

STRUCTURAL NOTES

GENERAL NOTES:

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS.

DESIGN LOADS:

THE STRUCTURAL SYSTEM FOR THE BUILDING HAS BEEN DESIGNED IN ACCORDANCE WITH THE 2023 FLORIDA BUILDING CODE, 8TH EDITION. THE FOLLOWING SUPERIMPOSED LOADINGS HAVE BEEN UTILIZED:

ROOF:

LIVE LOAD - 25 PSF
DEAD LOAD - 30 PSF

WIND:

WIND PER ASCE 7-22
3 SECOND GUST WIND SPEED = 170 MPH
RISK CATEGORY = II
EXPOSURE C

SPECIAL INSPECTIONS:

SPECIAL INSPECTION OF THE CONSTRUCTION IS REQUIRED BY THE STATE OF FLORIDA IN ACCORDANCE WITH CHAPTER 553 OF THE FLORIDA STATUTES. CONSTRUCTION SHALL BE INSPECTED IN ACCORDANCE WITH THE SPECIAL INSPECTION PLAN.

SHOP DRAWING REVIEW:

SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC.

SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. DRAWINGS SUBMITTED WITHOUT REVIEW WILL BE RETURNED UNCHECKED.

SHOP DRAWING SUBMITTALS SHALL INCLUDE ONE GOOD QUALITY REPRODUCIBLE DIGITAL SET OF PLANS AND THREE SETS OF SIGNED AND SEALED PLANS. ONE SET OF PLANS WILL BE RETAINED BY THE ENGINEER, ONE BY THE ARCHITECT, ONE BY THE LOCAL BUILDING DEPARTMENT (WHERE REQUIRED) AND THE CONTRACTOR SHALL MAKE COPIES FROM THE DIGITAL SET OF PLANS AS REQUIRED FOR DISTRIBUTION.

THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED IN WRITING BY THE ENGINEER.

CHANGES AND ADDITIONS MADE ON RE-SUBMITTALS SHALL BE CLEARLY FLAGGED AND NOTED. THE PURPOSE OF THE RE-SUBMITTALS SHALL BE CLEARLY NOTED ON THE LETTER OF TRANSMITTAL. ARCHITECT/ENGINEER REVIEW WILL BE LIMITED TO THOSE ITEMS CAUSING THE RE-SUBMITTAL.

SHOP DRAWINGS FOR SPECIALTY ENGINEERED PRODUCTS:

THE FOLLOWING SYSTEMS AND COMPONENTS AS A MINIMUM REQUIRE FABRICATION AND ERECTION DRAWINGS PREPARED BY A DELEGATED ENGINEER:

LIGHT GAGE STEEL EXTERIOR WALL SYSTEMS, ALUMINUM WALL SYSTEMS, OPEN WEB STEEL JOISTS, STRUCTURAL STEEL CONNECTIONS REQUIRING ENGINEERING, WINDOWS, DOORS.

SUBMITTALS SHALL CLEARLY IDENTIFY THE SPECIFIC PROJECT AND APPLICABLE CODES, LIST THE DESIGN CRITERIA, AND SHOW ALL DETAILS AND PLANS NECESSARY FOR PROPER FABRICATION AND INSTALLATION. CALCULATIONS AND SHOP DRAWINGS SHALL IDENTIFY SPECIFIC PRODUCT UTILIZED. GENERIC PRODUCTS WILL NOT BE ACCEPTED.

SHOP DRAWINGS AND CALCULATIONS SHALL BE PREPARED UNDER THE DIRECT SUPERVISION AND CONTROL OF THE DELEGATED ENGINEER.

SHOP DRAWINGS AND CALCULATIONS ARE REQUIRED TO BE DATED, SIGNED AND SEALED BY THE DELEGATED ENGINEER. COMPUTER PRINTOUTS ARE AN ACCEPTABLE SUBSTITUTE FOR MANUAL COMPUTATIONS PROVIDED THEY ARE ACCOMPANIED BY SUFFICIENT DESCRIPTIVE INFORMATION TO PERMIT THEIR PROPER EVALUATION. SUCH DESCRIPTIVE INFORMATION SHALL BEAR THE IMPRESSED SEAL AND SIGNATURE OF THE DELEGATED ENGINEER AS AN INDICATION THAT HE/SHE HAS ACCEPTED RESPONSIBILITY FOR THE RESULTS. THE STRUCTURAL ENGINEER WILL RETAIN ONE SIGNED AND SEALED SET OF PLANS FOR RECORD.

DRAWINGS PREPARED SOLELY TO SERVE AS A GUIDE FOR FABRICATION AND INSTALLATION (SUCH AS REINFORCING STEEL, SHOP DRAWINGS OR STRUCTURAL STEEL ERECTION DRAWINGS) AND REQUIRING NO ENGINEERING, DO NOT REQUIRE THE SEAL OF A DELEGATED ENGINEER.

CATALOG INFORMATION ON STANDARD PRODUCTS DOES NOT REQUIRE THE SEAL OF A DELEGATED ENGINEER.

REVIEW BY THE STRUCTURAL ENGINEER OF RECORD OF SUBMITTALS IS LIMITED TO VERIFYING THE FOLLOWING:

- A) THAT THE SPECIFIED STRUCTURAL SUBMITTALS HAVE BEEN FURNISHED.
- B) THAT THE STRUCTURAL SUBMITTALS HAVE BEEN SIGNED AND SEALED BY THE DELEGATED ENGINEER.
- C) THAT THE DELEGATED ENGINEER HAS UNDERSTOOD THE DESIGN INTENT AND HAS USED THE SPECIFIED STRUCTURAL CRITERIA. (NO DETAILED CHECK OF CALCULATIONS WILL BE MADE).
- D) THAT THE CONFIGURATION SET FORTH IN THE STRUCTURAL SUBMITTALS IS CONSISTENT WITH THE CONTRACT DOCUMENTS. (NO DETAILED CHECK OF DIMENSIONS OR QUANTITIES WILL BE MADE).

SUBMITTALS NOT MEETING THE ABOVE CRITERIA WILL NOT BE REVIEWED.

FOUNDATIONS:

USE THE FOLLOWING REPORT FOR COMPLETE GEOTECHNICAL RECOMMENDATIONS AND INSTALLATION PROCEDURES. SITE PREPARATION AND FOUNDATION INSTALLATION SHALL COMPLY WITH:

REPORT NO.: 3730.2500030.0000
PREPARED BY: UES
TITLED: RESTAURANT-RETAIL BUILDINGS & FUTURE GYM.
DATED: 6600 N MILITARY TRAIL, BOCA RATON, FLORIDA
APRIL 2, 2025

FOUNDATION DESIGN IS BASED ON A SOIL BEARING PRESSURE OF 3,000 PSF.

SOIL COMPACTION:

FOUNDATIONS ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 3,000 PSF. TOP-SOIL SHALL BE REMOVED TO A MINIMUM DEPTH OF ONE FOOT OVER THE ENTIRE BUILDING AREA AND 5 FEET MINIMUM BEYOND BUILDING LINES (INCLUDING ISOLATED COLUMNS). THE CLEAR SURFACES IN CONSTRUCTION AREAS SHOULD BE PROOF-ROLLED USING THE APPROPRIATE COMPACTION EQUIPMENT FOR SITE AND SOIL CONDITIONS. THE SUBSEQUENT FILL TO THE UNDERSIDE OF THE GROUND FLOOR SLAB SHALL BE ADDED IN LIFTS NOT EXCEEDING 12 INCHES AND EACH LIFT COMPACTED TO 98% OF A MODIFIED PROCTOR DENSITY DETERMINED IN ACCORDANCE WITH ASTM D 1557.

SITE PREPARATION AND GROUND IMPROVEMENT REQUIREMENTS SHALL COMPLY WITH THE FOLLOWING REPORT:

REPORT NO.: 3730.2500030.0000
PREPARED BY: UES
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VERIFICATION THAT THE COMPACTION REQUIREMENTS HAVE BEEN MET SHALL BE MADE BY AN INDEPENDENT GEOTECHNICAL CONSULTANT EMPLOYED BY THE OWNER AND APPROVED BY THE ENGINEER. LOCATIONS FAILING TO MEET THE REQUIREMENTS SHALL BE RECOMPACTED AND RETESTED AT THE CONTRACTOR'S EXPENSE AND AS DIRECTED BY THE INDEPENDENT GEOTECHNICAL CONSULTANT.

FORMWORK AND SHORING:

NO STRUCTURAL CONCRETE SHALL BE STRIPPED UNTIL IT HAS REACHED AT LEAST TWO THIRDS OF THE 28 DAY DESIGN STRENGTH. DESIGN, ERECTION AND REMOVAL OF ALL FORMWORK, SHORES AND RESHORES SHALL MEET THE REQUIREMENTS SET FORTH IN ACI STANDARDS 347 AND 301.

PLUMBING SLEEVES:

MINIMUM SLEEVE SPACING SHALL BE THREE DIAMETERS CENTER TO CENTER OF THE LARGER SLEEVE OR 6" CLEAR BETWEEN SLEEVES, WHICHEVER IS GREATER. PRIOR TO CONSTRUCTION SLEEVE LOCATIONS AND SIZES SHALL BE APPROVED BY THE ENGINEER.

CONSTRUCTION JOINTS:

ANY DEVIATION OR ADDITION OF CONSTRUCTION JOINT FROM THAT SHOWN ON THE PLANS, MUST BE REVIEWED BY THE ENGINEER, ALTERNATE OR ADDED CONSTRUCTION JOINT LOCATIONS ARE ACCEPTABLE ONLY AS A CHANGE ORDER, WHICH WILL INCLUDE ENGINEERING CHARGES BY THE ENGINEER OF RECORD FOR REDESIGN OF THE STRUCTURE, SHORING, ETC.

REINFORCING STEEL:

SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. SECURE APPROVAL OF SHOP DRAWINGS PRIOR TO COMMENCING FABRICATION.

GALVANIZED REINFORCING STEEL:

REINFORCING BARS AT EXPOSED AREAS SUCH AS BALCONIES AND AS SHOWN ON STRUCTURAL PLANS SHALL BE HOT DIPPED GALVANIZED ACCORDING TO ASTM A767/A767M.

B REINFORCEMENT COVER

COVER FOR REINFORCEMENT SHALL NOT BE LESS THAN THE FOLLOWING:

CONCRETE COVER, IN

- A) CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3"
- B) CONCRETE EXPOSED TO EARTH OR WEATHER:
 - No. 6 THROUGH No. 18 BARS 2"
 - No. 5 BAR, W31 OR D31 WIRE, AND SMALLER 1.5"
- C) CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND: SLABS, WALLS, JOIST:
 - No. 14 AND NO. 18 BARS 1.5"
 - No. 11 BAR AND SMALLER 0.75"
- D) BEAMS, COLUMNS:
 - PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS 1.5"
- E) SHELLS, FOLDED PLATE MEMBERS:
 - No. 6 BAR AND LARGER 0.75"
 - No. 5 BAR, W31 OR D31 WIRE, AND SMALLER 0.5"

WELDED WIRE FABRIC:

TO CONFORM TO ASTM A-185, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. MINIMUM LAP SHALL BE ONE SPACE PLUS TWO INCHES. USE OF FLAT MANUFACTURED SHEETS IS RECOMMENDED.

CONCRETE:

CONCRETE SHALL BE PER AN APPROVED MIX DESIGN PROPORTIONED TO ACHIEVE A STRENGTH AT 28 DAYS AS LISTED BELOW WITH A PLASTIC AND WORKABLE MIX:

3000 PSI FOR FOUNDATIONS AND SLABS ON GRADE.
4000 PSI FOR OTHER STRUCTURAL CONCRETE.

CONCRETE SHALL BE PLACED AND CURED ACCORDING TO ACI STANDARDS AND SPECIFICATIONS.

WHERE SPECIFIED CONCRETE STRENGTH OF COLUMN IS GREATER THAN THE SPECIFIED SLAB CONCRETE STRENGTH, HIGHER STRENGTH CONCRETE SHALL BE PUDDLED AT THE COLUMN. THE STRENGTH OF PUDDLED CONCRETE SHALL BE AT LEAST 0.72 TIMES THE STRENGTH OF COLUMN CONCRETE.

SUBMIT PROPOSED MIX DESIGN WITH RECENT FIELD CYLINDER OR LAB TESTS FOR REVIEW PRIOR TO USE. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. MIX SHALL MEET THE REQUIREMENTS OF ASTM C33 FOR COARSE AGGREGATE. CONCRETE SHALL COMPLY WITH THE REQUIREMENTS OF ASTM STANDARD C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME STAMPED WHEN CONCRETE IS BATCHED. THE MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE HALF (1-1/2) HOURS. IF FOR ANY REASON THERE IS A LONGER DELAY THAN THAT STATED ABOVE, THE CONCRETE SHALL BE DISCARDED. IT SHALL BE THE RESPONSIBILITY OF THE TESTING LAB TO NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NONCOMPLIANCE WITH THE ABOVE. SLABS SHALL BE CURED USING A DISSIPATING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE 1-D AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE. SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAILY. CALCIUM CHLORIDES SHALL NOT BE UTILIZED; OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE ENGINEER.

CONCRETE MIX DESIGNS SHALL INCLUDE A WRITTEN DESCRIPTION INDICATING WHERE EACH PARTICULAR MIX IS TO BE PLACED WITHIN THE STRUCTURE. IF ACCEPTED, PEA ROCK PUMP MIX USE IS LIMITED TO VERTICAL ELEMENT POURS AND BEAM POURS LESS THAN 60 LINEAL FEET PER POUR.

CONCRETE DESIGN MIX SUBMITTALS SHALL INCLUDE TESTED, STATISTICAL BACK-UP DATA AS PER CHAPTER 5 OF ACI 318.

WATER/CEMENT RATIO FOR CONCRETE AT EXTERIOR BALCONIES SHALL NOT EXCEED 0.40 BY WEIGHT.

WEATHER RESISTANCE:

WHEREVER CONCRETE BALCONIES (OR OTHER FLAT SURFACES) OF THE REINFORCED OR POST-TENSIONED CONCRETE SLABS ARE TO REMAIN EXPOSED TO THE WEATHER THROUGHOUT THE LIFE OF THE BUILDING, THE TOP SURFACE OF THE CONCRETE SHALL BE TREATED WITH A CLEAR NONFLAMMABLE PENETRATING SEALER OF THE ALKYL ALKOXY SILANE CLASSIFICATION, SUCH AS MASTERPROTECT H 200, OR OTHER ENGINEER APPROVED WEATHER RESISTANT SYSTEM. APPLICATION AND SURFACE PREPARATION SHALL BE IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS.

CONCRETE TESTING:

AN INDEPENDENT TESTING LABORATORY SHALL PERFORM THE FOLLOWING TESTS ON CAST IN PLACE CONCRETE:

- A) ASTM C143 - "STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE." MAXIMUM SLUMP SHALL BE 4" TO 6" INCHES.
- B) ASTM C39 - "STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS." A SEPARATE TEST SHALL BE CONDUCTED FOR EACH CLASS, FOR EVERY 50 CUBIC YARDS (OR FRACTION THEREOF), PLACED PER DAY, REQUIRED CYLINDER(S) QUANTITIES AND TEST AGE AS FOLLOWS:

1 AT 3 DAYS
1 AT 7 DAYS
2 AT 28 DAYS

ONE ADDITIONAL RESERVE CYLINDER TO BE TESTED UNDER THE DIRECTION OF THE ENGINEER, IF REQUIRED. IF 28 DAY STRENGTH IS ACHIEVED, THE ADDITIONAL CYLINDER(S) MAY BE DISCARDED.

PENETRATIONS:

NO PENETRATIONS SHALL BE MADE IN ANY STRUCTURAL MEMBERS OTHER THAN THOSE LOCATED ON THE STRUCTURAL DRAWINGS WITHOUT PREVIOUS APPROVAL OF THE ENGINEER. CONTRACTOR SHOULD SUBMIT SLAB DRAWINGS INDICATING ANY CONCENTRATION OF PIPES, OPENINGS OR PENETRATIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS PRIOR TO CONCRETE POURS.

CHEMICAL (ADHESIVE) ANCHORS:

ADHESIVE ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308 FOR CRACKED AND UNCRACKED CONCRETE. ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE HAVING A MINIMUM AGE OF 21 DAYS. HOLES SHALL BE DRY AT THE TIME OF INSTALLATION. ACI 355.4 TEMPERATURE CATEGORY 'B' ASSUMED IN DESIGN. PRIOR TO INSTALLATION OF ADHESIVE ANCHORS IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATIONS RESISTING SUSTAINED TENSION LOADS, INSTALLERS ARE REQUIRED TO BE CERTIFIED IN ACCORDANCE WITH THE ACI/CRS ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM AND MUST BE CONTINUOUSLY INSPECTED.

PRODUCTS SHALL BE AN EQUAL TWO-PART EPOXY POLYMER INJECTION SYSTEM, SUCH AS SIMPSON "SET-3G", RAMSET "EPGON", HILTI "HIT HY", OR ENGINEER APPROVED SUBSTITUTION. INSTALLED IN ACCORDANCE WITH MANUFACTURERS INSTRUCTIONS. INSTALLERS SHALL BE TRAINED BY THE MANUFACTURER'S REPRESENTATIVE.

MASONRY WALLS:

MASONRY UNITS SHALL MEET ASTM C 90 FOR HOLLOW LOAD BEARING TYPE MASONRY WITH UNIT STRENGTH OF 2000 PSI ON THE NET AREA (Fm = 2000 PSI). MORTAR SHALL BE TYPE "M" OR "S" AND MEET ASTM C-270. GROUT SHALL BE 2,000 PSI MINIMUM COMPRESSIVE STRENGTH AND MEET ASTM C-476. PROVIDE HOOKED DOWELS IN FOOTINGS FOR VERTICAL REINFORCING ABOVE. LAP SPLICES 48 BAR DIAMETERS.

BLOCK CELLS SHALL BE GROUT FILLED WITH VERTICAL REINFORCING BARS AT CORNERS, INTERSECTIONS, EACH SIDE OF OPENINGS OVER 4 FEET WIDE, AND AS SHOWN ON THE PLANS. DOWELS SHALL BE USED TO PROVIDE CONTINUITY INTO THE STRUCTURE ABOVE AND/OR BELOW, UNLESS NOTED OTHERWISE. USE METAL LATH, MORTAR, OR SPECIAL UNITS TO CONFINED CONCRETE AND GROUT TO AREA REQUIRED. MASONRY SHALL BE LAID IN RUNNING BOND PATTERN UNLESS NOTED OTHERWISE.

PROVIDE 9 GAGE GALVANIZED HORIZONTAL JOINT REINFORCING [DUR-O-WALL OR ENGINEER APPROVED SUBSTITUTION] AT ALTERNATE BLOCK COURSES.

IF REQUIRED, CONTROL JOINTS SHALL BE PROVIDED IN CONCRETE MASONRY CONSTRUCTION AT LOCATIONS INDICATED ON THE ARCHITECTURAL DRAWINGS. HORIZONTAL WALL REINFORCING SHALL BE STOPPED EACH SIDE OF CONTROL JOINTS. SEE ARCHITECTURAL DRAWINGS FOR SEALANT REQUIREMENTS AT CONTROL JOINTS.

SUBMIT PROPOSED GROUT MIX DESIGN FOR REVIEW PRIOR TO USE. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. GROUT SLUMP SHALL BE BETWEEN 8 AND 11 INCHES. USE OF SUPERPLASTICIZER IS PROHIBITED.

CELLS TO BE GROUT FILLED SHALL HAVE VERTICAL ALIGNMENT SUFFICIENT TO MAINTAIN A CLEAR, UNOBSTRUCTED, CONTINUOUS VERTICAL GROUT SPACE. CLEANOUT OPENINGS SHALL BE PROVIDED AT THE BOTTOM OF CELLS TO BE GROUT FILLED IN EACH POUR IN EXCESS OF 5 FEET IN HEIGHT. ANY OVERHANGING MORTAR OR OTHER OBSTRUCTION OR DEBRIS SHALL BE REMOVED FROM THE INSIDES OF SUCH CELL WALLS. THE CLEANOUTS SHALL BE SEALED BEFORE GROUTING, AFTER INSPECTION.

VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 192 BAR DIAMETERS. CELLS CONTAINING REINFORCEMENT SHALL BE FILLED SOLIDLY WITH GROUT. GROUT SHALL BE POURED IN LIFTS OF 4 FEET MAXIMUM HEIGHT. GROUT SHALL BE CONSOLIDATED AT TIME OF PLACING BY VIBRATING AND RECONSOLIDATED LATER BY VIBRATING BEFORE PLASTICITY IS LOST.

WHEN TOTAL GROUT POUR EXCEEDS 5 FEET IN HEIGHT, THE GROUT SHALL BE PLACED IN 4 FOOT LIFTS. MINIMUM CELL DIMENSION SHALL BE IN ACCORDANCE WITH TABLE 5 OF ACI 530.1 (3' X 3' FOR COARSE GROUT, 12 FT. MAXIMUM POUR HEIGHT).

WHEN THE GROUTING IS STOPPED FOR ONE HOUR OR LONGER, HORIZONTAL CONSTRUCTION JOINTS SHALL BE MADE BY STOPPING THE POUR OF GROUT NOT LESS THAN 1-1/2 INCH BELOW THE TOP OF THE UPPERMOST UNIT GROUTED.

