

[illegible]

Vero Classical School
51 Old Dixie Hwy. Vero Beach, FL 32962

VERO CLASSICAL
CHRISTIAN SCHOOL -
PHASE I

PROJECT LOCATION: 13TH ST. SW AND 58TH AVE. SW
VERO BEACH, FLORIDA



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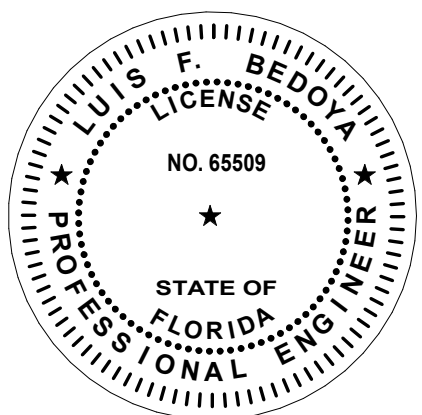
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BIBLIOJECT #24132

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LUIS F. BEDOYA, P.E.

REVISIONS		
MARK	DESCRIPTION	DATE

C1	A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD TESTING INCLUDING SLUMP TESTS AND CYLINDER BREAKS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
C2	CONCRETE WORK SHALL CONFORM TO ACI 318-19 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE.
C3	ALL CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES:

NOTES:

1. SLUMP FOR RAMPS AND SLOPING SURFACES SHALL NOT EXCEED 4".
2. ALL CONC MIXES SHALL HAVE A MAXIMUM SAND TO TOTAL AGGREGATE RATIO OF 0.50.
3. A 2" OR 3" PUMP SHALL BE ACCEPTABLE FOR COLUMNS AND THE BEAMS BUT WILL NOT BE ALLOWED FOR FOUNDATIONS, SLABS, TILT-UP PANELS AND CONCRETE BEAMS.
4. READY MIX SUPPLIER SHALL DESIGN THE MIXES THAT CONTAIN MULTIPLE AGGREGATES TO BE WELL GRADATED.
5. SLABS SHALL NOT BE AIR ENTRAINED.
6. FOR SLABS THAT SHALL RECEIVE MOISTURE SENSITIVE FLOORING:
 - a. CONTRACTOR SHALL WORK WITH THE READY MIX SUPPLIER TO PROVIDE A MIX DESIGN THAT WILL BE AT OR BELOW 75% RELATIVE HUMIDITY AT THE TIME THE FLOORING IS SCHEDULED TO BE INSTALLED.
 - b. DO NOT USE LIGHTWEIGHT AGGREGATES.
 - c. PROVIDE A MIX WITH GOOD SELF-DESICCATING PROPERTIES. CONSIDER ADDING 2%-4% SILICA FUME.
 - d. DO NOT HARD TROWEL THE SURFACE BUT INSTEAD PROVIDE A LIGHTLY TROWELLED SURFACE.

- C5 REBAR SHALL CONFORM TO ASTM A615 GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064 AND SHALL BE LAPPED MINIMUM ONE MESH + 2" WHERE SPLICED. ALL REINFORCING SHALL BE DOMESTICALLY PRODUCED. ALL REBAR THAT IS TO BE WELDED SHALL BE LOW ALLOY ASTM A706 GRADE 60.

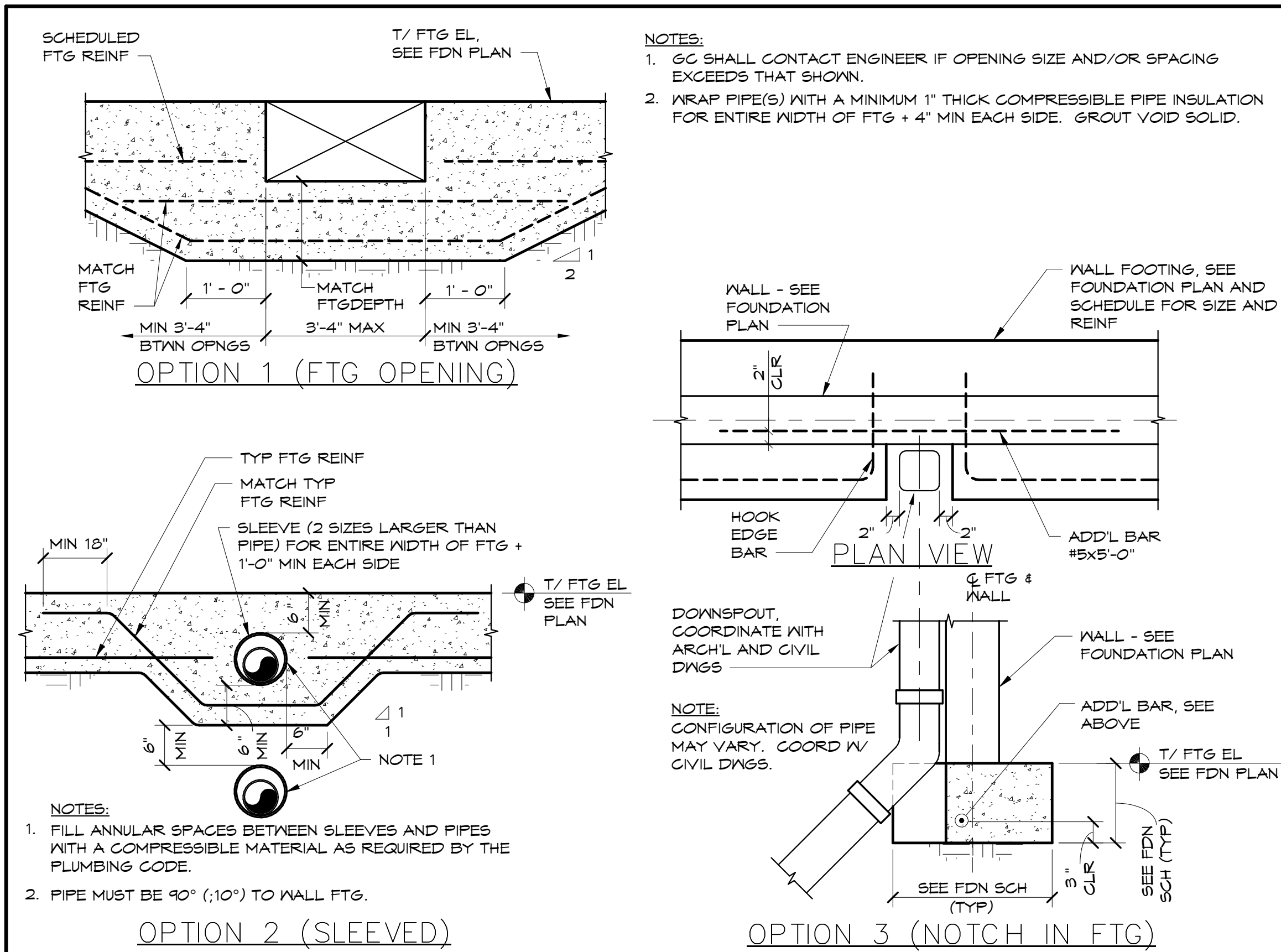
- C6 SPLICES AND ANCHORAGE OF REINFORCING SHALL BE AS FOLLOWS
(UNLESS OTHERWISE NOTED):
- WELDED WIRE FABRIC: 8"
ALL OTHER: #6 BAR & SMALLER: 48 DIA (12" MIN)
#7 BAR & LARGER: 60 DIA

- C7 REINFORCEMENT IN WALLS, FOOTINGS AND BEAMS SHALL BE CONTINUOUS AND LAPPED 48 BAR DIA AT SPLICE UNLESS OTHERWISE NOTED. HOOK AND LAP ALL CORNER AND INTERSECTING BARS. (SEE REINF DEVELOPMENT DETAIL).



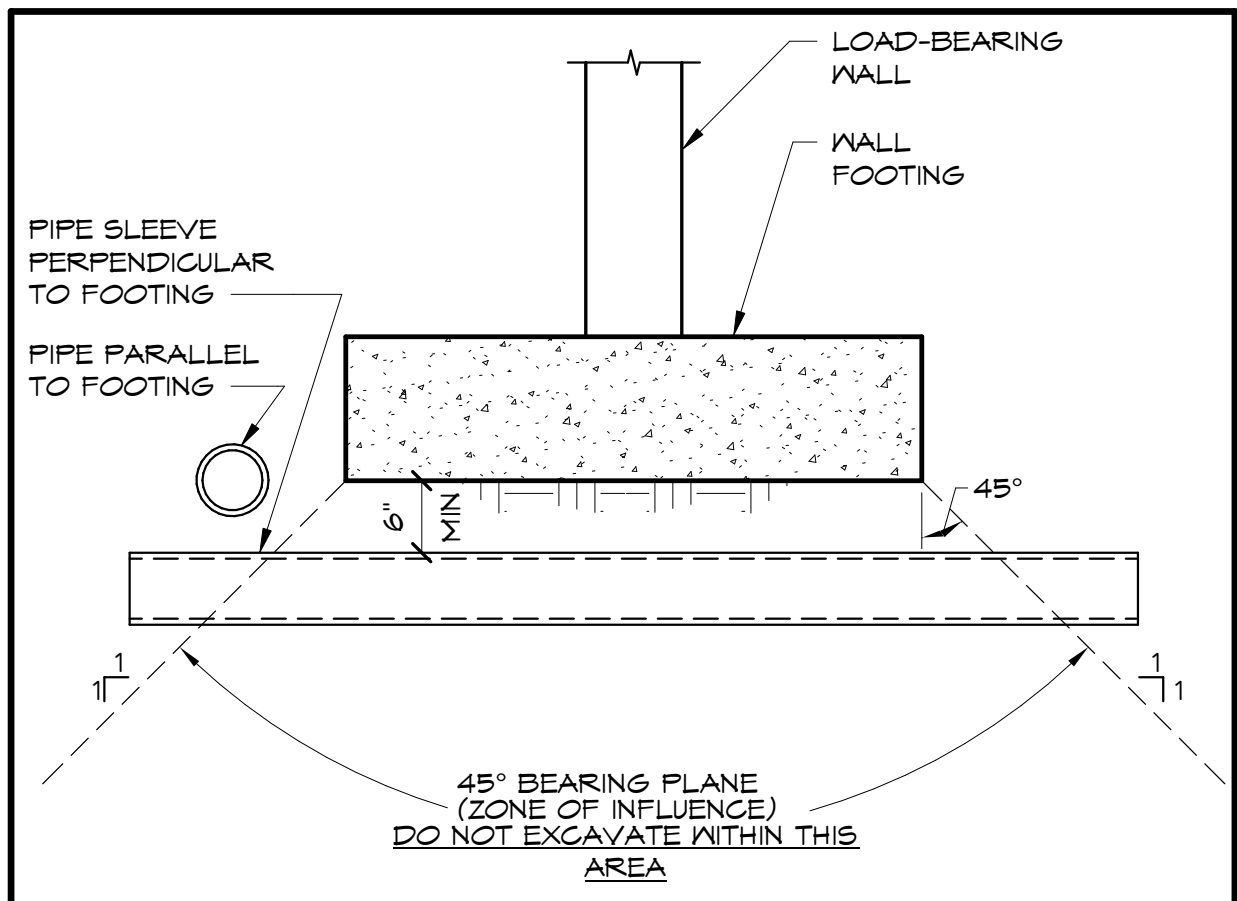
- C8 COVER FOR REINFORCING SHALL BE AS FOLLOWS:

C9 FOOTING PENETRATION DETAILS:



DETAIL – FOOTING PENETRATIONS

- FOOTING EXCAVATION DETAIL:



DETAIL - FOOTING
EXCAVATIONS

- | | | | | | | | |
|----------|--|-------|-------------------------------|-------|-------------------------------|--------|--------------------------|
| AAC | AUTOCLAVED AERATED CONCRETE | DBA | DEFORMED BAR ANCHOR | ID, # | POINT(S) | R | RADIUS |
| AB | ANCHOR BOLT | DIA | DIAMETER | LG | LONG | RC | REINFORCED CONCRETE |
| ABV | ABOVE | DIAG | DIAGONAL | LD | LIVE LOAD | REF | REFERENCE |
| A.C.I. | AMERICAN CONCRETE INSTITUTE | DL | DEAD LOAD | LLBB | LONG LEG BACK-TO-BACK | REIN | REINFORCEMENT (ING) |
| ADDL | ADDITIONAL | DN | DOWN | LLH | LONG LEG HORIZONTAL | REQ | REQUIRED |
| AF | ABOVE FINISH FLOOR | DET | DETAIL | LLV | LONG LEG VERTICAL | REQD | REQUIRED |
| AGG | AGGREGATE | DNG | DRAWING | LNTL | LONG LENT | ROOT | ROOT |
| A.I.S.C. | AMERICAN INSTITUTE OF STEEL CONSTRUCTION | DNL | DNCEL | LSL | LONG SLOTTED | RM | ROOM |
| A.I.S.I. | AMERICAN IRON AND STEEL | EA | EACH | LOC | LOCATION | RO | ROUGH OPENING |
| AL | ALUMINUM | EE | EACH END | LONG | LONGITUDINAL | RP | RADIUS POINT |
| ALT | ALTERNATE | EF | EACH FACE | LP | LOW POINT | RTN | RETURN |
| AR | ANCHOR ROD | ENG | EXPANSION JOINT | LRFD | LOAD & RESISTANCE FACTOR | R/M | REINFORCED WITH |
| ARCH | ARCHITECTURAL | EOS | EDGE OF SLAB | LN | LONG WAY | SGH | SCHEDULE |
| ASD | ALLOWABLE STRESS DESIGN | EL | ELEVATION | MAS | MASONRY | SDI | STEEL DECK INSTITUTE |
| ASTM | AMERICAN SOCIETY OF TESTING MATERIALS | EQA | EQUAL | MAS | MASONRY | SECT | SECTION |
| AWS. | AMERICAN WELDING SOCIETY | EQ | EQUAL SPACE(S) (ING) | MAX | MAXIMUM | SF | STEPPED FOOTING |
| | | ES | EACH SIDE | MB | MACHINE BOLT | SHT | SHEET |
| | | EX | EACH WAY | MBM | METAL BUILDING MFR | SHW | SHAW WALL |
| B/ | BOTTOM OF | EXIST | EXISTING | MC | MASONRY COLUMN | SI | SIMILAR |
| BB | BOND BEAM | EXP | EXPANSION | MCJ | MASONRY CONCRETE JT | SJ | SAWJOINT |
| BLDG | BUILDING | EXT | EXTERIOR | MCN | MOMENT CONNECTION | SJL | STEEL JOIST INSTITUTE |
| BLKG | BLOCKING | F/ | FACE OF | ME | MONOLITHIC EDGE | SL | SLOPE |
| BLM | BELOW | FBC | FLORIDA BUILDING CODE | MECH | MECHANICAL | SLBB | SLOTTED LEG BACK-TO-BACK |
| BM | BEAM | FD | FLORIDA DRAIN | MEZZ | MEZZANINE | SGS | SLAB ON GRADE |
| BOT | BOTTOM | FDN | FOUNDATION | MF | MONOLITHIC FOOTING | SP | SPACE |
| BP | BASE PLATE | FF | FINISHED FLOOR | MIN | MINIMUM | SPECBS | SPECIFICATIONS |
| BRDG | BRIDGING | FN | FINISH | MISC | MISCELLANEOUS | SQ | SQUARE |
| BRG | BEARING | FLR | FLOOR | MO | MASONRY OPENING | SS | STAINLESS STEEL |
| BRK | BRICK | FLG | FLANGE | MTL | METAL STUD | SSL | SHORT SLOTTED |
| BS | BOTH SIDES | FMG | FRAMING | MTL | METAL | STD | STANDARD |
| BTJ | BOLTED TIE JOIST | FS | FAR SIDE | MNF | MONOLITHIC WALL FOOTING | STIFFR | STIFFENER |
| BTYN | BETWEEN | FT | FOOT | NIC | NOT IN CONTRACT | STL | STEEL |
| | | TTG | TOOTH | NTS | NOT TO SCALE | STR | STRENGTH |
| C/C | CENTER TO CENTER | NS | NEAR SIDE | NTS | NOT TO SCALE | STRL | STRUCTURAL |
| CANT | CANTILEVER | GA | GAUGE | OC | OVERALL | SWM | SHORT WAY |
| CC | CONCRETE BEAM | GALV | GALVANIZED(D) | OA | OVERALL | SYMP | SYMMETRICAL |
| CC | CONCRETE COLUMN | GB | GRADE BEAM | OC | OVERALL | SYMP | SOUTHERN YELLOW PINE |
| CFMF | COLD FORMED METAL FRAMING | GC | GENERAL CONTRACTOR | OC | ON CENTER | T/B | TOP AND BOTTOM |
| CFB | COLD FORMED STEEL | GFC | GROUT FILLED CELL(S) / COURSE | OT | OUTSIDE DIMETER | TB | TIE BEAM |
| CJ | CAST-IN-PLACE | GLB | GULFMAN BEAM | OF | OUTSIDE FACE | TC | TIE COLUMN |
| CJP | CONSTRUCTION JOINT (OR CONTROL JOINT) | GR | GRADE | OH | OPPOSITE HAND | TEMP | TEMPERATURE |
| | | GT | GIRDER TRUSS | OHD | OVERHEAD | THK | THICK |
| E CL | ENTERLINE | HG | HOLLOW-CORE | OPN | OPENING | TOPS | TOPPING |
| CLR | CLEAR(ANCE) | HOK | HOT DIPPED GALVANIZED | OPP | OPPOSITE | TRANS | TRANSVERSE |
| CM | CONSTRUCTION MANAGER | HORIZ | HORIZONTAL | PAF | POWDER-ACTUATED | TYP | TYPICAL |
| CMU | CONCRETE MASONRY UNIT | HP | HIGH POINT | PB | PRE-CAST BEAM | T/ | TOP OF |
| COL | CONCRETE COLUMN | HS | HEADED STUD | PMB | PRE-ENGINEERED METAL BUILDING | UNO | UNLESS NOTED OTHERWISE |
| CONN | CONNECTION | ID | INSIDE DIAMETER | PERP | PERPENDICULAR | VFG | VERIFY IN FIELD |
| CONT | CONTINUOUS | IF | INSIDE FACE | PC | PRECAST | VERT | VERTICAL |
| CONTR | CONTRACTOR | INT | INTERIOR | P, PL | PLATE | WD | WOOD |
| COORD | COORDINATE | INT | INTERIOR | PL | PLACES | WO | WOOD |
| CSK | COUNTER SINK | JST | JOIST | PLF | POUNDS PER LINEAR FOOT | WO | WOOD OPENING (MASONRY) |
| CTR | CENTERED | JT | JOINT | PLYNO | PLYWOOD | WOF | WORKING FOOT |
| CY | CUBIC YARD | K | KIP | PNL | PANEL | WS | WATERSTOP |
| | | KO | KNOCK-OUT | PSF | POUNDS PER SQUARE FOOT | WVF | WELDED WIRE FABRIC |
| | | | | PSI | POUNDS PER SQUARE INCH | W | WITH |
| | | | | PTN | PARTITION | | |

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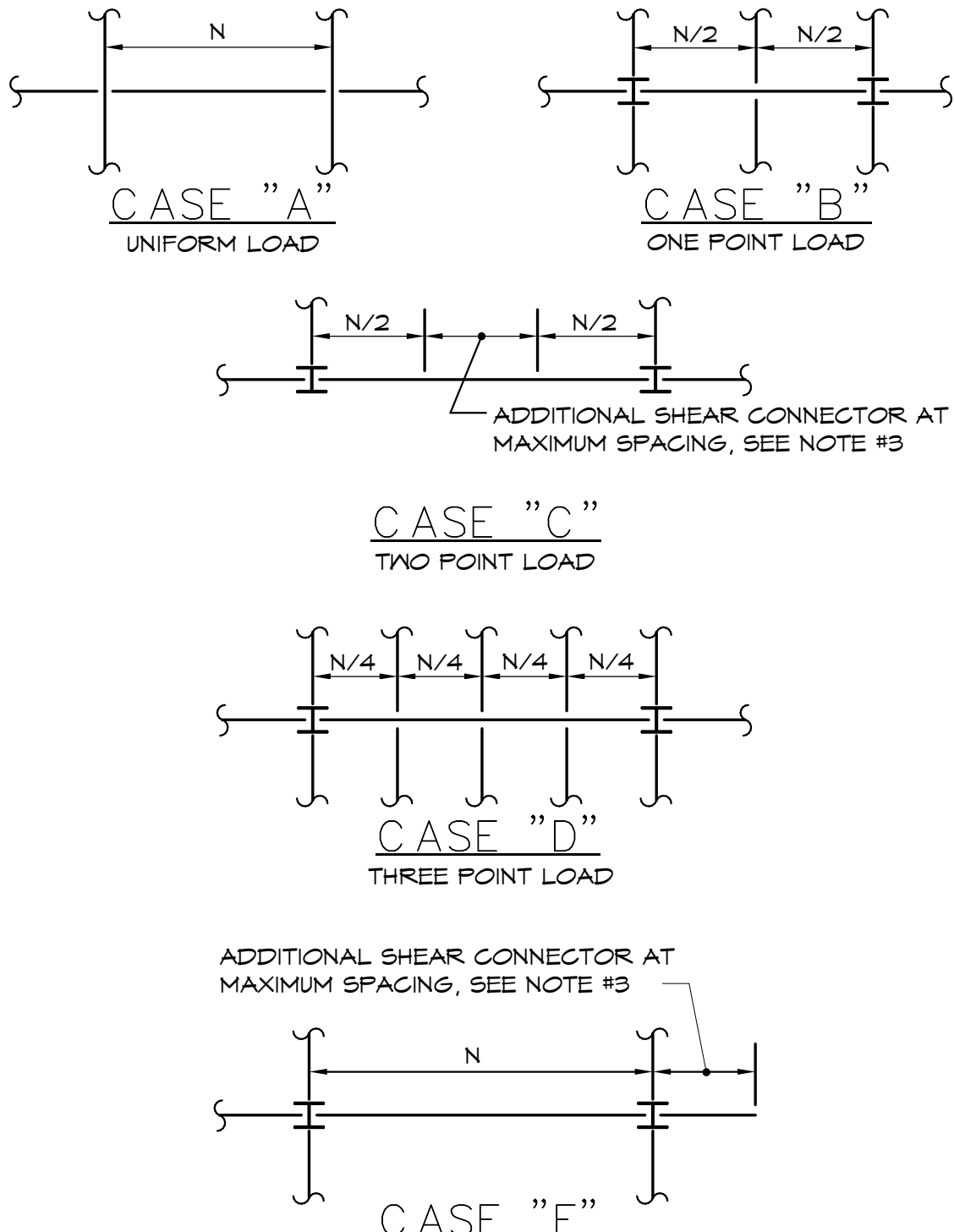
STRUCTURAL GENERAL NOTES & ABBREVIATIONS

COMPOSITE STEEL FLOOR SYSTEM

CS1 COMPOSITE BEAM CONNECTIONS SHALL BE PROVIDED AS DETAILED HEREIN.

