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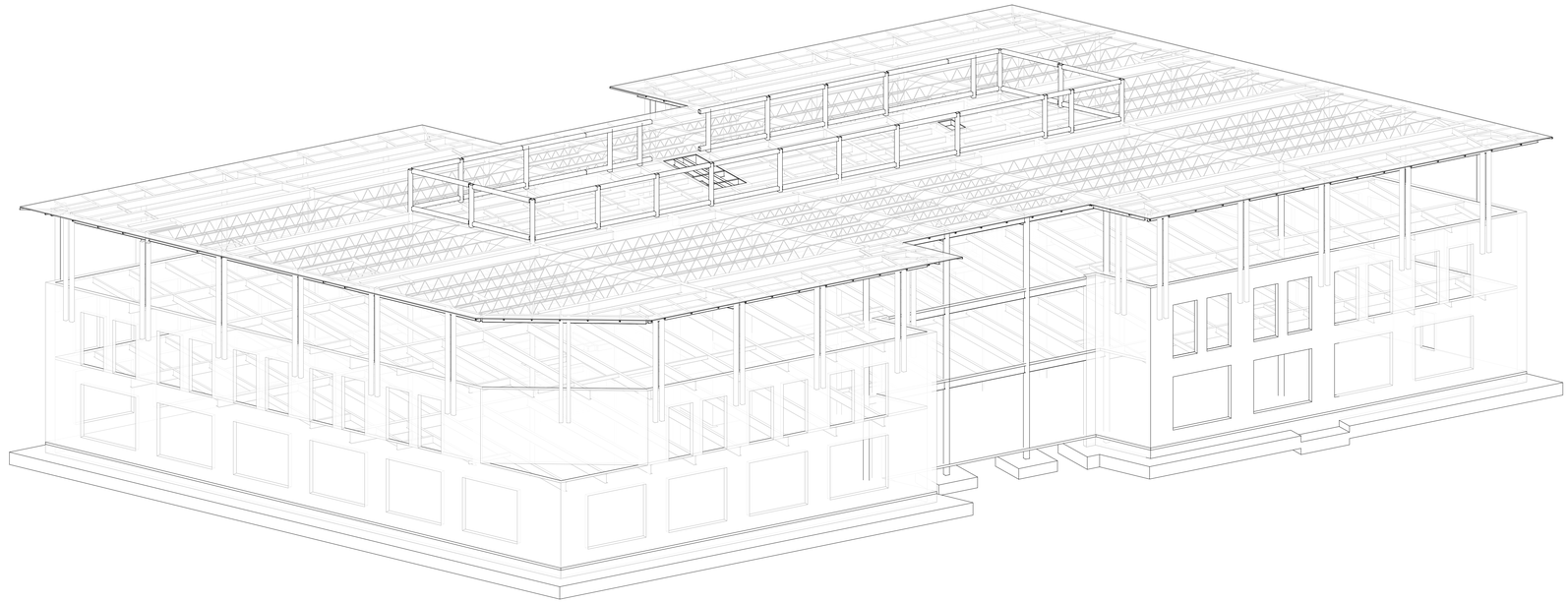

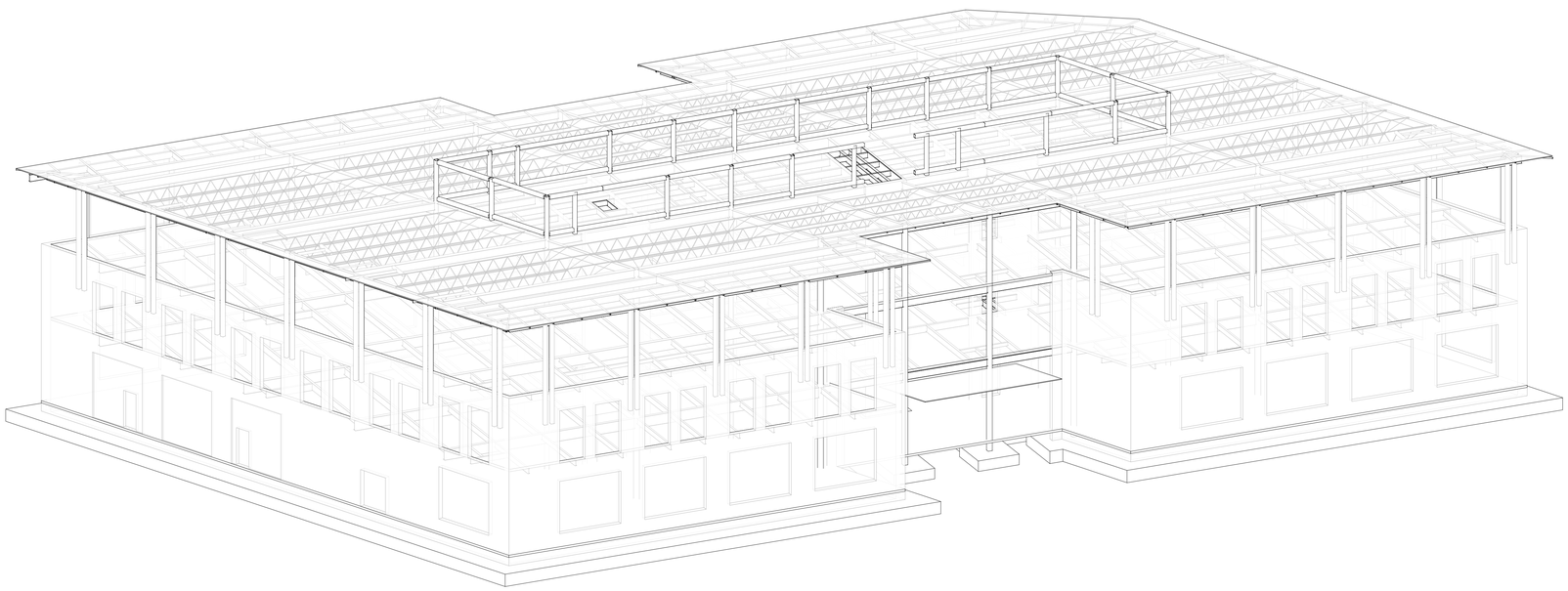

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Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
www.mcsal.com FL CA#26616

Straticon
Received:
06.12.2026

Δ	DATE	SUBMISSION
	06/12/2026	PERMIT SET

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PROFESSIONAL ENGINEER

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Elie El Zghayar, PhD, PE on **06/10/2026**
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TAMBONE COMPANIES

SEACOAST BANK HEADQUARTERS

815 SOUTH COLORADO AVE
STUART, FL 34994

PROJECT NO:
240089.00

COVER SHEET

SHEET NUMBER:
S000

THE LINE SHOWN ABOVE IS 3" LONG ON ORIGINAL ARCH D SHEETS ORIGINAL SCALE

GENERAL:

1. All work shall comply with the requirements of the 2020 Florida Building Code (7th Edition) (IBC 2018).
2. All dimensions and conditions shall be field verified prior to commencing work. Where dimensions and elevations of existing construction could affect the new construction, it is the Contractor's responsibility to make field measurements in time for their incorporation in the Shop Drawings. The Architect and Engineer shall be notified of any discrepancies that may exist.
3. Refer to architectural drawings for floor elevations, slopes, locations of depressed floor areas, and floor openings. The Contractor shall compare the structural drawings with the architectural drawings and report any discrepancy to the Architect and Engineer prior to construction.
4. Principal openings through the framing are shown on these drawings. The Contractor shall examine the structural, architectural and MEP drawings for the required openings and verify the size and location of all openings with the MEP Contractor. Provide all openings required by the Mechanical, Electrical, or Plumbing trades, whether shown or not shown in the structural drawings, however, any deviation from the openings shown on the structural drawings shall be brought to the Engineer's attention for review.
5. The Contractor shall furnish and place all supports, temporary and permanent, whether shoring, bracing, needling, underpinning, or sheet piling, necessary to brace existing walls or framing to remain, such that horizontal or vertical settlement concerns at nearby adjacent structures are mitigated. Keep temporary supports in place until permanent supports are installed. Design of these supports shall be by a Structural Engineer registered in the state in which the project is permitted and under the employ of the Contractor.
6. Alternate connection details may be approved if such details are submitted to the Engineer for review and acceptance is granted. The Engineer shall be the sole judge of acceptability and the Contractor's bid shall anticipate the use of those specific details shown on the drawings. Design of these proposed alternate connection details shall be by a Structural Engineer registered in the state in which the project is permitted and under the employ of the Contractor.
7. The safety of adjacent structures, property, workers, and the public, as affected by the construction of this project is the sole responsibility of the Contractor.
8. All Contractors are required to examine the drawings and specifications carefully, visit the site and fully inform themselves as to all existing conditions and limitations, prior to agreeing to perform the work. Failure to visit the site and familiarize themselves with the existing conditions and limitations will in no way relieve the Contractor from furnishing any materials or performing any work in accordance with drawings and specifications without additional cost to the Owner.
9. Structural drawings may represent construction with a reference scale. Due to the inherent process of drawing development and presentation, not all work may be shown "exact" in that scale. The drawings shall not be 'scaled' to obtain any missing information or to interpret any information not specifically dimensioned for "exact" detailing or construction purposes.
10. The Contract Documents represent final conditions. Stability of the structure during construction, as well as means and methods that impose temporary loading conditions on the incomplete structure is the sole responsibility of the General Contractor. This responsibility includes, but is not limited to, the design and installation of any temporary supports, shoring, and/or bracing required for the safety and stability of the structure during construction.
11. Proprietary products specified within these drawings are identified as a basis-of-design for an established performance criteria. Product substitutions shall be submitted to the Structural Engineer for approval prior to the procurement of the affected work. The product substitution request should include, at a minimum, the following:
    A. Product to be substituted with clear reference to the intended use in the structural specifications and drawings.
    B. Reasons for substitution, including changes to the contract cost and schedule.
    C. Manufacturer technical product data sheets, certifications, and test reports for the new product.
    D. A published statement of the new product's compliance with the building code adopted by the local jurisdiction.
    E. A statement of the new product's compatibility with the affected portions of the work.

SUBMITTAL REVIEW:

1. Review of Contractor Submittals, including but not limited to Shop Drawings, Product Data, Manufacturer Cut Sheets, etc. shall be for the limited purpose of checking for conformance with the information given and the design concept expressed in the Contract Documents. Review of such submittals shall not be for the purpose of determining the accuracy and completeness of other information, including but not limited to dimensions, elevations, and quantities. The review shall not constitute approval of safety precautions or, unless otherwise stated in the Contract Documents, any construction means, methods, techniques, sequences or procedures. Approval of a specific item shall not indicate approval of an assembly of which the item is a component.
2. Review shall be limited only to those elements listed on the transmittal that are indicated to have been submit for approval, even if other elements are included (e.g. a fabricator's model or erection plan). Changes to previously reviewed elements shall not be reviewed unless requested as Revise and Resubmit or unless a design change bulletin creates a change to the element. In the event of a change bulletin, only those elements affected by that bulletin, and which are clearly indicated as a change by clouds or similar distinguishing means, shall be reviewed.
3. Submittals and shop drawings are the property of the Contractor.
4. Submittals will not be reviewed unless first reviewed by the Contractor and indicated as such on the submittal.
5. Submittals sent "For Record" or "For Field Use" shall not be not reviewed.
6. Structural Submittals pertain to elements designed by the Engineer of Record and shown in the Contract Documents.
7. Structural Delegated Design Submittals pertain to elements where the Contract Documents specifically require the Contractor to provide professional design services or certifications by a licensed design professional (Delegated Design Professional) related to systems, materials or equipment. The Contract Documents specify the appropriate performance and design criteria that such services must satisfy. Submittals and Shop Drawings shall bear the seal and signature of the Delegated Design Professional and will be reviewed for conformance with the design criteria. The Engineer of Record shall be entitled to rely upon the adequacy, accuracy and completeness of the services, certifications and approvals performed or provided by the Delegated Design Professional.
8. Non-Structural Component Submittals shall be reviewed for the effect on the structure only. Component loads imposed on the structure shall be indicated directly on the submittal. The design of the components shall not be reviewed. Structural review and comments shall be limited to the overall effect on the building structure. The Engineer of Record shall be entitled to rely upon the adequacy, accuracy and completeness of the information provided.

DESIGN LOADS:

GRAVITY LOADING TABLE

OCCUPANCY:	SUPERIMPOSED DEAD LOAD:	LIVE LOAD:	LIVE LOAD REDUCTION:
OFFICES	20 PSF	50 PSF NOTE 1	YES
LOBBIES, COMMON AND ASSEMBLY AREAS	30 PSF	100 PSF	NO
RESTROOMS	30 PSF	60 PSF	YES
MECHANICAL	20 PSF	100 PSF NOTE 2	NO
STAIRS	5 PSF NOTE 3	100 PSF 300LB CONC.	YES
STORAGE (LIGHT)	15 PSF	125 PSF	NO
ROOF	30 PSF	20 PSF	YES

GRAVITY LOADING TABLE NOTES:

1. In addition to the tabulated live load, a non-reducible 20 psf partition live load shall be included.
2. Where unit/equipment weights are indicated on the drawings, the live load in these areas shall be revised accordingly.
3. The Superimposed Dead Load identified for the stairs includes miscellaneous attachments to the stairs. It does not include the self-weight of the stair structural framing members nor the weight of any non-structural architectural toppings/finishes.
4. For joist uplift calculations, assume SDL = 6 psf and LL = 0 psf. Deck SW = 3 psf

TRANSIENT LOADING TABLE

DESCRIPTION:	REMARKS:
WIND DESIGN DATA:	
ULTIMATE WIND SPEED (V_{ULT})	162 MPH
NOMINAL WIND SPEED (V_{ASD})	126 MPH
RISK CATEGORY	II
WIND EXPOSURE	B
ENCLOSURE CATEGORY	ENCLOSED
INTERNAL PRESSURE COEFFICIENT (GC_{Pi})	+/- 0.18
RAIN DESIGN DATA:	
RAIN LOAD	12 PSF (NOTE 3)
RAIN INTENSITY (100-YEAR RETURN PERIOD)	4.5 IN/HR

TRANSIENT LOADING TABLE NOTES:

1. Design lateral loads for the main wind force resisting systems are in accordance with ASCE 7-16 Chapter 27.
2. Design lateral loads for building components and cladding are in accordance with ASCE 7-16 Chapter 30. Refer to the Components and Cladding Wind Pressures Table for additional information.
3. The Rain Load pressure identified herein is based on the following assumptions. These assumptions shall be verified based on the final selected roofing system, primary drainage system, and secondary drainage system.
    A. Depth of water on undeflected roof up to inlet of secondary drainage system: ds = 1"
    B. Additional depth of water above the inlet of the secondary drainage system resulting from net accumulation (hydraulic head): dh = 1 1/4"

MOVEMENT & DEFLECTIONS:

1. DRIFT: Provisions shall be made for the building cladding system to accommodate a horizontal displacement at the top of any story relative to the bottom of the same story of H/400, where "H" is the height of the respective story. This drift shall correspond to a 25-year mean recurrence interval wind event. The Delegated tilt-up panel engineer's shear wall design shall comply with these drifts limits.
2. FLOOR & ROOF DEFLECTIONS: Provisions shall be made for the building cladding system, flooring systems, ceilings, interior partitions, glass walls, and other components attached to the floor or roof structure to accommodate floor or roof deflections as follows, where "L" is the respective member span:
    A. Floor Members:
        - Live Load: L/360
        - Total Load: L/240
    B. Roof Members:
        - Live Load: L/240
        - Total Load: L/180
3. VIBRATION: The Structural design does not account for vibrations, or perception thereof, resulting from external sources.

SPECIALTY ENGINEERING:

1. Specialty engineered items shall be designed by an engineer licensed in the state in which the project is permitted. The delegated engineering services shall be retained by the Supplier of that specialty item. The Specialty Engineer shall prepare detailed shop drawings that bear the seal and signature of the Delegated Design Professional responsible for the Specialty Engineering design. Signed-and-sealed calculations shall be provided for each unique item that requires delegated design. The following design responsibilities shall be assigned to a Specialty Engineer.
    A. Cold-formed metal framing.
    B. Curtainwall and storefront systems and all connections to superstructure required for them.
    C. Entrance canopies and connections to superstructure.
    D. Steel stair and all connections required for them.
    E. Handrail, guardrail, stair rail, fences, gates, other misc. metals, etc.
    F. Mechanical tie-downs to superstructure, mechanical support curbs, mechanical duct support, piping supporters, hanging equipment supports, and all associated connections to the superstructure.
    G. Form work, shoring, reshoring, etc.
    H. Precast concrete elements including precast concrete façade panels, soffit beams, joists, double tees, inverted tees, spandrels columns, and all connections to superstructure required for them.
    I. Brick-tie lateral supports.
    J. Crane and other heavy equipment, including foundation and shoring. A Specialty Engineer shall be employed by the contractor for all work associated with tower cranes, including foundation, bracing, holes in structure shoring, etc. This also applies to any heavy equipment that is to be attached to/or loaded on the structure. Complete signed-and-sealed shop drawings and calculations shall be provided for all affected elements and submitted to the Engineer of Record.
    K. All material testing required by the special inspection plan and building codes such as concrete cylinder tests, slump tests, masonry grout test and any other special tests required.
2. The Specialty Engineer responsible for their particular design discipline shall inspect all phases of it to ensure compliance with the design.
3. All costs associated with the use of a Specialty Engineer shall be included within the Supplier's bid price for the procured item.
4. Where additional structural steel supports are required/requested by a delegated engineer and/or their supplier, the Contractor shall account for it in their budget and the design shall be provided by their delegated engineer. This is applicable to cold-formed steel, architectural precast, prefabricated elements, and all other instances where a delegated engineer is making connections back to the structure.

FOUNDATIONS:

1. Refer to the Geotechnical Report for soil preparation and foundation installation procedures. The foundation design is based on the recommendations provided in the following Geotechnical Report:
    a. Report No.: AACE 25-101 (Rev2)
    b. Report Title: Subsurface Soil Exploration and Geotechnical Engineering Evaluation Seacoast Bank New Headquarters, Stuart, Martin County, Florida
    c. Prepared By: Anderson Andre Consulting Engineers, Inc
    d. Dated: December 29, 2025
2. All footings shall bear on soil having a minimum net allowable bearing capacity of 6,000 psf. Allowable bearing pressures are increased by XXX% for combined gravity and lateral loads.
3. The Geotechnical Engineer shall specify the test procedures required to verify allowable bearing pressures. Structural Engineer of Record shall be sent the tests results for review.
4. The foundation design is based on the following soil parameters as provided in the referenced Geotechnical Report:
    1. Soil Moist Unit Weight = 110 pcf
    2. Base/Soil Friction Coefficient = 0.45
    3. Ultimate Passive Resistance (Equiv. Fluid Pressure) = 50 pcf
5. Center all foundations underneath their supported elements unless noted otherwise.
6. Do not place foundations in water. Provide continuous control of surface and underground water as required during construction such that the work is achieved in a dry condition. Ensure groundwater levels under adjacent structures are not lowered during construction.
7. Building retaining walls rely on the completed floor slabs for lateral support. If required by the construction sequence, temporarily brace all building retaining walls until all floor slabs laterally supporting the walls are in place and have attained at least 75% of their specified design strength. Design of temporary braces shall be by a Structural Engineer licensed in the state in which the project is permitted under the employ of the Contractor.
8. Where foundation elements are to have fill on both sides, fill each side simultaneously, maintaining a common elevation.
9. Unless noted otherwise, retaining walls specified within these plans have not been designed to resist hydrostatic pressure that results from water build-up on the retained face of these walls. Positive drainage shall be provided to alleviate hydrostatic pressure build-up on these walls. This design shall be a coarse-aggregate encased drainage system or other similar means and shall be provided by others.
10. Refer to the Geotechnical report for soil compaction requirements beneath foundations and slabs-on-grade. Verification that soil compaction requirements have been met shall be made by the Geotechnical Engineer or their designated representative before any foundations are placed.
11. All finished foundation excavations shall be inspected and approved by the Geotechnical Engineer or their designated representative before any foundations are placed.
12. Coordinate under floor drain requirements with architectural and MEP drawings, as well as the requirements of the Geotechnical Engineer.

TILT-UP CONCRETE:

1. All tilt-up concrete work shall comply with ACI 318-14 "Building Code Requirements for Structural Concrete", ACI 551 "Tilt-Up Concrete Construction" Standards, ACI 301-10 "Specifications for Structural Concrete", ACI 117-10 "Specification for Tolerances for Concrete Construction and Materials" and all Tilt-Up Concrete Association (TCA) Standards and Publications.
2. Refer to the Architectural plans and specifications for additional detailing requirements at all exposed Tilt-Up Concrete elements for reveals, finishes, colors and other aesthetic information. Where concrete finish is not specified, comply with the requirements of ACI 301. Refer to the Architectural drawings for concrete details at door and window openings, drip grooves and edges, wash slopes, reglets, finishes, and miscellaneous embedded items.
3. The Tilt-Up Concrete Panels shown in these drawings shall be designed by a Delegated Engineer licensed in the state in which the project is permitted. The Delegated Engineering design includes all components necessary for a complete load transfer from the superstructure elements to the foundation and supporting structure. This includes panel thickness, concrete compressive strength, panel reinforcement, supplemental reinforcement, and all other elements required for a complete load path. In addition, the Delegated Engineer responsible for the tilt-up panel design shall design the panels for lifting, erection, bracing and all other functions necessary to achieve the installation of the tilt-up concrete panels. The Tilt-Up Concrete supplier shall prepare detailed shop drawings that bear the seal and signature of the Delegated Design Professional responsible for the Tilt-Up Concrete Design. Signed-and-sealed calculations shall be provided for each unique condition which requires delegated design.
4. The Contractor shall submit shop drawings that include tilt-up panel geometry, reinforcement layout (including additional reinforcement layout required for lifting, erection, and bracing), embed placement layout, and other relevant design information established in the drawings and specifications.
5. Tilt-Up Concrete shall be fabricated and erected by a TCA-certified company with TCA-certified personnel.
6. Construction joints shall not be placed within tilt-up concrete panels.
7. Verify that all reinforcement and embedded items are properly installed and secured in accordance with the Contract Documents prior to concrete placement.
8. Grout used for bearing connections for tilt-up concrete elements is part of the load transfer mechanism for these members. Grout shall be placed and sufficiently cured prior to any loading being applied to these members. Grout shall be non-shrink, pre-mixed and have a minimum 3-day compressive strength of 5,000 psi. Where the compressive strength of the supported tilt-up element exceeds 5,000 psi, the grout strength shall equal or exceed that of the tilt-up element.
9. Chamfer all exposed edges of concrete members 3/4" unless noted otherwise on architectural drawings.
10. Concrete placement tolerances shall conform with ACI 117 for placement tolerances.
11. Refer to the Gravity Loading Table on sheet S001 for vertical loading requirements. Refer to the Lateral Load Diagram & Table on sheet S002 for lateral load requirements for the design of the tilt panels.

CAST-IN-PLACE CONCRETE:

1. All concrete work shall comply with ACI 318-14 "Building Code Requirements for Structural Concrete", ACI 301-10 "Specifications for Structural Concrete" and ACI 117-10 "Specification for Tolerances for Concrete Construction and Materials".
2. Provide normal weight concrete, unless noted otherwise, with sand and gravel aggregate complying with ASTM C33 and Type I or Type II Portland Cement in compliance with ASTM C150.
3. All concrete shall be manufactured and delivered to the construction site in accordance with ASTM C94.
4. The time between the initial mixing of water into the cement and aggregate mix and the discharge of the concrete in its final position shall not exceed 90 minutes.
5. Concrete strength testing specimen shall be made and cured in conformance with ASTM C31 and tested in accordance with ASTM C39. Three (3) 4 in. by 8 in. cylinders shall be made at each occurrence of concrete placement for each individual mix with an additional cylinder taken at a frequency of every 50 cubic yards. Material test results shall be provided to the Structural Engineer within 7 days of testing.
6. Provide a construction submittal for each proposed concrete mix design including, at a minimum, the locations of placement, concrete materials, mixture proportions, and historical testing data demonstrating the mix design can meet the specified requirements.
7. Concrete mixes shall be well-graded, and shall contain aggregates that vary in size. It shall be permissible to use #89 stone (pea rock) as a filler aggregate, however, a larger sized coarse aggregate (#67 or #57 stone) shall be used where size restrictions allow.
8. Provide an air entrainment admixture for all concrete exposed to weather.
9. Construction joint locations, other than those shown on the drawings, shall be submitted to the Structural Engineer of Record for approval. Place all concrete without horizontal construction joints, unless noted otherwise. Provide continuous horizontal reinforcement through vertical construction joints. Provide a construction submittal identifying the intended concrete placement sequence and locations of construction joints.
10. Verify that all reinforcement and embedded items are properly installed and secured in accordance with the Contract Documents prior to concrete placement.
11. Pipes and conduits shall not be placed in concrete slabs closer than 3 diameters on center. Maximum outside diameter shall be 1/3 of the slab thickness. Conduits shall be located within the middle third of the slab. Position pipes and conduits so that they do not interfere with slab reinforcement. Aluminum conduits shall not be permitted to be placed in concrete.
12. Grout under column base plates and under other bearing plates shall be non-shrink, non-metallic grout with a minimum compressive strength of 5000 psi at 3 days.
13. All keys shall be 2" x 4" (nominal) unless otherwise shown on the drawings.
14. Chamfer all exposed edges of concrete members 3/4" unless noted otherwise on architectural drawings.
15. Begin placement of concrete cast on sloped surfaces at the lowest elevation and continue monolithically toward the higher elevations until the intended pour is completed.
16. Refer to architectural drawings for concrete finish requirements. Where concrete finish is not specified, comply with the requirements of ACI 301.
17. Refer to architectural drawings for concrete details at door and window openings, drip grooves and edges, wash slopes, reglets, finishes, and miscellaneous embedded items.
18. Concrete placement tolerances shall conform with ACI 117 for placement tolerances.
19. Vertical concrete elements (columns, walls, etc...) shall be poured continuously (monolithically) from floor to floor, unless otherwise approved by the EOR. Floors that are not engaged (bypassed) by double-height elements shall not be considered when determining the extents of the concrete pour.
20. Where a topping slab is placed on top of foam fill, the foam fill shall be lightweight structural foam conforming to requirements ASTM D6817 EPS15 (0.90 pcf weight) with a minimum compressive resistance of 3.6 psi at 1% deformation.
21. The concrete structural members are designed to receive the specified loading in the final design state. Temporary formwork required for constructing the concrete elements is the responsibility of the GC. Constructability related items including (but not limited to) formwork, form removal, shoring, and reshoring shall be designed and specified by a delegated engineer licensed in the state of Florida. The items listed below shall be specified by the Delegated Engineer responsible for Formwork/Shoring Design and shall not be less than these minimum requirements:
    A. For PT slabs, formwork is not permitted to be removed until all tendons are stressed.
    B. The following minimum percentages of the specified compressive strength (f'_c) shall be achieved prior to formwork removal:
        a. For Pile Caps: Minimum 20%
        b. For Concrete Columns: Minimum 40%
        c. For Concrete Walls: Minimum 20
        d. For Other Concrete Elements Besides Floor Structures: Minimum 75%
        e. For Concrete Floor Structures: Minimum 85%
    C. Following formwork removal, reshoring shall be provided until 100% of f'_c is achieved.
22. Refer to the 'Schedules' sheet series for the Classes of Concrete Schedule.

REINFORCING STEEL:

1. Concrete reinforcing steel detailing, fabrication, and placement shall comply with ACI and CRSI Standards and Specifications.
2. Indicate on the shop drawings, all necessary accessories to hold reinforcing securely in position. Bar supports shall consist of bolsters and chairs with a spacing not to exceed 4'-0". Refer to the CRSI Manual of Standard Practice for bar support recommendations.
3. Refer to the applicable schedules for dimensions of Tension Development Length (L_d), Tension Development Length of a Standard Hook (L_{dh}), and Class B Lap Splice Length. All lap splices specified in the plans shall be Class B Lap Splices, unless noted otherwise.
4. Where continuous bars are called for, run them continuously around corners and lap them at necessary splices, or provide hooks at discontinuous ends. Lap lengths shall be as given in the splice and development table. Lap beam top bars at mid-span and beam bottom bars at supports, unless noted otherwise.
5. Reinforcing bar splices shall be reviewed by (and at the discretion of) the structural engineer of record. Do not splice bars when length is less than or equal to 50'-0".
6. All stirrup legs shall hook around a longitudinal bar, no exceptions. Where the number of stirrup legs exceeds the specified number of longitudinal bars in a member, provide supplemental #4 bars as required to meet this requirement. The supplemental bars shall be spliced as required, and shall be developed with straight or with a standard hook at termination.
7. All hooks shown on drawings shall be ACI standard hooks, unless noted otherwise.
8. Provide welded wire reinforcing WWR in flat sheets. Lap all WWR one mesh plus two inches at splices.
9. All reinforcing steel shall be deformed bars complying with ASTM A615, Grade 60, unless noted otherwise.
10. Reinforcing steel specified to be welded shall comply with ASTM A706, Grade 60.
11. Inspection of concrete reinforcing steel shall refer to ACI 117-10 for tolerances regarding bar fabrication, assembly, and placement.

CONCRETE COVER TABLE

CONDITION			COVER
SURFACES CAST AGAINST EARTH			3"
FORMED SURFACES EXPOSED TO EARTH OR WEATHER	#5 BARS AND SMALLER		1 1/2"
	#6 BARS AND LARGER		2"
FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER	SLABS, WALLS, AND JOISTS	#11 BARS AND SMALLER	3/4"
		#14 BARS AND LARGER	1 1/2"
	BEAMS, COLUMNS, AND PIERS		1 1/2"
EXTERIOR POST-TENSIONED SLABS	TOP AND BOTTOM		1"
SLABS ON METAL DECK	TOP AND BOTTOM		3/4"
SLABS ON METAL DECK (PARKING)	TOP		1 1/2"
	BOTTOM		3/4"

POST-INSTALLED CONCRETE ANCHORS:

1. Mechanical Anchors: Refer to the plans for mechanical anchor type, size, and quantity. The mechanical anchor system shall be evaluated per ICC-ES Acceptance Criteria AC193 and be recognized as in compliance with the adopted building code by a published ICC-ES Evaluation Service Report (ESR). The specified mechanical anchor system shall be installed in accordance with manufacturer requirements.
2. Adhesive Anchors: Refer to the plans for adhesive anchor type, size, and quantity. The adhesive anchor system shall be evaluated per ICC-ES Acceptance Criteria AC308 and be recognized as in compliance with the adopted building code by a published ICC-ES Evaluation Service Report (ESR). The specified adhesive anchor system shall be installed in accordance with manufacturer requirements.
3. Unless otherwise authorized by the EOR, contractor shall perform a survey using ground penetration radar, or other concrete imaging methods, to locate existing reinforcement prior to installing post-installed anchors. Drilling/coring into existing reinforcement shall not be permitted.
4. The EOR shall be notified if the as-installed location of the post-installed anchor deviates more than 1/2" from the specified location.

