

GENERAL STRUCTURAL NOTES

DESIGN CRITERIA

- D1 ALL WORK SHALL CONFORM TO AT LEAST THE MINIMUM STANDARDS OF THE FLORIDA BUILDING CODE, 2023 EIGHTH EDITION.
- D2 DESIGN LOADS PER FLORIDA BLDG. CODE, SEE SHEET S2.1 FOR ROOF DESIGN LOADS. WIND DESIGN PER ASCE 7-22 (V_{ult})=170 MPH (ASD)=132 MPH RISK CATEGORY= II EXPOSURE C ENCLOSED BUILDING MAIN BUILDING DESIGN: q_w(GUL)=65.8 PSF q_w(GUL)=39.4 PSF q_w(E)=3" h=16'-10"
- D3 FOUNDATION DESIGN VALUES: 2500 PSF BASED ON SOIL REPORT PERFORMED BY: WHITESTONE DATED FEBRUARY 14, 2025.
- D4 TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE STRUCTURAL PLANS AND SPECIFICATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE LATEST EDITION.

GENERAL

- G1 THE GENERAL CONTRACTOR SHALL REVIEW AND DETERMINE THAT DIMENSIONS ARE COORDINATED BETWEEN ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO FABRICATION OR START OF CONSTRUCTION.
- G2 THE GENERAL CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORK AND PERSONS, AND OTHER PEOPLE DURING CONSTRUCTION. HE SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION.
- G3 NO STRUCTURAL MEMBER SHALL BE CUT, NOTCHED OR OTHERWISE REDUCED IN STRENGTH.
- G4 THE GENERAL CONTRACTOR SHALL COORDINATE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR ANCHORED, EMBEDDED, SUPPORTED ITEMS WHICH AFFECT THE STRUCTURAL DRAWINGS AND NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES.
- G6 NO PAPER SHOP DRAWINGS WILL BE REVIEWED BY THIS OFFICE. ALL SUBMITTALS MUST BE IN ELECTRONIC FORM, SIGNED AND SEALED PAPER SHOP DRAWINGS WILL BE STAMPED BY THIS OFFICE AFTER ALL COMMENTS HAVE BEEN INCORPORATED INTO THE DRAWINGS AND CALCULATIONS.

SLAB ON GRADE

- S1 UNLESS NOTED OTHERWISE IN THE GEOTECHNICAL REPORT REFERENCED ABOVE, COMPACT INTERIOR TO BE MODIFIED PROCTOR MAXIMUM DRY DENSITY (ASTM D1557). SOIL COMPACTION SHALL BE FIELD CONTROLLED BY REPRESENTATIVE TECHNIQUE OF QUALIFIED LABORATORY. EACH LAYER OF FILL SHALL NOT EXCEED 8" THICK AND SHALL BE COMPACTED PRIOR TO PLACEMENT OF NEXT LAYER.
- S2 MINIMUM PLACEMENT OF CONTROL JOINTS FOR FUTURE SLAB ON GRADE SHALL BE AS SET IN THE TABLE BELOW. JOINTS OCCUR IN EACH DIRECTION.

SLAB THICKNESS (IN.)	COLUMN SPACING (FT.)			
	15' OR LESS	15'-1" TO 25'-0"	25'-0" TO 45'-0"	45'-0" TO 60'-0"
4"	AT COL. GRID & 1/2 WAY BTWN (2 EQ.)	AT COL. GRID & 1/3 POINTS (3 EQ.)	AT COL. GRID & 1/4 POINTS (4 EQ.)	
5"	AT COL. GRID & 1/2 WAY BTWN (2 EQ.)	AT COL. GRID & 1/3 POINTS (3 EQ.)	AT COL. GRID & 1/4 POINTS (3 EQ.)	
6"	AT COL. GRID & 1/2 WAY BTWN (2 EQ.)	AT COL. GRID & 1/3 POINTS (3 EQ.)	AT COL. GRID & 1/4 POINTS (3 EQ.)	

CONCRETE AND REINFORCING

- C1 CONCRETE WORK SHALL CONFORM TO ACI CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318-19).
- C2 ALL CONCRETE SHALL HAVE A MINIMUM 28 DAY STRENGTH & PROPERTIES AS FOLLOWS:

	MAX. SLUMP	WATER/CMT
CONCRETE BEAMS, COLS	4000 PSI	52±1" 0.52
SLAB ON GRADE	4000 PSI	54±1" 0.52
FOUNDATIONS	3000 PSI	61±1" 0.58
FILLED CELLS	3000 PSI	91±1" 0.60
- C3 CONCRETE MIX DESIGN SUBMITTALS MUST INCLUDE THE AREA IN WHICH THE CONCRETE IS TO BE PLACED (E.G. FOUNDATIONS, SLAB-ON-GRADE, FILLED CELLS, COLUMNS, ETC.). FAILURE TO DO SO WILL CAUSE DELAY AND/OR REJECTION OF SUBMITTALS.
- C4 REBARS SHALL CONFORM TO ASTM-615 GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185.
- C5 MINIMUM COVER FOR REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED.

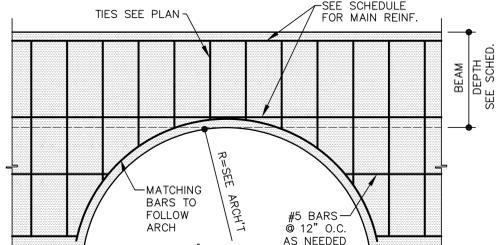
	3"	1 1/2" FROM TOP
FOOTINGS		
SLABS ON GRADE	1 1/2" (ON TIES)	
BEAMS		1 1/2" (ON TIES)
COLUMNS		

- C6 SPLICES AND ANCHORAGE OF REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED.
 - WELDED WIRE FABRIC 36 DIA. (12" MIN) MINIMUM REQUIREMENTS
- C7 REINFORCEMENT IN WALLS, FOOTINGS AND BEAMS SHALL BE CONTINUOUS UNLESS OTHERWISE NOTED. THIS SH-T- UNLESS OTHERWISE NOTED. HOOK AND LAP ALL CORNER AND INTERSECTING BARS. (SEE TYPICAL DETAILS)
- C8 TERMINATE ALL DISCONTINUED ELEVATED SLAB TOP BARS WITH 180 DEGREE STANDARD HOOK UNLESS OTHERWISE NOTED.
- C9 CONTINUOUS TOP BARS SHALL BE SPLICED AT MIDSPAN. CONTINUOUS BOTTOM BARS SHALL BE SPLICED AT CENTER-LINE OF SUPPORTS (OR AS SHOWN ON TYPICAL DETAILS)
- C10 AT CHANGES IN DIRECTION OF CONCRETE WALLS AND STRIP FOOTINGS, PROVIDE CORNER BARS AT SAME SIZE AND SPACING AS HORIZONTAL BARS. (REFER TO B/S4.0)

CLASS B TENSION LAP SPLICES

BAR SIZE	F _c 3000 PSI	F _c 4000 PSI
#3	32"	28"
#4	43"	37"
#5	53"	46"
#6	64"	55"
#7	93"	81"
#8	107"	92"
#9	120"	104"
#10	136"	117"
#11	151"	130"

NOTE: LAP LENGTHS SHOWN ARE FOR WORST CASE CLASS B TENSION LAPS.



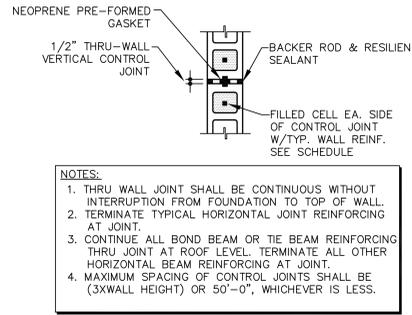
2 **ARCHED BEAM (AB) DETAIL**
N.T.S.