TIE BEAMS:

BEAMS WITH THE PREFIX "TB" SHALL BE OF CONCRETE, POURED AFTER THE BLOCK WALLS BELOW ARE IN PLACE. REINFORCING SHALL BE CONTINUOUS THROUGH THE BEAMS WITH MINIMUM LAP SPLICES OF 48 BAR DIAMETERS AND BENT BARS AT CORNERS. USE METAL LATH, MORTAR, OR SPECIAL UNITS TO CONFINED CONCRETE TO AREA REQUIRED, IN ACCORDANCE WITH ACI 530.1 (SOLID METAL OR FELT CAVITY CAPS ARE PROHIBITED).

CAST IN PLACE LINTELS:

MASONRY OPENINGS LESS THAN 16 FEET SHALL BE SPANNED WITH 8"x16" CONCRETE LINTELS WITH 2#5 REINFORCING BARS TOP AND BOTTOM & #3 TIES @12"O.C. MASONRY OPENINGS LESS THAN 12 FEET SHALL BE SPANNED WITH 8"x12" CONCRETE LINTELS WITH 2#5 REINFORCING BARS TOP AND BOTTOM & #3 TIES @12"O.C. MASONRY OPENINGS LESS THAN 6 FEET SHALL BE SPANNED WITH 8"x8" CONCRETE LINTELS WITH 2#5 REINFORCING BARS BOTTOM. LINTELS SHALL BEAR A MINIMUM OF 6" AT EACH END.

STRUCTURAL STEEL:

WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A-572 OR A-992 GRADE 50 AND OTHER SHAPES SHALL CONFORM TO ASTM A36 AND "THE SPECIFICATION FOR DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC. SHOP CONNECTIONS TO BE WELDED (UTILIZING 70XX ELECTRODES) AND FIELD CONNECTIONS TO BE BOLTED, UNLESS OTHERWISE NOTED ON STRUCTURAL DRAWINGS. STEEL SHALL RECEIVE ONE SHOP COAT AND ONE FIELD TOUCH UP COAT OF APPROVED PAINT, EXCEPT WHERE GALVANIZING IS INDICATED ON THE DRAWINGS.

STRUCTURAL TUBING SHALL CONFORM TO ASTM A-500, GRADE B, F_y = 46 KSI. STRUCTURAL PIPE SHALL CONFORM TO ASTM A-53 GRADE B, TYPE E OR S, F_y = 35 KSI. BEAM CONNECTIONS TO TUBE COLUMNS SHALL BE A.I.S.C. THRU-PLATE TYPE UNLESS SHOWN OTHERWISE.

BOLTED CONNECTIONS SHALL CONSIST OF MINIMUM 3/4 INCH DIAMETER ASTM A 325H HIGH STRENGTH BOLTS. BEAM CONNECTIONS SHALL BE DESIGNED BY THE FABRICATOR FOR THE REACTIONS SHOWN ON THE PLANS. IF NOT SHOWN, THE FABRICATOR SHALL DESIGN THE BEAM CONNECTIONS TO SUPPORT AN END REACTION OF W/2 KIPS FROM THE TABLES IN PART 2 "ALLOWABLE UNIFORM LOADS IN KIPS FOR BEAMS LATERALLY SUPPORTED" OF THE MANUAL OF STEEL CONSTRUCTION (9TH EDITION), BUT CONNECTIONS SHALL NOT HAVE LESS THAN 2 ROWS OF BOLTS. ANCHOR BOLTS SHALL CONFORM TO ASTM A 307 OR A 36 (THREADED ROD).

WELDING:

WELDING SHALL BE DONE BY WELDERS WITH CURRENT CERTIFICATION USING ASTM E70 SERIES ELECTRODE FOR SHOP WELDING A36 STEEL, AND E70 SERIES LOW HYDROGEN ELECTRODES FOR ALL WELDING OF HIGH STRENGTH STEELS AND FOR FIELD WELDING.

WELDS SHOWN ON STRUCTURAL DRAWINGS ARE MINIMUM DESIGN REQUIREMENTS. THE FABRICATOR'S SHOP DRAWINGS SHALL REFLECT WELDS IN ACCORDANCE WITH AWS REQUIREMENTS.

FULL PENETRATION GROOVE WELDS SHALL BE INSPECTED BY ULTRASONIC TESTING. TWENTY-FIVE PERCENT OF THE WELDS SHALL BE INSPECTED AT RANDOM UNLESS NOTED OTHERWISE. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

UNLESS NOTED OTHERWISE ON THE DRAWINGS, GROOVE WELDS SHALL BE FULL PENETRATION.

PROVIDE FILLET WELDS AT CONTACT POINTS BETWEEN STEEL MEMBERS SUFFICIENT TO DEVELOP THE ALLOWABLE TENSILE STRENGTH OF THE SMALLER MEMBER AT THE JOINT UNLESS DETAILED OTHERWISE ON THE DRAWINGS.

WINDOW AND DOOR SYSTEMS:

ALL DOOR AND WINDOW SYSTEMS SHALL BE DESIGNATED AS SPECIALITY ENGINEERED ITEMS AND THE DESIGN OF THESE "SYSTEMS" AND THEIR CONNECTIONS TO THE STRUCTURE DEPICTED ON THESE PLANS SHALL BE THE RESPONSIBILITY OF THE SUPPLIER/SPECIALTY ENGINEER. SUBMIT PRODUCT APPROVAL INFORMATION (OR SIGNED AND SEALED CALCULATIONS) FOR APPROVAL, UNLESS INDICATED OTHERWISE (BY PRODUCT APPROVALOR CALCULATIONS). THE MINIMUM CONNECTIONS OF THE "BUCKS" (2x4 P.T. SYP, MIN.) TO THE STRUCTURE SHALL BE MADE WITH 1/2" DIAMETER EXPANSION BOLTS (3 1/2" MIN. EMBEDMENT AND EDGE DISTANCE) AT 12" C/C SPACING AND 6" C/C FROM EACH MILLIONS AND/OR CORNER. ALL DOOR AND WINDOW SYSTEMS SHALL BE DESIGNED TO SAFELY RESIST THE WIND PRESSURES SHOWN ON THIS SHEET. SUBMIT SHOP DRAWINGS VERIFYING CONFORMANCE.

CONTRACTOR QUALIFICATIONS:

THE QUALIFICATIONS OF BOTH THE GENERAL CONTRACTOR AND THE SHELL CONTRACTOR MUST BE SUBMITTED TO THIS OFFICE FOR REVIEW AND APPROVAL. SUBMISSIONS SHOULD INCLUDE THE FOLLOWING: A COPY OF EACH CONTRACTOR'S VALID STATE LICENSE, PROOF OF LIABILITY INSURANCE LISTING STRUCTURES INTERNATIONAL, INC. AS ADDITIONAL INSURED, AND A CURRENT PROJECT LIST DEMONSTRATING EXPERIENCE LISTING AT LEAST FIVE PROJECTS OF SIMILAR SCOPE AND SCALE, WITH WORK BEING PERFORMED WITHIN THE SAME GEOGRAPHIC REGION.

STEEL JOISTS:

SHALL BE THE SIZE AND SPACING AS SHOWN ON THE STRUCTURAL DRAWINGS AND SHALL BE DESIGNED, FABRICATED, INSTALLED AND BRIDGED IN ACCORDANCE WITH THE STEEL JOIST INSTITUTE SPECIFICATIONS, ENDS OF BRIDGING LINES TERMINATING AT WALLS OR BEAMS SHALL BE ANCHORED THERETO AT TOP AND BOTTOM CHORDS. MINIMUM JOIST BRIDGING TERMINATION CONNECTIONS TO MASONRY SHALL BE L3 X 3 X 1/4 X 3" LONG WITH (1) 1/2" DIAMETER ANCHOR BOLT OR 1/4 X 4 X 4 WITH (1) 1/2" X 5" HEADED STUD TO CONCRETE. BRIDGING SHALL BE WELDED OR BOLTED AT POINTS OF CONTACT. WELD SHALL NOT DAMAGE THE JOIST. CROSS BRIDGING SHALL BE WELDED OR BOLTED AT ITS CENTER POINT.

K-SERIES JOISTS SHALL BEAR A MINIMUM OF 2-1/2" ON STEEL BEAMS AND 4" ON CONCRETE BEAMS. JOIST BEARING PLATES TO BE MINIMUM 3/8" X 5' X 7-1/2" WITH (2) 1/2" DIAMETER X 5' SHEAR STUD CONNECTORS. BEARING PLATES FOR BACK TO BACK SINGLE JOISTS SHALL BE MINIMUM 3/8" X 7-1/2" X 7-1/2" WITH (4) 1/2" DIAMETER X 5' SHEAR STUD CONNECTORS. BEARING PLATES SHALL BE CAST INTEGRALLY WITH THE CONCRETE BEAM. WELD JOISTS TO BEARING PLATES WITH A MINIMUM OF (2) 1/8" FILLET WELDS, UNLESS NOTED OTHERWISE.

LH SERIES JOISTS SHALL BEAR A MINIMUM OF 4" ON STEEL BEAMS AND 6" ON CONCRETE BEAMS. JOIST BEARING PLATES TO BE MINIMUM 3/8" X 6' X 9" WITH (2) 1/2" DIAMETER X 5' SHEAR STUD CONNECTORS. BEARING PLATES FOR BACK TO BACK SINGLE JOISTS SHALL BE MINIMUM 3/8" X 9" X 11-5/8" WITH (4) 1/2" DIAMETER X 5' SHEAR STUD CONNECTORS. BEARING PLATES SHALL BE CAST INTEGRALLY WITH THE CONCRETE BEAM. WELD JOISTS TO BEARING PLATE WITH A MINIMUM OF (2) 1/4" FILLET WELDS. UNLESS NOTED OTHERWISE, BACK TO BACK JOISTS SHALL BE OFFSET IF CONCRETE BEAM IS LESS THAN 12" NOMINAL WIDTH OR STEEL BEAM IS LESS THAN 8" WIDE.

SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWING SUBMITTAL SHALL INCLUDE LAYOUT, COMPONENT DESIGNATION, BRIDGING, AND PERTINENT SECTIONS AND DETAILS. SUBMITTALS FOR JOISTS, OTHER THAN STANDARD SJI CATALOG SELECTIONS WHICH HAVE BEEN CHECKED BY SJI, SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER.

JOIST BEARING:

WHERE STEEL BEAMS SUPPORT JOISTS FRAMING FROM ONE SIDE ONLY, OR WHERE JOISTS FROM ONE SIDE ARE 30% LONGER THAN THE JOISTS ON THE OPPOSITE OF THE BEAM, JOISTS SHALL BEAR FULL WIDTH OF THE STEEL BEAM OR 5-1/2", WHICHEVER IS LESS.

NET UPLIFT DESIGN:

STEEL ROOF JOISTS SHALL BE DESIGNED TO WITHSTAND NET UPLIFT INDICATED ON DRAWINGS. IN ADDITION TO THE STANDARD SJI BOTTOM CHORD BRIDGING, WHICH INCLUDES THE FIRST END PANELS, THE JOIST MANUFACTURER SHALL PROVIDE DESIGN CALCULATIONS FOR UPLIFT, EITHER CONFIRMING THE SJI BRIDGING REQUIREMENT OR PROVIDING A DESIGN ADEQUATE FOR THE UPLIFT. THIS SUBMITTAL SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER.

SHEET INDEX - RETAIL #1	
SHEET NUMBER	SHEET NAME
S-0	STRUCTURAL NOTES
S-0.1	WIND PRESSURES
S-0.2	ROOF CLADDING PRESSURES
S-0.3 & S-0.4	SCHEDULES
S-1	FOUNDATION LEVEL FLOOR PLAN
S-2	ROOF LEVEL FRAMING PLAN
S-3	COURTYARD FLOOR PLANS
S-4 TO S-4.2	SECTIONS & DETAILS
S-5	TYPICAL DETAILS

STEEL JOIST GIRDERS: SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS FOR JOIST GIRDERS AS PUBLISHED BY THE STEEL JOIST INSTITUTE (SJI). JOIST GIRDERS SHALL BE OF THE DEPTH AND SPACING SHOWN ON THE STRUCTURAL DRAWINGS, AND UNLESS OTHERWISE NOTED, JOIST GIRDERS SHALL BE DESIGNED AS SIMPLY SUPPORTED PRIMARY MEMBERS PROPORTIONED SUCH THAT THEY MAY BE ERECTED WITHOUT BRIDGING. JOIST GIRDER PANEL POINTS SHALL BE ALIGNED WITH SECONDARY MEMBERS.

CONCRETE SUPPORTS: UNLESS NOTED OTHERWISE ON STRUCTURAL DRAWINGS, JOIST GIRDERS SHALL BEAR ON 5/8" X 7" X 1'-0" STEEL BEARING PLATES WITH A MINIMUM OF FOUR 1/2" DIAMETER X 6" LONG SHEAR STUD CONNECTORS (CONFORMING TO AWS D1.1). THE CONNECTION TO THE BEARING PLATE SHALL BE DESIGNED TO RESIST UPLIFT AND SHEAR LOADS; HOWEVER, A MINIMUM OF TWO 1/4" FILLET WELDS TWO 3/4" DIAMETER HIGH STRENGTH BOLTS SHALL BE UTILIZED. MINIMUM JOIST GIRDER BEARING SHALL BE 6".

STEEL SUPPORTS: UNLESS NOTED OTHERWISE ON STRUCTURAL DRAWINGS, JOIST GIRDERS MAY BE DIRECTLY CONNECTED TO STEEL GIRDERS HAVING A FLANGE THICKNESS GREATER THAN 3/8". THE CONNECTION SHALL BE DESIGNED TO RESIST UPLIFT AND SHEAR LOADS; HOWEVER, A MINIMUM OF TWO 1/4" FILLET WELDS TWO 3/4" DIAMETER HIGH STRENGTH BOLTS SHALL BE UTILIZED. MINIMUM JOIST GIRDER BEARING SHALL BE 4 INCHES.

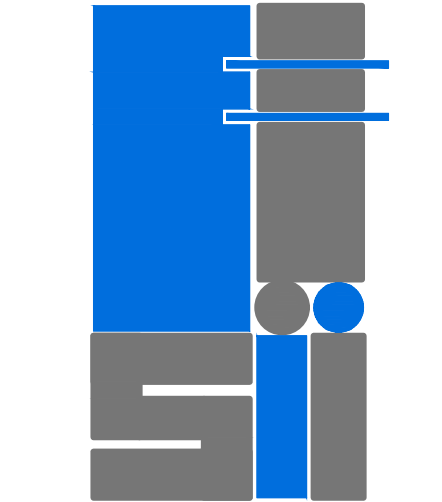
SUPPLIER SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR REVIEW BY THE STRUCTURAL ENGINEER PRIOR TO FABRICATION. SUBMITTALS SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER.

STEEL ROOF DECK:

SHALL BE GALVANIZED (G90) WIDE RIB STEEL ROOF DECK (WR) OF THE DEPTH AND GAGE AS NOTED ON PLANS AND SHALL CONFORM TO PROVISIONS OF THE STEEL DECK INSTITUTE (SDI) SPECIFICATIONS FOR NON-COMPOSITE STEEL FORM DECK. WELD PATTERN SHALL BE MINIMUM 30/4 UNLESS NOTED OTHERWISE.

LIGHT GAUGE STEEL FRAMING:

LIGHT GAUGE STEEL FRAMING SYSTEM SHALL BE DESIGNED IN ACCORDANCE WITH ACCEPTED ENGINEERING PRINCIPLES AND GOVERNING CODES. THE DESIGN SHALL BE PERFORMED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA. SHOP DRAWING SHALL BE SUBMITTED WHICH BEARS THE SIGNATURE, DATE, AND EMBOSSED SEAL OF THE ENGINEER. SHOP DRAWINGS SHALL CLEARLY INDICATE CONNECTIONS AND MATERIALS USED.



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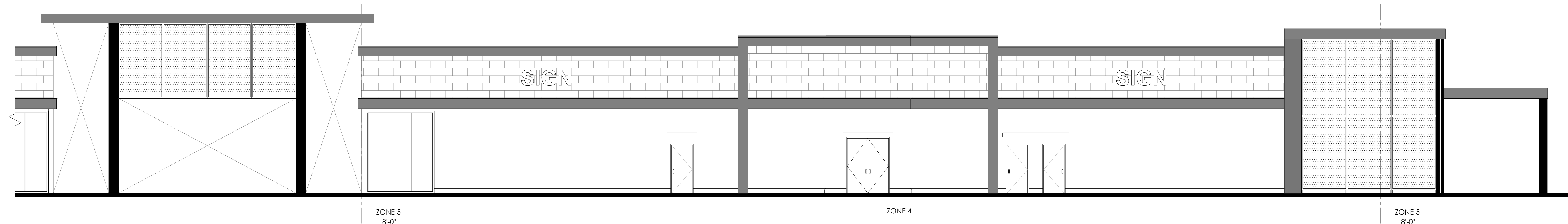
MONZER I. FARAMAWI
FLORIDA P.E. #57439

TO THE BEST OF MY KNOWLEDGE AND
ABILITY, THE COMPLETED STRUCTURE
DEPICTED ON THESE PLANS COMPLIES WITH
THE APPLICABLE MINIMUM BUILDING CODES.

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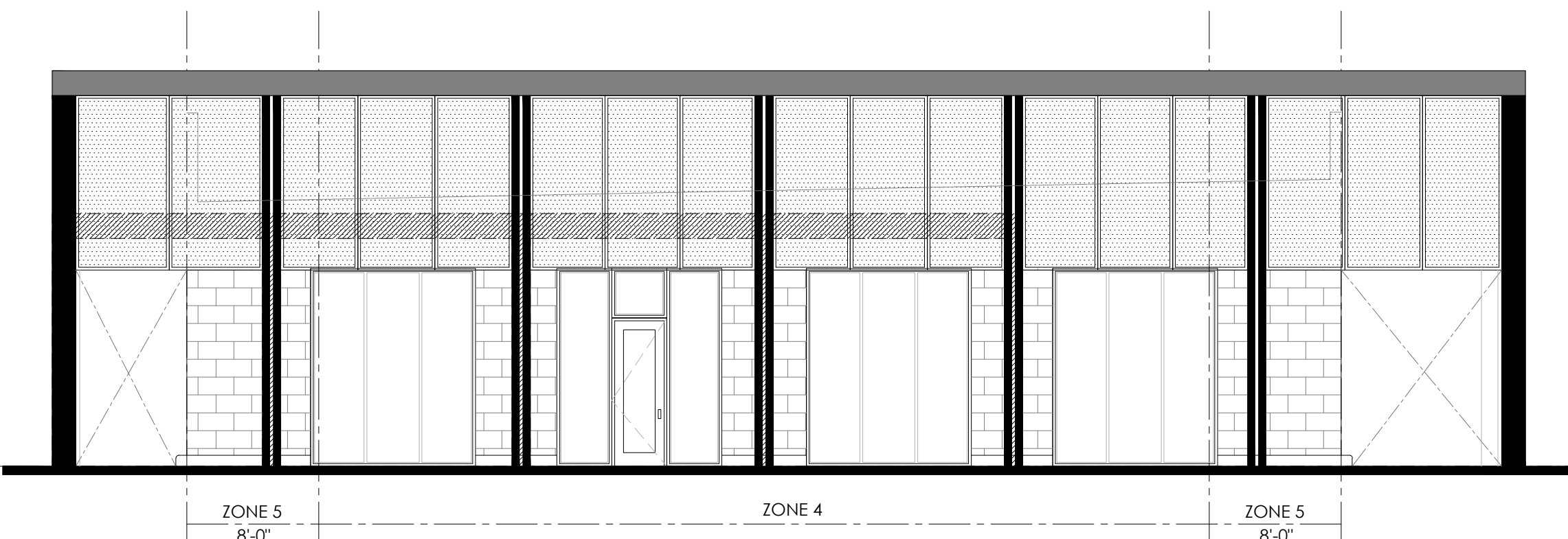
EAST ELEVATION (RETAIL 1)
1/8"=1'-0"



WEST ELEVATION (RETAIL 1)
1/8"=1'-0"



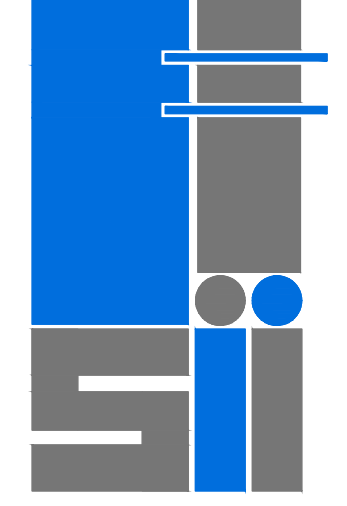
SOUTH ELEVATION (RETAIL 1)
1/8"=1'-0"



NORTH ELEVATION (RETAIL 1)
1/8"=1'-0"

WIND PRESSURE (ASD)				
AREA (FT²)	ZONE 4		ZONE 5	
	+ VE PRESSURE (PSF)	- VE PRESSURE (PSF)	+ VE PRESSURE (PSF)	- VE PRESSURE (PSF)
0-10	41	-45	41	-56
11-20	41	-45	41	-55
21-30	39	-43	39	-52
31-40	38	-42	38	-49
41-50	38	-41	38	-48
51-60	37	-41	37	-47
61-70	37	-40	37	-46
71-80	36	-40	36	-45
81-90	36	-39	36	-44
91-100	36	-39	36	-44
101-150	35	-39	35	-43
151-200	34	-38	34	-41

NOTE: FOR ALL DOORS & WINDOWS NOT VISIBLE IN THE ELEVATIONS,
REFER TO AREA TO DETERMINE CORRESPONDING PRESSURES.



