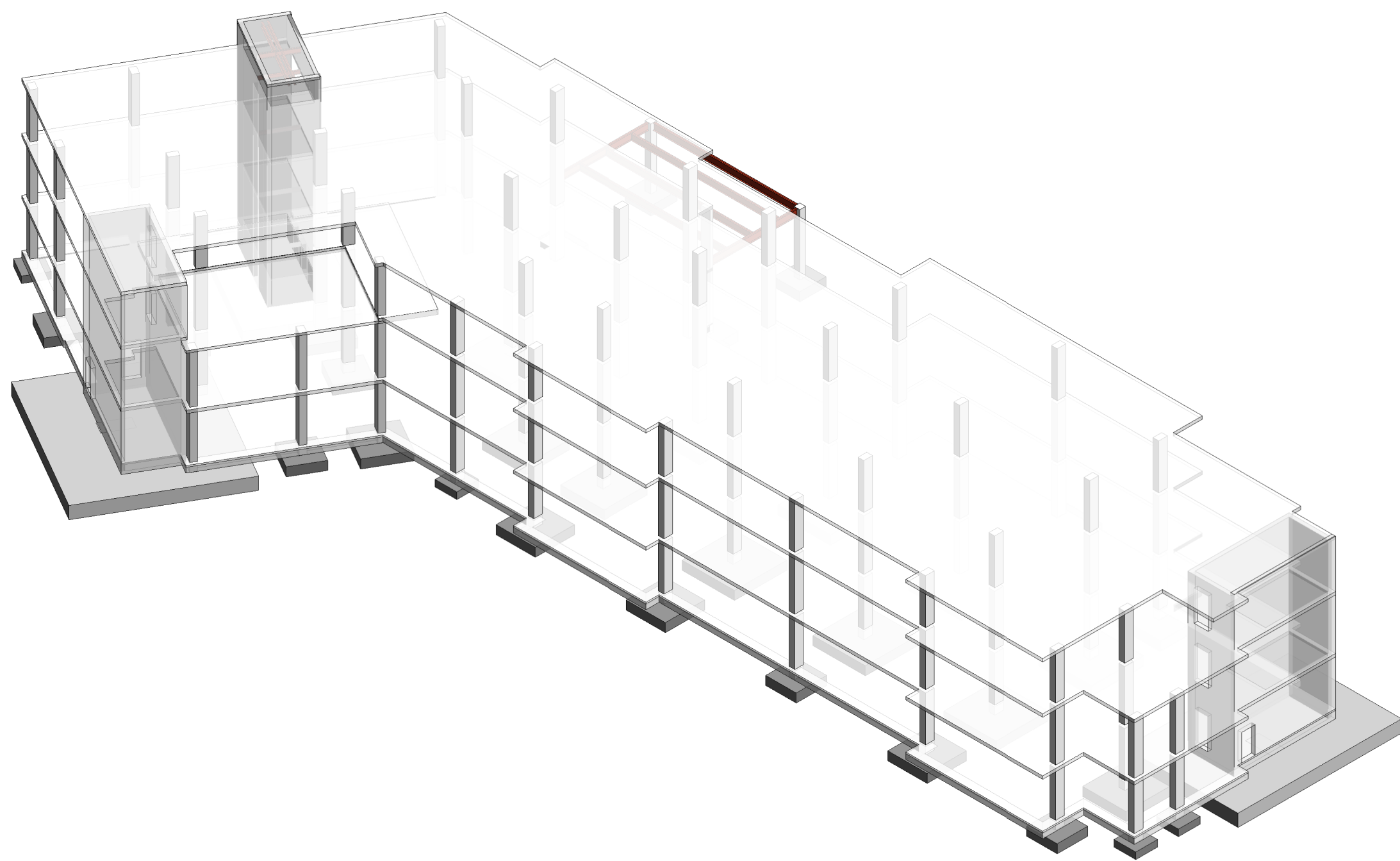


3D
S-001

ISOMETRIC VIEW - FOR REFERENCE ONLY



GENERAL REQUIREMENTS

- A. THE GENERAL STRUCTURAL NOTES EMPLOY THE FOLLOWING DEFINITIONS AND ABBREVIATIONS.
1. CONTRACT DOCUMENTS – THE LATEST SET OF DRAWINGS, SPECIFICATIONS, AND RECORDED ADDENDA AND AMENDMENTS ISSUED FOR BID OR CONSTRUCTION.
 2. LICENSED PROFESSIONAL (STRUCTURAL) ENGINEER – AN ENGINEER LICENSED IN THE STATE IN WHICH THE PROJECT IS LOCATED AND QUALIFIED TO PERFORM THE WORK REQUIRED.
 3. STRUCTURAL ENGINEER OF RECORD – LICENSED PROFESSIONAL ENGINEER WHO IS IN RESPONSIBLE CHARGE FOR THE PREPARATION, SIGNING, DATING, SEALING, AND ISSUING OF STRUCTURAL ENGINEERING DOCUMENTS FOR ENGINEERING SERVICE OR CREATIVE WORK.
 4. DELEGATED ENGINEER – A LICENSED PROFESSIONAL ENGINEER WHO PROVIDES SERVICES OR CREATIVE WORK REGARDING A PORTION OF THE ENGINEERING PROJECT. THE DELEGATED ENGINEER IS THE ENGINEER OF RECORD FOR THAT PORTION OF THE ENGINEERING PROJECT. TECHNICALLY, DELEGATED ENGINEERS FALL INTO ONE OF THE FOLLOWING CATEGORIES:
 - a. AN INDEPENDENT CONSULTANT
 - b. AN EMPLOYEE OR OFFICER OF AN ENTITY SUPPLYING COMPONENTS TO A FABRICATOR OR CONTRACTOR
 - c. AN EMPLOYEE OR OFFICER OF A FABRICATOR OR CONTRACTOR
 5. DELEGATED ENGINEERING DOCUMENTS – ENGINEERING DOCUMENTS THAT ARE PREPARED BY A DELEGATED ENGINEER.
 6. DESIGN TEAM – DESIGN PROFESSIONALS INCLUDING THE ARCHITECT, STRUCTURAL ENGINEER, CIVIL ENGINEER, MEP ENGINEER, AND ANY OTHER CONSULTANT THAT ISSUES CONTRACT DOCUMENTS.
 7. CONTRACTOR – GENERAL CONTRACTOR, CONSTRUCTION MANAGER, DESIGN BUILDER, OR ANY OTHER ENTITY CONTRACTED BY THE OWNER TO PERFORM THE WORK.
 8. SHOP DRAWINGS – DRAWINGS DEPICTING INSTALLATION MEANS AND METHODS AND CATALOG INFORMATION ON STANDARD PRODUCTS. SHOP DRAWINGS SHALL BE PREPARED BASED ON ENGINEERING DIRECTION CONTAINED IN CONTRACT DOCUMENTS BY A CONTRACTOR, FABRICATOR, MANUFACTURER, OR LICENSED PROFESSIONAL ENGINEER, FOR INCORPORATION INTO THE PROJECT.
 9. ESTABLISHED CHANNELS – AT THE ONSET OF THE PROJECT, ARCHITECT, OWNER, AND CONTRACTOR SHALL ESTABLISH DESIRED LINES OF COMMUNICATION BETWEEN ALL PROJECT PARTIES. THESE AGREED UPON LINES OF COMMUNICATION ARE THE ESTABLISHED CHANNELS.
- B. GENERAL STRUCTURAL NOTES ARE APPLICABLE TO THE DESIGN AND CONSTRUCTION OF THE ENTIRE PROJECT AND THIS ARE APPLICABLE TO EVERY SHEET WITHIN THIS PROJECT.
- C. WHERE A DETAIL, TYPICAL DETAIL, SECTION, TYPICAL SECTION, OR PLAN NOTE IS SHOWN FOR ONE CONDITION, IT SHALL APPLY FOR ALL SIMILAR OR LIKE CONDITIONS, UNLESS NOTED OTHERWISE.
- D. ISOMETRIC VIEWS ARE FOR INFORMATION PURPOSES ONLY AND DO NOT CONVEY ALL OF THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.
- E. SHOULD THE CONTRACTOR ENCOUNTER A CONFLICT BETWEEN THESE DRAWINGS AND ANY OTHER CONTRACT DOCUMENT OR APPLICABLE CODE OR STANDARD OF PRACTICE DURING BIDDING, THE PROVISION RESULTING IN THE GREATER COST APPLIES. SHOULD THE CONTRACTOR ENCOUNTER A CONFLICT DURING CONSTRUCTION, THE CONTRACTOR SHALL SUBMIT A WRITTEN REQUEST FOR CLARIFICATION TO THE DESIGN TEAM, WHO WILL PROVIDE A WRITTEN RESPONSE IN RETURN.
- F. SPECIFICATIONS HAVE BEEN ISSUED ON THIS PROJECT BY THE STRUCTURAL ENGINEER OF RECORD AND ARE AN INTEGRAL PART OF THE CONTRACT DOCUMENTS.
- G. SEE SPECIFICATIONS FOR MATERIALS TESTING REQUIREMENTS.
- H. THE CONTRACTOR SHALL SUPERVISE AND DIRECT ALL WORK AND SHALL BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, AND SEQUENCE. THE CONTRACTOR HAS SOLE RESPONSIBILITY FOR THE QUALITY AND CORRECTNESS OF THE WORK.
- I. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR COORDINATION OF THE STRUCTURAL WORK WITH OTHER TRADES INCLUDING, BUT NOT LIMITED TO: ARCHITECTURAL, CIVIL, AND MEP FOR FLOOR SLAB STEPS, SLOPES AND CURBS, FLOOR SLAB FINISH, OPENINGS IN STRUCTURAL FLOORS, ROOFS AND WALLS, ETC.
- J. THE BUILDING HAS BEEN DESIGNED BY THE STRUCTURAL ENGINEER OF RECORD TO RESIST THE CODE REQUIRED VERTICAL AND LATERAL FORCES IN ITS FULLY COMPLETED CONDITION. THE CONTRACTOR SHALL PROVIDE ALL REQUIRED BRACING, SHORING, AND OTHER CONSTRUCTION SUPPORTS NECESSARY TO ENSURE THE BUILDING'S STABILITY AND SAFETY THROUGHOUT THE DURATION OF CONSTRUCTION. FURTHER, THE CONTRACTOR SHALL NOT OVERLOAD THE STRUCTURE DURING CONSTRUCTION. THE CONTRACTOR SHALL RETAIN A LICENSED PROFESSIONAL ENGINEER TO PROVIDE THE ANALYSIS AND DESIGN NECESSARY TO DETERMINE POTENTIALLY OVERLOADED, UNSTABLE, OR HAZARDOUS CONDITIONS THAT MAY OCCUR AT ANY STAGE DURING CONSTRUCTION.
- K. THE CONTRACTOR SHALL NOT EMPLOY CONSTRUCTION MEANS OR METHODS THAT MAY DAMAGE UTILITIES, ADJACENT BUILDINGS, OR PROPERTY. DOCUMENTATION OF ADJACENT CONDITIONS PRIOR TO CONSTRUCTION IS RECOMMENDED. FURTHER, THE CONTRACTOR SHALL EITHER ADEQUATELY CONFINE THE SITE OR PROTECT ADJACENT PROPERTY FROM DAMAGE.
- L. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR PROJECT SAFETY AND OSHA REQUIREMENTS. SHOULD THE STRUCTURAL ENGINEER OF RECORD NOTIFY THE CONTRACTOR OF A POTENTIALLY UNSAFE CONDITION, IT IS SOLELY AS A COURTESY FROM ONE PROFESSIONAL TO ANOTHER. IT SHOULD NOT BE INTERPRETED AS THE STRUCTURAL ENGINEER OF RECORD ASSUMING ANY RESPONSIBILITY FOR PROJECT SAFETY.
- M. ALL STRUCTURES REQUIRE PERIODIC MAINTENANCE TO EXTEND LIFE SPAN AND ENSURE STRUCTURAL INTEGRITY FROM EXPOSURE TO THE ENVIRONMENT. A PLANNED PROGRAM OF MAINTENANCE SHALL BE ESTABLISHED BY THE BUILDING OWNER. THIS PROGRAM SHALL INCLUDE, BUT NOT BE LIMITED TO: PAINTING OF STRUCTURAL STEEL, PROTECTIVE COATINGS FOR CONCRETE, SEALANTS, CAULKED JOINTS, EXPANSION JOINTS, CONTROL JOINTS, SPALLS AND CRACKS IN CONCRETE, AND PRESSURE WASHING OF EXPOSED STRUCTURAL ELEMENTS EXPOSED TO A SALINE OR OTHER HARSH CHEMICAL ENVIRONMENT.
- N. THE USE OF DE-ICING CHEMICALS ON ANY EXPOSED STRUCTURAL ELEMENT IS DISCOURAGED AND WILL ACCELERATE DETERIORATION OF STRUCTURAL ELEMENTS.
- O. THE BUILDING OWNER SHALL NOT ALTER OR MODIFY ANY STRUCTURAL ELEMENT WITHOUT CONSULTING A LICENSED PROFESSIONAL ENGINEER. FURTHER, BUILDING OWNER SHALL NOT RENOVATE, REPURPOSE, ADD-ON TO, OR OTHERWISE MODIFY THE EXISTING STRUCTURAL SYSTEMS WITHOUT CONSULTING A LICENSED PROFESSIONAL ENGINEER.
- P. CONTRACT DRAWINGS SHOW MAJOR OPENINGS IN FLOORS AND WALLS AND DO NOT NECESSARILY SHOW ALL OPENINGS REQUIRING SHORING. THE CONTRACTOR SHALL COORDINATE ALL OPENING SIZES AND LOCATIONS BETWEEN ALL DISCIPLINES AND TRADES. ADDITIONAL OPENINGS, BLOCKOUTS, AND SLEEVES MAY BE REQUIRED AND SHALL BE CONSTRUCTED USING THE TYPICAL DETAILS AND/OR REQUIREMENTS WITHIN THE CONTRACT DOCUMENTS. OPENINGS REQUIRED, BUT NOT SHOWN ON THE STRUCTURAL DRAWINGS, MUST BE APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
- Q. THE CONTRACTOR SHALL COORDINATE PIPING AND CONDUIT EMBEDDED IN OR ATTACHED TO SLABS, SLAB-ON-DECK BEAMS, AND COLUMNS. ANY REQUIRED MODIFICATIONS TO STRUCTURAL MEMBERS OR THEIR REINFORCEMENT AS A RESULT OF EMBEDMENT OR ATTACHMENT SHALL BE SUBMITTED TO THE DESIGN TEAM FOR THEIR REVIEW. SEE GENERAL STRUCTURAL NOTES SECTION "DESIGN CRITERIA" FOR LIMITATIONS OF MEP LOADING ON STRUCTURAL SYSTEMS.
- R. THE STRUCTURAL ENGINEER OF RECORD'S ROLE DURING CONSTRUCTION
1. THE STRUCTURAL ENGINEER OF RECORD SHALL NOT ASSUME CONTROL OF, OR RESPONSIBILITY FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, PROJECT SAFETY, THE ACTS AND OMISSIONS OF THE CONTRACTOR, OR THEIR FAILURE TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
 2. STRUCTURAL ENGINEER OF RECORD SHALL NOT HAVE AUTHORITY TO STOP THE WORK OR AUTHORIZE CHANGES TO ANY CONTRACT SUM.
 3. PERIODIC SITE VISITS BY REPRESENTATIVES OF THE STRUCTURAL ENGINEER OF RECORD ARE SOLELY FOR THE PURPOSE OF BECOMING GENERALLY FAMILIAR WITH THE PROGRESS AND QUALITY OF THE WORK AND DETERMINING, IN GENERAL, IF THE WORK OBSERVED IS BEING PERFORMED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THIS LIMITED OBSERVATION SHOULD NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS AND THAT OBSERVATIONS ARE QUALITATIVE, NOT QUANTITATIVE. THIS LIMITED INFORMATION WILL BE USED TO ADVISE THE OWNER, CONTRACTOR, OR ARCHITECT OF POTENTIAL DEFICIENCIES.
- S. CLARIFICATION OF POSITION OF STRUCTURALLY FRAMING ELEMENTS
1. USE ONLY DIMENSIONS INDICATED ON THE DRAWINGS, DO NOT SCALE ANY DIMENSIONS.
 2. IF NOT INDICATED ON DRAWINGS, ASSUME EQUAL SPACING BETWEEN ESTABLISHED DIMENSIONS.
 3. CENTER LINES OF COLUMNS AND FOUNDATIONS SHALL COINCIDE WITH GRID LINE INTERSECTION, UNLESS NOTED OTHERWISE.
 4. CENTER LINES OF FOOTINGS, GRADE BEAMS, AND WALLS SHALL COINCIDE WITH CENTER LINES OF FOUNDATIONS, UNLESS NOTED OTHERWISE.
 5. CENTER LINES OF FRAMING MEMBERS SHALL COINCIDE WITH COLUMN CENTER LINES, UNLESS NOTED OTHERWISE.
 6. ELEVATIONS SHOWN ARE TO TOP OF FOUNDATIONS, SLABS, OR BEAMS, UNLESS NOTED OTHERWISE.
- T. SEE ARCHITECTURAL, CIVIL, MEP, AND VERTICAL TRANSPORTATION CONTRACT DOCUMENTS FOR ADDITIONAL INFORMATION RELATING TO THE COORDINATION OF STRUCTURAL COMPONENTS INCLUDING, BUT NOT LIMITED TO:
1. CIVIL
 - a. SITING OF BUILDING GRID LINES WITH RESPECT TO CITY BENCHMARKS
 - b. SITE PREPARATION
 - c. BACKFILLING MATERIALS AND REQUIREMENTS INCLUDING DRAINAGE ADJACENT TO RETAINING WALLS
 - d. SITE ELEMENTS OUTSIDE OF BUILDING ENVELOPE
 - e. NEW AND EXISTING SITE UTILITIES
 2. ARCHITECTURAL
 - a. PLAN DIMENSIONS AND PROJECT DATUM
 - b. SLAB EDGE DIMENSIONS AND FINISH ELEVATIONS
 - c. WATERPROOFING AND DAMP-PROOFING DETAILS
 - d. SLAB SLOPES, STEPS AND DEPRESSIONS, RAMPS, TRENCHES
 - e. EMBEDMENTS, INSERTS, BLOCKOUTS, ETC.
 - f. CONCRETE FINISHES AND TOPPING SLABS
 - g. CONCRETE CURBS AND HOUSEKEEPING PADS
 - h. INTERIOR NON-STRUCTURAL MASONRY PARTITIONS
 - i. LIFE SAFETY, FIRE RATING
 - j. CAST-IN-PLACE STAIRS AND SUPPORTS
 - k. OPERABLE PARTITIONS
 3. MEP
 - a. PIPE AND DUCT SIZES FOR OPENING AND SLEEVE COORDINATION
 - b. FLOOR DRAINS
 - c. UNDERFLOOR AND PERIMETER DRAINAGE SYSTEMS
 - d. EQUIPMENT CURBS
 - e. CONDUITS AND EMBEDMENTS IN WALLS AND SLABS
 4. VERTICAL TRANSPORTATION
 - a. INSERTS, HANGERS, TRENCHES, PITS, CONDUITS IN WALLS AND SLAB
- U. THIS BUILDING QUALIFIES AS A THRESHOLD BUILDING PER CHAPTER 653.71 OF THE FLORIDA STATUTES. AS SUCH, SPECIAL INSPECTIONS SHALL BE REQUIRED PER CHAPTER 653.73 OF THE FLORIDA STATUTES BY PERSONS DULY AUTHORIZED TO PERFORM THEM BY CHAPTER 61G15-35.003 OF THE FLORIDA ADMINISTRATIVE CODE. SEE SPECIAL INSPECTION PLAN FOR SPECIFIC INSPECTION REQUIREMENTS.

ELECTRONIC DATA/REPRODUCTION

- A. ALL INFORMATION CONTAINED IN THE ELECTRONIC FILES OF THE CONTRACT DOCUMENTS ARE INSTRUMENTS OF SERVICE OF THE ARCHITECT/STRUCTURAL ENGINEER OF RECORD AND SHALL NOT BE USED FOR OTHER PROJECTS, ADDITIONS TO THE PROJECT, OR THE COMPLETION OF THE PROJECT BY OTHERS. ELECTRONIC FILES OF THE STRUCTURAL DOCUMENTS REMAIN THE PROPERTY OF JEZERINAC GROUP AND IN NO CASE SHALL THEIR TRANSFER BE CONSIDERED A SALE.
- B. THE USE OF ELECTRONIC FILES OR REPRODUCTIONS OF THESE CONTRACT DOCUMENTS BY ANY CONTRACTOR, SUBCONTRACTOR, ERECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFIES THEIR ACCEPTANCE OF ALL INFORMATION SHOWN HEREIN AS CORRECT AND OBLIGATES THEMSELVES TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS OR OMISSIONS THAT MAY OCCUR HEREIN. THE USE OF ELECTRONIC FILES DOES NOT RELIEVE THE CONTRACTOR'S RESPONSIBILITY FOR PROPER CHECKING AND COORDINATION OF DIMENSIONS, DETAILS, SIZE, AND QUANTITIES.
- C. THE USE OF THE PREPARATION OF SHOP DRAWINGS, ALL INFORMATION NOT APPLICABLE TO THE SUBCONTRACT SHALL BE REMOVED FROM THE DRAWINGS, INCLUDING, BUT NOT LIMITED TO: SHEET NUMBERS, SECTION MARKS, TITLE BLOCKS, AND REFERENCES TO THE CONTRACT DOCUMENTS.
- D. WHEN USED FOR THE PREPARATION OF SHOP DRAWINGS, ALL INFORMATION NOT APPLICABLE TO THE SUBCONTRACT SHALL BE REMOVED FROM THE DRAWINGS, INCLUDING, BUT NOT LIMITED TO: SHEET NUMBERS, SECTION MARKS, TITLE BLOCKS, AND REFERENCES TO THE CONTRACT DOCUMENTS.
- E. THE SUBMITTAL IS REQUIRED.
- F. THE SUBMITTAL IS BASED ON THE LATEST DESIGN.
- G. THE SUBMITTAL IS CLEARLY CLOUDED FOR ALL THE DIFFERENCES FROM THE CONTRACT DOCUMENTS ON THE FIRST SUBMITTAL.
- H. THE SUBMITTAL IS CLEARLY CLOUDED FOR ALL CHANGES AND ADDITION FROM PREVIOUS SUBMITTAL.
- I. THE ARCHITECT'S AND STRUCTURAL ENGINEER OF RECORD'S COMMENTS FROM ANY PREVIOUS SUBMITTALS ARE ADDRESSED.
- J. THE WORK IS COORDINATED AMONGST ALL CONSTRUCTION TRADES.
- K. THE SUBMITTAL IS COMPLETE.
- L. THE SUBMITTAL SHALL INCLUDE A STAMP INDICATING PROJECT NAME AND LOCATION, SUBMITTAL NUMBER, AND SPECIFICATION SECTION NUMBER.
- M. THE STRUCTURAL ENGINEER OF RECORD'S REVIEW OF SUBMITTALS SHALL BE FOR GENERAL CONFORMANCE WITH THE DESIGN INTENT.
- N. THE STRUCTURAL ENGINEER OF RECORD SHALL RETURN, WITHOUT COMMENT, SUBMITTALS WHICH THE CONTRACTOR HAS NOT STAMPED OR WHICH DO NOT MEET THE ABOVE REQUIREMENTS.
- O. FOR THE COMPONENTS DESIGNED BY A DELEGATED ENGINEER, PROVIDE SHOP DRAWINGS, DESIGN CALCULATIONS, AND A COVER LETTER SIGNED AND SEALED BY THE DELEGATED ENGINEER. LETTER SHALL INDICATE THAT THE SHOP DRAWINGS ARE IN CONFORMANCE WITH THE DELEGATED ENGINEER'S CALCULATIONS. REFER TO APPLICABLE SPECIFICATION SECTIONS FOR ADDITIONAL REQUIREMENTS.
- P. DEFERRED SUBMITTALS ARE MANUFACTURER OR CONTRACTOR DESIGNED COMPONENTS PER THE CONTRACT DOCUMENTS. THESE ELEMENTS OF THE DESIGN ARE DEFERRED SUBMITTAL COMPONENTS AND HAVE NOT BEEN PERMITTED UNDER THE BASE BUILDING APPLICATION. DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE ARCHITECT/STRUCTURAL ENGINEER OF RECORD, WHO SHALL REVIEW THEM FOR GENERAL CONFORMANCE TO THE DESIGN OF THE BUILDING. THE CONTRACTOR SHALL SUBMIT THESE REVIEWED DEFERRED SUBMITTAL DOCUMENTS TO THE BUILDING OFFICIAL FOR APPROVAL. THESE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DESIGN TEAM HAS REVIEWED AND THE BUILDING OFFICIAL HAS APPROVED. SEE BELOW FOR THE LIST OF DEFERRED SUBMITTALS.
- Q. THE FOLLOWING SUBMITTALS ARE REQUIRED TO BE SUBMITTED FOR STRUCTURAL ENGINEER OF RECORD REVIEW AS OUTLINED IN THE SPECIFICATIONS.

031000	CONCRETE FORMWORK	(SS, CALC)
032000	CONCRETE REINFORCEMENT LAYOUT	(S)
033000	CONCRETE PRODUCT DATA	(S)
033000	DIMENSION PLANS AND SLEEVE LAYOUT DRAWINGS	(CALC, TA)
033000	CONCRETE MIX DESIGNS	(S)
033000	CONCRETE CONSTRUCTION JOINT LAYOUT	(S)
033000	SLAB-ON-GROUND SAWCUT CONTROL JOINT LAYOUT DRAWINGS	(S)
033816	POST-TENSIONING PRODUCT DATA	(S)
033816	POST-TENSIONING TENDON LAYOUT & INSTALLATION DRAWINGS	(DF, SS, CALC)
033816	POST TENSIONING TENDON FRICTION LOSS CALC	(DF, CALC)
033816	STRESSING RECORDS	(S)
042200	MASONRY PRODUCT DATA	(S)
042200	MASONRY REINFORCEMENT LAYOUT	(S)
051200	STRUCTURAL STEEL	(S)
051200	STRUCTURAL STEEL CONNECTIONS	(DF, S, CALC)
051300	STEEL DECK	(S)
054000	COLD-FORMED METAL FRAMING USED FOR EXTERIOR SHORING AND RESHORING	(SS, CALC) (SEE ARCH)
054000	HANDRAIL, GUARDRAIL, RAILING	(DF, SS, CALC, REC)
054400	PRE-ENGINEERED CFS TRUSS SHOP DRAWINGS	(DF, S)
054400	PRE-ENGINEERED CFS TRUSS DELEGATED DESIGN SUBMITTAL	(DF, SS)
142000	ELEVATOR	(DF, SS, CALC, REC)
312319	DEWATERING	(GEO, REC)

S =	SHOP DRAWING REQUIRED
DF =	DEFERRED SUBMITTAL
SS =	SIGNED AND SEALED SHOP DRAWINGS PREPARED BY A LICENSED DELEGATED ENGINEER IN THE STATE IN WHICH THE PROJECT IS LOCATED.
CALC =	SUPPORTING CALCULATIONS REQUIRED, SIGNED AND SEALED BY A LICENSED DELEGATED ENGINEER IN THE STATE IN WHICH THE PROJECT IS LOCATED.
REC =	ITEMS SUBMITTED FOR RECORD ONLY AND WILL NOT HAVE STRUCTURAL ENGINEER OF RECORD SHOP DRAWING STAMP AFFIXED.
GEO =	ITEMS SUBMITTED TO CONSTRUCTION GEOTECHNICAL ENGINEER FOR THEIR REVIEW.
TA =	ITEMS SUBMITTED TO OWNER'S TESTING AGENCY FOR THEIR REVIEW.
JST(S)	JOIST(S)
K	KIPS (1,000 POUNDS)
KLF	KIP PER LINEAR FOOT
KSF	KIP PER SQUARE FOOT
KSI	KIPS PER SQUARE INCH

GOVERNING CODES & STANDARDS

BUILDING CODE:	FBC 2023	FLORIDA BUILDING CODE, BUILDING
STANDARDS:	ASCE 7	AMERICAN SOCIETY OF CIVIL ENGINEERS: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
	ACI 301	CONCRETE CONSTRUCTION
	ACI 318	AMERICAN CONCRETE INSTITUTE: BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
	ACI 360	THE MASONRY SOCIETY: BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES
	ACI 341	AMERICAN INSTITUTE OF STEEL CONSTRUCTION: SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
	AWS D1.1	AMERICAN WELDING SOCIETY: STRUCTURAL WELDING CODE - STEEL
	AWS D1.3	AMERICAN WELDING SOCIETY: STRUCTURAL WELDING CODE - SHEET STEEL
	AWS D1.4	AMERICAN WELDING SOCIETY: STRUCTURAL WELDING CODE - REINFORCING STEEL
	AI SI 100	AMERICAN IRON AND STEEL INSTITUTE: NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS
	ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS

ABBREVIATIONS

ADOL	ADDITIONAL	L	LENGTH
ADJ	ADJACENT	LB(S)	POUNDS(S)
AFF	ABOVE FINISHED FLOOR	LL	LIVE LOAD
ALT	ALTERNATE	LLH	LONG LEG HORIZONTAL
APPROX	APPROXIMATE	LLV	LONG LEG VERTICAL
ARCH	ARCHITECT OR ARCHITECTURAL	LONG	LONGITUDINAL
ASD	ALLOWABLE STRESS DESIGN	LRFD	LOAD RESISTANCE FACTORED DESIGN
		LSH	LONG SIDE HORIZONTAL
B/	BOTTOM OF	LSV	LONG SIDE VERTICAL
BB	BACK-TO-BACK	LTS	LAP TENSION SPLICE
BLDG	BUILDING	LW	LIGHT WEIGHT
BLKG	BLOCKING	LWC	LIGHT WEIGHT CONCRETE
BP	BASE PLATE	M	MOMENT
BRG	BEARING	MAX	MAXIMUM
BOT	BOTTOM	MC	MOMENT CONNECTION(S)
BTWN	BETWEEN	MECH	MECHANICAL
C	COMPRESSION	MEP	MECHANICAL, ELECTRICAL, PLUMBING,
CFS	COLD-FORMED STEEL	NFR	FIRE PROTECTION
CJ	CONTROL JOINT	MD	MIDDLE
CIP	COMPLETE JOINT PENETRATION	MIN	MINIMUM
CL	CENTER LINE	MISC	MISCELLANEOUS
CLR	CLEAR OR CLEARANCE	N/C	NOT IN CONTRACT
CMU	CONCRETE MASONRY UNIT	NS	NEAR SIDE
COL	COLUMN	NSF	NOT SHOWN FOR CLARITY
CONC	CONCRETE	NTS	NOT TO SCALE
CONN(S)	CONNECTION(S)	NWC	NORMAL WEIGHT CONCRETE
CONST	CONSTRUCTION		
CONT	CONTINUOUS		
COORD	COORDINATE	OC	ON CENTER
		OD	OUTSIDE DIAMETER
D&E	DRILL & EPOXY	OF	OUTSIDE FACE
db	REINFORCING BAR DIAMETER	OH	OPPOSITE HAND
DBA	DEFORMED BAR ANCHOR	OPNG(S)	OPENING(S)
DCA	DEMAND CRITICAL WELD	OPP	OPPOSITE
°	DEGREE(S)	OSL	OUTSTANDING LEG
D	DIAMETER		
DIAG	DIAGONAL	PAF	POWDER ACTUATED FASTENER
DM(S)	DIMENSION(S)	PERP	PERPENDICULAR
DWL	DEAD LOAD	P/F	PREFORMED JOINT FILLER
DWG(S)	DRAWING(S)	PJP	PARTIAL JOINT PENETRATION
EA	EACH	PL	PLATE
EF	EACH FACE	PLF	POUNDS PER LINEAL FOOT
EXP	EXPANSION JOINT	PRE-CAST	PRE-CAST
ELEV	ELEVATION	PREFAB	PREFABRICATED
ELEV	ELEVATOR	PSF	POUNDS PER SQUARE FOOT
EOS	EDGE-OF-SLAB	PSI	POUNDS PER SQUARE INCH
EQ	EQUAL	PT	POST-TENSIONED
EQUIP	EQUIPMENT		
EW	EACH WAY	REF	REFERENCE
EXIST	EXISTING	REFIN	REINFORCE(D) (ING) OR (MENT)
EXP	EXPANSION	REQ(D)	REQUIRED
EXT	EXTERIOR	REV	REVISION
		RTU	ROOF TOP UNIT
FF	FACE-TO-FACE	SCHED	SCHEDULED
FD	FLOOR DRAIN	SDL	SUPERIMPOSED DEAD LOAD
FF	FINISH FLOOR	SER	STRUCTURAL ENGINEER OF RECORD
FND	FOUNDATION	SF	SQUARE FOOT (FEET)
FS	FAR SIDE	SIM	SIMILAR
FT	FEET	SLRS	SEISMIC LOAD RESISTING SYSTEM
FTG	FOOTING	SOG	SLAB-ON-GROUND
		SP	SPACE
GA	GAGE, GAUGE	SPEC(S)	SPECIFICATIONS
GAUV	GALVANIZED	SS	STAINLESS STEEL
GB	GRADE BEAM	STD	STANDARD
GC	GENERAL CONTRACTOR	STIFF	STIFFENER
GR	GIRDER	STR	STRUCTURE OR STRUCTURAL
GEN	GENERAL	SYM	SYMMETRICAL
GYP	GYPSONUM		
		T	TENSION
HCA	HEADED CONCRETE ANCHORS	T&B	TOP AND BOTTOM
HORIZ	HORIZONTAL	T&G	TONGUE & GROOVE
HSS	HOLLOW STRUCTURAL SECTION	T/	TOP OF
ID	INSIDE DIAMETER	TEMP	TEMPERATURE OR TEMPORARY
IF	INSIDE FACE	TYP	TYPICAL
IN	INCH	UNO	UNLESS NOTED OTHERWISE
INFO	INFORMATION	V	VEER
INT	INTERIOR	VERT	VERTICAL
ICC-ES	INTERNATIONAL CODE COUNCIL - EVALUATION SERVICE	VIF	VERIFY IN FIELD
		W/	WITH
		W/O	WITHOUT
		WP	WORK POINT
		WWR	WELDED WIRE REINFORCEMENT

STRUCTURAL THRESHOLD INSPECTIONS

- A. PER CHAPTER 653.71 OF THE FLORIDA STATUTES, THIS BUILDING QUALIFIES AS A THRESHOLD BUILDING.
- B. SPECIAL INSPECTORS OF THRESHOLD BUILDINGS (THRESHOLD INSPECTORS) SHALL MEET THE REQUIREMENTS OF RULE 61G15-35.003 OF THE FLORIDA ADMINISTRATIVE CODE.
- C. PER CHAPTER 653.73 OF THE FLORIDA STATUTES, THE ENFORCING AGENCY SHALL REQUIRE A THRESHOLD INSPECTOR TO PERFORM STRUCTURAL INSPECTIONS ON A THRESHOLD BUILDING PERMANENT TO A STRUCTURAL INSPECTION PLAN PREPARED BY THE STRUCTURAL ENGINEER OF RECORD.
- D. THRESHOLD INSPECTOR SHALL BE A LICENSED PROFESSIONAL ENGINEER IN THE STATE OF FLORIDA WHO IS CERTIFIED UNDER CHAPTER 471.015 OF THE FLORIDA STATUTES, TO CONDUCT INSPECTIONS OF A THRESHOLD BUILDING. FURTHER, THE THRESHOLD INSPECTOR MUST BE ON THE FLORIDA BOARD OF PROFESSIONAL ENGINEERS LIST OF PERSONS QUALIFIED TO BE THRESHOLD INSPECTORS.
- E. THE STRUCTURAL INSPECTION PLAN MUST BE SUBMITTED TO AND APPROVED BY THE ENFORCING AGENCY PRIOR TO THE ISSUANCE OF A BUILDING PERMIT FOR THE CONSTRUCTION OF A THRESHOLD BUILDING.
- F. THE FEE OWNER OF A THRESHOLD BUILDING SHALL SELECT AND PAY ALL COSTS OF EMPLOYING A THRESHOLD INSPECTOR, BUT THE THRESHOLD INSPECTOR SHALL BE RESPONSIBLE TO THE ENFORCEMENT AGENCY.
- G. THRESHOLD INSPECTIONS RELATED TO TEMPORARY CONDITIONS SUCH AS SHORING, RE-SHORING, AND TEMPORARY BRACING ARE REQUIRED TO BE PERFORMED BY THE CONTRACTOR'S DELEGATED ENGINEER. DELEGATED ENGINEER (NOT SPECIAL INSPECTOR) IS RESPONSIBLE FOR THE SUPERVISION, INSPECTION AND CERTIFICATION OF SUCH TEMPORARY SYSTEMS.
- H. SEE STRUCTURAL INSPECTION PLAN FOR FURTHER INFORMATION.

STRUCTURAL DRAWING LIST	
SHEET NUMBER	SHEET NAME
S-001	GENERAL NOTES
S-002A	LOADING CRITERIA
S-002B	LOAD PLANS
S-003	CONCRETE GENERAL NOTES
S-004	MASONRY GENERAL NOTES
S-005	STEEL GENERAL NOTES
S-121	FOUNDATION PLAN
S-122	SECOND FLOOR FRAMING PLAN
S-122-PT	SECOND FLOOR PT PLAN
S-122-R	SECOND FLOOR REINFORCING PLAN
S-123	THIRD FLOOR FRAMING PLAN
S-123-PT	THIRD FLOOR PT PLAN
S-123-R	THIRD FLOOR REINFORCING PLAN
S-124	ROOF FRAMING PLAN
S-124-PT	ROOF PT PLAN
S-124-R	ROOF REINFORCING PLAN
S-300	TYPICAL FOUNDATION DETAILS
S-310	TYPICAL SLAB-ON-GROUND DETAILS
S-320	TYPICAL CONCRETE SHEAR WALLS DETAILS
S-321	CONCRETE SHEAR WALL ELEVATIONS
S-322	CONCRETE SHEAR WALL ELEVATIONS (copy 1)
S-330	TYPICAL PT TENDON DETAILS
S-331	TYPICAL PT REINFORCEMENT DETAILS
S-332	TYPICAL PT REINFORCEMENT DETAILS
S-340	TYPICAL CONCRETE COLUMN DETAILS
S-360	CONCRETE STAIR DETAILS
S-360	TYPICAL CONCRETE FRAMING DETAILS
S-400	TYPICAL MASONRY DETAILS
S-500	STEEL FRAMING DETAILS
S-600	ROOF FRAMING DETAILS

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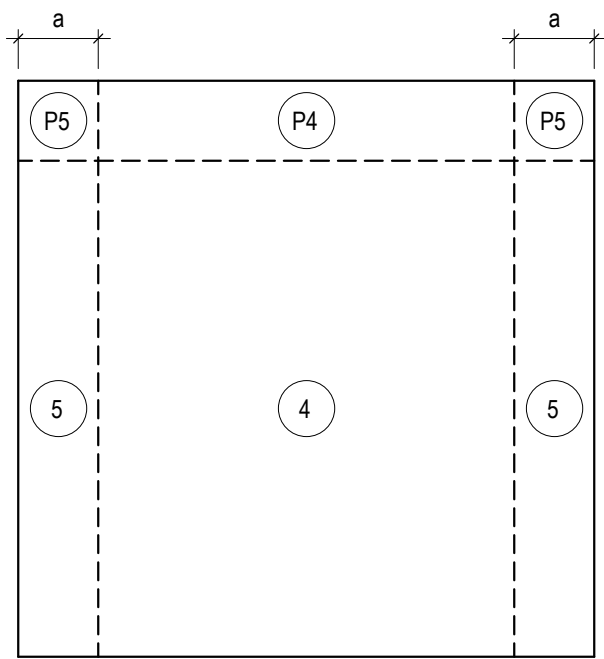
Project No.: 2021009
Date: 08/22/2025

GENERAL
NOTES

S-001

DESIGN CRITERIA

- A. STRUCTURE LOCATION:
LONGITUDE: -80.059
LATITUDE: 26.85
- B. LOADING:
1. SEE LOAD PLANS
2. RAIN LOAD:
DESIGN RAINFALL: 5"/HOUR (100-YEAR, 1-HOUR RAINFALL)
RAINWATER AT LOWEST POINT OF ROOF SHALL, (NOT POND DURING DESIGN RAINFALL / NOT EXCEED 6" DURING DESIGN RAINFALL)
DESIGN RAIN LOAD, R: 30 PSF
3. WIND LOAD:
ULTIMATE DESIGN WIND SPEED, V_{ult} : 170 MPH
NOMINAL DESIGN WIND SPEED, V_{nom} : 132 MPH
RISK CATEGORY: II
EXPOSURE CATEGORY: C
ENCLOSURE CLASSIFICATION: ENCLOSED
INTERNAL PRESSURE COEFFICIENT: ± 0.18
COMPONENTS & CLADDING DESIGN PRESSURES: SEE WIND PRESSURE DIAGRAMS
4. SEISMIC LOAD:
RISK CATEGORY: II
SEISMIC IMPORTANCE FACTOR, I_e : 1.0
SITE CLASS: D (ASSUMED)
MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER, S_{MS} : 0.055 g
MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER, S_{M1} : 0.025 g
SPECTRAL RESPONSE ACCELERATION PARAMETER, S_{MS} : 0.049 g
SPECTRAL RESPONSE ACCELERATION PARAMETER, S_{M1} : 0.035 g
SEISMIC DESIGN CATEGORY: A
SEISMIC FORCE RESISTING SYSTEM: ORDINARY REINFORCED CONCRETE SHEAR WALLS
REDUNDANCY FACTOR, ρ : 1.0
OVERSTRENGTH FACTOR, Q_e : 2.5
RESPONSE MODIFICATION FACTOR, R : 4
SEISMIC RESPONSE COEFFICIENT, C_s : 0.015
EFFECTIVE SEISMIC WEIGHT, W : 6750 KIPS
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE (ELF)
DESIGN BASE SHEAR: 255 KIPS
- C. HANDRAIL AND GUARDRAIL LOADS:
CONCENTRATED AND DISTRIBUTED LOADS ARE TO BE APPLIED AT THE HANDRAIL OR TOP RAIL IN ANY DIRECTION. CONCENTRATED AND DISTRIBUTED LOADS ARE NOT TO BE APPLIED CONCURRENTLY.
CONCENTRATED LOAD: 200 LB
DISTRIBUTED LOAD: 50 PLF
- D. FUTURE EXPANSION:
NO PROVISIONS HAVE BEEN MADE FOR FUTURE VERTICAL OR HORIZONTAL EXPANSION OF THE STRUCTURE.
- E. SERVICEABILITY:
1. DEFLECTION LIMITS: TOTAL LOAD DEFLECTION ONLY APPLIES TO THE DEFLECTION DUE TO THE CREEP COMPONENT OF LONG-TERM DEAD LOAD DEFLECTION PLUS THE SHORT-TERM DEFLECTION. LONG-TERM DEFLECTION OF WOOD STRUCTURAL MEMBERS SHALL BE CALCULATED IN ACCORDANCE WITH THE AWC NDS. IT IS PERMITTED TO ESTIMATE THE CREEP COMPONENT OF THE LONG-TERM DEFLECTION AS THE IMMEDIATE DEAD LOAD DEFLECTION.
a. ROOF MEMBERS:
• TOTAL LOAD DEFLECTION: L/240
• TRANSITORY LOAD DEFLECTION: L/360
b. FLOOR MEMBERS:
• TOTAL LOAD DEFLECTION: L/240
• LIVE LOAD DEFLECTION: L/360
c. EXTERIOR WALLS & CLADDING:
• WIND LOAD DEFLECTION: L/360
d. INTERIOR PARTITIONS:
• LIVE LOAD DEFLECTION: L/240
2. DRIFT LIMITS
a. INTERSTORY DRIFT: H/400
b. TOTAL STRUCTURE DRIFT: H/500

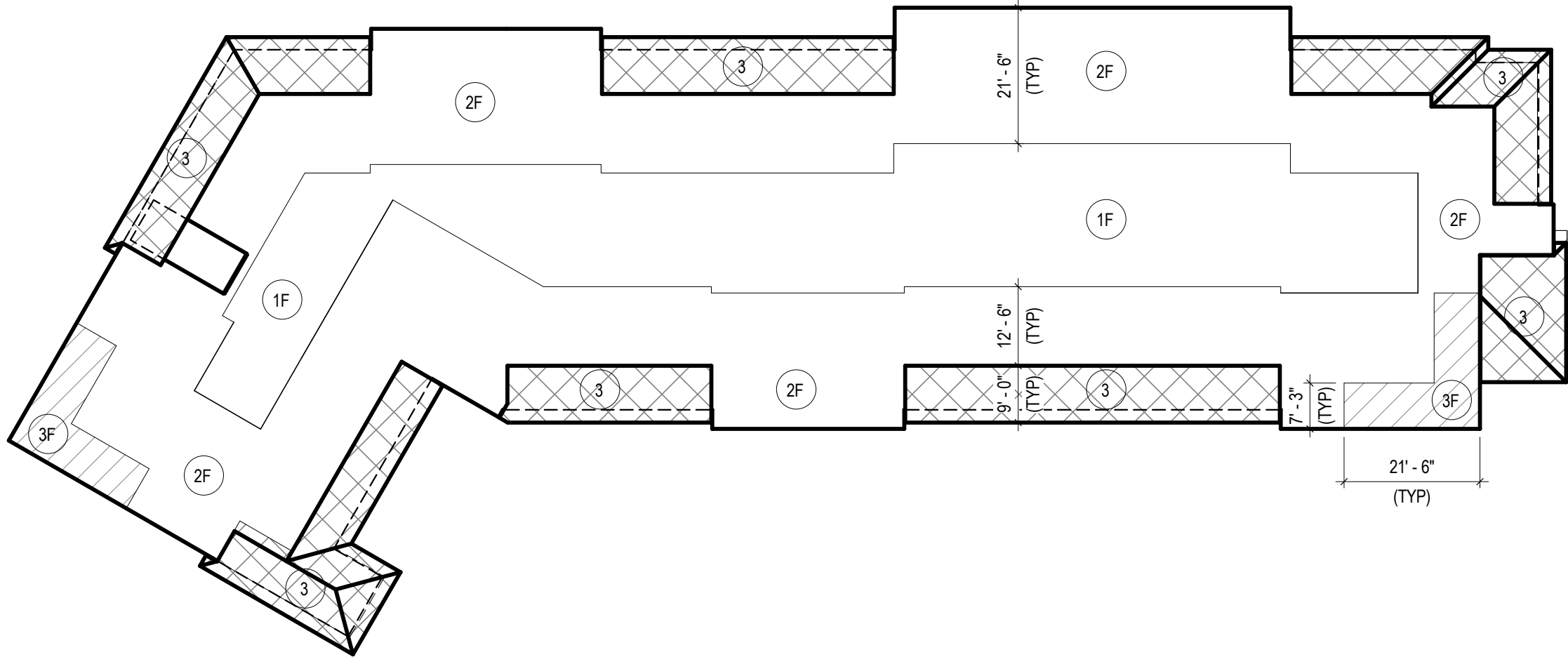


WALL ELEVATION

NOTES:

- FOR COMPONENTS HAVING EFFECTIVE AREAS IN BETWEEN TABULATED VALUES, DESIGN LOADS MAY BE INTERPOLATED. OTHERWISE DESIGN LOAD SHALL BE TAKEN FROM THE NEXT LOWEST TABULATED EFFECTIVE AREA.
- DESIGN VALUES SHOWN IN THIS TABLE ARE ULTIMATE VALUES FOR USE WITH LRFD DESIGN. VALUES MAY BE MULTIPLIED BY 0.6 FOR USE WITH SERVICE LEVEL OR ASD DESIGN. REFER TO THE BUILDING CODE FOR APPLICABLE LOAD COMBINATIONS.
- $a = 6'-8"$: SEE ROOF PLAN MAP BELOW FOR LOCATION OF a -ZONES. WALL a -ZONE LOCATIONS TO MATCH ROOF a -ZONES.
- POSITIVE PRESSURE VALUES REFER TO FORCES ACTING TOWARDS BUILDING OR COMPONENT FACE, NEGATIVE PRESSURE VALUES REFER TO FORCES ACTING AWAY FROM BUILDING OR COMPONENT FACE.
- EACH COMPONENT AND ITS CONNECTION SHALL BE DESIGNED FOR MAXIMUM POSITIVE AND NEGATIVE FORCES.
- PARAPET COMPONENTS AND CLADDING ARE THOSE ELEMENTS WHICH EXIST ABOVE THE HORIZONTAL PLANE OF THE ROOF AND SHALL BE DESIGNED FOR:
 - POSITIVE AND NEGATIVE PRESSURES 4 OR 5 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON OUTSIDE FACE.
 - POSITIVE PRESSURES 4 OR 5 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON ROOF SIDE FACE.
 - NEGATIVE PRESSURES 2 OR 3 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON ROOF SIDE FACE.
 - P4/5 SHALL BE APPLIED TO THE DESIGN OF THE STRUCTURAL ELEMENT OF THE PARAPET AND ITS CONNECTION, INCLUDING BUT NOT LIMITED TO THE STUD FRAMING OF THE PARAPET.
- A DESIGN WIND PRESSURE HORIZONTAL VALUE OF ____ PSF AND VERTICAL VALUE OF ____ PSF SHALL BE APPLIED TO COMPONENTS WHICH ARE EITHER ROOFTOP STRUCTURES OR ROOFTOP APPURTENANCES AND THEIR CONNECTION. EXAMPLES OF THIS ARE RTUs, AHUs, AND SCREEN WALLS.
- ROH# : DENOTES DESIGN WIND PRESSURE VALUES WHICH SHALL BE APPLIED AT ROOF OVERHANGS TO TOP SURFACE CLADDING OR SHEATHING AND ITS CONNECTION. SOFFIT CLADDING OR SHEATHING SHALL BE DESIGNED FOR SIMILAR PRESSURE TO THE ADJACENT WALL PRESSURE. A COMBINATION OF THESE FORCES SHALL BE APPLIED TO THE STRUCTURAL ELEMENT OF THE OVERHANG AND ITS CONNECTION, INCLUDING BUT NOT LIMITED TO THE STUD FRAMING OF THE OVERHANG.
- ALL DOORS TO BE RATED TO RESIST DESIGN WIND PRESSURES SPECIFIED.