MASONRY

- M1 MASONRY CONSTRUCTION SHALL CONFORM TO ACI STANDARD BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES TMS 402-16 AND SPECIFICATIONS FOR MASONRY STRUCTURES TMS 602-16.
- M2 CONCRETE BLOCKS SHALL CONFORM TO ASTM C-90. f_m = 2000 PSI
- M3 MORTAR SHALL COMPLY WITH ASTM C270, TYPE M OR S. COMPRESSIVE STRENGTH SHALL BE 1800 PSI, RESPECTIVELY. SITE TESTED MORTAR CUBES SHALL ACHIEVE A MINIMUM OF 80% OF THE DESIGN COMPRESSIVE STRENGTH)
- M4 BLOCK SHALL NOT BE MOISTENED BEFORE GROUTING.
- M5 ALL MASONRY CROSS WEBS SHALL BE FULLY BEDDED IN MORTAR AROUND CELLS TO BE GROUTED.
- M6 THE MINIMUM CONTINUOUS UNOBSTRUCTED CELL AREA IN CELL TO RECEIVE GROUT MUST BE NOT LESS THAN 2"x3". MORTAR FINIS MUST BE REMOVED AS BLOCK PLACEMENT PROCEEDS. MORTAR DROPPINGS MUST BE KEPT OUT OF CELLS WHICH ARE TO BE GROUTED.
- M7 REINFORCE WALLS WITH LADDER TYPE (ASTM A-82, #8 GAGE WIRE) REINFORCEMENT EQUAL TO DURO-WALL IN BED JOINTS AT 16" OC UNO, MEASURED VERTICALLY PLACE PER MFR INSTR. LAP ALL HORIZONTAL JOINT REINFORCING 8", MIN. EXTEND HORIZONTAL REINFORCEMENT 4 INCHES INTO CONC. COLS. OR THE COLS.

- M8 WHERE SHOWN, CELLS OF BLOCK MASONRY SHALL BE FILLED WITH GROUT WITH MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS, AND MEET ASTM C476. GROUT SHALL BE PROVIDED BY CONCRETE SUPPLIER FROM THEIR BATCH PLANT WITH A SLUMP OF 8" TO 10". JOB SITE MIXING OF GROUT SHALL NOT BE PERMITTED. TESTING SHALL CONFORM TO ASTM C1019.
- M9 GROUT FOR FILLED CELLS SHALL BE POURED OR PUMPED IN LIFTS NOT TO EXCEED FOUR (4) FEET IN HEIGHT, AND A MAX POUR OF 12 FT. FILLED CELLS SHALL BE CONSOLIDATED AT TIME OF POURING BY RODDING OR VIBRATING BETWEEN LIFTS.
- M10 PROVIDE KNOCK-OUT CURM AT BASE OF EACH FILLED CELL TO ALLOW VISUAL VERIFICATION OF COMPLETE GROUT PENETRATION (FOR LIFTS OF 5' OR LESS, A KNOCK-OUT AT BASE OF LIFT WILL NOT BE REQUIRED).
- M11 VERTICAL REINFORCING MUST HAVE A MINIMUM CLEARANCE OF 1/2" TO INSIDE FACE. MIN VERTICAL BAR LAP = 48" BAR DIAMETER. VERTICAL REINFORCEMENT IN WALLS SHALL BE SECURED AND LATERALLY SUPPORTED AGAINST DISPLACEMENT AT INTERVALS NOT EXCEEDING 192 x BAR DIAMETER NOR 10 FT.
- M12 GROUT PLACEMENT STOPPED FOR (1) HOUR OR MORE SHOULD BE STOPPED (1 1/2") BELOW THE TOP OF THE MASONRY UNIT TO PROVIDE A KEY FOR SUBSEQUENT GROUTING.
- M13 SEE FOUNDATION PLANS FOR ALL VERT REINFORCING. TYP VERTICAL REINFORCING SIZE & SPACING SHALL BE ABOVE AND BELOW ALL WALL OPENINGS.
- M14 TEMPORARY BRACING AND SHORING OF WALLS TO PROVIDE STABILITY DURING CONSTRUCTION TO BE THE RESPONSIBILITY OF THE CONTRACTOR.
- M15 MASONRY CONSTRUCTION MATERIALS AND INSPECTIONS SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATIONS FOR MASONRY STRUCTURES (ACI-ASCE 530.1)" EXCEPT AS MODIFIED BY THE REQUIREMENTS OF THESE DOCUMENTS.
- M16 PROVIDE FILLED PRECAST U-LINTELS WITH (1) #5 CONT AT ALL OPENINGS WHERE CONCRETE BEAMS ARE NOT SHOWN OR NOTED. MINIMUM UNFILLED LINTEL CAPACITY = 400 LB/IF FOR SPAN INDICATED.
- M17 STOPPING AND RESUMING WORK: RACK BACK 1/2-UNIT LENGTH IN EACH COURSE. DO NOT TOOTH. CLEAN EXPOSED SURFACES OF SET MASONRY WET UNITS LIGHTLY (IF REQ'D) AND REMOVE LOOSE MAS UNITS AND MORTAR PRIOR TO LAYING FRESH MASONRY.
- M18 REINFORCE MASONRY OPENINGS GREATER THAN 1'-0" WIDE, WITH HORIZ JT REIN PLACED IN (2) HORIZ JT'S APPROXIMATELY 8" APART, IMMEDIATELY ABOVE THE LINTEL AND IMMEDIATELY BELOW THE SILL. EXTEND REINFORCING A MINIMUM OF 2'-0" BEYOND JAMBS OF THE OPENING EXCEPT AT CONTROL JOINTS. SEE PLAN FOR ADDITIONAL REQUIREMENTS.
- M19 DO NOT APPLY UNIFORM LOADS TO MASONRY WALLS FOR (3) DAYS.
- M20 DO NOT APPLY CONCENTRATED LOADS TO MASONRY WALLS FOR (7) DAYS.
- M21 EXTEND ALL VERTICAL WALL REINFORCEMENT TO WITHIN 2" OF TOP OF WALL OR BEAM UNLESS NOTED OTHERWISE. TERMINATE REINFORCING WITH STANDARD ACI HOOK.
- M22 MASONRY CONTROL JOINT DETAIL

- M23 NEOPRENE PRE-FORMED GASKET
- M24 1/2" THRU-WALL VERTICAL CONTROL JOINT
- M25 BACKER ROD & RESILIENT SEALANT
- M26 FILLED CELL EA. SIDE OF CONTROL JOINT W/TYP. WALL REINF. SEE SCHEDULE



- NOTES:
 - THRU WALL JOINT SHALL BE CONTINUOUS WITHOUT INTERRUPTION FROM FOUNDATION TO TOP OF WALL.
 - TERMINATE TYPICAL HORIZONTAL JOINT REINFORCING AT JOINT.
 - CONTINUE ALL BOND BEAM OR THE BEAM REINFORCING THRU JOINT AT ROOF LEVEL. TERMINATE ALL OTHER HORIZONTAL BEAM REINFORCING AT JOINT.
 - MAXIMUM SPACING OF CONTROL JOINTS SHALL BE (3XWALL HEIGHT) OR 50'-0", WHICHEVER IS LESS.

