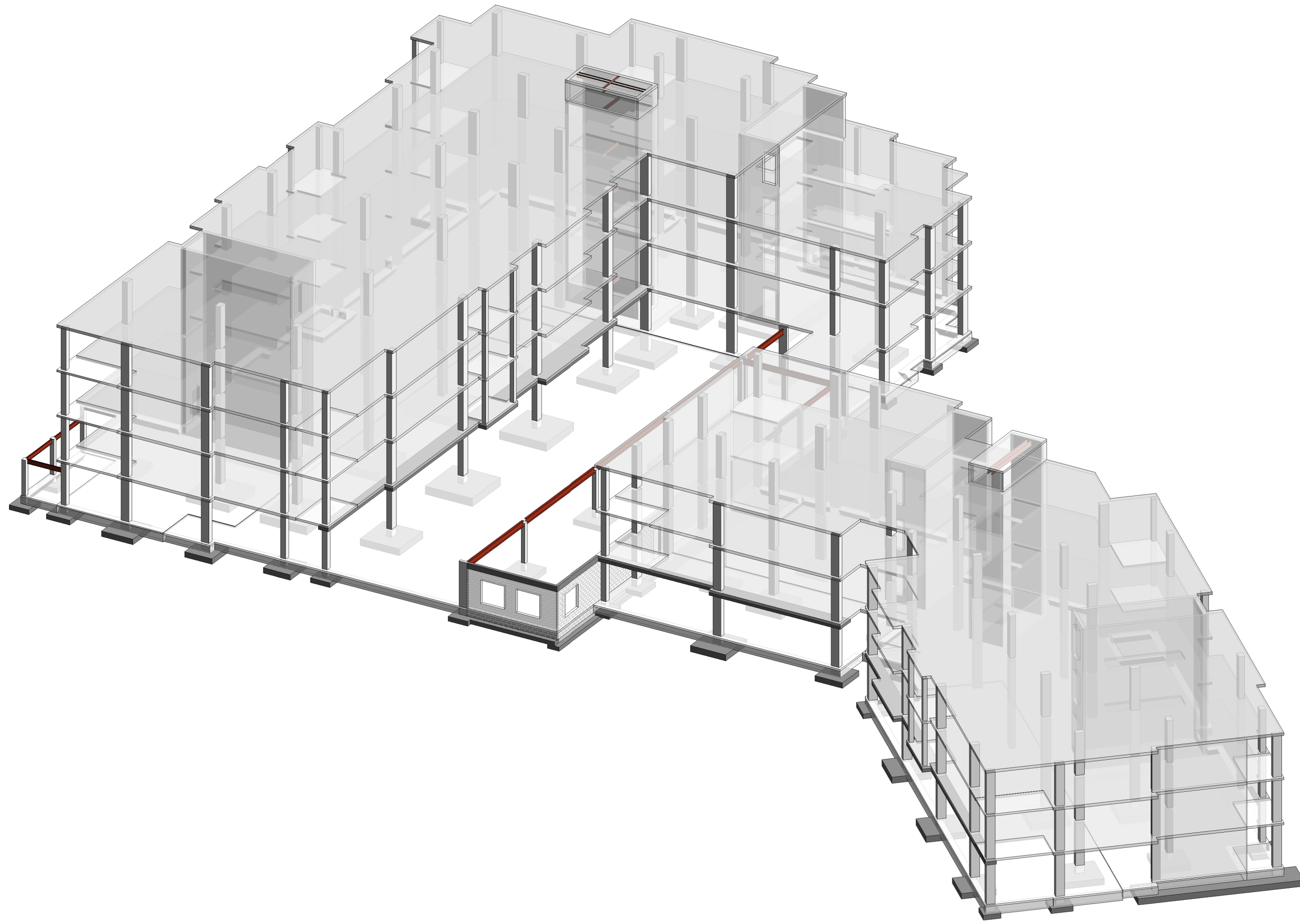


3D
S-001

ISOMETRIC VIEW - FOR REFERENCE ONLY



EARTHWORK & FOUNDATIONS

- A. GEOTECHNICAL INVESTIGATION REPORT
- FOUNDATION DESIGN IS BASED ON THE GEOTECHNICAL INVESTIGATION REPORT AS FOLLOWS:
 - REPORT TITLE: REPORT OF GEOTECHNICAL SERVICES - THREE RESIDENTIAL BUILDINGS
 - PROJECT ADDRESS: 601 UNIVERSE BOULEVARD, JUNO BEACH, FLORIDA 33408
 - REPORT NO. 25-239
 - DATED: SEPTEMBER 5, 2025
 - THE GEOTECHNICAL INVESTIGATION REPORT IS AVAILABLE TO THE CONTRACTOR UPON REQUEST TO THE OWNER. THE INFORMATION HEREIN MAY BE USED BY THE CONTRACTOR FOR HIS GENERAL REFERENCE ONLY. THE GEOTECHNICAL INVESTIGATION REPORT RECOMMENDATIONS SHALL SUPERSEDE THE MINIMUM CRITERIA STATED IN THE STRUCTURAL GENERAL NOTES.
- B. SHALLOW FOUNDATIONS
- FOUNDATIONS ARE DESIGNED IN ACCORDANCE WITH THE GEOTECHNICAL INVESTIGATION REPORT.
 - FOUNDATION SIZES AND REINFORCEMENT ARE BASED ON AN ALLOWABLE BEARING PRESSURE OF 4,200 PSF PER THE GEOTECHNICAL INVESTIGATION REPORT.
 - FOUNDATIONS SHALL BEAR A MINIMUM OF 2'-0" BELOW ADJACENT EXTERIOR GRADE.
 - FOUNDATIONS SHALL BEAR ON SOIL THAT HAS BEEN IMPROVED THROUGH VIBRO-FLOTATION PER THE GEOTECHNICAL REPORT.
 - PRIOR TO PLACEMENT OF CONCRETE, A QUALIFIED GEOTECHNICAL ENGINEER SHALL VERIFY SOILS CONFORMANCE TO THE RECOMMENDATIONS AND ASSUMPTIONS IN THE GEOTECHNICAL INVESTIGATION REPORT. ALL ADVERSE CONDITIONS SHALL BE REPORTED TO THE ARCHITECT/ STRUCTURAL ENGINEER OF RECORD.
 - SOILS BELOW FOUNDATIONS NOT MEETING DESIGN BEARING PRESSURE SHALL BE REMEDIATED PER THE GEOTECHNICAL INVESTIGATION REPORT AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF THE FOUNDATIONS.
 - CENTER ALL FOUNDATIONS UNDER THEIR RESPECTIVE COLUMNS OR WALLS, UNLESS NOTED OTHERWISE.
 - TOP OF FOUNDATION ELEVATIONS PROVIDED ON THE CONTRACT DRAWINGS ARE FOR PURPOSE OF THE CONTRACT AND SHALL BE ADJUSTED, AS REQUIRED, AT THE TIME OF EXCAVATION TO BEAR ON PROPERLY PREPARED SUPPORT SUBGRADE (PER THE GEOTECHNICAL ENGINEER'S RECOMMENDATIONS).
- C. EARTHWORK AND EXCAVATION
- THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING, BUT NOT LIMITED TO, LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS, AND UTILITIES IN ACCORDANCE WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT AND OSHA REGULATIONS.
 - EXCAVATION SHALL NOT OCCUR WITHIN ONE FOOT OF THE ANGLE OF REPOSE OF ANY SOIL BEARING FOUNDATION UNLESS THE FOUNDATION IS PROTECTED AGAINST SETTLEMENT.
 - THE EXTENT OF SUBGRADE PREPARATION SHALL EXTEND A MINIMUM OF 3'-0" BEYOND THE BUILDING PERIMETER.
 - THE CONTRACTOR SHALL PROVIDE A SUBGRADE BENEATH THE SLAB-ON-GROUND PER THE GEOTECHNICAL ENGINEER'S RECOMMENDATIONS.
 - UNLESS NOTED IN THE GEOTECHNICAL INVESTIGATION REPORT, COMPACT FILL TO 95% OF MAXIMUM DRY DENSITY AS DETERMINED BY MODIFIED PROCTOR ASTM D-1557. EACH LAYER SHALL NOT EXCEED 8" LOOSE THICKNESS. COMPACT PRIOR TO THE PLACEMENT OF THE NEXT LAYER. COMPACTOR SHALL MEET ALL RECOMMENDATIONS OF THE GEOTECHNICAL INVESTIGATION REPORT.
 - PLACEMENT OF FILL AND COMPACTATION SHALL BE MONITORED AND ACCEPTED BY A RETAINED TESTING AGENCY. PERFORM A MINIMUM OF ONE FIELD DENSITY TEST (ASTM D-1556 OR D-4938) FOR EVERY 2,500 SQUARE FEET OF EACH LAYER. THE TESTING AGENCY SHALL RANDOMLY SELECT TEST LOCATIONS.
 - THE CONTRACTOR SHALL DETERMINE THE EXTENT OF THE CONSTRUCTION DEWATERING SYSTEMS REQUIRED FOR THE EXCAVATION. AT A MINIMUM, THE CONTRACTOR SHALL PROVIDE POSITIVE DRAINAGE AWAY FROM THE BUILDING SITE.
 - THE CONTRACTOR SHALL SUBMIT CONSTRUCTION DEWATERING PLAN TO THE GEOTECHNICAL ENGINEER FOR APPROVAL PRIOR TO BEGINNING EXCAVATION.
 - THE CONTRACTOR SHALL INSTALL ALL NECESSARY DEWATERING SYSTEMS.
- D. RETAINING WALL
- RETAINING WALLS SHALL BE DESIGNED FOR THE FOLLOWING:
 - LATERAL EARTH PRESSURE AT REST: $\frac{1}{2}$ PSF PER FOOT OF DEPTH
 - ACTIVE EARTH PRESSURE: $\frac{1}{2}$ PSF PER FOOT OF DEPTH
 - PASSIVE EARTH PRESSURE RESISTANCE: $\frac{1}{2}$ PSF PER FOOT OF DEPTH
 - SOIL FRICTION FACTOR
 - RETAINING WALL DESIGN IS BASED ON DRAINED BACKFILL WITH NO BUILDUP OF WATER. THE CONTRACTOR SHALL PROVIDE A DRAINAGE SYSTEM IN ALL BACKFILL CONDITIONS (SEE CIVIL ARCHITECTURAL DRAWINGS FOR DRAINAGE SPECIFICATIONS).
 - DO NOT BACKFILL AGAINST CANTILEVERED RETAINING WALLS UNTIL CONCRETE COMPRESSIVE STRENGTH, f_c , REACHES ITS 28 DAY DESIGN STRENGTH. DO NOT BACKFILL PRIOR TO WATERPROOFING AND INSPECTION.

GENERAL REQUIREMENTS

- A. THE GENERAL STRUCTURAL NOTES EMPLOY THE FOLLOWING DEFINITIONS AND ABBREVIATIONS:
- CONTRACT DOCUMENTS - THE LATEST SET OF DRAWINGS, SPECIFICATIONS, AND RECORDED ADDENDA AND PROJECTS ADDITIONS TO THE PROJECT, OR THE COMBINATION OF THE PROJECT BY OTHERS. ELECTRONIC FILES OF THE STRUCTURAL DOCUMENTS REMAIN THE PROPERTY OF JEZERINAC GROUP AND IN NO CASE SHALL THEIR TRANSFER BE CONSIDERED A SALE.
 - THE USE OF ELECTRONIC FILES OR REPRODUCTIONS OF THESE CONTRACT DOCUMENTS BY ANY CONTRACTOR, SUBCONTRACTOR, ERECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFIES THEIR ACCEPTANCE OF ALL INFORMATION SHOWN HEREIN AS CORRECT AND OBLIGATES THEMSELVES TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS OR OMISSIONS THAT MAY OCCUR HEREIN. THE USE OF ELECTRONIC FILES DOES NOT RELIEVE THE CONTRACTOR'S RESPONSIBILITY FOR PROPER CHECKING AND COORDINATION OF DIMENSIONS, DETAILS, SIZE, AND QUANTITIES.
 - WHEN USED IN CONNECTION WITH SHOP DRAWINGS, ALL INFORMATION NOT APPLICABLE TO THE SUBCONTRACT SHALL BE REMOVED FROM THE DRAWINGS, INCLUDING, BUT NOT LIMITED TO: SHEET NUMBERS, SECTION MARKS, TITLE BLOCKS, AND REFERENCES TO THE CONTRACT DOCUMENTS.
- B. SUBMITTALS
- REFER TO DIVISION 01 OF SPECIFICATIONS FOR SUBMITTAL PROCEDURES AND REQUIREMENTS. REFER TO THE APPLICABLE SPECIFICATION SECTIONS FOR TECHNICAL CONTENT.
 - SUBMIT SPECIFIC COMPONENTS SUCH AS COLUMNS, FOUNDATIONS, ETC., IN A SINGLE PACKAGE. SUBMIT SIMILAR FLOORS TOGETHER.
 - ITEM WORKING DAYS PRIOR TO SUBMITTING SHOP DRAWINGS, THE CONTRACTOR SHALL SUBMIT, FOR REVIEW AND COMMENT BY THE STRUCTURAL ENGINEER OF RECORD, A SCHEDULE WHICH DETAILS THE ESTIMATED QUANTITY OF SHOP DRAWINGS AND THE DATE THE SHOP DRAWINGS WILL BE RECEIVED BY THE STRUCTURAL ENGINEER OF RECORD. THE STRUCTURAL ENGINEER OF RECORD SHALL HAVE THE OPPORTUNITY TO REVIEW THE PROPOSED SCHEDULE AND SUBMIT COMMENTS TO THE CONTRACTOR. THE FINAL SHOP DRAWING SCHEDULE SHALL BE DEVELOPED AND SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD. IN ACCORDANCE WITH THE SHOP DRAWING SCHEDULE, THE STRUCTURAL ENGINEER OF RECORD WILL RETURN THE SHOP DRAWINGS WITHIN TEN WORKING DAYS AFTER HAVING RECEIVED THE REPRODUCIBLE SHOP DRAWING.
 - THE CONTRACTOR SHALL REVIEW EACH SUBMITTAL PRIOR TO FORWARDING TO ARCHITECT AND STRUCTURAL ENGINEER OF RECORD. THE CONTRACTOR SHALL STAMP EACH SUBMITTAL VERIFYING THAT THE FOLLOWING IS ADDRESSED:
 - THE SUBMITTAL IS REQUESTED.
 - THE SUBMITTAL IS BASED ON THE LATEST DESIGN.
 - THE SUBMITTAL IS CLEARLY CLOUDED FOR ALL THE DIFFERENCES FROM THE CONTRACT DOCUMENTS ON THE FIRST SUBMITTAL.
 - THE SUBMITTAL IS CLEARLY CLOUDED FOR ALL CHANGES AND ADDITION FROM PREVIOUS SUBMITTALS.
 - THE ARCHITECT'S AND STRUCTURAL ENGINEER OF RECORD'S COMMENTS FROM ANY PREVIOUS SUBMITTALS ARE ADDRESSED.
 - THE WORK IS COORDINATED AMONGST ALL CONSTRUCTION TRADES.
 - THE SUBMITTAL IS COMPLETE.
 - THE SUBMITTAL SHALL INCLUDE A STAMP INDICATING PROJECT NAME AND LOCATION, SUBMITTAL NUMBER, AND SPECIFICATION SECTION NUMBER.
 - THE STRUCTURAL ENGINEER OF RECORD'S REVIEW OF SUBMITTALS SHALL BE FOR GENERAL CONFORMANCE WITH THE DESIGN INTENT.
 - THE CONTRACTOR HAS NOT STAMPED OR WHICH DO NOT MEET THE ABOVE REQUIREMENTS. SUBMITTALS WHICH THE CONTRACTOR HAS NOT STAMPED OR WHICH DO NOT MEET THE ABOVE REQUIREMENTS, THE CONTRACTOR SHALL NOT OVERLOAD THE STRUCTURE DURING CONSTRUCTION. THE CONTRACTOR SHALL RETAIN A LICENSED PROFESSIONAL ENGINEER TO PROVIDE THE ANALYSIS AND DESIGN NECESSARY TO DETERMINE POTENTIALLY OVERLOADED, UNSTABLE, OR HAZARDOUS CONDITIONS THAT MAY OCCUR AT ANY STAGE DURING CONSTRUCTION. THE CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS AND COORDINATE WITH THE CONTRACT DOCUMENTS AND SHOP DRAWINGS.
 - THE CONTRACTOR SHALL NOT EMPLOY CONSTRUCTION MEANS OR METHODS THAT MAY DAMAGE UTILITIES, ADJACENT BUILDINGS, OR PROPERTY. DOCUMENTATION OF ADJACENT CONDITIONS PRIOR TO CONSTRUCTION IS RECOMMENDED. FURTHER, THE CONTRACTOR SHALL EITHER ADEQUATELY CONFINE THE SITE OR PROTECT ADJACENT UTILITIES, STRUCTURES, OR PROPERTY.
 - THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR PROJECT SAFETY AND OSHA REQUIREMENTS. SHOULD THE STRUCTURAL ENGINEER OF RECORD NOTIFY THE CONTRACTOR OF A POTENTIALLY UNSAFE CONDITION, IT IS SOLELY AS A COURTESY FROM ONE PROFESSIONAL TO ANOTHER. IT SHOULD NOT BE INTERPRETED AS THE STRUCTURAL ENGINEER OF RECORD ASSUMING ANY RESPONSIBILITY FOR PROJECT SAFETY.
 - ALL STRUCTURES REQUIRE PERIODIC MAINTENANCE TO EXTEND LIFE SPAN AND ENSURE STRUCTURAL INTEGRITY FROM EXPOSURE TO THE ENVIRONMENT. A PLANNED PROGRAM OF MAINTENANCE SHALL BE ESTABLISHED BY THE BUILDING OWNER. THIS PROGRAM SHALL INCLUDE, BUT NOT BE LIMITED TO: PAINTING OF STRUCTURAL STEEL, PROTECTIVE COATINGS FOR CONCRETE, SEALANTS, CAULKED JOINTS, EXPANSION JOINTS, CONTRACTION JOINTS, SPALLS AND CRACKS IN CONCRETE, AND PRESSURE WASHING OF EXPOSED STRUCTURAL ELEMENTS EXPOSED TO A SALINE OR OTHER HARSH CHEMICAL ENVIRONMENT.
 - THE USE OF DE-ICING CHEMICALS ON ANY EXPOSED STRUCTURAL ELEMENT IS DISCOURAGED AND WILL ACCELERATE DETEIORATION OF STRUCTURAL ELEMENTS.
 - THE BUILDING OWNER SHALL NOT ALTER OR MODIFY ANY STRUCTURAL ELEMENT WITHOUT CONSULTING A LICENSED PROFESSIONAL ENGINEER. FURTHER, BUILDING OWNER SHALL NOT RENOVATE, REPAIR/USE, ADD-ON TO, OR OTHERWISE MODIFY THE EXISTING STRUCTURAL SYSTEMS WITHOUT CONSULTING A LICENSED PROFESSIONAL ENGINEER.
 - CONTRACT DRAWINGS SHOW MAJOR OPENINGS IN FLOORS AND WALLS AND DO NOT NECESSARILY SHOW ALL OPENINGS REQUIRED. THE CONTRACTOR SHALL COORDINATE ALL OPENING SIZES AND LOCATIONS BETWEEN ALL DISCIPLINES AND TRADES. ADDITIONAL OPENINGS, BLOCKOUTS, AND SLEEVES MAY BE REQUIRED AND SHALL BE CONSTRUCTED USING THE TYPICAL DETAILS AND/OR REQUIREMENTS WITHIN THE CONTRACT DOCUMENTS. OPENINGS REQUIRED, BUT NOT SHOWN ON THE STRUCTURAL DRAWINGS, MUST BE APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
 - THE CONTRACTOR SHALL COORDINATE PIPING AND CONDUIT EMBEDDED IN OR ATTACHED TO SLABS, SLAB-ON-DECK BEAMS, AND COLUMNS. ANY REQUIRED MODIFICATIONS TO STRUCTURAL MEMBERS OR THEIR REINFORCEMENT AS A RESULT OF EMBEDMENT OR ATTACHMENT MUST BE SUBMITTED TO THE DESIGN TEAM FOR THEIR REVIEW. SEE GENERAL STRUCTURAL NOTES SECTION "DESIGN CRITERIA" FOR LIMITATIONS OF MEP LOADING ON STRUCTURAL SYSTEMS.
 - THE STRUCTURAL ENGINEER OF RECORD'S ROLE DURING CONSTRUCTION
 - THE STRUCTURAL ENGINEER OF RECORD SHALL NOT ASSUME CONTROL OF, OR RESPONSIBILITY FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, PROJECT SAFETY, THE ACTS AND OMISSIONS OF THE CONTRACTOR, OR THEIR FAILURE TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
 - STRUCTURAL ENGINEER OF RECORD SHALL NOT HAVE AUTHORITY TO STOP THE WORK OR AUTHORIZE CHANGES TO ANY CONTRACT SUM.
 - PERIODIC SITE VISITS BY REPRESENTATIVES OF THE STRUCTURAL ENGINEER OF RECORD ARE SOLELY FOR THE PURPOSE OF BECOMING GENERALLY FAMILIAR WITH THE PROGRESS AND QUALITY OF THE WORK AND DETERMINING, IN GENERAL, IF THE WORK OBSERVED IS BEING PERFORMED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THIS LIMITED OBSERVATION SHOULD NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS AND THAT OBSERVATIONS ARE QUALITATIVE, NOT QUANTITATIVE. THIS LIMITED INFORMATION WILL BE USED TO ADVISE THE OWNER/CONTRACTOR/ARCHITECT OF POTENTIAL DEFICIENCIES.
 - CLARIFICATION OF POSITION OF STRUCTURALLY FRAMING ELEMENTS
 - USE ONLY DIMENSIONS INDICATED ON THE DRAWINGS. DO NOT SCALE ANY DIMENSIONS.
 - IF NOT INDICATED ON DRAWINGS, ASSUME EQUAL SPACING BETWEEN ESTABLISHED DIMENSIONS.
 - CENTER LINES OF COLUMNS AND FOUNDATIONS SHALL COINCIDE WITH GRID LINE INTERSECTION UNLESS NOTED OTHERWISE.
 - CENTER LINES OF FOUNDATIONS, GRADE BEAMS, AND WALLS SHALL COINCIDE WITH CENTER LINES OF FOUNDATIONS, UNLESS NOTED OTHERWISE.
 - CENTER LINES OF FRAMING MEMBERS SHALL COINCIDE WITH COLUMN CENTER LINES, UNLESS NOTED OTHERWISE.
 - ELEVATIONS SHOWN ARE TO TOP OF FOUNDATIONS, SLABS, OR BEAMS, UNLESS NOTED OTHERWISE.
 - SEE ARCHITECTURAL, CIVIL, MEP, AND VERTICAL TRANSPORTATION CONTRACT DOCUMENTS FOR ADDITIONAL INFORMATION RELATING TO THE COORDINATION OF STRUCTURAL COMPONENTS INCLUDING, BUT NOT LIMITED TO:
 - CIVIL
 - SITING OF BUILDING GRID LINES WITH RESPECT TO CITY BENCHMARKS
 - SITE PREPARATION
 - BACKFILLING MATERIALS AND REQUIREMENTS INCLUDING DRAINAGE ADJACENT TO RETAINING WALLS
 - NEW EXISTING SITE UTILITIES
 - ARCHITECTURAL
 - PLAN DIMENSIONS AND PROJECT DATUM
 - SLAB EDGE DIMENSIONS AND FINISH ELEVATIONS
 - WATERPROOFING AND DAMP PROOFING DETAILS
 - SLAB SLOPES, STEPS AND DEPRESSIONS, RAMPS, TRENCHES
 - EMBEDMENTS, INSERTS, BLOCKOUTS, ETC.
 - CONCRETE FINISHES AND TOPPING SLABS
 - CONCRETE CURBS AND HOUSEKEEPING PADS
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 - LIFE SAFETY, FIRE RATINGS
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 - MEP
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 - THIS BUILDING QUALIFIES AS A THRESHOLD BUILDING PER CHAPTER 553.71 OF THE FLORIDA STATUTES. AS SUCH, SPECIAL INSPECTIONS SHALL BE REQUIRED PER CHAPTER 553.79 OF THE FLORIDA STATUTES BY PERSONS DULY AUTHORIZED TO PERFORM THEM BY CHAPTER 553.15-55.003 OF THE FLORIDA ADMINISTRATIVE CODE. SEE SPECIAL INSPECTION PLAN FOR SPECIFIC INSPECTION REQUIREMENTS.

ELECTRONIC DATA/REPRODUCTION

- A. ALL INFORMATION CONTAINED IN THE ELECTRONIC FILES OF THE CONTRACT DOCUMENTS ARE INSTRUMENTS OF SERVICE OF THE ARCHITECT/STRUCTURAL ENGINEER OF RECORD AND SHALL NOT BE USED FOR OTHER PROJECTS, ADDITIONS TO THE PROJECT, OR THE COMBINATION OF THE PROJECT BY OTHERS. ELECTRONIC FILES OF THE STRUCTURAL DOCUMENTS REMAIN THE PROPERTY OF JEZERINAC GROUP AND IN NO CASE SHALL THEIR TRANSFER BE CONSIDERED A SALE.
- B. THE USE OF ELECTRONIC FILES OR REPRODUCTIONS OF THESE CONTRACT DOCUMENTS BY ANY CONTRACTOR, SUBCONTRACTOR, ERECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFIES THEIR ACCEPTANCE OF ALL INFORMATION SHOWN HEREIN AS CORRECT AND OBLIGATES THEMSELVES TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS OR OMISSIONS THAT MAY OCCUR HEREIN. THE USE OF ELECTRONIC FILES DOES NOT RELIEVE THE CONTRACTOR'S RESPONSIBILITY FOR PROPER CHECKING AND COORDINATION OF DIMENSIONS, DETAILS, SIZE, AND QUANTITIES.
- C. WHEN USED IN CONNECTION WITH SHOP DRAWINGS, ALL INFORMATION NOT APPLICABLE TO THE SUBCONTRACT SHALL BE REMOVED FROM THE DRAWINGS, INCLUDING, BUT NOT LIMITED TO: SHEET NUMBERS, SECTION MARKS, TITLE BLOCKS, AND REFERENCES TO THE CONTRACT DOCUMENTS.

SUBMITTALS

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- SUBMIT SPECIFIC COMPONENTS SUCH AS COLUMNS, FOUNDATIONS, ETC., IN A SINGLE PACKAGE. SUBMIT SIMILAR FLOORS TOGETHER.
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 - SITE PREPARATION
 - BACKFILLING MATERIALS AND REQUIREMENTS INCLUDING DRAINAGE ADJACENT TO RETAINING WALLS
 - NEW EXISTING SITE UTILITIES
 - ARCHITECTURAL
 - PLAN DIMENSIONS AND PROJECT DATUM
 - SLAB EDGE DIMENSIONS AND FINISH ELEVATIONS
 - WATERPROOFING AND DAMP PROOFING DETAILS
 - SLAB SLOPES, STEPS AND DEPRESSIONS, RAMPS, TRENCHES
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 - CONCRETE FINISHES AND TOPPING SLABS
 - CONCRETE CURBS AND HOUSEKEEPING PADS
 - INTERIOR NON-STRUCTURAL MASONRY PARTITIONS
 - LIFE SAFETY, FIRE RATINGS
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 - OPERABLE PARTITIONS
 - MEP
 - PIPE AND DUCT SIZES FOR OPENING AND SLEEVE COORDINATION
 - FLOOR DRAINS
 - UNDERFLOOR AND PERIMETER DRAINAGE SYSTEMS
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 - CONDUITS AND EMBEDMENTS IN WALLS AND SLABS
 - VERTICAL TRANSPORTATION
 - INSERTS, HANGERS, TRENCHES, PITS, CONDUITS IN WALLS AND SLAB
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03100	CONCRETE FORMWORK	(SS, CALC)
03200	CONCRETE REINFORCEMENT LAYOUT	(S)
03300	CONCRETE PRODUCT DATA	(S)
03300	DIMENSION PLANS AND SLEEVE LAYOUT DRAWINGS	(S)
03300	CONCRETE MIX DESIGNS	(CALC, TA)
03300	CONCRETE CONSTRUCTION JOINT LAYOUT	(S)
03300	SLAB-ON-GROUND SAWCUT CONTROL JOINT LAYOUT DRAWINGS	(S)
033816	POST-TENSIONING PRODUCT DATA	(S)
033816	POST-TENSIONING TENDON LAYOUT & INSTALLATION DRAWINGS	(DF, SS)
033816	POST TENSIONING TENDON FRICTION LOSS CALC	(DF, CALC)
033816	STRESSING RECORDS	(S)
042200	MASONRY PRODUCT DATA	(S)
042200	MASONRY REINFORCEMENT LAYOUT	(S)
051200	STRUCTURAL STEEL	(S)
051200	STRUCTURAL STEEL CONNECTIONS	(DF, S, CALC)
051300	STEEL DECK	(S)
064000	COLD-FORMED METAL FRAMING UPE FOR EXTERIOR SHORING AND RESHORING	(SS, CALC) (SEE ARCH)
064400	PRE-ENGINEERED CFS TRUSS SHOP DRAWINGS	(DF, S)
064400	PRE-ENGINEERED CFS TRUSS DELEGATED DESIGN SUBMITTAL	(DF, SS)
042000	ELEVATOR	(DF, SS, CALC, REC)
312319	DEWATERING	(GEO, REC)

- S = SHOP DRAWING REQUIRED
- DF = DEFERRED SUBMITTAL
- SS = SIGNED AND SEALED SHOP DRAWINGS PREPARED BY A LICENSED DELEGATED ENGINEER IN THE STATE IN WHICH THE PROJECT IS LOCATED.
- CALC = SUPPORTING CALCULATIONS REQUIRED, SIGNED AND SEALED BY A LICENSED DELEGATED ENGINEER IN THE STATE IN WHICH THE PROJECT IS LOCATED.
- REC = ITEMS SUBMITTED FOR RECORD ONLY AND WILL NOT HAVE STRUCTURAL ENGINEER OF RECORD SHOP DRAWING STAMP AFFIXED.
- GEO = ITEMS SUBMITTED TO CONSTRUCTION GEOTECHNICAL ENGINEER FOR THEIR REVIEW.
- TA = ITEMS SUBMITTED TO OWNER'S TESTING AGENCY FOR THEIR REVIEW.

GOVERNING CODES & STANDARDS

BUILDING CODE:	FBC 2023	FLORIDA BUILDING CODE, BUILDING
STANDARDS:	ASCE 7	AMERICAN SOCIETY OF CIVIL ENGINEERS' MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
	ACI 318	AMERICAN CONCRETE INSTITUTE: BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
	TMS 402	THE MASONRY SOCIETY' BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES
	ASCE 360	AMERICAN INSTITUTE OF STEEL CONSTRUCTION: SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
	ASCE 341	AMERICAN INSTITUTE OF STEEL CONSTRUCTION: SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS
	AWS D1.1	AMERICAN WELDING SOCIETY: STRUCTURAL WELDING CODE - STEEL
	AWS D1.3	AMERICAN WELDING SOCIETY: STRUCTURAL WELDING CODE - SHEET STEEL
	AWS D1.4	AMERICAN WELDING SOCIETY: STRUCTURAL WELDING CODE - REINFORCEMENT STEEL
	ISI 5100	AMERICAN IRON AND STEEL INSTITUTE: NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS
	AWC SPS	AMERICAN WOOD COUNCIL: NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION
	AWC SDWS	AMERICAN WOOD COUNCIL: SPECIAL DESIGN PROVISIONS FOR WIND AND SEISMIC
	ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS

ABBREVIATIONS

ADDL	ADDITIONAL	L	LENGTH
ADJ	ADJACENT	LB(S)	POUNDS(S)
AF	ABOVE FINISHED FLOOR	LL	LIVE LOAD
ALT	ALTERNATE	LLH	LONG LEGS HORIZONTAL
APPROX	APPROXIMATE	LLV	LONG LEGS VERTICAL
ARCH	ARCHITECT OR ARCHITECTURAL	LONG	LONGITUDINAL
B	ALLOWABLE STRESS DESIGN	LRFD	LOAD RESISTANCE FACTORED DESIGN
B/C	BEARING	LSH	LONG LEGS HORIZONTAL
B/	BOTTOM OF	LSV	LONG LEGS VERTICAL
B/B	BACK-TO-BACK	LTS	LAP TENSION SPlice
BLDG	BUILDING	LW	LIGHT WEIGHT
BLKG	BLOCKING	LWC	LIGHT WEIGHT CONCRETE
BRG	BRACING		
BOT	BOTTOM	M	MOMENT
BTWN	BETWEEN	MAX	MAXIMUM
		MC	MOMENT CONNECTION(S)
		MECH	MECHANICAL
		MEP	MECHANICAL, ELECTRICAL, PLUMBING, FIRE PROTECTION
		MFR	MANUFACTURER
		MID	MIDDLE
		MISC	MISCELLANEOUS
		MIN	MINIMUM
		NIC	NOT IN CONTRACT
		NS	NEAR SIDE
		NTS	NOT TO SCALE
		NWC	NORMAL WEIGHT CONCRETE
		NSFC	NOT SHOWN FOR CLARITY
		OC	ON CENTER
		OD	OUTSIDE DIAMETER
		OF	OUTSIDE FACE
		OH	OPPOSITE HAND
		OPN(S)	OPENING(S)
		OPP	OPPOSITE
		OSL	OUTSTANDING LEG
		PAF	POWDER ACTUATED FASTENER
		PERP	PERPENDICULAR
		PL	PERFORMED JOINT FILLER
		PLF	POUNDS PER LINEAL FOOT
		PCST	PRECAST
		PREFAB	PRE-FABRICATED
		PSF	POUNDS PER SQUARE FOOT
		PSI	POUNDS PER SQUARE INCH
		PT	POST-TENSIONED
		REF	REFERENCE
		REINF	REINFORCE(D) (ING) OR (MENT) REQUIRED
		REV	REVISION
		RTU	ROOF TOP UNIT
		SOHD	SCHEDULE(D)
		SOL	SUPERIMPOSED DEAD LOAD
		FS	STRUCTURAL ENGINEER OF RECORD
		SF	SQUARE FOOT (FEET)
		SIM	SIMILAR
		SLRS	SEISMIC LOAD RESISTING SYSTEM
		SG	SLAB-ON-GROUND
		SOC	SPEACE
		SPEC(S)	SPECIFICATION(S)
		SS	STAINLESS STEEL
		STD	STANDARD
		STIFFER	STIFFENER
		STIF	STIFF
		STR	STRUCTURE OR STRUCTURAL
		SYM	SYMMETRICAL
		T	TENSION
		T&B	TOP AND BOTTOM
		T&G	TONGUE & GROOVE
		T/	TOP OF
		TEMP	TEMPERATURE OR TEMPORARY
		INCH	TYPICAL
		INFO	INFORMATION
		INT	INTERIOR
		UNO	UNLESS NOTED OTHERWISE
		V	VERT
		VERT	VERTICAL
		VF	VERIFY IN FIELD
		K	KIPS (1,000 POUNDS)
		KSF	KIP PER SQUARE FOOT
		W	WITH
		W/O	WITHOUT
		WP	WORK POINT
		WWR	WELDED WIRE REINFORCEMENT

STRUCTURAL THRESHOLD INSPECTIONS

- PER CHAPTER 553.71 OF THE FLORIDA STATUTES, THIS BUILDING QUALIFIES AS A THRESHOLD BUILDING.
- SPECIAL INSPECTIONS OF THRESHOLD BUILDINGS (THRESHOLD INSPECTORS) SHALL MEET THE REQUIREMENTS OF RULE 61G15-38.003 OF THE FLORIDA ADMINISTRATIVE CODE.
- PER CHAPTER 553.79 OF THE FLORIDA STATUTES, THE ENFORCING AGENCY SHALL REQUIRE A THRESHOLD INSPECTOR TO PERFORM STRUCTURAL INSPECTIONS ON A THRESHOLD BUILDING PURSUANT TO A STRUCTURAL INSPECTION PLAN PREPARED BY THE STRUCTURAL ENGINEER OF RECORD.
- THRESHOLD INSPECTOR SHALL BE A LICENSED PROFESSIONAL ENGINEER IN THE STATE OF FLORIDA WHO IS CERTIFIED UNDER CHAPTER 471.015 OF THE FLORIDA STATUTES, TO CONDUCT INSPECTIONS OF A THRESHOLD BUILDING. FURTHER, THE THRESHOLD INSPECTOR MUST BE ON THE FLORIDA BOARD OF PROFESSIONAL ENGINEERS LIST OF PERSONS QUALIFIED TO BE THRESHOLD INSPECTORS.
- THE STRUCTURAL INSPECTION PLAN MUST BE SUBMITTED TO AND APPROVED BY THE ENFORCING AGENCY PRIOR TO THE ISSUANCE OF A BUILDING PERMIT FOR THE CONSTRUCTION OF A THRESHOLD BUILDING.
- THE FEE OWNER OF A THRESHOLD BUILDING SHALL SELECT AND PAY ALL COSTS OF EMPLOYING A THRESHOLD INSPECTOR, BUT THE THRESHOLD INSPECTOR SHALL BE RESPONSIBLE TO THE ENFORCING AGENCY.
- THRESHOLD INSPECTIONS RELATED TO TEMPORARY CONDITIONS SUCH AS SHORING, RESHORING, AND TEMPORARY BRACING ARE REQUIRED TO BE PERFORMED BY THE CONTRACTOR'S DELEGATED ENGINEER. DELEGATED ENGINEER (NOT SPECIAL INSPECTOR) IS RESPONSIBLE FOR THE SUPERVISION, INSPECTION AND CERTIFICATION OF SUCH TEMPORARY CONDITIONS.
- SEE STRUCTURAL INSPECTION PLAN FOR FURTHER INFORMATION.

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DESIGN CRITERIA

- A. STRUCTURE LOCATION:
LONGITUDE: -80.059
LATITUDE: 26.857
- B. LOADING:
1. DEAD & LIVE LOADS: SEE LOAD PLANS
2. RAIN LOAD:
DESIGN RAINFALL: 5"/HOUR (100-YEAR, 1-HOUR RAINFALL)
RAINFALL AT LOWEST POINT OF ROOF SHALL: NOT EXCEED 6" DURING DESIGN RAINFALL
DESIGN RAIN LOAD, R: 30 PSF
3. SNOW LOAD:
GROUND SNOW LOAD, P_g: 5 PSF
4. WIND LOAD:
ULTIMATE DESIGN WIND SPEED, V_{ult}: 170 MPH
NOMINAL DESIGN WIND SPEED, V_{nom}: 132 MPH
RISK CATEGORY: II
EXPOSURE CATEGORY: C
ENCLOSURE CLASSIFICATION: ENCLOSED
INTERNAL PRESSURE COEFFICIENT: +/- 0.18
COMPONENTS & CLADDING DESIGN PRESSURES: SEE WIND PRESSURE DIAGRAMS
5. SEISMIC LOAD:
A. AREA A
RISK CATEGORY: II
SEISMIC IMPORTANCE FACTOR, I_e: 1.0
SITE CLASS: D (ASSUMED)
MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER, S_{se}: 0.055 g
MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER, S₁: 0.025 g
SPECTRAL RESPONSE ACCELERATION PARAMETER, S_{ms}: 0.049 g
SPECTRAL RESPONSE ACCELERATION PARAMETER, S₀₁: 0.035 g
SEISMIC DESIGN CATEGORY: A
SEISMIC FORCE RESISTING SYSTEM: ORDINARY REINFORCED CONCRETE SHEAR WALLS
OVERSTRENGTH FACTOR, O_v: 2.5
RESPONSE MODIFICATION FACTOR, R: 4
SEISMIC RESPONSE COEFFICIENT, C_s: 0.015
EFFECTIVE SEISMIC WEIGHT, W: 7800 KIPS
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
DESIGN BASE SHEAR: 100 KIPS
B. AREA B
RISK CATEGORY: II
SEISMIC IMPORTANCE FACTOR, I_e: 1.0
SITE CLASS: D (ASSUMED)
MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER, S_{se}: 0.055 g
MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER, S₁: 0.025 g
SPECTRAL RESPONSE ACCELERATION PARAMETER, S_{ms}: 0.049 g
SPECTRAL RESPONSE ACCELERATION PARAMETER, S₀₁: 0.035 g
SEISMIC DESIGN CATEGORY: A
SEISMIC FORCE RESISTING SYSTEM: ORDINARY REINFORCED CONCRETE SHEAR WALLS
OVERSTRENGTH FACTOR, O_v: 2.5
RESPONSE MODIFICATION FACTOR, R: 4
SEISMIC RESPONSE COEFFICIENT, C_s: 0.015
EFFECTIVE SEISMIC WEIGHT, W: 6400 KIPS
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
DESIGN BASE SHEAR: 80 KIPS
C. HANDRAIL AND GUARDRAIL LOADS:
CONCENTRATED AND DISTRIBUTED LOADS ARE TO BE APPLIED AT THE HANDRAIL OR TOP RAIL IN ANY DIRECTION. CONCENTRATED AND DISTRIBUTED LOADS ARE NOT TO BE APPLIED CONCURRENTLY.
CONCENTRATED LOAD: 200 LB
DISTRIBUTED LOAD: 50 PLF
D. FUTURE EXPANSION:
NO PROVISIONS HAVE BEEN MADE FOR FUTURE VERTICAL OR HORIZONTAL EXPANSION OF THE STRUCTURE.
E. SERVICEABILITY:
1. DEFLECTION LIMITS: TOTAL LOAD DEFLECTION ONLY APPLIES TO THE DEFLECTION DUE TO THE CREEP COMPONENT OF LONG-TERM DEAD LOAD DEFLECTION PLUS THE SHORT-TERM DEFLECTION. LONG-TERM DEFLECTION OF WOOD STRUCTURAL MEMBERS SHALL BE CALCULATED IN ACCORDANCE WITH THE AWC NDS. IT IS PERMITTED TO ESTIMATE THE CREEP-COMPONENT OF THE LONG-TERM DEFLECTION AS THE IMMEDIATE DEAD LOAD DEFLECTION.
a. ROOF MEMBERS
• TOTAL LOAD DEFLECTION: L/240
• TRANSITORY LOAD DEFLECTION: L/360
b. FLOOR MEMBERS
• TOTAL LOAD DEFLECTION: L/240
• LIVE LOAD DEFLECTION: L/360
c. EXTERIOR WALLS & CLADDING
• WIND LOAD DEFLECTION: L/360
d. INTERIOR PARTITIONS
• LIVE LOAD DEFLECTION: L/240
2. DRIFT LIMITS
a. INTERSTORY DRIFT: H/400
b. TOTAL STRUCTURE DRIFT: H/500

C&C EXTERNAL PRESSURE LOADS (PSF)													
EVA (ft2)	ZONE												
	3	1F	2F	3F	4	5	P4	P5	ROH 1	ROH 2	ROH 3		
10	57	32	32	32	76	31	173	199	147	147	147		
	-140	-126	-167	-227	-82	-218	--	--	-166	-205	-205		
20	49	30	30	30	73	29	151	186	132	132	132		
	-121	-118	-156	-206	-79	-197	--	--	-151	-182	-182		
50	39	28	28	28	68	26	135	170	113	113	113		
	-95	-107	-142	-178	-74	-170	--	--	-131	-152	-152		
100	31	26	26	26	65	24	124	157	98	98	98		
	-76	-99	-131	-156	-71	-149	--	--	-116	-129	-129		
200	31	26	26	26	61	24	116	146	91	91	91		
	-76	-90	-120	-135	-68	-129	--	--	-113	-126	-126		
500	31	26	26	26	57	24	105	132	82	82	82		
	-76	-79	-106	-106	-63	-102	--	--	-108	-121	-121		

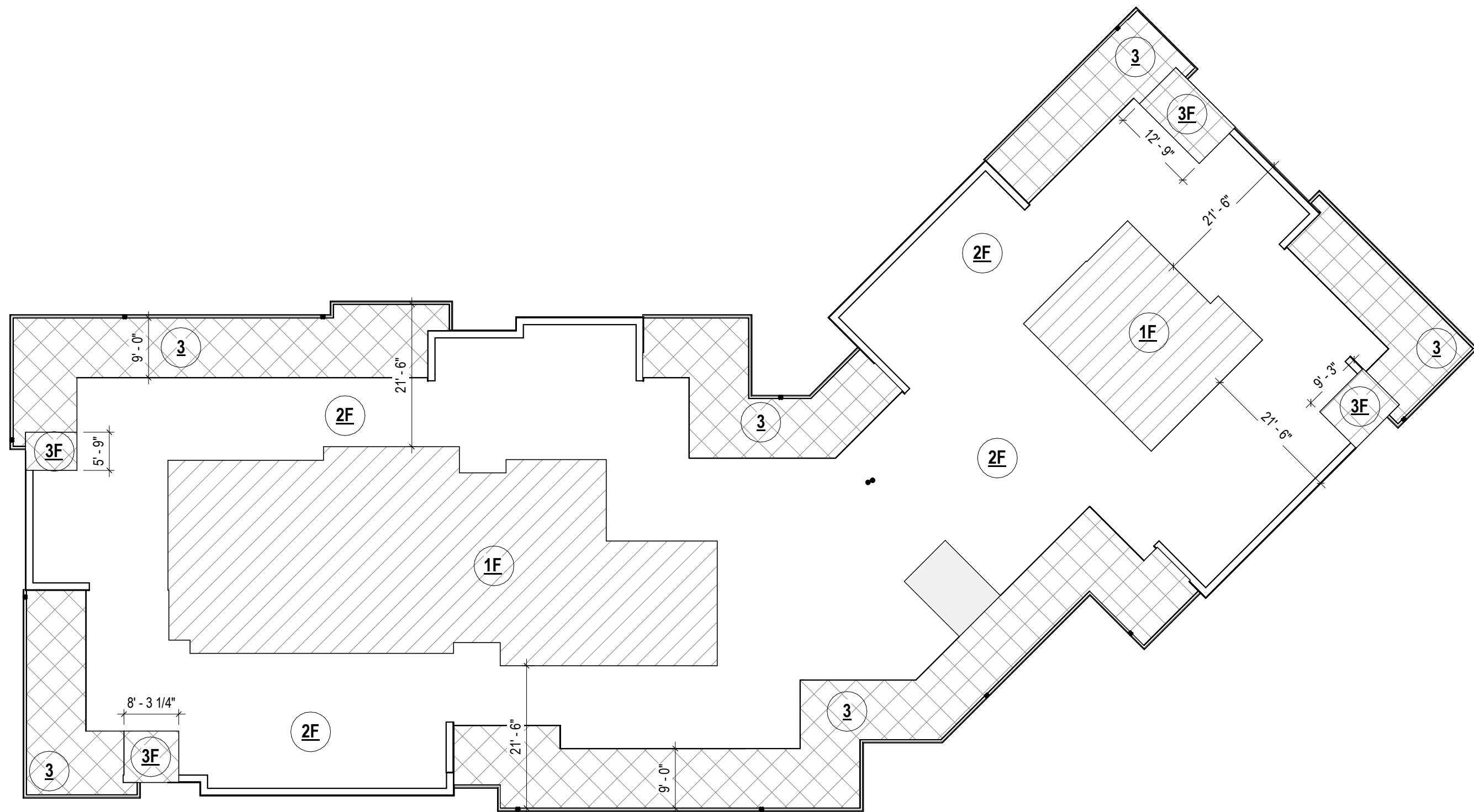
NOTES:

- FOR COMPONENTS HAVING EFFECTIVE AREAS IN BETWEEN TABULATED VALUES, DESIGN LOADS MAY BE INTERPOLATED. OTHERWISE DESIGN LOAD SHALL BE TAKEN FROM THE NEXT LOWEST TABULATED EFFECTIVE AREA.
- DESIGN VALUES SHOWN IN THIS TABLE ARE ULTIMATE VALUES FOR USE WITH LRFD DESIGN. VALUES MAY BE MULTIPLIED BY 0.6 FOR USE WITH SERVICE LEVEL OR ASD DESIGN. REFER TO THE BUILDING CODE FOR APPLICABLE LOAD COMBINATIONS.
- # + 99: SEE ROOF PLAN MAP BELOW FOR LOCATION OF # ZONES. WALL # ZONE LOCATIONS TO MATCH ROOF # ZONES.
- POSITIVE PRESSURE VALUES REFER TO FORCES ACTING TOWARDS BUILDING OR COMPONENT FACE. NEGATIVE PRESSURE VALUES REFER TO FORCES ACTING AWAY FROM BUILDING OR COMPONENT FACE.
- EACH COMPONENT AND ITS CONNECTION SHALL BE DESIGNED FOR MAXIMUM POSITIVE AND NEGATIVE FORCES.
- PARAPET COMPONENTS AND CLADDING ARE THOSE ELEMENTS WHICH EXIST ABOVE THE HORIZONTAL PLANE OF THE ROOF AND SHALL BE DESIGNED FOR:
 - POSITIVE AND NEGATIVE PRESSURES 4 OR 5 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON OUTSIDE FACE.
 - POSITIVE PRESSURES 4 OR 5 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON ROOF SIDE FACE.
 - NEGATIVE PRESSURES 2 OR 3 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON ROOF SIDE FACE.
 - P4S SHALL BE APPLIED TO THE DESIGN OF THE STRUCTURAL ELEMENT OF THE PARAPET AND ITS CONNECTION, INCLUDING BUT NOT LIMITED TO THE STUD FRAMING OF THE PARAPET.
- A DESIGN WIND PRESSURE HORIZONTAL VALUE OF ____ PSF AND VERTICAL VALUE OF ____ PSF SHALL BE APPLIED TO COMPONENTS WHICH ARE EITHER ROOFTOP STRUCTURES OR ROOFTOP APPURTENANCES AND THEIR CONNECTION. EXAMPLES OF THIS ARE RTUs, AHUs, AND SCREEN WALLS.
- ROH# DENOTES DESIGN WIND PRESSURE VALUES WHICH SHALL BE APPLIED AT ROOF OVERHANGS TO TOP SURFACE CLADDING OR SHEATHING AND ITS CONNECTION. SOFFIT CLADDING OR SHEATHING SHALL BE DESIGNED FOR SIMILAR PRESSURE TO THE ADJACENT WALL PRESSURE. A COMBINATION OF THESE FORCES SHALL BE APPLIED TO THE STRUCTURAL ELEMENT OF THE OVERHANG AND ITS CONNECTION, INCLUDING BUT NOT LIMITED TO THE STUD FRAMING OF THE OVERHANG.
- ALL DOORS TO BE RATED TO RESIST DESIGN WIND PRESSURES SPECIFIED.

C&C-A
S-002A

AREA A COMPONENTS AND CLADDING DIAGRAM

1/16" = 1'-0"



C&C-B
S-002A

AREA B COMPONENTS AND CLADDING DIAGRAM

1/16" = 1'-0"

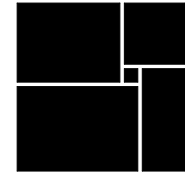
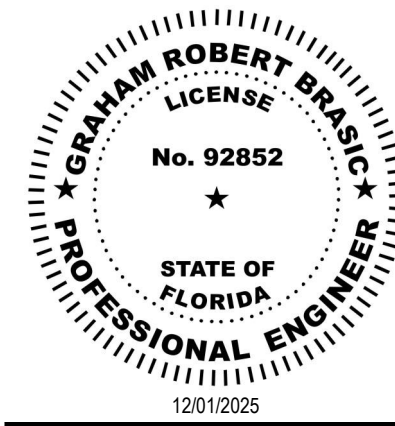


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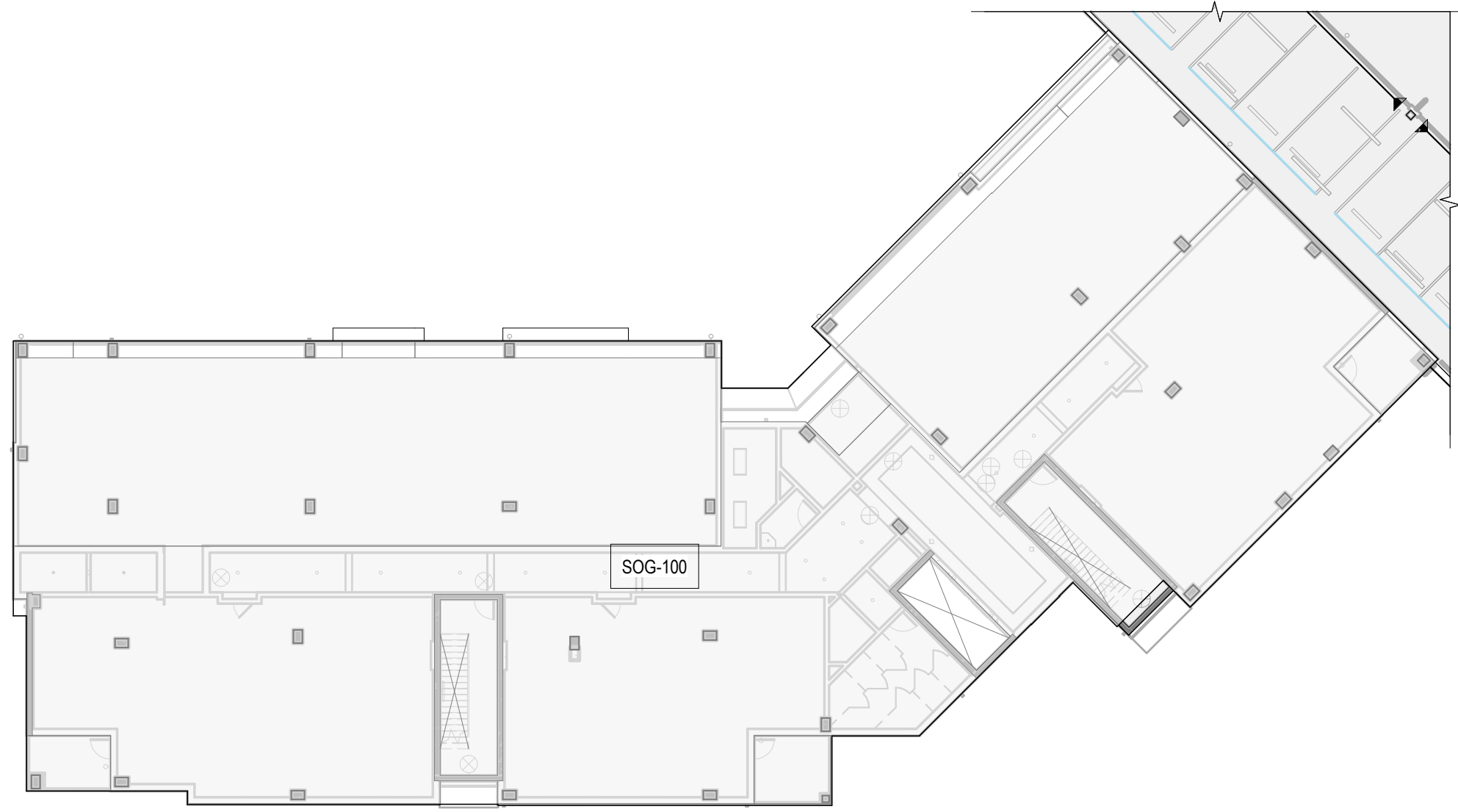
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Project No.: 2021009
Date: 12/01/2025

LOADING
CRITERIA

S-002A

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GB
S-002B

GROUND FLOOR LOAD PLAN - AREA B

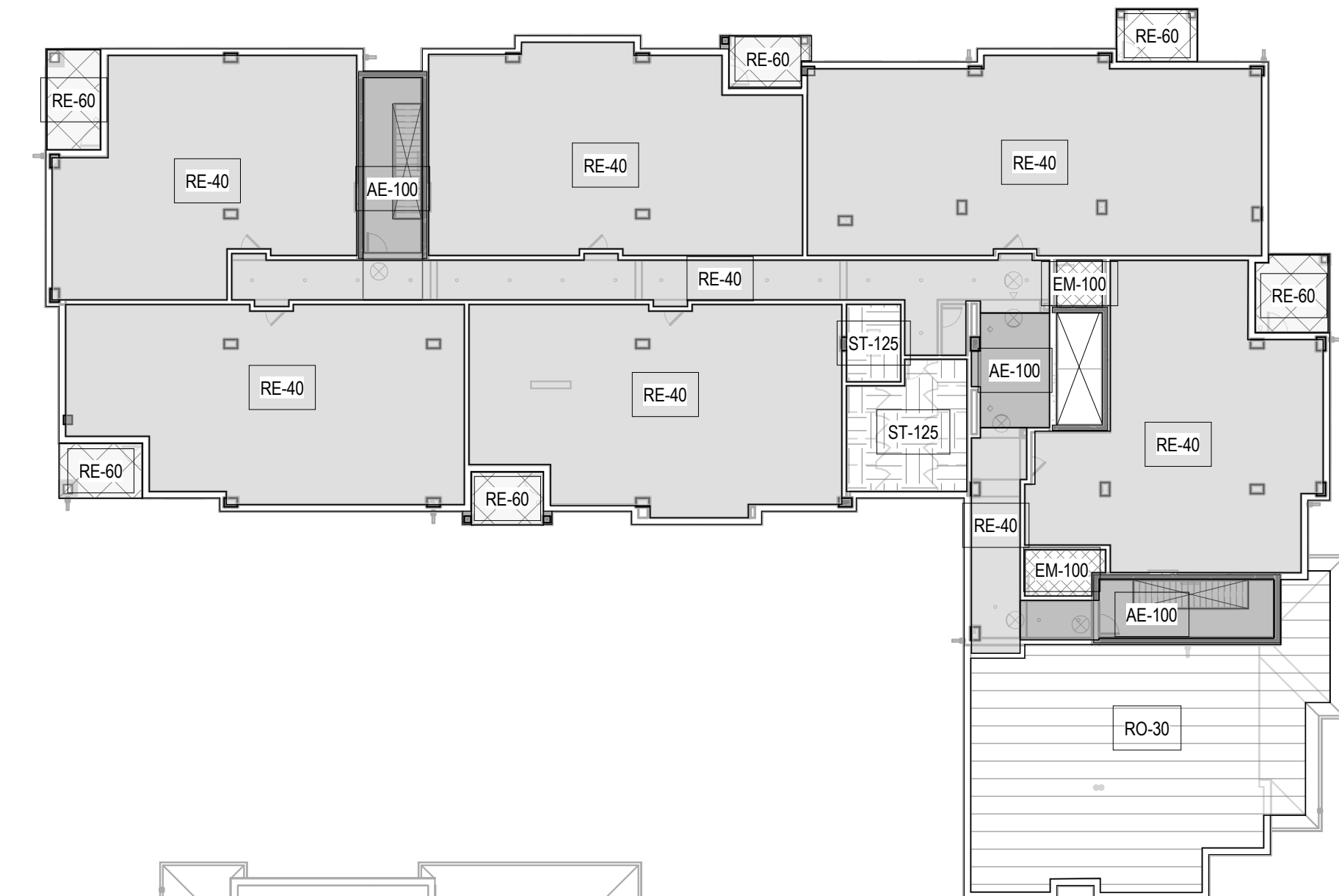
3/64" = 1'-0"



2A
S-002B

SECOND FLOOR LOAD PLAN - AREA A

3/64" = 1'-0"



4A
S-002B

FOURTH FLOOR PLAN - AREA A

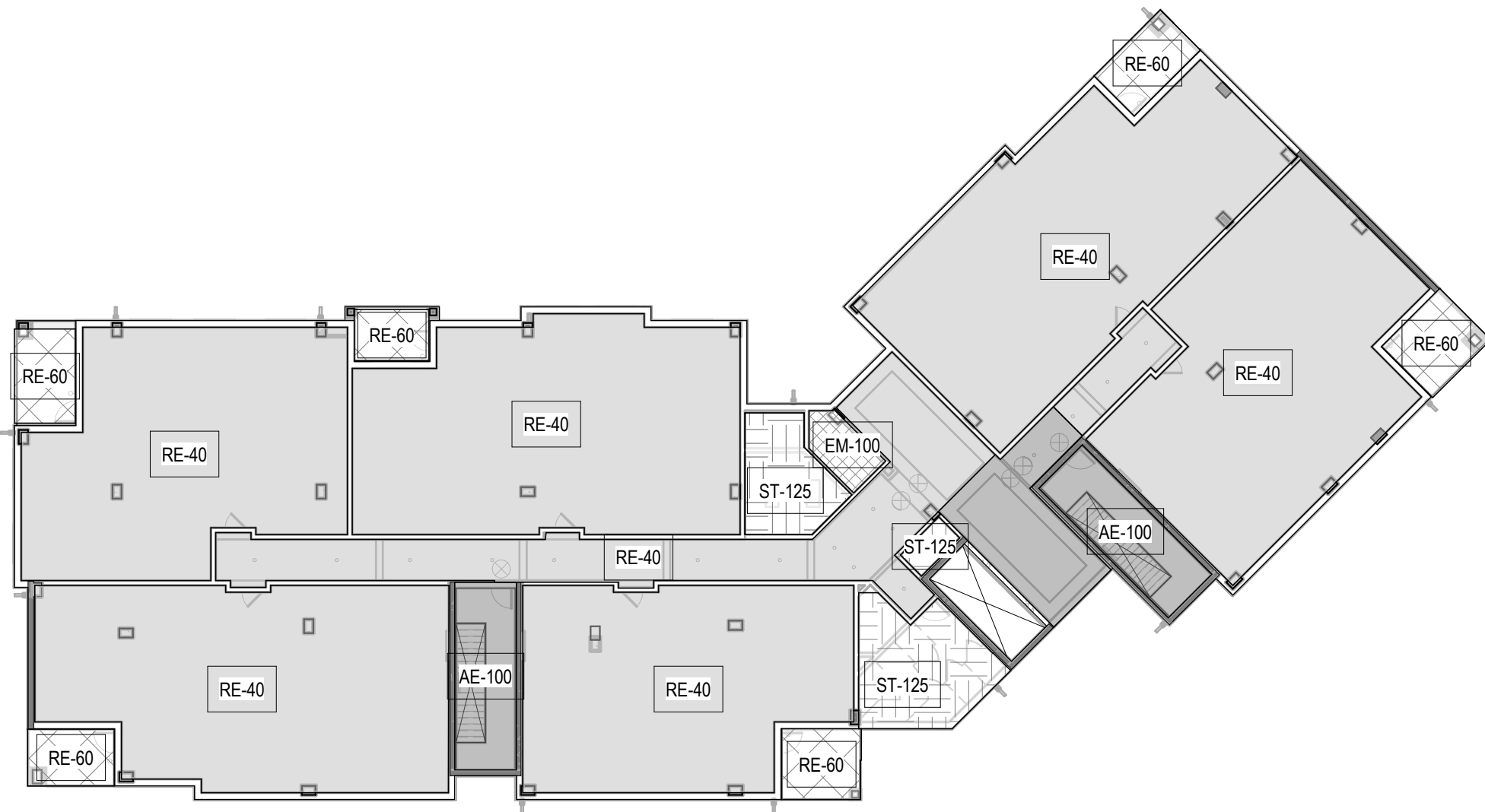
3/64" = 1'-0"



GA
S-002B

GROUND FLOOR LOAD PLAN - AREA A

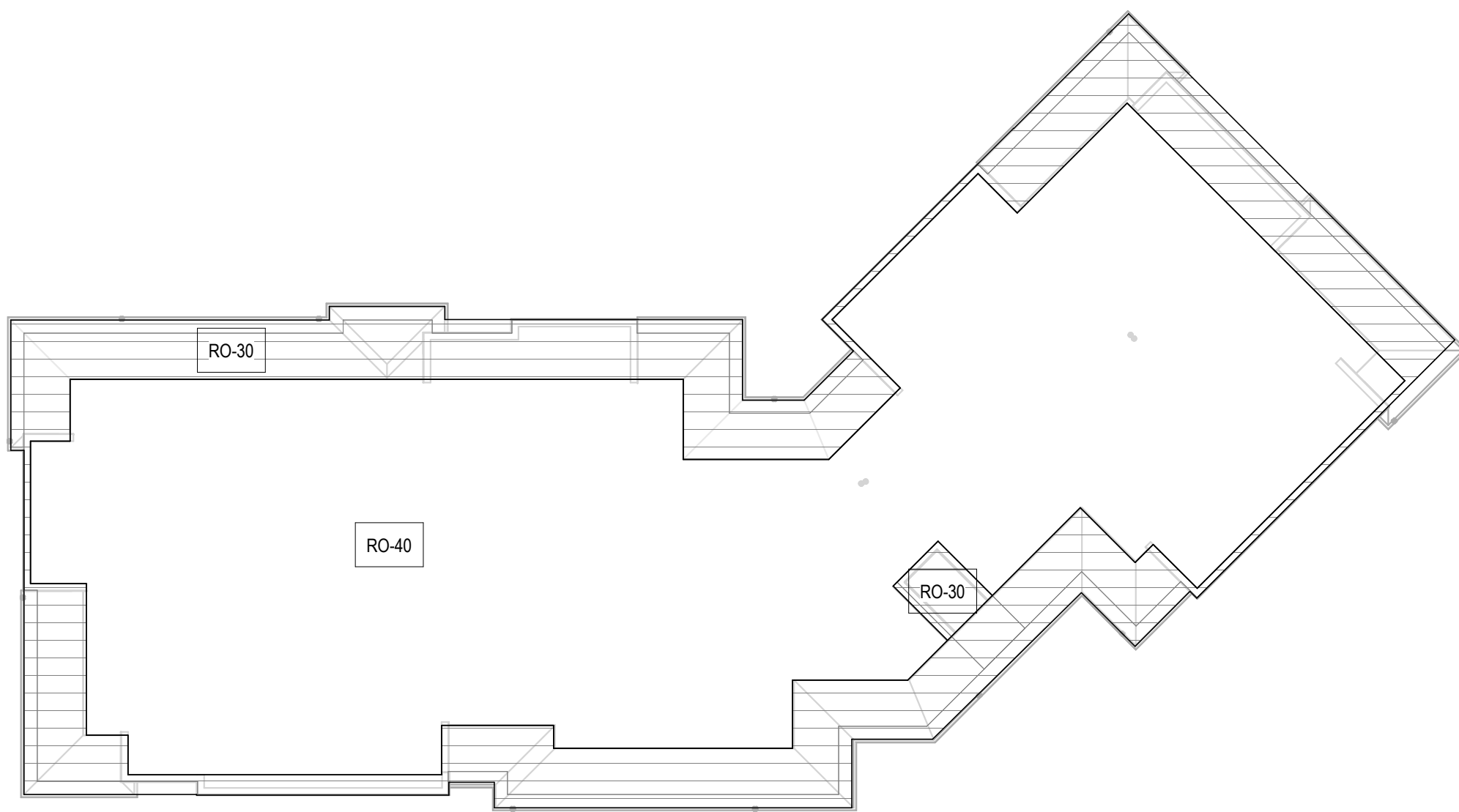
3/64" = 1'-0"



3B
S-002B

THIRD FLOOR LOAD PLAN - AREA B

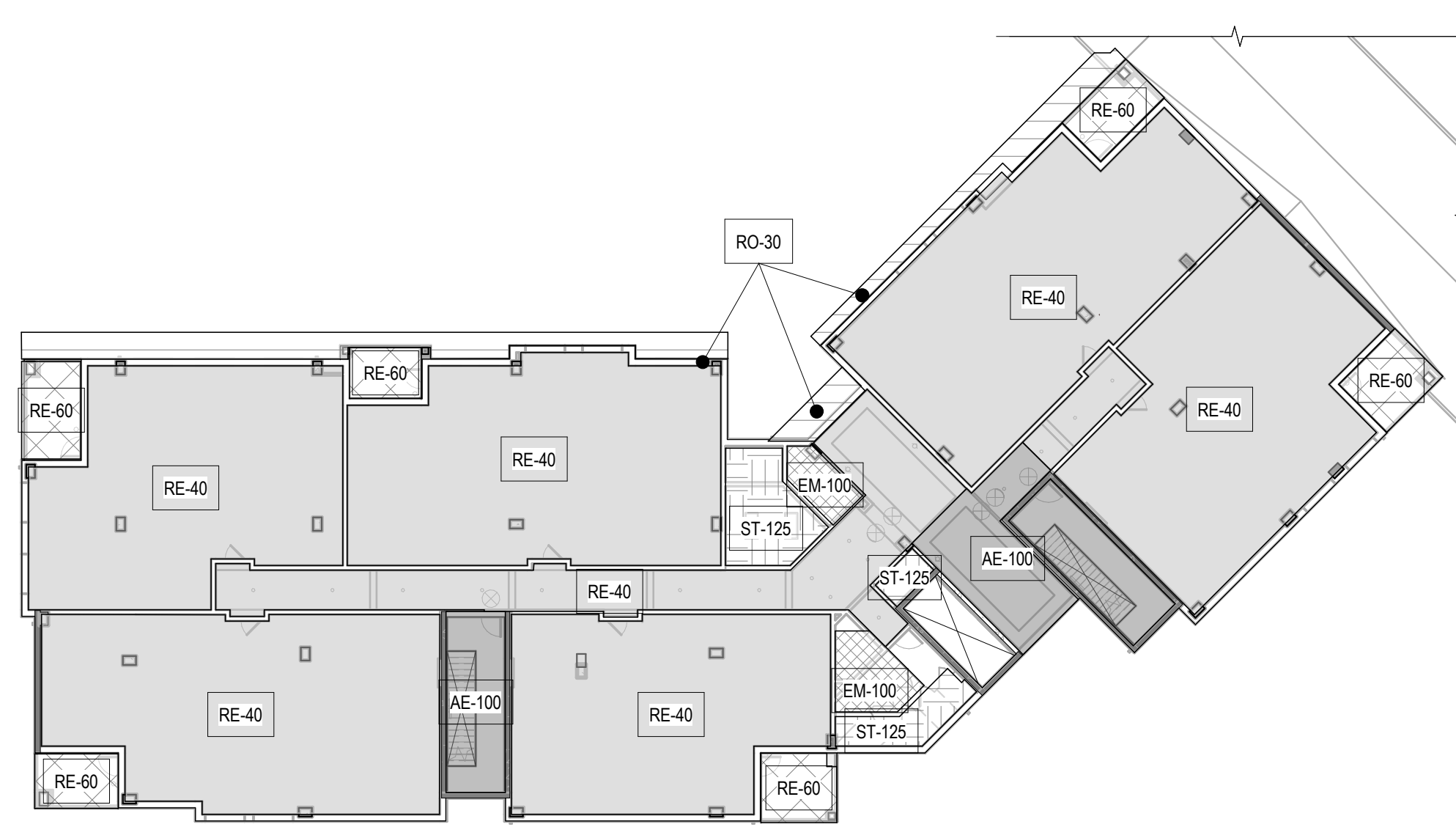
3/64" = 1'-0"



RB
S-002B

ROOF LEVEL LOAD PLAN - AREA B

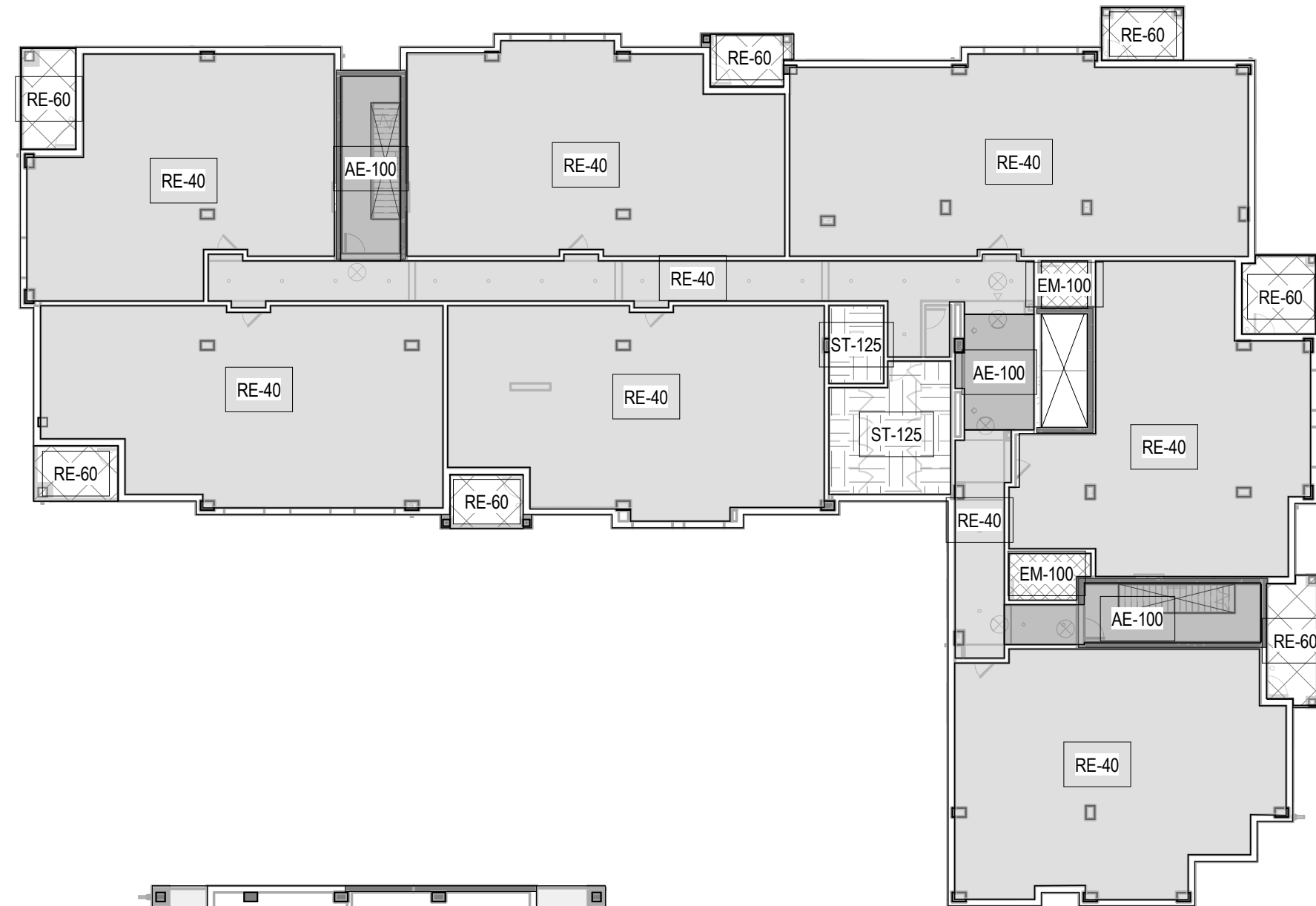
3/64" = 1'-0"



2B
S-002B

SECOND FLOOR LOAD PLAN - AREA B

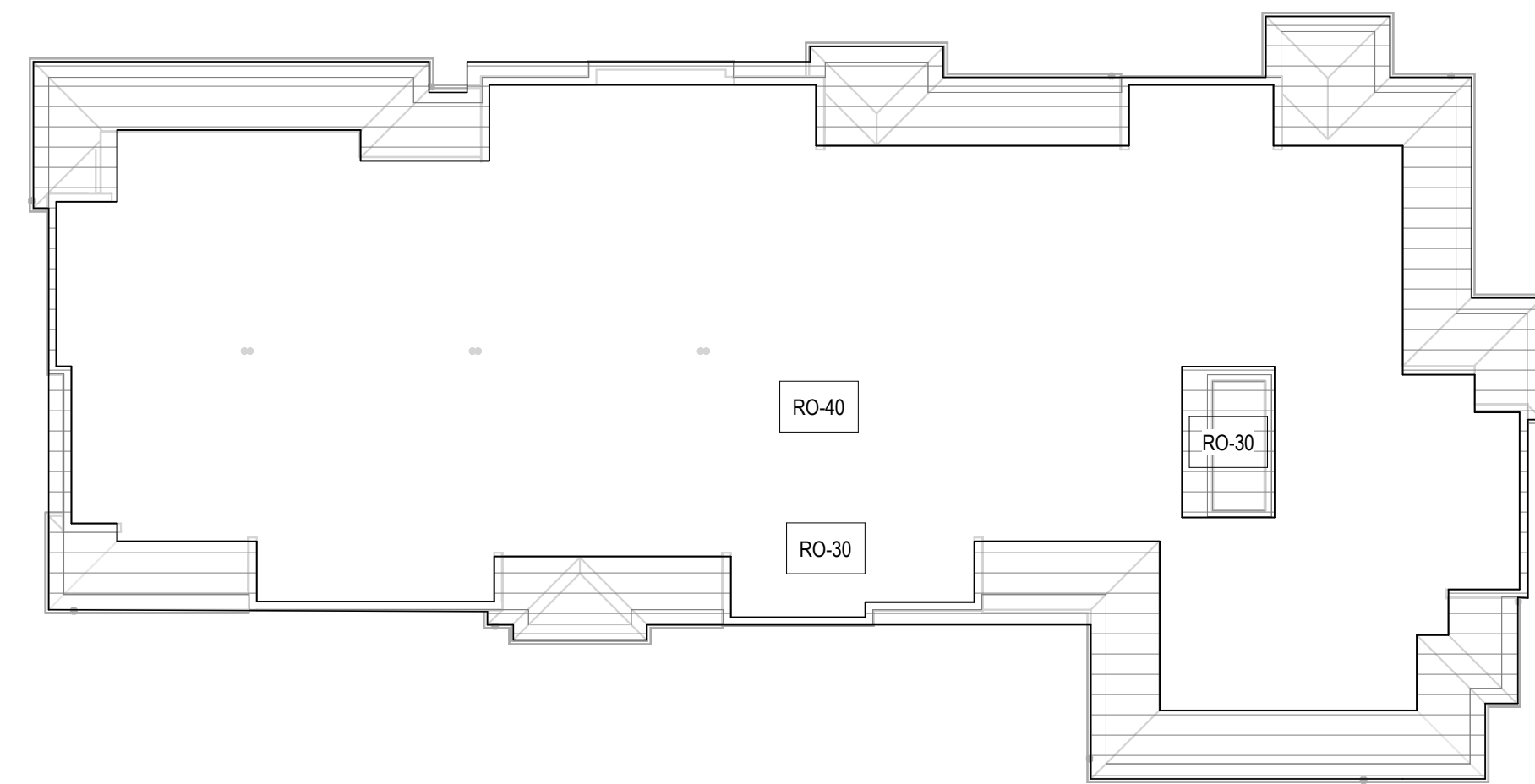
3/64" = 1'-0"



3A
S-002B

THIRD FLOOR PLAN - AREA A

3/64" = 1'-0"



RA
S-002B

ROOF LEVEL LOAD PLAN - AREA A

3/64" = 1'-0"

LOAD MAP KEY					
MARK	OCCUPANCY OR USE	SUPER-IMPOSED DEAD LOAD (PSF)	LIVE LOAD (PSF)	LIVE LOAD REDUCTION	COMMENTS
AE-100	ASSEMBLY/EGRESS	25	100	No	
EM-100	ELECTRICAL & MECHANICAL	25	100	No	
RE-40	RESIDENTIAL	25	40	Yes	
RE-60	RESIDENTIAL BALCONY	25	60	Yes	
RO-30	ROOF TYPICAL	35	30	Yes	
RO-40	ROOF WELL	10	40	Yes	
SOG-100	SLAB-ON-GROUND	25	100	No	
ST-125	STORAGE	25	125	No	

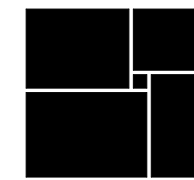
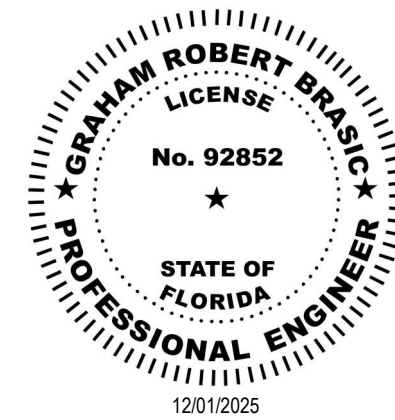


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Project No.: 2021009
Date: 12/01/2025

LOAD PLANS

S-002B

CONCRETE

- A. ALL CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH DIVISION 03 OF THE SPECIFICATIONS.
- B. FOR CONCRETE MIXTURE REQUIREMENTS SEE SCHEDULE ON THIS SHEET.
- C. THE USE OF RECYCLED CONCRETE IS PROHIBITED WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD.
- D. NORMAL WEIGHT CONCRETE SHALL BE USED FOR ALL CONCRETE MEMBERS UNLESS NOTED OTHERWISE. NORMAL WEIGHT CONCRETE SHALL HAVE A CURED DENSITY OF 145 PCF ±5 PCF. WHERE LIGHT WEIGHT CONCRETE IS SPECIFIED THE CURED DENSITY SHALL BE 112 PCF ±3 PCF.
- E. EACH MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER AND THE INTENDED LOCATION OF PLACEMENT ON THE SPECIFIC PROJECT SHALL BE CLEARLY STATED.
- F. ALL PROPOSED CONSTRUCTION JOINT LOCATIONS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL. HORIZONTAL CONSTRUCTION JOINTS SHALL NOT BE PERMITTED IN BEAMS, WALLS, AND SLABS UNLESS SPECIFICALLY SHOWN ON STRUCTURAL DRAWINGS OR BY WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD. FOR MILD REINFORCED MEMBERS, CONSTRUCTION JOINTS SHALL OCCUR WITHIN THE MIDDLE THIRD OF A MEMBER'S SPAN. ALL APPROVED CONSTRUCTION JOINTS SHALL BE INDICATED, DIMENSIONED, AND DETAILED ON THE CONCRETE REINFORCEMENT SHOP DRAWINGS.
- G. GIRDERS, BEAMS, HAUNCHES, DROP PANELS, DROP CAPS, AND CAPITALS SHALL BE POURED MONOLITHICALLY AS PART OF THE SLAB SYSTEM UNLESS NOTED OTHERWISE.
- H. PROVIDE A ¼ INCH EXPOSED CORNER OR BEAMS, WALLS, ETC. UNLESS NOTED OTHERWISE.
- I. CONCRETE CORING AND INSTALLATION OF DRILLED ANCHORS IS NOT PERMITTED WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD.
- J. REFER TO THE ARCHITECTURAL DRAWINGS FOR ALL CONCRETE DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL COORDINATE BETWEEN THE ARCHITECTURAL, STRUCTURAL, AND MEP DRAWINGS TO FURNISH DIMENSIONED DRAWINGS THAT LOCATE AND SIZE ALL SLAB EDGES, OPENINGS, AND PENETRATIONS. THESE DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL.
- K. EMBEDDED CONDUITS, PIPES, AND SLEEVES
1. THE OUTSIDE DIAMETER OF CONDUITS, PIPES, AND SLEEVES SHALL NOT EXCEED ONE-THIRD THE THICKNESS OF THE SLAB, WALL OR BEAM IN WHICH THEY ARE EMBEDDED. EMBEDMENTS SHALL NOT SIGNIFICANTLY REDUCE THE CAPACITY OF THE MEMBERS THEY PENETRATE.
2. THE MINIMUM CLEAR COVER FOR CONDUITS, PIPES, AND SLEEVES SHALL BE 1 1/2" FOR CONCRETE EXPOSED TO EARTH OR WEATHER AND 1" FOR CONCRETE NOT EXPOSED TO EARTH OR WEATHER.
3. ALUMINUM EMBEDMENTS AND EMBEDMENTS MADE OF ANY OTHER MATERIAL HARMFUL TO THE CONCRETE OR REINFORCEMENT ARE PROHIBITED.
4. EMBEDMENTS NOT SHOWN ON THE CONTRACT DOCUMENTS SHALL BE DESIGNED TO RESIST THE EFFECTS OF MATERIAL, PRESSURE, AND TEMPERATURE THAT THEY WILL BE SUBJECTED TO. THE WORK SHALL BE COORDINATED AMONGST ALL CONSTRUCTION TRADES.
5. THE CONTENTS OF EMBEDDED PIPES SHALL NOT FLOW UNTIL THE CONCRETE HAS REACHED ITS SPECIFIED DESIGN STRENGTH.
6. CONDUITS, PIPES, AND SLEEVES SHALL BE PLACED BETWEEN TOP AND BOTTOM LAYERS OF REINFORCEMENT IN SLABS AND BETWEEN INNER AND OUTER LAYERS OF REINFORCEMENT IN WALLS.
7. EMBEDDED ITEMS SHALL BE FABRICATED AND INSTALLED SUCH THAT CUTTING, BENDING, OR DISPLACEMENT OF REINFORCEMENT FROM ITS SPECIFIED LOCATION IS NOT REQUIRED.

CONCRETE REINFORCEMENT

- A. ALL CONCRETE REINFORCEMENT SHALL BE INSTALLED IN ACCORDANCE WITH DIVISION 03 OF THE SPECIFICATIONS.
- B. ALL REINFORCEMENT STEEL SHALL BE ASTM A615, GRADE 60 UNLESS NOTED OTHERWISE.
- C. WHERE WELDS ARE INDICATED FOR REINFORCEMENT STEEL ON THE DRAWINGS, REINFORCEMENT STEEL SHALL BE A706, GRADE 60 UNLESS OTHERWISE NOTED.
- D. WELDED WIRE REINFORCEMENT SHALL CONFORM TO THE MATERIAL REQUIREMENTS OF ASTM A1064.
- E. ALL 90°, 180°, AND 180° HOOKED REINFORCEMENT SPECIFIED AND GRAPHICALLY DEPICTED IN THE CONTRACT DOCUMENTS SHALL BE TAILED IN ACCORDANCE WITH A318 STANDARD HOOK GEOMETRY FOR DEFORMED BARS IN TENSION AND FOR STIRRUPS, TIES, AND HOOPS.
- F. FOR EVERY VERTICAL OR HORIZONTAL BAR DISCONTINUED BY AN OPENING, ONE BAR (MINIMUM OF 2 BARS) SHALL BE ADDED AT SIDE OF OPENING (HALF TO EACH SIDE, TYPICAL).
- G. FOR CONCRETE CLEAR COVER TO REINFORCEMENT SEE SCHEDULE ON THIS SHEET. UNLESS NOTED OTHERWISE, CLEAR COVER IN PARENTHESES () DENOTES CLEAR COVER WHEN THE AS-BUILT APPLICATION IS EXPOSED TO WEATHER.
- H. ALL LAP SPLICES SHALL BE CLASS B TENSION LAP SPLICES IN ACCORDANCE WITH ACI 318 UNLESS NOTED OTHERWISE. SEE LAP SPlice SCHEDULE ON THIS SHEET FOR LAP SPlice LENGTHS. UNLESS NOTED AS CONTINUOUS, REINFORCEMENT SHALL ONLY BE SPLICED AT LOCATIONS SHOWN ON THE CONTRACT DOCUMENTS. SPLICES AT NON-SPECIFIED LOCATIONS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL.
- I. A MINIMUM LAP SPlice OF 6" SHALL BE PROVIDED AT ALL END AND SIDE LAP CONDITIONS FOR WELDED WIRE REINFORCEMENT UNLESS NOTED OTHERWISE.
- J. MECHANICAL SPLICES ARE REQUIRED WHERE SPECIFIED ON THE CONTRACT DOCUMENTS. MECHANICAL SPLICES ARE ALSO REQUIRED WHERE SPECIFIED IN A318 AND A318M BARS. MECHANICAL SPLICES MAY ALSO BE USED AT THE CONTRACTORS OPTION, PROVIDED THE MECHANICAL SPLICES HAVE A CURRENT ICC-ES REPORT DEMONSTRATING THEY CAN DEVELOP 125% OF THE SPECIFIED YIELD STRENGTH OF THE BAR IN TENSION OR COMPRESSION.
- K. MECHANICAL SPLICES SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL.
- L. THE USE OF WELDED SPLICES IS PROHIBITED UNLESS NOTED OTHERWISE. THE CONTRACTOR SHALL SUBMIT THE LOCATIONS OF WELDED SPLICES TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL. IF APPROVED, WELDED SPLICES SHALL CONFORM TO THE REQUIREMENTS OF AWS D1.
- M. DOWELS SHALL MATCH SIZE AND SPACING OF PRIMARY REINFORCEMENT UNLESS NOTED OTHERWISE.
- N. SEE TYPICAL DETAIL FOR REINFORCEMENT REQUIRED AT OPENINGS AND PENETRATIONS.
- O. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE REINFORCEMENT BAR SIZES AND PLACEMENT. WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS, ELEVATIONS, AND DETAILS IS NOT ACCEPTABLE.