EWA (ft)	C&C External Pressure Loads - Main Roof (psf)										
	Zone										
	1	2	3	4	5	P4	P5	ROH 1	ROH 2	ROH 3	
10	59.7	59.7	59.7	59.7	80.1	228.0	228.0	154.7	154.7	154.7	
	-107.2	-147.9	-147.9	-147.9	-107.2	--	--	-175.1	-215.8	-215.8	
20	51.6	51.6	51.6	51.6	76.5	204.0	204.0	139.3	139.3	139.3	
	-95.0	-127.5	-127.5	-127.5	-83.3	--	--	-159.2	-191.8	-191.8	
50	40.7	40.7	40.7	40.7	71.7	172.2	172.2	119.0	119.0	119.0	
	-78.8	-100.5	-100.5	-78.5	-90.5	--	--	-138.3	-160.0	-160.0	
100	32.6	32.6	32.6	32.6	68.1	148.2	151.4	103.6	103.6	103.6	
	-66.5	-80.1	-80.1	-74.9	-83.3	--	--	-122.4	-136.0	-136.0	
200	32.6	32.6	32.6	32.6	64.5	144.6	144.6	96.4	96.4	96.4	
	-66.5	-80.1	-80.1	-80.1	-76.0	--	--	-118.8	-132.4	-132.4	
500	32.6	32.6	32.6	32.6	59.7	139.8	139.8	86.9	86.9	86.9	
	-66.5	-80.1	-80.1	-66.5	-66.5	--	--	-114.0	-127.6	-127.6	



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LOADING
CRITERIA

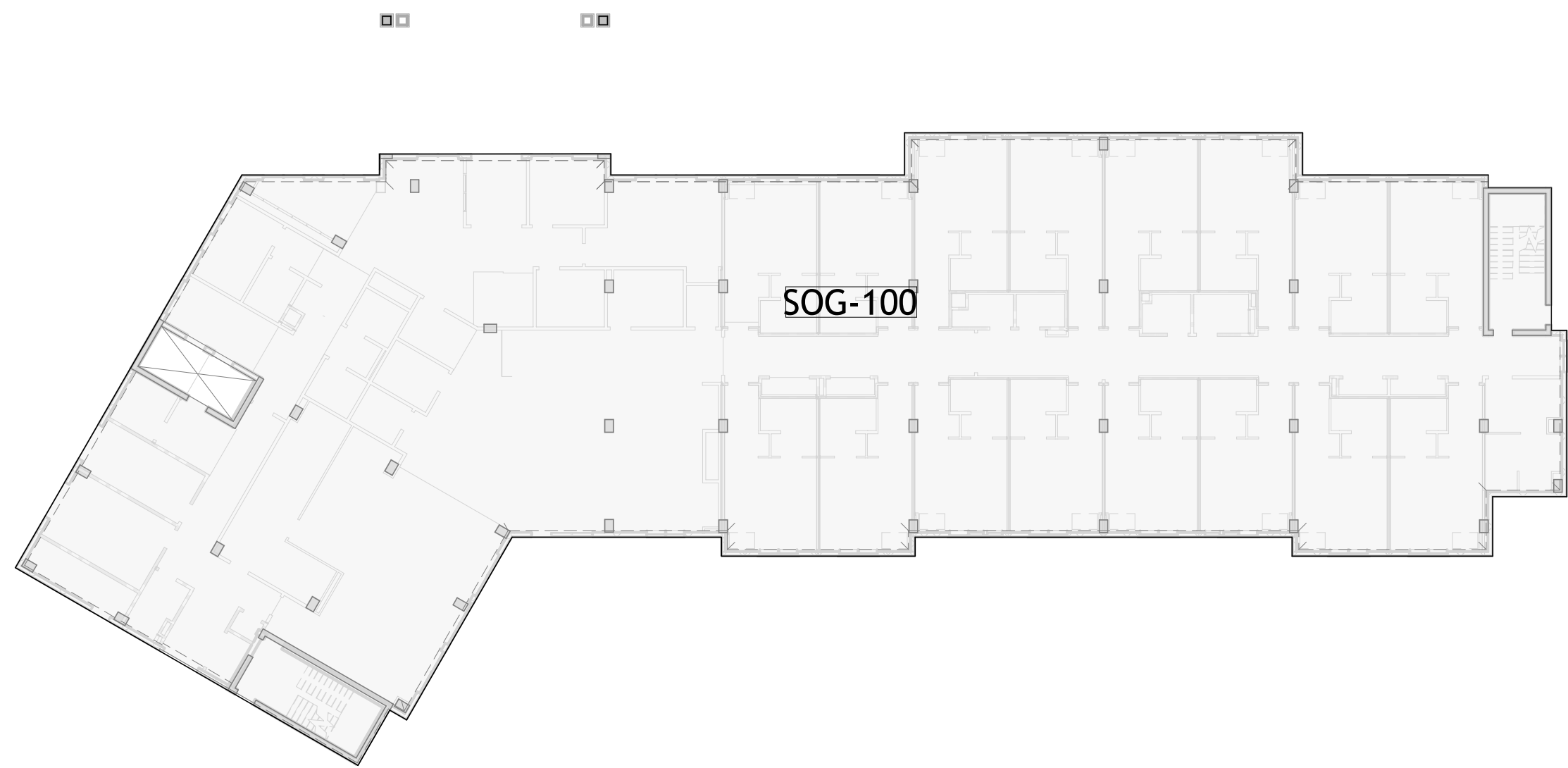
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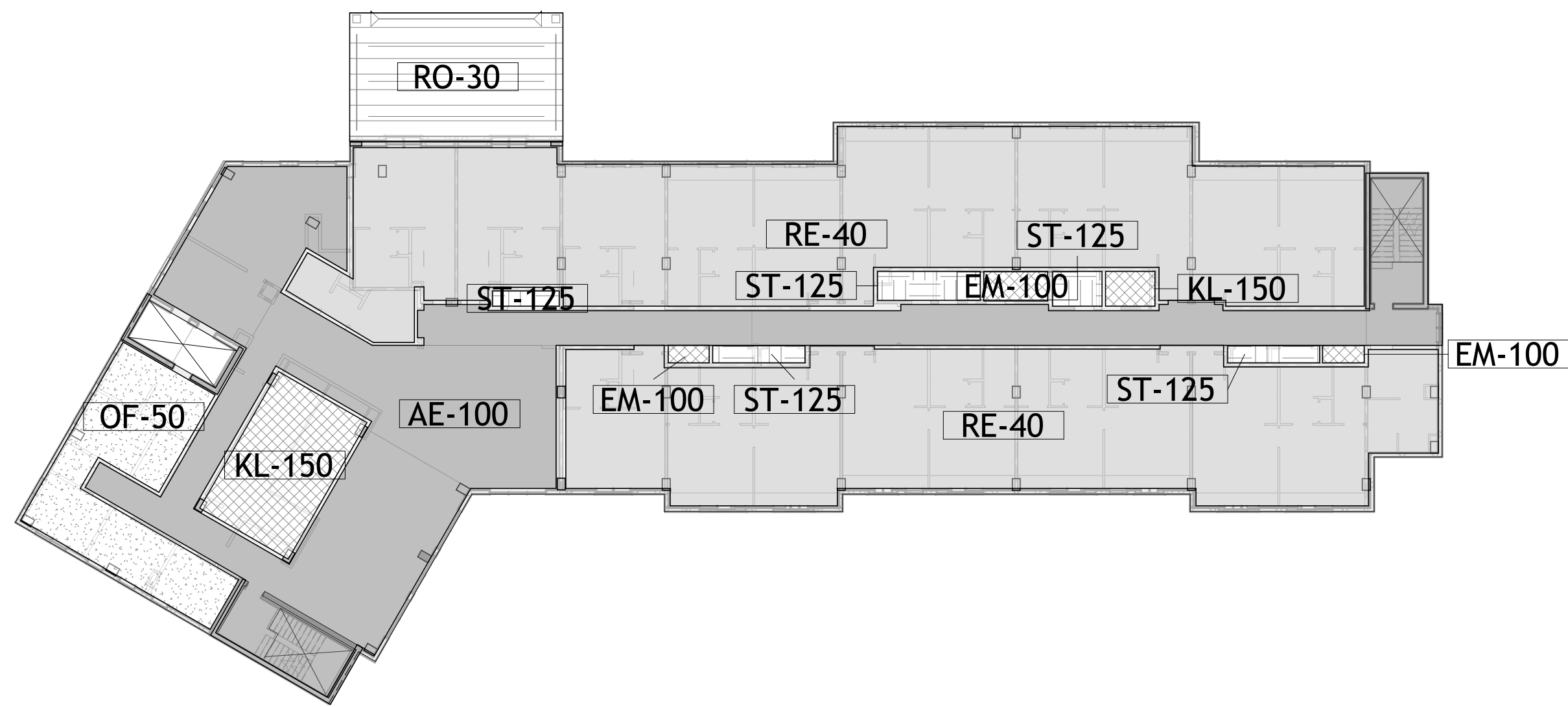
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L1
S-002B

GROUND FLOOR LOAD PLAN

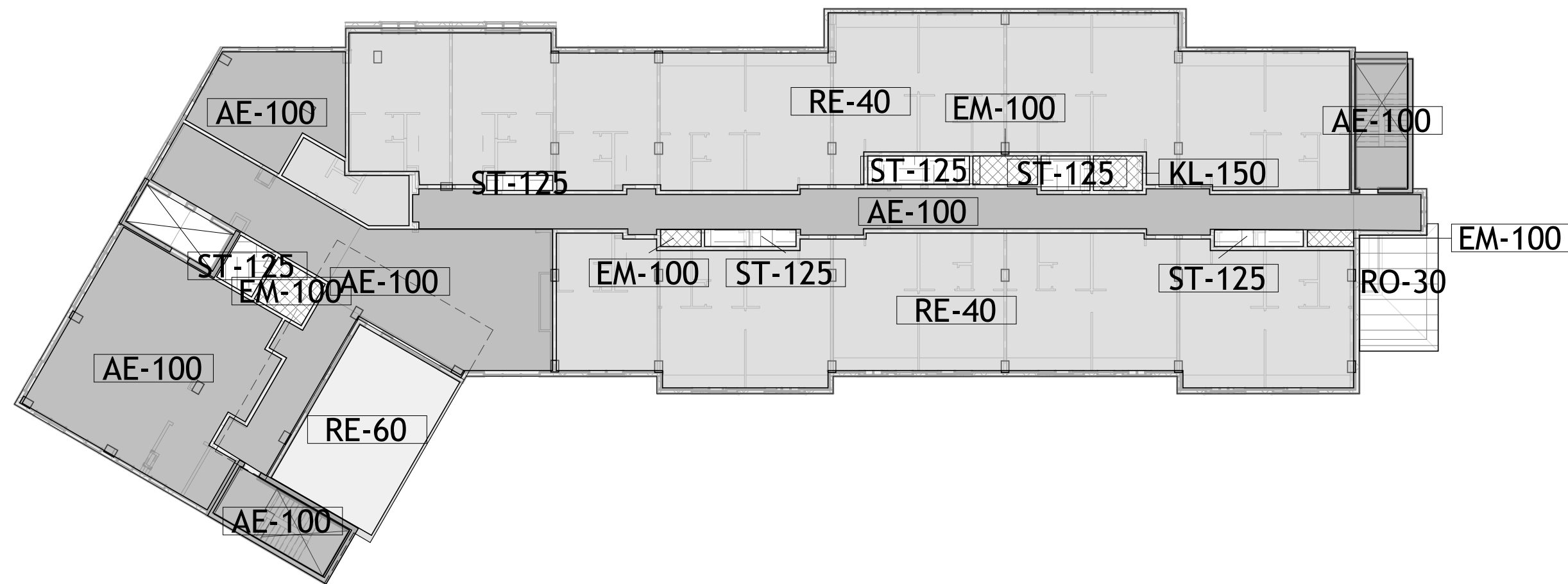
3/64" = 1'-0"



L2
S-002B

SECOND FLOOR LOAD PLAN

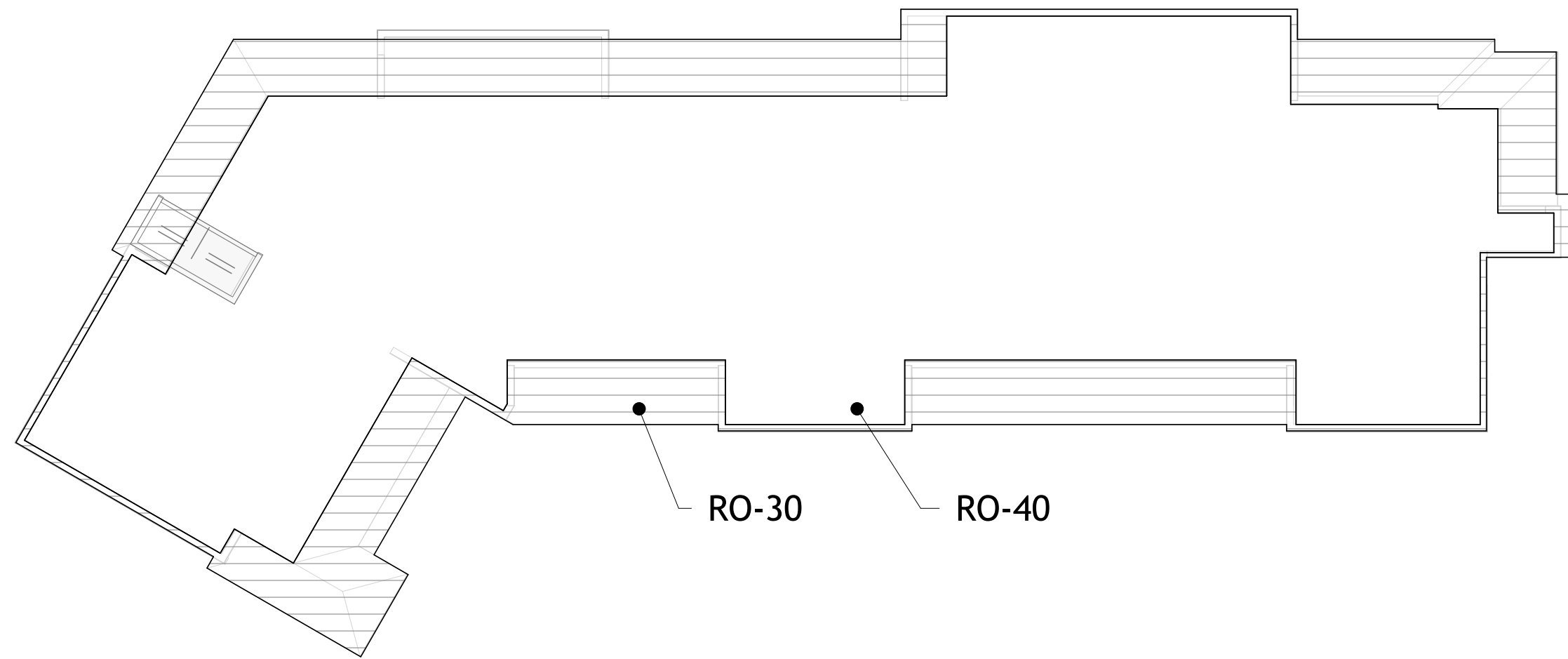
3/64" = 1'-0"



L3
S-002B

THIRD FLOOR LOAD PLAN

3/64" = 1'-0"



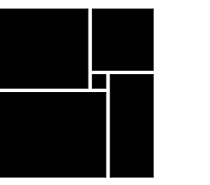
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ROOF LEVEL LOAD PLAN

3/64" = 1'-0"

LOAD MAP KEY					
MARK	OCCUPANCY OR USE	SUPERIMPOSED DEAD LOAD (PSF)	LIVE LOAD (PSF)	LIVE LOAD REDUCTION	COMMENTS
AE-100	ASSEMBLY/GRESS	25	100	No	
EM-100	ELECTRICAL & MECHANICAL	25	100	No	
KL-150	KITCHEN & LAUNDRY	25	150	No	
OF-50	OFFICES	25	50	Yes	
RE-40	RESIDENTIAL	25	40	Yes	
RE-60	BALCONY	25	60	No	
RO-30	ROOF TYPICAL	35	30	Yes	
RO-40	ROOF WELL	10	40	Yes	
SOG-100	SLAB-ON-GROUND	25	100	No	
ST-125	STORAGE	25	125	No	

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LOAD PLANS

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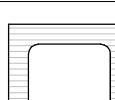
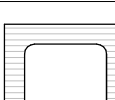
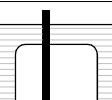
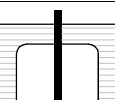
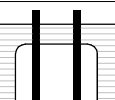
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- CONCRETE MASONRY**
- A. MANUFACTURE AND INSTALL ALL CONCRETE MASONRY IN ACCORDANCE WITH DIVISION 04 SPECIFICATIONS. ALL MASONRY DESIGN SHALL CONFORM TO TMS 602 AND ALL MASONRY CONSTRUCTION SHALL CONFORM TO TMS 602.
- B. ALL LOAD-BEARING, NON-LOAD-BEARING, AND BACKUP WALL CONCRETE MASONRY UNIT CONSTRUCTION SHALL CONFORM TO THE FOLLOWING MATERIAL STANDARDS:
- 1. CONCRETE MASONRY UNITS: ASTM C90, NORMAL WEIGHT (135 PCF)
 - 2. MORTAR: ASTM C270, TYPE 'S' OR 'M' PORTLAND CEMENT/LIME ONLY (USE TYPE 'M' MORTAR WHEN MASONRY IS IN DIRECT CONTACT WITH SOIL; TYPE 'S' IS IN ALL OTHER CONDITIONS)
 - 3. GROUT: ASTM C150, TYPE I (TYPE III MAY BE USED FOR COLD-WEATHER CONSTRUCTION)
 - 4. PORTLAND CEMENT: ASTM C207, TYPE 'S'
 - 5. HYDRATED LIME: ASTM C404 (FOR GROUT)
 - 6. AGGREGATE: ASTM A615, GRADE 60
 - 7. STEEL REINFORCEMENT: ASTM A1064, TRUSS OR LADDER TYPE, GALVANIZE PER ASTM A153, TYPE B-2
- C. CONCRETE MASONRY UNITS:
- 1. F_v SHALL BE 2000 PSI (MINIMUM NET AREA CMU COMPRESSIVE STRENGTH SHALL BE 2000 PSI).
 - 2. LAY CONCRETE MASONRY UNITS IN RUNNING BOND UNLESS NOTED OTHERWISE WITH UNITS DESIGNED TO ALIGN WITH WEBS IN EACH COURSE.
- D. MORTAR:
- 1. HEAD AND BED JOINTS SHALL BE 3/8 INCHES FOR THE THICKNESS OF THE FACE SHELL. WEBS ARE TO BE FULLY MORTARED IN ALL COURSES OF PIERS, COLUMNS AND PLASTERS. IN THE STARTING COURSE, AND WHERE AN ADJACENT CELL IS TO BE GROUTED, REMOVE MORTAR PROTRUSIONS EXTENDING 1/4 INCHES OR MORE INTO CELLS TO BE GROUTED.
 - 2. PROVIDE FULL FACE SHELL MORTAR COVERAGE ON MASONRY UNIT HORIZONTAL AND VERTICAL (BED AND HEAD) FACE SHELL JOINTS.
- E. GROUT:
- 1. MASONRY GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS.
 - 2. GROUT MIX SHALL CONTAIN PORTLAND CEMENT, AGGREGATE, AND A GROUT-ENHANCING SHRINKAGE-COMPENSATING ADMIXTURE.
 - 3. MAXIMUM SIZE OF AGGREGATE SHALL BE 3/8 INCH. SLUMP SHALL BE 8 TO 11 INCHES. WATER REDUCING ADMIXTURES MAY BE USED.
 - 4. GROUT ALL MASONRY CONTAINING REINFORCEMENT, AND WHERE INDICATED ON THE DRAWINGS, ALLOW MORTAR TO CURE 24 HOURS PRIOR TO GROUTING. PROVIDE CLEANOUT OPENINGS AT THE BASE OF THE CELLS CONTAINING REINFORCEMENT TO CLEAN THE CELL AND TO TIE THE VERTICAL BAR TO THE DOWEL. IN HIGH-LIFT GROUTING, USE 5'-0" (MAXIMUM) LIFTS, WITH 1/2 HOUR TO 1 HOUR BETWEEN LIFTS.
 - 5. GROUT SHALL BE VIBRATED WHILE PLACING TO ENSURE THAT CELLS ARE COMPLETELY FILLED.
- F. STEEL REINFORCEMENT:
- 1. PROVIDE VERTICAL REINFORCEMENT IN CELLS OF CONCRETE MASONRY UNITS (FULLY EMBEDDED IN GROUT) AS SHOWN ON THE PLANS AND OTHER DETAILS. MINIMUM REINFORCEMENT OF EXTERIOR MASONRY SHALL BE AS FOLLOWS:
 - a. 1-#5 AT A MAXIMUM SPACING OF 48 INCHES
 - b. 1-#5 AT EACH CORNER
 - c. HEAVIER REINFORCEMENT MAY BE REQUIRED BY PLAN NOTES OR DETAILS IN THE DRAWINGS.
 - 2. REINFORCE WALLS WHERE INDICATED ON THE DRAWINGS AND AT ALL INTERSECTIONS, EACH SIDE OF OPENINGS AND AT THE ENDS OF WALLS. USE BAR SPACERS AT 10 FEET ON CENTER WHERE GROUT POUR HEIGHT EXCEEDS 10 FEET.
 - 3. ALL VERTICAL REINFORCEMENT SHALL HAVE STANDARD HOOK INTO BOND BEAM. TERMINATE AT HIGHEST BOND BEAM IF MASONRY DOES NOT EXTEND TO ROOF OR GROUTED CELL IS NOT CONTINUOUS TO ROOF. HOOK SHALL EXTEND TO THE UPPERMOST HORIZONTAL REINFORCEMENT OF THE BOND BEAM AND HAVE A MINIMUM EMBEDMENT OF 6 INCHES.
 - 4. ALL HORIZONTAL REINFORCEMENT AT ENDS OF BOND BEAMS SHALL HAVE STANDARD HOOK INTO VERTICAL GROUTED CELL. PROVIDE CORNER BARS SUCH THAT HORIZONTAL REINFORCEMENT IS CONTINUOUS AROUND CORNERS.
 - 5. COVER TO STEEL REINFORCEMENT WITHIN MASONRY ELEMENTS SHALL NOT BE LESS THAN THE FOLLOWING:
 - a. EXPOSED TO EARTH OR WEATHER: 1 1/2 INCHES (#5 AND SMALLER BARS); 2 INCHES (#6 AND LARGER BARS)
 - b. NOT EXPOSED TO EARTH OR WEATHER: 1 1/2 INCHES
- G. JOINT REINFORCEMENT:
- 1. JOINT REINFORCEMENT SHALL BE LADDER TYPE, 9 GAUGE, SPACED VERTICALLY AT EVERY 2 COURSES UNLESS NOTED OTHERWISE.
 - 2. PROVIDE JOINT REINFORCEMENT SPACED VERTICALLY AT EVERY COURSE FOR MASONRY BELOW GRADE AND IN PARAPETS AND CANTILEVERED WALLS.
 - 3. PROVIDE TWO ROWS OF JOINT REINFORCEMENT AT EVERY COURSE AT TOP AND BOTTOM OF OPENINGS (EXTEND 24 INCHES EACH SIDE).
 - 4. PROVIDE TWO ROWS OF JOINT REINFORCEMENT AT EVERY COURSE AT BOND BEAMS.
 - 5. OVERLAP DISCONTINUOUS JOINT REINFORCEMENT BY AT LEAST 6 INCHES.
 - 6. USE PREFABRICATED CORNERS AND TEES.
 - 7. EXTEND JOINT REINFORCEMENT A MINIMUM OF 4 INCHES INTO THE TIE BEAM.
 - 8. REFER TO PLANS AND DETAILS FOR BONDED JOINT REQUIREMENTS AT WALL CORNERS AND INTERSECTIONS, WHERE INDICATED ON DRAWINGS, INTERLOCK WALLS WITH METAL TIES, ANCHORS, OR PREFABRICATED JOINT REINFORCEMENT UNLESS NOTED OTHERWISE ON DRAWINGS OR SEE SPECIFICATIONS.
 - 9. LONGITUDINAL WIRES OF JOINT REINFORCEMENT SHALL BE FULLY EMBEDDED IN MORTAR OR GROUT WITH A MINIMUM HORIZONTAL EDGE COVER OF 5/8 INCHES WHEN EXPOSED TO EARTH AND WEATHER AND 1/4 INCHES WHEN NOT EXPOSED TO EARTH OR WEATHER.
- H. REINFORCED MASONRY WALL CONSTRUCTION SHALL BE INSPECTED BY AN ENGINEER OR ARCHITECT IN ACCORDANCE WITH TMS 602.
- I. WHERE ANCHOR BOLTS, WEDGE ANCHORS, OR ANCHORS SET IN EPOXY ARE PLACED IN A MASONRY WALL, FILL CELLS WITH GROUT FOR BOLTED COURSE, ONE COURSE ABOVE AND TWO COURSES BELOW.
- J. USE PRESSURE-TREATED WOOD FOR WOOD IN CONTACT WITH MASONRY.
- K. CALCIUM CHLORIDE SHALL NOT BE USED IN MORTAR OR GROUT.
- L. REFER TO ARCHITECT'S DRAWINGS FOR THE EXTENT OF MASONRY WALLS AND DIMENSIONED LOCATION OF OPENINGS. NON-LOAD BEARING WALLS MAY NOT BE SHOWN ON THE STRUCTURAL DRAWINGS.
- M. CONCRETE MASONRY UNITS SHALL BE CUT BELOW BEAMS, LINTELS, OR BOND BEAMS AS REQUIRED IN ORDER TO SET CONTINUOUS BEAM, Lintel, OR BOND BEAMS AT THE PROPER ELEVATION.
- N. ALL CELLS BELOW GRADE AND SLAB-ON-GROUND SHALL BE FULLY GROUTED.
- O. THE FOLLOWING CRITERIA REGARDING PIPES AND CONDUITS EMBEDDED IN MASONRY SHALL BE ADHERED TO (SEE MEP DRAWINGS FOR LOCATIONS OF SLEEVES, PIPES, CONDUIT, ACCESSORIES, ETC). THESE CRITERIA WILL BE STRICTLY ENFORCED:
- 1. CONDUITS, PIPES, AND SLEEVES OF ANY MATERIAL NOT HARMFUL TO MASONRY AND MEETING THE CRITERIA BELOW SHALL BE PERMITTED TO BE EMBEDDED IN MASONRY. ALL OTHER CONDUITS, PIPES, AND SLEEVES SHALL NOT BE EMBEDDED WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD.
 - 2. CONDUITS AND PIPES OF ALUMINUM SHALL NOT BE EMBEDDED IN STRUCTURAL MASONRY.
 - 3. CONDUITS, PIPES, AND SLEEVES PASSING THROUGH A WALL SHALL NOT SIGNIFICANTLY IMPAIR THE STRENGTH OF THE CONSTRUCTION. CONDUITS, PIPES, AND SLEEVES SHALL NOT PASS THROUGH JAMBS, LINTELS, BOND BEAMS, OR SHEAR WALLS WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD.
 - 4. CONDUITS AND PIPES SHALL NOT BE SPACED CLOSER THAN 3 DIAMETERS OR WIDTHS ON CENTER.
 - 5. CONDUITS AND PIPES SHALL BE FABRICATED AND INSTALLED SO THAT CUTTING, BENDING, OR DISPLACEMENT OF REINFORCEMENT FROM ITS PROPER LOCATION WILL NOT BE REQUIRED.
 - 6. CONDUITS AND PIPES, WITH FITTINGS, EMBEDDED WITHIN A COLUMN OR WALL SHALL NOT DISPLACE MORE THAN 2 PERCENT OF THE NET SECTION OR AS REQUIRED BY FIRE PROTECTION.
- P. ALL MASONRY WALLS SHOWN ON THE STRUCTURAL DRAWINGS HAVE BEEN DESIGNED TO RESIST THE REQUIRED CODE VERTICAL AND LATERAL FORCES IN THE FINAL CONSTRUCTED CONFIGURATION ONLY ASSUMING FULL BRACING TOP, BOTTOM AND/OR SIDE OF WALL AS SHOWN. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROPERLY AND ADEQUATELY BRACE ALL MASONRY WALLS AT ALL STAGES DURING CONSTRUCTION TO RESIST ERECTION LOADS AND LATERAL LOADS THAT COULD OCCUR PRIOR TO THE COMPLETION OF CONSTRUCTION.
- Q. CONTROL JOINTS SHALL BE PROVIDED IN ALL CONCRETE MASONRY CONSTRUCTION. REFER TO TYPICAL CONTROL JOINT DETAIL FOR GUIDELINES AND SPACING.

CMU - REINFORCING SPLICES¹						
BAR WITH SPLICE	VERTICAL BARS¹		HORIZONTAL BARS¹			
						
	1 VERT.	2 VERT.	1 VERT. 1 HORIZ.	2 VERT. 1 HORIZ.	1 VERT. 2 HORIZ.	2 VERT. 2 HORIZ.
#4	21"	26"	21"	21"	26"	45"
#5	26"	40"	29"	26"	40"	70"
#6	43"	74"	57"	43"	74"	131"
#7	60"	107"	81"	60"	107"	

- NOTES:**
- SEE TYPICAL REBAR LAYOUT DETAIL FOR BAR PLACEMENT. WHERE BARS OF DIFFERENT SIZES ARE TO BE SPLICED, THE SPLICE LENGTH SHALL BE THAT REQUIRED FOR THE LARGER BAR.
 - SPLICES OF VERTICAL REINFORCEMENT SHALL BE PLACED NEXT TO THE MAIN BAR AS INDICATED IN THE ILLUSTRATION.
 - SPLICES OF HORIZONTAL REINFORCEMENT SHALL BE PLACED VERTICALLY OVER THE MAIN BAR.
 - SPLICES OF HORIZONTAL REINFORCEMENT IN WALLS CONTAINING TOW BARS PER COURSE SHALL BE STAGGERED.

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DESIGN
DEVELOPMENT

Project No.: 2021009
Date: 08/22/2025

MASONRY
GENERAL
NOTES



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JG Project #: 21.18.004

TO THE BEST OF THE ENGINEER'S
KNOWLEDGE, THE PLANS AND
SPECIFICATIONS COMPLY WITH THE
APPLICABLE BUILDING CODES AND
MATERIAL SPECIFICATIONS.

8/25/2025 2:19:13 AM

STRUCTURAL STEEL

- A. STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS UNLESS NOTED OTHERWISE ON THE CONTRACT DOCUMENTS:
- ROLLED SHAPES AND CHANNELS: ASTM A572 OR A992, MIN. YIELD STRENGTH 50 KSI
 - ANGLES FOR TRUSSES AND BRACES: ASTM A36 MIN YIELD STRENGTH 36 KSI
 - MISCELLANEOUS ANGLES: ASTM A36
 - HOLLOW STRUCTURAL SECTIONS: ASTM A500 GRADE C, MIN YIELD STRENGTH 46 KSI FOR ROUND AND 50 KSI FOR RECTANGULAR HSS
- B. CONNECTION MATERIAL SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS OR AS NEEDED FOR CONNECTION DESIGN:
- ANGLES: ASTM A36
 - WELDS: ASTM A992
 - PLATES: ASTM A36
 - BOLTS: ASTM A325
 - NUTS: ASTM A563
 - WASHERS: ASTM A438
 - ANCHOR RODS: ASTM F1554 GRADE 36 WITH WELDABILITY SUPPLEMENT S1
 - WELD ELECTRODES: MATCH FILLER METAL TO BASE METAL PER AWS D1.1
- C. WHERE NO CAMBER IS INDICATED, FABRICATE BEAMS SO THAT ANY UNID. CAMBER IS UPWARD AFTER ERECTION.
- D. CANTILEVERED BEAMS WITH NATURAL MILL CAMBER SHALL BE ERECTED SUCH THAT THE CAMBER IS ORIENTED DOWNWARD (OR CONCAVE UP).
- E. SPLICES SHALL BE ALLOWED ONLY AT LOCATIONS SPECIFICALLY INDICATED ON THE STRUCTURAL DRAWINGS UNLESS APPROVED OTHERWISE BY THE SER IN WRITING.
- F. FOR STEEL MEMBERS AND EMBEDMENTS EXPOSED TO WEATHER, PROVIDE HOT-DIPPED GALVANIZED FINISH.
- G. PROVIDE HOLES IN ALL STEEL AS REQUIRED TO PREVENT ANY ACCUMULATION OF WATER. ALL PENETRATIONS THROUGH MAIN MEMBERS SHALL NOT EXCEED 1/10 DIA. AND SHALL BE GROUND SMOOTH. THESE DRINGS MUST BE KEPT CLEAN AND OPEN.
- H. SHOW ALL COPIES, HOLES, OPENINGS AND MODIFICATIONS REQUIRED IN STRUCTURAL STEEL MEMBERS FOR ERECTION OR THE WORK OF OTHER TRADES ON THE SHOP DRAWINGS FOR APPROVAL BY THE ARCHITECT AND STRUCTURAL ENGINEER.
- I. FIELD MODIFICATIONS OF STRUCTURAL STEEL IS PROHIBITED WITHOUT PRIOR APPROVAL OF THE ARCHITECT AND STRUCTURAL ENGINEER.
- J. WHERE BEAM SHEAR IS NOT NOTED, DESIGN FOR 30K.
- K. ALL CONNECTIONS SHALL BE DESIGNED FOR THE SPECIFIED SHEAR, MOMENT, AND AXIAL LOADS ON THE DRAWINGS. THE CONNECTIONS SHALL BE DESIGNED FOR LOAD REVERSAL. ALL CONNECTIONS SPECIFIED ON PLAN ARE ULTIMATE LEVEL FORCES UNLESS NOTED OTHERWISE.
- L. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN, DETAILING, AND FABRICATION OF ALL STEEL FRAMING CONNECTIONS UNLESS SPECIFICALLY NOTED AS COMPLETELY DESIGNED BY THE ARCHITECT ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL RETAIN A STRUCTURAL ENGINEER LICENSED TO PERFORM THE WORK IN THE JURISDICTION WHERE THE PROJECT IS LOCATED, WHO SHALL DESIGN THE CONNECTIONS. SUBMIT SIGN AND SEALED CALCULATIONS TO THE ARCHITECT FOR REVIEW AND APPROVAL PRIOR TO STARTING FABRICATION.

CONNECTION DESIGN SHALL MEET THE REQUIREMENTS OF THE AISI SPECIFICATIONS AND THE BUILDING CODE. CONNECTIONS SHALL BE CAPABLE OF RESISTING VERTICAL AND HORIZONTAL LOADS LISTED ON THE DRAWINGS. CONNECTION DESIGN SHALL PROVIDE AN ADEQUATE LOAD PATH TO TRANSFER THE LOADS FROM EACH MEMBER, THROUGH THE CONNECTION, INTO THE SUPPORTING MEMBER, AND SHALL CONSIDER THE EFFECTS OF THE FORCES ON EACH MEMBER. PROVIDE STIFFENER PLATES, WELD COUPLER PLATE PLATES, ETC. AS REQUIRED. MEMBERS SHOWN ON THE DRAWINGS HAVE NOT BEEN SIZED FOR LOCAL EFFECTS AT CONNECTIONS.

STEEL CONNECTION DETAILS SHOW GENERAL CRITERIA FOR DESIGN AND DETAILING, AND ARE NOT INTENDED TO SHOW COMPLETE CONNECTION CONFIGURATIONS OR OTHER SPECIFIC INFORMATION THAT ARE THE RESPONSIBILITY OF THE CONNECTION DESIGN ENGINEER. ALTERNATIVE CONNECTION CONFIGURATION MAY BE SUBMITTED TO THE ARCHITECT FOR REVIEW AND APPROVAL. CONNECTIONS SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS ARE TO BE FABRICATED AS SHOWN.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ERECTION ADS THAT INCLUDE, BUT ARE NOT LIMITED TO ERECTION ANGLES, LIFT HOLES, AND OTHER AIDS.

STEEL BEAMS ARE EQUALLY SPACED BETWEEN DIMENSION POINTS AT THE MAXIMUM DECK SPAN LOCATION UNLESS NOTED OTHERWISE. MINIMUM CONNECTIONS SHALL BE A TWO-BOLT CONNECTION USING 3/4 INCH-DIAMETER A325 BOLTS IN SINGLE SHEAR UNLESS NOTED OTHERWISE. ALL HIGH-STRENGTH BOLTS SHALL BE INSTALLED, TIGHTENED, AND INSPECTED IN ACCORDANCE WITH THE RISC. BOLTS IN CONNECTIONS SHALL BE INSTALLED WITH FULL PRETENSION EXCEPT WHERE "SNUG-TIGHT" INSTALLATION IS SPECIFICALLY PERMITTED ON THE DRAWINGS. WHERE CONNECTIONS ARE NOTED AS SNUG-TIGHT, THE CONTRACTOR MAY INSTALL PER THE CRITERIA FOR SNUG-TIGHT BOLTS. BOLTS IN SLIP-CRITICAL CONNECTIONS SHALL BE INSTALLED USING TURN-OF-NUT PRETENSIONING, TWIST-OFF TYPE TENSION CONTROL, BOLT PRETENSIONING, OR DIRECT TENSION INDICATOR (DTI) PRETENSIONING. ALL BOLT HOLES SHALL BE STANDARD SIZE UNLESS NOTED OTHERWISE.

WELDING

- A. ALL WELDING SHALL BE PERFORMED IN STRICT ADHERENCE TO A WRITTEN WELDING PROCEDURE SPECIFICATION PER AMERICAN WELDING SOCIETY D1.1. ALL WELDING PARAMETERS SHALL FOLLOW THE ELECTRODE MANUFACTURER'S RECOMMENDATIONS. WELDING PROCEDURES SHALL BE SUBMITTED TO THE OWNER'S TESTING AGENCY FOR REVIEW BEFORE STARTING FABRICATION OR ERECTION. COPIES OF THE WELDING PROCEDURE SPECIFICATION SHALL BE ON SITE AND AVAILABLE TO ALL WORKERS AND THE SPECIAL INSPECTOR.
- B. ALL WELDS SHALL BE MADE USING LOW HYDROGEN ELECTRODES WITH MINIMUM TENSILE STRENGTH PER AWS D1.1 (MINIMUM 70 KSI).
- C. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE JOINT PREPARATIONS AND WELDING PROCEDURES THAT INCLUDE, BUT ARE NOT LIMITED TO: REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, COPIES, SURFACE ROUGHNESS VALUES, TAPERS, AND TRANSITIONS OF UNEQUAL PARTS.
- D. WELDING SHALL BE DONE BY WELDERS WITH CURRENT AMERICAN WELDING SOCIETY CERTIFICATION.
- E. FIELD WELDING SYMBOLS HAVE NOT NECESSARILY BEEN INDICATED ON THE DRAWINGS. WHERE SHOWN, PROPER FIELD WELDING PER AMERICAN WELDING SOCIETY D1.1 SHALL BE USED. WHERE NO FIELD WELDING SYMBOLS ARE SHOWN, IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE THE USE OF SHOP AND FIELD WELDS.
- F. ALL WELD SIZES SHALL BE THE LARGER OF: THE SIZE REQUIRED BY THE CONNECTION FORCES, MINIMUM SIZE PER ANSIAWS D1.1 OR 3/16 INCH MINIMUM FILLET WELD, UNLESS NOTED OTHERWISE.
- G. PROVIDE FILLET WELDS AT CONTACT POINTS BETWEEN STEEL MEMBERS SUFFICIENT TO DEVELOP THE ALLOWABLE TENSILE FORCE OF THE SMALLER MEMBER AT THE JOINT, UNLESS NOTED OTHERWISE.
- H. ALL FILLET WELDS SHALL BE VISUALLY INSPECTED BY THE TESTING FIRM.
- I. GROOVE WELDS SHALL BE FULL PENETRATION, UNLESS NOTED OTHERWISE.
- J. ALL COMPLETE JOINT PENETRATION WELDS SHALL BE ULTRASONICALLY TESTED UPON COMPLETION OF THE CONNECTION, EXCEPT PLATES LESS THAN OR EQUAL TO 1/4-INCH-THICK SHALL BE MAGNETIC PARTICLE TESTED. REDUCTION IN TESTING MAY BE MADE IN ACCORDANCE WITH THE BUILDING CODE WITH APPROVAL OF THE ENGINEER.
- K. A RUN-OFF TAB SHALL BE USED AT ALL BEVEL AND FULL PENETRATION WELDS. RUN-OFF TABS SHALL BE REMOVED BY HEAT CUTS AFTER WELD IS COMPLETED. GRIND SMOOTH WHERE REQUIRED BY DETAIL.
- L. WHERE REQUIRED BY DETAIL, REMOVE WELD BACKING BARS AND GRIND SMOOTH AFTER WELD IS COMPLETED.
- M. WHERE NECESSARY, REMOVE GALVANIZING OR PRIMER PRIOR TO WELDING.
- N. STEEL USING COMPLETE JOINT PENETRATION GROOVE WELDS THAT FUSE THROUGH THE THICKNESS OF THE FLANGE OR WEB SHALL HAVE A MINIMUM CHARTER V-NOTCH IMPACT TESTING VALUE AS FOLLOWS:
- ASTM A568M HOT-ROLLED SHAPES WITH A FLANGE THICKNESS EXCEEDING 2 INCHES AND BUILT-UP HEAVY SHAPES WITH PLATES EXCEEDING 2 INCHES IN THICKNESS: 20 FT-LB AT 70° FAHRENHEIT.
 - REGARDLESS OF THICKNESS, ALL TRUSSES, LATERAL SYSTEM MEMBERS (INCLUDING COLUMNS, WIND GIRDERS, BRACES, ETC.) 20 FT-LB AT 70° FAHRENHEIT.
 - STEEL EXPOSED TO TEMPERATURES IN SERVICE BELOW 50° FAHRENHEIT: 20 FT-LB AT SERVICE TEMPERATURE + 20° FAHRENHEIT; 40° FAHRENHEIT MAXIMUM.
 - WELD METAL: 20 FT-LB AT -20° FAHRENHEIT AND 40 FT-LB AT 70° FAHRENHEIT.
 - TESTING IS TO BE IN ACCORDANCE WITH ASTM A568M, SUPPLEMENTARY REQUIREMENT S30, CHARTER V; NOTCH IMPACT TEST FOR STRUCTURAL SHAPES - ALTERNATE CORE LOCATION, AT ROLLED SHAPES AND ASTM A673 FOR PLATES. AT ANY PERMITTED LOCATIONS. WELD METAL SHALL BE TESTED IN ACCORDANCE WITH ASTM E23, STANDARD METHODS FOR NOTCHED BAR IMPACT TESTING OF METALLIC MATERIALS FOR WELD METAL.

STEEL JOISTS

- A. MANUFACTURE AND ERECT ALL STRUCTURAL STEEL, JOISTS, JOIST GIRDERS, AND BRIDGING IN ACCORDANCE WITH SPECIFICATION SECTION 052100, SPECIFICATIONS OF THE STEEL JOIST INSTITUTE AND ALL OSHA REQUIREMENTS.
- B. JOIST MANUFACTURER SHALL DESIGN JOISTS PER LISTED DESIGN CRITERIA AND ANY ADDITIONAL LOADING SHOWN ON PLAN OR IN DETAILS. AT A MINIMUM, JOIST SHALL BE DESIGNED PER STEEL JOIST INSTITUTE (SJI) LOAD TABLES.
- C. JOISTS SHALL BE EQUALLY SPACED BETWEEN COLUMN LINES OR OTHER SPECIFICALLY LOCATED FRAMING MEMBERS UNLESS NOTED OTHERWISE.
- D. STEEL JOISTS, JOIST GIRDERS, BRIDGING, AND THEIR CONNECTIONS SHALL BE DESIGNED FOR NET UPLIFT (NEGATIVE PRESSURE) AS INDICATED IN THE STRUCTURAL DRAWINGS. REFER TO THE APPLICABLE BUILDING CODE LISTED IN DESIGN CRITERIA FOR LOAD COMBINATIONS.
- DEAD LOAD (MIN) = 8 PSF (FOR UPLIFT)
- E. STEEL JOISTS, JOIST GIRDERS, BRIDGING, AND THEIR CONNECTIONS SHALL BE DESIGNED FOR ADDITIONAL DOWN FORCE RESULTING FROM WIND (POSITIVE PRESSURE) AS INDICATED IN THE STRUCTURAL DRAWINGS.
- DEAD LOAD (MAXIMUM) = 20 PSF (SIMULTANEOUS WITH DOWNWARD WIND)
- F. JOIST MANUFACTURER SHALL DESIGN AND DETAIL ALL BRIDGING PER SJI REQUIREMENTS AND CLEARLY INDICATE LOCATION ON STEEL JOIST ERECTION DRAWINGS.
- BRIDGING SHALL BE DESIGNED TO FULLY BRACE TOP CHORD OF JOISTS UNDER SERVICE LOADS FOR JOISTS NOT BRACED BY STEEL ROOF DECK.
 - BOTTOM CHORD OF ROOF JOISTS SHALL BE DESIGNED FOR NET UPLIFT (COMPONENTS & CLADDING) SHOWN ON DIAGRAM ON S-002C, UNLESS NOTED OTHERWISE.
- G. AN ALLOWABLE STRESS INCREASE FOR LOAD COMBINATIONS INCLUDING WIND IS NOT PERMITTED.
- H. JOIST MANUFACTURER SHALL CAMBER JOISTS PER SJI CRITERIA.
- I. PROVIDE MINIMUM BEARING PER SJI REQUIREMENTS AND CONNECT TO STEEL SUPPORT AS FOLLOWS:
- K-SERIES: 3/16" x 1" FILLET WELD EACH SIDE.
 - LH AND DLH SERIES: 1/2" x 2" FILLET WELD EACH SIDE.
- J. PROVIDE STANDARD DEPTH OF BEARING FOR ALL JOISTS AS SHOWN BELOW UNLESS NOTED IN DRAWINGS:
- K-SERIES - 2 1/4"
 - LH - 5"
 - WHERE STEEL JOIST OR JOIST GIRDER SLOPE EXCEEDS 1/4 INCH PER FOOT, PROVIDE SLOPED BEARING AS NOTED IN SLOPED SEAT REQUIREMENTS OF SJI.
- K. JOIST MANUFACTURER SHALL DESIGN AND DETAIL FIELD BOLTING FOR ERECTION PER SJI REQUIREMENTS.

STEEL DECK GENERAL REQUIREMENTS

- A. ALL STEEL DECK SHALL BE MANUFACTURED AND INSTALLED IN ACCORDANCE WITH THE DIVISION 05 SPECIFICATIONS.
- B. STEEL DECK SHALL BE SUPPORTED BY A MINIMUM OF FOUR SUPPORT LOCATIONS (THREE SPAN CONDITION), UNLESS NOTED OTHERWISE.
- C. THE CONTRACTOR SHALL COORDINATE SLAB/DECK OPENING SIZES AND LOCATIONS PER ARCHITECTURAL AND MEP CONTRACT DOCUMENTS. THE CONTRACTOR SHALL PROVIDE OPENING SUPPORT FRAMING AND/OR REINFORCEMENT AS REQUIRED PER TYPICAL DETAILS AND SUBMIT PROPOSED SLAB/DECK OPENINGS FOR REVIEW BY THE STRUCTURAL ENGINEER OF RECORD.
- D. SHOP DRAWINGS SHALL BE SUBMITTED INDICATING:
- MATERIAL STRENGTH
 - SECTION PROPERTIES
 - DECK GAGE LAYOUT
 - FASTENER TYPE
 - CONNECTION PATTERN
 - CLOSURE ANGLES
- E. THE CAPACITY OF THE DECK SHALL BE BASED ON CURRENT ICC-ES EVALUATION REPORTS.

STEEL ROOF DECK

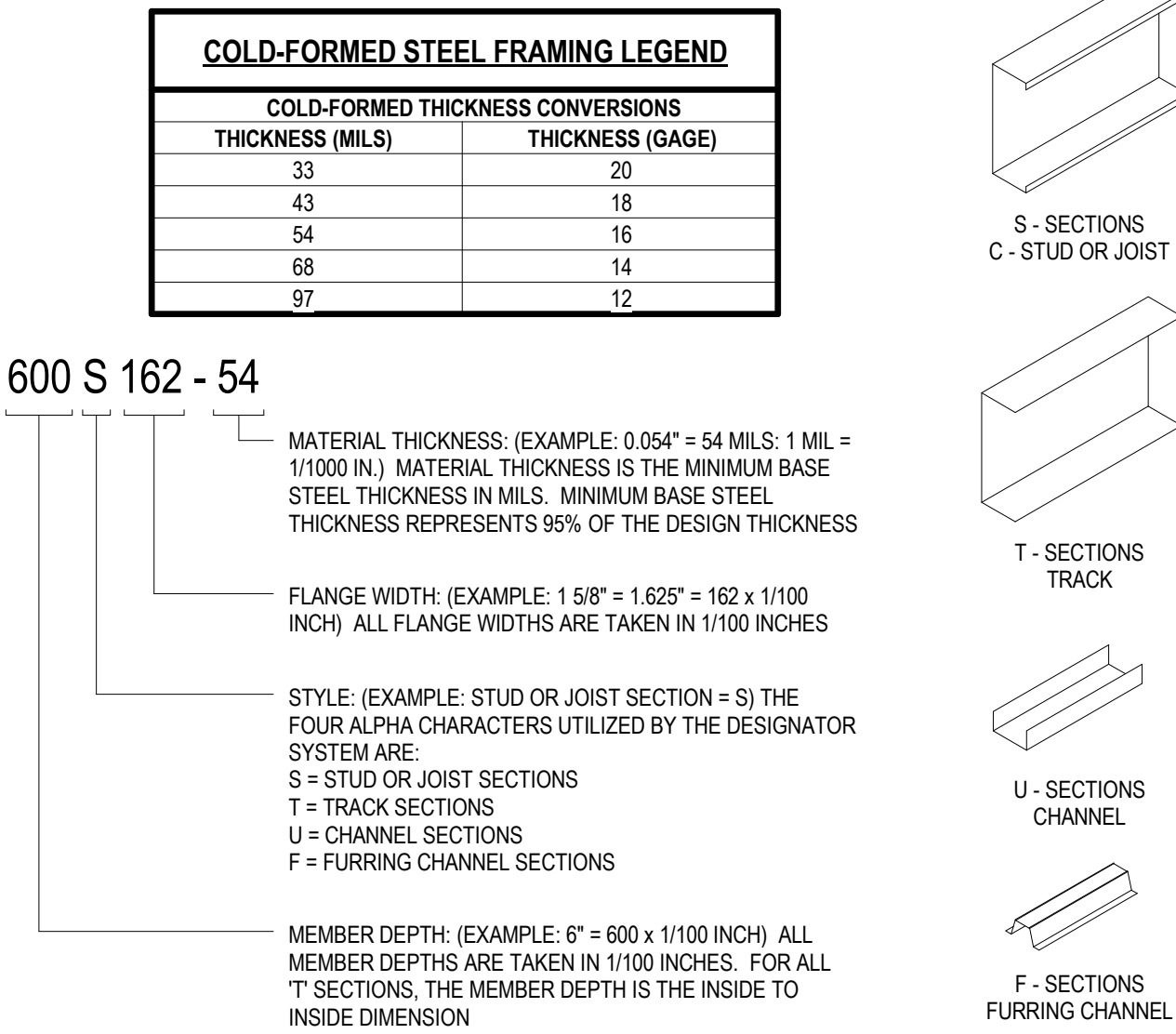
- A. STEEL ROOF DECK SHALL BE A MINIMUM YIELD STRENGTH OF 33 KSI, UNLESS NOTED OTHERWISE. ALL INTERIOR STEEL ROOF DECK SHALL CONFORM TO ASTM A1008, FACTORY PRIMED FOR PAINT. ALL EXPOSED STEEL ROOF DECK SHALL CONFORM TO ASTM A653 WITH G90 HOT-DIPPED GALVANIZATION, UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR EXTENTS.
- B. STEEL ROOF DECK SHALL BE ATTACHED TO STEEL SUPPORTS WITH 5/8 INCH DIAMETER PUDDLE WELDS AND TO COLD-FORMED METAL FRAMING WITH #12 SELF-DRILLING SCREWS UNLESS NOTED OTHERWISE. WHEN DECK THICKNESS IS LESS THAN 0.028 INCHES, WELDS MUST BE MADE THROUGH MINIMUM 16 GAGE WELDING WASHERS. SPACING OF WELDS SHALL BE AS SPECIFIED IN THE DECK ATTACHMENT SCHEDULE.
- C. WHERE STEEL MEMBERS ARE PARALLEL TO THE DECK FLUTES AND AT THE SAME ELEVATION OF THE BOTTOM OF THE DECK, ADJUST DECK LAYOUT AND WELD DECK TO STEEL WITH SAME WELDING AS REQUIRED FOR SIDE BOUNDARIES.
- D. ERECT STEEL DECK CLOSURES AND OTHER LIGHT GAGE MATERIAL REQUIRED TO PRODUCE A COMPLETED INSTALLATION.
- E. FLAT, RIDGE, AND VALLEY PLATES.
- F. UNLESS NOTED OTHERWISE, CONTRACTOR SHALL PROVIDE FLAT PLATES (20 GAGE MINIMUM) AT ALL LOCATIONS WHERE ROOF DECK CHANGES DIRECTION AND RIDGE OR VALLEY PLATES (20 GAGE MINIMUM) AT ALL LOCATIONS WHERE ROOF SLOPE EXCEEDS 1/4 INCH PER FOOT.
- G. DO NOT HANG CEILING, DUCTS, LIGHT FIXTURES, EQUIPMENT, OR OTHER ITEMS FROM THE ROOF DECK WITHOUT PRIOR APPROVAL FROM THE DECK SUPPLIER AND REVIEW BY THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD.
- H. SEE ARCHITECTURAL DRAWINGS FOR FINAL ROOF SLOPES. WHERE STRUCTURAL FRAMING DOES NOT CREATE THE SPECIFIED ROOF SLOPE, IT SHALL BE CREATED WITH RIGID INSULATION ABOVE THE DECK.

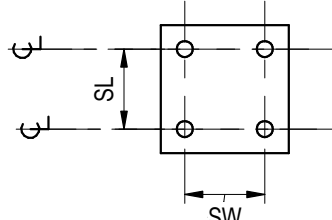
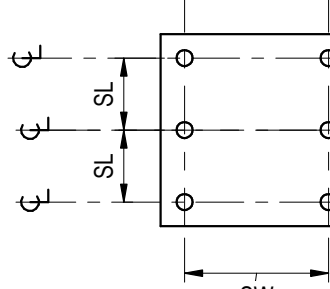
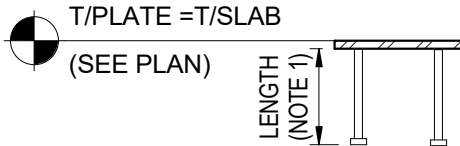
NON-COMPOSITE STEEL FORM DECK

- A. NON-COMPOSITE STEEL FORM DECK SHALL BE A MINIMUM YIELD STRENGTH OF 60 KSI AND SHALL CONFORM TO ASTM A653 WITH G90 HOT-DIPPED GALVANIZATION, UNLESS NOTED OTHERWISE.
- B. NON-COMPOSITE STEEL FORM DECK SHALL BE ATTACHED TO SUPPORTS WITH 5/8 INCH DIAMETER PUDDLE WELDS. WHEN DECK THICKNESS IS LESS THAN 0.028 INCHES, WELDS MUST BE MADE THROUGH MINIMUM 16 GAGE WELDING WASHERS. SPACING OF WELDS SHALL BE AS FOLLOWS:
- AT BUTTED ENDS AT 10 INCHES ON CENTER
 - AT PERIMETER EDGES OF BUILDING: AT 10 INCHES ON CENTER
 - INTERMEDIATE SUPPORTS: AT 10 INCHES ON CENTER
 - SIDE LAPS: FOR FORM DECK WITH SPANS 3-FEET OR GREATER, PROVIDE TWO CONNECTIONS PER SPAN. HEX HEAD SCREWS, SIZE #10, OR CRIMPING (BUTTON PUNCHING) MAY BE USED AT SIDE LAP CONNECTIONS.

COLD-FORMED STEEL

- A. DESIGN, FABRICATION, AND ERECTION OF COLD-FORMED STEEL SHALL CONFORM TO AISI S100.
- B. ALL STUDS, JOISTS, TRACK, BRIDGING, END CLOSURES, AND ACCESSORIES SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE REQUIREMENTS OF AISI S100.
- C. THE CONTRACTOR'S DELEGATED ENGINEER SHALL DESIGN ALL COLD-FORMED STEEL AND ITS CONNECTIONS TO THE BUILDING STRUCTURE. REFER TO ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR REQUIRED COLD-FORMED STEEL.
- D. ALL EXTERIOR COLD-FORMED STEEL AND ITS CONNECTIONS TO THE BUILDING STRUCTURE SHALL BE DESIGNED PER DESIGN CRITERIA AND COMPONENTS AND CLADDING WIND PRESSURES LISTED IN THE STRUCTURAL DOCUMENTS.
- E. ALL INTERIOR COLD-FORMED STEEL AND ITS CONNECTIONS TO THE BUILDING STRUCTURE SHALL BE DESIGNED PER DESIGN CRITERIA LISTED IN THE GENERAL STRUCTURAL NOTES AND A MINIMUM OF 3 PSF INTERNAL PRESSURE NORMAL TO THE STRONG AXIS OF FRAMING MEMBERS IN ADDITION TO DEAD LOAD.
- F. ALL EXTERIOR COLD-FORMED STEEL SHALL HAVE A MINIMUM G90 GALVANIZED COATING. ALL INTERIOR COLD-FORMED STEEL SHALL HAVE A MINIMUM G60 GALVANIZED COATING.
- G. ALL STUDS SHALL BE DESIGNED TO A MINIMUM GAUGE OF 43 MILS. STUD SPACING SHALL NOT EXCEED 24" ON CENTER.
- H. ALL COLD-FORMED STEEL, 1/4 MIL AND THICKER SHALL HAVE A MINIMUM YIELD STRENGTH (F_y) OF 50 KSI.
- I. ALL WELDING SHALL MEET REQUIREMENTS OF AWS D1.3 AND THE AISI STANDARD.
- J. ALL SCREWS OR PINS SHALL BE NON-CORROSIVE NO. 8-18 (Ø = 0.125") OR LARGER, UNLESS NOTED OTHERWISE. DO NOT USE STAINLESS STEEL OR COPPER-COATED FASTENERS.
- K. TRACKS SHALL BE THE SAME DEPTH AS STUDS OR JOISTS AND OF EQUAL OR THICKER GAUGE THAN STUDS OR JOISTS, UNLESS NOTED OTHERWISE. TRACKS SHALL BE CONNECTED IN ORDER TO SUPPORT STUDS OR JOISTS AT 24" ON CENTER. MAXIMUM, STUDS AND JOISTS SHALL BE CONNECTED TO TRACKS AT EACH SIDE.
- L. INSTALLATION OF CURTAIN WALL FRAMING SHALL ACCOMMODATE VERTICAL DISPLACEMENT OF THE PRIMARY STRUCTURE.
- M. DESIGN OF SLIP TRACKS SHALL CONFORM TO GUIDELINES ESTABLISHED IN STEEL STUD MANUFACTURER'S ASSOCIATION TECHNICAL NOTE NO. 1, PUBLISHED JANUARY 2020.
- N. PROVIDE THE STANDARD TRACK, CLIP ANGLES, BRACING, REINFORCEMENTS, FASTENERS, AND ACCESSORIES AS RECOMMENDED BY THE MANUFACTURER FOR THE APPLICATION INDICATED AND AS NEEDED TO PROVIDE A COMPLETE FRAMING SYSTEM. INSTALL THE FRAMING SYSTEM IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS AND RECOMMENDATIONS, UNLESS NOTED OTHERWISE.
- O. MATCH FILLER METAL TO BASE METAL PER AWS D1.3 FOR WELDING STEEL STUDS. ALL WELDING SHALL BE PERFORMED IN ACCORDANCE WITH AWS PROCEDURES. CONSULT MANUFACTURER FOR EQUIPMENT RECOMMENDATIONS AND PROPER ELECTRODE SELECTION. TOUCH UP WELDED AREAS WITH A ZINC-RICH PAINT.
- P. STUD-TO-STUD CONNECTIONS SHALL BE A MINIMUM OF (4) #8 TEK SCREWS AT EACH CONNECTION, UNLESS NOTED OTHERWISE.
- Q. RESISTANCE TO MINOR AXIS BENDING AND ROTATION SHALL BE PROVIDED BY GYPSUM BOARD, GYPSUM SHEATHING, PLYWOOD, HORIZONTAL BRACING, OR CHANNEL SHAPED COLD-FORMED STEEL FRAMING BLOCKING.
- R. SHOP DRAWINGS, INCLUDING CALCULATIONS, SHALL BE SIGNED AND SEALED BY A DELEGATED ENGINEER AND SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD AND ARCHITECT FOR REVIEW.
- S. SHOP DRAWINGS SHALL CLEARLY INDICATE ALL FRAMING SIZES, CONNECTIONS, AND BRACING. IF FRAMING DEPTH IS NOT INDICATED IN THE CONTRACT DOCUMENTS, THE MOST ECONOMICAL MEMBER AND CONNECTION MEETING THE DESIGN CRITERIA SHALL BE PROVIDED.
- T. CALCULATIONS SHALL CLEARLY INDICATE DESIGN LOADING, FRAMING SIZE, SPACING, ASSUMPTIONS, AND FORCES IMPOSED ONTO BUILDING STRUCTURE FROM CONNECTIONS.
- U. STEEL STUD MANUFACTURERS ASSOCIATION FOUR PART NOMENCLATURE IDENTIFIES MEMBER DEPTH, TYPE, FLANGE WIDTH AND GAUGE.



STEEL EMBED PLATE SCHEDULE									
MARK	PLATE GEOMETRY (IN)			EDGE DISTANCE	ANCHORS (AWS D1.1, TYPE B)			COMMENTS	REFERENCE DETAIL
	WIDTH	LENGTH	THICKNESS		#	DIAMETER	LENGTH		
									
	LAYOUT WITH 4 ANCHORS				LAYOUT WITH 6 ANCHORS				
NOTES:									
1. ANCHOR LENGTH INDICATED IS FINAL LENGTH AFTER BURNOFF.									
2. SEE PLAN FOR LOCATION OF KEYED SECTIONS & DETAILS REFRERENCING EMBED PLATES AND ATTACHMENT OF CONNECTING ELEMENTS.									
3. FOR LAYOUTS WITH MORE THAN 4 ANCHORS SEE KEYED SECTIONS/DETAILS FOR ANCHOR LAYOUTS.									