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
www.mcsal.com FL CA#26616

Straticon
Received:
06.12.2026

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	06/12/2026	PERMIT SET

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ELIE EL ZGHAIR LICENSE No.78483 STATE OF FLORIDA PROFESSIONAL ENGINEER

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Elie El Zghair, PE No. 0078483
Baker Barrios, PE No. 75403
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TAMBONE COMPANIES

SEACOAST BANK HEADQUARTERS

815 SOUTH COLORADO AVE
STUART, FL 34994

PROJECT NO.
240089.00

GENERAL NOTES

SHEET NUMBER:
S001

THE LINE SHOWN ABOVE IS 1" LONG ON ORIGINAL PLOT SHEETS ORIGINAL PAGE SIZE

STRUCTURAL STEEL:

1. All structural steel work shall comply with ANSI/AISC 360-16 "Specification for Structural Steel Buildings", Fifteenth Edition.
2. All steel to receive spray fireproofing shall not be shop primed. Interior steel not spray fireproofed shall be shop primed with a primer compatible with the finish.
3. All steel in unconditioned spaces or exposed to the weather in the completed structure, including bolts and other related accessories, shall be hot-dip galvanized in accordance with ASTM A123.
4. Any member sizes shown on the plans, and currently listed in the AISC Steel Construction Manual, which are not currently available must be brought to the Engineer of Record's attention prior to the award of the steel contract. No claim for additional cost will be accepted after the award, for member/built up member substitutions for these sizes.
5. All structural steel, including base plates and tops of anchor rods to be exposed to soil are to be hot-dip galvanized.
6. Composite steel deck, non-composite steel deck and concrete slabs help to stabilize the completed structure. The Contractor shall provide temporary erection bracing to maintain structural steel in proper position until permanently secured. Remove temporary bracing and their connections only after the erection of permanent members is complete, steel decks are properly fastened and concrete slabs are placed and cured. A completed structure has its boundary's defined by the building exterior and/or interior expansion joints where they exist between building segments.
7. All composite steel beams are designed assuming unshored construction, unless noted otherwise.
8. Provide an upward beam camber at the mid span between supports where indicated. Camber is to be provided by mill rolling, shop fabrication, or a combination of each. Structural steel beams have been cambered in an effort to limit extra concrete placement due to the deflection of the structural steel beams and girders. The Contractor may not assume that the cambers indicated eliminate the need for extra concrete placement.
9. Headed stud anchor shear connectors shall be 3/4"Ø, unless noted otherwise and shall be field applied.
10. Filler beams shall be spaced equally between established dimensions, unless noted otherwise.
11. Connections: Connections shown on these drawings are generally schematic. They are intended to define the spatial relationship of the framed members and show a feasible method of making the connection. Any connection that is not shown or is not completely detailed on the structural drawings shall be designed by a registered engineer licensed in the state in which the project is permitted and shall be retained by the Fabricator. Design of haunches or similar elements where connected and connecting members are at different elevations shall be included in the connection design scope by the delegated connection engineer.
12. Reactions indicated in the Contract Documents are factored using LRFD. The Fabricator shall provide a connection with a capacity at least equal to the LRFD factored reaction indicated. The connection design shall be provided in the shop drawings and bear the seal and signature of the Delegated Design Professional responsible for steel connection design. Signed-and-sealed calculations shall be provided for each connection that requires delegated design.
13. Connections specifically detailed in the Contract Documents may not be altered without written approval by the Engineer of Record. Where approved, altered connections shall be completely detailed within the shop drawings and bear the seal and signature of the Delegated Design Professional responsible for steel connection design. Any proposed detail alterations shall be specifically identified on the shop drawings by clouding or other distinguishable notation.
14. Bolted Connections: Bolted connections shall be in accordance with RCSC-2010 "Specification for Structural Joints Using High Strength Bolts". Bolted connections shall be made using minimum 3/4"Ø high strength bolts installed snug-tight, unless noted otherwise. Unless noted otherwise, bolts and connected parts specified to be installed as pre-tensioned slip-critical shall meet the requirements for a Class B surface in accordance with RCSC-2010 Section 3.2. Pre-tensioned slip critical members with a galvanized faying surface shall be prepared meeting the requirements of a Class C surface.
15. All bolts that are intended to transmit axial load as part of a connection shall be pre-tensioned in accordance with the requirements of RCSC-2010. Such connections include but are not limited to column splices, connections of beams and girders to columns in braced frames, bolted brace connections in braced frames, bolted moment connections, hanger connections, and connections noted as slip-critical.
16. Bolted connections for bracing members, shall meet the requirements for a slip-critical Class A surface in accordance with RCSC-2010.
17. Bolted joint calculations shall consider deformation of the bolt hole at service loads as a design consideration.
18. Welded Connections: Welded connections shall be made by certified welders, and shall be in accordance with AWS D1.1, 2010 "Structural Welding Code - Steel". Unless noted otherwise, electrodes for all field and shop welding shall conform to the E-70 Series. Where welding reinforcing bars, electrodes shall conform with the E-80 Series.
19. All around welds indicated herein shall be discontinuous at the flange tips of open sections.
20. The submerged arc welding process may be substituted for welding together plate girder sections. Fabricator to submit equivalent weld sizes to be used in lieu of the requirements specified herein.
21. Splicing of structural members where not detailed on the drawings is prohibited without prior approval of the Engineer of Record.
22. Provide welded stiffener plates on both sides of the web of beams at points of concentrated loads including beams supporting columns or running over the tops of columns or other beams. Minimum stiffener plate thickness shall be 5/8" or flange thickness of column above or below, whichever is greater.
23. Supports for metal deck around columns as well as screed plates, pour stops and slab supports at slab openings and slab edges shall be furnished by the contractor as required to complete the work. Unless noted otherwise, provide a min. 1/4" thick bent plate at all slab edges, and around deck openings or penetrations.
24. Cuts, holes, coping, etc. required for work of other trades shall be shown on the shop drawings and made in the shop. Cuts or burning of holes in structural steel members in the field will not be permitted, unless specifically approved in each case by the Engineer of Record.
25. Minimum connection plate thickness shall be 3/8", unless otherwise indicated in the Contract Drawings.
26. All rectangular HSS members to be orientated long side vertical unless noted otherwise.
27. All HSS shapes except diagonal bracing members (round, square rectangular, etc.) are to have a minimum 1/4" thick cap plate at all exposed ends. Cap plates to be seal welded all around, unless noted otherwise.
28. Hot rolled shapes with flange thickness exceeding 2 in. thick and built-up cross sections consisting of plates with a thickness exceeding 2 in. thick used as members subject to primary tensile forces due to tension or flexure and spliced using complete joint penetration groove welds shall be supplied with Charpy V-Notch impact test results that meet a minimum average value of 20 ft-lb at 70°F.

STEEL MATERIAL TABLE

STEEL ELEMENT	ASTM DESIGNATION	Fy	Fu
ANCHOR RODS	F1554 GR. 55	55 KSI	75 KSI
ANGLE (L)	A36	36 KSI	58 KSI
BASE PLATES	A572 GR.50	50 KSI	65 KSI
CHANNEL (C)	A36	36 KSI	58 KSI
H-PILE (HP)	A572 GR. 50	50 KSI	65 KSI
HEADED STUD ANCHOR (SHEAR CONNECTOR)	A108	-	65 KSI
HIGH STRENGTH BOLTS	A325 A490	-	120 KSI 150 KSI
HOLLOW STRUCTURAL SECTION - ROUND (HSS)	A500 GR. B	42 KSI	58 KSI
HOLLOW STRUCTURAL SECTION - RECT (HSS)	A500 GR. B	46 KSI	58 KSI
M-SECTION (M)	A36	36 KSI	58 KSI
MISC. CHANNEL (MC)	A36	36 KSI	58 KSI
PLATES	A36	36 KSI	58 KSI
PLATE GIRDERS	A572 GR. 50	50 KSI	65 KSI
PIPE	A53 GR. B	35 KSI	60 KSI
S-SECTION (S)	A36	36 KSI	58 KSI
WIDE FLANGE (W)	A992	50 KSI	65 KSI
WIDE FLANGE TEE (WT)	A992	50 KSI	65 KSI

STEEL MATERIAL TABLE NOTES:

1. Sections shall conform with the respective material designation unless noted otherwise in the drawings.

STEEL JOIST FRAMING:

1. Steel joist framing shall be designed, detailed, fabricated and erected in accordance with the Steel Joist Institute SJI-100 2015 Standard Specifications and the SJI Code of Standard Practice, 2015 Edition.
2. Refer to the plans for the SJI joist designation, location, and span length for K-Series Joists, KCS-Series Joists, LH-Series Joists, DLH-Series Joists, Special Joists, Joist Substitutes and Joist Girders. Refer to the plans for any additional loading requirements.
3. All joists shall be designed by a registered engineer licensed in the state in which the project is permitted. The delegated engineering services shall be retained by the Steel Joist Manufacturer. The Steel Joist Manufacturer shall prepare detailed shop drawings. Signed-and-sealed calculations shall be provided for review.
4. Filler joists shall be spaced equally between established dimensions, unless noted otherwise.
5. All steel joists shall be coated with a shop primer, unless noted otherwise.
6. All steel joists in unconditioned spaces or exposed to the weather in the completed structure, including bridging and other related accessories, shall be hot-dip galvanized in accordance with ASTM A123.
7. Locations, quantities, and sizes of steel joist bridging shall be designed and detailed by the Steel Joist Manufacturer and shown in the shop drawings.
8. Gravity, uplift, and lateral connection design and details for anchorage to the supporting structure shall be provided by the Steel Joist Manufacturer.
9. Joist top chord extensions shall be Type R and be designed to withstand the uniform loading requirements specified by the SJI designation of that joist as well as any additional loading requirements identified on the plans.
10. Where a steel girder (or similar support element) supports joists on only one side, extend the joist seats to the far edge of the flange to avoid an eccentric loading condition.
11. Where aligned joists in adjacent bays bear on a common support element, and minimum bearing cannot be achieved, shift at least one of the joists in plan and extend the joist seats to the far edge of the flange to avoid an eccentric loading condition. Modified plan locations shall be indicated within the joist shop drawings.
12. The joist supplier shall refer to the Components and Cladding Wind Pressure Diagrams sheet to determine uplift requirements. It shall be permitted to assume a superimposed dead load of 5 psf for joist uplift design.
13. Damaged/kinked joists shall not be installed.
14. Should joist retrofitting be required, it shall be designed by the specialty joist engineer.

STEEL JOIST BEARING GEOMETRY TABLE

DESIGNATION	STANDARD BEARING SEAT DEPTH	MINIMUM BEARING LENGTH ON STEEL	MINIMUM BEARING LENGTH ON MASONRY OR CONCRETE	MINIMUM BEARING PLATE WIDTH (FOR BEARING ON MASONRY OR CONCRETE)
K1-12	2 1/2"	2 1/2"	4"	7"
LH02-06	5"	2 1/2"	6"	9"
LH07-17, DLH10-17	5"	4"	6"	9"
DLH18-25	7 1/2"	6"	6"	14"
JOIST GIRDERS	7 1/2"	6"	6"	14"

COMPOSITE METAL FLOOR DECK:

1. Corrugated composite metal floor deck shall be manufactured from galvanized steel conforming to ASTM A653.
2. All composite metal floor deck in unconditioned spaces or exposed to weather shall have a G90 galvanized finish. All composite metal floor deck in interior conditioned spaces shall have a G60 galvanized finish.
3. Coordinate size and location of floor openings with architectural and mechanical drawings. Refer to typical details for supplemental framing required at openings.
4. Decks that do not meet the minimum section properties shown within this table, shall be submitted for review with signed-and-sealed calculations showing that the deck is capable of resisting the applied loads while satisfying all strength and serviceability requirements.
5. Refer to the the plan notes for deck fastening requirements.
6. Fabricate deck units in lengths to span three or more support spacings, where possible. No single spans shall be allowed without E.O.R. approval.
7. Where light-weight insulating concrete (LWIC) is specified, the metal deck shall be vented in conformance with the LWIC manufacturer's requirements.

COMPOSITE METAL FLOOR DECK PROPERTIES TABLE

DEPTH	TYPE	GAGE	I_P (IN^4/FT)	S_P (IN^3/FT)	I_N (IN^4/FT)	S_N (IN^3/FT)	F_Y (KSI)
1.5"	VL/VLI	20	0.182	0.218	0.216	0.226	50
		18	0.265	0.298	0.286	0.314	50
		16	0.362	0.389	0.362	0.396	40
	VLR	20	0.216	0.226	0.182	0.218	50
		18	0.286	0.314	0.265	0.298	50
		16	0.362	0.396	0.362	0.389	40
2"	VLI	20	0.403	0.326	0.402	0.337	50
		18	0.558	0.485	0.558	0.500	50
		16	0.704	0.663	0.704	0.663	40
3"	VLI	20	0.907	0.512	0.909	0.538	50
		18	1.252	0.761	1.252	0.794	50
		16	1.582	1.013	1.582	1.013	40

METAL ROOF DECK:

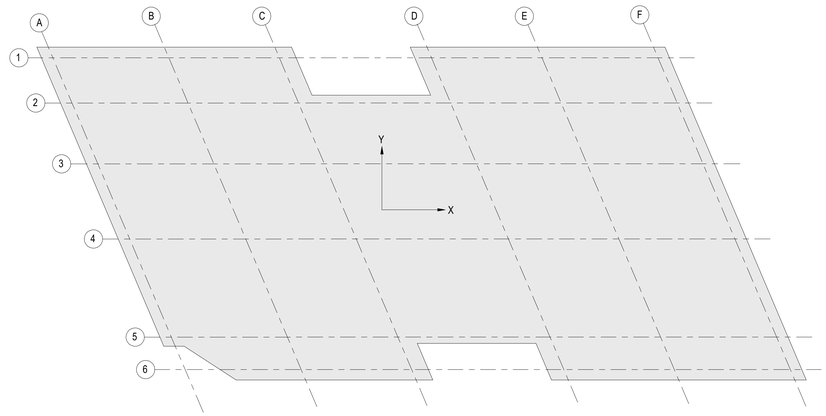
1. Corrugated metal roof deck shall be manufactured from galvanized steel conforming to ASTM A653.
2. All metal roof deck shall have a G90 galvanized finish.
3. Coordinate size and location of roof openings with architectural and mechanical drawings. Refer to typical details for supplemental framing required at openings.
4. Do not suspend pipes, ducts, conduit, or other related items from metal roof deck.
5. Decks that do not meet the minimum section properties shown within this table, shall be submitted for review with signed-and-sealed calculations showing that the deck is capable of resisting the applied loads while satisfying all strength and serviceability requirements.
6. Refer to the the plan notes for deck fastening requirements.
7. Fabricate deck units in lengths to span three or more support spacings, where possible. No single spans shall be allowed without E.O.R. approval.
8. Where light-weight insulating concrete (LWIC) is specified, the metal deck shall be vented in conformance with the LWIC manufacturer's requirements.

METAL ROOF DECK PROPERTIES TABLE

DEPTH	TYPE	GAGE	I_P (IN^4/FT)	S_P (IN^3/FT)	I_N (IN^4/FT)	S_N (IN^3/FT)	F_Y (KSI)
1.5"	B	20	0.201	0.222	0.234	0.247	50
		18	0.289	0.295	0.318	0.327	50
		16	0.373	0.373	0.408	0.411	50

COLD-FORMED METAL FRAMING:

1. Cold-formed metal framing information shown in these drawings is generally schematic. It is intended to define the spatial relationship of the framed members and show a feasible method of making a connection to the structure.
2. Cold-formed metal framing shall be designed by a registered engineer licensed in the state in which the project is permitted. The delegated engineering services shall be retained by the Cold-Formed Metal Supplier. The Cold-Formed Metal Supplier shall prepare detailed shop drawings that bear the seal and signature of the Delegated Design Professional responsible for the Cold-Formed Metal framing design. Signed-and-sealed calculations shall be provided for each unique condition that requires delegated design.



WIND LOAD DIAGRAM TABLE

FLOOR LEVEL	WIND	
	X	Y
ROOF	110K	250K
LEVEL 3	100K	220K
LEVEL 2	110K	235K

NOTES:

1. LOADS SHOWN IN THE TABLES ARE TOTAL DESIGN (ULTIMATE) LATERAL FORCES APPLIED AT EACH ELEVATED FLOOR LEVEL. THE DELEGATED TILT PANEL ENGINEER SHALL DISTRIBUTE THE LOADS UNIFORMLY ALONG THE LENGTH OF THE PERIMETER WALLS FOR THE LATERAL DESIGN OF THE TILT PANELS.
2. ROOF LEVEL WIND LOADS INCLUDE PARAPET CONTRIBUTIONS.

C3 S002 **WIND LOAD DIAGRAM**

SCALE: 1" = 20'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

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GENERAL NOTES

SHEET NUMBER:
S002

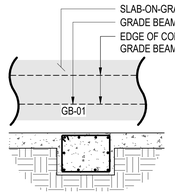
ABBREVIATIONS LIST

(E)	EXISTING
/	PER
@	AT
Ø	DIAMETER
ANCH	ANCHOR
ADDL	ADDITIONAL
AESS	ARCHIECTURALLY EXPOSED STRUCTURAL STEEL
ALT	ALTERNATE
ALUM	ALUMINUM
APPROX	APPROXIMATE
ARCH	ARCHITECT
BF	BRACED FRAME
BLDG	BUILDING
BM	BEAM
B/	BOTTOM OF
B.O.	BOTTOM OF
B.O.S.	BOTTOM OF STEEL
BOT	BOTTOM
BRDG	BRIDGING
BRG	BEARING
BTWN	BETWEEN
C	CHANNEL
CANT	CANTILEVER
CF	COLD-FORMED
CFMF	COLD-FORMED METAL FRAMING
CIP	CAST-IN-PLACE
CJ	CONTROL JOINT
CJP	COMPLETE JOINT PENETRATION
CL	CENTERLINE
CLNG	CEILING
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONST	CONSTRUCTION
CONT	CONTINUOUS
COORD	COORDINATE
CTR	CENTER
Db	BAR DIAMETER
DBA	DEFORMED BAR ANCHOR
DBL	DOUBLE
DIA	DIAMETER
DIAG	DIAGONAL
DIM	DIMENSION
DL	DEAD LOAD
DN	DOWN
DTL	DETAIL
DWG	DRAWING
DWL	DOWEL
EW	EACH WAY
EA	EACH
EE	EACH END
EF	EACH FACE
EJ	EXPANSION JOINT
EL	ELEVATION
ELEV	ELEVATION
EMBD	EMBED
ENGR	ENGINEER
EOR	ENGINEER OF RECORD
EOS	EDGE OF SLAB
EQ	EQUAL
EQ SP	EQUAL SPACING
EQUIP	EQUIPMENT
EQUIV	EQUIVALENT
ES	EACH SIDE
EW	EACH WAY
EXP	EXPANSION
EXT	EXTERIOR
FA	FACE
FAB	FABRICATE
FD	FLOOR DRAIN
FDN	FOUNDATION
FF	FINISH FLOOR
FIN	FINISH
FLR	FLOOR
FS	FAR SIDE
FT	FOOT
FTG	FOOTING
GA	GAGE
GALV	GALVANIZE
GB	GRADE BEAM
GEN	GENERAL
GR	GRADE
HAS	HEADED ANCHOR STUD
HDG	HOT DIP GALVANIZED
HNGR	HANGER
HK	HOOK
HORIZ	HORIZONTAL
HSS	HOLLOW-STRUCTURAL SECTION
HT	HEIGHT
HVAC	HEATING VENTILATION AND A/C
ID	INSIDE DIAMETER / INTERIOR DESIGN
IF	INSIDE FACE
IN	INCH
INT	INTERIOR
JT	JOINT
K	KIP (1000 POUNDS)
KSF	KIP PER SQUARE FOOT
KSI	KIP PER SQUARE INCH
LB	POUND
L_D	DEVELOPMENT LENGTH (TENSION) OF STRAIGHT REBAR
L_{DC}	DEVELOPMENT LENGTH (COMPRESSION) OF STRAIGHT REBAR
L_{DH}	DEVELOPMENT LENGTH (TENSION) OF STANDARD HOOK REBAR
L_{SC}	LAP SPLICE (COMPRESSION)
LL	LIVE LOAD
LLBB	LONG LEG BACK-TO-BACK
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG	LONGITUDINAL
LT	LIGHT
LWC	LIGHT-WEIGHT CONCRETE
MAS	MASONRY
MATL	MATERIAL
MAX	MAXIMUM
MC	MOMENT CONNECTION
MECH	MECHANICAL
MEP	MECHANICAL/ELECTRICAL/PLUMBING
MEZZ	MEZZANINE
MFR	MANUFACTURER
MIN	MINIMUM
MISC	MISCELLANEOUS
MLS	MASONRY LAP SPLICE
MTL	METAL
N	NORTH
NS	NON-SHRINK
NIC	NOT IN CONTRACT
NO	NUMBER
NOM	NOMINAL
NTS	NOT TO SCALE
OC	ON CENTER
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
OH	OPPOSITE HAND
OPNG	OPENING
OPP	OPPOSITE
OVS	OVERSIZED
PAF	POWDER ACTUADED FASTENER
PC	PRECAST CONCRETE
PEN	PENETRATION
PERP	PERPENDICULAR
PL	PLATE
PLF	POUNDS PER LINEAR FOOT
PREFAB	PREFABRICATED
PRELIM	PRELIMINARY
PS	PRE-STRESSED
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	POST-TENSIONED / PRESSURE TREATED
QTY	QUANTITY
RAD	RADIUS
RE	REGARDING
REF	REFERENCE
REINF	REINFORCEMENT
REQD	REQUIRED
RET	RETURN
REV	REVISION
RTU	ROOF TOP UNIT
S	SOUTH
SC	SLIP CRITICAL
SCHED	SCHEDULE
SECT	SECTION
SHT	SHEET
SIM	SIMILAR
SLBB	SHORT LEG BACK-TO-BACK
SLH	SHORT LEG HORIZONTAL
SLV	SHORT LEG VERTICAL
SOG	SLAB-ON-GRADE
SP	SPACE
SPEC	SPECIFICATION
SPRT	SUPPORT
SQ	SQUARE
SS	STAINLESS STEEL
STD	STANDARD
STIFF	STIFFENER
STIR	STIRRUP
STL	STEEL
STR	STRUCTURAL
SW	SHEAR WALL
SYM	SYMMETRICAL
T	TOP
T&B	TOP AND BOTTOM
THK	THICK
T.O.	TOP OF
T.O.F.	TOP OF FOUNDATION
T.O.S.	TOP OF STEEL
T.O.W.	TOP OF WALL
TRANS	TRANSVERSE
TYP	TYPICAL
ULT	ULTIMATE
UNO	UNLESS NOTHED OTHERWISE
VERT	VERTICAL
VIF	VERIFY IN FIELD
W/	WITH
W/O	WITHOUT
WF	WIDE FLANGE
WP	WOOD POST OR WORK POINT
WT	WIDE FLANGE T SECTION
WWR	WELDED WIRE REINFORCEMENT
WxH	WIDTHxHEIGHT

STRUCTURAL MARKS LEGEND

FOUNDATIONS

GB-##: GRADE BEAM



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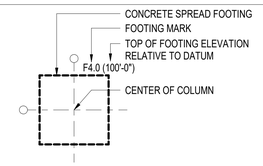

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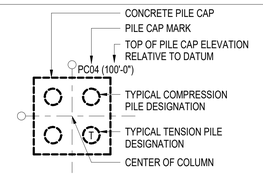

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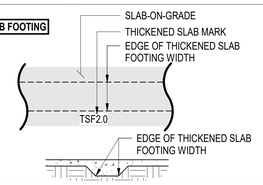

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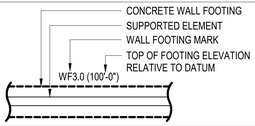
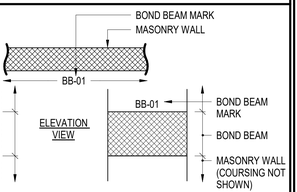


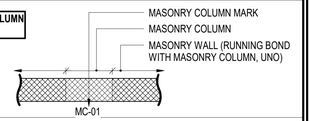
MASONRY

BB-##: BOND BEAM

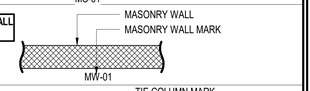


NOTE:
REFER TO THE PLANS AND GENERAL NOTES FOR ALL LOCATIONS WHERE A BOND BEAM IS REQUIRED. BOND BEAMS CALLED OUT ON PLAN ARE FOR LOCATIONS WITH REQUIREMENTS MORE STRINGENT THAN THE MINIMUM BOND BEAM REQUIREMENTS IDENTIFIED IN THE GENERAL NOTES AND APPLICABLE DETAILS.

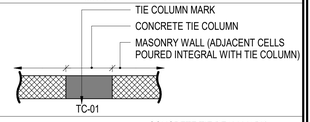
MC-##: MASONRY COLUMN



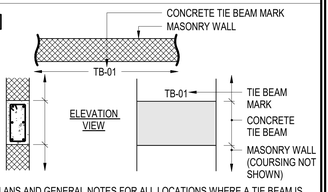
MW-##: MASONRY WALL



TC-##: TIE COLUMN



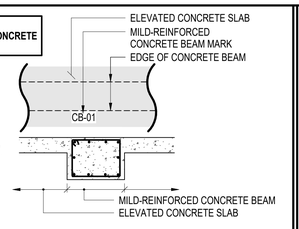
TB-##: TIE BEAM



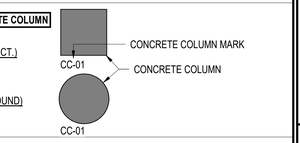
NOTE:
REFER TO THE PLANS AND GENERAL NOTES FOR ALL LOCATIONS WHERE A TIE BEAM IS REQUIRED. TIE BEAMS CALLED OUT ON PLAN ARE FOR LOCATIONS WITH REQUIREMENTS MORE STRINGENT THAN THE MINIMUM TIE BEAM REQUIREMENTS IDENTIFIED IN THE GENERAL NOTES AND APPLICABLE DETAILS.

CONCRETE

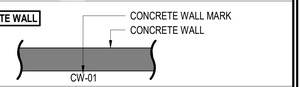
CB-##: MILD-REINFORCED CONCRETE BEAM



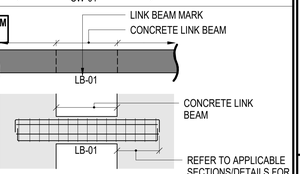
CC-##: CONCRETE COLUMN



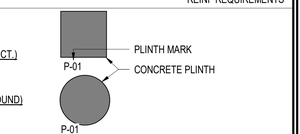
CW-##: CONCRETE WALL



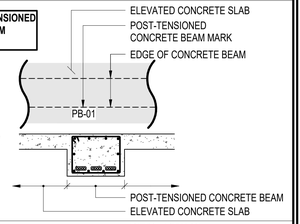
LB-##: LINK BEAM



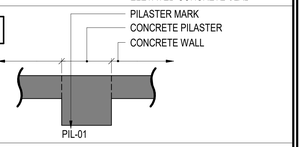
P-##: PLINTH



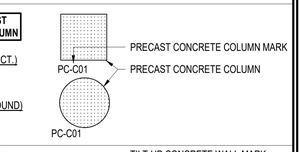
PB-##: POST-TENSIONED CONCRETE BEAM



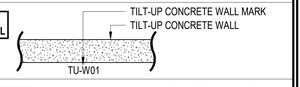
PIL-##: PILASTER



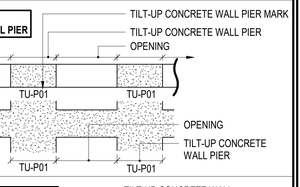
PC-C##: PRECAST CONCRETE COLUMN



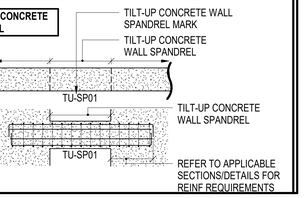
TU-W##: TILT-UP CONCRETE WALL



TU-P##: TILT-UP CONCRETE WALL PIER

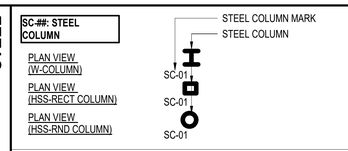


TU-S##: TILT-UP CONCRETE WALL SPANDREL

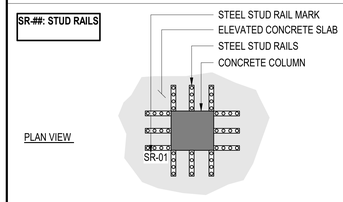


STEEL

SC-##: STEEL COLUMN

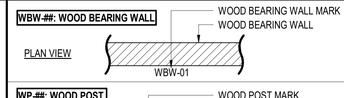


SR-##: STUD RAILS

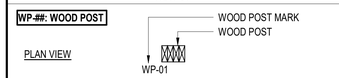


WOOD

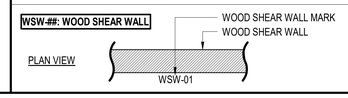
WBW-##: WOOD BEARING WALL



WP-##: WOOD POST



WSW-##: WOOD SHEAR WALL



STRUCTURAL MATERIAL LEGEND

CONCRETE	SURFACE PATTERN	
	CUT PATTERN (PLAN)	
	CUT PATTERN (SECTION)	
MASONRY	SURFACE PATTERN (ELEVATION)	
	CUT PATTERN (PLAN)	
MASONRY (NON LOAD BEARING)	SURFACE PATTERN (ELEVATION)	
	CUT PATTERN (PLAN)	
STRUCTURAL STEEL	CUT PATTERN	
WOOD	CUT PATTERN	
WOOD (SHEAR WALL)	CUT PATTERN	
PRECAST CONCRETE	CUT PATTERN	
TILT UP CONCRETE	CUT PATTERN	

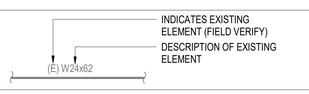
STRUCTURAL ANNOTATIONS LEGEND

ELEVATION MARKER

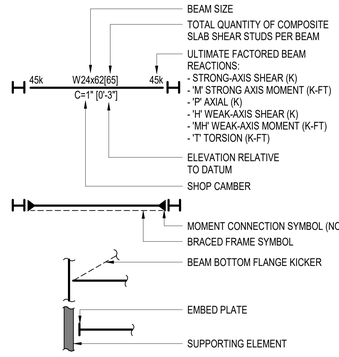
REINFORCING STEEL

SPAN DIRECTION

EXISTING STRUCTURAL ELEMENTS



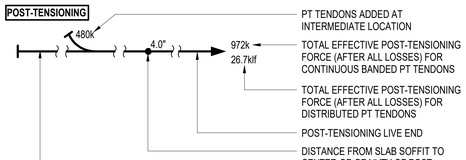
STEEL BEAMS



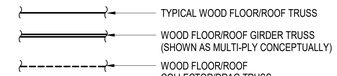
NOTES:

1. MOMENT CONNECTIONS INDICATED ON PLAN SHALL BE DESIGNED FOR THE FULL PLASTIC MOMENT CAPACITY (LRFD) AS INDICATED IN THE AISC MANUAL. SEE STRUCTURAL STEEL GENERAL NOTES FOR MORE INFORMATION.