STEEL JOISTS

- SJ1 GENERAL CONTRACTOR SHALL ENGAGE A CERTIFIED TESTING AGENCY TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
- SJ2 ALL DESIGN, FABRICATION AND ERECTION OF STEEL JOISTS AND BRIDGING SHALL BE IN STRICT ACCORDANCE WITH THE CURRENT SPECIFICATIONS OF STEEL JOIST INSTITUTE AND RECOMMENDED CODE OF STANDARD PRACTICE.
- SJ3 THE ENDS OF ALL BRIDGING LINES TERMINATING AT WALLS OR BEAMS SHALL BE ANCHORED TO THE WALL OR BEAM.
- SJ4 ALL STEEL JOISTS ARE TO BE CAMBERED AS SPECIFIED BY SJI.
- SJ5 PROVIDE BOTTOM AND/OR TOP CHORD EXTENSIONS AS SHOWN ON DRAWINGS.
- SJ6 UNLESS NOTED OTHERWISE, MINIMUM JOIST BEARING SHALL BE 2 1/2" ON A STEEL MEMBER OR EMBED PLATE.
- SJ7 BRIDGING SHALL BE FURNISHED AND INSTALLED TO MEET THE SIZE AND SPACING REQUIREMENTS OF THE SJI STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOISTS. ALL BRIDGING AND BRIDGING ANCHORS SHALL BE COMPLETELY INSTALLED BEFORE CONSTRUCTION LOADS ARE PLACED ON THE JOISTS. THE LAST TWO JOIST SPACES IN A LINE OF BRIDGING SHALL BE "X" TYPE. ALL JOISTS 40'-0" OR LONGER REQUIRE A ROW OF BOLTED BRIDGING TO BE IN PLACE BEFORE SLACKENING OF HOISTING LINES. OTHER JOISTS REQUIRE SIMILAR BRIDGING CONSULT LATEST SJI SPECIFICATIONS.
- SJ8 ALL HANGERS TO SUPPORT MECHANICAL EQUIPMENT, ETC. TO BE SUPPORTED BY THE BOTTOM CHORD OF JOISTS SHALL BE LOCATED AT THE PANEL POINT OF THE JOIST. IF HANGERS MUST BE LOCATED BETWEEN PANEL POINTS, PROVIDE JOIST STIFFENERS: 1 1/2 x 1 1/2 x 3/16 JOIST STIFFENERS MUST BE INSTALLED FROM HANGER TO OPPOSITE CHORD PANEL POINT BEFORE LOAD IS APPLIED.
- SJ9 CONTRACTOR TO FURNISH BAR JOIST CERTIFICATIONS SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION.
- SJ10 FOR NET UPLIFT SEE NET UPLIFT PLAN ON SHEET S1.1 PROVIDE UPLIFT BRIDGING.
- SJ11 ALL SPRINKLER AND ROOF DRAIN PIPES MUST BE SUPPORTED NO FURTHER THAN 3" FROM THE JOIST TOP CHORD PANEL POINTS. THIS WILL BE STRICTLY ENFORCED. WHEN PIPES ARE PERPENDICULAR TO JOISTS, HANGERS SHALL BE PROVIDED EVERY OTHER JOIST (APPROX 10'-0" OC). WHEN PIPES ARE PARALLEL TO JOISTS, TWO CASES EXIST: FIRST, PIPES THAT ARE 4" AND LESS MAY BE SUPPORTED BY A SINGLE JOIST WITH HANGERS NOT TO EXCEED 10'-0" OC. SECOND, PIPES THAT ARE LARGER THAN 4" MUST BE CENTERED BETWEEN TWO JOISTS AND SUPPORTED FROM L4x4x5/16 ANGLE BEARING ON JOIST TOP CHORD PANEL POINTS WITH SPACING NOT TO EXCEED 10'-0" OC. GENERAL CONTRACTOR SHALL COORDINATE THESE REQUIREMENTS WITH THE APPROPRIATE TRADES.
- SJ12 ALL ITEMS SUSPENDED FROM JOISTS (i.e. CATWALKS, BALCONIES, OPERABLE PARTITIONS, etc...) SHALL BE INSTALLED AFTER DEAD LOAD HAS BEEN APPLIED.

JOIST BRIDGING NOTES:

- JB1 BRIDGING STANDARD WITH THE MANUFACTURER AND COMPLYING WITH THE STEEL JOIST INSTITUTE STANDARD SPECIFICATIONS LOAD TABLES & WEIGHT TABLES OF THE LATEST ADOPTION SHALL BE USED FOR BRIDGING ALL JOISTS FURNISHED BY THE MANUFACTURER. POSITIVE ANCHORAGE SHALL BE PROVIDED AT THE ENDS OF EACH BRIDGING ROW AT BOTH TOP AND BOTTOM CHORDS.
- JB2 FOR "K" AND "LH" SERIES JOISTS HORIZONTAL BRIDGING IS RECOMMENDED FOR SPANS UP TO AND INCLUDING 60 FEET EXCEPT WHERE THE STEEL JOIST INSTITUTE STANDARD SPEC LOAD TABLES & WEIGHT TABLES REQUIRE BOLTED DIAGONAL BRIDGING FOR ERECTION STABILITY.
- JB3 "LH" AND "DLH" SERIES JOISTS EXCEEDING 60 FEET IN LENGTH SHALL HAVE BOLTED DIAGONAL BRIDGING FOR ALL ROWS.
- JB4 REFER TO S3 SECTION 6 IN THE "K" SERIES SPECIFICATIONS AND SECTION 105 IN THE "LH" AND "DLH" SERIES SPECIFICATIONS FOR ERECTION STABILITY REQUIREMENTS
- JB5 REFER TO APPENDIX E FOR OSHA STEEL JOIST ERECTION STABILITY REQUIREMENTS.
- JB6 HORIZONTAL BRIDGING SHALL CONSIST OF CONTINUOUS HORIZONTAL STEEL MEMBERS. THE 1/4" RATIO FOR HORIZONTAL BRIDGING SHALL NOT EXCEED 300.
- JB7 DIAGONAL CROSS BRIDGING CONSISTING OF ANGLES OR OTHER SHAPES CONNECTED TO THE TOP AND BOTTOM CHORDS, OF "K", "LH", AND "DLH" SERIES JOISTS SHALL BE USED WHEN REQUIRED BY THE APPLICABLE STEEL JOIST INSTITUTE STANDARD SPECIFICATIONS LOAD TABLES AND WEIGHT TABLES OF LATEST ADOPTION.
- JB8 DIAGONAL BRIDGING, WHEN USED, SHALL HAVE AN 1/4" RATIO < 200.

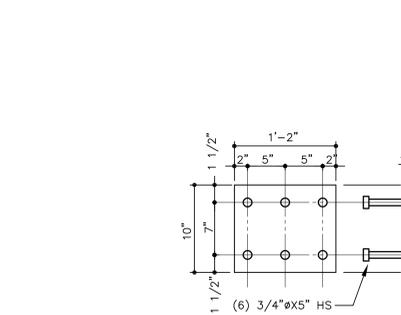
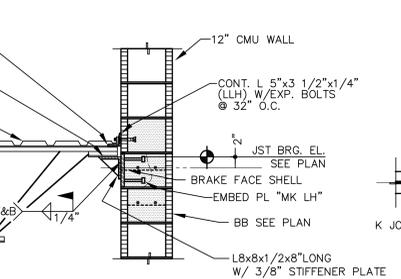


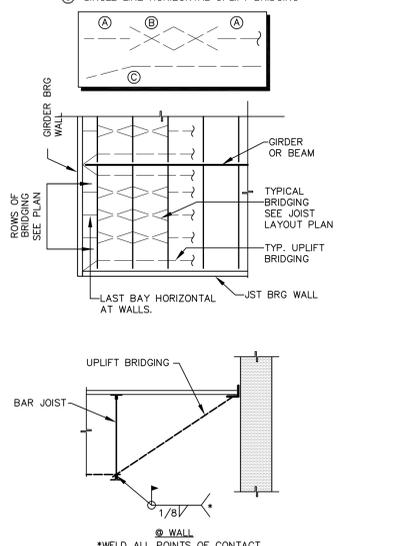
PLATE MK/LH DETAIL
SCALE: N.T.S.



1 **SECTION • JOIST BRG • SKEW**
N.T.S.

JOIST BRIDGING NOTES CONTINUED:

- JB9 WHEN BOLTED DIAGONAL ERECTION BRIDGING IS REQUIRED, THE FOLLOWING SHALL APPLY:
 - THE BRIDGING SHALL BE INDICATED ON THE JOIST LAYOUT PLAN.
 - THE JOIST LAYOUT PLAN SHALL BE THE EXCLUSIVE INDICATOR FOR THE PROPER PLACEMENT OF THIS BRIDGING.
 - SHOP INSTALLED BRIDGING CLIPS, OR FUNCTIONAL EQUIVALENT SHALL BE PROVIDED WHERE THE BRIDGING BOLTS TO THE STEEL JOISTS.
 - THE TWO PIECES OF BRIDGING ARE ATTACHED TO THE STEEL JOIST BY A COMMON BOLT, THE NUT THAT SECURES THE FIRST PIECE OF BRIDGING SHALL NOT BE REMOVED FROM THE BOLT FOR THE ATTACHMENT OF THE SECOND PIECE.
 - ERIDGING ATTACHMENTS SHALL NOT PROTRUDE ABOVE THE TOP CHORD OF THE STEEL JOISTS.
- JB10 PROVIDE UPLIFT BRIDGING AT FIRST BOTTOM CHORD PANEL POINT EACH END OF JOIST. REFER TO SECTION A/S1.1 FOR UPLIFT BRIDGING CONNECTION DETAILS.
- JB11 DO NOT WELD BRIDGING TO JOIST WEB MEMBERS. DO NOT HANG ANY MECHANICAL, ELECTRICAL, PLUMBING, ETC FROM BRIDGING.
- JB12 BRIDGING LEGEND FOR PLAN BELOW:
 - (A) HORIZONTAL BRIDGING ATTACHED TO TOP AND BOTTOM CHORD
 - (B) BOLTED OR WELDED CROSS BRIDGING AS SHOWN
 - (C) SINGLE LINE HORIZONTAL UPLIFT BRIDGING