POST-TENSIONED CONCRETE

- A. ALL POST-TENSIONED CONCRETE SHALL MEET THE REQUIREMENTS IN THE CONCRETE MIX SCHEDULE ON THIS SHEET. PRIOR TO STRESSING OF THE POST-TENSIONED SLAB, THE CONCRETE SHALL ATTAIN A MINIMUM OF TWO-THIRDS OF THE SPECIFIED 28-DAY COMPRESSIVE DESIGN STRENGTH, OR ALTERNATIVELY, THE MINIMUM REQUIRED BY THE POST-TENSIONING SUPPLIER IS GREATER.
- B. NO CONCRETE SHALL BE PLACED UNTIL THE POST-TENSIONING TENDONS AND REINFORCEMENT LOCATIONS HAVE BEEN INSPECTED AND APPROVED BY THE TESTING AGENCY.
- C. THE CONTRACTOR SHALL PREPARE AND SUBMIT A STRESSING SCHEDULE BEFORE POST-TENSIONING AND STRESSING SHALL BEGIN WITHIN 2 HOURS FROM THE TIME THAT THE CONCRETE ATTAINS THE INDICATED STRENGTH. POST-TENSIONED CONCRETE SLABS SHALL REMAIN SHORED UNTIL THE SLAB IS STRESSED.
- D. POST-TENSIONED TENDONS SHALL BE DETERMINED ON THE CONSTRUCTION DRAWINGS ARE EFFECTIVE FORCES AFTER ALL LOSSES ARE ACCOUNTED FOR.
1. POST-TENSIONING SUPPLIER SHALL DETERMINE TENDON LOSSES IN ACCORDANCE WITH ACI 318 AND SHALL PROVIDE SUFFICIENT CABLE TO DEVELOP FINAL EFFECTIVE FORCES AS INDICATED ON THE STRUCTURAL DRAWINGS.
- E. TENDONS SHALL BE STRESSED TO A MAXIMUM OF 80% OF f_{pu} WHILE ANCHORAGE STRESSES SHALL BE A MAXIMUM OF 70% OF f_{pu} . HOWEVER, STRESSES SHALL NOT BE GREATER THAN THE MAXIMUM ALLOWED RECOMMENDED BY THE MANUFACTURER OF THE POST-TENSIONING STEEL OR ANCHORAGE DEVICES.
- F. ALL POST-TENSIONED FORCES SHOWN ARE IN KIPS OR KIPS/FT.
- G. SLAB TENDONS FOR FORCES SHOWN IN KIPS ARE TO BE PLACED UNIFORMLY IN A BAND WIDTH ON EACH SIDE OF A COLUMN. PLACE EITHER TWO OR THREE TENDONS THROUGH A COLUMN SO THAT HALF OF THE REMAINING TENDONS ARE EQUAL ON EACH SIDE OF THE COLUMN.
- H. SLAB TENDONS FOR FORCES SHOWN IN KIPS/FT ARE TO BE PLACED UNIFORMLY BETWEEN INDICATED WIDTHS. AT LEAST TWO TENDONS SHALL PASS THROUGH EACH COLUMN.
- I. ALL POST-TENSIONED SLAB THICKNESSES ARE AS MARKED ON THE CONSTRUCTION DRAWINGS.
- J. AT DEAD ENDS AND STRESSING ENDS, TENDON CENTER OF GRAVITY SHALL BE AT CENTROID OF THE MEMBER, UNLESS NOTED OTHERWISE.
- K. POST-TENSIONED SLABS ARE DESIGNED BASED ON TENDONS BEING CONTINUOUS BETWEEN EDGES OF SLABS AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY ADDITIONAL INTERMEDIATE STRESSING JOINTS OR CLOSURE STRIPS REQUIRED BY THE CONTRACTOR MAY REQUIRE ADDITIONAL REINFORCEMENT AND SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW AND APPROVAL BEFORE STARTING CONSTRUCTION.
- L. COORDINATION OF THE DETAILING AND PLACEMENT OF TENDONS AND MILD REINFORCEMENT STEEL BETWEEN SUPPLIERS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
1. PLACE TOP REINFORCEMENT BARS UNIFORMLY IN A WIDTH EQUAL TO THE COLUMN WIDTH PLUS ONE AND A HALF TIMES THE SLAB THICKNESS ON EACH SIDE OF THE COLUMN. PLACE A MINIMUM OF THREE OR FOUR BARS THROUGH THE COLUMN SO THAT THE REMAINING BARS ARE EQUAL ON EACH SIDE OF THE COLUMN, WHEN NOT SHOWN OR CALLED OUT. PROVIDE THE SAME REINFORCEMENT AS SHOWN OR AS CALLED OUT FOR AREAS WITH SIMILAR CONDITIONS.
2. BOTTOM BARS SHALL BE CENTERED IN SPAN, UNLESS NOTED OTHERWISE.
3. PLACE TENDONS IN SMOOTH PARABOLIC DRAPES BETWEEN HIGH AND LOW POINTS SHOWN, UNLESS NOTED OTHERWISE.
4. TENDON LOW POINTS SHALL BE AT MID-SPAN BETWEEN SUPPORTS, UNLESS NOTED OTHERWISE.
5. OTHER TENDON CONTROL POINTS SHOWN ON THE STRUCTURAL DRAWINGS ARE DIMENSIONS TO THE CENTER OF GRAVITY FROM THE BOTTOM OF THE MEMBER, UNLESS NOTED OTHERWISE.
6. PROVIDE CHAIRS AND SUPPORT BARS AS REQUIRED TO HOLD TENDONS IN THE CORRECT POSITION DURING PLACEMENT OF CONCRETE.
7. TENDONS SHALL NOT DEVIATE FROM THEIR CORRECT POSITION BY MORE THAN ONE-EIGHTH OF AN INCH. SLIGHT DEVIATIONS IN TENDON SPACING WILL BE PERMITTED WHERE REQUIRED TO AVOID CONFLICTS WITH OPENINGS, INSERTS, ETC.
8. SHOULD CONFLICT ARISE BETWEEN TENDONS, MILD REINFORCEMENT STEEL, OR OTHER CONDUITS, TENDON LOCATIONS SHALL TAKE PRECEDENCE.
- M. CONTRACTOR SHALL CONDUCT CONTINUOUS INSPECTION AND RECORDING OF JACKING FORCES AND ELONGATIONS WHICH ARE TO BE IMMEDIATELY SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW AND ACCEPTANCE PRIOR TO THE REMOVAL OF EXCESS TENDON END MATERIAL.
- N. EXCESS TENDON END MATERIAL SHALL BE REMOVED ONLY BY A PLASMA CUTTER OR AN ACCEPTED SUBSTITUTION BY THE STRUCTURAL ENGINEER OF RECORD. NO TORCH CUTTING IS PERMITTED. TENDON ENDS SHALL NOT BE CUT UNTIL THE ENTIRE SLAB HAS BEEN SATISFACTORILY STRESSED AND THE STRUCTURAL ENGINEER OF RECORD HAS REVIEWED THE ELONGATIONS.
- O. FOLLOWING REMOVAL OF EXCESS TENDON END MATERIAL, GREASE CAPS SHALL BE PLACED IN ACCORDANCE WITH THE POST-TENSIONING SUPPLIER. POST-TENSIONING SUPPLIER SHALL INSPECT AND PROVIDE A SIGNED AND SEALED LETTER FROM A STRUCTURAL ENGINEER LICENSED TO PERFORM THE WORK IN THE JURISDICTION WHERE THE PROJECT IS LOCATED, STATING THAT THE CORROSION PROTECTION SYSTEM HAS BEEN INSTALLED IN ACCORDANCE WITH THE APPROVED DOCUMENTS.
- P. THE CONTRACTOR SHALL PROVIDE THE FOLLOWING INFORMATION SIGNED AND SEALED BY A STRUCTURAL ENGINEER LICENSED TO PERFORM THE WORK IN THE JURISDICTION WHERE THE PROJECT IS LOCATED:
1. DETAILED DESIGN OF TENDON END ANCHORAGES
2. THE CALCULATION OF STRESS LOSSES DUE TO CREEP, SHRINKAGE, TENDON RELAXATION, ANCHORAGE SLIP, AND FRICTION.
3. POST-TENSIONED SHOP DRAWINGS.

CONCRETE MIXTURE REQUIREMENTS									
	APPLICATION	EXPOSURE CLASS	F _c (PSI)	TEST AGE	MODULUS OF ELASTICITY (KSI)	MAXIMUM W/C M	AIR CONTENT	NOMINAL MAXIMUM AGGREGATE	MAXIMUM CONCRETE WEIGHT (PCF)
ON-GROUND	FOUNDATIONS	F0, S0, W1, C1	4000	28 DAYS	3605	SEE NOTE 2	SEE NOTE 3	1"	150
	SLAB-ON-GROUND (EXTERIOR)	F0, S0, W1, C2	5000	28 DAYS	4031	0.40	SEE NOTE 3	1"	150
	SLAB-ON-GROUND (INTERIOR)	F0, S0, W0, C0	3000	28 DAYS	3122	SEE NOTE 2	SEE NOTE 3	1"	150
VERTICAL	SHEAR WALLS AND COLUMNS (FIRST FLOOR)	F0, S0, W2, C2	5000	28 DAYS	4031	0.40	SEE NOTE 3	3/4"	150
	SHEAR WALLS AND COLUMNS (SECOND FLOOR TO ROOF)	F0, S0, W0, C0	5000	28 DAYS	4031	SEE NOTE 2	SEE NOTE 3	3/4"	150
	CMU FILLED CELLS	GROUT MIX SHALL BE USED FOR CMU FILLED CELLS - SEE CONCRETE MASONRY UNIT GENERAL NOTES, DETAILS, AND SPECIFICATIONS							
ELEVATED	POST-TENSIONED ELEVATED FRAMING (EXTERIOR)	F0, S0, W2, C2	5000 3000 48 HOURS	28 DAYS 28 DAYS 48 HOURS	4031 @ 28-Days	0.40	SEE NOTE 3	3/4"	150
	POST-TENSIONED ELEVATED FRAMING (INTERIOR)	F0, S0, W0, C0	5000 3000 48 HOURS	28 DAYS 28 DAYS 48 HOURS	4031 @ 29-Days	SEE NOTE 2	SEE NOTE 3	3/4"	150
	ELEVATED SLABS AND BEAMS (NON-PRESTRESSED)	F0, S0, W1, C1	5000	28 DAYS	4031	SEE NOTE 2	SEE NOTE 3	3/4"	150
NOTES:									
1. EXPOSURE CATEGORIES AND CLASSES FOR SULFATES, PERMEABILITY, AND CORROSION PROTECTION OF REINFORCEMENT IS CLASS ZERO UNLESS NOTED OTHERWISE.									
2. WATER/CEMENT RATIO SHALL BE AS REQUIRED FOR THE SPECIFIED CONCRETE MIX DESIGN. THERE IS NO MAXIMUM WATER/CEMENT RATIO REQUIREMENT FOR THE EXPOSURE CLASSIFICATION ASSOCIATED WITH THIS APPLICATION. MAXIMUM WATER/CEMENT RATIO IS NOT APPLICABLE FOR DURABILITY REQUIREMENTS IN LIGHTWEIGHT CONCRETE.									
3. THERE IS NO MANDATORY TARGET AIR CONTENT FOR THIS APPLICATION. THE CONTRACTOR MAY CHOOSE TO ADD AIR INTRINTEMENT TO IMPROVE THE WORKABILITY AND FINISHING PROPERTIES OF THE MIX. AIR CONTENT SHALL BE AS REQUIRED FOR THE SPECIFIED CONCRETE MIX.									
4. COARSE AGGREGATE SHALL BE ASTM #3 GRADE. SELECT GRADING CLASS PER TYPE OF CONSTRUCTION OR LOCATION USED, AND IN RELATION TO SPECIFIC WEATHER REGION. AGGREGATE SHALL BE FROM A SINGLE SOURCE. #67 GRADING SHALL BE SUEd FOR CONCRETE WITH 3/4 INCH MAXIMUM; #57 GRADING SHALL BE USED FOR CONCRETE WITH 1 INCH MAXIMUM.									

STRUCTURAL STEEL

- A. STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS UNLESS NOTED OTHERWISE ON THE CONTRACT DOCUMENTS:
1. ROLLED SHAPES: ASTM A572 OR A992, MIN. YIELD STRENGTH 50 KSI
 2. ANGLES FOR TRUSSES AND BRACES: ASTM A36 MIN YIELD STRENGTH 36 KSI
 3. CHANNELS & ANGLES: ASTM A36
 4. HOLLOW STRUCTURAL SECTIONS: ASTM A500 GRADE C, MIN YIELD STRENGTH 46 KSI FOR ROUND AND 50 KSI FOR RECTANGULAR HSS
- B. CONNECTION MATERIAL SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS OR AS NEEDED FOR CONNECTION DESIGN:
1. ANGLES: ASTM A36
 2. WTS: ASTM A992
 3. PLATES: ASTM A572, MIN YIELD STRENGTH 50 KSI
 4. BOLTS: ASTM A325
 5. NUTS: ASTM A305
 6. WASHERS: ASTM F436
 7. ANCHOR RODS: ASTM F1554 GRADE 36
 8. WELD ELECTRODES: MATCH FILLER METAL TO BASE METAL PER AWS D1.1
- C. WHERE NO CAMBER IS INDICATED, FABRICATE BEAMS SO THAT ANY NATURAL CAMBER IS UPWARD AFTER ERECTION.
- D. CANTILEVERED BEAMS WITH NATURAL MILL CAMBER SHALL BE ERECTED SUCH THAT THE CAMBER IS ORIENTED DOWNWARD OR CONCAVE UP.
- E. SPLICES SHALL BE ALLOWED ONLY AT LOCATIONS SPECIFICALLY INDICATED ON THE STRUCTURAL DRAWINGS UNLESS APPROVED OTHERWISE BY THE SER IN WRITING.
- F. FOR STEEL MEMBERS AND EMBEDMENTS EXPOSED TO WEATHER, PROVIDE HOT-DIPPED GALVANIZED FINISH. PROVIDE HOLES IN ALL STEEL AS REQUIRED TO PREVENT ANY ACCUMULATION OF WATER. ALL PENETRATIONS THROUGH MAIN MEMBERS SHALL NOT EXCEED 1 1/8" DIA. AND SHALL BE GROUND SMOOTH. THESE DRAINS MUST BE KEPT CLEAN AND OPEN.
- H. SHOW ALL COPES, HOLES, OPENINGS AND MODIFICATIONS REQUIRED IN STRUCTURAL STEEL MEMBERS FOR ERECTION OR THE WORK OF OTHER TRADES ON THE SHOP DRAWINGS FOR APPROVAL BY THE ARCHITECT AND STRUCTURAL ENGINEER.
- I. FIELD MODIFICATIONS OF STRUCTURAL STEEL IS PROHIBITED WITHOUT PRIOR APPROVAL OF THE ARCHITECT AND STRUCTURAL ENGINEER.
- J. WHERE BEAM SHEAR IS NOT NOTED, DESIGN FOR 10K.
- K. ALL CONNECTIONS SHALL BE DESIGNED FOR THE SPECIFIED SHEAR, MOMENT, AND AXIAL LOADS ON THE DRAWINGS. THE CONNECTIONS SHALL BE DESIGNED FOR LOAD REVERSAL. ALL CONNECTIONS FORCES SPECIFIED ON PLAN ARE ULTIMATE LEVEL FORCES UNLESS NOTED OTHERWISE.
- L. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN, DETAILING, AND FABRICATION OF ALL STEEL FRAMING CONNECTIONS UNLESS SPECIFICALLY NOTED AS COMPLETELY DESIGNED ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL RETAIN A STRUCTURAL ENGINEER LICENSED TO PERFORM THE WORK IN THE JURISDICTION WHERE THE PROJECT IS LOCATED, WHO SHALL DESIGN THE CONNECTIONS, SUBMIT SIGN AND SEALED CALCULATIONS TO THE ARCHITECT FOR REVIEW AND APPROVAL PRIOR TO STARTING FABRICATION.
- M. CONNECTION DESIGN SHALL MEET THE REQUIREMENTS OF THE AISI SPECIFICATIONS AND THE BUILDING CODE. CONNECTION DESIGN SHALL PROVIDE AN ADEQUATE LOAD PATH TO TRANSFER THE LOADS FROM EACH MEMBER, THROUGH THE CONNECTION, INTO THE SUPPORTING MEMBER, AND SHALL CONSIDER THE EFFECTS OF THE FORCES ON EACH MEMBER. PROVIDE STIFFENER PLATES, WEB DOUBLER PLATES, FLANGE CONTINUITY PLATES, ETC, AS REQUIRED. MEMBERS SHOWN ON THE DRAWINGS HAVE NOT BEEN SIZED FOR LOCAL EFFECTS AT CONNECTIONS.
- N. STEEL CONNECTION DETAILS SHOW GENERAL CRITERIA FOR DESIGN AND DETAILING, AND ARE NOT INTENDED TO SHOW COMPLETE CONNECTION CONFIGURATIONS OR OTHER SPECIFIC INFORMATION THAT ARE THE RESPONSIBILITY OF THE CONNECTION DESIGN ENGINEER. ALTERNATIVE CONNECTION CONFIGURATION MAY BE SUBMITTED TO THE ARCHITECT FOR REVIEW AND APPROVAL. CONNECTIONS SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS ARE TO BE FABRICATED AS SHOWN.
- O. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ERECTION AIDS THAT INCLUDE, BUT ARE NOT LIMITED TO ERECTION ANGLES, LIFT HOLES, AND OTHER AIDS.
- P. STEEL BEAMS ARE EQUALLY SPACED BETWEEN DIMENSION POINTS AT THE MAXIMUM DECK SPAN LOCATION UNLESS NOTED OTHERWISE. MINIMUM CONNECTIONS SHALL BE A TWO-BOLT CONNECTION USING 3/4 INCH- DIAMETER A325 BOLTS IN SINGLE SHEAR UNLESS NOTED OTHERWISE. ALL HIGH-STRENGTH BOLTS SHALL BE INSTALLED, TIGHTENED, AND INSPECTED IN ACCORDANCE WITH THE RCSC. BOLTS IN CONNECTIONS SHALL BE INSTALLED WITH FULL PRETENSION EXCEPT WHERE "SNUG-TIGHT" INSTALLATION IS SPECIFICALLY PERMITTED ON THE DRAWINGS. WHERE CONNECTIONS ARE NOTED AS SNUG-TIGHT, THE CONTRACTOR MAY INSTALL PER THE CRITERIA FOR SNUG-TIGHT BOLTS. BOLTS IN SLIP-CRITICAL CONNECTIONS SHALL BE INSTALLED USING TURN-OF-NUT PRETENSIONING, TWIST-OFF TYPE TENSION CONTROL, BOLT PRETENSIONING, OR DIRECT-TENSION-INDICATOR (DTI) PRETENSIONING. ALL BOLT HOLES SHALL BE STANDARD SIZE UNLESS NOTED OTHERWISE.

WELDING

- A. ALL WELDING SHALL BE PERFORMED IN STRICT ADHERENCE TO A WRITTEN WELDING PROCEDURE SPECIFICATION PER AMERICAN WELDING SOCIETY D1.1. ALL WELDING PARAMETERS SHALL BE WITHIN THE ELECTRODE MANUFACTURER'S RECOMMENDATIONS. WELDING PROCEDURES SHALL BE SUBMITTED TO THE OWNER'S TESTING AGENCY FOR REVIEW BEFORE STARTING FABRICATION OR ERECTION. COPIES OF THE WELDING PROCEDURE SPECIFICATION SHALL BE ON SITE AND AVAILABLE TO ALL WELDERS AND THE SPECIAL INSPECTOR.
- B. ALL WELDS SHALL BE MADE USING LOW HYDROGEN ELECTRODES WITH MINIMUM TENSILE STRENGTH PER AWS D1.1 (MINIMUM 70 KSI).
- C. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE JOINT PREPARATIONS AND WELDING PROCEDURES THAT INCLUDE, BUT ARE NOT LIMITED TO: REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, COPES, SURFACE ROUGHNESS VALUES, TAPERS, AND TRANSITIONS OF UNEQUAL PARTS.
- D. WELDING SHALL BE DONE BY WELDERS WITH CURRENT AMERICAN WELDING SOCIETY CERTIFICATION.
- E. FIELD WELDING SYMBOLS HAVE NOT NECESSARILY BEEN INDICATED ON THE DRAWINGS. WHERE SHOWN, PROPER FIELD WELDING PER AMERICAN WELDING SOCIETY D1.1 SHALL BE USED. WHERE NO FIELD WELDING SYMBOLS ARE SHOWN, IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE THE USE OF SHOP AND FIELD WELDS.
- F. ALL WELD SIZES SHALL BE THE LARGER OF: THE SIZE REQUIRED BY THE CONNECTION FORCES, MINIMUM SIZE PER ANS/AWS D1.1 OR 3/16 INCH MINIMUM FILLET WELD, UNLESS NOTED OTHERWISE.
- G. PROVIDE FILLET WELDS AT CONTACT POINTS BETWEEN STEEL MEMBERS SUFFICIENT TO DEVELOP THE ALLOWABLE TENSILE FORCE OF THE SMALLER MEMBER AT THE JOINT, UNLESS NOTED OTHERWISE.
- H. ALL FILLET WELDS SHALL BE VISUALLY INSPECTED BY THE TESTING FIRM.
- I. GROOVE WELDS SHALL BE FULL PENETRATION, UNLESS NOTED OTHERWISE.
- J. ALL COMPLETE JOINT PENETRATION WELDS SHALL BE ULTRASONICALLY TESTED UPON COMPLETION OF THE CONNECTION, EXCEPT PLATES LESS THAN OR EQUAL TO 1/4 INCH THICK SHALL BE MAGNETIC PARTICLE TESTED. REDUCTION IN TESTING MAY BE MADE IN ACCORDANCE WITH THE BUILDING CODE WITH APPROVAL OF THE ENGINEER.
- K. A RUN-OFF TAB SHALL BE USED AT ALL BEVEL AND FULL PENETRATION WELDS. RUN-OFF TABS SHALL BE REMOVED BY NEAT CUTS AFTER WELD IS COMPLETED. GRIND SMOOTH WHERE REQUIRED BY DETAIL.
- L. WHERE REQUIRED BY DETAIL, REMOVE WELD BACKING BARS AND GRIND SMOOTH AFTER WELD IS COMPLETED.
- M. WHERE NECESSARY, REMOVE GALVANIZING OR PRIMER PRIOR TO WELDING.
- N. STEEL USING COMPLETE JOINT PENETRATION GROOVE WELDS THAT FUSE THROUGH THE THICKNESS OF THE FLANGE OR WEB SHALL HAVE A MINIMUM CHARTPY V-NOTCH IMPACT TESTING VALUE AS FOLLOWS:
1. ASTM A660M HOT-ROLLED SHAPES WITH A FLANGE THICKNESS EXCEEDING 2 INCHES AND BUILT-UP HEAVY SHAPES WITH PLATES EXCEEDING 2 INCHES IN THICKNESS: 20 FT-LB AT 70° FAHRENHEIT.
 2. REGARDLESS OF THICKNESS, ALL TRUSSES, LATERAL SYSTEM MEMBERS (INCLUDING COLUMNS, WIND ORDERS, BRACES, ETC): 20 FT-LB AT 70° FAHRENHEIT.
 3. STEEL EXPOSED TO TEMPERATURES IN SERVICE BELOW 50° FAHRENHEIT: 20 FT-LB AT SERVICE TEMPERATURE + 20° FAHRENHEIT; 40° FAHRENHEIT MAXIMUM.
 4. WELD METAL: 20 FT-LB AT -20° FAHRENHEIT AND 40 FT-LB AT 70° FAHRENHEIT.
 5. TESTING IS TO BE IN ACCORDANCE WITH ASTM A660M, SUPPLEMENTARY REQUIREMENT S30, CHARTPY V-NOTCH IMPACT TEST FOR STRUCTURAL SHAPES - ALTERNATE CORE LOCATION, AT ROLLED SHAPES AND ASTM A670 FOR PLATES. AT ANY PERMITTED LOCATIONS, WELD METAL SHALL BE TESTED IN ACCORDANCE WITH ASTM E23, STANDARD METHODS FOR NOTCHED BAR IMPACT TESTING OF METALLIC MATERIALS FOR WELD METAL.

STEEL DECK GENERAL REQUIREMENTS

- A. ALL STEEL DECK SHALL BE MANUFACTURED AND INSTALLED IN ACCORDANCE WITH THE DIVISION 05 SPECIFICATIONS.
- B. STEEL DECK SHALL BE SUPPORTED BY A MINIMUM OF FOUR SUPPORT LOCATIONS (THREE SPAN CONDITION), UNLESS NOTED OTHERWISE.
- C. THE CONTRACTOR SHALL COORDINATE SLAB/DECK OPENING SIZES AND LOCATIONS PER ARCHITECTURAL AND MEP CONTRACT DOCUMENTS. THE CONTRACTOR SHALL PROVIDE OPENING SUPPORT FRAMING AND/OR REINFORCEMENT AS REQUIRED PER TYPICAL DETAILS AND SUBMIT PROPOSED SLAB/DECK OPENINGS FOR REVIEW BY THE STRUCTURAL ENGINEER OF RECORD.
- D. SHOP DRAWINGS SHALL BE SUBMITTED INDICATING:
1. MATERIAL STRENGTH
 2. SECTION PROPERTIES
 3. DECK GAGE, LAYOUT
 4. FASTENER TYPE
 5. CONNECTION PATTERN
 6. CLOSURE ANGLES
- E. THE CAPACITY OF THE DECK SHALL BE BASED ON CURRENT ICC-ES EVALUATION REPORTS.

STEEL ROOF DECK

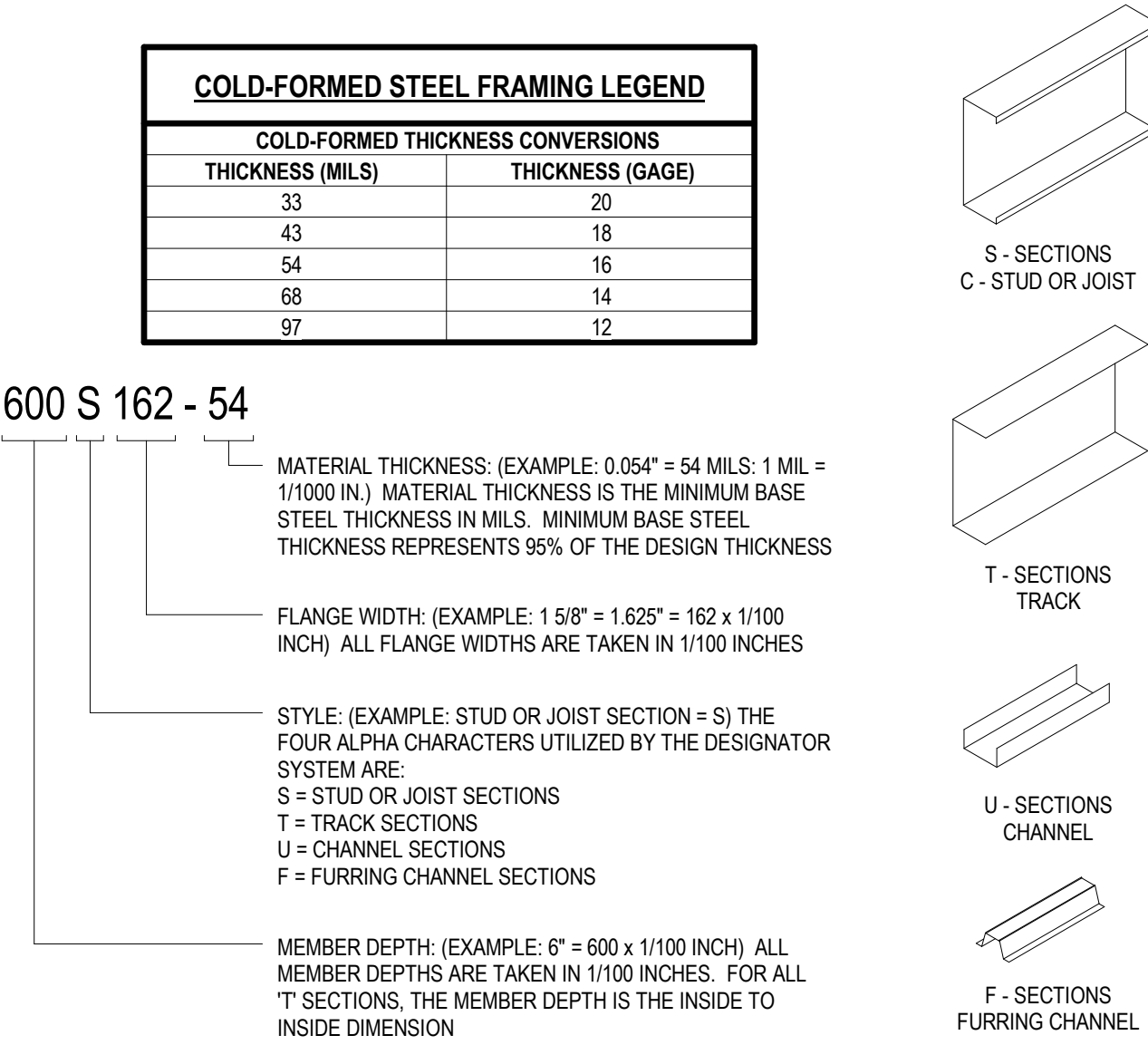
- A. STEEL ROOF DECK SHALL BE A MINIMUM YIELD STRENGTH OF 33 KSI, UNLESS NOTED OTHERWISE. ALL INTERIOR STEEL ROOF DECK SHALL CONFORM TO ASTM A1008, FACTORY PRIME FOR PAINT. ALL EXPOSED STEEL ROOF DECK SHALL CONFORM TO ASTM A653 WITH G90 HOT-DIPPED GALVANIZATION, UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR EXTENTS.
- B. STEEL ROOF DECK SHALL BE ATTACHED TO STEEL SUPPORTS WITH 5/8 INCH DIAMETER PUDDLE WELDS AND TO COLD-FORMED METAL FRAMING WITH #12 SELF-DRILLING SCREWS UNLESS NOTED OTHERWISE. WHEN DECK THICKNESS IS LESS THAN 0.028 INCHES, WELDS MUST BE MADE THROUGH MINIMUM 16 GAGE WELDING WASHERS. SPACING OF WELDS SHALL BE AS SPECIFIED IN THE DECK ATTACHMENT SCHEDULE.
- C. WHERE STEEL MEMBERS ARE PARALLEL TO THE DECK FLUTES AND AT THE SAME ELEVATION OF THE BOTTOM OF THE DECK, ADJUST DECK LAYOUT AND WELD DECK TO STEEL WITH SAME WELDING AS REQUIRED FOR SIDE BOUNDARIES.
- D. ERECT STEEL DECK CLOSURES AND OTHER LIGHT GAGE MATERIAL REQUIRED TO PRODUCE A COMPLETED INSTALLATION.
- E. FLAT, RIDGE, AND VALLEY PLATES: UNLESS NOTED OTHERWISE, CONTRACTOR SHALL PROVIDE FLAT PLATES (20 GAGE MINIMUM) AT ALL LOCATIONS WHERE ROOF DECK CHANGES DIRECTION AND RIDGE OR VALLEY PLATES (20 GAGE MINIMUM) AT ALL LOCATIONS WHERE ROOF SLOPE EXCEEDS 1/4 INCH PER FOOT.
- F. DO NOT HANG CEILING, DUCTS, LIGHT FIXTURES, EQUIPMENT, OR OTHER ITEMS FROM THE ROOF DECK WITHOUT PRIOR APPROVAL FROM THE DECK SUPPLIER AND REVIEW BY THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD.
- G. SEE ARCHITECTURAL DRAWINGS FOR FINAL ROOF SLOPES. WHERE STRUCTURAL FRAMING DOES NOT CREATE THE SPECIFIED ROOF SLOPE, IT SHALL BE CREATED WITH RIGID INSULATION ABOVE THE DECK.

NON-COMPOSITE STEEL FORM DECK

- A. NON-COMPOSITE STEEL FORM DECK SHALL BE A MINIMUM YIELD STRENGTH OF 60 KSI AND SHALL CONFORM TO ASTM A653 WITH G90 HOT-DIPPED GALVANIZATION, UNLESS NOTED OTHERWISE.
- B. NON-COMPOSITE STEEL FORM DECK SHALL BE ATTACHED TO SUPPORTS WITH 5/8 INCH DIAMETER PUDDLE WELDS. WHEN DECK THICKNESS IS LESS THAN 0.028 INCHES, WELDS MUST BE MADE THROUGH MINIMUM 16 GAGE WELDING WASHERS. SPACING OF WELDS SHALL BE AS FOLLOWS:
1. AT BUTTED ENDS: AT 10 INCHES ON CENTER
 2. AT PERIMETER EDGES OF BUILDING: AT 10 INCHES ON CENTER
 3. INTERMEDIATE SUPPORTS: AT 10 INCHES ON CENTER
 4. SIDE LAPS: FOR FORM DECK WITH SPANS 3'-6" OR GREATER, PROVIDE TWO CONNECTIONS PER SPAN. HEX HEAD SCREWS, SIZE #10, OR CRIMPING (BUTTON PUNCHING) MAY BE USED AT SIDE LAP CONNECTIONS.

COLD-FORMED STEEL

- A. DESIGN, FABRICATION, AND ERECTION OF COLD-FORMED STEEL SHALL CONFORM TO AISI S100.
- B. ALL STUDS, JOISTS, TRACK, BRIDGING, END CLOSURES, AND ACCESSORIES SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE REQUIREMENTS OF AISI S100.
- C. THE CONTRACTOR'S DELEGATED ENGINEER SHALL DESIGN ALL COLD-FORMED STEEL AND ITS CONNECTIONS TO THE BUILDING STRUCTURE. REFER TO ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR REQUIRED COLD-FORMED STEEL.
- D. ALL EXTERIOR COLD-FORMED STEEL AND ITS CONNECTIONS TO THE BUILDING STRUCTURE SHALL BE DESIGNED PER DESIGN CRITERIA AND COMPONENTS AND CLADDING WIND PRESSURES LISTED IN THE STRUCTURAL DOCUMENTS.
- E. ALL INTERIOR COLD-FORMED STEEL AND ITS CONNECTIONS TO THE BUILDING STRUCTURE SHALL BE DESIGNED PER DESIGN CRITERIA LISTED IN THE GENERAL STRUCTURAL NOTES AND A MINIMUM OF 5 PSF INTERNAL PRESSURE NORMAL TO THE STRONG AXIS OF FRAMING MEMBER IN ADDITION TO DEAD LOAD.
- F. ALL EXTERIOR COLD-FORMED STEEL SHALL HAVE A MINIMUM G90 GALVANIZED COATING. ALL INTERIOR COLD-FORMED STEEL SHALL HAVE A MINIMUM G60 GALVANIZED COATING.
- G. ALL STUDS SHALL BE DESIGNED TO A MINIMUM GAUGE OF 43 MILS. STUD SPACING SHALL NOT EXCEED 24" ON CENTER.
- H. ALL COLD-FORMED STEEL, 54 MIL AND THICKER SHALL HAVE A MINIMUM YIELD STRENGTH (F_y) OF 50 KSI.
- I. ALL WELDING SHALL MEET REQUIREMENTS OF AWS D1.3 AND THE AISI STANDARD.
- J. ALL SCREWS OR PINS SHALL BE NON-CORROSIVE NO. 8-18 (Ø = 0.125") OR LARGER, UNLESS NOTED OTHERWISE. DO NOT USE STAINLESS STEEL OR COPPER-COATED FASTENERS.
- K. TRACKS SHALL BE THE SAME DEPTH AS STUDS OR JOISTS AND OF EQUAL OR THICKER GAUGE THAN STUDS OR JOISTS, UNLESS NOTED OTHERWISE. TRACKS SHALL BE CONNECTED IN ORDER TO SUPPORT STUDS OR JOISTS AT 24" ON CENTER MAXIMUM. STUDS AND JOISTS SHALL BE CONNECTED TO TRACKS AT EACH SIDE.
- L. INSTALLATION OF CURTAIN WALL FRAMING SHALL ACCOMMODATE VERTICAL DISPLACEMENT OF THE PRIMARY STRUCTURE.
- M. DESIGN OF SLIP TRACKS SHALL CONFORM TO GUIDELINES ESTABLISHED IN STEEL STUD MANUFACTURER'S ASSOCIATION TECHNICAL NOTE NO. 1, PUBLISHED JANUARY 2000.
- N. PROVIDE THE STANDARD TRACK, CLIP ANGLES, BRACING, REINFORCEMENTS, FASTENERS, AND ACCESSORIES AS RECOMMENDED BY THE MANUFACTURER FOR THE APPLICATION INDICATED AND AS NEEDED TO PROVIDE A COMPLETE FRAMING SYSTEM. INSTALL THE FRAMING SYSTEM IN ACCORDANCE WITH THE MANUFACTURERS WRITTEN INSTRUCTIONS AND RECOMMENDATIONS, UNLESS NOTED OTHERWISE.
- O. MATCH FILLER METAL TO BASE METAL PER AWS D1.3 FOR WELDING STEEL STUDS. ALL WELDING SHALL BE PERFORMED IN ACCORDANCE WITH AWS PROCEDURES. CONSULT MANUFACTURER FOR EQUIPMENT RECOMMENDATIONS AND PROPER ELECTRODE SELECTION. TOUCH UP WELDED AREAS WITH A ZINC RICH PAINT.
- P. STUD-TO-STUD CONNECTIONS SHALL BE A MINIMUM OF (4) #8 TEK SCREWS AT EACH CONNECTION, UNLESS NOTED OTHERWISE.
- Q. RESISTANCE TO MINOR AXIS BENDING AND ROTATION SHALL BE PROVIDED BY GYPSUM BOARD, GYPSUM SHEATHING, PLYWOOD, HORIZONTAL BRACING, OR CHANNEL SHAPED COLD-FORMED STEEL FRAMING BLOCKING.
- R. SHOP DRAWINGS, INCLUDING CALCULATIONS, SHALL BE SIGNED AND SEALED BY A DELEGATED ENGINEER AND SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD AND ARCHITECT FOR REVIEW.
- S. SHOP DRAWINGS SHALL CLEARLY INDICATE ALL FRAMING SIZES, CONNECTIONS, AND BRACING. IF FRAMING DEPTH IS NOT INDICATED IN THE CONTRACT DOCUMENTS, THE MOST ECONOMICAL MEMBER AND CONNECTION MEETING THE DESIGN CRITERIA SHALL BE PROVIDED.
- T. CALCULATIONS SHALL CLEARLY INDICATE DESIGN LOADING, FRAMING SIZE, SPACING, ASSUMPTIONS, AND FORCES IMPOSED ONTO BUILDING STRUCTURE FROM CONNECTIONS.
- U. STEEL STUD MANUFACTURERS ASSOCIATION FOUR PART NOMENCLATURE IDENTIFIES MEMBER DEPTH, TYPE, FLANGE WIDTH AND GAUGE.



STEEL EMBED PLATE SCHEDULE								
MARK	PLATE GEOMETRY (IN)			EDGE DISTANCE	ANCHORS (AWS D1.1, TYPE B)			COMMENTS
	WIDTH	LENGTH	THICKNESS		#	DIAMETER	LENGTH	
EP1010	10"	10"	1/2"	1-1/2"	4	1/2"	4"	7/ S-600
EP1212	12"	12"	1/2"	1-1/2"	4	1/2"	6"	2/ S-600

EDGE DISTANCE (TYP)
LENGTH
WIDTH
EDGE DISTANCE (TYP)

EDGE DISTANCE (TYP)
LENGTH
WIDTH
EDGE DISTANCE (TYP)

EDGE DISTANCE (TYP)
LENGTH
WIDTH
EDGE DISTANCE (TYP)

T/PLATE = T/SLAB
(SEE PLAN)
LENGTH (NOTE 1)
THICKNESS

LAYOUT WITH 2 ANCHORS

LAYOUT WITH 4 ANCHORS

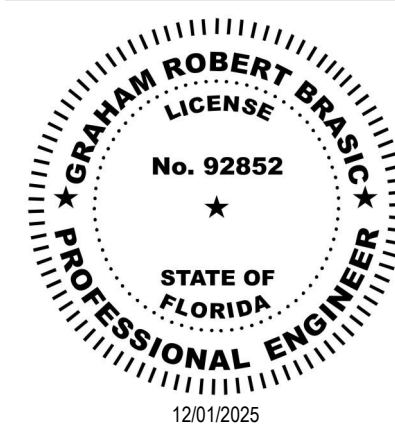
LAYOUT WITH 6 ANCHORS

1. ANCHOR LENGTH INDICATED IS FINAL LENGTH AFTER BURNOFF.

2. SEE PLAN FOR LOCATION OF KEYED SECTIONS & DETAILS REFERENCING EMBED PLATES AND ATTACHMENT OF CONNECTING ELEMENTS.

3. FOR LAYOUTS WITH MORE THAN 4 ANCHORS SEE KEYED SECTIONS/DETAILS FOR ANCHOR LAYOUTS.

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Project No.: 2021009
Date: 12/01/2025

STEEL
GENERAL
NOTES &
SCHEDULES

S-004

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CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 21.18.004

TO THE BEST OF THE ENGINEER'S
KNOWLEDGE, THE PLANS AND
SPECIFICATIONS COMPLY WITH THE
APPLICABLE BUILDING CODES AND
MATERIAL SPECIFICATIONS.

GENERAL

- A. THIS STRUCTURAL INSPECTION PLAN IS PREPARED IN ACCORDANCE WITH SECTION 553.79(5) OF THE FLORIDA STATUTES WHICH DESCRIBES THE REQUIREMENT THAT A STRUCTURAL INSPECTION PLAN BE PREPARED BY THE ENGINEER OF RECORD FOR A THRESHOLD BUILDING.
- B. THE PURPOSE OF THIS STRUCTURAL INSPECTION PLAN IS TO PROVIDE SPECIFIC INSPECTION PROCEDURES AND SCHEDULES SO THAT THE BUILDING CAN BE ADEQUATELY INSPECTED FOR GENERAL COMPLIANCE WITH THE CONTRACT DOCUMENTS.
- C. STRUCTURAL INSPECTIONS SHALL BE PERFORMED BY THE SPECIAL INSPECTOR REGISTERED UNDER SECTION 471 OF THE FLORIDA STATUTES AS AN ENGINEER.
- D. THE OWNER SHALL EMPLOY THE SERVICES OF A SPECIAL INSPECTOR IN COMPLIANCE WITH THE PROVISIONS OF CHAPTER 553.79 OF THE FLORIDA STATUTES.
- E. THE SPECIAL INSPECTOR SHALL INSPECT THE FRAMING SHOWN ON THE STRUCTURAL DRAWINGS IN ACCORDANCE WITH THIS PLAN TO VERIFY THAT THE WORK IS CONSTRUCTED IN GENERAL COMPLIANCE WITH THE CONTRACT DOCUMENTS, EXCEPT FOR ACCEPTED VARIATIONS.
- F. THE OWNER SHALL EMPLOY THE SERVICES OF A SPECIAL INSPECTOR IN COMPLIANCE WITH THE PROVISIONS OF CHAPTER 553.79 OF THE FLORIDA STATUTES.
- G. THE SPECIAL INSPECTOR SHALL INSPECT THE FRAMING SHOWN ON THE STRUCTURAL DRAWINGS IN ACCORDANCE WITH THIS PLAN TO VERIFY THAT THE WORK IS CONSTRUCTED IN GENERAL COMPLIANCE WITH THE CONTRACT DOCUMENTS, EXCEPT FOR ACCEPTED VARIATIONS.
- H. THE SPECIAL INSPECTOR SHALL PERFORM THE MANDATORY STRUCTURAL INSPECTIONS AS REQUIRED UNDER THE PROVISIONS OF THE FLORIDA BUILDING CODE, IN ACCORDANCE WITH THE SCOPE AND SCHEDULE OUTLINED HEREIN.
- I. THE SPECIAL INSPECTOR SHALL LOG EACH SITE VISIT DOCUMENTING THE PROGRESS OF THE WORK, THE RELEVANT TESTING CONDUCTED, THE AREAS INSPECTED, AND ANY OTHER ELEMENTS PERTINENT TO THE WORK PROGRESS AT THE TIME OF THE INSPECTION. THE LOG RECORD SHALL REMAIN ON SITE AND ACCESSIBLE AT ANY TIME.
- J. DAILY SIGNED AND SEALED FIELD REPORTS SHALL BE SUBMITTED ON A WEEKLY BASIS TO THE ENFORCING AGENCY, ARCHITECT OR STRUCTURAL ENGINEER OF RECORD, AND OWNER.
- K. THE SPECIAL INSPECTOR SHALL NOT MAKE DESIGN DECISIONS, DIRECT THE CONTRACTOR'S WORK, NOR BE RESPONSIBLE FOR CONSTRUCTION MEANS AND METHODS.
- L. THE SPECIAL INSPECTOR MAY SEND A FULL-TIME COLLEAGUE AS HIS AUTHORIZED REPRESENTATIVE TO THE PROJECT, BUT THAT PERSON SHALL BE KNOWLEDGEABLE AND HAVE A COLLEGE DEGREE IN CIVIL OR STRUCTURAL ENGINEERING.

CONTRACTOR RESPONSIBILITIES

- A. THE CONTRACTOR SHALL PROVIDE THE SPECIAL INSPECTOR WITH AN OFFICE AT THE CONSTRUCTION SITE THAT INCLUDES, AT MINIMUM, DESK, CHAIR, PLAN TABLE, PLAN RACK, INTERNET ACCESS, AND JANITORIAL SERVICES IN AIR-CONDITIONED SPACE.
- B. THE CONTRACTOR SHALL PROVIDE THE SPECIAL INSPECTOR WITH AN UPDATED COPY OF ALL PERMIT DOCUMENTS, REVIEWED AND APPROVED SHOP DRAWING SUBMITTALS, TEST REPORTS, AND OTHER ITEMS WITH THE LATEST INFORMATION IN A TIMELY MANNER.
- C. THE CONTRACTOR SHALL PROVIDE A MINIMUM OF 24 HOURS NOTICE TO THE SPECIAL INSPECTOR PRIOR TO THE NEED FOR ANY INSPECTION ON SITE.
- D. THE CONTRACTOR SHALL COOPERATE AND ASSIST THE SPECIAL INSPECTOR IN PERFORMING HIS INSPECTION DUTIES AND SHALL PROVIDE HIM FREE ACCESS TO THE WORK AT ALL TIMES.
- E. THE PRESENCE OF OR PROTECTION FROM THE SPECIAL INSPECTOR FROM HIS RESPONSIBILITY TO COMPLY WITH CONTRACT DOCUMENTS, THE CONTRACTOR HAS THE RESPONSIBILITY FOR ANY DEVIATIONS FROM THE CONTRACT DOCUMENTS AND THE COSTS OF RECTIFYING THOSE DEVIATIONS.
- F. THE CONTRACTOR MAY CORRECT ANY DEVIATIONS FROM THE CONTRACT DOCUMENTS OR MAY ISSUE A REQUEST TO THE DESIGN PROFESSIONAL OF ACCEPTANCE OF THE DEVIATION.
- G. CONSTRUCTION PERFORMED WITHOUT INSPECTION MAY REQUIRE TESTING OR MAY NEED TO BE REMOVED AS DETERMINED BY THE STRUCTURAL ENGINEER OR ARCHITECT OF RECORD.
- H. THE CONTRACTOR'S SHORING ENGINEER SHALL SUPERVISE, INSPECT, AND CERTIFY THE INSTALLATION OF ALL SHORING AND RE-SHORING PER APPROVED SHOP DRAWINGS. THE SHORING ENGINEER SHALL PROVIDE A SIGNED AND SEALED INSPECTION CERTIFICATION TO THE SPECIAL INSPECTOR PRIOR TO ALL CONCRETE POURS.

FREE OWNER RESPONSIBILITIES

- A. THE FEE OWNER SHALL RETAIN A GEOTECHNICAL ENGINEER TO PROVIDE INSPECTION SERVICES FOR MONITORING THE PREPARATION OF THE FOUNDATION BEARING SURFACE AND PILE FOUNDATION ELEMENTS.
- B. THE FEE OWNER SHALL RETAIN AN INDEPENDENT TESTING LABORATORY FOR MATERIAL SAMPLING AND TESTING SERVICES REQUIRED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE.
- C. THE FEE OWNER SHALL ARRANGE AND ENSURE THAT ALL NECESSARY UPDATED CONSTRUCTION DOCUMENTS WITH THE LATEST INFORMATION, APPROVED SHOP DRAWINGS, LATEST SKETCHES, AND ANY OTHER TYPE OF CORRESPONDENCE, WHICH MAY AFFECT THE INSPECTIONS BE FURNISHED TO THE SPECIAL INSPECTOR IN A TIMELY MANNER.
- D. THE FEE OWNER SHALL ENSURE THAT THE CONTRACTOR PROVIDES SHOP DRAWINGS FOR ALL PRECAST STRUCTURAL COMPONENTS, THESE SHOP DRAWINGS MUST BE DESIGNED, SIGNED AND SEALED BY A REGISTERED FLORIDA PROFESSIONAL ENGINEER AND HAVE BEEN REVIEWED AND STAMPED BY THE ENGINEER OF RECORD.

SPECIAL INSPECTOR RESPONSIBILITIES

- A. PRIOR TO STARTING WITH THEIR WORK, THE SPECIAL INSPECTOR, AND THEIR AUTHORIZED REPRESENTATIVE, IF APPLICABLE, SHALL ATTEND A PRE-CONSTRUCTION MEETING FOR THE PURPOSE OF BECOMING FAMILIAR WITH THE SPECIFIC STRUCTURAL COMPONENTS AND SYSTEM WHICH THEY WILL BE RESPONSIBLE FOR INSPECTING.
- B. THE SPECIAL INSPECTOR SHALL INSPECT ALL STRUCTURAL SYSTEMS NOTED BELOW AND IN COMPLIANCE WITH THOSE LISTED UNDER THE "GENERAL" HEADINGS.
- C. THE SPECIAL INSPECTOR SHALL VISIT THE SITE AT SUCH FREQUENCY TO SATISFY THEMSELVES THAT THEIR REPRESENTATIVE'S INSPECTIONS COMPLY WITH THIS PLAN.
- D. THE SPECIAL INSPECTOR SHALL DEDICATE THEIR ENTIRE TIME ON-SITE TO THE REQUIREMENTS OF THIS PLAN AND SHALL NOT PERFORM OTHER WORK, SUCH AS MATERIAL TESTING.

REPORTING

- A. THE SPECIAL INSPECTOR SHALL RECORD PROGRESS, WORKING CONDITIONS, OBSERVATIONS, TESTING, DEVIATIONS FROM THE CONTRACT DOCUMENTS, AND ANY REQUIRED CORRECTIVE ACTION. THEY SHALL RETAIN THE RECORDS FOR A MINIMUM OF 7 YEARS AFTER COMPLETION OF THE PROJECT.
- B. THE SPECIAL INSPECTOR SHALL IMMEDIATELY NOTIFY THE CONTRACTOR IN PERSON, AND THE ARCHITECT AND STRUCTURAL ENGINEER BY TELEPHONE, OF MATERIALS, TESTS, EQUIPMENT, WORKMANSHIP, OR CONSTRUCTION THAT:
- DOES NOT CONFORM TO THE CONTRACT DOCUMENTS, OR
 - IS NOT INSPECTED OR TESTED AND CANNOT BE INSPECTED OR TESTED IN PLACE.
- C. THE SPECIAL INSPECTOR SHALL THEN IMMEDIATELY ISSUE THOSE EXCEPTIONS IN WRITING TO THOSE LISTED ABOVE AND ATTACH A COPY TO THE DAILY INSPECTION REPORT.
- D. THE SPECIAL INSPECTOR SHALL KEEP AN EXCEPTIONS FILE AND REVIEW IT ON A DAILY BASIS, UPDATING AS EXCEPTIONS ARE RECTIFIED. IF ANY EXCEPTIONS ARE NOT RESOLVED IN A TIMELY MANNER, THE SPECIAL INSPECTOR SHALL ISSUE A NONCOMPLIANCE NOTICE TO THE CONTRACTOR AND SHALL COPY THE ENFORCING AGENCY, OWNER'S REPRESENTATIVE, ARCHITECT, AND STRUCTURAL ENGINEER.
- E. AFTER EACH INSPECTION, THE INSPECTOR SHALL WRITE AND SIGN AN INSPECTION REPORT. THE REPORT SHALL INCLUDE THE FOLLOWING:
- THE NAME AND LOCATION OF PROJECT, NAME OF INSPECTOR, PERMIT NUMBER, DATE, WORKING CONDITIONS, INCLUDING WEATHER AND TEMPERATURE, AND TYPE AND LOCATION OF WORK BEING PERFORMED.
 - DETAILS OF EACH INSPECTION, INCLUDING THE PRESENCE AND ACTIVITIES OF THE TESTING AGENCY.
 - NOTE DEFICIENCIES IN THE WORK AND ANY UNUSUAL CIRCUMSTANCES AFFECTING THE PERFORMANCE OF WORK, INCLUDING CHANGES IN MATERIALS OR WORK SEQUENCE, PLACING EMPHASIS ON RECURRING DEFICIENCIES.
 - IDENTIFY CORRECTIONS TO DEFICIENCIES LISTED IN PREVIOUS REPORTS.
- F. SINCE THE SPECIAL INSPECTOR DOES NOT CERTIFY THAT THE CONTRACT DOCUMENTS ARE IN COMPLIANCE WITH THE COVERING CODES, ALL STATEMENTS ISSUED SHALL REFER TO WHETHER OR NOT COMPLETED WORK IS IN SUBSTANTIAL ACCORDANCE WITH THE CONTRACT DOCUMENTS.
- G. THE REPORT SHALL ALSO COMMENT ON THE FOLLOWING, WHEN APPLICABLE:
- TEST REPORTS THAT DO NOT COMPLY WITH THE CONTRACT DOCUMENTS.
 - THE GEOTECHNICAL ENGINEER'S INSPECTION REPORTS.
 - SHORING AND RE-SHORING ENGINEER'S INSPECTION REPORTS.
 - CHANGES MADE IN THE FIELD.
- H. THE REPORT SHALL ALSO INCLUDE PHOTOGRAPHS, WHEN APPROPRIATE.
- I. THE SPECIAL INSPECTOR SHALL SUBMIT THE REPORTS TO THE ENFORCING AGENCY, OWNER'S REPRESENTATIVE, ARCHITECT, AND STRUCTURAL ENGINEER UNDER A SIGNED AND SEALED COVER LETTER ON A WEEKLY BASIS OR AS DIRECTED BY THE ENFORCING AGENCY.
- J. THE SPECIAL INSPECTOR SHALL POST AT THE JOBSITE A LOG SUMMARIZING ALL INSPECTIONS. THE HEADER SHALL CONTAIN, AT A MINIMUM, THE PROJECT NAME AND LOCATION, PERMIT NUMBER, AND THE NAME OF THE SPECIAL INSPECTOR, OWNER, AND CONTRACTOR. THE INSPECTOR SHALL WRITE AN ENTRY AFTER EACH INSPECTION THAT INCLUDES THE DATE, CONSTRUCTION PHASE, WORK DESCRIPTION, COMMENTS, HIS SIGNATURE, AND WHETHER THE WORK INSPECTED IS APPROVED OR REJECTED.
- K. UPON COMPLETION OF THE BUILDING AND PRIOR TO THE ISSUANCE OF A CERTIFICATE OF OCCUPANCY, THE SPECIAL INSPECTOR SHALL ISSUE A SIGNED AND SEALED STATEMENT ATTESTING THAT THE PART OF THE PROJECT UNDER THEIR INSPECTION RESPONSIBILITIES HAS BEEN CONSTRUCTED IN SUBSTANTIAL ACCORDANCE WITH THE CONTRACT DOCUMENTS. THIS STATEMENT SHALL BE IN ACCORDANCE WITH SECTION 553.79(7) OF THE FLORIDA STATUTES AND SHALL BE SUBMITTED TO THE ENFORCING AGENCY, OWNER, ARCHITECT, AND STRUCTURAL ENGINEER.

STRUCTURAL CAST-IN-PLACE CONCRETE

- A. THE CONTRACTOR SHALL NOTIFY THE SPECIAL INSPECTOR A MINIMUM OF 24 HOURS PRIOR TO THE PLACEMENT OF ANY STRUCTURAL CONCRETE.
- B. THE SPECIAL INSPECTOR SHALL:
- INSPECT SHORING, RE-SHORING, AND FORMWORK AS FOLLOWS:
 - VERIFY THAT THE DELETED SHORING AND RE-SHORING ENGINEER HAS CONFIRMED THAT THE SHORING AND RE-SHORING ARE IN COMPLIANCE WITH THEIR SHOP DRAWINGS (REFER TO SECTION 1.5F IN THE SPECIAL INSPECTION PLAN). ADDITIONALLY, THE SPECIAL INSPECTOR SHALL SPOT CHECK THE SHORING AND RE-SHORING FOR CONFORMANCE WITH THE SHORING AND RE-SHORING PLANS SUBMITTED TO THE ENFORCING AGENCY.
 - SPOT CHECK SHORING AND RE-SHORING LAYOUT, ALIGNMENT, AND MATERIALS.
 - CONFIRM THAT THE TIMING OF SHORING, RE-SHORING, AND FORMWORK PROCEDURES AND REMOVAL COMPLY WITH THE CONTRACT DOCUMENTS AND THE SHORING AND RE-SHORING DRAWINGS, WHERE APPROPRIATE. VERIFY THAT CONCRETE TEST CYLINDER STRENGTH IS ADEQUATE PRIOR TO FORM REMOVAL.
 - VERIFY THAT SLAB THICKNESS IS MAINTAINED AT SLAB DEPRESSIONS AND STEPS.
 - INSPECT STEEL REINFORCEMENT PER THE STRUCTURAL DRAWINGS, SUPPLEMENTED BY THE APPROVED SHOP DRAWINGS, TO VERIFY:
 - STEEL REINFORCEMENT GRADE, SIZE, QUANTITY, CONFIGURATION, AND SPACING.
 - MINIMUM CLEARANCE REQUIREMENTS FROM CONCRETE SURFACES.
 - STEEL REINFORCEMENT IS ADEQUATELY SUPPORTED AND TIED TO RESIST SHIFTING DURING CONCRETE POUR AND THAT CONCRETE COVER IS PROPER.
 - HOOKE REINFORCEMENT IS PLACED PROPERLY.
 - EMBEDMENT LENGTHS, SPLICE LOCATIONS, AND LAP SPLICE LENGTHS ARE ACCEPTABLE.
 - MECHANICAL COUPLERS ARE APPROVED AND INSTALLED PER MANUFACTURER'S SPECIFICATIONS.
 - PROPER TIE SPACING, PARTICULARLY AT BEAM/COLUMN INTERSECTIONS.
 - STEEL REINFORCEMENT LAYERS IN SLABS AND WALLS ARE PROPER.
 - ALL OPENINGS LARGER THAN 12" AND NOT SHOWN ON THE CONTRACT DOCUMENTS HAVE BEEN REPORTED TO THE STRUCTURAL ENGINEER OF RECORD. CHECK PLACEMENT OF ADDITIONAL REINFORCEMENT AROUND OPENINGS. OPENINGS THROUGH BEAMS ARE PROHIBITED WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD.
 - IN ONE-WAY SLABS, THAT TEMPERATURE STEEL IS CORRECT, INCLUDING PROPER LAYERING.
 - IN TWO-WAY SLABS, THAT:
 - THE COLUMN STRIP TOP BARS HAVE A UNIFORM SPACING AND ARE NOT BUNDLED TOGETHER.
 - THE ADDED TOP BARS ARE PLACED IN THE VICINITY OF THE COLUMN.
 - THE TOP AND BOTTOM HOOKED BARS HAVE BEEN PLACED AS REQUIRED, ESPECIALLY AT SLAB EDGES.
 - THE POSITION OF BARS AT SLAB OFFSETS AND DEPRESSIONS.
 - ALL STEEL REINFORCEMENT SURFACES ARE FREE OF EXCESS RUST OR OTHER COATING THAT MAY AFFECT THE BONDING CAPACITY.
- E. VERIFY THAT EXPANSION JOINT MATERIAL, DOWELTAIL SLOTS, ANCHORS, EMBEDDED CONDUITS, AND LOAD CARRYING EMBEDDED ITEMS ARE PROPERLY POSITIONED AND SECURED TO RESIST DISPLACEMENT DURING CONCRETE PLACEMENT. RELOCATION OF, OR MODIFICATION TO, STRUCTURAL ITEMS DUE TO CONFLICTS WITH REINFORCEMENT IS PROHIBITED WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD.
- F. INSPECT CONSTRUCTION JOINTS AS FOLLOWS:
- VERIFY THAT DOWELS, KEYWAYS, AND BULKHEADS COMPLY WITH THE CONTRACT DOCUMENTS.
 - CONFIRM THAT BEAM AND SLAB CONSTRUCTION JOINT LOCATIONS COMPLY WITH THE CONTRACTOR'S CONSTRUCTION JOINT PLAN SUBMITTED TO THE ARCHITECT.
- G. VERIFY THAT SAWCUT JOINTS ARE:
- PROVIDED ONLY WHERE INDICATED.
 - THE PROPER DEPTH, SPACING, AND LOCATION.
 - PROVIDED IN THE TIME FRAME INDICATED IN THE CONTRACT DOCUMENTS.
- H. CHECK THAT ALL FOREIGN MATERIAL IS REMOVED FROM FORMS OR EXCAVATIONS PRIOR TO CONCRETE PLACEMENT.
- I. BE ON SITE AT THE START OF EACH CONCRETE POUR REQUIRING INSPECTION BY THIS PLAN, AND SHALL REMAIN ON SITE FOR A SUFFICIENT TIME TO CONFIRM THAT CONCRETING PRACTICES ARE PROPER AND COMPLY WITH ACI 301, ASTM C94, AND OTHER RECOGNIZED INDUSTRY STANDARDS INCLUDING THE FOLLOWING:
- ALL STEEL REINFORCEMENT CORRECTIONS ARE COMPLETED PRIOR TO CONCRETE PLACEMENT.
 - THE FIRST CONCRETE TRUCK OF EACH TYPE OF CONCRETE POUR HAS THE PROPER CONCRETE MIX NUMBER AND STRENGTH AND THAT THE BATCH TIME LEAVES SUFFICIENT TIME TO POUR ALL CONCRETE FROM THE TRUCK. SPOT CHECK FUTURE TRUCKS.
 - THE TESTING AGENCY IS ON SITE FOR EACH TYPE OF CONCRETE POUR TO TEST CONCRETE AS REQUIRED BY THE CONTRACT DOCUMENTS. THIS SHALL INCLUDE MIXING TIME, TEMPERATURE, SLUMP, AND AIR CONTENT. REPORTING ANY CONCRETE DELIVERED THAT IS NOT AS SPECIFIED. ENSURE THAT ADDITION OF WATER TO THE CONCRETE MIX IN THE FIELD COMPLIES WITH THE GUIDELINES IN THE CONTRACT DOCUMENTS.
 - CONCRETE IS BEING CONVEYED FROM MIXER TO PLACE OF FINAL DEPOSIT BY RECOGNIZED INDUSTRY STANDARDS. CONCRETE IS BEING PLACED CONTINUOUSLY, OR IN A MANNER TO AVOID PLACING CONCRETE AGAINST HARDENED CONCRETE WHICH WOULD RESULT IN THE FORMATION OF SEAMS OR PLANES OF WEAKNESS.
 - CONCRETE IS BEING CONSOLIDATED AND THOROUGHLY WORKED AROUND REINFORCEMENT, EMBEDDED ITEMS, AND INTO CORNERS OF FORMS ELIMINATING AIR OR STONE POCKETS THAT MAY CAUSE HONEYCOMBING, PITTING, OR PLANES OF WEAKNESS.
 - SPOT CHECK THAT CURING PROCEDURES COMPLY WITH THE CONTRACT DOCUMENTS, ACI 308, STANDARD PRACTICE FOR CURING CONCRETE, AND OTHER RECOGNIZED INDUSTRY STANDARDS.
 - FOLLOWING FORMWORK REMOVAL, SPOT CHECK CONCRETE SURFACES FOR HONEYCOMBING AND VOIDS.
- J. INSPECT EXPANSION ANCHORS AND CHEMICAL ADHESIVE FOR ANCHORING STEEL REINFORCEMENT AND THREADED RODS USED TO SUPPORT WORK DESCRIBED IN THIS PLAN AS FOLLOWS:
- VERIFY HOLE DIAMETER, DEPTH, LOCATION, SPACING, EDGE DISTANCE, AND CONFIRM THAT THE HOLE HAS BEEN THOROUGHLY CLEANED AS REQUIRED BY MANUFACTURER.
 - VERIFY THAT EXPANSION ANCHORS ARE PROPERLY TIGHTENED.
 - VERIFY THAT THE EPOXY TYPE IS PROPER FOR THE APPLICATION, SUCH AS PASTE FOR HORIZONTAL HOLES. VERIFY THAT EPOXY MIXING AND INSTALLATION COMPLIES WITH MANUFACTURER'S REQUIREMENTS.

SHALLOW FOUNDATIONS

- A. THE SPECIAL INSPECTOR SHALL REVIEW THE SIZE AND CONFIGURATION OF FOUNDATIONS FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS.
- B. THE SPECIAL INSPECTOR SHALL REVIEW THE SIZE, PLACEMENT, AND CLEARANCES OF REINFORCEMENT FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS.
- C. THE SPECIAL INSPECTOR SHALL REVIEW DOWEL REINFORCEMENT AND DOWEL LENGTHS SUCH THAT LAP SPLICES WILL BE IN COMPLIANCE WITH THE CONTRACT DOCUMENTS.
- D. THE SPECIAL INSPECTOR SHALL VERIFY THAT REINFORCEMENT IS SUPPORTED AND TIED SUCH THAT IT WILL NOT BE DISPLACED DURING THE CONCRETE POUR.

SOIL OR FOUNDATION PREPARATION

- A. THE GEOTECHNICAL ENGINEER OF RECORD FOR THE PROJECT SHALL BE RETAINED BY THE FEE OWNER TO MONITOR THE SITE PREPARATION ACTIVITIES, FOUNDATION BEARING SURFACES, AND PILE FOUNDATION ELEMENTS.
- B. THE GEOTECHNICAL ENGINEER SHALL PROVIDE THE SPECIAL INSPECTOR WITH DAILY REPORTS AND A SUMMARY REPORT INDICATING THAT THE FOUNDATION BEARING SURFACE IS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE GEOTECHNICAL REPORT, SUCH THAT THE FOUNDATION FOR THE PROJECT WILL FUNCTION AS INTENDED.
- C. THE INDEPENDENT TESTING AGENCY RETAINED BY THE FEE OWNER SHALL MONITOR THE BACKFILL AND COMPACTION OPERATIONS AND PROVIDE THE SPECIAL INSPECTOR WITH REPORTS DOCUMENTING THAT THE OPERATIONS HAVE BEEN PERFORMED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT AND THE CONTRACT DOCUMENTS.
- D. ALL INSPECTION AND TESTING REPORTS PROVIDED TO THE SPECIAL INSPECTOR SHALL BE SIGNED AND SEALED BY AN ENGINEER LICENSED IN FLORIDA.

MASONRY

- A. THE SPECIAL INSPECTOR SHALL INSPECT ALL REINFORCED MASONRY PER ACI 530, ACI 530.1 AND AS FOLLOWS:
- SPOT CHECK THAT MATERIALS ARE PROPERLY STORED.
 - WORKMANSHIP.
 - UNIT SIZE, STRENGTH, AND WEIGHT.
 - MORTAR AND GROUT TYPE, MIXING, AND PLACEMENT.
 - PLACEMENT OF STEEL REINFORCEMENT, JOINT REINFORCEMENT, INSERTS, ANCHORS, AND OTHER STRUCTURAL ASPECTS.
 - CHECK CLEANOUT AREAS TO CONFIRM THAT CELLS TO BE REINFORCED ARE CLEAN AND FREE OF ALL FOREIGN MATERIAL, AND THAT VERTICAL BARS ARE EACH TIED TO A DOWEL.
 - CONTRACTION JOINT CONSTRUCTION.
 - DURING GROUTING, VERIFY THAT ALL CELLS ARE FILLED SOLID AND THAT GROUT IS CONSOLIDATED AS REQUIRED BY THE CONTRACT DOCUMENTS. IF IN DOUBT AFTER COMPLETION, CHECK USING A HAMMER.
 - CHECK THE BEAM AND THE COLUMN SIZE, SPACING, AND REINFORCEMENT.
 - CHECK CONNECTION OF SIDE OR TOP OF WALL TO SUPPORTING ELEMENTS.

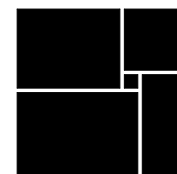
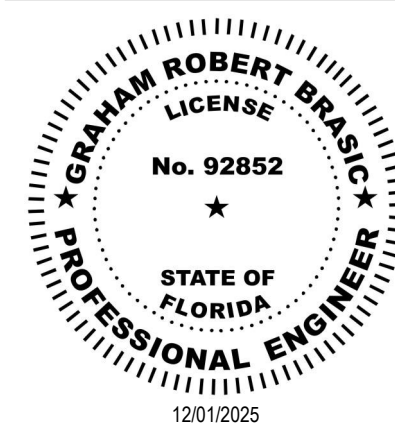
POST-INSTALLED ANCHORS

- A. THE STRUCTURAL INSPECTOR SHALL:
- PERFORM SPECIAL INSPECTIONS FOR ALL POST-INSTALLED ANCHORS PER THE PRODUCT MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS, APPLICABLE EVALUATION REPORTS, AND ANY ADDITIONAL REQUIREMENTS INVOKED WITHIN THE CONSTRUCTION DOCUMENTS. THIS INCLUDES, BUT IS NOT LIMITED TO, THE FOLLOWING:
 - ANCHORS MATCH INTENDED MANUFACTURER AND PRODUCT FOR EACH APPLICATION, OR APPROVED ALTERNATE SUBMITTED BY THE CONTRACTOR AND APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
 - ANCHOR PARAMETERS:
 - TYPE AND SPECIFIC PRODUCT
 - DIMENSIONS (DIAMETER & LENGTH)
 - MATERIAL/COATING
 - ADHESIVE ANCHOR EXPIRATION DATE
 - ANCHOR OR THREAD DAMAGE
 - PRESENCE OF SUBSTANCES THAT MAY INTERFERE WITH BOND (DUST, MUD, OIL, LOOSE RUST)
 - PLACEMENT:
 - RELATIVE TO FIXTURE GEOMETRY
 - RELATIVE TO OTHER ANCHORS (SPACING)
 - RELATIVE TO SUBSTRATE, CONFIRMING MINIMUM EDGE DISTANCES COMMUNICATED WITHIN THE CONTRACT DOCUMENTS
 - RELATIVE TO REBAR AND EMBEDMENTS WITHIN THE SUBSTRATE, CONFIRMING THAT NONDESTRUCTIVE TESTING (NOT) HAS BEEN PERFORMED ACCORDINGLY TO CONFIRM THAT DRILLING AND ANCHOR INSTALLATION DOES NOT DAMAGE EXISTING ELEMENTS
 - AGE OF SUBSTRATE MATERIAL (21 DAYS MINIMUM FROM CONCRETE PLACEMENT FOR ADHESIVE ANCHOR INSTALLATION)
 - MINIMUM CONCRETE THICKNESS OF SUBSTRATE TO RECEIVE POST-INSTALLED ANCHORS.
 - HOLES/DRILLING:
 - DRILLING METHOD (BIT TYPE AND DIAMETER)
 - HOLE DEPTH RELATIVE TO MANUFACTURER REQUIREMENTS FOR SPECIFIED NOMINAL EMBEDMENT
 - CLEANING PROCEDURES
 - PRESENCE OF ADJACENT EXISTING/ABANDONED HOLES
 - HOLE PROTECTION FOR CONTAMINANTS, DEBRIS, OR WATER/MOISTURE
 - TIMING OF CLEANING IMMEDIATELY BEFORE ANCHOR INSTALLATION, RECLEANING AS NECESSARY.
 - INSTALLATION AND TESTING:
 - TEMPERATURE LIMITS
 - SPECIFIED TORQUE AND NUMBER OF FULL TURNS REQUIRED TO ACHIEVE TORQUE. NOTIFY STRUCTURAL ENGINEER OF RECORD WHEN ANCHORS FAIL TO SET WITHIN MAXIMUM PERMITTED NUMBER OF TURNS PER MANUFACTURER
 - PROJECTION LENGTH AND REQUIRED EMBEDMENT.
 - ADHESIVE QUANTITY DISPENSED FOR HOLE DEPTH TO CONFIRM AIR BUBBLES/VOIDS ARE NOT PRESENT
 - APPROPRIATE ADHESIVE MIXING AND METERING PROCEDURES AND EQUIPMENT, INCLUDING MANUFACTURER REQUIREMENTS SPECIAL EQUIPMENT IF APPLICABLE FOR LONG HOLES AND HORIZONTAL/OVERHEAD APPLICATIONS
 - REVIEW FOR SYMPTOMS OF AIR BUBBLES WITHIN ADHESIVE (SPRING BACK OR POPPING SOUNDS DURING ANCHOR INSTALLATION)
 - WASHER CONDITIONS AND FLUSH CONDITION WITH FIXTURE
 - CONFIRM THAT ADHESIVE ANCHOR INSTALLERS HAVE THE NECESSARY CERTIFICATIONS (ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT).
 - PERFORM CONTINUOUS SPECIAL INSPECTIONS WHEN THERE IS A CHANGE IN PERSONNEL PERFORMING ANCHOR INSTALLATION.
 - PERFORM CONTINUOUS INSPECTIONS FOR ADHESIVE ANCHORS INSTALLED HORIZONTALLY, UPWARDLY, INCLINED, OR OVERHEAD.
 - BE FAMILIAR WITH PROOF TESTING REQUIREMENTS AND THE SUBMITTED ROOF TESTING PLAN BY THE INDEPENDENT TESTING LABORATORY. SEE GENERAL NOTES UNDER "POST INSTALLED ANCHORS"
 - VERIFY THAT FEE OWNER'S TESTING AGENCY HAS TESTED ALL REQUIRED ANCHORS PER THE APPROVED POST-INSTALLED ANCHOR TEST PLAN.
 - VERIFY THAT ANY FAILED ANCHORS HAVE BEEN REPLACED.
 - VERIFY THAT THE TESTING AGENCY HAS SUBMITTED SIGNED AND SEALED TEST REPORTS TO THE STRUCTURAL ENGINEER OF RECORD.

STRUCTURAL STEEL FRAMING

- A. THE SPECIAL INSPECTOR SHALL:
- INSPECT ALL STRUCTURAL STEEL FRAMING AS REQUIRED BY THE CONTRACT DOCUMENTS. USE BOTH THE CONTRACT DOCUMENTS AND THE SHOP DRAWINGS FOR ALL INSPECTIONS. VERIFY THAT ALL FIELD WELDERS ARE AWS CERTIFIED FOR THE TYPE OF WELDS BEING MADE.
 - PROVIDE ALL INSPECTIONS REQUIRED BY THE CONTRACT DOCUMENTS. COMPLETE ALL INSPECTIONS AND VERIFY COMPLIANCE PRIOR TO CONCRETE PLACEMENT OR CONCEALMENT.
 - CHECK STRUCTURAL STEEL FRAMING FOR POSSIBLE DAMAGE DURING SHIPPING.
 - CHECK STRUCTURAL STEEL FRAMING SIZES AND GRADES.
 - SPOT CHECK MEMBER STRAIGHTNESS, FINISH, AND CAMBER. CONFIRMING THAT PAINT IS ONLY APPLIED ON APPROPRIATE SURFACES.
 - INSPECT SETTING OF ANCHOR BOLTS, EMBEDS, AND OTHER MISCELLANEOUS STRUCTURAL ITEMS FOR SIZE, QUANTITY AND FINISH. CHECK THE INSTALLATION OF COLUMN BASE PLATES FOR PROPER LEVELING, GROUT TYPE, AND GROUT APPLICATION.
 - INSPECT ALL FIELD CONNECTIONS AND SPOT CHECK SHOP CONNECTIONS. VERIFY CONNECTION MATERIAL, SIZES, CONFIGURATIONS, AND FIT-UP.
 - VISUALLY EXAMINE ALL FIELD WELDS AND SPOT CHECK ALL SHOP WELDS FOR TYPE, SIZE, LENGTH, AND QUALITY. VERIFY THAT SPECIFIED TESTING IS PERFORMED BY THE TESTING AGENCY. VERIFY THAT WELDS ARE CLEAN AND FREE FROM SLAG AND THAT RUST PROTECTION HAS BEEN APPLIED PER THE SPECIFICATIONS.
 - VERIFY THE TYPE, SIZE, AND QUANTITY OF BOLTS IN ALL BOLTED CONNECTIONS. CHECK THAT BOLTS ARE CLEAN AND LUBRICATED, HAVE PROPER WASHERS, AND CONFORM TO THE SPECIFICATIONS. ENSURE BOLT HOLES ARE THE SPECIFIED TYPE AND SIZE. SPOT CHECK THE BOLT TIGHTENING SEQUENCE. VISUALLY VERIFY PROPER DEGREE OF BOLT TIGHTENING, PARTICULARLY OF ALL SLP CRITICAL BOLTS. CHECK 10% OF LOAD INDICATOR WASHERS WITH A FEELER GAUGE.
 - CHECK HEADED STUD ANCHORS FOR SIZE, LENGTH, SPACING, AND WELDING AS REQUIRED BY THE CONTRACT DOCUMENTS. VERIFY THAT SHEAR CONNECTIONS ARE TESTED BY TESTING LABORATORY.
 - CHECK ALL STEEL DECKS PER SDI SPECIFICATIONS AND AS FOLLOWS:
 - DECK TYPE, SIZE, GAGE, FINISH, ACCESSORIES, AND REINFORCEMENT AROUND OPENINGS.
 - SPACING AND TYPE OF ALL CONNECTIONS.
 - WELDING PROCEDURES.
 - WELD SIZE, SHAPE, QUALITY, RUST PROTECTION, AND WELD WASHERS WHERE APPLICABLE.
 - SCREW SIZE, TYPE, AND FINISH AND OTHER CONNECTION TO SUPPORTS.

