

STRUCTURAL NOTES

- CONSTRUCTION:
1. ALL CONSTRUCTION SHALL CONFORM TO THE FLORIDA BUILDING CODE 2004, COUNTY AND LOCAL AMENDMENTS AND REQUIREMENTS, ACI 318-94, AISC MANUAL OF STEEL CONSTRUCTION, PROJECT SPECIFICATION AND CONSTRUCTION DOCUMENTS.
 2. GENERAL CONTRACTOR SHALL VERIFY EXISTING CONDITIONS FOR CONFORMANCE WITH PLANS AND SPECIFICATIONS. NOTIFY ENGINEER OF ANY VARIATIONS PRIOR TO PROCEEDING.
 3. GENERAL CONTRACTOR SHALL COORDINATE ALL ARCHITECTURAL, ELECTRICAL AND MECHANICAL WITH STRUCTURAL PRIOR TO PROCEEDING. ANY DEVIATIONS OF STRUCTURAL DETAILS MUST BE CONFIRMED IN WRITING BY THE ENGINEER PRIOR TO PROCEEDING.
 4. VERIFY PLAN DIMENSIONS IN FIELD AND NOTIFY ENGINEER OF ANY DISCREPANCIES PRIOR TO PROCEEDING WITH CONSTRUCTION.
 5. CONTRACTOR SHALL SUBMIT CERTIFICATION FROM SOIL ENGINEER TO VERIFY 2,500 PSF SAFE SOIL BEARING VALUE.

SUPERIMPOSED LOADS

MAXIMUM WIND VELOCITY = 140 MPH EXPOSURE "C"

WIND IMPORTANCE FACTOR = 1.0

BUILDING CATEGORY 2

INTERNAL PRESSURE COEFFICIENT = ± 0.18

DL = 30 PSF ROOF

DL = 25 PSF

ENCLOSURE CATEGORY = ENCLOSED

DATUM:

TOP OF 1ST FLOOR SLAB IS INDICATED ON PLANS. (SEE PLANS AND SCHEDULES). SEE ARCHITECTURAL DRAWINGS FOR TYPE AND LIMITS OF FLOOR FINISHES, MASONRY OPENINGS, AND ALSO RECESSES.

DIMENSIONS:

CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE LOCATIONS IN FIELD AND CORRELATE WITH SHOP DRAWINGS AND CONSTRUCTION DOCUMENTS PRIOR TO CONSTRUCTION. NOTIFY ENGINEER OF ANY DISCREPANCIES PRIOR TO PROCEEDING WITH CONSTRUCTION.

CONCRETE:

1. CONCRETE SHALL REACH MINIMUM ULTIMATE STRENGTH $f'_c = 3000$ PSI MIN IN 28 DAYS FOR SLAB ON GRADE, COLUMNS, BEAMS, AND STRUCTURAL SLABS UNLESS OTHERWISE NOTED ON PLANS AND SCHEDULES.
2. SLUMP AND CYLINDER TESTS (ASTM C-31) SHALL BE MADE BY AN INDEPENDENT TESTING LABORATORY. PROVIDE 5 TESTS WITH EACH CONCRETE POUR AND/OR EACH 50 YD OF CONCRETE PLACED. SLUMP @ POUR SHALL BE 5 IN MAX. (6" FOR PUMP MIX USED IN FILLED CELLS). NO WATER IS TO BE ADDED ON THE JOB SITE.
3. CONTRACTOR SHALL SUBMIT COPIES OF MIX DESIGN TO ENGINEER FOR APPROVAL.

REINFORCING:

REINFORCING STEEL SHALL BE DEFORMED, NEW BILLET STEEL ASTM A615 GRADE 60. ALL SPLICES TO BE IN ACCORDANCE WITH CHAPTER 12, ACI 318-94, UNLESS OTHERWISE NOTED. FABRICATION AND PLACING OF STEEL SHALL BE IN ACCORDANCE WITH 1983 CODE STANDARDS AND PRACTICE PROCEDURES. FABRICATOR SHALL SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION. FABRICATOR SHALL COMPENSATE FOR FACE SHELL THICKNESS AS REQUIRED WHEN CUTTING STEEL FOR MASONRY UNITS.

