

GENERAL CONDITIONS

- IF MATERIALS, QUANTITIES, STRENGTHS OR SIZES INDICATED BY THE DRAWINGS OR SPECIFICATIONS ARE NOT IN AGREEMENT WITH THESE NOTES, THE MOST STRINGENT SHALL BE PROVIDED.
- STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH PROJECT SPECIFICATIONS. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
- IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE TO ENSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF WHATEVER TEMPORARY BRACING, GUYS OR TIE-DOWNS MAY BE NECESSARY. SUCH MATERIAL SHALL BE REMOVED AND SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER COMPLETION OF THE PROJECT.
- ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE VERIFIED BY THE CONTRACTOR TO CONFORM TO THOSE SHOWN ON THE ARCHITECTURAL DRAWINGS.
- THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. THE CONTRACTOR'S RESPONSIBILITY IS TO SUPPORT, BRACE AND SECURE EXISTING STRUCTURE AS REQUIRED. CONTRACTOR IS SOLELY RESPONSIBLE FOR THE SAFETY OF THE BUILDING DURING CONSTRUCTION.
- VISITS TO THE SITE BY THE ENGINEER WILL BE FOR OBSERVATION AND SHALL NOT INCLUDE INSPECTIONS. THE PURPOSE OF THE ENGINEER'S VISITS WILL BE TO ASSIST IN QUALITY CONTROL AND HELP ACHIEVE CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS. THE VISITS SHOULD NOT BE CONSTRUED AS SUPERVISION OF CONSTRUCTION.
- THE CONTRACTOR SHALL BE HELD RESPONSIBLE FOR HAVING VISITED THE SITE AND HAVING FAMILIARIZED HIMSELF WITH ALL EXISTING CONDITIONS. ANY QUESTIONS OR DISCREPANCIES FOUND WITH REGARD TO THE DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE DESIGNERS BEFORE SUBMITTING A PROPOSAL. FIELD MEASURE EXISTING CONDITIONS PRIOR TO FABRICATION OF MATERIALS.

DESIGN LOADS

- THE FOLLOWING DESIGN LOADS ARE IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 2023 (8TH EDITION) AND PROVISIONS OF THE ASCE 7-22.
- UNIFORMLY DISTRIBUTED DEAD LOADS:
 - PRECAST HOLLOW CORE PLANKS
 - FLOOR, CORRIDOR, & ROOF (8" HCP + 3/4" GYP TOPPING + MISC.) 76 PSF
 - BALCONY (8" HCP + 2" CONC TOPPING + MISC.) 94 PSF
- LIVE LOADS
 - PRIVATE ROOMS 40 PSF
 - BALCONIES 60 PSF
 - CORRIDORS AND PUBLIC AREAS 100 PSF
 - STAIRWAYS AND LANDINGS 100 PSF
 - STORAGE 125 PSF
 - ROOF AND AWNINGS 20 PSF
 - HANDRAILS AND GUARDS 50 PLF
 - (ANY DIRECTION AND ANY POINT) 200 LB
 - INTERMEDIATE RAILS 50 LB
 - (ANY DIRECTION AND ANY POINT) 250 LB
 - GRAB BARS (ANY DIRECTION AND ANY POINT) 250 LB
- WIND LOADS
 - ULTIMATE DESIGN WIND SPEED $V_{ult}=162$ MPH
 - NOMINAL DESIGN WIND SPEED $V_{ref}=126$ MPH
 - RISK CATEGORY II
 - WIND EXPOSURE D
 - ENCLOSURE CLASSIFICATION ENCLOSED
 - INTERNAL PRESSURE COEFFICIENT $GC_p = +1.0, 18$
 - PROJECT SITE IS NOT WITHIN WIND-BORNE DEBRIS REGION
 - SEE COMPONENTS AND CLADDING PRESSURES ON SHEET 50.2
- RAIN LOADS
 - RAIN LOAD INTENSITY $i = 4.5$ IN/HR
 - DESIGN RAIN LOAD $R =$
- FLOOD LOADS
 - FLOOD ZONE AE
 - BASE FLOOD ELEVATION (BFE) 5 FT

SUBMITTAL & SHOP DRAWINGS

- SUBMITTALS THAT DO NOT MEET THE REQUIREMENTS BELOW WILL BE RETURNED WITHOUT REVIEW.
- FOUR COPIES OF SHOP DRAWINGS SHALL BE SUBMITTED. FOLLOW THE ARCHITECT'S INSTRUCTIONS FOR DISTRIBUTION.
- THE GENERAL CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR THE FOLLOWING ITEMS
 - REINFORCING STEEL
 - PRECAST CONCRETE COMPONENTS (*)
 - STRUCTURAL STEEL
 - COLD FORMED STEEL FRAMING (*)
 - STAIRS AND RAILINGS (*)
 - ALUMINUM AWNINGS (*)

* INDICATES THAT SHOP DRAWINGS MUST BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF THE PROJECT. ALL SHOP DRAWINGS SIGNED AND SEALED BY A PROFESSIONAL ENGINEER SHALL BE ACCOMPANIED BY A SET OF SIGNED AND SEALED CALCULATIONS. SEE THE REQUIREMENTS BELOW.
- DETAILER SHALL BE RESPONSIBLE FOR CHECKING ALL ARCHITECTURAL, ELECTRICAL AND MECHANICAL DRAWINGS FOR OPENINGS AND EMBEDS, EFFECTING STRUCTURAL MEMBERS.
- THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED IN WRITING BY THE ENGINEER OF RECORD.
- CHANGES AND ADDITIONS MADE ON RESUBMITTALS SHALL BE CLEARLY INDICATED WITH A CLOUD AND NOTE. THE PURPOSE OF THE RESUBMITTAL SHALL CLEARLY BE NOTED ON THE TRANSMITTAL. THE REVIEW WILL BE LIMITED TO THOSE ITEMS CAUSING THE RESUBMITTAL.
- SHOP DRAWINGS SHALL BEAR THE INITIALS OF THE DETAILER'S CHECKER AND BE APPROVED BY THE CONTRACTOR BEFORE SUBMITTAL TO ENGINEER. NON-CONFORMING SUBMITTALS WILL BE RETURNED WITHOUT REVIEW.
 - IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC.
 - REVIEW OF THE SHOP DRAWING DOES NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS.
- THE USE OF REPRODUCTIONS OF THESE CONTRACT DRAWINGS BY ANY CONTRACTOR, SUB-CONTRACTOR, ERECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFIES HIS ACCEPTANCE OF ALL INFORMATION SHOWN HEREON AS CORRECT, AND OBLIGATES HIMSELF TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS THAT MAY OCCUR HEREON.
- IF REPRODUCTIONS OF THESE CONTRACT DRAWINGS ARE USED IN LIEU OF PREPARATION OF SHOP DRAWINGS, THE ARCHITECTS, ENGINEERS, OR OTHER DESIGN CONSULTANT'S TITLE BLOCK SHALL BE REMOVED AND REPLACED WITH A TITLE BLOCK LISTING THE FOLLOWING ITEMS:
 - NAME, ADDRESS, AND TELEPHONE NUMBER OF CONTRACTOR.
 - SUB-CONTRACTOR, ETC. SUBMITTING SHOP DRAWINGS.
 - SHEET NUMBER.
 - DATE DRAWINGS PREPARED, INITIALS OF PERSON WHO PREPARED DRAWINGS, AND PERSON WHO CHECKED DRAWINGS.

*ANY REPRODUCTION OF THESE CONTRACT DRAWINGS NOT COMPLYING WITH THE ABOVE WILL BE REJECTED WITHOUT REVIEW.
- CALCULATION REQUIREMENTS:
 - CALCULATIONS AS REQUIRED BY DRAWINGS SHALL BE SUBMITTED WITH THE REQUIRED SHOP DRAWINGS AT THE SAME TIME AS THE SUBMITTAL.
 - ALL CALCULATIONS SUBMITTED SHALL BE PRESENTED IN A LEGIBLE MANNER AND FOLLOW AN ORDERLY LOGICAL SEQUENCE.
 - ALL REFERENCES AND SOURCES OF INFORMATION SHALL BE LISTED WITHIN THE CALCULATIONS. COPIES OF PERTINENT SECTIONS SHALL BE INCLUDED.
 - BASIC LOADS AND DERIVATION OF FORCES, MOMENTS, SHEARS, REACTIONS, DEFLECTIONS, CAMBERS, ETC., SHALL BE INCLUDED. INCLUDE ALL UNBALANCED LOADING FORCES AS REQUIRED.
 - DESIGN ASSUMPTIONS SHALL BE CLEARLY STATED.
 - ALL COMPUTER GENERATED CALCULATIONS SHALL BE ACCOMPANIED WITH EXPLANATIONS OF OUTPUT/INPUT DATA (I.E. VARIABLE IDENTIFICATION, LOAD DERIVATION, ETC.), A BRIEF EXPLANATION OF GENERAL THEORY, METHOD AND ASSUMPTIONS USED SHALL BE INCLUDED.
 - ALL CALCULATIONS ARE TO BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF THE PROJECT.

FOUNDATIONS

- THE FOUNDATION DESIGN IS BASED ON AN ALLOWABLE BEARING PRESSURE OF 6,000 PSF IN ACCORDANCE WITH THE GEOTECHNICAL REPORT BY UES GEOTECHNICAL ENGINEERING PROJECT NO. 3330.2400019.0000 DATED FEBRUARY 23, 2024. REFER TO THE GEOTECHNICAL REPORT FOR SOIL IMPROVEMENT AND FOUNDATION REQUIREMENTS. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO ENSURE THAT THE SUBGRADE IS COMPACTED SUFFICIENTLY TO ACHIEVE THIS ALLOWABLE BEARING PRESSURE USED FOR DESIGN.
- EARTHWORK SHALL BE PERFORMED UNDER THE SUPERVISION OF A LICENSED SOIL TESTING COMPANY TO ASSURE COMPLIANCE WITH REQUIREMENTS OF THE SOILS REPORT AND SPECIFICATIONS.
- ALL FOUNDATIONS SHALL BE CENTERED UNDER THE COLUMNS OR WALL ABOVE UNLESS NOTED OTHERWISE.
- IF REQUIRED, PROVIDE A GRAVEL LAYER BELOW SLABS IN ACCORDANCE WITH THE GEOTECHNICAL REPORT FOR RADON GAS VENTING. REFER TO THE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ADDITIONAL DETAILS AND LOCATION OF VENT PIPES THROUGH SLABS.

REINFORCING STEEL

- ALL REINFORCING SHALL BE ASTM A615, GRADE 60. BARS SHALL SECURELY TIE IN PLACE WITH #16 DOUBLE-ANNEALED IRON WIRE. BARS SHALL BE SUPPORTED ON ACCEPTABLE CHAIRS. REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH THE ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING OF REINFORCED CONCRETE STRUCTURES." CONTRACTOR SHALL COORDINATE REINFORCING STEEL PLACEMENT DETAILS AND PROVIDE TEMPLATES FOR PLACING STEEL IN CONGESTED AREAS AS NECESSARY. SHOP DRAWINGS (INCLUDING PLACING PLANS AND ELEVATIONS) SHALL BE SUBMITTED TO AND REVIEWED BY THE ARCHITECT/ENGINEER BEFORE STARTING FABRICATION.
- WELDED WIRE FABRIC SHALL BE ELECTRICALLY WELDED AND CONFORM TO ASTM A185 (FLAT SHEET). YIELD STRENGTH 65,000 PSI. AN 8" MINIMUM LAP SHALL BE PROVIDED FOR SIDE AND END LAP. WELDED WIRE FABRIC SHALL SUPPORT IN APPROVED CHAIRS. REFER TO NOTE #6 UNDER SLAB ON GRADE CONSTRUCTION FOR FIBROUS REINFORCING OPTION.
- NO REINFORCING BARS SHALL BE SPLICED BY WELDING. AT THE CONTRACTOR'S OPTION, MECHANICAL BUTT SPLICING USING AN EXOTHERMIC WELDING PROCESS AND HIGH STRENGTH SLEEVES OR MECHANICAL CONNECTION SPLICING MAY BE USED, PROVIDED THAT THE MECHANICAL SPLICES SHALL BE ICBO APPROVED TO ACHIEVE A MINIMUM TENSILE STRENGTH OF 125 PERCENT OF THE SPECIFIED YIELD STRENGTH OF THE BAR. THE MINIMUM TENSILE STRENGTH REQUIREMENT SHALL BE INCREASED TO 160 PERCENT FOR MECHANICAL SPLICES AT THE INTERFACE OF DIAPHRAGMS AND THE LATERAL SYSTEM, AND FOR MECHANICAL SPLICES WITHIN ELEMENTS OF THE LATERAL SYSTEM. SPLICES DEVICES SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL. REINFORCING BARS SHALL BE LAP SPLICED FOR TENSION UNLESS NOTED OTHERWISE ON THE DRAWINGS. #14 AND #18 BARS SHALL NOT BE LAP SPLICED.
- WELDING OR TACK WELDING OF REINFORCING BARS TO OTHER BARS OR TOE PLATES, ANGLES ETC. IS PROHIBITED, EXCEPT WHERE SPECIFICALLY APPROVED BY THE ENGINEER. WHERE WELDING IS APPROVED, IT SHALL BE DONE BY AWS CERTIFIED WELDERS USING E9018 OR APPROVED ELECTRODES. WELDING PROCEDURES SHALL CONFORM TO THE REQUIREMENTS OF AWS D1.4.
- MINIMUM CAST-IN-PLACE CONCRETE COVER OVER REINFORCING STEEL, UNLESS NOTED OTHERWISE, SHALL BE AS FOLLOWS:
 - CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3 INCHES.
 - CONCRETE EXPOSED TO EARTH OR WEATHER: 1-1/2 INCHES FOR #5 BARS OR SMALLER 2 INCHES FOR #6 BARS OR LARGER.
- OTHER CONCRETE
 - WALLS - INTERIOR FACE: 1-1/2 INCHES FOR #14 AND #18 BARS 3/4 INCH FOR #11 BARS AND SMALLER
 - SLABS: #11 BARS AND SMALLER - 3/4 INCH
 - BEAMS AND COLUMNS - TIES, STIRRUPS AND SPIRALS: 1-1/2 INCHES
- PROVIDE CONTINUOUS REINFORCEMENT WHEREVER POSSIBLE. SPLICE ONLY AS SHOWN OR APPROVED. STAGGER SPLICES WHERE POSSIBLE. USE TENSION SPLICE CLASS "B" UNLESS NOTED OTHERWISE. DOWELS SHALL MATCH SIZE AND SPACING OF THE SPECIFIED REINFORCEMENT AND SHALL BE LAPPED WITH TENSION SPLICES UNLESS NOTED OTHERWISE. LAP LENGTHS EXPRESSED IN NUMBER OF BAR DIAMETERS SHALL BE AS FOLLOWS:

BAR SIZE	NORMAL WEIGHT CONCRETE F _c (PSI)				
	CLASS	3,000	4,000	5,000	6,000
#6 OR SMALLER	A	44 DIA	38 DIA	34 DIA	31 DIA
	B	57 DIA	49 DIA	44 DIA	40 DIA
#7 OR LARGER	A	55 DIA	47 DIA	42 DIA	39 DIA
	B	71 DIA	62 DIA	55 DIA	50 DIA

LAP SPLICE LENGTH NOTES:

- TABLE IS BASED ON (a) CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED NOT LESS THAN ONE BAR DIAMETER (BD), CLEAR COVER NOT LESS THAN ONE DB, AND STIRRUPS OR TIES THROUGHOUT THE LAP SPLICE LENGTH NOT LESS THAN THE CODE MINIMUM, OR (b) CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED NOT LESS THAN 2 DB AND CLEAR COVER NOT LESS THAN ONE DB. FOR ALL OTHER CASES, MULTIPLY TENSION LAP BY 1.5.
- FOR TENSION REINFORCEMENT WITH MORE THAN 12" OF CONCRETE BELOW, OR FOR VERTICAL REINFORCEMENT, MULTIPLY THE LAP SPLICE LENGTH INDICATED IN THE TABLE BY 1.3. HOWEVER, THE LAP SPLICE LENGTH SHALL NOT BE LESS THAN 12".
- FOR TENSION REINFORCEMENT IN LIGHTWEIGHT CONCRETE, MULTIPLY LAP SPLICED LENGTH BY 1.3.

CAST-IN-PLACE CONCRETE

- CONCRETE SHALL BE NORMAL WEIGHT AND HAVE THE FOLLOWING MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS:

PLACEMENT	MINIMUM STRENGTH	MAX WATER/CEMENT RATIO
FOOTINGS / SLAB ON GRADE:	3000 PSI	0.58
REMAINING CONCRETE:	4000 PSI	0.54
- ALL CONCRETE SHALL HAVE A SLUMP OF 4" PLUS OR MINUS 1", AND HAVE 2 TO 4% AIR ENTRAINMENT.
- CONCRETE MIX DESIGN SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF ACI 301 CHAPTER 3, METHOD 1 OR METHOD 2. SUBMIT BACKUP DATA AS REQUIRED BY CHAPTER 5 SECTION 5.3. OF THE LATEST EDITION OF ACI 318.
- ALL REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM A-615 GRADE 60.
- ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH "THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" ACI 318 LATEST EDITION, AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS," ACI 301, LATEST EDITION.
- ALL REINFORCING DETAILS SHALL CONFORM TO "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" ACI 315 LATEST EDITION, UNLESS DETAILED OTHERWISE ON THE STRUCTURAL DRAWINGS.
- CONTRACTOR SHALL REVIEW ARCHITECTURAL, ELECTRICAL AND MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF EMBEDDED ITEMS, SLEEVES, SLAB DEPRESSIONS, OPENINGS, ANCHOR BOLTS, SLOPES, ETC. REQUIRED BY OTHER TRADES. THESE ITEMS SHALL BE FURNISHED AND INSTALLED PRIOR TO PLACEMENT OF CONCRETE.
- WHERE BAR LENGTHS ARE GIVEN ON THE DRAWINGS, THE LENGTH OF ANY HOOK, IF REQUIRED, IS NOT INCLUDED. HOOKS SHALL BE PROVIDED AT DISCONTINUOUS ENDS OF ALL TOP BARS OF BEAMS AND AT SLAB EDGES.
- CONTRACTOR SHALL PROVIDE SPACERS, CHAIRS, BOLSTERS, ETC. NECESSARY TO SUPPORT REINFORCING STEEL. SUPPORT ITEMS WHICH BEAR ON EXPOSED CONCRETE SURFACES SHALL HAVE ENDS WHICH ARE PLASTIC TIPPED OR STAINLESS STEEL.
- CONTRACTOR SHALL PROVIDE 3/4 INCH CHAMFER ON ALL EXPOSED CORNERS OF COLUMNS, BEAMS, AND WALLS UNLESS OTHERWISE INDICATED ON THE ARCHITECTURAL DRAWINGS.
- THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:
 - CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH. 2" CONCRETE FORMED AND PERMANENTLY EXPOSED TO EARTH. 1-1/2", ALL OTHER CONCRETE.
- HORIZONTAL WALL AND FOOTING BARS SHALL BE BENT 1'-0" AROUND CORNERS OR CORNER BARS WITH 2'-0" LAP SHALL BE PROVIDED.
- HORIZONTAL KEYWAYS IN CONSTRUCTION JOINTS SHALL BE PROVIDED IN BEAMS, SUPPORTED SLABS, AND WALL FOOTINGS WITH A DEPTH OF 1-1/2" AND HEIGHT EQUAL TO ONE-THIRD OF THE MEMBER'S DEPTH. REINFORCEMENT SHALL BE CONTINUOUS THROUGH CONSTRUCTION JOINTS UNLESS OTHERWISE NOTED ON THE DRAWINGS. CONSTRUCTION JOINTS MAY BE USED ONLY AT LOCATIONS SHOWN ON THE DRAWINGS OR AT OTHER LOCATIONS APPROVED BY THE ARCHITECT.
- CONTRACTOR SHALL KEEP A COPY OF "FIELD REFERENCE MANUAL" (ACI PUBLICATION SP-15, LATEST EDITION) AT THE PROJECT FIELD OFFICE.
- MINIMUM LAP LENGTHS ON ALL REINFORCING BAR SPLICES SHALL BE 48 BAR DIAMETERS. FOR BEAMS AND ELEVATED SLABS, LAP BOTTOM STEEL AT THE SUPPORT AND TOP STEEL OVER THE MID-SPAN, UNLESS OTHERWISE NOTED.
- TESTING LABORATORY SHALL SUBMIT ONE COPY OF ALL CONCRETE TEST REPORTS DIRECTLY TO THE ENGINEER.
- PROVIDE YYPEX, OR APPROVED EQUAL, CONCRETE WATERPROOFING ADMIXTURE FOR RETAINING WALLS AND THEIR FOUNDATIONS BELOW GRADE.

CONCRETE SLAB ON GRADE

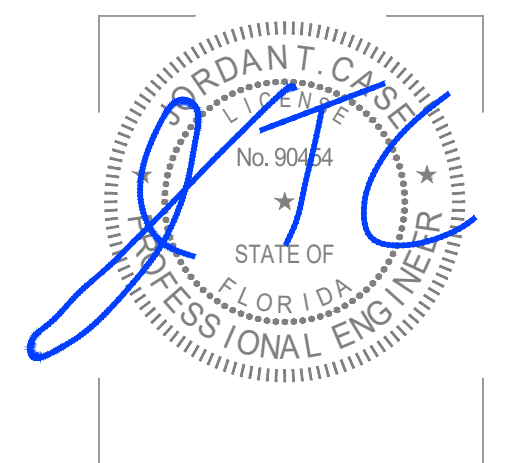
- SPECIFICATION: UNLESS NOTED OTHERWISE, SLAB ON GRADE CONSTRUCTION SHALL FOLLOW THE RECOMMENDATIONS OF "THE GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION ACI 302.1R-04".
- SUBGRADE PREPARATION: A. AREAS CONTAINING SLAB ON GRADE CONSTRUCTION SHALL BE STRIPPED TO SUFFICIENT DEPTH TO REMOVED ALL VEGETATION, TOP SOIL, ORGANIC AND OTHER UNSUITABLE MATERIALS.
- REINFORCING STEEL:
 - SEE THE DRAWINGS FOR TYPICAL SLAB REINFORCEMENT REQUIREMENTS.
 - LAP CONTINUOUS SLAB ON GRADE REINFORCING STEEL 30 BAR DIAMETERS AT SPLICES BUT NOT LESS THAN 12".
 - ALL REINFORCING STEEL FOR SLABS ON GRADE SHALL BE CHAIRED WITH SLAB BOLSTERS DESIGNED FOR SUPPORT ON SOIL TO PROVIDED SPECIFIED COVER TO REINFORCING STEEL.
- SEE DETAILS ON THE DRAWINGS FOR REQUIREMENTS OF CONSTRUCTION JOINTS, CONTROL JOINTS AND POUR SIZE AND SEQUENCE OF SLAB ON GRADE CONSTRUCTION.
- OPENINGS ON SLAB ON GRADE: UNLESS DETAILED OTHERWISE ON THE DRAWINGS, PROVIDE (1) #5 x OPENING WIDTH +3'-0" AROUND ALL PENETRATION THROUGH SLAB ON GRADE 10" WIDE OR GREATER AND (1) #5 x 3'-0" DIAGONAL CORNERS BARS. SUCH REINFORCEMENT SHALL BE PROVIDED WHETHER OPENING IS SHOWN ON DRAWINGS OR NOT.
- FIBROUS REINFORCING (OPTION): FIBROUS REINFORCING SHALL BE 100 PERCENT VIRGIN POLYPROPYLENE, FIBRILLATED FIBERS CONTAINING NO REPROCESS EDOLFIN MATERIALS AND SPECIFICALLY MANUFACTURED TO AN OPTIMUM GRADATION FOR USE AS CONCRETE SECONDARY REINFORCEMENT. VOLUME PER CUBIC YARD SHALL EQUAL A MINIMUM OF 0.1% (1.5 LBS).
- SLAB ON GRADE SHALL BE PLACED OVER A CONTINUOUS 10 MIL VAPOR BARRIER.

PRECAST HOLLOW CORE PLANKS

- PRECAST PLANKS SHALL BE 6" OR 8" PRESTRESSED HOLLOW CONCRETE PLANKS. MACHINE EXTRUDED WITH ZERO SLUMP AND 3'-4" OR 4'-0" NOMINAL WIDTH. THE PRESTRESSED CONCRETE MANUFACTURERS PLANT SHALL CONFORM TO THE REQUIREMENTS DEFINED IN PRESTRESSED CONCRETE INSTITUTES S 116-MANUAL FOR QUALITY CONTROL FOR PLANTS AND PRODUCTION OF PRECAST, PRESTRESSED CONCRETE PRODUCTS.
- CONCRETE COMPRESSIVE STRENGTH- 3500 PSI AT TRANSFER AND 5000 PSI AT 28 DAYS.
- PRESTRESSED STRAND- UNCOATED 7 WIRE CABLE CONFORMING TO ASTM A416 A21.D. DESIGN IN ACCORDANCE WITH ACI 318. LIVE LOAD DEFLECTION SHALL NOT EXCEED THE SPAN/360.
- THE PRESTRESSED HOLLOW CORE PLANKS SHALL MEET THE TESTED REQUIREMENTS FOR THE RESTRAINED AND UNRESTRAINED TWO (2) HOURS FIRE RESISTANCE RATING.
- SUBMIT SHOP DRAWINGS SHOWING SETTING PLAN, UNIT SIZE, STEEL CONTENT, AND ALL NECESSARY DETAILS. ALL SHOP DRAWINGS TO BE CHECKED AND INITIALED BY CONTRACTOR. SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF THE PROJECT. INSTALL A TIGHT, LEVEL POSITION ON TRUE LEVEL BEARING SURFACE TO DISTRIBUTE BEARING STRESS. MINIMUM BEARING 3" ON MASONRY AND 3" ON STRUCTURAL STEEL.
- SUPPLY AND INSTALL STEEL HANGERS WHERE INDICATED ON STRUCTURAL DRAWINGS FOR LARGE OPENINGS. ERECTORS SHALL DRILL ALL THRU OPENINGS UP TO 6" IN DIAMETER REQUESTED BY MECHANICAL TRADES ON SITE. OPENINGS LARGER THAN 6" TO BE LOCATED ON SHOP DRAWINGS AT THE TIME OF APPROVAL AND TO BE CUT IN THE FIELD. DO NOT CUT REINFORCEMENT WITHOUT APPROVAL OF PRECAST PLANK MANUFACTURER AND ENGINEER.
- GROUT PRECAST UNITS WITH 1:3 CEMENT-SAND GROUT MIX, PROPERLY FILLING SHEAR KEYWAYS BETWEEN UNITS.
- CONCRETE TOPPING OR LOADING SHALL NOT BE DONE PRIOR TO GROUTING OF KEYWAYS. TOPPING SHALL BE NORMAL WEIGHT CONCRETE OF 3000 PSI COMPRESSIVE STRENGTH TO A MINIMUM CONCRETE THICKNESS OF 2" REINFORCED WITH 6x6-W1.4xW1.4 WVF. ADEQUATE BOND TO BE ACCOMPLISHED BY CLEANING AND WETTING PLANKS BEFORE POURING TOPPING.
- NOTE TO GENERAL CONTRACTOR: USE EXTREME CAUTION WHEN DRILLING OR SHOOTING INTO PRECAST CONCRETE PLANKS. CONTACT PLANK MFR FOR LOCATION OF ALL STEEL. USE DRILL STOP TO AVOID DAMAGE TO STEEL.
- PROVIDE CONTROL JOINTS AT ALL EXTERIOR TOPPING EXPOSED TO VIEW. THE JOINTS SHALL BE PLACED DIRECTLY OVER PLANK JOINTS. JOINTS SHALL BE 1/8" WIDE x 1" DEEP GROOVE.

MASONRY CONSTRUCTION

- CONSTRUCTION SHALL MEET THE REQUIREMENTS OF ACI 530/ASCE 5, "BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY" AND ACI 530.1/ASCE6, "SPECIFICATIONS FOR THE DESIGN AND CONSTRUCTION OF LOAD BEARING CONCRETE MASONRY. ALL HOLLOW LOAD BEARING CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, MEDIUM WEIGHT, TYPE 1. ALL UNITS SHALL BE LAID IN RUNNING BOND USING TYPE S MORTAR WITH HEAD JOINTS. MASONRY MINIMUM STRENGTH F_m = 1500 PSI. MINIMUM BLOCK COMPRESSIVE STRENGTH IS 1800 PSI.
- USE TYPE S MORTAR IN ACCORDANCE WITH ASTM C270 EXCEPT TYPE M MORTAR BELOW GRADE. HEAD AND BED JOINTS SHALL BE 3/8" FOR THE THICKNESS OF THE FACE SHELL. WEBS ARE TO BE FULLY MORTARED IN ALL COURSES OF PIERS, COLUMNS AND PILASTERS. IN STARTING COURSE, AND WHERE AN ADJACENT CELL IS TO BE GROUTED, REMOVE MORTAR PROTRUSIONS EXTENDING 1/2" OR MORE INTO THE CELL TO BE GROUTED.
- USE STANDARD (9 GAUGE) HORIZONTAL JOINT REINFORCING CONFORMING TO ASTM A-82 IN EVERY OTHER COURSE. OVERLAP DISCONTINUOUS END 6". USE PREFABRICATED CORNER AND TEES. USE LADDER TYPE JOINT REINFORCING.
- ALL CELLS CONTAINING REINFORCEMENT SHALL BE FILLED SOLID WITH CONCRETE GROUT CONFORMING TO ASTM C-476. GROUT MIXED SHALL CONTAIN PORTLAND CEMENT ONLY PLUS AGGREGATE. MAXIMUM SIZE OF AGGREGATE SHALL BE 3/8 INCH. SLUMP SHALL BE 8 TO 11 INCHES. WATER REDUCING ADMIXTURES MAY BE USED. GROUT SHALL BE VIBRATED WHILE PLACING TO ENSURE THAT CELLS ARE COMPLETELY FILLED. MINIMUM GROUT COMPRESSIVE STRENGTH BASED ON 28 DAY TEST SHALL BE 3000 PSI USING 3 1/2"x3 1/2"x7" OR 3"x3"x6" TEST SPECIMENS CAST IN MOIST WITH A FIST NON-ABSORBENT BASE AND MASONRY UNITS HAVING THE SAME MOISTURE CONDITIONS AT THOSE BEING LAID FORMING THE SIDE OF THE SPECIMENS. SPECIMEN SHALL BE TESTED ACCORDING TO ASTM C39.
- VERTICAL BARS SHALL BE HELD IN POSITION AT THE TOP AND THE BOTTOM AND INTERVAL NOT EXCEEDING 8'-0". WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH CORRESPONDING VERTICAL BAR, IT SHALL NO BE SLOPED MORE THAN ONE HORIZONTAL IN SIX VERTICAL.
- THE MINIMUM SPLICE LENGTH FOR ALL VERTICAL BARS SHALL BE:
 - 31 INCHES FOR #5 BARS
 - 56 INCHES FOR #6 BARS
 - 75 INCHES FOR #7 BARS
- PROVIDE DOVETAIL ANCHORS AT 16" C/C, UNLESS NOTED OTHERWISE, WHERE MASONRY WALLS ABUT CONCRETE SURFACES.



PERMIT SET
07-11-2024

AA C000606

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REVISION

JOB NO.
22039

DATE
07-11-2024

SHEET

S0.1

STRUCTURAL STEEL

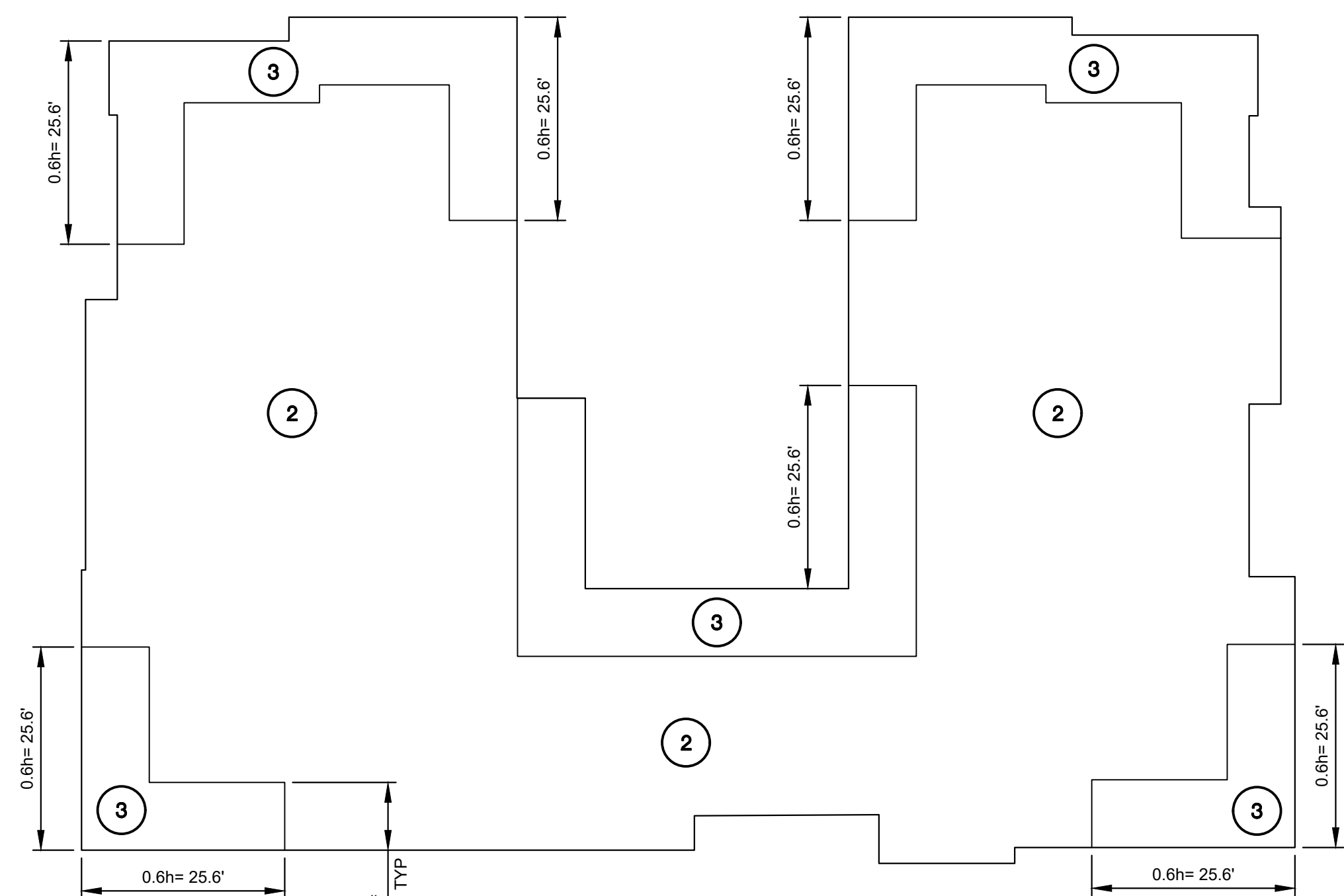
- ALL STRUCTURAL "W" SHAPES TO CONFORM TO ASTM A992 WITH MINIMUM YIELD STRENGTH OF 50KSI. ANGLES, CHANNEL AND PLATES SHALL CONFORM TO ASTM A36 WITH MINIMUM YIELD STRENGTH OF 36 KSI.
- ALL SHOP AND FIELD WELDING SHALL BE DONE BY CURRENTLY CERTIFIED WELDERS IN ACCORDANCE WITH AWS D1.1 "STRUCTURAL WELDING CODE" LATEST EDITION.
- USE E70XX ELECTRODES FOR ALL WELDING, UNLESS NOTED OTHERWISE. (GRIND SMOOTH ALL EXPOSED WELDS.
- ALL HSS SHAPES SHALL CONFORM TO ASTM A-500, GRADE B, WITH A MINIMUM YIELD STRESS OF 46 KSI.
- ALL BOLTS IN STRUCTURAL CONNECTIONS SHALL CONFORM TO ASTM A325 TYPE 1, HIGH STRENGTH BOLTS FOR STRUCTURAL STEEL JOINTS UNLESS NOTED OTHERWISE ON THE DRAWINGS. MINIMUM BOLT DIAMETER SHALL BE 3/4 INCHES, UNLESS NOTED OTHERWISE IN THE DRAWINGS OR IN THESE GENERAL NOTES. ALL BOLTED CONNECTIONS SHALL BE BEARING TYPE CONNECTIONS USING STANDARD HOLES WITH THREAD INCLUDED IN THE PLANES. HIGH STRENGTH BEARING BOLTS SHALL BE TIGHTENED USING AN IMPACT WRENCH TO A SNUG TIGHT CONDITION. THE SNUG TIGHT CONDITION IS DEFINED AS THE TIGHTENED ATTAINED BY FEW IMPACT OF AN IMPACT WRENCH OR THE FULL EFFORT OF A MAN USING AN ORDINARY SPUD WRENCH.
- HOT DIP GALVANIZING AFTER FABRICATION ALL STRUCTURAL STEEL ITEM AND THEIR CONNECTIONS PERMANENTLY EXPOSED TO THE OUTSIDE, WHETHER SPECIFIED ON THE DRAWING OR NOT.
- STRUCTURAL STEEL SHOP DRAWING SHALL SHOW ALL WELDING WITH AWS A2.4 SYMBOLS. ALL WELDING SHALL BE DONE BY AWS CERTIFIED WELDERS AND IN ACCORDANCE WITH AWS D1.1. WELDS SHOWN IN THE DRAWINGS ARE THE MINIMUM SIZES. INCREASE WELD SIZE TO AWS MINIMUM SIZES, BASED ON PLATE THICKNESS. THE MINIMUM WELD SIZE SHALL BE 3/16 INCH. FIELD WELDING SYMBOLS HAVE NOT NECESSARILY BEEN INDICATED ON THE DRAWINGS. WHERE SHOWN, PROPER FIELD WELDING PER AWS D1.1 SHALL BE USED. WHERE NO FIELD WELDING SYMBOLS ARE SHOWN, IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE THE USE OF SHOP AND FIELD WELDS.
- NON SHRINK GROUT FOR BASE PLATES AND BEARING PLATES SHALL BE NON METALLIC, SHRINKAGE RESISTANCE, PREMIXED, NON CORROSIVE, NON STAINING PRODUCT CONTAINING PORTLAND CEMENT, SILICA SANDS, SHRINKAGE COMPENSATING AGENTS, AND FLUIDITY IMPROVING COMPOUND. TWENTY EIGHT DAY COMPRESSIVE STRENGTH AS DETERMINED BY GROUT TUBE TESTS SHALL BE 6,000 PSI. GROUT SHALL BE PLACED IN A FLUID FLOWABLE STATE UNDER BASE PLATES THAT HAVE A FORM BUILT AROUND FOR GROUT CONFINEMENT. GROUT SHOULD BE CURED ACCORDING TO MANUFACTURER'S RECOMMENDATIONS. MINIMUM THICKNESS OF GROUT UNDER ALL BASE PLATES AND BEARING PLATES SHALL BE 1", UNLESS OTHERWISE SPECIFIED ON THE DRAWINGS.
- ANCHOR RODS SHALL BE ASTM F1554 GRADE 36 WITH CLASS 1A TREADS UNLESS NOTED OTHERWISE ON DRAWINGS. FURNISH HARDENED PLATE WASHERS, LOCK WASHERS AND MATCHING HEAVY HEX NUTS FOR SECURING THE BASE PLATE TO THE ANCHOR RODS. ALL NUTS USED WITH ANCHOR BOLTS SHALL BE HEX HEAD CONFORMING TO ASTM A563. WASHERS FOR ALL BASE PLATE SHALL BE 1/4" THICK PLATES EXTENDING MINIMUM 1" FROM EDGE OF BASE PLATE HOLE ON EACH SIDE WITH HOLES 1/16" LARGER THAN NOMINAL BOLT DIAMETER. WASHERS SHALL COMFORT TO A36 STEEL. ANCHOR RODS INSTALLATION SHALL BE COORDINATED WITH REINFORCING AND FORMWORK. AFTER BASE INSTALLATION, ANCHOR RODS NUTS SHALL BE INSTALLED TO A SNUG TIGHT CONDITION, NO HEATING OR BENDING OF THE ANCHOR RODS IS PERMITTED. HOLES IN THE BASE MATERIAL SHALL NOT BE ENLARGED.
- STEEL STAIR FRAMING AND CONNECTION DESIGN SHALL BE THE RESPONSIBILITY OF THE STAIRWAY SUPPLIER, PERFORMANCE BY OR UNDER THE SUPERVISION OF A PROFESSIONAL ENGINEER IN THE STATE OF FLORIDA. ALL STAIRWAY STEEL SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY A REGISTERED ENGINEER IN THE STATE OF "CHANGE TO STATE OF PROJECT".

COLD FORMED STEEL CONSTRUCTION

- DESIGN OF COLD FORMED STEEL FRAMING SHALL CONFORM TO THE LATEST EDITION OF "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STRUCTURAL STEEL MEMBERS (AIS)."
- MATERIAL: STUDS, RUNNER, AND ANGLES SHALL MEET THE REQUIREMENTS OF ASTM C955 WITH MINIMUM YIELD STRENGTH AS FOLLOWS:
16, 14, AND 12 GAGE STUDS 50 KSI
22, 20, AND 28 GAGE STUDS 33 KSI
RUNNERS 33 KSI
- GALVANIZED FINISH SHALL COMPLY WITH ASTM A653/A653M WITH A G90 COATING. ALL WELDS SHALL BE TOUCHED UP WITH ZINC-RICH PROTECTIVE PAINT FOR CORROSION RESISTANCE.
- THE COLD FORMED STEEL CONSTRUCTION SHALL BE A DELEGATED DESIGN. THE FABRICATOR SHALL FURNISH A STRUCTURAL SUBMITTAL BEARING THE SEAL AND SIGNATURE OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF FLORIDA. THIS SUBMITTAL SHALL BE CHECKED BY THE CONTRACTOR FOR COMPLETENESS AND CONTENT PRIOR TO SUBMITTAL TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW. THE SUBMITTAL SHALL INCLUDE COMPONENT DETAILS AND SYSTEM LAYOUT DRAWINGS. IT SHALL IDENTIFY THE PROJECT LIST LOADING AND OTHER CRITERIA. THE DRAWING SHALL IDENTIFY AND LOCATE COMPONENTS AND SHALL SPECIFY MEMBER SIZES BRACING, ANCHORAGE CONNECTIONS & ALL OTHER NECESSARY FABRICATION AND ERECTION INFORMATION. THE SUBMITTAL SHALL INCLUDE CALCULATIONS VERIFYING ITS ADEQUACY TO RESIST THE LOADS INDICATED ON THE CONSTRUCTION DOCUMENTS. FABRICATION SHALL NOT COMMENCE UNTIL THIS REVIEW IS COMPLETED.
- COLD FORMED STEEL FRAMING INCLUDE BUT NOT LIMITED TO TRUSSES, WALL, EXTERIOR CEILINGS, FASCIAS, AND SOFFITS.

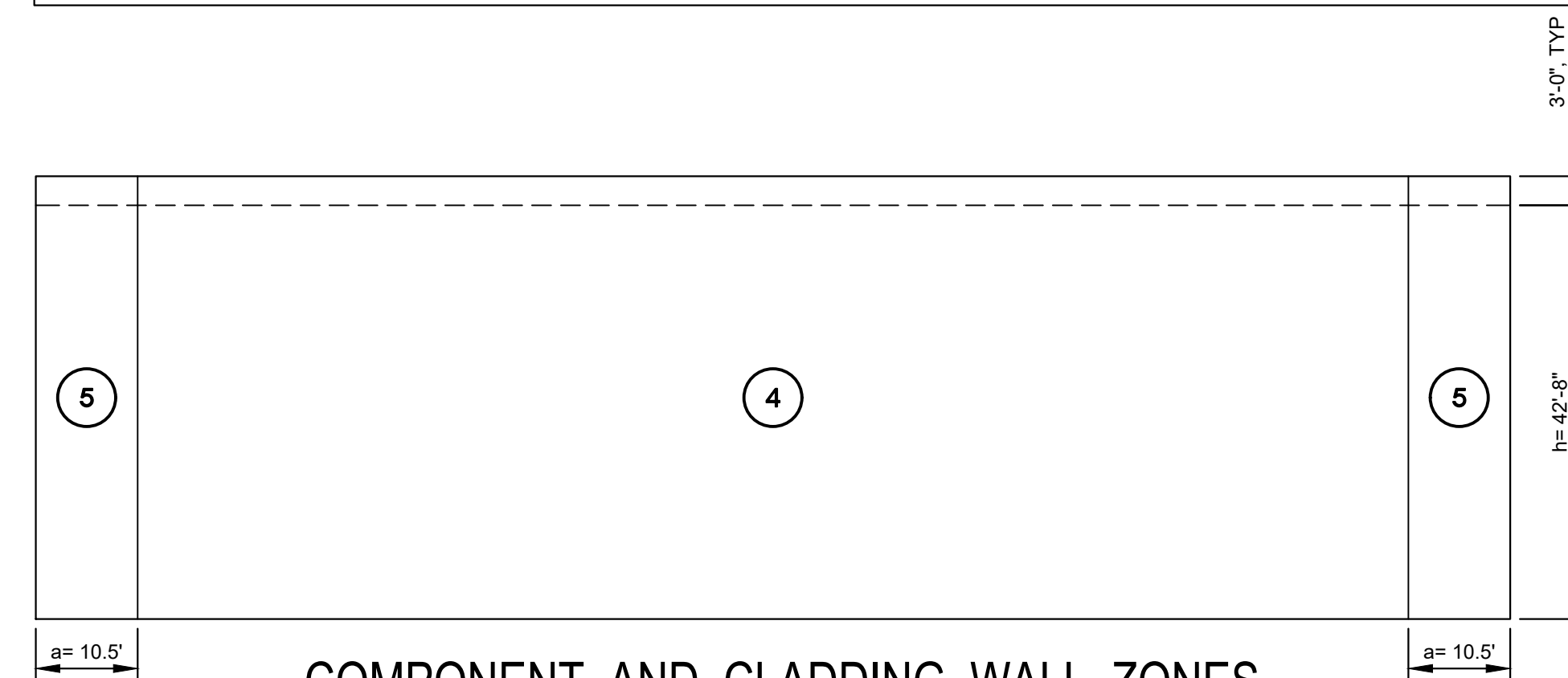
ABBREVIATIONS

ABBREV	ABBREVIATION	FD	FLOOR DRAIN	PC	PRECAST CONCRETE
ACI	AMERICAN CONCRETE INSTITUTE	FDN, FNDN	FOUNDATION	PCF	POUNDS PER CUBIC FEET
ADD	ADDITIVE	FIN	FINISH	PEN	PENETRATION
ADDL	ADDITIONAL	FLR	FLOOR	PERP	PERPENDICULAR
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	FMG	FRAMING	PJF	PRE-MOLDED BITUMINOUS JOINT FILLER
AIISI	AMERICAN IRON AND STEEL INSTITUTE	FP	FULL PENETRATION WELD	PL	PLATE
ALT	ALTERNATE / ALTERNATIVE	FS	FAR SIDE	PLF	POUNDS PER LINEAR FOOT
ALUM	ALUMINUM	FT	FOOT / FEET	PLY	PLYWOOD PREFAB PREFABRICATED
AR	ANCHOR ROD	FTG	FOOTING	PSF	POUNDS PER SQUARE FOOT
ARCH	ARCHITECTURE / ARCHITECTURAL			PSI	POUNDS PER SQUARE INCH
ASD	ALLOWABLE STRESS DESIGN			PT	PRESSURE TREATED
ASPH	ASPHALT	GA	GAGE / GAUGE		
ASTM	AMERICAN SOCIETY OF TESTING	GALV	GALVANIZED	QTY	QUANTITY
MATERIALS		GB	GRADE BEAM		
AWS	AMERICAN WELDING SOCIETY	GC	GENERAL CONTRACTOR		
		GEN	GENERAL	R	RADIUS
B/	BOTTOM OF	GL	GRID LINE	RCP	REINFORCED CONCRETE PIPE
BIT	BITUMINOUS / BITUMASTIC	GRTG	GRATING	RD	ROOF DRAIN
BLDG	BUILDING	GS	GALVANIZED STEEL	REF	REFERENCE
BLK	BLOCK			REINF	REINFORCEMENT
BM	BEAM	HC, HCP	HOLLOW CORE PLANK	REQ'D	REQUIRED
BMM	BENCH MARK	HDG	HOT DIP GALVANIZED	RW	RETAINING WALL
BOT	BOTTOM	HK	HOOK		
BP	BASE PLATE	HORIZ	HORIZONTAL	SCHED	SCHEDULE
BRG	BRACING	HSA	HEADED STUD ANCHOR	SDS	"SELF DRILLING" SHEET METAL SCREW(S)
BTWN	BETWEEN	HSB	HIGH STRENGTH BOLT	SIM	SIMILAR
BU	BUILT UP	HSS	HOLLOW STRUCTURAL STEEL	SF	SQUARE FOOT
		HT	HEIGHT	SFTG	STEP FOOTING
				SJ	SAWCUT / CONTRACTION JOINT
C	CHANNEL	IAW	IN ACCORDANCE WITH	SJ	STEEL JOIST INSTITUTE
CB	CONCRETE BEAM	ID	INSIDE DIAMETER	SOG	SLAB ON GRADE
CC	CONCRETE COLUMN	IF	INSIDE FACE	SPC	SPACE / SPACES
CF	CUBIC FEET (FOOT)	IN	INCH	SPCG	SPACING SPECS SPECIFICATION(S)
CFS	COLD FORMED STEEL	INFO	INFORMATION	SO	SQUARE
CI	CAST-IRON	INT	INTERIOR	SS	STAINLESS STEEL
CIP	CAST IN PLACE	INV	INVERT	SSLT	SHORT SLOTTED TRANSVERSE
CJ	CONSTRUCTION / CONTRACTION JOINT			SSMA	STEEL STUD MANUFACTURER'S ASSOCIATION
CL	CENTERLINE	JST	JOIST	STD	STANDARD
CLR	CLEAR / CLEARANCE	JT	JOINT	STIFF	STIFFENER
CM	CONCRETE MASONRY			STL	STEEL
CMU	CONCRETE MASONRY UNIT	K	KIP, 1000 LB	STRUCT	STRUCTURE / STRUCTURAL
COL	COLUMN	KO	KNOCK OUT	SYM	SYMMETRICAL
CONC	CONCRETE	KWY	KEYWAY		
CSA	CONCRETE SCREW ANCHOR(S)				
CONN	CONNECTION	L	ANGLE	T/	TOP OF
CONST	CONSTRUCTION	LB	POUND	T&B	TOP AND BOTTOM
CONT	CONTINUOUS	LDG	LANDING	TB	TIE BEAM
COORD	COORDINATE	LDT	LONG DIAMETER TAPCON	TDS	TURNED DOWN SLAB
CS	CAST-STEEL	LG	LONG	TEMP	TEMPERATURE
CTR	CENTER	LSTH	LENGTH	TENS	TENSION
CTRD	CENTERED	LLH	LONG LEG HORIZONTAL	THD	THREAD / THREADED
CVR	COVER	LLV	LONG LEG VERTICAL LOC LOCATION	THK	THICK
CY	CUBIC YARD	LRFD	LOAD RESISTANCE FACTOR DESIGN	THRU	THROUGH
		LSH	LONG SIDE HORIZONTAL	TOL	TOLERANCE
DBA	DEFORMED BAR ANCHOR	LSV	LONG SIDE VERTICAL	Typ	TYPICAL
DEG	DEGREE	LT WT	LIGHT WEIGHT	TSWF	THICKENED SLAB WALL FOOTINGS
DEMO	DEMOLITION			UNO	UNLESS NOTED OTHERWISE
DEPT	DEPARTMENT	MATL	MATERIAL	VERT	VERTICAL
DIA	DIAMETER	MAX	MAXIMUM	VOL	VOLUME
DIAG	DIAGONAL	MB	MASONRY BEAM / MACHINE BOLT		
DIM	DIMENSION	MECH	MECHANICAL	W, WF	WIDE FLANGE
DIST	DISTANCE	MFP	MECHANICAL / ELECTRICAL / PLUMBING	W /	WITH
DN	DOWN	MFG	MANUFACTURING	WD	WOOD
DR	DRAIN	MFR	MANUFACTURE / MANUFACTURER	WF	WALL FOOTING
DTL	DETAIL	MID	MIDDLE	WFE	WALL FOOTING EXTENSION
DWG	DRAWING(S)	MIN	MINIMUM	WH	WEEP HOLE
		MISC	MISCELLANEOUS	W / O	WITHOUT
EA	EACH	MO	MASONRY OPENING	WP	WORK POINT
EE	EACH END	MPH	MILES PER HOUR	WS	WATER STOP
EF	EACH FACE	MTL	METAL	WT	STRUCTURAL TEE SECTION
EJ	EXPANSION JOINT			WWR	WELDED WIRE REINFORCING
EL	ELEVATION				
ELEC	ELECTRIC / ELECTRICAL	NIC	NOT IN CONTRACT	@	AT
ELEV	ELEVATOR	NO	NUMBER	#	NUMBER OR POUND
ENGR	ENGINEER	NS	NEAR SIDE	+ -	PLUS OR MINUS
EOS	EDGE OF SLAB	NTS	NOT TO SCALE	Ø	DIAMETER
EQ	EQUAL			&	AND
EQUIP	EQUIPMENT	OC	ON CENTER	⊕	SECTION MODULUS
ES	EACH SIDE	OD	OUTSIDE DIAMETER	⊖	MOMENT OF INERTIA
EW	EACH WAY	OF	OUTSIDE FACE	⊗	CENTERLINE
EXIST	EXISTING	OPNG	OPENING	⊙	
EXP	EXPANSION	OPP	OPPOSITE PAR PARALLEL	⊚	
EXT	EXTERIOR			⊛	



COMPONENT AND CLADDING ROOF ZONES

	a	h
(FT)	(FT)	(FT)
HOTEL	10.5	42.7

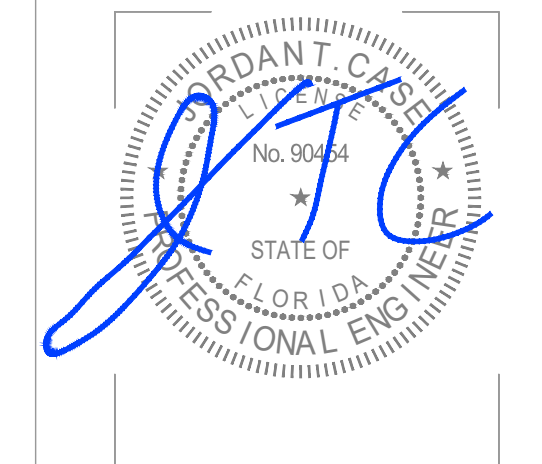


COMPONENT AND CLADDING WALL ZONES

COMPONENTS AND CLADDING ULTIMATE PRESSURES

AREA (FT ²)	ZONE 1'	ZONE 1	ZONE 2	ZONE 3	ZONE 4	ZONE 5	45'-8" PARAPET				49'-1" PARAPET									
							ZONE 4		ZONE 5		ZONE 4		ZONE 5							
							CASE A	CASE B	CASE A	CASE B	CASE A	CASE B	CASE A	CASE B						
10	28.9	-65.0	28.9	-113	65.0	-149	65.0	-149	65.0	-70.4	65.0	-86.6	195	-115	195	-132	198	-117	198	-134
20	27.1	-65.0	27.1	-106	62.1	-140	62.1	-140	62.1	-67.5	62.1	-80.9	183	-109	183	-123	185	-111	185	-125
50	24.7	-65.0	24.7	-95.8	58.3	-127	58.3	-127	58.3	-63.7	58.3	-73.3	166	-102	166	-111	168	-103	168	-113
100	22.9	-65.0	22.9	-88.3	55.4	-117	55.4	-117	55.4	-60.8	55.4	-67.5	153	-95.9	153	-103	155	-97.3	155	-104
200	22.9	-55.9	22.9	-80.9	52.4	-108	52.4	-108	52.4	-58.0	52.4	-61.8	141	-90	141	-93.9	143	-91.4	143	-95.3

- PRESSURE VALUES ARE INDICATED IN PSF (POUNDS / FT²).
- POSITIVE VALUES ACT TOWARD TO SURFACES WHILE NEGATIVE VALUES ACT AWAY FROM SURFACES.
- WIND PRESSURES SHOWN ARE ULTIMATE LOAD VALUES. TO OBTAIN SERVICE LOAD DESIGN PRESSURES FOR ASCE 7-22 LOAD COMBINATIONS, MULTIPLY THE PRESSURES SHOWN BY A FACTOR OF 0.60.



PERMIT SET
07-11-2024

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TRU HOTEL BY HILTON
RAJ PATEL
STUART, FLORIDA

REVISION

JOB NO.
22039
DATE
07-11-2024
SHEET

S0.2

STRUCTURAL THRESHOLD INSPECTION PLAN

PART 1 - ADMINISTRATION

1.1 EXPLANATION OF THE STRUCTURAL INSPECTION PLAN

- A. THIS IS A THRESHOLD BUILDING AS DEFINED BY CHAPTER 553 OF THE FLORIDA STATUTES AND REQUIRES THE OWNER TO RETAIN A SPECIAL INSPECTOR.
- B. THE SPECIAL INSPECTOR SHALL INSPECT THE FRAMING SHOWN ON THE STRUCTURAL DRAWINGS IN ACCORDANCE WITH THIS PLAN TO VERIFY THAT THE WORK IS CONSTRUCTED IN SUBSTANTIAL ACCORDANCE WITH THE CONTRACT DOCUMENTS, EXCEPT FOR ACCEPTED VARIATIONS
1. THE CONTRACT DOCUMENTS ARE DEFINED AS THE PERMITTED DRAWINGS, PROJECT SPECIFICATIONS AND RECORDED ADDENDA AND AMENDMENTS.
 2. EXCEPT AS NOTED, WORK OF THIS PLAN EXCLUDES THE INSPECTION OF ELEMENTS THAT DO NOT CONTRIBUTE TO THE CAPACITY OF THE PRIMARY STRUCTURAL FRAME. THIS INCLUDES, BUT IS NOT LIMITED TO: RAILINGS, FIRE PROTECTION, ROOFING, GLAZED WINDOW SYSTEMS, MECHANICAL/ELECTRICAL SYSTEMS, ARCHITECTURAL COMPONENTS, SITE WORK, AND OSHA SAFETY PROVISIONS OR OTHER SAFETY STANDARDS THAT APPLY DURING THE CONSTRUCTION PERIOD. SLAB-ON-GRADE IS SPECIFICALLY INCLUDED.

- C. PROPOSALS FOR SPECIAL INSPECTION SERVICES SHALL BE SEPARATE AND INDEPENDENT FROM PROPOSALS FOR ALL OTHER MATERIALS TESTING AND QUALITY ASSURANCE SERVICES.
- D. THE OWNER OR ENFORCING AGENCY MAY SUPPLEMENT THIS PLAN WITH ADDITIONAL INSPECTION REQUIREMENTS AS DEEMED NECESSARY DURING THE COURSE AT THE WORK.

