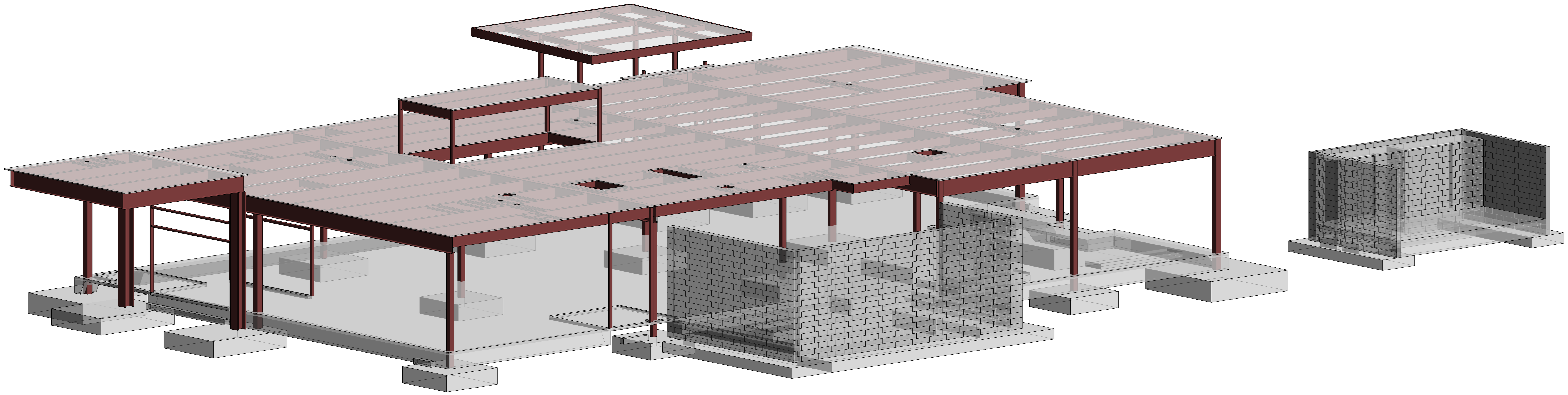
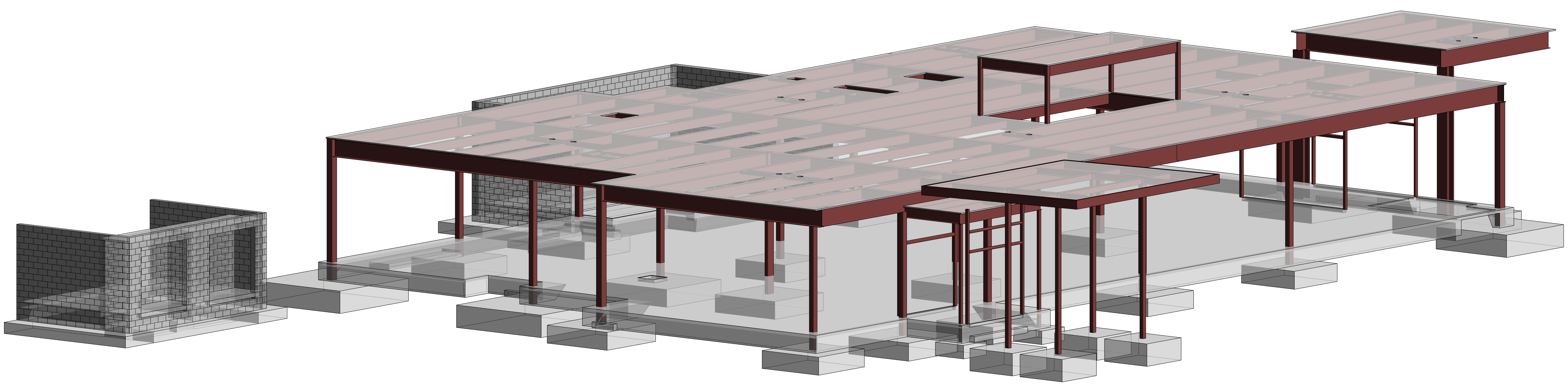


2 STRUCTURAL AXON VIEW
(FOR REFERENCE ONLY)



1 STRUCTURAL AXON VIEW
(FOR REFERENCE ONLY)

THIS SHEET MUST BE PRINTED IN COLOR

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REVISIONS

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STRUCTURAL AXON VIEW

S001

STRUCTURAL NOTES

A. GENERAL

1. NO PROVISION OF ANY REFERENCED STANDARD SPECIFICATION, MANUAL, OR CODE (WHETHER OR NOT SPECIFICALLY INCORPORATED BY REFERENCE IN THE CONTRACT DOCUMENTS) SHALL BE EFFECTIVE TO CHANGE THE DUTIES AND RESPONSIBILITIES OF OWNER, CONTRACTOR, ENGINEER, SUPPLIER, OR ANY OF THEIR CONSULTANTS, AGENTS, OR EMPLOYEES FROM THOSE SET FORTH IN THE CONTRACT DOCUMENTS. NOR SHALL IT BE EFFECTIVE TO ASSIGN TO THE STRUCTURAL ENGINEER OR ANY OF THE STRUCTURAL ENGINEER'S CONSULTANTS, AGENTS, OR EMPLOYEES ANY DUTY OR AUTHORITY TO SUPERVISE OR DIRECT THE FURNISHING OR PERFORMANCE OF THE WORK OR ANY DUTY OR AUTHORITY TO UNDERTAKE RESPONSIBILITIES CONTRARY TO THE PROVISIONS OF THE CONTRACT DOCUMENTS.
2. CONTRACT DOCUMENTS INCLUDE, BUT ARE NOT LIMITED TO, THE STRUCTURAL DOCUMENTS (DRAWINGS AND SPECIFICATIONS) BUT DO NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR MATERIAL PREPARED AND SUBMITTED BY THE CONTRACTOR.
3. REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION, OR ASSOCIATION OR TO CODES OF LOCAL OR STATE AUTHORITIES SHALL MEAN THE LATEST STANDARD, CODE, SPECIFICATION OR TENTATIVE SPECIFICATION ADOPTED AT THE DATE OF TAKING BIDS UNLESS SPECIFICALLY STATED OTHERWISE.
4. CONTRACT DOCUMENTS SHALL GOVERN IN THE EVENT OF A CONFLICT WITH THE CODE OF PRACTICE OR SPECIFICATIONS OF ACI, PCI, AISC, SJI, OR OTHER STANDARDS. WHERE A CONFLICT OCCURS WITHIN THE CONTRACT DOCUMENTS, THE STRICTEST REQUIREMENT SHALL GOVERN.
5. MATERIAL, WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE REFERENCED BUILDING CODE.
6. CONTRACTOR SHALL COORDINATE THE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND CIVIL DOCUMENTS. ARCHITECT/STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS, SEE THE ARCHITECTURAL DRAWINGS.
7. CONTRACTOR SHALL OBTAIN AND COORDINATE EDGE OF SLAB DIMENSIONS, OPENING LOCATIONS AND DIMENSIONS, DEPRESSED SLAB LOCATIONS AND EXTENTS, SLAB SLOPES, CURB LOCATIONS, AND CMU WALL LOCATIONS. ARCHITECT/STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION.
8. CONTRACTOR SHALL VERIFY THAT MISCELLANEOUS FRAMING SHOWN ON THE STRUCTURAL DRAWINGS FOR MECHANICAL EQUIPMENT, OWNER-FURNISHED ITEMS, PARTITIONS, ETC. IS CONSISTENT WITH THE REQUIREMENTS OF SUCH ITEMS. CONTRACTOR SHALL VERIFY EQUIPMENT WEIGHTS, REQUIRED OPENING SIZES, AND LOCATIONS IDENTIFIED ON THE STRUCTURAL DRAWINGS ARE IN AGREEMENT WITH FINAL ARCHITECTURAL AND MECHANICAL SHOP DRAWINGS AND SUBMITTALS.
9. CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, SAFETY, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION. CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA REGULATIONS.
10. THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED FORM. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY THE CONTRACTOR. CONTRACTOR IS RESPONSIBLE FOR CONSTRUCTABILITY ANALYSIS AND ERECTION PROCEDURES, INCLUDING DESIGN AND ERECTION OF FALSEWORK, TEMPORARY BRACING, ETC.
11. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED.
12. SUBMIT SHOP DRAWINGS THAT ADEQUATELY DEPICT THE STRUCTURAL ELEMENTS AND CONNECTIONS SHOWN IN THE CONTRACT DOCUMENTS. REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS REGARDING THE ARRANGEMENT AND SIZES OF MEMBERS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGN LOADS AND CONTRACT DOCUMENT DETAILS. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE ARCHITECT / STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL ENGINEER. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE ARCHITECT / STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.
13. WHERE A SECTION OR DETAIL IS SHOWN OR DETAILED FOR ONE CONDITION, IT SHALL APPLY TO ALL SIMILAR AND LIKE CONDITIONS. DETAILS LABELED "TYPICAL" OR "TYP." ON THE STRUCTURAL DRAWINGS APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR. THE CONTRACTOR SHALL CONSIDER ALL OF THE CONTRACT DOCUMENTS IN DETERMINING SIMILAR AND LIKE CONDITIONS.
14. USE ONLY DIMENSIONS INDICATED ON THE CONTRACT DOCUMENTS. DO NOT SCALE DRAWINGS OR MEASURE OBJECTS IN ELECTRONIC FILES. NOTIFY ARCHITECT/STRUCTURAL ENGINEER OF ANY DISCREPANCIES.
15. THE OWNER SHALL ESTABLISH A PERIODIC MAINTENANCE PROGRAM TO PROTECT THE STRUCTURE FROM DETERIORATION. THE MAINTENANCE PROGRAM IS THE RESPONSIBILITY OF THE OWNER AND SHOULD INCLUDE, BUT IS NOT LIMITED TO, THE FOLLOWING:

   PAINTING OF EXPOSED STEEL THAT IS NOT GALVANIZED
   INSPECTION AND MAINTENANCE OF PROTECTIVE COATINGS, SEALANTS, CAULKED JOINTS, EXPANSION JOINTS, AND CONTROL JOINTS
   REPAIR OF SPALLS AND CRACKS IN CONCRETE ELEMENTS
   REPAIR AND RESTORATION OF CORRODED ELEMENTS
   CLEANOUT OF DRAINS, INCLUDING ALL ROOF AND TRENCH DRAINS AND SCUPPERS
   CLEANING OF STRUCTURAL ELEMENTS EXPOSED TO HARSH CHEMICALS, INCLUDING DE-ICING CHEMICALS
   REPLACEMENT OF WORN BEARING PADS
16. THE USE OF STRUCTURAL BIM OR CAD FILES IS PROHIBITED WITHOUT WRITTEN CONSENT FROM THE STRUCTURAL ENGINEER.

B. CODE/DESIGN CRITERIA

1. STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE, 8TH EDITION (2023).
2. GRAVITY LOADS

   2.1. UNIFORM FLOOR LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):

GENERAL AREAS	80 PSF	(INCLUDES PARTITIONS)
CORRIDORS	100 PSF	
MECHANICAL ROOMS	150 PSF	(NON-REDUCIBLE)

   2.2. UNIFORM ROOF LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):

ROOF	20 PSF
GROUND SNOW LOAD, p_g	6 PSF
RAIN INTENSITY (15-MIN. STORM DURATION), i	11 IN/HR

   PONDING AND DRIFT EFFECTS HAVE BEEN INCLUDED IN THE DESIGN.

   2.3. CONCENTRATED FLOOR LOADS - DISTRIBUTED OVER AN AREA OF 2.5 FT^2, UNLESS NOTED OTHERWISE:

   HOSPITAL 1,000 LB
3. WIND LOADS:

BASIC DESIGN WIND SPEED, V	182 MPH
ALLOWABLE STRESS DESIGN WIND SPEED, V_{asd}	141 MPH
RISK CATEGORY	IV
WIND EXPOSURE	C
INTERNAL PRESSURE COEFFICIENT, GC_{pi}	± 0.18
TORNADO WIND SPEED, V_T	73 MPH
EFFECTIVE PLAN AREA, A_e	40,000 SQ. FT.
TORNADO INTERNAL PRESSURE COEFF., GC_{piT}	+0.55/-0.18

   THE PROJECT IS LOCATED IN A WIND-BORNE DEBRIS AND TORNADO-PRONE REGION AND REQUIRES MISSILE IMPACT RESISTANCE.
4. MAXIMUM ESTIMATED DEFLECTIONS LISTED BELOW ARE EXPECTED TO OCCUR AND SHALL BE CONSIDERED BY THE CONTRACTOR AND CLADDING DESIGNERS IN THE PERFORMANCE OF THE WORK:

   4.1. SPANDREL BEAM DEFLECTIONS: 1/2" SUPERIMPOSED LOAD DEFLECTION BETWEEN LEVELS. SUPERIMPOSED LOAD DEFLECTION IS THE DEFLECTION ANTICIPATED TO OCCUR BETWEEN ADJACENT LEVELS AFTER THE CLADDING HAS BEEN ERECTED. THE SUPERIMPOSED DEFLECTION DOES NOT INCLUDE DEFLECTIONS DUE TO THE SELF-WEIGHT OF THE STRUCTURE, NOR DOES IT INCLUDE AN ALLOWANCE FOR CONSTRUCTION TOLERANCE.

   4.2. MAXIMUM ESTIMATED DEFLECTIONS (IN INCHES) ARE AS FOLLOWS:

	LIVE LOAD	DEAD + LIVE LOAD
ROOF MEMBERS:	L/360	L/240

   WHERE L = SPAN LENGTH (IN INCHES) BETWEEN CENTERLINES OF SUPPORTS. FOR CANTILEVERS, L IS TWICE THE LENGTH OF THE CANTILEVER.

   4.3. MAXIMUM LATERAL INTER-STORY DRIFT DUE TO WIND LOADING (10-YEAR MEAN RECURRENCE INTERVAL) ARE AS FOLLOWS:

   H/400 AT THE CENTER OF MASS
   MAXIMUM OF 0.6" AT THE EDGE OF THE BUILDING
5. NO PROVISIONS HAVE BEEN MADE FOR FUTURE HORIZONTAL OR VERTICAL EXPANSION.

C. DEFERRED STRUCTURAL SUBMITTALS

1. DEFERRED SUBMITTALS, AS DEFINED BY THE BUILDING CODE, SHALL BE SUBMITTED TO THE BUILDING OFFICIAL BY THE CONTRACTOR. THE DEFERRED SUBMITTALS SHALL BE SIGNED AND SEALED BY A LICENSED ENGINEER IN THE PROJECT STATE.
2. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR THE DESIGN OF THE DEFERRED SUBMITTAL COMPONENTS OR THE CONNECTION TO THE STRUCTURE. THE STRUCTURAL DESIGN OF THE COMPONENTS AND THE CONNECTION TO THE STRUCTURE IS DELEGATED TO A SPECIALTY ENGINEER WHO SHALL BE ENGAGED BY THE CONTRACTOR, VENDOR, AND / OR SUPPLIER OF THE COMPONENTS AS PART OF THE DEFERRED SUBMITTAL PROCESS.
3. THE CONTRACTOR SHALL SUBMIT THE DEFERRED SUBMITTAL TO THE ARCHITECT / STRUCTURAL ENGINEER FOR REVIEW. AFTER REVIEW BY THE ARCHITECT / STRUCTURAL ENGINEER, THE CONTRACTOR SHALL SUBMIT THE REVIEWED SUBMITTAL TO THE BUILDING OFFICIAL PER SECTION 107.3 OF THE BUILDING CODE.
4. THE ITEMS LISTED BELOW ARE IDENTIFIED AS DEFERRED SUBMITTALS. REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND CIVIL DRAWINGS AND SPECIFICATIONS FOR ADDITIONAL DEFERRED SUBMITTAL COMPONENTS. ALL COSTS ASSOCIATED WITH THE PREPARATION OF THE DEFERRED SUBMITTAL, INCLUDING THE SPECIALTY ENGINEER'S DESIGN FEES, SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

   GROUND IMPROVEMENT
   CONCRETE FORMWORK
   STRUCTURAL STEEL CONNECTIONS
   HANDRAILS, AND GUARDRAILS
   COLD-FORMED METAL FRAMING
   EXTERIOR BUILDING SIGNAGE
   ANCHORAGE OF EXTERIOR ARCHITECTURAL, MECH., ELEC., AND PLUMBING EQUIPMENT
   ATTACHMENT OF ROOFTOP EQUIPMENT, PIPING, & DUCTWORK TO THE UNDERLYING STRUCTURE
   GLAZING SYSTEMS (e.g., CURTAIN WALL, WINDOW WALL, AND STOREFRONT SYSTEMS)
   SLOTTED CHANNEL STRUT FRAMING (e.g., UNISTRUT)
   OTHER ELEMENTS SPECIFICALLY IDENTIFIED IN THE CONTRACT DOCUMENTS
   BUILDING MAINTENANCE SYSTEMS
5. DEFERRED SUBMITTAL COMPONENTS SHALL BE DESIGNED FOR THE LOADS AS DEFINED BY THE APPLICABLE BUILDING CODE WITH DESIGN DATA DEFINED IN SECTION C OF THE STRUCTURAL NOTES.
6. THE DESIGNS OF ITEMS LISTED BELOW ARE THE RESPONSIBILITY OF THE CONTRACTOR BUT ARE NOT CONSIDERED DEFERRED SUBMITTALS AND ARE NOT TO BE SUBMITTED TO THE DESIGN TEAM. ALL COSTS ASSOCIATED WITH THE DESIGN OF THESE ELEMENTS, INCLUDING THE SPECIALTY ENGINEER'S DESIGN FEES, SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

   TEMPORARY BRACING / SHORING FOR STABILITY OF STRUCTURE DURING CONSTRUCTION
   ALL OTHER ELEMENTS IDENTIFIED IN THE CONTRACT DOCUMENTS

D. FOUNDATION

1. FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT PREPARED BY UES PROFESSIONAL SOLUTIONS, LLC REPORT NUMBER 240798 DATED APRIL 10, 2025. STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT FROM THOSE ASSUMED FOR DESIGN.
2. STRUCTURAL TESTING/INSPECTION AGENCY SHALL CERTIFY THE BEARING MEDIUM.
3. INDIVIDUAL SPREAD FOOTINGS AND CONTINUOUS FOOTINGS SHALL BEAR ON SOIL CAPABLE OF SUPPORTING 3000 PSF AND 3000 PSF, RESPECTIVELY.

   3.1. FOOTINGS SHALL BEAR AT LEAST 18" BELOW SLAB AND EXTERIOR SURFACE.

   3.2. NO FOOTINGS SHALL BEAR ON ROCK. UNDERCUT ROCK A MINIMUM OF 2 FEET BELOW BOTTOM OF FOOTING AND REPLACE WITH STRUCTURAL FILL.
4. PROOFROLL BUILDING AREAS WITH TWO COMPLETE COVERAGES OF A LOADED DUMP TRUCK OR SCRAPER. REPLACE SOFT AREAS WITH COMPACTED STRUCTURAL FILL AS REQUIRED BY THE SPECIFICATIONS.
5. DENSIFY BUILDING AREAS AND A MINIMUM OF 10'-0" OUTSIDE THE BUILDING PERIMETER USING A VIBRATORY ROLLER (SEE SPECIFICATIONS).

E. REINFORCEMENT

1. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60 UNLESS NOTED OTHERWISE.
2. WELDED WIRE REINFORCING SHALL CONFORM TO ASTM A1064 AND HAVE MINIMUM SIDE AND END LAPS OF ONE CROSS WIRE SPACING PLUS 2", BUT NOT LESS THAN 8".
3. SUBMIT SHOP DRAWINGS THAT ADEQUATELY DEPICT THE REINFORCING BAR SIZES AND PLACEMENT. WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS, ELEVATIONS, AND DETAILS IS NOT ACCEPTABLE.
4. PROVIDE DOWELS FROM FOUNDATIONS THE SAME SIZE AND NUMBER AS THE VERTICAL WALL OR COLUMN REINFORCING UNLESS NOTED OTHERWISE.
5. PLACE REINFORCEMENT AS FOLLOWS, UNLESS NOTED OTHERWISE:

   5.1. CAST-IN-PLACE CONCRETE REINFORCEMENT COVER

PERMANENTLY EXPOSED TO EARTH:	
CAST AGAINST THE EARTH	3" CLEAR
EXPOSED TO EARTH OR WEATHER:	
FOR BARS LARGER THAN A NO. 5 BAR	2" CLEAR
NO. 5 BARS OR SMALLER	1-1/2" CLEAR
NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH:	
SLABS	3/4" CLEAR

   5.2. MASONRY CONCRETE REINFORCEMENT COVER

   MASONRY REINFORCING STEEL SHALL BE PLACED IN THE CENTER OF CMU CELLS UNLESS NOTED OTHERWISE. IF REINFORCING STEEL IS NOTED TO BE PLACED OTHER THAN THE CENTER OF THE CELL, PLACE BAR TO ACHIEVE 2" CLEAR COVER BETWEEN REINFORCING AND OUTSIDE FACE OF CMU BLOCK.
6. REINFORCEMENT SHALL BE SPLICED ONLY AT LOCATIONS SHOWN OR NOTED IN THE STRUCTURAL DOCUMENTS, EXCEPT REINFORCEMENT MARKED "CONTINUOUS" CAN BE SPLICED AT LOCATIONS DETERMINED BY CONTRACTOR. SPLICES AT OTHER LOCATIONS SHALL BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER. REINFORCING STEEL SPLICES SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE:

   CONCRETE: CLASS B TENSION LAP - SEE REINFORCING LAP LENGTH SCHEDULE
   MASONRY: SEE REINFORCING LAP LENGTH SCHEDULE

F. CAST-IN-PLACE CONCRETE

1. CONCRETE WORK SHALL CONFORM TO ACI 318 AND CRSI STANDARDS.
2. CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES:

   2.1. NORMAL WEIGHT STRUCTURAL CONCRETE:

	EXPOSURE CLASS	28-DAY MIN. COMPRESSIVE STRENGTH, f'_c	MAX. w/cm RATIO	MAX. NOM. AGGREGATE SIZE
FOOTINGS	C1, W1	4,000 PSI	0.50	1-1/2"
PEDESTALS	C1, W1	4,000 PSI	0.50	1"
INTERIOR SLABS-ON-GROUND	C1	4,000 PSI	0.48	1"
EXTERIOR SLABS-ON-GROUND	F2, C1	4,500 PSI	0.45	1"
CURBS	F1,C1	5,000 PSI	0.40	3/4"

   ALL NORMAL WEIGHT CONCRETE SHALL BE CONSIDERED TO BE IN EXPOSURE CLASS F0, S0, W0, AND C0 ACCORDING TO ACI 318 UNLESS NOTED OTHERWISE ABOVE OR ELSEWHERE ON THE STRUCTURAL DRAWINGS.
3. CONCRETE MIX REQUIREMENTS

   3.1. ALL CONCRETE SHALL BE PROPORTIONED TO COMPLY WITH ACI 318 CHAPTER 19 IN ACCORDANCE WITH THE EXPOSURE CLASS INDICATED. WHERE REQUIREMENTS INDICATED DIFFER FROM REQUIREMENTS OF CHAPTER 19, THE STRICTER REQUIREMENT SHALL APPLY. REFER TO THE SPECIFICATIONS FOR OTHER REQUIREMENTS FOR VARIOUS EXPOSURE CLASSES RELATIVE TO THE CEMENT TYPE, AIR ENTRAINMENT REQUIREMENTS, CHLORIDE ION LIMITS, POZZOLAN LIMITS, AND SHRINKAGE LIMITS.

   3.2. CONCRETE SHALL BE CONSIDERED EXTERIOR CONCRETE IF THE CONCRETE IS PERMANENTLY EXPOSED TO THE WEATHER OR MOISTURE, OR IF IT IS IN AN UNCONDITIONED SPACE IN ITS COMPLETED CONFIGURATION.

   3.3. ALL CONCRETE SHALL SATISFY BOTH THE SPECIFIED MAXIMUM WATER TO CEMENT RATIO AND THE MINIMUM COMPRESSIVE STRENGTH, f'_c, REQUIREMENTS.
4. PIPES OR DUCTS SHALL NOT EXCEED ONE-THIRD THE SLAB OR WALL THICKNESS UNLESS SPECIFICALLY DETAILED. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION OF SLEEVES, ACCESSORIES, ETC.
5. REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ORNAMENTS, CLIPS, OR GROUNDS REQUIRED TO BE ENCASED IN CONCRETE AND FOR LOCATION OF ALL CONCRETE FINISHES AND SLAB DEPRESSIONS.
6. CONSTRUCTION JOINTS

   6.1. LOCATIONS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER.

   6.2. NO HORIZONTAL JOINTS ARE PERMITTED EXCEPT THOSE SHOWN ON THE STRUCTURAL DRAWINGS.
7. DEFECTIVE AREAS IN CONCRETE, INCLUDING BUT NOT LIMITED TO HONEY-COMBING, SPALLS, AND CRACKS WITH WIDTHS EXCEEDING 0.012", SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT / STRUCTURAL ENGINEER. REPAIR DEFECTIVE AREAS AS DIRECTED BY THE STRUCTURAL ENGINEER.

G. CONCRETE MASONRY

1. CONCRETE MASONRY WORK SHALL CONFORM TO TMS 402/602, BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES.
2. MINIMUM COMPRESSIVE STRENGTH OF CONCRETE MASONRY SHALL BE f'_m = 2,000 PSI.
3. MORTAR SHALL BE TYPE S AND SHALL COMPLY WITH THE BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY.
4. CONCRETE MASONRY UNITS SHALL BE GROUTED WITH 2,500 PSI COARSE GROUT AS SHOWN IN THE STRUCTURAL DOCUMENTS. GROUT SHALL CONFORM TO ASTM C476.
5. PROVIDE HORIZONTAL JOINT REINFORCEMENT WITH NO. 9 GAGE LONGITUDINAL WIRES AT 16" VERTICALLY UNLESS NOTED OTHERWISE. PROVIDE SPECIAL ACCESSORIES FOR CORNERS, INTERSECTIONS, ETC.
6. PROVIDE OPEN BOTTOM BEAM BLOCK UNITS WITH 3" DEEP MINIMUM WEB OPENINGS AT HORIZONTAL REINFORCEMENT LOCATIONS. A MINIMUM CLEAR SPACE OF ONE BAR DIAMETER SHALL BE PROVIDED BETWEEN THE REINFORCING BARS AND THE FACE OF MASONRY UNITS.
7. PROVIDE CONTRACTION (CONTROL) JOINTS IN ALL CONCRETE MASONRY WALLS AT LOCATIONS APPROVED BY THE ARCHITECT AT A MAXIMUM SPACING OF 1.5 TIMES THE WALL HEIGHT OR 25'-0", WHICHEVER IS LESS. THE DISTANCE TO THE FIRST CONTRACTION JOINT FROM CORNERS OR INTERSECTING WALLS IS LIMITED TO HALF OF THE ABOVE VALUE.
8. PROVIDE DOVETAIL ANCHORS AT 16" C/C, UNLESS NOTED OTHERWISE, WHERE MASONRY WALLS ABUT CONCRETE SURFACES OR STRUCTURAL STEEL MEMBERS.
9. SUBMIT WRITTEN CONSTRUCTION PROCEDURES PRIOR TO THE START OF MASONRY CONSTRUCTION.
10. MINIMUM VERTICAL WALL REINFORCEMENT SHALL BE #5@32" FOR INTERIOR WALLS AND #5@24" FOR EXTERIOR WALLS UNLESS NOTED OTHERWISE.

