ABBREVIATIONS AND ANCHOR BOLT

BM

BO

BOT

CLR

COL

CONN

CONT

DET

DIA

DWG

DWLS

ELEV

EQ

EXT

FDN

FIN

FLR FRMG

FS

FTG

HORIZ

INFO

INTER.

MAX

MECH

MISC

NS

PL

PLCS

PRCJ

REINF

SECT

STA

STD

STL

STIFF

SYMM

THD

T&B

TOS

TOW

TYP

UNO

VERT

VIF

TO

RJ

EΑ

CONST

BRCG

BOTTOM OF

BOTH SIDES

CEILING BEAM

CAST IN PLACE

CONTROL JOINT

COMPLETE PENETRATION

CONCRETE MASONRY UNIT

CENTERLINE

CLEAR

COLUMN

CONNECTION

CONTINUOUS

CONSTRUCTION

COORD COORDINATE

DETAIL

DIAMETER

DRAWING

DOWELS

ELEVATION

ELEVATION

EACH WAY

EXISTING

EXPANSION

EXTERNAL

FINISH

FLOOR

FRAMING

FAR SIDE

FOOTING

HORIZONTAL

HIGH POINT

INFORMATION

INTERMEDIATE

MACHINE BOLT

MISCELLANEOUS

LOW POINT

MECHANICAL

NEAR FACE

NEAR SIDE

ON CENTER

POUR JOINT

PROJECTION

ROOF JOIST

REINFORCING

ROOF RAFTER

PLATE

PLATE

SCHED SCHEDULE

PLACES

SECTION

SIMILAR

SPECS SPECIFICATIONS

STATION

STEEL

THREAD

TOP OF

TO FTG TOP OF FOOTING

TYPICAL

VERTICAL

TOP OF STEEL

VERIFY IN FIELD

TOP OF WALL

STANDARD

STIFFENER

SYMMETRY, SYMETRICAL

UNLESS NOTED OTHERWISE

TOP AND BOTTOM

OPNG OPENING...

MAXIMUM

MINIMUM

HIGH STRENGTH BOLTS

INVERT ELEVATION

GAUGE

JOINT

LONGIT LONGITUDINAL

FOUNDATION FAR FACE

EACH

EQUAL

CONC CONCRETE

BOTTOM

BRACING

STRUCTURAL NOTES

STRUCTURAL DESIGN CRITERIA

- D-1 CODES: EPCOT BUILDING CODE, 2018 EDITION, WIND DESIGN BASED ON ASCE 7-16.
- D-2 CODES: FLORIDA FIRE PREVENTION CODE, 7TH EDITION
- D-3 DESIGN LIVE LOADS:

GCpi = +/- 0.18

ROOF 20 PSF

D-4 WIND DESIGN VELOCITY 140 MPH ULTIMATE
RISK CATEGORY II
EXPOSURE C

D-5 PRESUMED ALLOWABLE SOIL BEARING PRESSURES: 2000 PSF

GENERAL NOTES

- G-1 THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO FABRICATION AND START OF CONSTRUCTION. REPORT ANY DISCREPANCIES TO ENGINEER PRIOR TO PROCEEDING WITH WORK.
- G-2 NO STRUCTURAL MEMBER SHALL BE CUT, NOTCHED OR OTHERWISE REDUCED IN SIZE OR STRENGTH WITHOUT THE WRITTEN PERMISSION OF THE ENGINEER OF RECORD.
- G-3 ALL DETAILS AND SECTIONS SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL BE CONSTRUED TO APPLY TO ANY SIMILAR SITUATION ELSEWHERE ON THE PROJECT UNLESS A SEPARATE DETAIL IS SHOWN.
- G-4 THE INTENTION OF THE PLANS AND SPECIFICATIONS IS TO PROVIDE ALL NECESSARY DETAILS TO CONSTRUCT A COMPLETE STRUCTURE. WHEN SPECIFIC INFORMATION IS MISSING OR IN CONFLICT, THE CONTRACTOR SHALL USE A SIMILAR DETAIL AND/OR THE MORE COSTLY ITEM OF CONFLICT. IF A DOUBT STILL EXISTS, CONTACT THE ARCHITECT/ENGINEER.
- G-5 REPRODUCTIONS OF THE CONTRACT DOCUMENTS SHALL NOT BE USED IN THE PREPARATION OF SHOP OR ERECTION DRAWINGS.

STRUCTURAL STEEL

- S-1 ALL STRUCTURAL STEEL WIDE FLANGE SHAPES SHALL COMPLY WITH ASTM A-992 WITH A MINIMUM YIELD STRENGTH OF 50 KSI. ALL MISCELLANEOUS STEEL SHALL CONFORM TO ASTM A-36 WITH A MINIMUM YIELD STRENGTH OF 36 KSI UNLESS NOTED OTHERWISE.
- S-2 FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE AISC "STEEL CONSTRUCTION MANUAL", FOURTEENTH EDITION, AND AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS", 2010 EDITION.
- S-3 ALL BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A-325. ALL BOLTS SHALL BE 3/4" DIAMETER UNLESS NOTED OTHERWISE. ALL BOLTS ARE IN BEARING TYPE CONNECTIONS UNLESS OTHERWISE INDICATED. THREADS OF BOLTS SHALL BE EXCLUDED FROM THE SHEAR PLANES.
- S-4 NUTS FOR STRUCTURAL BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A563. WASHERS FOR STRUCTURAL BOLTS SHALL MEET THE REQUIREMENTS OF ASTM F436.
- S-5 BOLTS, NUTS AND WASHERS SHALL BE PLAIN FINISH UNLESS NOTED OTHERWISE.
- S-6 BOLTS SHALL BE TIGHTENED TO A "TIGHT" CONDITION PER THE REQUIREMENTS OF AISC UNLESS NOTED
- S-7 BOLTS SHALL BE FULLY PRETENSIONED (WHERE INDICATED) BY USE OF SPECIAL BOLTS WITH TWIST OFF TIP (LEJEUNE BOLTS), BY TURN OF THE NUT METHOD OR BY USE OF A LOAD INDICATING DEVICE / WASHER.
- S-8 ALL SHOP AND FIELD WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS IN ACCORDANCE WITH THE AWS CODE "STRUCTURAL WELDING CODE STEEL" (AWS D1.1), LATEST EDITION.
- S-9 USE E70XX ELECTRODES FOR ALL WELDING UNLESS NOTED OTHERWISE.
- S-10 ALL SHOP AND FIELD WELDS SHALL BE INSPECTED BY AN INDEPENDENT THIRD PARTY ENGINEER AND A REPORT OF INSPECTION SHALL BE SUBMITTED FOR REVIEW. MAGNETIC PARTICLE OR DYE PENETRANT SHALL BE USED TO VERIFY WELDS.
- S-11 ALL SHOP AND FIELD BOLTED CONNECTIONS SHALL BE INSPECTED BY AN INDEPENDENT THIRD PARTY ENGINEER AND A REPORT OF INSPECTION SHALL BE SUBMITTED FOR REVIEW. A MINIMUM OF 10% OF ALL BOLTS SHALL BE TESTED WITH TORQUE WRENCH.
- S-12 ALL BOLTS CAST INTO CONCRETE SHALL BE ASTM A-36 OR ASTM A-307. ANCHOR BOLTS SHALL BE SET USING A TEMPLATE
- S-13 PIPE SECTIONS SHALL CONFORM TO ASTM A-53, TYPE E OR S, GRADE B, WITH A MINIMUM YIELD STRENGTH OF 36 KSI
- S-14 TUBE SECTIONS SHALL CONFORM TO ASTM A-500, GRADE B, WITH A MINIMUM YIELD STRENGTH OF 46 KSI.
- S-15 STEEL STUDS SHALL BE WELDED WITH A STUD WELDING GUN OR SIMILAR METHOD. MANUAL ARC WELDING OF STUDS IS NOT PERMITTED.
- S-16 GROUT FOR BASE PLATES SHALL BE NON-METALLIC, NON-SHRINK GROUT EQUAL TO FIVE STAR GROUT, AS MANUFACTURED BY U.S. GROUT, INC.
- S-17 HOLES DRILLED INTO CONCRETE FOR SETTING ANCHOR BOLTS IN EPOXY SHALL BE BLOWN OUT WITH COMPRESSED AIR, THEN BRUSHED OUT WITH A WIRE BRUSH OF THE APPROPRIATE DIAMETER, AND THEN BLOWN OUT AGAIN WITH COMPRESSED AIR PRIOR TO PLACEMENT OF THE EPOXY.
- S-18 STAINLESS STEEL FOR SHAPES, PLATES AND MISCELLANEOUS PARTS SHALL BE TYPE 304 0R 316 AS NOTED.
- S-19 CONSUMABLES FOR WELDING OF STAINLESS STEEL TO STAINLESS STEEL SHALL BE TYPE ER308L FOR TYPE 304 TO TYPE 304, TYPE ER316L FOR TYPE 316 TO TYPE 316 AND TYPE ER316L FOR TYPE 304 TO TYPE 316.
- S-20 CONSUMABLES FOR WELDING TYPE 304 OR 316 STAINLESS STEEL TO CARBON STEEL SHALL BE TYPE ER309L.
- S-21 STAINLESS STEEL BOLTS, NUTS AND WASHERS SHALL BE TYPE 304 OR 316 STAINLESS STEEL, AS NOTED.
- S-22 STAINLESS STEEL BOLTS SHALL MEET THE REQUIREMENTS OF ASTM F593, FABRICATED FROM TYPE 304 OR 316 AS NOTED. BOLTS SHALL BE ANNEALED.
- S-23 SUBMIT STRUCTURAL STEEL SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION OF STRUCTURAL STEEL. SHOP DRAWINGS TO SHOW ALL PIECE MARKS, CONNECTIONS AND ERECTION DRAWINGS. STRUCTURAL STEEL SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE OF FLORIDA.

