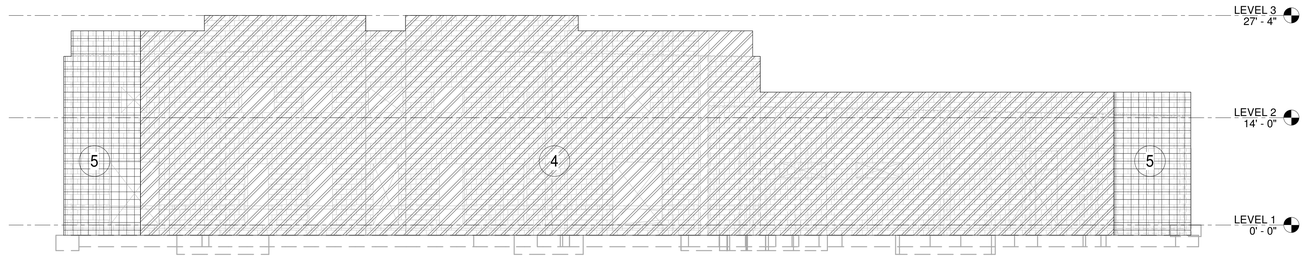
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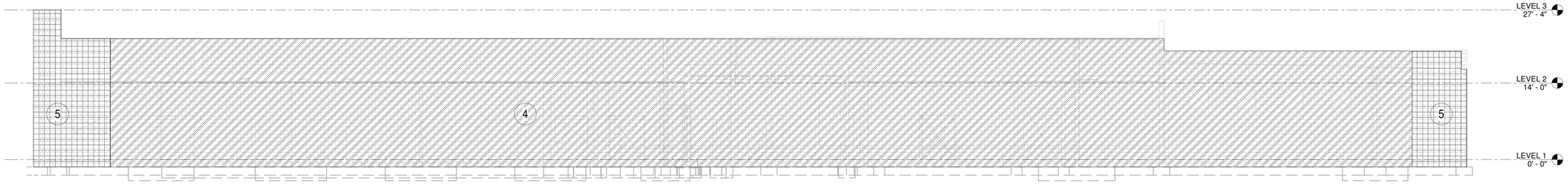
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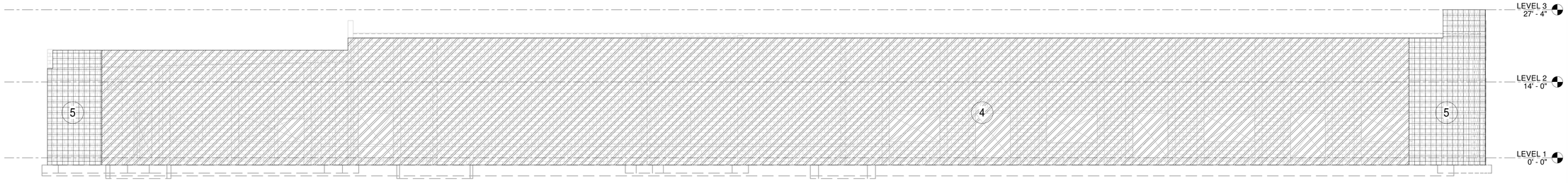
1 S0.11 NORTH ELEVATION- AUDETERIA - BUILDING 5
1/8" = 1'-0"



2 S0.11 SOUTH ELEVATION- AUDETERIA - BUILDING 5
1/8" = 1'-0"

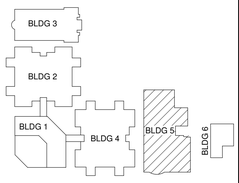


3 S0.11 EAST ELEVATION- AUDETERIA - BUILDING 5
1/8" = 1'-0"



4 S0.11 WEST ELEVATION- AUDETERIA - BUILDING 5
1/8" = 1'-0"

NOTE:
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KEYPLAN

CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONADO AR0012456
STEVEN G. SIEGEL AR99546
JOHN F. WRIGHT AR107341

SEAL:

CN=Nitin R Ranadive, dnQualifier=A01410D0000018DC34549AA00004BD39, O=Florida, C=US
Nitin Ranadive, P.E., No.81459
TRC Worldwide Engineering, dba Jenkins and Charland, INC.
2025.02.12 11:00:13-05'00'

NITIN RANADIVE LICENSE No. 81459 STATE OF FLORIDA PROFESSIONAL ENGINEER

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CONSULTANTS:

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WORLDWIDE ENGINEERING
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FAX: (954) 484-7834

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TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

	REVISION NAME	DATE

FOR BID:

DRAWING TITLE:

COMPONENTS AND CLADDING DIAGRAM - AUDETERIA - BUILDING 5

COMMISSION NUMBER:
24FTL018

DO NOT SCALE THE DRAWINGS

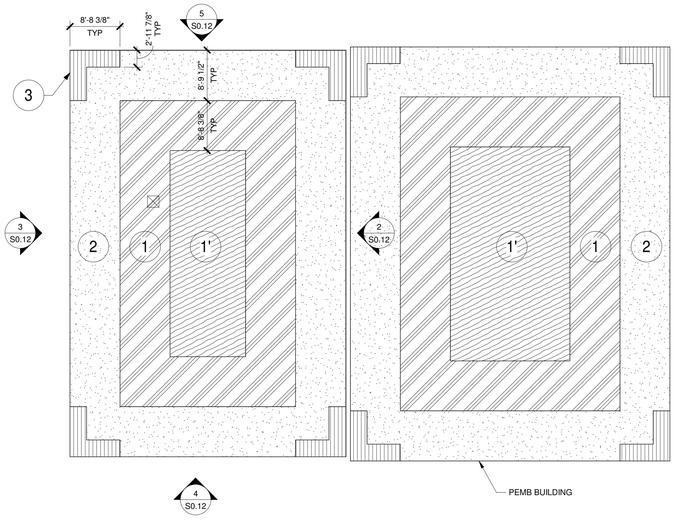
DRAWING NUMBER:

S0.11

THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"

3"=1'-0" 1 1/2"=1'-0" 1"=1'-0" 3/4"=1'-0" 1/2"=1'-0" 1/4"=1'-0" 1/8"=1'-0" 1/16"=1'-0"

LEGEND	
PATTERN	ZONE
	1
	1'
	2
	3



1 S0.12 COMPONENTS AND CLADDING DIAGRAM - CHILLER - BUILDING 6
3/32" = 1'-0"

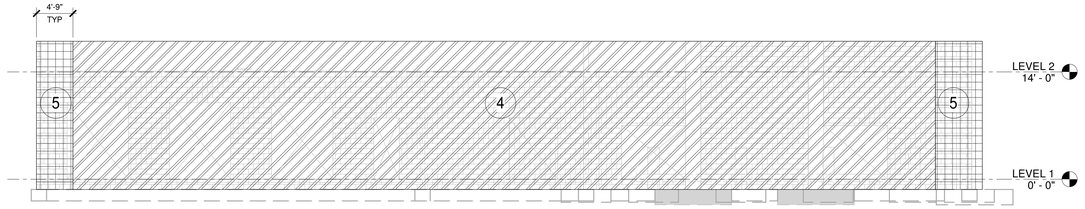
COMPONENT & CLADDING NOMINAL WIND PRESSURES (ASD) - CHILLER (PER ASCE 7-22)

ULTIMATE WIND SPEED, Vult	166 mph	RISK CATEGORY:	III	EDGE DISTANCE, a (ft)	4.9
NOMINAL WIND SPEED, Vasd	129 mph	EXPOSURE CATEGORY:	C	ROOF SLOPE (DEGREES)	0.25/12
		DIRECTIONALITY FACTOR, Kd =	0.85	MEAN ROOF HT, h (FT)	14.5

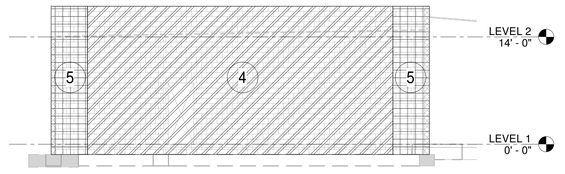
COMPONENT LOCATION		POSITIVE PRESSURES (psf)						NEGATIVE PRESSURES (psf)					
EFFECTIVE AREA, Ae (ft²)		10	20	50	100	200	500	10	20	50	100	200	500
ROOFS	ZONE 1	14.7	13.8	12.6	11.6	11.6	11.6	-57.6	-53.8	-48.8	-45.0	-41.2	-36.1
	ZONE 1'	14.7	13.8	12.6	11.6	11.6	11.6	-33.1	-33.1	-33.1	-33.1	-28.5	-22.4
	ZONE 2	14.7	13.8	12.6	11.6	11.6	11.6	-75.9	-71.1	-64.6	-59.7	-54.8	-48.4
	ZONE 3	14.7	13.8	12.6	11.6	11.6	11.6	-103.5	-93.7	-80.8	-71.1	-61.3	-48.4
	OVERHANG ZONE 1	14.7	13.8	12.6	11.6	11.6	11.6	-52.1	-51.1	-49.9	-49.0	-41.1	-30.6
	OVERHANG ZONE 1'	14.7	13.8	12.6	11.6	11.6	11.6	-52.1	-51.1	-49.9	-49.0	-41.1	-30.6
	OVERHANG ZONE 2	14.7	13.8	12.6	11.6	11.6	11.6	-70.4	-63.9	-55.3	-48.8	-42.3	-33.7
	OVERHANG ZONE 3	14.7	13.8	12.6	11.6	11.6	11.6	-98.0	-86.6	-71.5	-60.1	-48.7	-33.7
WALLS	ZONE 4	33.1	31.6	29.7	28.2	26.7	24.8	-35.8	-34.4	-32.4	-31.0	-29.5	-27.6
	ZONE 5	33.1	31.6	29.7	28.2	26.7	24.8	-44.1	-41.2	-37.3	-34.4	-31.4	-27.6
		CASE A						CASE B					
PARAPETS	INTERIOR ZONE	101.7	95.1	88.4	79.8	73.2	64.5	-60.1	-57.0	-53.0	-50.0	-46.9	-42.9
	CORNER ZONE	130.3	118.7	103.2	91.6	79.9	64.5	-68.7	-64.1	-58.1	-53.5	-48.9	-42.9

NOTES:

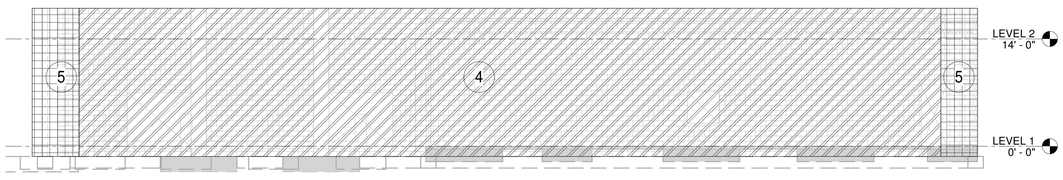
1. "Ae" INDICATES EFFECTIVE AREA AS DEFINED BY SECTION 26.2 OF ASCE 7.
2. PRESSURE VALUES IN ABOVE TABLE ARE BASED ON THE PARAMETERS LISTED AT THE TOP OF THE TABLE.
3. PRESSURES VALUES IN THE ABOVE TABLE ARE FOR ENCLOSED BUILDING CONDITION WITH INTERNAL PRESSURE COEFFICIENT OF +/-0.18
4. GLAZED OPENINGS SHALL BE PROTECTED IN ACCORDANCE WITH SECTION 26.10.3 OF ASCE 7.
5. POSITIVE OR NEGATIVE DESIGN PRESSURES SHALL NOT BE TAKEN LESS THAN 16PSF (ULTIMATE VALUE) OR 10PSF (NOMINAL VALUE).
6. POSITIVE WIND PRESSURES ACT TOWARD THE SURFACE, AND NEGATIVE PRESSURES ACT AWAY FROM THE SURFACE.
7. PRESSURES IN TABLE ARE ALLOWABLE PRESSURES. NO FURTHER REDUCTIONS PERMITTED.



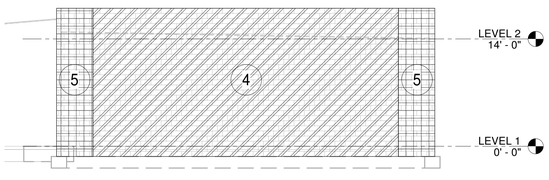
3 S0.12 SOUTH ELEVATION- CHILLER - BUILDING 6
1/8" = 1'-0"



4 S0.12 EAST ELEVATION- CHILLER - BUILDING 6
1/8" = 1'-0"

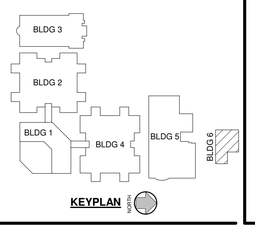


2 S0.12 NORTH ELEVATION- CHILLER - BUILDING 6
1/8" = 1'-0"



5 S0.12 WEST ELEVATION- CHILLER - BUILDING 6
1/8" = 1'-0"

NOTE:
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KEYPLAN

CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONADO AR0012456
STEVEN G. SIEGEL AR99546
JOHN F. WRIGHT AR101341

SEAL:

CN=Nitin R Ranadive, dnQualifier=A01410D000008 18DC34549AA0004BD39, O=Florida, C=US
Nitin Ranadive, P.E., No.81459
TRC Worldwide Engineering, dba Jenkins and Charland, INC.
2025.02.12 11:01:14-05'00'

NITIN RANADIVE LICENSE No. 81459 STATE OF FLORIDA PROFESSIONAL ENGINEER

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CONSULTANTS:

TRC
WORLDWIDE ENGINEERING
1230 N. University Dr.
PLANTATION, FL 33322
PHONE: (954) 484-7777
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TO THE BEST OF MY KNOWLEDGE AND ABILITY. THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

	REVISION NAME	DATE

FOR BID:

DRAWING TITLE:

COMPONENTS AND CLADDING DIAGRAM - CHILLER - BUILDING 6

COMMISSION NUMBER:
24FTL018

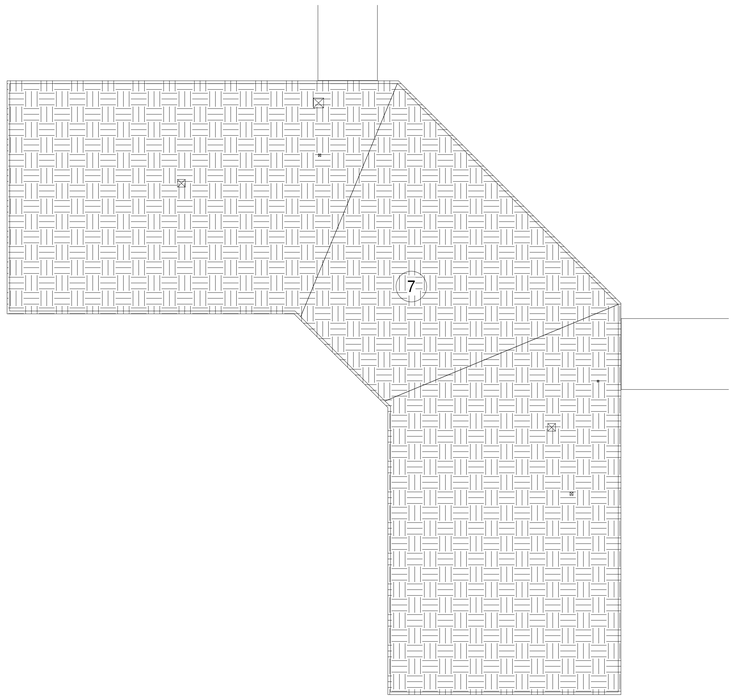
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DRAWING NUMBER:

S0.12

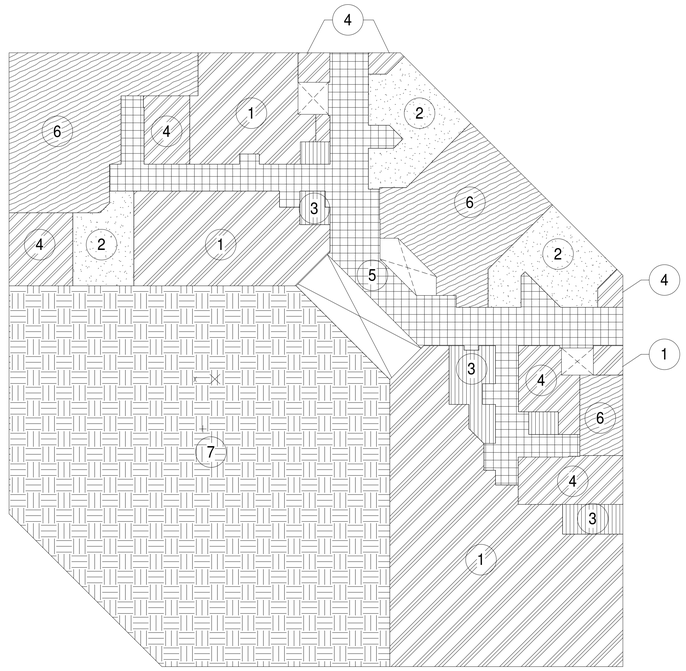
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2025-02-12 12:43:18
Autodesk Docs://24K054 - St Lucie WG K8/Western Grove K8 - SLPS_R24.rvt



2 S0.13 ROOF LOADING PLAN - ADMINISTRATION - BUILDING 1

1/16" = 1'-0"



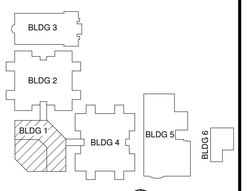
1 S0.13 SECOND FLOOR LOADING PLAN - ADMINISTRATION - BUILDING 1

1/16" = 1'-0"

LOADING PLAN LEGENDS					
PATTERN	ZONE	TYPE	LIVE LOAD	SUPER IMPOSED DEAD LOAD	REMARKS
	1	CLASSROOMS	40 PSF	30 PSF	
	2	OFFICE	50 PSF	30 PSF	
	3	RESTROOMS	60 PSF	30 PSF	
	4	STORAGE, MECHANICAL, ELECTRICAL ROOM	125 PSF	30 PSF	
	5	CORRIDOR	80 PSF	30 PSF	
	6	STAIRWAY, LABORATORIES	100 PSF	30 PSF	
	7	ROOF	30 PSF	20 PSF	
	8	ROOF	50 PSF	20 PSF	

NOTE: SUPERIMPOSED DEAD LOADS LISTED ABOVE INCLUDE 5 PSF ALLOWANCE FOR SPRINKLER PIPES WHICH IS A SUFFICIENT ALLOWANCE FOR SPRINKLER PIPES.

* INCLUDES A NON-REDUCIBLE 15 PSF PARTITION LOAD.



CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONOIO AR0012456
STEVEN G. SIEGEL AR99546
JOHN F. WRIGHT AR107341

SEAL:

CN=Nitin R Ranadive, dnQualifier=A01410D00000180C34549AA0004BD39, O=Florida, C=US
Nitin Ranadive, P.E., No.81459
TRC Worldwide Engineering, dba Jenkins and Charland, INC.
2025.02.12 11:01:45-05'00'

NITIN RANADIVE LICENSE No. 81459 STATE OF FLORIDA PROFESSIONAL ENGINEER

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PLANTATION, FL 33322
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FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

	REVISION NAME	DATE
1		

FOR BID:

DRAWING TITLE:

GRAVITY LOADING PLAN- ADMINISTRATION - BUILDING 1

COMMISSION NUMBER:
24FTL018

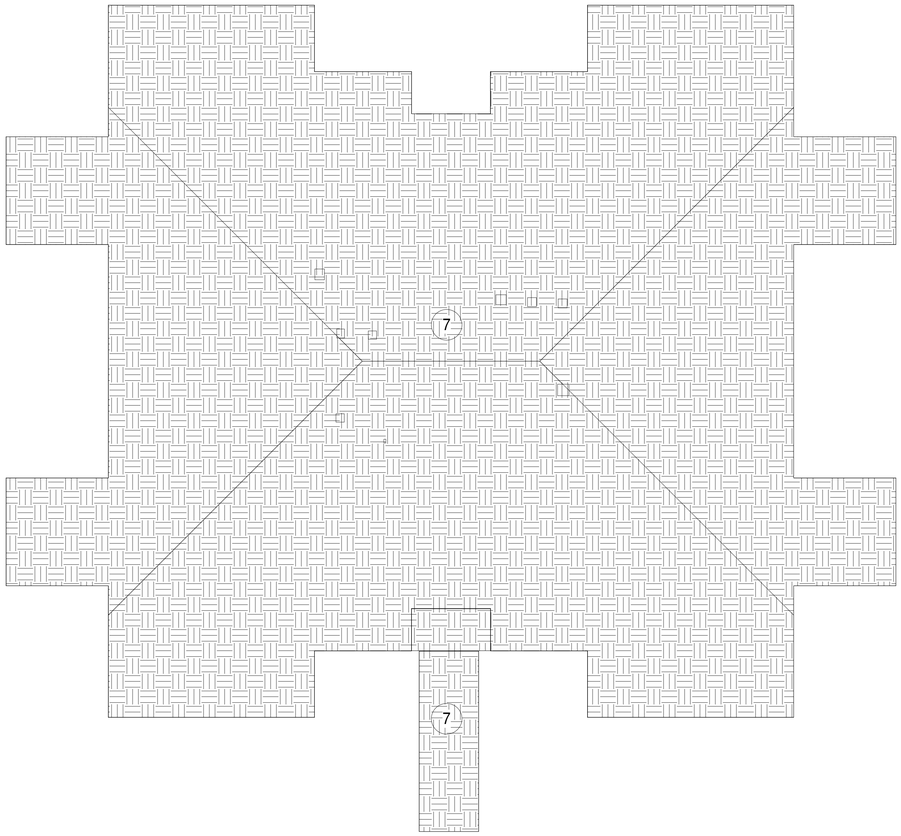
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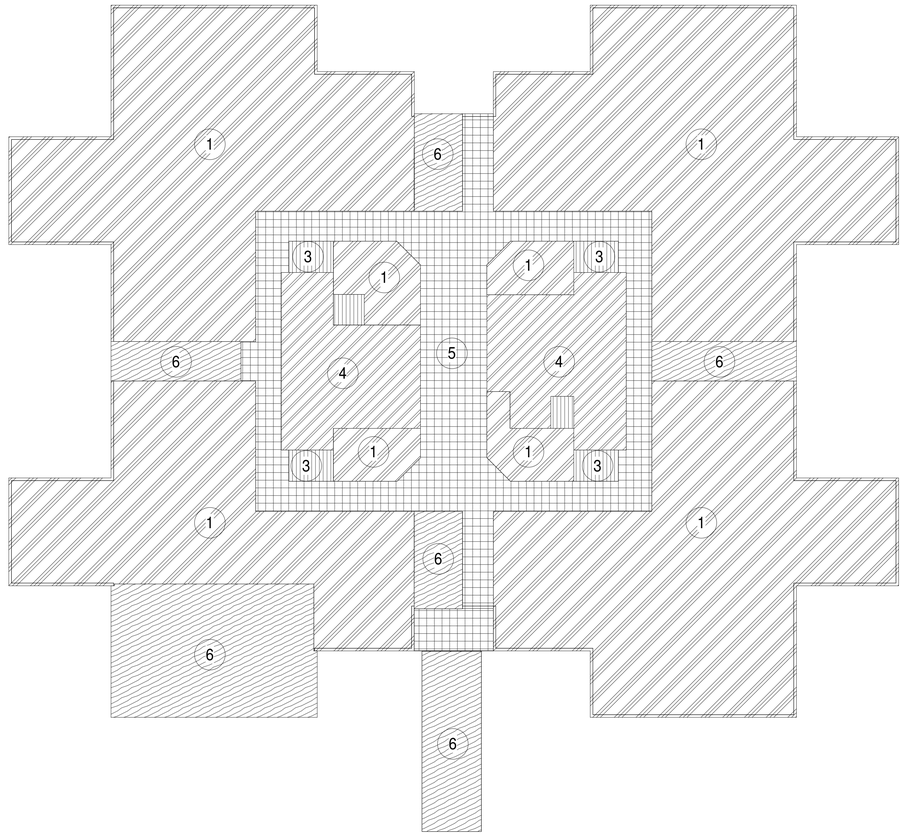
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2 S0.14 ROOF LOADING PLAN- CLASSROOMS 1 - BUILDING 2
1/16" = 1'-0"

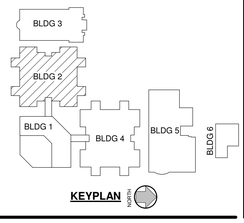


1 S0.14 SECOND FLOOR LOADING PLAN- CLASSROOMS 1 - BUILDING 2
1/16" = 1'-0"

LOADING PLAN LEGENDS					
PATTERN	ZONE	TYPE	LIVE LOAD	SUPER IMPOSED DEAD LOAD	REMARKS
	1	CLASSROOMS	40 PSF	30 PSF	
	2	OFFICE	50 PSF	30 PSF	
	3	RESTROOMS	60 PSF	30 PSF	
	4	STORAGE, MECHANICAL, ELECTRICAL ROOM	125 PSF	30 PSF	
	5	CORRIDOR	80 PSF	30 PSF	
	6	STAIRWAY, LABORATORIES	100 PSF	30 PSF	
	7	ROOF	30 PSF	20 PSF	
	8	ROOF	50 PSF	20 PSF	

NOTE: SUPERIMPOSED DEAD LOADS LISTED ABOVE INCLUDE 5 PSF ALLOWANCE FOR SPRINKLER PIPES WHICH IS A SUFFICIENT ALLOWANCE FOR SPRINKLER PIPES.

* INCLUDES A NON-REDUCIBLE 15 PSF PARTITION LOAD.



KEYPLAN

CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONOIO AR0012456
STEVEN G. SIEGEL AR99546
JOHN F. WRIGHT AR107341

SEAL:

CN=Nitin R Ranadive, dnQualifier=A01410D0000009318DC34549AA0004BD39E, O=Florida, C=US
Nitin Ranadive, P.E., No.81459
TRC Worldwide Engineering, dba Jenkins and Charland, INC.
2025.02.12 11:02:09-05'00'

NITIN RANADIVE LICENSE No. 81459 STATE OF FLORIDA PROFESSIONAL ENGINEER

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CONSULTANTS:

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FAX: (954) 484-7834

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FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

	REVISION NAME	DATE
1		

FOR BID:

DRAWING TITLE:

GRAVITY LOADING PLAN- CLASSROOMS 1 - BUILDING 2

COMMISSION NUMBER:
24FTL018

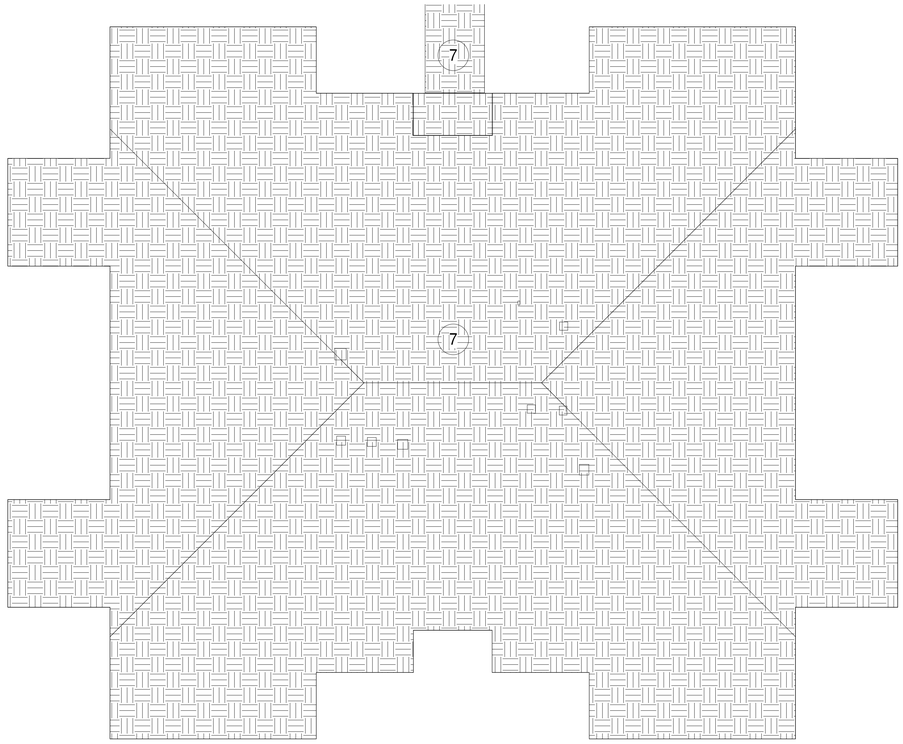
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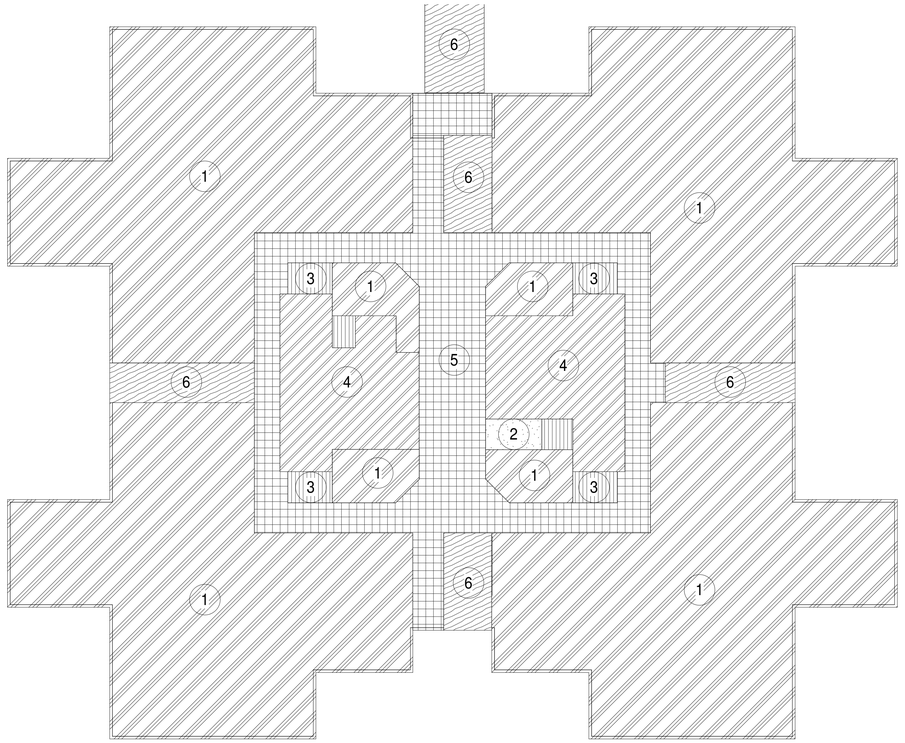
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3"=1'-0" 1 1/2"=1'-0" 1"=1'-0" 3/4"=1'-0" 1/2"=1'-0" 1/4"=1'-0" 1/8"=1'-0" 1/16"=1'-0"



2 S0.15 ROOF LOADING PLAN- CLASSROOMS 2 - BUILDING 4
1/16" = 1'-0"

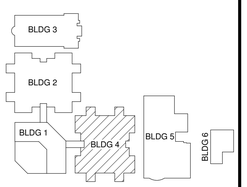


1 S0.15 SECOND FLOOR LOADING PLAN- CLASSROOMS 2 - BUILDING 4
1/16" = 1'-0"

LOADING PLAN LEGENDS					
PATTERN	ZONE	TYPE	LIVE LOAD	SUPER IMPOSED DEAD LOAD	REMARKS
	1	CLASSROOMS	40 PSF	30 PSF	
	2	OFFICE	50 PSF	30 PSF	
	3	RESTROOMS	60 PSF	30 PSF	
	4	STORAGE, MECHANICAL, ELECTRICAL ROOM	125 PSF	30 PSF	
	5	CORRIDOR	80 PSF	30 PSF	
	6	STAIRWAY, LABORATORIES	100 PSF	30 PSF	
	7	ROOF	30 PSF	20 PSF	
	8	ROOF	50 PSF	20 PSF	

NOTE: SUPERIMPOSED DEAD LOADS LISTED ABOVE INCLUDE 5 PSF ALLOWANCE FOR SPRINKLER PIPES WHICH IS A SUFFICIENT ALLOWANCE FOR SPRINKLER PIPES.

* INCLUDES A NON-REDUCIBLE 15 PSF PARTITION LOAD.



KEYPLAN

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spiezle

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SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
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STEVEN G. SIEGEL AR99546
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CN=Nitin R Ranadive, dnQualifier=A01410D000003 8DC34549AA0004BD39, O=Florida, C=US
Nitin Ranadive, P.E., No.81459
TRC Worldwide Engineering, dba Jenkins and Charland, INC.
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NITIN RANADIVE LICENSE No. 81459 STATE OF FLORIDA PROFESSIONAL ENGINEER

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FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

	REVISION NAME	DATE

FOR BID:

DRAWING TITLE:

GRAVITY LOADING PLAN- CLASSROOMS 2 - BUILDING 4

COMMISSION NUMBER:
24FTL018

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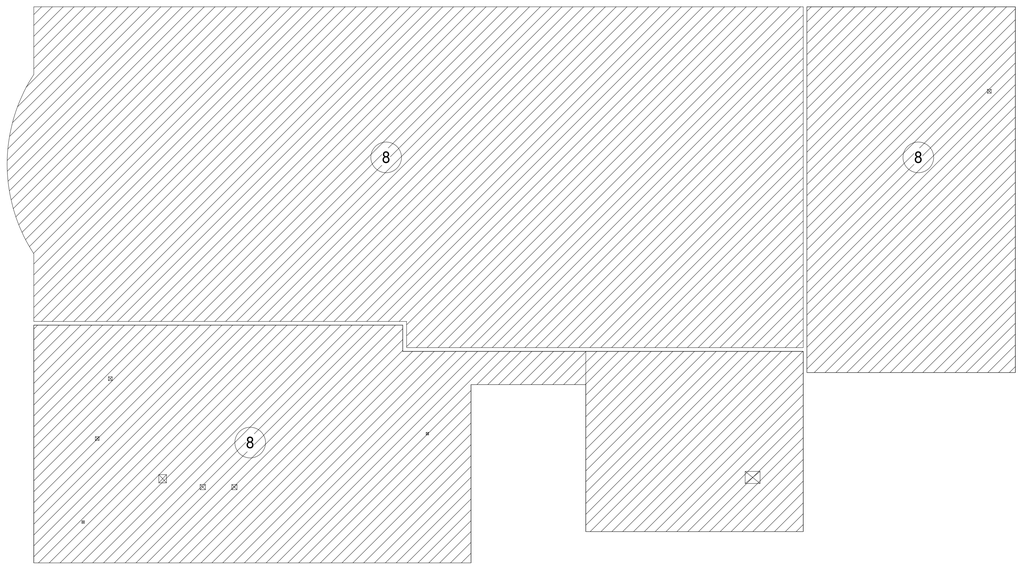
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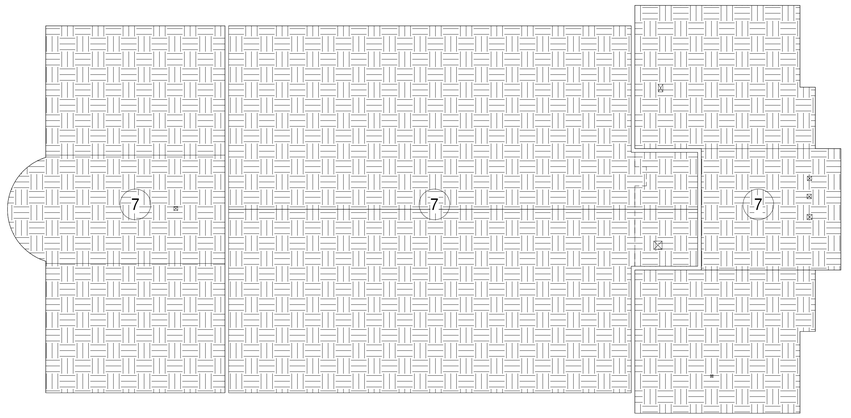
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1 1/2"=1'-0" 1 0 1 2
1"=1'-0" 1 0 1 2
3/4"=1'-0" 1 0 1 2
1/2"=1'-0" 0 1 2 4
1/4"=1'-0" 0 2 4 8
1/8"=1'-0" 0 4 8 16
1/16"=1'-0" 0 8 16 32

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1 S0.16 ROOF LOADING PLAN - AUDETERIA - BUILDING 5

1/16" = 1'-0"



2 S0.16 ROOF LOADING PLAN- GYM - BUILDING 3

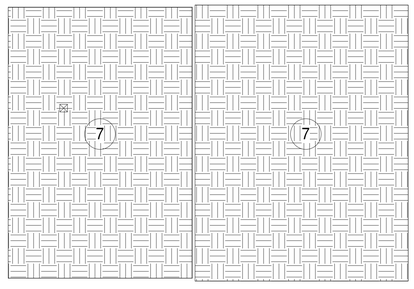
1/16" = 1'-0"

LOADING PLAN LEGENDS

PATTERN	ZONE	TYPE	LIVE LOAD	SUPER IMPOSED DEAD LOAD	REMARKS
	1	CLASSROOMS	40 PSF	30 PSF	
	2	OFFICE	50 PSF	30 PSF	
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	7	ROOF	30 PSF	20 PSF	
	8	ROOF	50 PSF	20 PSF	

NOTE: SUPERIMPOSED DEAD LOADS LISTED ABOVE INCLUDE 5 PSF ALLOWANCE FOR SPRINKLER PIPES WHICH IS A SUFFICIENT ALLOWANCE FOR SPRINKLER PIPES.

* INCLUDES A NON-REDUCIBLE 15 PSF PARTITION LOAD.



3 S0.16 ROOF LOADING PLAN - CHILLER - BUILDING 6

1/16" = 1'-0"

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PHONE: 772-794-2929

SIGNATURE:

ANTHONY DONOIO AR0012456
STEVEN G. SIEGEL AR99546
JOHN F. WRIGHT AR107341

SEAL:

CN=Nitin R Ranadive, dnQualifier=A01410D00000 18DC34549AA0004BD39, O=Florida, C=US
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TRC Worldwide Engineering, dba Jenkins and Charland, INC.
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FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

	REVISION NAME	DATE

FOR BID:

DRAWING TITLE:

GRAVITY LOADING PLAN- BUILDING 3,5 & 6

COMMISSION NUMBER:

24FTL018

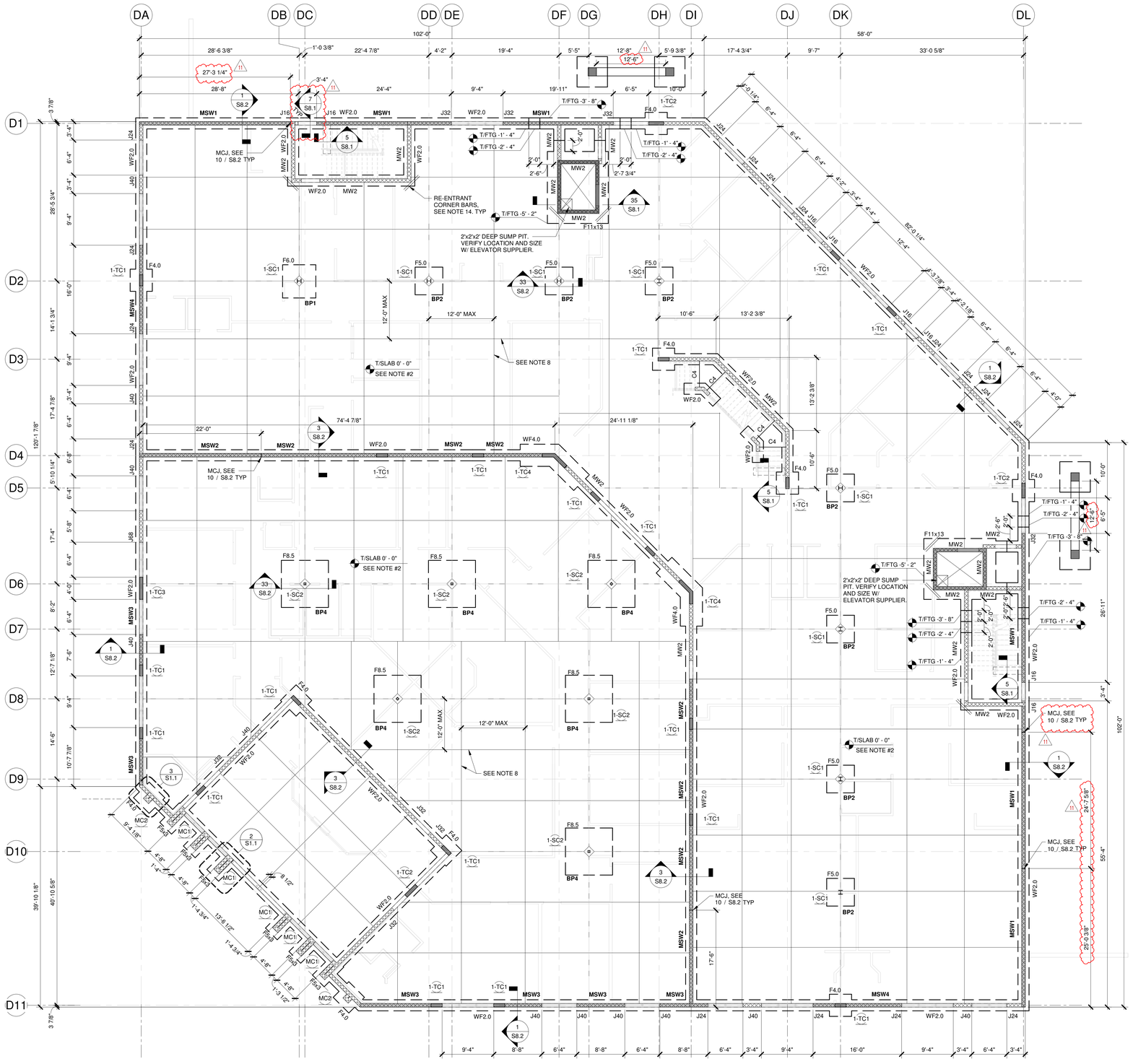
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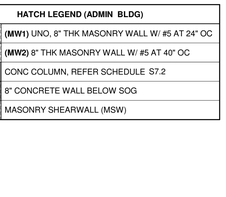
FOUNDATION & GROUND FLOOR PLAN NOTES (ADMIN BLDG):

1. T/FIRST FLOOR SLAB = REFERENCE ELEVATION 0'-0" U.N.O.
2. PROVIDE 5" THICK CONCRETE SLAB ON GRADE REINF W/ 6x6 W1.4xW1.4 WWR IN SHEETS (NOT ROLLS) CHAIRED TO TOP OF SLAB, SEE DETAIL 6 / S8.1 . PLACE CONCRETE ON VAPOR BARRIER PER ARCHL REQUIREMENTS BUT NOT LESS THAN 15 MILS. PREPARE SUBGRADE IN ACCORDANCE WITH GEOTECHNICAL REPORT.
3. SEE MEP DRAWINGS FOR SLAB-ON-GRADE PENETRATIONS.
4. TOP OF ALL FOOTINGS SHALL BE -1'-4" U.N.O.
5. CONTINUOUS WALL FOOTINGS SHALL EXTEND FULL LENGTH OF THE FOOTING WIDTH BEYOND END WALLS. U.N.O
6. STEP TOP OF CONTINUOUS WALL FOOTINGS AS NECESSARY FOR PIPES OR CONDUIT. SEE 19 / S8.1
7. FOR TYPICAL CMU WALL DETAILS SEE SCHEDULE ON THIS SHEET FOR DETAILS
8. PROVIDE CONTROL JOINTS PER SECTIONS 2 / S8.1 AND STRUCTURAL NOTES. CONSTRUCTION JOINTS MAY BE PLACED IN LIEU OF CJ AT CONTRACTOR'S DISCRETION, REFER AND SEE 3 / S8.1 .
9. FOR DETAILS OF FOOTINGS AT CORNERS AND INTERSECTIONS SEE 17 / S8.1 .
10. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION OR FABRICATION. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.
11. CENTERLINES OF COLUMNS AND BEAMS SHALL COINCIDE WITH THE FOUNDATION CENTERLINES, UNO.
12. CONCRETE SLAB-ON-GRADE SHALL BE FORMED ON COMPACTED SOILS. REFER TO GEOTECH REPORT FOR SUBGRADE PREPARATION.
13. FOR CONCRETE STRENGTHS REFER TO STRUCTURAL NOTES.
14. RE-ENTRANT CORNER BARS (TYP), SEE DETAIL 13 / S8.1
15. SEE SHEET S7.1 FOR STRUCTURAL SCHEDULES
16. SEE FOOTING SCHEDULE FOR ISOLATED FOOTINGS AND WALL FOOTING SIZE & REINFORCEMENT ON S7.1 SHEET.
17. SEE ARCH DRAWINGS FOR SLOPE AND FLOOR DRAINS IN SOG
18. REFER 9 / S8.3 FOR TYP. LINTEL SCHEDULE
19. SEE STRUCUTRAL NOTES AND GEOTECH REPORT FOR BEARING PRESSURE AND VIBRO REPLACEMENT STONE COLUMN FOUNDATION.

LEGEND:

TE-#	DENOTES THICKENED SLAB EDGE, SEE 33 / S8.1
TB#	DENOTES CONCRETE TIE BEAM TYPE, SEE SCHEDULE ON S7.1 SHEET
J#	MASONRY JAMB, REFER SCHEDULE ON S7.1
BP#	DENOTES BASE PLATE TYPE, SEE SCHEDULE ON 2 / S7.1 SHEET
MC#	DENOTES MASONRY COLUMN, SEE DETAIL ON THIS SHEET
MSW#	MASONRY SHEARWALL, REFER SCHEDULE ON S7.1
#-TC#	CONCRETE COLUMN REFER SCHEDULE ON S7.2
SC#	STEEL COLUMN REFER SCHEDULE ON S7.2
MC#	MASONRY COLUMN REFER DETAIL ON THIS SHEET
### plf	INDICATES ASD DIAPHRAGM SHEAR.