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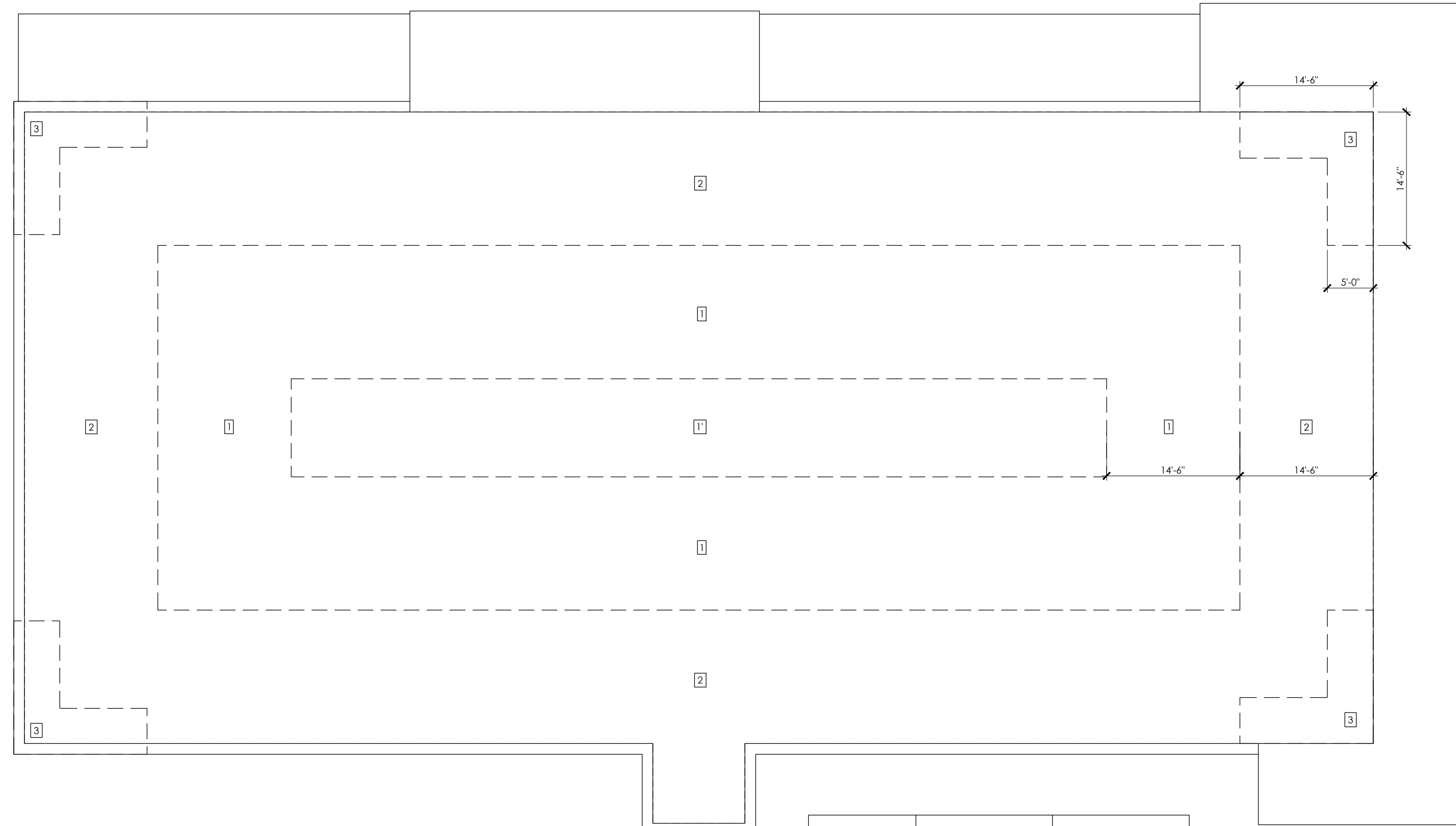
**BROKEN SOUND
RETAIL SHELL**
6600 NORTH MILITARY TRAIL
BOCA RATON, FL 33496

REVISIONS	

DRAWING TITLE: WIND PRESSURES RETAIL #1	
JOB No:	25-035
DATE:	10/15/2025
STATUS:	PERMIT SET
DRAWN:	A.F.
CHECKED:	M.I.F.

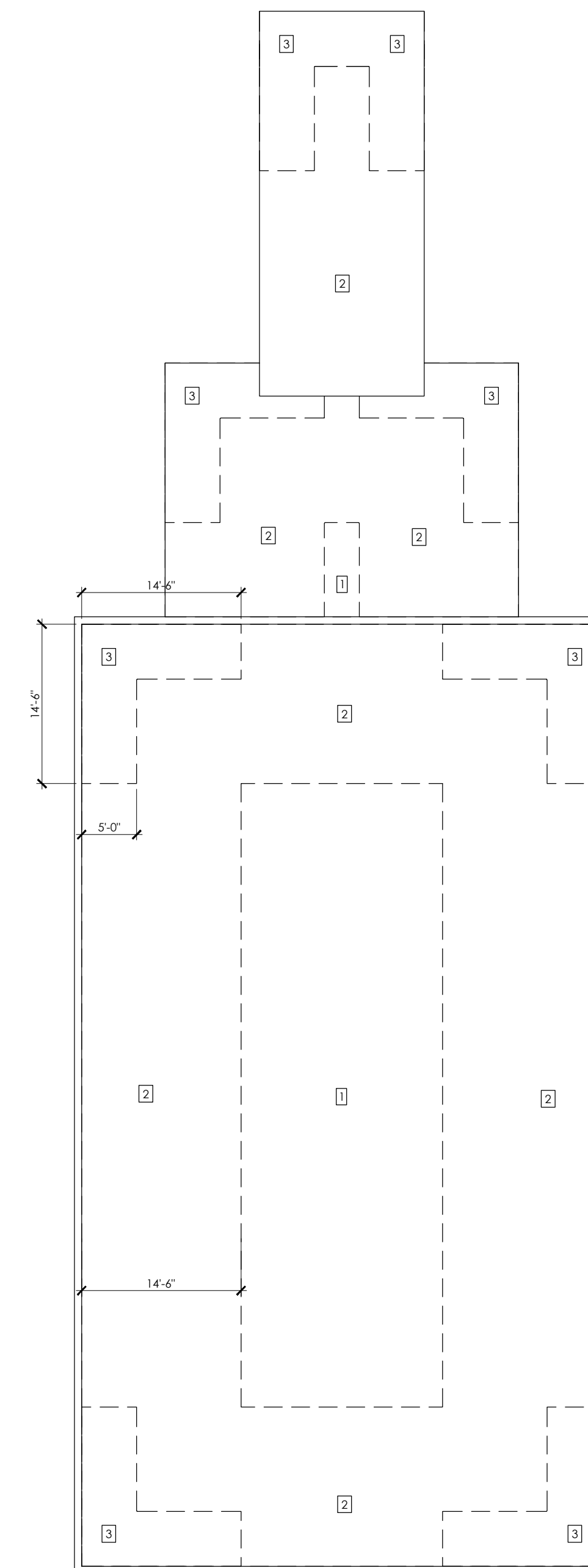
S-0.1

Received 6.26.26



<u>ZONE</u>	<u>ROOF UPLIFT</u>	<u>OVERHANG</u>
ZONE 1'	-38 PSF	-66 PSF
ZONE 1	-66 PSF	-66 PSF
ZONE 2	-87 PSF	-87 PSF
ZONE 3	-119 PSF	-119 PSF

ROOF COMPONENT AND CLADDING PRESSURES (RETAIL 1)
N.T.S

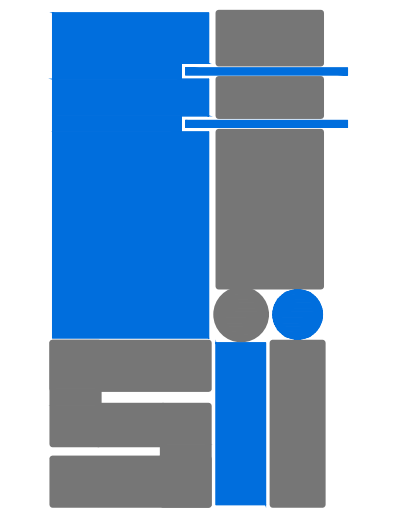


RETAIL CANOPY SHOWN FOR REFERENCE
ONLY. NOT PART OF THIS PERMIT SET. REFER
TO PERMIT # BCP-2026-00142



<u>ZONE</u>	<u>ROOF UPLIFT</u>	<u>OVERHANG</u>
ZONE 1'	-38 PSF	-66 PSF
ZONE 1	-66 PSF	-66 PSF
ZONE 2	-87 PSF	-87 PSF
ZONE 3	-119 PSF	-119 PSF

ROOF COMPONENT AND CLADDING PRESSURES (COURTYARD)
N.T.S



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REVISIONS

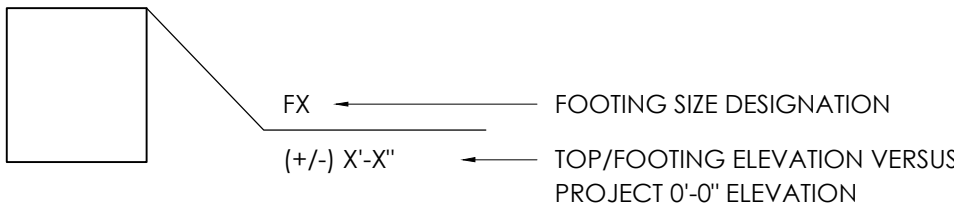
[illegible]

DRAWING TITLE:	
ROOF CLADDING	
PRESSURES	
RETAIL #1	
JOB No:	25-035
DATE:	10/15/2025
STATUS:	PERMIT SET
DRAWN:	A.F.
CHECKED:	M.I.F

S-0.2

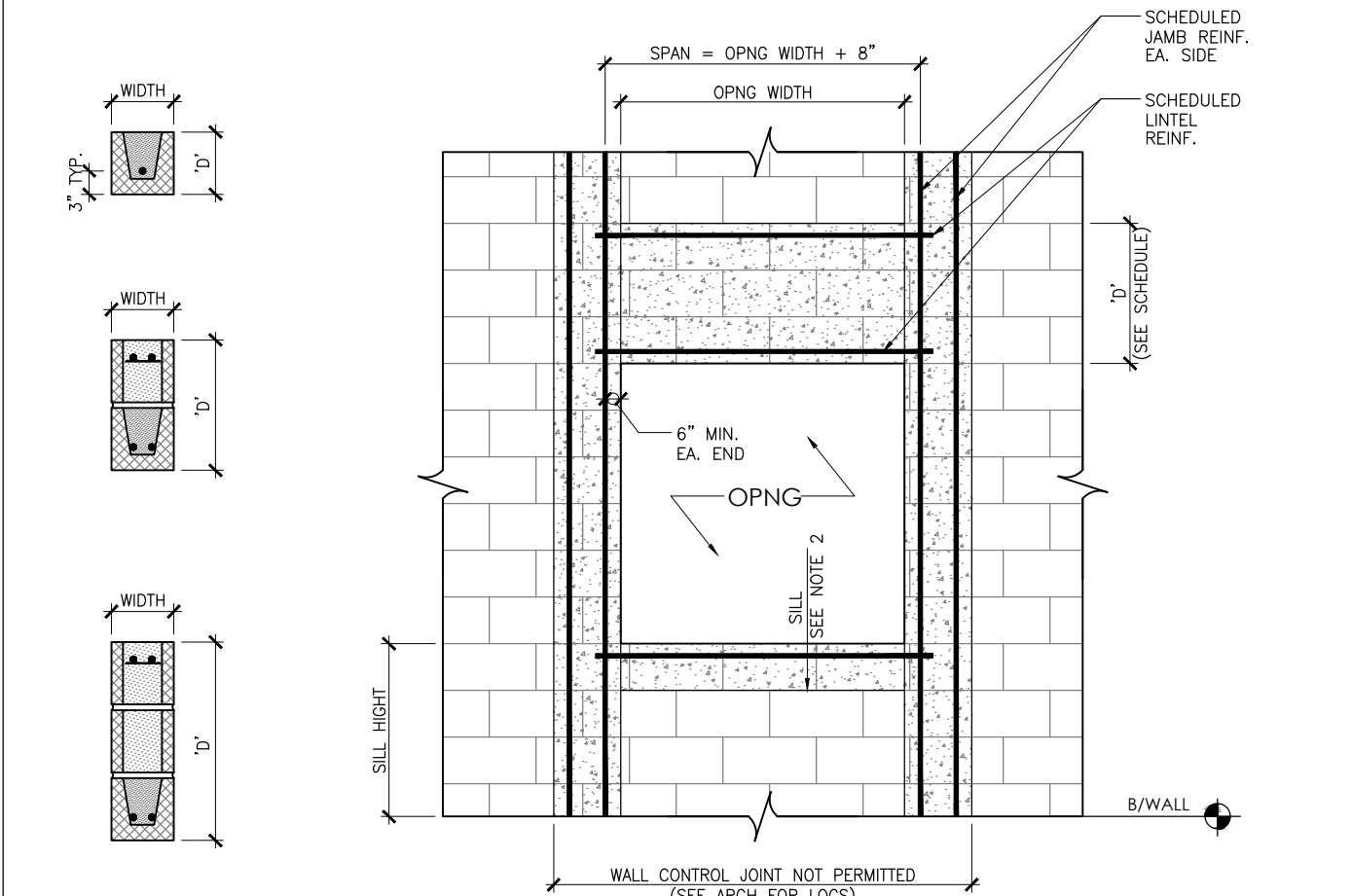
ABBREVIATION LEGEND

* @	AT	EXP	EXPANSION	O.H.	OVERHANG - OR -
* A.B.	ANCHOR BOLT	EXT.	EXTERIOR	OPP.	OPPOSITE HAND
* ABV	ABOVE	EXTR	EXTERIOR	OPNG	OPENING
* ACI	AMERICAN CONCRETE INSTITUTE	FDN	FOUNDATION	* O.S.B.	ORIENTED STRAND BOARD
ADD'L	ADDITIONAL	F.F.	FINISH FLOOR	* O.W.S.J.	OPEN WEB STEEL JOIST
A.F.F.	ABOVE FINISHED FLOOR	* FIN.	FINISH	* O.W.J.G.	OPEN WEB JOIST GIRDER
* AGGREG.	AGGREGATE	* F.L.	FLOOR	* P.A.	POST ABOVE
* AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	* FLR	FLOOR	* P.A.F.	POWDER-ACTUATED FASTENER
* AITC	AMERICAN INSTITUTE OF TIMBER CONSTRUCTION	* F.O.B.	FACE OF BLOCK	* P.C.F.	POUNDS PER CUBIC FOOT
ALTN	ALTERNATE	* F.N.	FACE NAIL	* PEN.	PENETRATION
ARCH.	ARCHITECT(URAL)	* F.O.C.	FACE OF CONCRETE	* PERF.	PERFORATED
ASTM	AMERICAN SOCIETY FOR TESTING & MATERIALS	* F.O.M.	FACE OF MASONRY	* PERP.	PERPENDICULAR
AESS	ARCHITECTURAL EXPOSED STRUCTURAL STEEL	* F.O.S.	FACE OF STUD	PL.	PLATE
AISI	AMERICAN IRON AND STEEL INSTITUTE	* F.P.	FIREPLACE - OR - FULL PENETRATION	* PLY.	PLYWOOD
AR	ANCHOR ROD	* FRMD	FRAMED	* PLYWD	PLYWOOD
ASD	ALLOWABLE STRESS DESIGN	* FRMG	FRAMING	* PR	PAIR
AWS	AMERICAN WELDING SOCIETY	* F.S.	FACE SIDE	* P.S.F.	POUNDS PER SQUARE FOOT
B/	BOTTOM OF	* FT	FOOT	* P.S.I.	POUNDS PER SQUARE INCH
* B.C.	BOTTOM CHORD	* FTG	FOOTING	* P.S.L.	PARALLEL-STRAND LUMBER
BLDG	BUILDING	F/	FACE OF	* PT	POINT
* BLK	BLOCK	FD	FLOOR DRAIN	P.T.	PRESSURE TREATED - OR - POST-TENSION
* BLKG	BLOCKING	FRT	FIRE RETARDANT TIMBER	PEMB	PRE-ENGINEERED METAL BUILDING
* BLW	BELOW	FV	FIELD VERIFY	PJF	PREFORMED JOINT FILLER
BM	BEAM	* G & N	GLUE AND NAIL	PLF	POUNDS PER LINEAL FOOT
* B.N.	BOUNDARY NAILING	GA.	GAGE, GAUGE	PPHCC	PRESTRESSED PRECAST HOLLOW CORE CONCRETE
* B.O.	BY OTHERS - OR - BOTTOM OF	GALV.	GALVANIZED	PREFAB	PRE-FABRICATED
* B.O.C.	BOTTOM OF CONCRETE	* G.B.	GRADE BEAM	QTY	QUANTITY
* B.O.F.	BOTTOM OF FOOTING	GENL.	GENERAL	RAD	RADIUS
* B.O.S.	BOTTOM OF STEEL	* GLB	GLUED-LAMINATED BEAM	* RECM'D	RECOMMEND(ATION)
BRG	BEARING	* GR.	GRADE	REF.	REFERENCE
* B.S.	BOTH SIDES	* G.T.	GIRDER TRUSS	REINF.	REINFORCED - OR - REINFORCEMENT
* B.T.	BANDED TENDONS	GYP.	GYPSON	REQD	REQUIRED
BOT	BOTTOM	* G.W.B.	GYPSON WALL BOARD	* RET.	RETAINING
BETW	BETWEEN	GC	GENERAL CONTRACTOR	* RETAIN.	RETAINING
BD	BOARD	GDR	GIRDER	* R.J.	ROUGH OPENING
BP	BASE PLATE	* HD	HOLDOWN	* R.O.	ROUGH OPENING
BRDG	BRIDGING	* H.D.G.	HOT-DIP GALVANIZED	R.R.	ROUGH RATER
* CALCS	CALCULATIONS	HDR	HEADER	* R/S	RESAWN (ROUGH-SAWN)
* CANT.	CANTILEVER	* H.H.N.	HEAVY HEX NUT	* R.W.	RETAINING WALL
* C.B.C.	CALIFORNIA BUILDING CODE	H.N.	HEAVY NUT	RWD	REDWOOD
C/C	CENTER TO CENTER	HORIZ.	HORIZONTAL	RD	ROOF DRAIN
* C.C.J.	CRACK CONTROL JOINT	* H.S.B.	HIGH STRENGTH BOLT	REV	REVISION
* C.F.	CUBIC FOOT	H.T.	HEAVY TIMBER	RTU	ROOF TOP UNIT
* C.J.	CEILING JOIST CENTERLINE	HCA	HEADED CONCRETE ANCHOR	* S.A.D.	SEE ARCHITECTURAL DRAWINGS
* CLG	CLEAR	HKD	HOKED	* S.B.	SOLID BLOCK
CLR	CLEAR	HSS	HOLLOW STRUCTURAL SECTION	* S.B.T.	SET-BACK TRUSS
* C.M.U.	CONCRETE MASONRY UNIT	I.D.	INSIDE DIAMETER	SCHED.	SCHEDULE
* COLL.	COLUMN	* INFO.	INFORMATION	* SEP.	SEPARATION
* CONC.	CONCRETE	INT.	INTERIOR	* S.G.T.	SUB-GIRDER TRUSS
* COND.	CONDITION	* INV.	INVERT(ED)	* SHT	SHEET
CONN.	CONNECTION	IE	INVERT ELEVATION	SIM.	SIMILAR
* CONSTR.	CONSTRUCTION	INSUL	INSULATION OR INSULATING	* S.M.S.	SHEET METAL SCREW
CONT.	CONTINUOUS	IT	JOINT	* S.J.	SOFFIT JOIST
* CONTIN.	CONTINUOUS	JST	JOIST	SPEC.	SPECIFICATION
* CONTR.	CONTRACTOR	K	KIPS (KILOPOUNDS)	* SQ.	SQUARE
* C.P.	COMPLETE PENETRATION WELD	* K.D.	KILN-DRIED	S.S.	STAINLESS STEEL
* C.S.	COUNTERSUNK	KLF	KIPS PER LINEAL FOOT	* S4S	SURFACED 4 SIDES
* C.T.	COLLECTOR TRUSS	* K.P.	KING POST	* STAGG.	STAGGER - OR - STAGGERED
CTR	CENTER	KSI	KIPS PER SQUIRE INCH	STD	STANDARD
* CTRD	CENTERED	KSF	KIPS PER SQUIRE FOOT	STIFF.	STIFFENER
* C.Y.	CUBIC YARD	* lbs	POUNDS	STL	STEEL
CFSF	COLD FORMED STEEL FRAMING	L.B.	LAC BOLT	* STRUCT.	STRUCTURAL
CL	CENTERLINE	* LF	LINEAL FOOT	SW	SHORT WAY
DBL	DOUBLE	* LONGIT.	LONGITUDINAL	SYM.	SYMMETRICAL
* DET.	DETAIL	LG	LONG	SER	STRUCTURAL ENGINEER OF RECORD
* D.F.	DOUGLAS FIR	L.L.	LIVE LOAD	SF	SQUIRE FOOT
DIA.	DIAMETER	* LSL	LAMINATED STRAND LUMBER	SHTHG	SHEATHING
DIA.G.	DIAGONAL	LT	LIGHT	SLH	SHORT LEG HORIZONTAL
* DIM.	DIMENSION	* LTWT	LIGHTWEIGHT	SLV	SHORT LEG VERTICAL
D.L.	DEAD LOAD	LVL	LAMINATED VENEER LUMBER	SPA	SPACES
* D.J.	DECK JOIST	L	LENGTH	* T + B	TOP AND BOTTOM
* D.T.	DRAG TRUSS	LFH	LONG FACE HORIZONTAL	* T + G	TONGUE AND GROOVE
D	DEEP	LFV	LONG FACE VERTICAL	* T.C.	TOP CHORD
* DTL	DETAIL	LLH	LONG LEG HORIZONTAL	THK.	THICK
DWG	DRAWING	LLV	LONG LEG VERTICAL	THKND	THICKENED
D&E	DRILL & EPOXY	LO	LOW	* T.N.	TOE NAIL
DBA	DEFORMED BAR ANCHOR	LOCs	LOCATIONS	* T.O.B.	TOP OF BLOCK
DEP	DEPRESSED	LRFD	LOAD RESISTANCE FACTORED DESIGN	* T.O.C.	TOP OF CONCRETE
DWL	DOWEL	LSH	LONG SIDE HORIZONTAL	* T.O.P.	TOP OF PLATE
DN	DOWN	LSV	LONG SIDE VERTICAL	* T.O.S.	TOP OF STEEL - OR - TOP OF SHEATHING
EXIST	EXISTING	LW	LONG WAY	* T.O.W.	TOP OF WALL
* E.B.	EXPANSION BOLT	LWC	LIGHT WEIGHT CONCRETE	T/	TOP OF
E.F.	EACH FACE	* MATL	MATERIAL	* T/P	TOP PLATE
E.J.	EXPANSION JOINT	MAX.	MAXIMUM	* TRANSV.	TRANSVERSE
ELEV.	ELEVATION	* M.B.	MACHINE BOLT	TS	STRUCTURAL TUBE
* EMB.	EMBEDMENT	* MECH.	MECHANICAL	TYP.	TYPICAL
* EMBED.	EMBEDMENT	* MEMB.	MEMBRANE	T & B	TOP & BOTTOM
* EN.	EDGE NAIL	* MID.	MIDDLE	T & G	TONGUE & GROOVE
* EQUIP.	EQUIPMENT	MIN.	MINIMUM	TEMP	TEMPORARY
* E.S.	EACH SIDE	* M.I.W.	MALLEABLE IRON WASHER	THRU	THROUGH
E.W.	EACH WAY	* M.O.	MASONRY OPENING	U.N.O.	UNLESS NOTED OTHERWISE
ENG	ENGINEER OR ENGINEERING	* MOIST.	MOISTURE	VERT.	VERTICAL
EOS	EDGE OF SLAB	MFR	MANUFACTURER	* V.I.F.	VERIFY IN FIELD
EQ	EQUAL	* MFRD	MANUFACTURED	W/	WITH
		MTL	METAL	WD	WOOD
		* MEZ	MEZZANINE	W.P.	WORK POINT - OR - WATERPROOF(ING)
		MISC	MISCELLANEOUS	* WT	WEIGHT
		* (N)	NOT APPLICABLE	* W.W.F.	WELDED WIRE FABRIC
		* N.A.	NON-DESTRUCTIVE TESTING	* W.W.M.	WELDED WIRE MESH
		* N.D.T.	NOT IN CONTRACT	W	WIDE
		* N.I.C.	NON-DESTRUCTIVE TESTING	W/O	WITHOUT
		* NLG	NUMBER	WWR	WELDED WIRE REINFORCEMENT
		* No.	NUMBER	#	NUMBER
		N.S.	NEAR SIDE		
		* N.S.G.	NON-SHRINK GROUT		
		N.T.S.	NOT TO SCALE		
		* O/	OVER		
		O.C.	ON CENTER		
		O.D.	OUTSIDE DIAMETER		
		* O.F.O.S.	OUTSIDE FACE OF STUD		



