STRUCTURAL NOTES

GENERAL NOTES

G-1 REVIEW ALL PROJECT DOCUMENTS PRIOR TO FABRICATION AND START OF CONSTRUCTION. REPORT ANY DISCREPANCIES TO ARCHITECT OR STRUCTURAL ENGINEER PRIOR TO PROCEEDING WITH WORK.

G-2 THE MASONRY WALLS ARE NOT DESIGNED TO WITHSTAND TEMPORARY CONSTRUCTION LOADS. IT IS THE CONTRACTOR'S RESPONSIBILITY AT ALL TIMES TO MAINTAIN WALL STABILITY DURING THE CONSTRUCTION PHASE OF THIS PROJECT.

G-3 IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROTECT EXISTING FACILITIES, STRUCTURES AND UTILITY LINES FROM ALL DAMAGE DURING CONSTRUCTION.

G-4 NO STRUCTURAL MEMBER SHALL BE CUT, NOTCHED OR OTHERWISE REDUCED IN SIZE OR STRENGTH WITHOUT PRIOR APPROVAL IN WRITING FROM THE STRUCTURAL ENGINEER.

G-5 COORDINATE STRUCTURAL AND OTHER DRAWINGS THAT ARE PART OF THE CONTRACT DOCUMENTS FOR ANCHORED, EMBEDDED OR SUPPORTED ITEMS WHICH AFFECT THE STRUCTURAL DRAWINGS.

G-6 ALL DETAILS AND SECTIONS ON THE DRAWINGS ARE INTENDED TO BE TYPICAL, AND SHALL BE CONSTRUED TO APPLY TO ANY SIMILAR SITUATION ELSEWHERE ON THE PROJECT EXCEPT WHERE A SEPARATE DETAIL IS SHOW.

G-7 THE INTENTION OF THE PLANS AND SPECIFICATIONS IS TO PROVIDE ALL NECESSARY DETAILS TO CONSTRUCT A COMPLETE STRUCTURE. WHEN SPECIFIC INFORMATION IS MISSING OR IS IN CONFLICT, THE CONTRACTOR SHALL USE A SIMILAR DETAIL AND/OR THE MORE COSTLY ITEM OF CONFLICT. THE CONFLICTS SHALL BE BROUGHT TO THE ATTENTION OF ARCHITECT.

G-8 THE ENGINEER SHALL NOT BE RESPONSIBLE FOR LAYOUT, DIMENSIONAL ERRORS OR DISCREPANCIES RESULTING FROM THE REPRODUCTION AND USE OF CONTRACT DRAWINGS SHALL NOT RELIEVE THE CONTRACTOR NOR SUBCONTRACTORS FROM THEIR RESPONSIBILITY TO ACCURATELY LAYOUT, COORDINATE, DETAIL, FABRICATE AND INSTALL A COMPLETE STRUCTURE.

G-9 REVIEW ALL SHOP DRAWINGS FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS AND FOR COMPLETENESS AND ANSWER ALL CONTRACTOR RELATED QUESTIONS. STAMP AND INITIAL ALL SHEETS PRIOR TO SUBMITTING SHOP DRAWINGS TO ARCHITECT FOR REVIEW. NONCOMPLIANCE WITH THIS REQUIREMENT WILL RESULT IN REJECTION OF SUBMITTAL.

G-10 GENERAL CONTRACTOR SHALL COORDINATE THE STRUCTURAL DRAWINGS WITH ALL OTHER DISCIPLINES. WHERE THERE ARE CONFLICTS IN INFORMATION PRESENTED IN THE DRAWINGS OR IF THE DRAWINGS ARE UNCLEAR OR INSUFFICIENT IN ANY MANNER HAT MAY INHIBIT THE CONTRACTORS UNDERSTANDING OF THE PROJECT, SUCH CONFLICTS SHALL BE BROUGHT TO THE ARCHITECT/ENGINEER'S ATTENTION PRIOR TO BIDDING AND THE NECESSARY ADJUSTMENTS SHALL BE MADE PER THEIR INSTRUCTIONS.

G-11 GENERAL CONTRACTOR SHALL REVIEW & APPROVE ALL SHOP DRAWINGS PRIOR TO SUBMITTAL TO ARCHITECT, OTHERWISE THEY WILL BE REJECTED.

G-12 PHOTOGRAPHIC REPRODUCTIONS OF THE STRUCTURAL DRAWINGS FOR SHOP DRAWINGS SHALL NOT BE PERMITTED.

G-13 CONTRACTOR SHALL VERIFY ALL FIELD CONDITIONS & DIMENSIONS RELATE TO THE SAME. WHERE THERE ARE CONFLICTS BETWEEN ACTUAL FIELD CONDITIONS SHALL BE BROUGHT TO THE ARCHITECT/ENGINEER'S ATTENTION AND THE NECESSARY ADJUSTMENTS MADE PER THEIR COORDINATION.

G-14 IF THERE ARE ANY DISCREPANCIES BETWEEN THE STRUCTURAL DRAWINGS, ARCHITECTURAL DRAWINGS, STRUCTURAL DETAILS, STRUCTURAL NOTES, THE PROJECT SPECIFICATIONS, OR APPLICABLE CODES THE STRICTEST SHALL GOVERN.

G-15 ALL WORK TO BE IN STRICT ACCORDANCE WITH THE 2023 FLORIDA BUILDING CODE, 8TH EDITION. AND ALL APPLICABLE LOCAL CODES.

G-16 ONLY WRITTEN CHANGES APPROVED BY THE ARCHITECT AND ENGINEER SHALL BE PERMITTED.

WOOD

W1 FOR MISCELLANEOUS LUMBER, PROVIDE FOLLOWING GRADE AND SPECIES EQUIVALENT: GRADE NO. 2, SOUTHERN PINE OR SPRUCE, SURFACES DRY
WOOD IN CONTACT W/ CONCRETE OR CMU. SHALL BE PRESSURE TREATED "P.T.".

W2 PROVIDE GALVANIZED METAL CONNECTORS AND FRAMING ANCHORS OF THE SIZE TYPE

FIELD VERIFY CONDITIONS

FV-1 CONTRACTOR SHALL VERIFY CONDITIONS AND DIMENSIONS RELATIVE TO THE PROJECT. WHEN THERE ARE CONFLICTS BETWEEN ACTUAL FIELD CONDITIONS AND DATA PRESENTED IN THE DRAWINGS, SUCH CONDITIONS SHALL BE CALLED TO THE ARCHITECT'S ATTENTION AND THE NECESSARY ADJUSTMENTS SHALL BE MADE PER THEIR INSTRUCTION.

FV-2 GENERAL CONTRACTOR SHALL REVIEW AND APPROVE SHOP DRAWINGS BEFORE SUBMITTING THEM TO THE ENGINEER, OTHERWISE THEY WILL BE REJECTED.

FV-3 IF THERE ARE ANY DISCREPANCIES OR CONFLICTS BETWEEN DRAWINGS, SPECIFICATIONS, OR APPLICABLE CODES THE STRICTER SHALL GOVERN. SUCH DISCREPANCIES SHALL BE CALLED TO THE ARCHITECT/ ENGINEER'S ATTENTION FOR REVIEW.

HC-17 PREPARE THE PRECAST CONCRETE SURFACE TO RECEIVE TOPPING IN ACCORDANCE WITH MANUFACTURE'S REQUIREMENTS.

HC-18 ALL WELDED CONNECTIONS SHALL BE DONE BY AWS CERTIFIED WELDERS. PROVIDE CERTIFICATES UPON REQUEST. WELDS SHALL BE MONITORED AND CERTIFIED. PROVIDE CERTIFICATIONS OF WELDS INCLUDING INSPECTION REPORTS UPON COMPLETION OF THE WELDING WORK.

HC-19 VERIFY THE EXACT SIZE AND LOCATION OF ALL FLOOR AND ROOF OPENINGS PRIOR TO DESIGN OF PRECAST.

HC-20 NO OPENINGS MAY BE CUT OUT OR DRILLED THROUGH THE PRECAST MEMBERS WITHOUT PRIOR WRITTEN APPROVAL FROM THE DELEGATED ENGINEER AND PROJECT A/ E.

HC-21 SUPPLEMENTARY STEEL HANGERS IF REQUIRED, SHALL BE PROVIDED BY THE MANUFACTURER.

HC-22 PRE-STRESSED CONCRETE SLABS BE FREE OF DEFECT. ANY DEFECTIVE MEMBERS SHALL BE EVALUATED BY THE DELEGATED ENGINEER AN A/ E FOR REPAIR AND REPLACEMENT.

HC-23 PRE-STRESSED CONCRETE UNITS SHALL BE ERECTED INTO FINAL POSITION UNDER THE SUPERVISION OF AND EMPLOYEE OF THE MANUFACTURER, OR A COMPETENT ERECTOR.

HC-24 ANY GAPS BETWEEN PLANK AND PARALLEL WALLS OR BEAMS ABOVE AND BELLOW CREATED BY CAMBER OR UNEVEN SURFACES ARE TO BE PACKED SOLID WITH GROUT.

HC-25 PRE-STRESSED CONCRETE UNITS SHALL BEAR ON BEARING PADS AT END SUPPORTS AS REQUIRED BY PRECAST MANUFACTURER TO ENSURE PROPER BEARING PER PCI STANDARDS. BEARING PADS SHALL BE MANUFACTURED EXPRESSLY FOR BEARING PURPOSES.

HC-26 IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT THE SUPPORTING STRUCTURE HAS ATTAINED MINIMUM REQUIRED STRENGTH PRIOR TO ERECTION OF PRECAST MEMBERS.

HC-27 THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING TRUE AND LEVEL BEARING SURFACES FOR THE PRECAST COMPONENTS.

HC-28 SUBMITTALS
A. SUBMIT SHOP DRAWINGS OF PRECAST CONCRETE COMPONENTS, ERECTION PLAN CONNECTION DETAILS, DESIGN LOADS, LOCATION OF ALL OPENINGS AND REACTIONS OF ALL PRECAST ELEMENTS SUPPORTED BY STRUCTURAL ELEMENTS SHOWN ON THE DRAWINGS.
SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY THE DELEGATED ENGINEER.
B. SIGNED AND SEALED DESIGN CALCULATIONS, PREPARED BY THE DELEGATED ENGINEER, SHALL BE SUBMITTED WITH SHOP DRAWINGS.
C. AWS WELDER CERTIFICATIONS, IF REQUIRED.
D.PRESTRESS STRAND SPECIFICATION, IF REQUIRED.
E. CERTIFICATION LETTER FOR PCI COMPLIANCE SHALL BE SUBMITTED WITH SHOP DRAWINGS.