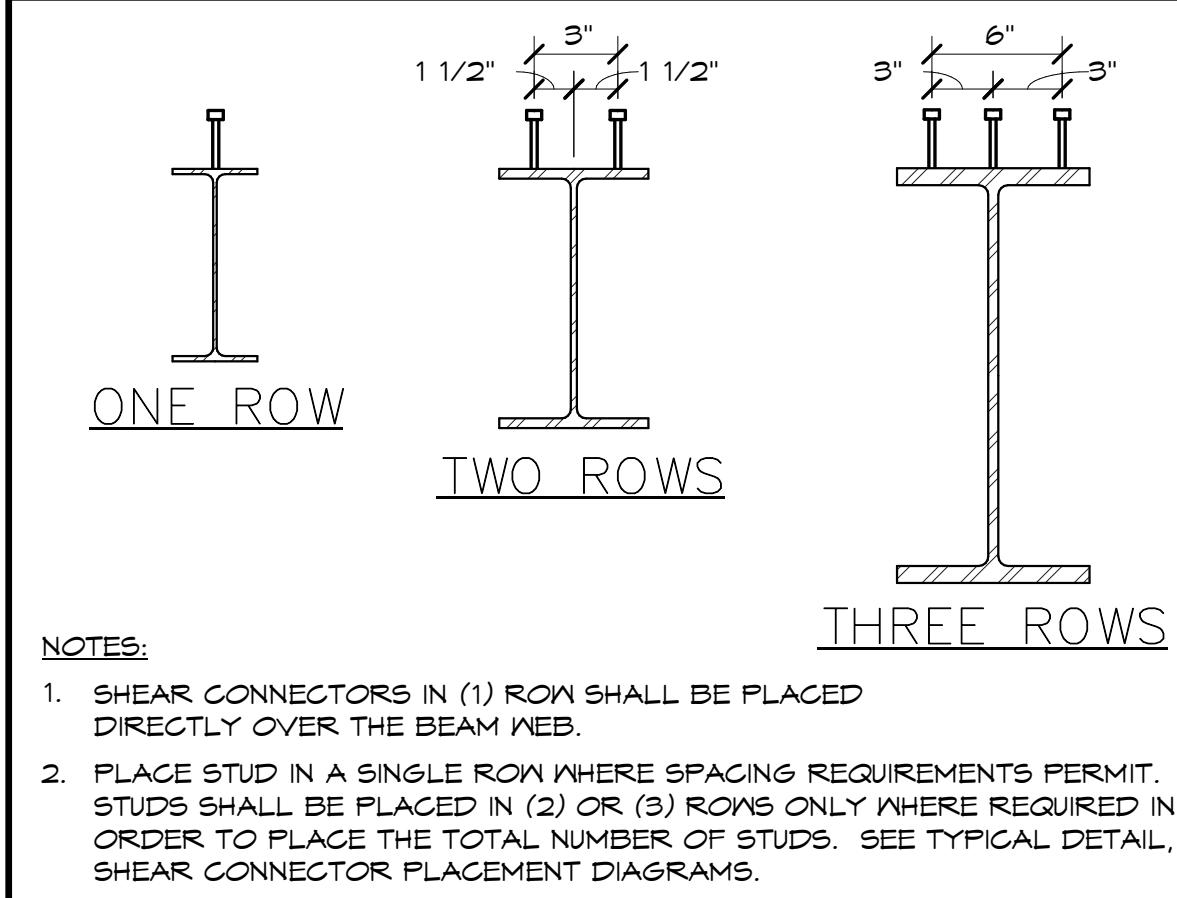
CS2 SHEAR CONNECTOR SIZE AND QUANTITIES ARE SHOWN IN THE FLOOR FRAMING PLAN NOTES. THE CONTRACTOR SHALL SUBMIT STUD PLACEMENT SHOP DRAWINGS. THE TOP OF THE STUDS MUST BE A MINIMUM OF 1 1/2" ABOVE THE FLUTES OF THE METAL DECK BUT NOT SO HIGH AS TO EXTEND ABOVE THE SLAB SURFACE AFTER THE SLAB HAS BEEN CAST AND PRE-COMPOSITE DEFLECTIONS HAVE OCCURRED. THE LENGTH OF THE STUD INDICATED IN THE FLOOR FRAMING PLAN NOTES IS THE IN-PLACE LENGTH AFTER WELDING.

CS3 COMPOSITE SHEAR CONNECTOR PLACEMENT DIAGRAMS:



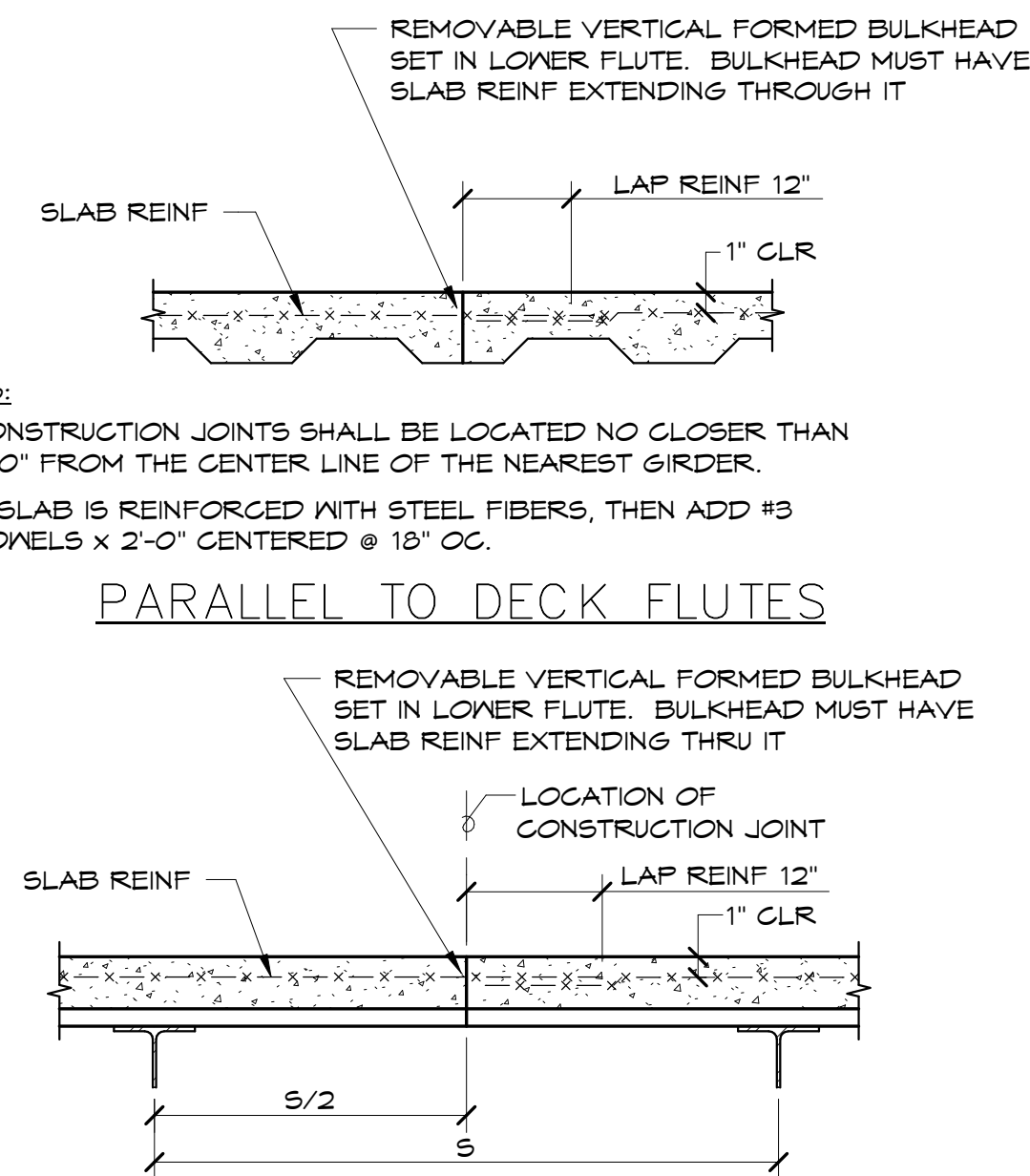
- NOTES:
- N = SPECIFIED NUMBER OF SHEAR CONNECTORS. REFER TO FRAMING PLAN(S).
 - UNLESS NOTED OTHERWISE ON PLANS OR IN THE COMPOSITE BEAM SCHEDULE, SHEAR CONNECTORS SHALL BE DISTRIBUTED ALONG THE LENGTH OF THE BEAM AS SHOWN ON DETAILS ABOVE.
 - MAXIMUM SPACING OF SHEAR CONNECTOR SHALL BE AS FOLLOWS:
 - BEAMS PERPENDICULAR TO DECK SPAN = 36"
 - BEAMS PARALLEL TO DECK SPAN = (8 x TOTAL SLAB THICKNESS)
 - MINIMUM SPACING OF SHEAR CONNECTOR SHALL BE AS FOLLOWS:
 - BEAMS PERPENDICULAR TO DECK SPAN = 3"
 - BEAMS PARALLEL TO DECK SPAN = 4 1/2"
 - WHERE STEEL DECK CORRUGATIONS DO NOT ALLOW FOR AN EVEN SPACING OF SHEAR CONNECTORS WITH (1) STUD IN EACH FLUTE, ADDITIONAL STUDS IN A SECOND ROW (AND THIRD ROW WHERE REQUIRED) SHALL BE PLACED SUCH THAT THE HIGHEST DENSITY OF SHEAR CONNECTORS OCCURS NEAR THE BEAM SUPPORT.
 - WHERE THE SPECIFIED NUMBER OF SHEAR CONNECTOR IS LESS THAN THE BEAM SPAN LENGTH DIVIDED BY THE MAXIMUM SPACING (SEE NOTE 3) ADDITIONAL SHEAR CONNECTORS SHALL BE PROVIDED SUCH THAT THE MAXIMUM SPACING IS NOT EXCEEDED AT ANY LOCATION IN THE SPAN.
 - SUBMIT SHOP DRAWINGS SHOWING PLACEMENT OF SHEAR CONNECTORS TO ARCHITECT FOR ENGINEER'S APPROVAL.

CS4 COMPOSITE SHEAR CONNECTOR SPACING DETAIL:



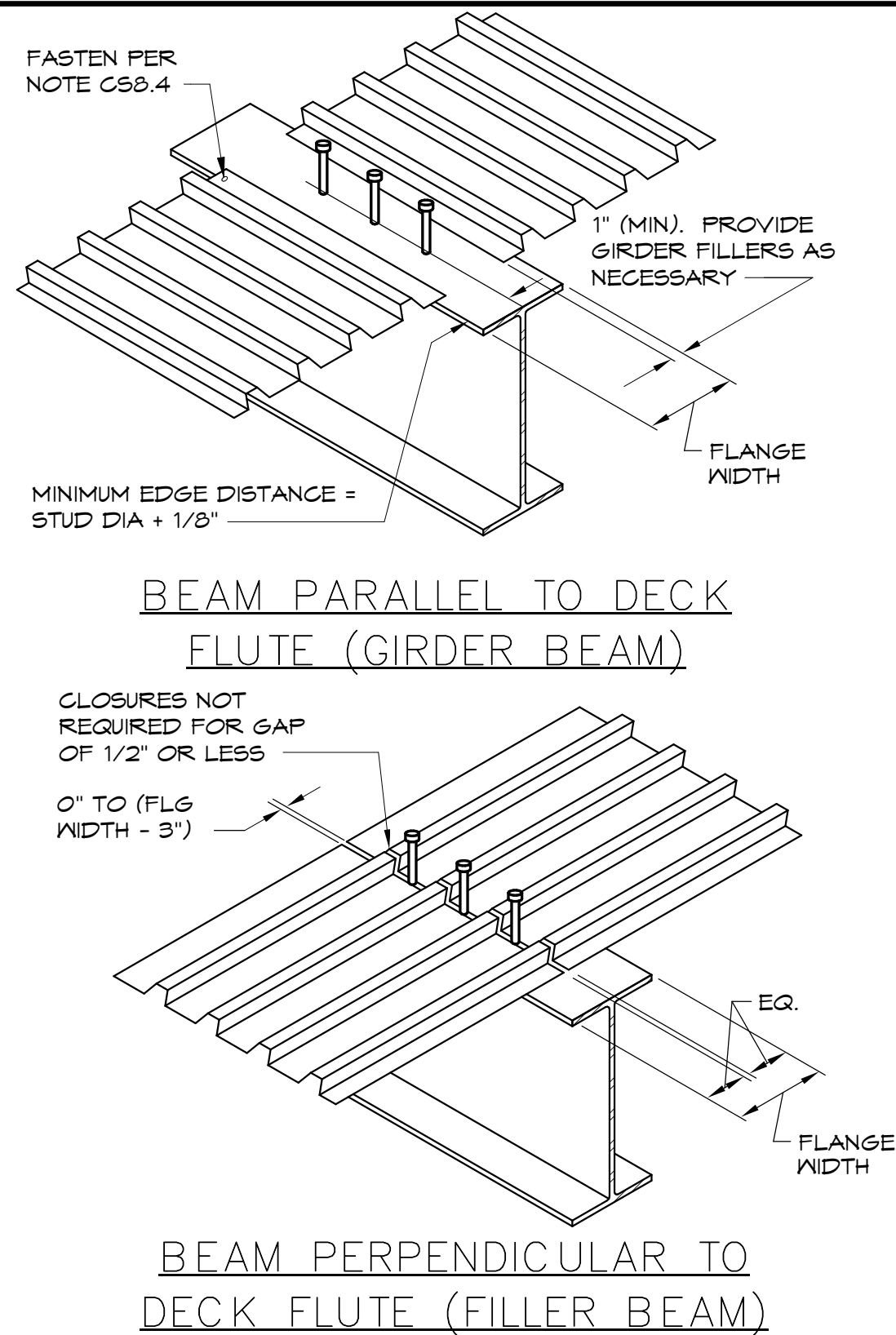
- NOTES:
- SHEAR CONNECTORS IN (1) ROW SHALL BE PLACED DIRECTLY OVER THE BEAM WEB.
 - PLACE STUD IN A SINGLE ROW WHERE SPACING REQUIREMENTS PERMIT. STUDS SHALL BE PLACED IN (2) OR (3) ROWS ONLY WHERE REQUIRED IN ORDER TO PLACE THE TOTAL NUMBER OF STUDS. SEE TYPICAL DETAIL, SHEAR CONNECTOR PLACEMENT DIAGRAMS.

CS5 COMPOSITE SLAB CONSTRUCTION DETAIL DIAGRAMS:



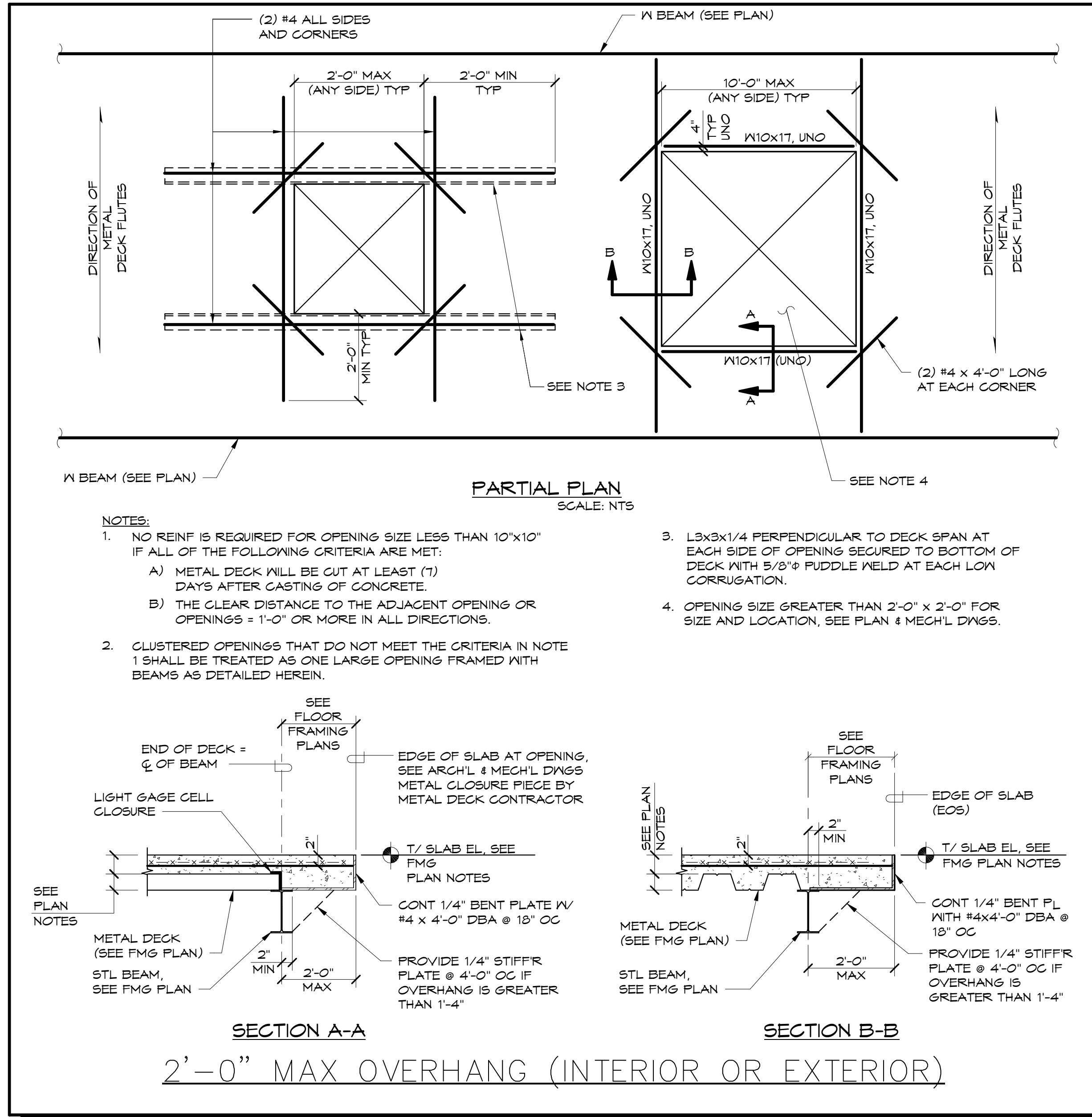
- NOTES:
- CONSTRUCTION JOINTS SHALL BE LOCATED NO CLOSER THAN 5'-0" FROM THE CENTER LINE OF THE NEAREST GIRDER.
 - IF SLAB IS REINFORCED WITH STEEL FIBERS, THEN ADD #3 DOWELS X 2'-0" CENTERED @ 18" OC.
- PARALLEL TO DECK FLUTES
- PERPENDICULAR TO DECK FLUTES
- NOTES:
- AT CONTRACTOR'S OPTION, PREFORMED PERMANENT METAL SHEAR KEY MAY BE USED WITH SLAB REINFORCING STOPPING EACH SIDE AND #3 DOWEL X 2'-0" CENTERED AT 18" OC.
 - IF SLAB IS REINFORCED WITH STEEL FIBERS, THEN ADD #3 DOWELS X 2'-0" CENTERED AT 18" OC.
 - CONTRACTOR IS RESPONSIBLE FOR PROVIDING CHAIRS FOR WWF TO MAINTAIN CLEARANCE SHOWN. RAISING THE WWF DURING THE CONCRETE POUR SHALL NOT BE PERMITTED.

CS6 COMPOSITE FLOOR DECK PLACEMENT DETAILS:



- NOTES:
- DECK SHALL BE FURNISHED IN MODULAR PANEL WIDTHS. ANY CUTTING THAT IS REQUIRED SHALL BE DONE BY THE DECK ERECTOR.
 - LATERAL AND LONGITUDINAL SPACING IS CONTROLLED BY SECTION 1.11.4 OF THE A.I.S.C.E. SPECIFICATIONS. FOR THE LOCATION OF THE STUD WITH RESPECT TO THE EDGE OF THE FLANGE, SEE THE STRUCTURAL WELDING CODE A.W.S., 429.8, (DETAIL 4).
 - WHEN STUDS ARE USED, DECK MUST NOT BE LAPPED IN ORDER TO PERMIT WELDING OF STUDS THROUGH SINGLE THICKNESS OF DECK. WELDING OF STUDS TO BEAM THROUGH (2) LAYERS OF DECK SHALL NOT BE PERMITTED.

CS7 PARTIAL TYPICAL FRAMING PLAN AT OPENINGS IN COMPOSITE SLAB:



- NOTES:
- NO REINF IS REQUIRED FOR OPENING SIZE LESS THAN 10'X10' IF ALL OF THE FOLLOWING CRITERIA ARE MET:
 - METAL DECK WILL BE CUT AT LEAST (7) DAYS AFTER CASTING OF CONCRETE.
 - THE CLEAR DISTANCE TO THE ADJACENT OPENING OR OPENINGS = 1'-0" OR MORE IN ALL DIRECTIONS.
 - CLUSTERED OPENINGS THAT DO NOT MEET THE CRITERIA IN NOTE 1 SHALL BE TREATED AS ONE LARGE OPENING FRAMED WITH BEAMS AS DETAILED HEREIN.
- 1.5X3X1/4" PERPENDICULAR TO DECK SPAN AT EACH SIDE OF OPENING SECURED TO BOTTOM OF DECK WITH 5/8" PUFFLE WELD AT EACH LOW CORRUGATION.
 - OPENING SIZE GREATER THAN 2'-0" X 2'-0" FOR SIZE AND LOCATION, SEE PLAN & MECH'L DWGS.