CONCRETE MASONRY UNIT LINTEL & JAMB SCHEDULE

SPAN	LINTEL DEPTH 'D'	LINTEL REINFORCING 8" TO 12" WALL	JAMB REINFORCING (SEE NOTE 5)	
<4'-0"	8"	(1)#5	(1)#5	
<4'-0" TO < 6'-0"	8"	(1)#5	(1)#5	
<6'-0" TO < 8'-0"	16"	(2)#5	(2)#5	



SECTIONS

ELEVATION

NOTES

1. LINTELS SHALL BE CONSTRUCTED WITH U-BLOCKS AT THE BOTTOM AND DEPRESSED WEB BLOCKS ABOVE AND SHALL BE FULLY GROUTED.
2. SILL REINFORCING REQUIREMENTS:
 - a. OPENING WIDTH < 4'-0" AND SILL HEIGHT < 3'-0" SILL REINFORCING NOT REQUIRED.
 - b. OPENING WIDTH < 4'-0" OR SILL HEIGHT > 3'-0" PROVIDE REINFORCED LINTEL ACCORDING SCHEDULE.
3. NOTIFY ENGINEER IF OPENING WIDTH EXCEEDS SCHEDULED WIDTHS.
4. SEE CONCRETE MASONRY NOTES, WALL REINFORCING SCHEDULE AND KEYED SECTIONS AND DETAILS FOR ADDITIONAL REINFORCING NOT SHOWN ON THIS SCHEDULE.
5. PROVIDE (1) BAR PER CELL IN JAMBS, TYPICAL. WHERE SCHEDULED JAMB REINFORCING EXCEEDS THE NUMBER OF CELLS, PROVIDE (2) BARS PER CELL LOCATED 3/4" CLEAR FROM EACH FACE SHELL.

CLASS 'B' TENSION LAP SPLICE LENGTH (IN)
(ACI 318 25.4.2 & 25.5.2.1)

BAR SIZE	3000 PSI	4000 PSI	5000 PSI	6000 PSI	7000 PSI
#3	21"	19"	17"	16"	16"
#4	29"	25"	22"	20"	19"
#5	36"	31"	28"	25"	23"
#6	43"	37"	33"	30"	28"
#7	62"	54"	48"	44"	41"
#8	71"	62"	55"	50"	47"
#9	80"	70"	62"	57"	53"
#10	89"	78"	70"	64"	60"
#11	100"	87"	78"	71"	66"
#14	NOT PERMITTED (25.5.1.1)				
#18	NOT PERMITTED (25.5.1.1)				

COMPRESSION LAP SPLICE LENGTH (IN)
(ACI 318 25.5.5.1)

BAR SIZE	3000 PSI
#3	12"
#4	15"
#5	19"
#6	23"
#7	26"
#8	30"
#9	34"
#10	38"
#11	42"
#14	NOT PERMITTED (25.5.1.1)
#18	NOT PERMITTED (25.5.1.1)

FOR BUILDING DEPARTMENT USE ONLY

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BROKEN SOUND RETAIL SHELL
6600 NORTH MILITARY TRAIL
BOCA RATON, FL 33496

REVISIONS

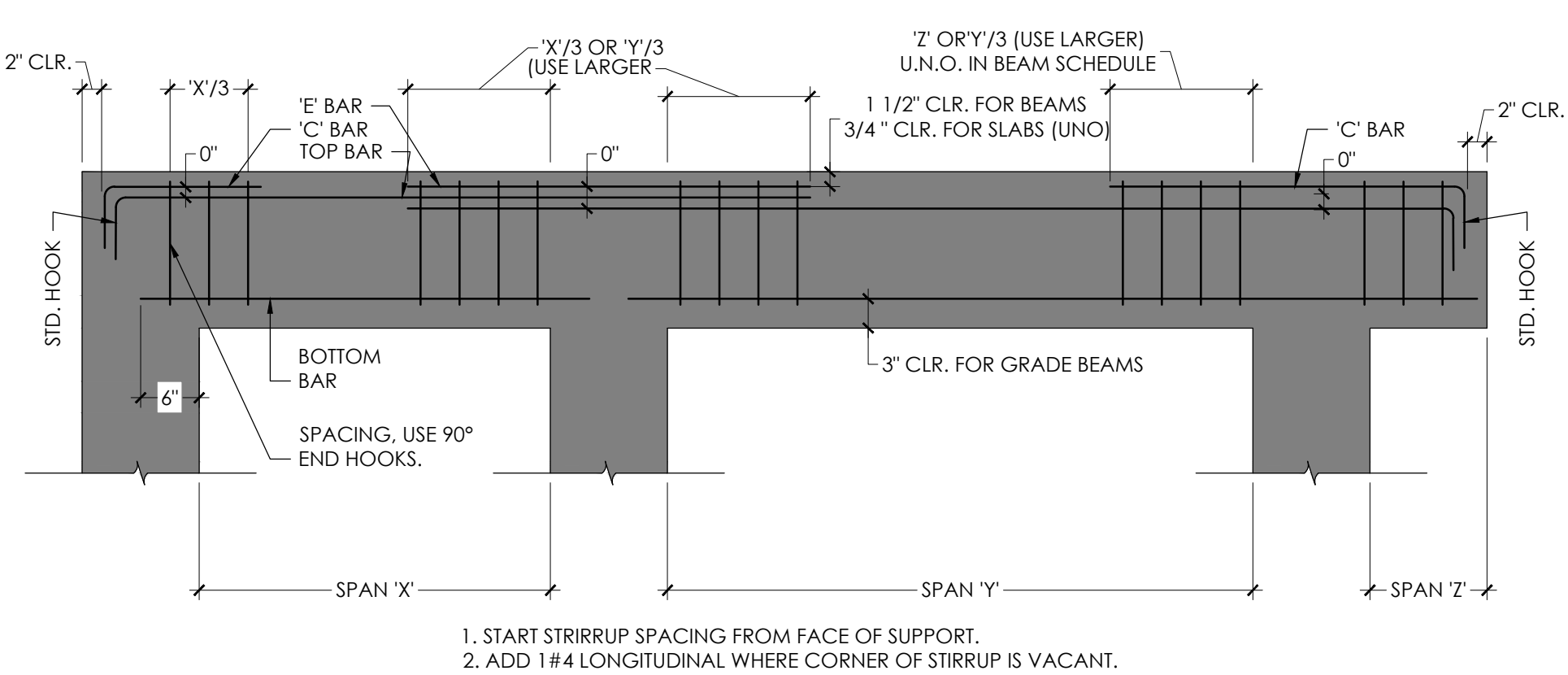
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SCHEDULES RETAIL #1

JOB No: 25-035
DATE: 10/15/2025
STATUS: PERMIT SET
DRAWN: A.F.
CHECKED: M.I.F.

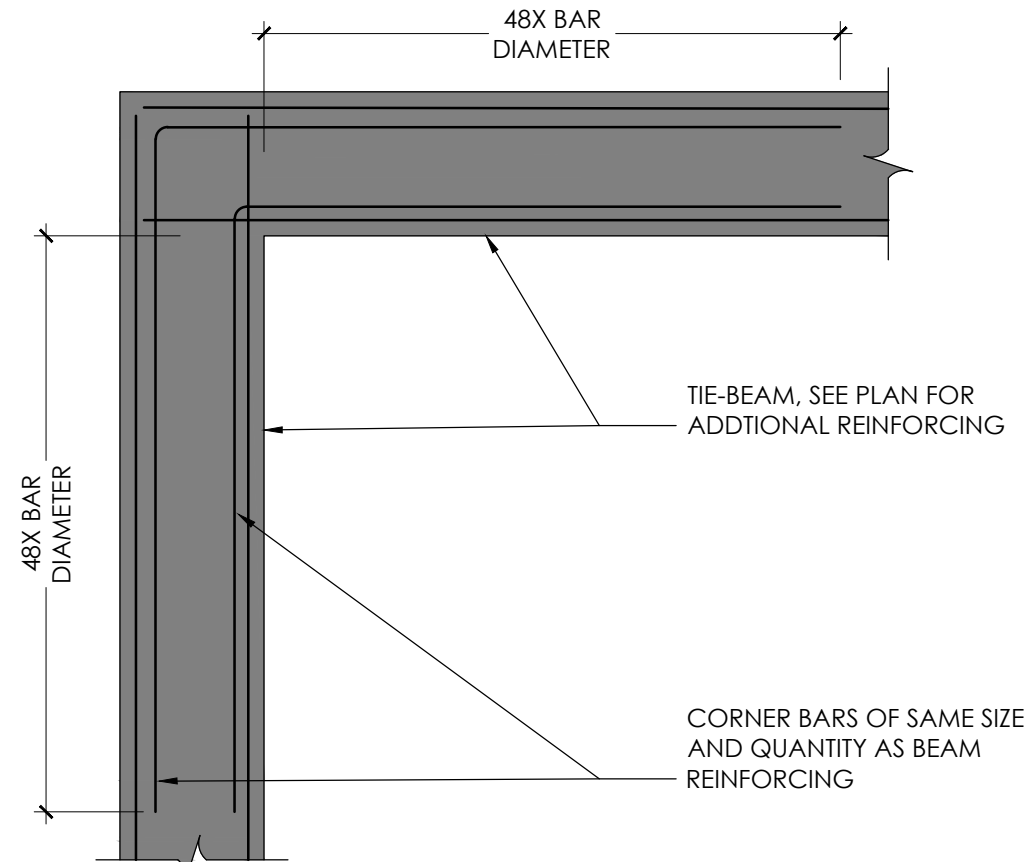
S-0.3

BEAM SCHEDULE						
BEAM MARK	SIZE (WxD)	REINFORCING				REMARKS
		BOTT.	TOP	'C'	'E'	
RTB-1	8" x 16"	2#6	2#6	--	--	#3@12"O.C.
RTB-2	8" x 24"	2#6	2#6	--	--	#3@12"O.C.

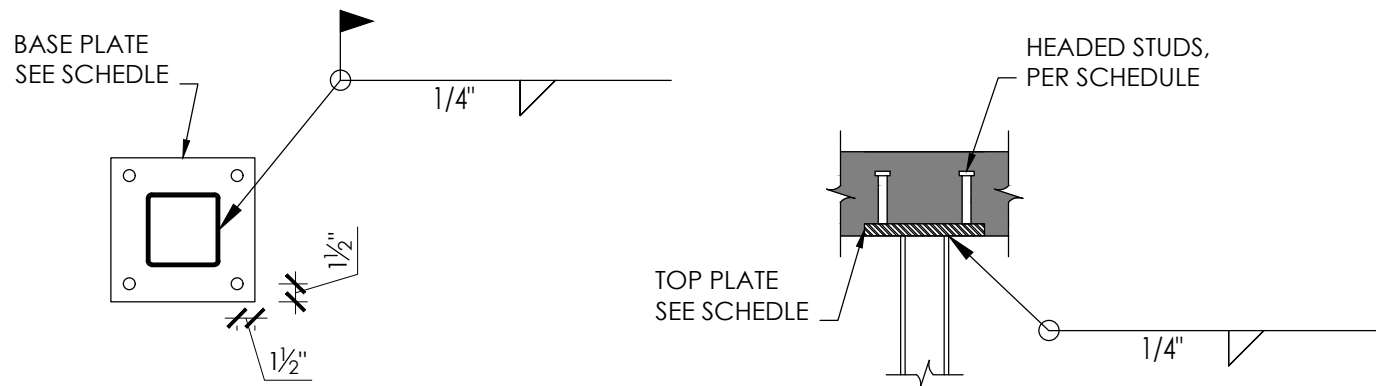
NOTE FOR ALL BEAMS:
SIZES ARE MINIMUM. BEAMS CAN BE POURED DEEPER TO ACCOMMODATE BEAM/DOOR HEIGHT.
FOR ALL BEAMS GREATER THAN 12" IN DEPTH PROVIDE A MINIMUM OF 1#4@12"O.C. EA. FACE OF BEAM.



TYPICAL BENDING DIAGRAM FOR BEAMS AND ONE WAY SLABS
N.T.S.



TYPICAL BEAM & TIE BEAM CORNER DETAIL
N.T.S.



BASE PLATE

TOP PLATE

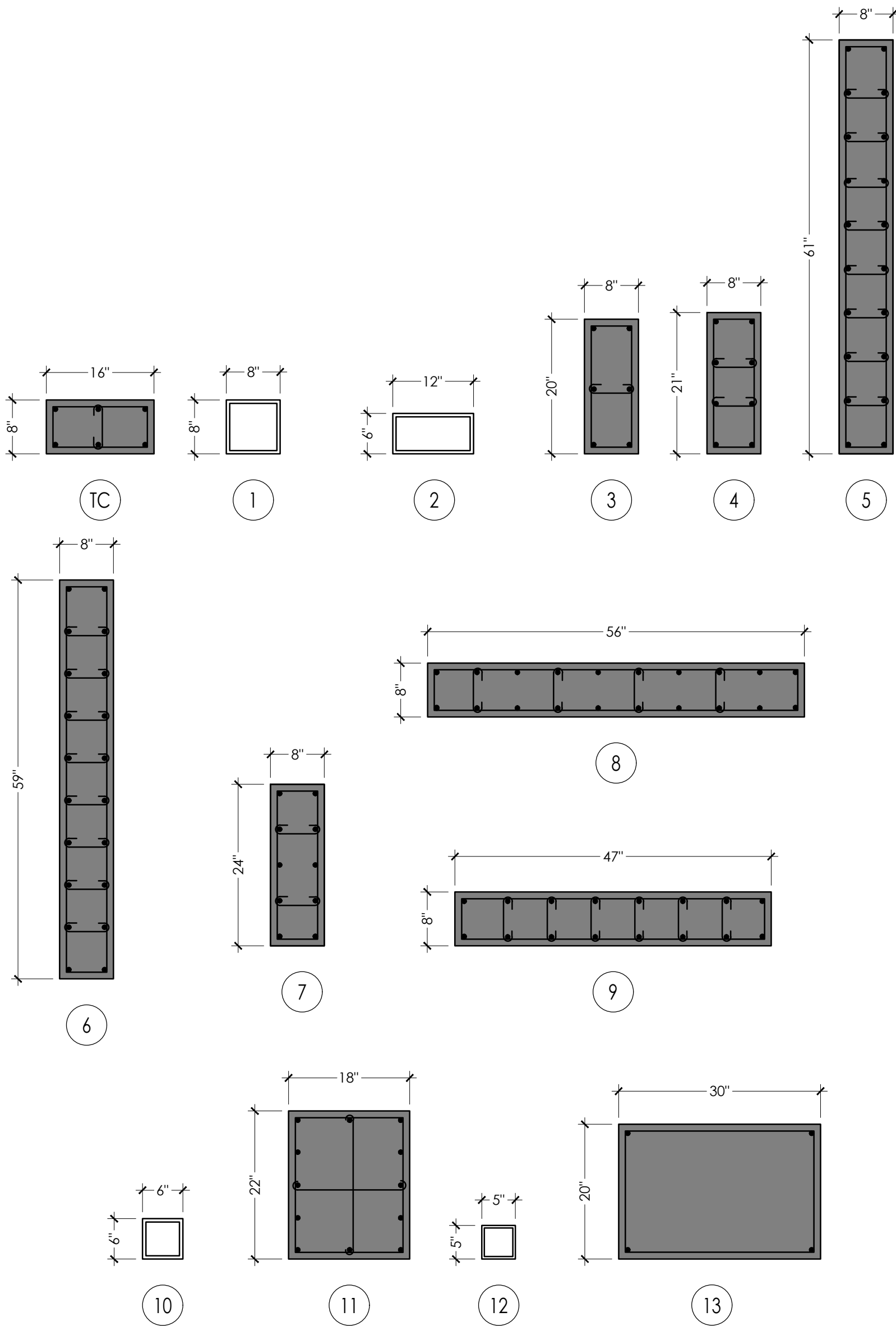
1

FOOTING SCHEDULE			
ALLOWABLE SOIL BEARING PRESSURE OF 2500 PSF			
FOOTING ID.	FOOTING SIZE	REINFORCING	REMARKS
W-24	24" WIDE x 12" DEEP x CONT.	3#5 BOTTOM CONT. & #5@12"O.C. BOTTOM TRANSVERSE	
W-30	30" WIDE x 12" DEEP x CONT.	4#5 BOTTOM CONT. & #5@10"O.C. BOTTOM TRANSVERSE	
W-48	48" WIDE x 14" DEEP x CONT.	5#5 BOTTOM CONT. & #6@12"O.C. BOTTOM TRANSVERSE	
F-72	6'-0" x 6'-0" x 18"DEEP	7#7 BOTTOM EACH WAY	
F-84	7'-0" x 7'-0" x 20"DEEP	10#7 BOTTOM EACH WAY	
F-108	9'-0" x 9'-0" x 24"DEEP	12#8 BOTTOM EACH WAY	
TE-1	8" x 8" x CONT.	1#5 CONT.	THICKENED EDGE

NOTE: TOP OF FOOTING SHALL BE 16" BELOW SLAB.