A BRIDGING DETAILS AND PLAN

STRUCTURAL STEEL

- S1 GENERAL CONTRACTOR SHALL ENGAGE A CERTIFIED TESTING AGENCY TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
- S2 STEEL WORK SHALL CONFORM TO THE AISC "SPECIFICATIONS" FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL BUILDINGS, LATEST EDITION.
- S3 STEEL TUBING SHALL CONFORM TO ASTM A500 GRADE B.
- S4 STRUCTURAL WF SHAPES SHALL BE TO ASTM A992 F_y=50KSI, UNLESS NOTED OTHERWISE. ALL OTHER SHAPES SHALL BE A36-08. ALL STRUCTURAL STEEL SHALL BE DOMESTICALLY PRODUCED.
- S5 BRACE AND MAINTAIN ALL STEEL IN ALIGNMENT UNTIL OTHER PARTS OF CONSTRUCTION NECESSARY FOR PERMANENT SUPPORT ARE COMPLETED. CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLING TEMPORARY SHORING AS REQUIRED FOR THE STABILITY OF THE STEEL FRAME UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN COMPLETED AND BUILDING IS ENCLOSED.
- S6 ALL WELDING SHALL CONFORM TO THE REQUIREMENTS OF "THE STANDARD CODE FOR WELDING IN BUILDING CONSTRUCTION" OF THE AMERICAN WELDING SOCIETY. WELDING ELECTRODES SHALL BE E70XX-LOW HYDROGEN FOR SHIELD AND METAL ARC WELDING.
- S7 GROUT FOR COLUMN BASE PLATES AND PRESET BEARING PLATES SHALL BE NON-SHRINK, NON-METALLIC GROUT. (5000 PSI MIN)
- S8 SUBMIT SHOP DRAWINGS INDICATING ALL SHOP AND ERECTION DETAILS INCLUDING PROFILES, SIZES, SPACING AND LOCATIONS OF STRUCTURAL MEMBERS, CONNECTION ATTACHMENTS, FASTENERS, LOADS AND TOLERANCES.
- S9 ALL WELDED CONN. SHALL BE 1/4" FILET ALL AROUND, UNO. ALL BOLTED CONN. SHALL BE 3/4" A325 BOLTS, UNO.
- S10 STEEL PAN STAIRS SHALL BE DESIGNED BY THE FABRICATOR AND SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE SAME STATE AS PROJECT LOCATION. DESIGN FOR 100 PSF LIVE LOAD.
- S11 ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED.

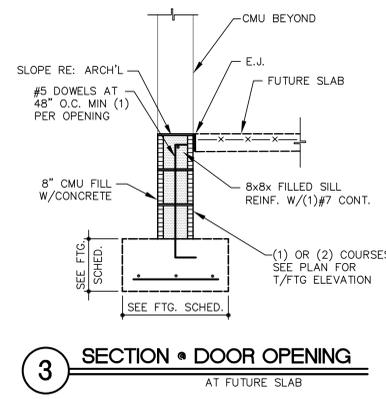
WALL SERVICE WIND PRESSURES

AREA (FT ²)	ZONE 4 (PSF)	ZONE 5 (PSF)
10	±39.2 PSF	±48.2 PSF
20	±37.6 PSF	±45.0 PSF
35	±36.3 PSF	±42.4 PSF
50	±35.4 PSF	±40.8 PSF
75	±34.5 PSF	±38.9 PSF
100	±33.8 PSF	±37.6 PSF
150	±32.9 PSF	±35.7 PSF
350	±31.0 PSF	±31.8 PSF

- NOTE 1: WIND PRESSURE SHOWN ARE SERVICE PRESSURES
- NOTE 2: 6" - 3" (WITHIN 4' DISTANCE FROM BUILDING OUTSIDE CORNERS USE ZONE 5, EVERYWHERE USE ZONE 4)