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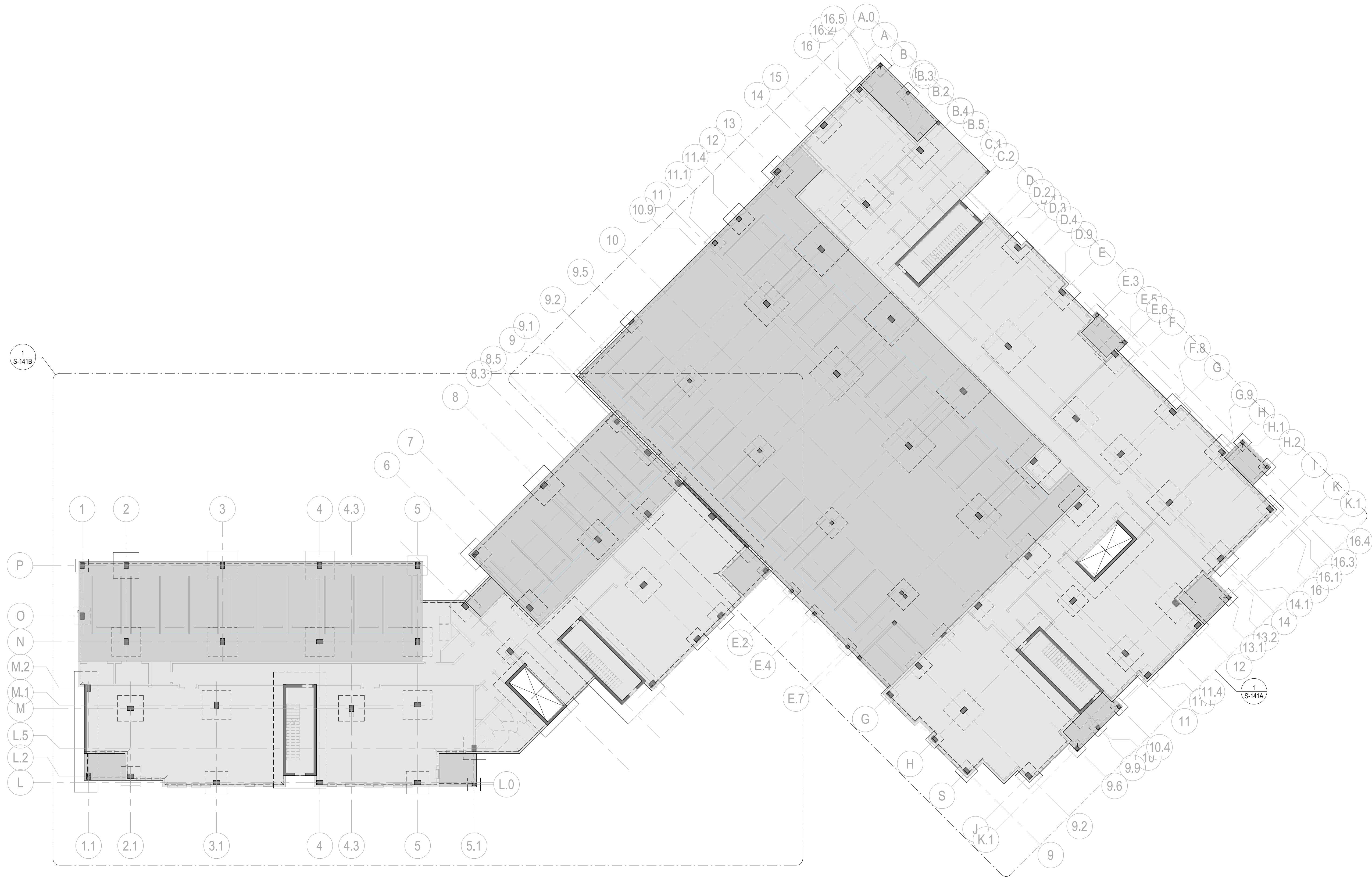
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SPECIAL
INSPECTIONS
PLAN

S-010


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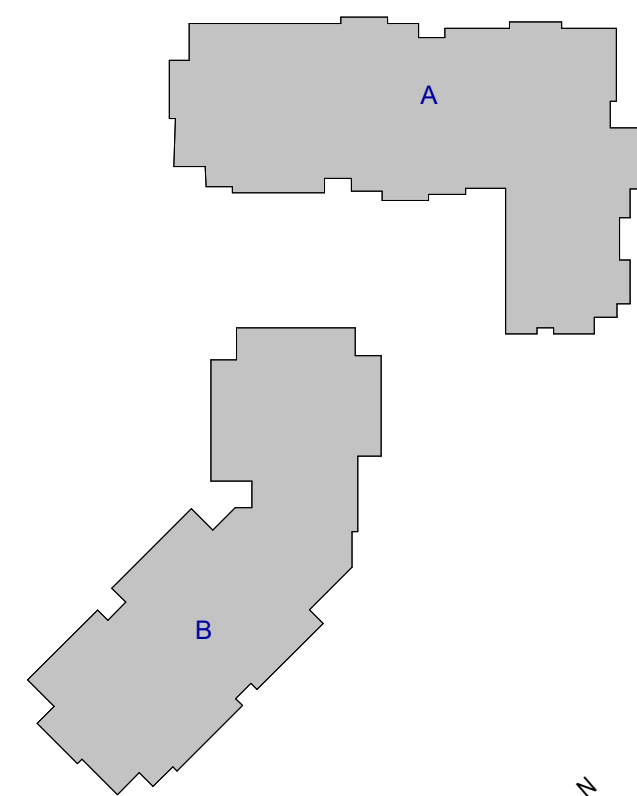
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1
S-121

OVERALL FOUNDATION PLAN

1/16" = 1'-0"



KEY PLAN



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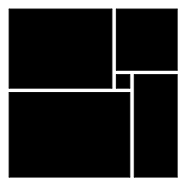
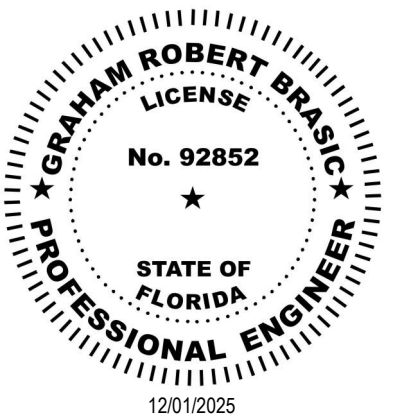
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	12/01/25	CONSTRUCTION DOCUMENTS
	08/22/25	DESIGN DEVELOPMENT
	04/07/25	SCHEMATIC DESIGN
NO.	DATE	DESCRIPTION

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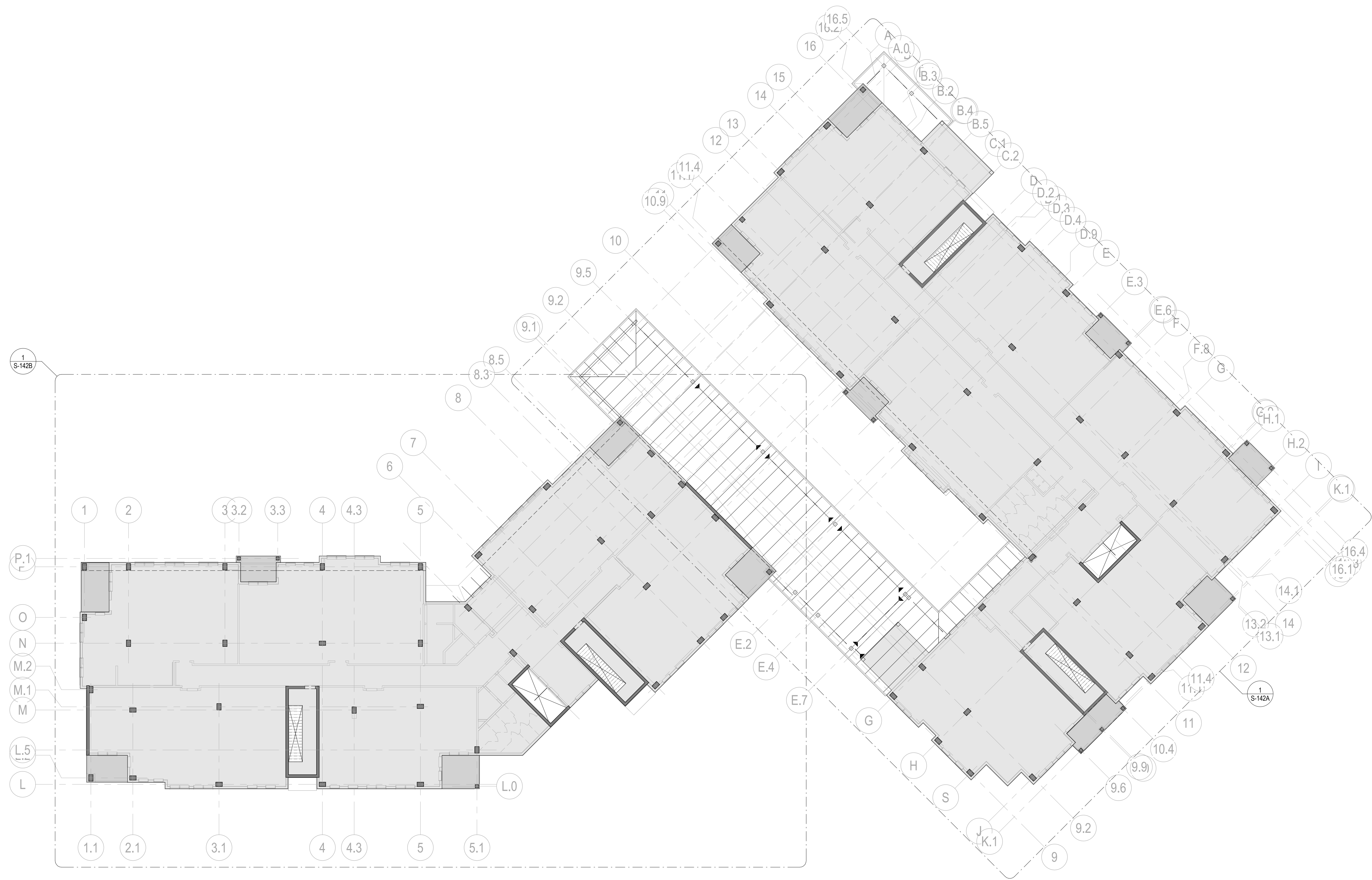
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OVERALL
FOUNDATION
PLAN

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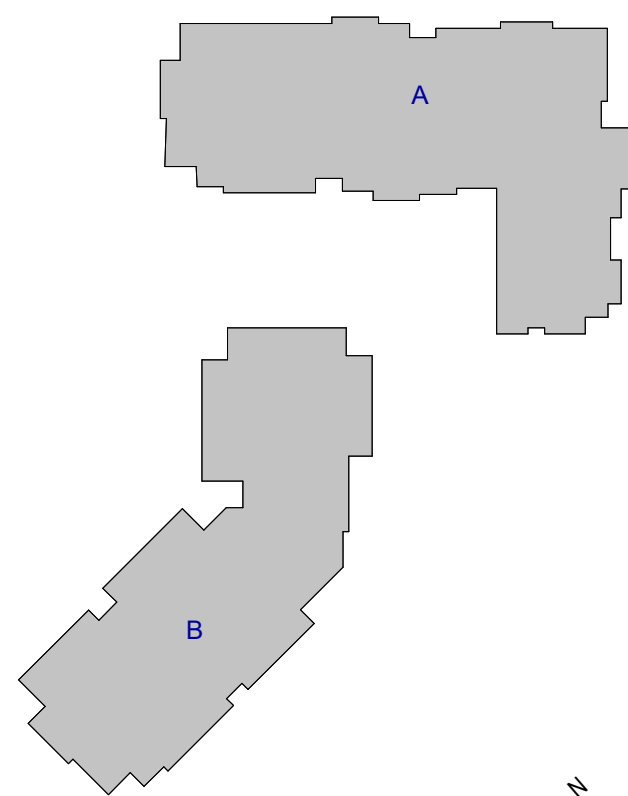


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1
S-123

THIRD FLOOR FRAMING PLAN

1/16" = 1'-0"



KEY PLAN

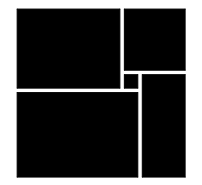
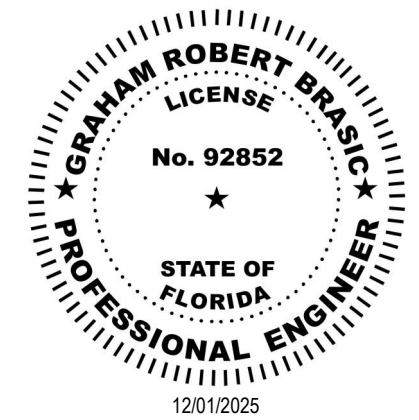


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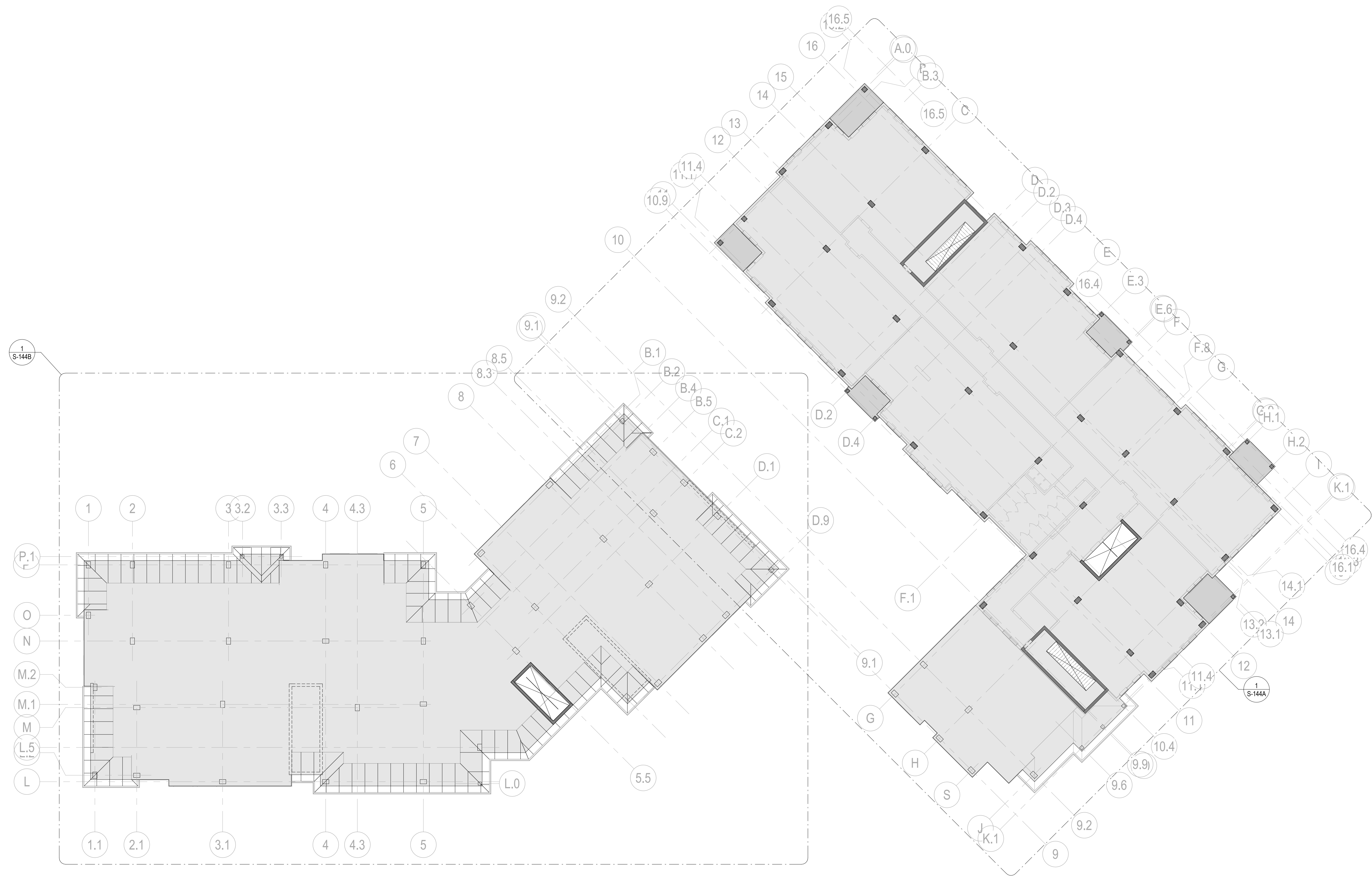
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OVERALL
THIRD FLOOR
PLAN

S-123

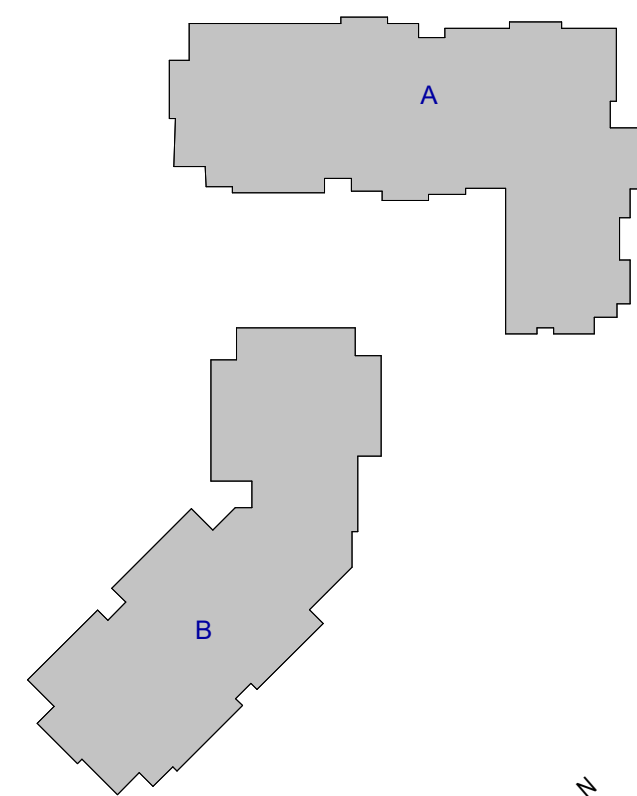
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S-124

OVERALL FOURTH FLOOR PLAN

1/16" = 1'-0"



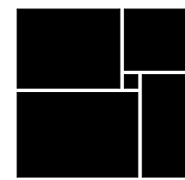
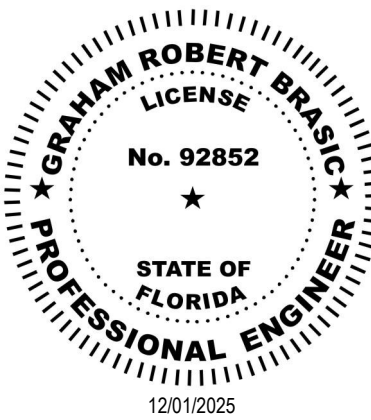
KEY PLAN



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OVERALL
FOURTH
FLOOR PLAN

S-124

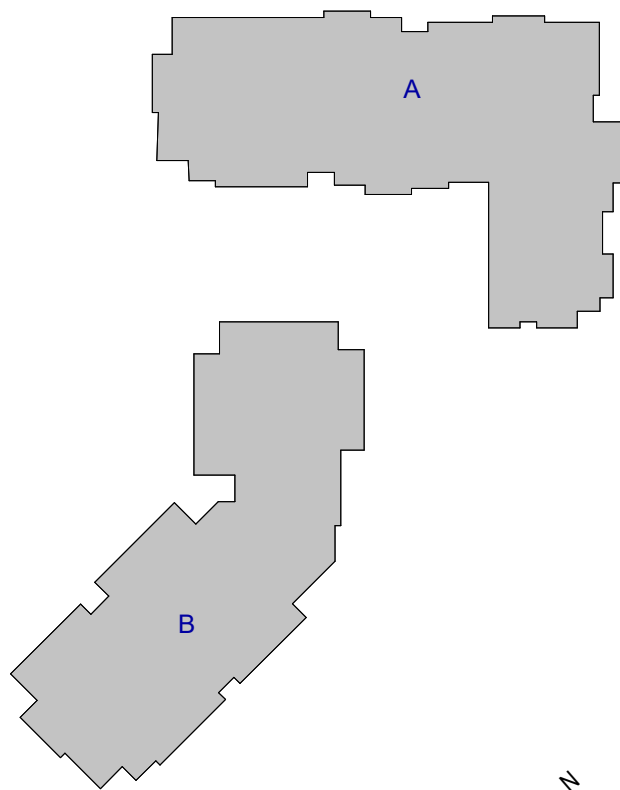
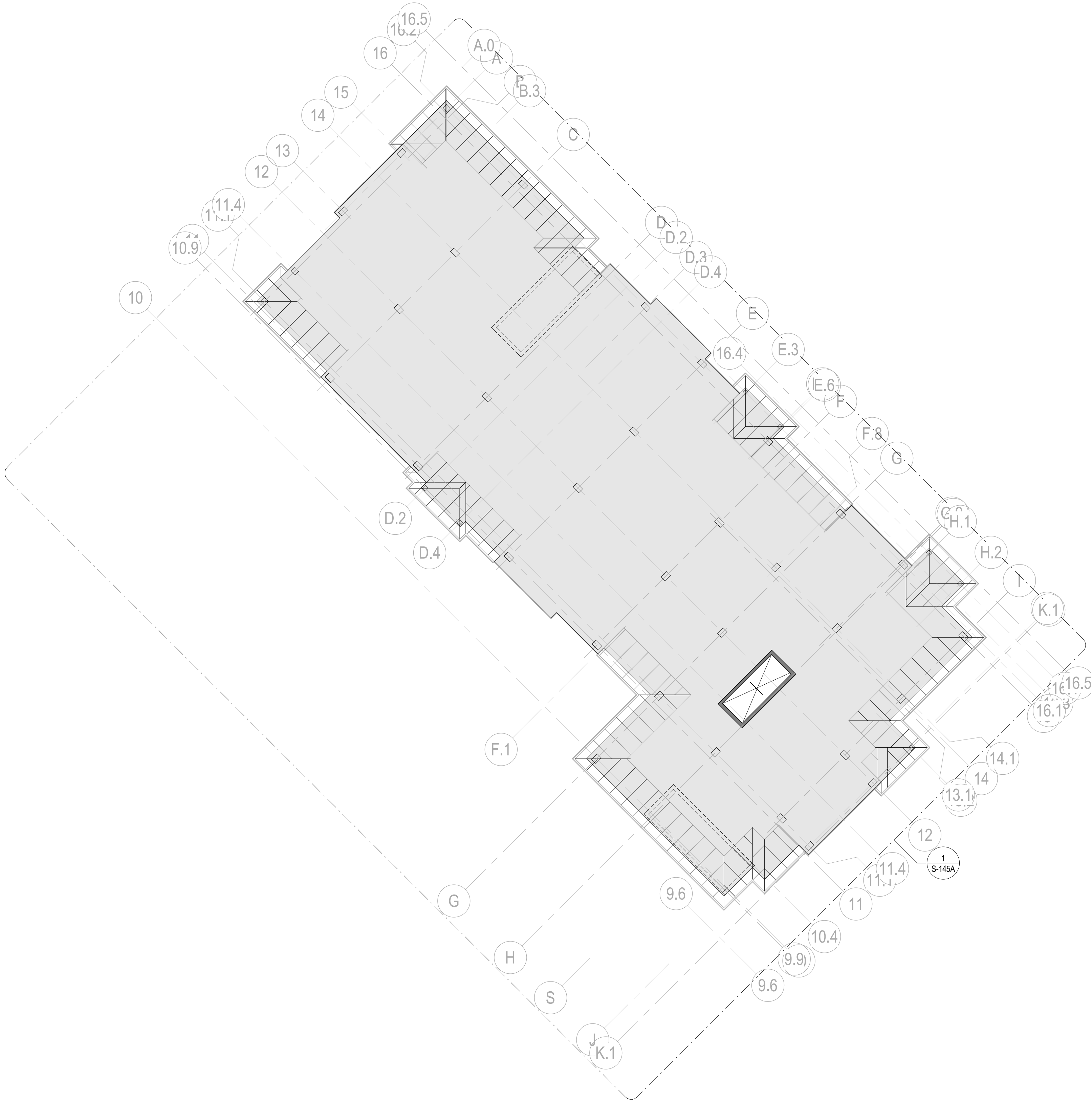
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S-125

OVERALL ROOF PLAN

1/16" = 1'-0"

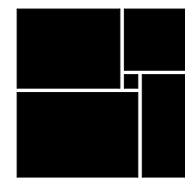
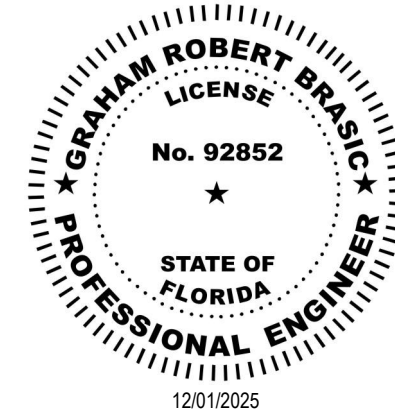


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OVERALL
ROOF PLAN

S-125

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1 S-141A PARTIAL FOUNDATION FRAMING PLAN - AREA A 1/8" = 1'-0"

FOUNDATION PLAN NOTES:

- X-# LOCATION MARK
CC#/# COLUMN MARK (SEE SCHEDULE ON THIS SHEET AND NOTE 2)
F#, W#, OR P# FOUNDATION OR PIER MARK (SEE SCHEDULE ON THIS SHEET)
(+) 2'-0" TOP OF FOOTING/PIER ELEVATION
- DENOTES CONCRETE COLUMN/PIER WALL FOR COLUMN SCHEDULE AND DETAILS SEE S-340 SERIES. FOR WALL DETAILS SEE S-320 SERIES.
- FOR TRENCHES ADJACENT TO FOUNDATIONS, SEE 1/ S-300. FOR PIPING PASSING UNDER WALL FOUNDATIONS, SEE 2/ S-300. PIPING PASSING UNDER FOOTINGS SHALL BE INSPECTED BEFORE FOUNDATIONS ABOVE ARE PREPARED.
- GENERAL CONTRACTOR SHALL COORDINATE PLUMBING AND UTILITIES LOCATIONS WITH FOUNDATIONS AS NEEDED. ADDITIONALLY, GENERAL CONTRACTOR SHALL COORDINATE FOUNDATION ELEVATIONS WITH PLUMBING AND UTILITIES AS NEEDED. FORWARD ANY FOUNDATION LOCATION CHANGE REQUESTS TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW AND APPROVAL.
- DENOTES STEP IN FOUNDATION, SEE 4/ S-300.
- DENOTES LOAD BEARING CONCRETE MASONRY UNIT (CMU) WALL WITH #X AT #X OC VERTICAL REINFORCEMENT, CENTERED IN FULLY-GROUTED CELL.
- DENOTES NON-LOAD BEARING CONCRETE MASONRY UNIT (CMU) WALL WITH #X AT #X OC VERTICAL REINFORCEMENT, CENTERED IN FULLY-GROUTED CELL.
- DENOTES SHOWER DEPRESSION.

CONTINUOUS WALL FOOTING SCHEDULE

MARK	GEOMETRY		REINFORCEMENT	
	WIDTH	THICKNESS	LONGITUDINAL	TRANSVERSE
WF-3	3'-0"	1'-0"		

MAT FOUNDATION SCHEDULE

MARK	THICKNESS	REINFORCEMENT	
		TOP	BOTTOM
MF18A	18"	#6 @ 12" OC EW	#6 @ 10" OC EW
MF18B	18"	#6 @ 12" OC EW	#6 @ 12" OC EW
MF24	24"	#6 @ 12" OC EW	#6 @ 12" OC EW

ISOLATED FOOTING SCHEDULE

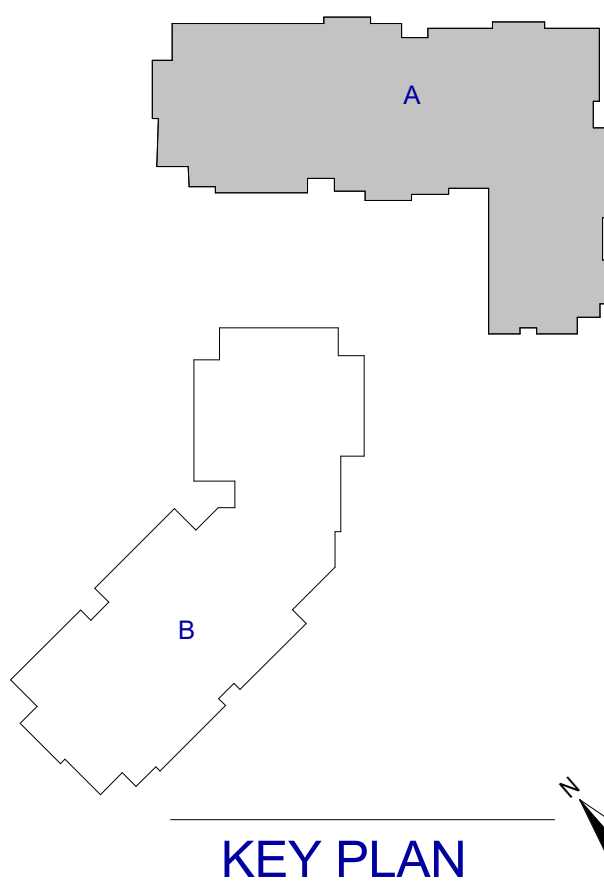
MARK	GEOMETRY			REINFORCEMENT	
	WIDTH	LENGTH	THICKNESS	TOP	BOTTOM
F4	4'-0"	4'-0"	14"	(4) - #5	
F5	5'-0"	5'-0"	14"	(5) - #5	
F6	6'-0"	6'-0"	14"	(7) - #5	
F7	7'-0"	7'-0"	16"	(9) - #5	
F7.5	7'-0"	7'-0"	16"	(9) - #5	
F8	6'-0"	8'-0"	16"	(9) - #6	
F9	9'-0"	9'-0"	18"	(9) - #7	
F9-12	9'-0"	12'-0"	20"	#6 @ 12" OC EW	#7 @ 10" OC EW
F10	10'-0"	10'-0"	20"	(11) - #7	
F11	11'-0"	11'-0"	22"	(13) - #7	
F12	12'-0"	12'-0"	24"	(12) - #8	

SLAB-ON-GROUND PLAN NOTES:

- REFERENCE BUILDING TOP-OF-SLAB ELEVATION = 0'-0" AT T/S LAB AT FIRST FLOOR. (SEE CIVIL FOR ABSOLUTE ELEVATION).
- SOG4— DENOTES SLAB-ON-GROUND MARK (SEE SCHEDULE ON THIS SHEET).
- SEE S-310 FOR SLAB-ON-GROUND CONTROL JOINT AND REINFORCEMENT REQUIREMENTS.
- INSTALL FOUNDATION AT BASE OF STAIR, SEE 6/ S-360.
- FOR THICKENED SLAB UNDER NON-LOAD BEARING CONCRETE MASONRY WALLS, SEE 10/ S-310.
- GENERAL CONTRACTOR SHALL COORDINATE HOUSEKEEPING PAD LOCATIONS.
- DENOTES STEP IN TOP OF SLAB, SEE 9/ S-310.
- SEE CIVIL DRAWINGS FOR BASE AND SUBGRADE PREPARATION INFORMATION.
- SEE ARCHITECTURAL DRAWINGS FOR:
 - VAPOR BARRIER REQUIREMENTS AND LOCATIONS.
 - ALL SLOPED SLAB AREAS. (MAINTAIN SLAB THICKNESS NOTED ON PLAN AS A MINIMUM IN ALL AREAS).
 - ALL DEPRESSIONED SLAB AND/OR RAISED SLAB AREAS. (MAINTAIN SLAB THICKNESS NOTED ON PLAN AS A MINIMUM IN ALL AREAS).
 - ALL DIMENSIONS NOT SHOWN, VERIFY ALL DIMENSIONS SHOWN IN STRUCTURAL DRAWINGS WITH ARCHITECTURAL DRAWINGS AND REPORT ANY DISCREPANCIES OR DIMENSIONS NOT SHOWN ON ARCHITECTURAL DRAWINGS FOR CLARIFICATION.
 - SLAB SLOPES, DRAINS, STEPS, PENETRATIONS, FINISHES, AND ANY OTHER ADDITIONAL INFORMATION.

SLAB-ON-GROUND SCHEDULE

MARK	THICKNESS	REINFORCEMENT
SOG4	4"	
SOG5	5"	



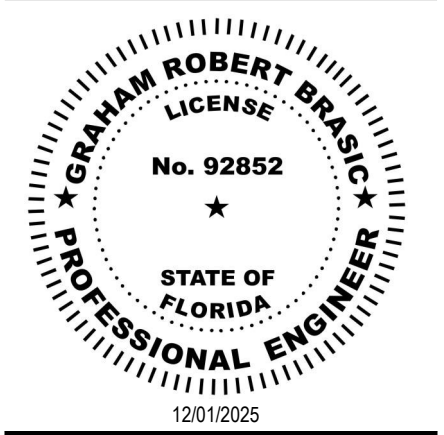
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Project No.: 2021009
Date: 12/01/2025

PARTIAL
FOUNDATION
PLAN AREA A

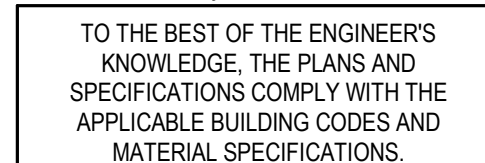
S-141A

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S-141B



A key plan map showing the location of the study area (shaded grey) within the larger context of the study area (white). The map includes a north arrow and a scale bar.



- | <u>SLAB-ON-GROUND SCHEDULE</u> | | |
|---------------------------------------|------------------|----------------------|
| MARK | THICKNESS | REINFORCEMENT |
| SOG4 | 4" | |
| SOG5 | 5" | |

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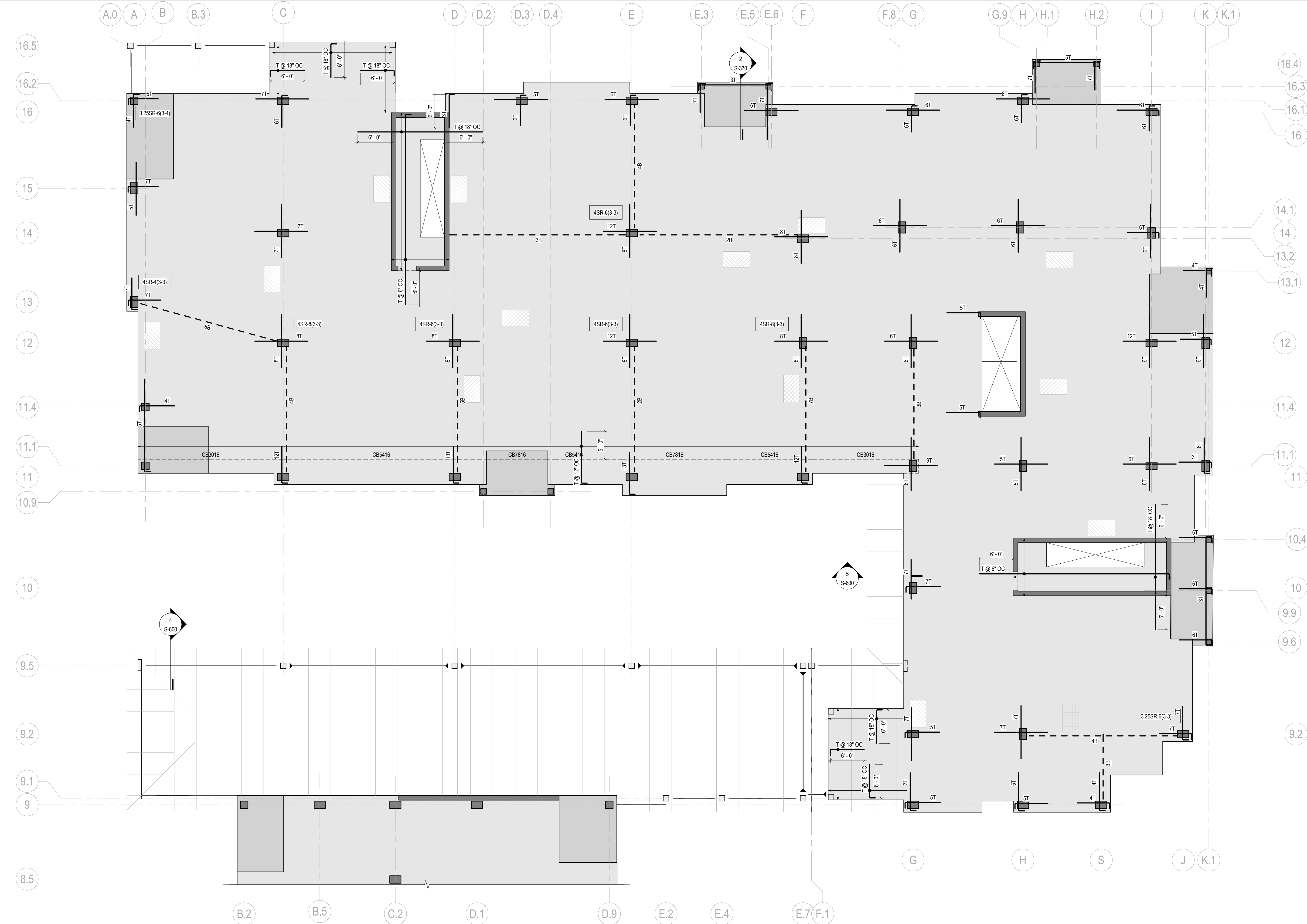




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S-142A-PT

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1
S-142A-R
PARTIAL SECOND FLOOR REINFORCING PLAN - AREA A
1/8" = 1'-0"

CONCRETE SLAB REINFORCEMENT PLAN NOTES:

- FOR TYPICAL SLAB REINFORCEMENT DETAILS, SEE S-330 SERIES. REINFORCING ON PLAN SHALL BE IN ADDITION TO REINFORCING IN TYPICAL SLAB REINFORCEMENT DETAILS.
- PROVIDE (4) - #5 CONTINUOUS REINFORCEMENT (2 TOP & 2 BOTTOM, SPACED AT 6" OC) AROUND PERIMETER OF SLAB AND AT ALL INTERIOR SLAB EDGES. SEE 11/ S-331 FOR CONCEPTUAL LAYOUT (NOT INDICATIVE OF PROJECT SPECIFIC GEOMETRY) AND SPICED DEVELOPMENT REQUIREMENTS.
- SLAB REINFORCEMENT SHALL BE #5 UNLESS NOTED OTHERWISE.
[#SR-#(##)] DENOTES SHEAR STUD RAILS (SEE DETAILS AND SCHEDULE ON S-332).
- SLAB REINFORCEMENT SYMBOLS AND NOMENCLATURE:
A. MAT REINFORCEMENT (SEE SLAB SCHEDULE).
B. ISOLATED/DISTRIBUTED REINFORCEMENT (SEE ---).

BAR SPACING OVER WIDTH OR QUANTITY SHOWN
ACI STANDARD HOOK
(ALL TOP BARS HOOKED AT EDGE-OF-SLAB EVEN WHEN NOT INDICATED).
SOLID LINE DENOTES TOP BARS (OR TOP & BOTTOM)
DASHED LINE DENOTES BOTTOM BARS
SINGLE ARROW DENOTES EXTENT OF BAR PLACEMENT. DOUBLE ARROW DENOTES EXTENT TO CONTINUE UNTIL EDGE-OF-SLAB OR END-OF-ELEMENT (AS APPLICABLE)

CONCRETE SLAB REINFORCEMENT PLAN NOTES:

C. TYPICAL NOMENCLATURE:

DENOTES QUANTITY OF BARS
T: DENOTES TOP BARS
B: DENOTES BOTTOM BARS

D. REINFORCEMENT AT COLUMNS:

CONCRETE COLUMN
(SEE FRAMING PLAN)
BARS EACH WAY
(SEE TYPICAL NOMENCLATURE)

POST-TENSIONED CONCRETE SLAB SCHEDULE

MARK	THICKNESS	TYPICAL REINFORCEMENT	
		TOP	BOTTOM
PT8	8"	SEE PLAN	#4 @ 48" OC EW

SHEAR STUD RAIL SCHEDULE

MARK	FIRST STUD SPACING	TYP STUD SPACING	# OF STUDS PER RAIL	NUMBER OF RAILS	
				SHORT SIDE	LONG SIDE
3.25SR-6(3-3)	2"	3 1/4"	6	3	3
3.25SR-6(3-4)	2"	3 1/4"	6	3	4
4SR-4(3-3)	3 1/4"	4"	4	3	3
4SR-6(3-3)	3 1/4"	4"	6	3	3
4SR-8(3-3)	3 1/4"	4"	8	3	3
4SR-14(3-3)	3 1/4"	4"	14	3	3
4SR-16(3-3)	3 1/4"	4"	16	3	3
4SR-20(3-3)	3 1/4"	4"	20	3	3
4SR-22(3-3)	3 1/4"	4"	22	3	3

CONCRETE BEAM SCHEDULE

MARK	GEOMETRY		REINFORCEMENT		
	WIDTH	DEPTH	TOP	BOTTOM	STIRRUPS
CB3016	30"	16"	(6)#6	(6)#6	#3 @ 10" OC (3 LEGS)
CB5416	54"	16"	(14)#6	(14)#6	#3 @ 10" OC (4 LEGS)
CB7816	78"	16"	(18)#6	(18)#6	#3 @ 10" OC (5 LEGS)

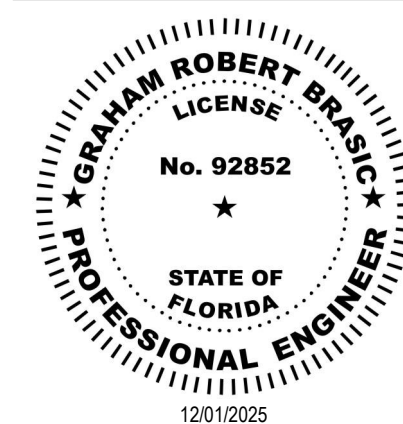


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KEY PLAN

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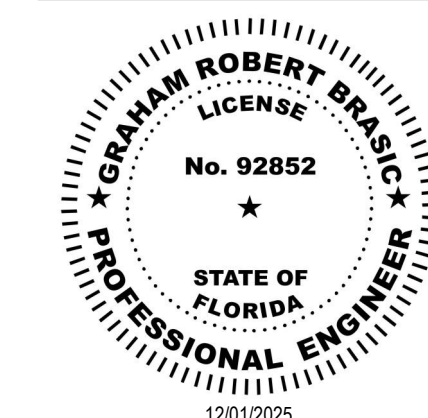
Project No.: 2021009
Date: 12/01/2025

PARTIAL
SECOND
FLOOR
REINFORCING
PLAN AREA A

S-142A-R

	12/01/25	CONSTRUCTION DOCUMENTS
	08/22/25	DESIGN DEVELOPMENT
NO.	DATE	DESCRIPTION

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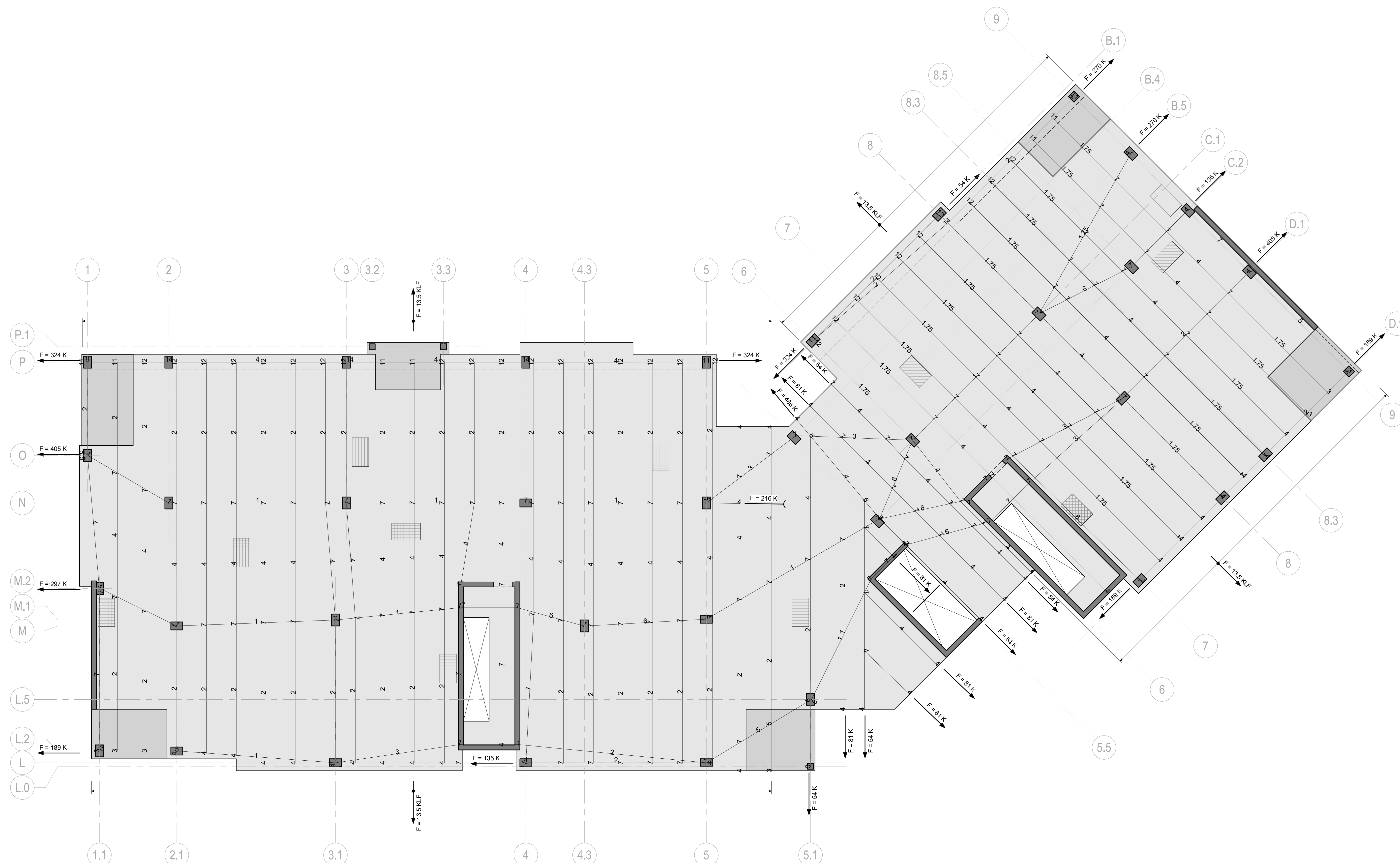
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Project No.: 20210
Date: 12/01/20

**PARTIAL
SECOND
FLOOR PT
PLAN AREA B**

S-142B-PT



1 PARTIAL SECOND FLOOR PT PLAN - AREA B
S-142B-PT 1/8" = 1'-0"

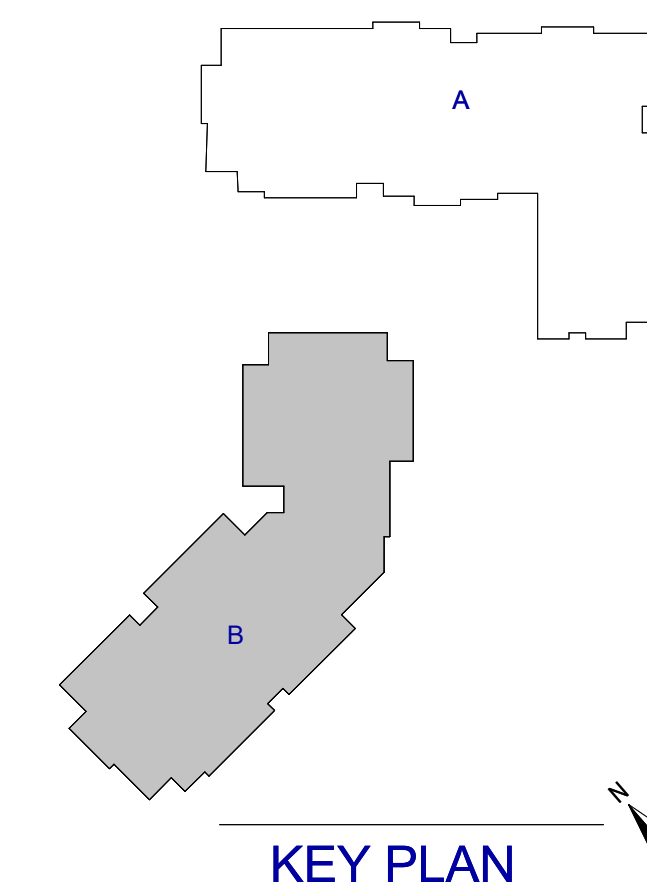
POST-TENSIONED CONCRETE SLAB PLAN NOTES

2. SEE FRAMING PLAN FOR SLAB GEOMETRY, REFERENCE ELEVATION, AND KEYED SECTIONS.
3. SEE POST-TENSIONED FRAMING GENERAL NOTES ON SHEET S-003 AND DIVISION 03 SPECIFICATIONS FOR GENERAL REQUIREMENTS.
4. FOR TYPICAL POST-TENSIONED CONCRETE SECTIONS AND DETAILS, SEE S-330 SERIES.
5. TENDON LAYOUT NOTES:
 - LINEWORK SHOWN FOR TENDON PATHS IS SCHEMATIC TO ILLUSTRATE DESIGN INTENT, BUT NOT MEANT TO INDICATE EXACT TENDON PATHS OR SPACING OF UNIFORM/DISTRIBUTED TENDON BUNDLES.
 - MINIMUM OF 12 TENDONS SHALL PASS THROUGH EACH COLUMN IN EACH DIRECTION, WITH TENDONS LOCATED INSIDE OF THE COLUMN VERTICAL REINFORCEMENT CAGE.
 - STRAIGHT LINE HORIZONTAL OFFSETS SHOWN IN THE LAYOUT PLAN SHALL BE ACCOMPLISHED WITH SMOOTH HORIZONTAL CURVE SWEEP PER DETAIL.
 - MAINTAIN CLEAR COVER BETWEEN EDGE-OF-SLAB (INCLUDING INTERIOR OPENINGS) AS INDICATED IN THE PROJECT SPECIFICATIONS AND TYPE DETAILS REFERENCED ABOVE.
6. TENDON DRAE ELEVATION NOTES:
 - # DENOTES PT TENDON DRAE ELEVATION IN INCHES, MEASURED FROM BOTTOM OF SLAB / FRAMING TO THE CENTER-OF-GRAVITY OF THE TENDONSTRAND BUNDLE (CGG).
 - AT DEAD ENDS AND STRESSING ENDS, CGGS SHALL BE LOCATED AT THE CENTROID OF THE FRAMING (MIN. DEPTH FOR SLAB CONDITIONS), UNLESS OTHERWISE NOTED.
 - TENDON HIGH POINTS SHALL BE LOCATED AT SUPPORTS (COLUMNS, WALLS, BEAMS, BANDED TENDON LINES) AS INDICATED ABOVE.
 - TENDON LOW POINTS SHALL BE LOCATED AT MID-SPAN BETWEEN COLUMNS, UNLESS NOTED OTHERWISE.
7. TENDON FORCE NOTES:
 - $F = K$ DENOTES EFFECTIVE PRESTRESS FORCE, IN KIPS, WITHIN BANDED TENDON GROUP OR BEAM.

BAND TENDON GROUPS SHALL BE PLACED IN FLAT BANDLED GROUPS OF NO MORE THAN 5 TENDONS PER BUNDLE, SPACED AT NO MORE THAN 12 INCHES ON CENTER, WITH TENDONS FLAT IN ONE LAYER. THE QUANTITY OF BAND TENDONS SHALL BE EQUALLY DISTRIBUTED EACH SIDE OF THE COLUMN AND SHALL BE A MINIMUM OF 2 TENDONS PASSING THROUGH THE COLUMN REINFORCEMENT CAGE PER NOTE ABOVE.

- F = $F = K L_f$ DENOTES EFFECTIVE PRESTRESS FORCE, IN KIPS PER LINEAR FOOT, IN UNIFORMLY DISTRIBUTED TENDONS

UNIFORMLY DISTRIBUTED TENDON BUNDLES SHALL BE PLACED IN FLAT BANDLED GROUPS OF NO MORE THAN 5 TENDONS PER BUNDLE, SPACED AT NO MORE THAN 5'-0" ON CENTER, WITH TENDONS FLAT IN ONE LAYER.



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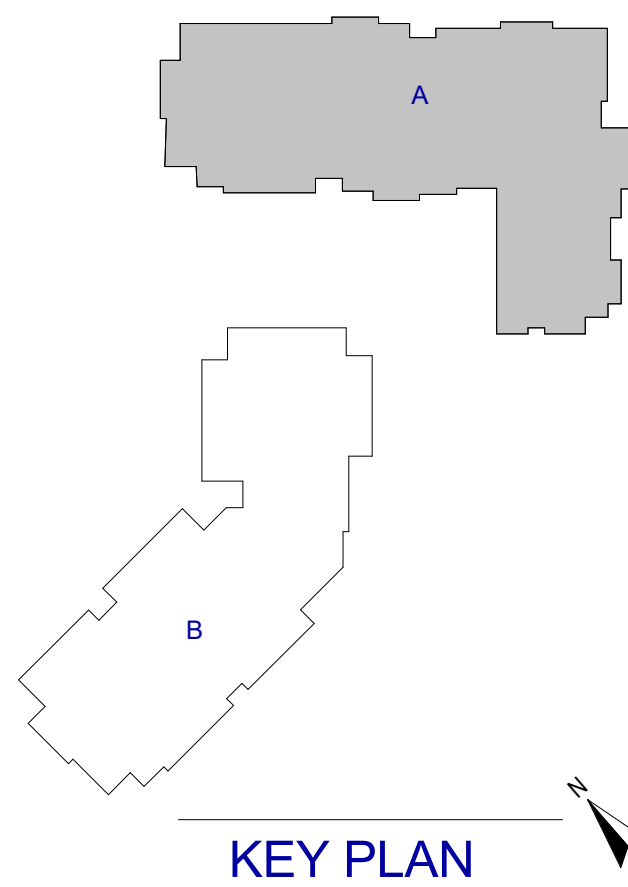
PARTIAL THIRD FLOOR FRAMING PLAN - AREA A

1/8" = 1'-0"

CONCRETE FRAMING PLAN NOTES:

- CONCRETE SLAB TAG:
PT## DENOTES SLAB MARK (SEE SCHEDULE ON THIS SHEET FOR SLAB THICKNESS AND REINFORCEMENT)
ARROWS DENOTE SLAB SPAN DIRECTION
- SEE PLAN FOR TOP OF SLAB ELEVATIONS.
- DENOTES CONCRETE COLUMN/PIER/WALL FOR COLUMN SCHEDULE AND DETAILS SEE S-340 SERIES. FOR WALL DETAILS SEE S-320 SERIES.
- SEE ARCHITECTURAL DRAWINGS FOR:
 - ALL SLOPED SLAB AREAS
 - ALL DIMENSIONS NOT SHOWN. VERIFY ALL DIMENSIONS SHOWN IN THE STRUCTURAL DRAWINGS WITH ARCHITECTURAL DRAWINGS AND REPORT ANY DISCREPANCIES OR DIMENSIONS NOT SHOWN ON THE ARCHITECTURAL DRAWINGS FOR CLARIFICATION.
- SEE MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR ADDITIONAL FLOOR PENETRATIONS, SLEEVES, AND INSERTS REQUIRED TO BE CAST IN THE SLAB.
 - SLEEVES AND PENETRATIONS WITHIN 48 INCHES OF THE FACE OF ANY CONCRETE COLUMN (NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS) SHALL BE SUBMITTED FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD.
 - SLEEVES AND PENETRATIONS INTERRUPTING BANDED LINES OF TENDONS (NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS) SHALL BE SUBMITTED FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD.
 - SLEEVES AND PENETRATIONS GREATER THAN 12 INCHES IN LENGTH OR WIDTH (NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS) SHALL BE SUBMITTED FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD.
- FOR REINFORCEMENT PLAN AND ADDITIONAL NOTES, SEE SHEET **S-144 X-R**.
- FOR POST-TENSIONED LAYOUT PLAN AND ADDITIONAL NOTES, SEE SHEET **S-144 X-PT**.
- DENOTES SHOWER DEPRESSION.

ELEVATED CONCRETE SLAB SCHEDULE				
MARK	THICKNESS	TYPICAL REINFORCEMENT		COMMENTS
		TOP	BOTTOM	
PT8	8"	SEE PLAN	#4 @ 48" OC EW	



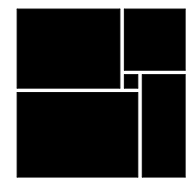
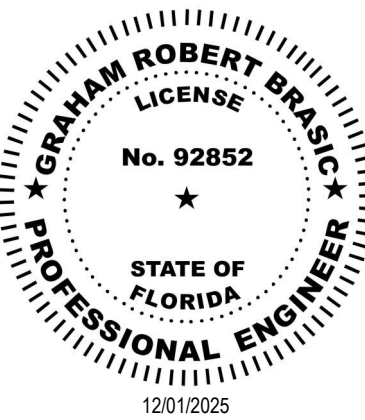
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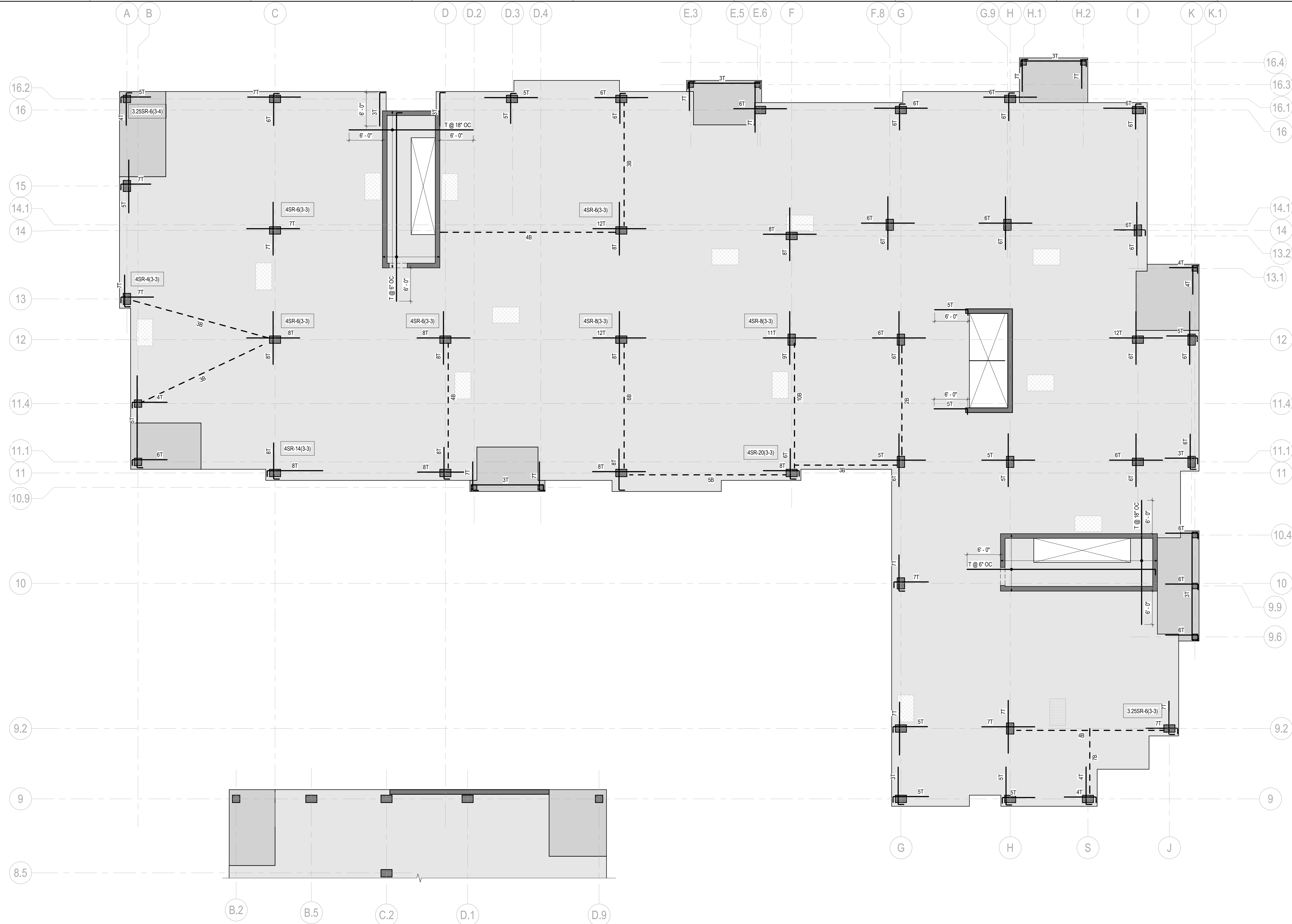
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Project No.: 2021009
Date: 12/01/2025

**PARTIAL
THIRD FLOOR
FRAMING
PLAN AREA A**

S-143A

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1
S-143A-R
PARTIAL THIRD FLOOR REINFORCING PLAN - AREA A
1/8" = 1'-0"

CONCRETE SLAB REINFORCEMENT PLAN NOTES:

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 - A. MAT REINFORCEMENT (SEE SLAB SCHEDULE).
 - B. ISOLATED/DISTRIBUTED REINFORCEMENT (SEE ---).

BAR SPACING OVER WIDTH OR QUANTITY SHOWN

ACI STANDARD HOOK
(ALL TOP BARS HOOKED AT EDGE-OF-SLAB EVEN WHEN NOT INDICATED).

SOLID LINE DENOTES
TOP BARS (OR TOP & BOTTOM)

DASHED LINE DENOTES
BOTTOM BARS

SINGLE ARROW DENOTES EXTENT OF BAR
PLACEMENT. DOUBLE ARROW DENOTES EXTENT
TO CONTINUE UNTIL EDGE-OF-SLAB OR END-OF-
ELEMENT (AS APPLICABLE)

CONCRETE SLAB REINFORCEMENT PLAN NOTES:

C. TYPICAL NOMENCLATURE:

DENOTES QUANTITY OF BARS

T: DENOTES TOP BARS

B: DENOTES BOTTOM BARS

D. REINFORCEMENT AT COLUMNS:

CONCRETE COLUMN
(SEE FRAMING PLAN)

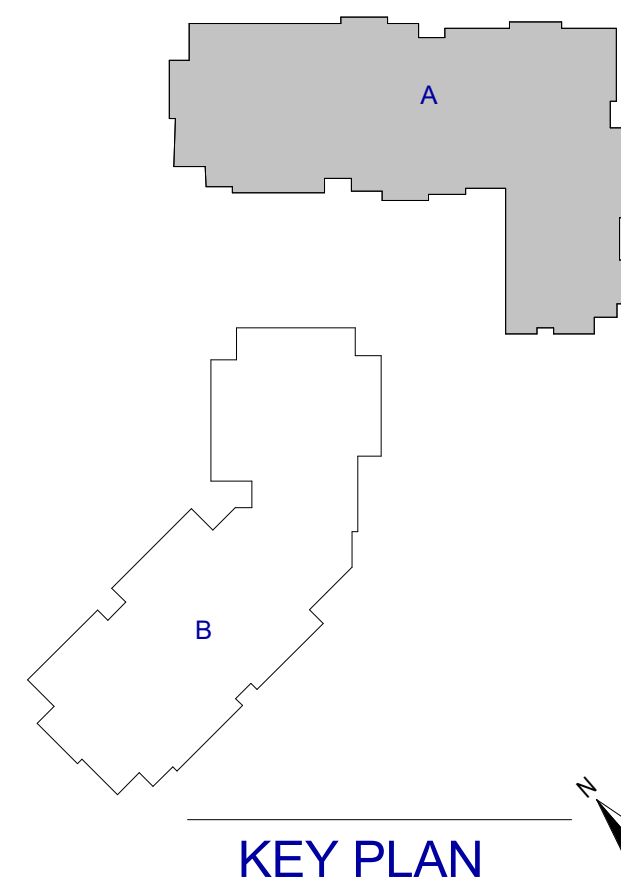
BARS EACH WAY
(SEE TYPICAL NOMENCLATURE)

POST-TENSIONED CONCRETE SLAB SCHEDULE

MARK	THICKNESS	TYPICAL REINFORCEMENT	
		TOP	BOTTOM
PT8	8"	SEE PLAN	#4 @ 48" OC EW

SHEAR STUD RAIL SCHEDULE

MARK	FIRST STUD SPACING	TYP STUD SPACING	# OF STUDS PER RAIL	NUMBER OF RAILS	
				SHORT SIDE	LONG SIDE
3.25SR-6(3-3)	2"	3 1/4"	6	3	3
3.25SR-6(3-4)	2"	3 1/4"	6	3	4
4SR-4(3-3)	3 1/4"	4"	4	3	3
4SR-6(3-3)	3 1/4"	4"	6	3	3
4SR-8(3-3)	3 1/4"	4"	8	3	3
4SR-14(3-3)	3 1/4"	4"	14	3	3
4SR-16(3-3)	3 1/4"	4"	16	3	3
4SR-20(3-3)	3 1/4"	4"	20	3	3
4SR-22(3-3)	3 1/4"	4"	22	3	3



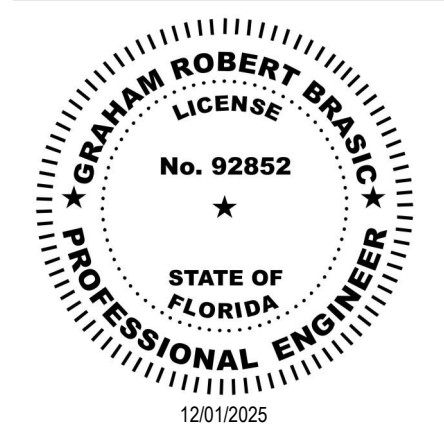
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NO.	DATE	DESCRIPTION
12/01/25	CONSTRUCTION	DOCUMENTS
08/22/25	DESIGN	DEVELOPMENT

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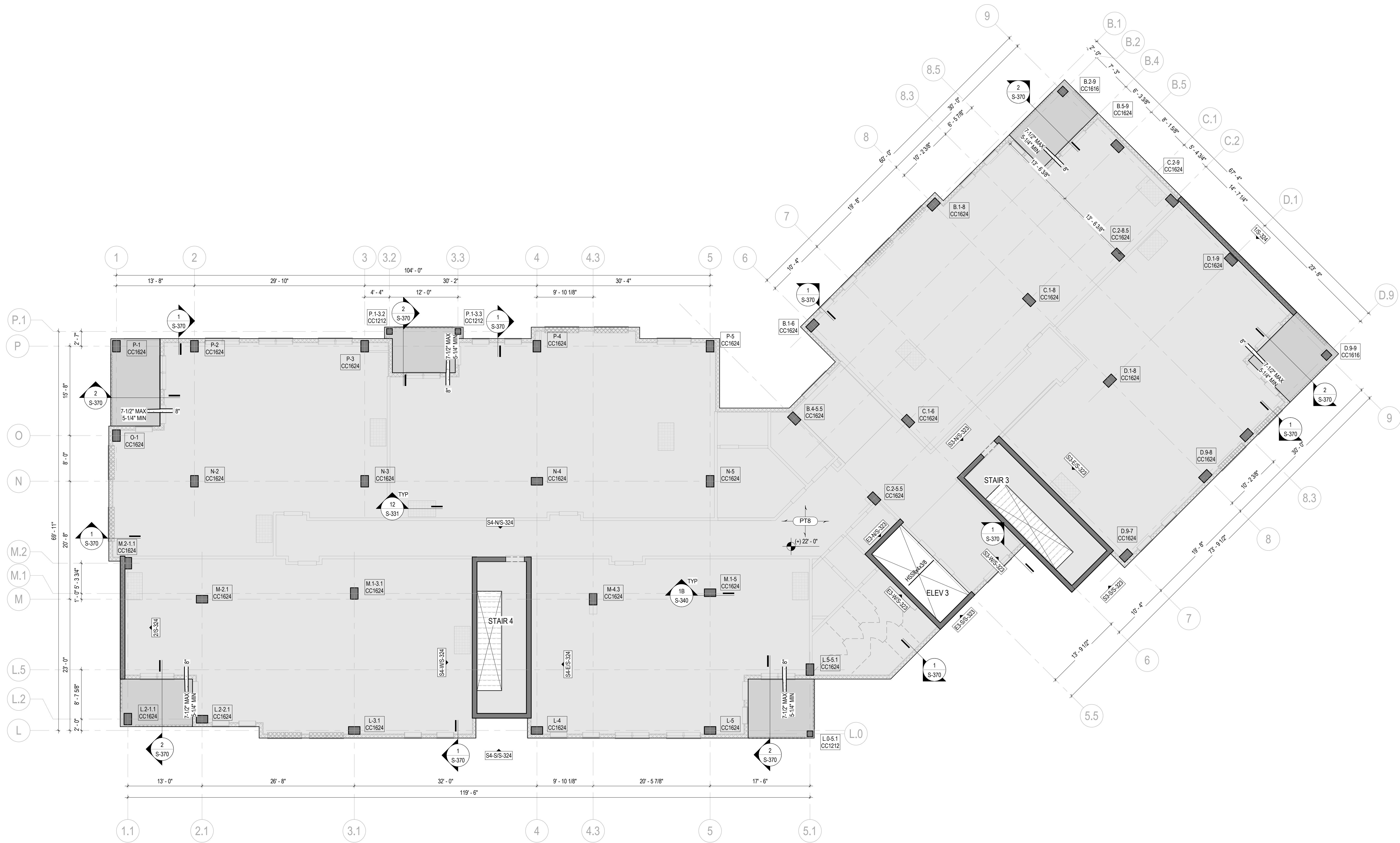
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Project No.: 2021009
Date: 12/01/2025

**PARTIAL
THIRD FLOOR
REINFORCING
PLAN AREA A**

S-143A-R

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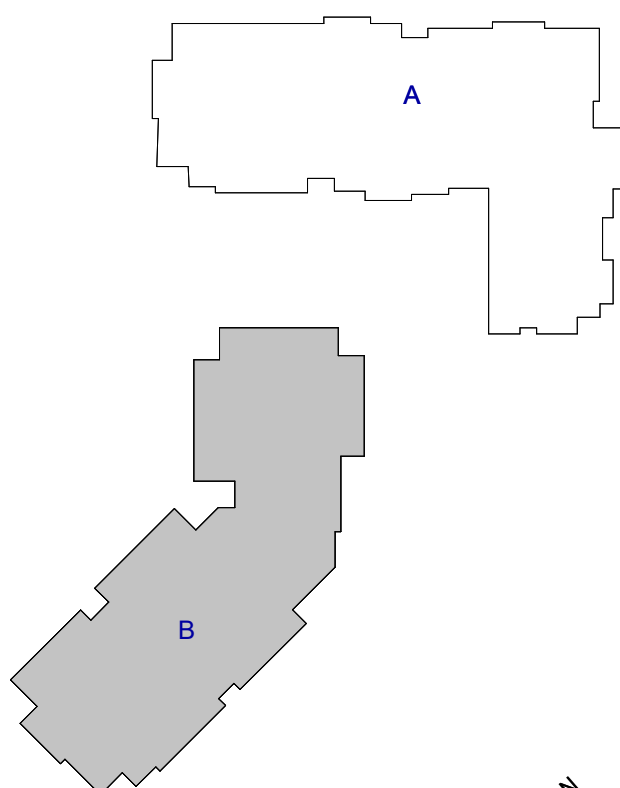
PARTIAL THIRD FLOOR FRAMING PLAN - AREA B

1/8" = 1'-0"

CONCRETE FRAMING PLAN NOTES:

- CONCRETE SLAB TAG:
PT## DENOTES SLAB MARK (SEE SCHEDULE ON THIS SHEET FOR SLAB THICKNESS AND REINFORCEMENT)
ARROWS DENOTE SLAB SPAN DIRECTION
- SEE PLAN FOR TOP OF SLAB ELEVATIONS.
- DENOTES CONCRETE COLUMN/PIER/WALL FOR COLUMN SCHEDULE AND DETAILS SEE S-340 SERIES. FOR WALL DETAILS SEE S-320 SERIES.
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- FOR POST-TENSIONED LAYOUT PLAN AND ADDITIONAL NOTES, SEE SHEET **S-144 X-PT**.
- DENOTES SHOWER DEPRESSION.

ELEVATED CONCRETE SLAB SCHEDULE				
MARK	THICKNESS	TYPICAL REINFORCEMENT		COMMENTS
		TOP	BOTTOM	
PT8	8"	SEE PLAN	#4 @ 48" OC EW	

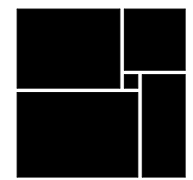
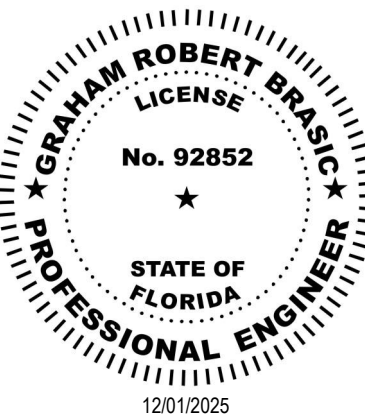


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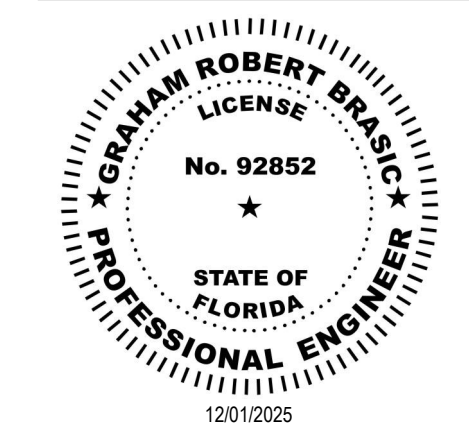
Project No.: 2021009
Date: 12/01/2025

**PARTIAL
THIRD FLOOR
FRAMING
PLAN AREA B**

S-143B

	12/01/25	CONSTRUCTION DOCUMENTS
	08/22/25	DESIGN DEVELOPMENT
NO.	DATE	DESCRIPTION

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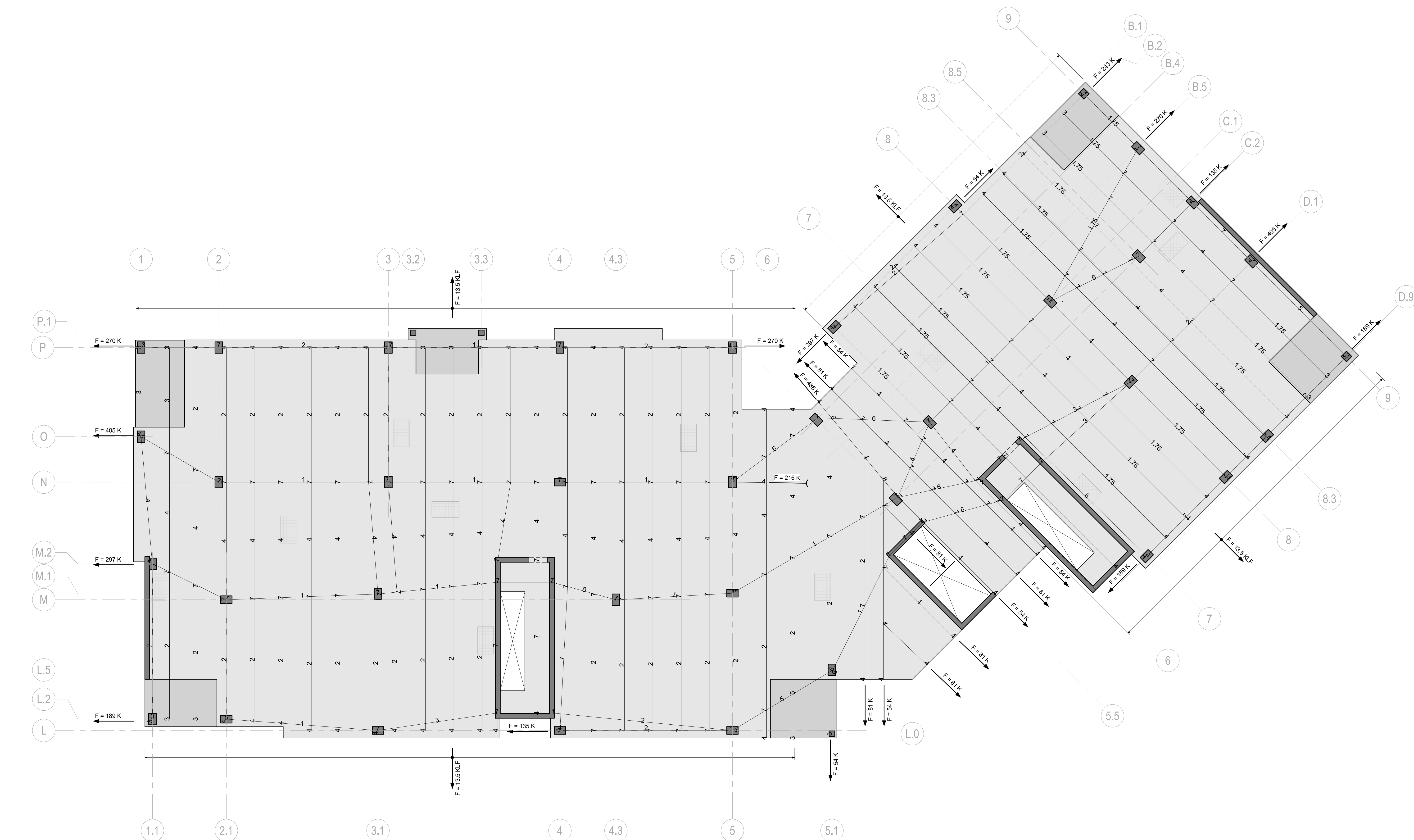
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Project No.: 2021009
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**PARTIAL
THIRD FLOOR
PT PLAN
AREA B**

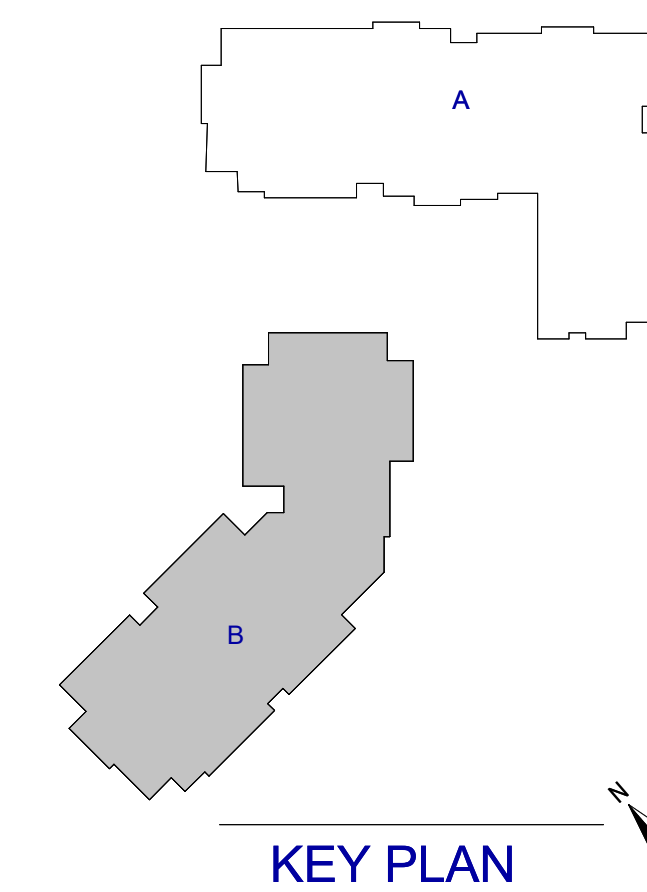
S-143B-PT



1 PARTIAL THIRD FLOOR PT PLAN - AREA B
S-143B-PT 1/8" = 1'-0"

POST-TENSIONED CONCRETE SLAB PLAN NOTES

1. SEE FRAMING PLAN FOR SLAB GEOMETRY, REFERENCE ELEVATION, AND KEYED SECTIONS.
2. SEE POST-TENSIONED FRAMING GENERAL NOTES ON SHEET S-003 AND DIVISION 03 SPECIFICATIONS FOR GENERAL REQUIREMENTS.
3. FOR TYPICAL POST-TENSIONED CONCRETE SECTIONS AND DETAILS, SEE S-330 SERIES.
4. TENDON LAYOUT NOTES:
 - LINEWORK SHOWN FOR TENDON PATHS IS SCHEMATIC TO ILLUSTRATE DESIGN INTENT, BUT NOT MEANT TO INDICATE EXACT TENDON PATHS OR SPACING OF UNIFORMLY DISTRIBUTED TENDON BUNDLES.
 - A MINIMUM OF (2) TENDONS SHALL PASS THROUGH EACH COLUMN IN EACH DIRECTION, WITH TENDONS LOCATED INSIDE OF THE COLUMN VERTICAL REINFORCEMENT CAGE.
 - STRAIGHT LINE HORIZONTAL OFFSETS SHOWN IN THE LAYOUT PLAN SHALL BE ACCOMPANIED WITH SMOOTH HORIZONTAL CURVED SWEEP PER DETAIL.
 - MAINTAIN CLEAR COVER BETWEEN EDGE-OF-SLAB (INCLUDING INTERIOR OPENINGS) AS INDICATED IN THE PROJECT SPECIFICATIONS AND TYPICAL DETAILS REFERENCED ABOVE.
5. TENDON DRAPE ELEVATION NOTES:
 - $\text{DENOTES PT TENDON DRAPE ELEVATION IN INCHES, MEASURED FROM BOTTOM OF SLAB / FRAMING TO THE CENTER-OF-GRAVITY OF THE TENDONS(STRAND BUNDLE, CGS)}$
 - AT DEAD ENDS AND STRESSING ENDS, CGS SHALL BE LOCATED AT THE CENTROID OF THE FRAMING (MID-DEPTH FOR SLAB CONDITIONS), UNLESS OTHERWISE NOTED.
 - TENDON HIGH POINTS SHALL BE LOCATED AT SUPPORTS (COLUMNS, WALLS, BEAMS, BANDED TENDON LINES) AS INDICATED ABOVE.
 - TENDON LOW POINTS SHALL BE LOCATED AT MID-SPAN BETWEEN SUPPORTS, UNLESS NOTED OTHERWISE.
6. TENDOR FORCE NOTES:
 - $F = F_K$ DENOTES EFFECTIVE PRESTRESS FORCE, IN KIPS, WITHIN BANDED TENDON GROUP OR BEAM.
 - BANDED TENDON GROUPS SHALL BE PLACED IN FLAT BUNDLED GROUPS OF NO MORE THAN (5) TENDONS PER BUNDLE, SPACED AT NO MORE THAN 12 INCHES ON CENTER, WITH TENDONS PLACED IN ONE LAYER. TOTAL QUANTITY OF BANDED TENDONS SHALL BE EQUALLY DISTRIBUTED EACH SIDE OF THE COLUMN, MINUS A MINIMUM OF (2) TENDONS PASSING THROUGH THE COLUMN REINFORCEMENT CAGE PER NOTE ABOVE.
 - $F = F_{KL}$ DENOTES EFFECTIVE PRESTRESS FORCE, IN KIPS PER LINEAR FOOT, IN UNIFORMLY DISTRIBUTED TENDON BUNDLES.
 - UNIFORMLY DISTRIBUTED TENDON BUNDLES SHALL BE PLACED IN FLAT BUNDLED GROUPS OF NO MORE THAN (5) TENDONS PER BUNDLE, SPACED AT NO MORE THAN 9" ON CENTER, WITH TENDONS PLACED IN ONE LAYER.

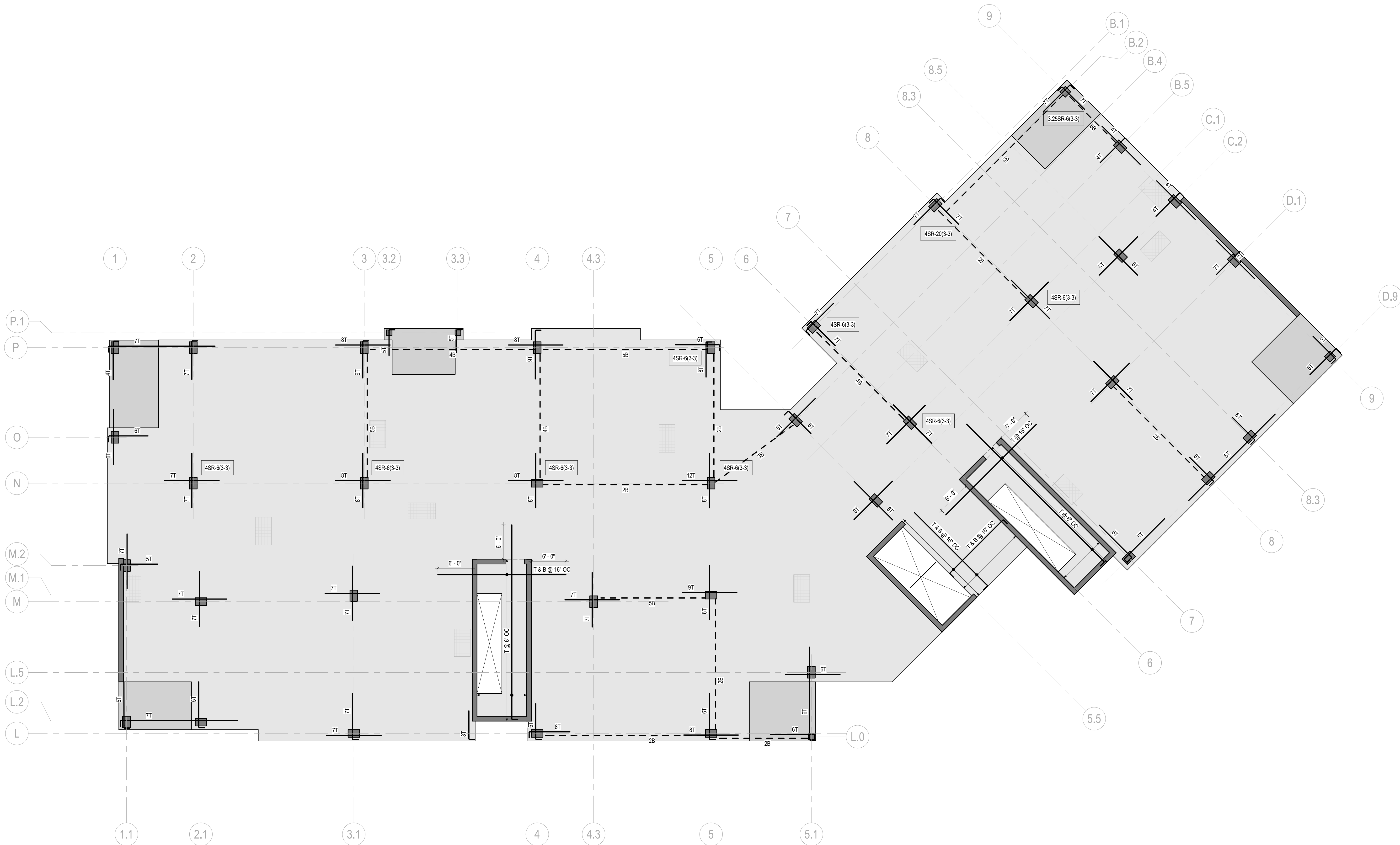


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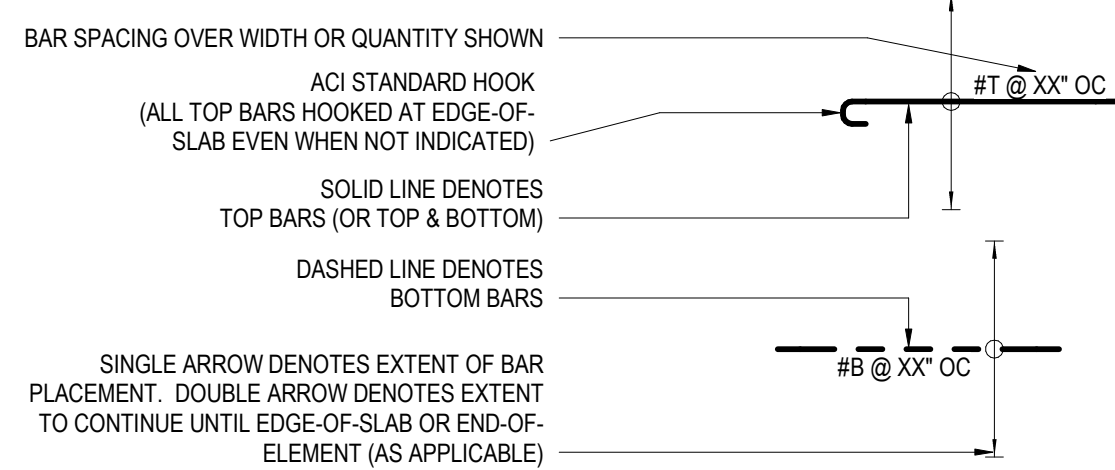
1
S-143B-R

PARTIAL THIRD FLOOR REINFORCING PLAN - AREA B

1/8" = 1'-0"

CONCRETE SLAB REINFORCEMENT PLAN NOTES:

- FOR TYPICAL SLAB REINFORCEMENT DETAILS, SEE S-330 SERIES. REINFORCING ON PLAN SHALL BE IN ADDITION TO REINFORCING IN TYPICAL SLAB REINFORCEMENT DETAILS.
- PROVIDE (4) - #5 CONTINUOUS REINFORCEMENT (2 TOP & 2 BOTTOM, SPACED AT 6" OC) AROUND PERIMETER OF SLAB AND AT ALL INTERIOR SLAB EDGES. SEE 11' S-331 FOR CONCEPTUAL LAYOUT (NOT INDICATIVE OF PROJECT SPECIFIC GEOMETRY) AND SPICE/DEVELOPMENT REQUIREMENTS.
- SLAB REINFORCEMENT SHALL BE #5 UNLESS NOTED OTHERWISE.
- #SR-#(#-#)** : DENOTES SHEAR STUD RAILS (SEE DETAILS AND SCHEDULE ON S-332).
- SLAB REINFORCEMENT SYMBOLS AND NOMENCLATURE:
 - MAT REINFORCEMENT (SEE SLAB SCHEDULE).
 - ISOLATED/DISTRIBUTED REINFORCEMENT (SEE ---).



CONCRETE SLAB REINFORCEMENT PLAN NOTES:

C. TYPICAL NOMENCLATURE:

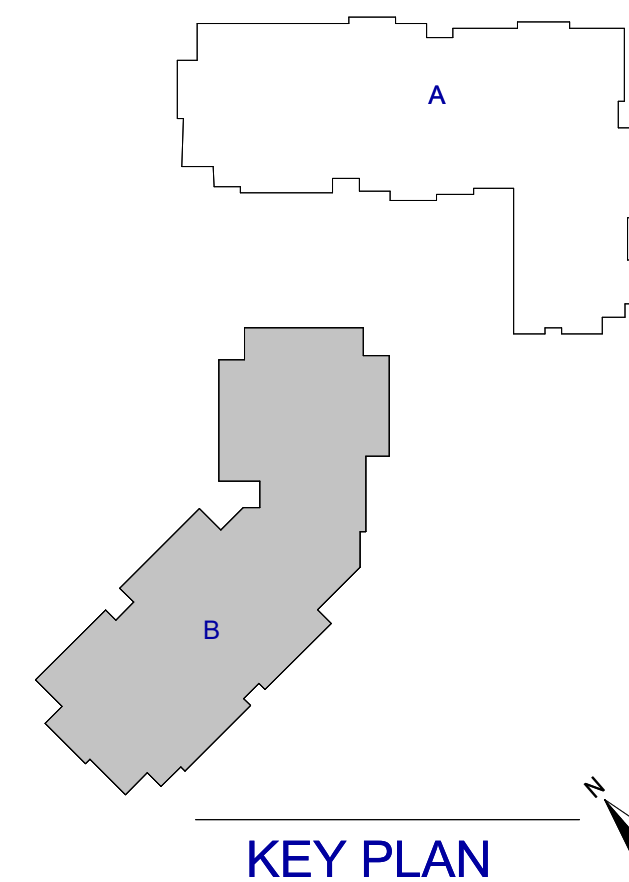
- # : DENOTES QUANTITY OF BARS
T : DENOTES TOP BARS
B : DENOTES BOTTOM BARS

D. REINFORCEMENT AT COLUMNS:

- CONCRETE COLUMN (SEE FRAMING PLAN)
BARS EACH WAY (SEE TYPICAL NOMENCLATURE)

POST-TENSIONED CONCRETE SLAB SCHEDULE			
MARK	THICKNESS	TYPICAL REINFORCEMENT	
		TOP	BOTTOM
PT8	8"	SEE PLAN	#4 @ 48" OC EW

SHEAR STUD RAIL SCHEDULE					
MARK	FIRST STUD SPACING	TYP STUD SPACING	# OF STUDS PER RAIL	NUMBER OF RAILS	
3 25SR-6(3-3)	2"	3 1/4"	6	3	3
3 25SR-6(3-4)	2"	3 1/4"	6	3	4
4SR-4(3-3)	3 1/4"	4"	4	3	3
4SR-6(3-3)	3 1/4"	4"	6	3	3
4SR-8(3-3)	3 1/4"	4"	8	3	3
4SR-14(3-3)	3 1/4"	4"	14	3	3
4SR-16(3-3)	3 1/4"	4"	16	3	3
4SR-20(3-3)	3 1/4"	4"	20	3	3
4SR-22(3-3)	3 1/4"	4"	22	3	3



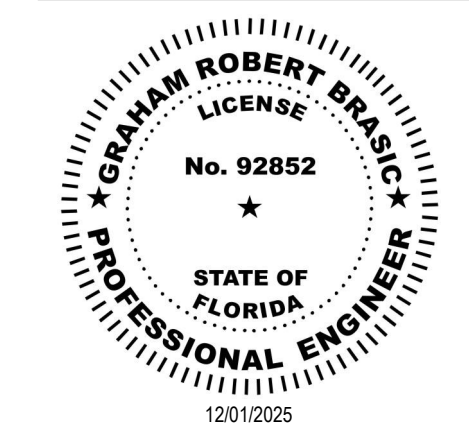
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NO.	DATE	DESCRIPTION
12/01/25	CONSTRUCTION	DOCUMENTS
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**PARTIAL
THIRD FLOOR
REINFORCING
PLAN AREA B**

S-143B-R

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S-144A

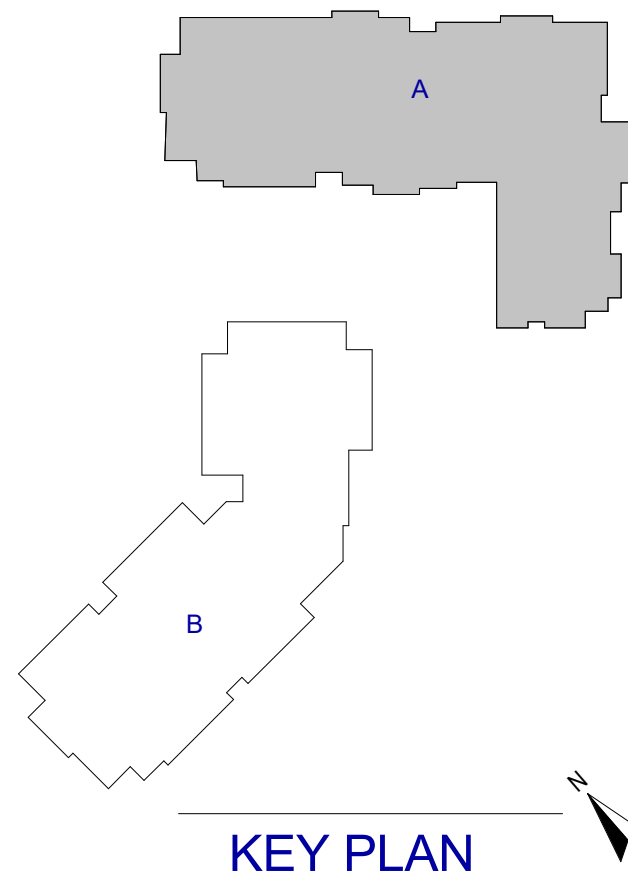
PARTIAL FOURTH FLOOR FRAMING PLAN - AREA A

1/8" = 1'-0"

CONCRETE FRAMING PLAN NOTES:

- CONCRETE SLAB TAG:
PT## DENOTES SLAB MARK (SEE SCHEDULE ON THIS SHEET FOR SLAB THICKNESS AND REINFORCEMENT)
ARROWS DENOTE SLAB SPAN DIRECTION
SEE PLAN FOR TOP OF SLAB ELEVATIONS.
- SEE PLAN FOR TOP OF SLAB ELEVATIONS.
DENOTES CONCRETE COLUMN/PIER/WALL FOR COLUMN SCHEDULE AND DETAILS SEE S-340 SERIES. FOR WALL DETAILS SEE S-320 SERIES.
- SEE ARCHITECTURAL DRAWINGS FOR:
ALL SLOPED SLAB AREAS
ALL DIMENSIONS NOT SHOWN. VERIFY ALL DIMENSIONS SHOWN IN THE STRUCTURAL DRAWINGS WITH ARCHITECTURAL DRAWINGS AND REPORT ANY DISCREPANCIES OR DIMENSIONS NOT SHOWN ON THE ARCHITECTURAL DRAWINGS FOR CLARIFICATION.
- SEE MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR ADDITIONAL FLOOR PENETRATIONS, SLEEVES, AND INSERTS REQUIRED TO BE CAST IN THE SLAB.
SLEEVES AND PENETRATIONS WITHIN 48 INCHES OF THE FACE OF ANY CONCRETE COLUMN (NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS) SHALL BE SUBMITTED FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD.
SLEEVES AND PENETRATIONS INTERRUPTING BANDED LINES OF TENDONS (NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS) SHALL BE SUBMITTED FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD.
SLEEVES AND PENETRATIONS GREATER THAN 12 INCHES IN LENGTH OR WIDTH (NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS) SHALL BE SUBMITTED FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD.
- FOR REINFORCEMENT PLAN AND ADDITIONAL NOTES, SEE SHEET **S-144X-R**.
- FOR POST-TENSIONED LAYOUT PLAN AND ADDITIONAL NOTES, SEE SHEET **S-144X-PT**.
- DENOTES SHOWER DEPRESSION.

ELEVATED CONCRETE SLAB SCHEDULE				
MARK	THICKNESS	TYPICAL REINFORCEMENT		COMMENTS
		TOP	BOTTOM	
PT8	8"	SEE PLAN	#4 @ 48" OC EW	



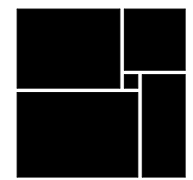
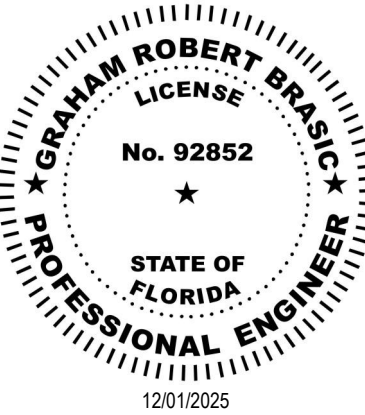
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Date: 12/01/2025

PARTIAL
FOURTH
FLOOR
FRAMING
PLAN AREA A

S-144A


$$1/8" = 1'-0"$$

POST-TENSIONED CONCRETE SLAB PLAN NOTES:

- POST-TENSIONED CONCRETE SLAB PLAN NOTES:

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S-144A-PT

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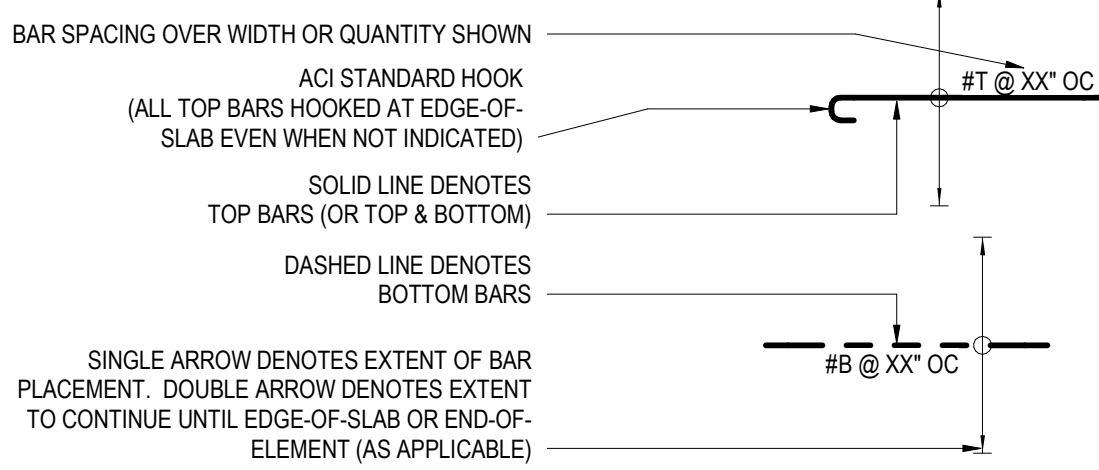
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S-144A-R

PARTIAL FOURTH FLOOR REINFORCING PLAN - AREA A

1/8" = 1'-0"

CONCRETE SLAB REINFORCEMENT PLAN NOTES:

- FOR TYPICAL SLAB REINFORCEMENT DETAILS, SEE S-330 SERIES. REINFORCING ON PLAN SHALL BE IN ADDITION TO REINFORCING IN TYPICAL SLAB REINFORCEMENT DETAILS.
- PROVIDE (4) - #5 CONTINUOUS REINFORCEMENT (2 TOP & 2 BOTTOM, SPACED AT 6" OC) AROUND PERIMETER OF SLAB AND AT ALL INTERIOR SLAB EDGES. SEE 11/ S-331 FOR CONCEPTUAL LAYOUT (NOT INDICATIVE OF PROJECT SPECIFIC GEOMETRY) AND SPICE DEVELOPMENT REQUIREMENTS.
- SLAB REINFORCEMENT SHALL BE #5 UNLESS NOTED OTHERWISE.
- [##SR-#(##)] DENOTES SHEAR STUD RAILS (SEE DETAILS AND SCHEDULE ON S-332).
- SLAB REINFORCEMENT SYMBOLS AND NOMENCLATURE:
 - MAT REINFORCEMENT (SEE SLAB SCHEDULE).
 - ISOLATED/DISTRIBUTED REINFORCEMENT (SEE ---).