COLUMN NOTES:

1. ALL COLUMN BARS SHALL LAP 5" X BAR NUMBER UNLESS OTHERWISE NOTED. SPLICE TO OCCUR IN LONG FACE OF COLUMN AND/OR INSIDE OF CONTINUOUS BEAM STEEL.
2. ALL COLUMN STEEL MAY BE SPLICED AS INDICATED ON DRAWINGS.
3. CONCRETE COVERAGE OVER REINFORCING SHALL BE 1-1/2" CLEAR TO TIES (UNLESS OTHERWISE NOTED); 1.25" FROM INSIDE OF FACE SHELL ON BLOCK COLUMN.
4. COLUMN BARS SHALL TERMINATE WITH 90 DEGREE HOOK AT ROOF. SEE DRAWINGS FOR ADDITIONAL DETAIL.

FORMWORK:

1. SHORING AND BRACING PROCEDURES SHALL FOLLOW THE "RECOMMENDED PRACTICE FOR CONCRETE FORMWORK", BASED ON AMERICAN CONCRETE INSTITUTE STANDARD ACI 347-68.
2. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS TO ENGINEER OF RECORD DEPICTING SHORING, RE-SHORING, AND RELATED WORKS FOR REVIEW. THESE SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY SHORING SPECIALTY REGISTERED ENGINEER.

TRUSSES

1. CONTRACTOR SHALL SUBMIT TO ENGINEER OF RECORD SHOP DRAWINGS DEPICTING DESIGN, CONNECTION TO ROOF AND LAYOUT OF TRUSSES.
2. THE SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY FLORIDA REGISTERED ENGINEER.

REINFORCED AND STANDARD LOAD BEARING MASONRY:

1. ALL BLOCK WALLS SHALL BE TWO CELL HOLLOW CONCRETE MASONRY REGULAR SIZE BLOCK, COLUMN BLOCK OR OTHER MASONRY UNITS MANUFACTURED IN CONFORMANCE WITH ASTM C90 AND PROJECT SPECIFICATIONS. THESE MASONRY UNITS SHALL PROVIDE A MINIMUM PRISM STRENGTH $f'_m = 2000$ PSI IN 28 DAYS AS DELIVERED TO THE JOB SITE. CONTRACTOR IS TO PROVIDE ENGINEER A CERTIFICATION FROM BLOCK MANUFACTURER.
2. ALL GROUTING USED SHALL BE COARSE MASONRY GROUT WITH A MINIMUM COMPRESSIVE STRENGTH = 3000 PSI IN 28 DAYS.

REINFORCED AND STANDARD LOAD BEARING MASONRY (CONT.):

3. TYPE M OR S MORTAR SHALL BE USED EXCLUSIVELY ON THIS PROJECT, AND PROPORTIONED AS OUTLINED UNDER ASTM C-270. HOLLOW UNITS SHALL BE Laid WITH FULL MORTAR COVERAGE ON HORIZONTAL AND VERTICAL FACE SHELLS, EXCEPT THAT WEBS SHALL ALSO BE BEDDED IN ALL COURSE OF COLUMNS, IN THE STARTING COURSE ON FOOTINGS, AND AT GROUTED CELLS. HORIZONTAL AND VERTICAL FACE JOINTS SHALL BE SHOVED TIGHT, MORTAR PROTRUSIONS EXTENDING INTO CELLS OR CAVITIES (THAT ARE TO BE REINFORCED AND GROUTED) SHALL BE REMOVED. ALLOW MINIMUM 24 HOURS FOR MORTAR TO CURE PRIOR TO GROUTING. SEE ARCHITECTURAL DRAWING FOR JOINT TREATMENT (AS REQUIRED), SEALING MASONRY UNITS, ETC.
4. MASONRY CONTRACTOR SHALL PROVIDE 9-GA "LADDER" TYPE "DUR-O-WAL" HORIZONTAL JOINT REINFORCEMENT AT ALTERNATE COURSES (16" C/C), TYP. ALL WALLS. DUR-O-WAL SHALL BE CONTINUOUS, WITH A MINIMUM SPLICE OF 6". PROVIDE PREFAB TEES AND CORNER SECTIONS AS REQUIRED. ALL UNTEL REINFORCING BARS SHALL EXTEND NOT LESS THAN 24" BEYOND EACH SIDE OF ALL OPENINGS. CONTRACTOR SHALL RETAIN THE SERVICE OF AN INDEPENDENT TESTING LABORATORY AND/OR CONSULTING ENGINEER TO PROVIDE TESTING IN CONFORMANCE WITH THE PROVISIONS OF PLANS, PROJECT SPECIFICATIONS AND BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES AND COMMENTARY BY ACI 530.
5. MASONRY CONTRACTOR SHALL LAY BLOCK UNITS SO AS TO ASSURE TIGHT HEAD JOINTS AND SNUG BED JOINTS FOR SHRINKAGE CONTROL WATER TIGHTNESS. ALL ADJUSTMENTS TO BLOCKS NOTED ON PLANS SHALL BE SAW CUT. SEE MECHANICAL DRAWINGS FOR LOCATION OF CONDUIT.