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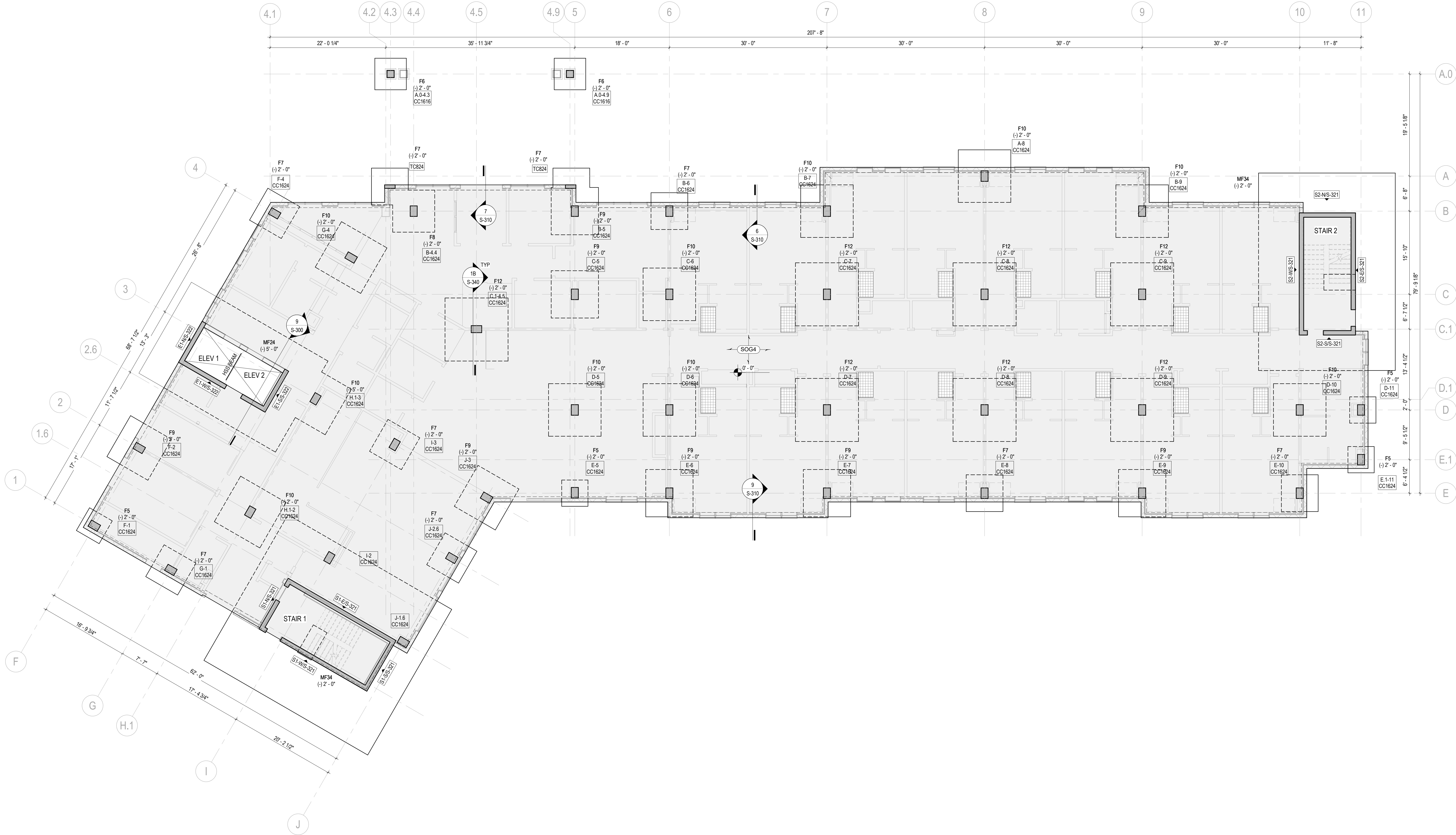
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1 S-121 FOUNDATION PLAN

1/8" = 1'-0"

FOUNDATION PLAN NOTES:

- X..X#** LOCATION MARK
(SEE SCHEDULE ON THIS SHEET AND NOTE 2)

CC##X# COLUMN MARK
(SEE SCHEDULE ON THIS SHEET)

F#, W#, OR P# FOUNDATION OR PIER MARK
(SEE SCHEDULE ON THIS SHEET)

(+) X' - X" TOP OF FOOTING/PIER ELEVATION
- [Symbol]** DENOTES CONCRETE COLUMN/PIER/WALL.
FOR COLUMN SCHEDULE AND DETAILS SEE **XXXX** SERIES.
FOR WALL DETAILS SEE **XXXX** SERIES.
- [Symbol]** DENOTES STEEL COLUMN. SEE **XXXX**.
- FOR STEEL COLUMN BASE PLATE INFORMATION, SEE **XXXX**.
- FOR TRENCHES ADJACENT TO FOUNDATIONS, SEE **XXXX**.
FOR PIPING PASSING UNDER WALL FOUNDATIONS, SEE **XXXX**.
FOR PIPING PASSING UNDER FOOTINGS ABOVE ARE PREPARED.
PIPING PASSING UNDER FOOTINGS SHALL BE INSPECTED BEFORE FOUNDATIONS ABOVE ARE PREPARED.
- GENERAL CONTRACTOR SHALL COORDINATE PLUMBING AND UTILITIES LOCATIONS WITH FOUNDATIONS AS NEEDED. ADDITIONALLY, GENERAL CONTRACTOR SHALL COORDINATE FOUNDATION ELEVATIONS WITH PLUMBING AND UTILITIES AS NEEDED. FORWARD ANY FOUNDATION LOCATION CHANGE REQUESTS TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW AND APPROVAL.
- [Symbol]** DENOTES STEP IN FOUNDATION. SEE **XXXX**.
- [Symbol]** DENOTES LOAD BEARING CONCRETE MASONRY UNIT (CMU) WALL WITH **XX** AT **XX** CC VERTICAL REINFORCEMENT, CENTERED IN FULLY-GROUTED CELL.
- [Symbol]** DENOTES NONLOAD BEARING CONCRETE MASONRY UNIT (CMU) WALL WITH **XX** AT **XX** CC VERTICAL REINFORCEMENT, CENTERED IN FULLY-GROUTED CELL.
- [Symbol]** DENOTES SHOWER DEPRESSION.

SLAB-ON-GROUND PLAN NOTES:

- REFERENCE BUILDING TOP-OF-SLAB ELEVATION = **(+) X' - X"** (**XX.XX NAVD**).
- [Symbol]** DENOTES SLAB-ON-GROUND MARK (SEE SCHEDULE ON THIS SHEET).
- SJ** DENOTES SLAB-ON-GROUND CONTROL JOINT. FOR CONTROL JOINT REQUIREMENTS, SEE **XXXX**.
- FOR RE-ENTRANT CORNER BARS, SEE **XXXX**.
- INSTALL THICKENED SLAB UNDER STAIR STRINGER. SEE **XXXX**.
- FOR THICKENED SLAB UNDER NON-LOAD BEARING CONCRETE MASONRY WALLS, SEE **XXXX**.
- GENERAL CONTRACTOR SHALL COORDINATE HOUSEKEEPING PAD LOCATIONS.
- [Symbol]** DENOTES STEP IN TOP OF SLAB. SEE **XXXX**.
- SEE CIVIL DRAWINGS FOR BASE AND SUBGRADE PREPARATION INFORMATION.
- SEE ARCHITECTURAL DRAWINGS FOR:
 - VAPOR BARRIER REQUIREMENTS AND LOCATIONS.
 - ALL SLOPED SLAB AREAS.
 - ALL DEPRESSED SLAB AND/OR RAISED SLAB AREAS.
 - MAINTAIN SLAB THICKNESS NOTED ON PLAN AS A MINIMUM IN ALL AREAS.
 - ALL DIMENSIONS NOT SHOWN. VERIFY ALL DIMENSIONS SHOWN IN STRUCTURAL DRAWINGS WITH ARCHITECTURAL DRAWINGS AND REPORT ANY DISCREPANCIES OR DIMENSIONS NOT SHOWN ON ARCHITECTURAL DRAWINGS FOR CLARIFICATION.
 - SLAB SLOPES, DRAINS, STEPS, PENETRATIONS, FINISHES, AND ANY OTHER ADDITIONAL INFORMATION.

SLAB-ON-GROUND SCHEDULE			
MARK	THICKNESS	REINFORCEMENT	REMARKS
SOG4	4"		

MAT FOUNDATION SCHEDULE			
MARK	THICKNESS	REINFORCEMENT	
		TOP	BOTTOM
MF24	24"		
MF34	34"		

ISOLATED FOOTING SCHEDULE					
MARK	GEOMETRY		REINFORCEMENT		
	WIDTH	LENGTH	THICKNESS	LONG BARS	SHORT BARS
F5	5'-0"	5'-0"	18"		
F6.0	6'-0"	6'-0"	24"		
F7	7'-0"	7'-0"	24"		
F8	8'-0"	8'-0"	24"		
F9	9'-0"	9'-0"	24"		
F10	10'-0"	10'-0"	24"		
F12	12'-0"	12'-0"	24"		

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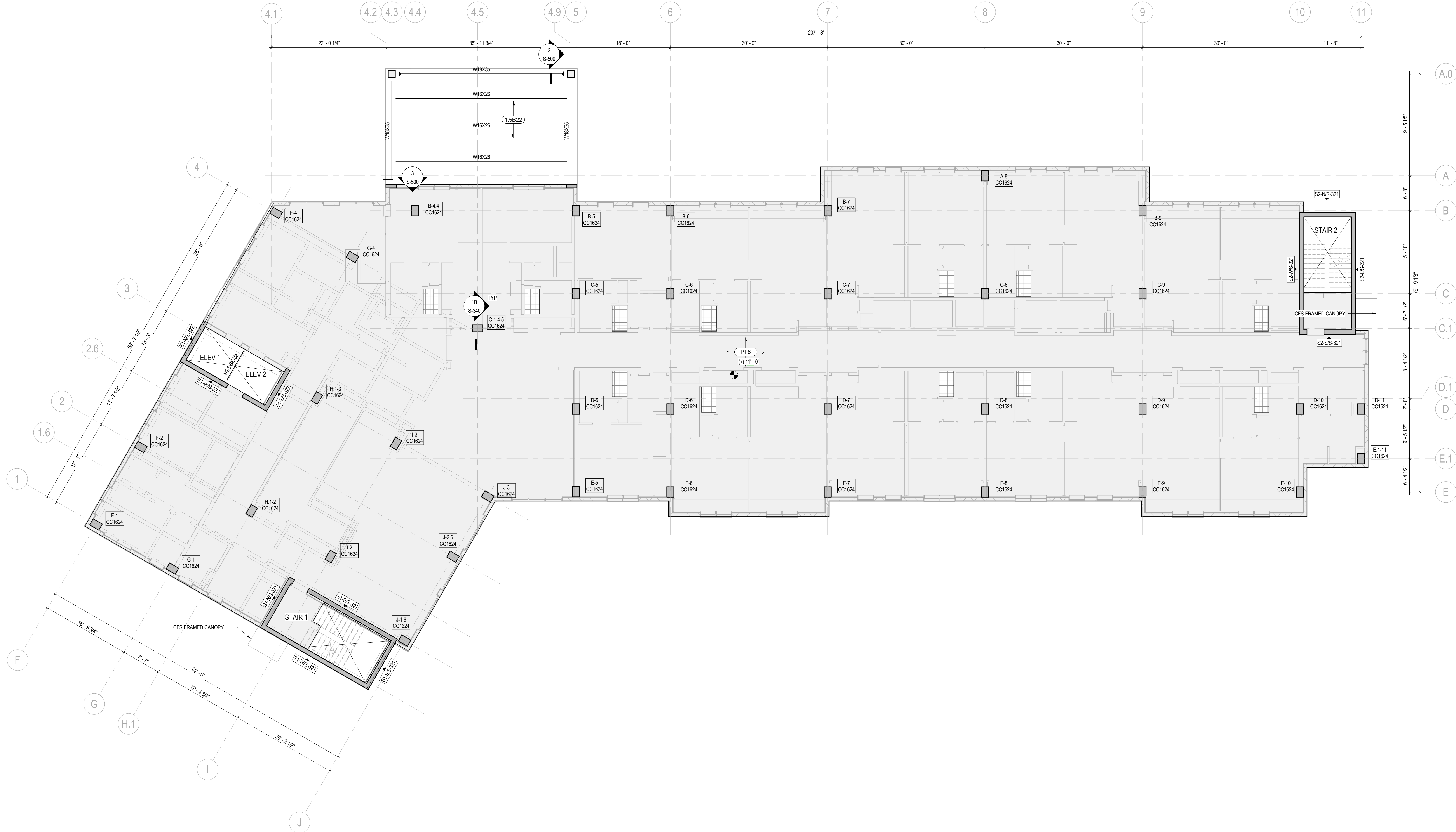
**DESIGN
DEVELOPMENT**

Project No.: 2021009
Date: 08/22/2025

**FOUNDATION
PLAN**

S-121

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1 SECOND FLOOR FRAMING PLAN
S-122 1/8" = 1'-0"

CONCRETE FRAMING PLAN NOTES:

- CONCRETE SLAB TAG:
ARROWS DENOTE SLAB SPAN DIRECTION
CS# DENOTES SLAB MARK
(SEE SCHEDULE ON THIS SHEET FOR SLAB THICKNESS AND REINFORCEMENT)
- SEE PLAN FOR TOP OF SLAB ELEVATIONS
- DENOTES CONCRETE COLUMN/PIER/WALL.
FOR COLUMN SCHEDULE AND DETAILS SEE **XXXX** SERIES.
FOR WALL DETAILS SEE **XXXX** SERIES.
- DENOTES LOAD BEARING CONCRETE MASONRY UNIT (CMU) WALL WITH **XX** AT **XX** OC VERTICAL REINFORCEMENT, CENTERED IN FULLY-GROUTED CELL.
- DENOTES NON-LOAD BEARING CONCRETE MASONRY UNIT (CMU) WALL WITH **XX** AT **XX** OC VERTICAL REINFORCEMENT, CENTERED IN FULLY-GROUTED CELL.
- DENOTES STEP IN TOP OF SLAB, SEE **XXXX**.
- SEE ARCHITECTURAL DRAWINGS FOR:
 - ALL SLOPED SLAB AREAS.
 - ALL DIMENSIONS NOT SHOWN. VERIFY ALL DIMENSIONS SHOWN IN THE STRUCTURAL DRAWINGS WITH ARCHITECTURAL DRAWINGS AND REPORT ANY DISCREPANCIES OR DIMENSIONS NOT SHOWN ON THE ARCHITECTURAL DRAWINGS FOR CLARIFICATION.
- SEE MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR ADDITIONAL FLOOR PENETRATIONS, SLEEVES, AND INSERTS REQUIRED TO BE CAST IN THE SLAB.
 - SLEEVES AND PENETRATIONS WITHIN 48 INCHES OF THE FACE OF ANY CONCRETE COLUMN (NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS) SHALL BE SUBMITTED FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD.
 - SLEEVES AND PENETRATIONS INTERRUPTING BANDED LINES OF TENDONS (NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS) SHALL BE SUBMITTED FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD.
 - SLEEVES AND PENETRATIONS GREATER THAN 12 INCHES IN LENGTH OR WIDTH (NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS) SHALL BE SUBMITTED FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD.
- FOR REINFORCEMENT PLAN AND ADDITIONAL NOTES, SEE SHEET **XXXX**.
- FOR POST-TENSIONED LAYOUT PLAN AND ADDITIONAL NOTES, SEE SHEET **XXXX**.
- DENOTES SHOWER DEPRESSIONS.

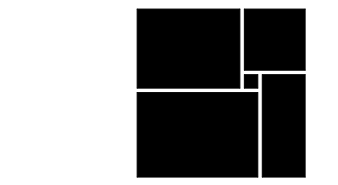


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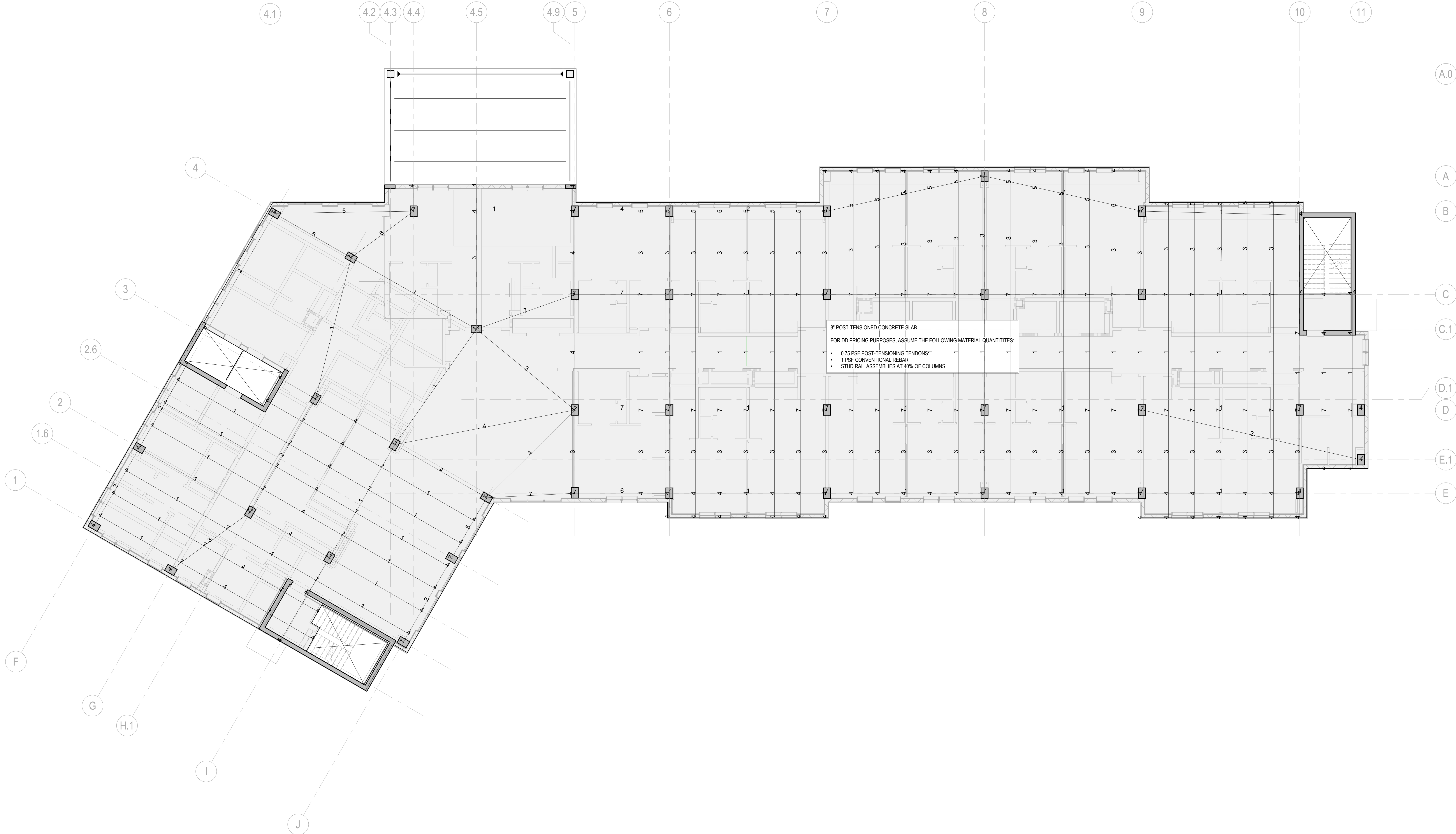
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DEVELOPMENT

Project No.: 2021009
Date: 08/22/2025

SECOND
FLOOR
FRAMING
PLAN

S-122

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8' POST-TENSIONED CONCRETE SLAB
FOR DD PRICING PURPOSES, ASSUME THE FOLLOWING MATERIAL QUANTITIES:
• 0.75 PSF POST-TENSIONING TENDONS*
• 1 PSF CONVENTIONAL REBAR
• STUD RAIL ASSEMBLIES AT 40% OF COLUMNS

1
S-122-PT

SECOND FLOOR PT PLAN

1/8" = 1'-0"

POST-TENSIONED CONCRETE SLAB PLAN NOTES:

- SEE FRAMING PLAN FOR SLAB GEOMETRY, REFERENCE ELEVATION, AND KEYED SECTIONS.
- SEE POST-TENSIONED FRAMING GENERAL NOTES ON SHEET **SXXXX** AND DIVISION 03 SPECIFICATIONS FOR GENERAL REQUIREMENTS.
- FOR TYPICAL POST-TENSIONED CONCRETE SECTIONS AND DETAILS, SEE **SXXXX**.
- TENDON LAYOUT NOTES:
 - LINEWORK SHOWN FOR TENDON PATHS IS SCHEMATIC TO ILLUSTRATE DESIGN INTENT, BUT NOT MEANT TO INDICATE EXACT TENDON PATHS OR SPACING OF UNIFORMLY DISTRIBUTED TENDON BUNDLES.
 - A MINIMUM OF (2) TENDONS SHALL PASS THROUGH EACH COLUMN IN EACH DIRECTION, WITH TENDONS LOCATED INSIDE OF THE COLUMN VERTICAL REINFORCEMENT CAGE.
 - STRAIGHT LINE HORIZONTAL OFFSETS SHOWN IN THE LAYOUT PLAN SHALL BE ACCOMPLISHED WITH SMOOTH HORIZONTAL CURVE/SWEEP PER DETAIL.
 - MAINTAIN CLEAR COVER BETWEEN EDGE-OF-SLAB (INCLUDING INTERIOR OPENINGS) AS INDICATED IN THE PROJECT SPECIFICATIONS AND TYPICAL DETAILS REFERENCED ABOVE.
- TENDON DRAPE ELEVATION NOTES:
 - # DENOTES PT TENDON DRAPE ELEVATION IN INCHES, MEASURED FROM BOTTOM OF SLAB / FRAMING TO THE CENTER-OF-GRAVITY OF THE TENDONS/STRAND BUNDLE (CGS).
 - AT DEAD ENDS AND STRESSING ENDS, CGS SHALL BE LOCATED AT THE CENTROID OF THE FRAMING (MID-DEPTH FOR SLAB CONDITIONS), UNLESS OTHERWISE NOTED.
 - TENDON HIGH POINTS SHALL BE LOCATED AT SUPPORTS (COLUMNS, WALLS, BEAMS, BANDED TENDON LINES) AS INDICATED ABOVE.
 - TENDON LOW POINTS SHALL BE LOCATED AT MID-SPAN BETWEEN SUPPORTS, UNLESS NOTED OTHERWISE.
- TENDON FORCE NOTES:
 - F = # K - DENOTES EFFECTIVE PRESTRESS FORCE, IN KIPS, WITHIN BANDED TENDON GROUP OR BEAM.
 - BANDED TENDON GROUPS SHALL BE PLACED IN FLAT BUNDLED GROUPS OF NO MORE THAN (5) TENDONS PER BUNDLE, SPACED AT NO MORE THAN 12 INCHES ON CENTER, WITH TENDONS FLAT IN ONE LAYER. TOTAL QUANTITY OF BANDED TENDONS SHALL BE EQUALLY DISTRIBUTED EACH SIDE OF THE COLUMN, MINUS A MINIMUM OF (2) TENDONS PASSING THROUGH THE COLUMN REINFORCEMENT CAGE PER NOTE ABOVE.
 - F = # KLF - DENOTES EFFECTIVE PRESTRESS FORCE, IN KIPS PER LINEAR FOOT, IN UNIFORMLY DISTRIBUTED TENDONS.
 - UNIFORMLY DISTRIBUTED TENDON BUNDLES SHALL BE PLACED IN FLAT BUNDLED GROUPS OF NO MORE THAN (5) TENDONS PER BUNDLE, SPACED AT NO MORE THAN 5'-0" ON CENTER, WITH TENDONS FLAT IN ONE LAYER.



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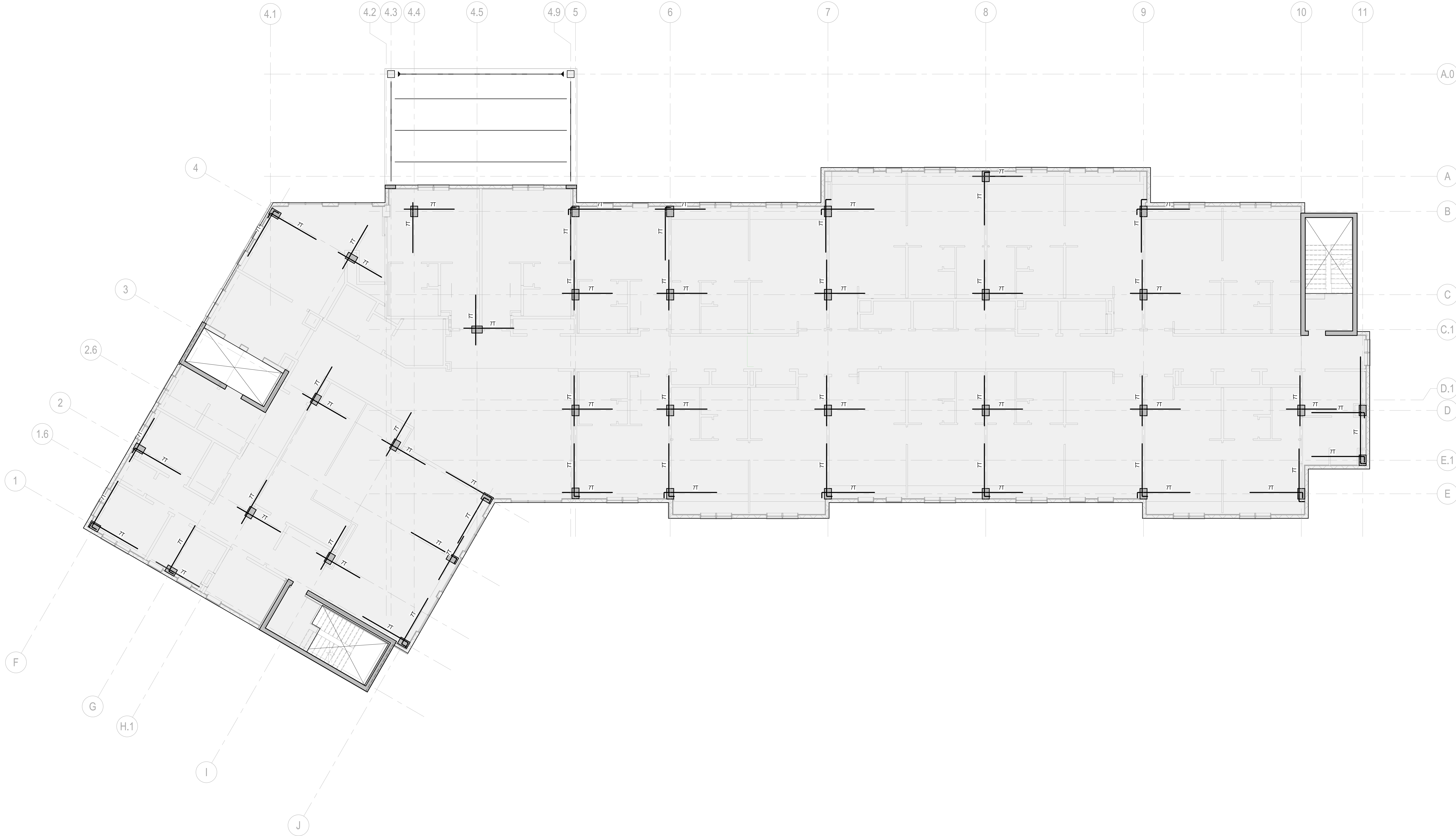
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SECOND FLOOR PT PLAN

S-122-PT

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1
S-122-R

SECOND FLOOR REINFORCING PLAN

1/8" = 1'-0"

CONCRETE SLAB REINFORCEMENT PLAN NOTES:

- FOR TYPICAL SLAB REINFORCEMENT DETAILS, SEE **SXXXX**.
- PROVIDE CONTINUOUS REINFORCEMENT AROUND PERIMETER OF SLAB AND AT ALL INTERIOR SLAB EDGES (SEE **SXXXX**). SEE **SXXXX** FOR CONCEPTUAL LAYOUT (NOT INDICATIVE OF PROJECT SPECIFIC GEOMETRY) AND SPLICE/DEVELOPMENT REQUIREMENTS.
- SLAB REINFORCEMENT SHALL BE #5 UNLESS NOTED OTHERWISE.
- #ISR-#(##) DENOTES SHEAR STUD RAILS (SEE SCHEDULE ON THIS SHEET AND **SXXXX**).
- SLAB REINFORCEMENT SYMBOLS AND NOMENCLATURE:

A. MAT REINFORCEMENT (SEE SCHEDULE).

PLACEMENT PRIORITY (SEE **SXXXX**)

ARROWS DENOTE EXTENT (SINGLE & DOUBLE ARROWS AS DEFINED IN NOTE S.B)

MAT REINFORCEMENT MARK (SEE SCHEDULE ON THIS SHEET)

B. ISOLATED/DISTRIBUTED REINFORCEMENT (SEE **SXXXX**).

BAR SPACING OVER WIDTH OR QUANTITY SHOWN

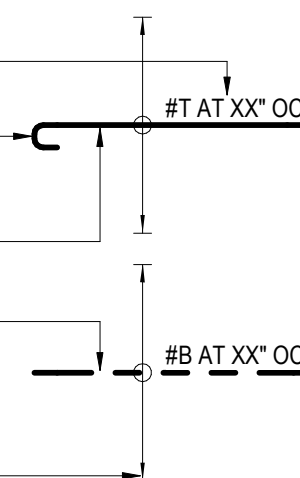
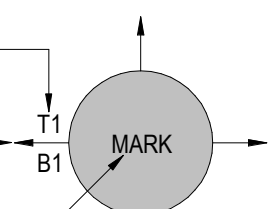
ACI STANDARD 180° HOOK (ALL TOP BARS HOOKED AT EDGE-OF-SLAB EVEN WHEN NOT INDICATED)

SOLID LINE DENOTES TOP BARS (OR TOP & BOTTOM)

DASHED LINE DENOTES BOTTOM BARS

SINGLE ARROW DENOTES EXTENT OF BAR PLACEMENT. DOUBLE ARROW DENOTES EXTENT TO CONTINUE UNTIL EDGE-OF-SLAB OR END-OF-ELEMENT (AS APPLICABLE)

- TYPICAL NOMENCLATURE (SEE **SXXXX**).
- REINFORCEMENT AT COLUMNS (SEE **SXXXX**).



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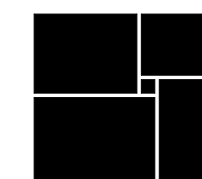
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SECOND
FLOOR
REINFORCING
PLAN

S-122-R

[illegible]

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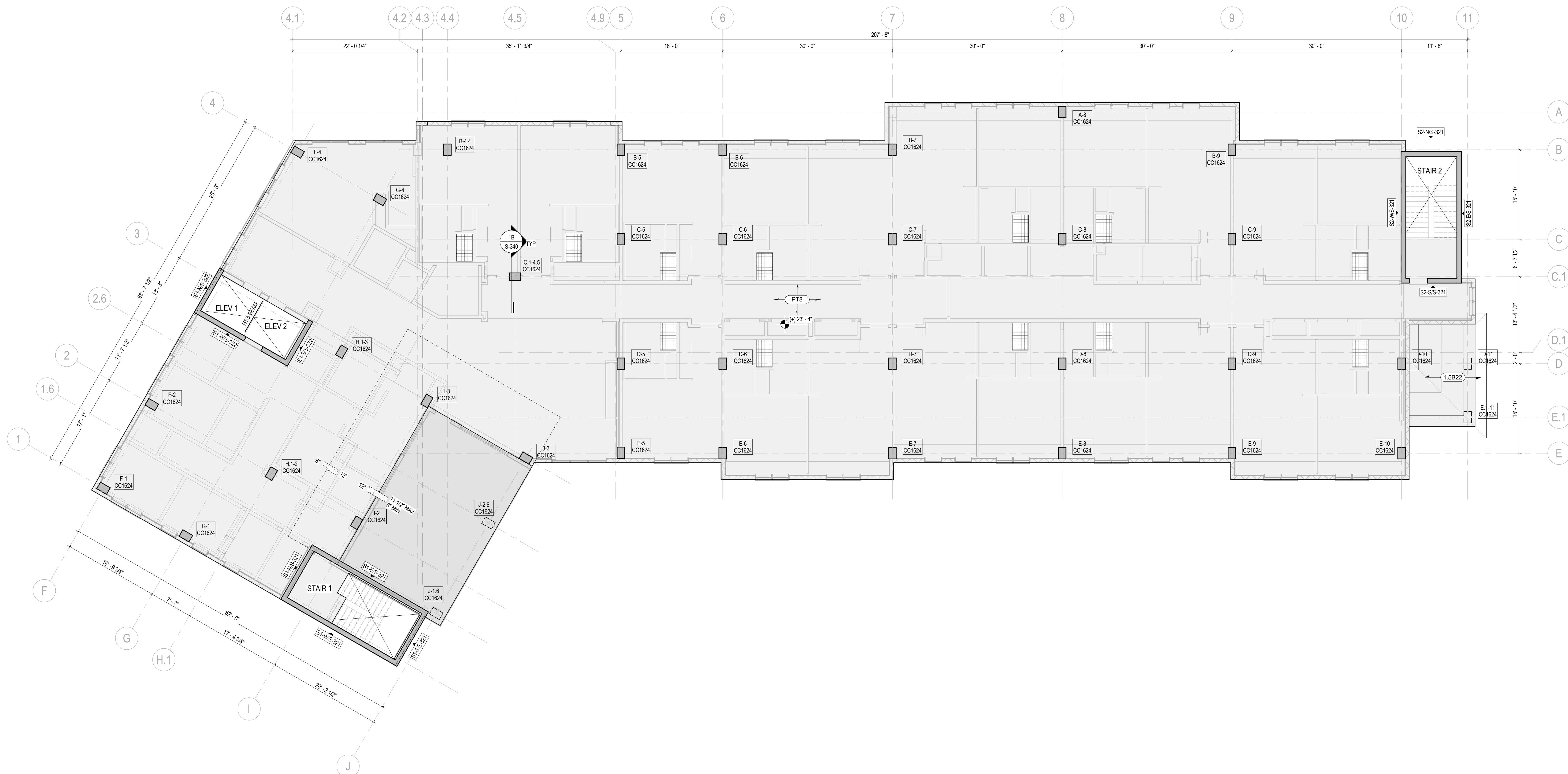
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THIRD FLOOR FRAMING PLAN

S-123



1
S-123

THIRD FLOOR FRAMING PLAN

1/8" = 1'-0"

CONCRETE FRAMING PLAN NOTES:

$$1/8'' = 1'-0''$$

CONCRETE FRAMING PLAN NOTES

- CONCRETE SLAB TAG
- ARROWS DENOTE SLAB SPAN DIRECTION
- CSM# DENOTES SLAB MARK
- SEE SCHEDULE ON THIS SHEET FOR SLAB THICKNESS AND REINFORCEMENT
2. SEE PLAN FOR TOP OF SLAB ELEVATIONS.
3. DENOTES CONCRETE COLUMN/PIER/VALVE
FOR COLUMN SCHEDULE AND DETAILS SEE SXXXX SERIES.
FOR WALL DETAILS SEE SXXXX SERIES.
4. DENOTES LOAD BEARING CONCRETE MASONRY UNIT (CMU) WALL WITH EX AT XX OR VERTICAL REINFORCEMENT, CENTERED IN FULLY-GROUTED CELL.
5. DENOTES NONLOAD BEARING CONCRETE MASONRY UNIT (CMU) WALL WITH EX AT XX OR VERTICAL REINFORCEMENT, CENTERED IN FULLY-GROUTED CELL.
6. DENOTES STEP IN TOP OF SLAB. SEE SXXXX.
7. SEE ARCHITECTURAL DRAWINGS FOR:
• ALL SLOPED SLAB AREAS.
• ALL DIMENSIONS NOT SHOWN. VERIFY ALL DIMENSIONS SHOWN IN THE STRUCTURAL DRAWINGS WITH ARCHITECTURAL DRAWINGS AND REPORT ANY DISCREPANCIES OR DIMENSIONS NOT SHOWN ON THE ARCHITECTURAL DRAWINGS FOR CLARIFICATION.
8. SEE MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR ADDITIONAL FLOOR PENETRATIONS. SLEEVES, AND INSERTS REQUIRED TO BE CAST IN THE SLAB.
• SLEEVES AND PENETRATIONS WITHIN 6 INCHES OF THE FACE OF ANY CONCRETE COLUMN NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS, SHALL BE SUBMITTED FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD.
• SLEEVES AND PENETRATIONS INTERRUPTING Banded LINES OF TENDONS, NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS, SHALL BE SUBMITTED FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD.
• SLEEVES AND PENETRATIONS GREATER THAN 12 INCHES IN LENGTH OR WIDTH NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS, SHALL BE SUBMITTED FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD.
9. FOR REINFORCEMENT PLAN AND ADDITIONAL NOTES, SEE SHEET SXXXX
10. FOR POST-TENSIONED LAYOUT PLAN AND ADDITIONAL NOTES, SEE SHEET SXXXX
11. DENOTES SHOWER DEPRESSIONS.

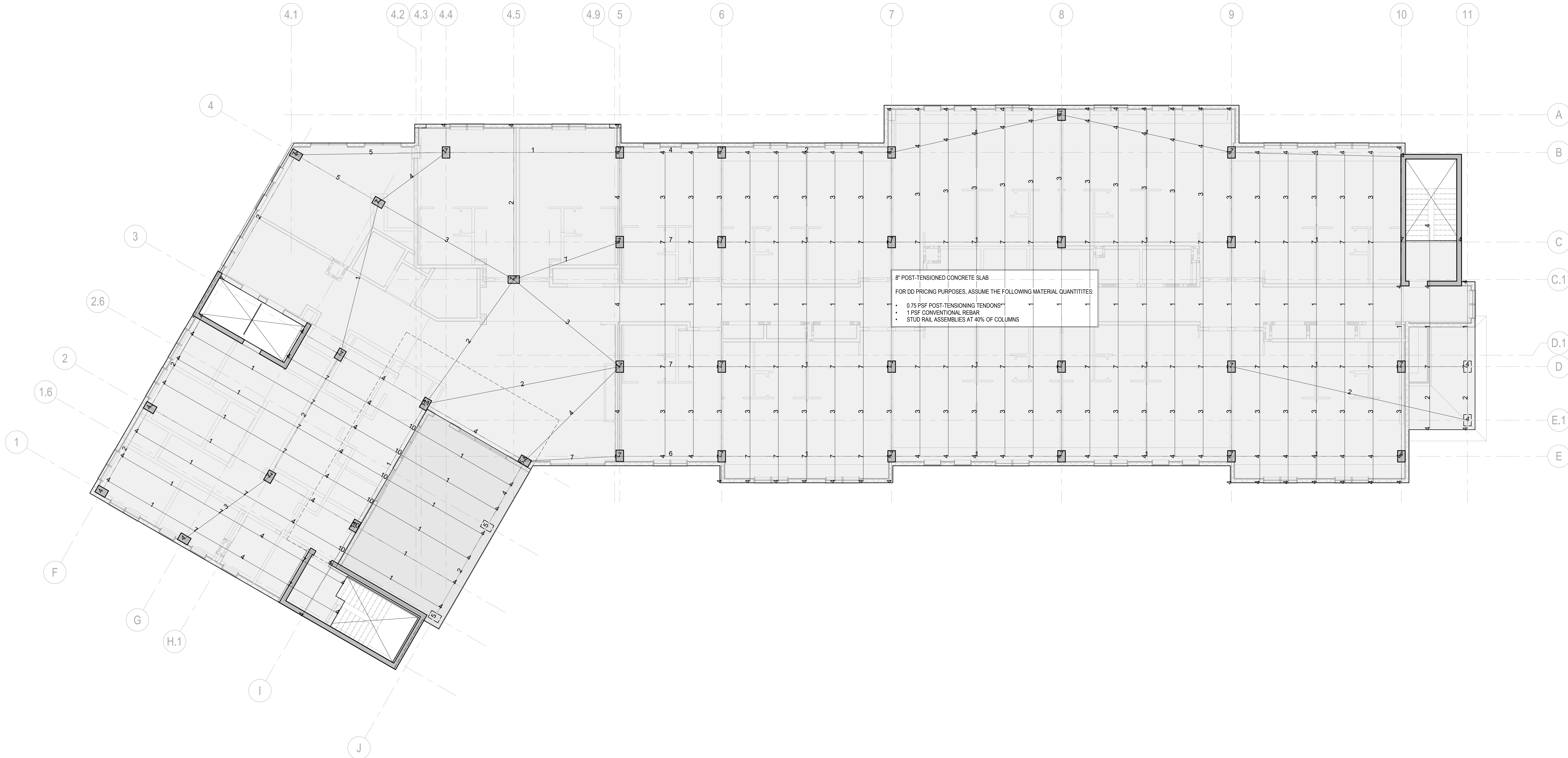


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1
S-123-PT

THIRD FLOOR PT PLAN

1/8" = 1'-0"

POST-TENSIONED CONCRETE SLAB PLAN NOTES:

- SEE FRAMING PLAN FOR SLAB GEOMETRY, REFERENCE ELEVATION, AND KEYED SECTIONS.
- SEE POST-TENSIONED FRAMING GENERAL NOTES ON SHEET **S-XXXX** AND DIVISION 03 SPECIFICATIONS FOR GENERAL REQUIREMENTS.
- FOR TYPICAL POST-TENSIONED CONCRETE SECTIONS AND DETAILS, SEE **S-XXXX**.
- TENDON LAYOUT NOTES:
 - LINEWORK SHOWN FOR TENDON PATHS IS SCHEMATIC TO ILLUSTRATE DESIGN INTENT, BUT NOT MEANT TO INDICATE EXACT TENDON PATHS OR SPACING OF UNIFORMLY DISTRIBUTED TENDON BUNDLES.
 - A MINIMUM OF (2) TENDONS SHALL PASS THROUGH EACH COLUMN IN EACH DIRECTION, WITH TENDONS LOCATED INSIDE OF THE COLUMN VERTICAL REINFORCEMENT CAGE.
 - STRAIGHT LINE HORIZONTAL OFFSETS SHOWN IN THE LAYOUT PLAN SHALL BE ACCOMPLISHED WITH SMOOTH HORIZONTAL CURVE/SWEEP PER DETAIL.
 - MAINTAIN CLEAR COVER BETWEEN EDGE-OF-SLAB (INCLUDING INTERIOR OPENINGS) AS INDICATED IN THE PROJECT SPECIFICATIONS AND TYPICAL DETAILS REFERENCED ABOVE.
- TENDON DRAPE ELEVATION NOTES:
 - # : DENOTES PT TENDON DRAPE ELEVATION IN INCHES, MEASURED FROM BOTTOM OF SLAB / FRAMING TO THE CENTER-OF-GRAVITY OF THE TENDONS/STRAND BUNDLE (CGS).
 - AT DEAD ENDS AND STRESSING ENDS, CGS SHALL BE LOCATED AT THE CENTROID OF THE FRAMING (MID-DEPTH FOR SLAB CONDITIONS), UNLESS OTHERWISE NOTED.
 - TENDON HIGH POINTS SHALL BE LOCATED AT SUPPORTS (COLUMNS, WALLS, BEAMS, BANDED TENDON LINES) AS INDICATED ABOVE.
 - TENDON LOW POINTS SHALL BE LOCATED AT MID-SPAN BETWEEN SUPPORTS, UNLESS NOTED OTHERWISE.
- TENDON FORCE NOTES:
 - F = # K : DENOTES EFFECTIVE PRESTRESS FORCE, IN KIPS, WITHIN BANDED TENDON GROUP OR BEAM.
 - BANDED TENDON GROUPS SHALL BE PLACED IN FLAT BUNDLED GROUPS OF NO MORE THAN (5) TENDONS PER BUNDLE, SPACED AT NO MORE THAN 12 INCHES ON CENTER, WITH TENDONS FLAT IN ONE LAYER.
 - TOTAL QUANTITY OF BANDED TENDONS SHALL BE EQUALLY DISTRIBUTED EACH SIDE OF THE COLUMN, MINUS A MINIMUM OF (2) TENDONS PASSING THROUGH THE COLUMN REINFORCEMENT CAGE PER NOTE ABOVE.
 - F = # KLF : DENOTES EFFECTIVE PRESTRESS FORCE, IN KIPS PER LINEAR FOOT, IN UNIFORMLY DISTRIBUTED TENDONS.
 - UNIFORMLY DISTRIBUTED TENDON BUNDLES SHALL BE PLACED IN FLAT BUNDLED GROUPS OF NO MORE THAN (5) TENDONS PER BUNDLE, SPACED AT NO MORE THAN 5'-0" ON CENTER, WITH TENDONS FLAT IN ONE LAYER.

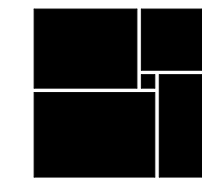


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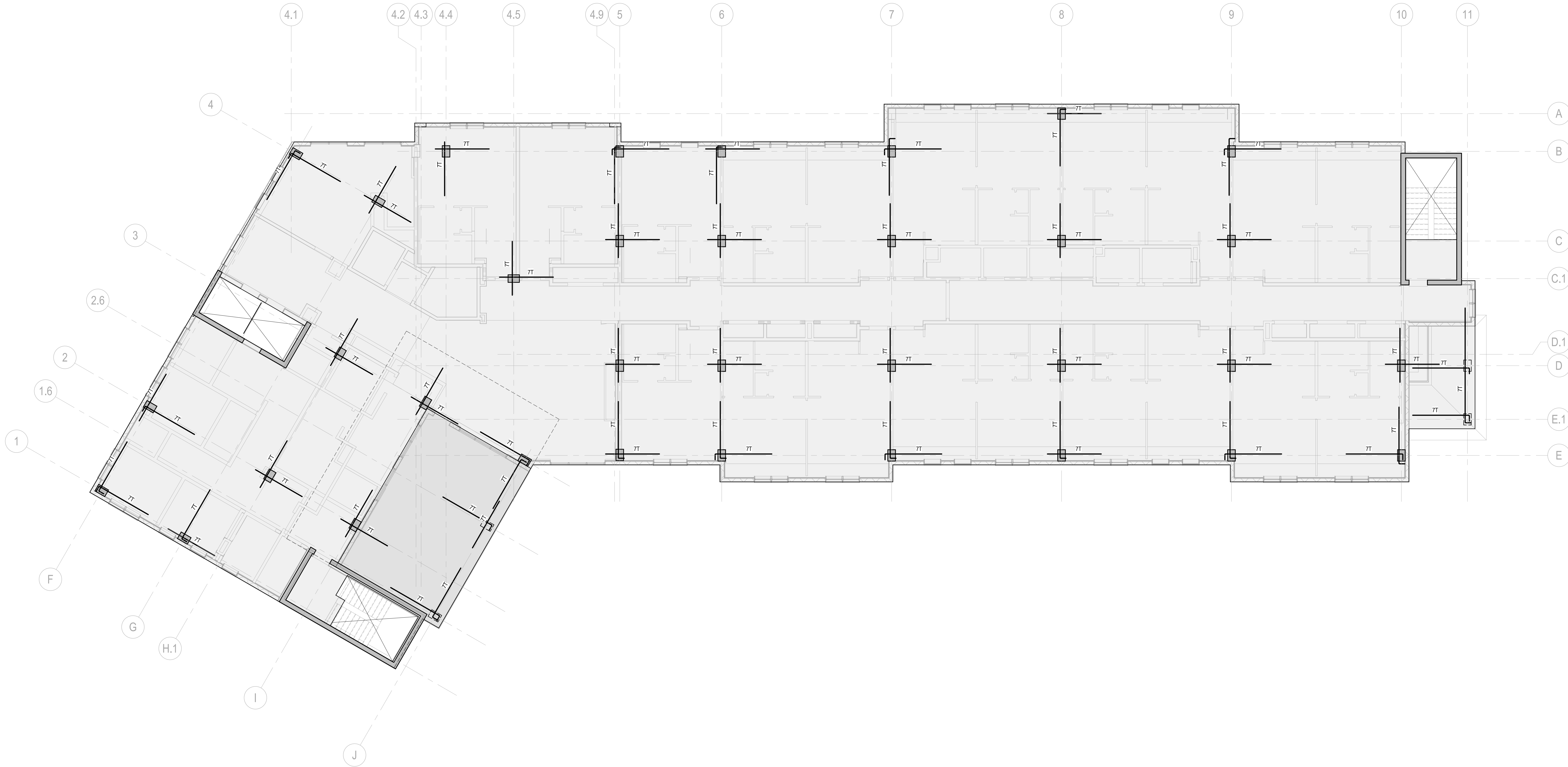
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THIRD FLOOR PT PLAN

S-123-PT

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1
S-123-R

THIRD FLOOR REINFORCING PLAN

1/8" = 1'-0"

CONCRETE SLAB REINFORCEMENT PLAN NOTES:

- FOR TYPICAL SLAB REINFORCEMENT DETAILS, SEE **SXXXX**.
- PROVIDE CONTINUOUS REINFORCEMENT AROUND PERIMETER OF SLAB AND AT ALL INTERIOR SLAB EDGES (SEE **SXXXX**). SEE **SXXXX** FOR CONCEPTUAL LAYOUT (NOT INDICATIVE OF PROJECT SPECIFIC GEOMETRY) AND SPLICE/DEVELOPMENT REQUIREMENTS.
- SLAB REINFORCEMENT SHALL BE #5 UNLESS NOTED OTHERWISE.
- #SR-#(R)** DENOTES SHEAR STUD RAILS (SEE SCHEDULE ON THIS SHEET AND **SXXXX**).
- SLAB REINFORCEMENT SYMBOLS AND NOMENCLATURE:

A. MAT REINFORCEMENT (SEE SCHEDULE).

PLACEMENT PRIORITY (SEE **SXXXX**)

ARROWS DENOTE EXTENT

(SINGLE & DOUBLE ARROWS AS

DEFINED IN NOTE 5.B)

MAT REINFORCEMENT MARK

(SEE SCHEDULE ON THIS SHEET)

B. ISOLATED/DISTRIBUTED REINFORCEMENT (SEE **SXXXX**)

BAR SPACING OVER WIDTH OR QUANTITY SHOWN

ACI STANDARD 180° HOOK

(ALL TOP BARS HOOKED AT EDGE-OF-

SLAB EVEN WHEN NOT INDICATED)

SOLID LINE DENOTES

TOP BARS (OR TOP & BOTTOM)

DASHED LINE DENOTES

BOTTOM BARS

SINGLE ARROW DENOTES EXTENT OF BAR

PLACEMENT: DOUBLE ARROW DENOTES EXTENT

TO CONTINUE UNTIL EDGE-OF-SLAB OR END-OF-

ELEMENT (AS APPLICABLE)

C. TYPICAL NOMENCLATURE (SEE **SXXXX**)

D. REINFORCEMENT AT COLUMNS (SEE **SXXXX**)



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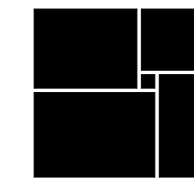
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THIRD FLOOR REINFORCING PLAN

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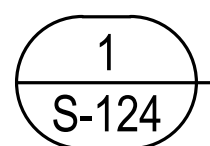
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S-124



1/8" = 1'-0"
CONCRETE FRAMING PLAN NOTES

-

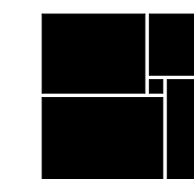
2
S-124

$$1/8^* = 1' - 0$$


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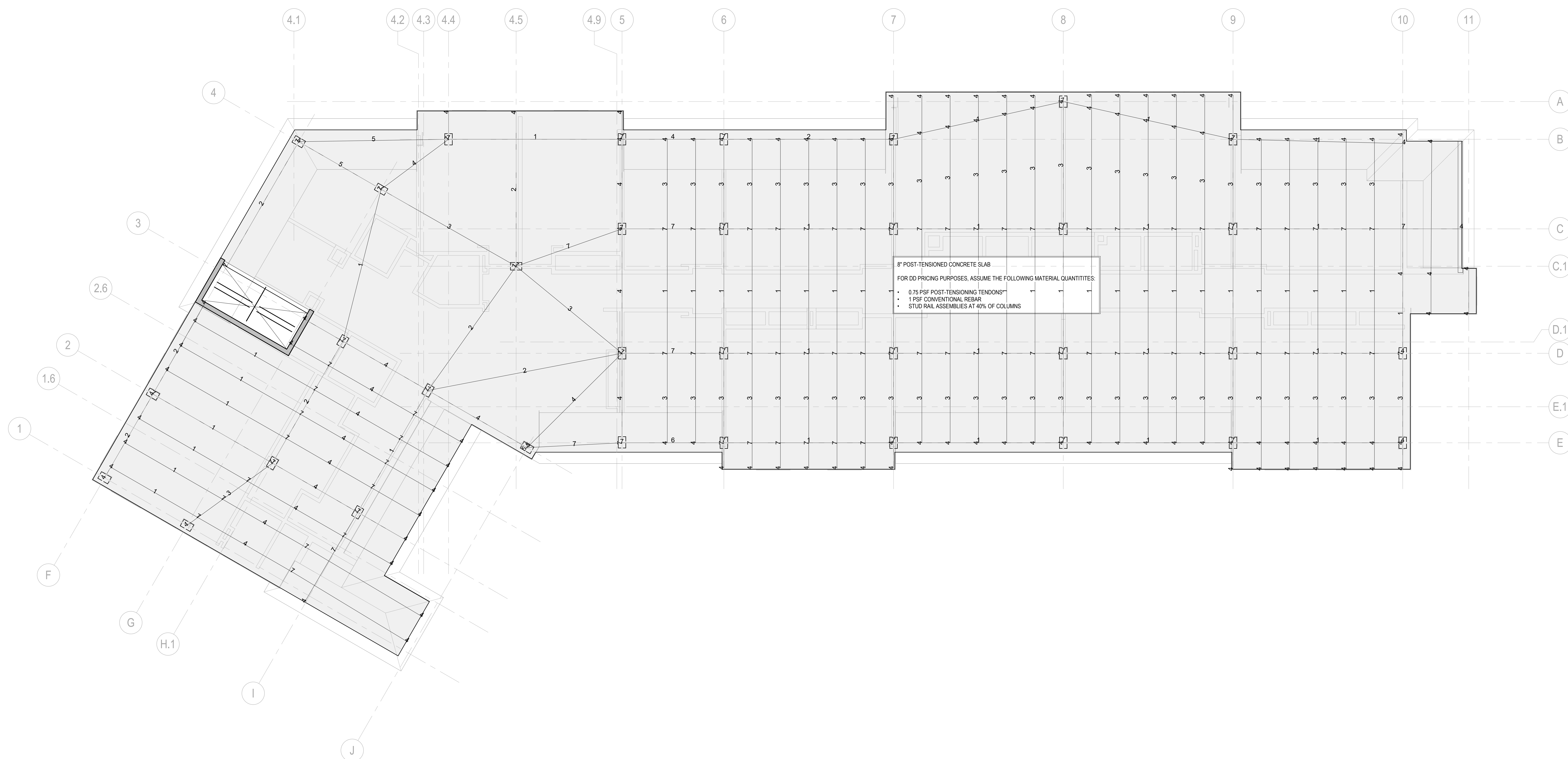
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ROOF P
PLAN

S-124-PT

[illegible]

ROOF PT PLAN

$$1/8'' = 1'-0''$$

POST-TENSIONED CONCRETE SLAB PLAN NOTES

1. SEE FRAMING PLAN FOR SLAB GEOMETRY, REFERENCE ELEVATION, AND KEYED SECTIONS.
2. SEE POST-TENSIONED FRAMING GENERAL NOTES ON SHEET XXXXX AND DIVISION 03 SPECIFICATIONS FOR GENERAL REQUIREMENTS.
3. FOR TYPICAL POST-TENSIONED CONCRETE SECTIONS AND DETAILS, SEE XXXXX.
4. TENDON LAYOUT NOTES:
 - LINEWORK SHOWING FOR TENDON PATHS IS SCHEMATIC TO ILLUSTRATE DESIGN INTENT, BUT NOT MEANT TO INDICATE EXACT TENDON PATHS OR SPACING OF TENDON OR UNIFORMITY OF DISTRIBUTED TENDON BUNDLES.
 - A MINIMUM OF 10' TENDON SPACING SHALL PASS THROUGH EACH COLUMN IN EACH DIRECTION, WITH TENDONS LOCATED INSIDE OF THE COLUMN VERTICAL REINFORCEMENT CAGE.
 - STRAIGHT LINE HORIZONTAL ORIENTATIONS SHOWN IN THE LAYOUT PLAN SHALL BE ACCOMPLISHED WITH SMOOTH ORIENTAL CURVE/SWEEP PER DETAILS.
 - MAINTAIN CLEAR COVER BETWEEN EDGE-OF-SLAB (INCLUDING INTERIOR OPENINGS) AS INDICATED IN THE PROJECT SPECIFICATIONS AND TYPICAL DETAILS REFERRED ABOVE.
5. TENDON DRAPE ELEVATION NOTES:
 - # DENOTES TP TENDON DRAPE ELEVATION IN INCHES, MEASURED FROM BOTTOM OF SLAB / FRAMING TO THE CENTER-OF-GRAVITY OF THE TENDON'S STRAND BUNDLE (CGS).
 - AT JOISTS AND STRUTTING BEAMS, CGS SHALL BE LOCATED AT THE CENTROID OF THE FRAMING (MID-DEPTH FOR CREATION OF NOTCHES), UNLESS OTHERWISE NOTED.
 - TENDON HIGHT POINTS SHALL BE LOCATED AT SUPPORTS (COLUMNS, WALLS, BEAMS, BANDED TENDON LINES) AS INDICATED ABOVE.
 - TENDON HIGHT POINTS SHALL BE LOCATED AT MID-SPAN BETWEEN SUPPORTS, UNLESS NOTED OTHERWISE.

BANDED TENDON GROUPS SHALL BE PLACED IN FLAT BUNDLED GROUPS OF NO MORE THAN (5) TENDONS PER BUNDLE, SPACED AT NO MORE THAN 12 INCHES ON CENTER, WITH TENDONS FLAT IN ONE LAYER. TOTAL QUANTITY OF BANDED TENDONS SHALL BE EQUALLY DISTRIBUTED EACH SIDE OF THE COLUMN, MINUS A MINIMUM OF (2) TENDONS PASSING THROUGH THE COLUMN REINFORCEMENT CAGE PER NOTE ABOVE.

- $F = \# \text{KLF}$ DENOTES EFFECTIVE PRESTRESS FORCE, IN KIPS PER LINEAR FOOT, IN UNIFORMLY DISTRIBUTED TENDONS.

UNIFORMLY DISTRIBUTED TENDON BUNDLES SHALL BE PLACED IN FLAT BUNDLED GROUPS OF NO MORE THAN (5) TENDONS PER BUNDLE, SPACED AT NO MORE THAN 5'-0" ON CENTER, WITH TENDONS FLAT IN ONE LAYER.