REINFORCED CONCRETE

- RC-1 ALL CONCRETE DESIGN AND PLACEMENT SHALL BE IN STRICT ACCORDANCE WITH ACI "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (ACI 318-08) AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301-00).
- RC-2 STRUCTURAL CONCRETE SHALL CONFORM TO ACI 301 SPECIFICATIONS AND SHALL DEVELOP A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 4000 PSI FOR ALL ELEMENTS EXCEPT SLAB ON GRADE AND FOUNDATIONS WHICH SHALL BE 3000 PSI. MAXIMUM WATER CEMENT RATIO SHALL BE 0.50.
- RC-3 CEMENT SHALL BE ASTM C150, TYPE I UNLESS NOTED OTHERWISE. MINIMUM CEMENT CONTENT SHALL BE 470 POUNDS. UP TO 25% OF CEMENTITIOUS MATERIAL MAY BE FLYASH.
- RC-4 USE REGULAR WEIGHT CONCRETE FOR ALL STRUCTURAL MEMBERS, MINIMUM 140 PCF.
- RC-5 AGGREGATES SHALL BE CLEAN, WELL GRADED AND SIZED AS FOLLOWS AND CONCRETE MIXES USING THEM SHALL HAVE THE FOLLOWING SLUMPS:

LOCATION	SLUMP	LARGE	AGGREGATE SIZE
FOUNDATIONS	3"+/- 1"		#57 OR #67
COLUMNS/BEAMS/WALLS UP TO 10" MIN. DIM.	8" MAX.*		#89 OR 3/8"
COLUMNS/BEAMS/WALLS OVER 10" MIN DIM.	8" MAX.*		#57 OR #67
SLABS ON GRADE STRUCTURAL SLABS	8" MAX.*		#57 OR #67
WORKING SLABS	4"+/- 1"		#57 OR #67
TOPPING SLABS	8" MAX.*		#89 OR 3/8"
STAIRS/SLOPING SURFACES	3"+/- 1"		#57 OR #67
MISCELLANEOUS (EQUIPMENT PADS ETC.)	AS REQ'D		AS REQ'D

- *INDICATES SUPERPLASTICIZED CONCRETE
- RC-6 USE OF "JITTERBUGS" TO CONSOLIDATE CONCRETE WILL NOT BE PERMITTED.
- RC-7 CONCRETE SHALL BE PLACED IN FORMS AS NEAR AS POSSIBLE TO ITS FINAL LOCATION. CONCRETE SHALL BE CONSOLIDATED BY MECHANICAL VIBRATORS. VIBRATORS SHALL NOT BE USED TO MOVE CONCRETE.
- RC-8 SCREED CONCRETE SLABS TO REQUIRED PLANE, FLOAT WITH BULL FLOAT AND FINISH WITH STEEL TROWEL TO SMOOTH CONCRETE FINISH UNLESS NOTED OTHERWISE. WALKWAYS AND SIMILAR SURFACES SHALL HAVE BROOM FINISH
- RC-9 APPLY LIQUID MEMBRANE FORMING CURING COMPOUND EQUAL TO ASTM C309 COMPATIBLE WITH SUBSEQUENT FINISHES TO BE APPLIED TO CONCRETE.
- RC-10 IF CONCRETE IS TO BE PUMPED, THE MINIMUM PUMP SIZE SHALL BE 4" FOR #57 AND #67 AGGREGATES, AND SHALL BE 2" FOR #89 AND 3/8" AGGREGATES. SLUMP IS TO BE MEASURED AT THE DISCHARGE POINT OF THE CONCRETE.
- RC-11 ALL REINFORCING STEEL SHALL BE DETAILED, FABRICATED AND INSTALLED IN ACCORDANCE WITH ACI-318 AND THE "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE ACI-315, LATEST EDITION.
- RC-12 REINFORCING STEEL SHALL BE NEW BILLET DEFORMED BARS, FREE OF SCALE, RUST, OIL OR OTHER SUBSTANCES INHIBITING BOND CONFORMING TO ASTM A-615, GRADE 60, WITH MINIMUM YIELD STRENGTH OF 60,000
- RC-13 LAP BOTTOM STEEL 12" OVER SUPPORTS FOR BEAMS AND SIMILAR FRAMING MEMBERS UNLESS OTHERWISE NOTED. FOR ALL OTHER REINFORCING (HORIZONTAL OR VERTICAL) USE FOLLOWING LAP SPLICE LENGTHS:

NON-TOP BAR

OR VERTICAL

LAP SPLICE REQUIREMENTS:

BAR SIZE AND LAP LENGTH

TOP BAR*

f'c=3000 PSI

		OIL VEILIONE
#3	28"	22"
#4	37"	29"
#5	47"	36"
#6	56"	43"
#7	81"	63"
#8	93"	72"
#9	105"	81"
#10	118"	91"
#11	131"	101
f'c=4000 PSI		
	TOP BAR*	NON-TOP BAR OR VERTICAL
#3	24"	19"
#4	32"	25"
#5	40"	31"
#6	48"	37"
#7	70"	54"
#8	80"	62"
#9	91"	70"
#10	102"	79"
#11	113"	87"
f'c=5000 PSI		
	TOP BAR*	NON-TOP BAR OR VERTICAL
#3	22"	17"
#4	29"	22"
#5	36"	28"
#6	43"	33"
#7	63"	49"
#8	72"	55"
#9	81"	63"
#10	91"	70"
#11	101"	78"

*SPLICES IN TOP BARS SHALL BE MADE AT MID-SPAN OF MEMBER UNLESS NOTED OTHERWISE. A TOP BAR IS DEFINED AS A BAR WITH MORE THAN 12" OF CONCRETE BELOW THE BAR. HORIZONTAL REINFORCING IN A WALL SHALL BE CONSIDERED AS TOP BARS.

RC-14 TERMINATE ALL DISCONTINUOUS TOP BARS WITH STANDARD 90-DEGREE ACI HOOK (PLACED VERTICALLY) UNLESS NOTED OTHERWISE.

RC-15 PROVIDE THE FOLLOWING MINIMUM CONCRETE COVER OVER REINFORCING BARS:

3" CLEAR FOOTINGS (BOTTOM AND UNFORMED SIDES) FOOTINGS (TOP) 2" CLEAR BEAMS AND COLUMNS (TO TIES) 1 1/2" CLEAR SLABS (TOP AND BOTTOM STEEL) 3/4" CLEAR INTERIOR 1 1/2" CLEAR **EXTERIOR** CONCRETE WALLS 1" CLEAR INTERIOR **EXTERIOR** 2" CLEAR SLABS ON GRADE TOP 1" CLEAR BOTTOM 1 1/2" CLEAR

RC-16 SHOP DRAWINGS FOR REINFORCING STEEL FABRICATION AND PLACEMENT SHALL BE APPROVED PRIOR TO REINFORCING STEEL FABRICATION OR PLACEMENT.

RC-17 SLEEVE ALL PENETRATIONS THROUGH BEAMS AND SLABS INDIVIDUALLY. CORE DRILLING WILL NOT BE PERMITTED. SUBMIT LOCATION AND SIZE OF SLEEVES TO ENGINEER FOR REVIEW PRIOR TO PLACING SLEEVES.

RC-18 HOLES DRILLED INTO CONCRETE FOR SETTING ANCHOR BOLTS OR REINFORCING BARS IN EPOXY SHALL BE BLOWN OUT WITH COMPRESSED AIR, THEN BRUSHED OUT WITH A WIRE BRUSH OF THE APPROPRIATE DIAMETER, AND THEN BLOWN OUT AGAIN WITH COMPRESSED AIR PRIOR TO PLACEMENT OF THE EPOXY.

- RC-19 NO REINFORCING BARS SHALL BE CUT TO ACCOMMODATE THE INSTALLATION OF ANCHORS, EMBEDS OR OTHER ITEMS.
- RC-20 USE THE STRUCTURAL DRAWINGS (INCLUDING REVISIONS AND ADDENDA) IN CONJUNCTION WITH THE APPROVED SHOP DRAWINGS FOR PLACEMENT OF REINFORCING STEEL.
- RC-21 ALL EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED 3/4" UNLESS NOTED OTHERWISE.
- RC-22 ALL EMBED ITEMS SHALL BE SECURELY TIED IN PLACE PRIOR TO CONCRETE PLACEMENT. "WET STICKING" OF EMBEDS OR BARS IS NOT PERMITTED.
- RC-23 AT CHANGES IN DIRECTION OF CONCRETE BEAMS PROVIDE CORNER BARS OF THE SAME SIZE AND QUANTITY AS ALL LONGITUDINAL BARS.

STEEL JOISTS

SJ-1 ALL DESIGN, FABRICATION, AND ERECTION OF STEEL JOISTS AND BRIDGING SHALL BE IN STRICT ACCORDANCE WITH THE CURRENT SPECIFICATIONS OF THE STEEL JOIST INSTITUTE. SUBMIT SIGNED AND SEALED CALCULATIONS FOR JOISTS PREPARED BY AN ENGINEER LICENSED IN THE STATE OF FLORIDA.

SJ-2 DESIGN STEEL ROOF JOISTS FOR UPLIFT FORCES PER WIND UPLIFT DIAGRAMS OF THE ROOF AS SHOWN ON THE DRAWINGS UNLESS NOTED OTHERWISE. DEAD LOAD OF ROOF SHALL BE CONSIDERED AS 7.5 POUNDS PER SQUARE FOOT AND MAY BE SUBTRACTED FROM THE WIND UPLIFT VALUES FOR DESIGN PURPOSES.

SJ-3 BOTTOM CHORD OF ALL JOISTS SHALL BE CONSTRUCTED OF DOUBLE ANGLES.

SJ-4 STEEL JOISTS SHALL BE WELDED TO SUPPORTING STRUCTURAL STEEL WITH 2 – 3/16" FILLET WELDS, EACH 2 INCHES LONG ALONG EDGES OF JOIST SEAT OR CONNECTED WITH 2 – 1/2" ASTM A 307 BOLTS.

SJ-5 REFER TO SPECIFIC DETAILS FOR THE LENGTH OF BEARING ENDS AND BOTTOM CHORD EXTENSIONS. COORDINATE BOTTOM CHORD EXTENSION REQUIREMENTS WITH ARCHITECTURAL DRAWINGS.

SJ-6 PROVIDE HORIZONTAL TOP AND BOTTOM CHORD BRIDGING, AND "X" BRIDGING PER STEEL JOIST INSTITUTE SPECIFICATIONS AND AS SHOWN ON PLANS.

SJ-7 PROVIDE UPLIFT BRIDGING AS REQUIRED TO MEET STEEL JOIST INSTITUTE SPECIFICATIONS AND AS SHOWN ON PLANS.

