- D1 ALL WORK SHALL CONFORM TO AT LEAST THE MINIMUM STANDARDS OF THE FLORIDA BUILDING CODE (FBC), EIGHTH EDITION (2023) AND THE LIFE SAFETY CODES. WIND DESIGN FOR COMPONENTS AND CLADDING SHALL BE BASED ON THE ASCE 7-22 CODE WITH A WIND RISK CATEGORY FOR EDUCATIONAL FACILITIES OF III. THE BUILDING SHELL HAS BEEN DESIGNED FOR ASCE 7-22 CODE WITH A WIND RISK CATEGORY FOR EDUCATIONAL FACILITIES OF III, THIS APPLIES TO ALL STRUCTURAL COMPONENTS INCLUDING ALUMINUM MALKMAY CANOPIES, FLAG POLES, SITE LIGHTING POLES, BLEACHERS AND THE
- D2 DESIGN LOAD VALUES ARE INDICATED ON THE APPROPRIATE SHEETS AS FOLLOWS: FOUNDATION DESIGN - SEE FOUNDATION NOTE F2 ON THIS SHEET

FLOOR FRAMING DESIGN - SHEET S102 ROOF FRAMING DESIGN - SHEET S103 WIND DESIGN - SHEET SOO4

15 MINUTE RAINFALL INTENSITY: 10.1 IN/HR 60 MINUTE RAINFALL INTENSITY: 4.89 IN/HR

- D3 TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE STRUCTURAL PLANS AND SPECIFICATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE BUILDING CODE.
- D4 EVERY REASONABLE EFFORT HAS BEEN MADE TO ENSURE COORDINATION BETWEEN THESE DRAWINGS AND THE BOUND STRUCTURAL SPECIFICATIONS. SHOULD THERE BE ANY DISCREPANCIES, THE CONTRACTOR SHALL THEN REQUEST A CLARIFICATION IN WRITING.

SPECIALTY ENGINEERING REQUIREMENTS

RAIN DESIGN:

- SE1 STEEL PAN STAIRS, SHIPS LADDERS AND OTHER ROOF ACCESS LADDERS SHALL BE DESIGNED BY THE FABRICATOR'S SPECIALTY ENGINEER AND SHALL INCLUDE STRINGERS, TREADS, HAND RAILINGS, PLATFORMS (AS REQUIRED), PAN INSERTS AND MISCELLANEOUS SUPPORTS AND CONNECTIONS. SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. MINIMUM DESIGN LIVE LOAD SHALL BE 100 PSF FOR STEEL PAN STAIRS. SUBMITTALS SHALL INCLUDE THE JOINTING IN THE CONCRETE FILL AS REQUIRED TO MITIGATE PLASTIC SHRINKAGE CRACKING.
- SE2 GUARDRAILS, HANDRAILS, POSTS AND SUPPORT CONNECTIONS SHALL BE DESIGNED BY THE FABRICATOR'S SPECIALTY ENGINEER. SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY A ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. DUE CONSIDERATION SHALL BE GIVEN TO EXPANSION & CONTRACTION BY PROVIDING SLIP JOINTS AS REQUIRED. DESIGN LOADING(S) SHALL CONFORM TO ALL REQUIREMENTS OF THE BUILDING CODE (SEE DESIGN CRITERIA FOR THE APPLICABLE BUILDING CODE).
- SE3 EXTERIOR CURTAIN WALLS SHALL BE DESIGNED BY THE VENDOR'S SPECIALTY ENGINEER AND SHALL INCLUDE FRAME, GLASS, GLAZING AND CONNECTIONS. SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. DESIGN LOADING(5) SHALL CONFORM TO ALL REQUIREMENTS OF THE BUILDING CODE (SEE DESIGN CRITERIA FOR APPLICABLE BUILDING CODE). THE VENDOR SHALL PROVIDE WINDOW WALL REACTIONS TO THE ARCHITECT.
- FLAGPOLES AND SITE LIGHTING POLES SHALL BE DESIGNED BY THE POLE VENDOR'S SPECIALTY ENGINEER AND SHALL INCLUDE POLES FOUNDATIONS AND CONNECTIONS. SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. MINIMUM DESIGN LOADS SHALL CONFORM TO NAAMM FP1001-17 "SPECIFICATIONS FOR DESIGN LOADS OF METAL
- ALUMINUM AMNINGS, WALKWAY CANOPIES AND THEIR FOUNDATIONS SHALL BE DESIGNED BY THE FABRICATOR'S SPECIALTY ENGINEER AND SHALL INCLUDE FRAME, COVERING AND CONNECTIONS. SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY A STRUCTURAL ENGINEER IN THE SAME STATE AS THE PROJECT LOCATION. DESIGN LOADING(S) SHALL CONFORM TO ALL REQUIREMENTS OF THE BUILDING CODE. (SEE DESIGN CRITERIA FOR THE APPLICABLE BUILDING CODE). ALUMINUM WALKWAY CANOPY MANUFACTURER SHALL PROVIDE CONCRETE SUB-CONTRACTOR BIDDERS WITH FOUNDATION AND REINFORCING REQUIREMENTS FOR THEIR CANOPY SYSTEM PRIOR TO BID.
- SE6 EXTERIOR LIGHT STEEL FRAMING, INCLUDING BUT NOT LIMITED TO: WALLS, EXTERIOR CEILINGS, FASCIAS AND SOFFITS SHALL BE DESIGNED BY A SPECIALTY ENGINEER. STRUCTURAL ELEMENTS HAVE BEEN PROVIDED FOR THE ATTACHMENT OF THE LIGHT STEEL FRAMING. THE LIGHT STEEL SYSTEM SUPPLIER SHALL DESIGN AND DETAIL ALL CONNECTIONS TO THESE ELEMENTS. ANY FURTHER ELEMENTS REQUIRED FOR THE SUPPORT SHALL BE DESIGNED AND SUPPLIED AS PART OF THE LIGHT STEEL SYSTEM. SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY AN ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. DESIGN LOADING(S) SHALL CONFORM TO ALL REQUIREMENTS OF THE BUILDING CODE (SEE DESIGN CRITERIA FOR THE APPLICABLE BUILDING CODE) AND MAXIMUM DEFLECTIONS SHALL BE AS FOLLOWS: L/600 (BRICK VENEER), L/360 (STUCCO) & L/240 (FLEXIBLE FINISHES). A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
- CONTRACTOR SHALL SUBMIT FOR ENGINEER'S APPROVAL A SHORING/RE-SHORING PLAN WITH SECTIONS AND CALCULATIONS, SIGNED, DATED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE SAME STATE AS PROJECT LOCATION.
- SE8 REFER TO STRUCTURAL STEEL NOTES FOR CONNECTION ENGINEERING
- PROVIDE ENGINEERED SUBMITTALS, SIGNED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION, FOR ALL MECHANICAL, ELECTRICAL AND PLUMBING SUPPORTS OR ATTACHMENTS NOT INCLUDED WITHIN THE CONSTRUCTION DOCUMENTS FOR ALL ITEMS THAT REQUIRE ANYTHING OTHER THAN THE MANUFACTURER'S STANDARD HARDWARE OR ARE EXPOSED TO WIND LOADS. AN EXAMPLE OF POSSIBLE ITEMS THAT THIS WOULD APPLY TO ARE, INCLUDING BUT NOT LIMITED TO, ROOFTOP AND/OR WALL MOUNTED DUCTS, PIPES AND TRANSFORMERS.

GENERAL

- THE GENERAL CONTRACTOR SHALL REVIEW AND DETERMINE THAT DIMENSIONS ARE COORDINATED BETWEEN ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO FABRICATION OR START OF CONSTRUCTION.
- THE GENERAL CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, THE WORK PERSONNEL AND OTHER PEOPLE DURING CONSTRUCTION. HE SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION.
- NO STRUCTURAL MEMBER SHALL BE CUT, NOTCHED OR OTHERWISE REDUCED IN STRENGTH.
- THE GENERAL CONTRACTOR SHALL COORDINATE ARCHITECTURAL MECHANICAL AND ELECTRICAL DRAWINGS FOR ANCHORED EMBEDDED AND SUPPORTED ITEMS WHICH AFFECT THE STRUCTURAL DRAWINGS AND NOTIFY THE ARCHITECT/ENGINEER OF ANY
- ALL SHOP DRAWING SUBMITTALS SHALL BE SUBMITTED VIA ELECTRONIC MEDIA (i.e. PDF OR DWF FORMAT). HARD COPY SUBMITTALS WILL NOT BE ACCEPTED.
 - 1. SUBMITTALS SHALL NOT BE SECURED IN ANY FORMAT THAT WILL PREVENT COMMENTS FROM BEING ADDED.
 - 2. SUBMITTALS THAT ARE REQUIRED TO BE SIGNED AND SEALED SHALL BE SUBMITTED WITH A VISIBLE INK SEAL OR SHADED RAISED SEAL AT TIME OF FIRST SUBMITTAL, AND SHALL BE DIGITALLY SIGNED THAT MEETS THE REQUIREMENTS OF THE STATE BOARD WHERE THE PROJECT IS LOCATED.
 - 3. SUBMITTALS THAT ARE MARKED **PRELIMINARY** OR ARE OTHERWISE INCOMPLETE WILL BE RETURNED REJECTED AND OR NOT REVIEWED. SUBMITTALS SHALL BE IN FINAL OR COMPLETED STATE PRIOR TO SUBMISSION TO BBM.
- G6 ANY SUBMITTALS RECEIVED BY ARCH/ENG THAT HAVE NOT BEEN CHECKED BY THE GC AND HIS SUB-CONTRACTOR SHALL BE RETURNED MITHOUT REVIEW.
- 67 ALL SECTIONS AND DETAILS SHALL BE CONSTRUED TO BE TYPICAL OR SIMILAR UNLESS ANOTHER SECTION OR DETAIL IS NOTED.
- 68 ANY CONFLICTS NOTICED, OR OBSERVED, BETWEEN THE WRITTEN SPECIFICATIONS AND THE CONSTRUCTION DOCUMENTS DURING PROJECT BIDDING OR PROJECT CONSTRUCTION SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE STRUCTURAL ENGINEER-OF-RECORD. IF SUCH DISCREPANCY IS NOT NOTICED OR BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER-OF-RECORD FOR WRITTEN CLARIFICATION, THE CONTRACTOR/SUB-CONTRACTOR SHALL PROVIDE, AT PROJECT BID OR DURING PROJECT CONSTRUCTION, THE MORE STRINGENT AND/OR MORE COSTLY OF THE TWO ITEMS IN THE BID AND/OR FINAL INSTALLATION.
- "BBM STRUCTURAL ENGINEERS" ASSUMES NO RISK OR LIABILITY FOR THE SITE SAFETY OR WELL-BEING OF ANY CONTRACTOR, SUB-CONTRACTOR NOR THEIR EMPLOYEES DURING THE CONSTRUCTION OF THE PROJECT CONTAINED IN THESE DOCUMENTS.
- G10 GENERAL CONTRACTOR/CONSTRUCTION MANAGER SHALL SUPPLY ALL SUB-CONTRACTORS WITH THE STRUCTURAL GENERAL NOTE SHEETS AS WELL AS THE STRUCTURAL DRAWINGS.
- THE STRUCTURAL STEEL AND OPEN WEB STEEL JOISTS SHALL BE FABRICATED AND ERECTED IN FULL CONFORMANCE WITH THE "OSHA STEEL ERECTION STANDARD". IF THE CONSTRUCTION DRAWINGS DEVIATE FROM THE OSHA STANDARD THEN THE FABRICATOR SHALL PROVIDE SUBMITTALS THAT CLEARLY INDICATE THE DEVIATION WITH A REVISION CLOUD AND REQUEST APPROVAL FROM "BBM" TO MAKE THE CHANGE SO THAT CONFORMANCE WITH THE OSHA STANDARD IS ASSURED.
- G12 THE CONTRACTOR'S MEANS AND METHODS SHALL FULLY CONFORM TO THE REQUIREMENTS OF SEI/ASCE 37 (DESIGN LOADS ON STRUCTURES DURING CONSTRUCTION) UNTIL ALL OF THE STRUCTURAL ELEMENTS ARE IN PLACE AND HAVE RECEIVED THE INSPECTOR'S APPROVAL
- G13 REFER TO ARCHITECTURAL DRAWINGS FOR ROOF COVERINGS. ROOF COVERINGS FOR ENHANCED HURRICANE PROTECTION AREA (EHPA) FACILITIES SHALL BE PROVIDED IN ACCORDANCE WITH THE LATEST ASTM AND FACTORY MUTUAL STANDARDS FOR MATERIALS AND WIND UPLIFT FORCES. ROOFS SHALL BE INSPECTED BY A LICENSED ENGINEER/ARCHITECT AND A REPRESENTATIVE OF THE ROOFING MANUFACTURER AND REPORTS SHALL BE SUBMITTED TO THE OWNER AND ARCHITECT

FOUNDATIONS

- F1 THE FOUNDATION AND SLAB-ON-GRADE DESIGN CONTAINED HEREIN IS BASED SOLELY UPON THE PROJECT'S GEOTECHNICAL REPORT (REPORT NUMBER 2401809-b2, DATED NOVEMBER 18, 2024, AS PREPARED BY KSM ENGINEERING AND TESTING). BBM HAS RELIED EXCLUSIVELY ON THE CONTENTS AND RECOMMENDATIONS WITHIN THIS REPORT, BUT ACCEPTS NO RESPONSIBILITY WHATSOEVER FOR ITS CONTENTS OR ACCURACY. THE GENERAL CONTRACTOR/CONSTRUCTION MANAGER SHALL MAKE THEMSELVES FAMILIAR WITH THE REPORT BEFORE THE COMMENCEMENT OF CONSTRUCTION.
- F2 FOUNDATION DESIGN IS BASED ON A NET ALLOWABLE SOIL BEARING PRESSURE OF 2500 PSF. COMPACTION UNDER ALL FOUNDATIONS SHALL ALSO BE AS STATED IN NOTE SOG1.
- F3 A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY-STANDARD SOIL DENSITY TESTS TO ENSURE CONFORMANCE WITH GEOTECHNICAL SOILS REPORT. SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
- F4 CONTRACTOR, IN CONJUNCTION WITH GEOTECHNICAL FIELD REPRESENTATIVE, SHALL DETERMINE IF ANY UNSUITABLE CONDITIONS ARE DISCOVERED DURING EXCAVATION WHICH MOULD PREVENT ATTAINMENT OF THE DESIGN SOIL PRESSURE RECOMMENDED BY THE SOILS REPORT.
- FOOTINGS SHALL BE CAST TO THE SCHEDULED SIZE AND SHALL NOT BE OVERSIZED BY MORE THAN 6" ON ANY SIDE FOR FOOTING WIDTH OF AT LEAST 6'-O". FOR FOOTINGS LESS THAN 6'-O" IN WIDTH THE MAXIMUM OVERSIZING SHALL BE 3".
- CONTRACTOR SHALL BE PREPARED FOR AND SHALL INCLUDE COST OF FORMING FOUNDATIONS SHOULD THE EARTH NOT PROVIDE ADEQUATE BANK STABILITY.

FLORIDA PRODUCT APPROVAL / NOTICE OF ACCEPTANCE

- FPA1 THE STRUCTURAL ROOF DECK(5) FOR THIS PROJECT HAVE BEEN DESIGNED TO BE COMPLIANT WITH THE CODES AND PRESSURES LISTED HEREIN. ANY PRODUCTS INSTALLED OVER THE STRUCTURAL DECK(S) SHALL HAVE BEEN TESTED AS AN ASSEMBLY. INCLUSIVE OF THE STRUCTURAL DECK, AND SHALL BE SUBMITTED FOR APPROVAL MITH THE FPA/NOA CLEARLY IDENTIFIED. FAILURE TO PROVIDE THIS INFORMATION SHALL BE CAUSE FOR REJECTION BY THE ARCHITECT AND BBM STRUCTURAL ENGINEERS, INC. ALL WARRANTIES MUST BE PART OF THE ROOFING SUBMITTAL. ANY WARRANTY CONTAINING LANGUAGE WHICH "EXCLUDES GALE AND/OR HURRICANE FORCE WIND DAMAGE", OR "EXCLUDES DAMAGE FROM GALE AND/OR HURRICANE FORCE WINDS", OR ANY OTHER SIMILAR VERBIAGE, SHALL BE REJECTED AND THE ENTIRE PRODUCT ASSEMBLY SHALL NOT BE
- PERMITTED FOR USAGE ON THE PROJECT. FPA2 RATIONAL ANALYSIS OF THE ROOF SYSTEM SHALL NOT BE PERMITTED WITHOUT THE WRITTEN CONSENT OF THE OWNER ARCHITECT AND ENGINEER, AND THEN, ONLY WHERE PERMITTED

BY THE LOCAL JURISDICTION HAVING DIRECT AUTHORITY.

TILT-UP PANELS

- TUP1 ALL PANELS ARE VIEWED FROM THE INSIDE.
- TUP2 PANEL THICKNESS SHALL BE AS INDICATED ON PLANS. SPECIAL ATTENTION MUST BE GIVEN TO THE LOCATION AND PLACEMENT OF THE
- TUP3 REFER TO THE ARCHITECTURAL DRAWINGS FOR FINISH REQUIREMENTS, CHAMFERS, ETC.
- TUP4 PANELS SHALL NOT BE LIFTED UNTIL CONCRETE HAS ATTAINED THE MINIMUM MODULUS OF RUPTURE AND COMPRESSIVE STRENGTH AS REQUIRED BY LIFTING ENGINEER.
- TUP5 THE CONTRACTOR SHALL HIRE SPECIALTY ENGINEER(S) TO PROVIDE DESIGN OF ALL ASPECTS OF THE LIFTING, BRACING AND TEMPORARY ERECTION STAGES FOR TILT-UP PANELS. THE ENGINEERING SHALL INCLUDE LIFTING INSERTS, BRACING DESIGN, BRACE CONNECTIONS VERIFICATION OF ADJOINING SURFACES WHICH BRACES ATTACH, TEMPORARY CONNECTIONS AT THE BASE OF PANEL WHILE PANEL IS TEMPORARILY BRACED & ANY ADDITIONAL REINFORCING STEEL REQUIRED FOR LIFTING OR BRACING OPERATION. PROVIDE SHOP DRAWINGS AND CALCULATIONS SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE SAME STATE AS PROJECT LOCATION. HOWEVER, NO ADDITIONAL REINFORCING SHALL BE ADDED WITHOUT THE EXPRESSED APPROVAL OF THE ENGINEER. THE DESIGNERS OF THE LIFTING INSERTS MUST CONSIDER THE REINFORCING ALREADY PRESENT IN THE PANELS AS INDICATED IN THIS SET OF CONSTRUCTION DRAWINGS. IN NO INSTANCE DOES ENGINEER OF RECORD IMPLY OR ACCEPT ANY RESPONSIBILITY FOR ANY MEANS, METHODS OR ADDITIONAL REQUIREMENTS AS RELATED TO TECHNIQUE, OR SAFETY OF THE PANEL LIFTING OPERATION. THE SLAB THICKNESS SHOWN HEREIN DOES NOT TAKE INTO CONSIDERATION SPECIFIC LOADS THAT MAY NEED TO BE RESISTED DURING THE BRACING OPERATION FOR THE TILT-UP PANELS. THE LIFTING / BRACING SPECIALTY ENGINEER FOR THIS PROJECT SHALL BE RESPONSIBLE FOR DESIGNING AND NOTIFYING ARCHITECT, ENGINEER AND GENERAL CONTRACTOR OF ANY AND ALL CONDITIONS RELATED TO THE LIFTING AND BRACING WHICH MAY REQUIRE ADDITIONAL SLAB THICKNESS. THIS INFORMATION IS TO BE CLEARLY TRANSMITTED TO ALL PARTIES IN A TIMELY MANNER TO ALLOW FOR ANY AND ALL ADDITIONS / MODIFICATIONS TO BE INSTALLED.
- THE CONTRACTOR SHALL PROVIDE PANEL SHOP DRAWINGS THAT ACCURATELY SHOW THE LOCATION OF ALL EMBEDDED ITEMS, (I.E. PLATES ANGLES, ETC.) THE LOCATION OF ALL OPENINGS AND THE CORRESPONDING DIMENSIONS. NO PANEL WORK SHALL BE PERFORMED WITHOUT APPROVED SHOP DRAWINGS.
- TUP7 MISCELLANEOUS OPENINGS MAY BE REQUIRED FOR FIRE LINES, PLUMBING, SANITARY LINES, ELECTRICAL CONDUITS, ETC. CORE DRILLING AFTER ERECTION OF THE PANELS MUST HAVE THE APPROVAL OF THE ARCHITECT AND ENGINEER PRIOR TO PERFORMANCE OF THE WORK.
- TUP8 THE REINFORCING STEEL SUPPLIER SHALL PROVIDE SHOP DRAWINGS INDICATING ALL THE NECESSARY INFORMATION REQUIRED TO ACCURATELY POSITION THE REBAR AS INDICATED, INCLUDING ANY ADDITIONAL REINFORCING AS REQUIRED BY SPECIALTY LIFTING/BRACING ENGINEERING AS NOTED IN TUP5 ABOVE. ENSURE CHAIRS, BOLSTERS OR OTHER MEANS OF SUPPORTING REBARS ARE PROVIDED AND ACCURATELY DETAILED. ALL REINFORCING BARS SHALL HAVE 48 BAR DIAMETER LAP.
- ALL REINFORGING STEEL AND EMBEDDED ITEMS SHALL BE ACCURATELY POSITIONED AND ADEQUATELY SECURED PRIOR TO PLACEMENT OF CONCRETE DO NOT WET-SET ITEMS AS THE CONCRETE IS PLACED.
- TUP10 THE TILT-UP CONTRACTOR/SUBCONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE TEMPORARY PANEL BRACING AND THE SAFETY OF THE WORKERS DURING BRACING INSTALLATION AND REMOVAL. BRACING REMOVAL SHALL ONLY OCCUR AFTER ALL STRUCTURAL MEMBERS ARE INSTALLED, FULL' SECURED AND INSPECTED.
- TUP11 TILT-UP PANELS SHALL BE CONNECTED TOGETHER AT CORNERS AND AT INTERSECTIONS PER DETAILS SHOWN ON SHEET S601.

SLAB ON GRADE

- SOG1 UNLESS NOTED OTHERWISE IN THE GEOTECHNICAL REPORT. COMPACT INTERIOR FILL TO 95% OF MODIFIED PROCTOR MAXIMUM DRY DENSITY (ASTM D1557). SOIL COMPACTION SHALL BE FIELD-CONTROLLED BY A REPRESENTATIVE TECHNICIAN OF A QUALIFIED LABORATORY. EACH LAYER OF FILL SHALL NOT EXCEED 12" THICK AND SHALL BE COMPACTED PRIOR TO PLACEMENT OF NEXT LAYER.
- 50G2 MAXIMUM SPACING OF CONTROL JOINTS (i.e. SAWCUT JOINT OR CONSTRUCTION JOINT) SHALL BE AS SET IN THE TABLE BELOW, OR AS NOTED ON PLANS. THE MORE STRINGENT SHALL APPLY. PATTERNS SHALL BE APPROXIMATELY SQUARE WITH A RATIO OF LONG SIDE TO SHORT SIDE NOT EXCEEDING 1.5 TO 1. SEE SLAB-ON-GRADE DETAILS FOR ADDITIONAL INFORMATION.

SLAB THICKNESS (IN)	★ 3/4" OR LARGER AGGREGATE SPACING (FT)
4	12
5	13
6	14
7 AND GREATER	15

AGGREGATE LESS THAN 3/4" ARE NOT ACCEPTABLE.

* MIX DESIGNS CONTAINING

- SOG3 GENERAL CONTRACTOR SHALL COORDINATE EXACT LOCATION OF SJ'S AND CJ'S WITH ARCHITECTURAL FLOOR FINISHES TO ENSURE SLAB JOINTS DO NOT READ THROUGH.
- 5064 SLAB THICKNESS SHALL BE INCREASED AS REQUIRED TO PROVIDE ADEQUATE SUPPORT FOR CRANE LOADS WITHOUT CRACKING SLAB.

POST-INSTALLED ANCHORS

- POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER-OF-RECORD (SEOR) PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REBAR. HOLES SHALL BE DRILLED AND CLEANED AND PRODUCT SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII). SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE SPECIFIED BELOW, SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER-OF-RECORD ALONG WITH CALCULATIONS THAT ARE PREPARED & SEALED BY A REGISTERED PROFESSIONAL ENGINEER IN THE SAME STATE AS PROJECT LOCATION. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING EQUIVALENT OR BETTER PERFORMANCE VALUES OF THE SPECIFIED PRODUCT USING THE APPROPRIATE
- DESIGN PROCEDURE AND/OR STANDARD(S) AS REQUIRED BY THE BUILDING CODE. PIA2 THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS. ADDITIONALLY, INSTALLATION OF ADHESIVE ANCHORS IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION SHALL BE DONE BY CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS CERTIFIED THROUGH ACI AND IN ACCORDANCE WITH ACI 318-19 (SECTION 17.2.3). PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED PRIOR TO COMMENCEMENT OF INSTALLATION.

<u>POST-INSTALLED ANCHORS (CONT)</u>

CONCRETE ANCHORS:

- A. MECHANICAL ANCHORS FOR USE IN CRACKED AND UNCRACKED CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193. PRE-APPROVED MECHANICAL ANCHORS
- 1. DEWALT "POWER-STUD+ SD1" (ICC-ES ESR-2818) 2. DEWALT "POWER-STUD SD2, SD4 (30455) AND SD6 (31655)"
- (ICC-ES ESR-2502)
- 3. DEMALT "SCREW-BOLT+" (ICC-ES ESR-3889) 4. DEMALT "SNAKE+" (ICC-ES ESR-2272)
- 5. DEMALT "MINI-UNDERCUT+" (ICC-ES ESR-3912) 6. HILTI KWIK BOLT-TZ2 EXPANSION ANCHOR SAFE SET SYSTEM WITH AT
- MODULE FOR APPLICABLE SIZES (ICC-ES ESR-4266) 7. HILTI KWIK BOLT 1 EXPANSION ANCHOR SAFE SET SYSTEM WITH HOLLOW DRILL BIT AND VACUUM AND AT MODULE FOR APPLICABLE
- SIZES PER IAPMO UES ER-678 8. HILTI KWIK HUS-EZ (KH-EZ), KH-EZ CRC, KH-EZ SS316, KH-EZ C
- KH-EZ E, KH-EZ I, AND KH-EZ P SCREW ANCHOR SAFE SET SYSTEM WITH HOLLOW DRILL BIT AND VACUUM PER ICC-ES ESR-3027
- 9. SIMPSON STRONG-TIE "STRONG-BOLT 2" (ICC-ES ESR-3037) 10. SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR-2713)
- 11. SIMPSON STRONG-TIE "STAINLESS STEEL TITEN HD" (IAPMO UES ER-493) (316 OR 304 STAINLESS STEEL)
- B. ADHESIVE ANCHORS FOR USE IN CRACKED AND UNCRACKED CONCRETE SHALL BE INSPECTED AS FOLLOWS: AT THE ONSET OF EACH APPLICATION, A MANUFACTURER'S REPRESENTATIVE MUST BE PRESENT TO WITNESS AT LEAST FIVE COMPLETE INSTALLATIONS. INSTALLERS MUST BE TRAINED BY THE MANUFACTURER AND MANUFACTURER SHALL SUBMIT DOCUMENTATION TO THE CONTRACTOR INDICATING TRAINING HAS TAKEN PLACE. TRAINED INSTALLERS SHALL PROVIDE WRITTEN DOCUMENTATION TO THE CONTRACTOR THAT ALL ANCHORS HAVE BEEN INSTALLED PER THE MPII. CONTRACTOR SHALL SUBMIT THIS DOCUMENTATION TO THE SEOR. ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE HAVING MINIMUM AGE OF 21 DAYS. HOLES SHALL BE DRY AT THE TIME OF INSTALLATION. ADHESIVES SHALL HAVE MAX IN-SERVICE SHORT-TERM TEMPERATURE OF 150°F, AND MAX IN-SERVICE LONG-TERM TEMPERATURE OF 110°F. PRIOR TO INSTALLATION OF ADHESIVE ANCHORS IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATIONS RESISTING SUSTAINED TENSION LOADS INSTALLERS ARE REQUIRED TO BE CERTIFIED IN ACCORDANCE WITH THE ACI. CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM AND MUST BE CONTINUOUSLY INSPECTED. ANCHORS SHALL ALSO HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC308. PRE-APPROVED ADHESIVE ANCHORS INCLUDE THE FOLLOWING PRODUCTS:

3. HILTI

- a. "PURE 220+" WITH DUSTX+ HOLLOW DRILL BIT SYSTEM OR USING TRADITIONAL BLOW OUT METHOD PER ICC-ES ESR-5144
- b. AC200+ WITH DUSTX+ HOLLOW DRILL BIT SYSTEM OR USING TRADITIONAL BLOW OUT METHOD PER ICC-ES ESR-4027. 2. DEWALT AC200+ GOLD (ICC-ES ESR-4027)
- a. HILTI HIT-HY 200 V3 SAFE SET SYSTEM WITH THE HILTI HIT-Z ROD PER ICC ESR-4868. b. HILTI HIT-HY 200 V3 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT
- (TE-CD OR TE-YD) AND VACUUM WITH HAS-E THREADED ROD PER ICC ESR-4868. C. HILTI HY 200 V3 ADHESIVE ANCHOR USING TRADITIONAL CLEANING
- METHOD, 2 BLOWS OF COMPRESSED AIR (90 PSI MINIMUM, OIL FREE COMPRESSOR), 2 BRUSHES WITH SAME SIZE AS HOLE WIRE BRUSH AND 2 MORE BLOWS, (2x2x2 CLEANING METHOD) PER ICC ESR-4868.

d. HILTI HIT-RE 500-V3 EPOXY ADHESIVE ANCHORING SYSTEM WITH

- HAS-E THREADED ROD PER ICC ESR-3814 FOR SLOW CURE APPLICATIONS. e. HILTI KMIK-X DUAL ACTION ANCHOR SAFESET SYSTEM WITH KHC
- CAPSULE ADHESIVE AND KWIK-HUS EZ PER ICC ESR-5065. 4. SIMPSON STRONG-TIE "SET-3G" (ICC-ES ESR-4057)
- 5. SIMPSON STRONG-TIE "SET-XP" (ICC-ES ESR-2508) 6. SIMPSON STRONG-TIE "AT-XP" (IAPMO UES ER-263)
- 7. SIMPSON STRONG-TIE "ET-HP" (ICC-ES ESR-3372)
- POMDER-ACTUATED FASTENERS (PAF) SHALL BE BY SIMPSON STRONG-TIE (ICC-ES ESR-2138), HILTI (ICC-ES ESR-1663 AND ESR-2269), DEWALT "POWER DRIVEN FASTENERS" (ICC-ES ESR-2024) OR ENGINEER-APPROVED EQUAL.
- CORDLESS DIRECT FASTENERS (CDF) SHALL BE BY DEWALT (ICC-ES ESR-4076) OR ENGINEER-APPROVED EQUAL.