STRUCTURAL STEEL

1. STRUCTURAL STEEL TO BE ASTM A-36 FOR ANGLES, BEAMS, AND PLATES, FABRICATED AND ERRECTED PER LATEST AISC SPECIFICATION. ALL TUBE / PIPE COLUMNS SHALL BE CONCRETE FILLED AND CONFORM TO ASTM 500 GRADE B $F_y = 48$ KSI.
2. ALL ANCHOR BOLTS SHALL CONFORM TO ASTM A-325 BOLTS WITH DOUBLE NUT LEVELING FOR COLUMNS. ALL SPLICE AND CONNECTION MATERIAL SHALL CONFORM TO ASTM A-325 BOLTS WITH WASHERS (UNLESS OTHERWISE NOTED).
3. BRACING (AISC) SECTION 1.25. THE ERECTION OF STEEL FRAMING SHALL BE CARRIED UP TRUE AND PLUMB WITHIN THE LIMITS DEFINED IN SECTION 7 OF THE AISC CODE OF STANDARD PRACTICE AND TEMPORARY BRACING SHALL BE INTRODUCED WHEREVER NECESSARY TO ACCOMMODATE ALL LOADS SUPERIMPOSED ON THE STRUCTURE, INCLUDING EQUIPMENT AND THE OPERATION OF SAME. SUCH BRACING SHALL BE LEFT IN PLACE AS LONG AS MAY BE REQUIRED FOR SAFETY.
4. ALL SPLICE AND CONNECTION MATERIAL SHALL CONFORM TO ASTM A-36. ALL FIELD SPLICES SHALL BE WELDED.
5. ERECTION BOLTS MAY BE USED FOR ALIGNMENT.
6. FABRICATOR SHALL SUBMIT SHOP DRAWINGS TO ENGINEER FOR REVIEW PRIOR TO FABRICATION.

SHOP DRAWING NOTES:

SUBMIT TO ARCHITECT FOR REVIEW SHOP DRAWINGS, PRODUCT DATA AND/OR SAMPLES FOR ITEMS LISTED BELOW, INDICATING METHOD OF CONSTRUCTION, DETAIL LAYOUTS, DIMENSIONS, DIAGRAM, SCHEDULES, BROCHURES, COLOR SELECTION CHARTS OR CHIPS, AND OTHER DATA AS REQUIRED TO FULLY EXPLAIN THE INTENDED MATERIAL, AND INSTALLATION TO ALLOW SELECTION OF COLOR OR FINISHES.

CONTRACTOR TO ALLOW ONE TO TWO WEEKS FOR REVIEW DEPENDING ON SUBMITTAL DELAYS AND OR CHANGE ORDERS WILL NOT BE ALLOWED DUE TO LATE SUBMITTALS BY THE CONTRACTOR.

THE SHOP DRAWING SUBMITTALS ARE REVIEWED AS A 'COURTESY' BY THE ARCHITECT. THE REVIEW IS NOT INTENDED TO BE CONSTRUED AS AN APPROVAL OF ANY KIND, AND SHALL NOT BE THE BASIS FOR CHANGES OR DELETIONS FROM THE CONTRACT DOCUMENTS. CHANGES OR DELETIONS SHALL BE FIRST SUBMITTED TO THE ARCHITECT IN WRITING PRIOR TO THE SUBMITTAL OF THE SHOP DRAWINGS, ALONG WITH THE REASON FOR THE REQUEST.