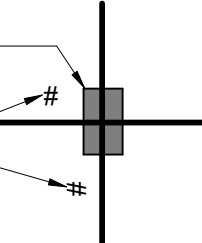
CONCRETE SLAB REINFORCEMENT PLAN NOTES:

C. TYPICAL NOMENCLATURE:

DENOTES QUANTITY OF BARS
T: DENOTES TOP BARS
B: DENOTES BOTTOM BARS

D. REINFORCEMENT AT COLUMNS:

CONCRETE COLUMN
(SEE FRAMING PLAN)
BARS EACH WAY
(SEE TYPICAL NOMENCLATURE)

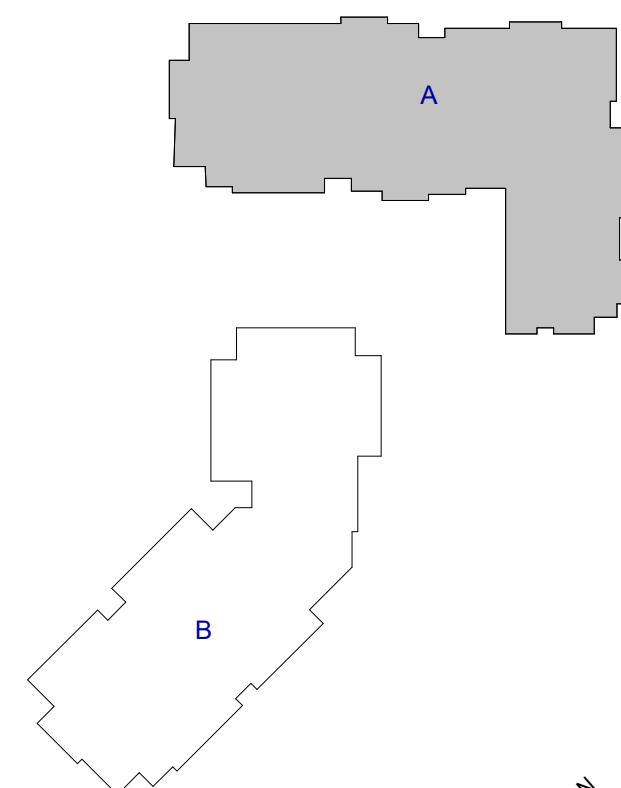


POST-TENSIONED CONCRETE SLAB SCHEDULE

MARK	THICKNESS	TYPICAL REINFORCEMENT	
		TOP	BOTTOM
PT8	8"	SEE PLAN	#4 @ 48" OC EW

SHEAR STUD RAIL SCHEDULE

MARK	FIRST STUD SPACING	TYP STUD SPACING	# OF STUDS PER RAIL	NUMBER OF RAILS	
				SHORT SIDE	LONG SIDE
3.25SR-6(3-3)	2"	3 1/4"	6	3	3
3.25SR-6(3-4)	2"	3 1/4"	6	3	4
4SR-4(3-3)	3 1/4"	4"	4	3	3
4SR-6(3-3)	3 1/4"	4"	6	3	3
4SR-8(3-3)	3 1/4"	4"	8	3	3
4SR-14(3-3)	3 1/4"	4"	14	3	3
4SR-16(3-3)	3 1/4"	4"	16	3	3
4SR-20(3-3)	3 1/4"	4"	20	3	3
4SR-22(3-3)	3 1/4"	4"	22	3	3



KEY PLAN

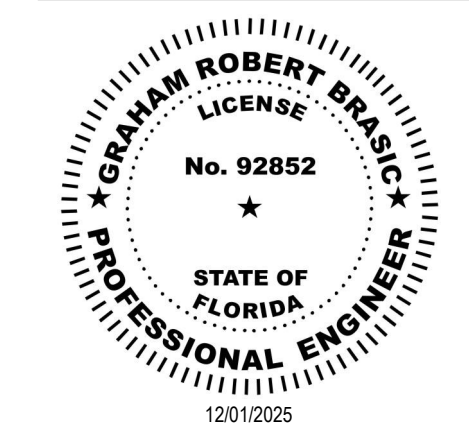
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	DESIGN	
	DEVELOPMENT	
NO.	DATE	DESCRIPTION

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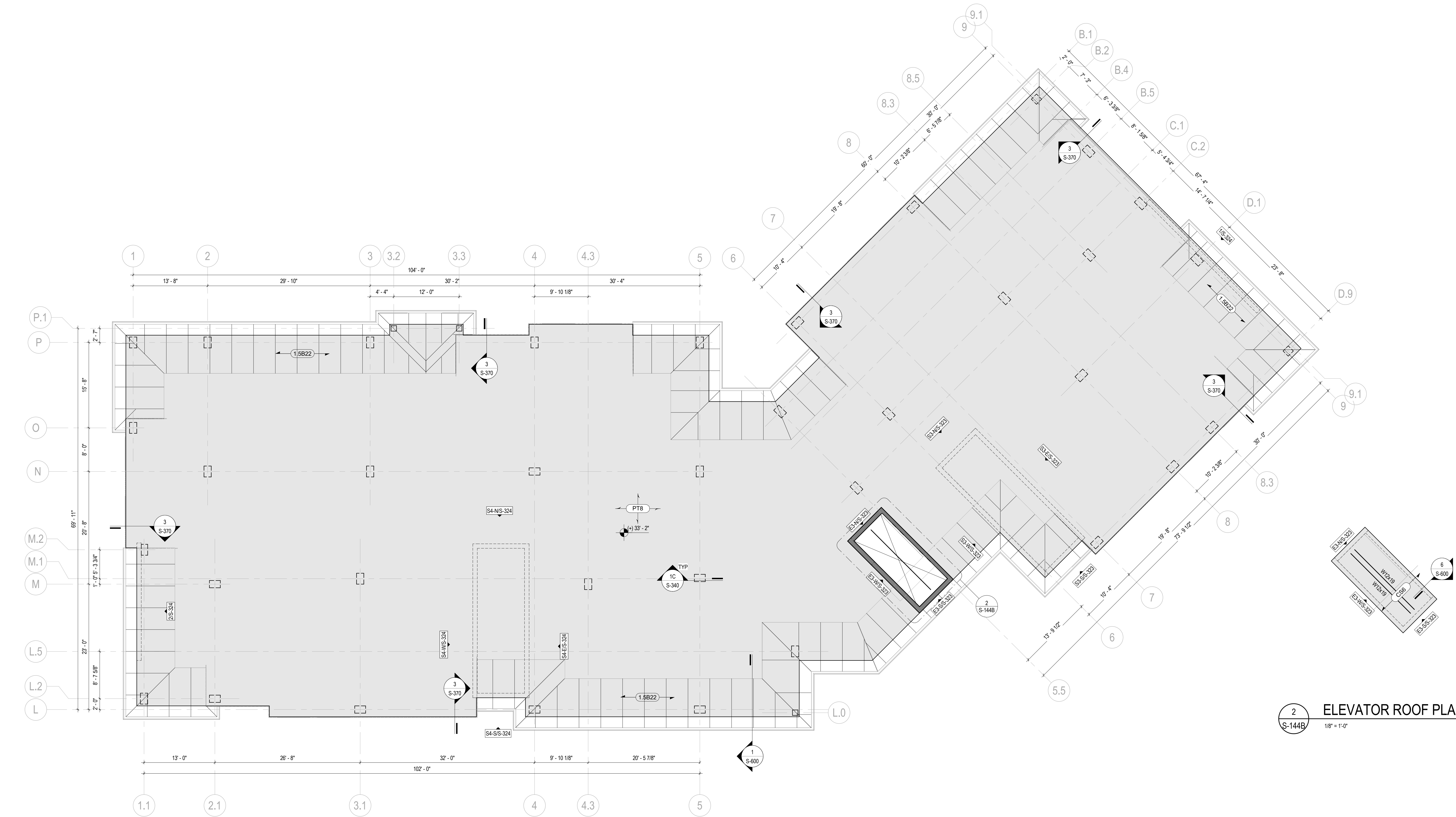
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**PARTIAL
FOURTH
FLOOR
REINFORCING
PLAN AREA A**

S-144A-R

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S-144B
PARTIAL ROOF FRAMING PLAN - AREA B
1/8" = 1'-0"

CONCRETE FRAMING PLAN NOTES:

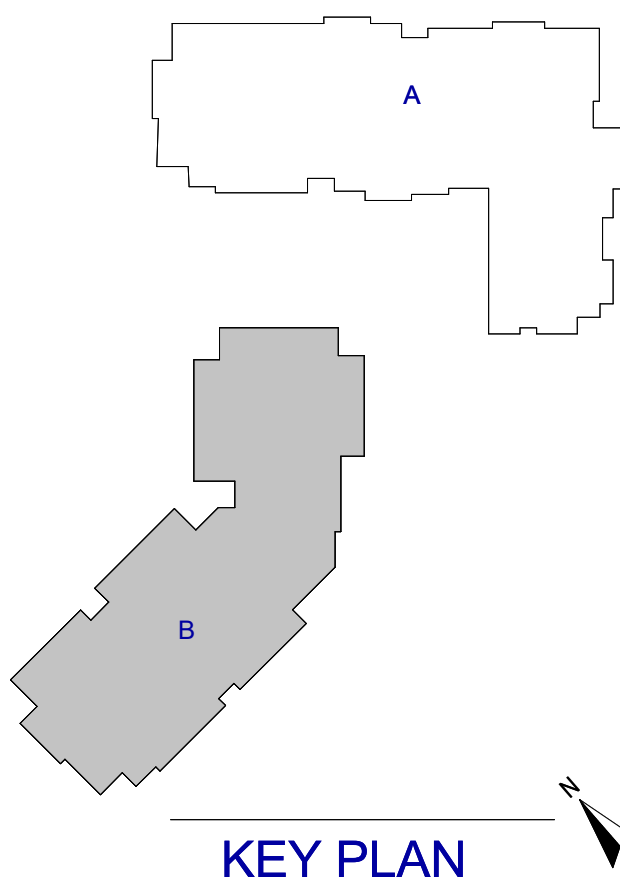
- CONCRETE SLAB TAG:
PT## DENOTES CONCRETE COLUMN/PIERWALL. FOR WALL DETAILS SEE S-320 SERIES.
ARROWS DENOTE SLAB SPAN DIRECTION.
PT## DENOTES SLAB MARK (SEE SCHEDULE ON THIS SHEET FOR DECK THICKNESS AND REINFORCEMENT).
- SEE PLAN FOR TOP OF SLAB ELEVATIONS.
- DENOTES CONCRETE COLUMN/PIERWALL. FOR COLUMN SCHEDULE AND DETAILS SEE S-340 SERIES. FOR WALL DETAILS SEE S-320 SERIES.
- SEE ARCHITECTURAL DRAWINGS FOR:
ALL SLOPED SLAB AREAS.
ALL DIMENSIONS NOT SHOWN. VERIFY ALL DIMENSIONS SHOWN IN THE STRUCTURAL DRAWINGS WITH ARCHITECTURAL DRAWINGS AND REPORT ANY DISCREPANCIES OR DIMENSIONS NOT SHOWN ON THE ARCHITECTURAL DRAWINGS FOR CLARIFICATION.
- SEE MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR ADDITIONAL FLOOR PENETRATIONS, SLEEVES, AND INSERTS REQUIRED TO BE CAST IN THE SLAB.
SLEEVES AND PENETRATIONS WITHIN 48 INCHES OF THE FACE OF ANY CONCRETE COLUMN (NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS) SHALL BE SUBMITTED FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD.
SLEEVES AND PENETRATIONS INTERRUPTING BANDED LINES OF TENDONS (NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS) SHALL BE SUBMITTED FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD.
SLEEVES AND PENETRATIONS GREATER THAN 12 INCHES IN LENGTH OR WIDTH (NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS) SHALL BE SUBMITTED FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD.
- FOR REINFORCEMENT PLAN AND ADDITIONAL NOTES, SEE SHEET **S-144X-R**.
- FOR POST-TENSIONED LAYOUT PLAN AND ADDITIONAL NOTES, SEE SHEET **S-144X-PT**.
- DENOTES SHOWER DEPRESSION.

METAL ROOF TRUSS PLAN NOTES:

- ROOF DECK TAG:
ARROWS DENOTE DECK SPAN DIRECTION.
DENOTES DECK MARK (SEE SCHEDULE ON THIS SHEET FOR DECK PROPERTIES AND ATTACHMENT PATTERNS).
- SEE ARCHITECTURAL DRAWINGS FOR TRUSS BEARING ELEVATION.
- SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL DIMENSIONS NOT SHOWN AND ALL SLAB ELEVATIONS.
- ROOF TRUSSES SHALL BE SUPPORTED FRONT AND BACK BY GALVANIZED DOWNTURNED CHANNEL SPANNING BETWEEN AND WELDED TO GALVANIZED EMBEDS IN TOP OF SLAB. SEE S-600.
- TRUSS SYSTEM IS OVERBUILT AND THUS DOES NOT HAVE LATERAL ELEMENTS FOR DIAPHRAGM BRACING. TRUSS CONNECTIONS SHALL BE DESIGNED FOR CONCURRENT VERTICAL AND HORIZONTAL LOADS TO TRANSFER REACTIONS TO DOWNTURNED CHANNELS BELOW.
- TG: DENOTES PRE-ENGINEERED METAL ROOF TRUSS GIRDER.
- HG: DENOTES PRE-ENGINEERED METAL ROOF HIP GIRDER.

ELEVATED CONCRETE SLAB SCHEDULE				
MARK	THICKNESS	TYPICAL REINFORCEMENT		COMMENTS
		TOP	BOTTOM	
PT8	8"	SEE PLAN	#4 @ 16" OC EW	

2
S-144B
ELEVATOR ROOF PLAN - AREA B
1/8" = 1'-0"

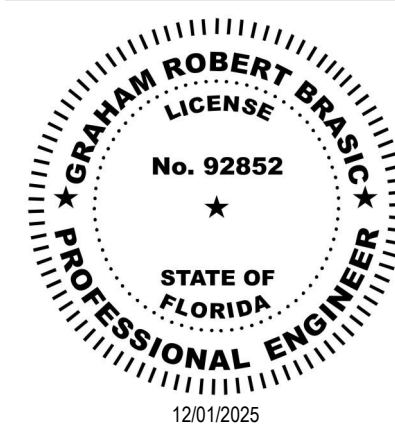


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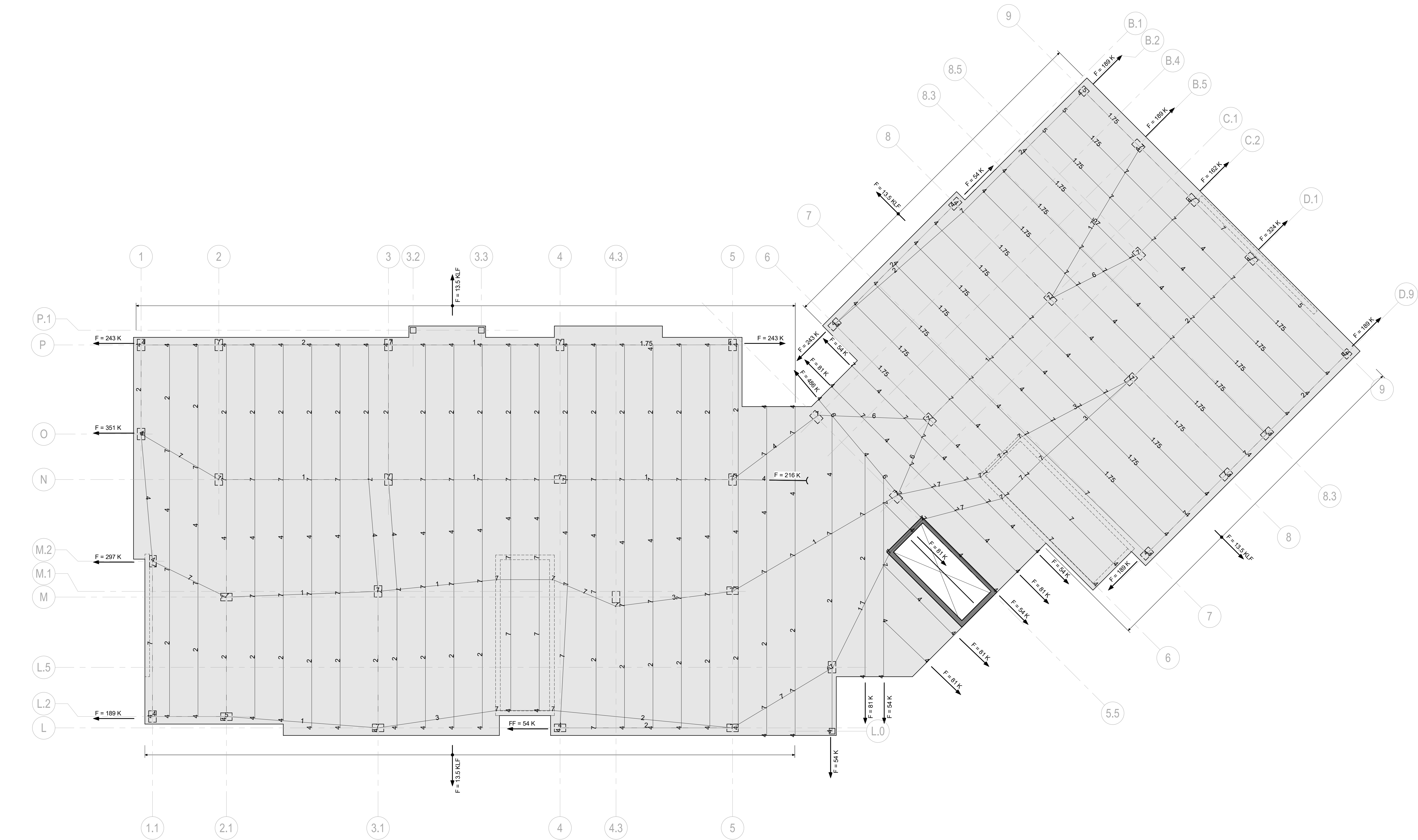
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Date: 12/01/2025

**PARTIAL
ROOF
FRAMING
PLAN AREA B**

S-144B

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1
S-144B-PT PARTIAL ROOF PT PLAN - AREA B
1/8" = 1'-0"

POST-TENSIONED CONCRETE SLAB PLAN NOTES:

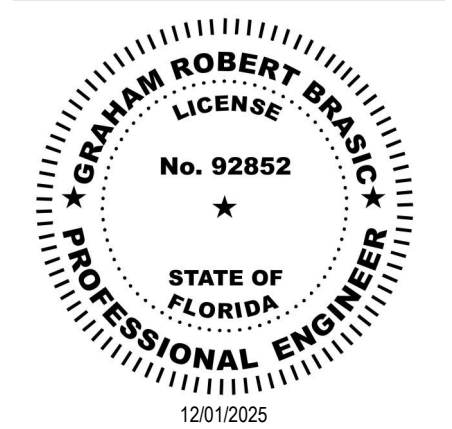
- SEE FRAMING PLAN FOR SLAB GEOMETRY, REFERENCE ELEVATION, AND KEYED SECTIONS.
- SEE POST-TENSIONED FRAMING GENERAL NOTES ON SHEET S-003 AND DIVISION 03 SPECIFICATIONS FOR GENERAL REQUIREMENTS.
- FOR TYPICAL POST-TENSIONED CONCRETE SECTIONS AND DETAILS, SEE S-330 SERIES.
- TENDON LAYOUT NOTES:
 - LINEWORK SHOWN FOR TENDON PATHS IS SCHEMATIC TO ILLUSTRATE DESIGN INTENT, BUT NOT MEANT TO INDICATE EXACT TENDON PATHS OR SPACING OF UNIFORMLY DISTRIBUTED TENDON BUNDLES.
 - A MINIMUM OF (2) TENDONS SHALL PASS THROUGH EACH COLUMN IN EACH DIRECTION, WITH TENDONS LOCATED INSIDE OF THE COLUMN VERTICAL REINFORCEMENT CAGE.
 - STRAIGHT LINE HORIZONTAL OFFSETS SHOWN IN THE LAYOUT PLAN SHALL BE ACCOMPLISHED WITH SMOOTH HORIZONTAL CURVESWEEP PER DETAIL.
 - MAINTAIN CLEAR COVERS BETWEEN EDGE OF SLAB (INCLUDING INTERIOR OPENINGS) AS INDICATED IN THE PROJECT SPECIFICATIONS AND TYPICAL DETAILS REFERENCED ABOVE.
- TENDON DRAPE ELEVATION NOTES:
 - # DENOTES PT TENDON DRAPE ELEVATION IN INCHES, MEASURED FROM BOTTOM OF SLAB / FRAMING TO THE CENTER-OF-GRAVITY OF THE TENDONSTRAND BUNDLE (CGS).
 - AT DEAD ENDS AND STRESSING ENDS, CGS SHALL BE LOCATED AT THE CENTROID OF THE FRAMING (MID-DEPTH FOR SLAB CONDITIONS), UNLESS OTHERWISE NOTED.
 - TENDON HIGH POINTS SHALL BE LOCATED AT SUPPORTS (COLUMNS, WALLS, BEAMS, Banded TENDON LINES) AS INDICATED ABOVE.
 - TENDON LOW POINTS SHALL BE LOCATED AT MID-SPAN BETWEEN SUPPORTS, UNLESS NOTED OTHERWISE.
- TENDON FORCE NOTES:
 - F = # K DENOTES EFFECTIVE PRESTRESS FORCE, IN KIPS, WITHIN Banded TENDON GROUP OR BEAM.
 - Banded TENDON GROUPS SHALL BE PLACED IN FLAT BUNDLED GROUPS OF NO MORE THAN (5) TENDONS PER BUNDLE, SPACED AT NO MORE THAN 12 INCHES ON CENTER, WITH TENDONS FLAT IN ONE LAYER. TOTAL QUANTITY OF Banded TENDONS SHALL BE EQUALLY DISTRIBUTED EACH SIDE OF THE COLUMN, MINUS A MINIMUM OF (2) TENDONS PASSING THROUGH THE COLUMN REINFORCEMENT CAGE PER NOTE ABOVE.
 - F = # KLF DENOTES EFFECTIVE PRESTRESS FORCE, IN KIPS PER LINEAR FOOT, IN UNIFORMLY DISTRIBUTED TENDONS.
 - UNIFORMLY DISTRIBUTED TENDON BUNDLES SHALL BE PLACED IN FLAT BUNDLED GROUPS OF NO MORE THAN (5) TENDONS PER BUNDLE, SPACED AT NO MORE THAN 5'-0" ON CENTER, WITH TENDONS FLAT IN ONE LAYER.



TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

NO.	DATE	DESCRIPTION
12/01/25	CONSTRUCTION	DOCUMENTS
08/22/25	DESIGN	DEVELOPMENT

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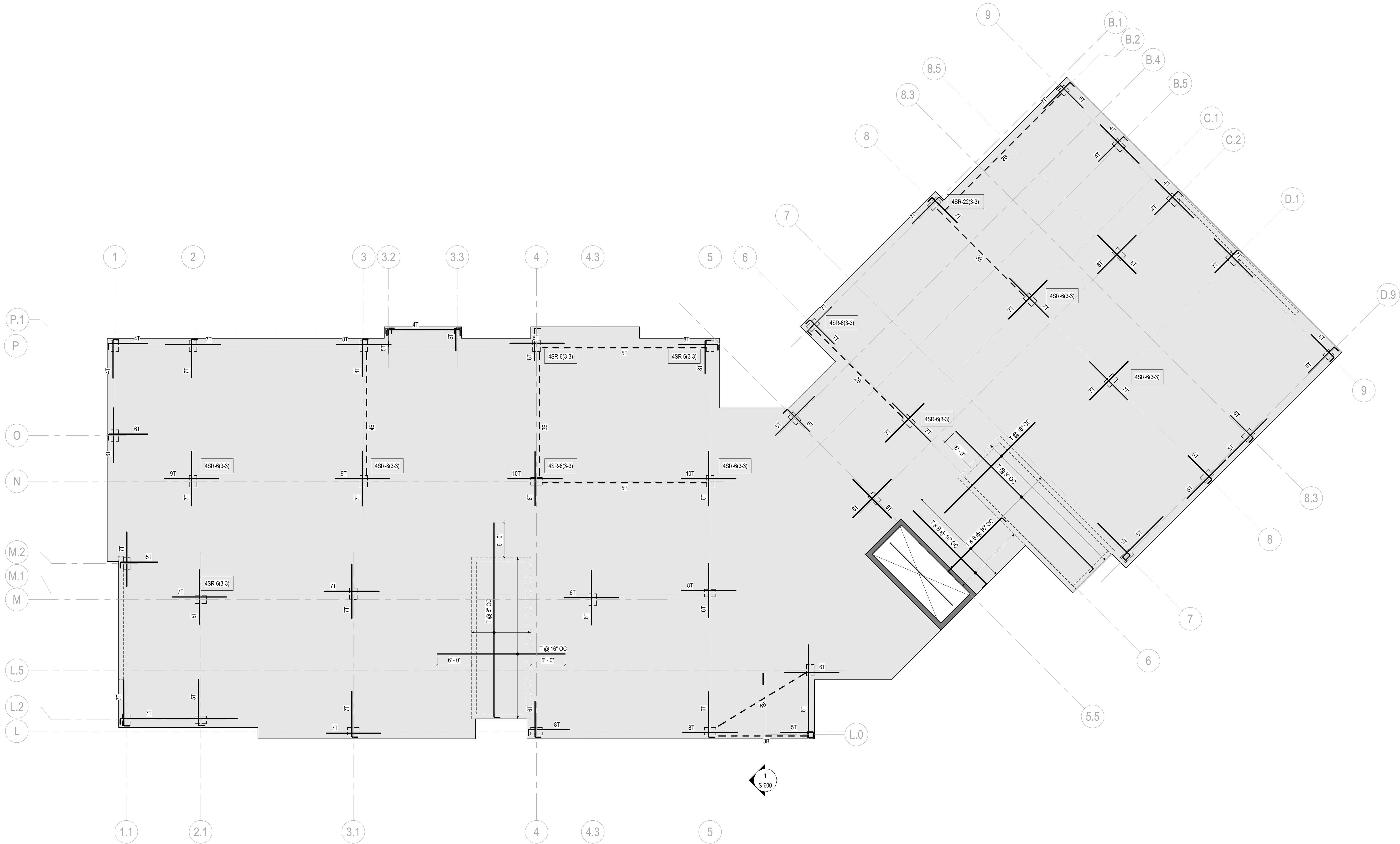
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Project No.: 2021009
Date: 12/01/2025

PARTIAL
ROOF PT
PLAN AREA B

S-144B-PT

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1 S-144B-R PARTIAL ROOF REINFORCING PLAN - AREA B

1/8" = 1'-0"

CONCRETE SLAB REINFORCEMENT PLAN NOTES:

- FOR TYPICAL SLAB REINFORCEMENT DETAILS, SEE S-330 SERIES. REINFORCING ON PLAN SHALL BE IN ADDITION TO REINFORCING IN TYPICAL SLAB REINFORCEMENT DETAILS.
- PROVIDE (4) - #5 CONTINUOUS REINFORCEMENT (2 TOP & 2 BOTTOM, SPACED AT 6" OC) AROUND PERIMETER OF SLAB AND AT ALL INTERIOR SLAB EDGES. SEE 11/ S-331 FOR CONCEPTUAL LAYOUT (NOT INDICATIVE OF PROJECT SPECIFIC GEOMETRY) AND SPlicing/DEVELOPMENT REQUIREMENTS.
- SLAB REINFORCEMENT SHALL BE #5 UNLESS NOTED OTHERWISE.
- ##SR-#(##) - # DENOTES SHEAR STUD RAILS (SEE DETAILS AND SCHEDULE ON S-332).
- SLAB REINFORCEMENT SYMBOLS AND NOMENCLATURE:
 - A. MAT REINFORCEMENT (SEE SLAB SCHEDULE).
 - B. ISOLATED/DISTRIBUTED REINFORCEMENT (SEE ---).

BAR SPACING OVER WIDTH OR QUANTITY SHOWN
ACI STANDARD HOOK
(ALL TOP BARS HOOKED AT EDGE-OF-
SLAB EVEN WHEN NOT INDICATED)
SOLID LINE DENOTES
TOP BARS (OR TOP & BOTTOM)
DASHED LINE DENOTES
BOTTOM BARS
SINGLE ARROW DENOTES EXTENT OF BAR
PLACEMENT. DOUBLE ARROW DENOTES EXTENT
TO CONTINUE UNTIL EDGE-OF-SLAB OR END-OF-
ELEMENT (AS APPLICABLE)

CONCRETE SLAB REINFORCEMENT PLAN NOTES:

C. TYPICAL NOMENCLATURE

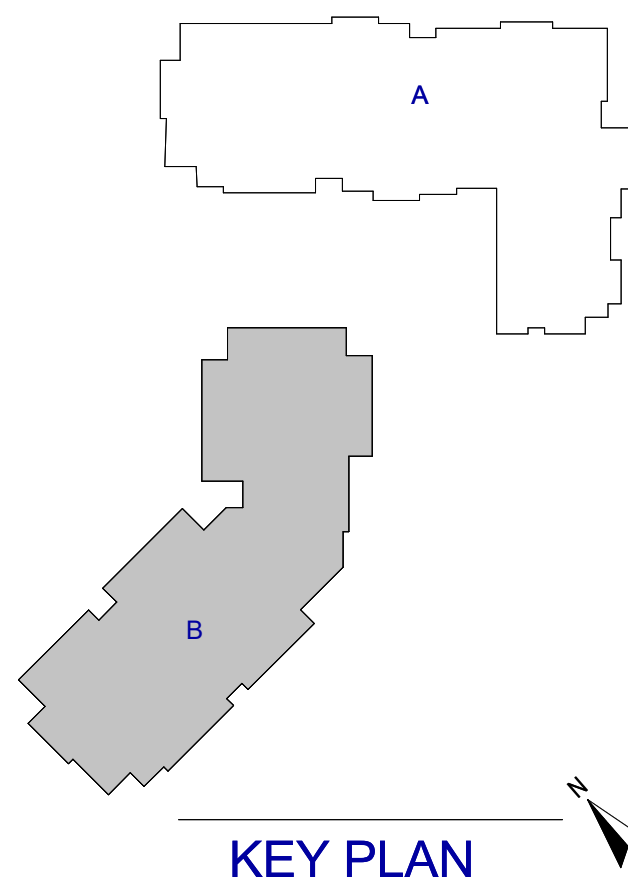
DENOTES QUANTITY OF BARS
T: DENOTES TOP BARS
B: DENOTES BOTTOM BARS

D. REINFORCEMENT AT COLUMNS

CONCRETE COLUMN
(SEE FRAMING PLAN)
BARS EACH WAY
(SEE TYPICAL NOMENCLATURE)

POST-TENSIONED CONCRETE SLAB SCHEDULE			
MARK	THICKNESS	TYPICAL REINFORCEMENT	
		TOP	BOTTOM
PT8	8"	SEE PLAN	#4 @ 48" OC EW

SHEAR STUD RAIL SCHEDULE					
MARK	FIRST STUD SPACING	TYP STUD SPACING	# OF STUDS PER RAIL	NUMBER OF RAILS	
				SHORT SIDE	LONG SIDE
3.25SR-6(3-3)	2"	3 1/4"	6	3	3
3.25SR-6(3-4)	2"	3 1/4"	6	3	4
4SR-4(3-3)	3 1/4"	4"	4	3	3
4SR-6(3-3)	3 1/4"	4"	6	3	3
4SR-8(3-3)	3 1/4"	4"	8	3	3
4SR-14(3-3)	3 1/4"	4"	14	3	3
4SR-16(3-3)	3 1/4"	4"	16	3	3
4SR-20(3-3)	3 1/4"	4"	20	3	3
4SR-22(3-3)	3 1/4"	4"	22	3	3



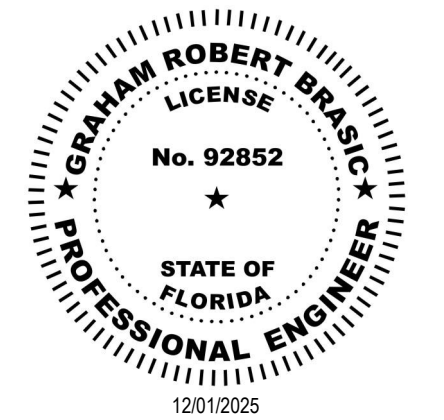
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JG Project #: 21.18.004

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NO.	DATE	DESCRIPTION
12/01/25	CONSTRUCTION	DOCUMENTS
08/22/25	DESIGN	DEVELOPMENT

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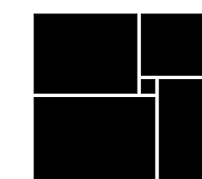
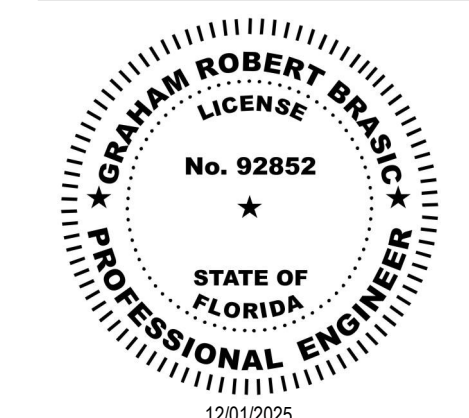
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Date: 12/01/2025

**PARTIAL
ROOF
REINFORCING
PLAN AREA B**

S-144B-R

	12/01/25	CONSTRUCTION DOCUMENTS
	08/22/25	DESIGN DEVELOPMENT
	04/07/25	SCHEMATIC DESIGN
NO.	DATE	DESCRIPTION

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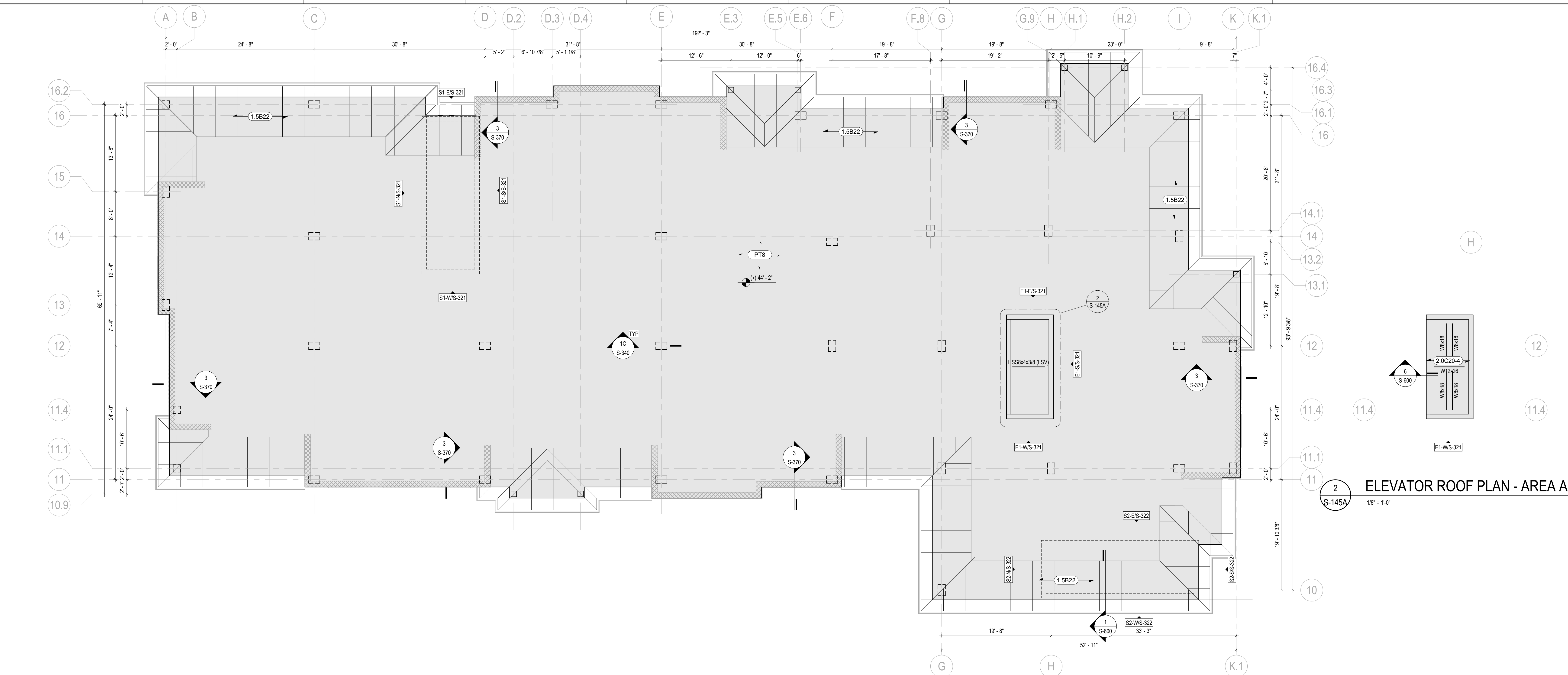
Project No.: 2021009
Date: 12/01/2025

**PARTIAL
ROOF
FRAMING
PLAN AREA A**

S-145A

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

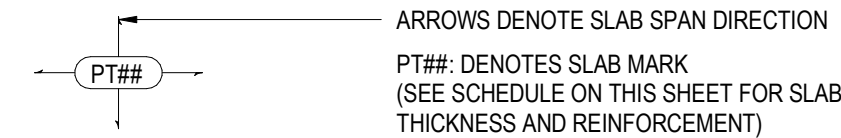
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1 PARTIAL ROOF FRAMING PLAN - AREA A
S-145A 1/8" = 1'-0"

CONCRETE FRAMING PLAN NOTES:

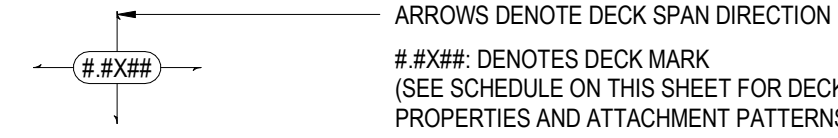
1. CONCRETE SLAB TAG:



2. SEE PLAN FOR TOP OF SLAB ELEVATIONS.
3.  DENOTES CONCRETE COLUMN/PERWALL FOR COLUMN SCHEDULE AND DETAILS SEE S-340 SERIES.
FOR WALL DETAILS SEE S-330 SERIES.
4. SEE ARCHITECTURAL DRAWINGS FOR:
 - ALL SLOPED SLAB AREAS.
 - ALL DIMENSIONS NOT SHOWN. VERIFY ALL DIMENSIONS SHOWN IN THE STRUCTURAL DRAWINGS WITH ARCHITECTURAL DRAWINGS. VERIFY ALL DISCREPANCIES OR DIMENSIONS NOT SHOWN ON THE ARCHITECTURAL DRAWINGS FOR CLARIFICATION.
5. SEE MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR ADDITIONAL FLOOR PENETRATIONS, SLEEVES, AND INSERTS REQUIRED TO BE CAST IN THE SLAB.
SLEEVES AND PENETRATIONS WITHIN 48 INCHES OF THE FACE OF ANY CONCRETE COLUMN NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD.
SLEEVES AND PENETRATIONS INTERRUPTING BANDED LINES OF TENDONS (NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS) SHALL BE SUBMITTED FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD.
SLEEVES AND PENETRATIONS GREATER THAN 12 INCHES IN LENGTH OR WIDTH (NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS) SHALL BE SUBMITTED FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD.
6. FOR REINFORCEMENT PLAN AND ADDITIONAL NOTES, SEE SHEET S-14X-X.R.
7. FOR POST-TENSIONED LAYOUT PLAN AND ADDITIONAL NOTES, SEE SHEET S-14X-X.P.T.
8.  DENOTES SHOWER DEPRESSION.

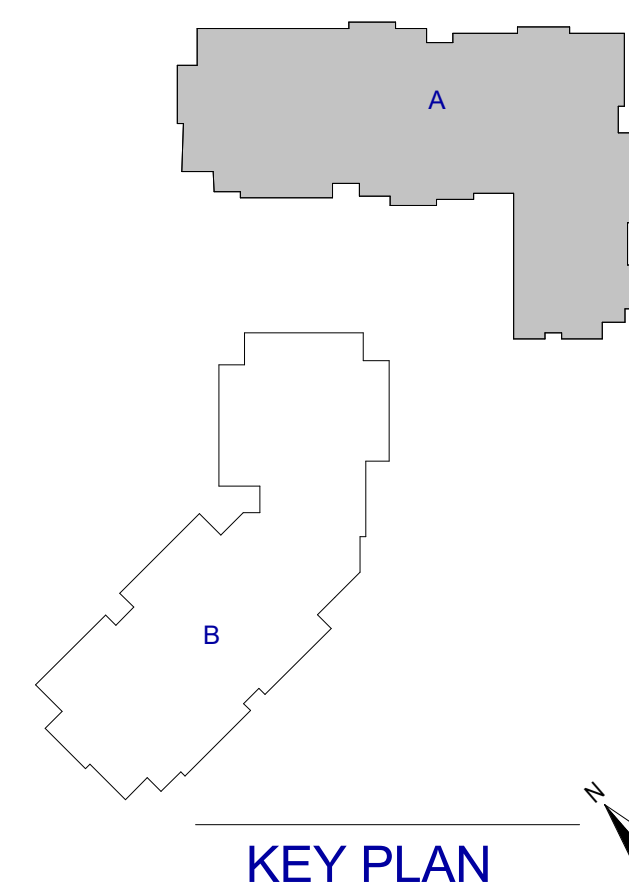
METAL ROOF TRUSS PLAN NOTES

1. ROOF DECK TAG:



2. SEE ARCHITECTURAL DRAWINGS FOR TRUSS BEARING ELEVATION.
3. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL DIMENSIONS NOT SHOWN AND ALL SLAB ELEVATIONS.
4. ROOF TRUSSES SHALL BE SUPPORTED FRONT AND BACK BY GALVANIZED DOWNTURNED CHANNEL SPANNING BETWEEN AND WELDED TO GALVANIZED EMBEDS IN TOP OF SLAB. SEE S-600.
5. TRUSS SYSTEM IS OVERBUILT AND THIS DOES NOT HAVE LATERAL ELEMENTS FOR DIAPHRAGM BRACING. TRUSS CONNECTIONS SHALL BE DESIGNED FOR CONCURRENT VERTICAL AND HORIZONTAL LOADS TO TRANSFER REACTIONS TO DOWNTURNED CHANNELS BELOW.
6. TG: DENOTES PRE-ENGINEERED METAL ROOF TRUSS GIRDER.
7. HG: DENOTES PRE-ENGINEERED METAL ROOF HIP GIRDER.

<u>ELEVATED CONCRETE SLAB SCHEDULE</u>				
MARK	THICKNESS	TYPICAL REINFORCEMENT		COMMENTS
		TOP	BOTTOM	
PT8	8"	SEE PLAN	#4 @ 48" OC EW	



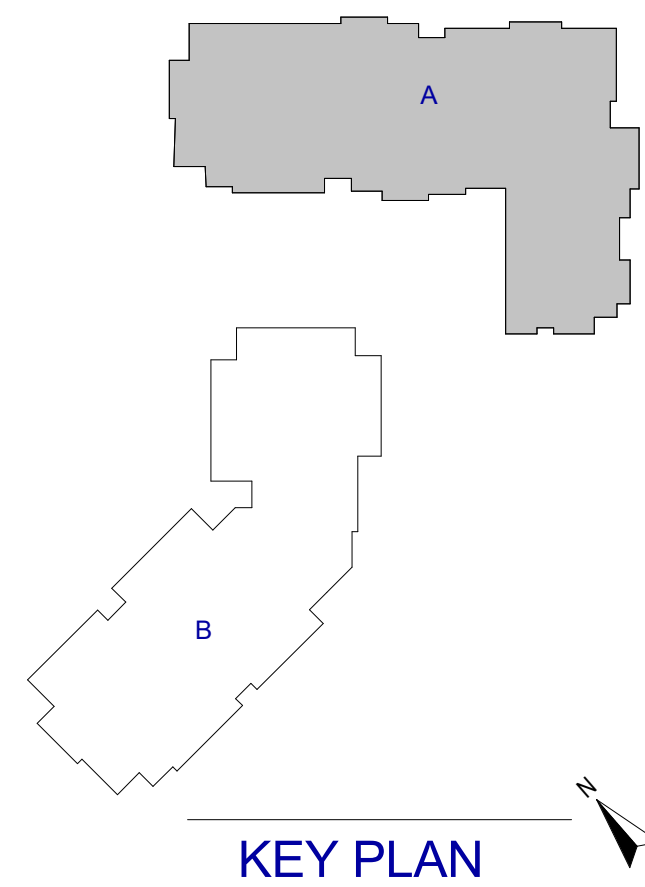
KEY PLAN

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1 S-145A-PT PARTIAL ROOF PT PLAN - AREA A 1/8" = 1'-0"

POST-TENSIONED CONCRETE SLAB PLAN NOTES:

- SEE FRAMING PLAN FOR SLAB GEOMETRY, REFERENCE ELEVATION, AND KEYED SECTIONS.
- SEE POST-TENSIONED FRAMING GENERAL NOTES ON SHEET S-003 AND DIVISION 03 SPECIFICATIONS FOR GENERAL REQUIREMENTS.
- FOR TYPICAL POST-TENSIONED CONCRETE SECTIONS AND DETAILS, SEE S-330 SERIES.
- TENDON LAYOUT NOTES:
 - LINEWORK SHOWN FOR TENDON PATHS IS SCHEMATIC TO ILLUSTRATE DESIGN INTENT, BUT NOT MEANT TO INDICATE EXACT TENDON PATHS OR SPACING OF UNIFORMLY DISTRIBUTED TENDON BUNDLES.
 - A MINIMUM OF (2) TENDONS SHALL PASS THROUGH EACH COLUMN IN EACH DIRECTION, WITH TENDONS LOCATED INSIDE OF THE COLUMN VERTICAL REINFORCEMENT CAGE.
 - STRAIGHT LINE HORIZONTAL OFFSETS SHOWN IN THE LAYOUT PLAN SHALL BE ACCOMPLISHED WITH SMOOTH HORIZONTAL CURVES/SWEEP PER DETAIL.
 - MAINTAIN CLEAR COVER BETWEEN EDGE-OF-SLAB (INCLUDING INTERIOR OPENINGS) AS INDICATED IN THE PROJECT SPECIFICATIONS AND TYPICAL DETAILS REFERENCED ABOVE.
- TENDON DRAPE ELEVATION NOTES:
 - # DENOTES PT TENDON DRAPE ELEVATION IN INCHES, MEASURED FROM BOTTOM OF SLAB / FRAMING TO THE CENTER-OF-GRAVITY OF THE TENDONS/STRAND BUNDLE (CGS).
 - AT DEAD ENDS AND STRESSING ENDS, CGS SHALL BE LOCATED AT THE CENTROID OF THE FRAMING (MID-DEPTH FOR SLAB CONDITIONS), UNLESS OTHERWISE NOTED.
 - TENDON HIGH POINTS SHALL BE LOCATED AT SUPPORTS (COLUMNS, WALLS, BEAMS, BANDED TENDON LINES) AS INDICATED ABOVE.
 - TENDON LOW POINTS SHALL BE LOCATED AT MID-SPAN BETWEEN SUPPORTS, UNLESS NOTED OTHERWISE.
- TENDON FORCE NOTES:
 - F = # K DENOTES EFFECTIVE PRESTRESS FORCE, IN KIPS, WITHIN BANDED TENDON GROUP OR BEAM.
 - BANDED TENDON GROUPS SHALL BE PLACED IN FLAT BUNDLED GROUPS OF NO MORE THAN (5) TENDONS PER BUNDLE, SPACED AT NO MORE THAN 12 INCHES ON CENTER, WITH TENDONS FLAT IN ONE LAYER. TOTAL QUANTITY OF BANDED TENDONS SHALL BE EQUALLY DISTRIBUTED EACH SIDE OF THE COLUMN, MINUS A MINIMUM OF (2) TENDONS PASSING THROUGH THE COLUMN REINFORCEMENT CAGE PER NOTE ABOVE.
 - F = # KLF DENOTES EFFECTIVE PRESTRESS FORCE, IN KIPS PER LINEAR FOOT, IN UNIFORMLY DISTRIBUTED TENDONS.
 - UNIFORMLY DISTRIBUTED TENDON BUNDLES SHALL BE PLACED IN FLAT BUNDLED GROUPS OF NO MORE THAN (5) TENDONS PER BUNDLE, SPACED AT NO MORE THAN 5'-0" ON CENTER, WITH TENDONS FLAT IN ONE LAYER.

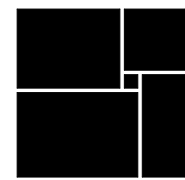
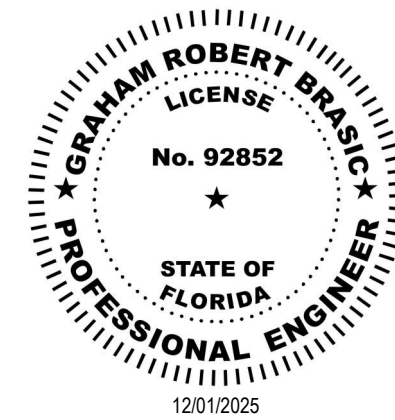


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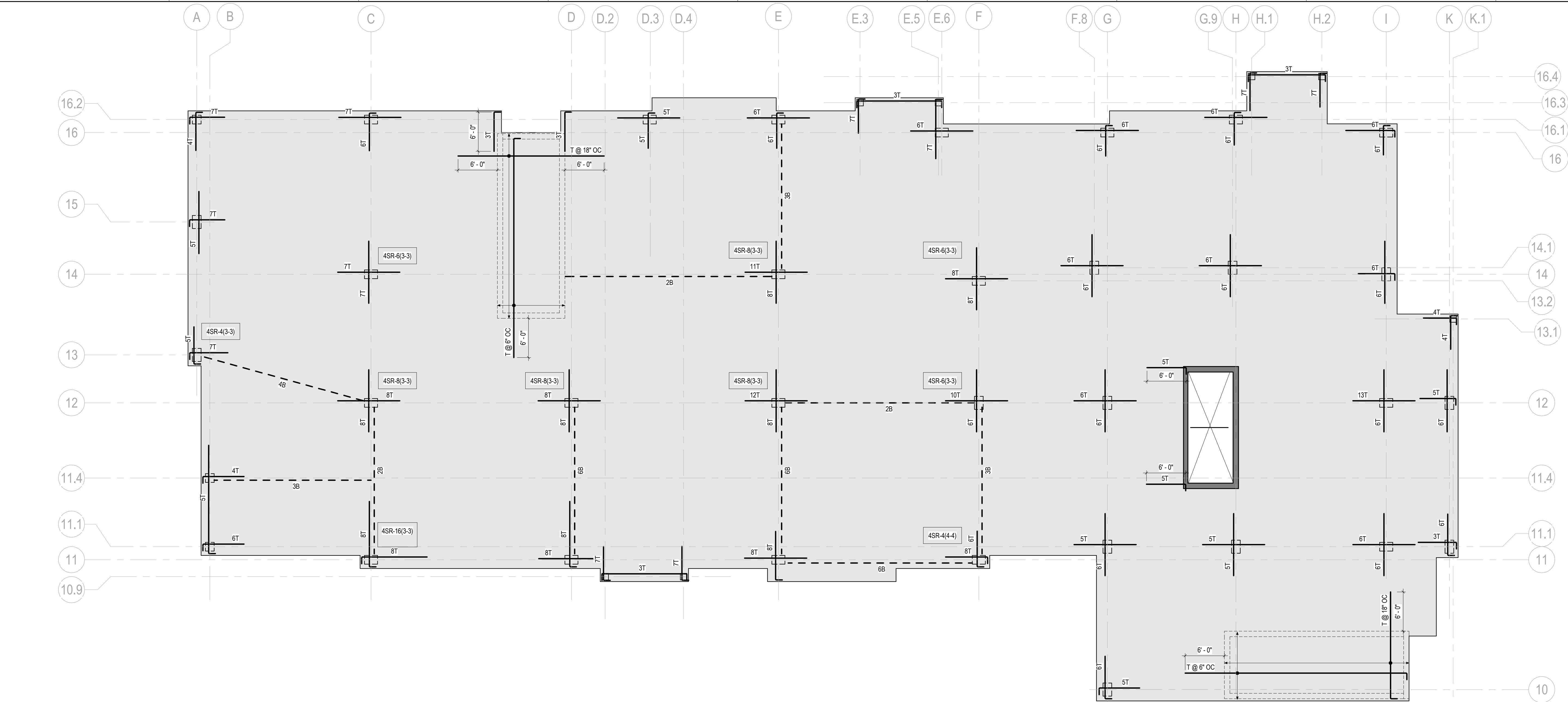
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Project No.: 2021009
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PARTIAL
ROOF PT
PLAN AREA A

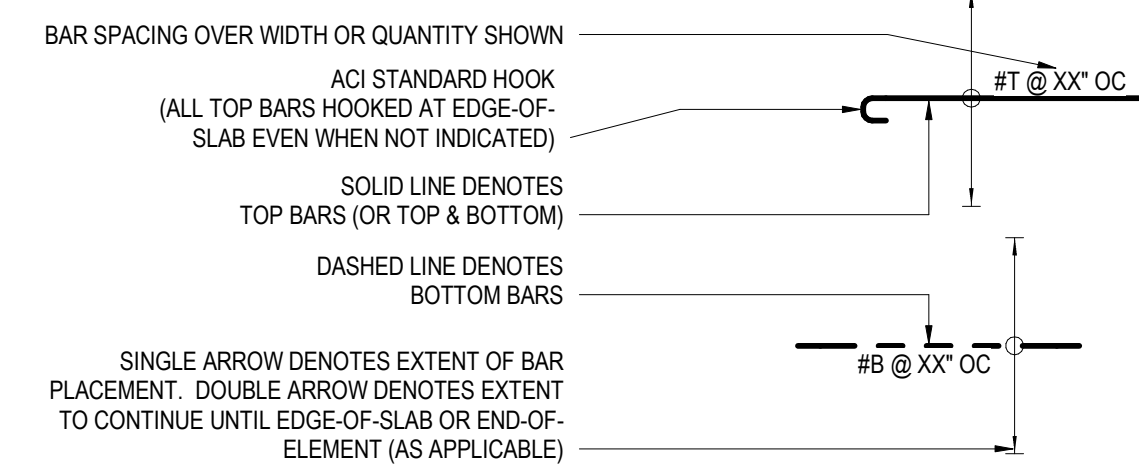
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1 S-145A-R PARTIAL ROOF REINFORCING PLAN - AREA A 1/8" = 1'-0"

- CONCRETE SLAB REINFORCEMENT PLAN NOTES:
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 - PROVIDE (4) - #5 CONTINUOUS REINFORCEMENT (2 TOP & 2 BOTTOM, SPACED AT 6" OC) AROUND PERIMETER OF SLAB AND AT ALL INTERIOR SLAB EDGES. SEE 111 S-331 FOR CONCEPTUAL LAYOUT (NOT INDICATIVE OF PROJECT SPECIFIC GEOMETRY) AND SPICE/DEVELOPMENT REQUIREMENTS.
 - SLAB REINFORCEMENT SHALL BE #5 UNLESS NOTED OTHERWISE.
 - #SR-#(##-##) DENOTES SHEAR STUD RAILS (SEE DETAILS AND SCHEDULE ON S-332).
 - SLAB REINFORCEMENT SYMBOLS AND NOMENCLATURE:
 - A. MAT REINFORCEMENT (SEE SLAB SCHEDULE).
 - B. ISOLATED/DISTRIBUTED REINFORCEMENT (SEE ---).



- CONCRETE SLAB REINFORCEMENT PLAN NOTES:
- C. TYPICAL NOMENCLATURE:

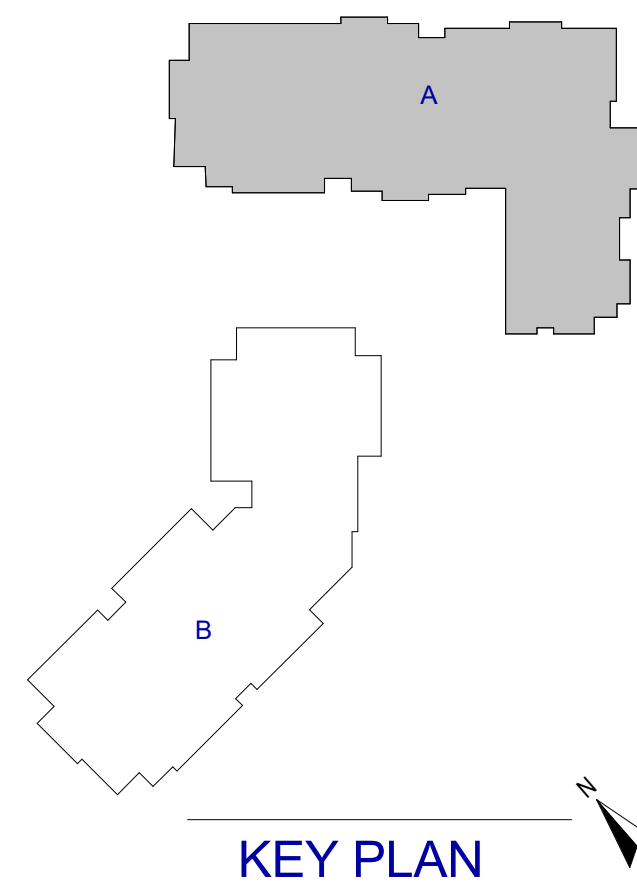
DENOTES QUANTITY OF BARS
T: DENOTES TOP BARS
B: DENOTES BOTTOM BARS

- D. REINFORCEMENT AT COLUMNS:

CONCRETE COLUMN (SEE FRAMING PLAN)
BARS EACH WAY (SEE TYPICAL NOMENCLATURE)

POST-TENSIONED CONCRETE SLAB SCHEDULE				
MARK	THICKNESS	TYPICAL REINFORCEMENT		
		TOP	BOTTOM	
PT8	8"	SEE PLAN	#4 @ 48" OC EW	

SHEAR STUD RAIL SCHEDULE					
MARK	FIRST STUD SPACING	TYP STUD SPACING	# OF STUDS PER RAIL	NUMBER OF RAILS	
				SHORT SIDE	LONG SIDE
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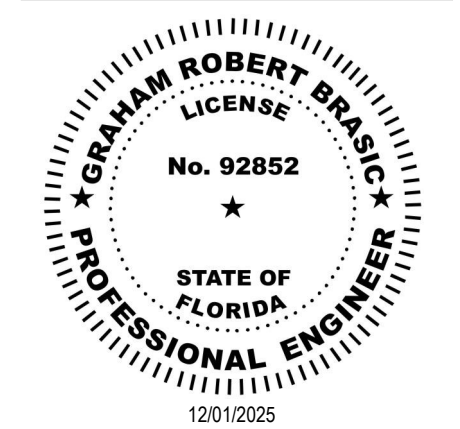


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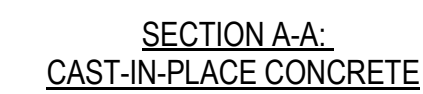
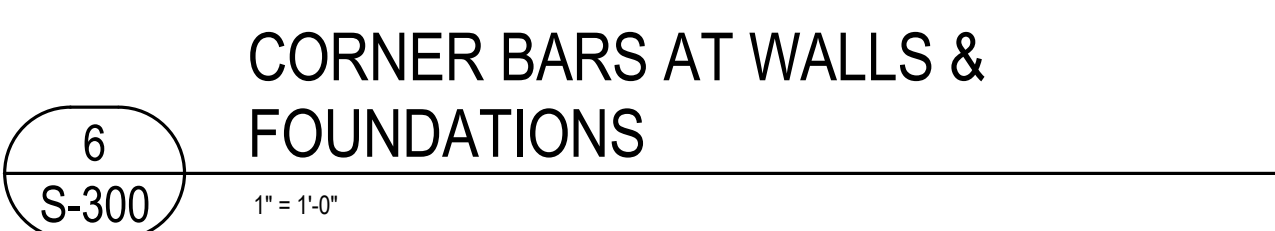
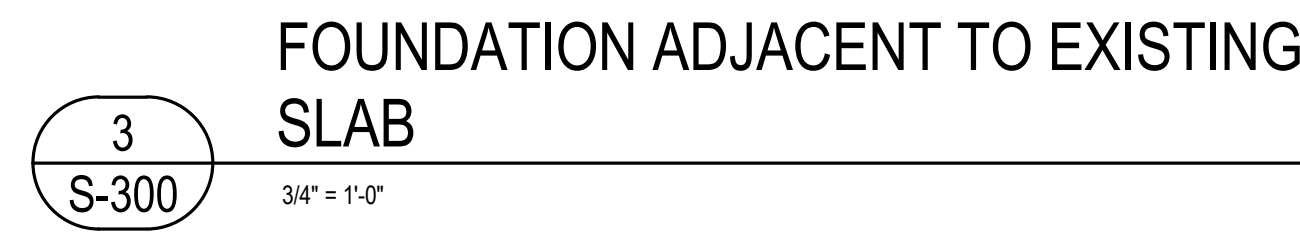
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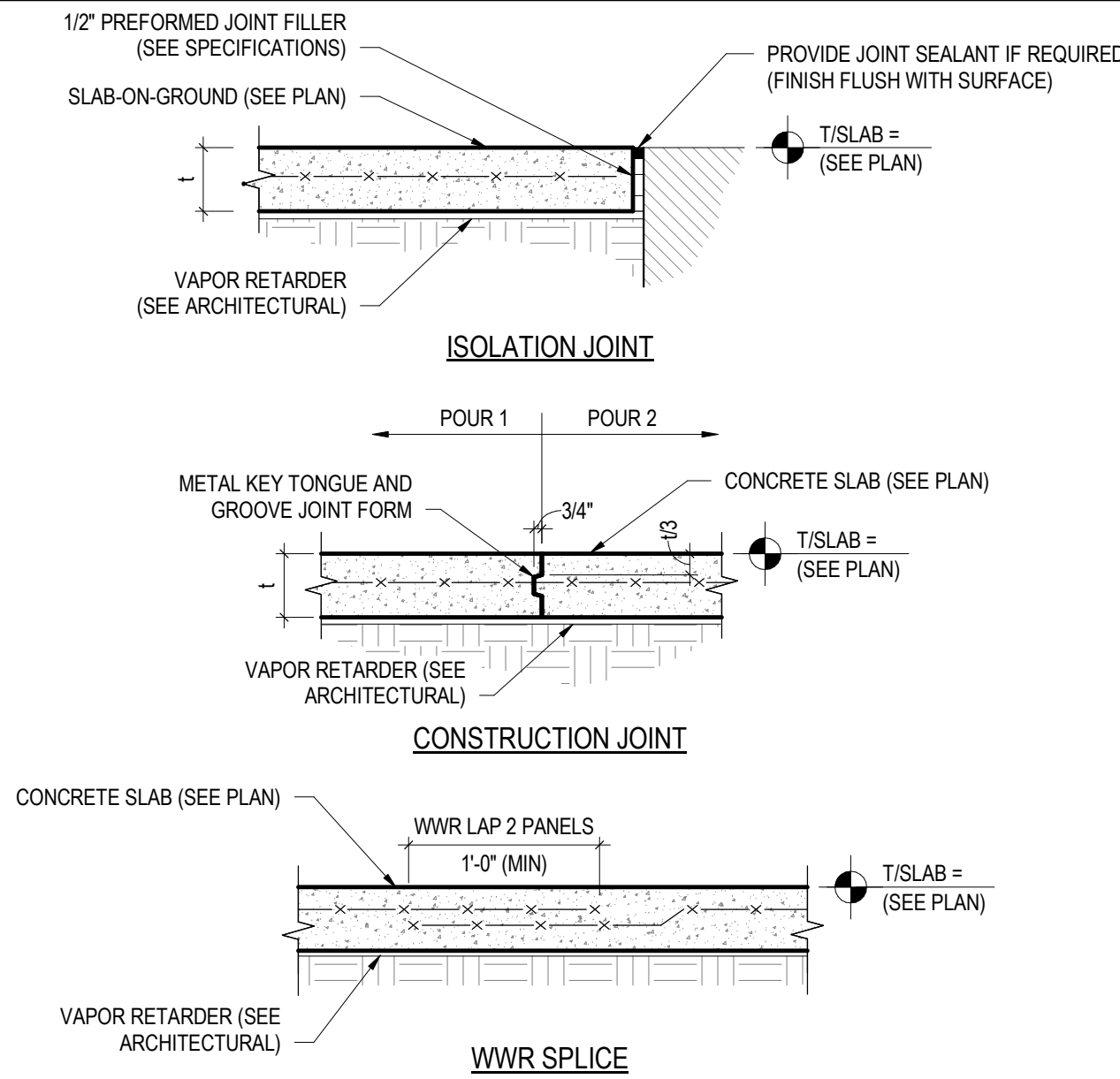
Project No.: 2021009
Date: 12/01/2025

**PARTIAL
ROOF
REINFORCING
PLAN AREA A**

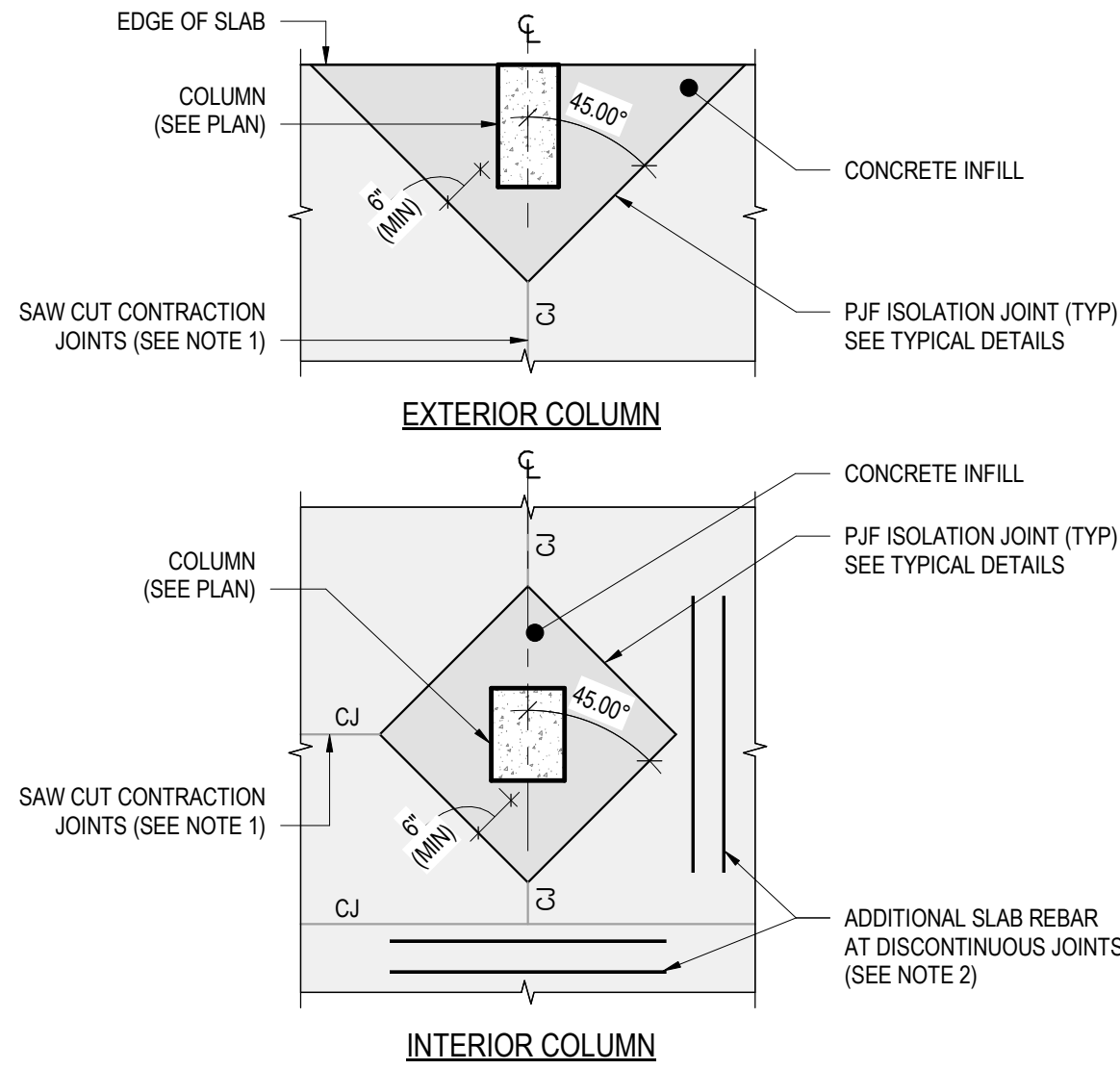
S-145A-R



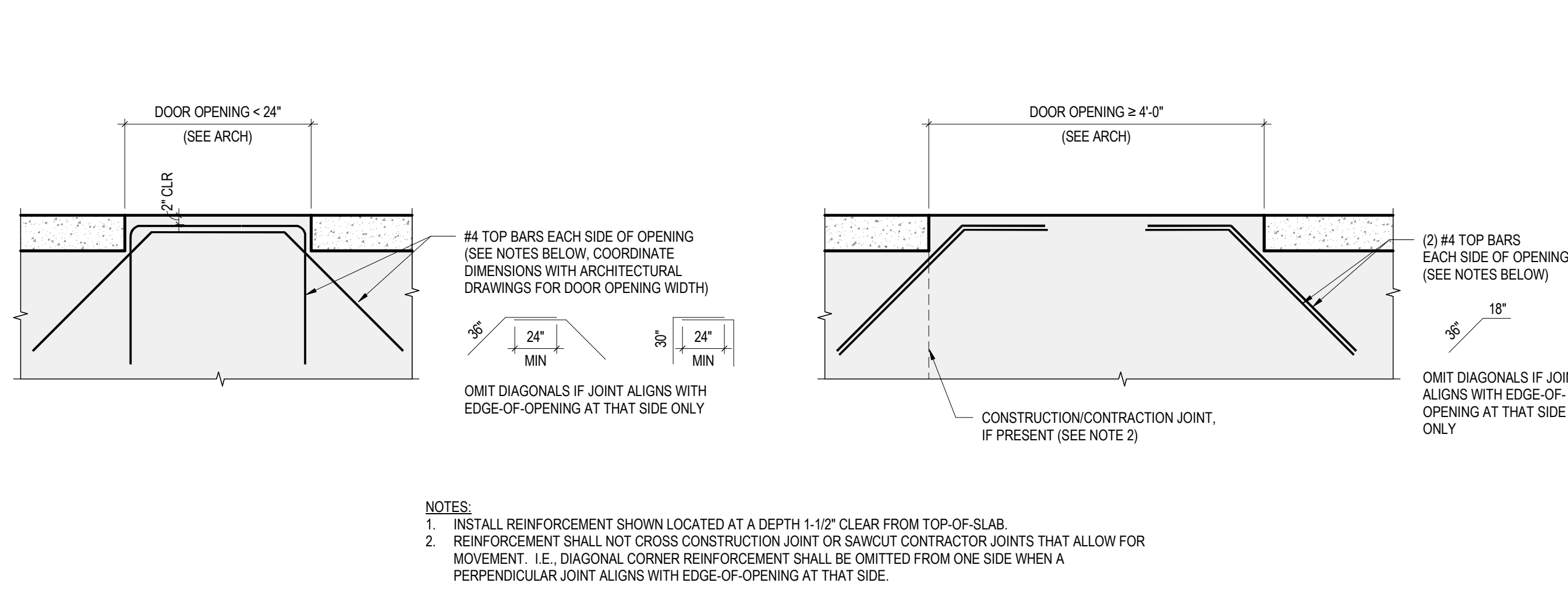
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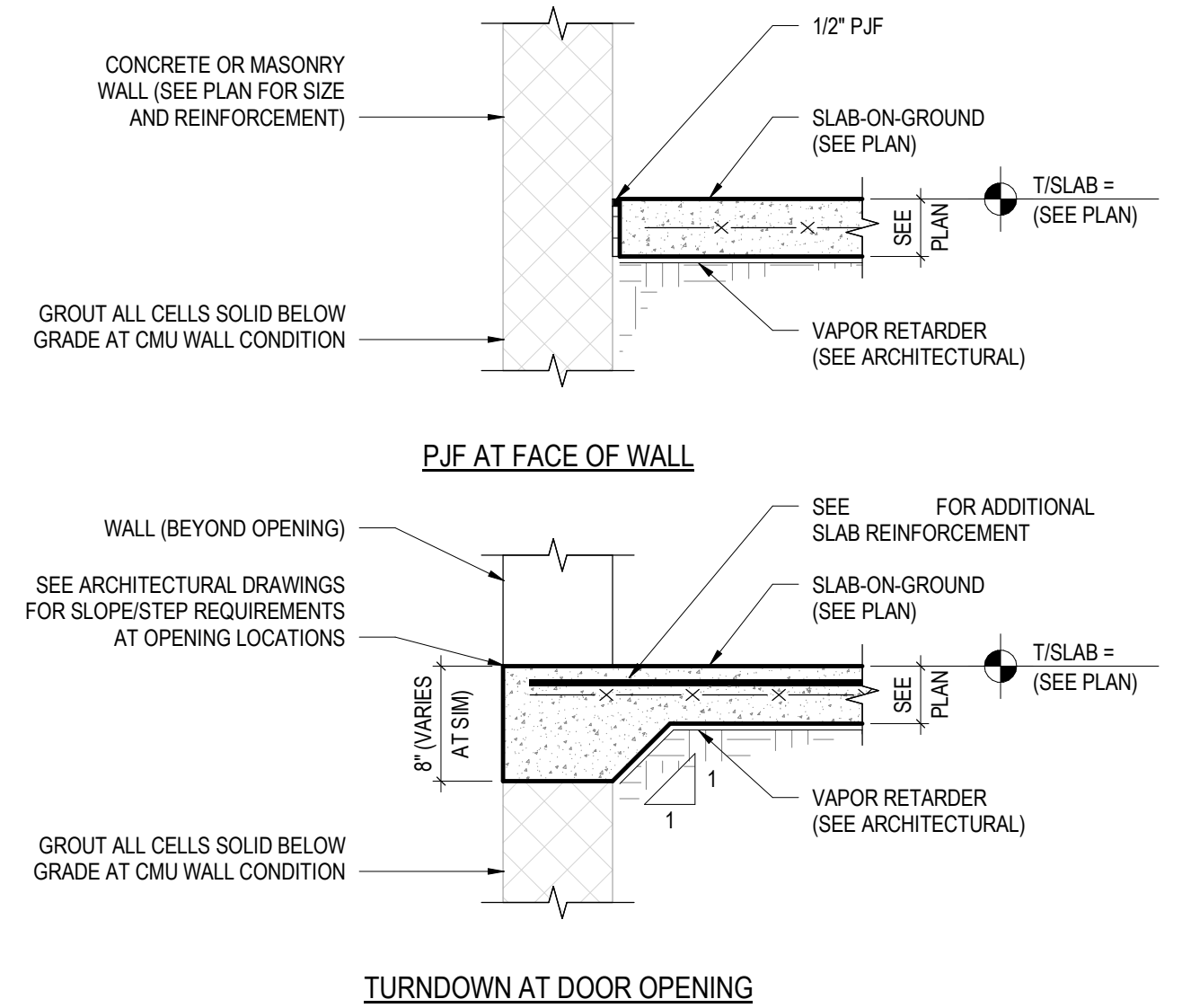
1
S-310
3/4" = 1'-0"



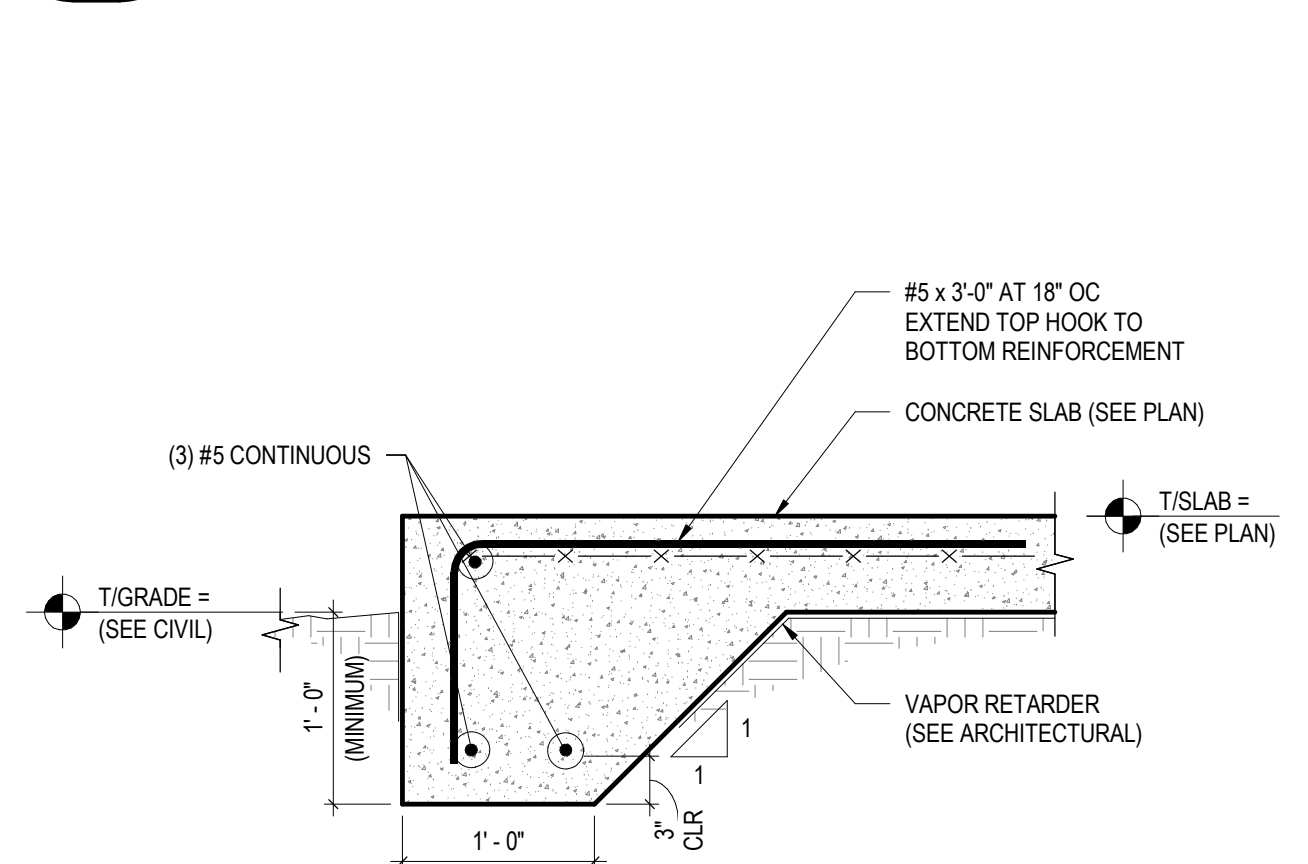
2
S-310
3/8" = 1'-0"



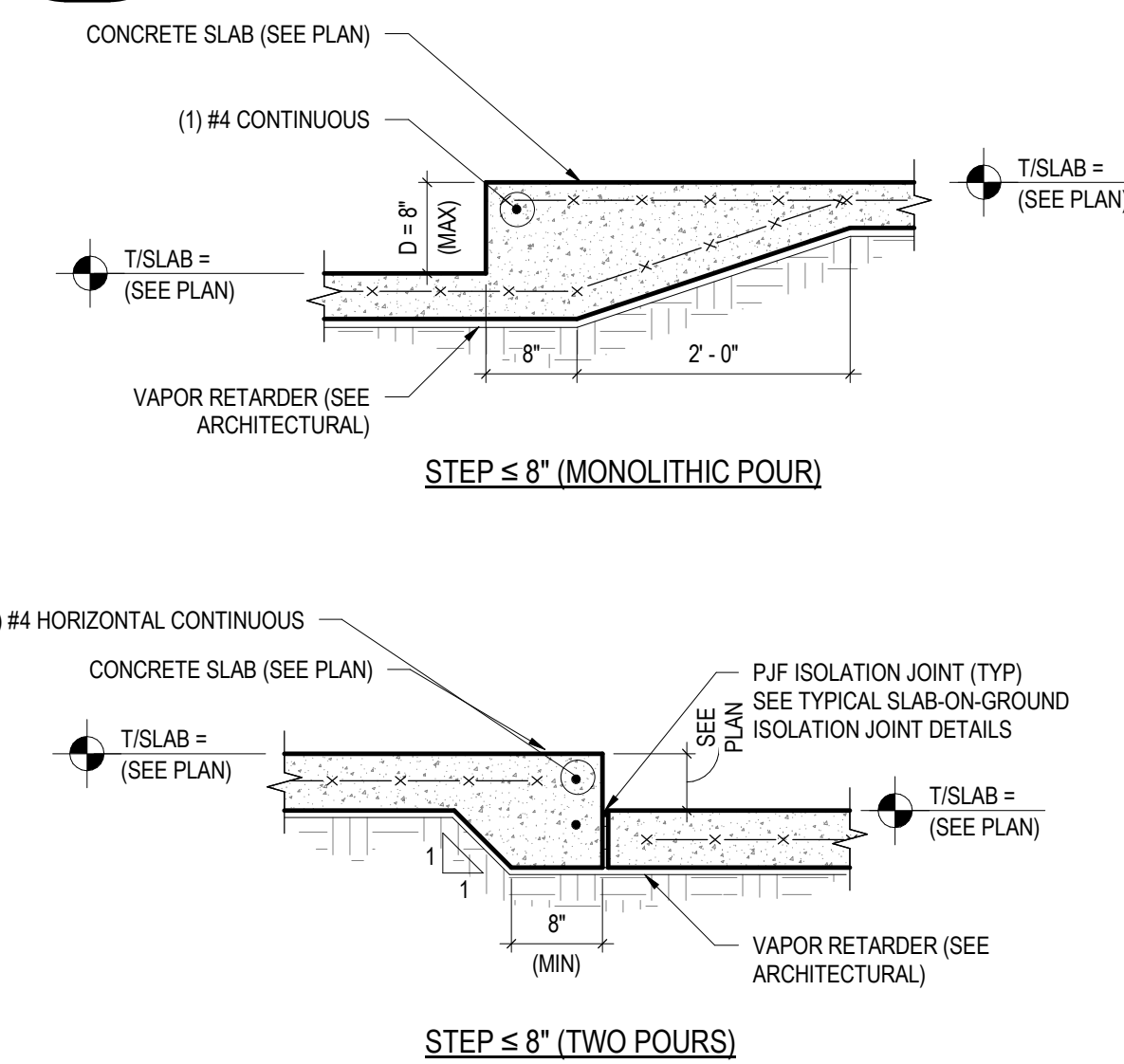
3
S-310
3/4" = 1'-0"



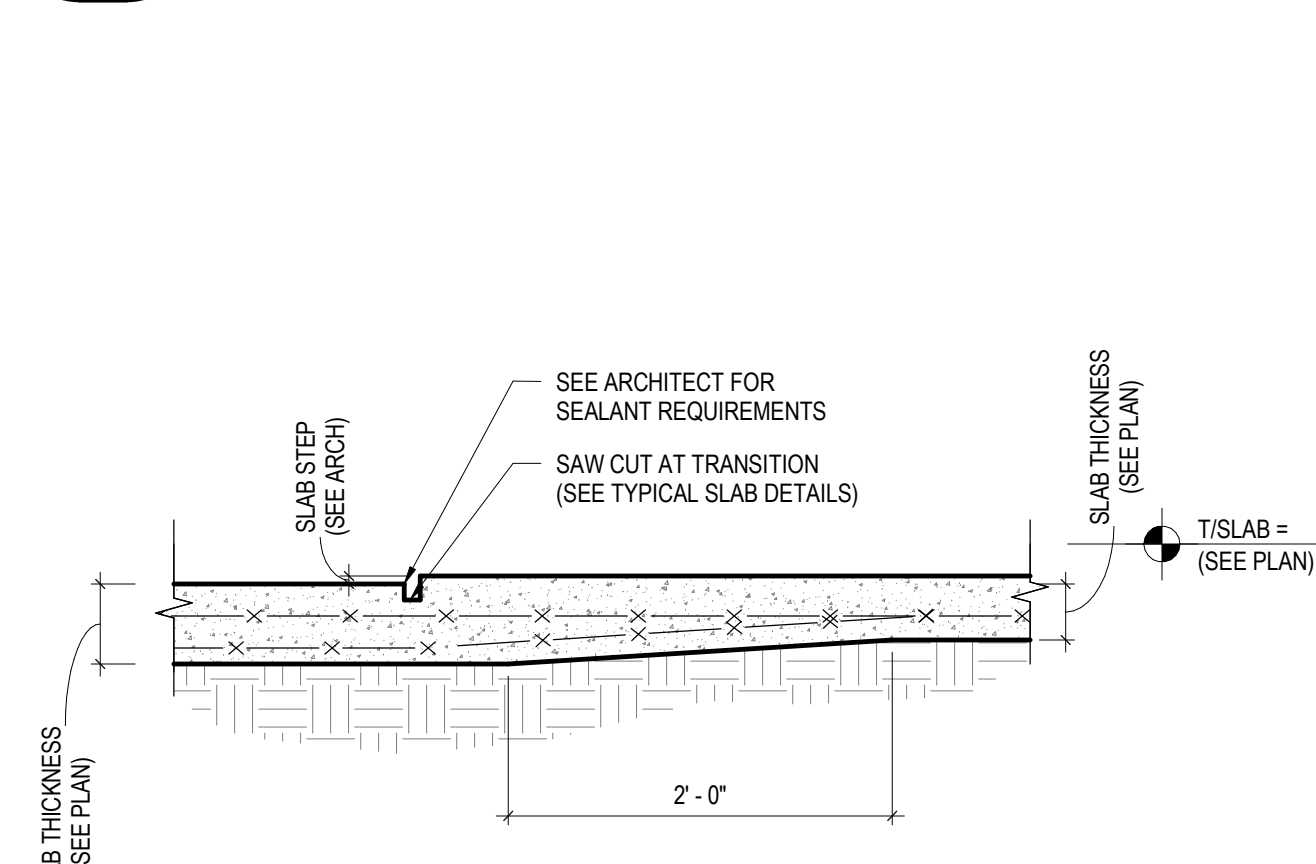
4
S-310
1/2" = 1'-0"



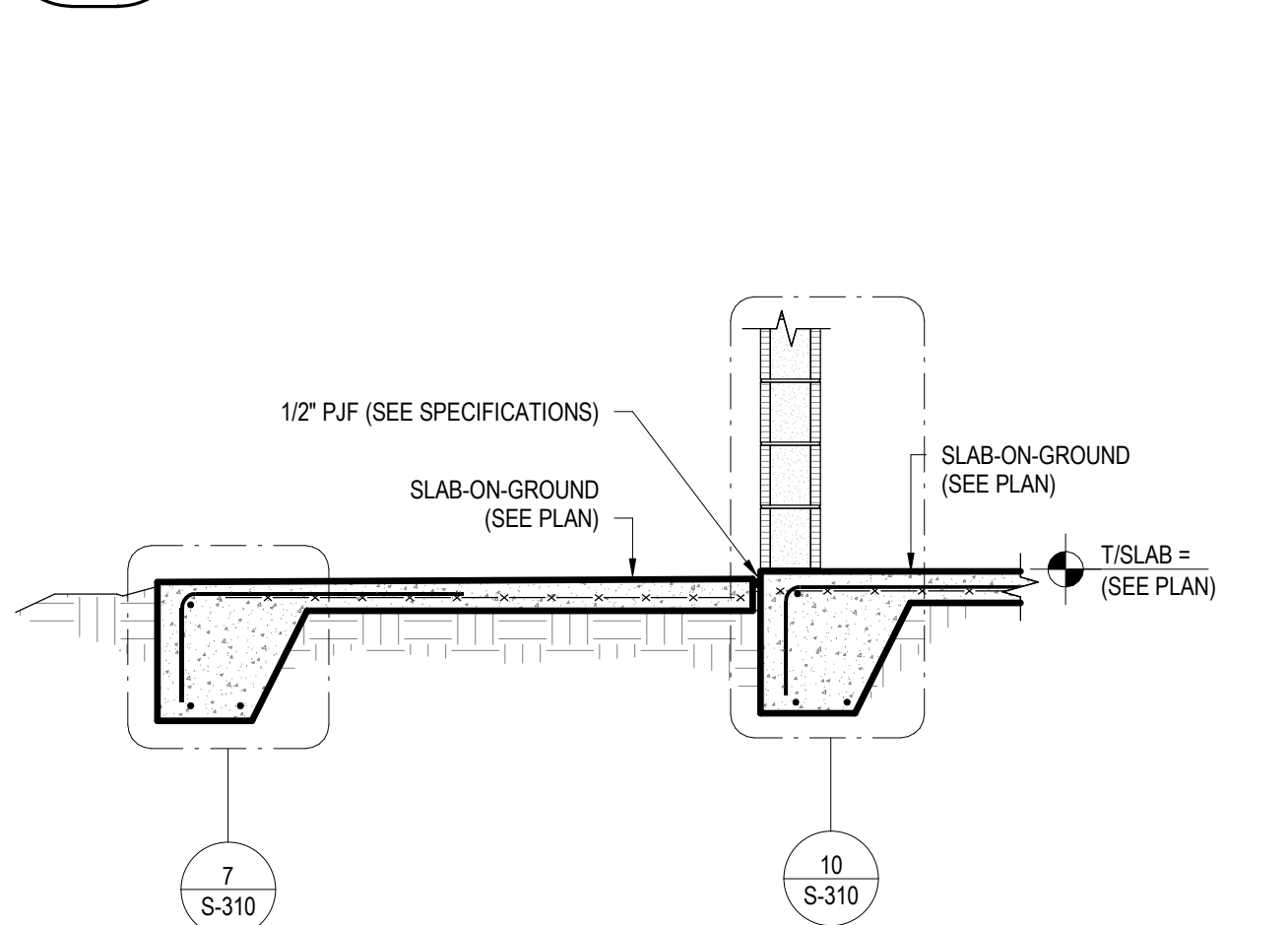
5
S-310
1/2" = 1'-0"



