ARCH D

GENERAL CONDITIONS:

1. THE FOLLOWING NOTES SHALL APPLY TO ALL STRUCTURAL DRAWINGS.
2. ALL DESIGN AND CONSTRUCTION SHALL BE BASED ON AND IN ACCORDANCE WITH THE 2023 FLORIDA BUILDING CODE, 8TH EDITION.
3. THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS AND NOTIFY THE ARCHITECT PRIOR TO FABRICATION AND CONSTRUCTION. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS, SEE THE ARCHITECTURAL DRAWINGS.
4. IF MATERIAL QUANTITIES, STRENGTHS OR SIZES INDICATED BY THE DRAWINGS OR SPECIFICATIONS ARE NOT IN AGREEMENT WITH THESE NOTES, THE BETTER QUALITY AND/OR QUANTITY, STRENGTH OR NOTED SHALL BE PROVIDED.
5. IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO DETERMINE ERECTION PROCEDURE AND SEQUENCE TO ENSURE THE SAFETY OF THE STRUCTURE AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF TEMPORARY BRACING, GUYS, TIE-DOWNS OR SHORING MAY BE NECESSARY. SUCH MATERIAL SHALL BE REMOVED AND SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER THE COMPLETION OF THE PROJECT.
6. IT SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR FOR INITIATING, MAINTAINING AND SUPERVISING ALL SAFETY PROCEDURES . THE STRUCTURAL ENGINEER OF RECORD IS NOT RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION OR FOR RELATED SAFETY PROCEDURES.
7. THE STRUCTURAL DRAWINGS ARE ONE DISCIPLINE OF THE CONTRACT DOCUMENTS AND DO NOT BY THEMSELVES CONTAIN ALL THE INFORMATION REQUIRED TO PROPERLY COMPLETE THE PROJECT STRUCTURE. THE GENERAL CONTRACTOR SHALL REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS AND COORDINATE THE INFORMATION INDICATED IN THESE CONSTRUCTION DOCUMENTS WITH THE STRUCTURAL DRAWINGS TO PROPERLY CONSTRUCT THE PROJECT.
8. ALL DETAILS, SECTIONS AND NOTES INDICATED ON THE CONSTRUCTION DOCUMENTS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE UNLESS OTHERWISE SHOWN.
9. SPECIALTY ENGINEERED PRODUCTS

A. THE GENERAL CONTRACTOR IS RESPONSIBLE TO COORDINATE THE PROPER SUBMISSION OF SHOP DRAWINGS FOR SPECIALTY ENGINEERED PRODUCTS WHICH SHALL BE SIGNED AND SEALED BY A LICENSED PROFESSIONAL ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED. CONTRACTOR'S RESPONSIBILITY TO ASSURE THAT THE SPECIALTY ENGINEERED SHOP DRAWINGS ARE SUBMITTED IN A TIMELY MANNER SO AS TO ALLOW REVIEWS AND RESUBMISSIONS AS REQUIRED. ALL SPECIALTY ENGINEERED PRODUCTS SHALL BE DESIGNED FOR THE APPROPRIATE GRAVITY LOADS AND WIND LOADS INCLUDING UPLIFT AND LATERAL LOADS. INTERIOR SPECIALTY PRODUCTS SHALL BE DESIGNED FOR LATERAL LOADS TO ASSURE STABILITY. SPECIALTY ENGINEERED PRODUCTS SHALL BE, BUT ARE NOT LIMITED TO, THE FOLLOWING:

1. MISCELLANEOUS STEEL
2. LADDERS
3. HANDRAILS
4. CLADDING SYSTEMS

10. DESIGN LOADS:
    - A. DESIGN ROOF DEAD LOAD = 25 PSF
    - B. DESIGN ROOF LIVE LOAD = 20 PSF
    - C. DESIGN LIVE LOAD FOR FLOOR: 100 PSF
    - D. DESIGN WIND LOAD:
        1. ULTIMATE DESIGN WIND SPEED (3 SECOND GUST), V_{ULT} = 160 MPH. NOMINAL DESIGN WIND SPEED (3 SECOND GUST), V_{ASD} = 124 MPH.
        2. RISK CATEGORY: II
        3. WIND EXPOSURE CATEGORY: C
        4. COMPONENTS AND CLADDING WIND PRESSURE: SEE WIND LOAD SCHEDULE THIS SHT.
        5. INTERNAL PRESSURE COEFFICIENTS +/- 0.18
        6. WIND-BORNE DEBRIS REGION
            A. WITHIN 1 MILE OF THE MEAN HIGH WATER LINE WHERE AN EXPOSURE D CONDITION EXISTS UPWIND OF THE WATER LINE AND THE BASIC WIND SPEED IS EQUAL TO OR GREATER THAN 130 MPH. OR;
            B. IN AREAS WHERE THE BASIC WIND SPEED IS EQUAL TO OR GREATER THAN 140 MPH.
11. SHOP DRAWINGS:
    - A. THE CONTRACTOR SHALL SUBMIT, AS REQUIRED, PRINTS OF SHOP DRAWINGS FOR ALL FABRICATED MATERIALS TO ARCHITECT FOR REVIEW. REPRODUCTION OF CONTRACT DRAWINGS FOR SHOP DRAWINGS WILL NOT BE PERMITTED.
    - B. REVIEW OF SHOP DRAWINGS BY THE ARCHITECT/ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF THOSE SHOP DRAWINGS.
    - C. SHOP DRAWINGS REQUIRING A SPECIAL ENGINEERING DESIGN BY THE FABRICATOR SHALL BE STAMPED BY A PROFESSIONAL ENGINEER OF RECORD IN THE STATE OF THE PROJECT LOCATION BEFORE SUBMITTING FOR REVIEW BY THE ARCHITECT/ENGINEER. THESE DRAWINGS SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF OBSERVATIONS.

12. LIST OF DEFERRED SUBMITTALS: (2)
    - A. CONCRETE MIX DESIGNS
    - B. CONCRETE REINFORCING STEEL AND WELDED WIRE FABRIC
    - C. STRUCTURAL STEEL
    - D. CONCRETE MASONRY UNIT SUBMITTALS AND OTHER MASONRY ACCESSORIES
    - E. STEEL OPEN WEB JOISTS AND METAL DECK
    - F. PRE-FABRICATED CANOPIES AND AWNINGS
    - G. COLD FORMED METAL FRAMING

MASONRY WALL CONSTRUCTION:

1. HOLLOW LOAD BEARING UNITS SHALL BE NORMAL WEIGHT, GRADE N, TYPE 2, CONFORMING TO ASTM C90, WITH A MINIMUM NET COMPRESSIVE STRENGTH OF 2000 PSI (f'm = 1500 PSI).
2. MORTAR SHALL BE TYPE M OR S, CONFORMING TO ASTM C270.
3. COARSE GROUT SHALL CONFORM TO ASTM C476 WITH A MAX. AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI W/ SLUMP OF 8-11 INCHES.
4. VERTICAL REINFORCEMENT SHALL BE AS NOTED ON THE DRAWINGS. FILL ALL CELLS CONTAINING REINFORCING WITH COARSE GROUT.
5. VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM AND AT A MAXIMUM SPACING OF 8'-0". REINFORCEMENT SHALL BE PLACED IN THE CENTER OF THE MASONRY CELL (TYPICAL) UNLESS OTHERWISE NOTED. SEE TYPICAL GROUTING DETAILS FOR ADDITIONAL INFORMATION.
6. REINFORCING STEEL SHALL BE LAPPED MINIMUM 48 BAR DIAMETERS.
7. HORIZONTAL WALL REINFORCING SHALL BE STANDARD LADDER TYPE DUR-O-WAL (9 GA.) HOT DIPPED GALVANIZED AT 16" O.C. (VERTICALLY) UNLESS SHOWN OTHERWISE ON THE DRAWINGS.
8. SPLICED WIRE REINFORCEMENT SHALL BE LAPPED AT LEAST 6" AND CONTAIN AT LEAST ONE CROSS WIRE OF EACH PIECE OF REINFORCEMENT WITHIN THE 6" LAP. USE STANDARD 'T' AND 'L' SHAPED PIECES AT INTERSECTIONS AND CORNERS.
9. WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL CORE, IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL IN SIX VERTICALS. DOWELS SHALL BE GROUTED INTO A CORE IN VERTICAL ALIGNMENT, EVEN THOUGH IT IS IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCEMENT.
10. CONFORM TO BUILDING CODE FOR MASONRY STRUCTURES TMS 402-2016 AND SPECIFICATION FOR MASONRY STRUCTURES TMS 602-16.

FOUNDATION:

1. **FOUNDATION DESIGN: PRIOR TO SUBGRADE PREPARATION, FOUNDATION FABRICATION AND CONSTRUCTION, THE PRESUMPTIVE DESIGN CRITERIA BELOW SHALL BE CONFIRMED BY A LICENSED GEOTECHNICAL ENGINEER IN A REPORT TO BE REVIEWED BY ARCHITECT AND STRUCTURAL ENGINEER.**
    - *A. SHALLOW CONVENTIONAL FOUNDATION WITH ALLOWABLE SOIL BEARING PRESSURE OF 2500 PSF MIN.*
    - *B. MONOLITHIC FOOTINGS W/ 18" MIN. EMBEDMENT BELOW LOWEST ADJACENT FINISH GRADE.*
    - *C. STEM WALL AND SPREAD FOOTINGS WITH 18" MIN. EMBEDMENT BELOW LOWEST ADJACENT FINISH GRADE*
    - *D. ACCEPTABLE TOTAL AND DIFFERENTIAL SETTLEMENTS.*
2. THE SITE SHALL BE PREPARED UNIFORM IN ACCORDANCE WITH CIVIL DRAWINGS, SPECIFICATIONS, SOILS REPORT AND THE ALLOWABLE BEARING PRESSURE.
3. ALL EXCAVATIONS AND BUILDING PADS SHALL BE INSPECTED BY A QUALIFIED GEOTECHNICAL ENGINEER TO VERIFY THE DESIGN ASSUMPTIONS AND REPORT ADVERSE CONDITIONS.
4. WHERE FILL IS REQUIRED, IT SHALL BE PLACED IN ACCORDANCE WITH INSTRUCTIONS OF A QUALIFIED GEOTECHNICAL ENGINEER TO MAINTAIN DESIGN BEARING PRESSURE.
5. FOOTING ELEVATIONS GIVEN ARE FOR THE PURPOSE OF DESIGN. SOIL BELOW FOOTING NOT MEETING DESIGN BEARING PRESSURE SHALL BE EXCAVATED TO A DEPTH OF VERIFIABLE DESIGN PRESSURE AND BACKFILLED PER SOIL REPORT RECOMMENDATIONS TO LEVEL OF FOUNDATION BEARING. THIS SHALL BE APPROVED BY A QUALIFIED GEOTECHNICAL ENGINEER.
6. ALL EXCAVATION SHALL BE KEPT DRY. EXCAVATE TO DEPTHS AND DIMENSIONS INDICATED. TAKE EVERY PRECAUTION TO GUARD AGAINST ANY MOVEMENT OR SETTLEMENT OF ADJACENT STRUCTURES, UTILITIES, PIPING, ETC.
7. PROVIDE ANY BRACING OR SHORING NECESSARY TO AVOID SETTLEMENT OR DISPLACEMENT OF EXISTING FOUNDATION OR STRUCTURES.
8. BACKFILL AGAINST WALLS SHALL BE PLACED EVENLY EACH SIDE UNLESS SHORING IS PROVIDED BY THE CONTRACTOR. SHORING SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND SHALL REMAIN IN PLACE UNTIL STRUCTURAL ELEMENT BRACING THE WALL ARE IN PLACE AND HAVE REACHED FULL DESIGN STRENGTH.
9. TERMITE PROTECTION: TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMITICIDES, INCLUDING SOIL APPLIED PESTICIDES, BAITING SYSTEMS, AND PESTICIDES APPLIED TO WOOD, OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELED FOR USE AS A PREVENTATIVE TREATMENT TO NEW CONSTRUCTION. SEE SECTION 202 "REGISTERED TERMITICIDE". UPON COMPLETION OF THE APPLICATION OF THE TERMITE PROTECTIVE TREATMENT, A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING DEPARTMENT BY THE LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. TREATMENT IS IN ACCORDANCE WITH RULES AND LAWS ESTABLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES."