POST-TENSIONING



WOOD TRUSSES



Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
www.mcsal.com FL CA#26616

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Elie El Zghayar, PhD, PE on 06/10/2026
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TAMBONE COMPANIES

SEACOAST BANK HEADQUARTERS

815 SOUTH COLORADO AVE
STUART, FL 34994

PROJECT NO.:
240089.00

LEGENDS

SHEET NUMBER:
S003

THE LINE SHOWN ABOVE IS 1" LONG ON ORIGINAL PAPER SIZE SHEETS OR ORIGINAL PAPER SIZE

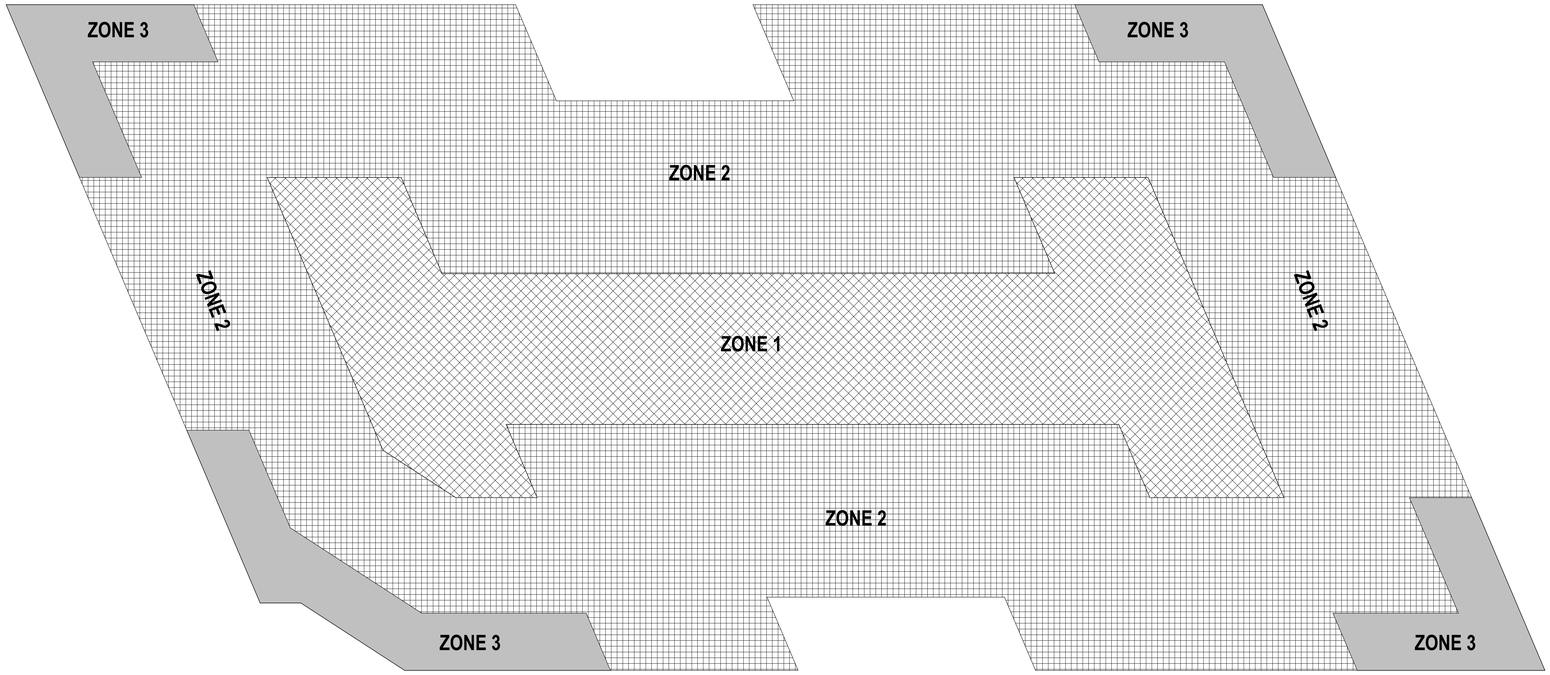
COMPONENTS AND CLADDING PRESSURE TABLE

ULTIMATE WIND SPEED (Vult)	162 MPH
WIND EXPOSURE	B
INTERNAL PRESSURE COEFFIENT (GCpi)	+/-0.18
MEAN ROOF HEIGHT (h)	55'-0"
END ZONE LENGTH (a)	11'-6"

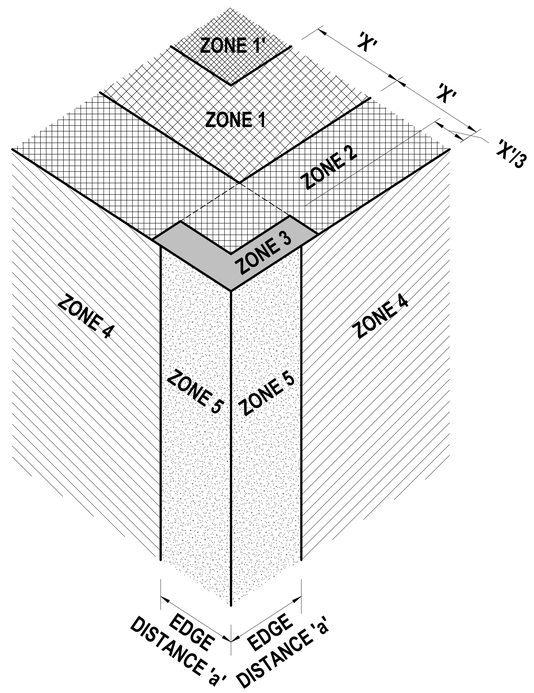
LOCATION		ZONE	EFFECTIVE AREA (FT^2)	NEGATIVE PRESSURE (PSF)	POSITIVE PRESSURE (PSF)
WALL	INTERIOR	4	10	- 55.5	+ 51.2
			20	- 53.2	+ 48.9
			50	- 50.2	+ 45.9
			100	- 47.9	+ 43.7
			500	- 42.7	+ 38.4
	EDGE	5	10	- 68.3	+ 51.2
			20	- 63.7	+ 48.9
			50	- 57.7	+ 45.9
			100	- 53.2	+ 43.7
			500	- 42.7	+ 38.4
ROOF	INTERIOR	1	10	- 89.1	+ 22.8
			20	- 83.2	+ 21.3
			50	- 75.5	+ 19.4
			100	- 69.6	+ 18.0
			500	- 55.9	+ 18.90
	EDGE	2	10	- 117.5	+ 22.8
			20	- 110.0	+ 21.3
			50	- 100.0	+ 19.4
			100	- 92.4	+ 18.0
			500	- 74.9	+ 18.0
	CORNER	3	10	- 160.2	+ 22.8
			20	- 145.1	+ 21.3
			50	- 125.1	+ 19.4
			100	- 110.0	+ 18.0
			500	- 74.9	+ 18.0
OVERHANG (NOTE 3)	INTERIOR	2OH	10	- 117.5	+ 74.0
			20	- 107.5	+ 70.2
			50	- 94.1	+ 65.3
			100	- 84.1	+ 61.7
			500	- 60.7	+ 56.4
	EDGE	3OH	10	- 160.2	+ 74.0
			20	- 142.6	+ 70.2
			50	- 119.3	+ 65.3
			100	- 101.6	+ 61.7
			500	- 60.7	+ 56.4

NOTES:

1. WIND PRESSURES ACT NORMAL (ORTHOGONAL) TO THE RESPECTIVE SURFACE. POSITIVE PRESSURES ARE APPLIED TOWARD THE SURFACE, NEGATIVE PRESSURES ARE APPLIED AWAY FROM THE SURFACE.
2. THE PRESSURES LISTED ABOVE ARE ULTIMATE STRENGTH WIND PRESSURES. FOR SERVICE LEVEL (OR ASD) WIND PRESSURES, MULTIPLY THE ABOVE VALUES BY 0.6.
3. ROOF WIND PRESSURES AT SOFFITS OF OVERHANGS SHALL BE TAKEN AS EQUAL TO THE WALL PRESSURES AT THEIR RESPECTIVE ZONES.



B3 / S004 **C&C WIND PRESSURE PLAN**
SCALE: 1" = 10'-0"



A2 / S004 **TYPICAL C&C EDGE/CORNER SCHEMATIC (ASCE 7-22)**
SCALE: 6" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
www.mcsal.com FL CA#26616

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ELIE EL ZGHAYAR LICENSE No.78483 STATE OF FLORIDA PROFESSIONAL ENGINEER

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Elie El Zghayar, PhD, PE on **06/12/2026**
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TAMBONE COMPANIES

SEACOAST BANK HEADQUARTERS

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STUART, FL 34994

PROJECT NO:
240089.00

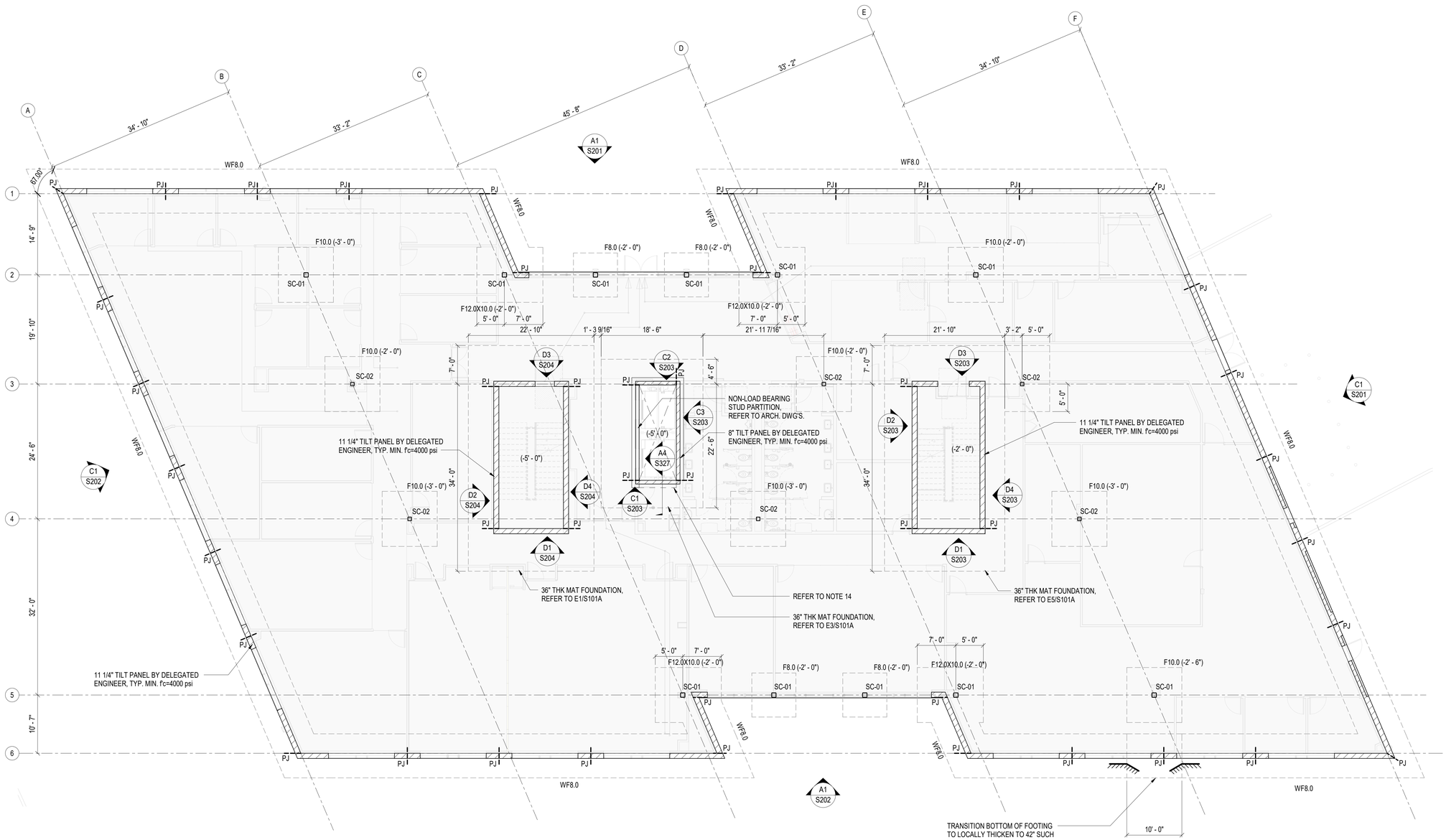
COMPONENTS AND CLADDING WIND PRESSURES

SHEET NUMBER:
S004

THE LINE SHOWN ABOVE IS 1" LONG ON ORIGINAL 22x34 SHEETS ORIGINAL PAGE SIZE

FOUNDATION & SLAB-ON-GRADE PLAN NOTES:

1. FOUNDATION DETAILS SHOWN ARE PLACE HOLDERS.
2. CIVIL DRAWINGS ELEVATION 12'-0" = DATUM ELEVATION 0'-0".
3. REFER TO PLAN FOR TOP OF FOUNDATION ELEVATION. TOP OF SLAB-ON-GRADE ELEVATION SHALL BE 0'-0" UNLESS OTHERWISE NOTED.
4. ALL DIMENSIONS HAVE BEEN GENERATED BY THE ARCHITECT AND SHOULD BE VERIFIED WITH THE LATEST SET OF ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.
5. REFER TO ARCHITECTURAL FOR SLAB DEPRESSIONS, OPENINGS, AND OTHER INFORMATION NOT SHOWN HERE.
6. STRUCTURAL DRAWINGS SHALL NOT BE USED AS ERECTION DRAWINGS.
7. SLAB-ON-GRADE CONSTRUCTION: SHALL BE NORMAL-WEIGHT CONCRETE. THICKNESS AND REINFORCING SHALL BE AS FOLLOWS:
   A. TYPICAL SLABS-ON-GRADE SHALL BE 5" THICK REINFORCED WITH WWR 4x4-W4.0xW4.0 OR WWR 6x6-W5.5xW5.5.
   B. MECHANICAL ROOM SLABS-ON-GRADE SHALL BE 5" THICK REINFORCED WITH WWR 4x4-W4.0xW4.0 OR WWR 6x6-W5.5xW5.5.
8. MAINTAIN SLAB-ON-GRADE THICKNESS AT ALL DEPRESSIONS AND SLOPES. PROVIDE CONTROL JOINTS IN AN ORTHOGONAL GRID, AT A SPACING NOT EXCEEDING 12'-0" AND WITH A PLAN ASPECT RATION NOT EXCEEDING 2:1. PROVIDE A VAPOR BARRIER AS SPECIFIED B THE WATERPROOFING CONSULTANT AND/OR ARCHITECT.
9. REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS AT CONTINUOUS FOUNDATION CORNERS & INTERSECTIONS, AS WELL AS AT SLAB REENTRANT CORNERS, THICKENED SLAB CORNERS & INTERSECTIONS.
10. FOUNDATIONS (CONTINUOUS & ISOLATED) SHALL BE CENTERED ON THEIR RESPECTIVE SUPPORTED ELEMENT UNLESS NOTED OTHERWISE.
11. REFER TO CIVIL AND MEP DRAWINGS FOR SLAB PENETRATION LOCATIONS & UNDERGROUND UTILITY INFORMATION. INSTALLATION OF UNDERGROUND UTILITIES SHALL NOT UNDERMINE FOUNDATION OR SLAB ELEMENTS. CONTRACTOR TO COORDINATE SEQUENCING AS REQUIRED.
12. CORING THROUGH THE SLAB-ON-GRADE IS NOT PERMITTED. IN LIEU OF CORING, PROVIDE SLEEVES CAST-IN WITH THE SLAB POUR. SLEEVE SIZES AND LOCATIONS SHALL BE IDENTIFIED ON THE SHOP DRAWINGS AND COORDINATED WITH THE EOR PRIOR TO CONSTRUCTION.
13. FINAL PLAN LOCATIONS, ELEVATIONS, QUANTITIES, ETC... FOR ALL ELEVATOR STEEL SHALL BE COORDINATED WITH ARCHITECTURAL & ELEVATOR MANUFACTURER DRAWINGS.
14. ELEVATOR PIT WALLS SHALL BE 16" THICK, REINFORCED WITH #6@12" VERTICAL EACH FACE & #6@12" HORIZONTAL EACH FACE.
15. REFER TO TILT PANEL ELEVATIONS FOR PANEL THICKNESS.



A1 / S101 SLAB ON GRADE PLAN

SCALE: 1/8" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

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Structural Engineers
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TAMBONE COMPANIES

SEACOAST BANK HEADQUARTERS

815 SOUTH COLORADO AVE
STUART, FL 34994

PROJECT NO:
240089.00

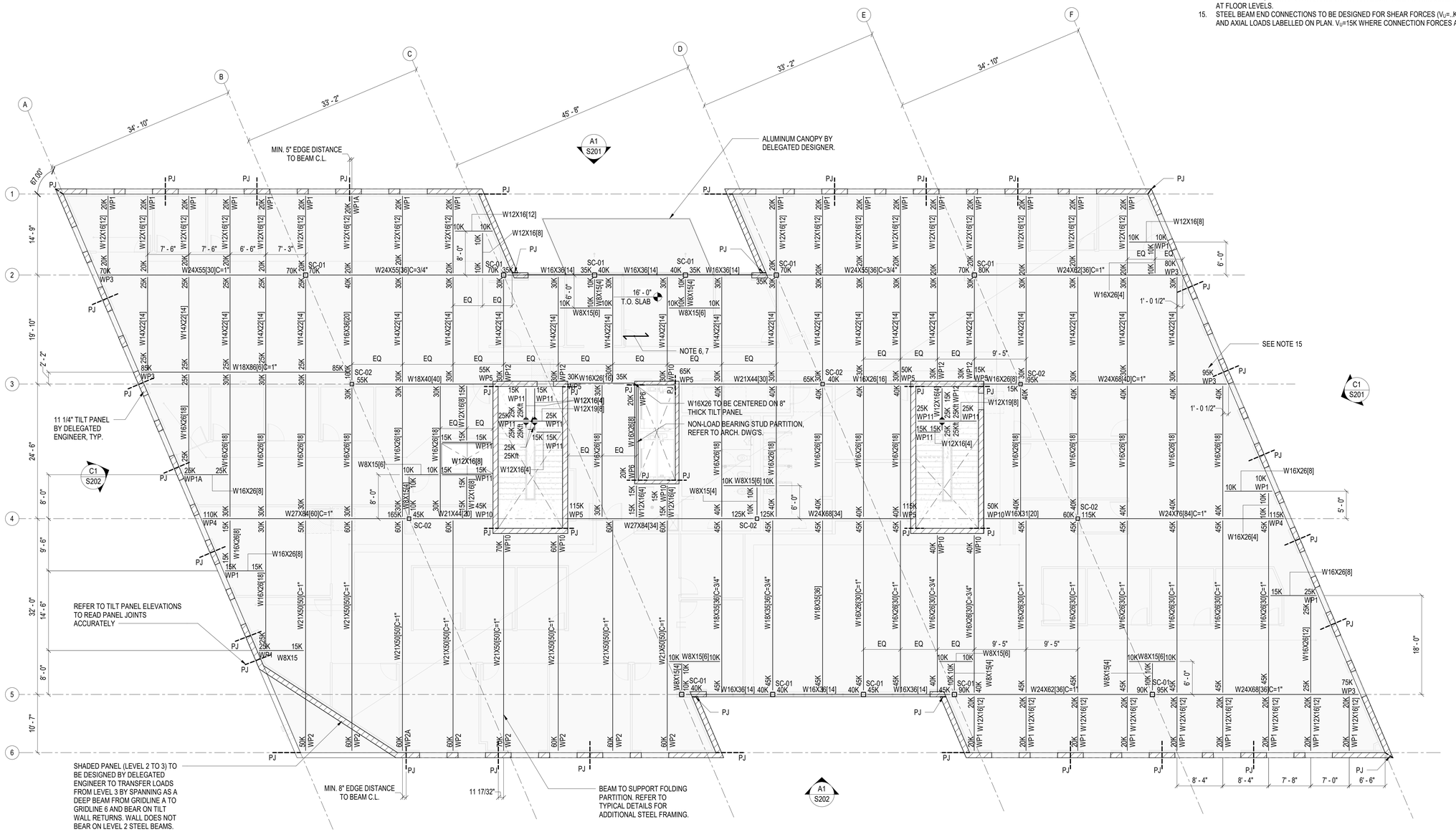
SLAB ON GRADE PLAN

SHEET NUMBER:
S101

THE LINE SHOWN ABOVE IS 1" LONG ON ORIGINAL PLOT IF NOT SHEET IS ORIGINAL PAPER SIZE

FLOOR FRAMING PLAN NOTES:

1. CIVIL DRAWINGS ELEVATION 12'-0" = DATUM ELEVATION 0'-0".
2. TOP OF SLAB ELEVATION IS INDICATED IN PLAN.
3. ALL DIMENSIONS HAVE BEEN GENERATED BY THE ARCHITECT AND SHOULD BE VERIFIED WITH THE LATEST SET OF ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.
4. REFER TO ARCHITECTURAL DRAWINGS FOR SLAB DEPRESSIONS, OPENINGS, AND OTHER INFORMATION NOT SHOWN HERE.
5. STRUCTURAL DRAWINGS SHALL NOT BE USED AS ERECTION DRAWINGS.
6. FLOOR CONSTRUCTION SHALL BE 3 1/2" NORMAL WEIGHT CONCRETE ON 2" DEEP (5 1/2" TOTAL THICKNESS), 18 GAUGE COMPOSITE DECK, REINFORCED WITH WWR 6x6-W2.1x2.21. PROVIDE MINIMUM THREE-SPAN CONDITION, NO SINGLE SPANS ALLOWED WITHOUT EOR APPROVAL.
7. METAL DECK FASTENING CRITERIA:
   A. AT INTERMEDIATE SUPPORTS AND AT ENDS WITH 5/8" PUDDLE WELDS AT EACH LOW FLUTE. FASTEN DECK TO MEMBERS RUNNING PARALLEL TO DECK SPAN WITH 5/8" PUDDLE WELDS AT 12" MAX SPACING. NOTE THAT IT WILL BE PERMISSIBLE TO REPLACE A DECK WELD WITH A HEADED STUD (SHEAR CONNECTOR) WELDED THROUGH THE DECK.
   B. AT SIDE LAPS, FASTEN UNITS TOGETHER USING #10 TEK SCREWS AT A SPACING NOT EXCEEDING ONE-HALF OF THE SPAN, OR 36"; THE LESSER VALUE SHALL GOVERN.
8. REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS AT RE-ENTRANT CORNERS.
9. PROVIDE (3)-#6 CONTINUOUS REINFORCEMENT ALONG SLAB PERIMETER AT ALL SLAB EDGES. PROVIDE CLASS 'B' LAP SPLICE AS REQUIRED. LOCATE ALL SUPPLEMENTAL REINFORCING AT SLAB MID-DEPTH.
10. REFER TO MEP DRAWINGS FOR ELEVATED SLAB PENETRATIONS, REINFORCE OPENINGS FOR PENETRATIONS PER TYPICAL DETAIL REQUIREMENTS.
11. CORING THROUGH ELEVATED SLABS WILL NOT BE PERMITTED. PROVIDE SLEEVES AND BLOCKOUTS AS REQUIRED FOR FLOOR BOXES, PIPES, AND OTHER MEP RELATED ITEMS. ALL SLEEVE AND BLOCKOUT LOCATIONS SHALL BE IDENTIFIED ON THE SHOP DRAWINGS AND COORDINATED WITH THE EOR PRIOR TO CONSTRUCTION.
12. IT WILL NOT BE PERMISSIBLE TO RUN CONDUIT, PIPES, OR OTHER MISCELLANEOUS NON-STRUCTURAL ITEMS WITHIN THE ELEVATED SLAB.
13. FINAL PLAN LOCATIONS, ELEVATIONS, QUANTITIES, ETC... FOR ALL ELEVATOR STEEL SHALL BE COORDINATED WITH ARCHITECTURAL & ELEVATOR MANUFACTURER DRAWINGS.
14. DELEGATED STAIR ENGINEER SHALL DESIGN THE STAIR FRAMING (HSS STRINGERS OR STANDARD MC SHAPES WITH ADEQUATE LATERAL CAPACITY) TO BRACE TILT-PANELS. CONTRACTOR SHALL COORDINATE EMBEDDED CONNECTIONS IN TILT-PANELS BETWEEN DELEGATED TILT-PANEL ENGINEER AND STAIR MANUFACTURER. SHOULD IT NOT BE DESIRED TO BRACE TILT-PANELS WITH STAIR FRAMING, CONTRACTOR SHALL CARRY AN ALLOWANCE FOR MISCELLANEOUS HSS BRACING AT FLOOR LEVELS.
15. STEEL BEAM END CONNECTIONS TO BE DESIGNED FOR SHEAR FORCES (V_U=..K), MOMENTS (M_U=15Kft), AND AXIAL LOADS LABELLED ON PLAN. V_U=15K WHERE CONNECTION FORCES ARE NOT SHOWN.



A1 S102 LEVEL 2 FRAMING PLAN

SCALE: 1/8" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
www.mcsal.com FL CA#26616

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STUART, FL 34994

PROJECT NO:
240089.00

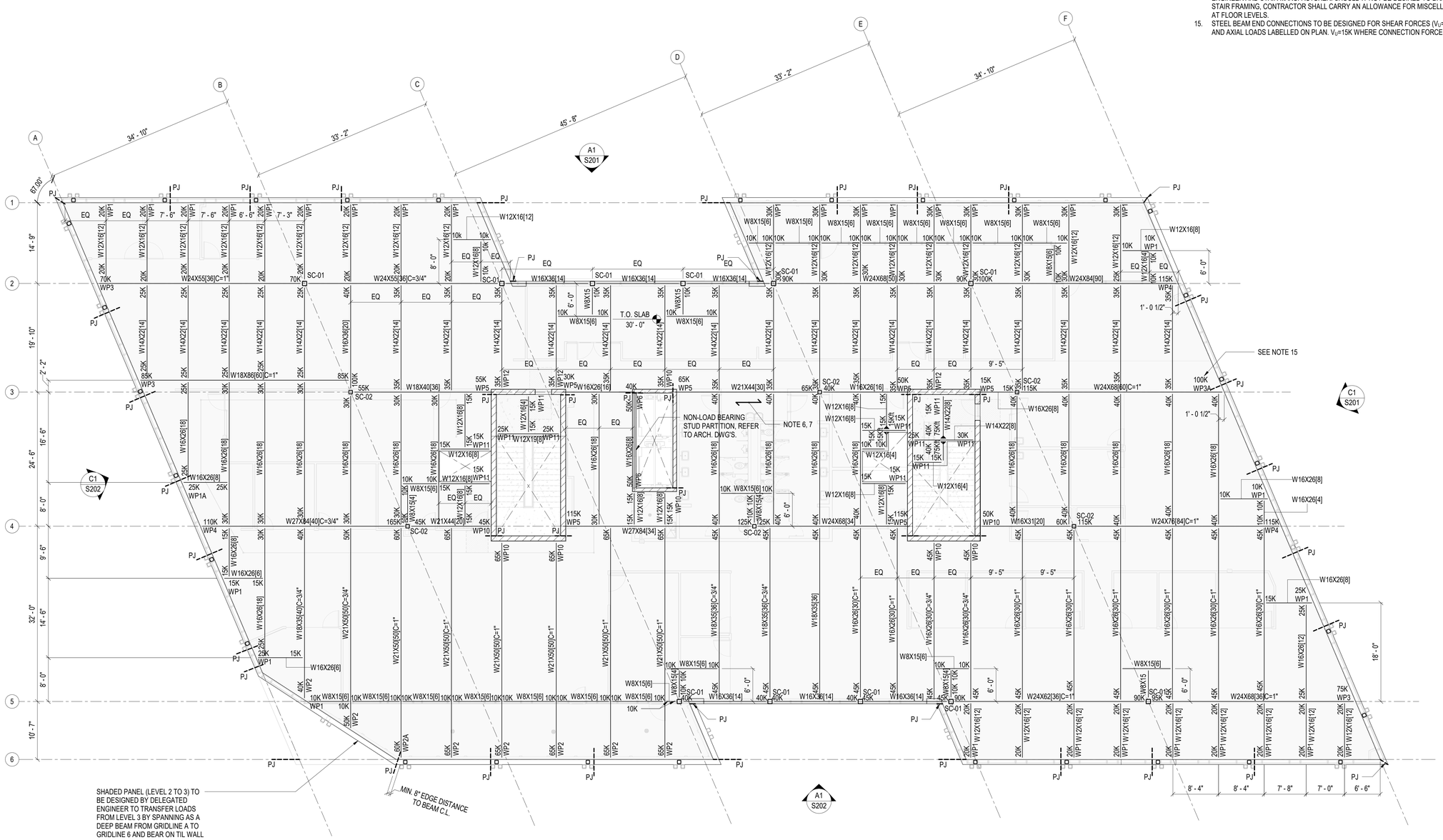
LEVEL 2 FRAMING PLAN

SHEET NUMBER:
S102

THE LINE SHOWN ABOVE IS 3" LONG ON ORIGINAL PLOT. IF NOT SCALE ACCORDINGLY.

FLOOR FRAMING PLAN NOTES:

1. CIVIL DRAWINGS ELEVATION 12'-0" = DATUM ELEVATION 0'-0".
2. TOP OF SLAB ELEVATION IS INDICATED IN PLAN.
3. ALL DIMENSIONS HAVE BEEN GENERATED BY THE ARCHITECT AND SHOULD BE VERIFIED WITH THE LATEST SET OF ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.
4. REFER TO ARCHITECTURAL DRAWINGS FOR SLAB DEPRESSIONS, OPENINGS, AND OTHER INFORMATION NOT SHOWN HERE.
5. STRUCTURAL DRAWINGS SHALL NOT BE USED AS ERECTION DRAWINGS.
6. FLOOR CONSTRUCTION SHALL BE 3 1/2" NORMAL WEIGHT CONCRETE ON 2" DEEP (5 1/2" TOTAL THICKNESS), 18 GAUGE COMPOSITE DECK, REINFORCED WITH WWR 6x6-W2.1x2.21. PROVIDE MINIMUM THREE-SPAN CONDITION, NO SINGLE SPANS ALLOWED WITHOUT EOR APPROVAL.
7. METAL DECK FASTENING CRITERIA:
   A. AT INTERMEDIATE SUPPORTS AND AT ENDS WITH 5/8" PUDDLE WELDS AT EACH LOW FLUTE. FASTEN DECK TO MEMBERS RUNNING PARALLEL TO DECK SPAN WITH 5/8" PUDDLE WELDS AT 12" MAX SPACING. NOTE THAT IT WILL BE PERMISSIBLE TO REPLACE A DECK WELD WITH A HEADED STUD (SHEAR CONNECTOR) WELDED THROUGH THE DECK.
   B. AT SIDE LAPS, FASTEN UNITS TOGETHER USING #10 TEK SCREWS AT A SPACING NOT EXCEEDING ONE-HALF OF THE SPAN, OR 36"; THE LESSER VALUE SHALL GOVERN.
8. REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS AT RE-ENTRANT CORNERS.
9. PROVIDE (3)-#6 CONTINUOUS REINFORCEMENT ALONG SLAB PERIMETER AT ALL SLAB EDGES. PROVIDE CLASS 'B' LAP SPLICE AS REQUIRED. LOCATE ALL SUPPLEMENTAL REINFORCING AT SLAB MID-DEPTH.
10. REFER TO MEP DRAWINGS FOR ELEVATED SLAB PENETRATIONS, REINFORCE OPENINGS FOR PENETRATIONS PER TYPICAL DETAIL REQUIREMENTS.
11. CORING THROUGH ELEVATED SLABS WILL NOT BE PERMITTED. PROVIDE SLEEVES AND BLOCKOUTS AS REQUIRED FOR FLOOR BOXES, PIPES, AND OTHER MEP RELATED ITEMS. ALL SLEEVE AND BLOCKOUT LOCATIONS SHALL BE IDENTIFIED ON THE SHOP DRAWINGS AND COORDINATED WITH THE EOR PRIOR TO CONSTRUCTION.
12. IT WILL NOT BE PERMISSIBLE TO RUN CONDUIT, PIPES, OR OTHER MISCELLANEOUS NON-STRUCTURAL ITEMS WITHIN THE ELEVATED SLAB.
13. FINAL PLAN LOCATIONS, ELEVATIONS, QUANTITIES, ETC... FOR ALL ELEVATOR STEEL SHALL BE COORDINATED WITH ARCHITECTURAL & ELEVATOR MANUFACTURER DRAWINGS.
14. DELEGATED STAIR ENGINEER SHALL DESIGN THE STAIR FRAMING (HSS STRINGERS OR STANDARD MC SHAPES WITH ADEQUATE LATERAL CAPACITY) TO BRACE TILT-PANELS. CONTRACTOR SHALL COORDINATE EMBEDDED CONNECTIONS IN TILT-PANELS BETWEEN DELEGATED TILT-PANEL ENGINEER AND STAIR MANUFACTURER. SHOULD IT NOT BE DESIRED TO BRACE TILT-PANELS WITH STAIR FRAMING, CONTRACTOR SHALL CARRY AN ALLOWANCE FOR MISCELLANEOUS HSS BRACING AT FLOOR LEVELS.
15. STEEL BEAM END CONNECTIONS TO BE DESIGNED FOR SHEAR FORCES (V_U=..K), MOMENTS (M_U=..Kft) AND AXIAL LOADS LABELLED ON PLAN. V_U=15K WHERE CONNECTION FORCES ARE NOT SHOWN.