SJ-8 ANCHOR ENDS OF ALL BRIDGING. WHERE BRIDGING TERMINATES AT CONCRETE OR MASONRY, PROVIDE A 3x3x1/4 ANGLE x 8" LONG WITH 2 - 1/2" DIAMETER x 6" WEDGE ANCHORS. WHERE BRIDGING TERMINATES AT STEEL FRAMING, WELD BRIDGING TO MEMBER.

SJ-9 TOP CHORD MEMBERS SHALL BE DESIGNED FOR A SUPERIMPOSED POINT LOAD OF 200 POUNDS LOCATED AT ANY POINT BETWEEN ANY TWO TOP CHORD PANEL POINTS.

SJ-10 BOTTOM CHORD MEMBERS SHALL BE DESIGNED FOR A SUPERIMPOSED POINT LOAD OF 200 POUNDS LOCATED AT ANY POINT BETWEEN ANY TWO BOTTOM CHORD PANEL POINTS.

SJ-11 IF A POINT LOAD GREATER THAN 200 POUNDS IS LOCATED BETWEEN ANY TWO TOP CHORD PANEL POINTS PROVIDE A PAIR OF BACK TO BACK L2-1/2 x 2-1/2 x 1/4" ANGLES FROM THE LOCATION OF THE LOAD TO THE NEAREST BOTTOM CHORD PANEL POINT. WELD WITH 3/16 INCH FILLET WELDS AT ALL POINTS OF CONTACT.

SJ-12 IF A POINT LOAD GREATER THAN 200 POUNDS IS LOCATED BETWEEN ANY TWO BOTTOM CHORD PANEL POINTS PROVIDE A PAIR OF BACK TO BACK L2-1/2 \times 2-1/2 \times 1/4" ANGLES FROM THE LOCATION OF THE LOAD TO THE NEAREST TOP CHORD PANEL POINT. WELD WITH 3/16 INCH FILLET WELDS AT ALL POINTS OF CONTACT.

SJ-13. STEEL JOISTS AND UNDERSIDE OF METAL DECK TO BE PAINTED PER OWNER COLOR SELECTION AT KENNEL WINGS ONLY.

METAL DECK

MD-1 ROOF DECK SUPPORTING INSULATION AND ROOFING MEMBRANE SHALL BE AS FOLLOWS:

1 1/2", 22 GAGE, WIDE RIB, TYPE 1.5B22 GALVANIZED METAL ROOF DECK AS MANUFACTURED BY VULCRAFT OR EQUAL (SEE PLAN FOR LOCATION). ERECT DECK AS THREE SPAN CONTINUOUS. USE 36/7 WELDING PATTERN WITH 5/8" PLUG WELDS AT SUPPORTS. AT SUPPORTS PARALLEL TO DECK PROVIDE 5/8" PLUG WELDS AT 6" O.C. PROVIDE #10 TEK SCREWS AT 12" O.C. AT SIDE LAPS.



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> L COMMERCIAL BUILDING 1

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Description

Revisions

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ROBERT G. JOHNSON

Date

Project No: 23-089

Drawn By: H.P.

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FLORIDA, PE #61899

Sheet Title
STRUCTURAL NOTES

1S-100

MASONRY

- M-1 MASONRY CONSTRUCTION SHALL CONFORM TO THE "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" (ACI 530-05) AND "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 530.1-05).
- M-2 THE CONTRACTOR SHALL OBTAIN COPIES OF THE MASONRY CODE AND SPECIFICATIONS FOR REFERENCE AT THE JOB SITE.
- M-3 THE STRUCTURES HAVE BEEN DESIGNED AS BEARING WALL STRUCTURES. ALL 8" AND 12" MASONRY UNITS SHALL BE LAID PRIOR TO PLACEMENT OF COLUMNS, BEAMS AND SLABS IN THE SAME STORY. (THIS DOES NOT INCLUDE NON-BEARING MASONRY PARTITIONS THAT TERMINATE BELOW AND DO NOT CONTACT THE FLOOR OR ROOF ABOVE.)
- M-4 USE CONCRETE MASONRY UNITS COMPLYING WITH THE REQUIREMENTS OF ASTM C 90, TYPE II AND MEETING THE REQUIREMENTS OF U.L. D-2 (2 HOUR RATED). USE TYPE "S" MORTAR WITH A MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI.
- M-5 MASONRY PRISM STRENGTH (f'm) SHALL BE 1500 PSI MINIMUM.
- M-6 REINFORCING STEEL SHALL BE NEW BILLET DEFORMED BARS, FREE OF SCALE, RUST, OIL OR OTHER SUBSTANCES INHIBITING BOND CONFORMING TO ASTM A-615, GRADE 60, WITH MINIMUM YIELD STRENGTH OF 60,000 PSI.
- M-7 PROVIDE FULL MORTAR BEDDING AROUND ALL FILLED CELLS WITH VERTICAL REINFORCING.
- M-8 REINFORCE ALL WALLS WITH LADDER TYPE HORIZONTAL JOINT REINFORCING IN BE JOINTS AT 16" O.C. MEASURED VERTICALLY. LAP SPLICE ALL HORIZONTAL JOINT REINFORCING 6" MINIMUM. PROVIDE PREFABRICATED "TEE" AND "ELL" SECTIONS AT INTERSECTING WALLS.
- M-9 DISCONTINUE HORIZONTAL JOINT REINFORCING AT MASONRY CONTROL JOINTS.
- M-10 MAXIMUM HEIGHT OF GROUT LIFT FOR FILLING CELLS SHALL NOT EXCEED 8 FEET. PROVIDE CLEAN OUT HOLES AT BASE OF FILLED CELLS FOR REMOVAL OF MORTAR DROPPINGS AND OTHER DEBRIS.
- M-11 GROUT FOR FILLED CELLS SHALL BE VIBRATED DURING PLACEMENT USING A "PENCIL" TYPE VIBRATOR. VIBRATOR SHALL EXTEND FULL HEIGHT OF GROUT LIFT.
- M-12 UNLESS NOTED OTHERWISE, PROVIDE 8" DEEP PRECAST "U" LINTEL OVER ALL MASONRY WALL OPENINGS GREATER THAN 16".
 REINFORCE LINTEL WITH 2-#5 BARS AND FILL SOLID WITH GROUT. CUT OUT BOTTOM OF LINTEL AT ENDS TO ALLOW CONTINUATION OF FILLED CELL REINFORCING AND GROUT.
- M-13 PROVIDE 2-#4 HORIZONTAL BARS IN GROUT FILLED KNOCK OUT BLOCK AT SILL OF ALL OPENINGS.
- M-14 PROVIDE 1-#5 IN GROUT FILLED CELL EACH SIDE OF DOOR AND WINDOW OPENINGS FULL HEIGHT OF WALL.
- M-15 PROVIDE 2-#5 HORIZONTAL BARS IN GROUT FILLED KNOCK OUT COURSE AT TOP OF INTERIOR PARTITIONS AND OTHER NON-LOAD-BEARING WALLS. PROVIDE L4x4x1/4 x 0'-8" EACH SIDE OF WALL WITH L3x3x1/4 DIAGONAL BRACE PERPENDICULAR TO WALL AT 1 TO 1 SLOPE TO STRUCTURE ABOVE. SPACE BRACES AT 6'-0" O.C. MAX.
- M-16 FILL ADDITIONAL MASONRY CELLS WITH GROUT AT LOCATIONS SHOWN ON SECTIONS TO ACCOMMODATE ANCHORAGE OF DRILLED ANCHORS. COORDINATE WITH ARCHITECTURAL AND STRUCTURAL DRAWINGS.
- M-17 GROUT FOR FILLED CELLS AND LINTELS SHALL MEET THE REQUIREMENTS OF ASTM C-476, FINE OR COARSE GROUT.
- M-18 HOLES DRILLED INTO GROUT FILLED MASONRY FOR SETTING ANCHOR BOLTS OR REINFORCING BARS IN EPOXY SHALL BE BLOWN OUT WITH COMPRESSED AIR, THEN BRUSHED OUT WITH A NYLON BRISTLE BRUSH OF THE APPROPRIATE DIAMETER, AND THEN BLOWN OUT AGAIN WITH COMPRESSED AIR PRIOR TO PLACEMENT OF THE EPOXY.
- M-19 PROVIDE ONE VERTICAL BAR OF THE SAME SIZE AS THE TYPICAL WALL REINFORCING AT FIRST CELL ON EACH SIDE OF OPENING FOR OPENINGS LESS THAN 48 INCHES. PROVIDE TWO VERTICAL BARS OF THE SAME SIZE AS THE TYPICAL WALL REINFORCING AT FIRST TWO CELLS ON EACH SIDE OF OPENING FOR OPENINGS GREATER THAN 48 INCHES.
- M-20 AT LOCATIONS WHERE MASONRY OPENINGS INTERRUPT VERTICAL FILLED CELL REINFORCING, START SAME NUMBER AND SIZE OF BARS AT LINTEL AND CONTINUE UP FULL HEIGHT.
- M-21 LAP VERTICAL MASONRY REINFORCING BARS A MINIMUM OF 48 BAR DIAMETERS OR 1'-6" MINIMUM
- M-22 AT CHANGES IN DIRECTION OF MASONRY BEAMS PROVIDE CORNER BARS OF THE SAME SIZE AND QUANTITY AS ALL LONGITUDINAL BARS.