CS8 COMPOSITE FLOOR DECK INSTALLATION:

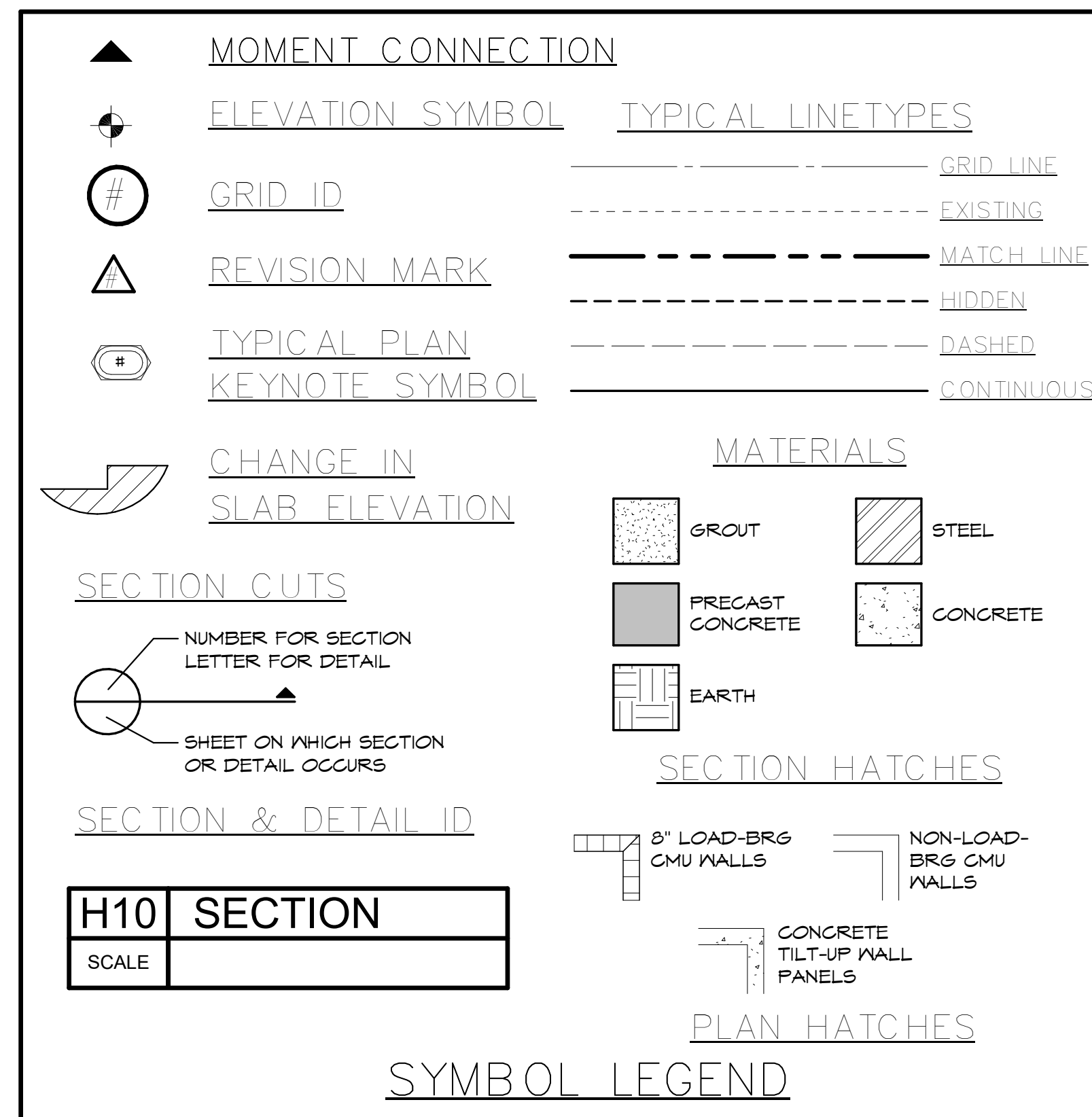
- NOTES:
- INSTALL TEMPORARY SHORING, IF REQUIRED, BEFORE PLACING DECK PANELS.
 - PLACE DECK PANELS ON STRUCTURAL SUPPORTS AND ADJUST TO FINAL POSITION WITH ENDS ALIGNED. ATTACH FIRMLY TO THE SUPPORTS IMMEDIATELY AFTER PLACEMENT IN ORDER TO FORM A SAFE WORKING PLATFORM.
 - CUT AND NEATLY FIT DECK UNITS AND ACCESSORIES AROUND OPENINGS AND OTHER WORK PROJECTING THROUGH OR ADJACENT TO THE DECKING.
 - ANCHOR FLOOR DECK UNITS TO STEEL SUPPORTING MEMBERS BY #11 X 1/2" LONG 1/4" DIA. PULLEY ACTIVATED FASTENERS, WITH A FASTENER PLACED IN EACH DECK FLUTE CORRUGATION, AND AT 12" ON CENTER AT DECK EDGES. (CC OPTION TO SUBSTITUTE ARG SPOT PUFFLE WELDS OF THE FOLLOWING DIAMETER AND SPACING (OR FILLET WELDS OF EQUAL STRENGTH) IN LIEU OF HILT FASTENERS:
 - MINIMUM VISIBLE WELD DIAMETER = 5/8" AT EACH CORRUGATION.
 - WELD EDGE RIBS OF PANELS AT EACH SUPPORT. SPACE ADDITIONAL WELDS AN AVERAGE OF 12" OC BUT NOT MORE THAN 18" OC IN ANY ONE LOCATION.
 - FASTEN SIDE LAPS AND PERIMETER EDGE OF UNITS BETWEEN SUPPORTS AT INTERVALS NOT EXCEEDING 24" OC, USING ONE OF THE FOLLOWING METHODS:
 - #10 SELF-DRILLING SCREWS
 - CRIMP OR BUTTON PUNCH
 - FOR DECKS THAT ARE 20ga AND HEAVIER: ARG PUFFLE WELDS 5/8" MINIMUM VISIBLE DIAMETER OR 1" LONG FILLET WELDS.

- INSTALL DECK ENDS OVER SUPPORTS WITH A MINIMUM END BEARING OF 2".
- FASTEN FOUR STOPS AND GIRDER FILLERS TO SUPPORTING STRUCTURE ACCORDING TO THE MFR'S RECOMMENDATIONS.
- FASTEN COLUMN CLOSURES, CELL CLOSURES AND Z CLOSURES TO DECK TO PROVIDE TIGHT FITTING CLOSURES AT OPEN ENDS OF RIBS AND SIDES OF DECKING. FASTEN CELL CLOSURES AT CHANGES OF DIRECTION OF FLOOR DECK UNITS.

CS9 COMPOSITE STEEL FLOOR UNITS SHALL BE FABRICATED FROM STEEL CONFORMING TO SECTION A3 OF THE LATEST EDITION OF THE AMERICAN IRON AND STEEL INSTITUTE, SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS. THE STEEL USED SHALL HAVE A MINIMUM YIELD STRENGTH OF 50 KSI (345 MPa). SEE CHART BELOW FOR MINIMUM SECTION PROPERTIES REQUIRED FOR STEEL DECK. PROPERTIES SHOWN ARE REPRODUCED FROM THE VULGRAFT MANUAL.

COMPOSITE FLOOR DECK						
DECK TYPE	DESIGN THICK.	Ip in ⁴ /ft.	Sp in ³ /ft.	Ix in ⁴ /ft.	Sx in ³ /ft.	Iy in ⁴ /ft.
3.0VLI22	0.0248	0.110	0.387	0.718	0.410	
3.0VLI20	0.0350	0.401	0.512	0.909	0.530	
3.0VLI18	0.0474	1.252	0.761	1.252	0.754	

- CS10 ALL FIELD WELDING OF DECK SHALL BE IN STRICT CONFORMANCE WITH AWS/A601.3 STRUCTURAL WELDING CODE.
- CS11 GALVANIZING SHALL CONFORM TO ASTM-A653, STRUCTURAL QUALITY AND FEDERAL SPEC. QQ-S-17B TO PROVIDE A MINIMUM COATING PROTECTION OF 690.
- CS12 COMPOSITE BEAMS SHALL BE CAMBERED AS INDICATED ON THE FLOOR FRAMING PLAN(S). IF NO CAMBER IS SPECIFIED, THE FABRICATOR SHALL ENSURE THAT THE "NATURAL" CAMBER IN ALL ERECTED BEAMS OCCURS IN AN UPWARD DIRECTION.
- CS13 NO CONDUIT IN SLAB IS ALLOWED.



A1 STRUCTURAL GENERAL NOTES

3/4" = 1'-0"

A13 SYMBOL LEGEND

NOT TO SCALE



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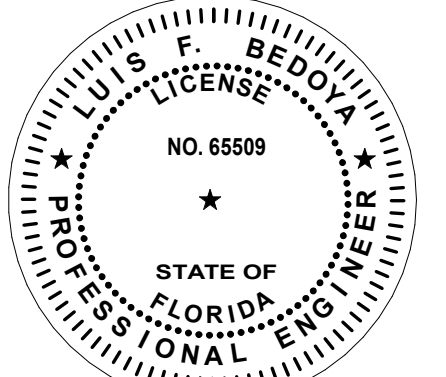
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STRUCTURAL GENERAL
NOTES & SYMBOL LEGEND

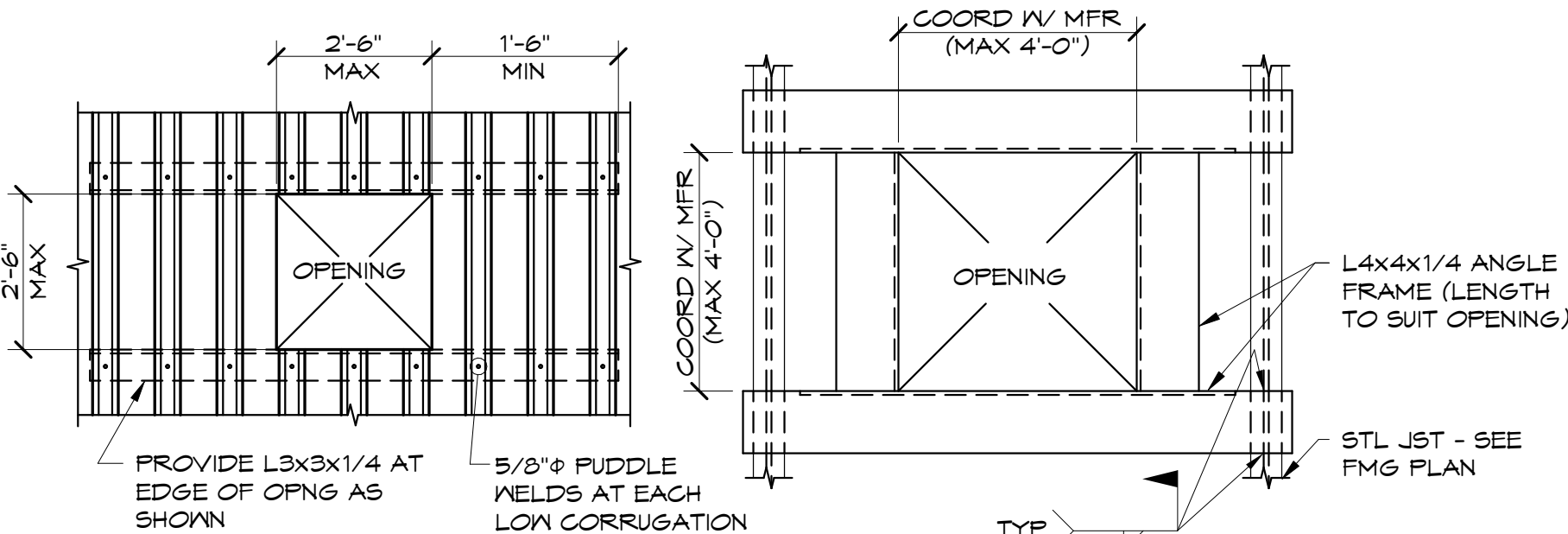
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STEEL ROOF DECK

- RD1 SEE ROOF FRAMING PLAN(S) FOR STEEL DECK ATTACHMENT TO STRUCTURE.
- RD2 STEEL ROOF DECK UNITS SHALL BE FABRICATED FROM STEEL CONFORMING TO SECTION A3 OF THE LATEST EDITION OF THE AMERICAN IRON AND STEEL INSTITUTE SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS. THE STEEL USED SHALL BE GRADE 80.
- RD3 IF OPTED BY GC, ALL FIELD WELDING OF DECK SHALL BE IN STRICT CONFORMANCE WITH ANSI/AWS D1.3 STRUCTURAL WELDING CODE.
- RD4 ALL SCREWS SHALL COMPLY WITH ASTM 1513, ICC AC43, AND ICC AC119. FASTENERS SHALL BE INSTALLED PERPENDICULAR TO ELEMENT SO AS TO PROPERLY SEAT FASTENER HEAD, AND TORQUED PER MANUFACTURER'S SPECIFICATIONS, NOT TO EXCEED MAXIMUM RECOMMENDED TORQUE.
- RD5 GALVANIZING SHALL CONFORM TO ASTM-A653, STRUCTURAL QUALITY AND FEDERAL SPEC. GG-5-TT5.
- RD6 SEE CHART BELOW FOR MINIMUM SECTION PROPERTIES REQUIRED FOR STEEL DECK. PROPERTIES SHOWN ARE REPRODUCED FROM THE VULCRAFT MANUAL.

ROOF DECK					
DECK TYPE	DESIGN THICK.	I_p IN ² /FT	S_p IN ³ /FT	I_x IN ⁴ /FT	S_x IN ³ /FT
B22	0.0295	0.155	0.106	0.183	0.192
B20	0.0350	0.201	0.234	0.222	0.241
B10	0.0474	0.284	0.318	0.295	0.321

RD7 SEE PARTIAL PLAN BELOW FOR METAL DECK OPENING FRAMING:

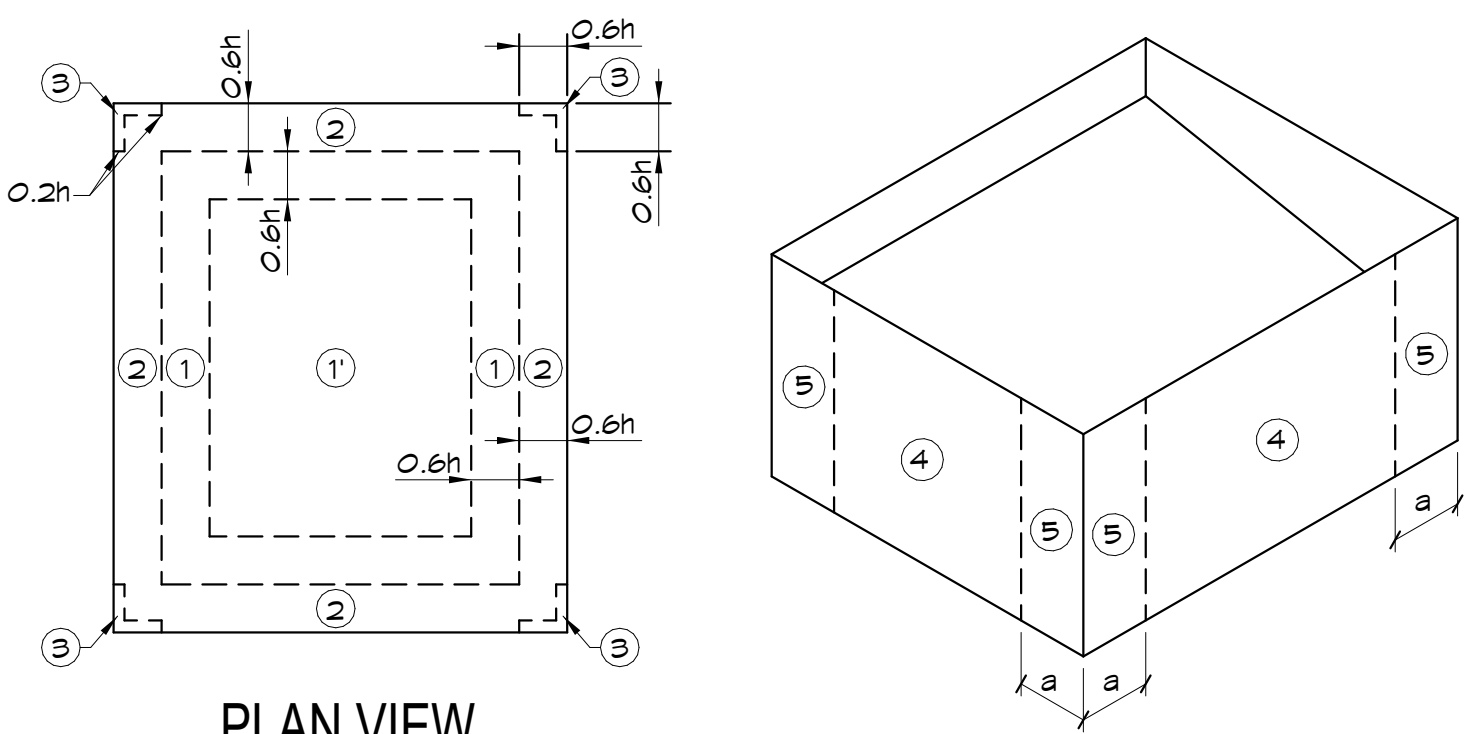


OPTION 1

OPTION 2

NOTES:

- A. FOR OPENINGS WITH A MAXIMUM DIMENSION OF 6" TO 1'-0", REINFORCE OPENING WITH A 20GA GALV FLAT PLATE 1'-0" LARGER THAN THE OPENING. ATTACH WITH 1" WELDS AT EACH RIB ALL AROUND.
- B. FOR OPENINGS WITH A MAX DIMENSION OF 1'-0" TO 2'-6", SEE DETAIL OPTION 1 ABOVE.
- C. FOR OPENINGS WITH A MAX DIMENSION OF 4'-0", SEE DETAIL OPTION 2 ABOVE.
- RD8 NO ITEMS SHALL BE HUNG DIRECTLY FROM THE ROOF DECK UNLESS INDICATED OTHERWISE IN THE DRAWINGS.
- RD9 PROVIDE METAL CLOSURE STRIPS AT OPEN UNCOVERED ENDS AND EDGES OF ROOF DECKING AND IN VOIDS BETWEEN DECKING AND OTHER CONSTRUCTION. WELD INTO POSITION TO PROVIDE A COMPLETE ENCLOSED DECKING INSTALLATION. PROVIDE FLEXIBLE CLOSURE STRIPS INSTEAD OF METAL CLOSURES, AT CONTRACTOR'S OPTION, WHEREVER THEIR USE WILL ENSURE COMPLETE CLOSURE. INSTALL WITH ADHESIVE IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.



PLAN VIEW
(ROOF)

NOTES:

1. A (K_d) OF 0.85 HAS BEEN USED IN THE DEVELOPMENT OF THESE VALUES. THE USE OF THESE VALUES SHALL ONLY BE APPLIED WHEN USED IN CONJUNCTION WITH LOAD COMBINATIONS SPECIFIED IN SECTIONS 2.3 AND 2.4 OF ASCE 7-22.
2. PRESSURES AND SUCTIONS ON SOFFITS SHALL BE THE SAME AS CORRESPONDING WALL ZONES 4 & 5.
3. IF THE STRUCTURE IS INSURED BY FACTORY MUTUAL (FM) THE GROSS UPLIFT DESIGN PRESSURES SHOWN HEREIN SHALL BE DOUBLE FOR ROOF COVERINGS.
4. THE 'ULTIMATE' WIND LOADS SHOWN IN THE COMPONENTS AND CLADDING SCHEDULE SHALL BE MULTIPLIED BY 0.8 TO REDUCE THEM DOWN TO 'SERVICE' LEVEL ALL TESTED ASSEMBLIES, INCLUDING BUT NOT LIMITED TO DOORS, WINDOWS AND ROOF ASSEMBLIES.
5. PARAPET WIND LOADS ARE BASED ON A 'SOLID' PARAPET WITH NO INTERNAL PRESSURE. ACTUAL PARAPET CONSTRUCTION MAY DICTATE THAT INTERNAL PRESSURE BE ADDED TO THE EXTERNAL PRESSURE IN ACCORDANCE WITH ASCE 7-22 SECTION 30.6.
6. UPLIFT VALUES FOR ZONE 3 MAY BE USED IN LIEU OF ZONE 3 FOR CORNER ZONES IF PARAPET HEIGHT WITH RESPECT TO FINISHED ROOF IS GREATER THAN 3 FT.
7. FOR ATTACHED CANOPIES ON BUILDING, REFERENCE ASCE 7-22 SECTION 30.4 FOR PRESSURE COEFFICIENTS.
8. DEAD LOAD OF 10 PSF (NO FURTHER REDUCTION ALLOWED) SHALL BE USED TO CONVERT GROSS UPLIFT TO NET UPLIFT FOR OPEN WEB JOIST DESIGN.