H. POST-INSTALLED ANCHORS AND REINFORCING STEEL

1. POST-INSTALLED ANCHORS AND REINFORCING STEEL SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER PRIOR TO INSTALLING POST-INSTALLED ANCHORS OR REINFORCING STEEL IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS OR REINFORCING STEEL.
2. CONTRACTOR SHALL ARRANGE ON-SITE INSTALLATION TRAINING BY THE MANUFACTURER FOR EACH PRODUCT TO BE INSTALLED. SUBMIT TO THE STRUCTURAL ENGINEER DOCUMENTATION CONFIRMING TRAINING OF ALL PERSONNEL WHO WILL BE INSTALLING PRODUCTS. TRAINING AND DOCUMENTATION SHALL OCCUR PRIOR TO THE COMMENCEMENT OF PRODUCT INSTALLATION. INSTALLATION OF ADHESIVE ANCHOR PRODUCTS IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATION RESISTING SUSTAINED TENSION LOADS SHALL BE CONDUCTED BY AN INSTALLER CERTIFIED IN ACCORDANCE WITH THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM. PROOF OF CERTIFICATION SHALL BE MAINTAINED AT THE JOB SITE.
3. EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR OR REINFORCING LOCATIONS. CONTRACTOR SHALL LOCATE THE POSITION OF REINFORCING BARS AND P.T. TENDONS BY FERROSCAN, GPR, X-RAY, CHIPPING, OR OTHER MEANS PRIOR TO ANCHOR INSTALLATION. THE CONTRACTOR SHALL NOT DAMAGE ANY REINFORCING.
4. ADHESIVE ANCHOR INSERTS SHALL BE ALL THREAD RODS OF THE FOLLOWING MATERIAL UNLESS NOTED OTHERWISE:

   INTERIOR ENVIRONMENTS: ASTM F1554 Gr. 36
   EXTERIOR ENVIRONMENTS: ASTM F1554 Gr. 36 GALV. PER ASTM B695, CLASS 65 TYPE I
   CORROSIVE ENVIRONMENTS: ASTM A193 GR. B8M TYPE 316
5. MECHANICAL AND SCREW ANCHORS IN EXTERIOR AND CORROSIVE ENVIRONMENTS SHALL BE GALVANIZED OR STAINLESS STEEL AND SHALL APPROVED BY THE MANUFACTURER FOR THE EXPOSURE.
6. PROVIDE THE FOLLOWING INSTALLATION CONDITIONS FOR ADHESIVE ANCHORS, NOTIFY THE STRUCTURAL ENGINEER IF THESE CONDITIONS DO NOT EXIST:

   HOLES DRILLED WITH HAMMER DRILL WITH CARBIDE TIPPED DRILL BIT
   DRY HOLE
   CONCRETE CURED FOR A MINIMUM OF 21 DAYS
   TEMPERATURE CATEGORY B (110° F LONG TERM AND 130° F SHORT TERM)

I. STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED ACCORDING TO ANSI/AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND ANSI/AISC 303 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
2. STRUCTURAL STEEL SHALL BE OF THE FOLLOWING GRADE UNLESS NOTED OTHERWISE ON DRAWINGS:

W, WT, C, AND MC SHAPES	ASTM A992
L, M, S, MT, AND ST SHAPES	ASTM A572, GRADE 50
HSS SHAPES	ASTM A500, GRADE C
STEEL PIPE SHAPES	ASTM A53, GRADE B
PLATES AND BARS	ASTM A572, GRADE 50
OUTRIGGERS AND BENT PLATES	ASTM A36 OR ASTM A572. GRADE 50
ROUND ROD	ASTM A36

3. BOLTS, ANCHOR RODS, AND HEADED STUDS:

   3.1. ALL BOLTS SHALL BE GROUP 120 OR GROUP 150 HIGH-STRENGTH BOLTS WITH A 3/4" MINIMUM DIAMETER UNLESS NOTED OTHERWISE. THE FOLLOWING CONNECTIONS SHALL BE SLIP-CRITICAL:

   SHEAR CONNECTIONS IN MOMENT CONNECTIONS WITH WELDED JOINTS (MOMENT FRAMES, CANTILEVERS, ETC.)
   FIELD MODIFIED CONNECTIONS
   OTHER CONNECTIONS, AS NOTED IN THE DRAWINGS

   ALL OTHER CONNECTIONS MAY BE BEARING TYPE. BOLT SHEAR STRENGTH SHALL BE DETERMINED IN ACCORDANCE WITH ANSI/ASIC 360.

   3.2. ANCHOR RODS SHALL CONFORM TO ASTM F1554, GRADE 55 WITH SUPPLEMENT UNLESS NOTED OTHERWISE.
4. CONNECTIONS:

   4.1. STEEL CONNECTIONS SHALL BE DETAILED BASED ON THE DESIGN INFORMATION PROVIDED IN THE CONTRACT DOCUMENTS. DEVIATION FROM THE CONNECTION DETAILS DEPICTED IN THE CONTRACT DOCUMENTS SHALL NOT BE PERMITTED WITHOUT ADVANCE WRITTEN PERMISSION FROM THE STRUCTURAL ENGINEER.

   4.2. THE SERVICES OF A CONNECTION DESIGN SPECIALTY ENGINEER (CDSE) SHALL BE INCLUDED IN THE CONTRACTOR'S SCOPE OF SERVICES. THE CDSE SHALL BE LICENSED IN THE PROJECT STATE. THE CDSE IS RESPONSIBLE FOR REVIEWING THE STEEL SHOP DRAWINGS TO ENSURE THAT ALL CONNECTION DESIGN DETAILS HAVE BEEN CORRECTLY SHOWN ON THE SHOP DRAWINGS. THE CDSE SHALL SUBMIT A SIGNED AND SEALED LETTER CONFIRMING THE ABOVE REVIEW WITH EACH SHOP DRAWING SUBMITTAL.

   4.3. SHEAR CONNECTIONS:

   4.3.1. SHEAR CONNECTIONS SHALL BE DETAILED AS DOUBLE-ANGLE, SINGLE-PLATE, SINGLE-ANGLE, OR TEE CONNECTIONS IN ACCORDANCE WITH CONNECTION TABLES IN PART 10 OF THE "STEEL CONSTRUCTION MANUAL."

   4.3.2. CONNECTIONS SHALL BE DETAILED TO CARRY A MINIMUM AXIAL LOAD OF 0.05 × THE REQUIRED SHEAR DESIGN LOAD FOR THE CONNECTION.

   4.4. FOR WELDED CONNECTIONS, USE PREQUALIFIED WELDED JOINTS IN ACCORDANCE WITH AISC AND THE STRUCTURAL WELDING CODE OF THE AMERICAN WELDING SOCIETY. "NON-PREQUALIFIED JOINTS" SHALL BE QUALIFIED PRIOR TO FABRICATION.

   4.5. THE REQUIRED REACTION USING LRFD LOAD COMBINATIONS SHALL BE AS SHOWN ON THE STRUCTURAL DRAWINGS. IF REACTIONS ARE NOT SHOWN:

   FOR BEAMS W10 AND SMALLER: 10 KIP SHEAR CAPACITY
   FOR BEAMS W12 AND LARGER: CONTACT THE STRUCTURAL ENGINEER

   4.6. REVIEW OF THE SHOP DRAWINGS AND/OR CONNECTION CALCULATIONS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR AND CDSE OF THE FULL RESPONSIBILITY FOR THE DESIGN AND ADEQUACY OF SUCH CONNECTIONS.
5. THE STRUCTURAL STEEL MEMBERS HAVE NOT BEEN DESIGNED TO ACCOMMODATE THE TORSION RESULTING FROM THE ECCENTRIC LOADING OF THE PRECAST PANELS, CURTAINWALL SYSTEMS, LIGHT GAUGE METAL FRAMING, ETC. SUPPLEMENTAL SECONDARY BRACING SHALL BE DESIGNED AND PROVIDED BY THE SUPPLIER TO ELIMINATE TORSION.
6. ALL STEEL EXPOSED TO WEATHER OR MOISTURE SHALL BE GALVANIZED UNLESS OTHERWISE DIRECTED BY THE ARCHITECT.
7. THE LATERAL LOAD-RESISTING SYSTEM INCLUDES STRUCTURAL STEEL, NON-STRUCTURAL STEEL ELEMENTS, AND THE DIAPHRAGM AS INDICATED BELOW. ALL ELEMENTS OF THE LATERAL LOAD-RESISTING SYSTEM AND DIAPHRAGM ARE REQUIRED TO BE COMPLETE AS INDICATED AND DETAILED IN THE STRUCTURAL CONTRACT DOCUMENTS TO PROVIDE THE LATERAL STRENGTH AND STABILITY OF THE STEEL STRUCTURE. THE STRUCTURE SHALL BE CONSIDERED UNSTABLE UNTIL THESE SYSTEMS AND ELEMENTS ARE COMPLETE.

   7.1. THE LATERAL LOAD-RESISTING SYSTEM FOR THE STEEL STRUCTURE INCLUDES THE FOLLOWING ELEMENTS AS INDICATED AND DETAILED IN THE STRUCTURAL CONTRACT DOCUMENTS:

   MOMENT FRAMES
   CONNECTIONS, BASEPLATES, ANCHOR RODS, AND GROUT

   7.2. THE LATERAL LOAD-RESISTING DIAPHRAGM FOR THE STEEL STRUCTURE INCLUDES THE FOLLOWING ELEMENTS AS INDICATED AND DETAILED IN THE STRUCTURAL CONTRACT DOCUMENTS:

   STEEL ROOF DECK

   7.3. SPECIFIC ELEMENTS, AS IDENTIFIED WITHIN THE PLANS AND DETAILS OF THE STRUCTURAL CONTRACT DRAWINGS, SHALL ALSO BE CONSIDERED PART OF THE LATERAL LOAD-RESISTING SYSTEM.
8. STABILITY BRACING: THE STABILITY OF STRUCTURAL STEEL ELEMENTS, INCLUDING INDIVIDUAL HOT-ROLLED STEEL SHAPES AND FABRICATED TRUSSES, IS PROVIDED BY THE FOLLOWING ELEMENTS AS INDICATED AND DETAILED IN THE STRUCTURAL CONTRACT DOCUMENTS. THESE ELEMENTS SHALL BE COMPLETE AS SHOWN IN THE STRUCTURAL CONTRACT DOCUMENTS BEFORE ANY TEMPORARY MEANS AND METHODS REQUIRED FOR ERECTION ARE REMOVED.

   STEEL ROOF DECK
   STRUCTURAL STEEL BRACING AND KICKERS
9. THE WALL THICKNESS OF ROLLED HSS MEMBERS SHOWN ON THE PLANS IS THE MINIMUM THICKNESS REQUIRED FOR STRUCTURAL PURPOSES. THE CONTRACTOR SHALL INCREASE THE WALL THICKNESS OR EMPLOY OTHER CONSTRUCTION MEANS AS REQUIRED TO PREVENT DISTORTION, WARPING, OR OIL-CANNING OF THE HSS CROSS SECTION.

J. STEEL DECK

1. STEEL DECK SHALL BE PLACED OVER MULTIPLE SPANS WHEREVER POSSIBLE. WHERE SINGLE-SPAN DECK IS REQUIRED, THE CONTRACTOR SHALL DRAW SPECIFIC ATTENTION TO THOSE LOCATIONS ON THE SHOP DRAWINGS.
2. SUBMIT SHOP DRAWINGS SHOWING THE STEEL DECK PROFILE, GAGE, PHYSICAL PROPERTIES, AND LAYOUT. THE SUBMITTAL SHALL INCLUDE ALL ACCESSORIES AND INSTALLATION DETAILS. IF DECK OTHER THAN THE BASIS OF DESIGN IS PROVIDED, THE SUBMITTAL SHALL INCLUDE LOAD TABLES DEMONSTRATING THE DECK MEETS OR EXCEEDS THE BASIS OF DESIGN. THE LOAD TABLES SHALL BE IN ACCORDANCE WITH THE STEEL DECK INSTITUTE (SDI) REQUIREMENTS.
3. ROOF DECK:

   3.1. THE 1 1/2" TYPE B ROOF DECK BASIS OF DESIGN IS 1.5B ROOF DECK PRODUCED BY VULCRAFT (IAPMO UES ER-0652). OTHER DECK MANUFACTURERS ARE PERMITTED, PROVIDED THE FOLLOWING MINIMUM DECK PROPERTIES ARE MET OR EXCEEDED:

GAGE	18
YIELD STRESS	50 KSI
MOMENT OF INERTIA (POSITIVE BENDING), I(+)	0.277 IN^4/FT
MOMENT OF INERTIA (NEGATIVE BENDING), I(-)	0.290 IN^4/FT
SECTION MODULUS (POSITIVE MOMENT), S(+)	0.306 IN^3/FT
SECTION MODULUS (NEGATIVE MOMENT), S(-)	0.318 IN^3/FT

   3.2. THE 3" TYPE N ROOF DECK BASIS OF DESIGN IS 3NL ROOF DECK PRODUCED BY VULCRAFT (IAPMO UES ER-0652). OTHER DECK MANUFACTURERS ARE PERMITTED, PROVIDED THE FOLLOWING MINIMUM DECK PROPERTIES ARE MET OR EXCEEDED:

GAGE	18
YIELD STRESS	50 KSI
MOMENT OF INERTIA (POSITIVE BENDING), I(+)	1.123 IN^4/FT
MOMENT OF INERTIA (NEGATIVE BENDING), I(-)	1.200 IN^4/FT
SECTION MODULUS (POSITIVE MOMENT), S(+)	0.660 IN^3/FT
SECTION MODULUS (NEGATIVE MOMENT), S(-)	0.675 IN^3/FT

   3.3. DECK FINISH SHALL BE GALVANIZED G60.

   3.4. PROVIDE VENTED DECK IN ACCORDANCE WITH SECTION 1917 OF THE FLORIDA BUILDING CODE WHEN USED WITH LIGHTWEIGHT INSULATING CONCRETE ROOFING.

K. COLD-FORMED METAL STUDS

1. COLD-FORMED STEEL DESIGN, FABRICATION, AND ERECTION SHALL BE IN ACCORDANCE WITH ANSI/AISI S100 "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS."
2. DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS AND THEIR CONNECTIONS SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. SUBMIT SHOP DRAWINGS SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE. SHOP DRAWINGS SHALL INCLUDE PLACING DRAWINGS FOR FRAMING MEMBERS SHOWING SIZE AND GAUGE DESIGNATIONS, NUMBER, TYPE, LOCATION, AND SPACING. INDICATE SUPPLEMENTAL STRAPPING, BRACING, SPLICES, BRIDGING, ACCESSORIES, AND DETAILS REQUIRED FOR PROPER INSTALLATION. REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS REGARDING ARRANGEMENT AND SIZES OF MEMBERS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGN LOADS AND CONTRACT DOCUMENT DETAILS. SUCH REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR THE DESIGN OF THE COLD-FORMED STEEL STRUCTURAL MEMBERS AND THEIR CONNECTIONS.
3. COLD-FORMED STEEL WALL SYSTEMS SHALL BE DESIGNED TO ACCOMMODATE RELATIVE SUPERIMPOSED LOAD DEFLECTIONS BETWEEN LEVELS.

L. CHANNEL STRUT FRAMING SUPPORT SYSTEM (UNISTRUT)

1. PROVIDE AND INSTALL SLOTTED CHANNEL STRUT FRAMING SUPPORT SYSTEM (UNISTRUT OR EQUIVALENT) FOR THE SUPPORT OF OWNER FURNISHED MEDICAL EQUIPMENT.
2. DESIGN OF SLOTTED CHANNEL STRUT FRAMING SUPPORT SYSTEM SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. SUBMIT SHOP DRAWINGS SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE. SHOP DRAWINGS SHALL SHOW PLAN LAYOUT, TYPICAL ELEVATIONS, DETAILS, AND ANCHORAGE TO THE STRUCTURE. REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS. SUCH REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR THE DESIGN OF THE SLOTTED CHANNEL STRUT FRAMING SUPPORT SYSTEM.
3. THE SYSTEM STRUCTURAL DESIGN SHALL ADEQUATELY SUPPORT THE MEDICAL EQUIPMENT IN THE VENDOR DRAWINGS PREPARED BY THE MANUFACTURER.
4. MEMBERS SHALL BE LEVEL IN BOTH DIRECTIONS TO WITHIN 1/16".
5. MEMBERS SHALL BE PARALLEL TO WITHIN 1/16".
6. SYSTEM SHALL PERMIT A MINIMUM ADJUSTMENT OF 6".
7. COORDINATE INSTALLATION WITH OTHER TRADES.

THIS SHEET MUST BE PRINTED IN COLOR

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REVISIONS

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DATE	11-20-2025

STRUCTURAL NOTES

S002

REINFORCING LAP LENGTH SCHEDULE

TENSION LAP SPLICE LENGTHS, (INCHES) FOR GRADE 60 (4) UNCOATED BARS, f'c = 3,000psi; NORMAL-WEIGHT CONCRETE (5)

BAR SIZE	LAP CLASS	LAP LENGTH PER SPACING AND COVER CASE (3)					
		TOP BARS (6) CATEGORY (2)			OTHER BARS CATEGORY (2)		
		1	2	3	1	2	3
#3	A	32	22	13	25	17	12
	B	42	28	17	32	22	13
#4	A	43	29	17	33	22	13
	B	56	37	23	43	29	17
#5	A	54	36	22	41	28	17
	B	70	47	28	54	36	22
#6	A	64	43	26	50	33	20
	B	84	56	34	64	43	26
#7	A	94	63	38	72	48	29
	B	122	81	49	94	63	38
#8	A	107	72	43	82	55	33
	B	139	93	56	107	72	43
#9	A	121	81	49	93	62	38
	B	157	105	63	121	81	49
#10	A	136	91	55	105	70	42
	B	177	118	71	136	91	55
#11	A	151	101	61	116	78	47
	B	196	131	79	151	101	61

TENSION LAP SPLICE LENGTHS, (INCHES) FOR GRADE 60 (4) UNCOATED BARS, f'c = 4,000psi; NORMAL-WEIGHT CONCRETE (5)

BAR SIZE	LAP CLASS	LAP LENGTH PER SPACING AND COVER CASE (3)					
		TOP BARS (6) CATEGORY (2)			OTHER BARS CATEGORY (2)		
		1	2	3	1	2	3
#3	A	28	19	12	22	15	12
	B	36	24	15	28	19	12
#4	A	37	25	15	29	19	12
	B	48	32	20	37	25	15
#5	A	47	31	19	36	24	15
	B	60	40	24	47	31	19
#6	A	56	37	22	43	29	17
	B	72	48	29	56	37	22
#7	A	81	54	33	63	42	25
	B	106	70	42	81	54	33
#8	A	93	62	37	71	48	29
	B	121	80	48	93	62	37
#9	A	105	70	42	81	54	33
	B	136	91	55	105	70	42
#10	A	118	79	47	91	61	37
	B	153	102	62	116	79	47
#11	A	131	87	53	101	67	41
	B	170	113	68	131	87	53

TENSION LAP SPLICE LENGTHS, (INCHES) FOR GRADE 60 (4) UNCOATED BARS, f'c = 5,000psi; NORMAL-WEIGHT CONCRETE (5)

BAR SIZE	LAP CLASS	LAP LENGTH PER SPACING AND COVER CASE (3)					
		TOP BARS (6) CATEGORY (2)			OTHER BARS CATEGORY (2)		
		1	2	3	1	2	3
#3	A	25	17	12	19	13	12
	B	33	22	13	25	17	12
#4	A	33	22	14	26	17	12
	B	43	29	18	33	22	14
#5	A	42	28	17	32	22	13
	B	54	36	22	42	28	17
#6	A	50	33	20	38	26	16
	B	65	43	26	50	33	20
#7	A	73	49	29	58	37	23
	B	94	63	38	73	49	29
#8	A	83	55	33	64	43	26
	B	108	72	43	83	55	33
#9	A	94	63	38	72	48	29
	B	122	81	49	94	63	38
#10	A	105	70	42	81	54	33
	B	137	91	55	105	70	42
#11	A	117	76	47	90	60	36
	B	152	101	61	117	78	47

TENSION LAP SPLICE LENGTHS, (INCHES) FOR GRADE 60 (4) UNCOATED BARS, f'c = 7,000psi; NORMAL-WEIGHT CONCRETE (5)

BAR SIZE	LAP CLASS	LAP LENGTH PER SPACING AND COVER CASE (3)					
		TOP BARS (6) CATEGORY (2)			OTHER BARS CATEGORY (2)		
		1	2	3	1	2	3
#3	A	21	14	12	16	12	12
	B	26	18	12	21	14	12
#4	A	28	19	12	22	15	12
	B	37	25	15	28	19	12
#5	A	35	24	14	27	18	12
	B	46	31	18	35	24	14
#6	A	42	28	17	33	22	13
	B	55	37	22	42	28	17
#7	A	61	41	25	47	32	19
	B	80	53	32	61	41	25
#8	A	70	47	28	54	36	22
	B	91	61	37	70	47	28
#9	A	79	53	32	61	41	25
	B	103	69	41	79	53	32
#10	A	89	59	36	69	46	28
	B	116	77	47	89	59	36
#11	A	99	66	40	76	51	31
	B	128	86	52	99	66	40

TENSION LAP SPLICE LENGTHS, (INCHES) FOR GRADE 60 (4) UNCOATED BARS, f'c =8,000psi; NORMAL-WEIGHT CONCRETE (5)

BAR SIZE	LAP CLASS	LAP LENGTH PER SPACING AND COVER CASE (3)					
		TOP BARS (6) CATEGORY (2)			OTHER BARS CATEGORY (2)		
		1	2	3	1	2	3
#3	A	20	13	12	15	12	12
	B	26	17	12	20	13	12
#4	A	26	18	12	20	14	12
	B	34	23	14	26	18	12
#5	A	33	22	13	25	17	12
	B	43	29	17	33	22	13
#6	A	40	26	16	30	20	12
	B	51	34	21	40	26	16
#7	A	58	38	23	44	30	18
	B	75	50	30	58	38	23
#8	A	66	44	26	51	34	20
	B	85	57	34	66	44	26
#9	A	74	49	30	57	38	23
	B	96	64	39	74	49	30
#10	A	83	56	34	64	43	26
	B	108	72	44	83	56	34
#11	A	93	62	37	71	48	29
	B	120	80	48	93	62	37

TENSION LAP SPLICE LENGTHS, (INCHES) FOR GRADE 60 (4) UNCOATED BARS, f'c ≥ 10,000psi; NORMAL-WEIGHT CONCRETE (5)

BAR SIZE	LAP CLASS	LAP LENGTH PER SPACING AND COVER CASE (3)					
		TOP BARS (6) CATEGORY (2)			OTHER BARS CATEGORY (2)		
		1	2	3	1	2	3
#3	A	18	12	12	14	12	12
	B	23	16	12	18	12	12
#4	A	24	16	12	18	12	12
	B	31	21	12	24	16	12
#5	A	30	20	12	23	15	12
	B	38	26	16	30	20	12
#6	A	35	24	14	27	18	12
	B	46	31	19	35	24	14
#7	A	51	34	21	40	27	16
	B	67	45	27	51	34	21
#8	A	59	39	24	45	30	18
	B	76	51	31	59	39	24
#9	A	66	44	27	51	34	21
	B	86	57	35	66	44	27
#10	A	75	50	30	57	38	23
	B	97	65	39	75	50	30
#11	A	83	55	33	64	43	26
	B	108	72	43	83	55	33

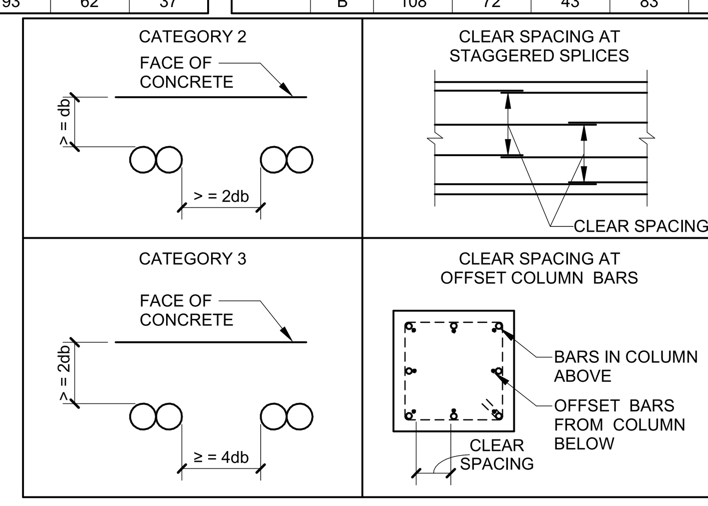
LAP NOTES:

1. LAP SPLICE LENGTHS ARE BASED ON ACI 318 SECTION 25.4.2.
2. CATEGORY DEFINITIONS: (SEE ADJACENT FIGURES FOR ADDITIONAL INFORMATION)

   CATEGORY 1 - OTHER CASES - DOES NOT MEET CATEGORY 2 OR 3.

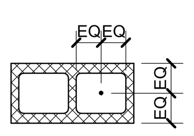
   CATEGORY 2 - CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED NOT LESS THAN 2db AND CLEAR COVER NOT LESS THAN db.

   CATEGORY 3 - CLEAR COVER NOT LESS THAN 2db AND THE CLEAR SPACING NOT LESS THAN 4db.
3. MINIMUM LAP SPLICE SHALL NOT BE LESS THAN 12 INCHES.
4. MULTIPLY ABOVE LAP LENGTHS BY 1.53 FOR GRADE 80 REINFORCING STEEL.
5. MULTIPLY ABOVE LAP LENGTHS BY 1.3 FOR LIGHTWEIGHT CONCRETE.
6. TOP BARS ARE DEFINED AS HORIZONTAL REINFORCEMENT SO PLACED THAT MORE THAN 12 INCHES OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE DEVELOPMENT LENGTH OR SPLICE.
7. COMPRESSION LAPS FOR GR. 60 UNCOATED BARS IN NORMAL WEIGHT CONCRETE WITH A MINIMUM STRENGTH OF 3,000 PSI SHALL BE 30db. USE 48db FOR GR. 80 COMPRESSION LAP.
8. IF BARS OF DIFFERENT SIZE ARE LAP SPLICED IN TENSION, SPLICE LENGTH SHALL BE THE GREATER OF A CLASS A LAP OF THE LARGER BAR AND A CLASS B LAP OF THE SMALLER BAR.