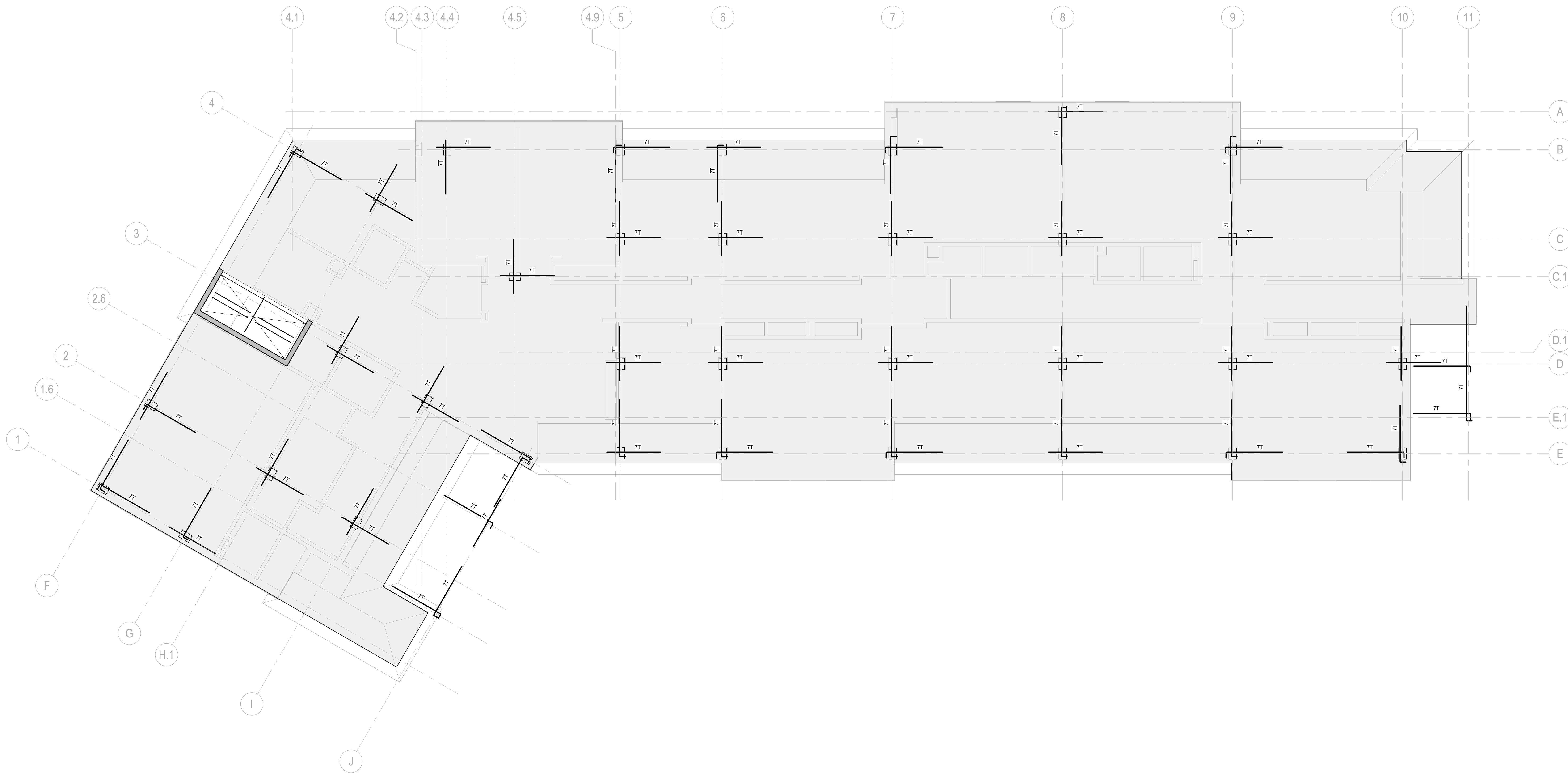


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1
S-124-R

ROOF REINFORCING PLAN

1/8" = 1'-0"

CONCRETE SLAB REINFORCEMENT PLAN NOTES:

- FOR TYPICAL SLAB REINFORCEMENT DETAILS, SEE **SXXXX**.
- PROVIDE CONTINUOUS REINFORCEMENT AROUND PERIMETER OF SLAB AND AT ALL INTERIOR SLAB EDGES (SEE **SXXXX**). SEE **SXXXX** FOR CONCEPTUAL LAYOUT (NOT INDICATIVE OF PROJECT SPECIFIC GEOMETRY) AND SPLICE/DEVELOPMENT REQUIREMENTS.
- SLAB REINFORCEMENT SHALL BE #5 UNLESS NOTED OTHERWISE.
- #SR-#(#-#) : DENOTES SHEAR STUD RAILS (SEE SCHEDULE ON THIS SHEET AND **SXXXX**).
- SLAB REINFORCEMENT SYMBOLS AND NOMENCLATURE:

A. MAT REINFORCEMENT (SEE SCHEDULE).

PLACEMENT PRIORITY (SEE **SXXXX**)

ARROWS DENOTE EXTENT (SINGLE & DOUBLE ARROWS AS DEFINED IN NOTE 5.B)

MAT REINFORCEMENT MARK (SEE SCHEDULE ON THIS SHEET)

B. ISOLATED/DISTRIBUTED REINFORCEMENT (SEE **SXXXX**).

BAR SPACING OVER WIDTH OR QUANTITY SHOWN

ACI STANDARD 180° HOOK (ALL TOP BARS HOOKED AT EDGE-OF-SLAB EVEN WHEN NOT INDICATED)

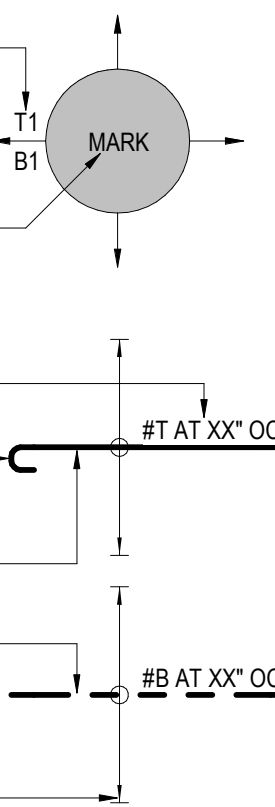
SOLID LINE DENOTES TOP BARS (OR TOP & BOTTOM)

DASHED LINE DENOTES BOTTOM BARS

SINGLE ARROW DENOTES EXTENT OF BAR PLACEMENT. DOUBLE ARROW DENOTES EXTENT TO CONTINUE UNTIL EDGE-OF-SLAB OR END-OF-ELEMENT (AS APPLICABLE)

C. TYPICAL NOMENCLATURE (SEE **SXXXX**).

D. REINFORCEMENT AT COLUMNS (SEE **SXXXX**).



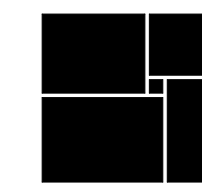
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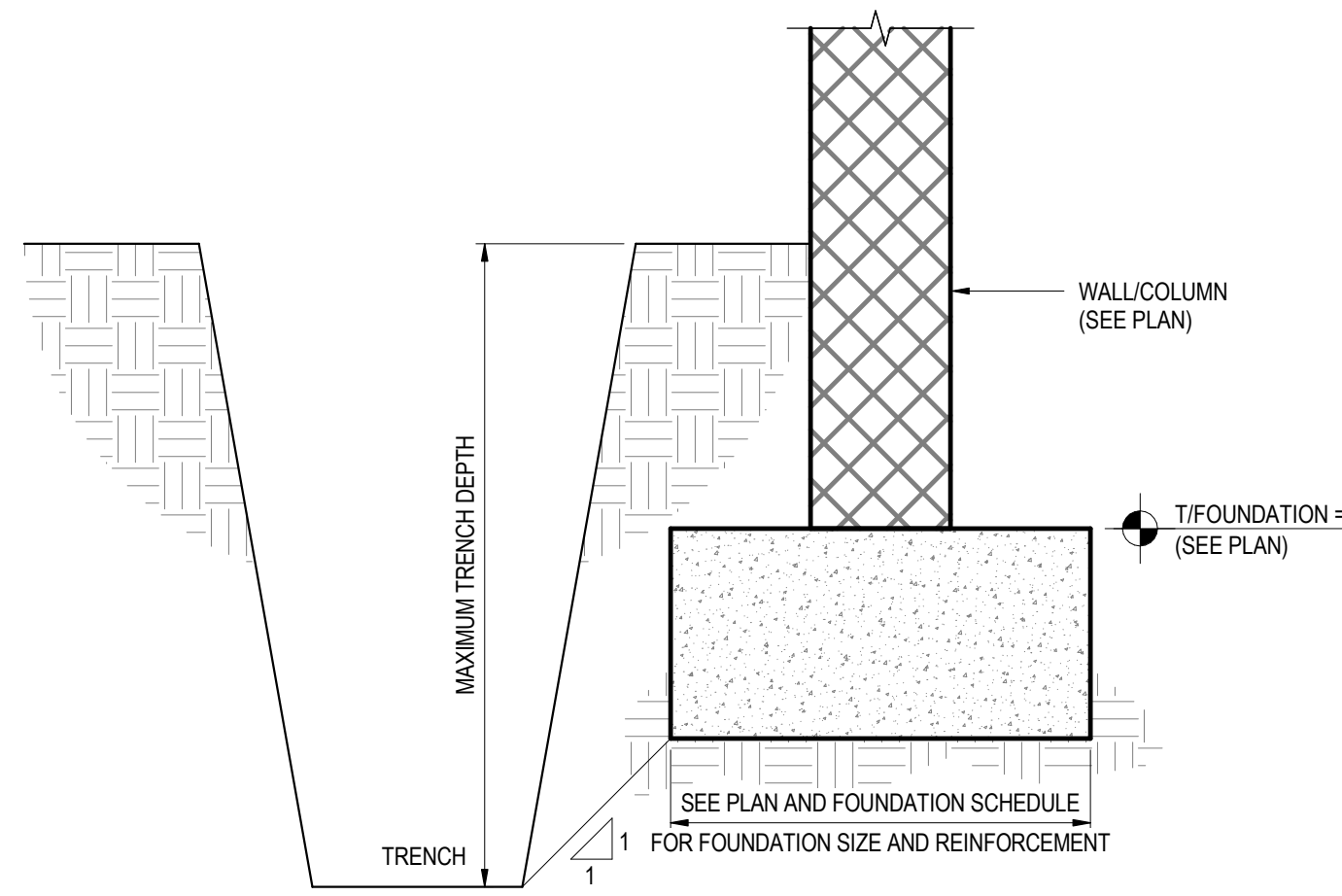
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ROOF REINFORCING PLAN

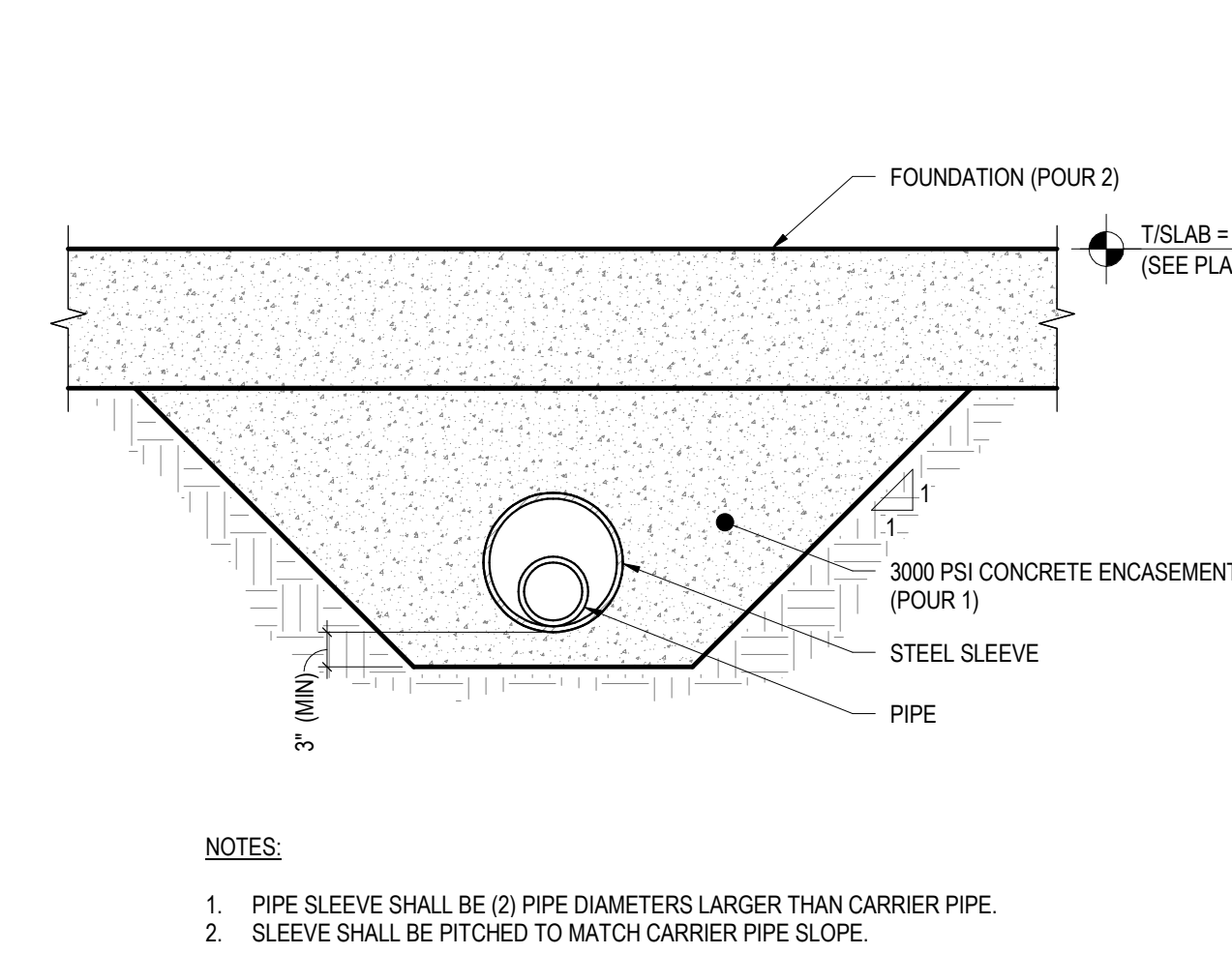
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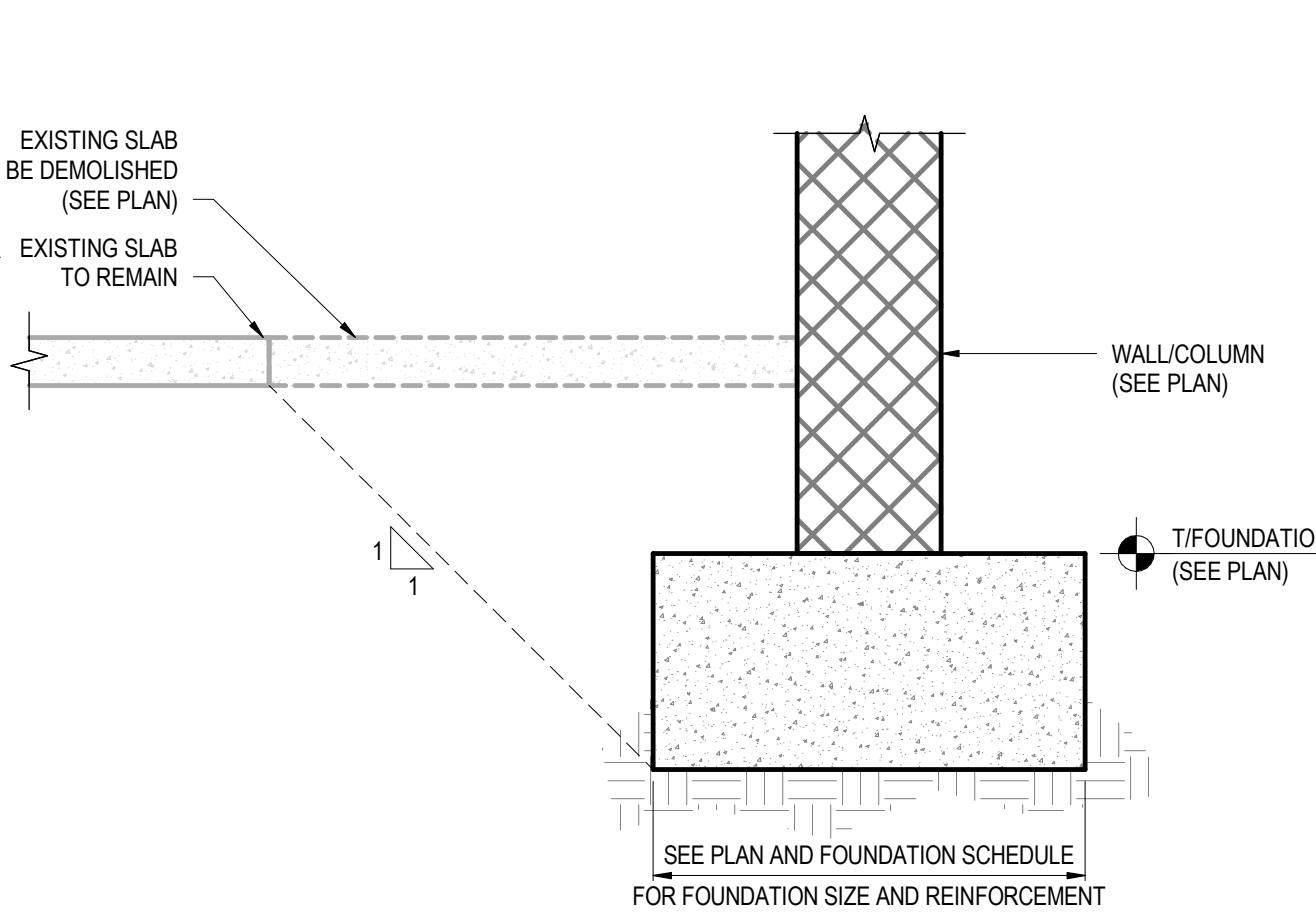
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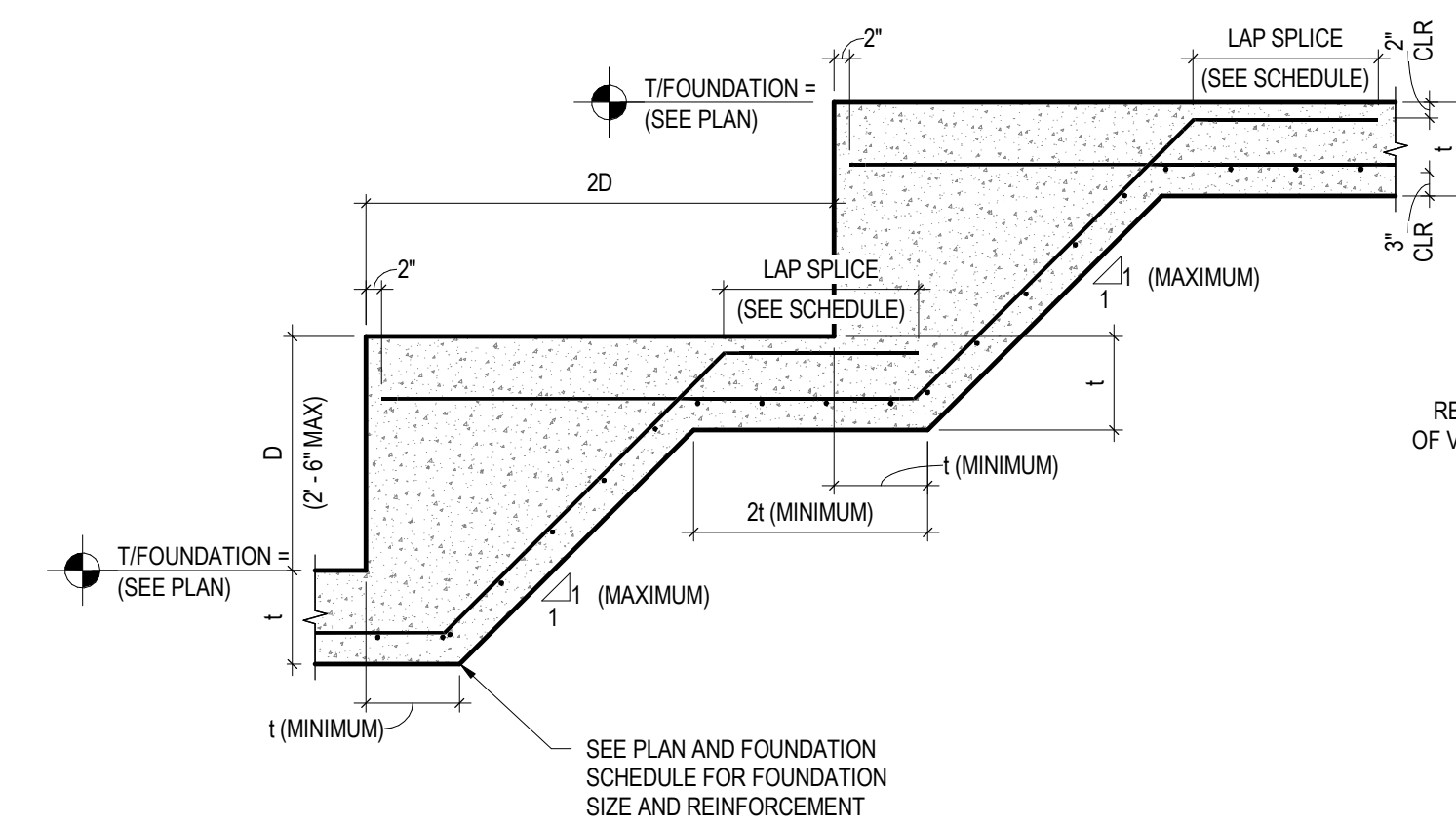
1 FOUNDATION ADJACENT TO TRENCH
3/4" = 1'-0"



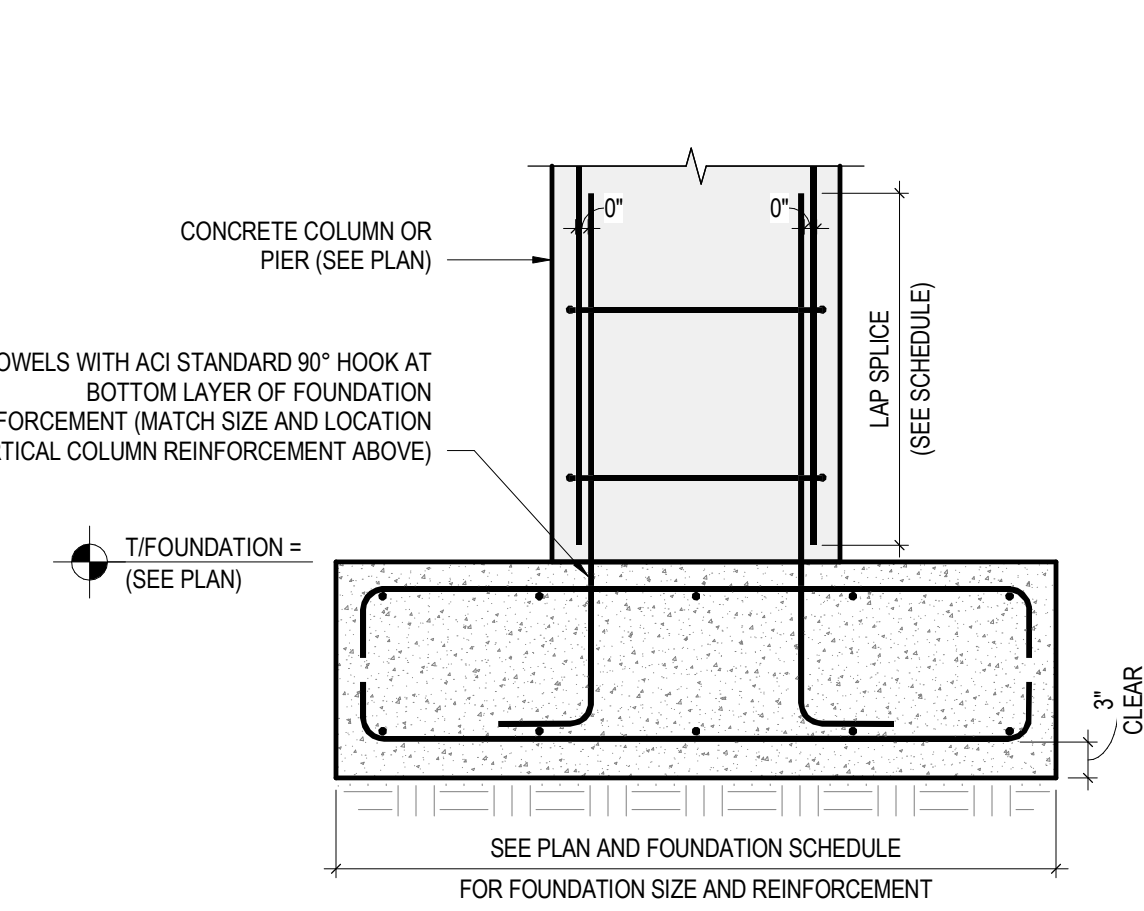
2 PIPE UNDER FOUNDATION
3/4" = 1'-0"



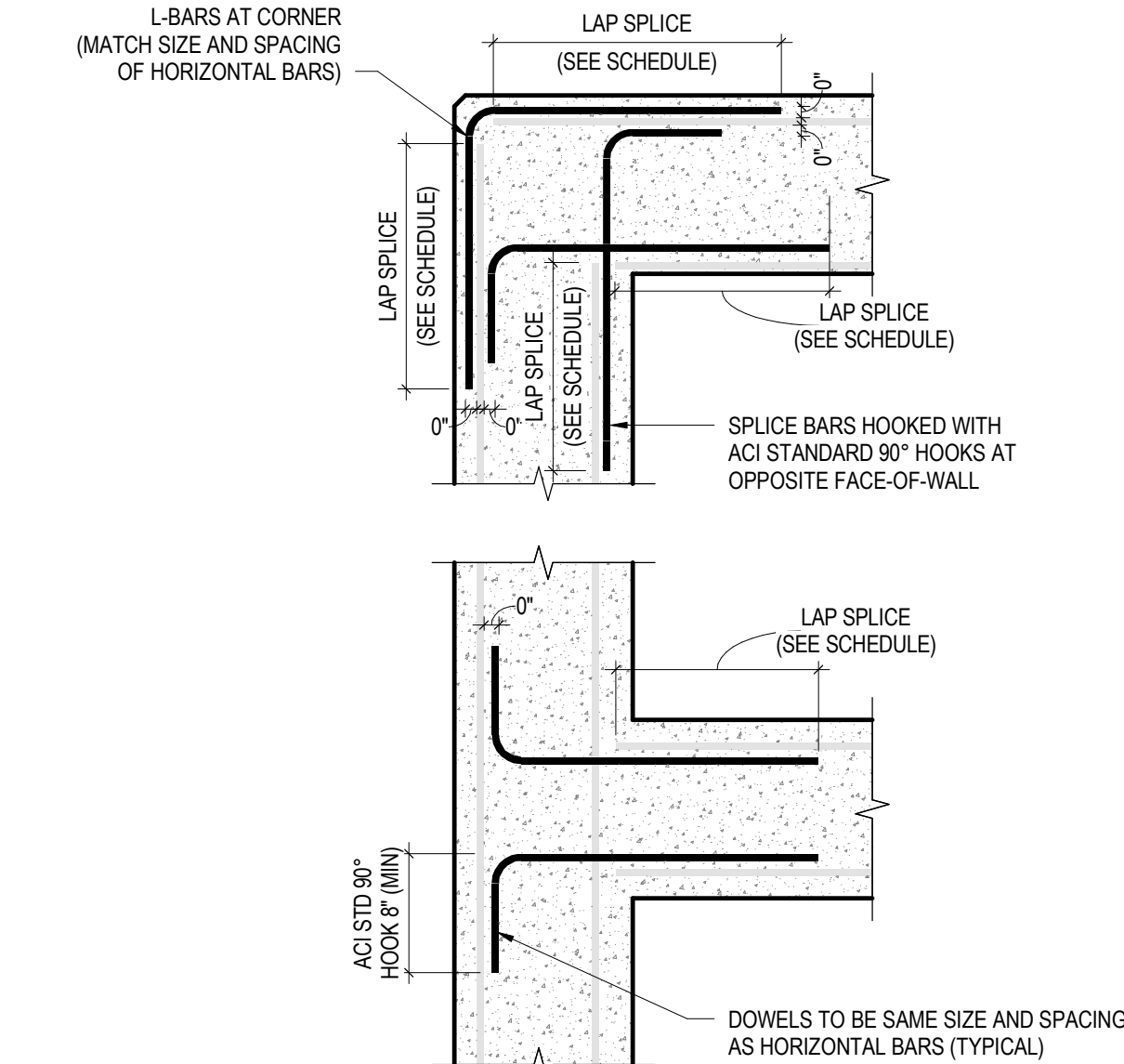
3 FOUNDATION ADJACENT TO EXISTING SLAB
3/4" = 1'-0"



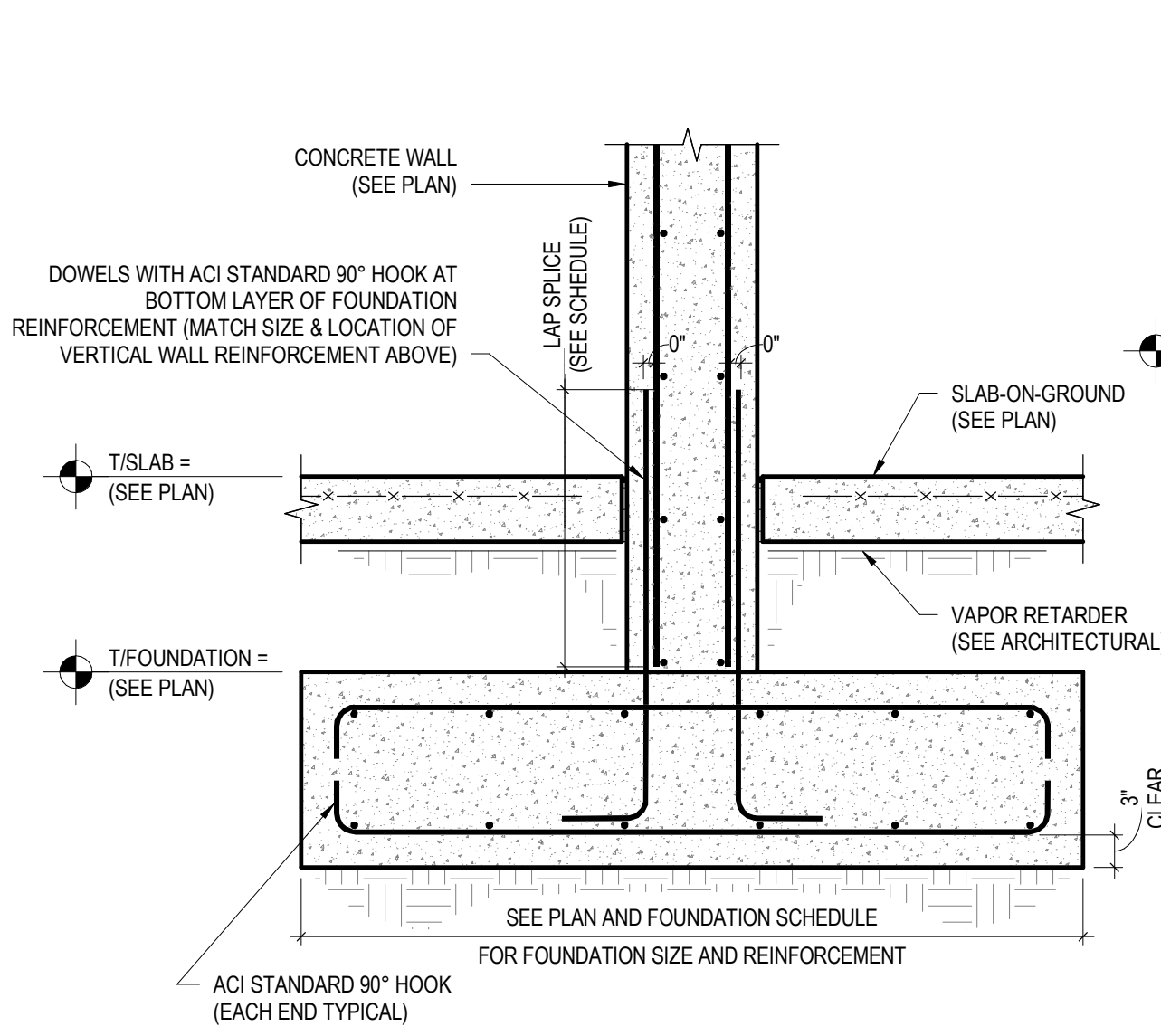
4 STEPPED CONTINUOUS WALL FOUNDATION
1/2" = 1'-0"



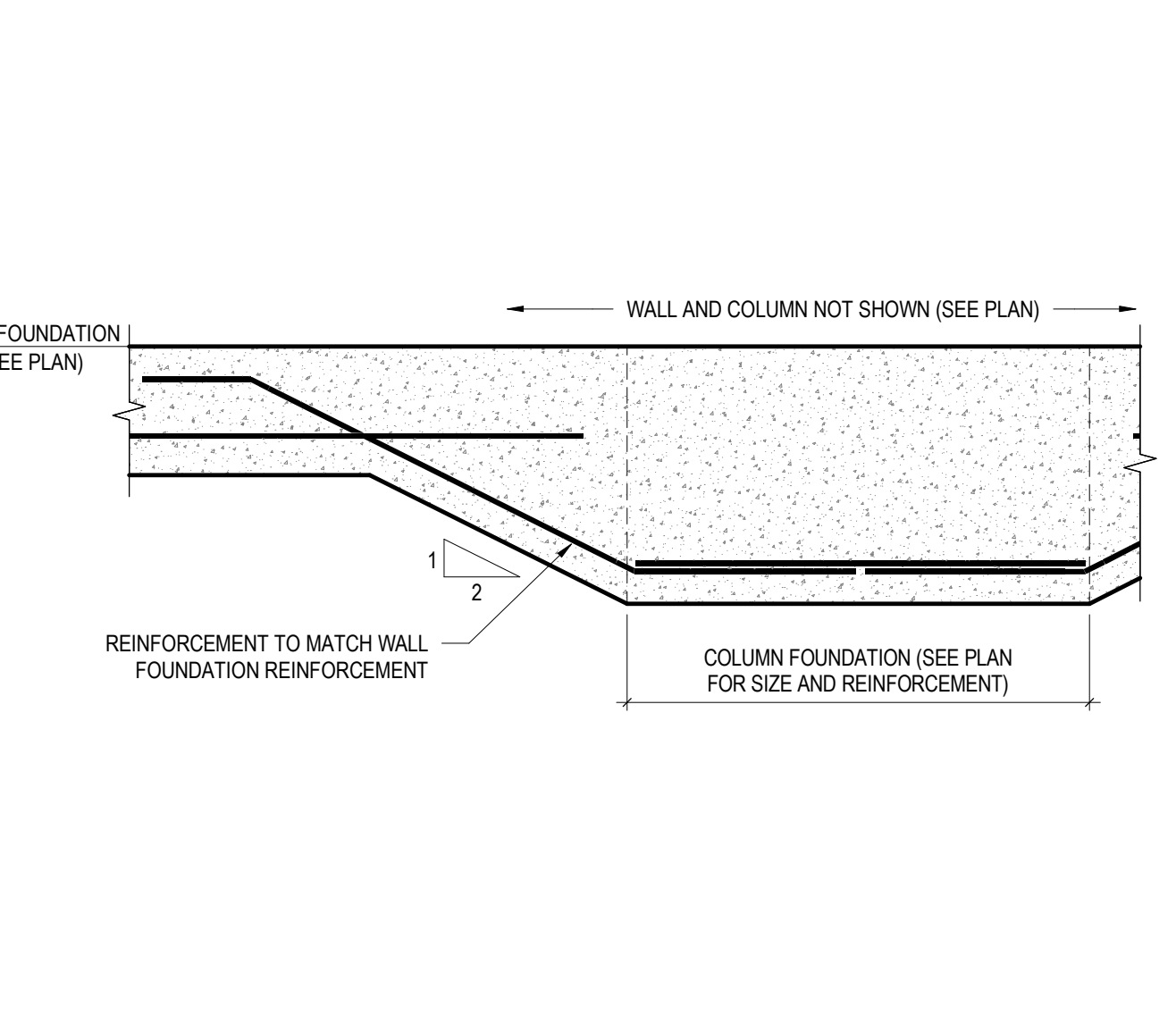
5 CONCRETE COLUMN/PIER FOUNDATION
3/4" = 1'-0"



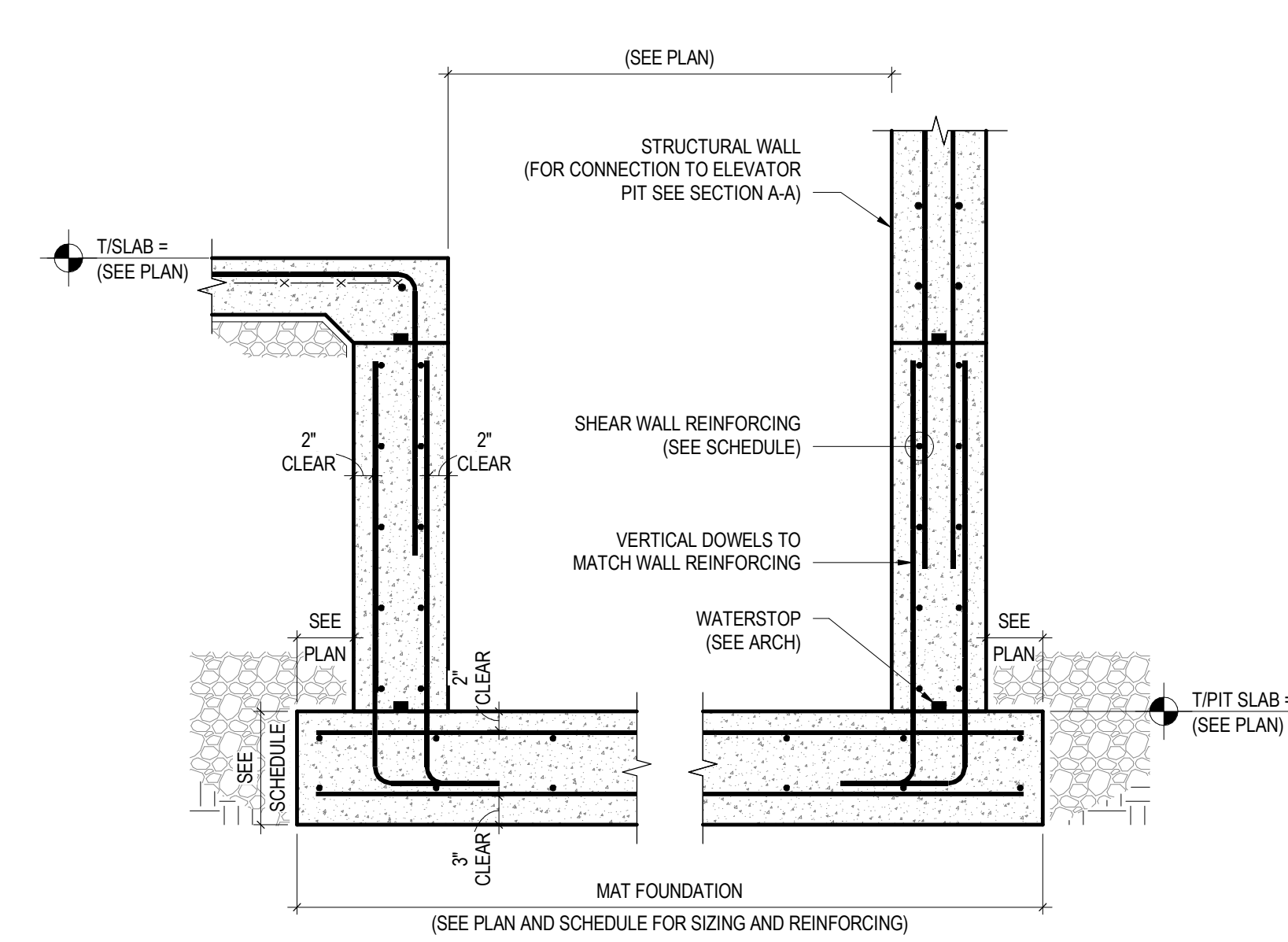
6 CORNER BARS AT WALLS & FOUNDATIONS
1" = 1'-0"



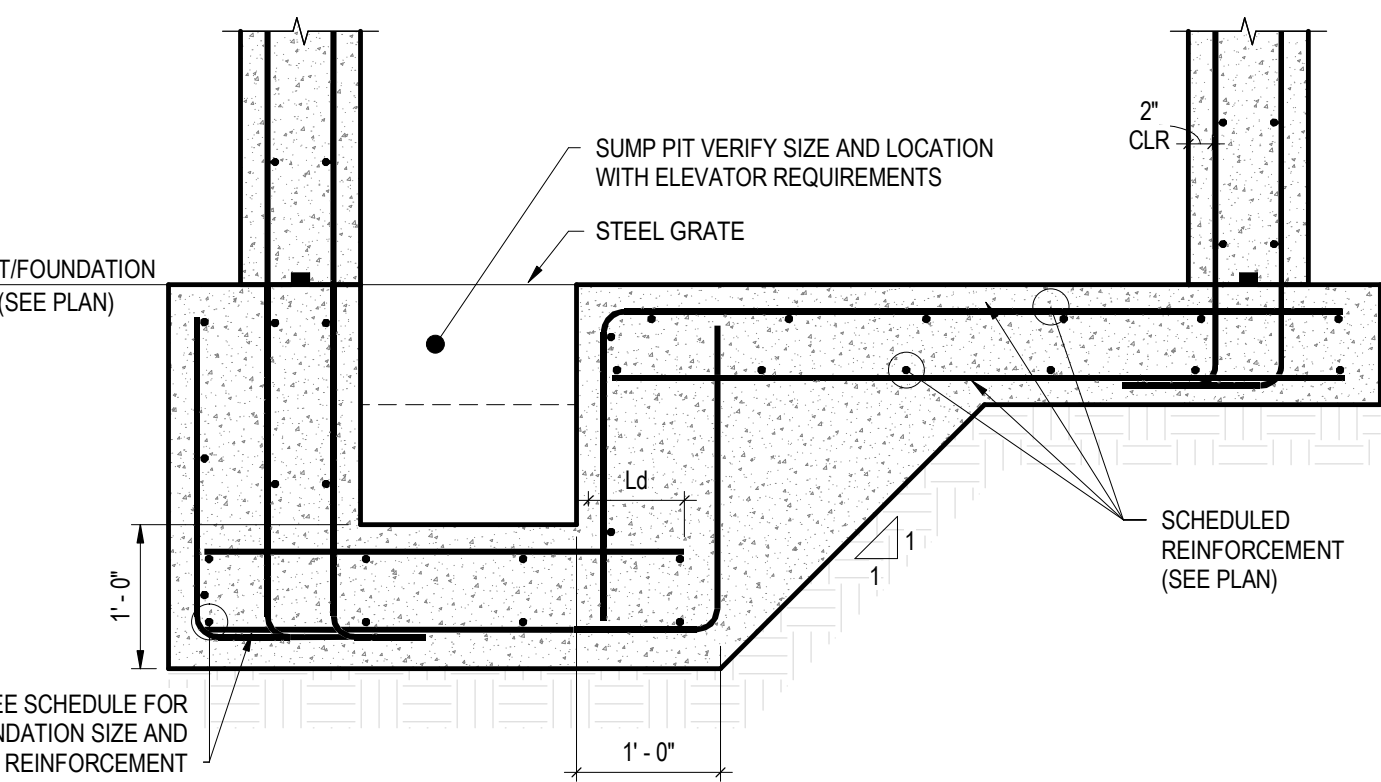
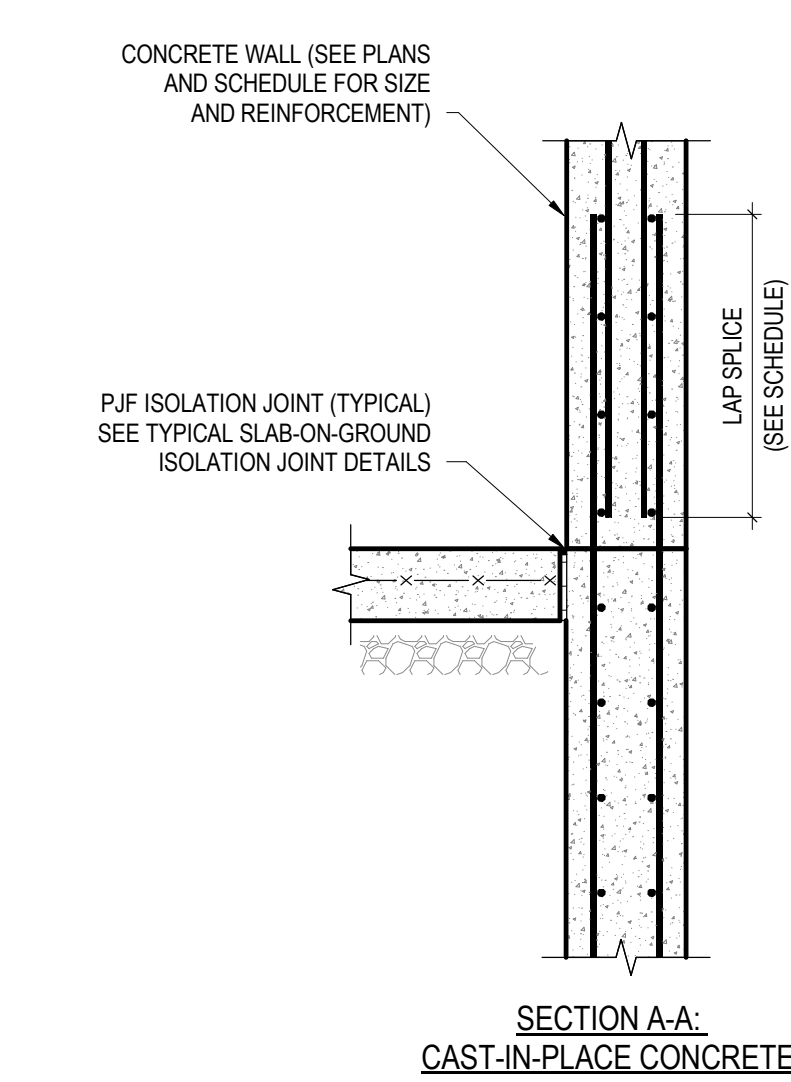
7 FOUNDATION AT CONCRETE WALL
3/4" = 1'-0"



8 WALL FOUNDATION TO COLUMN FOUNDATION TRANSITION
3/4" = 1'-0"



9 ELEVATOR PIT
3/4" = 1'-0"



10 ELEVATOR SUMP PIT
3/4" = 1'-0"

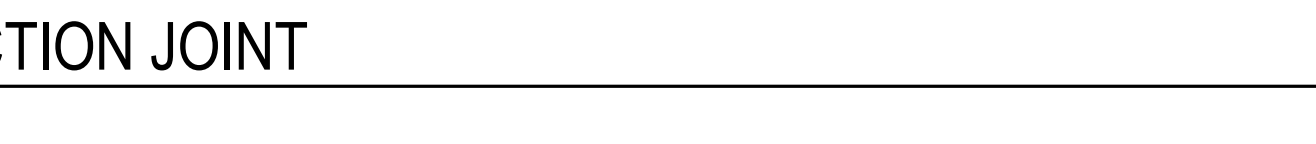
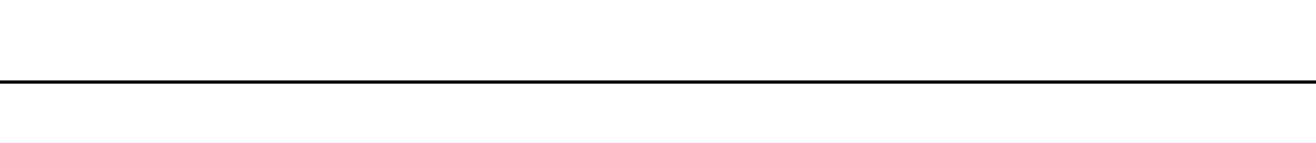
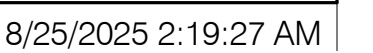
- NOTES:
- GC SHALL CONFIRM ALL DIMENSIONS WITH ELEVATOR MANUFACTURER PRIOR TO CONSTRUCTION.

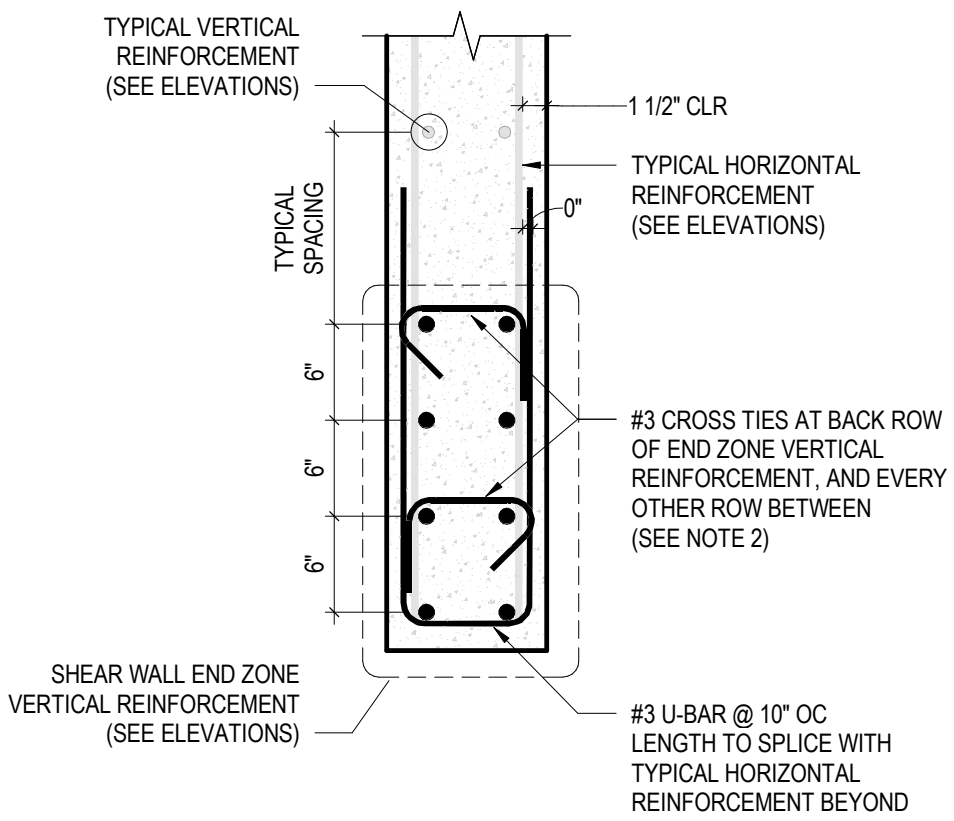
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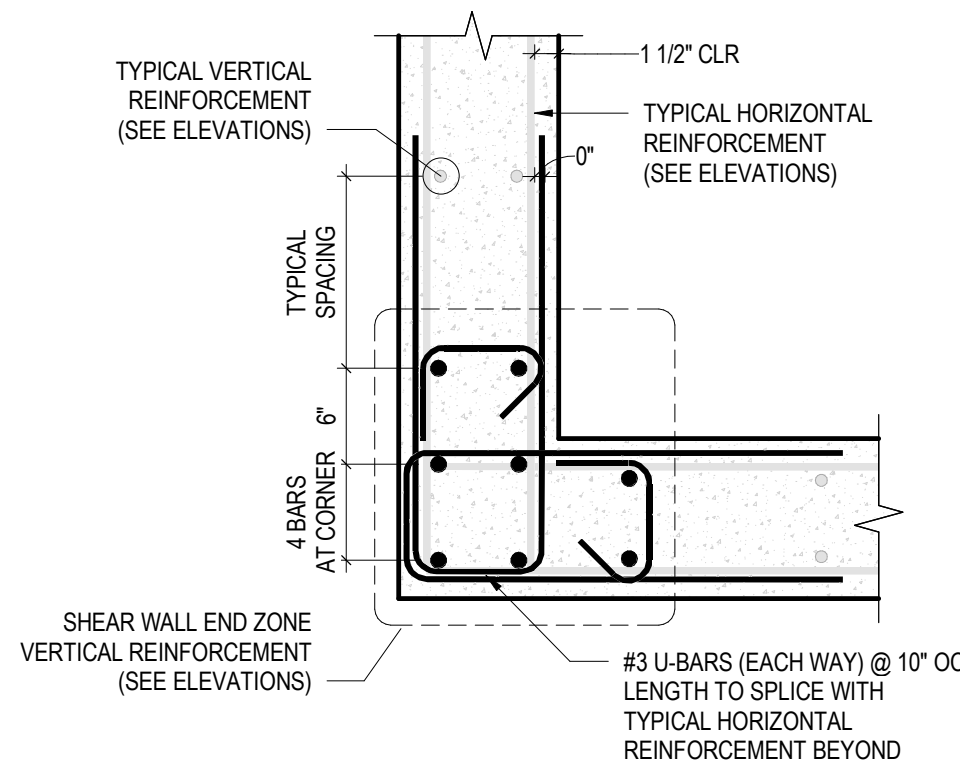
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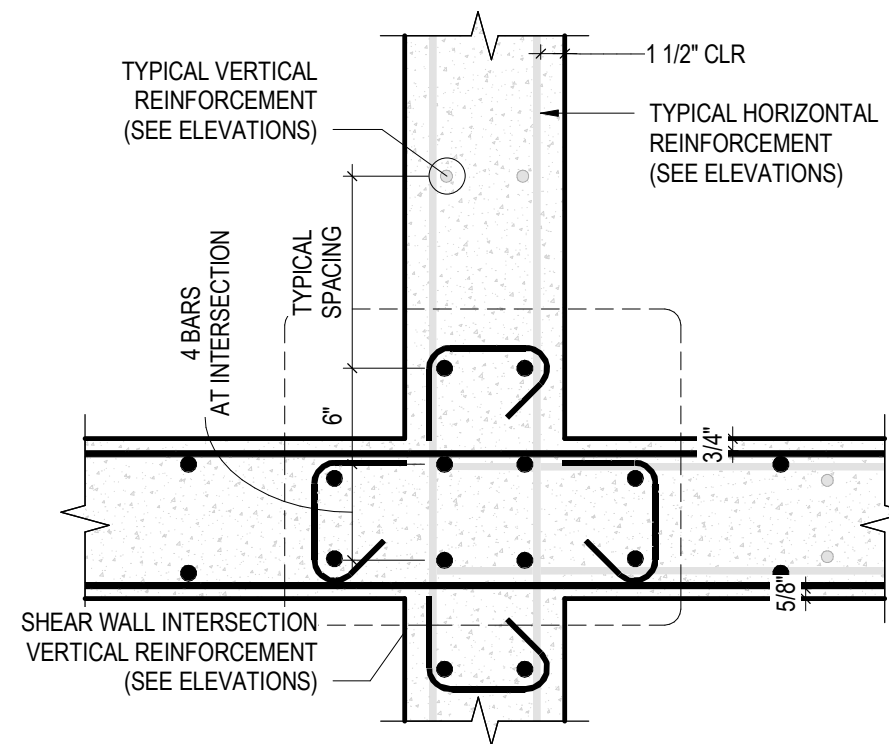




ENDS OF WALLS & AT OPENINGS
(E-TYPE MARK ON ELEVATIONS)