HATCH LEGEND (ADMIN BLDG)
(MW1) UNO, 8" THK MASONRY WALL W/ #5 AT 24" OC
(MW2) 8" THK MASONRY WALL W/ #5 AT 40" OC
CONC COLUMN, REFER SCHEDULE S7.2
8" CONCRETE WALL BELOW SOG
MASONRY SHEARWALL (MSW)



CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

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PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

	REVISION NAME	DATE
11	ASI #11	09.29.2025

FOR BID:

DRAWING TITLE:

FOUNDATION/FIRST FLOOR FRAMING PLAN- ADMINISTRATION - BUILDING 1

COMMISSION NUMBER:
24FTL018

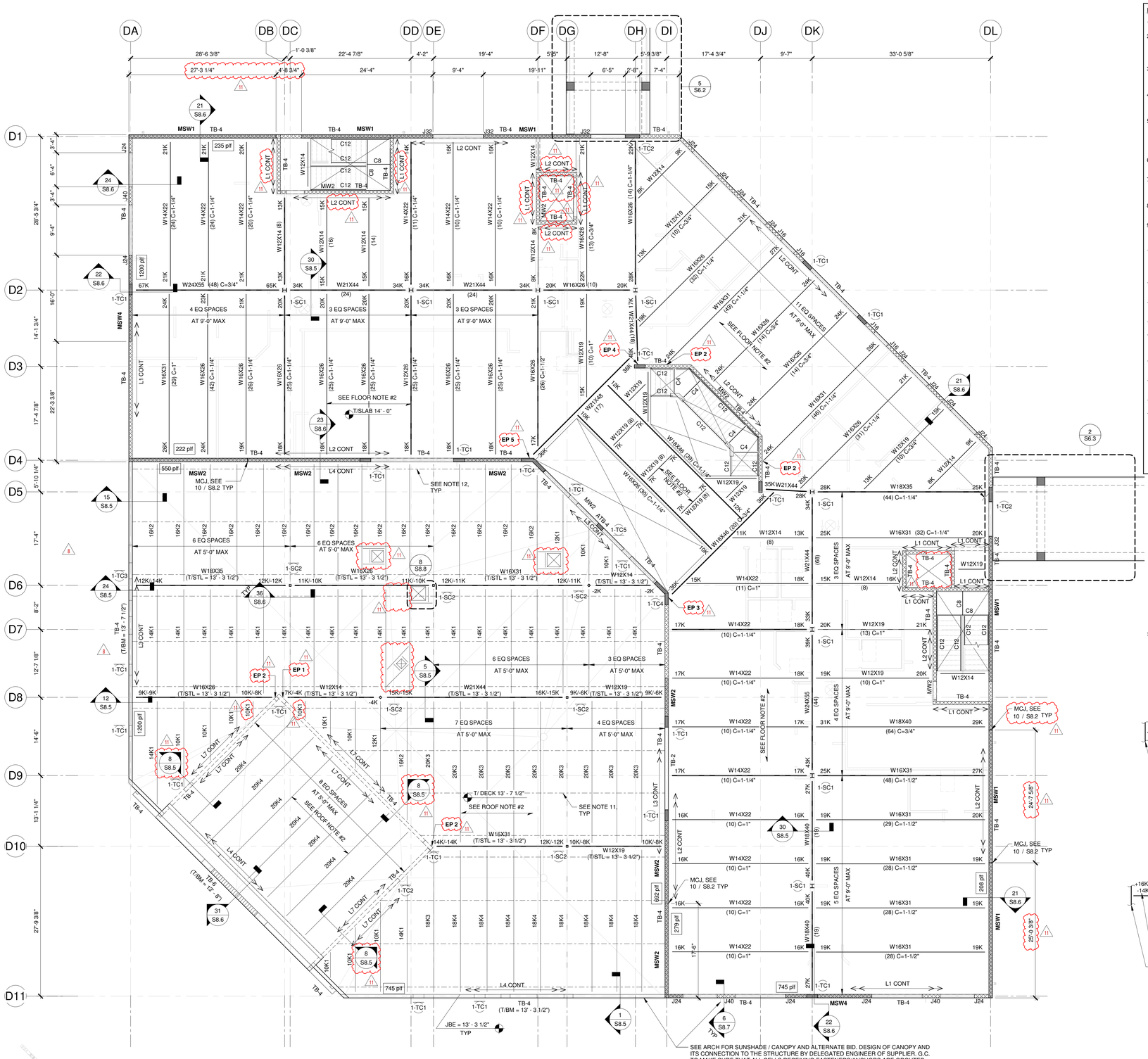
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1 S1.2 SECOND FLOOR FRAMING PLAN - ADMINISTRATION - BUILDING 1
1/8" = 1'-0"

NORTH

ROOF FRAMING NOTES (ADMIN BLDG):

1. TOP OF STEEL ELEVATION = SEE PLAN.
2. 1-1/2" (20 GAGE) G90 GALV TYPE B ROOF DECK. FASTEN ROOF DECK TO SUPPORTS PER ROOF DECK ATTACHMENT SCHEDULE ON TYPICAL ROOF DETAILS SHEET S8.6. ROOF DECK TO SPAN MINIMUM THREE SPANS
3. REFER TO STRUCTURAL NOTES AND PROJECT SPECIFICATIONS FOR DEFINITON OF SYMBOLS, ABBREVIATIONS, AND OTHER INFORMATION AND CRITERIA NOT SHOWN ON PLAN.
4. LOADING CRITERIA IS DEFINED ON DRAWINGS. NOTIFY ENGINEER OF SPECIAL ROOF/CEILING LOADING CONDITIONS NOT OTHERWISE IDENTIFIED ON THE CONTRACT DOCUMENTS.
5. PRIOR TO FABRICATION AND CONSTRUCTION, THE CONTRACTOR SHALL VERIFY DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT / ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION.
6. ◄ INDICATES SPAN DIRECTION OF STRUCTURAL DECK. SEE NOTES ON THIS SHEET FOR DECK SPECIFICATIONS.
7. TOP OF ROOF AND CEILING SLOPE ELEVATIONS AND LOCATIONS VARY AND SHALL BE COORDINATED WITH THE ARCHITECTURAL DRAWINGS PRIOR TO FABRICATION.
8. FRAMING SHALL BE EQUALLY SPACED BETWEEN COLUMN BAYS UNLESS NOTED OTHERWISE OR DIMENSIONED ON PLAN.
9. COORDINATE FINAL SIZE AND LOCATION OF ROOF OPENINGS, PENETRATIONS, AND CHASES WITH ARCHITECTURAL AND MEP/FP DRAWINGS.
10. DIMENSIONS TO ROOF TOP EQUIPMENT ARE APPROXIMATE AND SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO FABRICATION AND UPON FINAL SELECTION OF EQUIPMENT AND CURBING.
11. HORIZONTAL BRIDGING PER JOIST MFR MIN AS SHOWN ON PLAN, TYP. REFER 35 / S8.6
12. UPLIFT BRIDGING PER JOIST MFR MIN AS SHOWN ON PLAN, TYP. REFER 35 / S8.6 (SIM)
13. ► DENOTES DECK SPAN DIRECTION.
14. ◄► DENOTES MOMENT CONNECTION. DESIGN MOMENT CONNECTION FOR FULL MOMENT AND SHEAR CAPACITY OF BEAM.
15. BEAM END REACTIONS NOT INDICATED IN PLAN ARE ASD SERVICE LOADS.
16. FOR BEAM REACTIONS NOT INDICATED IN PLAN DESIGN THE CONNECTION FOR A MINIMUM REACTION OF 6K.
17. REFER 9 / S8.3 FOR TYP. LINTEL SCHEDULE
18. **L3** - INDICATES L5x3x3/8 (LLV) CONT W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC
    L4 - INDICATES L5x3x3/8 (LLV) CONT W/ VERTICAL LEG NOTCHED AS NEEDED W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU / CONC
    L7 - INDICATES L6x4x3/8 CONT W/ VERTICAL LEG NOTCHED AS NEEDED ANCHORED W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE AT 12" OC.

COMPOSITE FLOOR NOTES (ADMIN BLDG):

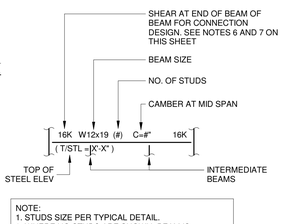
1. TOP OF SLAB ELEVATION = 14' - 0".
   TOP OF STEEL = 13' - 5-1/2".
2. UNO FLOOR DECK TO BE 4 1/2" NORMAL WEIGHT CONCRETE (FC = 3000 PSI) ON 2" (20 GA) COMPOSITE DECK, 2VLI BY VULCRAFT (6 1/2" TOTAL THICKNESS) W/ 6x6-W2.1xW2.1 WELDED WIRE REINFORCING (WWR) IN SHEETS NOT ROLLS, CHAIRED TO THE TOP OF SLAB. SHORE DECK IF LESS THAN (3) CONTINUOUS SPANS AND IN ACCORDANCE W/ MAX SDI UNSHORED CLEAR SPAN PUBLISHED BY MFR UNTIL CONCRETE HAS REACHED 75% COMPRESSIVE STRENGTH. FLOOR DECK TO SPAN MINIMUM THREE SPAN FOR UNSHORED CONDITION. SEE PLAN FOR EXTENTS
3. FLOOR LOADS:
   DEAD LOAD = SEE LOADING PLANS
   LIVE LOAD = SEE LOADING PLANS
4. ALL COMPOSITE FLOOR BEAMS SHALL HAVE A MINIMUM OF 3/4"ø X 5 1/2" HEADED STUDS AT 12" MAXIMUM (ONE STUD PER BEAM MIN UNO) SEE DETAIL
5. CONTRACTOR SHALL VERIFY ALL SLAB OPENING DIMENSIONS AND LOCATION WITH ARCH AND MECH CONTRACTOR PRIOR TO FABRICATION OF STEEL.
6. BEAM END REACTIONS INDICATED ON PLAN ARE ASD SERVICE LOADS.
7. FOR BEAM REACTIONS NOT INDICATED IN PLAN DESIGN THE CONNECTION FOR A MINIMUM REACTION OF 6K.
8. FOR DIMENSIONS NOT SHOWN SEE ARCHITECTURAL DRAWINGS.
9. **L1** - INDICATES L6x4x1/2 (LLV) CONT W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC
   L2 - INDICATES L5x3x3/8 (LLV) CONT W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU

LEGEND:

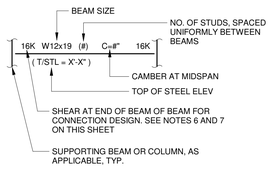
TE-#	DENOTES THICKENED SLAB EDGE, SEE 33 / S8.1
TB#	DENOTES CONCRETE TIE BEAM TYPE, SEE SCHEDULE ON S7.1 SHEET
J#	MASONRY JAMB, REFER SCHEDULE ON S7.1
BP#	DENOTES BASE PLATE TYPE, SEE SCHEDULE ON 2 / S7.1 SHEET
MC#	DENOTES MASONRY COLUMN, SEE DETAIL ON THIS SHEET
MSW#	MASONRY SHEARWALL, REFER SCHEDULE ON S7.1
#-TC#	CONCRETE COLUMN REFER SCHEDULE ON S7.2
SC#	STEEL COLUMN REFER SCHEDULE ON S7.2
MC#	MASONRY COLUMN REFER DETAIL ON THIS SHEET
### plf	INDICATES ASD DIAPHRAGM SHEAR.

HATCH LEGEND (ADMIN BLDG)

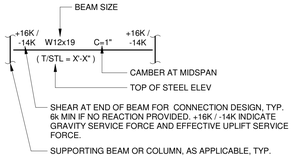
	(MW1) UNO, 8" THK MASONRY WALL W/ #5 AT 24" OC
	(MW2) 8" THK MASONRY WALL W/ #5 AT 40" OC
	CONC COLUMN, REFER SCHEDULE S7.2
	8" CONCRETE WALL BELOW SOG
	MASONRY SHEARWALL (MSW)



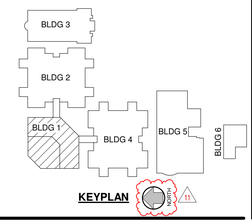
NOTE:
1. STUDS SIZE PER TYPICAL DETAIL.
2. WHERE NO STUDS ARE SHOWN, BEAM IS NON-COMPOSITE. SEE FOR STUD PLACEMENT.



COMPOSITE BEAM LEGEND



NON-COMPOSITE BEAM LEGEND



KEYPLAN

CODE REVIEW:

CERTIFICATE:

spiezle
SPIEZLE ARCHITECTURAL GROUP INC.
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Certificate of Authorization No. 35826

THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

Δ	REVISION NAME	DATE
4	ADDENDUM 4	01.27.2025
8	ASI #8	06.19.2025
11	ASI #11	09.29.2025

FOR BID:

DRAWING TITLE:

SECOND FLOOR FRAMING PLAN - ADMINISTRATION - BUILDING 1

COMMISSION NUMBER:
24FTL018

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:
S1.2

THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"

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Autodesk Docs://24K054 - St Lucie WG K8/Western Grove K8 - SLPS_R24.rvt



SEE ARCH FOR SUNSHADE / CANOPY AND ALTERNATE BID. DESIGN OF CANOPY AND ITS CONNECTION TO THE STRUCTURE BY DELEGATED ENGINEER OF SUPPLIER. G.C. TO MAKE SURE THAT ALL CELLS RECEIVING FASTENERS/ANCHORS ARE GROUTED SOLID AND HAVE ACHIEVED GROUT STRENGTH. TYP FOR ALL SUNSHADES AND CANOPIES.

1 S1.3 ROOF FRAMING PLAN - ADMINISTRATION - BUILDING 1

1/8" = 1'-0"

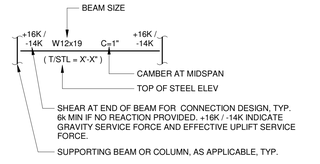
ROOF FRAMING NOTES (ADMIN BLDG):

1. TOP OF STEEL ELEVATION = SEE PLAN.
2. 1-1/2" (20 GAGE) G90 GALV TYPE B ROOF DECK. FASTEN ROOF DECK TO SUPPORTS PER ROOF DECK ATTACHMENT SCHEDULE ON TYPICAL ROOF DETAILS SHEET S8.8. ROOF DECK TO SPAN MINIMUM THREE SPANS
3. REFER TO STRUCTURAL NOTES AND PROJECT SPECIFICATIONS FOR DEFINITON OF SYMBOLS, ABBREVIATIONS, AND OTHER INFORMATION AND CRITERIA NOT SHOWN ON PLAN.
4. LOADING CRITERIA IS DEFINED ON DRAWINGS. NOTIFY ENGINEER OF SPECIAL ROOF/CEILING LOADING CONDITIONS NOT OTHERWISE IDENTIFIED ON THE CONTRACT DOCUMENTS.
5. PRIOR TO FABRICATION AND CONSTRUCTION, THE CONTRACTOR SHALL VERIFY DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT / ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION.
6. ⟋⟍ INDICATES SPAN DIRECTION OF STRUCTURAL DECK. SEE NOTES ON THIS SHEET FOR DECK SPECIFICATIONS.
7. TOP OF ROOF AND CEILING SLOPE ELEVATIONS AND LOCATIONS VARY AND SHALL BE COORDINATED WITH THE ARCHITECTURAL DRAWINGS PRIOR TO FABRICATION.
8. FRAMING SHALL BE EQUALLY SPACED BETWEEN COLUMN BAYS UNLESS NOTED OTHERWISE OR DIMENSIONED ON PLAN.
9. COORDINATE FINAL SIZE AND LOCATION OF ROOF OPENINGS, PENETRATIONS, AND CHASES WITH ARCHITECTURAL AND MEP/FP DRAWINGS.
10. DIMENSIONS TO ROOF TOP EQUIPMENT ARE APPROXIMATE AND SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO FABRICATION AND UPON FINAL SELECTION OF EQUIPMENT AND CURBING.
11. HORIZONTAL BRIDGING PER JOIST MFR MIN AS SHOWN ON PLAN, TYP. REFER 35 / S8.6
12. UPLIFT BRIDGING PER JOIST MFR MIN AS SHOWN ON PLAN, TYP. REFER 35 / S8.6 (SIM)
13. ⟋⟍ DENOTES DECK SPAN DIRECTION.
14. ◂▸ DENOTES MOMENT CONNECTION. DESIGN MOMENT CONNECTION FOR FULL MOMENT AND SHEAR CAPACITY OF BEAM.
15. BEAM END REACTIONS INDICATED ON PLAN ARE ASD SERVICE LOADS.
16. FOR BEAM REACTIONS NOT INDICATED IN PLAN DESIGN THE CONNECTION FOR A MINIMUM REACTION OF 6K.
17. REFER 9 / S8.3 FOR TYP. LINTEL SCHEDULE
18. **L3** - INDICATES L5x3x3/8 (LLV) CONT W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC
    L4 - INDICATES L5x3x3/8 (LLV) CONT W/ VERTICAL LEG NOTCHED AS NEEDED W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU / CONC
    L7 - INDICATES L6x4x3/8 CONT W/ VERTICAL LEG NOTCHED AS NEEDED ANCHORED W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE AT 12" OC.

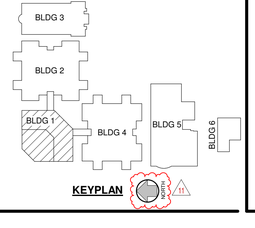
LEGEND:

TE-#	DENOTES THICKENED SLAB EDGE, SEE 33 / S8.1
TB#	DENOTES CONCRETE TIE BEAM TYPE, SEE SCHEDULE ON S7.1 SHEET
J#	MASONRY JAMB, REFER SCHEDULE ON S7.1
BP#	DENOTES BASE PLATE TYPE, SEE SCHEDULE ON 2 / S7.1 SHEET
MC#	DENOTES MASONRY COLUMN, SEE DETAIL ON THIS SHEET
MSW#	MASONRY SHEARWALL, REFER SCHEDULE ON S7.1
#-TC#	CONCRETE COLUMN REFER SCHEDULE ON S7.2
SC#	STEEL COLUMN REFER SCHEDULE ON S7.2
MC#	MASONRY COLUMN REFER DETAIL ON THIS SHEET
### plf	INDICATES ASD DIAPHRAGM SHEAR.

HATCH LEGEND (ADMIN BLDG)	
	(MW1) UNO, 8" THK MASONRY WALL W/ #5 AT 24" OC
	(MW2) 8" THK MASONRY WALL W/ #5 AT 40" OC
	CONC COLUMN, REFER SCHEDULE S7.2
	8" CONCRETE WALL BELOW SOG
	MASONRY SHEARWALL (MSW)



NON-COMPOSITE BEAM LEGEND



KEYPLAN

CODE REVIEW:

CERTIFICATE:

spiezle

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STEVEN G. SIEGEL AR99546
JOHN F. WRIGHT AR107341

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TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

Δ	REVISION NAME	DATE
8	ASI #8	06.19.2025
11	ASI #11	09.29.2025

FOR BID:

DRAWING TITLE:

ROOF FRAMING PLAN - ADMINISTRATION - BUILDING 1

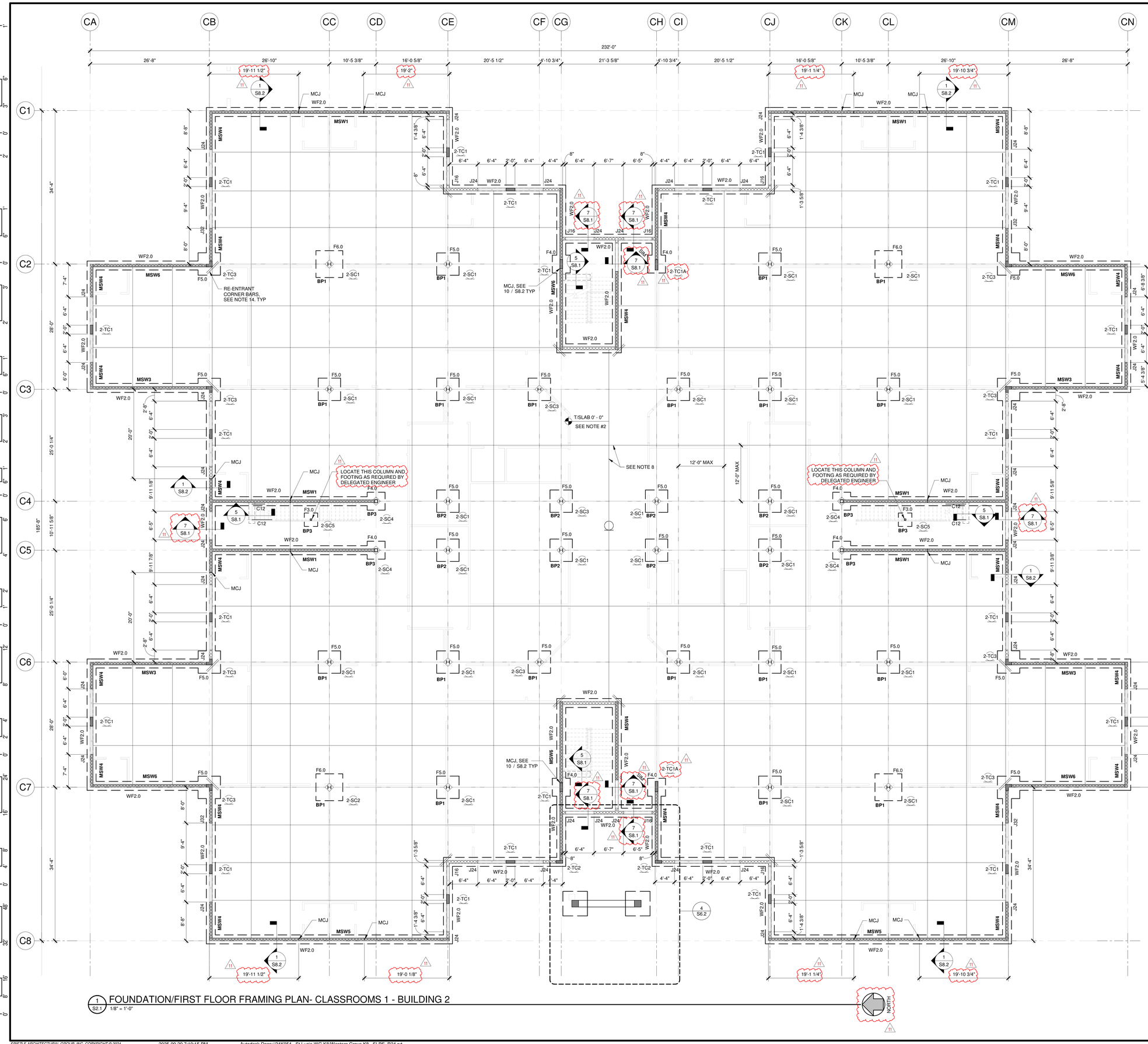
COMMISSION NUMBER:
24FTL018

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S1.3

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FOUNDATION & GROUND FLOOR PLAN NOTES (CLASSROOM BLDG 2 & 4):

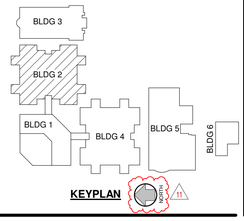
1. T/FIRST FLOOR SLAB = REFERENCE ELEVATION 0'-0" U.N.O.
2. PROVIDE 5" THICK CONCRETE SLAB ON GRADE REINF W/ 6x6 W1.4xW1.4 WWR IN SHEETS (NOT ROLLS) CHAIRED TO TOP OF SLAB, SEE DETAIL 6 / S8.1 . PLACE CONCRETE ON VAPOR BARRIER PER ARCHL REQUIREMENTS BUT NOT LESS THAN 15 MILS. PREPARE SUBGRADE IN ACCORDANCE WITH GEOTECHNICAL REPORT.
3. SEE MEP DRAWINGS FOR SLAB-ON-GRADE PENETRATIONS.
4. TOP OF ALL FOOTINGS SHALL BE -1'-4" U.N.O.
5. CONTINUOUS WALL FOOTINGS SHALL EXTEND FULL LENGTH OF THE FOOTING WIDTH BEYOND END WALLS. U.N.O
6. STEP TOP OF CONTINUOUS WALL FOOTINGS AS NECESSARY FOR PIPES OR CONDUIT. SEE 19 / S8.1
7. FOR TYPICAL CMU WALL DETAILS SEE SCHEDULE ON THIS SHEET FOR DETAILS
8. PROVIDE CONTROL JOINTS PER SECTIONS 2 / S8.1 AND STRUCTURAL NOTES. CONSTRUCTION JOINTS MAY BE PLACED IN LIEU OF CJ AT CONTRACTOR'S DISCRETION, REFER AND SEE 3 / S8.1 .
9. FOR DETAILS OF FOOTINGS AT CORNERS AND INTERSECTIONS SEE 17 / S8.1 .
10. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION OR FABRICATION. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.
11. CENTERLINES OF COLUMNS AND BEAMS SHALL COINCIDE WITH THE FOUNDATION CENTERLINES, UNO.
12. CONCRETE SLAB-ON-GRADE SHALL BE FORMED ON COMPACTED SOILS. REFER TO GEOTECH REPORT FOR SUBGRADE PREPARATION.
13. FOR CONCRETE STRENGTHS REFER TO STRUCTURAL NOTES.
14. RE-ENTRANT CORNER BARS (TYP), SEE DETAIL 13 / S8.1
15. SEE SHEET S7.1 FOR STRUCTURAL SCHEDULES
16. SEE FOOTING SCHEDULE FOR ISOLATED FOOTINGS AND WALL FOOTING SIZE & REINFORCEMENT ON S7.1 SHEET.
17. SEE ARCH DRAWINGS FOR SLOPE AND FLOOR DRAINS IN SOG
18. REFER 9 / S8.3 FOR TYP. LINTEL SCHEDULE
19. SEE STRUCUTRAL NOTES AND GEOTECH REPORT FOR BEARING PRESSURE AND VIBRO REPLACEMENT STONE COLUMN FOUNDATION.

LEGEND:

TE-#	DENOTES THICKENED SLAB EDGE, SEE 33 / S8.1
TB#	DENOTES CONCRETE TIE BEAM TYPE, SEE SCHEDULE ON S7.1 SHEET
J#	MASONRY JAMB, REFER SCHEDULE ON S7.1
BP#	DENOTES BASE PLATE TYPE, SEE SCHEDULE ON 2 / S7.1 SHEET
MC#	DENOTES MASONRY COLUMN, SEE DETAIL ON THIS SHEET
MSW#	MASONRY SHEARWALL, REFER SCHEDULE ON S7.1
#-TC#	CONCRETE COLUMN REFER SCHEDULE ON S7.2
SC#	STEEL COLUMN REFER SCHEDULE ON S7.2
MC#	MASONRY COLUMN REFER DETAIL ON THIS SHEET
### plf	INDICATES ASD DIAPHRAGM SHEAR.

CLASSROOM HATCH LEGEND

	(MW1) UNO 8" THK MASONRY WALL W/ #5 AT 24" OC
	CONC COLUMN, REFER SCHEDULE S7.2
	MASONRY SHEARWALL



CODE REVIEW:

CERTIFICATE:

spiezle

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FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

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FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

Δ	REVISION NAME	DATE
11	ASI #11	09.29.2025

FOR BID:

DRAWING TITLE:

FOUNDATION/FIRST FLOOR FRAMING PLAN- CLASSROOMS 1 - BUILDING 2

COMMISSION NUMBER:
24FTL018

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:
S2.1

THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"

CA CB CC CD CE CF CG CH CI CJ CK CL CM CN

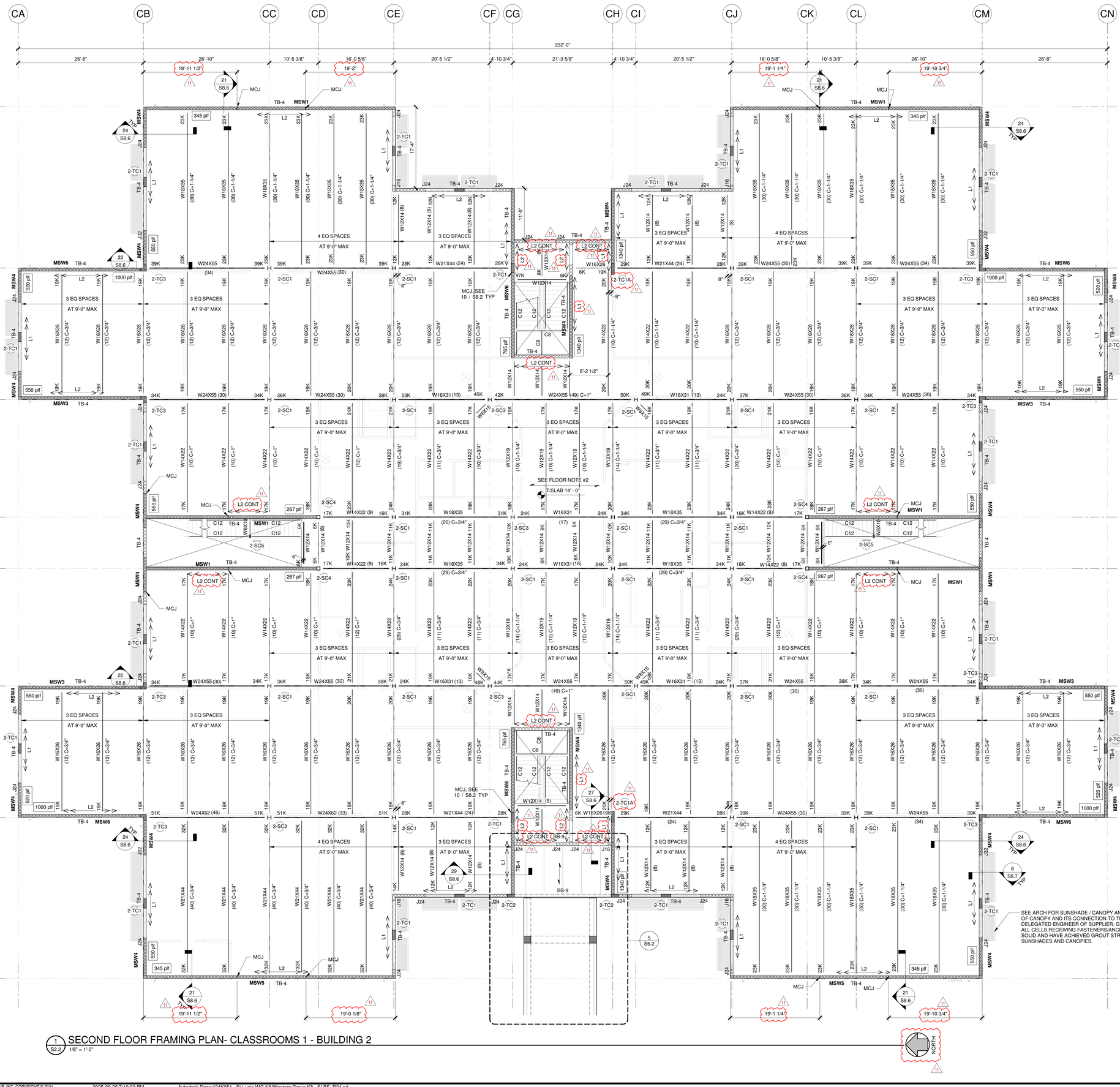
232'-0"

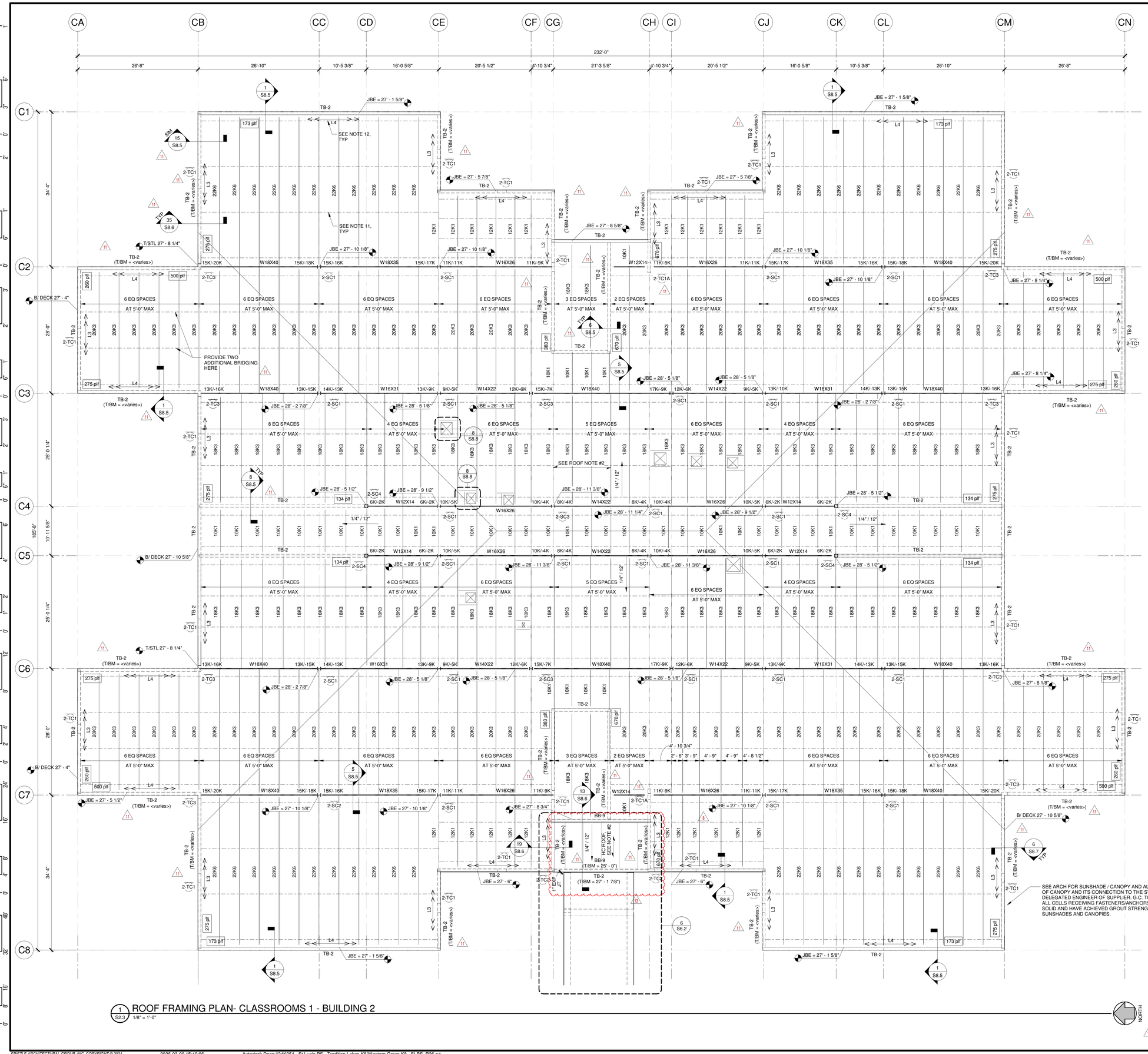
26'-8" | 26'-10" | 10'-5 3/8" | 16'-0 5/8" | 20'-5 1/2" | 4'-10 3/4" | 21'-3 5/8" | 4'-10 3/4" | 20'-5 1/2" | 16'-0 5/8" | 10'-5 3/8" | 26'-10" | 26'-8"

19'-11 1/2" | 19'-2" | 19'-1 1/4" | 19'-10 3/4"

C1 34'-4" | C2 28'-0" | C3 25'-0 1/4" | C4 18'5"-8" | C5 25'-0 1/4" | C6 28'-0" | C7 34'-4" | C8

3"=1'-0" | 1 1/2"=1'-0" | 1"=1'-0" | 3/4"=1'-0" | 1/2"=1'-0" | 1/4"=1'-0" | 1/8"=1'-0" | 1/16"=1'-0"


MCJ
TB-4
MSW1
MSW3
MSW4
MSW5
MSW6
2-TC1
2-TC1A
2-TC2
2-TC3
2-SC1
2-SC2
2-SC3
2-SC4
2-SC5
L1
L2
L2 CONT
J16
J24
J32
345 plf
550 plf
520 plf
1000 plf
765 plf
1340 plf
267 plf
3 EQ SPACES
AT 9'-0" MAX
4 EQ SPACES
AT 9'-0" MAX
MCJ, SEE 10 / S8.2 TYP
SEE FLOOR NOTE #2
T/SLAB 14' - 0"
BB-9
W12X14
W16X26
W18X35
W14X22
W21X44
W24X55
W24X62
W16X31
W16X89
W16X100
W8X15
W8X10
W12X19
W12X14 (8)
W12X14 (6)
W16X26 (12) C=3/4"
W18X35 (30) C=1-1/4"
W14X22 (10) C=1"
W14X22 (12) C=1"
W14X22 (19) C=3/4"
W14X22 (20) C=3/4"
W14X22 (11) C=3/4"
W21X44 (40) C=3/4"
W12X19 (10) C=1-1/4"
W12X19 (14) C=1-1/4"
W12X14 11K
W12X14 10K
W12X14 8K
W12X14 6K
W14X22 (9) 16K
W18X35 (29) C=3/4"
W16X31 (18)
W16X31 (13)
W24X55 (30)
W24X55 (34)
W24X55 (49) C=1"
W24X62 (46)
W24X62 (33)
W21X44 (24)
W16X26 19K
W16X26 19K
W16X26 23K
W16X26 28K
W16X26 29K
W16X26 39K
W16X26 51K
W14X22 17K
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W14X22 34K
W18X35 34K
W18X35 24K
W18X35 22K
W18X35 23K
W18X35 28K
W18X35 29K
W18X35 39K
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W21X44 28K
W21X44 39K
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W24X62 51K
W16X31 (13) 45K
W16X31 (13) 50K
W24X55 (30) 36K
W24X55 (30) 38K
W24X55 (30) 49K
W24X55 (34) 39K
W24X62 (33) 51K
W24X62 (46) 51K
W16X89 2616K
W16X100 2619K
W12X14 (5)
W21X44 (24) 28K
W21X44 (24) 29K
W16X26 29K
W24X55 (30) 29K
W24X55 (34) 29K
W24X55 (30) 23K
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W24X55 (34) 22K
W24X55 (30) 28K
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W24X55 (30) 8K
W24X55 (34) 8K
W24X55 (30) 6K
W24X55 (34) 6K
W12X14 (8) 12K
W12X14 (8) 10K
W12X14 (8) 8K
W12X14 (8) 6K
W12X14 (6) 12K
W12X14 (6) 10K
W12X14 (6) 8K
W12X14 (6) 6K
W12X14 (6) 4K
W12X19 (10) C=1-1/4"
W12X19 (14) C=1-1/4"
W14X22 (10) C=1"
W14X22 (12) C=1"
W14X22 (19) C=3/4"
W14X22 (20) C=3/4"
W14X22 (11) C=3/4"
W18X35 (30) C=1-1/4"
W18X35 (29) C=3/4"
W21X44 (40) C=3/4"
W16X26 (12) C=3/4"
W14X22 (10) C=1-1/4"
W14X22 (9) 16K
W14X22 (9) 17K
W16X31 (18) 24K
W16X31 (13) 24K
W16X31 (13) 45K
W16X31 (13) 50K
W16X31 (13) 24K
W24X55 (49) C=1" 42K
W24X55 (49) C=1" 50K
W24X55 (49) C=1" 49K
W24X62 (33) 51K
W24X62 (46) 51K



ROOF FRAMING NOTES (CLASSROOM 2 & 4):

1. TOP OF STEEL ELEVATION = SEE PLAN.
2. 1-1/2" (20 GAGE) G90 GALV TYPE B ROOF DECK. FASTEN ROOF DECK TO SUPPORTS PER ROOF DECK ATTACHMENT SCHEDULE ON TYPICAL ROOF DETAILS SHEET S8.8. ROOF DECK TO SPAN MINIMUM THREE SPANS. 13
   AT ADMIN BLDG TO CLASSROOM BUILDING CONNECTION, ROOF TO BE 6" HOLLOW CORE PRECAST PLANKS, 6HC WITH 2" TOPPING REINFORCED W/ 6x6-W2.1XW2.1 SLOPE OF HOLLOW CORE AND TOPPING TO MATCH THE METAL ROOF DECK.
3. REFER TO STRUCTURAL NOTES AND PROJECT SPECIFICATIONS FOR DEFINITON OF SYMBOLS, ABBREVIATIONS, AND OTHER INFORMATION AND CRITERIA NOT SHOWN ON PLAN.
4. LOADING CRITERIA IS DEFINED ON DRAWINGS. NOTIFY ENGINEER OF SPECIAL ROOF/CEILING LOADING CONDITIONS NOT OTHERWISE IDENTIFIED ON THE CONTRACT DOCUMENTS.
5. PRIOR TO FABRICATION AND CONSTRUCTION, THE CONTRACTOR SHALL VERIFY DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT / ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION.
6. ▰—► INDICATES SPAN DIRECTION OF STRUCTURAL DECK. SEE NOTES ON THIS SHEET FOR DECK SPECIFICATIONS.
7. TOP OF ROOF AND CEILING SLOPE ELEVATIONS AND LOCATIONS VARY AND SHALL BE COORDINATED WITH THE ARCHITECTURAL DRAWINGS PRIOR TO FABRICATION.
8. FRAMING SHALL BE EQUALLY SPACED BETWEEN COLUMN BAYS UNLESS NOTED OTHERWISE OR DIMENSIONED ON PLAN.
9. COORDINATE FINAL SIZE AND LOCATION OF ROOF OPENINGS, PENETRATIONS, AND CHASES WITH ARCHITECTURAL AND MEP/FP DRAWINGS.
10. DIMENSIONS TO ROOF TOP EQUIPMENT ARE APPROXIMATE AND SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO FABRICATION AND UPON FINAL SELECTION OF EQUIPMENT AND CURBING.
11. HORIZONTAL BRIDGING PER JOIST MFR MIN AS SHOWN ON PLAN, TYP. REFER 35 / S8.6
12. UPLIFT BRIDGING PER JOIST MFR MIN AS SHOWN ON PLAN, TYP. REFER 35 / S8.6 (SIM)
13. ◄► DENOTES MOMENT CONNECTION. DESIGN MOMENT CONNECTION FOR FULL MOMENT AND SHEAR CAPACITY OF BEAM.
14. BEAM END REACTIONS INDICATED ON PLAN ARE ASD SERVICE LOADS.
15. FOR BEAM REACTIONS NOT INDICATED IN PLAN DESIGN THE CONNECTION FOR A MINIMUM REACTION OF 6K.
16. THE DECK SHALL BE PRECAST 6" HOLLOW CORE SPANNING AS SHOWN ON THE PLAN W/ 2" MIN. CONCRETE TOPPING REINF W/ 6X6-W2.1XW2.1 WIRE MESH REINFORCEMENT (USE FLAT SHEETS NOT IN ROLLS) AT TOP OF THE TOPPING. SEE ARCH FOR SLOPES IN THE TOPPING.
17. REFER 9 / S8.3 FOR TYP. LINTEL SCHEDULE
18. **L3** - INDICATES L5x3x3/8 (LLV) CONT W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU / CONC 11
    L4 - INDICATES L5x3x3/8 (LLV) CONT W/ VERTICAL LEG NOTCHED AS NEEDED W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU / CONC

LEGEND:

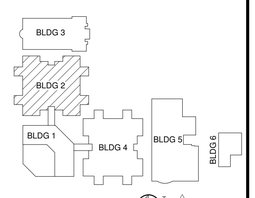
TE-#	DENOTES THICKENED SLAB EDGE, SEE 33 / S8.1
TB#	DENOTES CONCRETE TIE BEAM TYPE, SEE SCHEDULE ON S7.1 SHEET
J#	MASONRY JAMB, REFER SCHEDULE ON S7.1
BP#	DENOTES BASE PLATE TYPE, SEE SCHEDULE ON 2 / S7.1 SHEET
MC#	DENOTES MASONRY COLUMN, SEE DETAIL ON THIS SHEET
MSW#	MASONRY SHEARWALL, REFER SCHEDULE ON S7.1
#-TC#	CONCRETE COLUMN REFER SCHEDULE ON S7.2
SC#	STEEL COLUMN REFER SCHEDULE ON S7.2
MC#	MASONRY COLUMN REFER DETAIL ON THIS SHEET
### plf	INDICATES ASD DIAPHRAGM SHEAR.

CLASSROOM HATCH LEGEND	
	(MW1) UNO 8" THK MASONRY WALL W/ #5 AT 24" OC
	CONC COLUMN, REFER SCHEDULE S7.2
	MASONRY SHEARWALL

BEAM SIZE
+16K / -14K W12x19 C=1" +16K / -14K
(T/STL = X'-X")
CAMBER AT MIDSPAN
TOP OF STEEL ELEV
SHEAR AT END OF BEAM FOR CONNECTION DESIGN, TYP. 6K MIN IF NO REACTION PROVIDED. +16K / -14K INDICATE GRAVITY SERVICE FORCE AND EFFECTIVE UPLIFT SERVICE FORCE.
SUPPORTING BEAM OR COLUMN, AS APPLICABLE, TYP.

NON-COMPOSITE BEAM LEGEND

SEE ARCH FOR SUNSHADE / CANOPY AND ALTERNATE BID. DESIGN OF CANOPY AND ITS CONNECTION TO THE STRUCTURE BY DELEGATED ENGINEER OF SUPPLIER. G.C. TO MAKE SURE THAT ALL CELLS RECEIVING FASTENERS/ANCHORS ARE GROUTED SOLID AND HAVE ACHIEVED GROUT STRENGTH. TYP FOR ALL SUNSHADES AND CANOPIES.