LIGHT GAGE STEEL TRUSSES

- LT-1 PRE-ENGINEERED PRE-FABRICATED LIGHT GAGE STEEL TRUSSES, TRUSS BRACING AND CONNECTIONS WITHIN THE TRUSS SYSTEM SHALL BE DESIGNED FOR THE LOADS INDICATED ON THE DRAWINGS AND BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA WHO SHALL BE THE TRUSS SYSTEM ENGINEER.
- LT-2 SIGNED AND SEALED SHOP DRAWINGS SHOWING TRUSS CONFIGURATIONS WITH MEMBER SIZES AND CONNECTIONS, TRUSS LAYOUT WITH PIECE MARKS, REQUIRED TRUSS TO TRUSS CONNECTIONS, DESIGN CRITERIA, DURATION FACTORS, ERECTION DETAILS AND ALL BRACING FOR WIND AND ERECTION LOADS SHALL BE SUBMITTED FOR APPROVAL PRIOR TO FABRICATION OF THE TRUSSES.
- LT-3 PRE-FABRICATED LIGHT GAGE STEEL TRUSSES SHALL BE FABRICATED FROM STEEL MEETING THE REQUIREMENTS OF ASTM A-446 WITH MINIMUM YIELD STRENGTH OF 40 KSI.
- LT-4 TRUSSES SHALL BE DESIGNED SUCH THAT THE CENTERLINES OF THE MEMBERS SHALL BE COINCIDENT AT JOINTS.
- LT-5 THE NUMBER OF PANELS AND DIRECTION OF THE WEB MEMBERS SHALL BE SELECTED TO SUIT THE DESIGN REQUIREMENTS. SEE DRAWINGS FOR TRUSS PROFILES AND ANY SPECIAL CONDITIONS/REQUIREMENTS FOR PANEL POINTS.
- LT-6 DESIGN OF METAL CONNECTED ROOF TRUSSES TO COMPLY WITH:

FLORIDA BUILDING CODE, 2014 EDITION EPCOT BUILDING CODE 2015 EDITION WITH 2016 AND 2017 SUPPLEMENTS AISI "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS"

LT-7 TRUSS DESIGN LOADS AS FOLLOWS:

SLOPED TOP CHORD ROOF TRUSSES, 6:12 PITCH:

TOP CHORD	
DEAD LOAD	10 PSF
LIVE LOAD	30 PSF
BOTTOM CHORD	
DEAD LOAD	20 PSF
LIVE LOAD	10 PSF

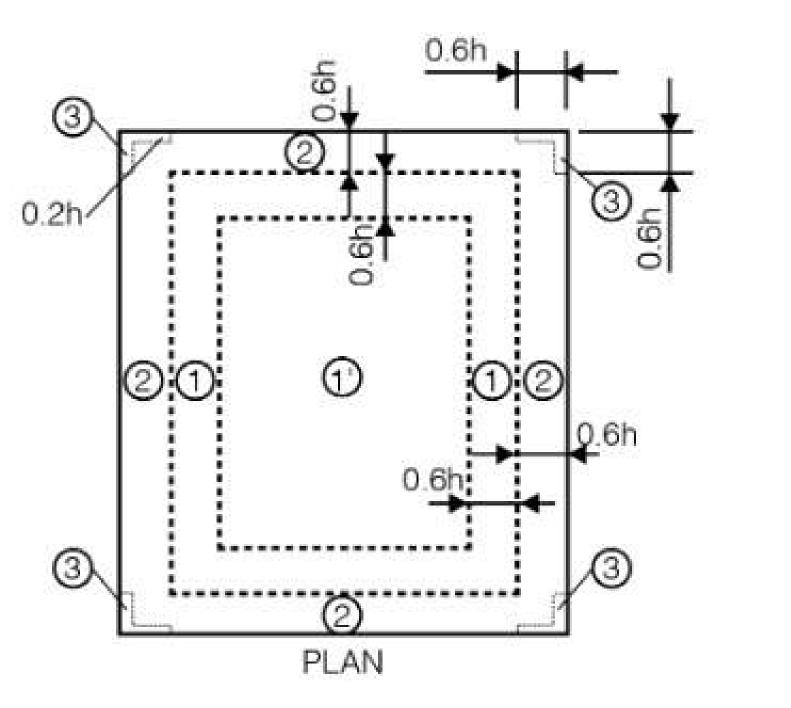
NET WIND UPLIFT ON TRUSS SHALL BE BASED ON WIND PRESSURES AS SHOWN IN THE COMPONENT AND CLADDING PRESSURE TABLE. WIND FORCES SHALL BE CONSIDERED ACTING BOTH INWARD AND OUTWARD. FULL WIND DESIGN PRESSURES SHALL BE USED IN CALCULATION OF DEFLECTIONS. REDUCED WIND PRESSURES AS PERMITTED IN THE BUILDING CODE ARE NOT PERMITTED FOR DEFLECTION CALCULATIONS. DEAD LOAD EQUAL TO THE ACTUAL WEIGHT OF THE TRUSSES AND ROOF STRUCTURE MAY BE DEDUCTED FROM THE WIND UPLIFT PRESSURES.

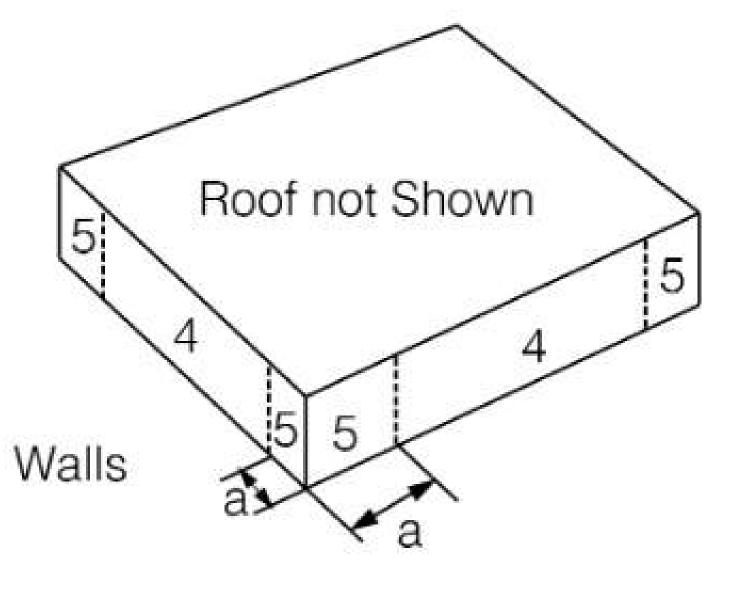
- LT-8 PROVIDE PERMANENT HORIZONTAL BRIDGING AND BRACING AS DESIGNED BY THE TRUSS FABRICATOR.
- LT-9 NO STRUCTURAL MEMBER SHALL BE CUT, NOTCHED OR OTHERWISE REDUCED IN SIZE OR STRENGTH WITHOUT THE WRITTEN PERMISSION OF THE ENGINEER OF RECORD

Zone	Figure	A = 200.00 ft ² psf	A = 500.00 ft ² psf	A > 1000.00 ft ² psf
1	30.3-2A	9.60 -30.01	9.60 -26.35	9.60 -26.35
1'	30.3-2A	9.60 -20.75	9.60 -16.31	9.60 -12.95
2	30.3-2A/30.3-1	19.50 -39.99	18.09 -35.28	18.09 -35.28
3	30.3-2A/30.3-1	19.50 -39.99	18.09 -35.28	18.09 -35.28
4	30.3-1	19.50 -21.51	18.09 -20.10	18.09 -20.10
4_P (P1/P4)	30.3-1/30.3-2A	15.92 -17.99	14.47 -16.54	14.47 -16.54
4_P (P3/P2)	30.3-1/30.3-2A	15.92 -36.99	14.47 -32.15	14.47 -32.15
4_P (P1-P2)/(P3-P4)	30.3-1/30.3-2A	52.91 33.91	46.62 31.00	46.62 31.00
5	30.3-1	19.50 -22.92	18.09 -20.10	18.09 -20.10
5_P (P1/P4)	30.3-1/30.3-2A	15.92 -19.44	14.47 -16.54	14.47 -16.54
5_P (P3/P2)	30.3-1/30.3-2A	15.92 -36.99	14.47 -32.15	14.47 -32.15
5_P (P1-P2)/(P3-P4)	30.3-1/30.3-2A	52.91 35.36	46.62 31.00	46.62 31.00

Wind Pressure Summary for C&C Zones based Upon Areas Ch 30 Pt 1 (Table 1 of 2) All wind pressures include a Load Factor (LF) of 0.6

Zone	Figure	A ≤ 10.00 ft ² psf	A = 20.00 ft ² psf	A = 50.00 ft ² psf	A = 100.00 ft ² psf
1	30.3-2A	10.72 -41.98	10.05 -39.21	9.60 -35.55	9.60 -32.78
1'	30.3-2A	10.72 -24.11	10.05 -24.11	9.60 -24.11	9.60 -24.11
2	30.3-2A/30.3-1	24.11 -55.38	23.05 -51.81	21.63 -47.11	20.57 -43.55
3	30.3-2A/30.3-1	24.11 -55.38	23.05 -51.81	21.63 -47.11	20.57 -43.55
4	30.3-1	24.11 -26.12	23.05 -25.06	21.63 -23.64	20.57 -22.58
4_P (P1/P4)	30.3-1/30.3-2A	20.67 -22.74	19.57 -21.64	18.12 -20.18	17.02 -19.09
4_P (P3/P2)	30.3-1/30.3-2A	20.67 -52.82	19.57 -49.16	18.12 -44.32	17.02 -40.65
4_P (P1-P2)/(P3-P4)	30.3-1/30.3-2A	73.49 43.40	68.73 41.21	62.43 38.30	57.67 36.11
5	30.3-1	24.11 -32.15	23.05 -30.02	21.63 -27.19	20.57 -25.06
5_P (P1/P4)	30.3-1/30.3-2A	20.67 -28.94	19.57 -26.74	18.12 -23.83	17.02 -21.64
5_P (P3/P2)	30.3-1/30.3-2A	20.67 -52.82	19.57 -49.16	18.12 -44.32	17.02 -40.65
5_P (P1-P2)/(P3-P4)	30.3-1/30.3-2A	73.49 49.61	68.73 46.31	62.43 41.95	57.67 38.66





WIND PRESSURES SHOWN INCLUDE A LOAD FACTOR OF 0.6.
 (PSF) PER ASCE 7-16.
 POSITVIE AND NEGATIVE SIGNIFY PRESSURES ACTING TOWARDS OR AWAY FROM BUILDING SURFACE.

3. FOR LRFD PRESSURES DIVIDE THE TABLE VALUES BY 0.6.



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DESIGN GROUP, INC.
STRUCTURAL ENGINEERS

INTERNATIONAL ENGINEERING SERVICES DESIGN GROUP, INC.