PRE-FABRICATED LIGHT STEEL TRUSSES

- A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER
- SHOP DRAWING SUBMITTALS, INCLUDING, BUT NOT LIMITED TO, PLANS, DETAILS AND CALCULATIONS SHALL BE SUBMITTED TO ARCHITECT FOR REVIEW PRIOR TO FABRICATION. CALCULATIONS SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE SAME STATE AS PROJECT LOCATION. "BBM STRUCTURAL ENGINEERS" WILL REQUIRE THAT THE ENGINEERED DRAWINGS FOR THE STEEL TRUSSES BE REVIEWED FOR COMPATIBILITY WITH THE DESIGN INTENT OF THE STRUCTURE PRIOR TO FABRICATION. ANY AND ALL COSTS ASSOCIATED WITH FABRICATING TRUSSES FROM SUBMITTALS NOT BEARING OUR SHOP DRAWING STAMP AND APPROVAL WILL BE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR.
- SEE ROOF FRAMING PLANS AND ARCHITECTURAL DRAWINGS FOR EXTENT AND SLOPES OF STEEL TRUSSES.
- ALL TRUSSES SHALL BE DESIGNED BY A SPECIALTY ENGINEER WITH A MINIMUM OF (5) YEARS EXPERIENCE IN SIMILAR TYPE STRUCTURES.
- ALL TRUSSES SHALL BE TEMPORARILY AND PERMANENTLY BRACED AS REQUIRED BY TRUSS MFR. BRACING AND BRIDGING BY TRUSS MFR.
- MINIMUM THICKNESS FOR TOP CHORD OF ALL TRUSSES SHALL BE 18 GA.
- THE ROOF FRAMING PLAN SHOWN HEREIN IS A GRAPHICAL REPRESENTATION ONLY. REFER TO TRUSS SUBMITTALS FOR ACTUAL LAYOUT, TRUSS PROFILES AND CONNECTION REQUIREMENTS AT ALL TRUSS SUPPORTS.
- ALL TRUSS-TO-TRUSS & TRUSS-TO-STRUCTURE CONNECTIONS SHALL BE DESIGNED AND DETAILED BY THE TRUSS MANUFACTURER. CONNECTIONS SHALL BE DESIGNED FOR DEAD, LIVE AND WIND (BOTH UPLIFT AND LATERAL) LOADS. THE WIND PRESSURE AND SUCTION EXERTED ON THE WALLS MAY BE FOUND IN THE WIND SCHEDULES.
- TRUSSES SHALL BE SHOP FABRICATED, INCLUDING ANY FIELD SPLICE CONNECTION COMPONENTS AND SHIPPED TO SITE IN MAXIMUM LENGTHS AND HEIGHTS. FIELD FABRICATION OF TRUSSES WILL NOT BE PERMITTED.
- ALL TRUSS MEMBERS SHALL BE GALVANIZED METAL STUD STOCK AND GALVANIZED SHEET OF SIZES AND GAGES TO BE DETERMINED BY THE MANUFACTURER'S STRUCTURAL ANALYSIS. GALVANIZED CUSTOM SHAPE AND CONNECTOR SYSTEMS SPECIFICALLY DESIGNED FOR USE IN TRUSSES WILL BE ACCEPTABLE.
- REVIEW ARCHITECTURAL FLOOR PLANS FOR OPERABLE WALLS THAT ARE SUSPENDED FROM THE TRUSSES. ANALYSIS FOR CLOSED, PARTIALLY OPEN AND OPEN POSITION LOAD CURVES IS REQUIRED. REFER TO WALL MANUFACTURER FOR SPECIAL DEFLECTION CRITERIA.
- REVIEW ARCHITECTURAL REFLECTED CEILING PLANS AND SECTIONS FOR SPECIAL CEILING CONDITIONS, INCLUDING CEILING SLOPES, TROFFERS, COFFERS, TRAYS, STEPS AND OTHER SPECIAL FEATURES.
- THE SUPPORTS FOR SCISSOR, ARCHED CHORDS OR ANY OTHER SIMILAR TYPE TRUSS, UNLESS NOTED OTHERWISE, SHALL BE DESIGNED WITH A PINNED CONNECTION AT ONE END AND A HORIZONTAL ROLLER AT THE OTHER END THE SUPPORTING STRUCTURE, UNLESS NOTED OTHERWISE, HAS NOT BEEN DESIGNED FOR ANY GRAVITY LOAD HORIZONTAL FORCE DUE TO DEFLECTION DO NOT DESIGN THESE TYPE OF TRUSSES WITH PINNED SUPPORTS EACH END.



SCHOOL

Vero Classical School 51 Old Dixie Hwy, Vero Beach, FL 32962

VERO CLASSICAL CHRISTIAN SCHOOL PHASE I

PROJECT LOCATION: 13TH ST. SW AND 58TH AVE. SW VERO BEACH, FLORIDA



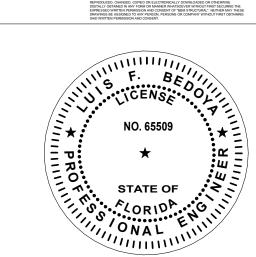
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EB: 5343

BBM PROJECT #24133

LUIS F. BEDOYA, P.E.

REVISIONS DESCRIPTION

COMM. NO.: 2024117 ISSUE DATE: 12/23/2024 DRAWN BY: DB

STRUCTURAL GENERAL NOTES

CONCRETE AND REINFORCING

- A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD TESTING INCLUDING SLUMP TESTS AND CYLINDER BREAKS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
- CONCRETE WORK SHALL CONFORM TO ACI 318-19 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE.
- ALL CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES:

LOCATION	28 DAY STRENGTH	SLUMP	COARSE AGGREGATE(S)	COMMENTS	EXPOSURE CATEGORIES & CLASSES (PER ACI 318-19 SECTION 19.3.1)			ER
					F	5	M	C
FOUNDATIONS	3000 PSI	4" +/- 1"	1"		FO	50	MO	C 1
SLAB-ON-GRADE: FOOT TRAFFIC	3000 PSI	4" +/- 1"	1"		F0	50	MO	co
WALLS	4000 PSI	4" +/- 1"	3/8" \$ 1"		F0	50	MO	co
TILT-UP PANELS	4000 PSI	4" +/- 1"	3/8" \$ 1"		F0	50	MO	C1
GROUT UNDER TILT-UP PANELS	6000 PSI	8" TO 11"	NONE		F0	50	MO	co
ELEVATED SLABS (MTL DECK)	3000 PSI	4" +/- 1"	3/8" \$ 1"		FO	50	MO	co
TOPPINGS & METAL PAN STAIRS	4000 PSI	7" +/- 1"	3/8"	ADD 1 1/2 gal/CY OF TETRAGUARD OR ECLIPSE SHRINKAGE CONTROL ADDITIVE	FO	50	MO	co

NOTES:

- SLUMP FOR RAMPS AND SLOPING SURFACES SHALL NOT EXCEED 4".
- 2. ALL CONC MIXES SHALL HAVE A MAXIMUM SAND TO TOTAL AGGREGATE RATIO OF 0.50.
- 3. A 2" OR 3" PUMP SHALL BE ACCEPTABLE FOR COLUMNS AND TIE BEAMS BUT WILL NOT BE ALLOWED FOR FOUNDATIONS, SLABS, TILT-UP PANELS AND CONCRETE BEAMS.
- F. READY MIX SUPPLIER SHALL DESIGN THE MIXES THAT CONTAIN MULTIPLE
- AGGREGATES TO BE WELL GRADATED.
- 5. SLABS SHALL NOT BE AIR ENTRAINED.
- 6. FOR SLABS THAT SHALL RECEIVE MOISTURE SENSITIVE FLOORING:
- a. CONTRACTOR SHALL WORK WITH THE READY MIX SUPPLIER TO PROVIDE A MIX DESIGN THAT WILL BE AT OR BELOW 75% RELATIVE HUMIDITY AT THE TIME THE FLOORING IS SCHEDULED TO BE INSTALLED.
- b. DO NOT USE LIGHTWEIGHT AGGREGATES.
- C. PROVIDE A MIX WITH GOOD SELF-DESICCATING PROPERTIES. CONSIDER ADDING 2%-4% SILICA
- d. DO NOT HARD TROWEL THE SURFACE BUT INSTEAD PROVIDE A LIGHTLY TROWELLED SURFACE.

CONCRETE MIX DESIGN SUBMITTALS:

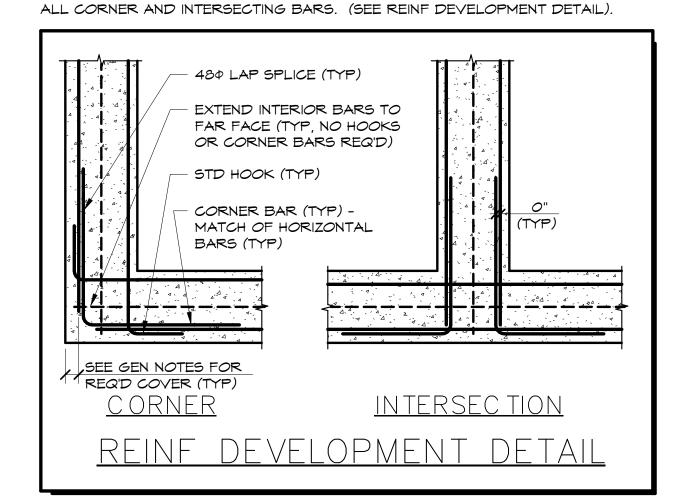
- EACH MIX DESIGN SHALL BE LABELED TO INDICATE THE AREA IN WHICH THE CONCRETE IS TO BE PLACED (I.E. FOUNDATIONS, SLAB- ON-GRADE, COLUMNS, ETC.). FAILURE TO DO SO WILL CAUSE DELAY AND/OR REJECTION OF SUBMITTALS.
- 2. PROPOSED MIX DESIGN SHALL BE IN ACCORDANCE WITH METHOD 1 OR METHOD 2 OF ACI 301. PROVIDE SUPPORTING DATA IN TABULAR FORM FOR EACH SEPARATE PROPOSED MIX.
- 3. SUBMIT CONCRETE MIX DESIGN FOR EACH PROPOSED CLASS OF CONCRETE.
- REBAR SHALL CONFORM TO ASTM A615 GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064 AND SHALL BE LAPPED MINIMUM ONE MESH + 2" WHERE SPLICED. ALL REINFORCING SHALL BE DOMESTICALLY PRODUCED. ALL REBAR THAT IS TO BE WELDED SHALL BE LOW ALLOY ASTM A706 GRADE 60.
- SPLICES AND ANCHORAGE OF REINFORCING SHALL BE AS FOLLOWS (UNLESS OTHERWISE NOTED):

MELDED WIRE FABRIC: 8"

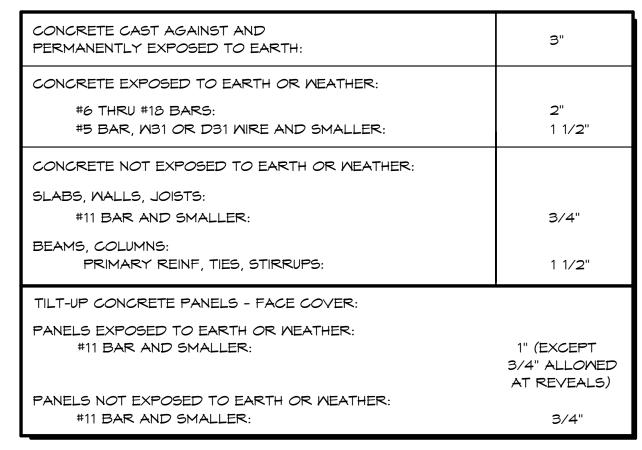
ALL OTHER:

#6 BAR & SMALLER: 48 DIA (12" MIN) #7 BAR & LARGER: 60 DIA

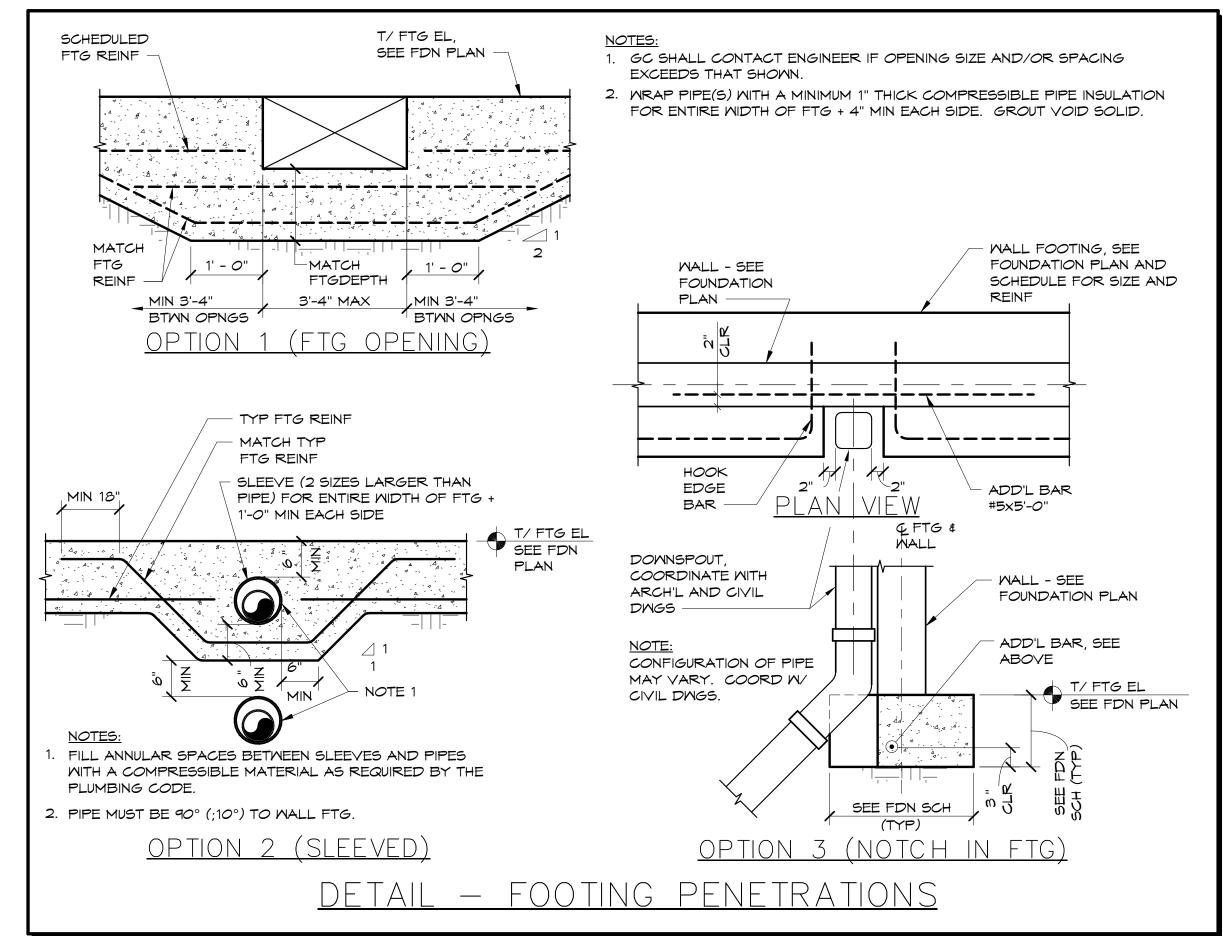
REINFORCEMENT IN WALLS, FOOTINGS AND BEAMS SHALL BE CONTINUOUS AND LAPPED 48 BAR DIA AT SPLICE UNLESS OTHERWISE NOTED. HOOK AND LAP



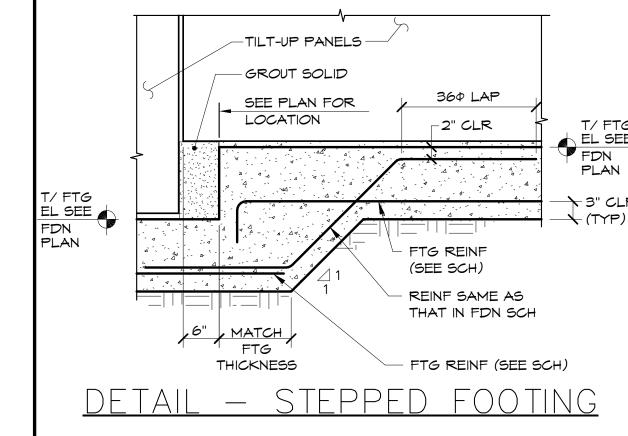
COVER FOR REINFORCING SHALL BE AS FOLLOWS:



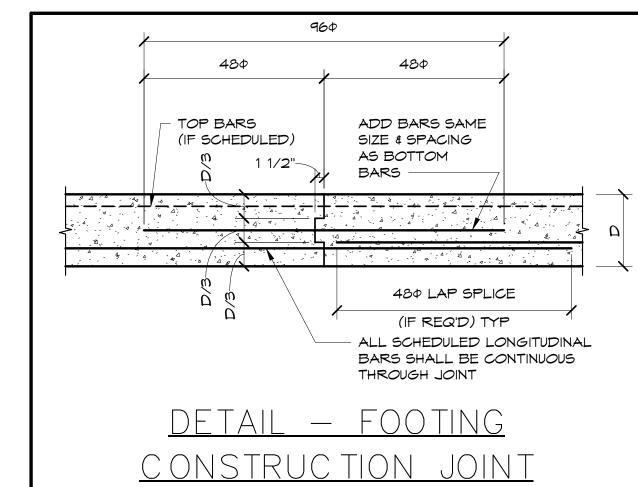
FOOTING PENETRATION DETAILS:

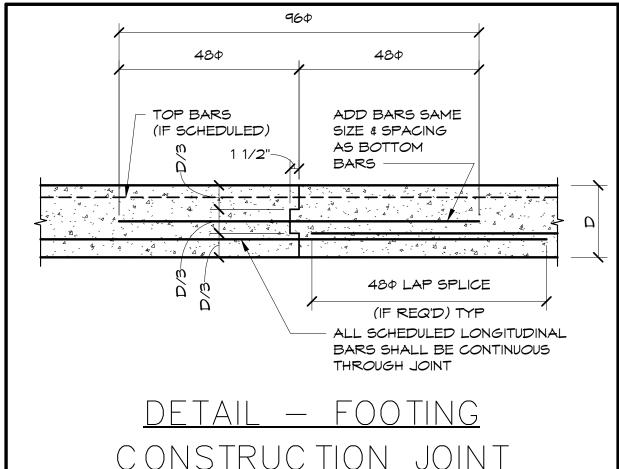


STEPPED FOOTING DETAIL (IF REQUIRED):



FOOTING CONSTRUCTION JOINT (IF REQUIRED):







SCHOOL

Vero Classical School 51 Old Dixie Hwy, Vero Beach, FL 32962

VERO CLASSICAL CHRISTIAN SCHOOL -PHASE I

PROJECT LOCATION: 13TH ST. SW AND 58TH AVE. SW VERO BEACH, FLORIDA



834 N Orange Ave,

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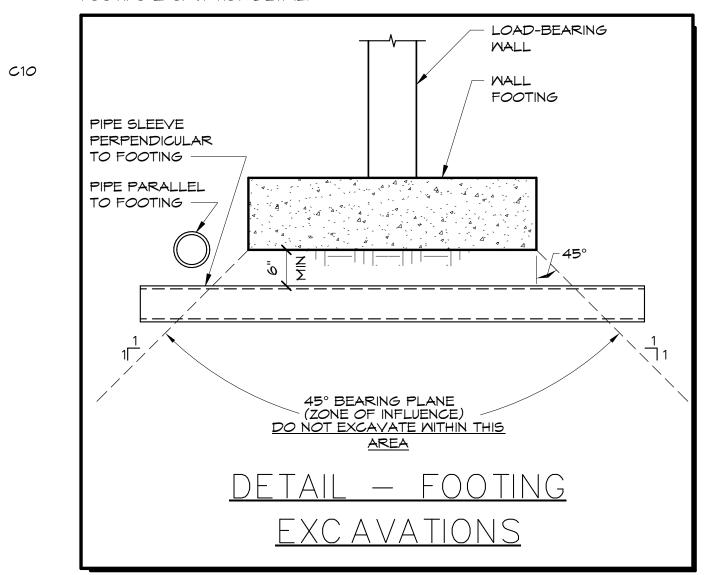
EB: 5343 BBM PROJECT #24133 CENSE

NO. 65509

LUIS F. BEDOYA, P.E. Florida Professional Engineer No. 65509

REVISIONS					
MARK	DESCRIPTION	DATE			

FOOTING EXCAVATION DETAIL



APPRE VATIONS							
			ABBREVIATION	CND			
AAC	AUTOCLAVED AERATED CONCRETE	DBA	DEFORMED BAR ANCHOR	lb, #	POUND(S)	R	RADIUS
AB	ANCHOR BOLT	DIA	DIAMETER	LĠ	LONG	RC	REINFORCED CONCRETE
ABV	AB <i>O</i> VE	DIAG	DIAGONAL	LL	LIVE LOAD	REF	REFERENCE
A.C.I.	AMERICAN CONCRETE INSTITUTE	DL	DEAD LOAD	LLBB	LONG LEG BACK-TO-BACK	REINF	REINFORCE(D) (ING)
ADDL	ADDITIONAL	DN	DOWN	LLH	LONG LEG HORIZONTAL	REQ	REQUIRE
AFF	ABOVE FINISH FLOOR	DTL	DETAIL	LLY	LONG LEG VERTICAL	REQ'D	REQUIRED
AGGR	AGGREGATE	DNG	DRAMING	LNTL	LINTEL	RF	ROOF
A.I.S.C.	AMERICAN INSTITUTE OF STEEL	DML	DOMEL	LSL	LONG SLOTTED	RM	ROOM
	CONSTRUCTION			LOC	LOCATION	RO	ROUGH OPENING
A.I.S.I.	AMERICAN IRON AND STEEL	EA	EACH	LONG	LONGITUDINAL	RP	RADIUS POINT
	INSTITUTE	EE	EACH END	LP	LOW POINT	RTN	RETURN
AL	ALUMINUM	EF	EACH FACE	LRFD	LOAD & RESISTANCE FACTOR	R/W	REINFORCED WITH
ALT	ALTERNATE	EJ	EXPANSION JOINT		DESIGN		
AR	ANCHOR ROD	ENG	ENGINEER	LM	LONG MAY	SCH	SCHEDULE
ARCH	ARCHITECT(URAL)	EOS	EDGE OF SLAB	MAS	MASONRY	SDI	STEEL DECK INSTITUTE
ASD	ALLOWABLE STRESS DESIGN	EL	ELEVATION	MATL	MATERIAL	SECT	SECTION
ASTM	AMERICAN SOCIETY OF TESTING	EQ	EQUAL		MAXIMUM	SF	STEPPED FOOTING
	MATERIALS	EQ SP	EQUAL SPACE(S) (ING)	MAX MB	MACHINE BOLT	SHT	SHEET
A.M.S.	AMERICAN WELDING SOCIETY	ES	EACH SIDE	MB MBM		SHM	SHEAR MALL
.		EM	EACH MAY		METAL BUILDING MFR	SIM	SIMILAR
B/	BOTTOM OF	EXIST	EXISTING	MC	MASONRY COLUMN	SJ	SAMCUT JOINT
BB	BOND BEAM	EXP	EXPANSION	MCJ	MASONRY CONTROL JT	SJI	STEEL JOIST INSTITUTE
BLDG	BUILDING	EXT	EXTERIOR	MCN	MOMENT CONNECTION	SL	SLOPE
BLKG	BLOCKING	- ,	-14-4-	ME	MONOLITHIC EDGE	SLBB	SHORT LEG BACK-TO-BACK
BLM	BELOW	F/	FACE OF	MECH	MECHANICAL	50G	SLAB ON GRADE
BM	BEAM	FBC	FLORIDA BUILDING CODE	MEZZ	MEZZANINE	SP	SPACE(S)
BOT	BOTTOM	FD	FLOOR DRAIN	MF	MONOLITHIC FOOTING	SPECS	SPECIFICATIONS
BP	BASE PLATE	FDN	FOUNDATION	MFR	MANUFACTURE(ER)	5Q	SQUARE
BRDG	BRIDGING	FF	FINISHED FLOOR	MIN	MINIMUM	55	STAINLESS STEEL
BRG	BEARING	FIN	FINISH	MISC	MISCELLANEOUS	SSL	SHORT SLOTTED
BRK	BRICK	FLR	FLOOR	MO	MASONRY OPENING	STD	STANDARD
BS	BOTH SIDES	FLG	FLANGE	MS	METAL STUD	STIFFR	STIFFENER
BTJ	BOLTED TIE JOIST	FMG	FRAMING	MTL	METAL	STL	STEEL
BTMN	BETWEEN	FS 	FAR SIDE	MMF	MONOLITHIC WALL FOOTING	STR	STRENGTH
		FT	FOOT	NUC	NOT IN CONTRACT	STRL	STRUCTURAL
C/C	CENTER TO CENTER	FTG	FOOTING	NIC	NOT IN CONTRACT	SM	SHORT WAY
CANT	CANTILEVER	ga	GAUGE	NS NTS	NEAR SIDE		SYMMETRICAL
CB	CONCRETE BEAM	GALY	GALVANIZE(D)	NIS	NOT TO SCALE	SYP	SOUTHERN YELLOW PINE
CC	CONCRETE COLUMN	GB	GRADE BEAM	OA	OVERALL	511	SOUTHLINK TELECOPY TINE
CFMF	COLD FORMED METAL FRAMING	<i>6</i> 0	GENERAL CONTRACTOR	00	ON CENTER	T\$B	TOP AND BOTTOM
CFS	COLD-FORMED STEEL	GFC	GROUT FILLED CELL(S) / COURSE	0D	OUTSIDE DIAMETER	TB	TIE BEAM
CIP	CAST-IN-PLACE	GLB	GLU-LAM BEAM	OF	OUTSIDE FACE	TC	TIE COLUMN
CJ	CONSTRUCTION JOINT (OR	GLB GR	GRADE	0H	OPPOSITE HAND	TEMP	TEMPERATURE
- -	CONTROL JOINT)	GR	GRADE GIRDER TRUSS	0HD	OVERHEAD	THK	THICK
G, CL	CENTERLINE	O 1		OPNG	OPENING	TOPG	TOPPING
Q, OL CLR	CLEAR(ANCE)	HC	HOLLOW-CORE	OPP	OPPOSITE		TRANSVERSE
CM	CONSTRUCTION MANAGER	HDG	HOT DIPPED GALVANIZED	OFF	OI I OBITE	TYP	TYPICAL
CMU	CONCRETE MASONRY UNIT	HK	HOOK	PAF	PONDER-ACTUATED	T/	TOP OF
COL	COLUMN	HORIZ	HORIZONTAL	1 / 11	FASTENER(S)		
	CONCRETE	HP	HIGH POINT	PB	PRE-CAST BEAM	UNO	UNLESS NOTED OTHERWISE
	CONNECTION	HS	HEADED STUD	PEMB	PRE-ENGINEERED METAL		
	CONTINUOUS	HSS	HOLLOW STRUCTURAL SECTION	FEMB	BUILDING	VIF_	VERIFY IN FIELD
	CONTRACTOR	· — —				VERT	VERTICAL
	COORDINATE	ID	INSIDE DIAMETER	PERP	PERPENDICULAR		
CSK	COUNTER SINK	iF	INSIDE FACE	PC	PRECAST	ND	MOOD
CTR	CENTER	INT	INTERIOR	P. PL	PLATE	MF	MALL FOOTING
CTRD	CENTERED			PLS	PLACES	MO	MINDOM OPENING (MASONRY)
CY	CUBIC YARD	JST	JOIST	PLF	POUNDS PER LINEAR FOOT	MP	WORKING POINT
<u> </u>		JJ	TAIOL		PLYWOOD	MS	WATERSTOP
		j	301111	PNL	PANEL	MMF	MELDED MIRE FABRIC
			1 1	PSF	POUNDS PER SQUARE FOOT	M/	MITH
		K	KIP				• ***
		K KO	KIP KNOCK-OUT	PSI PTN	POUNDS PER SQUARE INCH PARTITION		