KEYPLAN

CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
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Certificate of Authorization No. 35826

TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

Δ	REVISION NAME	DATE
8	ASI #8	06.19.2025
11	ASI #11	09.29.2025
13	ASI #17	03.06.2026

FOR BID:

DRAWING TITLE:

ROOF FRAMING PLAN- CLASSROOMS 1 - BUILDING 2

COMMISSION NUMBER:

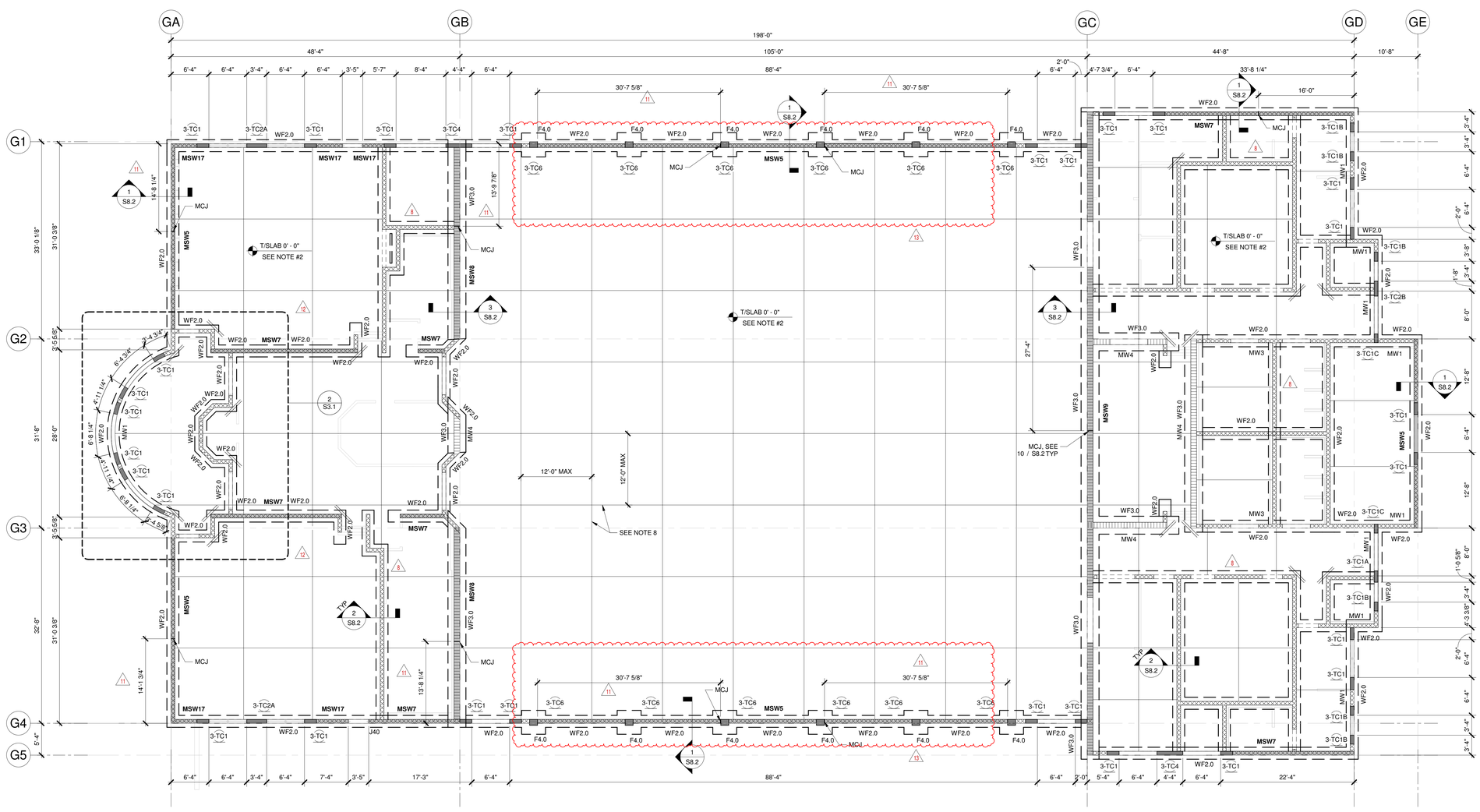
24FTL018

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S2.3

THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"



1 S3.1 FOUNDATION/FIRST FLOOR FRAMING PLAN- GYM - BUILDING 3

1/8" = 1'-0"

NORTH

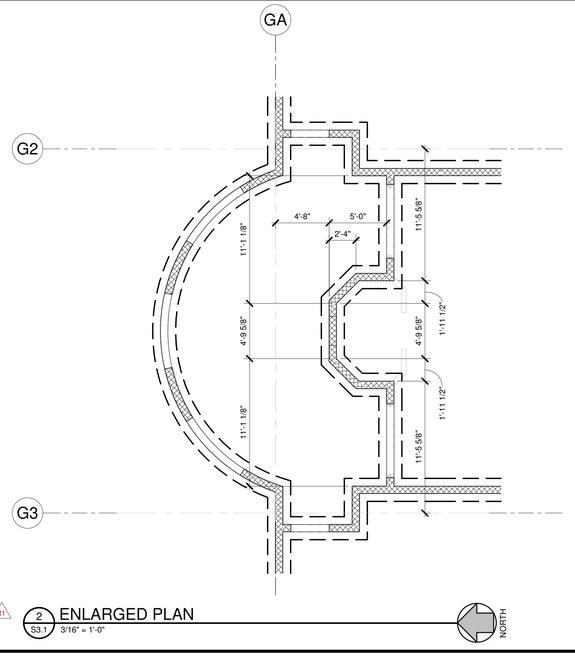
FOUNDATION & GROUND FLOOR PLAN NOTES (GYM BLDG):

1. T/FIRST FLOOR SLAB = REFERENCE ELEVATION 0'-0" U.N.O.
2. PROVIDE 5" THICK CONCRETE SLAB ON GRADE REINF W/ 6x6 W1.4xW1.4 WWR IN SHEETS (NOT ROLLS) CHAIRED TO TOP OF SLAB, SEE DETAIL 6 / S8.1 . PLACE CONCRETE ON VAPOR BARRIER PER ARCHL REQUIREMENTS BUT NOT LESS THAN 15 MILS. PREPARE SUBGRADE IN ACCORDANCE WITH GEOTECHNICAL REPORT.
3. SEE MEP DRAWINGS FOR SLAB-ON-GRADE PENETRATIONS.
4. TOP OF ALL FOOTINGS SHALL BE -1'-4" U.N.O.
5. CONTINUOUS WALL FOOTINGS SHALL EXTEND FULL LENGTH OF THE FOOTING WIDTH BEYOND END WALLS. U.N.O
6. STEP TOP OF CONTINUOUS WALL FOOTINGS AS NECESSARY FOR PIPES OR CONDUIT. SEE 19 / S8.1
7. FOR TYPICAL CMU WALL DETAILS SEE SCHEDULE ON THIS SHEET FOR DETAILS
8. PROVIDE CONTROL JOINTS PER SECTIONS 2 / S8.1 AND STRUCTURAL NOTES. CONSTRUCTION JOINTS MAY BE PLACED IN LIEU OF CJ AT CONTRACTOR'S DISCRETION, REFER AND SEE 3 / S8.1 .
9. FOR DETAILS OF FOOTINGS AT CORNERS AND INTERSECTIONS SEE 17 / S8.1 .
10. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION OR FABRICATION. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.
11. CENTERLINES OF COLUMNS AND BEAMS SHALL COINCIDE WITH THE FOUNDATION CENTERLINES, UNO.
12. CONCRETE SLAB-ON-GRADE SHALL BE FORMED ON COMPACTED SOILS. REFER TO GEOTECH REPORT FOR SUBGRADE PREPARATION.
13. FOR CONCRETE STRENGTHS REFER TO STRUCTURAL NOTES.
14. RE-ENTRANT CORNER BARS (TYP), SEE DETAIL 13 / S8.1
15. SEE SHEET S7.1 FOR STRUCTURAL SCHEDULES
16. SEE FOOTING SCHEDULE FOR ISOLATED FOOTINGS AND WALL FOOTING SIZE & REINFORCEMENT ON S7.1 SHEET.
17. SEE ARCH DRAWINGS FOR SLOPE AND FLOOR DRAINS IN SOG
18. REFER 9 / S8.3 FOR TYP. LINTEL SCHEDULE
19. SEE STRUCUTRAL NOTES AND GEOTECH REPORT FOR SOIL IMPROVEMENT

LEGEND:

TE-#	DENOTES THICKENED SLAB EDGE, SEE 33 / S8.1
TB#	DENOTES CONCRETE TIE BEAM TYPE, SEE SCHEDULE ON S7.1 SHEET
J#	MASONRY JAMB, REFER SCHEDULE ON S7.1
BP#	DENOTES BASE PLATE TYPE, SEE SCHEDULE ON 2 / S7.1 SHEET
MC#	DENOTES MASONRY COLUMN, SEE DETAIL ON THIS SHEET
MSW#	MASONRY SHEARWALL, REFER SCHEDULE ON S7.1
#-TC#	CONCRETE COLUMN REFER SCHEDULE ON S7.2
SC#	STEEL COLUMN REFER SCHEDULE ON S7.2
MC#	MASONRY COLUMN REFER DETAIL ON THIS SHEET
### plf	INDICATES ASD DIAPHRAGM SHEAR.

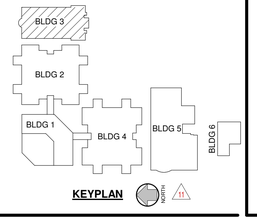
GYM HATCH LEGEND	
	(MW1) 8" THK MASONRY WALL W/ #5 AT 24" OC
	(MW3) UNO 8" THK MASONRY WALL W/ #5 AT 48" OC
	(MW4) 12" THK MASONRY WALL W/ #5 AT 24" OC
	MASONRY SHEARWALL - 8" THK
	MASONRY SHEARWALL - 12" THK
	CONC COLUMN, REFER SCHEDULE S7.2



2 S3.1 ENLARGED PLAN

3/16" = 1'-0"

NORTH



KEYPLAN

CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
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FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
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FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

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REVISIONS:

#	REVISION NAME	DATE
4	ADDENDUM 4	01.27.2025
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11	ASI #11	09.29.2025
12	ASI #14	12.05.2025
13	ASI #17	03.06.2026

FOR BID:

DRAWING TITLE:

FOUNDATION/FIRST FLOOR FRAMING PLAN- GYM - BUILDING 3

COMMISSION NUMBER:

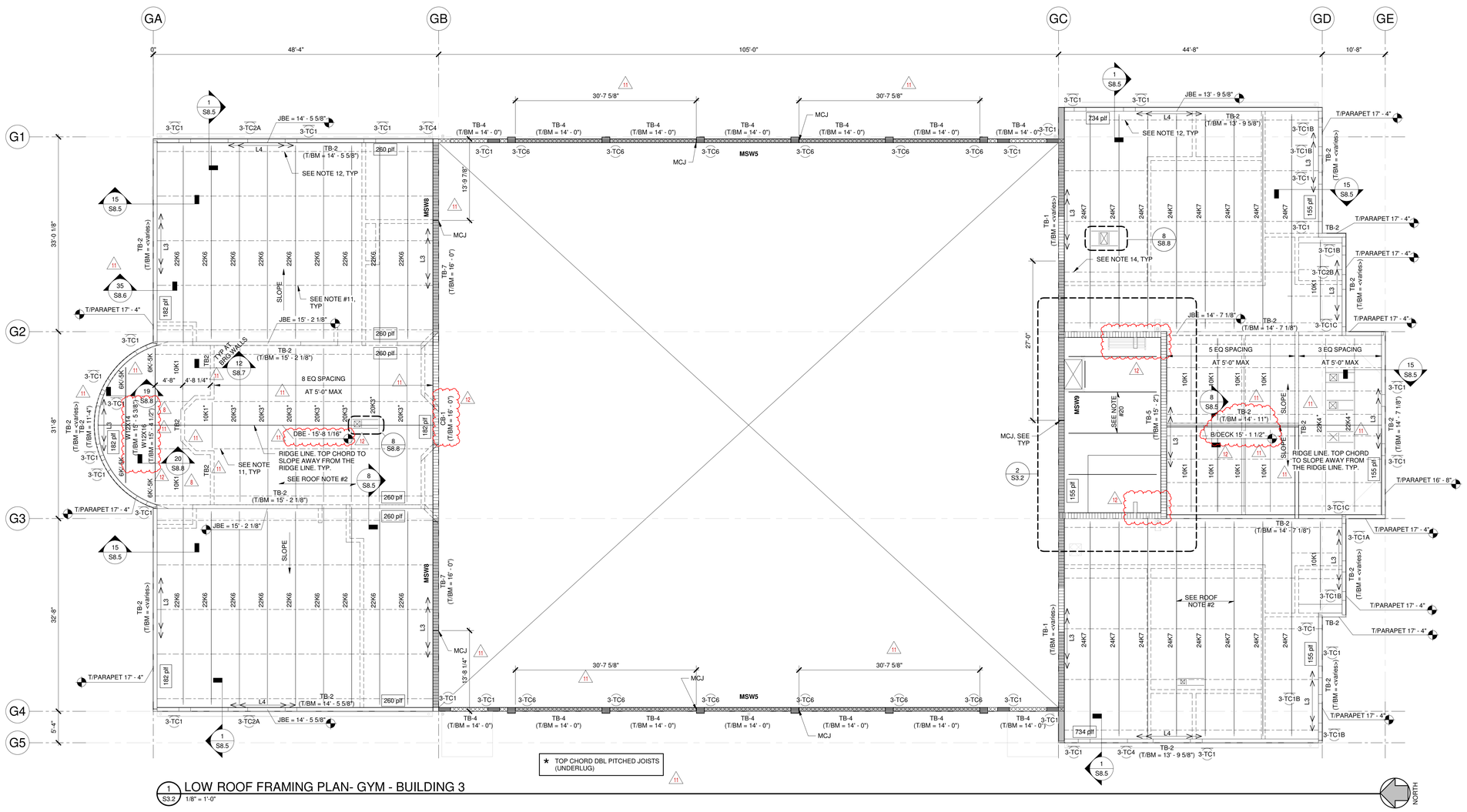
24FTL018

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S3.1

THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"



1 S3.2 LOW ROOF FRAMING PLAN- GYM - BUILDING 3
1/8" = 1'-0"

2 S3.2 PART PLAN
3/16" = 1'-0"

NORTH

ROOF FRAMING NOTES (GYM BLDG):

1. TOP OF STEEL ELEVATION = SEE PLAN.
2. 1-1/2" (20 GAGE) G90 GALV TYPE B ROOF DECK. FASTEN ROOF DECK TO SUPPORTS PER ROOF DECK ATTACHMENT SCHEDULE ON TYPICAL ROOF DETAILS SHEET S8.8. ROOF DECK TO SPAN MINIMUM THREE SPANS AT ADMIN BLDG TO CLASSROOM BUILDING CONNECTION, ROOF TO BE 6" HOLLOW CORE PRECAST PLANKS, 6HC WITH MINIMUM 2" TOPPING REINFORCED W/ 6x6-W2.1XW2.1 FOR SLOPE AND MAX THICKNESS OF TOPPING SEE ARCH.
3. REFER TO STRUCTURAL NOTES AND PROJECT SPECIFICATIONS FOR DEFINITON OF SYMBOLS, ABBREVIATIONS, AND OTHER INFORMATION AND CRITERIA NOT SHOWN ON PLAN.
4. LOADING CRITERIA IS DEFINED ON DRAWINGS. NOTIFY ENGINEER OF SPECIAL ROOF/CEILING LOADING CONDITIONS NOT OTHERWISE IDENTIFIED ON THE CONTRACT DOCUMENTS.
5. PRIOR TO FABRICATION AND CONSTRUCTION, THE CONTRACTOR SHALL VERIFY DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT / ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION.
6. INDICATES SPAN DIRECTION OF STRUCTURAL DECK. SEE NOTES ON THIS SHEET FOR DECK SPECIFICATIONS.
7. TOP OF ROOF AND CEILING SLOPE ELEVATIONS AND LOCATIONS VARY AND SHALL BE COORDINATED WITH THE ARCHITECTURAL DRAWINGS PRIOR TO FABRICATION.
8. FRAMING SHALL BE EQUALLY SPACED BETWEEN COLUMN BAYS UNLESS NOTED OTHERWISE OR DIMENSIONED ON PLAN.
9. COORDINATE FINAL SIZE AND LOCATION OF ROOF OPENINGS, PENETRATIONS, AND CHASES WITH ARCHITECTURAL AND MEP/FP DRAWINGS.
10. DIMENSIONS TO ROOF TOP EQUIPMENT ARE APPROXIMATE AND SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO FABRICATION AND UPON FINAL SELECTION OF EQUIPMENT AND CURBING.
11. HORIZONTAL BRIDGING PER JOIST MFR MIN AS SHOWN ON PLAN, TYP. REFER 35 / S8.6
12. UPLIFT BRIDGING PER JOIST MFR MIN AS SHOWN ON PLAN, TYP. REFER 35 / S8.6 (SIM)

ROOF FRAMING NOTES (GYM BLDG):

13. ◂▸ DENOTES MOMENT CONNECTION. DESIGN MOMENT CONNECTION FOR FULL MOMENT AND SHEAR CAPACITY OF BEAM.
14. BEAM END REACTIONS INDICATED ON PLAN ARE ASD SERVICE LOADS.
15. FOR BEAM REACTIONS NOT INDICATED IN PLAN DESIGN THE CONNECTION FOR A MINIMUM REACTION OF 6K.
16. THE DECK SHALL BE PRECAST 6" HOLLOW CORE SPANNING AS SHOWN ON THE PLAN W/ 2" MIN. CONCRETE TOPPING REINF W/ 6X6-W2.1XW2.1 WIRE MESH REINFORCEMENT (USE FLAT SHEETS NOT IN ROLLS) AT TOP OF THE TOPPING. SEE ARCH FOR SLOPES IN THE TOPPING.
17. REFER 9 / S8.3 FOR TYP. LINTEL SCHEDULE
18. **L3** - INDICATES L5x3x3/8 (LLV) W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU/ CONC
L4 - INDICATES L5x3x3/8 (LLV) CONT W/ VERTICAL LEG NOTCHED AS NEEDED W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU / CONC

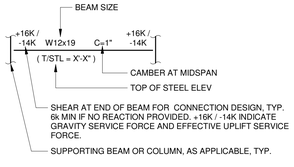
COMPOSITE FLOOR NOTES (GYM BLDG):

19. TOP OF SLAB ELEVATION = 13' - 7".
TOP OF STEEL = 13' - 0 1/2".
20. UNO FLOOR DECK TO BE 4 1/2" NORMAL WEIGHT CONCRETE (F'C = 3000 PSI) ON 2" (20 GA) COMPOSITE DECK, 2VLI BY VULCRAFT (6 1/2" TOTAL THICKNESS) W/ 6x6-W2.1xW2.1 WELDED WIRE REINFORCING (WWR) IN SHEETS NOT ROLLS, CHAIRED TO THE TOP OF SLAB. SHORE DECK IF LESS THAN (3) CONTINUOUS SPANS AND IN ACCORDANCE W/ MAX SDI UNSHORED CLEAR SPAN PUBLISHED BY MFR UNTIL CONCRETE HAS REACHED 75% COMPRESSIVE STRENGTH. FLOOR DECK TO SPAN MINIMUM THREE SPAN FOR UNSHORED CONDITION. SEE PLAN FOR EXTENTS
21. FLOOR LOADS:
DEAD LOAD = SEE LOADING PLANS
LIVE LOAD = SEE LOADING PLANS
22. ALL COMPOSITE FLOOR BEAMS SHALL HAVE A MINIMUM OF 3/4"ø X 5 1/2" HEADED STUDS AT 12" MAXIMUM (ONE STUD PER BEAM MIN UNO) SEE DETAIL
23. CONTRACTOR SHALL VERIFY ALL SLAB OPENING DIMENSIONS AND LOCATION WITH ARCH AND MECH CONTRACTOR PRIOR TO FABRICATION OF STEEL.
24. BEAM END REACTIONS INDICATED ON PLAN ARE ASD SERVICE LOADS.
25. FOR BEAM REACTIONS NOT INDICATED IN PLAN DESIGN THE CONNECTION FOR A MINIMUM REACTION OF 6K.
26. FOR DIMENSIONS NOT SHOWN SEE ARCHITECTURAL DRAWINGS.
27. **L1** - INDICATES L6x4x1/2 (LLV) CONT W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC
L2 - INDICATES L5x3x3/8 (LLV) CONT W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU / CONC

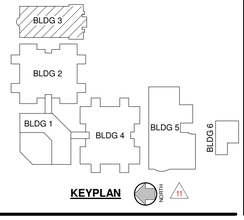
LEGEND:

TE-#	DENOTES THICKENED SLAB EDGE, SEE 33 / S8.1
TB#	DENOTES CONCRETE TIE BEAM TYPE, SEE SCHEDULE ON S7.1 SHEET
J#	MASONRY JAMB, REFER SCHEDULE ON S7.1
BP#	DENOTES BASE PLATE TYPE, SEE SCHEDULE ON 2 / S7.1 SHEET
MC#	DENOTES MASONRY COLUMN, SEE DETAIL ON THIS SHEET
MSW#	MASONRY SHEARWALL, REFER SCHEDULE ON S7.1
#-TC#	CONCRETE COLUMN REFER SCHEDULE ON S7.2
SC#	STEEL COLUMN REFER SCHEDULE ON S7.2
MC#	MASONRY COLUMN REFER DETAIL ON THIS SHEET
### plf	INDICATES ASD DIAPHRAGM SHEAR.

GYM HATCH LEGEND	
	(MW1) 8" THK MASONRY WALL W/ #5 AT 24" OC
	(MW3) UNO 8" THK MASONRY WALL W/ #5 AT 48" OC
	(MW4) 12" THK MASONRY WALL W/ #5 AT 24" OC
	MASONRY SHEARWALL - 8" THK
	MASONRY SHEARWALL - 12" THK
	CONC COLUMN, REFER SCHEDULE S7.2



NON-COMPOSITE BEAM LEGEND



KEYPLAN

CODE REVIEW:

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spiezle

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VERO BEACH, FL 32960
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11	ASI #11	09.29.2025
12	ASI #14	12.05.2025

FOR BID:

DRAWING TITLE:

LOW ROOF FRAMING PLAN- GYM - BUILDING 3

COMMISSION NUMBER:

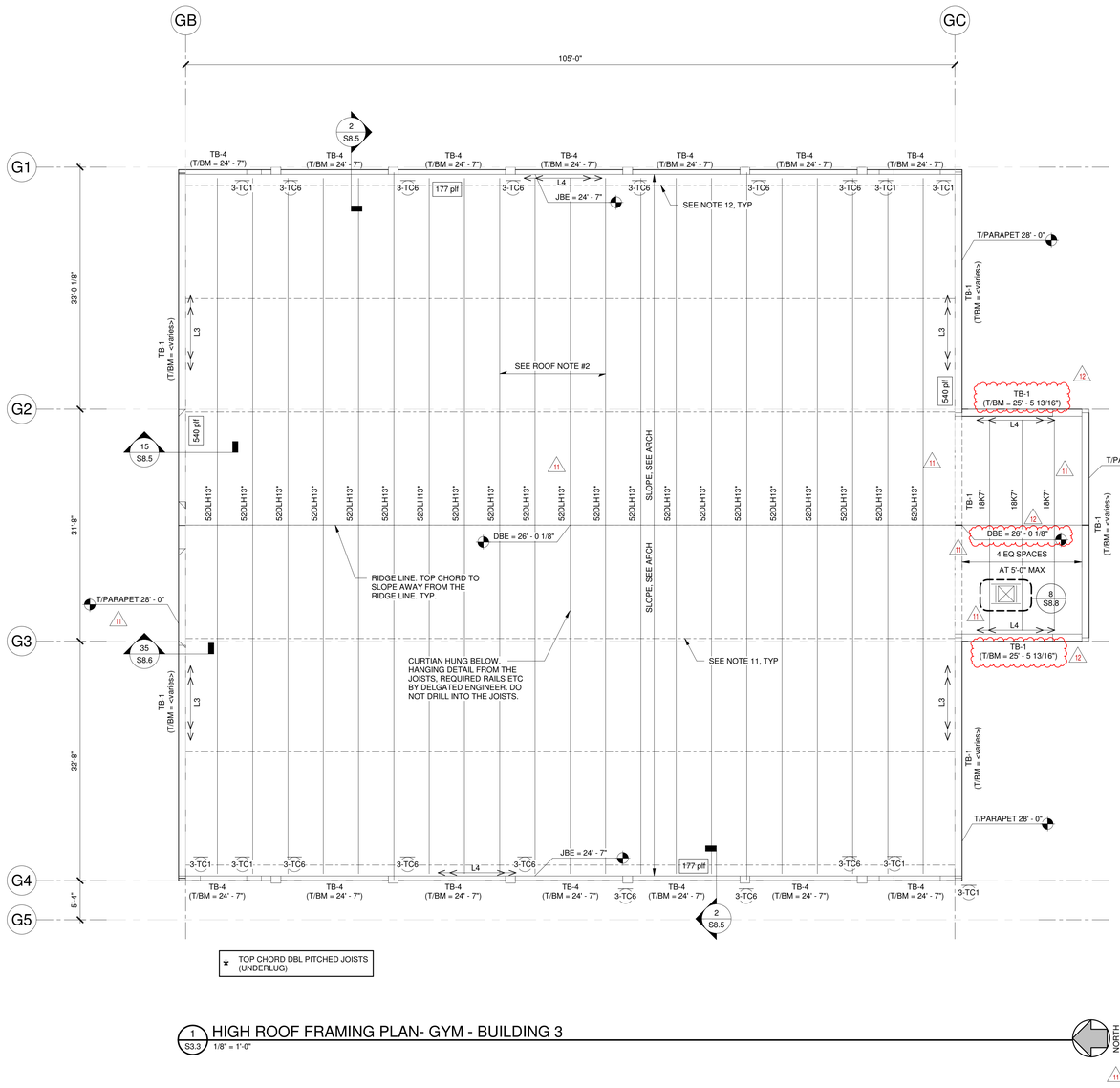
24FTL018

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S3.2

THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"



* TOP CHORD DBL PITCHED JOISTS (UNDERLUG)

1 S3.3 HIGH ROOF FRAMING PLAN- GYM - BUILDING 3

1/8" = 1'-0"

NORTH

11

ROOF FRAMING NOTES (GYM BLDG):

1. TOP OF STEEL ELEVATION = SEE PLAN.
2. 1-1/2" (20 GAGE) G90 GALV TYPE B ROOF DECK. FASTEN ROOF DECK TO SUPPORTS PER ROOF DECK ATTACHMENT SCHEDULE ON TYPICAL ROOF DETAILS SHEET S8.8. ROOF DECK TO SPAN MINIMUM THREE SPANS
   AT ADMIN BLDG TO CLASSROOM BUILDING CONNECTION, ROOF TO BE 6" HOLLOW CORE PRECAST PLANKS, 6HC WITH MINIMUM 2" TOPPING REINFORCED W/ 6x6-W2.1XW2.1 FOR SLOPE AND MAX THICKNESS OF TOPPING SEE ARCH.
3. REFER TO STRUCTURAL NOTES AND PROJECT SPECIFICATIONS FOR DEFINITON OF SYMBOLS, ABBREVIATIONS, AND OTHER INFORMATION AND CRITERIA NOT SHOWN ON PLAN.
4. LOADING CRITERIA IS DEFINED ON DRAWINGS. NOTIFY ENGINEER OF SPECIAL ROOF/CEILING LOADING CONDITIONS NOT OTHERWISE IDENTIFIED ON THE CONTRACT DOCUMENTS.
5. PRIOR TO FABRICATION AND CONSTRUCTION, THE CONTRACTOR SHALL VERIFY DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT / ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION.
6. ▰▱ INDICATES SPAN DIRECTION OF STRUCTURAL DECK. SEE NOTES ON THIS SHEET FOR DECK SPECIFICATIONS.
7. TOP OF ROOF AND CEILING SLOPE ELEVATIONS AND LOCATIONS VARY AND SHALL BE COORDINATED WITH THE ARCHITECTURAL DRAWINGS PRIOR TO FABRICATION.
8. FRAMING SHALL BE EQUALLY SPACED BETWEEN COLUMN BAYS UNLESS NOTED OTHERWISE OR DIMENSIONED ON PLAN.
9. COORDINATE FINAL SIZE AND LOCATION OF ROOF OPENINGS, PENETRATIONS, AND CHASES WITH ARCHITECTURAL AND MEP/FP DRAWINGS.
10. DIMENSIONS TO ROOF TOP EQUIPMENT ARE APPROXIMATE AND SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO FABRICATION AND UPON FINAL SELECTION OF EQUIPMENT AND CURBING.
11. HORIZONTAL BRIDGING PER JOIST MFR MIN AS SHOWN ON PLAN, TYP. REFER 35 / S8.6
12. UPLIFT BRIDGING PER JOIST MFR MIN AS SHOWN ON PLAN, TYP. REFER 35 / S8.6 (SIM)

ROOF FRAMING NOTES (GYM BLDG):

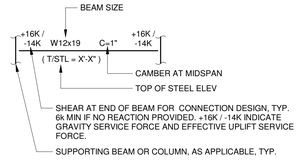
13. ◂▸ DENOTES MOMENT CONNECTION. DESIGN MOMENT CONNECTION FOR FULL MOMENT AND SHEAR CAPACITY OF BEAM.
14. BEAM END REACTIONS INDICATED ON PLAN ARE ASD SERVICE LOADS.
15. FOR BEAM REACTIONS NOT INDICATED IN PLAN DESIGN THE CONNECTION FOR A MINIMUM REACTION OF 6K.
16. THE DECK SHALL BE PRECAST 6" HOLLOW CORE SPANNING AS SHOWN ON THE PLAN W/ 2" MIN. CONCRETE TOPPING REINF W/ 6X6-W2.1XW2.1 WIRE MESH REINFORCEMENT (USE FLAT SHEETS NOT IN ROLLS) AT TOP OF THE TOPPING. SEE ARCH FOR SLOPES IN THE TOPPING.
17. REFER 9 / S8.3 FOR TYP. LINTEL SCHEDULE
18. **L3** - INDICATES L5x3x3/8 (LLV) W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU/ CONC
    L4 - INDICATES L5x3x3/8 (LLV) CONT W/ VERTICAL LEG NOTCHED AS NEEDED W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU / CONC

LEGEND:

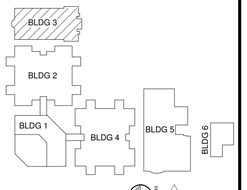
TE-#	DENOTES THICKENED SLAB EDGE, SEE 33 / S8.1
TB#	DENOTES CONCRETE TIE BEAM TYPE, SEE SCHEDULE ON S7.1 SHEET
J#	MASONRY JAMB, REFER SCHEDULE ON S7.1
BP#	DENOTES BASE PLATE TYPE, SEE SCHEDULE ON 2 / S7.1 SHEET
MC#	DENOTES MASONRY COLUMN, SEE DETAIL ON THIS SHEET
MSW#	MASONRY SHEARWALL, REFER SCHEDULE ON S7.1
#-TC#	CONCRETE COLUMN REFER SCHEDULE ON S7.2
SC#	STEEL COLUMN REFER SCHEDULE ON S7.2
MC#	MASONRY COLUMN REFER DETAIL ON THIS SHEET
### plf	INDICATES ASD DIAPHRAGM SHEAR.

GYM HATCH LEGEND

	(MW1) 8" THK MASONRY WALL W/ #5 AT 24" OC
	(MW3) UNO 8" THK MASONRY WALL W/ #5 AT 48" OC
	(MW4) 12" THK MASONRY WALL W/ #5 AT 24" OC
	MASONRY SHEARWALL - 8" THK
	MASONRY SHEARWALL - 12" THK
	CONC COLUMN, REFER SCHEDULE S7.2



NON-COMPOSITE BEAM LEGEND



KEYPLAN

CODE REVIEW:

CERTIFICATE:

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONADIO AR0012456
STEVEN G. SIEGEL AR99546
JOHN F. WRIGHT AR107341

SEAL:

CONSULTANTS:

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TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

#	REVISION NAME	DATE
11	ASI #11	09.29.2025
12	ASI #14	12.05.2025

FOR BID:

DRAWING TITLE:

HIGH ROOF FRAMING PLAN- GYM - BUILDING 3

COMMISSION NUMBER:

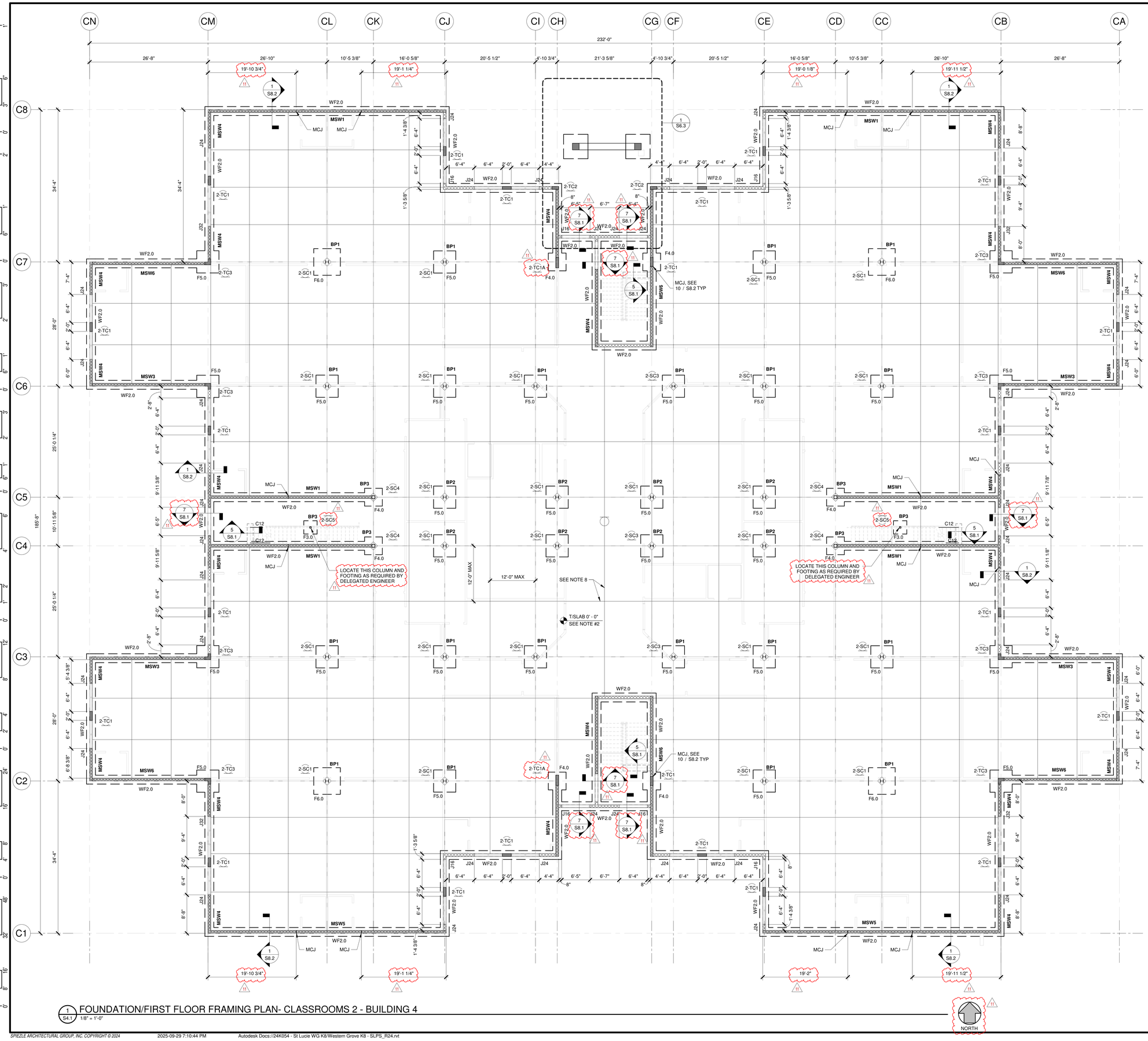
24FTL018

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S3.3

THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"



1 S4.1 FOUNDATION/FIRST FLOOR FRAMING PLAN- CLASSROOMS 2 - BUILDING 4
1/8" = 1'-0"

FOUNDATION & GROUND FLOOR PLAN NOTES (ADMIN BLDG):

1. T/FIRST FLOOR SLAB = REFERENCE ELEVATION 0'-0" U.N.O.
2. PROVIDE 5" THICK CONCRETE SLAB ON GRADE REINF W/ 6x6 W1.4xW1.4 WWR IN SHEETS (NOT ROLLS) CHAIRED TO TOP OF SLAB. SEE DETAIL 6 / S8.1 . PLACE CONCRETE ON VAPOR BARRIER PER ARCHL REQUIREMENTS BUT NOT LESS THAN 15 MILS. PREPARE SUBGRADE IN ACCORDANCE WITH GEOTECHNICAL REPORT.
3. SEE MEP DRAWINGS FOR SLAB-ON-GRADE PENETRATIONS.
4. TOP OF ALL FOOTINGS SHALL BE -1'-4" U.N.O.
5. CONTINUOUS WALL FOOTINGS SHALL EXTEND FULL LENGTH OF THE FOOTING WIDTH BEYOND END WALLS. U.N.O
6. STEP TOP OF CONTINUOUS WALL FOOTINGS AS NECESSARY FOR PIPES OR CONDUIT. SEE 19 / S8.1
7. FOR TYPICAL CMU WALL DETAILS SEE SCHEDULE ON THIS SHEET FOR DETAILS
8. PROVIDE CONTROL JOINTS PER SECTIONS 2 / S8.1 AND STRUCTURAL NOTES. CONSTRUCTION JOINTS MAY BE PLACED IN LIEU OF CJ AT CONTRACTOR'S DISCRETION, REFER AND SEE 3 / S8.1 .
9. FOR DETAILS OF FOOTINGS AT CORNERS AND INTERSECTIONS SEE 17 / S8.1 .
10. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION OR FABRICATION. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.
11. CENTERLINES OF COLUMNS AND BEAMS SHALL COINCIDE WITH THE FOUNDATION CENTERLINES, UNO.
12. CONCRETE SLAB-ON-GRADE SHALL BE FORMED ON COMPACTED SOILS. REFER TO GEOTECH REPORT FOR SUBGRADE PREPARATION.
13. FOR CONCRETE STRENGTHS REFER TO STRUCTURAL NOTES.
14. RE-ENTRANT CORNER BARS (TYP), SEE DETAIL 13 / S8.1
15. SEE SHEET S7.1 FOR STRUCTURAL SCHEDULES
16. SEE FOOTING SCHEDULE FOR ISOLATED FOOTINGS AND WALL FOOTING SIZE & REINFORCEMENT ON S7.1 SHEET.
17. SEE ARCH DRAWINGS FOR SLOPE AND FLOOR DRAINS IN SOG
18. REFER 9 / S8.3 FOR TYP. LINTEL SCHEDULE
19. SEE STRUCUTRAL NOTES AND GEOTECH REPORT FOR BEARING PRESSURE AND VIBRO REPLACEMENT STONE COLUMN FOUNDATION.

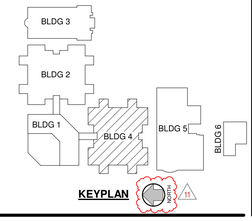
LEGEND:

TE-#	DENOTES THICKENED SLAB EDGE, SEE 33 / S8.1
TB#	DENOTES CONCRETE TIE BEAM TYPE, SEE SCHEDULE ON S7.1 SHEET
J#	MASONRY JAMB, REFER SCHEDULE ON S7.1
BP#	DENOTES BASE PLATE TYPE, SEE SCHEDULE ON 2 / S7.1 SHEET
MC#	DENOTES MASONRY COLUMN, SEE DETAIL ON THIS SHEET
MSW#	MASONRY SHEARWALL, REFER SCHEDULE ON S7.1
#-TC#	CONCRETE COLUMN REFER SCHEDULE ON S7.2
SC#	STEEL COLUMN REFER SCHEDULE ON S7.2
MC#	MASONRY COLUMN REFER DETAIL ON THIS SHEET
### plf	INDICATES ASD DIAPHRAGM SHEAR.

CLASSROOM HATCH LEGEND	
	(MW1) UNO 8" THK MASONRY WALL W/ #5 AT 24" OC
	CONC COLUMN, REFER SCHEDULE S7.2
	MASONRY SHEARWALL

BASE PLATES ARE ADDED 8

KEYPLAN



CODE REVIEW:

CERTIFICATE:

spiezle

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FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

Δ	REVISION NAME	DATE
4	ADDENDUM 4	01.27.2025
8	ASI #8	06.19.2025
11	ASI #11	09.29.2025

FOR BID:

DRAWING TITLE:

FOUNDATION/FIRST FLOOR FRAMING PLAN- CLASSROOMS 2 - BUILDING 4

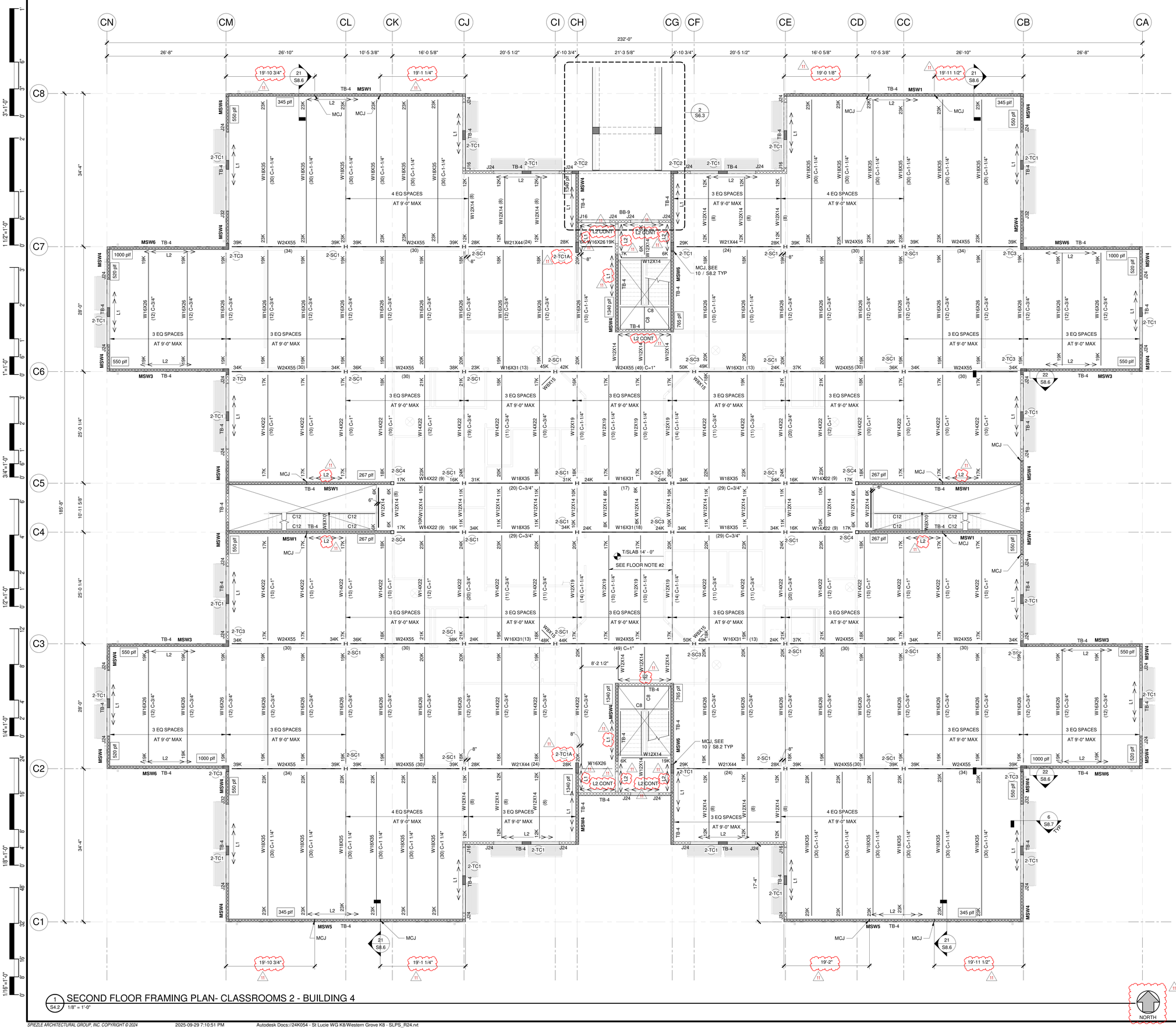
COMMISSION NUMBER:
24FTL018

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S4.1

THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"



1 S4.2 SECOND FLOOR FRAMING PLAN- CLASSROOMS 2 - BUILDING 4
1/8" = 1'-0"

COMPOSITE FLOOR NOTES (CLASSROOM 2 & 4):

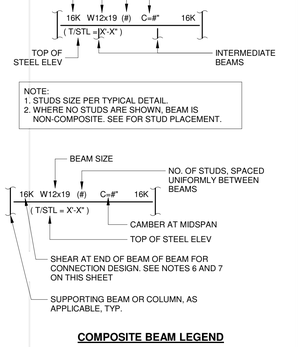
1. TOP OF SLAB ELEVATION = +14' - 0".
   TOP OF STEEL = 13' - 5 1/2".
2. UNO FLOOR DECK TO BE 4 1/2" NORMAL WEIGHT CONCRETE (F'C = 3000 PSI) ON 2" (20 GA) COMPOSITE DECK, 2VLI BY VULCRAFT (6 1/2" TOTAL THICKNESS) W/ 6x6-W2.1xW2.1 WELDED WIRE REINFORCING (WWR) IN SHEETS NOT ROLLS, CHAIRED TO THE TOP OF SLAB. SHORE DECK IF LESS THAN (3) CONTINUOUS SPANS AND IN ACCORDANCE W/ MAX SDI UNSHORED CLEAR SPAN PUBLISHED BY MFR UNTIL CONCRETE HAS REACHED 75% COMPRESSIVE STRENGTH. FLOOR DECK TO SPAN MINIMUM THREE SPAN FOR UNSHORED CONDITION. SEE PLAN FOR EXTENTS AT ADMIN BLDG TO CLASSROOM BUILDING CONNECTION, FLOOR TO BE 8" HOLLOW CORE PRECAST PLANKS, 8HC WITH MINIMUM 2" TOPPING REINFORCED W/ 6x6-W2.1xW2.1 FOR SLOPE AND MAX THICKNESS OF TOPPING SEE ARCH.
3. FLOOR LOADS:
   DEAD LOAD = SEE LOADING PLANS
   LIVE LOAD = SEE LOADING PLANS
4. ALL COMPOSITE FLOOR BEAMS SHALL HAVE A MINIMUM OF 3/4"ø X 5 1/2" HEADED STUDS AT 12" MAXIMUM (ONE STUD PER BEAM MIN UNO) SEE DETAIL
5. CONTRACTOR SHALL VERIFY ALL SLAB OPENING DIMENSIONS AND LOCATION WITH ARCH AND MECH CONTRACTOR PRIOR TO FABRICATION OF STEEL.
6. BEAM END REACTIONS INDICATED ON PLAN ARE ASD SERVICE LOADS.
7. FOR BEAM REACTIONS NOT INDICATED IN PLAN DESIGN THE CONNECTION FOR A MINIMUM REACTION OF 6K.
8. FOR DIMENSIONS NOT SHOWN SEE ARCHITECTURAL DRAWINGS.
9. REFER 9 / S8.3 FOR TYP. LINTEL SCHEDULE
10. **L1** - INDICATES L6x4x1/2 (LLV) CONT W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU / CONC
    L2 - INDICATES L5x3x3/8 (LLV) CONT W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU / CONC

LEGEND:

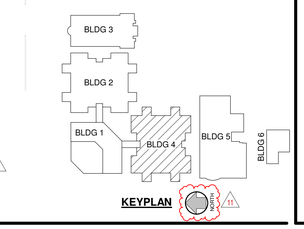
TE-#	DENOTES THICKENED SLAB EDGE, SEE 33 / S8.1
TB#	DENOTES CONCRETE TIE BEAM TYPE, SEE SCHEDULE ON S7.1 SHEET
J#	MASONRY JAMB, REFER SCHEDULE ON S7.1
BP#	DENOTES BASE PLATE TYPE, SEE SCHEDULE ON 2 / S7.1 SHEET
MC#	DENOTES MASONRY COLUMN, SEE DETAIL ON THIS SHEET
MSW#	MASONRY SHEARWALL, REFER SCHEDULE ON S7.1
#-TC#	CONCRETE COLUMN REFER SCHEDULE ON S7.2
SC#	STEEL COLUMN REFER SCHEDULE ON S7.2
MC#	MASONRY COLUMN REFER DETAIL ON THIS SHEET
### plf	INDICATES ASD DIAPHRAGM SHEAR.