1. THE FOLLOWING IS A LIST OF ITEMS WHICH NEED TO BE SUBMITTED FOR REVIEW. (SEE STRUCTURAL DRAWINGS FOR ADDITIONAL REQUIREMENTS)

- A. STRUCTURAL STEEL
 - B. WOOD TRUSSES
 - C. STOREFRONT SYSTEMS
 - D. PLUMBING FIXTURES AND TRIM. (AS APPLICABLE)
 - E. FIRE PROTECTION SYSTEMS. (AS APPLICABLE)
 - F. HVAC EQUIPMENT. (AS APPLICABLE)
 - G. LIGHTING FIXTURES
 - H. ELECTRICAL DISTRIBUTION EQUIPMENT (SWITCHBOARD / PANELBOARDS)
 - I. PRE-FABRICATED MASONRY TRIM
 - J. PRE-FABRICATED E.F.I.S. TRIM
 - K. FABRIC ANNINGS AND FRAMES
- ADDITIONAL REQUIREMENTS FOR ALL SUBMITTALS:

PROCEDURAL REQUIREMENTS:

COORDINATE SUBMITTALS WITH THE PROPOSED SCHEDULE AND THE ACTUAL PROGRESS OF WORK.

USE SEPARATE TRANSMITTAL FORMS FOR EACH SUBMITTAL.

STAMP AND SIGN THAT COORDINATION AND APPROVAL HAS OCCURRED IN ACCORDANCE WITH ARTICLE 3.12 OF THE GENERAL CONDITIONS A-201. NOTIFY A/E IN WRITING, DENOTING ANY DEVIATIONS FROM THE CONTRACT DOCUMENTS; OTHERWISE PLACE THE STATEMENT "SUBMITTAL IS PER PLANS AND SPECIFICATIONS" ON THE TRANSMITTAL FORM.

SUBMITTALS WITHOUT SIGNED APPROVALS OR SUBMITTALS WHICH ARE INCOMPLETE, CONTAIN NUMEROUS ERRORS, OR HAVE ONLY BEEN CHECKED SUPERFICIALLY, WILL BE RETURNED FOR RESUBMISSION.

ARCHITECT'S REVIEW OF SUBMITTALS DOES NOT RELIEVE CONTRACTOR OF RESPONSIBILITY FOR DEVIATIONS FROM CONTRACT DOCUMENTS UNLESS CONTRACTOR HAS SPECIFICALLY INFORMED ARCHITECT IN WRITING OF SUCH DEVIATION AT TIME OF SUBMITTAL.

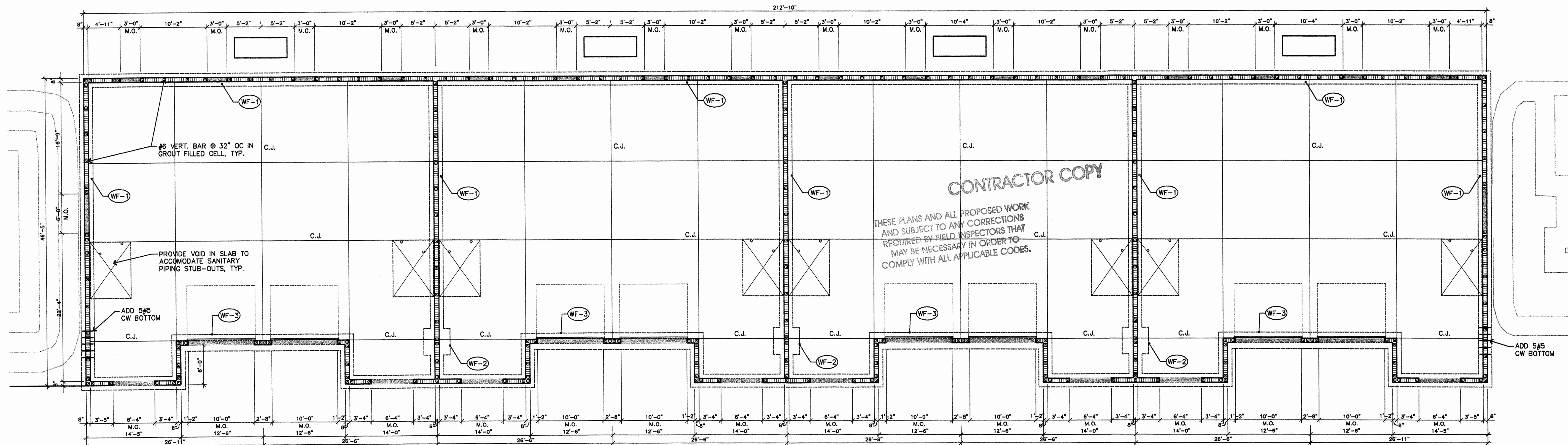
THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR QUANTITIES, DIMENSIONS AND MISCELLANEOUS NOTATIONS WHICH MAY BE SHOWN ON THE SUBMITTALS.