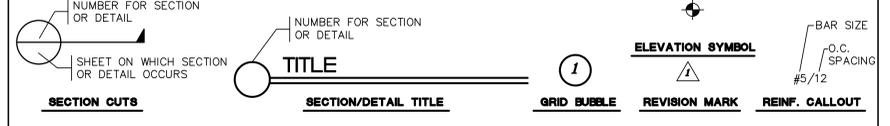
STRUCTURAL ABBREVIATIONS

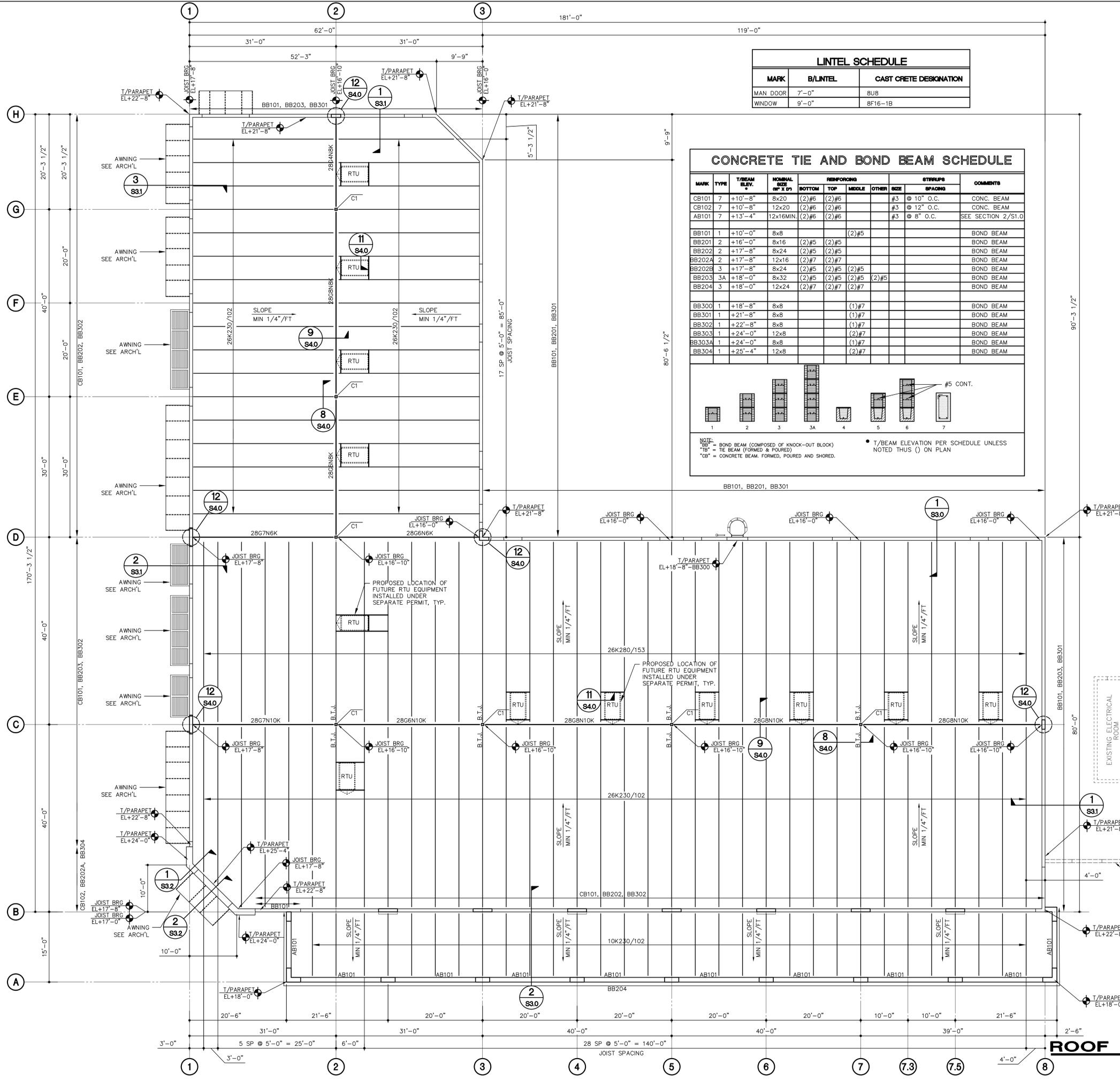
AB	ANCHOR BOLT	JST	JOIST
ABV	ABOVE	K	KNOCK OUT
A.C.I.	AMERICAN CONCRETE INSTITUTE	KG	LONG
ADD'L	ADDITIONAL	LL	LIVE LOAD
AGGR	ABOVE FINISH FLOOR AGGREGATE	LLH	LONG LEG HORIZONTAL
A.I.S.C.	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	LLV	LONG LEG VERTICAL
A.I.S.I.	AMERICAN IRON AND STEEL INSTITUTE	LNTL	LINTEL
AL	ALUMINUM	LONG	LONG SLOTTED HOLES
ALT	ALTERNATE	LP	LOW POINT
ARCM	ARCHITECT(URAL)	MAS	MASONRY
A.S.T.M.	AMERICAN SOCIETY OF TESTING MATERIALS	MAX	MAXIMUM
A.W.S.	AMERICAN WELDING SOCIETY	MBM	METAL BUILDING MFR
B/	BOTTOM OF	MC	MOMENT CONNECTION
BB	BOND BEAM	MCJ	MASONRY CONTROL JT
BLW	BELOW	MECH	MECHANICAL
BM	BEAM	MEZZ	MEZZANINE
BOT	BOTTOM	MFR	MANUFACTURE(ER)
BRG	BRIDGING	MIN	MINIMUM
BRG	BEARING	MO	MASONRY OPENING
BRK	BRICK	MS	METAL STUD
BS	BOTH SIDES	MTL	METAL
BTW	BETWEEN	NS	NEAR SIDE
C/C	CENTER TO CENTER	NTS	NOT TO SCALE
CANT	CANTILEVER	OC	ON CENTER
CB	CONCRETE BEAM	OF	OUTSIDE DIAMETER
CC	CONCRETE COL	OP	OUTSIDE FACE
CIP	CAST IN PLACE	OPNG	OPENING
CJ	CONSTRUCTION JOINT OR CONTROL JOINT	OPP	OPPOSITE
CLR	CLEAR(ANCE)	PAF	POWDER ACTUATED FASTENERS
CM	CONCRETE MASONRY UNIT	PERP	PERPENDICULAR
CMU	CONCRETE MASONRY UNIT	PC	PRECAST
CONC	CONCRETE	PL	PLATE
CONN	CONNECTION	PLYWD	PLYWOOD
CONT	CONTINUOUS	REQ	REQUIRED
CONTR	CONTRACTOR	REQ'D	REQUIRED
CSK	COUNTER SINK	RF	ROOF
CTR	CENTER	RW	RETAINING WALL
CTR'D	CENTERED	SCH	SCHEDULE
DIA	DIAMETER	SE	SLAB EDGE
DL	DEAD LOAD	SECT	SECTION
DN	DOWN	SHT	SHEET
DTL	DETAIL	SM	SIMILAR
DWG	DRAWING	SAWJCT	SAWCUT JOINT
DWL	DOWEL	SJ	STEEL JOIST INSTITUTE
EA	EACH END	SLO	SLOPE
EE	EACH FACE	SP	SPACE(S)
EJ	EXPANSION JOINT	SPCS	SPECIFICATIONS
ENG	ENGINEER	SQ	SQUARE
EL	ELEVATION	SS	STAINLESS STEEL
EQ	EQUAL	STD	STANDARD
EQ SP	EQUAL SPACE(S) (ING)	STL	STEEL
ES	EACH SIDE	STR	STRENGTH
EA	EACH END	STR	STRUCTURAL
EXT	EXTERIOR	SW	STEEL WALL
F	FACE OF	SYMM	SYMMETRICAL
F/D	FLOOR DRAIN	SYP	SOUTHERN YELLOW PINE
FDN	FOUNDATION	TB	TIE BEAM
FIN	FINISH	T&B	TOP & BOTTOM
FL	FLOOR	TDS	TURN DOWN SLAB
FLG	FLANGE	THK	THICK
FS	FAR SIDE	THNS	THINNESS
FT	FOOT	TOPG	TOPPING
FTG	FOOTING	TYP	TYPICAL
GA	GAGE, GAUGE	TYP	TYPICAL
GALV	GALVANIZED	UNO	UNLESS NOTED OTHERWISE
GB	GRADE BEAM	VERT	VERTICAL
GC	GENERAL CONTRACTOR	WF	WALL FOOTING
GLB	GLU-LAM BEAM	WO	WINDOW OPENING (MASONRY)
GR	GRADE	WP	WATERPROOF
HC	HOLLOW CORE	WS	WATERSTOP
HOK	HOOK	WWF	WELDED WIRE FABRIC
HORIZ	HORIZONTAL	W/	WITH
HP	HORIZONTAL		
HS	HEADED STUD		
ID	INSIDE DIAMETER		
IF	INSIDE FACE		
INT	INTERIOR		



3 **SECTION • DOOR OPENING**
AT FUTURE SLAB

SYMBOL LEGEND





LINTEL SCHEDULE

MARK	B/LINTEL	CAST CRETE DESIGNATION
MAN DOOR	7'-0"	BU8
WINDOW	9'-0"	8F16-1B

CONCRETE TIE AND BOND BEAM SCHEDULE

MARK	TYPE	T/B ELEV.	NOMINAL SIZE (W' X D')	REINFORCING				STIRRUPS	COMMENTS
				BOTTOM	TOP	MIDDLE	OTHER		
CB101	7	+10'-8"	8x20	(2)#6	(2)#6		#3 @ 10" O.C.	CONC. BEAM	
CB102	7	+10'-8"	12x20	(2)#6	(2)#6		#3 @ 12" O.C.	CONC. BEAM	
AB101	7	+13'-4"	12x16MIN.	(2)#6	(2)#6		#3 @ 8" O.C.	SEE SECTION 2/S1.0	
BB101	1	+10'-0"	8x8			(2)#5		BOND BEAM	
BB201	2	+16'-0"	8x16	(2)#5	(2)#5			BOND BEAM	
BB202	2	+17'-8"	8x24	(2)#5	(2)#5			BOND BEAM	
BB202A	2	+17'-8"	12x16	(2)#7	(2)#7			BOND BEAM	
BB202B	3	+17'-8"	8x24	(2)#5	(2)#5	(2)#5		BOND BEAM	
BB203	3A	+18'-0"	8x32	(2)#5	(2)#5	(2)#5		BOND BEAM	
BB204	3	+18'-0"	12x24	(2)#7	(2)#7			BOND BEAM	
BB300	1	+18'-8"	8x8			(1)#7		BOND BEAM	
BB301	1	+21'-8"	8x8			(1)#7		BOND BEAM	
BB302	1	+22'-8"	8x8			(1)#7		BOND BEAM	
BB303	1	+24'-0"	12x8			(2)#7		BOND BEAM	
BB303A	1	+24'-0"	8x8			(1)#7		BOND BEAM	
BB304	1	+25'-4"	12x8			(2)#7		BOND BEAM	

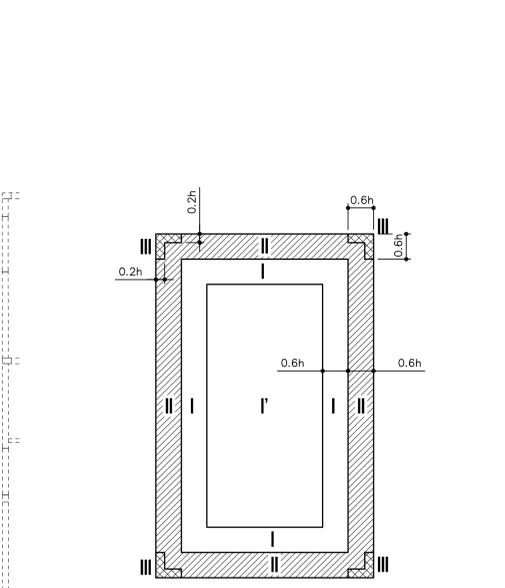


NOTE: BB = BOND BEAM (COMPOSED OF KNOCK-OUT BLOCK)
 TB = TIE BEAM (FORMED & POURED)
 CB = CONCRETE BEAM, FORMED, POURED AND SHORED.