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STRUCTURAL GENERAL **NOTES & ABBREVIATIONS**

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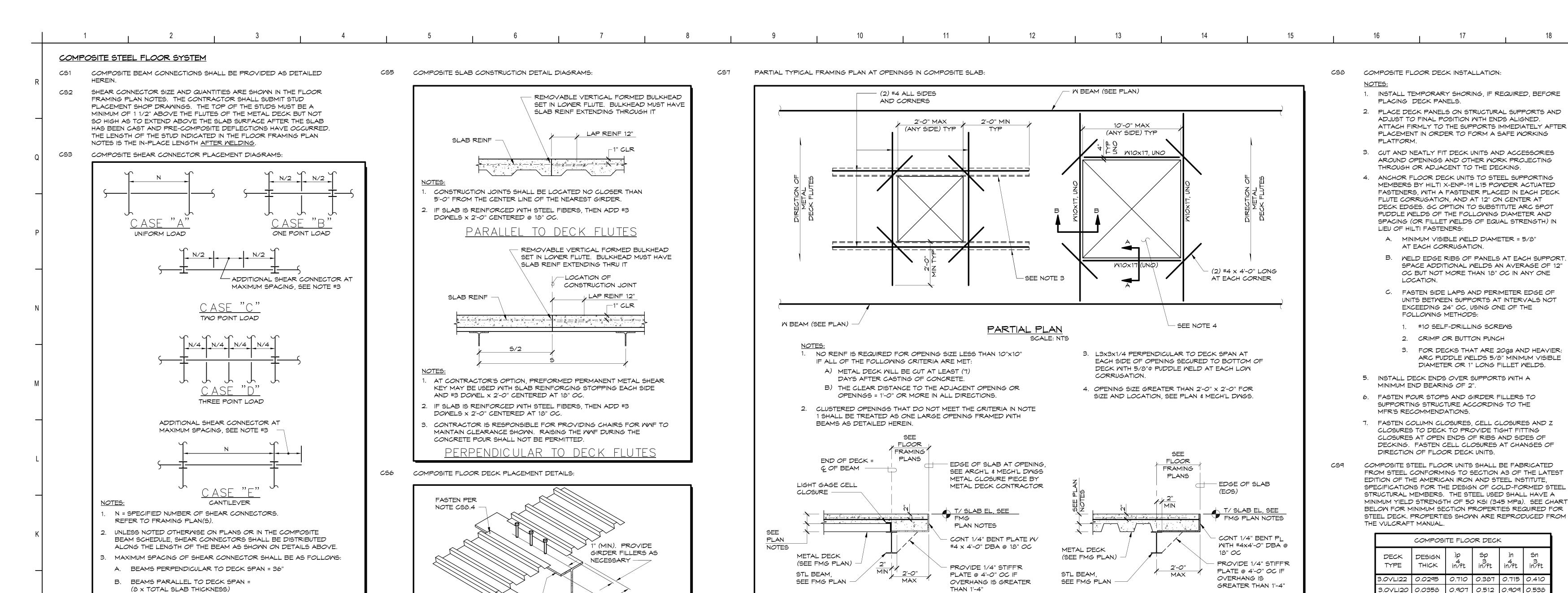
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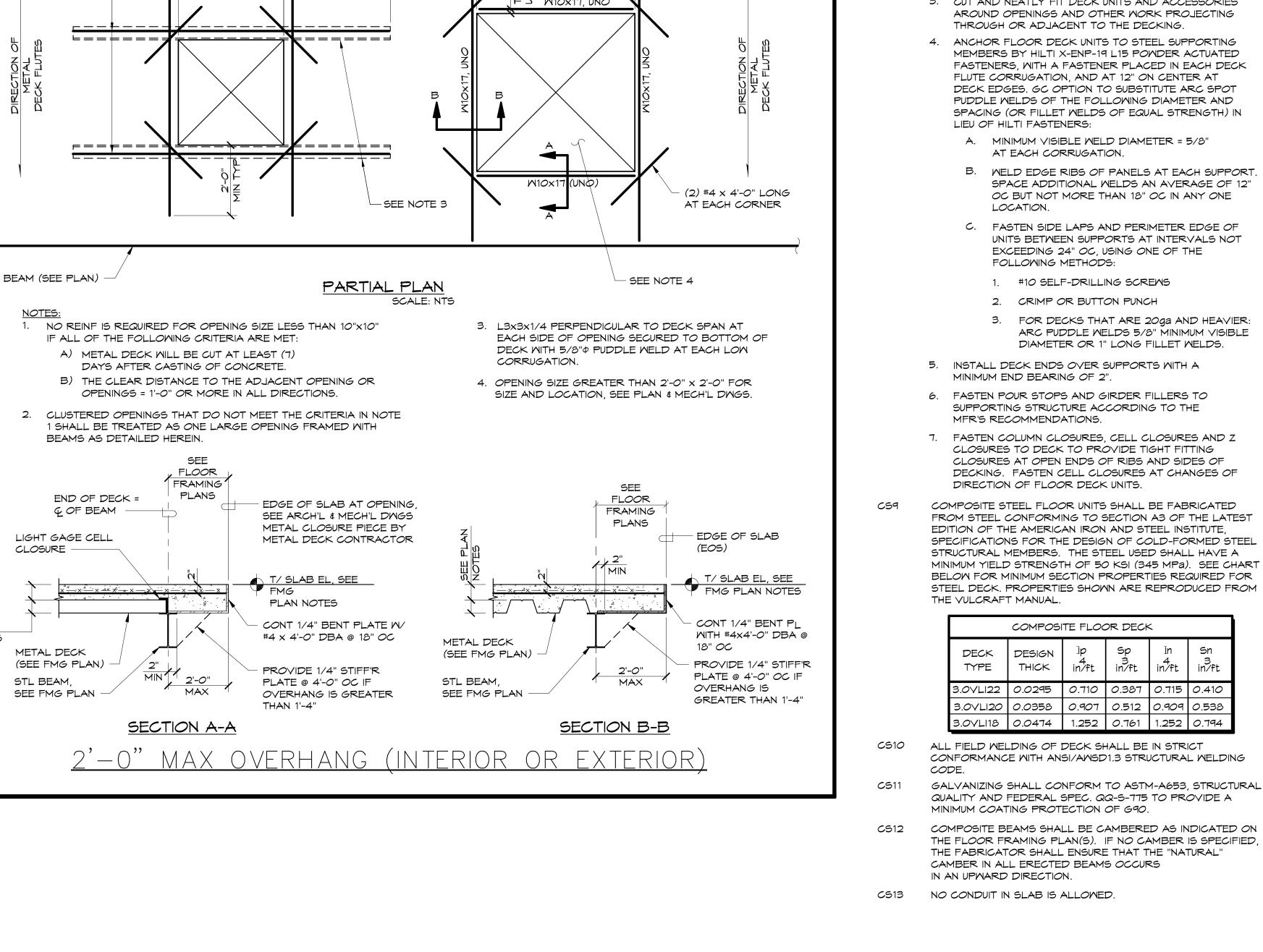
3/4" = 1'-0"

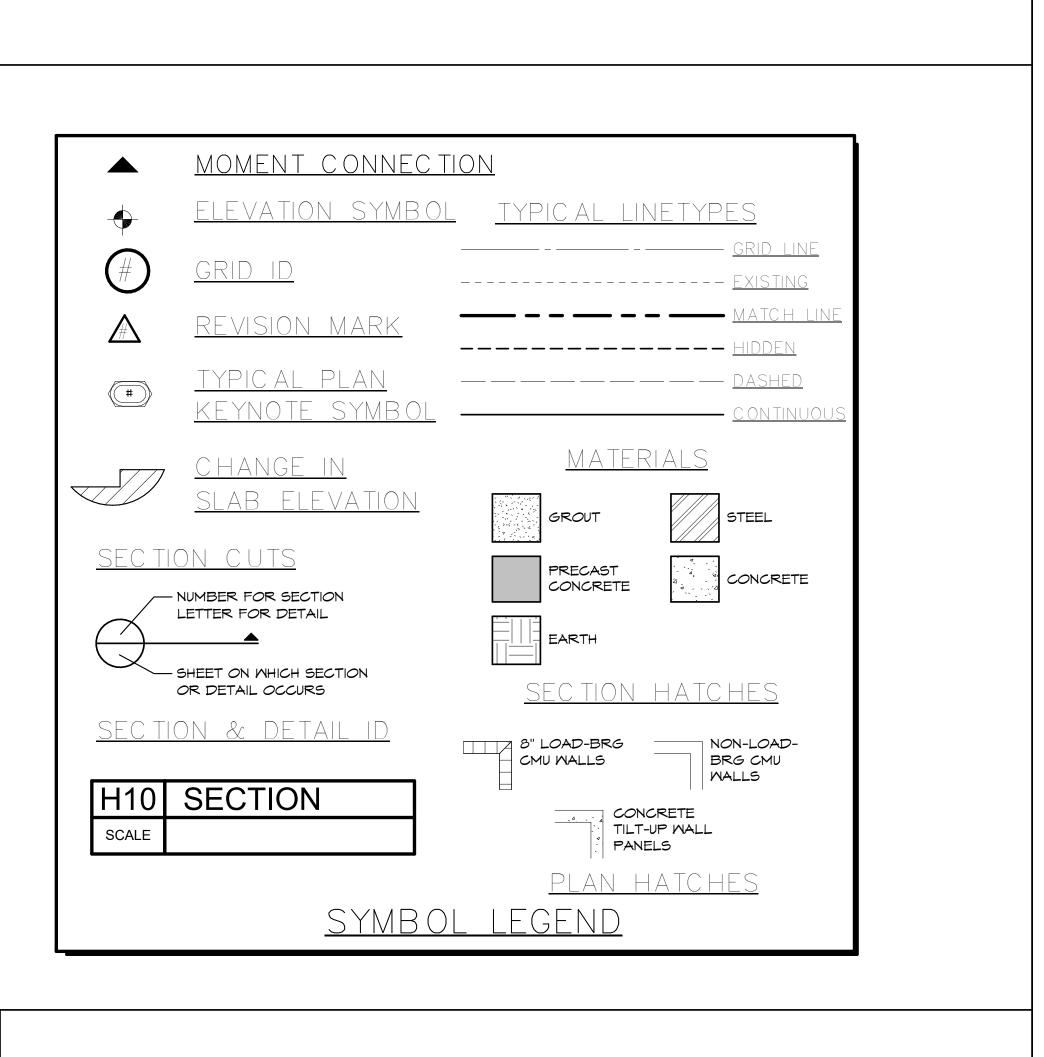
STRUCTURAL GENERAL NOTES

ABBREDVIATIONS

NOT TO SCALE









VERO CLASSICAL CHRISTIAN SCHOOL -PHASE I

PROJECT LOCATION: 13TH ST. SW AND 58TH AVE. SW VERO BEACH, FLORIDA



SCHENKEL SHULTZ

834 N Orange Ave, Winter Park, FL 32789 voice 407-872-3322

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p: 407 - 645 - 3423 EB: 5343 BBM PROJECT #24133

NO. 65509 CORIDA.

LUIS F. BEDOYA, P.E. Florida Professional Engineer No. 65509

REVISIONS DATE DESCRIPTION

NOTES & SYMBOL LEGEND

STRUCTURAL GENERAL

COMM. NO.: 2024117

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ISSUE DATE: 12/23/2024

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3/4" = 1'-0"

4. MINIMUM SPACING OF SHEAR CONNECTOR SHALL BE AS

5. MHERE STEEL DECK CORRUGATIONS DO NOT ALLOW FOR AN EVEN

SPACING OF SHEAR CONNECTORS WITH (1) STUD IN EACH FLUTE.

ADDITIONAL STUDS IN A SECOND ROW (AND THIRD ROW WHERE

SHEAR CONNECTORS OCCURS NEAR THE BEAM SUPPORT.

REQUIRED) SHALL BE PLACED SUCH THAT THE HIGHEST DENSITY OF

WHERE THE SPECIFIED NUMBER OF SHEAR CONNECTOR IS LESS THAN

THE BEAM SPAN LENGTH DIVIDED BY THE MAXIMUM SPACING (SEE

NOTE 3) ADDITIONAL SHEAR CONNECTORS SHALL BE PROVIDED

SUCH THAT THE MAXIMUM SPACING IS NOT EXCEEDED AT ANY

SHEAR CONNECTORS IN (1) ROW SHALL BE PLACED

SHEAR CONNECTOR PLACEMENT DIAGRAMS.

DIRECTLY OVER THE BEAM WEB.

TWO ROWS

2. PLACE STUD IN A SINGLE ROM WHERE SPACING REQUIREMENTS PERMIT.

STUDS SHALL BE PLACED IN (2) OR (3) ROWS ONLY WHERE REQUIRED IN

ORDER TO PLACE THE TOTAL NUMBER OF STUDS. SEE TYPICAL DETAIL,

THREE ROWS

SUBMIT SHOP DRAWINGS SHOWING PLACEMENT OF SHEAR CONNECTORS TO ARCHITECT FOR ENGINEER'S APPROVAL.

A. BEAMS PERPENDICULAR TO DECK SPAN = 3"

B. BEAMS PARALLEL TO DECK SPAN = 4 1/2"

FOLLOWS:

LOCATION IN THE SPAN.

COMPOSITE SHEAR CONNECTOR SPACING DETAIL:

STRUCTURAL GENERAL NOTES

- FLANGE

MINIMUM EDGE DISTANCE =

BEAM PARALLEL TO DECK

FLUTE (GIRDER BEAM)

BEAM PERPENDICULAR TO

DECK FLUTE (FILLER BEAM)

LATERAL AND LONGITUDINAL SPACING IS CONTROLLED BY SECTION

1.11.4 OF THE A.I.S.C.E. SPECIFICATIONS. FOR THE LOCATION OF THE

WHEN STUDS ARE USED, DECK MUST NOT BE LAPPED IN ORDER TO

PERMIT WELDING OF STUDS THROUGH SINGLE THICKNESS OF DECK.

WELDING OF STUDS TO BEAM THROUGH (2) LAYERS OF DECK SHALL

DECK SHALL BE FURNISHED IN MODULAR PANEL WIDTHS. ANY CUTTING THAT IS REQUIRED SHALL BE DONE BY THE DECK

STUD WITH RESPECT TO THE EDGE OF THE FLANGE, SEE THE

STRUCTURAL WELDING CODE A.W.S., 428.8. (DETAIL 4).

STUD DIA + 1/8" ---

CLOSURES NOT

O" TO (FLG

MIDTH - 3")

NOT BE PERMITTED.

NOTES:

OF 1/2" OR LESS

REQUIRED FOR GAP

SYMBOL LEGEND

NOT TO SCALE

STEEL ROOF DECK

MANUAL.

SEE ROOF FRAMING PLAN(S) FOR STEEL DECK ATTACHMENT TO STRUCTURE.

STEEL ROOF DECK UNITS SHALL BE FABRICATED FROM STEEL CONFORMING TO SECTION AS OF THE LATEST EDITION OF THE AMERICAN IRON AND STEEL INSTITUTE, SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS. THE STEEL USED SHALL BE GRADE 80.

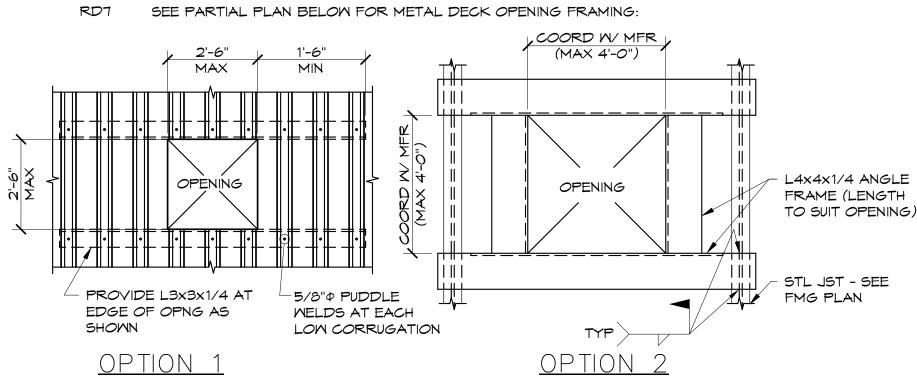
IF OPTED BY GC, ALL FIELD WELDING OF DECK SHALL BE IN STRICT CONFORMANCE WITH ANSI/AWS D1.3 STRUCTURAL WELDING CODE.

ALL SCREMS SHALL COMPLY MITH ASTM 1513, ICC AC43, AND ICC AC118. FASTENERS SHALL BE INSTALLED PERPENDICULAR TO ELEMENT SO AS TO PROPERLY SEAT FASTENER HEAD, AND TORQUED PER MANUFACTURES SPECIFICATIONS, NOT TO EXCEED MAXIMUM RECOMMENDED TORQUE.

GALVANIZING SHALL CONFORM TO ASTM-A653, STRUCTURAL QUALITY AND FEDERAL SPEC. QQ-S-775.

SEE CHART BELOW FOR MINIMUM SECTION PROPERTIES REQUIRED FOR STEEL DECK. PROPERTIES SHOWN ARE REPRODUCED FROM THE VULCRAFT

ROOF DECK							
DECK TYPE	DESIGN THICK	Ip 4 in/ft	Sp 3 in/ft	In 4 in∕ft	Sn 3 in/ft		
B22	0.0295	0.155	0.186	0.183	0.192		
B20	0.0358	0.201	0.234	0.222	0.247		
B18	0.0474	0.289	0.318	0.295	0.327		



A. FOR OPENINGS WITH A MAXIMUM DIMENSION OF 6" TO 1'-0", REINFORCE OPENING WITH A 20ga GALV FLAT PLATE 1'-O" LARGER THAN THE OPENING. ATTACH WITH 1" WELDS AT EACH RIB ALL AROUND.

B. FOR OPENINGS WITH A MAX DIMENSION OF 1'-0" TO 2'-6", SEE DETAIL OPTION 1 ABOVE.

FOR OPENINGS WITH A MAX DIMENSION OF 4'-0",

SEE DETAIL OPTION 2 ABOVE. NO ITEMS SHALL BE HUNG DIRECTLY FROM THE ROOF DECK UNLESS

ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.

INDICATED OTHERWISE IN THE DRAWINGS.

PROVIDE METAL CLOSURE STRIPS AT OPEN UNCOVERED ENDS AND EDGES OF ROOF DECKING AND IN VOIDS BETWEEN DECKING AND OTHER CONSTRUCTION. WELD INTO POSITION TO PROVIDE A COMPLETE ENCLOSED DECKING INSTALLATION. PROVIDE FLEXIBLE CLOSURE STRIPS INSTEAD OF METAL CLOSURES, AT CONTRACTOR'S OPTION, WHEREVER THEIR USE MILL ENSURE COMPLETE CLOSURE. INSTALL MITH ADHESIVE IN

STEEL JOISTS

A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.

ALL DESIGN, FABRICATION AND ERECTION OF STEEL JOISTS AND BRIDGING SHALL BE IN STRICT ACCORDANCE WITH THE SPECIFICATIONS OF STEEL JOIST INSTITUTE (OPEN WEB STEEL JOIST AND JOIST GIRDERS SJI PUBLICATION SJI 100-2015) AND RECOMMENDED CODE OF STANDARD PRACTICE.

THE ENDS OF ALL BRIDGING LINES TERMINATING AT WALLS OR BEAMS SHALL BE ANCHORED TO THE WALL OR BEAM.

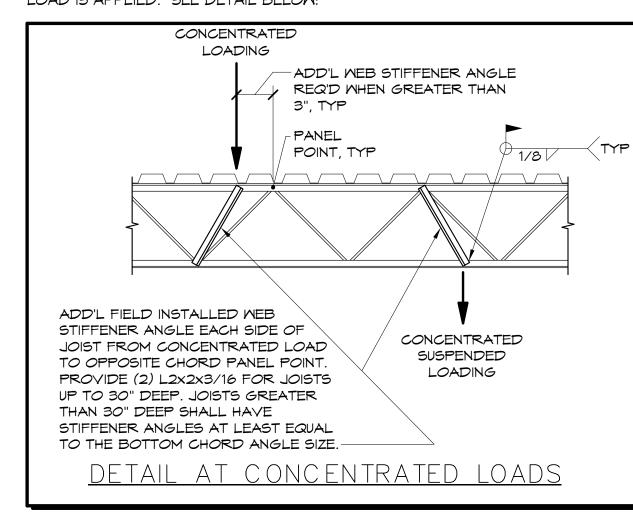
ALL STEEL JOISTS ARE TO BE CAMBERED AS SPECIFIED BY STEEL JOIST INSTITUTE.

PROVIDE BOTTOM AND/OR TOP CHORD EXTENSIONS AS SHOWN ON DRAMINGS.

UNLESS NOTED OTHERWISE, MINIMUM JOIST BEARING SHALL BE 2 1/2" FOR K-SERIES JOISTS, 4" FOR LH, DLH AND SLH 15-18, AND 6" FOR SLH 19-25 ON A STEEL MEMBER OR EMBED PLATE.

BRIDGING SHALL BE FURNISHED AND INSTALLED TO MEET THE SIZE AND SPACING REQUIREMENTS OF THE SJI STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOISTS. ALL BRIDGING AND BRIDGING ANCHORS SHALL BE COMPLETELY INSTALLED BEFORE CONSTRUCTION LOADS ARE PLACED ON THE JOISTS.

ALL HANGERS, CURBS AND/OR ROOFTOP FRAMES TO SUPPORT MECHANICAL EQUIPMENT, ETC., TO BE SUPPORTED BY THE JOISTS SHALL BE LOCATED AT THE PANEL POINTS OF THE JOISTS IF POSSIBLE. HOWEVER, IF THE CONCENTRATED LOAD MUST BE LOCATED FURTHER THAN 3" FROM A PANEL POINT, PROVIDE MEB STIFFENER ANGLES. MEB STIFFENERS MUST BE INSTALLED EACH SIDE OF JOIST FROM CONCENTRATED LOAD TO OPPOSITE CHORD PANEL POINT BEFORE LOAD IS APPLIED. SEE DETAIL BELOW:



CONTRACTOR TO FURNISH BAR JOIST CERTIFICATIONS SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. THE SPECIALTY ENGINEER FOR THE STEEL JOIST SUPPLIER SHALL ALSO CERTIFY THAT THE STEEL JOIST BOTTOM CHORDS WILL SAFELY RESIST THE WIND UPLIFTS, CONSIDERING THE SPACING OF BRIDGING.

PROVIDE UPLIFT BRIDGING PER TABULATED PRESSURES ON SHEET SOO4.

ALL ITEMS SUSPENDED FROM JOISTS (I.E. CATWALKS, BALCONIES, OPERABLE PARTITIONS, ETC.) SHALL BE INSTALLED AFTER DEAD LOAD HAS BEEN

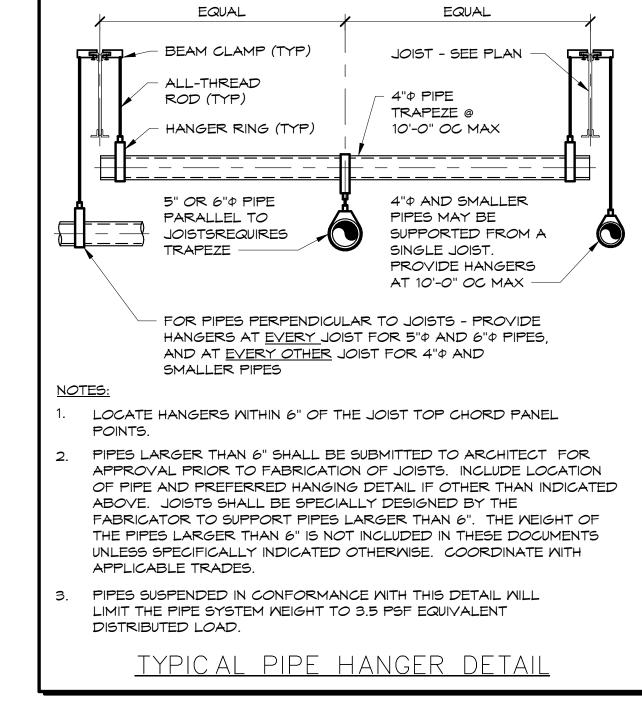
BOLTED TIE JOISTS (BTJ) ARE USED IN STEEL FRAMES WHERE COLUMNS ARE NOT FRAMED IN AT LEAST TWO DIRECTIONS WITH STRUCTURAL STEEL MEMBERS. JOIST(S) AT COLUMN LINES SHALL BE FIELD BOLTED AT THE COLUMNS WITH TWO 1/2" BOLTS TO PROVIDE LATERAL STABILITY DURING CONSTRUCTION.

STEEL JOISTS SHALL RECEIVE SHOP COAT OF PRIMER (COLOR AS DIRECTED BY ARCHITECT) EXCEPT THOSE AREAS WHICH WILL RECEIVE SPRAY-ON FIRE PROTECTION.

ANY STEEL JOIST WITHIN A 4'-O" DISTANCE FROM A PARALLEL SUPPORT SHALL BE FABRICATED IN SUCH A WAY THAT CAMBER OF THE JOIST WILL NOT CAUSE A PROBLEM INSTALLING THE METAL DECK. CAMBER SHALL BE INTERPOLATED USING SJI STANDARD CAMBER SCHEDULE AND REDUCED BASED ON THE REDUCTION OF TRIBUTARY SPACING USING A 5'-O" SPACING AS STANDARD FOR REQUIRED CAMBER.