SHOP DRAWINGS ARE NEW FIELD COORDINATED DRAWINGS FOR THIS PROJECT SHOWING ACTUAL DETAILS, DIMENSIONS, MATERIALS, ASSEMBLY METHODS AND ATTACHMENTS FROM DESIGN DRAWINGS.

SHOP DRAWINGS - PROVIDE ONE REPRODUCIBLE COPY AND ONE BLUELINE PRINT FOR REVIEW.

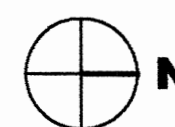
FOUNDATION SCHEDULE				
MARK	SIZE			REINFORCING
	LENGTH	WIDTH	DEPTH	
WF-1	CONT	24"	12"	3 # 5 CONT BOTT, 1#5 CONT TOP. 1 # 4 CW BOTT, @ 24" O.C.
WF-2	48"	48"	18"	5 # 6 EA WAY BOTTOM
WF-3	CONT	30"	12"	3 # 5 CONT BOTT, 1#5 CONT TOP. 1 # 45 CW BOTT, @ 24" O.C.

CONCRETE BEAM SCHEDULE		
MARK	SIZE	ELEVATION TOP OF BM
BB16	8"x16" BOND BEAM W/ 2#6 CONTINUOUS.	8'-6"
CB14	8"x14" CONC BEAM WITH 2#6 TOP AND BOTT AND #3 STIR AT 12" O.C.	9'-2"
CB16	8"x16" CONC BEAM WITH 2#6 TOP AND BOTT WITH #3 STIR AT 24" O.C.	12'-8"



FOUNDATION PLAN

SCALE: 1/8"=1'-0"



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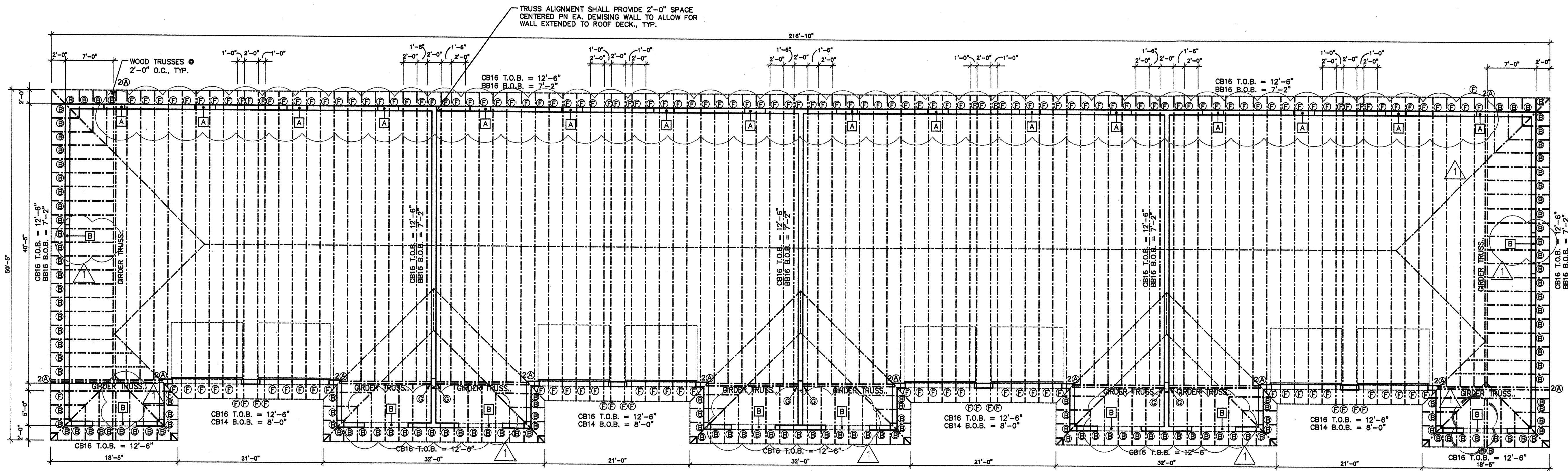
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FOUNDATION PLAN
REVISIONS

SHEET

S-1

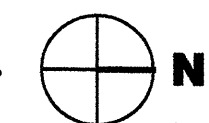
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28004

04-08-08



ROOF FRAMING PLAN

SCALE: 1/8"=1'-0"