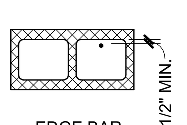


CONCRETE MASONRY UNIT REINFORCING LAP LENGTH SCHEDULE

BAR SIZE	LAP LENGTH			
	8" CMU		12" & LARGER CMU	
	CENTER	EDGE	CENTER	EDGE
#3	15"	15"	15"	15"
#4	20"	25"	20"	25"
#5	25"	39"	25"	39"
#6	38"	73"	30"	73"
#7	52"	100"	35"	100"
#8	79"	150"	50"	150"
#9	103"	191"	64"	191"



CENTER BAR

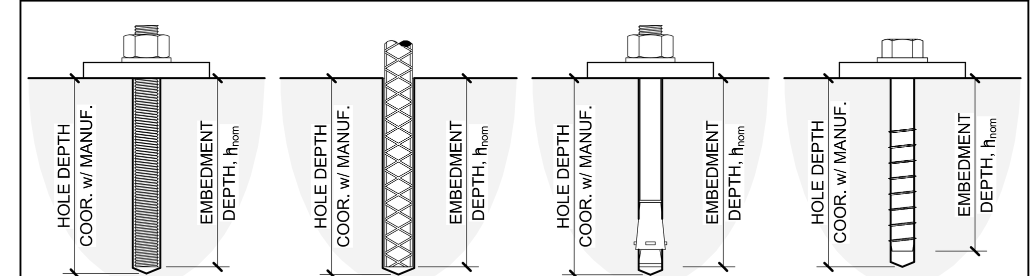


EDGE BAR

NOTES:

1. LAP LENGTHS ARE CALCULATED PER ARTICLE 6 OF TMS 402 FOR GR. 60 REINFORCING AND f'_m = 2,000 PSI WITH A MINIMUM LENGTH OF 40 BAR DIAMETERS.
2. EDGE BARS SHALL HAVE A MINIMUM 1/2" GROUT BETWEEN MASONRY AND REINFORCING BAR.
3. CENTERED BAR LAP LENGTHS SHALL ONLY BE USED WHEN THE MINIMUM REINFORCMENT SPACING IS 8" (ONE PER CELL) AND THE LAP IS CENTERED WITHIN THE CELL.
4. LAP LENGTHS GIVEN IN THE ABOVE TABLE SHALL BE INCREASED BY 50% FOR EPOXY COATED REBAR.
5. CONTACT STRUCTURAL ENGINEER FOR REINFORCING AND CMU SIZES NOT SHOWN.

POST-INSTALLED ANCHOR INFORMATION



ADHESIVE ANCHOR w/ THREADED ROD | **ADHESIVE ANCHOR w/ REINFORCING** | **EXPANSION ANCHOR** | **SCREW ANCHOR**

NOTES:
1. POST INSTALLED ANCHORS SHALL BE ONE OF THE PRODUCTS LISTED IN THE TABLE BELOW, UNLESS A SUBSTITUTION IS APPROVED BY THE STRUCTURAL ENGINEER.
2. THE EMBEDMENT DEPTH SPECIFIED IS THE MINIMUM LENGTH OF THE ANCHOR INTO THE HOLE (NOMINAL EMBEDMENT DEPTH, h_{nom}).
3. COORDINATE THE ACTUAL ANCHOR LENGTH w/ THE SPECIFIED MINIMUM EMBEDMENT DEPTH, ATTACHMENT THICKNESS, WASHERS, ETC.
4. ANCHORS AND REINFORCING STEEL SHALL BE INSTALLED PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII).
5. COORDINATE THE REQUIRED DRILL BIT TYPE, NOMINAL DIAMETER, AND HOLE DEPTH w/ THE ANCHOR MANUFACTURER AND THE CODE EVALUATION REPORT. CLEAN THE HOLE AS REQUIRED BY THE MANUFACTURER AND CODE EVALUATION REPORT REQUIREMENTS FOR THE SELECTED INSTALLATION METHOD.
6. DO NOT DAMAGE ANY EXISTING REINFORCING, INCLUDING PT TENDONS, DURING THE INSTALLATION PROCESS OF POST-INSTALLED ANCHORS.
7. ANCHORS SHALL BE INSTALLED WITHIN 6° OF PERPENDICULAR TO THE CONCRETE OR MASONRY SURFACE, UNLESS NOTED OTHERWISE.
8. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS. IF NO SPACING OR EDGE DISTANCES ARE SPECIFIED ON THE STRUCTURAL DRAWINGS, REFER TO APPLICABLE EVALUATION REPORT FOR CRITICAL SPACING AND EDGE DISTANCES.
9. REFERENCE THE STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
10. MINIMUM EMBEDMENT DEPTHS SHALL BE THE FOLLOWING, UNLESS NOTED OTHERWISE:
    8 × ANCHOR DIAMETER FOR EXPANSION AND SCREW ANCHORS
    12 × ANCHOR DIAMETER OR REINFORCING DIAMETER FOR ADHESIVE ANCHORS.

PERMISSIBLE POST-INSTALLED ANCHOR PRODUCTS

ANCHOR TYPE	MANUFACTURER	PRODUCT NAME	BASE MATERIAL		
			CONCRETE	SOLID GROUTED CMU	HOLLOW CMU
EXPANSION ANCHOR	SIMPSON STRONG-TIE	STRONG-BOLT 2	ICC ESR-3037	IAPMO-UES ER-240	-
	HILTI	KWIK BOLT-TZ2	ICC ESR-4266	ICC ESR-4561	-
	DEWALT	POWER-STUD+ SD2	ICC ESR-2502	-	-
		POWER-STUD+ SD1	-	ICC ESR-2966	-
SCREW ANCHOR	SIMPSON STRONG-TIE	TITEN-HD	ICC ESR-2713	ICC ESR-1056	ICC ESR-1056
	HILTI	KWIK HUS-EZ	ICC ESR-3027	ICC ESR-3056	-
		KWIK-CON II	-	-	NO REPORT
	DEWALT	SCREW-BOLT+	ICC ESR-3889	ICC ESR-4042	-
		TAPPER +	-	-	ICC ESR-3196
SHALLOW ANCHOR	HILTI	HDI-P-TZ	ICC ESR-4236	-	-
	DEWALT	MINI-UNDERCUT+	ICC ESR-3912	-	-
ADHESIVE ANCHORS	SIMPSON STRONG-TIE	SET-3G	ICC ESR-4057	ICC ESR-4844	ICC ESR-4844
		AT-3G FAST-CURE	ICC ESR-5026	-	-
	HILTI	HIT-RE 500 V3 SLOW CURE	ICC ESR-3814	-	-
		HIT-HY 200 V3 FAST CURE	ICC ESR-4868	-	-
		HIT-HY 200 V3	-	ICC ESR-4878	-
		HIT-HY 270	-	ICC ESR-4143	ICC ESR-4143
	DEWALT	PURE110+	ICC ESR-3298	-	-
		AC100+ GOLD	-	ICC ESR-3200	ICC ESR-3200

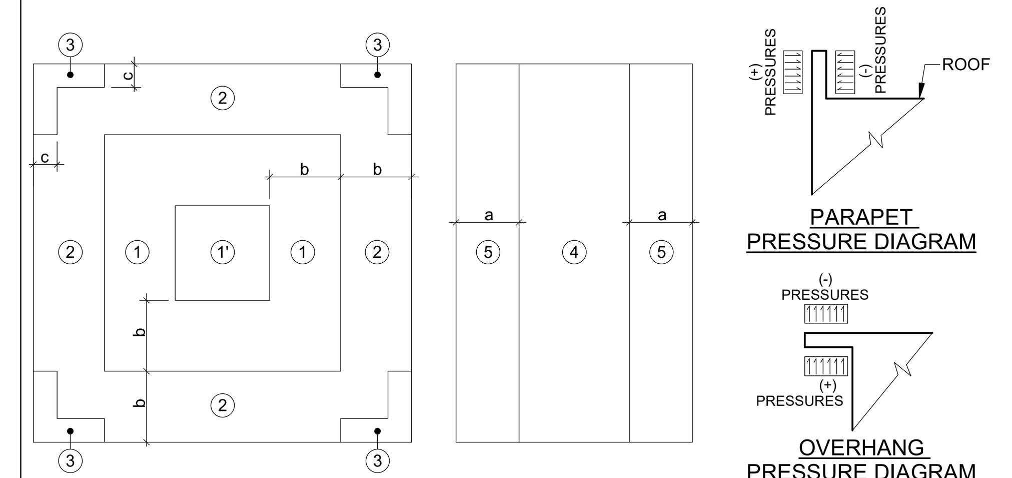
TABLE NOTES:

1. ANCHOR PRODUCT MUST HAVE AN EVALUATION REPORT LISTED TO BE USED IN THE APPLICABLE BASE MATERIAL. IF A "-" IS LISTED IN THE TABLE ABOVE, THEN THE ANCHOR PRODUCT IS NOT PERMITTED IN THAT BASE MATERIAL.
2. ANCHORS IN EXTERIOR AND CORROSIVE ENVIRONMENTS SHALL BE APPROVED BY THE MANUFACTURER FOR THE EXPOSURE.
3. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE SPECIFIED IN THIS TABLE SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER ALONG WITH CALCULATIONS THAT ARE SIGNED AND SEALED BY A LICENSED PROFESSIONAL ENGINEER DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING EQUIVALENT PERFORMANCE (MINIMUM) OF THE SPECIFIED PRODUCTS. THE SUBSTITUTION REQUEST SHALL INCLUDE CODE EVALUATION REPORTS STATING THAT THE PRODUCTS ARE APPROVED FOR THE INTENDED USE AND COMPLIANT WITH THE APPLICABLE BUILDING CODE. THE CALCULATIONS SHALL USE THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S) AS REQUIRED BY THE BUILDING CODE.

COMPONENT AND CLADDING WIND PRESSURE DIAGRAM

(PER ASCE 7-16, CHAPTER 30, PART 1 AND PART 6)

H≤60FT



ROOF PRESSURE DIAGRAM

WALL PRESSURE DIAGRAM

PARAPET PRESSURE DIAGRAM

OVERHANG PRESSURE DIAGRAM

PARAMETERS:
BASIC WIND SPEED: 182 MPH
RISK CATEGORY IV
a = 5'-6"
b = 8'-4"
c = 2'-10"

EFFECTIVE AREA (ft^2) - SEE NOTE 2	PRESSURE (PSF) - SEE NOTES 1 - 4										
	ROOF ZONE				WALLS		OVERHANGS			PARAPETS	
	ZONE 1'	ZONE 1	ZONE 2	ZONE 3	ZONE 4	ZONE 5	ZONE 1'/1	ZONE 2	ZONE 3	ZONE 4	ZONE 5
≤ 10	+29, -66	+29, -115	+29, -152	+29, -207	+66, -72	+66, -88	+16, -104	+16, -141	+16, -196	+215, -127	+275, -145
20	+27, -66	+27, -107	+27, -142	+27, -187	+63, -69	+63, -82	+16, -102	+16, -128	+16, -173	+201, -120	+250, -135
50	+25, -66	+25, -97	+25, -129	+25, -161	+59, -65	+59, -74	+16, -100	+16, -110	+16, -143	+182, -112	+218, -122
100	+23, -66	+23, -90	+23, -119	+23, -142	+56, -62	+56, -69	+16, -98	+16, -97	+16, -120	+168, -105	+193, -113
200	+23, -57	+23, -82	+23, -109	+23, -122	+53, -59	+53, -63	+16, -82	+16, -84	+16, -97	+154, -99	+169, -103
≥ 500	+23, -45	+23, -72	+23, -97	+23, -97	+50, -55	+50, -55	+16, -61	+16, -67	+16, -67	+136, -90	+136, -90

1. (+) AND (-) SIGNS INDICATED PRESSURES ACTING TOWARDS AND AWAY FROM THE SURFACE, RESPECTIVELY. SEE THE PARAPET AND OVERHANG DIAGRAMS FOR PRESSURE DIRECTION ON THOSE ELEMENTS.
2. USE THE PRESSURE ASSOCIATED WITH THE CLOSEST EFFECTIVE AREA THAT DOES NOT EXCEED THE ACTUAL EFFECTIVE AREA. ALTERNATIVELY, LINEAR INTERPOLATION IS PERMITTED.
3. IF A PARAPET ≥ 3'-0" IN HEIGHT IS PROVIDED AROUND THE PERIMETER OF A FLAT ROOF:
   THE POSITIVE VALUES FOR ZONE 3 SHALL BE EQUAL TO THOSE FOR WALL ZONE 5.
   THE POSITIVE VALUES FOR ZONE 2 SHALL BE EQUAL TO THOSE FOR WALL ZONE 4.
4. THE WIND PRESSURES USED TO DESIGN ANY DEFERRED SUBMITTAL ELEMENT SHALL BE CALCULATED BY THE REGISTERED PROFESSIONAL ENGINEER DESIGNING THOSE ELEMENTS. NOTIFY THE STRUCTURAL ENGINEER OF ANY DISCREPANCIES.

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SCALE: 3"=1'-0" | SCALE: 1-1/2"=1'-0" | SCALE: 1"=1'-0" | SCALE: 3/4"=1'-0" | SCALE: 1/2"=1'-0" | SCALE: 1/4"=1'-0" | SCALE: 1/8"=1'-0" | SCALE: 3/32"=1'-0" | SCALE: 1/16"=1'-0" | SCALE: 1/32"=1'-0"

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AHCA Client Code/File Number: 23/23960220-19206

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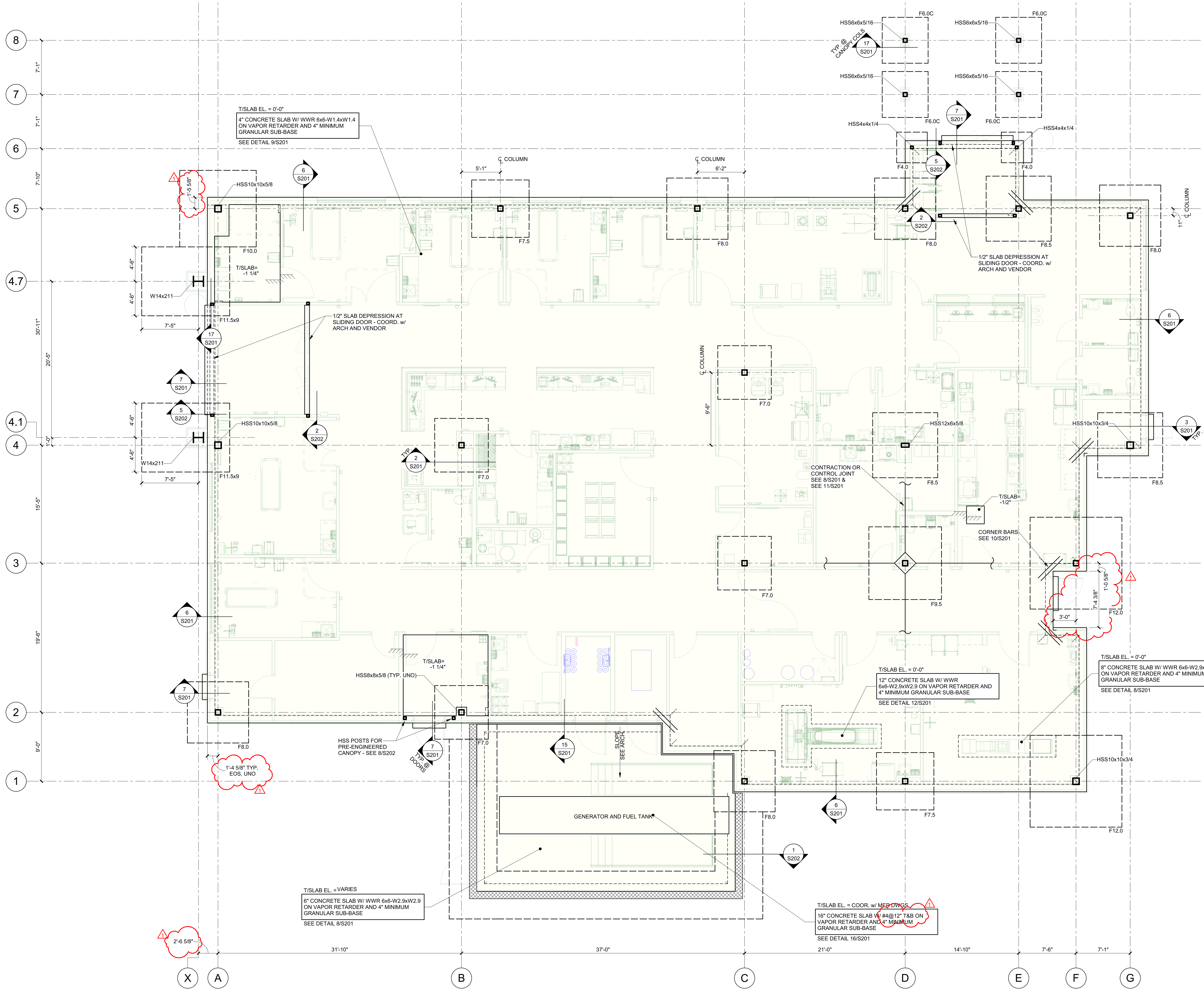
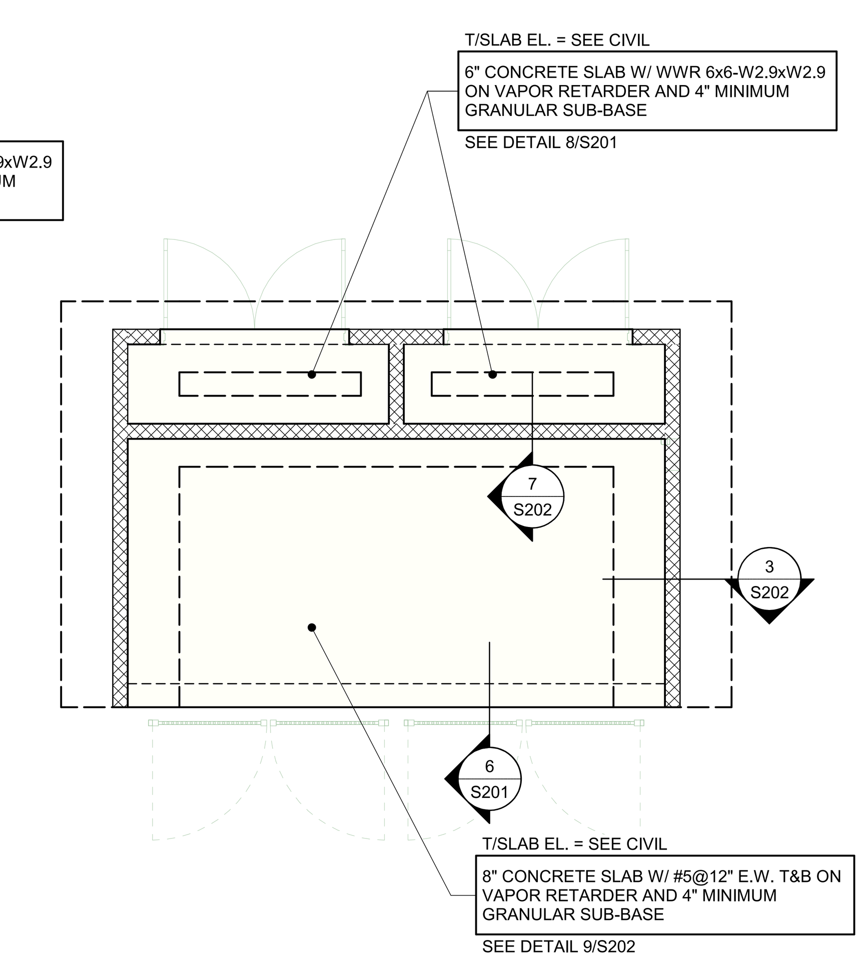
STRUCTURAL NOTES

S003





LEVEL 1 FOUNDATION PLAN

0' 2'-8" 5'-4" 10'-8"

SHEET NOTES:

1. REFERENCE ELEVATON 0'-0" = 28.50' (REFER TO CIVIL DRAWINGS).
2. TOP OF FOUNDATION ELEVATIONS ARE 2'-0" BELOW TYPICAL FINISHED FLOOR ELEVATION UNLESS NOTED OTHERWISE.
3. FOUNDATION ELEMENTS ARE CENTERED UNDER COLUMNS UNLESS NOTED OTHERWISE.
4. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN. COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
5. REFER TO ARCHITECTURAL DRAWINGS FOR EDGE OF SLAB DIMENSIONS.
6. TOP OF SLAB ELEVATIONS ARE APPROXIMATE. REFER TO ARCHITECTURAL DRAWINGS FOR TOP OF SLAB ELEVATIONS AND SLOPES.
7. COORDINATE ALL SLAB DEPRESSIONS WITH MEP AND ARCHITECTURAL DRAWINGS.
8. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS AND EXTENTS OF NON-LOAD BEARING CMU WALLS. COORDINATE WITH STRUCTURAL DETAILS.
9. ELEVATOR BRACING AND RAIL SUPPORT REQUIREMENTS OF THE ELEVATOR BEING SUPPLIED SHALL BE COORDINATED BY THE GENERAL CONTRACTOR. NOTIFY THE STRUCTURAL ENGINEER OF ANY INCONSISTENCIES.
10. CONTRACTOR SHALL FIELD VERIFY EXISTING CONDITIONS AND DIMENSIONS RELATED TO THE NEW STRUCTURE PRIOR TO FABRICATION AND CONSTRUCTION. NOTIFY ARCHITECT/STRUCTURAL ENGINEER OF ANY DISCREPANCIES.
11. SHEET AND TYPICAL DETAIL REFERENCES:
    S0 SERIES STRUCTURAL NOTES
    1/S201 FOOTING SCHEDULE
    8/S201 SLAB ON GROUND
    S251 CMU WALLS
12. MARK AND LEGEND KEY:
    F#.# FOOTING

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PLAN NORTH
TRUE NORTH

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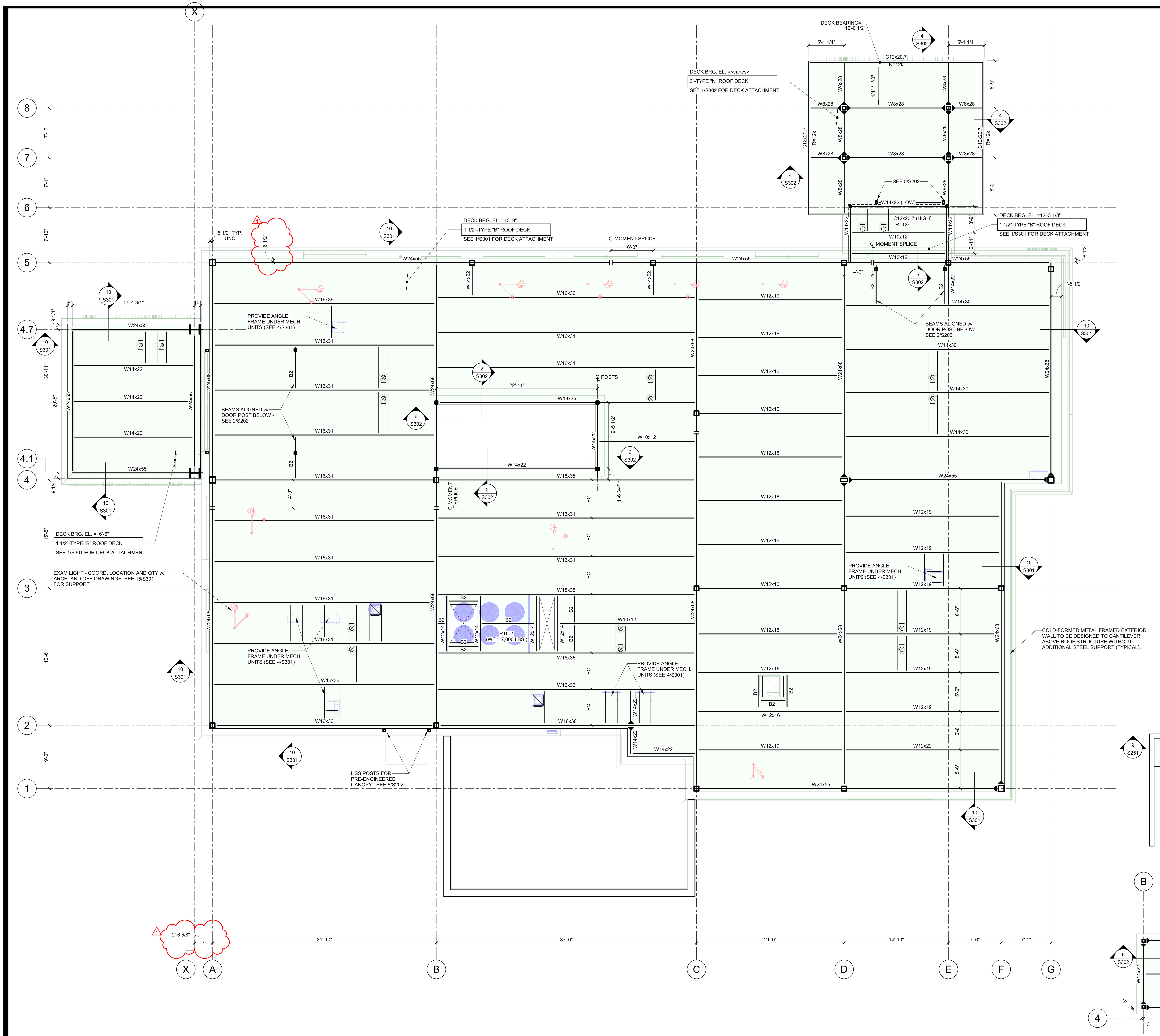
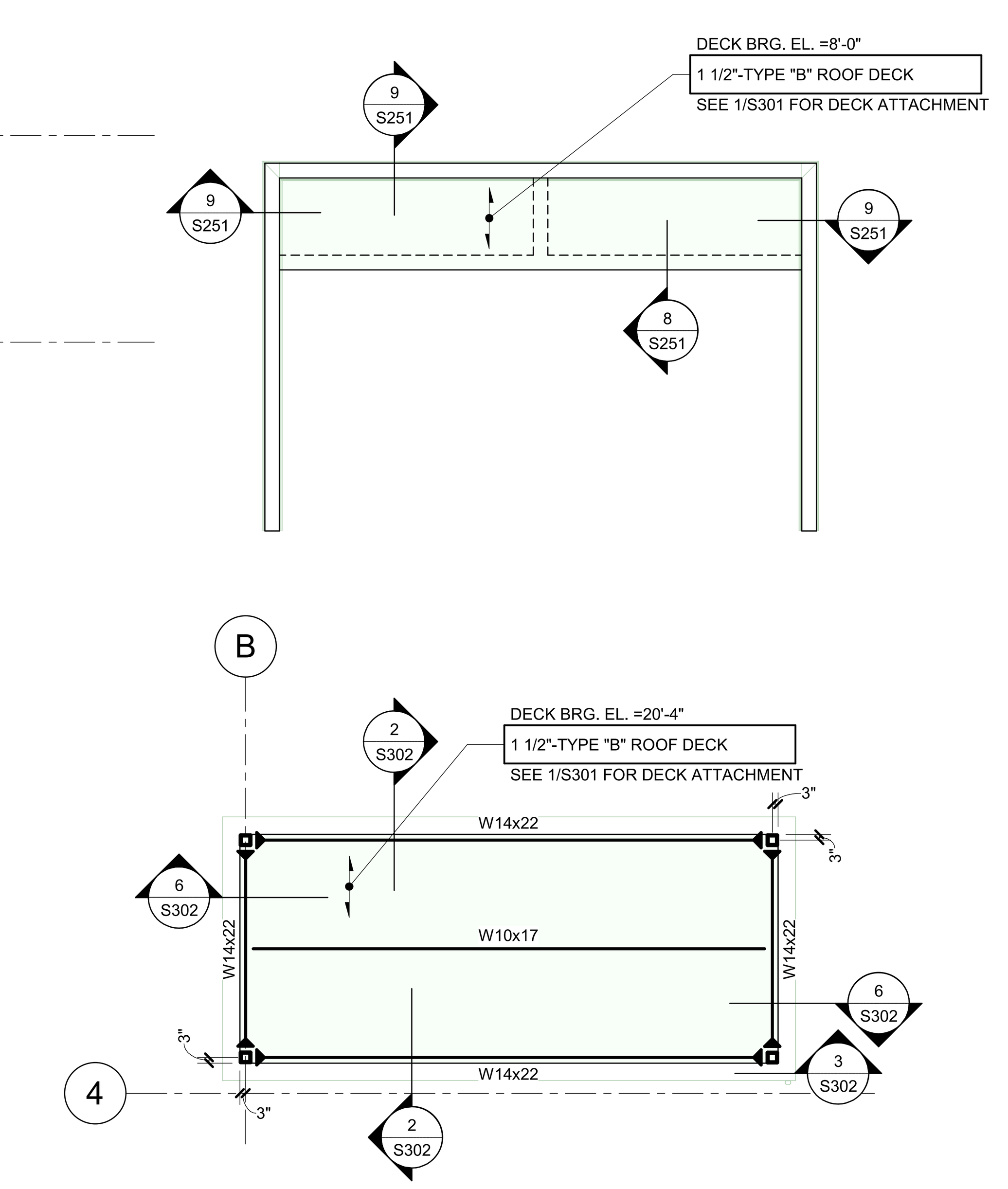
LEVEL 1 FOUNDATION PLAN