WIND DESIGN CRITERIA

Ultimate Design Wind Speed	160 mph
Service Design Wind Speed	130.1 mph
Risk Category	III
Exposure Category	C
Enclosure Building	Enclosed Building
Internal Pressure Coef.	+/-0.10

COMPONENT AND CLADDING ULTIMATE WIND PRESSURES

h = 47.0 ft a = 20.2 ft

Roof Area	Surface Pressure (psf)			
	10 sf	100 sf	500 sf	1000 sf
Negative Zone 1	-124.1	-86.9	-71.9	-71.9
Negative Zone 1	-113	-71.3	-48.2	-38.3
Negative Zone 2	-163.7	-120.7	-104.3	-104.3
Negative Zone 3	-223.1	-159.1	-104.3	-104.3
Positive All Zones	31.7	29.1	29.1	29.1

Overhang Zone 1a	-112.2	-105.6	-66.0	-66.0
Overhang Zone 2	-151.0	-105.2	-72.6	-72.6
Overhang Zone 3	-211.2	-129.6	-72.6	-72.6

Overhang pressures in the table above assume an internal pressure coefficient (C_{pi}) of 0.0
Overhang soffit pressure equals adj wall pressure (which includes internal pressure of 11.1 psf)

Parapet Area	Solid Parapet Pressure (psf)				
	10 sf	20 sf	50 sf	100 sf	500 sf
Zone 2	213.4	198.6	181.3	167.5	135.4
Zone 3	273.5	244.0	216.7	192.2	167.7

CASE B: Interior zone	-126.1	-119.7	-111.2	-104.9	-90.5	-90.0
Corner zone	-144.1	-134.5	-121.0	-112.3	-102.7	-90.0

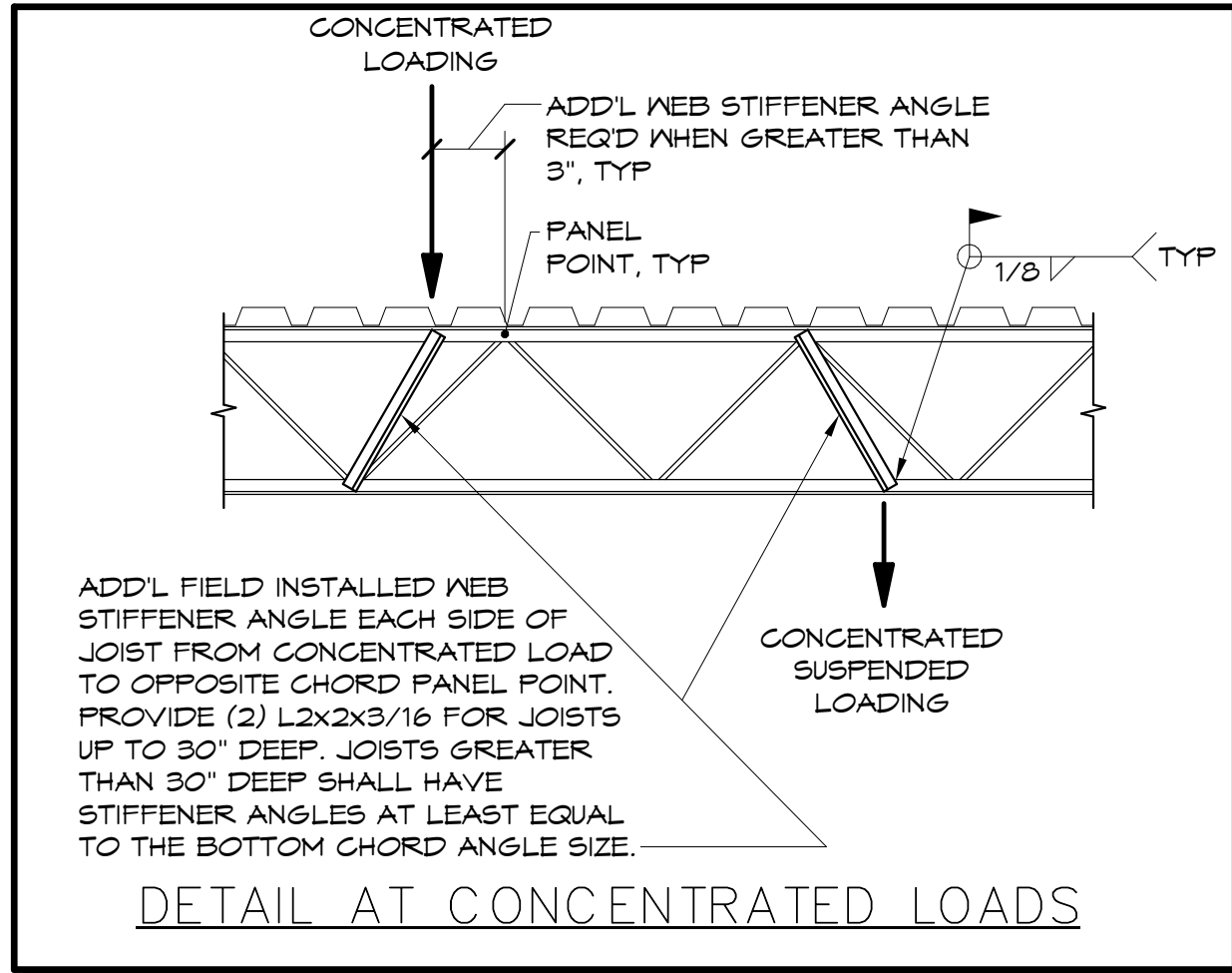
Wall Area	Surface Pressure (psf)			
	10 sf	100 sf	200 sf	500 sf
Negative Zone 4	-71.2	-66.7	-63.6	-59.4
Negative Zone 5	-85.0	-74.1	-67.1	-59.4
Positive Zone 4 & 5	11.3	60.0	57.6	53.5

TORNADO DESIGN CRITERIA

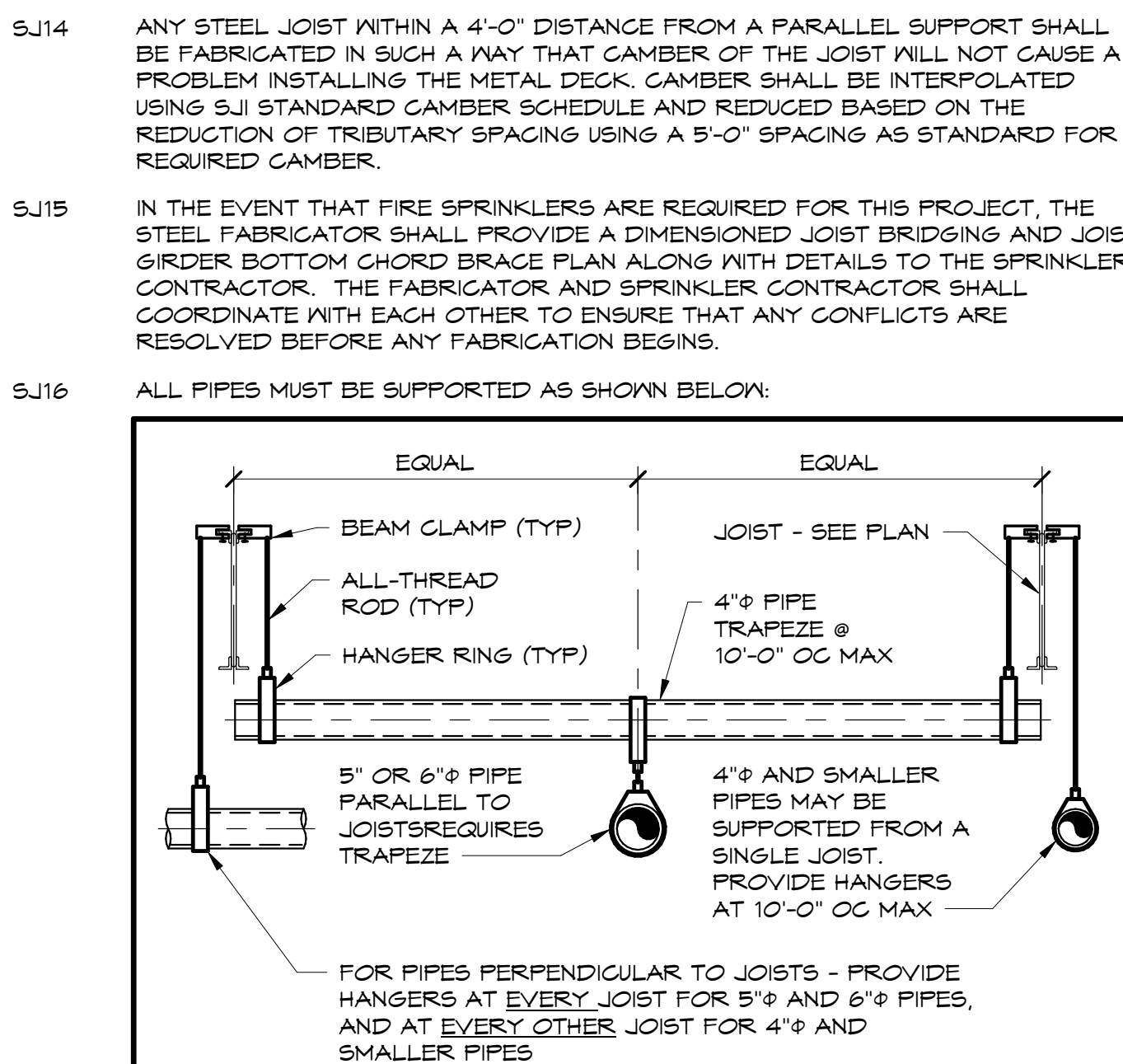
Tornado Wind Speed	50 mph
Effecting Area	47,456 sf
Internal Pressure Coef.	0.55
	-0.10
TORNADO DESIGN NOT REQUIRED	

STEEL JOISTS

- SJ1 A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
- SJ2 ALL DESIGN, FABRICATION AND ERECTION OF STEEL JOISTS AND BRIDGING SHALL BE IN STRICT ACCORDANCE WITH THE SPECIFICATIONS OF STEEL JOIST INSTITUTE (OPEN WEB STEEL JOIST AND JOIST GIRDERS SJ1 PUBLICATION SJ1 100-2015) AND RECOMMENDED CODE OF STANDARD PRACTICE.
- SJ3 THE ENDS OF ALL BRIDGING LINES TERMINATING AT WALLS OR BEAMS SHALL BE ANCHORED TO THE WALL OR BEAM.
- SJ4 ALL STEEL JOISTS ARE TO BE CAMBERED AS SPECIFIED BY STEEL JOIST INSTITUTE.
- SJ5 PROVIDE BOTTOM AND/OR TOP CHORD EXTENSIONS AS SHOWN ON DRAWINGS.
- SJ6 UNLESS NOTED OTHERWISE, MINIMUM JOIST BEARING SHALL BE 2 1/2" FOR K-SERIES JOISTS, 4" FOR LH, DLH AND SLH 15-10, AND 6" FOR SLH 19-25 ON A STEEL MEMBER OR EMBED PLATE.
- SJ7 BRIDGING SHALL BE FURNISHED AND INSTALLED TO MEET THE SIZE AND SPACING REQUIREMENTS OF THE SJ STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOISTS. ALL BRIDGING AND BRIDGING ANCHORS SHALL BE COMPLETELY INSTALLED BEFORE CONSTRUCTION LOADS ARE PLACED ON THE JOISTS.
- SJ8 ALL HANGERS, CURBS AND/OR ROOFTOP FRAMES TO SUPPORT MECHANICAL EQUIPMENT, ETC., TO BE SUPPORTED BY THE JOISTS SHALL BE LOCATED AT THE PANEL POINTS OF THE JOISTS IF POSSIBLE. HOWEVER, IF THE CONCENTRATED LOAD MUST BE LOCATED FURTHER THAN 3" FROM A PANEL POINT, PROVIDE WEB STIFFENER ANGLES. WEB STIFFENERS MUST BE INSTALLED EACH SIDE OF JOIST FROM CONCENTRATED LOAD TO OPPOSITE CHORD PANEL POINT BEFORE LOAD IS APPLIED. SEE DETAIL BELOW.
- SJ9 CONTRACTOR TO FURNISH BAR JOIST CERTIFICATIONS SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. THE SPECIALTY ENGINEER FOR THE STEEL JOIST SUPPLIER SHALL ALSO CERTIFY THAT THE STEEL JOIST BOTTOM CHORDS WILL SAFELY RESIST THE WIND UPLIFTS, CONSIDERING THE SPACING OF BRIDGING.
- SJ10 PROVIDE UPLIFT BRIDGING PER TABULATED PRESSURES ON SHEET 5004.
- SJ11 ALL ITEMS SUSPENDED FROM JOISTS (I.E. CATWALKS, BALCONIES, OPERABLE PARTITIONS, ETC.) SHALL BE INSTALLED AFTER DEAD LOAD HAS BEEN APPLIED.
- SJ12 BOLTED TIE JOISTS (BTJ) ARE USED IN STEEL FRAMES WHERE COLUMNS ARE NOT FRAMED IN AT LEAST TWO DIRECTIONS WITH STRUCTURAL STEEL MEMBERS. JOIST(S) AT COLUMN LINES SHALL BE FIELD BOLTED AT THE COLUMNS WITH TWO 1/2" BOLTS TO PROVIDE LATERAL STABILITY DURING CONSTRUCTION.
- SJ13 STEEL JOISTS SHALL RECEIVE SHOP COAT OF PRIMER (COLOR AS DIRECTED BY ARCHITECT) EXCEPT THOSE AREAS WHICH WILL RECEIVE SPRAY-ON FIRE PROTECTION.



DETAIL AT CONCENTRATED LOADS



- NOTES:
1. LOCATE HANGERS WITHIN 6" OF THE JOIST TOP CHORD PANEL POINTS.
2. PIPES LARGER THAN 6" SHALL BE SUBMITTED TO ARCHITECT FOR APPROVAL PRIOR TO FABRICATION OF JOISTS. INCLUDE LOCATION OF PIPE AND PREFERRED HANGING DETAIL IF OTHER THAN INDICATED ABOVE. JOISTS SHALL BE SPECIALLY DESIGNED BY THE FABRICATOR TO SUPPORT PIPES LARGER THAN 6". THE WEIGHT OF THE PIPES LARGER THAN 6" IS NOT INCLUDED IN THESE DOCUMENTS UNLESS SPECIFICALLY INDICATED OTHERWISE. COORDINATE WITH APPLICABLE TRADES.
3. PIPES SUSPENDED IN CONFORMANCE WITH THIS DETAIL WILL LIMIT THE PIPE SYSTEM WEIGHT TO 3.5 PSF EQUIVALENT DISTRIBUTED LOAD.

TYPICAL PIPE HANGER DETAIL

- SJ17 FABRICATOR SHALL ENSURE THAT ALL OSHA REQUIREMENTS ARE MET. PARTICULAR ATTENTION SHALL BE PAID TO THE ERECTION PROCESS. BOLTED CONNECTIONS MAY BE REQUIRED. SUBMIT DETAILS FOR APPROVAL.
- SJ18 THE JOIST MANUFACTURER SHALL NOT INCREASE ALLOWABLE STRESSES.
- SJ19 THE OPEN WEB STEEL JOISTS SHALL BE FABRICATED AND ERECTED IN FULL CONFORMANCE WITH THE 'OSHA STEEL ERECTION STANDARD'. IF THE CONSTRUCTION DRAWINGS DEVIATE FROM THE OSHA STANDARD THEN THE FABRICATOR SHALL PROVIDE SUBMITTALS THAT CLEARLY INDICATE THE DEVIATION WITH A REVISION CLOUD AND REQUEST APPROVAL FROM BBM TO MAKE THE CHANGE SO THAT CONFORMANCE WITH THE OSHA STANDARD IS ASSURED.
- SJ20 K-SERIES STEEL JOISTS WITH SPANS 40'-0" AND LONGER SHALL BE ERECTED IN PANELS SO THAT BOLTED CONNECTIONS ARE NOT REQUIRED (EXCEPT AT THE COLUMN LINES). THE GC SHALL INSURE THAT ALL RELATED JOIST FRAMING COMPONENTS ARE COORDINATED TO MEET THIS REQUIREMENT.
- SJ21 ALL ROOFS THAT EXCEED 1/4" FT SLOPE SHALL HAVE THE JOIST BEARING SEATS SLOPED AS REQUIRED PER STEEL JOIST INSTITUTE.

STRUCTURAL STEEL

- S51 A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
- S52 FABRICATE AND ERECT STRUCTURAL STEEL IN CONFORMANCE WITH THE LATEST VERSION OF AISC 360-16.
- S53 MATERIAL SPECIFICATIONS:
- ALL STEEL SHALL BE PRODUCED DOMESTICALLY.
- ROLLED SHAPES, PLATES AND BARS: ASTM A572 GR 50, EXCEPT WIDE-FLANGE 4 FT SECTIONS, WHICH SHALL BE ASTM A992.
- HOLLOW STRUCTURAL SECTION (HSS): ASTM A500, GRADE C.
- ANCHOR BOLTS, RODS, NUTS AND WASHERS: PER BASE PLATE SCHEDULE.
- HEADED STUDS: ASTM A108, GRADE 1015 THROUGH 1020, COLD-FINISHED CARBON STEEL, A58 D1.1, TYPE B.
- BOLTED STRUCTURAL CONNECTIONS: UNLESS NOTED OTHERWISE, ALL BOLTS SHALL BE 3/4" ASTM A325, TYPE N. BOLTS INDICATED LESS THAN 5/8" SHALL BE ASTM A307.
- WELDED CONNECTIONS: ELECTRODES - E70XX UNO (LOW HYDROGEN). FILLET WELDS SHALL BE 3/16" UNO.
- S54 HIGH-STRENGTH FIELD-BOLTED CONNECTIONS SHALL BE INSTALLED, TIGHTENED, TESTED AND INSPECTED ACCORDING TO 'SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS' BY THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (RCSC). ALL BOLTS IN STEEL TO STEEL CONNECTIONS SHALL BE BROUGHT TO A 'SNUG-TIGHT' CONDITION, AS DEFINED IN THE SPECIFICATION. ALL BOLTS IN STEEL TO EMBED CONNECTIONS SHALL BE TIGHT WITH PEENED THREADS. SLIP-CRITICAL (SC) BOLTS MUST BE FULLY TENSIONED PER SPECIFICATION.
- S55 STANDARD NON-SLOPED AND NON-SKEWED SHEAR CONNECTIONS HAVE BEEN DESIGNED AND THE NECESSARY INFORMATION MAY BE FOUND IN THE SCHEDULES. THE ULTIMATE (I.E. FACTORED) REACTIONS HAVE BEEN PROVIDED AT EACH END OF EACH MEMBER SHOULD THE FABRICATOR WISH TO RE-ENGINEER THE CONNECTIONS TO THEIR PREFERENCES. SHOULD THE FABRICATOR WISH TO RE-ENGINEER THE CONNECTIONS, THEY MUST PROVIDE SUBMITTALS AND CALCULATIONS THAT HAVE BEEN PREPARED AND SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE SAME STATE AS THE PROJECT LOCATION.
- S55A NON-STANDARD SLOPED AND/OR SKEWED SHEAR CONNECTIONS SHALL BE DESIGNED & DETAILED BY THE FABRICATOR'S ENGINEER. PROVIDE SIGNED & SEALED CONNECTION SUBMITTAL AND CALCULATIONS FOR REVIEW.
- S56 ALL WIDE FLANGE FLOOR MEMBERS SHALL BE CONNECTED TO THE SUPPORTING STRUCTURE AS DETAILED IN CONNECTION SCHEDULES ON SHEET 5501. UNLESS SPECIFICALLY NOTED OTHERWISE ON PLANS, ANY FLOOR MEMBER SUPPORTING ANOTHER FLOOR MEMBER SHALL BE CONNECTED AS DETAILED IN DOUBLE SHEAR SCHEDULE M1/5501. SINGLE SHEAR CONNECTIONS AS DETAILED IN SCHEDULES F1/5501 AND M1/5501 SHALL ONLY BE USED FOR FLOOR MEMBERS SUPPORTING DECK/SLAB ONLY (I.E. FILLER BEAMS) OR AS SPECIFICALLY NOTED OTHERWISE. THE USE OF A DOUBLE SHEAR CONNECTION MAY BE REQUIRED FOR A 'TYPICAL' FLOOR BEAM DUE TO BEAM REACTION. REFER TO REACTION NOTED ON PLAN(S) & COORDINATE WITH SCHEDULED MAXIMUM VALUES TO DETERMINE CONNECTION TYPE REQUIRED. SEE PARTIAL SCHEMATIC FRAMING PLAN BELOW THAT ILLUSTRATES WHERE TYPICAL SINGLE AND DOUBLE SHEAR CONNECTIONS ARE REQUIRED.
- S57 ALL WIDE FLANGE ROOF MEMBERS SHALL BE CONNECTED TO THE SUPPORTING STRUCTURE AS DETAILED IN THE CONNECTION SCHEDULES ON SHEET 5501. UNLESS SPECIFICALLY NOTED OTHERWISE ON PLAN, ALL ROOF MEMBERS SHALL BE CONNECTED AS DETAILED IN THE SINGLE SHEAR SCHEDULES F1/5501 AND M1/5501.
- S58 BRACE AND MAINTAIN ALL STEEL IN ALIGNMENT UNTIL OTHER PARTS OF CONSTRUCTION NECESSARY FOR PERMANENT SUPPORT ARE COMPLETED. CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLING TEMPORARY SHORING AS REQUIRED FOR THE STABILITY OF THE STEEL FRAME UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN COMPLETED AND BUILDING IS ENCLOSED.
- S59 ALL WELDING SHALL CONFORM TO THE REQUIREMENTS OF 'THE STANDARD CODE FOR WELDING IN BUILDING CONSTRUCTION' OF THE AMERICAN WELDING SOCIETY.
- S510 GROUT FOR COLUMN BASE PLATES AND PRESET BEARING PLATES SHALL BE NON-SHRINK, NON-METALLIC GROUT (5000 PSI MIN).
- S511 SUBMIT SHOP DRAWINGS INDICATING ALL SHOP AND ERECTION DETAILS INCLUDING PROFILES, SIZES, SPACING AND LOCATIONS OF STRUCTURAL MEMBERS, CONNECTION ATTACHMENTS, FASTENERS, LOADS AND TOLERANCES.
- S512 ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIP GALVANIZED IN ACCORDANCE WITH ASTM A123 FOR MEMBERS AND ASTM A193 FOR CONNECTION ELEMENTS. EXCEPT THAT ALL ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS) SHALL BE BLAST CLEANED AND COATED IN ACCORDANCE WITH THE STRUCTURAL STEEL AND PAINT SPECIFICATIONS.
- S513 STRUCTURAL STEEL SHALL RECEIVE A SHOP COAT OF PRIMER (COLOR AS DIRECTED BY ARCHITECT) EXCEPT THOSE AREAS WHICH WILL RECEIVE SPRAY-ON FIRE PROTECTION, OR WHERE HEADED STUDS ARE TO BE WELDED.
- S514 PROVIDE ALLOWANCE FOR 300 FEET OF L3x3x1/4 AND LABOR TO INSTALL SAME AT ARCHITECT OR ENGINEER'S DIRECTION. CREDIT OWNER WITH ALL MATERIAL AND LABOR NOT USED.
- S515 STEEL BEAMS SHALL BE CAMBERED AS INDICATED ON THE FRAMING PLAN(S). IF NO CAMBER IS SPECIFIED, THE FABRICATOR SHALL ENSURE THAT THE 'NATURAL' CAMBER IN ALL ERECTED BEAMS OCCURS IN AN UPWARD DIRECTION.
- S516 ALL STEEL BEAMS THAT ARE PARALLEL TO BAR JOISTS SHALL BE CAMBERED TO MATCH THE JOIST CAMBER AND THE TOP OF THE BEAM SHALL MATCH THE TOP OF THE JOIST, TYP UNO.
- S517 THE STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN FULL CONFORMANCE WITH THE 'OSHA STEEL ERECTION STANDARD'. IF THE CONSTRUCTION DRAWINGS DEVIATE FROM THE OSHA STANDARD THEN THE FABRICATOR SHALL PROVIDE SUBMITTALS THAT CLEARLY INDICATE THE DEVIATION WITH A REVISION CLOUD AND REQUEST APPROVAL FROM BBM TO MAKE THE CHANGE SO THAT CONFORMANCE WITH THE OSHA STANDARD IS ASSURED.
- S518 REFER TO SPECIALTY ENGINEERING (SE) NOTES FOR DELEGATED ENGINEERING REQUIREMENTS.