1.2 QUALIFICATIONS OF THE SPECIAL INSPECTOR

- A. THE SPECIAL INSPECTOR MAY SEND AN AUTHORIZED REPRESENTATIVE UNDER HIS RESPONSIBLE CHARGE TO UNDERTAKE INSPECTIONS AND SITE VISITS. AS USED IN THIS PLAN, THE TERM SPECIAL INSPECTOR INCLUDES HIS REPRESENTATIVE, UNLESS INDICATED OTHERWISE.
- B. BOTH THE SPECIAL INSPECTOR AND THE REPRESENTATIVE SHALL MEET THE QUALIFICATIONS AND STANDARDS SET FORTH BY RULE CHAPTER 61G15-35 AND CHAPTER 471 OF THE FLORIDA STATUTES. RESUMES OF BOTH THE SPECIAL INSPECTOR AND THE REPRESENTATIVE(S) SHALL BE SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW.
- C. THE SPECIAL INSPECTOR SHALL PROVIDE TO THE OWNER INSURANCE CERTIFICATE FOR ALL APPLICABLE COVERAGES, INCLUDING GENERAL LIABILITY, AUTOMOBILE, WORKMEN'S COMPENSATION, AND PROFESSIONAL LIABILITY SPECIFICALLY COVERING THE INSPECTIONS DESCRIBED IN THIS PLAN.

1.3 RESPONSIBILITIES AND LIMITATIONS OF THE SPECIAL INSPECTOR

- A. THE SPECIAL INSPECTOR IS RESPONSIBLE TO THE OWNER AND THE ENFORCING AGENCY HAVING JURISDICTION FOR THIS PROJECT. THE SPECIAL INSPECTOR AND THE AUTHORIZED REPRESENTATIVE SHALL:
1. BECOME FAMILIAR WITH THE SPECIFIC STRUCTURAL COMPONENTS AND SYSTEMS THEY WILL BE INSPECTING.
 2. DEVELOP A THOROUGH KNOWLEDGE OF THE APPLICABLE CONTRACT DOCUMENTS, ACCEPTED SUBMITTALS, AND GOVERNING CODES.
 3. EXERCISE GOOD JUDGMENT.
- B. PRIOR TO STARTING WORK, THE SPECIAL INSPECTOR SHALL PROVIDE TO THE OWNER AND STRUCTURAL ENGINEER A CERTIFIED AFFIDAVIT ATTESTING THAT:
1. HE IS COMPETENT TO PROVIDE THE ENGINEERING SERVICES FOR THE SPECIFIC TYPE OF STRUCTURE.
 2. HE HAS REVIEWED THE CONTRACT DOCUMENTS AND UNDERSTANDS THEIR CONTENT AND THE SUPPORT CONVEYED THEREIN.
 3. HE HAS THE STRUCTURAL INSPECTION PLAN, UNDERSTANDS THE INTENT AND INTENDS TO COMPLY WITH ITS REQUIREMENTS.

- C. THE SPECIAL INSPECTOR IS RESPONSIBLE FOR INSPECTING THE STRUCTURE AS PREVIOUSLY DESCRIBED. REPORTS SHALL IDENTIFY WORK PERFORMED IN GENERAL CONFORMANCE WITH THE CONTRACT DOCUMENTS, AS WELL AS THE LOCATION AND DESCRIPTION OF ANY DEVIATIONS.
- D. THE SPECIAL INSPECTOR IS RESPONSIBLE FOR VERIFYING THAT THE WORK OF THE TESTING AGENCY COMPLIES WITH THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.
- E. THE SPECIAL INSPECTOR SHALL VISIT THE SITE AT FREQUENCY TO SATISFY HIMSELF THAT HIS REPRESENTATIVE'S INSPECTIONS COMPLY WITH THIS PLAN.
- F. THE SPECIAL INSPECTOR SHALL DEDICATE HIS ENTIRE TIME ON-SITE TO THE REQUIREMENTS OF THIS PLAN AND SHALL NOT PERFORM OTHER WORK, SUCH AS MATERIAL TESTING.
- G. THE SPECIAL INSPECTOR SHALL CALL THE STRUCTURAL ENGINEER AT LEAST ONCE EACH WEEK TO DISCUSS DETAILS OF THE PROJECT AND TO CONFIRM THAT HE HAS UP-TO-DATE SKETCHES AND RFI RESPONSES.
- H. THE PRESENCE OF THE SPECIAL INSPECTOR DOES NOT RELIEVE THE ENFORCING AGENCY, THE ARCHITECT OR THE STRUCTURAL ENGINEER OF THEIR RESPONSIBILITIES. THE SPECIAL INSPECTOR SHALL NOT MAKE DESIGN DECISIONS OR INTERPRETATIONS OF THE CONTRACT DOCUMENTS.

1.4 REPORTING

- A. THE SPECIAL INSPECTOR SHALL RECORD PROGRESS, WORKING CONDITIONS, OBSERVATIONS, TESTING, DEVIATIONS FROM THE CONTRACT DOCUMENTS, AND ANY REQUIRED CORRECTIVE ACTION. HE SHALL RETAIN THE RECORDS FOR A MINIMUM OF 7 YEARS AFTER COMPLETION OF THE PROJECT.
- B. THE SPECIAL INSPECTOR SHALL IMMEDIATELY NOTIFY THE CONTRACTOR IN PERSON, AND THE ARCHITECT AND STRUCTURAL ENGINEER BY TELEPHONE, OF MATERIALS TESTS, EQUIPMENT, WORKMANSHIP OR CONSTRUCTION THAT:
1. DOES NOT CONFORM TO THE CONTRACT DOCUMENTS, OR
 2. IS NOT INSPECTED OR TESTED AND CANNOT BE INSPECTED OR TESTED IN PLACE.

- C. SPECIAL INSPECTOR SHALL KEEP AN EXCEPTIONS FILE AND REVIEW IT ON A DAILY BASIS, UPDATING AS EXCEPTIONS ARE RECTIFIED. IF ANY EXCEPTIONS ARE NOT RESOLVED IN A TIMELY MANNER, THE SPECIAL INSPECTOR SHALL ISSUE A NON-COMPLIANCE NOTICE TO THE CONTRACTOR AND SHALL COPY THE ENFORCING AGENCY, OWNER'S REPRESENTATIVE, ARCHITECT AND STRUCTURAL ENGINEER.
- D. AFTER EACH INSPECTION, THE INSPECTOR SHALL WRITE AND SIGN ON INSPECTION REPORT. THE REPORT SHALL INCLUDE THE FOLLOWING:

1. THE NAME AND LOCATION OF PROJECT; NAME OF INSPECTOR; PERMIT NUMBER; DATE; WORKING CONDITIONS; INCLUDING WEATHER AND TEMPERATURE; AND TYPE AND LOCATION OF WORK BEING PERFORMED.
2. DETAILS OF EACH INSPECTION, INCLUDING THE PRESENCE AND ACTIVITIES OF THE TESTING AGENCY.
3. NOTE DEFICIENCIES IN THE WORK AND ANY UNUSUAL CIRCUMSTANCES AFFECTING THE PERFORMANCE OF WORK, INCLUDING CHANGES IN MATERIALS OR WORK SEQUENCE. PLACE EMPHASIS ON RECURRING DEFICIENCIES.
4. IDENTIFY CORRECTIONS TO DEFICIENCIES LISTED IN PREVIOUS REPORTS.

- E. SINCE THE SPECIAL INSPECTOR DOES NOT CERTIFY THAT THE CONTRACT DOCUMENTS ARE IN COMPLIANCE WITH THE GOVERNING CODES, ALL STATEMENTS ISSUED SHALL REFER TO WHETHER OR NOT COMPLETED WORK IS IN SUBSTANTIAL ACCORDANCE WITH THE CONTRACT NUMBERS.

1.5 CONTRACTOR REQUIREMENTS

- A. THE CONTRACTOR SHALL PROVIDE THE SPECIAL CONTRACTOR WITH AN OFFICE AT THE CONSTRUCTION SITE THAT INCLUDES, AS A MINIMUM, A DESK, CHAIR, PLAN TABLE, PLAN RACK, FILING CABINET, TELEPHONE, UTILITIES, AIR CONDITIONING, JANITORIAL SERVICE, AND ACCESS TO A FAX MACHINE OR TO THE INTERNET.
- B. THE CONTRACTOR SHALL FURNISH TO THE SPECIAL INSPECTOR COPIES OF ALL REVIEWED AND APPROVED SUBMITTALS FOR STRUCTURAL ELEMENTS, EXCLUDING CALCULATIONS, IN A TIMELY MANNER.
- C. THE CONTRACTOR SHALL COOPERATE WITH AND ASSIST THE SPECIAL INSPECTOR IN PERFORMING HIS INSPECTION DUTIES AND SHALL PROVIDE HIM WITH FREE ACCESS TO THE WORK AT ALL TIMES.
- D. THE INSPECTOR SHALL MAKE SPOT INSPECTIONS OF WORK IN PROGRESS IN AN EFFORT TO DETECT DEVIATIONS EARLY. HOWEVER, THIS DOES NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR CORRECTING DEVIATIONS FROM THE CONTRACT DOCUMENTS DISCOVERED AT ANY TIME.
- E. IN ORDER TO ASSURE TIMELY AND APPROPRIATE INSPECTIONS, THE CONTRACTOR SHALL NOTIFY THE SPECIAL INSPECTOR OF CONSTRUCTION SCHEDULES AND PLANNED OPERATIONS A MINIMUM OF 24 HOURS PRIOR TO THE TIME OF THE INSPECTION. REINFORCING STEEL INSPECTIONS SHALL BE SCHEDULED FOR AT LEAST ONE HOUR PRIOR TO CONCRETE DELIVERY.
- F. IF ANY OF THE CONTRACTOR'S WORK IS NOT IN COMPLIANCE WITH THE CONTRACT DOCUMENTS, HE MAY EITHER CORRECT IT OR SUBMIT TO THE ARCHITECT/ENGINEER A REQUEST FOR ACCEPTANCE OF THE DEVIATION.
- G. CONSTRUCTION THAT IS NOT INSPECTED AND THAT CANNOT BE INSPECTED MAY REQUIRE TESTING OR REMOVAL AS DETERMINED BY THE STRUCTURAL ENGINEER.
- H. IF WORK IS NOT SUBSTANTIAL IN ACCORDANCE WITH THE CONTRACT DOCUMENTS, OR IF ANY CONSTRUCTION IS NOT INSPECTED AND CANNOT BE INSPECTED, THE SPECIAL INSPECTOR WILL NOT BE ABLE TO ISSUE A COMPLETION STATEMENT AND THE BUILDING WILL NOT RECEIVE A CERTIFICATE OF OCCUPANCY.
- I. SPECIAL INSPECTIONS DO NOT RELIEVE THE CONTRACTOR OF HIS RESPONSIBILITY FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS, ANY STATUTORY OR CONTRACTUAL OBLIGATIONS, NOR HIS RESPONSIBILITIES TO CARRY OUT QUALITY CONTROL INSPECTIONS AND TESTING. THE CONTRACTOR HAS THE SOLE RESPONSIBILITY FOR ANY DEVIATIONS FROM THE CONTRACT DOCUMENTS AND SHALL BEAR THE COST OF ANY REQUIRED CORRECTIONS. WHILE THE SPECIAL INSPECTOR SHALL COOPERATE WITH THE CONTRACTOR, HE SHALL NEITHER DIRECT THE CONTRACTOR'S WORK NOR BE RESPONSIBLE FOR HIS CONSTRUCTION MEANS AND METHODS.

1.6 OWNER REQUIREMENTS

- A. THE OWNER SHALL ARRANGE FOR THE SPECIAL INSPECTOR TO RECEIVE ALL APPROPRIATE CONTRACT DOCUMENTS, INCLUDING TWO COMPLETE SETS OF ALL ARCHITECTURAL AND STRUCTURAL DOCUMENTS, INCLUDING DRAWINGS AND SPECIFICATIONS, AND THE GEOTECHNICAL REPORT AT THE START OF CONSTRUCTION. HE SHALL ALSO ENSURE THAT THE SPECIAL INSPECTOR RECEIVES IN A TIMELY MANNER ALL MATERIAL TEST REPORTS AND TWO COPIES OF ALL STRUCTURAL CHANGES, REVISIONS, ADDENDA, ETC.
- B. THE OWNER SHALL RETAIN A GEOTECHNICAL CONSULTANT AND TESTING AGENCY AS REQUIRED BY THE CONTRACT DOCUMENTS.

PART 2 - INSPECTIONS

2.1 GENERAL

THE FOLLOWING IS A GENERAL INSPECTION PLAN DESCRIBING WORK TO BE PERFORMED BY THE SPECIAL INSPECTOR. THE PURPOSE OF THE STRUCTURAL INSPECTION PLAN IS TO PROVIDE SPECIFIC AREAS FOR INSPECTION SO THAT THE BUILDING CAN BE ADEQUATELY INSPECTED FOR COMPLIANCE WITH THE PERMIT DOCUMENTS. THE FOLLOWING ARE NOT INSPECTOR CHECK LISTS BUT POINT OUT SOME CRITICAL AREAS REQUIRING SPECIFIC ATTENTION BY THE SPECIAL INSPECTOR.

2.2 SHALLOW FOUNDATION PREPARATION

- A. THE GEOTECHNICAL CONSULTANT OR TESTING AGENCY RETAINED BY THE OWNER SHALL MONITOR AND TEST BACKFILL AND COMPACTION OPERATIONS AND SHALL CONFIRM THAT SUBGRADE PREPARATION OF ALL SHALLOW FOUNDATIONS COMPLIES WITH THE GEOTECHNICAL REPORT AND THE CONTRACT DOCUMENTS. HE SHALL FURNISH THE SPECIAL INSPECTOR WITH DAILY REPORTS AS WELL AS THE COMPLETION REPORT REQUIRED BY THE CONTRACT DOCUMENTS.

2.3 SLAB-ON-GRADE PREPARATION

- A. THE GEOTECHNICAL CONSULTANT OR TESTING AGENCY RETAINED BY THE OWNER SHALL MONITOR AND TEST ALL SLAB-ON-GRADE BACKFILL AND COMPACTION OPERATIONS TO CONFIRM THAT SUBGRADE PREPARATION COMPLIES WITH THE GEOTECHNICAL REPORT AND THE CONTRACT DOCUMENTS. HE SHALL FURNISH THE SPECIAL INSPECTOR WITH DAILY REPORTS.
- B. VERIFY THAT SUBGRADE DISTURBED BY OTHER TRADES IS PROPERLY RECOMPACTED.

2.4 CAST-IN-PLACE CONCRETE

- A. THE CONTRACTOR SHALL NOTIFY THE SPECIAL INSPECTOR A MINIMUM OF 24 HOURS PRIOR TO THE PLACEMENT OF ANY STRUCTURAL CONCRETE.
- B. VERIFY THAT THE SLAB THICKNESS IS MAINTAINED AT SLAB DEPRESSIONS AND STEPS.
- C. INSPECT REINFORCING STEEL AND POST-TENSIONING TENDONS PER THE STRUCTURAL DRAWINGS, SUPPLEMENTED BY THE APPROVED SHOP DRAWINGS, TO VERIFY THE FOLLOWING:

1. REINFORCING STEEL GRADE, SIZE, QUANTITY, CONFIGURATION AND SPACING.
2. POST-TENSIONING TENDON SIZE, QUANTITY, PROFILES, ANCHORAGE AND SPACING.
3. BURSTING STEEL AT ANCHORAGE ENDS AND CORNERS FOR POST-TENSIONING TENDONS.
4. TENDON SHEATHING IS INTACT.
5. REINFORCING STEEL AND TENDONS ARE ADEQUATELY SUPPORTED AND TIED TO RESIST DISPLACEMENT DURING CONCRETE POUR AND THAT CONCRETE COVER IS PROPER.
6. HOOKED BARS ARE PLACED PROPERLY.
7. EMBEDMENT LENGTHS SPLICE LOCATIONS AND LAP SPLICE LENGTHS ARE ACCEPTABLE.
8. REPORT ALL SLAB OPENINGS LARGER THAN 12" AND NOT SHOWN ON THE CONTRACT DOCUMENTS TO THE ARCHITECT/ENGINEER. CHECK ELEMENTS FOR ADDITIONAL REINFORCEMENT AROUND OPENINGS. OPENINGS THROUGH BEAMS ARE PROHIBITED WITHOUT PRIOR APPROVAL OF THE ARCHITECT/ENGINEER.
9. CONFIRM THAT REINFORCING STEEL SURFACES ARE FREE OF EXCESS RUST OR OTHER COATINGS THAT MAY ADVERSELY AFFECT BONDING CAPACITY. VERIFY THAT FORM OIL, WHERE USED, IS APPLIED BEFORE REINFORCING STEEL IS PLACED.
10. VERIFY THAT TENSIONING DEVICES HAVE BEEN CALIBRATED.
11. OBSERVE POST-TENSIONING STRESSING AND CONFIRM PROPER SEQUENCE.
12. COMPARE MEASURED TENDON ELONGATIONS WITH CALCULATED ELONGATIONS TO VERIFY CONFORMANCE WITH TOLERANCES.

D. INSPECT WELDED WIRE REINFORCEMENT TO VERIFY THE FOLLOWING:

1. WIRE SIZE AND SPACING IN EACH DIRECTION COMPLY WITH THE STRUCTURAL DRAWINGS.
2. WWR IS ADEQUATELY SUPPORTED AND TIED TO RESIST DISPLACEMENT DURING CONCRETE POUR AND THAT CONCRETE COVER IS PROPER.
3. SPLICES ARE ADEQUATE LENGTH AND PROPERLY TIED.
4. WWR SURFACES ARE FREE OF EXCESS RUST OR OTHER COATINGS THAT MAY ADVERSELY AFFECT BONDING CAPACITY.

- E. CHECK THAT ANCHORS, EMBEDDED CONDUITS, AND LOAD CARRYING EMBEDDED ITEMS ARE PROPERLY POSITIONED AND SECURED TO RESIST DISPLACEMENT DURING CONCRETING. RELOCATION OF OR MODIFICATION TO STRUCTURAL ITEMS DUE TO CONFLICT WITH REINFORCING IS PROHIBITED WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER.
- F. VERIFY THAT SAWCUT JOINTS ARE:

1. PROVIDED ONLY WHERE INDICATED.
2. THE PROPER DEPTH, SPACING AND LOCATION.
3. PROVIDED IN THE TIME FRAME INDICATED IN THE CONTRACT DOCUMENTS.

- G. CHECK THAT ALL FOREIGN MATERIAL IS REMOVED FROM FORMS OR EXCAVATIONS PRIOR TO CONCRETING.
- H. BE ON SITE AT THE START OF EACH CONCRETE POUR REQUIRING INSPECTION BY THIS PLAN. REMAIN ON SITE FOR A SUFFICIENT TIME TO CONFIRM THAT CONCRETING PRACTICES ARE PROPER AND COMPLY WITH ACI 301, ASTM C94 AND OTHER RECOGNIZED INDUSTRY STANDARDS, INCLUDING THE FOLLOWING:

1. ALL REINFORCING STEEL CORRECTIONS ARE COMPLETED PRIOR TO CASTING CONCRETE.
2. THE FIRST CONCRETE TRUCK OF EACH TYPE OF CONCRETE POUR HAS THE PROPER CONCRETE MIX NUMBER AND STRENGTH AND THAT THE BATCH TIME LEAVES SUFFICIENT TIME TO POUR ALL CONCRETE FROM THE TRUCK. SPOT CHECK FUTURE TRUCKS.
3. THE TESTING AGENCY IS ON SITE FOR EACH TYPE OF CONCRETE POUR TO TEST CONCRETE AS REQUIRED BY THE CONTRACT DOCUMENTS, INCLUDING MIXING TIME, TEMPERATURE, SLUMP, AND AIR CONTENT, AND REPORTS ANY CONCRETE DELIVERED THAT IS NOT AS SPECIFIED. CHECK THAT ADDITION OF WATER TO THE CONCRETE MIX IN THE FIELD COMPLIES WITH THE GUIDELINES IN THE CONTRACT DOCUMENTS.
4. CONCRETE IS BEING CONVEYED FROM MIXER TO PLACE OF FINAL DEPOSIT BY RECOGNIZED INDUSTRY STANDARDS. CONCRETE IS BEING PLACED CONTINUOUSLY, OR IN A MANNER TO AVOID PLACING CONCRETE AGAINST HARDENED CONCRETE, RESULTING IN THE FORMATION OF SEAMS OR PLANES OF WEAKNESS.
5. CONCRETE IS BEING CONSOLIDATED AND THOROUGHLY WORKED AROUND REINFORCEMENT, EMBEDDED ITEMS AND INTO CORNERS OF FORMS, ELIMINATING AIR OR STONE PACKETS THAT MAY CAUSE HONEYCOMBING, PITTING OR PLANES OF WEAKNESS.
6. SPOT CHECK THAT DURING PROCEDURES COMPLY WITH THE CONTRACT DOCUMENTS, ACI 308, "STANDARD PRACTICE FOR CURING CONCRETE" AND OTHER RECOGNIZED INDUSTRY STANDARDS.

- I. INSPECT EXPANSION ANCHORS AND CHEMICAL ADHESIVE FOR ANCHORING REINFORCING STEEL AND THREADED RODS USED TO SUPPORT WORK DESCRIBED IN THIS PLAN AS FOLLOWS:

1. VERIFY HOLES DIAMETER, DEPTH, LOCATION SPACING AND EDGE DISTANCE AND CONFIRM THAT HOLE HAS BEEN THOROUGHLY CLEANED AS REQUIRED BY MANUFACTURER.
2. VERIFY THAT EXPANSION ANCHORS ARE PROPERLY TIGHTENED.
3. VERIFY THAT THE EPOXY TYPE IS PROPER FOR THE APPLICATION, SUCH AS PASTE FOR HORIZONTAL HOLES. VERIFY THAT EPOXY MIXING AND INSTALLATION COMPLY WITH MANUFACTURER'S REQUIREMENTS.

2.5 MASONRY

- A. INSPECT ALL REINFORCED MASONRY PER ACI 530, ACI 530.1 AND AS FOLLOWS:

1. SPOT CHECK THAT MATERIALS ARE PROPERLY STORED.
2. WORKMANSHIP
3. UNIT SIZE, STRENGTH, AND WEIGHT
4. MORTAR AND GROUT TYPE, MIXING AND PLACEMENT.
5. PLACEMENT OF REINFORCING STEEL AND JOINT REINFORCING, INSERTS, AND ANCHORS AND OTHER STRUCTURAL ASPECTS.
6. CHECK CLEANOUT AREAS TO CONFIRM THAT CELLS TO BE REINFORCED ARE CLEAN AND FREE OF ALL FOREIGN MATERIAL AND THAT VERTICAL BAR IS TIED
7. CONTROL JOINT CONSTRUCTION.
8. DURING GROUTING, VERIFY THAT ALL CELLS ARE FILLED SOLID AND THAT GROUT IS CONSOLIDATED AS REQUIRED BY THE CONTRACT DOCUMENTS. IF IN DOUBT AFTER COMPLETION, CHECK USING A HAMMER.
9. CHECK CONNECTION OF SIDE OR TOP OF WALL TO SUPPORTING ELEMENTS.

2.6 HOLLOW CORE PLANKS

- A. GENERAL
1. VERIFY THAT THE DESIGN IS BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF FLORIDA.
 2. CHECK FOR PROPER ON-SITE STORAGE PRIOR TO ERECTION.
 3. VERIFY THAT THE PLANKS AND OTHER COMPONENTS ARE IN ACCORDANCE WITH ALL REQUIREMENTS PRIOR TO ERECTION.
 4. CONFIRM THAT THE SHOP DRAWINGS ON SITE ARE APPROVED BY THE ARCHITECT AND ENGINEER.
 5. CHECK THAT ALL CONNECTIONS ARE PER THE CONSTRUCTION DOCUMENTS AND THAT THE PRECAST PLANKS ARE INSTALLED AS DESIGNED.
 6. VERIFY REBAR PLACEMENT BEFORE GROUTING AT ALL REINFORCEMENT.
 7. VERIFY THAT ALL TEST REPORTS AND CERTIFICATES ARE SUBMITTED BY THE PLANT FABRICATOR AS REQUIRED BY THE DESIGN DOCUMENTS AND SPECIFICATIONS.
 8. VERIFY THAT THE CAMBER REQUIREMENTS ARE IN ACCORDANCE WITH THE PRECAST MANUFACTURER'S SHOP DRAWINGS.
 9. VERIFY THAT THE PLANK BEARING MEETS THE DESIGN AND CODE REQUIREMENTS.

2.7 STEEL FRAMING

- A. INSPECT ALL STEEL MEMBERS AND FRAMING AS REQUIRED BY THE CONTRACT DOCUMENTS. USE BOTH THE CONTRACT DOCUMENTS AND THE SHOP DRAWINGS FOR ALL INSPECTIONS. VERIFY THAT ALL FIELD WELDERS ARE AWS CERTIFIED FOR THE TYPE OF WELDS BEING MADE.
- B. STRUCTURAL STEEL SHOP INSPECTIONS:

1. THE CONTRACT DOCUMENTS PLACE RESPONSIBILITY FOR SHOP INSPECTION AND TESTING WITH THE FABRICATOR'S QUALITY CONTROL PROGRAM. THE SPECIAL INSPECTOR SHALL BECOME FAMILIAR WITH THIS PROGRAM.
 2. REVIEW THE TEST REPORTS; VERIFY WELDER'S CERTIFICATES, AND SPOT CHECK SPECIFIC CONDITIONS. ISSUE A REPORT OF ALL FINDINGS.
- C. FIELD INSPECTION OF STEEL CONSTRUCTION: PROVIDE ALL INSPECTION REQUIRED BY THE CONTRACT DOCUMENTS. COMPLETE ALL INSPECTIONS AND VERIFY COMPLIANCE PRIOR TO CONCRETING OR CONCEALMENT.

1. CHECK STEEL AS RECEIVED FOR POSSIBLE DAMAGE DURING SHIPPING.
2. CHECK STRUCTURAL STEEL MEMBER SIZES AND GRADES.
3. SPOT CHECK MEMBER STRAIGHTNESS, FINISH, AND CAMBER. CONFIRM THAT PAINT IS ONLY APPLIED ON APPROPRIATE SURFACE.
4. INSPECT SETTING OF ANCHOR BOLTS, EMBEDS, AND OTHER MISCELLANEOUS STRUCTURAL ITEMS FOR SIZES, QUANTITY AND FINISH. CHECK THE INSTALLATION OF COLUMN BASE PLATES FOR PROPER LEVELING, GROUT TYPE AND GROUT APPLICATION.
5. INSPECT ALL FIELD CONNECTIONS AND SPOT CHECK SHOP CONNECTIONS. VERIFY CONNECTION MATERIAL, SIZES, CONFIGURATIONS, AND FIT-UP.

- a. VISUALLY EXAMINE ALL FIELD WELDS AND SPOT CHECK ALL SHOP WELDS FOR TYPE, SIZE, LENGTH, AND QUALITY. VERIFY THAT SPECIFIED TESTING IS PERFORMED BY THE TESTING AGENCY. VERIFY THAT WELDS ARE CLEAN AND FREE FROM SLAG AND THAT RUST PROTECTION HAS BEEN APPLIED AS PER SPECIFICATIONS.
- b. VERIFY THE TYPE, SIZE, AND QUANTITY OF BOLTS IN ALL BOLTED CONNECTIONS. CHECK THAT BOLTS ARE CLEAN AND LUBRICATED, HAVE PROPER WASHERS, AND CONFORM TO THE SPECIFICATIONS. CHECK THAT BOLT HOLES ARE THE SPECIFIED TYPE AND SIZE. SPOT CHECK THE BOLT TIGHTENED SEQUENCE.

- D. OPEN WEB STEEL JOISTS
1. INSPECT STEEL JOISTS TO VERIFY TYPE, SIZE, SPACING, CONNECTIONS, STRAIGHTNESS AND FINISH. INSPECT FROM CONSTRUCTION DOCUMENTS AND SHOP DRAWINGS.
 2. CHECK THAT BEARING CONDITIONS CONFORM TO REQUIREMENTS OF SJI AND CONTRACT DOCUMENTS.
 3. VERIFY THAT THE TEMPORARY BRACING AND PERMANENT CROSS-BRIDGING ARE BEING IMPLEMENTED IN ACCORDANCE WITH THE REQUIREMENTS OF THE CONSTRUCTION DOCUMENTS AND ERECTION DRAWINGS.
 4. INSPECT SETTING OF SHELF ANGLES, BEARING PLATES, AND MISCELLANEOUS STRUCTURAL ITEMS TO VERIFY SIZE, QUANTITY, LOCATION, AND FINISH.
 5. VISUALLY EXAMINE ALL FIELD WELDS. VERIFY THAT WELDERS ARE AWS CERTIFIED.
 6. CHECK THAT ALL ITEMS ATTACHED TO, SUSPENDED FROM, OR BEARING ON THE JOISTS OCCUR AS SHOWN IN THE CONSTRUCTION DRAWINGS.

- E. CHECK ALL STEEL DECK PER SDI SPECIFICATIONS AND AS FOLLOWS:
1. DECK TYPE, SIZE, GAUGE, FINISH, ACCESSORIES, AND REINFORCEMENT AROUND OPENINGS.
 2. SPACING AND TYPE OF ALL CONNECTIONS.
 3. WELDING PROCEDURES.
 4. WELD SIZE, SHAPE, QUALITY, RUST PROTECTIONS, AND WELD WASHERS, WHERE APPLICABLE.
 5. SCREW SIZE, TYPE, AND FINISH AND OTHER CONNECTIONS TO SUPPORTS.

- 2.8 COLD FORMED STRUCTURAL STEEL
- A. INSPECT ALL COLD FORMED STRUCTURAL STEEL MEMBERS AND FRAMING AS REQUIRED BY THE CONTRACT DOCUMENTS. USE BOTH THE CONTRACT DOCUMENTS AND THE SHOP DRAWINGS FOR ALL INSPECTIONS.
- B. INSPECT COLD FORMED STRUCTURAL STEEL MEMBERS FOR:
1. POSSIBLE DAMAGE DURING SHIPPING.
 2. SIZE, TYPE, AND GRADE PRIOR TO ERECTION.
 3. STRAIGHTNESS, FINISH, AND CAMBER.
 4. CONNECTION MATERIAL, SIZES, AND CONFIGURATIONS ARE PER THE CONSTRUCTION DOCUMENTS OR SHOP DRAWINGS, AS WELL AS JOINT PREPARATION.
 5. PROPER ERECTION MARKINGS.
 6. PROPER PLACEMENT OF MEMBERS.

PART 3 - TESTING AND SUBMITTALS

3.1 MATERIAL TESTING

- A. THE TESTING AGENCY RETAINED BY THE OWNER SHALL PERFORM ALL TESTS DESCRIBED IN THE CONTRACT DOCUMENTS AND SHALL PROVIDE A REPORT OF EACH TEST OF WORK DESCRIBED IN THIS PLAN TO THE OWNER, CONTRACTOR, ARCHITECT/STRUCTURAL ENGINEER AND SPECIAL INSPECTOR.
- B. THE SPECIAL INSPECTOR SHALL REVIEW ALL MATERIAL TEST REPORTS UPON RECEIPT AND SHALL INCLUDE IN HIS REPORT ANY RESULTS THAT DO NOT COMPLY WITH THE CONTRACT DOCUMENTS.
- C. THE SPECIAL INSPECTOR MAY REQUEST THE OWNER'S REPRESENTATIVE TO AUTHORIZE ADDITIONAL TESTS WHEN APPROPRIATE.

3.2 STRUCTURAL SUBMITTALS

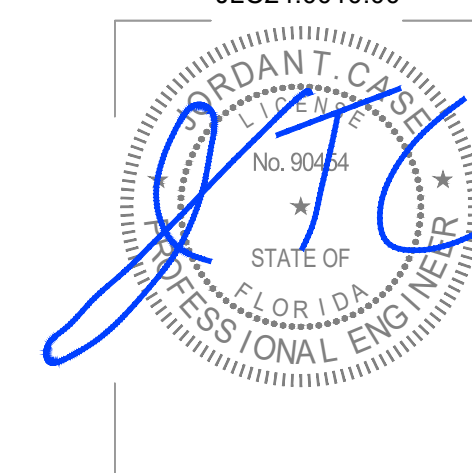
- A. THE SPECIAL INSPECTOR SHALL REVIEW AND BECOME FAMILIAR WITH ALL SUBMITTALS. THE CONTRACTOR SHALL PROVIDE THE SPECIAL INSPECTOR WITH ONE COPY OF ALL STRUCTURAL SUBMITTALS, EXCLUDING CALCULATIONS, AND ALL ACCEPTED ALTERNATES FOR HIS RECORD AND USE. ALL SHOP DRAWINGS AND OTHER SUBMITTALS REFERENCED IN THIS PLAN SHALL CONTAIN EVIDENCE OF REVIEW AND ACCEPTANCE BY THE CONTRACTOR, ARCHITECT AND STRUCTURAL ENGINEER.



Construction Manager



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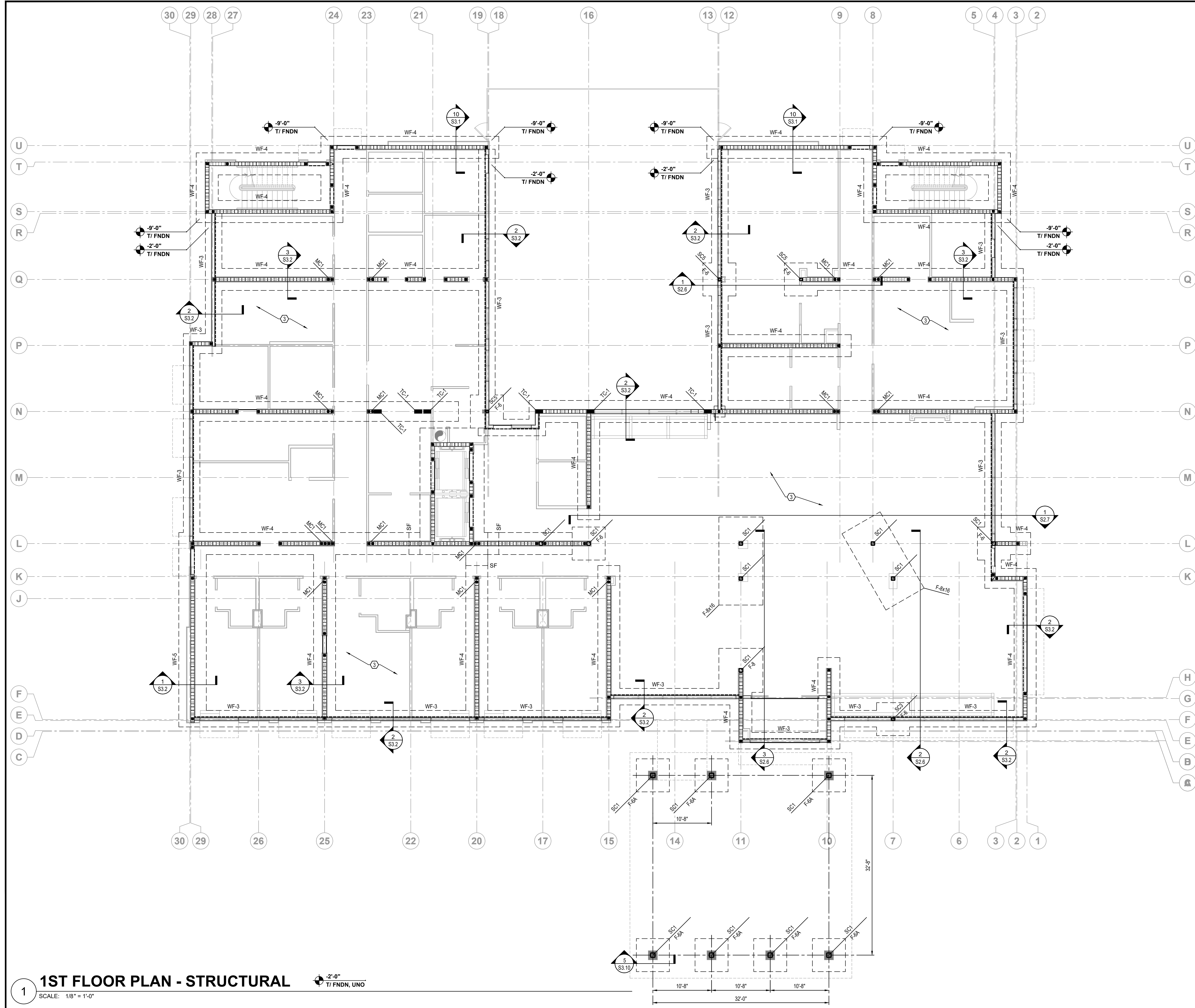
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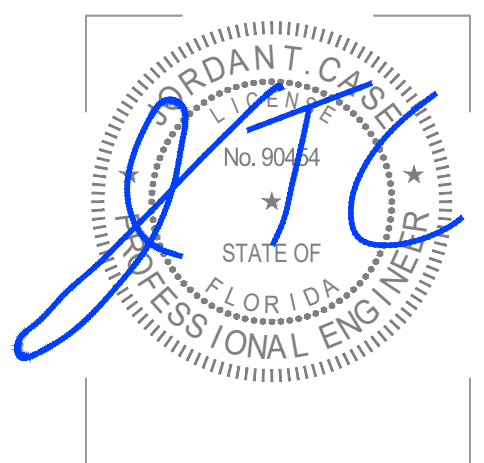
FOUNDATION PLAN NOTES

- 1 SEE SHEETS S0.1 AND S0.2 FOR GENERAL STRUCTURAL NOTES.
- 2 DO NOT SCALE DRAWINGS - SEE THE ARCHITECTURAL DRAWINGS FOR ADDITIONAL DIMENSIONS NOT SHOWN. VERIFY ALL DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION. IF A DISCREPANCY SHOULD OCCUR, CONTACT THE ARCHITECT IN WRITING FOR CLARIFICATION BEFORE PROCEEDING.
- 3 TYPICAL SLAB ON GRADE CONSTRUCTION:
4" MINIMUM THICK CONCRETE SLAB REINFORCED WITH ONE LAYER 6x6 - W2.9xW2.9 WWR OR #4 AT 16" OC, MAX EACH WAY OVER CONTINUOUS VAPOR BARRIER ON COMPACTED SUBGRADE. SEE THE ARCHITECTURAL DRAWINGS FOR VAPOR BARRIER REQUIREMENTS. COORDINATE ALL SLAB ELEVATIONS, DEPRESSIONS, AND SLOPES WITH THE ARCHITECTURAL DRAWINGS. SEE THE ARCHITECTURAL AND/OR CIVIL DRAWINGS FOR ACTUAL TOP OF SLAB ELEVATIONS.
- 4 REFER TO THE ELECTRICAL AND PLUMBING DRAWINGS FOR UNDERGROUND UTILITIES AND SLAB PENETRATIONS.
- 5 PROVIDE CORNER BARS WHERE ALL THICKENED SLAB WALL FOOTINGS CHANGE DIRECTION, AND AT ALL THICKENED SLAB WALL FOOTING INTERSECTIONS. SEE SECTION 1/S3.1 FOR REQUIREMENTS.
- 6 PROVIDE ADDITIONAL RE-ENTRANT CORNER BARS: (2) - #4x5'-0" SPACED AT 3" OC, VERTICALLY, PLACED WITHIN MID-DEPTH OF CONCRETE SLAB THICKNESS AND 2" CLEAR, HORIZONTALLY, FROM RE-ENTRANT CORNERS.
- 7 MASONRY WALLS REINFORCED WITH (1) - #5 VERTICAL (WITH MATCHING DOWEL AT FOOTINGS) IN SOLID GROUT FILLED CELLS AT ALL CORNERS, INTERSECTIONS, ADJACENT MASONRY OPENINGS (COORDINATE SIZE, LOCATIONS, AND ELEVATIONS WITH ARCH. DWGS.) AND BETWEEN AT 24" OC, MAX. TYPICAL SPACING OF REINFORCED CELLS APPLIES ABOVE AND BELOW ALL MASONRY OPENINGS. EXTEND VERTICAL REINFORCING BARS THRU (SPLICE / LAP AS REQUIRED) TO UPPER CONCRETE TIE BEAM OR KNOCK-OUT BLOCK COURSE AND TERMINATE WITH STANDARD 90 DEGREE HOOK.
- 8 THE SLAB AREA BETWEEN CONTROL JOINTS SHALL NOT EXCEED 150 SF, MAX. SEE THE TYPICAL SLAB JOINT DETAIL FOR ADDITIONAL REQUIREMENTS.
- 9 SEE THE ARCHITECTURAL DRAWINGS FOR CONSTRUCTION INFORMATION OF INTERIOR NON-LOAD BEARING WALLS NOT SHOWN.
- 10 STEEL STAIR. SEE THE ARCHITECTURAL DRAWINGS FOR STAIR CONSTRUCTION REQUIREMENTS AND GEOMETRY. COORDINATE ALL STAIR SUPPORT LOCATIONS AND ATTACHMENT WITH THE MFR/SUPPLIER'S APPROVED SHOP DRAWINGS.

LEGEND

- INDICATES 8" CMU WALL REINFORCED IN SOLID GROUT FILLED CELLS WITH MATCHING DOWELS AT THE FOUNDATION. SEE PLAN NOTE 7 FOR REINFORCEMENT SIZE AND SPACING.
- (g) INDICATES SLAB CONTROL JOINT. COORDINATE THE JOINT LAYOUT WITH THE ARCHITECTURAL FLOOR FINISH.
- (TSWF-) INDICATES THICKENED SLAB WALL FOOTING TYPES. SEE PLAN AND "FOOTING SCHEDULE" FOR SIZE AND REINFORCING REQUIREMENTS.
- (MC-) INDICATES MASONRY COLUMN MARK. SEE THE TYPICAL MASONRY COLUMN DETAILS.
- (F-) INDICATES ISOLATED SPREAD FOOTING. SEE THE FOOTING SCHEDULE FOR SIZE AND REINFORCING REQUIREMENTS.
- (WF-) INDICATES WALL FOOTING. SEE THE FOOTING SCHEDULE FOR SIZE AND REINFORCING REQUIREMENTS.
- (C-) INDICATES CONCRETE COLUMN MARK. SEE THE TYPICAL CONCRETE COLUMN DETAILS FOR SIZE AND REINFORCING REQUIREMENTS.
- (TC-) INDICATES CONCRETE TIE COLUMN MARK. SEE THE TYPICAL CONCRETE TIE-COLUMN DETAILS FOR SIZE AND REINFORCING REQUIREMENTS.
- (MC-) INDICATES MASONRY COLUMN MARK. SEE THE TYPICAL MASONRY COLUMN DETAILS FOR SIZE AND REINFORCING REQUIREMENTS.
- INDICATES STEEL COLUMN. SEE THE COLUMN SCHEDULE FOR SIZE.

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Construction Manager

1 **1ST FLOOR PLAN - STRUCTURAL**
 SCALE: 1/8" = 1'-0"
 -9'-0" T/FNDN
 -2'-0" T/FNDN

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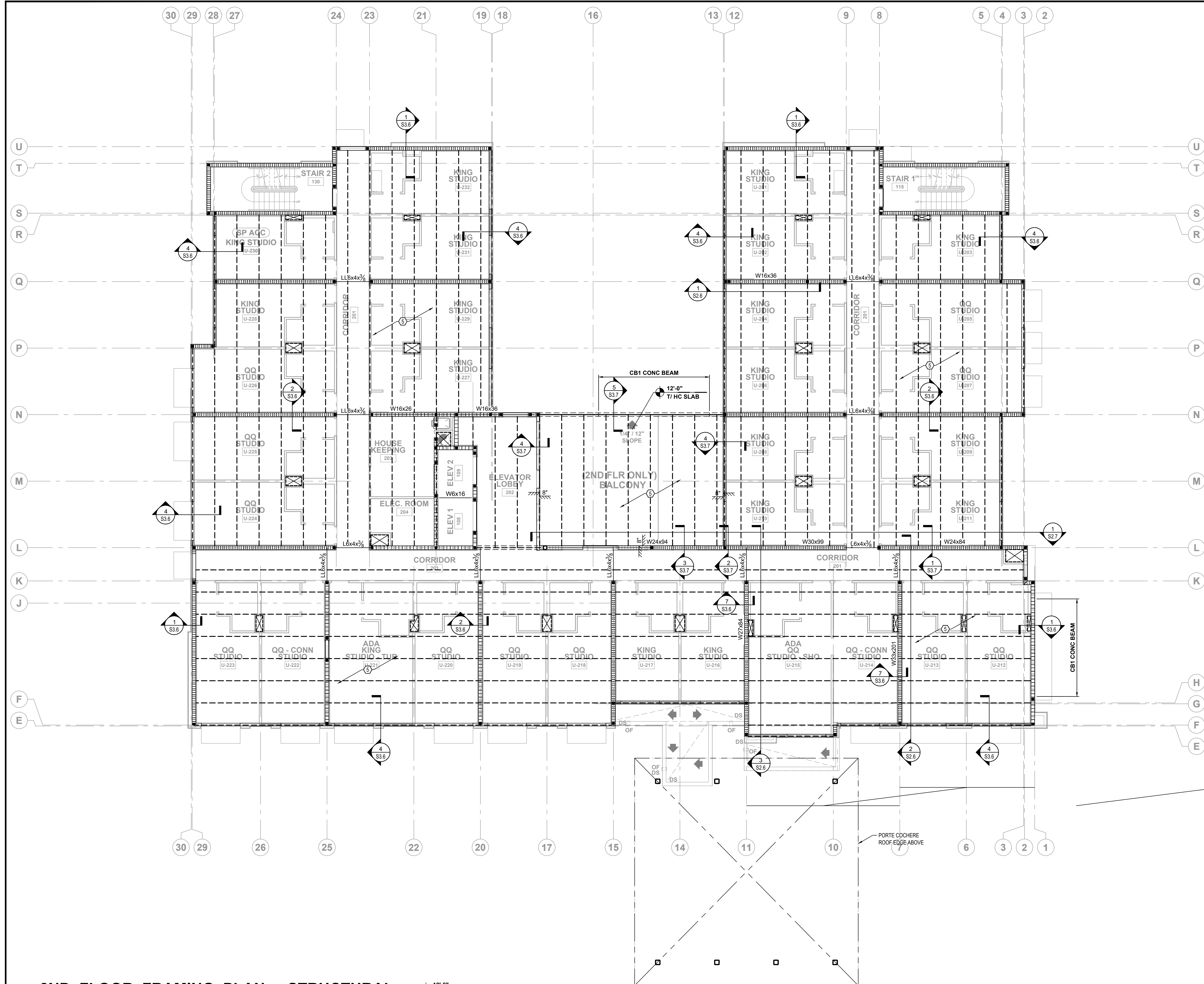
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 SHEET

S2.1



FLOOR FRAMING PLAN NOTES

- 1 SEE SHEETS S0.1 AND S0.2 FOR GENERAL STRUCTURAL NOTES.
- 2 DO NOT SCALE DRAWINGS. SEE ARCH DWGS FOR ADDITIONAL DIMENSIONS NOT SHOWN. VERIFY ALL DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION. IF A DISCREPANCY SHOULD OCCUR, CONTACT THE ARCHITECT IN WRITING FOR CLARIFICATION BEFORE PROCEEDING.
- 3 HOLLOW CORE PLANKS AND PRECAST PLANKS WILL BEAR ON MASONRY BOND BEAMS AND CONCRETE BEAMS. UNO. PROVIDE (BB-1) UNO AT MASONRY BOND BEAMS AND AT CONCRETE BEAMS SEE PLAN FOR ADDITIONAL LABELS. SEE CONCRETE BEAM SCHEDULE FOR TYPICAL SIZE AND REINFORCING FOR BOTH THE MASONRY BOND BEAM AND CONCRETE BEAM. SEE ELEVATION LEGEND FOR TYPICAL TOP OF BEAM AND CONCRETE BEAM ELEVATIONS. UNO.
- 4 PROVIDE CORNER BARS WHERE ALL CONCRETE BEAMS AND MASONRY BOND BEAMS CHANGE DIRECTION AND AT ALL CONCRETE BEAMS AND TIE BEAM INTERSECTIONS. SEE TYPICAL REINFORCING SECTIONS FOR ADDITIONAL INFORMATION.
- 5 TYPICAL ELEVATED FLOOR SLAB CONSTRUCTION: 8" THICK PRECAST HOLLOW CORE CONCRETE PLANKS WITH 2" (MIN) THICK NORMAL WEIGHT CONCRETE TOPPING SLAB REINFORCED W/ POLYPROPYLENE FIBERS. COORDINATE ALL TOP OF SLAB ELEVATIONS, SLOPES, DEPRESSIONS, AND RECESSES WITH ARCH'L DWGS (FOR ACTUAL TOP OF SLAB ELEVATIONS, SEE ARCH'L DWGS). VERIFY FLOOR FINISHES WITH ARCH'L DRAWINGS. COORDINATE ALL SLAB PENETRATION LOCATIONS WITH ARCH'L AND MEP DWGS.
- 6 TYPICAL ELEVATED BALCONY SLAB CONSTRUCTION: 6" THICK PRECAST CONCRETE PLANKS WITH 2" (MIN) THICK NORMAL WEIGHT CONCRETE TOPPING SLAB REINFORCED W/ POLYPROPYLENE FIBERS. COORDINATE ALL TOP OF SLAB ELEVATIONS, SLOPES, DEPRESSIONS, AND RECESSES WITH ARCH'L DWGS.
- 7 SEE ARCH'L DWGS FOR CONSTRUCTION INFORMATION OF INTERIOR NON-LOAD BEARING PARTITION WALLS NOT SHOWN.
- 8 TYPICAL STAIR / LANDING CONSTRUCTION: SEE ARCH'L DWGS FOR STAIR INFORMATION AND GEOMETRY. COORDINATE SUPPORT LOCATIONS AND ATTACHMENT W/ MANUFACTURER / SUPPLIERS APPROVED SHOP DRAWINGS. STAIR LANDING CONSTRUCTION WILL BE 8" THICK PRECAST HOLLOW CORE CONCRETE PLANKS WITH 2" MIN THICK NORMAL WEIGHT CONCRETE TOPPING SLAB REINFORCED W/ POLYPROPYLENE FIBERS.
- 9 PROVIDE MASONRY LINTELS OVER ALL OPENINGS IN MASONRY WALLS, UNO ON PLAN (COORDINATE EXACT SIZE, LOCATION, AND ELEVATION WITH ARCH'L DRAWINGS). SEE TYPICAL MASONRY OPENING SCHEDULE ON SHEET S3.2 FOR LINTEL TYPE SIZES AND REINFORCING.
- 10 MASONRY WALLS REINFORCED WITH (1) - #5 VERTICAL IN SOLID GROUT FILLED CELLS AT ALL CORNERS, INTERSECTIONS, ADJACENT MASONRY OPENINGS (COORDINATE SIZE, LOCATIONS, AND ELEVATIONS WITH ARCH'L DWGS.) AND BETWEEN AT 32" O.C. (MAX). TYPICAL SPACING OF REINFORCED CELLS APPLIES ABOVE AND BELOW ALL MASONRY OPENINGS. EXTEND VERTICAL REINFORCING BARS THRU (SPlice / LAP AS REQUIRED) TO UPPER CONCRETE TIE BEAM OR KNOCK-OUT BLOCK COURSE AND TERMINATE WITH STANDARD 90 DEGREE HOOK.

LEGEND

- INDICATES 8" CMU WALL REINFORCED IN SOLID GROUTED FILLED CELLS WITH STD HOOKS AT FOUNDATION LEVEL. SEE PLAN NOTE 10 FOR REINFORCEMENT SIZE AND SPACING.
- (BB-) INDICATES MASONRY BOND BEAM. SEE CONCRETE BEAM SCHEDULE.
- (CB-) INDICATES CONCRETE BEAM. SEE CONCRETE BEAM SCHEDULE.
- (TB-) INDICATES CONCRETE TIE BEAM. SEE CONCRETE BEAM SCHEDULE.
- (CC-) INDICATES CONCRETE COLUMN MARK. SEE TYPICAL CONCRETE COLUMN DETAILS.
- (TC-) INDICATES CONCRETE TIE COLUMN MARK. SEE TYPICAL CONCRETE TIE COLUMN DETAILS.
- (MC-) INDICATES MASONRY COLUMN MARK. SEE TYPICAL MASONRY COLUMN DETAILS.
- INDICATES STEEL COLUMN. SEE STEEL COLUMN SCHEDULE FOR SIZE.
- INDICATES STEEL BEAM. SEE PLAN FOR SIZE AND SUPPORT.
- INDICATES HOLLOW-CORE SLAB JOINTS SHOWN FOR ILLUSTRATIVE PURPOSES ONLY. THE FINAL JOINT LOCATIONS SHALL BE DETERMINED BY THE HOLLOW-CORE MANUFACTURER.
- INDICATES OPENINGS REQUIRED IN HOLLOW-CORE SLAB. MANUFACTURE TO DETERMINE IF SECONDARY HANGERS ARE REQUIRED.

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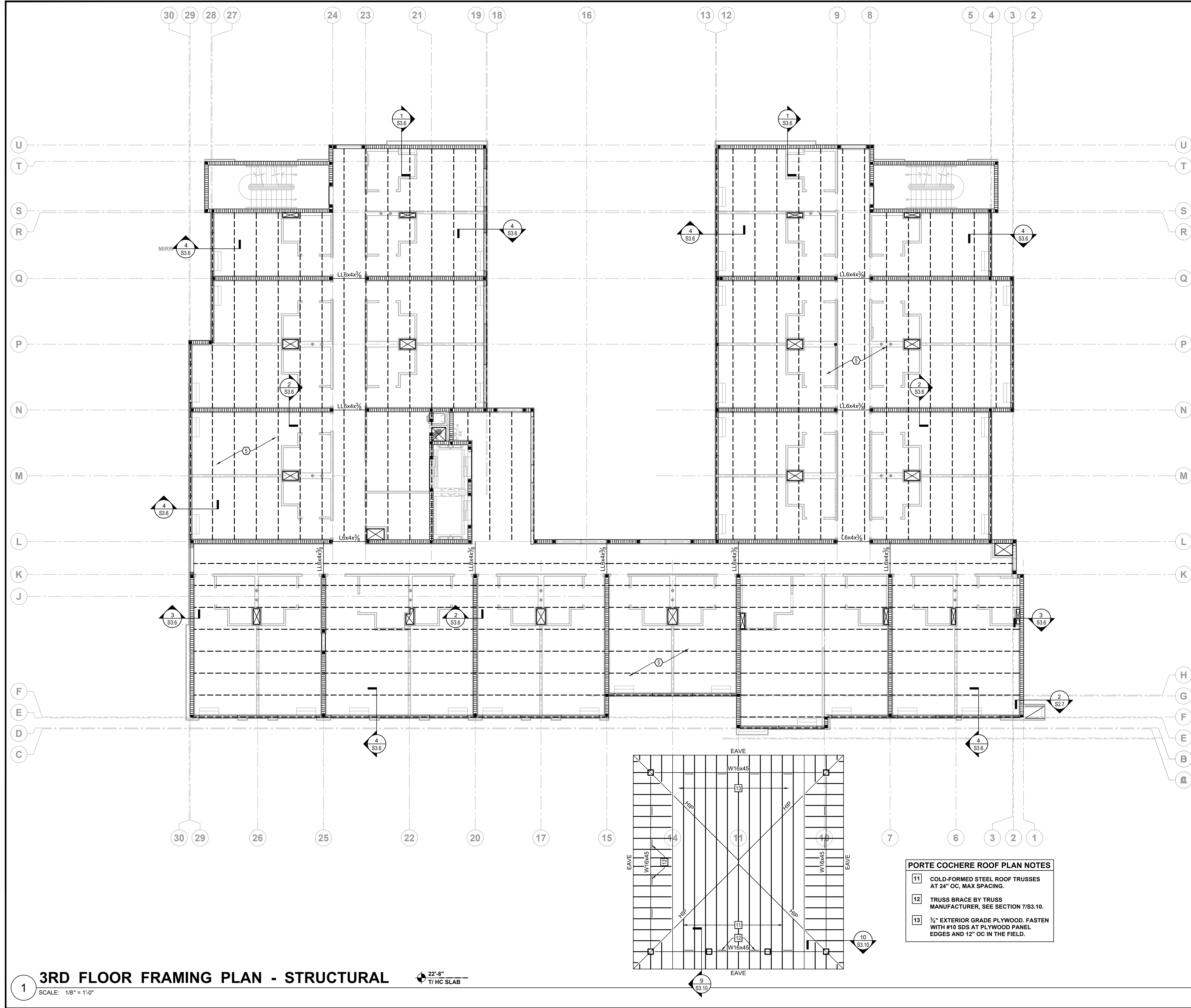
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1 2ND FLOOR FRAMING PLAN - STRUCTURAL
SCALE: * = 1"=0"
12'-8" T/HC SLAB, UNO



FLOOR FRAMING PLAN NOTES

- 1 SEE SHEETS S0.1 AND S0.2 FOR GENERAL STRUCTURAL NOTES.
- 2 DO NOT SCALE DRAWINGS. SEE ARCH DWGS FOR ADDITIONAL DIMENSIONS NOT SHOWN. VERIFY ALL DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION. IF A DISCREPANCY SHOULD OCCUR, CONTACT THE ARCHITECT IN WRITING FOR CLARIFICATION BEFORE PROCEEDING.
- 3 HOLLOW CORE PLANKS AND PRECAST PLANKS WILL BEAR ON MASONRY BOND BEAMS AND CONCRETE BEAMS. UNO. PROVIDE (BB-1) UNO AT MASONRY BOND BEAMS AND AT CONCRETE BEAMS SEE PLAN FOR ADDITIONAL LABELS. SEE CONCRETE BEAM SCHEDULE FOR TYPICAL SIZE AND REINFORCING FOR BOTH THE MASONRY BOND BEAM AND CONCRETE BEAM. SEE ELEVATION LEGEND FOR TYPICAL TOP OF BEAM AND CONCRETE BEAM ELEVATIONS. UNO.
- 4 PROVIDE CORNER BARS WHERE ALL CONCRETE BEAMS AND MASONRY BOND BEAMS CHANGE DIRECTION AND AT ALL CONCRETE BEAMS AND TIE BEAM INTERSECTIONS. SEE TYPICAL REINFORCING SECTIONS FOR ADDITIONAL INFORMATION.
- 5 TYPICAL ELEVATED FLOOR SLAB CONSTRUCTION: 8" THICK PRECAST HOLLOW CORE CONCRETE PLANKS WITH 2" (MIN) THICK NORMAL WEIGHT CONCRETE TOPPING SLAB REINFORCED W/ POLYPROPYLENE FIBERS. COORDINATE ALL TOP OF SLAB ELEVATIONS, SLOPES, DEPRESSIONS, AND RECESSES WITH ARCH'L DWGS (FOR ACTUAL TOP OF SLAB ELEVATIONS, SEE ARCH'L DWGS). VERIFY FLOOR FINISHES WITH ARCH'L DRAWINGS. COORDINATE ALL SLAB PENETRATION LOCATIONS WITH ARCH'L AND MEP DWGS.
- 6 TYPICAL ELEVATED BALCONY SLAB CONSTRUCTION: 6" THICK PRECAST CONCRETE PLANKS WITH 2" (MIN) THICK NORMAL WEIGHT CONCRETE TOPPING SLAB REINFORCED W/ POLYPROPYLENE FIBERS. COORDINATE ALL TOP OF SLAB ELEVATIONS, SLOPES, DEPRESSIONS, AND RECESSES WITH ARCH'L DWGS.
- 7 SEE ARCH'L DWGS FOR CONSTRUCTION INFORMATION OF INTERIOR NON-LOAD BEARING PARTITION WALLS NOT SHOWN.
- 8 TYPICAL STAIR / LANDING CONSTRUCTION: SEE ARCH'L DWGS FOR STAIR INFORMATION AND GEOMETRY. COORDINATE SUPPORT LOCATIONS AND ATTACHMENT W/ MANUFACTURER / SUPPLIERS APPROVED SHOP DRAWINGS. STAIR LANDING CONSTRUCTION WILL BE 8" THICK PRECAST HOLLOW CORE CONCRETE PLANKS WITH 2" MIN THICK NORMAL WEIGHT CONCRETE TOPPING SLAB REINFORCED W/ POLYPROPYLENE FIBERS.
- 9 PROVIDE MASONRY LINTELS OVER ALL OPENINGS IN MASONRY WALLS, UNO ON PLAN (COORDINATE EXACT SIZE, LOCATION, AND ELEVATION WITH ARCH'L DRAWINGS). SEE TYPICAL MASONRY OPENING SCHEDULE ON SHEET S3.2 FOR LINTEL TYPE SIZES AND REINFORCING.
- 10 MASONRY WALLS REINFORCED WITH (1) - #5 VERTICAL IN SOLID GROUT FILLED CELLS AT ALL CORNERS, INTERSECTIONS, ADJACENT MASONRY OPENINGS (COORDINATE SIZE, LOCATIONS, AND ELEVATIONS WITH ARCH'L DWGS.) AND BETWEEN AT 32" O.C. (MAX). TYPICAL SPACING OF REINFORCED CELLS APPLIES ABOVE AND BELOW ALL MASONRY OPENINGS. EXTEND VERTICAL REINFORCING BARS THRU (SPlice / LAP AS REQUIRED) TO UPPER CONCRETE TIE BEAM OR KNOCK-OUT BLOCK COURSE AND TERMINATE WITH STANDARD 90 DEGREE HOOK.