LICENSE NO. EB 5734 ROBERT G. JOHNSON, PE 61899
7350 FUTURES DRIVE, SUITE 8 PHONE NO. (407) 363-9600
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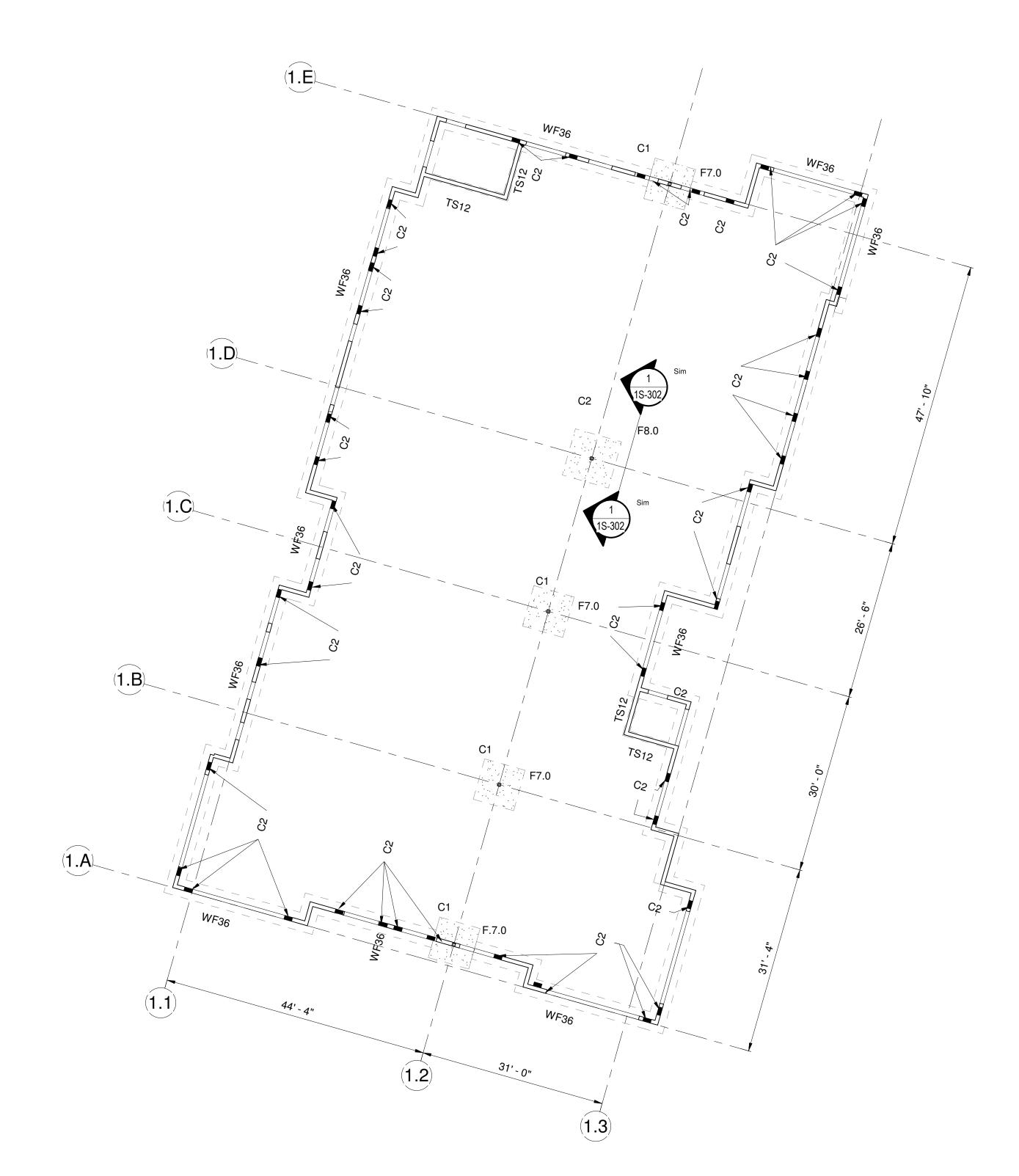
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Project No: Drawn By:		ROBERT G. JO FLORIDA, PE	
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STRUCTURAL NOTES & WIND PRESSURES

1S-101



	COLUMN SCHEDULE			
TYPE	TYPE SIZE REINFORCEMENT			
C1	HSS 6X6X1/4"	NONE		
C2	HSS 6X6X3/8"	NONE		
C2	RC-7-5/8"X15-5/8"	RC-COLUMN - 7-5/8 X15-5/8 W/ 4 - #5 VERTICAL & #3 TIES AT 8" O.C		
C3	RC-7-5/8X12"	RC-COLUMN - 7-5/8 X12-5/8 W/ 4 - #5 VERTICAL & #3 TIES AT 8" O.C		

	FOUNDATION SCHEDULE			
TYPE	TYPE SIZE REINFORCEMENT			
WF36	WF36 3'X1'XCONT. (4) - #5 CONT. & #5 TRANSVERSE @32" O.C			
TS12	TS12 1'X1'XCONT. (2) - #5 CONT.			
F7.0	7'X7'X18"	(9) - #5 EACH WAY, TOP & BOTTOM.		
F8.0	8'X8'X18"	(11) - #5 EACH WAY, TOP & BOTTOM.		

NOTE:
1. TOP OF FOUNDATION IS AT -2'-0" (2' BELOW TOP OF SLAB).
2. #5 VERTICAL BARS @24 O.C IN FULLY GROUTED CELLS.

NOTES:

1. CONTRACTOR SHALL COORDINATE WITH ALL TRADES FOR ITEMS RECESSED OR EMBEDDED IN MASONRY WALLS. DO NOT PLACE EMBEDDED ITEMS IN GROUT FILLED CELLS OR SOLID CAST CONRETE BEAMS AND COLUMNS.

2. CONDUIT MAY BE PLACED ON FOUNDATIONS OR INSTALLED TO PENETRATE VERTICALLY THROUGH FOUNDATIONS. CONDUITS SHALL BE SEPARATED BY A MINIMUM OF THREE CONDUIT DIAMETERS. 3. VERTICAL PENETRATIONS THROUGH A CONCRETE BEAM OR REINFORCED MASONRY ARE PERMITTED. CONDUITS SHALL BE SEPARATED BY A MINIMUM OF THREE CONDUITS DIAMETERS. REINFORCING MAY NOT BE DISPLACED BY CONDUITS.

4. CONTRACTOR SHALL COORDINATE WITH LIGHTNING PROTECTION CONTRACTOR FOR LOCATION AND PLACEMENT OF LIGHTNING PROTECTION CABLES IN CONDUIT. DESIGN INTENT IS FOR LIGHTNING PROTECTION CABLING FEED THROUGH CONCRETE MASONRY UNITS. DO NOT PLACE LIGHTNING PROTECTION CONDUIT IN GROUT FILLED CELLS EXCEPT AS NOTED ABOVE 5. CONTRACTOR SHALL COORDINATE WITH ELECTRICAL CONTRACTOR FOR PLACEMENT OF RECESSED JUNCTION BOXES, RECESSED SWITCH BOXES, OTHER RECESSED ITEMS, AND CONDUIT INSTALLED IN MASONRY WALLS. DO NOT PLACE RECESSED BOXES OR OTHER RECESSED ITEMS IN GROUT FILLED OR SOLID CONCRETE BEAMS OR COLUMNS.



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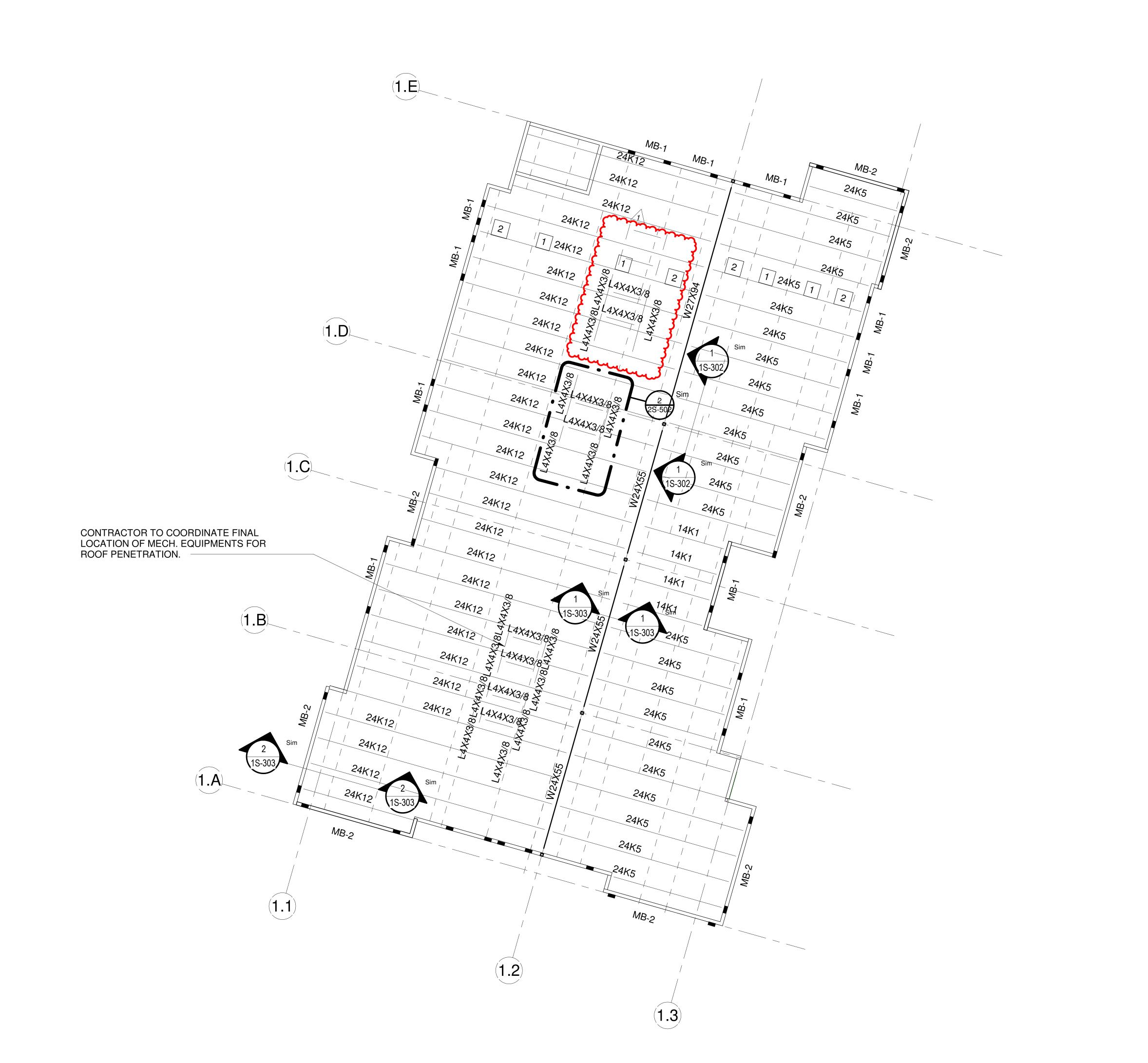
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FOUNDATION PLAN

1S-201

FOUNDATION PLAN



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PLAN NOTES: 1. JOIST BRIDGING - TYP - 3 ROWS OF TOP CHORD BRIDGING.