HC-29 SUBMITTED SHOP DRAWINGS MUST BE CHECKED AND SIGNED BY THE GENERAL CONTRACTOR.

HC-30 STRUCTURAL REVIEW SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS REGARDING ARRANGEMENT AND SIZES OF MEMBERS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGN LOADS AND CONTRACT DOCUMENT DETAILS. SUCH REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF THE FULL RESPONSIBILITY FOR THE DESIGN OF THE PRECAST AND PRECAST CONNECTIONS NOT SPECIFIED IN THE CONTRACT DOCUMENTS.

HC-31 HOLLOW CORE PLANKS SHALL BE EQUAL TO OR BETTER THAN THOSE MANUFACTURED BY SPI MIAMI. REF. SPIMIAMI.COM

STRUCTURAL STEEL

S-1 FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE AISC "MANUAL OF STRUCTURAL CONSTRUCTION" NINTH EDITION AND THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" 1989 EDITION.

S-2 STELL DESIGNATION:
WIDE FLANGES…….ASTM A992 Fy=50K.S.I
ALL OTHER ROLLED
SHAPES AND PLATES……ASTM A36, Fy=36K.S.I
PIPE COLUMNS……ASTM A53, GRADE B Fy=36K.S.I
STRUCTURAL TUBING…….ASTM A500, GRADE B Fy=46K.S.I

S-3 ALL HIGH-STRENGTH BOLTS SHALL MEET THE REQUIREMENTS OF THE "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS."

S-4 UNLESS OTHERWISE NOTED, BOLTS SHALL BE 3/4" DIAMETER A-325 & SHALL BE BEARING TYPE CONNECTIONS. IF A CERTAIN SITUATION IS NOT DETAILED USE A SIMILAR DETAIL CONNECTION SHALL GENERALLY FOLLOW THE TYPES SHOWN IN AISC MANUAL OF STEEL CONSTRUCTION.

S-5 ALL BOLTS CAST IN CONCRETE SHALL CONFORM TO ASTM A-36 OR A-307

S-6 SHOP AND FIELD WELDING SHALL BE DONE BY CURRENTLY CERTIFIED WELDERS IN ACCORDANCE WITH AWS DLI "STRUCTURAL WELDING CODE," LATEST EDITION.

S-7 WELDS SHALL CONFORM TO AISC STANDARDS & LOAD TABLES. USE E70XX ELECTRODES UNLESS NOTED OTHERWISE FRIND SMOOTH ALL EXPOSED WELDS.

S-8 SUBMIT STRUCTURAL STEEL SHOPDRAWINGS FOR REVIEW PRIOR TO FABRICATON. CLEARLY SHOW ALL PIECE MARKS, CONNECTIONS & ERECTION DETAILS. SPLICES NOT SHOWN ON CONTRACT DRAWINGS ARE TO BE CLEARLY NOTED FOR APPROVAL.

S-9 DO NOT WELD TO EMBEDS UNTIL CONCRETE HAS CURED AT LEAST 72 HOURS. USE APPROPRIATE WELDING PROCESSES TO LIMIT HEAT BUILDUP IN EMBED TO AVOID PLATE EXPANSION AND CRACKING OF CONCRETE.

S-10 WELDED CONNECTIONS SHALL DEVELOP THE FULL SHEAR AND/ OR MOMENT CAPACITY OF THE MEMBERS CONNECTED. AS PER AISC.

S-11 BOLTED STEEL CONECTIONS ARE TO BE STANDARD AISC BOLTED CONNECTIONS PER THE AISC MANUAL AND SHALL BE CAPABLE OF SUPPORTING MAXIMUM ALLOWABLE UNIFORM BEAM LOADS, AS DETERMINED FROM THE TABLES PF UNIFORM LOAD CONSTANTS OF THE AISC MANUAL. ALL BOLTS SHALL BE STRENGTH ASTM A325.

ROOF METAL DECK (CANOPY)

MD-1 MINIMUM PROPERTIES: WIDE RIB (VULCRAFT OR EQUAL) 1-1/2", 20 GA., TYPE B, GALVANIZED METAL DECK
FY: 33 KSI. MIN. IP: 0.201 IN ⁴/FT MIN. IN: 0.222 IN ⁴/FT
MIN. SP: 0.234 IN ³/FT MIN. SN: 0.247 IN ³/FT

MD-2 MIN. ATTACHMENT TO CANOPY RED IRON STEEL:
PROVIDE A 36/7 PATTERN OF #12 TEKS FIELD, & #12 TEKS @ 6"O.C. ALONG ALL RMD EDGES.
PROVIDE (4) #10 TEK SIDELAP FASTENER PER SPAN.

MASONRY

M-1 MASONRY UNITS SHALL CONFORM TO ASTM C90, GRADE N WUTH A MINIMUM COMPRESSIVE STRENGTH OF MASONRY (F'M) LAID IN RUNNING BOND.

M-2 USE TYPE "M" MORTAR WITH MINIMUM COMPRESSIVE STRENGTH OF 1800PSI. INACCORDANCE WITH ASTM C270

M-3 PROVIDE #5 VERTICAL AT LOCATIONS SHOWN IN PLAN, CORNERS & ENDS/WALLS. VERTICAL REINFORCEMENT "FC" FOR NEW MASONRY WALLS IN MIN. 2500 PSI SOLID (8 TO 10" SLUMP) GROUTED CMU CELL FROM FOUNDATION TO TOP OF WALL.

M-4 PROVIDE #5 DOWEL TO FOOTING AT EACH VERTICAL REBAR LOCATION & PROVIDE # 5x24x24" CORNER BAR INTO BEAM @ T/WALL AT FC. (AT CLUSTERS OF FC TOP CORNER BAR MAY BE SPACED OUT TO 24" MAX. o.c.

M-5 REINFORCING FOR FILLED CELLS SHALL CONFORM TO ASTM A615, GRADE 60. PROVIDE 32" MIN. LAP SPLICE FOR REBAR.

M-6 EXPANSION BOLTS SHALL BE "HILTI KWIK BOLT II" OR APPROVED EQUIVALENT. MINIMUM EMBED SHALL BE 5" FOR ¾" DIA., 3 ½" DIA. AND 2 ½" FOR 3/8" DIA.

M-7 CHEMICAL ADHESIVE ANCHORS SHALL BE "HILTI HIT HY-150" OR APPROVED EQUAL.
MINIMUM EMBEDMENT SHALL BE 6 5/8" FOR ¾" DIA., 4 ½" FOR ½" DIA., 3 ½" FOR 3/8" DIA. DO NOT CUT EXISTING REINFORCING TO INSTALL.

M-8 THREADED ANCHORS SHALL BE ASTM A36 OR A307 EQUALITY. ALL CMU CELLS TO RECEIVE ANCHORS SHALL BE GROUTED SOLID. NOTIFY ENGINEER PRIOR TO SUBSTITUTION OF ANY BOLTS INDICATED IN THE DRAWINGS.

REINFORCED CONCRETE

RC-1 ALL CONCRETE DESIGN PLACEMENT SHALL BE IN STRICT ACCORDANCE WITH THE ACI "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE," ACI 318-95.

RC-2 STRUCTURAL CONCRETE SHALL CONFORM TO ACI 301 SPECIFICATIONS. ALL CONCRETE TYPE I PORTLAND CEMENT, (ASTM C 150) 3000 PSI COMPRESSIVE STRENGTH AT 28 DAYS (U.O.N.)
WATER: POTABLE CHLORIDE: NONE

SLUMP:
FOOTINGS 4" +/- 1"
COLUMNS 4" +/- 1"
SLABS 4" +/- 1"

RC-3 PROVIDE NORMAL WEIGHT AGGREGATES IN COMPLIANCE WITH THE REQUIREMENTS OF ASTM C33.

RC-4 FLYASH WHEN USED, SHALL BE LIMITED TO 20% OF THE CEMENTITIOUS MATERIAL.
ALL PUMPED CONCRETE WITH #57 AGGREGATE IS TO CONTAIN A HIGH RANGE WATER REDUCING AGENT. MINIMUM SIZE OF DISCHARGE TO BE 4" I.D.

RC-5A 2" I.D. DISCHARGE MAY BE USED WITH #8 AGGREGATE. USE PLASTICIZER ADMIXTURE IF NECESSARY TO INCREASE SLUMPS BEYOND THAT NOTED ABOVE.

RC-6 CHAMFER ALL EDGES OF EXPOSED CONCRETE ¾" UNLESS NOTED OTHERWISE.

RC-7 ALL REINFORCING STEEL SHALL BE DETAILED, FABRICATED AND INSTALLED IN ACCORDANCE WITH ACI 315 AND ACI DETAILING MANUAL, ACI 315 CURRENT EDITIONS.

RC-8 REINFORCING STEEL SHALL BE NEW DEFORMED BARS THAT ARE FREE FROM RUST, SCALE AND OIL AND CONFORM TO ASTM A615, GRADE 60, WITH MINIMUM YIELD STRENGTH = 60,000. WHERE WELDING REQUIRED ASTM A706, GRADE 60

RC-9 ALL SLABS ON GRADE SHALL BE REINFORCED WITH WELDED WIRE FABRIC UNLESS NOTED OTHERWISE IN THE DRAWINGS.

RC-10 WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185.

RC-11 LAP CONT. BOTTOM STEEL OVER SUPPORT AND CONT. TOP STEEL AT MID SPAN UNLESS OTHERWISE SPECIFIED. MIN. LENGTH OF LAPPED SPLICES SHALL BE AS FOLLOWS UNLESS NOTED. STAGGER SPLICES SO THAT NO TWO ADJACENT BARS ARE SPLICED IN THE SAME LOCATION UNLESS SHOWN OTHERWISE. WELDED WIRE FABRIC SHALL BE LAPPED ON FULL MESH AT THE SIDES AND ONE FULL MESH PLUS 2" AT THE ENDS. MAKE ALL BARS CONTINUOUS AROUND CORNERS OF PROVIDE CORNER BARS OF EQUAL SIZE AND SPACING.