A1 / S103 **LEVEL 3 FRAMING PLAN**
SCALE: 1/8" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
www.mcsal.com FL CA#26616

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815 SOUTH COLORADO AVE
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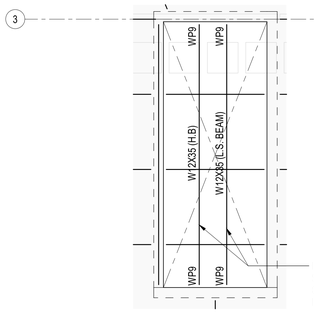
PROJECT NO:
240089.00

LEVEL 3 FRAMING PLAN

SHEET NUMBER:
S103

THE LINE SHOWN ABOVE IS 3" LONG ON ORIGINAL ANSI D SHEETS ORIGINAL PAPER SIZE

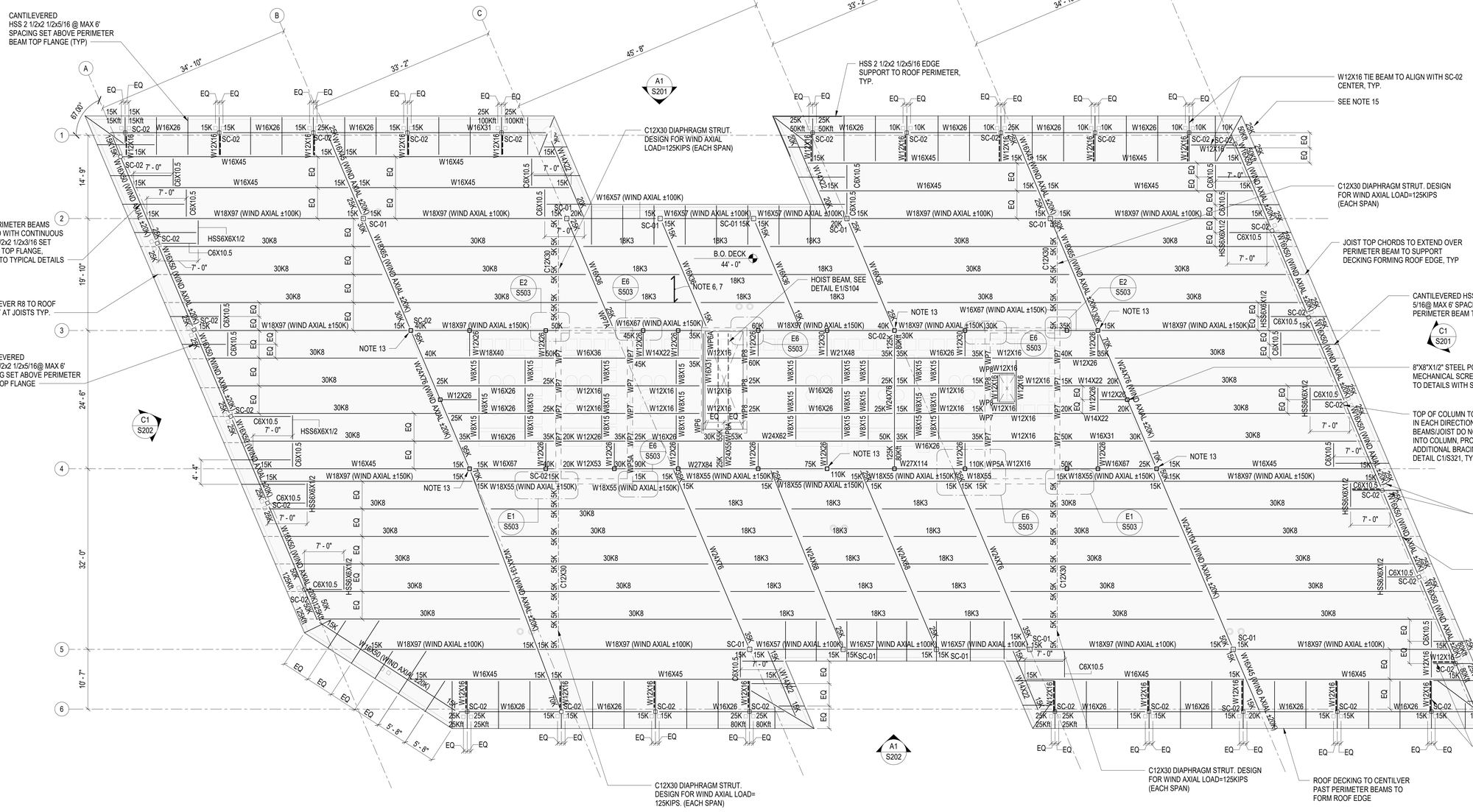


E1 / S104 HOIST BEAM FRAMING PLAN

SCALE: 1/4" = 1'-0"

ROOF FRAMING PLAN NOTES:

1. CIVIL DRAWINGS ELEVATION 12'-0" = DATUM ELEVATION 0'-0".
2. BOTTOM OF DECK ELEVATION VARIES. REFER TO PLAN.
3. ALL DIMENSIONS HAVE BEEN GENERATED BY THE ARCHITECT AND SHOULD BE VERIFIED WITH THE LATEST SET OF ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.
4. REFER TO ARCHITECTURAL DRAWINGS FOR OPENINGS, AND OTHER INFORMATION NOT SHOWN HERE.
5. STRUCTURAL DRAWINGS SHALL NOT BE USED AS ERECTION DRAWINGS.
6. ROOF CONSTRUCTION SHALL BE 1 1/2" DEEP, TYPE B, 18 GAUGE METAL ROOF DECK. PROVIDE MINIMUM THREE-SPAN CONDITION, NO SINGLE SPANS ALLOWED WITHOUT EOR APPROVAL.
7. METAL DECK FASTENING CRITERIA:
   A. AT INTERMEDIATE SUPPORTS AND AT ENDS WITH 3/4" PUDDLE WELDS IN A 36/7 PATTERN.
   B. AT SIDE LAPS AND MEMBERS PARALLEL TO DECK SPAN, FASTEN USING 12 TEK SCREWS AT 8".
8. REFER TO MEP DRAWINGS FOR DECK PENETRATIONS, REINFORCE PENETRATIONS PER TYPICAL DETAIL REQUIREMENTS.
9. ALL DECK PENETRATION LOCATIONS SHALL BE IDENTIFIED ON THE SHOP DRAWINGS AND COORDINATED WITH THE EOR PRIOR TO CONSTRUCTION.
10. EQUIPMENT WEIGHT AND LOCATIONS TO BE CONFIRMED BY ARCHITECT.
11. FINAL PLAN LOCATIONS, ELEVATIONS, QUANTITIES, ETC... FOR ALL ELEVATOR STEEL SHALL BE COORDINATED WITH ARCHITECTURAL & ELEVATOR MANUFACTURER DRAWINGS.
12. WHERE TOP OF BEAM AND GIRDER ELEVATIONS DIFFER, DELEGATED CONNECTION ENGINEER SHALL DESIGN HAUNCHED CONNECTIONS AS REQUIRED BASED ON MEMBER GEOMETRY CONSTRAINTS.
13. COLUMNS CONTINUE ABOVE ROOF TO FORM SURROUND FRAMING TO MECHANICAL ROOF SPACE
14. ALL JOISTS TO BE DETAILED TO ALSO BRACE W-SECTION BOTTOM FLANGES.
15. STEEL BEAM END CONNECTIONS TO BE DESIGNED FOR SHEAR FORCES (V_U=..K), MOMENTS (M_U=..Kft) AND AXIAL LOADS LABELLED ON PLAN. V_U=15K WHERE CONNECTION FORCES ARE NOT SHOWN.



A1 / S104 ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

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189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
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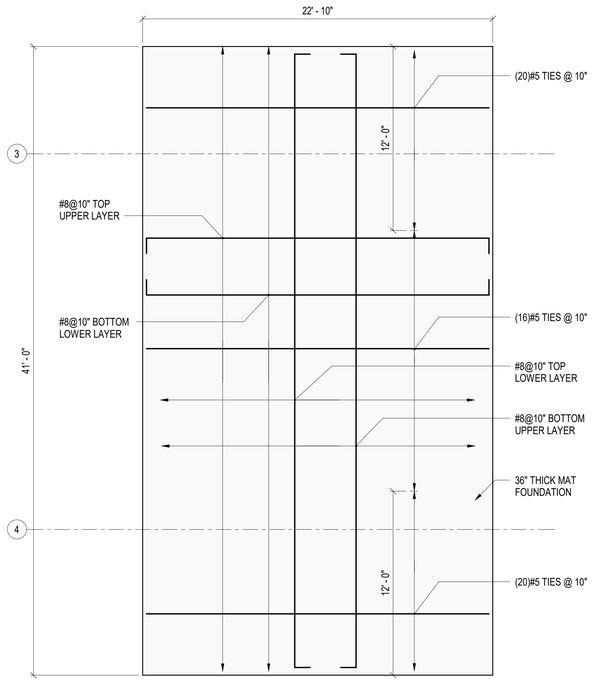
PROJECT NO:
240089.00

ROOF FRAMING PLAN

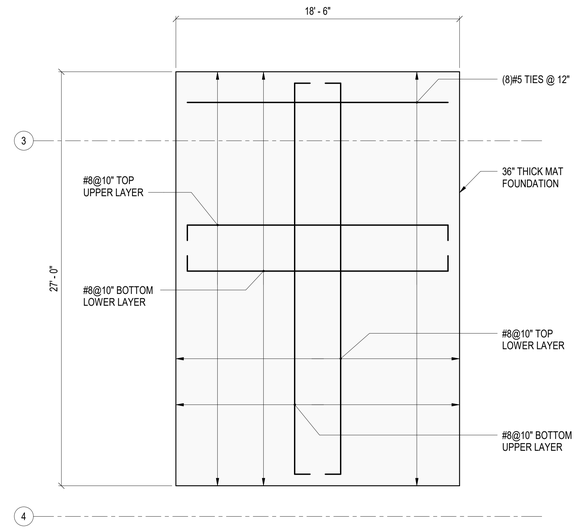
SHEET NUMBER:
S104

THE LINE SHOWN ABOVE IS
1/2" LONG ON ORIGINAL
SHEETS ORIGINAL PAGE SIZE



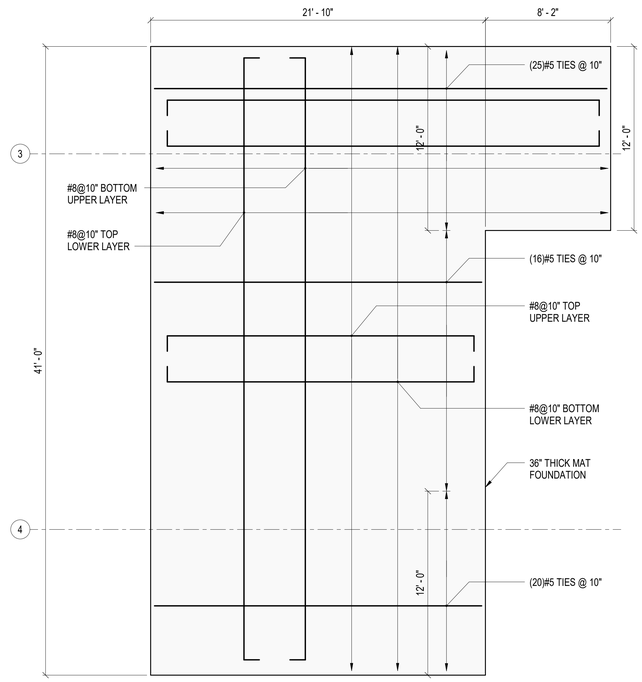
E1 / S105 MAT FOUNDATION REINFORCING PLAN

SCALE: 1/4" = 1'-0"



E3 / S105 MAT FOUNDATION REINFORCING PLAN

SCALE: 1/4" = 1'-0"



E5 / S105 MAT FOUNDATION REINFORCING PLAN

SCALE: 1/4" = 1'-0"

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189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
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PROJECT NO:
240089.00

MAT FOUNDATION REINFORCING PLAN

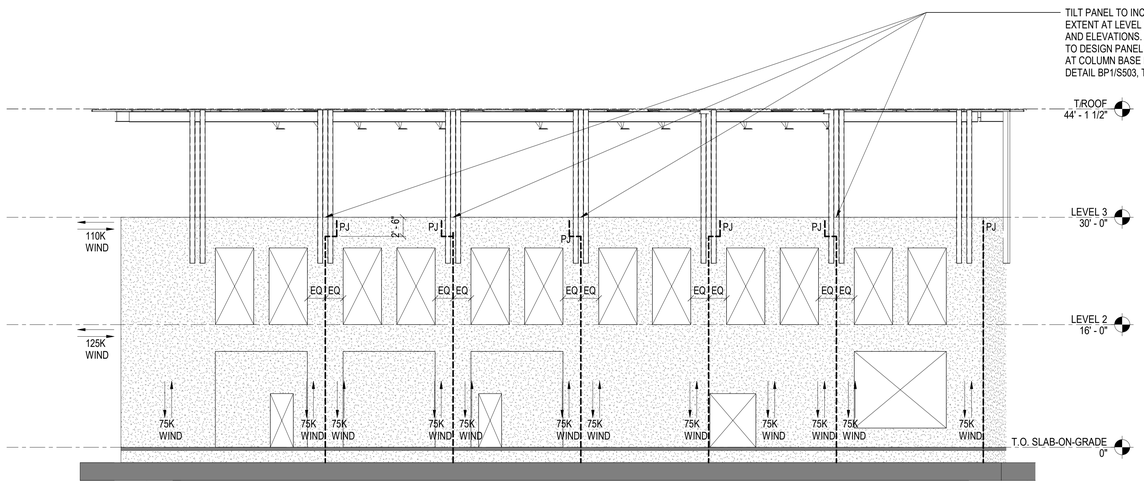
SHEET NUMBER:
S105

THE LINE SHOWN ABOVE IS 1" LONG ON ORIGINAL PAPER SIZE SHEETS ORIGINAL PAPER SIZE

NOTES:

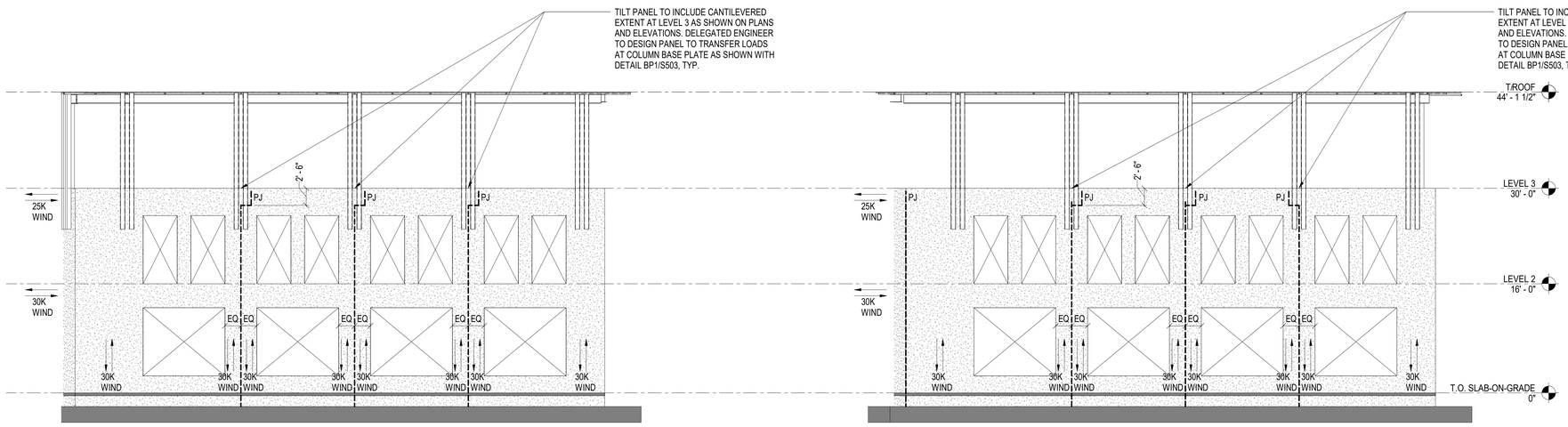
1. TILT-UP CONCRETE WALL PANELS SHALL BE DESIGNED BY A DELEGATED ENGINEER LICENSED IN THE STATE OF FLORIDA. PANELS ARE SHOWN HERE SCHEMATICALLY. REFER TO ARCHITECTURAL DRAWINGS FOR REVEALS, RECESSES, AND ADDITIONAL REQUIREMENTS.
2. CANTILEVER AND BACKSPAN TO BE POURED MONOLITHICALLY. DELEGATED TILT PANEL ENGINEER SHALL DESIGN CANTILEVER TO SUPPORT LATERAL / GRAVITY LOADS (AS REQUIRED) IMPOSED BY ORTHOGONAL CANTILEVERED PANEL. CANTILEVERED PANEL SHALL NOT IMPOSE GRAVITY LOADS ON STEEL ROOF FRAMING.
3. DELEGATED ENGINEER IS RESPONSIBLE FOR THE DESIGN AND SETTING OUT OF PANEL JOINT CONNECTIONS. PROVIDE A MINIMUM OF 2 CONNECTIONS PER LEVEL WITH JOINTS PROVIDED AS PER THE ELEVATIONS. JOINTS TO AVOID BEAM AND JOIST EMBED CONNECTIONS. PANEL JOINTS TO BE DESIGNED SUCH THAT EACH WALL ELEVATION ACTS AS A UNIFORM SECTION WHEN RESISTING LATERAL LOADS.

TILT PANEL KEY	
	8" THICK
	11 1/4" THICK



C1 S201 **ELEVATION - EXTERIOR - EAST**

SCALE: 1/8" = 1'-0"



A1 S201 **ELEVATION - EXTERIOR - NORTH**

SCALE: 1/8" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
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Suite 730
Orlando, FL 32801 407-955-5992
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ELIE EL ZGHAYAR
LICENSE
No.78483
STATE OF FLORIDA
PROFESSIONAL ENGINEER

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Elie El Zghayar, PhD, PE on **06/10/2026**
PE, PE No. 78483
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TAMBONE COMPANIES

SEACOAST BANK HEADQUARTERS

815 SOUTH COLORADO AVE
STUART, FL 34994

PROJECT NO:
240089.00

ELEVATIONS - EXTERIOR BUILDING

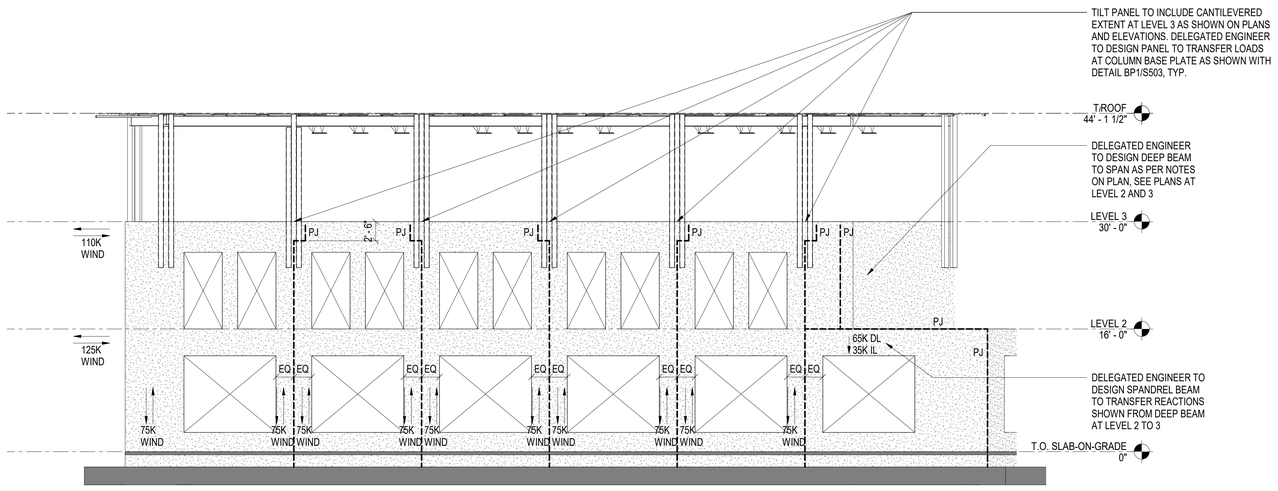
SHEET NUMBER:
S201

THE LINE SHOWN ABOVE IS 1" LONG ON ORIGINAL PRINTS SHEET'S ORIGINAL PAPER SIZE

NOTES:

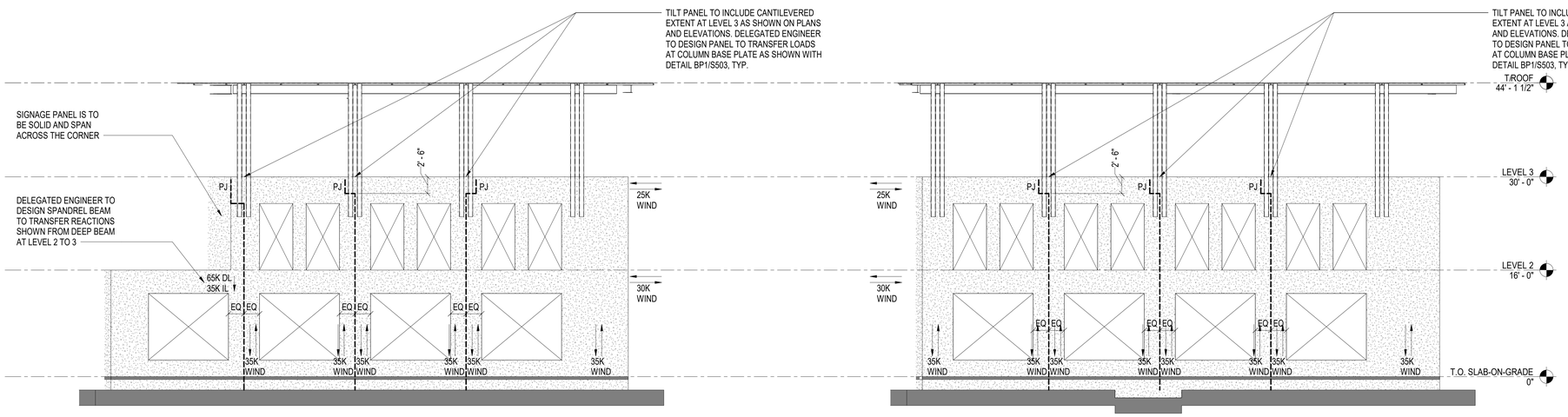
1. TILT-UP CONCRETE WALL PANELS SHALL BE DESIGNED BY A DELEGATED ENGINEER LICENSED IN THE STATE OF FLORIDA. PANELS ARE SHOWN HERE SCHEMATICALLY. REFER TO ARCHITECTURAL DRAWINGS FOR REVEALS, RECESSES, AND ADDITIONAL REQUIREMENTS.
2. CANTILEVER AND BACKSPAN TO BE POURED MONOLITHICALLY. DELEGATED TILT PANEL ENGINEER SHALL DESIGN CANTILEVER TO SUPPORT LATERAL / GRAVITY LOADS (AS REQUIRED) IMPOSED BY ORTHOGONAL CANTILEVERED PANEL. CANTILEVERED PANEL SHALL NOT IMPOSE GRAVITY LOADS ON STEEL ROOF FRAMING.
3. DELEGATED ENGINEER IS RESPONSIBLE FOR THE DESIGN AND SETTING OUT OF PANEL JOINT CONNECTIONS. PROVIDE A MINIMUM OF 2 CONNECTIONS PER LEVEL WITH JOINTS PROVIDED AS PER THE ELEVATIONS. JOINTS TO AVOID BEAM AND JOIST EMBED CONNECTIONS. PANEL JOINTS TO BE DESIGNED SUCH THAT EACH WALL ELEVATION ACTS AS A UNIFORM SECTION WHEN RESISTING LATERAL LOADS.

TILT PANEL KEY	
	8" THICK
	11 1/4" THICK



C1 / S202 **ELEVATION - EXTERIOR - WEST**

SCALE: 1/8" = 1'-0"



A1 / S202 **ELEVATION - EXTERIOR - SOUTH**

SCALE: 1/8" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
www.mcsal.com FL CA#26616

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ELIE EL ZGHAYAR LICENSE No.78483 STATE OF FLORIDA PROFESSIONAL ENGINEER

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TAMBONE COMPANIES

SEACOAST BANK HEADQUARTERS

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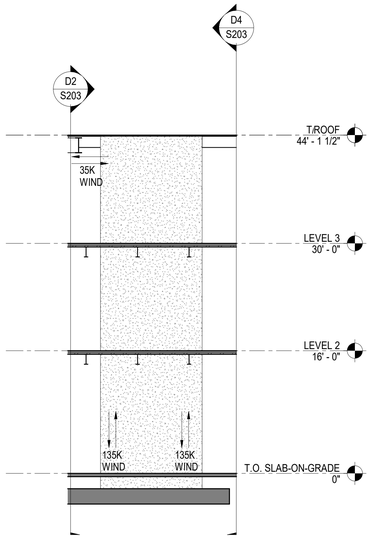
STUART, FL 34994

PROJECT NO:
240089.00

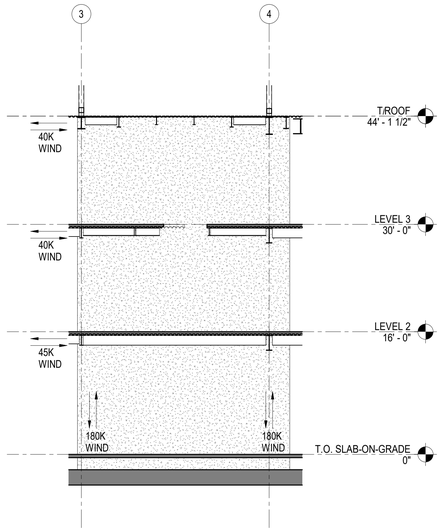
ELEVATIONS - EXTERIOR BUILDING

SHEET NUMBER:
S202

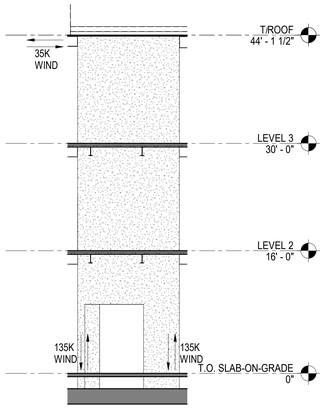
THE LINE SHOWN ABOVE IS 1/2" LONG ON ORIGINAL 24x36 SHEETS ORIGINAL PAPER SIZE



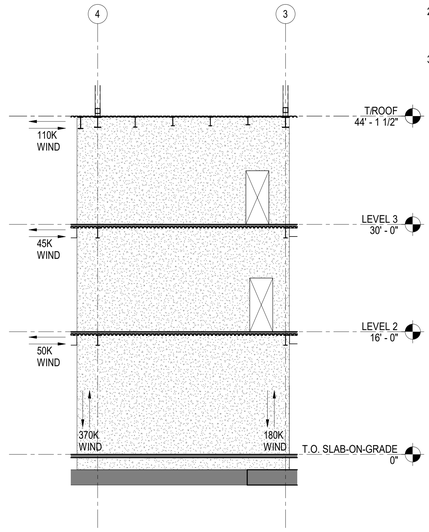
D1 S203 **ELEVATION - INTERIOR - SOUTH**
SCALE: 1/8" = 1'-0"



D2 S203 **ELEVATION - INTERIOR - WEST**
SCALE: 1/8" = 1'-0"



D3 S203 **ELEVATION - INTERIOR- NORTH**
SCALE: 1/8" = 1'-0"



D4 S203 **ELEVATION - INTERIOR - EAST**
SCALE: 1/8" = 1'-0"

NOTES:

1. TILT-UP CONCRETE WALL PANELS SHALL BE DESIGNED BY A DELEGATED ENGINEER LICENSED IN THE STATE OF FLORIDA. PANELS ARE SHOWN HERE SCHEMATICALLY. REFER TO ARCHITECTURAL DRAWINGS FOR REVEALS, RECESSES, AND ADDITIONAL REQUIREMENTS.
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TILT PANEL KEY	
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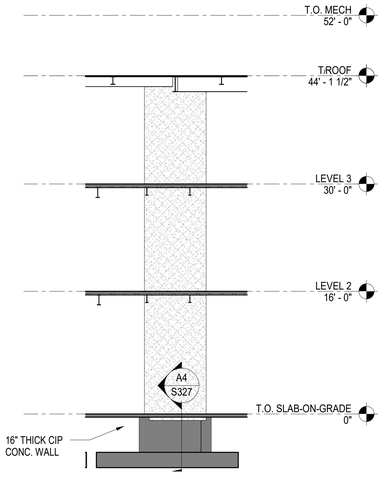
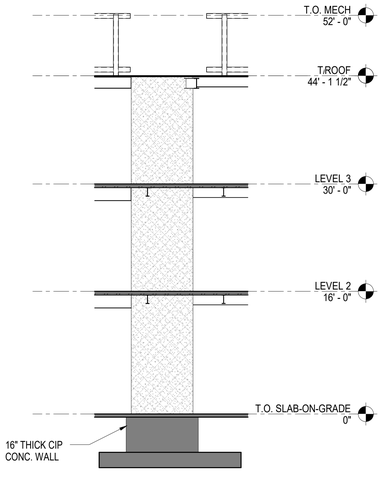
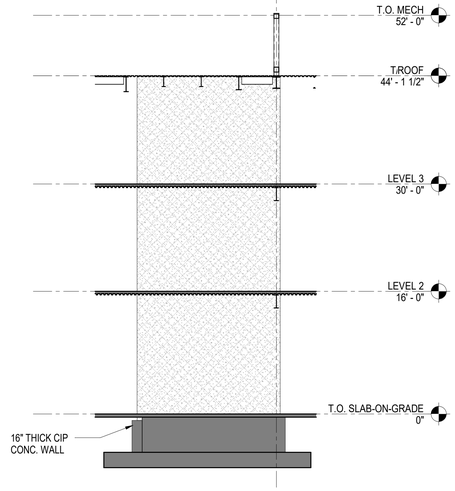
	11 1/4" THICK



C1 S203 **ELEVATION - INTERIOR - SOUTH**
SCALE: 1/8" = 1'-0"



C2 S203 **ELEVATION - INTERIOR - NORTH**
SCALE: 1/8" = 1'-0"



C3 S203 **ELEVATION - INTERIOR - EAST**
SCALE: 1/8" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
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AA0002981 | LC26000427

McNamara-Salvia
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240089.00

ELEVATIONS - INTERIOR BUILDING

SHEET NUMBER:
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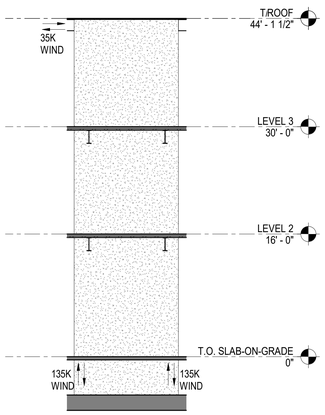
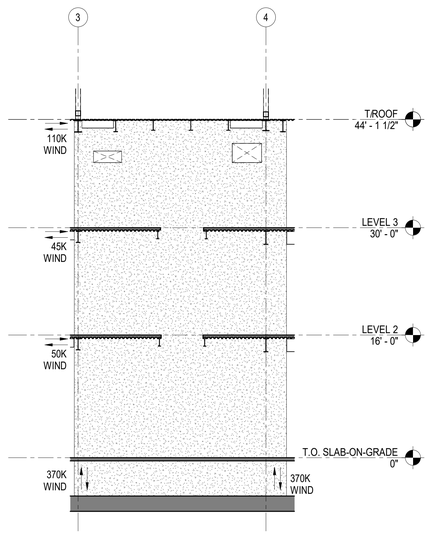
THE LINE SHOWN ABOVE IS 1" LONG ON ORIGINAL PLOT. IF NOT, SHEET IS ORIGINAL PAPER SIZE



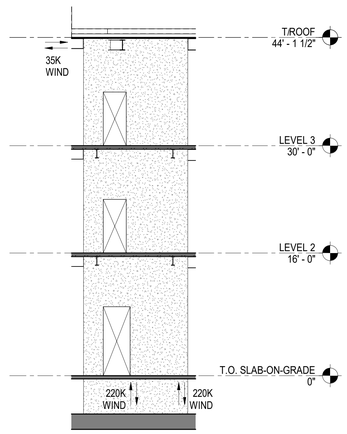
D1 S204 **ELEVATION - INTERIOR - SOUTH**

SCALE: 1/8" = 1'-0"



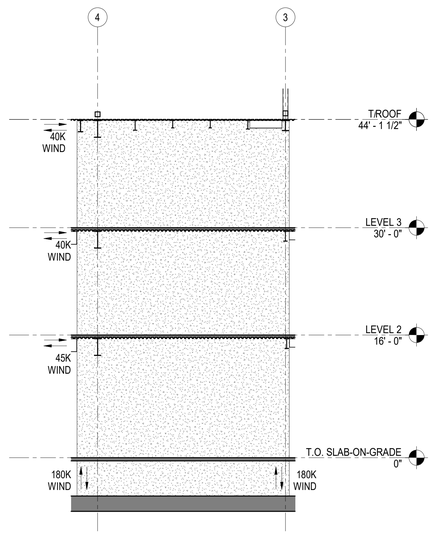
D2 S204 **ELEVATION - INTERIOR - WEST**

SCALE: 1/8" = 1'-0"



D3 S204 **ELEVATION - INTERIOR - NORTH**

SCALE: 1/8" = 1'-0"



D4 S204 **ELEVATION - INTERIOR - EAST**

SCALE: 1/8" = 1'-0"

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ELEVATIONS - INTERIOR BUILDING