CLASSROOM HATCH LEGEND

	(MW1) UNO 8" THK MASONRY WALL W/ #5 AT 24" OC
	CONC COLUMN, REFER SCHEDULE S7.2
	MASONRY SHEARWALL



COMPOSITE BEAM LEGEND



CODE REVIEW:

CERTIFICATE:

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONADO AR0012456
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FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

#	REVISION NAME	DATE
4	ADDENDUM 4	01.27.2025
11	ASI #11	09.29.2025

FOR BID:

DRAWING TITLE:

SECOND FLOOR FRAMING PLAN- CLASSROOMS 2 - BUILDING 4

COMMISSION NUMBER:
24FTL018

DO NOT SCALE THE DRAWINGS

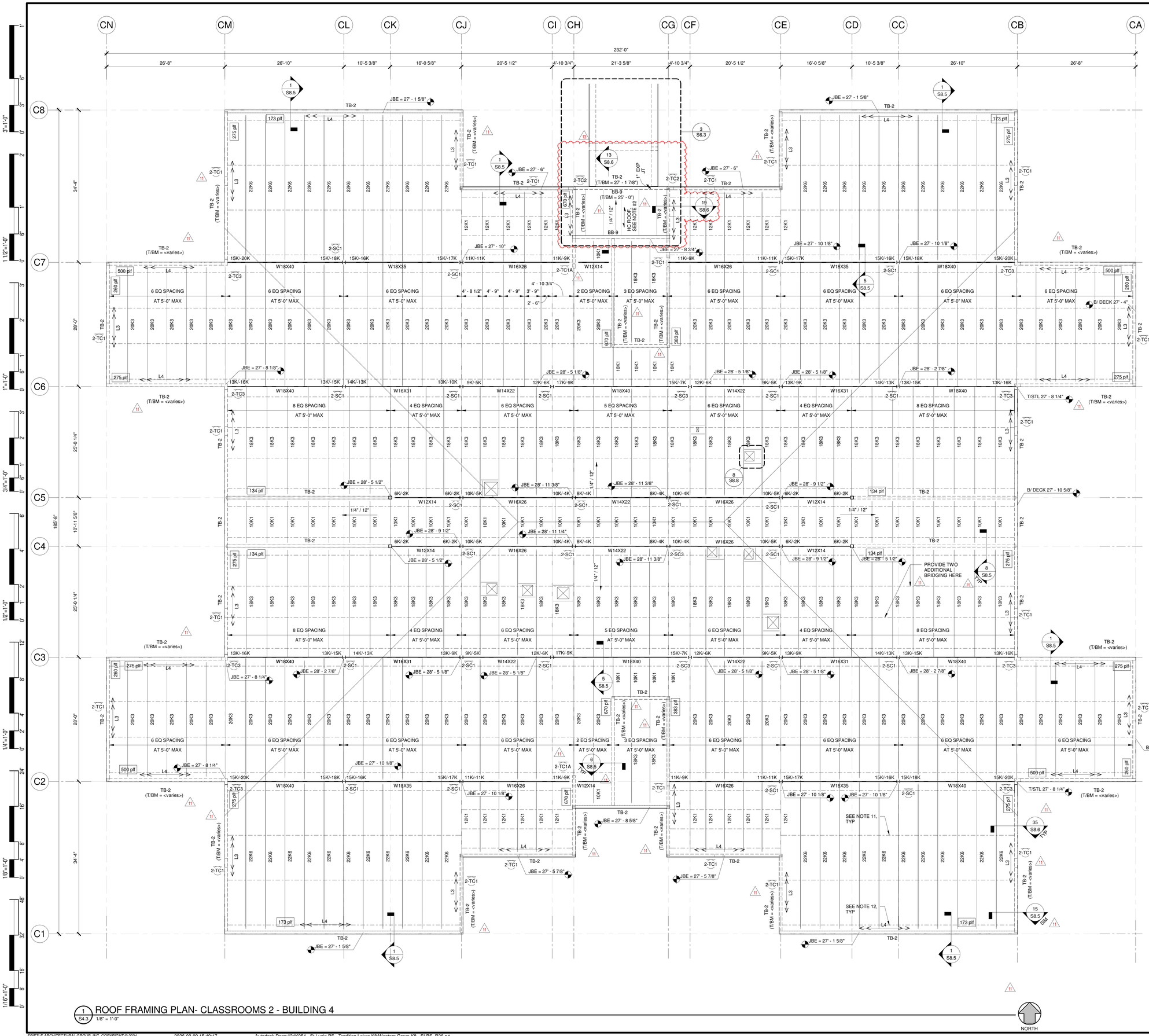
DRAWING NUMBER:

S4.2

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2025-09-29 7:10:51 PM Autodesk Docs://24K054 - St Lucie WG K8/Western Grove K8 - SLPS_R24.rvt



1 S4.3 ROOF FRAMING PLAN- CLASSROOMS 2 - BUILDING 4
1/8" = 1'-0"

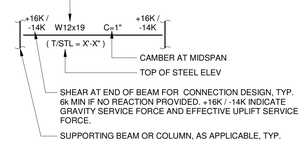
ROOF FRAMING NOTES (CLASSROOM 2 & 4):

1. TOP OF STEEL ELEVATION = SEE PLAN.
2. 1-1/2" (20 GAGE) G90 GALV TYPE B ROOF DECK. FASTEN ROOF DECK TO SUPPORTS PER ROOF DECK ATTACHMENT SCHEDULE ON TYPICAL ROOF DETAILS SHEET S8.8. ROOF DECK TO SPAN MINIMUM THREE SPANS. 13
   AT ADMIN BLDG TO CLASSROOM BUILDING CONNECTION, ROOF TO BE 6" HOLLOW CORE PRECAST PLANKS, 6HC WITH 2" TOPPING REINFORCED W/ 6x6-W2.1XW2.1 SLOPE OF HOLLOW CORE AND TOPPING TO MATCH THE METAL ROOF DECK.
3. REFER TO STRUCTURAL NOTES AND PROJECT SPECIFICATIONS FOR DEFINITON OF SYMBOLS, ABBREVIATIONS, AND OTHER INFORMATION AND CRITERIA NOT SHOWN ON PLAN.
4. LOADING CRITERIA IS DEFINED ON DRAWINGS. NOTIFY ENGINEER OF SPECIAL ROOF/CEILING LOADING CONDITIONS NOT OTHERWISE IDENTIFIED ON THE CONTRACT DOCUMENTS.
5. PRIOR TO FABRICATION AND CONSTRUCTION, THE CONTRACTOR SHALL VERIFY DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT / ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION.
6. ⌐→ INDICATES SPAN DIRECTION OF STRUCTURAL DECK. SEE NOTES ON THIS SHEET FOR DECK SPECIFICATIONS.
7. TOP OF ROOF AND CEILING SLOPE ELEVATIONS AND LOCATIONS VARY AND SHALL BE COORDINATED WITH THE ARCHITECTURAL DRAWINGS PRIOR TO FABRICATION.
8. FRAMING SHALL BE EQUALLY SPACED BETWEEN COLUMN BAYS UNLESS NOTED OTHERWISE OR DIMENSIONED ON PLAN.
9. COORDINATE FINAL SIZE AND LOCATION OF ROOF OPENINGS, PENETRATIONS, AND CHASES WITH ARCHITECTURAL AND MEP/FP DRAWINGS.
10. DIMENSIONS TO ROOF TOP EQUIPMENT ARE APPROXIMATE AND SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO FABRICATION AND UPON FINAL SELECTION OF EQUIPMENT AND CURBING.
11. HORIZONTAL BRIDGING PER JOIST MFR MIN AS SHOWN ON PLAN, TYP. REFER 35 / S8.6
12. UPLIFT BRIDGING PER JOIST MFR MIN AS SHOWN ON PLAN, TYP. REFER 35 / S8.6 (SIM)
13. ◂▸ DENOTES MOMENT CONNECTION. DESIGN MOMENT CONNECTION FOR FULL MOMENT AND SHEAR CAPACITY OF BEAM.
14. BEAM END REACTIONS INDICATED ON PLAN ARE ASD SERVICE LOADS.
15. FOR BEAM REACTIONS NOT INDICATED IN PLAN DESIGN THE CONNECTION FOR A MINIMUM REACTION OF 6K.
16. THE DECK SHALL BE PRECAST 6" HOLLOW CORE SPANNING AS SHOWN ON THE PLAN W/ 2" MIN. CONCRETE TOPPING REINF W/ 6X6-W2.1XW2.1 WIRE MESH REINFORCEMENT (USE FLAT SHEETS NOT IN ROLLS) AT TOP OF THE TOPPING. SEE ARCH FOR SLOPES IN THE TOPPING.
17. REFER 9 / S8.3 FOR TYP. LINTEL SCHEDULE
18. **L3** - INDICATES L5x3x3/8 (LLV) CONT W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU / CONC
    L4 - INDICATES L5x3x3/8 (LLV) CONT W/ VERTICAL LEG NOTCHED AS NEEDED W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU / CONC

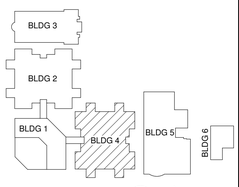
LEGEND:

TE-#	DENOTES THICKENED SLAB EDGE, SEE 33 / S8.1
TB#	DENOTES CONCRETE TIE BEAM TYPE, SEE SCHEDULE ON S7.1 SHEET
J#	MASONRY JAMB, REFER SCHEDULE ON S7.1
BP#	DENOTES BASE PLATE TYPE, SEE SCHEDULE ON 2 / S7.1 SHEET
MC#	DENOTES MASONRY COLUMN, SEE DETAIL ON THIS SHEET
MSW#	MASONRY SHEARWALL, REFER SCHEDULE ON S7.1
#-TC#	CONCRETE COLUMN REFER SCHEDULE ON S7.2
SC#	STEEL COLUMN REFER SCHEDULE ON S7.2
MC#	MASONRY COLUMN REFER DETAIL ON THIS SHEET
### plf	INDICATES ASD DIAPHRAGM SHEAR.

CLASSROOM HATCH LEGEND	
	(MW1) UNO 8" THK MASONRY WALL W/ #5 AT 24" OC
	CONC COLUMN, REFER SCHEDULE S7.2
	MASONRY SHEARWALL



NON-COMPOSITE BEAM LEGEND



KEYPLAN

CODE REVIEW:

CERTIFICATE:

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TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

#	REVISION NAME	DATE
4	ADDENDUM 4	01.27.2025
11	ASI #11	09.29.2025
13	ASI #17	03.06.2026

FOR BID:

DRAWING TITLE:

ROOF FRAMING PLAN- CLASSROOMS 2 - BUILDING 4

COMMISSION NUMBER:

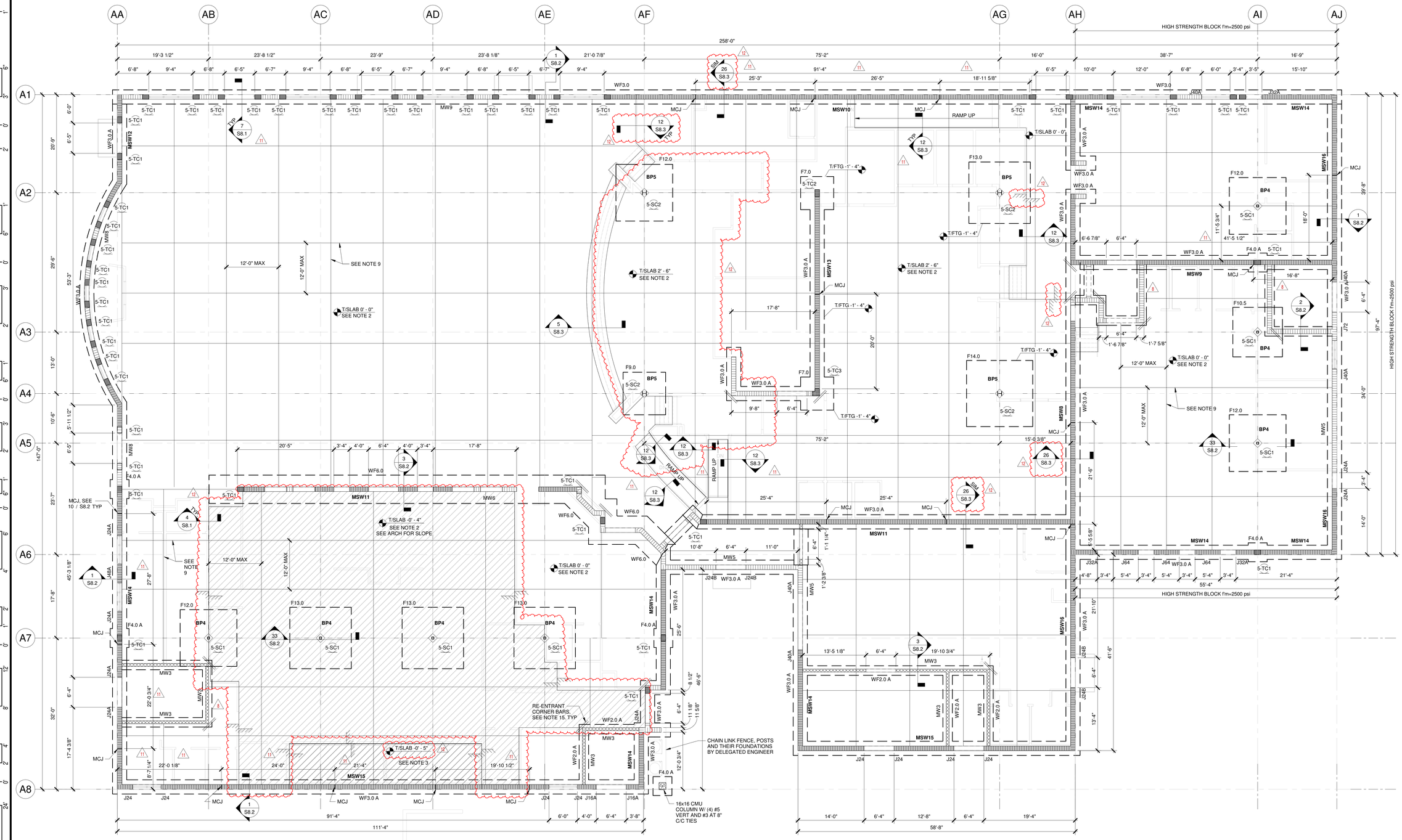
24FTL018

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S4.3

THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"



1 S5.1 FOUNDATION/FIRST FLOOR- AUDETERIA - BUILDING 5
1/8" = 1'-0"

FOUNDATION & GROUND FLOOR PLAN NOTES:

1. T/FIRST FLOOR SLAB = REFERENCE ELEVATION 0'-0" U.N.O.
2. PROVIDE 5" THICK 4,000 PSI NW (145 PCF) CONCRETE SLAB ON GRADE REINF W/ 6x6 W1.4xW1.4 WWR IN SHEETS NOT ROLLS CHAIRED TO TOP OF SLAB, SEE DETAIL 6 / S8.1 . PLACE CONCRETE ON VAPOR BARRIER PER ARCHL REQUIREMENTS BUT NOT LESS THAN 15 MILS. PREPARE SUBGRADE IN ACCORDANCE WITH GEOTECHNICAL REPORT.
3. PROVIDE 7" THICK 4,000 PSI NW CONCRETE SLAB ON GRADE REINF W/ 6x6 W2.4xW2.4 WWR IN SHEETS NOT ROLLS CHAIRED TO TOP OF SLAB, SEE DETAIL 6 / S8.1 . PLACE CONCRETE ON VAPOR BARRIER PER ARCHL REQUIREMENTS BUT NOT LESS THAN 15 MILS. PREPARE SUBGRADE IN ACCORDANCE WITH GEOTECHNICAL REPORT.
4. SEE MEP DRAWINGS FOR SLAB-ON-GRADE PENETRATIONS.
5. TOP OF ALL FOOTINGS SHALL BE -1'-4" U.N.O.
6. CONTINUOUS WALL FOOTINGS SHALL EXTEND FULL LENGTH OF THE FOOTING WIDTH BEYOND END WALLS. U.N.O
7. STEP TOP OF CONTINUOUS WALL FOOTINGS AS NECESSARY FOR PIPES OR CONDUIT. SEE 19 / S8.1
8. FOR TYPICAL CMU WALL DETAILS SEE SCHEDULE ON THIS SHEET FOR DETAILS
9. PROVIDE CONTROL JOINTS PER SECTIONS 2 / S8.1 AND STRUCTURAL NOTES. CONSTRUCTION JOINTS MAY BE PLACED IN LIEU OF CJ AT CONTRACTOR'S DISCRETION, REFER AND SEE 3 / S8.1 .

FOUNDATION & GROUND FLOOR PLAN NOTES:

10. FOR DETAILS OF FOOTINGS AT CORNERS AND INTERSECTIONS SEE 17 / S8.1
11. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION OR FABRICATION. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.
12. CENTERLINES OF COLUMNS AND BEAMS SHALL COINCIDE WITH THE FOUNDATION CENTERLINES, UNO.
13. CONCRETE SLAB-ON-GRADE SHALL BE FORMED ON COMPACTED SOILS. REFER TO GEOTECH REPORT FOR SUBGRADE PREPARATION.
14. FOR CONCRETE STRENGTHS REFER TO STRUCTURAL NOTES.
15. RE-ENTRANT CORNER BARS (TYP), SEE DETAIL 13 / S8.1
16. SEE SHEET S7.1 FOR STRUCTURAL SCHEDULES
17. SEE FOOTING SCHEDULE FOR ISOLATED FOOTINGS AND WALL FOOTING SIZE & REINFORCEMENT ON S7.1 SHEET.
18. SEE ARCH DRAWINGS FOR SLOPE AND FLOOR DRAINS IN SOG
19. REFER 9 / S8.3 FOR TYP. LINTEL SCHEDULE

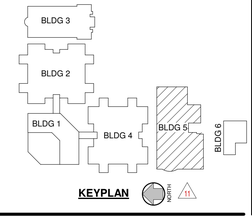
LEGEND:

TE-#	DENOTES THICKENED SLAB EDGE, SEE 33 / S8.1
TB#	DENOTES CONCRETE TIE BEAM TYPE, SEE SCHEDULE ON S7.1 SHEET
J#	MASONRY JAMB, REFER SCHEDULE ON S7.1
BP#	DENOTES BASE PLATE TYPE, SEE SCHEDULE ON 2 / S7.1 SHEET
MC#	DENOTES MASONRY COLUMN, SEE DETAIL ON THIS SHEET
MSW#	MASONRY SHEARWALL, REFER SCHEDULE ON S7.1
#-TC#	CONCRETE COLUMN REFER SCHEDULE ON S7.2
SC#	STEEL COLUMN REFER SCHEDULE ON S7.2
MC#	MASONRY COLUMN REFER DETAIL ON THIS SHEET
### plf	INDICATES ASD DIAPHRAGM SHEAR.

AUDETERIA HATCH LEGEND

	(MW3) 8" THK MASONRY WALL W/ #5 AT 48" OC
	(MW4) UNO 12" THK MASONRY WALL W/ #5 AT 24" OC
	(MW5) 12" THK MASONRY WALL W/ #6 AT 24" OC
	(MW6) 12" THK MASONRY WALL W/ #6 AT 32" OC
	(MW8) 12" THK MASONRY WALL W/ #6 AT 8" OC
	(MW9) 12" THK MASONRY WALL W/ #6 AT 16" OC
	CONC COLUMN, REFER SCHEDULE S7.2
	CONCRETE WALL
	MASONRY SHEARWALL - 12" THK

ALL EXTERIOR WALLS TO BE SOLID GROUTED



KEYPLAN

CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONADIO AR0012456
STEVEN G. SIEGEL AR99546
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CONSULTANTS:

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www.trcww.com

Certificate of Authorization No. 35826

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FILE No. 24FTL018

PERMIT SET 2-13-2025

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FOR

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9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

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8	ASI #8	06.19.2025
11	ASI #11	09.29.2025
12	ASI #14	12.05.2025

FOR BID:

DRAWING TITLE:

FOUNDATION/FIRST FLOOR- AUDETERIA - BUILDING 5

COMMISSION NUMBER:

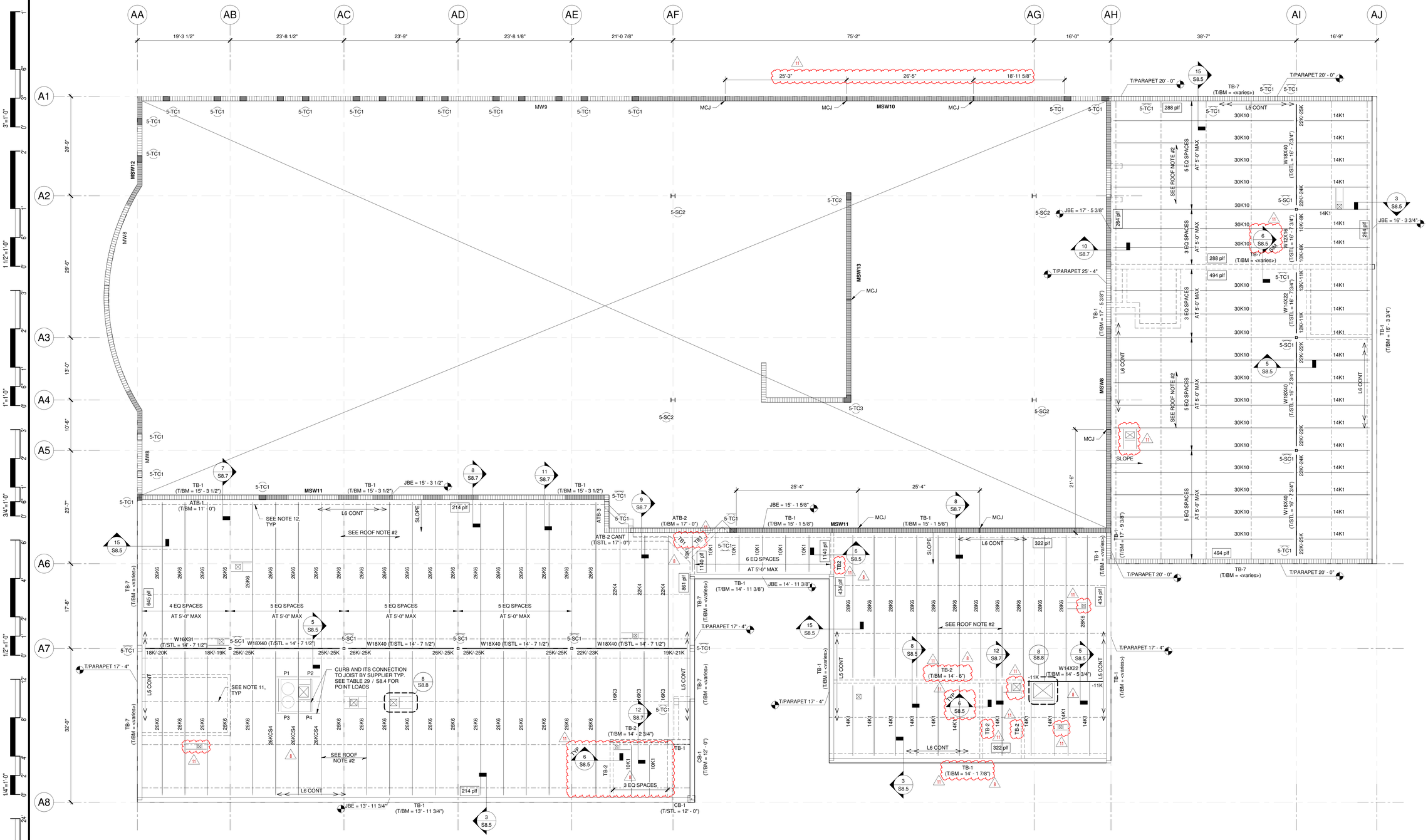
24FTL018

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S5.1

THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"



1 ROOF FRAMING PLAN - AUDETERIA - BUILDING 5

S5.2 — 1/8" = 1'-0"

ROOF FRAMING NOTES:

1. TOP OF STEEL ELEVATION = SEE PLAN.
2. 1-1/2" (20 GAGE) G90 GALV TYPE B ROOF DECK. FASTEN ROOF DECK TO SUPPORTS PER ROOF DECK ATTACHMENT SCHEDULE ON TYPICAL ROOF DETAILS SHEET S8.8. ROOF DECK TO SPAN MINIMUM THREE SPANS.
3. REFER TO STRUCTURAL NOTES AND PROJECT SPECIFICATIONS FOR DEFINITON OF SYMBOLS, ABBREVIATIONS, AND OTHER INFORMATION AND CRITERIA NOT SHOWN ON PLAN.
4. LOADING CRITERIA IS DEFINED ON DRAWINGS. NOTIFY ENGINEER OF SPECIAL ROOF/CEILING LOADING CONDITIONS NOT OTHERWISE IDENTIFIED ON THE CONTRACT DOCUMENTS.
5. PRIOR TO FABRICATION AND CONSTRUCTION, THE CONTRACTOR SHALL VERIFY DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. DISSCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT / ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION.
6. INDICATES SPAN DIRECTION OF STRUCTURAL DECK. SEE NOTES ON THIS SHEET FOR DECK SPECIFICATIONS.
7. TOP OF ROOF AND CEILING SLOPE ELEVATIONS AND LOCATIONS VARY AND SHALL BE COORDINATED WITH THE ARCHITECTURAL DRAWINGS PRIOR TO FABRICATION.
8. FRAMING SHALL BE EQUALLY SPACED BETWEEN COLUMN BAYS UNLESS NOTED OTHERWISE OR DIMENSIONED ON PLAN.

ROOF FRAMING NOTES:

9. COORDINATE FINAL SIZE AND LOCATION OF ROOF OPENINGS, PENETRATIONS, AND CHASES WITH ARCHITECTURAL AND MEP/FP DRAWINGS.
10. DIMENSIONS TO ROOF TOP EQUIPMENT ARE APPROXIMATE AND SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO FABRICATION AND UPON FINAL SELECTION OF EQUIPMENT AND CURBING.
11. HORIZONTAL BRIDGING PER JOIST MFR MIN AS SHOWN ON PLAN, TYP.
12. CROSS BRIDGING IN SPACE ADJACENT TO WALL BY JOIST MFR, TYP.
13. UPLIFT BRIDGING PER JOIST MFR MIN AS SHOWN ON PLAN, TYP.
14. BRIDGING BY JOIST MFR UNO, SEE 35 / S8.6 FOR DETAILS.
15. DENOTES DECK SPAN DIRECTION.
16. DENOTES MOMENT CONNECTION. DESIGN MOMENT CONNECTION FOR FULL MOMENT AND SHEAR CAPACITY OF BEAM.
17. REFER 9 / S8.3 FOR TYP. LINTEL SCHEDULE
18. **L5** - INDICATES L5x3x3/8 (LLV) CONT W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU / CONC
    L6 - INDICATES L5x3x3/8 (LLV) W/ VERTICAL LEG NOTCHED AS NEEDED CONT W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU / CONC

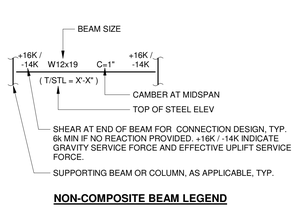
LEGEND:

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TB#	DENOTES CONCRETE TIE BEAM TYPE, SEE SCHEDULE ON S7.1 SHEET
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BP#	DENOTES BASE PLATE TYPE, SEE SCHEDULE ON 2 / S7.1 SHEET
MC#	DENOTES MASONRY COLUMN, SEE DETAIL ON THIS SHEET
MSW#	MASONRY SHEARWALL, REFER SCHEDULE ON S7.1
#-TC#	CONCRETE COLUMN REFER SCHEDULE ON S7.2
SC#	STEEL COLUMN REFER SCHEDULE ON S7.2
MC#	MASONRY COLUMN REFER DETAIL ON THIS SHEET
### plf	INDICATES ASD DIAPHRAGM SHEAR.

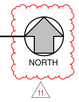
AUDETERIA HATCH LEGEND

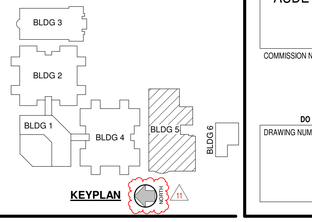
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	(MW9) 12" THK MASONRY WALL W/ #6 AT 16" OC
	CONC COLUMN, REFER SCHEDULE S7.2
	CONCRETE WALL
	MASONRY SHEARWALL - 12" THK

ALL EXTERIOR WALLS TO BE SOLID GROUTED



NON-COMPOSITE BEAM LEGEND





KEYPLAN

CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:

ANTHONY DONADO AR0012456
STEVEN G. SIEGEL AR99546
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FOR BID:

DRAWING TITLE:

ROOF FRAMING PLAN - AUDETERIA - BUILDING 5

COMMISSION NUMBER:

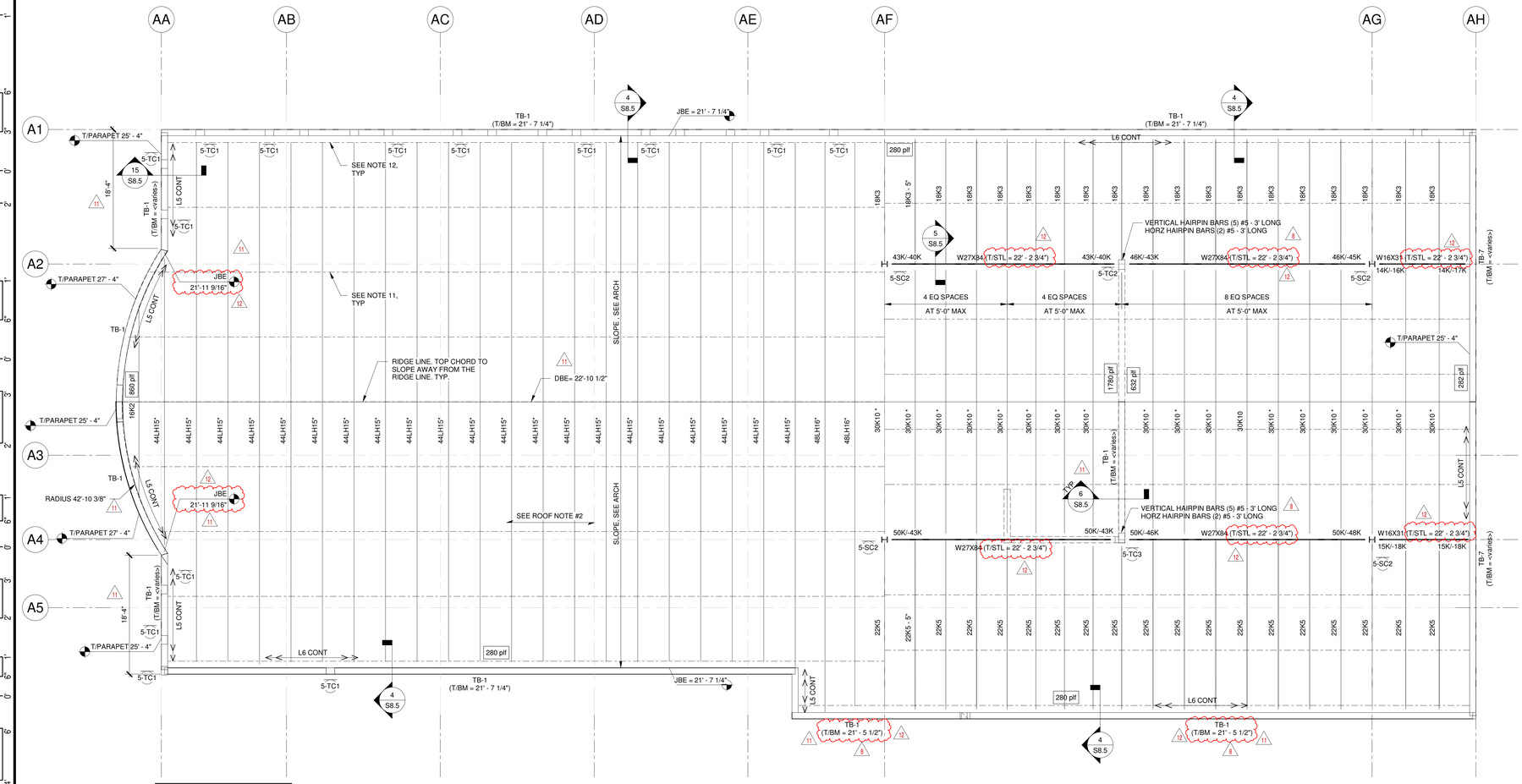
24FTL018

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S5.2

THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"



1 S5.3 HIGH ROOF FRAMING PLAN - AUDETERIA - BUILDING 5
1/8" = 1'-0"

NORTH

* TOP CHORD DBL PITCHED JOISTS (UNDERLUG)

ROOF FRAMING NOTES:
1. TOP OF STEEL ELEVATION = SEE PLAN.
2. 1-1/2" (20 GAGE) G90 GALV TYPE B ROOF DECK. FASTEN ROOF DECK TO SUPPORTS PER ROOF DECK ATTACHMENT SCHEDULE ON TYPICAL ROOF DETAILS SHEET S8.8. ROOF DECK TO SPAN MINIMUM THREE SPANS.
3. REFER TO STRUCTURAL NOTES AND PROJECT SPECIFICATIONS FOR DEFINITON OF SYMBOLS, ABBREVIATIONS, AND OTHER INFORMATION AND CRITERIA NOT SHOWN ON PLAN.
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6. INDICATES SPAN DIRECTION OF STRUCTURAL DECK. SEE NOTES ON THIS SHEET FOR DECK SPECIFICATIONS.
7. TOP OF ROOF AND CEILING SLOPE ELEVATIONS AND LOCATIONS VARY AND SHALL BE COORDINATED WITH THE ARCHITECTURAL DRAWINGS PRIOR TO FABRICATION.
8. FRAMING SHALL BE EQUALLY SPACED BETWEEN COLUMN BAYS UNLESS NOTED OTHERWISE OR DIMENSIONED ON PLAN.

ROOF FRAMING NOTES:
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10. DIMENSIONS TO ROOF TOP EQUIPMENT ARE APPROXIMATE AND SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO FABRICATION AND UPON FINAL SELECTION OF EQUIPMENT AND CURBING.
11. HORIZONTAL BRIDGING PER JOIST MFR MIN AS SHOWN ON PLAN, TYP.
12. CROSS BRIDGING IN SPACE ADJACENT TO WALL BY JOIST MFR, TYP.
13. UPLIFT BRIDGING PER JOIST MFR MIN AS SHOWN ON PLAN, TYP.
14. BRIDGING BY JOIST MFR UNO, SEE 35 / S8.6 FOR DETAILS.
15. DENOTES DECK SPAN DIRECTION.
16. DENOTES MOMENT CONNECTION. DESIGN MOMENT CONNECTION FOR FULL MOMENT AND SHEAR CAPACITY OF BEAM.
17. REFER 9 / S8.3 FOR TYP. LINTEL SCHEDULE
18. **L5** - INDICATES L5x3x3/8 (LLV) CONT W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU/ CONC
11 **L6** - INDICATES L5x3x3/8 (LLV) W/ VERTICAL LEG NOTCHED AS NEEDED CONT W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU / CONC

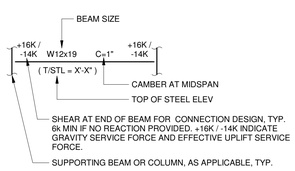
LEGEND:

TE-#	DENOTES THICKENED SLAB EDGE, SEE 33 / S8.1
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BP#	DENOTES BASE PLATE TYPE, SEE SCHEDULE ON 2 / S7.1 SHEET
MC#	DENOTES MASONRY COLUMN, SEE DETAIL ON THIS SHEET
MSW#	MASONRY SHEARWALL, REFER SCHEDULE ON S7.1
#-TC#	CONCRETE COLUMN REFER SCHEDULE ON S7.2
SC#	STEEL COLUMN REFER SCHEDULE ON S7.2
MC#	MASONRY COLUMN REFER DETAIL ON THIS SHEET
### plf	INDICATES ASD DIAPHRAGM SHEAR.

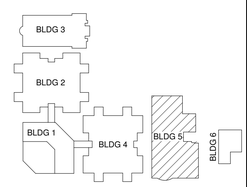
AUDETERIA HATCH LEGEND

	(MW3) 8" THK MASONRY WALL W/ #5 AT 48" OC
	(MW4) UNO 12" THK MASONRY WALL W/ #5 AT 24" OC
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	CONC COLUMN, REFER SCHEDULE S7.2
	CONCRETE WALL
	MASONRY SHEARWALL - 12" THK

ALL EXTERIOR WALLS TO BE SOLID GROUTED



NON-COMPOSITE BEAM LEGEND



KEYPLAN

CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONADO AR0012456
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JOHN F. WRIGHT AR103341

SEAL:

CONSULTANTS:

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FOR BID:

DRAWING TITLE:

HIGH ROOF FRAMING PLAN - AUDETERIA - BUILDING 5

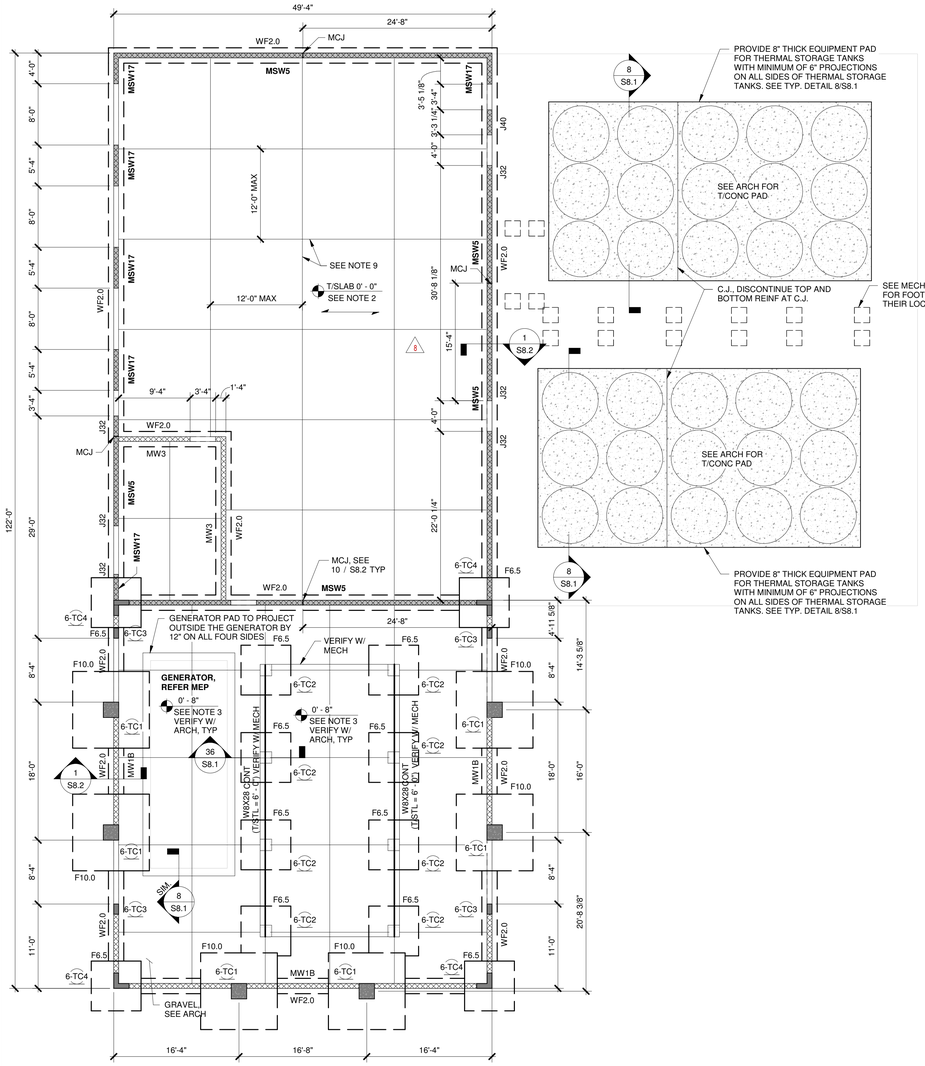
COMMISSION NUMBER:
24FTL018

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:
S5.3

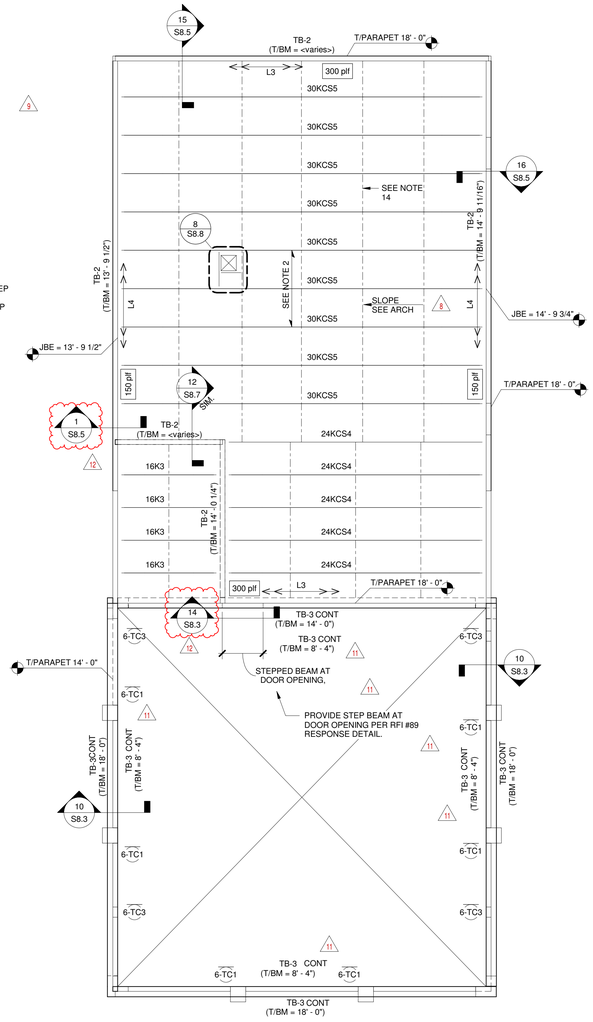
THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"

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2025-12-05 12:25:50
Autodesk Docs://24K054 - St Lucie WG K8/Western Grove K8 - SLPS_R24.rvt



1 FOUNDATION/FIRST FLOOR- CHILLER - BUILDING 6
S6.1 1/8" = 1'-0"

NORTH



2 ROOF FRAMING PLAN - CHILLER - BUILDING 6
S6.1 1/8" = 1'-0"

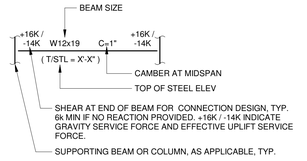
NORTH

HATCH LEGEND (CHILLER BLDG)

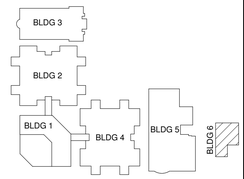
	(MW1) 8" THK MASONRY WALL W/ #5 AT 24" OC
	(MW1B) 8" THK MASONRY WALL W/ #5 AT 16" OC
	(MW3) UNO, 8" THK MASONRY WALL W/ #5 AT 48" OC
	CONC COLUMN, REFER SCHEDULE S7.2
	MASONRY SHEARWALL (MSW)

LEGEND:

TE-#	DENOTES THICKENED SLAB EDGE, SEE 33 / S8.1
TB#	DENOTES CONCRETE TIE BEAM TYPE, SEE SCHEDULE ON S7.1 SHEET
J#	MASONRY JAMB, REFER SCHEDULE ON S7.1
BP#	DENOTES BASE PLATE TYPE, SEE SCHEDULE ON 2 / S7.1 SHEET
MC#	DENOTES MASONRY COLUMN, SEE DETAIL ON THIS SHEET
MSW#	MASONRY SHEARWALL, REFER SCHEDULE ON S7.1
#-TC#	CONCRETE COLUMN REFER SCHEDULE ON S7.2
SC#	STEEL COLUMN REFER SCHEDULE ON S7.2
MC#	MASONRY COLUMN REFER DETAIL ON THIS SHEET
### plf	INDICATES ASD DIAPHRAGM SHEAR.