BAR SIZE:	SPLICE SIZE:
#3	16"
#4	22"
#5	27"
#6	35"

RC-12 TERMINATE ALL DISCONTINUOUS TOP WITH STANDARD 90 DEGREE HOOK (PLACED VERTICALLY) UNLESS NOTED OTHERWISE.

RC-13 PROVIDE THE FOLLOWING CONCRETE COVERAGE'S OVER REINFORCING,
B.O. FOOTINGS & UNFORMED EDGES- 3" CLR.
BEAMS & COLUMNS (TIES)- 1 ½" CLR.
SLABS (TOP & BOT.-STEEL)
INTERIOR- ¾" CLR.
EXTERIOR- 1 ½" CLR.
CONC. WALLS- AS NOTED (1 ½" CLR. MIN.)

RC-14 SHOP DRAWINGS FOR PLACEMENT SHALL BE SUBMITTED FOR REVIEW PRIOR TO REBAR FABRICATION.

RC-15 NO REINFORCING BARS SHALL BE CUT TO ACCOMMODATE THE INSTALLATION OF ANCHORS, EMBEDS OR OTHER ITEMS.
USE THE STRUCTURAL DRAWINGS INCLUDING REVISIONS AND ADDENDA IN CONJUNCTION WITH REVIEWED SHOP DRAWINGS FOR PLACEMENT OF REINFORCING.

RC-16 AT CHANGES IN DIRECTION OF CONCRETE WALLS, BEAMS & STRIP FOOTING, PROVIDE CORNER BARS OF SAME SIZE AND QUANTITY (U.O.N.) AS HORIZONTAL STEEL. REFER TO TYPICAL DETAIL.\

RC-17 ALL EMBEDDED ITEMS SHALL BE SECURELY TIED IN PLACE PRIOR TO CONCRETE PLACEMENT.
RC-18 WEDGE BOLTS SHALL BE ITW RAMSET/REDHEAD BOLTS OR APPROVED EQUIVALENT
INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS, 3/4" DIAMETER WEDGE BOLTS SHALL BE OF LENGTH REQUIRED TO PROVIDE 3-1/4" MIN. EMBEDMENT, 2.0 KIPS TENSION AND 4,1 KIPS SHEAR WORKING LOADS IN 3000psi CONCRETE (U.O.N.) DO NOT CUT EXISTING REINFORCING TO INSTALL.
RC-19 DOWEL BAR SPLICERS SHALL BE A COMPLETE ASSEMBLY, INCLUDING COUPLER, FROM ATTACHMENT (WHEN REQUIRED) AND THREADED BAR CAPABLE OF DEVELOPING THE BAR BEING SPLICED IN ACCORDANCE WITH ACI 318 SECTION 12.14.3.

RC-20 PLACE CONCRETE PER ACI 304. USE INTERNAL MECHANICAL VIBRATION FOR ALL CONCRETE. LIMIT MAXIMUM FREE FALL DROP OF CONCRETE TO 6'-0" FOR #57 AGGREGATE AND 8'-0" FOR #8 AGGREGATE. ALL PRECAUTIONS SHOULD BE TAKEN TO AVOID SEGREGATION OF CONCRETE DURING PLACEMENT.

RC-21 HEADED STUD ANCHORS SHALL BE ASTM A 108, GRADE S, 1010 THROUGH 1020 AS MANUFACTURED BY NELSON STUD OR APPROVED EQUIVALENT. STUD WELDING SHALL CONFORM TO AWS DI.1 "STRUCTURAL WELDING CODE."

RC-22 CHEMICAL ADHESIVE ANCHORS SHALL BE "HILTI HIT HY-150" ANCHORING SYSTEM OR "SIMPSON SET-XP" ANCHORING SYSTEM OR APPROVED EQUIVALENT INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS, DO NOT CUT EXISTING REINFORCING TO INSTALL.

RC-23 PLACEMENT OF CONDUIT AND PIPES IN CONCRETE SHALL CONFORM TO ACI 318, SECTION 6,3.

RC-24 SLABS AND GRADE BEAMS SHALL NOT HAVE JOINTS IN A HORIZONTAL PLANE. ALL REINFORCEMENT SHALL BE CONTINUOUS THROUGH ALL CONSTRUCTION JOINTS.

STRUCTURAL DESIGN CRITERIA

D-1 CODES: - FLORIDA BUILDING CODE 8th. EDITION 2023

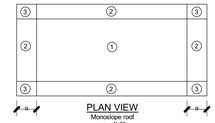
D-2 DESIGN LIVE LOADS:
ROOFS30 PSF
FLOOR SLAB100 PSF.

D-3 ALLOWABLE SOIL BEARING PRESSURE 2000PSF.

D-4 SEISMIC: ZONE OF MINIMAL SEISMIC ACTIVITY, WIND LOADS CONTROL DESIGN.

D-5 DESIGN WIND SPEED 175 MPH,: vasd= 136MPH, EXPOSURE C;
BUILDING CATEGORY II; IMPORTANCE FACTOR (I)=1.0

COMPONENTS & CLADDING WIND PRESSURES:



PLAN VIEW
Monoslope roof
a=+ 4'-0"

ROOF Pressures in psf
Multiply by 0.6 for ASD pressures

TRIBUTARY AREA	Interior 1	Perimeter 2	Corner 3
area <= 20SF	-70.8	118.7	-179.7
20 SF <area <= 50 SF	28.8 -69.0	28.8 -106.1	28.8 -148.0
50 SF <area <=100 SF	27.0 -66.6	27.0 -98.6	27.0 -107.4
100 < area	24.6 -64.8	24.6 -76.8	24.6 -76.8
	22.8	22.8	22.8

ELEVATION
Any roof type
a=4'-0"

WALL Pressures in psf
Multiply by 0.6 for ASD pressures

TRIBUTARY AREA	Interior 4	End 5
area <= 20SF	-66.1 63.7	-85.3 63.7
20 SF <area <= 50 SF	66.3 60.8	-79.5 60.8
50 SF <area <=100 SF	-62.5 57.0	72.0 57.0
100 < area	-59.6 54.2	-66.2 54.2

SHALLOW FOUNDATIONS

SF-1 THE FOUNDATIONS HAVE BEEN DESIGNED FOR A MINIMUM 2000PSF, ALLOWABLE SOIL BEARING PRESSURE. SOIL BEARING PRESSURE. CONTRACTOR TO NOTIFY ARCHITECT IF SOIL CONDITIONS ARE UNCOVERED THAT PREVENT THE REQUIRED SOIL BEARING PRESSURE FROM BEING OBTAINED. BEC STRONGLY RECOMMENDS A SOILS REPORT BE COMMISSIONED BY THE OWNER TO ENSURE GOOD SOILS CONDITIONS & PREPARATION SO THAT THE SOILS ARE CONFIRMED TO MEET 2000PSF ALLOWABLE SOIL BEARING PRESSURE.

SF-2 CAUTION SHALL BE USED WHEN OPERATING VIBRATORY COMPACTION EQUIPMENT NEAR EXISTING STRUCTURES TO AVOID THE RISK OF DAMAGING THE EXISTING STRUCTURE.

SF-3 CENTER ALL FOOTINGS UNDER THEIR RESPECTIVE COLUMNS OR WALLS UNLESS OTHERWISE SHOWN ON PLANS. MAXIMUM MISPLACEMENT OR ECCENTRICITY-2". TOLERANCE FOR MISLOCATION OF COLUMN DOWELS OR ANCHOR BOLTS TO BE PER ACI OR AISC STANDARDS.

SF-4 HORIZONTAL JOINTS IN FOOTINGS WILL NOT BE PERMITTED.

SF-5 WHERE VERTICAL CONSTRUCTION JOINTS OCCUR IN CONTINUOUS FOOTING, PROVIDE A MINIMUM CONTINUOUS 2" x 4" KEY WAY ACROSS JOINT FOR EACH 12" OF DEPTH.

SF-6 COORDINATE PLUMBING LINES WITH FOOTING LOCATIONS FOR INTERFACE OR INTERFERENCE. INDIVIDUAL FOOTINGS SHOULD BE LOWERED & CONTINUOUS WALL FOOTINGS SHOULD BE STEPPED DOWN PER DETAIL IN DRAWINGS.
ARCHITECT/ENGINEER SHALL BE CONSULTED.
SF-7 EXCAVATING UNDER OR NEAR IN-PLACE FOOTINGS/FOUNDATIONS WHICH DISTURBS THE COMPACTED SOIL BENEATH THE FOOTINGS/FOUNDATIONS WILL NOT BE PERMITTED.
SF-8 REINFORCING SHALL BE SUPPORTED ON PRECAST CONCRETE PADS, DOWELS FOR COLUMNS AND FILLED CELLS SHALL BE SECURED IN PLACE PRIOR TO POURING CONCRETE. USE TEMPLATES FOR SETTING COLUMN DOWELS AND ANCHOR BOLTS.
SF-9 BACKFILL COMPACTION WORK ADJACENT TO RETAINING WALLS, FOUNDATION WALLS ETC. SHALL USE LIGHT DUTY HAND OPERATED TAMPERS WITHIN 3'-0" DISTANCE FROM THE WALL.

BUILDING CONSTRUCTION TYPE: VB
PER FLORIDA BUILDING CODE 2023, 8th Edition

STUDIO 407

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Issued For

PERMIT SET	06/14/24

Revisions

#	Description	Date

Seal

This item has been electronically signed and sealed by Jeffrey Gaither, RA on the Date and/or time stamp shown using a digital signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Project No: 22-169
Drawn By: Jeff Gaither, AIA AR93666

Sheet Title

STRUCTURAL NOTES

S001

6/13/2024 11:29:25 AM

PLYWOOD SHEATHING:

WALL: Use 7/16" OR APA 32/16 RATED, EXP. 1, PLYWOOD SHEATHING. FIRST 48" OF WALL FROM GRADE SHALL HAVE PRESSURE TREATED PLYWOOD, OR ZIP SHEATHING. ATTACH TO STUDS WITH 8d NAILS AT 6" O.C. IN FIELD OF PANEL, AND 12" O.C. ALL OTHER SUPPORTS. FOR SHEAR WALLS, REFER TO THE SHEAR WALL SCHEDULE FOR FASTENER REQUIREMENTS.