- #### ROOF FRAMING NOTES:
- ROOF ASSEMBLY TO BE TPO ROOFING WITH INSULATION OVER 1.5", 20GA TYPE "B" GALV. G90 METAL ROOF DECK CONTINUOUS OVER 3 OR MORE SPANS. ATTACH DECK TO ALL SUPPORTS WITH 5/8" PUDDLE WELDS ON SDI 36/9 WELD PATTERN. ATTACH DECK AT PERIMETER W/ 5/8" PUDDLE WELDS @ 6" O.C. USE #12 @ 7" o.c. TEK SIDELAP FASTENERS.
 - TOP OF PARAPET EL. SEE PLAN
 - SEE STRUCTURAL NOTE SHEET (S1.0) FOR BRIDGING REQUIREMENTS FOR JOIST & JOIST GIRDERS.
 - SEE SECTIONS FOR FRAMING DETAILS.
 - PROVIDE SPECIAL JOISTS AND AC FRAMING (SEE DETAIL A/S4.3) AT ALL ROOF TOP A/C UNITS. REFER TO MECHANICAL HVAC PLAN FOR LOCATION AND WEIGHTS OF EACH UNIT. DESIGN JOISTS FOR ADDITIONAL WEIGHT OF ROOF TOP UNITS.
 - SEE PLAN FOR JOIST BEARING ELEVATIONS.
 - SEE THIS SHEET FOR ROOF DESIGN LOADS AND FOR UPLIFT DIAGRAM.

FLAT ROOF DESIGN LOADS

BAR JOIST LIVE LOAD GRID C-D	BAR JOIST LIVE LOAD REST	GIRDER LIVE LOAD	SUPERIMPOSED TO DEAD LOAD	SUPERIMPOSED BC DEAD LOAD
30 PSF	20 PSF	20 PSF	20 PSF	5 PSF



SERVICE ASD ROOF UPLIFT PLAN

ZONE	ROOFING	JOISTS 320SF	JOISTS 633SF	GIRDERS
I'	-36.2 PSF	-24.1 PSF	-19.4 PSF	-17.2 PSF
I	-62.9 PSF	-42.2 PSF	-39.5 PSF	-17.2 PSF
II	-83.0 PSF	-56.3 PSF	-52.9 PSF	-31.9 PSF
III	-83.0 PSF	-56.3 PSF	-52.9 PSF	-42.7 PSF

* JOISTS AND GIRDER UPLIFT SHOWN IS GROSS UPLIFT. FOR JOISTS WITH SPANS EQUAL TO OR GREATER THAN 46 FEET, AN UPWARD OR DOWNWARD WIND PRESSURE OF 16PSF DOES NOT NEED TO BE CONSIDERED IN THE DESIGN COMBINATIONS AS THE TRIBUTARY AREA IS GREATER THAN 700 SF. (ASCE-7 SECT. 30.2.3)

ROOF FRAMING PLAN

SCALE: 1/8"=1'-0"

PROPOSED BLDG 'P'

MARK	DESCRIPTION	DATE

WVA ARCHITECTURE, LLC
 ARCHITECTURE / PLANNING

170 West Hibiscus Blvd, Suite 308
 Fort Lauderdale, Florida 33301
 Tel: 954-752-8111 | 954-3482
 Fax: 954-752-5258

SEAL:

 CONSULTANT

SHADOWWOOD (EDENS), LLC
 500 E. Broward Blvd.
 Suite #1620
 Fort Lauderdale, Florida 33301
 Ph: 954-670-8990
 Fax: 954-670-8991

OWNER REPRESENTATION

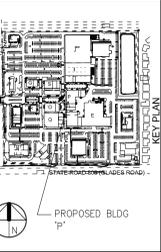
SHADOWWOOD SQUARE BUILDING 'P' SHELL DRAWINGS
 Palm Beach County, Florida

STRUCTURAL ROOF FRAMING PLAN

JOB NUMBER	JSG 25014
SCALE	1/8"=1'-0"
PERMIT DATE	
ISSUE DATE	05/02/25
BID DATE	

DRAWN BY: S.G.
 CHECKED BY: P.S.
 DISCIPLINE: STRUCTURAL
 PLAN TYPE:

S21



MARK	DESCRIPTION	DATE

WWA ARCHITECTURE, LLC
ARCHITECTURE / PLANNING

170 West Hibiscus Blvd, Suite 308
Orlando, Florida 32804
P: 407-752-5258

SEAL:

JOHNSON GROUP
STRUCTURAL GROUP

P.E. #0008863
P.O. BOX 1000
170 West Hibiscus Blvd, Suite 308
Orlando, Florida 32804
P: 407-752-5258
F: 407-752-5259
www.johnsongroup.com