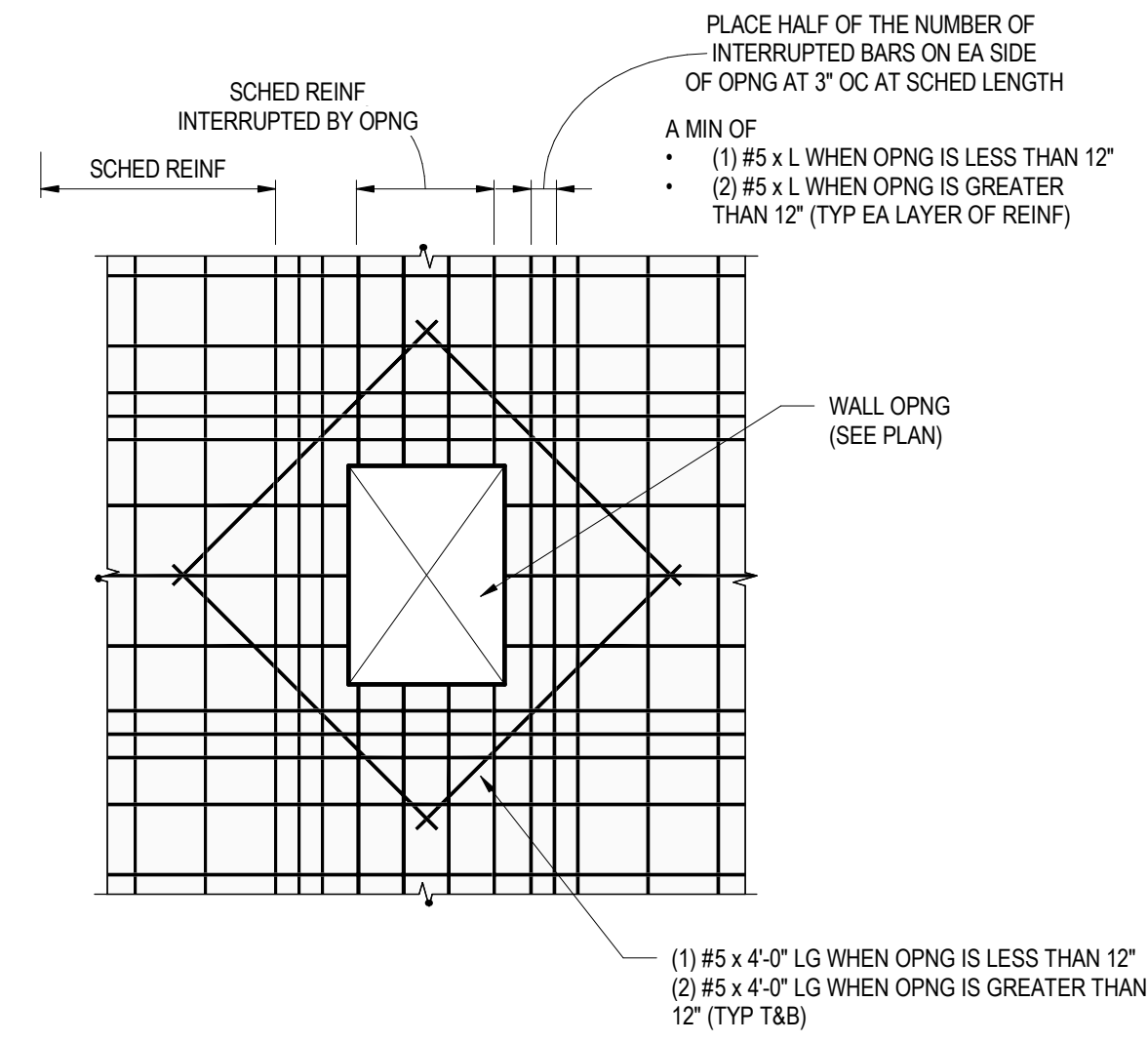
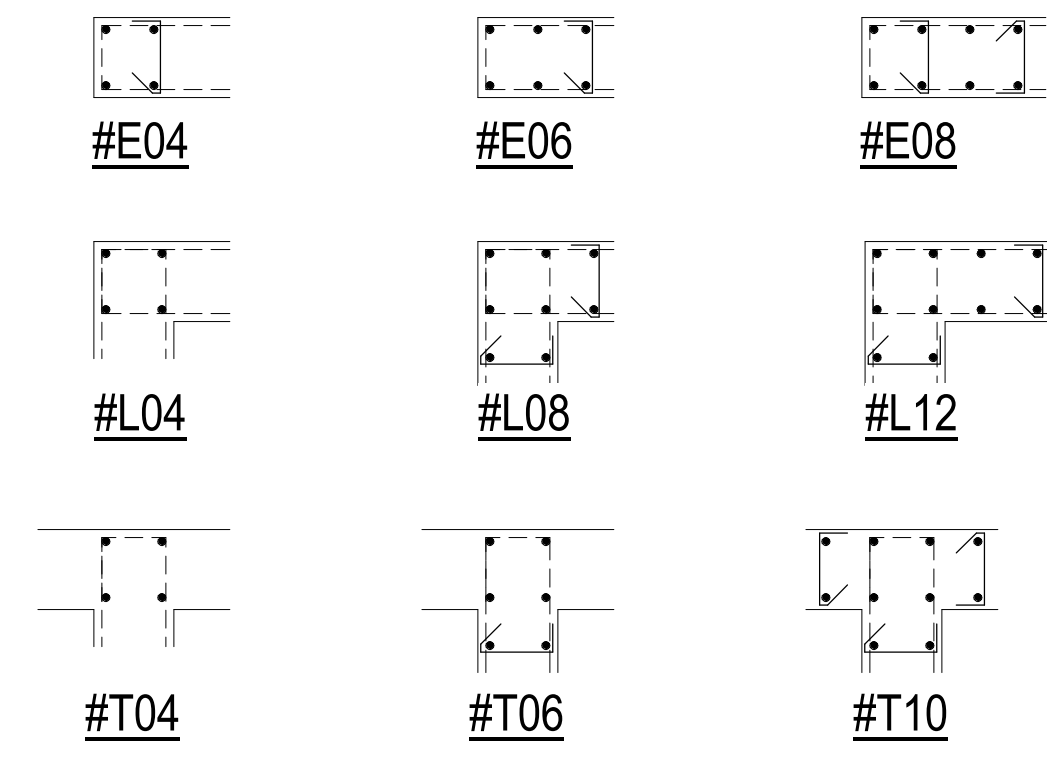
CORNER CONDITION
(L-TYPE MARK ON ELEVATIONS)



INTERSECTION CONDITION
(T-TYPE MARK ON ELEVATIONS)

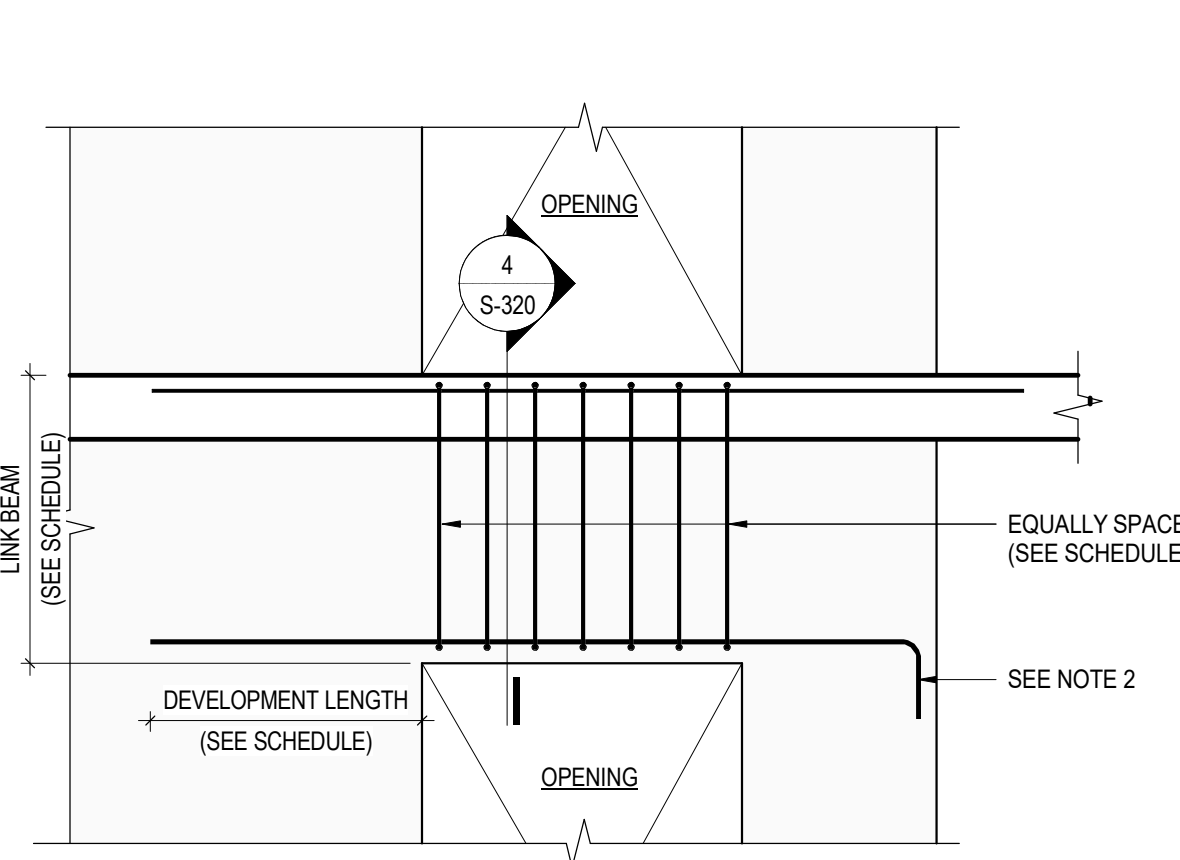
NOTES:

- BAR LAPS SHOWN OFFSET FOR CLARITY.
- HORIZONTAL REINFORCEMENT SHALL BE LAPPED WITH BARS STACKED VERTICALLY.
- VERTICAL REINFORCEMENT SHALL BE LAPPED WITH BARS ALIGNED PARALLEL TO FACE-OF-WALL (OFFSET BENDS SHALL NOT BE PERMITTED).
- CROSS TIE BAR ORIENTATION SHALL BE ALTERNATED AT EACH TIE IN BOTH THE HORIZONTAL AND VERTICAL DIRECTION.
- BAR COUNT INDICATED IN SHEAR WALL VERTICAL REINFORCEMENT SCHEDULE IS THE TOTAL QUANTITY OF BARS REQUIRED, DISTRIBUTED HALF EACH FACE AND THROUGHOUT CORNER/INTERSECTION CONDITIONS AS INDICATED IN DIAGRAMS BELOW.



2 TYPICAL WALL OPENING

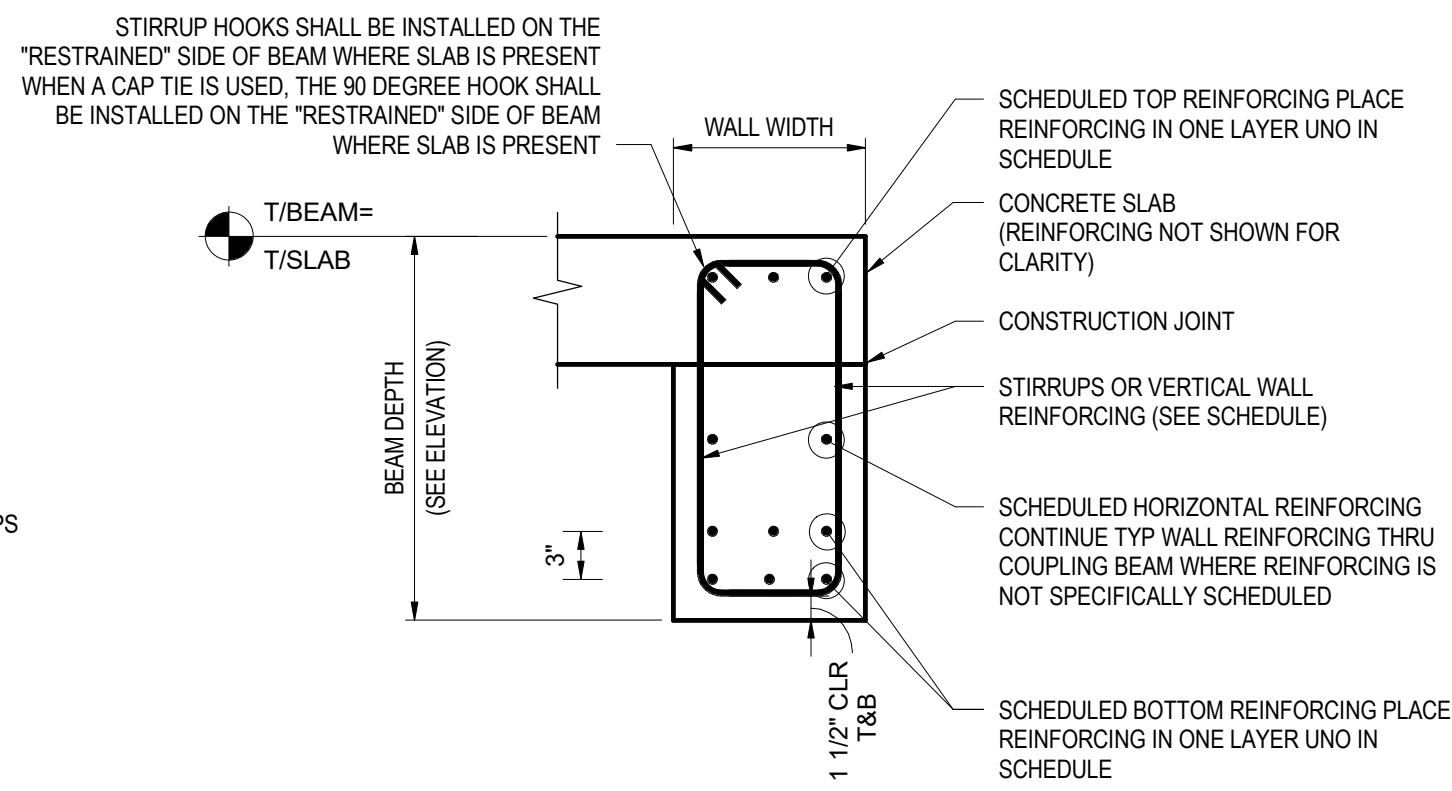
1 SHEAR WALL END ZONE AND CORNER/INTERSECTION REBAR LAYOUTS



NOTES:

- SHEAR WALL REINFORCEMENT IS NOT SHOWN IN THIS ELEVATION FOR CLARITY.
- WHEN LOCATED AT NEAR A CORNER OR END-OF-WALL, LINK BEAM LONGITUDINAL REINFORCEMENT SHALL BE TERMINATED WITH AN ACI STANDARD 90° HOOK. GC OPTION TO USE A HEADED TERMINATOR IN LIEU OF HOOK - SUBMIT PRODUCT DATA FOR SER REVIEW. TOP REINFORCING SHALL CONTINUE INTO SLAB BEYOND WHEN GEOMETRY ALLOWS.

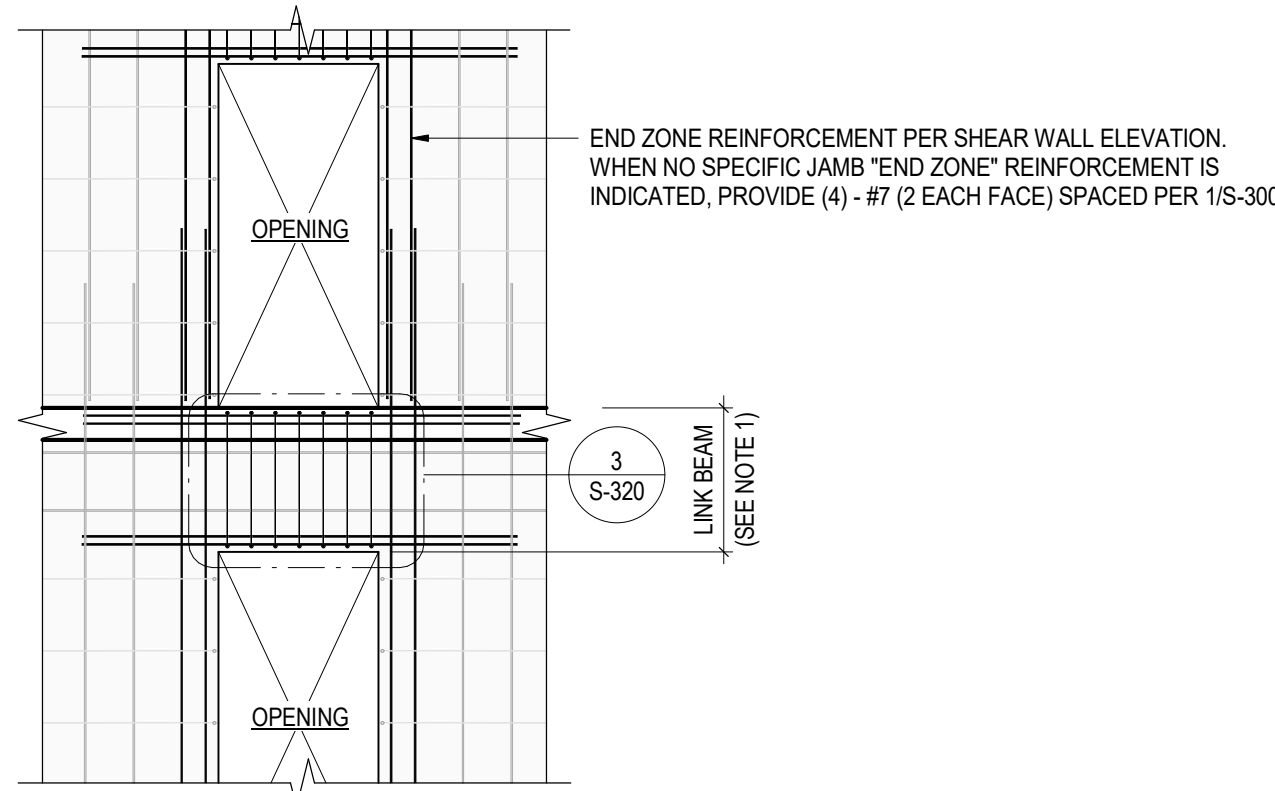
3 SHEAR WALL LINK BEAM ELEVATION



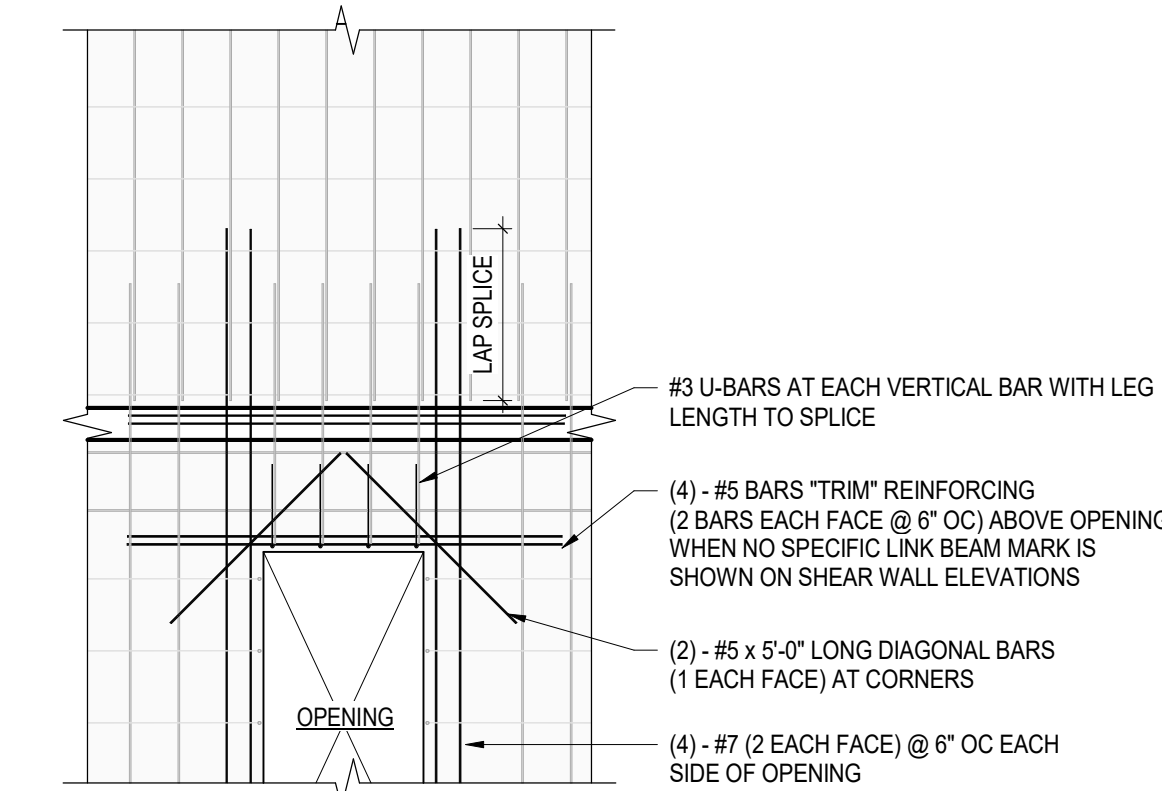
NOTES:

- CLOSED STIRRUP MAY BE EITHER ONE CONTINUOUS BAR WITH A 135 DEGREE HOOK ON EITHER END AROUND THE SAME LONGITUDINAL BAR, OR A U SHAPED STIRRUP WITH 135 DEGREE HOOKS AT EITHER END AND AN OPEN STIRRUP AS A CAP WITH A 135 DEGREE HOOK AT ONE END AND A 90 DEGREE HOOK AT THE OTHER (ACI 318 STANDARD OPTIONS FOR DETAILING OF CLOSED STIRRUP).
- CONCRETE STRENGTH OF THE COUPLING BEAM SHALL MATCH THE SPECIFIED STRENGTH OF THE WALL BELOW, INCLUDING THE SLAB DEPTH.

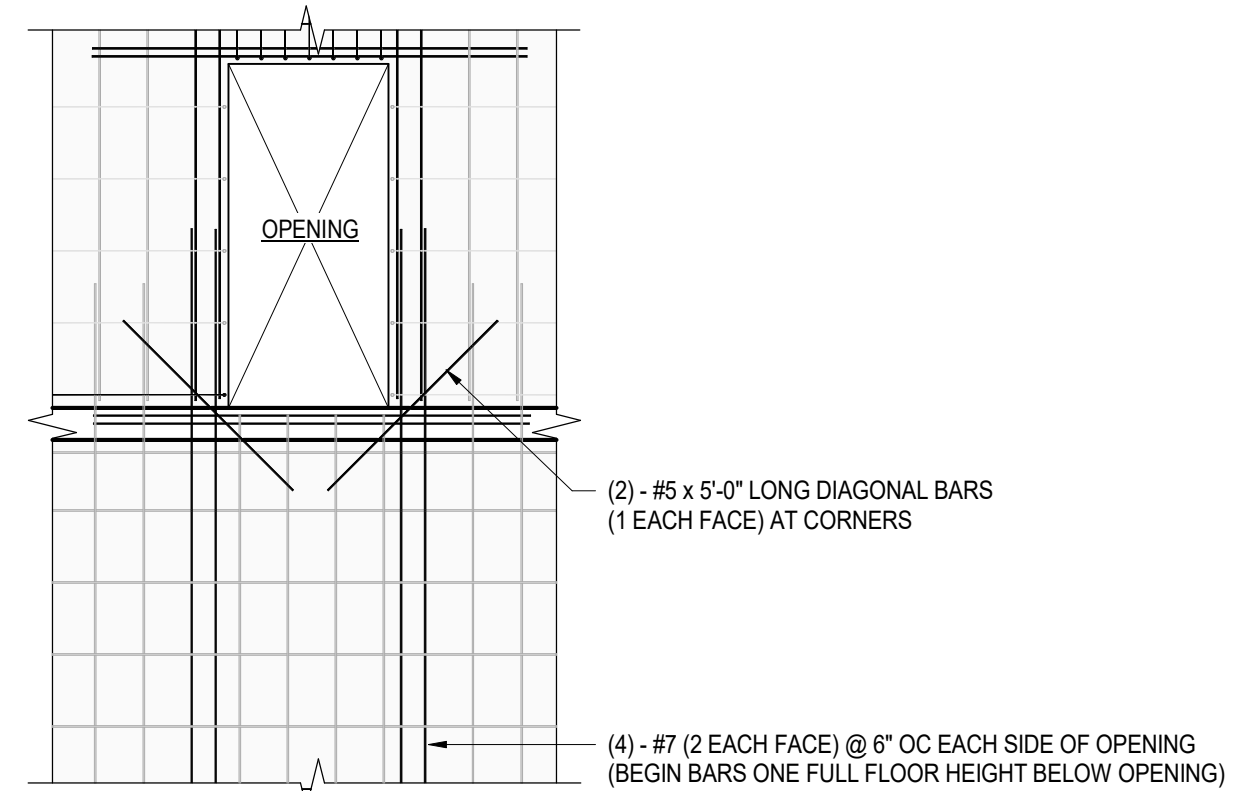
4 LINK BEAM - SECTION



TYPICAL STACKED OPENINGS



DISCONTINUOUS OPENING ALIGNMENT - NO OPENING ABOVE
(UNLESS NOTED OTHERWISE ON SHEAR WALL ELEVATIONS)

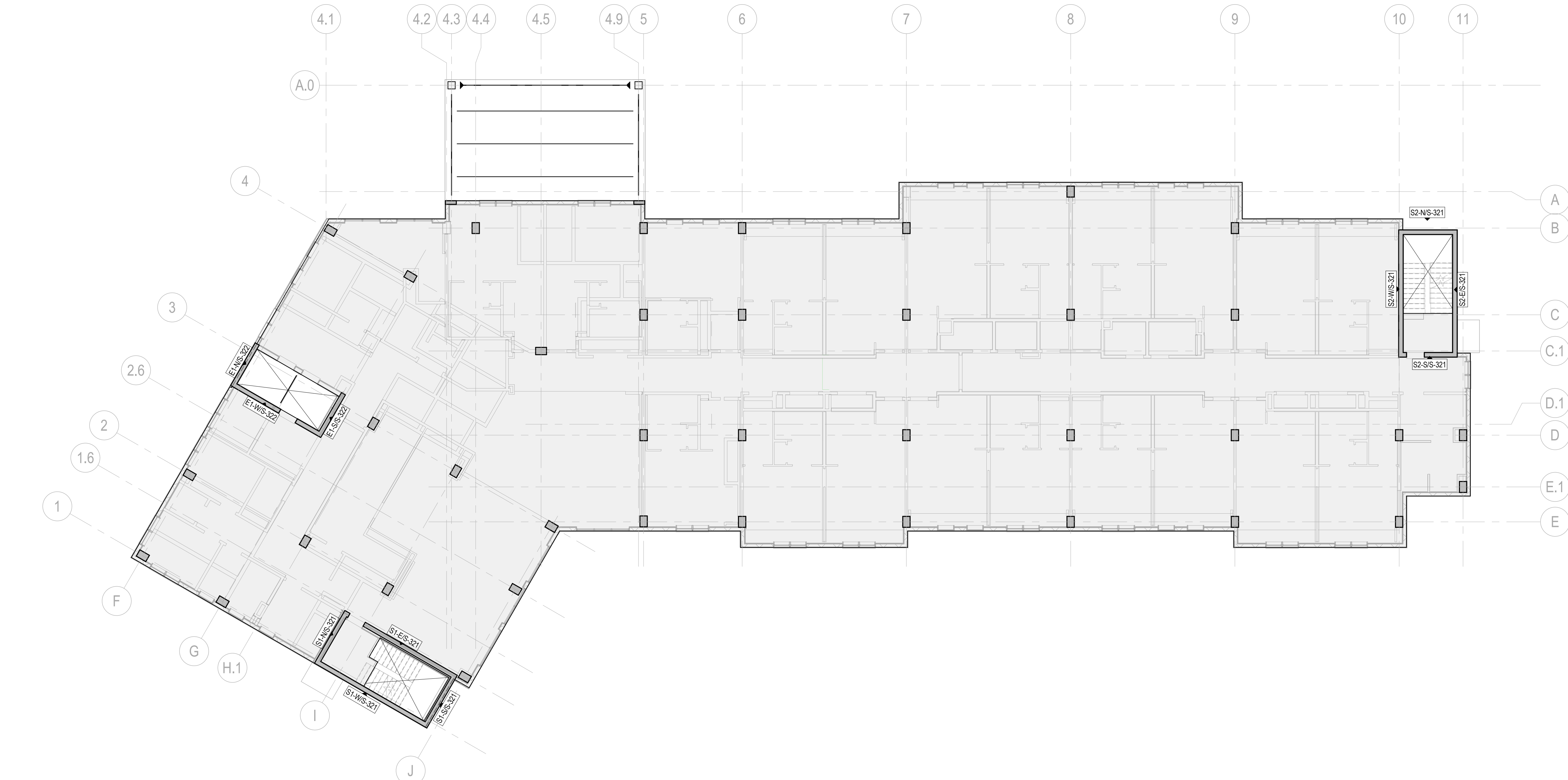


DISCONTINUOUS OPENING ALIGNMENT - NO OPENING BELOW
(UNLESS NOTED OTHERWISE ON SHEAR WALL ELEVATIONS)

NOTES:

- SEE SHEAR WALL ELEVATIONS AND SCHEDULE FOR LINK BEAM REQUIREMENTS. WHEN NO LINK BEAM IS INDICATED, PROVIDE (4) - #7 TOP & BOTTOM BARS WITH #3 DOUBLE U-BAR STIRRUPS WITH SPACING TO MATCH TYPICAL VERTICAL WALL REINFORCEMENT.

5 BAR ARRANGEMENT AT OPENINGS (UNO) - SECTION



SW S-320 SHEAR WALL KEY PLAN

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JG Project #: 21.18.004

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**WATERFORD CAMPUS - ASSISTED
LIVING MEMORY CARE BUILDING**
601 UNIVERSE BLVD JUNO BEACH, FL 33048

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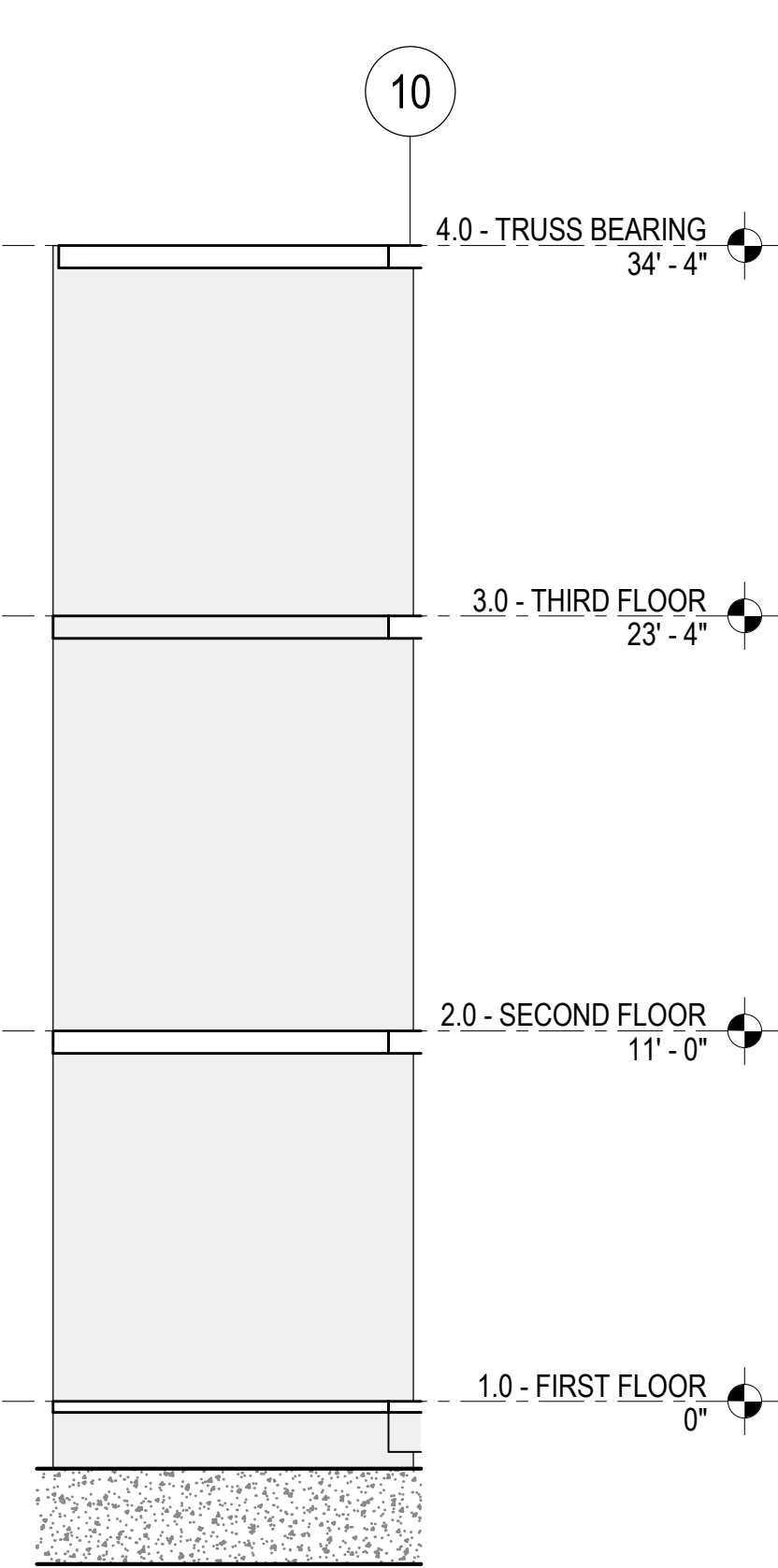
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DEVELOPMENT**

Project No.: 2021009
Date: 08/22/2025

**TYPICAL
CONCRETE
SHEAR WALLS
DETAILS**

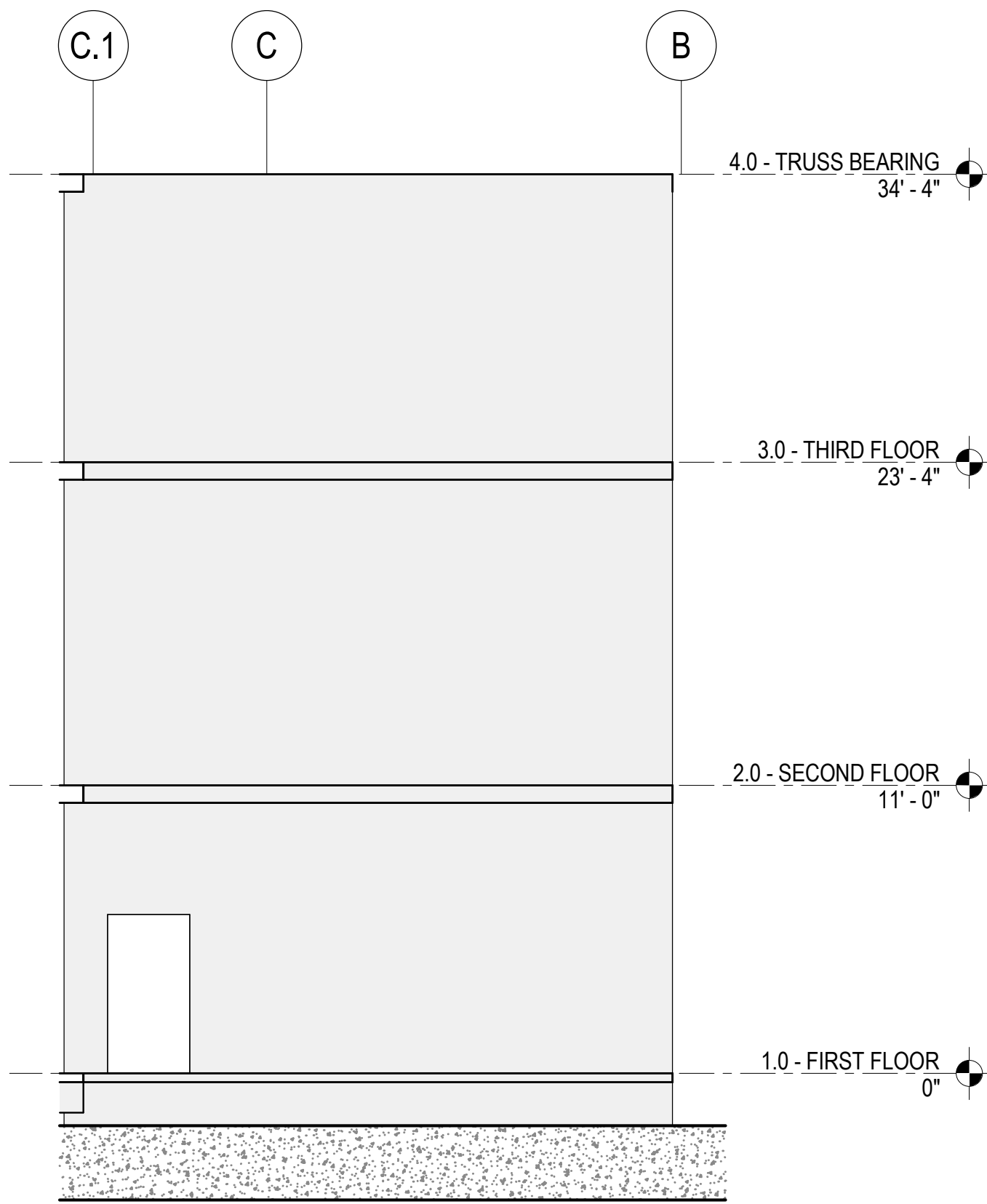
S-320



S2-N
S-321

STAIR 2 NORTH WALL ELEVATION

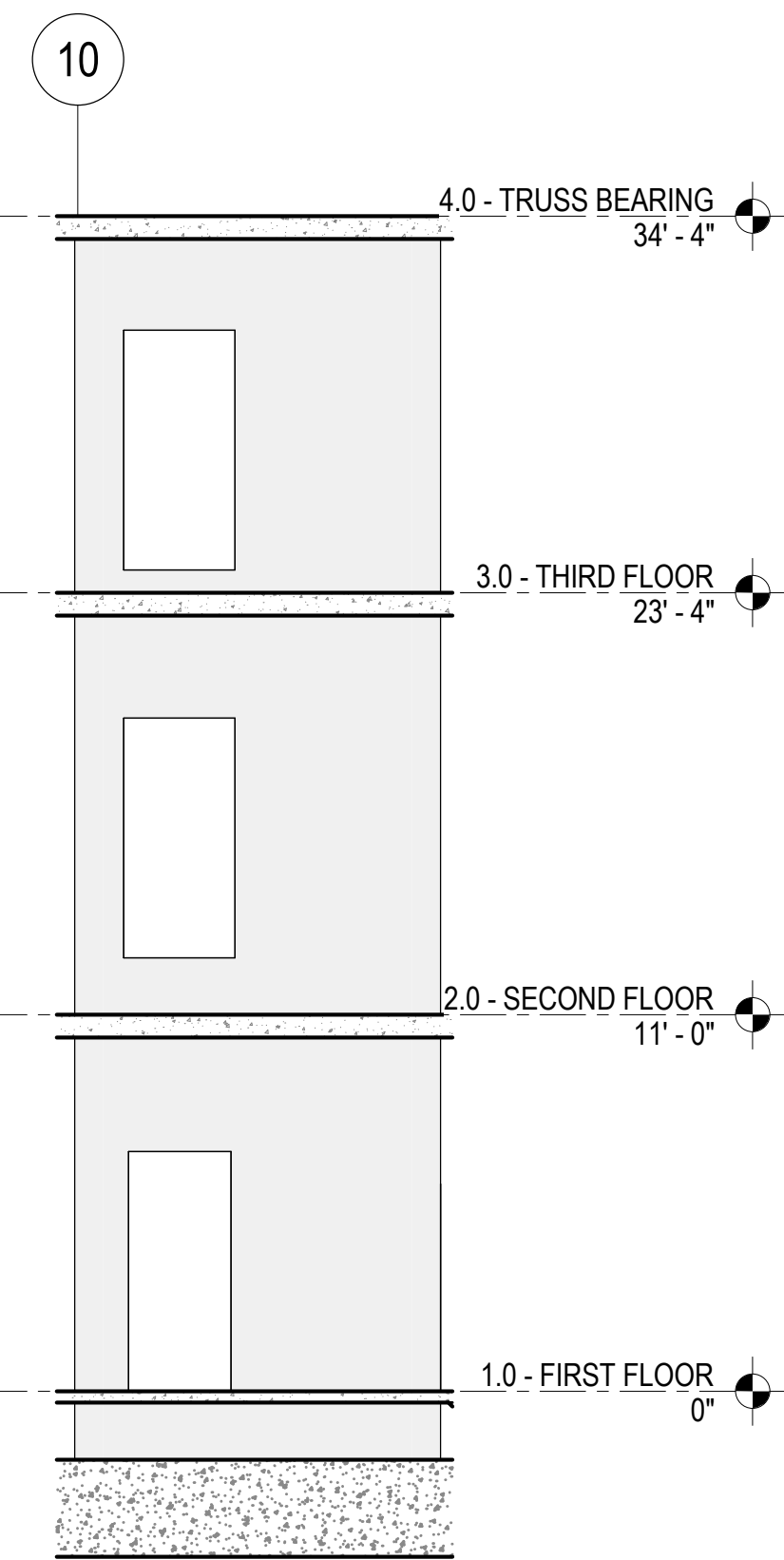
3/16" = 1'-0"



S2-E
S-321

STAIR 2 EAST WALL ELEVATION

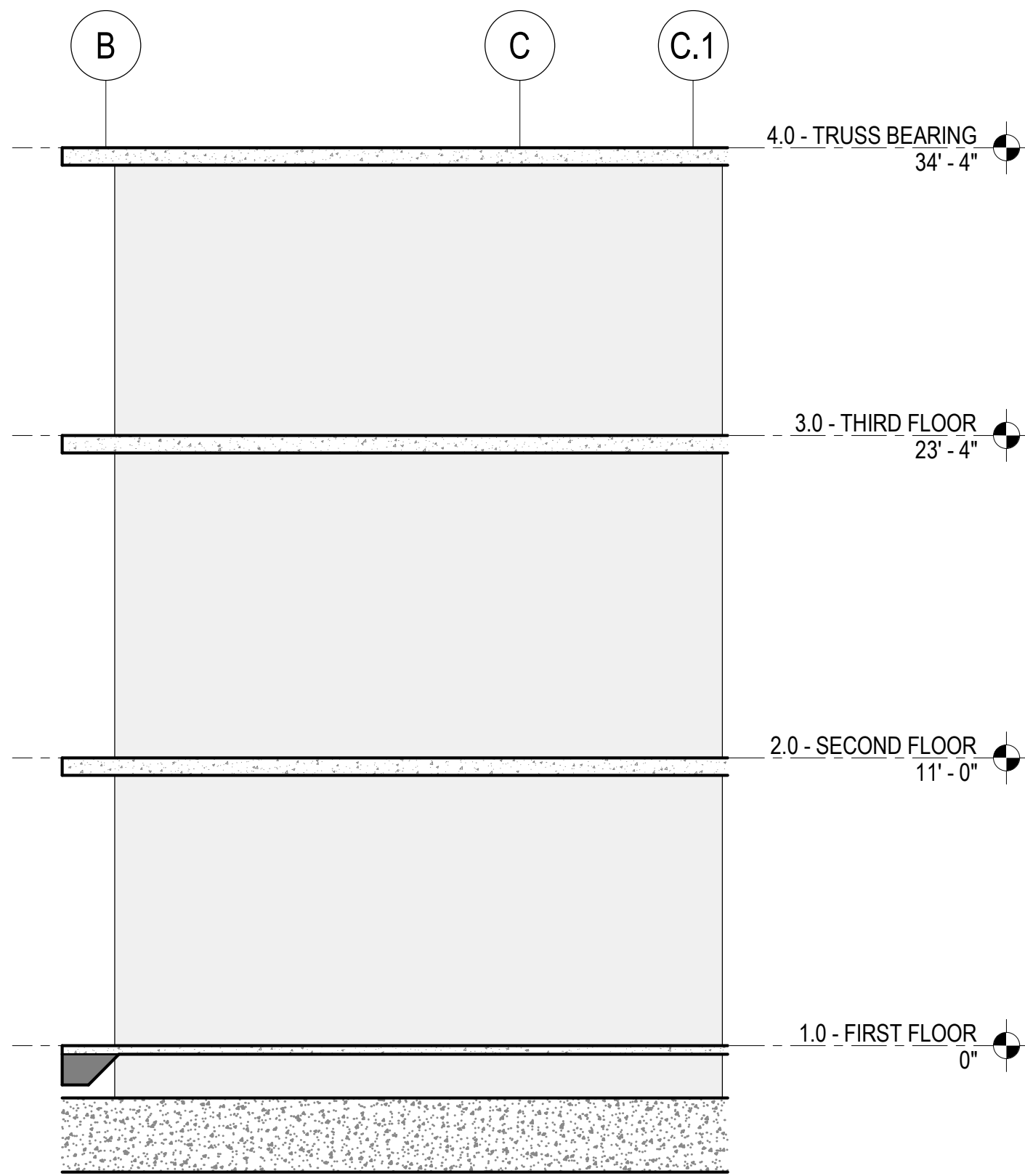
3/16" = 1'-0"



S2-S
S-321

STAIR 2 SOUTH WALL ELEVATION

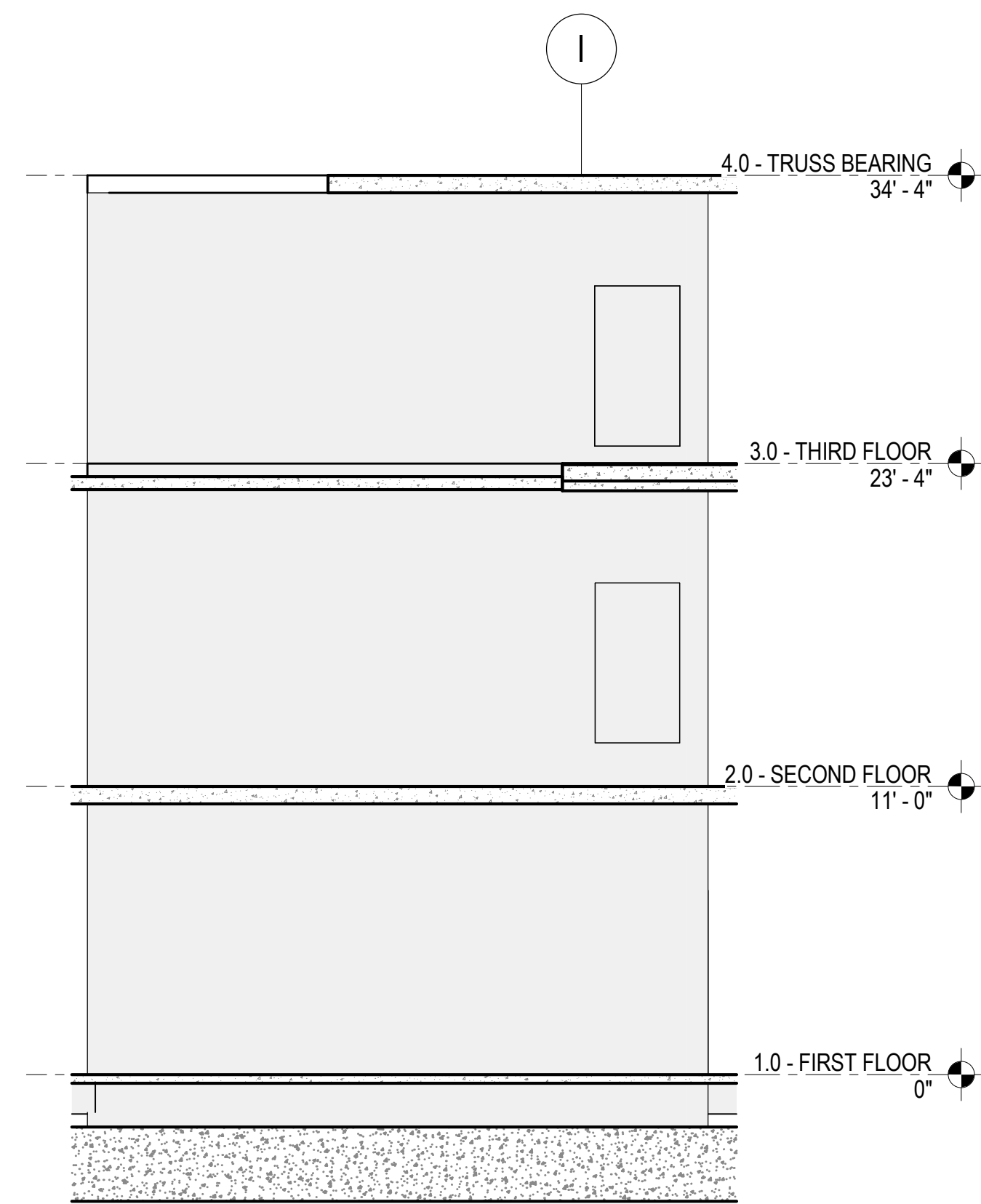
3/16" = 1'-0"



S2-W
S-321

STAIR 2 WEST WALL ELEVATION

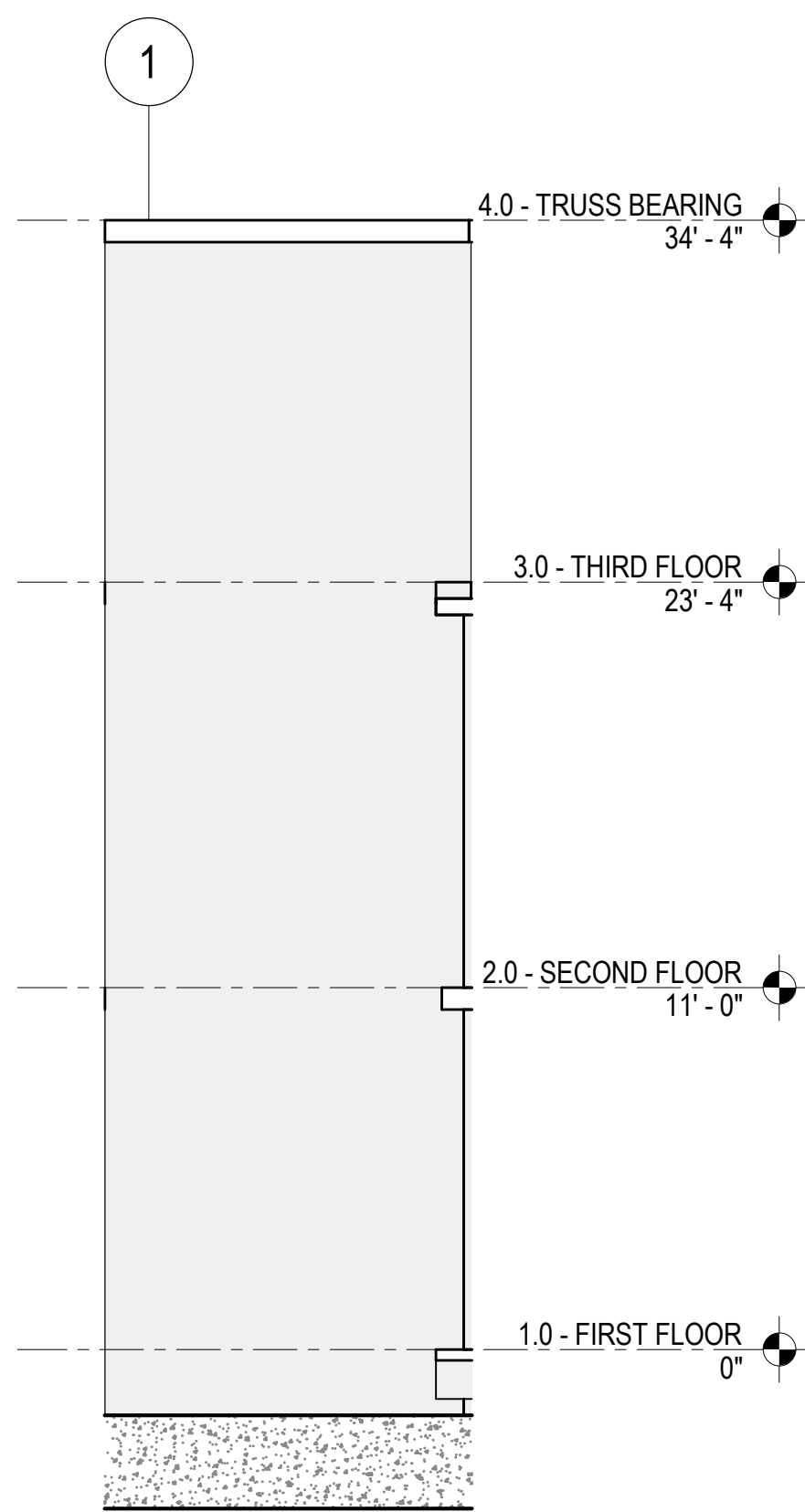
3/16" = 1'-0"



S1-E
S-321

STAIR 1 EAST WALL ELEVATION

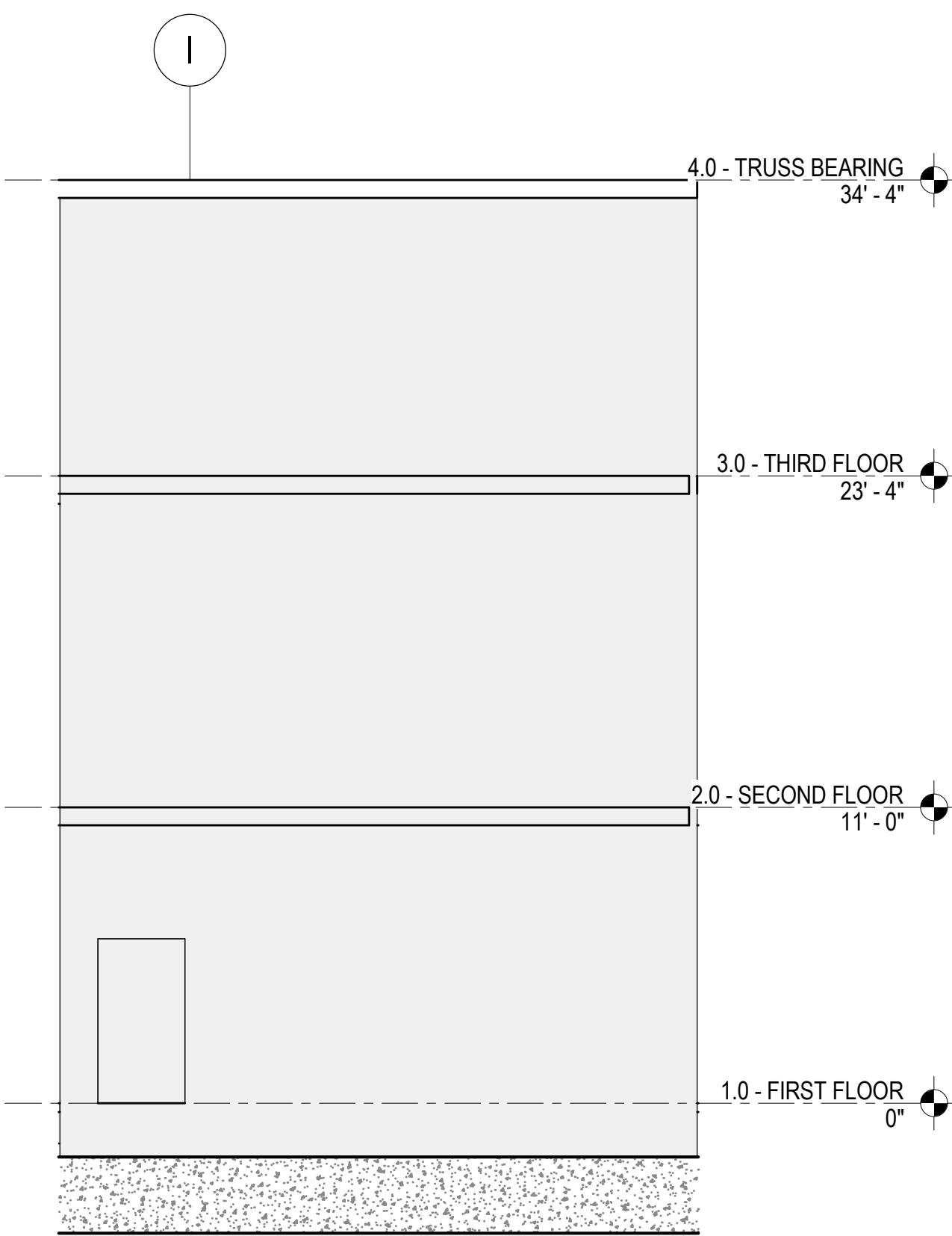
3/16" = 1'-0"



S1-S
S-321

STAIR 1 SOUTH WALL ELEVATION

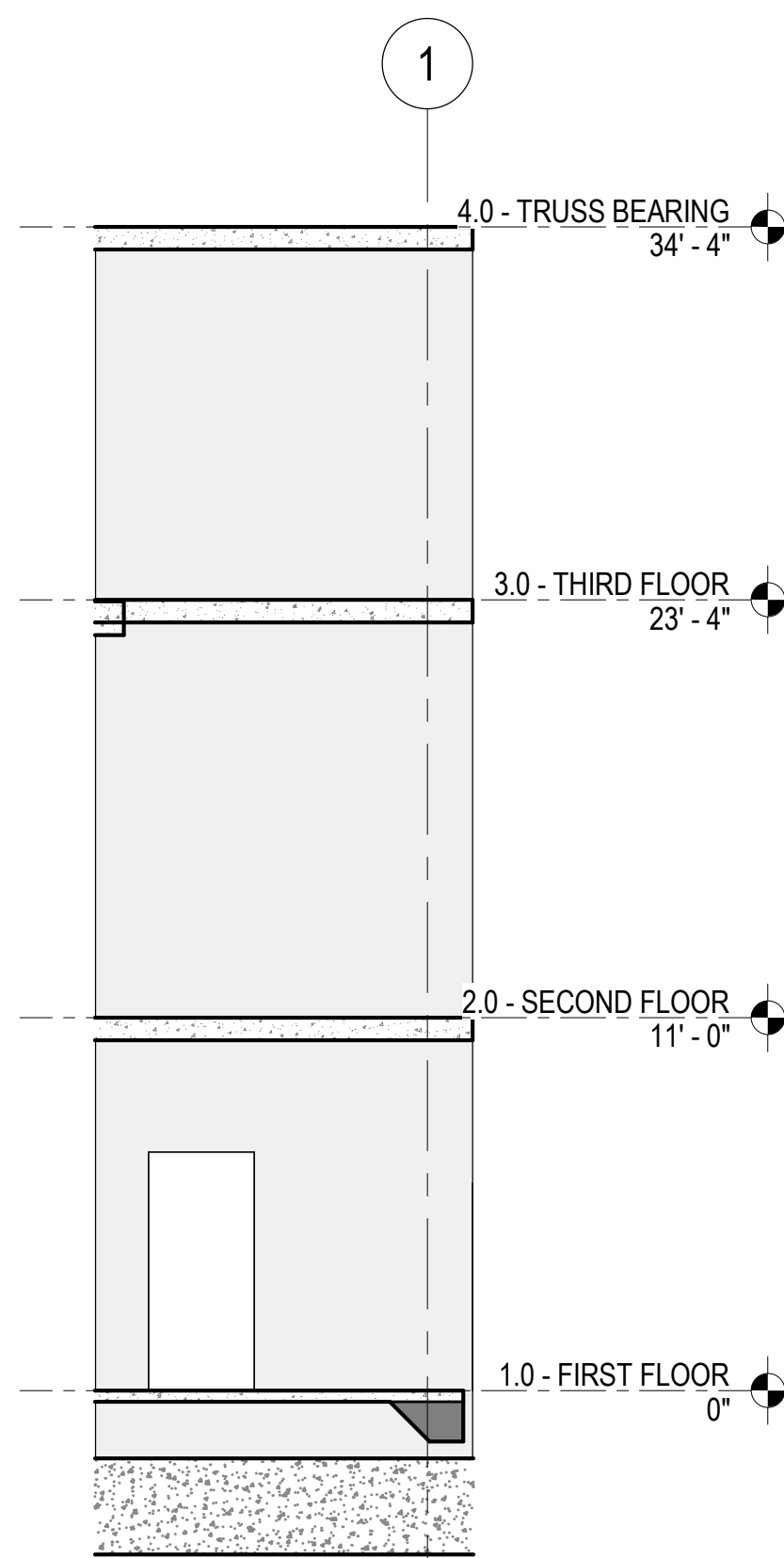
3/16" = 1'-0"



S1-W
S-321

STAIR 1 WEST WALL ELEVATION

3/16" = 1'-0"



S1-N
S-321

STAIR 1 NORTH WALL ELEVATION

3/16" = 1'-0"

- SHEAR WALL ELEVATION NOTES:**
1. DENOTES 10" WALL THICKNESS
DENOTES 12" WALL THICKNESS
 2. SEE ARCHITECTURAL DRAWING FOR ROUGH OPENING DIMENSIONS
 3. SHEAR WALL REINFORCEMENT TAGS:
 - #1## DENOTES TYPICAL VERTICAL REINFORCEMENT (SEE SCHEDULE ON THIS SHEET)
 - #E## DENOTES VERTICAL END ZONE REINFORCEMENT (SEE SCHEDULE ON THIS SHEET)
 - #H## DENOTES TYPICAL HORIZONTAL REINFORCEMENT (SH12 UNLESS NOTED OTHERWISE - (SEE SCHEDULE ON THIS SHEET))
 - C#H# DENOTES COUPLING BEA, MARK (SEE SCHEDULE ON THIS SHEET)
 4. WHERE NOT SPECICALLU NOTED WITH #E## OR C#H# REINFORCING, PROVIDE (4) #7 BARS AT ALL EDGES OF OPENINGS AND ENDS OF WALLS WITH (2) BARS EACH FACE SPACED AT 6" OC. BARS SHALL BE FULLY DEVELOPED FROM THE EDGE OF THE OPENING AND SHALL BE HOOKED AT WALL ENDS WHERE REQUIRED.
 5. ALL BARS SHALL BE FULLY LAPPED, INCLUDING DOWELS INTO FOUNDATION ELEMENTS.
 6. SEE DETAILS ON S-330.
 7. GC SHALL VERIFY ALL DIMENSIONS AND LOCATIONS OF WALLS AND OPENINGS WITH ARCHITECTURAL DRAWINGS. NOTIFY DESIGN TEA, OF DISCREPANCIES.