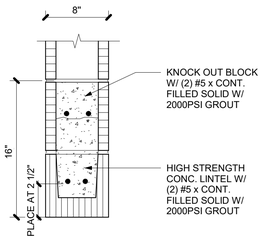
ROOF: Use (15/32" SLOPED SURFACES OR 19/32" FLAT SURFACES) (32/16 OR 40/20) RATED, EXP. 1, PLYWOOD SHEATHING. ATTACH TO TRUSSES WITH 8d RING SHANK NAILS AT 6" O.C. IN FIELD OF PANEL, AND 12" O.C. ALL OTHER SUPPORTS.

WOOD CONNECTIONS – ALL NAILS USED FOR STRUCTURAL FRAMING MEMBERS SHALL BE COMMON WIRE, U.N.O. ALL NAILS, TRUSS HANGERS, TRUSS ANCHORS AND STRAPS SHALL BE GALVANIZED FOR CORROSIVE RESISTANCE. ALL METAL STRAPS MUST BE INSTALLED WITH EQUAL LENGTHS ABOUT THE JOINT LINE. USE SIMPSON STRONG–TIE CONNECTOR PRODUCTS OR APPROVED EQUAL. TOE NAILING WILL NOT BE PERMITTED.

ALL STRUCTURAL WOOD MEMBERS ARE DESIGNED AS "DRY–USE". MOISTURE CONTENT MUST BE 19% OR LESS. STORE WOOD FRAMING ABOVE GROUND AND UNDER TARPS WITH PROPER AIR CIRCULATION.

ALL LUMBER SHALL BE SOUTHERN PINE SPECIES #2 GRADE OR APPROVED EQUAL. ALLOWABLE DESIGN STRESSES SHALL FOLLOW NATIONAL DESIGN SPECIFICATION (NDS) (LATEST EDITION).

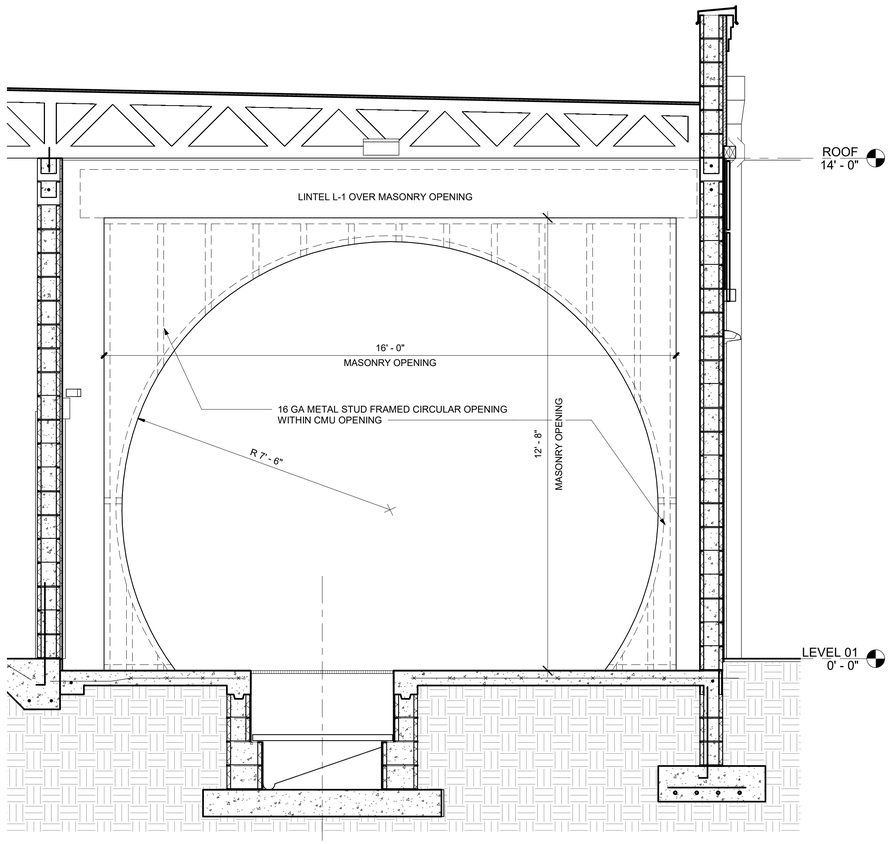
PROVIDE SP ACQ PRESSURE TREATED LUMBER IN ACCORDANCE WITH AWPA STANDARDS TO A MINIMUM 0.40 PCF RETENTION WHERE LUMBER IS IN CONTACT WITH CONCRETE/MASONRY OR OUTSIDE OF BUILDING. ALL METAL CONNECTORS IN CONTACT WITH PRESSURE TREADED LUMBER SHALL BE GALVANIZED WITH A RATING OF G–185 AND CONFORM TO ASTM A653. ALL NAILS AND SCREWS USED WITH PRESSURE TREATED LUMBER ARE TO BE HOT–DIPPED GALVANIZED AND TO CONFORM TO ASTM A153 CLASS D. ELECTROGALVANIZED FASTENERS SHALL HAVE A CLASS RATING PER ASTM B695 NO LESS THAN 55. ALUMINUM NOT TO BE USED IN DIRECT CONTACT WITH ACQ TREATED LUMBER.



TYPICAL OVER DOOR MASONRY OPENINGS "M.O" UP TO 18' - 0" (TYP. UNLESS OTHERWISE NOTED "U.O.N")

L-1 8F16-2B/2T PRESTRESSED CASTCRETE OR EQUIVALENT

L-1 SECTION



2 LINTEL DETAILS

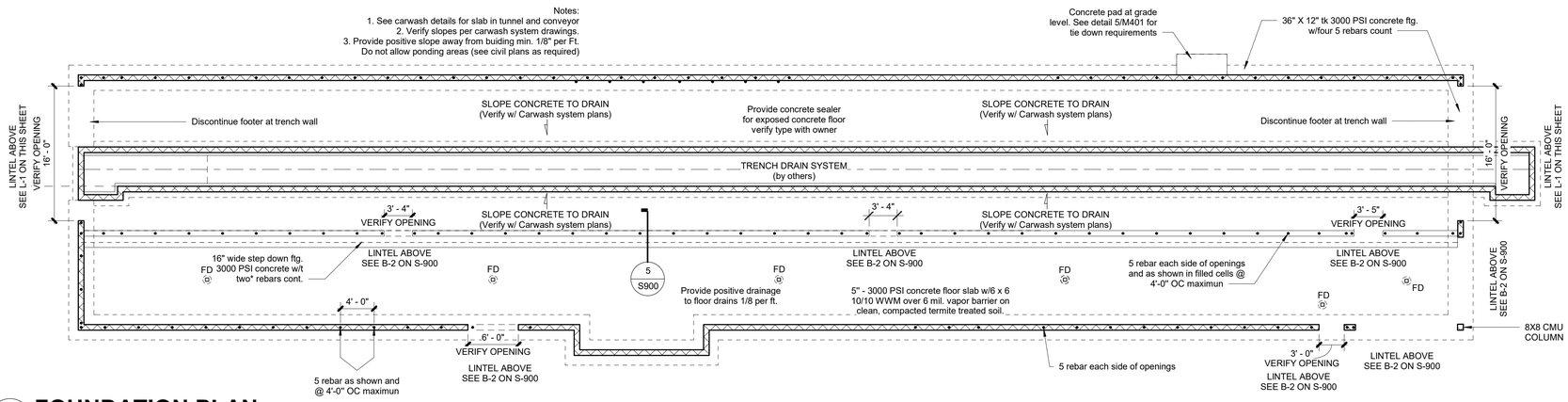
12" = 1'-0"

3 OPENING FRAMING

1/2" = 1'-0"

05400 - STRUCTURAL COLD-FORMED STEEL (CFMF) FRAMING:

1. ALL COLD FORMED STEEL FRAMING SHALL CONFORM TO THE AISI/COFS/NASPEC NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, THE AISI MANUAL COLD-FORMED STEEL DESIGN (LATEST EDITION), AISI CODE OF STANDARD PRACTICE FOR COLD-FORMED STEEL STRUCTURAL FRAMING (LATEST EDITION), AISI/COFS/GP-2004 STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS (LATES EDITION) AND COLD-FORMED STEEL ENGINEERS INSTITUTE (CFSEI) PUBLICATIONS, AS MODIFIED OR CLARIFIED HEREIN.
2. STEEL STUDS, JOISTS, LINTELS, AND RUNNER TRACK MEMBERS SHALL BE OF TYPE SHOWN ON THE DRAWINGS AND SPECIFICATIONS CONFORMING TO ASTM A-446 GRADE C WITH HOT DIPPED GALVANIZED COATING CONFORMING TO ASTM A525 (CLASS G90).
3. MINIMUM STEEL GRADES (FY): 12 GA (97 MILS), 14 GA (68 MILS); 16 GA (54 MILS) STUDS AND TRACK: 50 KSI; 18 GA (43 MILS), 20 GA (33 MILS) STUDS AND TRACK: 33 KSI.
4. STRUCTURAL LIGHT GAGE CFS FRAMING AND THEIR CONNECTIONS SHALL BE AS DEPICTED ON THE STRUCTURAL PLANS AND DETAILS.
5. ATTACHMENTS, CLOSURES, HARDWARE, ETC., SHALL BE AS SHOWN AND/OR IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS.
6. SUBMIT PROPOSED LIGHT GAGE CFS MANUFACTURERS DATA AND LOAD TABLES FOR REVIEW.
7. TOLERANCES TO COMPLY WITH ASTM C955.
8. WELDING TO COMPLY WITH COLD-FORMED STEEL ENGINEERS INSTITUTE TECH NOTE 5606-1 - WELDING COLD-FORMED STEEL.
9. REPAIR DAMAGED OR UNCOATED GALVANIZED COATINGS PER ASTM A780.
10. FRAMING MEMBERS SHALL BE CUT SQUARELY OR AT AN ANGLE AS REQUIRED TO FIT SQUARELY AGAINST ABUTTING MEMBERS. MEMBERS SHALL BE HELD FIRMLY IN PLACE UNTIL PROPERLY JOINED.
11. JOINING OF STRUCTURAL MEMBERS SHALL BE MADE WITH SELF-DRILLING SCREWS OR WELDING.
12. WIRE TYING OF FRAMING MEMBERS IN STRUCTURAL APPLICATIONS SHALL NOT BE PERMITTED.
13. ATTACHMENT OF COLLATERAL MATERIALS TO STEEL MEMBERS SHALL BE MADE WITH SELF-DRILLING SCREWS OR HARDENED SCREW SHANK NAILS.
14. STUDS SHALL SIT SQUARELY IN THE TOP AND BOTTOM RUNNER TRACK WITH FIRM ABUTMENT AGAINST TRACK WEBS. STUDS SHALL BE ALIGNED OR PLUMBED AND SECURELY FASTENED TO THE FLANGES OF BOTH TOP AND BOTTOM RUNNER TRACK.
15. BRIDGING TO BE SUPPLIED AND INSTALLED PER CFS STUD MANUFACTURER RECOMMENDATIONS (5'-0" O.C. MAX AND WITHIN 1'-0" OF DEFLECTION TRACKS).
16. LATERAL BRACING SHALL BE PROVIDED BY USE OF PLYWOOD SHEATHING, GYPSUM SHEATHING, OR BY HORIZONTAL STRAPS OR COLD-ROLLED CHANNELS. BRACING SHALL CONFORM TO SECTION D3 OF THE AISI SPECIFICATIONS.
17. LIGHT GAGE FRAMING CONNECTORS SPECIFIED BY PART NUMBER OR MODEL NAME ARE STANDARD CONNECTORS FABRICATED BY THE STEEL NETWORK (TSN), RALEIGH, N.C., 888-474-4878. CONNECTORS SHALL BE INSTALLED PER THE REQUIREMENTS OF THE LIGHT STEEL FRAMING CONNECTIONS CATALOG (LATEST EDITION) BY THE STEEL NETWORK, USING THE NUMBER/SIZE OF FASTENERS SHOWN IN THESE DRAWINGS. WHERE NUMBER/SIZE IS NOT SHOWN IN THESE DRAWINGS, USE THE FASTENERS SPECIFIED IN THE CATALOG TO OBTAIN THE MAXIMUM CAPACITY OF THE SPECIFIED CONNECTOR. SUBSTITUTION OF GENERIC BENT PLATE LIGHT GAGE CONNECTORS IS NOT ALLOWED WITHOUT DESIGN CALCULATIONS SHOWING EQUAL OR BETTER CAPACITY TO THE SPECIFIED TSN CONNECTOR, PREPARED AND SIGNED AND SEALED BY A FLORIDA REGISTERED ENGINEER EXPERIENCED IN LIGHT GAGE STEEL CONNECTION DESIGN.



1 FOUNDATION PLAN

1/8" = 1'-0"

0 4' 8' 12' 16'

SCALE 3/16" = 1'-0"

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Sheet Title

FOUNDATION PLAN

S201

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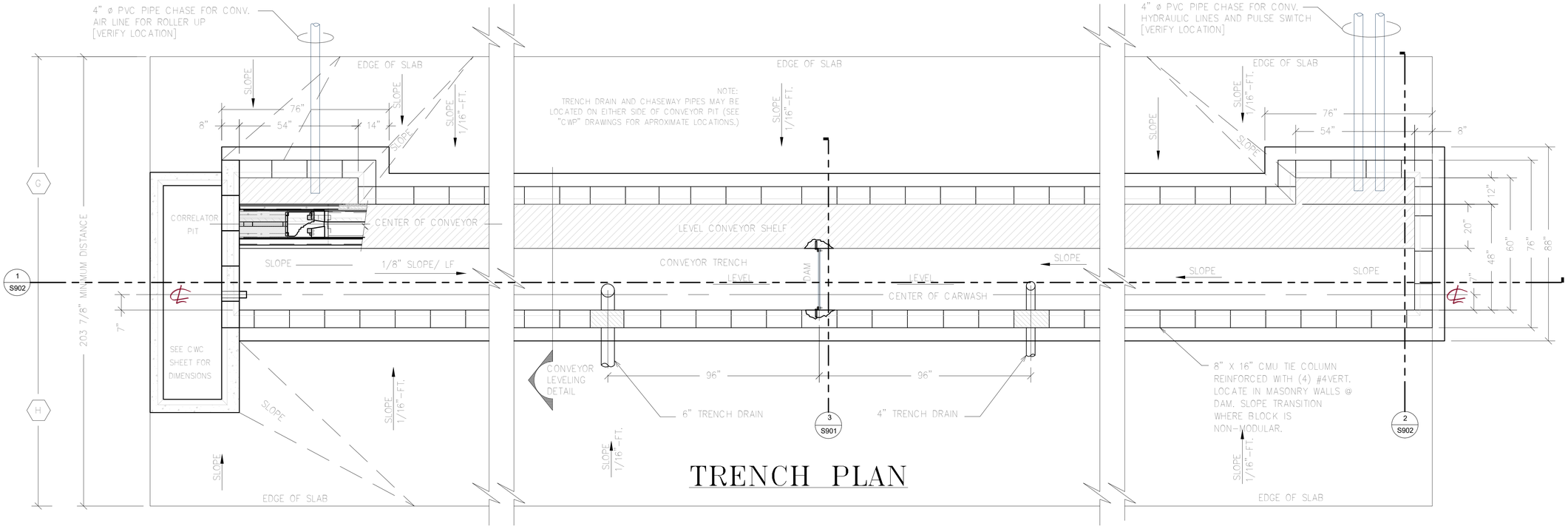
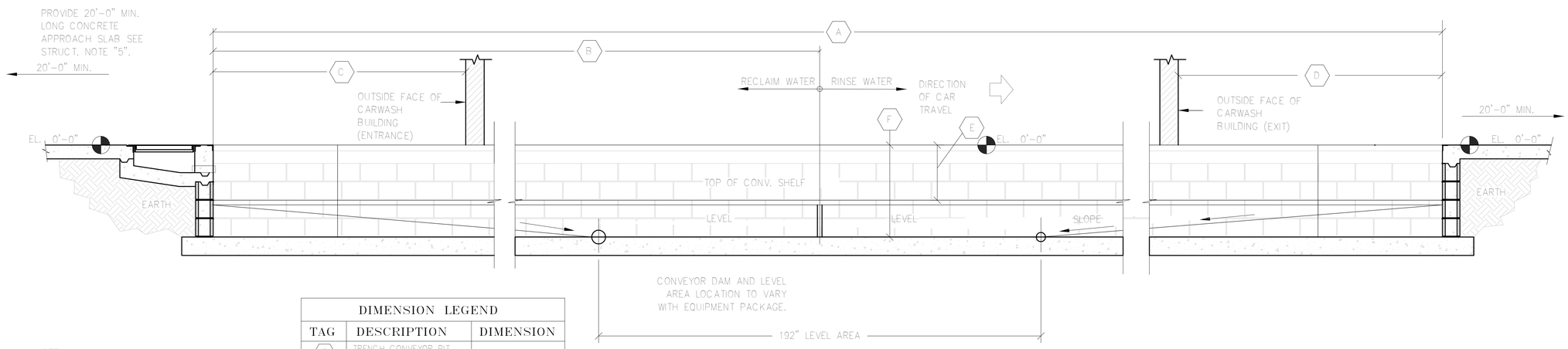


TRENCH PLAN



TRENCH ELEVATION

NOTE:

- THE FINISHED FLOOR SHOULD BE LEVEL FROM ENTRANCE TO EXIT, HOWEVER, A 1" RISE EVERY 50 FT IS PERMISSABLE WITH A MAXIMUM 2" RISE OVER THE ENTIRE LENGTH.
- SEE TYPICAL TRENCH SECTION FOR REQUIRED CONSTRUCTION TOLERANCES FOR TRENCH CONSTRUCTION.

DIMENSION LEGEND		
TAG	DESCRIPTION	DIMENSION
A	TRENCH CONVEYOR PIT LENGTH	PER CW EQUIP. PLANS
B	DAM LOCATION ENTRANCE EDGE OF CONVEYOR PIT	DAM
C	TRENCH PIT OUTSIDE BUILDING (ENTRANCE)	ENTRANCE
D	TRENCH PIT OUTSIDE BUILDING (EXIT)	EXIT
E	CONVEYOR SHELF DEPTH FRONT-W-PULL.=21-3/4" REAR-W-PUSH.=19-3/4"	FWP RWP
F	MAX DEPTH OF TRENCH	DEPTH
G	CENTERLINE OF EQUIPMENT TO DRIVERS SIDE WALL.	DS
H	CENTERLINE OF EQUIPMENT TO PASSENGER SIDE WALL.	DS

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Sheet Title
TRENCH PLAN

S202

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SHEET NOTES

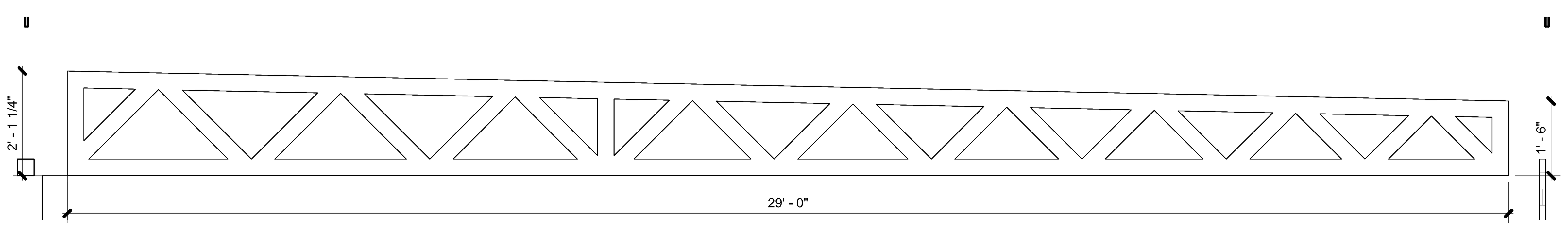
A.2 - SCUPPER

WOOD NOTES:

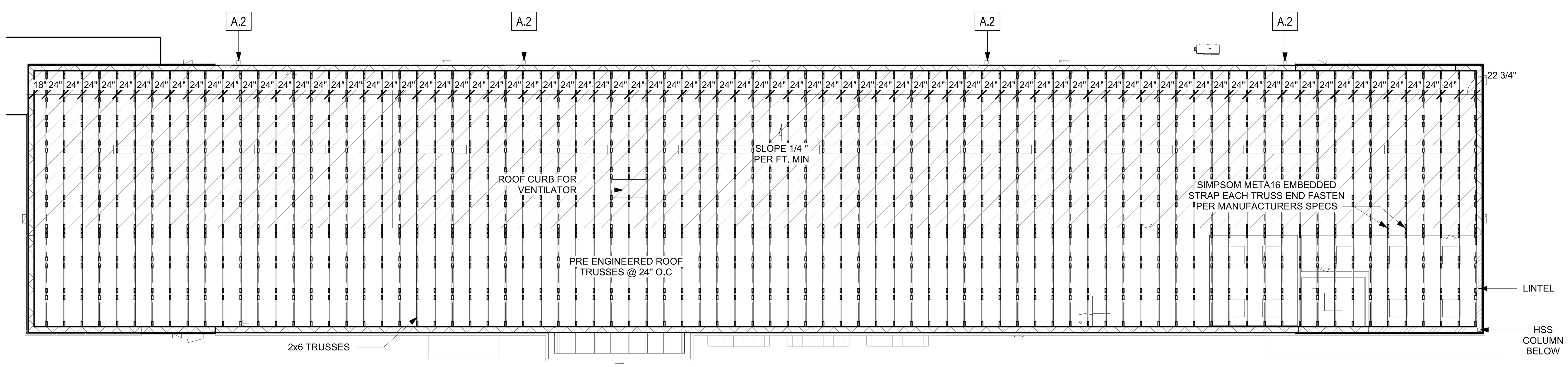
1. ALL FRAMING MEMBERS SHALL BE WOOD MEMBERS ITH PROPERTIES EQUAL OR BETTER THAN THOSE GIVRN BY THE SOUTHERN PINE INSPECTION BUREAU FOR SURFACED DRIED SOUTHERN PINE NO. 2 (MINIMUN ALLOWABLE BENDING STRESS – 1200 PSI FOR SINGLE MEMBER USES AND 1300 PSI FOR REPETITIVE MEMBER USES. MODULUS OF ELASTICITY =1.600.000 PSI).
2. ALL LUMBER IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE TREATED OR SEPARATED BY A MOISTURE BARRIER ≠ FELT, ALUMINUM, ETC.
3. ALL ROOF RAFTERS AND PREFABRICASTED WOOD TRUSSES SHALL BE SECURELY FASTENED TO THEIR SUPPORTING WALLS OR BEAMS WITH HURRICANE STRAPS OR ANCHORS AS REQUIRED FOR UPLIFT.
4. PRE-FABRICATED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF THE "NATIONAL" DESIGN SPECIFICATIONS FOR STRESS GRADE LUMBER AND IT'S "FASTENERS' AS RECOMMENDED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION.
5. TRUSS MEMBERS AND CONNECTIONS SHALL BE PROPORTIONED A(WITH A MAXIMUN ALLOWABLE STRESS INCREASE FOR LOAD DURATION OF 25% TO WHITSTAND THE LOADS GIVEN IN THE NOTES.
6. TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND DESIGN NOTES PREPARED BY QUALIFIED PERSON FOR APROVAL BY THE ENGINEER OF RECORD. DESIGN NOTES TO INCLUDE THE RATED LOAD CAPACITY OF THE CONECTORS CAPACITIES AND MANUFACTURERS LICENCE TO FABRICATE TRUSSES UTILIZING THE CONECTOR SYSTEM PROPOSED.
7. BRIDGING FOR PRE-ENGINEREED TRUSSES SHALL BE AS REQUIRED BY TRUSS MANUFACTURER IN ADDITION TO MEMBERS SHOWN ON THE DRAWING.
8. ALL ROOF DECK SHALL BE 5/8'' NOM. CDX PLYWOOD. USE 8d RING SHANKED NAILS WITH 6'' SPACING AT ALL PANEL EDGES AND 6'' SPACING AT INTERMEDIATE SUPPORTS. PROVIDE PLYWOOD CLIPS AT UNSUPPORTED EDGES FOR 5/8'' FOR 5/8'' CDX PLYWOOD.
9. ALL WOOD FRAMING DETAILS SHALL BE IN ACCORDANCE WITH ''NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION", NAIONAL FOREST PRODUCTS ASSOCIATION, UNLESS DETAILED OTHERWISE ON STRUCTURAL DRAWINGS.
10. ROOF LIVE LOAD 30 PSF ROAD DEAD LOAD 17PSF
11. SEE TRUSS MANUFACTURER DESIGN PLANS FOR EXACT MEMBER LOCATIONS AND UPLIFT VALUES, IF UPLIFT VALUES EXCEED ANY STRAP NOTED, CONSULT ENGINEER OF RECORD.
12. THE ENGINEER OF RECORD RESERVES THE RIGHT TO MAKE ANY REVISION TO BEARING CONDITIONS AND TIEDOWNS AFTER TRUSS LOAD INFORMATION IS SUPPLIED AND TRUSS MANUFACTURER LAYOUT IS REVIEWED BY THIS OFFICE.
13. REFER TO STRAP MANUFACTURER SPECIFICATIONS FOR QUANTITY AND TYPE OF FASTENERS REQUIRED TO ACHIEVE UPLIFT RESISTANCE.
14. CONECTORS INSTALLED ON PRESSURE TREATED WOOD SHALL BE STAINLESS STEEL OR HOT – DIPPED GALVANIZED. FASTENERS (NAILS, SCREWS, OR BOLTS) SHALL HAVE SAME FINISH.

NOTES:

- BRACE TRUSSES PER MANUFACTURERS SPECIFICATIONS.
- ALL WORK TO BE DONE IN STRICT ACCORDANCE WITH APPLICABLE CODES AND ORDINANCES.
- ROOF FARMING TO BE PRE ENGINEERED WOOD TRUSSES @ 2'0'' OC EXCEPT WHERE NOTED.
- ALL EXPOSED WOOD SHALL BE PRESSURE TREATED. USE GALVANIZED NAILS, BOLTS AND CONNECTORS.
- USE GALVANICED METAL CONNECTORS FOR WOOD TRUSSES AND BEAMS.
- ALL FRAMING SHALL BE SOUTHERN YELLOW PINE #2.
- DO NOT SCALE DRAWINGS. USE DIMENSIONS SHOWN ON THE DRAWINGS. CONTRACTOR SHALL VERIFY ALL DIMENSIONS ON SITE. NOTIFY ENGINEER REGARDING ANY DISCREPANCIES.
- FASTEN ROOF SHEATING TO ALL FRAMING MEMBERS WITH 8D RING SHANK NAILS @ 6'' OC TO ALL MEMBERS.
- INSTALL FLASHING IN ALL VALLEYS AND WHERE ROOF MEETS VERTICSL SURFACES. (INCLUDING SKYLIGHTS)
- THE PURPOSE OF THE FRAMING PLAN IS TO RELAY THE DESIGNER'S CONCEPT OF ROOF, CEILINGS AND FLOOR DESIGNS. IT IS THE TRUSS MANUFACTURER'S RESPONSIBILITY TO DESIG AND ENGINEER TRUSSES FROM THE INFORMATION SUPPLIED BY THE ENTIRE SET OF PLANS. ANY CHANGES THAT WOULD AFFECT THE FOUNDATION PLAN AND FLOOR PLAN SHALL BE BROUGHT TO THE DESIGNER'S INMEDIATE ATTENTION. THE DESIGNER ASSUMES NOT RESPONSIBILITY FOR MISINTERPRETATION OF DRAWINGS OR UNAUTORIZED REVISIONS.
- SEE TRUSS MANUFACTURER DESIG PLANS FOR EXACT MEMBER LOCATIONS AND UPLIFT VALUES. IF UPLIFT VALUES EXCEED ANY STRAP NOTED, CONSULT ENGINEER OF RECORD.
- THE ENGINEER OF RECORD RESERVES THE RIGHT TO MAKE ANY REVISION TO BEARING CONDITIONS AND TIEDOWNS AFTER TRUSS LOAD INFORMATION IS SUPPLIED AND TRUSS MANUFACTURER LAYOUT IS REVIEWED BY THIS OFFICE.
- REFER TO STRAP MANUFACTURER SPECIFICATIONS FOR QUANTITY AND TYPE OF FASTENERS REQUIRED TO ACHIEVE UPLIFT RESISTANCE.
- CONNECTORS INSTALLED ON PRESSURE TREATED WOOD SHALL BE STAINLESS STEEL OR HOT-DIPPED GALVANIZED. FASTENERS (NAILS, SCREWS, OR BOLTS) SHALL HAVE SAME FINISH.
- REFER TO ENGINEREED TRUSS MANUFACTURER'S PLAN FOR CONNECTION OF TRUSSES TO GIRDER IF NO HANGER IS NOTED, USE SIMPSOM THA29 FOR SINGLE TRUSSES UP TO 2300# REACTION. 150# UPLIFT.