NON-COMPOSITE BEAM LEGEND



KEYPLAN

FOUNDATION & GROUND FLOOR PLAN NOTES:

1. T/FIRST FLOOR SLAB = REFERENCE ELEVATION 0'-0" U.N.O. SEE ARCH FOR SLOPES 3
2. PROVIDE 6" THICK SLAB ON GRADE REINF W/ 6x6 W2.1xW2.1 WWR IN SHEETS NOT ROLLS CHAIRED TO TOP OF SLAB, SEE DETAIL 6 / S8.1 . PLACE CONCRETE ON VAPOR BARRIER PER ARCHL REQUIREMENTS BUT NOT LESS THAN 15 MILS. PREPARE SUBGRADE IN ACCORDANCE WITH GEOTECHNICAL REPORT.
3. PROVIDE 12" THICK SLAB ON GRADE REINF W/ #5@12" OC TOP & BOTTOM, SEE DETAIL 8 / S8.1, PLACE CONCRETE ON VAPOR BARRIER PER ARCHL REQUIREMENTS BUT NOT LESS THAN 15 MILS. PREPARE SUBGRADE IN ACCORDANCE WITH GEOTECHNICAL REPORT.
4. SEE MEP DRAWINGS FOR SLAB-ON-GRADE PENETRATIONS.
5. TOP OF ALL FOOTINGS SHALL BE -1'-4" U.N.O.
6. CONTINUOUS WALL FOOTINGS SHALL EXTEND LENGTH WISE 1/2 THE FOOTING WIDTH BEYOND END WALLS.
7. STEP TOP OF CONTINUOUS WALL FOOTINGS AS NECESSARY FOR PIPES OR CONDUIT. SEE 19 / S8.1
8. FOR TYPICAL CMU WALL DETAILS SEE SCHEDULE ON THIS SHEET FOR DETAILS
9. PROVIDE CONTROL JOINTS PER SECTIONS 2 / S8.1 AND STRUCTURAL NOTES, CONSTRUCTION JOINTS MAY BE PLACED IN LIEU OF CJ AT CONTRACTOR'S DISCRETION, REFER AND SEE 3 / S8.1 .
10. FOR DETAILS OF FOOTINGS AT CORNERS AND INTERSECTIONS SEE 17 / S8.1 .
11. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION OR FABRICATION. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.
12. CENTERLINES OF COLUMNS AND BEAMS SHALL COINCIDE WITH THE FOUNDATION CENTERLINES, UNO.
13. CONCRETE SLAB-ON-GRADE SHALL BE FORMED ON COMPACTED SOILS. REFER TO GEOTECH REPORT FOR SUBGRADE PREPARATION.
14. FOR CONCRETE STRENGTHS REFER TO STRUCTURAL NOTES.
15. RE-ENTRANT CORNER BARS (TYP), SEE DETAIL 13 / S8.1
16. SEE SHEET S7.1 FOR STRUCTURAL SCHEDULES
17. SEE FOOTING SCHEDULE FOR ISOLATED FOOTINGS AND WALL FOOTING SIZE & REINFORCEMENT ON S7.1 SHEET.
18. SEE STRUCUTRAL NOTES AND GEOTECH REPORT FOR SOIL IMPROVEMENT.

ROOF FRAMING NOTES:

1. TOP OF STEEL ELEVATION = SEE PLAN.
2. 1-1/2" (20 GAGE) G90 GALV TYPE B ROOF DECK. FASTEN ROOF DECK TO SUPPORTS PER ROOF DECK ATTACHMENT SCHEDULE ON TYPICAL ROOF DETAILS SHEET S8.8
3. REFER TO STRUCTURAL NOTES AND PROJECT SPECIFICATIONS FOR DEFINITON OF SYMBOLS, ABBREVIATIONS, AND OTHER INFORMATION AND CRITERIA NOT SHOWN ON PLAN.
4. LOADING CRITERIA IS DEFINED ON DRAWINGS. NOTIFY ENGINEER OF SPECIAL ROOF/CEILING LOADING CONDITIONS NOT OTHERWISE IDENTIFIED ON THE CONTRACT DOCUMENTS.
5. PRIOR TO FABRICATION AND CONSTRUCTION, THE CONTRACTOR SHALL VERIFY DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT / ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION.
6. INDICATES SPAN DIRECTION OF STRUCTURAL DECK. SEE NOTES ON THIS SHEET FOR DECK SPECIFICATIONS.
7. TOP OF ROOF AND CEILING SLOPE ELEVATIONS AND LOCATIONS VARY AND SHALL BE COORDINATED WITH THE ARCHITECTURAL DRAWINGS PRIOR TO FABRICATION.
8. FRAMING SHALL BE EQUALLY SPACED BETWEEN COLUMN BAYS UNLESS NOTED OTHERWISE OR DIMENSIONED ON PLAN.
9. COORDINATE FINAL SIZE AND LOCATION OF ROOF OPENINGS, PENETRATIONS, AND CHASES WITH ARCHITECTURAL AND MEP/FP DRAWINGS.
10. DIMENSIONS TO ROOF TOP EQUIPMENT ARE APPROXIMATE AND SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO FABRICATION AND UPON FINAL SELECTION OF EQUIPMENT AND CURBING.
11. HORIZONTAL BRIDGING PER JOIST MFR MIN AS SHOWN ON PLAN, TYP.
12. CROSS BRIDGING IN SPACE ADJACENT TO WALL BY JOIST MFR, TYP.
13. UPLIFT BRIDGING PER JOIST MFR MIN AS SHOWN ON PLAN, TYP.
14. BRIDGING BY JOIST MFR UNO, SEE 35 / S8.6 FOR DETAILS.
15. DENOTES DECK SPAN DIRECTION.
16. DENOTES MOMENT CONNECTION. DESIGN MOMENT CONNECTION FOR FULL MOMENT AND SHEAR CAPACITY OF BEAM.
17. **L3** - INDICATES L5x3x3/8 (LLV) CONT W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC
L4 - INDICATES L5x3x3/8 (LLV) CONT W/ VERTICAL LEG NOTCHED AS NEEDED W/ 5/8" DIA x 4" EMBEDMENT LENGTH TITEN HD ANCHORS BY SIMPSON STRONGTIE @12" OC IN FULLY GROUTED CMU / CONC

CODE REVIEW:

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FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

△	REVISION NAME	DATE
4	ADDENDUM 4	01.27.2025
8	ASI #8	06.19.2025
9	ASI #9	08.26.2025
11	ASI #11	09.29.2025
12	ASI #14	12.05.2025

FOR BID:

DRAWING TITLE:

FOUNDATION/FIRST FLOOR AND ROOF - CHILLER - BUILDING 6

COMMISSION NUMBER:
24FTL018

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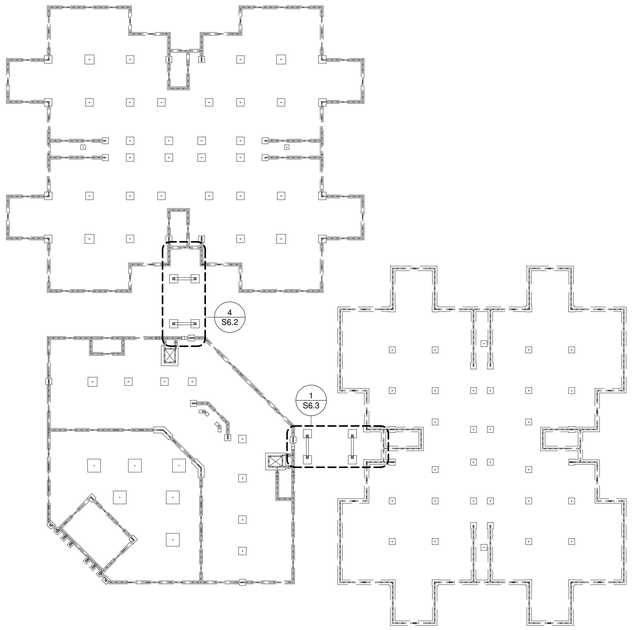
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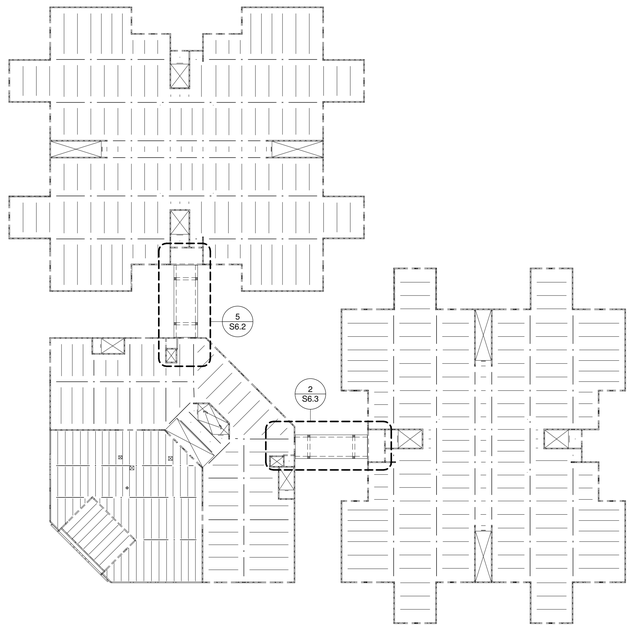
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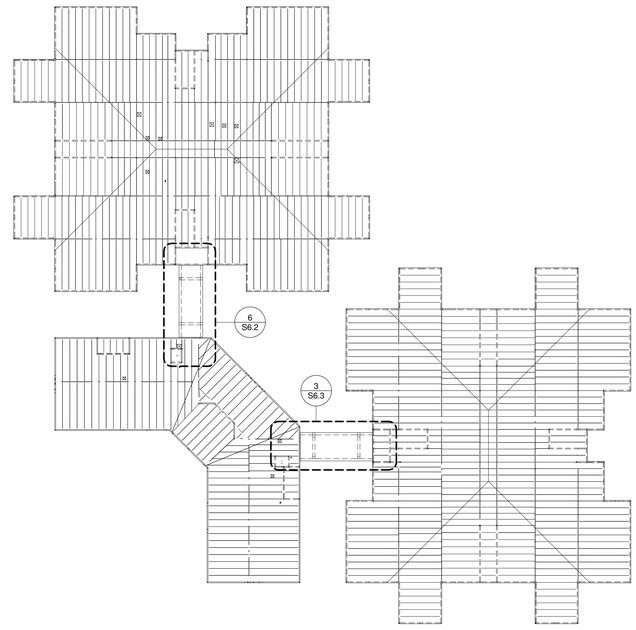
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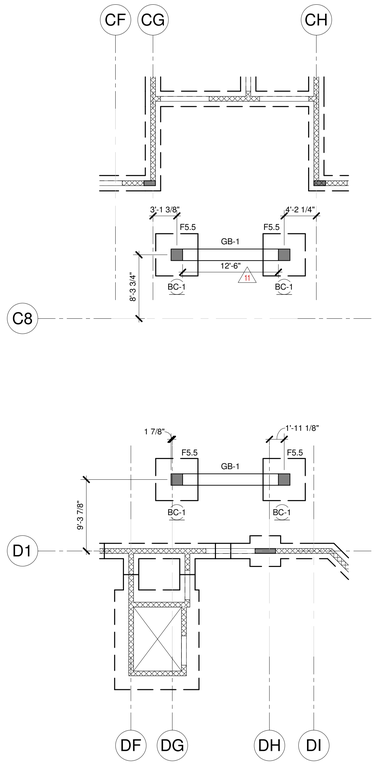
1 S6.2 OVERALL BRIDGE FOUNDATION PLAN
1" = 40'-0"



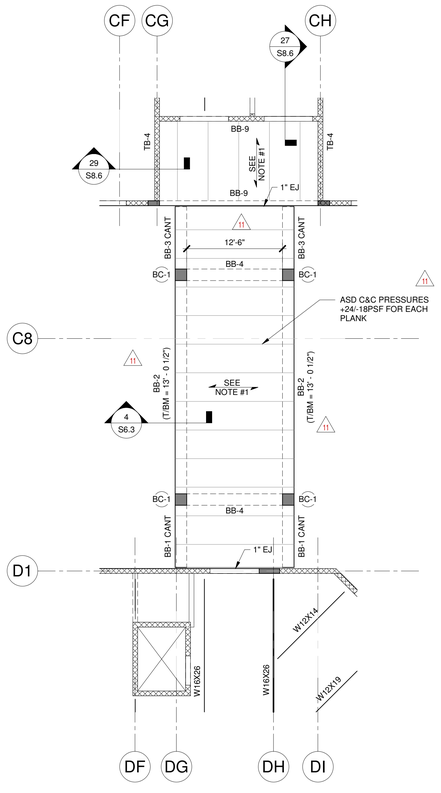
2 S6.2 OVERALL BRIDGE PLAN LEVEL - 2
1" = 40'-0"



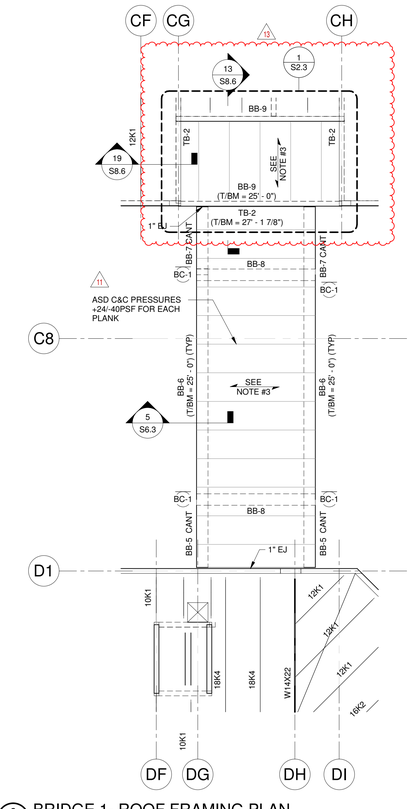
3 S6.2 OVERALL BRIDGE ROOF PLAN
1" = 40'-0"



4 S6.2 BRIDGE-1 FOUNDATION PLAN
1/8" = 1'-0"

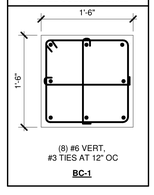


5 S6.2 BRIDGE-1 - LVL 2 FRAMING PLAN
1/8" = 1'-0"



6 S6.2 BRIDGE 1- ROOF FRAMING PLAN
1/8" = 1'-0"

BRIDGE COLUMN DETAIL



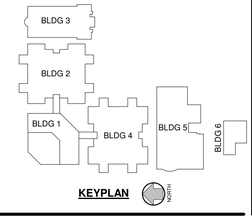
BRIDGE FOUNDATION PLAN NOTES:
1. STRUCUTRAL NOTES AND GEOTECH REPORT FOR SOIL IMPROVEMENT.

BRIDGE BASE PLAN NOTES:
1. THE DECK SHALL BE PRECAST 8" HOLLOW CORE SPANNING AS SHOWN ON THE PLAN W/ 2" MIN. CONCRETE TOPPING REINF W/ 6X6-W2.1XW2.1 WIRE MESH REINFORCEMENT (USE FLAT SHEETS NOT IN ROLLS) AT TOP OF THE TOPPING. SEE ARCH FOR SLOPES IN THE TOPPING.
2. SEE ARCH FOR ELEVATION, FINISHES ETC.

BRIDGE ROOF PLAN NOTES:
3. THE DECK SHALL BE PRECAST 6" HOLLOW CORE SPANNING AS SHOWN ON THE PLAN W/ 2" MIN. CONCRETE TOPPING REINF W/ 6X6-W2.1XW2.1 WIRE MESH REINFORCEMENT (USE FLAT SHEETS NOT IN ROLLS) AT TOP OF THE TOPPING. SEE ARCH FOR SLOPES IN THE TOPPING.
4. SEE ARCH FOR ELEVATION, W.P. AND FINISHES.

BRIDGE BEAM SCHEDULE

MARK	SIZE WxD (IN)	BOTTOM REINFORCING	TOP REINFORCING	'M' (# of Bars EF) REINFORCING	'C' REINFORCING	'E' REINFORCING	STIRRUP S1	STIRRUP S2	COMMENTS
BB-1 CANT	18 x 24	(4) #5	(4) #5	-	(4) #5	-	2L - #4 AT 12" OC	2L - #4 AT 12" OC	
BB-2	18 x 24	(4) #5 + (4) #5*	(4) #5	-	(4) #5	-	2L - #4 AT 18" OC	2L - #4 AT 18" OC	
BB-3 CANT	18 x 24	(4) #5	(4) #5	-	(4) #5	-	2L - #4 AT 12" OC	2L - #4 AT 12" OC	
BB-4	18 x18	(4) #6	(4) #6	-	-	-	2L - #4 AT 18" OC	2L - #4 AT 18" OC	
BB-5 CANT	18 x 24	(4) #5	(4) #5	-	(4) #5	-	2L - #4 AT 12" OC	2L - #4 AT 12" OC	
BB-6	18 x 24	(4) #5 + (4) #5*	(4) #5	-	(4) #5	-	2L - #4 AT 18" OC	2L - #4 AT 18" OC	
BB-7 CANT	18 x 24	(4) #5	(4) #5	-	(4) #5	-	2L - #4 AT 18" OC	2L - #4 AT 18" OC	
BB-8	18 x18	(4) #6	(4) #6	-	-	-	2L - #4 AT 12" OC	2L - #4 AT 12" OC	
BB-9	7 5/8 x 24	(2) #7	(2) #7	-	-	-	2L - #4 AT 12" OC	2L - #4 AT 12" OC	
GB-1	18 x 18	(4) #6	(4) #6	-	-	-	2L - #4 AT 18" OC	2L - #4 AT 18" OC	



KEYPLAN

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FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

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Δ	REVISION NAME	DATE
11	ASI #11	09.29.2025
13	ASI #17	03.06.2026

FOR BID:

DRAWING TITLE:

BRIDGE OVERALL PLAN

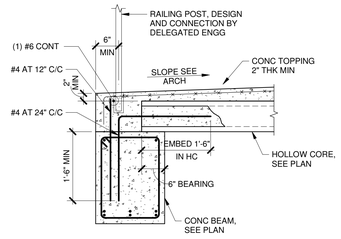
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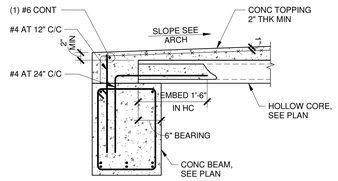
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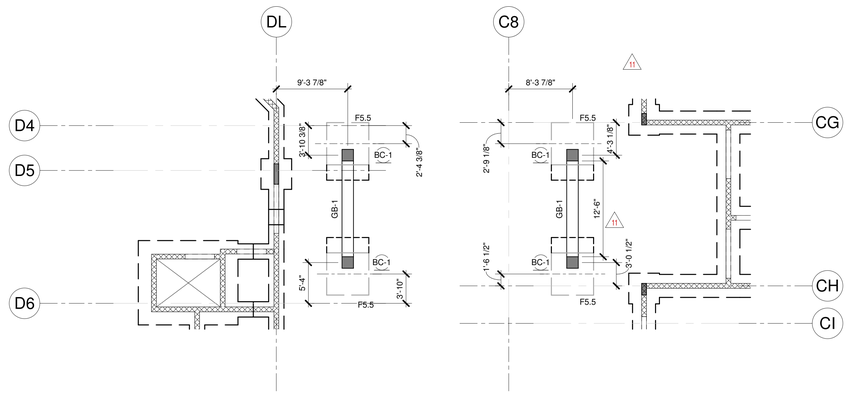
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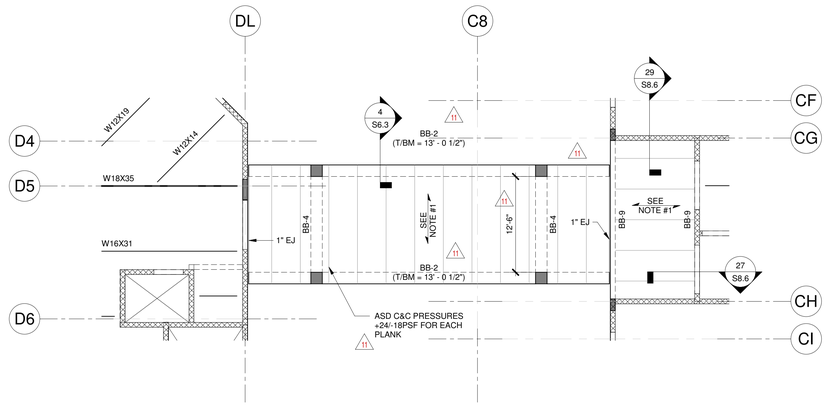
4 S6.3 SECTION
3/4" = 1'-0"



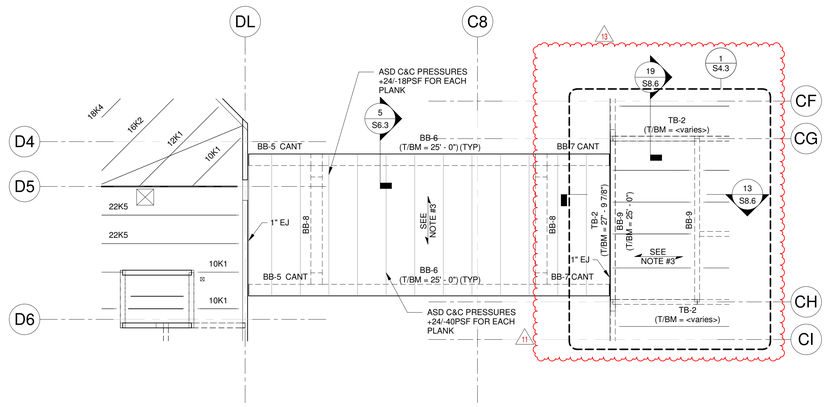
5 S6.3 SECTION
3/4" = 1'-0"



1 S6.3 BRIDGE-2- FOUNDATION PLAN
1/8" = 1'-0"

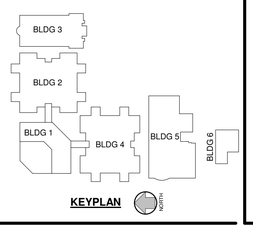


2 S6.3 BRIDGE-2- LVL 1 FRAMING PLAN
1/8" = 1'-0"



3 S6.3 BRIDGE-2- ROOF FRAMING PLAN
1/8" = 1'-0"

13
NOTE: SEE S6.2 FOR NOTES AND BEAM SCHEDULE.



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FOR BID:

DRAWING TITLE:

BRIDGE OVERALL PLAN

COMMISSION NUMBER:
24FTL018

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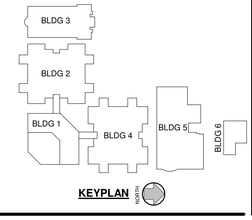
S6.3

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1 S6.4 WALKWAY CANOPY OVERALL PLAN
3/64" = 1'-0"



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CN=Nitin R Ranadive, dnQualifier=A01410D0000018DC34549AA0004BD39, O=Florida, C=US
Nitin Ranadive, P.E., No.81459
TRC Worldwide Engineering, dba Jenkins and Charland, INC.
2025.02.12 11:10:44-05'00'

NITIN RANADIVE LICENSE No. 81459 STATE OF FLORIDA PROFESSIONAL ENGINEER

This item has been electronically signed and sealed by Nitin Ranadive, PE on the date adjacent to the seal.

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CONSULTANTS:

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REVISIONS:

	REVISION NAME	DATE
1		

FOR BID:

DRAWING TITLE:

WALKWAY CANOPY PLAN

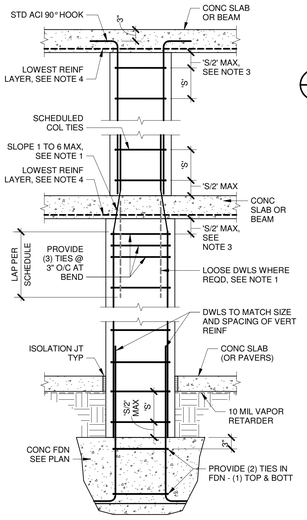
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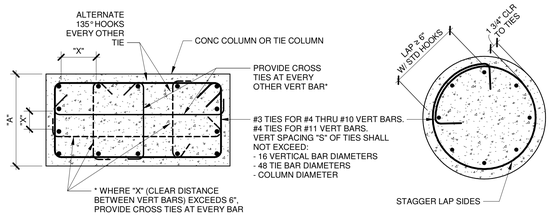
S6.4

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NOTES:
1. WHERE COLUMN FACE ABOVE IS OFFSET 3" OR GREATER, VERTICAL BARS SHALL NOT BE OFFSET BENT, LOOSE DOWELS SHALL BE PROVIDED.
2. 'S' DENOTES SCHEDULED COLUMN TIE SPACING, SEE COLUMN SCHEDULE. 'S/2' DENOTES 1/2 THE SCHEDULED COLUMN TIE SPACING.
3. WHERE BEAMS FRAME FROM FOUR DIRECTIONS, TERMINATE TIES NO MORE THAN 3" BELOW THE LOWEST HORIZONTAL REINFORCEMENT IN THE SHALLOWEST BEAM.
4. ASSUME 1" ABOVE SLAB OFFSET FOR PT CONSTRUCTION WHERE NO BOTTOM BARS ARE PRESENT.

5 S7.1 CONCRETE COLUMN OR TIE COLUMN DETAIL
3/4" = 1'-0"



NOTE: DISTRIBUTE VERTICAL BARS UNIFORMLY AROUND COLUMN PERIMETER EXCEPT WHERE "A" IS LESS THAN 12". WHERE "A" IS LESS THAN 12" DO NOT PROVIDE INTERMEDIATE VERTICAL BARS ON SHORT COLUMN FACES.

1 S7.1 COLUMN REINFORCING DETAILS
1" = 1'-0"

MASONRY JAMB SCHEDULE	
J16	(2)#5 V AT 8" OC
J16A	(2)#6 V AT 8" OC
J24	(3)#5 V AT 8" OC
J24A	(3)#6 V AT 8" OC
J24B	(6)#6 V AT 8" OC
J32	(4)#5 V AT 8" OC
J32A	(4)#6 V AT 8" OC
J40	(5)#5 V AT 8" OC
J40A	(5)#6 V AT 8" OC
J48A	(6)#6 V AT 8" OC
J64	(8)#5 V AT 8" OC
J68	(8)#5 V AT 8" OC
J72	(9)#5 V AT 8" OC

MASONRY SHEAR WALL (MSW) SCHEDULE		
WALL TAG	VERT. REINF	NO. OF CHORD CELLS AT EACH END
MSW1 (8")	#5 AT 24" OC	(1) #5
MSW2 (8")	#5 AT 40" OC	(3) #5
MSW3 (8")	#5 AT 24" OC	(2) #5
MSW4 (8")	#5 AT 24" OC	(5) #5
MSW5 (8")	#5 AT 24" OC	(1) #5
MSW6 (8")	#5 AT 24" OC	(3) #5
MSW7 (8")	#5 AT 24" OC	(2) #5
MSW8 (12")	#5 AT 24" OC	(3) #5
MSW9 (12")	#5 AT 24" OC	(2) #5
MSW10 (12")	#6 AT 16" OC	(2) #6
MSW11 (12")	#6 AT 32" OC	(3) #6
MSW12 (12")	#6 AT 8" OC	(5) #8
MSW13 (12")	#5 AT 24" OC	(5) #6
MSW14 (12")	#6 AT 24" OC	(2) #6
MSW15 (12")	#6 AT 32" OC	(1) #6
MSW16 (12")	#6 AT 24" OC	(1) #6
MSW17 (8")	#5 AT 8" OC	(1) #5

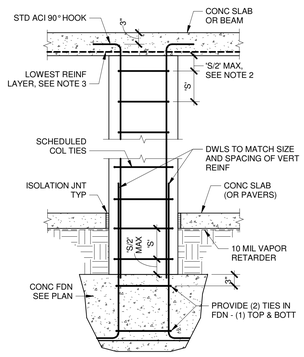
FOOTING SCHEDULE

MARK	LENGTH	FTG WIDTH	THICKNESS	BOTTOM REINFORCING LW	BOTTOM REINFORCING SW	TOP REINFORCING LW	TOP REINFORCING SW	COMMENTS
F3.0	3'-0"	3'-0"	1'-0"	(5)#5	(5)#5	-	-	
F4.0	4'-0"	4'-0"	1'-6	(6)#5	(6)#5	-	-	
F4.0 A	4'-0"	4'-0"	1'-6	(6)#5	(6)#5	(6)#5	(6)#5	
F5.0	5'-0"	5'-0"	1'-6	(6)#6	(6)#6	-	-	
F5.5	5'-6"	5'-6"	2'-0"	(7)#6	(7)#6	(7)#6	(7)#6	
F5x3	5'-0"	3'-0"	1'-6	(4) #6	(6) #6	(4) #6	(6) #6	
F6.0	6'-0"	6'-0"	2'-0"	(7)#6	(7)#6	-	-	
F6.5	6'-6"	6'-6"	1'-8"	(8)#6	(8)#6	(8)#5	(8)#5	
F6.5 A	6'-6"	6'-6"	2'-0"	(8)#6	(8)#6	(8)#6	(8)#6	
F6.5x10.5	10'-0"	6'-6"	2'-0"	(8)#6	(13)#6	(8)#6	(13)#6	
F7.0	7'-0"	7'-0"	1'-6	(8)#5	(8)#5	(8)#5	(8)#5	
F8.5	8'-6"	8'-6"	2'-0	(11) #6	(11) #6	(10) #6	(10) #6	
F9.0	9'-0"	9'-0"	2'-6	(10)#7	(10)#7	(10)#7	(10)#7	
F10.0	10'-0"	10'-0"	2'-0	(11) #6	(11) #6	(11) #6	(11) #6	
F10.5	10'-6"	10'-6"	2'-6	(12)#7	(12)#7	(12)#7	(12)#7	
F11x13	11'-0"	13'-0"	1'-6"	#6 @ 10" OC	#6 @ 10" OC	#6 @ 10" OC	#6 @ 10" OC	
F12.0	12'-0"	12'-0"	2'-6	(13)#7	(13)#7	(13)#7	(13)#7	
F13.0	13'-0"	13'-0"	2'-6	(15)#7	(15)#7	(15)#7	(15)#7	
F14.0	14'-0"	14'-0"	2'-6	(16)#7	(16)#7	(16)#7	(16)#7	
WF2.0	CONT	2'-0"	1'-6"	(3) #5	#5 @ 9" OC	(3) #5	#5 @ 16" OC	
WF2.0 A	CONT	2'-0"	1'-6"	(3) #5	#5 @ 9" OC	(3) #5	#5 @ 9" OC	
WF3.0	CONT	3'-0"	2'-0"	(4) #5	#5 @ 9" OC	(4) #5	#6 @ 16" OC	
WF3.0 A	CONT	3'-0"	1'-6"	(4) #5	#5 @ 9" OC	(4) #5	#5 @ 9" OC	
WF4.0	CONT	4'-0"	1'-6"	(5)#5	#5 @ 9" OC	(5)#5	#5 @ 9" OC	
WF6.0	CONT	6'-0"	2'-0"	(7) #6	#6 @ 10" OC	(7) #6	#6 @ 10" OC	

CONCRETE BEAM SCHEDULE

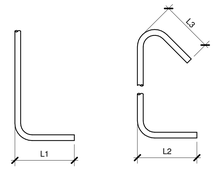
MARK	SIZE WxD (IN)	BOTTOM REINFORCING	TOP REINFORCING	'M' (# of Bars EF) REINFORCING	'C' REINFORCING	'E' REINFORCING	STIRRUP S1	STIRRUP S2	COMMENTS
ATB-1	12 x 36	(3) #9 + (3) #9*	(3) #9				2L - #4 AT 6" OC	2L - #4 AT 12" OC	* SECOND LAYER
ATB-2	12 x 36	(3) #9 + (3) #9*	(3) #9 + (3) #9*				2L - #4 AT 6" OC	2L - #4 AT 12" OC	* SECOND LAYER
ATB-2 CANT	12 x 36	(3) #9 + (3) #9*	(3) #9 + (3) #9*	-	-	-	2L - #5 AT 5" OC	2L - #5 AT 5" OC	
ATB-3	12 x 24	(3) #9	(3) #9	-	-	-	2L - #4 AT 18" OC	2L - #4 AT 18" OC	
ATB-4	8 x 24	(2) #6 + (2) #6*	(2) #6	-	-	-	2L - #4 AT 8" OC	2L - #4 AT 8" OC	* SECOND LAYER
CB-1	12 x 24	(3) #9	(3) #9 + (3) #9*	-	-	-	2L - #5 AT 5" OC	2L - #5 AT 5" OC	
TB-1	12 x 16	(3) #6	(3) #6	-	-	-	#3 AT 8" OC	#3 AT 8" OC	HOOK TOP AND BOTTOM BARS
TB-2	8 x 16	(2) #6	(2) #6	-	-	-	#3 AT 8" OC	#3 AT 8" OC	HOOK TOP AND BOTTOM BARS
TB-3	16 x 16	(3) #6	(3) #6	-	-	-	#3 AT 6" OC	#3 AT 12" OC	(1) #5 FACE REINF ON EA FACE. ALL LONG REINF TO BE CONT OR PROVIDE CLASS B STAGGERED SPLICE. HOOK TOP AND BOTTOM BARS
TB-4	8 x 24	(2) #6	(2) #6	-	-	-	#3 AT 8" OC	#3 AT 8" OC	HOOK TOP AND BOTTOM BARS
TB-5	12 x 40	(3) #6	(3) #6	-	-	-	#3 AT 6" OC	#3 AT 12" OC	HOOK TOP AND BOTTOM BARS
TB-6	44 x 16	(7) #6	(7) #6	-	-	-	#5 AT 6" OC	#5 AT 6" OC	HOOK TOP AND BOTTOM BARS
TB-7	12 x 24	(3) #6	(3) #6	-	-	-	#3 AT 6" OC	#3 AT 12" OC	<varies>

8



NOTES:
1. 'S' DENOTES SCHEDULED COLUMN TIE SPACING, SEE COLUMN SCHEDULE. 'S/2' DENOTES 1/2 THE SCHEDULED COLUMN TIE SPACING.
2. WHERE BEAMS FRAME FROM FOUR DIRECTIONS, TERMINATE TIES NO MORE THAN 3" BELOW THE LOWEST HORIZONTAL REINFORCEMENT IN THE SHALLOWEST BEAM.
3. ASSUME 1" ABOVE SLAB OFFSET FOR PT CONSTRUCTION WHERE NO BOTTOM BARS ARE PRESENT.

4 S7.1 CONCRETE COLUMN OR TIE COLUMN DETAIL
3/4" = 1'-0"



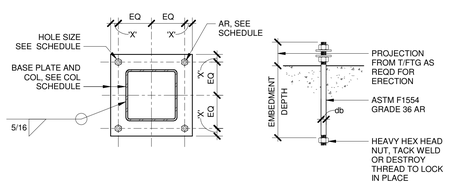
REINF HOOKS **STIRRUP & TIE HOOKS**

SCHEDULE			
BAR SIZE	REINF HOOK	STIRRUP & TIE HOOKS	
	L1	L2	L3
#3	6"	4"	2 1/2"
#4	8"	4 1/2"	3"
#5	10"	-	-
#6	1'-0"	-	-
#7	1'-2"	-	-
#8	1'-4"	-	-
#9	1'-7"	-	-
#10	1'-10"	-	-
#11	2'-0"	-	-

3 S7.1 STD HOOK LENGTHS
1" = 1'-0"

BASE PLATE AND ANCHOR ROD SCHEDULE					
MARK	SIZE LxW (A36 UNO)	THICKNESS	NO. & ANCHOR ROD SIZE	BASE PLATE TYPE	COMMENTS
BP1	0'-10"x1'-2" LS ALONG WEB	3/4"	(4) 3/4" DIA W/ 12" EMBED	TYPE A	
BP2	0'-10"x1'-2" LS ALONG WEB	5/8" 6	(4) 3/4" DIA W/ 12" EMBED	TYPE A	
BP3	0'-10"x1'-2" LS ALONG CMU	1/2"	(4) 3/4" DIA W/ 12" EMBED	TYPE A	
BP4	0'-10"x0'-10"	3/4"	(4) 3/4" DIA W/ 12" EMBED	TYPE A	
BP5	1'-1"x1'-4"	7/8"	(4) 3/4" DIA W/ 12" EMBED	TYPE A	

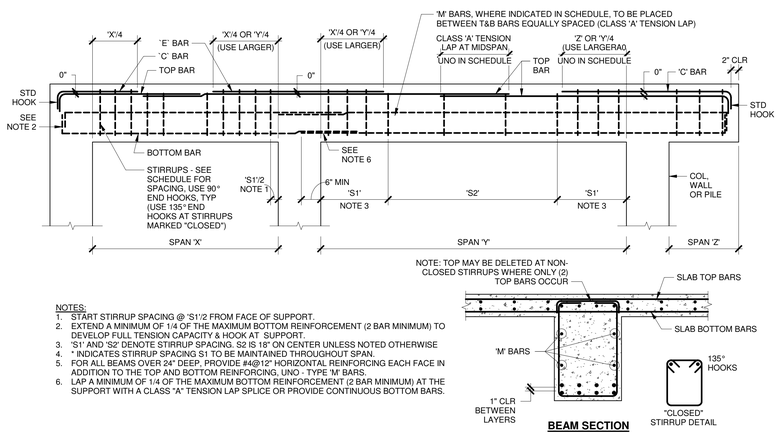
2 S7.1 BASE PLATE SCHEDULE
1/8" = 1'-0"



ANCHOR ROD (AR) SCHEDULE					
ANCHOR ROD DIAMETER (db)	EMBEDMENT (UNO)	MAXIMUM HOLE SIZE	'X' EDGE DISTANCE	MIN WASHER SIZE	MIN WASHER THICKNESS
3/4"	1'-0"	1 5/16"	1 1/2"	2"	1/4"
1"	1'-6"	1 13/16"	2 1/8"	3"	3/8"
1 1/4"	1'-10"	2 1/16"	2 5/8"	3"	1/2"
1 1/2"	2'-6"	2 5/16"	3"	3 1/2"	1/2"
1 3/4"	2'-6"	2 3/4"	3 1/2"	4"	5/8"
2"	3'-3"	3 1/4"	4 1/8"	5"	3/4"

NOTES:
1. EMBEDMENT DEPTH DOES NOT APPLY TO BRACED FRAME COLUMNS OR MOMENT FRAME COLUMNS. SEE FRAME DETAILS FOR EMBEDMENT.
2. WELD DOES NOT APPLY TO BRACED FRAME COLUMNS OF MOMENT FRAME COLUMNS. SEE FRAME DETAILS FOR WELDS.
3. ANCHOR ROD EMBEDMENTS DO NOT APPLY TO ANCHOR RODS EMBEDDED INTO PIERS.
4. 'X'= EDGE DISTANCE, SEE SCHEDULE.
5. SEE BASEPLATE SCHEDULE FOR ANCHOR ROD SIZES.

8 S7.1 ANCHOR ROD SCHEDULE & DETAILS
1" = 1'-0"



BEAM SECTION

NOTES:
1. START STIRRUP SPACING @ 'S1'/2 FROM FACE OF SUPPORT.
2. EXTEND A MINIMUM OF 1/4 OF THE MAXIMUM BOTTOM REINFORCEMENT (2 BAR MINIMUM) TO DEVELOP FULL TENSION CAPACITY & HOOK AT SUPPORT.
3. 'S1' AND 'S2' DENOTE STIRRUP SPACING. S2 IS 18" ON CENTER UNLESS NOTED OTHERWISE
4. * INDICATES STIRRUP SPACING S1 TO BE MAINTAINED THROUGHOUT SPAN.
5. FOR ALL BEAMS OVER 24" DEEP, PROVIDE #4@12" HORIZONTAL REINFORCING EACH FACE IN ADDITION TO THE TOP AND BOTTOM REINFORCING, UNO - TYPE 'M' BARS.
6. LAP A MINIMUM OF 1/4 OF THE MAXIMUM BOTTOM REINFORCEMENT (2 BAR MINIMUM) AT THE SUPPORT WITH A CLASS "A" TENSION LAP SPLICE OR PROVIDE CONTINUOUS BOTTOM BARS.

7 S7.1 TYPICAL BENDING DIAGRAM FOR BEAMS & ONE WAY SLABS
3/4" = 1'-0"

CLASS 'A' SPLICE SCHEDULE

DEVELOPMENT LENGTH SHALL BE PER FOLLOWING TABLE MODIFIED PER NOTES BELOW

GRADE 60 STEEL

NORMAL WEIGHT CONCRETE STRENGTH

BAR	3000 PSI	4000 PSI	5000 PSI	6000 PSI	7000 PSI	8000 PSI	9000 PSI	10000 PSI AND HIGHER
#3	1'-5"	1'-3"	1'-1"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"
#4	1'-10"	1'-7"	1'-5"	1'-4"	1'-3"	1'-2"	1'-1"	1'-1"
#5	2'-3"	2'-0"	1'-10"	1'-8"	1'-6"	1'-5"	1'-4"	1'-4"
#6	2'-9"	2'-5"	2'-2"	2'-0"	1'-10"	1'-10"	1'-8"	1'-8"
#7	4'-0"	3'-6"	3'-2"	2'-10'	2'-8"	2'-6"	2'-4"	2'-3"
#8	4'-7"	4'-0"	3'-7"	3'-3"	3'-0"	2'-10"	2'-8"	2'-7"
#9	5'-2"	4'-6"	4'-0"	3'-8"	3'-5"	3'-2"	3'-1"	3'-0"
#10	5'-10"	5'-1"	4'-6"	4'-2"	3'-10"	3'-7"	3'-5"	3'-4"
#11	6'-6"	5'-7"	5'-0"	4'-7"	4'-3"	4'-0"	3'-10"	3'-8"

CLASS 'B' SPLICE SCHEDULE

DEVELOPMENT LENGTH SHALL BE PER FOLLOWING TABLE MODIFIED PER NOTES BELOW

GRADE 60 STEEL

NORMAL WEIGHT CONCRETE STRENGTH

BAR	3000 PSI	4000 PSI	5000 PSI	6000 PSI	7000 PSI	8000 PSI	9000 PSI	10000 PSI AND HIGHER
#3	1'-9"	1'-6"	1'-5"	1'-3"	1'-2"	1'-1"	1'-0"	1'-0"
#4	2'-4"	2'-1"	1'-10"	1'-8"	1'-7"	1'-5"	1'-4"	1'-4"
#5	3'-3"	2'-7"	2'-4"	2'-1"	2'-0"	1'-10"	1'-9"	1'-8"
#6	3'-7"	3'-1"	2'-9"	2'-6"	2'-4"	2'-2"	2'-1"	2'-0"
#7	5'-2"	4'-6"	4'-0"	3'-8"	3'-5"	3'-2"	3'-0"	2'-10"
#8	6'-0"	5'-2"	4'-7"	4'-2"	4'-0"	3'-8"	3'-5"	3'-3"
#9	6'-8"	5'-10"	5'-2"	4'-9"	4'-5"	4'-1"	3'-10"	3'-8"
#10	7'-6"	6'-6"	5'-10"	5'-4"	5'-0"	4'-7"	4'-4"	4'-2"
#11	8'-4"	7'-3"	6'-6"	6'-0"	5'-6"	5'-1"	4'-10"	4'-7"

NOTES:
1. FOR CLEAR SPACING BETWEEN BARS <db AND/OR CLEAR COVER <db, MULTIPLY BY 1.5.
2. FOR TOP BARS MULTIPLY BY 1.3.
3. FOR EPOXY COATED BARS, IF SPECIFIED OR APPROVED AS AN ALTERMNATE, MULTIPLY BY 1.3.
4. FOR MMFX BARS, IF SPECIFIED OR USED, USE GRADE 75 KSI VALUES.
5. WHERE MORE THAN ONE FACTOR APPLIES, PRODUCT OF ALL APPLICABLE FACTORS SHALL BE APPLIED.
6. IF DETAILER IS TO USE A DIFFERENT SCHEDULE, HE/SHE MUST SUBMIT A SEALED LETTER INDICATING THAT HIS/HER VALUES CORRESPOND TO CURRENT ACI 318 CODE.