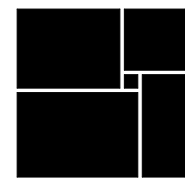
TYPICAL VERTICAL REINFORCING			
MARK	BAR SIZE	BAR SPACING	COMMENTS

VERTICAL CORNER REINFORCING			
MARK	BAR SIZE	# BARS (1/2 EF)	COMMENTS

VERTICAL END REINFORCING			
MARK	BAR SIZE	# BARS (1/2 EF)	COMMENTS

10" CONCRETE SHEAR WALLS
FOR DD PRICING PURPOSES, ASSUME THE
FOLLOWING MATERIAL QUANTITIES:
• 3.5 PSF CONVENTIONAL REBAR

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Project No.: 2021009
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CONCRETE
SHEAR WALL
ELEVATIONS

S-321

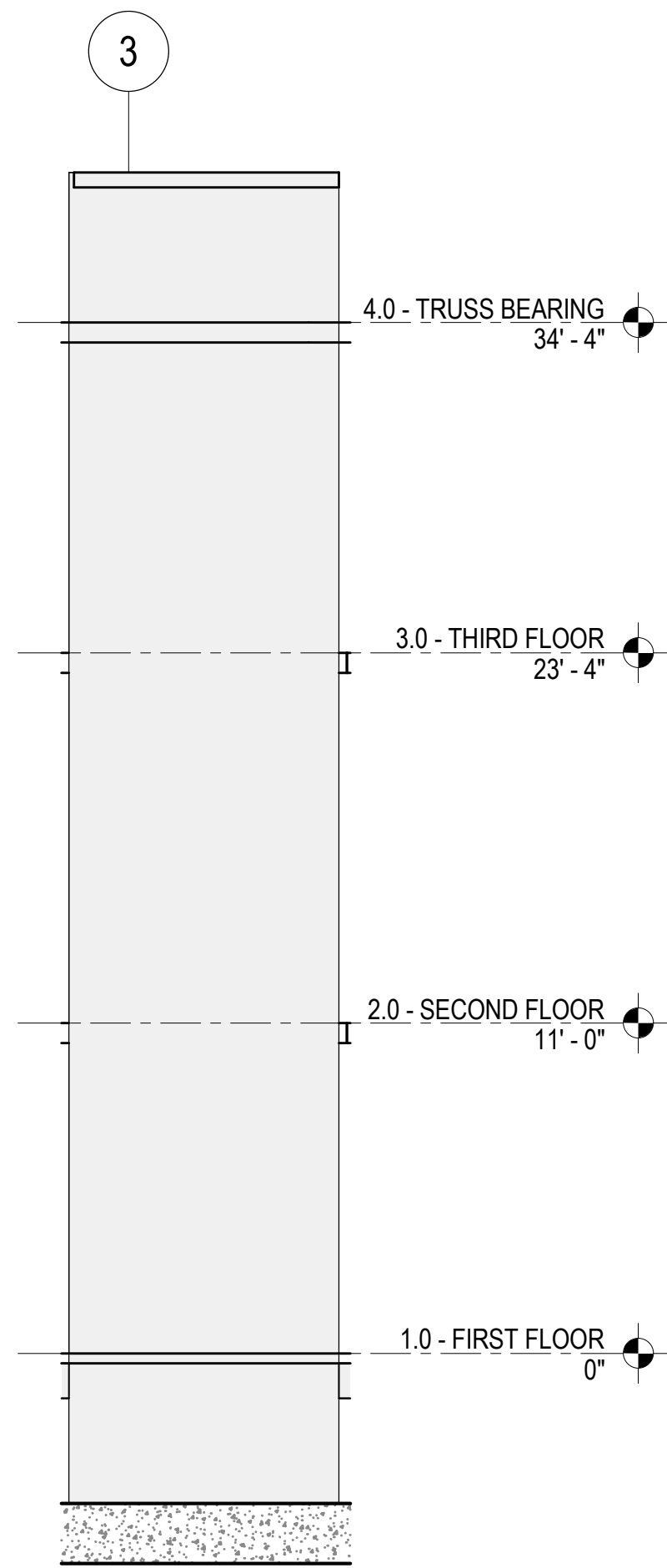
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JG Project #: 21.18.004

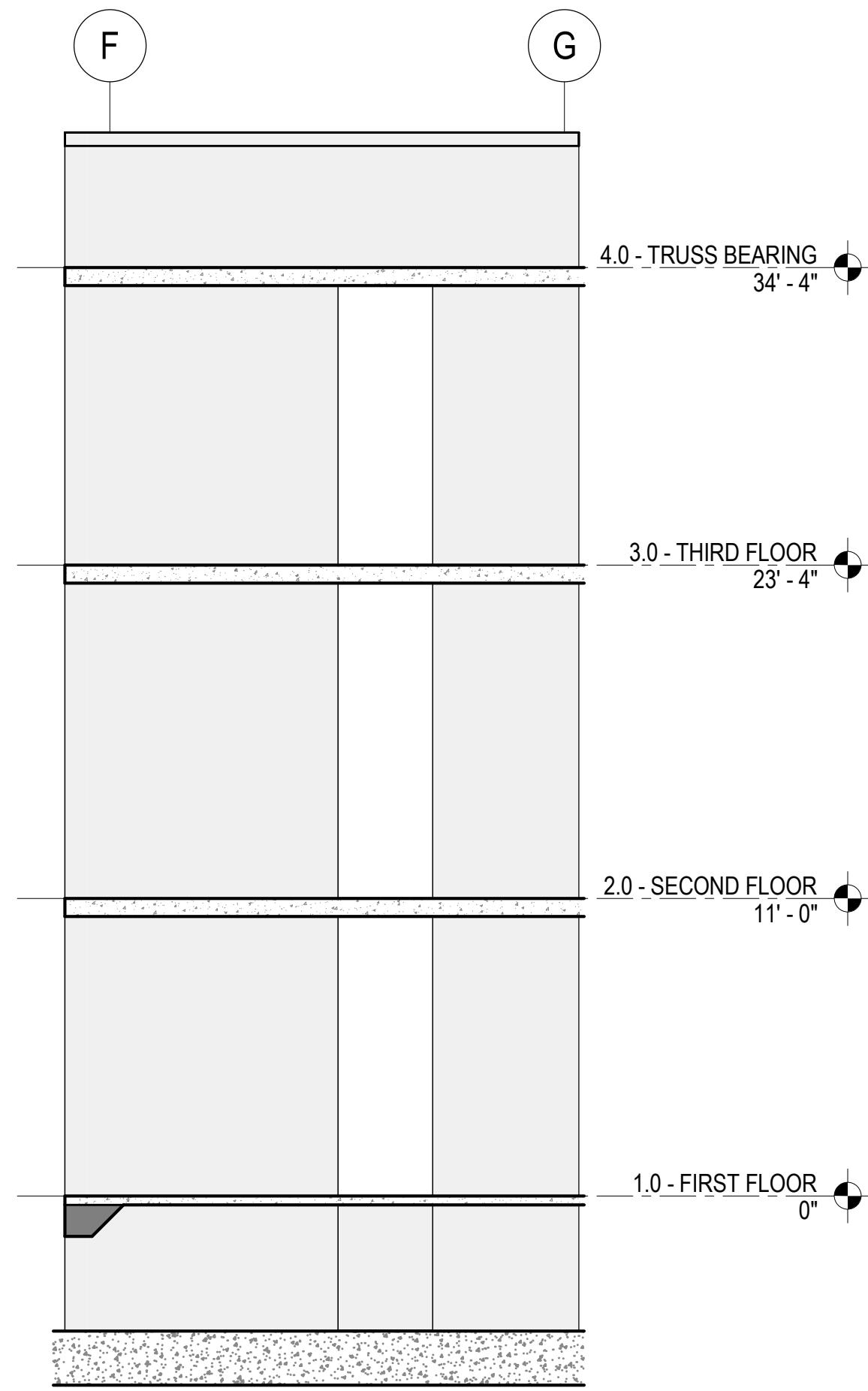
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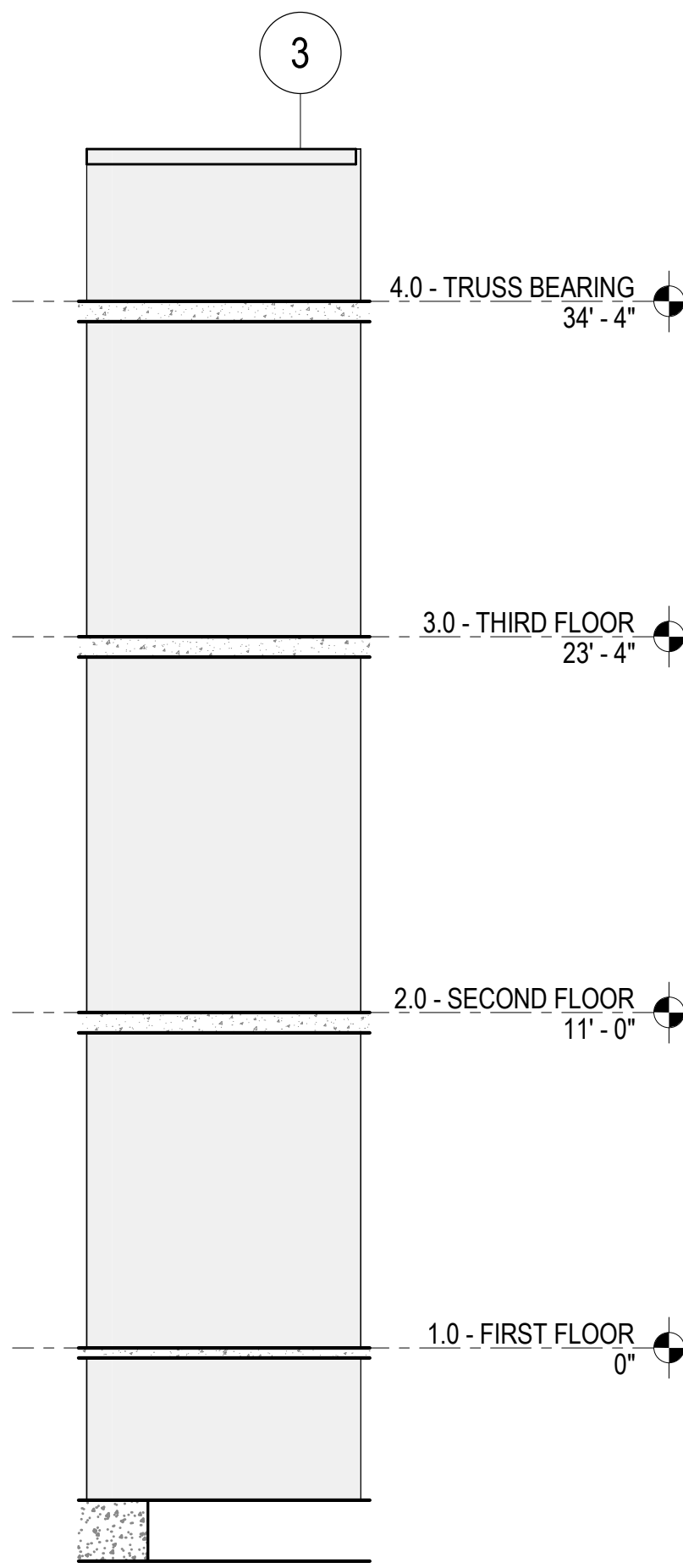
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E1-N
S-322
ELEVATOR 1 NORTH WALL ELEVATION
3/16" = 1'-0"



E1-W
S-322
ELEVATOR 1 WEST WALL ELEVATION
3/16" = 1'-0"



E1-S
S-322
ELEVATOR 2 SOUTH WALL ELEVATION
3/16" = 1'-0"

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 - CBW# DENOTES COUPLING BEA, MARK (SEE SCHEDULE ON THIS SHEET)
 4. WHERE NOT SPECICALLU NOTED WITH #EV# OR CBW# REINFORCING, PROVIDE (4) #7 BARS AT ALL EDGES OF OPENINGS AND ENDS OF WALLS WITH (2) BARS EACH FACE SPACED AT 6" OC. BARS SHALL BE FULLY DEVELOPED FROM THE EDGE OF THE OPENING AND SHALL BE HOOKED AT WALL ENDS WHERE REQUIRED.
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 6. SEE DETAILS ON S-330.
 7. GC SHALL VERIFY ALL DIMENSIONS AND LOCATIONS OF WALLS AND OPENINGS WITH ARCHITECTURAL DRAWINGS. NOTIFY DESIGN TEA, OF DISCREPANCIES.

TYPICAL VERTICAL REINFORCING			
MARK	BAR SIZE	BAR SPACING	COMMENTS

VERTICAL CORNER REINFORCING			
MARK	BAR SIZE	# BARS (1/2 EF)	COMMENTS

VERTICAL END REINFORCING			
MARK	BAR SIZE	# BARS (1/2 EF)	COMMENTS

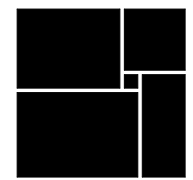
10' CONCRETE SHEAR WALLS
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• 3.5 PSF CONVENTIONAL REBAR



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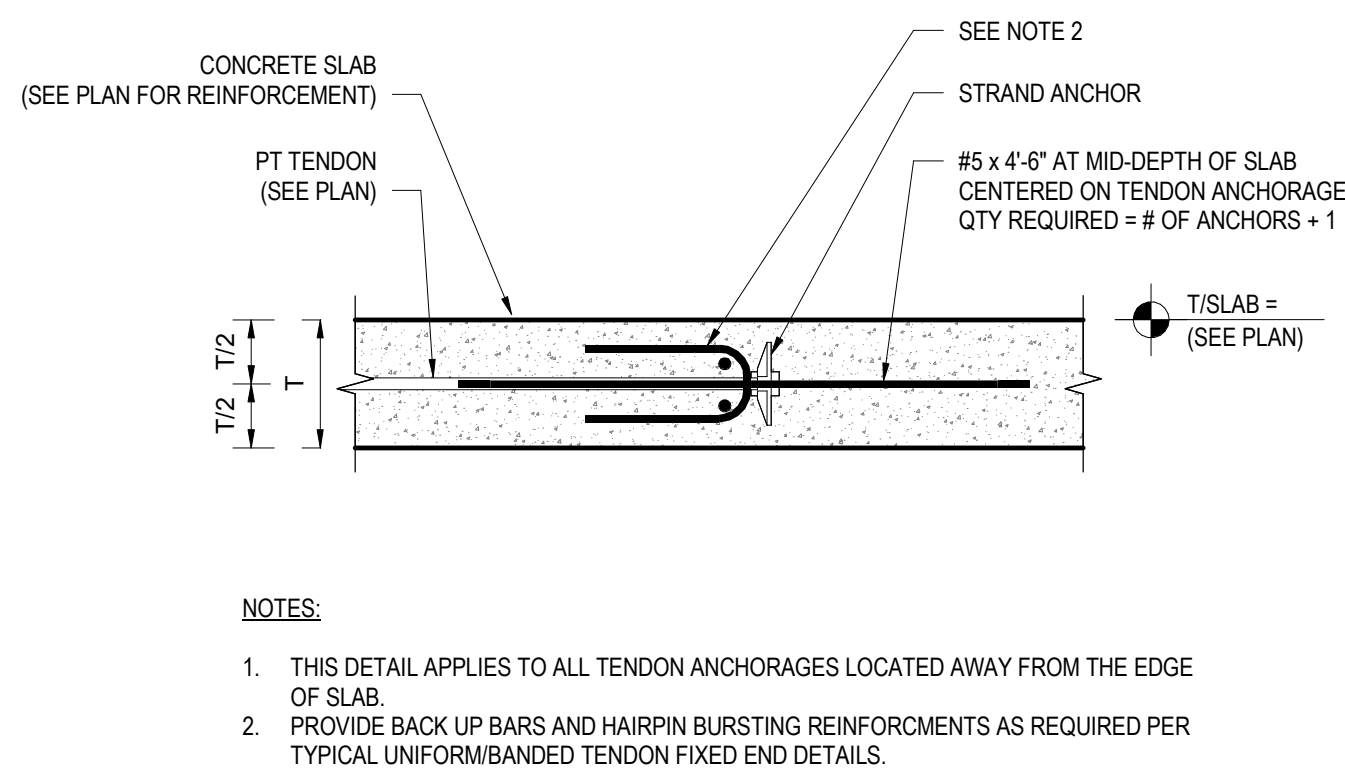
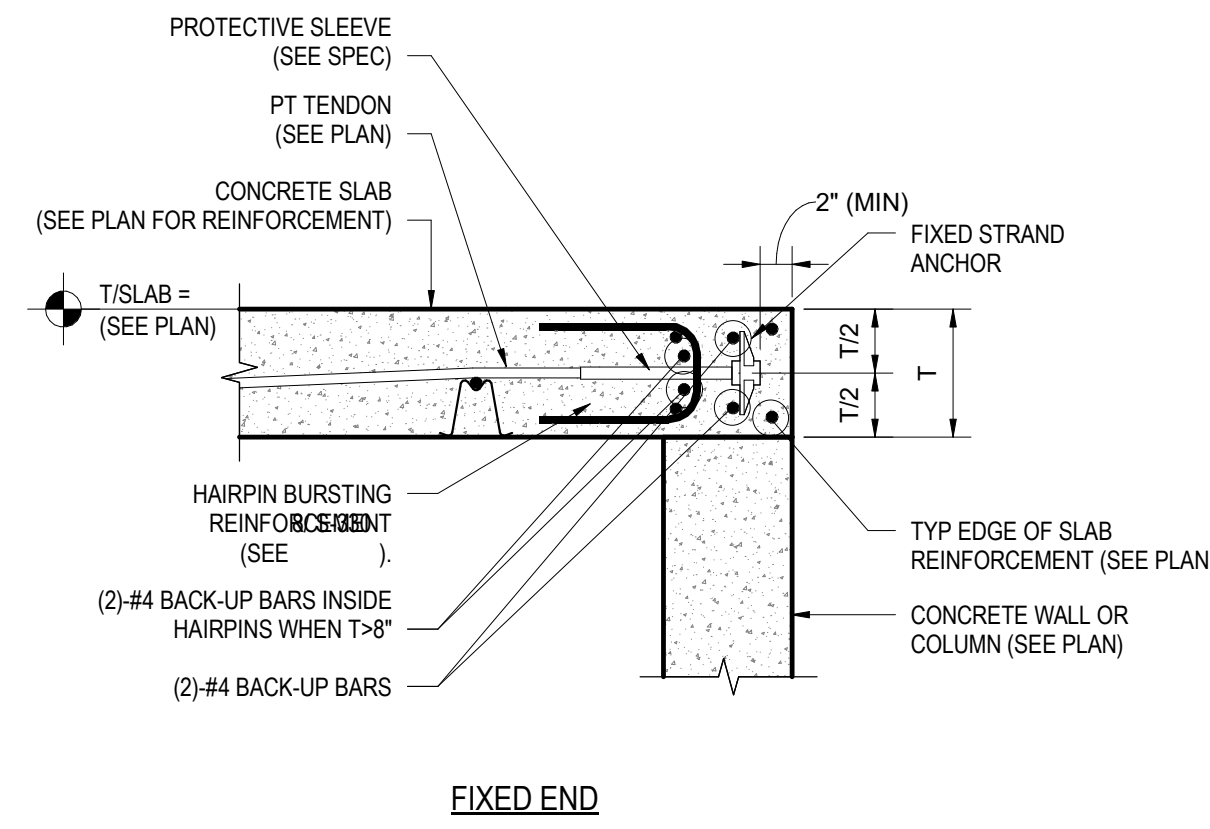
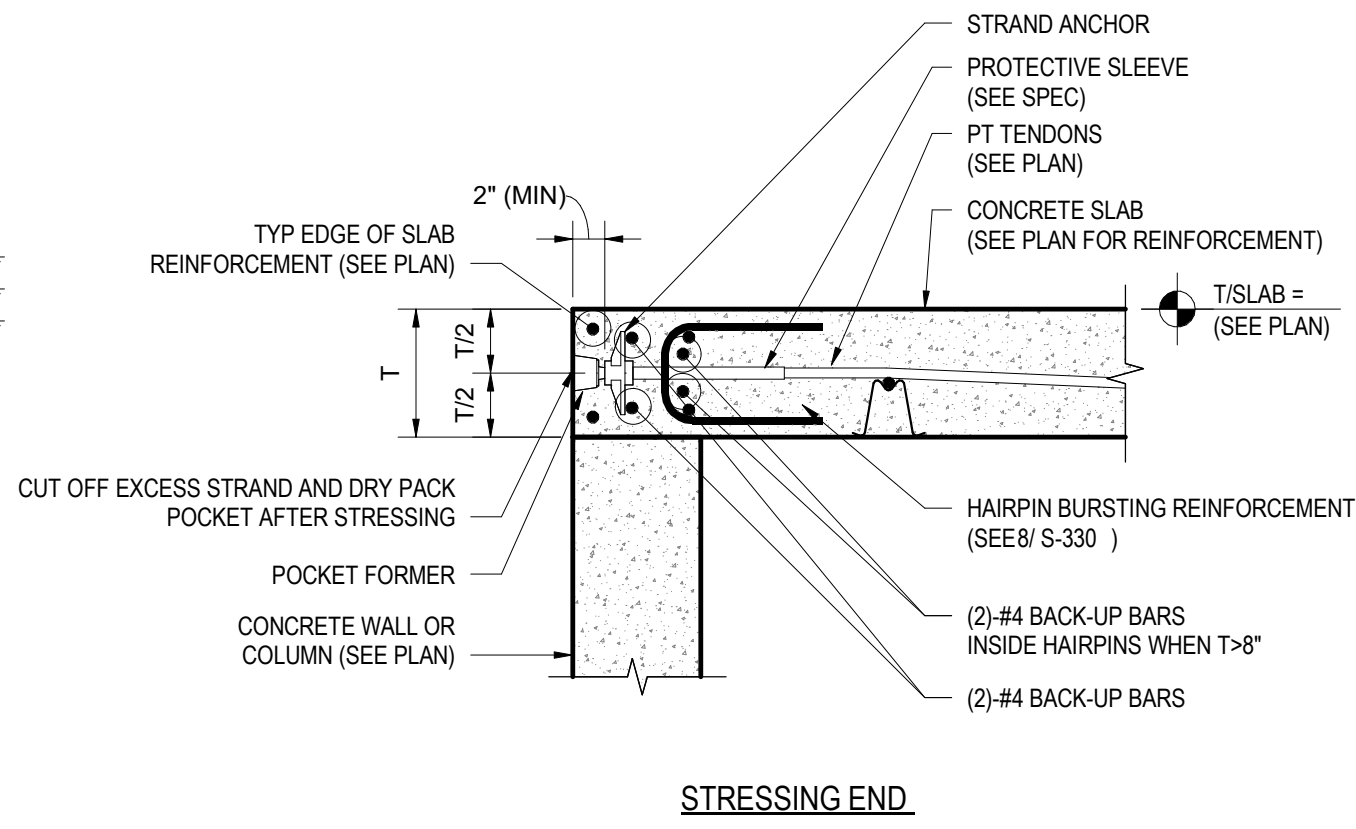
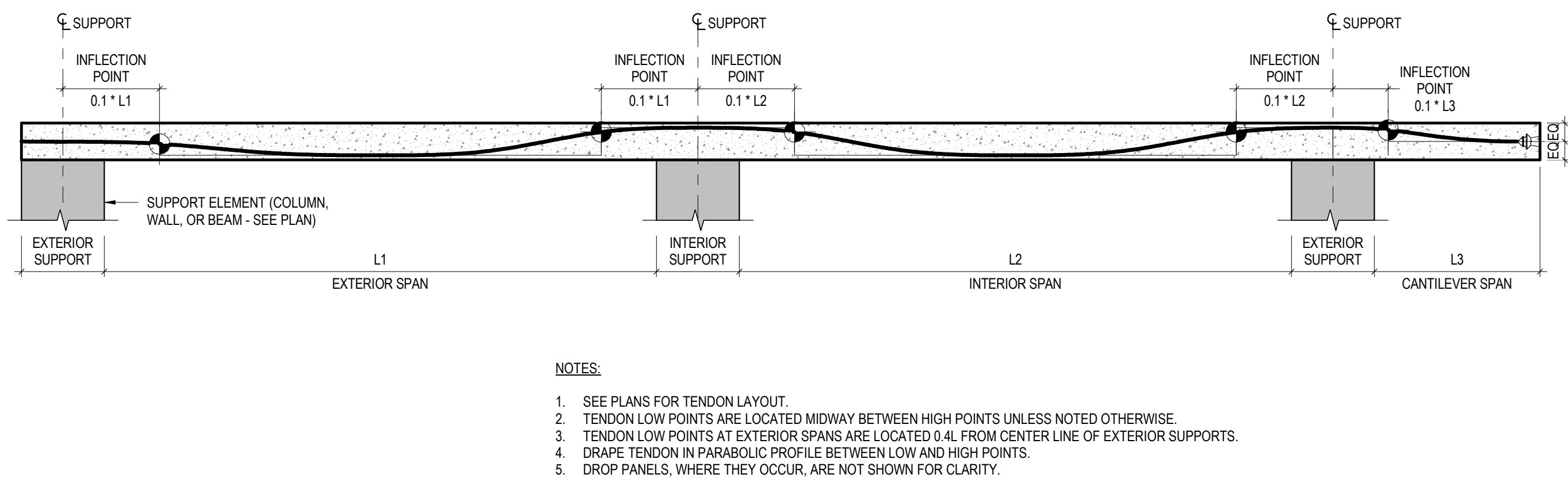
Project No.: 2021009
Date: 08/22/2025

CONCRETE
SHEAR WALL
ELEVATIONS
Copy 1

S-322

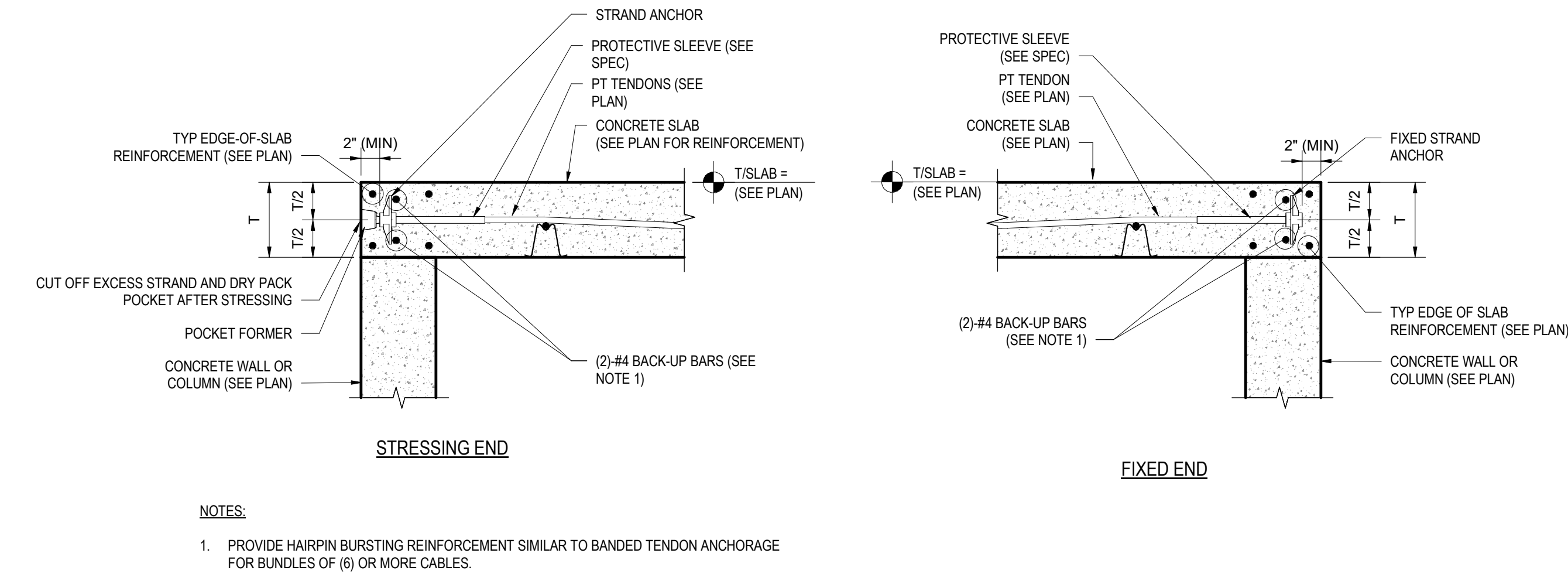
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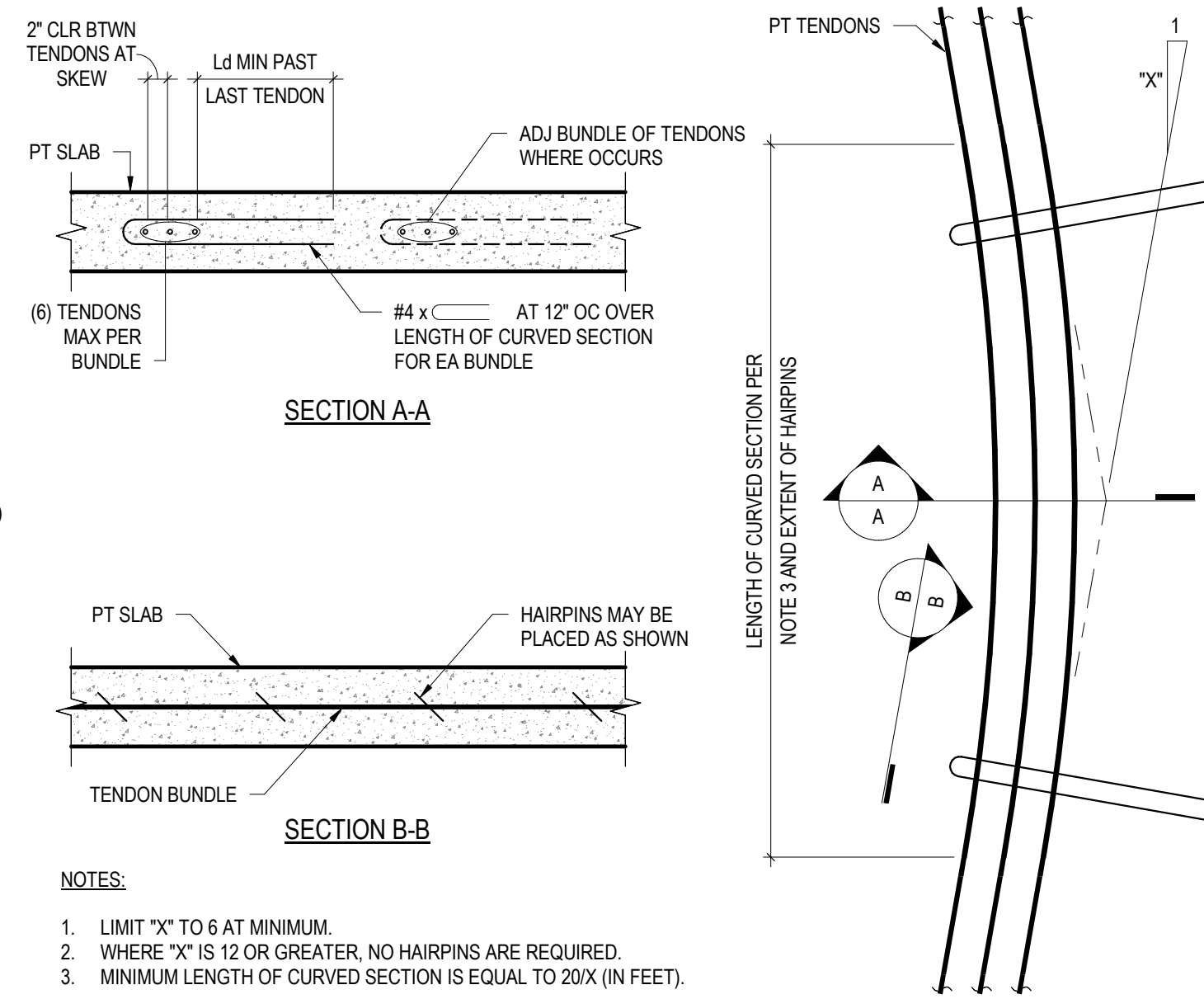
1 TYPICAL TENDON PROFILE

1/2" = 1'-0"



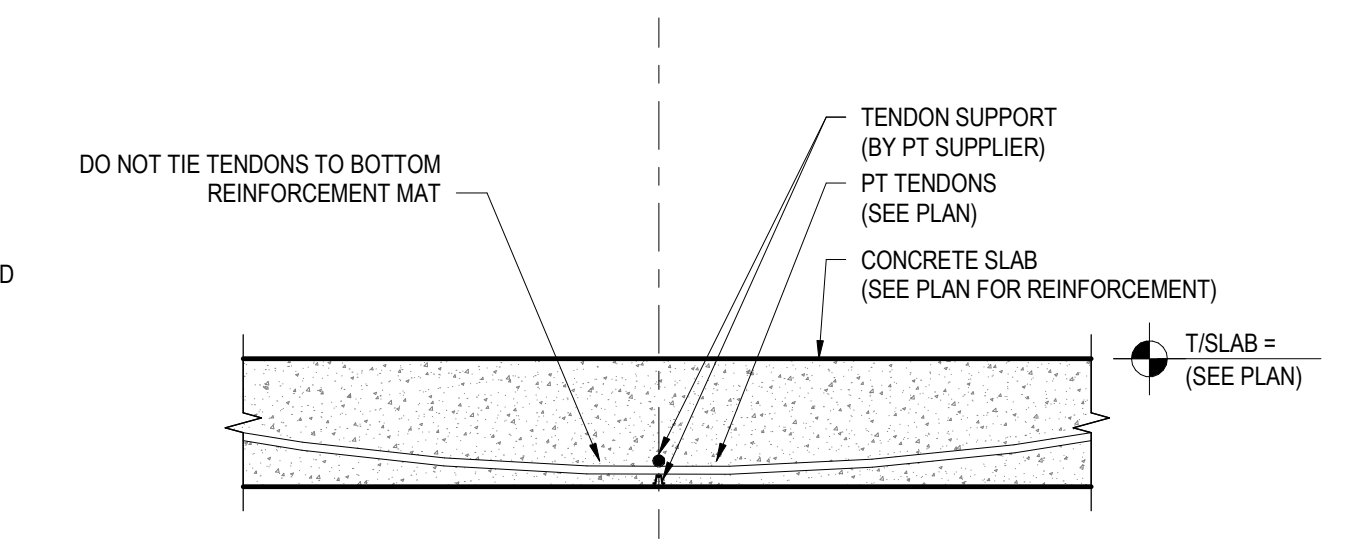
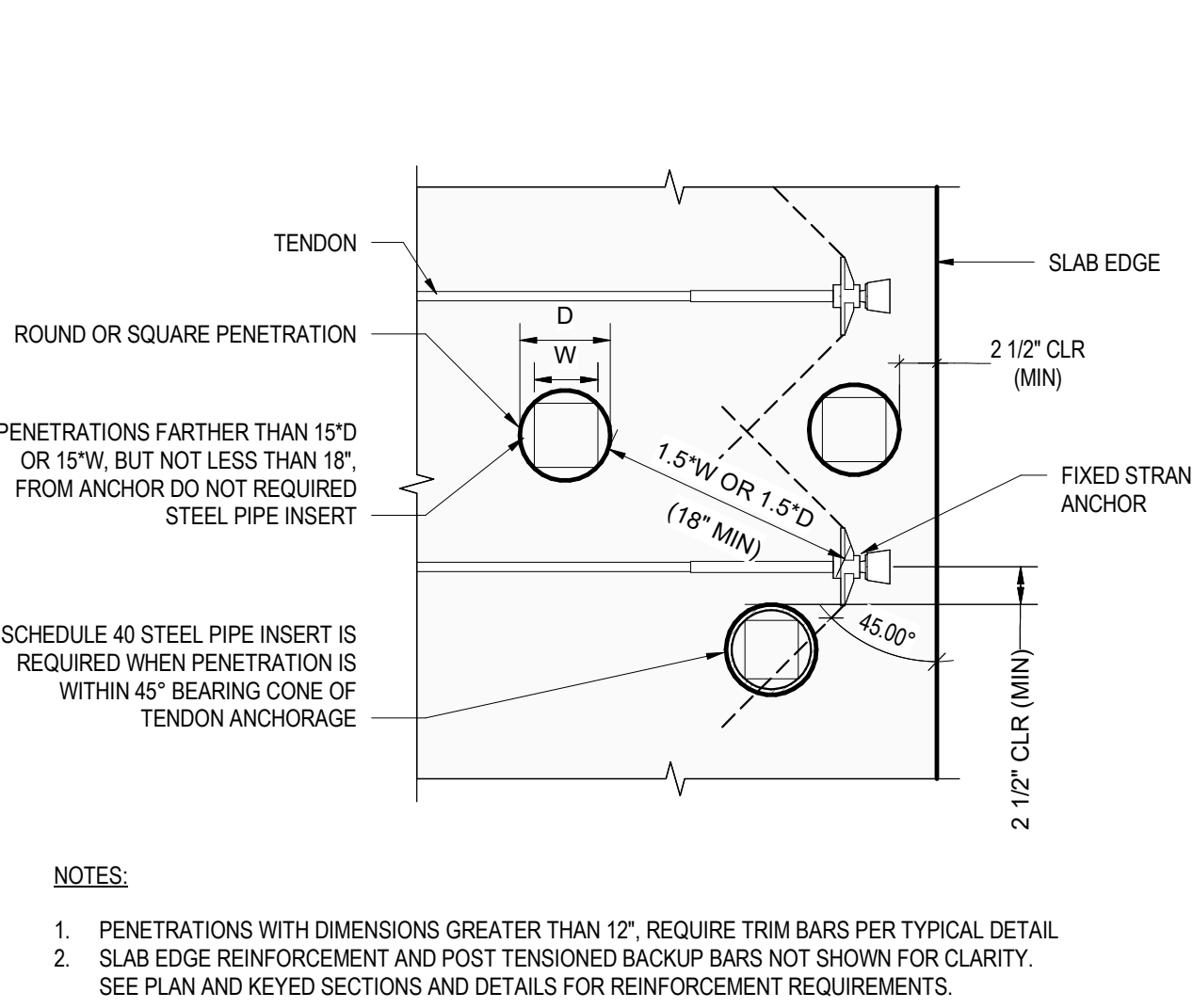
2 BANDED TENDON ANCHORAGE CONDITIONS

NTS



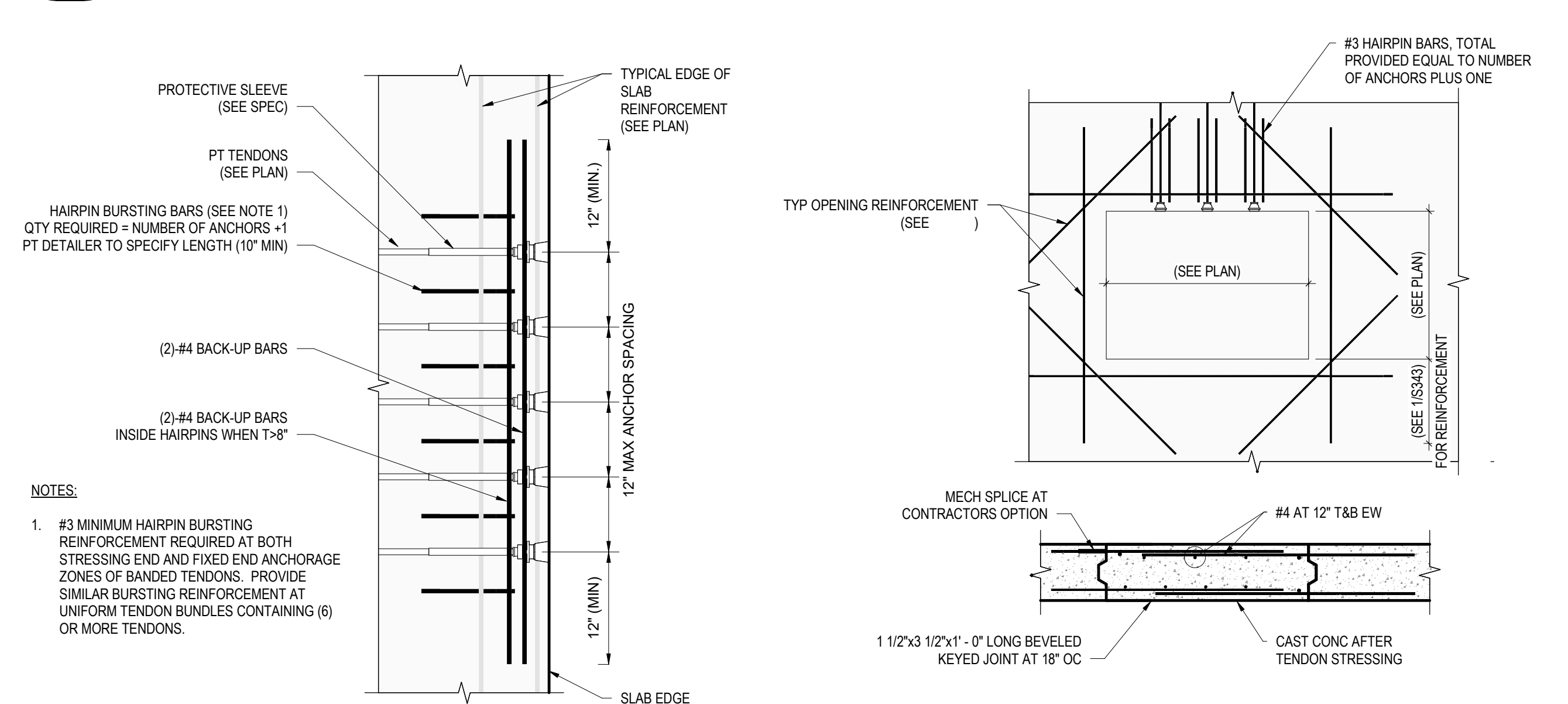
3 ANCHORAGE AT ADDED TENDON

NTS



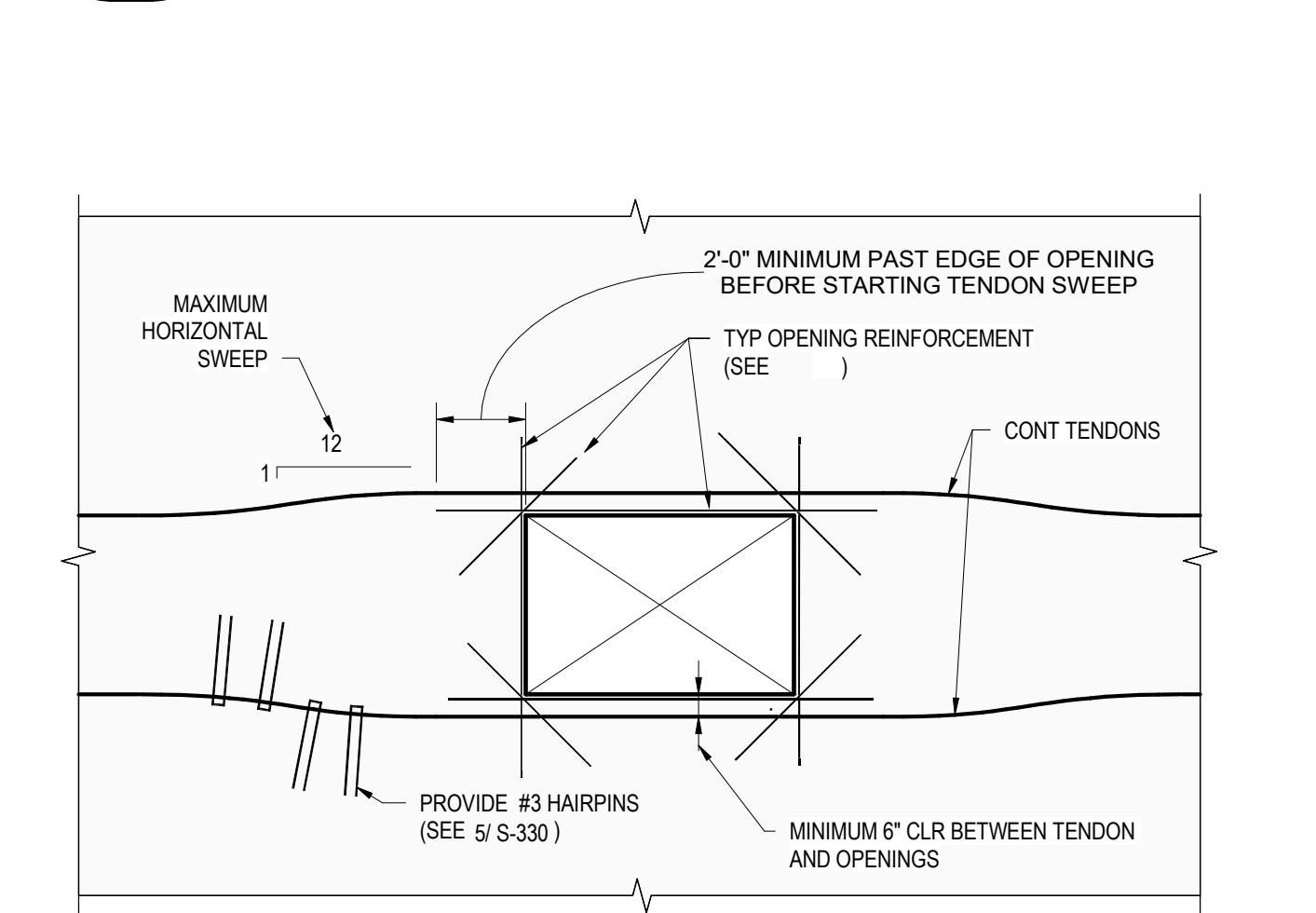
4 UNIFORM TENDON ANCHORAGE CONDITIONS

NTS



5 TYPICAL TENDON HORIZONTAL CURVE

3/4" = 1'-0"



6 SLEEVES NEAR TENDON ANCHORAGE

1" = 1'-0"



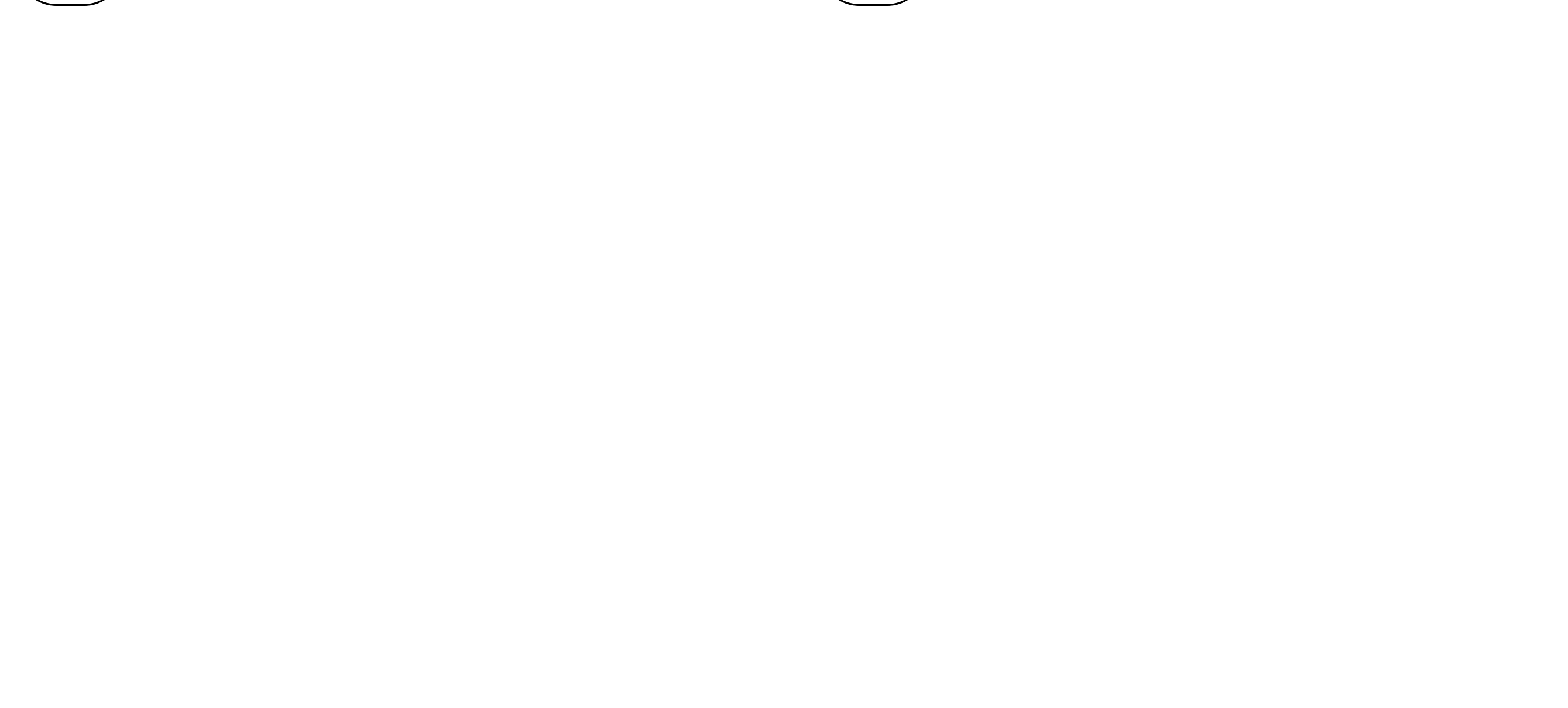
7 PT TENDON LOW POINT - SECTION

1" = 1'-0"



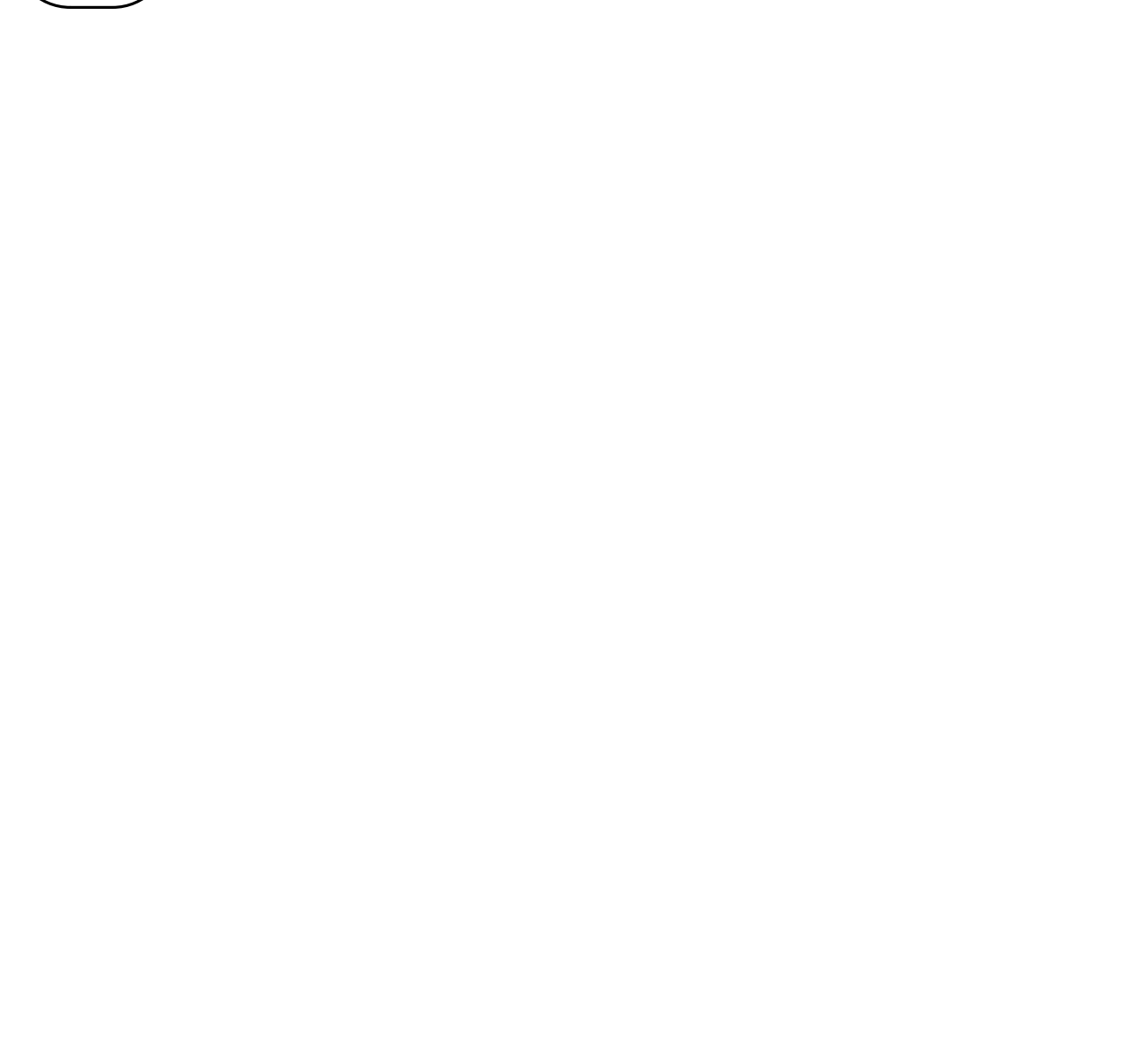
8 TYPICAL BANDED ANCHORAGE ZONE

NTS



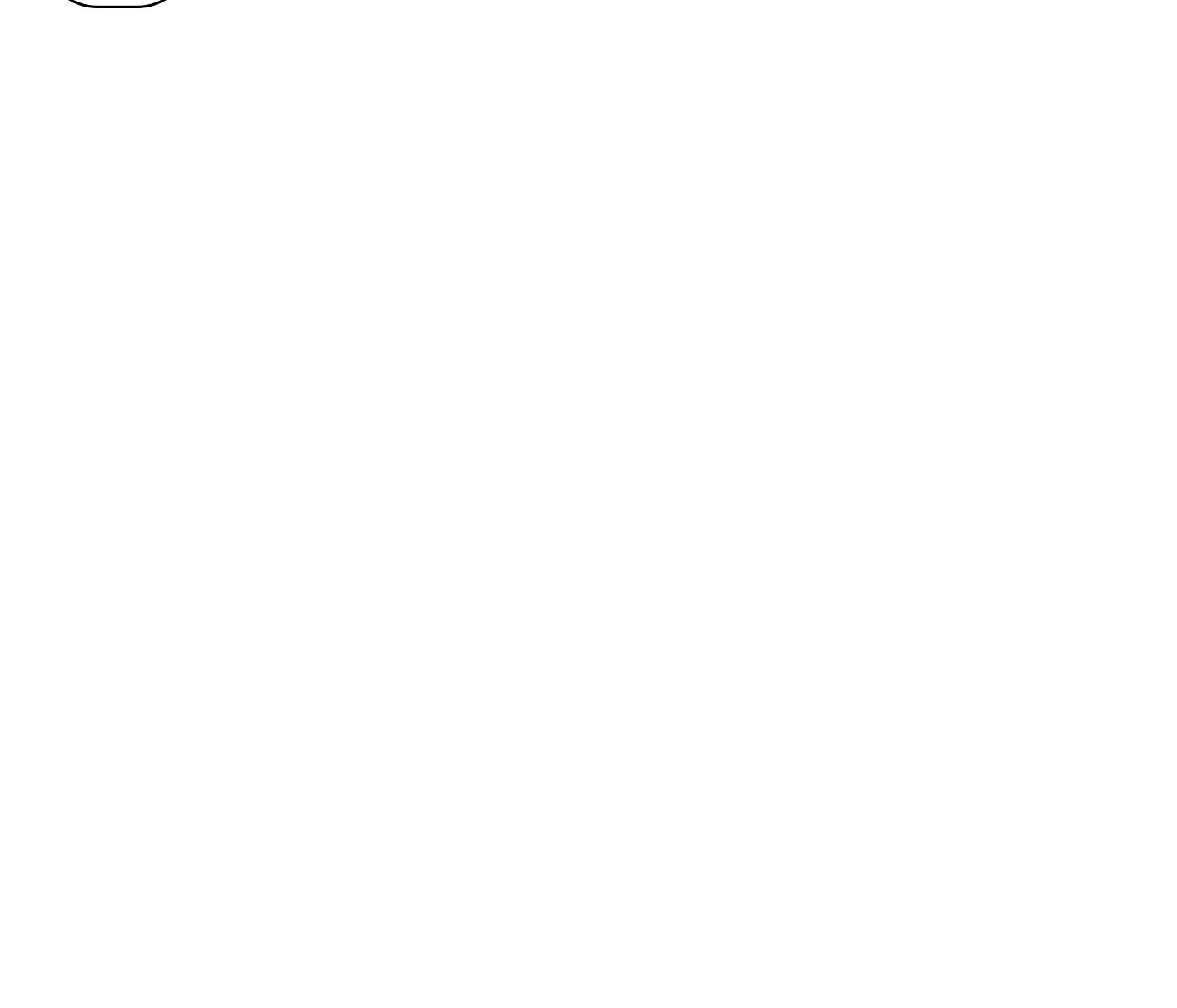
9 STRESSING BLOCKOUT

1/2" = 1'-0"