IN THE EVENT THAT FIRE SPRINKLERS ARE REQUIRED FOR THIS PROJECT, THE STEEL FABRICATOR SHALL PROVIDE A DIMENSIONED JOIST BRIDGING AND JOIST GIRDER BOTTOM CHORD BRACE PLAN ALONG WITH DETAILS TO THE SPRINKLER CONTRACTOR. THE FABRICATOR AND SPRINKLER CONTRACTOR SHALL COORDINATE WITH EACH OTHER TO ENSURE THAT ANY CONFLICTS ARE RESOLVED BEFORE ANY FABRICATION BEGINS.

ALL PIPES MUST BE SUPPORTED AS SHOWN BELOW:



FABRICATOR SHALL ENSURE THAT ALL OSHA REQUIREMENTS ARE MET. PARTICULAR ATTENTION SHALL BE PAID TO THE ERECTION PROCESS. BOLTED CONNECTIONS MAY BE REQUIRED. SUBMIT DETAILS FOR APPROVAL.

5J18 THE JOIST MANUFACTURER MAY NOT INCREASE ALLOWABLE STRESSES. THE OPEN MEB STEEL JOISTS SHALL BE FABRICATED AND ERECTED IN FULL CONFORMANCE WITH THE "OSHA STEEL ERECTION STANDARD". IF THE CONSTRUCTION DRAWINGS DEVIATE FROM THE OSHA STANDARD THEN THE FABRICATOR SHALL PROVIDE SUBMITTALS THAT CLEARLY INDICATE THE DEVIATION WITH A REVISION CLOUD AND REQUEST APPROVAL FROM BBM TO MAKE THE CHANGE SO THAT CONFORMANCE WITH THE OSHA STANDARD IS

K-SERIES STEEL JOISTS WITH SPANS 40'-O" AND LONGER SHALL BE ERECTED IN PANELS SO THAT BOLTED CONNECTIONS ARE NOT REQUIRED (EXCEPT AT THE COLUMN LINES). THE GC SHALL INSURE THAT ALL RELATED JOIST FRAMING COMPONENTS ARE COORDINATED TO MEET THIS REQUIREMENT.

SJ21 ALL ROOFS THAT EXCEED 1/4"/FT SLOPE SHALL HAVE THE JOIST BEARING SEATS SLOPED AS REQUIRED PER STEEL JOIST INSTITUTE.

STRUCTURAL STEEL

A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.

FABRICATE AND ERECT STRUCTURAL STEEL IN CONFORMANCE

WITH THE LATEST VERSION OF AISC 360-16. MATERIAL SPECIFICATIONS:

> ALL STEEL SHALL BE PRODUCED DOMESTICALLY. ROLLED SHAPES, PLATES AND BARS: ASTM A572 GR 50, EXCEPT WIDE-FLANGE & WT SECTIONS, WHICH SHALL BE ASTM A992. HOLLOW STRUCTURAL SECTION (HSS): ASTM A500, GRADE C.

ANCHOR BOLTS, RODS, NUTS AND WASHERS: PER BASE PLATE SCHEDULE.

HEADED STUDS: ASTM A108, GRADE 1015 THROUGH 1020, COLD-FINISHED CARBON STEEL, AMS D1.1, TYPE B.

HYDROGEN). FILLET WELDS SHALL BE 3/16" UNO.

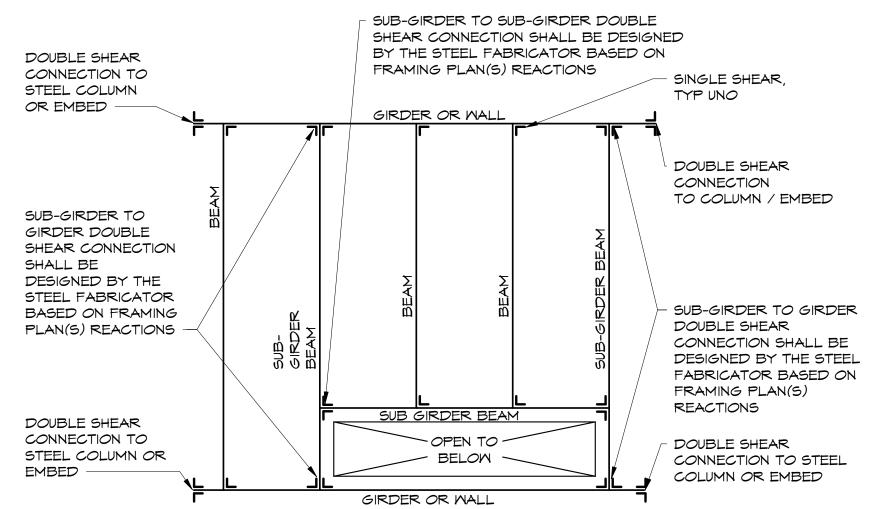
BOLTED STRUCTURAL CONNECTIONS: UNLESS NOTED OTHERWISE, ALL BOLTS SHALL BE 3/4" PASTM A325, TYPE N. BOLTS INDICATED LESS THAN 5/8" PSHALL BE ASTM A307. MELDED CONNECTIONS: ELECTRODES - ETOXX UNO (LOW

HIGH-STRENGTH FIELD-BOLTED CONNECTIONS SHALL BE INSTALLED, TIGHTENED, TESTED AND INSPECTED ACCORDING TO "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS" BY THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (RCSC). ALL BOLTS IN STEEL TO STEEL CONNECTIONS SHALL BE BROUGHT TO A "SNUG-TIGHT" CONDITION, AS DEFINED IN THE SPECIFICATION. ALL BOLTS IN STEEL TO EMBED CONNECTIONS SHALL BE FINGER-TIGHT WITH PEENED THREADS. SLIP-CRITICAL (SC) BOLTS MUST BE FULLY TENSIONED PER SPECIFICATION.

STANDARD NON-SLOPED AND NON-SKEMED SHEAR CONNECTIONS HAVE BEEN DESIGNED AND THE NECESSARY INFORMATION MAY BE FOUND IN THE SCHEDULES. THE ULTIMATE (i.e. FACTORED) REACTIONS HAVE BEEN PROVIDED AT EACH END OF EACH MEMBER SHOULD THE FABRICATOR WISH TO RE-ENGINEER THE CONNECTIONS TO THEIR PREFERENCES. SHOULD THE FABRICATOR WISH TO RE-ENGINEER THE CONNECTIONS, THEY MUST PROVIDE SUBMITTALS AND CALCULATIONS THAT HAVE BEEN PREPARED AND SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE SAME STATE AS THE PROJECT LOCATION.

NON-STANDARD SLOPED AND/OR SKEWED SHEAR CONNECTIONS SHALL BE DESIGNED & DETAILED BY THE FABRICATOR'S ENGINEER. PROVIDE SIGNED & SEALED CONNECTION SUBMITTAL AND CALCULATIONS FOR

ALL WIDE FLANGE FLOOR MEMBERS SHALL BE CONNECTED TO THE SUPPORTING STRUCTURE AS DETAILED IN CONNECTION SCHEDULES ON SHEET S501. UNLESS SPECIFICALLY NOTED OTHERWISE ON PLAN(S), ANY FLOOR MEMBER SUPPORTING ANOTHER FLOOR MEMBER SHALL BE CONNECTED AS DETAILED IN DOUBLE SHEAR SCHEDULE M1/S501. SINGLE SHEAR CONNECTIONS AS DETAILED IN SCHEDULES F1/S501 AND M7/S501 SHALL ONLY BE USED FOR FLOOR MEMBERS SUPPORTING DECK/SLAB ONLY (I.E. FILLER BEAMS) OR AS SPECIFICALLY IDENTIFIED ON PLAN OR SECTION. THE USE OF A DOUBLE SHEAR CONNECTION MAY BE REQUIRED FOR A "TYPICAL" FLOOR BEAM DUE TO BEAM REACTION. REFER TO REACTION NOTED ON PLAN(S) & COORDINATE WITH SCHEDULED MAXIMUM VALUES TO DETERMINE CONNECTION TYPE REQUIRED. SEE PARTIAL SCHEMATIC FRAMING PLAN BELOW THAT ILLUSTRATES WHERE TYPICAL SINGLE AND DOUBLE SHEAR CONNECTIONS ARE REQUIRED.



PARTIAL SCHEMATIC FRAMING PLAN TO ILLUSTRATE TYPICAL SINGLE & DOUBLE SHEAR CONNECTION LOCATIONS

ALL WIDE FLANGE ROOF MEMBERS SHALL BE CONNECTED TO THE SUPPORTING STRUCTURE AS DETAILED IN THE CONNECTION SCHEDULES ON SHEET 5501. UNLESS SPECIFICALLY NOTED OTHERWISE ON PLAN, ALL ROOF MEMBERS SHALL BE CONNECTED AS DETAILED IN THE SINGLE SHEAR SCHEDULES F1/S501 AND M7/S501.

BRACE AND MAINTAIN ALL STEEL IN ALIGNMENT UNTIL OTHER PARTS OF CONSTRUCTION NECESSARY FOR PERMANENT SUPPORT ARE COMPLETED. CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLING TEMPORARY SHORING AS REQUIRED FOR THE STABILITY OF THE STEEL FRAME UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN COMPLETED AND BUILDING IS ENCLOSED.

ALL MELDING SHALL CONFORM TO THE REQUIREMENTS OF "THE STANDARD CODE FOR WELDING IN BUILDING CONSTRUCTION" OF THE AMERICAN WELDING

GROUT FOR COLUMN BASE PLATES AND PRESET BEARING PLATES SHALL BE NON-SHRINK, NON-METALLIC GROUT (5000 PSI MIN).

SUBMIT SHOP DRAWINGS INDICATING ALL SHOP AND ERECTION DETAILS INCLUDING PROFILES. SIZES, SPACING AND LOCATIONS OF STRUCTURAL MEMBERS, CONNECTION ATTACHMENTS, FASTENERS, LOADS AND TOLERANCES.

ALL STEEL EXPOSED TO MEATHER SHALL BE HOT DIP GALVANIZED IN ACCORDANCE WITH ASTM A123 FOR MEMBERS AND ASTM A153 FOR CONNECTION ELEMENTS, EXCEPT THAT ALL ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS) SHALL BE BLAST CLEANED AND COATED IN ACCORDANCE WITH THE STRUCTURAL STEEL AND PAINT SPECIFICATIONS.

STRUCTURAL STEEL SHALL RECEIVE A SHOP COAT OF PRIMER (COLOR AS DIRECTED BY ARCHITECT) EXCEPT THOSE AREAS WHICH WILL RECEIVE SPRAY-ON FIRE PROTECTION, OR WHERE HEADED STUDS ARE TO BE WELDED.

PROVIDE ALLOMANCE FOR 300 FEET OF L3x3x1/4 AND LABOR TO INSTALL

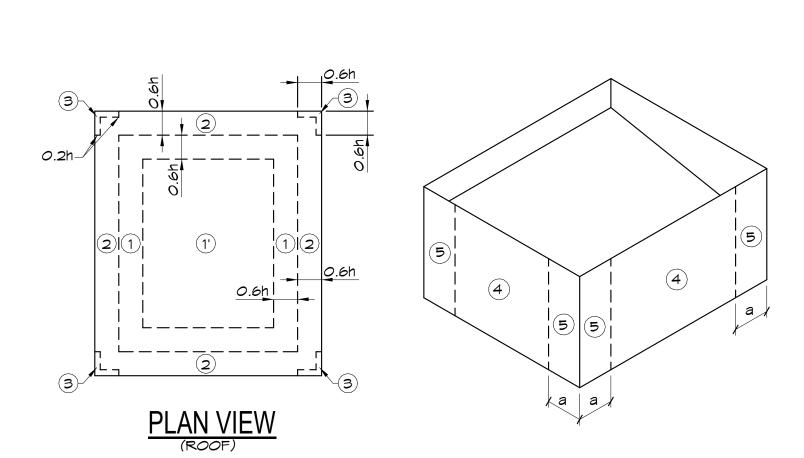
SAME AT ARCHITECT OR ENGINEER'S DIRECTION. CREDIT OWNER WITH ALL MATERIAL AND LABOR NOT USED. STEEL BEAMS SHALL BE CAMBERED AS INDICATED ON THE FRAMING PLAN(S) IF NO CAMBER IS SPECIFIED, THE FABRICATOR SHALL ENSURE THAT THE

"NATURAL" CAMBER IN ALL ERECTED BEAMS OCCURS IN AN UPWARD DIRECTION. ALL STEEL BEAMS THAT ARE PARALLEL TO BAR JOISTS SHALL BE CAMBERED TO MATCH THE JOIST CAMBER AND THE TOP OF THE BEAM SHALL MATCH THE

THE STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN FULL CONFORMANCE WITH THE "OSHA STEEL ERECTION STANDARD". IF THE CONSTRUCTION DRAWINGS DEVIATE FROM THE OSHA STANDARD THEN THE FABRICATOR SHALL PROVIDE SUBMITTALS THAT CLEARLY INDICATE THE DEVIATION WITH A REVISION CLOUD AND REQUEST APPROVAL FROM BBM TO MAKE THE CHANGE SO THAT CONFORMANCE WITH THE OSHA STANDARD IS

TOP OF THE JOIST, TYP UNO.

REFER TO SPECIALTY ENGINEERING (SE) NOTES FOR DELEGATED ENGINEERING REQUIREMENTS.



1. A (kd) OF 0.85 HAS BEEN USED IN THE DEVELOPMENT OF THESE VALUES. THE USE OF THESE VALUES SHALL ONLY BE APPLIED WHEN USED IN CONJUNCTION WITH LOAD COMBINATIONS

SPECIFIED IN SECTIONS 2.3 AND 2.4 OF ASCE 7-22. 2. PRESSURES AND SUCTIONS ON SOFFITS SHALL BE THE SAME AS CORRESPONDING WALL ZONES 4 & 5.

3. IF THE STRUCTURE IS INSURED BY FACTORY MUTUAL (FM) THE GROSS UPLIFT DESIGN PRESSURES SHOWN HEREIN SHALL BE DOUBLE FOR ROOF COVERINGS.

4. THE "ULTIMATE" WIND LOADS SHOWN IN THE COMPONENTS AND CLADDING SCHEDULE SHALL BE MULTIPLIED BY 0.6 TO REDUCE THEM DOWN TO "SERVICE" LEVEL ALL TESTED ASSEMBLIES, INCLUDING BUT NOT LIMITED TO DOORS, WINDOWS AND ROOF ASSEMBLIES.

5. PARAPET WIND LOADS ARE BASED ON A "SOLID" PARAPET WITH NO INTERNAL PRESSURE. ACTUAL PARAPET CONSTRUCTION MAY DICTATE THAT INTERNAL PRESSURE BE ADDED TO THE EXTERNAL

PRESSURE IN ACCORDANCE WITH ASCE 7-22 SECTION 30.6. 6. UPLIFT VALUES FOR ZONE 2 MAY BE USED IN LIEU OF ZONE 3 FOR CORNER ZONES IF PARAPET HEIGHT WITH RESPECT TO FINISHED

ROOF IS GREATER THAN 3 ft. 7. FOR ATTACHED CANOPIES ON BUILDING, REFERENCE ASCE 7-22 SECTION 30.9 FOR PRESSURE COEFFICIENTS.

8. DEAD LOAD OF 10 PSF (NO FURTHER REDUCTION ALLOWED) SHALL BE USED TO CONVERT GROSS UPLIFT TO NET UPLIFT FOR OPEN WEB JOIST DESIGN.

MIND DESIGN CRITERIA

Ultimate Design Wind Speed	168 mph
Service Design Wind Speed	130.1 mph
Risk Category	III
Exposure Category	C
Enclosure Classif.	Enclosed Building
Internal Pressure Coef.	+/-0.18

COMPONENT AND CLADDING ULTIMATE WIND PRESSURES

h = 47.0 ft a = 28.2 ft

_					
Roof	Surface Pressure (psf)				
Area	10 sf	100 sf	500 sf	1000	
Negative Zone 1	-124.1	-96.9	-77.9	-77.9	
Negative Zone 1'	-71.3	-71.3	-48.2	-38.3	
Negative Zone 2	-163.7	-128.7	-104.3	-104.	
Negative Zone 3	-223.1	-153.1	-104.3	-104.	
Positive All Zones	31.7	25.1	25.1	25.1	
Overhang Zone 1&1'	-112.2	-105.6	-66.0	-66.0	
Overhang Zone 2	-151.8	-105.2	-72.6	-72.6	
O	011 0	1000	701		

Overhang Zone 3 -211.2 -129.6 -72.6 -72.6 Overhang pressures in the table above assume an internal pressure coefficient (Gcpi) of O.O Overhang soffit pressure equals adj wall pressure (which includes internal pressure of 11.9 psf)

	Parapet		Solid	Parapet ¹	Pressure	(psf)	
	Area	10 sf	20 sf	50 sf	100 sf	200 sf	500 sf
CASE A:	<i>Zo</i> ne 2 :	213.4	199.6	181.3	167.5	153.7	135.4
	Zone 3 :	273.5	249.0	216.7	192.2	167.7	135.4
CASE B: Int	erior zone :	-126.1	-119.7	-111.2	-104.9	-98.5	-90.0
C	orner zone :	-144.1	-134.5	-121.8	-112.3	-102.7	-90.0

_				
Mall	Surface Pressure (psf)			
Area	10 sf	100 sf	200 sf	500 sf
Negative Zone 4	-77.2	-66.7	-63.6	-59.4
Negative Zone 5	-95.0	-74.1	-67.7	-59.4
Positive Zone 4 & 5	71.3	60.8	57.6	53.5

TORNADO DESIGN CRITERIA

Tornado Wind Speed Effecting Area 47,456 sf 0.55 Internal Pressure Coef -0.18 TORNADO DESIGN NOT REQUIRED

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ISSUE DATE: 12/23/2024

STRUCTURAL GENERAL

NOTES & WIND SCHEDULES

Vero Classical

SCHOOL

PROJECT LOCATION: 13TH ST. SW AND 58TH AVE. SW

SCHENKEL

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STRUCTURAL

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Maitland, FL 32751

p: 407 - 645 - 3423

BBM PROJECT #24133

CENSE

NO. 65509

NO NORIDA.

LUIS F. BEDOYA, P.E.

Florida Professional Engineer No. 65509

REVISIONS

DESCRIPTION

EB: 5343

SEE FOR POLICY AND INFORMATION

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Vero Classical School

PHASE I

VERO BEACH, FLORIDA

834 N Orange Ave,

Winter Park, FL 32789

voice 407-872-3322

51 Old Dixie Hwy, Vero Beach, FL 32962

VERO CLASSICAL

CHRISTIAN SCHOOL -

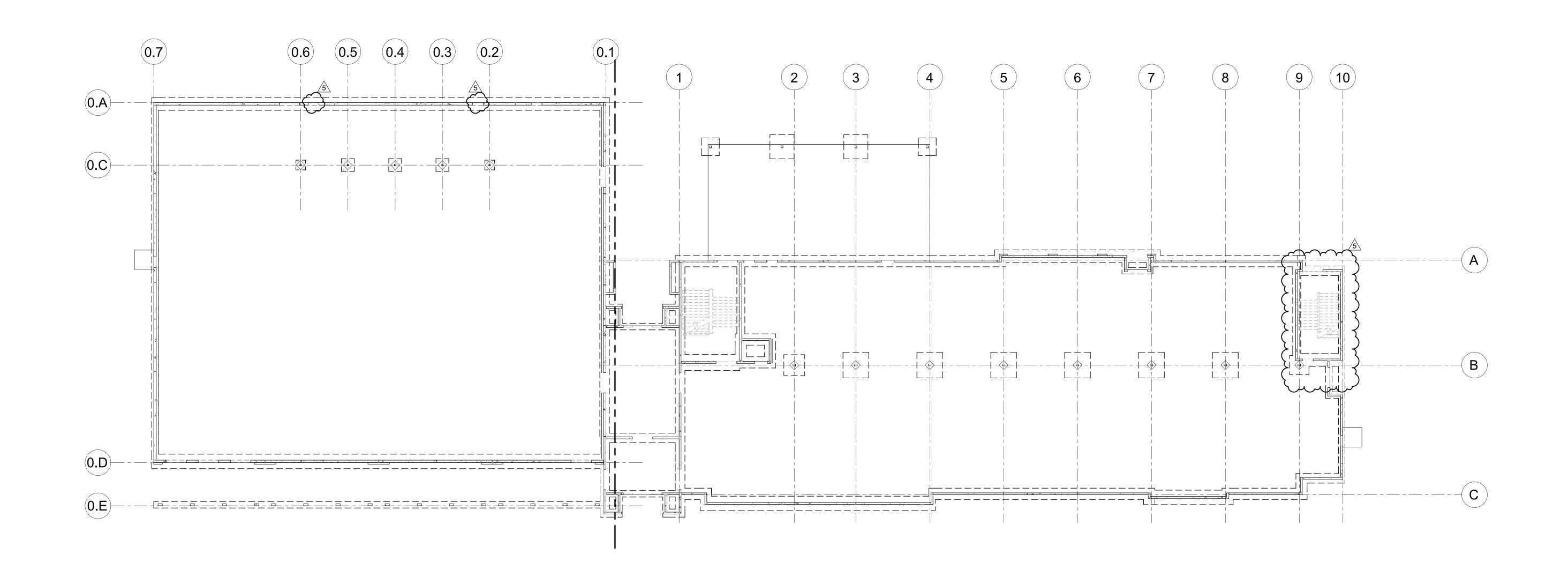
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12" = 1'-0"

WIND SCHEDULE

NOT TO SCALE

STRUCTURAL GENERAL NOTES



FOUNDATION PLAN NOTES:

- 1. FLOOR SLAB SHALL BE 5" THICK CONCRETE REINF WITH 6x6-W2.1xW2.1. UNO IN ARCHITECTURAL SPECIFICATIONS, PROVIDE 10 MIL (MIN) VAPOR RETARDER (ASTM E 1745-11) ON COMPACTED SUBGRADE. SEE "SLAB-ON-GRADE DETAILS" ON SHEET S201 FOR PLACEMENT OF REINF.
- 2. T/ SLAB EL = 100'-0" (TYP, UNO). REFERENCE ONLY SEE CIVIL DWGS FOR ACTUAL ELEVATION.
- 3. T/ WALL FTG EL = 98'-8" (TYP, UNO).
- 4. T/ COL FTG EL = 98'-8" (TYP, UNO).
- 5. SEE SHEETS S603 TO S606 FOR TILT-UP PANEL THICKNESSES. PANEL DESIGN INCLUDES UP TO A 3/4"
 DEEP REVEAL ALLOWANCE, TYP.
- 6. ALL TILT-UP PANELS ARE VIEWED FROM THE INSIDE OF THE BUILDING.
- 7. STEP AND/OR LOWER FOUNDATIONS WHERE SHOWN AND AS NECESSARY TO AVOID INTERFERENCE WITH OTHER TRADES. SEE CONCRETE GENERAL NOTES FOR DETAILS AND SECTIONS. PARTICULAR ATTENTION SHALL BE PAID TO DOWNSPOUTS ENSURING THAT PROPER ACTIONS HAVE BEEN TAKEN TO PREVENT PIPES FROM CONFLICTING WITH THE FOUNDATION SYSTEM.
- ALL FTGS ARE CENTERED BENEATH THE BEARING WALLS AND COLUMNS (TYP, UNO).
- 9. SEE SHEET S601 AND S602 FOR PANEL REINF, EMBEDDED ITEMS AND JOINT DETAILS.
- 10. SEE SHEETS 5001 THRU 5004 FOR STRUCTURAL GENERAL
- 11. MAINTAIN STRUCTURAL SLAB THICKNESS AT ALL FLOOR SLOPES AND DEPRESSIONS.
- 12. THE CONTRACTOR SHALL COORDINATE ALL UNDERGROUND UTILITIES, PIPES, ETC...WITH THE FOUNDATION PLAN AND FOUNDATION ELEVATIONS. FOOTING PENETRATION DETAILS MAY BE FOUND IN THE CONCRETE AND REINFORCING SECTION OF THE STRUCTURAL GENERAL NOTES.

FOUNDATION LEGEND:

- C INDICATES STEEL COLUMN. SEE SCHEDULE ON
- SHEET S501 FOR INFO.

 F INDICATES PAD FOOTING. SEE FOUNDATION
- SCHEDULE ON SHEET S501 FOR INFO.

 WF INDICATES WALL FOOTING. SEE FOUNDATION
- SCHEDULE ON SHEET S501 FOR INFO.

 ME INDICATES MONOLITHIC EDGE. SEE FOUNDATION
- SCHEDULE ON SHEET 5501 FOR INFO.

 MMF INDICATES MONOLITHIC WALL FOOTING. SEE
- FOUNDATION SCHEDULE ON SHEET S501 FOR INFO.
- MF INDICATES MONOLITHIC FOOTING. SEE FOUNDATION SCHEDULE ON SHEET S501 FOR INFO.

FOUNDATION PLAN KEY NOTES:

- (2) #4x4'-0" LONG @ 3" OC PLACED 2" CLR FROM CORNER, CENTERED IN SLAB (TYP WHERE SHOWN).
- SLAB DEPRESSION, SEE DETAIL E15/S201. REFERENCE

 ARCH DWGS FOR EXACT SIZE LOCATION AND DEPTH OF
- ARCH DWGS FOR EXACT SIZE, LOCATION AND DEPTH OF DEPRESSION.
- TURN DOWN WALL FOUNDATION ON TOP OF ELEVATOR MAT, REF A1/5201.
- MMF20 @ STAIRS COLUMNS IF REQUIRED, REF N4/S201.