LEGEND

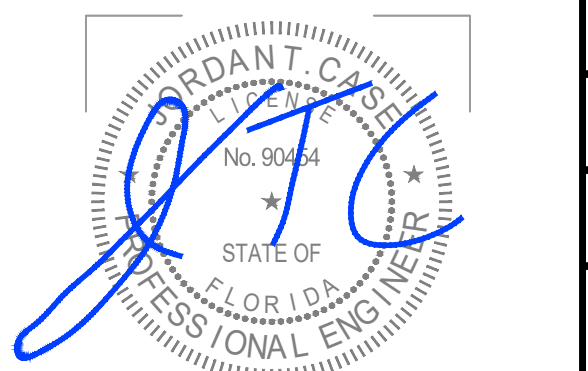
- INDICATES 8" CMU WALL REINFORCED IN SOLID GROUTED FILLED CELLS. PROVIDE MATCHING DOWELS WITH STD HOOKS AT FOUNDATION LEVEL. SEE PLAN NOTE 10 FOR REINFORCEMENT SIZE AND SPACING.
- (BB-) INDICATES MASONRY BOND BEAM. SEE CONCRETE BEAM SCHEDULE.
- (CB-) INDICATES CONCRETE BEAM. SEE CONCRETE BEAM SCHEDULE.
- (TB-) INDICATES CONCRETE TIE BEAM. SEE CONCRETE BEAM SCHEDULE.
- (CC-) INDICATES CONCRETE COLUMN MARK. SEE TYPICAL CONCRETE COLUMN DETAILS.
- (TC-) INDICATES CONCRETE TIE COLUMN MARK. SEE TYPICAL CONCRETE TIE COLUMN DETAILS.
- (MC-) INDICATES MASONRY COLUMN MARK. SEE TYPICAL MASONRY COLUMN DETAILS.
- INDICATES STEEL COLUMN. SEE STEEL COLUMN SCHEDULE FOR SIZE.
- INDICATES STEEL BEAM. SEE PLAN FOR SIZE AND SUPPORT.
- INDICATES HOLLOW-CORE SLAB JOINTS SHOWN FOR ILLUSTRATIVE PURPOSES ONLY. THE FINAL JOINT LOCATIONS SHALL BE DETERMINED BY THE HOLLOW-CORE MANUFACTURER.
- INDICATES OPENINGS REQUIRED IN HOLLOW-CORE SLAB. MANUFACTURE TO DETERMINE IF SECONDARY HANGERS ARE REQUIRED.

PORTE COCHERE ROOF PLAN NOTES

- 11 COLD-FORMED STEEL ROOF TRUSSES AT 24" OC, MAX SPACING.
- 12 TRUSS BRACE BY TRUSS MANUFACTURER, SEE SECTION 7/S3.10.
- 13 3/4" EXTERIOR GRADE PLYWOOD. FASTEN WITH #10 SDS AT PLYWOOD PANEL EDGES AND 12" OC IN THE FIELD.



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Construction Manager

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TRU HOTEL BY HILTON
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 STUART, FLORIDA

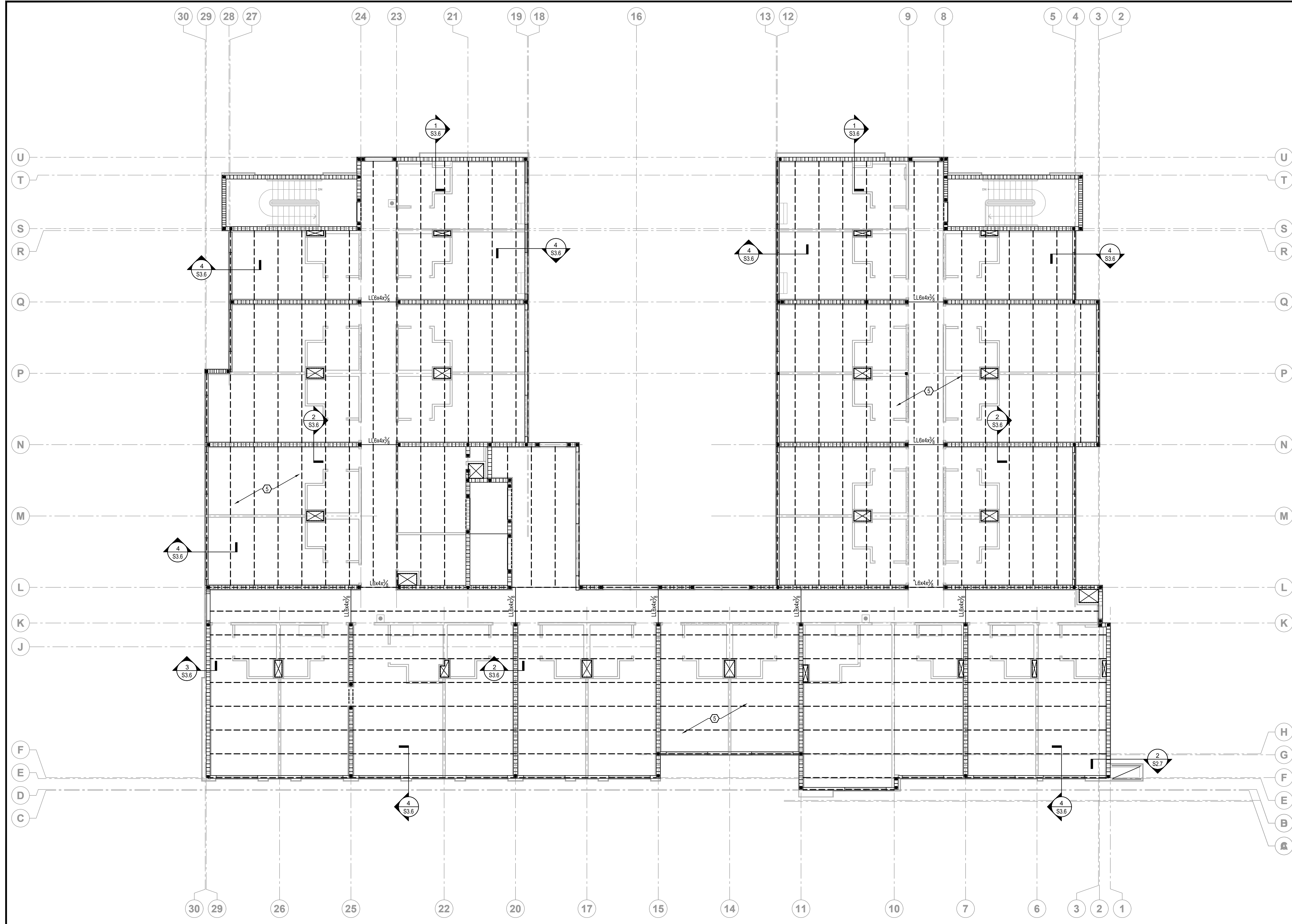
REVISION

JOB NO.
 22039
 DATE
 07-11-2024
 SHEET

S2.3

3RD FLOOR FRAMING PLAN - STRUCTURAL
 SCALE: 1/8" = 1'-0"

22'-8"
 7/ HC SLAB



FLOOR FRAMING PLAN NOTES

- 1 SEE SHEETS S0.1 AND S0.2 FOR GENERAL STRUCTURAL NOTES.
- 2 DO NOT SCALE DRAWINGS. SEE ARCH DWGS FOR ADDITIONAL DIMENSIONS NOT SHOWN. VERIFY ALL DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION. IF A DISCREPANCY SHOULD OCCUR, CONTACT THE ARCHITECT IN WRITING FOR CLARIFICATION BEFORE PROCEEDING.
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LEGEND

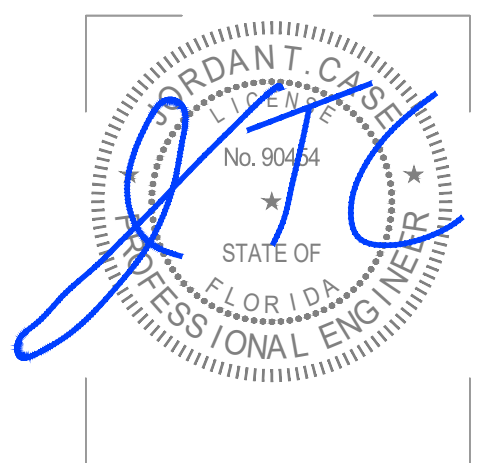
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Construction Manager

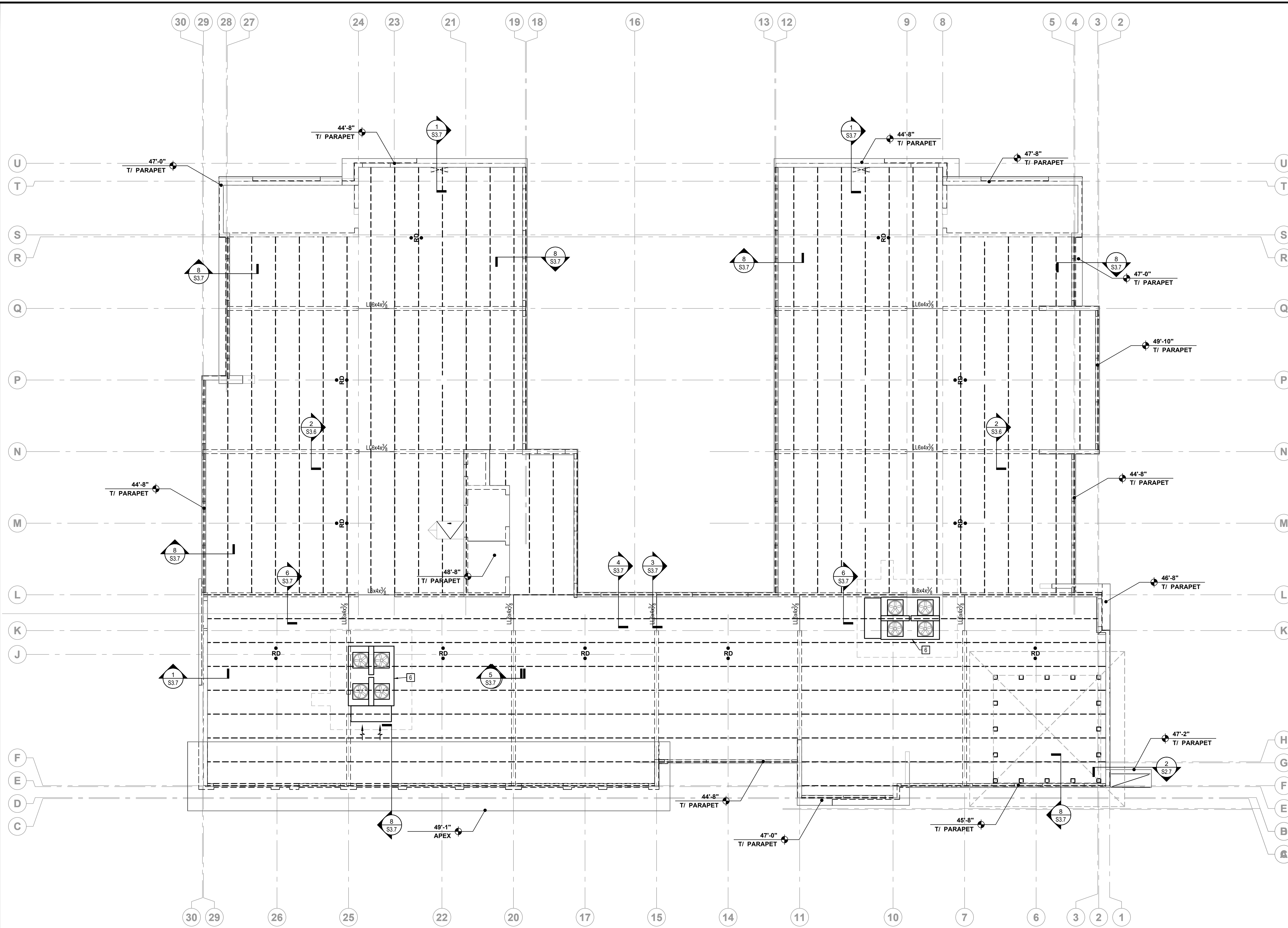
REVISION

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SHEET

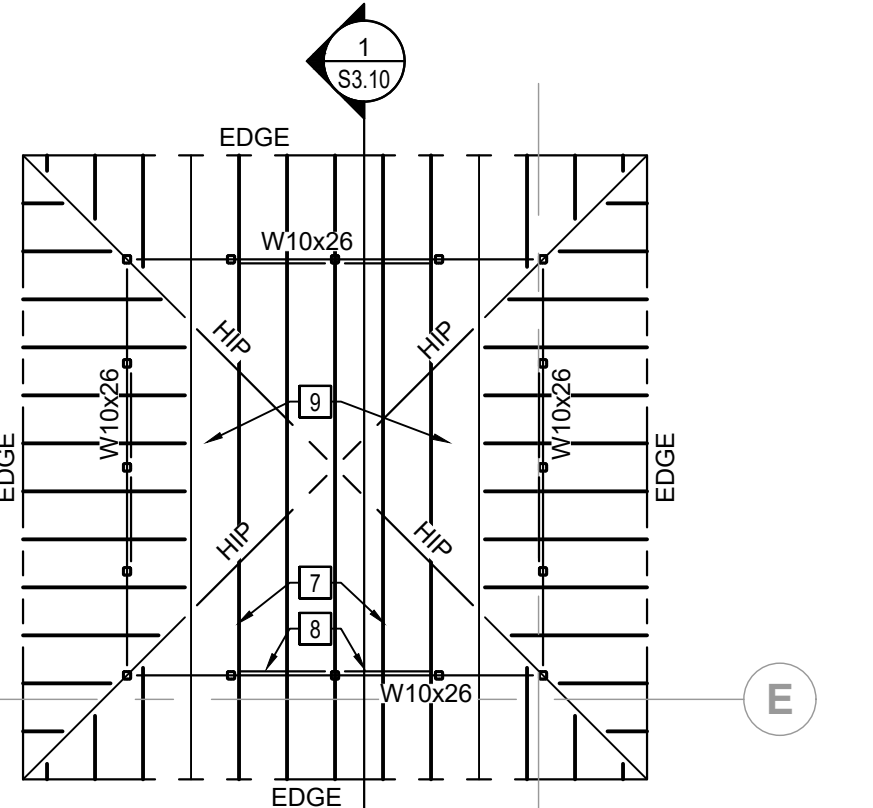
S2.4

4TH FLOOR FRAMING PLAN - STRUCTURAL
SCALE: 1/8" = 1'-0"

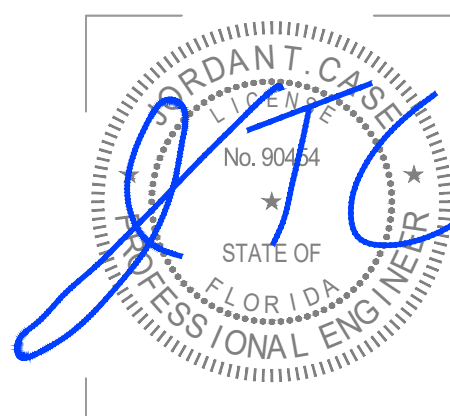
32'-8"
7/ HC SLAB



- ### HIGH ROOF PLAN NOTES
- 7 COLD-FORMED STEEL ROOF TRUSSES AT 24" OC, MAX SPACING.
 - 8 TRUSS BRACE BY TRUSS MANUFACTURER. SEE SECTION 7/S3.10.
 - 9 3/4" EXTERIOR GRADE PLYWOOD. FASTEN WITH #10 SDS AT PLYWOOD PANEL EDGES AND 12" OC IN THE FIELD.



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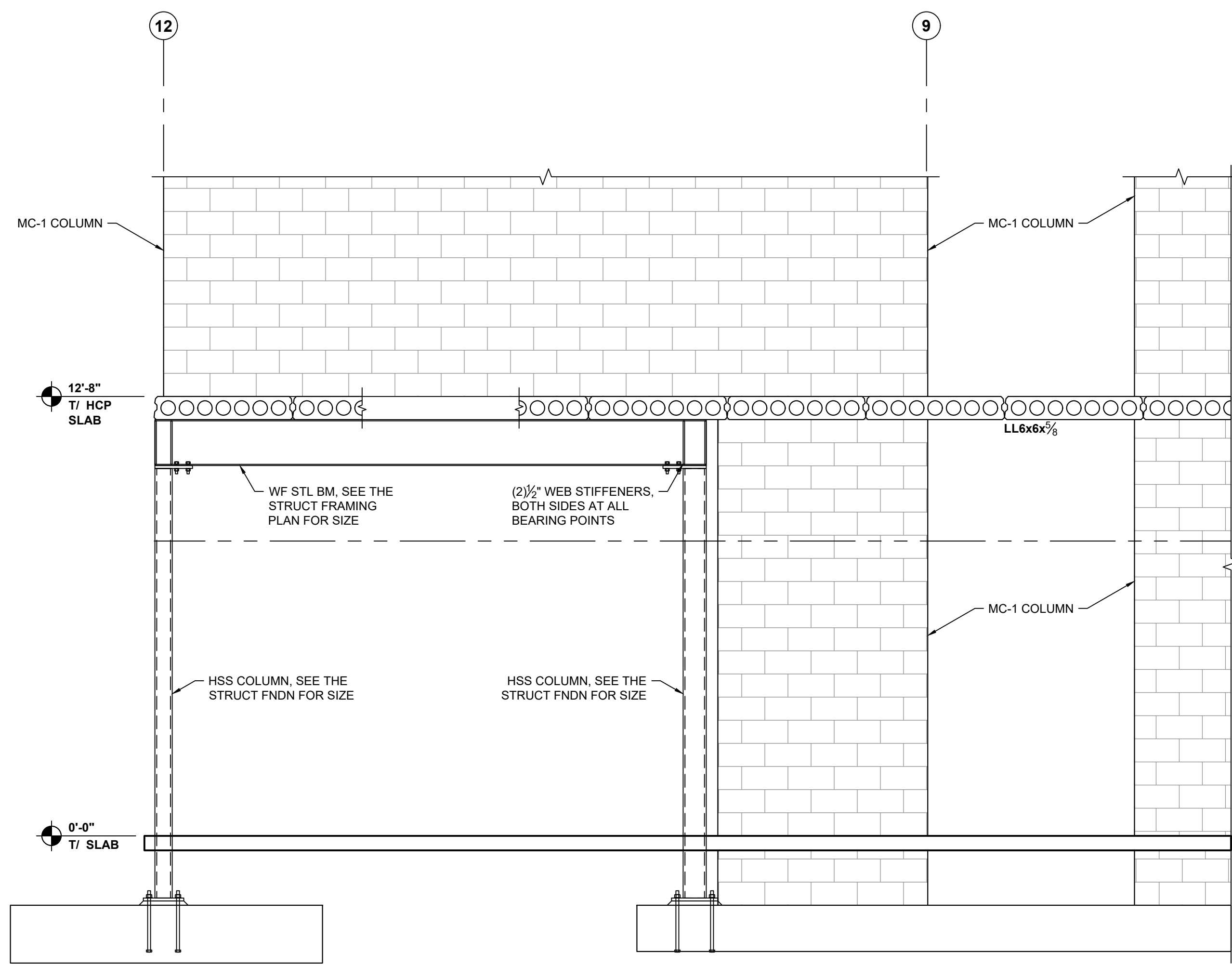


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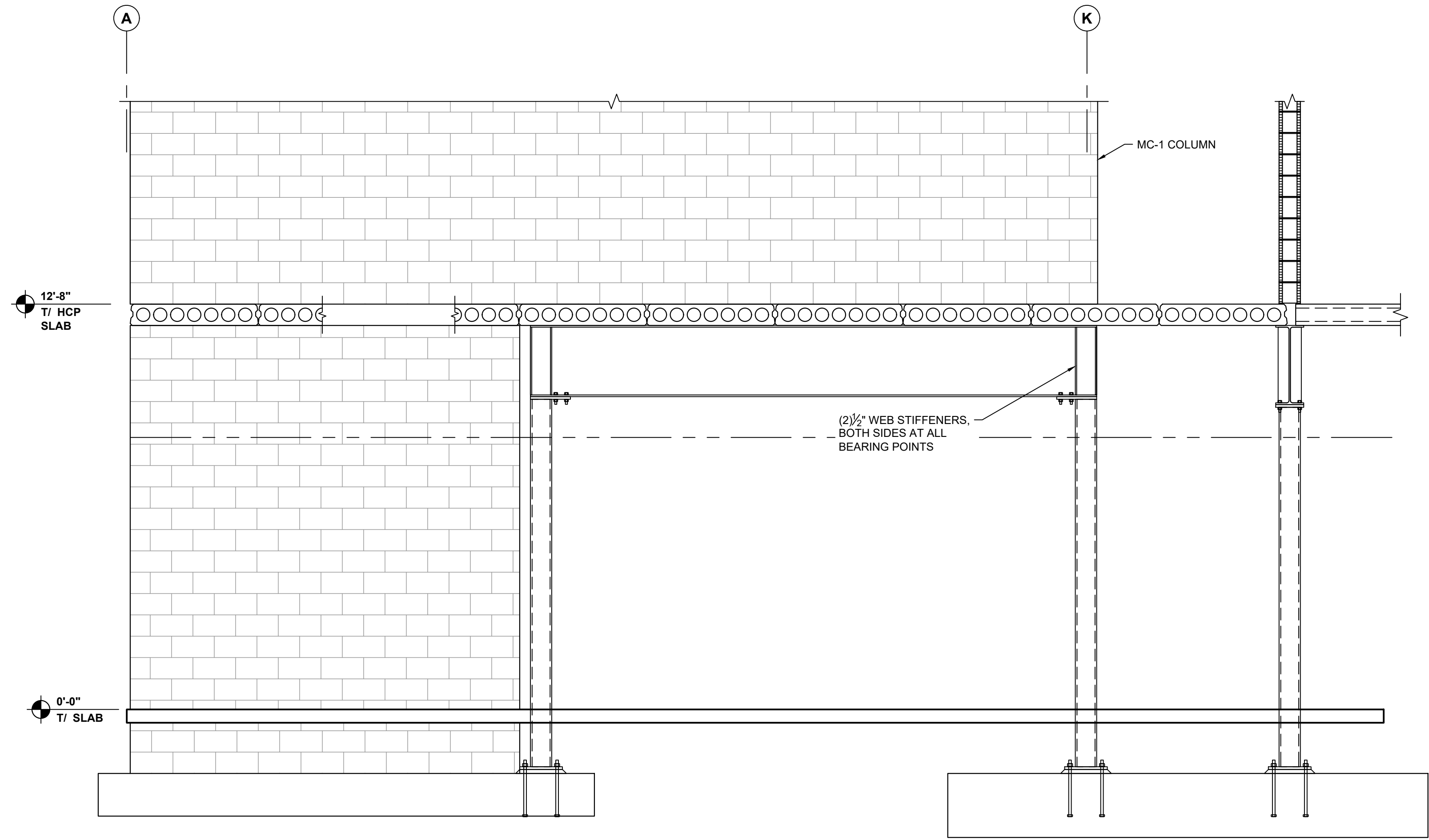
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HARTER - ADAMS P.A.
 ARCHITECTS AND PLANNERS
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 RAJ PATEL
 STUART, FLORIDA

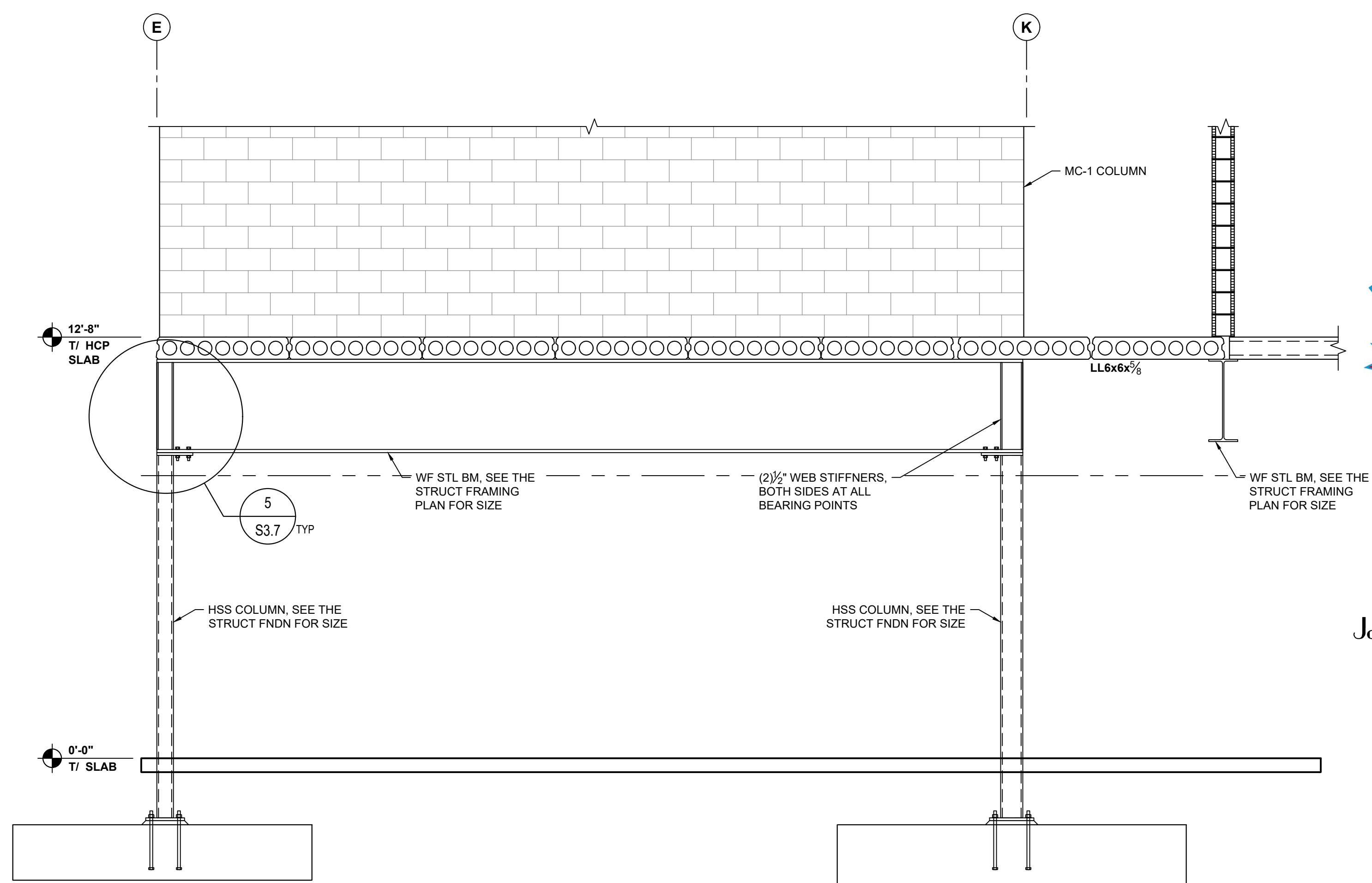
REVISION
JOB NO. 22039
DATE 07-11-2024
SHEET S2.5



1 GRID LINE Q BUILDING SECTION
SCALE: 3/8" = 1'-0"



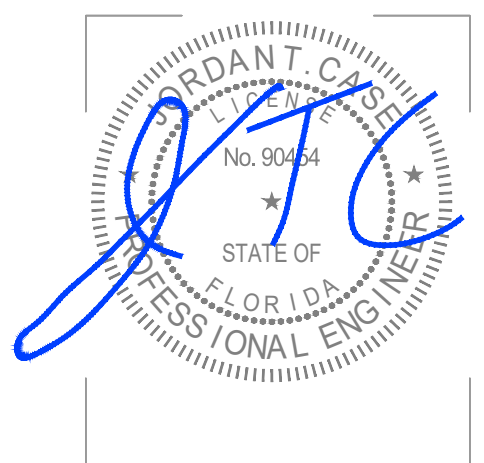
3 GRID LINE 11 BUILDING SECTION
SCALE: 3/8" = 1'-0"



2 GRID LINE 7 BUILDING SECTION
SCALE: 3/8" = 1'-0"



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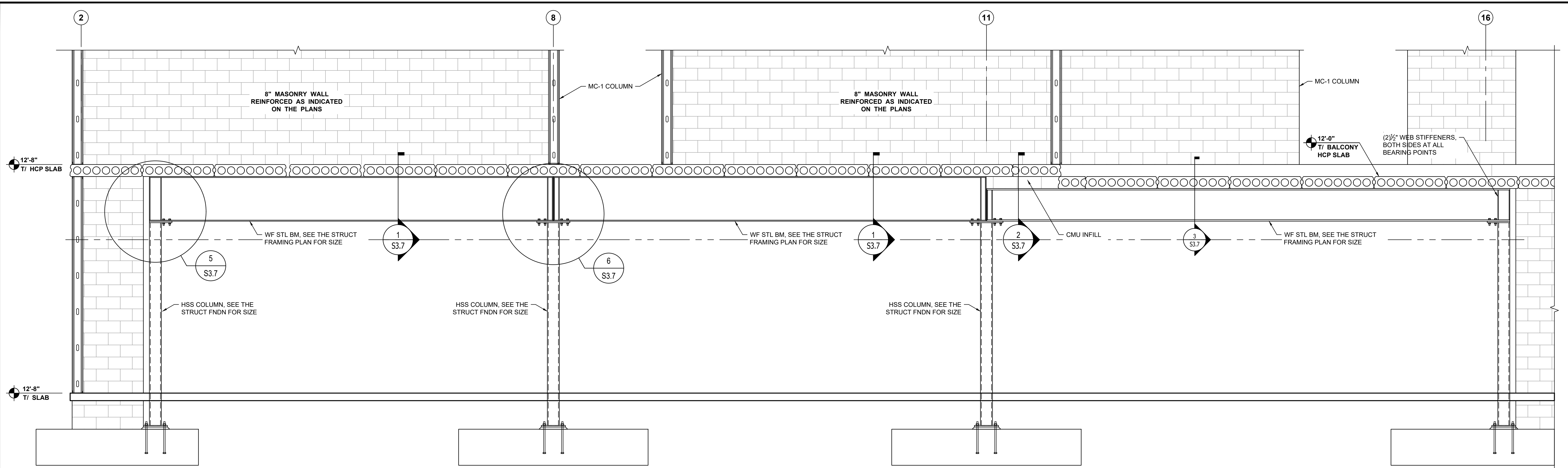


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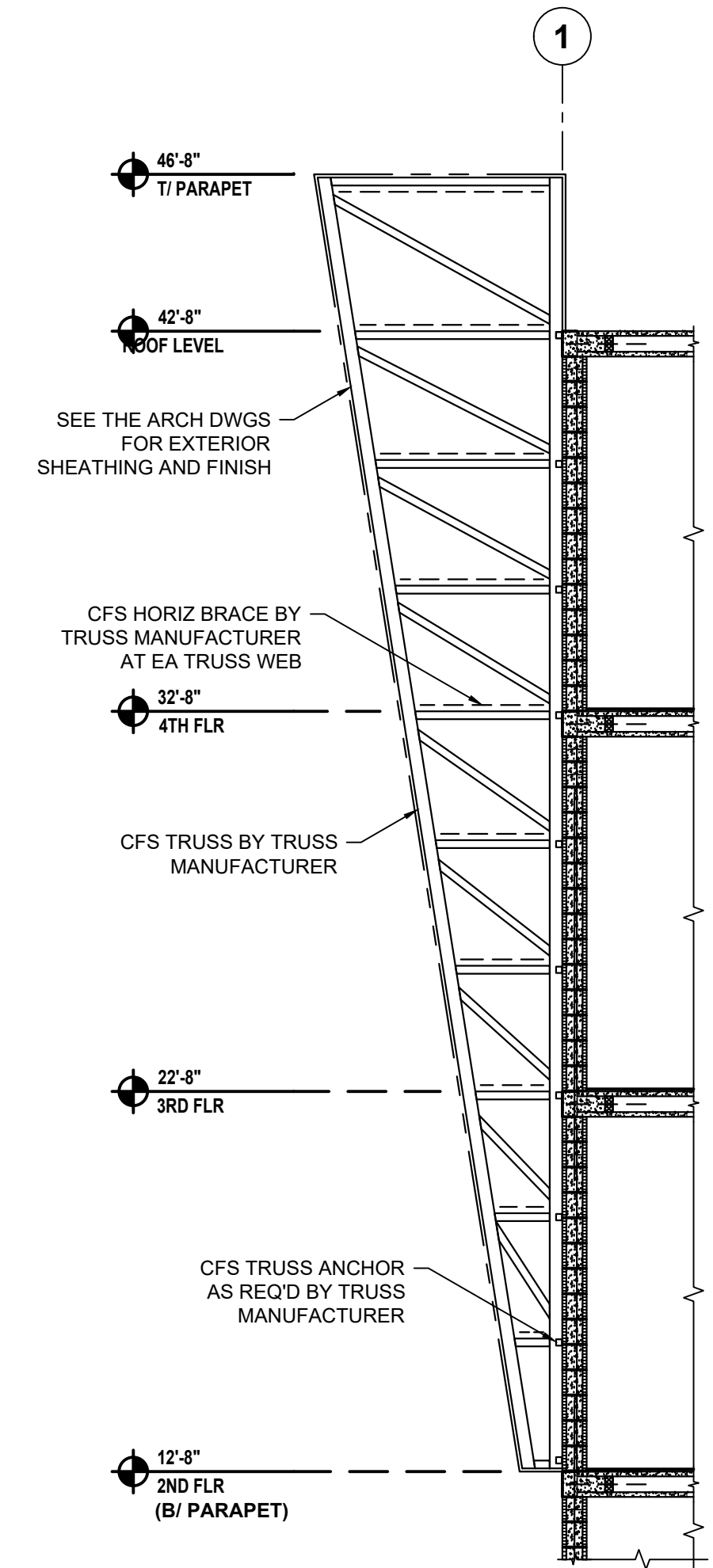
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REVISION
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SHEET S2.6



1 GRID LINE L BUILDING SECTION
SCALE: 3/8" = 1'-0"



2 PARAPET SECTION
SCALE: 1/4" = 1'-0"

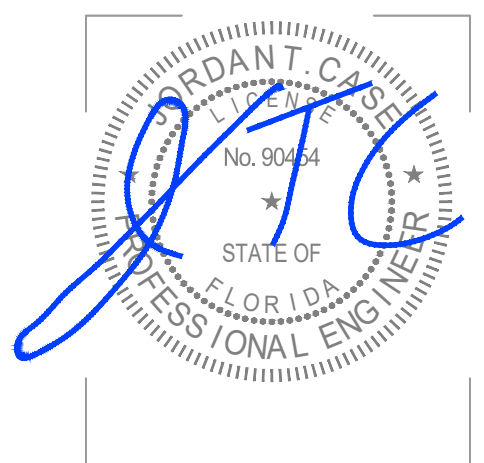
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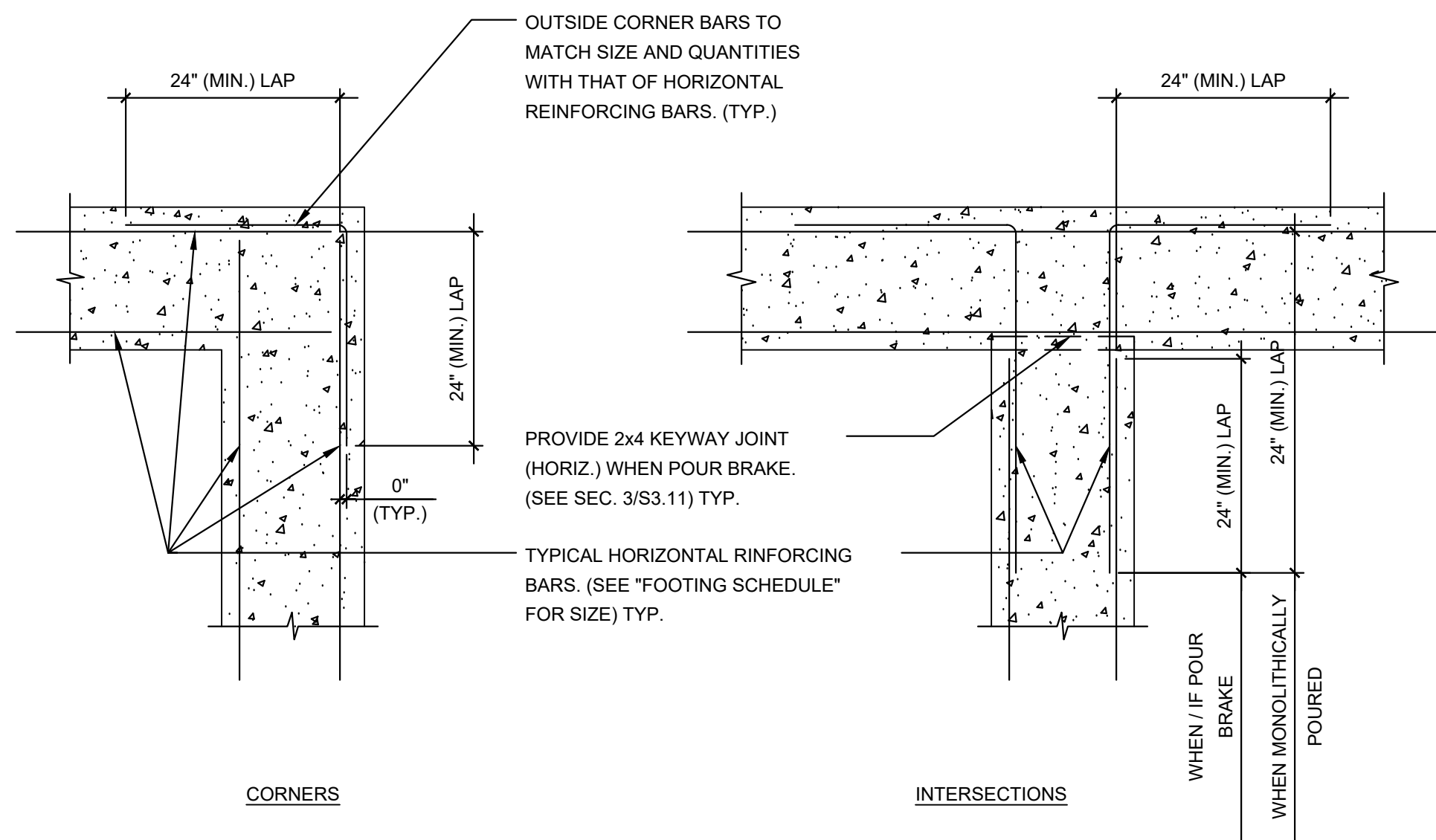
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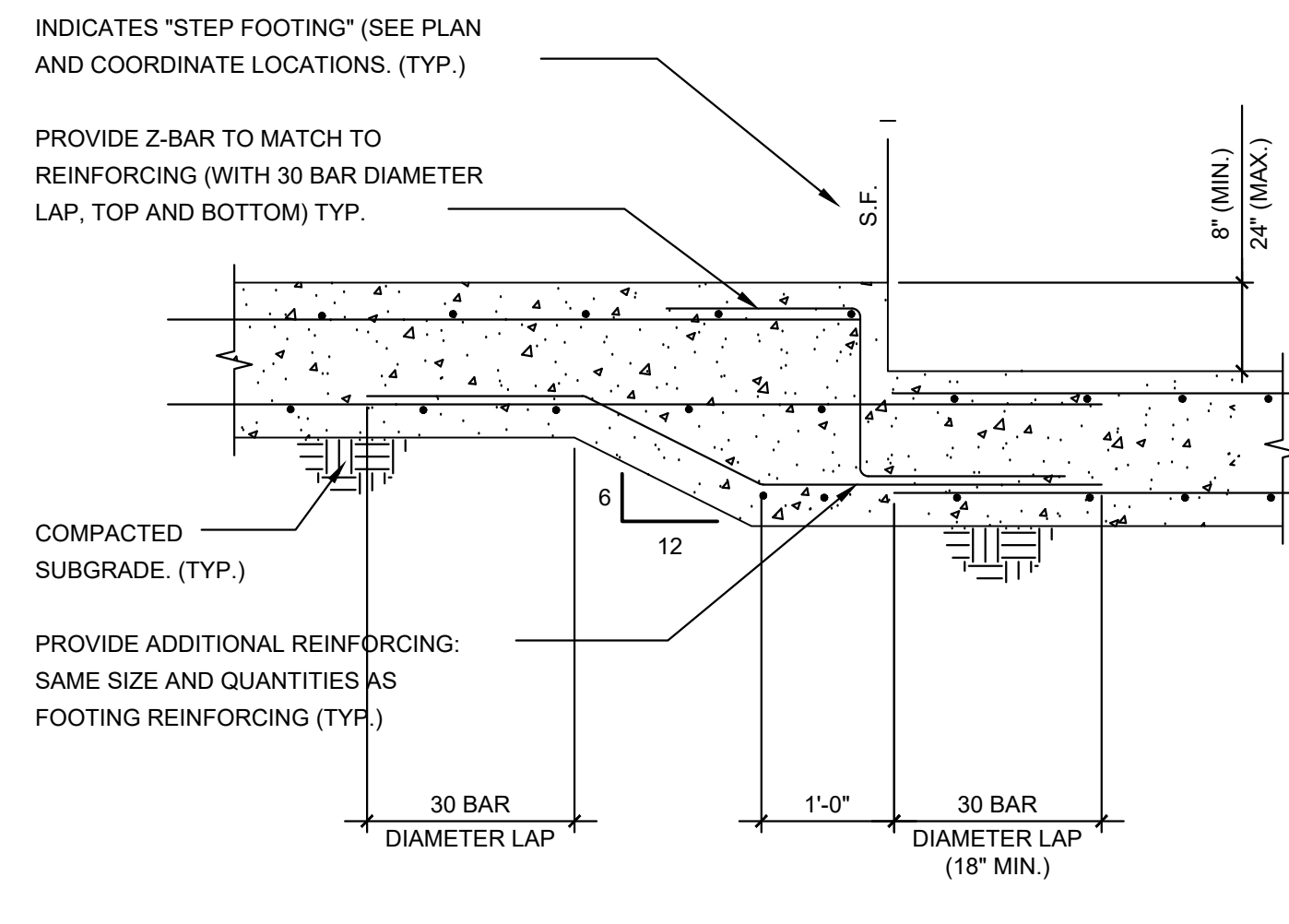
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S2.7



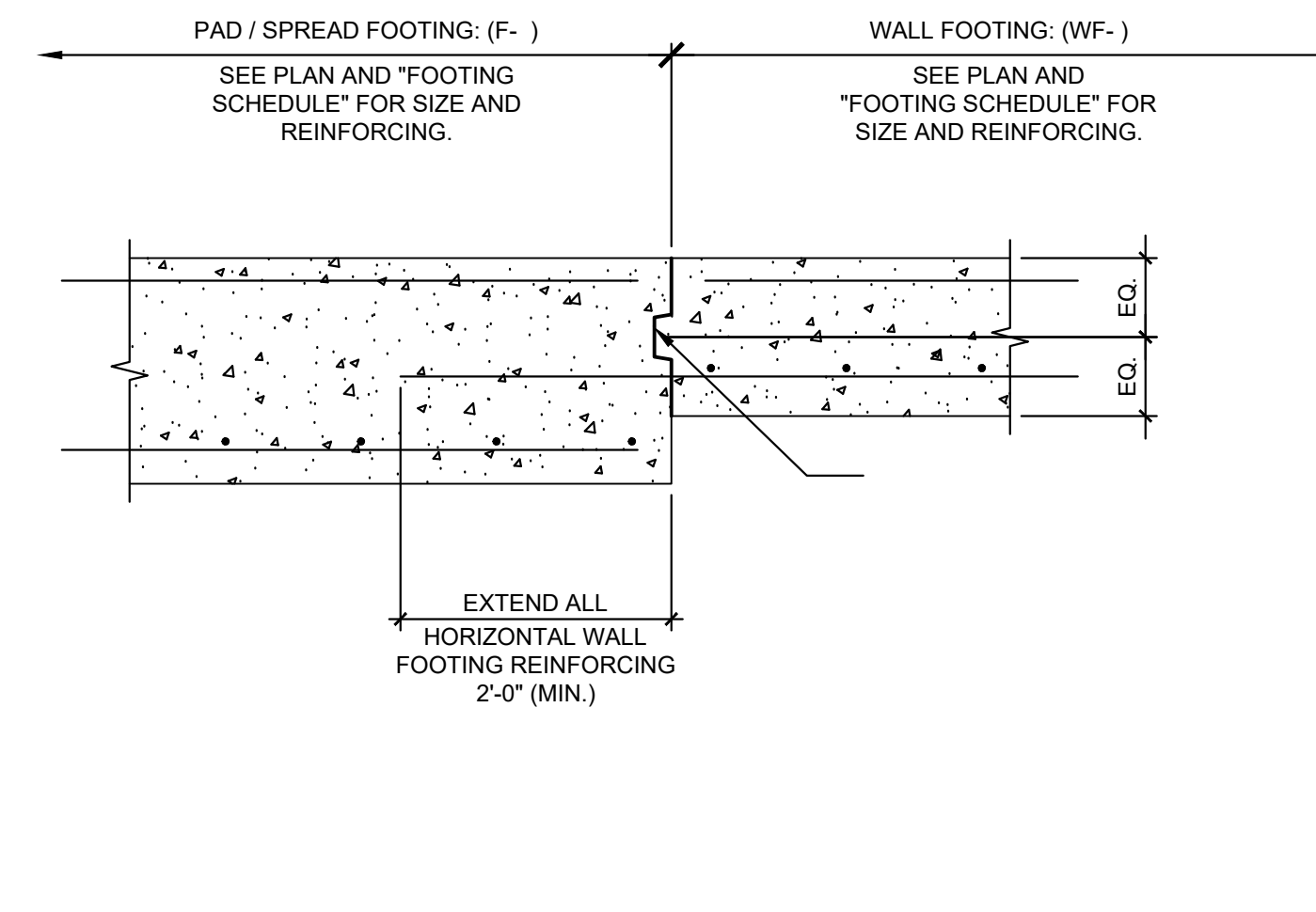
1 HORIZONTAL REINFORCING: FOOTINGS, WALLS, BOND AND CONCRETE BEAMS

SCALE: 3/4" = 1'-0"



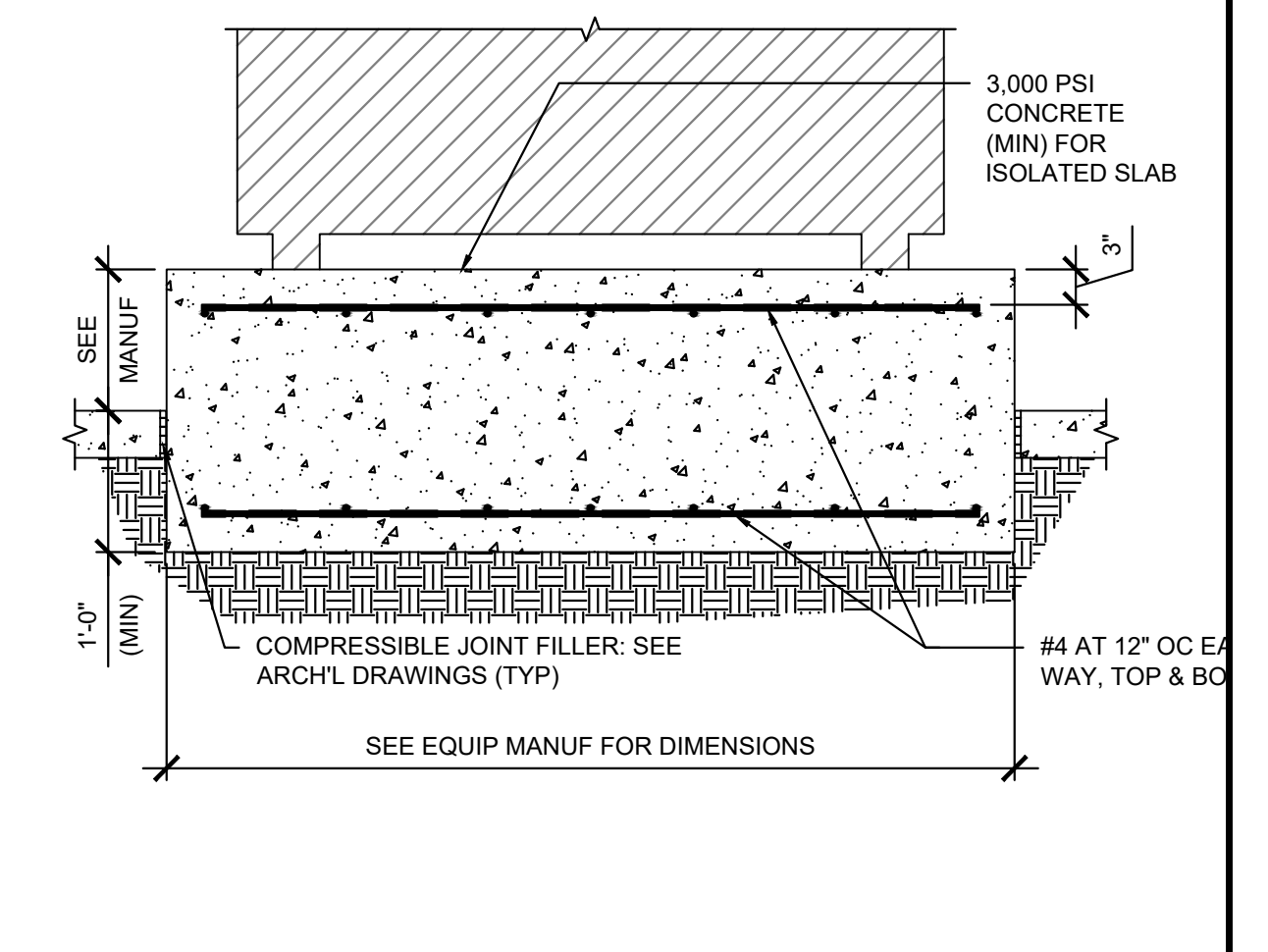
2 TYPICAL STEP FOOTING DETAIL

SCALE: 3/4" = 1'-0"



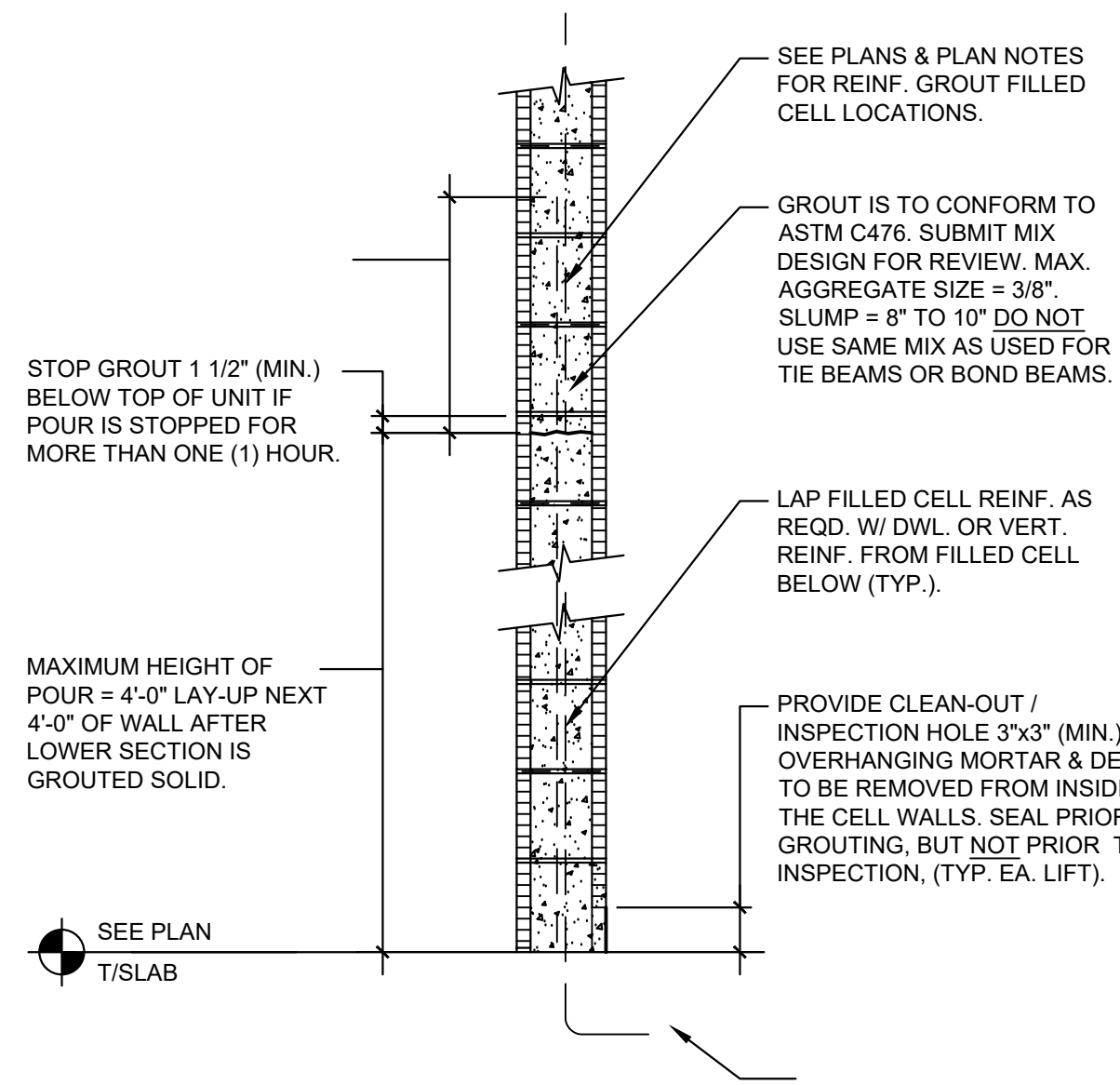
3 TYPICAL FOOTING DETAIL

SCALE: 3/4" = 1'-0"



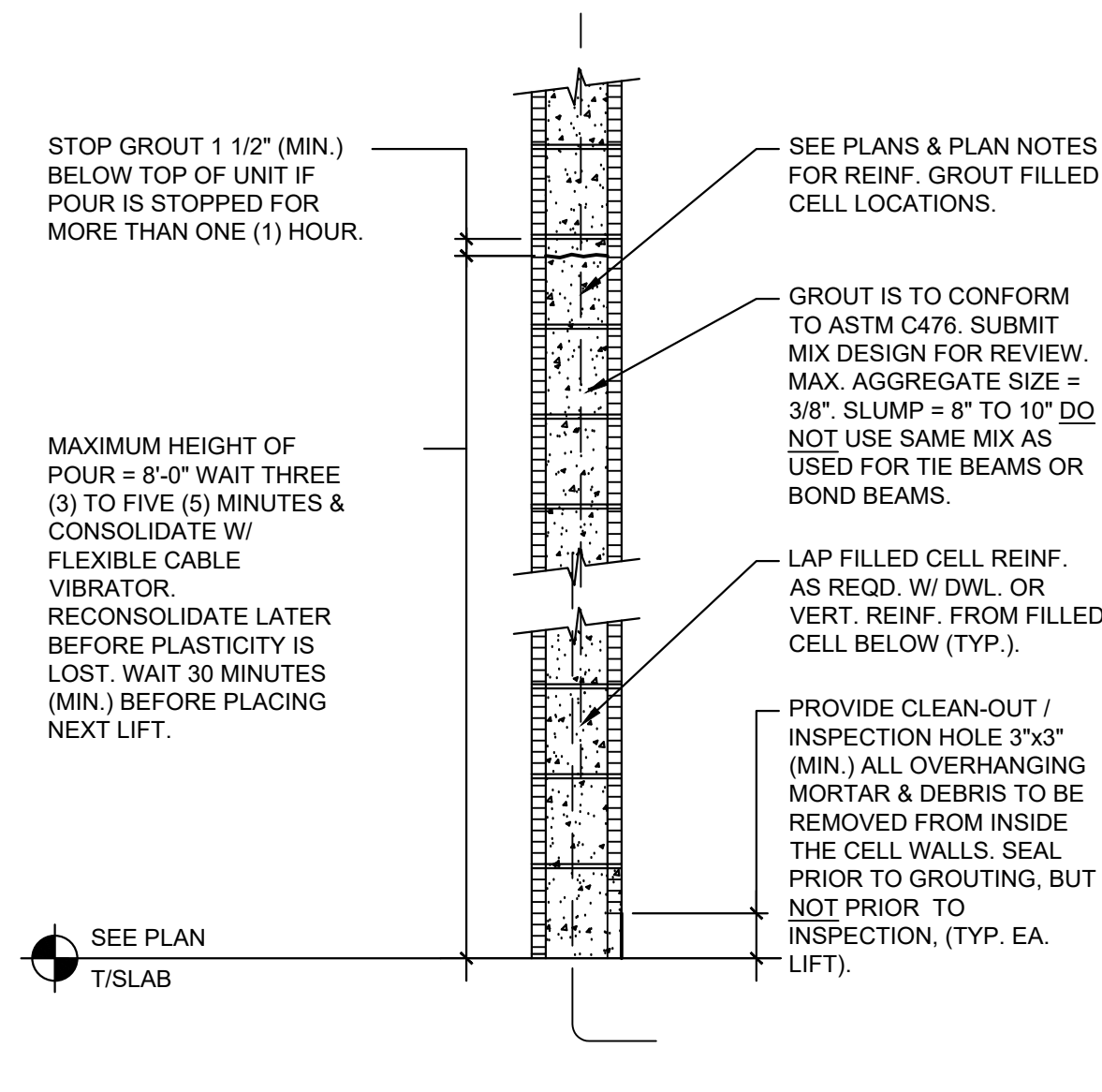
4 TYP. ELEVATED DRYER PAD

SCALE: 3/4" = 1'-0"



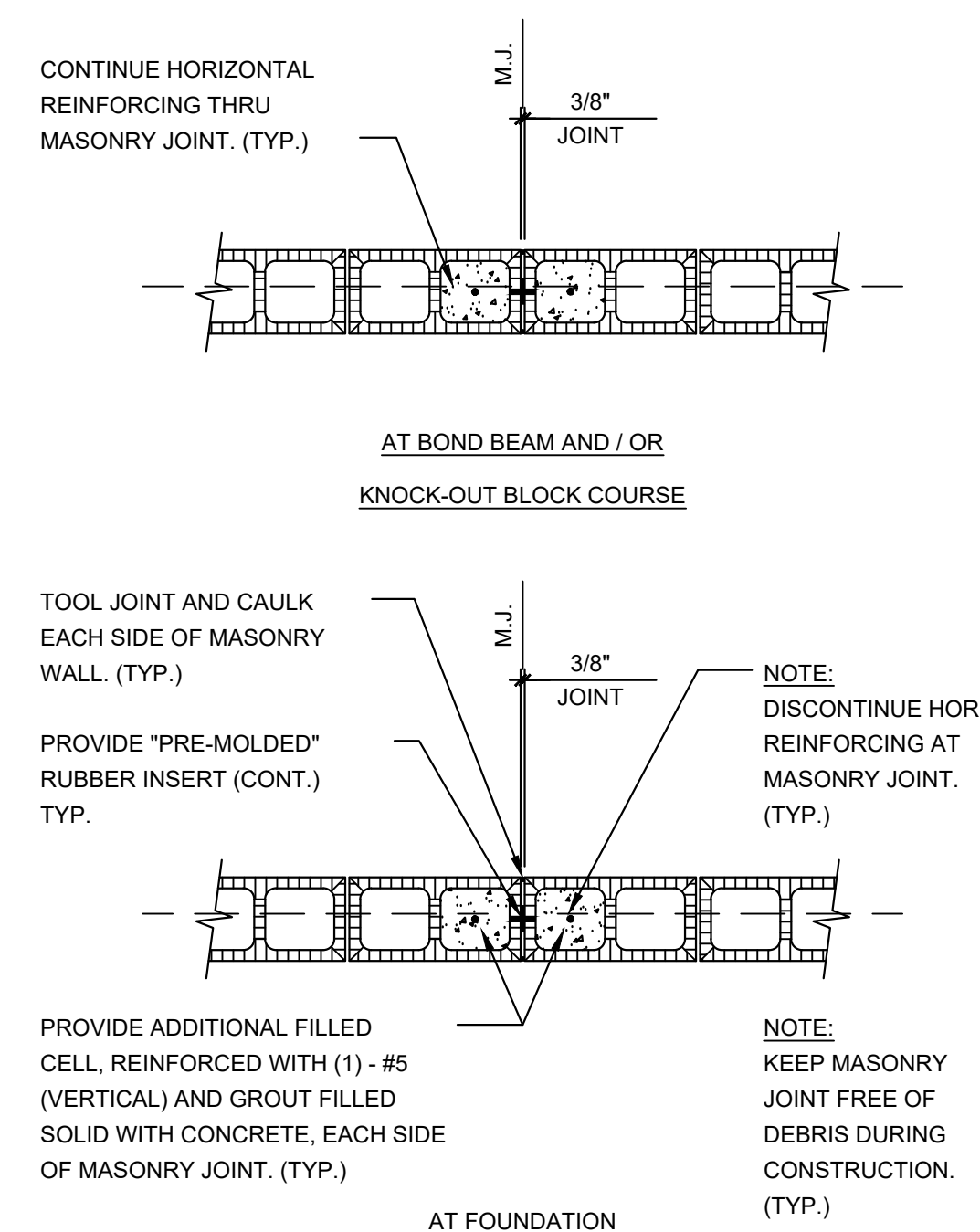
5 TYPICAL LOW LIFT FILLED CELL GROUTING DIAGRAM

NTS.