SHEET NUMBER:
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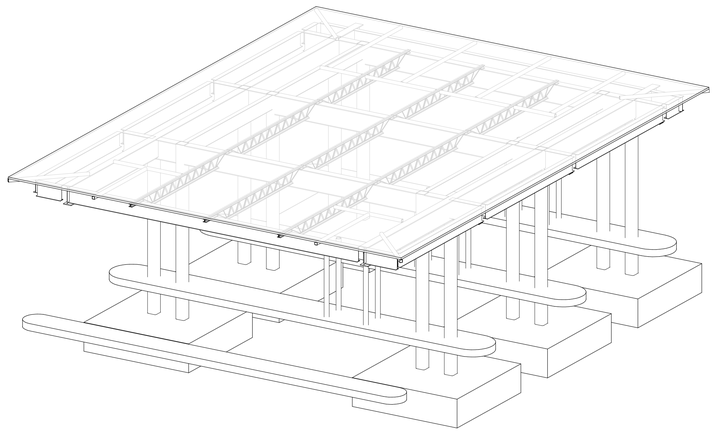
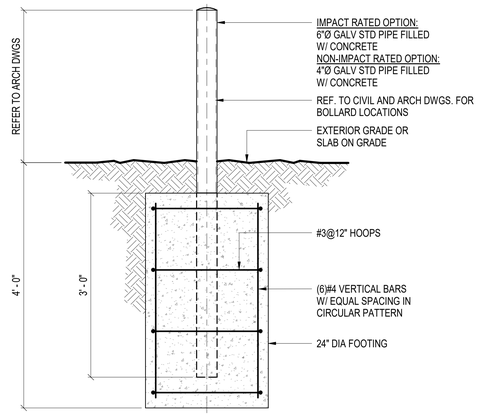
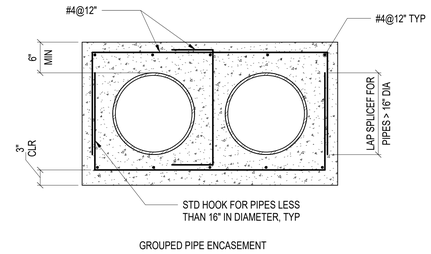




D4 / S210 **TYPICAL BOLLARD AT GRADE DETAIL**

SCALE: 1" = 1'-0"



GROUPED PIPE ENCASEMENT

D6 / S210 **PIPE ENCASEMENT DETAIL**

SCALE: 1" = 1'-0"

STEEL COLUMN AND BASE PLATE SCHEDULE-per sheet

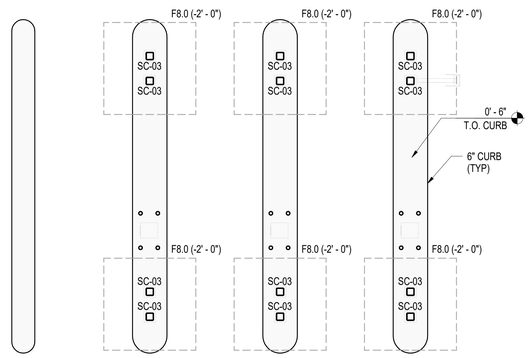
MARK	SIZE	BASE PLATE DIMENSIONS			FILLET WELD SIZE	ANCHOR RODS	REMARKS
		LENGTH	WIDTH	THICKNESS			
SC-03	HSS8X8X5/8	14"	14"	1 1/4"	5/16"	(4) 1"	PROVIDE (2)#5-U-BARS ADJACENT ANCHORS, EXTEND U-BAR ENDS TDL

STEEL COLUMN AND BASE PLATE SCHEDULE NOTES:

1. ANCHOR RODS SHALL BE EMBEDDED A MINIMUM OF 1'-0" INTO THE FOUNDATION, UNLESS NOTED OTHERWISE.
2. REFER TO SCHEDULES WITH S501

SPREAD FOOTING SCHEDULE-PER SHEET

MARK	WIDTH	LENGTH	THICKNESS	REINFORCEMENT		REMARKS
				BOTTOM	TOP	
F8.0	8' - 0"	8' - 0"	24"	(10)#7 EACH WAY	(10)#7 EACH WAY	

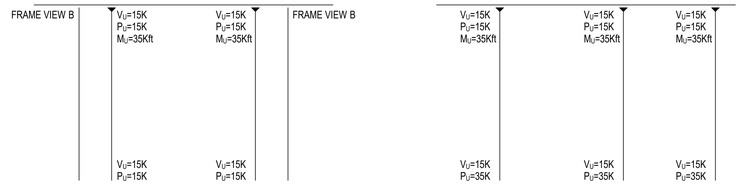


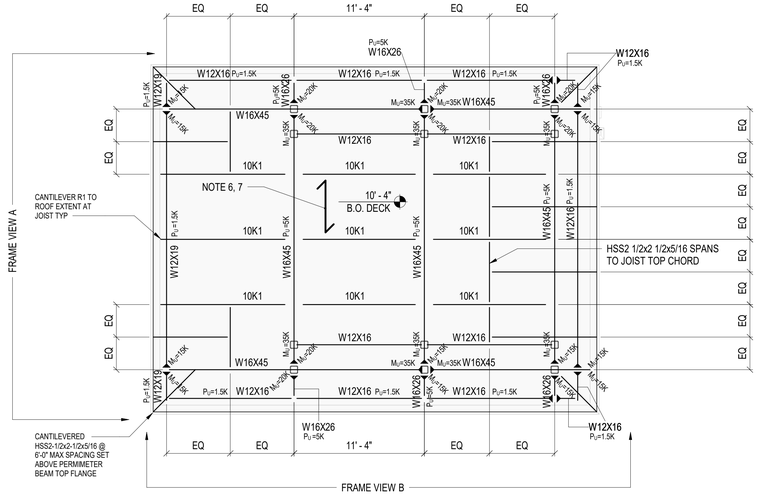
A1 / S210 **SLAB ON GRADE PLAN - BANK DRIVE-THRU**

SCALE: 3/16" = 1'-0"

FOUNDATION & SLAB-ON-GRADE PLAN NOTES:

1. FOUNDATION DETAILS SHOWN ARE PLACE HOLDERS.
2. CIVIL DRAWINGS ELEVATION 12'-0" = DATUM ELEVATION 0'-0".
3. REFER TO PLAN FOR TOP OF FOUNDATION ELEVATION. TOP OF SLAB-ON-GRADE ELEVATION SHALL BE 0'-0" UNLESS OTHERWISE NOTED.
4. ALL DIMENSIONS HAVE BEEN GENERATED BY THE ARCHITECT AND SHOULD BE VERIFIED WITH THE LATEST SET OF ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.
5. REFER TO ARCHITECTURAL FOR SLAB DEPRESSIONS, OPENINGS, AND OTHER INFORMATION NOT SHOWN HERE.
6. STRUCTURAL DRAWINGS SHALL NOT BE USED AS ERECTION DRAWINGS.
7. SLAB-ON-GRADE CONSTRUCTION: SHALL BE NORMAL-WEIGHT CONCRETE. THICKNESS AND REINFORCING SHALL BE AS FOLLOWS:
   A. TYPICAL SLABS-ON-GRADE SHALL BE 6" THICK REINFORCED WITH WWR 4x4-W4.0xW4.0 OR WWR 6x6-W5.5xW5.5.
8. MAINTAIN SLAB-ON-GRADE THICKNESS AT ALL DEPRESSIONS AND SLOPES. PROVIDE CONTROL JOINTS IN AN ORTHOGONAL GRID, AT A SPACING NOT EXCEEDING 12'-0" AND WITH A PLAN ASPECT RATION NOT EXCEEDING 2:1. PROVIDE A VAPOR BARRIER AS SPECIFIED B THE WATERPROOFING CONSULTANT AND/OR ARCHITECT.
9. REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS AT CONTINUOUS FOUNDATION CORNERS & INTERSECTIONS, AS WELL AS AT SLAB REENTRANT CORNERS, THICKENED SLAB CORNERS & INTERSECTIONS.
10. FOUNDATIONS (CONTINUOUS & ISOLATED) SHALL BE CENTERED ON THEIR RESPECTIVE SUPPORTED ELEMENT UNLESS NOTED OTHERWISE.
11. REFER TO CIVIL AND MEP DRAWINGS FOR SLAB PENETRATION LOCATIONS & UNDERGROUND UTILITY INFORMATION. INSTALLATION OF UNDERGROUND UTILITIES SHALL NOT UNDERMINE FOUNDATION OR SLAB ELEMENTS. CONTRACTOR TO COORDINATE SEQUENCING AS REQUIRED.
12. CORING THROUGH THE SLAB-ON-GRADE IS NOT PERMITTED. IN LIEU OF CORING, PROVIDE SLEEVES CAST-IN WITH THE SLAB POUR. SLEEVE SIZES AND LOCATIONS SHALL BE IDENTIFIED ON THE SHOP DRAWINGS AND COORDINATED WITH THE EOR PRIOR TO CONSTRUCTION.
13. FINAL PLAN LOCATIONS, ELEVATIONS, QUANTITIES, ETC... FOR ALL ELEVATOR STEEL SHALL BE COORDINATED WITH ARCHITECTURAL & ELEVATOR MANUFACTURER DRAWINGS.





A4 / S210 **ROOF FRAMING PLAN - BANK DRIVE-THRU**

SCALE: 3/16" = 1'-0"

ROOF FRAMING PLAN NOTES:

1. CIVIL DRAWINGS ELEVATION 12'-0" = DATUM ELEVATION 0'-0".
2. BOTTOM OF DECK ELEVATION VARIES. REFER TO PLAN.
3. ALL DIMENSIONS HAVE BEEN GENERATED BY THE ARCHITECT AND SHOULD BE VERIFIED WITH THE LATEST SET OF ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.
4. REFER TO ARCHITECTURAL DRAWINGS FOR OPENINGS, AND OTHER INFORMATION NOT SHOWN HERE.
5. STRUCTURAL DRAWINGS SHALL NOT BE USED AS ERECTION DRAWINGS.
6. ROOF CONSTRUCTION SHALL BE 1 1/2" DEEP, TYPE B, 20 GAUGE METAL ROOF DECK. PROVIDE MINIMUM THREE-SPAN CONDITION, NO SINGLE SPANS ALLOWED WITHOUT EOR APPROVAL.
7. METAL DECK FASTENING CRITERIA:
   A. AT INTERMEDIATE SUPPORTS AND AT ENDS WITH 3/4" PUDDLE WELDS IN A 36/7 PATTERN.
   B. AT SIDE LAPS AND MEMBERS PARALLEL TO DECK SPAN, FASTEN USING 12 TEK SCREWS AT 8".
8. REFER TO MEP DRAWINGS FOR DECK PENETRATIONS, REINFORCE PENETRATIONS PER TYPICAL DETAIL REQUIREMENTS.
9. ALL DECK PENETRATION LOCATIONS SHALL BE IDENTIFIED ON THE SHOP DRAWINGS AND COORDINATED WITH THE EOR PRIOR TO CONSTRUCTION.
10. EQUIPMENT WEIGHT AND LOCATIONS TO BE CONFIRMED BY ARCHITECT.
11. FINAL PLAN LOCATIONS, ELEVATIONS, QUANTITIES, ETC... FOR ALL ELEVATOR STEEL SHALL BE COORDINATED WITH ARCHITECTURAL & ELEVATOR MANUFACTURER DRAWINGS.
12. WHERE TOP OF BEAM AND GIRDER ELEVATIONS DIFFER, DELEGATED CONNECTION ENGINEER SHALL DESIGN HAUNCHED CONNECTIONS AS REQUIRED BASED ON MEMBER GEOMETRY CONSTRAINTS.
13. COLUMNS CONTINUE ABOVE ROOF TO FORM SURROUND FRAMING TO MECHANICAL ROOF SPACE
14. ALL JOISTS TO BE DETAILED TO ALSO BRACE W-SECTION BOTTOM FLANGES.
15. DESIGN ALL BEAM CONNECTIONS TO RESIST
    V_U=15K
    P_U=AXIAL LOAD-SEE PLAN
    M_U=DESIGN MOMENT-SEE PLAN
    CONSIDER ALL LOADS AS REVERSIBLE

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
www.mcsal.com FL CA#26616

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ELIE EL ZGHAIR
LICENSE
No.78483
STATE OF FLORIDA
PROFESSIONAL ENGINEER

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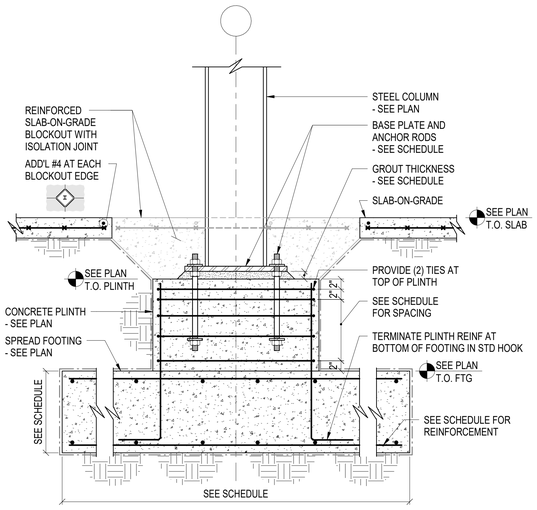
240089.00

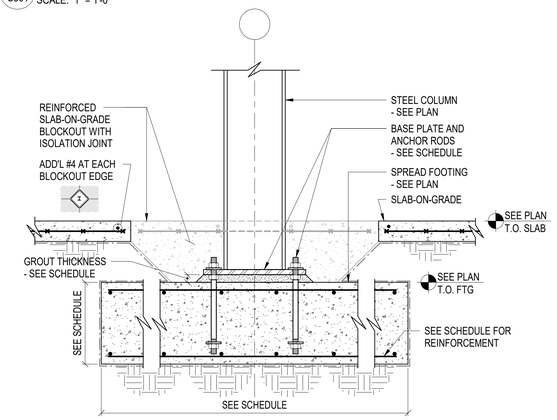
PARTIAL PLANS - BANK DRIVE-THRU

SHEET NUMBER:
S210

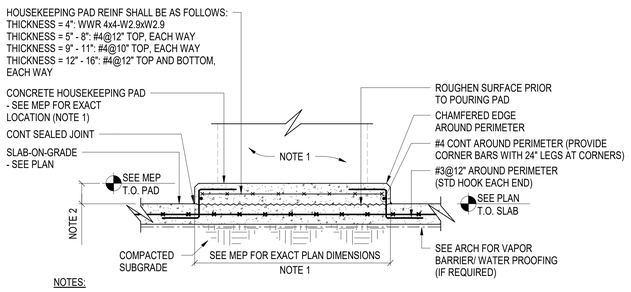
THE LINE SHOWN ABOVE IS 1" LONG ON ORIGINAL DRAWINGS. SCALE IF ORIGINAL IMAGE SIZE IS SMALLER.



D1 / S301 **TYPICAL STEEL COLUMN ON SPREAD FOOTING (PLINTH)**
SCALE: 1" = 1'-0"



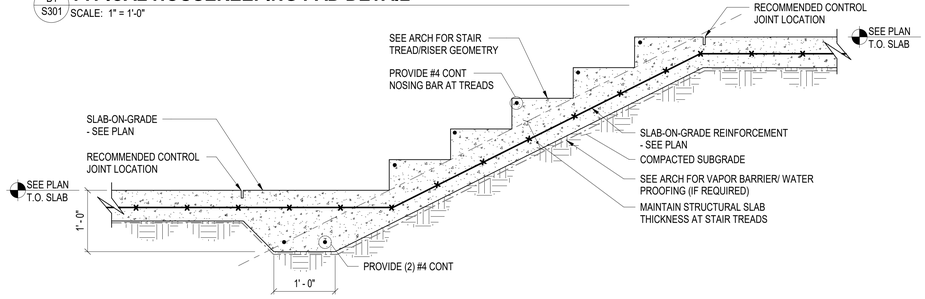
C1 / S301 **TYPICAL STEEL COLUMN ON SPREAD FOOTING**
SCALE: 1" = 1'-0"



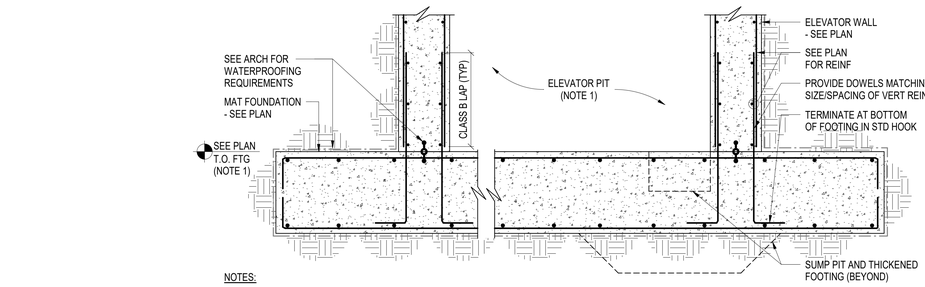
NOTES:

1. SEE THE MEP/ARCHITECTURAL PLANS FOR THE EXACT LOCATION AND ALL OTHER INFORMATION REGARDING EQUIPMENT TO BE PLACED ON THE CONCRETE HOUSEKEEPING PAD. THE PERMANENT ANCHORAGE OF EQUIPMENT TO THE PAD SHALL BE PROVIDED BY THE CONTRACTOR AND THE EQUIPMENT SUPPLIER.
2. THE TOP OF PAD ELEVATION SHALL BE DETERMINED BASED ON MEP AND ARCHITECTURAL REQUIREMENTS.

B1 / S301 **TYPICAL HOUSEKEEPING PAD DETAIL**
SCALE: 1" = 1'-0"

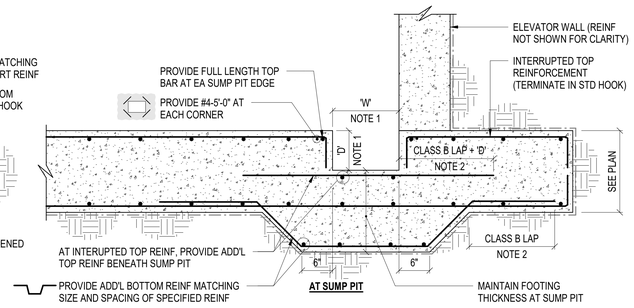


A1 / S301 **TYPICAL STAIR-ON-GRADE DETAIL**
SCALE: 1" = 1'-0"

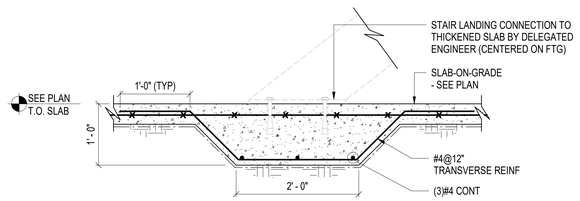


NOTES:

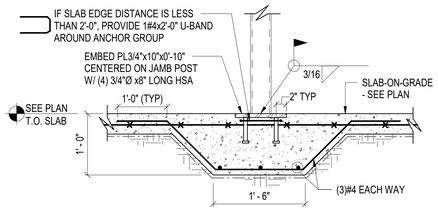
1. GC SHALL COORDINATE THE EXACT ELEVATOR PIT INTERIOR DIMENSIONS, ELEVATOR PIT FOOTING DEPTH, SUMP PIT SIZE AND SUMP PIT LOCATION WITH THE APPROVED ELEVATOR SHOP DRAWINGS.
2. IF CLASS B LAP DISTANCE CANNOT BE ACHIEVED WITH THE STRAIGHT BAR AT THE FOUNDATION EDGE, TERMINATE THE BAR IN A STANDARD HOOK.



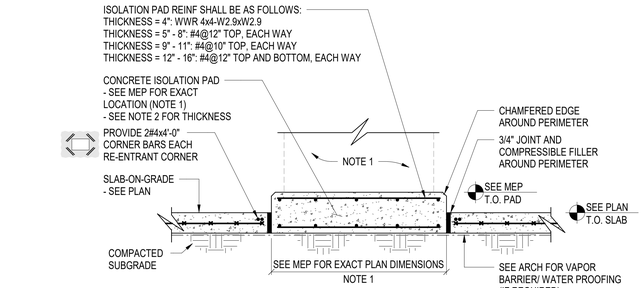
D3 / S301 **TYPICAL ELEVATOR PIT DETAIL**
SCALE: 1" = 1'-0"



D2 / S301 **TYPICAL THICKENED SLAB AT STAIR LANDING**
SCALE: 1" = 1'-0"



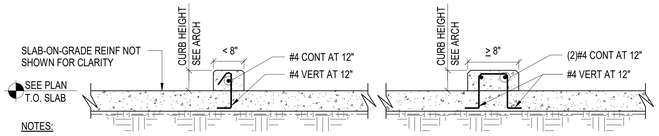
D4 / S301 **TYPICAL THICKENED SLAB AT INTERIOR JAMB POST**
SCALE: 1" = 1'-0"



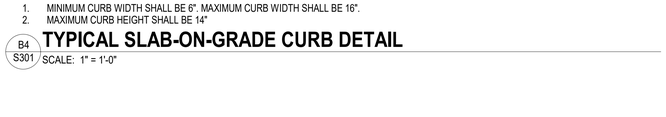
NOTES:

1. SEE THE MEP/ARCHITECTURAL PLANS FOR THE EXACT LOCATION AND ALL OTHER INFORMATION REGARDING EQUIPMENT TO BE PLACED ON THE CONCRETE ISOLATION PAD. THE PERMANENT ANCHORAGE OF EQUIPMENT TO THE PAD SHALL BE PROVIDED BY THE CONTRACTOR AND THE EQUIPMENT SUPPLIER.
2. THE EXACT THICKNESS OF THE CONCRETE PAD SHALL BE DETERMINED BY THE FOLLOWING:
   A. TOP OF PAD ELEVATION SHALL BE DETERMINED BASED ON MEP AND ARCHITECTURAL REQUIREMENTS.
   B. BOTTOM OF PAD SHALL NOT BE ABOVE THE BOTTOM OF SLAB-ON-GRADE ELEVATION.

B2 / S301 **TYPICAL ISOLATION PAD DETAIL**
SCALE: 1" = 1'-0"



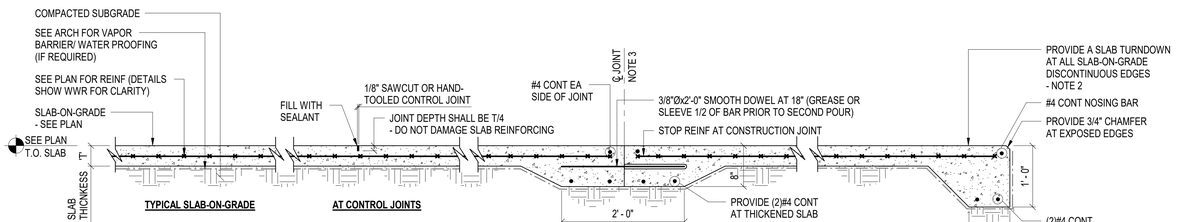
C4 / S301 **TYPICAL SLAB-ON-GRADE STEP DETAILS**
SCALE: 1" = 1'-0"



NOTES:

1. MINIMUM CURB WIDTH SHALL BE 6". MAXIMUM CURB WIDTH SHALL BE 16".
2. MAXIMUM CURB HEIGHT SHALL BE 14"

B4 / S301 **TYPICAL SLAB-ON-GRADE CURB DETAIL**
SCALE: 1" = 1'-0"



NOTES:

1. SAW CUT CONTROL JOINTS SHALL BE PLACED AS SOON AS THE CONCRETE WILL SUPPORT A PERSON'S WEIGHT WITHOUT PERMANENT DEFORMATION AND THE JOINT CUTTING PROCESS DOES NOT DISLODGE AGGREGATES. THE CONTROL JOINT PANEL AREA SHALL NOT EXCEED 15'-0" X 15'-0" AND SHALL BE LOCATED WITH AN ASPECT RATIO AS CLOSE AS POSSIBLE TO 1:1. SUBMIT SAW CUT LOCATIONS TO THE ENGINEER FOR REVIEW PRIOR TO CONSTRUCTION..
2. WHERE A DISCONTINUOUS SLAB EDGE ABUTS A VERTICAL SURFACE (i.e. SLAB OR WALL) PROVIDE 1/2" ISOLATION JOINT WITH COMPRESSIBLE FILLER MATERIAL. THIS DOES NOT APPLY TO SLAB EDGES DETAILED TO CONNECT TO ADJACENT SURFACES.
3. WHERE SLAB-ON-GRADE EXPANSION JOINTS ARE SPECIFIED ON PLAN, PROVIDE A MIN. 3/4" WIDE JOINT BETWEEN BOTH THICKENED EDGE SECTIONS AND PROVIDE A COMPRESSIBLE JOINT FILLER WITHIN THE JOINT GAP AND AN ELASTOMERIC JOINT SEALER AT THE TOP OF THE JOINT. ALL REINFORCING SHOWN FOR THE CONSTRUCTION JOINT IS APPLICABLE FOR THE THICKENED EDGES AT AN EXPANSION JOINT.

A3 / S301 **TYPICAL SLAB-ON-GRADE DETAILS**
SCALE: 1" = 1'-0"

Baker Barrios
ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000
INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
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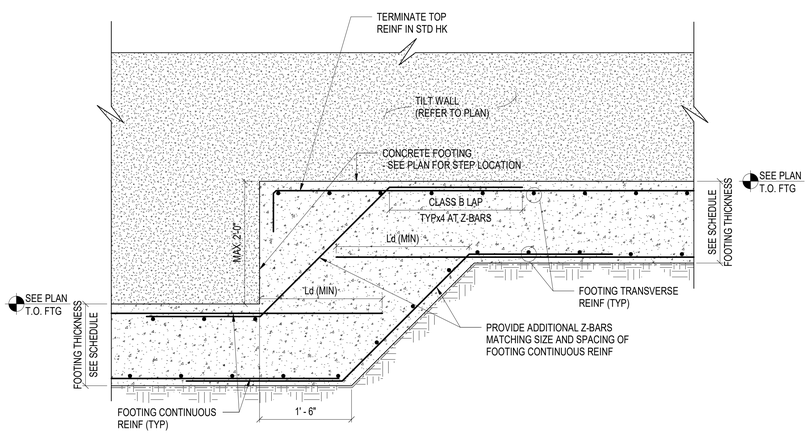
FOUNDATION AND SOG TYPICAL DETAILS

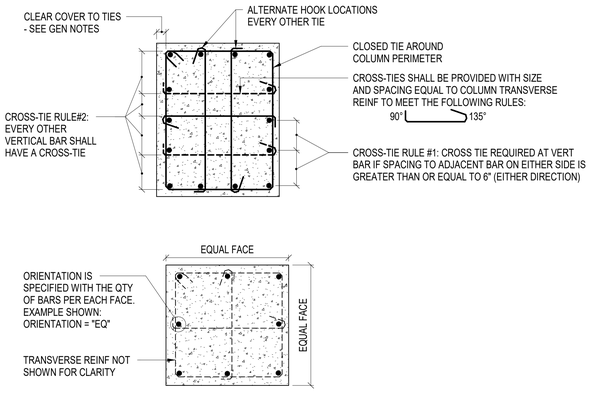
SHEET NUMBER:
S301

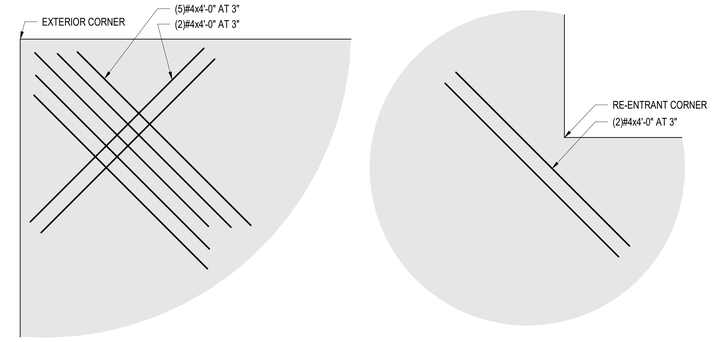
IF THIS LINE SHOWN ABOVE IS NOT 1" LONG, THIS IS NOT A FULL SIZE SHEET. ADJUST ACCORDINGLY.



NOTES:

1. DETAIL ALSO APPLIES TO HELICAL PILE CAPS. REFER TO PLAN FOR STEP LOCATIONS.



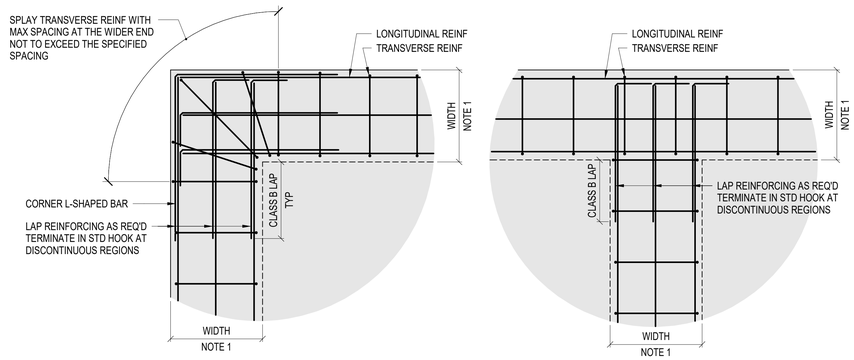


AT EXTERIOR CORNER

AT INTERIOR RE-ENTRANT CORNER

D1 TYPICAL WALL FOOTING STEP AT TILT UP WALL

D1 / S302 SCALE: 1" = 1'-0"



NOTES: AT CORNERS AT INTERSECTIONS

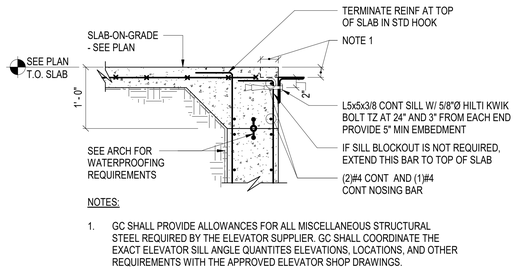
1. THIS DETAIL ALSO APPLIES TO CONTINUOUS THICKENED SLABS, THICKENED SLAB EDGES, WALL FOOTINGS, AND OTHER SIMILAR CONDITIONS.

C1 TYPICAL ADDITIONAL REINFORCEMENT AT CORNERS AND INTERSECTIONS

C1 / S302 SCALE: 1" = 1'-0"

D3 TYPICAL CONCRETE PLINTH REINFORCEMENT

D3 / S302 SCALE: 1" = 1'-0"



NOTES:

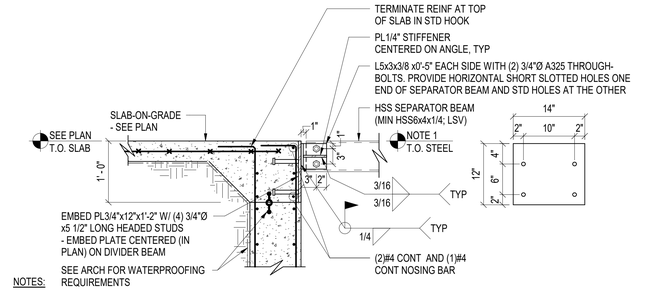
1. GC SHALL PROVIDE ALLOWANCES FOR ALL MISCELLANEOUS STRUCTURAL STEEL REQUIRED BY THE ELEVATOR SUPPLIER. GC SHALL COORDINATE THE EXACT ELEVATOR SILL ANGLE QUANTITES ELEVATIONS, LOCATIONS , AND OTHER REQUIREMENTS WITH THE APPROVED ELEVATOR SHOP DRAWINGS.

C3 TYPICAL ELEVATOR SILL AT SLAB-ON-GRADE

C3 / S302 SCALE: 1" = 1'-0"

D5 TYPICAL SLAB-ON-GRADE REINFORCEMENT AT CORNERS

D5 / S302 SCALE: 1" = 1'-0"

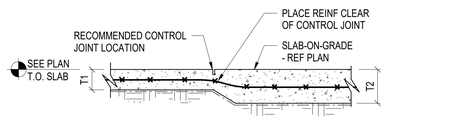


NOTES:

1. GC SHALL PROVIDE ALLOWANCES FOR ALL MISCELLANEOUS STRUCTURAL STEEL REQUIRED BY THE ELEVATOR SUPPLIER. GC SHALL COORDINATE THE EXACT ELEVATOR SEPARATOR BEAM QUANTITIES ELEVATIONS, LOCATIONS, AND OTHER REQUIREMENTS WITH THE APPROVED ELEVATOR SHOP DRAWINGS.