SHADOWWOD (EDENS), LLC
OWNER IDENTIFICATION

500 E. Broward Blvd.
Suite #1620
Fort Lauderdale, Florida 33301
Ph: 954-670-8990
Fax: 954-670-8991

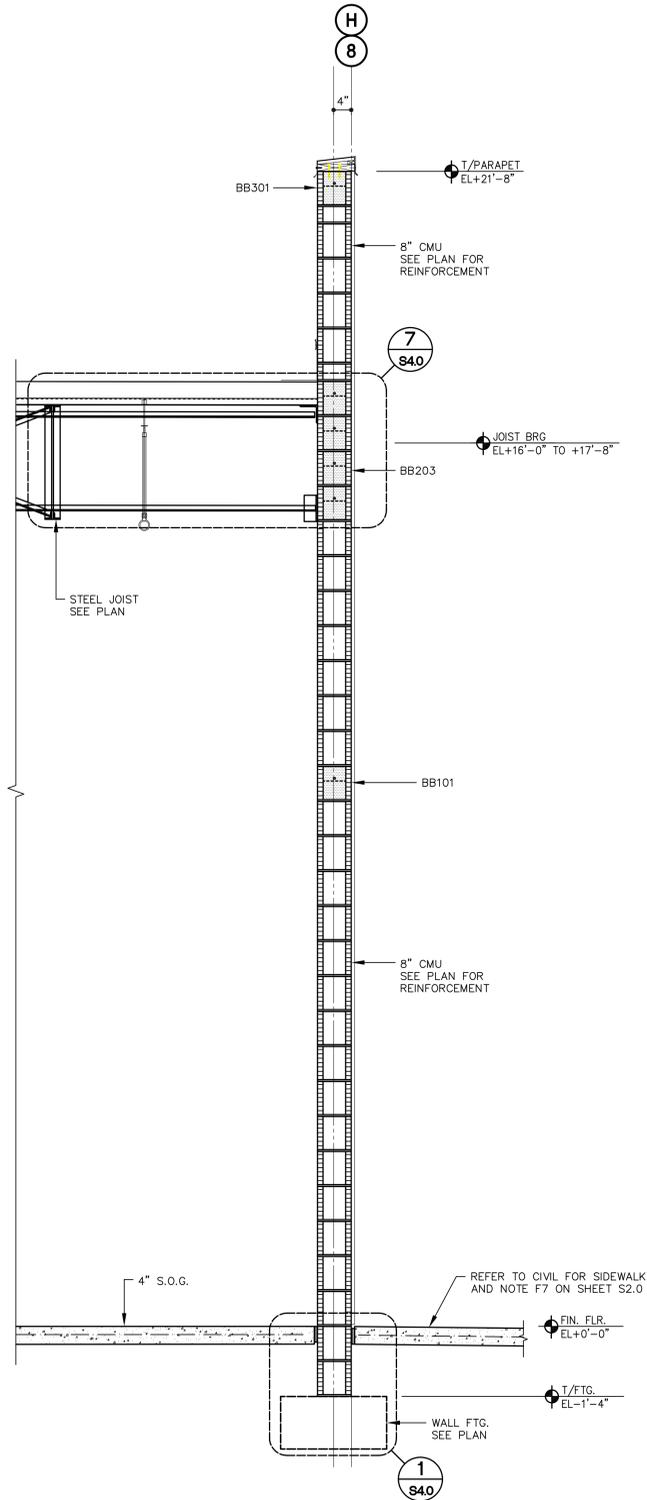
SHADOWWOOD SQUARE BUILDING 'P'
SHELL DRAWINGS

Palm Beach County, Florida

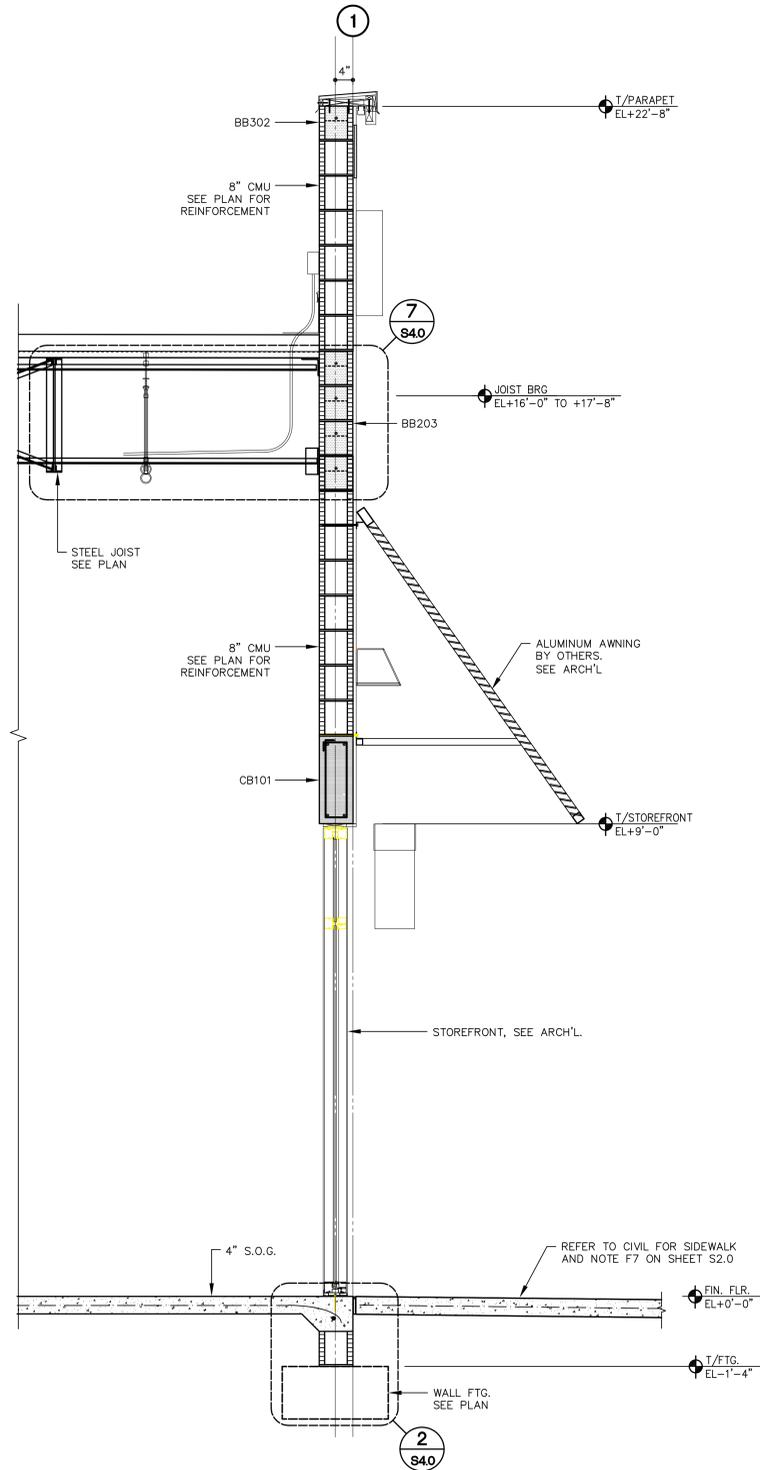
STRUCTURAL BUILDING SECTIONS

JOB NUMBER	JSG 25014
SCALE	3/4"=1'-0"
PERMIT DATE	
ISSUE DATE	05/02/25
BID DATE	

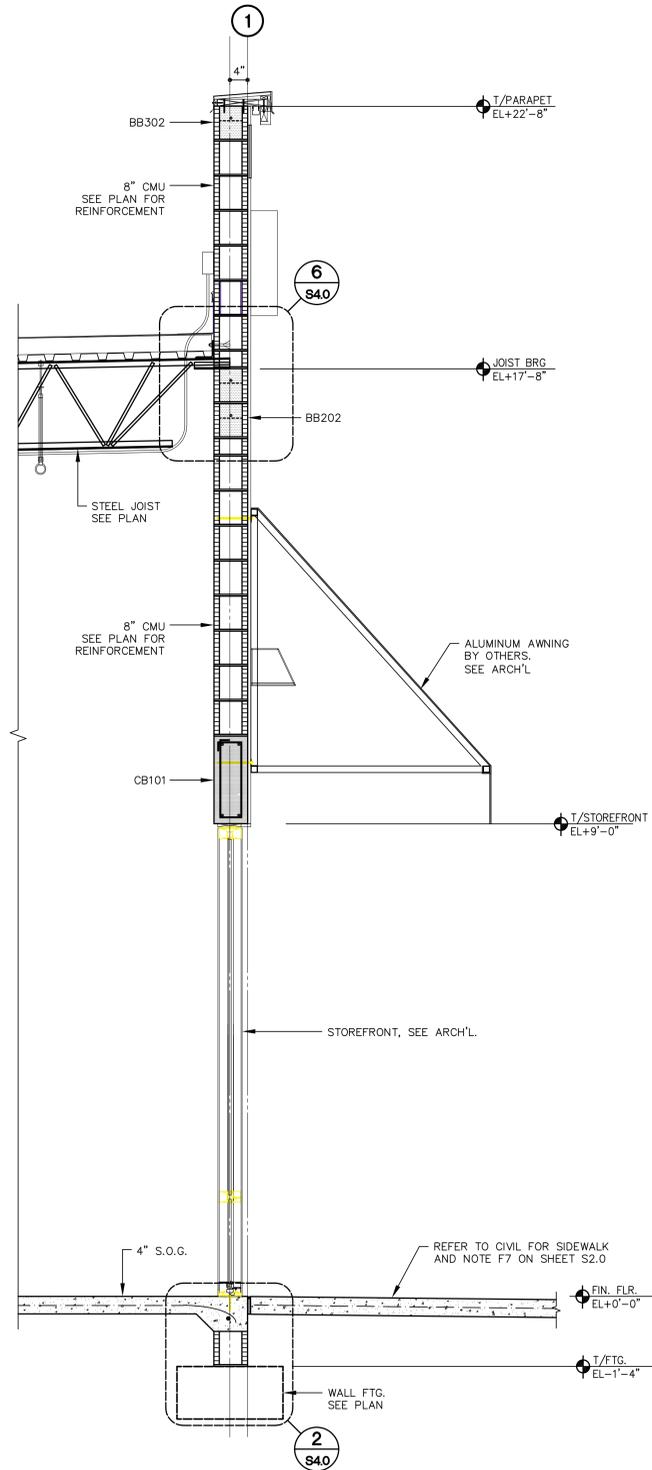
DRAWN BY	S.G.
CHECKED BY	P.S.
DISCIPLINE	STRUCTURAL
PLAN TYPE	
SHEET NUMBER	S31



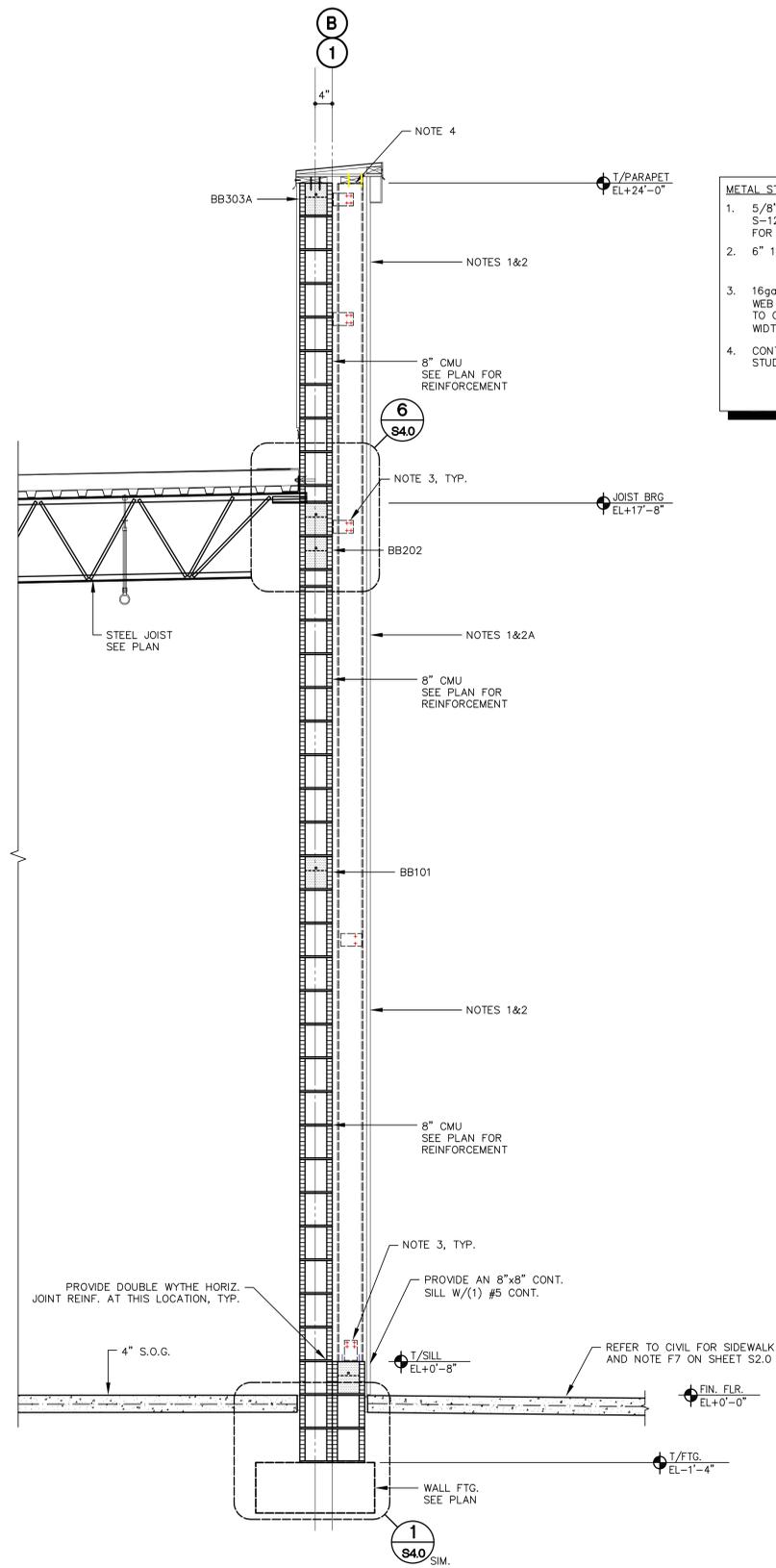
1 BUILDING SECTION
SCALE: 3/4"=1'-0"