S101



ROOF FRAMING PLAN

0' 2'-8" 5'-4" 10'-8"



CLERESTORY FRAMING PLAN

0' 4'-0" 8'-0" 16'-0"

SHEET NOTES:

1. ALL BEAMS SHALL BE SPACED EQUALLY IN COLUMN BAYS, UNLESS NOTED OTHERWISE.
2. EDGE OF SLAB/BENT PLATE IS LOCATED 6" OFF OF BEAM CENTERLINES AT INTERIOR OPENINGS, UNLESS NOTED OTHERWISE.
3. REFER TO FOUNDATION PLAN AND ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN. COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
4. COORDINATE ALL SLAB OPENINGS AND DEPRESSIONS WITH MEP AND ARCHITECTURAL DRAWINGS.
5. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS AND EXTENTS OF NON-LOAD BEARING CMU WALLS. COORDINATE WITH STRUCTURAL DETAILS.
6. WEIGHTS SHOWN FOR MECHANICAL EQUIPMENT ARE MAXIMUMS. NOTIFY STRUCTURAL ENGINEER IF ACTUAL WEIGHTS EXCEED THE VALUES GIVEN.
7. COORDINATE THE LOCATION OF FRAMING SUPPORTING MECHANICAL EQUIPMENT WITH THE EQUIPMENT SUPPLIER. NOTIFY STRUCTURAL ENGINEER IF EQUIPMENT SUPPORT REQUIREMENTS DIFFER FROM WHAT IS SHOWN IN THE STRUCTURAL DOCUMENTS.
8. SHEET AND TYPICAL DETAIL REFERENCES:

S0 SERIES	STRUCTURAL NOTES
13/S301	W-SHAPE TO HSS MOMENT CONNECTIONS
8/S301	BEAM TO BEAM MOMENT CONNECTIONS
S3 SERIES	FRAMING SECTIONS AND DETAILS
S251	CMU WALLS

9. MARK AND LEGEND KEY:
   MOMENT CONNECTION
10. ULTIMATE (LRFD) REACTIONS FOR STEEL BEAMS SUPPORTING NON-COMPOSITE DECK SHALL BE AS FOLLOWS, UNLESS NOTED OTHERWISE:

SIZE	REACTION	SIZE	REACTION	SIZE	REACTION
W8	12 KIPS	W16	30 KIPS	W24	48 KIPS
W10	12 KIPS	W18	30 KIPS	W27	66 KIPS
W12	12 KIPS	W21	48 KIPS	W30	66 KIPS
W14	30 KIPS				

11. STEEL BEAM SCHEDULE:

MARK	SIZE	ULTIMATE (LRFD) REACTION
B1	W8x10	12 KIPS
B2	W10x12	12 KIPS
B3	W12x14	12 KIPS
C1	C8X11.5	12 KIPS

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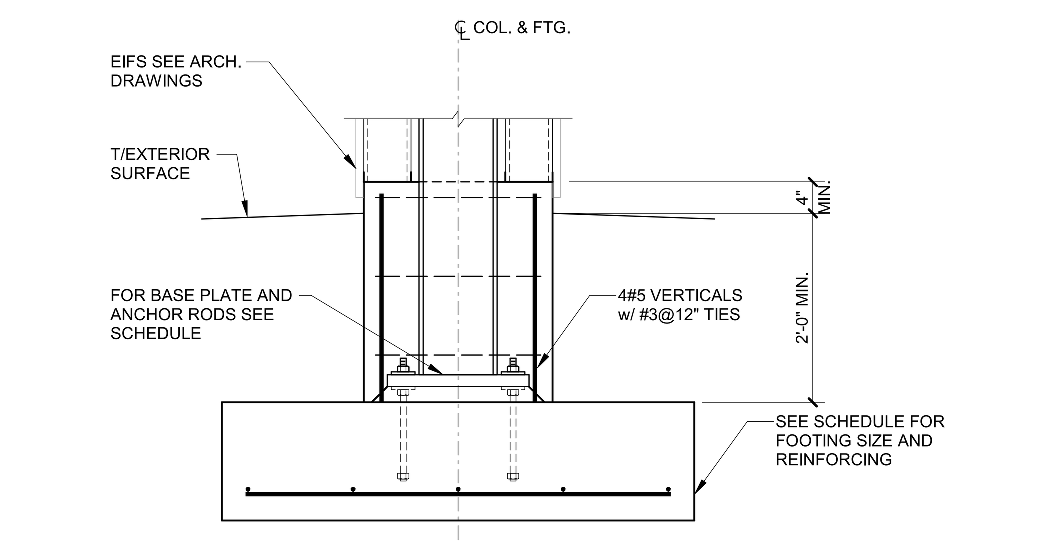
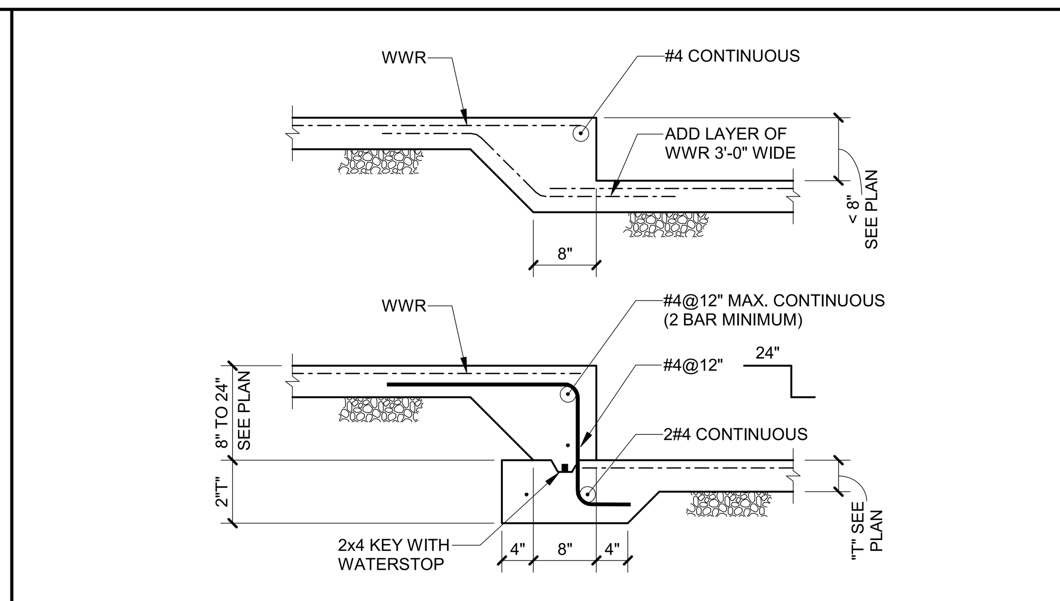
ROOF FRAMING PLAN

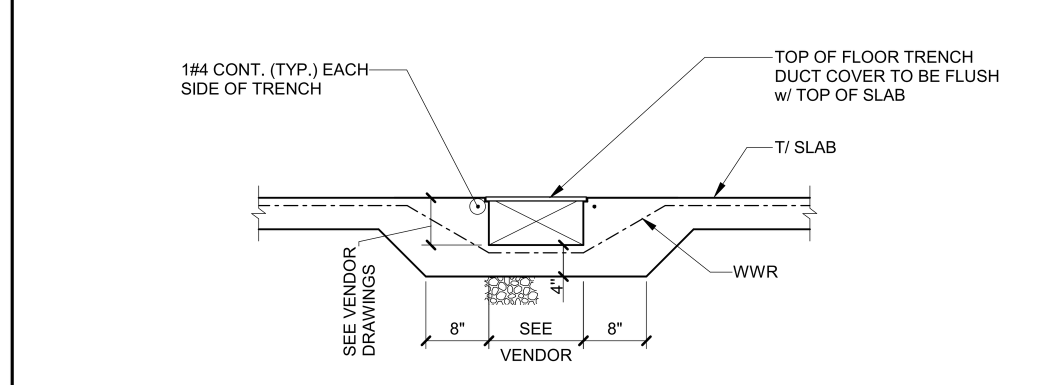
S102



17 SECTION AT CANOPY COLUMN



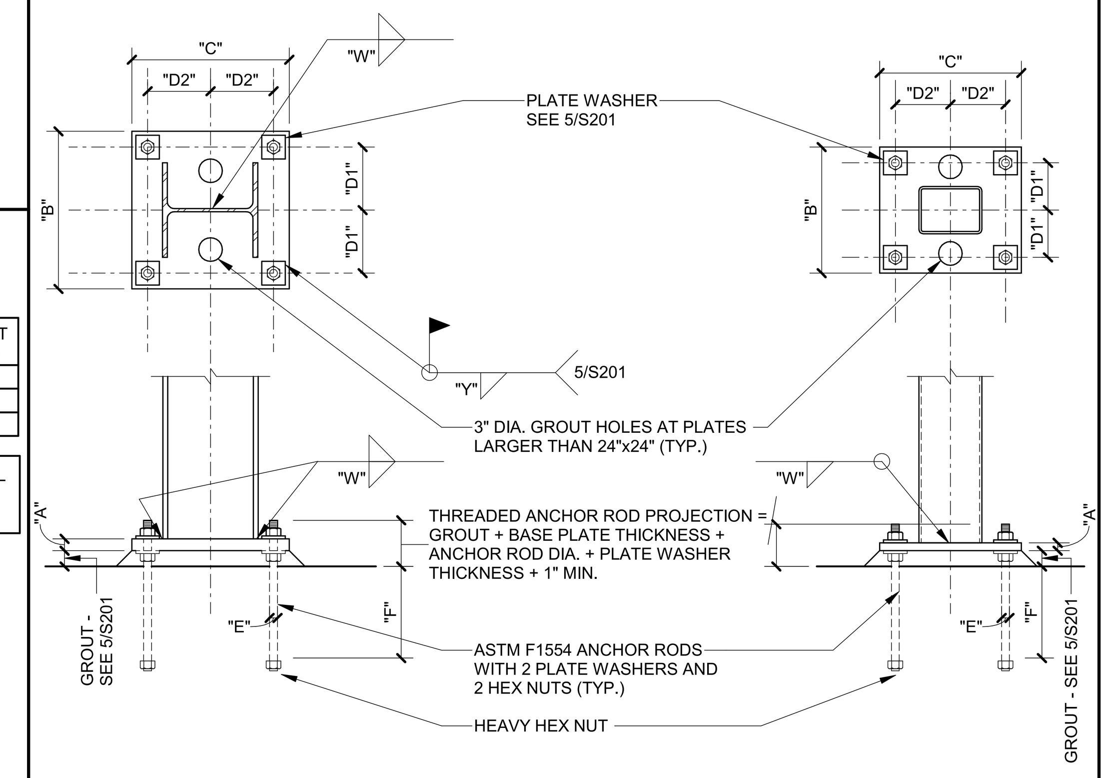
13 TYP. DEPRESSED SLAB ON GROUND DETAILS



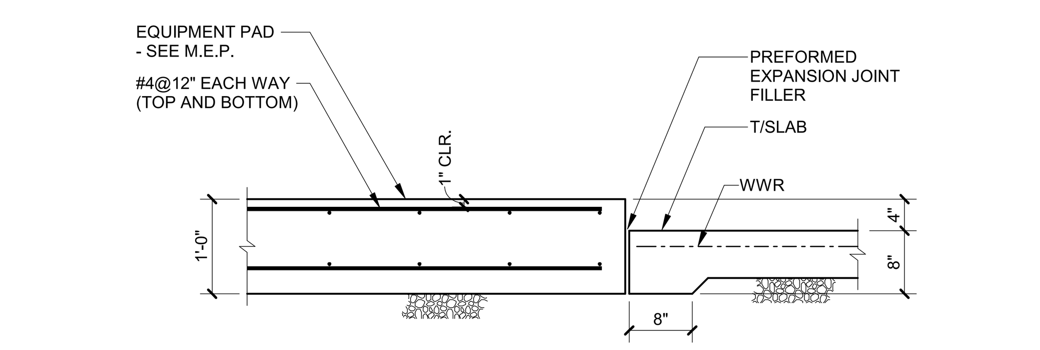
9 TYPICAL SECTION THROUGH NEW FLOOR TRENCH DUCT

BASE PLATE & ANCHOR ROD SCHEDULE									
COLUMN SIZE	BASE PLATE					ANCHOR RODS			WELD
	A	B	C	D1	D2	E	F	№	W
HSS4x4x1/4	1"	12"	12"	4"	4"	1"	12"	4	1/4
HSS6x6x5/16	1"	16"	16"	5 3/4"	5 3/4"	1 1/4"	15"	4	3/4
HSS8x8x5/8	1 3/4"	24"	24"	9 1/4"	9 1/4"	1 1/2"	18"	4	CJP
HSS10x10	2 1/4"	28"	28"	11 1/4"	11 1/4"	1 3/4"	18"	4	CJP
HSS12x6x5/8	2"	24"	24"	9 1/4"	9 1/4"	1 3/4"	20"	4	CJP
W14x211	2"	28"	28"	11 1/4"	11 1/4"	1 1/2"	18"	4	CJP

1

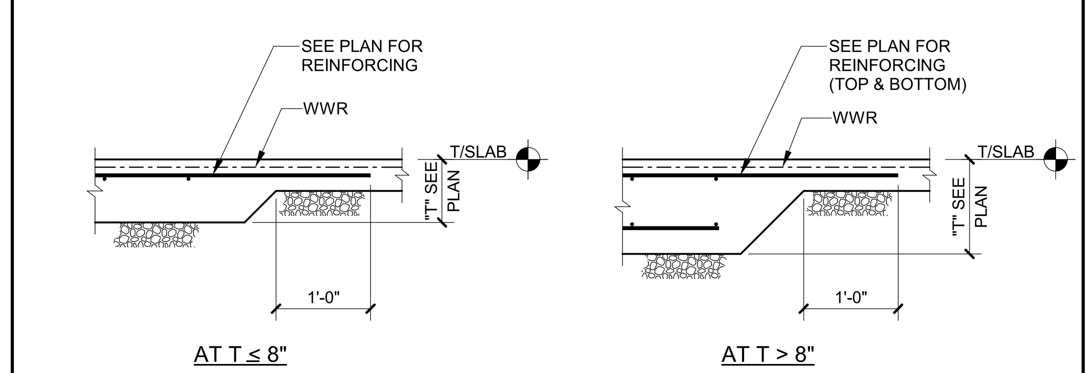


4 BASE PLATE AND ANCHOR ROD SCHEDULE



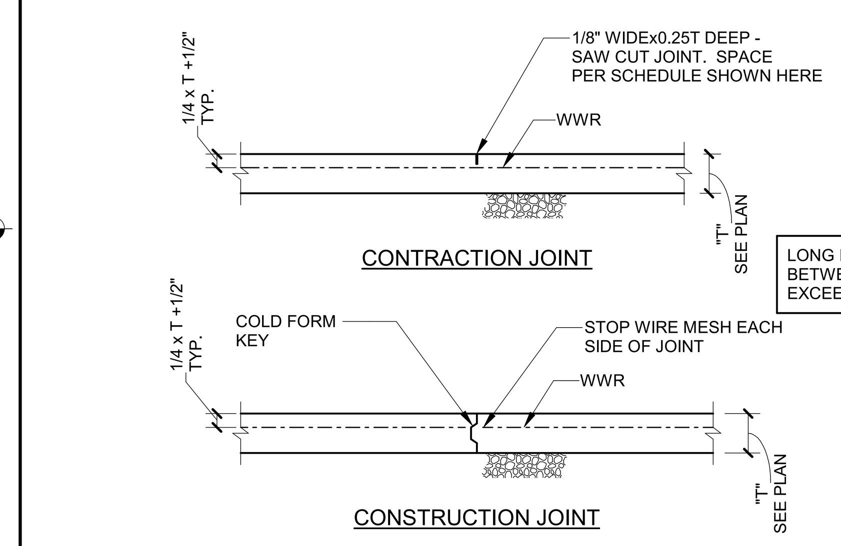
COORDINATE SIZE AND LOCATION WITH M.E.P.

16 TYPICAL EQUIPMENT PAD ON GROUND SECTION



NOTE: OFFSET CONTRACTION JOINT TO AVOID INTERSECTING THICKENED SLAB AREA

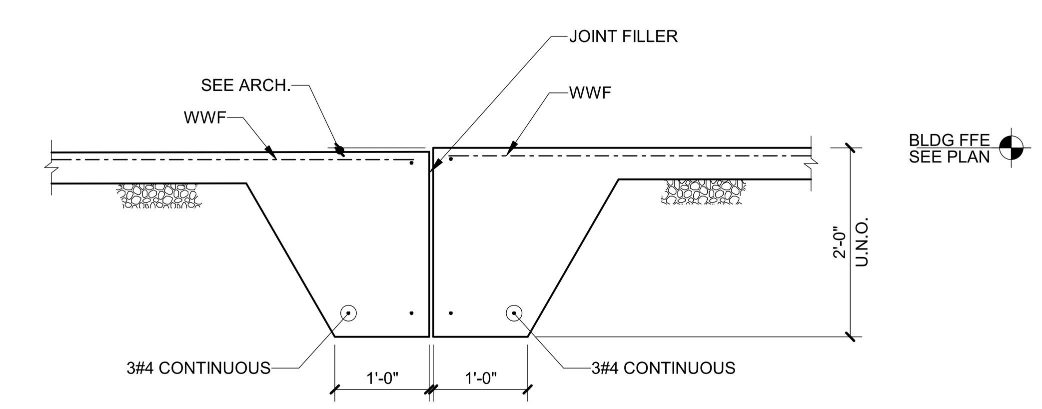
12 TYPICAL THICKENED SLAB ON GROUND DETAILS



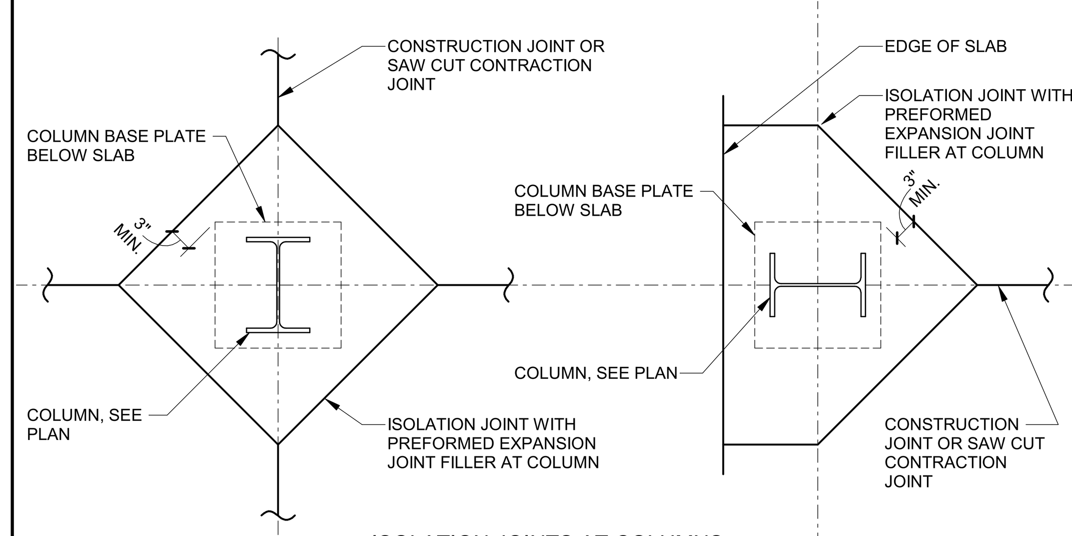
SLAB DEPTH "T"	MAX JOINT SPACING
4"	12'-0"
5"	15'-0"
>6"	18'-0"

LONG DIMENSION OF SLAB PANEL BETWEEN JOINTS SHALL NOT EXCEED 1.5 × SHORT DIMENSION

8 TYPICAL SLAB ON GROUND DETAILS

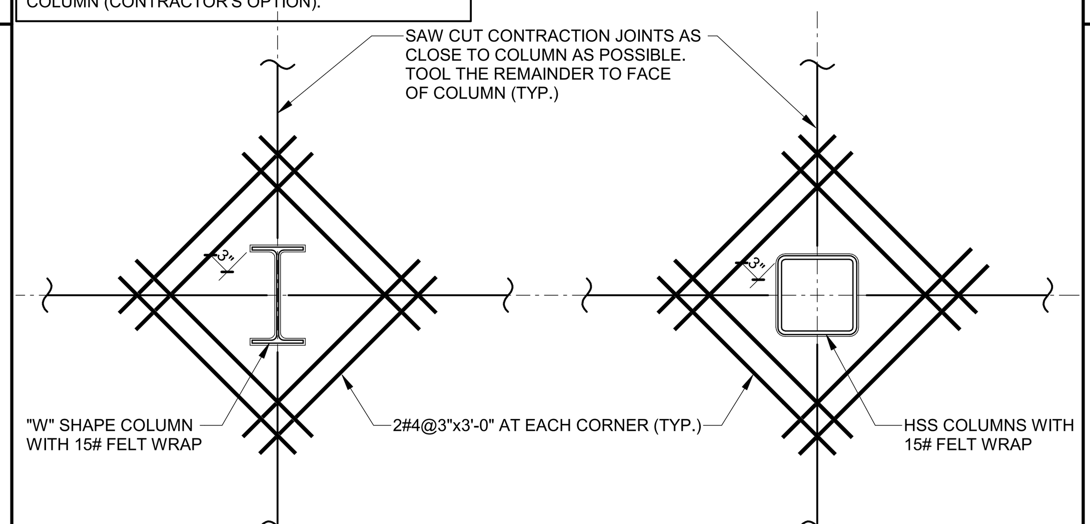


15 SECTION AT GENERATOR ENCLOSURE



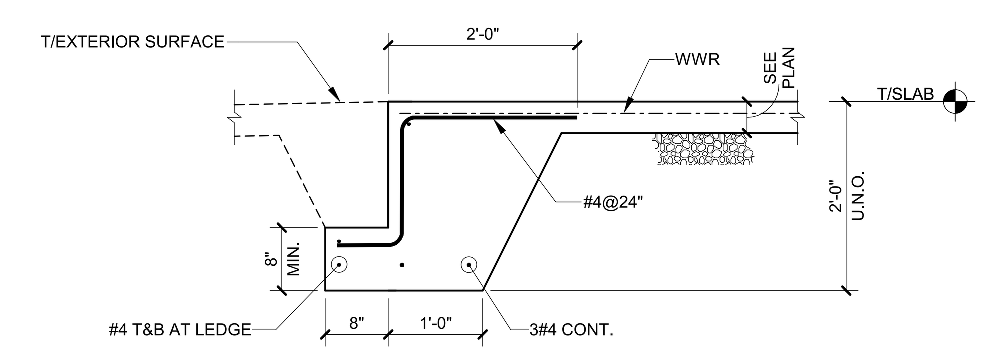
ISOLATION JOINTS AT COLUMNS

NOTE:
ISOLATION OF COLUMNS IS REQUIRED. PROVIDE EITHER ISOLATION JOINTS OR FELT WRAP AT COLUMN (CONTRACTOR'S OPTION).