CONCRETE:

1. CONCRETE MEMBERS TO HAVE THE FOLLOWING MINIMUM COMPRESSIVE STRENGTHS AT 28 DAYS:
    - A. FOUNDATION, SLAB ON GRADE 3000 PSI
    - B. BEAMS, COLUMNS 4000 PSI
2. ALL CONCRETE SHALL BE READY MIX AND MEET THE FOLLOWING REQUIREMENTS:
    - A. CONCRETE SLUMP SHALL BE 4 INCHES PLUS OR MINUS 1".
    - B. CONCRETE SHALL HAVE 2 TO 4 PERCENT AIR ENTRAINMENT.
    - C. ALL CONCRETE TO HAVE MAXIMUM WATER/CEMENT RATIO OF 0.54.
3. CONCRETE MIX SHALL BE IN ACCORDANCE WITH THE 2010 EDITION OF ACI 301 CHAPTER 3, METHOD 1 OR METHOD 3. CONTRACTOR SHALL SUBMIT BACKUP DATA PER CHAPTER 5 SECTION 5.3 OF ACI 318-14.
4. ALL REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM A-615 GRADE 60. SUBMIT ALL REINFORCING STEEL SHOP DRAWINGS FOR APPROVAL PRIOR TO ANY FABRICATION.
5. CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS REQUIRED BY ACI SPECIFICATIONS.
6. WELDED WIRE FABRIC SHALL CONFORM WITH ASTM A 1064. WELDED WIRE FABRIC SHALL BE LAPPED AT LEAST 12 INCHES UNLESS NOTED OTHERWISE.
7. LAP SPLICE FOR BARS, U.N.O.: 31" FOR #4 BARS, 39" FOR #5 BARS, 47" FOR #6 BARS.
8. PROVIDE ACI STANDARD HOOKS UNLESS NOTED OTHERWISE ON DRAWINGS.
9. ALL CONCRETE WORK SHALL CONFORM TO ACI 318-19 "THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE STRUCTURES", AND ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS."
10. ALL CONCRETE DETAILS SHALL CONFORM TO ACI 315-05 "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" UNLESS NOTED OTHERWISE ON THE DRAWINGS.
11. CONTRACTOR SHALL REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS AND SIZES OF SLEEVES, OPENINGS, EMBEDDED ITEMS, SLAB RECESSES, SLOPES, ECT. THESE ITEMS SHALL BE COORDINATED WITH OTHER TRADES AND INSTALLED PRIOR TO CONCRETE PLACEMENT.
12. CONTRACTOR SHALL VERIFY ANCHOR BOLT SIZES AND LOCATIONS PRIOR TO CONCRETE PLACEMENT.
13. BAR LENGTHS PROVIDED ON DRAWINGS DO NOT INCLUDE HOOK LENGTH. HOOKS SHALL BE PROVIDED AT TOP BARS AT BEAM ENDS AND SLAB EDGES.
14. CONTRACTOR SHALL PROVIDE CHAIRS, BOLSTERS, SPACERS, ETC. AS REQUIRED TO SECURELY SUPPORT REINFORCEMENT. SUPPORT ITEMS ON EXPOSED CONCRETE SHALL BE PLASTIC SUPPORT REINFORCEMENT. TIPPED OR STAINLESS STEEL. IN HIGHLY CORROSIVE ENVIRONMENTS, SUPPORT ITEM SHALL BE PLASTIC.
15. THE CONTRACTOR SHALL SEAL ALL HAIRLINE CRACKING IN CONCRETE SLAB ON GRADE WITH A CRACK SUPPRESSION KIT SUCH AS LATICRETE OR APPROVED EQUAL.
16. A COPY OF THE "FIELD REFERENCE MANUAL" ACI MNL-15 (20) SHALL BE KEPT BY THE CONTRACTOR ON SITE.
17. ONE COPY OF ALL THE CONCRETE TEST RESULTS SHALL BE SUBMITTED BY THE TESTING AGENCY DIRECTLY TO THE ENGINEER OF RECORD.

METAL DECK:

1. STEEL ROOF DECK SHALL BE:
    - A. FINISHED AND GAGE AS SHOWN ON ROOF PLAN AS MANUFACTURED BY VULCRAFT/NUCOR OR APPROVED EQUAL.
    - B. MANUFACTURER SHALL BE A MEMBER OF THE STEEL DECK INSTITUTE.
    - C. ROOF DECK MUST COMPLY WITH STEEL DECK INSTITUTE STANDARDS. ALL ROOF DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS.
2. ALL ROOF DECK TO BE DESIGNED, MANUFACTURED, AND INSTALLED IN ACCORDANCE WITH LATEST FACTORY MUTUAL STANDARDS.

STRUCTURAL STEEL:

1. CONFORM TO AISC MANUAL OF STEEL CONSTRUCTION, 15TH EDITION (2017)
2. MATERIALS:
    ASTM A 992, FY=50 KSI IN GENERAL.
    ASTM A 500, GRADE B, FY=46 KSI FOR STRUCTURAL TUBING.
    ASTM A 501, FY=36 KSI FOR PIPES.
    ASTM A 325, TYPE I, FOR HIGH STRENGTH BOLTS.
    ASTM A 307, GRADE A, ANCHOR BOLTS.
    ASTM A 36, FY=36 KSI FOR PLATES, BARS, RODS, AND ANGLES.
    ASTM A 563, HEAVY HEX NUTS.
    ASTM A 436, HARDENED STEEL WASHERS.
3. THE DESIGN OF CONNECTIONS FOR ANY PORTION OF THE STRUCTURE NOT INDICATED ON THE DRAWINGS SHALL BE DESIGNED BY THE FABRICATOR AS FOLLOWS:

    A. STANDARD CONNECTIONS SHALL BE USED WHERE POSSIBLE.

    B. ALL SHOP CONNECTIONS SHALL BE WELDED OR HIGH STRENGTH BOLTED. FIELD CONNECTIONS SHALL BE HIGH STRENGTH BOLTED WHERE POSSIBLE.

    C. UNLESS NOTED, BOLTS SHALL BE 3/4" DIAMETER ASTM A-325 TYPE N BEARING CONNECTIONS. BOLTS SHALL BE "SNUG TIGHT."

4. MEMBERS SUPPORTING DECK AT THE PERIMETER OF THE BUILDING SHALL BE CONTINUOUS EXCEPT AT EXPANSION JOINT. BUTT WELD CONTINUOUS MEMBERS PLACED END TO END.
5. STRUCTURAL STEEL SHALL RECEIVE ONE SHOP COAT OF RUST INHIBITIVE PRIMER UNLESS THE STEEL IS TO RECEIVE SPRAY ON CEMENTITUOUS FIREPROOFING. SEE ARCHITECTURAL DRAWINGS. ALL EXPOSED STEEL SHALL BE HOT DIPPED GALVANIZED OR COATED WITH CORROSION INHIBITED PAINT.
6. STEEL COLUMNS AND BASE PLATES SHALL HAVE MINIMUM 3" CONCRETE COVER PROTECTION.
7. EXPOSED WELDS SHALL RECEIVE COAT OF RUST INHIBITIVE PRIMER PER STEEL SUPPLIER'S RECOMMENDATIONS

OPEN WEB STEEL JOISTS:

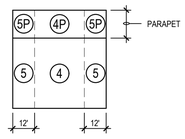
1. ALL JOISTS SHALL BE FABRICATED WITH STANDARD CAMBER AND ERECTED IN ACCORDANCE WITH STEEL JOIST INSTITUTE, SJI-100-15.
2. K-SERIES STEEL JOISTS SHALL BE CONNECTED TO STEEL BY 1/8" WELD, 1-1/2" EACH SIDE or (2) 1/2" DIAMETER BOLTS.
3. EXTEND LOWER JOIST CHORD AT ALL COLUMNS PER OSHA REQUIREMENTS. EXTEND AFTER DEAD LOADS ARE APPLIED.
4. HORIZONTAL BRIDGING SHALL BE AN ANGLE AT TOP AND BOTTOM, DESIGNED FOR L/r=300 OR LESS (U.N.).
5. CROSS BRIDGING SHALL BE AN ANGLE DESIGNED FOR L/r=200 OR LESS UNLESS NOTED.
6. BOTTOM CHORD OF ROOF JOIST SHALL BE DESIGNED FOR NET UPLIFT OF 23 PSF.
7. UNLESS NOTED K-SERIES STEEL JOIST SHALL HAVE 2 1/2" DEEP BEARING. WHERE STEEL JOIST OR GIRDER SLOPE EXCEEDS 1/4" PER FT., PROVIDE SLOPED BEARING.
8. ALL JOIST SHALL RECEIVE A COAT OF RUST INHIBITIVE PRIMER. FINISH JOIST PER ARCHITECT.
9. STEEL JOISTS SHALL BE DESIGNED, FABRICATED AND ERECTED TO THE REQUIREMENTS OF THE SPECIFICATIONS OF THE STEEL JOIST INSTITUTE FOR SERIES K JOISTS.
10. MANUFACTURER SHALL BE A MEMBER OF THE STEEL JOIST INSTITUTE. PROVIDE BRIDGING IN ACCORDANCE WITH SJI STANDARDS UNLESS NOTED OTHERWISE ON THE DRAWINGS.