6 TYPICAL HIGH LIFT FILLED CELL GROUTING DIAGRAM

NTS.



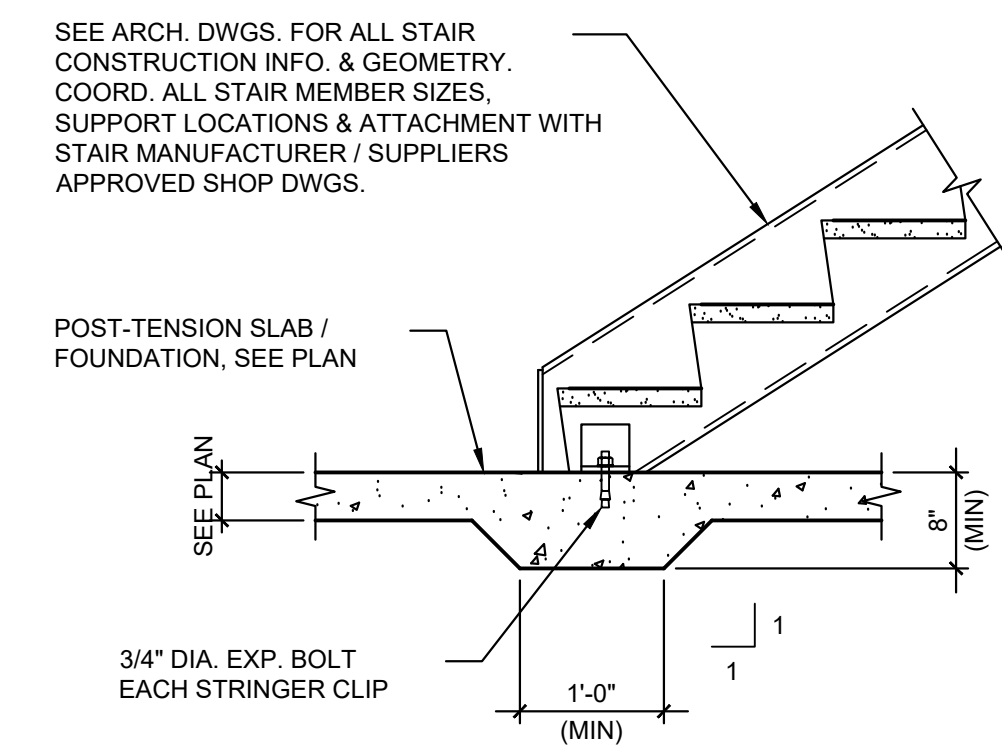
SPACING FOR CONTROL JOINT	
1.5 x WALL HEIGHT = MASONRY JOINT	SPACING IN FEET (NOT TO EXCEED)
12'-0" FROM WALL CORNERS ZONES AND	
24'-0" FOR INTERIOR WALL ZONES	

7 TYPICAL MASONRY JOINTS

SCALE: 3/4" = 1'-0"

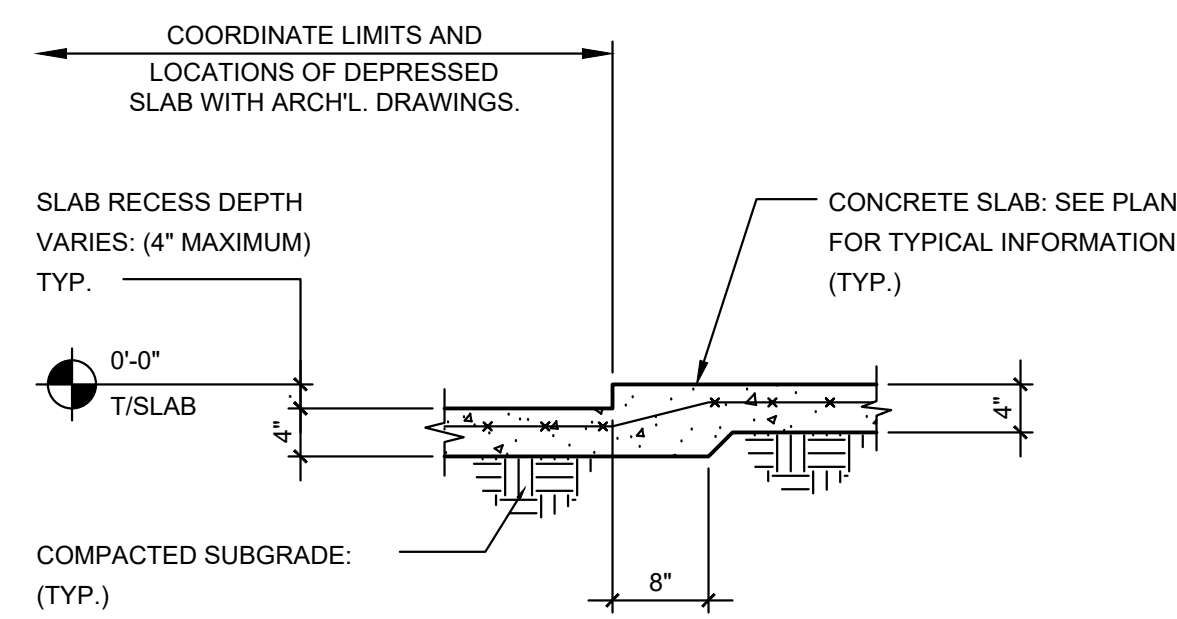
NOTE: DO NOT GROUT FILL CELLS UNTIL MORTAR HAS SET SUFFICIENTLY TO WITHSTAND THE PRESSURE OF THE GROUT. (WAIT 24 HRS. MIN.)

NOTE: DO NOT GROUT FILL CELLS UNTIL MORTAR HAS SET SUFFICIENTLY TO WITHSTAND THE PRESSURE OF THE GROUT. (WAIT 24 HRS. MIN.)



8 TYP. STAIR AT FOUNDATION

SCALE: 3/4" = 1'-0"

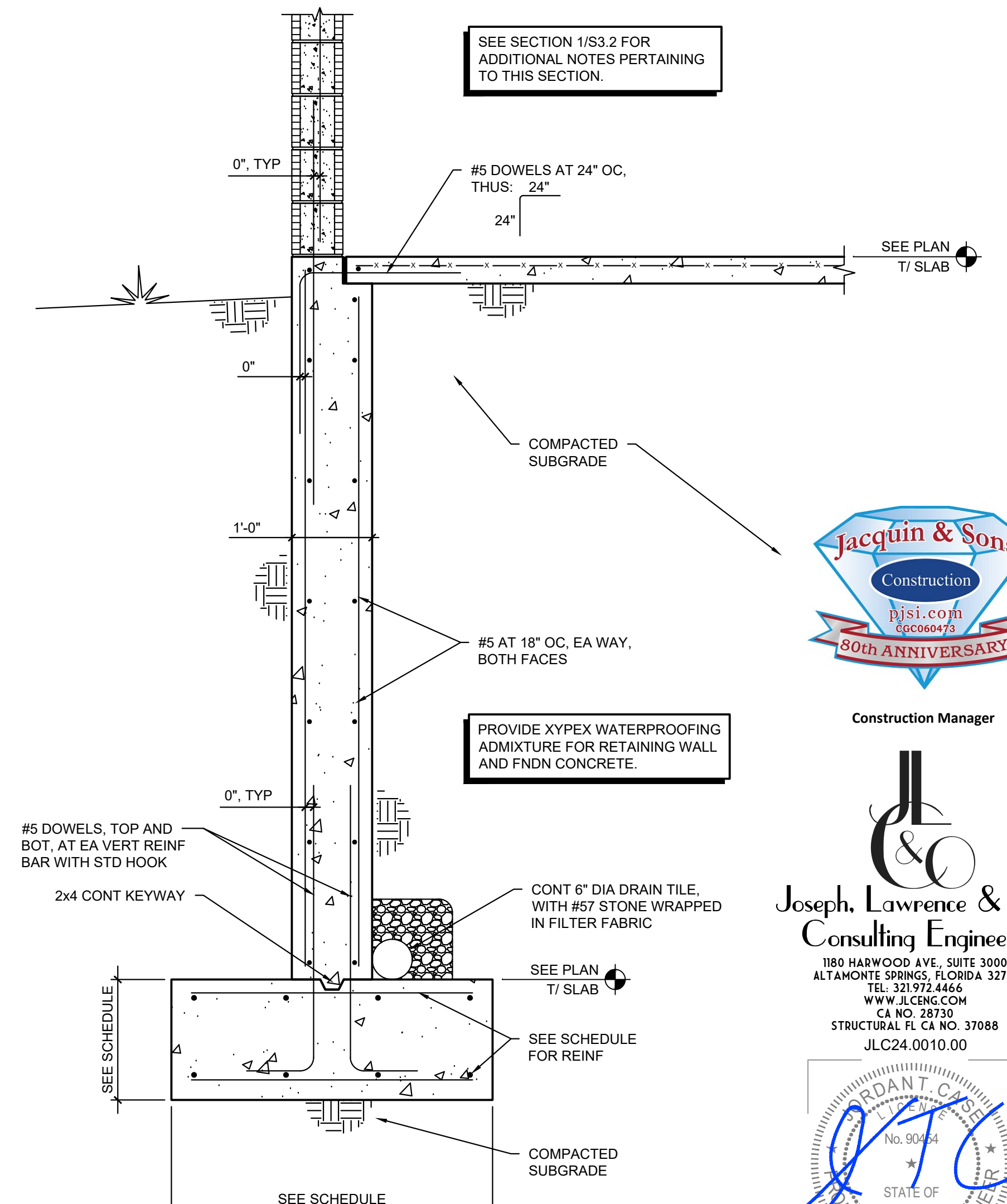


9 DEPRESSED SLAB DETAIL

SCALE: 3/4" = 1'-0"

MARK	WIDTH x LENGTH (FT-IN x FT-IN)	DEPTH (IN)	REINFORCING				COMMENTS
			BOTTOM		TOP		
			LONG	SHORT	LONG	SHORT	
TS-24	2-0 x CONT	12	(2) #5		(1) #5		
WF-4	4-0 x CONT	12	(4) #5	#6 AT 18" OC			
WF-5	5-0 x CONT	12	(5) #5	#6 AT 18" OC			
F-6	6-0 x 6-0	18	(5) #6	(5) #6			
F-6A	6-0 x 6-0	18	(5) #6	(5) #6			
F-8	8-0 x 8-0	22	(6) #8	(6) #8			
F-8x16	8-0 x 16-0	22	(8) #8	(16) #6	(8) #6	(16) #6	

NOTE: EXCAVATIONS UNDER FOUNDATIONS TO INSTALL PIPING AND CONDUIT RUNS ARE PROHIBITED. ANY PIPE THAT PASSES UNDER A FOUNDATION OR THROUGH A FOUNDATION WALL SHALL HAVE A PIPE SLEEVE. THE SLEEVE SHALL BE TWO PIPE SIZES GREATER THAN THE PIPE PASSING THROUGH THE WALL. UNDERMINING OF FOUNDATIONS WOULD REQUIRE THE CONTRACTOR TO RETAIN A LICENSED STRUCTURAL ENGINEER IN THE STATE OF FLORIDA WHO SHALL INVESTIGATE THE CONDITIONS AND DESIGN TEMPORARY SHORING AS REQUIRED BY THE FLORIDA BUILDING CODE.

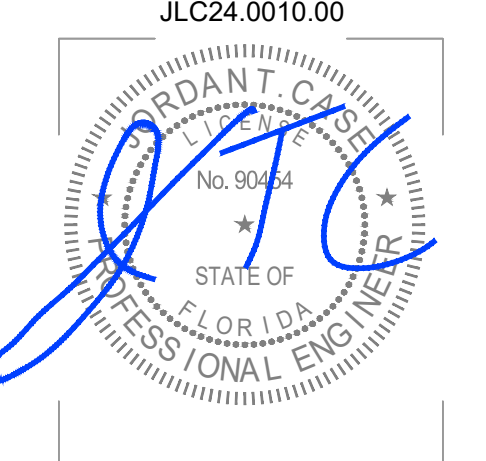


10 SECTION

SCALE: 3/4" = 1'-0"



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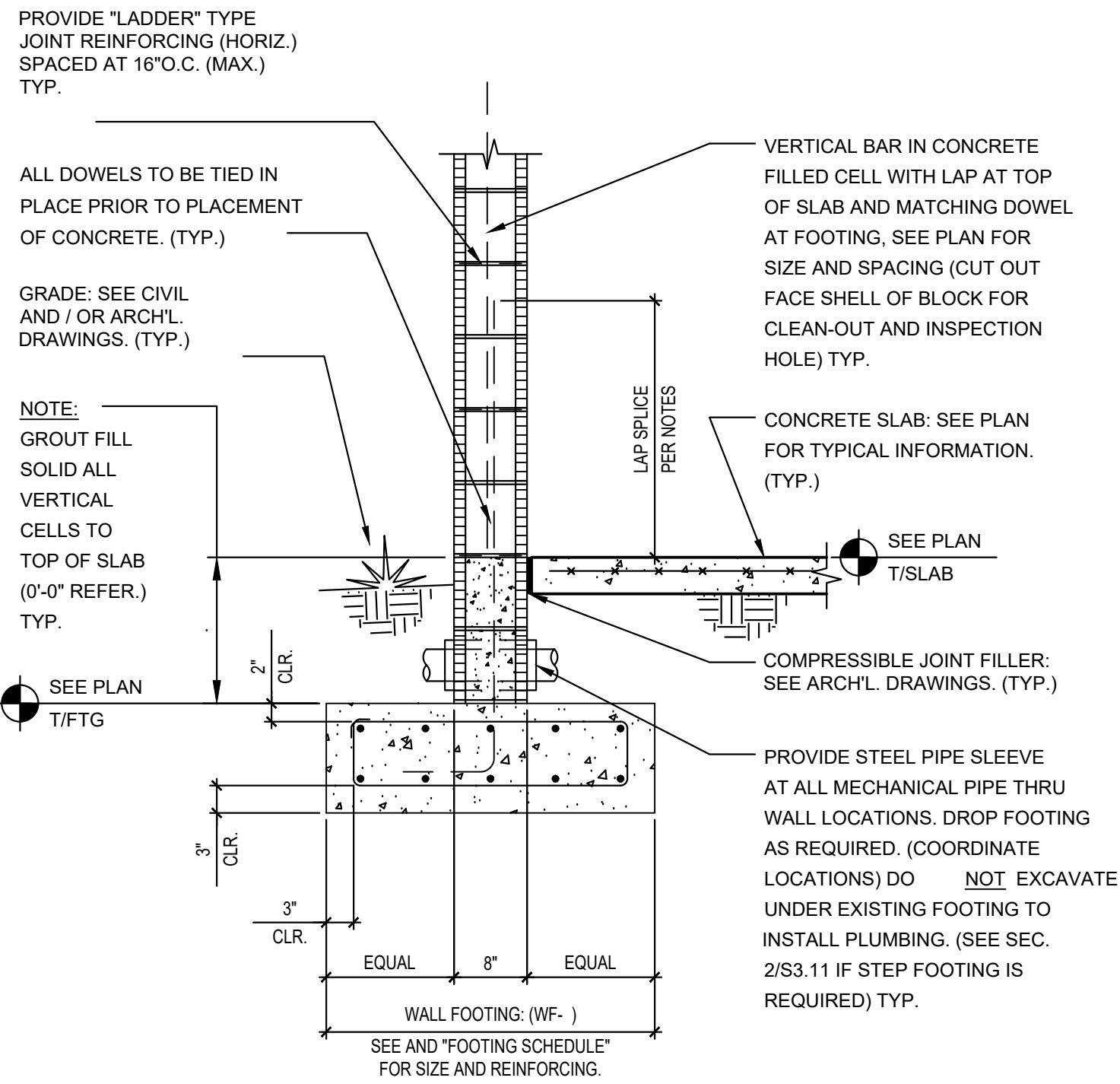
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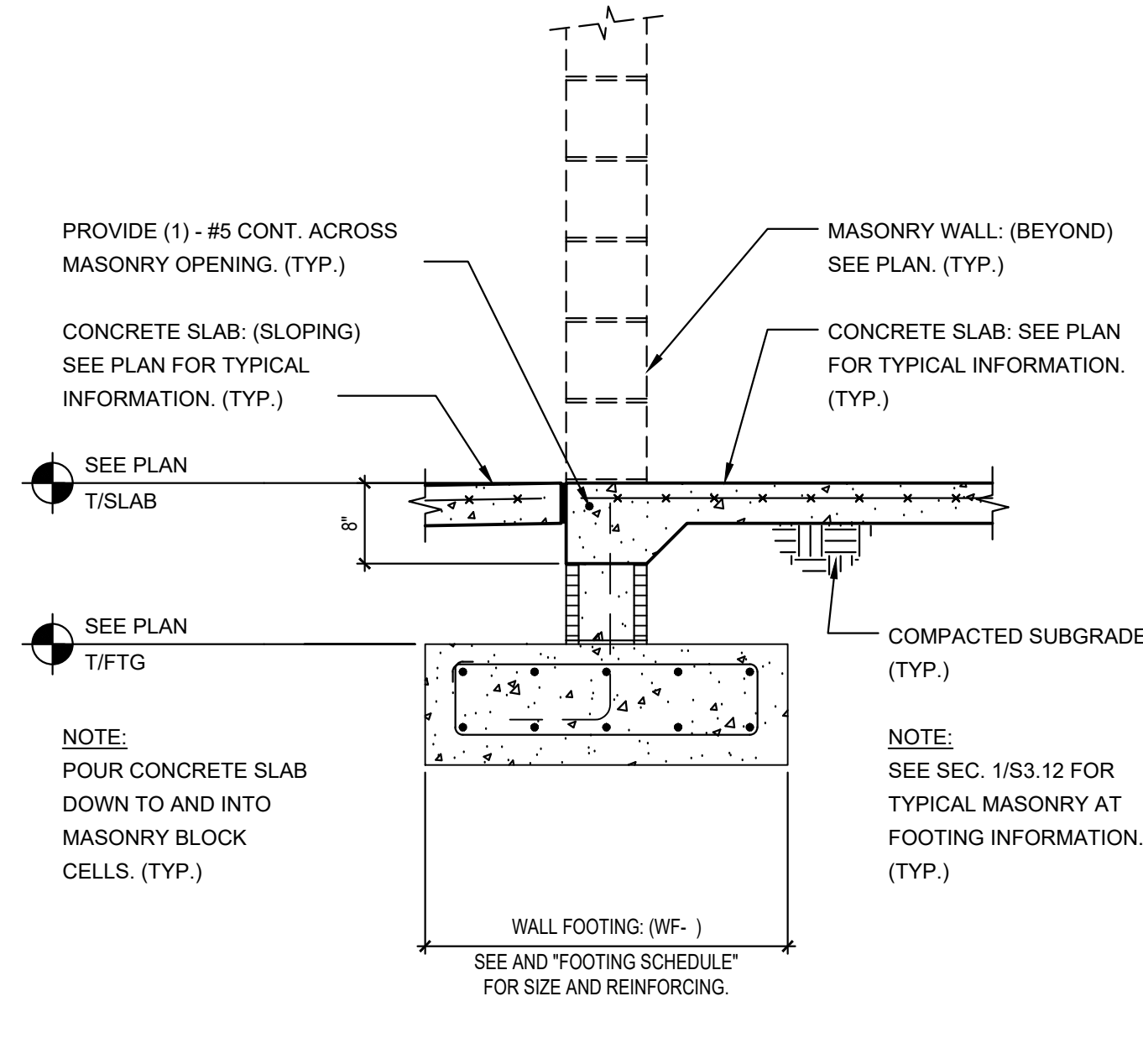
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SHEET

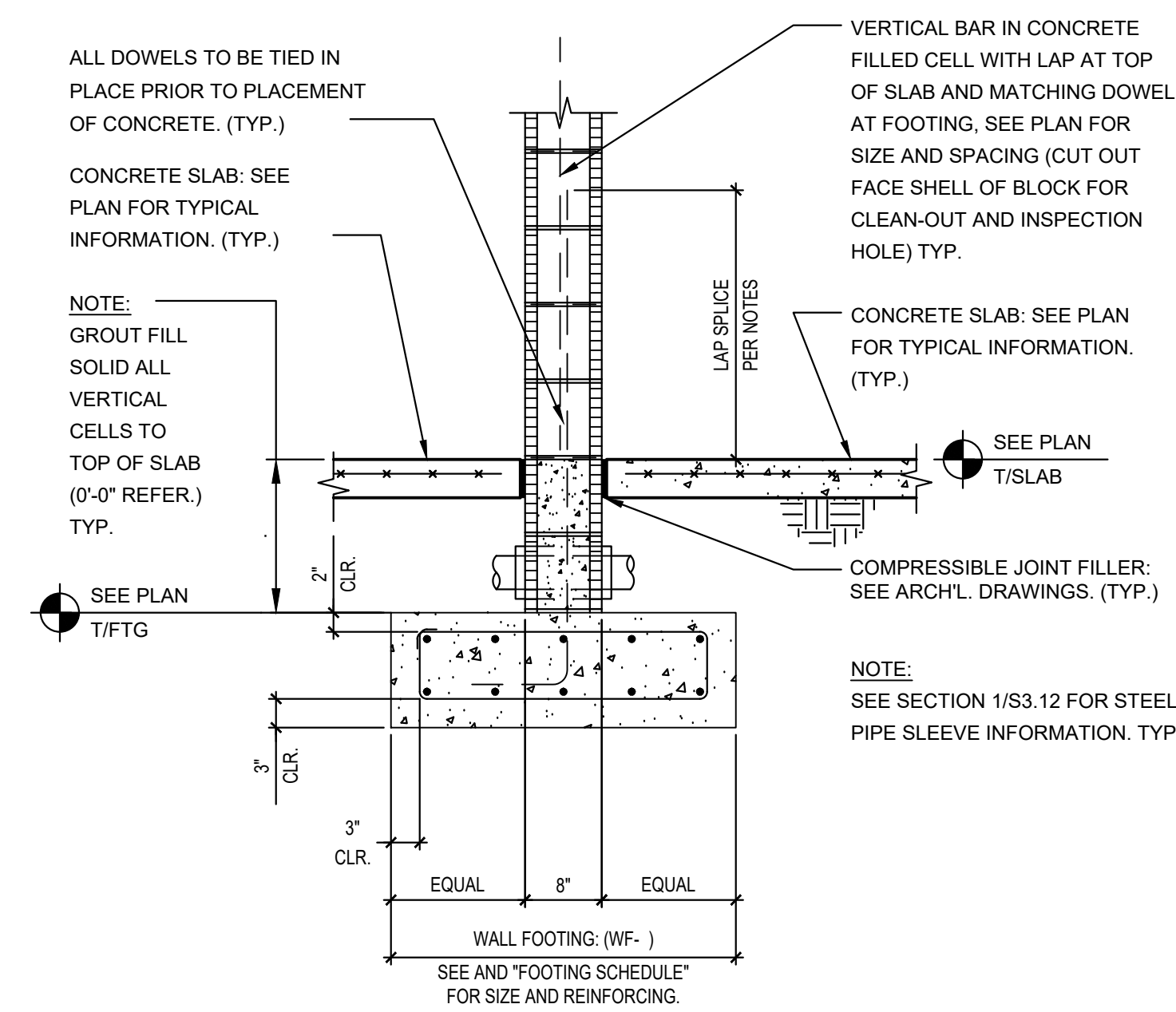
S3.1



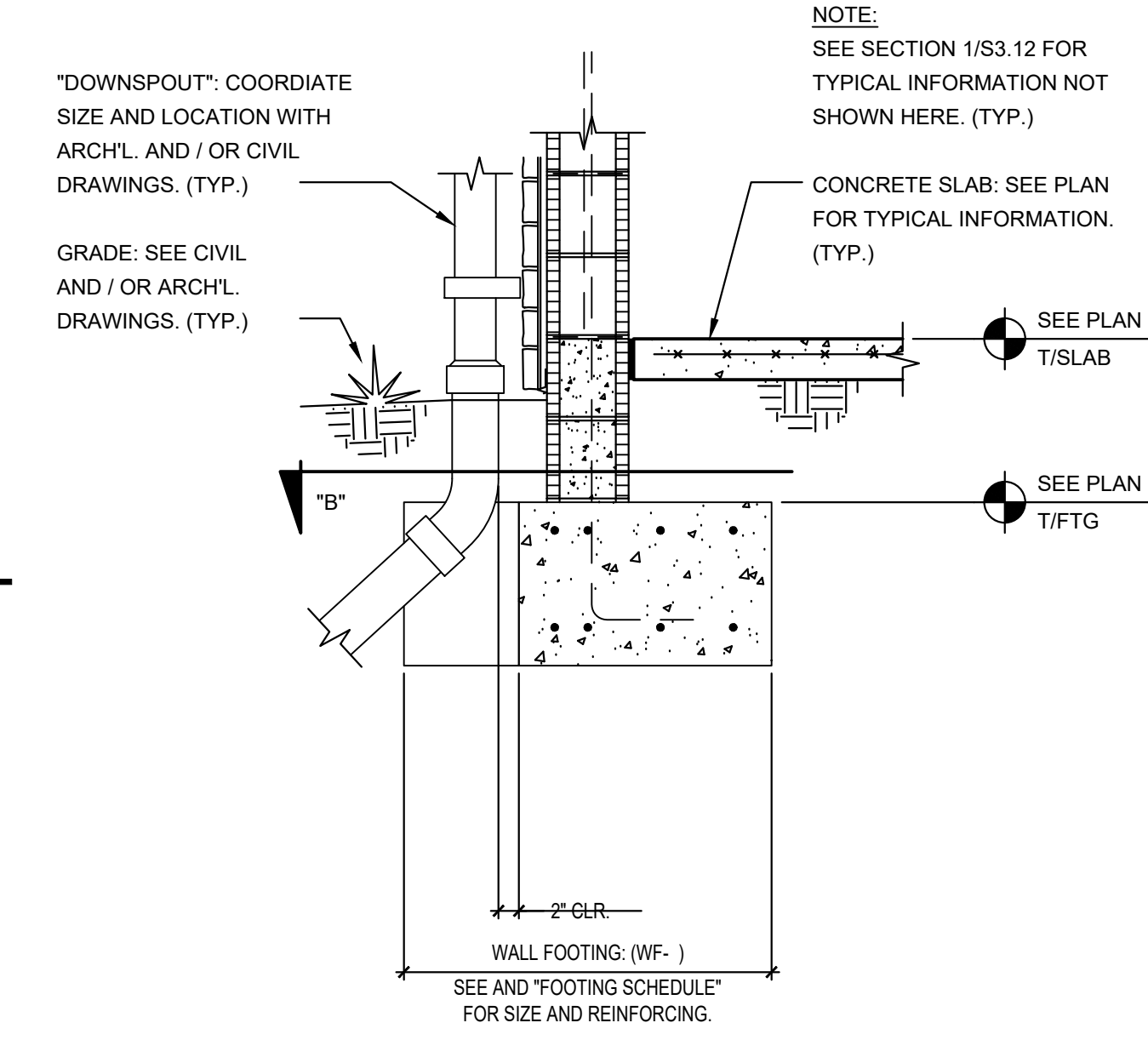
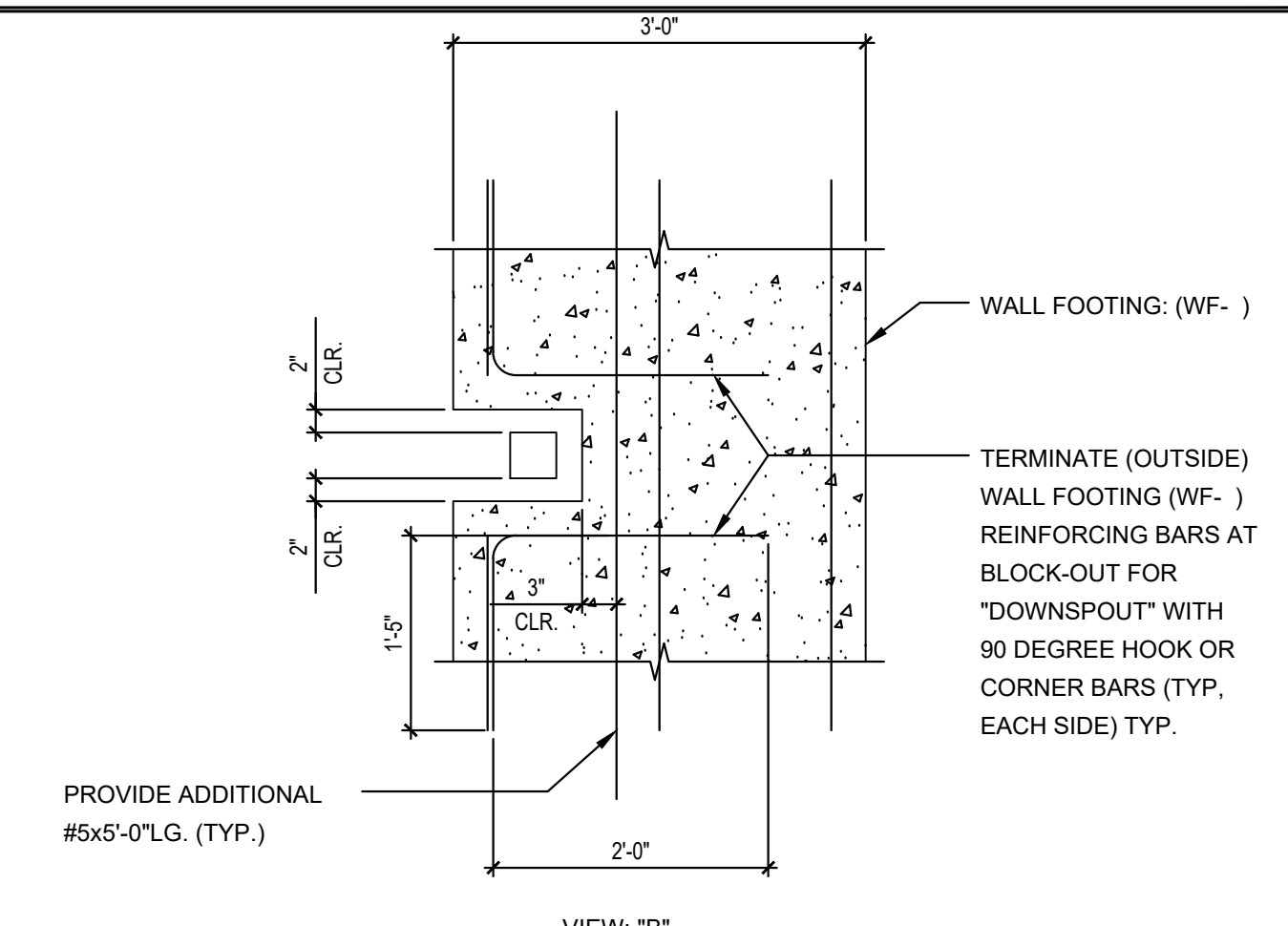
1 EXTERIOR CMU BRG. WALL AT FOOTING
SCALE: 3/4" = 1'-0"



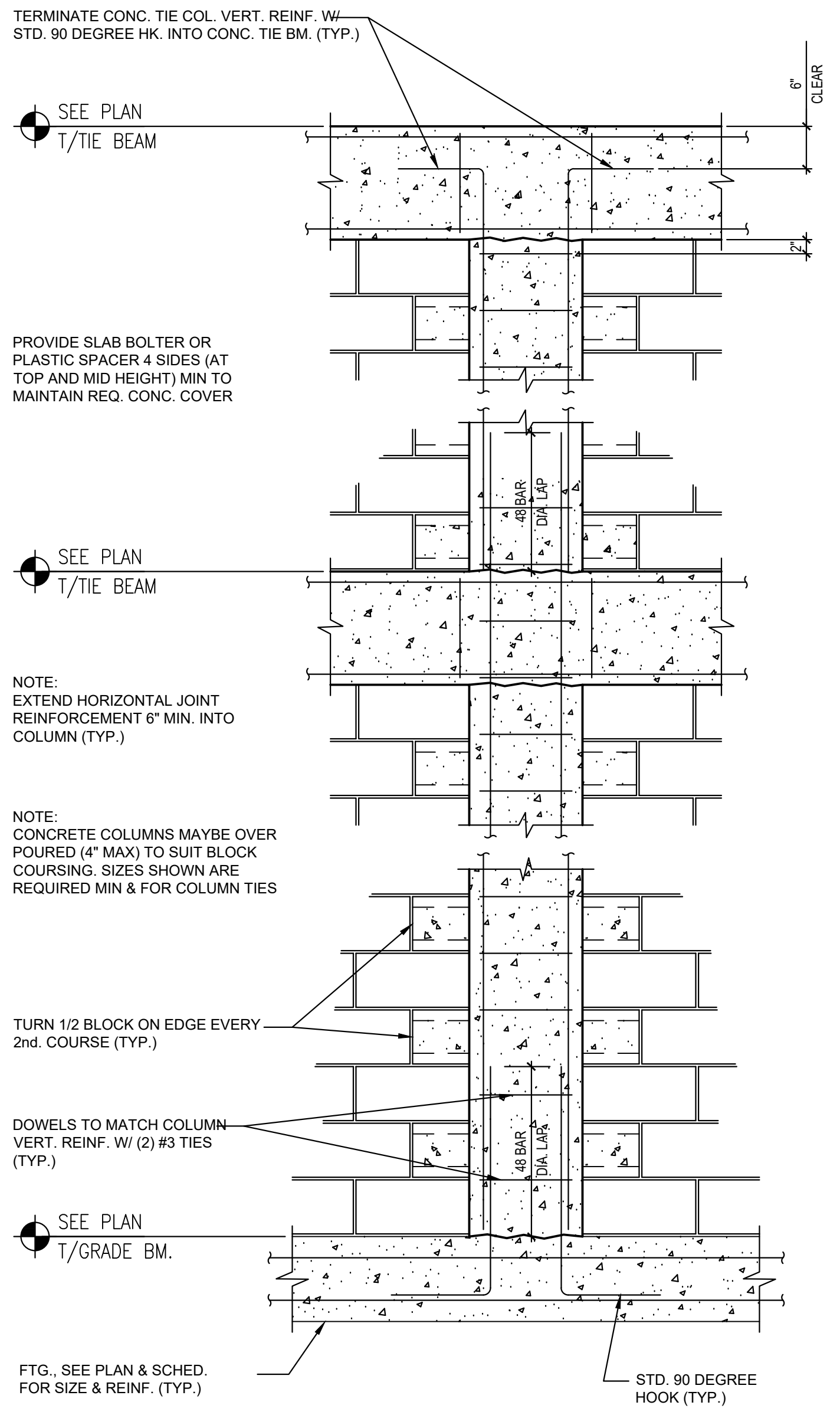
2 SECTION
SCALE: 3/4" = 1'-0"



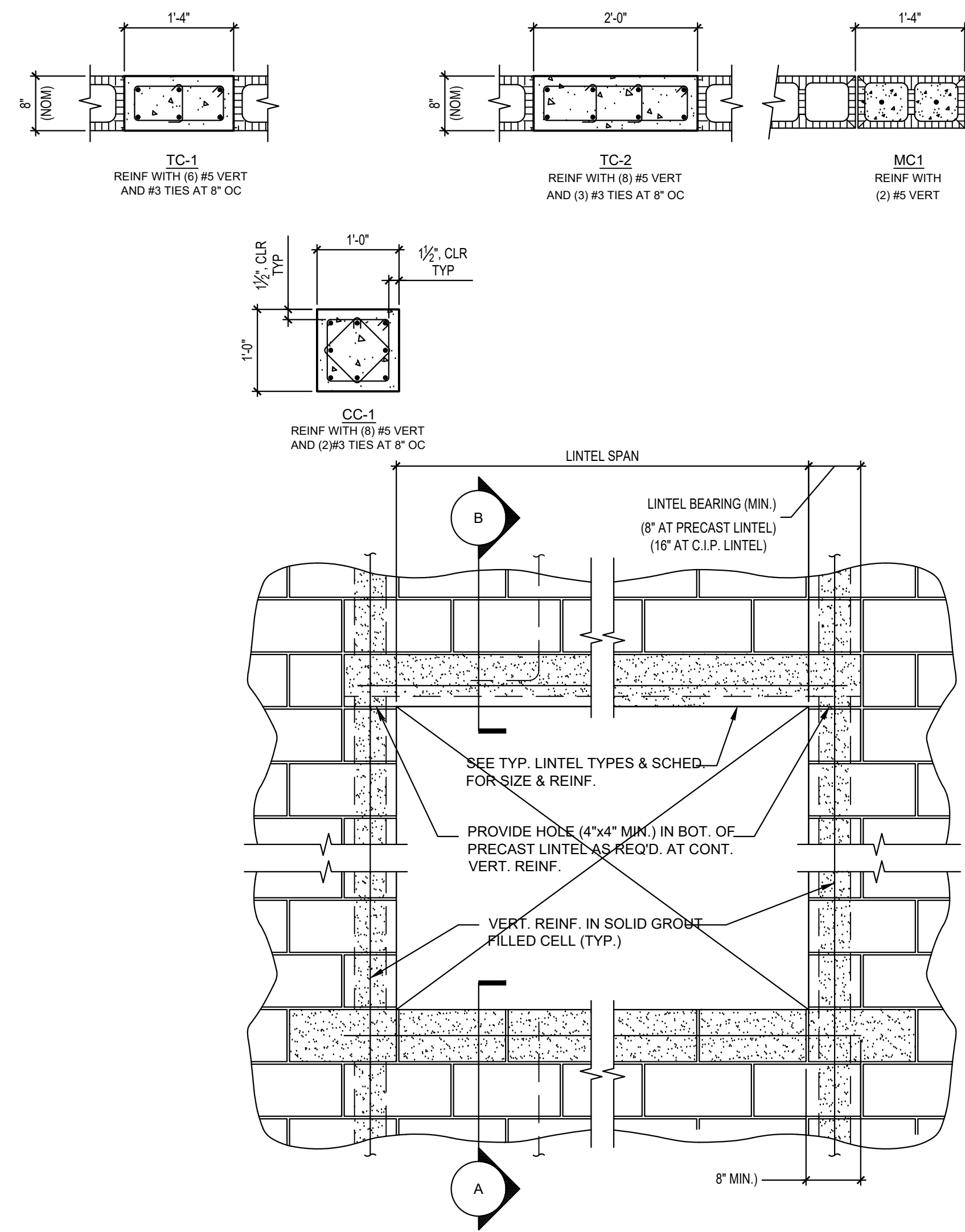
3 INTERIOR CMU BRG. WALL AT FOOTING
SCALE: 3/4" = 1'-0"



4 TYP. SECTION AT DOWNSPOUT PENETRATING CMU BRG. WALL FOOTING.
SCALE: 3/4" = 1'-0"



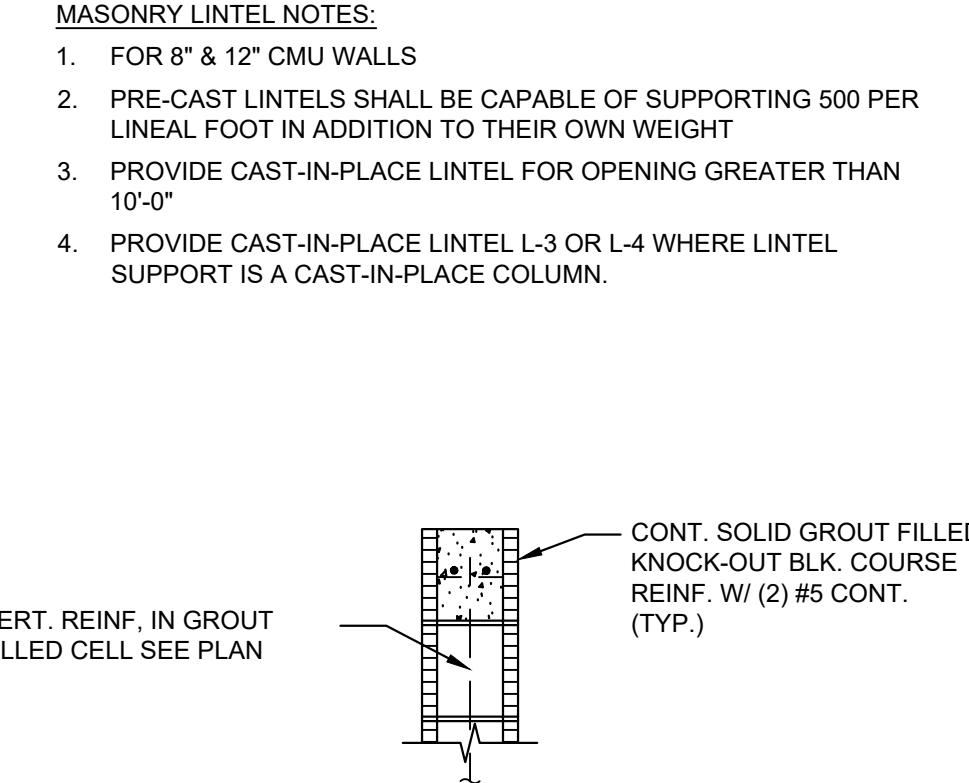
5 CONCRETE TIE COLUMN AT FOOTING AND TYPES
SCALE: 3/4" = 1'-0"



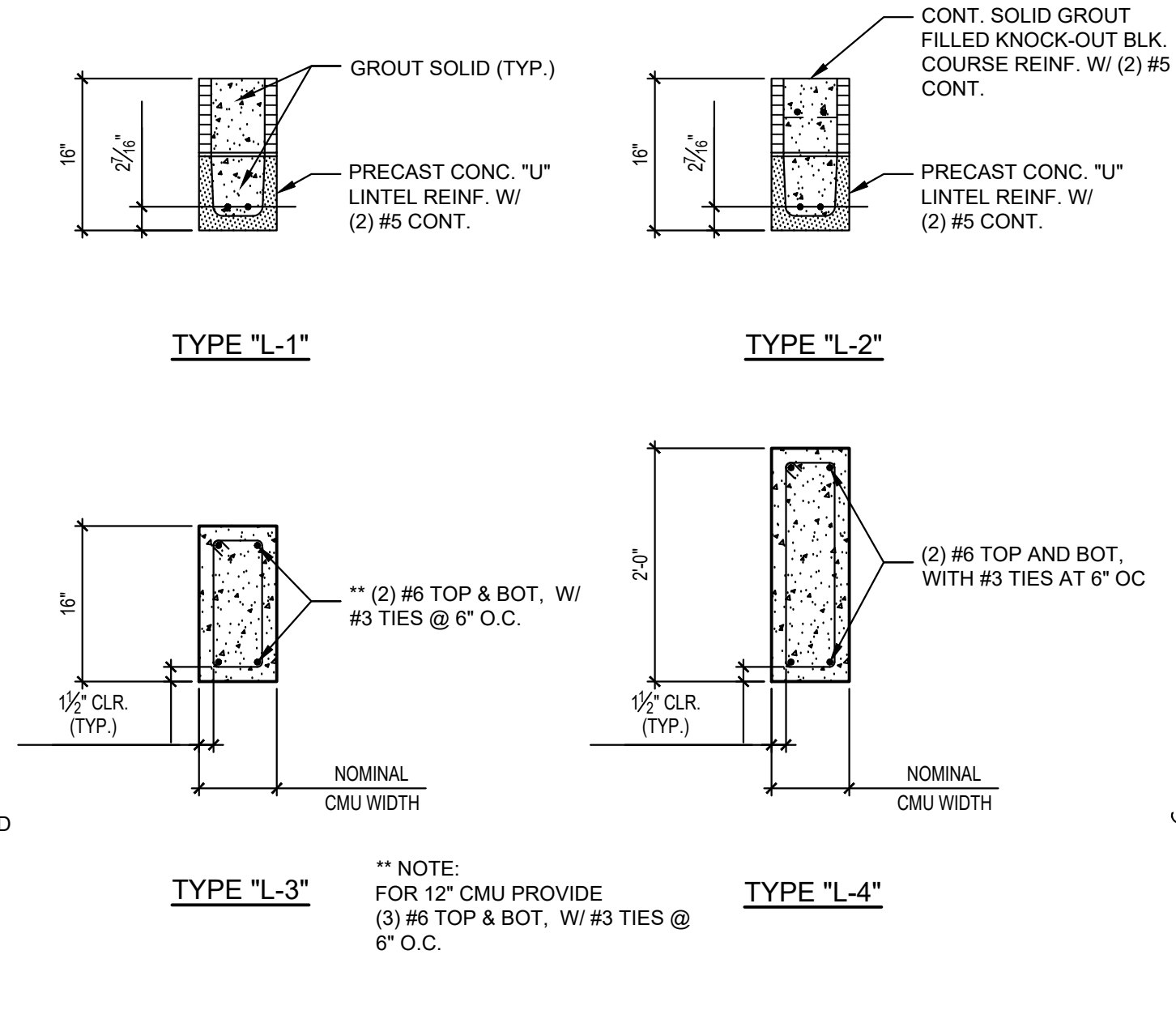
6 TYPICAL MASONRY OPENING DETAIL
SCALE: 3/4" = 1'-0" (FOR 8" AND 12" CMU)

LINTEL SCHEDULE

TYPE	MAXIMUM SPAN	REMARK
L-1	6'-0"	
L-2	10'-0"	
L-3	16'-0"	C.I.P. CONCRETE
L-4	20'-0"	C.I.P. CONCRETE



A TYP. WINDOW SILL
SCALE: 3/4" = 1'-0"

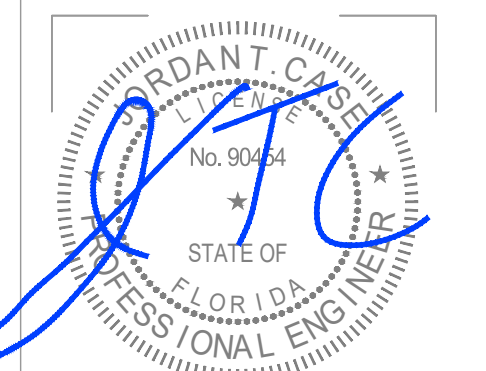


B TYPICAL LINTEL TYPES
SCALE: 3/4" = 1'-0"



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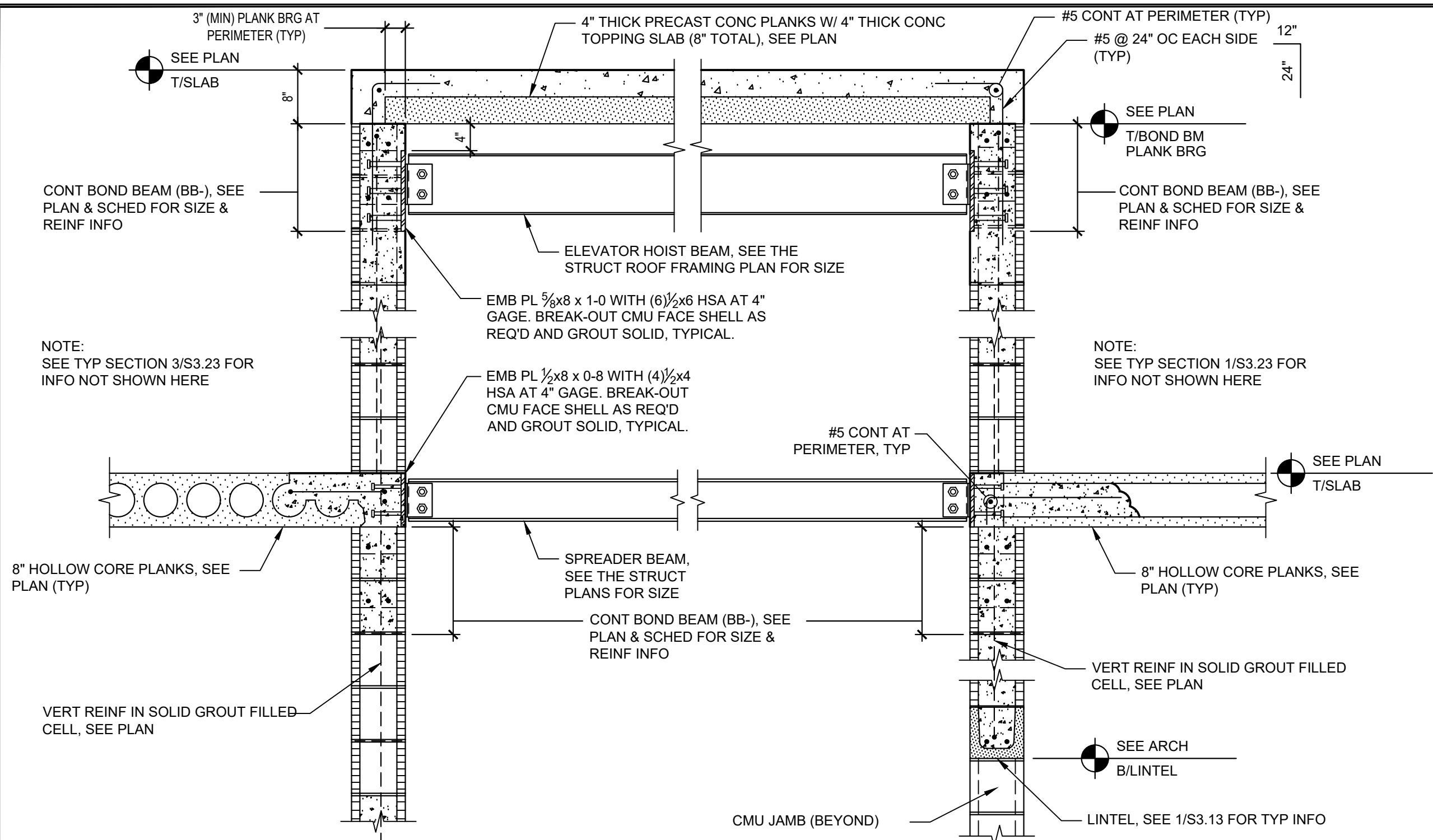
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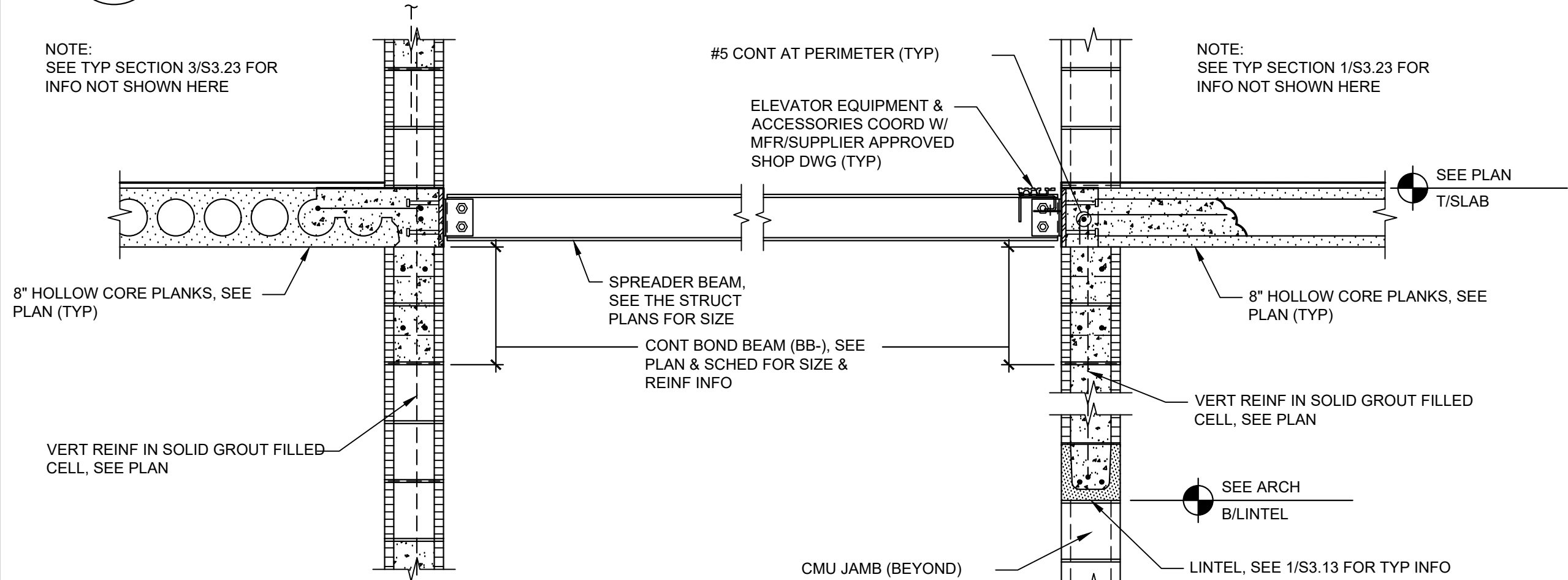
DATE 07-11-2024