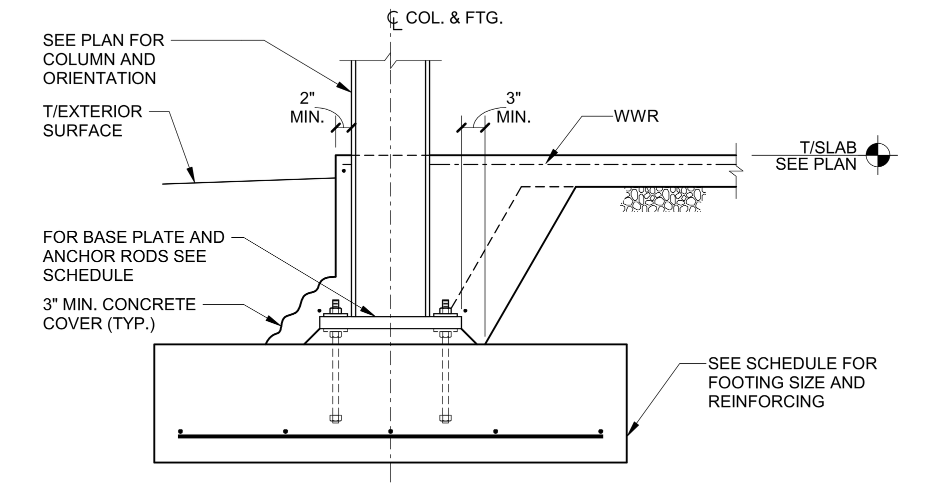


FELT WRAPPED COLUMNS

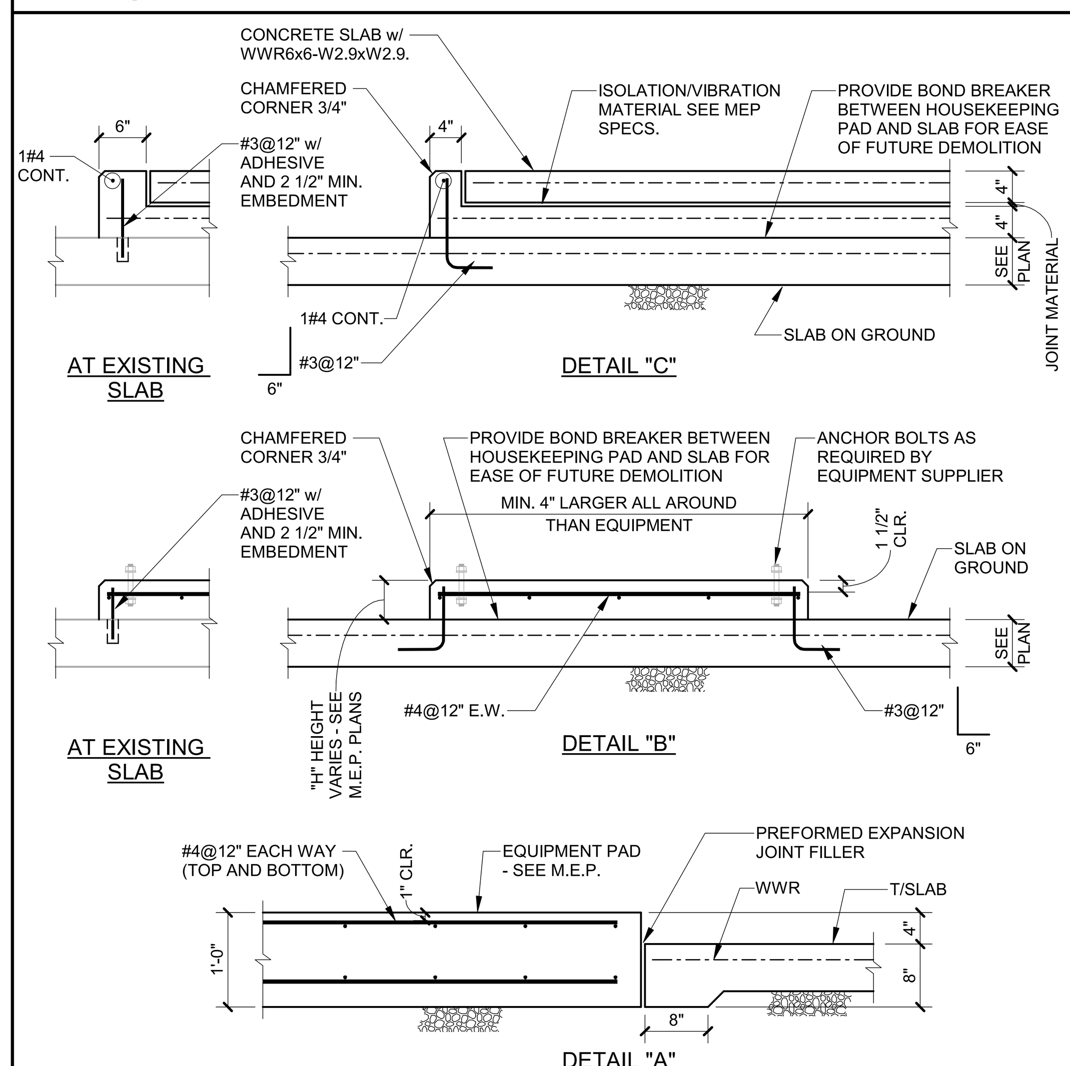
11 TYPICAL SLAB ON GROUND JOINT DETAILS AT STEEL COLUMNS



7 TYPICAL TURNED DOWN SLAB AT ENTRANCES

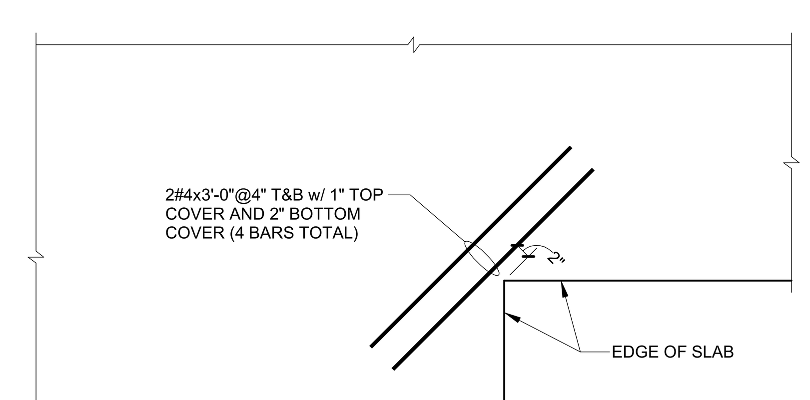


3 TYPICAL EXTERIOR COLUMN FOOTING DETAIL

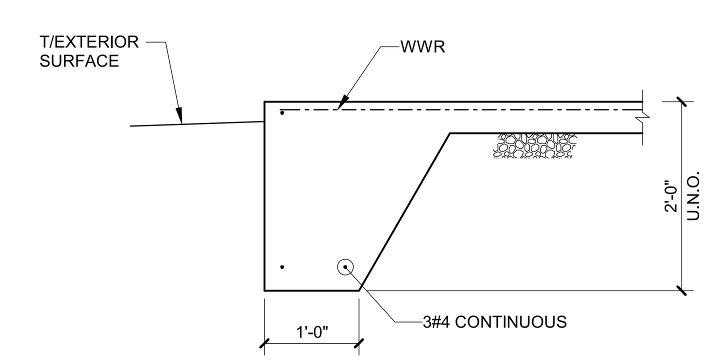


NOTES:
1. THE CONTRACTOR SHALL PROVIDE CONCRETE PADS ADEQUATE FOR THE SUPPORT OF THE M.E.P. EQUIPMENT. THE HEIGHT "H" OF MECHANICAL EQUIPMENT PADS SHALL BE COORDINATED WITH THE ARCHITECTURAL AND M.E.P. DRAWINGS.
2. EXACT SIZES, LOCATIONS, HEIGHT, AND ANY SPECIAL DETAILS FOR THE PADS SHALL BE OBTAINED FROM THE VENDORS BEFORE INSTALLATION OF THE PADS.
3. ALL EMBEDDED ITEMS SHALL BE COORDINATED WITH THE EQUIPMENT SUPPLIER.
4. USE DETAIL "C" ONLY UNDER EQUIPMENT SUPPORTED ON FLOATING SLABS. SEE MEP DRAWINGS FOR LOCATIONS.

14 TYPICAL EQUIPMENT PAD AND CURB DETAILS



10 TYPICAL REINFORCING AT SLAB ON GROUND RE-ENTRANT CORNERS

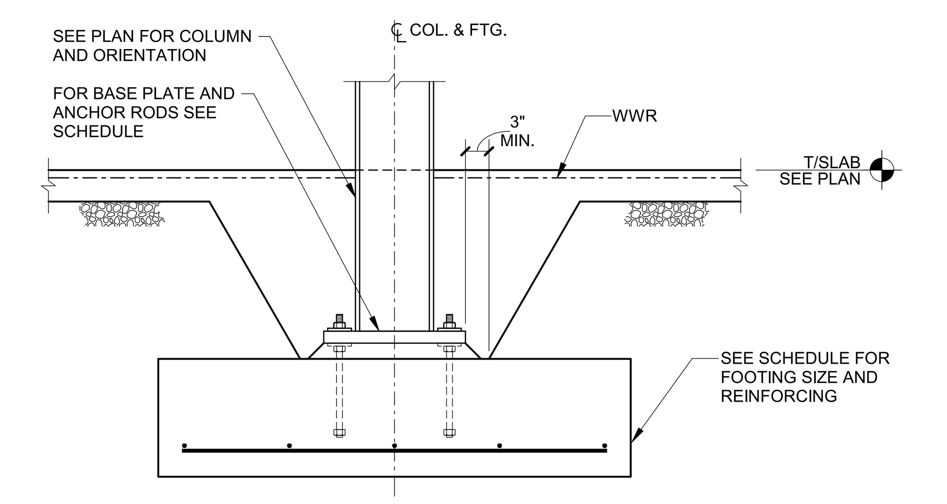


6 TYPICAL TURNED DOWN SLAB

ANCHOR ROD DIAMETER (INCHES)	MAXIMUM HOLE DIAMETER (INCHES)	MINIMUM WASHER WIDTH (INCHES) NOTE 1	MINIMUM WASHER THICKNESS (INCHES)	BASE PLATE GROUT (INCHES)	WELD, "Y" WASHER TO BASE PLATE (INCHES)
3/4	1 5/16	2	1/4	2	3/16, NOTE 2
7/8	1 9/16	2 1/2	5/16	2	3/16, NOTE 2
1	1 7/8	3	3/8	2	3/16, NOTE 2
1 1/4	2 1/8	3 1/2	1/2	3	3/16
1 1/2	2 3/8	4	1/2	3	3/16
1 3/4	2 7/8	4 1/2	5/8	4	1/4
2	3 1/4	5	3/4	4	5/16
2 1/2	3 3/4	5 1/2	7/8	4	5/16

NOTES:
1. PROVIDE SQUARE PLATE WASHERS CONFORMING TO ASTM A36 WITH STANDARD HOLES.
2. WELDS ARE NOT REQUIRED FOR GRAVITY COLUMNS.

5 PLATE WASHER & GROUT THICKNESS SCHEDULE



2 TYPICAL INTERIOR COLUMN FOOTING DETAIL

FOOTING SCHEDULE						
MARK	LENGTH	WIDTH	THICKNESS	TOP REINFORCEMENT	BOTTOM REINFORCEMENT	REMARKS
F4.0	4'-0"	4'-0"	1'-6"		7#5 E.W.	
F6.0C	6'-0"	6'-0"	2'-6"		9#6 E.W.	
F7.0	7'-0"	7'-0"	2'-0"		9#6 E.W.	
F7.5	7'-6"	7'-6"	2'-0"		9#6 E.W.	
F8.0	8'-0"	8'-0"	2'-0"		10#6 E.W.	
F8.5	8'-6"	8'-6"	2'-0"		11#6 E.W.	
F9.5	9'-6"	9'-6"	2'-0"		8#8 E.W.	
F10.0	10'-0"	10'-0"	2'-6"		9#8 E.W.	
F11.5x9	11'-6"	9'-0"	2'-0"		11#6 LONG BARS, 13#6 SHORT BARS	
F12.0	12'-0"	12'-0"	2'-6"		10#8 E.W.	

1 FOOTING SCHEDULE

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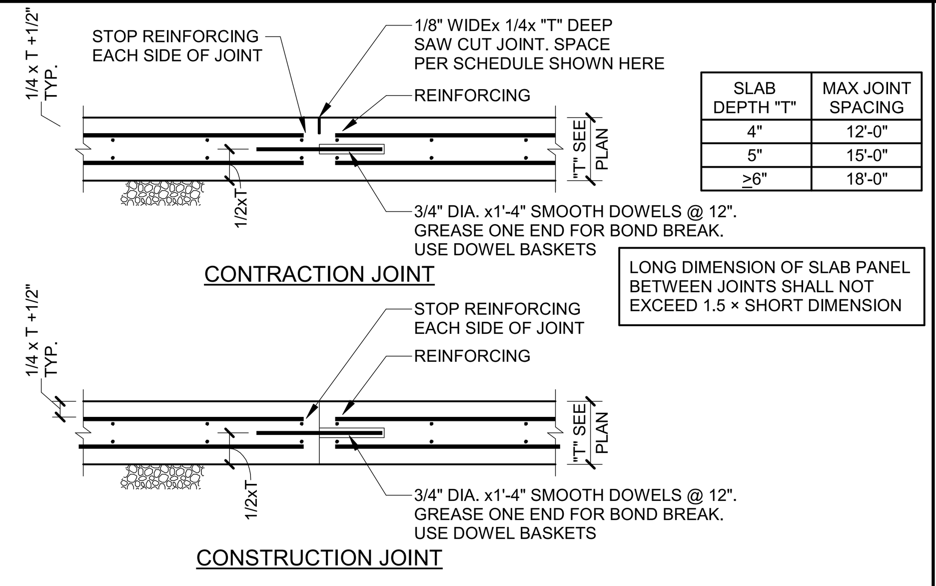
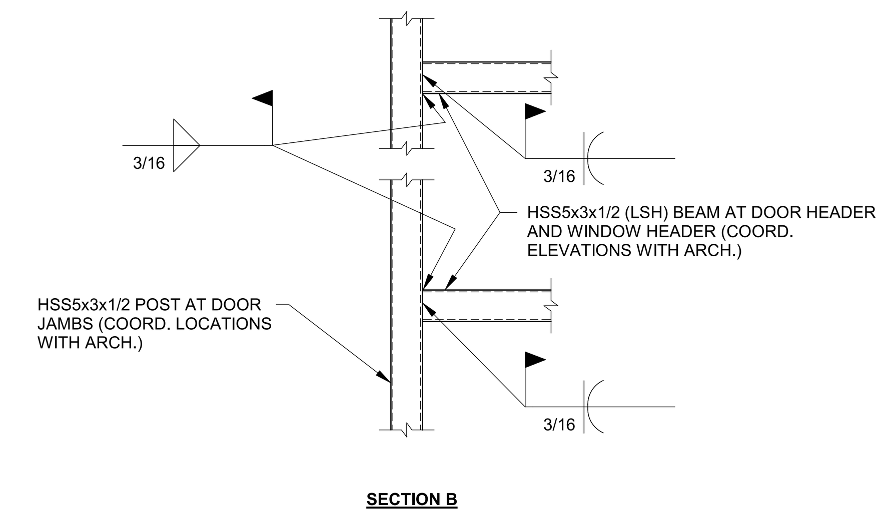
FOUNDATION SECTION AND DETAILS

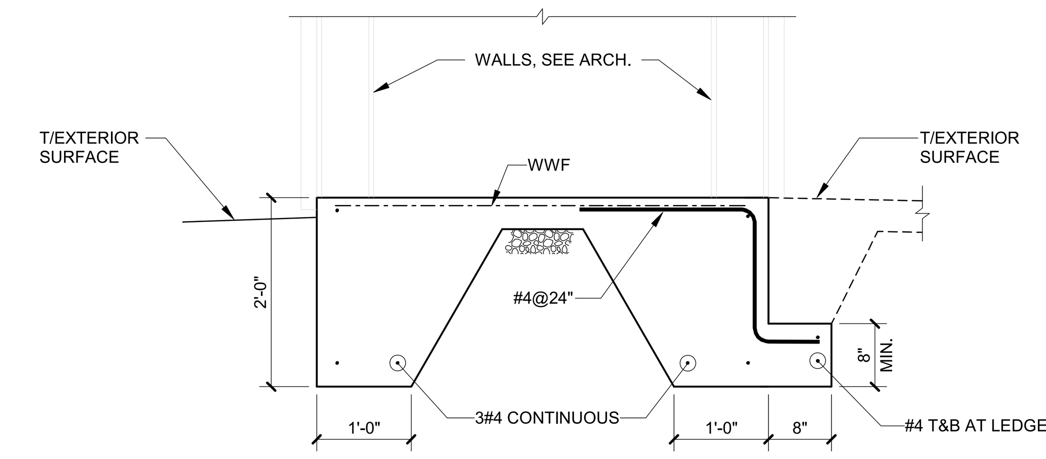
S201



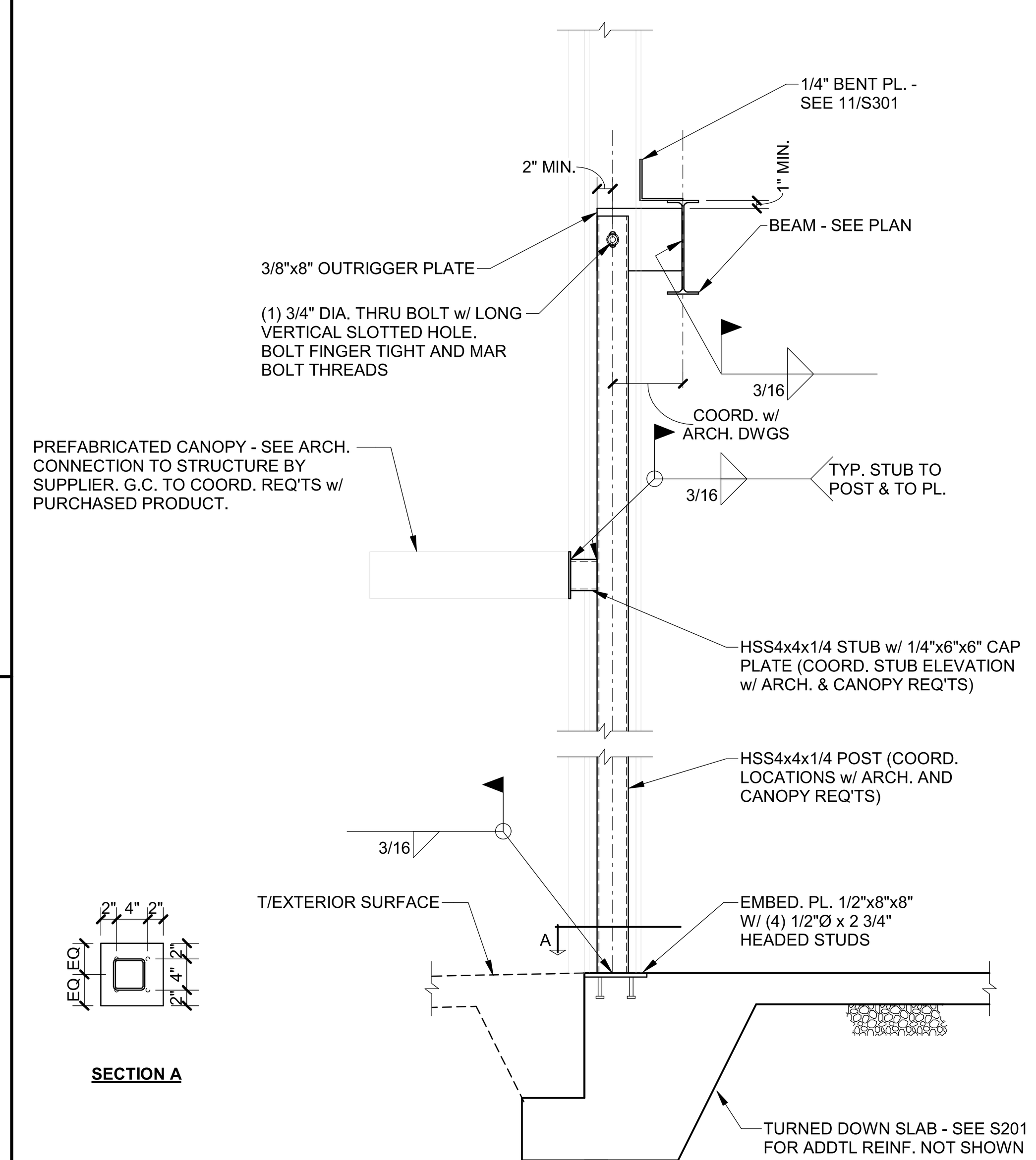
SLAB DEPTH "T"	MAX JOINT SPACING
4"	12'-0"
5"	15'-0"
>6"	18'-0"