6
S-310
1" = 1'-0"



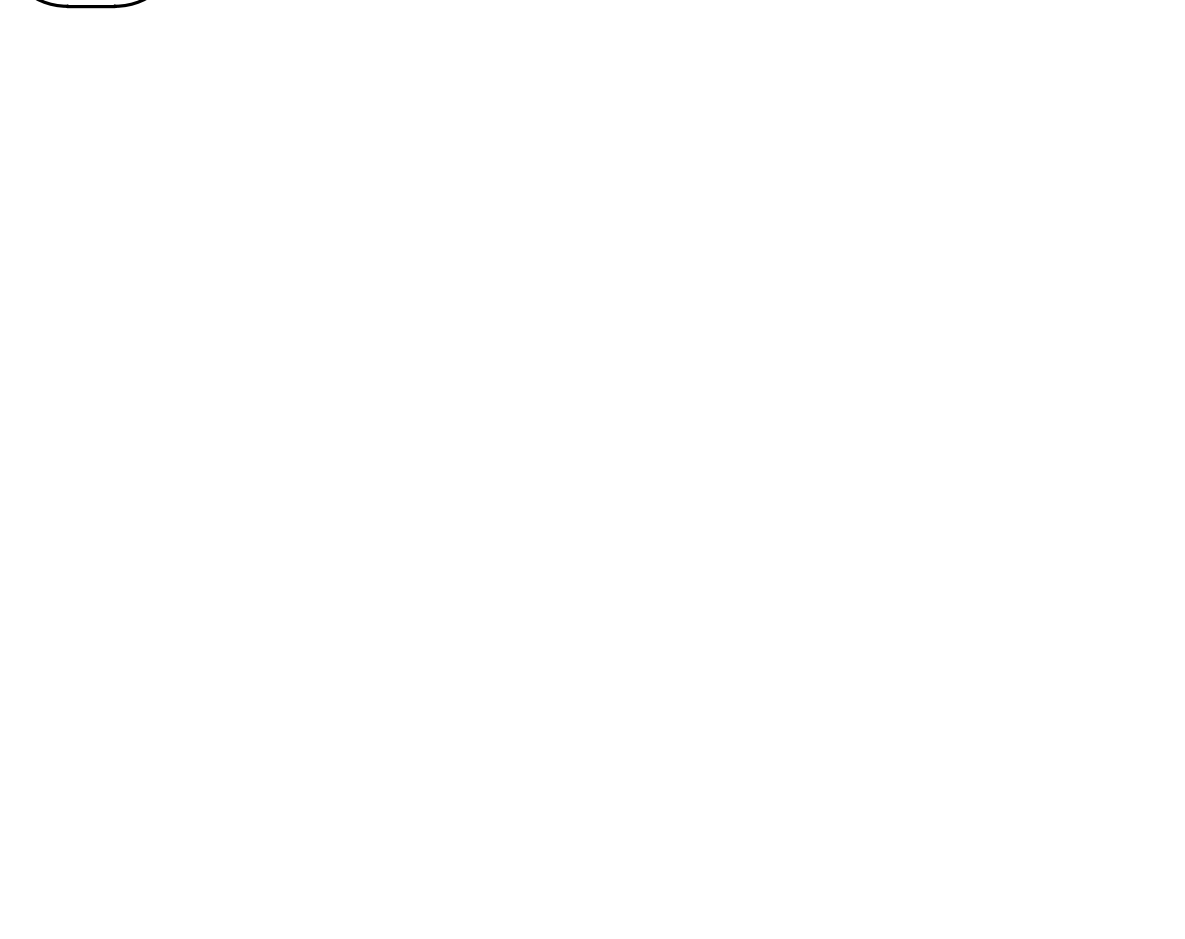
7
S-310
1" = 1'-0"



8
S-310
1" = 1'-0"



9
S-310
3/4" = 1'-0"



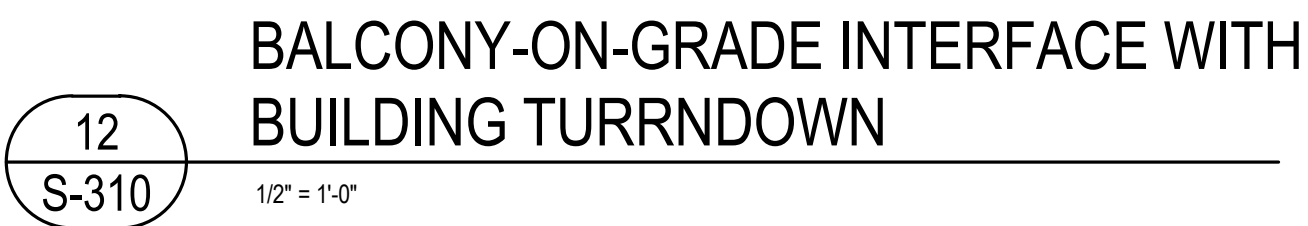
10
S-310
1" = 1'-0"



11
S-310
1" = 1'-0"



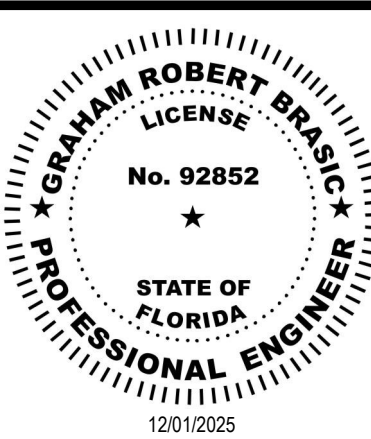
12
S-310
1/2" = 1'-0"



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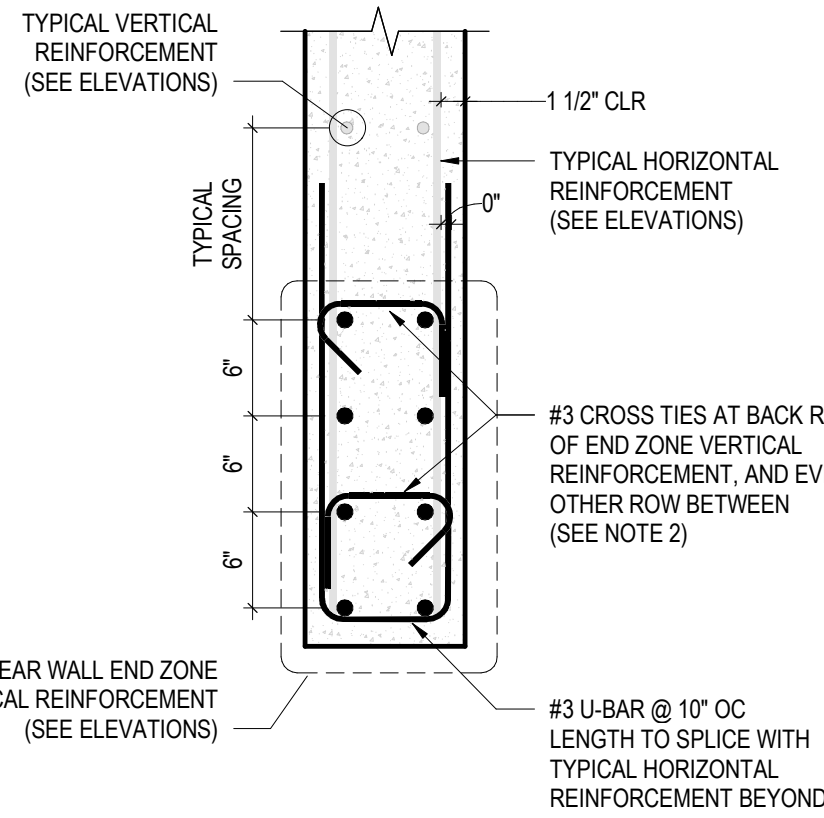
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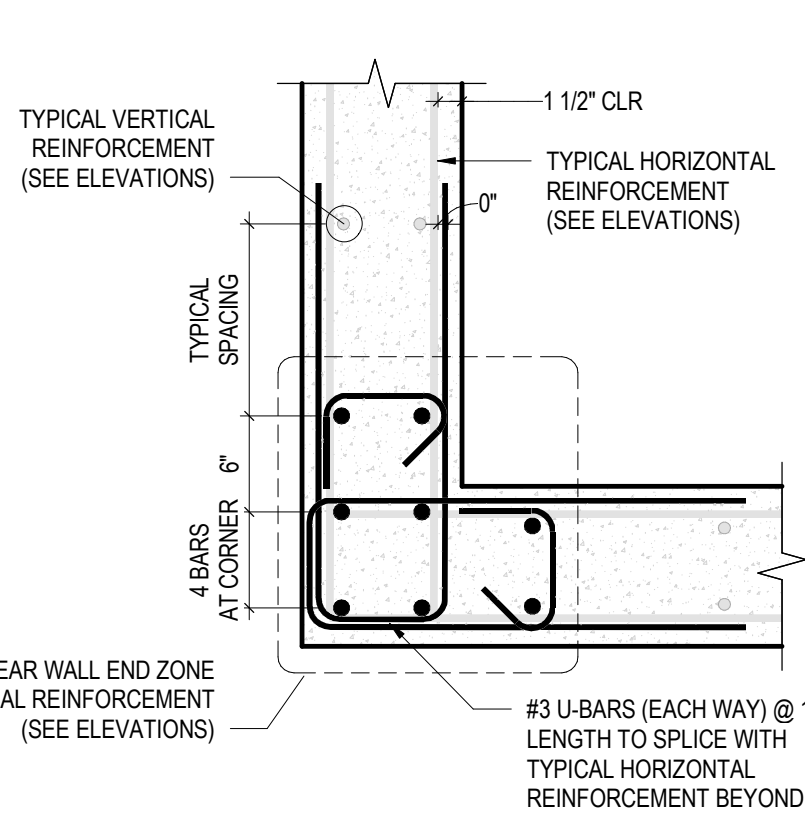
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**TYPICAL
SLAB-ON-GROUND
DETAILS**

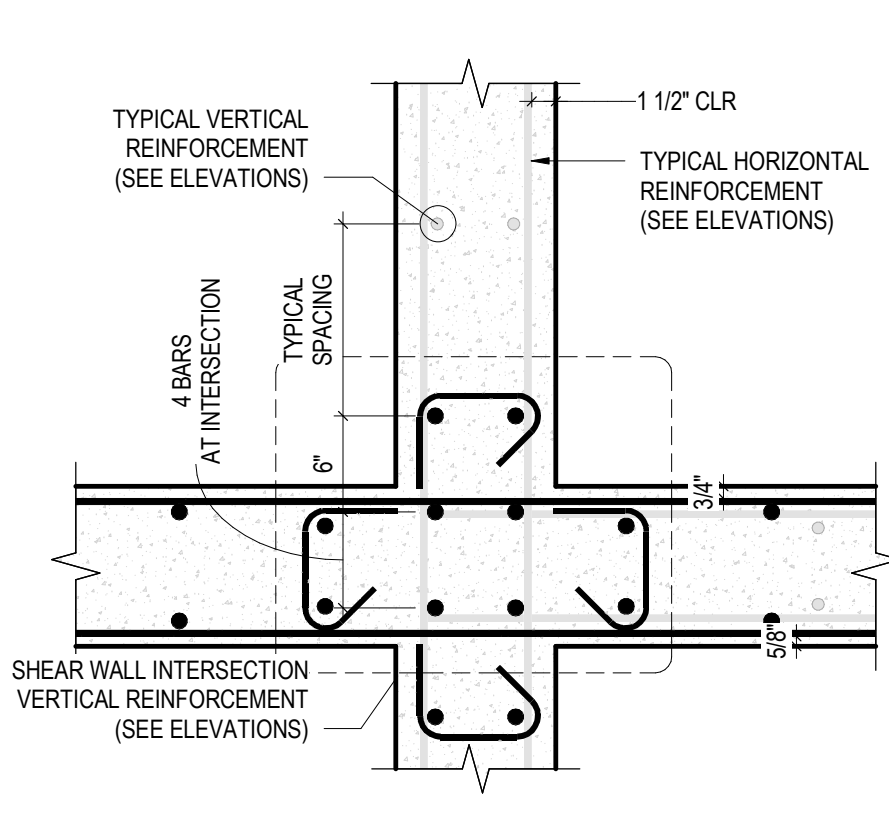
S-310



ENDS OF WALLS & AT OPENINGS
(E-TYPE MARK ON ELEVATIONS)

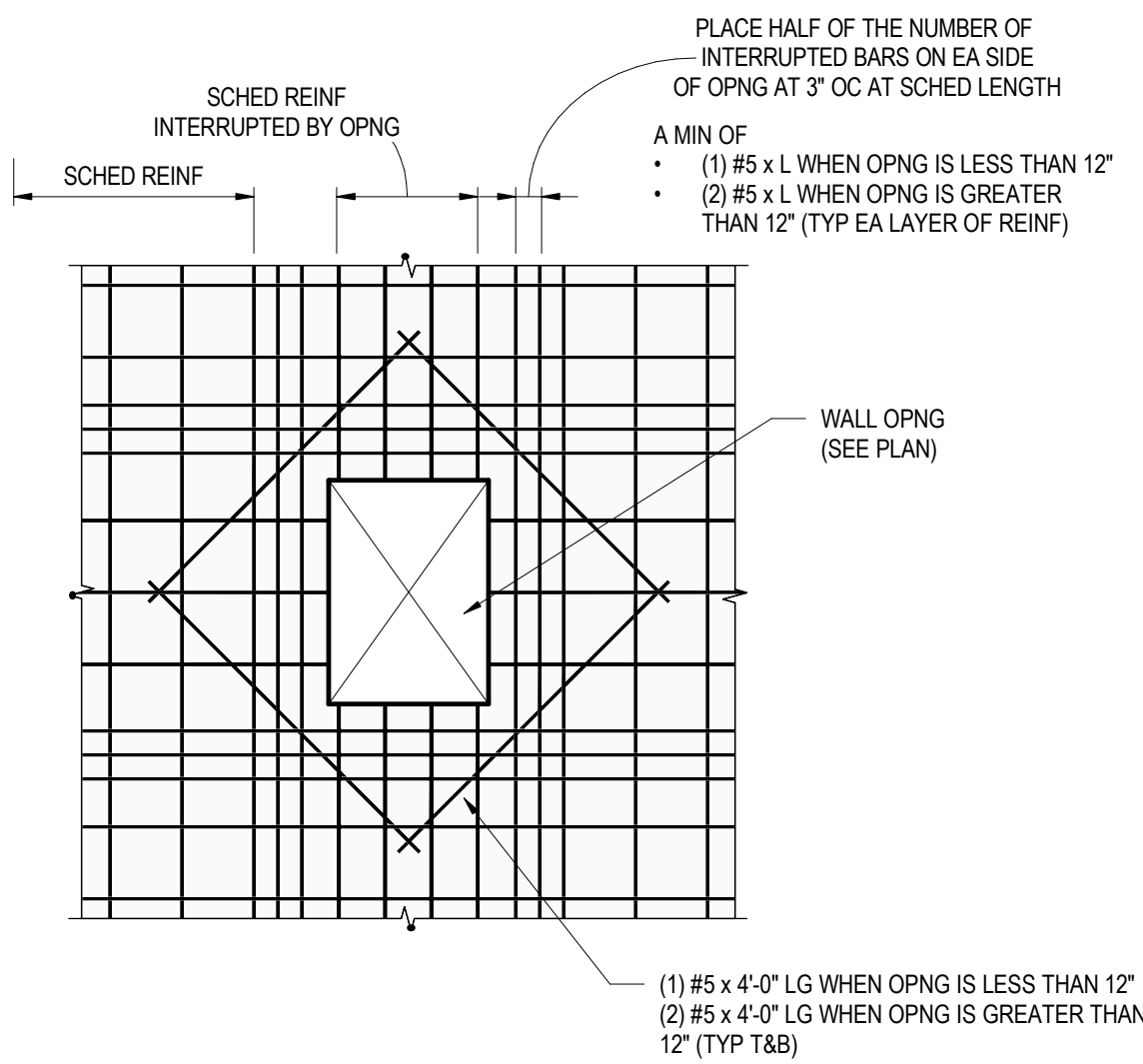
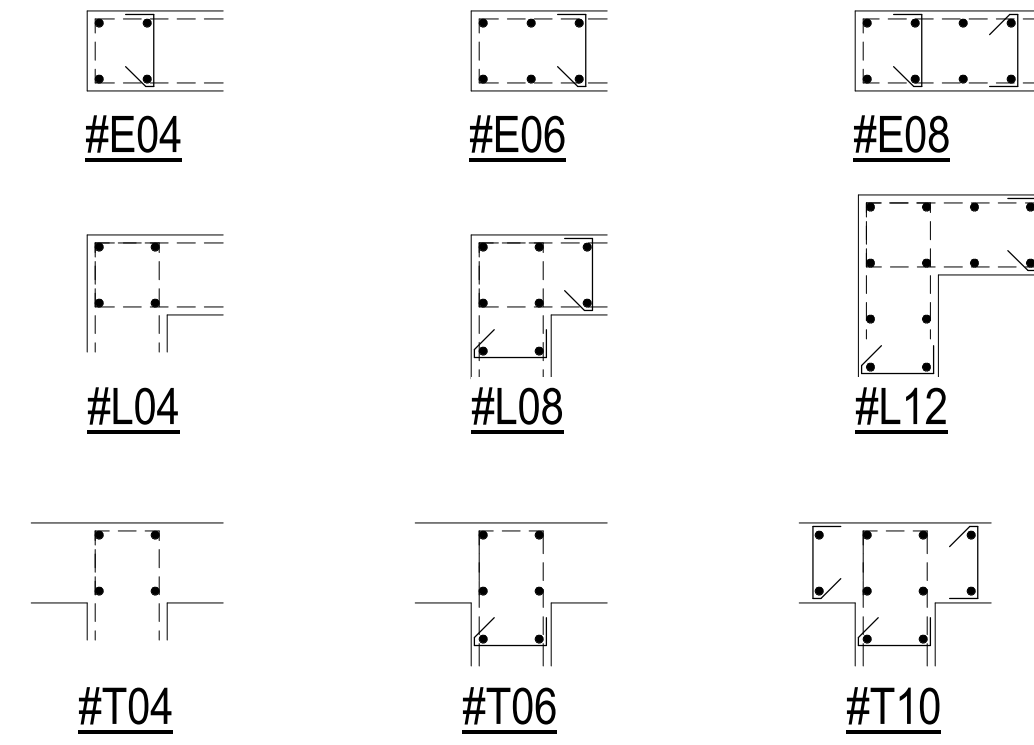


CORNER CONDITION
(L-TYPE MARK ON ELEVATIONS)

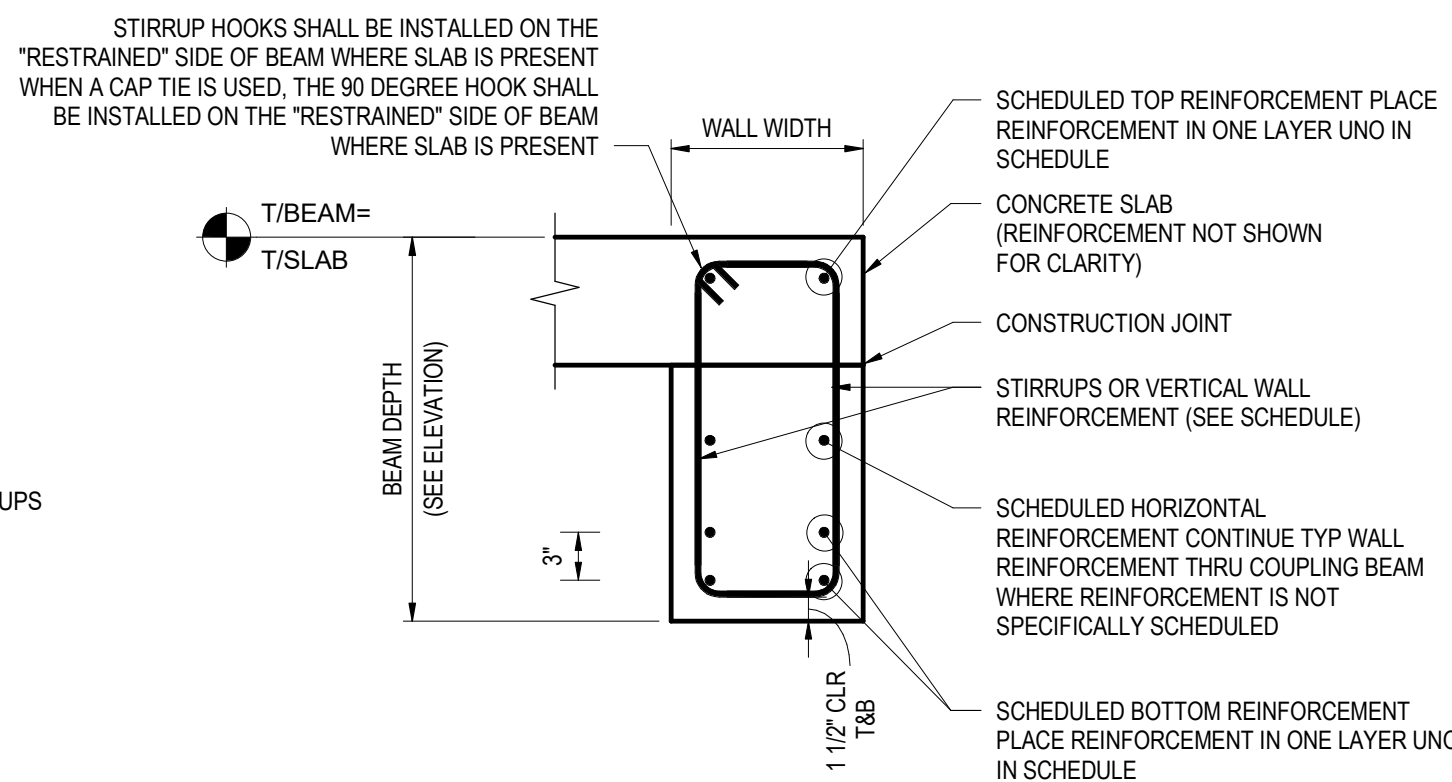
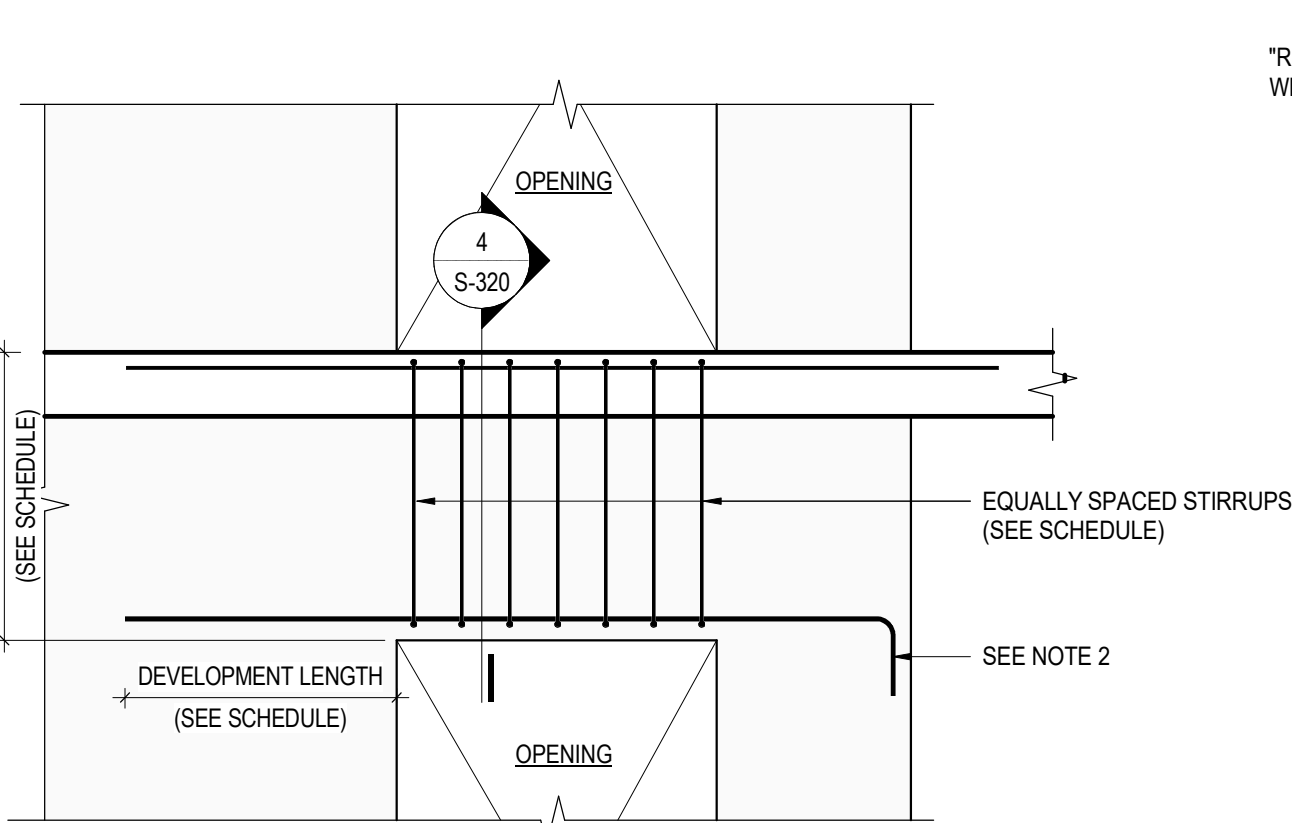


INTERSECTION CONDITION
(T-TYPE MARK ON ELEVATIONS)

- NOTES:
- BAR LAPS SHOWN OFFSET FOR CLARITY.
 - HORIZONTAL REINFORCEMENT SHALL BE LAPPED WITH BARS STACKED VERTICALLY
 - VERTICAL REINFORCEMENT SHALL BE LAPPED WITH BARS ALIGNED PARALLEL TO FACE-OF-WALL (OFFSET BENDS SHALL NOT BE PERMITTED)
 - CROSS TIE BAR ORIENTATION SHALL BE ALTERNATED AT EACH TIE IN BOTH THE HORIZONTAL AND VERTICAL DIRECTION
 - BAR COUNT INDICATED IN SHEAR WALL VERTICAL REINFORCEMENT SCHEDULE IS THE TOTAL QUANTITY OF BARS REQUIRED, DISTRIBUTED HALF EACH FACE AND THROUGHOUT CORNER/INTERSECTION CONDITIONS AS INDICATED IN DIAGRAMS BELOW.



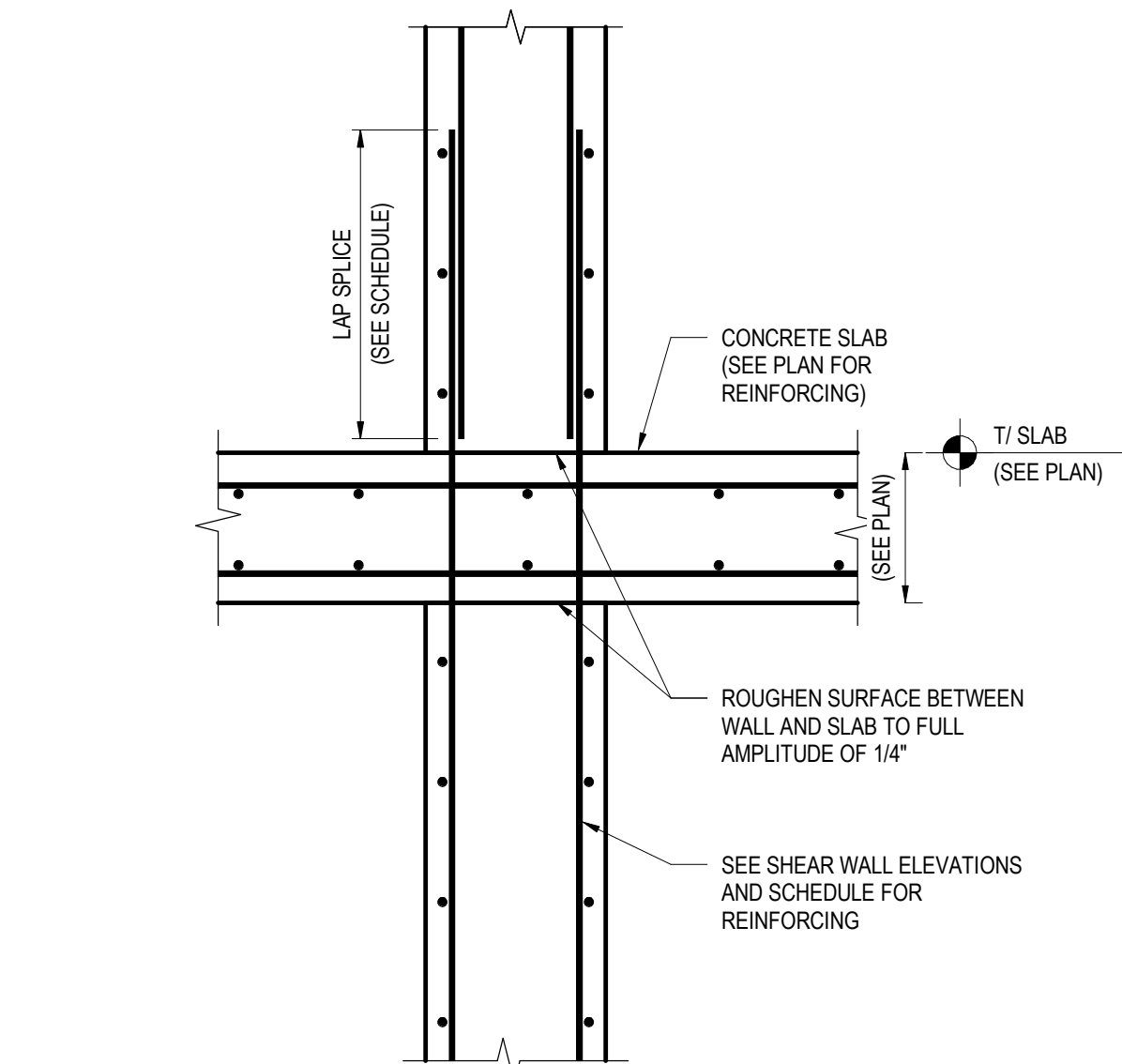
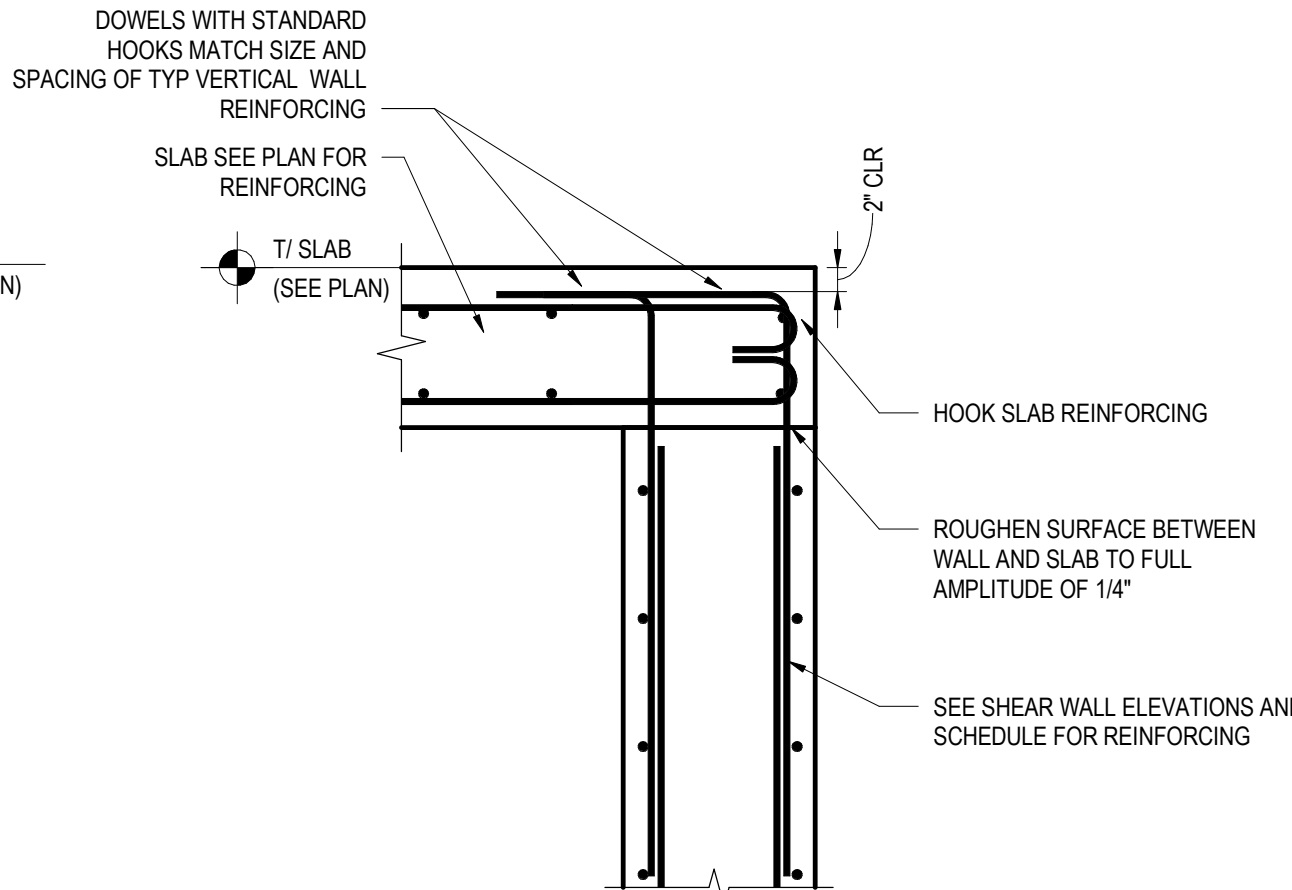
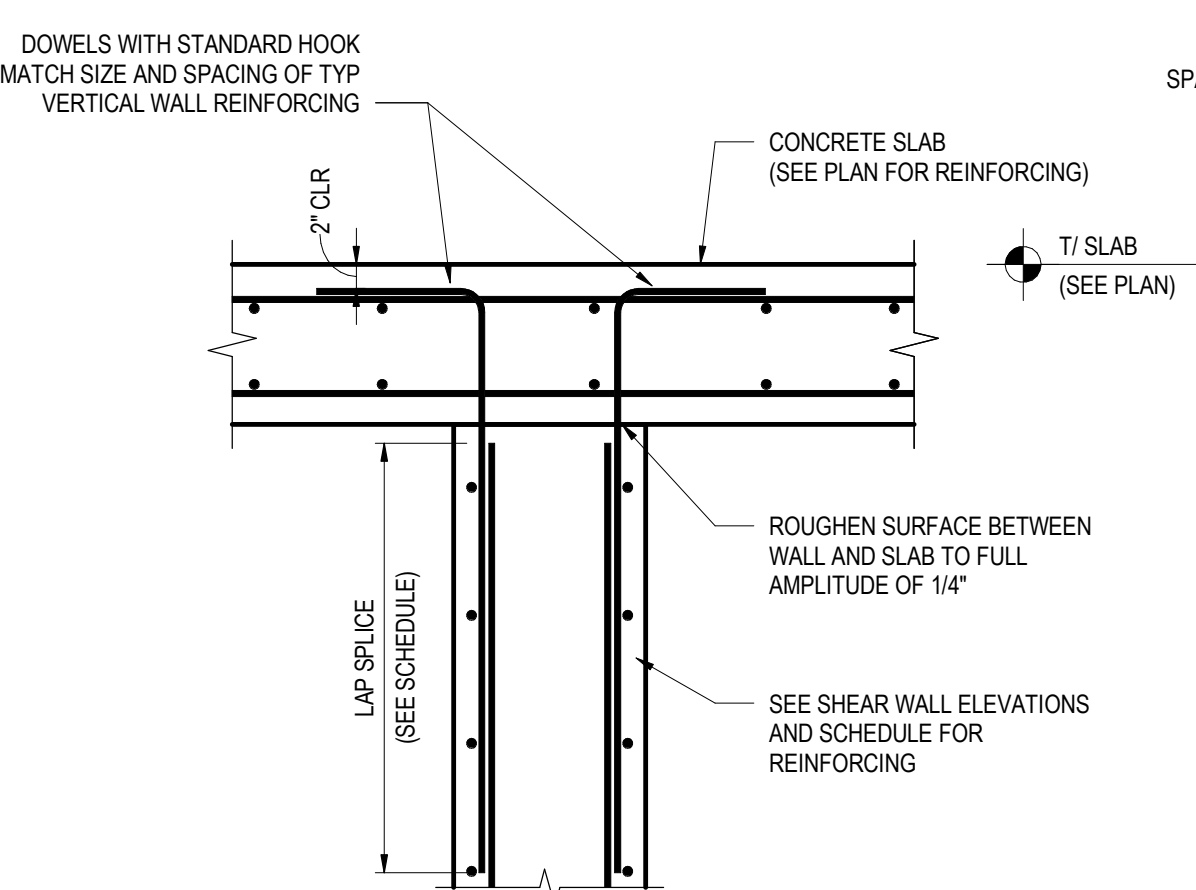
1 SHEAR WALL END ZONE AND CORNER/INTERSECTION REBAR LAYOUTS
1" = 1'-0"



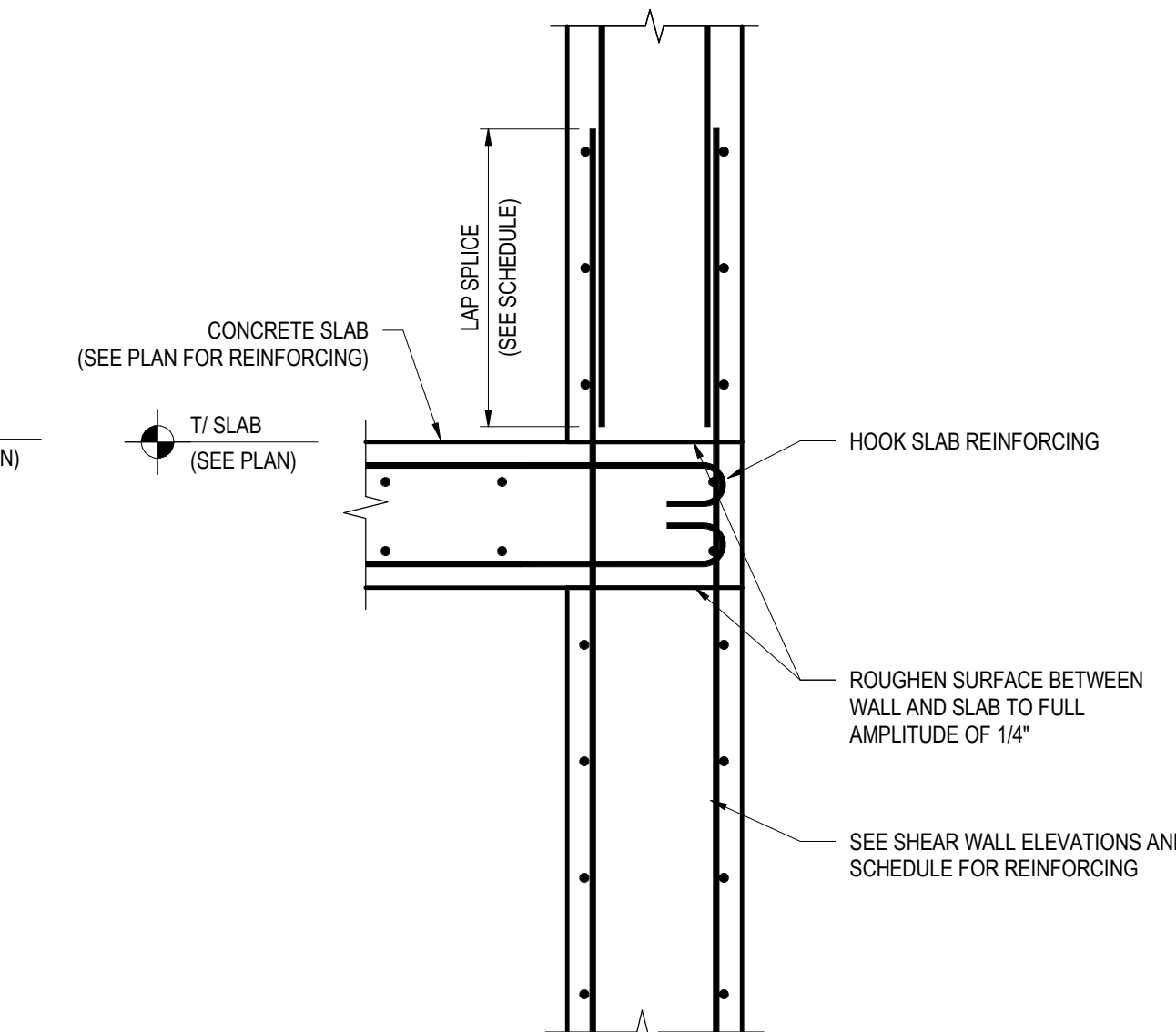
- NOTES:
- SHEAR WALL REINFORCEMENT IS NOT SHOWN IN THIS ELEVATION FOR CLARITY
 - WHEN LOCATED AT NEAR A CORNER OR END-OF-WALL, LINK BEAM LONGITUDINAL REINFORCEMENT SHALL BE TERMINATED WITH AN ACI STANDARD 90° HOOK. GO OPTION TO USE A HEADED TERMINATOR IN LIEU OF HOOK - SUBMIT PRODUCT DATA FOR SER REVIEW. TOP REINFORCEMENT SHALL CONTINUE INTO SLAB BEYOND WHEN GEOMETRY ALLOWS.
- NOTES:
- CLOSED STIRRUP MAY BE EITHER ONE CONTINUOUS BAR WITH A 135 DEGREE HOOK ON EITHER END AROUND THE SAME LONGITUDINAL BAR, OR A U-SHAPED STIRRUP WITH 135 DEGREE HOOKS AT EITHER END AND AN OPEN STIRRUP AS A CAP WITH A 135 DEGREE HOOK AT ONE END AND A 90 DEGREE HOOK AT THE OTHER (ACI 318 STANDARD OPTIONS FOR DETAILING OF CLOSED STIRRUP)
 - CONCRETE STRENGTH OF THE COUPLING BEAM SHALL MATCH THE SPECIFIED STRENGTH OF THE WALL BELOW, INCLUDING THE SLAB DEPTH.

3 SHEAR WALL LINK BEAM ELEVATION
1/2" = 1'-0"

4 SHEAR WALL LINK BEAM SECTION
1" = 1'-0"

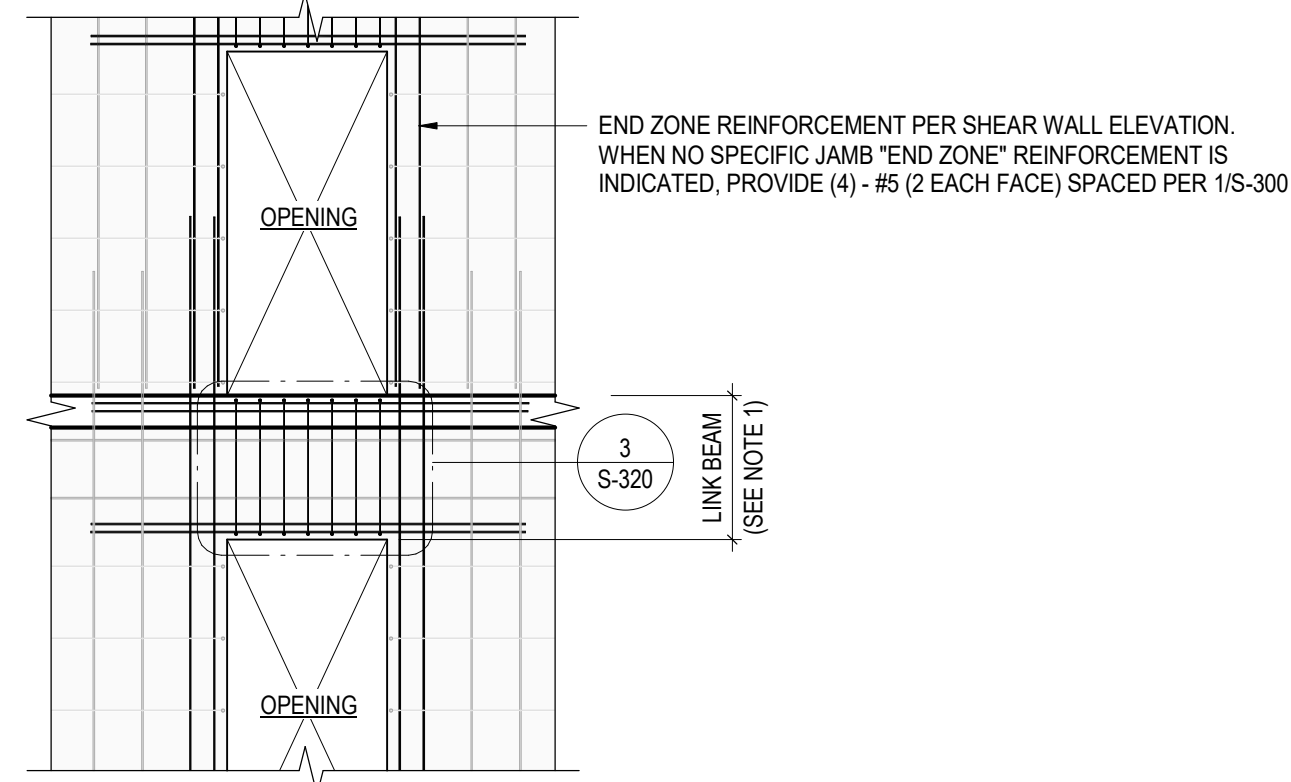


INTERIOR WALL

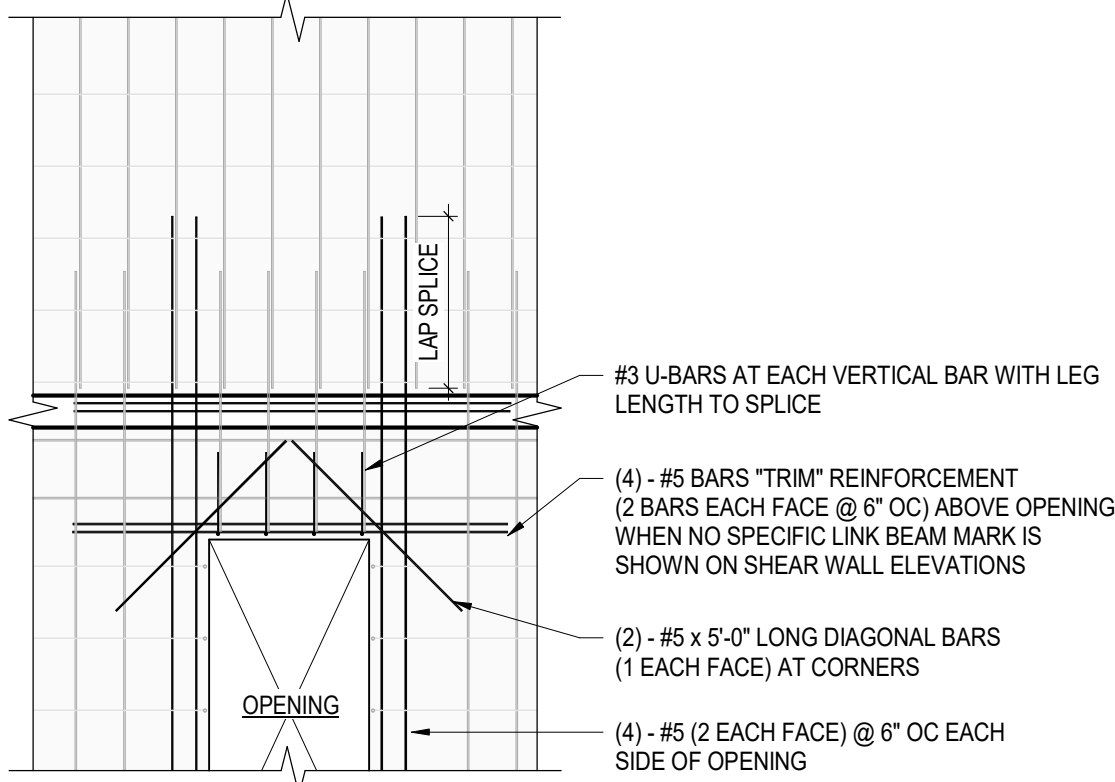


EXTERIOR WALL

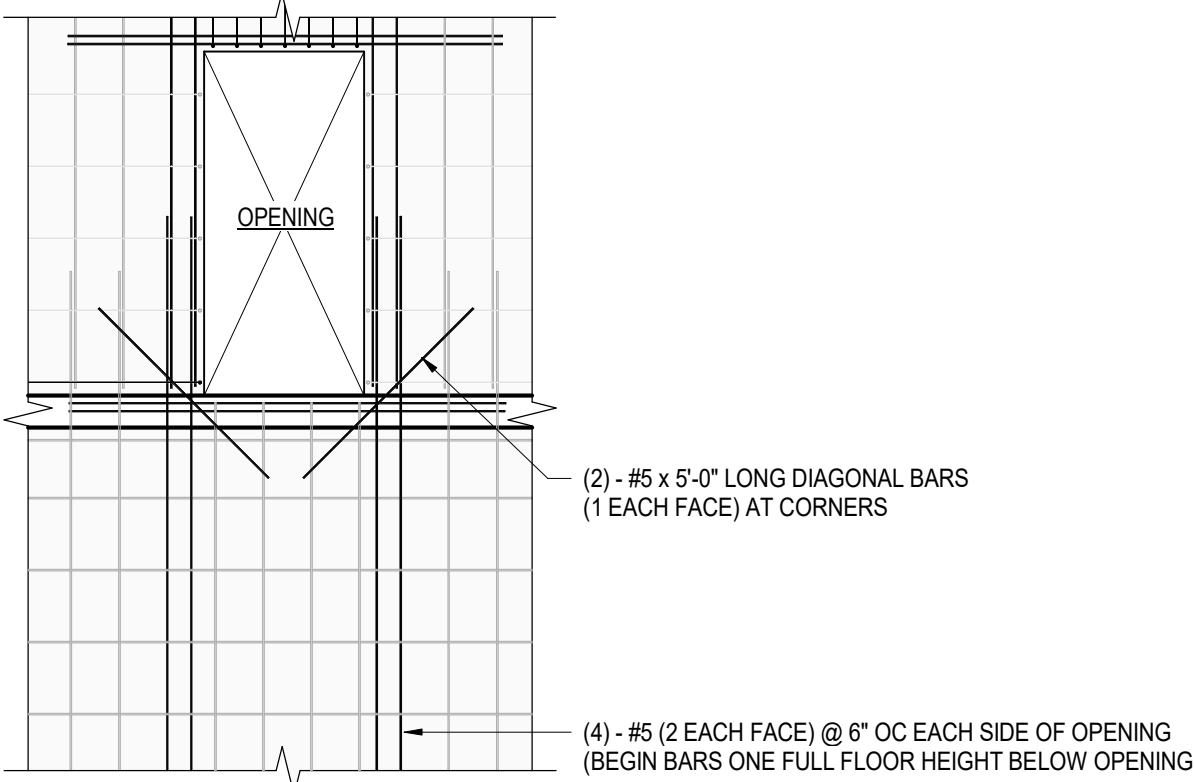
6 TYPICAL CONCRETE SLAB-TO-SHEAR WALL CONNECTIONS - SECTION
1" = 1'-0"



TYPICAL STACKED OPENINGS



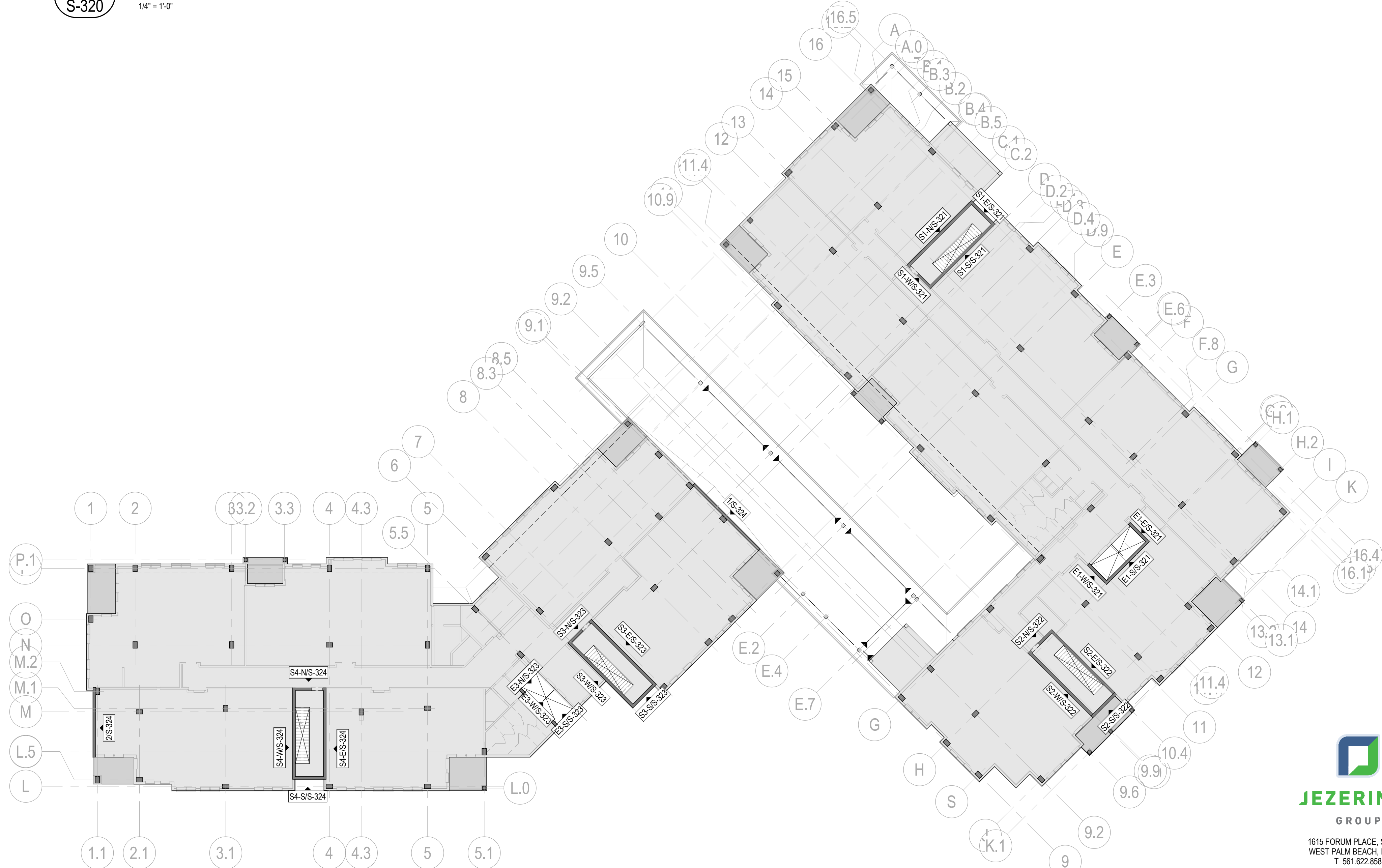
DISCONTINUOUS OPENING ALIGNMENT - NO OPENING ABOVE
(UNLESS NOTED OTHERWISE ON SHEAR WALL ELEVATIONS)



DISCONTINUOUS OPENING ALIGNMENT - NO OPENING BELOW
(UNLESS NOTED OTHERWISE ON SHEAR WALL ELEVATIONS)

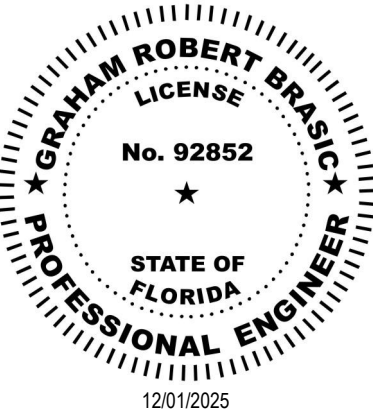
- NOTES:
- SEE SHEAR WALL ELEVATIONS AND SCHEDULE FOR LINK BEAM REQUIREMENTS. WHEN NO LINK BEAM IS INDICATED, PROVIDE (4) - #7 TOP & BOTTOM BARS WITH #3 DOUBLE U-BAR STIRRUPS WITH SPACING TO MATCH TYPICAL VERTICAL WALL REINFORCEMENT

5 BAR ARRANGEMENT AT OPENING (UNO)
1/4" = 1'-0"



SW SHEAR WALL KEY PLAN
1" = 20'-0"

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Date: 12/01/2025

TYPICAL
CONCRETE
SHEAR WALL
DETAILS

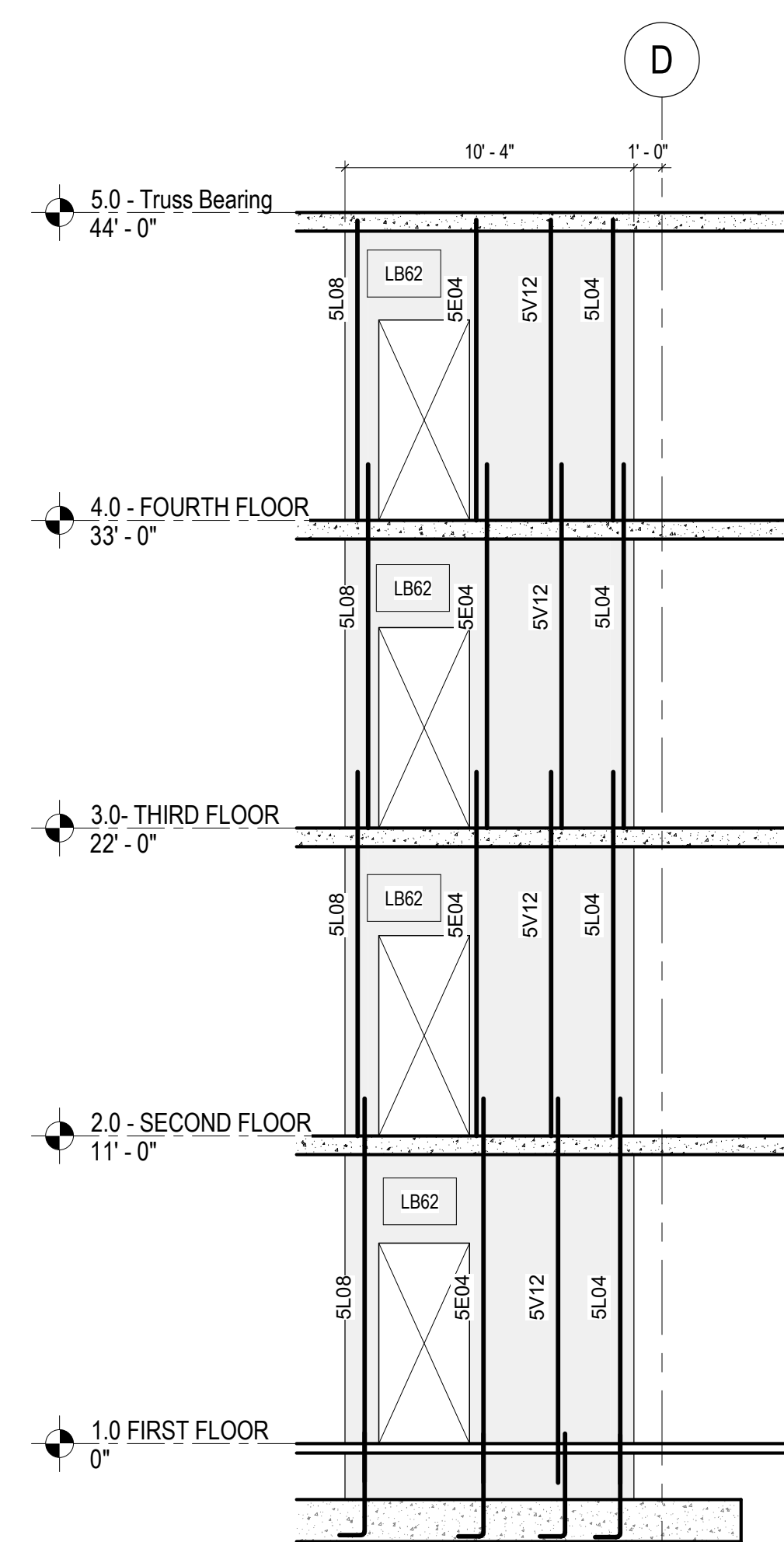
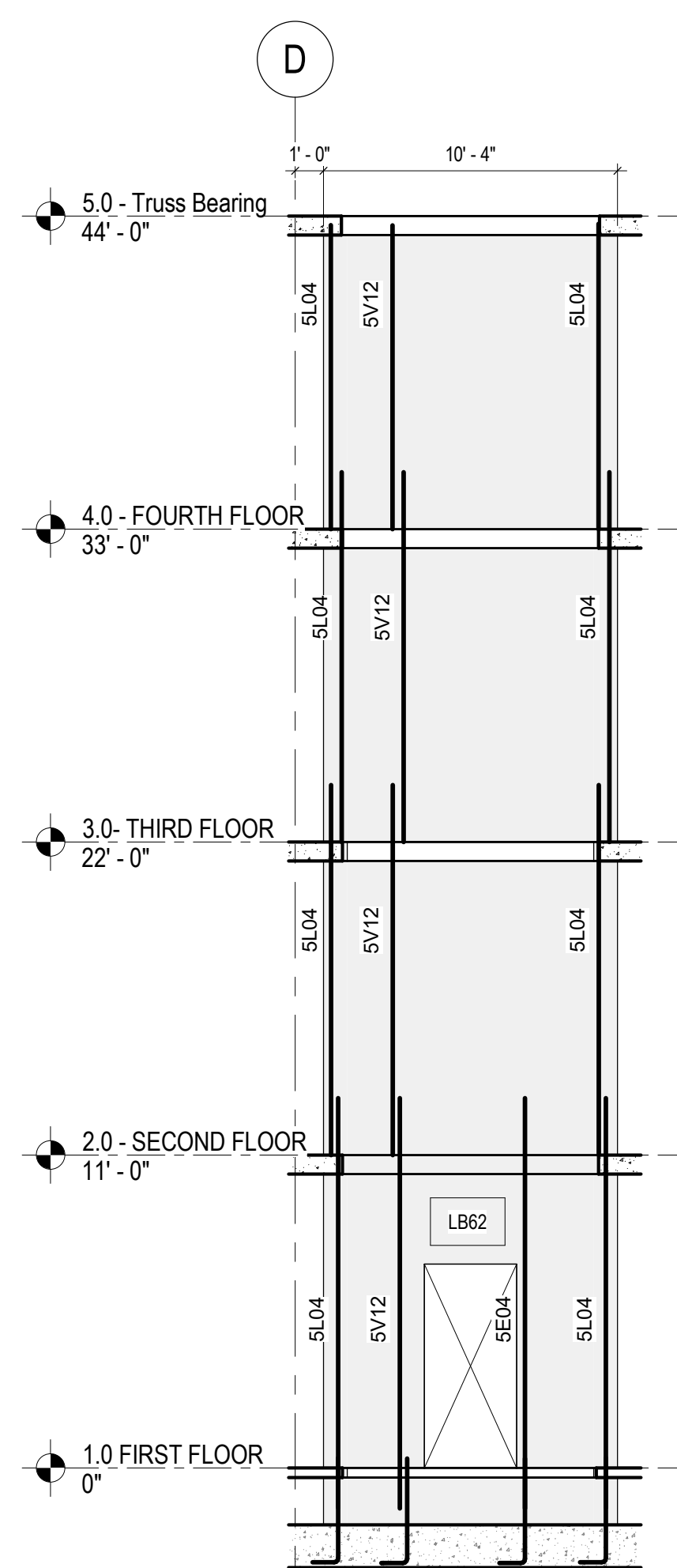
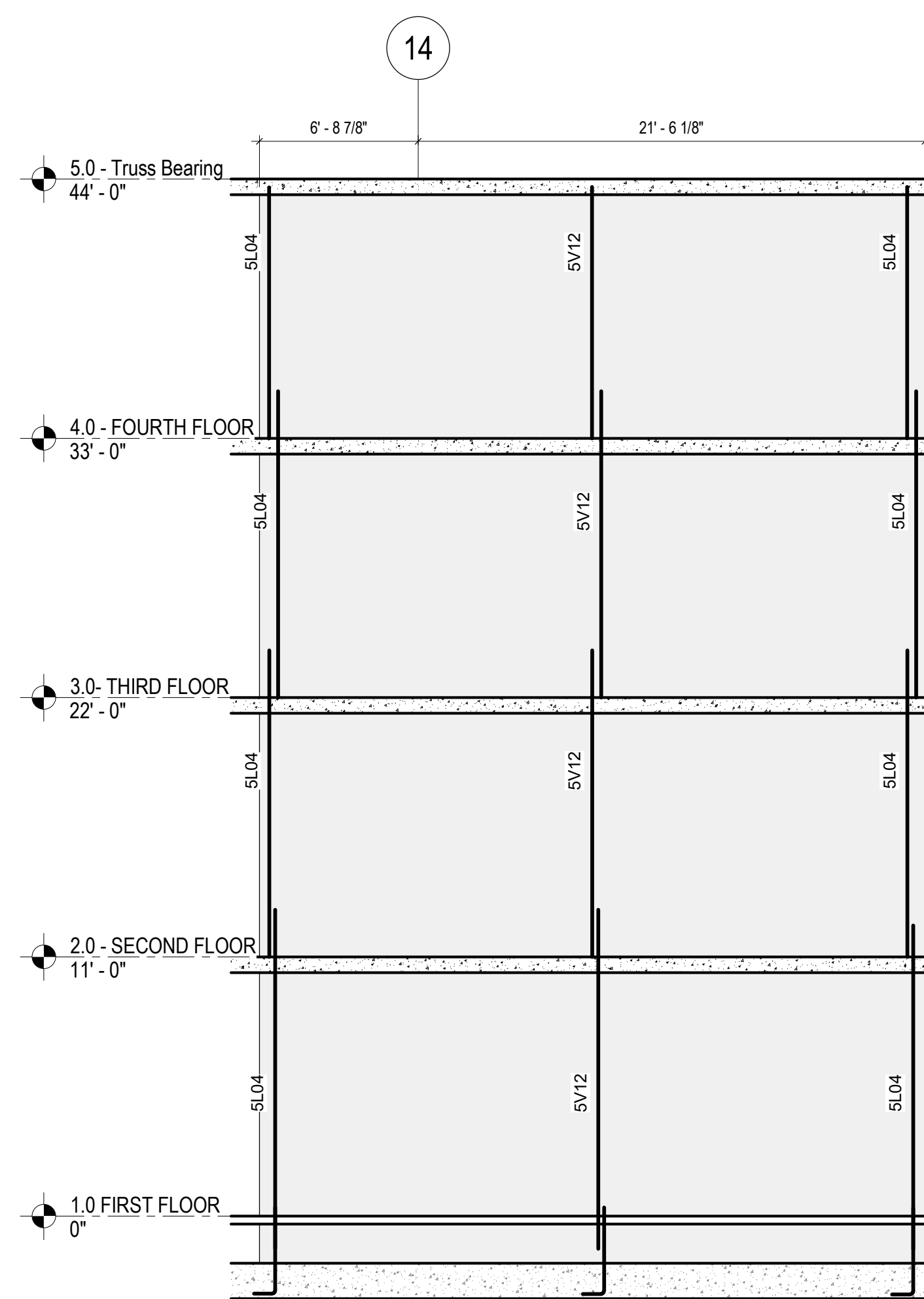
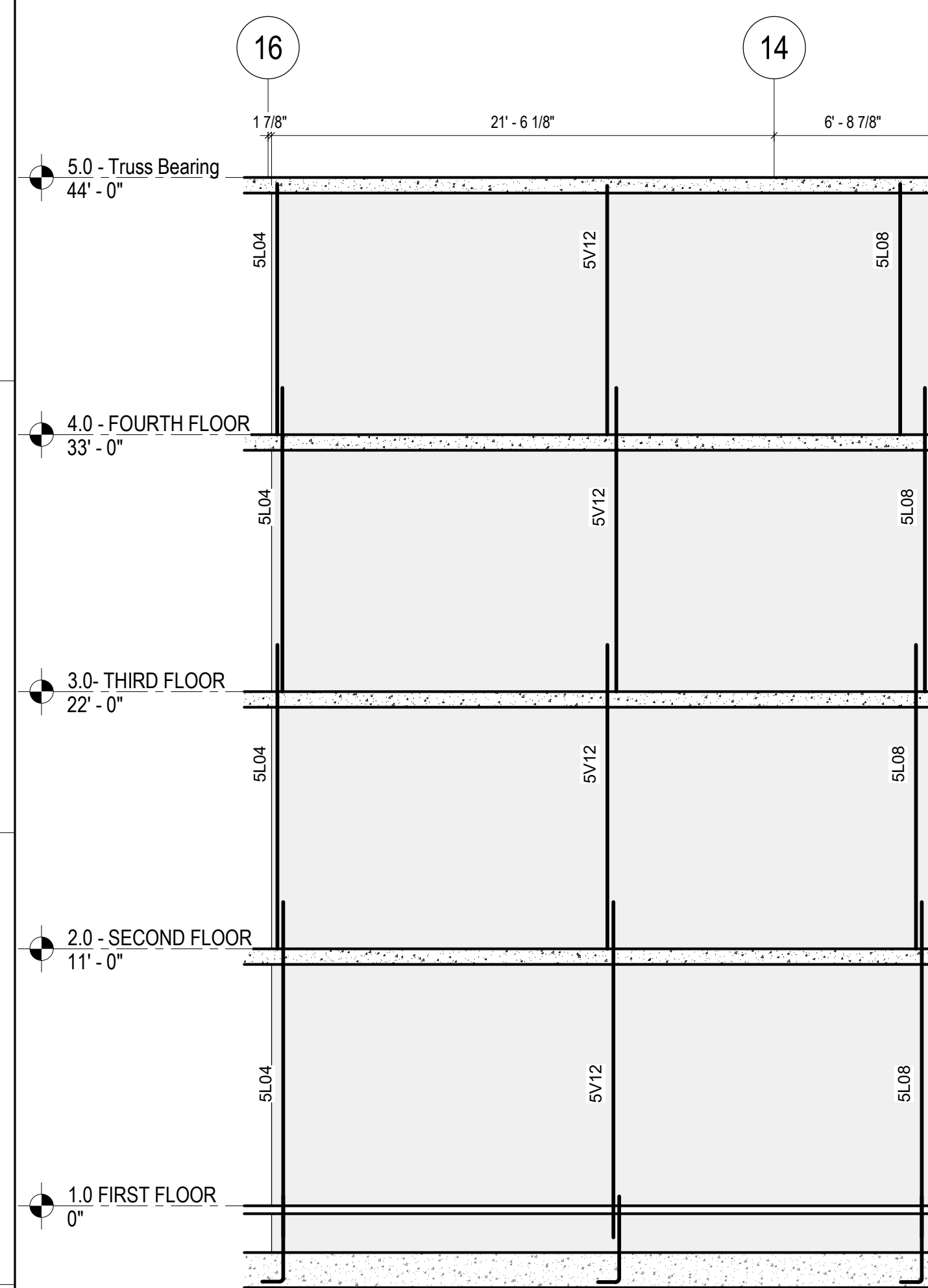
S-320



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12/1/2025 6:04:51 PM



SHEAR WALL ELEVATION NOTES:

1.  DENOTES 10" WALL THICKNESS
 DENOTES 12 WALL THICKNESS
2. SEE ARCHITECTURAL DRAWING FOR ROUGH OPENING DIMENSIONS
3. REBAR WALL REINFORCEMENT TAGS:
 - * #VAR# DENOTES TYPICAL VERTICAL REINFORCEMENT (SEE SCHEDULE ON THIS SHEET)
 - * #E#F# DENOTES VERTICAL END ZONE REINFORCEMENT (SEE SCHEDULE ON THIS SHEET)
 - * #H#F# DENOTES TYPICAL HORIZONTAL REINFORCEMENT (#H12 UNLESS NOTED OTHERWISE - (SEE SCHEDULE ON THIS SHEET)
 - * CB#F# DENOTES COUPLING BEAM MARK (SEE SCHEDULE ON THIS SHEET)
4. ALL BARS SHALL BE FULLY LAPPED, INCLUDING DOWELS INTO FOUNDATION ELEMENTS.
5. SEE DETAILS ON S-320.
6. GC SHALL VERIFY ALL DIMENSIONS AND LOCATIONS OF WALLS AND OPENINGS WITH ARCHITECTURAL DRAWINGS. NOTIFY DESIGN TEAM OF DISCREPANCIES.

TYPICAL HORIZONTAL REINFORCING		
MARK	BAR SIZE	BAR SPACING
5H12	#5	12"

<u>TYPICAL VERTICAL REINFORCING</u>		
MARK	BAR SIZE	BAR SPACING
5V12	#5	12"

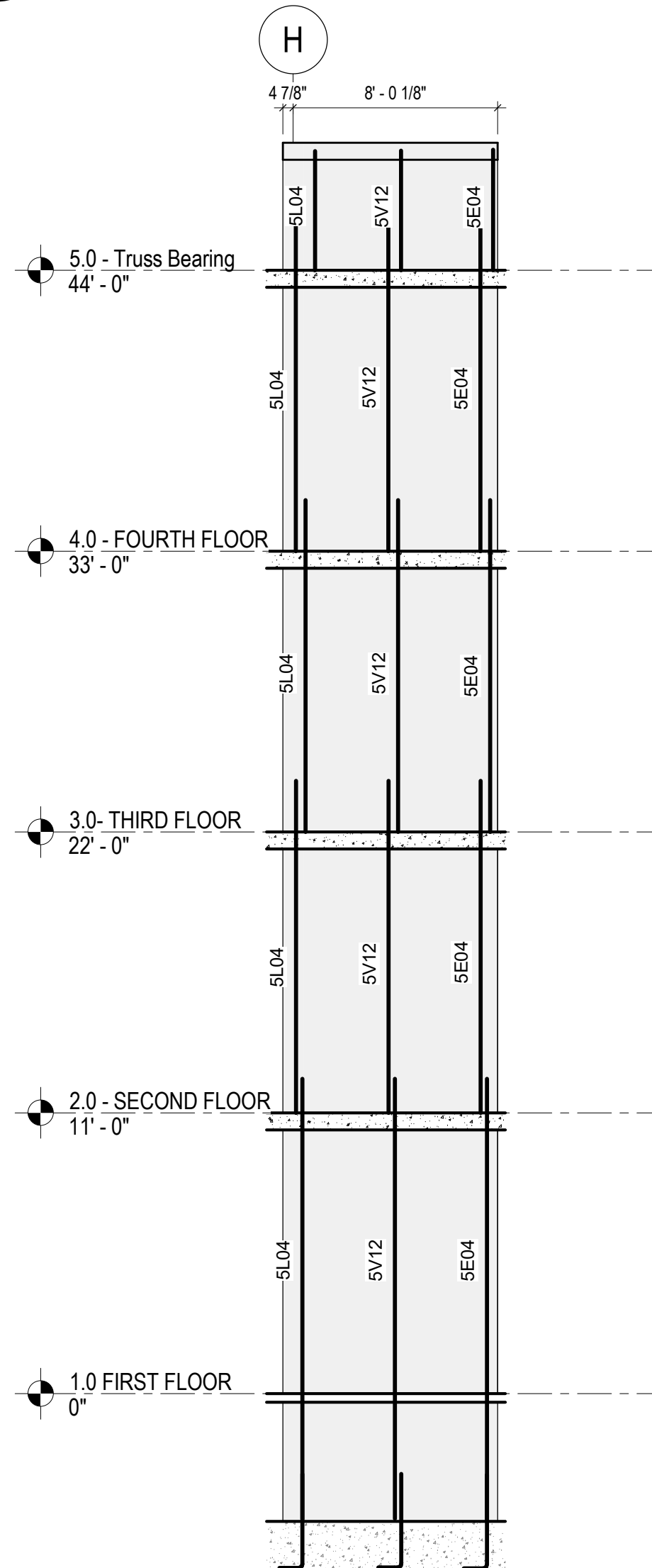
<u>VERTICAL CORNER REINFORCING</u>		
MARK	BAR SIZE	# BARS (1/2 EF)
5L04	#5	4
5L08	#5	8

<u>VERTICAL END REINFORCING</u>		
MARK	BAR SIZE	# BARS (1/2 EF)
5E04	#5	4

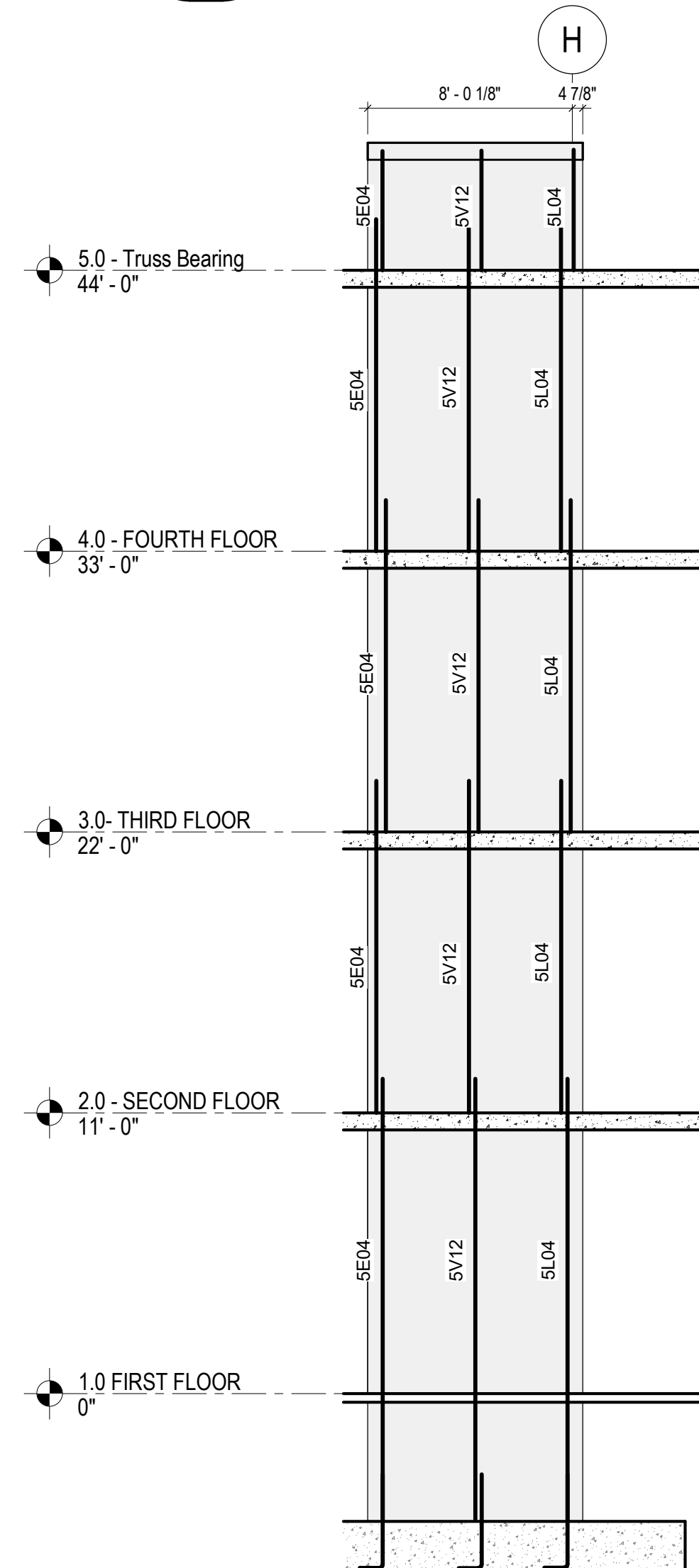
<u>LINK BEAM SCHEDULE</u>			
MARK	REINFORCEMENT		
	LONGITUDINAL	STIRRUPS	SKIN
LB62	(2) - #6 (T&B)	#4 @ 12" OC	SEE NOTE 1

NOTES:
1. #5 @ 9" OC (EACH FACE) SKIN REINFORCEMENT SHALL BE PROVIDED FOR ALL LINK BEAMS GREATER THAN 36" DEEP, FULL-HEIGHT OF LINK BEAM.

STAIR 1 NORTH WALL ELEVATION



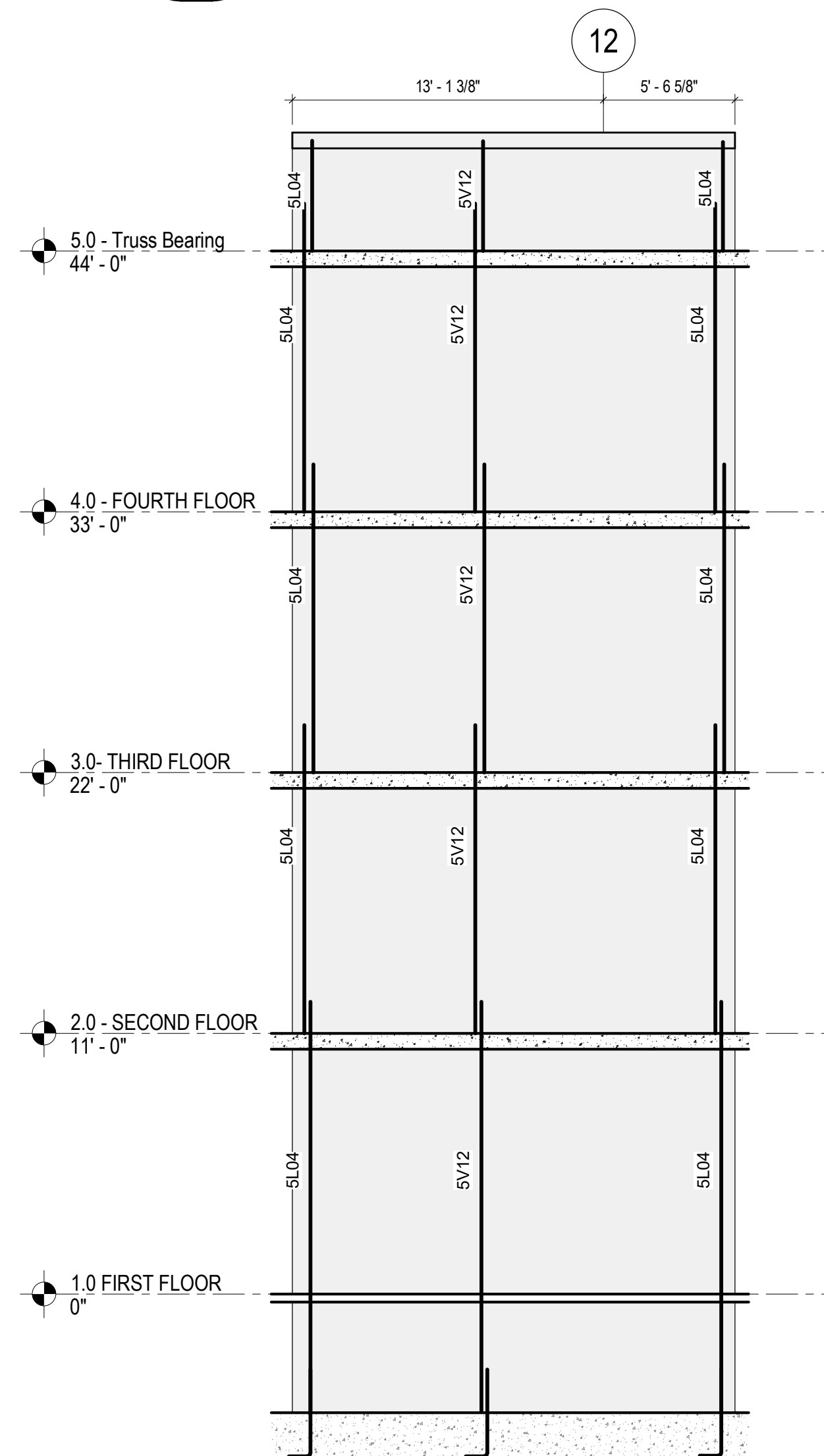
STAIR 1 SOUTH WALL ELEVATION



S1-E
S-321

STAIR 1 EAST WALL ELEVATION

3/16" = 1'-0"



S1-W
S-321

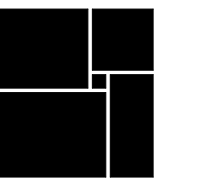
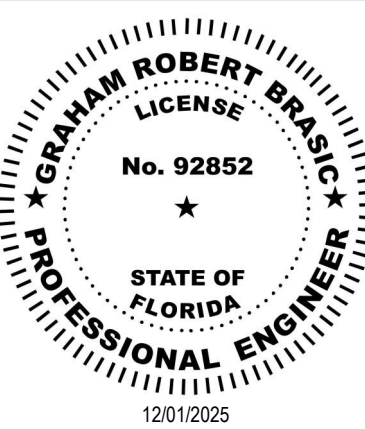
STAIR 1 WEST WALL ELEVATION

3/16" = 1'-0"

E1-E
S-321 **ELEVATOR 1 EAST WALL ELEVATION**
3/16" = 1'-0"

E1-W
S-321 **ELEVATOR 1 WEST WALL ELEVATION**
3/16" = 1'-0"

E1-S ELEVATOR 1 SOUTH WALL ELEVATION
S-321 3/16" = 1'-0"



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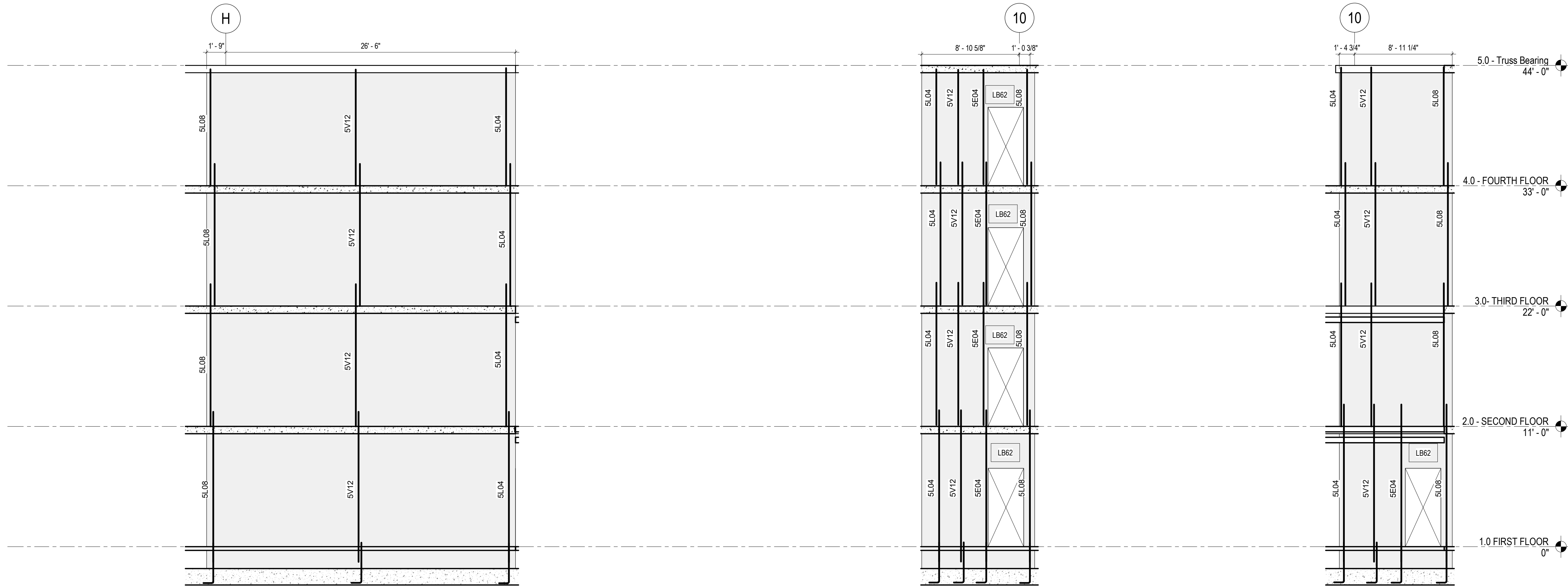
**CONCRETE
SHEAR WALL
ELEVATIONS -
AREA A**

S-321



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- SHEAR WALL ELEVATION NOTES:**
1. DENOTES 10" WALL THICKNESS
 DENOTES 12" WALL THICKNESS
 2. SEE ARCHITECTURAL DRAWING FOR ROUGH OPENING DIMENSIONS
 3. SHEAR WALL REINFORCEMENT TAGS:
 - #V## DENOTES TYPICAL VERTICAL REINFORCEMENT (SEE SCHEDULE ON THIS SHEET)
 - #E## DENOTES TYPICAL VERTICAL END ZONE REINFORCEMENT (SEE SCHEDULE ON THIS SHEET)
 - #H## DENOTES TYPICAL HORIZONTAL REINFORCEMENT (SH12 UNLESS NOTED OTHERWISE - (SEE SCHEDULE ON THIS SHEET))
 - CB## DENOTES COUPLING BEAM, MARK (SEE SCHEDULE ON THIS SHEET)
 4. ALL BARS SHALL BE FULLY LAPPED, INCLUDING DOWELS INTO FOUNDATION ELEMENTS.
 5. SEE DETAILS ON S-320.
 6. GC SHALL VERIFY ALL DIMENSIONS AND LOCATIONS OF WALLS AND OPENINGS WITH ARCHITECTURAL DRAWINGS. NOTIFY DESIGN TEAM OF DISCREPANCIES.