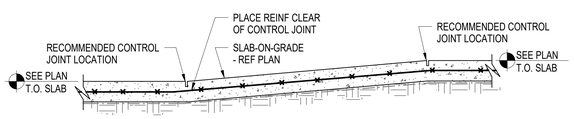
C5 TYPICAL ELEVATOR SEPARATOR BEAM DETAIL

C5 / S302 SCALE: 1" = 1'-0"



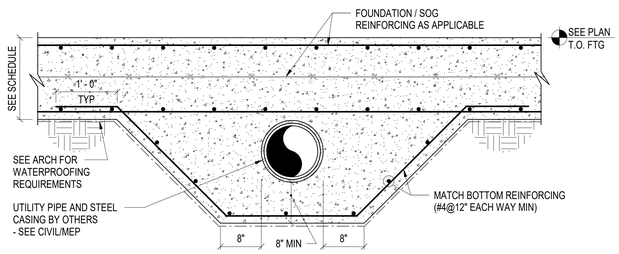
B5 TYPICAL SLAB-ON-GRADE THICKNESS TRANSITION

B5 / S302 SCALE: 1" = 1'-0"



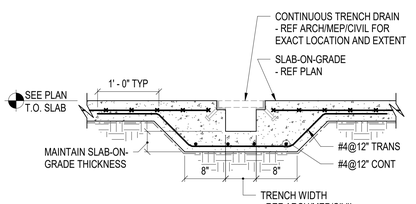
A1 TYPICAL SLAB-ON-GRADE RAMP

A1 / S302 SCALE: 1" = 1'-0"



A3 TYPICAL ENCASED PIPE AT FOUNDATION / SLAB-ON-GRADE

A3 / S302 SCALE: 1" = 1'-0"



A5 TYPICAL SLAB-ON-GRADE TRENCH DRAIN

A5 / S302 SCALE: 1" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
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240089.00

FOUNDATION AND SOG TYPICAL DETAILS

SHEET NUMBER:
S302

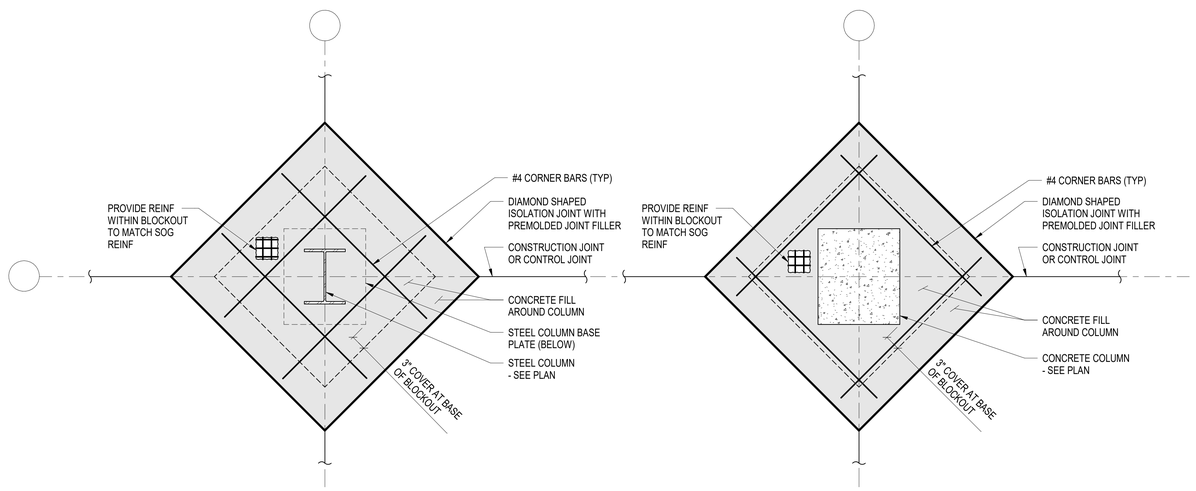
THE LINE SHOWN ABOVE IS 1 INCH LONG ON ARCH D SHEETS ORIGINAL PAGE SIZE

NOTES:

1. NEATLY CUT/REMOVE EXISTING SLAB-ON-GRADE FOR THE INSTALLATION OF NEW UNDERGROUND UTILITIES. THE WIDTH OF TRENCHING SHALL BE DETERMINED AS REQUIRED FOR THE UTILITY INSTALLATION.
2. REFER TO THE CIVIL/MEP PLANS FOR EXACT LOCATIONS OF PROPOSED UNDERGROUND UTILITIES. GC SHALL COORDINATE EXISTING UTILITIES SUCH THAT THEY ARE NOT DAMAGED DURING EXISTING SLAB-ON-GRADE DEMOLITION.

A1 / S303 **SLAB-ON-GRADE TRENCHING DETAIL**

SCALE: 1" = 1'-0"



A3 / S303 **TYPICAL SLAB-ON-GRADE BLOCKOUT AT COLUMN**

SCALE: 1" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
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FOUNDATION AND SOG TYPICAL DETAILS

SHEET NUMBER:
S303

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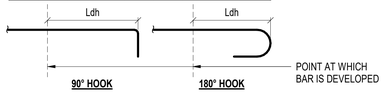
TENSION DEVELOPMENT LENGTH OF STANDARD HOOK (L_{dh}) TABLE

BAR SIZE	CONCRETE COMPRESSIVE STRENGTH, f'_c (PSI)							
	3000	4000	5000	6000	7000	8000	9000	10000
#3	9	8	7	6	6	6	6	6
#4	11	10	9	8	8	7	7	6
#5	14	12	11	10	9	9	8	8
#6	17	15	13	12	11	11	10	9
#7	20	17	15	14	13	12	12	11
#8	22	19	17	16	15	14	13	12
#9	25	22	20	18	17	16	15	14
#10	28	24	22	20	18	17	16	15
#11	31	27	24	22	20	19	18	17



90° HOOK 180° HOOK

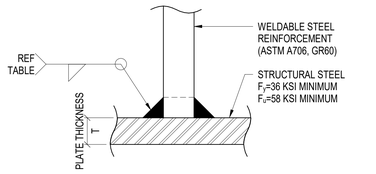
TENSION DEVELOPMENT LENGTH OF STANDARD HOOK TABLE:

1. TABLE IS BASED ON GRADE 60 REINFORCEMENT IN NORMAL-WEIGHT CONCRETE.
2. FOR EPOXY-COATED, OR ZINC-AND-EPOXY DUAL COATED REINFORCEMENT, MULTIPLY BY 1.2.
3. Ldh DIMENSION SHALL BE TAKEN FROM THE OUTSIDE FACE OF BAR AT THE HOOKED END.
4. STANDARD HOOKS SHOWN ON PLAN CAN BE 90 DEGREE OR 180 DEGREE INTERCHANGEABLY AS REQUIRED FOR GEOMETRIC FIT UP (UNO).
5. REFER TO THE STANDARD HOOK GEOMETRY TABLES FOR ADDITIONAL INFORMATION.

WELDABLE STEEL REINFORCEMENT TABLE (PERPENDICULAR TO STEEL)

REINFORCEMENT SIZE	NOMINAL WELD SIZE	MIN. PLATE THICKNESS (T)
#3	3/16	1/4"
#4	1/4	1/4"
#5	5/16	3/8"
#6	5/16	1/2"
#7	3/8	1/2"
#8	1/2	5/8"
#9	1/2	5/8"
#10	5/8	3/4"
#11	5/8	3/4"

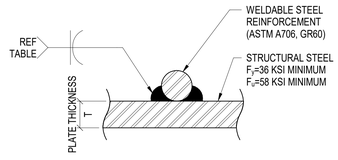
* WELDS SHALL BE MADE WITH E80 ELECTRODES.



WELDABLE STEEL REINFORCEMENT TABLE (PARALLEL TO STEEL)

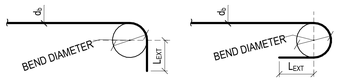
REINFORCEMENT SIZE	EFFECTIVE THROAT SIZE	MIN. WELD LENGTH (PLATE THICKNESS < 1/2")	MIN. WELD LENGTH (PLATE THICKNESS ≥ 1/2")
#3	0.075"	3"	3"
#4	0.100"	3"	3"
#5	0.125"	3"	3"
#6	0.150"	4"	4"
#7	0.175"	6"	5"
#8	0.200"	7"	5"
#9	0.230"	9"	6"
#10	0.260"	11"	6"
#11	0.290"	13"	7"

* WELDS SHALL BE MADE WITH E80 ELECTRODES.



STANDARD HOOK GEOMETRIES FOR DEFORMED BARS IN TENSION

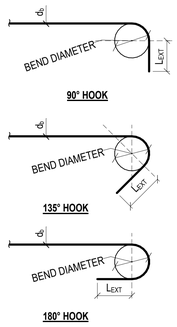
STD HOOK TYPE	BAR SIZE	MINIMUM INSIDE BEND DIAMETER (INCH)	STRAIGHT EXTENSION L_{EXT} (INCH)
90°	#3 - #8	$6d_b$	$12d_b$
	#9 - #11	$8d_b$	
	#14, #18	$10d_b$	
180°	#3 - #8	$6d_b$	LARGER OF $4d_b$ AND 2 1/2"
	#9 - #11	$8d_b$	
	#14, #18	$10d_b$	



90° HOOK 180° HOOK

STANDARD HOOK GEOMETRIES FOR STIRRUPS, TIES, AND HOOPS

STD HOOK TYPE	BAR SIZE	MINIMUM INSIDE BEND DIAMETER (INCH)	STRAIGHT EXTENSION L_{EXT} (INCH)
90°	#3 - #5	$4d_b$	LARGER OF $6d_b$ AND 3"
	#6 - #8	$6d_b$	$12d_b$
135°	#3 - #5	$4d_b$	LARGER OF $6d_b$ AND 3"
	#6 - #8	$6d_b$	
90°	#3 - #5	$4d_b$	LARGER OF $4d_b$ AND 2 1/2"
	#6 - #8	$6d_b$	



COMPRESSION DEVELOPMENT (L_{dc}) AND COMPRESSION LAP SPLICE (L_{sc}) LENGTH TABLE

BAR SIZE	CONCRETE COMPRESSIVE STRENGTH, f'_c (PSI)			
	$f'_c \leq 4000$		$f'_c \geq 5000$	
	L_{dc}	COMP. LAP (L_{sc})	L_{dc}	COMP. LAP (L_{sc})
#3	12"	12"	12"	12"
#4	12"	15"	12"	15"
#5	12"	19"	12"	19"
#6	15"	23"	14"	23"
#7	17"	27"	16"	27"
#8	19"	31"	19"	31"
#9	22"	34"	21"	34"
#10	25"	39"	23"	39"
#11	27"	43"	26"	43"

COMPRESSION DEVELOPMENT AND LAP SPLICE LENGTH TABLE NOTES:

1. TABLE IS BASED ON GRADE 60 REINFORCEMENT IN NORMAL-WEIGHT CONCRETE.
2. WHEN LAP SPLICING BARS OF DIFFERENT SIZES, THE REQUIRED SPLICE LENGTH SHALL BE TAKEN AS THE LARGER OF THE SMALLER BAR LAP SPLICE LENGTH, AND THE LARGER BAR DEVELOPMENT LENGTH.

TENSION DEVELOPMENT (L_d) AND LAP SPLICE LENGTH TABLE (HORIZONTAL REINFORCEMENT)

BAR SIZE	CASTING POSITION	CONDITION	CONCRETE COMPRESSIVE STRENGTH, f'_c (PSI)															
			3000		4000		5000		6000		7000		8000		9000		10000	
			CLASS A (L_d)	CLASS B	CLASS A (L_d)	CLASS B	CLASS A (L_d)	CLASS B	CLASS A (L_d)	CLASS B	CLASS A (L_d)	CLASS B	CLASS A (L_d)	CLASS B	CLASS A (L_d)	CLASS B	CLASS A (L_d)	CLASS B
#3	BOTTOM	I	17	22	15	19	13	17	12	16	12	14	12	14	12	13	12	12
		II	25	33	22	28	20	25	18	23	17	21	16	20	15	19	14	18
	OTHER	I	22	28	19	25	17	22	16	20	14	19	14	18	13	17	12	16
		II	33	42	28	37	25	33	23	30	21	28	20	26	19	25	18	23
#4	BOTTOM	I	22	29	19	25	17	23	16	21	15	19	14	18	13	17	12	16
		II	33	43	29	37	26	34	24	31	22	28	21	27	19	25	18	24
	OTHER	I	29	38	25	33	23	29	21	27	19	25	18	23	17	22	16	21
		II	43	56	37	49	34	44	31	40	28	37	27	35	25	33	24	31
#5	BOTTOM	I	28	36	24	31	22	28	20	26	18	24	17	22	16	21	15	20
		II	42	54	36	47	32	42	30	38	27	35	26	33	24	31	23	30
	OTHER	I	36	47	31	41	28	36	26	33	24	31	22	29	21	27	20	26
		II	54	70	47	61	42	54	38	50	35	46	33	43	31	41	30	39
#6	BOTTOM	I	33	43	29	37	26	34	24	31	22	28	21	27	19	25	18	24
		II	50	65	43	56	39	50	35	46	33	42	31	40	29	37	27	36
	OTHER	I	43	56	37	49	34	44	31	40	28	37	27	35	25	33	24	31
		II	65	84	56	73	50	65	46	59	42	55	40	52	37	49	36	46
#7	BOTTOM	I	48	63	42	54	38	49	34	45	32	41	30	39	28	36	27	35
		II	72	94	63	81	56	73	51	67	48	62	45	58	42	54	40	52
	OTHER	I	63	81	54	71	49	63	45	58	41	54	39	50	36	47	35	45
		II	94	122	81	106	73	95	67	86	62	80	58	75	54	71	52	67
#8	BOTTOM	I	55	72	48	62	43	56	39	51	36	47	34	44	32	42	30	39
		II	83	107	72	93	64	83	59	76	54	70	51	66	48	62	45	59
	OTHER	I	72	93	62	81	56	72	51	66	47	61	44	57	42	54	39	51
		II	107	139	93	121	83	108	76	99	70	91	66	86	62	81	59	77
#9	BOTTOM	I	62	81	54	70	48	63	44	57	41	53	38	50	36	47	34	44
		II	93	121	81	105	72	94	66	85	61	79	57	74	54	70	51	66
	OTHER	I	81	105	70	91	63	81	57	74	53	69	50	64	47	61	44	58
		II	121	157	105	136	94	121	85	111	79	103	74	96	70	91	66	86
#10	BOTTOM	I	69	90	60	78	54	69	49	63	45	59	42	55	40	52	38	49
		II	103	134	89	116	80	104	73	95	68	88	63	82	60	78	57	74
	OTHER	I	90	116	78	101	69	90	63	82	59	76	55	71	52	67	49	64
		II	134	174	116	151	104	135	95	123	88	114	82	107	78	101	74	96
#11	BOTTOM	I	76	98	66	85	59	76	54	70	50	65	47	60	44	57	42	54
		II	113	147	98	128	88	114	80	104	74	97	70	90	66	85	62	81
	OTHER	I	98	128	85	111	76	99	70	90	65	84	60	78	57	74	54	70
		II	147	191	128	166	114	148	104	135	97	125	90	117	85	111	81	105

- HORIZONTAL REINFORCEMENT VALUES APPLY TO HORIZONTALLY-ORIENTED (AND INCLINED) REINFORCEMENT FOR CONCRETE BEAMS, GIRDERS, SLABS, MAT FOUNDATIONS AND OTHER SIMILAR HORIZONTAL CONCRETE ELEMENTS (UNLESS NOTED OTHERWISE).
- THIS TABLE ALSO APPLIES TO WALL HORIZONTAL REINFORCEMENT.

TENSION DEVELOPMENT (L_d) AND LAP SPLICE LENGTH TABLE (VERTICAL REINFORCEMENT)

BAR SIZE	CONDITION	CONCRETE COMPRESSIVE STRENGTH, f'_c (PSI)															
		3000		4000		5000		6000		7000		8000		9000		10000	
		CLASS A (L_d)	CLASS B	CLASS A (L_d)	CLASS B	CLASS A (L_d)	CLASS B	CLASS A (L_d)	CLASS B	CLASS A (L_d)	CLASS B	CLASS A (L_d)	CLASS B	CLASS A (L_d)	CLASS B	CLASS A (L_d)	CLASS B
#5	I	28	36	24	31	22	28	20	26	18	24	17	22	16	21	15	20
	II	42	54	36	47	32	42	30	38	27	35	26	33	24	31	23	30
#6	I	33	43	29	37	26	34	24	31	22	28	21	27	19	25	18	24
	II	50	65	43	56	39	50	35	46	33	42	31	40	29	37	27	36
#7	I	48	63	42	54	38	49	34	45	32	41	30	39	28	36	27	35
	II	72	94	63	81	56	73	51	67	48	62	45	58	42	54	40	52
#8	I	55	72	48	62	43	56	39	51	36	47	34	44	32	42	30	39
	II	83	107	72	93	64	83	59	76	54	70	51	66	48	62	45	59
#9	I	62	81	54	70	48	63	44	57	41	53	38	50	36	47	34	44
	II	93	121	81	105	72	94	66	85	61	79	57	74	54	70	51	66
#10	I	69	90	60	78	54	69	49	63	45	59	42	55	40	52	38	49
	II	103	134	89	116	80	104	73	95	68	88	63	82	60	78	57	74
#11	I	76	98	66	85	59	76	54	70	50	65	47	60	44	57	42	54
	II	113	147	98	128	88	114	80	104	74	97	70	90	66	85	62	81

- VERTICAL CONCRETE REINFORCEMENT VALUES APPLY TO VERTICALLY-ORIENTED REINFORCEMENT FOR CONCRETE COLUMNS, PIERS, PLINTHS, PILES, WALLS, AND OTHER SIMILAR VERTICAL CONCRETE ELEMENTS (UNLESS NOTED OTHERWISE).
- THIS TABLE *DOES NOT* INCLUDE TENSION DEVELOPMENT/LAP REQUIREMENTS FOR WALL HORIZONTAL REINFORCEMENT.

TENSION DEVELOPMENT AND LAP SPLICE LENGTH TABLE NOTES (APPLIES TO VERTICAL AND HORIZONTAL CONCRETE ELEMENT TABLES):

1. CONDITION I OR CONDITION II IS DETERMINED AS FOLLOWS. FOR A GIVEN CONCRETE ELEMENT, THE MOST FREQUENT BAR SPACING PATTERN SHALL BE USED TO DETERMINE THE CONDITION.
   A. CONDITION I: CLEAR SPACING OF BARS BEING DEVELOPED OR LAP-SPLICED IS NOT LESS THAN TWO TIMES THE BAR DIAMETER, AND THE CLEAR COVER IS AT LEAST EQUAL TO THE BAR DIAMETER.
   B. CONDITION II: ALL OTHER CASES.
2. FOR EPOXY-COATED, OR ZINC AND EPOXY DUAL-COATED REINFORCEMENT, MULTIPLY VALUES AS FOLLOWS:
   A. WHERE CLEAR COVER IS LESS THAN THREE TIMES THE BAR DIAMETER, OR WHERE CLEAR SPACING IS LESS THAN SIX TIMES THE BAR DIAMETER, MULTIPLY VALUES BY 1.5.
   B. FOR ALL OTHER CONDITIONS, MULTIPLY VALUES BY 1.2.
   GALVANIZED (ZINC-COATED) REINFORCEMENT NEED NOT BE FACTORED FROM THE TABULATED VALUES.
3. TABLE IS BASED ON GRADE 60 REINFORCEMENT IN NORMAL-WEIGHT CONCRETE.
4. BAR DEVELOPMENT LENGTH (L_d) SHALL EQUAL THE CLASS A SPLICE LENGTHS FROM THE TABLE. ALL LAP SPLICES SHALL BE OF THE CLASS B VARIETY, UNLESS NOTED OTHERWISE.
5. AT SPLICES, WHERE THE CLEAR BAR SPACING IS LESS THAN TWO TIMES THE BAR DIAMETER, STAGGER EVERY LAP SPLICE SUCH THAT ADJACENT LAP SPLICE REGIONS DO NOT OVERLAP AT ANY POINT.
6. WHEN LAP SPLICING BARS OF DIFFERENT SIZES, THE REQUIRED SPLICE LENGTH SHALL BE TAKEN AS THE LARGER OF THE SMALLER BAR LAP SPLICE LENGTH, AND THE LARGER BAR DEVELOPMENT LENGTH.

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
www.mcsal.com FL CA#26616

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SEACOAST BANK HEADQUARTERS

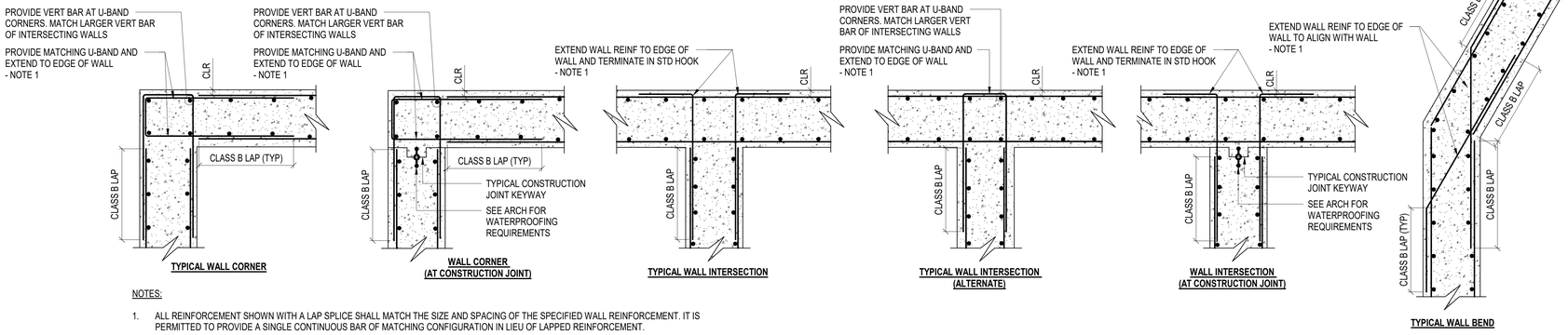
815 SOUTH COLORADO AVE
STUART, FL 34994

PROJECT NO:
240089.00

CONCRETE TYPICAL DETAILS

SHEET NUMBER:
S311

THE LINE SHOWN ABOVE IS
1" LONG ON ORIGINAL PLOT
SCALE IF ORIGINAL PAPER SIZE IS NOT

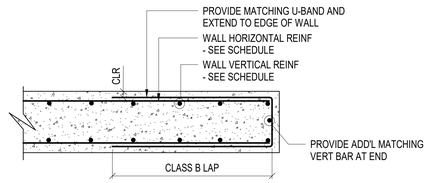


NOTES:

1. ALL REINFORCEMENT SHOWN WITH A LAP SPLICE SHALL MATCH THE SIZE AND SPACING OF THE SPECIFIED WALL REINFORCEMENT. IT IS PERMITTED TO PROVIDE A SINGLE CONTINUOUS BAR OF MATCHING CONFIGURATION IN LIEU OF LAPPED REINFORCEMENT.

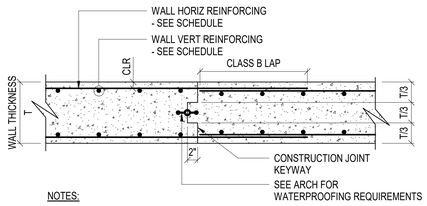
D2 / S312 TYPICAL CONCRETE WALL HORIZONTAL REINFORCING AT CORNERS AND INTERSECTIONS

SCALE: 1" = 1'-0"



C1 / S312 TYPICAL CONCRETE WALL HORIZONTAL REINFORCING AT ENDS

SCALE: 1" = 1'-0"

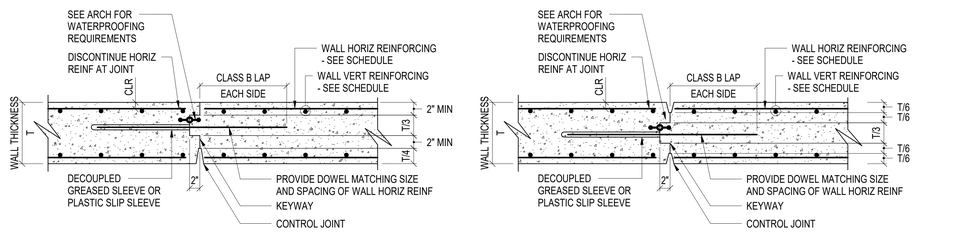


NOTES:

1. CONTRACTOR SHALL SUBMIT PROPOSED LOCATIONS OF CONSTRUCTION JOINTS TO THE STRUCTURAL ENGINEER FOR REVIEW.
2. CONSTRUCTION JOINT LOCATIONS SHALL BE COORDINATED WITH THE REINFORCING STEEL SUPPLIER AND ERECTOR PRIOR TO SUBMISSION OF SHOP DRAWINGS.

C2 / S312 TYPICAL CONCRETE WALL CONSTRUCTION JOINT

SCALE: 1" = 1'-0"

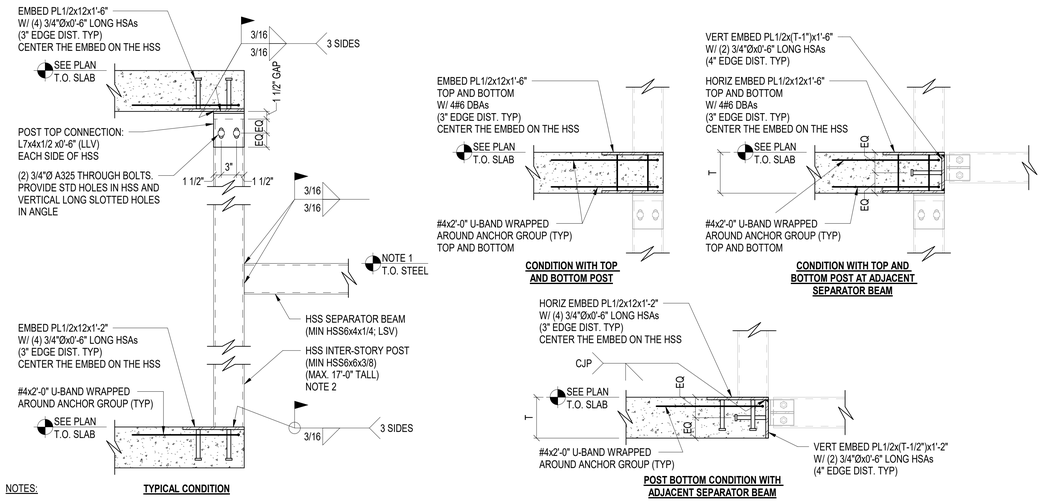


NOTES:

1. CONTROL JOINTS SHALL BE SPACED AT THE LESSER OF: 30xT, 3x WALL HEIGHT, OR 30'-0". COORDINATE THE CONTROL JOINT SPACING AND EXACT LOCATIONS WITH THE ARCHITECTURAL DRAWINGS.
2. DOUBLE-SIDED CONTROL JOINTS ARE PREFERRED AT WALLS WHERE BOTH SIDES OF THE WALL ARE EXPOSED TO VIEW.
3. CONTRACTOR SHALL SUBMIT PROPOSED LOCATIONS OF CONTROL JOINTS TO THE STRUCTURAL ENGINEER FOR REVIEW.
4. CONTROL JOINT LOCATIONS SHALL BE COORDINATED WITH THE REINFORCING STEEL SUPPLIER AND ERECTOR PRIOR TO SUBMISSION OF SHOP DRAWINGS.

C4 / S312 TYPICAL CONCRETE WALL CONTROL JOINT

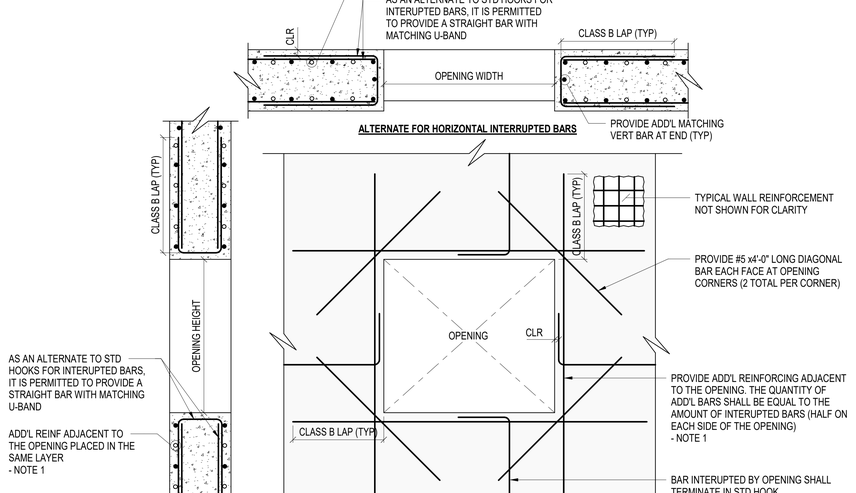
SCALE: 1" = 1'-0"



1. GC SHALL PROVIDE ALLOWANCES FOR ALL MISCELLANEOUS STRUCTURAL STEEL REQUIRED BY THE ELEVATOR SUPPLIER. GC SHALL COORDINATE THE EXACT INTER-STORY POST AND ELEVATOR SEPARATOR BEAM QUANTITIES ELEVATIONS, LOCATIONS, AND OTHER REQUIREMENTS WITH THE APPROVED ELEVATOR SHOP DRAWINGS.
2. FOR AN INTER-STORY POST HEIGHT THAT IS 12'-0" OR LESS, IT IS PERMITTED TO USE AN HSS4x4x3/8 POST IN LIEU OF THAT SHOWN. IN THIS INSTANCE, THE POST TOP CONNECTION SHALL BE 0'-4" LONG AND THE SEPARATOR BEAM CONNECTION TO THE INTER-STORY POST SHALL BE A 3/16" FILLET WELD TOP/BOTTOM AND 3/16" FLARE BEVEL WELD EACH SIDE IN LIEU OF THAT SHOWN.
3. THE SEPARATOR BEAM SIZES SHOWN HEREIN ARE DESIGNED FOR AN ALLOWABLE LOAD OF 750 LBS OUT-OF-PLANE AND 750 LBS IN PLANE TO LIMIT SEPARATOR BEAM TO 1/8" MAXIMUM DEFLECTION. THE SEPARATOR BEAM LENGTH IS ASSUMED TO BE 10'-0" MAXIMUM. GC SHALL CONFIRM THAT THESE DESIGN CRITERIA LIMITS ARE ACCEPTABLE WITH THE SELECTED ELEVATOR SUPPLIER.

A1 / S312 TYPICAL INTER-STORY ELEVATOR SEPARATOR BEAM DETAIL

SCALE: 1" = 1'-0"



NOTES:

1. THE ADDITIONAL BARS ADJACENT TO THE OPENING SHALL BE PLACED IN BETWEEN THE BARS SPECIFIED FOR THE WALL. ADDITIONAL REINFORCEMENT ADJACENT TO OPENING SHALL NOT BE LESS THAN (2)#5 EACH SIDE EACH FACE.
2. THIS DETAIL APPLIES TO EACH FACE OF THE WALL REINFORCING.

A4 / S312 TYPICAL CONCRETE WALL OPENING

SCALE: 1" = 1'-0"

Baker Barrios
ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000
INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
www.mcsal.com FL CA#26616

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	06/12/2026	PERMIT SET

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TAMBONE COMPANIES

SEACOAST BANK HEADQUARTERS

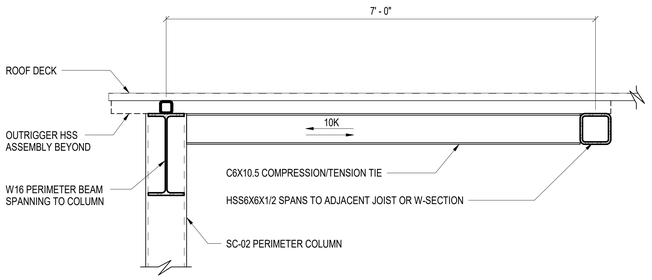
815 SOUTH COLORADO AVE
STUART, FL 34994

PROJECT NO:
240089.00

CONCRETE TYPICAL DETAILS

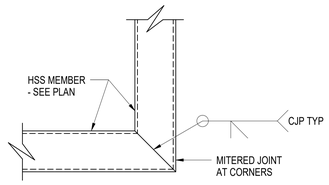
SHEET NUMBER:
S312

THE LINE SHOWN ABOVE IS 3 INCHES LONG ON ORIGINAL SHEETS ORIGINAL PAGE SIZE



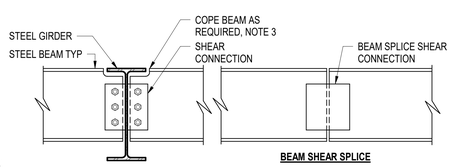
C1 / S321 ADDITIONAL BRACING DETAIL

SCALE: 1" = 1'-0"



C5 / S321 TYPICAL HSS MITERED JOINT CONNECTION

SCALE: 1" = 1'-0"

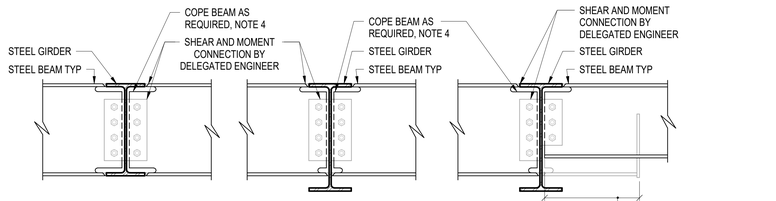


NOTES:

1. SHEAR AND AXIAL CONNECTIONS SHALL BE DESIGNED BY A DELEGATED ENGINEER LICENSED IN THE STATE IN WHICH THE PROJECT IS PERMITTED.
2. REFER TO PLAN FOR THE LRFD ULTIMATE SHEAR FORCES AND AXIAL FORCES FOR WHICH THESE CONNECTIONS SHALL BE DESIGNED TO TRANSFER.
3. BEAM COPES SHALL BE DESIGNED BY THE DELEGATED ENGINEER TO VERIFY THE BEAM ADEQUACY AFTER THIS MODIFICATION.
4. WELD/BOLT GROUPS SHALL BE DESIGNED TO RESOLVE ECCENTRICITIES AND IMPART SHEAR FORCE ONLY AT BEAM WEBS.