COLUMN SCHEDULE				
COLUMN ID. NO.	COLUMN SIZE	VERT. REINFORCING OR BASE PLATE	TIES OR CAP PLATE	REMARKS
TC	8" x 16"	6#6	#3@8"O.C.	
1	HSS 8" x 8" x 1/2"	1" x 12 x 12" W/(4) 3/4" Ø x 20" LONG ANCHOR BOLTS	1/2" x 10" x 10"	
2	HSS6" x 12" x 1/2"	1" x 16 x 12" W/(4) 3/4" Ø x 18" LONG ANCHOR BOLTS	3/4" x 10" x 10"	
3	8" x 20"	6#6	#3@8"O.C.	
4	8" x 21"	8#6	#3@8"O.C.	
5	8" x 61"	20#5	#3@8"O.C.	
6	8" x 59"	20#5	#3@8"O.C.	
7	8" x 24"	10#6	#3@8"O.C.	
8	8" x 56"	20#5	#3@8"O.C.	
9	8" x 47"	16#6	#3@8"O.C.	
10	HSS 6" x 6" x 1/2"	3/4" x 10" x 10" W/(4) 3/4" Ø x 18" LONG ANCHOR BOLTS	1/2" x 8" x 8"	
11	22" x 18"	12#6	#3@12"O.C.	
12	HSS 5" x 5" x 1/2"	3/4" x 10" x 10" W/(4) 3/4" Ø x 18" LONG ANCHOR BOLTS	1/2" x 8" x 8"	
13	30" x 20"			

- ⊖ INDICATES COLUMN TERMINATES
- ⊙ INDICATES COLUMN CONTINUES
- ⊕ INDICATES COLUMN STARTS



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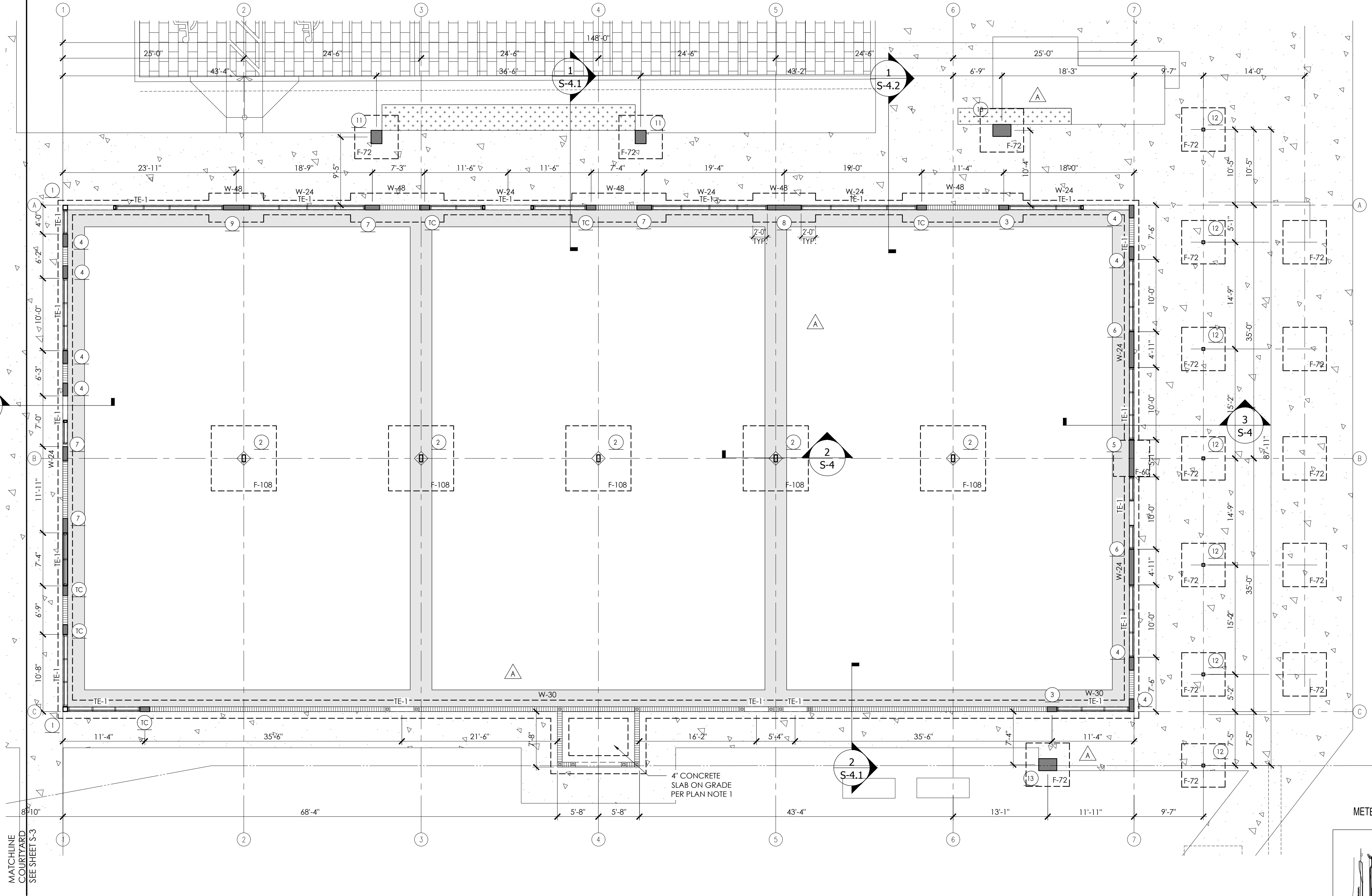
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S-0.4

PLAN NOTES:

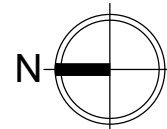
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- SLAB CURING IS REQUIRED. SEE STRUCTURAL NOTES ON SHEET S-0.
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MATCHLINE
COURTYARD
SEE SHEET S-3

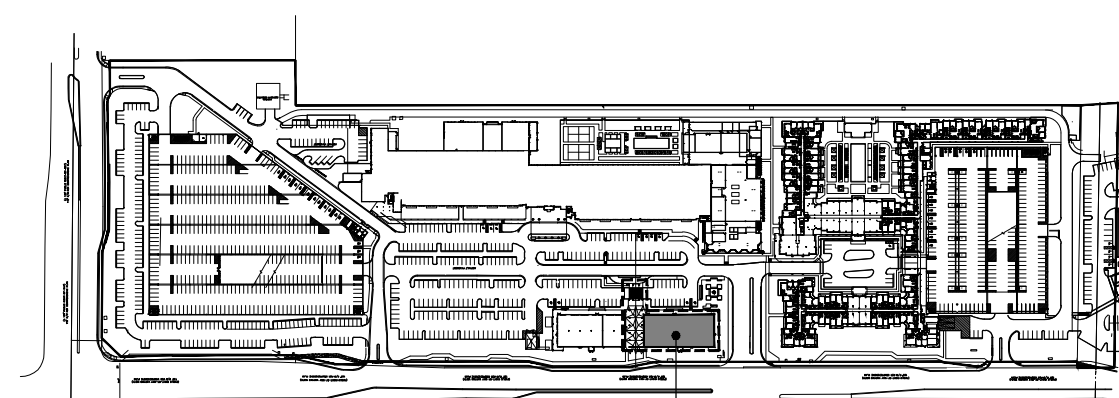


FOUNDATION LEVEL FLOOR PLAN

1/8"=1'-0"



METER



KEY PLAN
N.T.S.

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REVISIONS

REVISIONS	BLDG DEPT COMMENTS
A	GENERAL COORDINATION 1/09/2026

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FLOOR PLAN
RETAIL #1

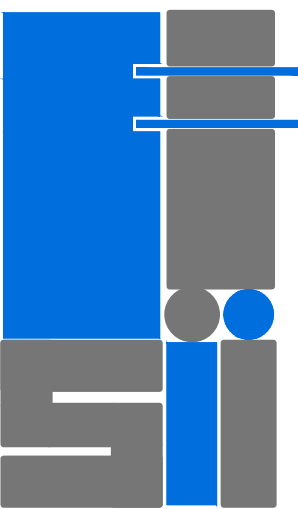
JOB No: 25-035
DATE: 10/15/2025
STATUS: PERMIT SET
DRAWN: A.F.
CHECKED: M.I.F.

S-1

PLAN NOTES:

1. ROOF SYSTEM WILL BE BUILT AS SHOWN ON THIS PLAN. THE ROOF SYSTEM DEPICTED ON THIS PLAN SHALL CONSIST OF PRE-ENGINEERED METAL TRUSSES WITH METAL DECK AS SHOWN ON PLAN.
2. REFER TO ARCHITECTURAL DRAWINGS FOR REQUIRED ROOFING, ROOFING DETAILS, ETC.
3. FOR DIMENSIONS NOT SHOWN HERE, SEE FOUNDATION, SECOND FLOOR PLANS AND THE ARCHITECTURAL DRAWINGS.
4. REFER TO STRUCTURAL NOTES (S-1) FOR ADDITIONAL INFORMATION AND REQUIREMENTS.
5. VERIFY ALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION AND/OR FABRICATION. ALL ELEVATIONS SHOWN ON THESE PLANS ARE RELATIVE TO THE TOP OF THE GROUND FLOOR SLAB AT 0'-0".
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7. L2 - INDICATES L3X3X1/4" ANGLE ATTACHED TO BEAMS OR TO CMU WALL (GROUTED CELL) WITH 1/2"Ø HILTI-HY 200 ADHESIVE ANCHOR AT 24" O.C. MAX.
8. CONTRACTOR TO PROVIDE THE PROPER SHORING WHERE REQUIRED.
9. SEE ARCH'L AND MEP PLANS FOR ACTUAL LOCATION OF A/C UNITS.

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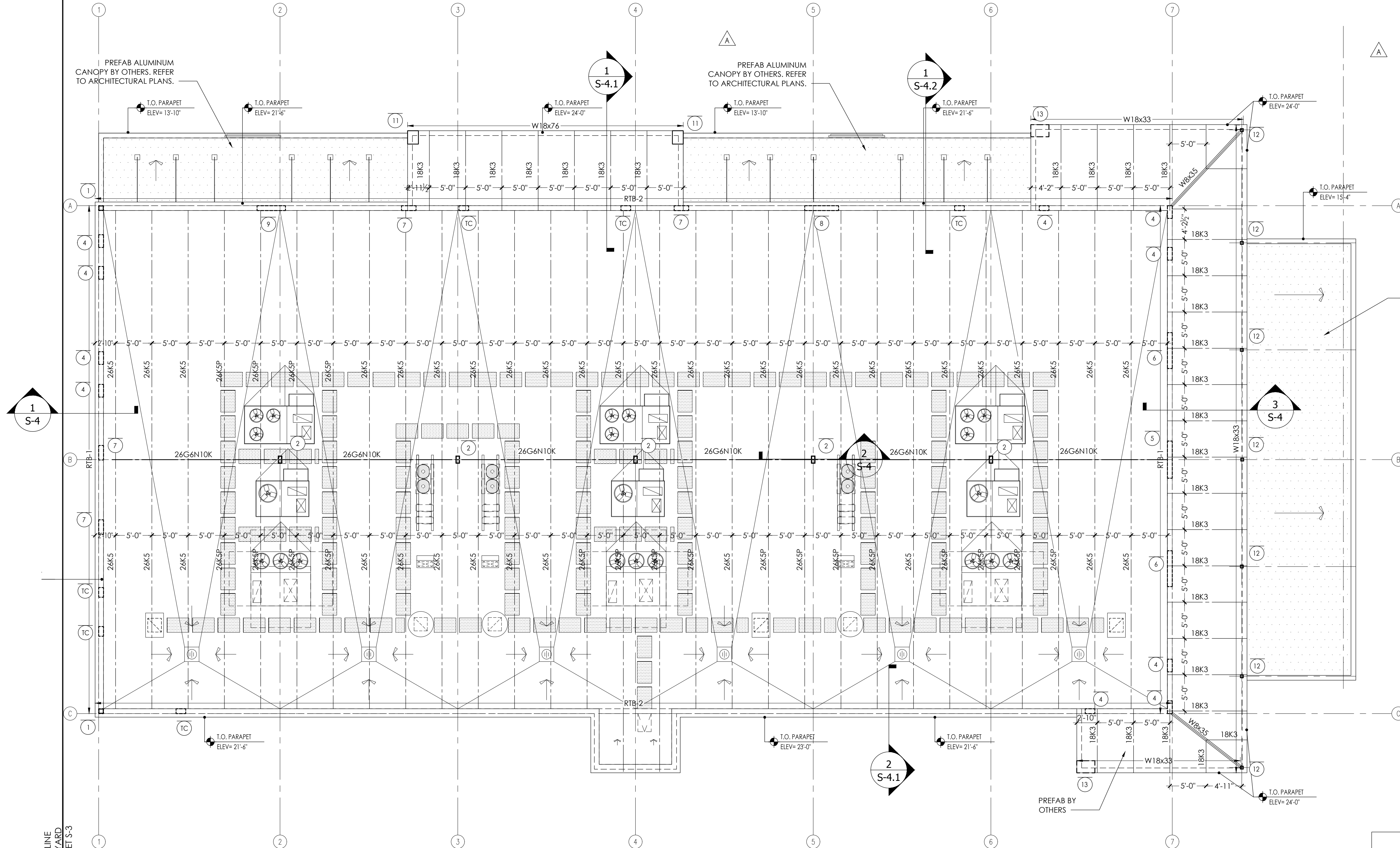
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BLDG DEPT COMMENTS
GENERAL COORDINATION
1/09/2026

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ROOF LEVEL
FLOOR PLAN
RETAIL #1

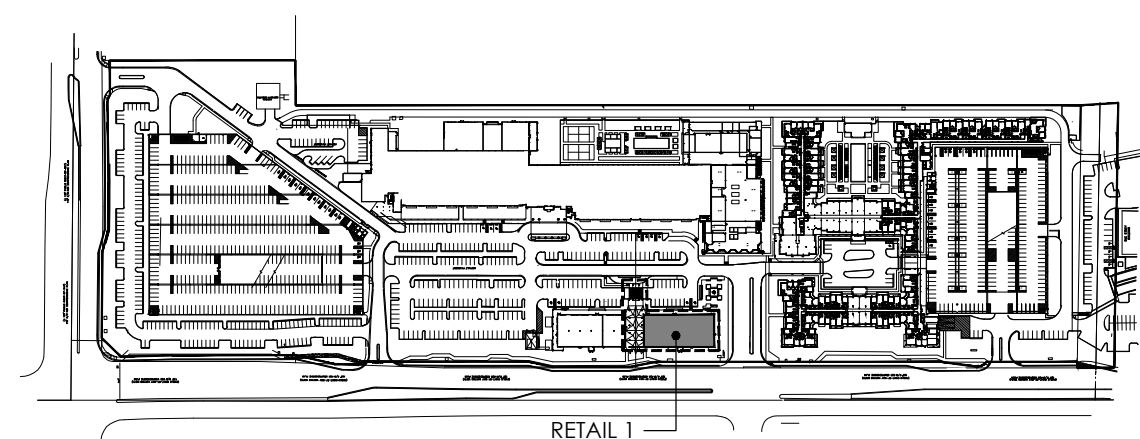
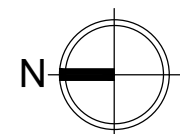
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S-2

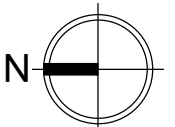


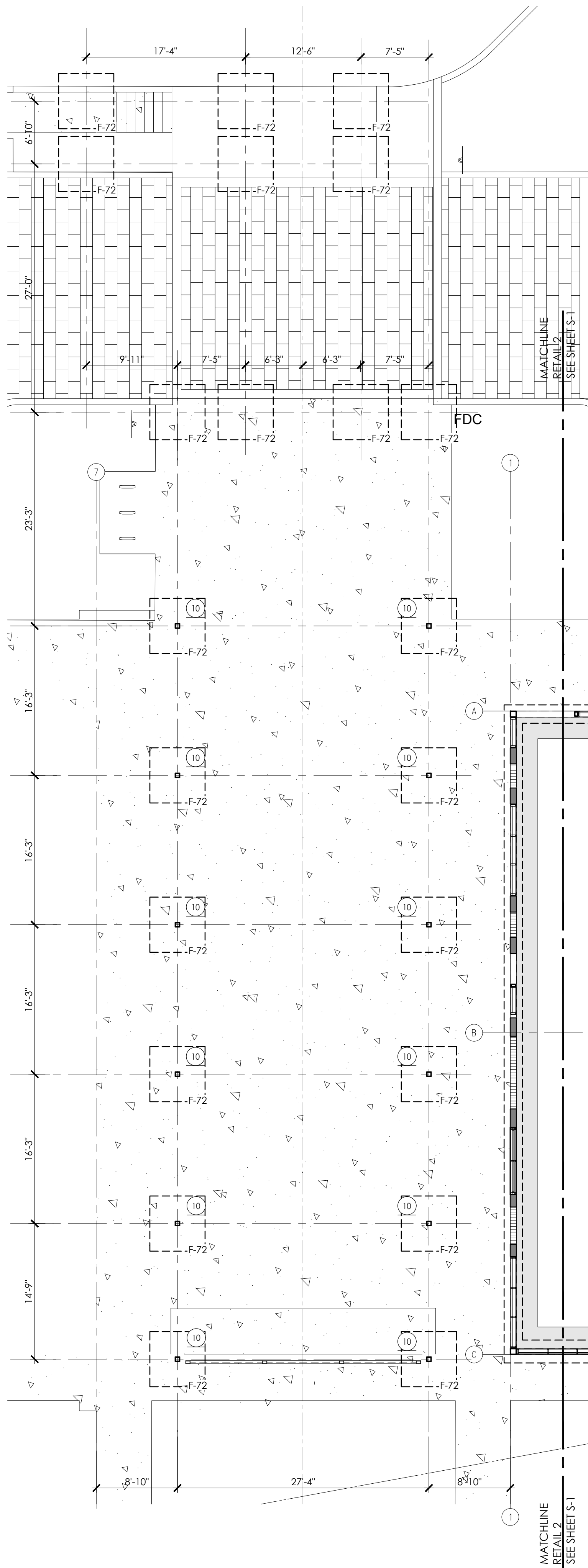
ROOF LEVEL FLOOR PLAN

1/8" = 1'-0"



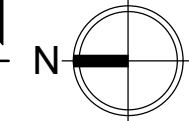
KEY PLAN
N.T.S.





FOUNDATION LEVEL FLOOR PLAN

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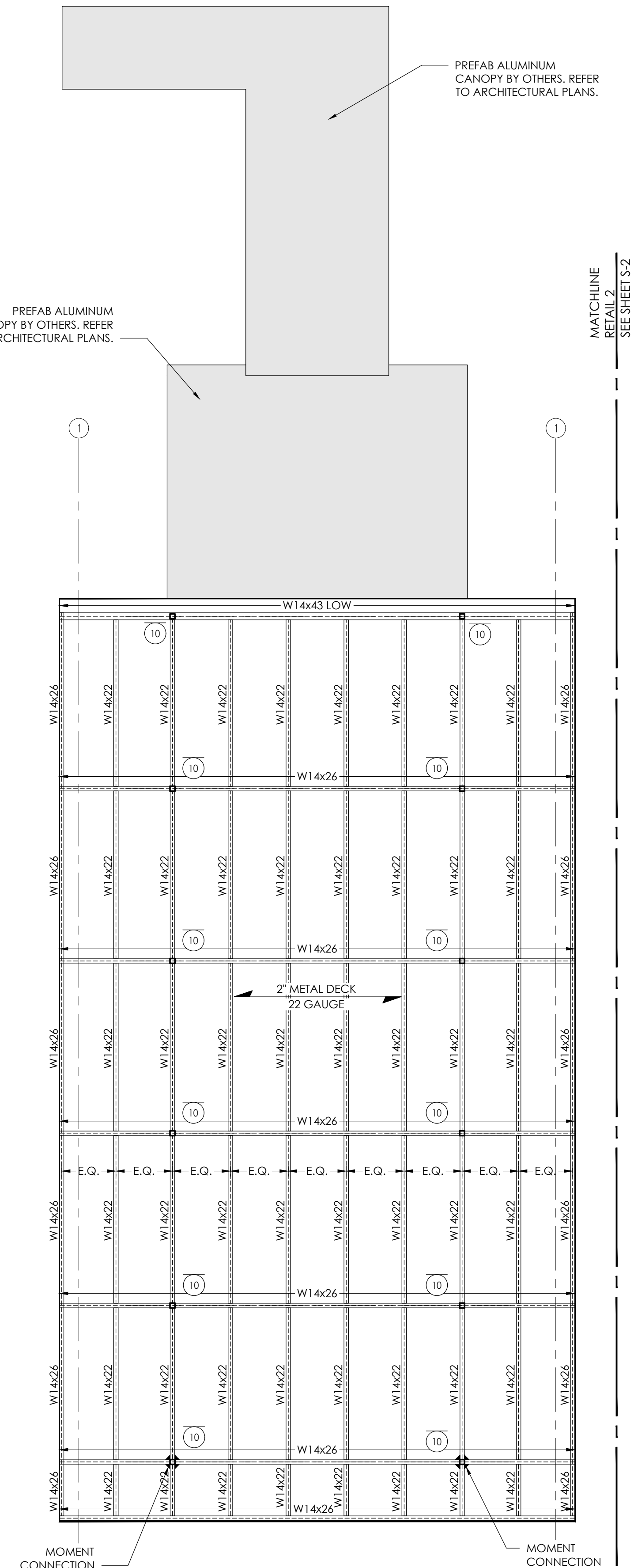


RETAIL CANOPY SHOWN FOR REFERENCE ONLY. NOT PART OF THIS PERMIT SET. REFER TO PERMIT # BCP-2026-00142



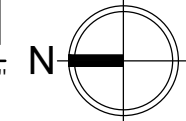
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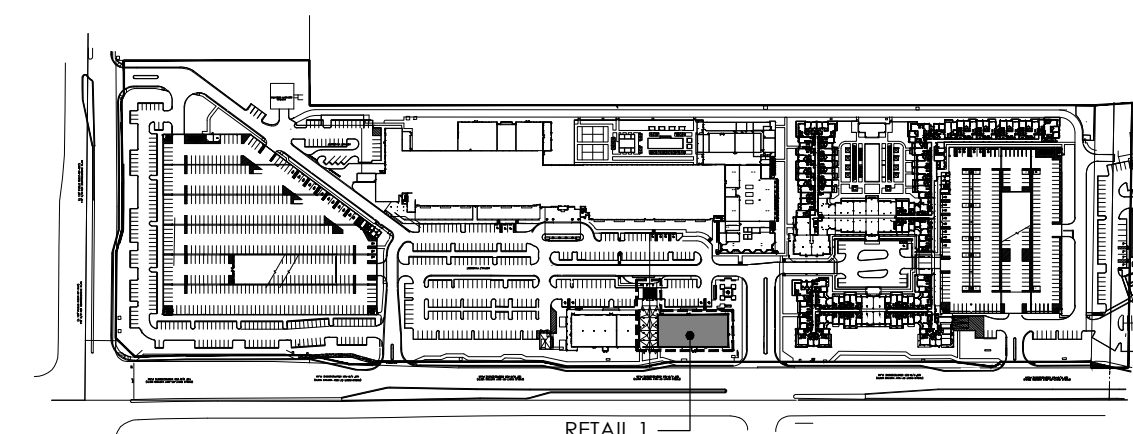


PREFAB ALUMINUM CANOPY BY OTHERS. REFER TO ARCHITECTURAL PLANS.