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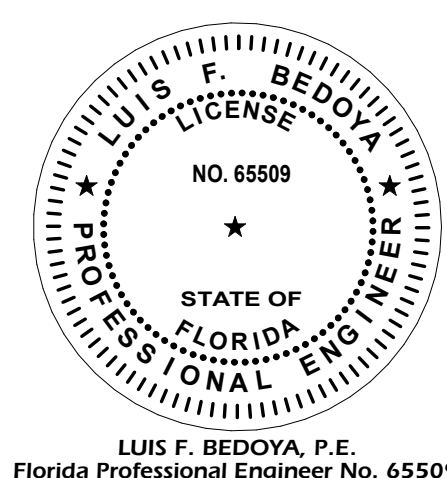
VERO CLASSICAL
CHRISTIAN SCHOOL -
PHASE I

PROJECT LOCATION: 13TH ST. SW AND 58TH AVE. SW
VERO BEACH, FLORIDA



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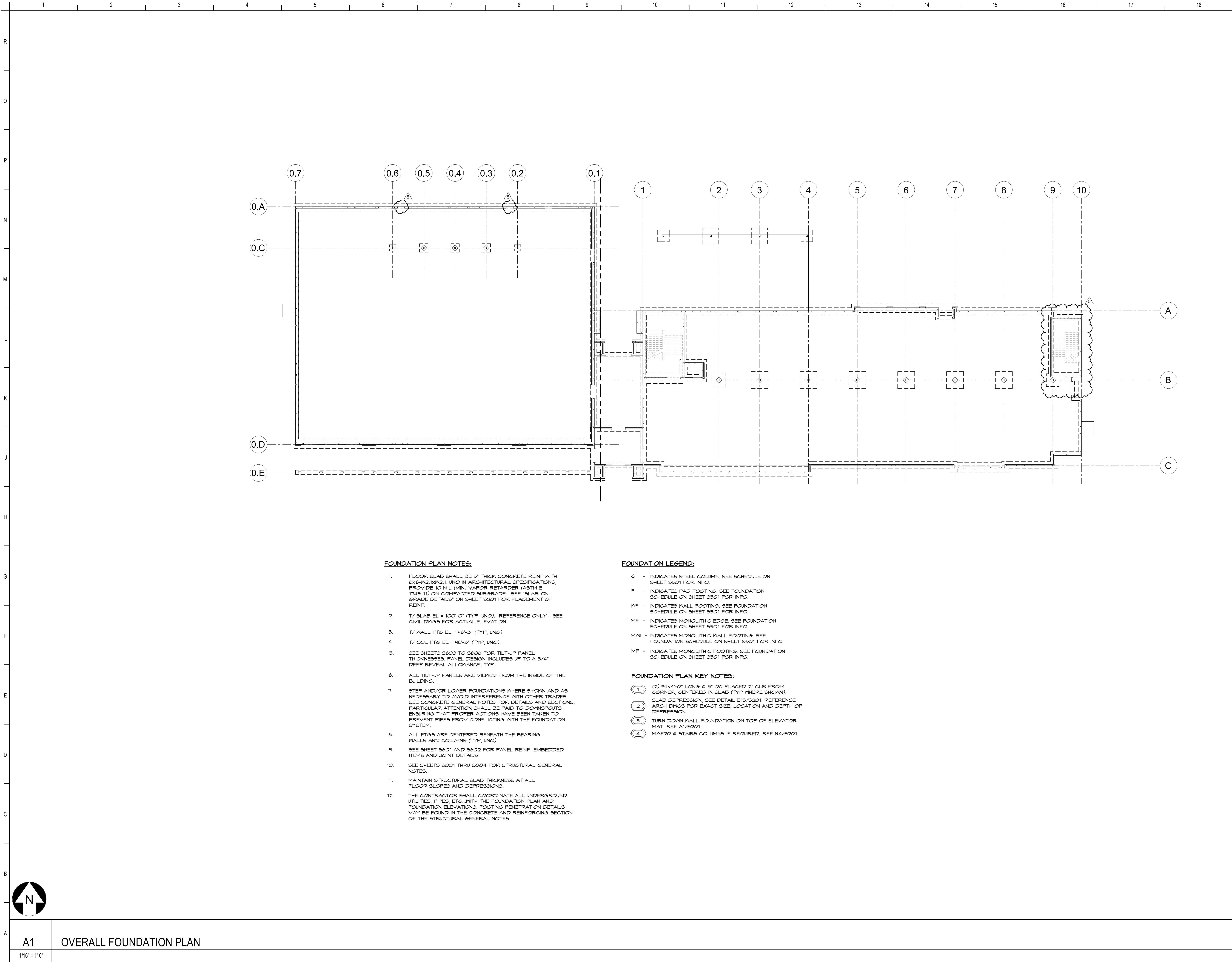
A1 WIND SCHEDULE

A13 STRUCTURAL GENERAL NOTES

COMM. NO.: 2024117
ISSUE DATE: 12/23/2024
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STRUCTURAL GENERAL
NOTES & WIND SCHEDULES

S004
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VERO CLASSICAL
CHRISTIAN SCHOOL -
PHASE I

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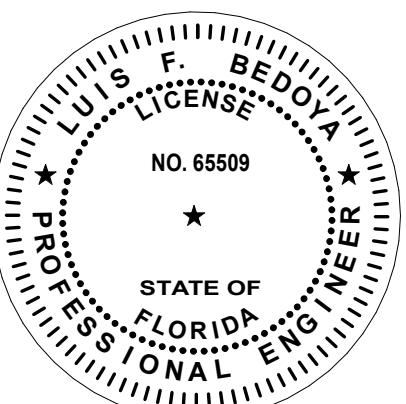
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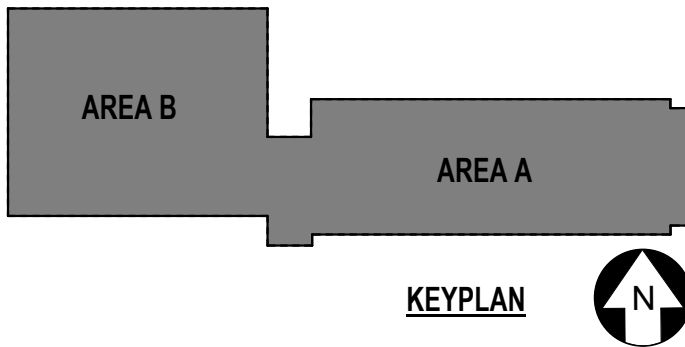


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BBM PROJECT #24133



LUIS F. BEDOYA, P.E.
Florida Professional Engineer No. 65509

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MARK	DESCRIPTION	DATE
5	Addendum 01 - R/C BUILDING REVIEW COMMENTS	6/6/2025



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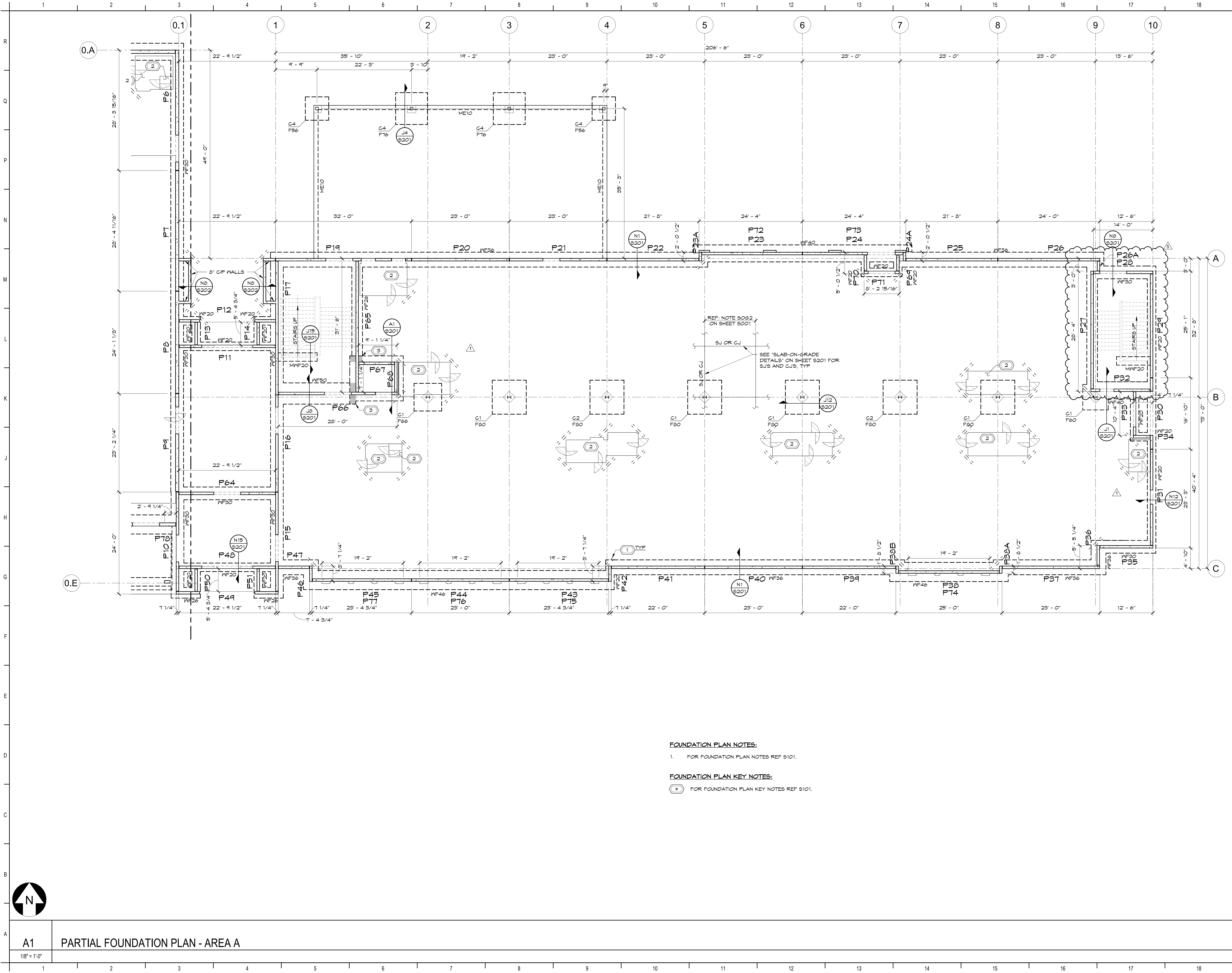
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OVERALL FOUNDATION
PLAN

S101

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FOUNDATION PLAN NOTES:

1. FOR FOUNDATION PLAN NOTES REF S101.

FOUNDATION PLAN KEY NOTES:

1. FOR FOUNDATION PLAN KEY NOTES REF S101.



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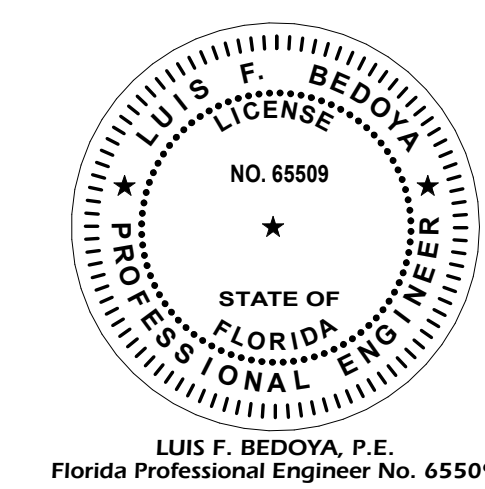
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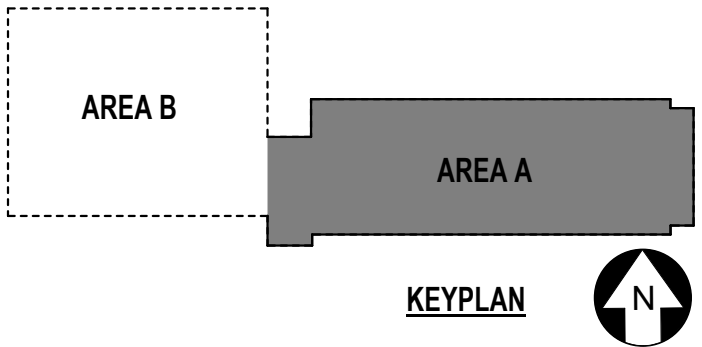


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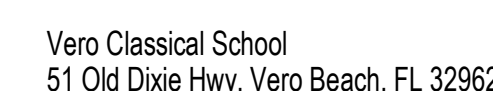
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1	Addendum 01	11/04/2025
2	Addendum 02 - SEE BUILDING REVIEW COMMENTS	11/05/2025



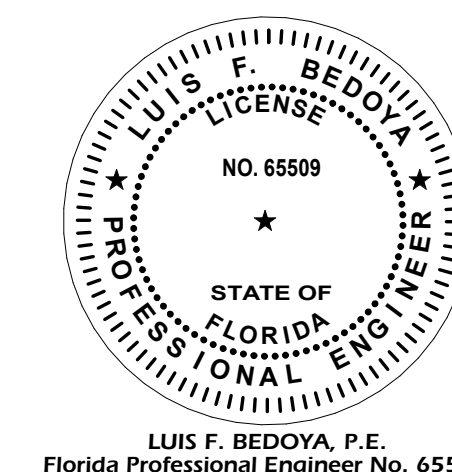
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**PARTIAL FOUNDATION
PLAN - AREA A**

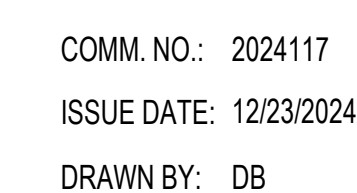
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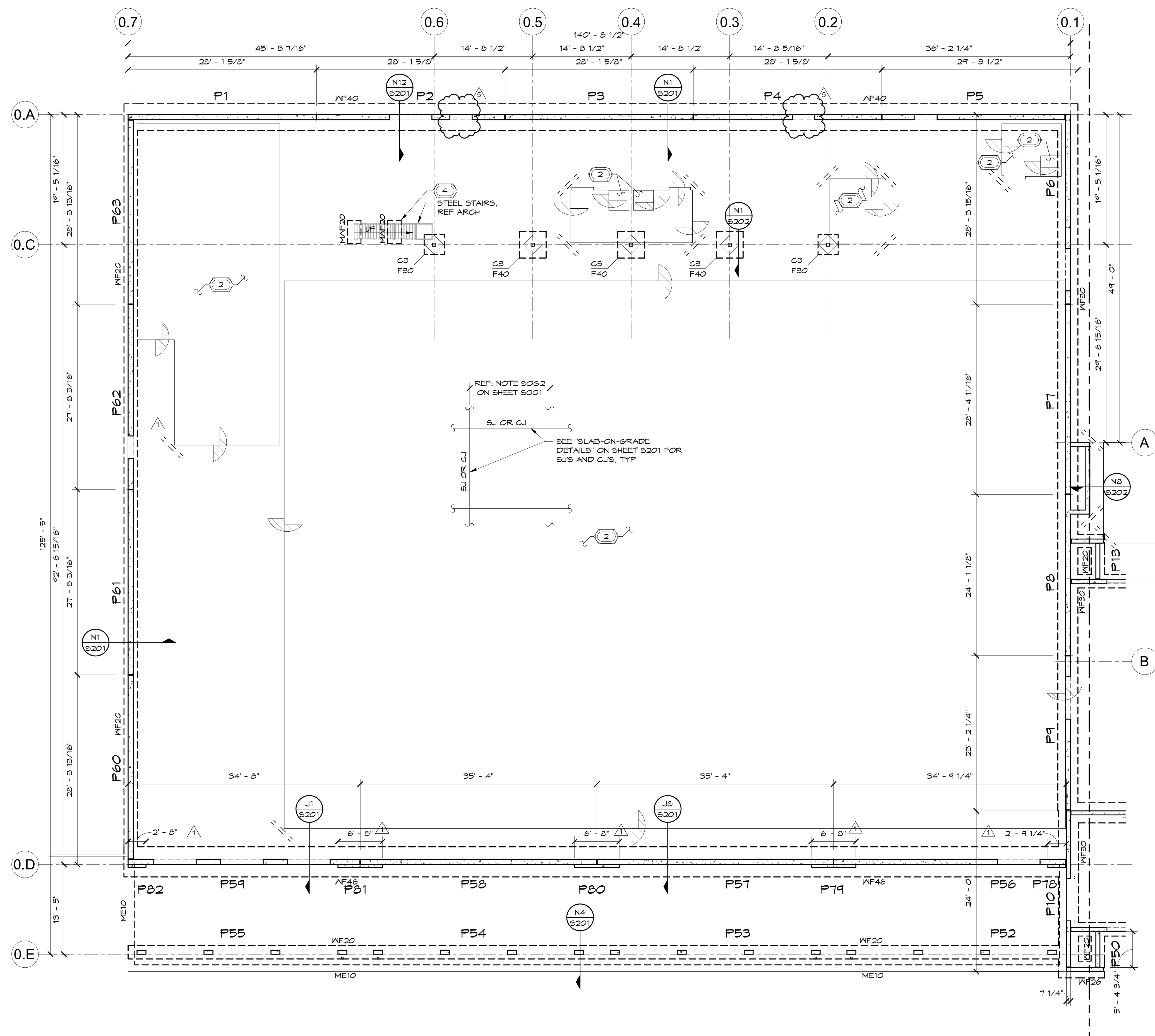
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1	Addendum -01	01/24/2025
5	Addendum -04 RDC BUILDING REVIEW COMMENTS	06/03/25



S101.B
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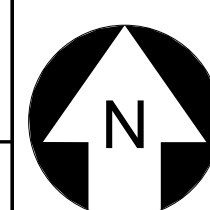


FOUNDATION PLAN NOTES:

1. FOR FOUNDATION PLAN NOTES REF S101.

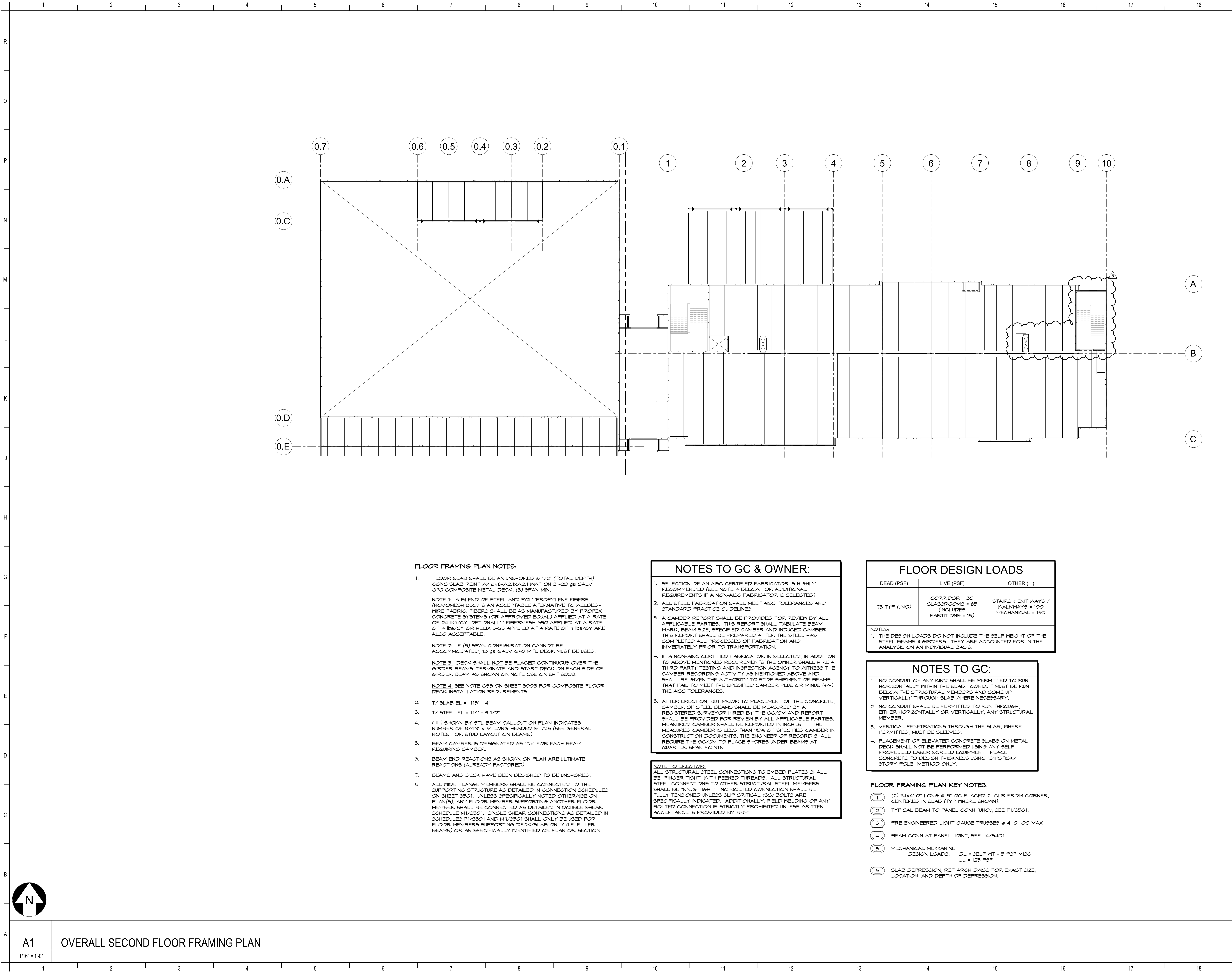
FOUNDATION PLAN KEY NOTES:

- FOR FOUNDATION PLAN KEY NOTES REF S101



A1

PARTIAL FOUNDATION PLAN - AREA B



FLOOR FRAMING PLAN NOTES:

- FLOOR SLAB SHALL BE AN UNSHORED 6 1/2" (TOTAL DEPTH) CONC SLAB REINF 1/4" 6x6-A2.1X42.1 W/WF ON 3"-20 GA GALV 690 COMPOSITE METAL DECK. (3) SPAN MIN.
NOTE 1: A BLEND OF STEEL AND POLYPROPYLENE FIBERS (NOVOMESH 650) IS AN ACCEPTABLE ALTERNATIVE TO WELDED-WIRE FABRIC. FIBERS SHALL BE AS MANUFACTURED BY PROPEX CONCRETE SYSTEMS (OR APPROVED EQUAL) APPLIED AT A RATE OF 24 LBS/CY. OPTIONALLY FIBERMESH 650 APPLIED AT A RATE OF 4 LBS/CY OR HELIX 5-25 APPLIED AT A RATE OF 7 LBS/CY ARE ALSO ACCEPTABLE.
NOTE 2: IF (3) SPAN CONFIGURATION CANNOT BE ACCOMMODATED, 18 GA GALV 690 MTL DECK MUST BE USED.
NOTE 3: DECK SHALL NOT BE PLACED CONTINUOUS OVER THE GIRDER BEAMS. TERMINATE AND START DECK ON EACH SIDE OF GIRDER BEAM AS SHOWN ON NOTE C56 ON SHT S003.
NOTE 4: SEE NOTE C58 ON SHEET S003 FOR COMPOSITE FLOOR DECK INSTALLATION REQUIREMENTS.
- T/ SLAB EL = 115' - 4"
- T/ STEEL EL = 114' - 9 1/2"
- (#) SHOWN BY STL BEAM CALLOUT ON PLAN INDICATES NUMBER OF 3/4" x 5' LONG HEADED STUDS (SEE GENERAL NOTES FOR STUD LAYOUT ON BEAMS).
- BEAM CAMBER IS DESIGNATED AS "C" FOR EACH BEAM REQUIRING CAMBER.
- BEAM END REACTIONS AS SHOWN ON PLAN ARE ULTIMATE REACTIONS (ALREADY FACTORED).
- BEAMS AND DECK HAVE BEEN DESIGNED TO BE UNSHORED.
- ALL WIDE FLANGE MEMBERS SHALL BE CONNECTED TO THE SUPPORTING STRUCTURE AS DETAILED IN CONNECTION SCHEDULES ON SHEET S501. UNLESS SPECIFICALLY NOTED OTHERWISE ON PLAN(S), ANY FLOOR MEMBER SUPPORTING ANOTHER FLOOR MEMBER SHALL BE CONNECTED AS DETAILED IN DOUBLE SHEAR SCHEDULE M1/S501. SINGLE SHEAR CONNECTIONS AS DETAILED IN SCHEDULES F1/S501 AND M1/S501 SHALL ONLY BE USED FOR FLOOR MEMBERS SUPPORTING DECK/SLAB ONLY (I.E. FILLER BEAMS) OR AS SPECIFICALLY IDENTIFIED ON PLAN OR SECTION.