	MANUFACTURER	COMPONENT	CONNECTOR	PRODUCT APPROVAL #	UPLIFT MAX	CONNECTOR CAPACITY	LATERAL LOADS	CONNECTOR CAPACITY	GENERAL NOTES
(A)	SIMPSON	GIRDER TRUSS	SIMPSON HGT-2 W/ 3/4" ANCHOR BOLT TO CONC BM & (B) 10d TO GIRDER	474.115	7200 #	8665 #	F1 F2	F1 F2	UPLIFT LOADS
(B)	SIMPSON	SIDE TRUSSES	SIMPSON H6 STRAP W/ 16-8d	474.410	250 #	915 #	F1 F2	F1 F2	UPLIFT LOADS
(F)	SIMPSON	MAIN TRUSSES	SIMPSON HHETA20 W/12-10d X1 1/2 USP LPTA W/10d X 1 1/2" NAILS	1901.24	1826 #	2235 #	F1 F2	F1 F2	UPLIFT LOADS
(G)	SIMPSON	GIRDER TRUSS	SIMPSON THGB2 W/ 10-10d & 2-3/4" MB TO CARRIED MEMBER 4-3/4" MB TO CONC COLUMN	FL 6482	5400 #	7540 #	F1 F2	F1 F2	LATERAL LOADS

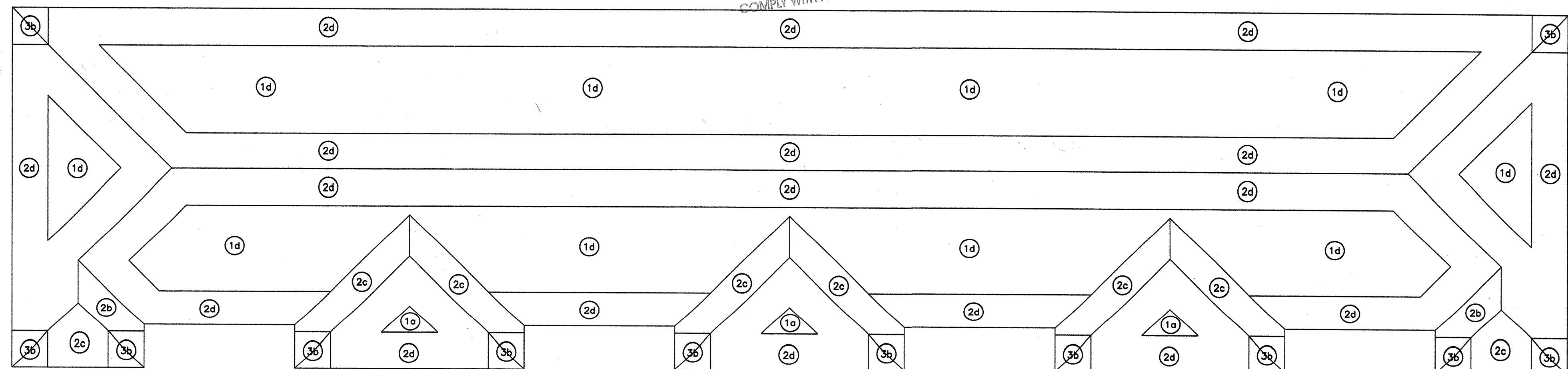
PRECAST LINTEL SCHEDULE				
MARK	LOAD	MAX SPAN	LINTEL	MAX CAPACITY
(A)	1650#/LF	3'-2"	CASTORETE 8F8-1B	2646#/LF
(B)	800#/LF	6'-4"	CASTORETE 8F16-1B/1T	3396#/LF

CONTRACTOR COPY

THESE PLANS AND ALL PROPOSED WORK AND SUBJECT TO ANY CORRECTIONS REQUIRED BY FIELD INSPECTORS THAT MAY BE NECESSARY IN ORDER TO COMPLY WITH ALL APPLICABLE CODES.

ZONE	PRESSURE (140MPH)
1a	+28.4 -45.2
1d	+20.02 -41.0
2b	+39.9 -86.5
2c	+22.5 -74.9
2d	+20.0 -66.0
3b	+25.9 -86.5

ADJUSTMENT FACTOR FOR BUILDING HEIGHT AND EXPOSURE = 1.40



ROOF PRESSURE

SCALE: 3/32"=1'-0"

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28004-S-2.dwg	ROOF FRAMING PLAN / ROOF PRESSURE
REVISIONS	PLAN REVIEW COMMENTS 10/08/08
DRAWN	RIC
DATE	04-02-2008

SHEET

S-2

COMM. NO. 28004

10-14-08