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VERO CLASSICAL CHRISTIAN SCHOOL -PHASE I

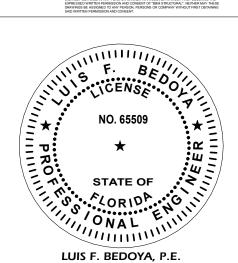
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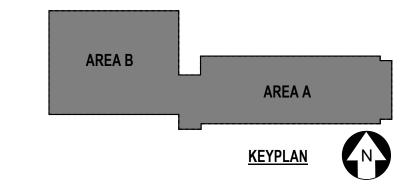


Florida Professional Engineer No. 65509

REVISIONS

MARK DESCRIPTION DATE

5 Addendum -04 _ IRC BUILDING REVIEW COMMENTS 6/6/2025



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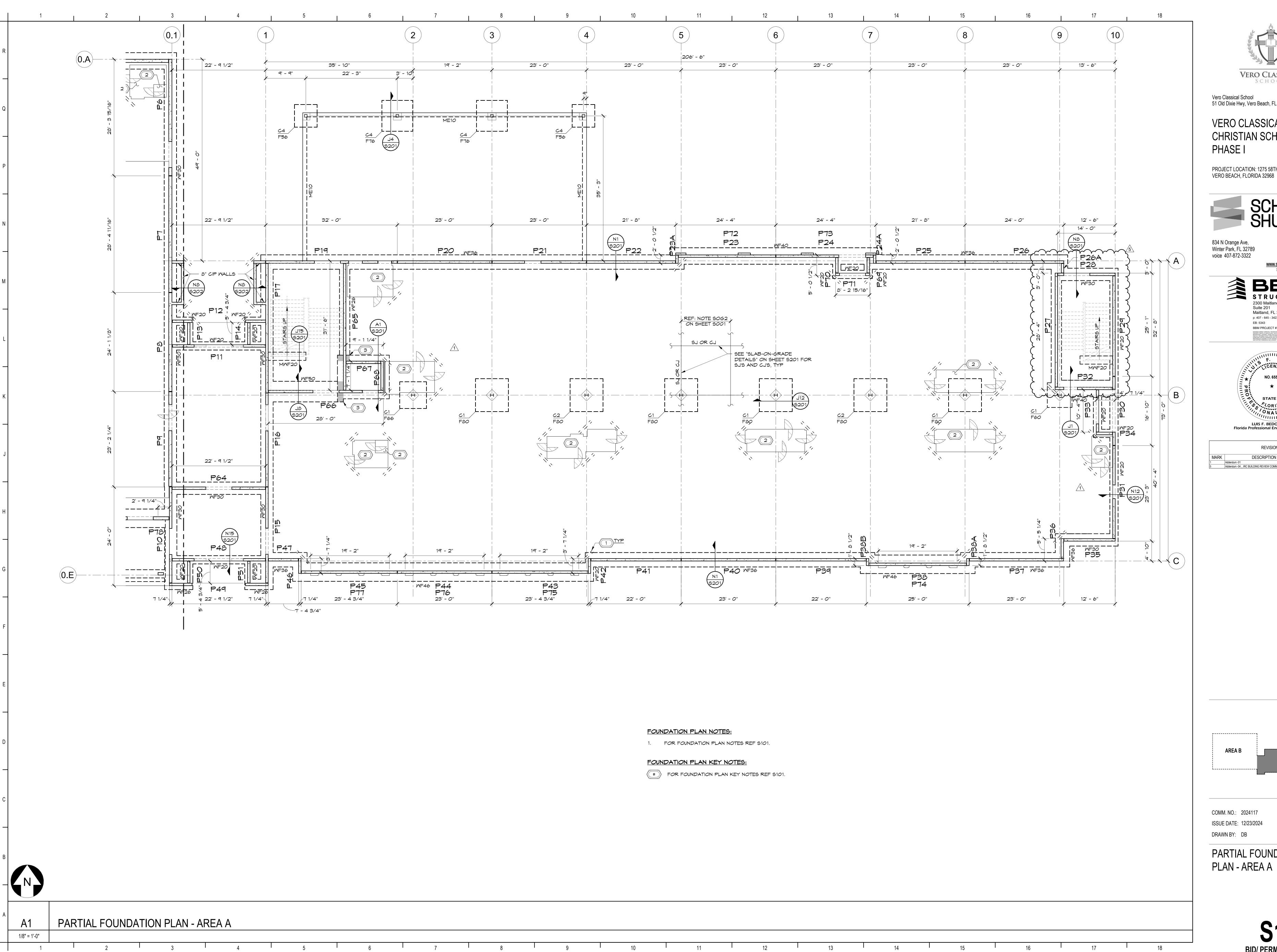
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OVERALL FOUNDATION PLAN

1/16" = 1'-0"

OVERALL FOUNDATION PLAN





VERO CLASSICAL CHRISTIAN SCHOOL -

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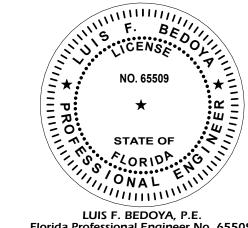


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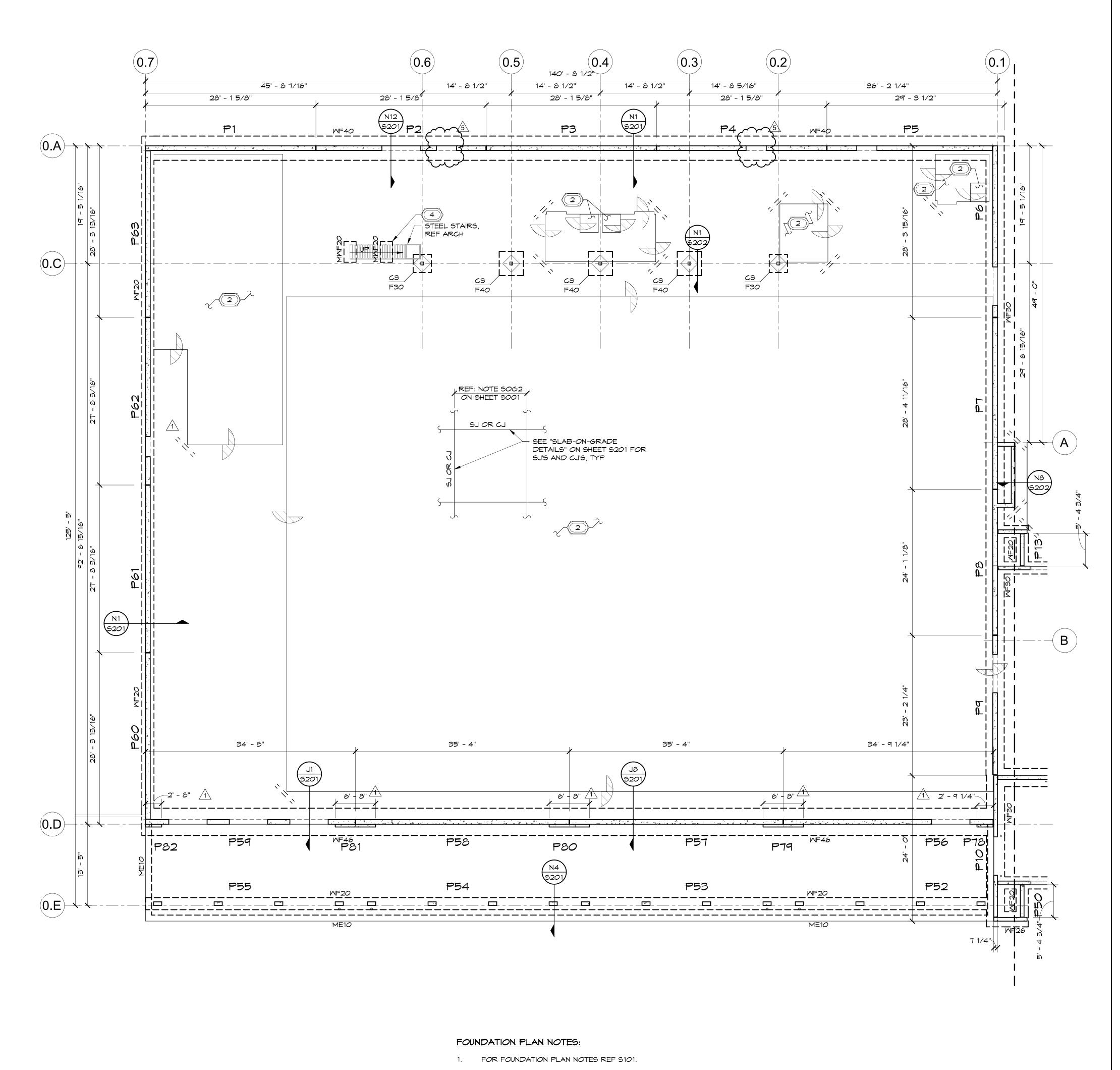
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AREA B

COMM. NO.: 2024117 ISSUE DATE: 12/23/2024

PARTIAL FOUNDATION PLAN - AREA A



FOUNDATION PLAN KEY NOTES:

FOR FOUNDATION PLAN KEY NOTES REF 5101.



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VERO CLASSICAL CHRISTIAN SCHOOL -PHASE I

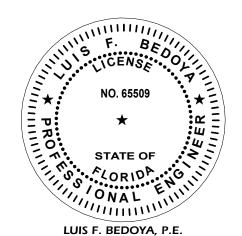
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EB: 5343
BBM PROJECT #24133
BIGGET APPROJECT #24133
BIGGET APPROJECT #24133
BIGGET APPROJECT #24164
BIGGET APPROVED A



AREA B

AREA A

KEYPLAN

COMM. NO.: 2024117

ISSUE DATE: 12/23/2024

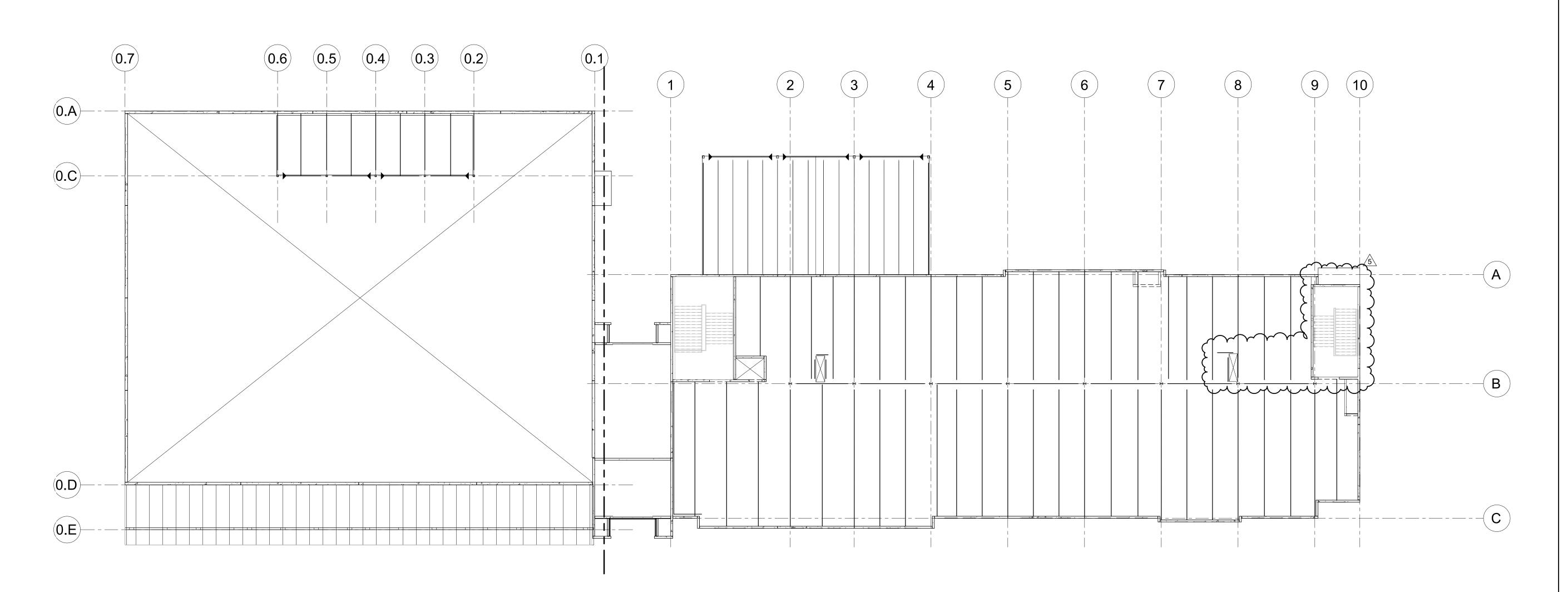
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PARTIAL FOUNDATION PLAN - AREA B

S101.B
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A1 PARTIAL FOUNDATION PLAN - AREA B

1/8" = 1'-0"



FLOOR FRAMING PLAN NOTES:

1. FLOOR SLAB SHALL BE AN UNSHORED 6 1/2" (TOTAL DEPTH) CONC SLAB REINF W/ 6x6-W2.1xW2.1 WWF ON 3"-20 ga GALV G90 COMPOSITE METAL DECK, (3) SPAN MIN.

> NOTE 1: A BLEND OF STEEL AND POLYPROPYLENE FIBERS (NOVOMESH 850) IS AN ACCEPTABLE ATERNATIVE TO WELDED-WIRE FABRIC. FIBERS SHALL BE AS MANUFACTURED BY PROPEX CONCRETE SYSTEMS (OR APPROVED EQUAL) APPLIED AT A RATE OF 24 lbs/CY. OPTIONALLY FIBERMESH 650 APPLIED AT A RATE OF 4 lbs/CY OR HELIX 5-25 APPLIED AT A RATE OF 7 lbs/CY ARE ALSO ACCEPTABLE.

NOTE 2: IF (3) SPAN CONFIGURATION CANNOT BE ACCOMMODATED, 18 ga GALV G90 MTL DECK MUST BE USED.

NOTE 3: DECK SHALL NOT BE PLACED CONTINUOUS OVER THE GIRDER BEAMS. TERMINATE AND START DECK ON EACH SIDE OF GIRDER BEAM AS SHOWN ON NOTE CS6 ON SHT SOO3.

NOTE 4: SEE NOTE CS8 ON SHEET SO03 FOR COMPOSITE FLOOR DECK INSTALLATION REQUIREMENTS.

2. T/ SLAB EL = 115' - 4"

- 3. T/ STEEL EL = 114' 9 1/2"
- 4. (#) SHOWN BY STL BEAM CALLOUT ON PLAN INDICATES NUMBER OF 3/4" \$\phi x 5" LONG HEADED STUDS (SEE GENERAL) NOTES FOR STUD LAYOUT ON BEAMS).
- BEAM CAMBER IS DESIGNATED AS "C=" FOR EACH BEAM REQUIRING CAMBER.
- BEAM END REACTIONS AS SHOWN ON PLAN ARE ULTIMATE
- REACTIONS (ALREADY FACTORED).
- ALL WIDE FLANGE MEMBERS SHALL BE CONNECTED TO THE SUPPORTING STRUCTURE AS DETAILED IN CONNECTION SCHEDULES ON SHEET S501. UNLESS SPECIFICALLY NOTED OTHERWISE ON PLAN(S), ANY FLOOR MEMBER SUPPORTING ANOTHER FLOOR MEMBER SHALL BE CONNECTED AS DETAILED IN DOUBLE SHEAR SCHEDULE M1/S501. SINGLE SHEAR CONNECTIONS AS DETAILED IN SCHEDULES F1/S501 AND MT/S501 SHALL ONLY BE USED FOR FLOOR MEMBERS SUPPORTING DECK/SLAB ONLY (I.E. FILLER BEAMS) OR AS SPECIFICALLY IDENTIFIED ON PLAN OR SECTION.

BEAMS AND DECK HAVE BEEN DESIGNED TO BE UNSHORED.

NOTES TO GC & OWNER:

SELECTION OF AN AISC CERTIFIED FABRICATOR IS HIGHLY RECOMMENDED (SEE NOTE 4 BELOW FOR ADDITIONAL REQUIREMENTS IF A NON-AISC FABRICATOR IS SELECTED).

ALL STEEL FABRICATION SHALL MEET AISC TOLERANCES AND STANDARD PRACTICE GUIDELINES.

- A CAMBER REPORT SHALL BE PROVIDED FOR REVIEW BY ALL APPLICABLE PARTIES. THIS REPORT SHALL TABULATE BEAM MARK, BEAM SIZE, SPECIFIED CAMBER AND INDUCED CAMBER. THIS REPORT SHALL BE PREPARED AFTER THE STEEL HAS COMPLETED ALL PROCESSES OF FABRICATION AND IMMEDIATELY PRIOR TO TRANSPORTATION.
- IF A NON-AISC CERTIFIED FABRICATOR IS SELECTED, IN ADDITION TO ABOVE MENTIONED REQUIREMENTS THE OWNER SHALL HIRE A THIRD PARTY TESTING AND INSPECTION AGENCY TO WITNESS THE CAMBER RECORDING ACTIVITY AS MENTIONED ABOVE AND SHALL BE GIVEN THE AUTHORITY TO STOP SHIPMENT OF BEAMS THAT FAIL TO MEET THE SPECIFIED CAMBER PLUS OR MINUS (+/-) THE AISC TOLERANCES.
- AFTER ERECTION, BUT PRIOR TO PLACEMENT OF THE CONCRETE, CAMBER OF STEEL BEAMS SHALL BE MEASURED BY A REGISTERED SURVEYOR HIRED BY THE GC/CM AND REPORT SHALL BE PROVIDED FOR REVIEW BY ALL APPLICABLE PARTIES. MEASURED CAMBER SHALL BE REPORTED IN INCHES. IF THE MEASURED CAMBER IS LESS THAN 75% OF SPECIFIED CAMBER IN CONSTRUCTION DOCUMENTS, THE ENGINEER OF RECORD SHALL REQUIRE THE GC/CM TO PLACE SHORES UNDER BEAMS AT QUARTER SPAN POINTS.

ALL STRUCTURAL STEEL CONNECTIONS TO EMBED PLATES SHALL BE "FINGER TIGHT" WITH PEENED THREADS. ALL STRUCTURAL STEEL CONNECTIONS TO OTHER STRUCTURAL STEEL MEMBERS SHALL BE "SNUG TIGHT". NO BOLTED CONNECTION SHALL BE FULLY TENSIONED UNLESS SLIP CRITICAL (SC) BOLTS ARE SPECIFICALLY INDICATED. ADDITIONALLY, FIELD MELDING OF ANY BOLTED CONNECTION IS STRICTLY PROHIBITED UNLESS WRITTEN ACCEPTANCE IS PROVIDED BY BBM.

FLOOR DESIGN LOADS

DEAD (PSF)	LIVE (PSF)	OTHER()					
73 TYP (UNO)	CORRIDOR = 80 CLASSROOMS = 65 (INCLUDES PARTITIONS = 15)	STAIRS & EXIT WAYS / WALKWAYS = 100 MECHANICAL = 150					
NOTES: 1. THE DESIGN LOADS DO NOT INCLUDE THE SELF WEIGHT OF THE STEEL BEAMS & GIRDERS. THEY ARE ACCOUNTED FOR IN THE							

NOTES TO GC:

ANALYSIS ON AN INDIVIDUAL BASIS.

NO CONDUIT OF ANY KIND SHALL BE PERMITTED TO RUN HORIZONTALLY WITHIN THE SLAB. CONDUIT MUST BE RUN BELOW THE STRUCTURAL MEMBERS AND COME UP VERTICALLY THROUGH SLAB WHERE NECESSARY.

- . NO CONDUIT SHALL BE PERMITTED TO RUN THROUGH, EITHER HORIZONTALLY OR VERTICALLY, ANY STRUCTURAL
- 3. VERTICAL PENETRATIONS THROUGH THE SLAB, WHERE PERMITTED, MUST BE SLEEVED.
- . PLACEMENT OF ELEVATED CONCRETE SLABS ON METAL DECK SHALL NOT BE PERFORMED USING ANY SELF PROPELLED LASER SCREED EQUIPMENT. PLACE CONCRETE TO DESIGN THICKNESS USING "DIPSTICK/ STORY-POLE" METHOD ONLY.

FLOOR FRAMING PLAN KEY NOTES:

(2) #4x4'-0" LONG @ 3" OC PLACED 2" CLR FROM CORNER, CENTERED IN SLAB (TYP WHERE SHOWN).

TYPICAL BEAM TO PANEL CONN (UNO), SEE F1/S501. PRE-ENGINEERED LIGHT GAUGE TRUSSES @ 4'-0" OC MAX

BEAM CONN AT PANEL JOINT, SEE J4/5401.

(5) MECHANICAL MEZZANINE DESIGN LOADS: DL = SELF WT + 5 PSF MISC LL = 125 PSF

(6) SLAB DEPRESSION, REF ARCH DWGS FOR EXACT SIZE, LOCATION, AND DEPTH OF DEPRESSION.



Vero Classical School 51 Old Dixie Hwy, Vero Beach, FL 32962

VERO CLASSICAL CHRISTIAN SCHOOL -PHASE I

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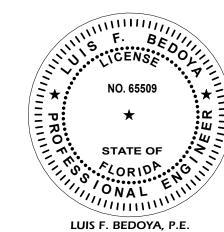


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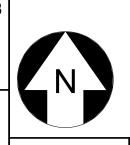


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AREA B AREA A

COMM. NO.: 2024117 ISSUE DATE: 12/23/2024 DRAWN BY: DB

OVERALL SECOND FLOOR FRAMING PLAN



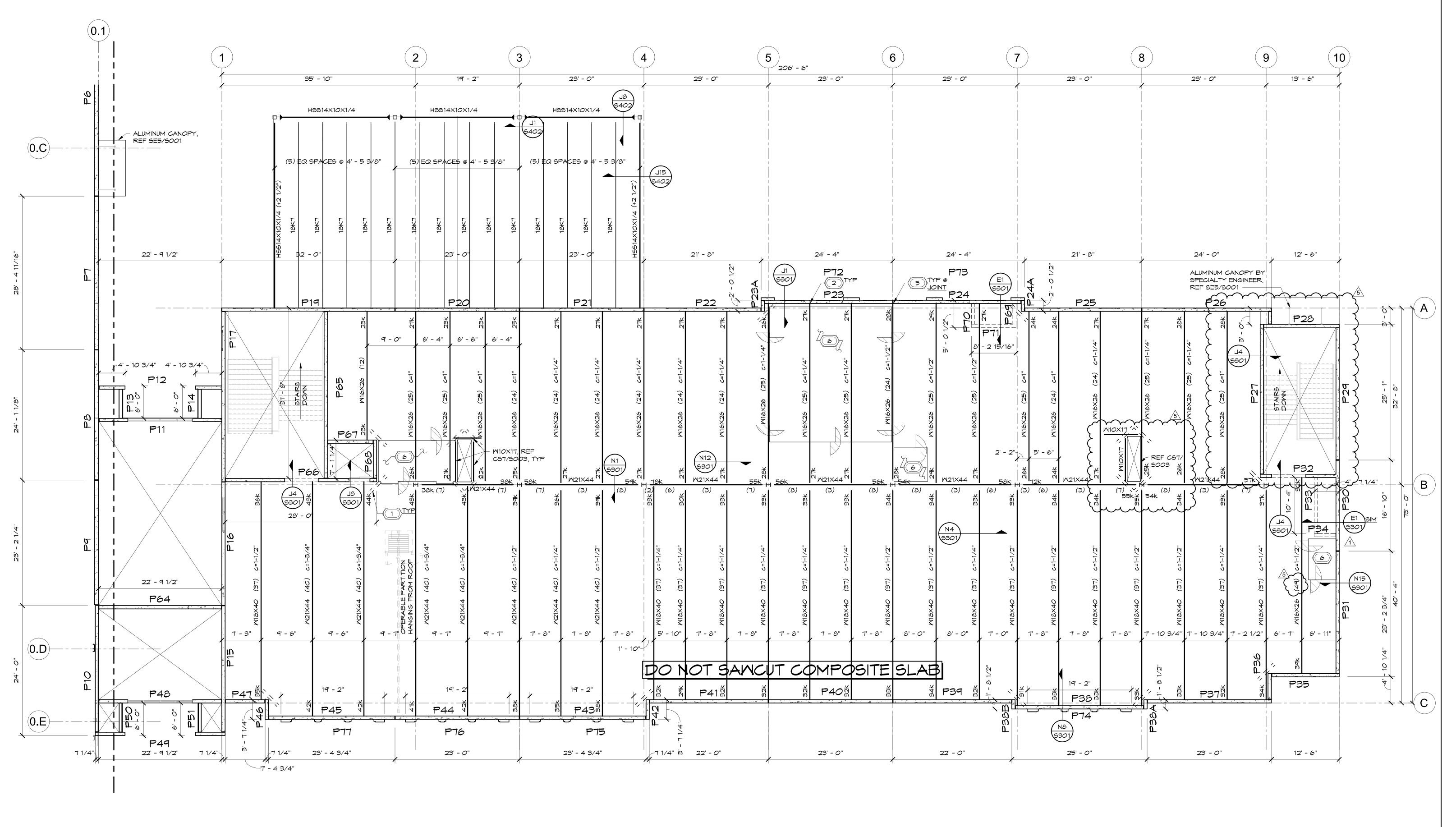
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OVERALL SECOND FLOOR FRAMING PLAN

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VERO CLASSICAL SCHOOL

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VERO CLASSICAL CHRISTIAN SCHOOL -PHASE I

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LUIS F. BEDOYA, P.E.
Florida Professional Engineer No. 6550

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 Addendum -01
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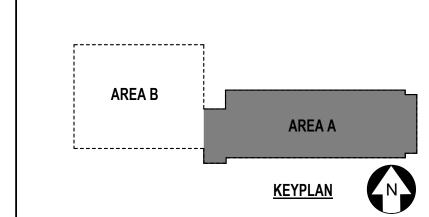
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 6/6/2025

FLOOR FRAMING PLAN NOTES:

1. FOR FLOOR FRAMING PLAN NOTES REF 5102.

FLOOR FRAMING PLAN KEY NOTES:

FOR FLOOR FRAMING PLAN KEY NOTES REF 5102.



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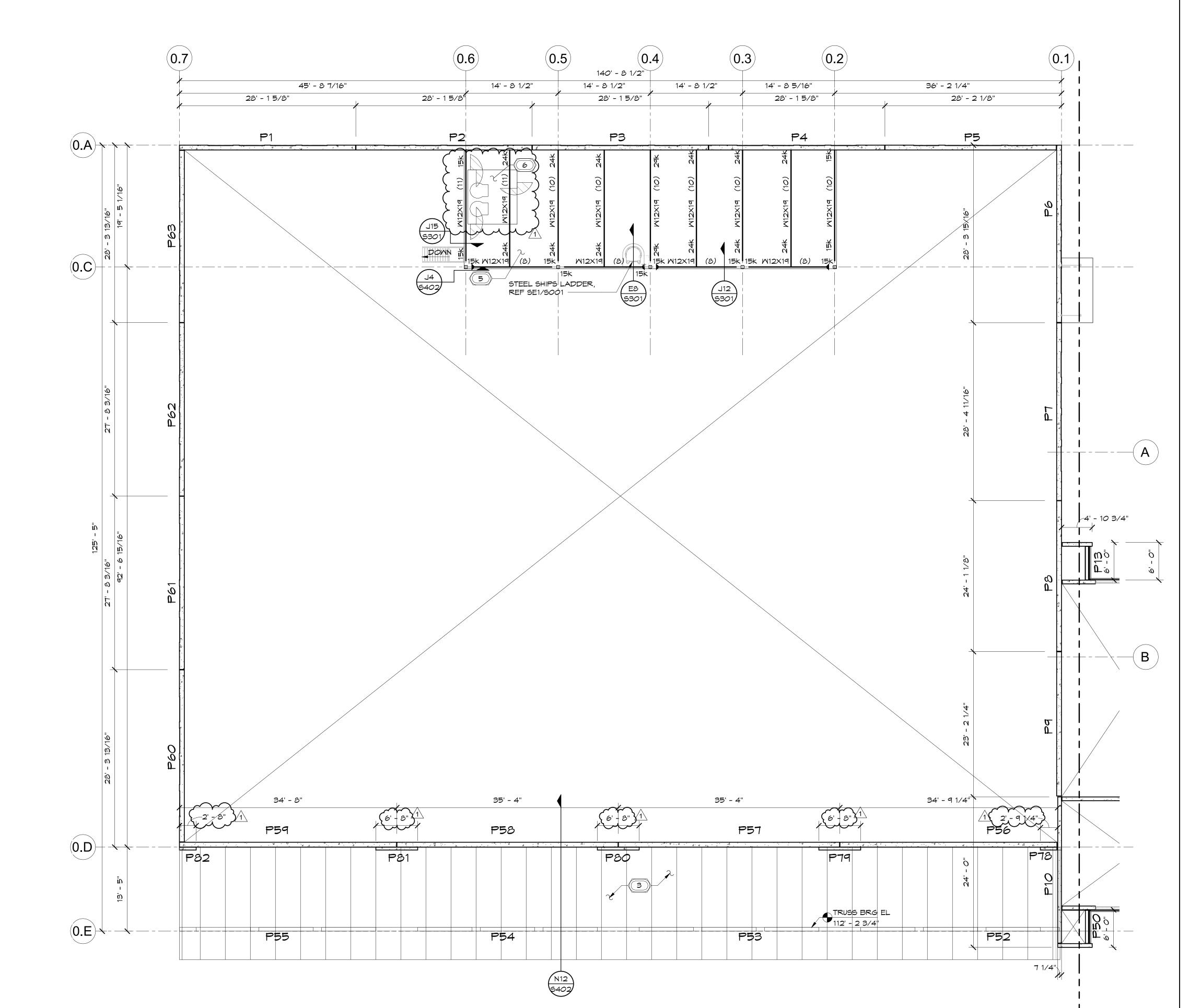
PARTIAL SECOND FLOOR FRAMING PLAN - AREA A

S102.A
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1/8" = 1'-0"

PARTIAL SECOND FLOOR FRAMING PLAN - AREA A

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8. ROOFING ON 1 1/2"-22 ga, TYPE "B" GALV G90 METAL DECK SECURED WITH #12 TEK SCREWS AT SUPPORTS AND #10 TEK SCREWS AT SIDELAPS ON LIGHT GAGE METAL MEMBERS. PROVIDE A 36/7 PATTERN AT SUPPORTS W/ SPACING AT EXTREME SIDES OF 6" OC. (3) SCREWS PER SPAN SHALL BE PROVIDED AT SIDELAPS. FLORIDA PRODUCT APPROVALS AND/OR NOTICE OF ACCEPTANCE TESTING DATA MAY REQUIRE A DIFFERENT ATTACHMENT PATTERN, DIFFERENT SIDELAP SPACING AND/OR DIFFERENT ATTACHMENT TYPE. PROVIDE THE MORE STRINGENT OF WHAT IS CALLED FOR HEREIN AND WHAT THE TESTING REQUIRES.