10 TENDONS AT SLAB OPENING

NTS



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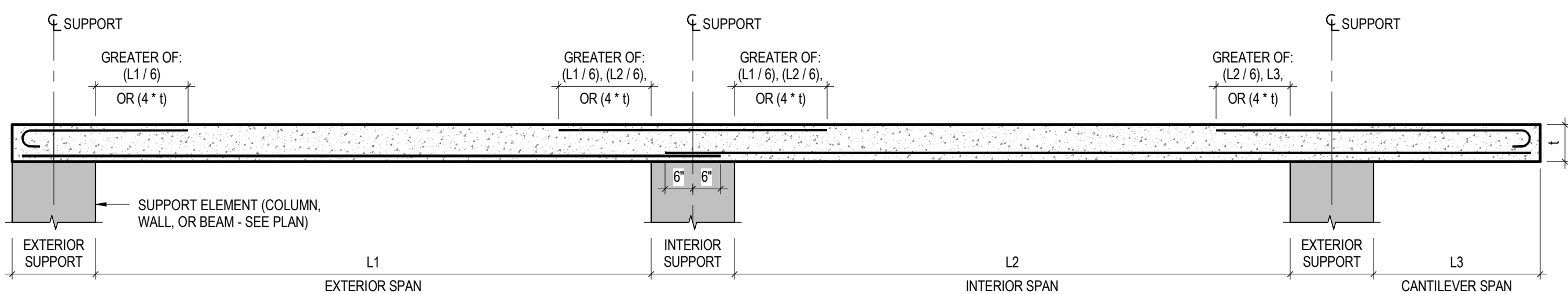
TYPICAL PT
TENDON
DETAILS

S-330

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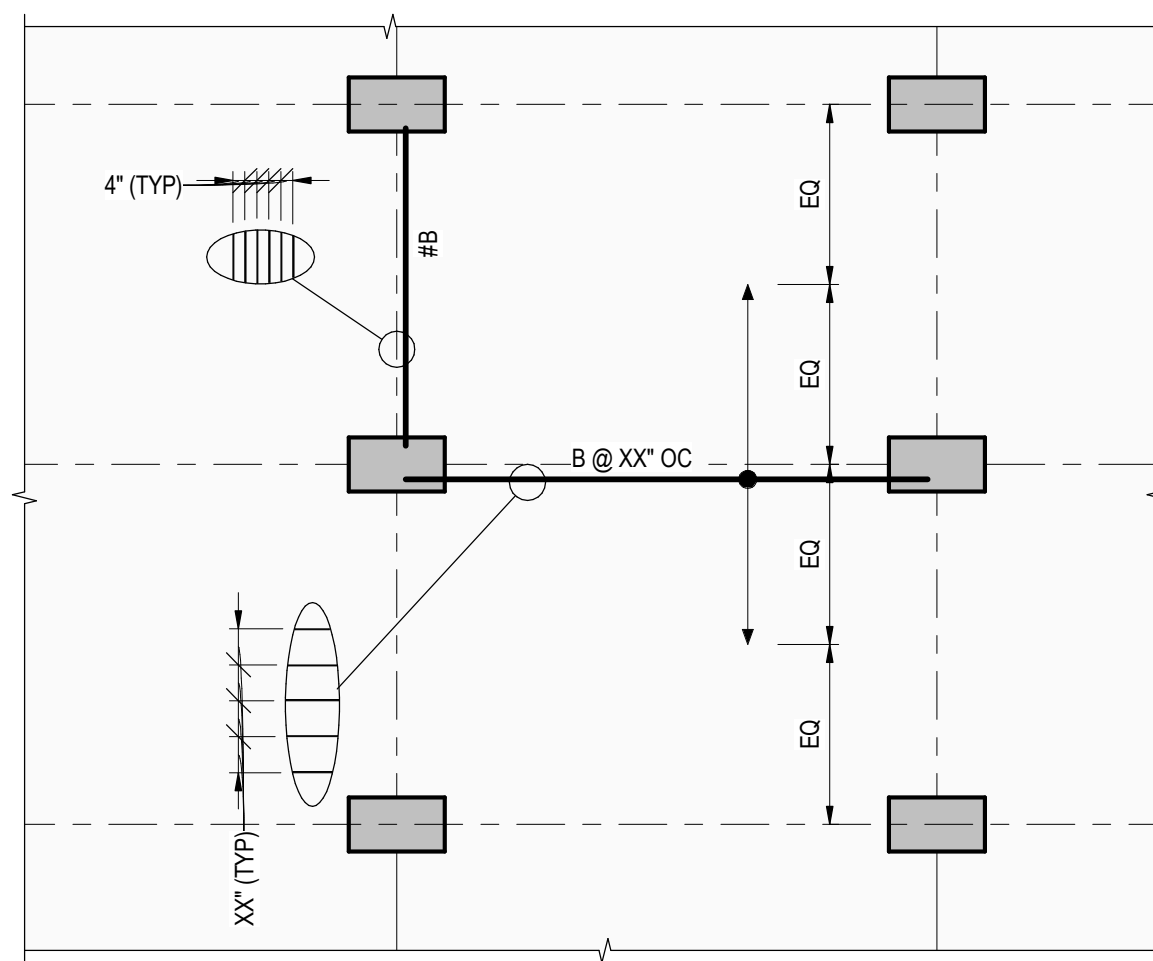
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NOTES:

- DRAWING IS NOT TO SCALE.
- CLEAR COVER PER CONCRETE SLAB CLEAR COVER SCHEDULE ON.
- BAR LENGTHS SHOWN ABOVE SHALL BE USED UNLESS NOTED OR SHOWN OTHERWISE ON DRAWINGS.
- TERMINATE TOP BARS AT EDGE-OF-SLAB OR EDGE-OF-OPENING WITH ACI STANDARD 180° HOOKS TURNED FROM VERTICAL AS REQUIRED TO MAINTAIN CLEAR COVER.
- BOTTOM BARS SHALL BE DETAILED AS FOLLOWS:
 - WHEN SHOWN ON PLAN AS CONTINUOUS BETWEEN SUPPORTS - FULL-LENGTH WITH MINIMUM EMBEDMENTS INTO SUPPORTS EACH END AS SHOWN ABOVE.
 - WHEN SHOWN ON PLAN AS PARTIAL LENGTH - LENGTH EQUAL TO THAT INDICATED ON PLAN OR HALF OF THE SPAN IN WHICH THEY ARE PLACED (WHICHEVER IS GREATER) WITH 1/4 OF THE SPECIFIC BAR QUANTITY (ROUNDED UP - MINIMUM OF 2) CONTINUOUS BETWEEN SUPPORTS WITH MINIMUM EMBEDMENT INTO SUPPORTS EACH END AS SHOWN ABOVE.

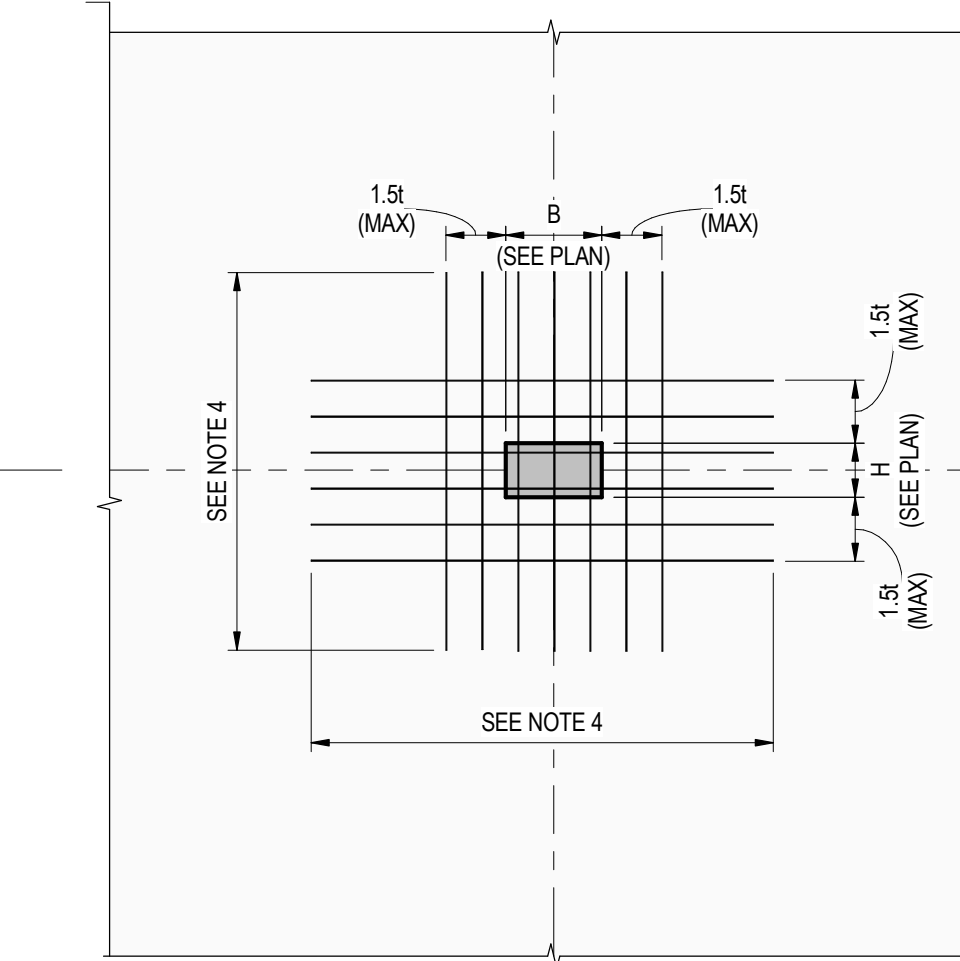
1 TYPICAL POST-TENSIONED SLAB BAR LENGTHS
S-331 1/2" = 1'-0"



NOTES:

- BARS SHOWN AS #8 SHALL BE CENTERED ALONG COLUMN LINE UNLESS SPECIFICALLY NOTED OTHERWISE.
- BAR SPACED FOR BARS SHOWN AS #8 SHALL BE 4" OC UNLESS NOTED OTHERWISE.
- BARS SHOWN AS SPACED AT "x" SHALL BE OF A QUANTITY AS REQUIRED TO FILL EVENLY THE EXTENTS SHOWN ON PLAN.
- BOTTOM BARS SHALL BE CENTERED AT MID SPAN UNLESS NOTED OTHERWISE.
- SEE TYPICAL POST TENSIONED SLAB REINFORCING LAYOUT DETAIL FOR BAR EXTENTS.

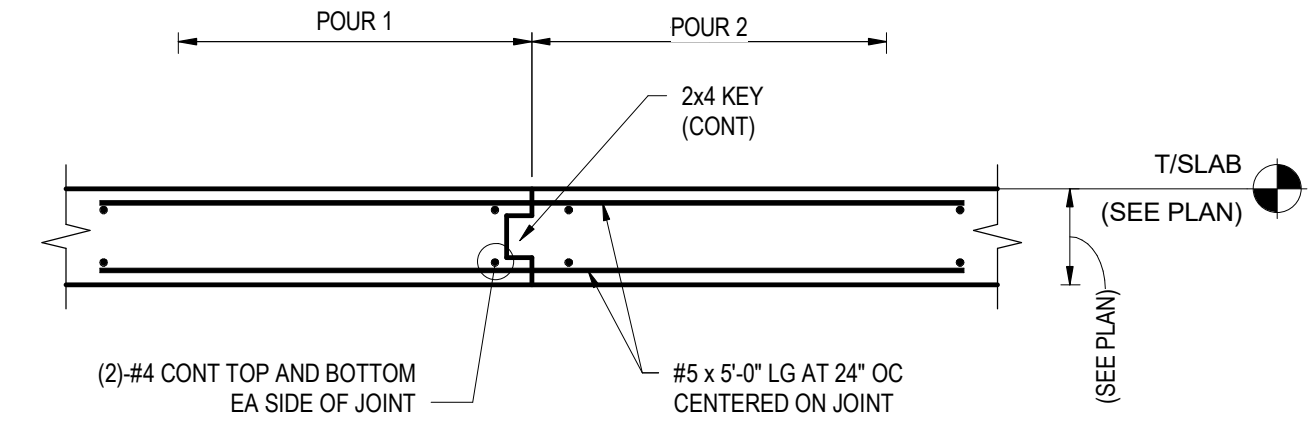
2 BOTTOM BAR PLACEMENT
S-331 NTS



NOTES:

1. DENOTES SLAB THICKNESS AT COLUMN LOCATION.
- PLACE A MINIMUM OF 2 OR 3 BARS THROUGH COLUMN SO THAT REMAINING BARS ARE EQUAL ON EACH SIDE OF COLUMN.
- BAR SPACING SHALL BE SUCH THAT ALL BARS FIT WITHIN (B OR H) + 3L, BUT SHALL NOT BE SPACED GREATER THAN 12" OC.
- SEE TYPICAL POST TENSIONED SLAB REINFORCING LAYOUT DETAIL FOR BAR LENGTHS.

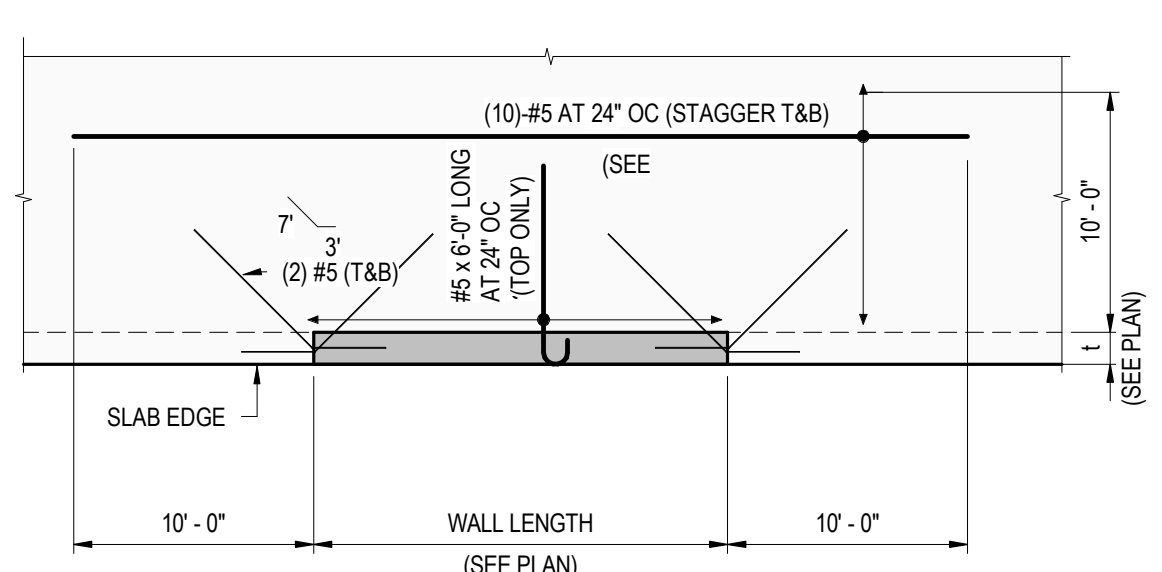
3 TOP BAR PLACEMENT AT COLUMNS
S-331 NTS



NOTES:

- SUBMIT PROPOSED CONSTRUCTION JOINT LOCATIONS FOR REVIEW PRIOR TO CONSTRUCTION.
- SCHEDULE REINFORCING AND/OR REINFORCING INDICATED ON PLAN SHALL BE CONTINUOUS THROUGH ALL CONSTRUCTION JOINTS. MAINTAIN SPLICE LOCATIONS INDICATED IN FRAMING SECTIONS. ADDITIONAL SPLICES ARE NOT PERMITTED.
- WHERE POST TENSION EXISTS, CONSTRUCTION JOINTS SHALL BE LOCATED SUCH THAT THE POST TENSIONING IS ANCHORED AT THE CENTER OF GRAVITY OF THE SLAB CROSS SECTION.
- INTENTIONALLY ROUGHEN THE FACE OF SLAB AT CONSTRUCTION JOINT TO 1/4" MINIMUM AMPLITUDE FOR SLABS ≤ 6" THICK. INSTALL 2X4 CONT KEY AT SLABS > 6" THICK ALTERNATIVELY, STAY IN PLACE GALVANIZED STEEL BULKHEAD FORMS CAPABLE OF PROVIDING THE SPECIFIED SURFACE ROUGHNESS MAY BE USED.
- CLEAR COVER TO ADDITIONAL JOINT REINFORCING SHALL BE AS REQUIRED FOR TYPICAL REINFORCING AT EACH LEVEL.

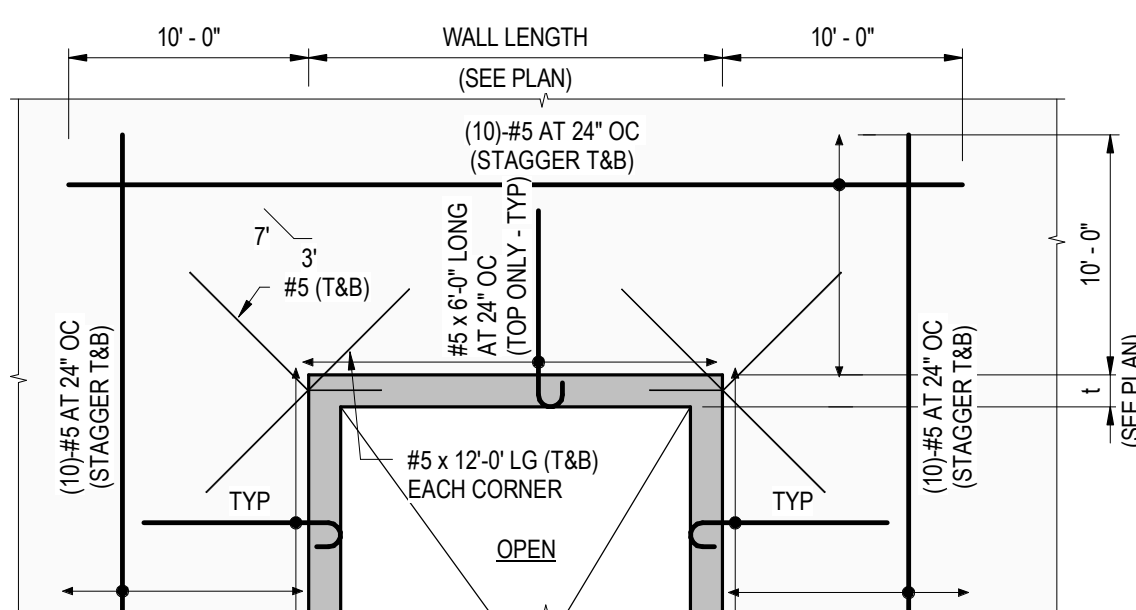
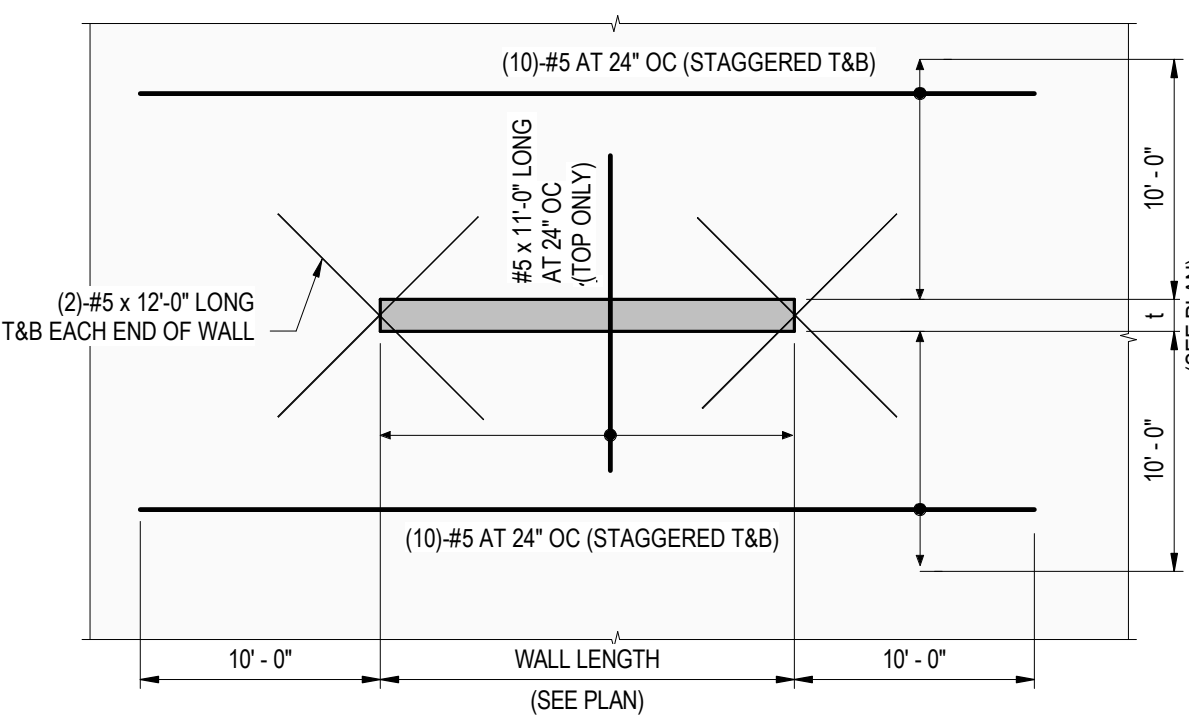
4 CONSTRUCTION JOINT IN PT SLAB
S-331 NTS



NOTES:

- AT BASEMENT OR RETAINING WALL CONDITIONS, REINFORCING INDICATED SHALL BE INSTALLED CONTINUOUS ALONG FULL LENGTH OF WALL AND SLICE CONNECTIONS SHALL BE PROVIDED.

5 ADDITIONAL SLAB REINFORCEMENT AT CONCRETE WALLS
S-331 1/8" = 1'-0"

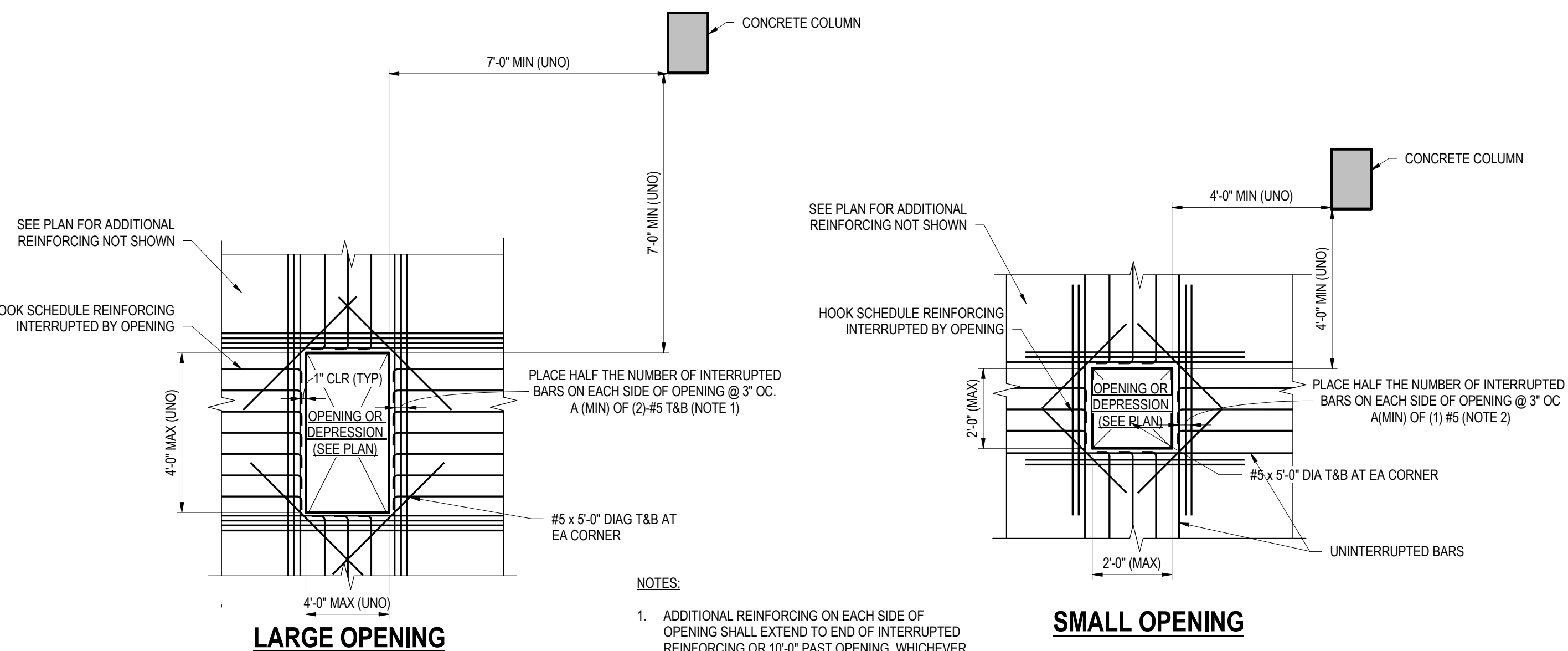


NOTES:

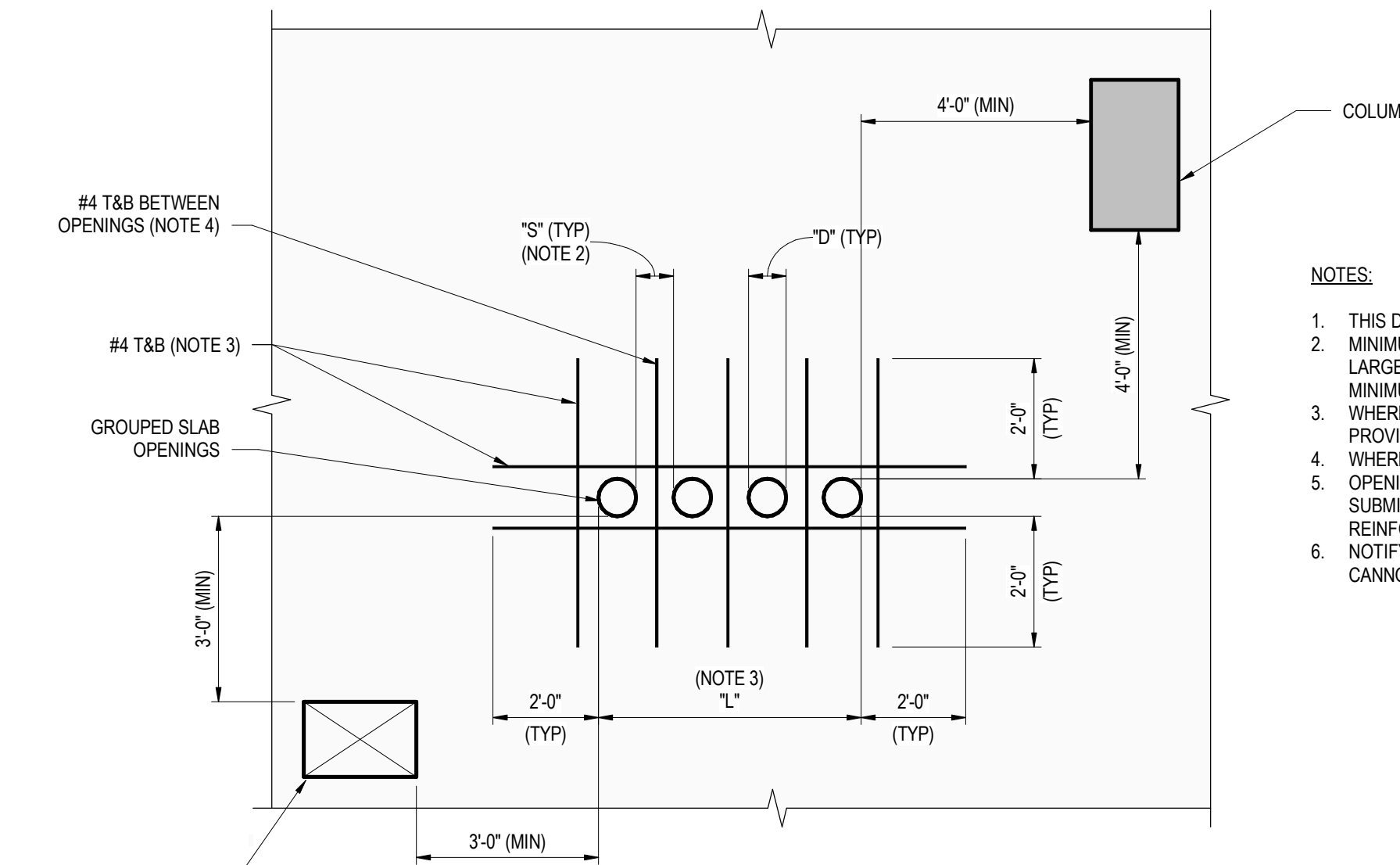
- CONVENTIONAL TOP BAR REINFORCEMENT AS NOTED ON PLAN IN THE BANDED DIRECTION SHALL BE PLACED IN THE SAME LAYER AS THE BANDED POST TENSION TENDONS.
- CONVENTIONAL TOP BAR REINFORCEMENT AS NOTED ON PLAN IN THE UNIFORM DIRECTION SHALL BE PLACED IN THE SAME LAYER AS THE UNIFORM POST TENSION TENDONS, AND SHALL BE DIRECTLY BELOW THE BANDED LAYER.
- WHERE UNIFORM TENDONS ARE LOCATED FAR ENOUGH FROM THE COLUMN CENTERLINE TO NOT CONFLICT WITH BANDED TENDONS OR REINFORCEMENT, UNIFORM TENDONS SHALL BE LOCATED ABOVE THE BANDED TENDONS TO ACHIEVE THE DRAPE ELEVATION NOTED ON THE PLANS.

6 PLACEMENT PRIORITIES AT COLUMNS
S-331 3/4" = 1'-0"

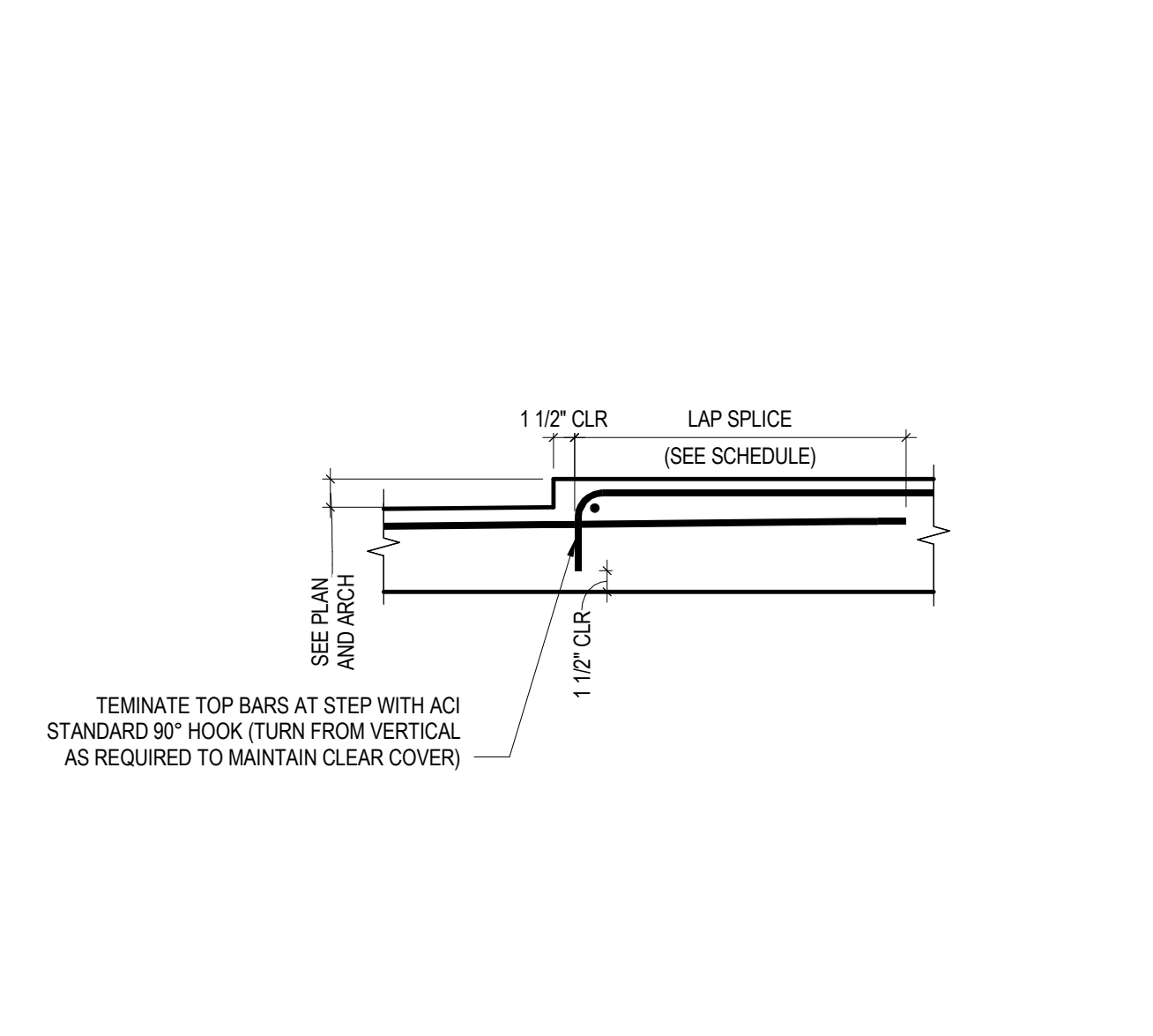
7 TYPICAL PT SLAB FOLD - DETAIL
S-331 3/4" = 1'-0"



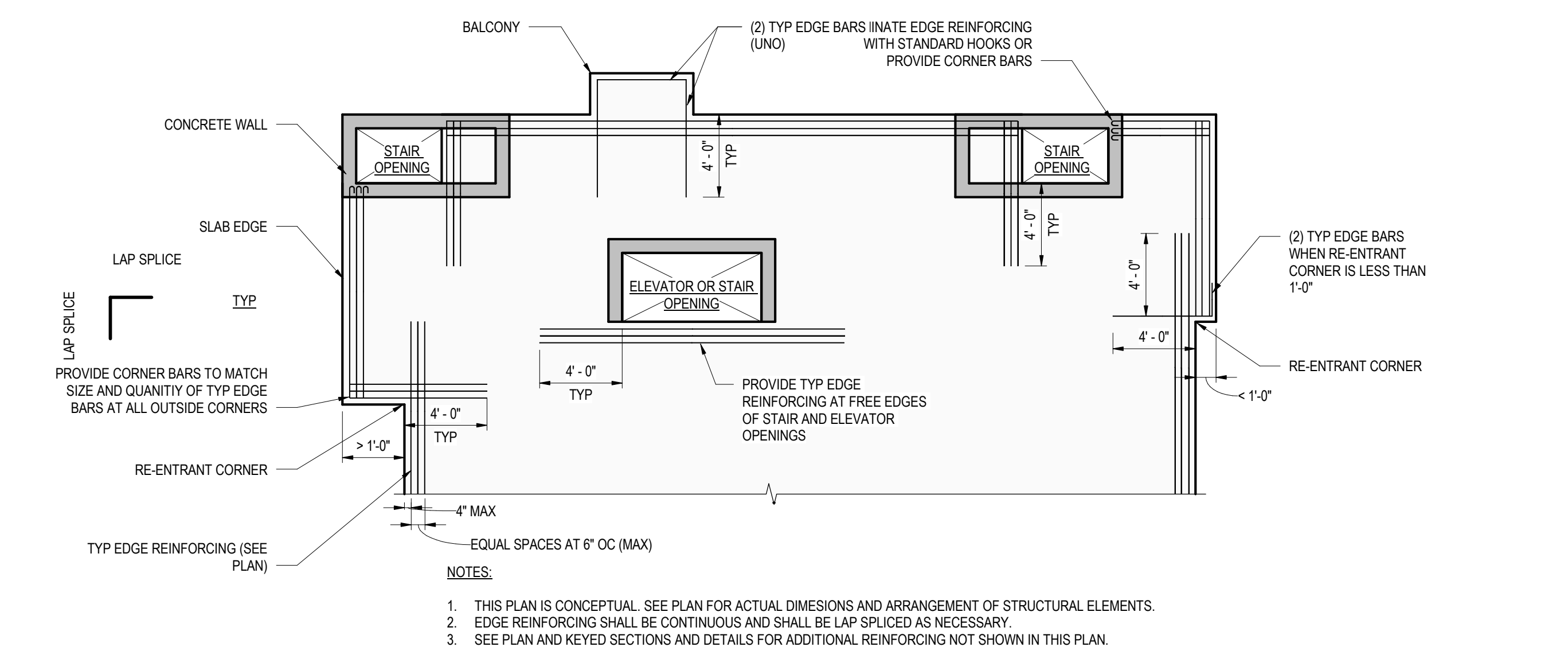
8 TYPICAL SLAB OPENING REINFORCING AT POST-TENSIONED SLAB - PLAN
S-331 3/8" = 1'-0"



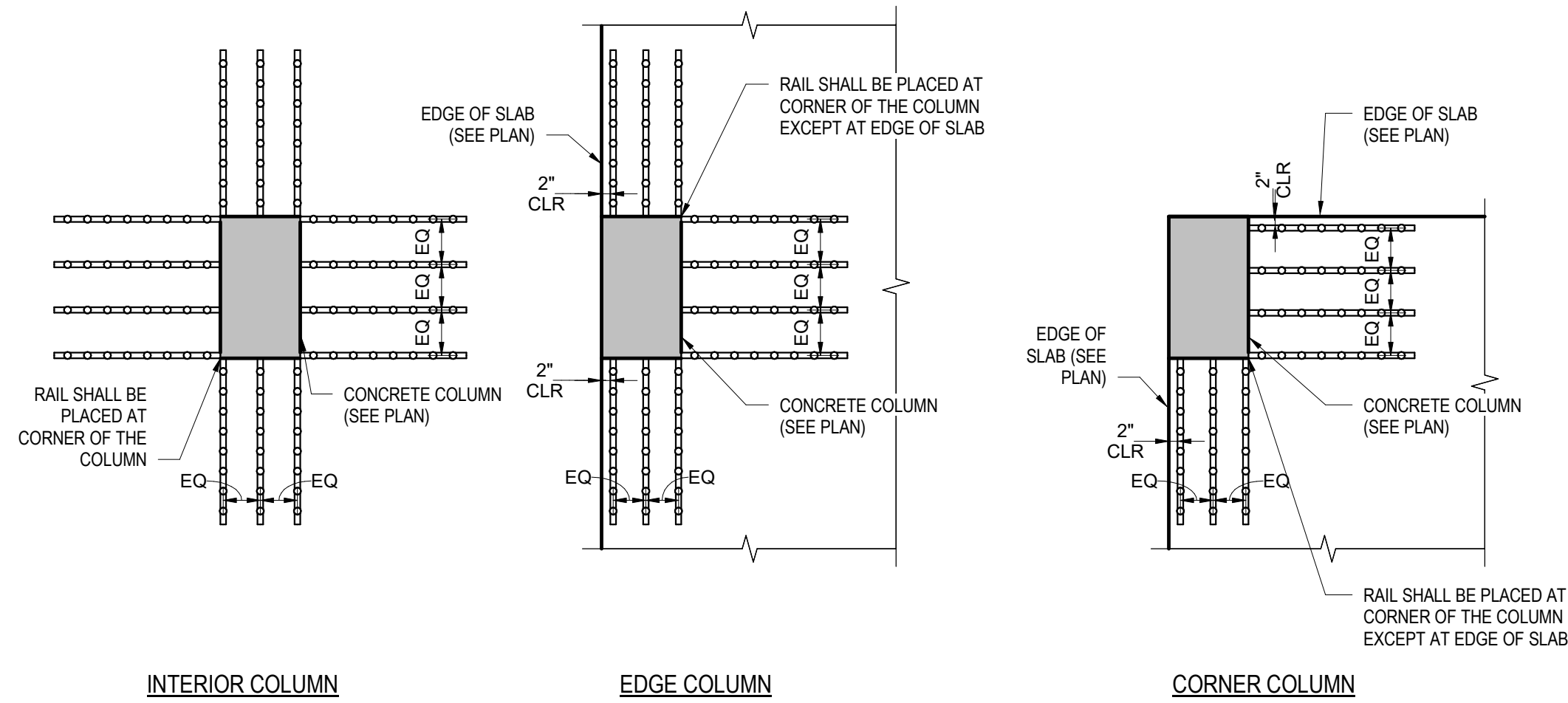
9 TYPICAL GROUPED OPENING REINFORCING AT POST-TENSIONED SLAB - PLAN
S-331 1/2" = 1'-0"



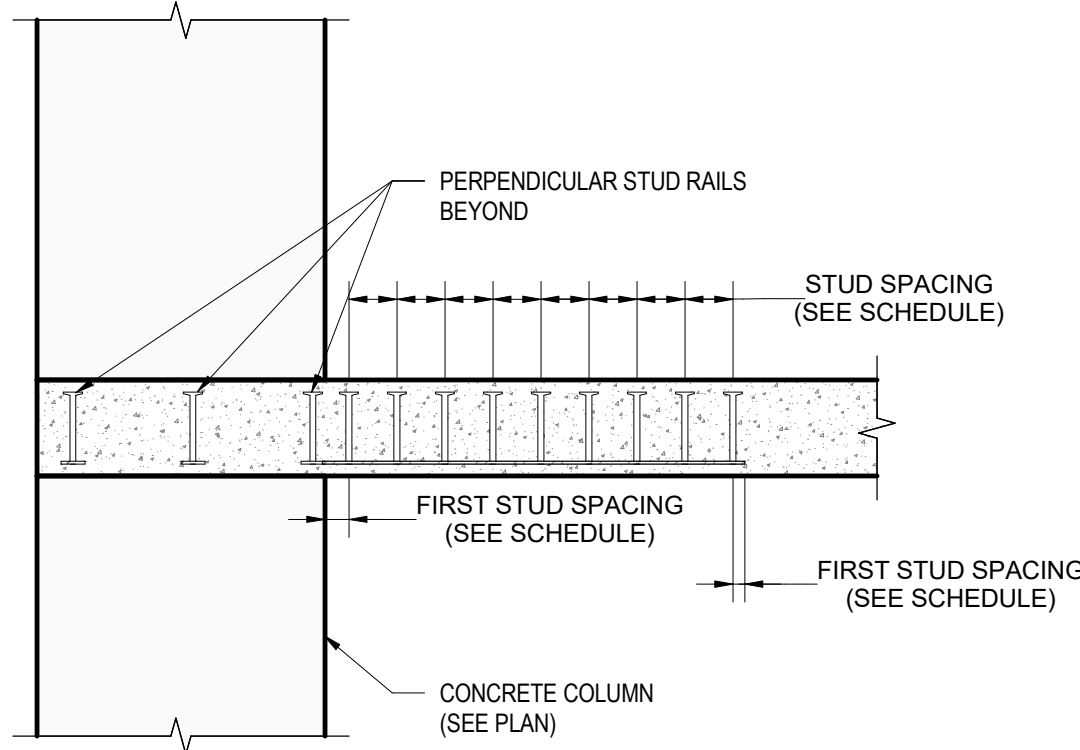
10 REBAR TRANSITION AT SLAB STEP
S-331 1" = 1'-0"



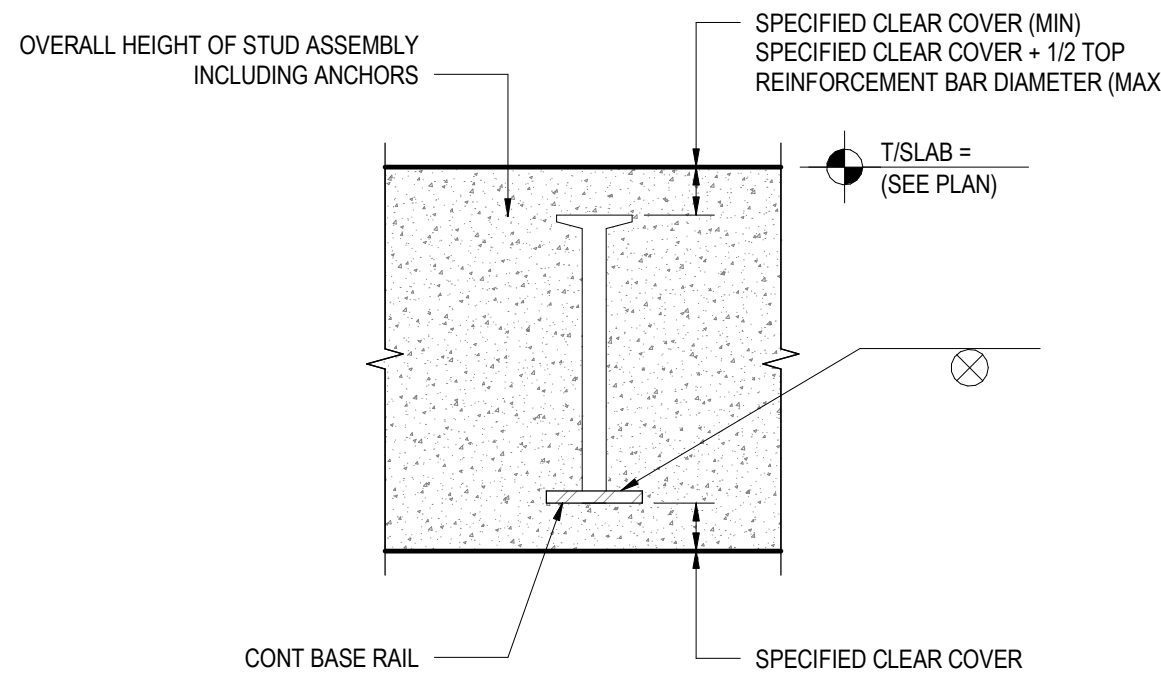
11 TYPICAL ELEVATED SLAB EDGE REINFORCING CONCEPTUAL PLAN
S-331 3/16" = 1'-0"



1 TYPICAL SHEAR STUD RAIL CONFIGURATIONS - PLAN



2 TYPICAL SHEAR STUD RAIL LAYOUT - ELEVATION



3 TYPICAL SHEAR STUD LAYOUT - SECTION

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Project No.: 2021009
Date: 08/22/2025

TYPICAL PT
REINFORCEMENT
DETAILS

S-332



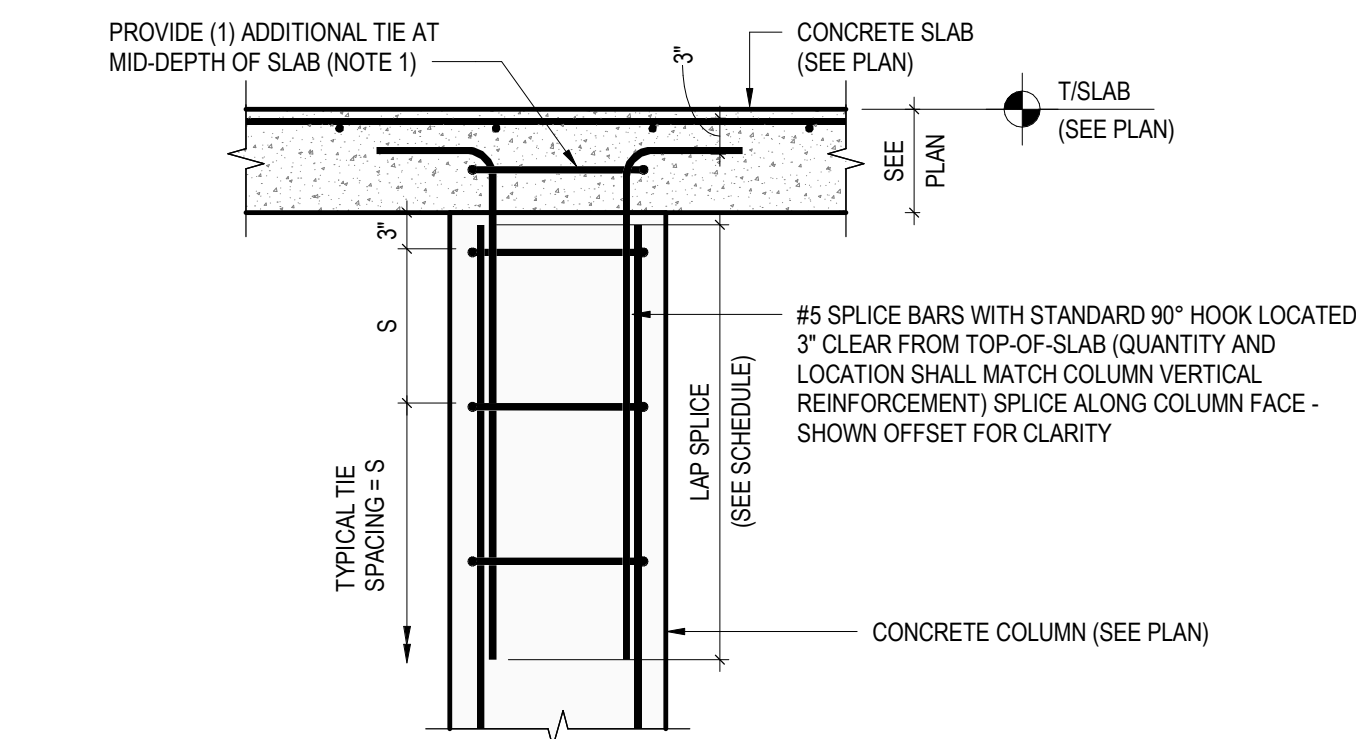
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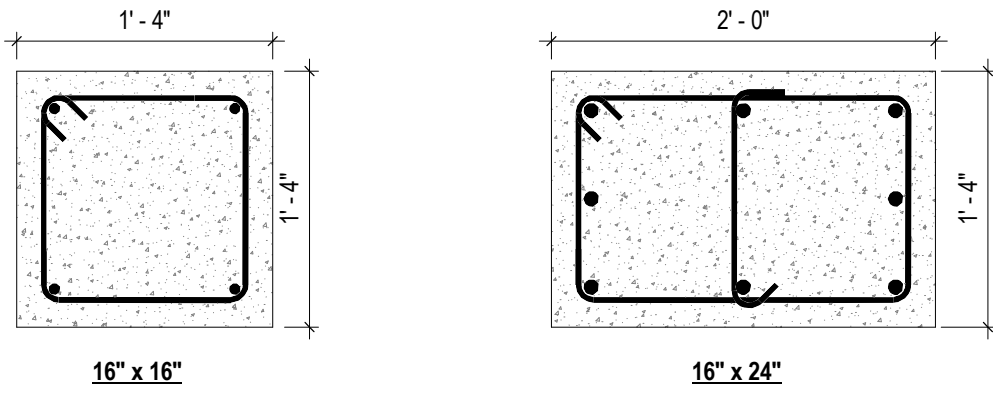
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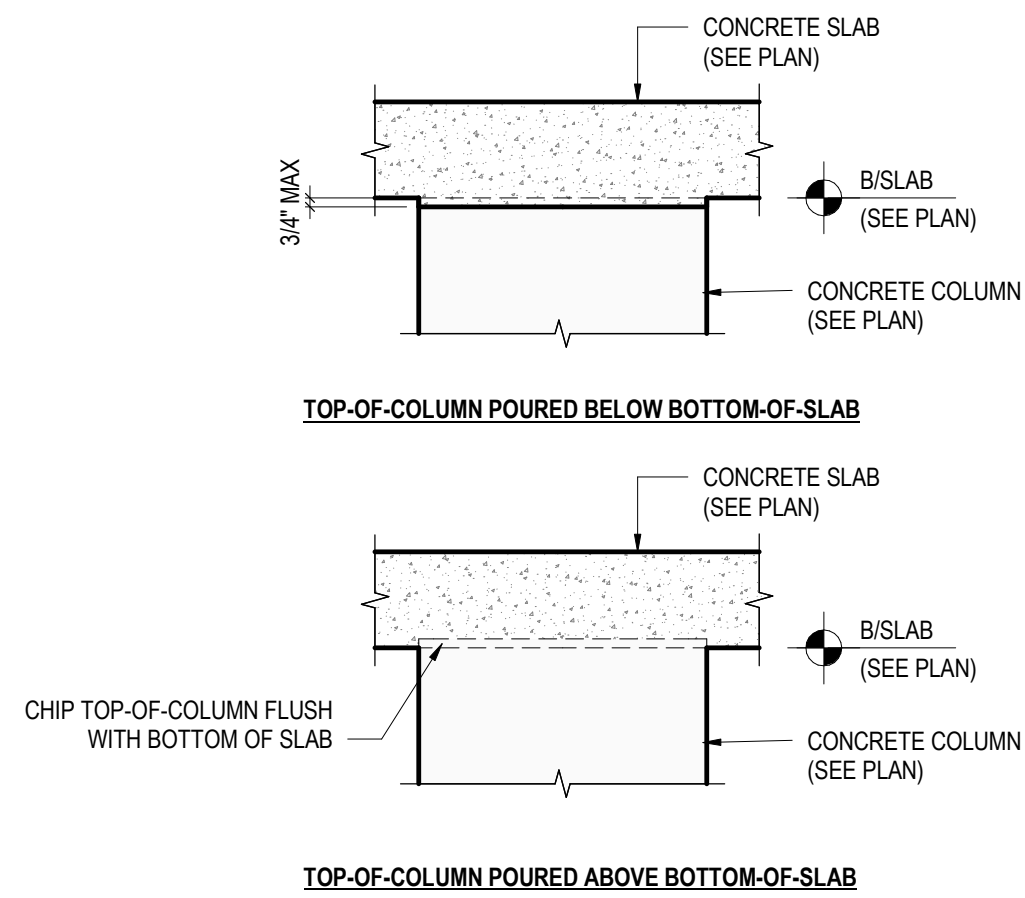
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NOTES:
1. ADDITIONAL TIE WITHIN SLAB DEPTH NOT REQUIRED WHEN A BOTTOM MAT OF SLAB REINFORCEMENT IS PRESENT AT COLUMN LOCATION



STRUCTURAL COLUMN SCHEDULE					
Type	GEOMETRY		REINFORCING		REMARKS
	WIDTH	DEPTH	VERTICAL	TIES	
CC1816	16"	16"	(4) #8	#3 @ 14" OC	
CC1824	16"	24"	(8) #7	#3 @ 14" OC	



NOTES:
1. DEVIATION FROM MAXIMUM TOLERANCES SHOWN ABOVE SHALL BE CORRECTED PRIOR TO PLACEMENT OF CONCRETE SLAB. WHEN TOP-OF-COLUMN IS INSTALLED BELOW BOTTOM-OF-SLAB BY MORE THAN THE TOLERANCE SHOWN ABOVE, THE DIFFERENCE SHALL BE POURED CONCRETE WITH STRENGTH EQUAL TO THE REQUIRED COLUMN STRENGTH.