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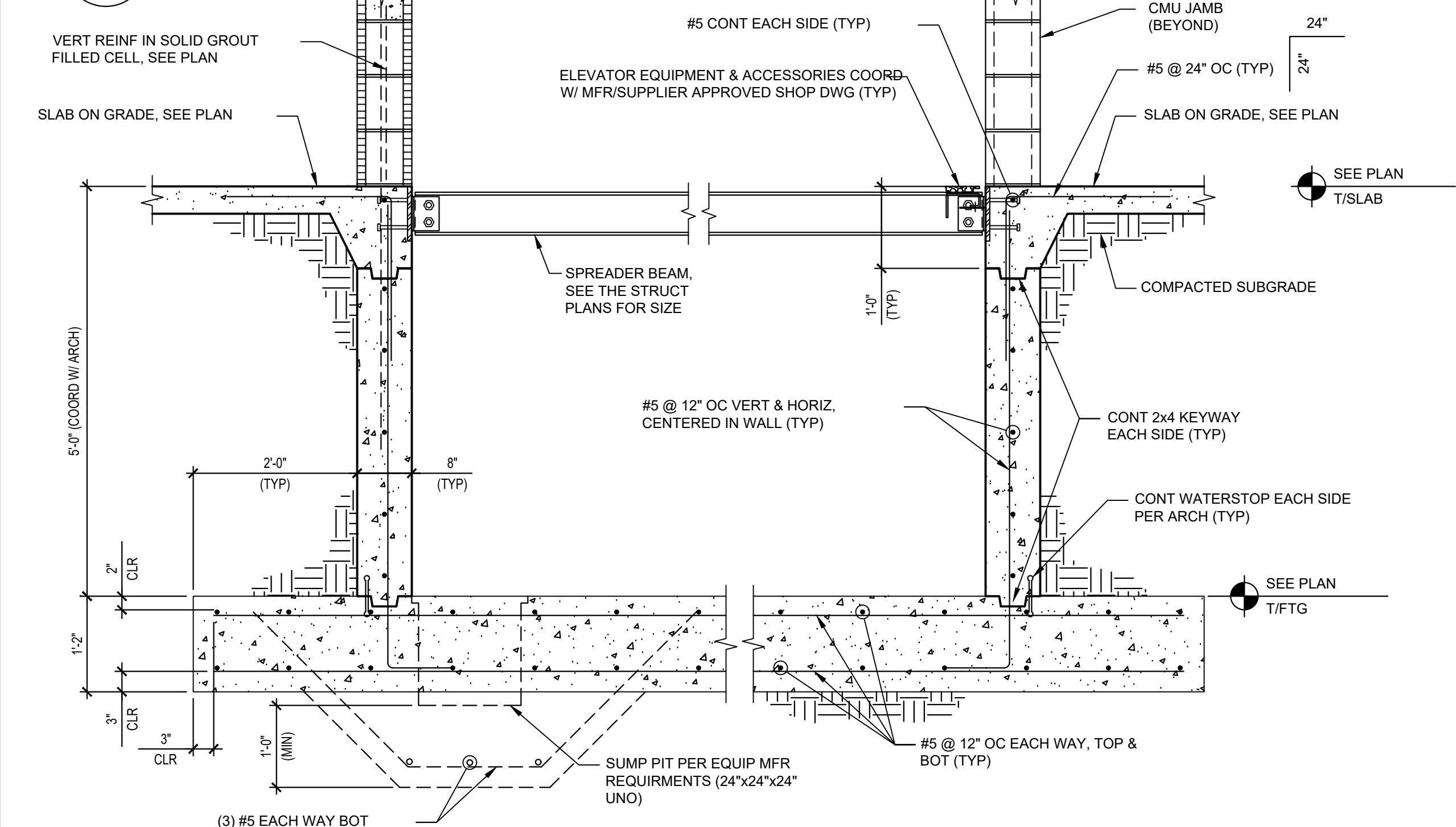
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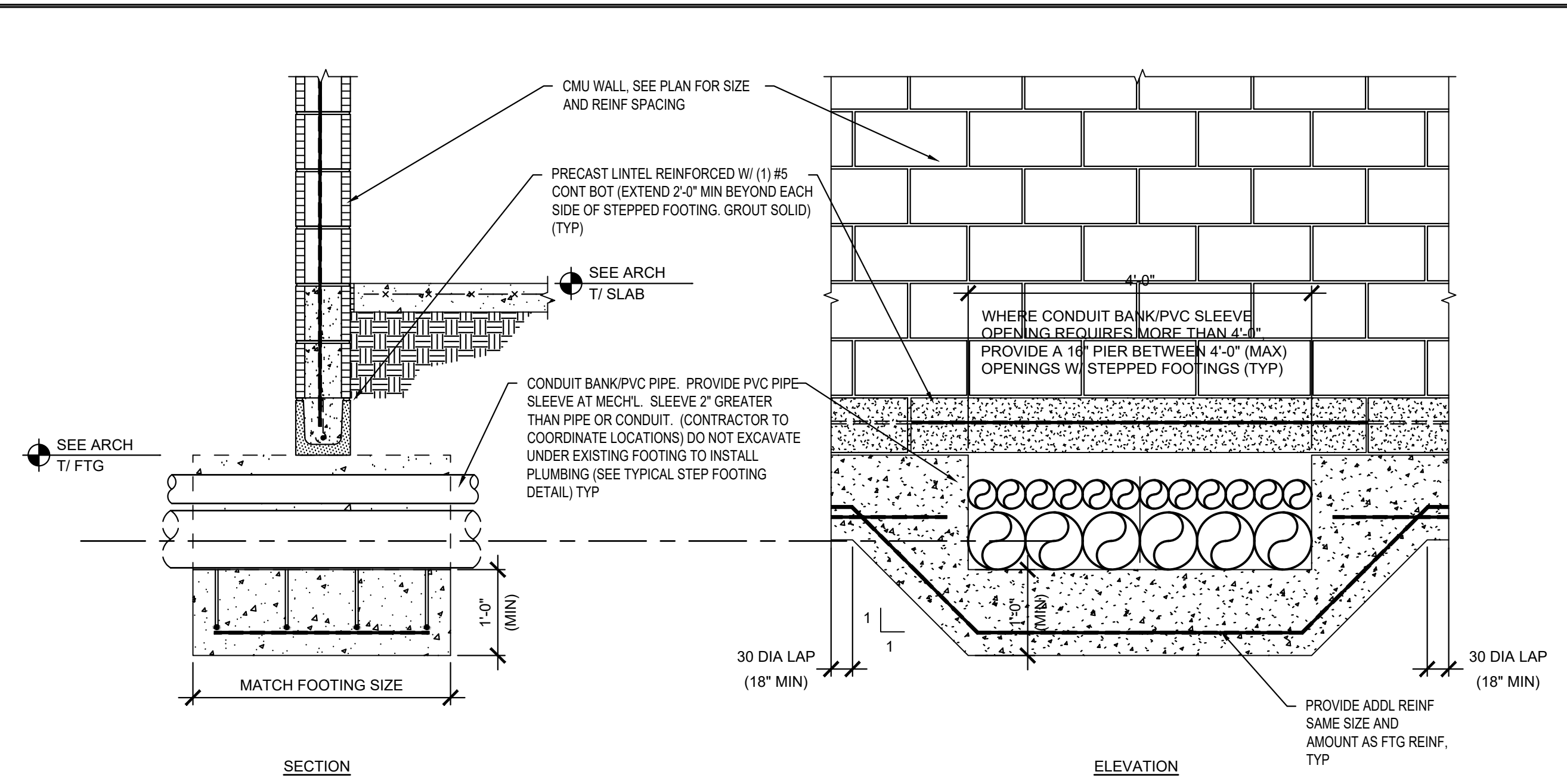
3 ELEVATOR SECTION
SCALE: 3/4" = 1'-0"



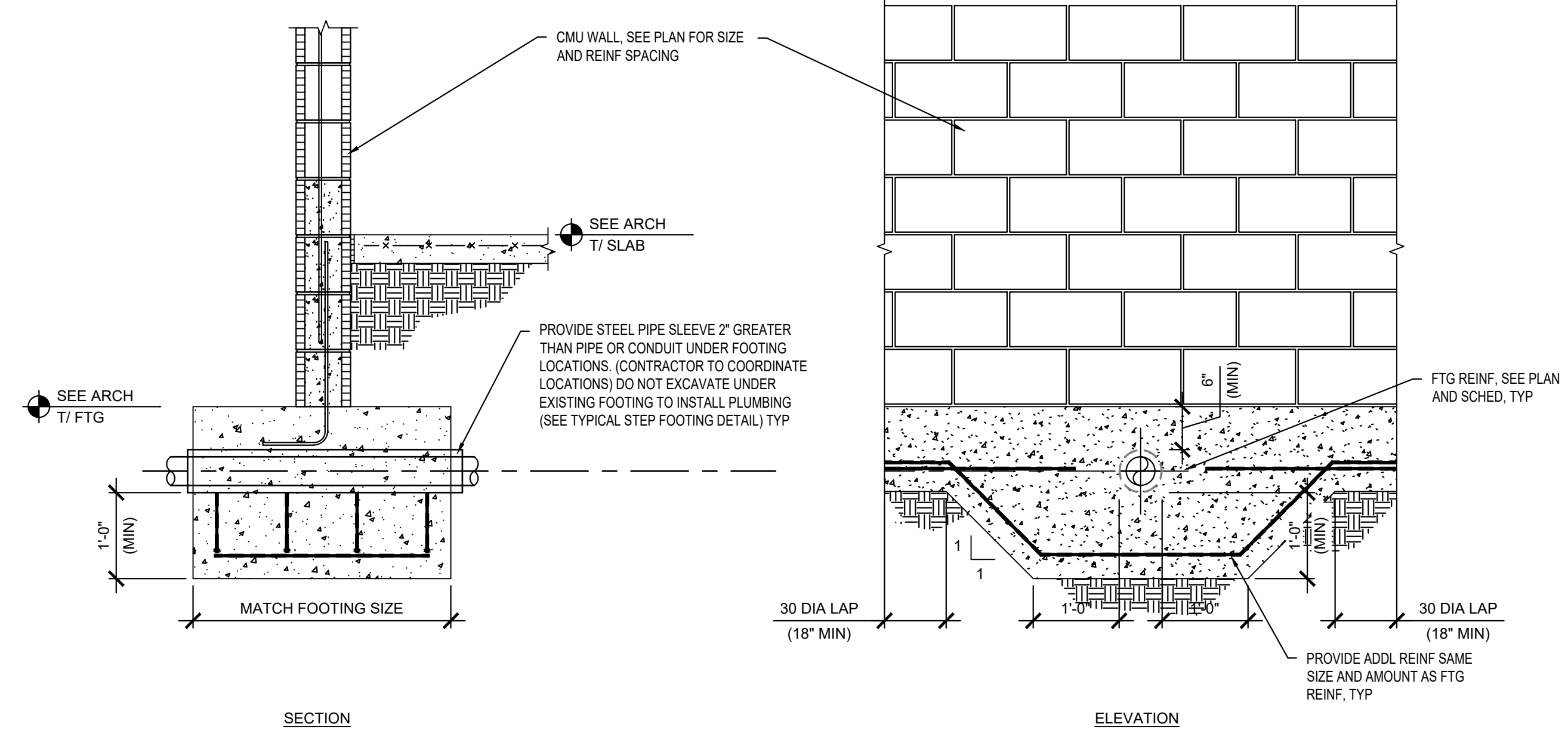
2 ELEVATOR SECTION
SCALE: 3/4" = 1'-0"



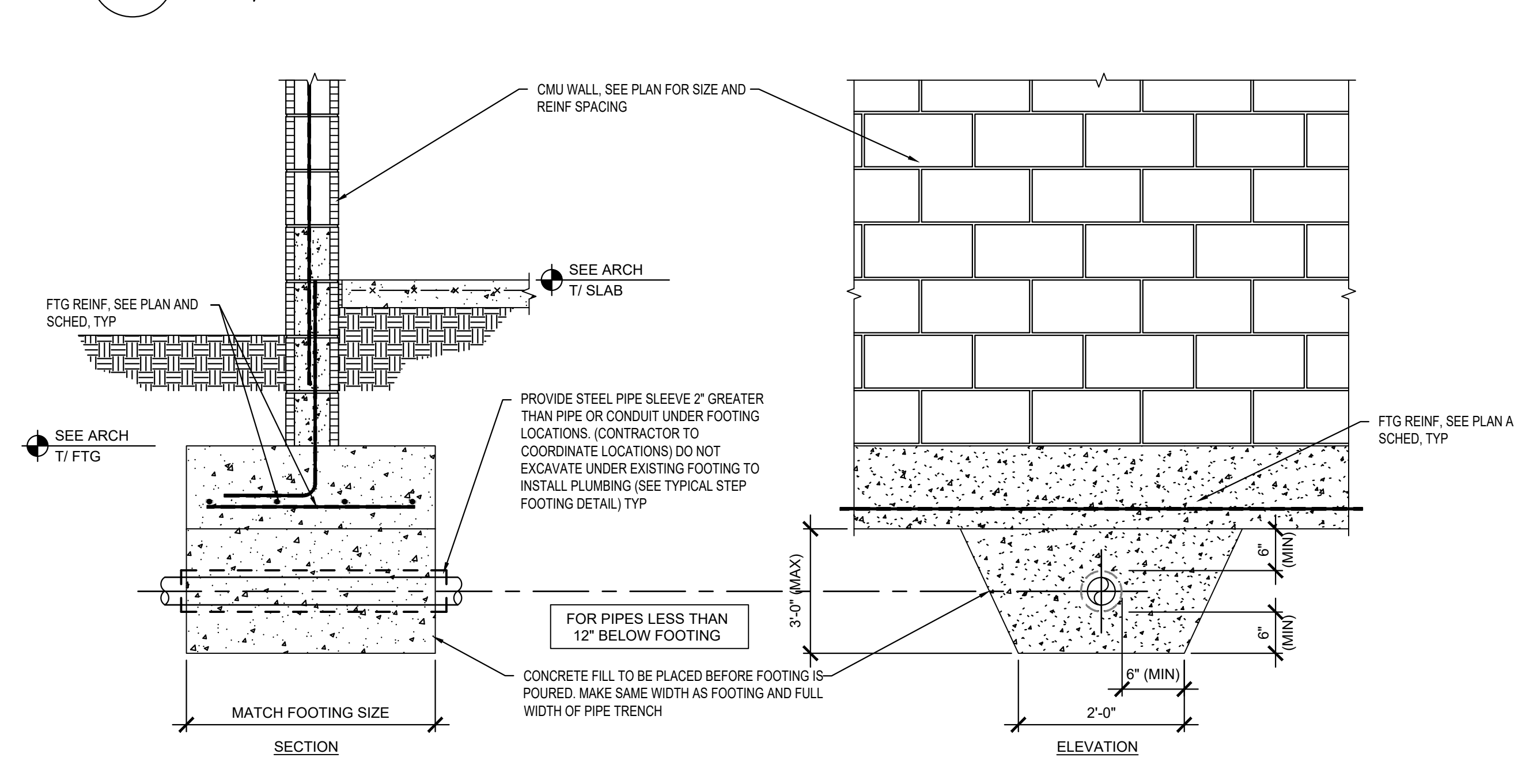
1 ELEVATOR SECTION
SCALE: 3/4" = 1'-0"



6 TYP PENETRATION THRU FOOTING (CONDUIT BANK)
SCALE: 3/4" = 1'-0"



5 TYP PIPE PENETRATION THRU FOOTING
SCALE: 3/4" = 1'-0"



4 TYP PIPE PENETRATION BELOW FOOTING
SCALE: 3/4" = 1'-0"

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No. 9044
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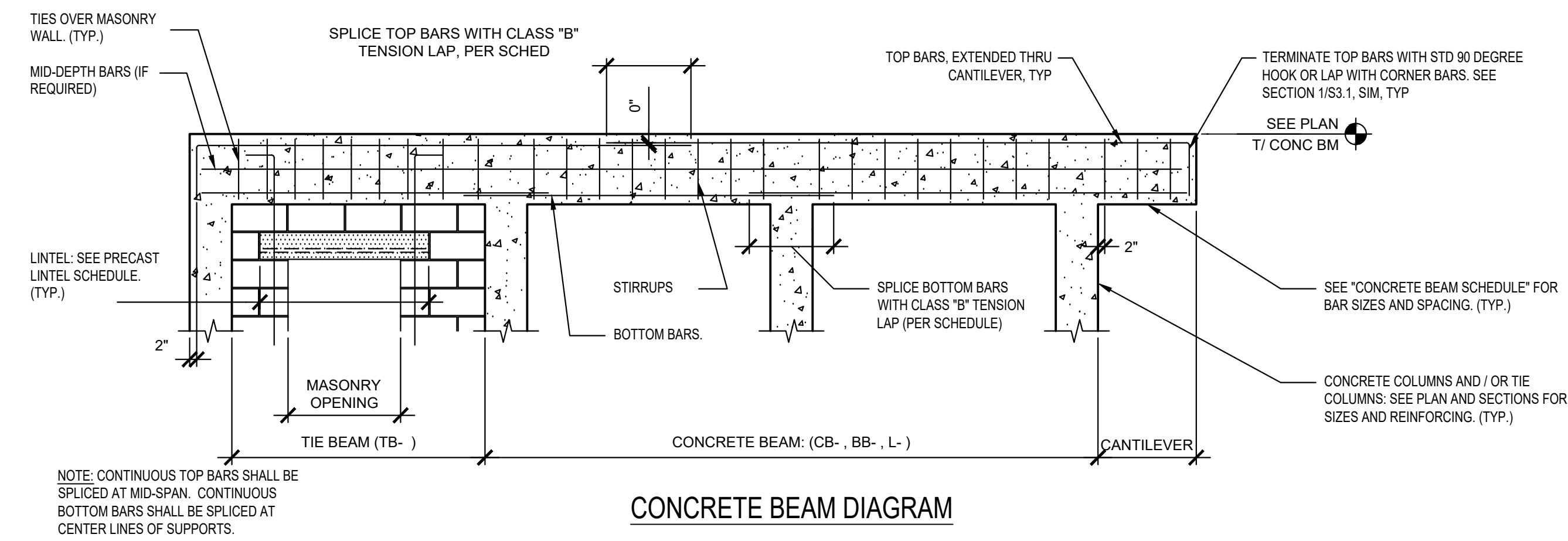
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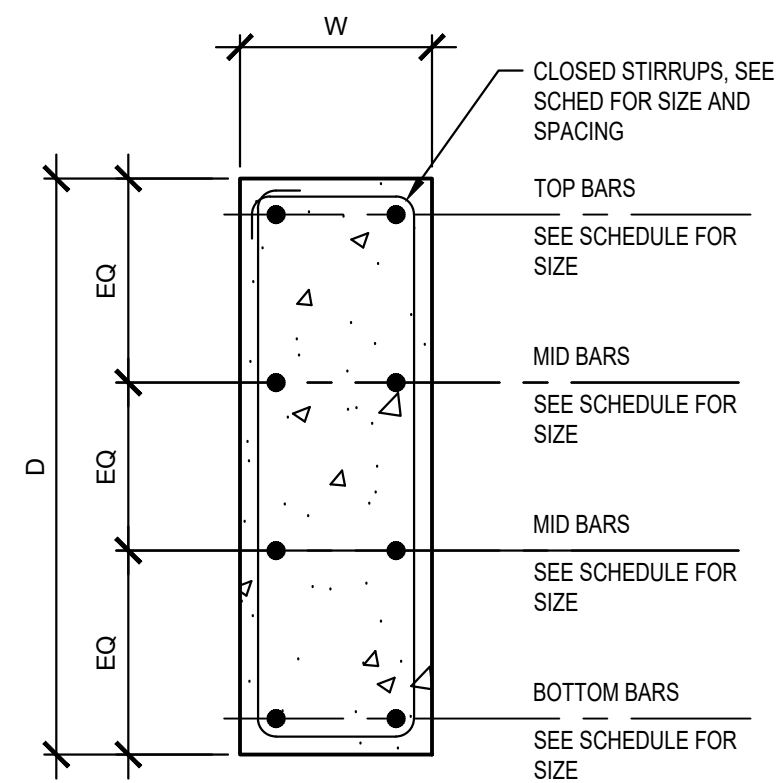
JOB NO. 22039	DATE 07-11-2024
SHEET S3.3	



CONCRETE BEAM DIAGRAM

CONCRETE BEAM SCHEDULE									
BEAM MARK	BEAM SIZE (W x D) (IN x IN)	TOP BARS	MID BARS	BOTTOM BARS	"C"	"E"	STIRRUPS		REMARKS
							SIZE	SPACING, IN	
BB1	8 x 16	(2) #5	-	(2) #5	-	-	-	-	MASONRY BOND BEAM
CB1	8 x 32	(2) #5	(2) #5	(2) #5	-	-	#3	12	DETAIL A

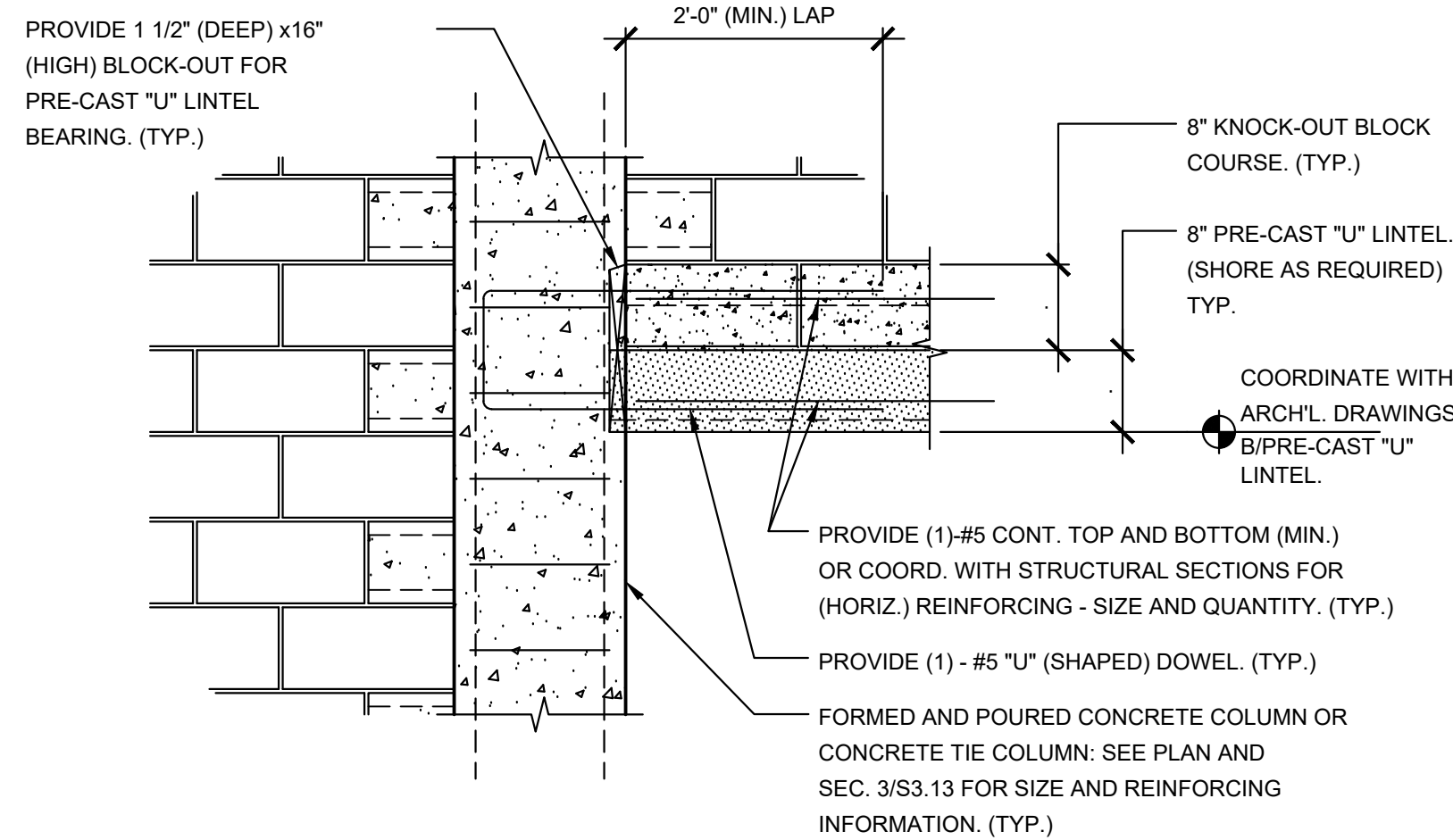
NOTE:
 1. CONCRETE TIE BEAMS (TB-) AND CONCRETE BEAMS (CB-) WIDTHS SHOWN IN SCHEDULE ARE NOMINAL.
 2. WHERE CONCRETE BEAMS HAVE ADJACENT MASONRY, ADJUST BEAM WIDTHS TO MATCH MASONRY WIDTH. ADJUST STIRRUP WIDTHS ACCORDINGLY.



DETAIL A

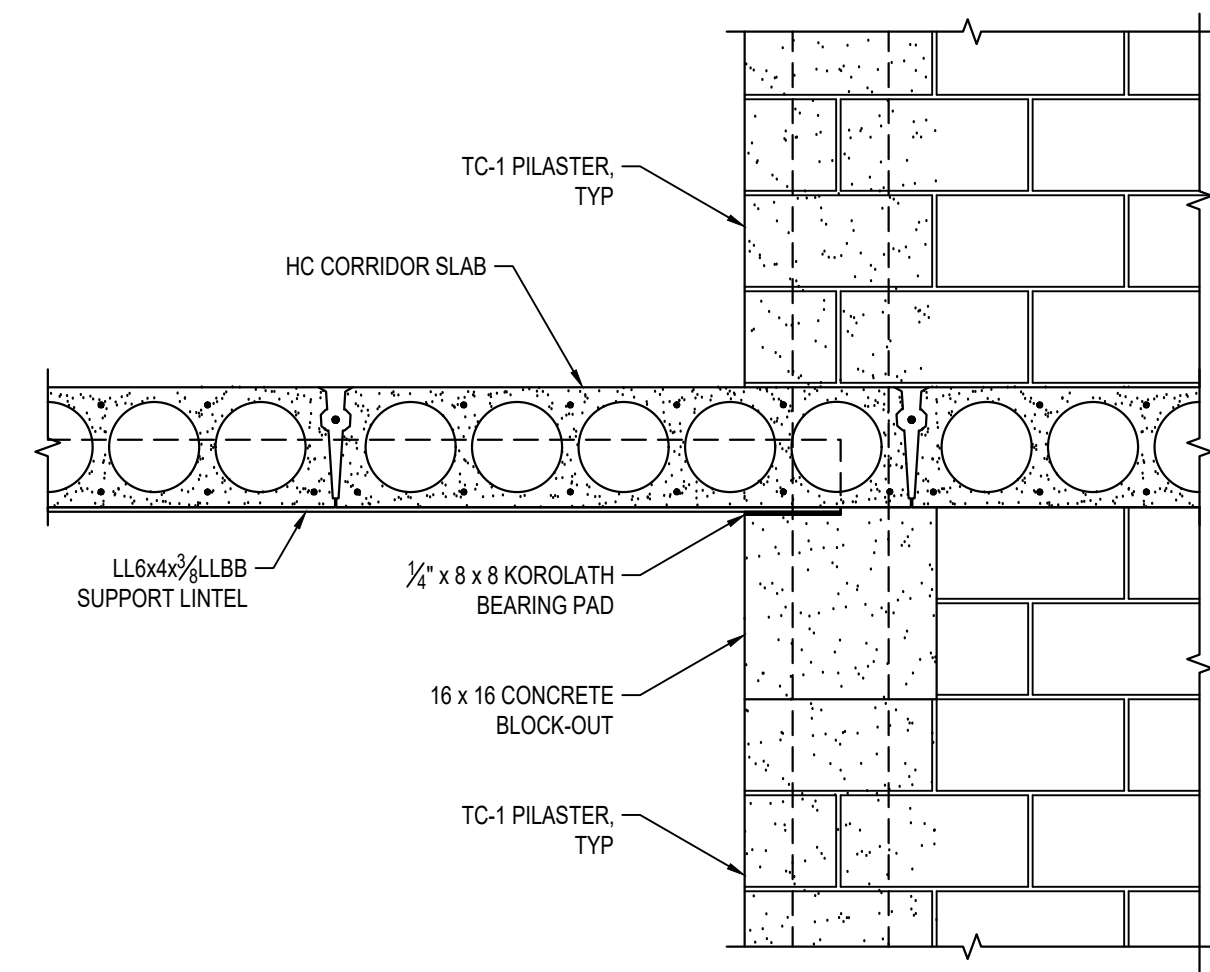
1 CONCRETE BEAM SCHEDULE

SCALE: 3/4" = 1'-0"



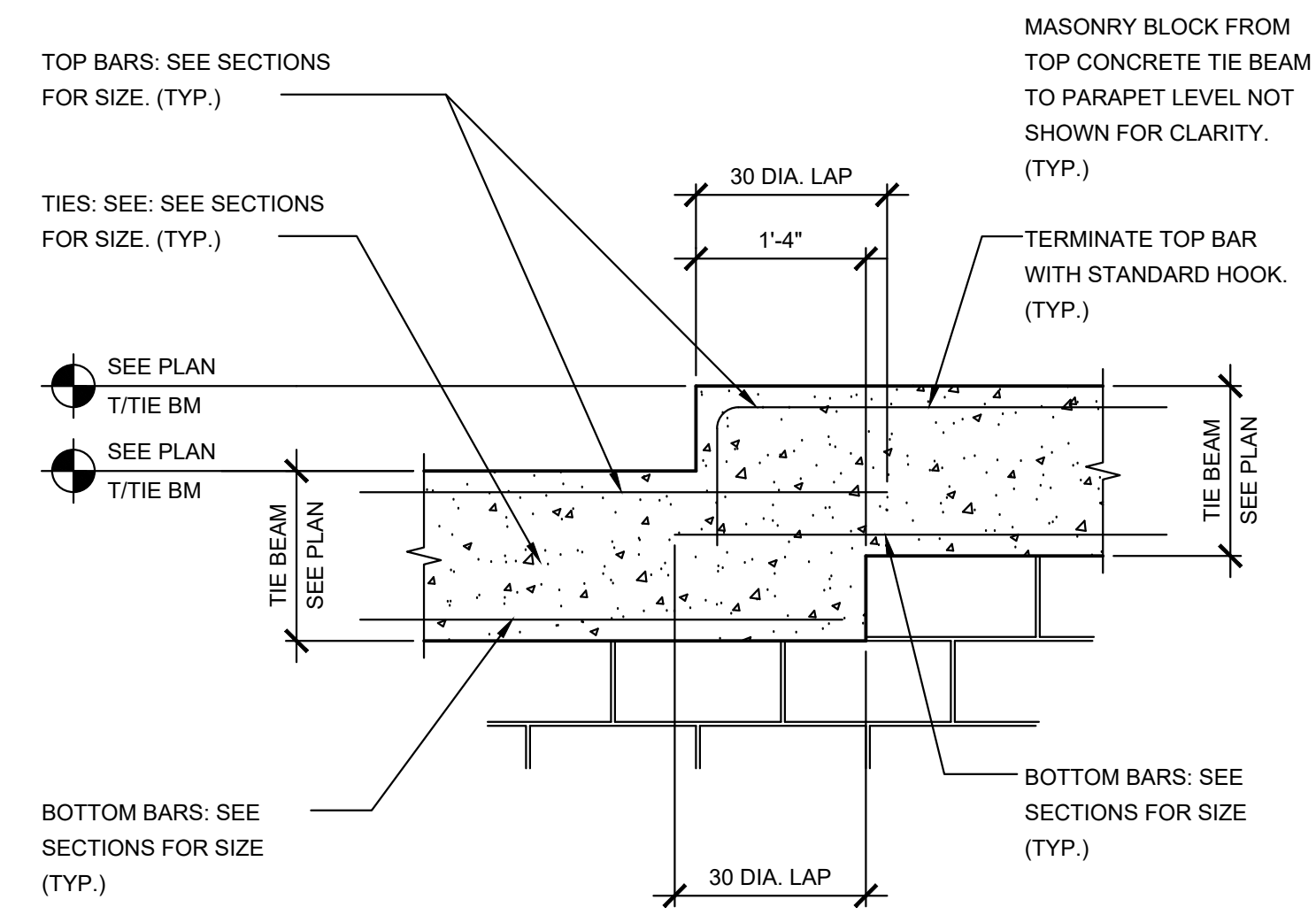
3 TYP. PRE-CAST 'U' LINTEL AT CONCRETE COLUMN.

SCALE: 3/4" = 1'-0"

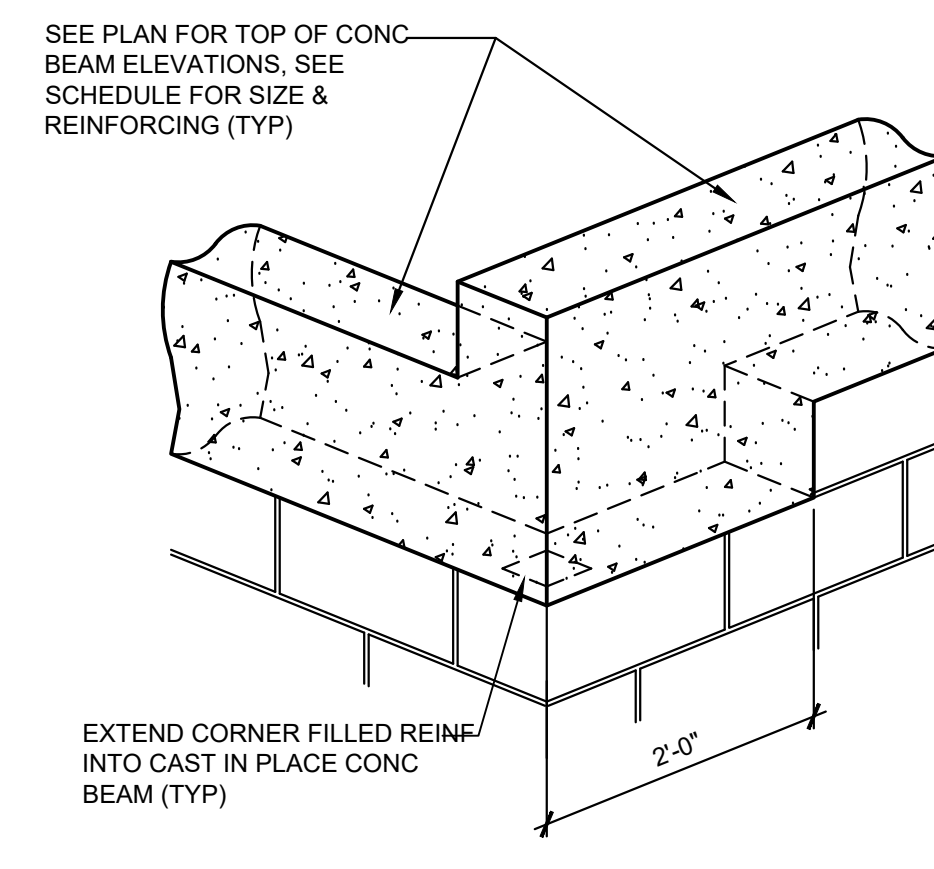


4 STEEL LINTEL AT CORRIDOR

SCALE: 3/4" = 1'-0"



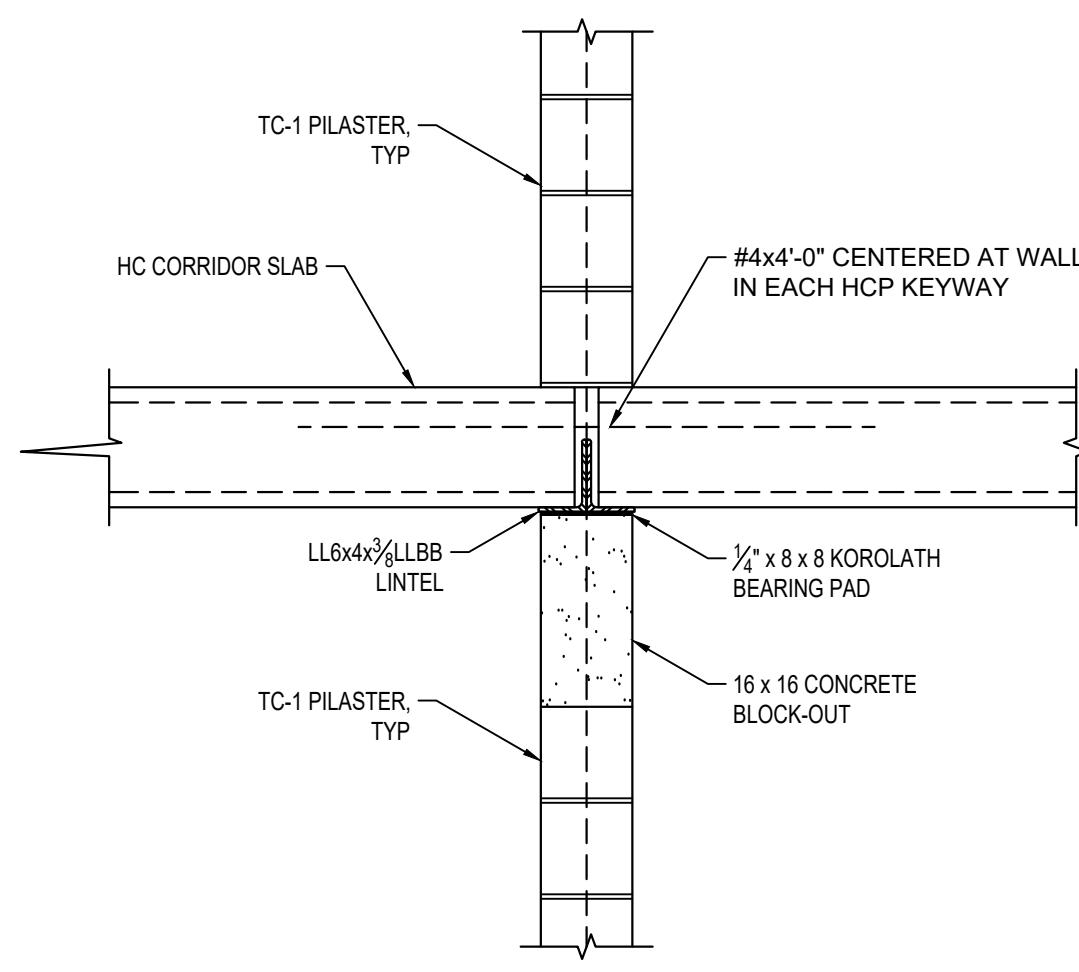
ELEVATION VIEW



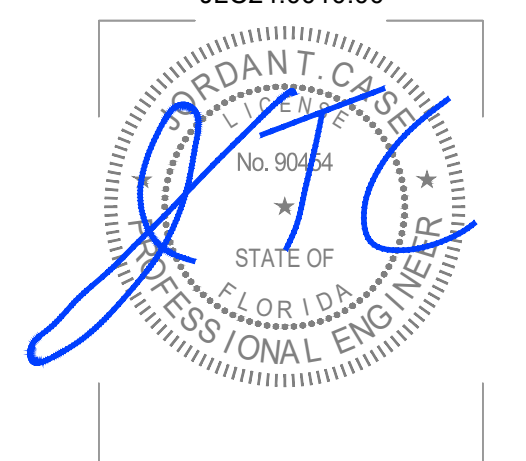
CORNER VIEW

2 STEPPED TIE BEAM DETAIL

SCALE: 3/4" = 1'-0"



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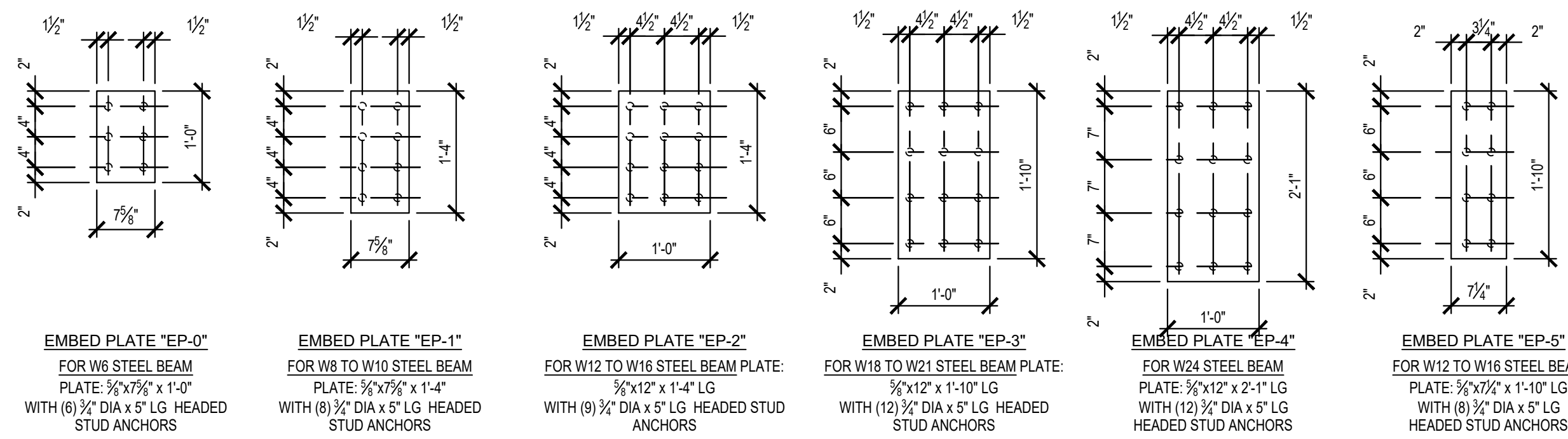
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S3.4



SINGLE PLATE - BEAM TO COLUMN (WF OR HSS) OR CONCRETE EMBED CONNECTIONS				
MIN CONNX FOR BEAM SIZES SHOWN	MAX CONNX FOR BEAM SIZES SHOWN	NUMBER OF BOLT SPACES (IN, FT-IN)	PLATE LENGTH (FT-IN)	ALLOWABLE SHEAR (KIP)
W8, W10, W12	W8, W10	1 AT 3 = 0-3	0-6	19.8
W12, W14, HSS-10	W12, W14, HSS-10	2 AT 3 = 0-6	0-9	31.8
W16, W18	W16	3 AT 3 = 0-9	1-0	42.4
W20, W21, W24	W18	4 AT 3 = 1-0	1-3	53.0
W27, W30	W21	5 AT 3 = 1-3	1-6	63.6
W33, W36	W24, W27	6 AT 3 = 1-6	1-9	74.2

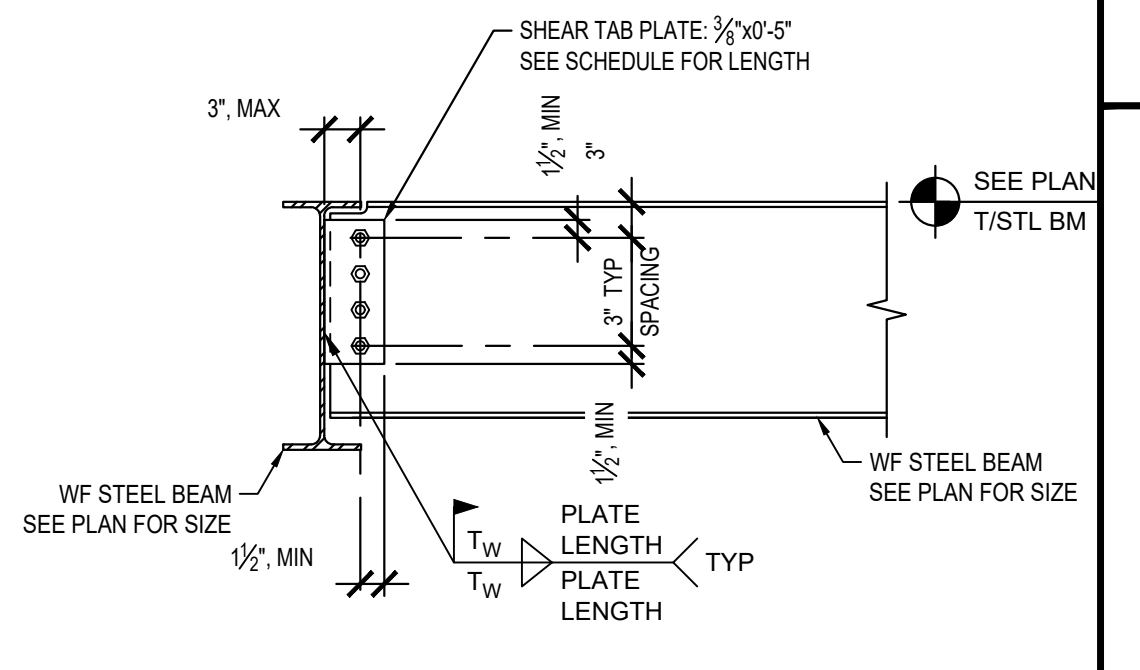
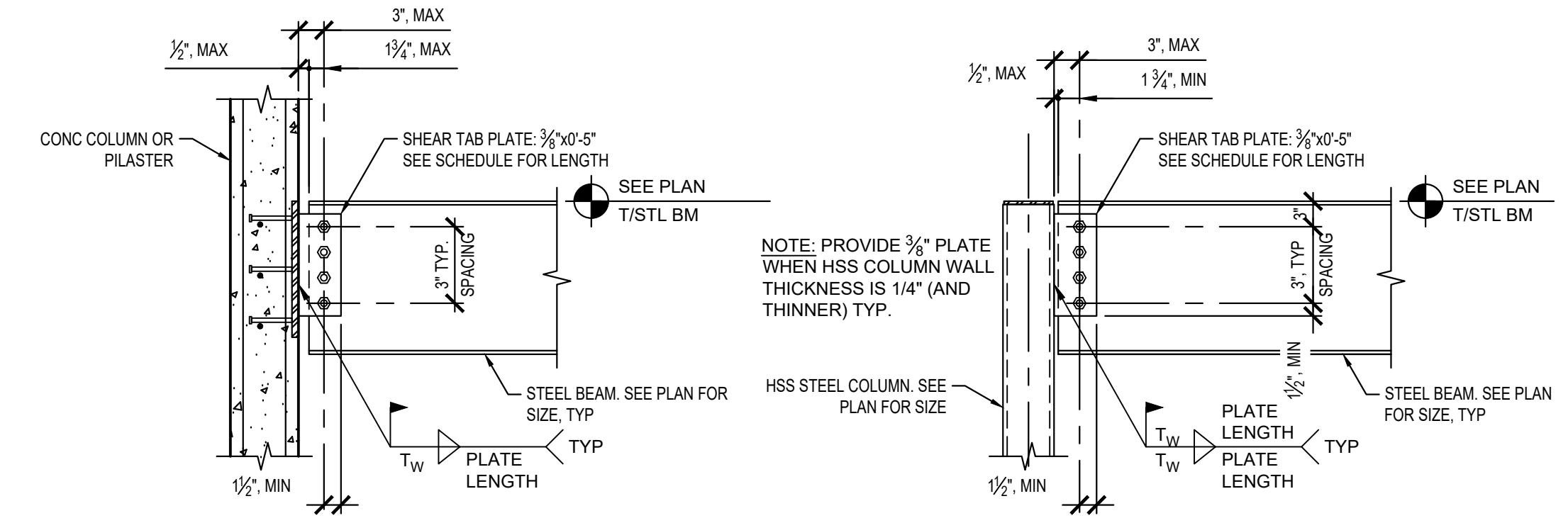
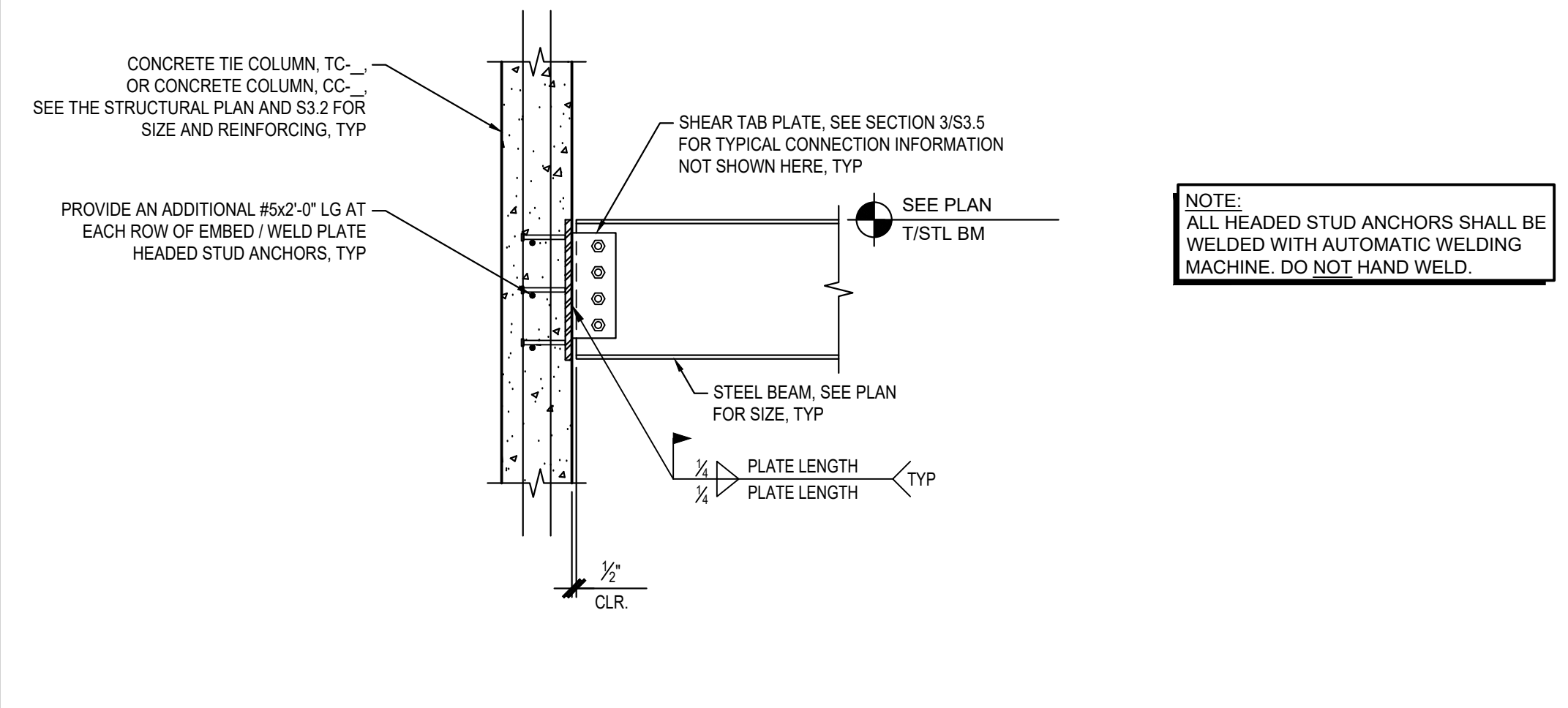
NOTES:

- CONNECTIONS USE $\frac{3}{4}$ " DIA, A325-N BOLTS.
- CONNECTIONS USE $\frac{3}{8}$ " THICK, A36 PLATES.
- SHEAR PLATES WELDED WITH $T_{\text{W}} = \frac{1}{2}$ " FILLET WELDS.
- CONNECTIONS ARE FOR SIMPLE BEAM SHEAR ONLY.
- ALLOWABLE SHEAR LOADS ARE ASD LOADS.
- PROVIDE $\frac{1}{16}$ " x 1" LONG HORIZ SLOTTED HOLES IN SHEAR PLATE.
- PROVIDE $\frac{1}{16}$ " DIA HOLES IN BEAM WEB.
- COVER ALL SLOTTED HOLES WITH $\frac{3}{16}$ " THICK PLATE WASHERS OR $\frac{5}{16}$ " THICK CONTINUOUS BAR.
- PROVIDE CONTINUOUS CONTACT BETWEEN FLAYING SURFACES.
- FINGER TIGHTEN BOLTS, AFTER COMPLETED INSTALLATION, JAM / SPOIL THREADS, TYPICAL.

SINGLE PLATE - BEAM TO BEAM CONNECTION			
BEAM SIZE	NUMBER OF BOLT SPACES (IN, FT-IN)	PLATE LENGTH (FT-IN)	ALLOWABLE SHEAR (KIP)
W6, W8, W10	1 AT 3 = 0-3	0-6	16.5
W12, W14	2 AT 3 = 0-6	0-9	28.8
W16	3 AT 3 = 0-9	1-0	41.5
W18	4 AT 3 = 1-0	1-3	54.1
W21, W24	5 AT 3 = 1-3	1-6	66.5
W27	6 AT 3 = 1-6	1-9	78.7

NOTES:

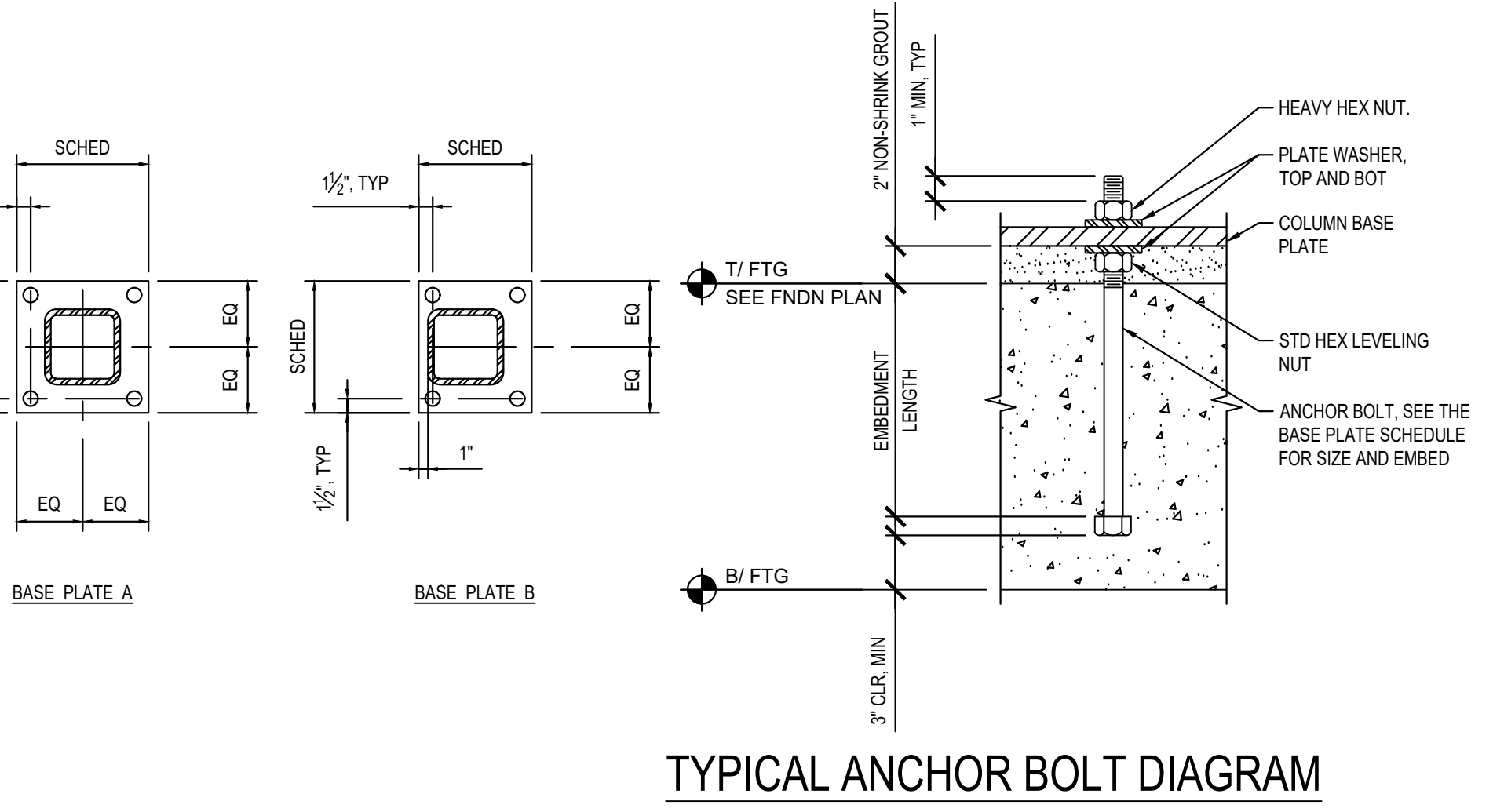
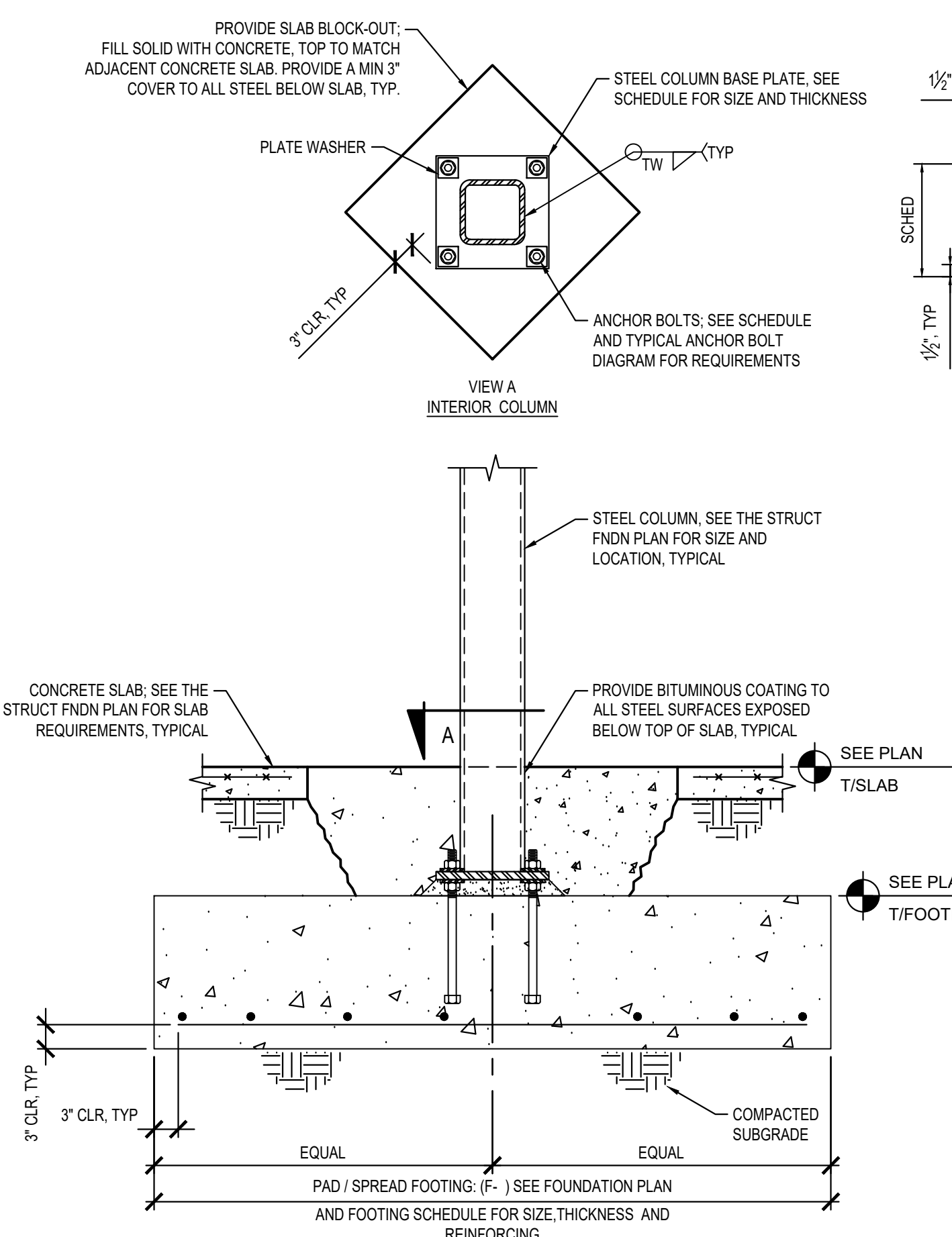
- CONNECTIONS USE $\frac{3}{4}$ " DIA, A325-N BOLTS.
- CONNECTIONS USE $\frac{3}{8}$ " THICK, A36 PLATES.
- SHEAR PLATES WELDED WITH $T_{\text{W}} = \frac{1}{2}$ " FILLET WELDS.
- CONNECTIONS ARE FOR SIMPLE BEAM SHEAR ONLY.
- ALLOWABLE SHEAR LOADS ARE ASD LOADS.
- PROVIDE $\frac{1}{16}$ " x 1" LONG HORIZ SLOTTED HOLES IN SHEAR PLATE.
- PROVIDE $\frac{1}{16}$ " DIA HOLES IN BEAM WEB.
- COVER ALL SLOTTED HOLES WITH $\frac{3}{16}$ " THICK PLATE WASHERS OR $\frac{5}{16}$ " THICK CONTINUOUS BAR.
- PROVIDE CONTINUOUS CONTACT BETWEEN FLAYING SURFACES.
- FINGER TIGHTEN BOLTS, AFTER COMPLETED INSTALLATION, JAM / SPOIL THREADS, TYPICAL.