NOTES TO GC & OWNER:

- SELECTION OF AN AISC CERTIFIED FABRICATOR IS HIGHLY RECOMMENDED (SEE NOTE 4 BELOW FOR ADDITIONAL REQUIREMENTS IF A NON-AISC FABRICATOR IS SELECTED).
- ALL STEEL FABRICATION SHALL MEET AISC TOLERANCES AND STANDARD PRACTICE GUIDELINES.
- A CAMBER REPORT SHALL BE PROVIDED FOR REVIEW BY ALL APPLICABLE PARTIES. THIS REPORT SHALL TABULATE BEAM MARK, BEAM SIZE, SPECIFIED CAMBER AND INDUCED CAMBER. THIS REPORT SHALL BE PREPARED AFTER THE STEEL HAS COMPLETED ALL PROCESSES OF FABRICATION AND IMMEDIATELY PRIOR TO TRANSPORTATION.
- IF A NON-AISC CERTIFIED FABRICATOR IS SELECTED, IN ADDITION TO ABOVE MENTIONED REQUIREMENTS THE OWNER SHALL HIRE A THIRD PARTY TESTING AND INSPECTION AGENCY TO WITNESS THE CAMBER RECORDING ACTIVITY AS MENTIONED ABOVE AND SHALL BE GIVEN THE AUTHORITY TO STOP SHIPMENT OF BEAMS THAT FAIL TO MEET THE SPECIFIED CAMBER PLUS OR MINUS (+/-) THE AISC TOLERANCES.
- AFTER ERECTION, BUT PRIOR TO PLACEMENT OF THE CONCRETE, CAMBER OF STEEL BEAMS SHALL BE MEASURED BY A REGISTERED SURVEYOR HIRED BY THE GC/GM AND REPORT SHALL BE PROVIDED FOR REVIEW BY ALL APPLICABLE PARTIES. MEASURED CAMBER SHALL BE REPORTED IN INCHES. IF THE MEASURED CAMBER IS LESS THAN 75% OF SPECIFIED CAMBER IN CONSTRUCTION DOCUMENTS, THE ENGINEER OF RECORD SHALL REQUIRE THE GC/GM TO PLACE SHORES UNDER BEAMS AT QUARTER SPAN POINTS.

NOTE TO ERECTOR:
ALL STRUCTURAL STEEL CONNECTIONS TO EMBED PLATES SHALL BE "FINGER TIGHT" WITH PEENED THREADS. ALL STRUCTURAL STEEL CONNECTIONS TO OTHER STRUCTURAL STEEL MEMBERS SHALL BE "SNUG TIGHT". NO BOLTED CONNECTION SHALL BE FULLY TENSIONED UNLESS SLIP CRITICAL (SC) BOLTS ARE SPECIFICALLY INDICATED. ADDITIONALLY, FIELD WELDING OF ANY BOLTED CONNECTION IS STRICTLY PROHIBITED UNLESS WRITTEN ACCEPTANCE IS PROVIDED BY BBM.

FLOOR DESIGN LOADS

DEAD (PSF)	LIVE (PSF)	OTHER ()
75 TYP (UNO)	CORRIDOR = 80 CLASSROOMS = 65 (INCLUDES PARTITIONS = 15)	STAIRS & EXIT WAYS / WALKWAYS = 100 MECHANICAL = 150

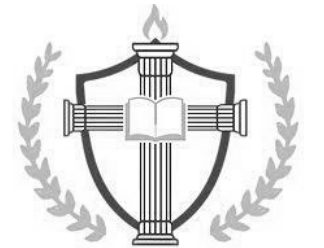
NOTES:
1. THE DESIGN LOADS DO NOT INCLUDE THE SELF WEIGHT OF THE STEEL BEAMS & GIRDERS. THEY ARE ACCOUNTED FOR IN THE ANALYSIS ON AN INDIVIDUAL BASIS.

NOTES TO GC:

- NO CONDUIT OF ANY KIND SHALL BE PERMITTED TO RUN HORIZONTALLY WITHIN THE SLAB. CONDUIT MUST BE RUN BELOW THE STRUCTURAL MEMBERS AND COME UP VERTICALLY THROUGH SLAB WHERE NECESSARY.
- NO CONDUIT SHALL BE PERMITTED TO RUN THROUGH, EITHER HORIZONTALLY OR VERTICALLY, ANY STRUCTURAL MEMBER.
- VERTICAL PENETRATIONS THROUGH THE SLAB, WHERE PERMITTED, MUST BE SLEEVED.
- PLACEMENT OF ELEVATED CONCRETE SLABS ON METAL DECK SHALL NOT BE PERFORMED USING ANY SELF PROPELLED LASER SCREED EQUIPMENT. PLACE CONCRETE TO DESIGN THICKNESS USING "DIPSTICK/STORY-POLE" METHOD ONLY.

FLOOR FRAMING PLAN KEY NOTES:

- (2) 4"x4'-0" LONG @ 3' OC PLACED 2' CLR FROM CORNER, CENTERED IN SLAB (TYP WHERE SHOWN).
- TYPICAL BEAM TO PANEL CONN (UNO), SEE F1/S501.
- PRE-ENGINEERED LIGHT GAUGE TRUSSES @ 4'-0" OC MAX
- BEAM CONN AT PANEL JOINT, SEE J4/S401.
- MECHANICAL MEZZANINE
DESIGN LOADS: DL = SELF WT + 5 PSF MISG
LL = 125 PSF
- SLAB DEPRESSION, REF ARCH DVNGS FOR EXACT SIZE, LOCATION, AND DEPTH OF DEPRESSION.



VERO CLASSICAL
SCHOOL

Vero Classical School
51 Old Dixie Hwy, Vero Beach, FL 32962

VERO CLASSICAL
CHRISTIAN SCHOOL -
PHASE I

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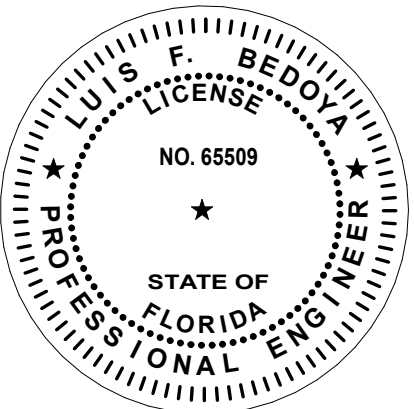
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LUIS F. BEDOYA, P.E.
Florida Professional Engineer No. 65509

REVISIONS		
MARK	DESCRIPTION	DATE
5	Adopted 01_RFC BUILDING REVIEW COMMENTS	6/6/2025

AREA B

AREA A

KEYPLAN



COMM. NO.: 2024117

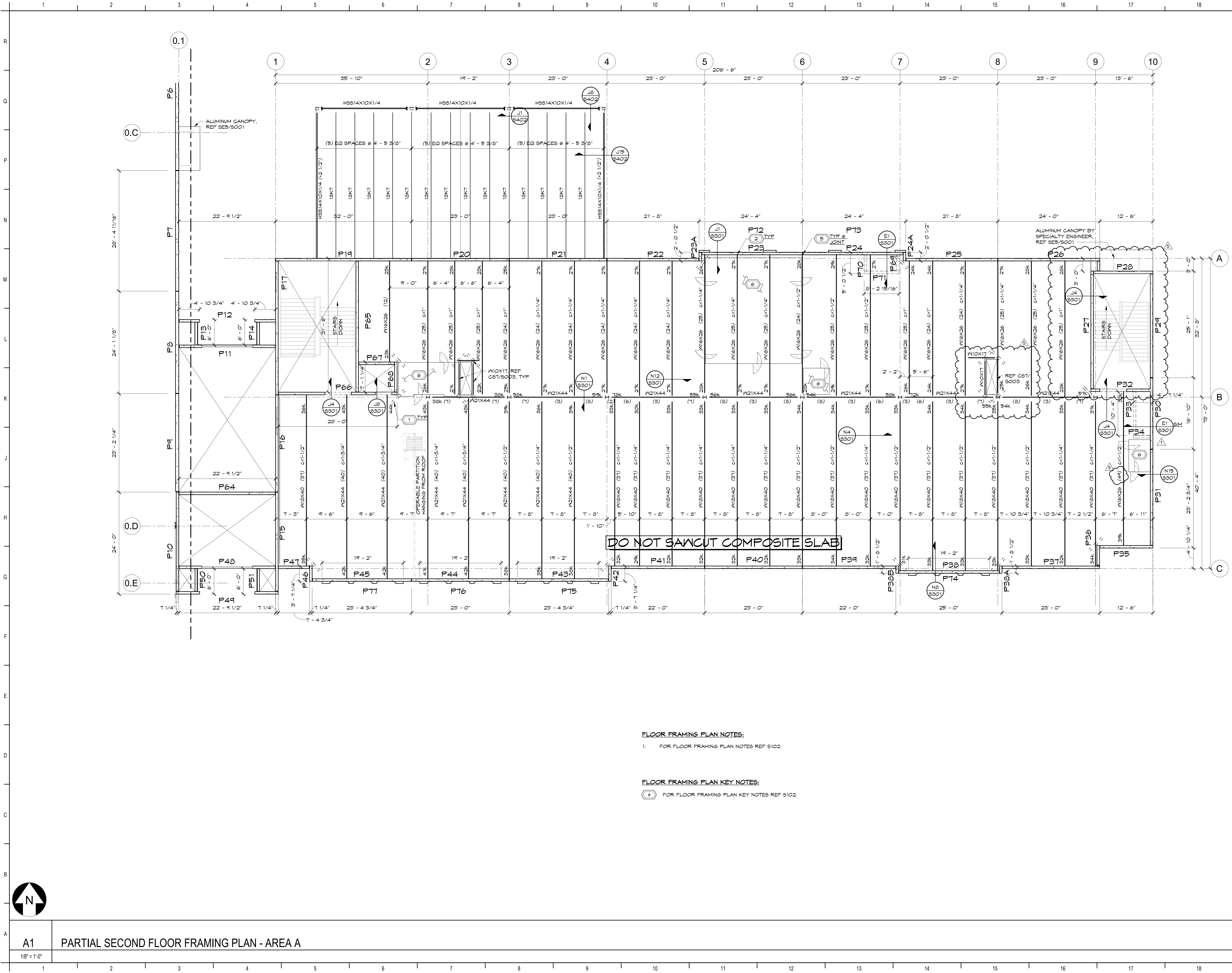
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OVERALL SECOND FLOOR
FRAMING PLAN

S102

BID/ PERMIT DOCUMENTS



FLOOR FRAMING PLAN NOTES:

1. FOR FLOOR FRAMING PLAN NOTES REF S102.

FLOOR FRAMING PLAN KEY NOTES:

1. FOR FLOOR FRAMING PLAN KEY NOTES REF S102.



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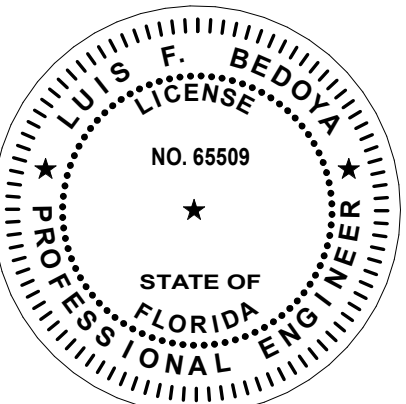
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MARK	REVISIONS	DESCRIPTION	DATE
1	ADDENDUM 01		12/23/2024
2	ADDENDUM 02	FOR BUILDING REVIEW COMMENTS	12/23/2024

AREA B

AREA A

KEYPLAN



COMM. NO.: 2024117

ISSUE DATE: 12/23/2024

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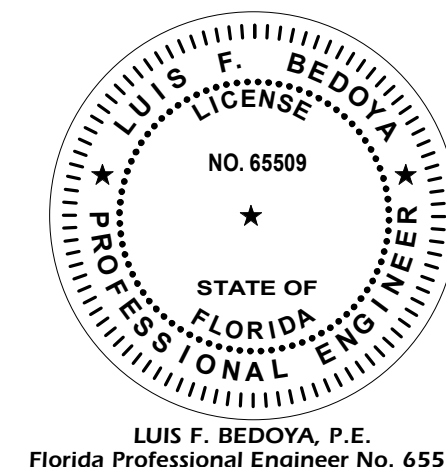
PARTIAL SECOND FLOOR
FRAMING PLAN - AREA A

S102.A

BID / PERMIT DOCUMENTS



PROJECT LOCATION: 13TH ST. SW AND 58TH AVE. SW
VERO BEACH, FLORIDA



KEYPLAN

S102.B
BID/ PERMIT DOCUMENTS



1. FOR FLOOR FRAMING PLAN NOTES REF S102
2. T/ MEZZANINE SLAB EL = 115' - 0"
T/ STEEL EL = 114' 5 1/2"

FLOOR FRAMING PLAN KEY NOTES:

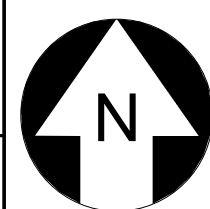
-  FOR FLOOR FRAMING PLAN KEY NOTES REF S102

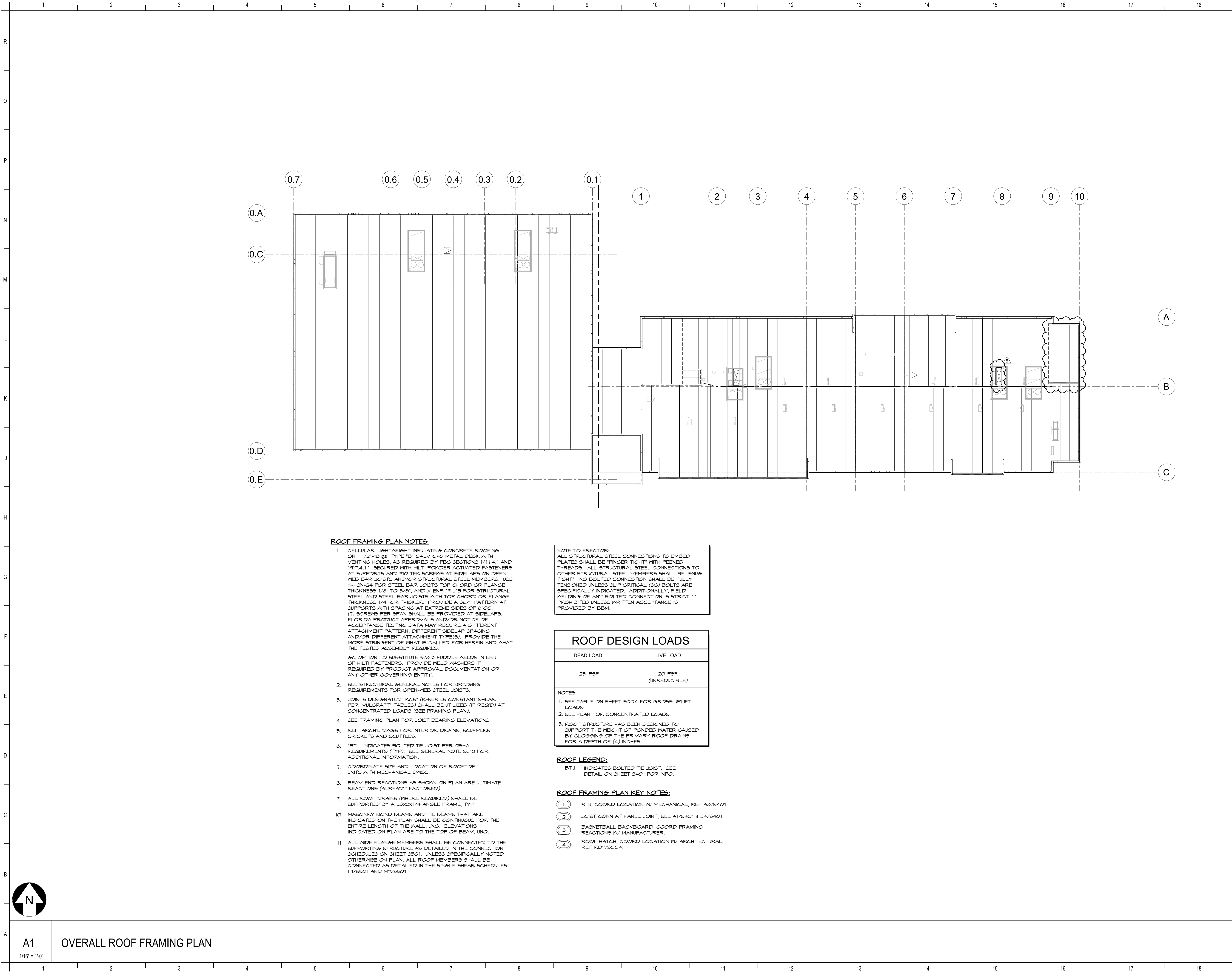
- LIGHT GAGE TRUSS ROOF FRAMING PLAN NOTES:

1. ALL LIGHT STEEL TRUSSES SHALL BE DESIGNED AND CERTIFIED BY TRUSS MANUFACTURER'S REGISTERED ENGINEER. ALL TRUSS-TO-TRUSS & TRUSS-TO-STRUCTURE CONNECTIONS SHALL BE DESIGNED AND DETAILED BY THE TRUSS MANUFACTURER. CONNECTIONS SHALL BE DESIGNED FOR DEAD, LIVE AND WIND (BOTH UPLIFT & LATERAL) LOADS. THE WIND PRESSURE AND Suction EXERTED ON THE WALLS MAY BE FOUND IN THE WIND SCHEDULES.
2. THE TRUSS MANUFACTURER SHALL VERIFY ALL DIMENSIONS AND SUBMIT SHOP DRAWINGS TO ARCHITECT FOR APPROVAL.
3. SEE ARCHITECTURAL BUILDING SECTIONS, STRUCTURAL FRAMING PLANS AND TRUSS ELEVATIONS FOR ROOF FITCHES (TYP).
4. SEE ARCHITECTURAL BUILDING SECTIONS, WALL SECTIONS AND STRUCTURAL FRAMING PLAN(S) FOR BEARING HEIGHTS.
5. TRUSS SPACING SHALL BE MAX 4'-0" OC (INO).
6. REFER TO ARCHITECTURAL DRAWINGS FOR FRAMED-DOWN CEILINGS, VOLUME CEILINGS AND OTHER INTERIOR TREATMENTS.
7. FIC PRE-FABRICATED LIGHT STEEL TRUSS NOTES IN GENERAL NOTE SHEETS FOR BRIDGING REQUIREMENTS FOR METAL TRUSSES.
8. ROOFING ON 1/2"-22 gage type "B" GALV G90 METAL DECK SECURED WITH #12 TIE SCREWS AT SUPPORTS AND #10 TIE SCREWS AT SIDELAP. ACCEPTED LIGHT GAGE METAL TRUSSES PROVIDE A 3/64" PATTERNS AT SUPPORTS &/V SPACING AT EXTREME SIDES OF 6" OC. (3) SCREWS PER SPAN SHALL BE PROVIDED AT SIDELAPS. FLORIDA PRODUCT APPROVALS AND/OR NOTICE OF SIDELAP ACCEPTANCE TESTING DATA MAY REQUIRE A DIFFERENT ATTACHMENT PATTERN, DIFFERENT SIDELAP SPACING AND/OR DIFFERENT ATTACHMENT TYPE. PROVIDE THE MORE STRINGENT OF WHAT IS CALLED FOR HEREIN AND IN FLORIDA PRODUCT APPROVALS.

NOTE TO GC:
ALL TEMPORARY AND PERMANENT BRACING FOR THE TRUSS SYSTEM SHALL BE DESIGNED BY A SPECIALTY ENGINEER. PROVIDE SIGNED AND SEALED SHOP DRAWINGS FOR REVIEW AND APPROVAL.