2. UPLIFT BRIDGING AT FIRST BOTTOM CHORD PANEL POINTS @ END OF JOISTS.

3. JOIST SPACING SHALL BE 5' O.C. UNLESS NOTED OTHERWISE.

ROOF CONSTRUCTION:
1-1/2" DEEP, 20 GAGE WIDE RIB "B" DECK EQUAL TO 1.5B20 ROOF DECK AS PER MFGR BY VULCRAFT 5/8" PUDDLE WELDS ON 36/7 PATTERN W/#10 SIDELAP FASTENERS AT 12" O.C. TOP OF JOIST BEARING AND TOP OF STEEL SHALL BE 17'-4".

LINTEL SCHEDULE: REFER TO SHEET-2S-502

MB-1-REFER TO SCHEDULE FOR LINTEL SIZES & TYPE.

MB-2-REFER TO SCHEDULE FOR LINTEL SIZES & TYPE.



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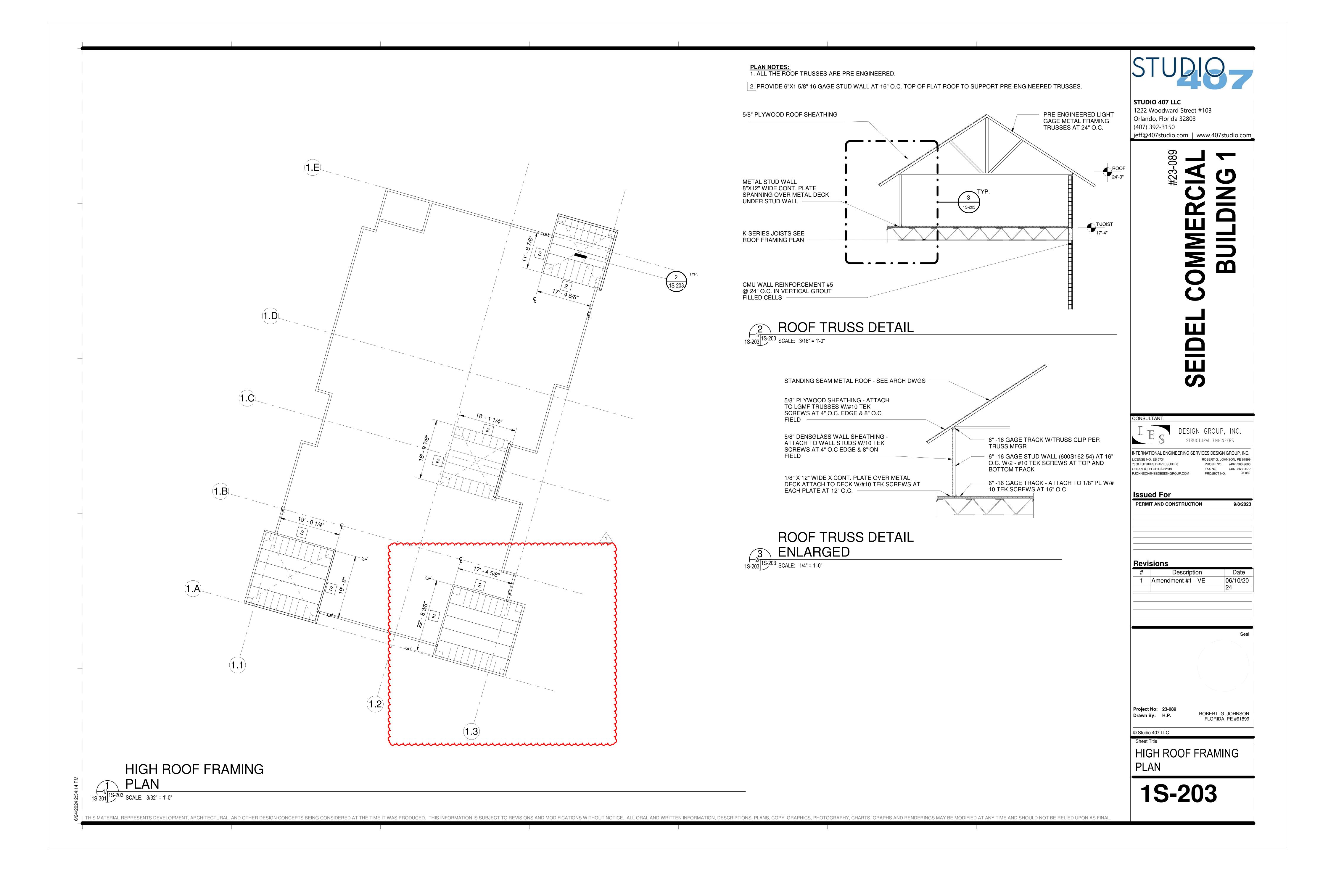
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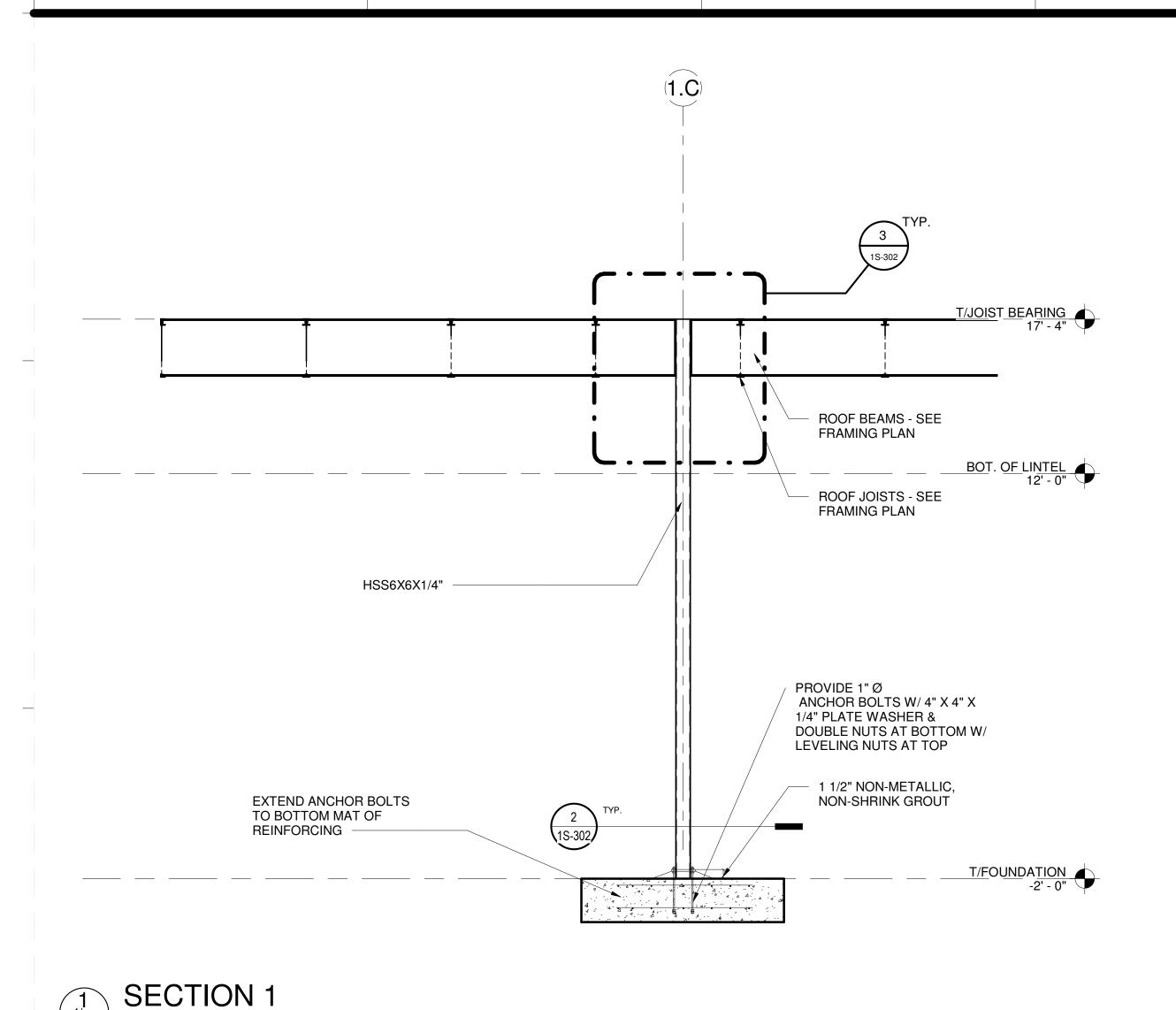
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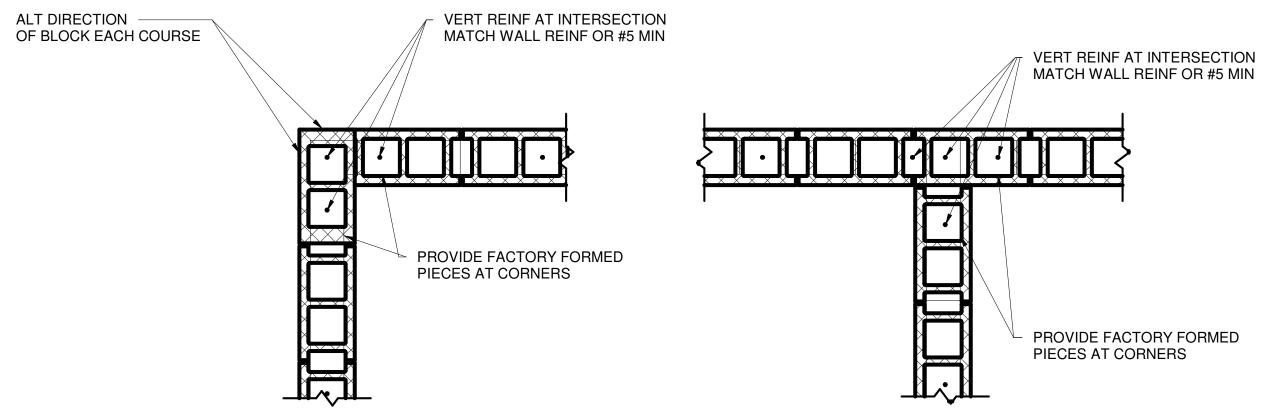
ROOFING PLAN