COLD FORMED METAL FRAMING (METAL STUDS AND JOISTS):

1. DESIGN, FABRICATIONS AND ERECTION SHALL CONFORM TO AISI "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS", LATEST EDITION. ALL METAL STUDS SHALL BE GALVANIZED.
2. ALL 43 MIL (18 GAUGE) AND 33 MIL (20 GAUGE) STUDS, JOISTS, TRACK, BRIDGING END CLOSURES AND ACCESSORIES SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE REQUIREMENTS OF AISI "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" WITH A MINIMUM YIELD OF 33,000 PSI (U.N.O.).
3. ALL MATERIAL AND ACCESSORIES SHALL BE FORMED FROM STEEL HAVING A G-60 GALVANIZED COATING, MEETING ASTM A 653.
4. UNLESS NOTED, ALL SCREWS OR PINS SHALL BE NON CORROSIVE NO. 8-18 (D=.125") OR LARGER (DO NOT USE STAINLESS STEEL OR COPPER COATED FASTENERS).
5. UNLESS NOTED, TRACKS SHALL BE SAME DEPTH AS STUDS OR JOISTS AND EQUAL OR THICKER GAUGE THAN STUDS OR JOISTS. TRACKS SHALL BE CONNECTED TO SUPPORTS AT 16" MAX. STUDS OR JOISTS SHALL BE CONNECTED TO TRACKS AT EACH SIDE.
6. THE QUANTITY OF STUDS AND JOISTS DISPLACED OR CUT FOR OPENING SHALL BE PLACED HALF ON EACH SIDE OF OPENING PER METAL STUD HEADER SCHEDULE ON TYPICAL DETAIL SHEET.
7. SEE FRAMING NOTES THIS SHEET.

COMPONENT AND CLADDING WALL WIND PRESSURES (PSF)

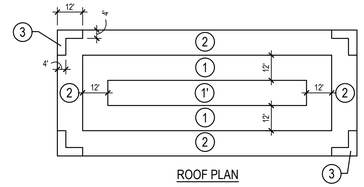
WALL ZONES \ EFFECTIVE WIND AREA	V_{ASD} = 124 MPH, EXP. C RISK CATEGORY II, GCpi = +/- 0.18			
	10	20	50	100
4	+35.7/-38.5	+34.0/-36.9	+31.9/-34.8	+30.3/-33.3
5	+35.7/-47.6	+34.0/-44.4	+31.9/-40.2	+30.3/-36.9
4P	+110.4/-74.2	+103.9/-70.9	+95.5/-66.7	+89.1/-63.5
5P	+110.4/-83.3	+109.9/-78.4	+95.5/-72.1	+89.1/-67.2



WALL ELEVATION

COMPONENT AND CLADDING ROOF WIND PRESSURES (PSF)

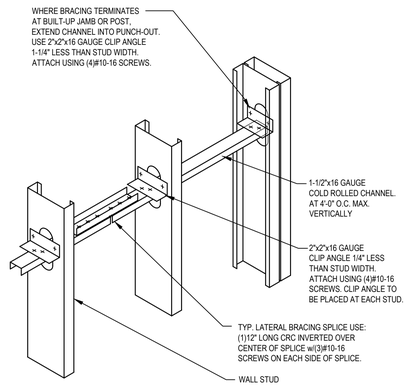
ROOF ZONES \ EFFECTIVE WIND AREA	V_{ASD} = 124 MPH, EXP. C RISK CATEGORY II, GCpi = +/- 0.18			
	10	20	50	100
1	+14.6/-56.8	+13.6/-53.0	+12.3/-48.0	+11.4/-44.3
1'	+14.6/-32.5	+13.6/-32.5	+12.3/-32.5	+11.4/-32.5
2	+14.6/-74.7	+13.6/-69.9	+12.3/-63.5	+11.4/-58.9
3	+14.6/-101.8	+13.6/-92.3	+12.3/-79.5	+11.4/-69.9



ROOF PLAN

METAL STUD FRAMING NOTES:

1. EXTERIOR METAL STUD FRAMING: (SEE ARCH FOR LOCATION AND EXTENTS OF METAL STUD FRAMING)
    - A. METAL STUDS SHALL BE 6" X 1-5/8 X 18GA. METAL STUDS AT 16" O.C. MAX.
    - B. METAL TRACKS: U.N.O., CONT. METAL TRACKS SHOWN ON SECTIONS SHALL BE 18 GAGE.
    - C. SEE BRIDGING AND BRACING DETAILS THIS SHEET.
2. CONNECTIONS (UNLESS NOTED OTHERWISE):
    - A. *TRACK TO STUD*: ATTACH TRACK TO EACH STUD W/ (1) #10 TEK SCREW AT EACH FLANGE.
    - B. *TRACK TO CONC. OR C.M.U.*: ATTACH TRACK TO CONC. OR C.M.U. W/ (2) 1/4"Ø X 1" EMBED TAPCONS NEAR EA. END AND AT 16" O.C. MAX. IN BETWEEN.
    - C. *TRACK TO STRUCTURAL STEEL*: ATTACH TRACK TO STRUCTURAL STEEL W/(2) 0.145 PAF'S NEAR EA. END AND AT 16" O.C. MAX. IN BETWEEN.
    - D. *TRACK TO METAL ROOF DECK*: ATTACH TRACK TO METAL ROOF DECK W/ (2) #12 TEK SCREWS NEAR EA. END AND AT 12" O.C. MAX. IN BETWEEN.
    - E. *CLIP TO C.M.U. OR CONCRETE*: SHALL BE WITH 3X3X3" LONG X 16GA CLIP FOR 3 5/8" STUDS AND 4X4X4" LONG X 16GA CLIP FOR LARGER SIZE STUDS, W/ (4) 1/4"Ø X 1" EMBED. TAPCONS TO C.M.U. OR CONCRETE, AND (4) #10 TEK SCREWS TO STUD, TYP.
    - F. *CLIP TO STRUCTURAL STEEL*: SHALL BE WITH 3X3X3" LONG X 16GA CLIP FOR 3 5/8" STUDS AND 4X4X4" LONG X 16 GA. FOR LARGER SIZE STUDS, W/ (4) O.145 PAF TO STRUCTURAL STEEL, AND (4) #10 TEK SCREWS TO STUD, TYP.
    - G. *CLIP TO METAL STUD*: SHALL BE WITH 3X3X3" LONG X 16GA CLIP FOR 3 5/8" STUDS AND 4X4X4" LONG X 16GA. CLIP FOR LARGER SIZE STUDS, W/ AND (4) #10 TEK SCREWS TO STUD EA. LEG, TYP.
    - H. *STUD WEB TO STUD WEB*: SHALL BE WITH (4) #10 TEK SCREWS.
3. SHEATHING:
    - A. SEE ARCH DRAWINGS FOR LOCATIONS AND EXTENTS
    - B. ATTACH 5/8" CDX PLYWOOD TO STUDS W/ #10 X 1 1/2" TEK SCREWS AT 6" O.C AT PANEL EDGES, AND AT 12" O.C. IN FIELD.
    - C. ATTACH 5/8"" DENSGLASS TO METAL STUDS W/#8 X 1-1/4" BUGLE HEAD SCREWS AT 8" O.C. AT PANEL EDGES, AND AT 8" O.C. IN FIELD.



TYPICAL LT. GAUGE LATERAL BRACING

NOTE: DETAIL IS NOT REQUIRED IF BOTH FLANGES OF METAL STUDS ARE SHEATHED

SMA
ARCHITECTURE and INTERIORS

SMA ARCHITECTURE + INTERIORS
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SEAL

THIS ITEM HAS BEEN ELECTRONICALLY SIGNED AND SEALED BY JUAN B CANO, P.E. ON 2/26/2026 USING A DIGITAL SIGNATURE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JUAN BAUTISTA CANO LICENSE No. 69377 STATE OF FLORIDA PROFESSIONAL ENGINEER

JUAN B. CANO, P.E.
FLORIDA LICENSE NO. 69377

PROJECT INFORMATION

PROJECT:

FLORIDA BLUE

PROJECT ADDRESS:

THE SHOPPES AT ST LUCIE WEST
PORT ST LUCIE, FL

PROJECT NO.: 250044

ACTIVE DESIGN PHASE

- ☐ FOR REVIEW
- ☐ FOR PERMITTING
- ☐ SCHEMATIC DESIGN
- ☐ DESIGN DEVELOPMENT
- ☐ CONSTRUCTION BIDDING
- ■ CONSTRUCTION DOCUMENTS
- ☐ AS-BUILT RECORD SET

REVISION INFORMATION

NO.	DATE	DESCRIPTION
2	02/27/26	PERMIT COMMENTS / TENANT COORD.

KEY PLAN

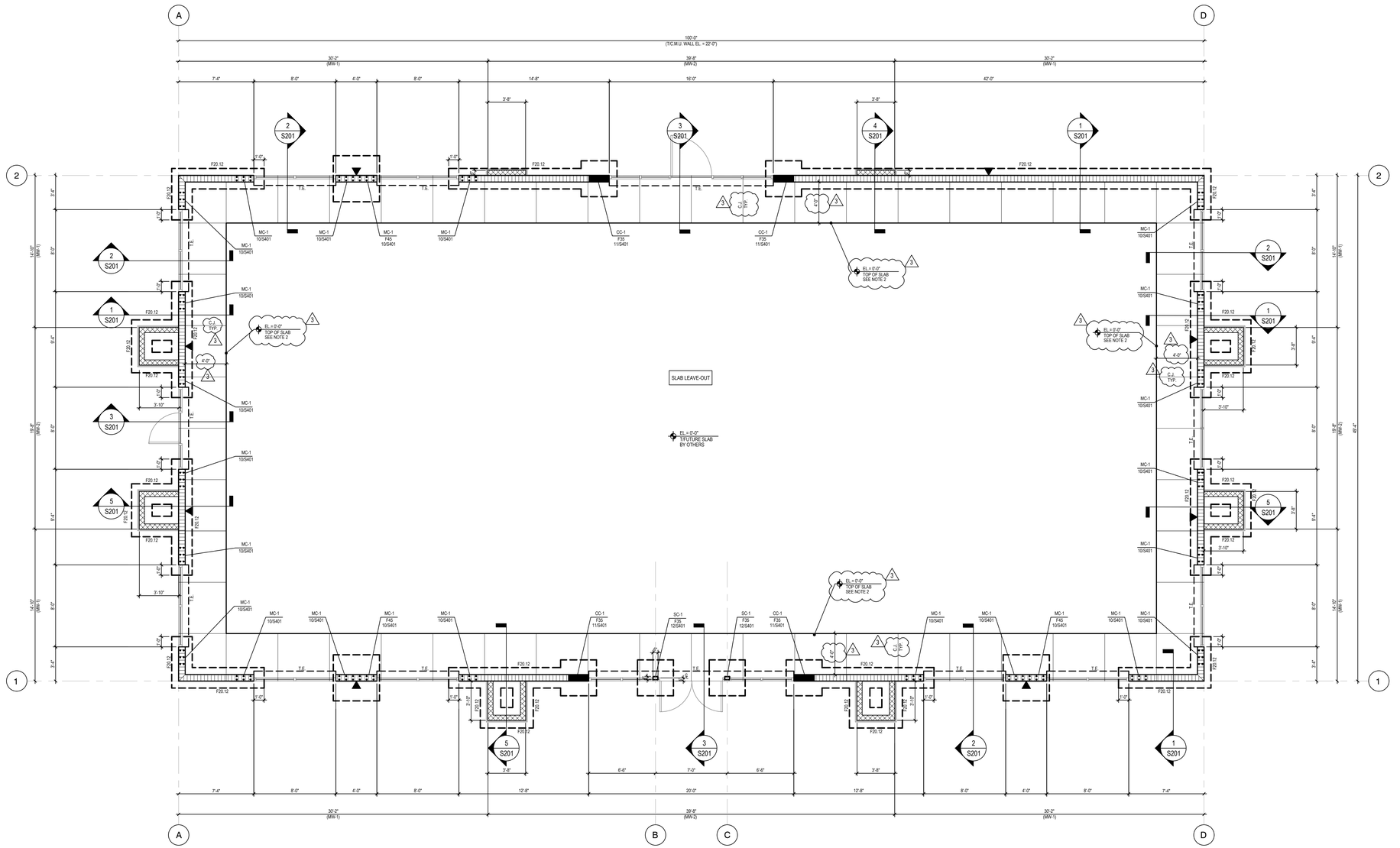
SHEET INFORMATION

SHEET ISSUED: 01/09/2026
DESIGNED BY: JBC
DRAWN BY: RED
REVIEWED BY: JBC
SHEET TITLE:

Structural Notes

SHEET NO.:

S001



FOOTING SCHEDULE		
MARK	SIZE	REINFORCING
F20.12	2'-0" WIDE X 12" DEEP X CONT.	(2) #5 CONT. BOTTOM AND #5 AT 48" O.C. TRANSV. BOTT.
F3S	3'-6" X 3'-6" X 12" DEEP	(4) #5 E.W. BOTTOM
F4S	4'-6" X 4'-6" X 12" DEEP	(5) #5 E.W. BOTTOM

MASONRY WALL SCHEDULE					
LABEL	TYPE	VERT. REINF. AT FULL HEIGHT SECTIONS	VERT. REINF. BELOW AND ABOVE OPENINGS	JAMBS EA. SIDE OF OPENING (U.N.O.)	ENDS AND CORNERS
MW-1	8" C.M.U.	#5 AT 16" O.C. (CENTERED)	#5 AT 16" O.C. (CENTERED)	SEE PLAN	PER S201
MW-2	8" C.M.U.	(2) #5 AT 16" O.C., (1 EA. FACE)	(2) #5 AT 16" O.C., (1 EA. FACE)	SEE PLAN	PER S201
C.M.U. CURB UNDER METAL STUDS (REAR WALL)	6" C.M.U.	#4 AT 24" O.C. (CENTERED)	N/A	N/A	PER S201
C.M.U. CURB UNDER METAL STUDS (FRONT, SIDE WALLS)	8" C.M.U.	#4 AT 24" O.C. (CENTERED)	N/A	N/A	PER S201

NOTES: 1. ALL MASONRY VERT. REINF. CELLS SHALL BE FILLED W/ 3000 PSI GROUT MIX, TYP.
2. PROVIDE HORIZONTAL JOINT REINF. PER. S201, MASONRY WALL NOTE 7.

1 Foundation Plan

1/4" = 1'-0"

FOUNDATION NOTES:

1.) EL. = +0'-0" IS FOR REFERENCE ONLY. SEE C/VIL FOR NAVD ELEVATION.

2.) 4" MIN.THICK CONC. SLAB ON GRADE REINF. W/ 6X6 W2.1XW2.1 WWF (MID-DEPTH) ON VAPOR BARRIER (SEE ARCH) ON PREPARED SUBGRADE PER GEOTECHNICAL ENGINEER'S RECOMMENDATIONS.

3.) MW-# DENOTES MASONRY WALLS. SEE SCHEDULE THIS SHEET.

4.) DENOTES METAL STUD FRAMING ON C.M.U. CURBS, SEE ARCH FOR LOCATION AND EXTENTS, SEE S001, SEE SECTIONS.

5.) MC-# DENOTES MASONRY COLUMN, SEE DETAIL ON S401.

6.) CC-# DENOTES CONCRETE COLUMN, SEE DETAIL ON S401.

7.) SC-# DENOTES STEEL COLUMN, SEE SCHEDULE THIS SHEET.

8.) F-# DENOTES STEM OR SPREAD CONCRETE FOOTING, SEE SCHEDULE THIS SHEET. TOP OF CONCRETE FOOTING ELEVATION = -1'-4" BELOW TOP OF SLAB. BOTTOM OF ALL FOOTINGS SHALL BE 18" MIN. BELOW LOWEST ADJACENT FINISH GRADE. VERIFY ALL TOP AND BOTTOM OF FOOTING ELEVATIONS W/ CIVIL AND MEP PRIOR TO FABRICATION AND CONSTRUCTION. SEE FOUNDATION NOTE 1 ON S001.

9.) T.E. DENOTES CONCRETE THICKENED EDGE, SEE SECTION.

10.) DENOTES C.M.U. WALL CONTROL JOINT LOCATION. SEE DETAIL ON S401.

11.) C.J. DENOTES SLAB ON GRADE CONTROL JOINT, SEE DETAIL ON S401.

12.) SEE ARCH FOR DIMENSIONS NOT SHOWN.

13.) SEE ARCH AND MEP FOR ALL SLEEVES, PIPES, INSERTS AND EMBEDDED ITEMS.

14.) SEE ARCH AND MEP FOR SLAB ON GRADE SLOPES AND RECESSES.

15.) G.C. VERIFY W/ARCH/MEP DRAWINGS FOR ROOF DRAIN LINES, STORM LINES, FIRE RISERS, AND SANITARY LINES.

STEEL COLUMN SCHEDULE			
MARK	SIZE	BASE PL.	A.B.
SC-1	HSS 6x4x3/8	10x10x3/4	(4) 3/4"Ø F1554 GRADE 36
NOTE: ANCHOR BOLT EMBEDMENT = FOOTING THICKNESS - 4"			

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JUAN BAUTISTA CANO
69377
STATE OF FLORIDA
PROFESSIONAL ENGINEER
04/07/26
FOR REVIEW

JUAN B. CANO, P.E.
FLORIDA LICENSE NO. 69377

PROJECT INFORMATION

PROJECT:

FLORIDA BLUE

PROJECT ADDRESS:

THE SHOPPES AT ST LUCIE WEST
PORT ST LUCIE, FL

PROJECT NO.: 250044

ACTIVE DESIGN PHASE

- ☐ FOR REVIEW
- ☐ FOR PERMITTING
- ☐ SCHEMATIC DESIGN
- ☐ DESIGN DEVELOPMENT
- ☐ CONSTRUCTION BIDDING
- ■ CONSTRUCTION DOCUMENTS
- ☐ AS-BUILT RECORD SET

REVISION INFORMATION

NO.	DATE	DESCRIPTION
3	04/07/26	PERMIT COMMENTS

KEY PLAN

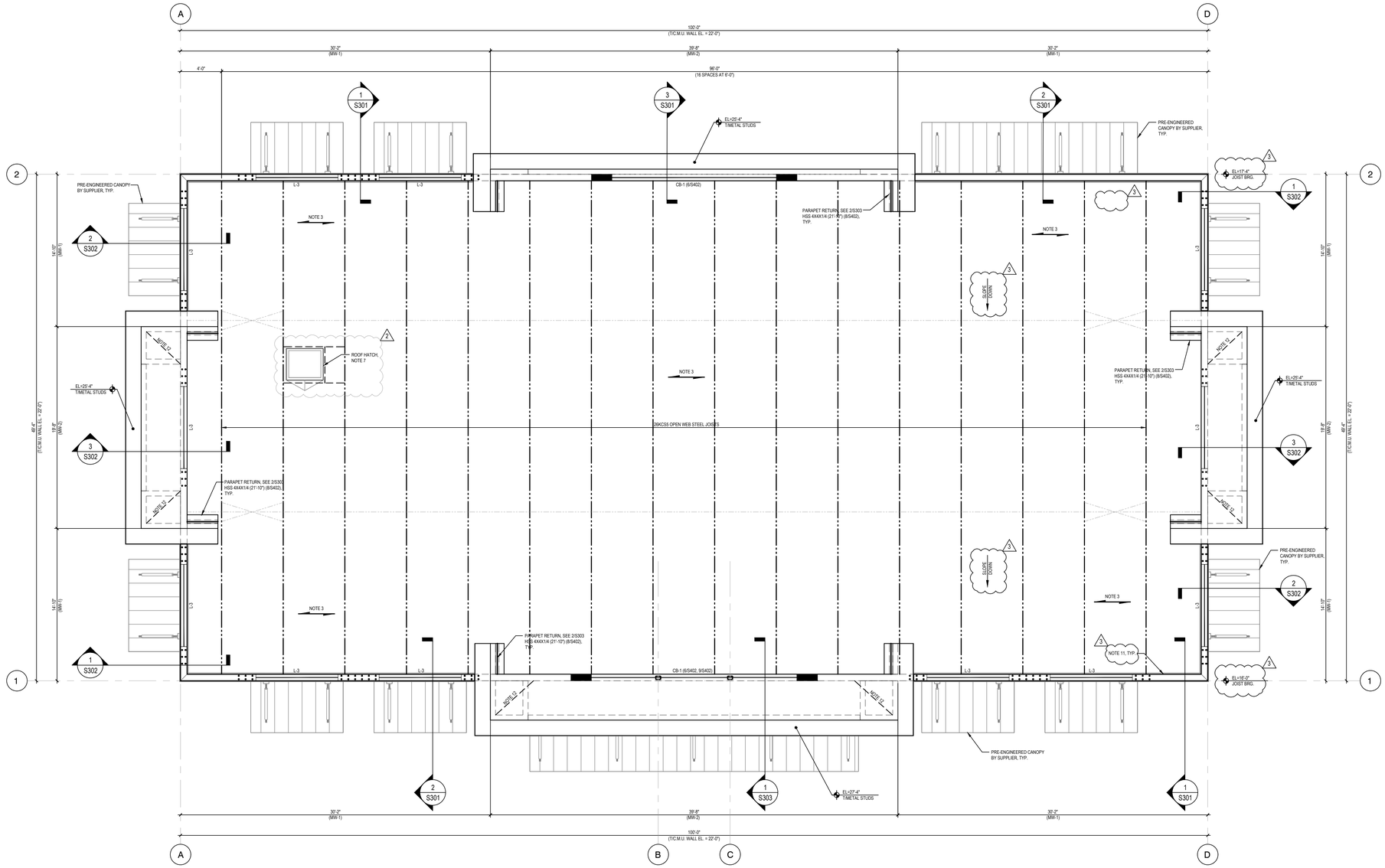
SHEET INFORMATION

SHEET ISSUED: 01/09/2026
DESIGNED BY: JBC
DRAWN BY: RED
REVIEWED BY: JBC
SHEET TITLE:

Foundation Plan

SHEET NO.:

S101



1 Roof Framing Plan

1/4" = 1'-0"

FRAMING NOTES:

1. OPEN WEB STEEL JOISTS SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE SPECIFICATION ON THE STEEL JOIST INSTITUTE. BOTTOM CHORD FOR K SERIES JOISTS SHALL BE ANGLES OR TEES. SEE STRUCTURAL NOTES FOR BRIDGING REQUIREMENTS. JOISTS SHALL BE SPACED AS SHOWN ON PLAN. JOIST MANUFACTURER SHALL BE A MEMBER OF THE STEEL JOIST INSTITUTE.
2. ——▷|◁—— DENOTES JOIST BRIDGING PER JOIST MANUFACTURER'S RECOMMENDATIONS.
3. ROOF METAL DECK SHALL BE 1 1/2" TYPE B 20 GAGE GALVANIZED (VERIFY FINISH W/ ARCH) ORIENTED PERPENDICULAR TO SUPPORTS. ATTACH DECK TO SUPPORTS WITH 5/8"Ø PUDDLE WELDS WITH USING A 36/7 PATTERN AND AT 6" O.C. AT DECK EDGE, AND WITH (4) #10 TEK SCREWS SIDE LAP FASTENERS BETWEEN SUPPORTS, TYP.
4. CB-# DENOTES CONCRETE BEAM, SEE SCHEDULE THIS SHEET.
5. L-# DENOTES CAST-CRETE PRECAST LINTEL, SEE SCHEDULE ON S402.
6. ROOFTOP MECHANICAL EQUIPMENT: PRIOR TO FABRICATION AND CONSTRUCTION, THE GC SHALL VERIFY ROOFTOP EQUIPMENT QUANTITY, LOCATION, AND WEIGHTS W/ ARCHITECT, MECHANICAL ENGINEER, TENANT IMPROVEMENT DRAWINGS, AND EQUIPMENT SUPPLIER. REPORT ANY DISCREPANCIES TO ARCHITECT AND STRUCTURAL ENGINEER.
7. ROOF OPENINGS: PRIOR TO FABRICATION AND CONSTRUCTION, THE GC SHALL VERIFY ROOF OPENINGS QUANTITY, LOCATION, AND EXTENTS W/ ARCHITECT, MECHANICAL ENGINEER, TENANT IMPROVEMENT DRAWINGS, AND EQUIPMENT SUPPLIER. REPORT ANY DISCREPANCIES TO ARCHITECT AND STRUCTURAL ENGINEER. PROVIDE ANGLE SUPPORT FRAMING PER 2/S402.
8. RTU'S: SEE NOTES 6. REINFORCE SUPPORTING JOISTS PER 1/S402, PROVIDE ANGLE SUPPORT FRAMING PER 2/S402, AND ATTACH UNIT CURB TO ROOF STRUCTURE PER 3/S402.
9. SEE ARCH FOR DIMENSIONS NOT SHOWN.
10. THE G.C. SHALL PROVIDE ROOF DRAINS AND SCUPPERS AS REQUIRED TO ALLOW FOR COMPLETE DRAINAGE OF ROOF AT ALL TIMES TO AVOID PONDING OF WATER.
11. AT SCUPPER LOCATIONS (SEE ARCH FOR LOCATIONS), PROVIDE CAST-CRETE PRECAST LINTEL 6F8-1B ABOVE OPENING, TYP.
12. 6"X 1 5/8"-18 GA METAL STUD DIAGONAL NEAR TOP OF C.M.U. WALL AND NEAR BOTTOM OF STUDS ABOVE STREFRONT, ATTACH TO C.M.U. WALL W/ CLIP PER S001, ATTACH TO METAL STUDS W/ (4) #10 TEK SCREWS, TYP.

CONCRETE BEAM SCHEDULE

MARK	SIZE BxH	REINFORCING			STIRRUPS	
	(inches)	BOTT	TOP	INT	TIES	SPACING
CB-1	8 X 24	(4)#6 (IN 2 LAYERS) (CONTINUOUS)	(2)#5	(2)#5	#3	AT 10" O.C.

C.M.U. BOND BEAM SCHEDULE

(BOND BEAMS SHALL BE FILLED SOLID W/ 3000 PSI GROUT)

BOND BEAM	LOCATION
8" X 8" MIN. X CONT. W/ (1) #5 CONT.	AT TOP OF ALL C.M.U. PARAPETS. AT TOP OF 8" C.M.U. CURBS UNDER METAL STUDS.
8" X 16" MIN. X 2'-0" LONG MIN. W/ (2) #5 CONT.. (1)TOP, (1)BOTTOM	AT CANOPY TIE-RODS ATTACHMENT TO C.M.U. AT PARAPET RETURN HSS 4X4
8" X 40" MIN. X CONT. W/ (5) #5 CONT. (1) EA. COURSE	JOIST BRG. (FRONT & REAR WALLS), T/BOND BEAM = BRG. EL. + 24" ROOF DECK (SIDE WALLS), STEP AS REQUIRED

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LICENSE NO. 69377
STATE OF FLORIDA
PROFESSIONAL ENGINEER
04/07/26
FOR REVIEW

JUAN B. CANO, P.E.
FLORIDA LICENSE NO. 69377

PROJECT INFORMATION

PROJECT:

FLORIDA BLUE

PROJECT ADDRESS:

THE SHOPPES AT ST LUCIE WEST
PORT ST LUCIE, FL

PROJECT NO.: 250044

ACTIVE DESIGN PHASE

- ☐ FOR REVIEW
- ☐ FOR PERMITTING
- ☐ SCHEMATIC DESIGN
- ☐ DESIGN DEVELOPMENT
- ☐ CONSTRUCTION BIDDING
- ■ CONSTRUCTION DOCUMENTS
- ☐ AS-BUILT RECORD SET

REVISION INFORMATION

NO.	DATE	DESCRIPTION
2	02/27/26	PERMIT COMMENTS / TENANT COORD.
3	04/07/26	PERMIT COMMENTS

KEY PLAN

SHEET INFORMATION

SHEET ISSUED:	01/09/2026
DESIGNED BY:	JBC
DRAWN BY:	RED
REVIEWED BY:	JBC

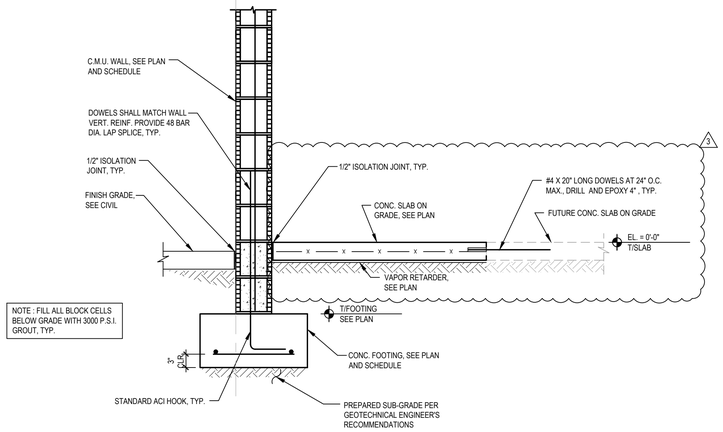
SHEET TITLE:

Roof Framing Plan

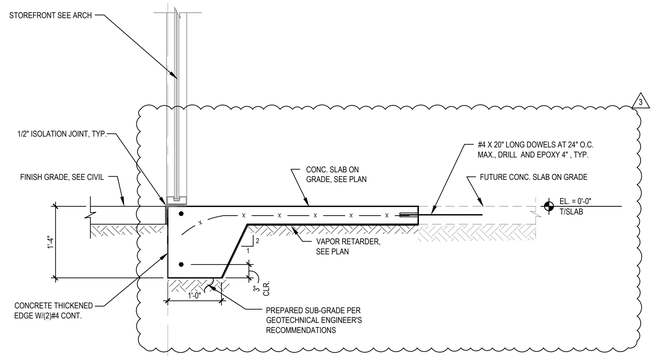
SHEET NO.:

S102

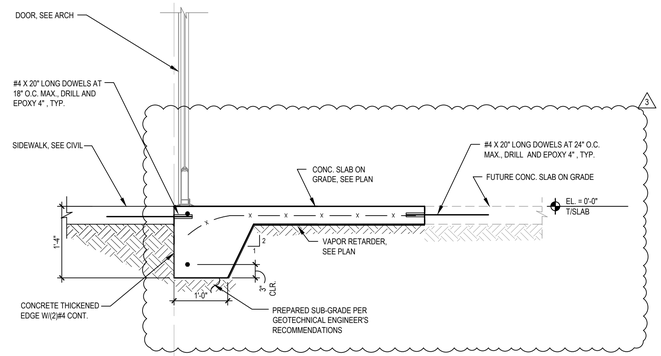
ARCH D



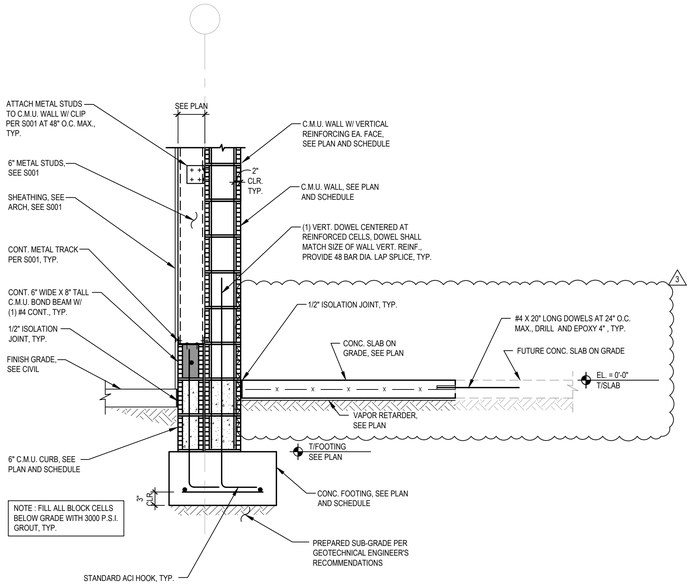
SECTION 1 — S201
3/4" = 1'-0"



SECTION 2 — S201
3/4" = 1'-0"



SECTION 3 — S201
3/4" = 1'-0"



SECTION 4 — S201
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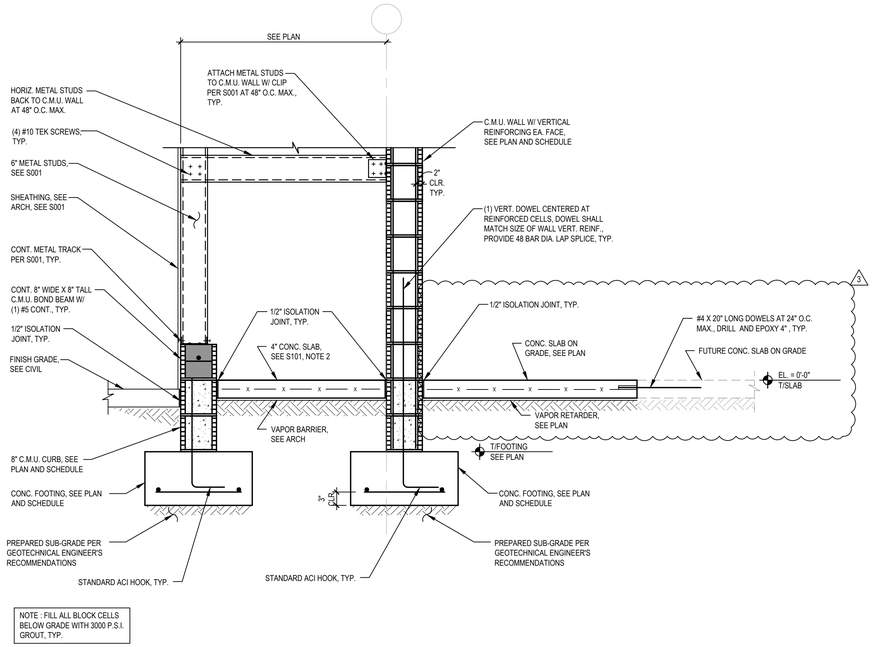