1 TYPICAL STEEL TO CONCRETE CONNECTION
SCALE: 3/4" = 1'-0"

2 TYPICAL W-BM. TO EMBED PLATE OR HSS COLUMN SHEAR CONNECTION DETAIL
SCALE: 3/4" = 1'-0"

3 TYPICAL WF BM TO WF BM SHEAR CONNECTION DETAIL
SCALE: 3/4" = 1'-0"



TYPICAL ANCHOR BOLT DIAGRAM

BASE PLATE SCHEDULE									
COLUMN MARK	COLUMN SIZE	BASE PLATE SIZE (IN x IN x FT-IN)	ANCHOR BOLTS DIA (IN)	BASE PLATE HOLE DIA (IN)	T _W (IN)	EMBED (IN)	PLATE WASHER (IN x IN x FT-IN)	PLATE WASHER HOLE DIA (IN)	BASE PLATE
SC1	HSS8x6x $\frac{1}{2}$	1 $\frac{1}{2}$ x 14 x 1-2	$\frac{7}{8}$	1 $\frac{1}{16}$	$\frac{3}{8}$	12	$\frac{3}{8} \times 2\frac{1}{2} \times 0-2\frac{1}{2}$	1 $\frac{5}{16}$	A
SC2	HSS8x8x $\frac{1}{2}$	1 $\frac{1}{2}$ x 12 x 1-2	$\frac{7}{8}$	1 $\frac{1}{16}$	$\frac{3}{8}$	12	$\frac{3}{8} \times 2\frac{1}{2} \times 0-2\frac{1}{2}$	1 $\frac{5}{16}$	B
SC3	HSS8x6x $\frac{1}{2}$	1 $\frac{1}{2}$ x 10 x 1-2	$\frac{7}{8}$	1 $\frac{1}{16}$	$\frac{3}{8}$	12	$\frac{3}{8} \times 2\frac{1}{2} \times 0-2\frac{1}{2}$	1 $\frac{5}{16}$	B
SC4	HSS8x6x $\frac{1}{2}$	1 $\frac{1}{2}$ x 14 x 1-2	$\frac{7}{8}$	1 $\frac{1}{16}$	$\frac{3}{8}$	12	$\frac{3}{8} \times 2\frac{1}{2} \times 0-2\frac{1}{2}$	1 $\frac{5}{16}$	A
SC5	HSS8x6x $\frac{1}{2}$	1 $\frac{1}{2}$ x 12 x 1-2	$\frac{7}{8}$	1 $\frac{1}{16}$	$\frac{3}{8}$	12	$\frac{3}{8} \times 2\frac{1}{2} \times 0-2\frac{1}{2}$	1 $\frac{5}{16}$	B

NOTES:

- BASE PLATES SHALL BE F_y = 36 KSI, UNO.
- ANCHOR RODS SHALL BE ASTM 1554, GRADE 36, UNO.
- IN LIEU OF ANCHOR BOLTS, A THREADED ROD, OF EQUAL STRENGTH, MAY BE SUBSTITUTED WITH A HEAVY HEX NUT TACK WELDED TO THE ANCHOR ROD.
- ALL BOLTS SHALL BE, UNLESS NOTED OTHERWISE, TIGHTENED TO A "SNUG TIGHT" CONDITION AS DEFINED BY AISC AFTER THE CONCRETE IS AT LEAST 14 DAYS OLD.
- ALL ANCHOR BOLTS AND WASHERS SHALL BE HOT DIPPED GALVANIZED.
- ANCHOR BOLT MINIMUM PROJECTION SHALL ACCOMMODATE GROUT THICKNESS, BASE PLATE THICKNESS, WASHERS AND NUTS, PLUS 1", TYPICAL.

4 INTERIOR STEEL COLUMN AT FOUNDATION
SCALE: 3/4" = 1'-0"

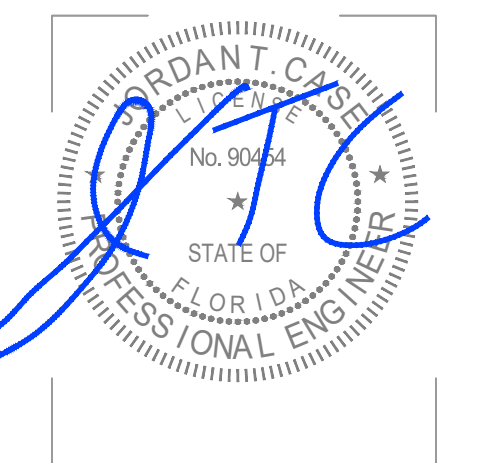
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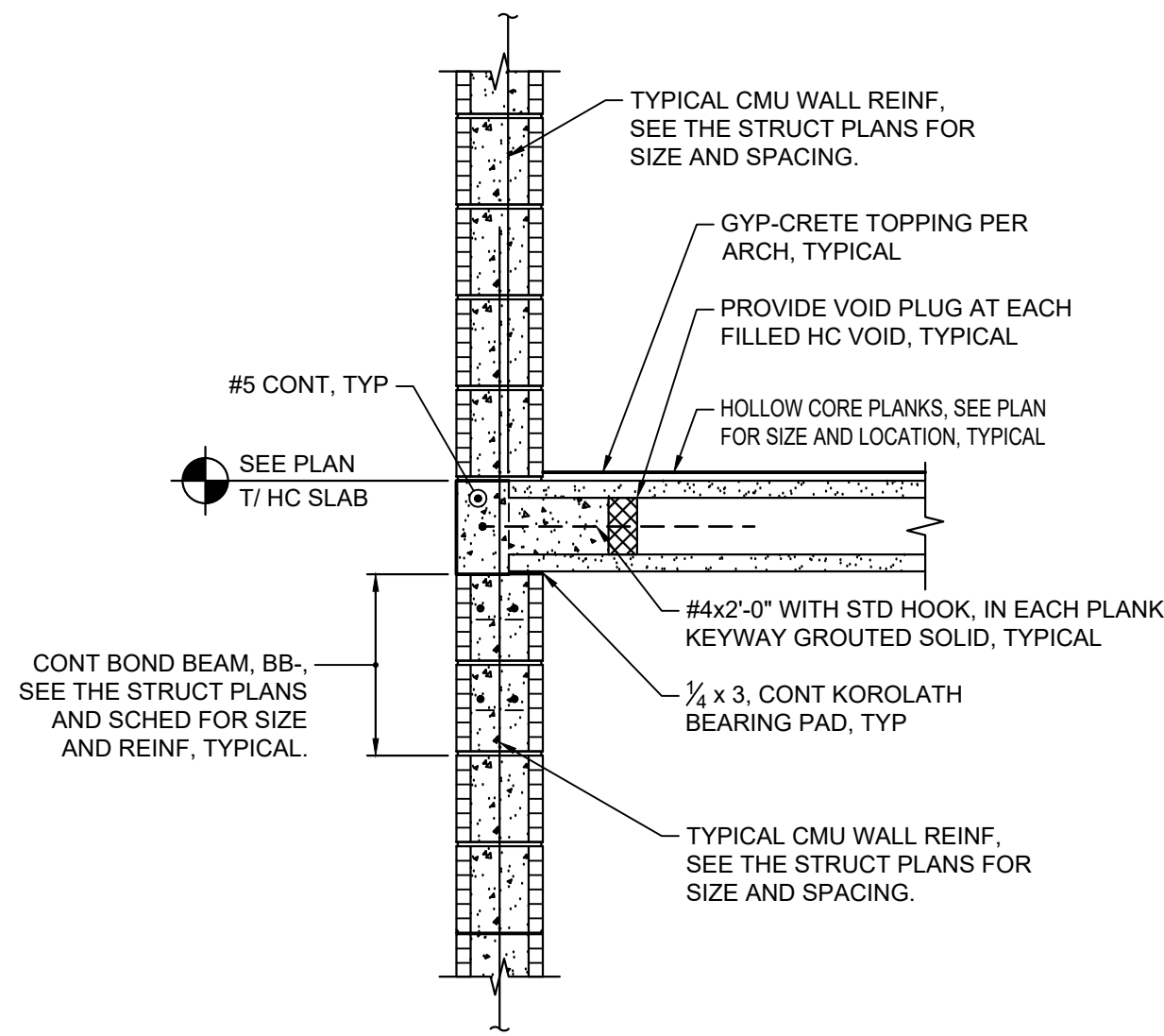


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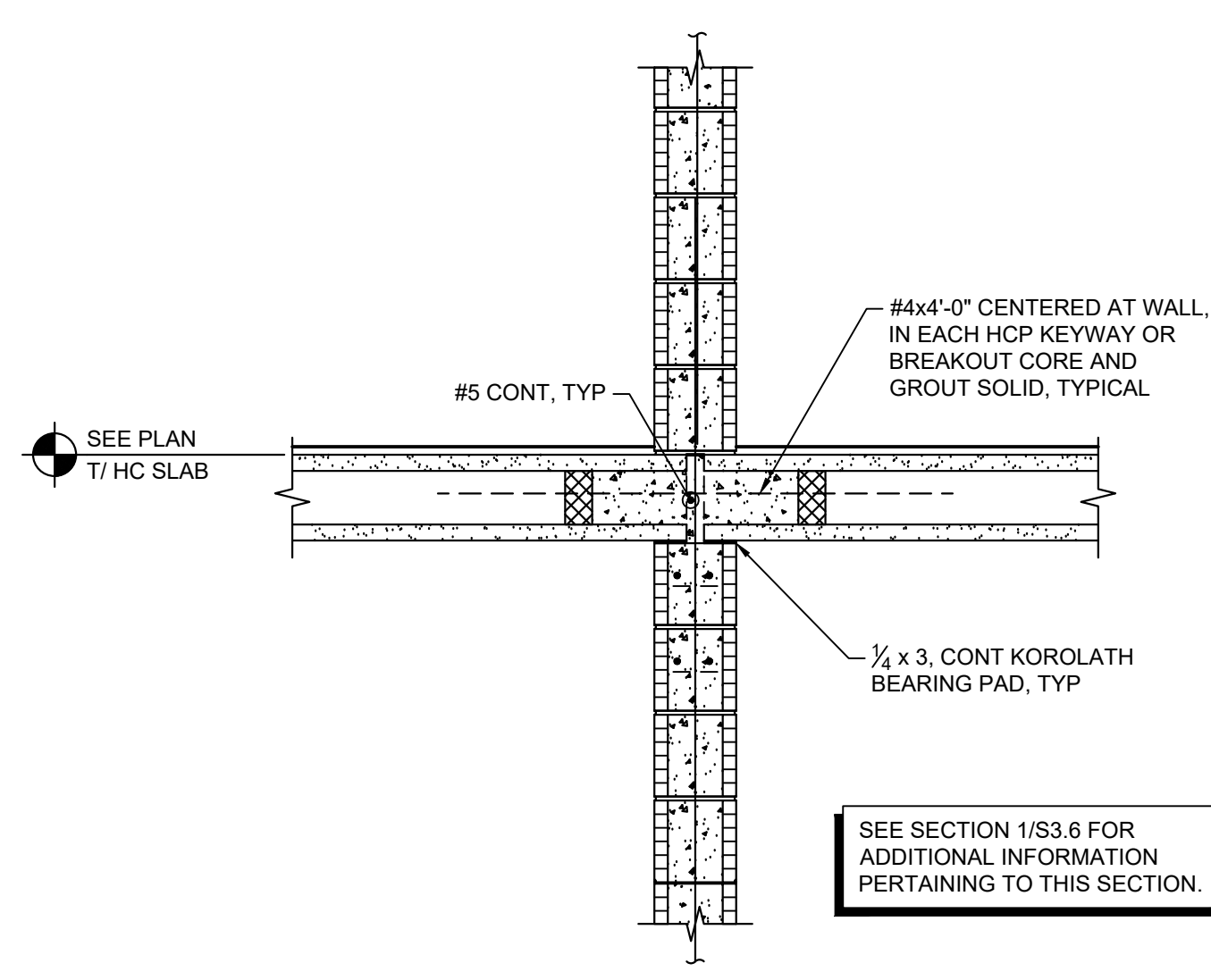
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S3.5



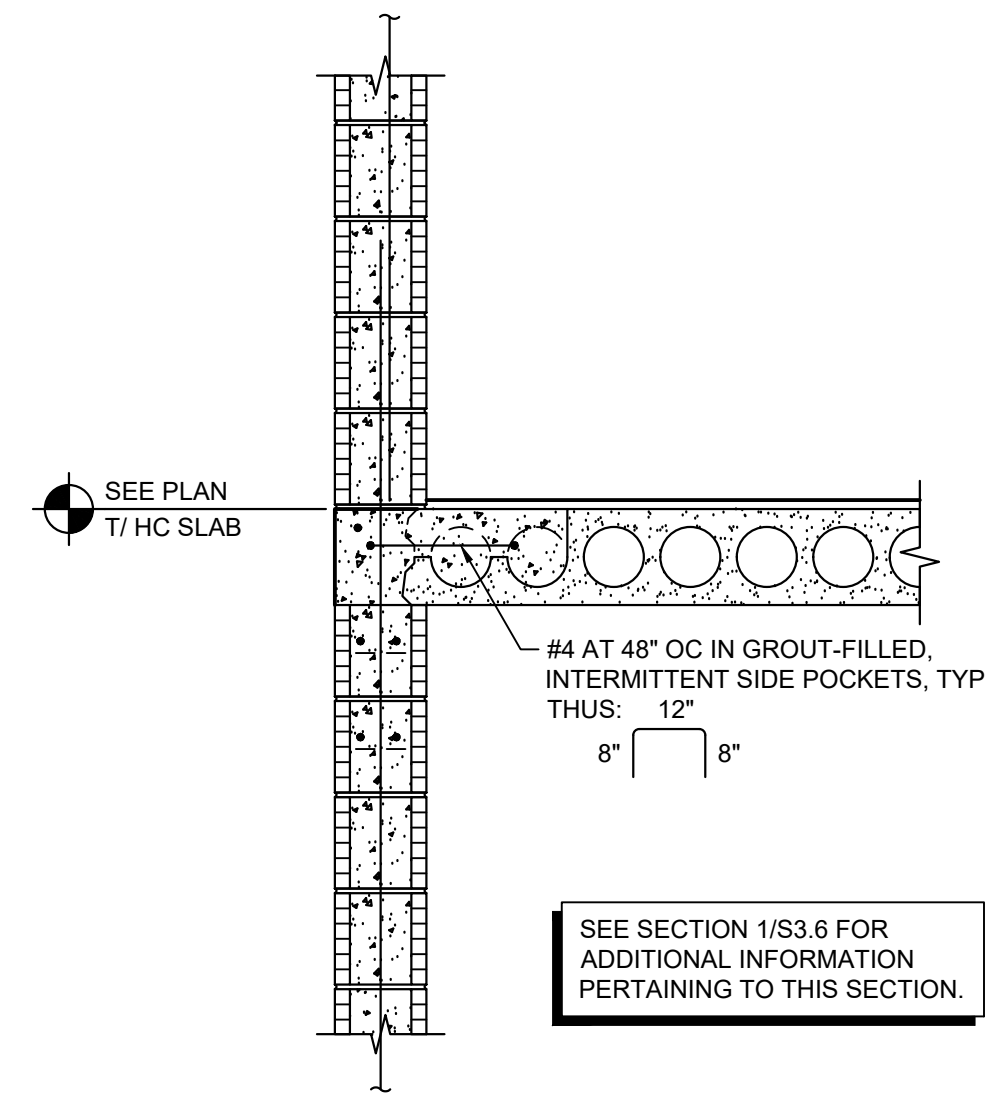
1 TYPICAL HOLLOW CORE PLANK
END BEARING AT CMU WALL

SCALE: 3/4" = 1'-0"



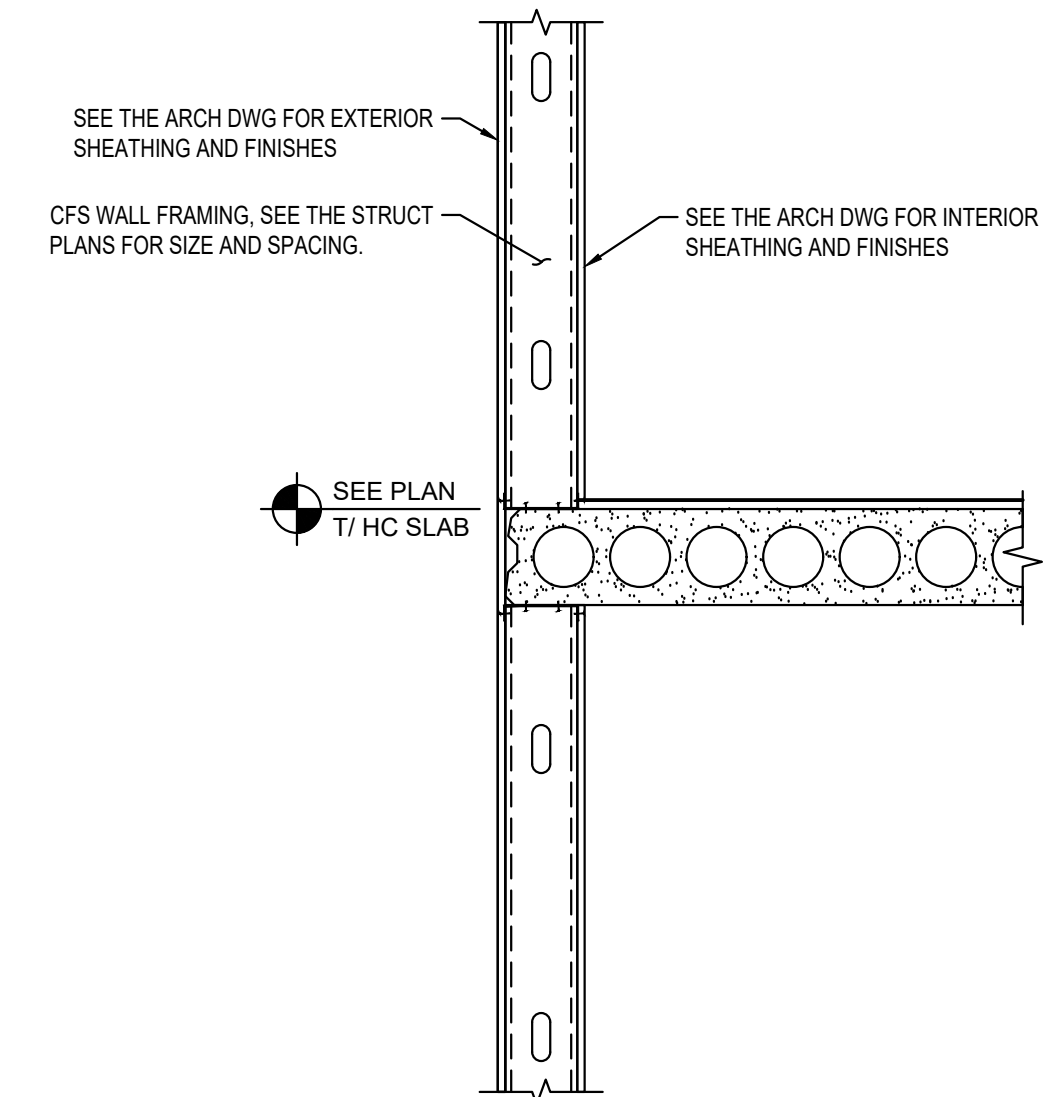
2 TYPICAL HOLLOW CORE PLANK
END BEARING AT CMU WALL

SCALE: 3/4" = 1'-0"



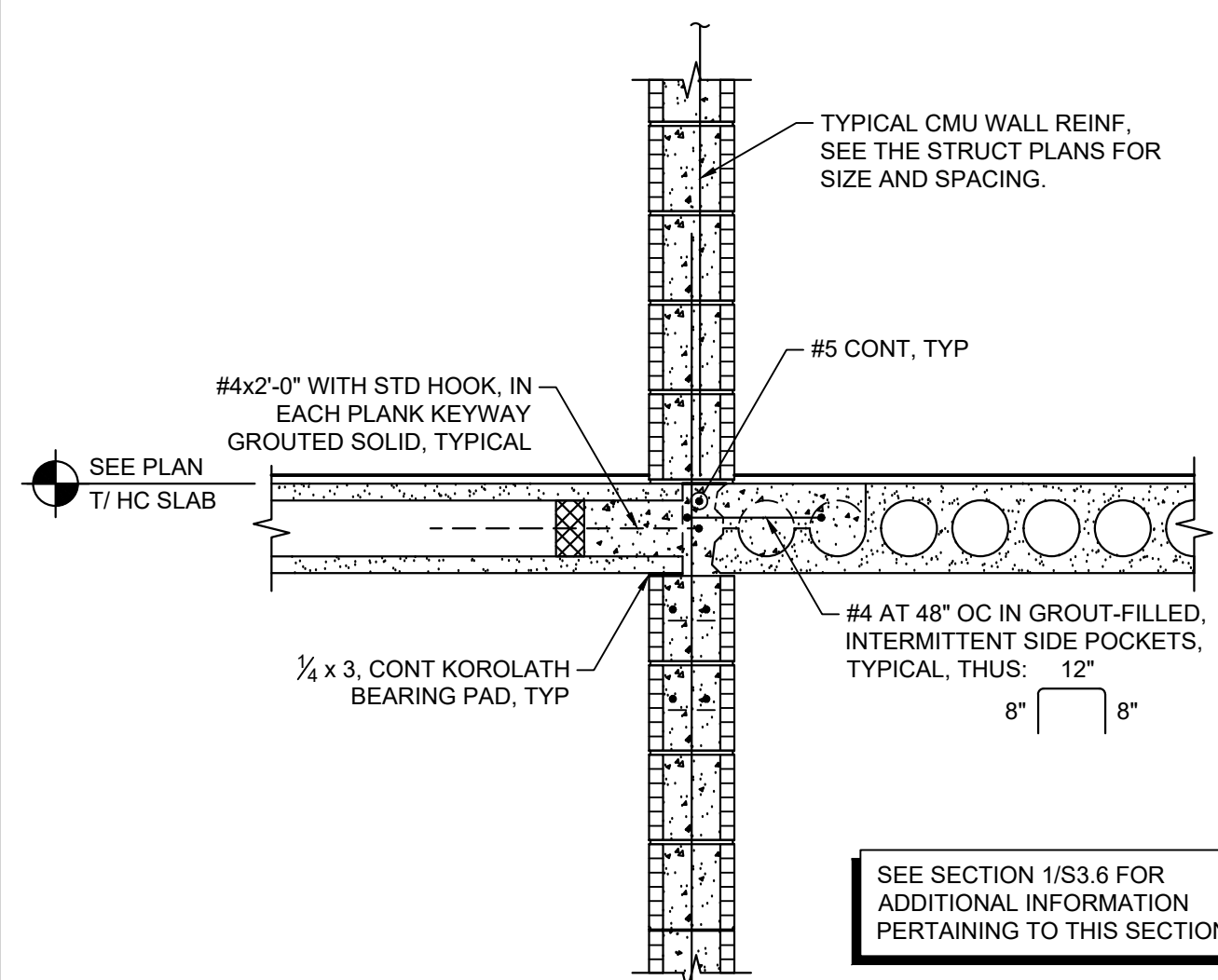
3 TYPICAL HOLLOW CORE PLANK
SIDE BEARING AT CMU WALL

SCALE: 3/4" = 1'-0"



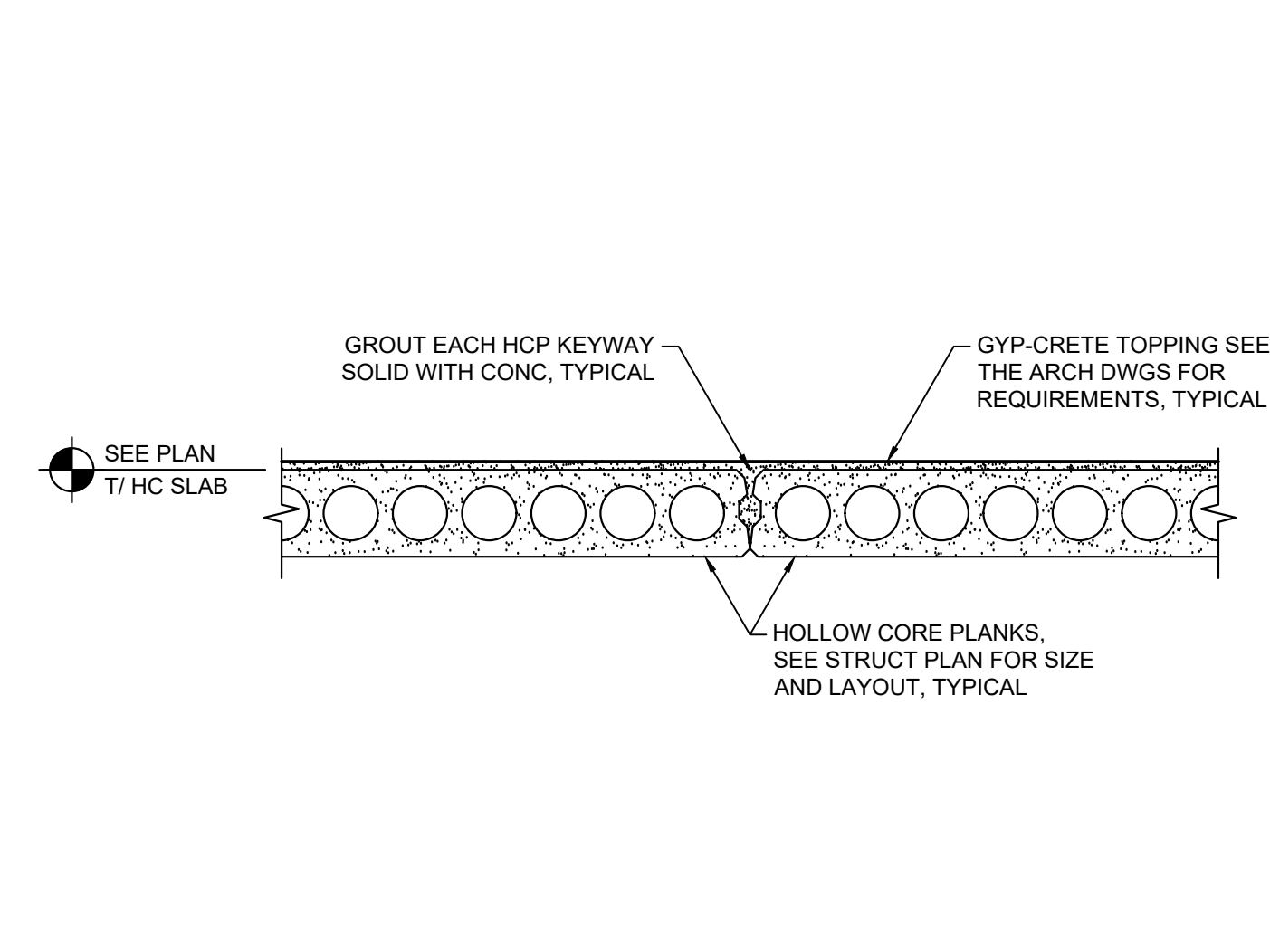
4 CFS WALL DETAIL

SCALE: 3/4" = 1'-0"



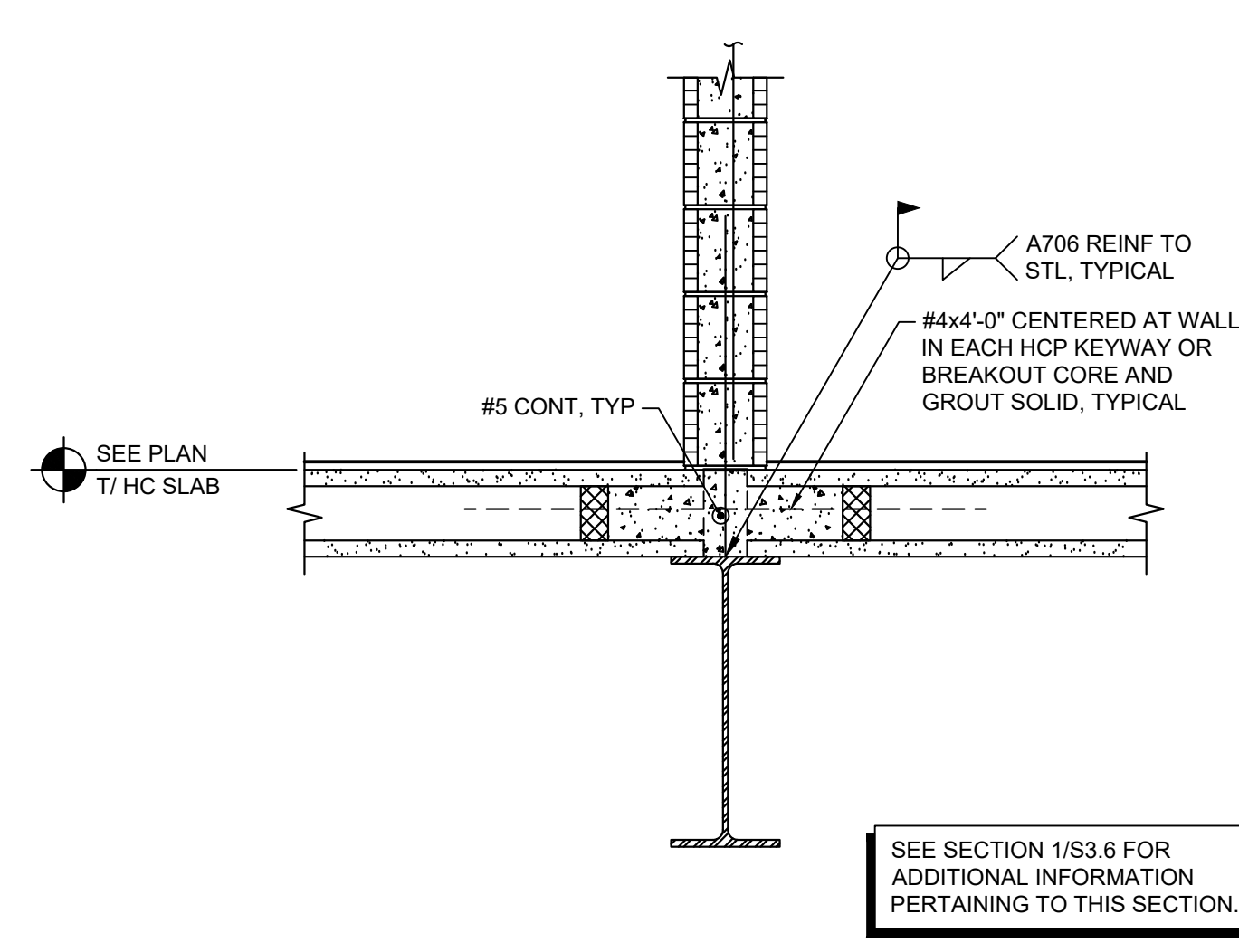
5 TYPICAL HOLLOW CORE PLANK
END / SIDE BEARING AT CMU WALL

SCALE: 3/4" = 1'-0"



6 TYPICAL HOLLOW CORE PLANK
TO PLANK, GROUDED FILLED, KEYWAY

SCALE: 3/4" = 1'-0"



7 TYPICAL HOLLOW CORE PLANK
END BEARING ON W-BEAM

SCALE: 3/4" = 1'-0"

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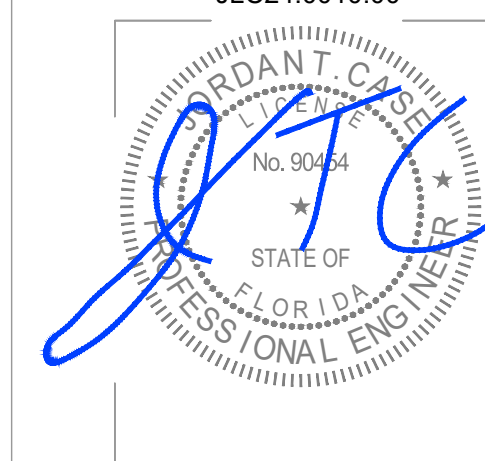
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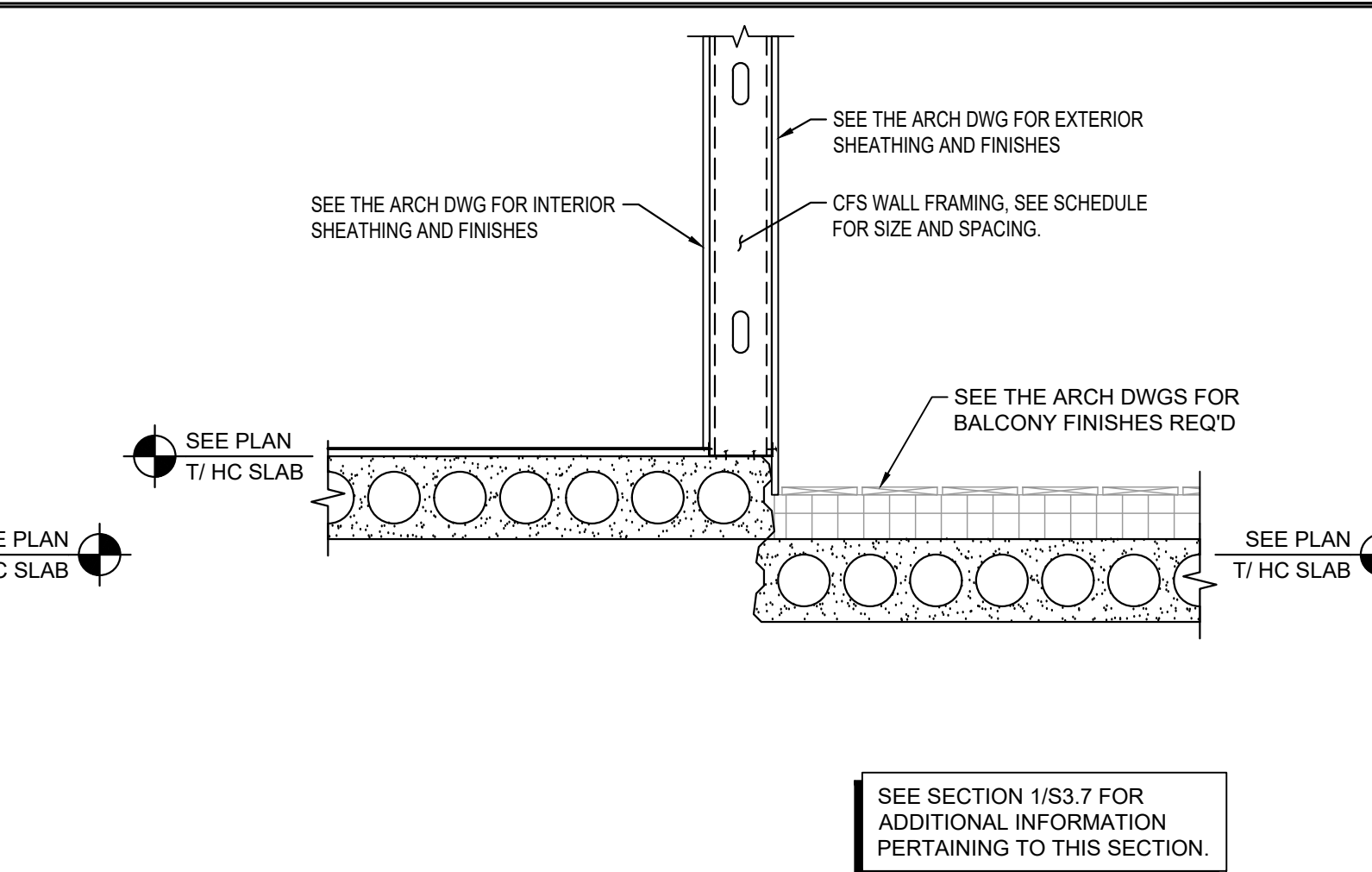
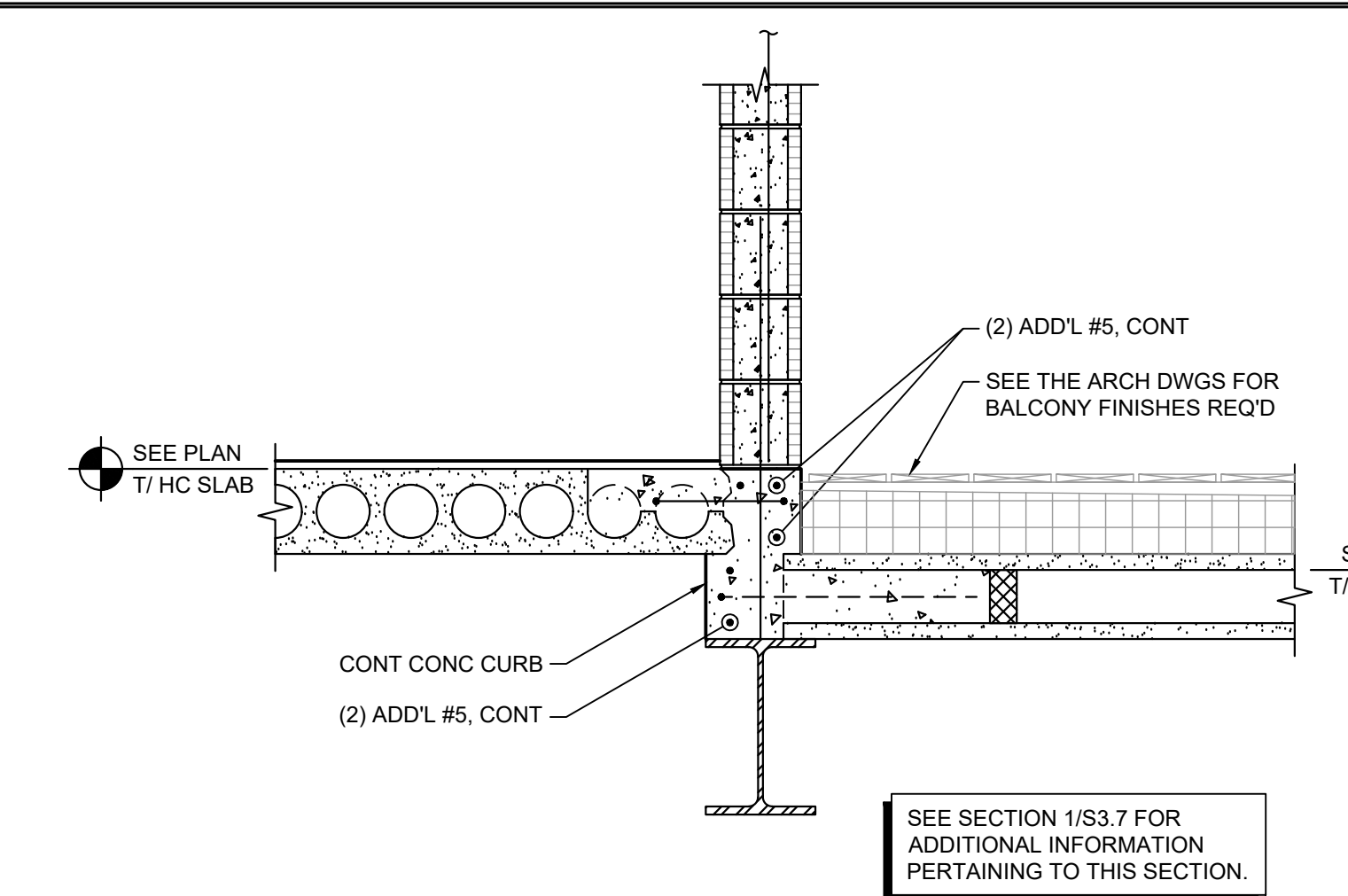
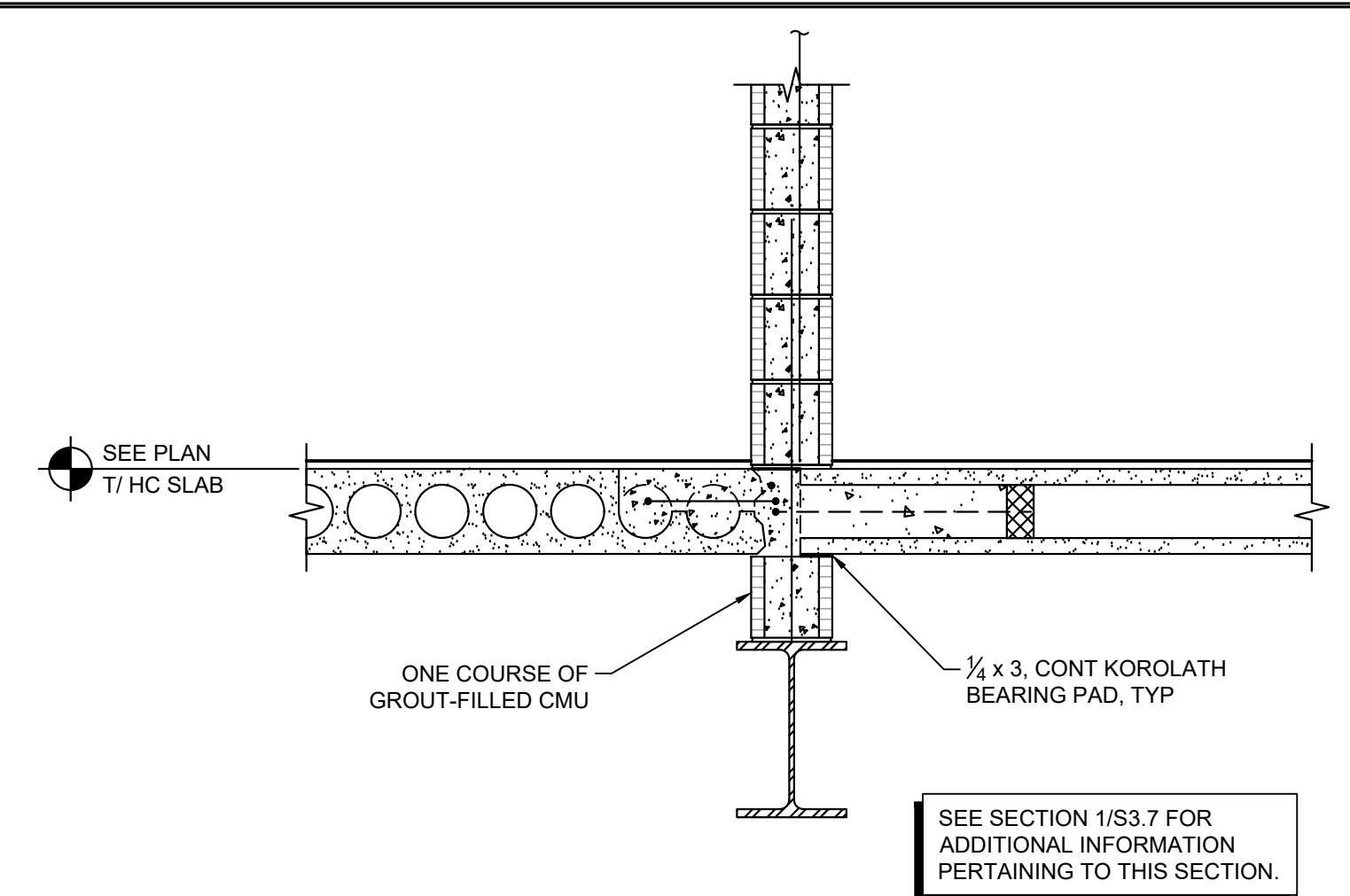
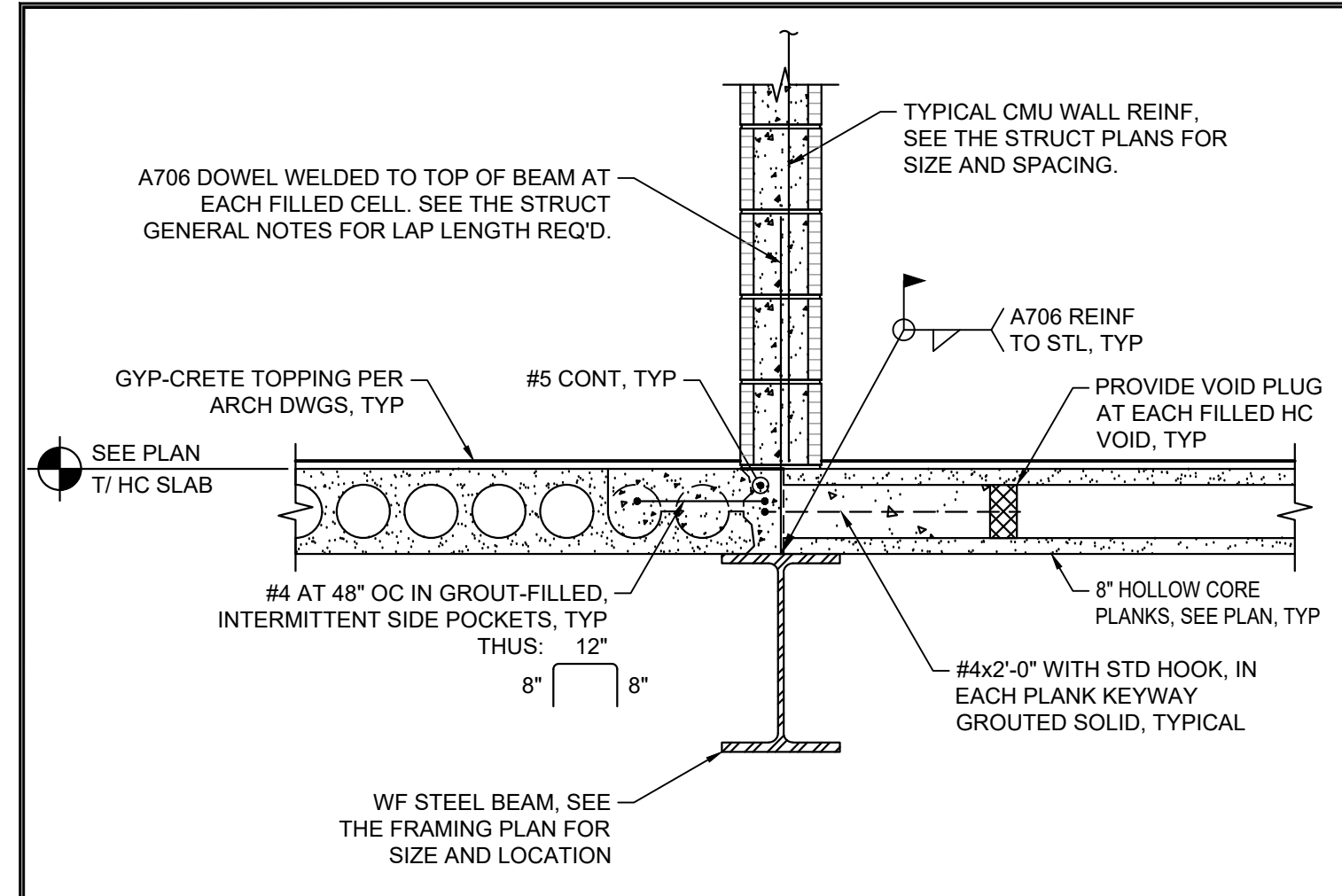
S3.6



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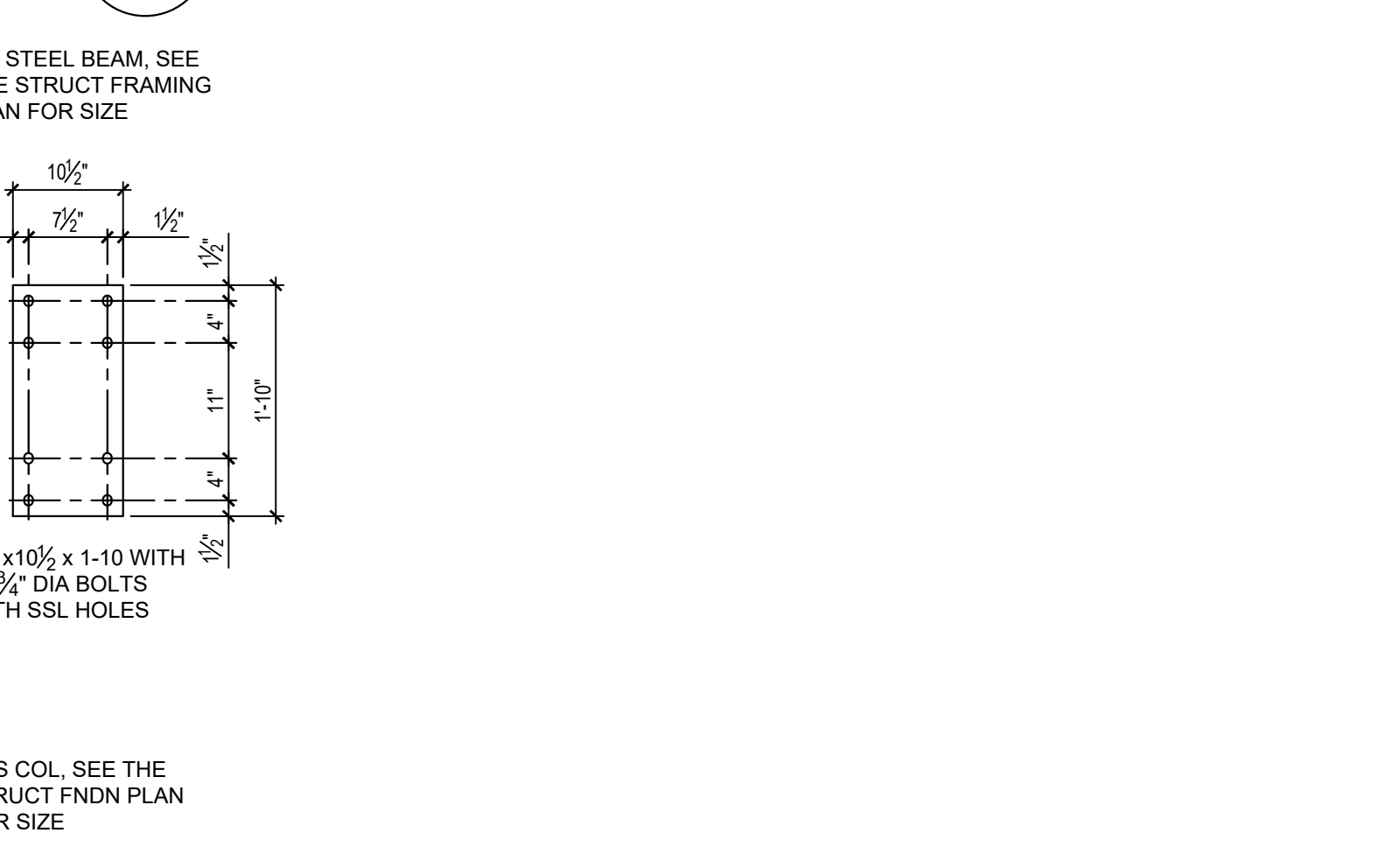
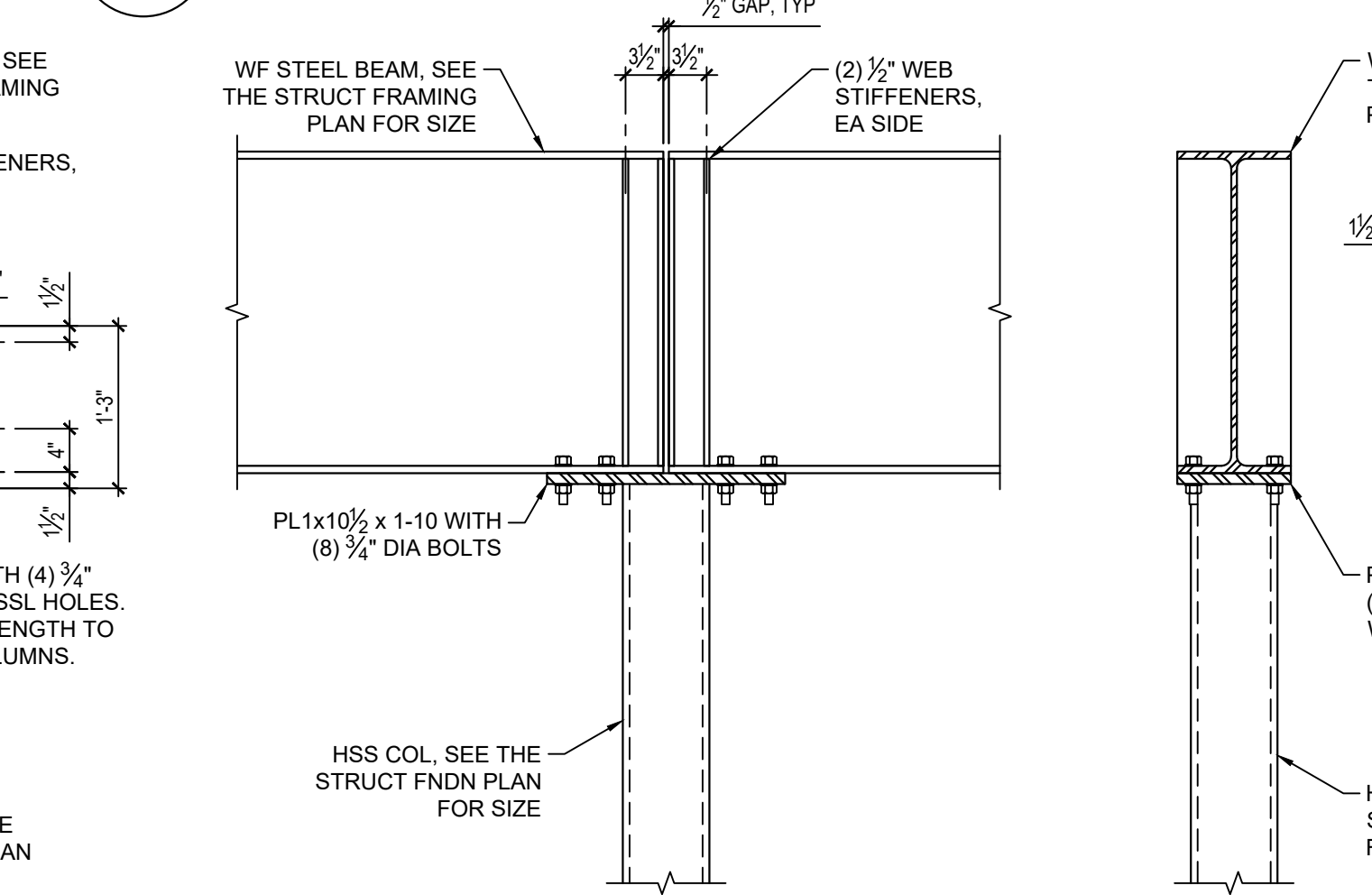
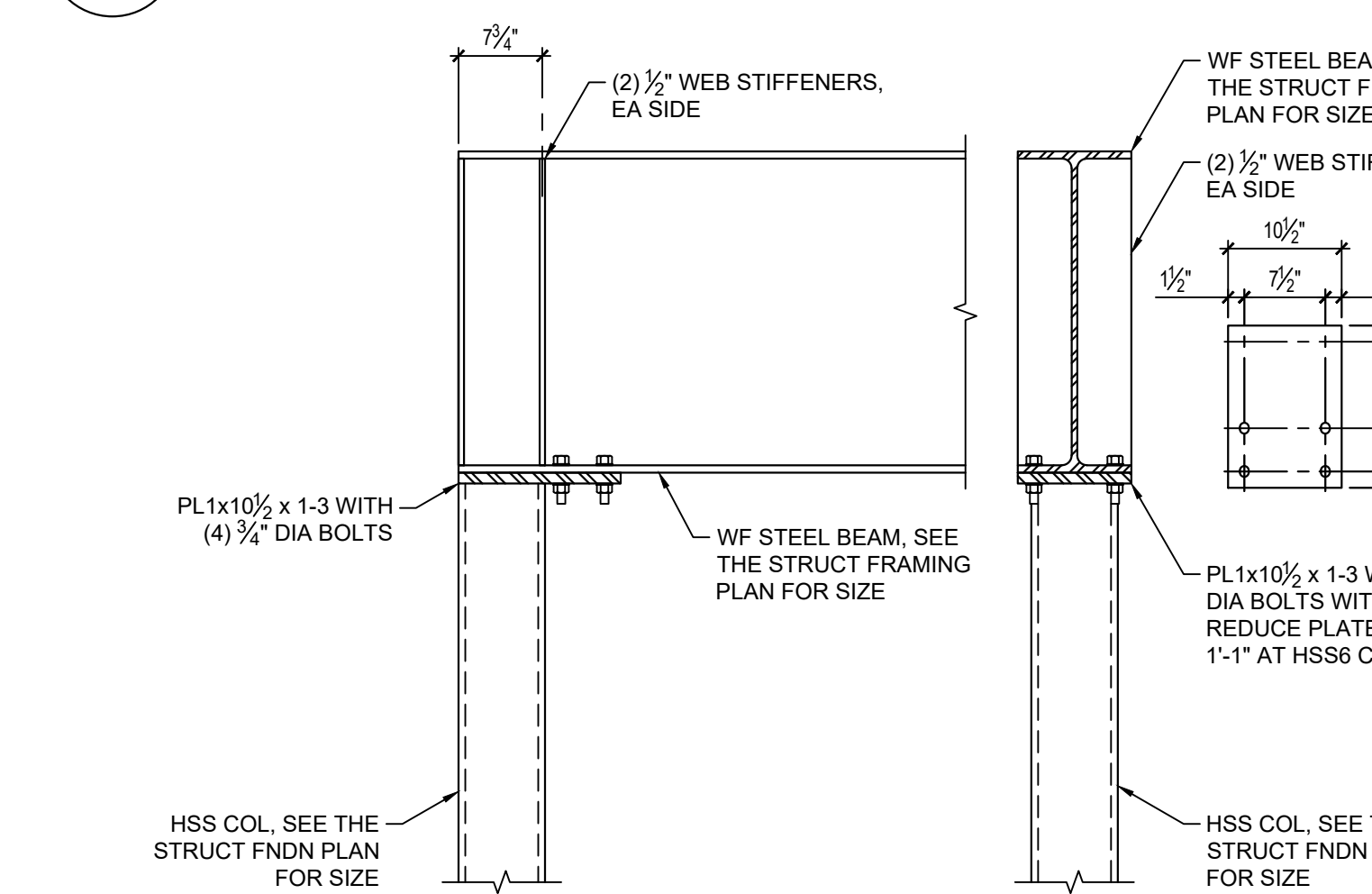
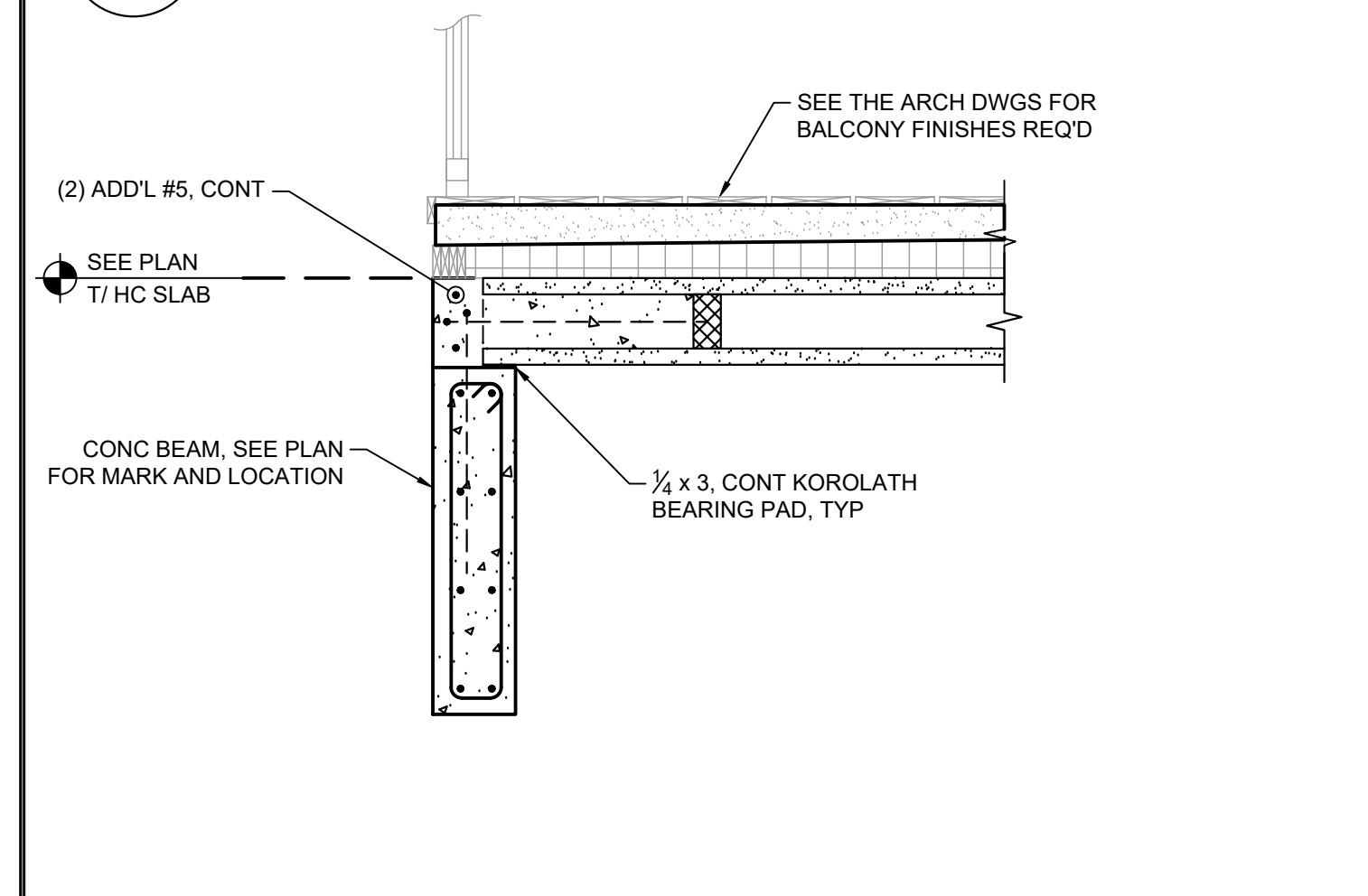