TRUSS DESIGN BASIS			
DEAD LOAD (PSF)		LIVE LOAD (PSF)	
TOP CHORD	BOTTOM CHORD	TOP CHORD	BOTTOM CHORD
25	10	20 REDUCIBLE	0





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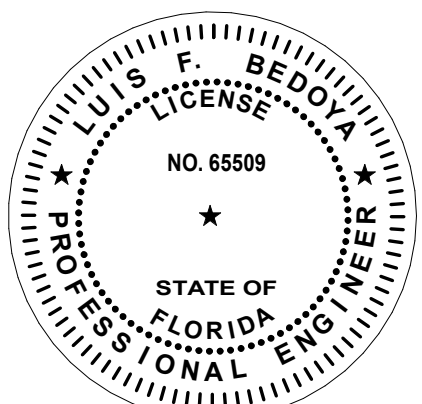
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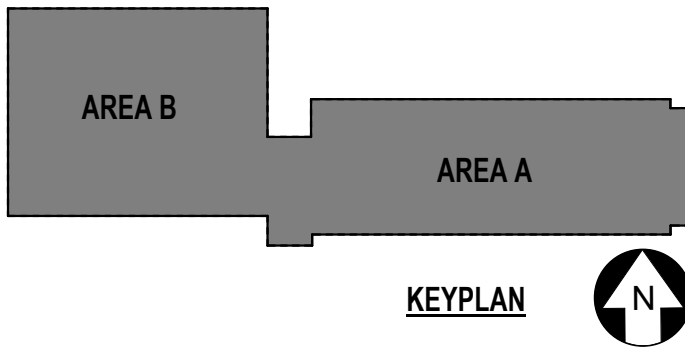


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REVISIONS		
MARK	DESCRIPTION	DATE
5	Adendum 01 - R/C BUILDING REVIEW COMMENTS	08/2025



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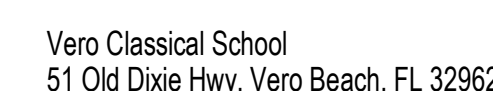
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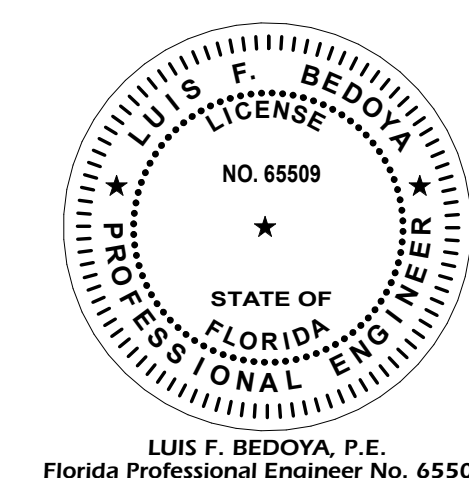
OVERALL ROOF FRAMING
PLAN

S103

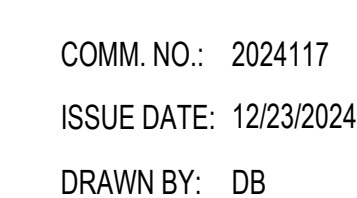
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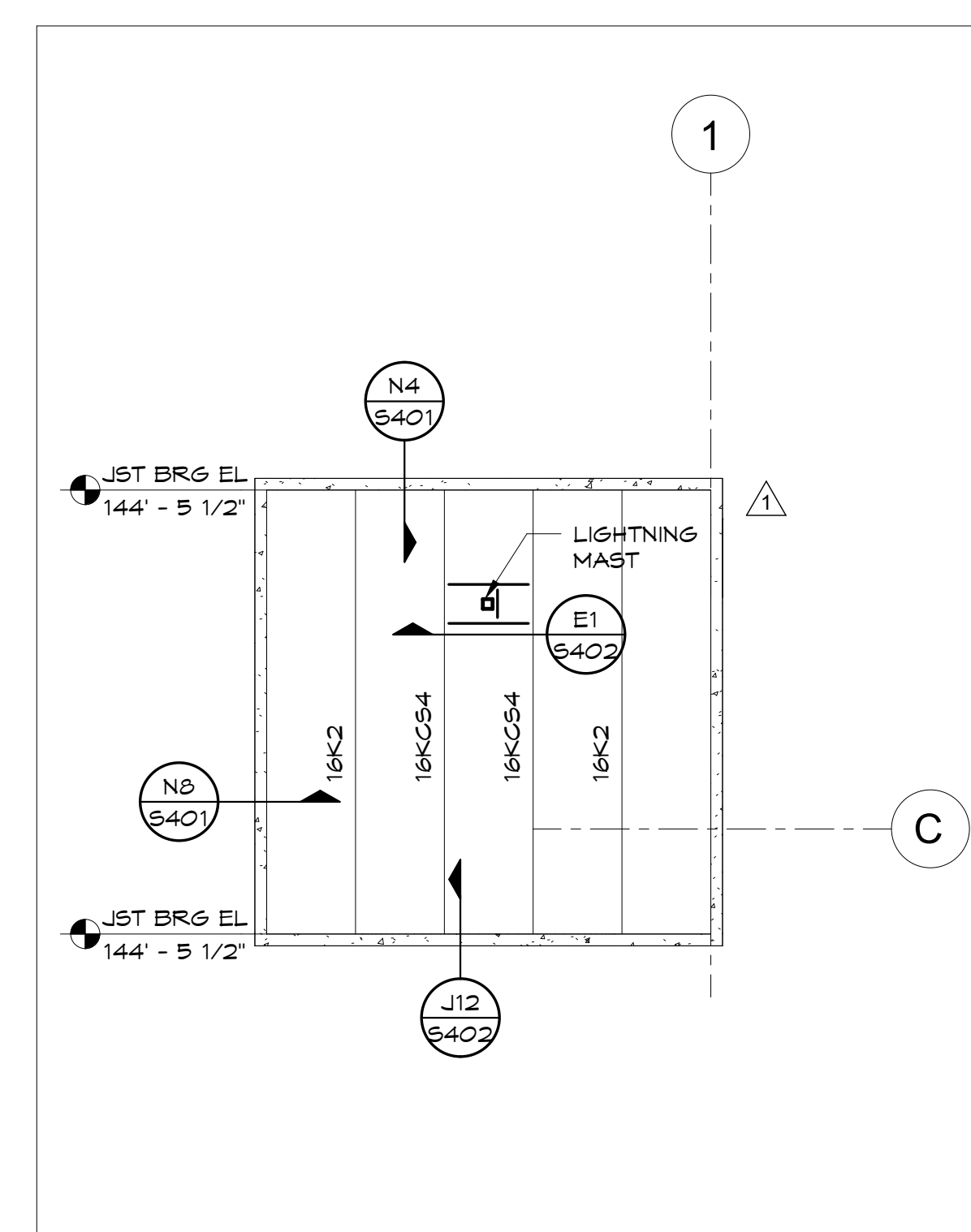
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MARK	DESCRIPTION	DATE
1	Addendum -01	01/24/2025
2	Addendum -02	02/07/2025
6	Addendum -04 IRC BUILDING DEVIATION COMMENTS	04/09/2025



S103.A
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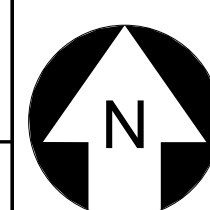


ROOF FRAMING PLAN NOTES:

1. FOR ROOF FRAMING PLAN NOTES REF S103.

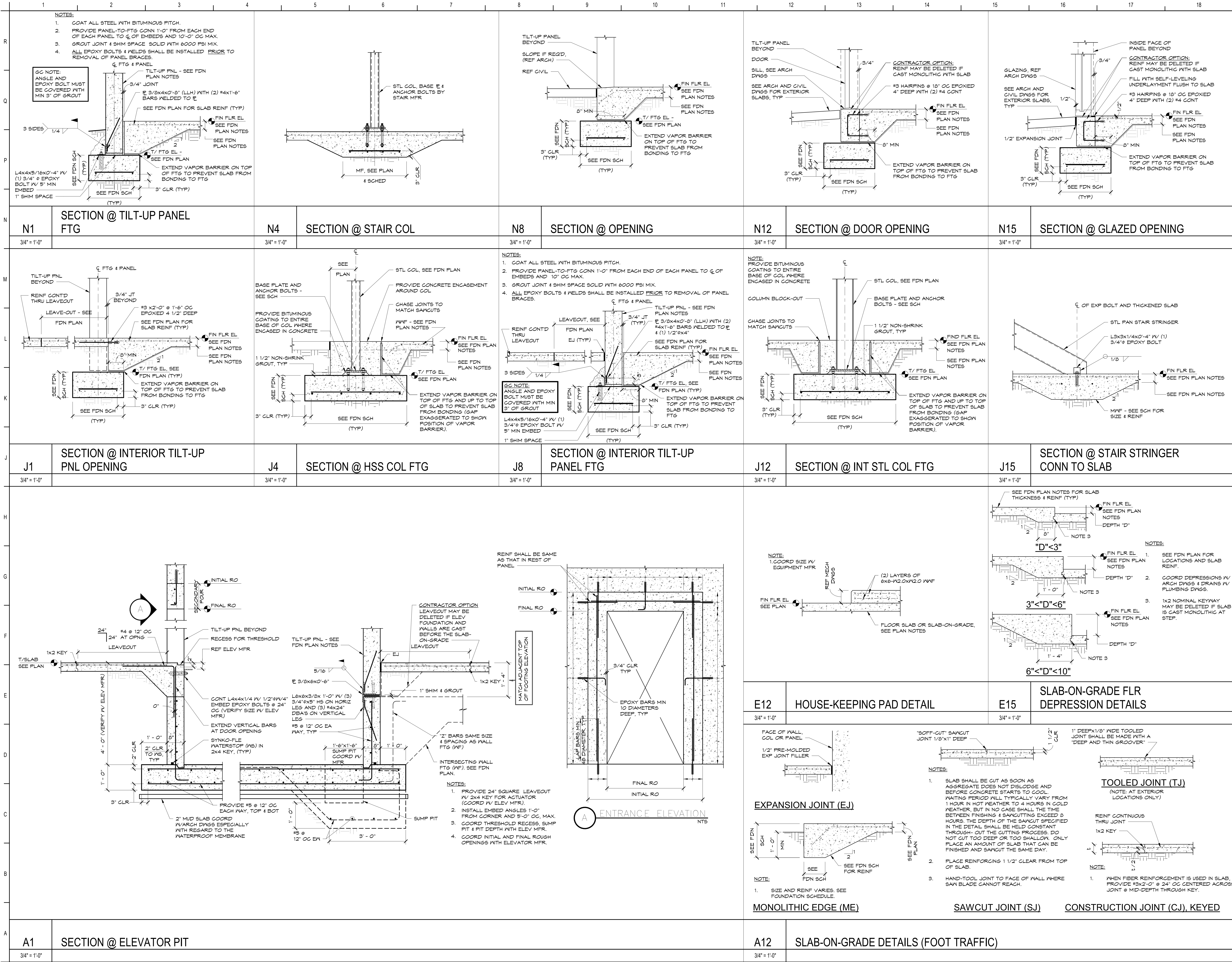
ROOF FRAMING PLAN KEY NOTES:

 FOR ROOF FRAMING PLAN KEY NOTES REF S103.



A1	PARTIAL ROOF FRAMING PLAN - AREA A
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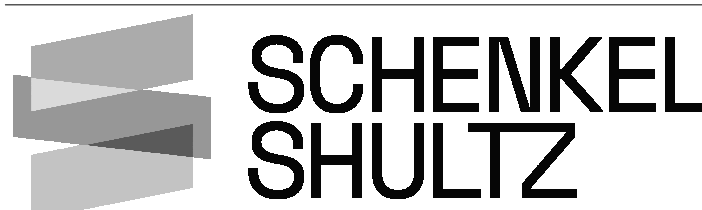
A16	UPPER ROOF FRAMING PLAN
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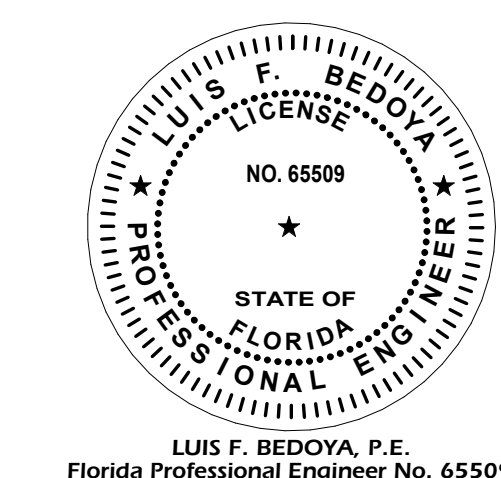


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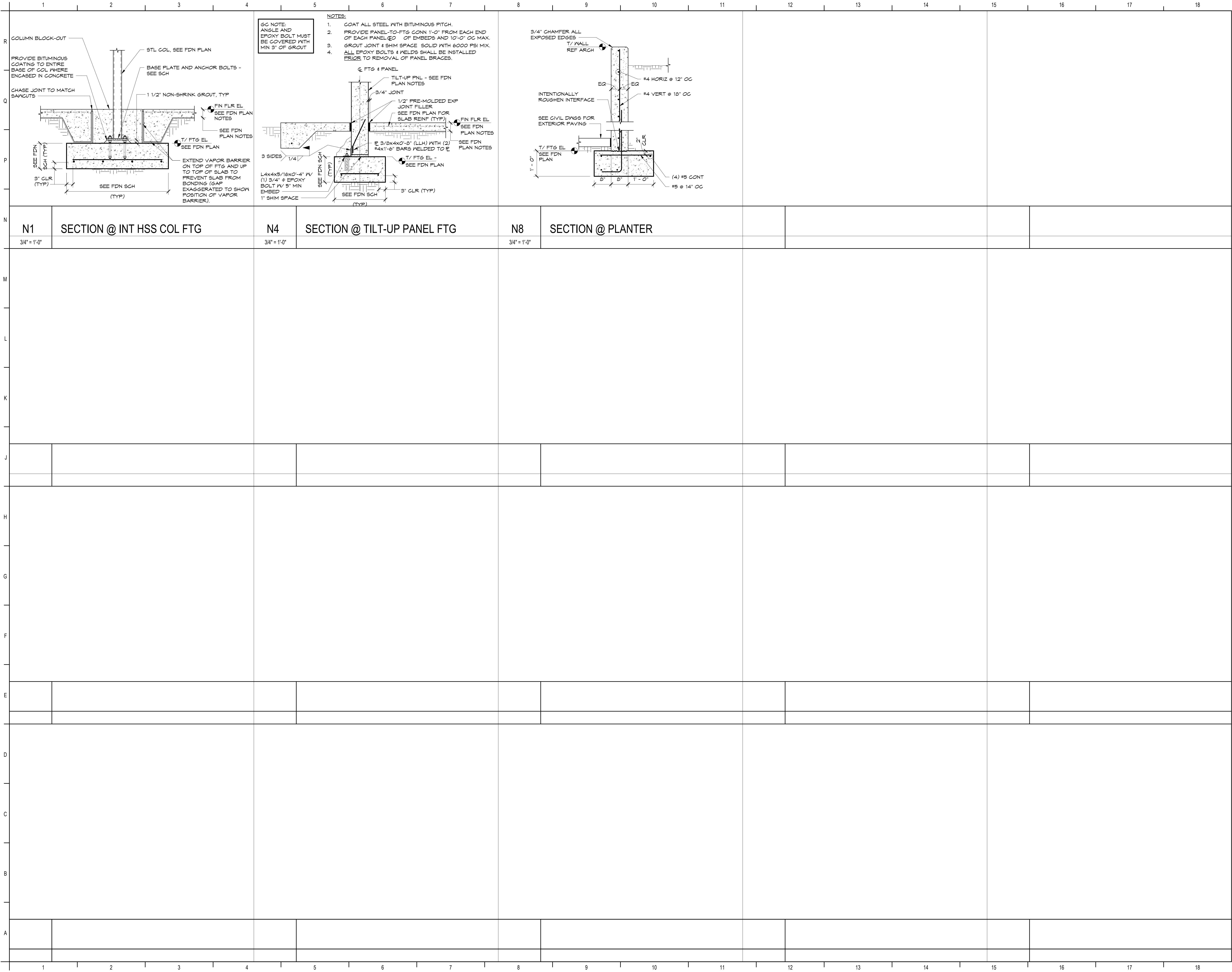
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FOUNDATION SECTIONS AND DETAILS

S201

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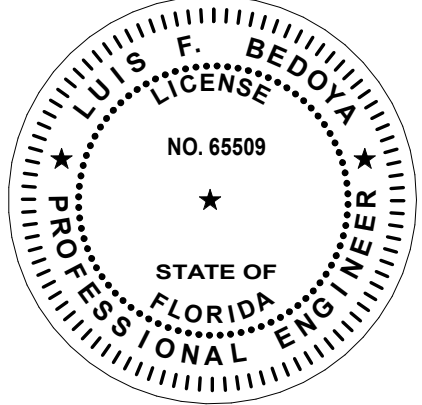
VERO CLASSICAL CHRISTIAN SCHOOL - PHASE I

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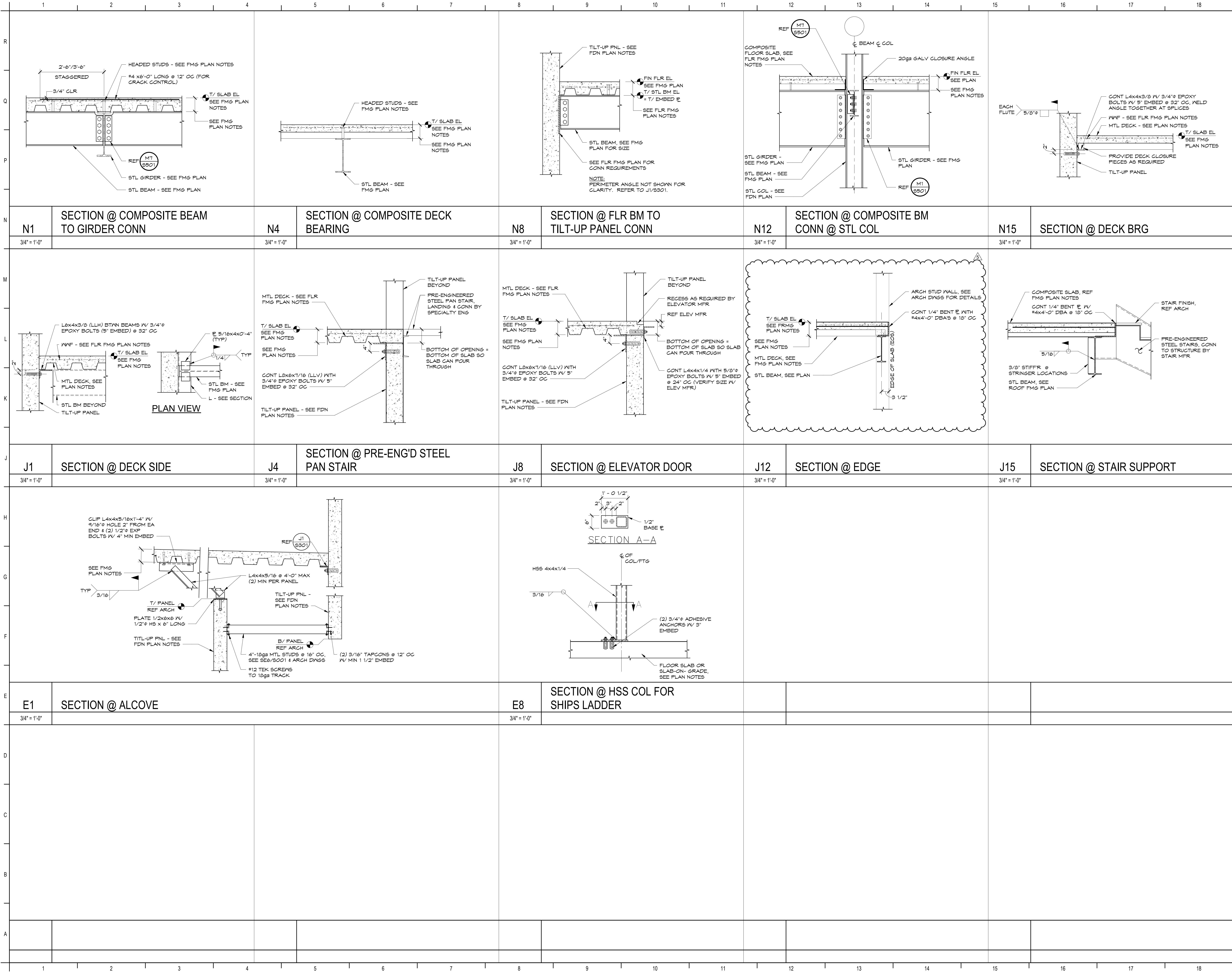