SECTION 5 — S201
3/4" = 1'-0"

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JUAN BAUTISTA CANO — 69377 — STATE OF FLORIDA — PROFESSIONAL ENGINEER
04/07/26 FOR REVIEW

JUAN B. CANO, P.E.
FLORIDA LICENSE NO. 69377

PROJECT INFORMATION

PROJECT:

FLORIDA BLUE

PROJECT ADDRESS:

THE SHOPPES AT ST LUCIE WEST
PORT ST LUCIE, FL

PROJECT NO.: 250044

ACTIVE DESIGN PHASE

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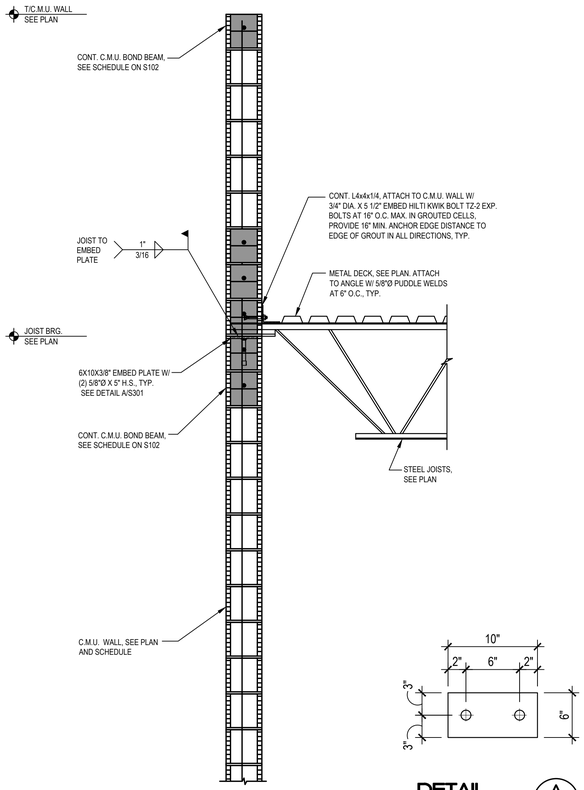
DRAWN BY:	RED
REVIEWED BY:	JBC

SHEET TITLE:

Sections

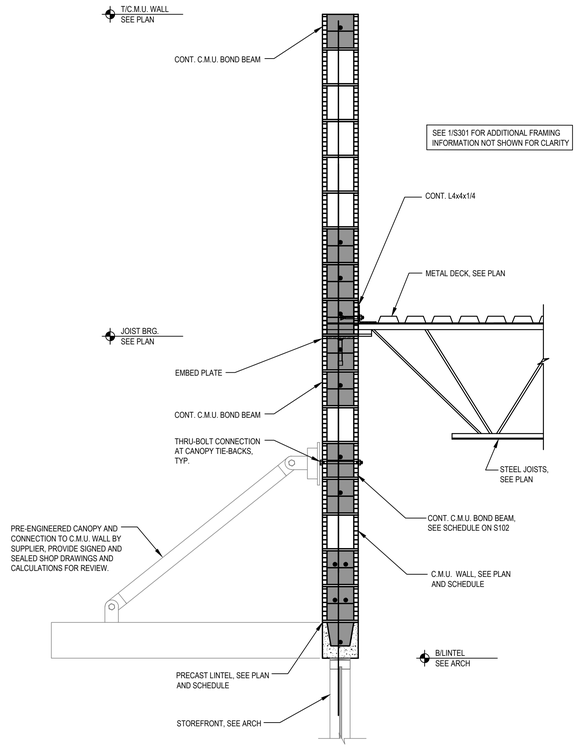
SHEET NO.:

S201



DETAIL A — S301
1"=1'-0"

SECTION 1 — S301
3/4" = 1'-0"



SECTION 2 — S301
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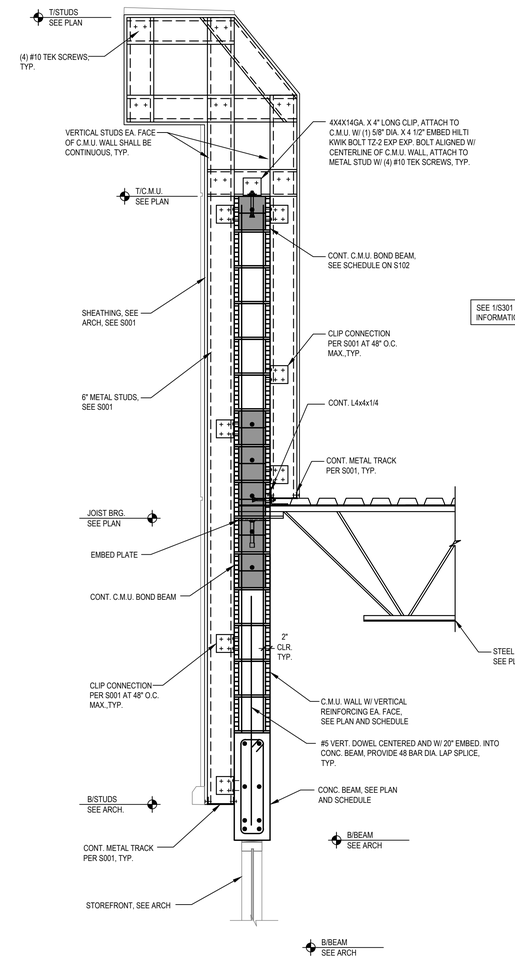

SECTION 3 — S301
3/4" = 1'-0"

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JUAN B. CANO, P.E.
FLORIDA LICENSE NO. 69377

PROJECT INFORMATION

PROJECT:

FLORIDA BLUE

PROJECT ADDRESS:

THE SHOPPES AT ST LUCIE WEST
PORT ST LUCIE, FL

PROJECT NO.: 250044

ACTIVE DESIGN PHASE

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- ☐ DESIGN DEVELOPMENT
- ☐ CONSTRUCTION BIDDING
- ■ CONSTRUCTION DOCUMENTS
- ☐ AS-BUILT RECORD SET

REVISION INFORMATION

NO.	DATE	DESCRIPTION

KEY PLAN

SHEET INFORMATION

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DESIGNED BY:	JBC
DRAWN BY:	RED
REVIEWED BY:	JBC

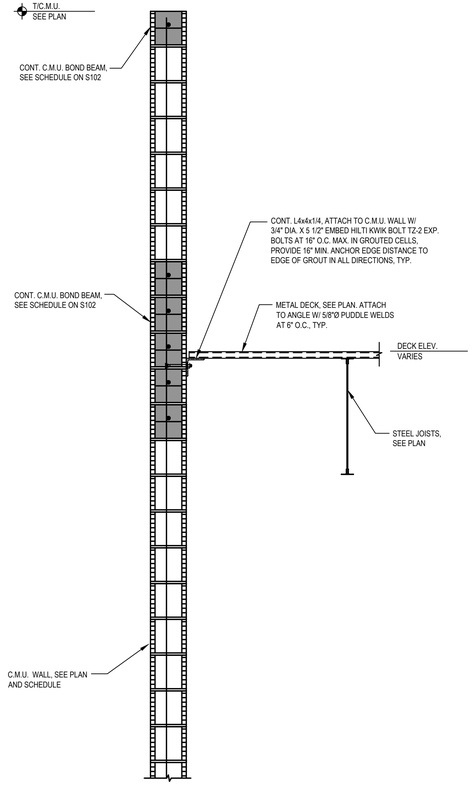
SHEET TITLE:

Sections

SHEET NO.:

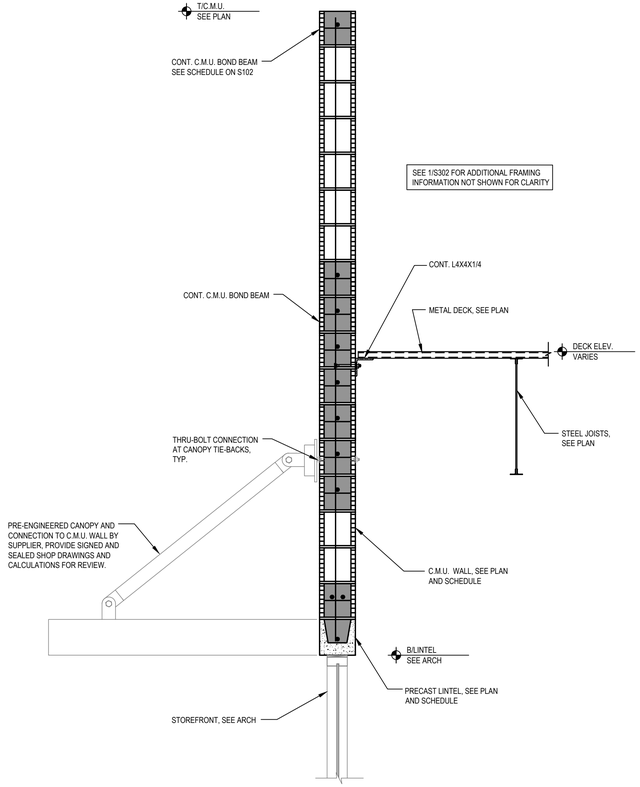
S301

ARCH D



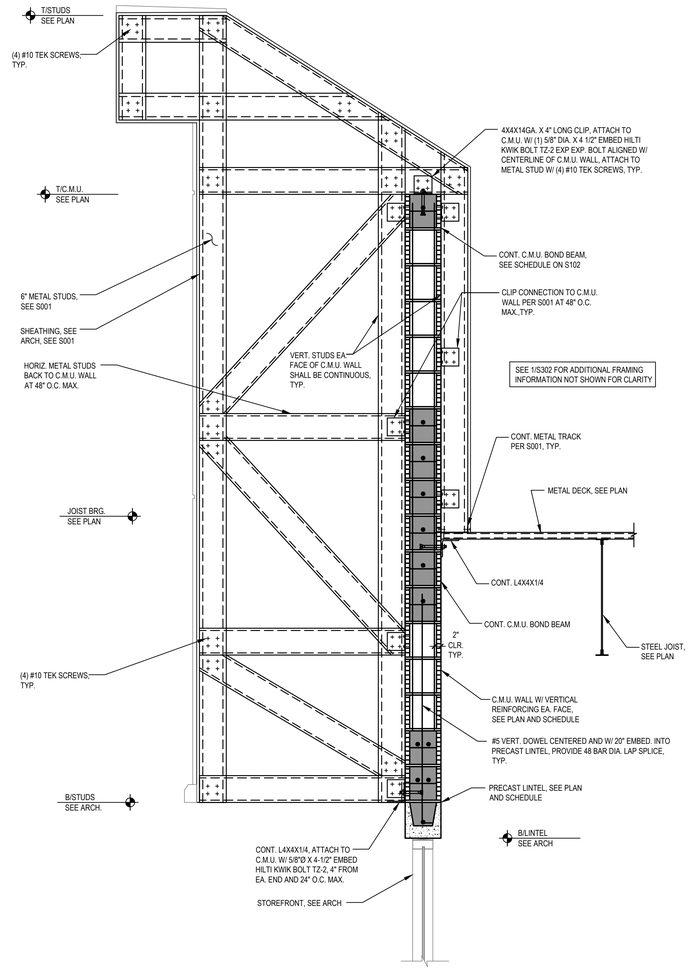
SECTION 1 / S302

3/4" = 1'-0"