TOP-OF-COLUMN ELEVATION TOLERANCE

1C CONCRETE COLUMN AT ROOF

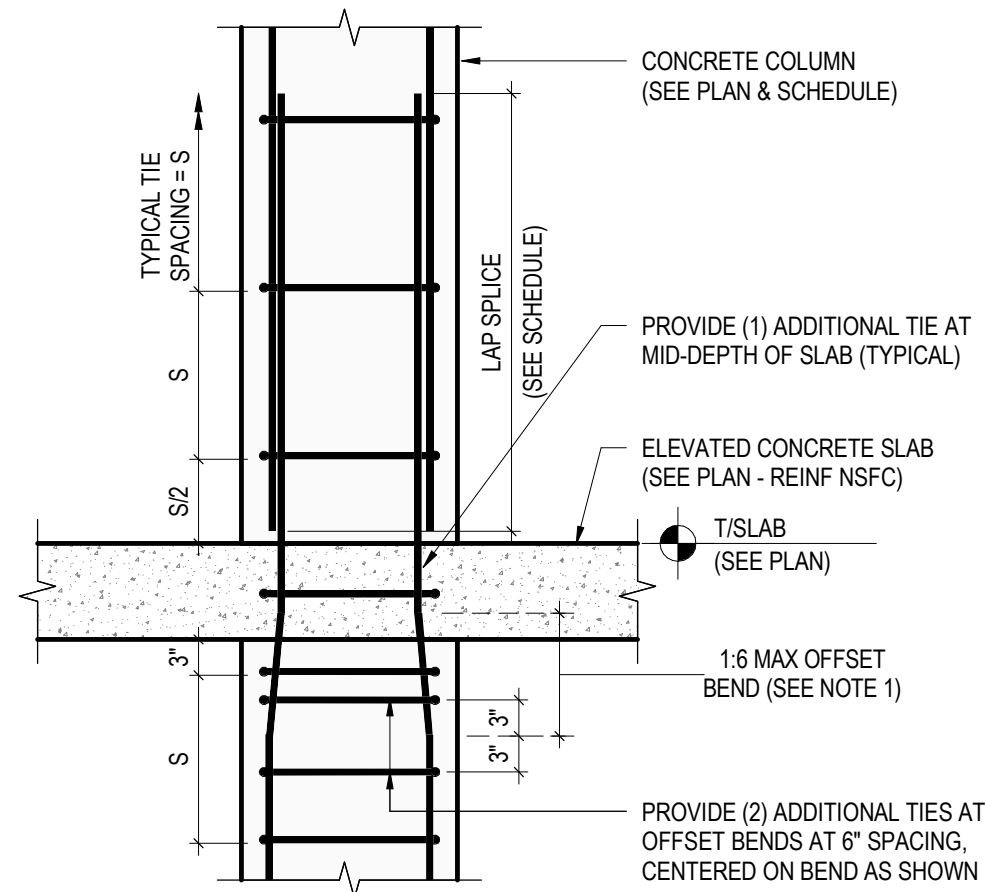
3/4" = 1'-0"

2 COLUMN PLAN DETAILS

1" = 1'-0"

3 TOP-OF-COLUMN ELEVATION TOLERANCE

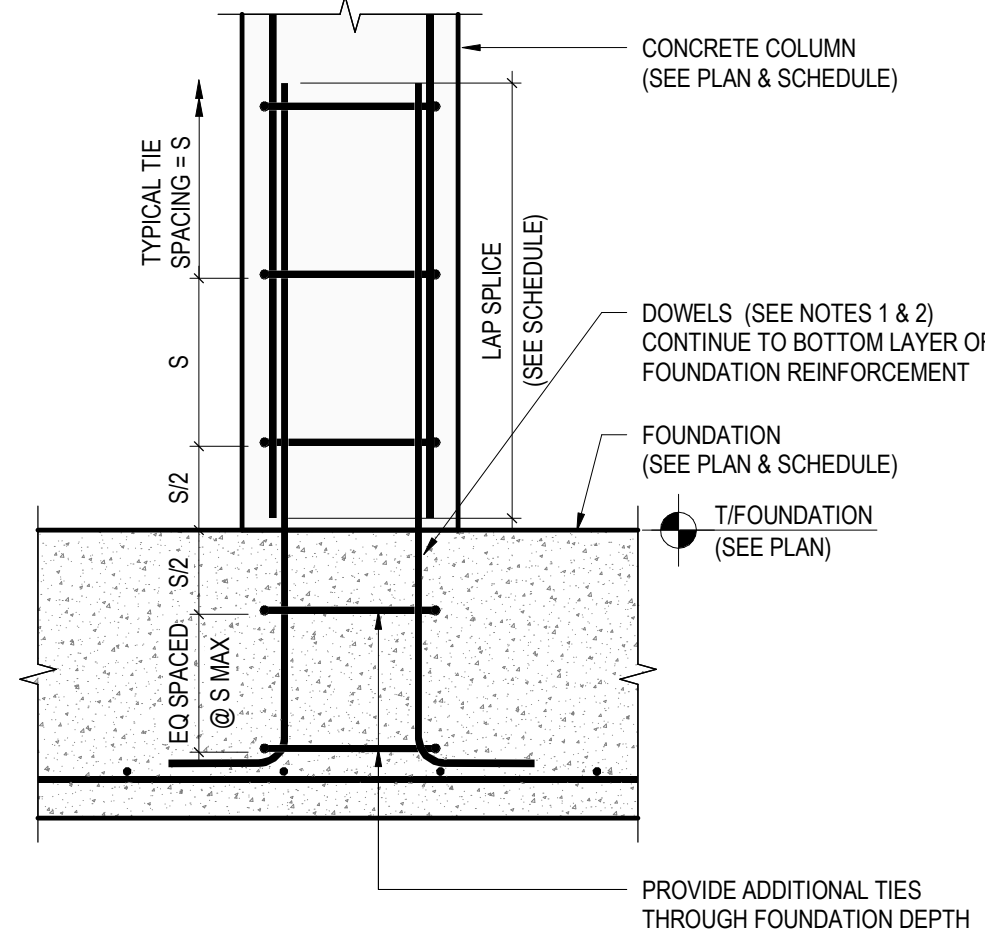
3/4" = 1'-0"



NOTES:
1. OFFSET BARS SHALL BE BENT PRIOR TO PLACEMENT IN COLUMN FORMS

1B CONCRETE COLUMN AT SLAB

3/4" = 1'-0"



NOTES:
1. DOWELS SHALL MATCH SIZE & QUANTITY OF COLUMN VERTICAL REINFORCEMENT ABOVE.
2. DOWELS SHALL BE PLACED FOR A CONTACT LAP SPlice WITH VERTICAL REINFORCEMENT ABOVE IN-LINE WITH FACE-OF-COLUMN (DO NOT OFFSET FROM FACE-OF-COLUMN). AT RECTANGULAR COLUMNS, OFFSET SHALL BE ALONG THE LONG FACE OF THE COLUMN AS SHOWN BELOW.

1A CONCRETE COLUMN AT FOUNDATION

3/4" = 1'-0"

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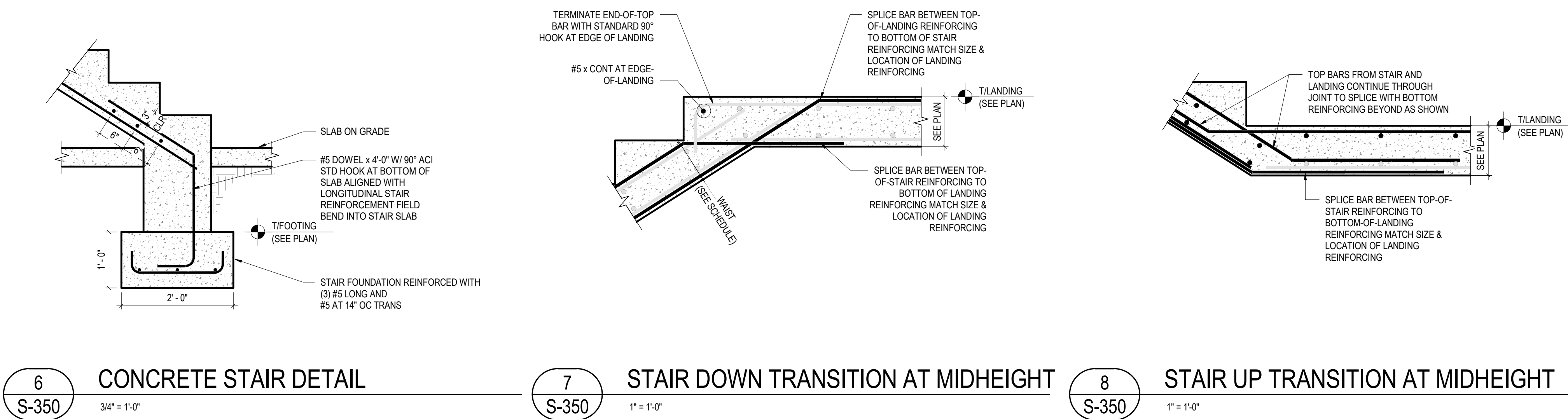
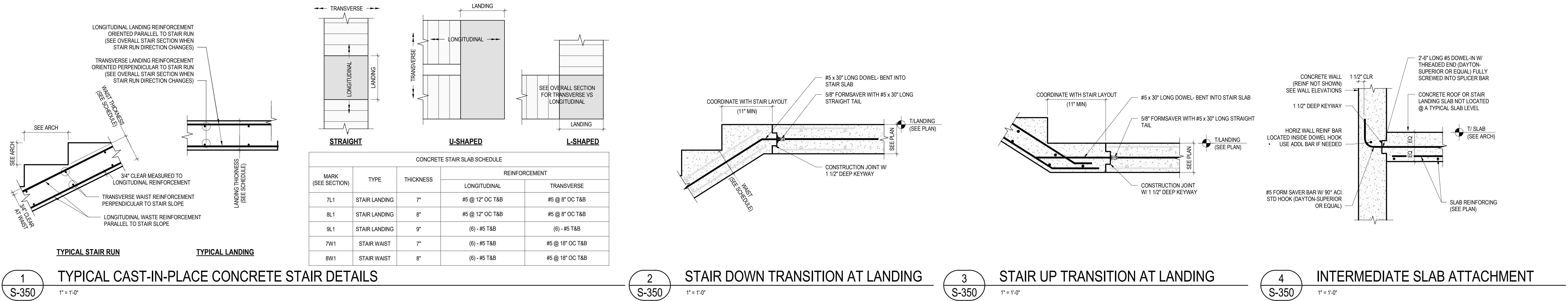
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**TYPICAL
CONCRETE
COLUMN
DETAILS**

S-340

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CONCRETE
STAIR
DETAILS

S-350



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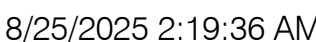


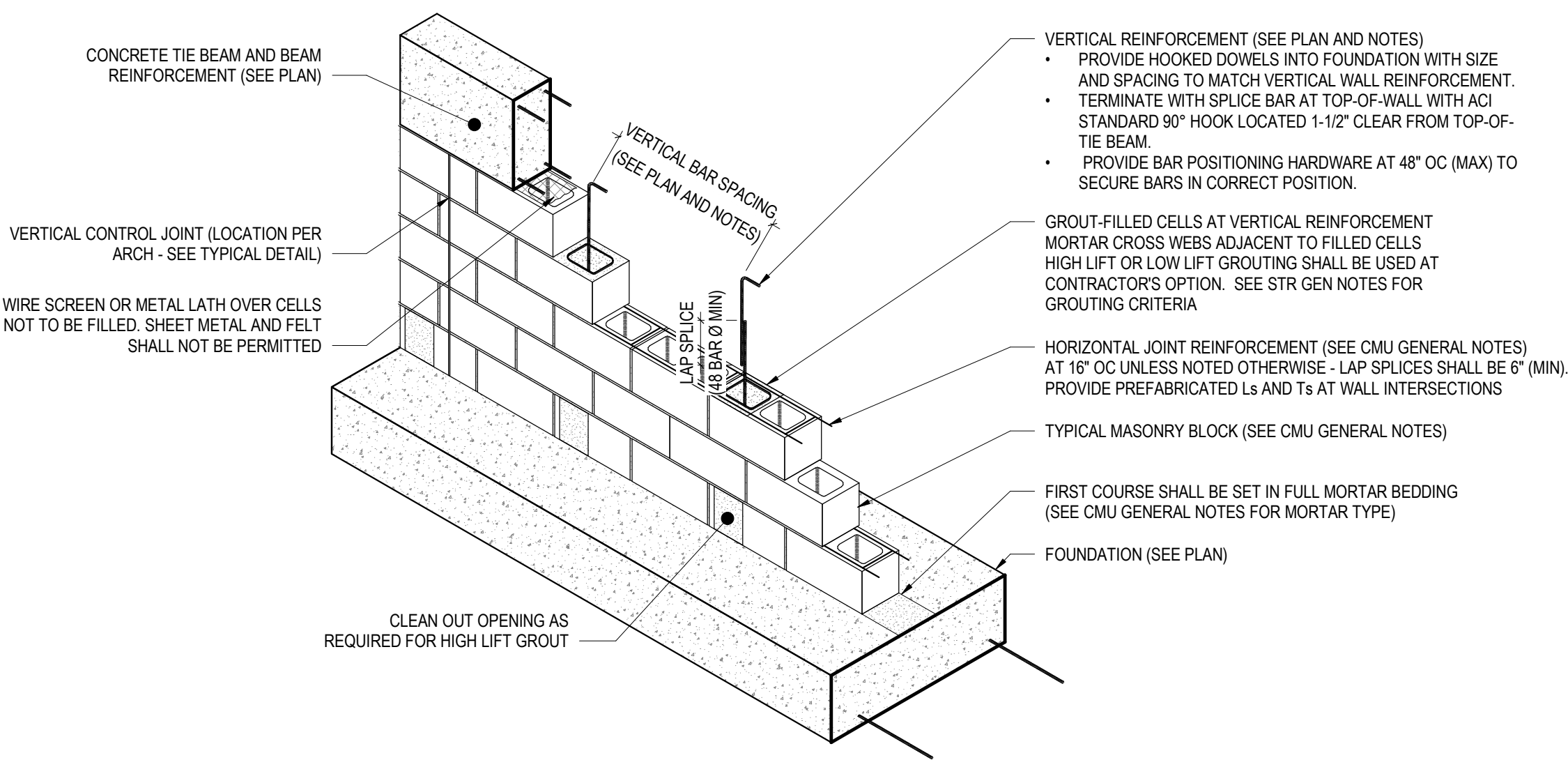
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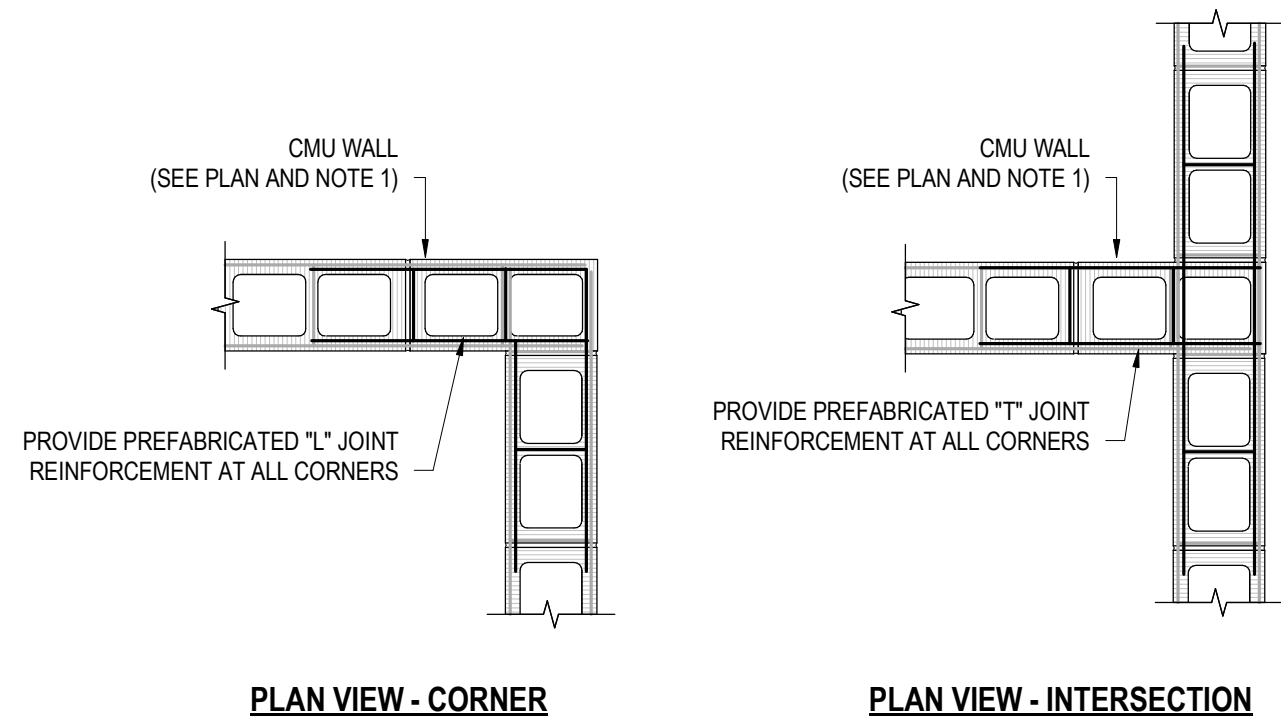
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S-360

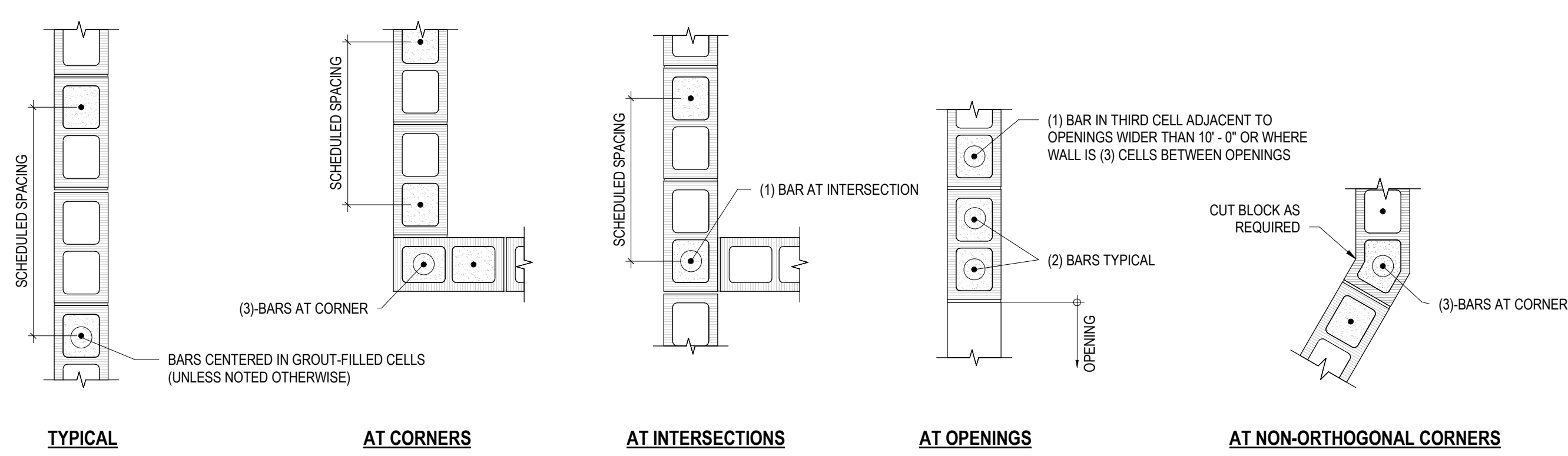




1 TYPICAL CMU WALL CONSTRUCTION - PERSPECTIVE
1/2" = 1'-0"

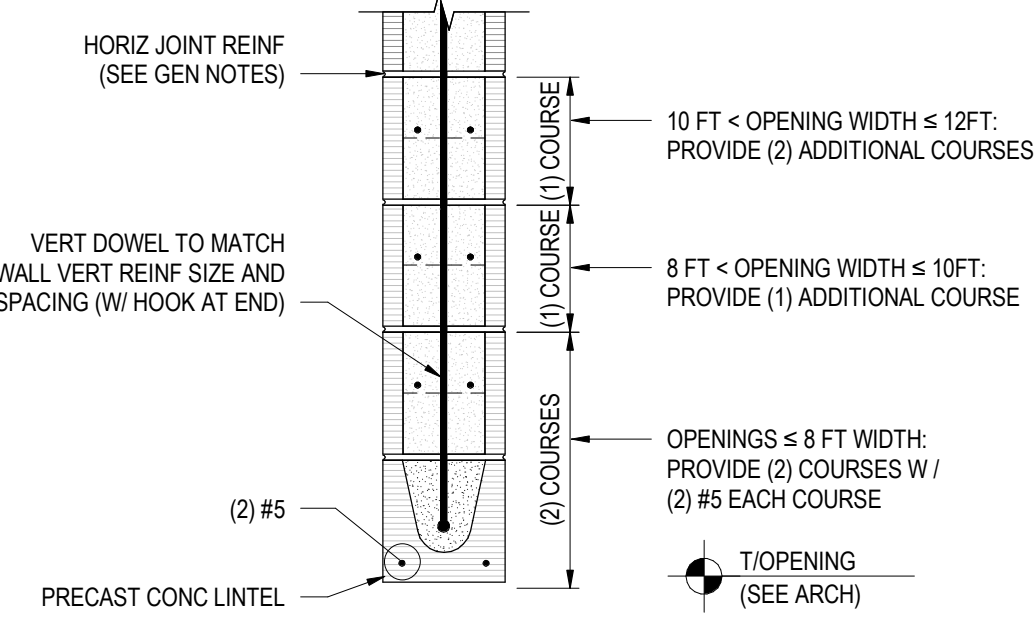
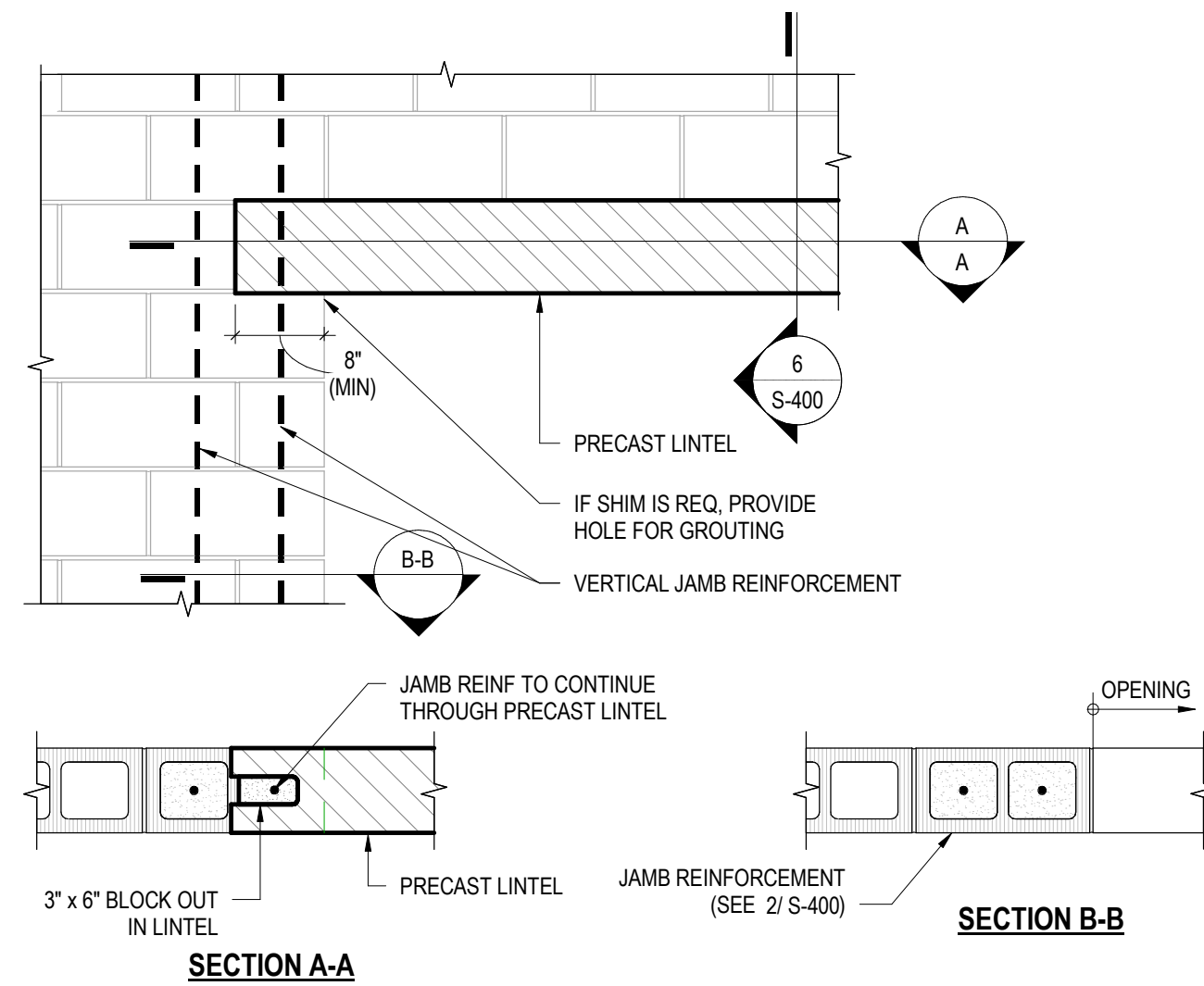


NOTES:
1. VERTICAL REINFORCEMENT AND GROUTED CELLS NOT SHOWN ON THIS DETAIL - SEE TYPICAL DETAIL

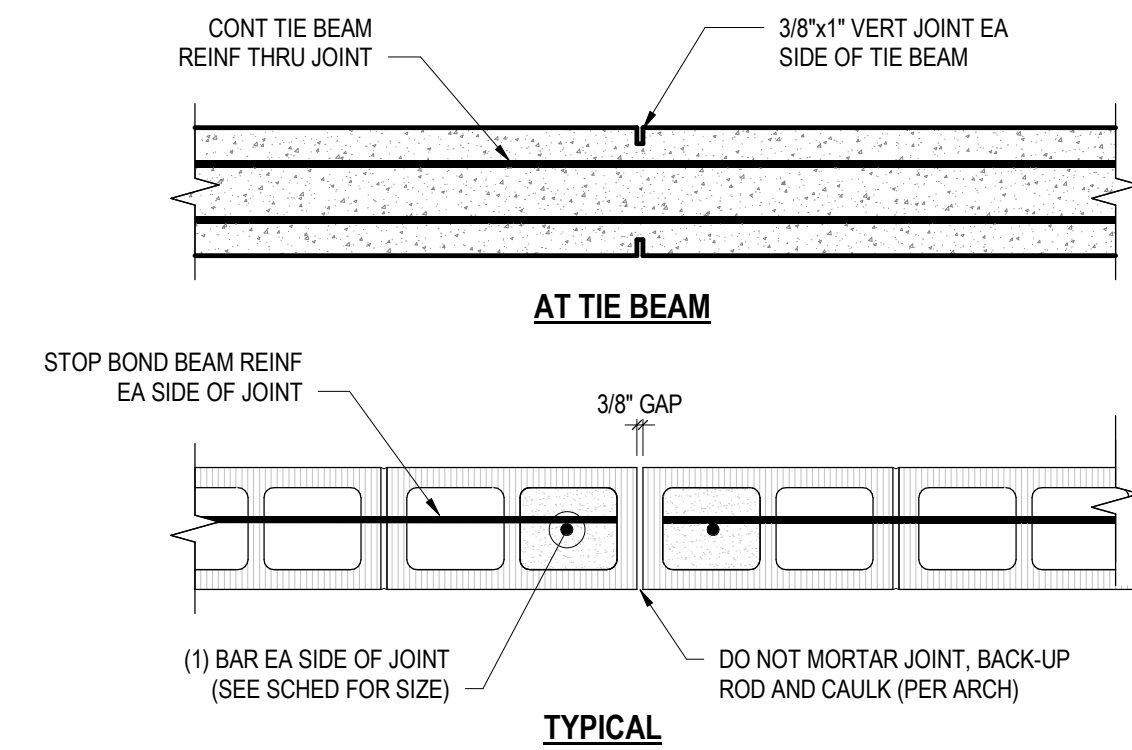


NOTES:
1. HORIZONTAL JOINT REINFORCEMENT NOT SHOWN FOR CLARITY. SEE "MASONRY NOTES" FOR JOINT REINFORCEMENT, INCLUDING PREFABRICATED CORNERS AND TEES.
2. BUILD ALL INTERSECTIONS AND CORNERS IN RUNNING BOND.

2 TYPICAL CMU WALL REINFORCEMENT
3/4" = 1'-0"

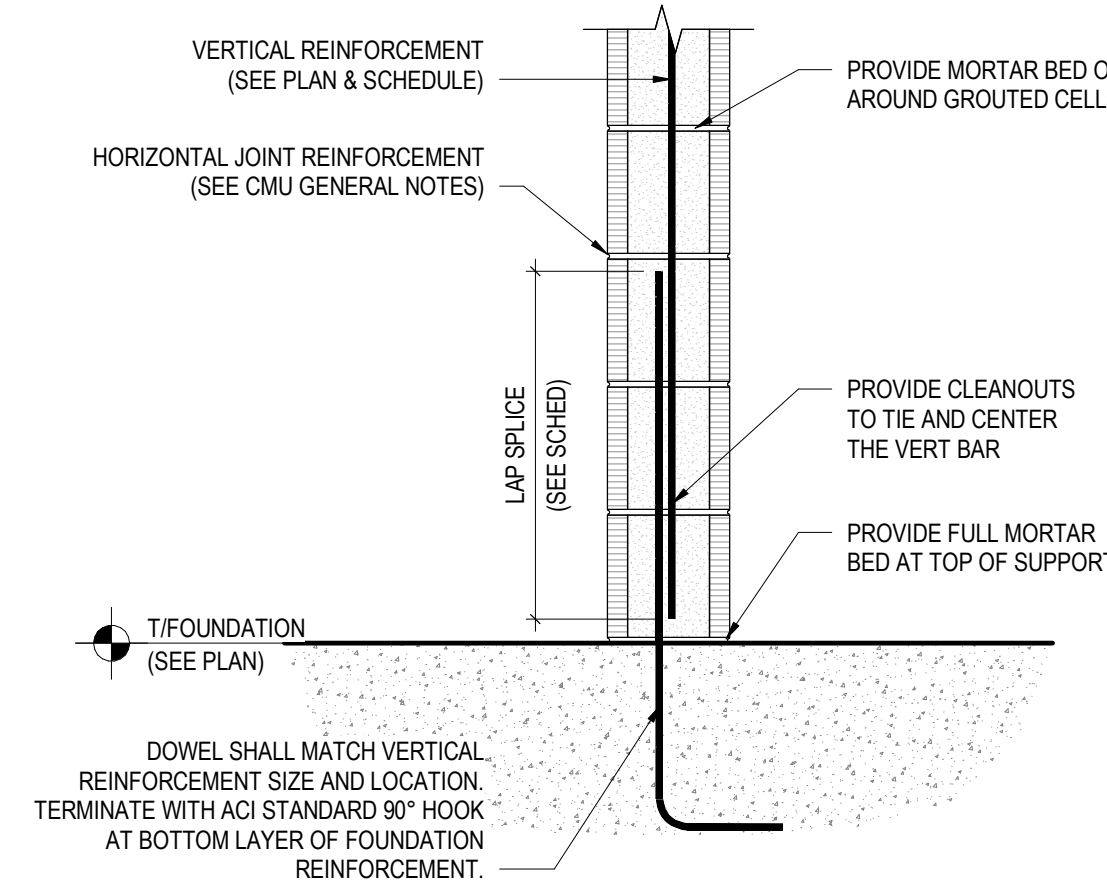


NOTES:
1. MINIMUM LINTEL BEARING, 8" EACH END
2. EXTEND REINFORCING BARS 1'-0" BEYOND FACE OF SUPPORT

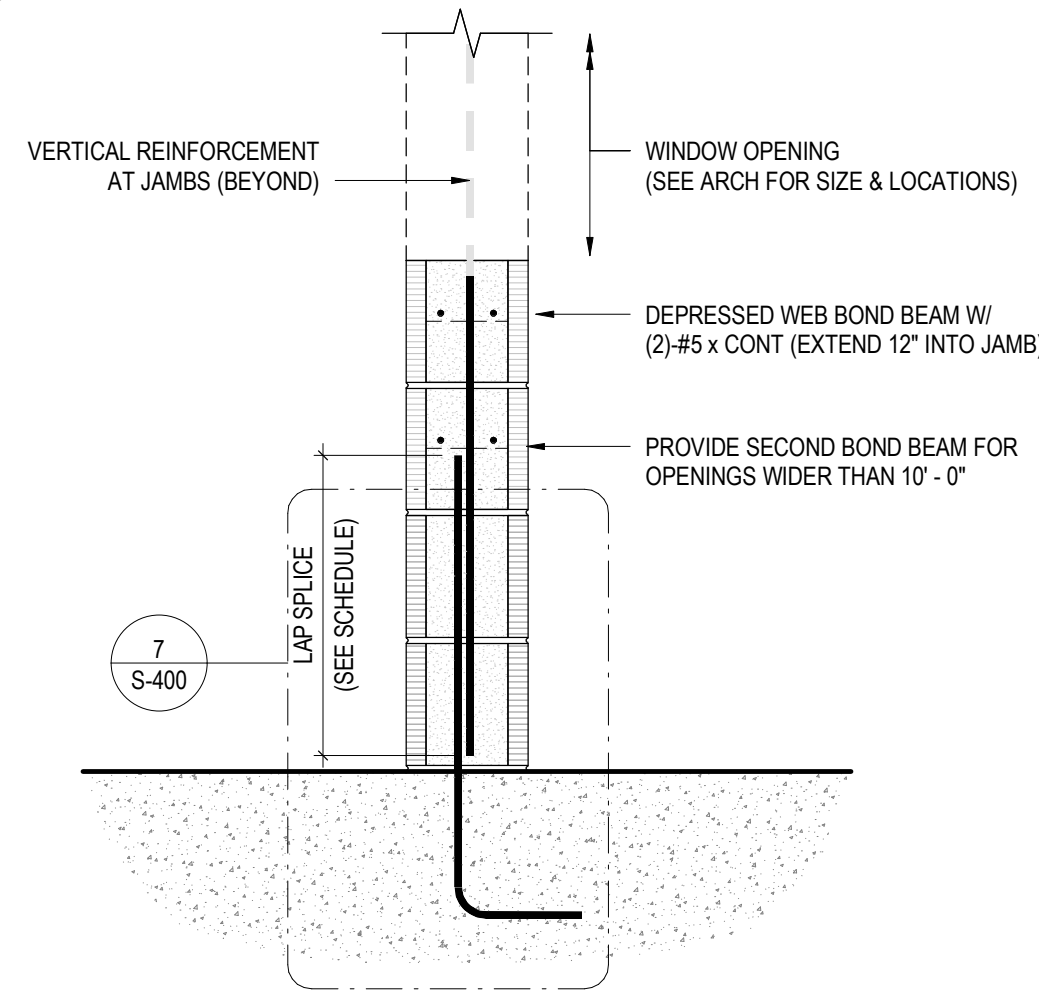


NOTES:
1. CONTROL JOINTS SHALL BE LOCATED PER ARCHITECTURAL DRAWINGS AND SHALL MATCH VENER CONTROL JOINTS WHERE APPLICABLE.
2. CONTROL JOINTS SHALL NOT BE LOCATED THROUGH OPENINGS OR LINTEL BEARINGS LENGTHS EACH SIDE OF OPENINGS UNLESS SPECIFICALLY NOTED OTHERWISE.
3. WHERE NOT EXPLICITLY NOTED OR SHOWN OTHERWISE, LOCATE CONTROL JOINTS AS FOLLOWS:
• SPACING NOT TO EXCEED 20'-0" IN CONTINUOUS WALLS
• LOCATED NOT FURTHER THAN 10'-0" FROM WALL CORNERS
• AT LOCATIONS WHERE WALL HEIGHT CHANGES
• AT LOCATIONS WHERE WALL THICKNESS CHANGES
4. GC SHALL SUBMIT SHOP DRAWINGS OF CONTROL JOINT LAYOUTS PRIOR TO OR ALONG WITH FOUNDATION REBAR SHOP DRAWINGS FOR APPROPRIATE COORDINATION WITH DOWEL PLACEMENT.

3 CMU CONTROL JOINTS
1" = 1'-0"



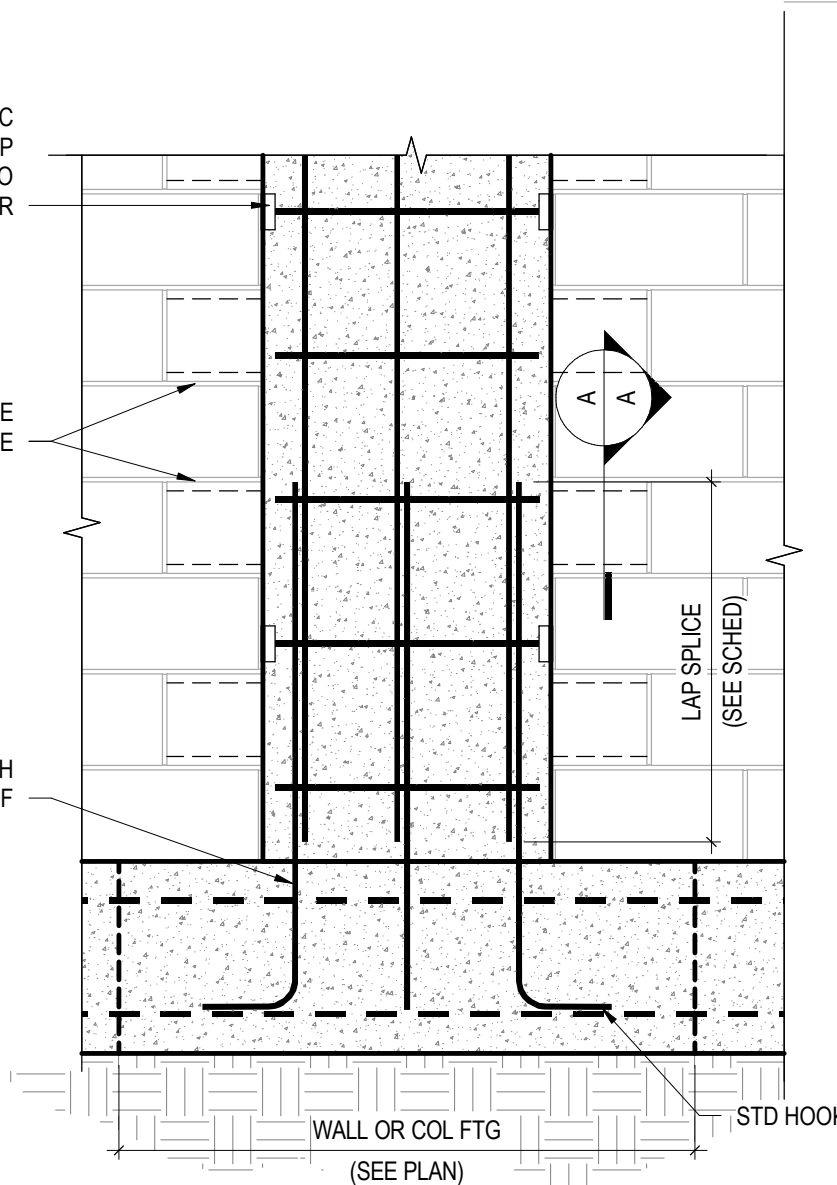
4 TYPICAL CMU JOINT REINFORCEMENT
3/4" = 1'-0"



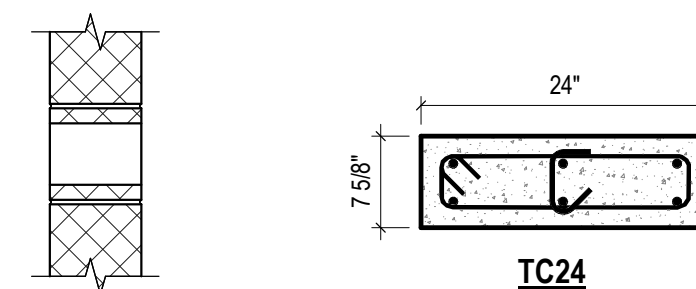
PROVIDE REINF CHAIR OR PLASTIC SPACER AT FOUR SIDES (AT TOP AND MID-HEIGHT) MIN TO MAINTAIN REQ CONC COVER

TURN 1/2 BLOCK ON EDGE EVERY SECOND COURSE

DOWELS TO MATCH COLUMN REINF



5 CMU PRECAST LINTEL
3/4" = 1'-0"

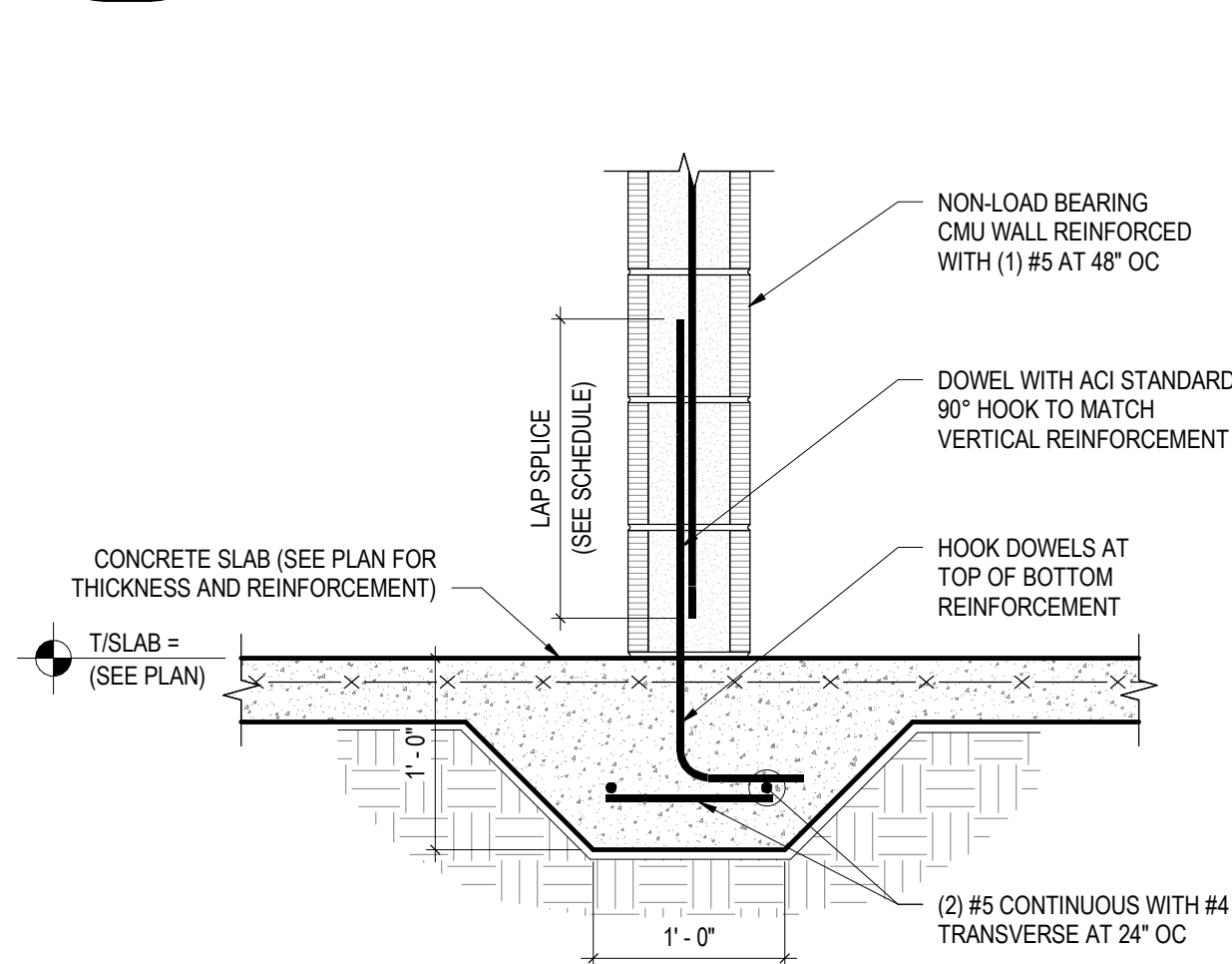


SECTION A-A

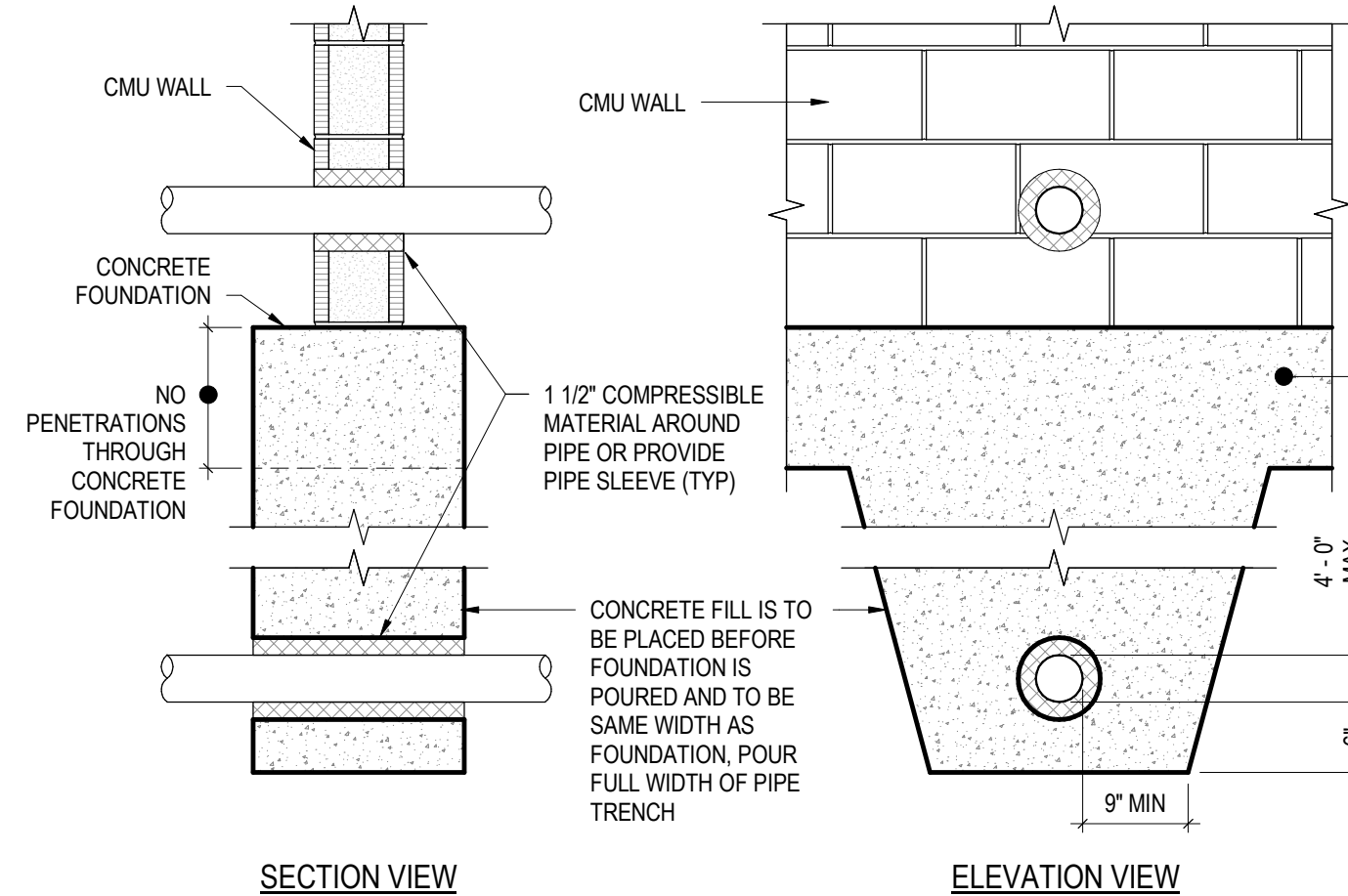
CONCRETE TIE-COLUMN SCHEDULE					
MARK	GEOMETRY	DEPTH	REINFORCING	TIE	REMARKS
TC24	24"	24"	(6) #6	#3 @ 8" OC	

NOTE:
1. EXTEND COLUMN TO UPPERMOST TIE BEAM AND TERMINATE VERTICAL REINFORCEMENT WITH STANDARD HOOK 3" FROM TOP OF THE BEAM

6 TYPICAL CMU LINTEL AT OPENING
1" = 1'-0"



7 BASE-OF-CMU WALL CONNECTION
1" = 1'-0"



8 CMU SILL - SECTION
1" = 1'-0"

9 TYPICAL CONCRETE TIE COLUMN
3/4" = 1'-0"

10 THICKENED SLAB AT CMU WALL
1" = 1'-0"

11 PIPE PENETRATION THROUGH WALL OR BELOW FOUNDATION
3/4" = 1'-0"

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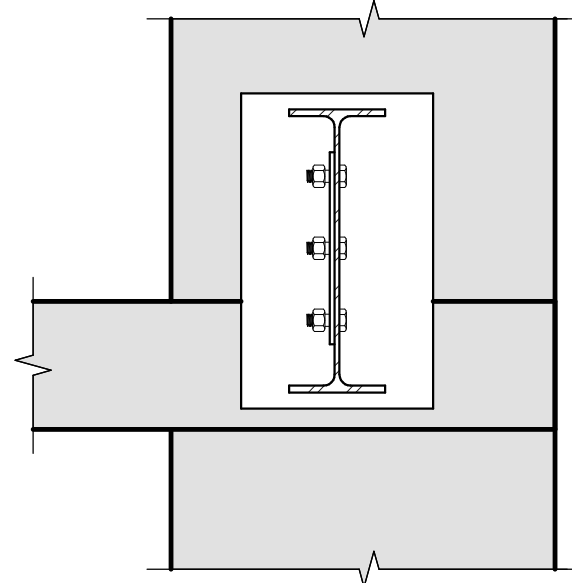
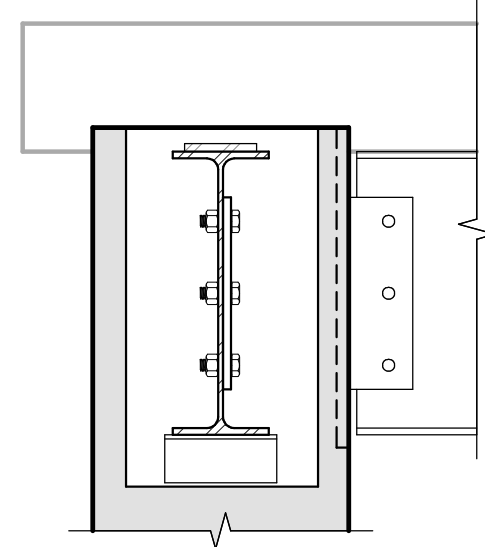
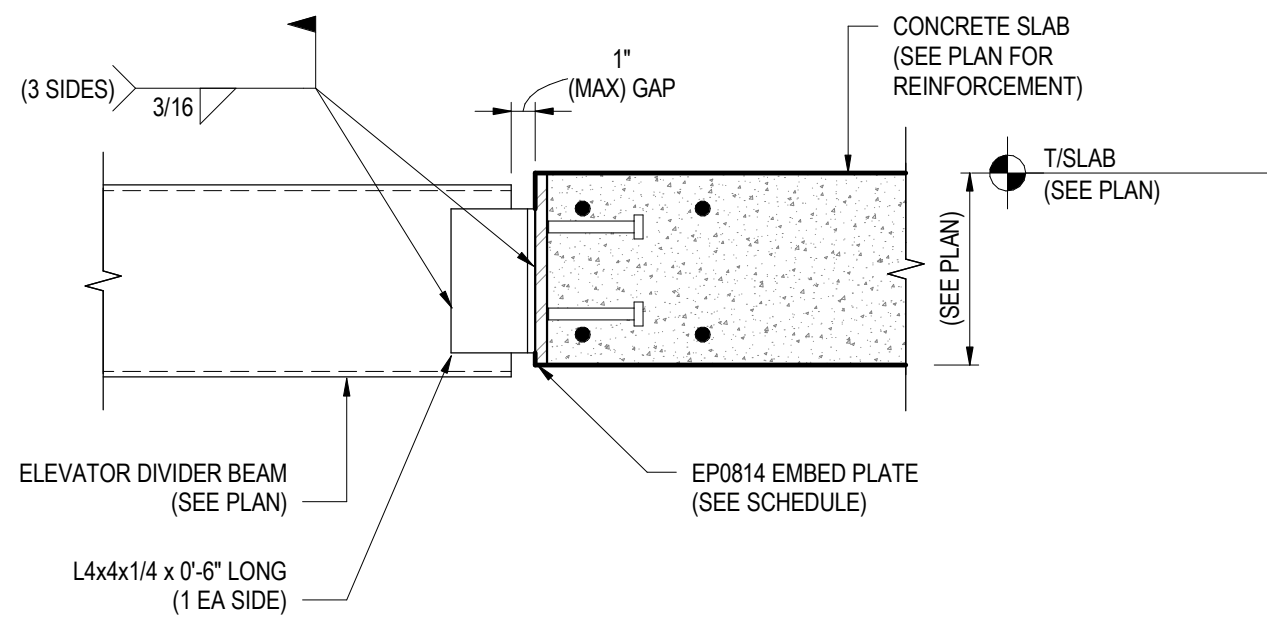
TYPICAL
MASONRY
DETAILS

S-400

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1
S-500

DIVIDER BEAM CONNECTION

NTS

2
S-500

STEEL BEAM TO COLUMN CONNECTION AT
PORTE COCHERE

1" = 1'-0"

3
S-500

STEEL BEAM TO TIE COLUMN
CONNECTION AT PORTE COCHERE

1" = 1'-0"

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STEEL
FRAMING
DETAILS

S-500

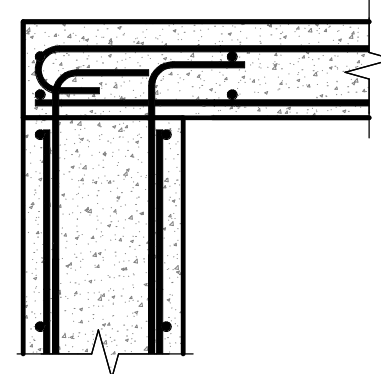
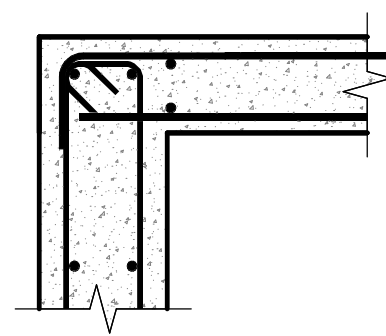
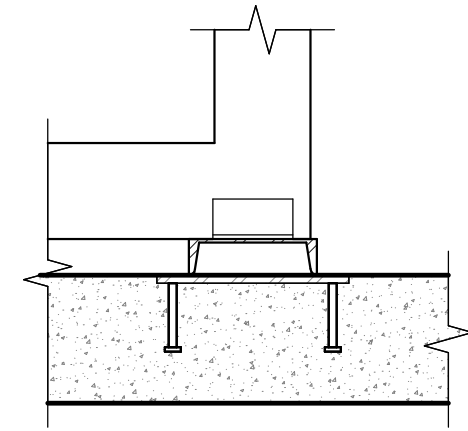


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1
S-600

CFS ROOF TRUSS CONNECTION TO SLAB

 $1'' = 1'-0''$

2
S-600

CO
1" = 1'-0"

ELEVATOR POP-UP CAP-TO-BEAM CONNECTION

 $1'' = 1'-0''$ 

WA
1" = 1'-0"

ELEVATOR POP-UP CAP CONNECTION TO WALL

$$1^* = 1'-0$$


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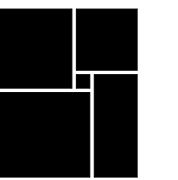
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ROOF FRAMING DETAILS

S-600