LIGHT GAGE TRUSS ROOF FRAMING PLAN NOTES:

1. ALL LIGHT STEEL TRUSSES SHALL BE DESIGNED AND CERTIFIED BY TRUSS MANUFACTURER'S REGISTERED ENGINEER. ALL TRUSS-TO-TRUSS & TRUSS-TO-STRUCTURE CONNECTIONS SHALL BE DESIGNED AND DETAILED BY THE TRUSS MFR. CONNECTIONS SHALL BE DESIGNED FOR DEAD, LIVE AND WIND (BOTH UPLIFT & LATERAL) LOADS. THE WIND PRESSURE AND SUCTION EXERTED ON THE WALLS MAY BE

3. SEE ARCHITECTURAL BUILDING SECTIONS, STRUCTURAL FRAMING PLANS AND TRUSS ELEVATIONS FOR ROOF

6. REFER TO ARCHITECTURAL DRAWINGS FOR FRAMED-

5. TRUSS SPACING SHALL BE MAX 4'-0" OC (UNO).

2. THE TRUSS MANUFACTURER SHALL VERIFY ALL DIMENSIONS AND SUBMIT SHOP DRAWINGS TO ARCHITECT FOR APPROVAL

4. SEE ARCHITECTURAL BUILDING SECTIONS, WALL SECTIONS AND STRUCTURAL FRAMING PLAN(S) FOR BEARING HEIGHTS.

DOWN CEILINGS, VOLUME CEILINGS AND OTHER INTERIOR

7. SEE PRE-FABRICATED LIGHT STEEL TRUSS NOTES IN GENERAL NOTE SHEETS FOR BRIDGING REQUIREMENTS FOR METAL

FOUND IN THE WIND SCHEDULES.

PITCHES (TYP).

TREATMENTS.

NOTE TO GC:

ALL TEMPORARY AND PERMANENT BRACING FOR THE TRUSS SYSTEM SHALL BE DESIGNED BY A SPECIALTY ENGINEER. PROVIDE SIGNED AND SEALED SHOP DRAWINGS FOR REVIEW AND APPROVAL.

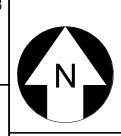
TRUSS DESIGN BASIS						
DEAD LOA	AD (PSF)	LIVE LOAD (PSF)				
TOP CHORD	BOTTOM CHORD	TOP CHORD	BOTTOM CHORD			
25	10	20 REDUCIBLE	0			

FLOOR FRAMING PLAN NOTES:

1. FOR FLOOR FRAMING PLAN NOTES REF 5102. 2. T/ MEZZANINE SLAB EL = 115' - 0" T/ STEEL EL = 114' 5 1/2"

FLOOR FRAMING PLAN KEY NOTES:

FOR FLOOR FRAMING PLAN KEY NOTES REF S102.



1/8" = 1'-0"

PARTIAL SECOND FLOOR FRAMING PLAN - AREA B

1/24/2025 1:57:55 PM Autodesk Docs://2024117 Vero Classical School/2024117-VERO CLASSICAL SCHOOL-ST-R2025.rvt

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AREA B AREA A

VERO CLASSICAL SCHOOL

Vero Classical School

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Winter Park, FL 32789

voice 407-872-3322

VERO BEACH, FLORIDA

51 Old Dixie Hwy, Vero Beach, FL 32962

VERO CLASSICAL

CHRISTIAN SCHOOL -

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BBM PROJECT #24133

LUIS F. BEDOYA, P.E.

Florida Professional Engineer No. 65509

REVISIONS

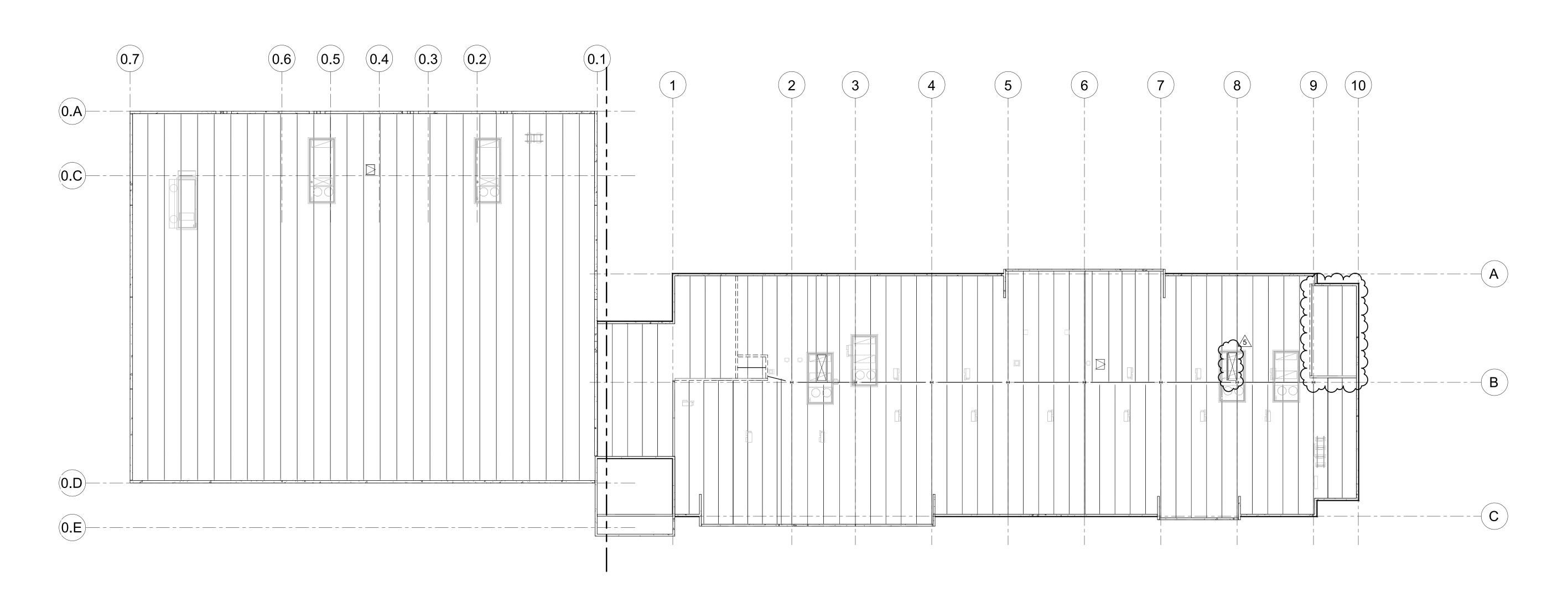
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PARTIAL SECOND FLOOR FRAMING PLAN - AREA B



ROOF FRAMING PLAN NOTES:

1. CELLULAR LIGHTWEIGHT INSULATING CONCRETE ROOFING ON 1 1/2"-18 ga, TYPE "B" GALV G90 METAL DECK WITH VENTING HOLES, AS REQUIRED BY FBC SECTIONS 1917.4.1 AND 1917.4.1.1 SECURED WITH HILTI POWDER ACTUATED FASTENERS AT SUPPORTS AND #10 TEK SCREMS AT SIDELAPS ON OPEN WEB BAR JOISTS AND/OR STRUCTURAL STEEL MEMBERS. USE X-HSN-24 FOR STEEL BAR JOISTS TOP CHORD OR FLANGE THICKNESS 1/8" TO 3/8", AND X-ENP-19 L15 FOR STRUCTURAL STEEL AND STEEL BAR JOISTS WITH TOP CHORD OR FLANGE THICKNESS 1/4" OR THICKER. PROVIDE A 36/7 PATTERN AT SUPPORTS WITH SPACING AT EXTREME SIDES OF 6"OC. (7) SCREWS PER SPAN SHALL BE PROVIDED AT SIDELAPS. FLORIDA PRODUCT APPROVALS AND/OR NOTICE OF ACCEPTANCE TESTING DATA MAY REQUIRE A DIFFERENT ATTACHMENT PATTERN, DIFFERENT SIDELAP SPACING AND/OR DIFFERENT ATTACHMENT TYPE(S). PROVIDE THE MORE STRINGENT OF WHAT IS CALLED FOR HEREIN AND WHAT THE TESTED ASSEMBLY REQUIRES.

GC OPTION TO SUBSTITUTE 5/8" PUDDLE WELDS IN LIEU OF HILTI FASTENERS. PROVIDE WELD WASHERS IF REQUIRED BY PRODUCT APPROVAL DOCUMENTATION OR ANY OTHER GOVERNING ENTITY.

- 2. SEE STRUCTURAL GENERAL NOTES FOR BRIDGING REQUIREMENTS FOR OPEN-WEB STEEL JOISTS.
- 3. JOISTS DESIGNATED "KCS" (K-SERIES CONSTANT SHEAR PER "VULCRAFT" TABLES) SHALL BE UTILIZED (IF REQ'D) AT CONCENTRATED LOADS (SEE FRAMING PLAN).
- 4. SEE FRAMING PLAN FOR JOIST BEARING ELEVATIONS.
- 5. REF: ARCH'L DWGS FOR INTERIOR DRAINS, SCUPPERS, CRICKETS AND SCUTTLES.
- 6. "BTJ" INDICATES BOLTED TIE JOIST PER OSHA REQUIREMENTS (TYP). SEE GENERAL NOTE SJ12 FOR ADDITIONAL INFORMATION.
- 7. COORDINATE SIZE AND LOCATION OF ROOFTOP UNITS WITH MECHANICAL DWGS.
- 8. BEAM END REACTIONS AS SHOWN ON PLAN ARE ULTIMATE REACTIONS (ALREADY FACTORED).
- 9. ALL ROOF DRAINS (WHERE REQUIRED) SHALL BE
- SUPPORTED BY A L3x3x1/4 ANGLE FRAME, TYP.
- 10. MASONRY BOND BEAMS AND TIE BEAMS THAT ARE INDICATED ON THE PLAN SHALL BE CONTINUOUS FOR THE ENTIRE LENGTH OF THE WALL, UNO. ELEVATIONS INDICATED ON PLAN ARE TO THE TOP OF BEAM, UNO.
- 11. ALL WIDE FLANGE MEMBERS SHALL BE CONNECTED TO THE SUPPORTING STRUCTURE AS DETAILED IN THE CONNECTION SCHEDULES ON SHEET S501. UNLESS SPECIFICALLY NOTED OTHERWISE ON PLAN, ALL ROOF MEMBERS SHALL BE CONNECTED AS DETAILED IN THE SINGLE SHEAR SCHEDULES F1/S501 AND M7/S501.

NOTE TO ERECTOR: ALL STRUCTURAL STEEL CONNECTIONS TO EMBED PLATES SHALL BE "FINGER TIGHT" WITH PEENED THREADS. ALL STRUCTURAL STEEL CONNECTIONS TO OTHER STRUCTURAL STEEL MEMBERS SHALL BE "SNUG TIGHT". NO BOLTED CONNECTION SHALL BE FULLY TENSIONED UNLESS SLIP CRITICAL (SC) BOLTS ARE SPECIFICALLY INDICATED. ADDITIONALLY, FIELD MELDING OF ANY BOLTED CONNECTION IS STRICTLY PROHIBITED UNLESS WRITTEN ACCEPTANCE IS

ROOF DESIGN LOADS DEAD LOAD LIVE LOAD 20 PSF 25 PSF (UNREDUCIBLE) . SEE TABLE ON SHEET 5004 FOR GROSS UPLIFT 2. SEE PLAN FOR CONCENTRATED LOADS. 3. ROOF STRUCTURE HAS BEEN DESIGNED TO SUPPORT THE WEIGHT OF PONDED WATER CAUSED BY CLOGGING OF THE PRIMARY ROOF DRAINS

ROOF LEGEND:

PROVIDED BY BBM.

BTJ - INDICATES BOLTED TIE JOIST. SEE DETAIL ON SHEET S401 FOR INFO.

ROOF FRAMING PLAN KEY NOTES:

FOR A DEPTH OF (4) INCHES.

- RTU, COORD LOCATION W/ MECHANICAL, REF A8/5401.
- JOIST CONN AT PANEL JOINT, SEE A1/5401 & E4/5401.
- BASKETBALL BACKBOARD, COORD FRAMING REACTIONS W/ MANUFACTURER.

ROOF HATCH, COORD LOCATION W/ ARCHITECTURAL, REF RD7/5004.



Vero Classical School

PHASE I

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Winter Park, FL 32789

voice 407-872-3322

51 Old Dixie Hwy, Vero Beach, FL 32962

VERO CLASSICAL

CHRISTIAN SCHOOL -

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VERO BEACH, FLORIDA 32968

1/16" = 1'-0"

OVERALL ROOF FRAMING PLAN

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LUIS F. BEDOYA, P.E. Florida Professional Engineer No. 65509 REVISIONS

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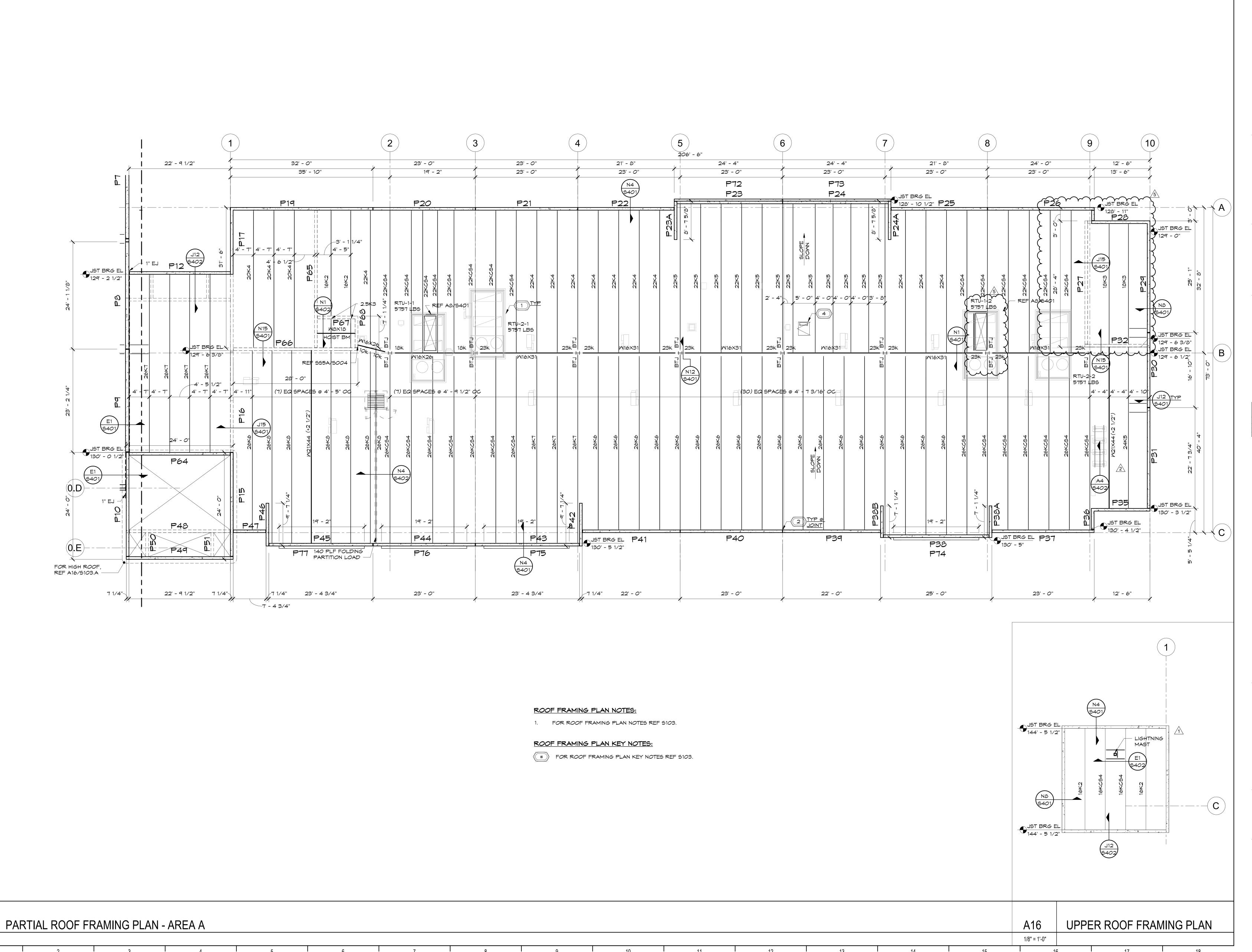
PLAN

ISSUE DATE: 12/23/2024

OVERALL ROOF FRAMING

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AREA A





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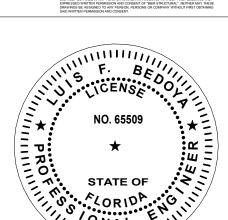
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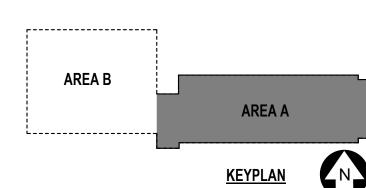


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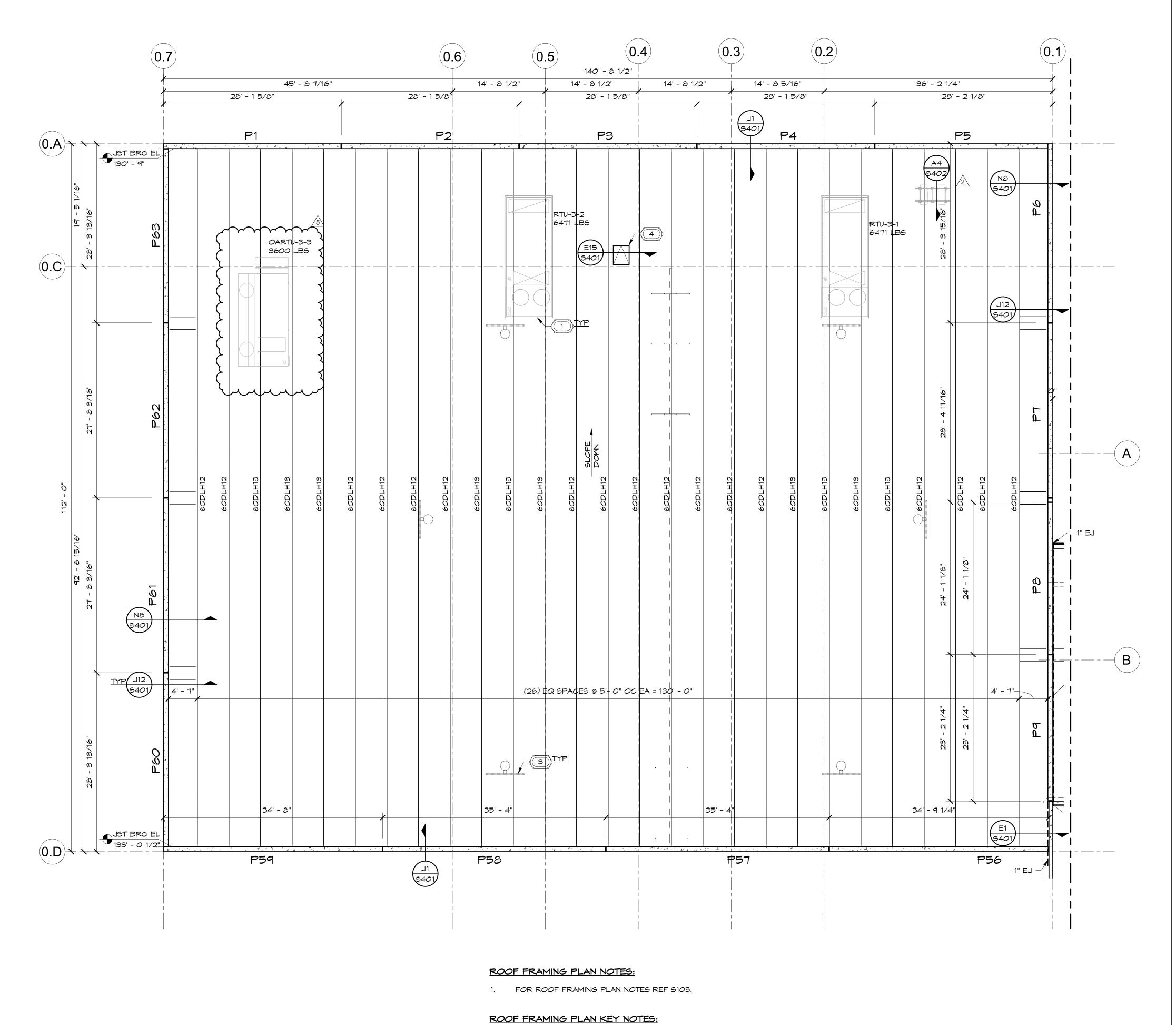
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PARTIAL ROOF FRAMING PLAN - AREA A

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1/8" = 1'-0"



FOR ROOF FRAMING PLAN KEY NOTES REF 5103.



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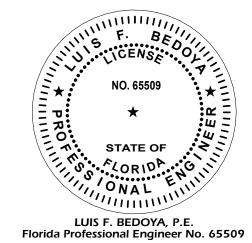
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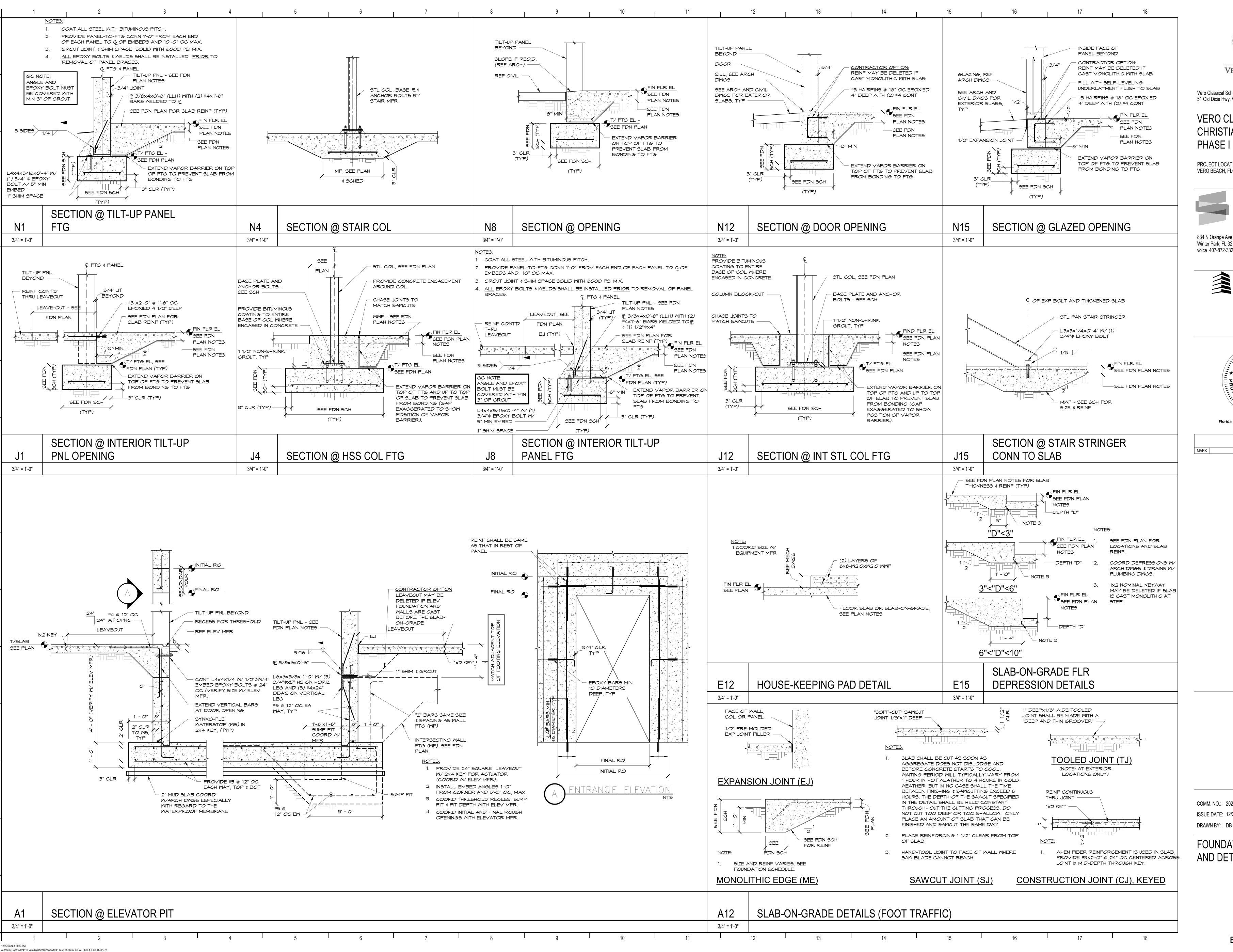
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PARTIAL ROOF FRAMING PLAN - AREA B

PARTIAL SECOND FLOOR FRAMING PLAN - AREA B 1/8" = 1'-0"





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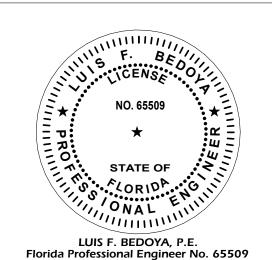


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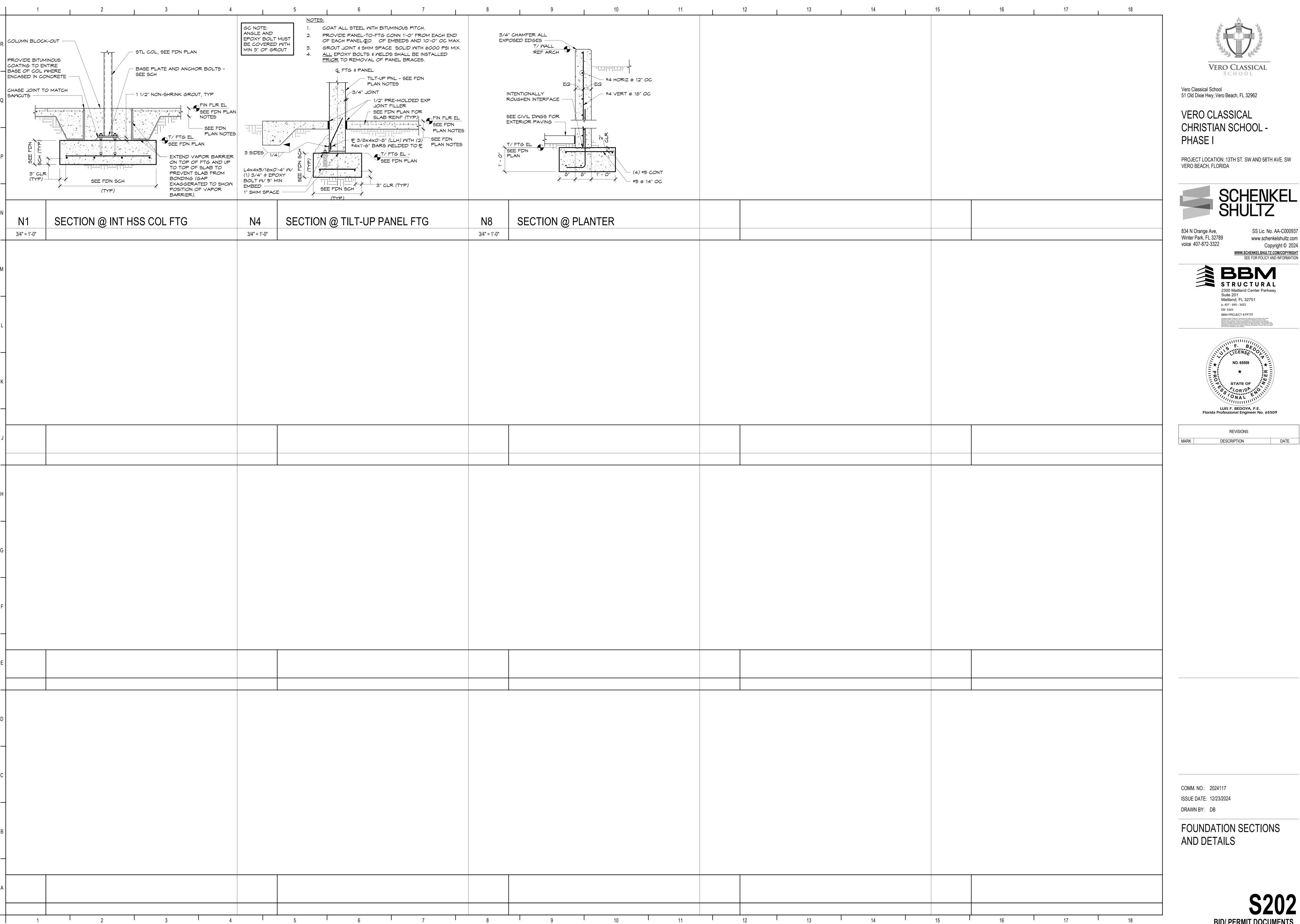