TYPICAL HORIZONTAL REINFORCING		
MARK	BAR SIZE	BAR SPACING
SH12	#5	12"

TYPICAL VERTICAL REINFORCING		
MARK	BAR SIZE	BAR SPACING
SV12	#5	12"

VERTICAL CORNER REINFORCING		
MARK	BAR SIZE	# BARS (1/2 EF)
SL04	#5	4
SL08	#5	8

VERTICAL END REINFORCING		
MARK	BAR SIZE	# BARS (1/2 EF)
SE04	#5	4

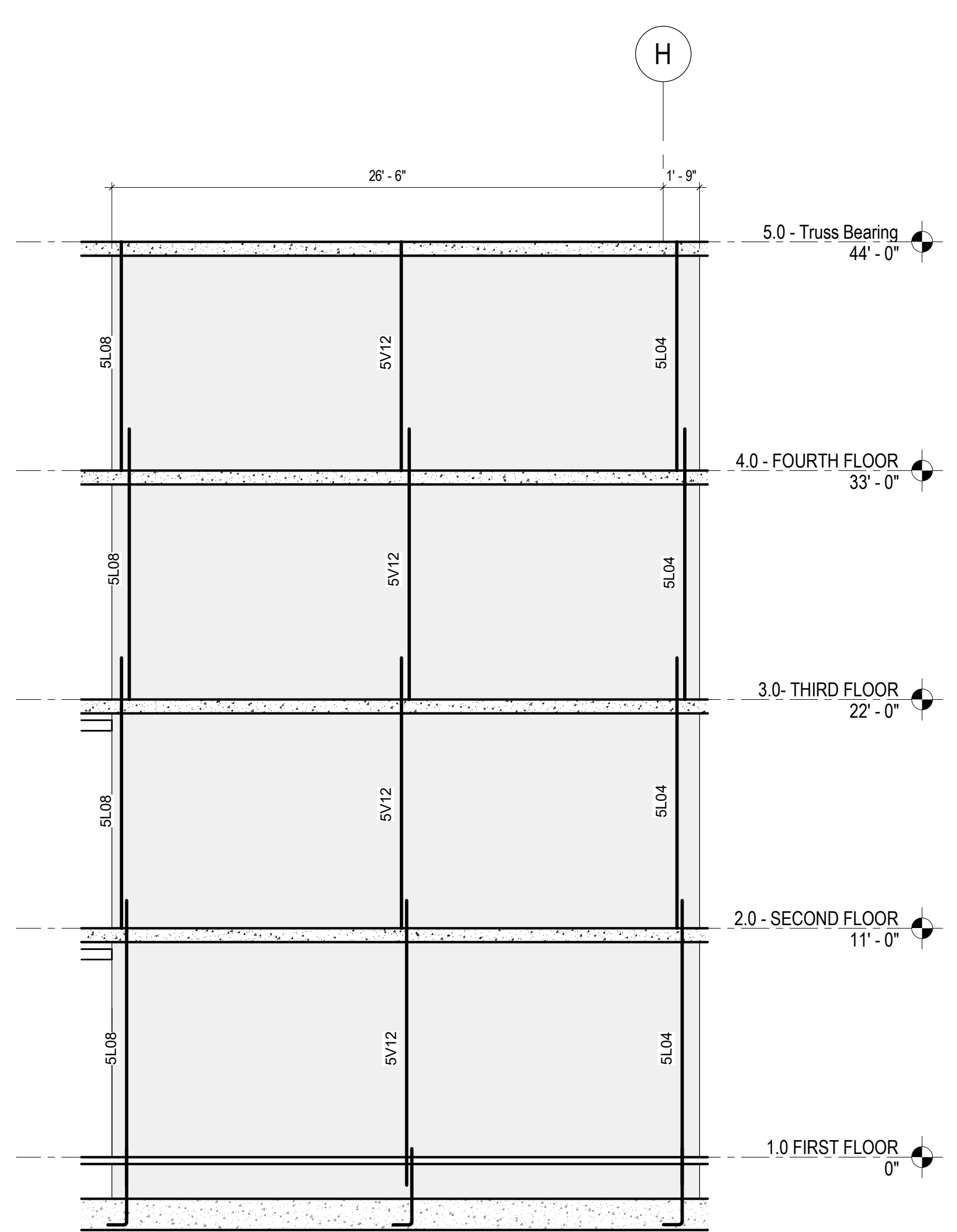
LINK BEAM SCHEDULE			
MARK	REINFORCEMENT		
	LONGITUDINAL	STIRRUPS	SKIN
LB62	(2) - #6 (T&B)	#4 @ 12" OC	SEE NOTE 1

NOTES:
1. #5 @ 9" OC (EACH FACE) SKIN REINFORCEMENT SHALL BE PROVIDED FOR ALL LINK BEAMS GREATER THAN 30" DEEP, FULL-HEIGHT OF LINK BEAM.

S2-W
S-322 STAIR 2 WEST WALL ELEVATION
3/16" = 1'-0"

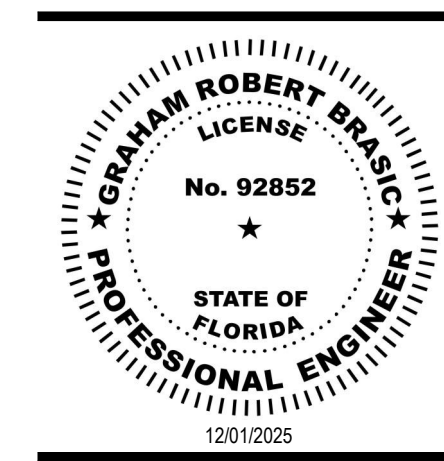
S2-N
S-322 STAIR 2 NORTH WALL ELEVATION
3/16" = 1'-0"

S2-S
S-322 STAIR 2 SOUTH WALL ELEVATION
3/16" = 1'-0"



S2-E
S-322 STAIR 2 EAST WALL ELEVATION
3/16" = 1'-0"

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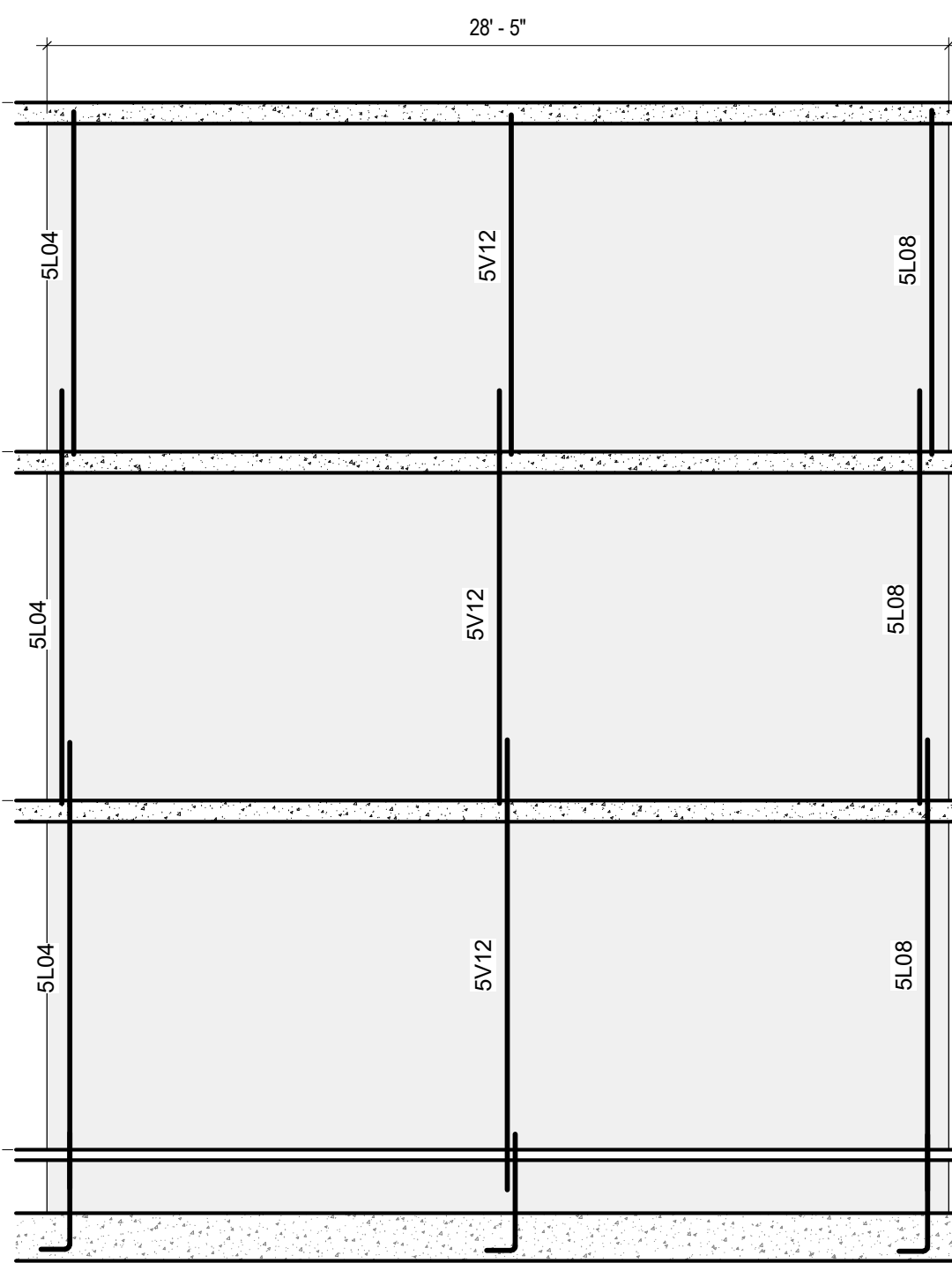
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**CONCRETE
SHEAR WALL
ELEVATIONS -
AREA A**

S-322

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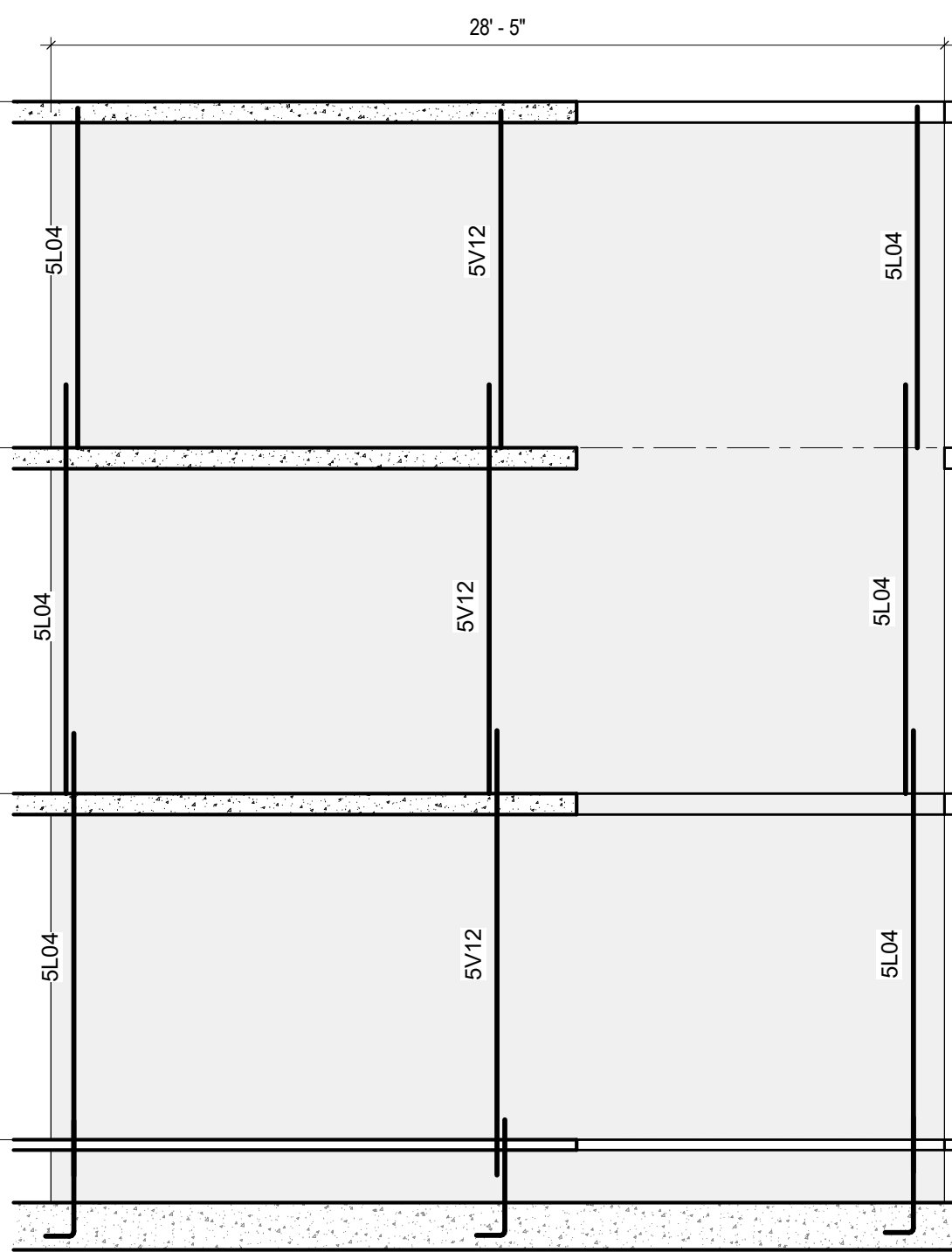
Autodesk Docs://The Waterford (Stage 2)/2021009_WCR_LL-BLDG_STRUCT_R24.rvt



S3-E
S-323

STAIR 3 EAST WALL ELEVATION

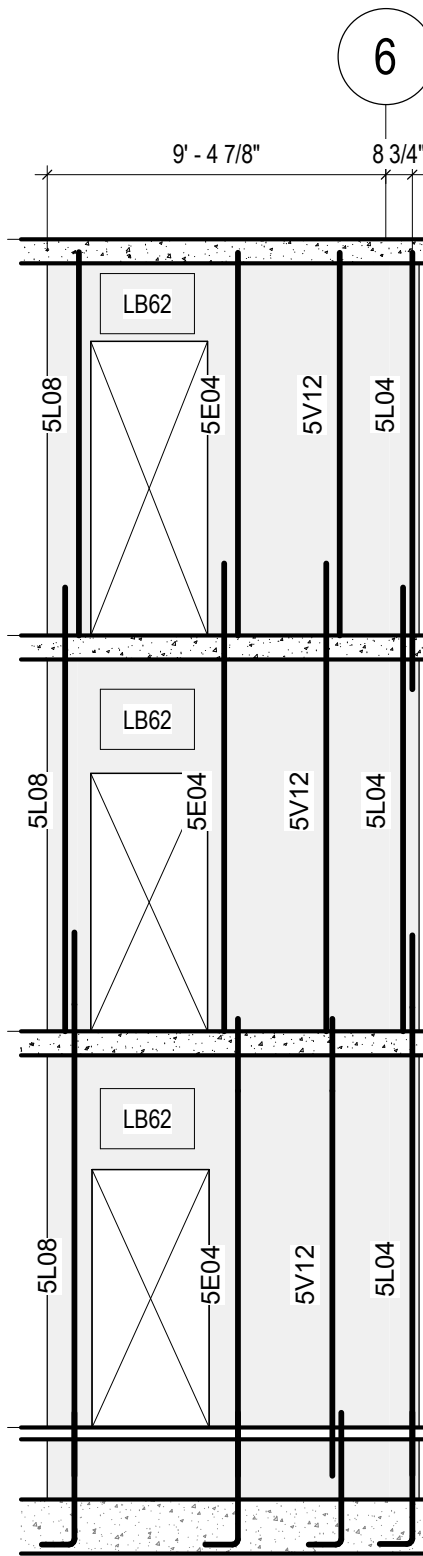
3/16" = 1'-0"



S3-W
S-323

STAIR 3 WEST WALL ELEVATION

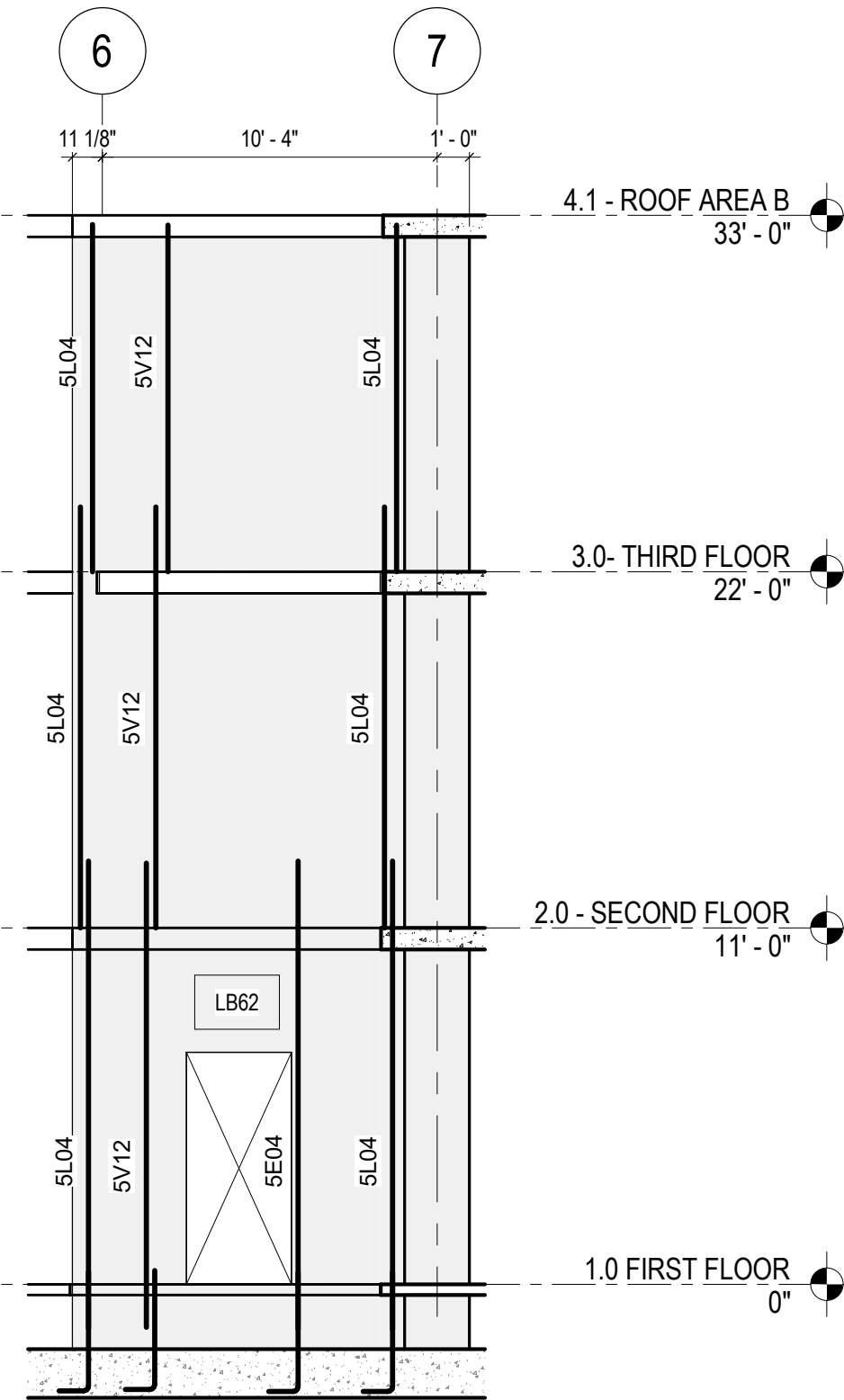
3/16" = 1'-0"



S3-N
S-323

STAIR 3 NORTH WALL ELEVATION

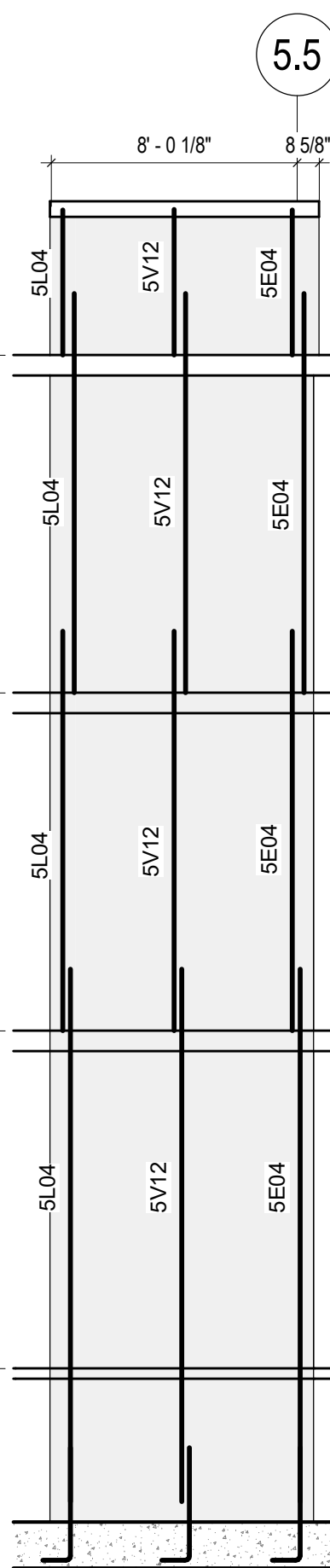
3/16" = 1'-0"



S3-S
S-323

STAIR 3 SOUTH WALL ELEVATION

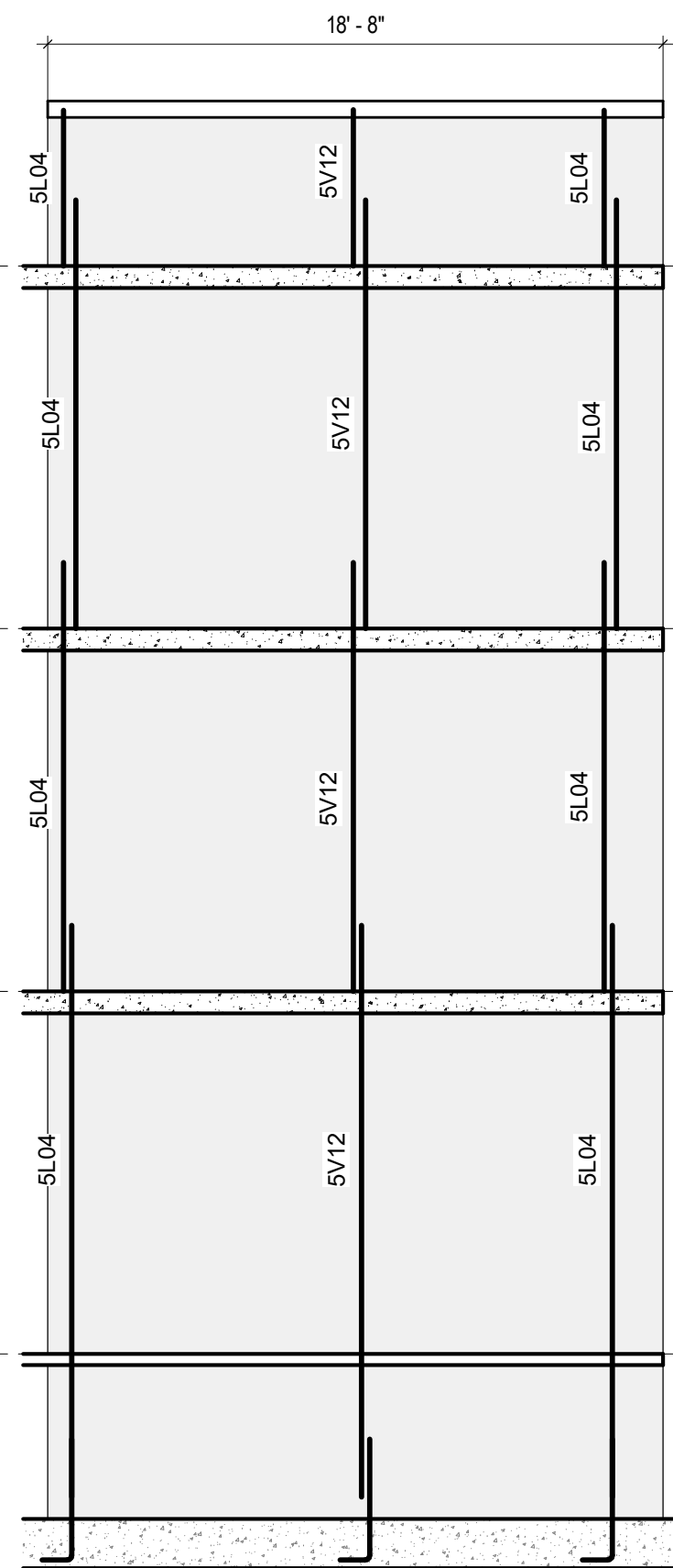
3/16" = 1'-0"



E3-S
S-323

ELEVATOR 3 SOUTH WALL ELEVATION

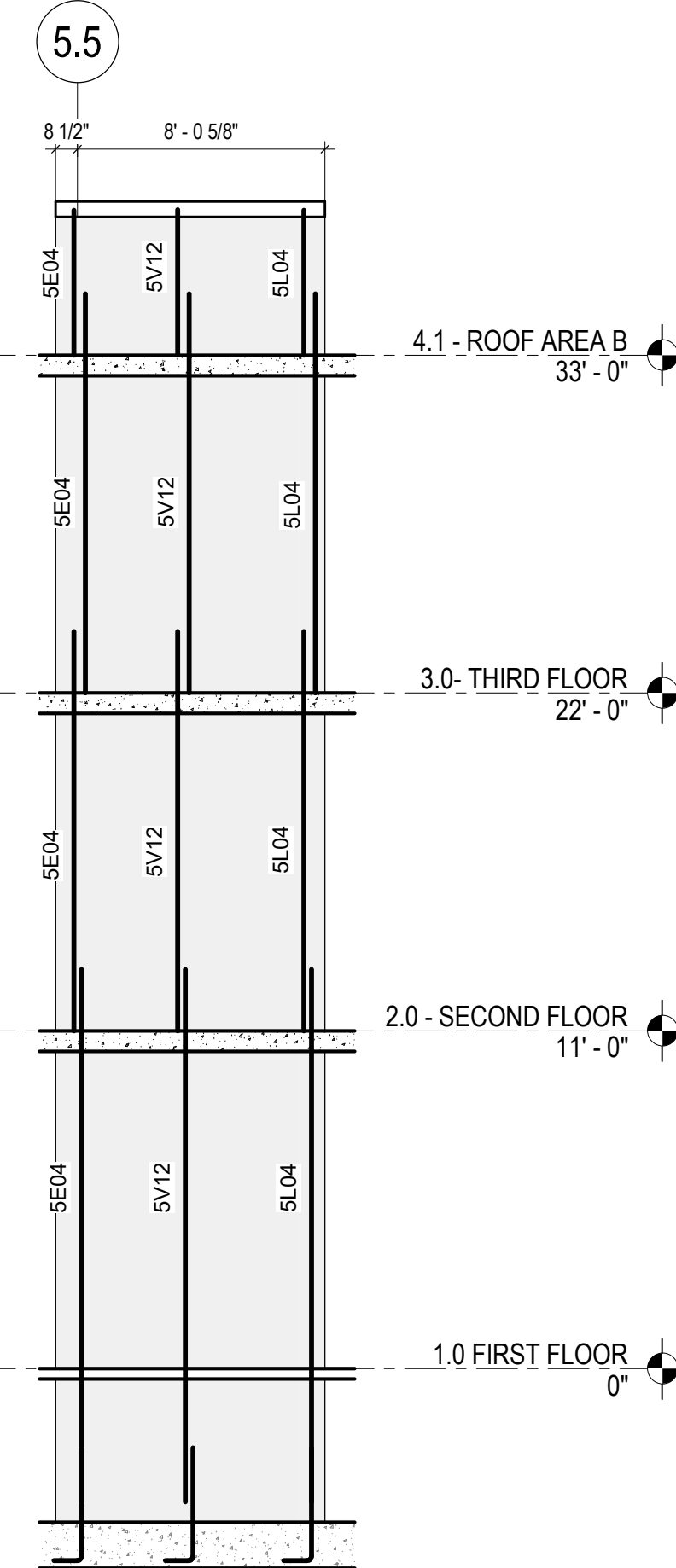
3/16" = 1'-0"



E3-W
S-323

ELEVATOR 3 WEST WALL ELEVATION

3/16" = 1'-0"



E3-N
S-323

ELEVATOR 3 NORTH WALL ELEVATION

3/16" = 1'-0"

SHEAR WALL ELEVATION NOTES:

1. DENOTES 10" WALL THICKNESS
DENOTES 12" WALL THICKNESS
2. SEE ARCHITECTURAL DRAWING FOR ROUGH OPENING DIMENSIONS
3. SHEAR WALL REINFORCEMENT TAGS:
 - #V## DENOTES TYPICAL VERTICAL REINFORCEMENT (SEE SCHEDULE ON THIS SHEET)
 - #E## DENOTES TYPICAL END ZONE REINFORCEMENT (SEE SCHEDULE ON THIS SHEET)
 - #H## DENOTES TYPICAL HORIZONTAL REINFORCEMENT (SH12 UNLESS NOTED OTHERWISE - (SEE SCHEDULE ON THIS SHEET))
 - CB## DENOTES COUPLING BEAM, MARK (SEE SCHEDULE ON THIS SHEET)
4. ALL BARS SHALL BE FULLY LAPPED, INCLUDING DOWELS INTO FOUNDATION ELEMENTS.
5. SEE DETAILS ON S-320.
6. GC SHALL VERIFY ALL DIMENSIONS AND LOCATIONS OF WALLS AND OPENINGS WITH ARCHITECTURAL DRAWINGS. NOTIFY DESIGN TEAM OF DISCREPANCIES.

TYPICAL HORIZONTAL REINFORCING		
MARK	BAR SIZE	BAR SPACING
SH12	#5	12"

TYPICAL VERTICAL REINFORCING		
MARK	BAR SIZE	BAR SPACING
SV12	#5	12"

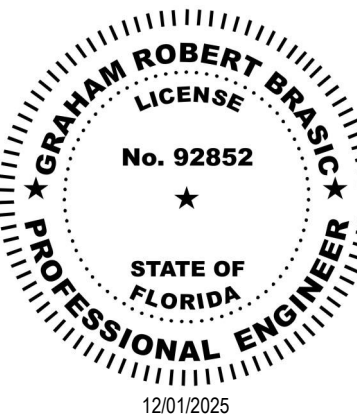
VERTICAL CORNER REINFORCING		
MARK	BAR SIZE	# BARS (1/2 EF)
SL04	#5	4
SL08	#5	8

VERTICAL END REINFORCING		
MARK	BAR SIZE	# BARS (1/2 EF)
SE04	#5	4

LINK BEAM SCHEDULE			
MARK	REINFORCEMENT		
	LONGITUDINAL	STIRRUPS	SKIN
LB62	(2) - #6 (T&B)	#4 @ 12" OC	SEE NOTE 1

NOTES:
1. #5 @ 9" OC (EACH FACE) SKIN REINFORCEMENT SHALL BE PROVIDED FOR ALL LINK BEAMS GREATER THAN 30" DEEP, FULL-HEIGHT OF LINK BEAM.

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Project No.: 2021009
Date: 12/01/2025

CONCRETE
SHEAR WALL
ELEVATIONS -
AREA B

S-323

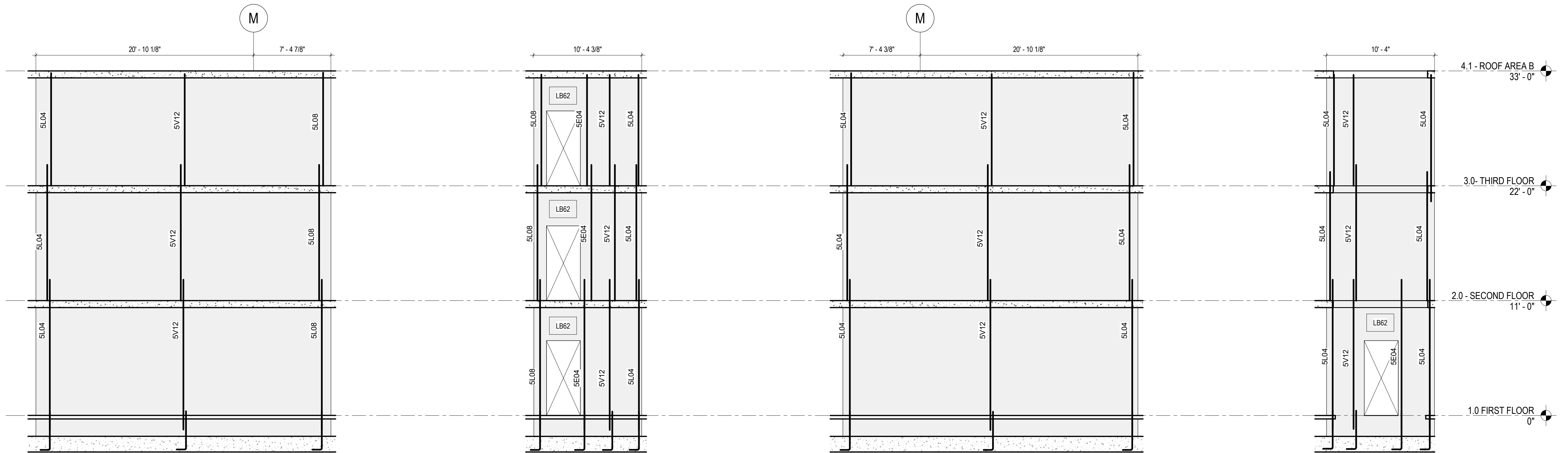


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S4-E
S-324
STAIR 4 EAST WALL ELEVATION
3/16" = 1'-0"

S4-N
S-324
STAIR 4 NORTH WALL ELEVATION
3/16" = 1'-0"

S4-W
S-324
STAIR 4 WEST WALL ELEVATION
3/16" = 1'-0"

S4-S
S-324
STAIR 4 SOUTH WALL ELEVATION
3/16" = 1'-0"

- SHEAR WALL ELEVATION NOTES:
1. DENOTES 10" WALL THICKNESS
DENOTES 12" WALL THICKNESS
 2. SEE ARCHITECTURAL DRAWING FOR ROUGH OPENING DIMENSIONS
 3. SHEAR WALL REINFORCEMENT TAGS:
 - #V## DENOTES TYPICAL VERTICAL REINFORCEMENT (SEE SCHEDULE ON THIS SHEET)
 - #E## DENOTES VERTICAL END ZONE REINFORCEMENT (SEE SCHEDULE ON THIS SHEET)
 - #H## DENOTES TYPICAL HORIZONTAL REINFORCEMENT (5H12 UNLESS NOTED OTHERWISE - (SEE SCHEDULE ON THIS SHEET))
 - CB## DENOTES COUPLING BEAM, MARK (SEE SCHEDULE ON THIS SHEET)
 4. ALL BARS SHALL BE FULLY LAPPED, INCLUDING DOWELS INTO FOUNDATION ELEMENTS.
 5. SEE DETAILS ON S-320.
 6. GC SHALL VERIFY ALL DIMENSIONS AND LOCATIONS OF WALLS AND OPENINGS WITH ARCHITECTURAL DRAWINGS. NOTIFY DESIGN TEAM OF DISCREPANCIES.

TYPICAL HORIZONTAL REINFORCING		
MARK	BAR SIZE	BAR SPACING
5H12	#5	12"

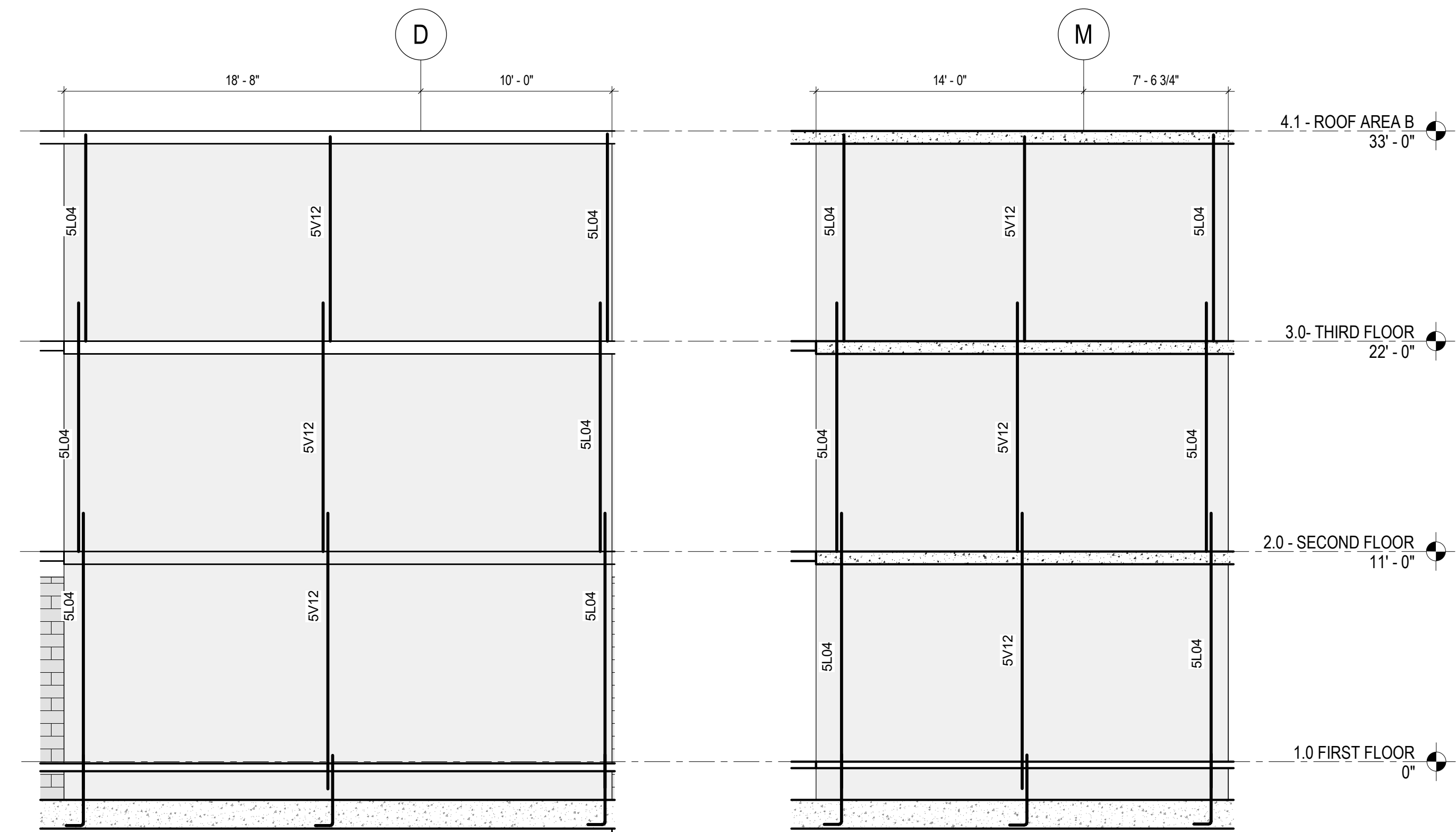
TYPICAL VERTICAL REINFORCING		
MARK	BAR SIZE	BAR SPACING
5V12	#5	12"

VERTICAL CORNER REINFORCING		
MARK	BAR SIZE	# BARS (1/2 EF)
5L04	#5	4
5L08	#5	8

VERTICAL END REINFORCING		
MARK	BAR SIZE	# BARS (1/2 EF)
5E04	#5	4

LINK BEAM SCHEDULE			
MARK	REINFORCEMENT		
	LONGITUDINAL	STIRRUPS	SKIN
LB62	(2) - #6 (T&B)	#4 @ 12" OC	SEE NOTE 1

NOTES:
1. #5 @ 9" OC (EACH FACE) SKIN REINFORCEMENT SHALL BE PROVIDED FOR ALL LINK BEAMS GREATER THAN 30" DEEP, FULL-HEIGHT OF LINK BEAM.



1
S-324
WALL ELEVATION A
3/16" = 1'-0"

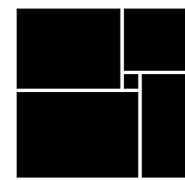
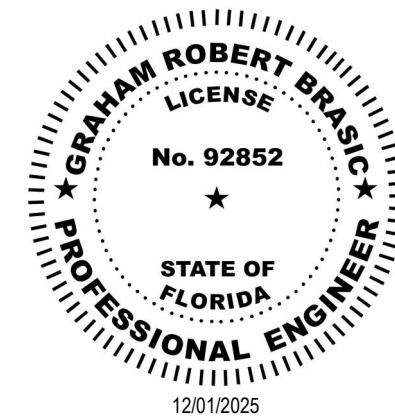
2
S-324
WALL ELEVATION M
3/16" = 1'-0"



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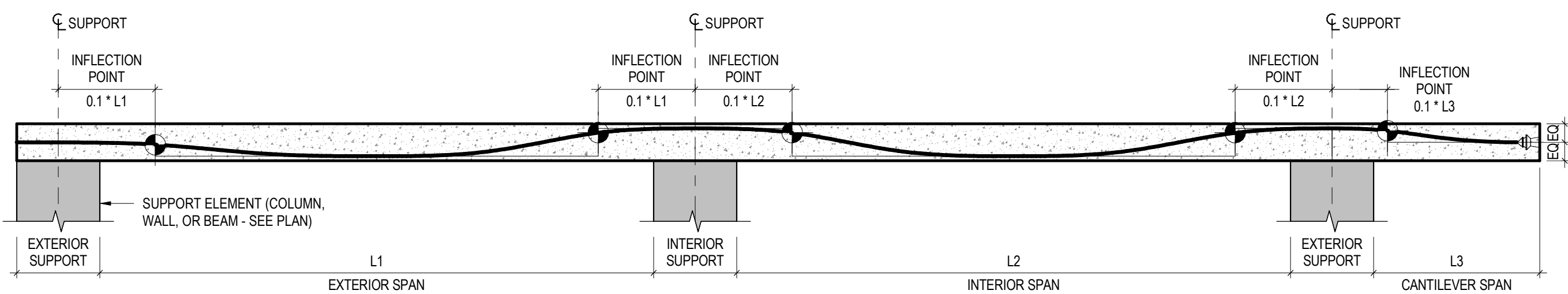
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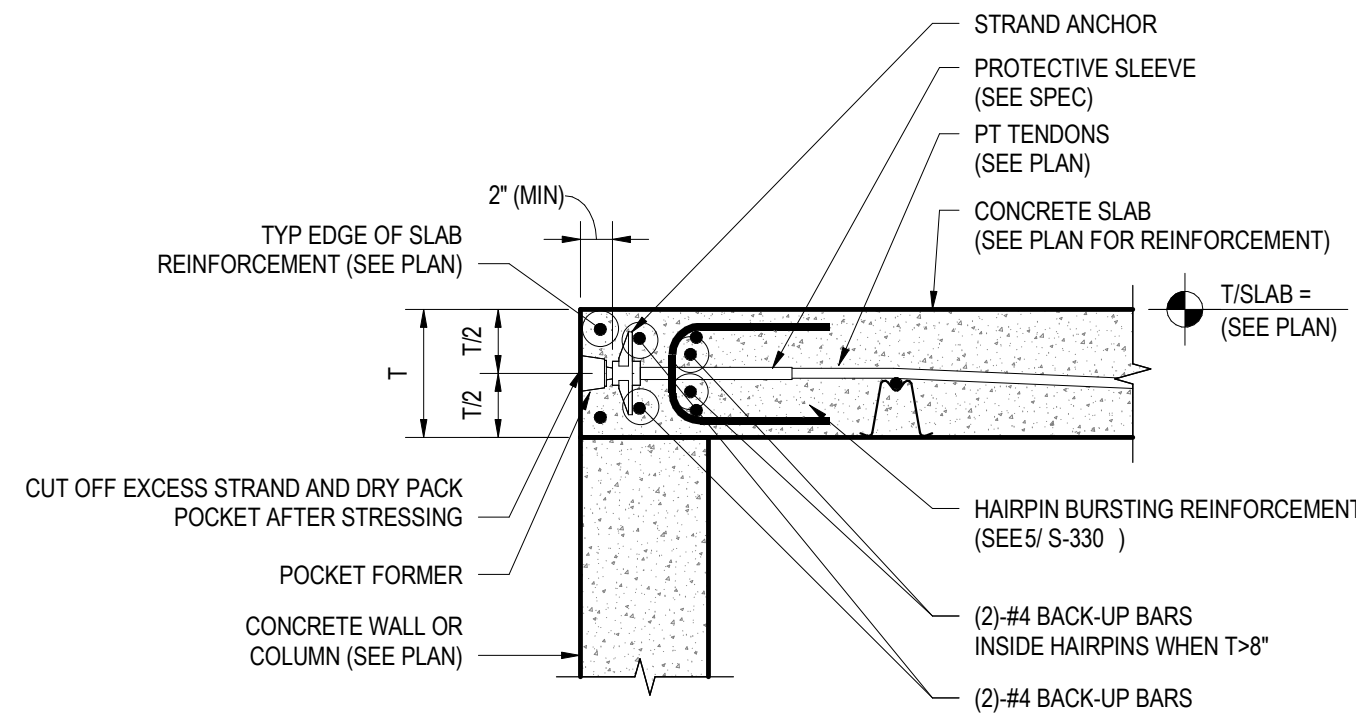
CONCRETE
SHEAR WALL
ELEVATIONS -
AREA B

S-324

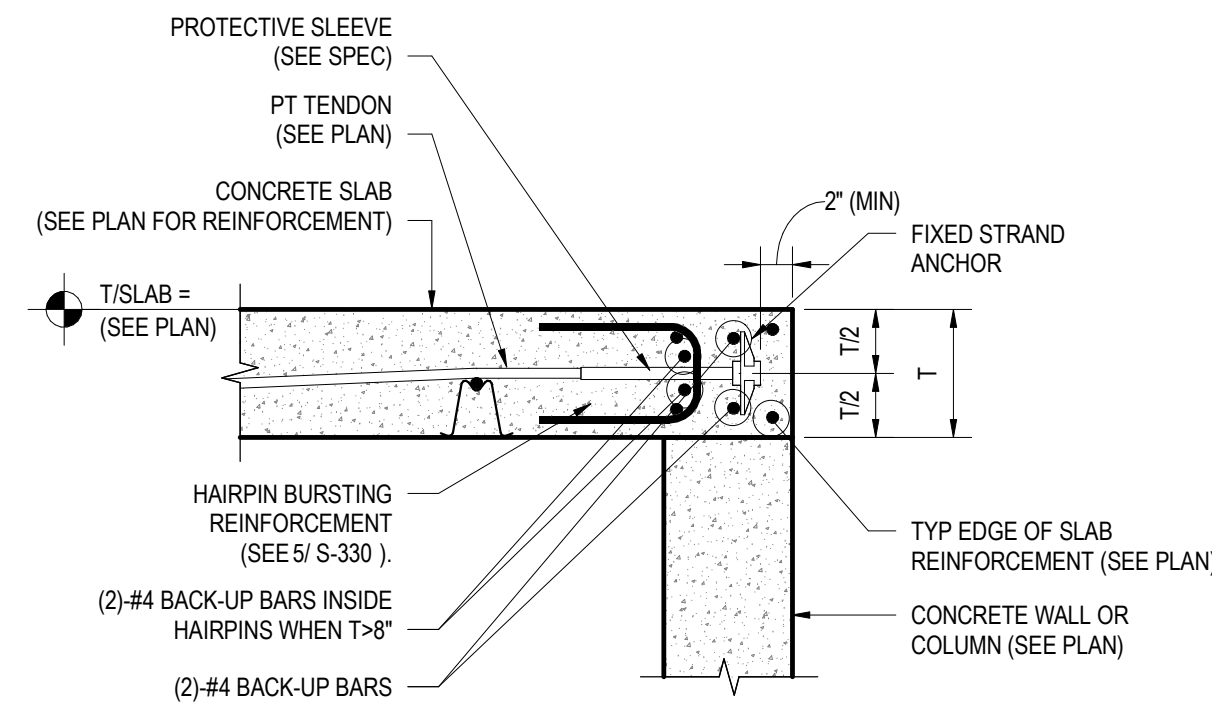
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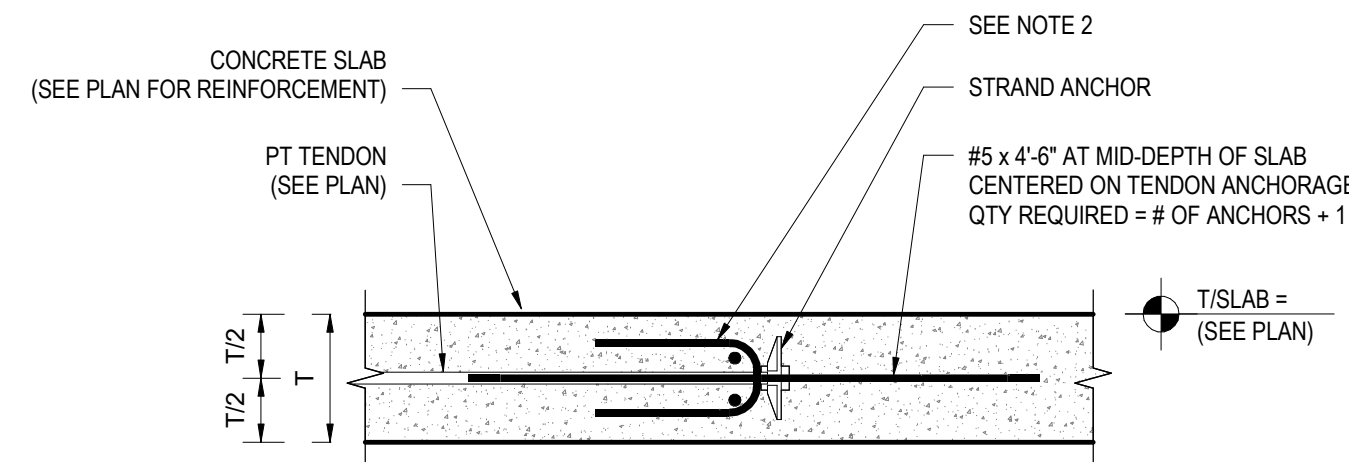
- NOTES:
- SEE PLANS FOR TENDON LAYOUT.
 - TENDON LOW POINTS ARE LOCATED MIDWAY BETWEEN HIGH POINTS UNLESS NOTED OTHERWISE.
 - TENDON LOW POINTS AT EXTERIOR SPANS ARE LOCATED 0.4L FROM CENTER LINE OF EXTERIOR SUPPORTS.
 - DRAPE TENDON IN PARABOLIC PROFILE BETWEEN LOW AND HIGH POINTS.
 - DROP PANELS, WHERE THEY OCCUR, ARE NOT SHOWN FOR CLARITY.



STRESSING END



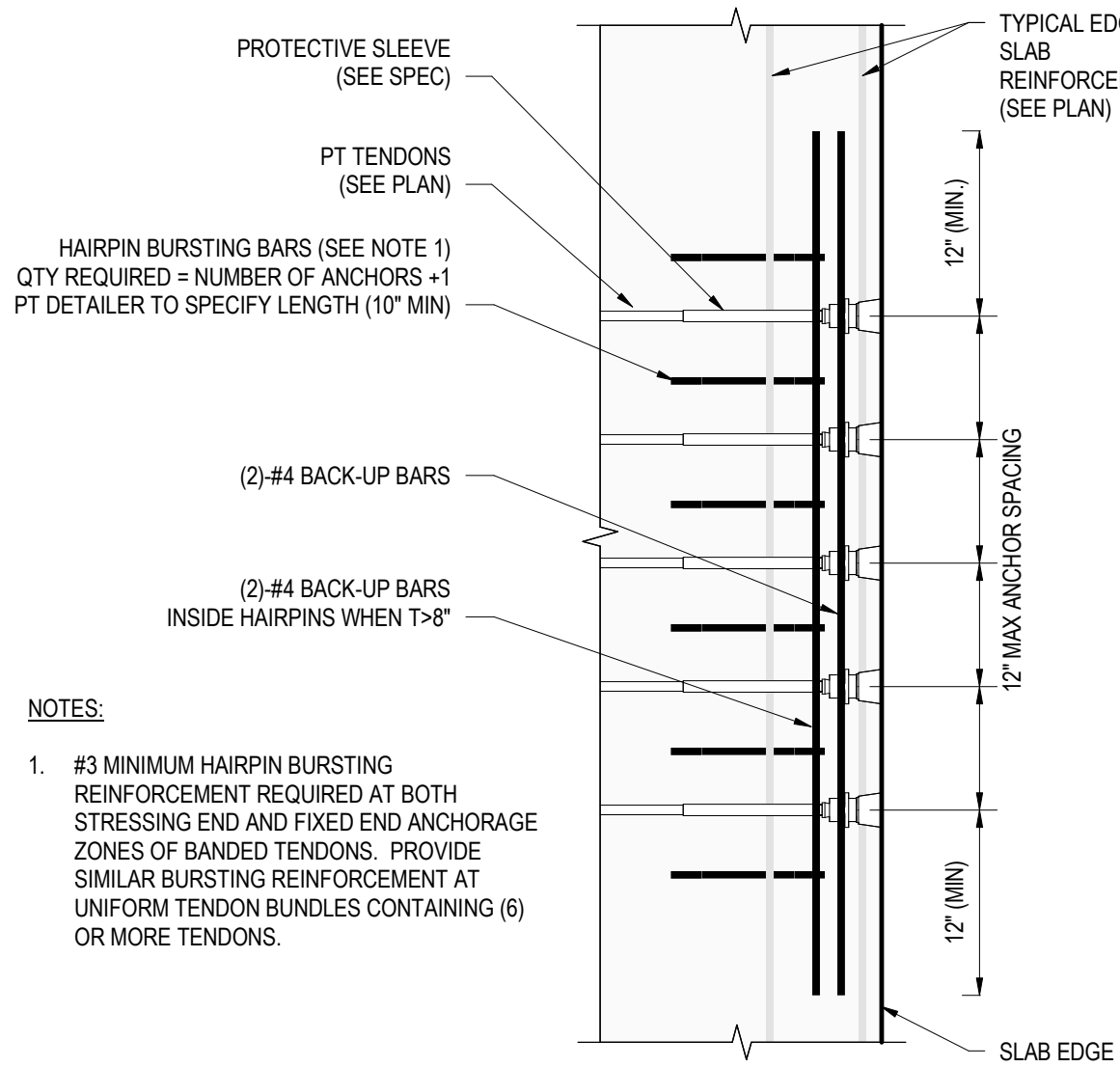
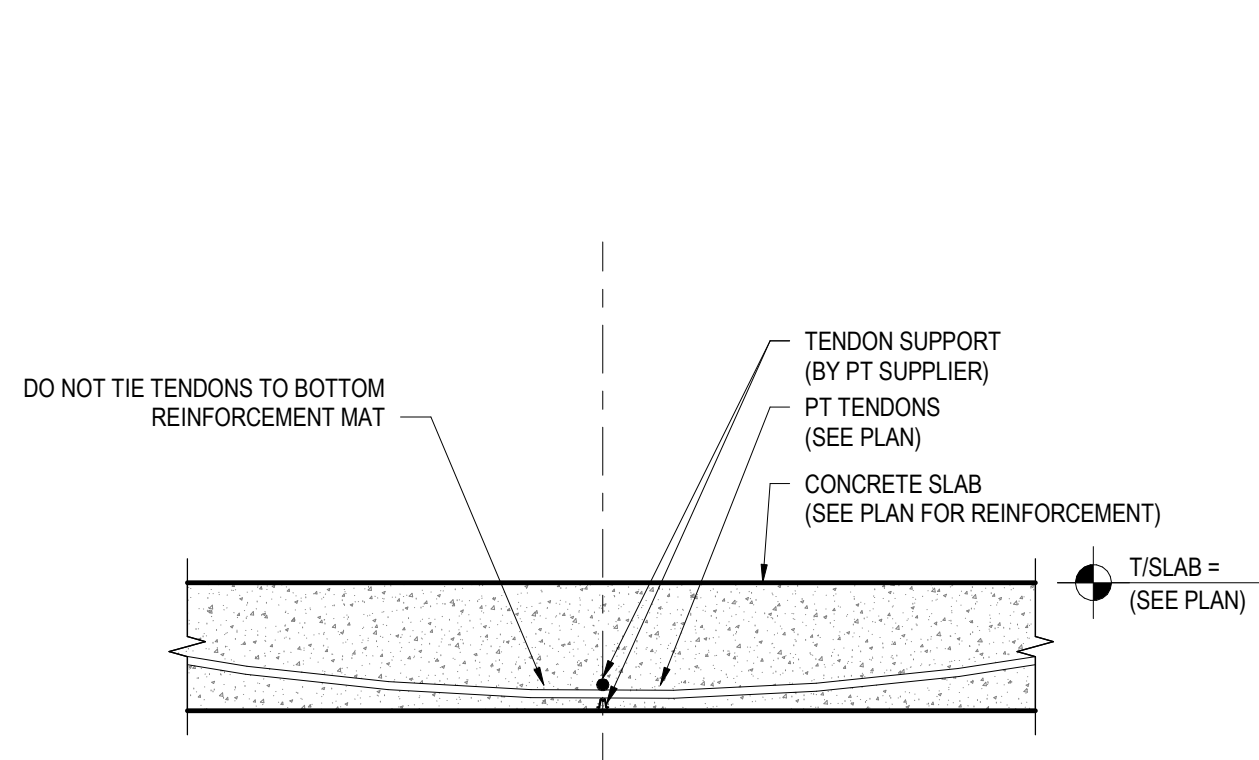
FIXED END



- NOTES:
- THIS DETAIL APPLIES TO ALL TENDON ANCHORAGES LOCATED AWAY FROM THE EDGE OF SLAB.
 - PROVIDE BACK UP BARS AND HAIRPIN BURSTING REINFORCEMENTS AS REQUIRED PER TYPICAL UNIFORMBANDED TENDON FIXED END DETAILS.

1 TYPICAL TENDON PROFILE

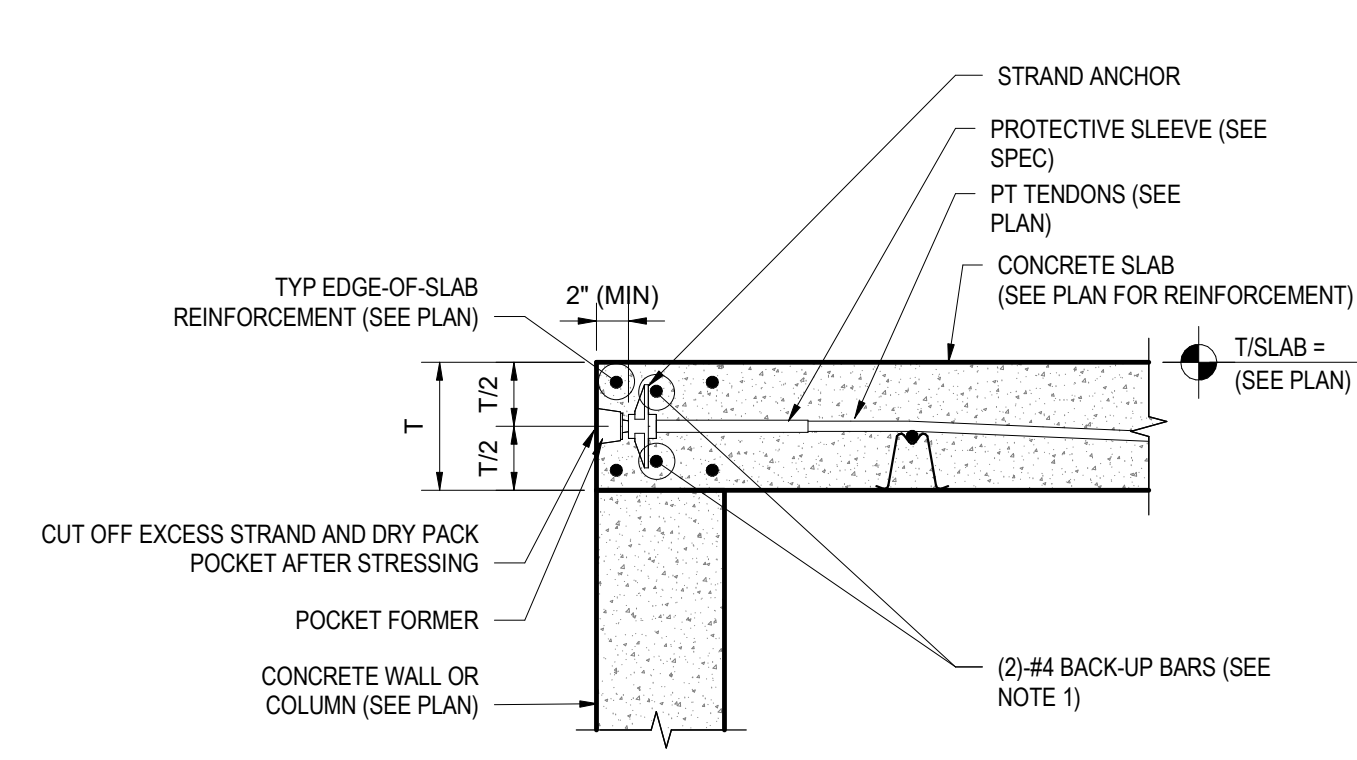
S-330 1/2" = 1'-0"



- NOTES:
- #3 MINIMUM HAIRPIN BURSTING REINFORCEMENT REQUIRED AT BOTH STRESSING END AND FIXED END ANCHORAGE ZONES OF BANDED TENDONS. PROVIDE SIMILAR BURSTING REINFORCEMENT AT UNIFORM TENDON BUNDLES CONTAINING (6) OR MORE TENDONS.

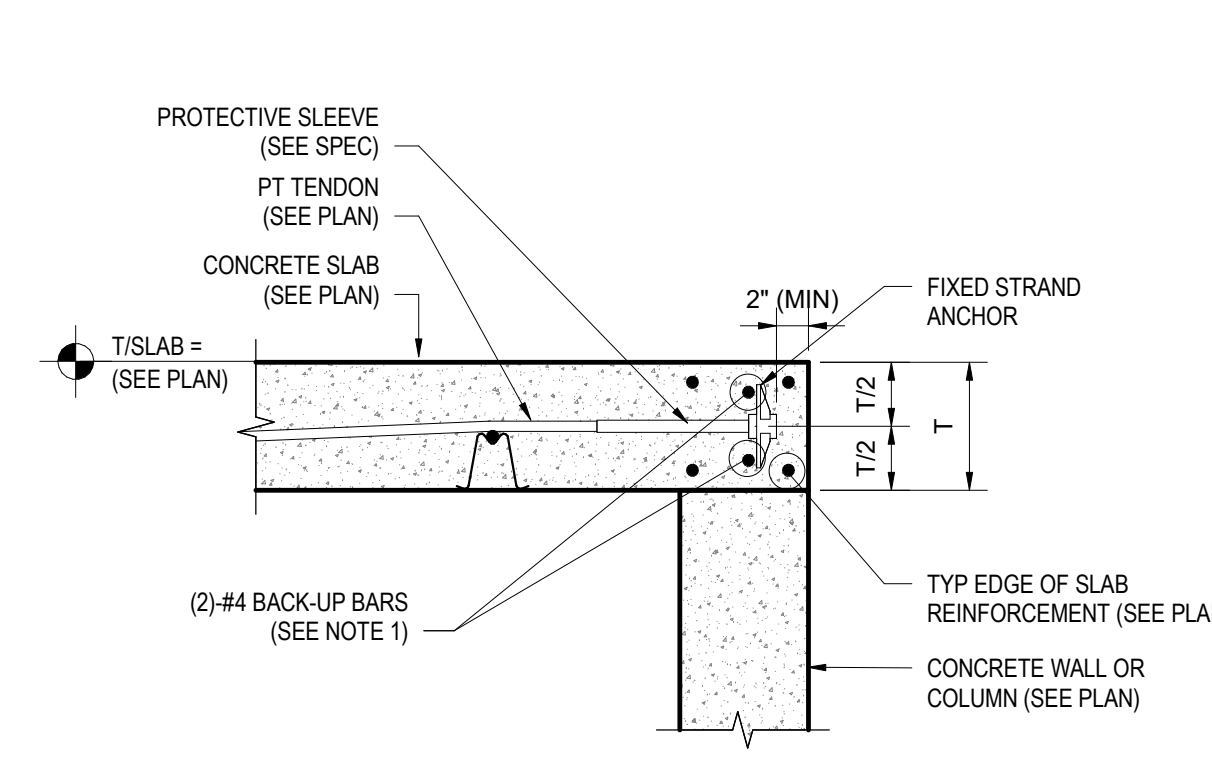
2 BANDED TENDON ANCHORAGE CONDITIONS

S-330 NTS



STRESSING END

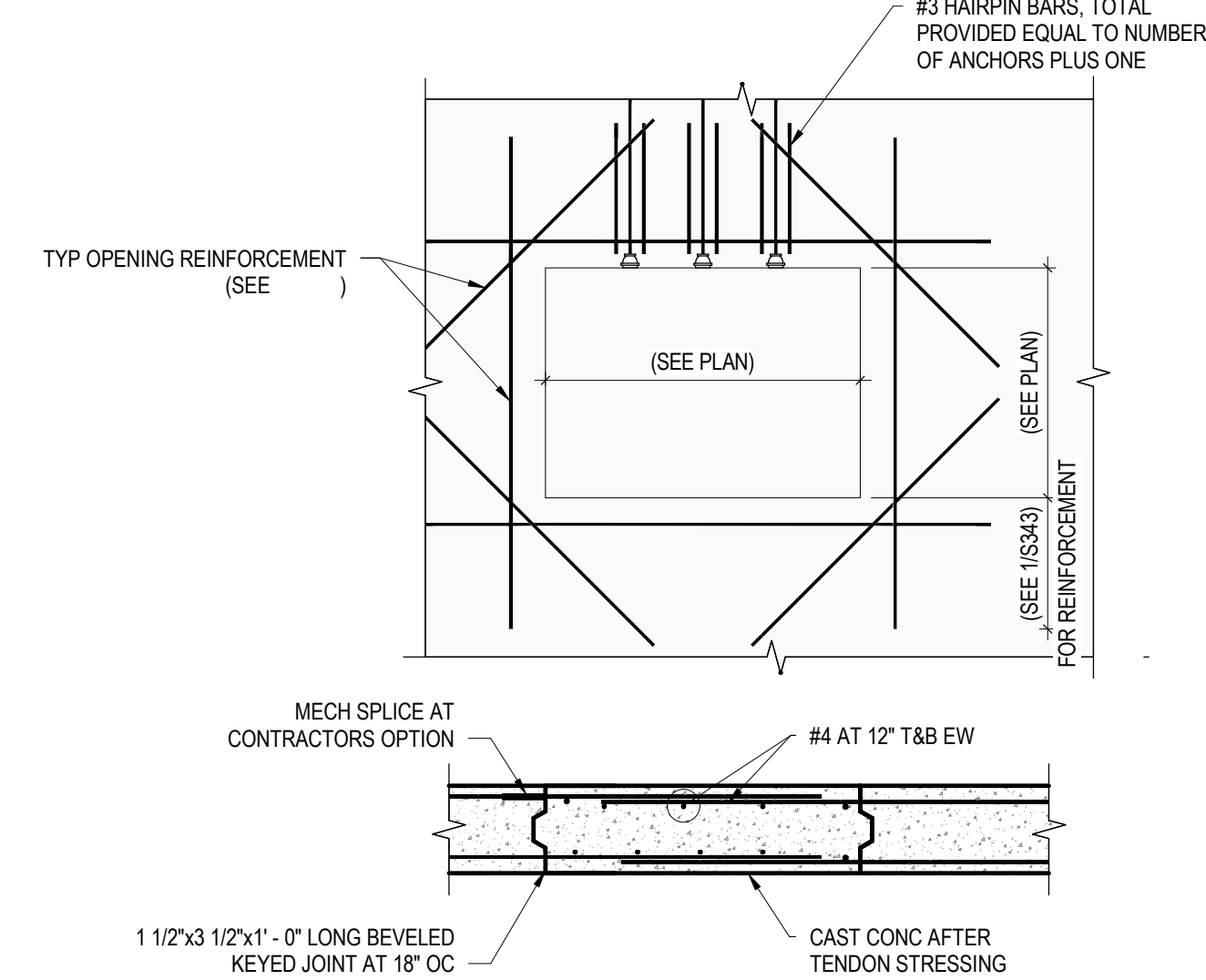
- NOTES:
- PROVIDE HAIRPIN BURSTING REINFORCEMENT SIMILAR TO BANDED TENDON ANCHORAGE FOR BUNDLES OF (6) OR MORE CABLES.



FIXED END

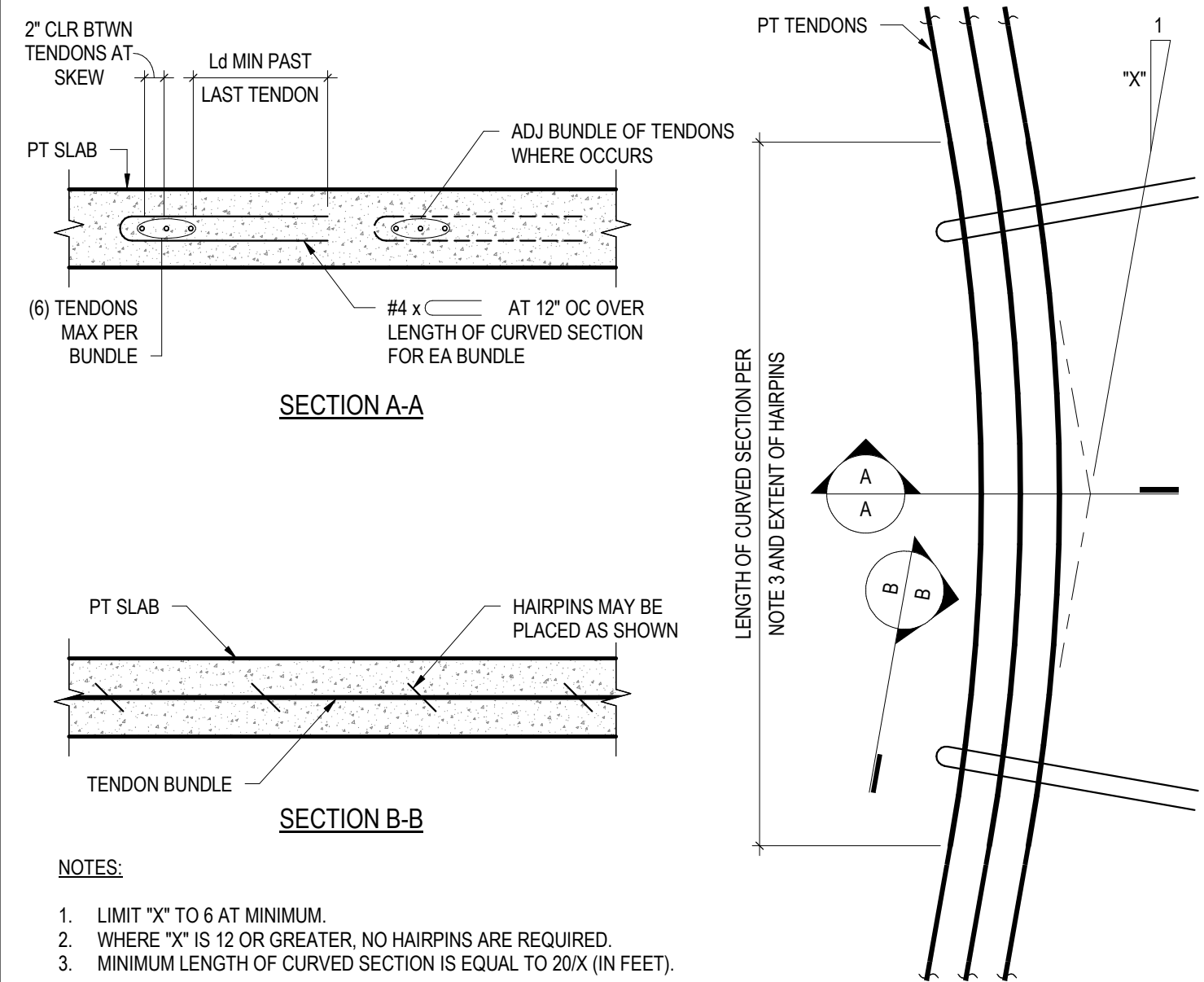
3 ANCHORAGE AT ADDED TENDON

S-330 NTS



4 PT TENDON LOW POINT - SECTION

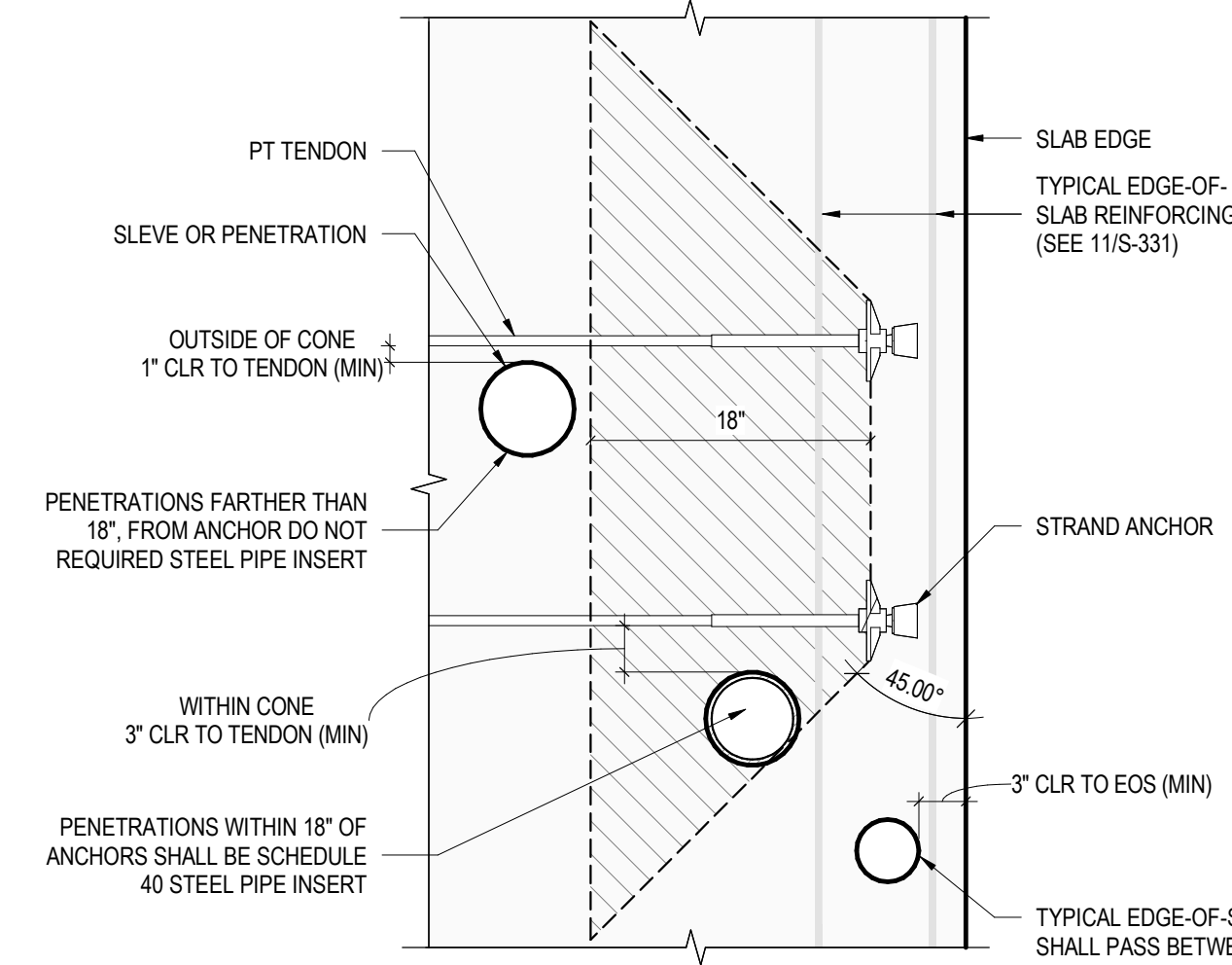
S-330 1" = 1'-0"



- NOTES:
- LIMIT "X" TO 6 AT MINIMUM.
 - WHERE "X" IS 12 OR GREATER, NO HAIRPINS ARE REQUIRED.
 - MINIMUM LENGTH OF CURVED SECTION IS EQUAL TO 20X (IN FEET).

5 TYPICAL BANDED ANCHORAGE ZONE

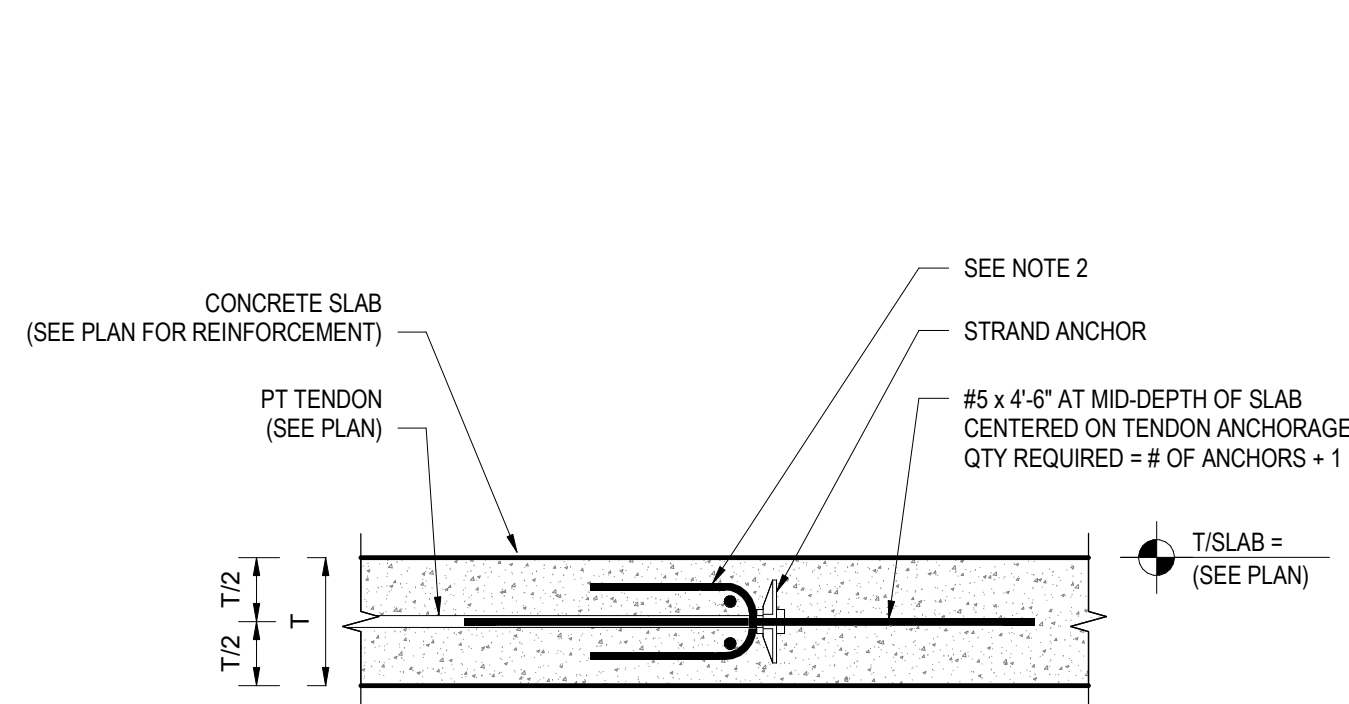
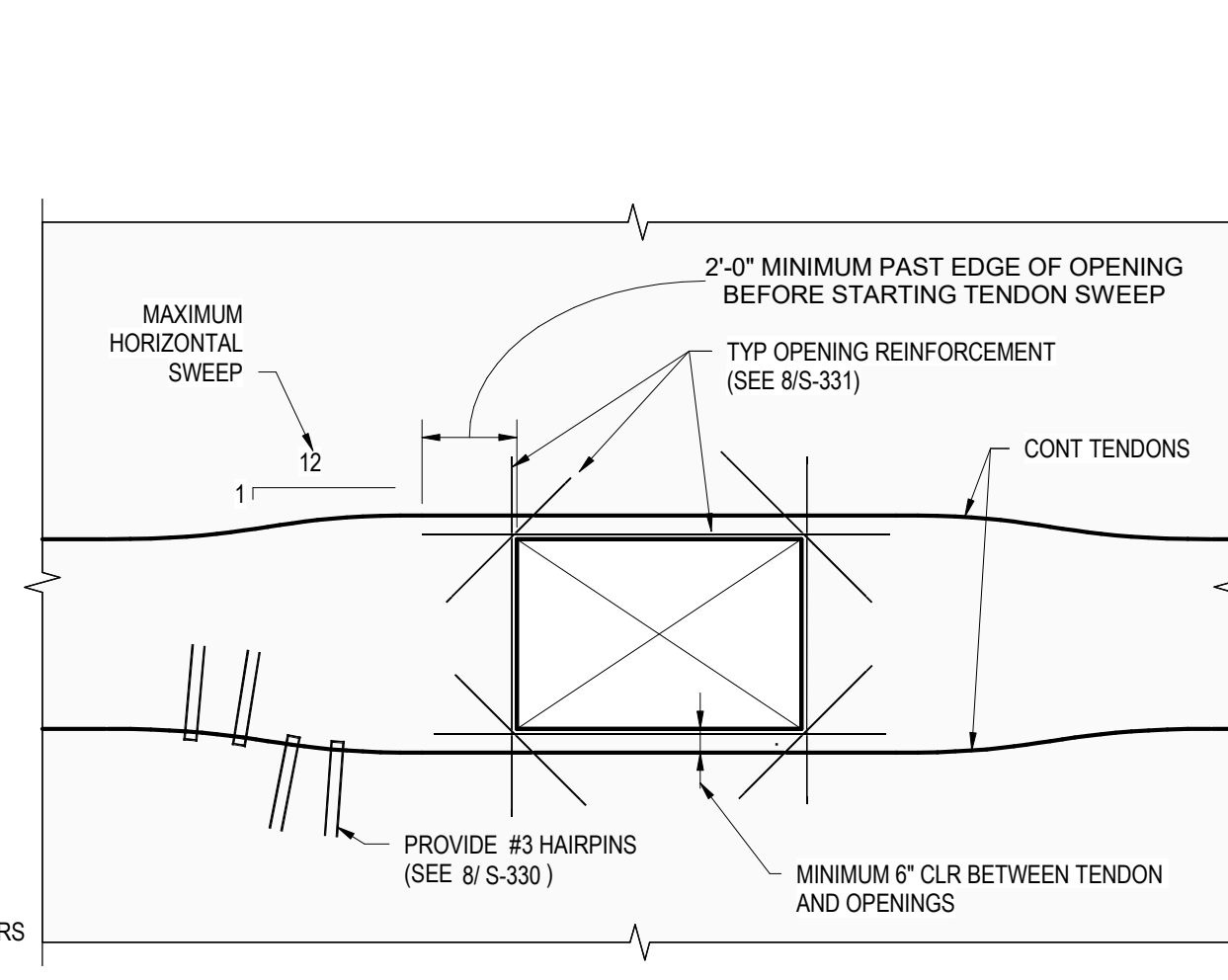
S-330 NTS



- NOTES:
- PENETRATIONS WITH DIMENSIONS GREATER THAN 1/2" REQUIRE TRIM BARS PER TYPICAL DETAIL.
 - SLAB EDGE REINFORCEMENT AND POST TENSIONED BACKUP BARS NOT SHOWN FOR CLARITY. SEE PLAN AND KEYED SECTIONS AND DETAILS FOR REINFORCEMENT REQUIREMENTS.

6 UNIFORM TENDON ANCHORAGE CONDITIONS

S-330 NTS



- NOTES:
- THIS DETAIL APPLIES TO ALL TENDON ANCHORAGES LOCATED AWAY FROM THE EDGE OF SLAB.
 - PROVIDE BACK UP BARS AND HAIRPIN BURSTING REINFORCEMENTS AS REQUIRED PER TYPICAL UNIFORMBANDED TENDON FIXED END DETAILS.

7 STRESSING BLOCKOUT

S-330 1/2" = 1'-0"



8 TYPICAL TENDON HORIZONTAL CURVE

S-330 3/4" = 1'-0"



9 SLEEVES NEAR TENDON ANCHORAGE

S-330 1" = 1'-0"



10 TENDONS AT SLAB OPENING

S-330 NTS

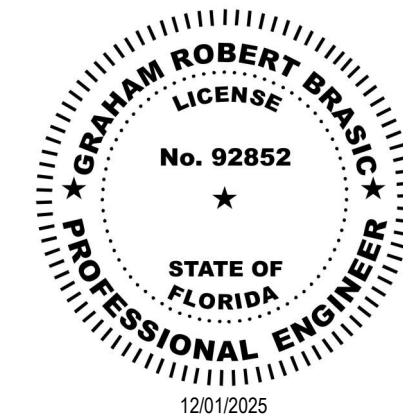


11 ANCHORAGE AT ADDED TENDON

S-330 NTS



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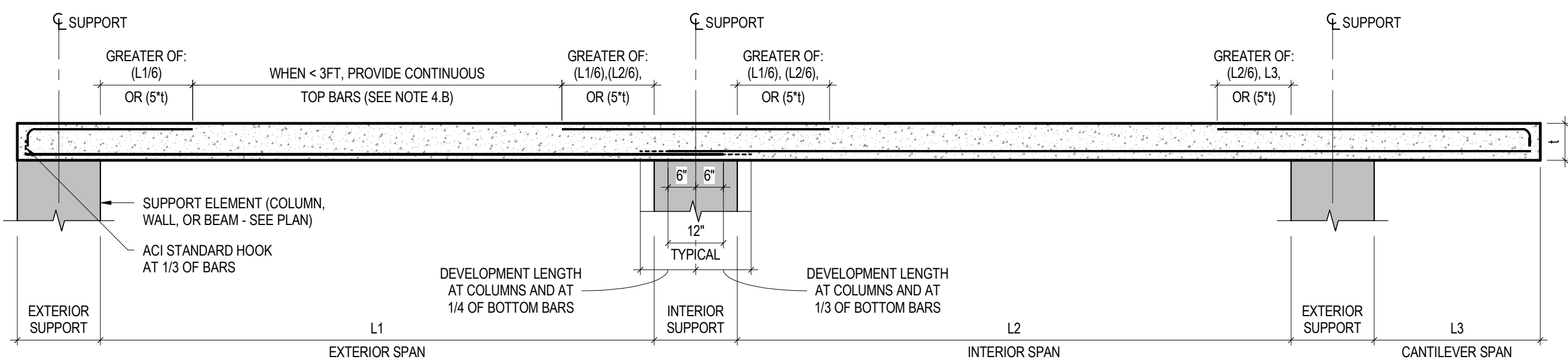
TYPICAL PT
TENDON
DETAILS

S-330

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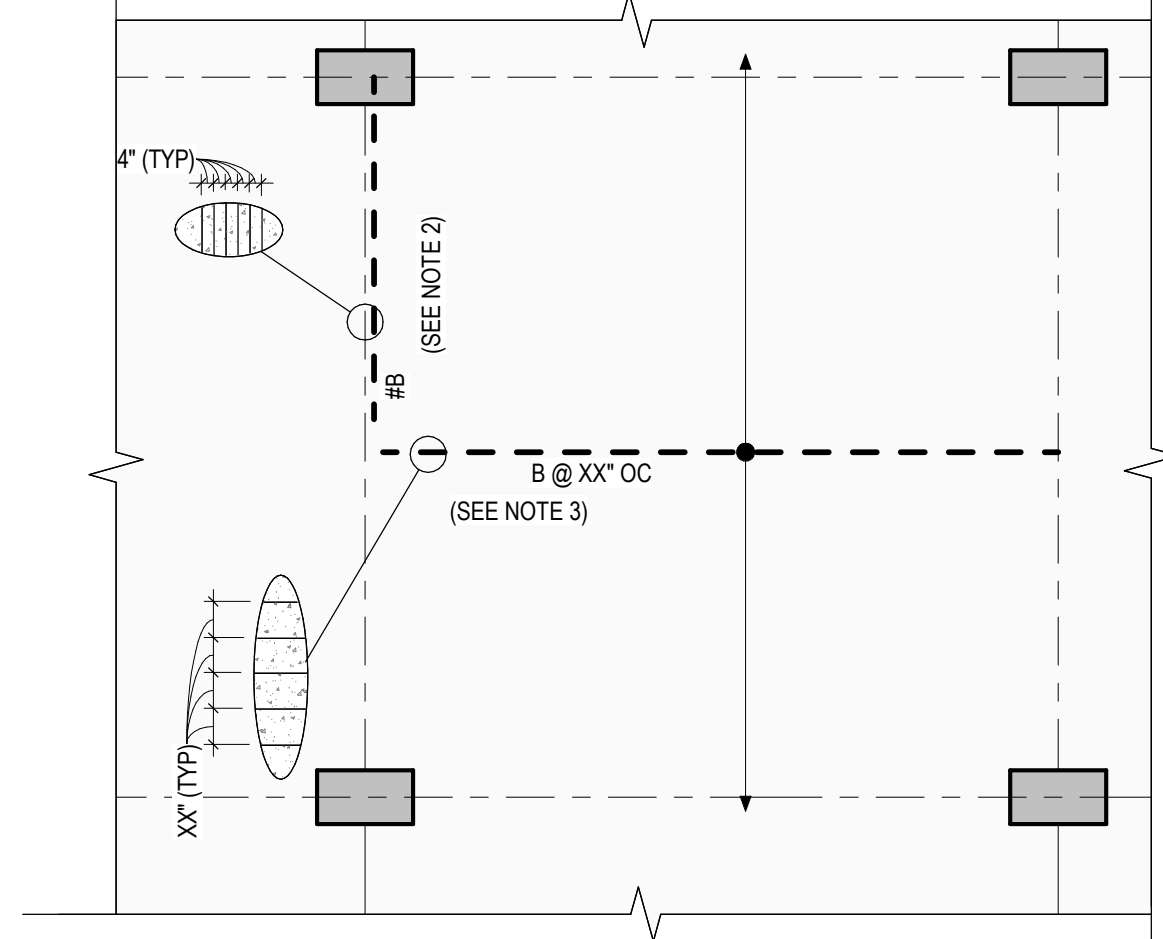


NOTES:

- DRAWING IS NOT TO SCALE.
- CLEAR COVER PER CONCRETE SLAB CLEAR COVER SCHEDULE ON
- BAR LENGTHS SHOWN ABOVE SHALL BE USED UNLESS NOTED OR SHOWN OTHERWISE ON DRAWINGS.
- TOP BAR NOTES
 - TERMINATE TOP BARS AT EDGE-OF-SLAB OR EDGE-OF-OPENING WITH ACI STANDARD HOOKS TURNED FROM VERTICAL AS REQUIRED TO MAINTAIN CLEAR COVER.
 - TOP BARS AT ADJACENT COLUMNS IN SHORT SPANS SHALL BE CONTINUOUS WHEN THE GAP BETWEEN END-OF-BAR IS 3' OR LESS.