9 TYPICAL SLAB ON GROUND DETAILS

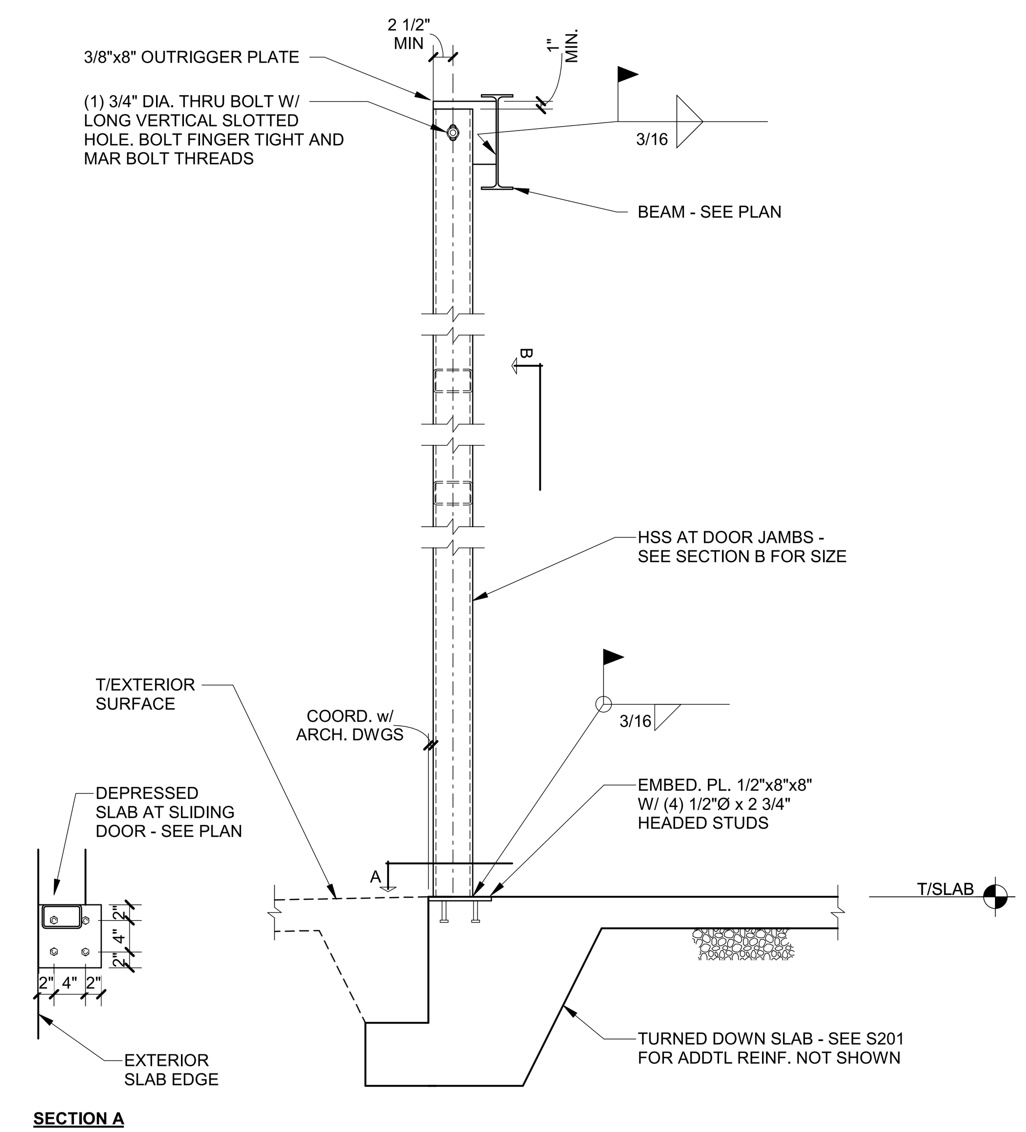




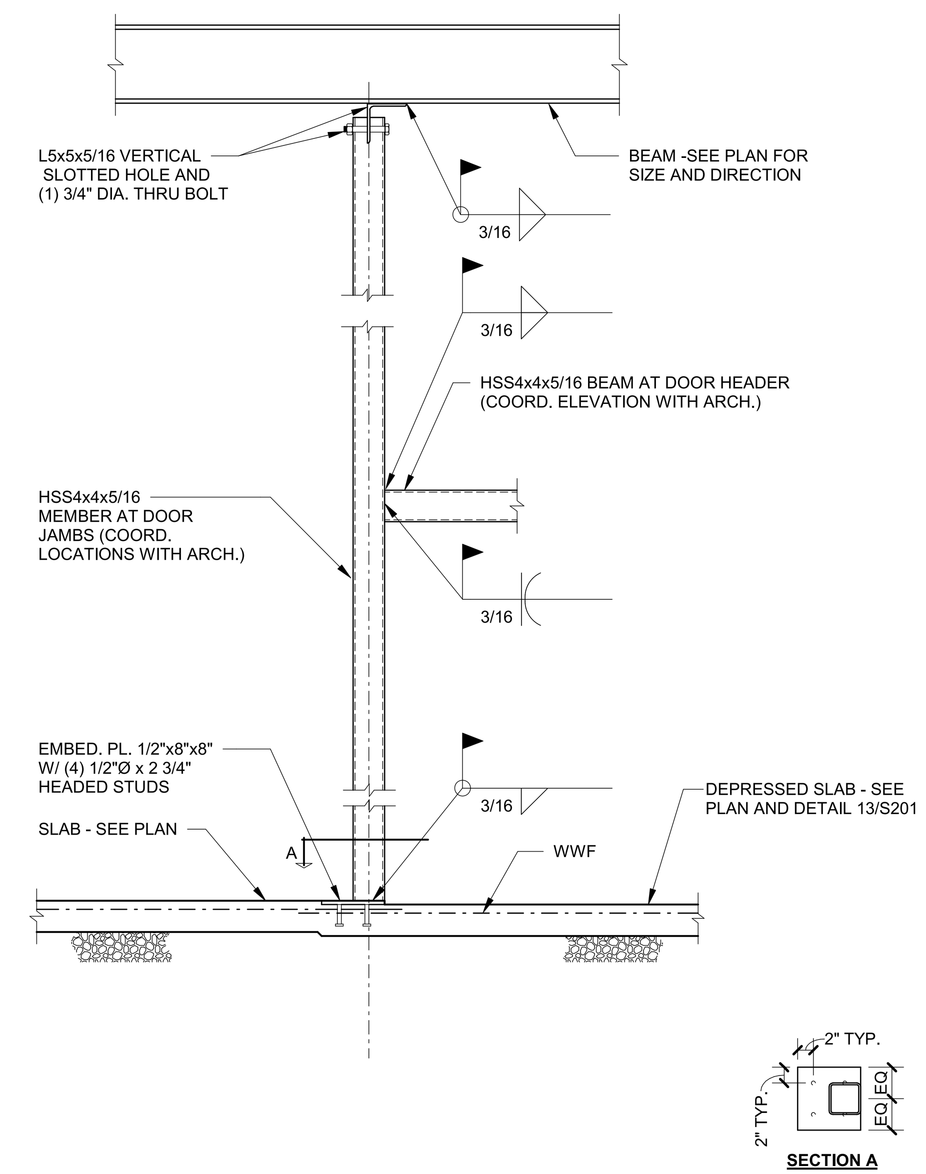
4 SECTION AT AMBULANCE STORAGE



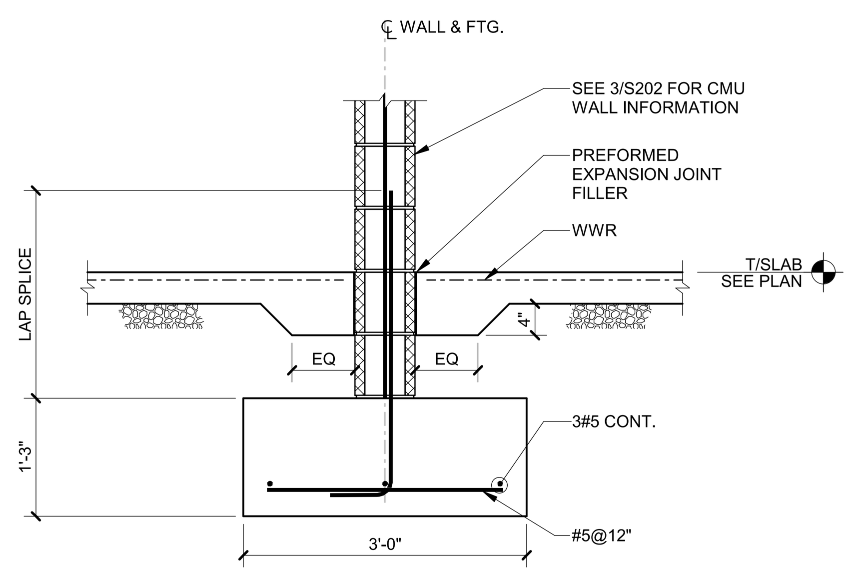
8 TYPICAL SECTION AT PREFABRICATED CANOPY



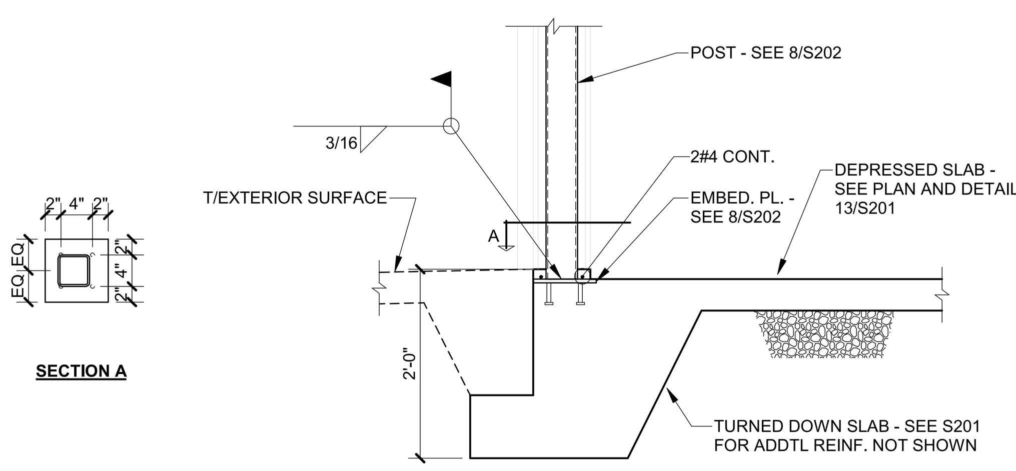
5 TYPICAL SECTION AT EXTERIOR OPERABLE DOOR SUPPORT



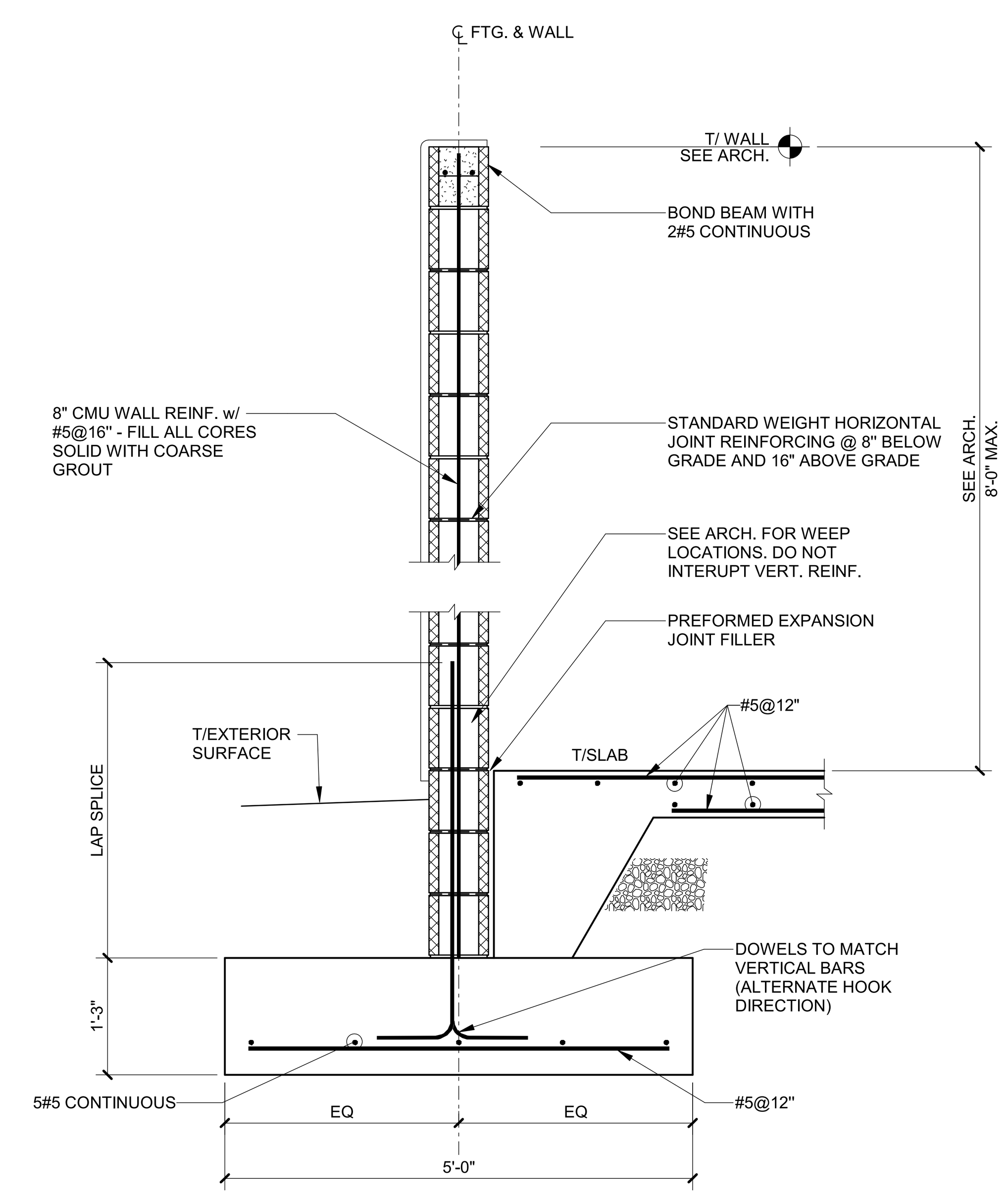
2 TYPICAL SECTION AT INTERIOR OPERABLE DOOR SUPPORT



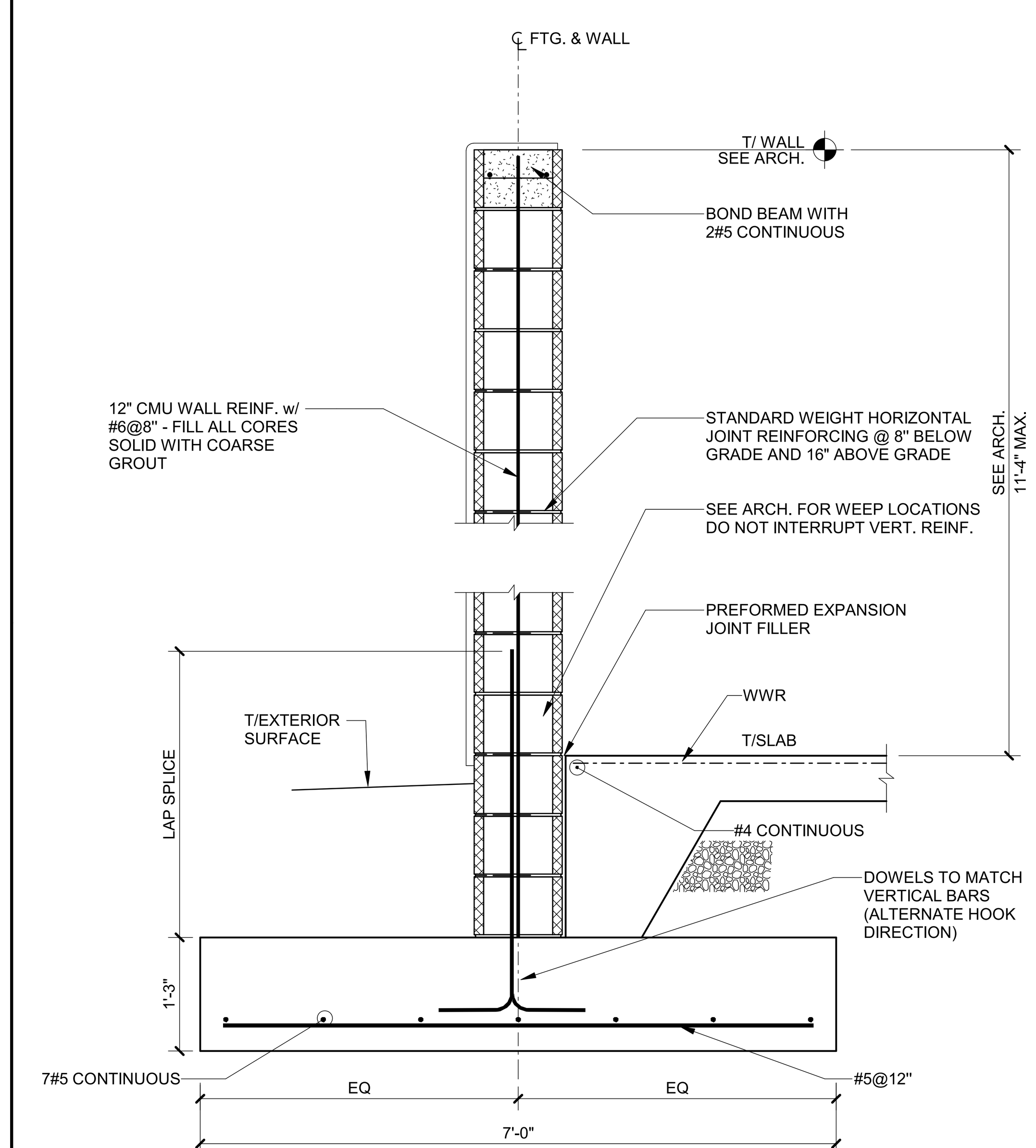
7 SECTION AT LOAD BEARING CMU WALL



6 SECTION AT PREFABRICATED CANOPY AT SLAB DEPRESSION



3 SECTION AT DUMPSTER ENCLOSURE



1 SECTION AT GENERATOR ENCLOSURE

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REVISIONS

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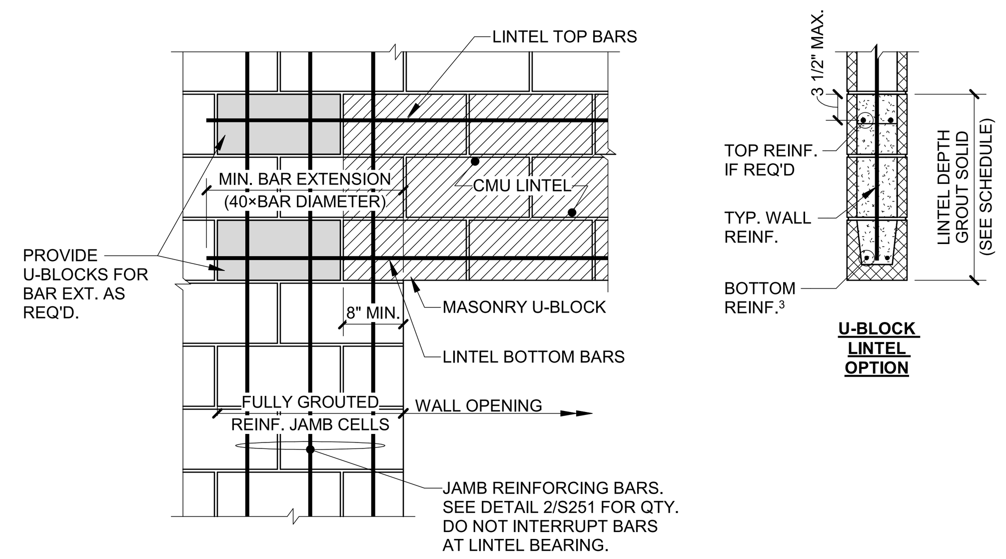
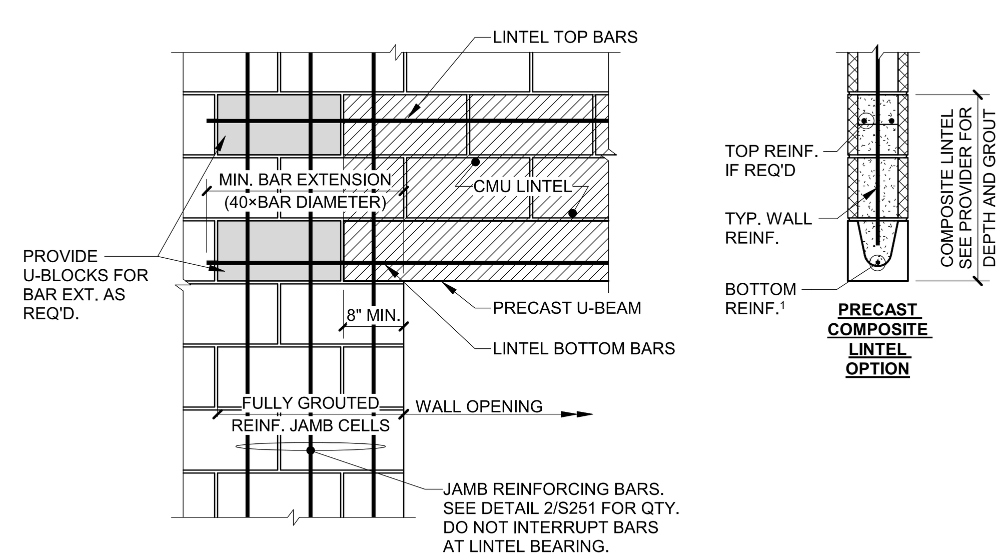
PROJ.	A04025
DATE	11-20-2025

FOUNDATION SECTION AND DETAILS

S202

UNMARKED LINTEL SCHEDULE										
OPENING WIDTH	U-BLOCK OPTION				PRECAST COMPOSITE OPTION[1]					
	EXT. WALL		INT. WALL		EXT. WALL			INT. WALL		
	NOMINAL DEPTH	REINF.	NOMINAL DEPTH	REINF.	GRAVITY LOAD	UPLIFT LOAD	NOMINAL DEPTH[2]	GRAVITY LOAD	UPLIFT LOAD	NOMINAL DEPTH[2]
1'-4" TO 4'-0"	8"	2#4B	8"	2#4B	550 PLF	500 PLF	8"	550 PLF	500 PLF	8"
4'-0" TO 6'-0"	8"	2#4B	8"	2#4B	550 PLF	500 PLF	8"	550 PLF	500 PLF	8"
6'-0" TO 8'-0"	16"	2#4B 2#4T	16"	2#4B 2#4T	550 PLF	500 PLF	8"	550 PLF	500 PLF	8"
8'-4" TO 10'-6"	16"	2#5B 2#5T	16"	2#4B 2#4T	550 PLF	500 PLF	16"	550 PLF	500 PLF	16"
OVER 10'-0"	CONTACT STRUCTURAL ENGINEER									

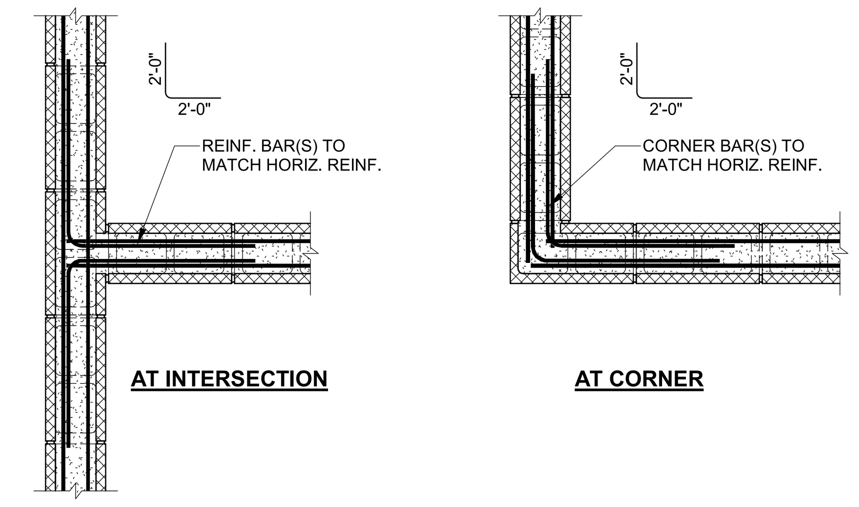




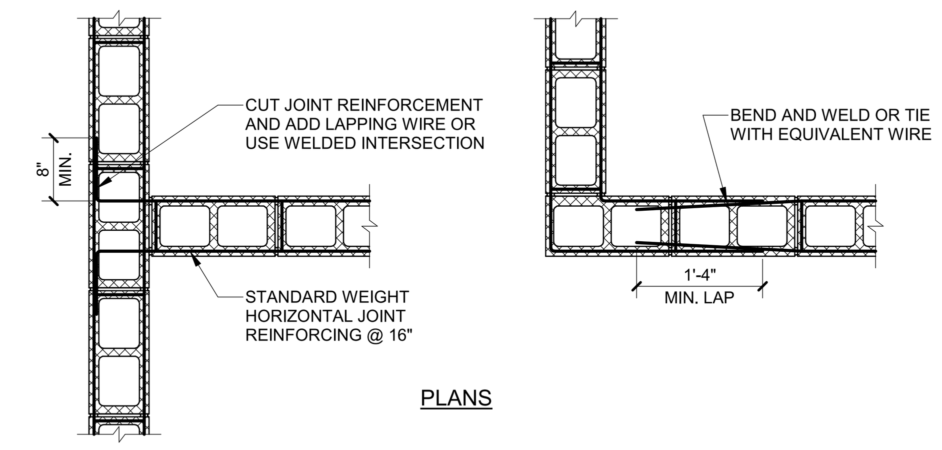
NOTES:

1. THE DESIGN OF THE PRECAST COMPOSITE LINTEL IS A DEFERRED SUBMITTAL. THE CONSTRUCTION SHALL BE PER THE MANUFACTURER'S SIGNED AND SEALED DATA. SUBMIT DATA TO STRUCTURAL ENGINEER INDICATING THE SELECTED LINTELS BEING USED FOR REVIEW.
2. THE NOMINAL DEPTHS OF THE PRECAST COMPOSITE LINTELS ARE AN ESTIMATE AND ARE SUBJECT TO CHANGE.
3. PROVIDE 1/2" MIN. GROUT AROUND BOTTOM REINFORCING IN U-BLOCKS.

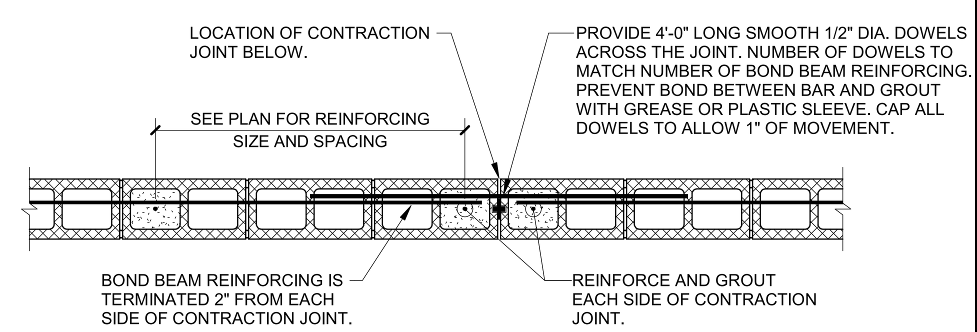
7 CMU LINTEL SCHEDULE



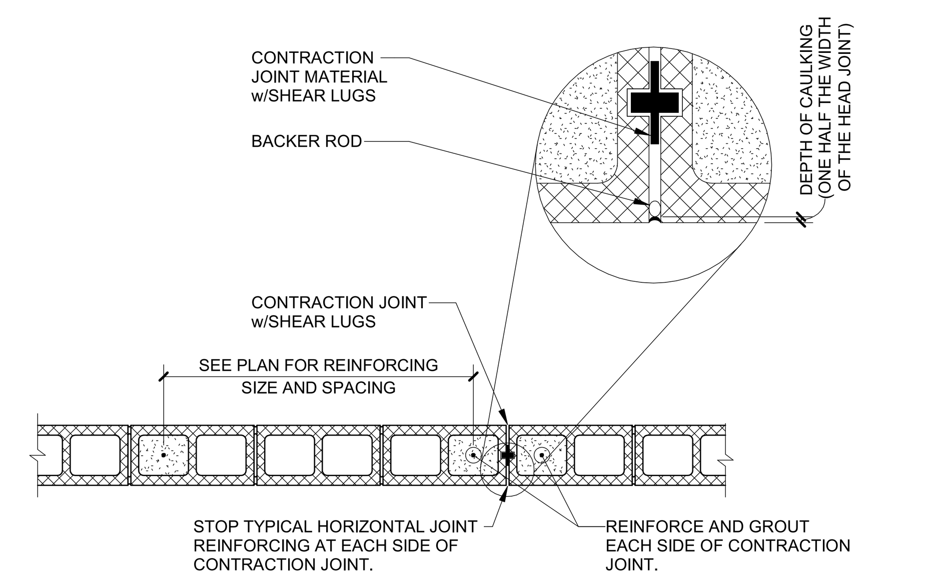
5 TYPICAL BOND BEAM REINF. AT INTERSECTING CMU WALLS



3 TYPICAL JOINT REINF. AT INTERSECTING CMU WALLS



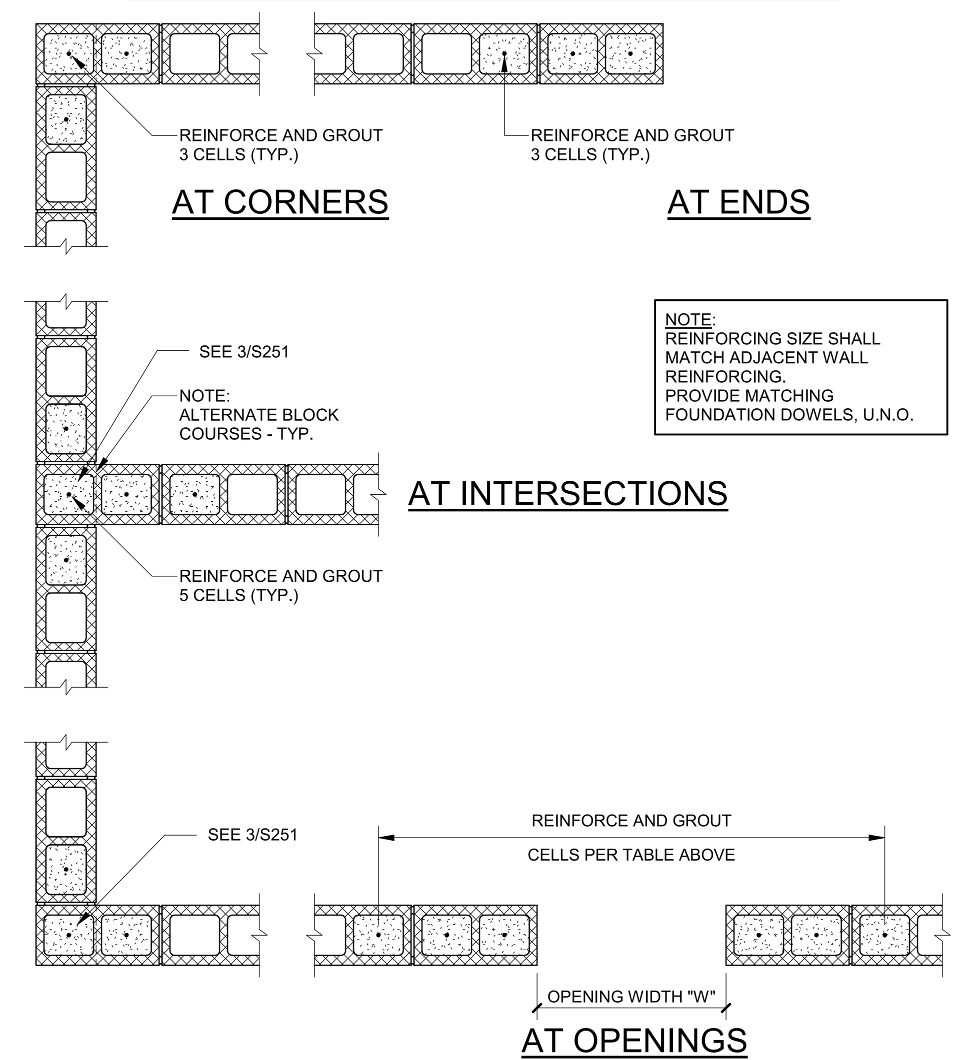
BOND BEAM REINFORCING AT CONTRACTION JOINT



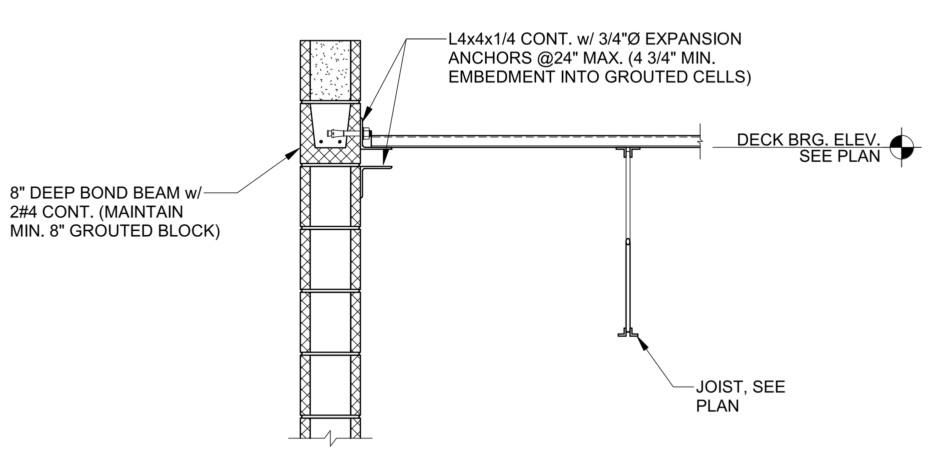
WALL REINFORCING AT CONTRACTION JOINT

4 TYPICAL CONTRACTION JOINT DETAILS

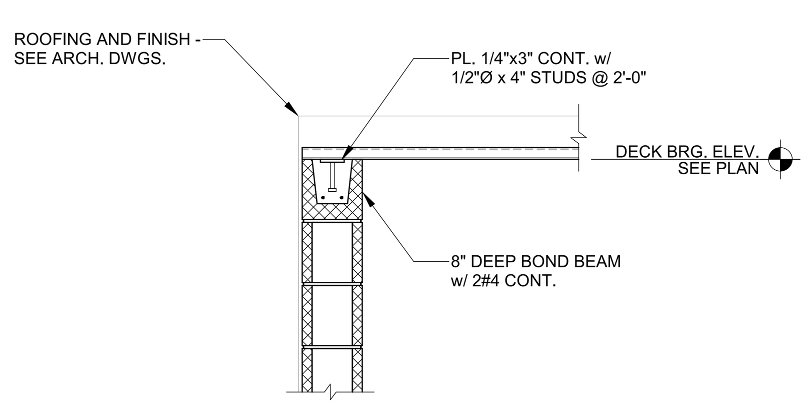
CMU WALL OPENING REINFORCEMENT SCHEDULE	
OPENING WIDTH "W"	VERTICAL REINFORCEMENT (EACH SIDE)
UP TO 6'-0"	2 BARS TO MATCH TYPICAL WALL REINFORCING
6'-0" TO 8'-0"	3 BARS TO MATCH TYPICAL WALL REINFORCING
8'-0" TO 12'-0"	4 BARS TO MATCH TYPICAL WALL REINFORCING