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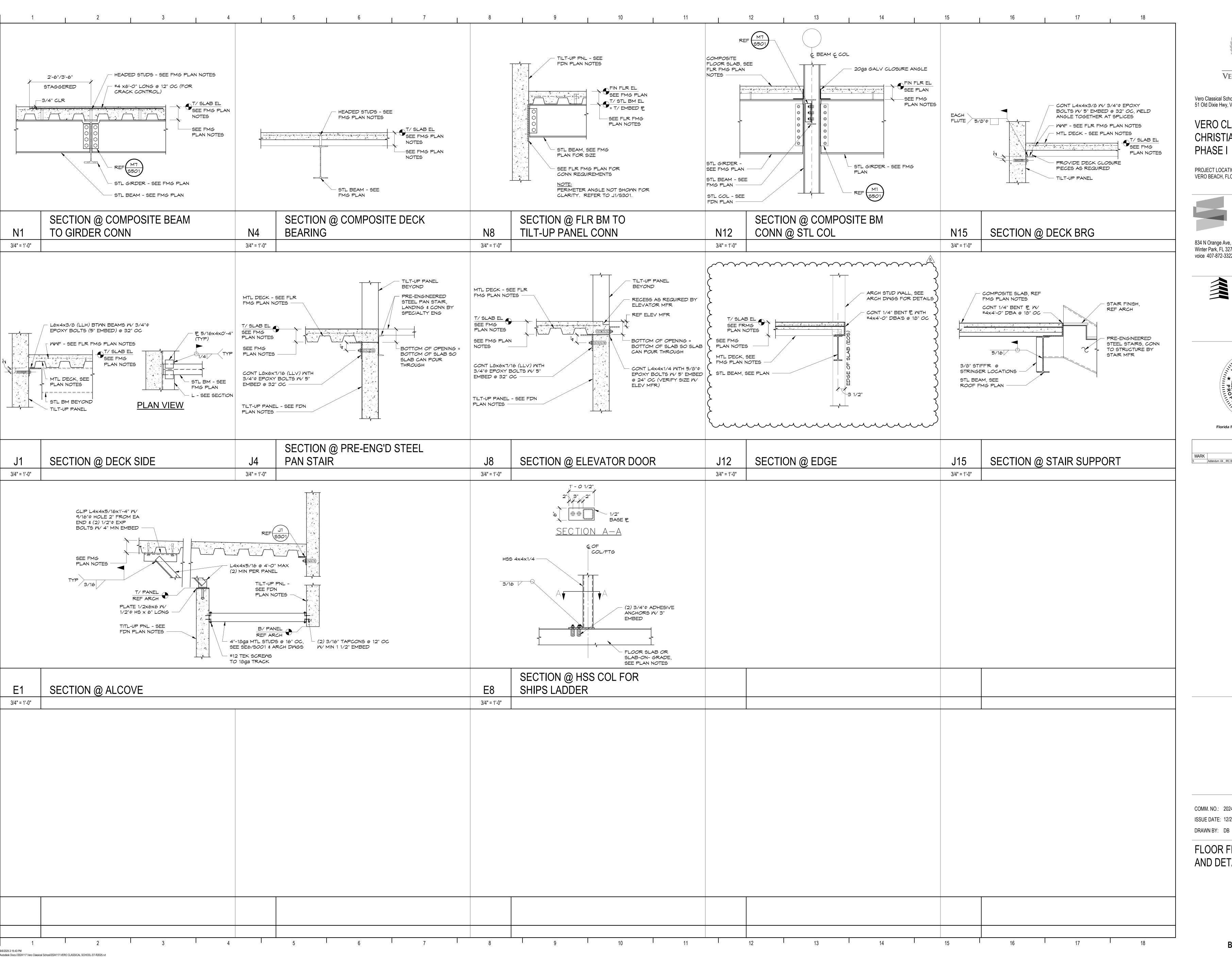
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FOUNDATION SECTIONS AND DETAILS



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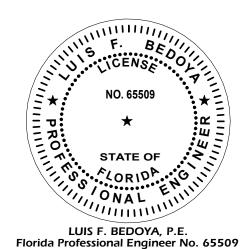
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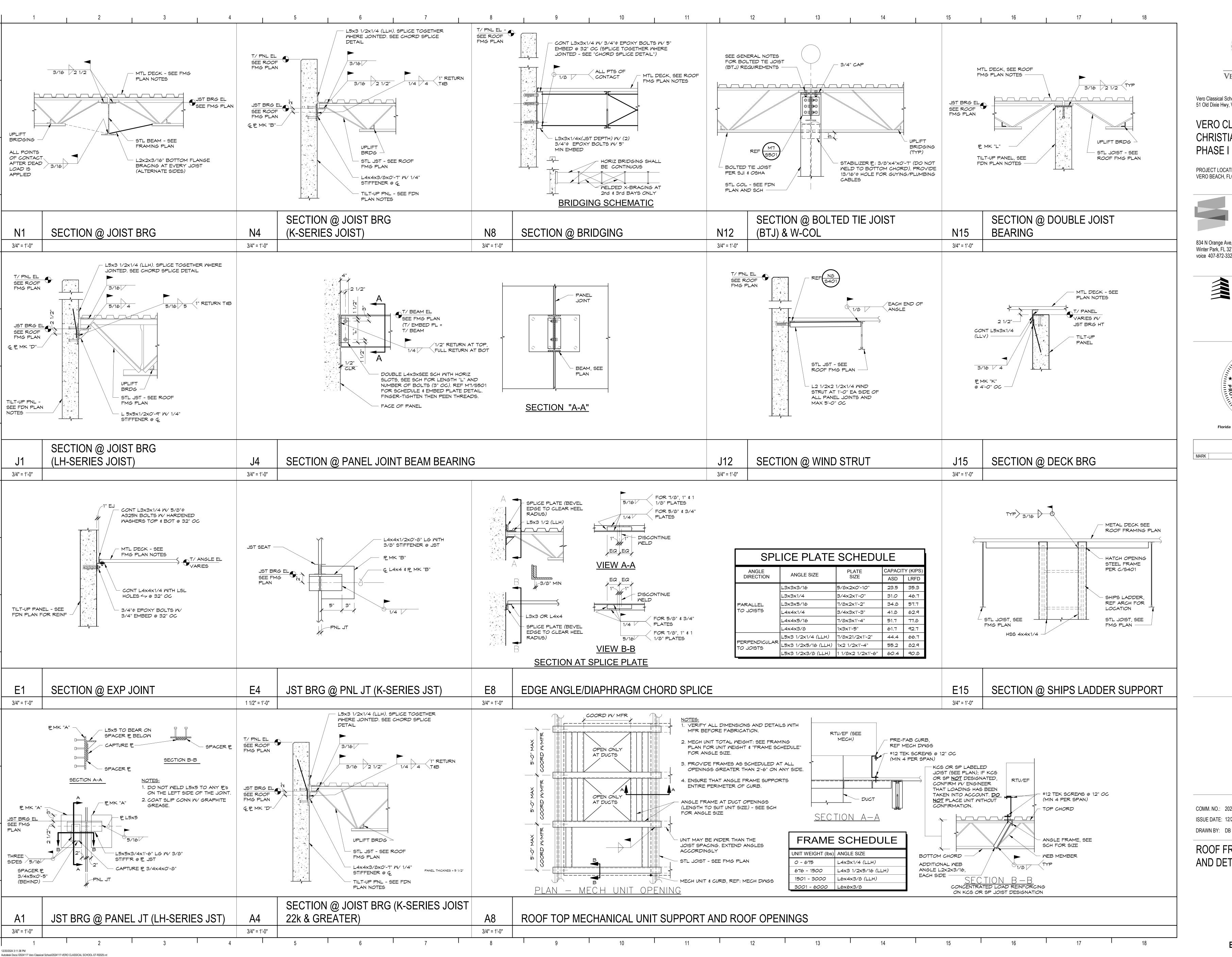




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FLOOR FRAMING SECTIONS AND DETAILS





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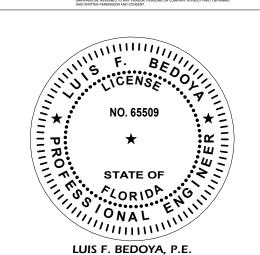
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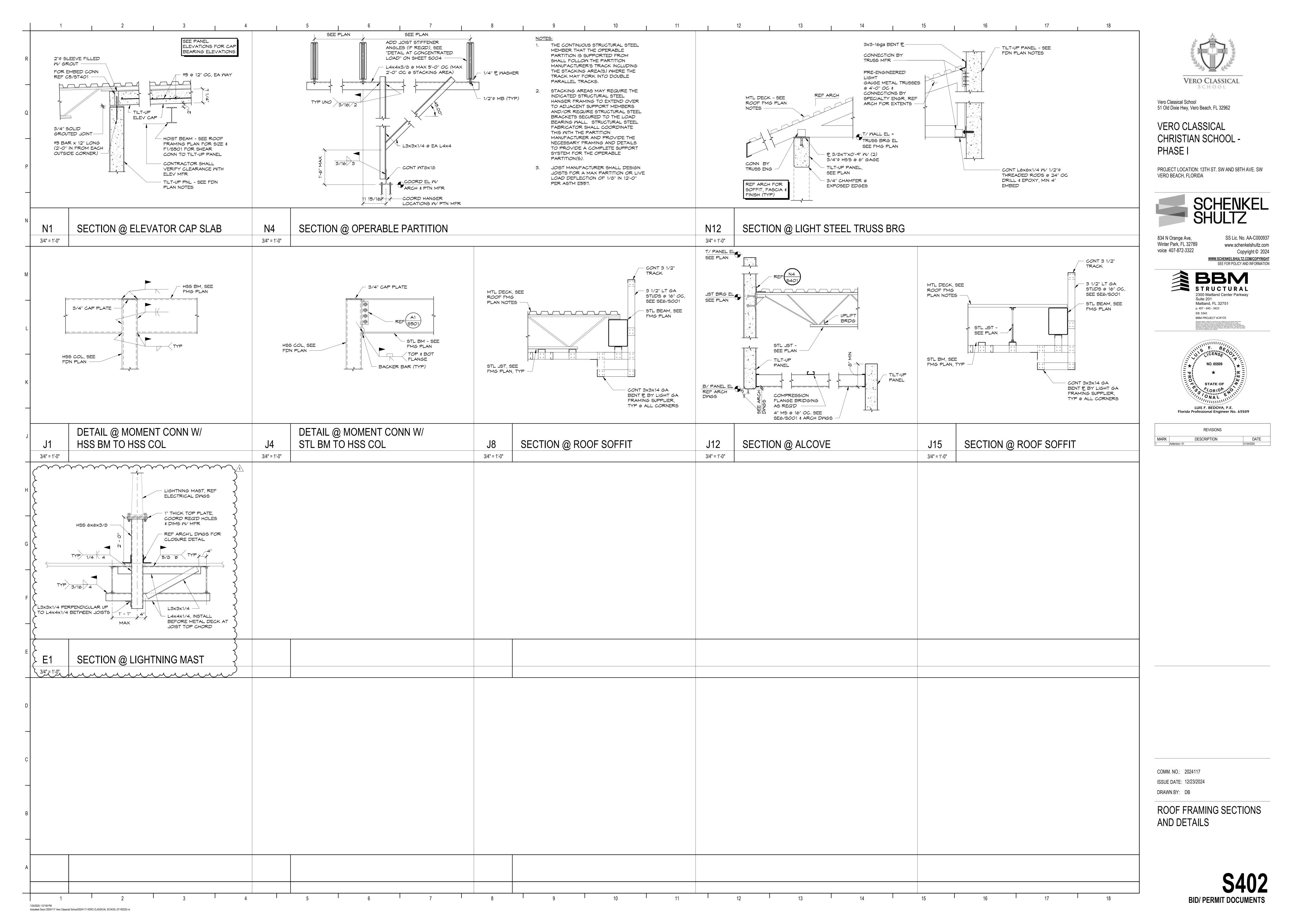
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ROOF FRAMING SECTIONS AND DETAILS

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THIS CONNECTION SHALL BE USED FOR GIRDER TO COLUMN CONNECTIONS ONLY. SUB-GIRDER TO GIRDER & SUB-GIRDER TO SUB-GIRDER CONNECTIONS SHALL BE DESIGNED BY THE STEEL FABRICATOR BASED ON THE REACTIONS SHOWN ON PLAN. THIS SCHEDULE ASSUMES GIRDER IS 2 1/4"-

1/2"___1 3/4"`

SEE FMG PLAN

- DOUBLE L4x3x5/16 WITH

STD HOLES. SEE SCH FOR

LENGTH "L" AND NUMBER

*O*F B*O*LTS (3" *O*C)

- FLANGE OF COLUMN

SHEAR CONNECTION SCHEDULE						
BEAM SIZE SEE PLAN)	NUMBER OF 3/4" DIA A325N BOLTS	ANGLE LENGTH "L"	MAX ULTIMATE END REACTION (KIPS)			
M8, M10	2	5 1/2"	29.8			
M12, M14	э	8 1/2"	52.6			
W16	4	11 1/2"	87.7			
M18	n	14 1/2"	131.7			

NOTES:

FOR BEAMS NOT SHOWN HEREIN, FABRICATOR SHALL DESIGN THE SHEAR CONNECTION BASED ON THE REACTION SHOWN ON THE PLAN.

184.1

ANGLE MATERIAL SHALL BE ASTM A36. BEAM MATERIAL SHALL BE 50 KSI.

6 | 17 1/2"

- FABRICATOR SHALL CHECK BEAM WEB TEAR-OUT (BLOCK SHEAR) IF BEAM IS COPED.
- PROVIDE SHORT SLOTTED HOLES IN BEAM WEB. BOLTS SHALL BE INSTALLED "SNUG-TIGHT".

2 1/4"	STL BEAM T/ BEAM EL SEE FMG PLAN	
1/2" 13/4"		/2" RETURN AT TO FULL RETURN AT E
	- SINGLE L4x3x3/8 WITH H SHORT SLOTS. SEE SCHE LENGTH AND NUMBER OF (3" OC)	DULE FOR
	- WEB OF GIRDER / COLUN	MN

MAX 3 1/2" FOR M8 & MAX

6" FOR ALL OTHERS

SHEAR CONNECTION SCHEDULE				
BEAM SIZE (SEE PLAN)	NUMBER OF 3/4" DIA A325N BOLTS	ANGLE LENGTH "L"	MAX DEPTH OF COPE "dc"	MAX ULTIMATE END REACTION (KIPS)
M8, M10	2	5 1/2"	1 1/8"	31.8
W12	3	8 1/2"	1 1/4"	47.7
W14, W16	4	11 1/2"	1 1/2"	63.6
M18	5	14 1/2"	1 1/2"	79.5
M21	6	17 1/2"	1 1/2"	95.4
NOTE				

- FOR BEAMS NOT SHOWN HEREIN, FABRICATOR SHALL DESIGN THE SHEAR CONNECTION BASED ON THE REACTION SHOWN ON THE PLAN. ANGLE MATERIAL SHALL BE ASTM A36.
- BEAM MATERIAL SHALL BE 50 KSI AND MAXIMUM MEB THICKNESS SHALL BE 1/2". END REACTIONS ARE NOT VALID FOR BEAMS WITH
- MEB THICKNESS GREATER THAN 1/2". FABRICATOR SHALL VERIFY THAT CONNECTION IS ADEQUATE IF BEAM
- PROVIDE WASHER OVER SLOTTED HOLES.

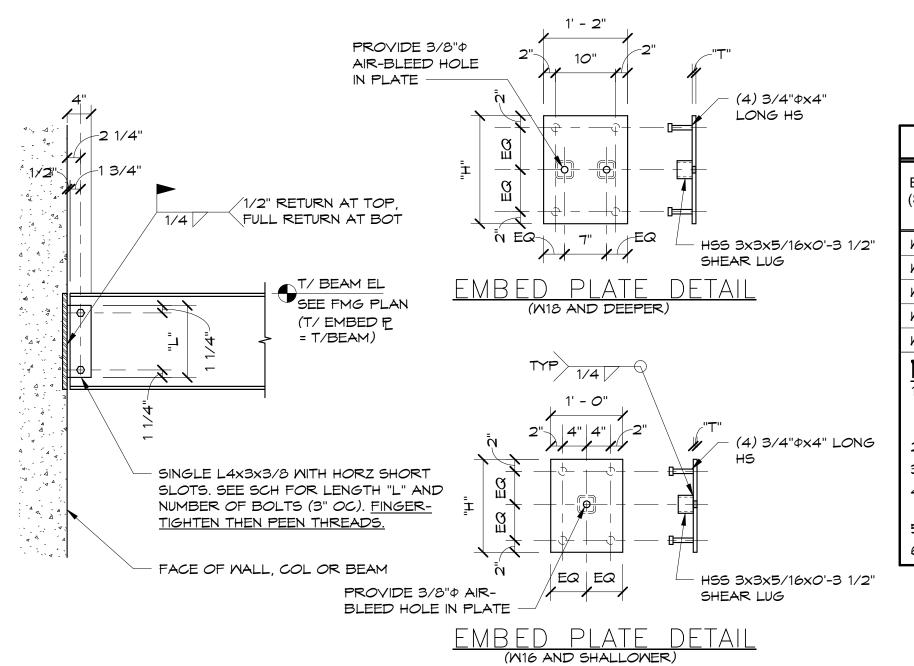
IS COPED MORE THAN INDICATED.

WHERE BEAMS ARE ON EACH SIDE OF A GIRDER OR COLUMN WEB. THE ANGLES MUST BE STAGGERED.

BOLTS SHALL BE INSTALLED "SNUG-TIGHT".

TYP FLOOR GIRDER TO COL CONN (DOUBLE SHEAR) NOT TO SCALE NOT TO SCALE

TYP FLOOR & ROOF BEAM TO GIRDER / COLUMN CONN (SINGLE SHEAR)



BEAM SIZE	NUMBER OF 3/4" DIA	ANGLE LENGTH	EMBE	D PLATE	MAX UL END RE	TIMATE ACTION
(SEE PLAN)	A325N BOLTS	"L"	HEIGHT "H"	THICKNESS	(KIF 3000 PSI	PS) 4000 PSI
M8, M10	2	5 1/2"	10"	1/2"	29.4	31.8
M12	3	8 1/2"	14"	1/2"	29.4	39.2
M14, M16	4	11 1/2"	16"	5/8"	32.1	42.8
M18	5	14 1/2"	18"	5/8"	50.8	67.8
M21	6	17 1/2"	21"	3/4"	53.5	71.4

- THE SHEAR CONNECTION BASED ON THE REACTION SHOWN ON THE PLAN.
- PLATE AND ANGLE MATERIAL SHALL BE ASTM A36. BEAM MATERIAL SHALL BE 50 KSI.
- FABRICATOR SHALL CHECK BEAM MEB TEAR-OUT (BLOCK SHEAR) IF BEAM IS COPED.
- PROVIDE SHORT SLOTTED HOLES IN BEAM WEB. INSTALL BOLTS FINDER-TIGHT THEN PEEN THREADS.

SHEAR CONNECTION SCHEDULE MARK C1 W10x33 C2 M10x39 C3 HSS 6x6x1/4 C4 HSS 10×10×1/2

- ANCHOR RODS SHALL BE ASTM F1554 (GRADE 36) UNO, THREADED EACH END WITH NUT AT BOTTOM. PLATE WASHER AT BOTTOM NUT IS NOT REQ'D. INCREASE FOOTING THICKNESS AT ANCHOR RODS TO PROVIDE MIN 3" COVER. SEE NOTE BELOW IN ANCHOR ROD DETAIL.
- PLATE WASHERS SHALL HAVE HOLE WITH DIAMETER EQUAL TO ROD DIAMETER + 1/16". CIRCULAR OR SQUARE WASHERS ARE ACCEPTABLE.

COLUMN & BASE PLATE SCHEDULE

1 1/4 9 3/4

1/2 9 3/4

ANCHOR ROD

(INCHES)

COMMENTS

VERO CLASSICAL

SCHOOL

PROJECT LOCATION: 13TH ST. SW AND 58TH AVE. SW

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BBM

STRUCTURAL 2300 Maitland Center Parkway

Suite 201

EB: 5343

Maitland, FL 32751

BBM PROJECT #24133

LUIS F. BEDOYA, P.E.

REVISIONS

DESCRIPTION

p: 407 - 645 - 3423

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DATE

Vero Classical School

PHASE I

VERO BEACH, FLORIDA

834 N Orange Ave,

Winter Park, FL 32789

voice 407-872-3322

51 Old Dixie Hwy, Vero Beach, FL 32962

VERO CLASSICAL

CHRISTIAN SCHOOL -

BASE PLATE SIZE

(INCHES)

SIZE

NOTES:

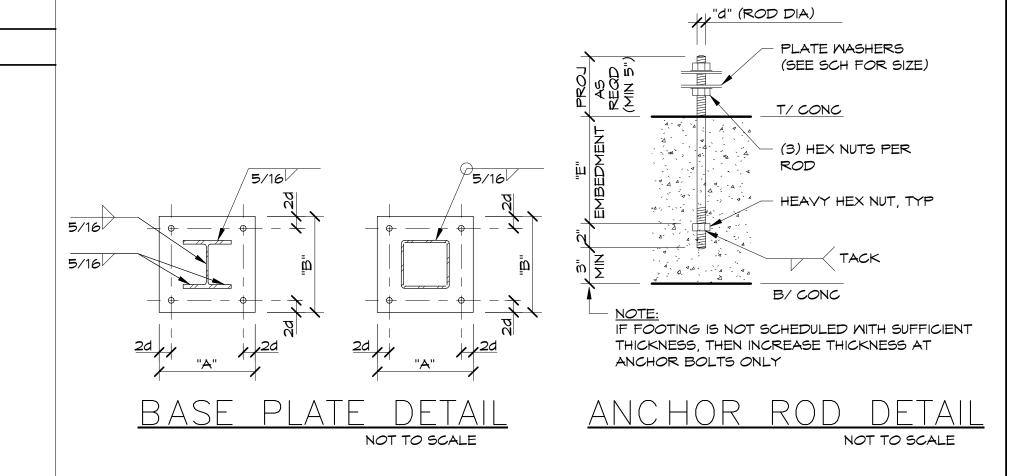
. IT SHALL BE ACCEPTABLE TO PROVIDE A HEAVY HEX HEAD BOLT IN LIEU OF THE THREADED ROD WITH HEAVY HEX NUT AT BOTTOM.

16 | 16 | 1 1/4 | 9 | 3/4

12 12 3/4 9 3/4

OVERSIZE HOLES AND PLATE WASHERS FOR BASE PLATES

OVEROIZE HOLLO WIND I EMILE WATCHLING FOR DATE I EMILE					
ROD DIAMETER	HOLE DIAMETER	PLATE WASHER	ROD DIAMETER	HOLE DIAMETER	PLATE WASHER
3/4"	1 5/16"	1/4"x2"x2"	1 1/2"	2 3/8"	1/2"x4"
7/8"	1 9/16"	5/16"x2 1/2"x2 1/2"	1 3/4"	2 7/8"	5/8"×4 1/2"
1"	1 7/8"	3/8"x3"x3"	2"	3 1/4"	3/4"×5"
1 1/4"	2 1/8"	1/2"x3 1/2"x3 1/2"	2 1/2"	3 3/4"	7/8"x5 1/2"



COLUMN & BASE PLATE SCHEDULE

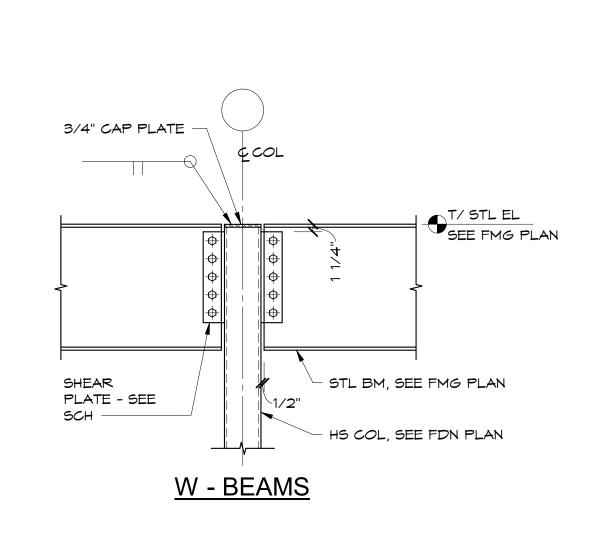
NOT TO SCALE

MARK	SIZE (L x W x D)	REINFORCING	REMARKS
F26	2'-6" × 2'-6" × 1'-0"	(3) #5 EA WAY, BOT	PAD FOOTING
F30	3'-0" × 3'-0" × 1'-0"	(3) #5 EA WAY, BOT	PAD FOOTING
F36	3'-6" × 3'-6" × 1'-0"	(4) #5 EA WAY, BOT	PAD FOOTING
F40	4'-0" × 4'-0" × 1'-0"	(4) #5 EA WAY, BOT	PAD FOOTING
F46	4'-6" × 4'-6" × 1'-0"	(4) #5 EA WAY, BOT	PAD FOOTING
F50	5'-0" × 5'-0" × 1'-0"	(5) #5 EA WAY, BOT	PAD FOOTING
F56	5'-6" × 5'-6" × 1'-1"	(6) #5 EA WAY, BOT	PAD FOOTING
F60	6'-0" x 6'-0" x 1'-2"	(7) #5 EA WAY, BOT	PAD FOOTING
F66	6'-6" x 6'-6" x 1'-2"	(7) #6 EA WAY, BOT	PAD FOOTING
F70	7'-0" × 7'-0" × 1'-4"	(8) #6 EA WAY, BOT	PAD FOOTING
F76	7'-6" × 7'-6" × 1'-4"	(9) #6 EA WAY, BOT	PAD FOOTING
F80	8'-0" x 8'-0" x 1'-5"	(8) #7 EA WAY, BOT	PAD FOOTING
F86	8'-6" × 8'-6" × 1'-6"	(7) #8 EA WAY, BOT	PAD FOOTING
MF20	CONT x 2'-0" x 1'-0"	(2) #5 CONT \$ #5 @ 48" TRANS, BOT	MALL FOOTING
MF26	CONT x 2'-6" x 1'-0"	(3) #5 CONT & #5 @ 48" TRANS, BOT	MALL FOOTING
MF30	CONT x 3'-0" x 1'-0"	(3) #5 CONT & #5 @ 24" TRANS, BOT	MALL FOOTING
MF36	CONT x 3'-6" x 1'-0"	(4) #5 CONT & #5 @ 14" TRANS, BOT	MALL FOOTING
MF40	CONT x 4'-0" x 1'-0"	(4) #5 CONT & #5 @ 12" TRANS, BOT	MALL FOOTING
WF46	CONT x 4'-6" x 1'-0"	(4) #5 CONT & #5 @ 12" TRANS, BOT	MALL FOOTING
MF50	CONT x 5'-0" x 1'-0"	(5) #5 CONT & #5 @ 10" TRANS, BOT	MALL FOOTING
WF56	CONT x 5'-6" x 1'-1"	(6) #5 CONT & #5 @ 10" TRANS, BOT	MALL FOOTING
MF60	CONT × 6'-0" × 1'-2"	(7) #5 CONT & #5 @ 8" TRANS, BOT	MALL FOOTING
VEC.		(0) #5 54 1014	
MF26	2'-6" × 2'-6" × 1'-0"	(3) #5 EA WAY, BOT	MONOLITHIC FOOTING
MWF20	CONT × 2'-0" × 1'-0"	(2) #5 CONT & #5 @ 48" TRANS, BOT	MONOLITHIC WALL FT
ME10	CONT × 1'-0" × 1'-4"	(1) #5 CONT, BOT	MONOLITHIC EDGE