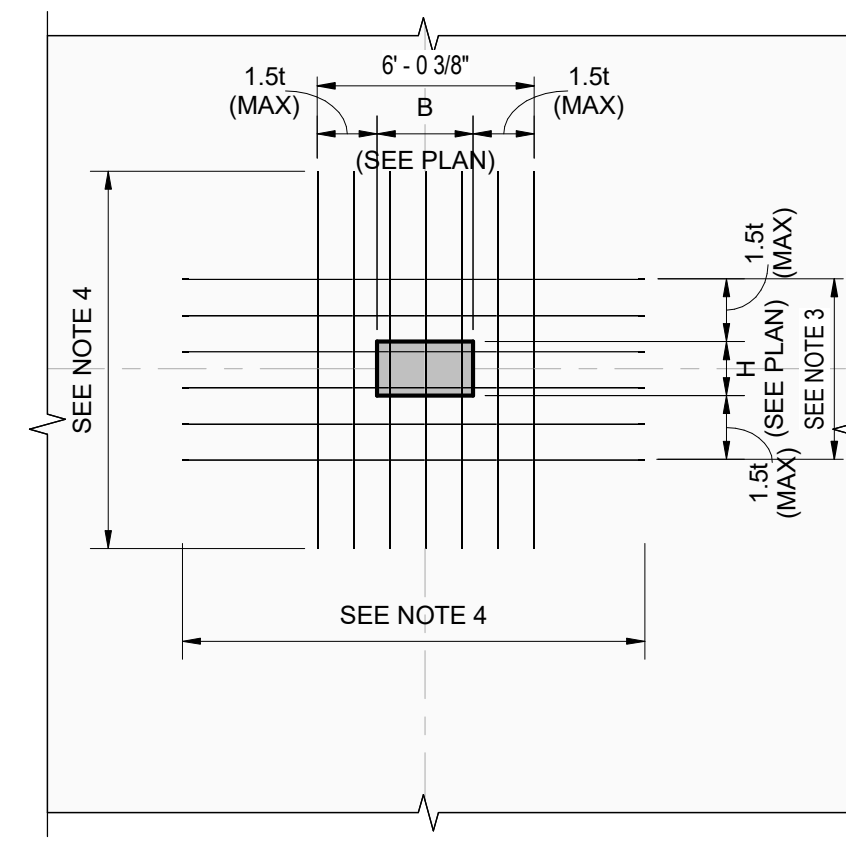
BOTTOM BARS NOTES

- TYPICAL BOTTOM MAT SHALL EXTEND FULL-LENGTH BETWEEN SUPPORTS.
- LENGTH OF ADDITIONAL BOTTOM BARS NOTED ON PLAN SHALL BE AS INDICATED ON PLAN OR EQUAL TO ONE HALF OF THE SPAN IN WHICH THEY ARE LOCATED, WHICHEVER IS GREATER, WITH 1/3 OF THE BARS CONTINUING FULL-LENGTH BETWEEN SUPPORTS.
- FOR ALL BOTTOM BARS, "FULL-LENGTH BETWEEN SUPPORTS" IS DEFINED AS THE LENGTH BETWEEN CENTER-OF-SUPPORT PLUS 6" EACH END. ADDITIONALLY, A MINIMUM OF 1/3 OF THE SPECIFIED BARS (AT EXTERIOR SPANS) OR 1/4 OF THE SPECIFIED BARS (AT INTERIOR SPANS) SHALL EXTEND A FULL DEVELOPMENT LENGTH BEYOND THE CENTERLINE OF SUPPORT EACH END. ANY BOTTOM BAR PASSING THROUGH A COLUMN SHALL EXTEND A FULL DEVELOPMENT LENGTH BEYOND THE CENTERLINE.



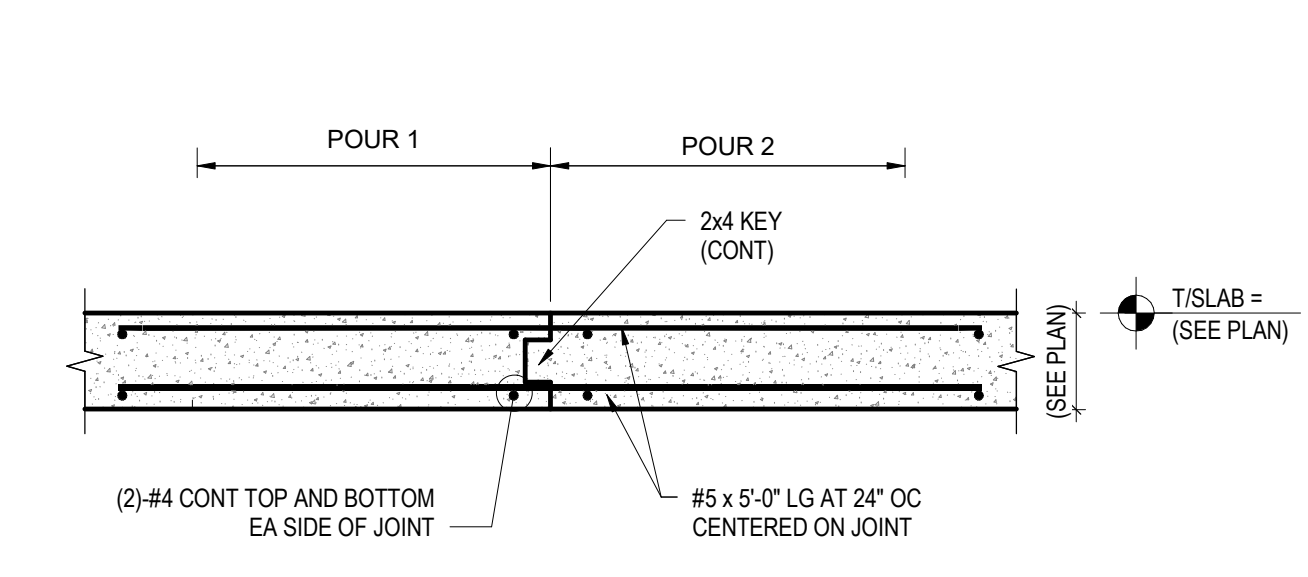
NOTES:

- BARS SHOWN AS #8 SHALL BE CENTERED ALONG COLUMN LINE UNLESS SPECIFICALLY NOTED OTHERWISE.
- BAR SPACING FOR BARS SHOWN AS #8 SHALL BE 4" OC UNLESS NOTED OTHERWISE.
- BARS SHOWN AS SPACED AT XX" SHALL BE OF A QUANTITY AS REQUIRED TO FILL EVENLY THE EXTENTS SHOWN ON PLAN.
- BOTTOM BARS SHALL BE CENTERED AT MID SPAN UNLESS NOTED OTHERWISE.
- SEE TYPICAL POST TENSIONED SLAB REINFORCEMENT LAYOUT DETAIL FOR BAR EXTENTS.



NOTES:

1. DENOTES SLAB THICKNESS AT COLUMN LOCATION.
2. PLACE A MINIMUM OF 2 OR 3 BARS THROUGH COLUMN (WITHIN COLUMN CAGE), WITH REMAINING GBARS EQUALLY DISTRIBUTED ON EACH SIDE OF COLUMN.
3. TOP BARS SHALL BE EQUALLY SPACED WITHIN A WIDTH EQUAL TO THE COLUMN DIMENSION + 3" AS SHOWN ABOVE, BUT SHALL NOT BE SPACED GREATER THAN 12" OC. PROVIDE ADDITIONAL BARS IF REQUIRED TO MEET MAXIMUM SPACING REQUIREMENT.
4. BAR SPACING SHALL BE SUCH THAT ALL BARS FIT WITHIN (B OR H) + 3", BUT SHALL NOT BE SPACED GREATER THAN 12" OC PROVIDE ADDITIONAL BARS AS REQUIRED.
5. SEE TYPICAL POST TENSIONED SLAB REINFORCEMENT LAYOUT DETAIL FOR BAR LENGTHS.

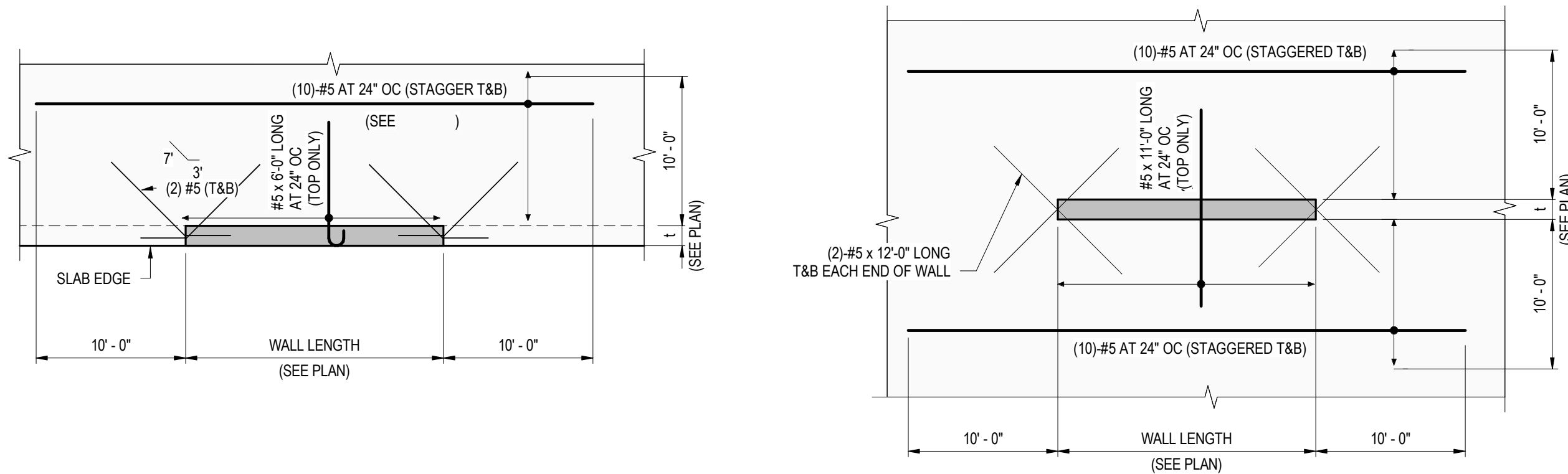


NOTES:

- SUBMIT PROPOSED CONSTRUCTION JOINT LOCATIONS FOR REVIEW PRIOR TO CONSTRUCTION.
- SCHEDULE REINFORCEMENT AND/OR REINFORCEMENT INDICATED ON PLAN SHALL BE CONTINUOUS THROUGH ALL CONSTRUCTION JOINTS. MAINTAIN SPLICE LOCATIONS INDICATED IN FRAMING SECTIONS. ADDITIONAL SPLICES ARE NOT PERMITTED.
- WHERE POST TENSION EXISTS, CONSTRUCTION JOINTS SHALL BE LOCATED SUCH THAT THE POST TENSIONING IS ANCHORED AT THE CENTER OF GRAVITY OF THE SLAB CROSS SECTION.
- INTENTIONALLY ROUGHEN THE FACE OF SLAB AT CONSTRUCTION JOINT TO 1/4" MINIMUM AMPLITUDE FOR SLABS ≤ 6" THICK. INSTALL 2X4 CONT KEY AT SLABS > 6" THICK ALTERNATIVELY, STAY IN PLACE GALVANIZED STEEL BULKHEAD FORMS CAPABLE OF PROVIDING THE SPECIFIED SURFACE ROUGHNESS MAY BE USED.
- CLEAR COVER TO ADDITIONAL JOINT REINFORCEMENT SHALL BE AS REQUIRED FOR TYPICAL REINFORCEMENT AT EACH LEVEL.

1 SLAB BAR LENGTHS

NTS

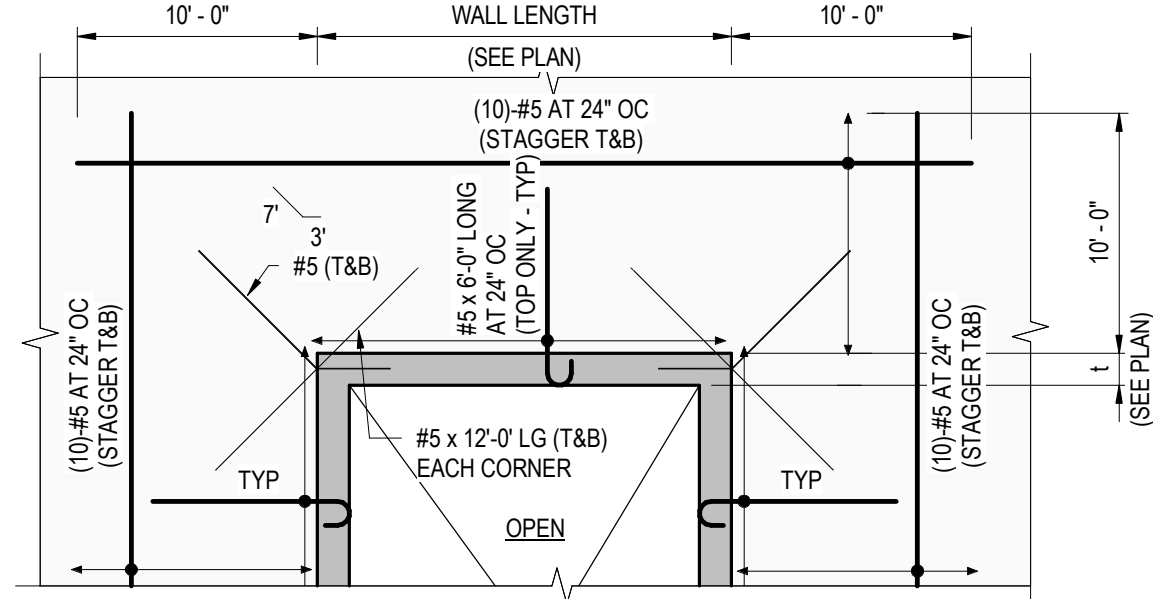


NOTES:

- AT BASEMENT OR RETAINING WALL CONDITIONS, REINFORCEMENT INDICATED SHALL BE INSTALLED CONTINUOUS ALONG FULL LENGTH OF WALL AND SLOPE CONNECTIONS SHALL BE PROVIDED

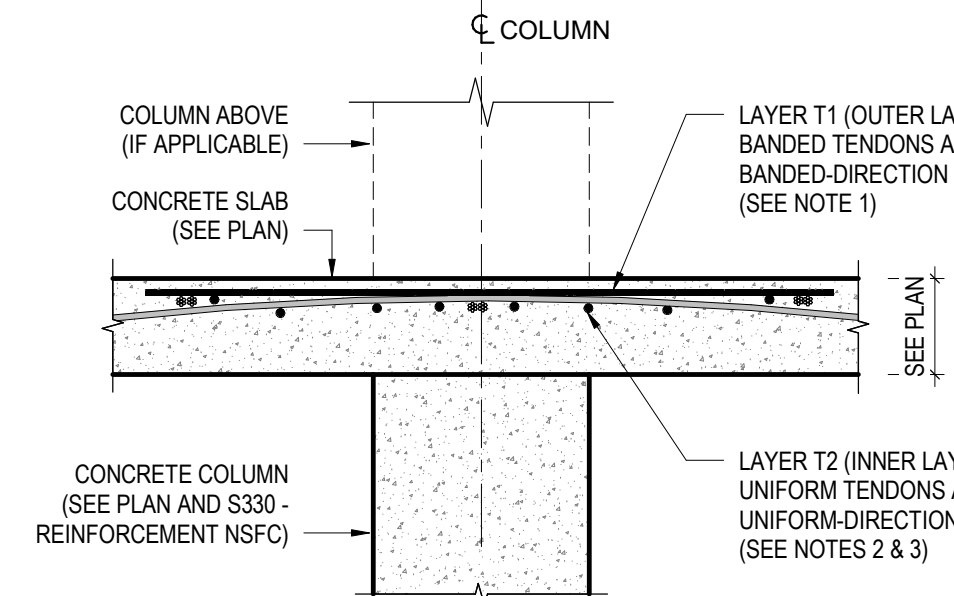
2 BOTTOM BAR PLACEMENT

NTS



3 TOP BAR PLACEMENT AT COLUMNS

NTS

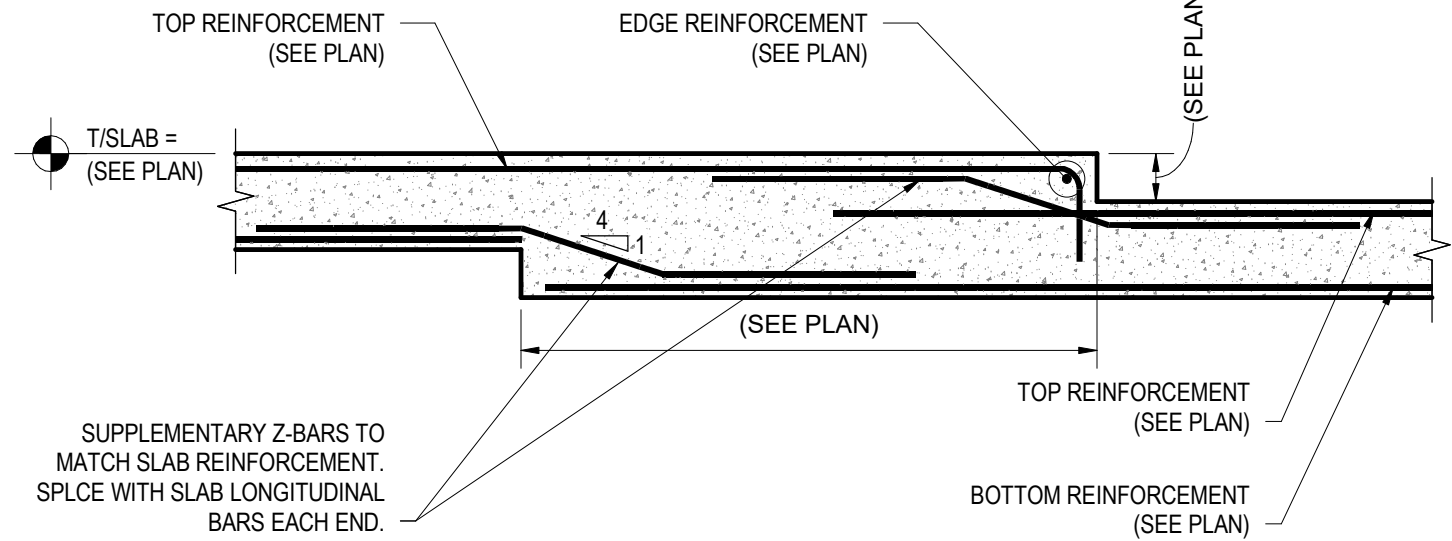


NOTES:

- CONVENTIONAL TOP BAR REINFORCEMENT AS NOTED ON PLAN IN THE BANDED DIRECTION SHALL BE PLACED IN THE SAME LAYER AS THE BANDED POST TENSION TENDONS.
- CONVENTIONAL TOP BAR REINFORCEMENT AS NOTED ON PLAN IN THE UNIFORM DIRECTION SHALL BE PLACED IN THE SAME LAYER AS THE UNIFORM POST TENSION TENDONS, AND SHALL BE DIRECTLY BELOW THE BANDED LAYER.
- WHERE UNIFORM TENDONS ARE LOCATED FAR ENOUGH FROM THE COLUMN CENTERLINE TO NOT CONFLICT WITH BANDED TENDONS OR REINFORCEMENT, UNIFORM TENDONS SHALL BE LOCATED ABOVE THE BANDED TENDONS TO ACHIEVE THE DRAPE ELEVATION NOTED ON THE PLANS.

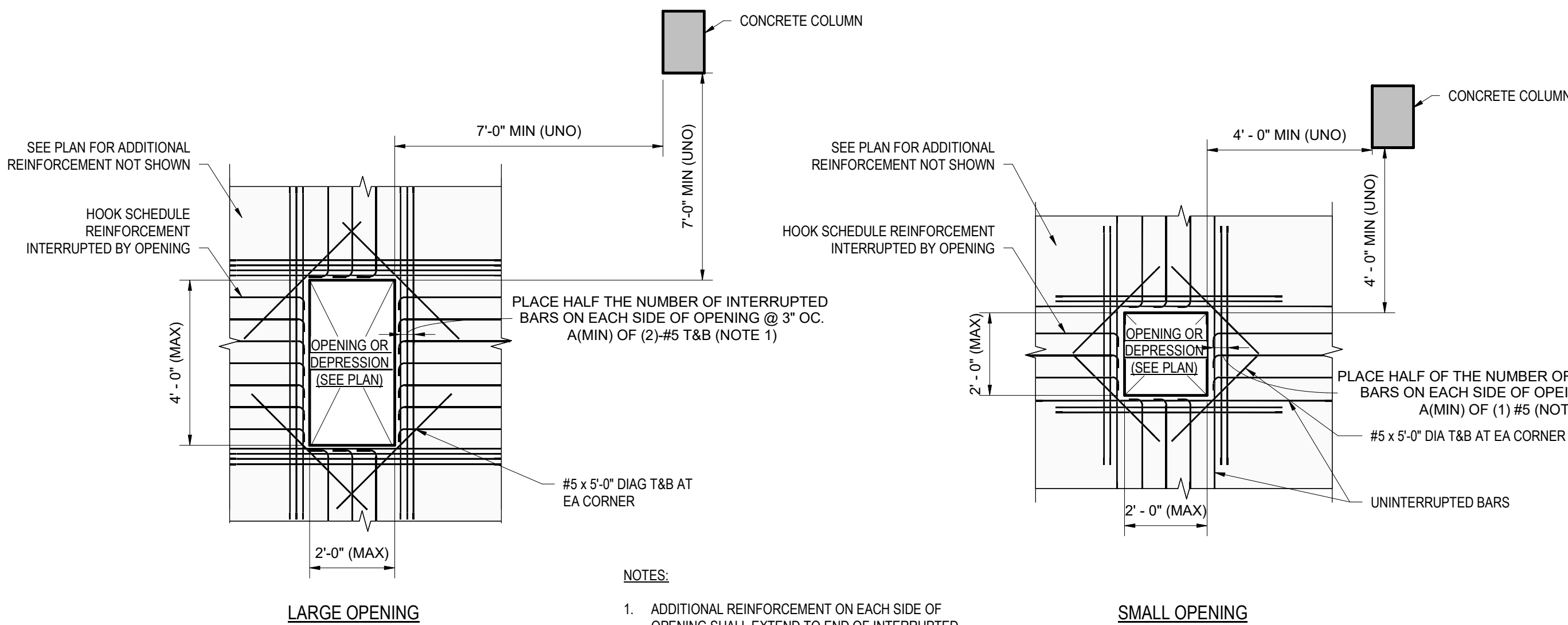
4 CONSTRUCTION JOINT IN SLAB

NTS



5 ADDITIONAL SLAB REINFORCEMENT AT CONCRETE WALLS

1/8" = 1'-0"

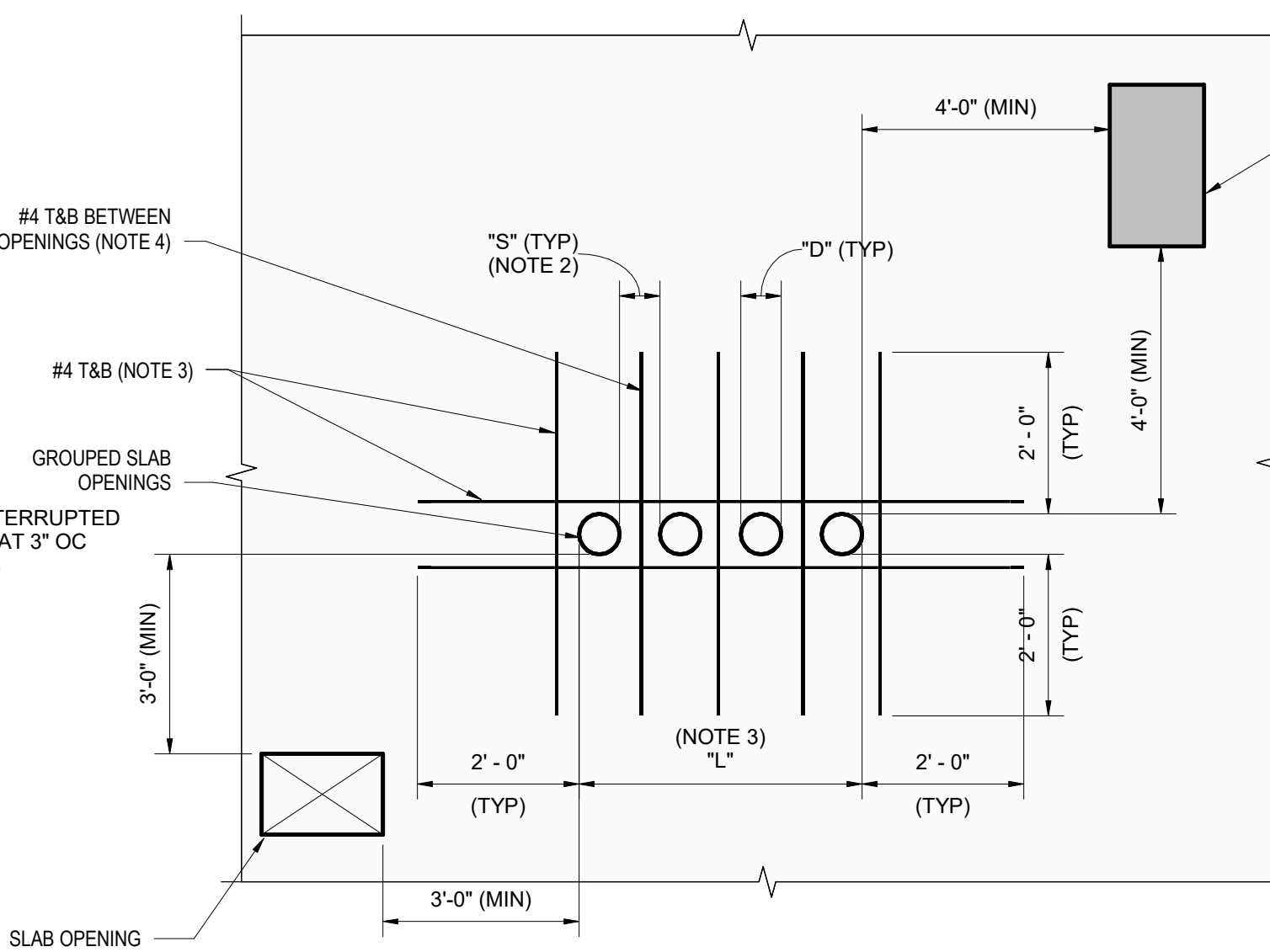


NOTES:

- ADDITIONAL REINFORCEMENT ON EACH SIDE OF OPENING SHALL EXTEND TO END OF INTERRUPTED REINFORCEMENT OR 12" PAST OPENING, WHICHEVER IS SHORTER.

6 PLACEMENT PRIORITIES AT COLUMNS

3/4" = 1'-0"

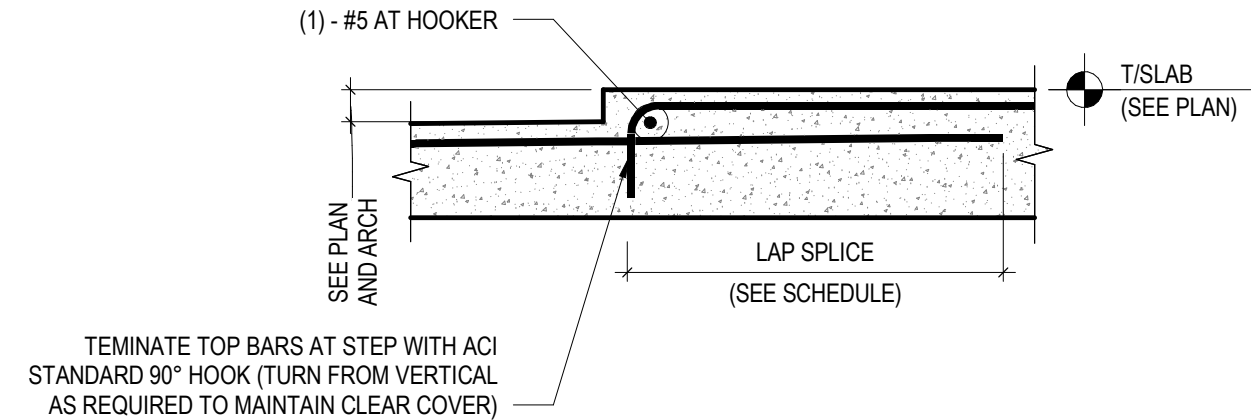


NOTES:

- THIS DETAIL APPLIES TO GROUPED OPENINGS WITH D ≤ 8".
- MINIMUM OPENING SPACE, "S" = OPENING WIDTH, "D", OR 3" (WHICHEVER IS LARGER), WHERE A TENDON MUST PASS BETWEEN OPENINGS. "S" = 8" MINIMUM.
- WHERE "L" IS > 1'-0" OR THERE ARE (3) OR MORE GROUPED OPENINGS, PROVIDE #4 TOP AND BOTTOM ON EACH SIDE OF GROUPED OPENINGS.
- WHERE "S" ≤ 1'-0", PROVIDE #4 TOP AND BOTTOM BETWEEN OPENINGS.
- OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED FOR REVIEW WITH SLAB SHOP DRAWINGS. ADDITIONAL SLAB REINFORCEMENT MAY BE REQUIRED BEYOND WHAT IS DETAILED.
- NOTIFY THE STRUCTURAL ENGINEER OF RECORD IF THESE CONDITIONS CANNOT BE MET.

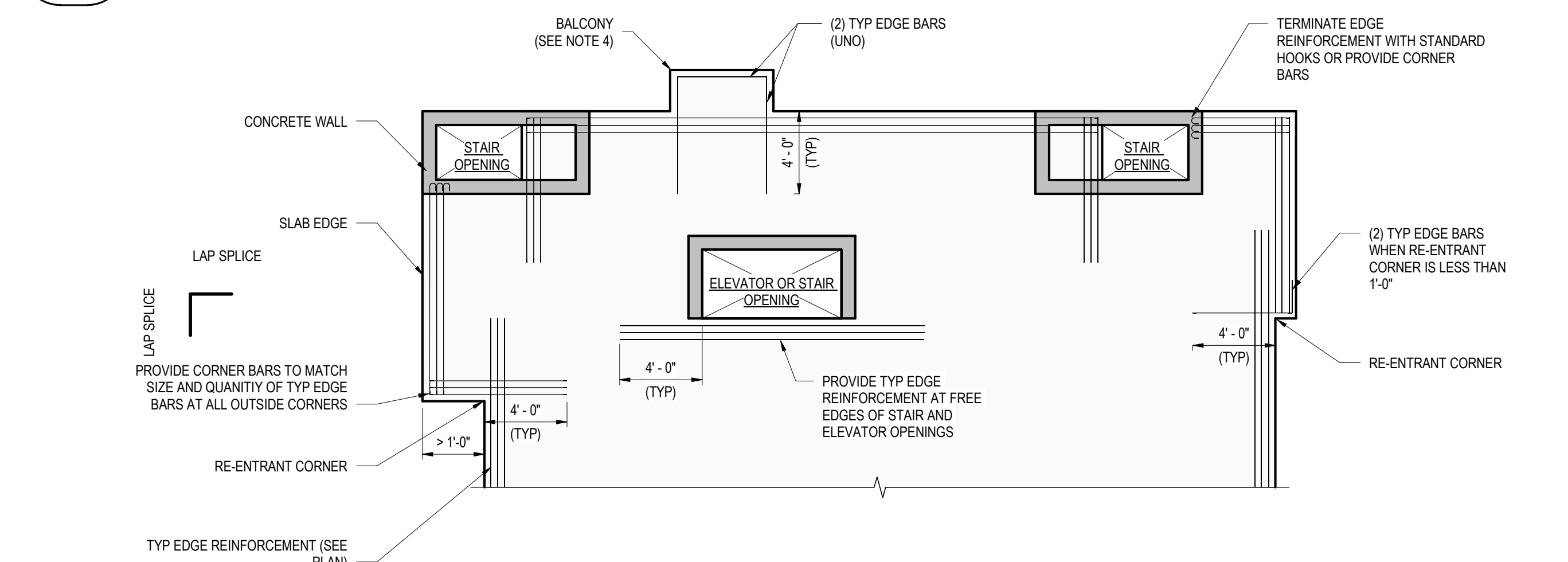
7 TYPICAL PT SLAB FOLD - DETAIL

3/4" = 1'-0"



8 TYPICAL SLAB OPENING REINFORCEMENT AT POST-TENSIONED SLAB - PLAN

3/8" = 1'-0"

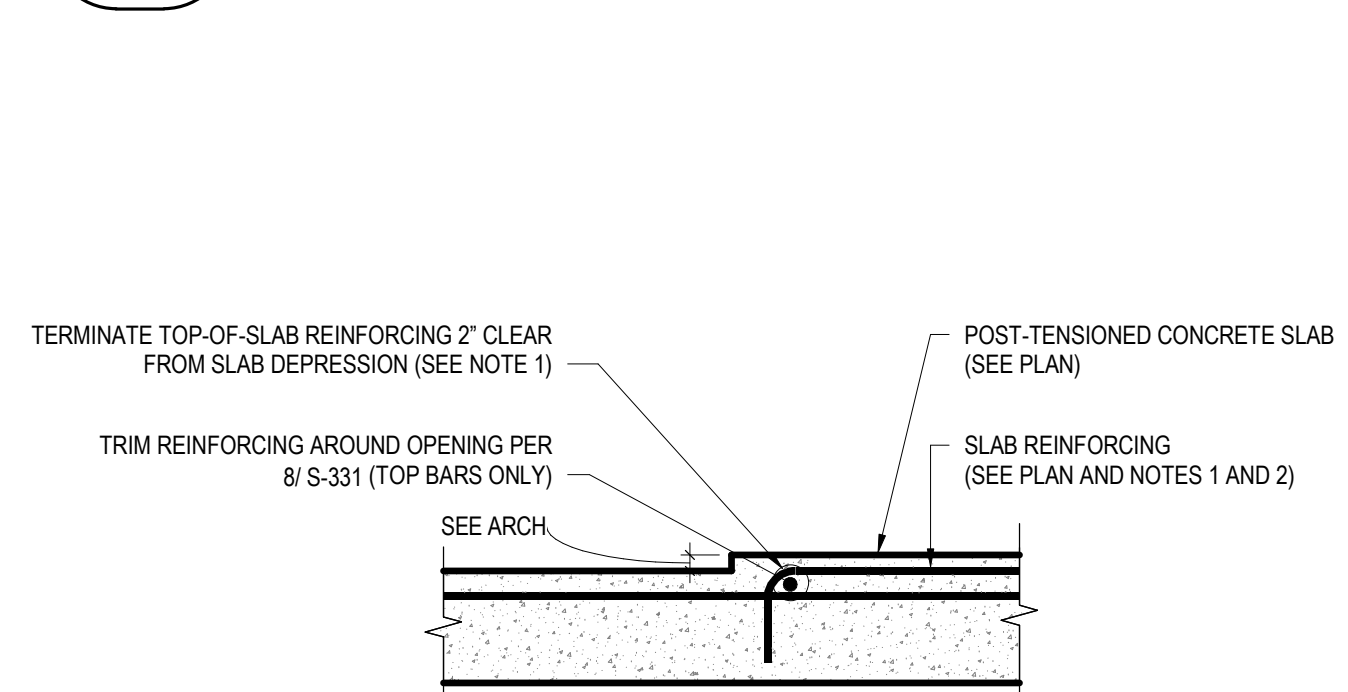


NOTES:

- THIS PLAN IS CONCEPTUAL. SEE PLAN FOR ACTUAL DIMENSIONS AND ARRANGEMENT OF STRUCTURAL ELEMENTS.
- EDGE REINFORCEMENT SHALL BE CONTINUOUS AND SHALL BE LAP SPICED AS NECESSARY.
- SEE PLAN AND KEYED SECTIONS AND DETAILS FOR ADDITIONAL REINFORCEMENT NOT SHOWN IN THIS PLAN.
- ALL BALCONY REINFORCEMENT, INCLUDING EDGE REINFORCEMENT, SHALL BE GALVANIZED.

9 TYPICAL GROUPED OPENING REINFORCEMENT AT POST-TENSIONED SLAB - PLAN

1/2" = 1'-0"



NOTES:

- WHERE SLAB DEPRESSION INTERRUPTS TOP-OF-SLAB REINFORCING, TERMINATE TYPICAL BARS 2" CLEAR FROM EDGE OF DEPRESSION AS SHOWN AND PROVIDE ADDITIONAL BARS (SIZE AND QUANTITY SHALL MATCH ADJACENT TOP REINFORCING LOCATED 1" CLEAR FROM DEPRESSION TOP OF SLAB.) LENGTH OF ADDITIONAL BARS AS REQUIRED TO SPLICE WITH ADJACENT TYPICAL BARS.
- WHERE SLAB DEPRESSION INTERRUPTS TOP-OF-SLAB REINFORCING AT COLUMN, REBAR SHALL EXTEND TO L6 PER 17-1 IN ADDITION TO THE REQUIREMENT OUTLINED IN NOTE 1.

TYPICAL SHOWER DEPRESSION AT POST TENSIONED SLAB

1" = 1'-0"

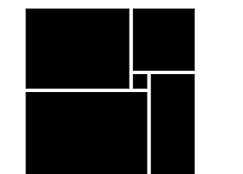
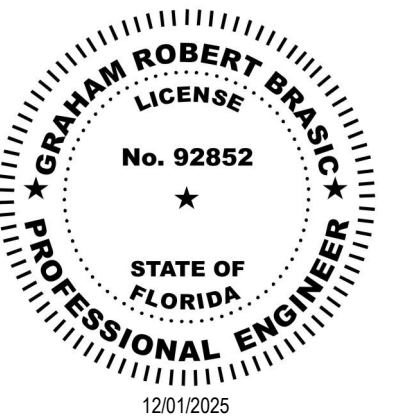
11 TYPICAL ELEVATED SLAB EDGE REINFORCEMENT CONCEPTUAL PLAN

3/16" = 1'-0"

12 TYPICAL SHOWER DEPRESSION AT POST TENSIONED SLAB

1" = 1'-0"

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TYPICAL PT
REINFORCEMENT
DETAILS



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A circular professional engineer seal for the State of Florida. The outer ring contains the text "GRAHAM ROBERT BRASIC" at the top and "PROFESSIONAL ENGINEER" at the bottom, separated by two stars. Inside the ring, the word "LICENSE" is at the top, "No. 92852" is in the center, and "STATE OF FLORIDA" is at the bottom, with a single star between the license number and the state name. At the very bottom, the expiration date "12/01/2025" is printed.



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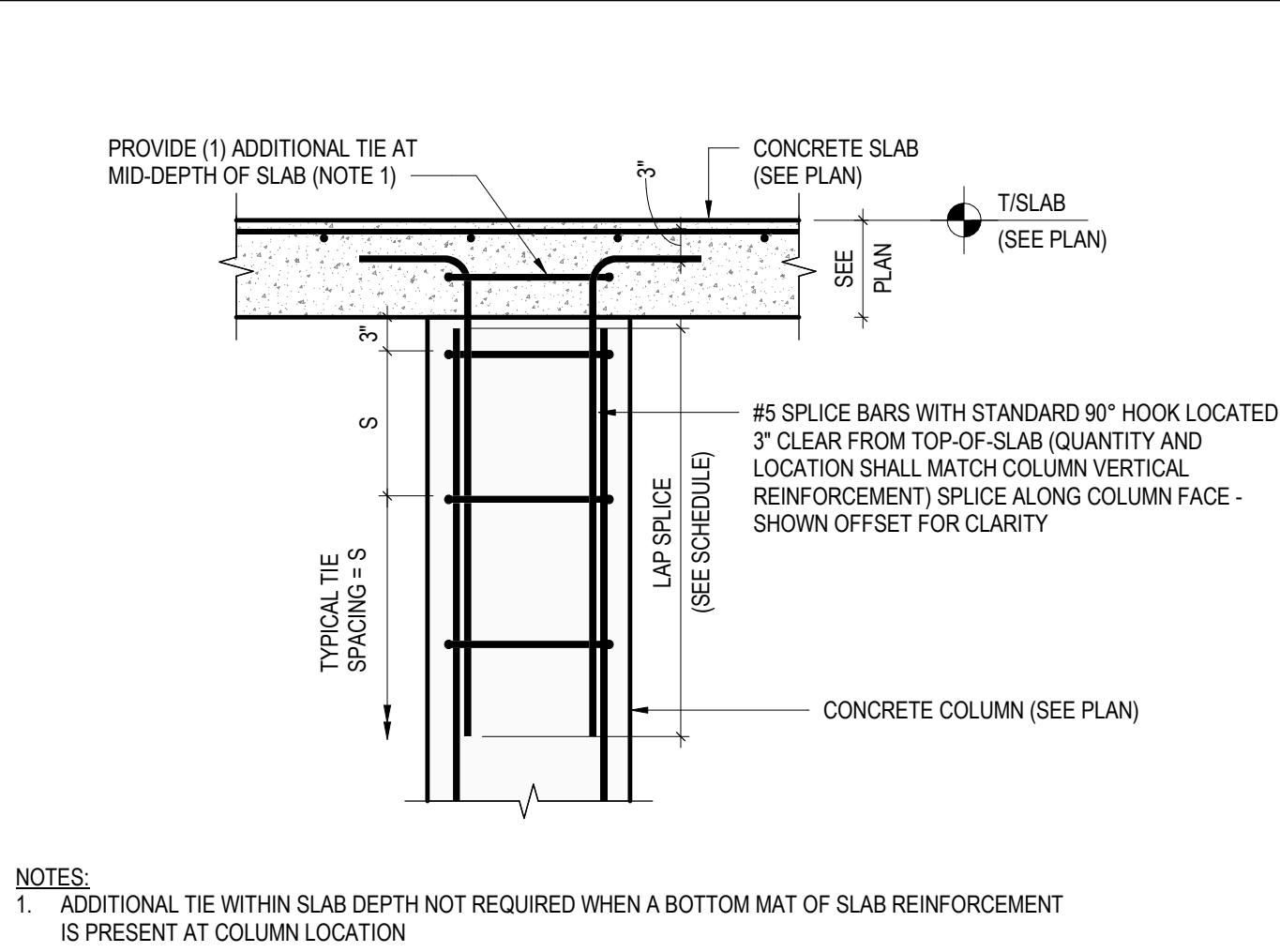
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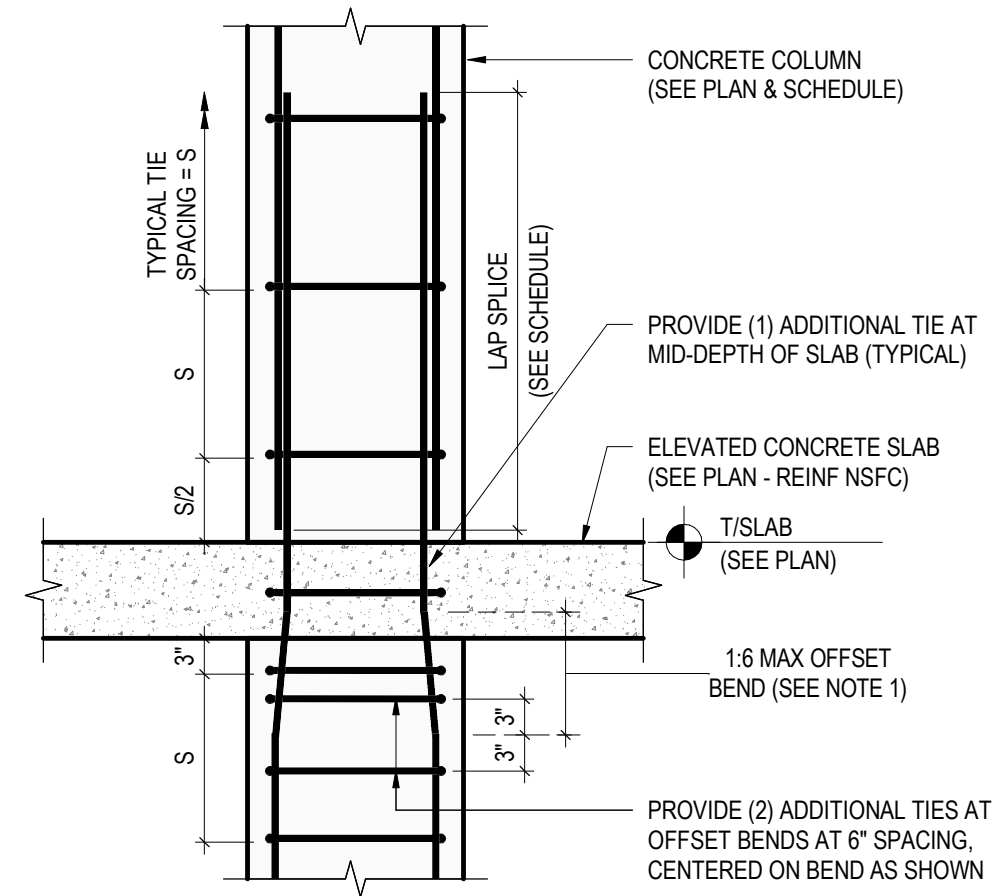
TYPICAL PT SHEAR STU RAILS DETAILS

S-332

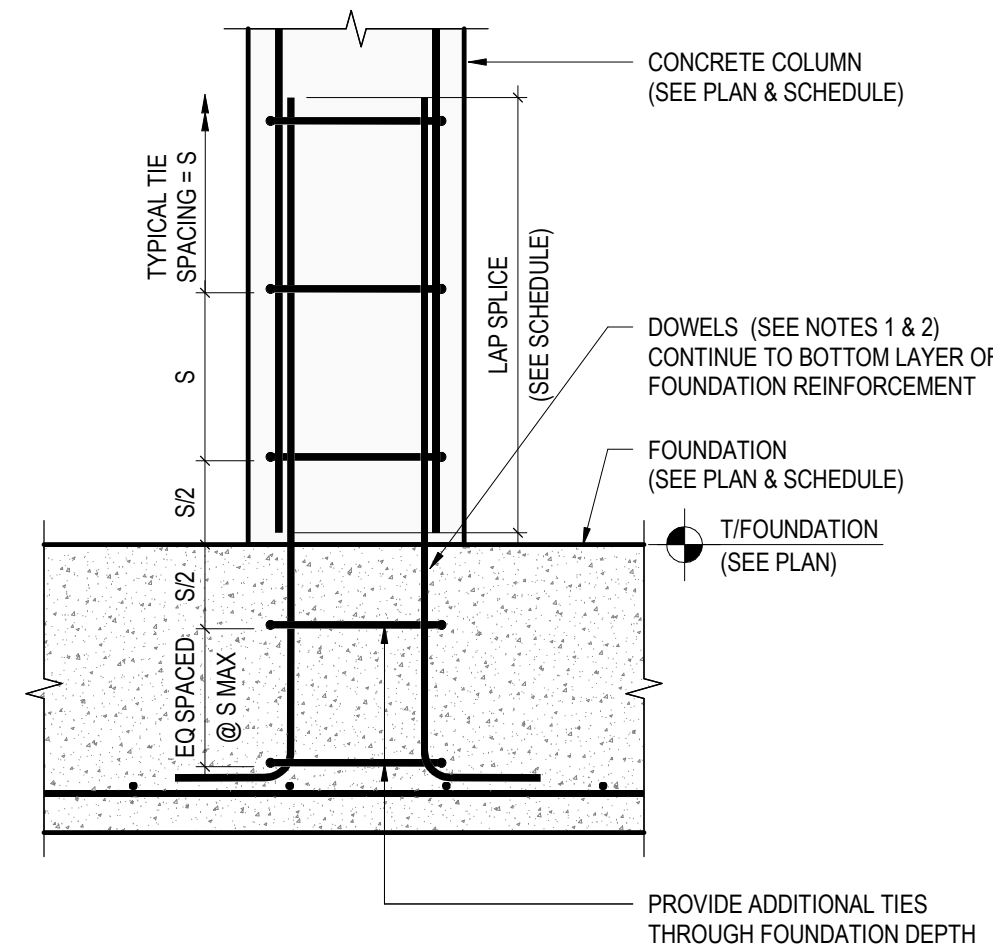
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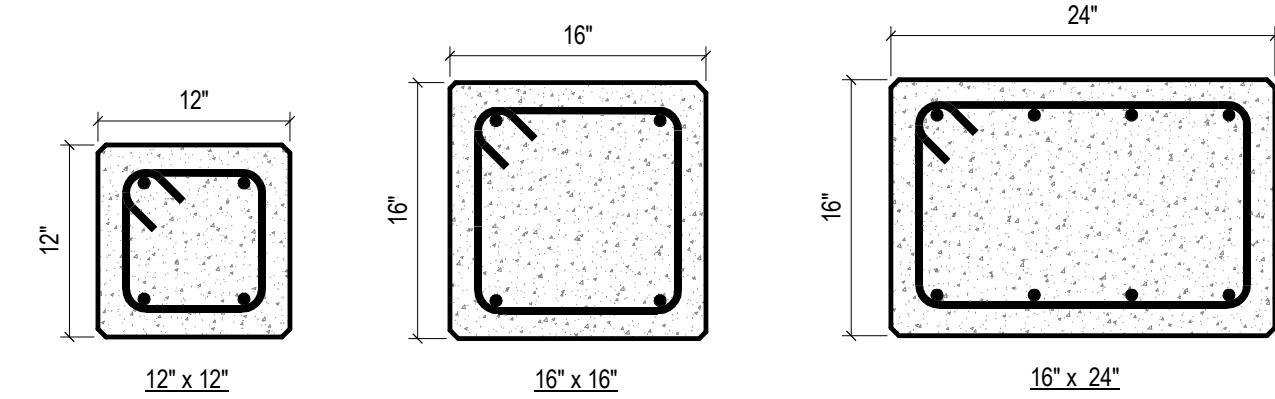
1C
S-340
3/4" = 1'-0"



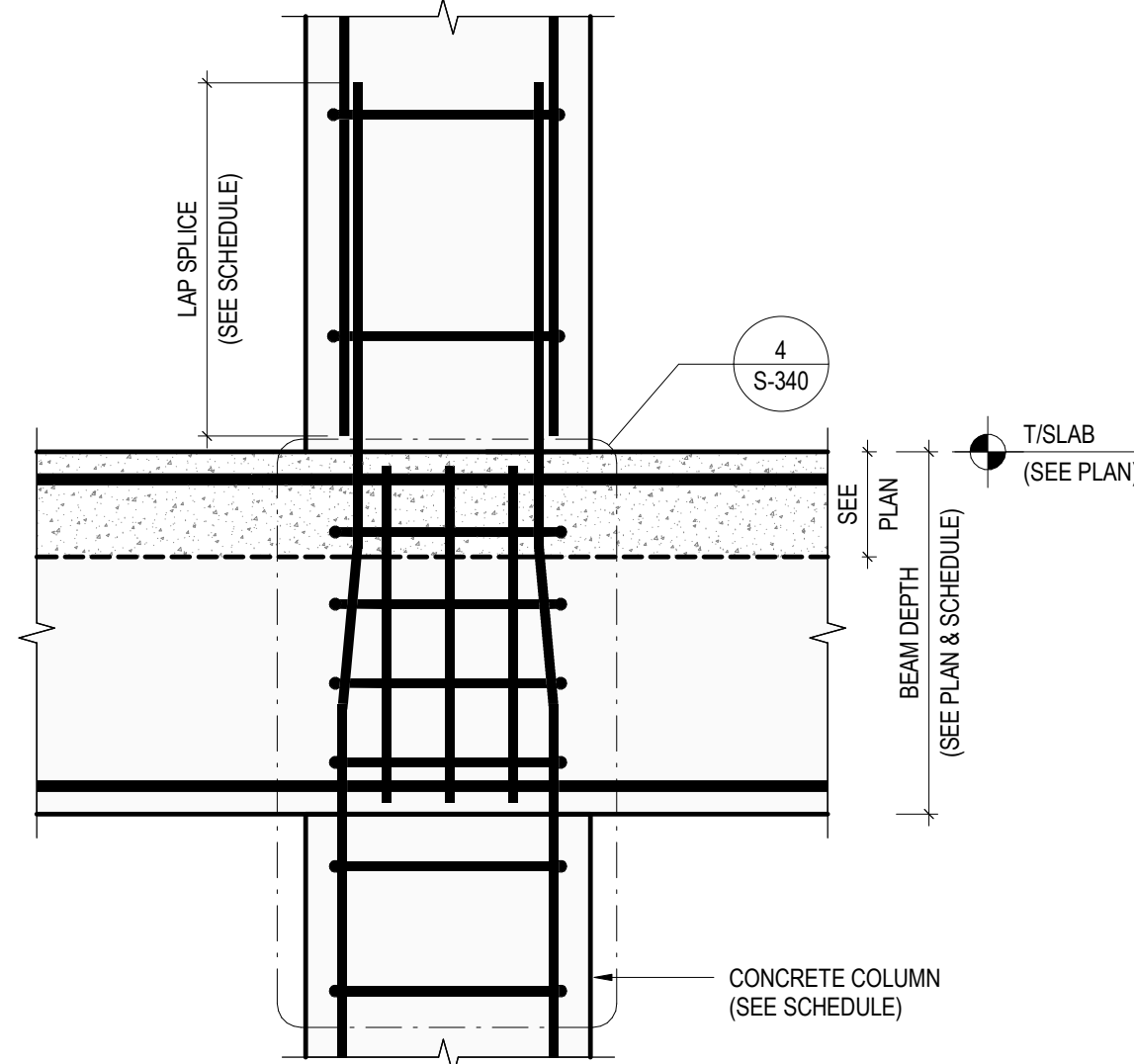
1B
S-340
3/4" = 1'-0"



1A
S-340
3/4" = 1'-0"

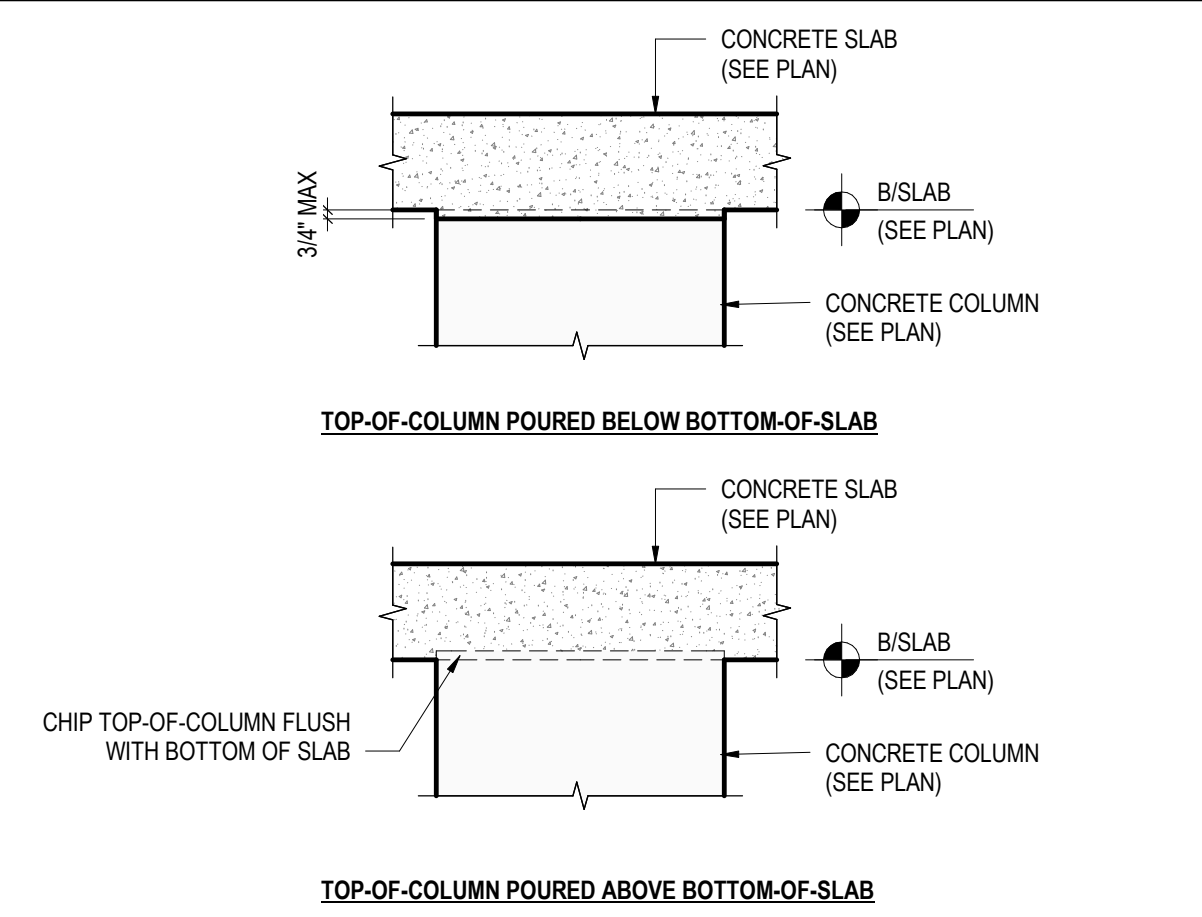


2
S-340
1" = 1'-0"



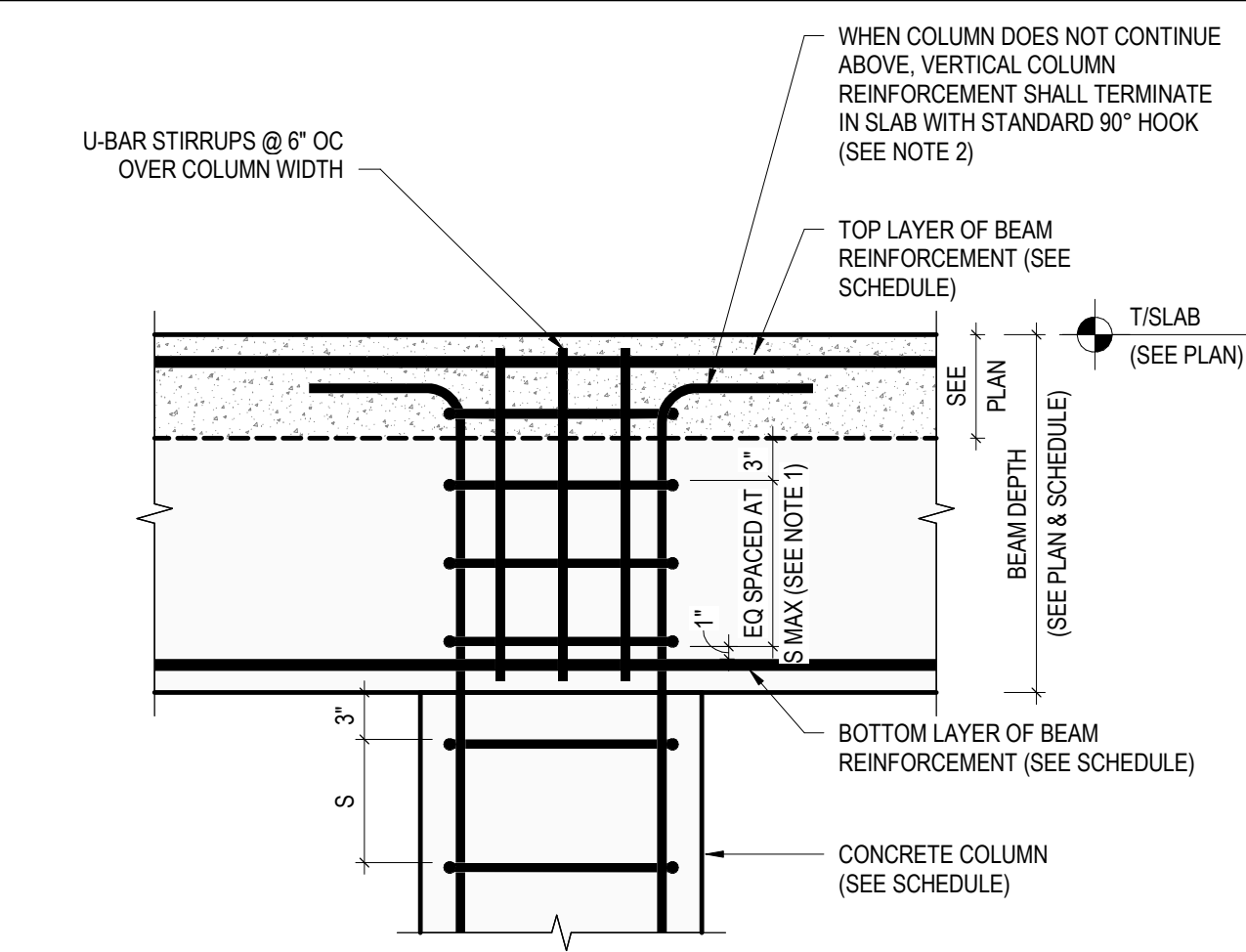
5
S-340
1" = 1'-0"

CONCRETE COLUMN SCHEDULE					
MARK	GEOMETRY		REINFORCING		REMARKS
	WIDTH	DEPTH	VERTICAL	TIES	
CC1212	12"	12"	(4) - #6	#3 @ 12" OC	
CC1616	16"	16"	(4) - #8	#3 @ 14" OC	
CC1624	16"	24"	(8) - #7	#3 @ 14" OC	



NOTES:
1. DEVIATION FROM MAXIMUM TOLERANCES SHOWN ABOVE SHALL BE CORRECTED PRIOR TO PLACEMENT OF CONCRETE SLAB. WHEN TOP-OF-COLUMN IS INSTALLED BELOW BOTTOM-OF-SLAB BY MORE THAN THE TOLERANCE SHOWN ABOVE, THE DIFFERENCE SHALL BE POURED CONCRETE WITH STRENGTH EQUAL TO THE REQUIRED COLUMN STRENGTH.

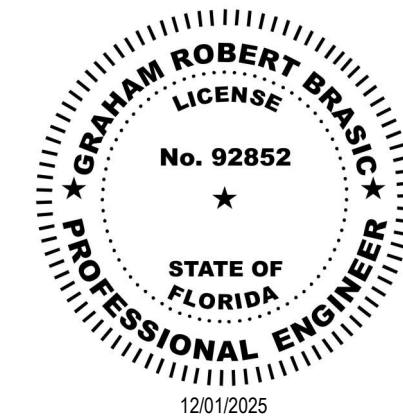
3
S-340
3/4" = 1'-0"



NOTES:
1. TIES NOT REQUIRED OVER DEPTH OF BEAM WHEN BEAMS OR BRACKETS FRAME INTO A COLUMN FROM FOUR DIRECTIONS.
2. BEAM REINFORCEMENT ELEVATION OVER COLUMN SHALL REMAIN AS SHOWN ON CONCRETE BEAM DETAILS. COORDINATE HOOK ELEVATION OF COLUMN VERTICAL REINFORCEMENT WITH BEAM TOP REINFORCEMENT ACCORDINGLY.

4
S-340
1" = 1'-0"

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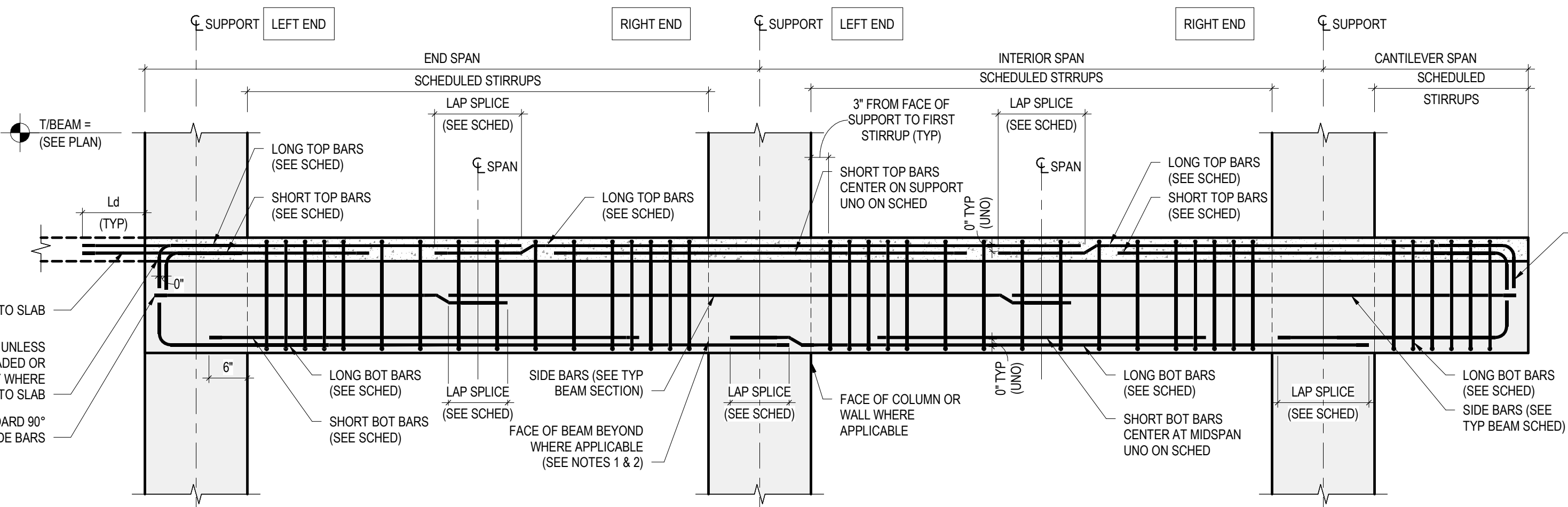
TYPICAL
CONCRETE
COLUMN
DETAILS

S-340

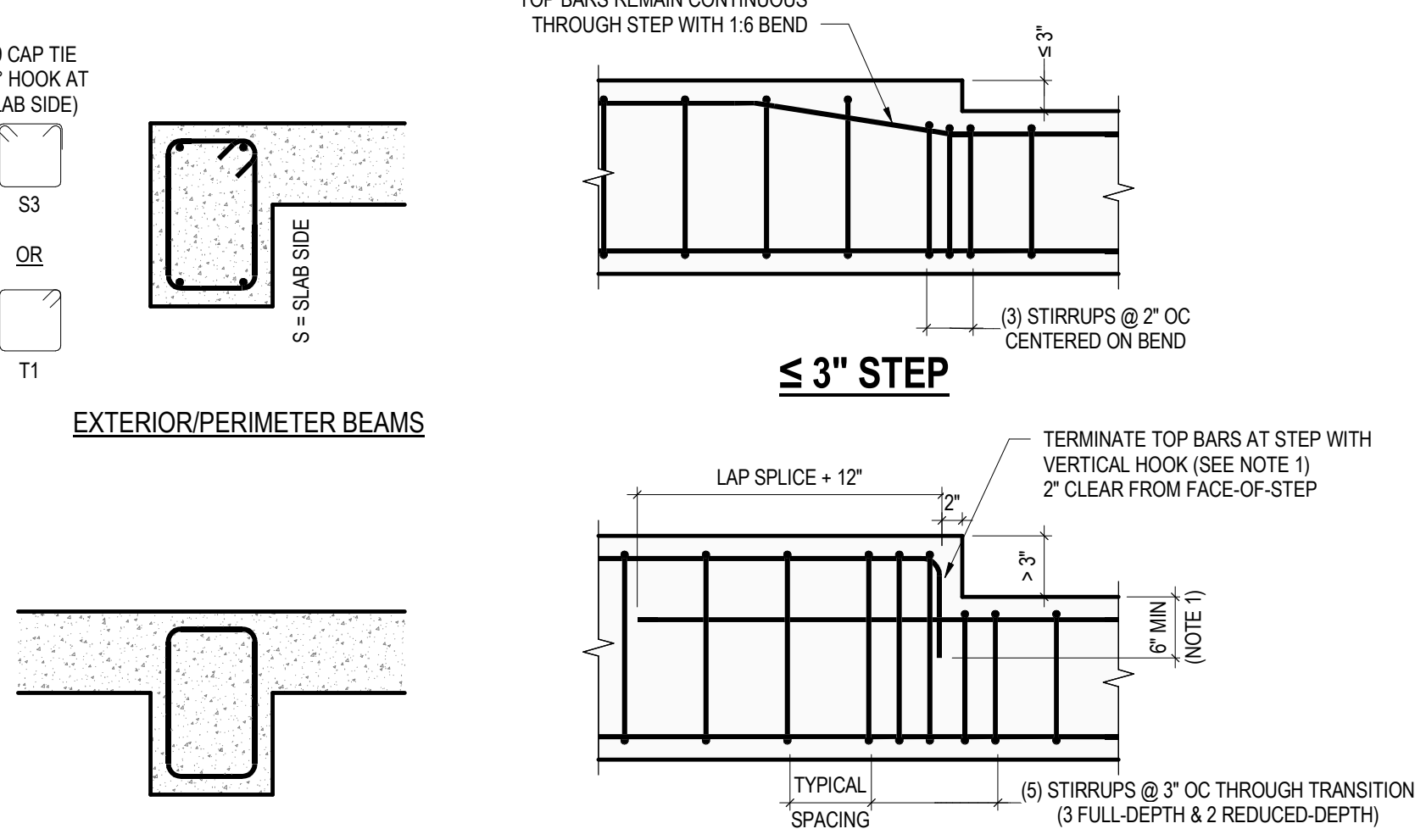
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- NOTES:
- ELEVATION AS SHOWN REPRESENTS STIRRUP ARRANGEMENT AT COLUMN SUPPORTS.
 - FOR STIRRUP ARRANGEMENT AT BEAM SUPPORT SEE TYPICAL DETAILS AT BEAM JOINT SUPPORT.
 - CONTRACTOR SHALL MINIMIZE NUMBER OF SPLICES.
 - IF SPLICES ARE REQUIRED THE SPLICES SHALL BE LOCATED WHERE INDICATED ON THIS DETAIL.



- NOTES:
- VERTICAL HOOK LENGTH AT TERMINATED TOP BARS SHALL BE SUCH THAT 6" MINIMUM LENGTH IS BELOW THE TOP-OF-REDUCED BEAM SECTION, OR A STANDARD ACI 90° HOOK LENGTH, WHICHEVER IS GREATER

CONCRETE BEAM SCHEDULE					
MARK	GEOMETRY		REINFORCEMENT		
	WIDTH	DEPTH	TOP	BOTTOM	STIRRUPS
CB3016	30"	16"	(6)-#6	(6)-#6	#3 @ 10" OC (3 LEGS)
CB5416	54"	16"	(14)-#6	(14)-#6	#3 @ 10" OC (4 LEGS)
CB7816	78"	16"	(18)-#6	(18)-#6	#3 @ 10" OC (6 LEGS)

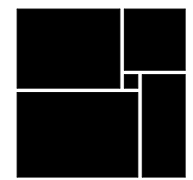
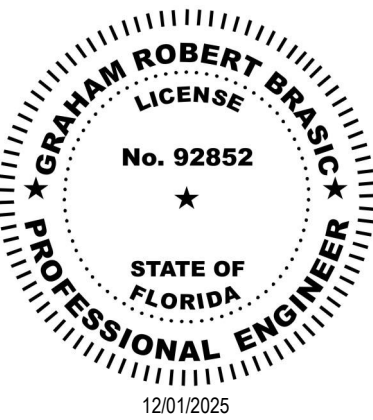
1 TYPICAL CONCRETE BEAM ELEVATION

3/4" = 1'-0"

2 BEAM STEP TRANSITION

3/4" = 1'-0"

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TYPICAL
CONCRETE
BEAM
DETAILS

S-350



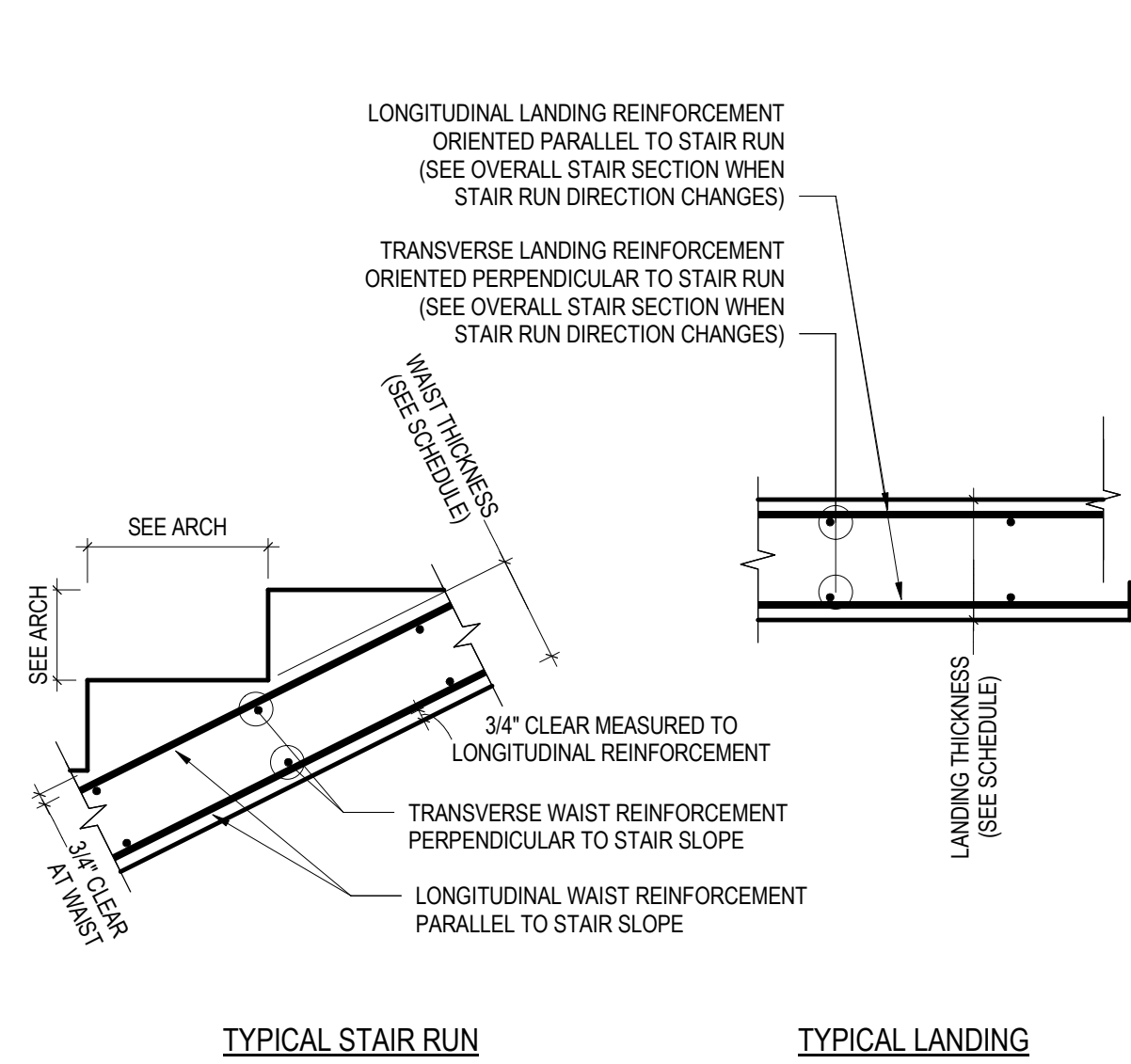
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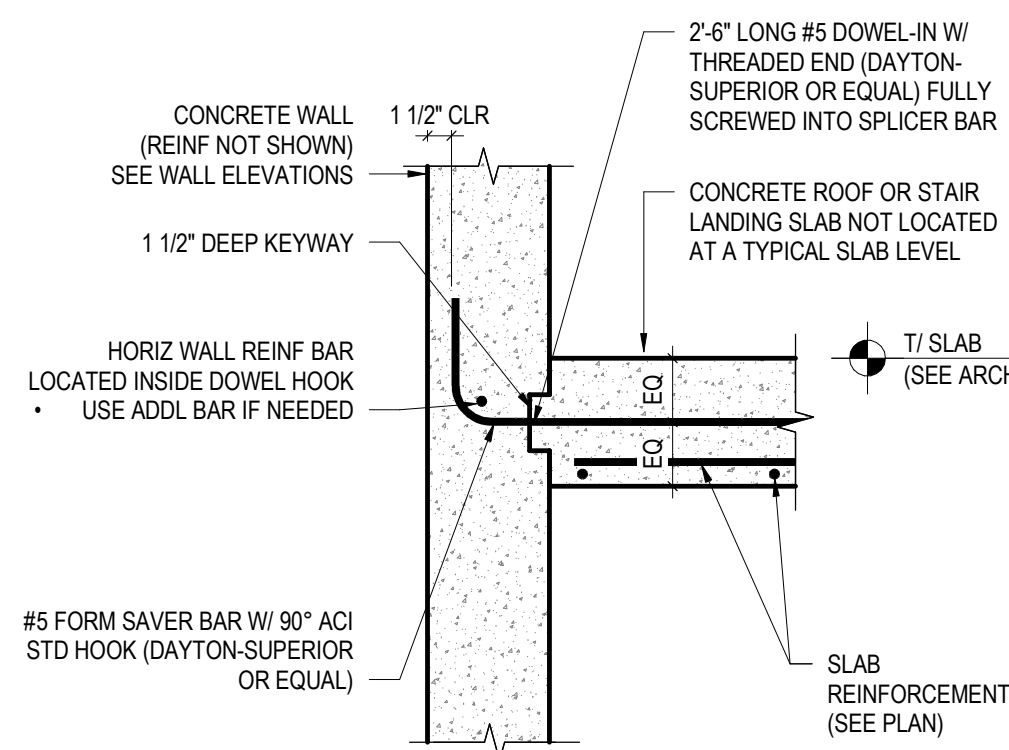
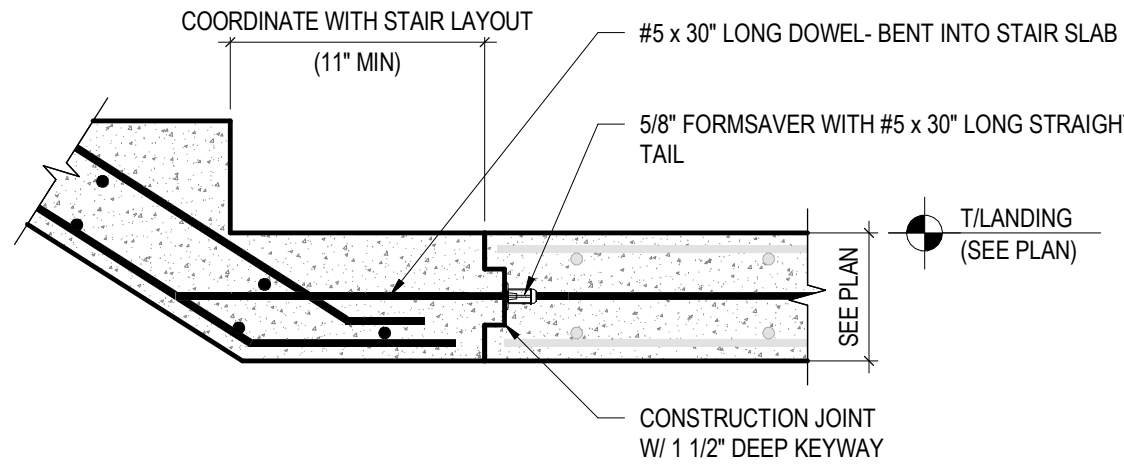
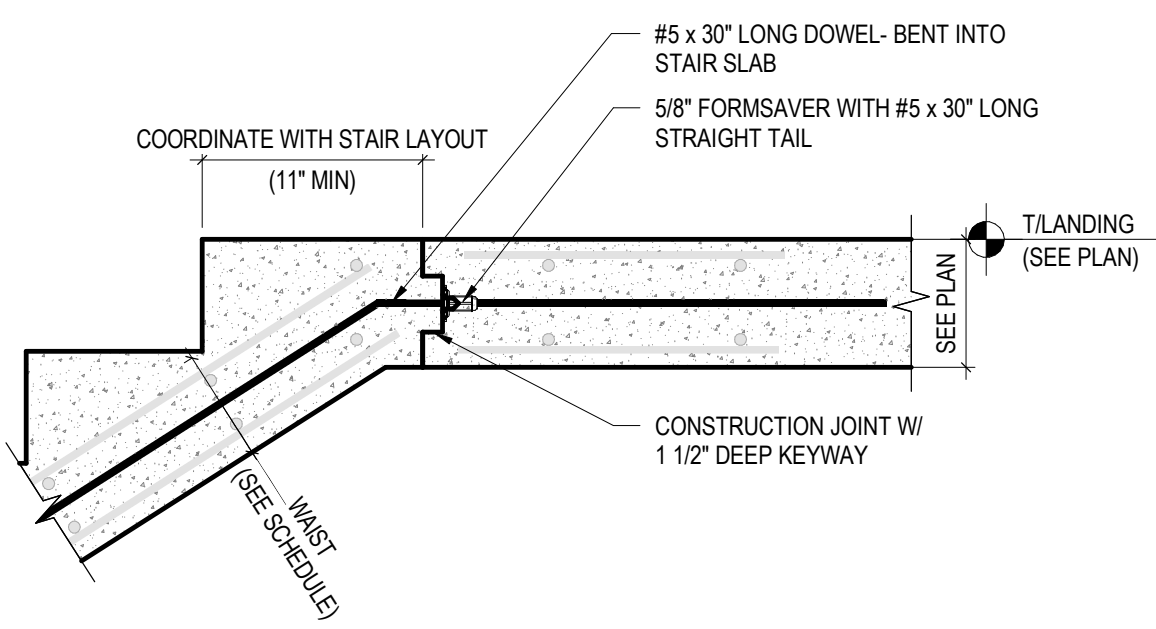
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CONCRETE STAIR SLAB SCHEDULE				
MARK (SEE SECTION)	TYPE	THICKNESS	REINFORCEMENT	
			LONGITUDINAL	TRANSVERSE
7L1	STAIR LANDING	7"	#5 AT 12" OC T&B	#5 AT 8" OC T&B
8L1	STAIR LANDING	8"	#5 AT 12" OC T&B	#5 AT 8" OC T&B
9L1	STAIR LANDING	9"	(6) - #5 T&B	(6) - #5 T&B
7W1	STAIR WAIST	7"	(6) - #5 T&B	#5 AT 18" OC T&B
8W1	STAIR WAIST	8"	(6) - #5 T&B	#5 AT 18" OC T&B

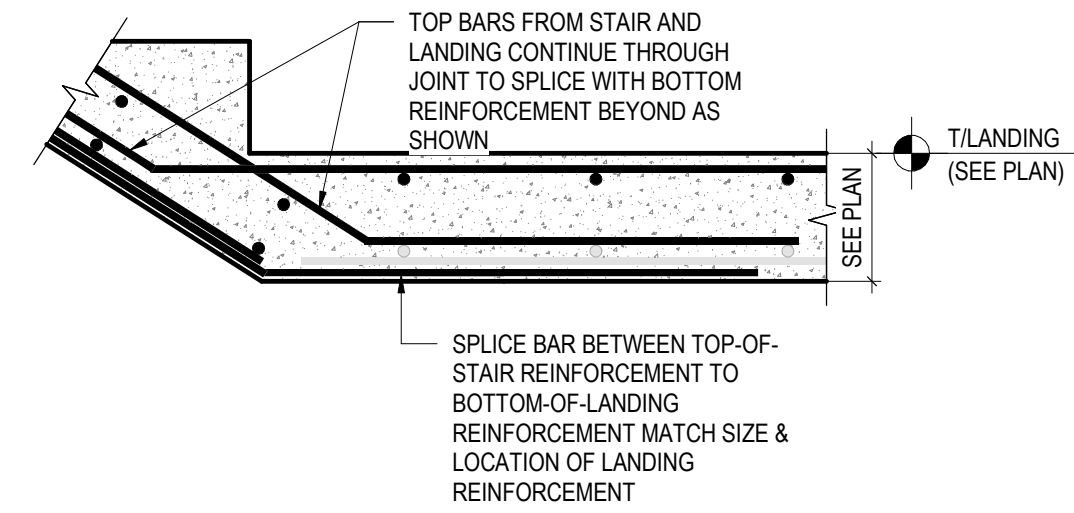
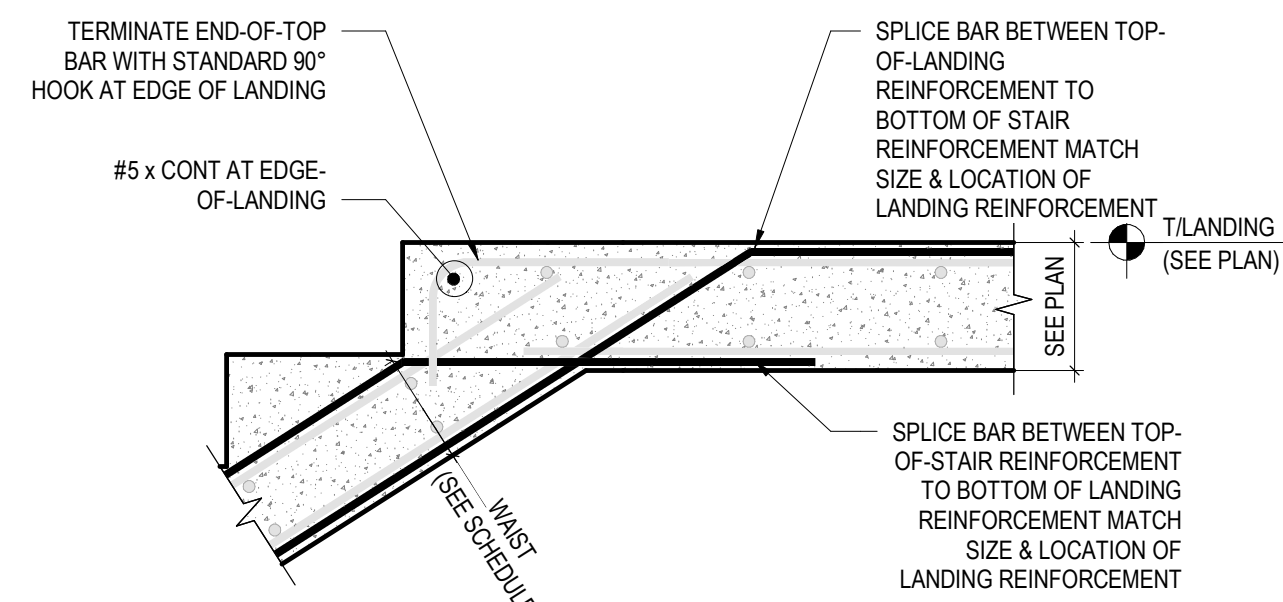
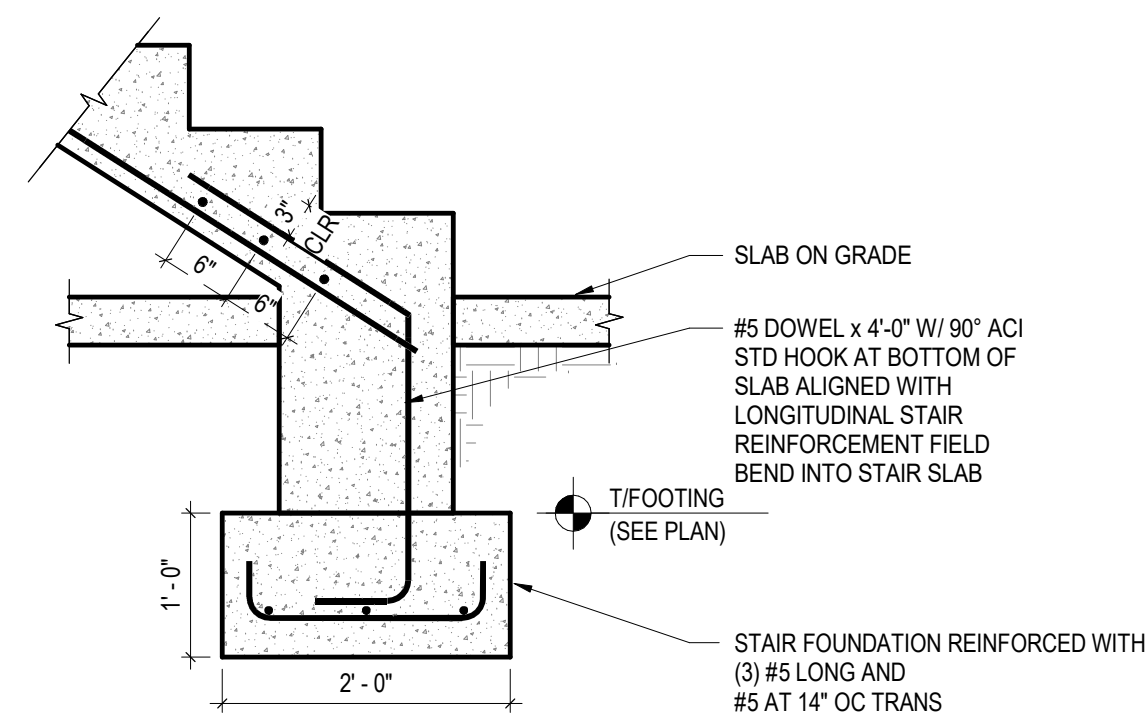