B1 / S321 TYPICAL STEEL BEAM-TO-BEAM SHEAR CONNECTION

SCALE: 1" = 1'-0"

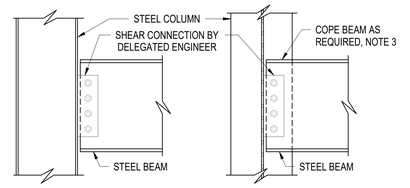


NOTES:

1. SHEAR, AXIAL AND MOMENT CONNECTIONS SHALL BE DESIGNED BY A DELEGATED ENGINEER LICENSED IN THE STATE IN WHICH THE PROJECT IS PERMITTED.
2. REFER TO PLAN FOR THE LRFD ULTIMATE SHEAR FORCES AND AXIAL FORCES FOR WHICH THESE CONNECTIONS SHALL BE DESIGNED TO TRANSFER.
3. MOMENT CONNECTIONS INDICATED ON PLAN SHALL BE DESIGNED FOR THE FULL PLASTIC MOMENT CAPACITY (LRFD) AS INDICATED IN THE AISC MANUAL. SEE STRUCTURAL STEEL GENERAL NOTES FOR MORE INFORMATION.
4. BEAM COPES SHALL BE DESIGNED BY THE DELEGATED ENGINEER TO VERIFY THE BEAM ADEQUACY AFTER THIS MODIFICATION.

B3 / S321 TYPICAL STEEL BEAM-TO-BEAM MOMENT CONNECTIONS

SCALE: 1" = 1'-0"



NOTES:

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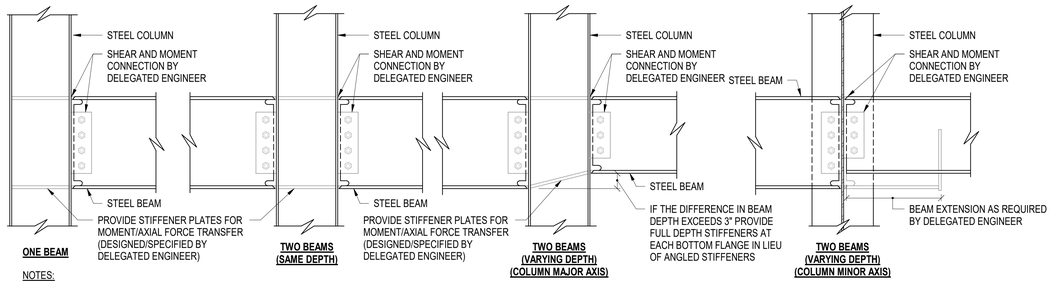
2. REFER TO PLAN FOR THE LRFD ULTIMATE SHEAR FORCES AND AXIAL FORCES FOR WHICH THESE CONNECTIONS SHALL BE DESIGNED TO TRANSFER.
3. BEAM COPES SHALL BE DESIGNED BY THE DELEGATED ENGINEER TO VERIFY THE BEAM ADEQUACY AFTER THIS MODIFICATION.
4. WELD/BOLT GROPUS SHALL BE DESIGNED TO RESOLVE ECCENTRICITIES AND IMPART SHEAR FORCE ONLY AT BEAM WEBS.

B5 / S321 TYPICAL STEEL BEAM-TO-COLUMN SHEAR CONNECTIONS

SCALE: 1" = 1'-0"



NOTES:

1. SHEAR, AXIAL AND MOMENT CONNECTIONS SHALL BE DESIGNED BY A DELEGATED ENGINEER LICENSED IN THE STATE IN WHICH THE PROJECT IS PERMITTED.
2. REFER TO PLAN FOR THE LRFD ULTIMATE SHEAR FORCES AND AXIAL FORCES FOR WHICH THESE CONNECTIONS SHALL BE DESIGNED TO TRANSFER.
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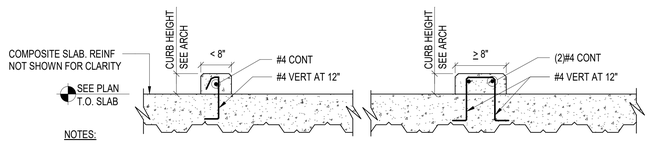
4. BEAM COPES SHALL BE DESIGNED BY THE DELEGATED ENGINEER TO VERIFY THE BEAM ADEQUACY AFTER THIS MODIFICATION.

A1 / S321 TYPICAL STEEL BEAM-TO-COLUMN MOMENT CONNECTIONS

SCALE: 1" = 1'-0"



NOTES:

1. MINIMUM CURB WIDTH SHALL BE 6". MAXIMUM CURB WIDTH SHALL BE 16".
2. MAXIMUM CURB HEIGHT SHALL BE 14"

A4 / S321 TYPICAL COMPOSITE SLAB CURB DETAIL

SCALE: 1" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
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ELIE EL ZGHAYER LICENSE No.78483 STATE OF FLORIDA PROFESSIONAL ENGINEER

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TAMBONE COMPANIES

SEACOAST BANK HEADQUARTERS

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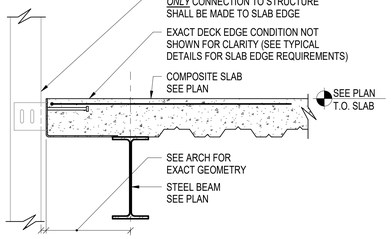
STUART, FL 34994

PROJECT NO:
240089.00

STEEL TYPICAL DETAILS

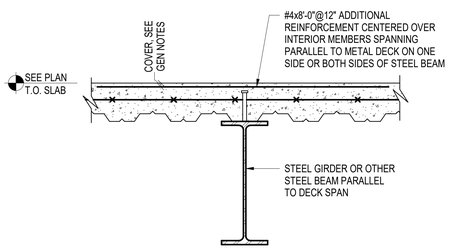
SHEET NUMBER:
S321

THE LINE SHOWN ABOVE IS 3 INCHES LONG ON ORIGINAL PAPER SIZE



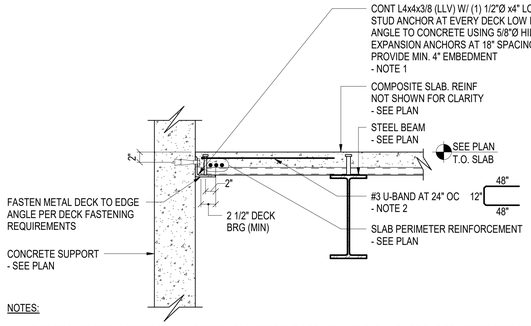
TYPICAL LATERAL-ONLY CONNECTION AT COMPOSITE DECK SLAB EDGE

D1 / S322 SCALE: 1" = 1'-0"



ADDITIONAL REINFORCEMENT AT STEEL BEAMS PARALLEL TO DECK SPAN

C1 / S322 SCALE: 1" = 1'-0"

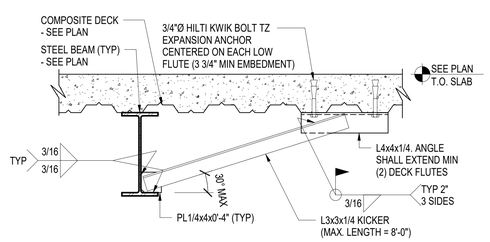


NOTES:

1. THIS DECK EDGE SUPPORT DETAIL ALSO APPLIES WHERE THE DECK SPAN IS PARALLEL TO THE CONCRETE SUPPORT. WHEN THE DECK SPAN RUNS PARALLEL, THE HEADED STUD ANCHORS AT THE DECK EDGE SUPPORT ANGLE SHALL BE SPACED AT 12" OC.
2. POSITION U-BANDS SUCH THAT THE VERTICAL HEADED STUDS IN (2) CONSECUTIVE LOW FLUTES ARE CONFINED WITHIN THE BAND.

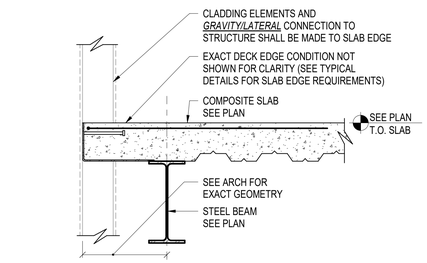
TYPICAL COMPOSITE SLAB BEARING AT CONCRETE SUPPORT

B1 / S322 SCALE: 1" = 1'-0"



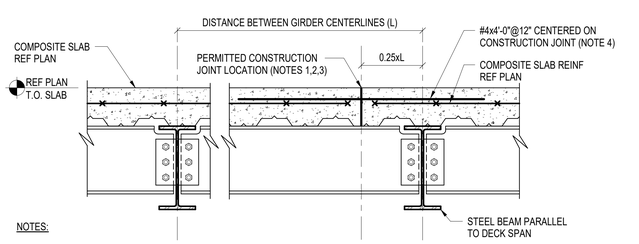
TYPICAL BEAM BOTTOM FLANGE KICKER (AT COMPOSITE DECK)

A1 / S322 SCALE: 1" = 1'-0"



TYPICAL GRAVITY AND LATERAL CONNECTION AT COMPOSITE DECK SLAB EDGE

E3 / S322 SCALE: 1" = 1'-0"

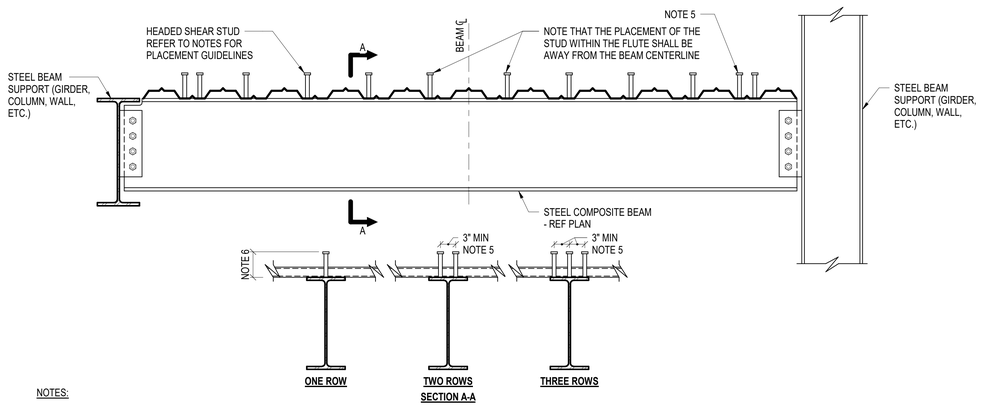


NOTES:

1. CONSTRUCTION JOINTS WITHIN COMPOSITE SLABS SHALL BE PLACED PARALLEL TO THE DECK SPAN.
2. CONTRACTOR SHALL SUBMIT PROPOSED LOCATIONS OF CONSTRUCTION JOINTS TO THE STRUCTURAL ENGINEER FOR REVIEW.
3. CONSTRUCTION JOINT LOCATIONS SHALL BE COORDINATED WITH THE REINFORCING STEEL SUPPLIER AND ERECTOR PRIOR TO SUBMISSION OF SHOP DRAWINGS.
4. FOR INSTANCES WHERE DISTANCE BETWEEN GIRDER CENTERLINES (L) IS 8'-0" OR LESS, IT SHALL BE PERMITTED TO UTILIZE TYPICAL 'ADDITIONAL REINFORCEMENT AT STEEL BEAMS PARALLEL TO DECK SPAN' (#4x8'-0"@12" CENTERED ON THE GIRDER) IN LIEU OF THAT SHOWN.

TYPICAL COMPOSITE SLAB CONSTRUCTION JOINT

E5 / S322 SCALE: 1" = 1'-0"

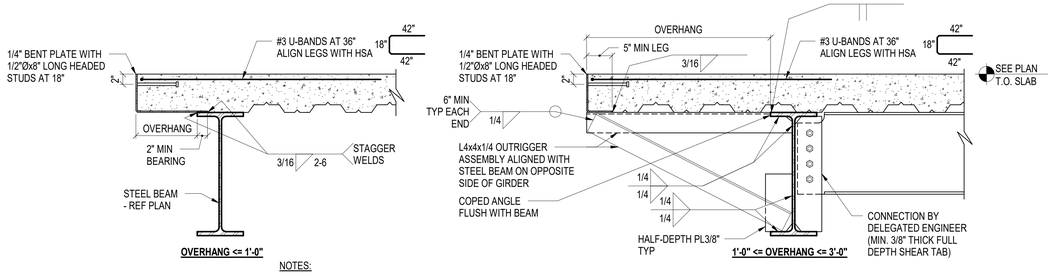


NOTES:

1. HEADED STUDS SHALL BE UNIFORMLY DISTRIBUTED, AND POSITIONED WITHIN EACH FLUTE ON THE SIDE OF THE FLUTE CLOSER TO THE RESPECTIVE BEAM SUPPORT (AWAY FROM THE BEAM CENTERLINE).
2. REFER TO PLAN FOR HEADED STUD QUANTITY FOR EACH FRAMING MEMBER.
3. MAXIMUM HEADED SHEAR STUD SPACING SHALL BE 3'-0". WHERE QUANTITY SPECIFIED IN PLAN RESULTS IN A CONDITION WHERE MAX. SPACING IS EXCEEDED, PROVIDE ADDITIONAL STUDS AS NEEDED TO MEET THIS REQUIREMENT.
4. WHERE NUMBER OF STUDS SPECIFIED EXCEEDS THE NUMBER OF FLUTES, BEGIN PLACING THE NEXT ROW (UP TO 3 MAX ROWS ACROSS A FLANGE) AT THE SUPPORTS, WORKING FROM THE BEAM ENDS TOWARDS THE CENTERLINE.
5. WHERE MINIMUM SPACING BETWEEN MULTIPLE ROWS CANNOT BE ACHIEVED WITHIN A SINGLE FLUTE. IT WILL BE PERMISSIBLE TO PLACE A SECOND (OR THIRD) WHERE APPLICABLE ON THE SIDE OF THE FLUTE CLOSER TO THE CENTERLINE OF THE BEAM.
6. THE HEADED STUD LENGTH SHALL BE SUCH THAT THE TOP OF THE HEADED STUD IS 1 1/2" FROM THE TOP OF THE CONCRETE SLAB.

TYPICAL HEADED SHEAR STUD PLACEMENT ON COMPOSITE BEAM

C3 / S322 SCALE: 1" = 1'-0"

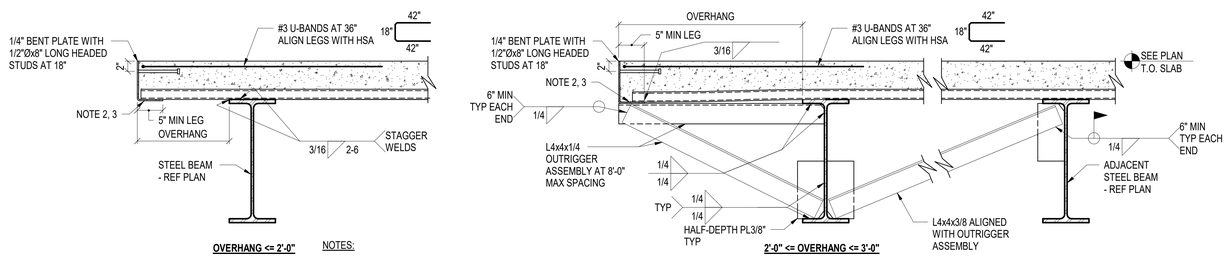


NOTES:

1. NOT ALL REINFORCING SHOWN. REFER TO PLAN NOTES AND TYPICAL DETAILS FOR MORE INFORMATION.
2. REFER TO PLAN FOR DECK INFORMATION AND FASTENING REQUIREMENTS.

TYPICAL SLAB EGDE SUPPORT AT COMPOSITE DECK (WEAK-AXIS OVERHANG)

B3 / S322 SCALE: 1" = 1'-0"



NOTES:

1. NOT ALL REINFORCING SHOWN. REFER TO PLAN NOTES AND TYPICAL DETAILS FOR MORE INFORMATION.
2. REFER TO PLAN FOR DECK INFORMATION AND FASTENING REQUIREMENTS.
3. PROVIDE 2" MIN DECK BEARING.

TYPICAL SLAB EGDE SUPPORT AT COMPOSITE DECK (STRONG-AXIS OVERHANG)

A3 / S322 SCALE: 1" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
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PROJECT NO:
240089.00

STEEL TYPICAL DETAILS

SHEET NUMBER:
S322

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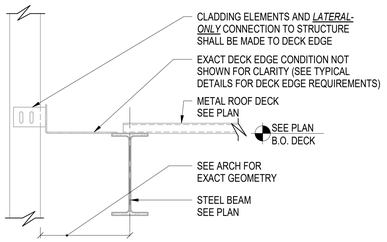
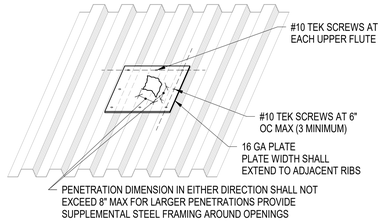


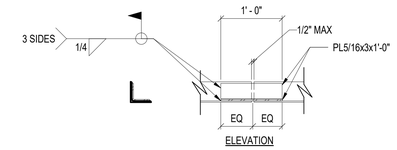
TYPICAL LATERAL-ONLY CONNECTION AT METAL ROOF DECK EDGE

C1 / S323 SCALE: 1" = 1'-0"



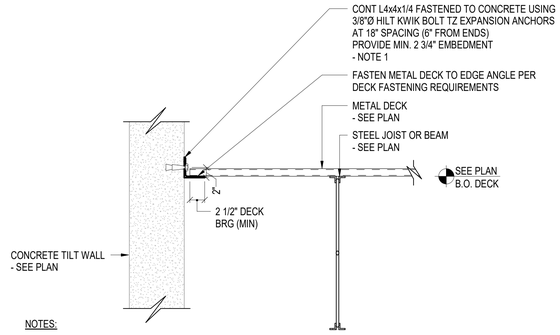
TYPICAL PENETRATION IN METAL ROOF DECK (8" MAX PENETRATION SIZE)

C3 / S323 SCALE: 1" = 1'-0"



ELEVATION

EDGE ANGLE SPLICE DETAIL

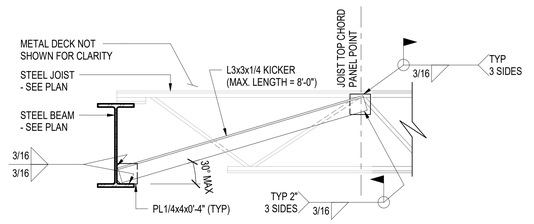


NOTES:

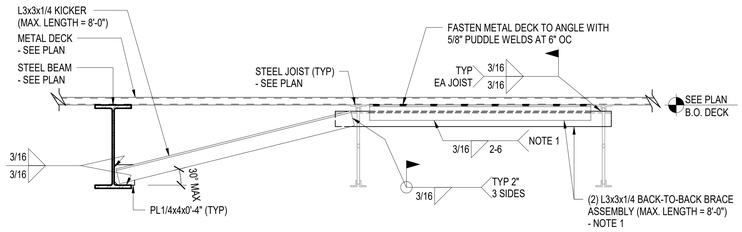
1. THIS DECK EDGE SUPPORT DETAIL ALSO APPLIES WHERE THE DECK SPAN IS PARALLEL TO THE CONCRETE SUPPORT.
2. AT LOCATIONS WHERE NO PARAPET IS PRESENT, VERTICAL ANGLE LEG SHALL BE TURNED DOWN.

METAL DECK BEARING AT CONCRETE SUPPORT

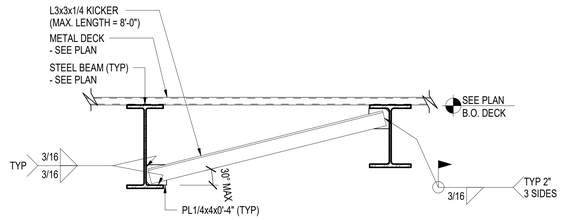
C5 / S323 SCALE: 1" = 1'-0"



STEEL BEAM TO PERPENDICULAR JOIST



STEEL BEAM TO PARALLEL JOIST



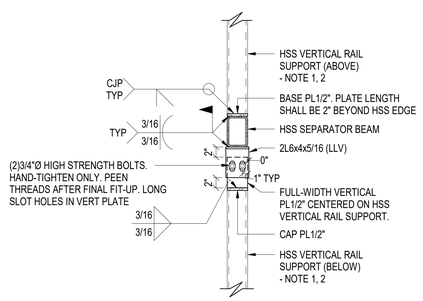
STEEL BEAM TO PARALLEL STEEL BEAM

NOTES:

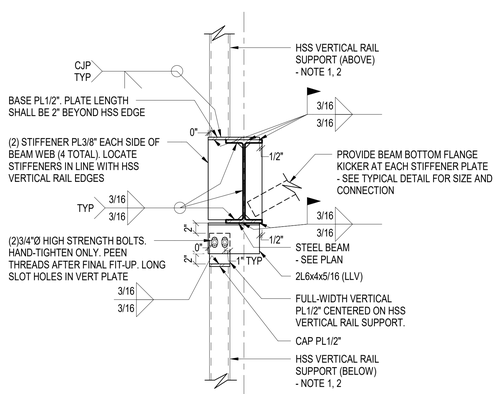
1. THE FUNCTION OF THE BACK-TO-BACK BRACE ASSEMBLY IS TO FASTEN THIS ASSEMBLY TO THE DECK. IF THE JOIST TOP CHORD DEPTH IS GREATER THAN THE ANGLE DEPTH, PROVIDE AN ANGLE DEPTH OF SUFFICIENT SIZE TO ACHIEVE THIS CONNECTION.

TYPICAL BEAM BOTTOM FLANGE KICKER (AT METAL DECK)

B1 / S323 SCALE: 1" = 1'-0"



AT SEPARATOR BEAM



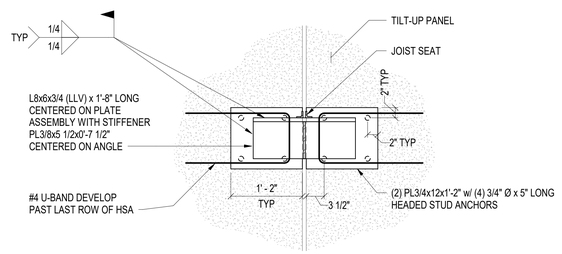
AT FLOOR FRAMING

NOTES:

1. GC SHALL COORDINATE THE EXACT ELEVATOR VERTICAL RAIL SUPPORT REQUIREMENTS AND EXACT LOCATIONS WITH THE APPROVED ELEVATOR SHOP DRAWINGS.
2. HSS VERTICAL RAIL SUPPORT SIZE SHALL BE :
   A. FOR SPANS UP TO 14'-8": HSS12x4x5/8
   B. FOR SPANS UP TO 20'-6": HSS12x6x5/8

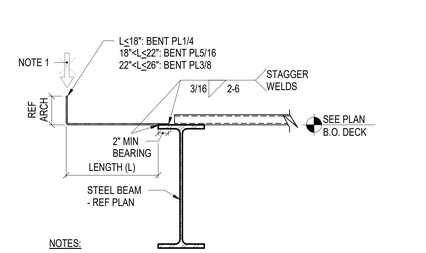
ELEVATOR STEEL (INTERMEDIATE VERTICAL RAIL SUPPORTS)

A1 / S323 SCALE: 1" = 1'-0"



TYPICAL JOIST SEAT AT PANEL JOINT

A3 / S323 SCALE: 1" = 1'-0"



NOTES:

1. DO NOT MAKE GRAVITY ATTACHMENTS TO THE BENT PLATE.

TYPICAL DECK EDGE SUPPORT AT METAL ROOF DECK

A5 / S323 SCALE: 1" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
www.mcsal.com FL CA#26616

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ELIE EL ZGHAYAR LICENSE No.78483 STATE OF FLORIDA PROFESSIONAL ENGINEER

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TAMBONE COMPANIES

SEACOAST BANK HEADQUARTERS

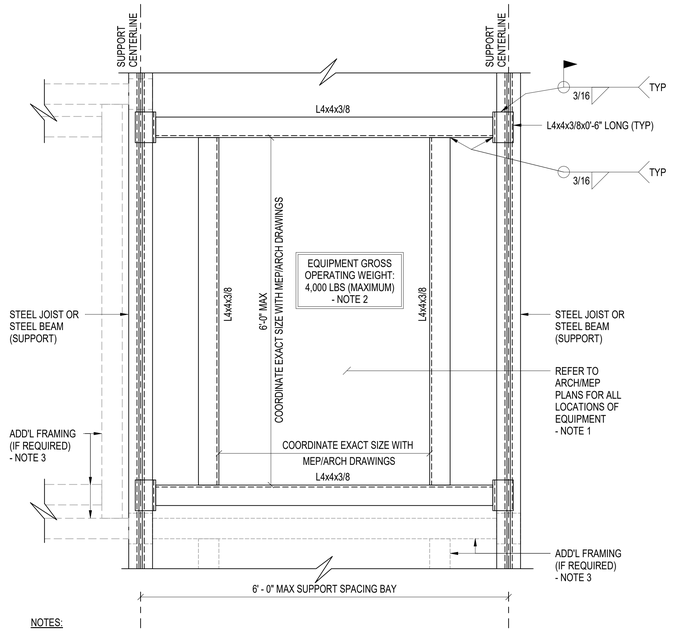
815 SOUTH COLORADO AVE
STUART, FL 34994

PROJECT NO:
240089.00

STEEL TYPICAL DETAILS

SHEET NUMBER:
S323

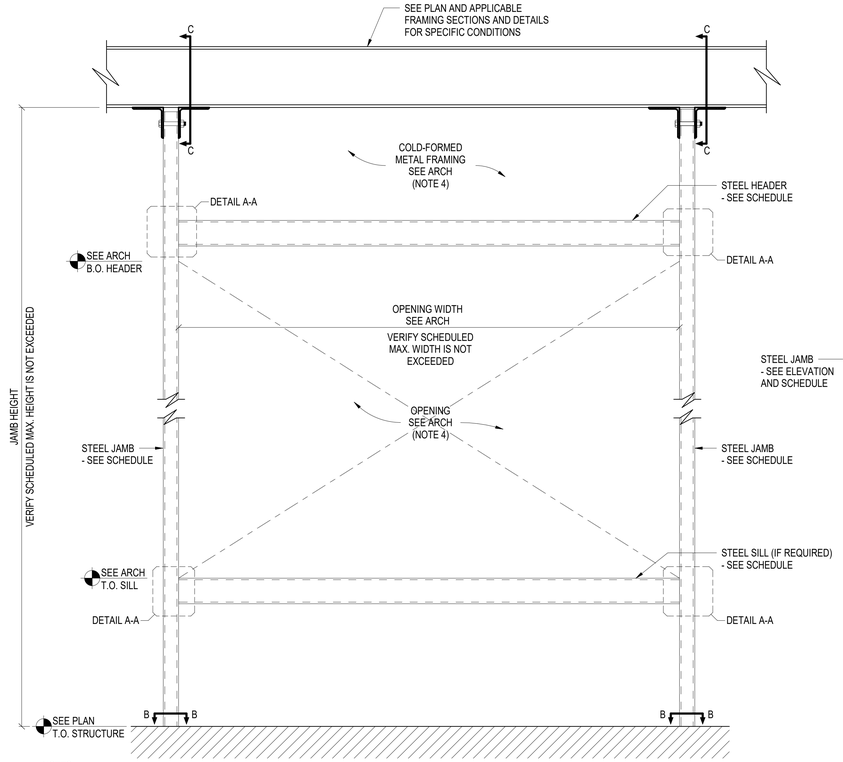
THE LINE SHOWN ABOVE IS 3" LONG ON ORIGINAL DRAWINGS SHEETS ORIGINAL PAGE SIZE



NOTES:

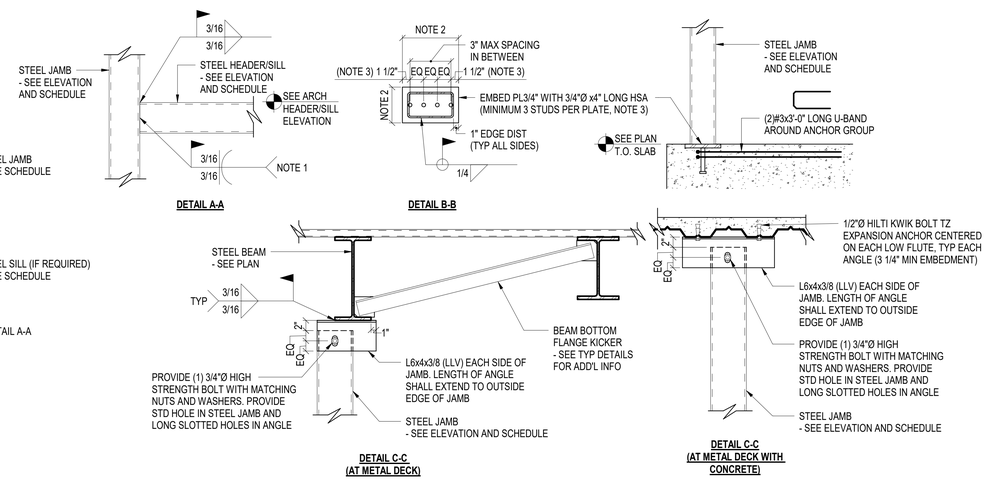
1. THIS DETAIL MAY APPLY IN MORE CONDITIONS THAN THOSE SHOWN ON THE STRUCTURAL PLANS. REFER TO THE ARCHITECTURAL AND MEP PLANS FOR ALL LOCATIONS OF EQUIPMENT SUPPORTED ON THE STEEL STRUCTURE. THE CONTRACTOR SHALL COORDINATE THE STRUCTURAL FRAMING WITH THE FINAL APPROVED EQUIPMENT SHOP DRAWINGS.
2. THE CONTRACTOR SHALL VERIFY THAT THE ACTUAL WEIGHTS INSTALLED DO NOT EXCEED THE MAXIMUM WEIGHTS SHOWN FOR THE RESPECTIVE DETAIL.
3. FOR EQUIPMENT GEOMETRIES THAT EXTEND BEYOND A TYPICAL SUPPORT SPACING BAY OR BEYOND THE MAXIMUM SPECIFIED FRAME SPACING, PROVIDE SIMILAR FRAMING TO PROVIDE ADEQUATE SUPPORT THROUGHOUT THE UNIT AS REQUIRED BY THE MANUFACTURER.
4. THIS DETAIL SHALL ALSO APPLY TO BASIC ROOF OPENINGS NOT SUPPORTING EQUIPMENT, EXCEPT THAT L3x3x1/4 ANGLES MAY BE USED IN LIEU OF THE L4x4x3/8 ANGLES. THE SAME DIMENSIONAL CONSTRAINTS SHALL APPLY.
5. AT INSTANCES WHERE THE ANGLE ASSEMBLY IS SUPPORTING EQUIPMENT WEIGHING LESS THAN 2,000 LBS, IT IS PERMITTED TO USE L3x3x5/16 ANGLES IN LIEU OF THAT SHOWN.