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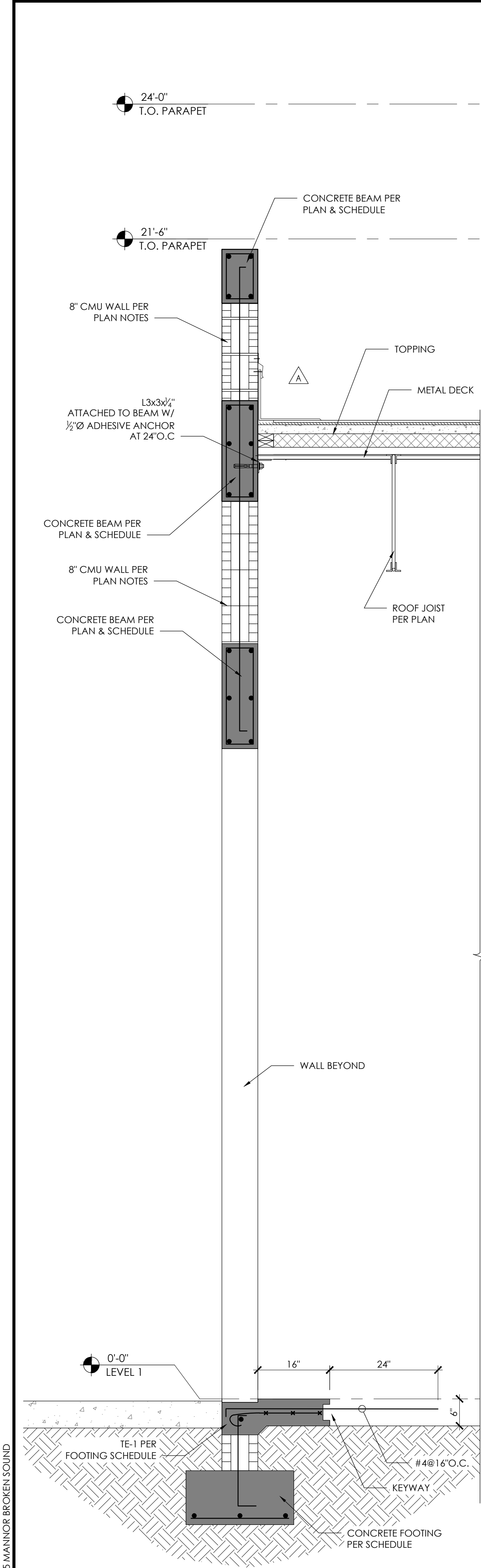
REVISIONS

BLDG DEPT COMMENTS
GENERAL COORDINATION
1/09/2026

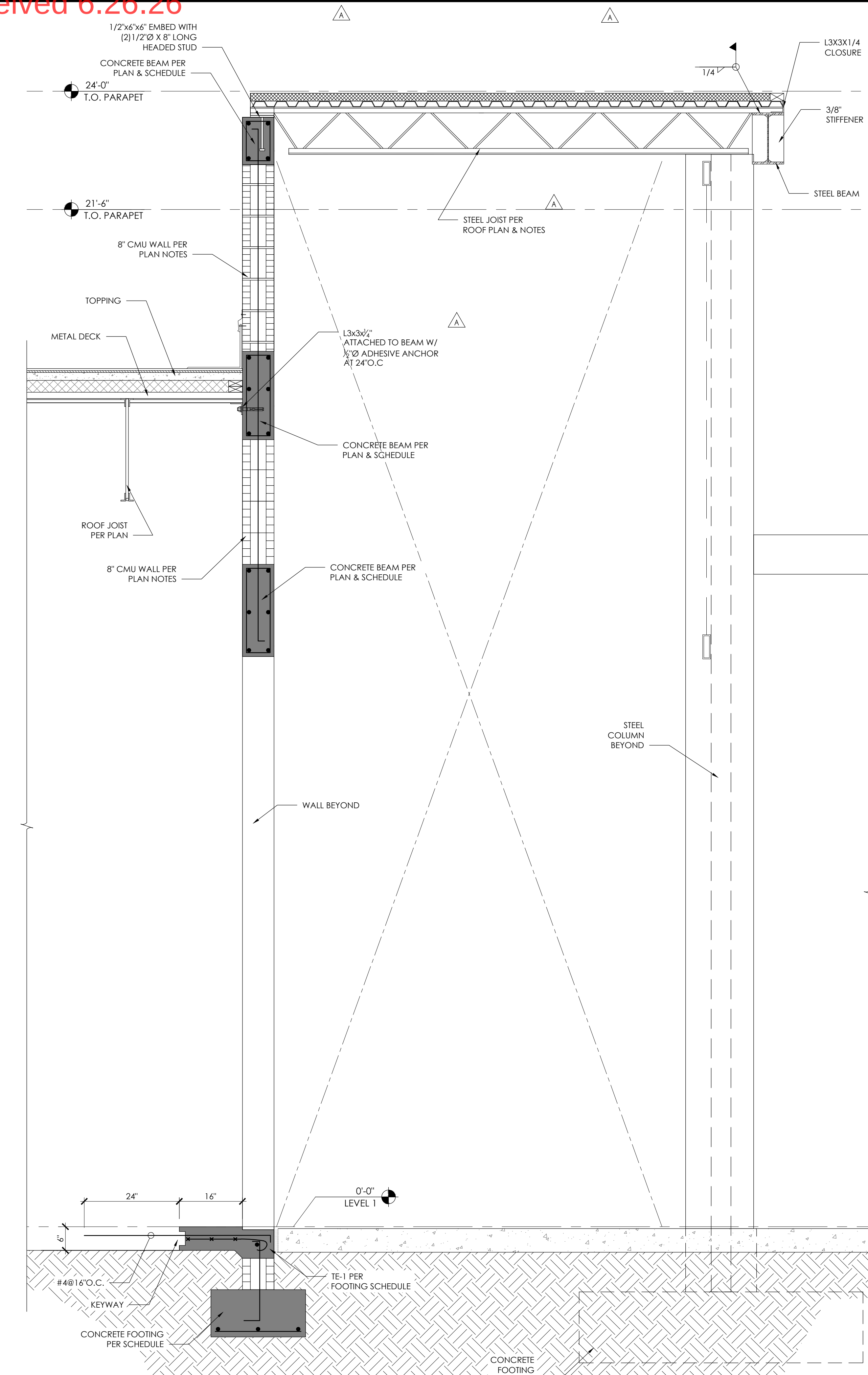
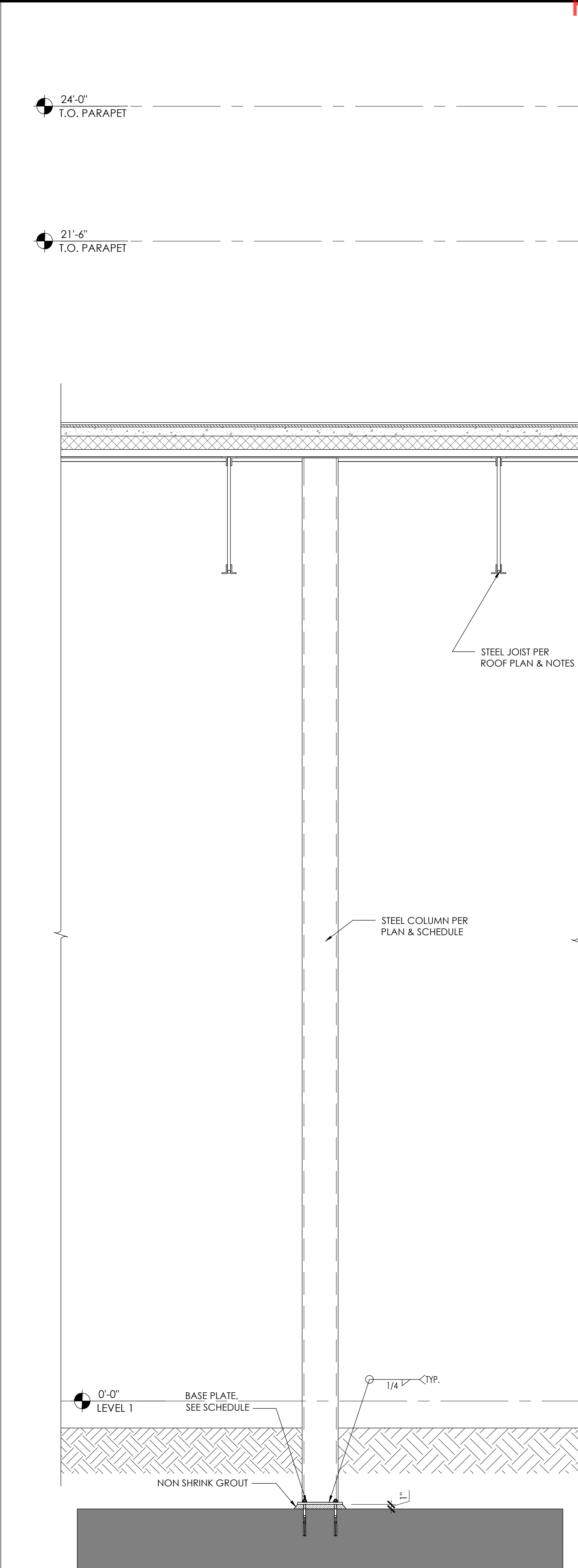
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RETAIL 1
FLOOR PLANS
COURTYARD

JOB No: 25-035
DATE: 10/15/2025
STATUS: PERMIT SET
DRAWN: A.F.
CHECKED: M.L.F.

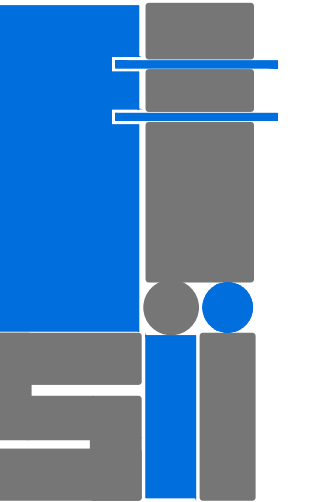
S-3



04/23/26 - 11:25 AM - 25-035 MANNOR BROKEN SOUND



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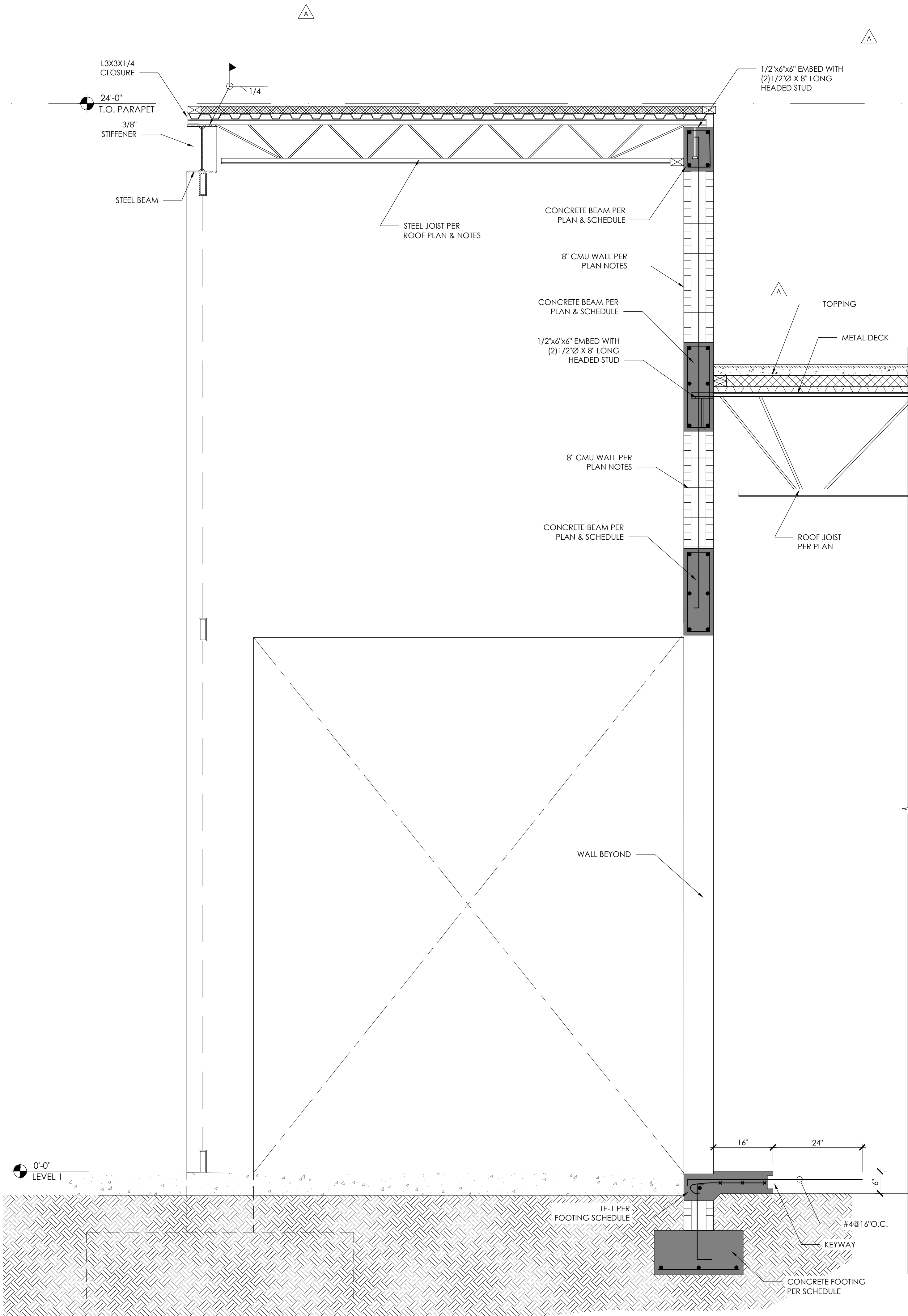
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A	BLDG DEPT COMMENTS GENERAL COORDINATION 1/09/2026

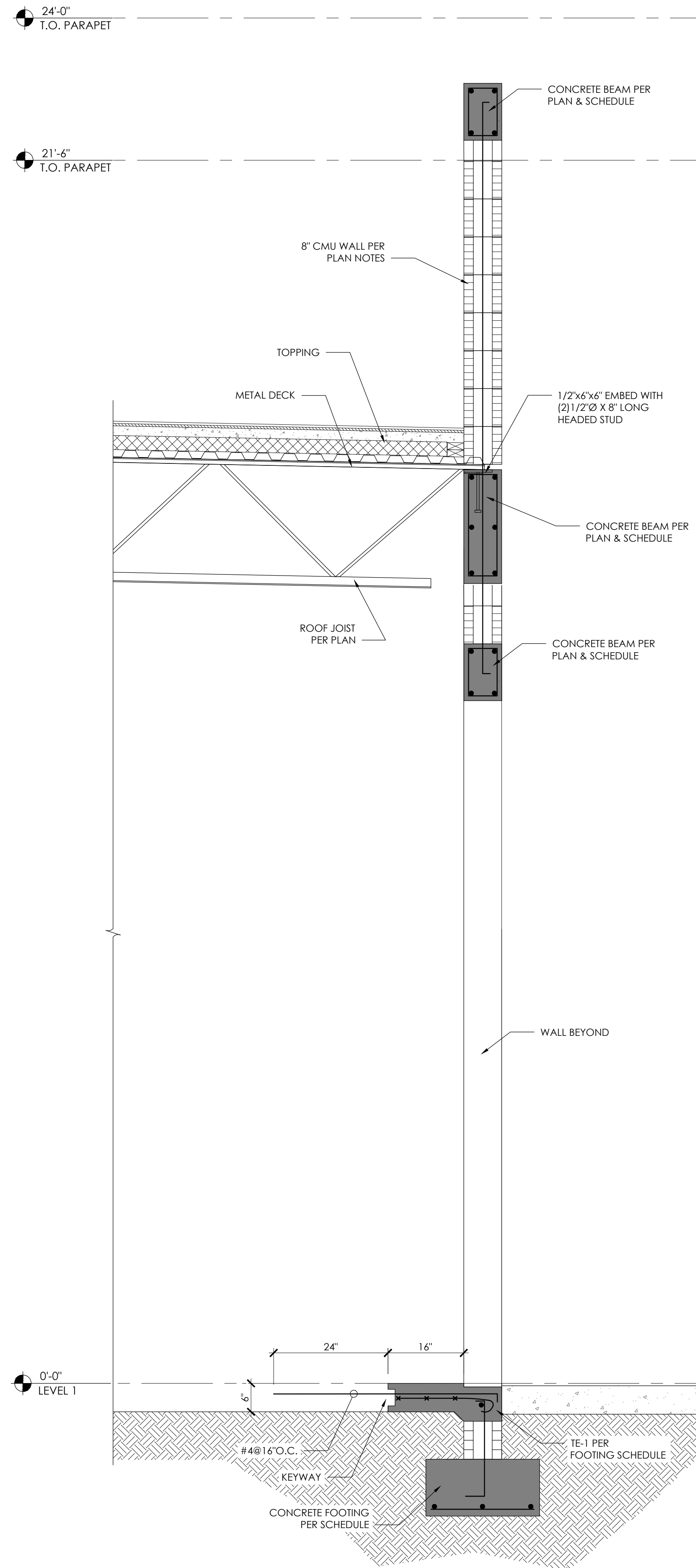
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RETAIL #1**

JOB No: 25-035
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S-4

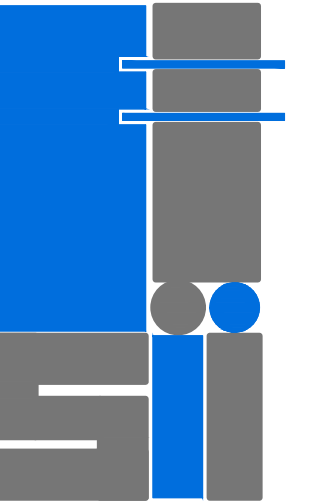


SECTION 1
3/4"=1'-0"



SECTION 2
3/4"=1'-0"

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C.A. #9446

MONZER I. FARAMAWI
FLORIDA P.E. #57439

TO THE BEST OF MY KNOWLEDGE AND
ABILITY, THE COMPLETED STRUCTURE
DEPICTED ON THESE PLANS COMPLIES WITH
THE APPLICABLE MINIMUM BUILDING CODES.

THIS ITEM HAS BEEN ELECTRONICALLY SIGNED
& SEALED BY MONZER I. FARAMAWI, P.E., S.I.
ON THE DATE ADJACENT TO THE SEAL.
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**BROKEN SOUND
RETAIL SHELL**
6600 NORTH MILITARY TRAIL
BOCA RATON, FL 33496

REVISIONS

REVISIONS	BLDG DEPT COMMENTS
 A	GENERAL COORDINATION 1/09/2026

DRAWING TITLE:
**SECTIONS & DETAILS
RETAIL #1**

JOB No: 25-035
DATE: 10/15/2025
STATUS: PERMIT SET
DRAWN: A.F.
CHECKED: M.L.F.