1 TYPICAL CAST-IN-PLACE CONCRETE STAIR DETAILS
1" = 1'-0"

2 STAIR DOWN TRANSITION AT LANDING
1" = 1'-0"

3 STAIR UP TRANSITION AT LANDING
1" = 1'-0"

4 INTERMEDIATE SLAB ATTACHMENT
1" = 1'-0"



5 CONCRETE STAIR AT FOUNDATION
3/4\" = 1'-0"

6 STAIR DOWN TRANSITION AT MIDHEIGHT
1" = 1'-0"

7 STAIR UP TRANSITION AT MIDHEIGHT
1" = 1'-0"



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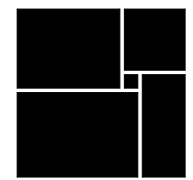
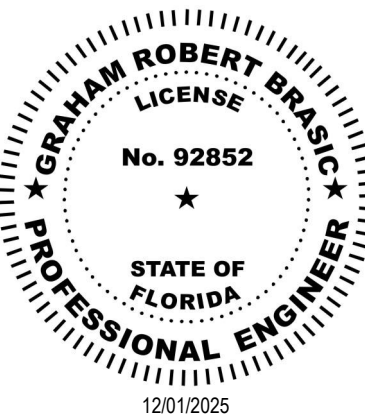
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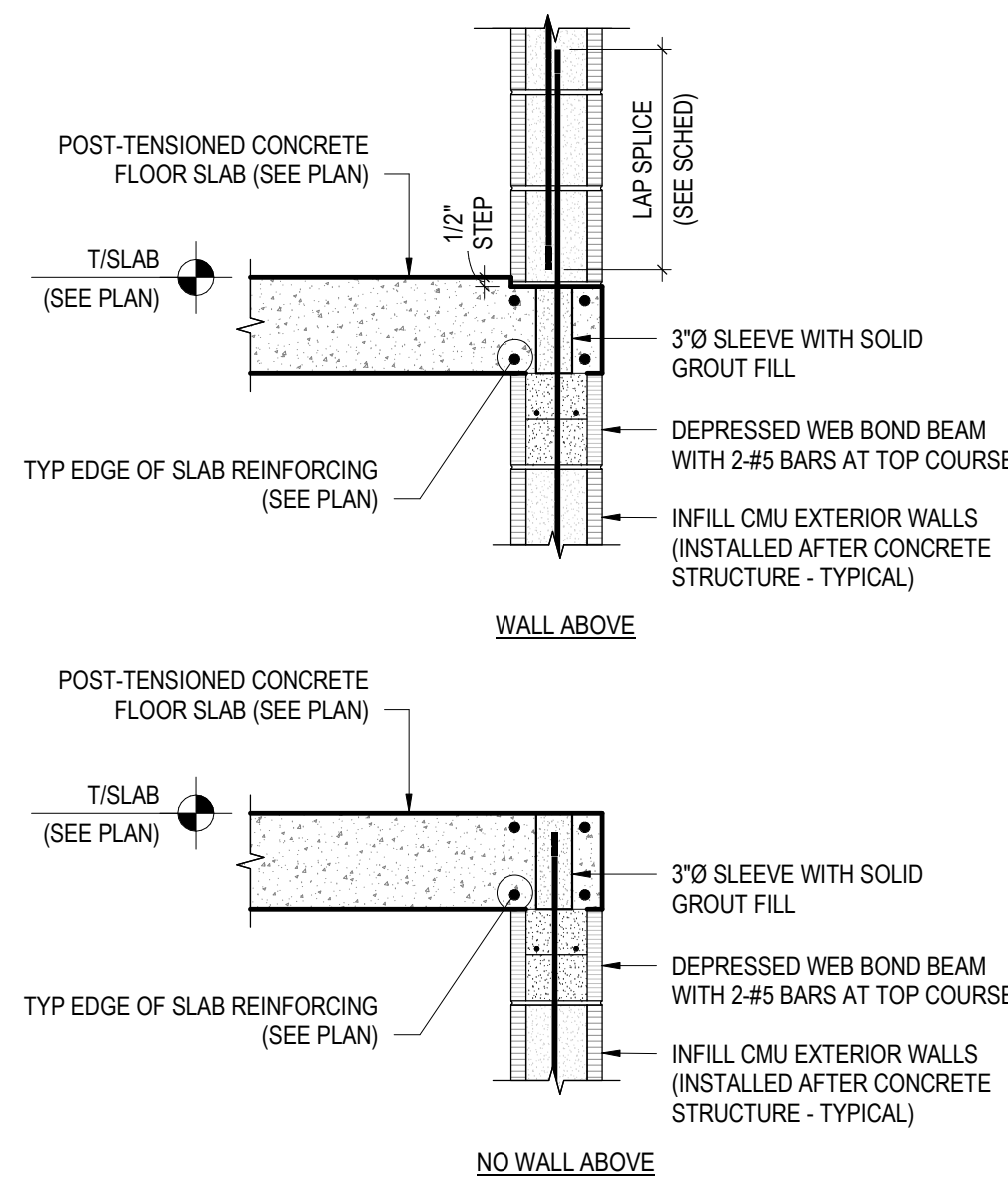
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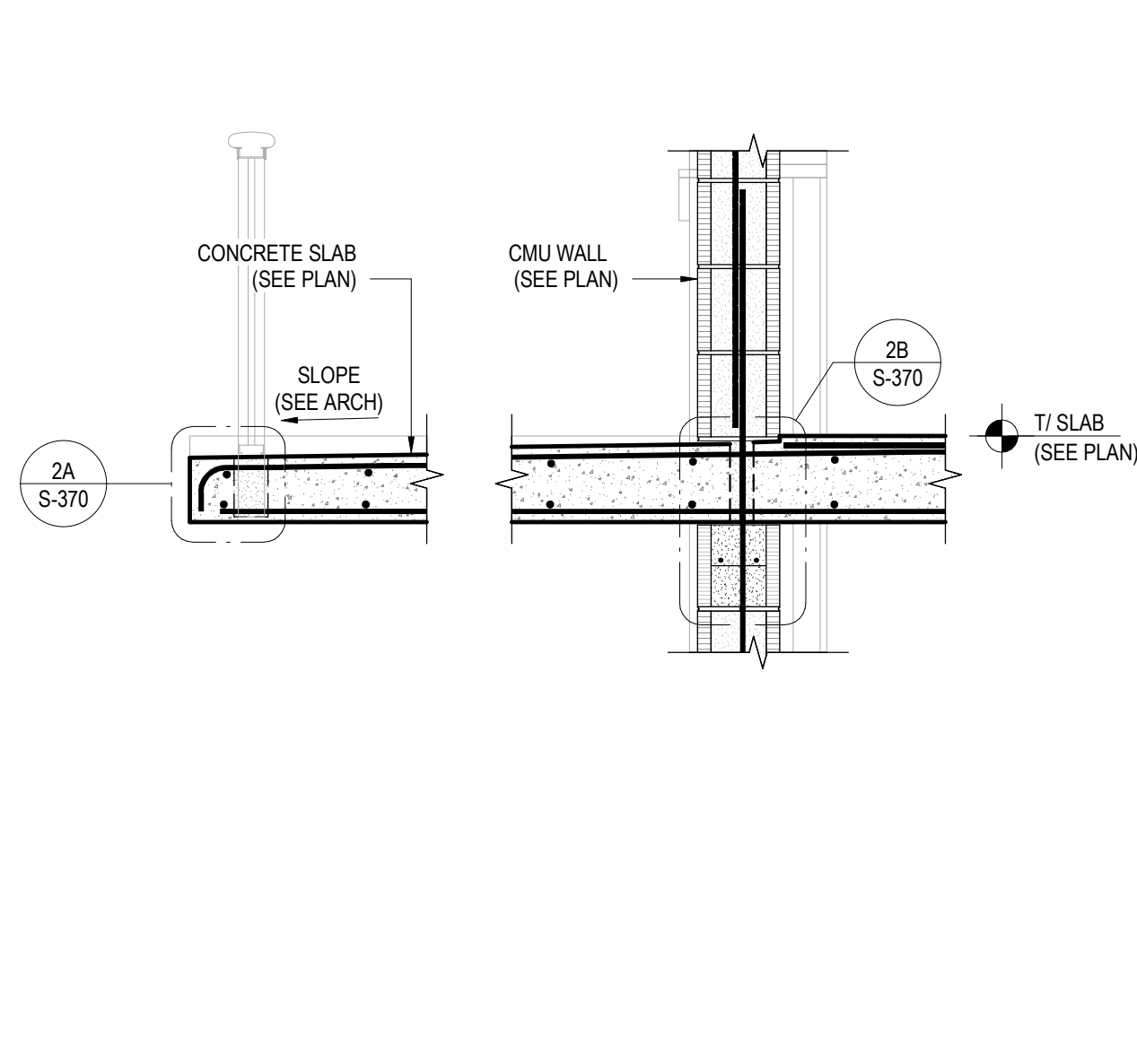
CONCRETE
STAIR
DETAILS

S-360

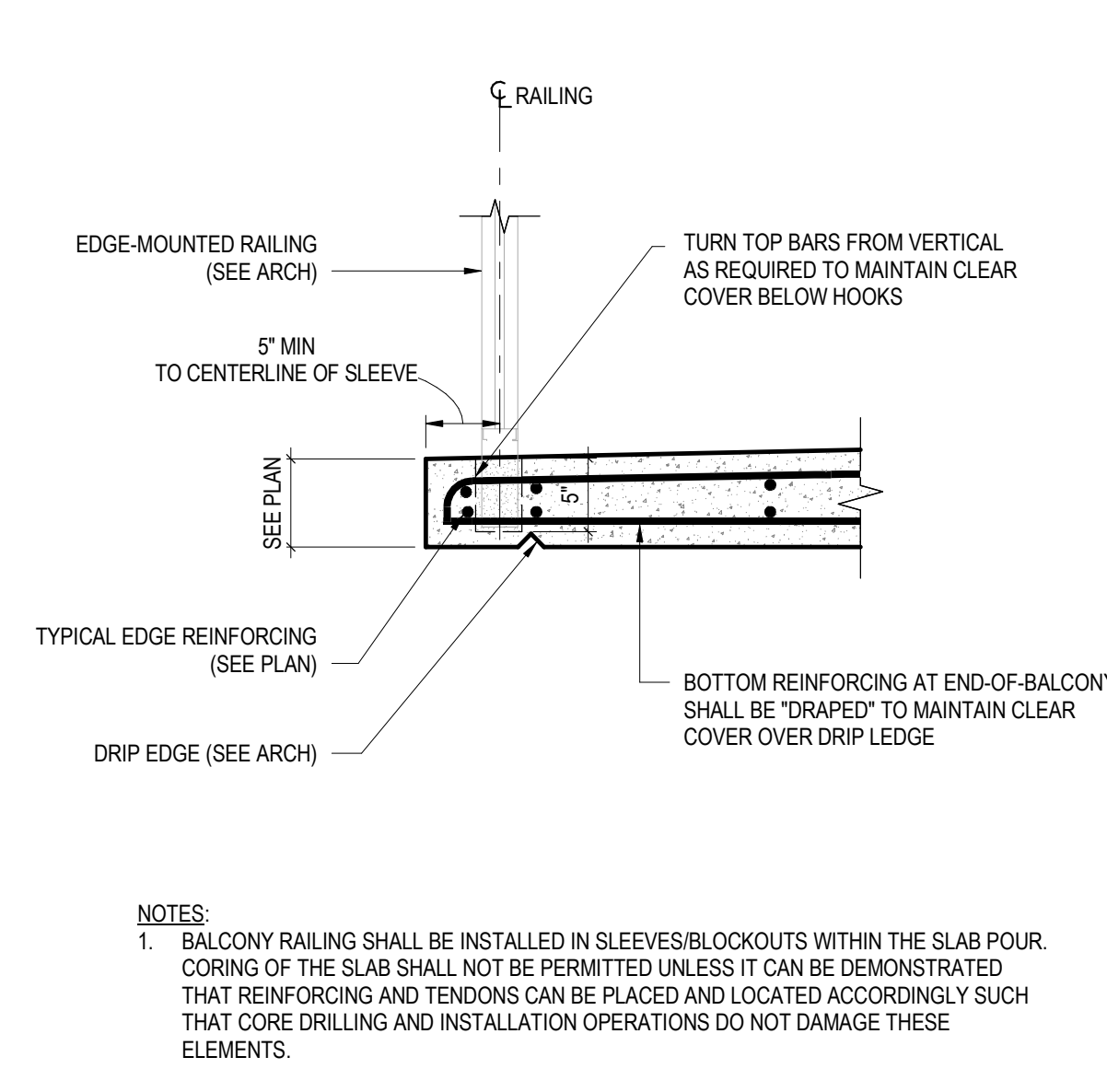
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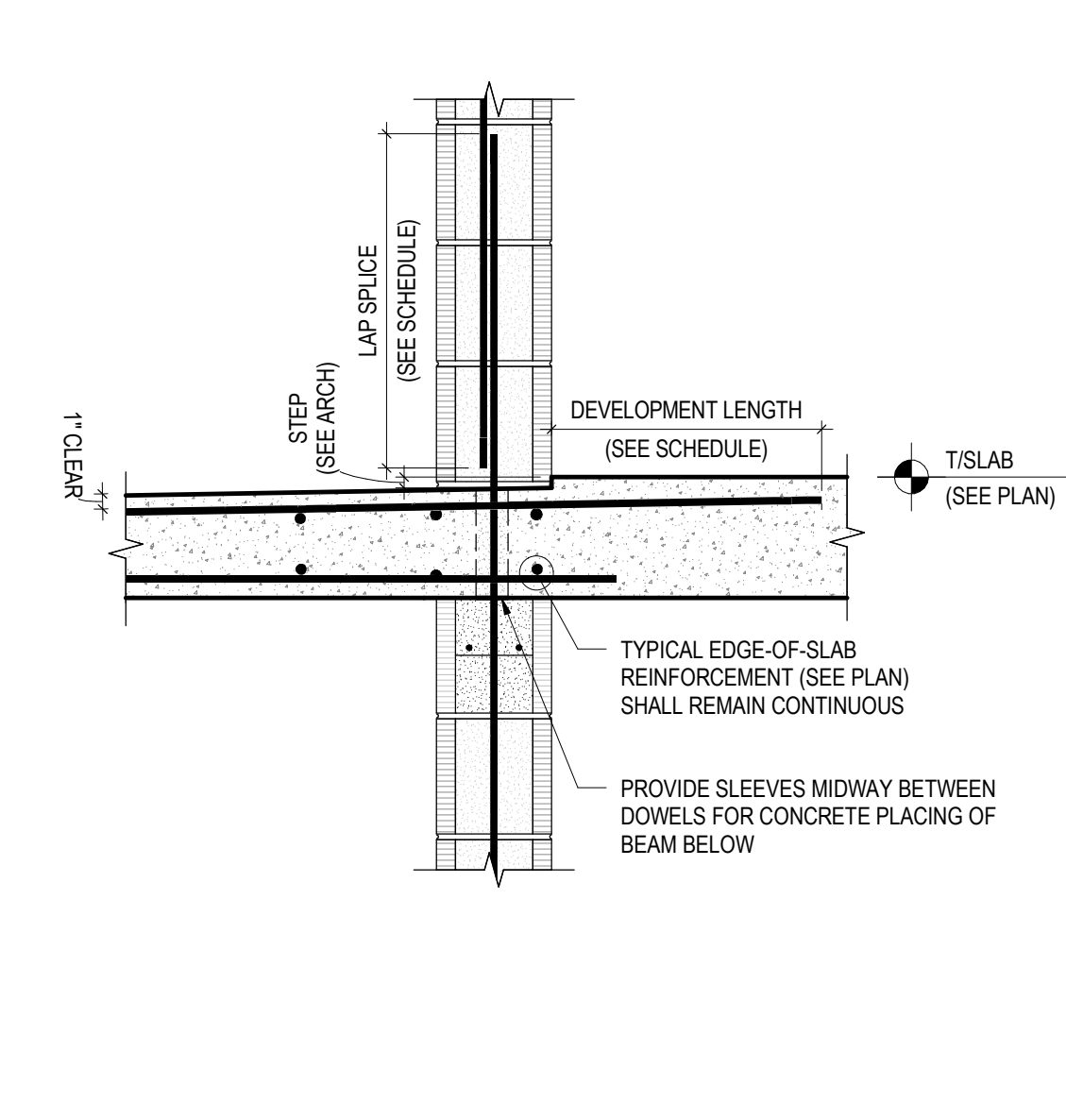
1 TYPICAL EDGE-OF-SLAB
3/4" = 1'-0"



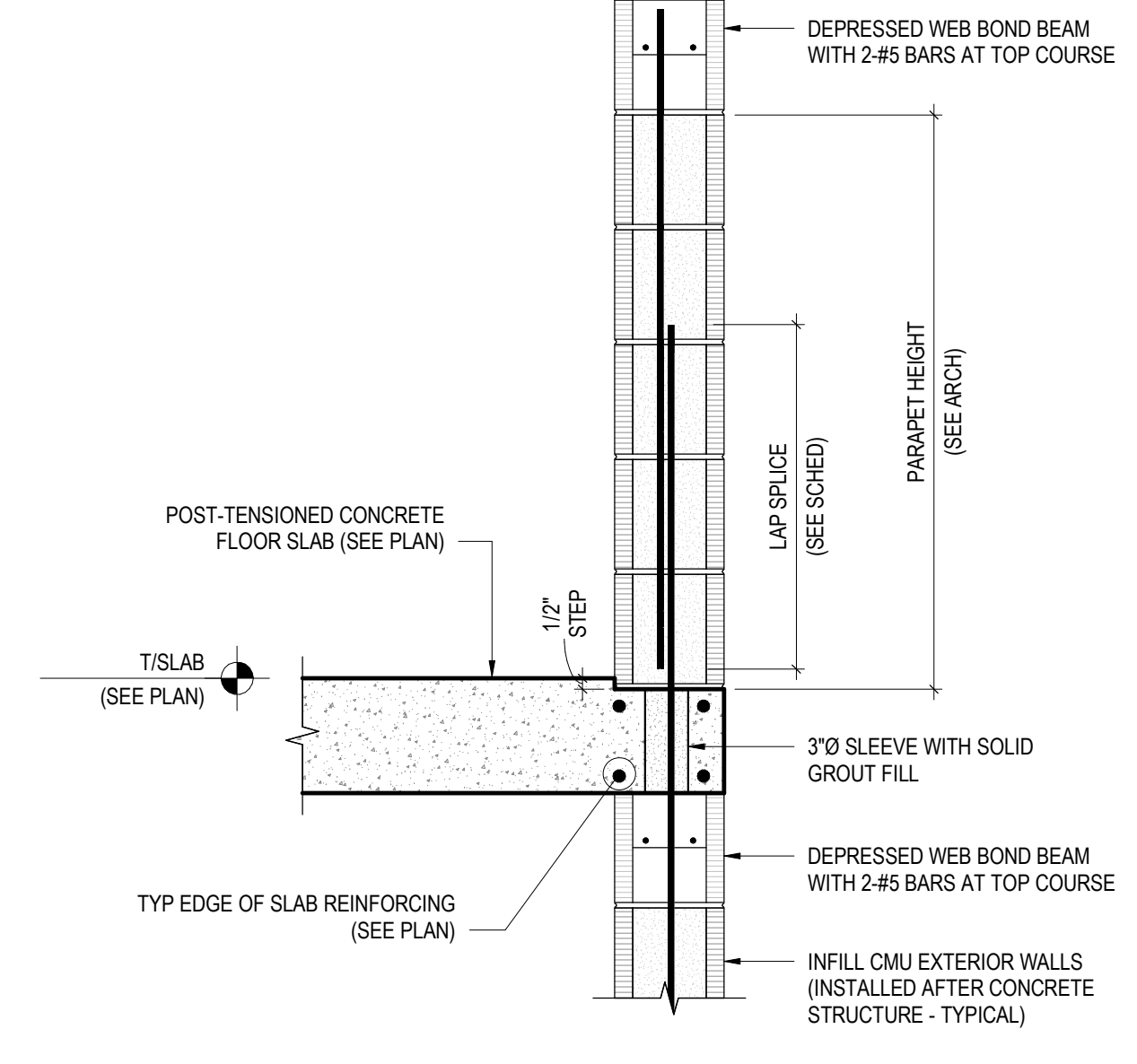
2 TYPICAL BALCONY FRAMING
3/4" = 1'-0"



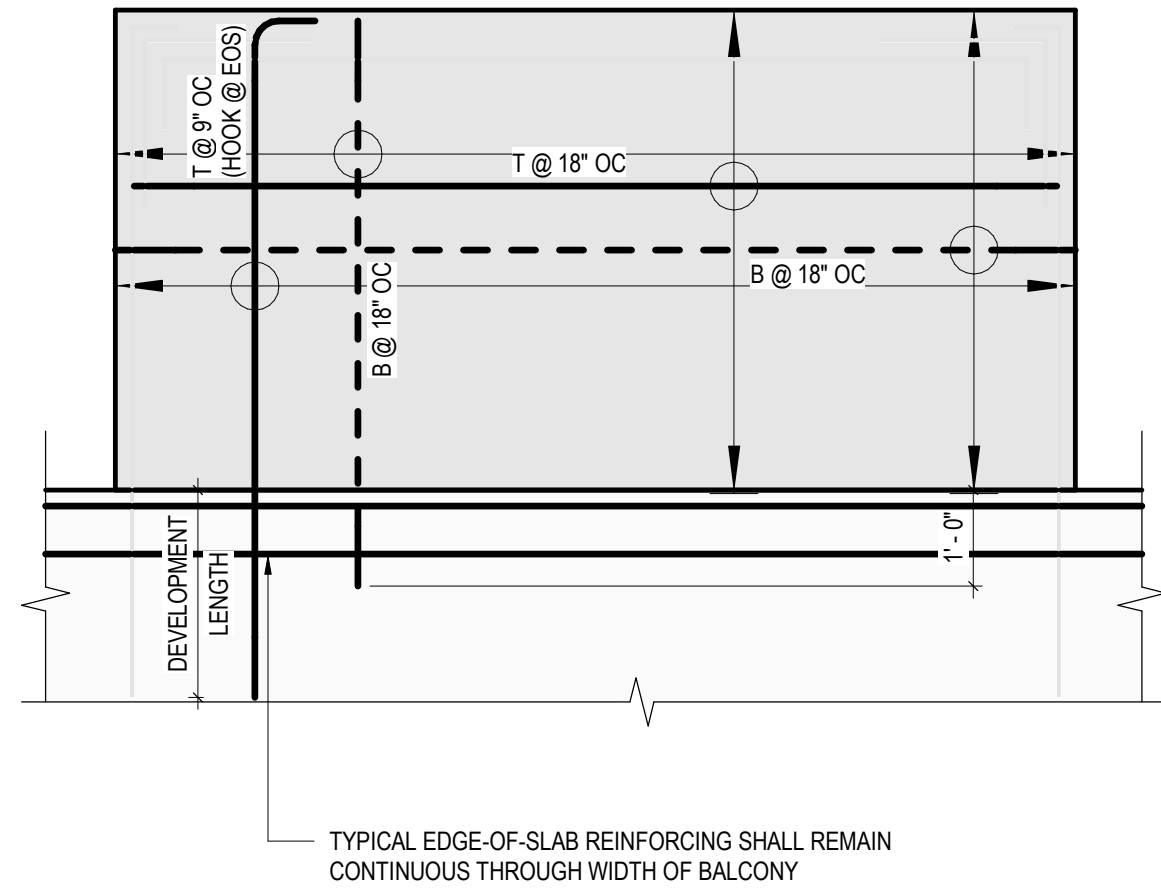
2A EDGE OF BALCONY SECTION
1" = 1'-0"



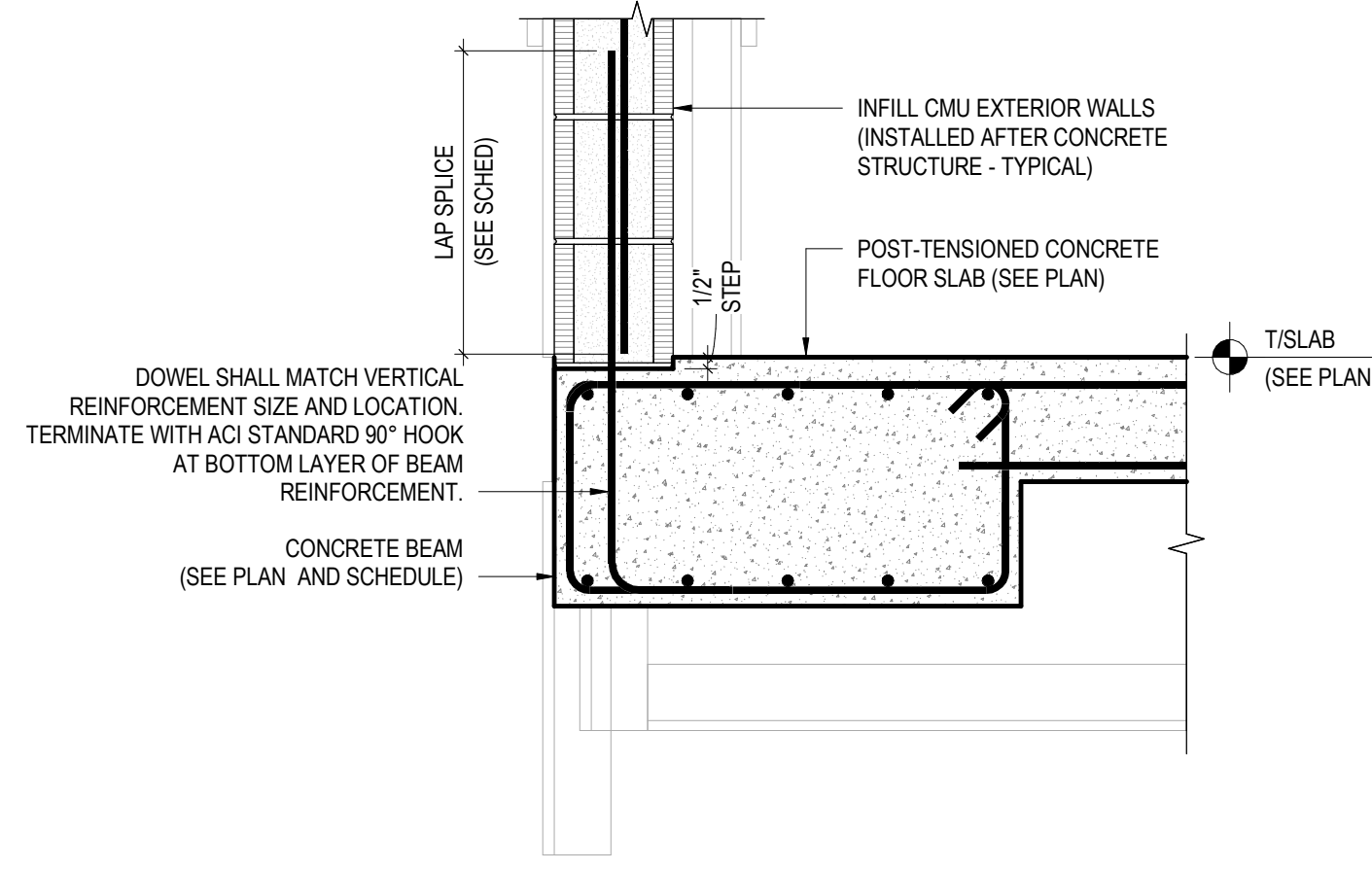
2B TYPICAL TRANSITION AT BALCONY
1" = 1'-0"



3 TYPICAL EDGE-OF-SLAB AT PARAPET
1" = 1'-0"



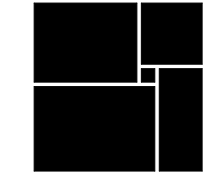
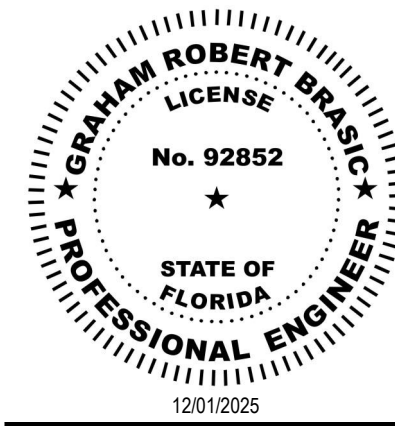
4 BALCONY SLAB REINFORCEMENT
1/2" = 1'-0"



5 CONCRETE BEAM AT EDGE-OF-SLAB - SECOND LEVEL
1" = 1'-0"

NOTES:
1. BALCONY RAILING SHALL BE INSTALLED IN SLEEVES/BLOCKOUTS WITHIN THE SLAB POUR. CORING OF THE SLAB SHALL NOT BE PERMITTED UNLESS IT CAN BE DEMONSTRATED THAT REINFORCING AND TENDONS CAN BE PLACED AND LOCATED ACCORDINGLY SUCH THAT CORE DRILLING AND INSTALLATION OPERATIONS DO NOT DAMAGE THESE ELEMENTS.

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TYPICAL
CONCRETE
FRAMING
DETAILS

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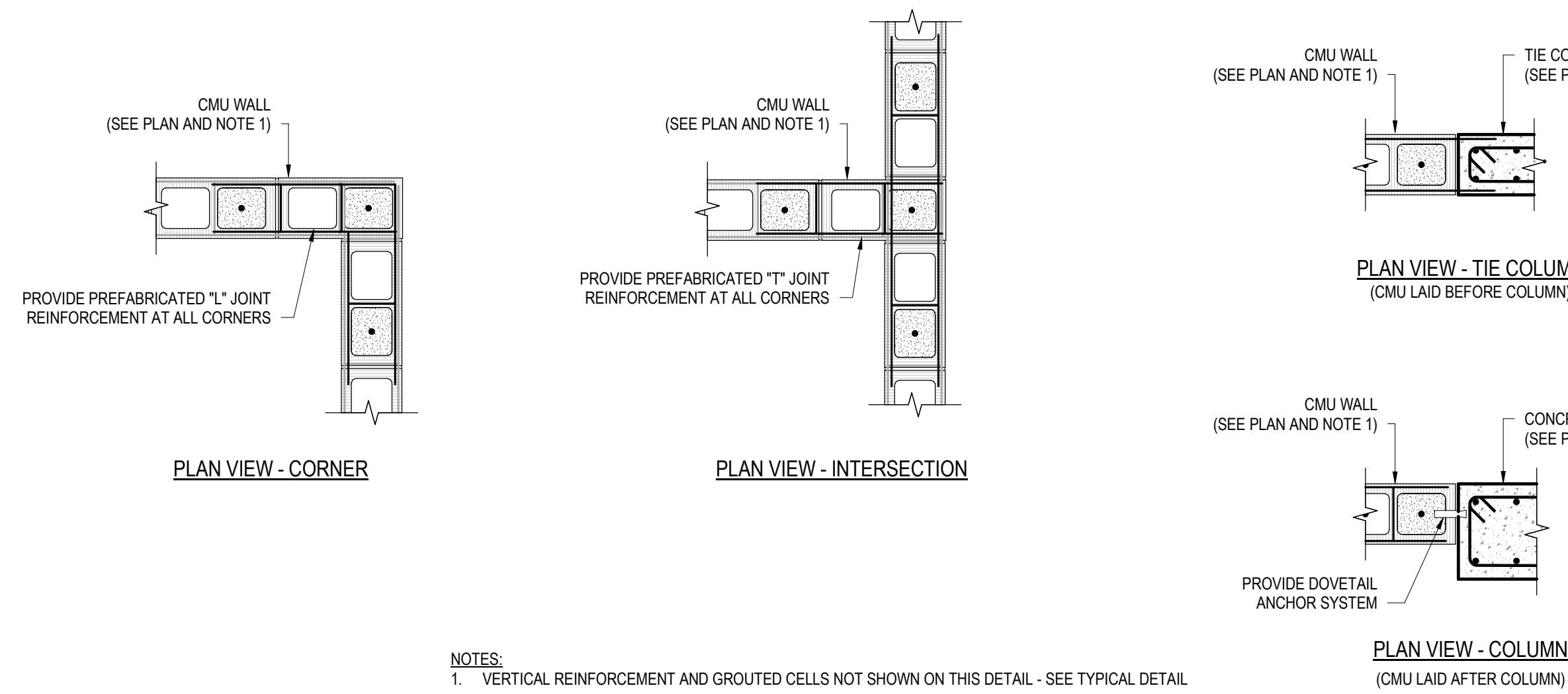
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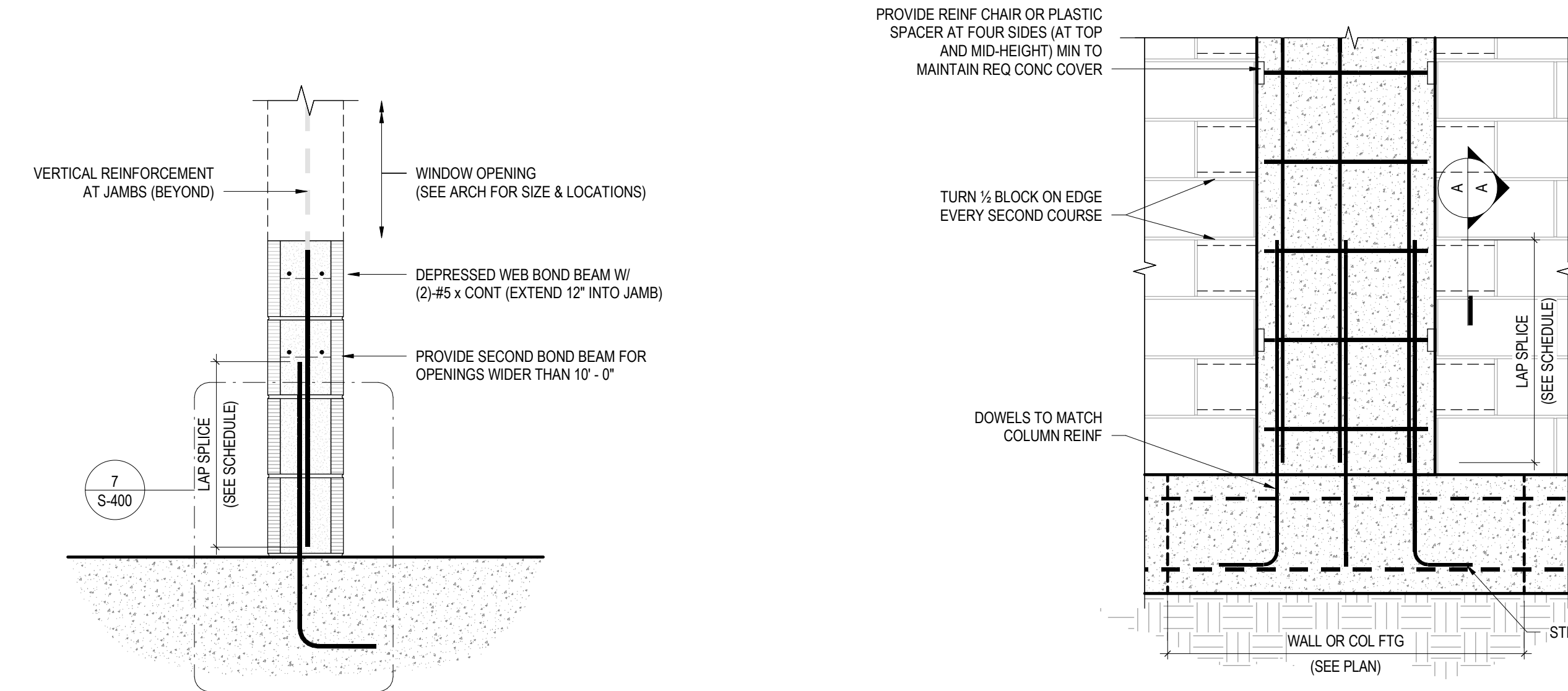
1 CMU WALL CONSTRUCTION - PERSPECTIVE

1/2" = 1'-0"



4 CMU JOINT REINFORCEMENT

3/4" = 1'-0"



8 CMU SILL - SECTION

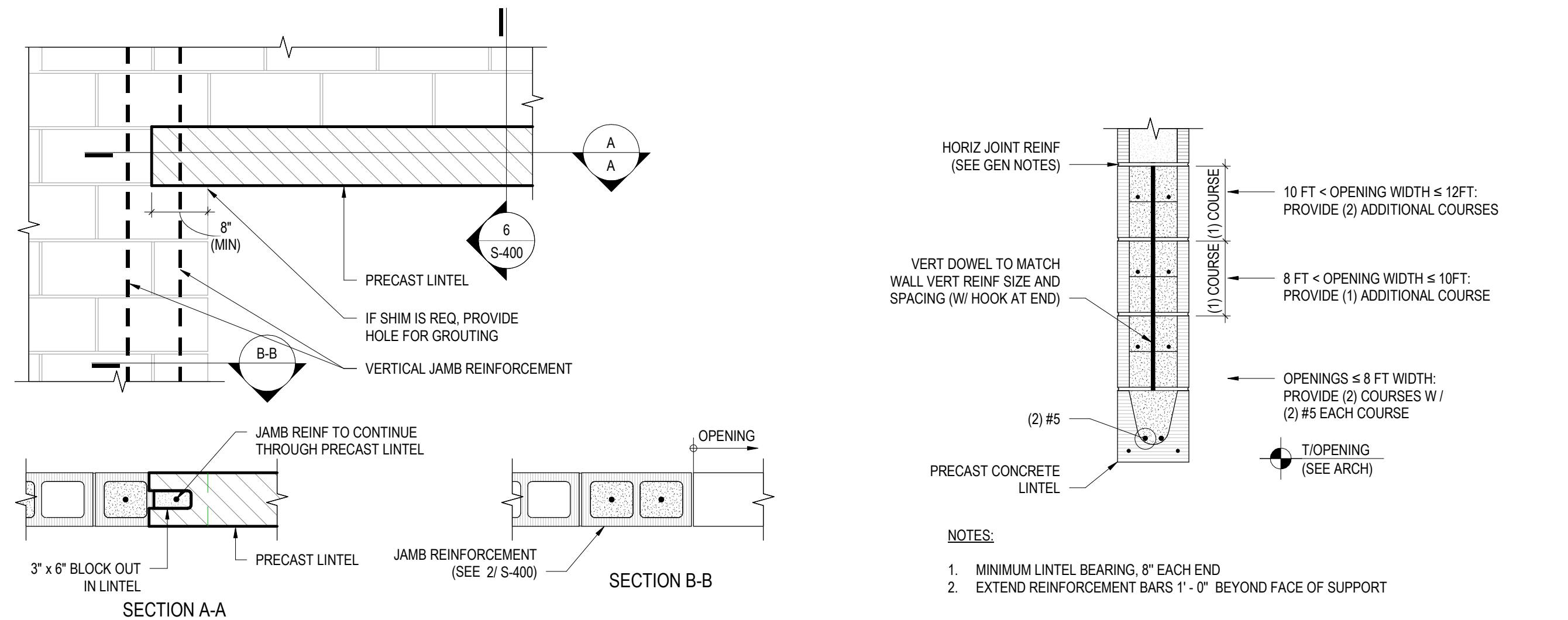
1" = 1'-0"

9 TYPICAL CONCRETE TIE COLUMN

3/4" = 1'-0"

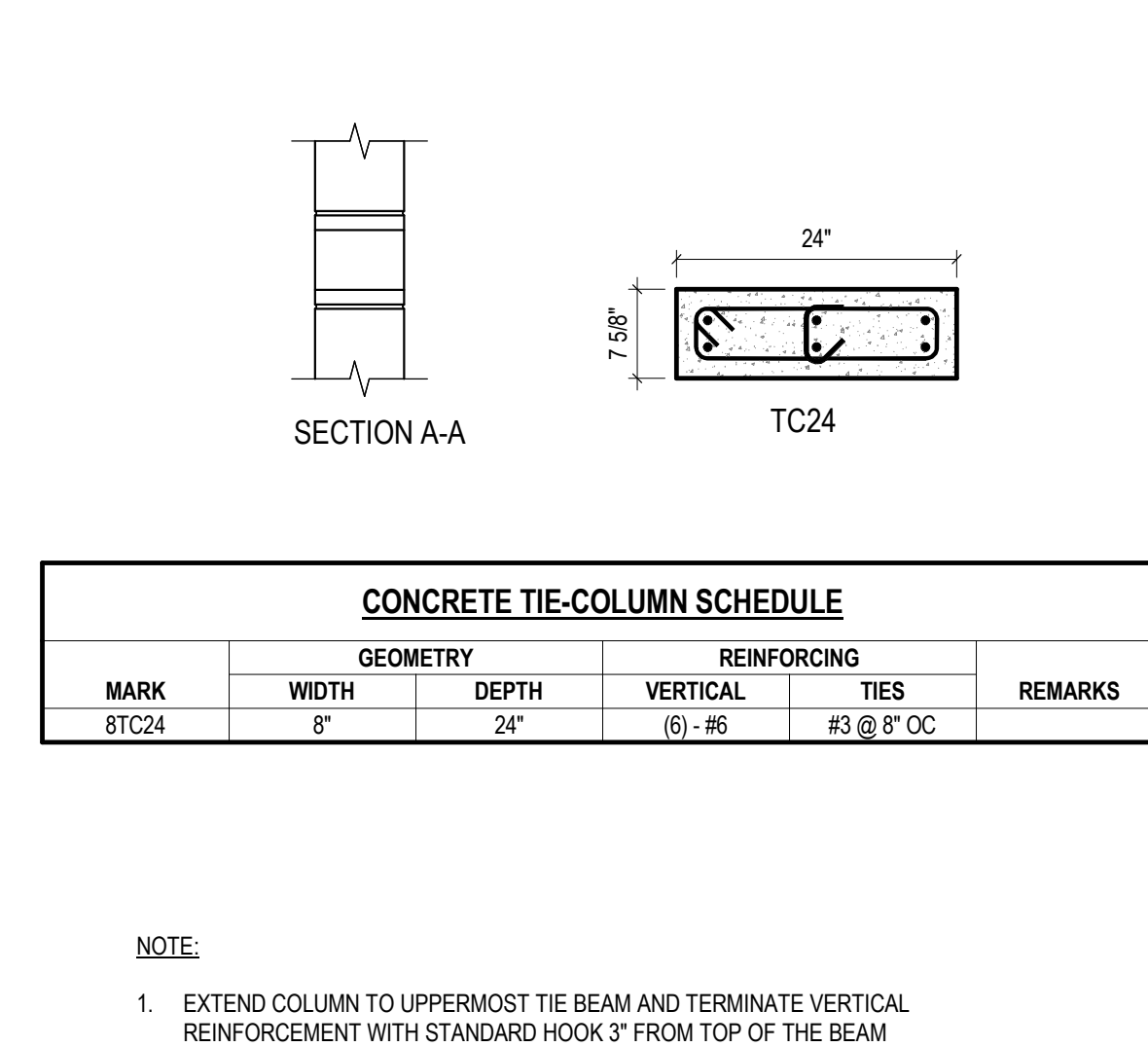
2 CMU WALL REINFORCEMENT

3/4" = 1'-0"



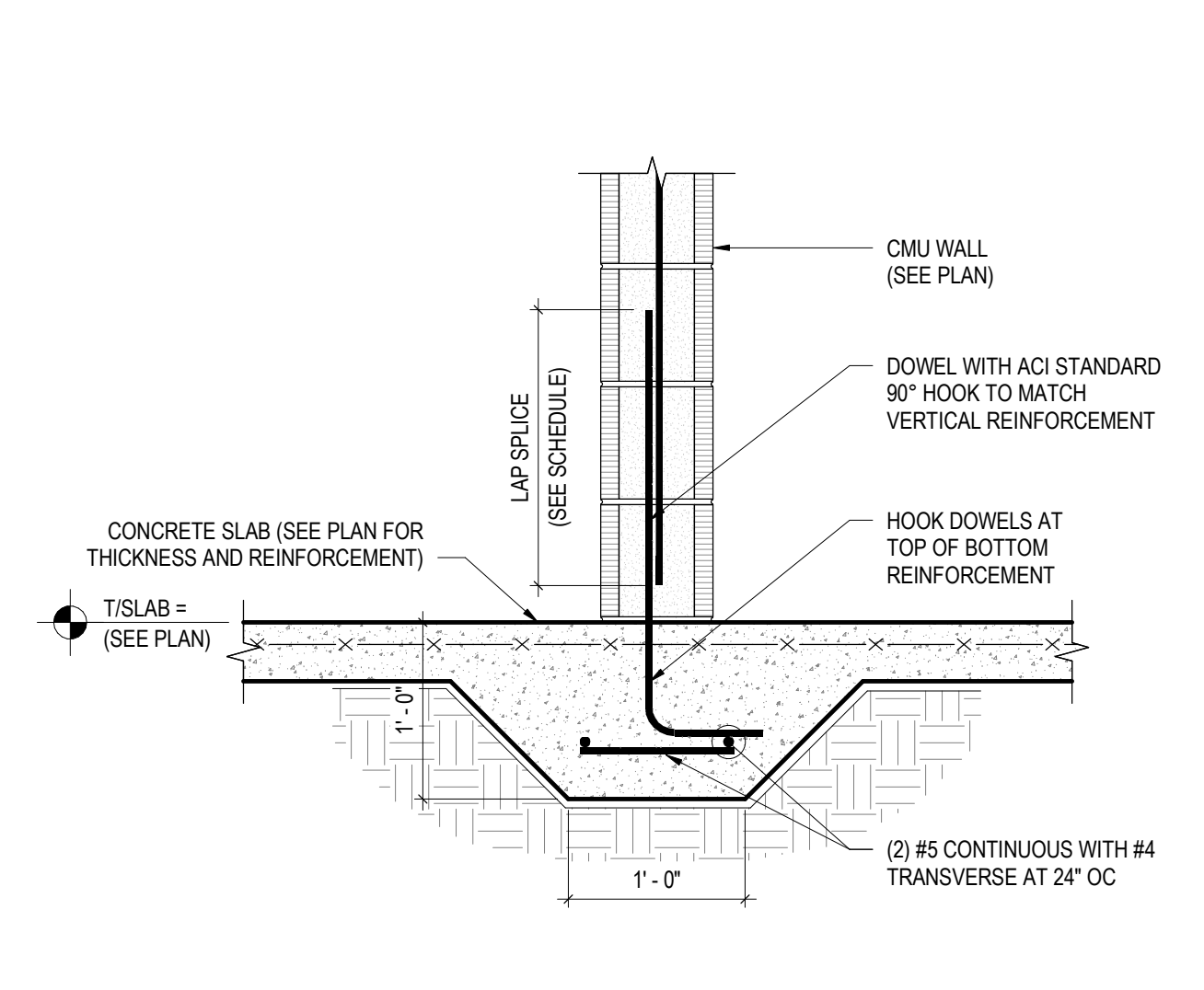
5 CMU PRECAST LINTEL

3/4" = 1'-0"



6 CMU LINTEL AT OPENING

1" = 1'-0"

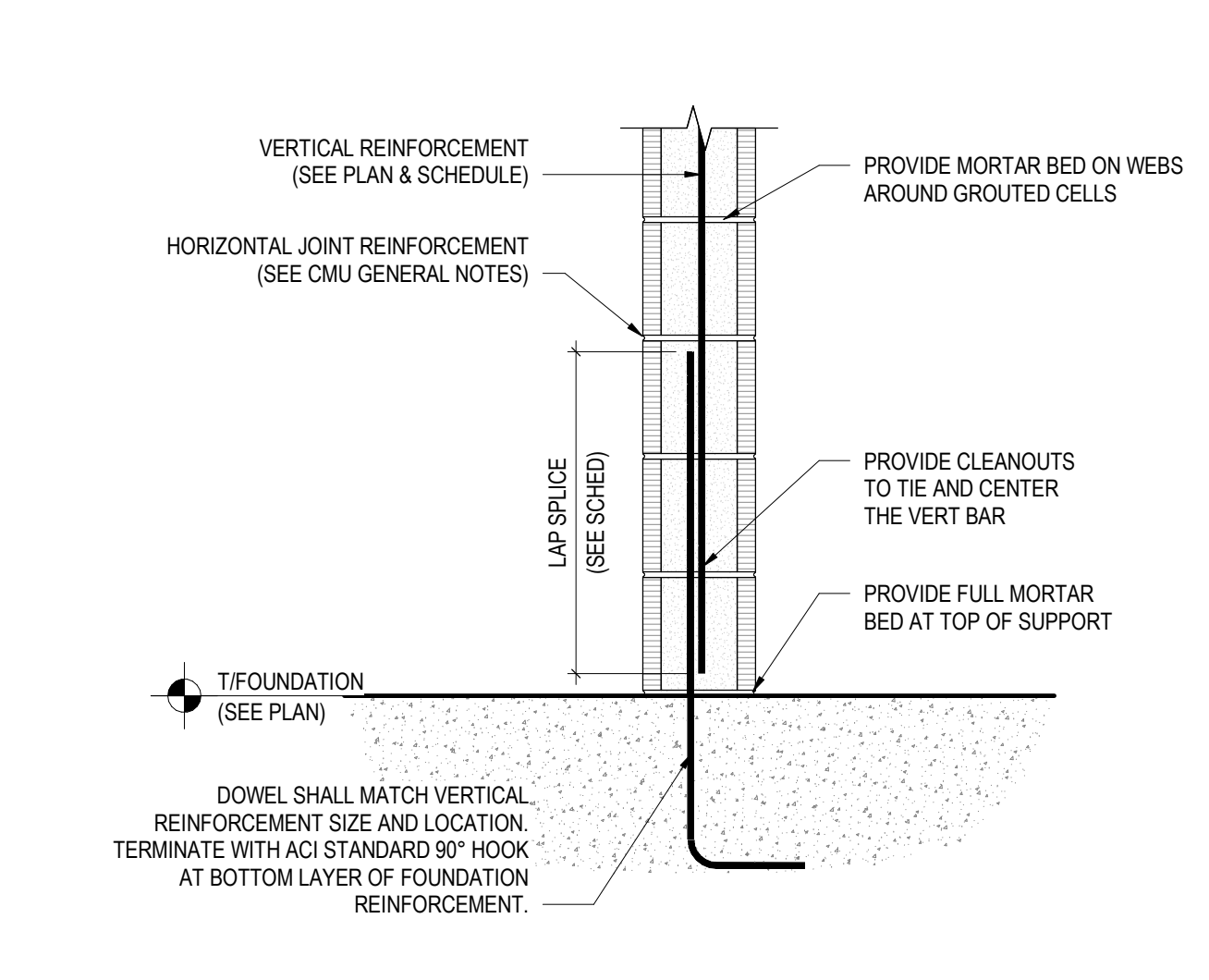


10 THICKENED SLAB AT CMU WALL

1" = 1'-0"

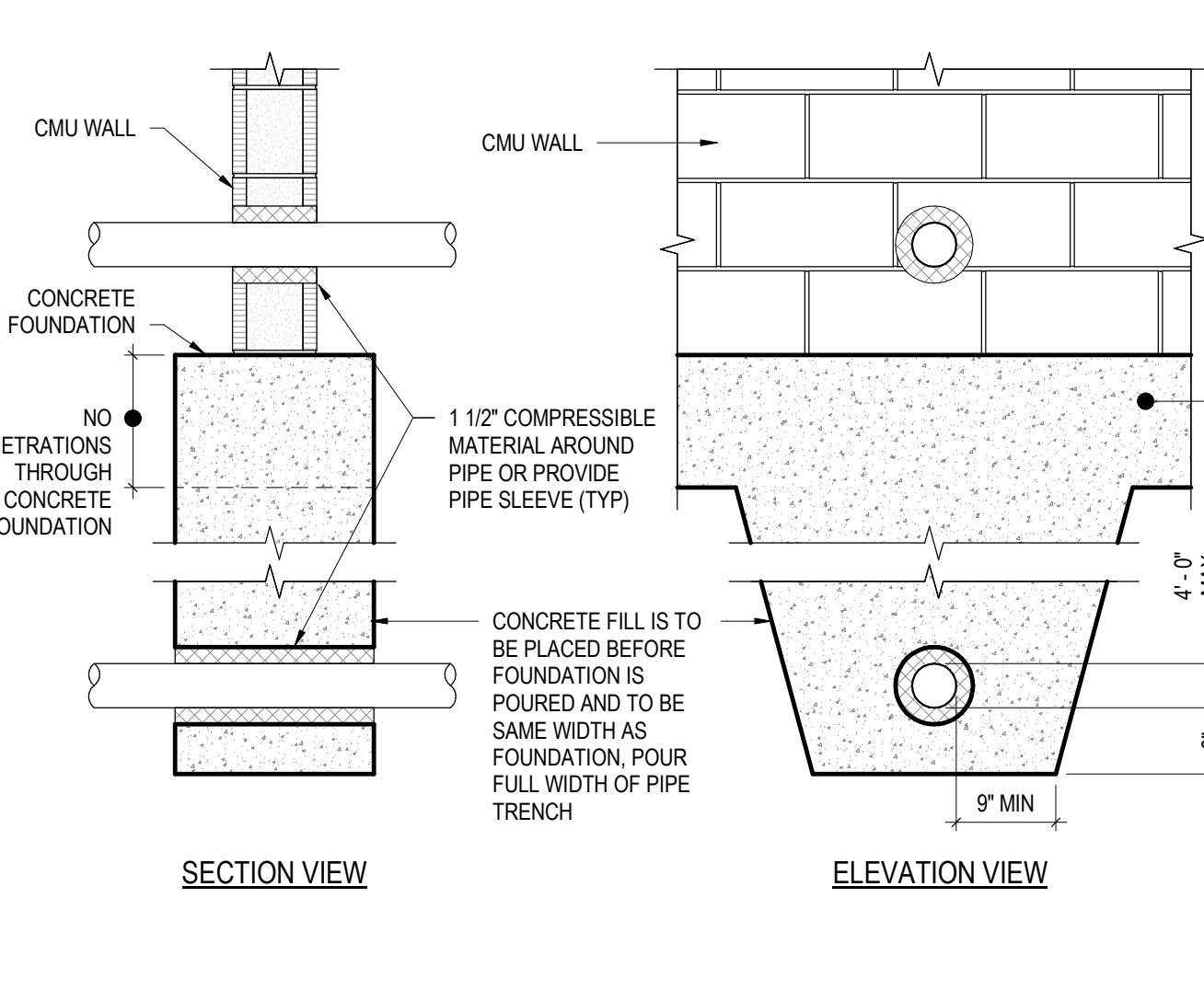
3 CMU CONTROL JOINTS

1" = 1'-0"



7 BASE-OF-CMU WALL CONNECTION

1" = 1'-0"

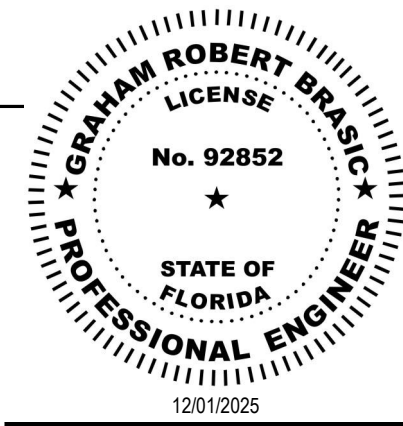


11 PIPE PENETRATION THROUGH WALL OR BELOW FOUNDATION

3/4" = 1'-0"



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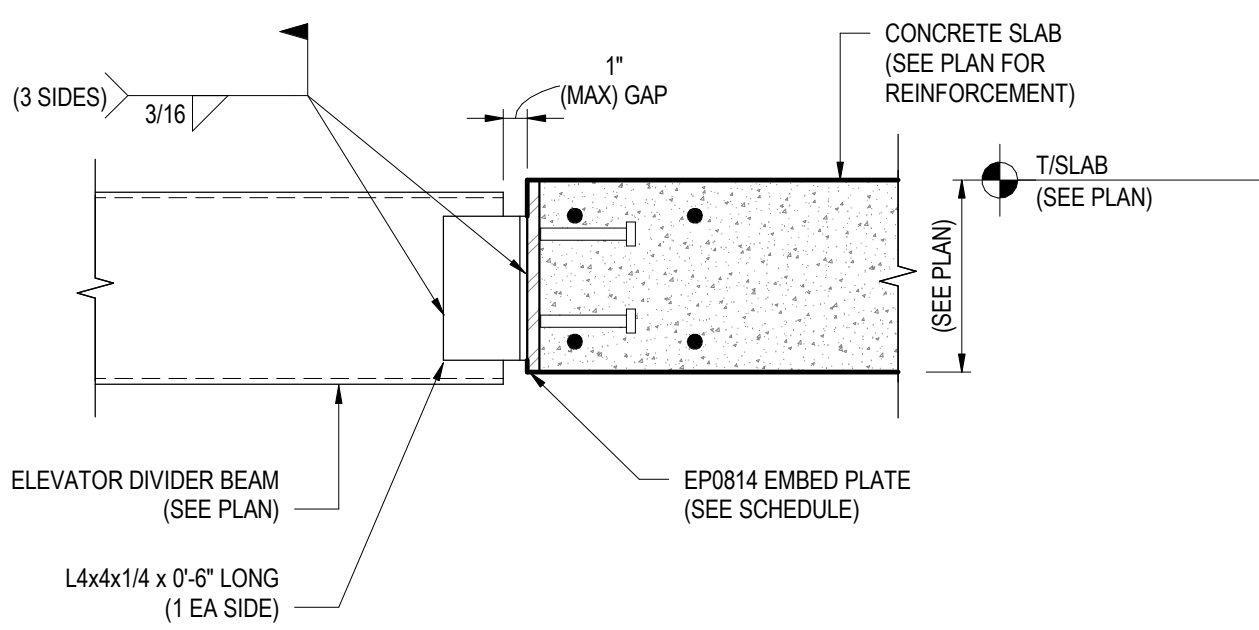
TYPICAL
MASONRY
DETAILS

S-400

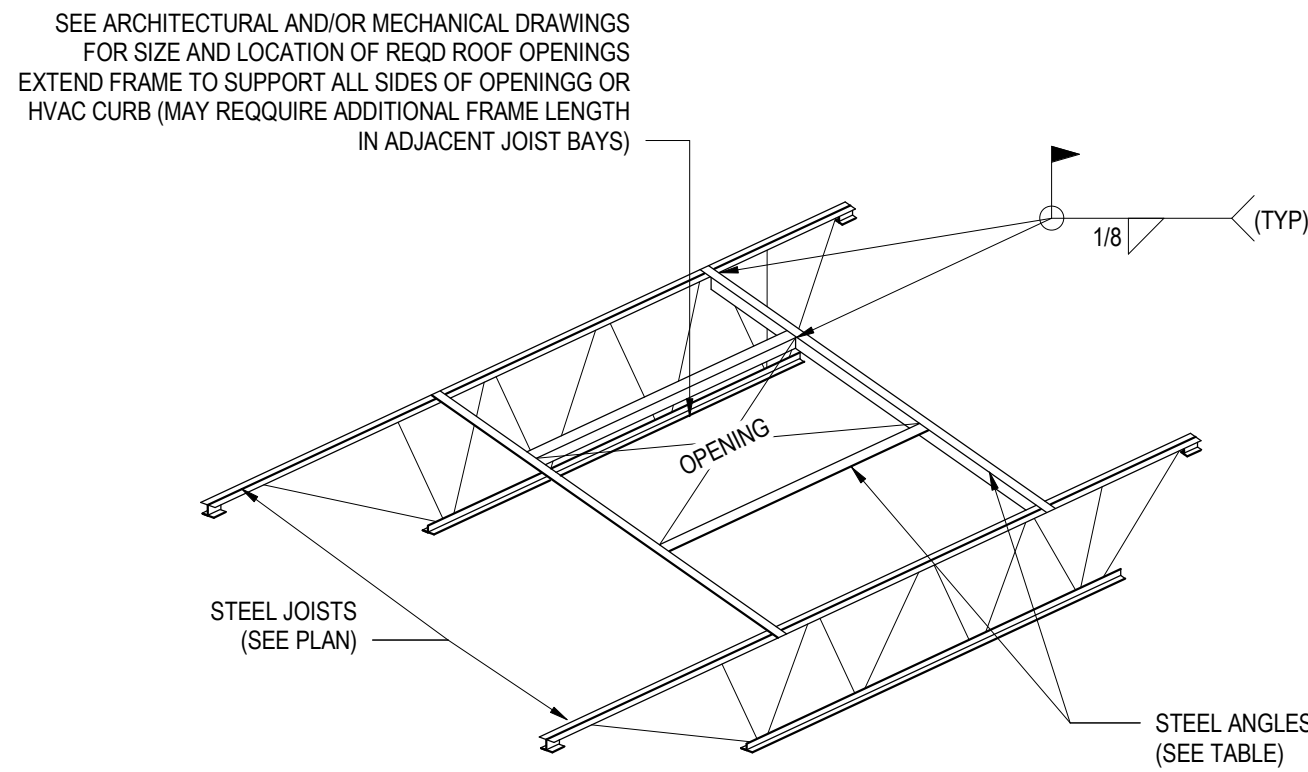


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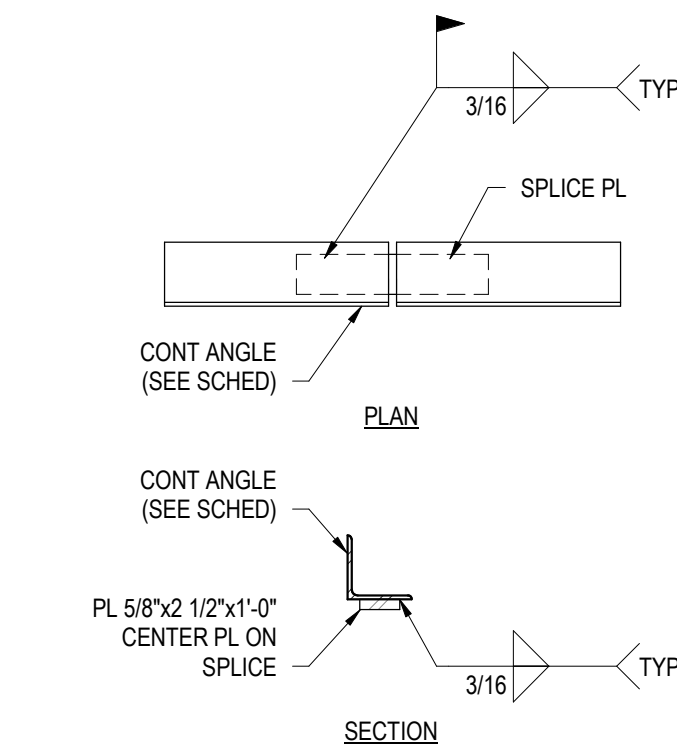
1
S-500
NTS
DIVIDER BEAM CONNECTION



2
S-500
3/8" = 1'-0"
TYPICAL ROOF OPENING/HVAC SUPPORT FRAMING

SPAN	ANGLE SIZE
UP TO 4'-0"	L3X3X3/16
4'-1" TO 6'-0"	L4X3X1/4 (LLV)
6'-1" TO 8'-0"	L5X3X1/4 (LLV)
8'-1" TO 10'-0"	L6X4X5/16 (LLV)
HVAC UNIT SUPPORT	SEE 9/S-510

- NOTE:
1. COPE VERTICAL LEG OF BEARING ANGLE 3" (TYPICAL)
- NOTES:
1. PROVIDE SIMILAR FRAMING AT ROOF DRAINS.
 2. WELD/CONNECT DECK TO ALL FRAMING AT 8" OC MAXIMUM SPACING. COORDINATE WITH ENGINEER FOR SPECIAL DETAILS REQUIRED TO CONNECT ANGLES TO EXISTING DECK IN AN INSTALLATION OF NEW UNIT ON EXISTING ROOF FRAMING.
 3. IN AN INSTALLATION OF NEW UNIT ON EXISTING ROOF FRAMING, INSTALL L3x3x1/4 VERTICAL ANGLE WITHIN WEB OF EXISTING JOIST AND WELD END OF UNIT/OPENING SUPPORT ANGLE TO VERTICAL ANGLE WITH 4" OF 3/16" FIELD FILLET WELD. SEE TYPICAL JOIST WEB REINFORCEMENT DETAIL FOR REINFORCEMENT FOR POINT LOADS NOT AT JOIST CHORD PANEL POINT LOCATIONS.
 4. WHEN JOIST BRIDGING CONFLICTS WITH ROOF OPENING FRAMES, STOP BRIDGING AT EACH SIDE OF OPENING, PROVIDE CROSS BRIDGING AT LAST BRIDGING SPACE EACH SIDE OF OPENING AND CONNECT ENDS OF BRIDGING TO OPENING FRAMING. ADD ADDITIONAL BRIDGING AND CROSS BRIDGING ON EACH SIDE OF OPENING ON EACH SIDE OF CUT BRIDGING AREA WITH BRIDGING EXTENDED ONE BAY BEYOND EACH SIDE OF OPENING.
 5. ANCHOR EQUIPMENT AND CURB TO MISCELLANEOUS FRAMING SHOWN AS REQUIRED FOR WIND AND/OR SEISMIC FORCES. COORDINATE WITH EQUIPMENT/CURB MANUFACTURER (CONNECTION DESIGN BY OTHERS).



- NOTES:
1. PROVIDE IN ALL DECK ANGLES SPECIFIED TO BE CONTINUOUS

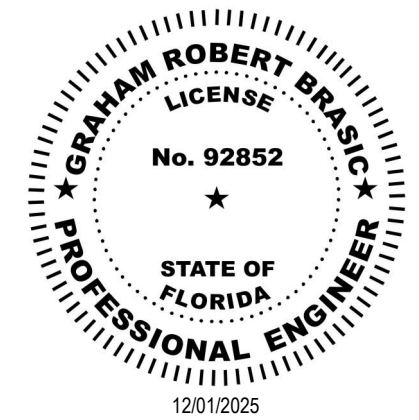
3
S-500
1" = 1'-0"
CONTINUOUS ANGLE SPLICE DETAIL



1615 FORUM PLACE, SUITE 3A
WEST PALM BEACH, FL 33401
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www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 21.18.004

TO THE BEST OF THE ENGINEER'S
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601 UNIVERSE BLVD. JUNO BEACH, FL 33408



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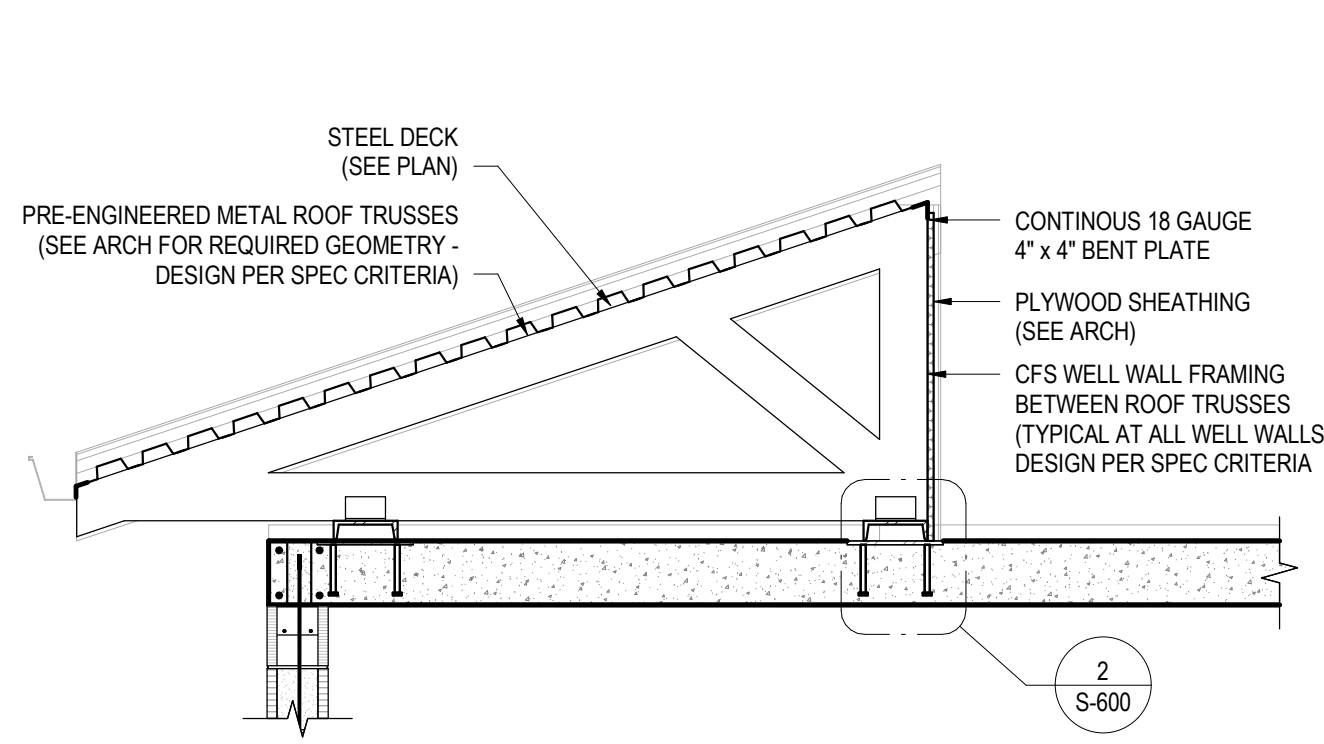
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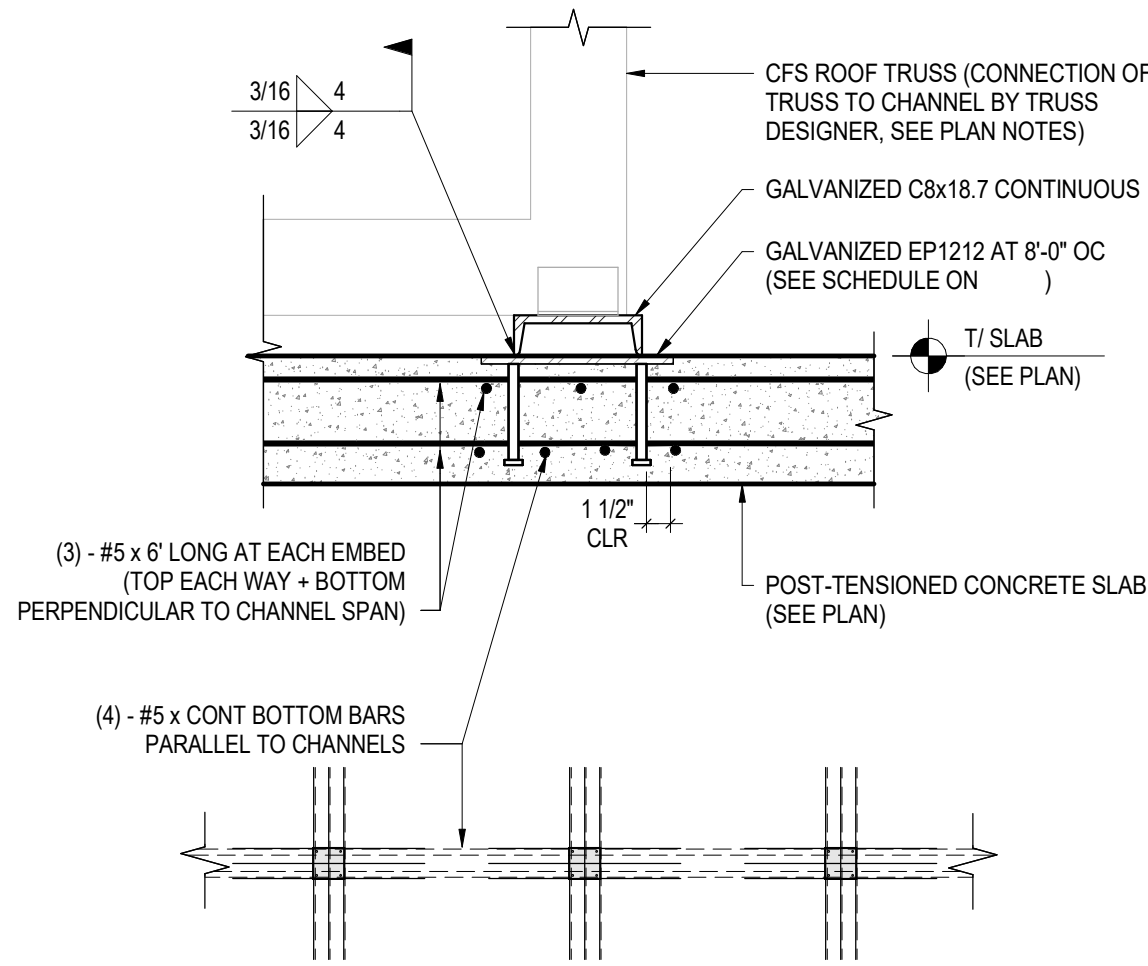
STEEL
FRAMING
DETAILS

S-500



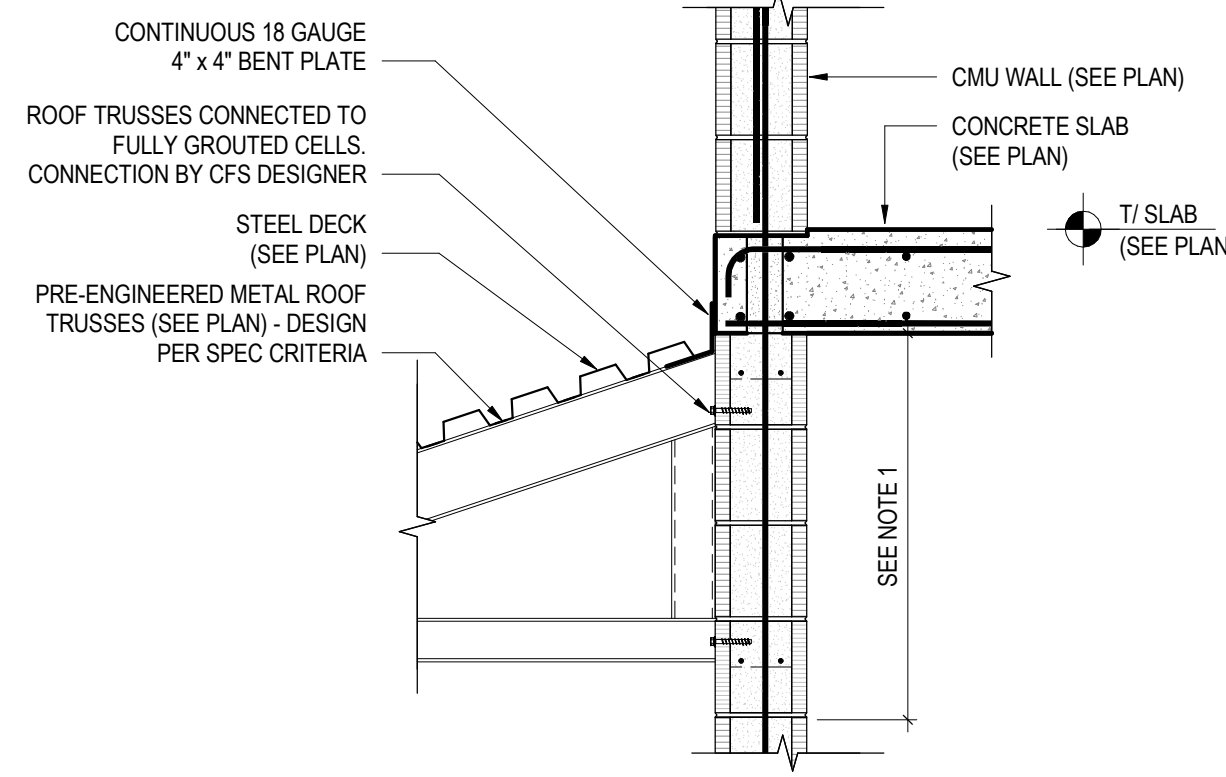
1
S-600
1/2" = 1'-0"

TYPICAL ROOF TRUSS - SECTION



2
S-600
1" = 1'-0"

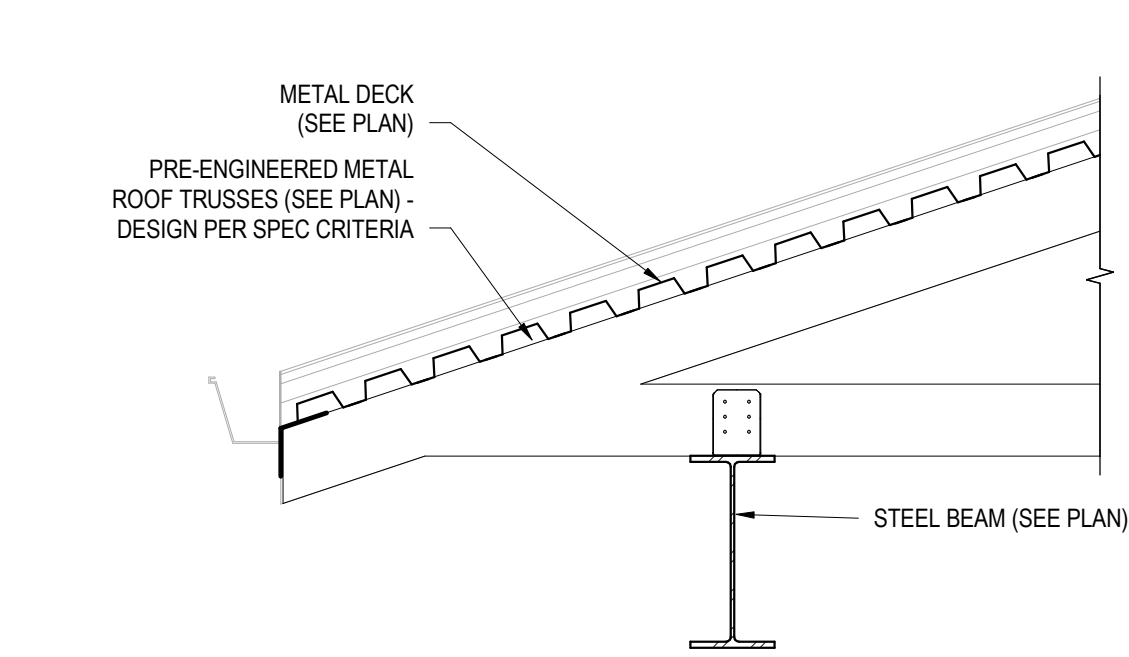
CFS ROOF TRUSS CONNECTION



NOTES:
1. FULLY GROUT THE TOP (4) CELLS OF WALL TO PROVIDE SOLID CONNECTIONS FOR ANCHORS.
COORDINATE EDGE DISTANCE REQUIREMENTS WITH FASTENERS PER CONNECTION DESIGN AND
GROUT ADDITIONAL CELL(S) AS REQUIRED.

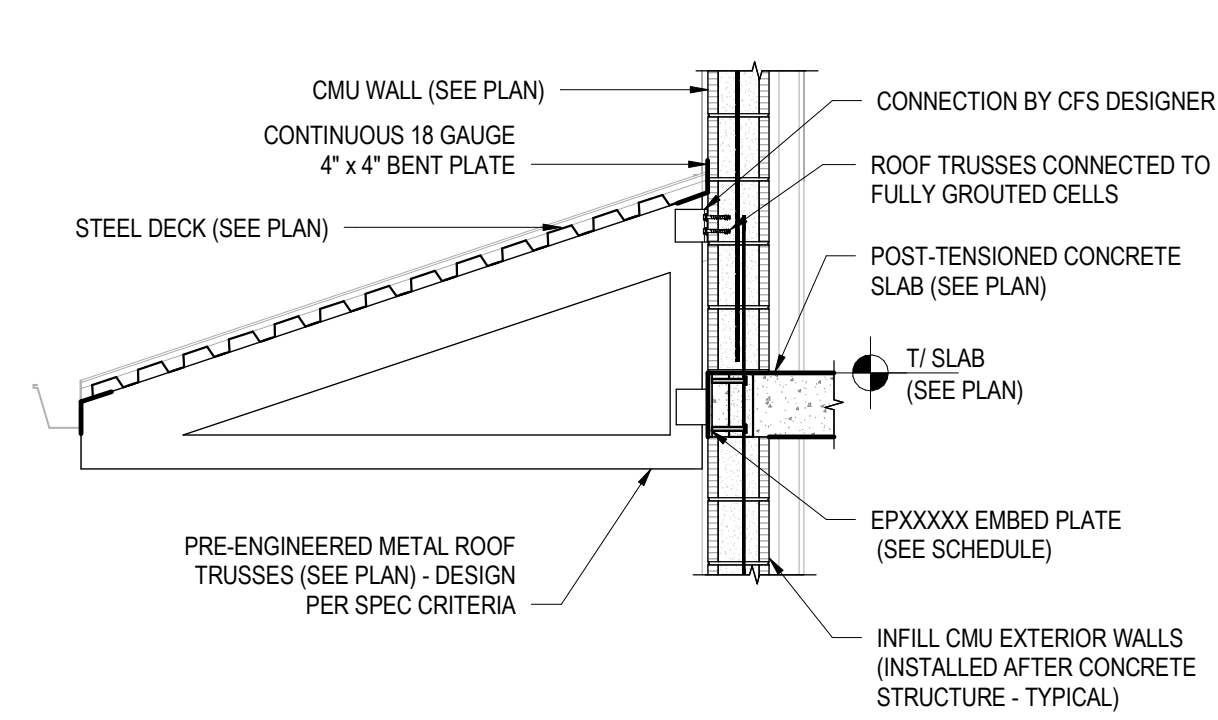
3
S-600
3/4" = 1'-0"

AREA B CANOPY CONNECTION



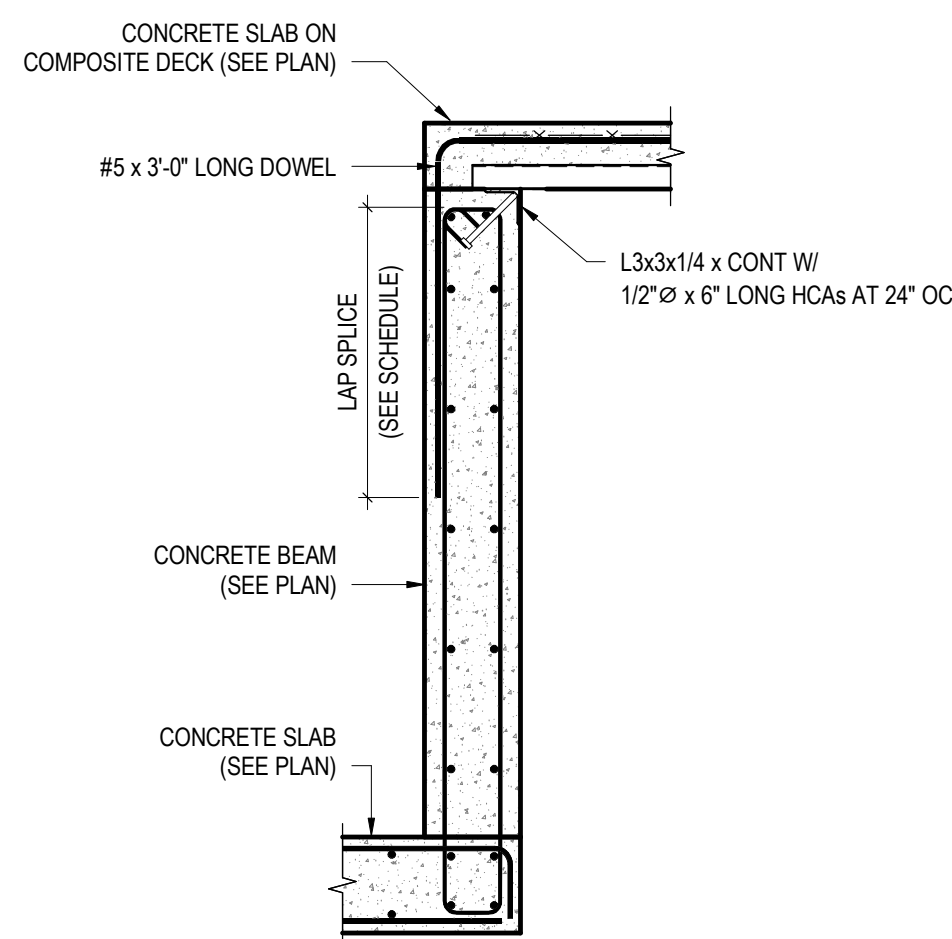
4
S-600
3/4" = 1'-0"

CFS ROOF TRUSS TO STEEL BEAM
CONNECTION AT PARKING ROOF



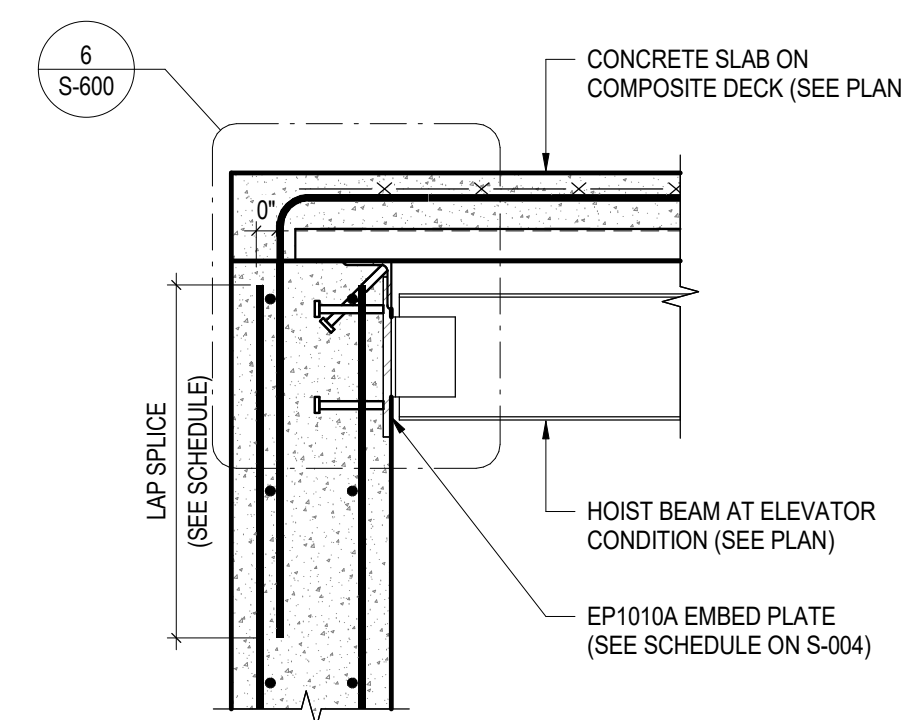
5
S-600
1/2" = 1'-0"

ROOF TRUSS TO WALL CONNECTION



6
S-600
3/4" = 1'-0"

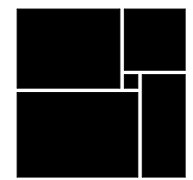
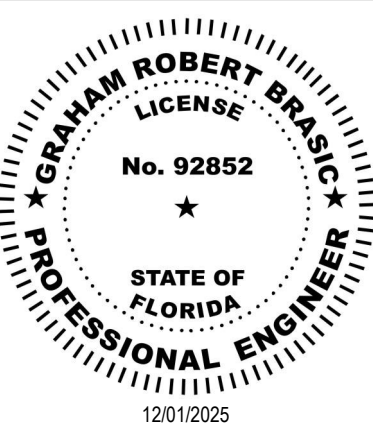
ELEVATOR POP-UP CAP-TO-BEAM
CONNECTION



7
S-600
1" = 1'-0"

ELEVATOR/STAIR CAP SLAB - SECTION

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ROOF
FRAMING
DETAILS

S-600



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