6 S7.1 CLASS 'B' SPLICE SCHEDULE
1/2" = 1'-0"

CODE REVIEW:

CERTIFICATE:

spiezle

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Certificate of Authorization No. 35826

TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

ASI #8_06/19/2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

8	REVISION NAME	DATE
4	ADDENDUM 4	01.27.2025
8	ASI #8	06.19.2025

FOR BID:

DRAWING TITLE:

SCHEDULES & DETAILS

COMMISSION NUMBER:

24FTL018

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S7.1

THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"

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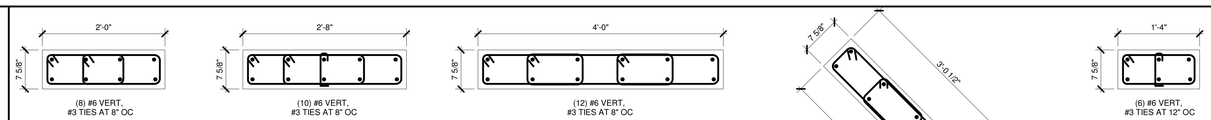
LEVEL 3														LEVEL 3
27' - 4"														27' - 4"
LEVEL 2														LEVEL 2
14' - 0"	5-SC2 - W10X49	5-SC1 - HSS5X5X1/2	5-SC1 - HSS5X5X1/2	5-SC1 - HSS5X5X1/2	2SC1 - W8X31	2SC1 - W8X31	2SC1 - W8X31	2-SC3 - W8X28	2-SC5 - HSS4X4X1/4	2-SC4 - HSS8X8X1/4	2SC1 - W8X31	2-SC3 - W8X28	2SC1 - W8X31	14' - 0"
LEVEL 1														LEVEL 1
0' - 0"														0' - 0"
Column Locations	A2-AF, A2-AG, A4-AF, A4-AG	A2(2' - 9 5/8")-AH(38' - 7"), A7-AE	A3-AH(38' - 7"), A6(-23' - 7")-AI	A7-AB, A7-AC, A7-AD	C2-CC, C2-CE, C2-CJ, C2-CL	C3-CC, C3-CL, C6-CC, C6-CL	C3-CE, C3-CI, C3-CJ, C6-CE, C6-CI, C6-CJ	C3-CF, C6-CF	C4(-4' - 1 7/8")-CC(-4' - 1 1/2"), C4(-4' - 1 7/8")-CL(3' - 8 7/8")	C4-CD, C4-CK, C5-CK	C4-CE	C4-CG	C4-CH, C5-CG, C5-CH	C4-CJ, C5-CE, C5-CJ	

LEVEL 3						LEVEL 3
27' - 4"						27' - 4"
LEVEL 2						LEVEL 2
14' - 0"	2-SC4 - HSS8X8X1/4	2-SC2 - W8X40	2SC1 - W8X31	1-SC1 - W8X35	1-SC2 - HSS4X4X1/2	14' - 0"
LEVEL 1						LEVEL 1
0' - 0"						0' - 0"
Column Locations	C5-CD	C7-CC	C7-CE, C7-CJ, C7-CL	D2-DB, D2-DD, D2-DF, D2-DH, D5-DK, D7-DK, D9-DK, D10(-7' - 4 1/4")-DK	D6-DC, D6-DE, D6-DG(4' - 0 5/8"), D8-DD(-5' - 8"), D8-DG, D10-DG	

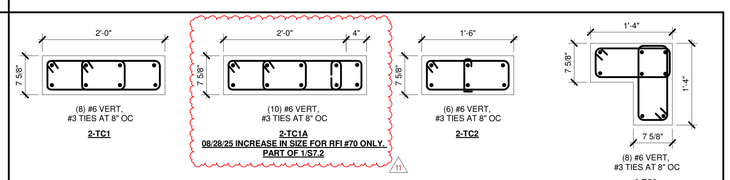
STEEL COLUMN SCHEDULE

1/8" = 1'-0"

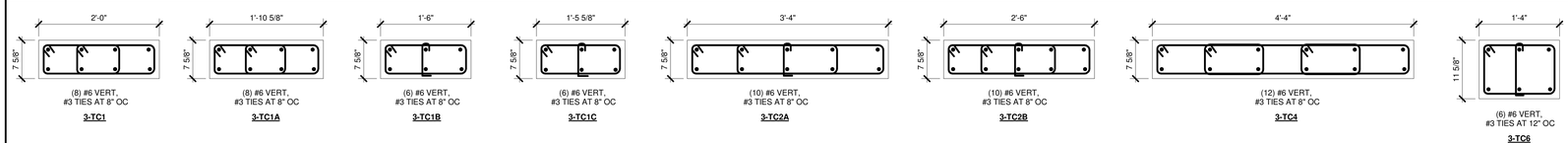
ADMIN BUILDING



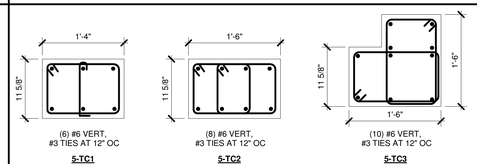
CLASSROOM BUILDING 2 & 4



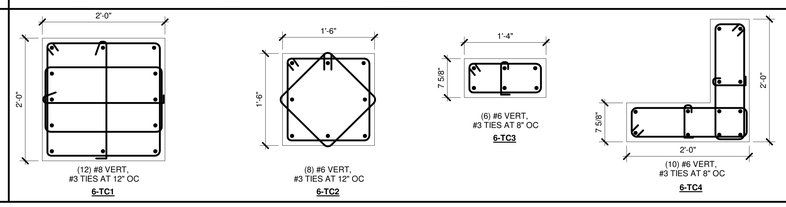
GYM BUILDING



AUDETERIA BUILDING



CHILLER BUILDING



COLUMN SCHEDULE

1" = 1'-0"

CODE REVIEW:

CERTIFICATE:

spiezle

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TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

1	REVISION NAME	DATE
11	ASI #11	09.29.2025

FOR BID:

DRAWING TITLE:

COLUMN SCHEDULES

COMMISSION NUMBER:

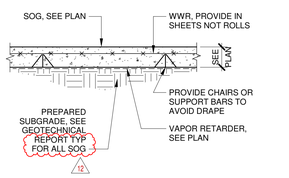
24FTL018

DO NOT SCALE THE DRAWINGS

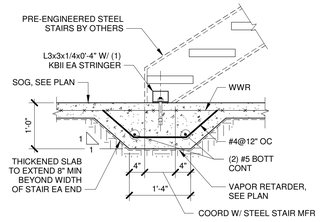
DRAWING NUMBER:

S7.2

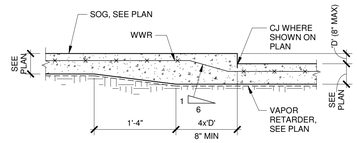
THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"



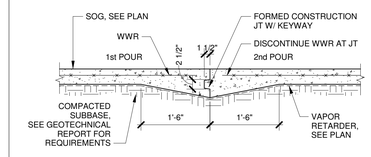
6 WWR AT SOG
S8.1 1" = 1'-0"



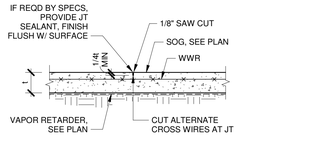
5 STEEL STAIR FOOTING
S8.1 3/4" = 1'-0"



4 DEPRESSED SOG
S8.1 1" = 1'-0"

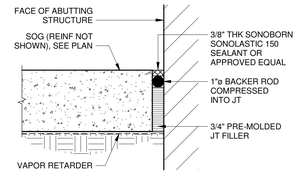


3 CONSTRUCTION JOINT AT SOG
S8.1 3/4" = 1'-0"



SAW CUT AS SOON AS POSSIBLE WITHOUT RAVELING CONCRETE (4 TO 6 HOURS AFTER POUR)

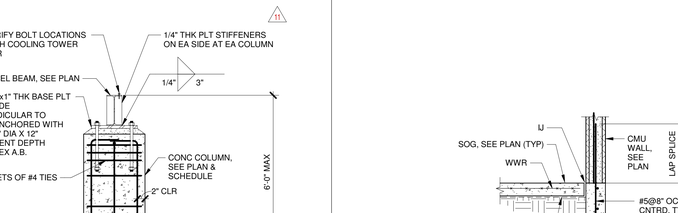
2 CONTROL JOINT AT SOG (CJ)
S8.1 1" = 1'-0"



NOTES:
1. PROVIDE SEALANT FOR EXTERNAL JOINTS ONLY.
2. CLEAN AND PREPARE SURFACES TO RECEIVE SEALANT IN ACCORDANCE W/ MFR'S RECOMMENDATIONS.

1 ISOLATION JOINT AT SOG (IJ)
S8.1 3" = 1'-0"

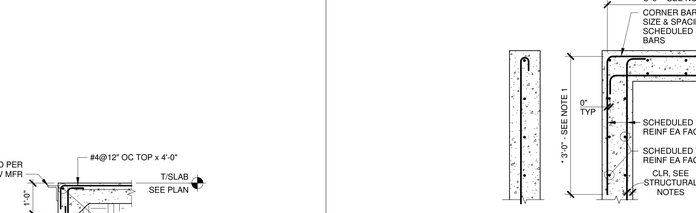
NOTE: EXTERIOR CMU WALLS NOT SHOWN FOR CLARITY



CORNER **EDGE** **INTERIOR**

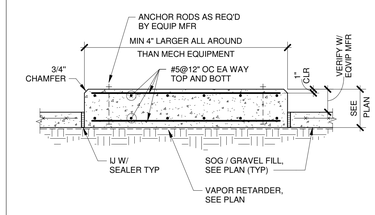
12 ISOLATION JOINTS AT HSS COLUMNS
S8.1 1/2" = 1'-0"

NOTE: EXTERIOR CMU WALLS NOT SHOWN FOR CLARITY

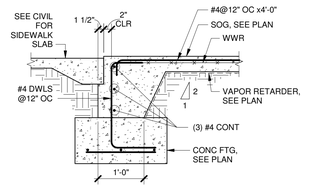


CORNER **EDGE** **INTERIOR**

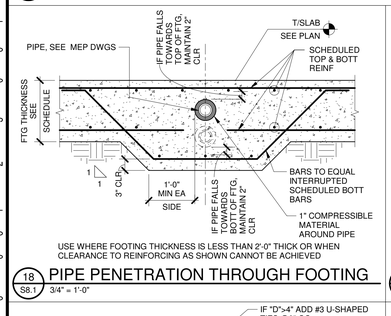
10 ISOLATION JOINTS AT CONCRETE COLUMNS
S8.1 1/2" = 1'-0"



8 EQUIPMENT PAD AT SOG
S8.1 3/4" = 1'-0"

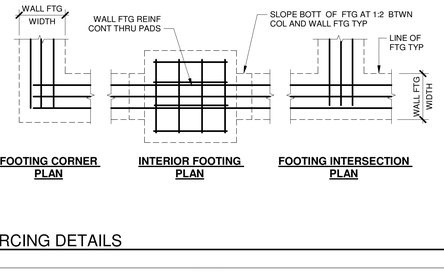


7 TURNED DOWN SLAB EDGE AT FOOTING
S8.1 3/4" = 1'-0"



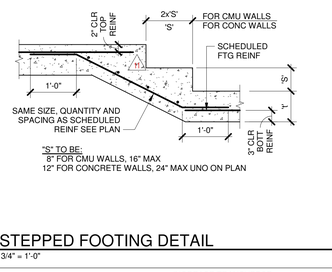
USE WHERE FOOTING THICKNESS IS LESS THAN 2'-0" THICK OR WHEN CLEARANCE TO REINFORCING AS SHOWN CANNOT BE ACHIEVED

18 PIPE PENETRATION THROUGH FOOTING
S8.1 3/4" = 1'-0"



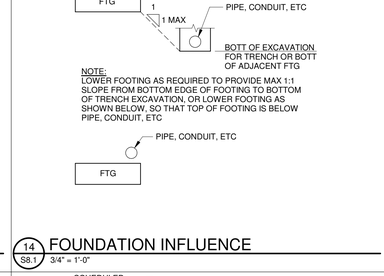
FOOTING CORNER PLAN **INTERIOR FOOTING PLAN** **FOOTING INTERSECTION PLAN**

17 FOOTING REINFORCING DETAILS
S8.1 3/4" = 1'-0"



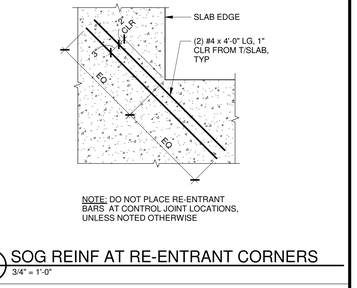
"S" TO BE:
8" FOR CMU WALLS, 16" MAX
12" FOR CONCRETE WALLS, 24" MAX UNO ON PLAN

15 STEPPED FOOTING DETAIL
S8.1 3/4" = 1'-0"



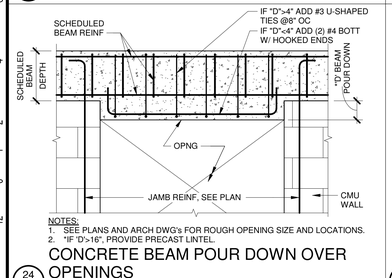
NOTE:
LOWER FOOTING AS REQUIRED TO PROVIDE MAX 1:1 SLOPE FROM BOTTOM EDGE OF FOOTING TO BOTTOM OF TRENCH EXCAVATION, OR LOWER FOOTING AS SHOWN BELOW, SO THAT TOP OF FOOTING IS BELOW PIPE, CONDUIT, ETC

14 FOUNDATION INFLUENCE
S8.1 3/4" = 1'-0"



NOTE: DO NOT PLACE RE-ENTRANT BARS AT CONTROL JOINT LOCATIONS, UNLESS NOTED OTHERWISE

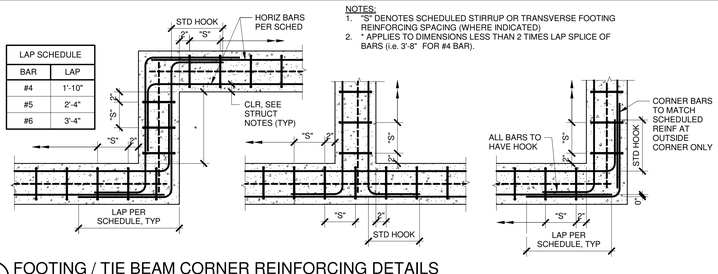
13 SOG REINF AT RE-ENTRANT CORNERS
S8.1 3/4" = 1'-0"



NOTES:
1. SEE PLANS AND ARCH DWG's FOR ROUGH OPENING SIZE AND LOCATIONS.
2. "IF "D">16", PROVIDE PRECAST LINTEL.

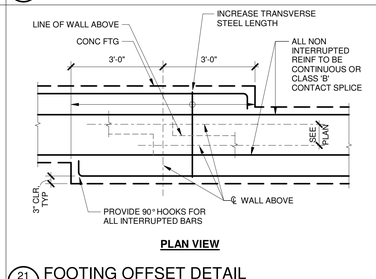
24 CONCRETE BEAM POUR DOWN OVER OPENINGS
S8.1 3/4" = 1'-0"

LAP SCHEDULE	
BAR	LAP
#4	1'-10"
#5	2'-4"
#6	3'-4"



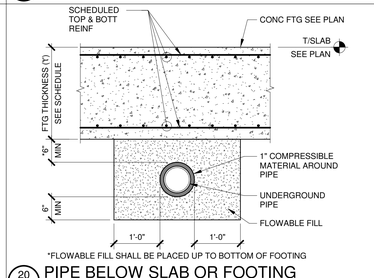
NOTES:
1. "S" DENOTES SCHEDULED STIRRUP OR TRANSVERSE FOOTING REINFORCING SPACING (WHERE INDICATED)
2. * APPLIES TO DIMENSIONS LESS THAN 2 TIMES LAP SPLICE OF BARS (i.e. 3'-8" FOR #4 BAR).

23 FOOTING / TIE BEAM CORNER REINFORCING DETAILS
S8.1 1" = 1'-0"



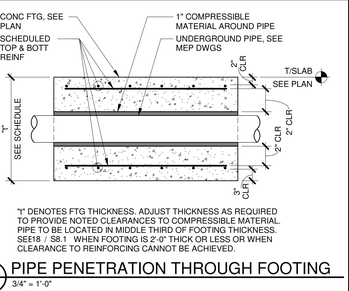
PLAN VIEW

21 FOOTING OFFSET DETAIL
S8.1 1/2" = 1'-0"



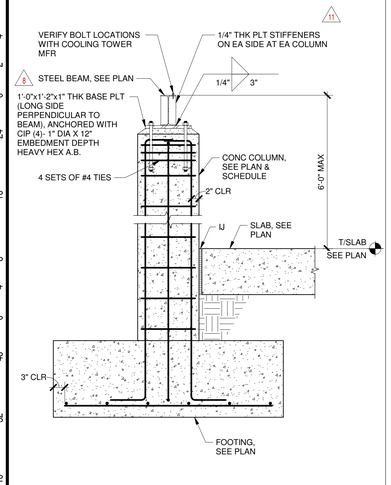
*FLOWABLE FILL SHALL BE PLACED UP TO BOTTOM OF FOOTING

20 PIPE BELOW SLAB OR FOOTING
S8.1 3/4" = 1'-0"

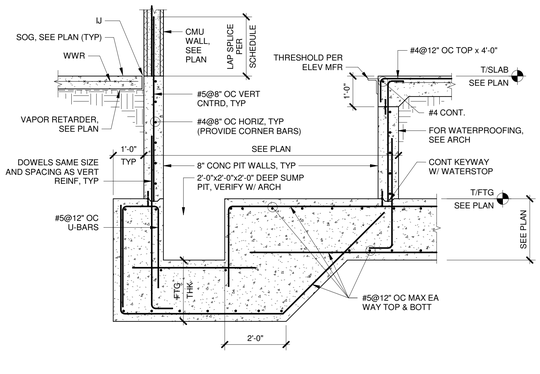


"T" DENOTES FTG THICKNESS. ADJUST THICKNESS AS REQUIRED TO PROVIDE NOTED CLEARANCES TO COMPRESSIBLE MATERIAL. PIPE TO BE LOCATED IN MIDDLE THIRD OF FOOTING THICKNESS. SEE18 / S8.1 WHEN FOOTING IS 2'-0" THICK OR LESS OR WHEN CLEARANCE TO REINFORCING CANNOT BE ACHIEVED.

19 PIPE PENETRATION THROUGH FOOTING
S8.1 3/4" = 1'-0"



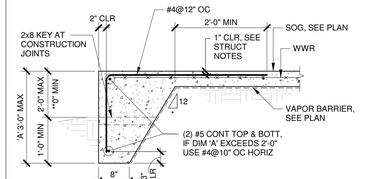
36 COOLING TOWER PIER DETAIL
S8.1 3/4" = 1'-0"



35 ELEVATOR PIT SECTION
S8.1 1/2" = 1'-0"

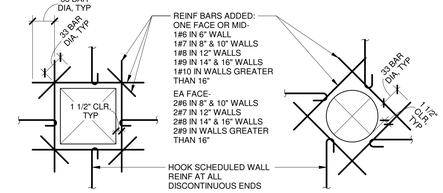
NOTES:
1. *34 BAR DIA PLUS WALL THICKNESS LESS 2" OR 3'-0" (MINIMUM), WHICHEVER IS GREATER.
2. *34 BAR DIA PLUS 1/2 WALL THICKNESS OR 2'-6" (MINIMUM), WHICHEVER IS GREATER.
3. SEE PLAN FOR WALL THICKNESS AND REINFORCING SIZE AND SPACING.
4. NOT APPLICABLE AT SHEARWALLS. FOR SHEARWALL REINFORCING, SEE PLANS AND SCHEDULES.

27 REINFORCING AT WALL ENDS, CORNERS & INTERSECTIONS - BARS ON ONE FACE
S8.1 3/4" = 1'-0"



NOTES:
* AS REQUIRED NOT LESS THAN 6 UNLESS FORMED EDGE
** OR AS INDICATED ON ARCHL, COORDINATE W/ CIVIL

33 TURNED DOWN SLAB EDGE
S8.1 3/4" = 1'-0"



32 REINFORCING AT CONCRETE WALL OPENINGS
S8.1 3/4" = 1'-0"

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Autodesk Docs://24K054 - St Lucie WG K8/Western Grove K8 - SLPS_R24.rvt

CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
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VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONADO AR0012456
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CONSULTANTS:

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1230 N. University Dr.
PLANTATION, FL 33322
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Certificate of Authorization No. 35826

TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

Δ	REVISION NAME	DATE
8	ASI #8	06.19.2025
11	ASI #11	09.29.2025
12	ASI #14	12.05.2025

FOR BID:

DRAWING TITLE:

TYPICAL FOUNDATION DETAILS

COMMISSION NUMBER:

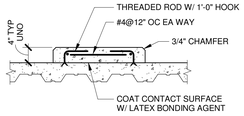
24FTL018

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S8.1

THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"



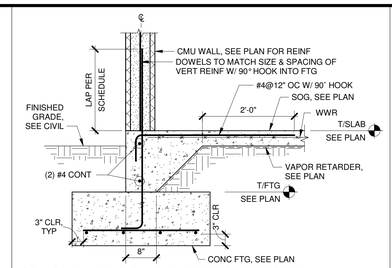
AT SUPPORTED SLAB

NOTES:
1. FOR SIZE & LOCATION, REFER TO ARCH & MEP DWGS.
2. CONCRETE FOR PADS SHALL BE NORMAL WT W/ f'c = 4,000 PSI.
3. THREADED RODS TO BE 3/8"ø A36 STEEL IN EXPANSION INSERTS @18" OC ALL AROUND.

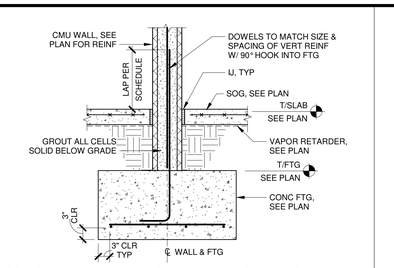
6 S8.2 HOUSEKEEPING PAD AT SOG & ELEVATED SLAB
3/4" = 1'-0"

SCHEDULE		
WALL THICKNESS	BAR	LAP
8"	#4	1'-8"
8"	#5	2'-1"
8"	#6	3'-7"
8"	#7	4'-11"
12"	#4	1'-8"
12"	#5	2'-1"
12"	#6	3'-3"
12"	#7	3'-10"

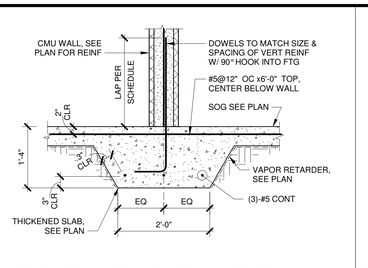
5 S8.2 CMU VERTICAL REINFORCING LAP SCHEDULE
3/4" = 1'-0"



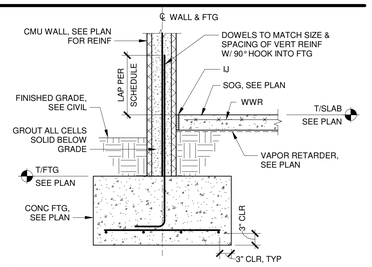
4 S8.2 CMU AT CONCRETE POURDOWN
3/4" = 1'-0"



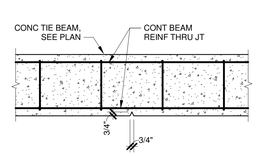
3 S8.2 CMU AT INTERIOR WALL FOOTING
3/4" = 1'-0"



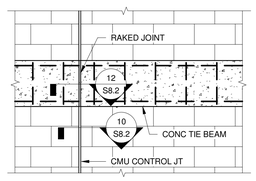
2 S8.2 CMU AT THICKENED SLAB FOOTING
3/4" = 1'-0"



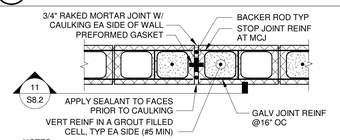
1 S8.2 CMU AT EXTERIOR WALL FOOTING
3/4" = 1'-0"



12 S8.2 SECTION AT MCJ
1" = 1'-0"

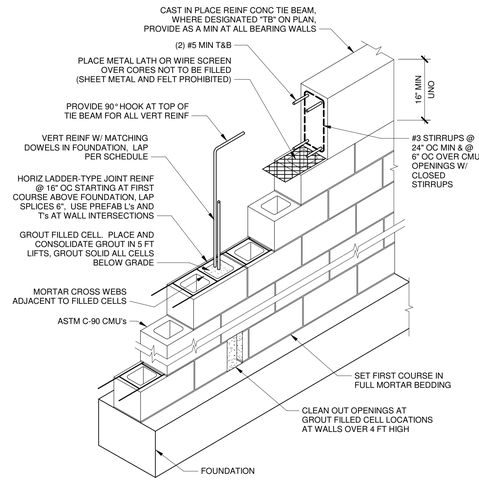


11 S8.2 SECTION AT MCJ
1/2" = 1'-0"

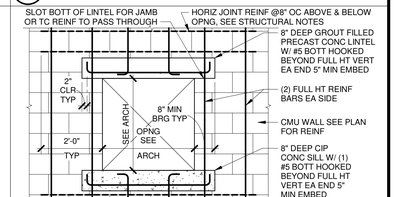


NOTES:
1. MAXIMUM ALLOWABLE JOINT SPACING = 25'-0" UNLESS NOTED OTHERWISE ON PLAN.
2. JOINTS IN FINISH MUST ALIGN WITH MASONRY (CMU) JOINTS.
3. SEE PLAN FOR REINFORCING SIZE AND SPACING.
4. JOINTS ARE NOT TO OCCUR IN A SHEAR WALL UNLESS SPECIFICALLY DETAILED ON PLAN. CONTACT THE STRUCTURAL ENGINEER OF RECORD IN THE EVENT THAT A JOINT FALLS WITHIN A SHEAR WALL.
5. DO NOT PLACE MASONRY CONTROL JOINTS WITHIN 24" OF LOAD BEARING PILASTERS OR OPENINGS.
6. CONTRACTOR TO VERIFY CONTROL JOINT LOCATIONS WITH PROJECT ARCHITECT.

10 S8.2 MASONRY CONTROL JOINT (MCJ) REINFORCING
1" = 1'-0"

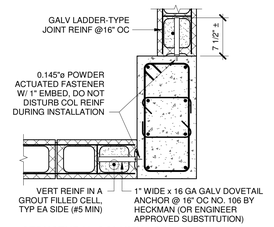


15 S8.2 TYP MASONRY (CMU) WALL CONSTRUCTION
3/4" = 1'-0"

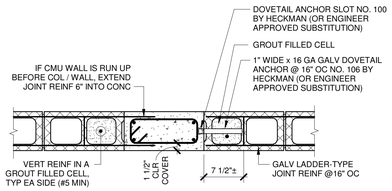


FOR OPENINGS 3'-0"x3'-0" OR SMALLER

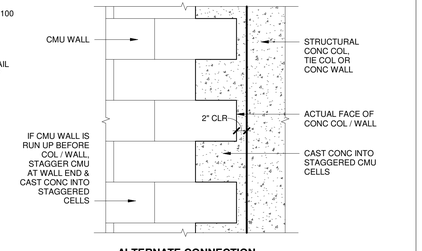
7 S8.2 MINOR CMU WALL OPNGS
1/2" = 1'-0"



STRUCTURAL COLUMN / CONCRETE WALL

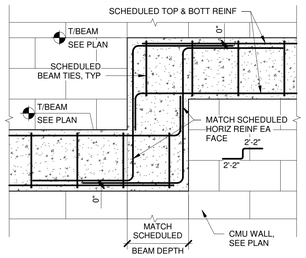


TIE COLUMN / CONCRETE WALL

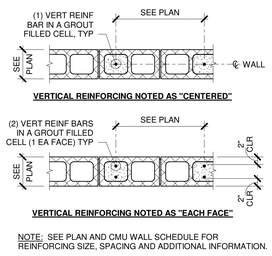


ALTERNATE CONNECTION

18 S8.2 CMU ANCHORAGE TO CONCRETE
1" = 1'-0"

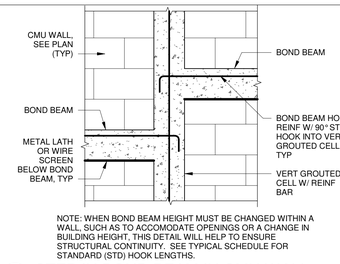


24 S8.2 STEP IN TIE BEAM IN CMU WALL
3/4" = 1'-0"



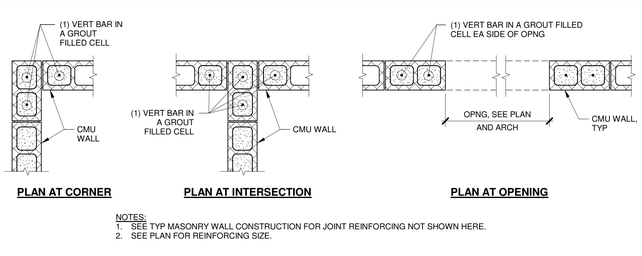
NOTE: SEE PLAN AND CMU WALL SCHEDULE FOR REINFORCING SIZE, SPACING AND ADDITIONAL INFORMATION.

23 S8.2 CMU BAR PLACEMENT REQUIREMENTS
3/4" = 1'-0"



NOTE: WHEN BOND BEAM HEIGHT MUST BE CHANGED WITHIN A WALL, SUCH AS TO ACCOMODATE OPENINGS OR A CHANGE IN BUILDING HEIGHT, THIS DETAIL WILL HELP TO ENSURE STRUCTURAL CONTINUITY. SEE TYPICAL SCHEDULE FOR STANDARD (STD) HOOK LENGTHS.

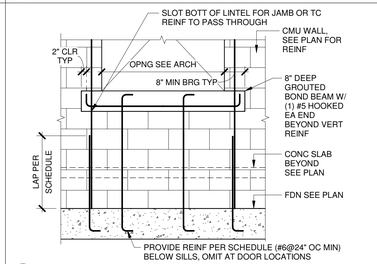
22 S8.2 STEP IN BOND BEAM IN CMU WALL
3/4" = 1'-0"



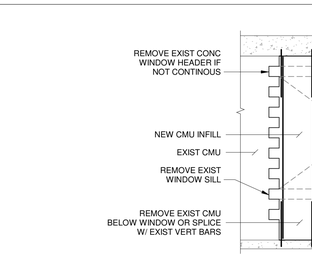
PLAN AT CORNER **PLAN AT INTERSECTION** **PLAN AT OPENING**

NOTES:
1. SEE TYP MASONRY WALL CONSTRUCTION FOR JOINT REINFORCING NOT SHOWN HERE.
2. SEE PLAN FOR REINFORCING SIZE.

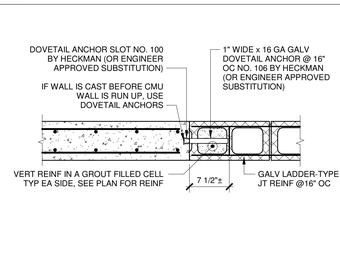
21 S8.2 REINFORCING AT CMU WALL CORNERS, INTERSECTIONS AND OPENINGS
3/4" = 1'-0"



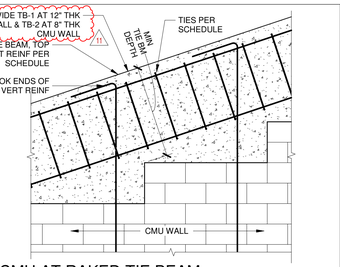
19 S8.2 CMU WALL OPNG W/ BOND BEAM SILL
1/2" = 1'-0"



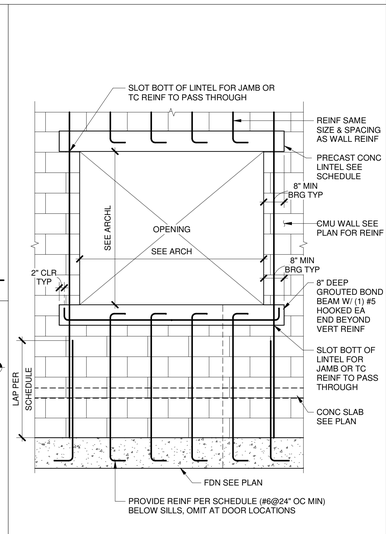
30 S8.2 CMU INFILL AT EXIST WINDOW OPNG
1/4" = 1'-0"



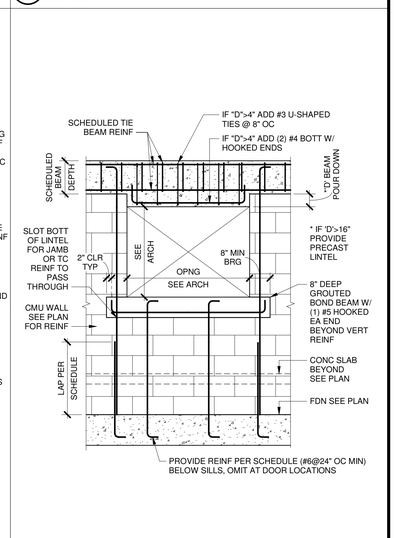
28 S8.2 CMU ANCHORAGE TO CONCRETE WALL
1" = 1'-0"



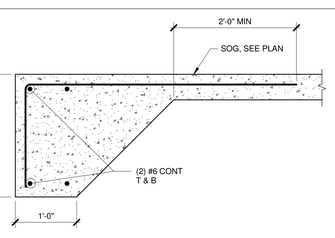
27 S8.2 CMU AT RAKED TIE BEAM
1/2" = 1'-0"



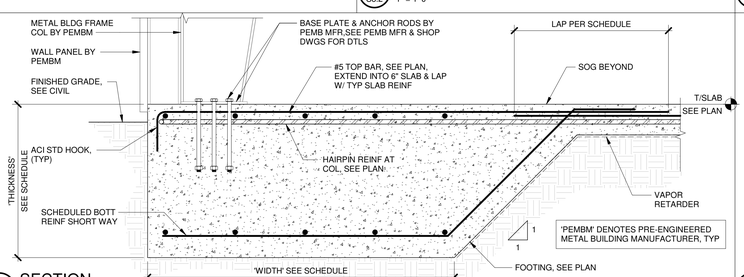
CMU WINDOW OPNG W/ PRECAST LINTEL & BOND BM SILL
1/2" = 1'-0"



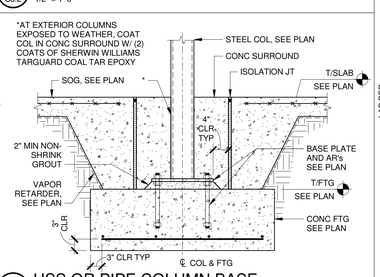
31 S8.2 CMU WALL OPNG AT CONC TIE BEAM
1/2" = 1'-0"



36 S8.2 SECTION
1" = 1'-0"



35 S8.2 SECTION
1" = 1'-0"



33 S8.2 HSS OR PIPE COLUMN BASE
3/4" = 1'-0"

CODE REVIEW:

CERTIFICATE:

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
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VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
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Certificate of Authorization No. 35826

TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

Δ	REVISION NAME	DATE
11	ASI #11	09.29.2025

FOR BID:

DRAWING TITLE:

TYPICAL MASONRY DETAILS

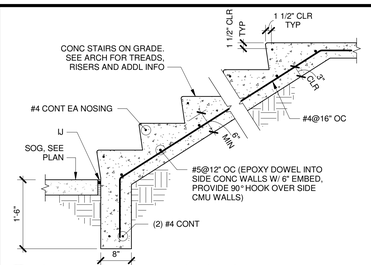
COMMISSION NUMBER:
24FTL018

DO NOT SCALE THE DRAWINGS

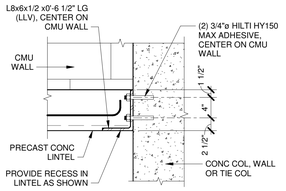
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S8.2

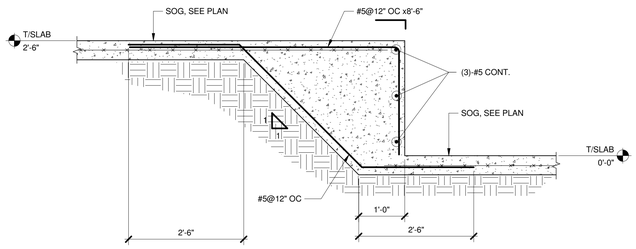
THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"



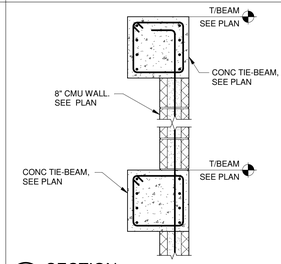
5 S8.3 STAIR ON GRADE SECTION
3/4" = 1'-0"



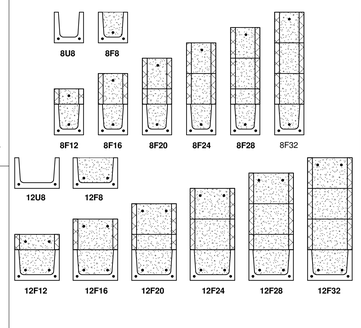
4 S8.3 PRECAST LINTEL SUPPORT AT CAST-IN-PLACE CONCRETE
1" = 1'-0"



12 S8.3 SECTION
3/4" = 1'-0"

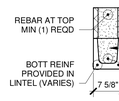


10 S8.3 SECTION
3/4" = 1'-0"

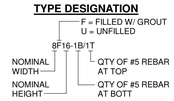


LINTEL SCHEDULE			
LINTEL SPAN	8" CMU WALL	12" CMU WALL	
	LINTEL DESIGNATION	LINTEL DESIGNATION	REMARKS
UP TO 4'-0"	8 F8-1B	12 F8-2B	-
FROM 4'-1" TO 6'-0"	8 F8-1B/1T	12 F8-2B/2T	-
FROM 6'-1" TO 8'-0"	8 F16-1B/1T	12 F16-2B/2T	-
FROM 8'-1" TO 10'-0"	8 F16-2B/2T	12 F16-2B/2T	-
FROM 10'-1" TO 13'-0"	8 F24-2B/2T	12 F24-2B/2T	-
OVER 13'-0"	CAST IN PLACE BEAM	CAST IN PLACE BEAM	SEE PLANS

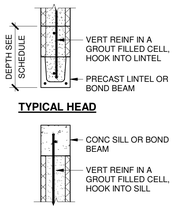
NOTES:
1. LINTELS SHALL BE "CAST-CRETE" OR EQUIVALENT PRECAST CONCRETE UNO.
2. LINTELS SHALL BEAR 8" MIN ON CMU/CONCRETE AND 3" MINIMUM ON STEEL ANGLES.



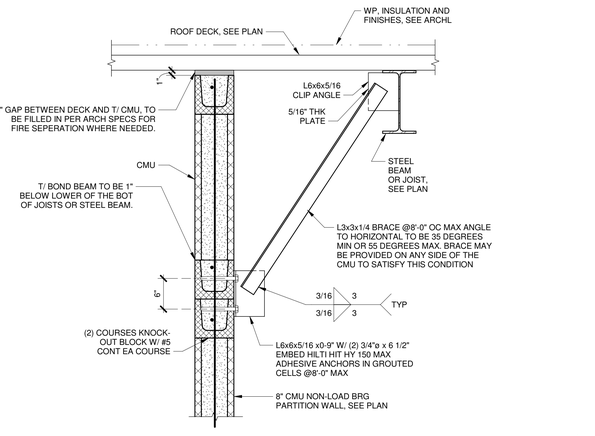
TYPE DESIGNATION



9 S8.3 LINTEL SCHEDULE AND DETAILS
3/4" = 1'-0"

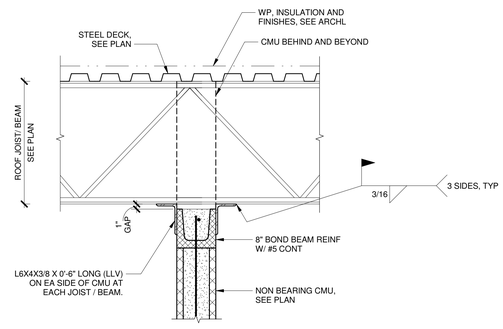


7 S8.3 CMU WALL HEAD, JAMB AND SILL DETAIL
3/4" = 1'-0"



11 RFI #44

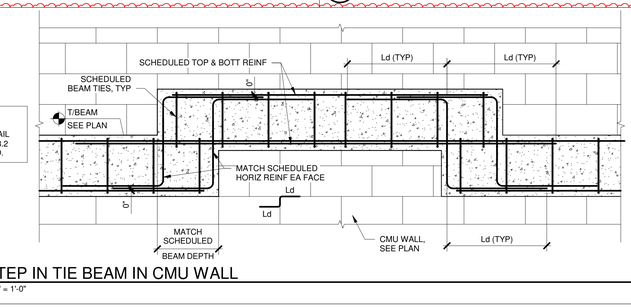
24 S8.3 BRACING AT CMU NON LOAD BEARING PARTITION WALL PARALLEL TO JOIST/ BEAM
1" = 1'-0"



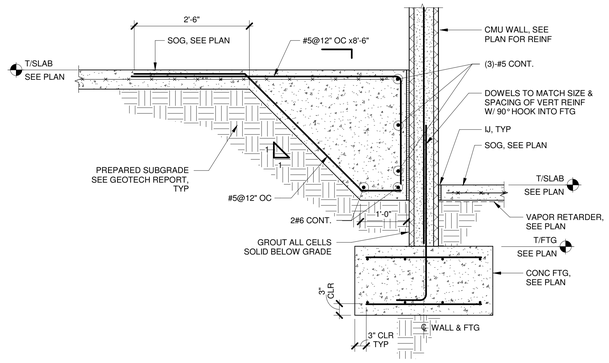
12

11 RFI #44

22 S8.3 BRACING AT NON-BEARING PARTITION WALL, PERPENDICULAR TO BEAM/ JOIST
1" = 1'-0"



14 S8.3 STEP IN TIE BEAM IN CMU WALL
3/4" = 1'-0"



26 S8.3 CMU AT INTERIOR WALL FOOTING
3/4" = 1'-0"

CODE REVIEW:

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SPIEZLE ARCHITECTURAL GROUP INC.
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PHONE: 772-794-2929

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FOR

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REVISIONS:

△	REVISION NAME	DATE
11	ASI #11	09.29.2025
12	ASI #14	12.05.2025

FOR BID:

DRAWING TITLE:

TYPICAL CMU DETAILS

COMMISSION NUMBER:
24FTL018

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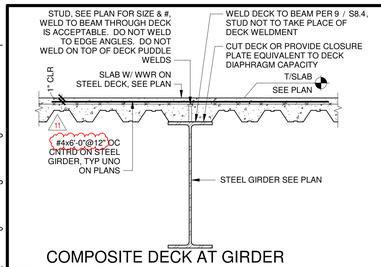
S8.3

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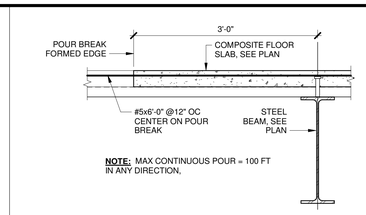
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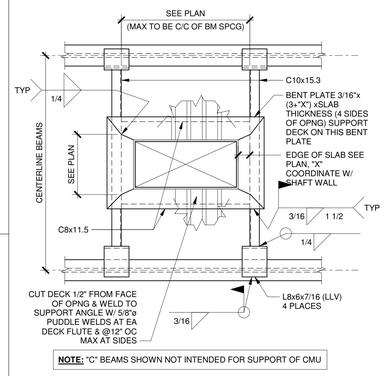
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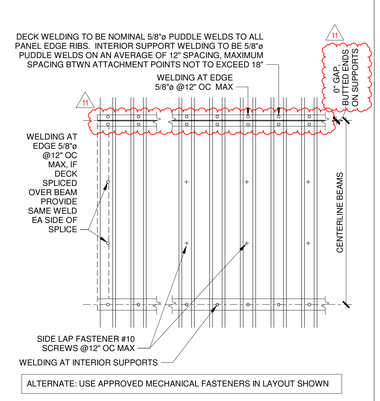
6 COMPOSITE DECK AT GIRDER CONNECTION
S8.4 3/4" = 1'-0"



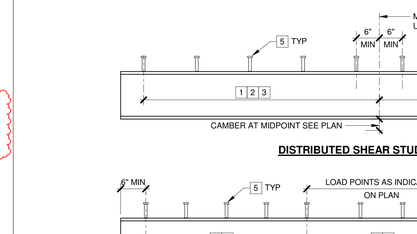
5 COMPOSITE FLOOR CONSTRUCTION JOINT
S8.4 1" = 1'-0"



10 COMPOSITE SLAB OPENING DETAIL
S8.4 3/4" = 1'-0"



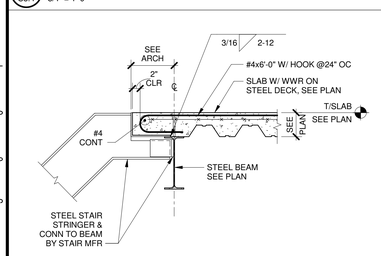
9 COMPOSITE FLOOR DECK FASTENING
S8.4 3/4" = 1'-0"



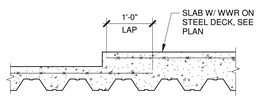
SHEAR STUD PLACEMENT

1. EQUALLY DISTRIBUTE ONE HALF OF SHEAR CONNECTORS INDICATED ON PLAN EACH SIDE OF BEAM MIDPOINT OR LOAD POINTS INDICATED ON PLANS, STARTING FROM EACH END OF SPAN. SEE PLAN FOR NUMBER OF STUDS.
2. SPACING OF SHEAR CONNECTORS SHALL NOT EXCEED 8 TIMES THE TOTAL SLAB THICKNESS AND NO GREATER THAN 36" OC. SPACING SHALL BE NO CLOSER THAN 6x STUD DIAMETER OR DISTANCE BETWEEN FLUTES FOR STEEL DECKS WHEN DECK FLUTE IS PERPENDICULAR TO BEAM. COORDINATE STUD LAYOUT WITH DECK SECTIONS USED.
3. DOUBLE OR TRIPLE STUDS IF REQUIRED. DOUBLE AND TRIPLE STUD PLACEMENT TO BE SYMMETRICAL ABOUT BEAM WEB. MAINTAIN 4x STUD DIAMETER TRANSVERSE SPACING BETWEEN STUDS IN PARALLEL ROWS, WITH A MINIMUM EDGE DISTANCE OF 1 1/2 STUD DIAMETERS, 1" MINIMUM.
4. EQUALLY DISTRIBUTE SHEAR CONNECTORS INDICATED ON PLANS FOR EACH GIRDER SEGMENT BETWEEN LOAD POINTS (BEAMS WHICH FRAME INTO GIRDER).
5. SEE PLAN FOR STUD SIZE. NUMBER OF STUDS REQUIRED PER BEAM IS INDICATED BY (#).