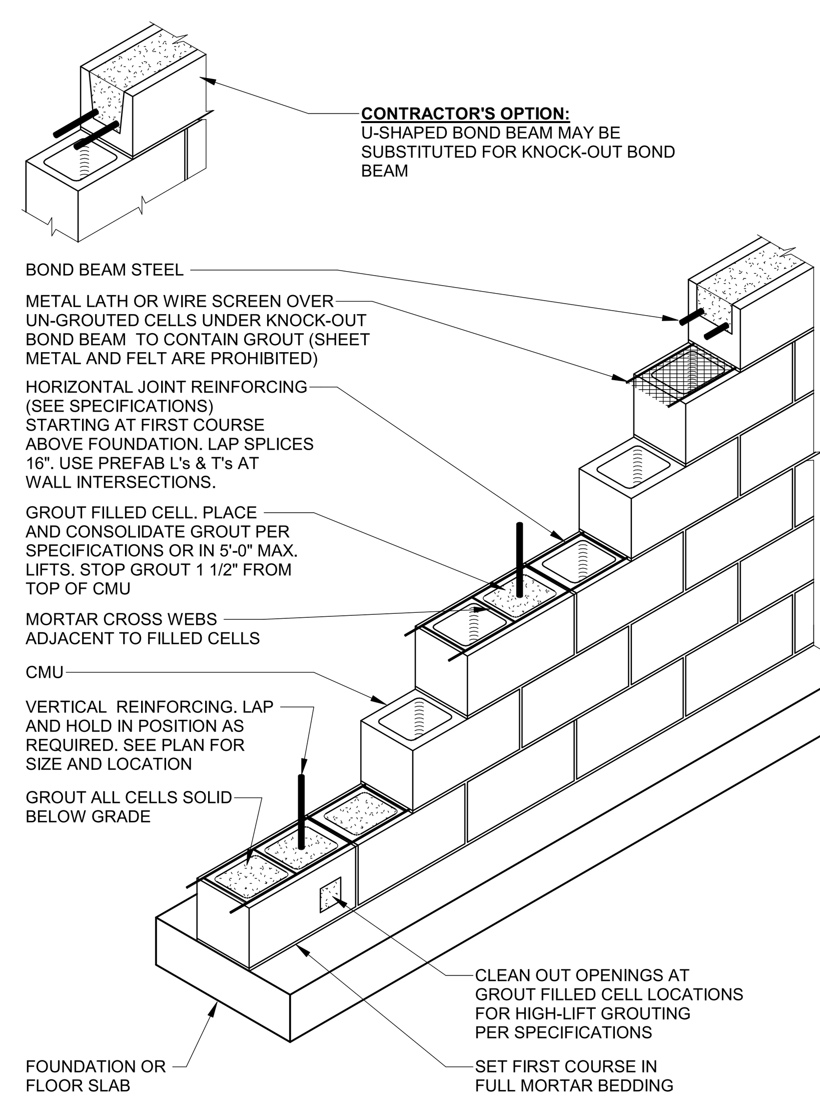
2 TYP REINF. AT LOAD BEARING CMU WALLS CORNERS, ENDS, INTERSECTIONS & OPENINGS



9 DECK SUPPORT DETAIL

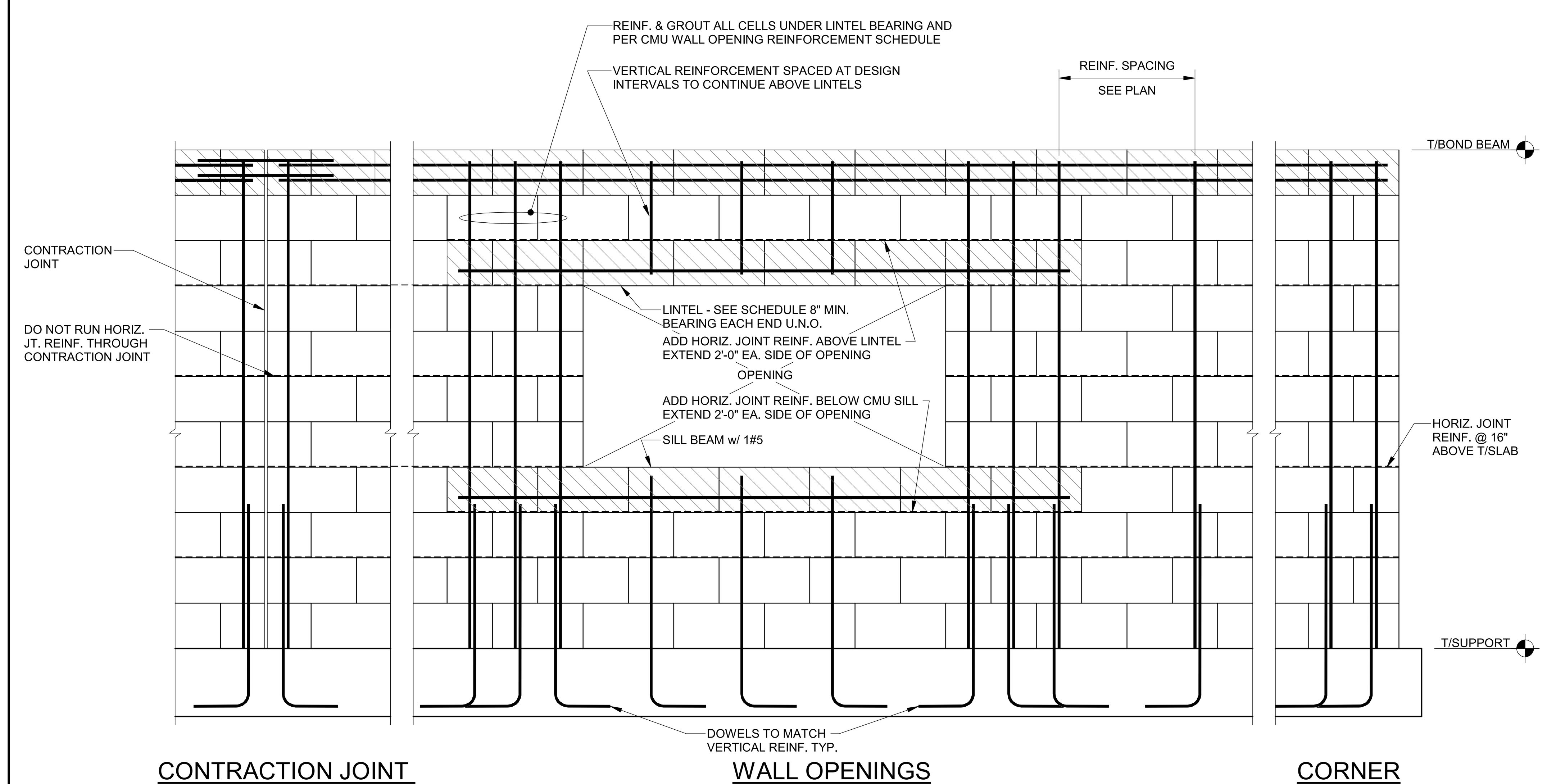


8 DECK SUPPORT DETAIL



NOTE: SEE STRUCTURAL NOTES AND DETAILS FOR REINFORCEMENT SIZE AND SPACING.

6 TYPICAL CMU WALL CONSTRUCTION



CONTRACTION JOINT

WALL OPENINGS

REINF. AT DOOR OPENINGS SIM.

CORNER

NOTES:

1. SEE STRUCTURAL NOTES FOR MAXIMUM SPACING OF CONTROL JOINTS.
2. LOCATE CONTROL JOINTS 2'-0" MINIMUM FROM SIDES OF OPENINGS

1 TYPICAL CMU WALL REINFORCING-ELEVATION (LOAD BEARING WALLS)

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REVISIONS	
DR. BY	CAF
CK. BY	JDP
PROJ.	A04025
DATE	11-20-2025

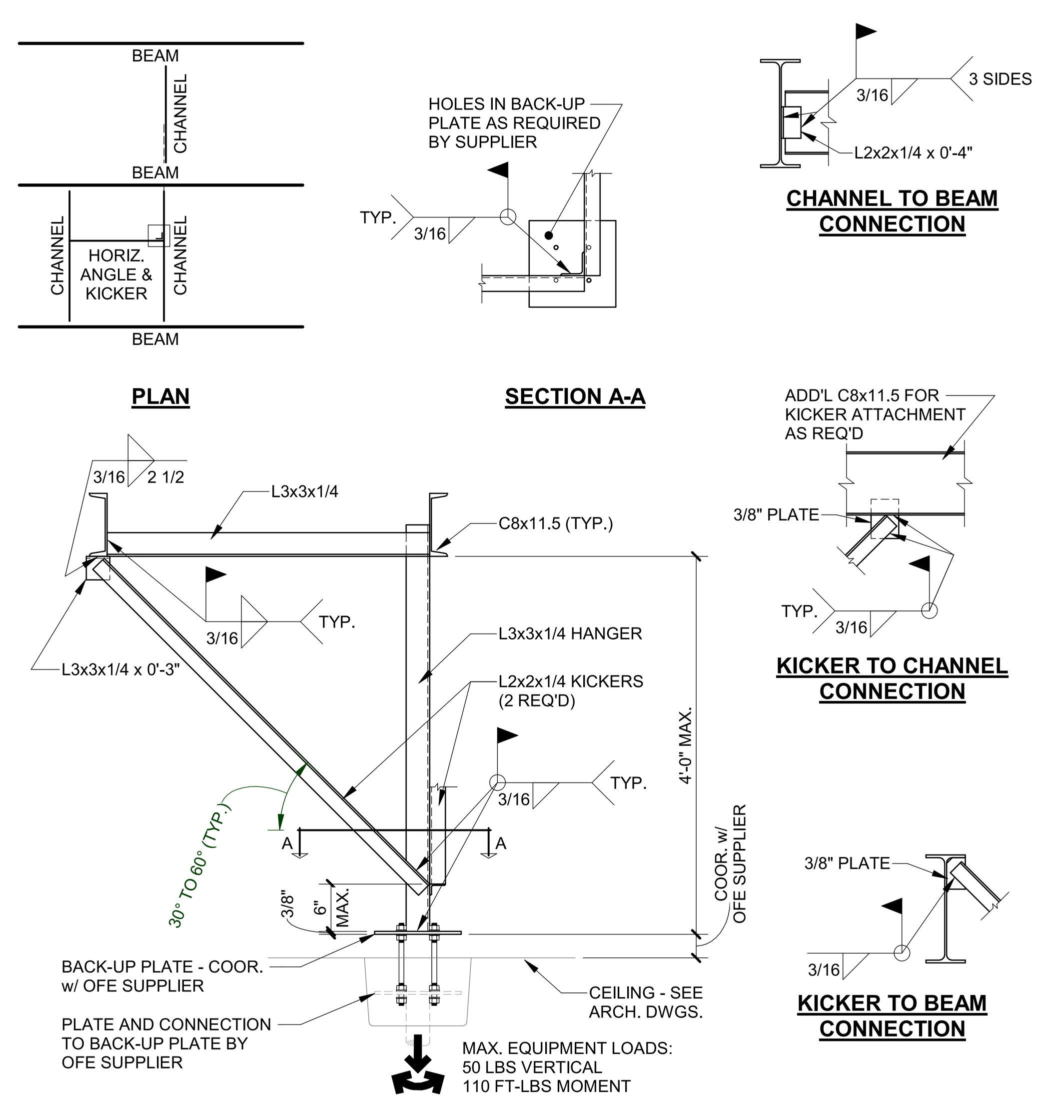
MASONRY SECTIONS AND DETAILS

S251

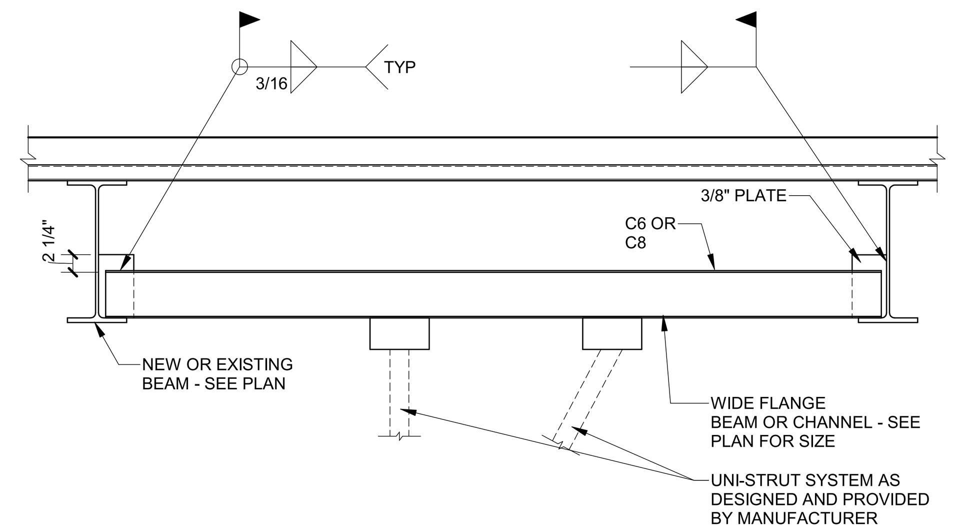
SCALE: 3"=1'-0" SCALE: 1-1/2"=1'-0" SCALE: 1"=1'-0" SCALE: 3/4"=1'-0" SCALE: 1/2"=1'-0" SCALE: 1/4"=1'-0" SCALE: 1/8"=1'-0" SCALE: 3/32"=1'-0" SCALE: 1/16"=1'-0" SCALE: 1/32"=1'-0"

NOTES:

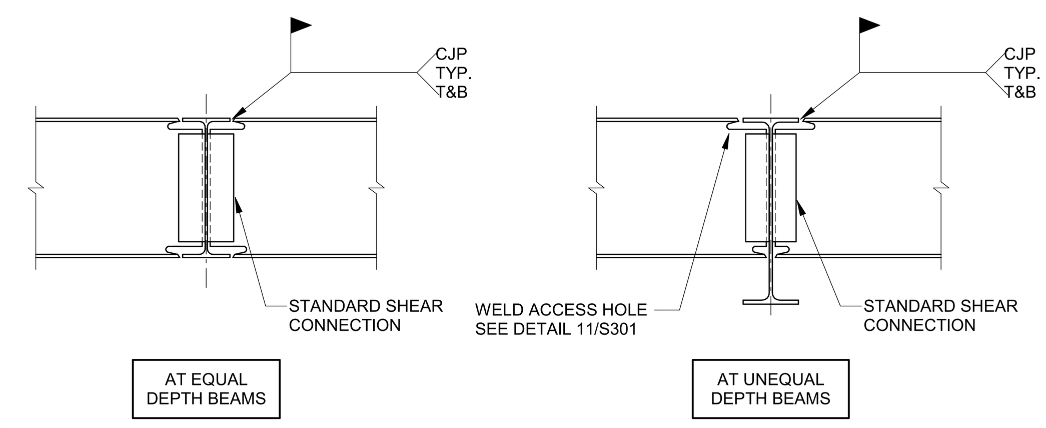
1. UNISTRUT MAY BE SUBMITTED AS AN ALTERNATE TO STRUCTURAL STEEL. THE UNISTRUT SUPPORT SHALL BE DESIGNED BY THE UNISTRUT SUPPLIER AND SUBMITTED AS A SUBSTITUTION REQUEST.
2. THE BACK-UP PLATE MAY BE OMITTED AND REPLACED WITH AN ANCHOR PLATE PROVIDED BY THE OFE SUPPLIER. CONTRACTOR SHALL COORDINATE WITH THE OFE SUPPLIER.
3. SEE ARCHITECTURAL AND OFE DRAWINGS FOR LOCATIONS, ORIENTATIONS, AND QUANTITIES.



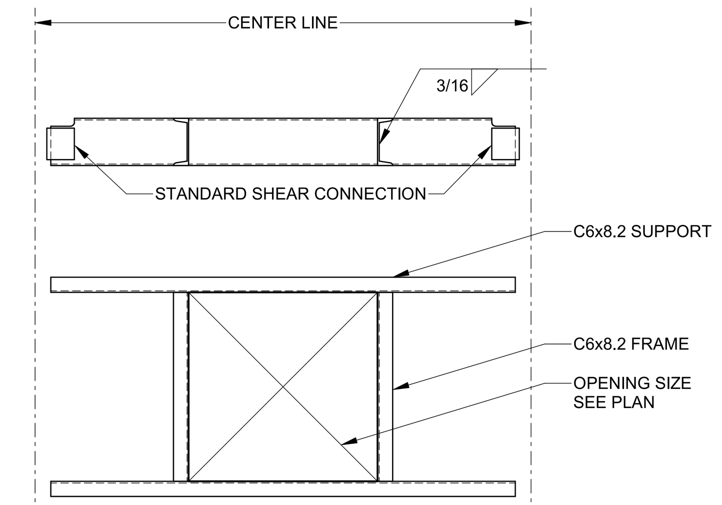
15 EXAM LIGHT SUPPORT



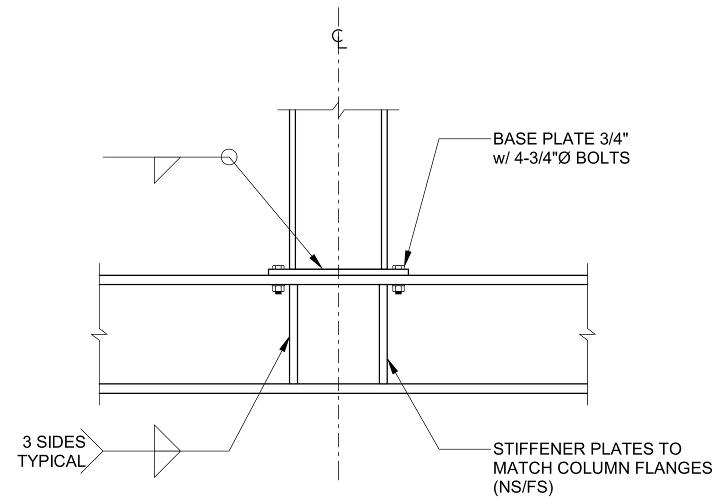
12 TYPICAL UNI-STRUT ATTACHMENT DETAIL



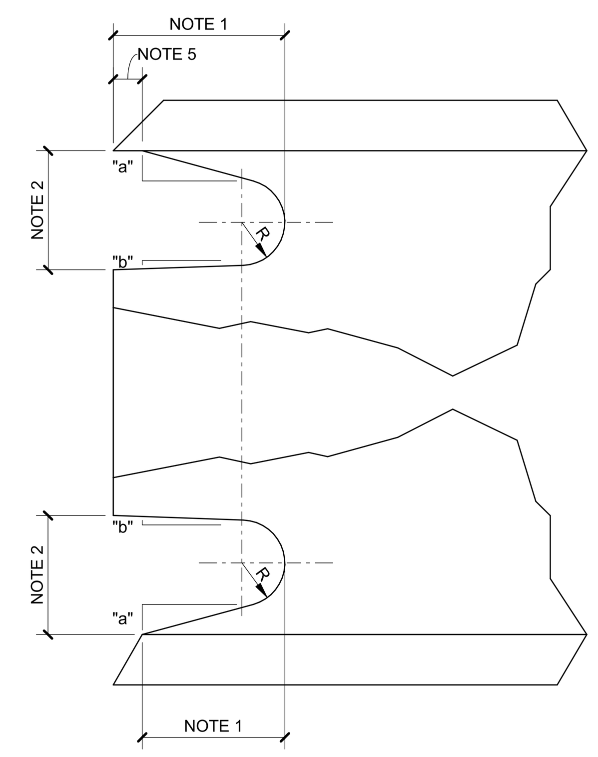
8 TYPICAL MOMENT CONNECTION DETAILS



5 TYPICAL ROOF OPENING FRAME (GREATER THAN 30" FOR LARGEST LENGTH)



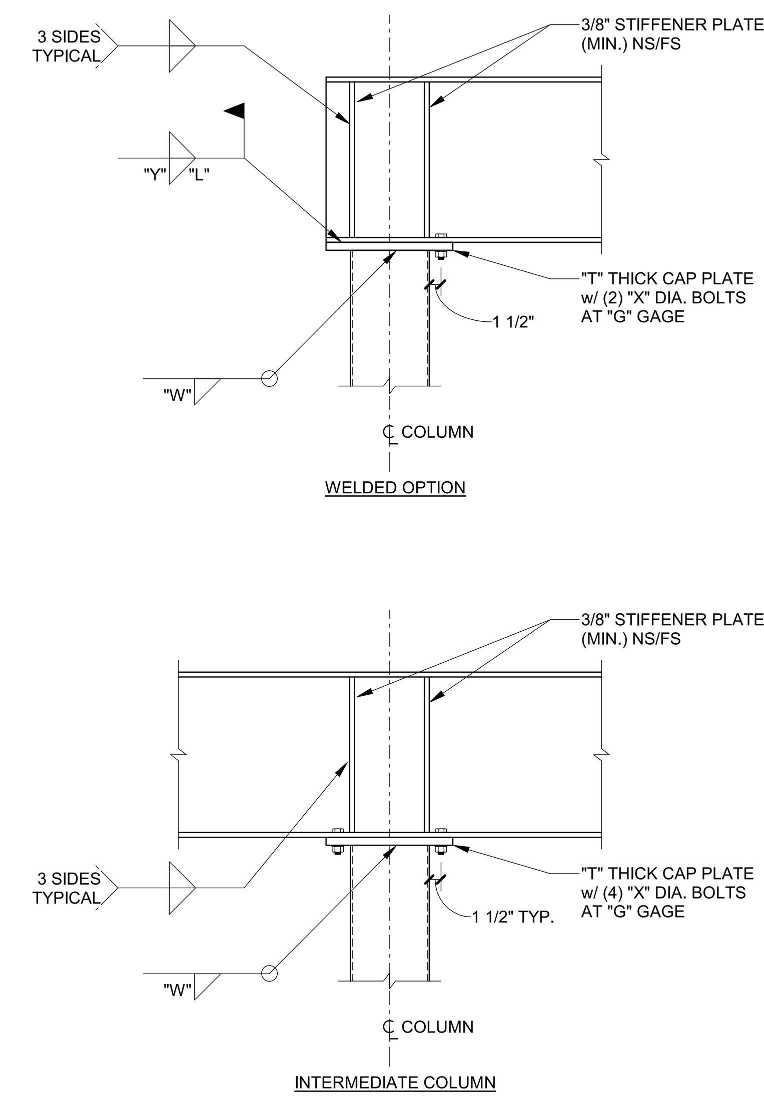
14 TYPICAL TRANSFER COLUMN BASE CONNECTION



NOTES:

1. LENGTH: GREATER OF 1.5tw OR 1 1/2".
2. HEIGHT: GREATER OF 1.0tw OR 1" BUT NEED NOT EXCEED 2"
3. R = 3/8" MIN. GRIND THE THERMALLY CUT SURFACES OF WELD ACCESS HOLES IN HEAVY SHAPES AS DEFINED IN AISC 360-10 SECTIONS A3.1(c) AND (d)
4. SLOPE "a" FORMS A TRANSITION FORM THE WEB TO THE FLANGE. SLOPE "b" MAY BE HORIZONTAL.
5. THE BOTTOM OF THE TOP FLANGE IS TO BE CONTOURED TO PERMIT THE TIGHT FIT BACKING BARS WHERE THEY ARE TO BE USED.
6. THE WEB-TO FLANGE WELD OF BUILT UP MEMBERS IS TO BE HELD BACK A DISTANCE OF AT LEAST THE WELD SIZE FORM THE EDGE OF THE ACCESS HOLE.