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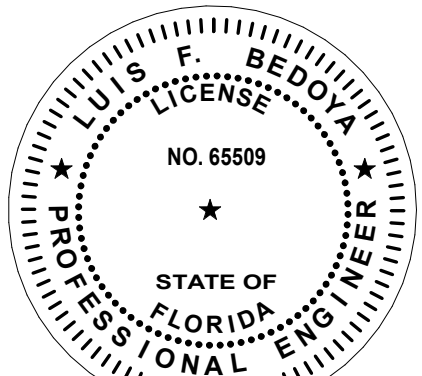
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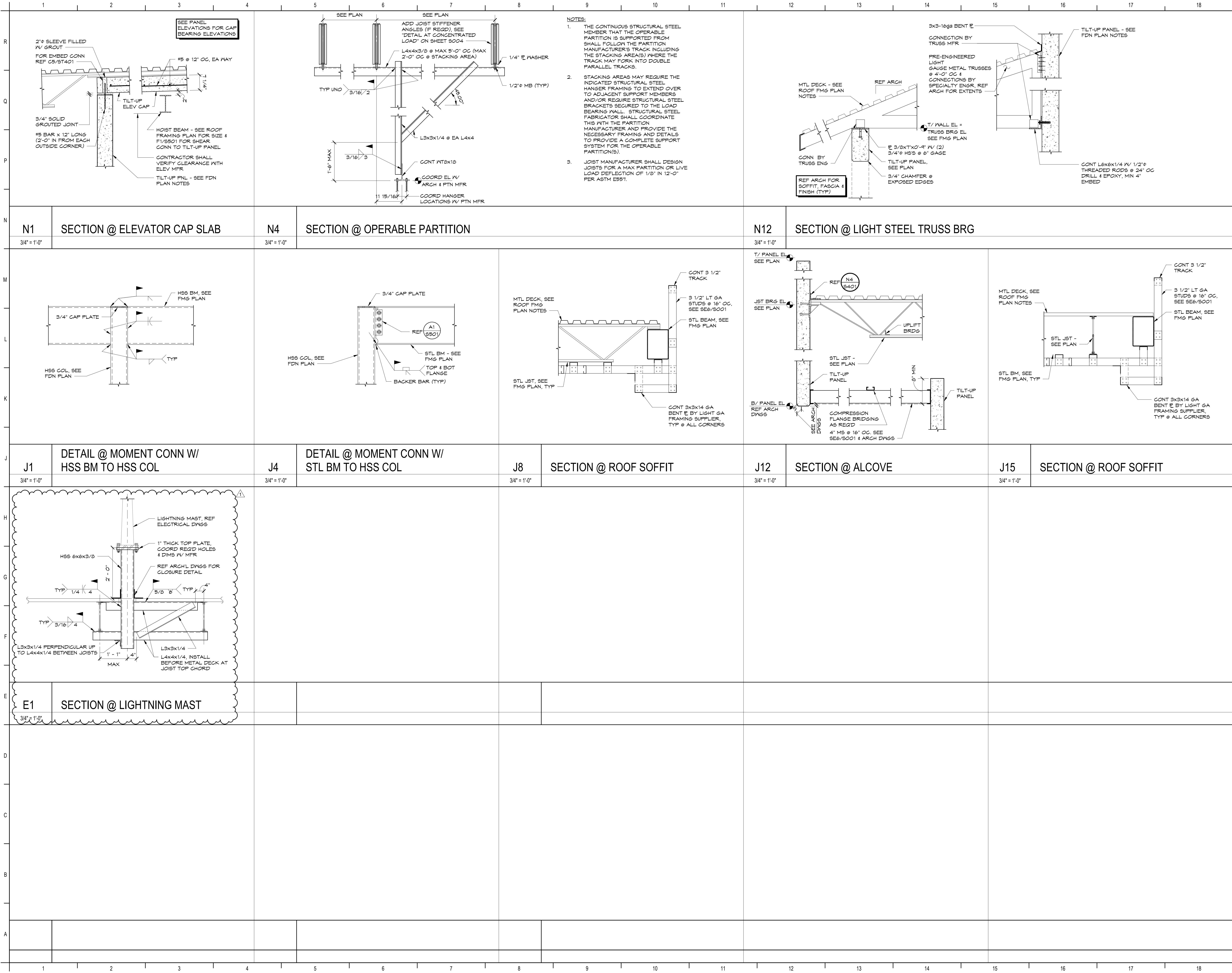
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5	Issued for Bidding	12/23/2024

COMM. NO.: 2024117
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FLOOR FRAMING SECTIONS AND DETAILS

S301

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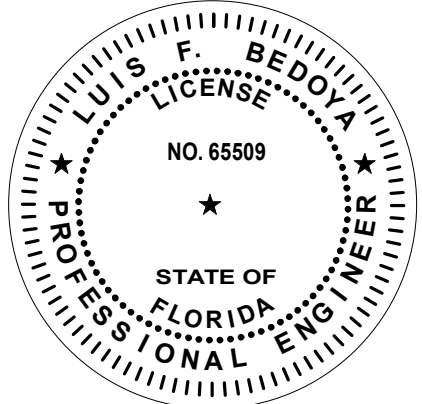
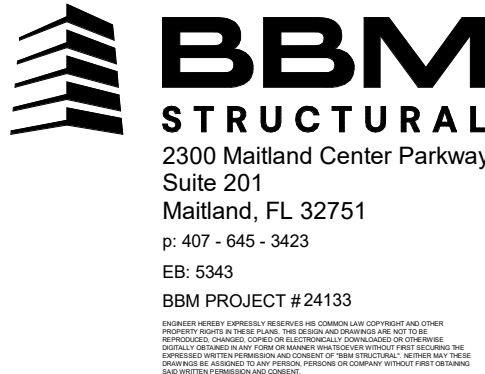


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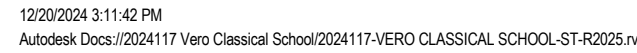
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ROOF FRAMING SECTIONS AND DETAILS

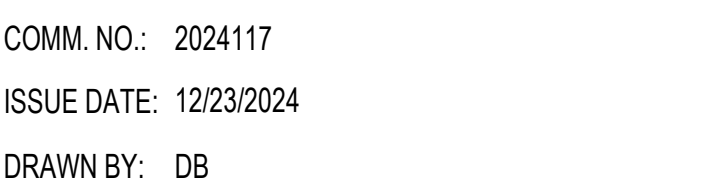
S402

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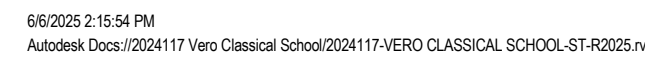




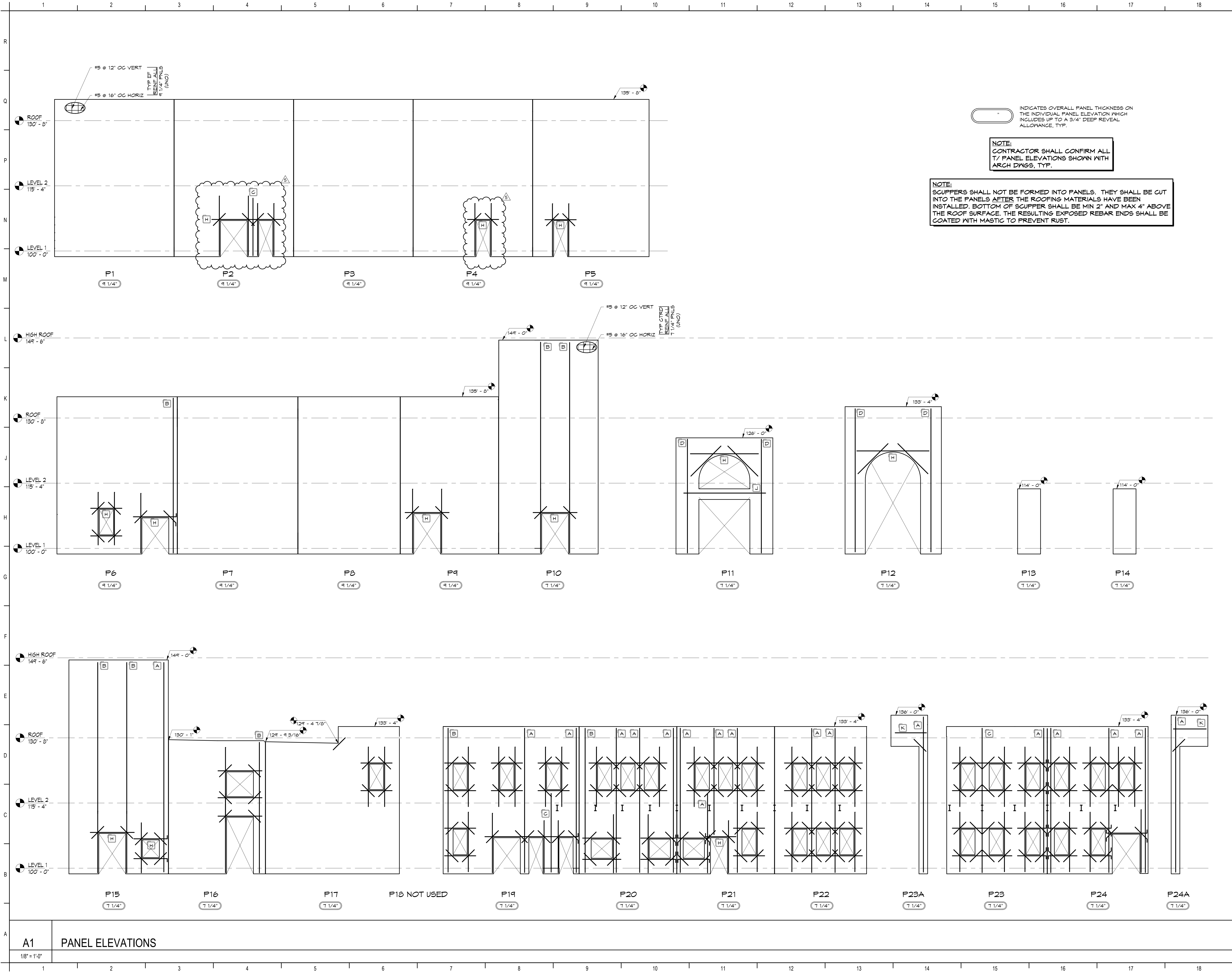
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TILT-UP PANEL NOTES, DETAILS, AND SCHEDULES



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VERO CLASSICAL
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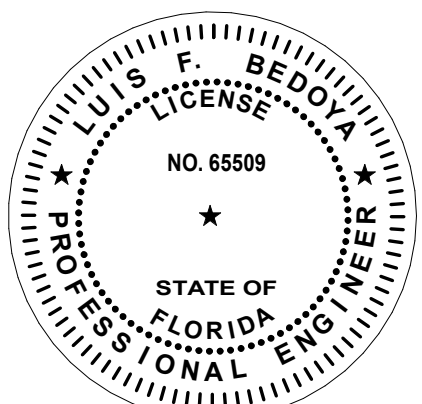
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5	Adopted 03 - REC BUILDING REVIEW COMMENTS	08/03/25

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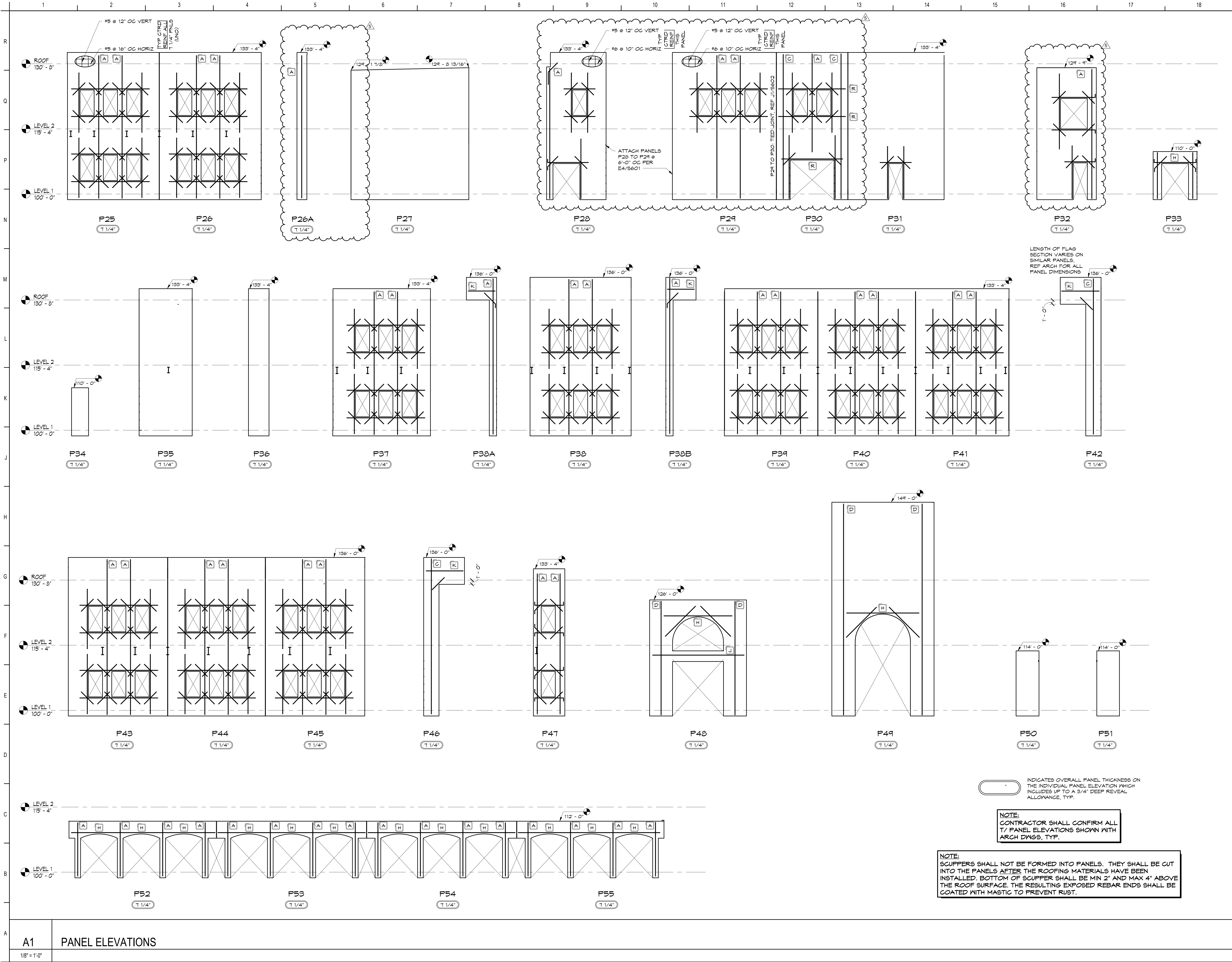
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TILT-UP PANEL ELEVATIONS

S603

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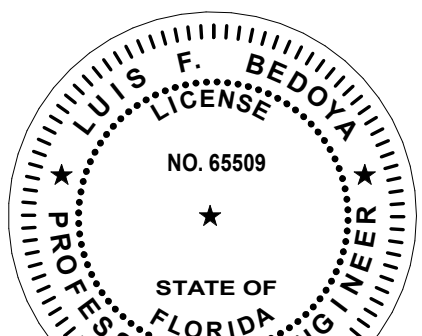
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5	Adopted 03 - R/C BUILDING REVIEW COMMENTS	08/2025

COMM. NO.: 2024117

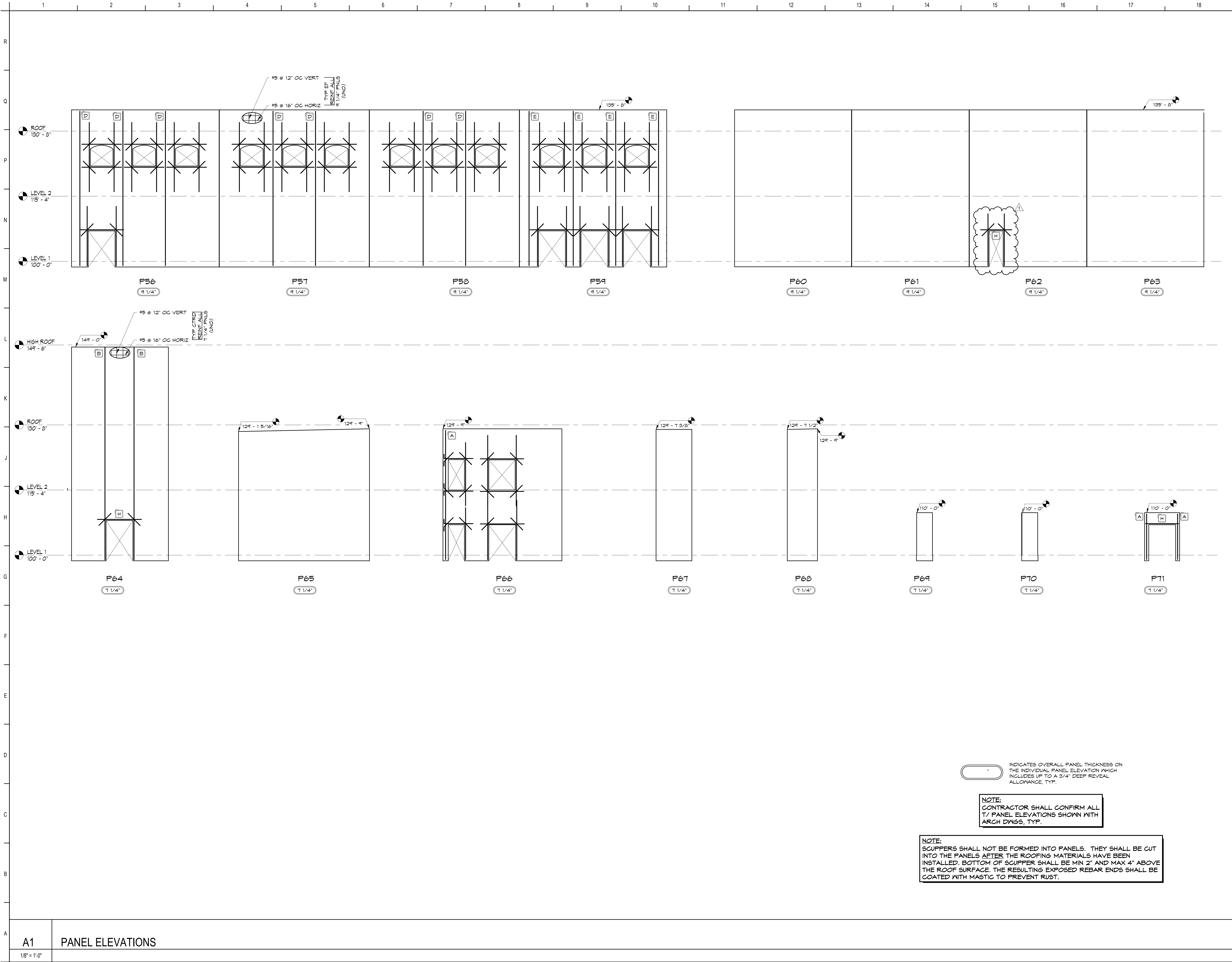
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TLT-UP PANEL ELEVATIONS

S604

BID/ PERMIT DOCUMENTS



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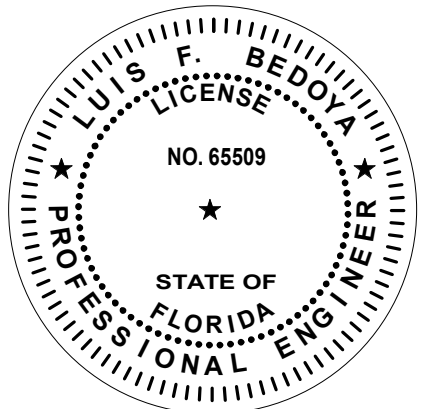
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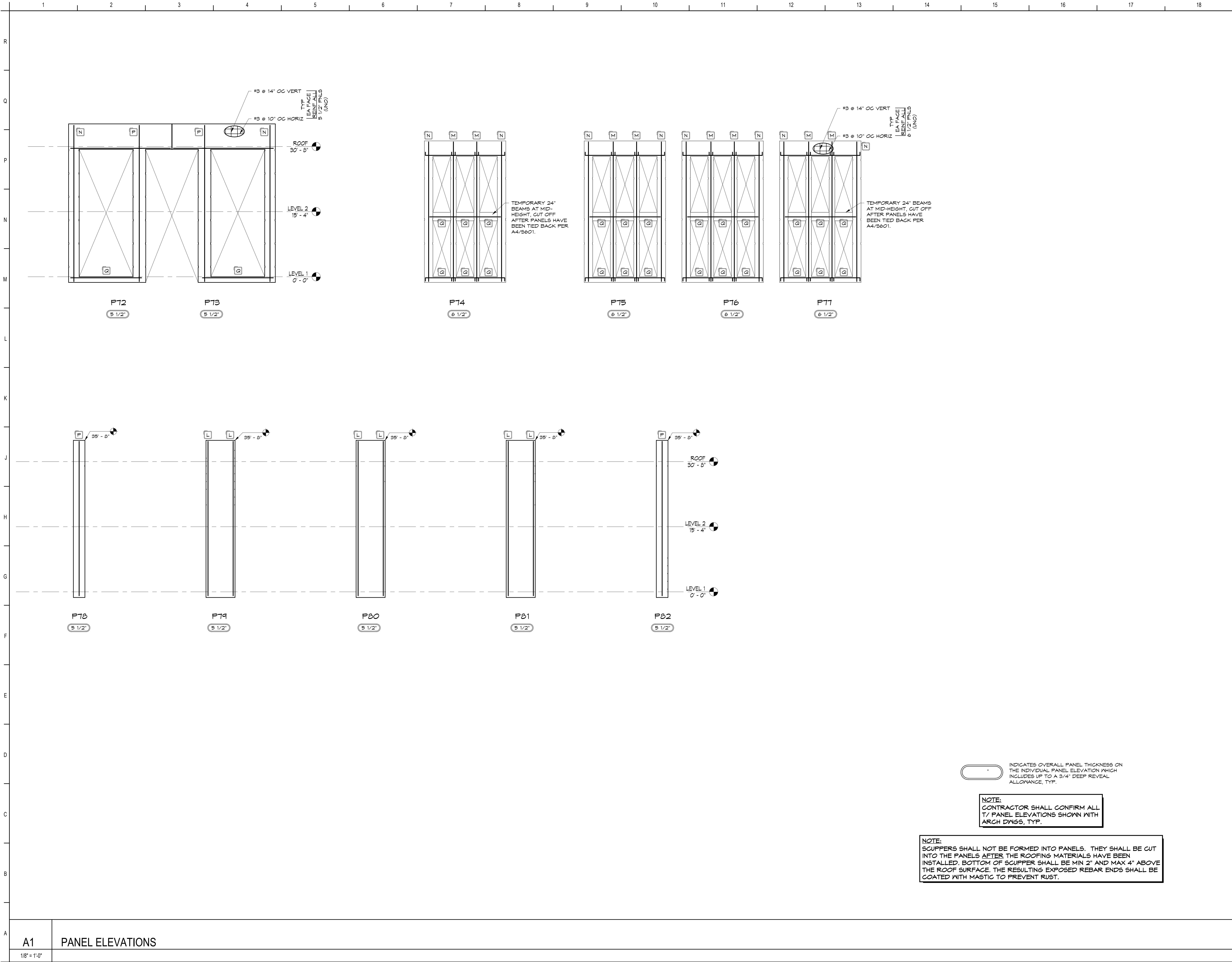
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TILT-UP PANEL ELEVATIONS

S605

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VERO CLASSICAL CHRISTIAN SCHOOL - PHASE I

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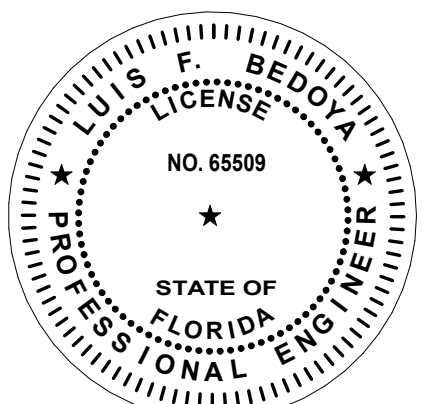
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TLT-UP PANEL ELEVATIONS

S606

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