2 **TRUSS TYPE 1**
1/2" = 1'-0"



1 **ROOF FRAMING**
1/8" = 1'-0"

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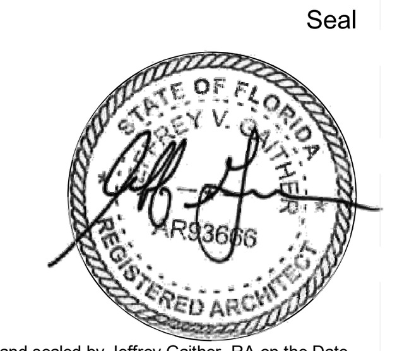

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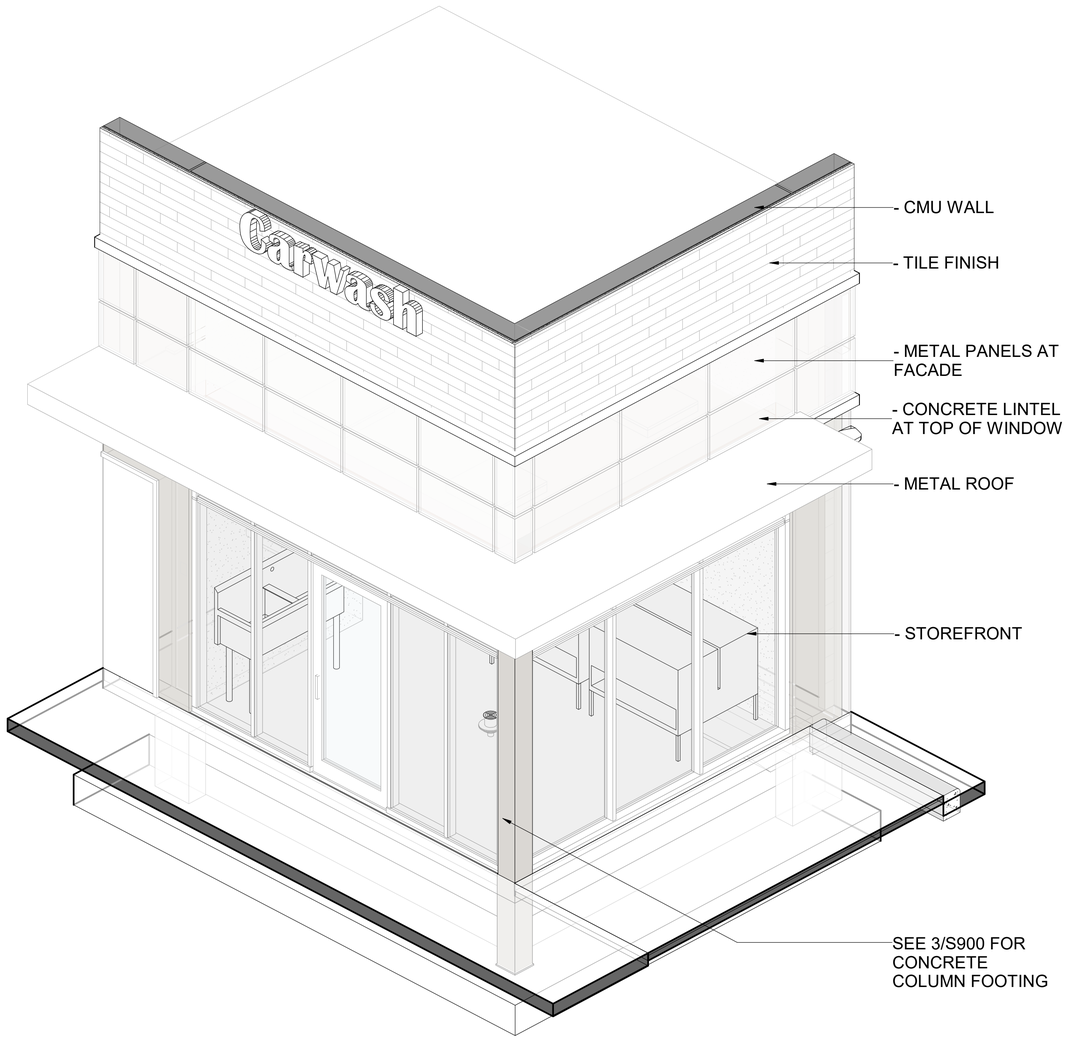
Sheet Title

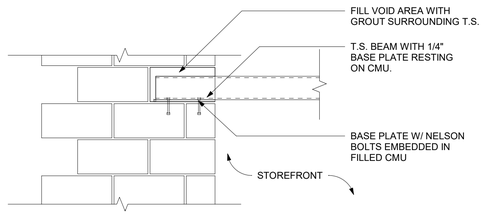
FRAMING PLANS

S203



AXONOMETRIC VIEW - CORNER WINDOW STRUCTURE

2



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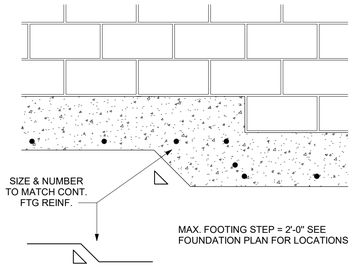
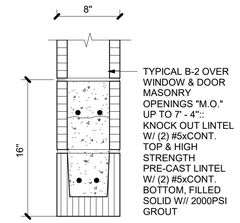
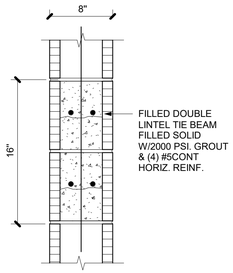


STEP FOOTING DETAIL

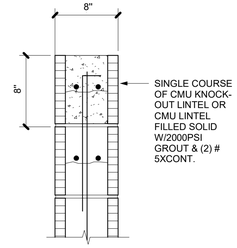


TYPICAL B-2 OVER WINDOW & DOOR MASONRY OPENINGS "M.O" UP TO 7' - 4" (TYP. UNLESS OTHERWISE NOTED "U.O.N")

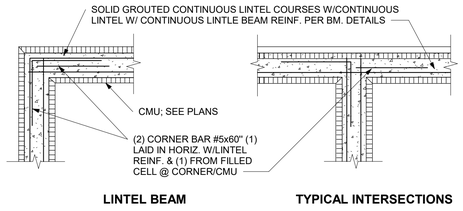
B-2 SECTION



B-2 SECTION

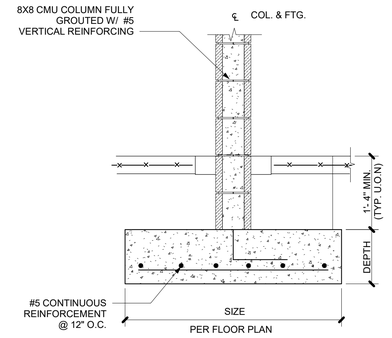


B-1 SECTION

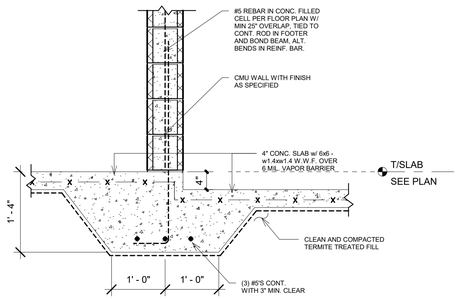


LINTEL BEAM — TYPICAL INTERSECTIONS

TYP. CORNER REINF. DET.

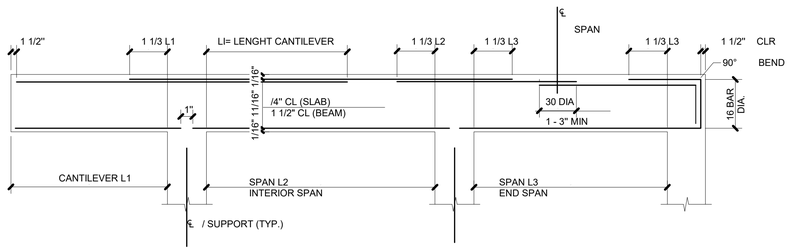


3 TYP STEEL CO. FOOTING



INTERIOR CMU WALL FOOTING

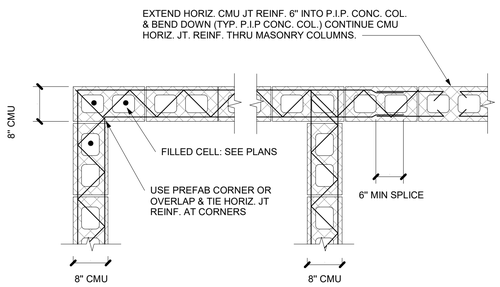
5 3/4" = 1'-0"



NOTES:

1. THIS DIAGRAM SHALL APPLY TO POURED IN PLACECONC. BEAMS & SLABS U.O.N.
2. TOP & BOTT. STEEL ARE IN SEPARATE LAYERS.
3. TOP CONT. STEEL LAPS TYPICALLY AT MID-SPAN OR A MIN. OF 10' - 0" FROM ℄ OF COL. OR BEAM.
4. STIRRUP SPACING BEGINS AT FACE OF SUPPORT U.O.N.

TYPICAL CONCRETE BEAM REINFORCING DETAIL



TYP. CMU JOINT REINF. DET.

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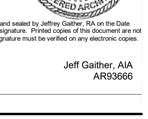

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Sheet Title
STRUCTURAL DETAILS