2 BUILDING SECTION
SCALE: 3/4"=1'-0"



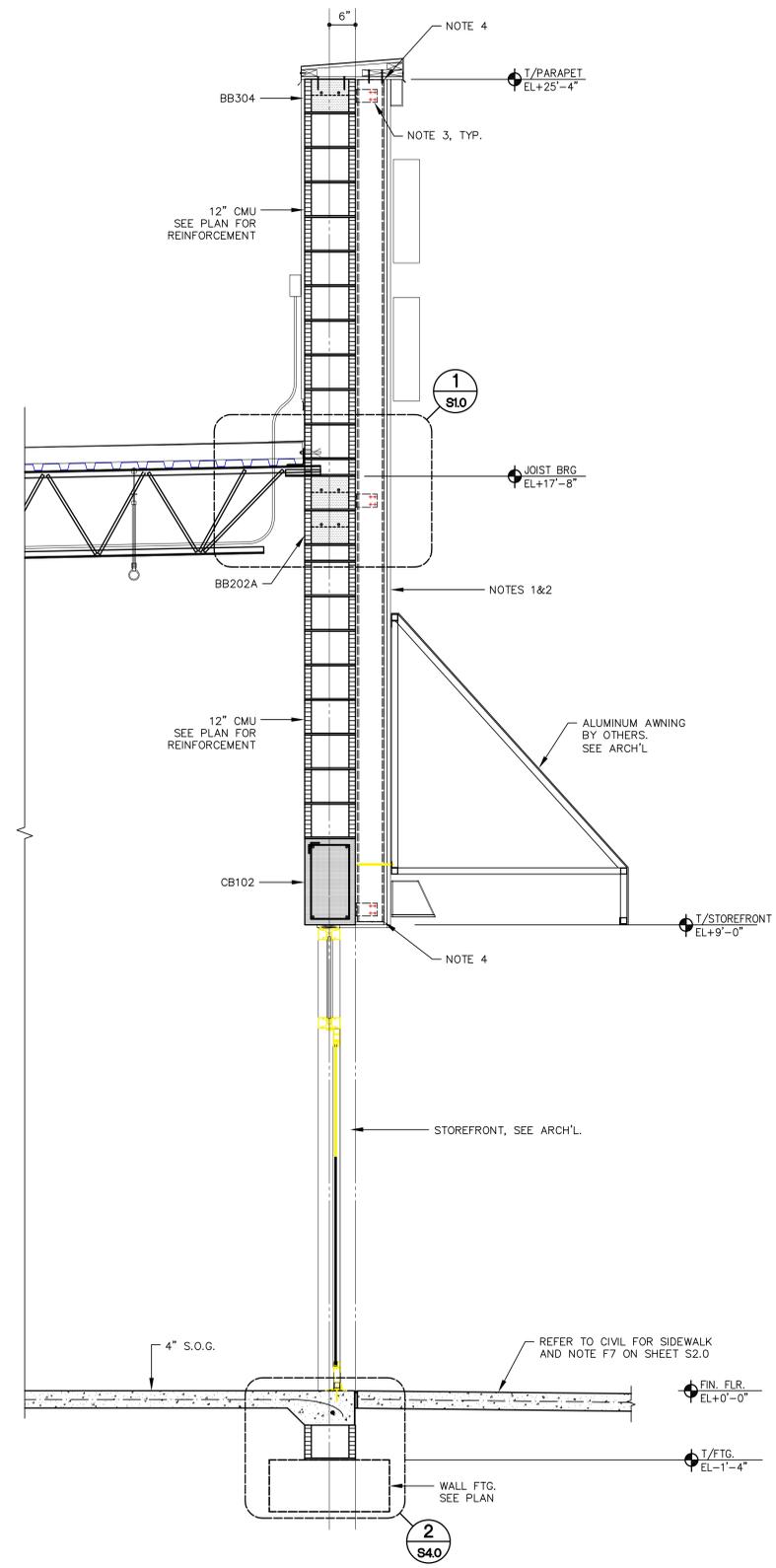
3 BUILDING SECTION
SCALE: 3/4"=1'-0"



1 BUILDING SECTION
SCALE: 3/4"=1'-0"

METAL STUD (50KSI) AND PLYWOOD CONNECTIONS:

- 5/8" THK FRT PLYWOOD SHEATHING. PROVIDE #8-1 1/2" S-12 ROCK ON SCREWS AT 8" O.C. ALONG STUD SPAN FOR PLYWOOD TO MTL STUD FLANGE CONN.
- 6" 18ga. CEE MTL STUDS AT 16" O.C.
- 16ga. CLIP ANGLE W/(4)#10 TEKS FOR CLIP TO MTL STUD WEB CONN. AND (4) 1/4"x1-3/4" TAPCONS FOR CLIP TO CMU WALL CONN. MAKE CLIP ANGLE FULL WIDTH/HEIGHT OF MTL STUDS.
- CONT. 18ga. MTL ANGLE. USE (2)#10 TEKS FOR ANGLE TO STUD CONN.



2 BUILDING SECTION
SCALE: 3/4"=1'-0"



MARK	DESCRIPTION	DATE

WWA ARCHITECTURE, LLC
ARCHITECTURE / PLANNING

170 West Hibiscus Blvd, Suite 308
Fort Lauderdale, Florida 33301
Tel: 954-752-8411 Fax: 954-752-8442
P: 954-752-8288

SEAL:

PERDIO RAFAEL SALAMUN
LICENSE
NO. 70346
STATE OF FLORIDA
PROFESSIONAL ENGINEER

JOHNSON STRUCTURAL GROUP
P.E. #0008893
1700 N.W. 10th Ave., Suite 1000
Fort Lauderdale, FL 33304
(954) 982-8888 (954) 982-8899
Fax: 954-982-8899
www.johnsonstructural.com

SHADOWWOOD (EDENS), LLC
500 E. Broward Blvd.
Suite #1620
Fort Lauderdale, Florida 33301
Ph: 954-670-8990
Fax: 954-670-8991

SHADOWWOOD SQUARE BUILDING 'P' SHELL DRAWINGS
Palm Beach County, Florida

STRUCTURAL BUILDING SECTIONS

JOB NUMBER	JSG 25014
SCALE	3/4"=1'-0"
PERMIT DATE	
ISSUE DATE	05/02/25
BID DATE	

DRAWN BY	S.G.
CHECKED BY	P.S.
DISCIPLINE	STRUCTURAL
PLAN TYPE	
SHEET NUMBER	S3.2