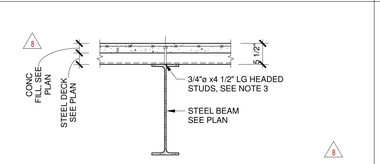
8 COMPOSITE STEEL BEAM STUD PLACEMENT
S8.4 3/4" = 1'-0"



12 STEEL STAIR CONNECTION
S8.4 3/4" = 1'-0"

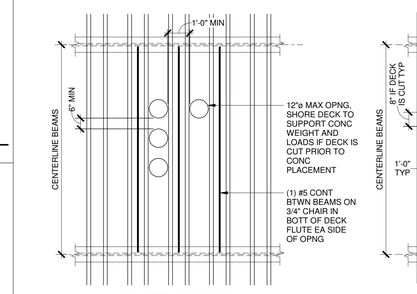


11 DEPRESSED SLAB AT STEEL DECK
S8.4 3/4" = 1'-0"

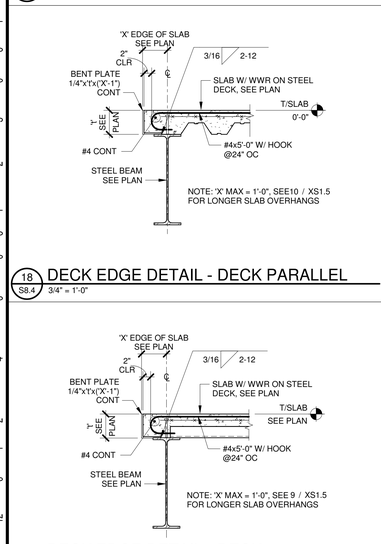


NOTES:
1. SEE PLAN FOR STEEL DECK DPTH, TYPE, GALVANIZINF REQUIREMENTS , CONCRETE THICKNESS AND TOTAL DEPTH.
2. 28 DAY COMPRESSIVE STRENGTH OF CONCRETE F'c=3,000 PSI.
3. QUANTITY OF STUDS ARE NOTED THUS [#] ON PLAN.
4. FASTEN FLOOR DECK TO SUPPORTS WITH 5/8"ø PUDDLE WELDS @12" OC. CONNECT SIDELAPS WITH #10 TEK SCREWS @3'-0" OC.

15 COMPOSITE SLAB / BEAM DETAIL
S8.4 3/4" = 1'-0"

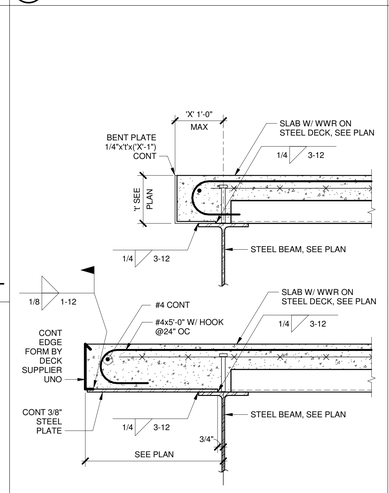


20 COMPOSITE SLAB PIPE PENETRATION DETAIL
S8.4 1" = 1'-0"

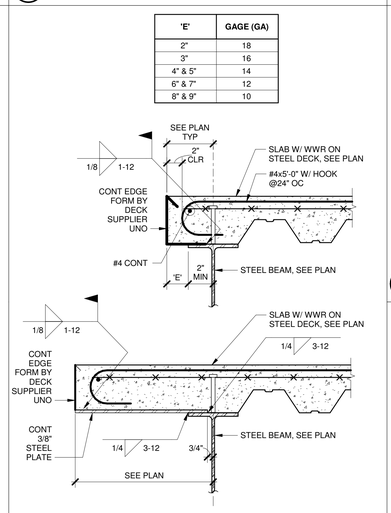


18 DECK EDGE DETAIL - DECK PARALLEL
S8.4 3/4" = 1'-0"

24 DECK EDGE DETAIL - DECK PERPENDICULAR
S8.4 3/4" = 1'-0"



23 DECK EDGE DETAIL - DECK PERPENDICULAR
S8.4 1 1/2" = 1'-0"



22 DECK EDGE DETAIL - DECK PARALLEL
S8.4 1 1/2" = 1'-0"

21 DECK TRANSITION AT STEEL BEAM
S8.4 3/4" = 1'-0"

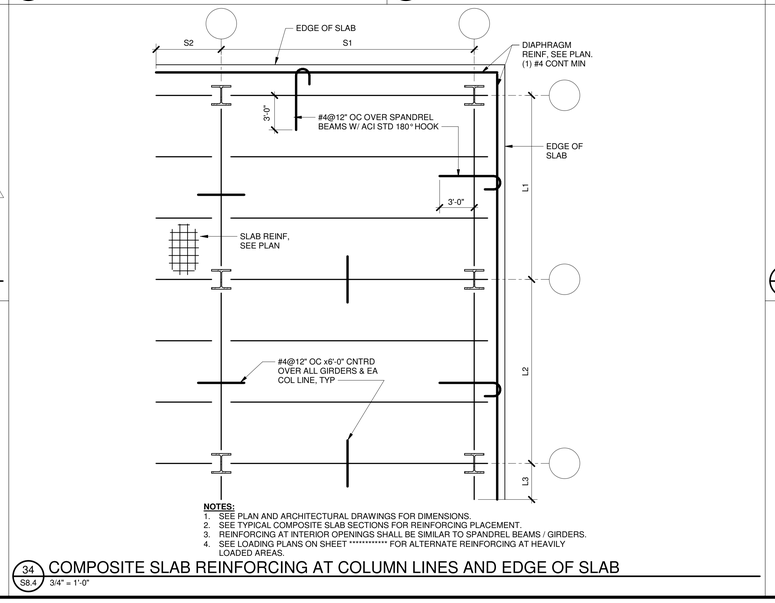
VERTICAL REACTIONS FROM COOLING TOWERs, RTU				
		LATERAL WIND		UPLIFT
	EQUIPMENT + CURB	WIND - X	WIND - Y	
P1	+535	-1445	+1285	-1230
P2	+380	+1445	+428	-610
P3	+535	-1445	-1285	-1230
P4	+380	+1445	-428	-610

NOTES:
1. ALL REACTION FORCES ARE IN LBS.
2. WIND REACTIONS ARE FOR ALLOWABLE STRESS DESIGN. USE APPROPRIATE LOAD FACTORS IN COMBINATIONS.
3. REACTIONS DUE TO LATERAL WIND ARE REVERSIBLE. DELEGATED ENGINEER FOR TRUSSES/ JOISTS TO DESIGN FOR THE FORCE REVERSAL IN COMBINATION WITH ALL THE OTHER FORCES ACTING ON THE TRUSSES/JOISTS INCLUDING THE UPLIFT FORCES ABOVE.

8

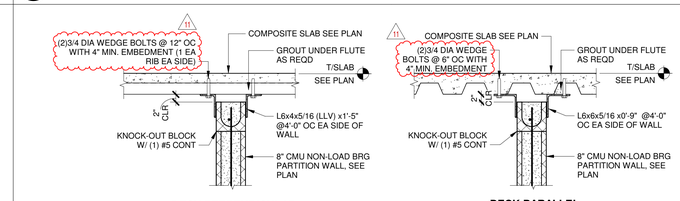
29 SCHEDULE
S8.4 1/8" = 1'-0"

'E'	GAGE (GA)
2"	18
3"	16
4" & 5"	14
6" & 7"	12
8" & 9"	10



NOTES:
1. SEE PLAN AND ARCHITECTURAL DRAWINGS FOR DIMENSIONS.
2. SEE TYPICAL COMPOSITE SLAB SECTIONS FOR REINFORCING PLACEMENT.
3. REINFORCING AT INTERIOR OPENINGS SHALL BE SIMILAR TO SPANDREL BEAMS / GIRDERS.
4. SEE LOADING PLANS ON SHEET ********** FOR ALTERNATE REINFORCING AT HEAVILY LOADED AREAS.

34 COMPOSITE SLAB REINFORCING AT COLUMN LINES AND EDGE OF SLAB
S8.4 3/4" = 1'-0"



NOTE: DO NOT ATTACH ANGLES TO CMU PARTITION WALL.

26 BRACING AT CMU NON LOAD BEARING PARTITION WALL
S8.4 3/4" = 1'-0"

CODE REVIEW:

CERTIFICATE:

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
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CONSULTANTS:

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FAX: (954) 484-7834

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Certificate of Authorization No. 35826

TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

Δ	REVISION NAME	DATE
8	ASI #8	06.19.2025
11	ASI #11	09.29.2025

FOR BID:

DRAWING TITLE:

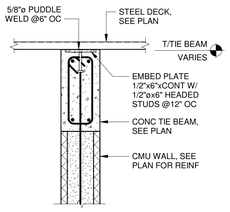
COMPOSITE STEEL DETAILS

COMMISSION NUMBER:
24FTL018

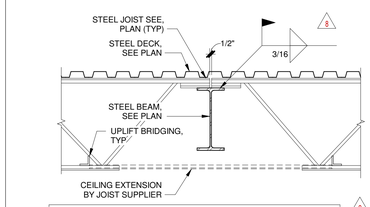
DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:
S8.4

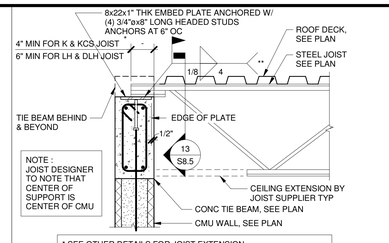
THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"



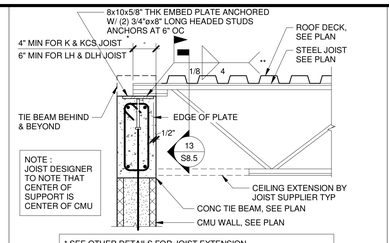
6 S8.5 DECK BRG OVER CMU WALL
1" = 1'-0"



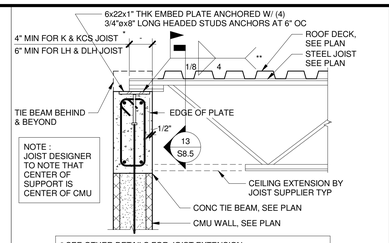
5 S8.5 JOIST BRG AT STEEL BEAM
3/4" = 1'-0"



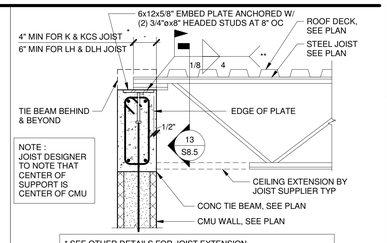
4 S8.5 JOIST END BRG ON CMU WALL
1" = 1'-0"



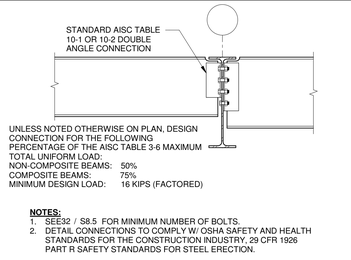
3 S8.5 JOIST END BRG ON CMU WALL
1" = 1'-0"



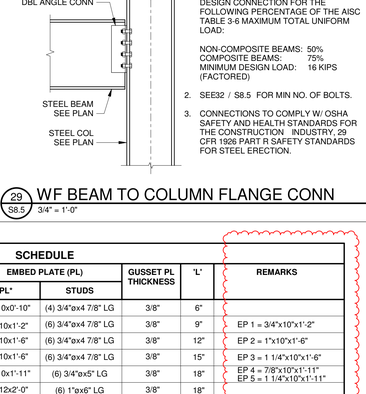
2 S8.5 JOIST END BRG ON CMU WALL
1" = 1'-0"



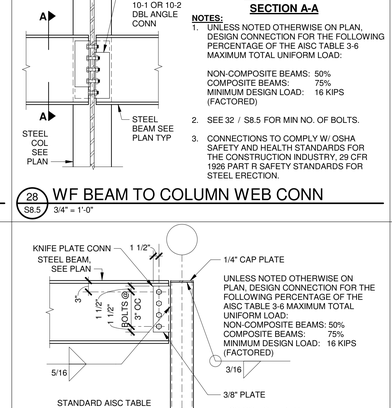
1 S8.5 JOIST END BRG ON CMU WALL
1" = 1'-0"



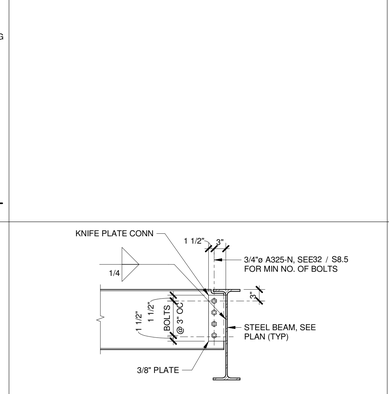
12 S8.5 STEEL BEAM TO CONCRETE BEAM
1" = 1'-0"



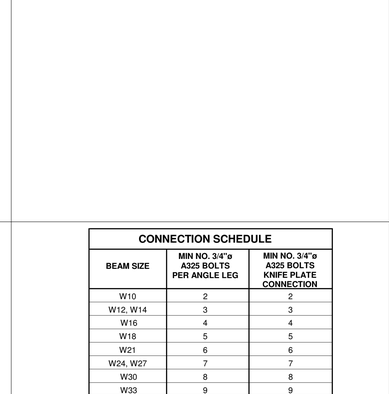
17 S8.5 DECK SUPPORT AT CONCRETE TIE BEAM
1" = 1'-0"



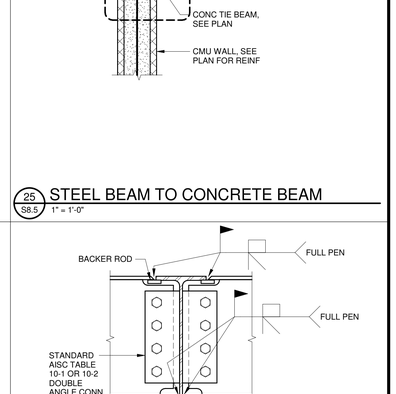
16 S8.5 JOIST END BRG AT CMU PARAPET WALL
1" = 1'-0"



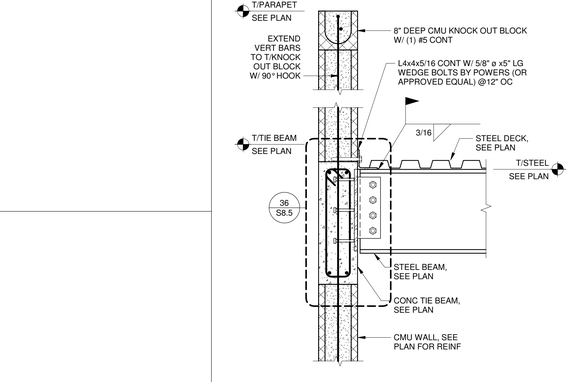
15 S8.5 DECK BRG AT CONCRETE BEAM
1" = 1'-0"



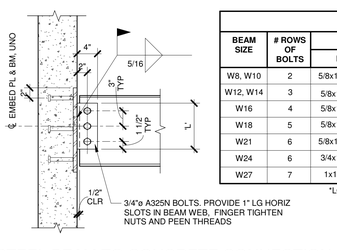
14 S8.5 SECTION
1" = 1'-0"



13 S8.5 SECTION
1" = 1'-0"



8 S8.5 JOIST BRG ON CMU WALL
1" = 1'-0"



UNLESS NOTED OTHERWISE ON PLAN, DESIGN CONNECTION FOR THE FOLLOWING PERCENTAGE OF THE AISC TABLE 3-6 MAXIMUM TOTAL UNIFORM LOAD:
NON-COMPOSITE BEAMS: 50%
COMPOSITE BEAMS: 75%
MINIMUM DESIGN LOAD: 16 KIPS (FACTORED)

18 S8.5 STEEL BEAM TO HSS COLUMN CONN
3/4" = 1'-0"

24 S8.5 STEEL BEAM TO CONCRETE BEAM
1" = 1'-0"

NOTES:
1. UNLESS NOTED OTHERWISE ON PLAN, DESIGN CONNECTION FOR THE FOLLOWING PERCENTAGE OF THE AISC TABLE 3-6 MAXIMUM TOTAL UNIFORM LOAD:
   NON-COMPOSITE BEAMS: 50%
   COMPOSITE BEAMS: 75%
   MINIMUM DESIGN LOAD: 16 KIPS (FACTORED)
2. SEE32 / S8.5 FOR MIN NO. OF BOLTS.
3. CONNECTIONS TO COMPLY W/ OSHA SAFETY AND HEALTH STANDARDS FOR THE CONSTRUCTION INDUSTRY, 29 CFR 1926 PART R SAFETY STANDARDS FOR STEEL ERECTION.

23 S8.5 WF BEAM TO COLUMN CONNECTIONS
3/4" = 1'-0"

UNLESS NOTED OTHERWISE ON PLAN, DESIGN CONNECTION FOR THE FOLLOWING PERCENTAGE OF THE AISC TABLE 3-6 MAXIMUM TOTAL UNIFORM LOAD:
NON-COMPOSITE BEAMS: 50%
COMPOSITE BEAMS: 75%
MINIMUM DESIGN LOAD: 16 KIPS (FACTORED)

NOTES:
1. SEE32 / S8.5 FOR MINIMUM NUMBER OF BOLTS.
2. DETAIL CONNECTIONS TO COMPLY W/ OSHA SAFETY AND HEALTH STANDARDS FOR THE CONSTRUCTION INDUSTRY, 29 CFR 1926 PART R SAFETY STANDARDS FOR STEEL ERECTION.

30 S8.5 WF BEAM TO BEAM CONN
3/4" = 1'-0"

NOTES:
1. UNLESS NOTED OTHERWISE ON PLAN, DESIGN CONNECTION FOR THE FOLLOWING PERCENTAGE OF THE AISC TABLE 3-6 MAXIMUM TOTAL UNIFORM LOAD:
   NON-COMPOSITE BEAMS: 50%
   COMPOSITE BEAMS: 75%
   MINIMUM DESIGN LOAD: 16 KIPS (FACTORED)
2. SEE32 / S8.5 FOR MIN NO. OF BOLTS.
3. CONNECTIONS TO COMPLY W/ OSHA SAFETY AND HEALTH STANDARDS FOR THE CONSTRUCTION INDUSTRY, 29 CFR 1926 PART R SAFETY STANDARDS FOR STEEL ERECTION.

29 S8.5 WF BEAM TO COLUMN FLANGE CONN
3/4" = 1'-0"

NOTES:
1. UNLESS NOTED OTHERWISE ON PLAN, DESIGN CONNECTION FOR THE FOLLOWING PERCENTAGE OF THE AISC TABLE 3-6 MAXIMUM TOTAL UNIFORM LOAD:
   NON-COMPOSITE BEAMS: 50%
   COMPOSITE BEAMS: 75%
   MINIMUM DESIGN LOAD: 16 KIPS (FACTORED)
2. SEE 32 / S8.5 FOR MIN NO. OF BOLTS.
3. CONNECTIONS TO COMPLY W/ OSHA SAFETY AND HEALTH STANDARDS FOR THE CONSTRUCTION INDUSTRY, 29 CFR 1926 PART R SAFETY STANDARDS FOR STEEL ERECTION.

28 S8.5 WF BEAM TO COLUMN WEB CONN
3/4" = 1'-0"

25 S8.5 STEEL BEAM TO CONCRETE BEAM
1" = 1'-0"

SCHEDULE						
BEAM SIZE	# ROWS OF BOLTS	EMBED PLATE (PL)		GUSSET PL THICKNESS	'L'	REMARKS
		PL*	STUDS			
W8, W10	2	5/8x10x0'-10"	(4) 3/4"øx4 7/8" LG	3/8"	6"	
W12, W14	3	5/8x10x1'-2"	(6) 3/4"øx4 7/8" LG	3/8"	9"	EP 1 = 3/4"x10"x1'-2"
W16	4	5/8x10x1'-6"	(6) 3/4"øx4 7/8" LG	3/8"	12"	EP 2 = 1"x10"x1'-6"
W18	5	5/8x10x1'-6"	(6) 3/4"øx4 7/8" LG	3/8"	15"	EP 3 = 1 1/4"x10"x1'-6"
W21	6	5/8x10x1'-11"	(6) 3/4"øx5" LG	3/8"	18"	EP 4 = 7/8"x10"x1'-11" EP 5 = 1 1/4"x10"x1'-11"
W24	6	3/4x12x2'-0"	(6) 1"øx6" LG	3/8"	18"	
W27	7	1x12x2'-3"	(6) 1"øx6" LG	3/8"	21"	

*LONG DIMENSION VERTICAL

8 11

36 S8.5 STEEL BEAM TO CONCRETE CONNECTION SCHEDULE
1" = 1'-0"

UNLESS NOTED OTHERWISE ON PLAN, DESIGN CONNECTION FOR THE FOLLOWING PERCENTAGE OF THE AISC TABLE 3-6 MAXIMUM TOTAL UNIFORM LOAD:
NON-COMPOSITE BEAMS: 50%
COMPOSITE BEAMS: 75%
MINIMUM DESIGN LOAD: 16 KIPS (FACTORED)

NOTES:
1. SEE32 / S8.5 FOR MIN NO. OF BOLTS.
2. SHORT-SLOTTED HOLES ARE PERMITTED IN BEAM WEB

34 S8.5 STEEL BEAM TO HSS COLUMN CONN
3/4" = 1'-0"

NOTE: SHORT-SLOTTED HOLES ARE PERMITTED IN BEAM WEB

UNLESS NOTED OTHERWISE ON PLAN, DESIGN CONNECTION FOR THE FOLLOWING PERCENTAGE OF THE AISC TABLE 3-6 MAXIMUM TOTAL UNIFORM LOAD:
NON-COMPOSITE BEAMS: 50%
COMPOSITE BEAMS: 75%
MINIMUM DESIGN LOAD: 16 KIPS (FACTORED)

33 S8.5 ALTERNATE WF BEAM TO BEAM CONN
3/4" = 1'-0"

CONNECTION SCHEDULE		
BEAM SIZE	MIN NO. 3/4"ø A325 BOLTS PER ANGLE LEG	MIN NO. 3/4"ø A325 BOLTS KNIFE PLATE CONNECTION
W10	2	2
W12, W14	3	3
W16	4	4
W18	5	5
W21	6	6
W24, W27	7	7
W30	8	8
W33	9	9
W36	10	10
W40	11	11
W44	12	12

MIN RQMTS FOR WF BM TO COL & BM TO BM CONNECTION SCHEDULE

32 S8.5 SECTION
3/4" = 1'-0"

31 S8.5 GRAVITY BEAM MOMENT CONN
1 1/2" = 1'-0"

CODE REVIEW:

CERTIFICATE:

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONADO AR0012456
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JOHN F. WRIGHT AR107341

SEAL:

CONSULTANTS:

TRC WORLDWIDE ENGINEERING
1230 N. University Dr.
PLANTATION, FL 33322
PHONE: (954) 474-7777
FAX: (954) 484-7834

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Certificate of Authorization No. 35826

TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

△	REVISION NAME	DATE
4	ADDENDUM 4	01.27.2025
8	ASI #8	06.19.2025
11	ASI #11	09.29.2025

FOR BID:

DRAWING TITLE:

TYPICAL STEEL DETAILS

COMMISSION NUMBER:
24FTL018

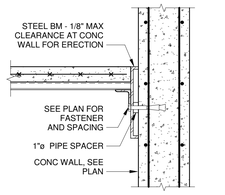
DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S8.5

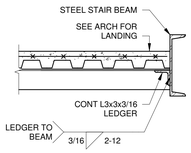
THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"

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2025-09-29 7:11:41 PM
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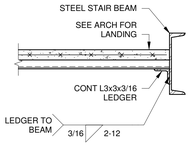


NOTE: STEEL BM CONNECTION APPLIES ALSO TO STAIR STRINGER PORTION OF CRIPPLE BEAM SB1

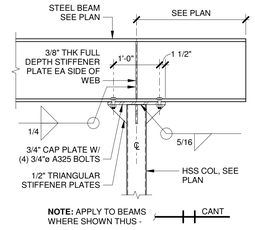
6 STAIR BEAM AT CONC COLUMN OR WALL
S8.6 1" = 1'-0"



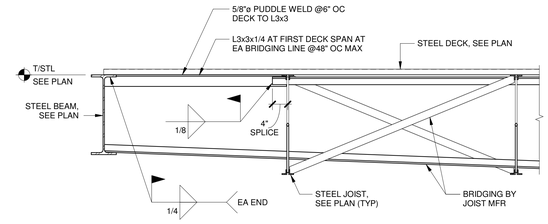
5 STAIR LANDING AT STEEL BEAM
S8.6 1" = 1'-0"



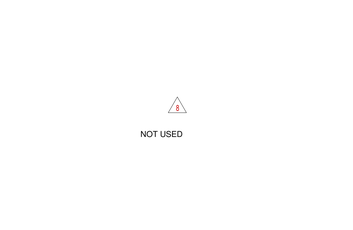
4 STAIR LANDING AT STEEL BEAM
S8.6 1" = 1'-0"



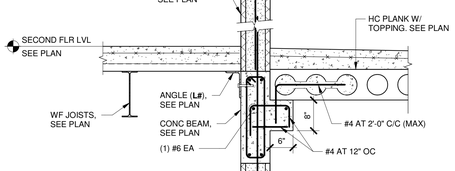
3 WF BEAM CANT AT HSS COLUMN
S8.6 3/4" = 1'-0"



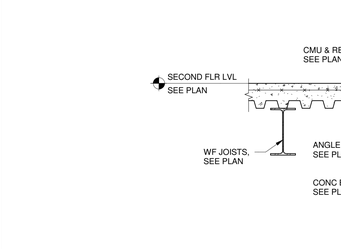
2 BRIDGING TERMINATION AT WF BEAM
S8.6 3/4" = 1'-0"



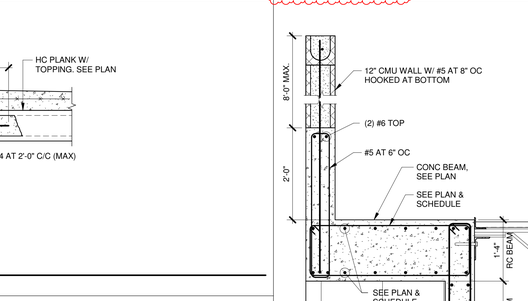
12 JOIST TO HSS COLUMN CONN
S8.6 3/4" = 1'-0"



11 EDGE ANGLE SPLICE DETAIL
S8.6 1 1/2" = 1'-0"

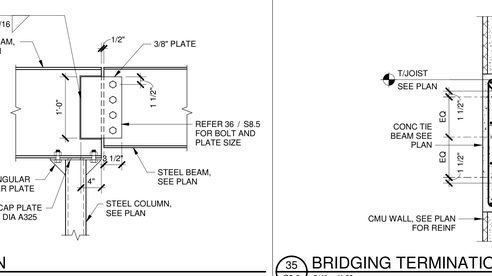


9 JOIST EXTENSION AT STEEL BEAM
S8.6 3/4" = 1'-0"

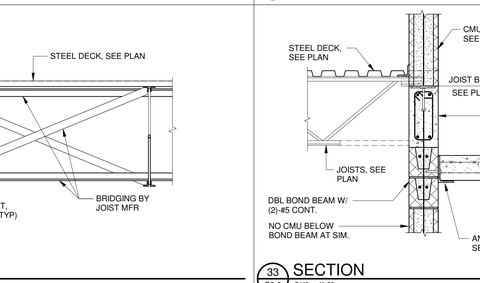


13

13 SECTION
S8.6 3/4" = 1'-0"



18 STEEL STAIR CONNECTION AT LANDING
S8.6 3/4" = 1'-0"



16 JOIST BRACING AT STEEL COLUMN
S8.6 3/4" = 1'-0"

20 JOIST BEARING AT STEEL BEAM
S8.6 3/4" = 1'-0"

8

NOT USED

8 NOT USED
S8.6 3/4" = 1'-0"

24 SECTION
S8.6 3/4" = 1'-0"

23 SECTION
S8.6 3/4" = 1'-0"

22 GIRDER SUPPORT AT CONC COLUMN
S8.6 3/4" = 1'-0"

21 BEAM SUPPORT AT CMU WALL
S8.6 3/4" = 1'-0"

13

19 SECTION
S8.6 3/4" = 1'-0"

36 SECTION
S8.6 1" = 1'-0"

35 BRIDGING TERMINATION AT CONC BEAM
S8.6 3/4" = 1'-0"

33 SECTION
S8.6 3/4" = 1'-0"

32 SECTION
S8.6 3/4" = 1'-0"

31 SECTION
S8.6 3/4" = 1'-0"

CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
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Certificate of Authorization No. 35826

TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

Δ	REVISION NAME	DATE
4	ADDENDUM 4	01.27.2025
8	ASI #8	06.19.2025
11	ASI #11	09.29.2025
13	ASI #17	03.06.2026

FOR BID:

DRAWING TITLE:

TYPICAL STEEL DETAILS

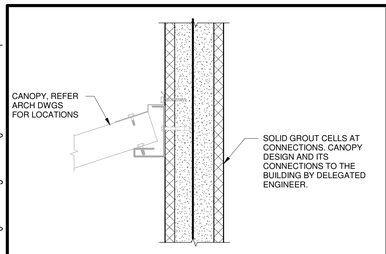
COMMISSION NUMBER:
24FTL018

DO NOT SCALE THE DRAWINGS

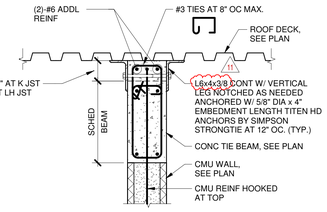
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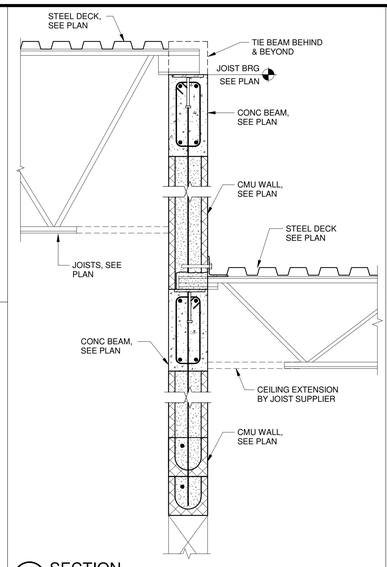
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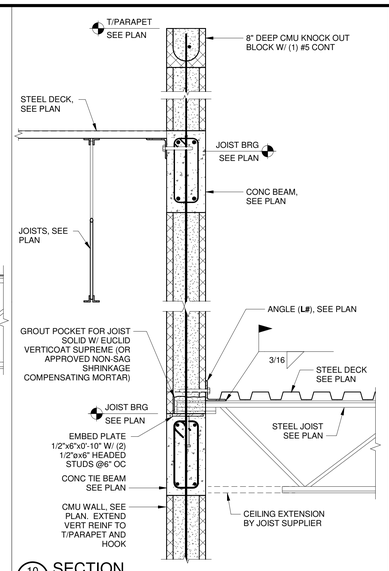
6 S8.7 SECTION
1 1/2" = 1'-0"



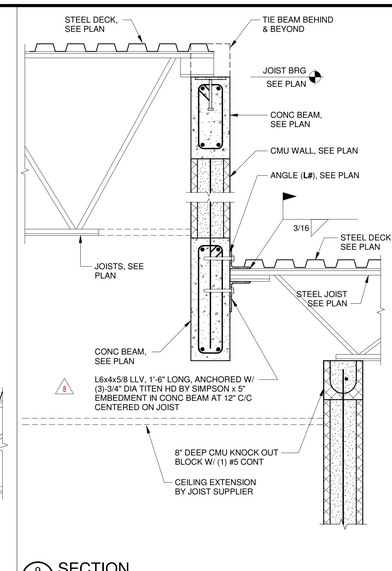
DECK BEARING DETAIL AT INTERNAL BEARING WALL
12 S8.7 1" = 1'-0"



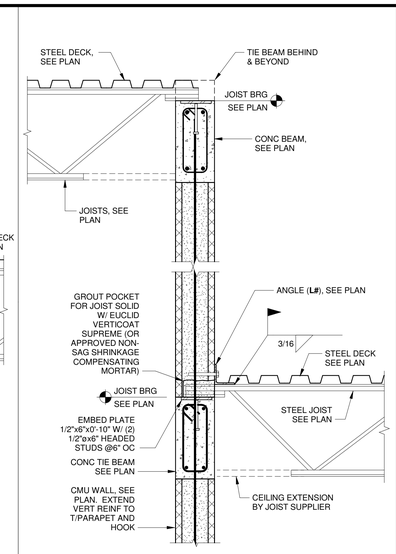
11 S8.7 SECTION
1" = 1'-0"



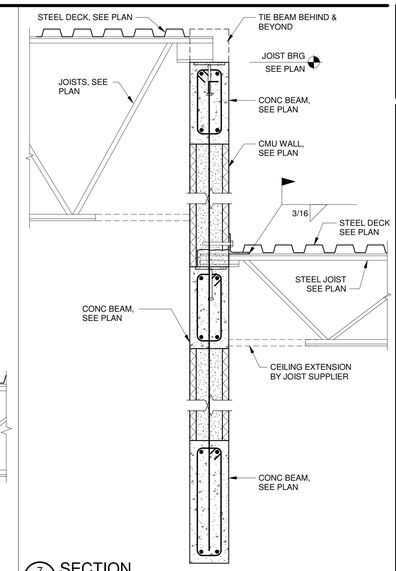
10 S8.7 SECTION
1" = 1'-0"



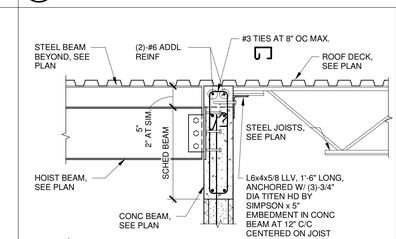
9 S8.7 SECTION
1" = 1'-0"



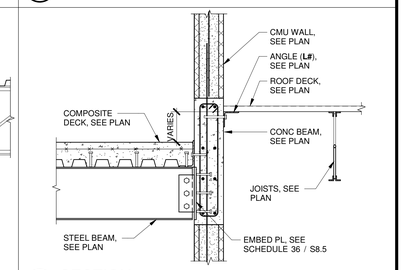
8 S8.7 SECTION
1" = 1'-0"



7 S8.7 SECTION
1" = 1'-0"



14 S8.7 SECTION
3/4" = 1'-0"



13 S8.7 SECTION
3/4" = 1'-0"

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TYPICAL STEEL DETAILS

COMMISSION NUMBER:
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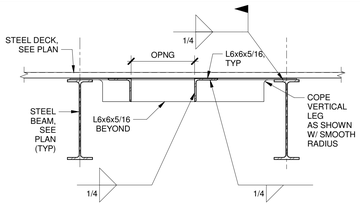
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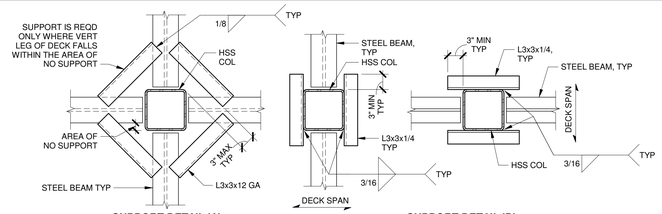
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NOTE: PROVIDE ABOVE FRAMING AROUND ALL OPENINGS INDICATED ON STRUCTURAL, ARCHITECTURAL, AND MECH DRAWINGS. CONTRACTOR IS RESPONSIBLE FOR COORDINATING OPENING SIZES W/ PURCHASED EQUIPMENT.

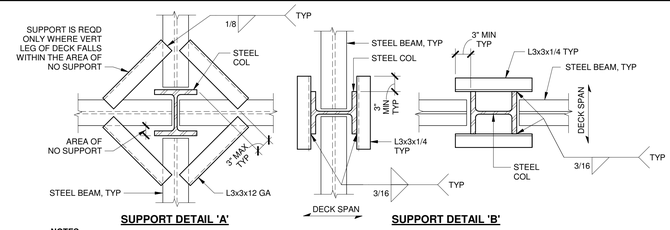
6 S8.8 ROOF OPENING DETAIL
3/4" = 1'-0"



SUPPORT DETAIL 'A' **SUPPORT DETAIL 'B'**

NOTES:
1. FOR COMPOSITE STEEL DECK W/ CONCRETE, CONTRACTOR MAY OMIT ANGLES. SHORE AS REQUIRED DURING CONCRETE PLACEMENT.
2. PROVIDE ANGLES AS REQUIRED WHERE BEAMS DO NOT FRAME TO COLUMNS.

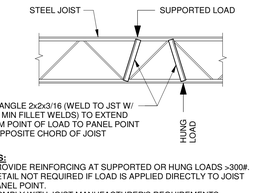
5 S8.8 DECK SUPPORT AT HSS COLUMN
1" = 1'-0"



SUPPORT DETAIL 'A' **SUPPORT DETAIL 'B'**

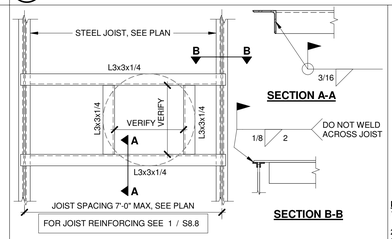
NOTES:
1. FOR COMPOSITE STEEL DECK W/ CONCRETE, CONTRACTOR MAY OMIT ANGLES. SHORE AS REQUIRED DURING CONCRETE PLACEMENT.
2. PROVIDE ANGLES AS REQUIRED WHERE BEAMS DO NOT DO FRAME TO COLUMNS.

3 S8.8 DECK SUPPORT AT WF COLUMN
1" = 1'-0"

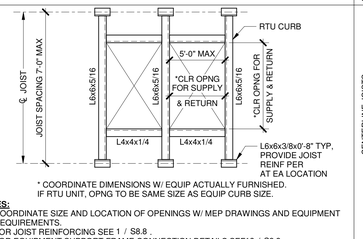


NOTES:
1. PROVIDE REINFORCING AT SUPPORTED OR HUNG LOADS >300#.
2. DETAIL NOT REQUIRED IF LOAD IS APPLIED DIRECTLY TO JOIST PANEL POINT.
3. COMPLY WITH JOIST MANUFACTURER'S REQUIREMENTS.

1 S8.8 JOIST REINFORCING AT POINT LOADS
3/4" = 1'-0"



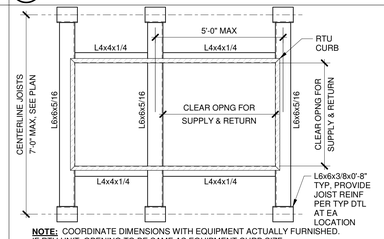
11 S8.8 ROOF DRAIN SUPPORT FRAMING AT STEEL JOISTS
3/4" = 1'-0"



* COORDINATE DIMENSIONS W/ EQUIP ACTUALLY FURNISHED. IF RTU UNIT, OPNG TO BE SAME SIZE AS EQUIP CURB SIZE.

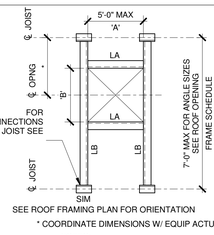
NOTES:
1. COORDINATE SIZE AND LOCATION OF OPENINGS W/ MEP DRAWINGS AND EQUIPMENT REQUIREMENTS.
2. FOR JOIST REINFORCING SEE 1 / S8.8 .
3. FOR EQUIPMENT SUPPORT FRAME CONNECTION DETAILS SEE16 / S8.8 .

10 S8.8 ROOF OPENING / EQUIPMENT SUPPORT FRAME
3/4" = 1'-0"



NOTE: COORDINATE DIMENSIONS WITH EQUIPMENT ACTUALLY FURNISHED. IF RTU UNIT, OPENING TO BE SAME AS EQUIPMENT CURB SIZE.

9 S8.8 ROOF OPENING/EQUIPMENT SUPPORT FRAME
1/2" = 1'-0"



ROOF OPENING FRAME SCHEDULE			
DIM 'A'	DIM 'B'	L-SIZE 'A'	L-SIZE 'B'
2'-0"	4'-0"	L3x3x1/4	L3x3x1/4
3'-0"	5'-0"	L3x3x1/4	L3x3x5/16
4'-0"	6'-0"	L3x3x1/4	L4x4x5/16
5'-0"	7'-0"	L3x3x1/4	L6x6x5/16

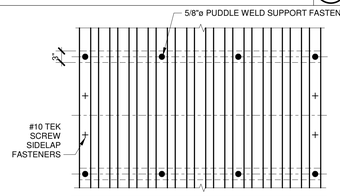
NOTE: WHEN JOIST BRIDGING INTERFERES W/ ROOF OPNG FRAMES, STOP BRIDGING AT EA SIDE OF OPNG AND 'X' BRIDGE LAST BRIDGING SPACE EA SIDE OF OPNG. ADD ADDL BRIDGING W/ 'X' BRIDGING AND HORIZONTAL BRIDGING ON EA SIDE OF CUT BRIDGING AREA. EXTEND ADDED BRIDGING PAST CUT BRIDGING

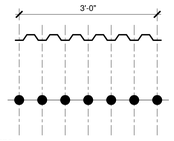
SEE ROOF FRAMING PLAN FOR ORIENTATION

* COORDINATE DIMENSIONS W/ EQUIP ACTUALLY FURNISHED. IF RTU UNIT, OPNG TO BE SAME SIZE AS EQUIP CURB SIZE.

NOTES:
1. COORDINATE SIZE AND LOCATION OF OPENINGS W/ MEP DRAWINGS AND EQUIPMENT REQUIREMENTS.
2. FOR JOIST REINFORCING SEE 1 / S8.8 .

8 S8.8 ROOF OPENING / EQUIP SUPPORT FRAME
3/4" = 1'-0"

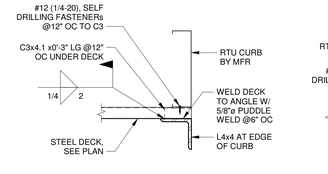




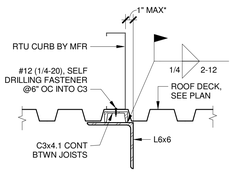
SCHEDULE		
MARK	# OF WELDS AT SUPPORTS	# OF SIDE LAP FASTENERS BETWEEN SUPPORTS
36/7	(7) 5/8" DIA WELDS	(10) SIDELAP #10 SCREWS

NOTES:
1. DECK SHALL BE ATTACHED TO ALL STRUCTURAL SUPPORTS W/ 5/8"ø PUDDLE WELDS.
2. SIDE LAPS BETWEEN STRUCTURAL SUPPORTS SHALL BE FASTENED BY #10 TEK SCREWS.
3. DECK SHALL BE ATTACHED TO ALL PERIMETER SUPPORTS WITH 5/8"ø PUDDLE WELDS AT 6" OC MAX.
4. END LAPS SHALL BE A MINIMUM OF 3" AND SHALL OCCUR OVER SUPPORTS.