11 TYPICAL WELD ACCESS HOLE DETAIL

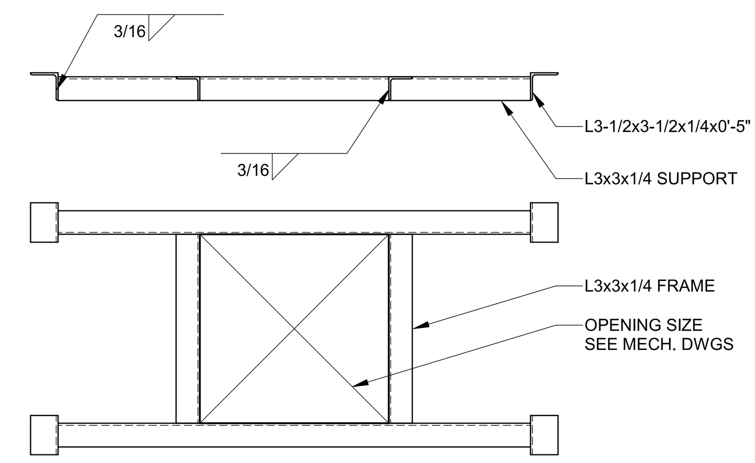


CAP PLATE SCHEDULE

BEAM SIZE	PLATE	BOLTS			WELDS		
	"T"	"X"	"G"	GRADE	"W"	"Y"	"L"
W24x55	1"	1"	4"	A490	CJP	1/2"	3"
W24x68	1"	1"	4 1/4"	A325	CJP	1/4"	3"

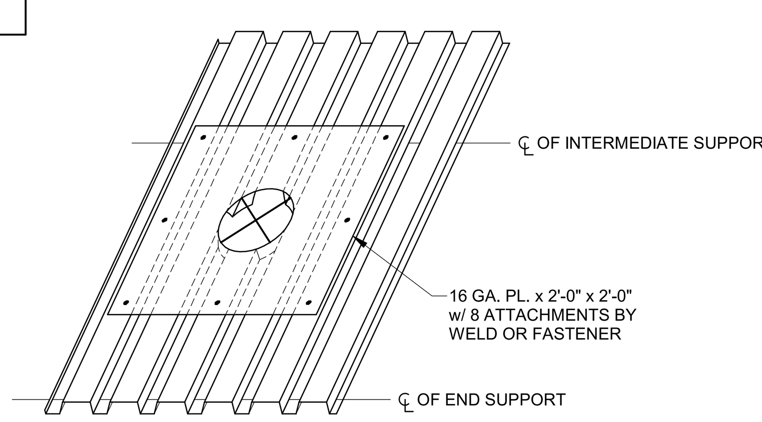
NOTE: ** INDICATES WELDED OPTION IS NOT APPLICABLE - BOLTS REQUIRED.

7 TYPICAL BEAM TO HSS CONNECTION CONTINUOUS/CANTILEVERED BEAMS

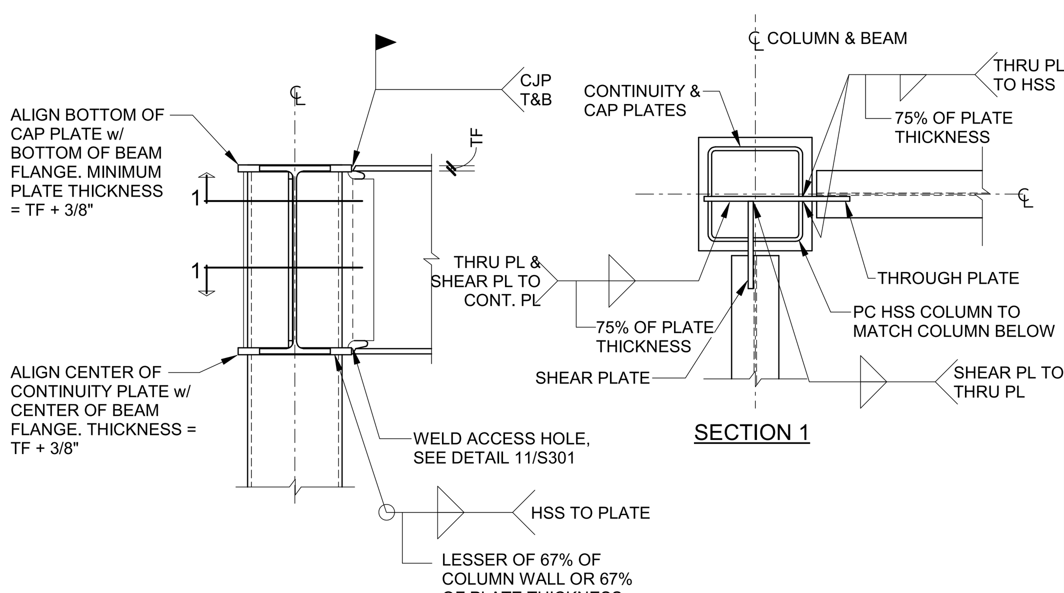


4 TYPICAL ROOF OPENING FRAME (LESS THAN 30" FOR LARGEST LENGTH)

NOTE: PLATE IS NOT REQUIRED FOR OPENINGS WITH 6" DIAMETER OR LESS



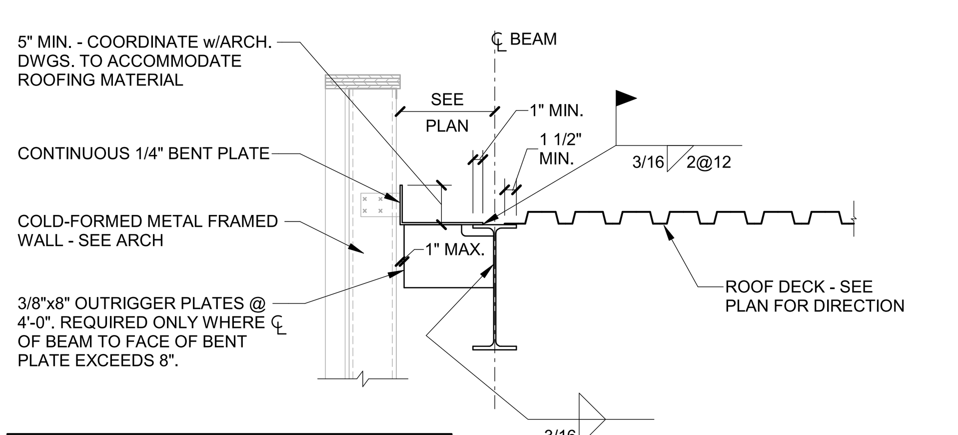
3 TYPICAL ROOF DECK OPENING (LESS THAN 14" DIAMETER)



NOTES:

1. SHEAR AND THROUGH PLATE, SIZE AND NUMBER OF BOLTS, AND WELDS TO BE DESIGNED FOR FACTORED REACTIONS SHOWN ON STRUCTURAL DRAWINGS.
2. TF = BEAM FLANGE THICKNESS
3. CONTINUITY PLATE STEEL GRADE SHALL MATCH THE BEAM STEEL GRADE. ALTERNATE: WHERE THE PLATE IS 1.4 TIMES THICKER THAN THE BEAM FLANGE, THE PLATE STEEL GRADE MAY BE A36.
4. SEE SPECIFICATIONS FOR WELD METAL REQUIREMENTS.

13 TYPICAL BEAM TO HSS MOMENT CONNECTION



NOTE:

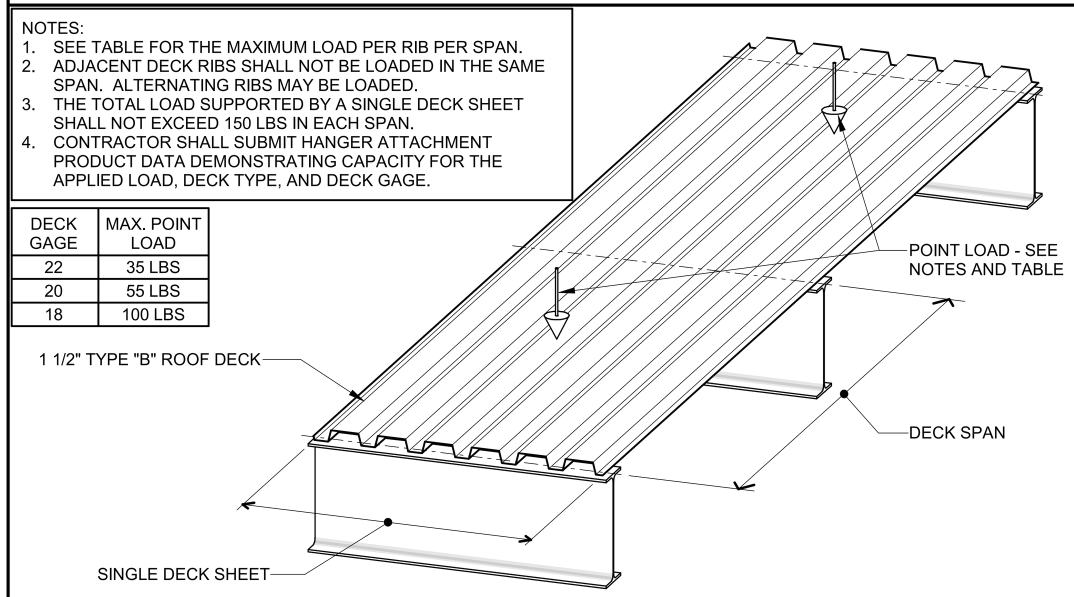
WALL FRAMING AND ITS CONNECTION TO THE STRUCTURE TO BE DESIGNED BY THE SUPPLIER.

10 TYPICAL SECTION AT ROOF

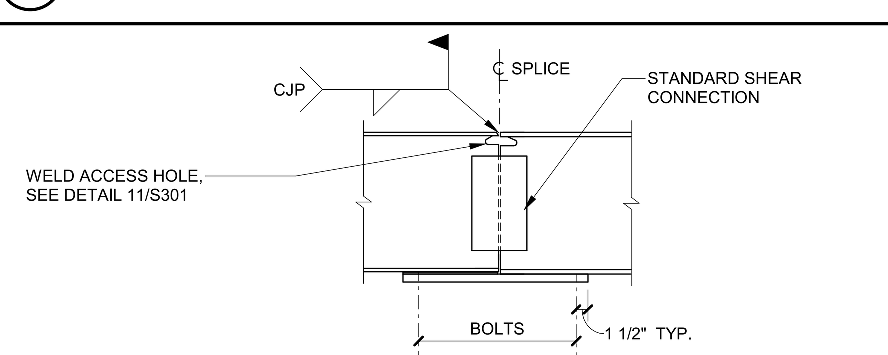
NOTES:

1. SEE TABLE FOR THE MAXIMUM LOAD PER RIB PER SPAN.
2. ADJACENT DECK RIBS SHALL NOT BE LOADED IN THE SAME SPAN. ALTERNATING RIBS MAY BE LOADED.
3. THE TOTAL LOAD SUPPORTED BY A SINGLE DECK SHEET SHALL NOT EXCEED 150 LBS IN EACH SPAN.
4. CONTRACTOR SHALL SUBMIT HANGER ATTACHMENT PRODUCT DATA DEMONSTRATING CAPACITY FOR THE APPLIED LOAD, DECK TYPE, AND DECK GAGE.

DECK GAGE	MAX. POINT LOAD
22	35 LBS
20	55 LBS
18	100 LBS

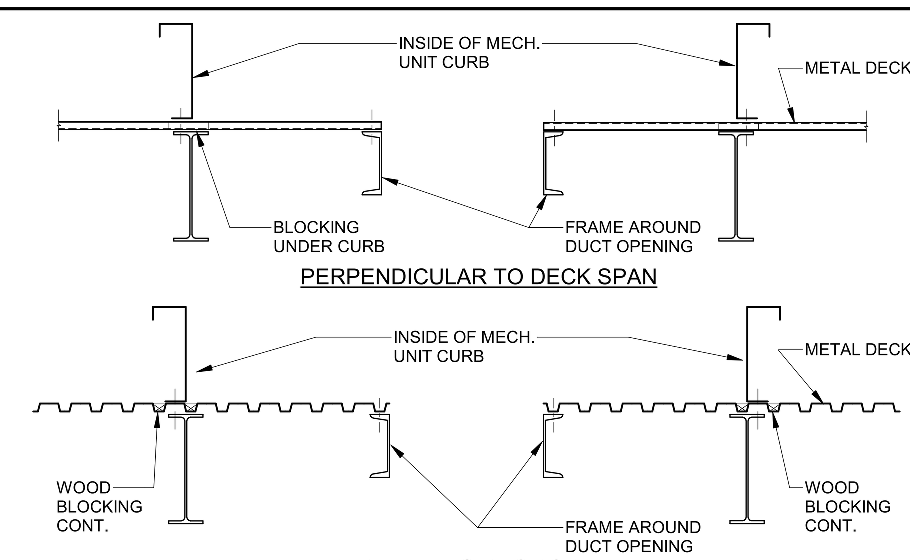


2 TYP. ROOF DECK POINT LOAD RESTRICTIONS (1 1/2" TYPE "B" ROOF DECK)

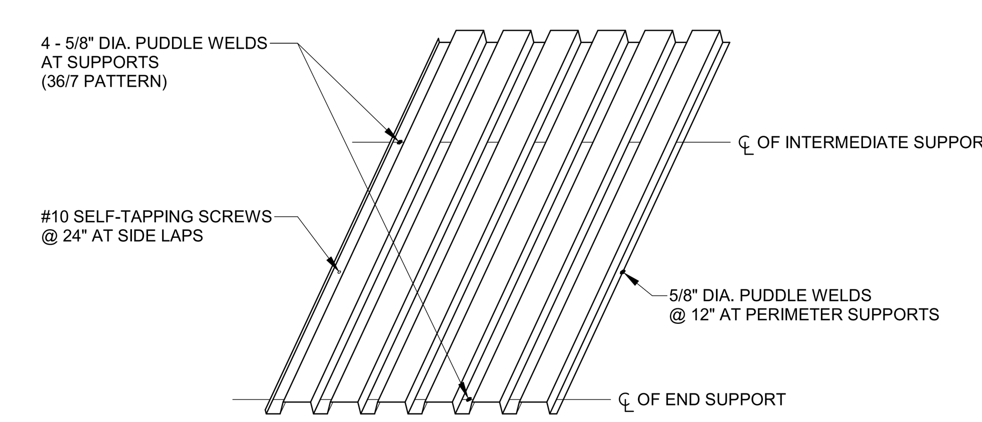


BEAM SIZE	BOLTS			PLATE SIZE
	DIA.	NO.	SPA.	
W24x55	3/4"	6 EA. BEAM	3" PARALLEL TO BEAM 3 1/2" PERPENDICULAR	1/2"x7"
W24x68	3/4"	12 EA. BEAM	3" PARALLEL TO BEAM 5 1/2" PERPENDICULAR	5/8"x9"

9 TYPICAL WELDED/BOLTED MOMENT SPLICE



6 MECHANICAL UNIT CURB MOUNTING DETAIL



1 TYPICAL ROOF DECK ATTACHMENT (1 1/2" TYPE "B" ROOF DECK)

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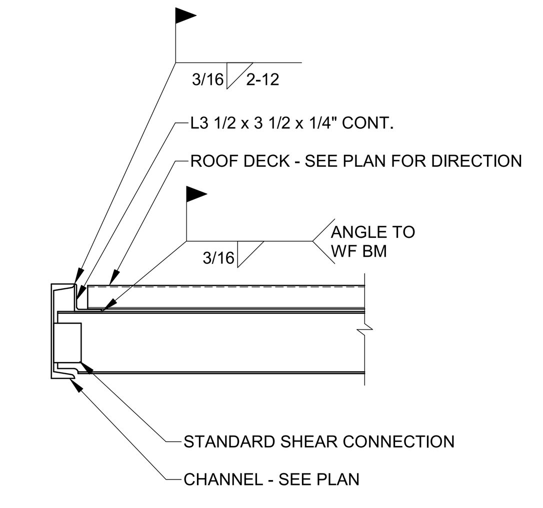
DATE	11-20-2025

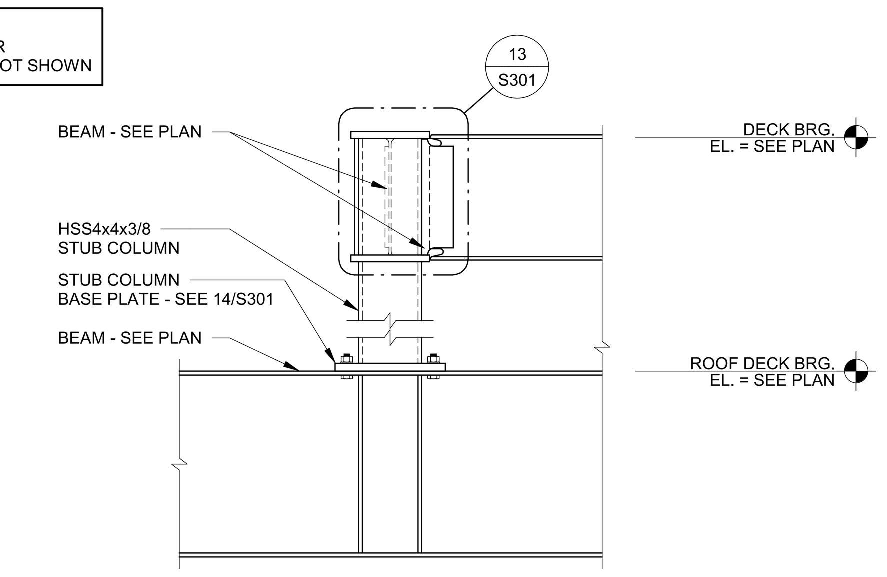
FRAMING SECTIONS AND DETAILS

S301



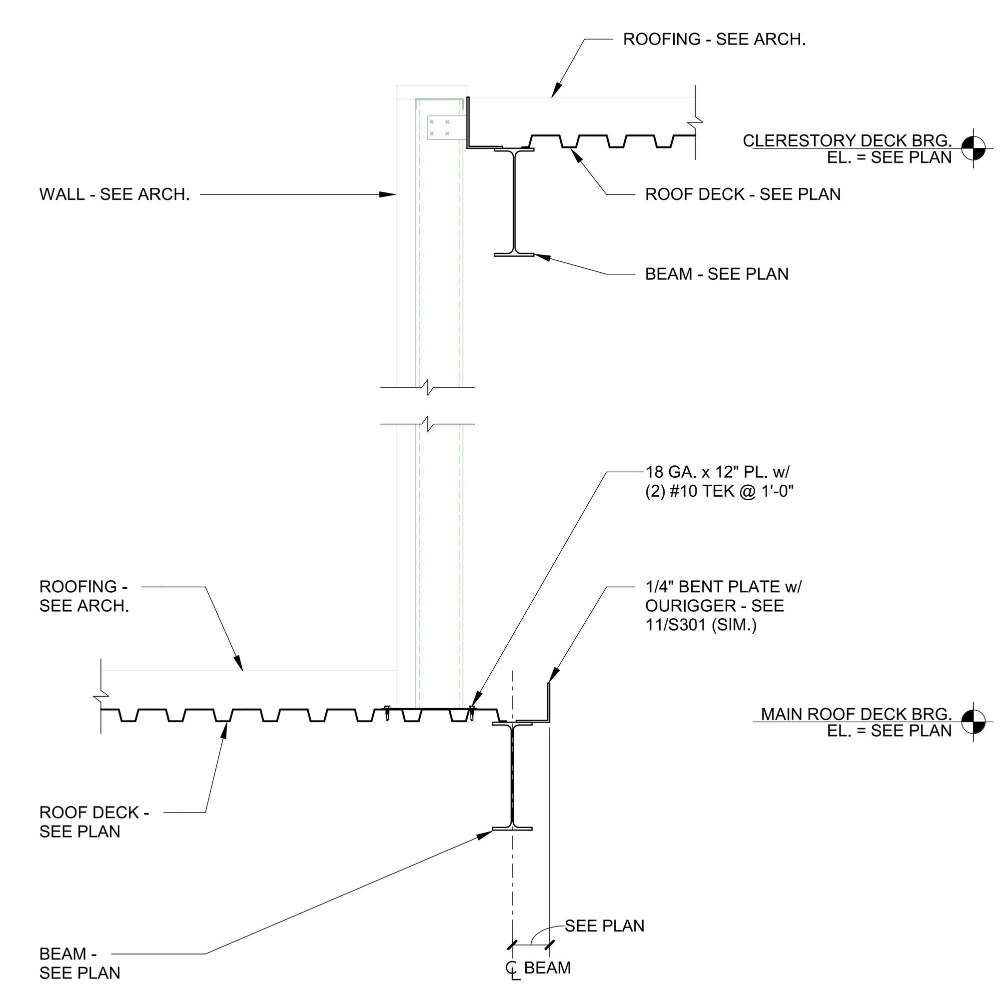
4 TYPICAL SECTION AT ENTRY CANOPY

NOTE:
SEE 10/S301 FOR INFORMATION NOT SHOWN



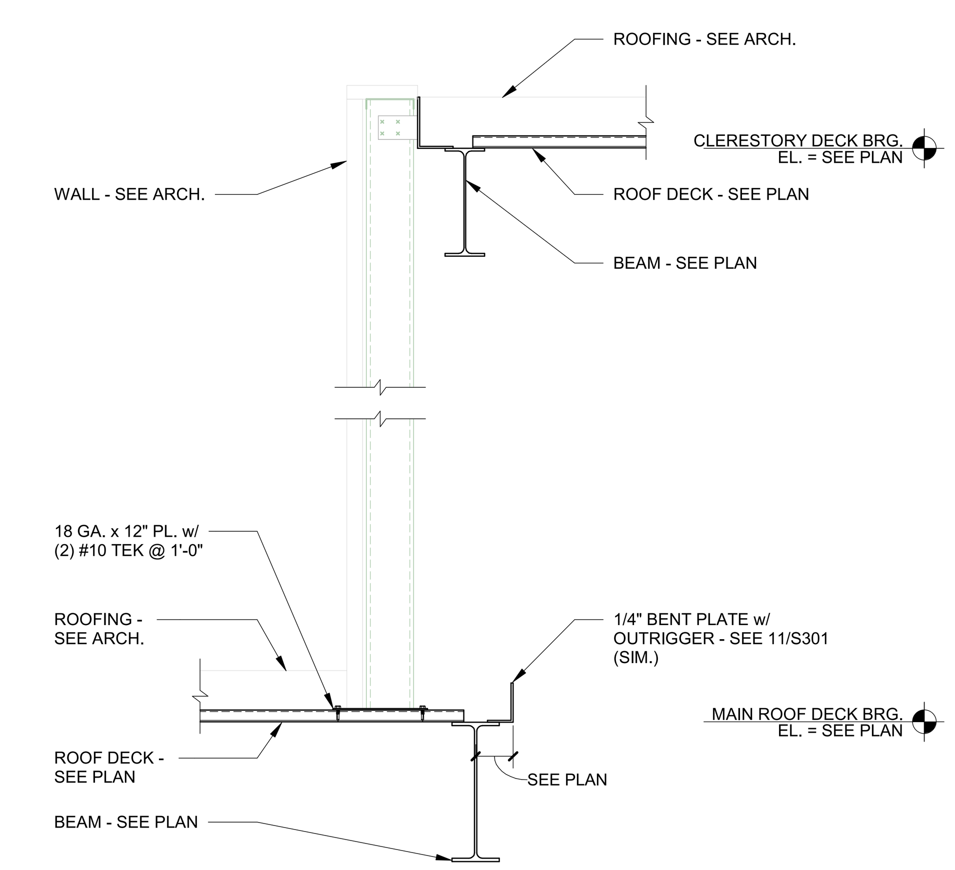
3 SECTION AT CLERESTORY

NOTE:
SEE 10/S301 FOR INFORMATION NOT SHOWN



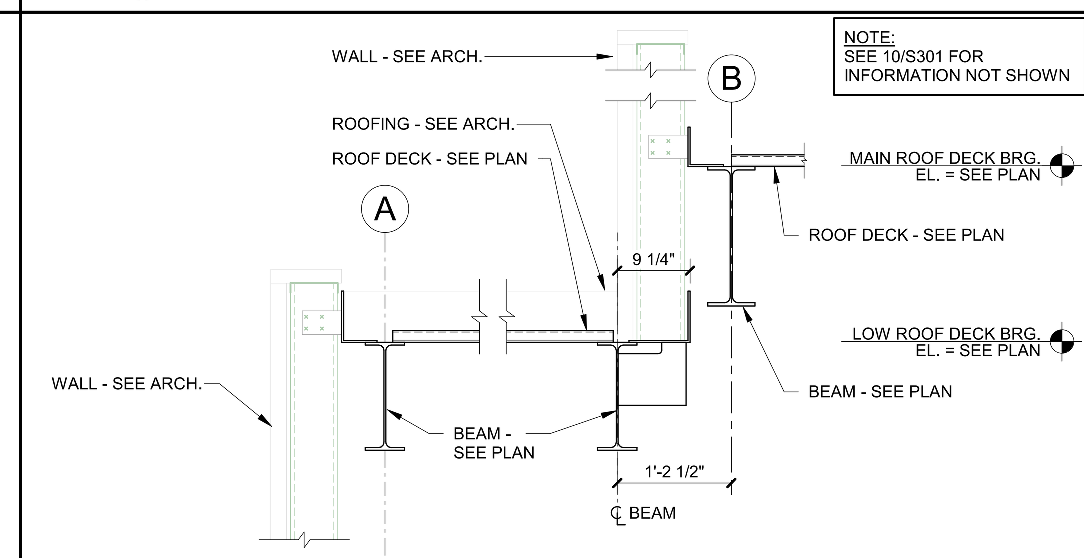
6 SECTION AT CLERESTORY

NOTE:
SEE 10/S301 FOR INFORMATION NOT SHOWN

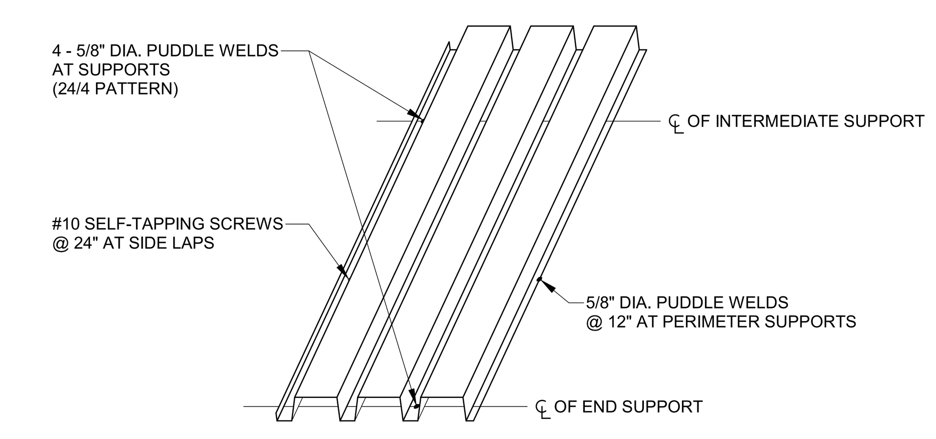


2 SECTION AT CLERESTORY

NOTE:
SEE 10/S301 FOR INFORMATION NOT SHOWN



5 SECTION AT LOW ROOF



1 TYPICAL ROOF DECK ATTACHMENT
(3" TYPE "N" ROOF DECK)

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FRAMING SECTIONS AND DETAILS

S302

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SCALE: 3"=1'-0" SCALE: 1-1/2"=1'-0" SCALE: 1"=1'-0" SCALE: 3/4"=1'-0" SCALE: 1/2"=1'-0" SCALE: 1/4"=1'-0" SCALE: 1/8"=1'-0" SCALE: 3/32"=1'-0" SCALE: 1/16"=1'-0" SCALE: 1/32"=1'-0"