S-4.1

24'-0"
T.O. ROOF FRAME

21'-6"
T.O. PARAPET

CONCRETE BEAM PER
PLAN & SCHEDULE

8" CMU WALL PER
PLAN NOTES

CONCRETE BEAM PER
PLAN & SCHEDULE

1/2"x6"x6" EMBED WITH
(2) 1/2"Ø X 8" LONG
HEADED STUD

8" CMU WALL PER
PLAN NOTES

CONCRETE BEAM PER
PLAN & SCHEDULE

TOPPING

METAL DECK

ROOF JOIST
PER PLAN

WALL BEYOND

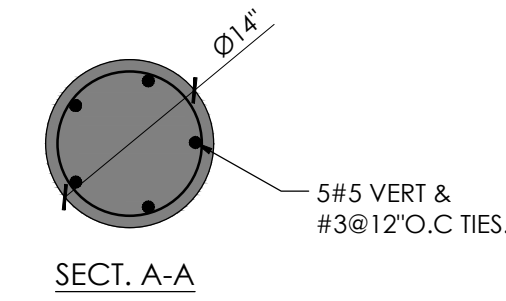
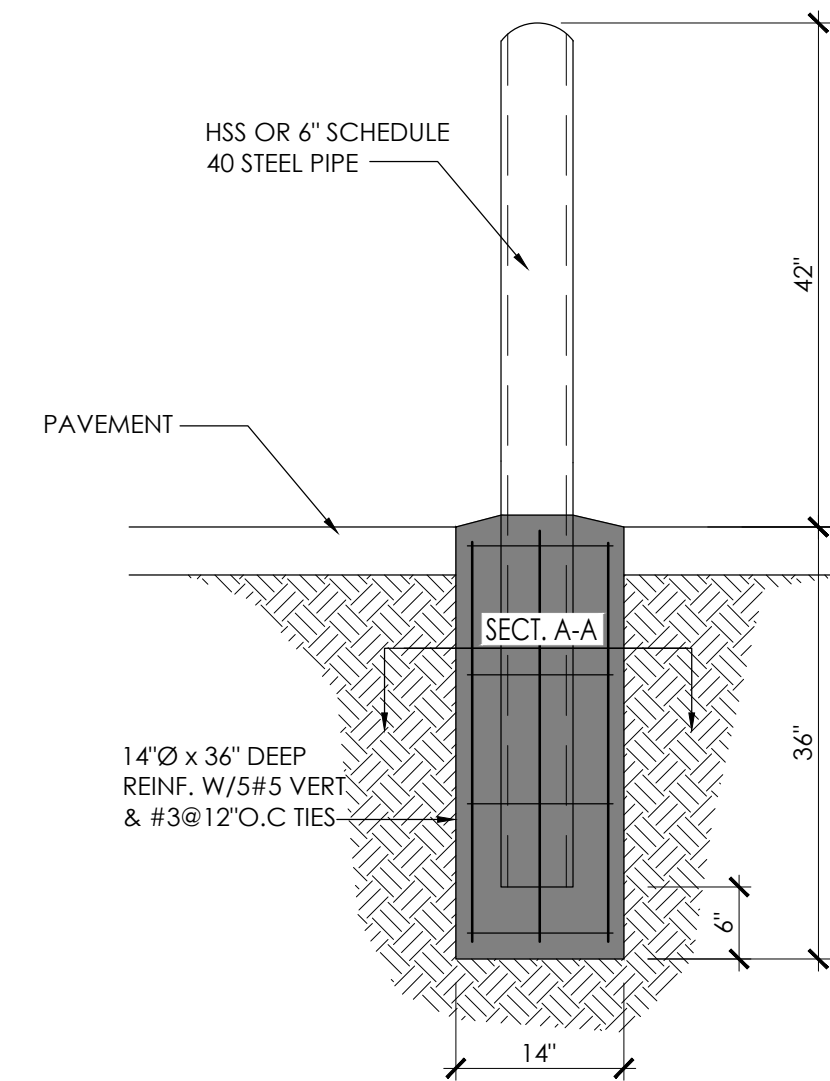
0'-0"
LEVEL 1

TE-1 PER
FOOTING SCHEDULE

#4@16" O.C.

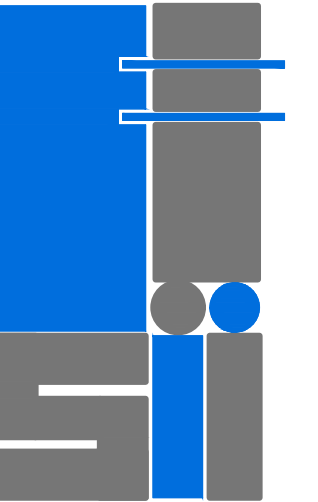
KEYWAY

CONCRETE FOOTING
PER SCHEDULE



BOLLARD DETAIL (TYP.)
N.T.S

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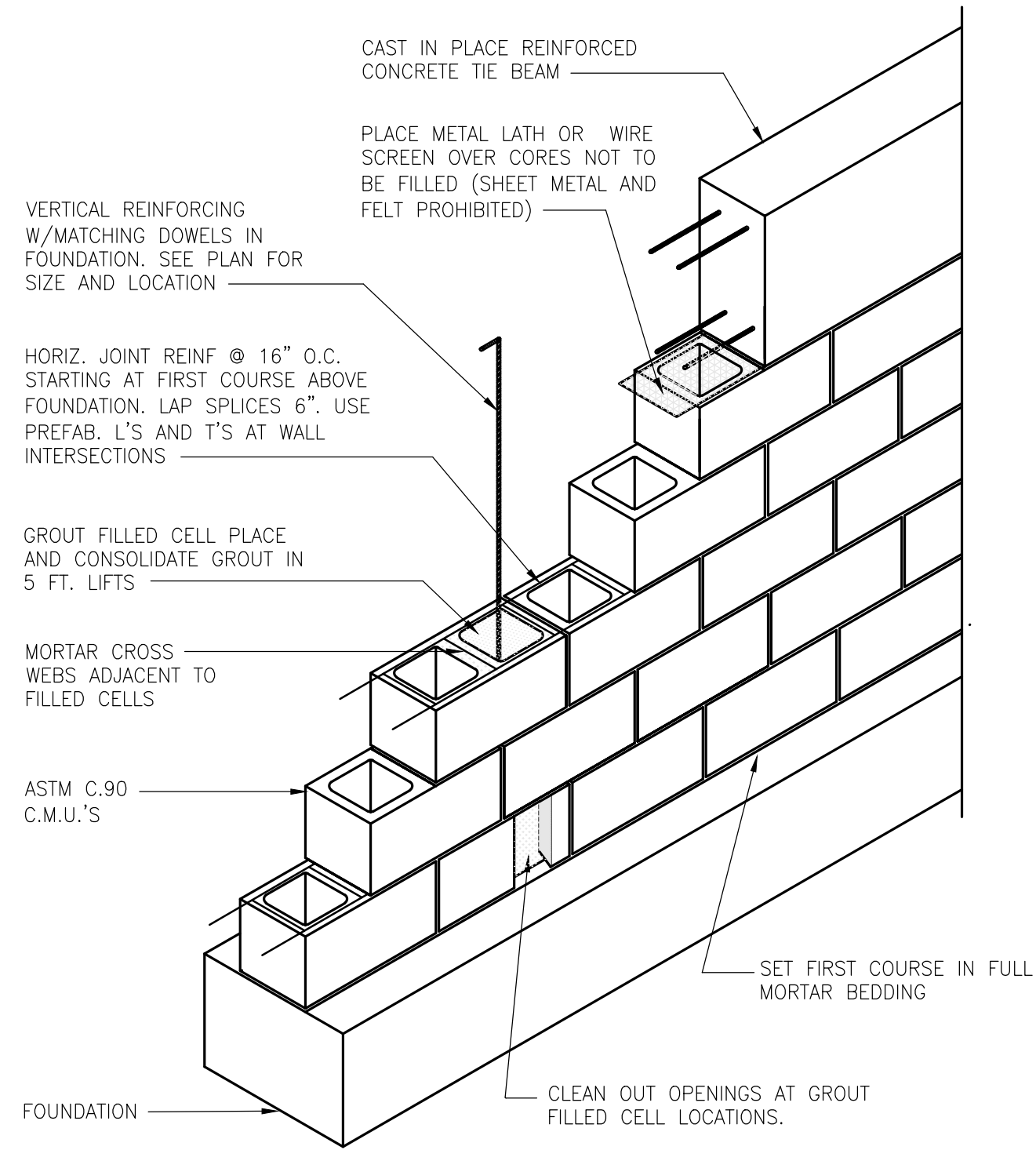
REVISIONS

BLDG DEPT COMMENTS
GENERAL COORDINATION 1/09/2026

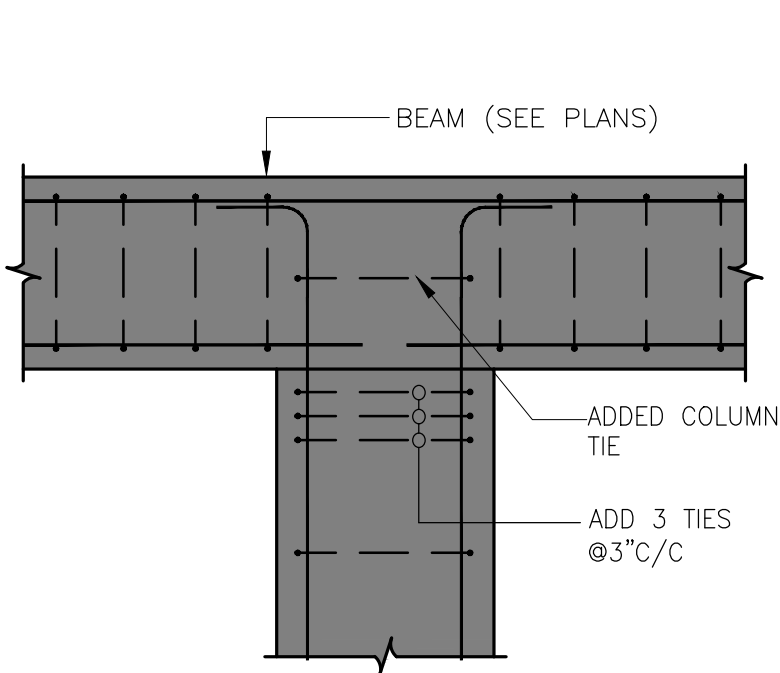
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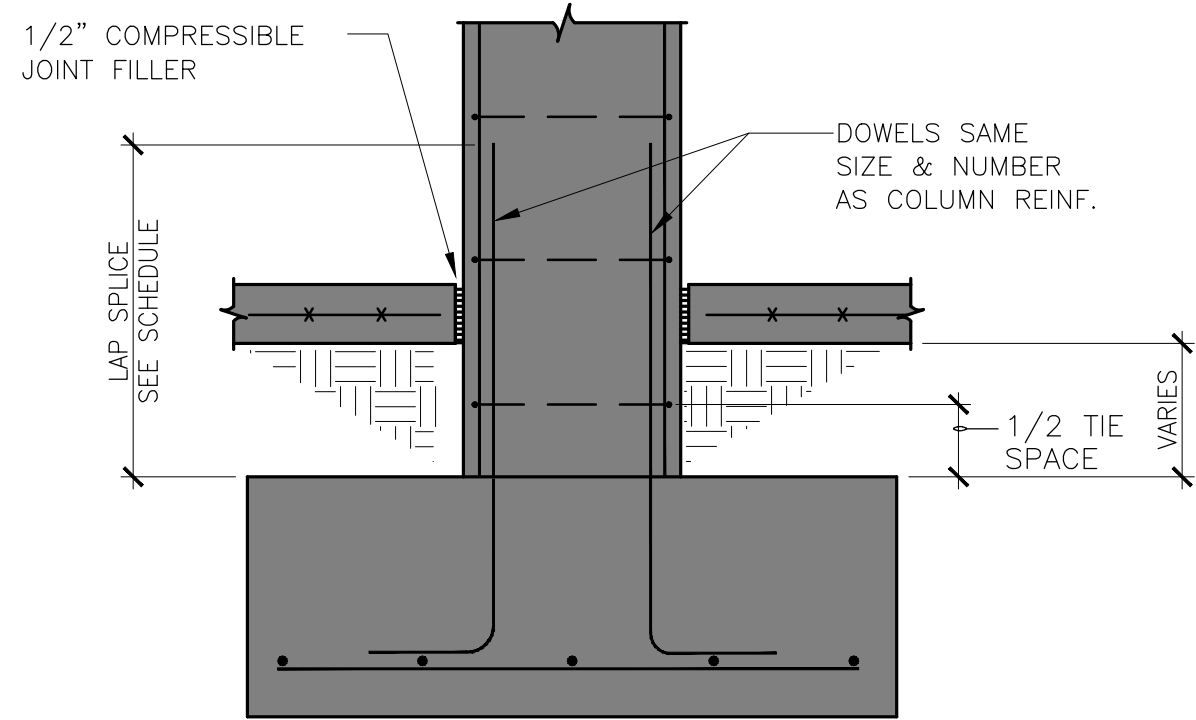
S-4.2