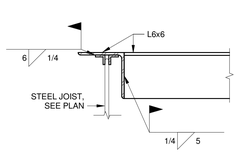
18 S8.8 ROOF DECK ATTACHMENT SCHEDULE
3/4" = 1'-0"



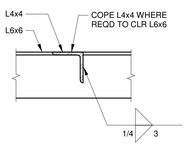
CURB AND DECK SUPPORT



CURB AT SIDE DECK BEARING

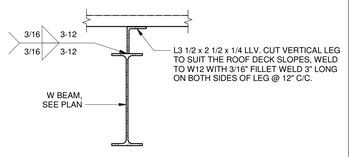


ANGLE TO JOIST CONNECTION



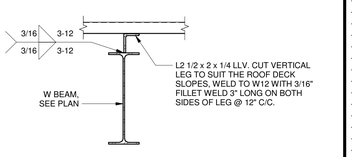
ANGLE TO ANGLE CONNECTION

16 S8.8 ROOF OPENING/EQUIPMENT SUPPORT FRAME CONNECTION DETAILS
1 1/2" = 1'-0"



AT W12x16

20 S8.8 DECK SUPPORT SECTION
1 1/2" = 1'-0"



AT W12x14

19 S8.8 DECK SUPPORT SECTION
1 1/2" = 1'-0"

11

CODE REVIEW:

CERTIFICATE:

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

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FOR BID:

DRAWING TITLE:

TYPICAL ROOF DETAILS

COMMISSION NUMBER:
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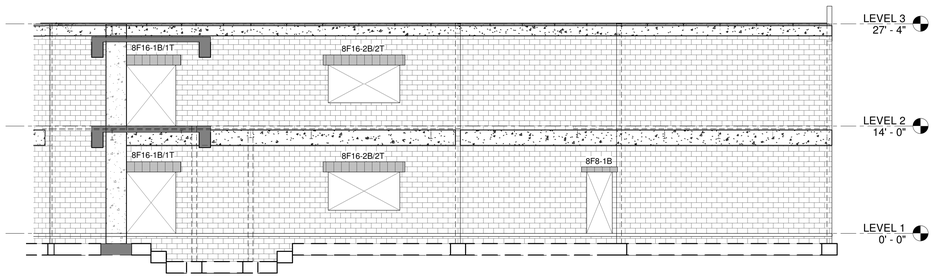
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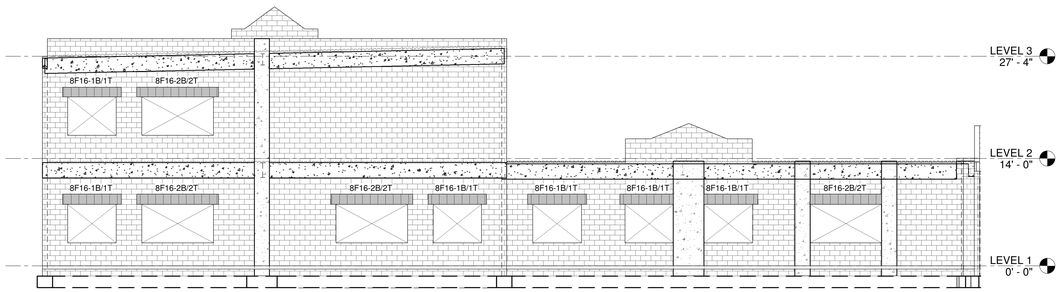
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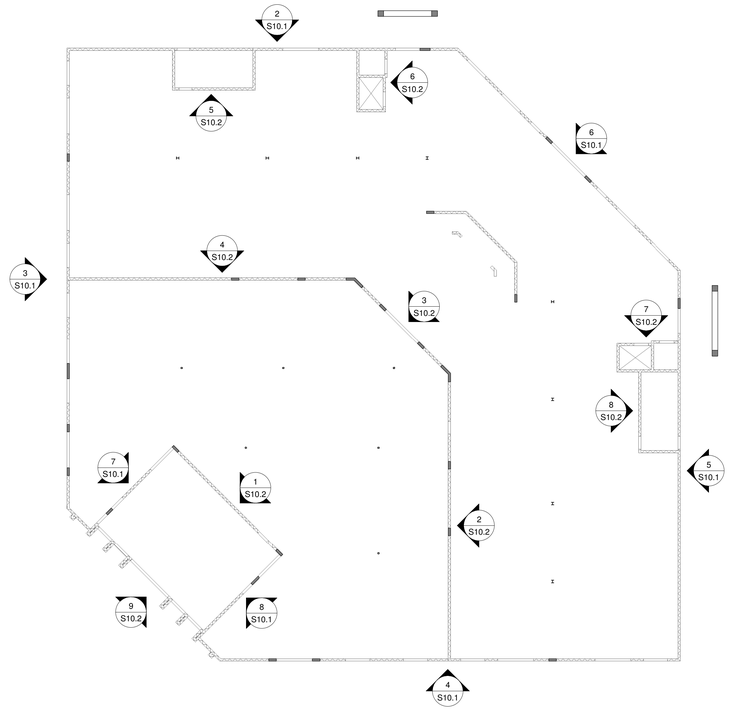
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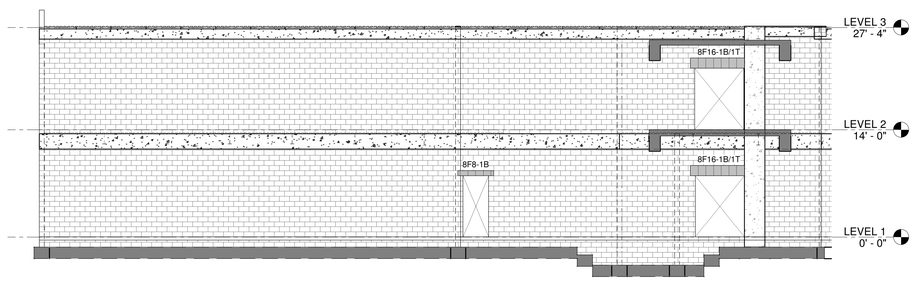
2 S10.1 BUILDING ELEVATION
1/8" = 1'-0"



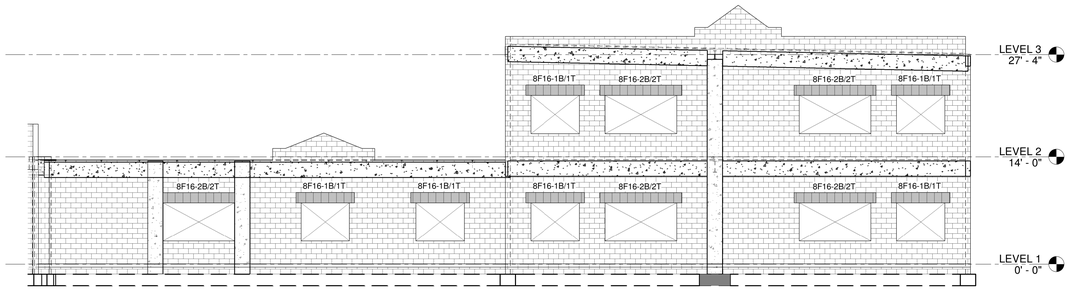
3 S10.1 BUILDING ELEVATION
1/8" = 1'-0"



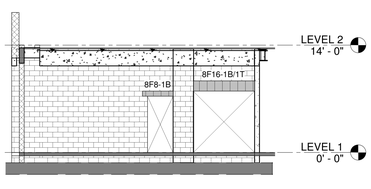
1 S10.1 WALL ELEVATION PLAN- ADMINISTRATION - BUILDING 1
1/16" = 1'-0"



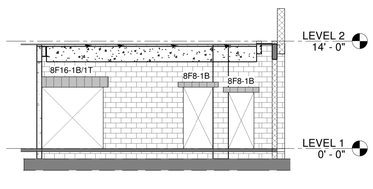
5 S10.1 BUILDING ELEVATION
1/8" = 1'-0"



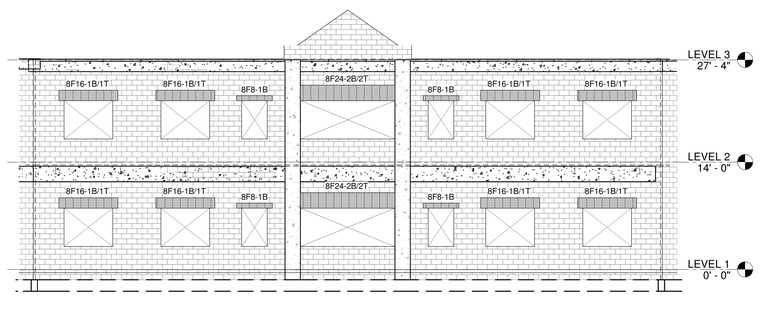
4 S10.1 BUILDING ELEVATION
1/8" = 1'-0"



8 S10.1 BUILDING ELEVATION
1/8" = 1'-0"



7 S10.1 BUILDING ELEVATION
1/8" = 1'-0"



6 S10.1 BUILDING ELEVATION
1/8" = 1'-0"

NOTE:
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CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONADO AR0012456
STEVEN G. SIEGEL AR99546
JOHN F. WRIGHT AR107341

SEAL:

CN=Nitin R Ranadive, oid=Qualifier=A01410D000000018DC34549AA0004BD39, O=Florida, C=US
Nitin Ranadive, P.E., No.81459
TRC Worldwide Engineering, dba Jenkins and Charland, INC.
2025.02.12 11:17:56-05'00'

NITIN RANADIVE LICENSE No. 81459 STATE OF FLORIDA PROFESSIONAL ENGINEER

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CONSULTANTS:

TRC
WORLDWIDE ENGINEERING
1230 N. University Dr.
PLANTATION, FL 33322
PHONE: (954) 484-7777
FAX: (954) 484-7834

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TO THE BEST OF MY KNOWLEDGE AND ABILITY. THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

#	REVISION NAME	DATE
4	ADDENDUM 4	01.27.2025

FOR BID:

DRAWING TITLE:

BUILDING ELEVATION - ADMINISTRATION - BUILDING 1

COMMISSION NUMBER:
24FTL018

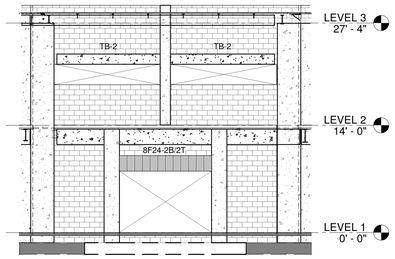
DO NOT SCALE THE DRAWINGS

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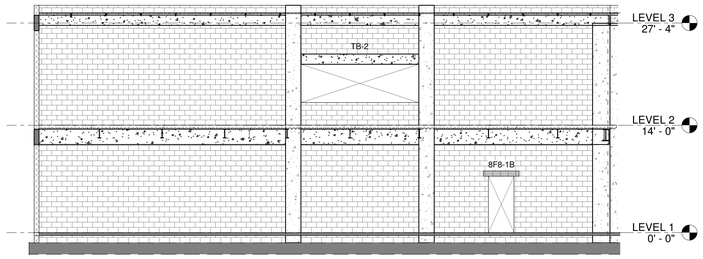
S10.1

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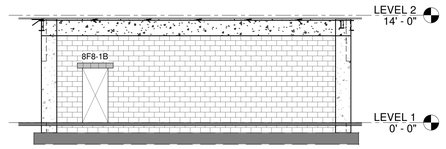
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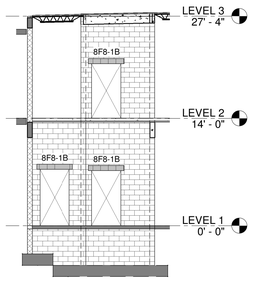
3 S10.2 BUILDING ELEVATION
1/8" = 1'-0"



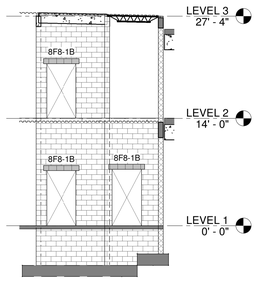
2 S10.2 BUILDING ELEVATION
1/8" = 1'-0"



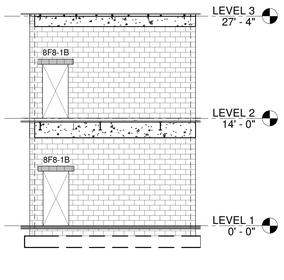
1 S10.2 BUILDING ELEVATION
1/8" = 1'-0"



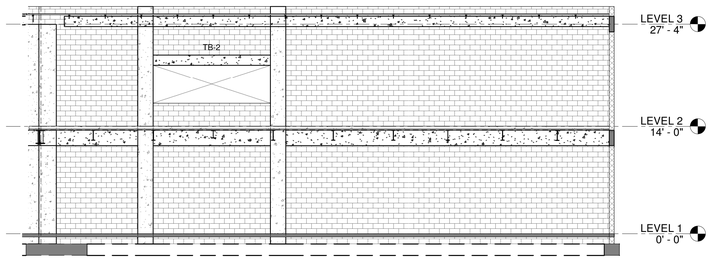
7 S10.2 BUILDING ELEVATION
1/8" = 1'-0"



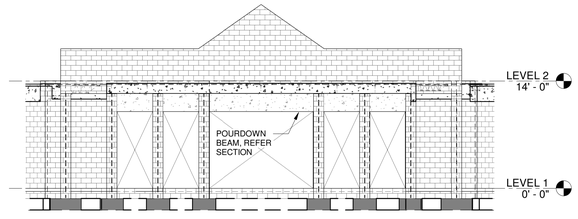
6 S10.2 BUILDING ELEVATION
1/8" = 1'-0"



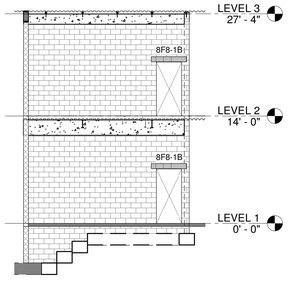
5 S10.2 BUILDING ELEVATION
1/8" = 1'-0"



4 S10.2 BUILDING ELEVATION
1/8" = 1'-0"



9 S10.2 BUILDING ELEVATION
1/8" = 1'-0"



8 S10.2 BUILDING ELEVATION
1/8" = 1'-0"

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CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONOHO AR0012456
STEVEN G. SIEGEL AR99546
JOHN F. WRIGHT AR107341

SEAL:

CN=Nitin R Ranadive, dnQualifier=A01410D00000018DC34549AA0004BD39, O=Florida, C=US
Nitin Ranadive, P.E., No.81459
TRC Worldwide Engineering, dba Jenkins and Charland, INC.
2025.02.12 11:18:15-05'00'

NITIN RANADIVE LICENSE No. 81459 STATE OF FLORIDA PROFESSIONAL ENGINEER

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CONSULTANTS:

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1230 N. University Dr.
PLANTATION, FL 33322
PHONE: (954) 484-7777
FAX: (954) 484-7834

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Certificate of Authorization No. 35826

TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

	REVISION NAME	DATE
1		

FOR BID:

DRAWING TITLE:

BUILDING ELEVATION - ADMINISTRATION - BUILDING 1

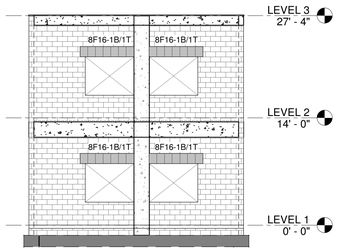
COMMISSION NUMBER:
24FTL018

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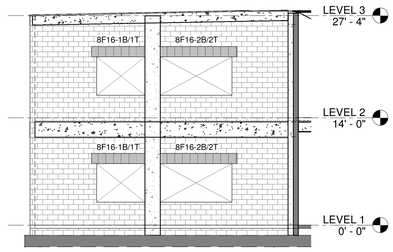
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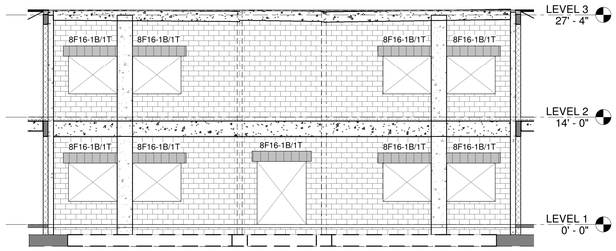
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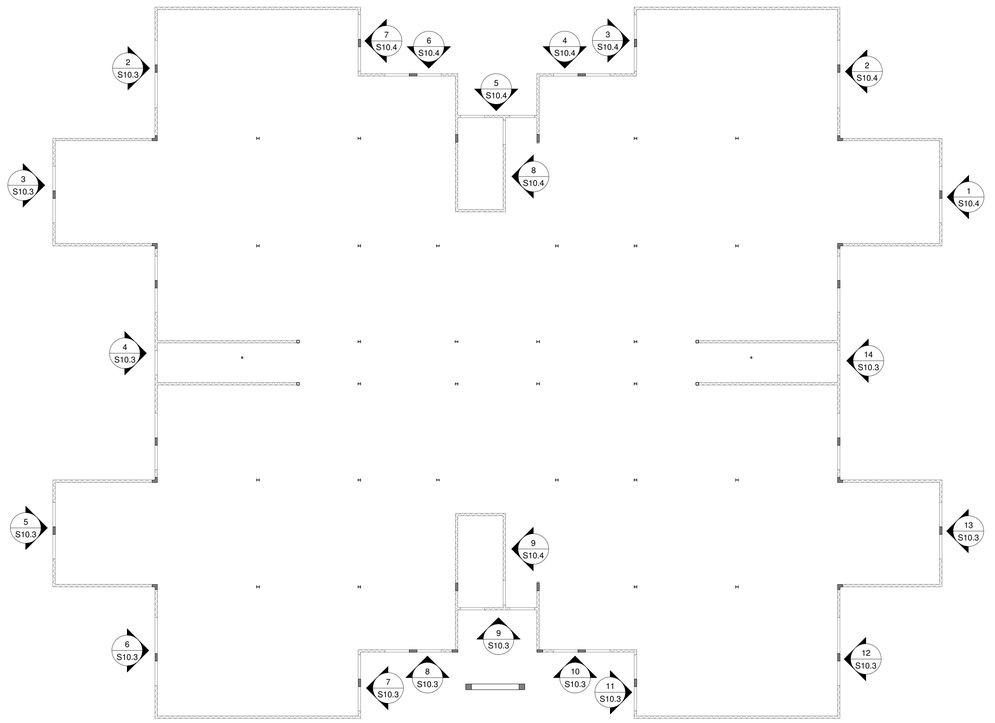
3 S10.3 BUILDING ELEVATION
1/8" = 1'-0"



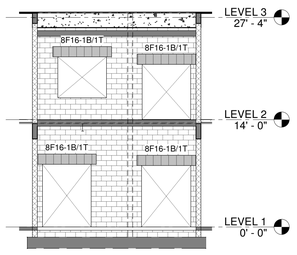
2 S10.3 BUILDING ELEVATION
1/8" = 1'-0"



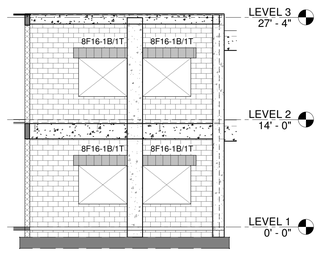
4 S10.3 BUILDING ELEVATION
1/8" = 1'-0"



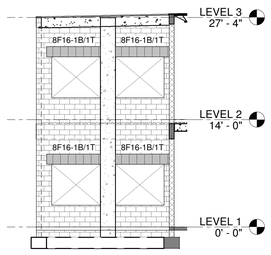
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1/16" = 1'-0"



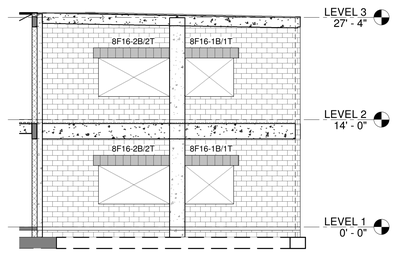
9 S10.3 BUILDING ELEVATION
1/8" = 1'-0"



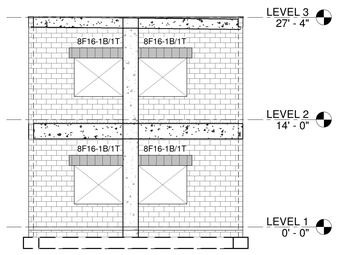
8 S10.3 BUILDING ELEVATION
1/8" = 1'-0"



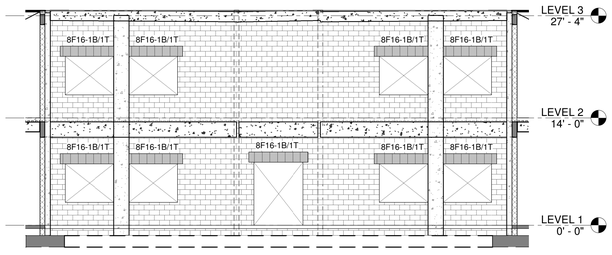
7 S10.3 BUILDING ELEVATION
1/8" = 1'-0"



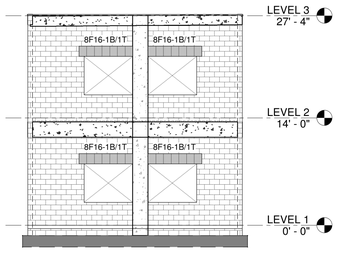
6 S10.3 BUILDING ELEVATION
1/8" = 1'-0"



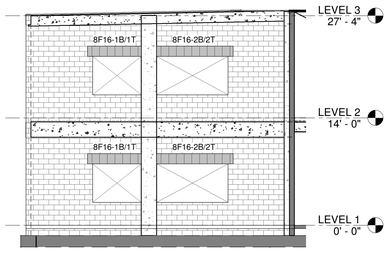
5 S10.3 BUILDING ELEVATION
1/8" = 1'-0"



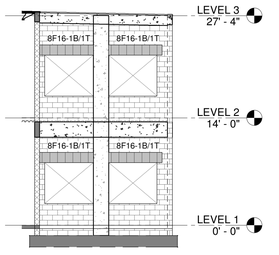
14 S10.3 BUILDING ELEVATION
1/8" = 1'-0"



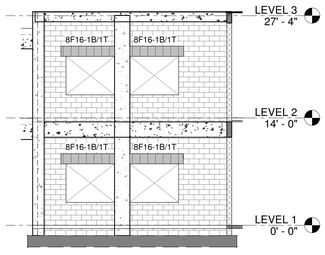
13 S10.3 BUILDING ELEVATION
1/8" = 1'-0"



12 S10.3 BUILDING ELEVATION
1/8" = 1'-0"



11 S10.3 BUILDING ELEVATION
1/8" = 1'-0"



10 S10.3 BUILDING ELEVATION
1/8" = 1'-0"

NOTE:
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spiezle

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2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
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ANTHONY DONADO AR0012456
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JOHN F. WRIGHT AR107341

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CN=Nitin R Ranadive, dnQualifier=A01410D0000018DC34549AA0004BD3, O=Florida, C=US
Nitin Ranadive, P.E., No.81459
TRC Worldwide Engineering, dba Jenkins and Charland, INC.
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NITIN RANADIVE
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No. 81459
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FOR CODE REVIEW:

REVISIONS:

	REVISION NAME	DATE
1		

FOR BID:

DRAWING TITLE:

BUILDING ELEVATION - CLASSROOM - BUILDING 2

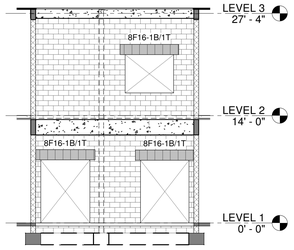
COMMISSION NUMBER:
24FTL018

DO NOT SCALE THE DRAWINGS

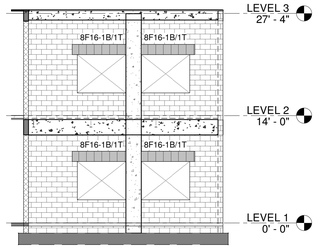
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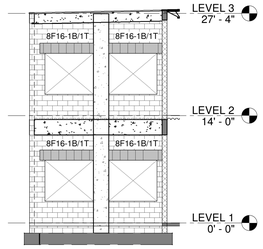
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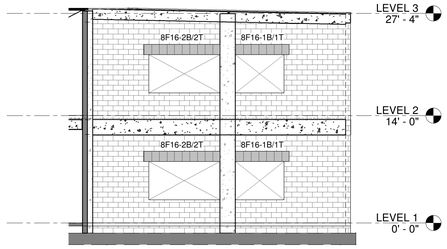
5 S10.4 BUILDING ELEVATION
1/8" = 1'-0"



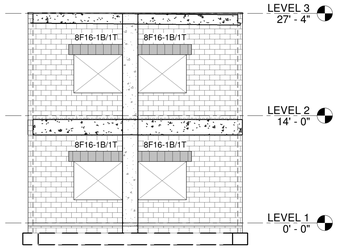
4 S10.4 BUILDING ELEVATION
1/8" = 1'-0"



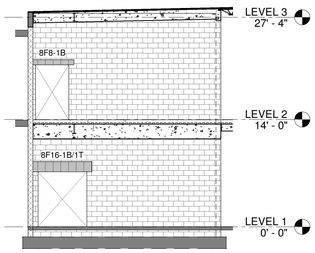
3 S10.4 BUILDING ELEVATION
1/8" = 1'-0"



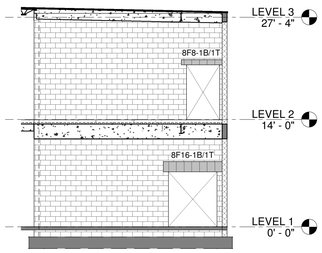
2 S10.4 BUILDING ELEVATION
1/8" = 1'-0"



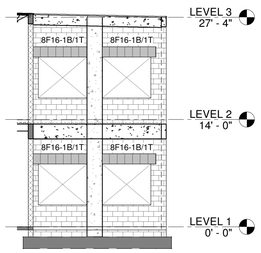
1 S10.4 BUILDING ELEVATION
1/8" = 1'-0"



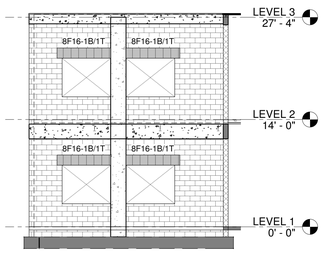
9 S10.4 BUILDING ELEVATION
1/8" = 1'-0"



8 S10.4 BUILDING ELEVATION
1/8" = 1'-0"



7 S10.4 BUILDING ELEVATION
1/8" = 1'-0"



6 S10.4 BUILDING ELEVATION
1/8" = 1'-0"

NOTE:
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CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
2001 9TH AVENUE
SUITE 308
VERO BEACH, FL 32960
PHONE: 772-794-2929

SIGNATURE:
ANTHONY DONADO AR0012456
STEVEN G. SIEGEL AR99546
JOHN F. WRIGHT AR107341

SEAL:

CN=Nitin R Ranadive, dnQualifier=A01410D0000018DC34549AA00004BD39, O=Florida, C=US
Nitin Ranadive, P.E., No.81459
TRC Worldwide Engineering, dba Jenkins and Charland, INC.
2025.02.12 11:19:08-05'00'

NITIN RANADIVE
LICENSE
No. 81459
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

This item has been electronically signed and sealed by Nitin Ranadive, PE on the date adjacent to the seal.

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CONSULTANTS:

TRC
WORLDWIDE ENGINEERING
1230 N. University Dr.
PLANTATION, FL 33322
PHONE: (954) 484-7777
FAX: (954) 484-7834

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Certificate of Authorization No. 35826

TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

	REVISION NAME	DATE

FOR BID:

DRAWING TITLE:

BUILDING ELEVATION - CLASSROOM - BUILDING 2

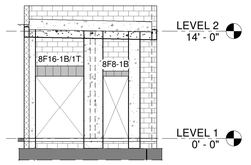
COMMISSION NUMBER:
24FTL018

DO NOT SCALE THE DRAWINGS

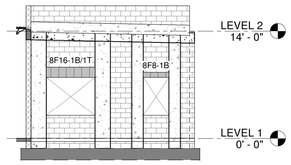
DRAWING NUMBER:

S10.4

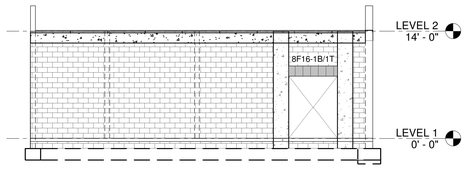
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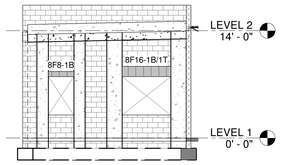
4 S10.5 BUILDING ELEVATION
1/8" = 1'-0"



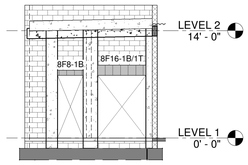
3 S10.5 BUILDING ELEVATION
1/8" = 1'-0"



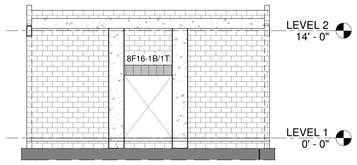
2 S10.5 BUILDING ELEVATION
1/8" = 1'-0"



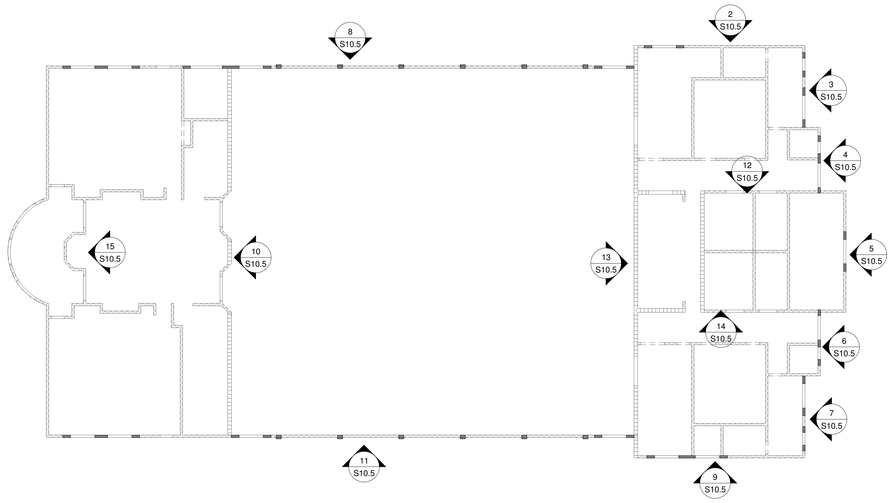
7 S10.5 BUILDING ELEVATION
1/8" = 1'-0"



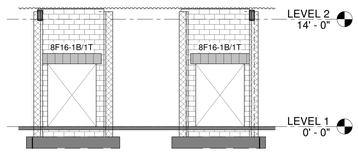
6 S10.5 BUILDING ELEVATION
1/8" = 1'-0"



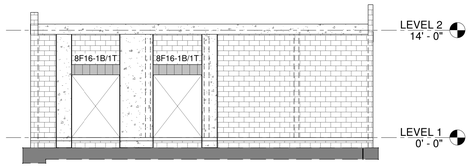
5 S10.5 BUILDING ELEVATION
1/8" = 1'-0"



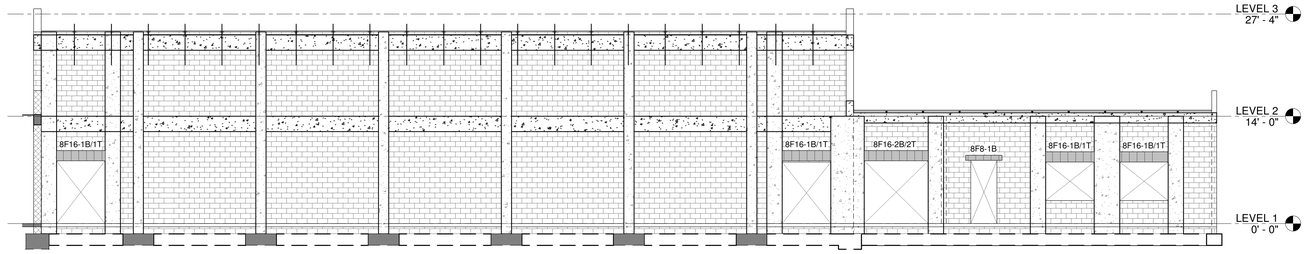
1 S10.5 WALL ELEVATION- GYM - BUILDING 3
1/16" = 1'-0"



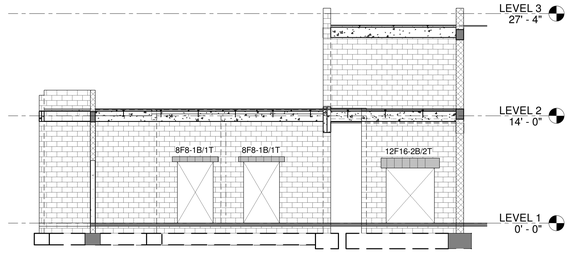
10 S10.5 BUILDING ELEVATION
1/8" = 1'-0"



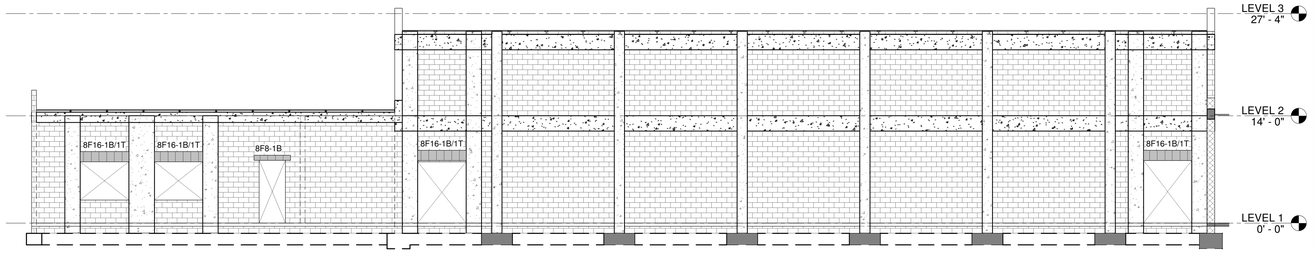
9 S10.5 BUILDING ELEVATION
1/8" = 1'-0"



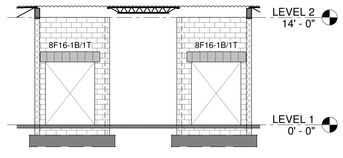
8 S10.5 BUILDING ELEVATION
1/8" = 1'-0"



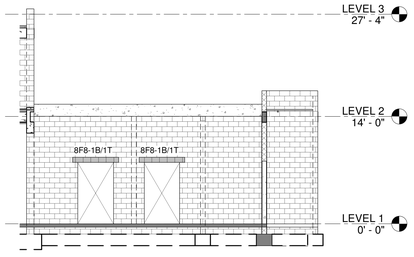
12 S10.5 BUILDING ELEVATION
1/8" = 1'-0"



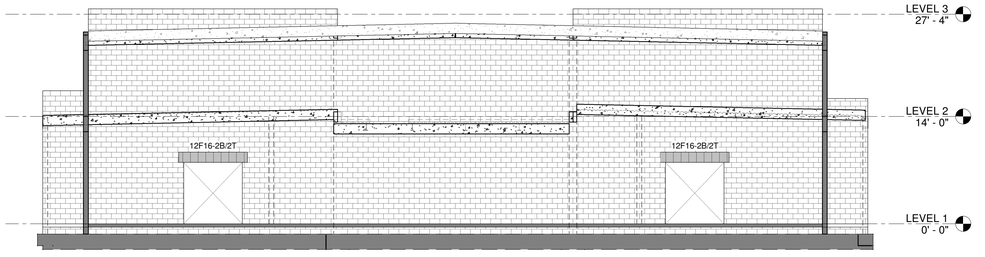
11 S10.5 BUILDING ELEVATION
1/8" = 1'-0"



15 S10.5 BUILDING ELEVATION
1/8" = 1'-0"



14 S10.5 BUILDING ELEVATION
1/8" = 1'-0"



13 S10.5 BUILDING ELEVATION
1/8" = 1'-0"

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spiezle

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STEVEN G. SIEGEL AR99546
JOHN F. WYNDHT AR107341

SEAL:

CN=Nitin R Ranadive, dnQualifier=A01410D0000018DC34549AA0004BD3, O=Florida, C=US
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2025.02.12 11:19:57-05'00'

NITIN RANADIVE LICENSE No. 81459 STATE OF FLORIDA PROFESSIONAL ENGINEER

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Certificate of Authorization No. 35826

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FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

	REVISION NAME	DATE

FOR BID:

DRAWING TITLE:

BUILDING ELEVATION - GYM - BUILDING 3

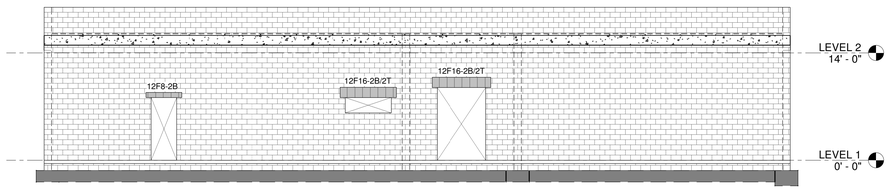
COMMISSION NUMBER:
24FTL018

DO NOT SCALE THE DRAWINGS

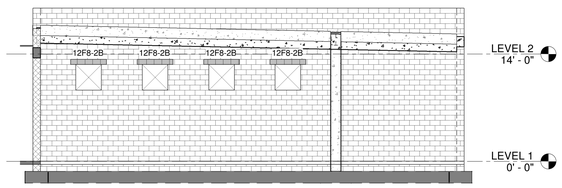
DRAWING NUMBER:

S10.5

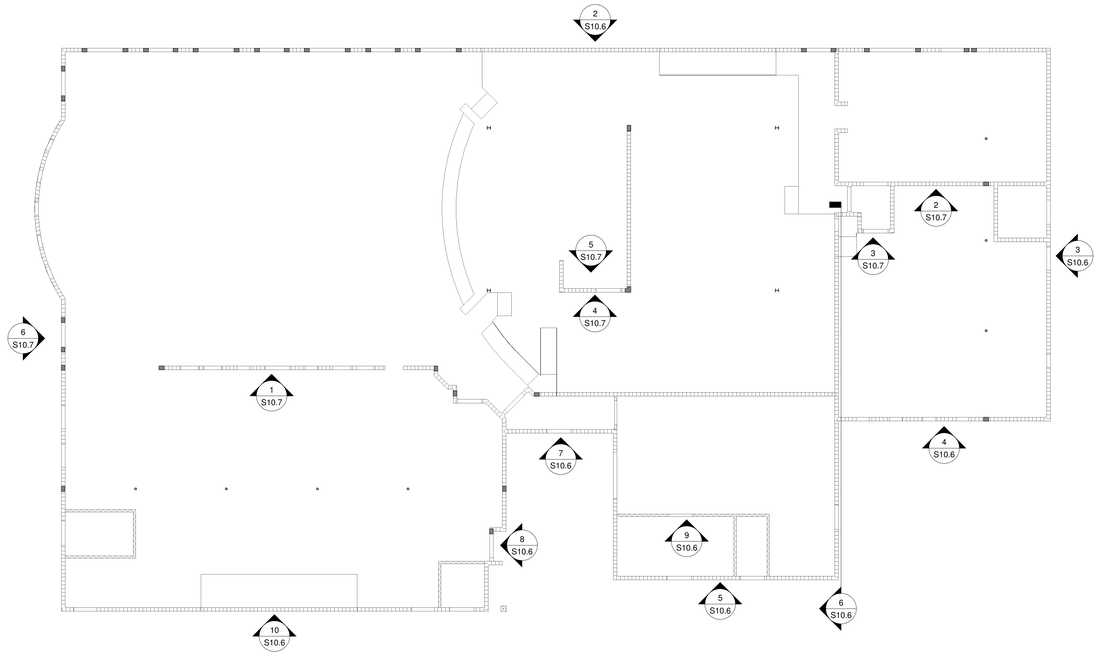
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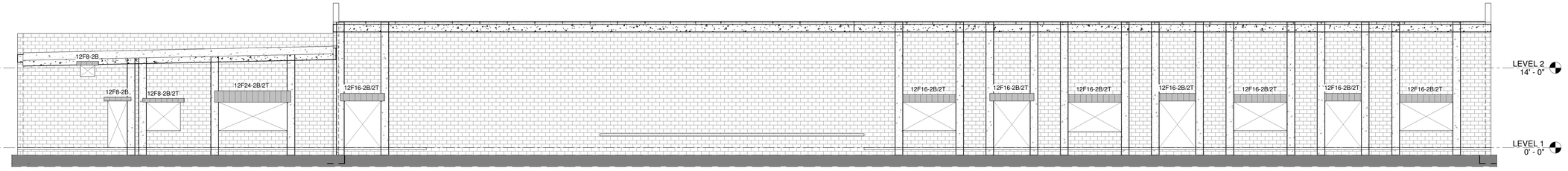
3 S10.6 BUILDING ELEVATION
1/8" = 1'-0"



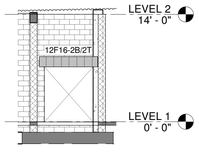
4 S10.6 BUILDING ELEVATION
1/8" = 1'-0"



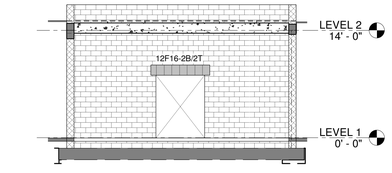
1 S10.6 WALL ELEVATION- AUDETERIA - BUILDING 5
1/16" = 1'-0"



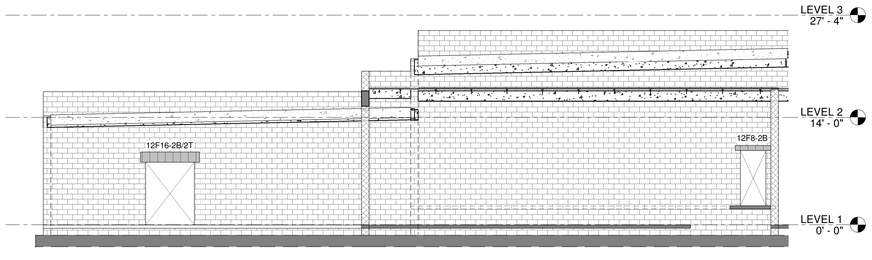
2 S10.6 BUILDING ELEVATION
1/8" = 1'-0"



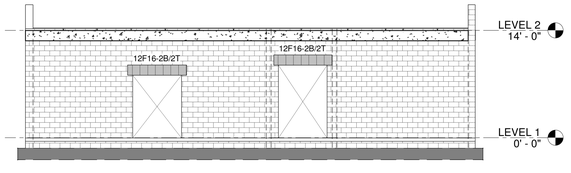
8 S10.6 BUILDING ELEVATION
1/8" = 1'-0"



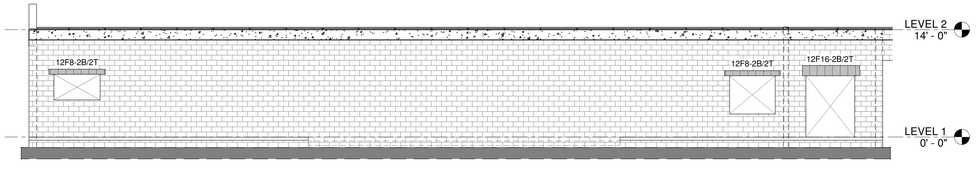
7 S10.6 BUILDING ELEVATION
1/8" = 1'-0"



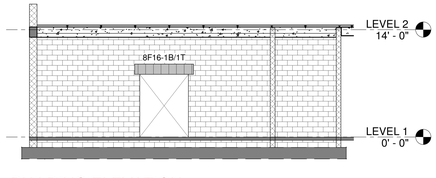
6 S10.6 BUILDING ELEVATION
1/8" = 1'-0"



5 S10.6 BUILDING ELEVATION
1/8" = 1'-0"



10 S10.6 BUILDING ELEVATION
1/8" = 1'-0"



9 S10.6 BUILDING ELEVATION
1/8" = 1'-0"

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spiezle

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SEAL:

CN=Nitin R Ranadive, dnQualifier=A01410D000 0018DC34549AA0004BD 39,O=Florida, C=US Nitin Ranadive, P.E., No.81459 TRC Worldwide Engineering, dba Jenkins and Charland, INC. 2025.02.12 11:20:24-05'00'

NITIN RANADIVE LICENSE No. 81459 STATE OF FLORIDA PROFESSIONAL ENGINEER

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FILE No. 24FTL018

PERMIT SET 2-13-2025

PROJECT:

ST. LUCIE WEST GROVE K-8 SCHOOL

10701 SW SUNDANCE VISTA BLVD
PORT ST. LUCIE, FL 34987

FOR

ST. LUCIE SCHOOL DISTRICT

9461 BRANDYWINE LANE, PORT ST. LUCIE, FL 34986

FOR CODE REVIEW:

REVISIONS:

	REVISION NAME	DATE

FOR BID:

DRAWING TITLE:

BUILDING ELEVATION - AUDETERIA - BUILDING 5

COMMISSION NUMBER:
24FTL018

DO NOT SCALE THE DRAWINGS

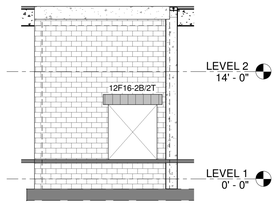
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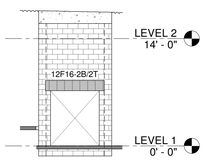
THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"

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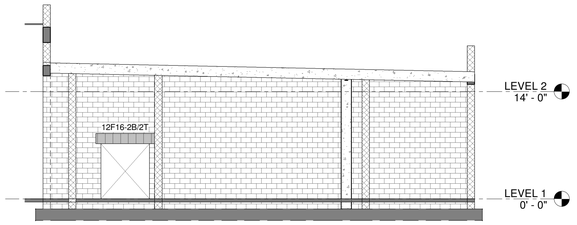
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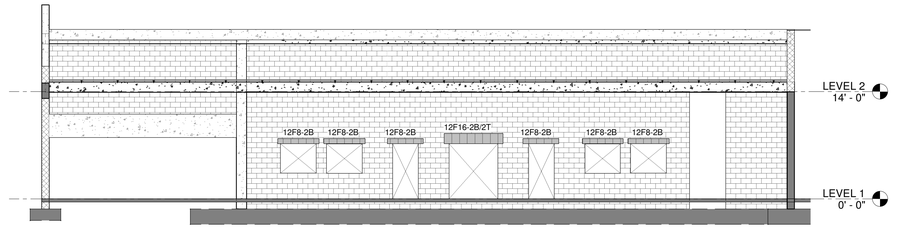
4 S10.7 BUILDING ELEVATION
1/8" = 1'-0"



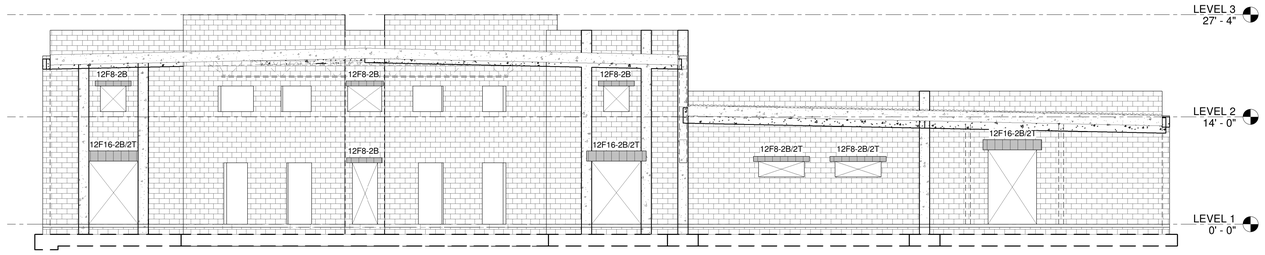
3 S10.7 BUILDING ELEVATION
1/8" = 1'-0"



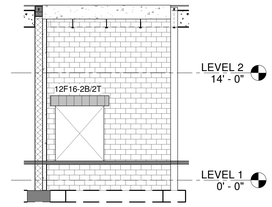
2 S10.7 BUILDING ELEVATION
1/8" = 1'-0"



1 S10.7 BUILDING ELEVATION
1/8" = 1'-0"



6 S10.7 BUILDING ELEVATION
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BUILDING ELEVATION - AUDETERIA - BUILDING 5

COMMISSION NUMBER:
24FTL018

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S10.7

THIS DRAWING IS FORMATTED TO BE PRINTED AT 30"x42"

3"=1'-0" 1 1/2"=1'-0" 1"=1'-0" 3/4"=1'-0" 1/2"=1'-0" 1/4"=1'-0" 1/8"=1'-0" 1/16"=1'-0"