1 SECTION
SCALE: 3/4" = 1'-0"

2 SECTION
SCALE: 3/4" = 1'-0"

3 SECTION
SCALE: 3/4" = 1'-0"

4 SECTION
SCALE: 3/4" = 1'-0"



5 SECTION
SCALE: 3/4" = 1'-0"

6 SECTION
SCALE: 3/4" = 1'-0"

7 SECTION
SCALE: 3/4" = 1'-0"

8 SECTION
SCALE: 3/4" = 1'-0"

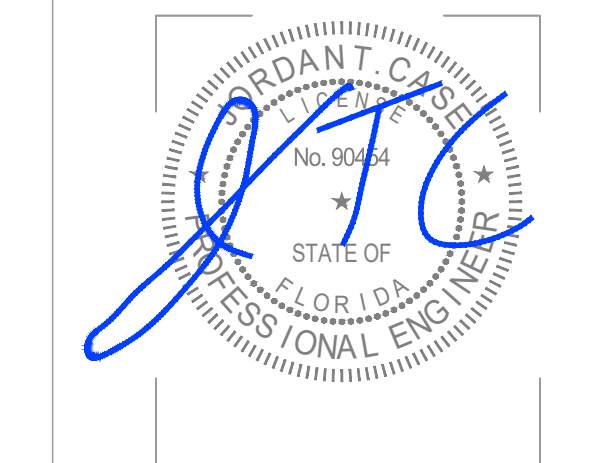
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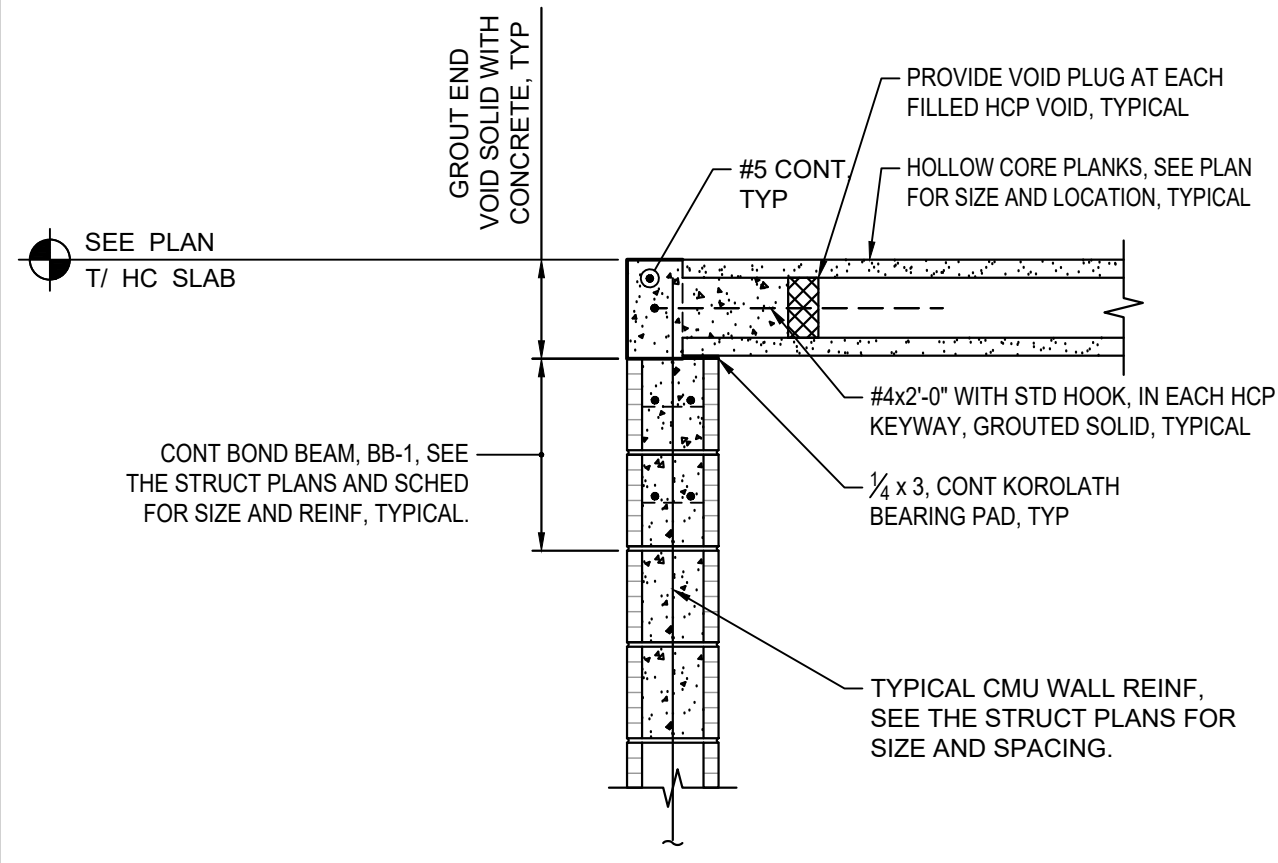
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RAJ PATEL
STUART, FLORIDA



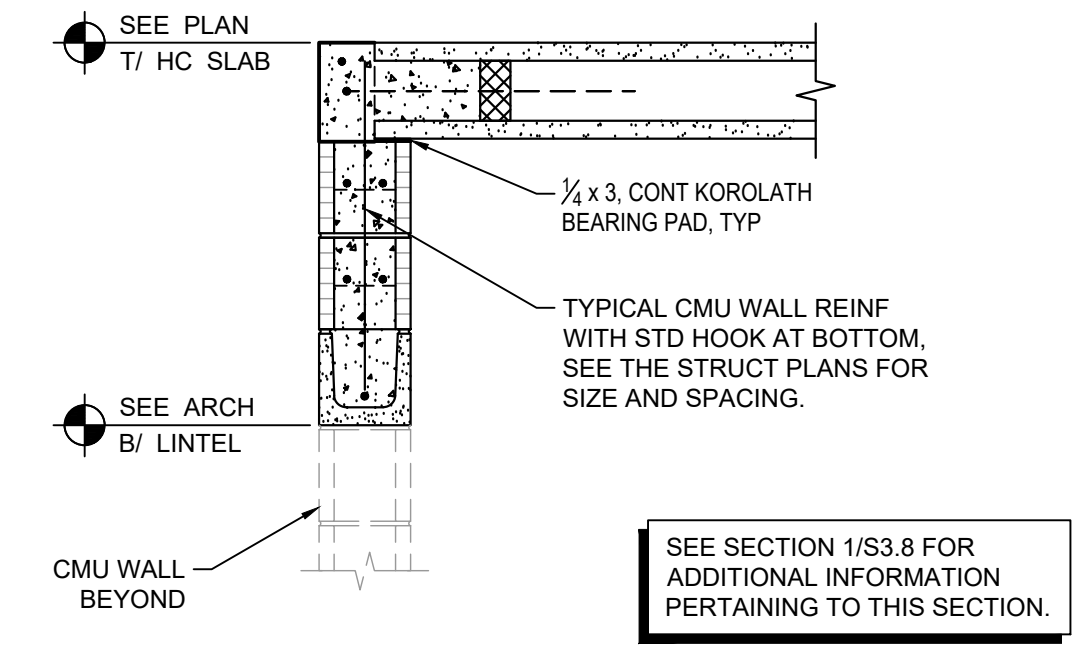
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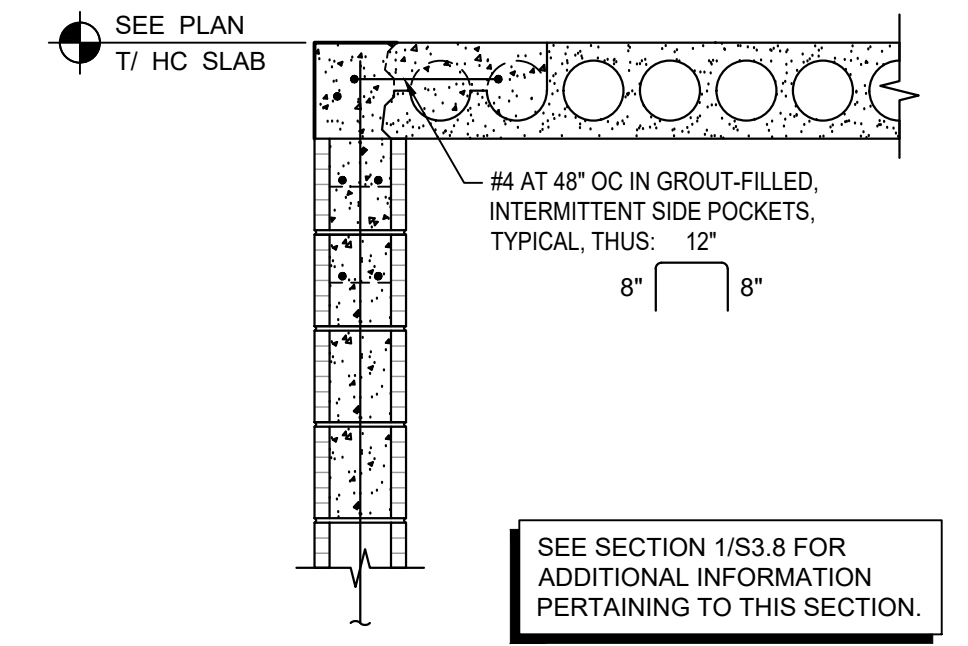
REVISION
JOB NO.
22039
DATE
07-11-2024
SHEET
S3.7



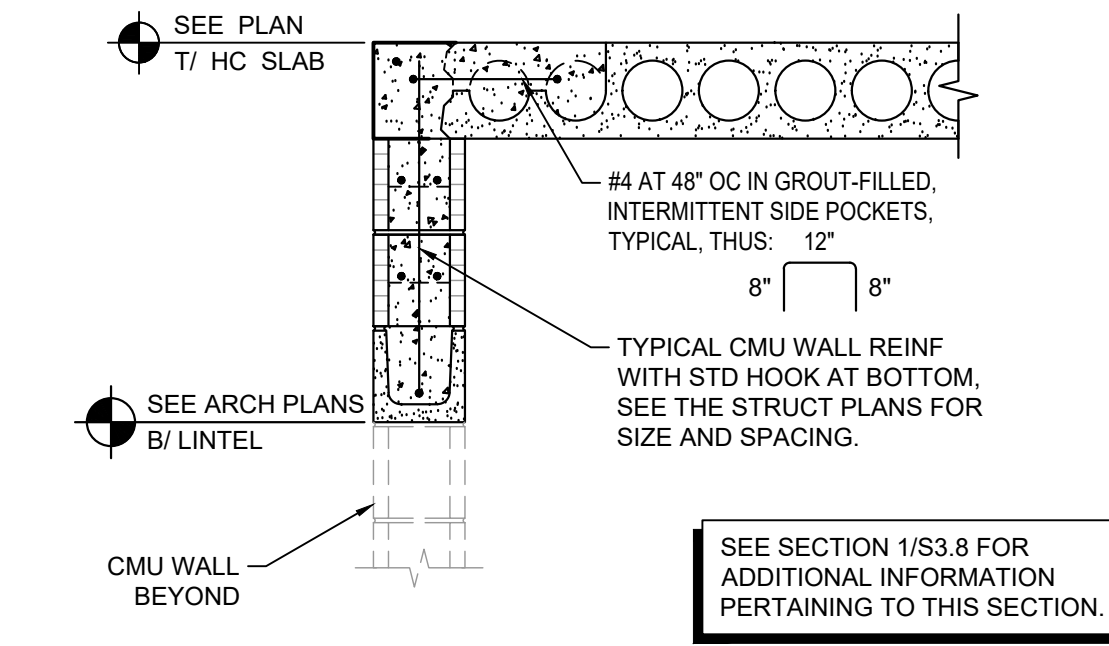
1 TYPICAL HOLLOW CORE PLANK END BEARING AT CMU WALL
SCALE: 3/4" = 1'-0"



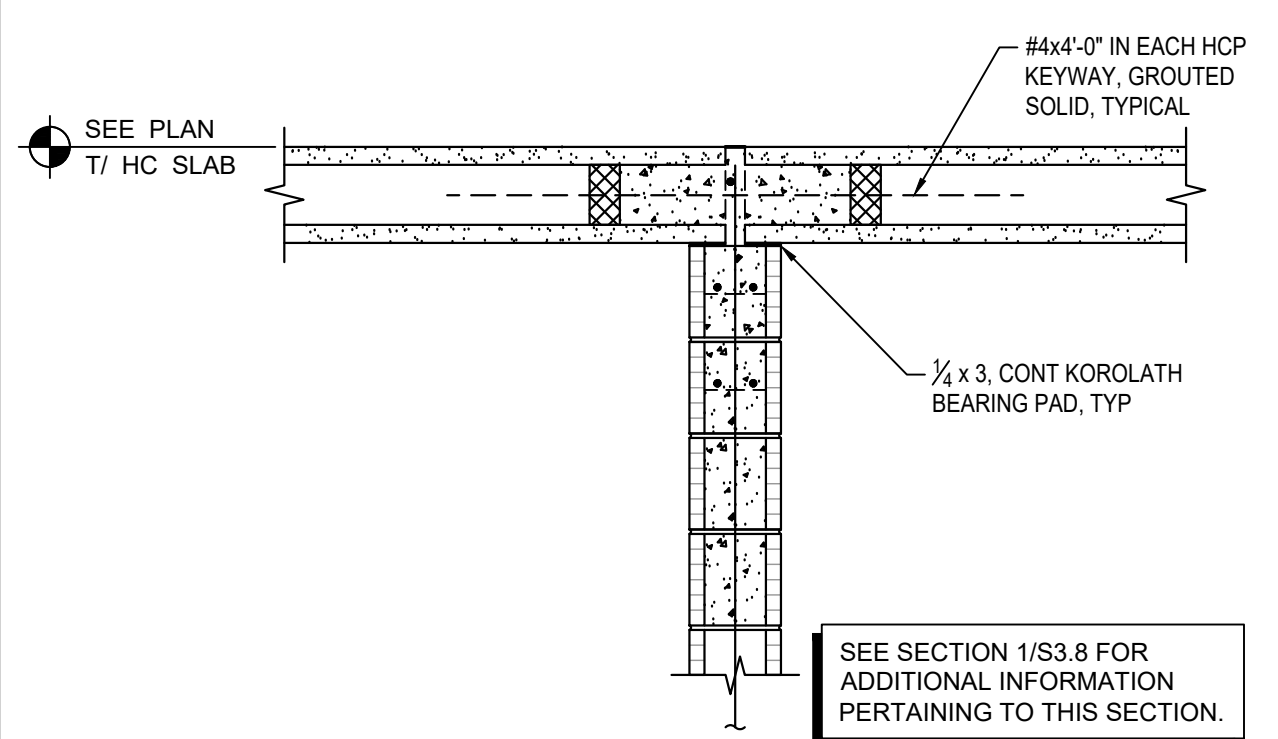
2 TYPICAL HOLLOW CORE PLANK END BEARING AT CMU WALL
SCALE: 3/4" = 1'-0"



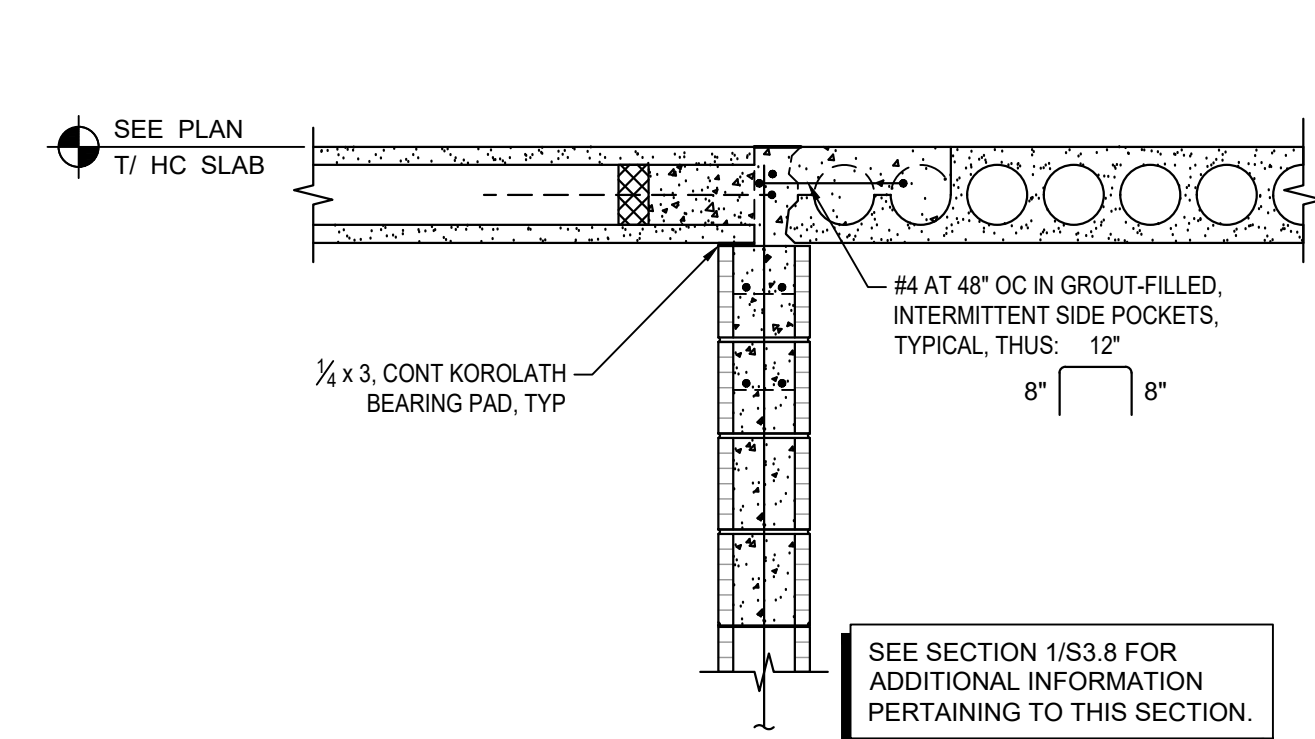
3 TYPICAL HOLLOW CORE PLANK SIDE BEARING AT CMU WALL
SCALE: 3/4" = 1'-0"



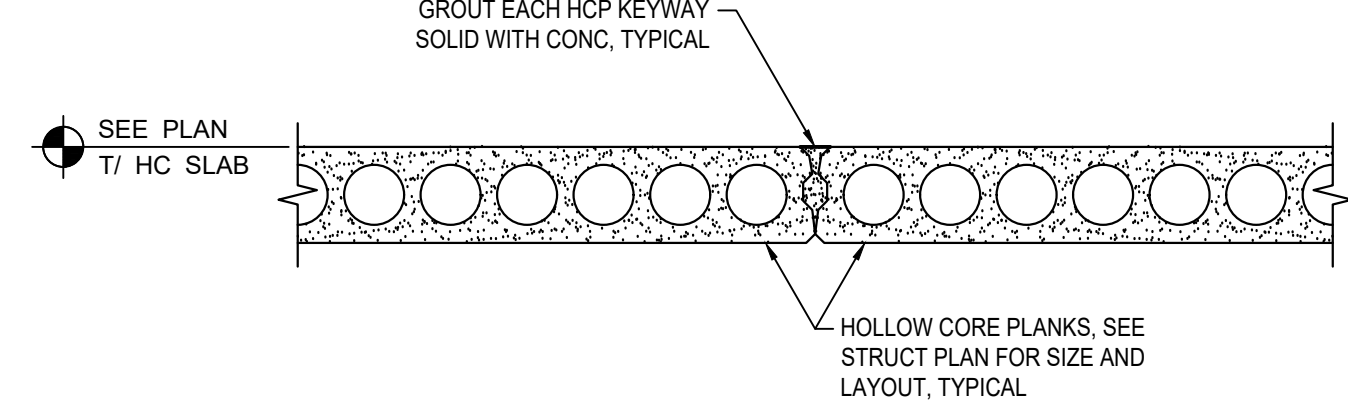
4 TYPICAL HOLLOW CORE PLANK SIDE BEARING AT CMU WALL
SCALE: 3/4" = 1'-0"



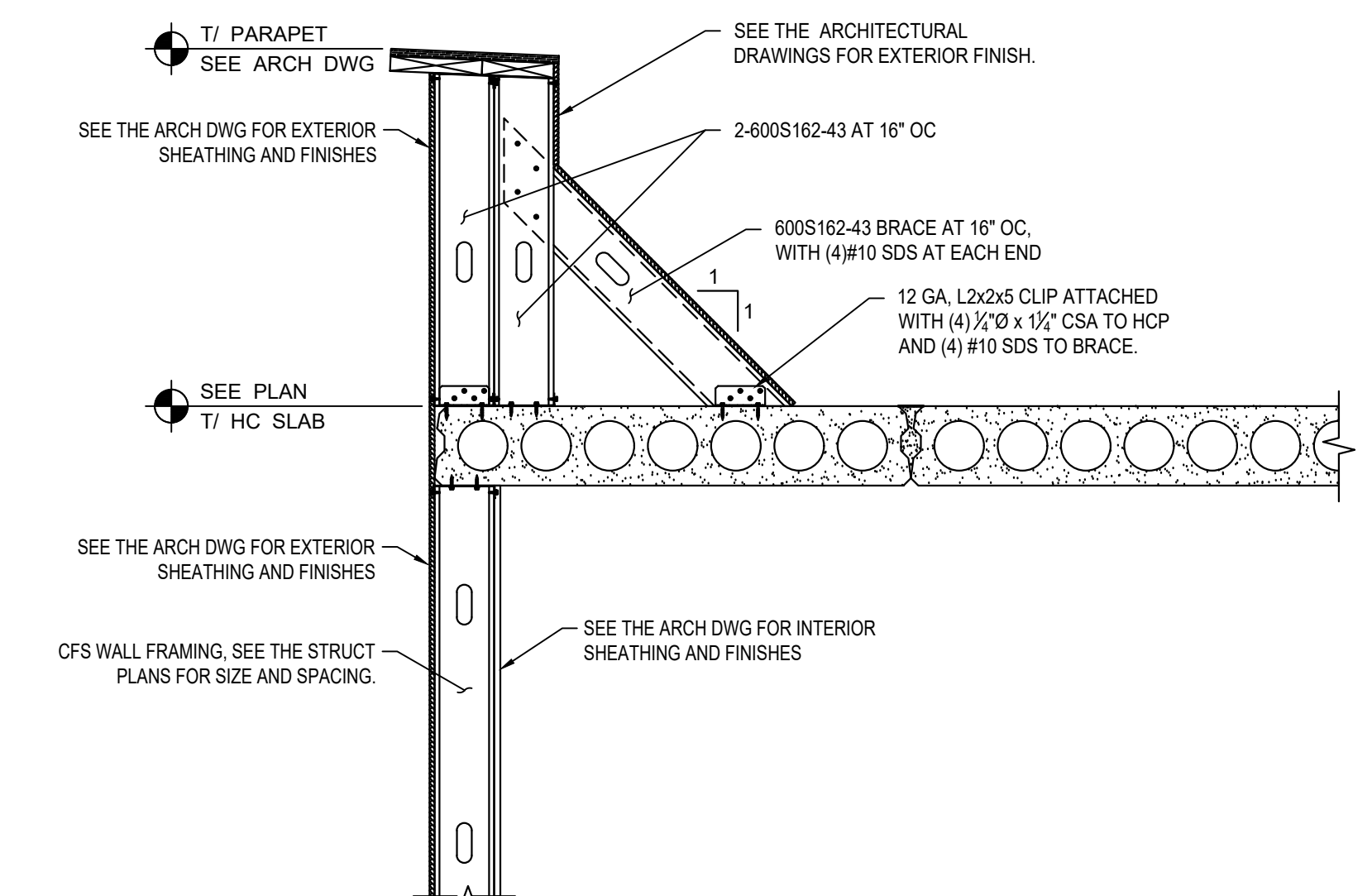
5 TYPICAL HOLLOW CORE PLANK END BEARING AT CMU WALL
SCALE: 3/4" = 1'-0"



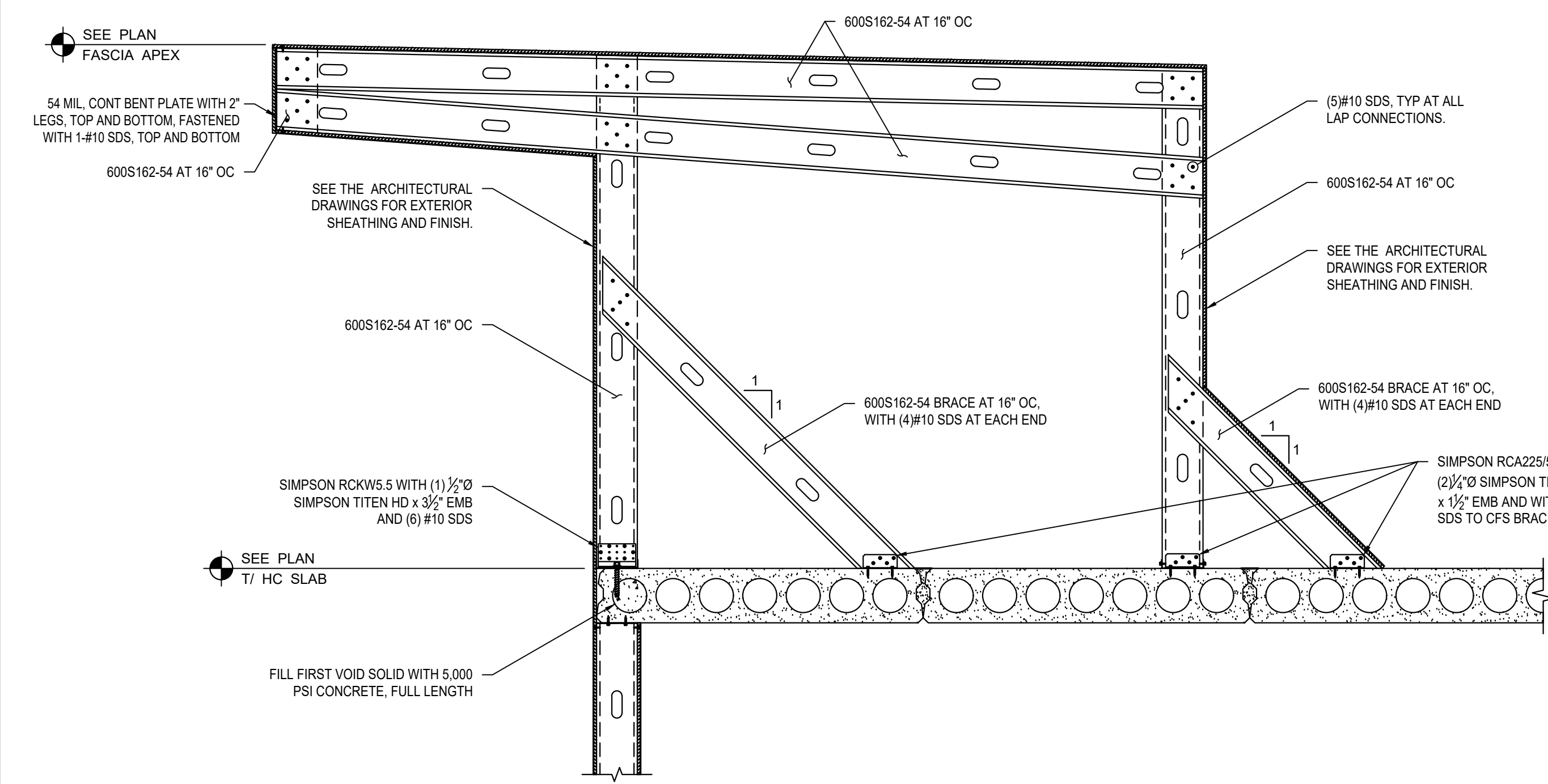
6 HOLLOW CORE PLANK BEARING AT CMU WALL
SCALE: 3/4" = 1'-0"



7 TYPICAL HOLLOW CORE PLANK TO PLANK, GROUT FILLED, KEYWAY
SCALE: 3/4" = 1'-0"



8 PARAPET SECTION
SCALE: 3/4" = 1'-0"



9 TYPICAL HOLLOW CORE PLANK END BEARING AT CMU WALL
SCALE: 3/4" = 1'-0"

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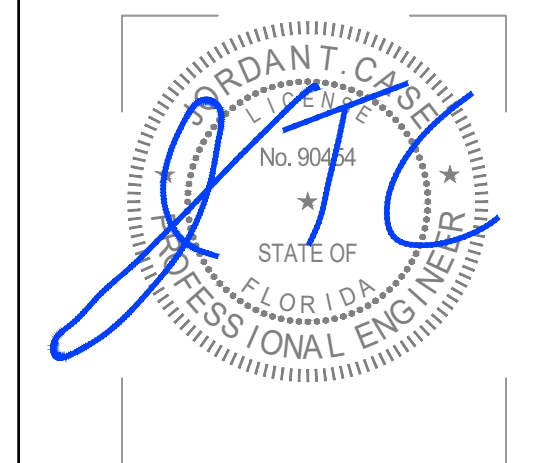
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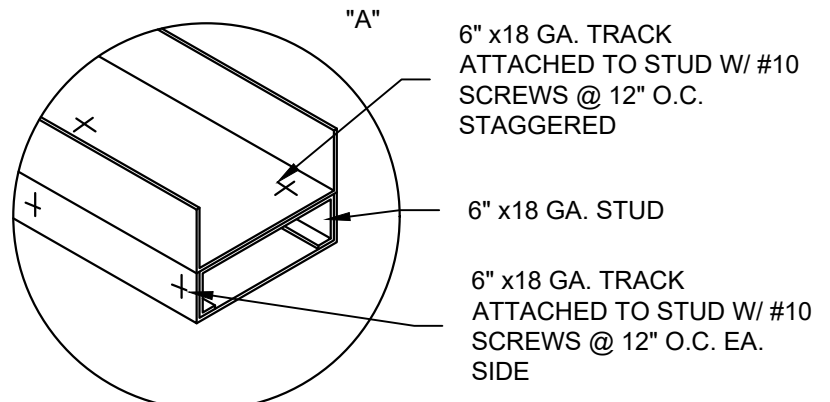


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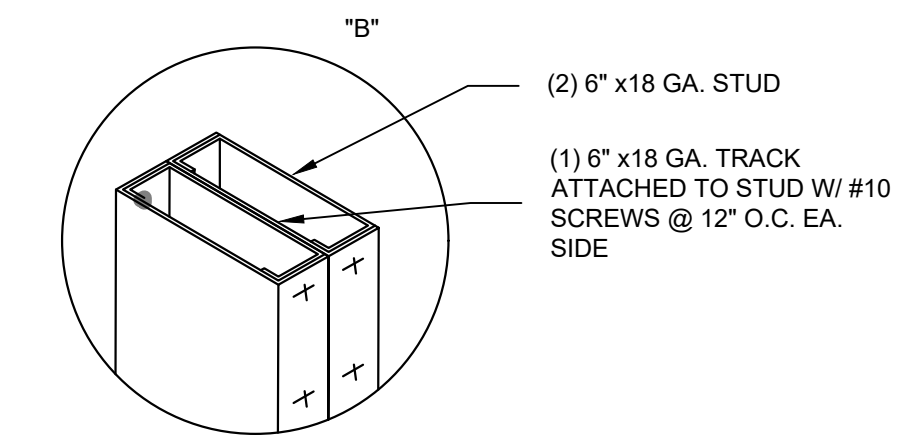
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JOB NO. 22039
DATE 07-11-2024
SHEET **S3.8**

1ST FLOOR FRAMING

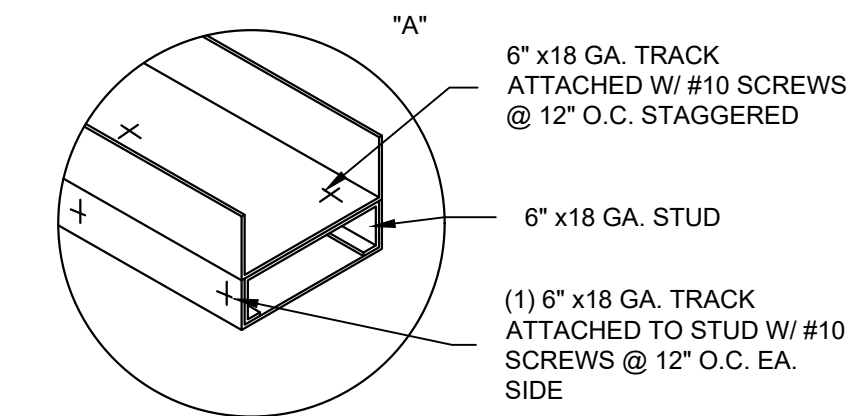


MAXIMUM OPENING WIDTH 8'-0"

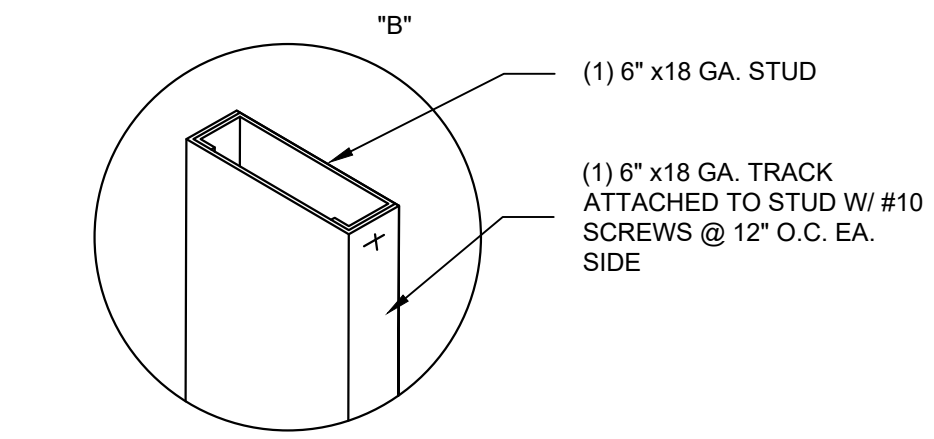
FOR OPENING GREATER THAN 8'-0" USE DETAIL ON SK.XX



2ND - 4TH FLOOR FRAMING



MAXIMUM OPENING WIDTH 8'-0"



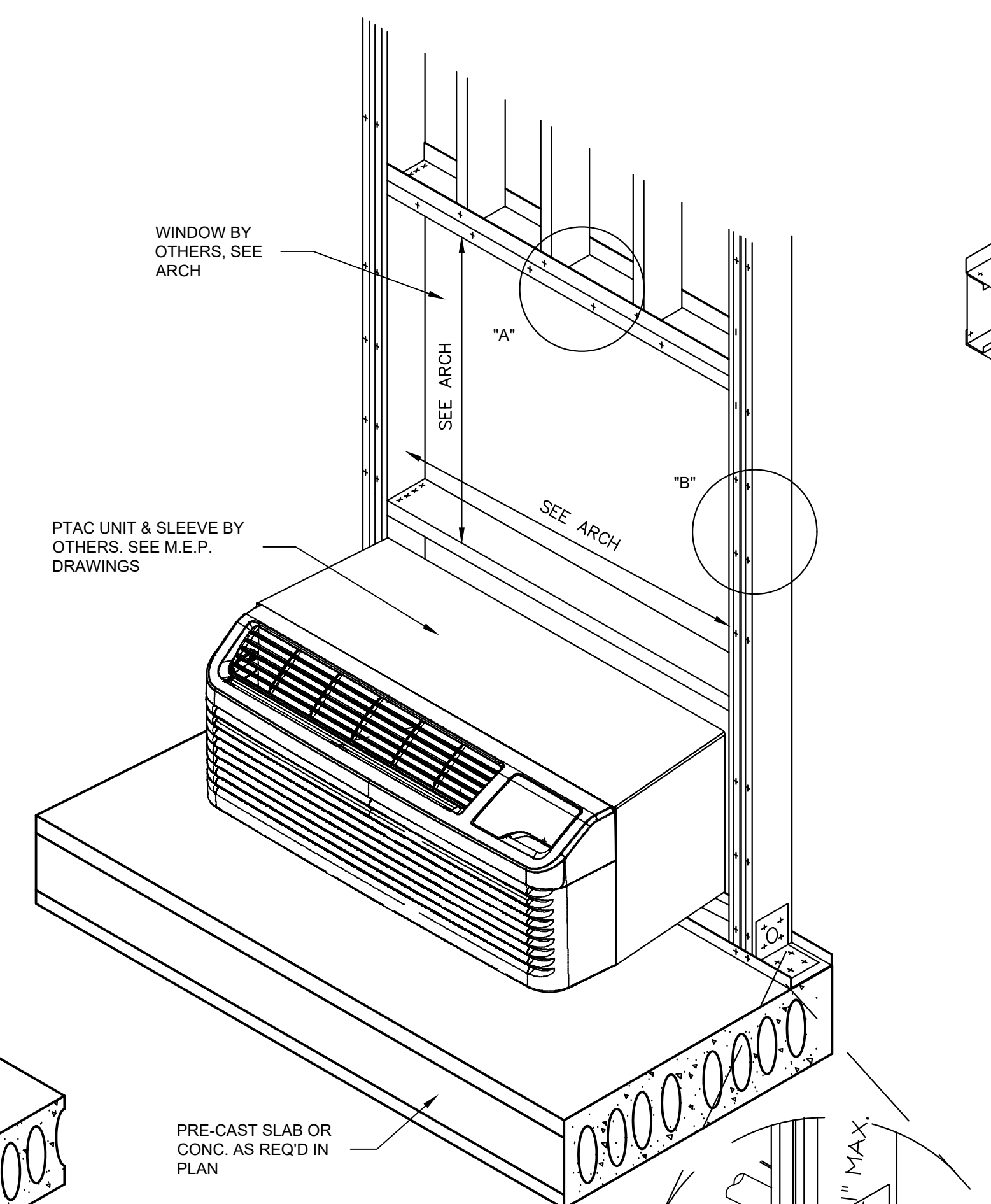
18 GA. CLIP ANGLE ATTACHED W/ (4) #10 SCREWS EACH FLANGE @ EACH CORNER OF OPENING

SILL WHERE REQ'D

CRIPPLE STUDS SAME GAUGE & SPACING AS FULLSIZE STUDS WHERE REQ'D

18 GA CLIP ANGLE ATTACHED W/ (4) #10 SCREWS TO STUDS & (4) 0.157 Ø P.A.F. TO SLAB @ TOP & BOT. & EA. SIDE OF OPENING. NOTE: CLIP MAY BE PLACED ON INSIDE OF JAMB

1 **FRAMED OPENING DETAIL**
SCALE: 3/4" = 1'-0"



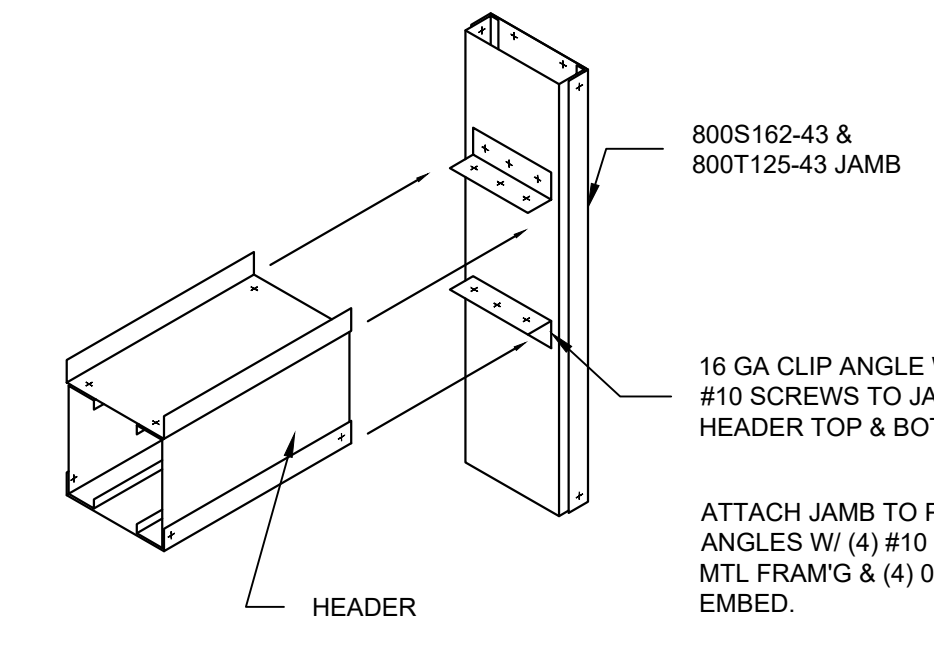
WINDOW BY OTHERS, SEE ARCH

PTAC UNIT & SLEEVE BY OTHERS, SEE M.E.P. DRAWINGS

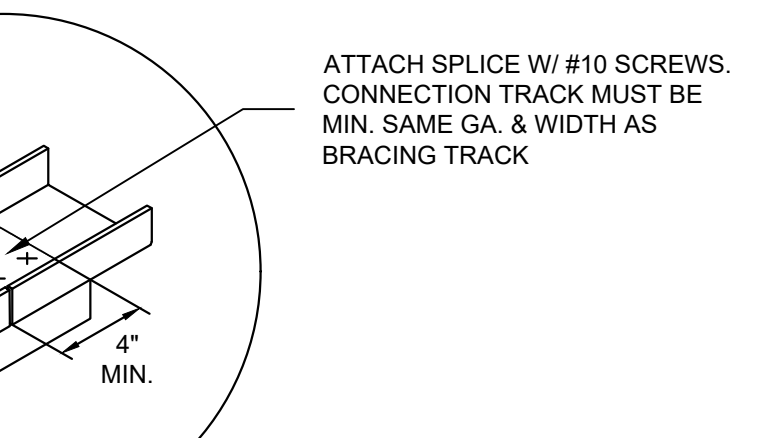
PRE-CAST SLAB OR CONC. AS REQ'D IN PLAN

USE 16 GA. CLIP ANGLE TO PLANKS W/ 1" Ø HOLE FOR CONDENSATE LINE. USE (4) 0.157 Ø P.A.F. W/ 1" EMBED TO CONC. & (4) #10 SCREWS TO JAMB

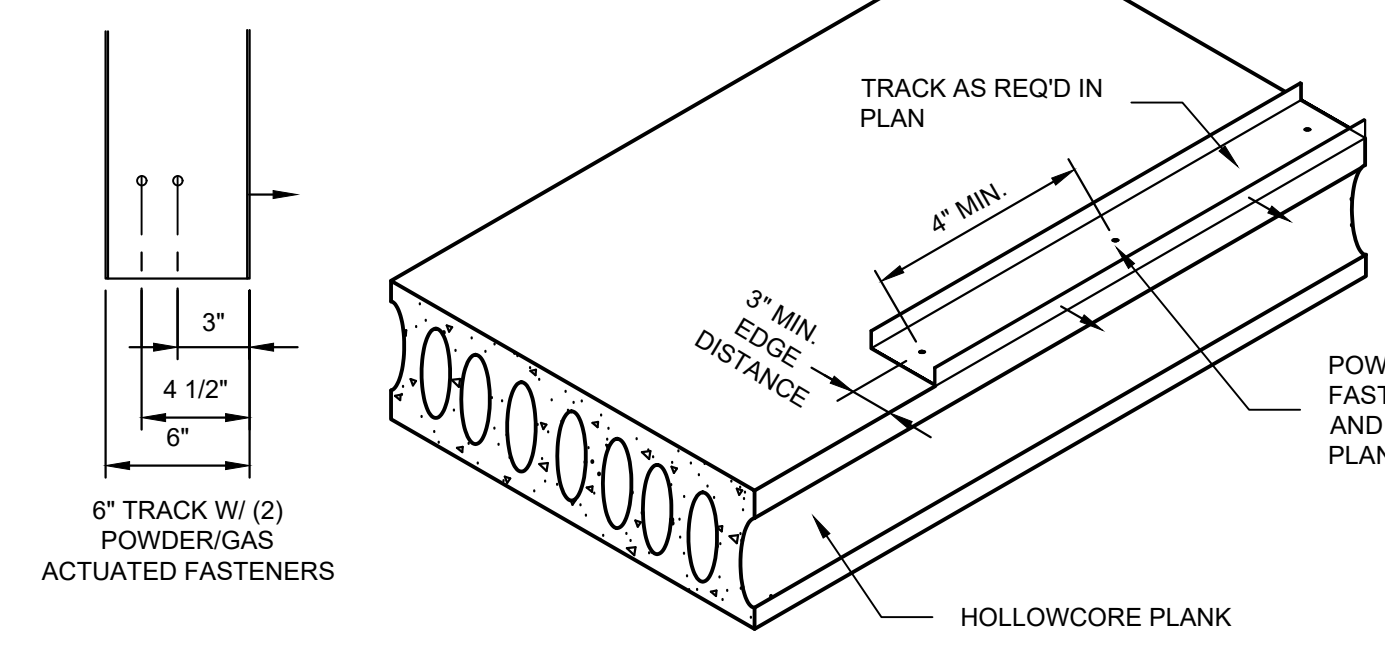
SEE PLUMBING PLANS FOR SIZE OF CONDENSATE LINE



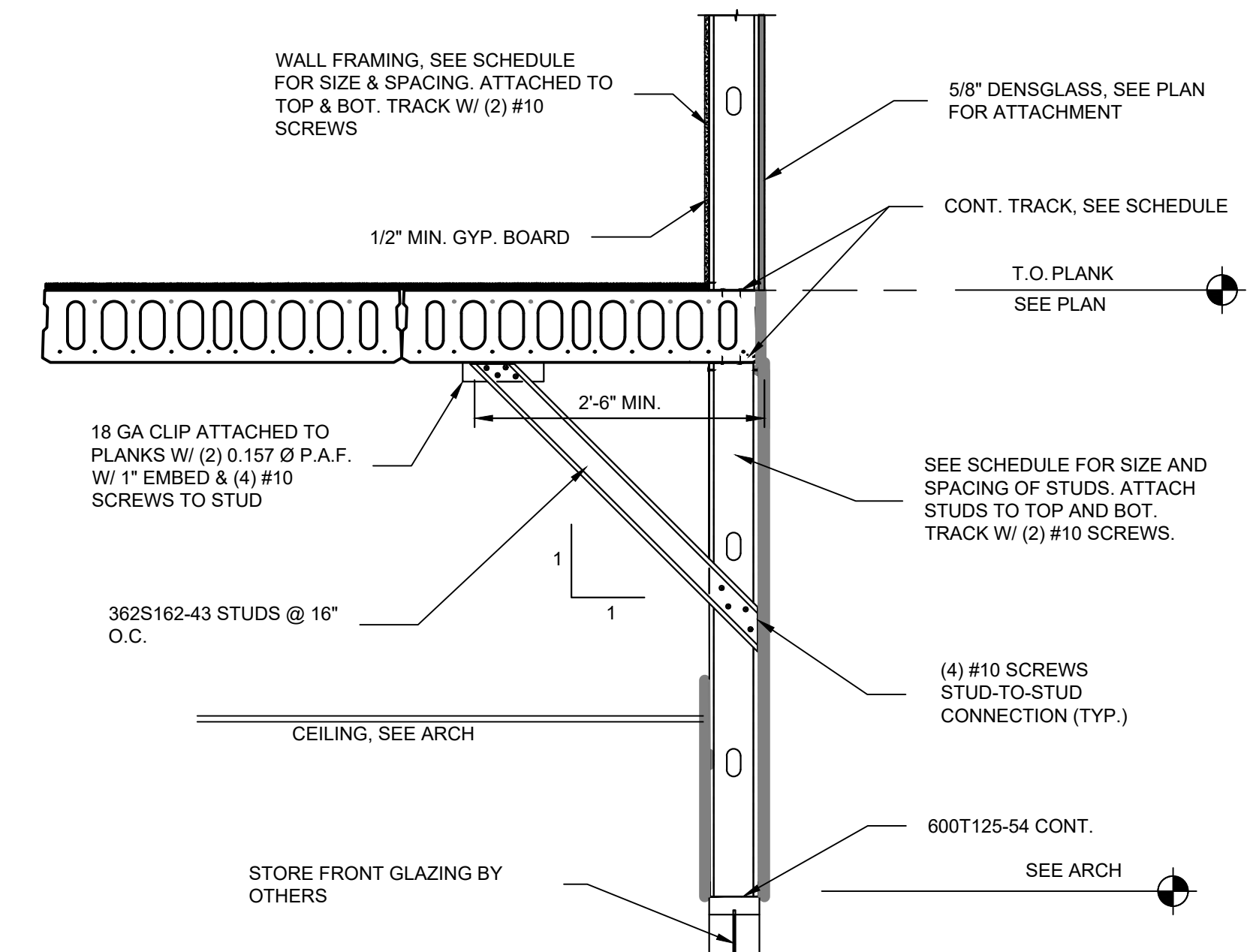
HEAD TO JAMB CONNECTION DETAIL



2 **WALL LATERAL BRACING DETAIL**
SCALE: 3/4" = 1'-0"



3 **PERIMETER WALL TRACK DETAIL**
SCALE: 3/4" = 1'-0"



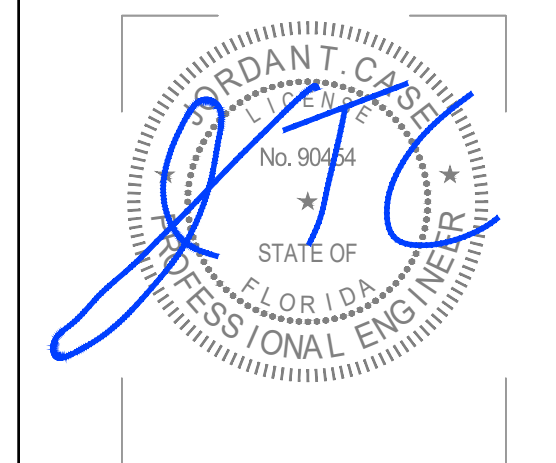
4 **EXTERIOR FRAMED WALL AT STRIP WINDOWS DETAIL**
SCALE: 3/4" = 1'-0"

EXTERIOR METAL TRACK SCHEDULE

FLOOR	STUD SIZE AND SPACING	TRACK SIZE		TRACK CONNECTION	
		BOTTOM:	TOP:	BOTTOM:	TOP:
FOURTH	SEE PLAN	600T125-43	DEFLECTION 600T250-43	(1)-0.157" DIA PAF AT 8" OC, STAGGERED (1" EMBED)	(1)-0.157" DIA PAF AT 8" OC, STAGGERED (1" EMBED)
THIRD	SEE PLAN	600T125-43	DEFLECTION 600T250-43	(1)-0.157" DIA PAF AT 8" OC, STAGGERED (1" EMBED)	(1)-0.157" DIA PAF AT 8" OC, STAGGERED (1" EMBED)
SECOND	SEE PLAN	600T125-43	DEFLECTION 600T250-43	(1)-0.157" DIA PAF AT 8" OC, STAGGERED (1" EMBED)	(1)-0.157" DIA PAF AT 8" OC, STAGGERED (1" EMBED)
FIRST	SEE PLAN	600T125-43	DEFLECTION 600T250-43	(2)-0.157" DIA PAF AT 8" OC (1" EMBED)	(2)-0.157" DIA PAF AT 8" OC (1" EMBED)