1 ROOF PLAN
1S-301 1S-202 SCALE: 3/32" = 1'-0"





1. HORIZ JOINT REINFORCEMENT SPACED AT 16" OC UNO. 2. TERMINATE JOINT REINFORCEMENT AT EACH SIDE OF WALL CJ.

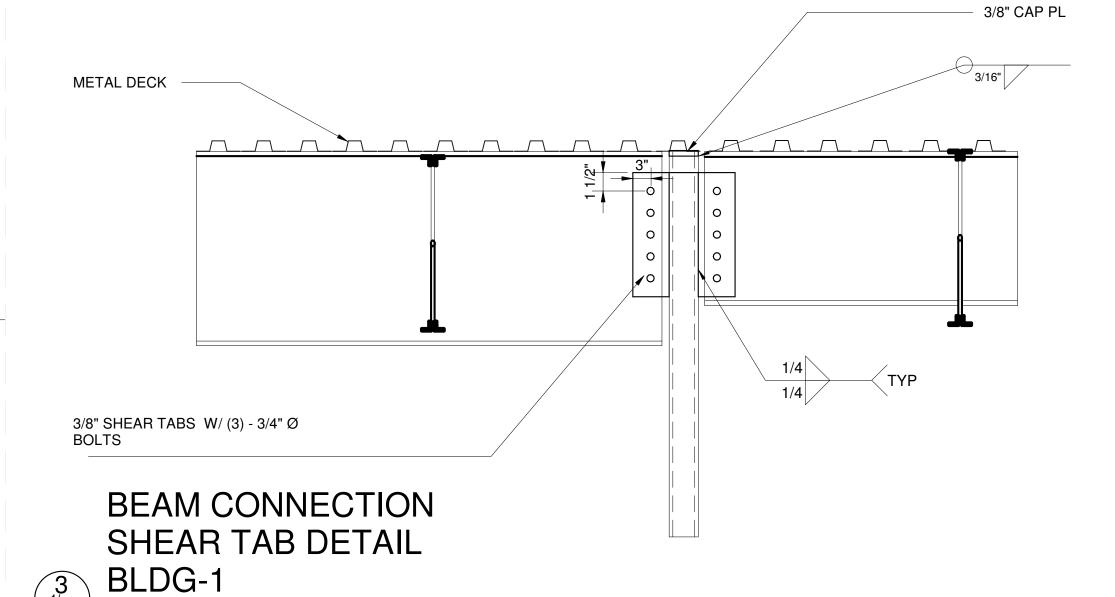


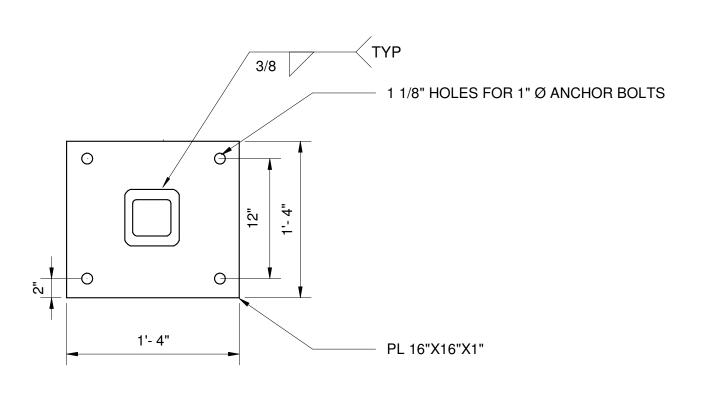
INTERSECTION OF MASONRY WALLS

4 CMU CORNER DETAIL

1S-302 SCALE: 1" = 1'-0"

CORNER OF MASONRY WALLS





COLUMN BASE PLATE

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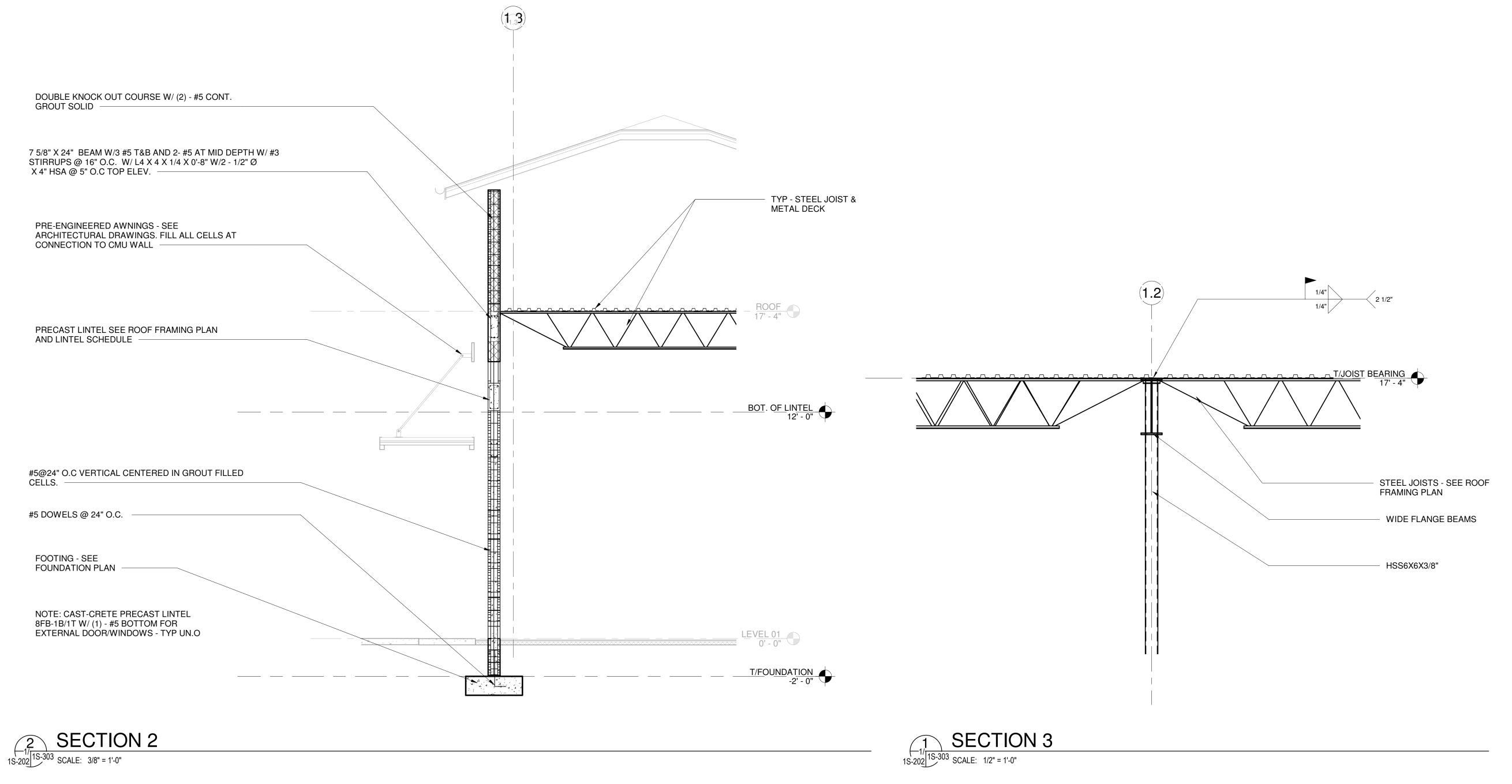
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BUILDING SECTIONS

1S-302



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COMMERCIAL BUILDING 1

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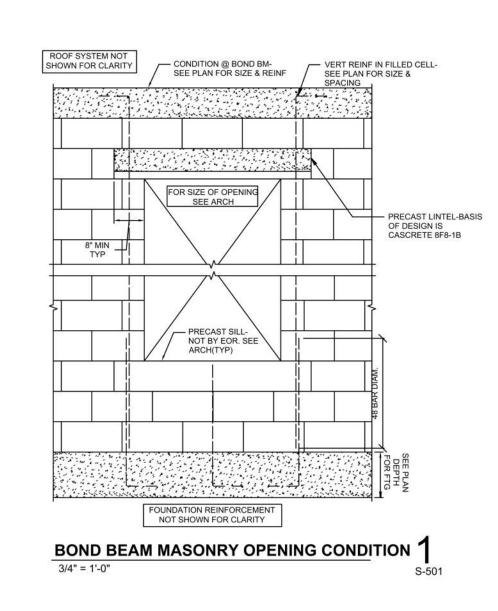
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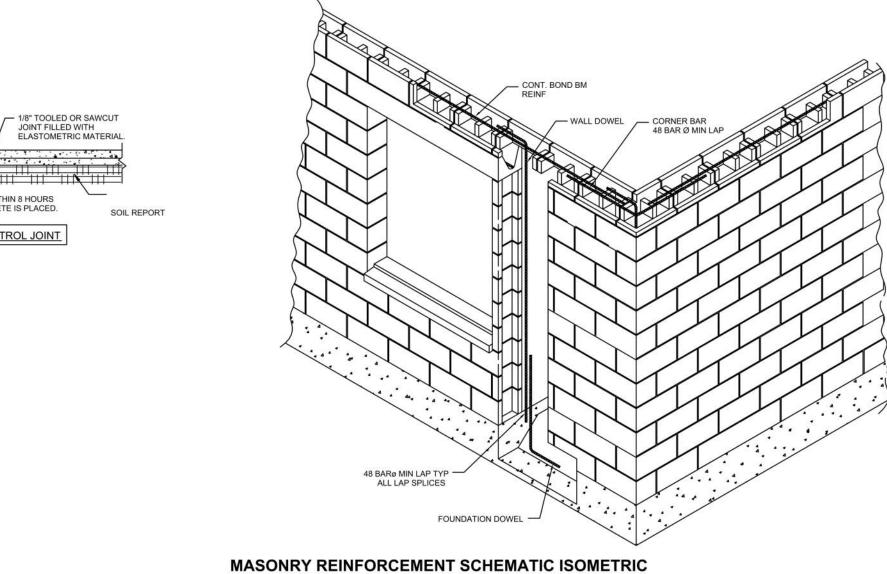
1S-303