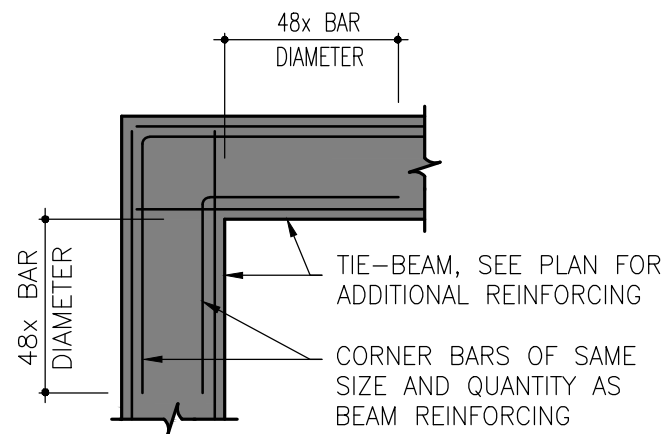
TYPICAL MASONRY WALL CONSTRUCTION



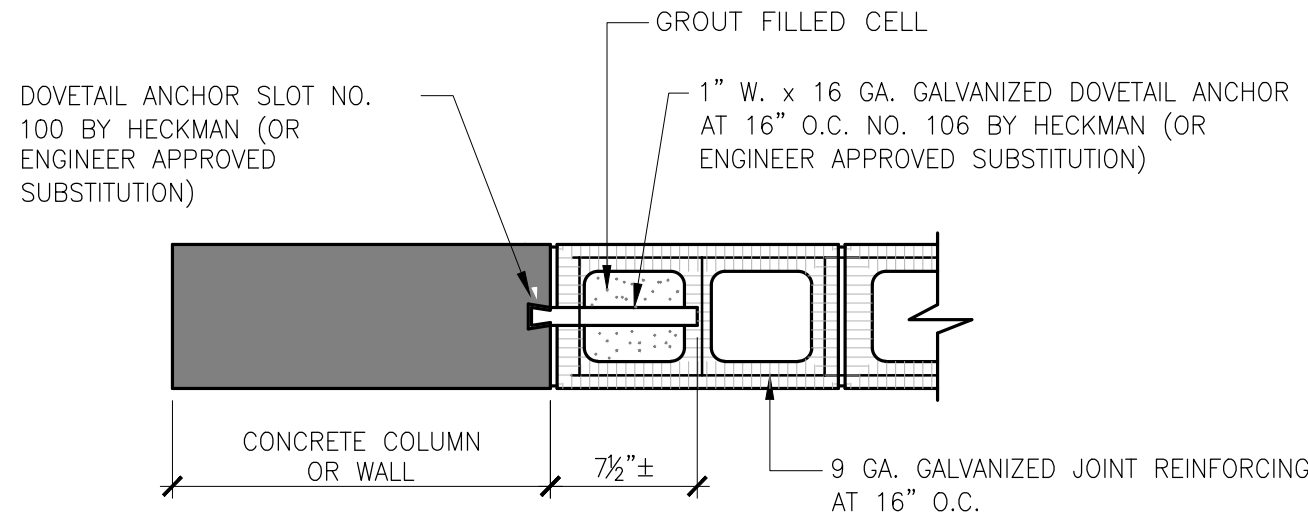
CONCRETE COLUMN TERMINATION AT BEAM



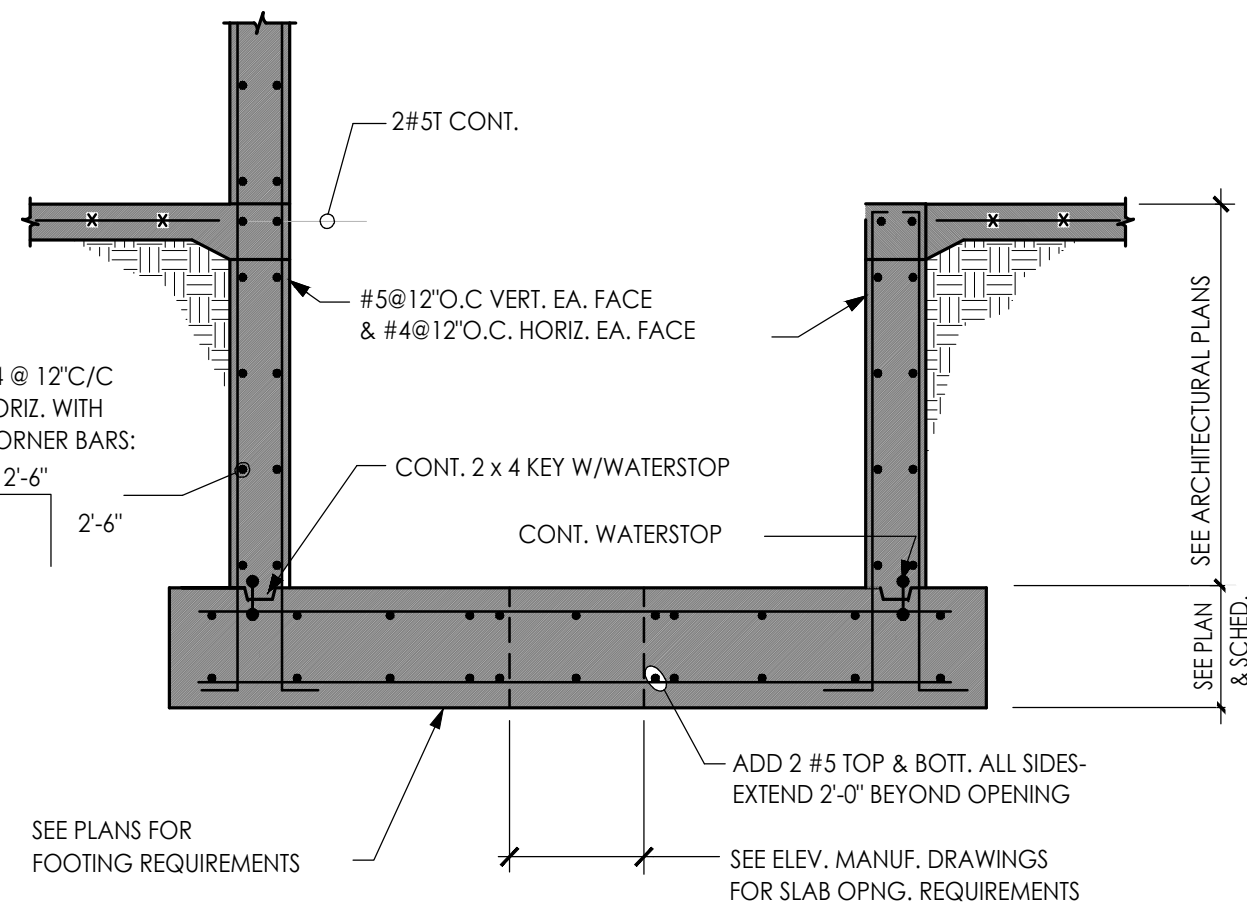
CONCRETE COLUMN BASE DETAIL AT FOUNDATION



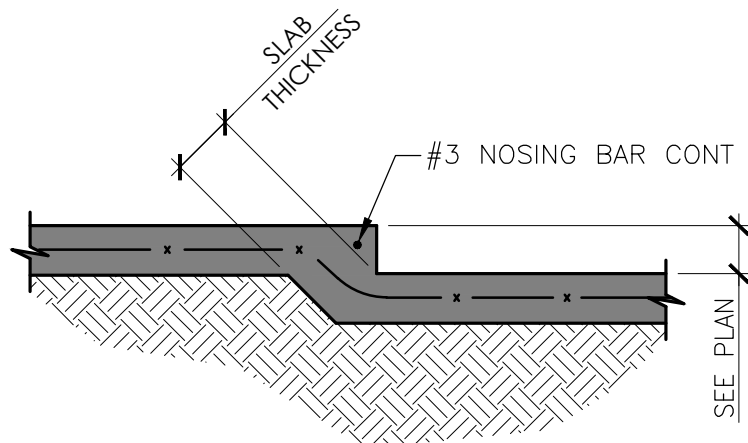
TYPICAL BEAM, TIE-BEAM CORNER DETAIL



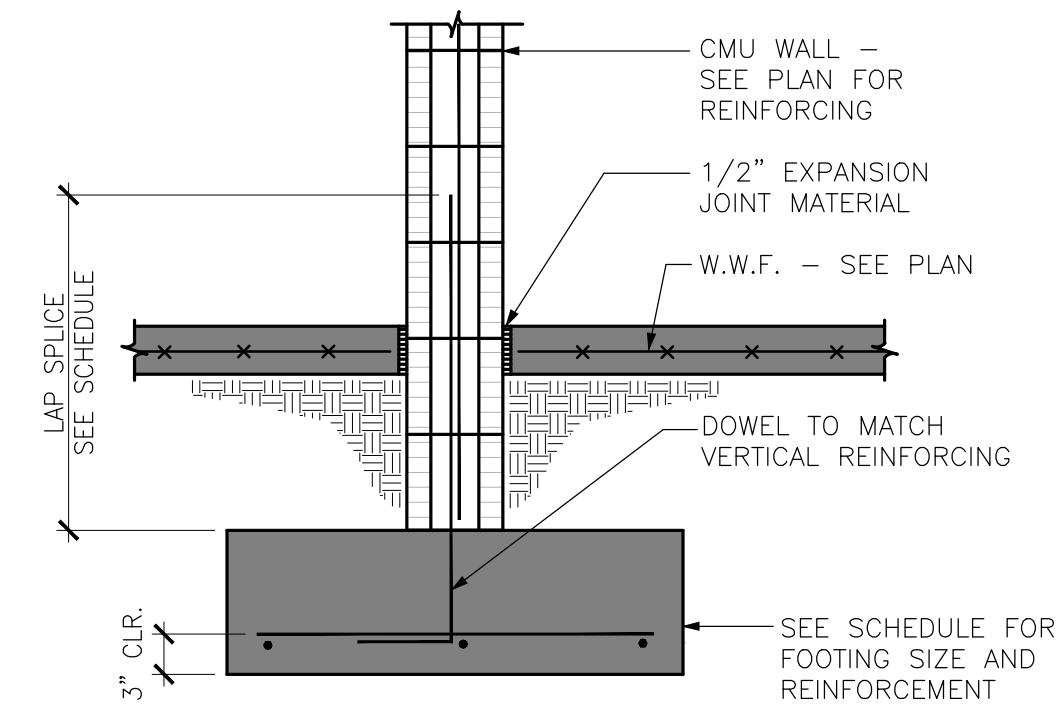
DOVETAIL ANCHOR DETAIL



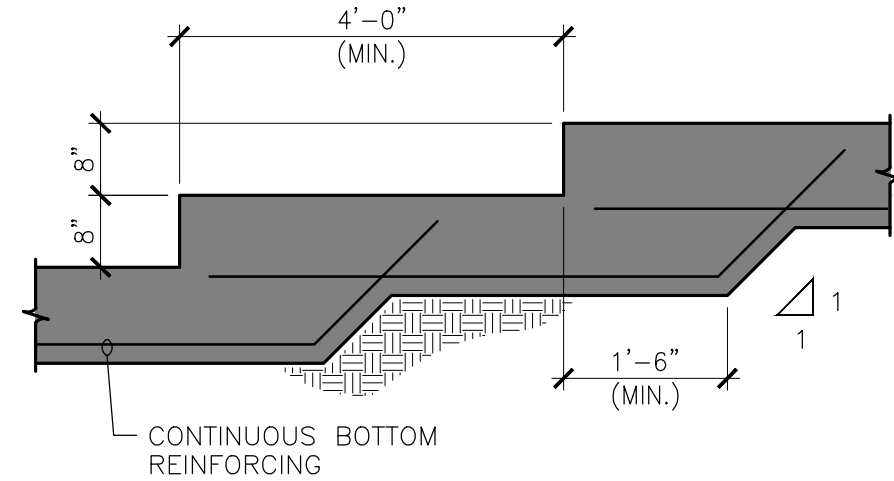
TYPICAL ELEVATOR PIT DETAIL



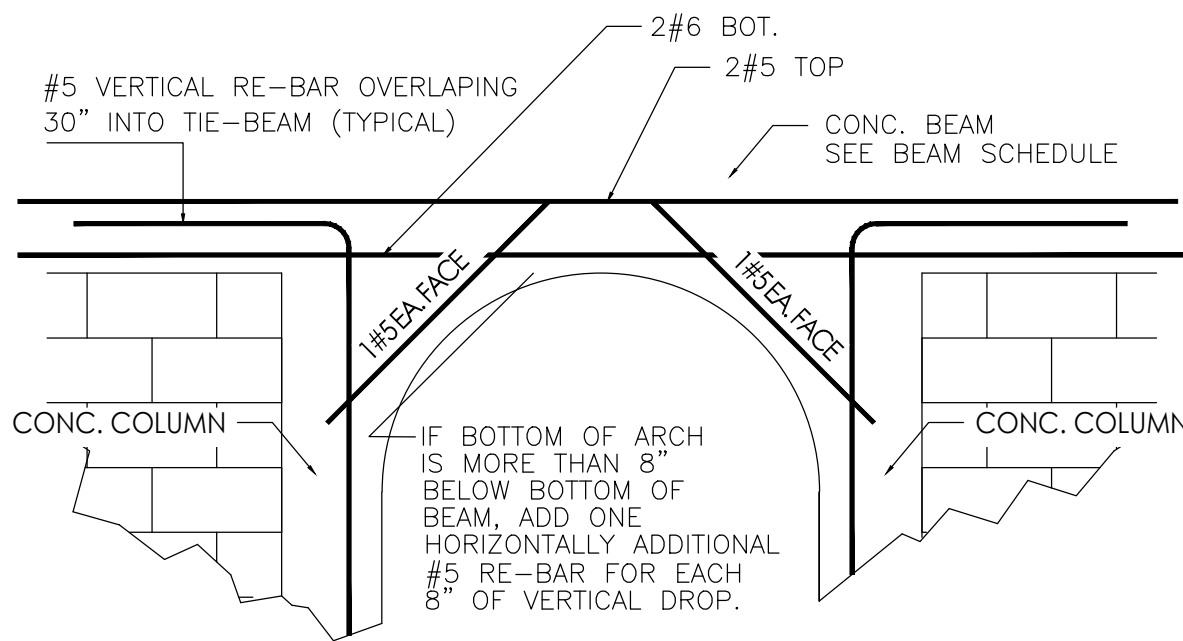
SLAB DEPRESSION



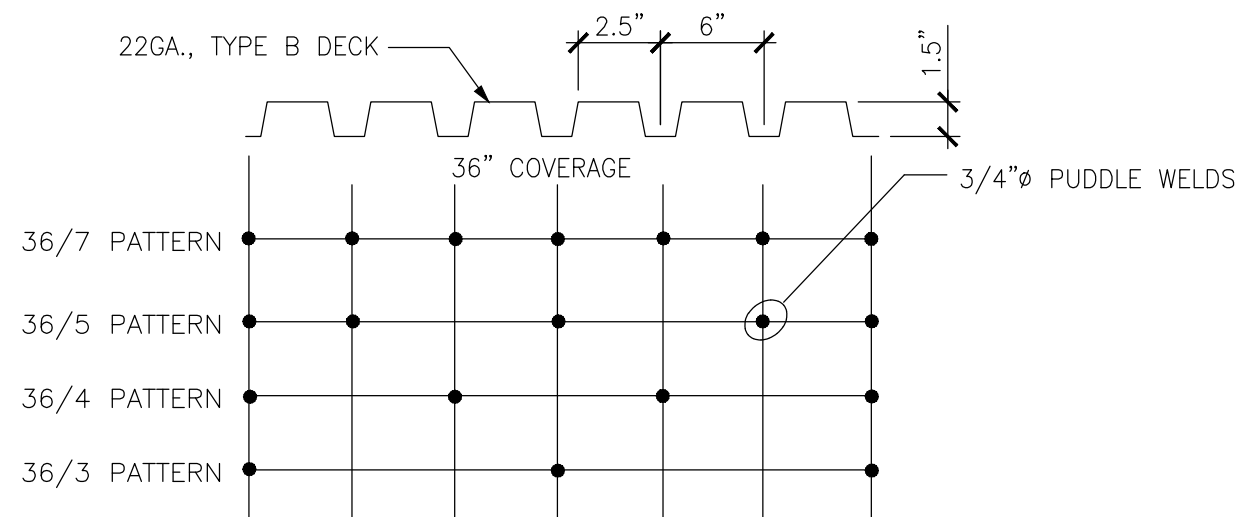
TYPICAL WALL FOOTING SECTION



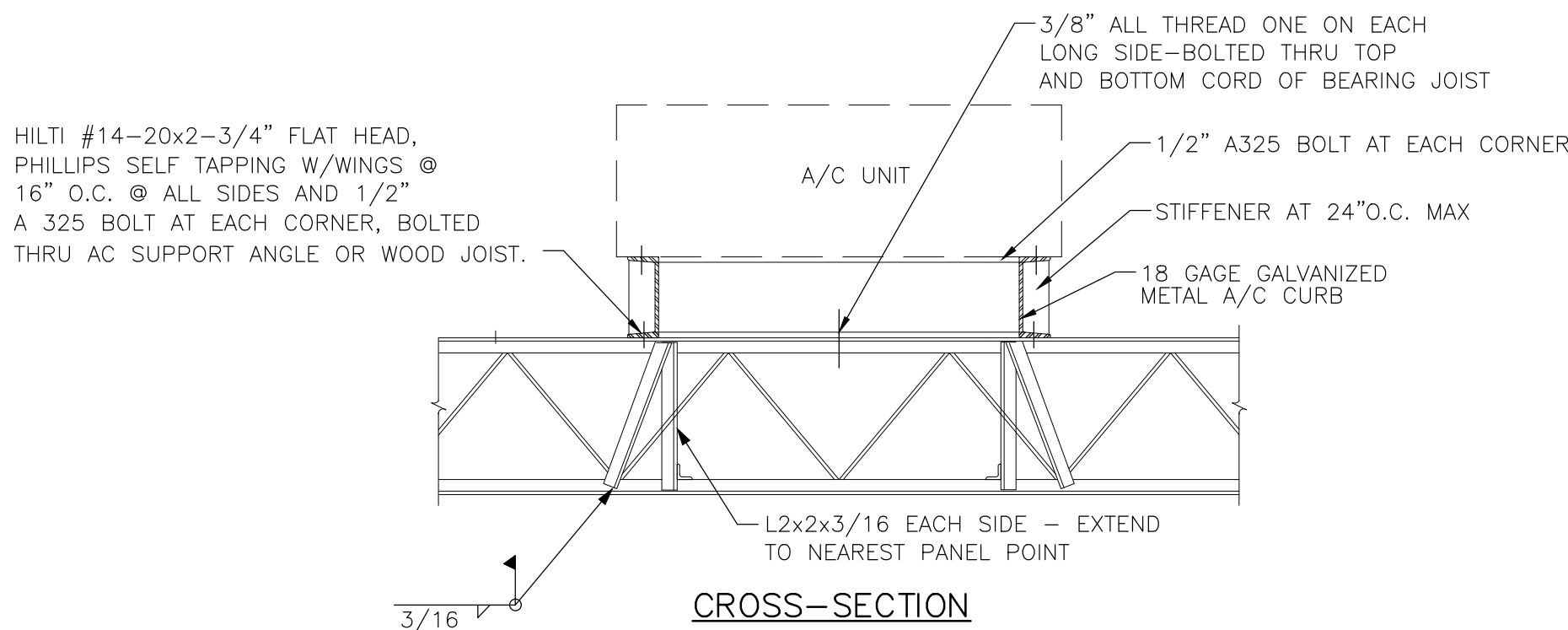
TYPICAL STEPPED FOOTING DETAIL



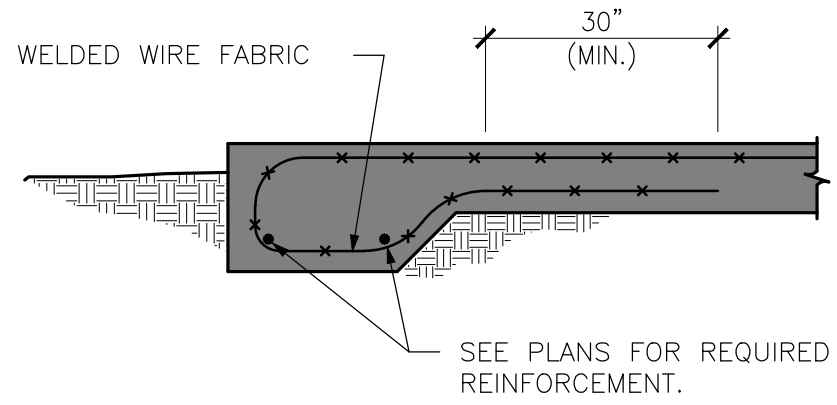
TYPICAL ARCHED OPENING



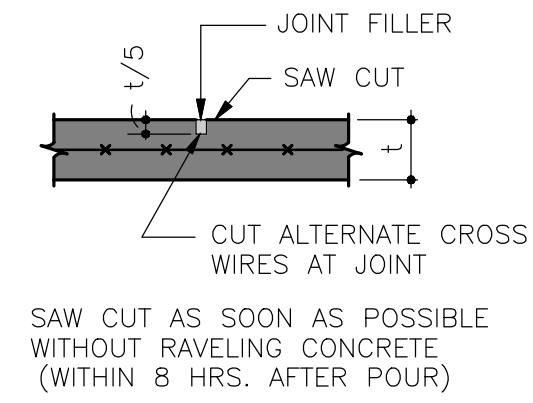
TYPICAL FASTENER LAYOUT DETAIL



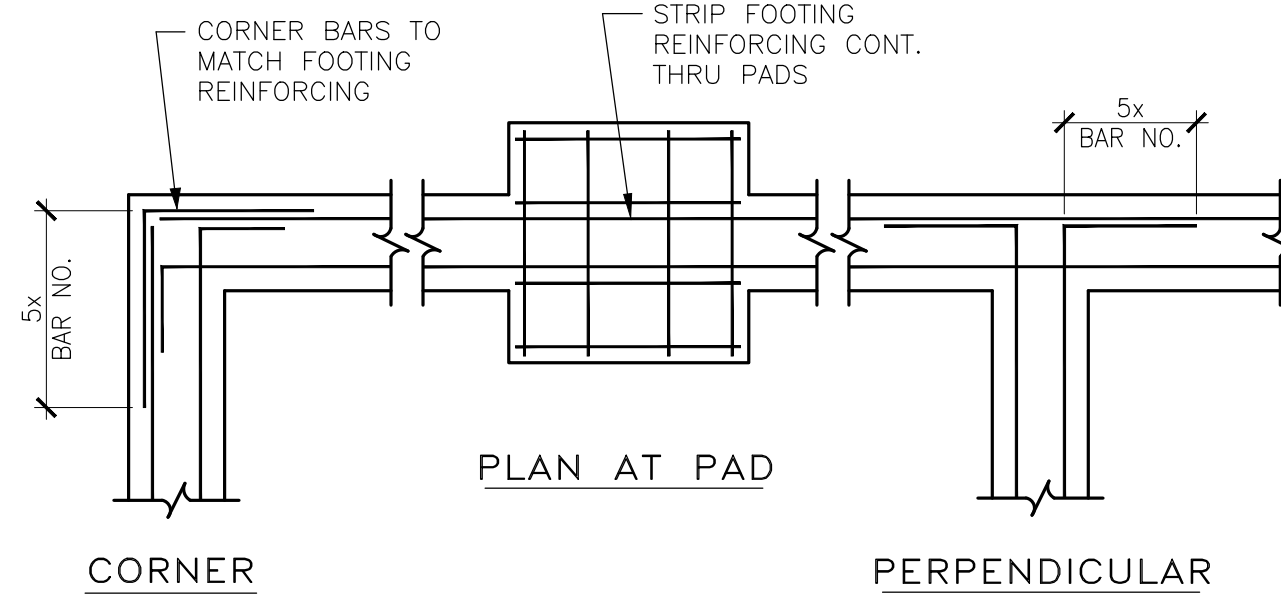
TYPICAL CURB MOUNTED A/C SUPPORT DETAILS



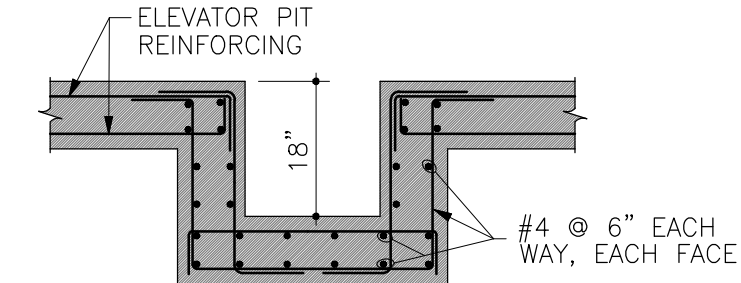
TYPICAL SECTION THRU THICKENED CONCRETE EDGE



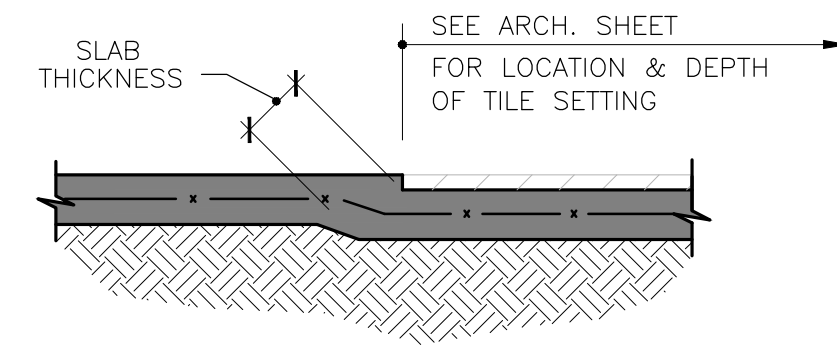
TYPICAL CONTROL JOINT DETAIL (CJ)



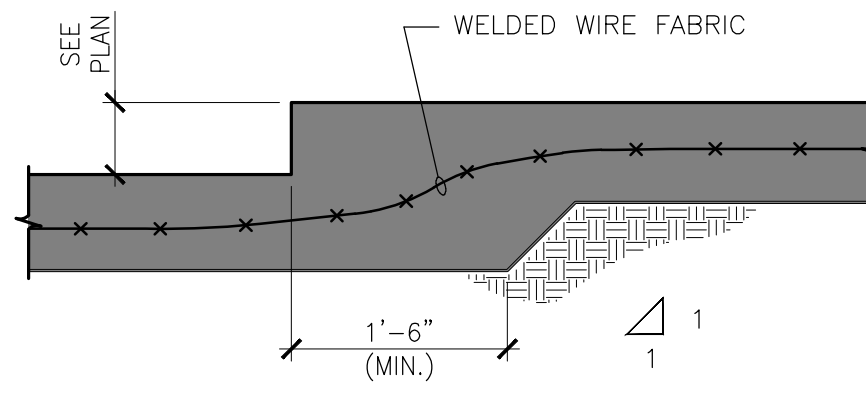
TYPICAL FOOTING INTERSECTON DETAIL



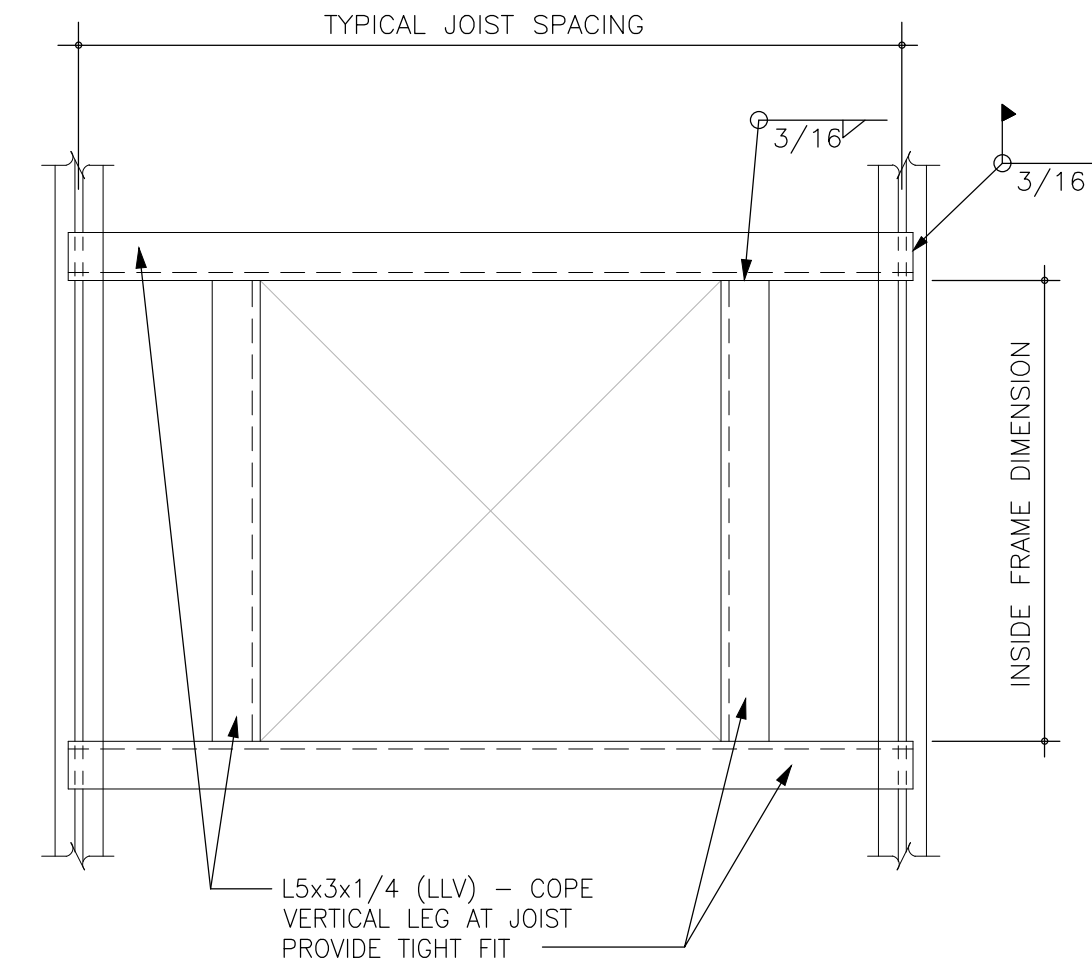
SUMP PIT DETAIL



TYPICAL DETAIL SLAB DEPRESSION AT CONVENTIONAL SET TILES



TYPICAL STEPPED SLAB DETAIL



NOTE: COORDINATE SIZE OF OPENING WITH MECHANICAL REQUIREMENTS.

TYPICAL ROOF OPENING REINFORCING DETAIL

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