S900

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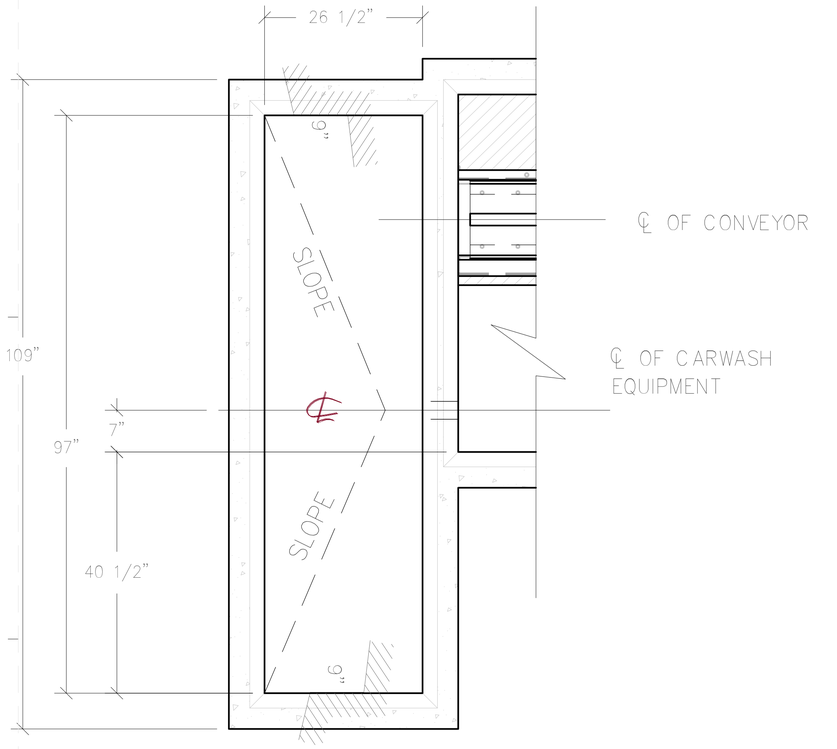
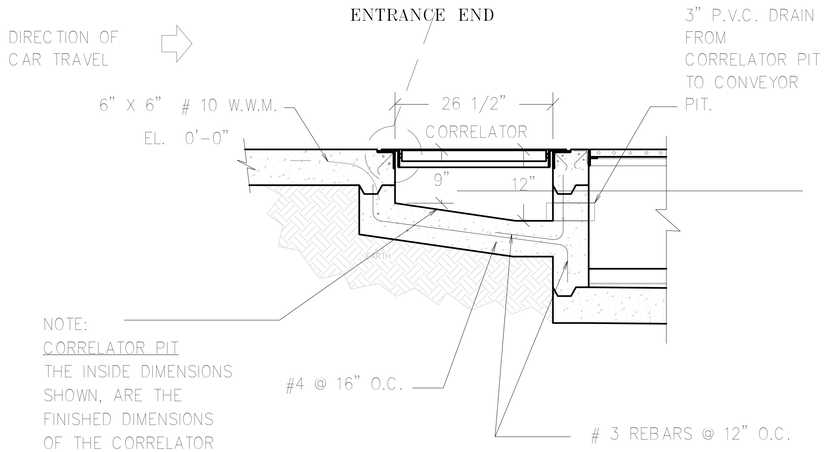
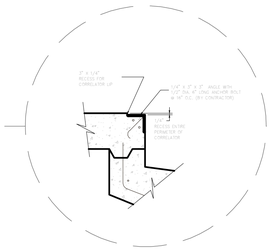


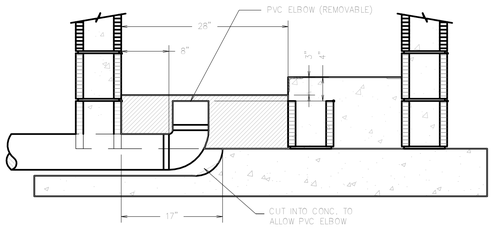
1/S301 CORRELATOR DETAIL PLAN



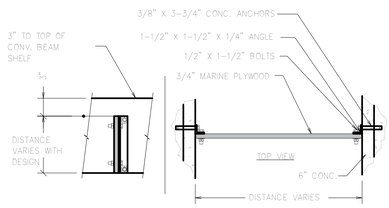
2/S301 SECTION THRU CORRELATOR



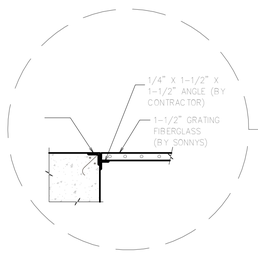
DETAIL CORRELATOR LIP ANGLE
SCALE: N.T.S.



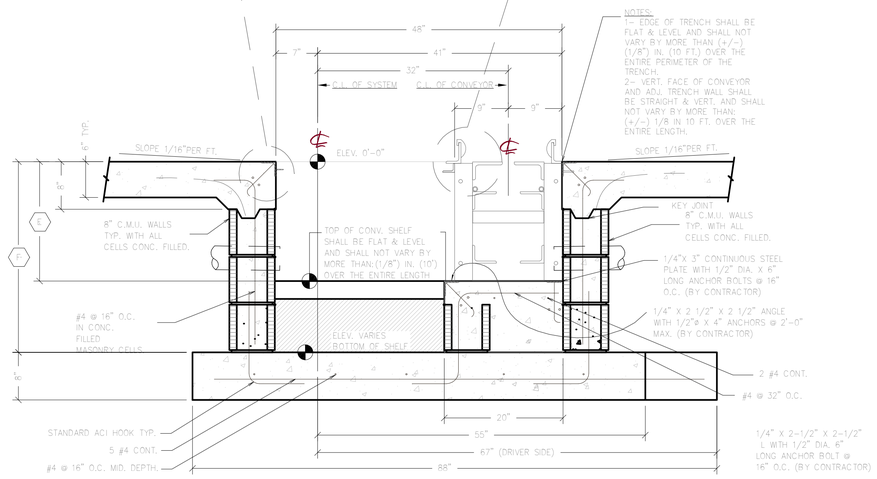
TRENCH DRAINAGE PIPE



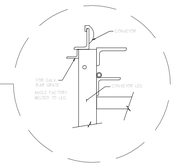
TRENCH DAM SECTION



BAR GRATE DETAIL



3/S301 SECTION THRU TRENCH



DETAIL

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Federal Hwy
Waters Car Wash
STUART, FL

CONSULTANT:

Issued For

PERMIT SET	06/14/24

Revisions

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Project No: 22-169
Drawn By: Jeff Gaither, AIA AR93666

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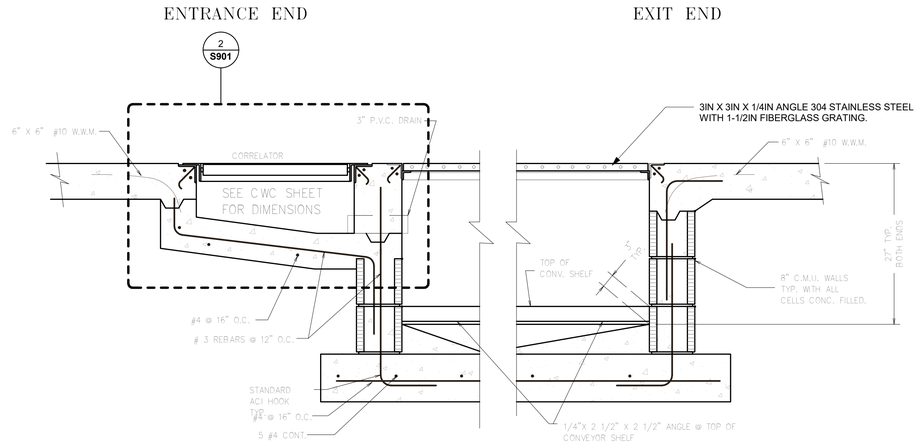
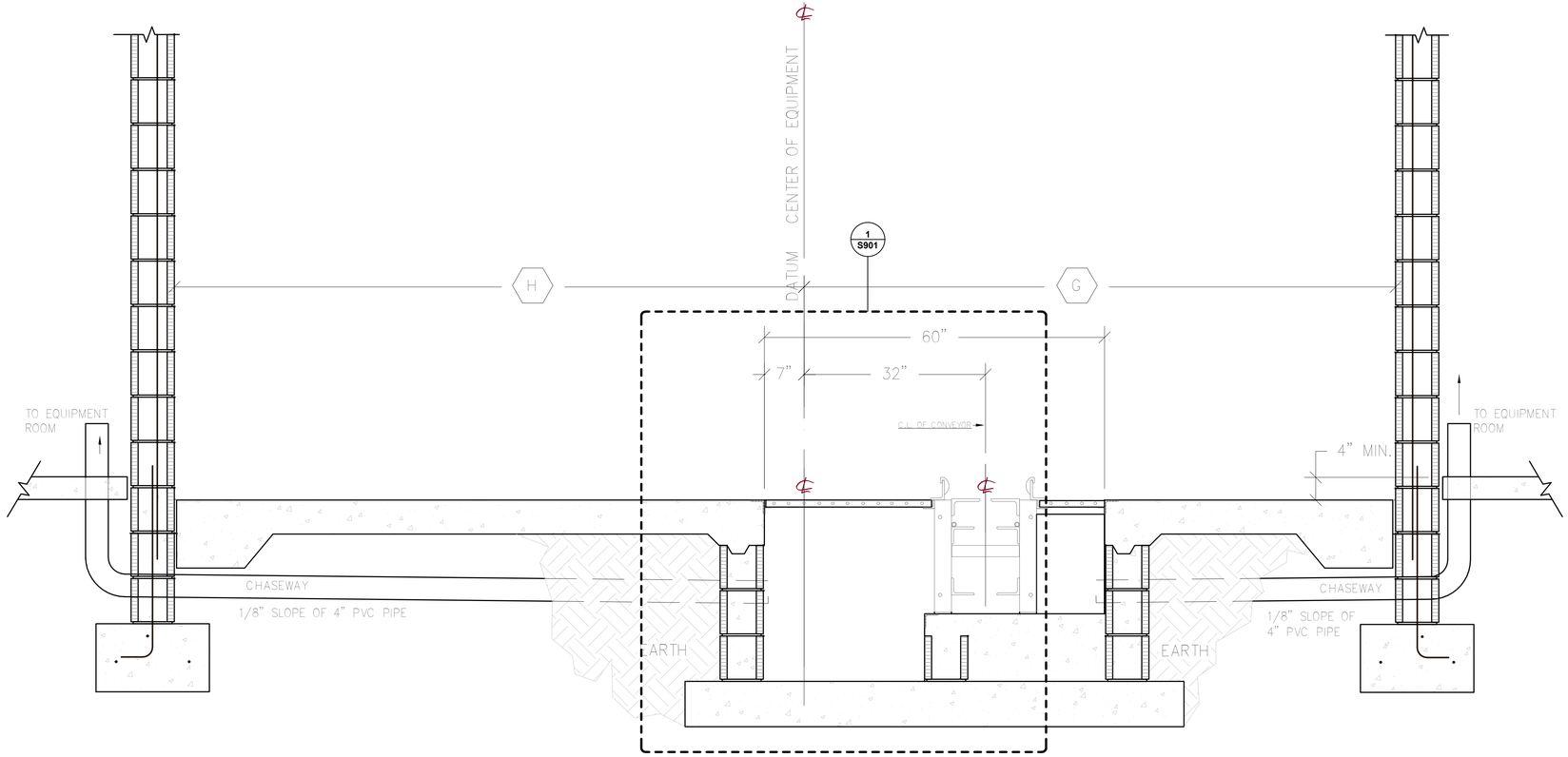
TRENCH STRUCTURAL DETAILS

S901

6/13/2024 11:29:32 AM



1/S902 SECTION THRU TRENCH/CORRELATOR



2/S902 SECTION THRU TUNNEL

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STUDIO 407

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