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SCHEDULES

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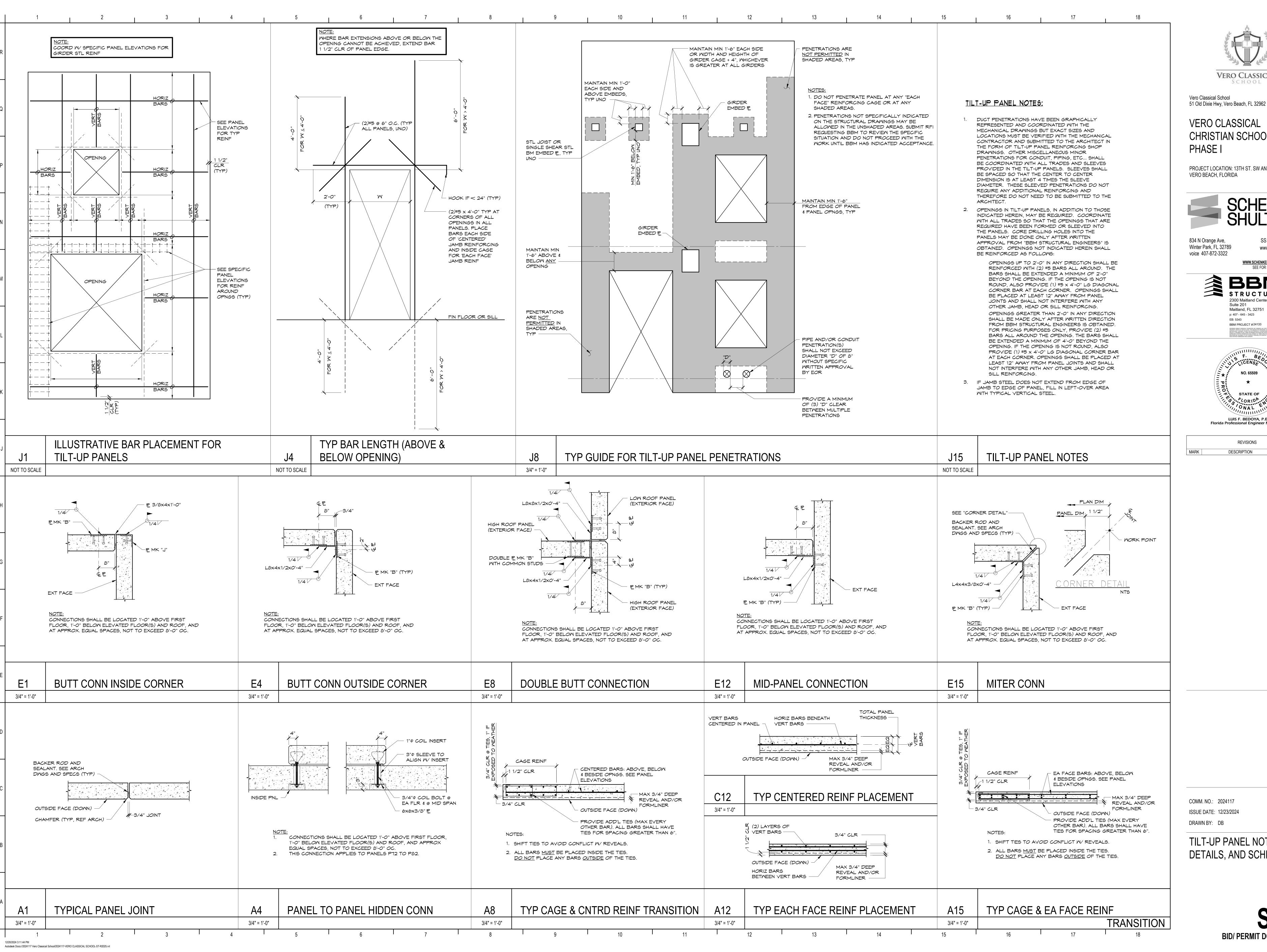
BEAM DEPTH	NUMBER OF PLATE 3/4" DIA LENGTH		FILLET WELD	CAPACITY (KIPS)		
(SEE PLAN)	A325N BOLTS	"L" (INCHES)	THICKNESS (36 KSI)	1 \ /1	ASD	LRFD
8, 10	2	5 1/2"	5/16	1/4	20.4	30.6
12, 14	3	8 1/2"	5/16	1/4	31.8	47.7
16	4	11 1/2"	5/16	1/4	42.4	63.6
18	5	14 1/2"	5/16	1/4	53. <i>0</i>	79.5
21	6	17 1/2"	5/16	1/4	63.6	95.4
24	7	20 1/2"	5/16	1/4	72.4	111.0
27	8	23 1/2"	5/16	1/4	84.7	127.0
30	9	26 1/2"	5/16	1/4	94.8	142.0
SEE SCH FOR NUMBER OF BOLTS (3" OC) SEE SCH SHEAR PLATE DETAIL			5HA THE 2. PRO 3. BEA 4. PRO 5. BOI	ALL DESIGN THE REACTION SHO OVIDE HORIZ SI AMS SHALL BE OVIDE HARDEN LTS SHALL BE	SHOWN HEREIN, THE E SHEAR CONNECTI OWN ON THE PLAN. LOTS IN SHEAR PLA 50 KSI. ED WASHER OVER INSTALLED "SNUG-1 END PLATE FOR H	ON BASED ON ATE. SLOTTED HOLI

TYPICAL BEAM TO HSS COL CONNECTION FOUNDATION SCHEDULE

12/20/2024 3:11:42 PM Autodesk Docs://2024117 Vero Classical School/2024117-VERO CLASSICAL SCHOOL-ST-R2025.rvt

3/4" = 1'-0"

3/4" = 1'-0"





Vero Classical School

VERO CLASSICAL CHRISTIAN SCHOOL -PHASE I

PROJECT LOCATION: 13TH ST. SW AND 58TH AVE. SW VERO BEACH, FLORIDA



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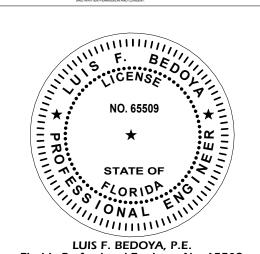
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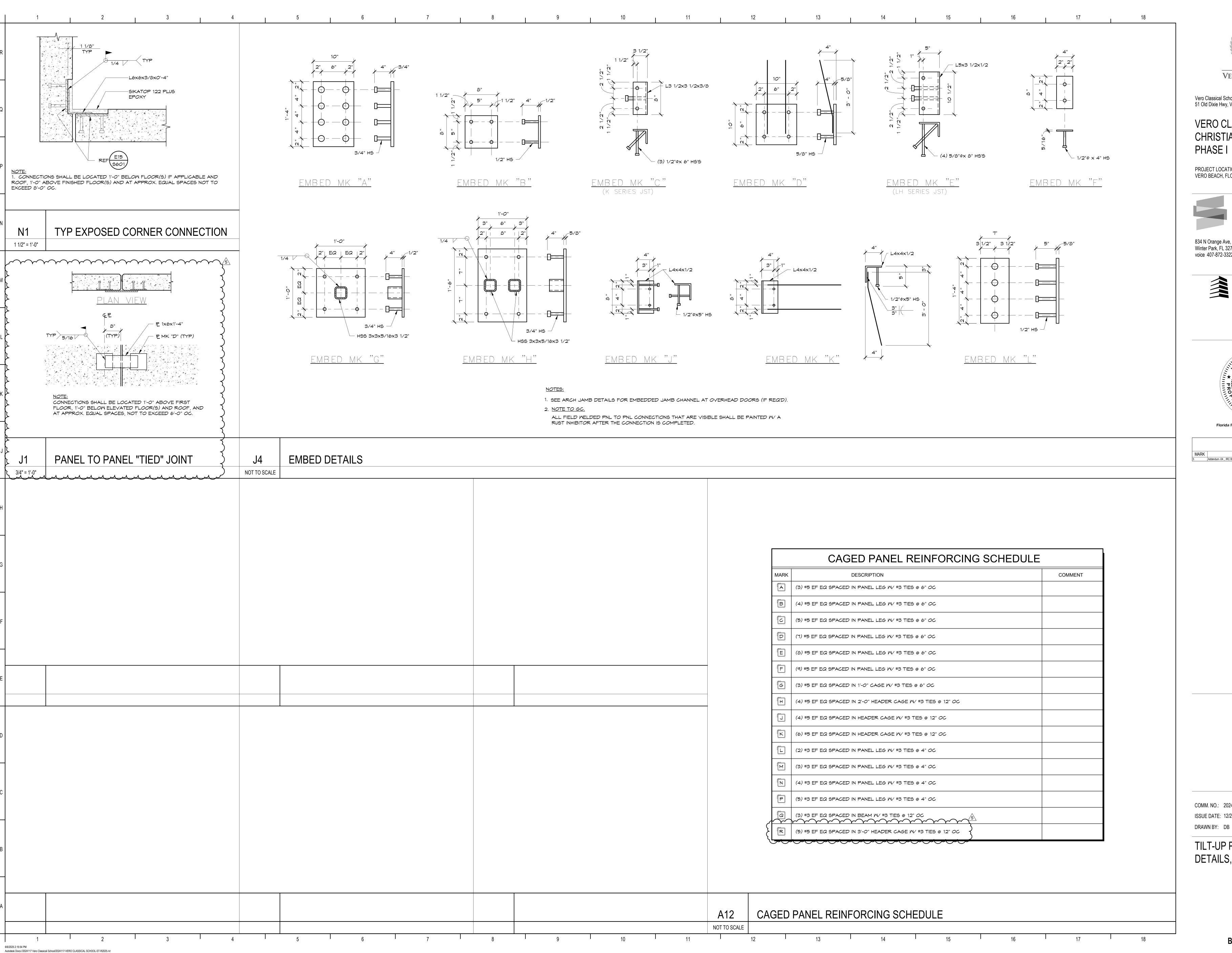
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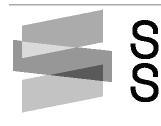
TILT-UP PANEL NOTES, DETAILS, AND SCHEDULES





VERO CLASSICAL CHRISTIAN SCHOOL -PHASE I

PROJECT LOCATION: 1275 58TH AVE. SW VERO BEACH, FLORIDA 32968



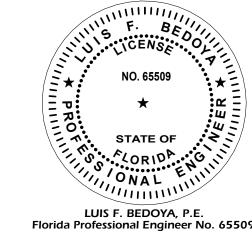
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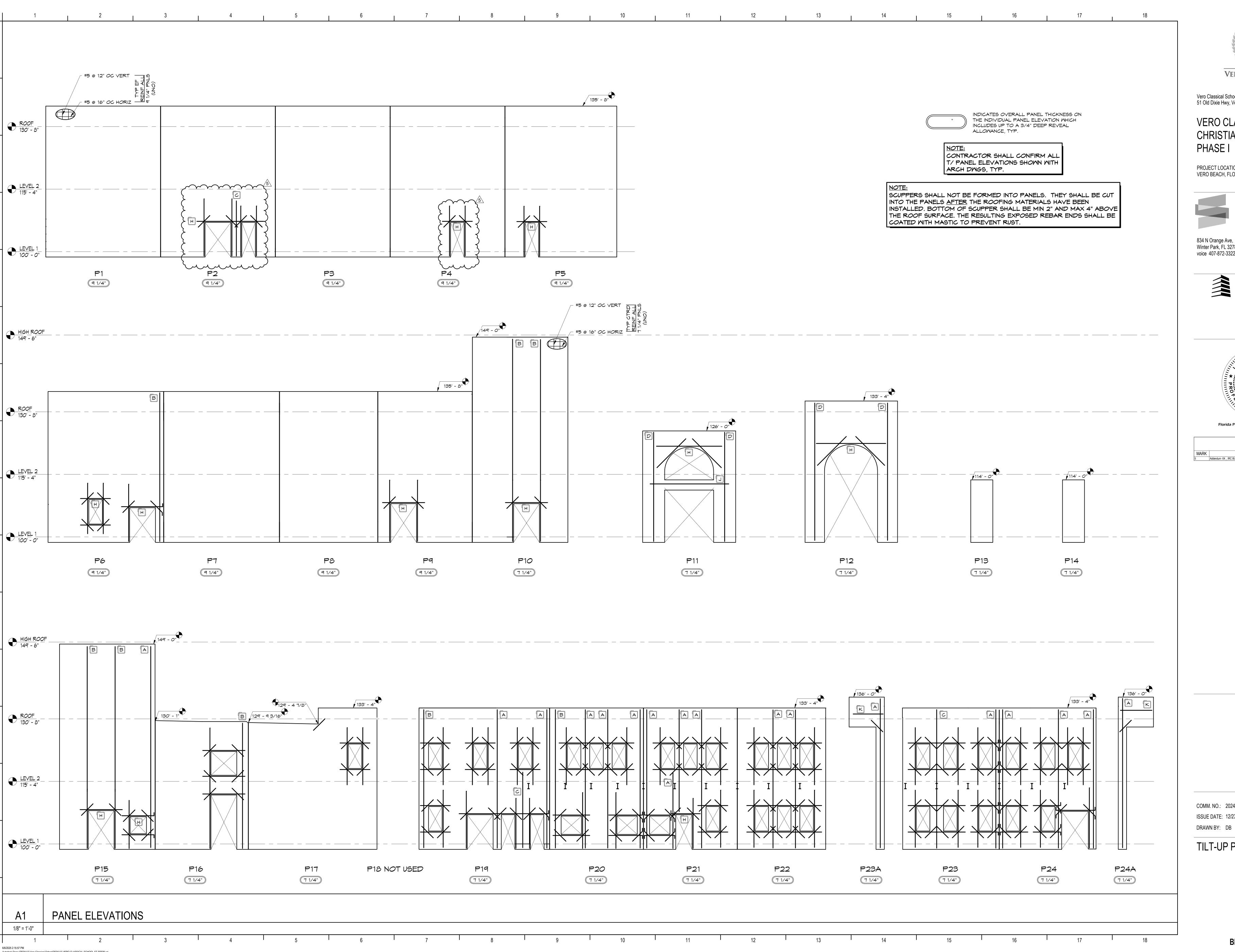
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REVISIONS DESCRIPTION Addendum -04 _ IRC BUILDING REVIEW COMMENTS

ISSUE DATE: 12/23/2024 DRAWN BY: DB

TILT-UP PANEL NOTES, DETAILS, AND SCHEDULES





VERO CLASSICAL CHRISTIAN SCHOOL -PHASE I

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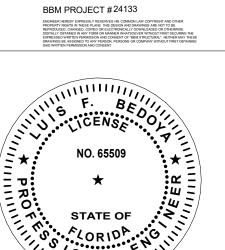


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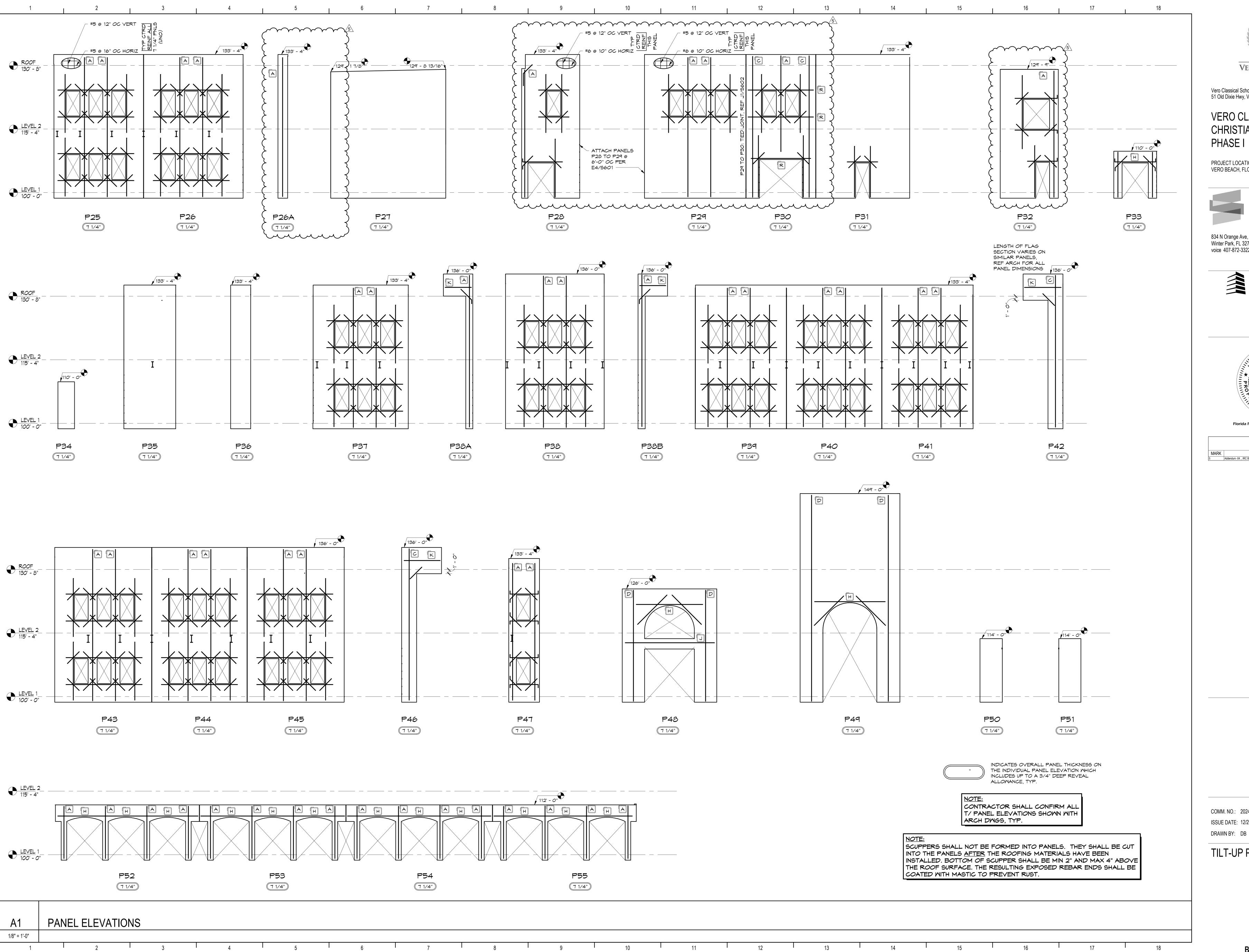
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EB: 5343



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TILT-UP PANEL ELEVATIONS



VERO CLASSICAL

Vero Classical School 51 Old Dixie Hwy, Vero Beach, FL 32962

VERO CLASSICAL CHRISTIAN SCHOOL -PHASE I

PROJECT LOCATION: 1275 58TH AVE. SW VERO BEACH, FLORIDA 32968

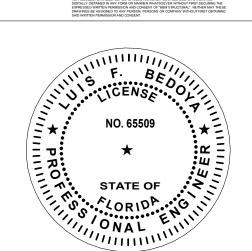


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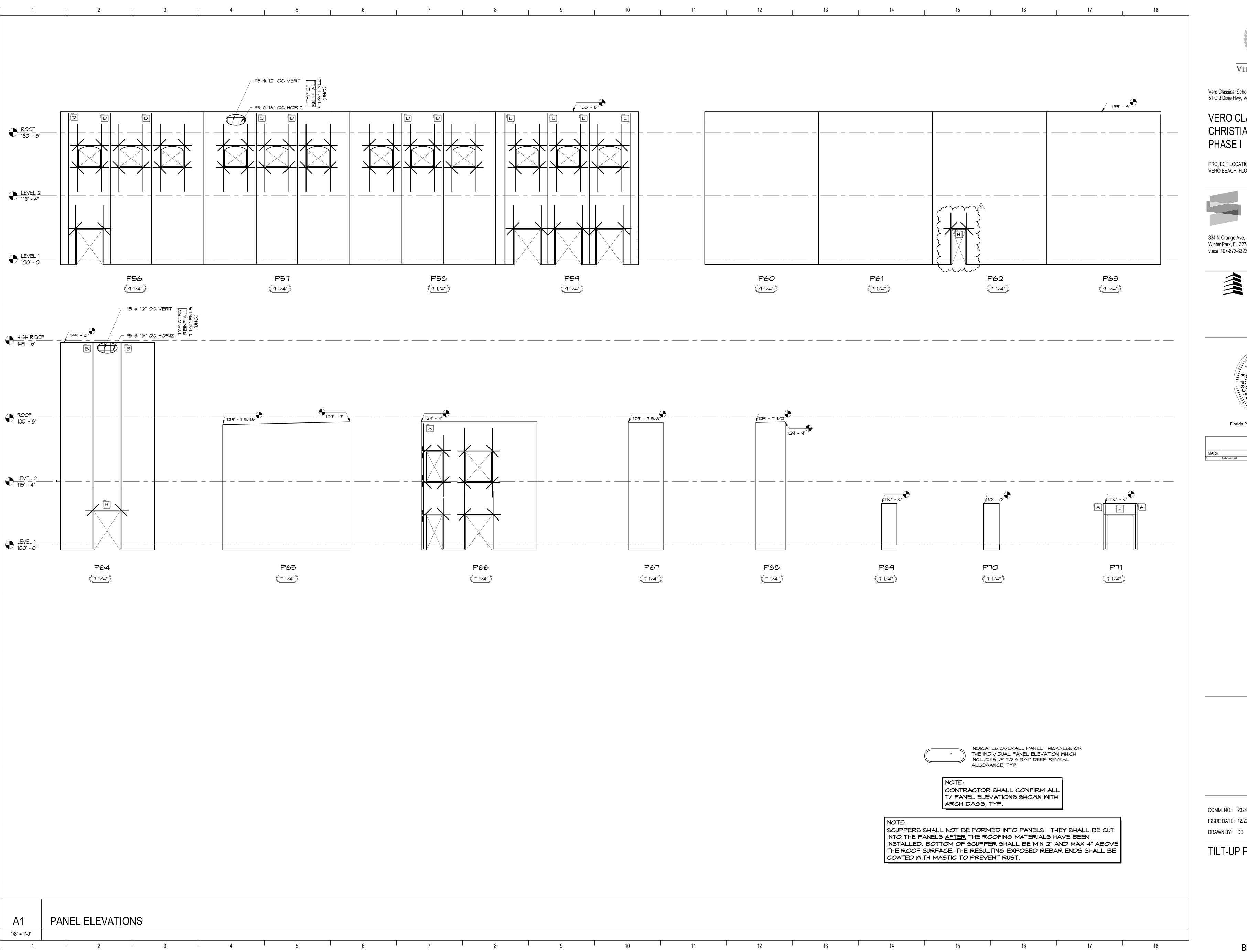
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TILT-UP PANEL ELEVATIONS





VERO CLASSICAL CHRISTIAN SCHOOL -PHASE I

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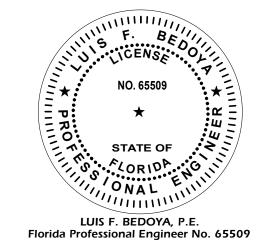


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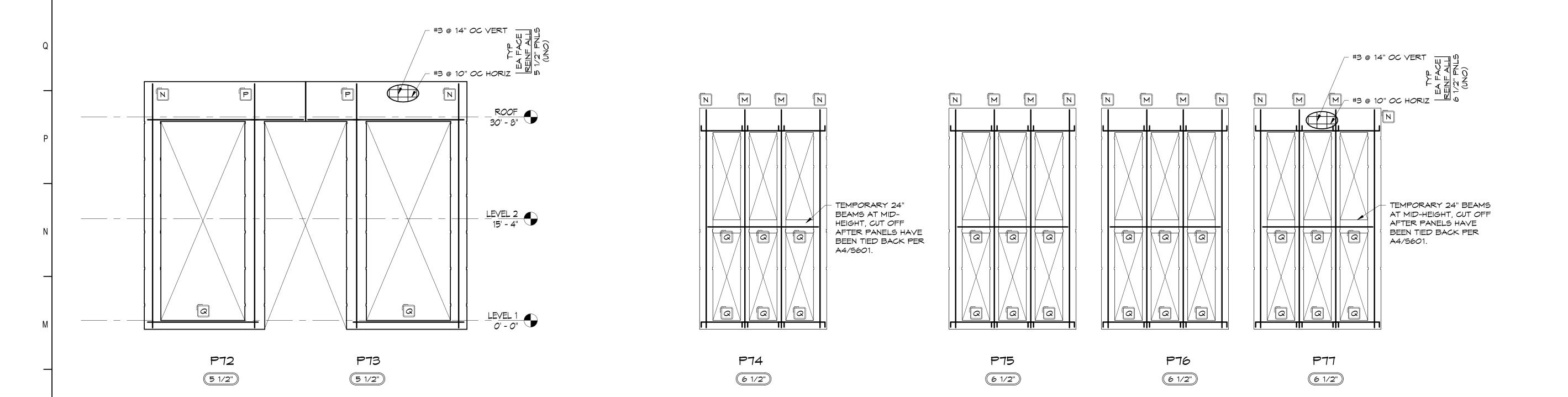
p: 407 - 645 - 3423 BBM PROJECT #24133

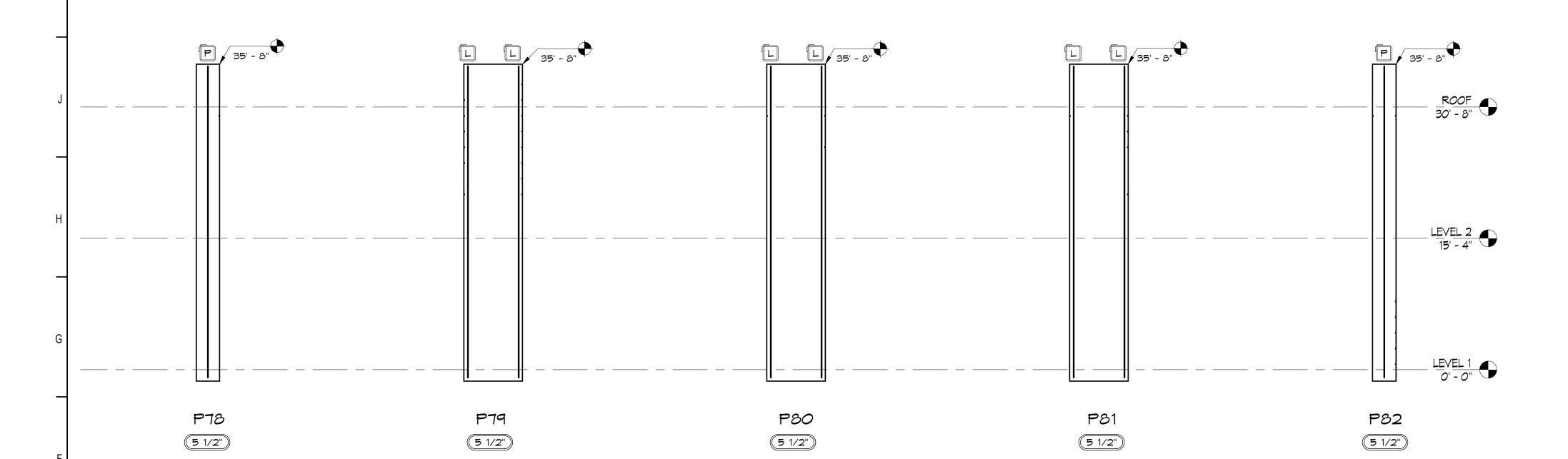


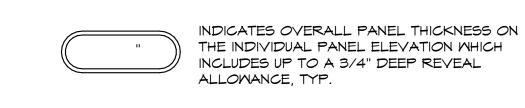
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TILT-UP PANEL ELEVATIONS







NOTE: CONTRACTOR SHALL CONFIRM ALL T/ PANEL ELEVATIONS SHOWN WITH ARCH DWGS, TYP.

NOTE:
SCUPPERS SHALL NOT BE FORMED INTO PANELS. THEY SHALL BE CUT INTO THE PANELS AFTER THE ROOFING MATERIALS HAVE BEEN INSTALLED. BOTTOM OF SCUPPER SHALL BE MIN 2" AND MAX 4" ABOVE THE ROOF SURFACE. THE RESULTING EXPOSED REBAR ENDS SHALL BE COATED WITH MASTIC TO PREVENT RUST.

VERO CLASSICAL SCHOOL

Vero Classical School 51 Old Dixie Hwy, Vero Beach, FL 32962

VERO CLASSICAL CHRISTIAN SCHOOL -PHASE I

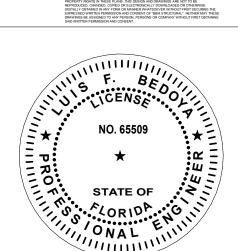
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LUIS F. BEDOYA, P.E. Florida Professional Engineer No. 65509

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TILT-UP PANEL ELEVATIONS

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A1 PANEL ELEVATIONS

1/8" = 1'-0"

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