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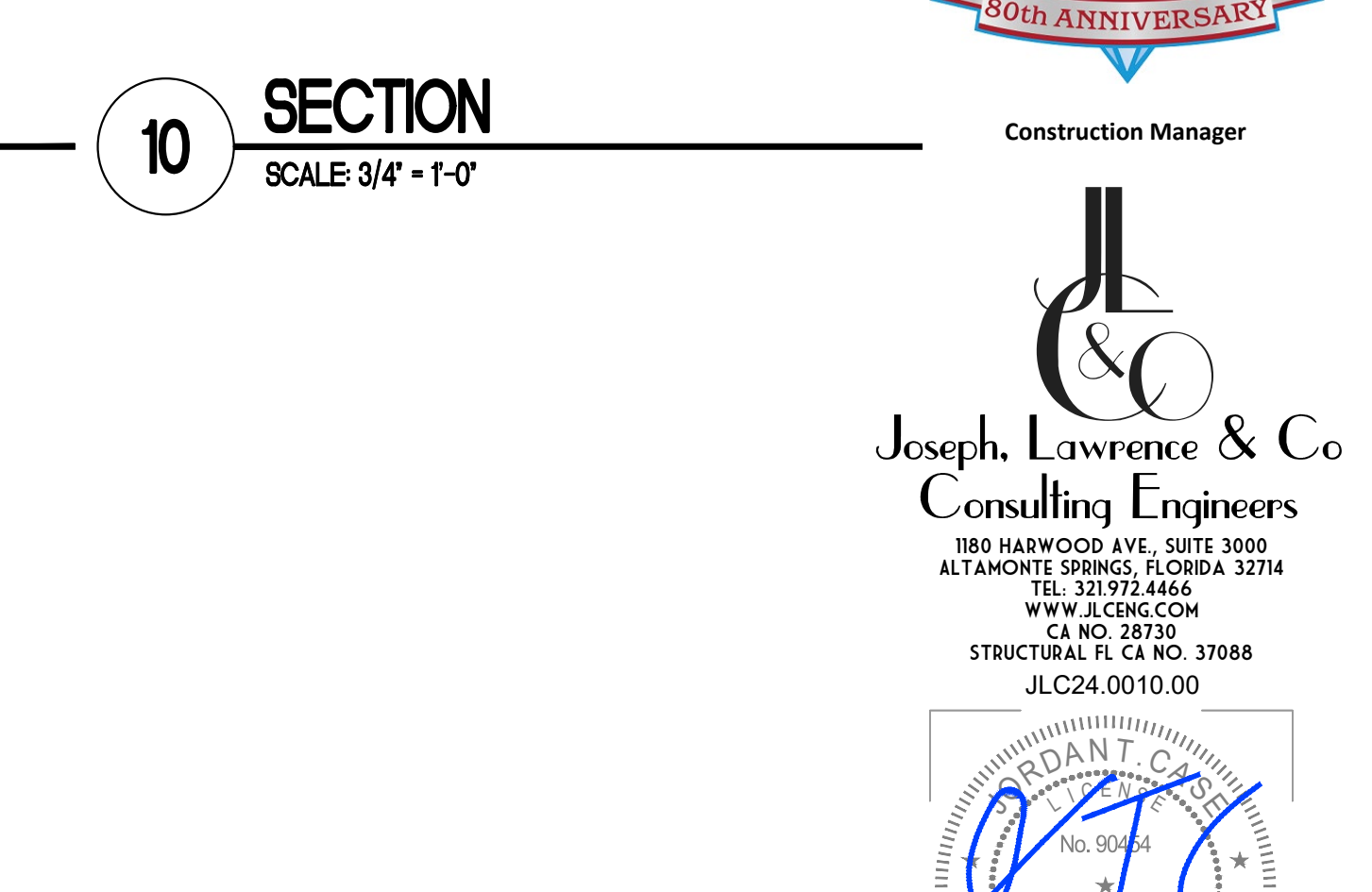
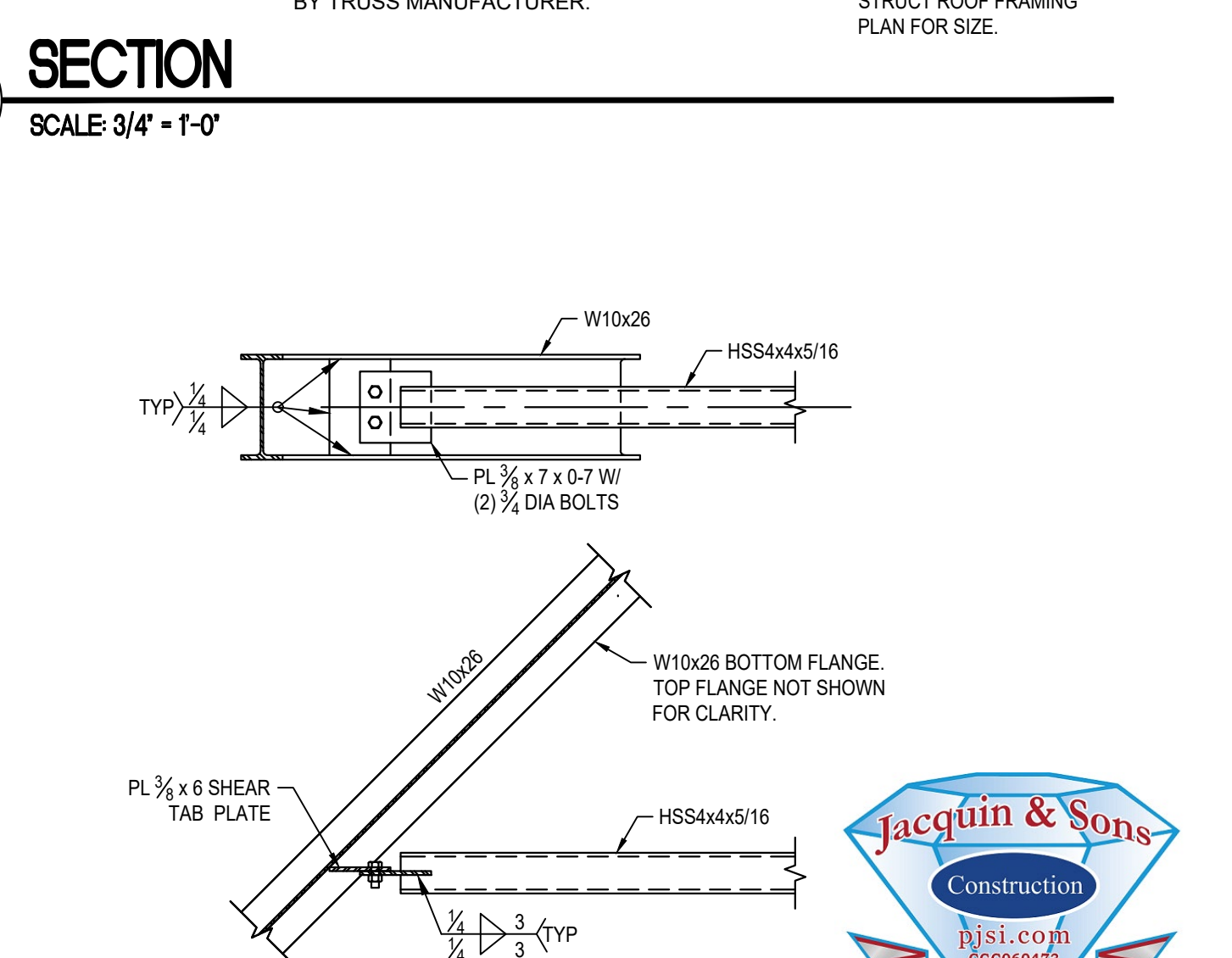
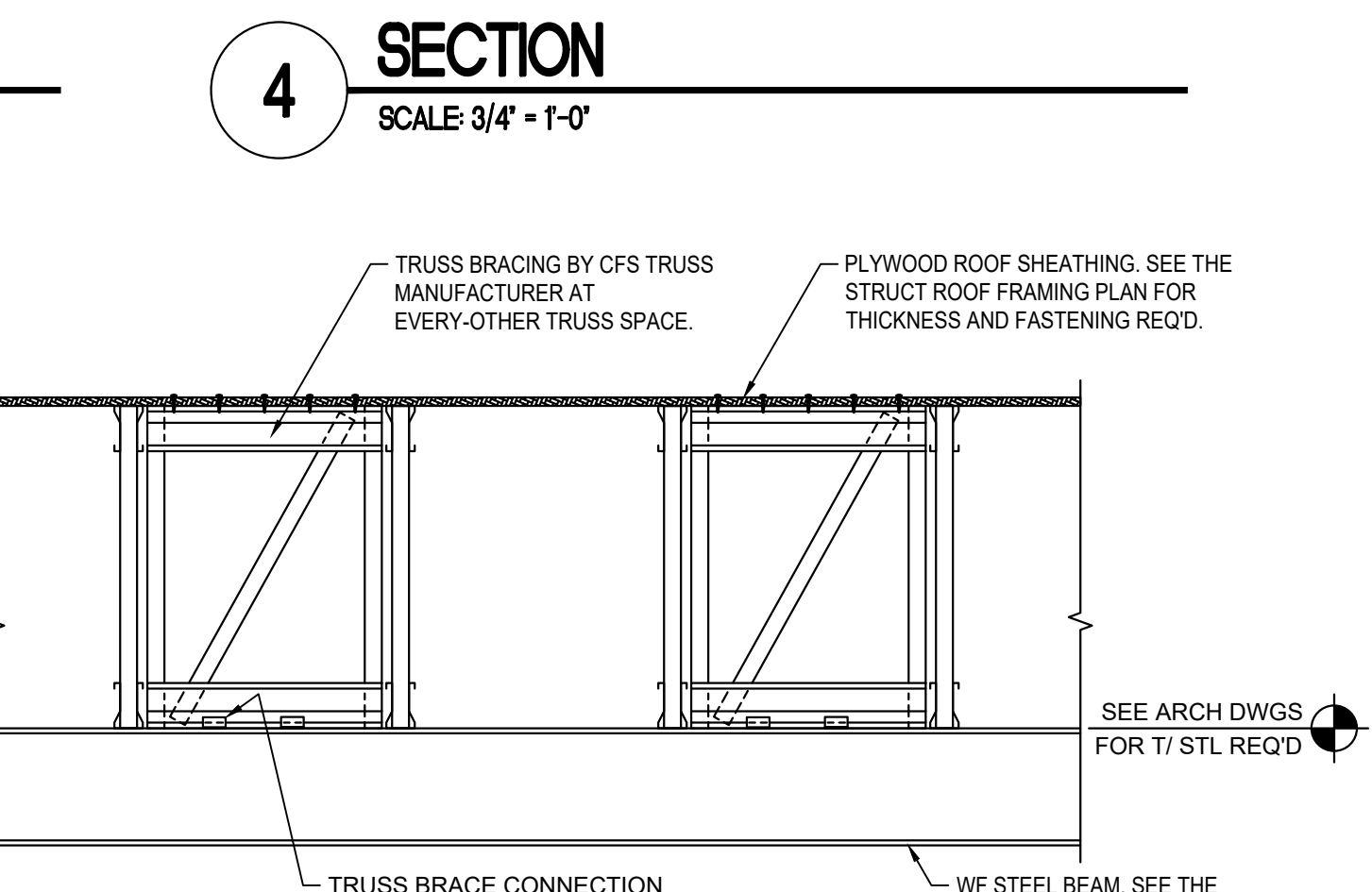
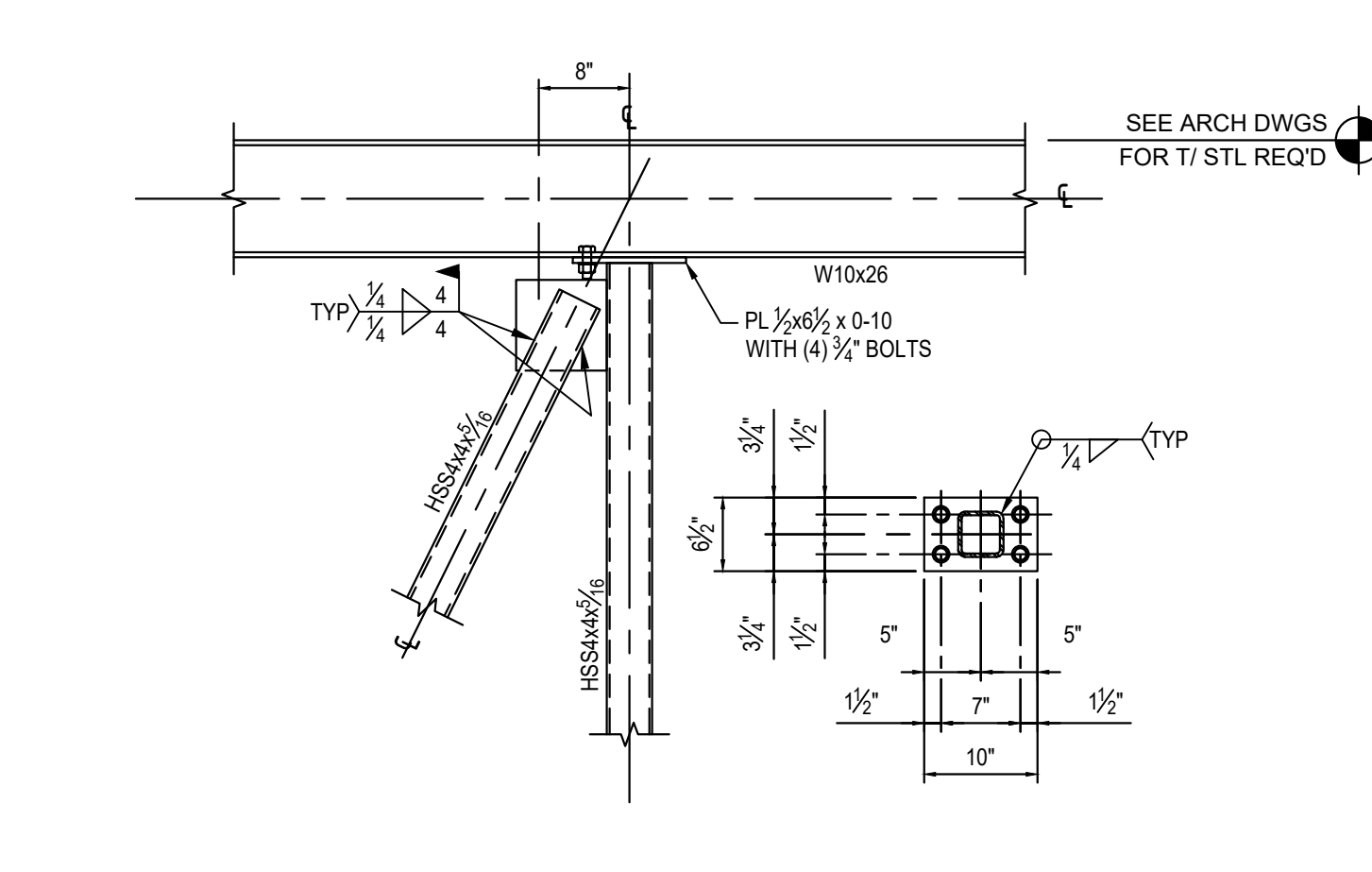
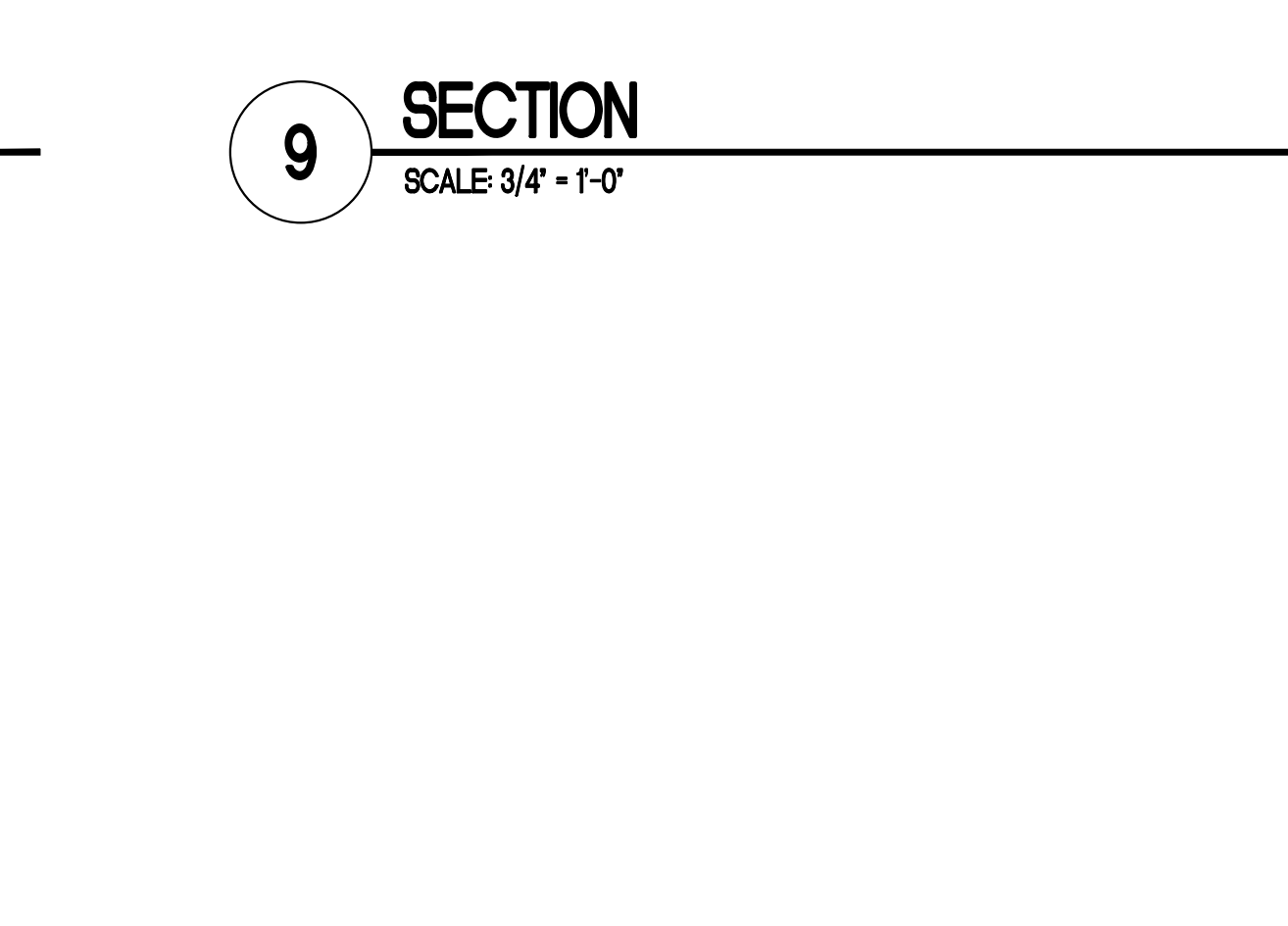
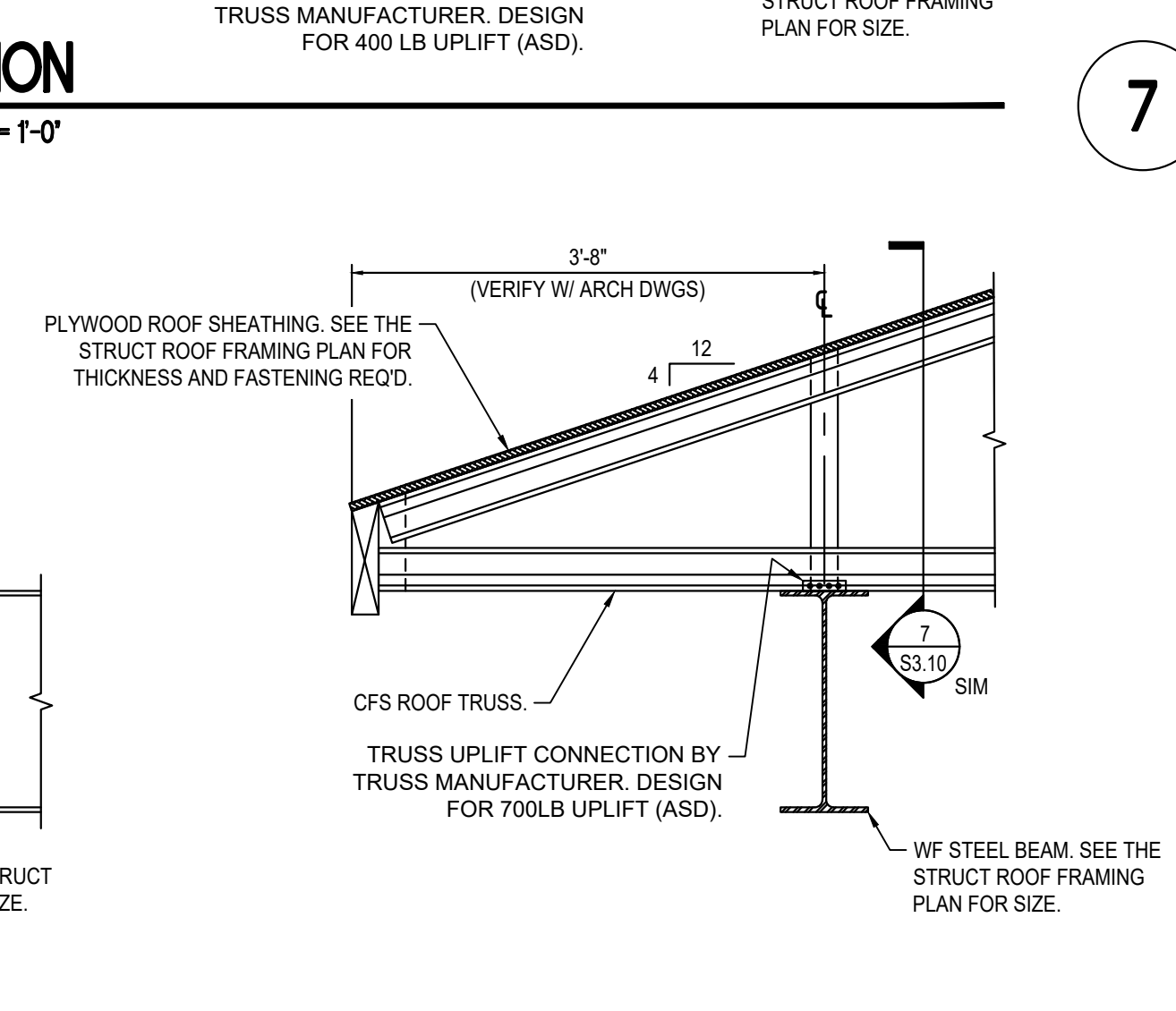
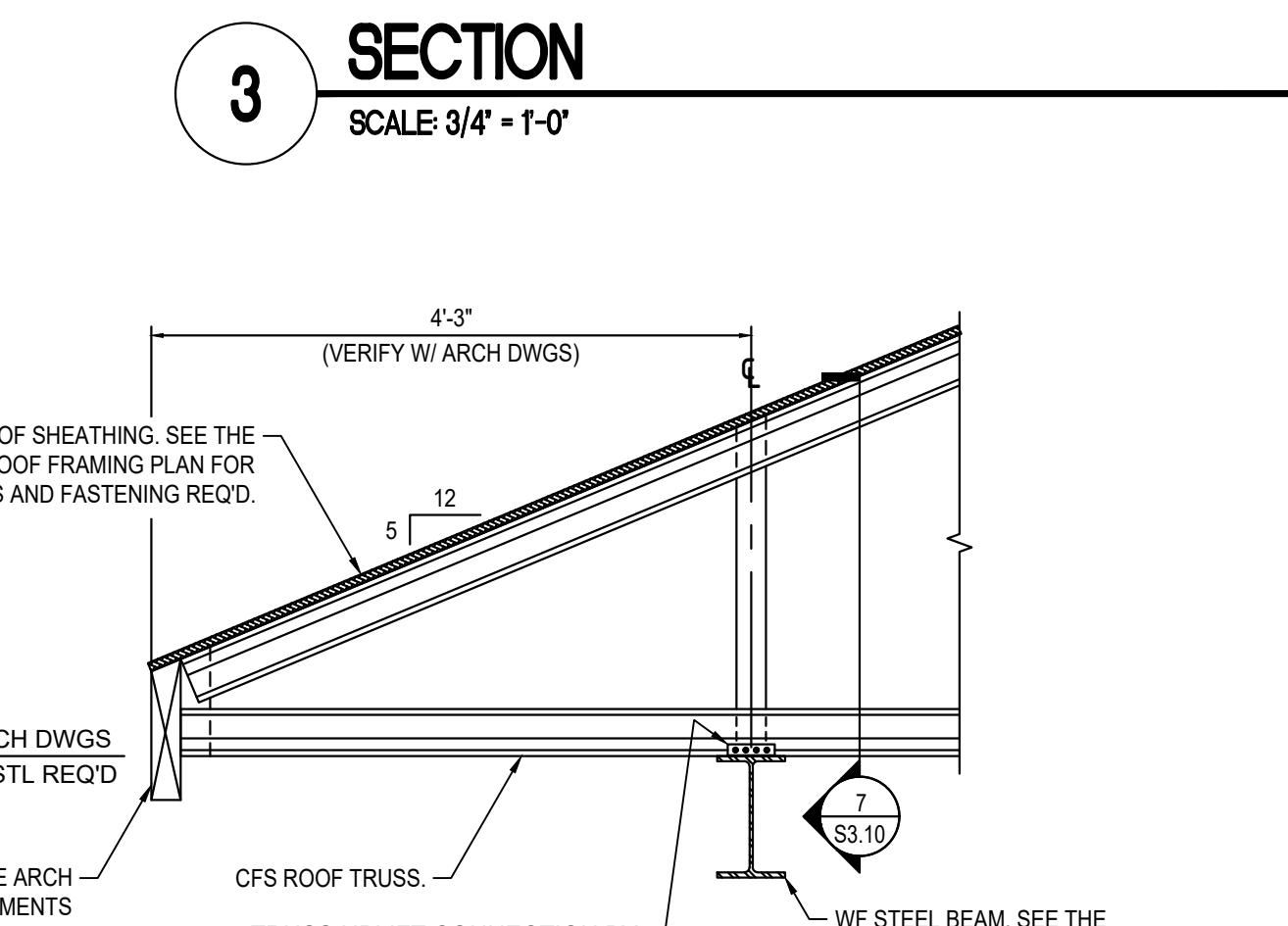
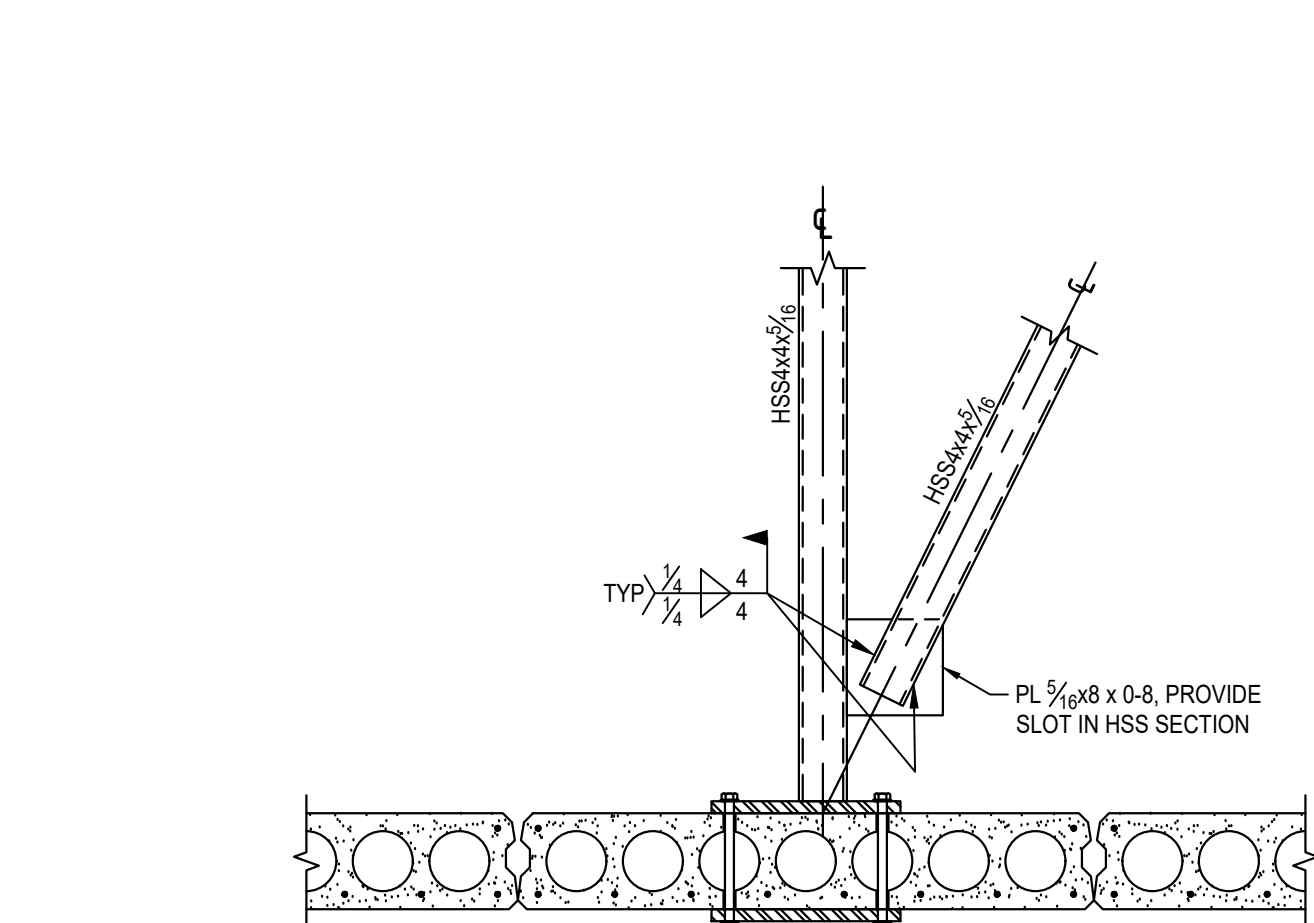
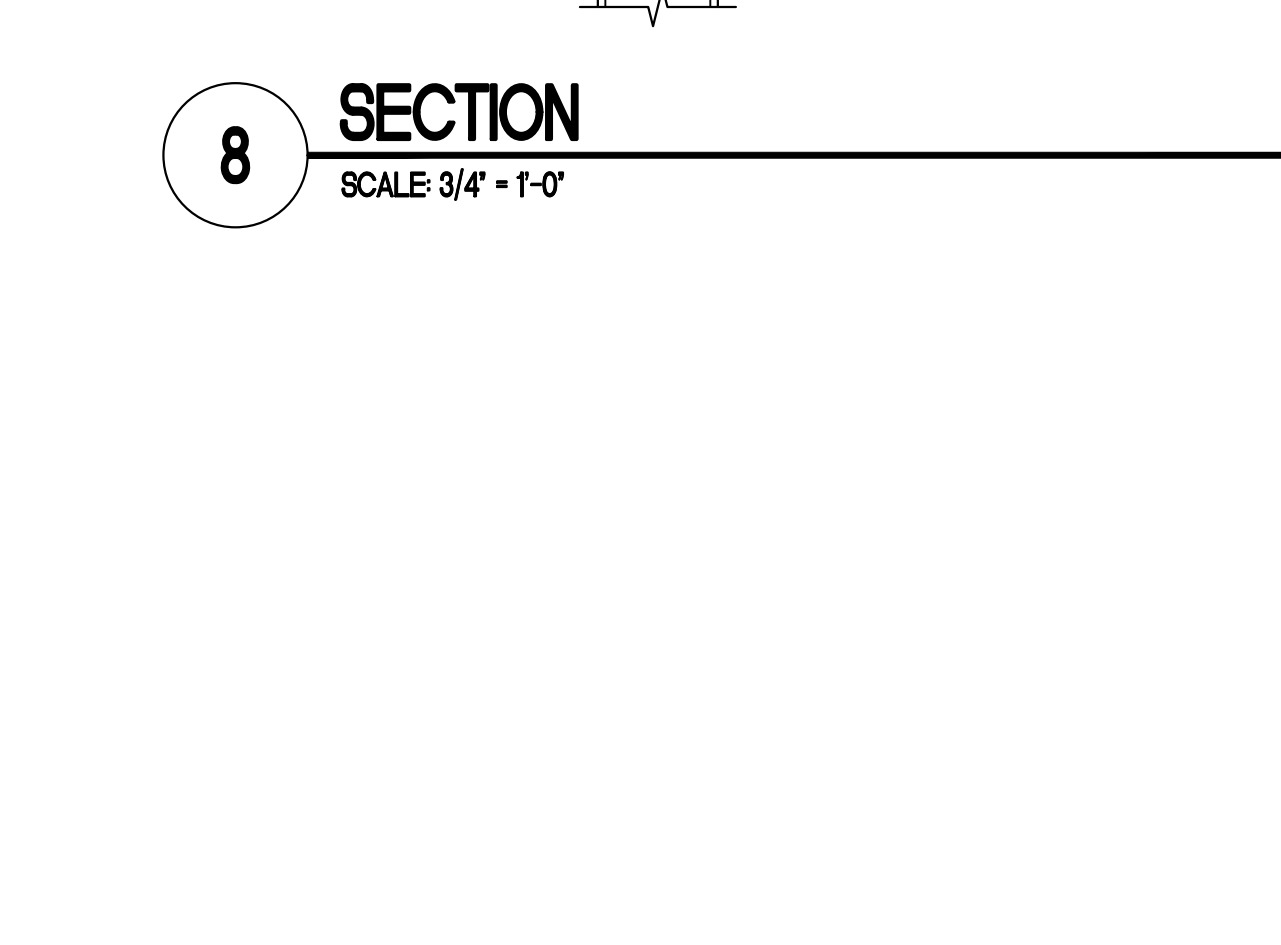
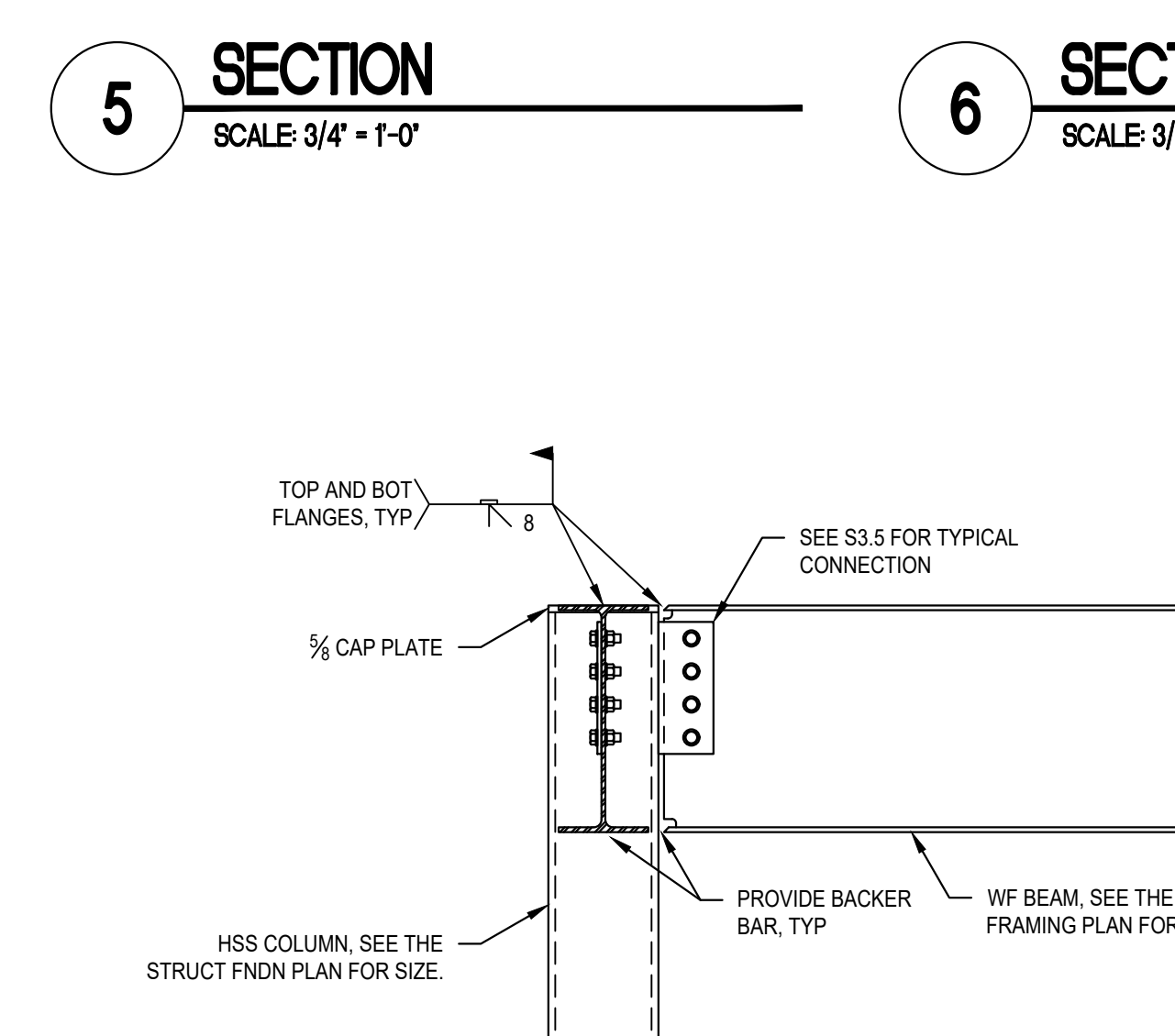
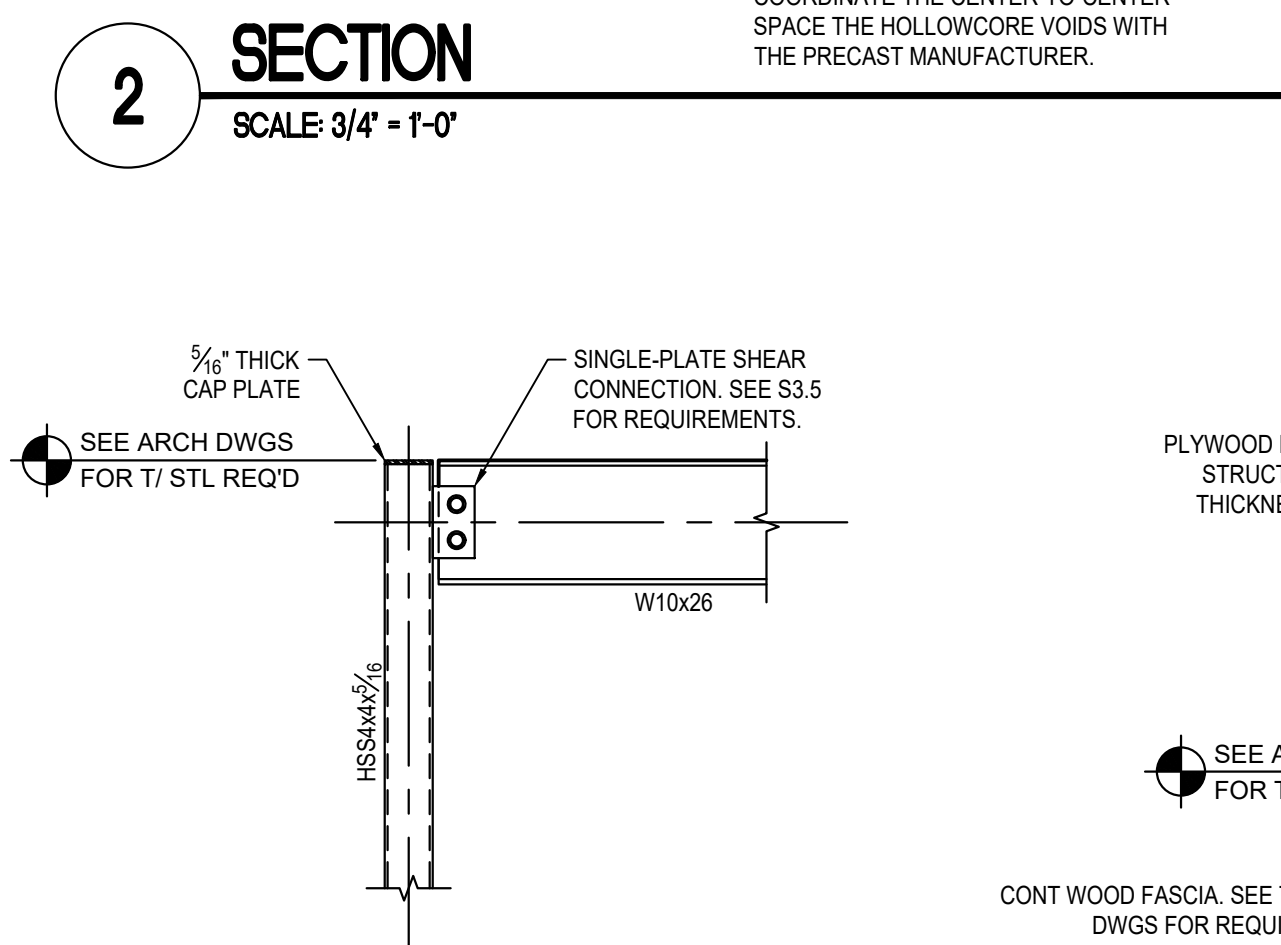
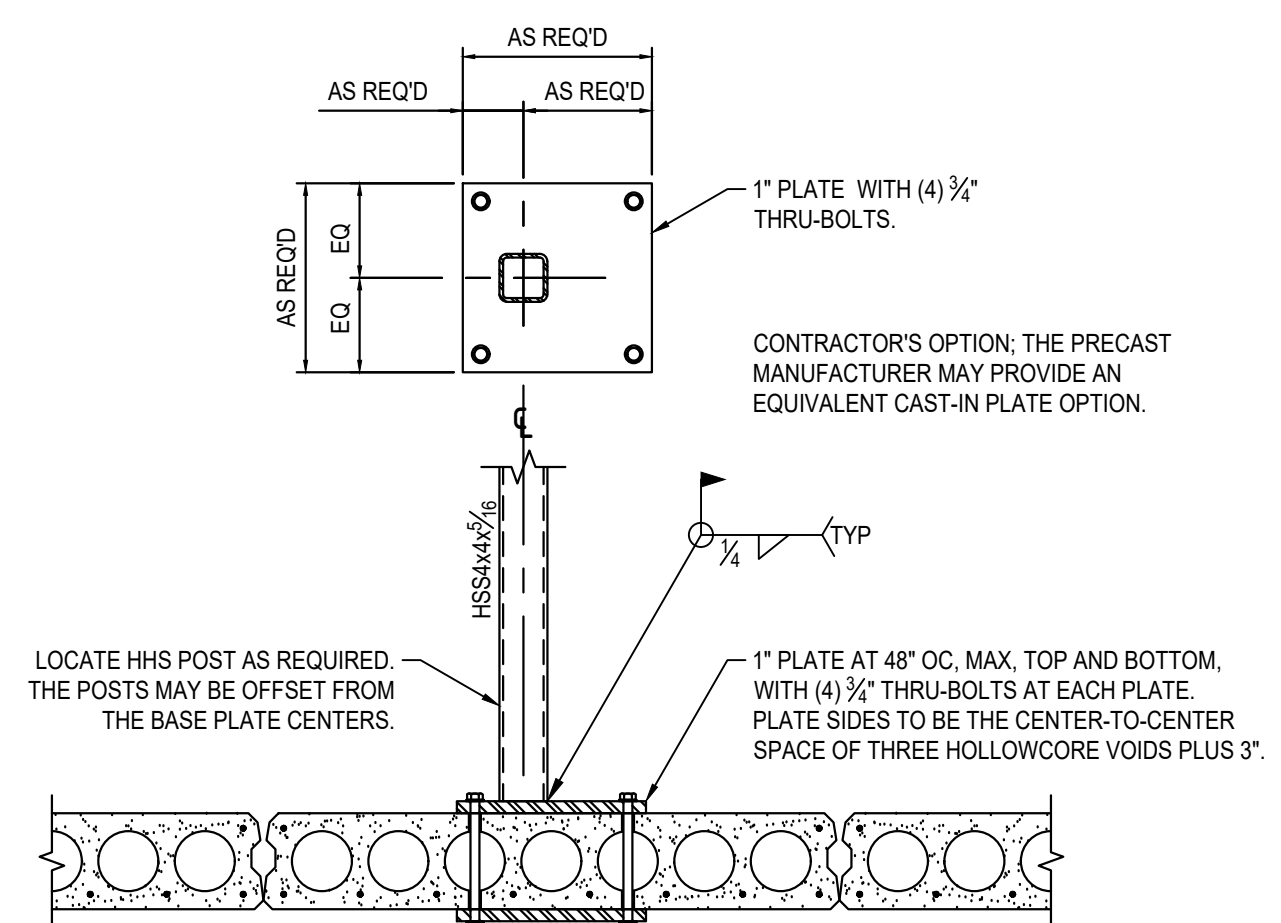
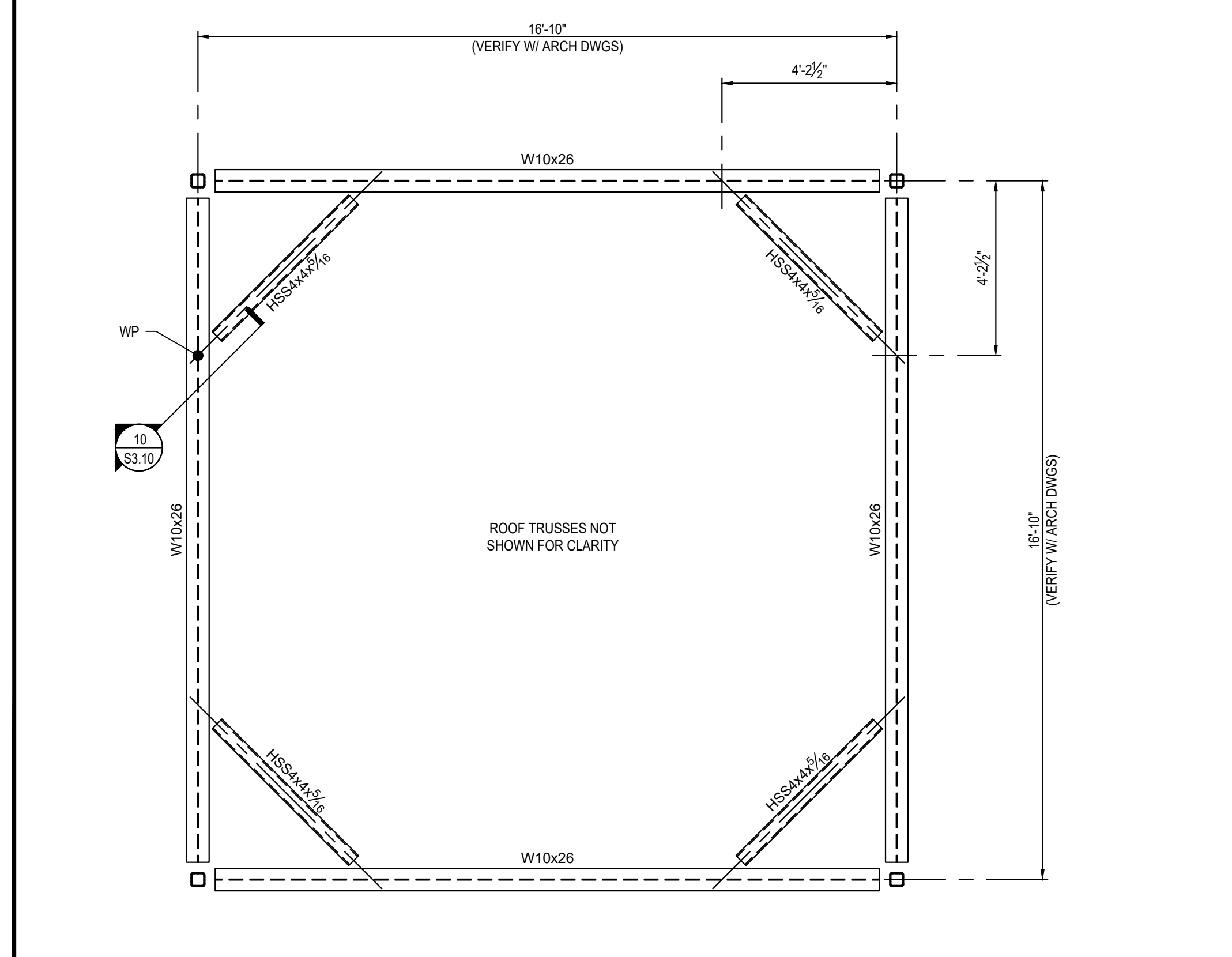
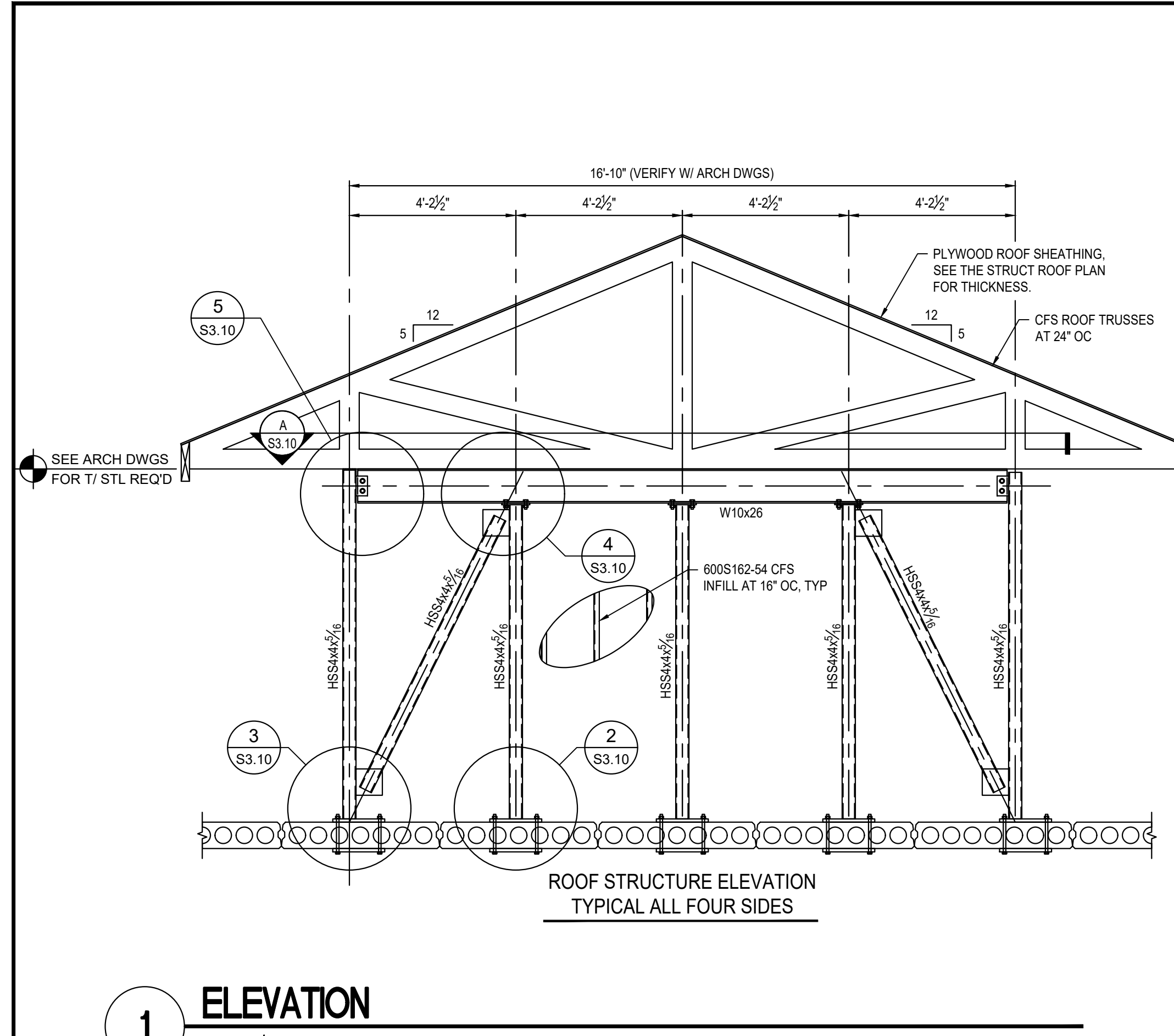
REVISION

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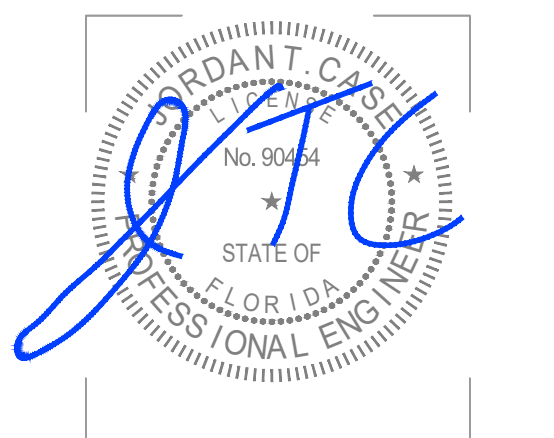
DATE 07-11-2024

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S3.9



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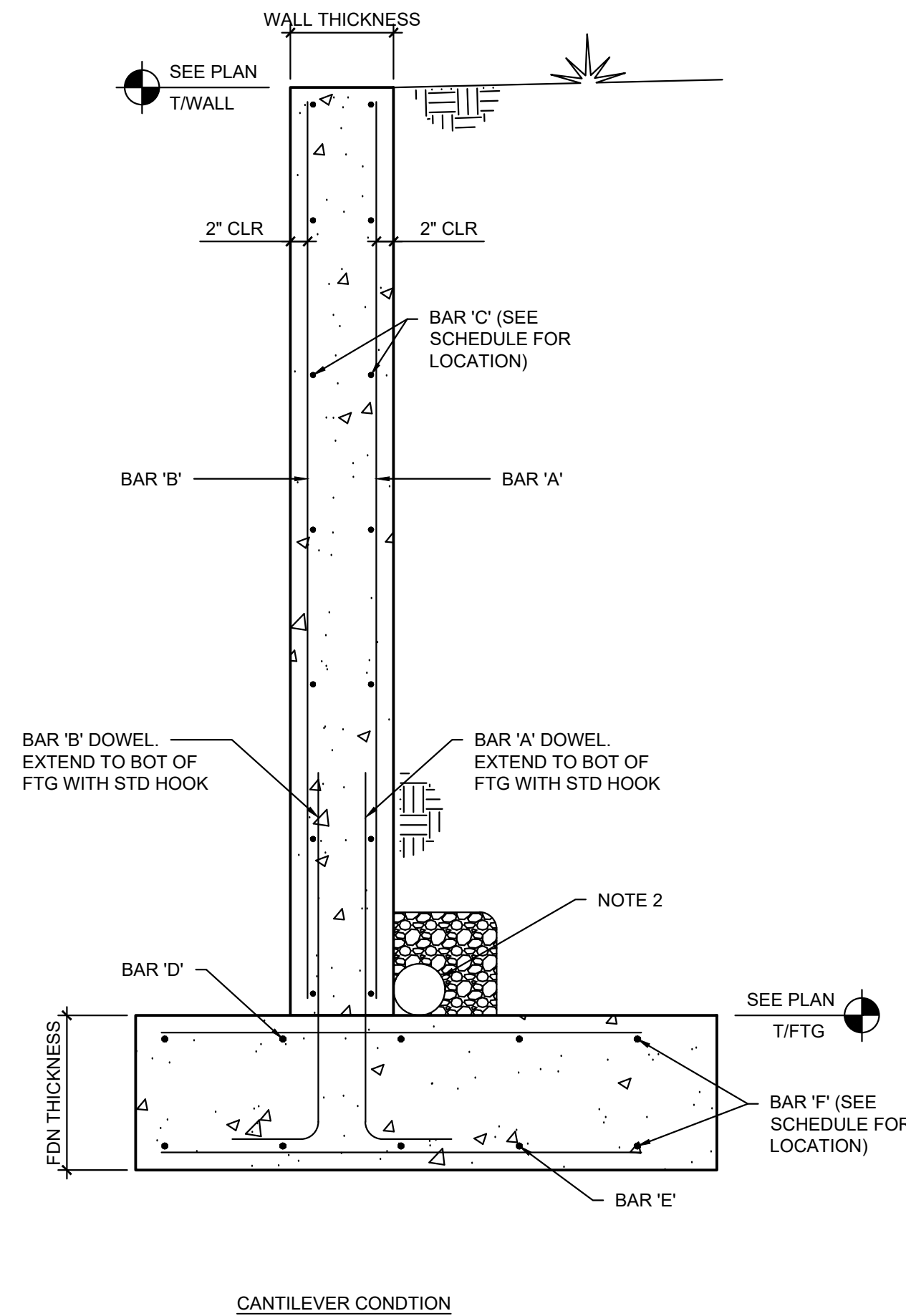
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SHEET S3.10

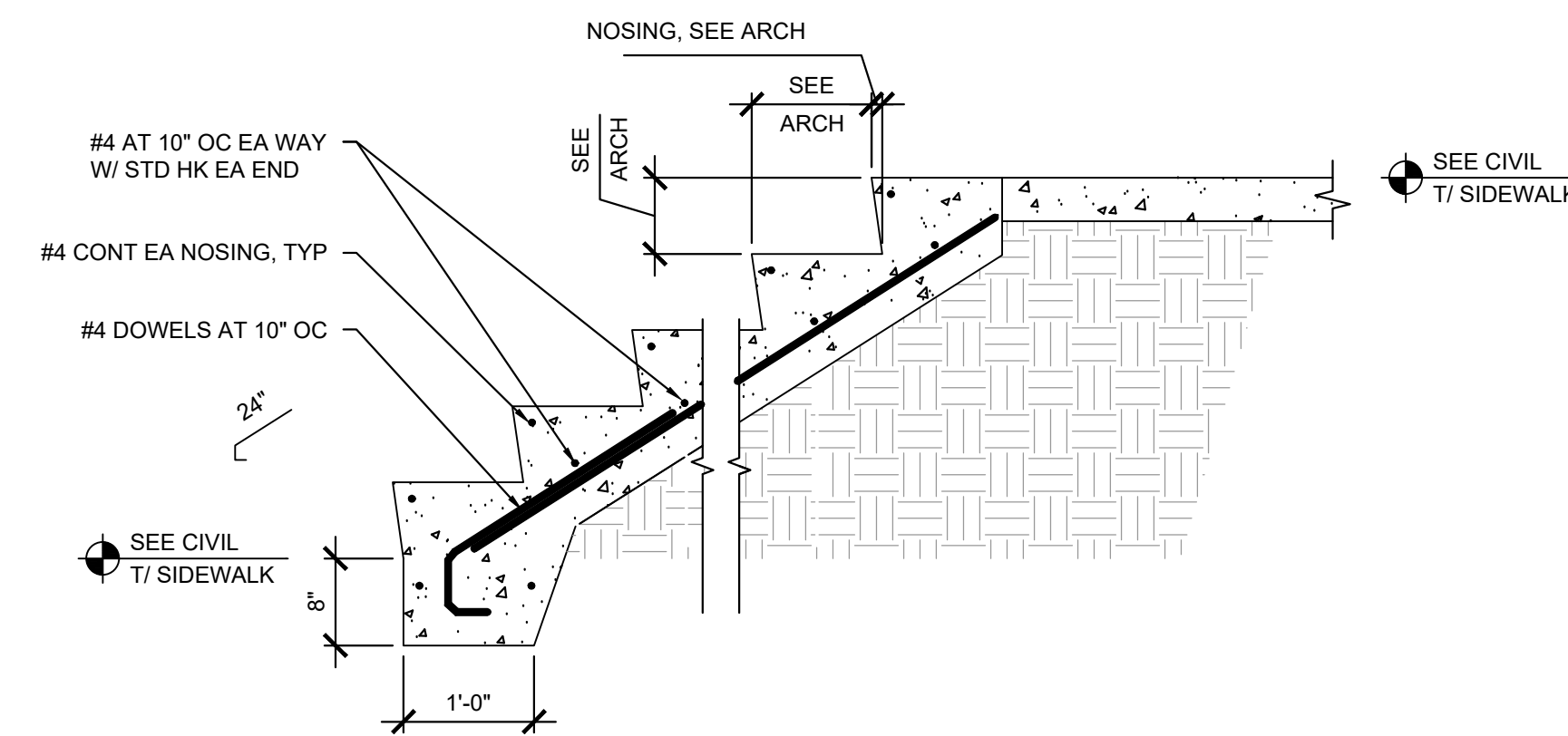
RETAINING WALL SCHEDULE

WALL HEIGHT	WALL					FOUNDATION						
	WALL WIDTH (IN)	BAR 'A'	BAR 'B'	BAR 'C' (HORIZONTAL REINFORCEMENT)		TOE (FT-IN)	HEEL (FT-IN)	THICKNESS (IN)	BAR 'D' (TOP BARS)	BAR 'E' (BOTTOM BARS)	BAR 'F' (CONTINUOUS REINFORCEMENT)	
				SIZE & SPACING	LOCATION						SIZE & SPACING	LOCATION
4'-0"	8"	#5 AT 12" OC	-	#5 AT 18" OC	CENTERED	1'-7"	2'-5"	12"	#5 AT 12" OC	#5 AT 12" OC	#5 AT 12" OC	TOP & BOT
6'-0"	8"	#5 AT 12" OC	#5 AT 12" OC	#5 AT 18" OC	EACH FACE	3'-0"	5'-0"	14"	#5 AT 12" OC	#5 AT 12" OC	#5 AT 12" OC	TOP & BOT
8'-0"	8"	#8 AT 12" OC	#5 AT 12" OC	#5 AT 18" OC	EACH FACE	3'-0"	6'-0"	18"	#6 AT 12" OC	#8 AT 12" OC	#6 AT 12" OC	TOP & BOT
10'-0"	10"	#8 AT 8" OC	#5 AT 12" OC	#5 AT 12" OC	EACH FACE	3'-0"	6'-0"	17"	#7 AT 12" OC	#8 AT 8" OC	#6 AT 12" OC	TOP & BOT
12'-0"	12"	#8 AT 8" OC	#5 AT 12" OC	#5 AT 12" OC	EACH FACE	4'-6"	11'-6"	20"	#8 AT 12" OC	#8 AT 8" OC	#6 AT 12" OC	TOP & BOT

- NOTES:**
1. MINIMUM BACKFILLING REQUIREMENTS
 - A. FOR 'CANTILEVER' RETAINING WALLS, THE WALL SHALL NOT BE BACKFILLED AGAINST UNTIL BOTH THE WALL AND ITS FOUNDATION HAVE REACHED THE MINIMUM SPECIFIED COMPRESSIVE STRENGTH.
 - B. FOR 'RESTRAINED' RETAINING WALLS, THE WALL SHALL BE SHORED Laterally UNTIL THE SLAB THAT IT IS CONNECTED TO HAS BEEN POURED AND REACHES ITS MINIMUM SPECIFIED COMPRESSIVE STRENGTH. IN ADDITION, THE WALL SHALL NOT BE BACKFILLED AGAINST UNTIL BOTH THE WALL AND ITS FOUNDATION HAVE REACHED THE MINIMUM SPECIFIED COMPRESSIVE STRENGTH.
 2. THESE RETAINING WALLS HAVE NOT BEEN DESIGNED TO RESIST HYDROSTATIC PRESSURE THAT RESULTS FROM WATER BUILD-UP ON THE RETAINED FACE OF THE WALL. POSITIVE DRAINAGE SHALL BE PROVIDED BY OTHERS
 3. PROVIDE STANDARD HOOKS AS SHOWN WITHIN THIS DETAIL

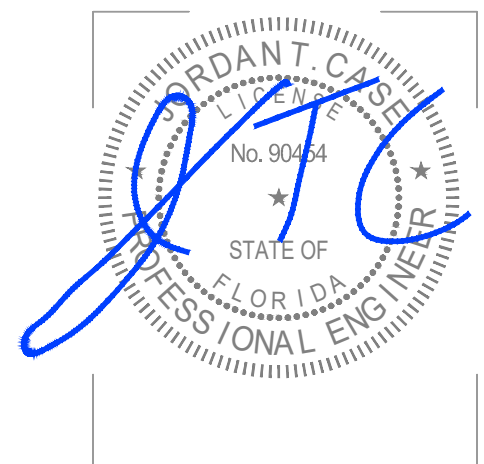


1 TYP RETAINING WALL DETAIL
SCALE: 3/4" = 1'-0"



1 TYP CIP STAIR DETAIL AT RETAINING WALLS
SCALE: 3/4" = 1'-0"


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