SECTION 2 / S302

3/4" = 1'-0"



SECTION 3 / S302

3/4" = 1'-0"

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JUAN B. CANO, P.E.
FLORIDA LICENSE NO. 69377

PROJECT INFORMATION

PROJECT:

FLORIDA BLUE

PROJECT ADDRESS:

THE SHOPPES AT ST LUCIE WEST
PORT ST LUCIE, FL

PROJECT NO.: 250044

ACTIVE DESIGN PHASE

- ☐ FOR REVIEW
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KEY PLAN

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SHEET TITLE:	

Sections

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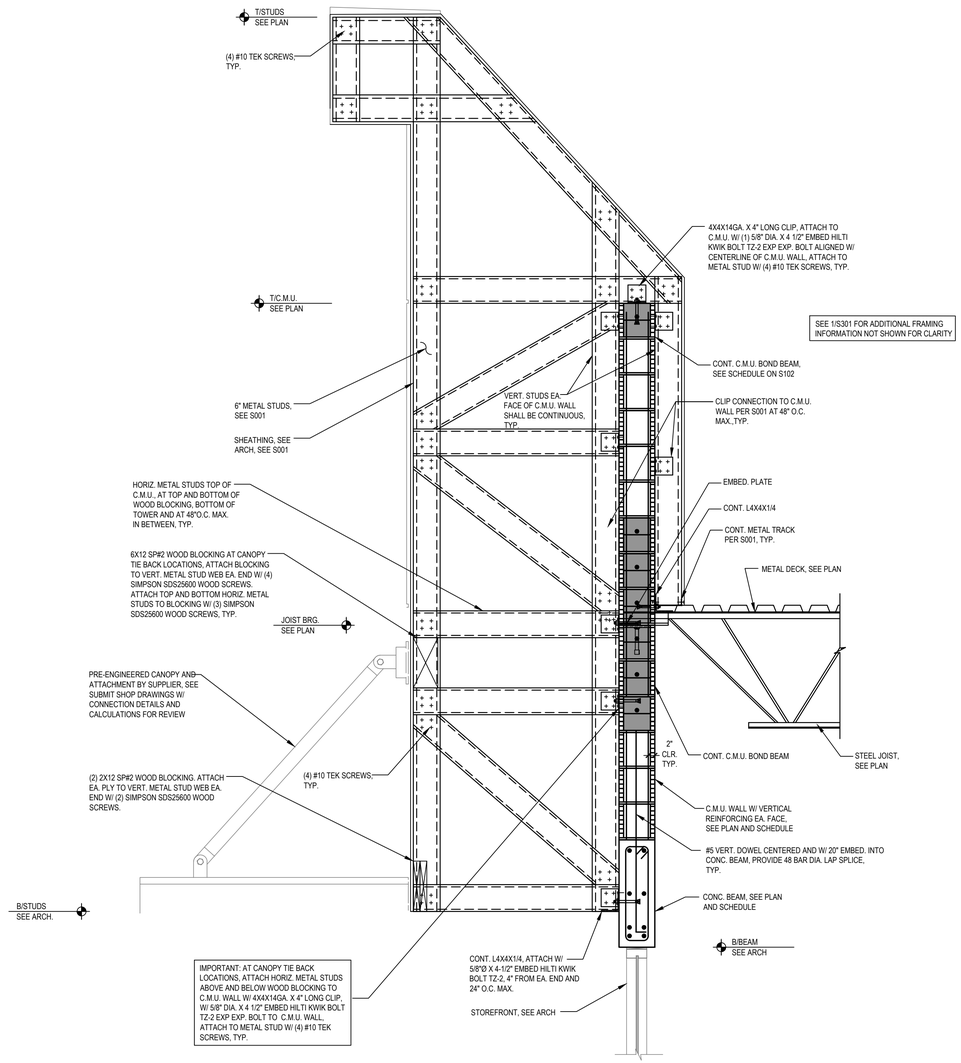
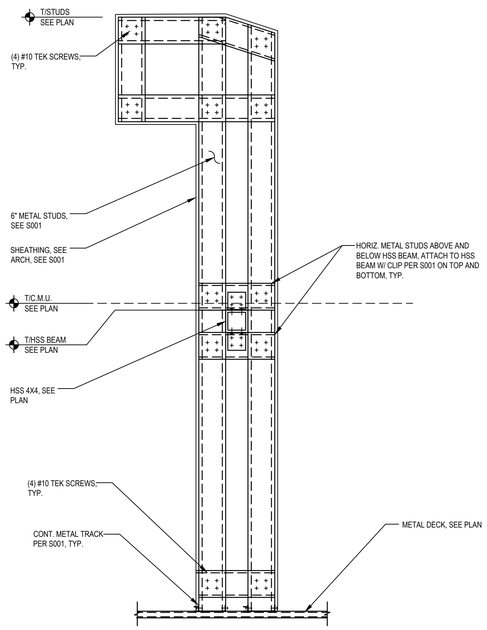
S302



SECTION 1
3/4" = 1'-0"
S303



SECTION 2
3/4" = 1'-0"
S303

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THE SHOPPES AT ST LUCIE W/EST
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PROJECT NO.: 250044

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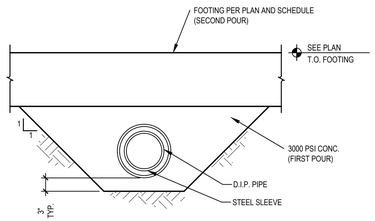
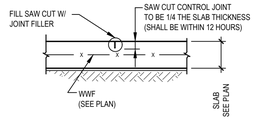
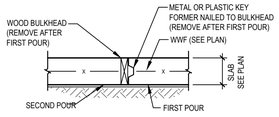


TYP. PIPE UNDER FTG.

DETAIL 1 — N.T.S. — S401

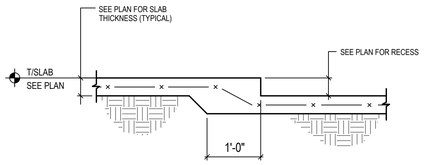


SAW CUT CONTROL JOINTS



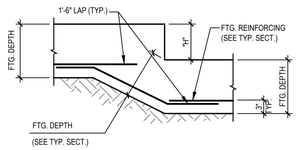
SLAB CONSTRUCTION JOINT

DETAIL 2 — N.T.S. — S401



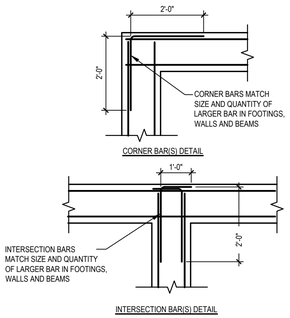
TYP. SLAB RECESS

DETAIL 3 — N.T.S. — S401



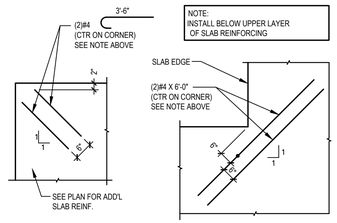
TYP. STEPPED FTG.

DETAIL 4 — N.T.S. — S401



TYPICAL BAR DETAILS

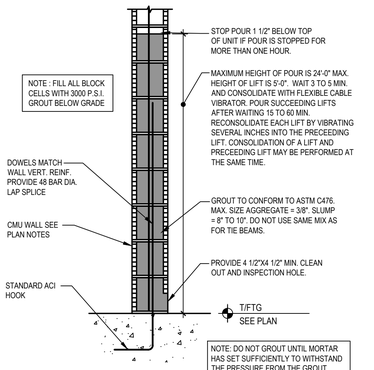
DETAIL 5 — N.T.S. — S401



TYP. SLAB CORNER REINF.

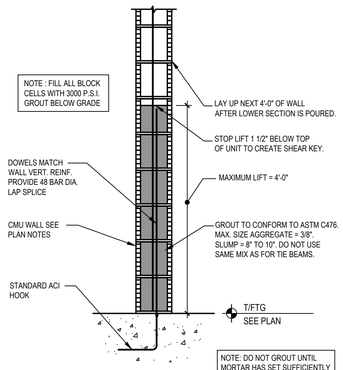
DETAIL 6 — N.T.S. — S401

NOTE: THE CONTRACTOR MAY ELECT TO PLACE GROUT IN 8'-0" LIFTS PROVIDING THAT THE SLUMP OF THE GROUT IS BETWEEN 10" AND 11" WHILE THE MINIMUM COMPRESSIVE STRENGTH OF THE GROUT IS MAINTAINED AT 3000 PSI AND THERE ARE NO INTERMEDIATE REINFORCED BOND BEAMS PLACED BETWEEN THE TOP AND BOTTOM OF THE POUR. INSPECTION HOLES SHALL BE AT 4'-0" O.C. CONTRACTOR SHALL BRACE WALL AS REQUIRED DURING GROUTING PROCEDURES.



NOTE: DO NOT GROUT UNTIL MORTAR HAS SET SUFFICIENTLY TO WITHSTAND THE PRESSURE FROM THE GROUT. WAIT NOT LESS THAN 24 HOURS.

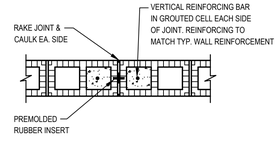
TYPICAL HIGH LIFT CMU GROUTING



NOTE: DO NOT GROUT UNTIL MORTAR HAS SET SUFFICIENTLY TO WITHSTAND THE PRESSURE FROM THE GROUT. WAIT NOT LESS THAN 24 HOURS.

TYPICAL LOW LIFT CMU GROUTING

DETAIL 7 — N.T.S. — S401



NOTE:
DISCONTINUE C.M.U. WALL HORIZ.
LADDER REINF. AT JOINT

PLAN VIEW

CMU WALL CONTROL JOINT DETAIL

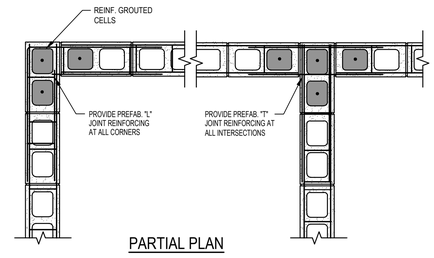
NOTES :

1.- CONTROL JOINT SPACING IS NOT TO EXCEED 25'-0" O.C. IN WALLS WITH MORE THAN 25'-0" OF UNITERUPTED MASONRY. REFER TO DWGS. FOR SPECIFIED LOCATIONS AS NOTED THUS ▲

2. SAW CUT C.M.U. BOND BEAMS, 1" DEEP TO CONTINUE WALL CONTROL JOINT TO TOP OF WALL. DO NOT CUT CONCRETE BEAMS AND PRECAST LINTELS.