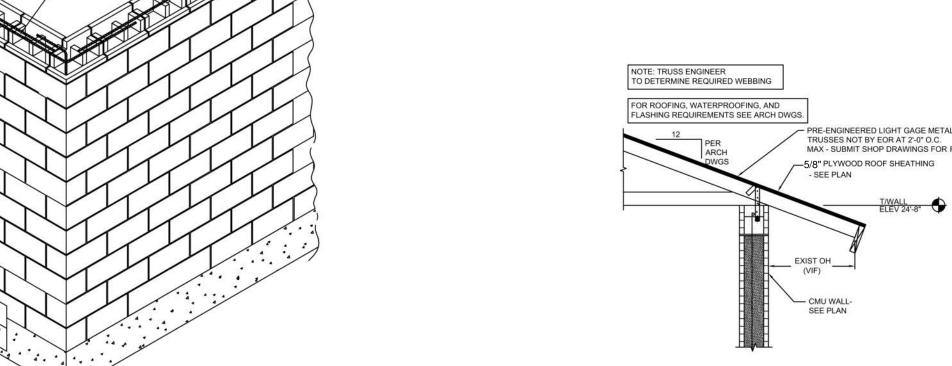
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SAW JOINT WITHIN 8 HOURS AFTER CONCRETE IS PLACED.

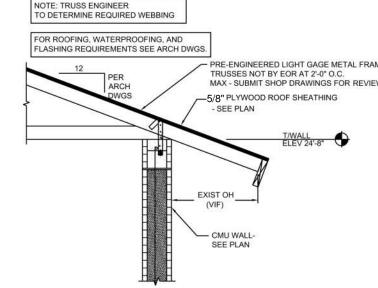
SAWED CONTROL JOINT





PRE-ENGINEERED LIGHT GAGE METAL FRAMING TRUSSES NOT BY EOR AT 2'-0" O.C. MAX - SUBMIT SHOP DRAWINGS FOR REVIEW

WOOD FRAME DETAIL AT TYPICAL TRUSS CONNECTION 3



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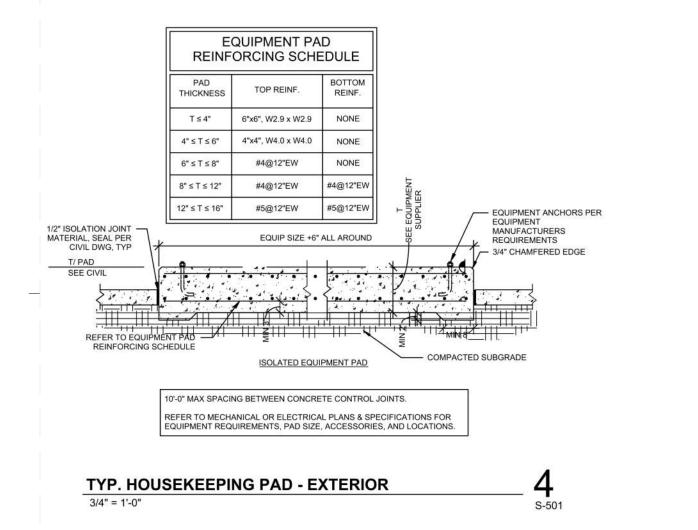
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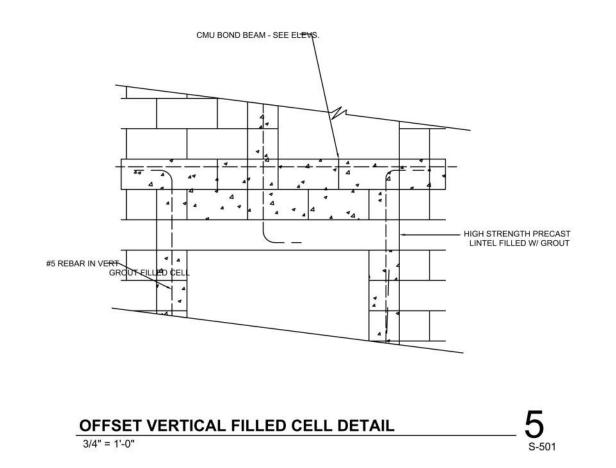
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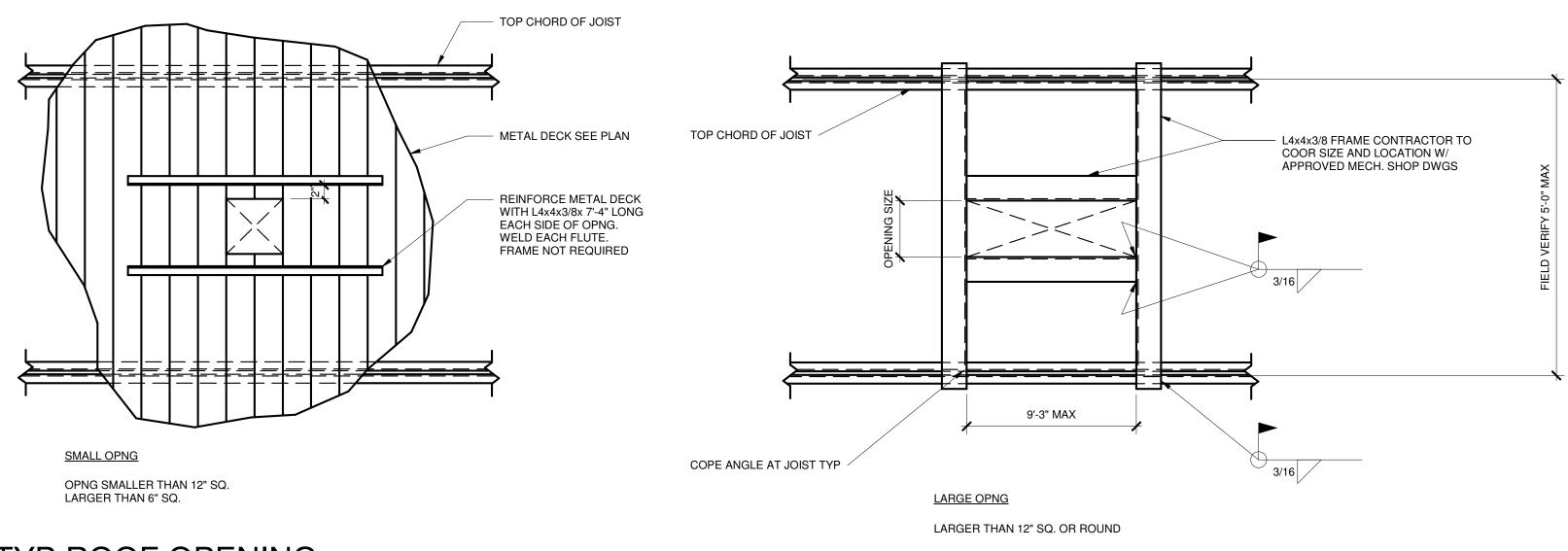
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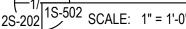
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Project No: Drawn By:	ROBERT G. JOHNS

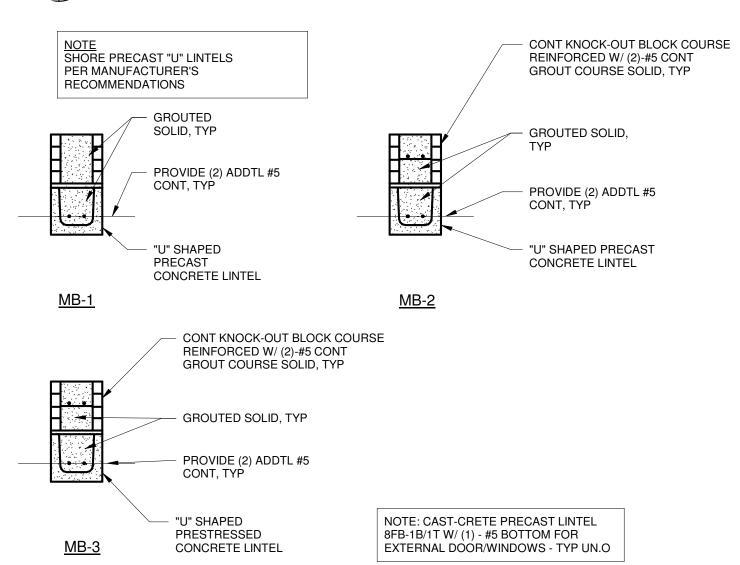
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STRUCTURAL DETAILS



TYP ROOF OPENING







MASONRY LINTEL SCHEDULE (CAST-CRETE LINTEL)			
MARK	LENGTH (L)	CAST-CRETE MARK	REMARK
MB-1	2' -10"< L < 5"-10"	8F8-18 PRECAST	
MB-2	5' -10"< L < 14'-0"	8F16-1B PRECAST	
MB-3	14' -0"< L < 21'-4"	8F16-2B/2T PRESTRESSED	

NOTES:

1. PROVIDE MASONRY LINTEL OVER ALL OPNGS. IF NO LINTEL IS SPECIFIED, PROVIDE MB-

- PROVIDE MINIMUM END BEARING OF 8". CUT OUT BOTTOM OF LINTEL AT END TO ALLOW CONTINUATION OF FILLED CELL REINF.
- 3. MASONRY LINTEL SUBSTITUTIONS MUST BE APPROVED BY THE STRUCT ENGINEER OF RECORD PRIOR TO INSTALLATION.

SCHEDULE



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