3.- CONTINUE ALL C.M.U. BOND BEAM AND CONC. BEAM REINF. THROUGH THE JOINT.

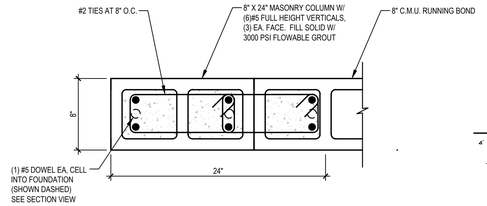
DETAIL 8 — N.T.S. — S401



PARTIAL PLAN

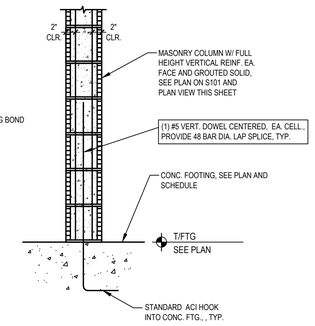
TYPICAL CMU WALL DETAILS

DETAIL 9 — N.T.S. — S401



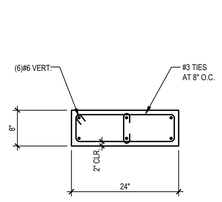
MC-1 (PLAN VIEW)

(TOP OF MC-1 = TOP OF C.M.U. WALL)



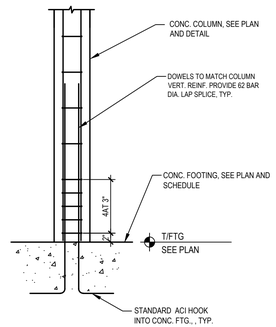
MC-1 (SECTION VIEW)

DETAIL 10 — N.T.S. — S401



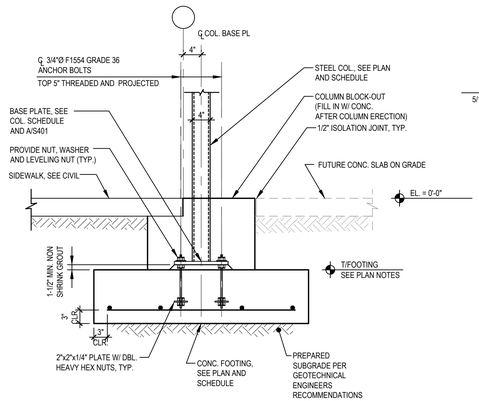
CC-1 (PLAN VIEW)

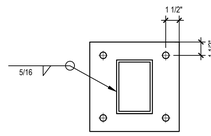
(TOP OF CC-1 = TOP OF C.M.U. WALL)



CC-1 (SECTION VIEW)

DETAIL 11 — N.T.S. — S401





BASE PLATE DETAIL

DETAIL A — N.T.S. — S401

DETAIL 12 — N.T.S. — S401

SMA
ARCHITECTURE and INTERIORS
SMA ARCHITECTURE + INTERIORS
100 COLONIAL CENTER PKWY
SUITE 250
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Structural Design
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Orlando, Florida 32810
352.514.4938

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JUAN BAUTISTA CANO LICENSE No. 69377 STATE OF FLORIDA PROFESSIONAL ENGINEER

JUAN B. CANO, P.E.
FLORIDA LICENSE NO. 69377

PROJECT INFORMATION

PROJECT:

FLORIDA BLUE

PROJECT ADDRESS:

THE SHOPPES AT ST LUCIE WEST
PORT ST LUCIE, FL

PROJECT NO.: 250044

ACTIVE DESIGN PHASE

- ☐ FOR REVIEW
- ☐ FOR PERMITTING
- ☐ SCHEMATIC DESIGN
- ☐ DESIGN DEVELOPMENT
- ☐ CONSTRUCTION BIDDING
- ■ CONSTRUCTION DOCUMENTS
- ☐ AS-BUILT RECORD SET

REVISION INFORMATION

NO.	DATE	DESCRIPTION

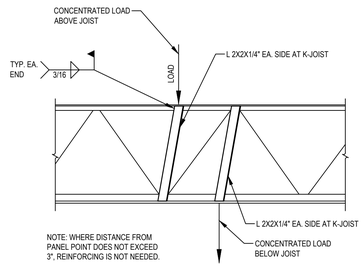
KEY PLAN

SHEET INFORMATION

SHEET ISSUED:	01/09/2026
DESIGNED BY:	JBC
DRAWN BY:	RED
REVIEWED BY:	JBC

SHEET TITLE: Details

SHEET NO.: S401



CONCENTRATED LOAD STRUT SUPPORT DETAIL

FOR LOADS AT OTHER THAN PANEL POINT LOCATIONS

DETAIL 1 / S402

N.T.S.

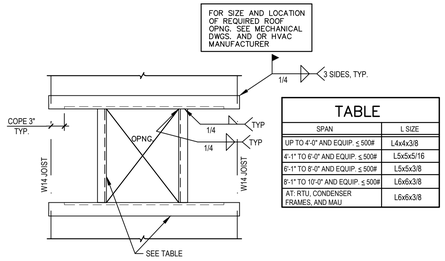
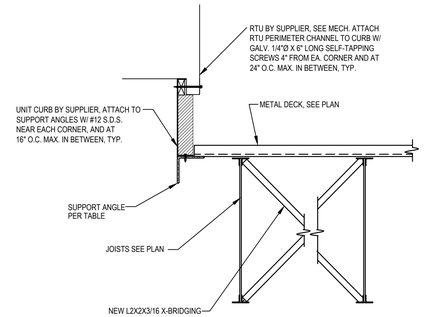


TABLE	
SPAN	L SIZE
UP TO 4'-0" AND EQUIP. ≤ 500#	L4x4x3/8
4'-1" TO 6'-0" AND EQUIP. ≤ 500#	L5x5x5/16
6'-1" TO 8'-0" AND EQUIP. ≤ 500#	L5x5x3/8
8'-1" TO 10'-0" AND EQUIP. ≤ 500#	L6x6x3/8
AT: RTU, CONDENSER FRAMES, AND MAU	L6x6x3/8

TYP. ROOF OPENING HVAC EQUIP. SUPPORT FMG.

DETAIL 2 / S402

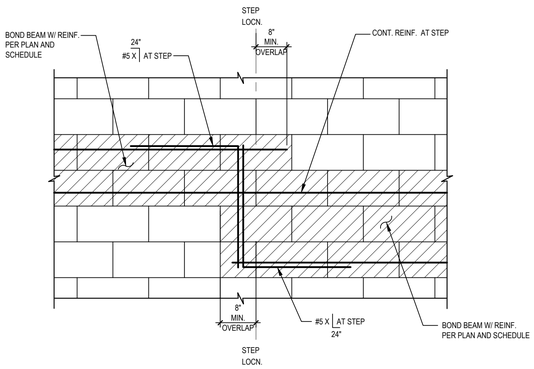
N.T.S.



TYP. RTU HOLDOWN

DETAIL 3 / S402

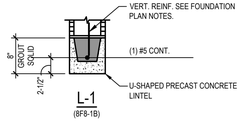
N.T.S.



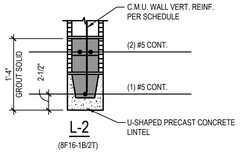
BOND BEAM STEP DETAIL

DETAIL 4 / S402

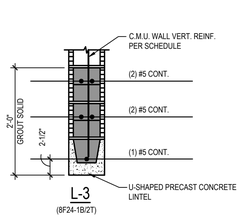
3/4" = 1'-0"



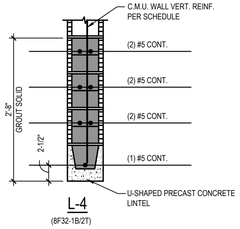
L-1 (8F8-1B)



L-2 (8F16-1B/2T)



L-3 (8F24-1B/2T)



L-4 (8F32-1B/2T)

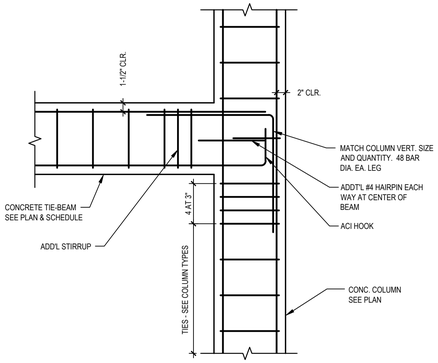
8" PRECAST SCHEDULE

DETAIL 5 / S402

N.T.S.

NOTES:

1. 8" PRECAST LINTELS BY CASTCRETE CORPORATION OR APPROVED EQUAL.
2. SHORE PRECAST LINTEL PER MANUFACTURER RECOMMENDATIONS.
3. SEE THE ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATION OF ALL OPENINGS.
4. PROVIDE 4" MINIMUM BEARING EACH END.
5. EXTEND REINF. 16" MIN. BEYOND OPENING EA. END, TYP.

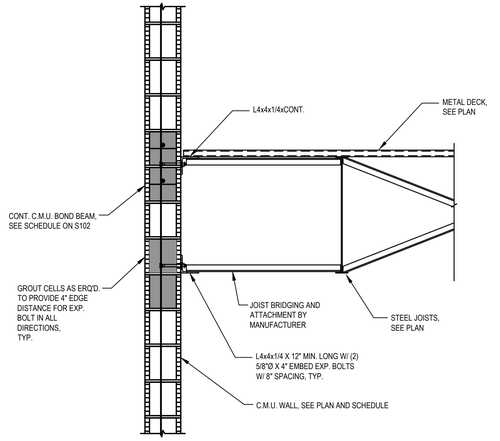


TYPICAL CONCRETE BEAM TO CONCRETE COLUMN CONNECTION

(NOTE: C.M.U. ABOVE CONCRETE BEAM NOT SHOWN FOR CLARITY)

DETAIL 6

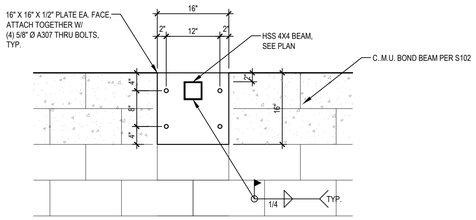
3/4" = 1'-0"



JOIST BRIDGING TO C.M.U. WALL

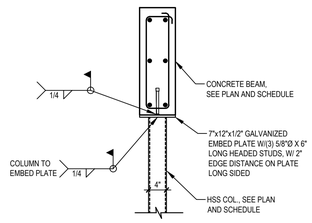
DETAIL 7

3/4" = 1'-0"



DETAIL 8

3/4" = 1'-0"



NOTE: SURROUNDING FRAMING NOT SHOWN FOR CLARITY

DETAIL 9

3/4" = 1'-0"

SMA
ARCHITECTURE and INTERIORS

SMA ARCHITECTURE + INTERIORS
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