C4 / S324 **TYPICAL STEEL ROOF FRAMING AT EQUIPMENT SUPPORT**
SCALE: 1" = 1'-0"



SECONDARY STEEL OPENING FRAMING SCHEDULE

OPENING TYPE	MAX. OPENING WIDTH	MAX. JAMB HEIGHT	JAMB SIZE	SILL SIZE (IF APPLIACBLE)	HEADER SIZE	REMARKS
1	8'-0"	12'-0"	HSS5x5x5/16	HSS5x5x5/16	HSS5x5x5/16	
2	16'-0"	12'-0"	HSS5x5x5/16	HSS9x5x5/16 (LSV)	HSS9x5x5/16 (LSV)	
3	8'-0"	22'-0"	HSS10x5x3/8	HSS5x5x5/16	HSS5x5x5/16	



NOTES:

1. WHERE THE STEEL JAMB DEPTH IS LARGER THAN THE HEAD/SILL WIDTH, PROVIDE A 3/16" FILLET WELD ALL AROUND IN LIEU OF THE WELD SHOWN.
2. COORDINATE THE EXACT EMBED PLATE DIMENSIONS WITH THE STEEL JAMB DIMENSIONS. THE EMBED PLATE DIMENSIONS SHALL NOT BE LESS THAN 6" EACH WAY.
3. COORDINATE THE EXACT QUANTITY AND LAYOUT OF HEADED STUD ANCHORS WITH THE EMBED PLATE DIMENSIONS, EDGE DISTANCE DIMENSIONS, AND MAX. SPACING. IF THE SHORTER PLATE DIMENSION IS GREATER THAN OR EQUAL TO 8", PROVIDE (2) ROWS OF ANCHORS.
4. ALL NON-STRUCTURAL CLADDING-TYPE CONNECTIONS SHALL BE MADE TO THE OPENING FRAMING STEEL STRUCTURE AND SHALL BE PROVIDED BY THE SUPPLIER OF SUCH COMPONENTS WITH DELEGATED ENGNIEERING DESIGN OR PRE-APPROVED CODE-COMPLIANT SYSTEMS AS REQUIRED.
5. REFER TO THE ARCHITECTURAL PLANS AND DETAILS FOR THE EXACT PLAN LOCATION AND ELEVATIONS OF THE STRUCTURAL JAMBS, SILLS, AND HEADERS.
6. VERIFY THAT THE MAX. OPENING WIDTH AND THE MAX. JAMB HEIGHT AS SHOWN ON THIS SCHEDULE ARE NOT EXCEEDED.

A1 / S324 **TYPICAL SECONDARY STEEL OPENING FRAMING DETAILS**
SCALE: 1" = 1'-0"

Baker Barrios
ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000
INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
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TAMBONE COMPANIES

SEACOAST BANK HEADQUARTERS

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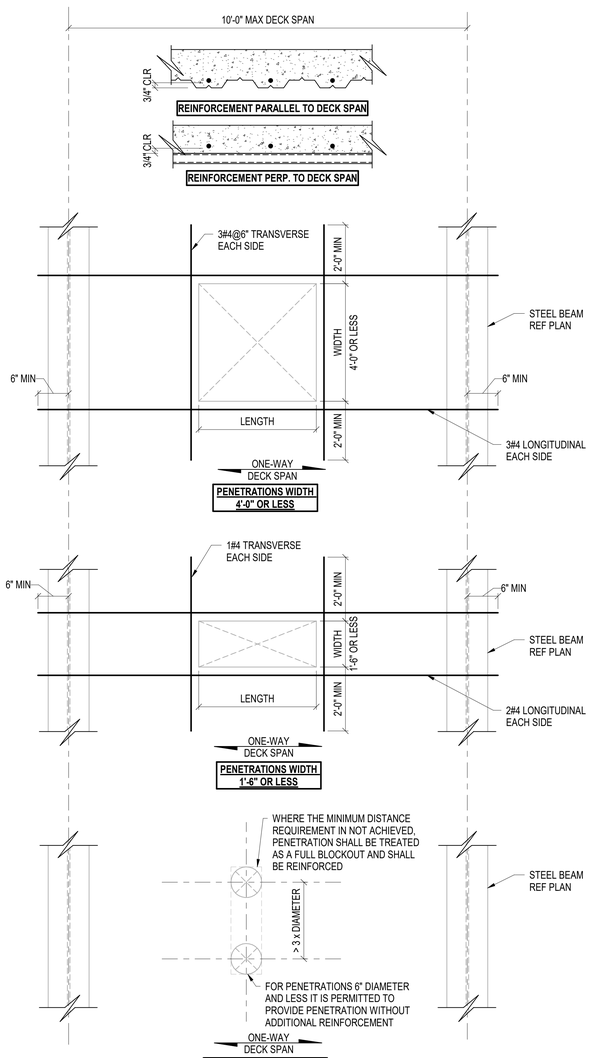
STUART, FL 34994

PROJECT NO:
240089.00

STEEL TYPICAL DETAILS

SHEET NUMBER:
S324

THE LINE SHOWN ABOVE IS
1" WHEN PLOTTED AT 100%
SHEET'S ORIGINAL PAGE SIZE

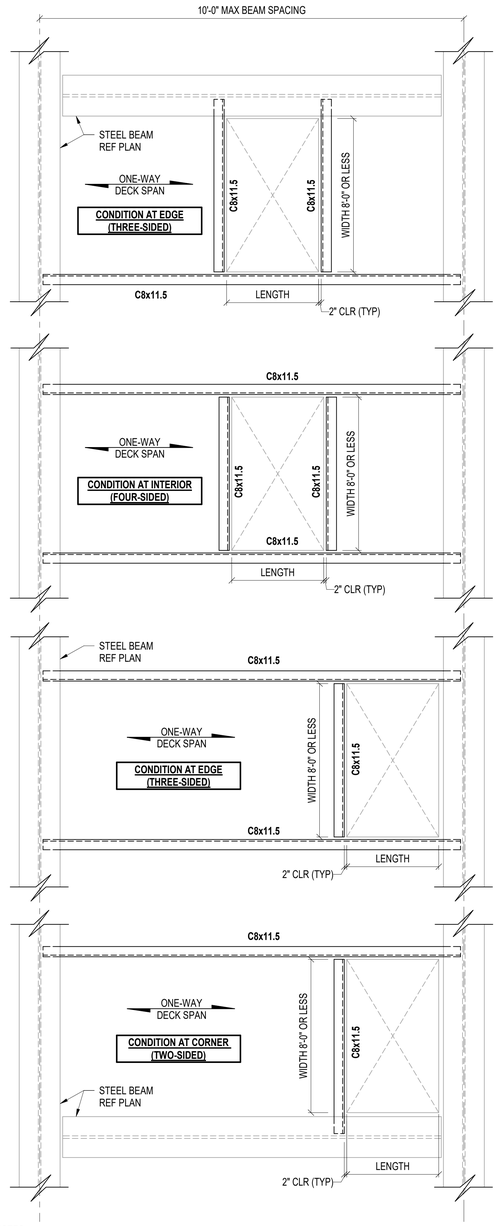


NOTES:

1. COMPOSITE SLAB OPENINGS SHALL BE FORMED/SLEEVED WITHIN THE SLAB POUR. THE METAL DECK WITHIN THE FORMED OPENING AREA SHALL REMAIN DURING CONCRETE PLACEMENT. FIELD CUT THE METAL DECK WITHIN THE FORMED OPENING AREA AFTER THE CONCRETE HAS ATTAINED 75% OF THE SPECIFIED COMPRESSIVE STRENGTH.
2. GENERAL CONTRACTOR SHALL COORDINATE ALL COMPOSITE SLAB PENETRATIONS AND SHOW THEM ON THE REINFORCEMENT SHOP DRAWINGS. PENETRATIONS WHICH REQUIRE STRUCTURAL STEEL SHALL BE SHOWN ON THE STRUCTURAL STEEL SHOP DRAWINGS.

TYPICAL MISCELLANEOUS STEEL FRAMING AT COMPOSITE SLAB OPENINGS AND PENETRATIONS (4'-0" AND LESS)

A1 / S325 SCALE: 1" = 1'-0"

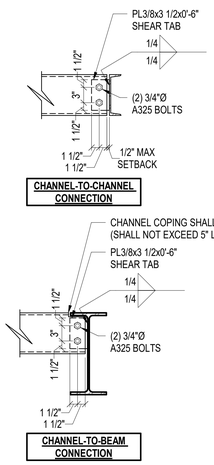


NOTES:

1. COMPOSITE SLAB OPENINGS SHALL BE FORMED/SLEEVED WITHIN THE SLAB POUR. THE METAL DECK WITHIN THE FORMED OPENING AREA SHALL REMAIN DURING CONCRETE PLACEMENT. FIELD CUT THE METAL DECK WITHIN THE FORMED OPENING AREA AFTER THE CONCRETE HAS ATTAINED 75% OF THE SPECIFIED COMPRESSIVE STRENGTH.
2. GENERAL CONTRACTOR SHALL COORDINATE ALL COMPOSITE SLAB PENETRATIONS AND SHOW THEM ON THE REINFORCEMENT SHOP DRAWINGS. PENETRATIONS WHICH REQUIRE STRUCTURAL STEEL SHALL BE SHOWN ON THE STRUCTURAL STEEL SHOP DRAWINGS.
3. PROVIDE 14 GA DECK CLOSURE AT COMPOSITE SLAB OPENINGS.
4. SHEAR STUDS ARE NOT REQUIRED FOR MISCELLANEOUS STEEL CHANNEL MEMBERS. FASTEN METAL DECK TO THESE MEMBERS PER PLAN.

TYPICAL MISCELLANEOUS STEEL FRAMING AT COMPOSITE SLAB OPENINGS AND PENETRATIONS (8'-0" AND LESS)

A2 / S325 SCALE: 1" = 1'-0"

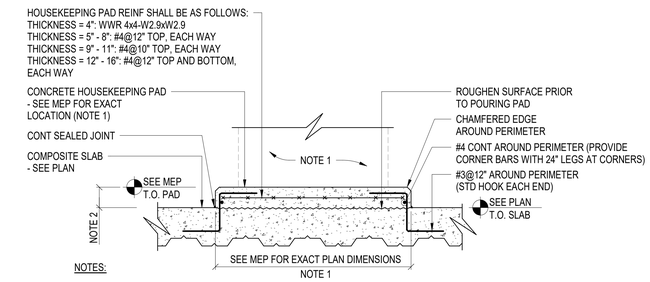


NOTES:

1. GC SHALL PROVIDE ALLOWANCES FOR ALL MISCELLANEOUS STRUCTURAL STEEL REQUIRED BY THE ELEVATOR SUPPLIER. GC SHALL COORDINATE THE EXACT ELEVATOR SILL ANGLE QUANTITES ELEVATIONS, LOCATIONS, AND OTHER REQUIREMENTS WITH THE APPROVED ELEVATOR SHOP DRAWINGS.

TYPICAL ELEVATOR SILL AT COMPOSITE SLAB

B5 / S325 SCALE: 1" = 1'-0"



NOTES:

1. SEE THE MEP/ARCHITECTURAL PLANS FOR THE EXACT LOCATION AND ALL OTHER INFORMATION REGARDING EQUIPMENT TO BE PLACED ON THE CONCRETE HOUSEKEEPING PAD. THE PERMANENT ANCHORAGE OF EQUIPMENT TO THE PAD SHALL BE PROVIDED BY THE CONTRACTOR AND THE EQUIPMENT SUPPLIER.
2. THE TOP OF PAD ELEVATION SHALL BE DETERMINED BASED ON MEP AND ARCHITECTURAL REQUIREMENTS.

TYPICAL HOUSEKEEPING PAD DETAIL ON COMPOSITE SLAB

A5 / S325 SCALE: 1" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
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AA0002981 | LC26000427

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TAMBONE COMPANIES

SEACOAST BANK HEADQUARTERS

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PROJECT NO:
240089.00

STEEL TYPICAL DETAILS

SHEET NUMBER:
S325

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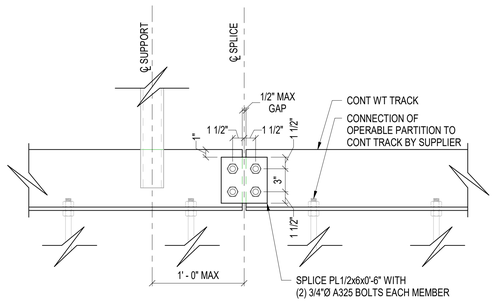
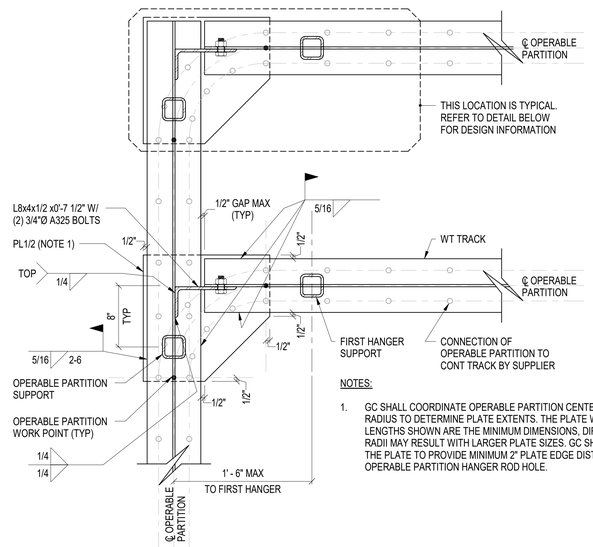


D1 / S326 OPERABLE PARTITION SUPPORTS - WT TRACK SPLICE DETAIL

SCALE: 1 1/2" = 1'-0"

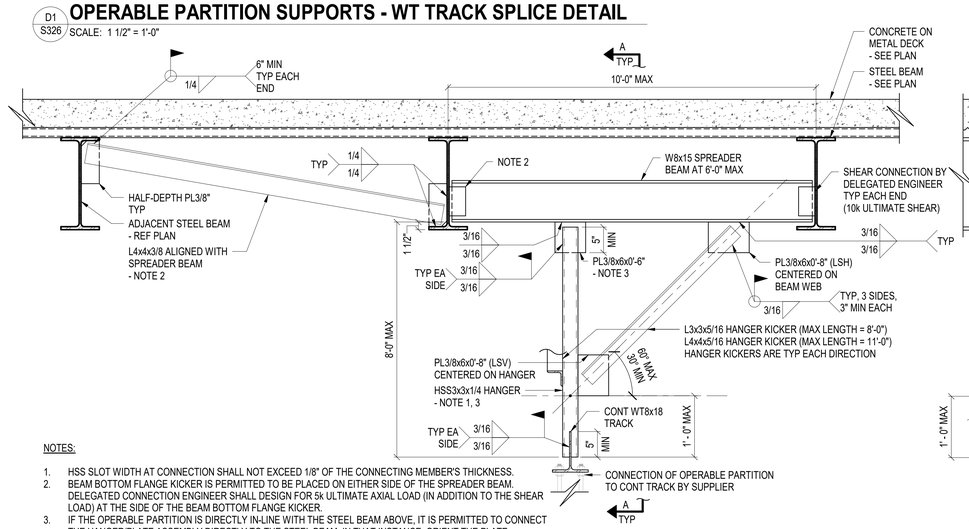


NOTES:

1. GC SHALL COORDINATE OPERABLE PARTITION CENTERLINE RADIUS TO DETERMINE PLATE EXTENTS. THE PLATE WIDTHS AND LENGTHS SHOWN ARE THE MINIMUM DIMENSIONS, DIFFERENT RADII MAY RESULT WITH LARGER PLATE SIZES. GC SHALL SIZE THE PLATE TO PROVIDE MINIMUM 2" PLATE EDGE DISTANCE TO AN OPERABLE PARTITION HANGER ROD HOLE.

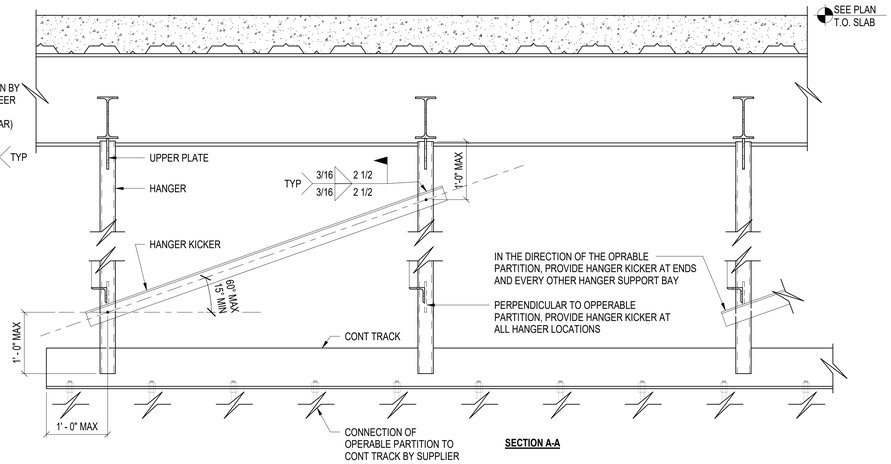
D4 / S326 OPERABLE PARTITION SUPPORTS - TRACK AT RADIUS

SCALE: 1 1/2" = 1'-0"



NOTES:

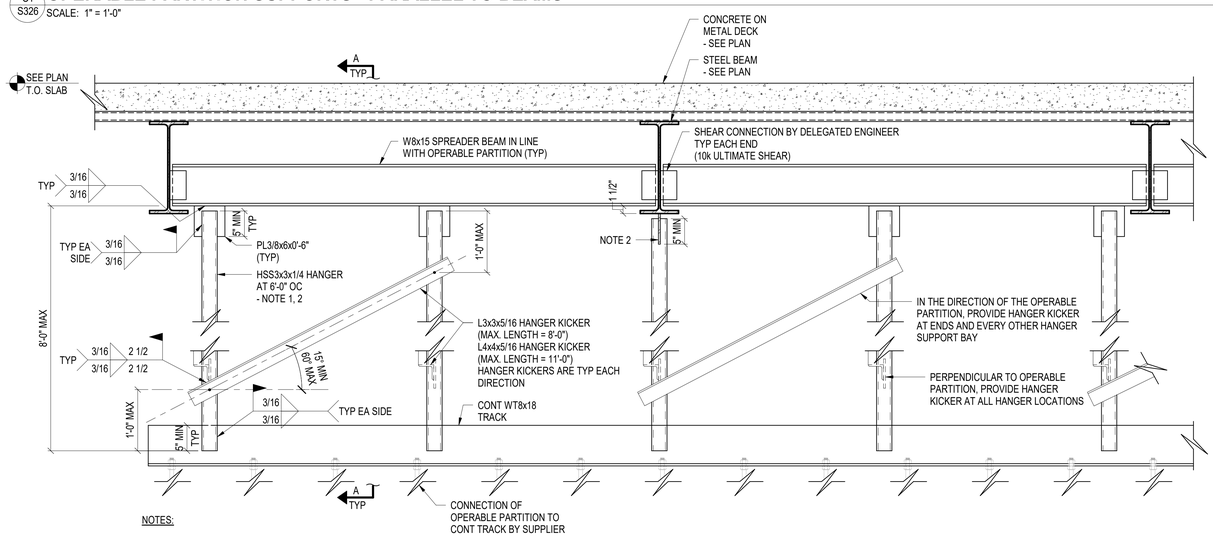
1. HSS SLOT WIDTH AT CONNECTION SHALL NOT EXCEED 1/8" OF THE CONNECTING MEMBER'S THICKNESS.
2. BEAM BOTTOM FLANGE KICKER IS PERMITTED TO BE PLACED ON EITHER SIDE OF THE SPREADER BEAM. DELEGATED CONNECTION ENGINEER SHALL DESIGN FOR 5k ULTIMATE AXIAL LOAD (IN ADDITION TO THE SHEAR LOAD) AT THE SIDE OF THE BEAM BOTTOM FLANGE KICKER.
3. IF THE OPERABLE PARTITION IS DIRECTLY IN-LINE WITH THE STEEL BEAM ABOVE, IT IS PERMITTED TO CONNECT THE HANGER/PLATE ASSEMBLY DIRECTLY TO THE STEEL BEAM. IN THAT INSTANCE, ORIENT THE PLATE CONNECTING TO THE BEAM BOTTOM FLANGE PARALLEL WITH THE BEAM WEB.



SECTION A-A

C1 / S326 OPERABLE PARTITION SUPPORTS - PARALLEL TO BEAMS

SCALE: 1" = 1'-0"

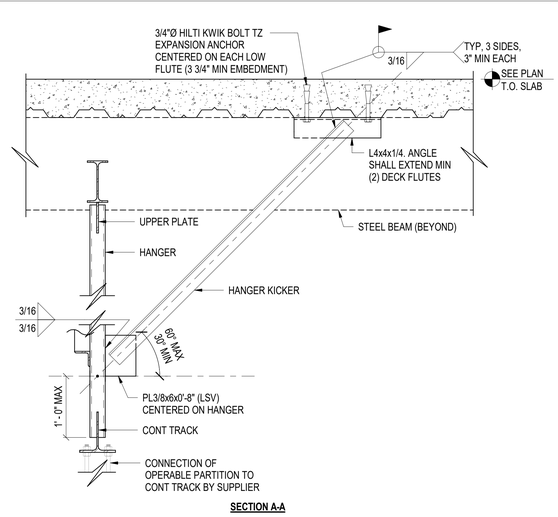


NOTES:

1. HSS SLOT WIDTH AT CONNECTION SHALL NOT EXCEED 1/8" OF THE CONNECTING MEMBER'S THICKNESS.
2. IF THE OPERABLE PARTITION IS DIRECTLY IN-LINE WITH THE STEEL BEAM ABOVE, IT IS PERMITTED TO CONNECT THE HANGER/PLATE ASSEMBLY DIRECTLY TO THE STEEL BEAM. IN THAT INSTANCE, ORIENT THE PLATE CONNECTING TO THE BEAM BOTTOM FLANGE PARALLEL WITH THE BEAM WEB.

A1 / S326 OPERABLE PARTITION SUPPORTS - PERP. TO BEAMS

SCALE: 1" = 1'-0"



SECTION A-A

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
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AA0002981 | LC26000427

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PROJECT NO:
240089.00

STEEL TYPICAL DETAILS

SHEET NUMBER:
S326

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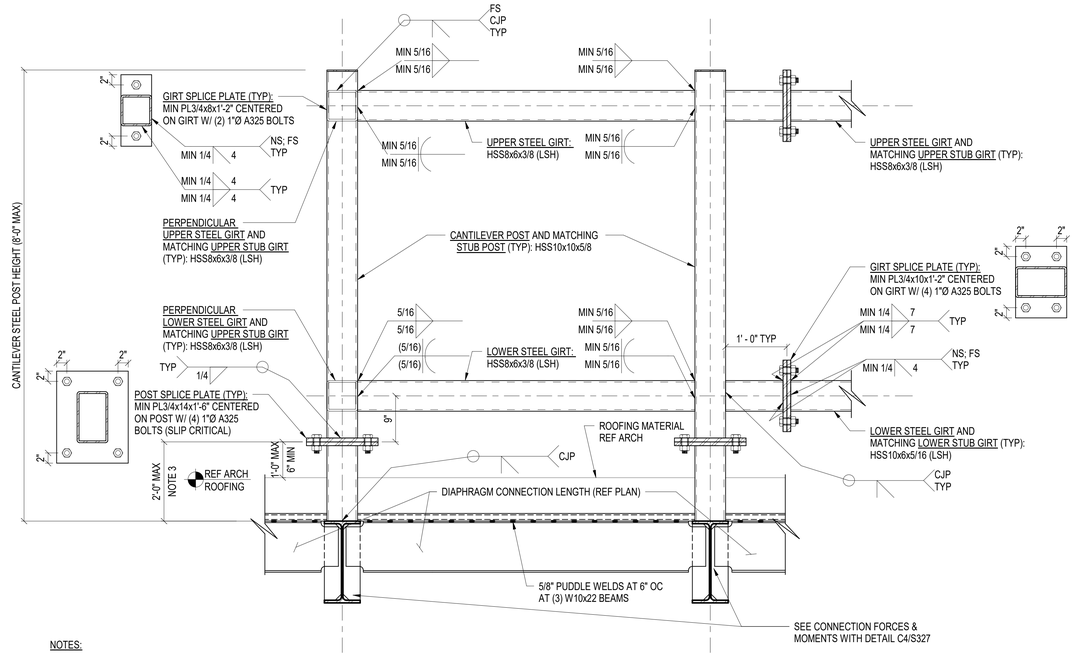
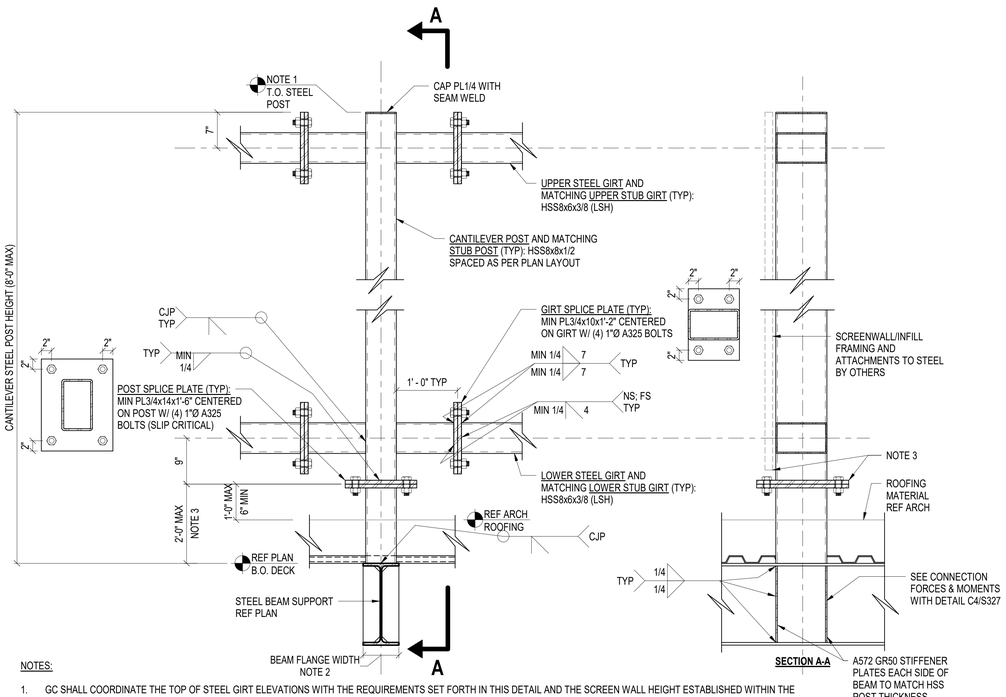


NOTES:

1. GC SHALL COORDINATE THE TOP OF STEEL GIRT ELEVATIONS WITH THE REQUIREMENTS SET FORTH IN THIS DETAIL AND THE SCREEN WALL HEIGHT ESTABLISHED WITHIN THE ARCHITECTURAL DRAWINGS.
2. GC SHALL COORDINATE STEEL BEAM MEMBER SIZES SUCH THAT THE STEEL BEAM FLANGE WIDTH IS WIDER THAN THE HSS POST WIDTH + 1/2" EACH SIDE. IF THIS IS NOT ACHIEVED, THE STEEL BEAM SHALL BE UPSIZED TO THE NEXT HEAVIEST BEAM OF MATCHING DEPTH THAT ACHIEVES THIS BEAM WIDTH. IF THIS OCCURS, SUBMIT THESE LOCATIONS TO EOR FOR REVIEW.
3. GC SHALL COORDINATE THE EXACT ELEVATION OF THE POST SPLICE PLATE WITH THE BOTTOM OF SCREEN WALL, ROOFING THICKNESS, AND OTHER RELEVANT CRITERIA.
4. EXPOSED STRUCTURAL STEEL SHALL BE HOT-DIP GALVANIZED.
5. GC OPTION: WHERE BOLTED SPLICES ARE SHOWN, IT IS PERMITTED TO PROVIDE A SINGLE CONTINUOUS MEMBER IN LIEU OF THE BOLTED SPLICE.

TYPICAL CANTILEVER STEEL SCREEN WALL (ON STEEL BEAM) - HEIGHTS UP TO 8'-0" (SCREENWALL AT CORNERS)

C1 / S327 SCALE: 1" = 1'-0"



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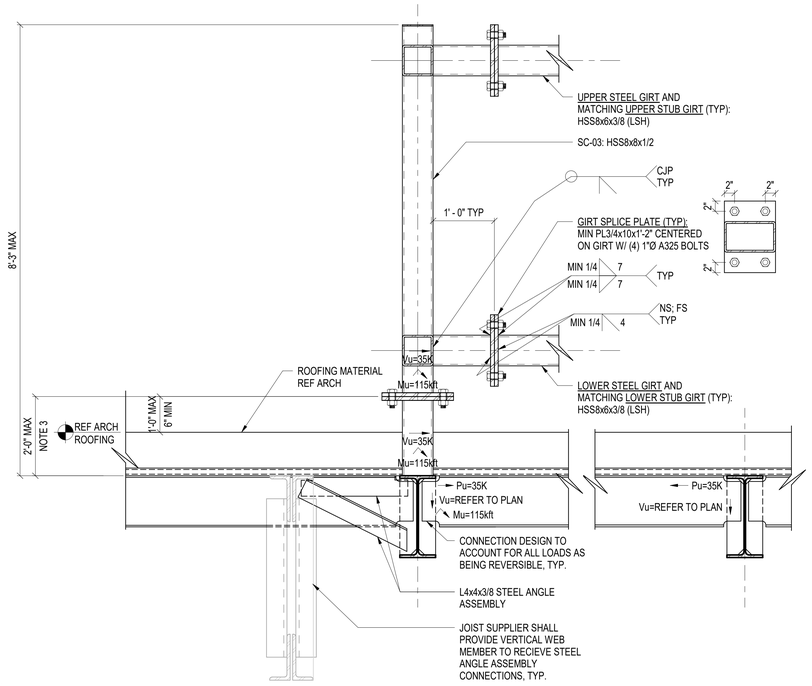
2. GC SHALL COORDINATE STEEL BEAM MEMBER SIZES SUCH THAT THE STEEL BEAM FLANGE WIDTH IS WIDER THAN THE HSS POST WIDTH + 1/2" EACH SIDE. IF THIS IS NOT ACHIEVED, THE STEEL BEAM SHALL BE UPSIZED TO THE NEXT HEAVIEST BEAM OF MATCHING DEPTH THAT ACHIEVES THIS BEAM WIDTH. IF THIS OCCURS, SUBMIT THESE LOCATIONS TO EOR FOR REVIEW.
3. GC SHALL COORDINATE THE EXACT ELEVATION OF THE POST SPLICE PLATE WITH THE BOTTOM OF SCREEN WALL, ROOFING THICKNESS, AND OTHER RELEVANT CRITERIA.
4. EXPOSED STRUCTURAL STEEL SHALL BE HOT-DIP GALVANIZED.
5. GC OPTION: WHERE BOLTED SPLICES ARE SHOWN, IT IS PERMITTED TO PROVIDE A SINGLE CONTINUOUS MEMBER IN LIEU OF THE BOLTED SPLICE.

TYPICAL CANTILEVER STEEL SCREEN WALL (ON STEEL BEAM) - HEIGHTS UP TO 8'-0" (SCREENWALL PERPENDICULAR TO BEAMS)

A1 / S327 SCALE: 1" = 1'-0"

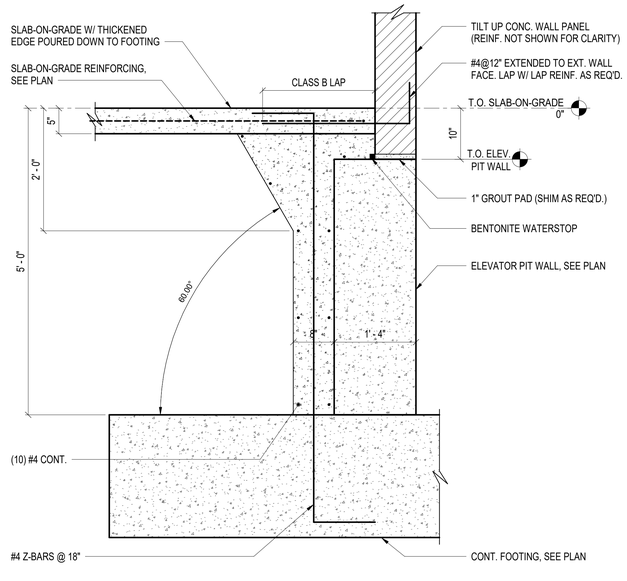


NOTES:

1. GC SHALL COORDINATE THE TOP OF STEEL GIRT ELEVATIONS WITH THE REQUIREMENTS SET FORTH IN THIS DETAIL AND THE SCREEN WALL HEIGHT ESTABLISHED WITHIN THE ARCHITECTURAL DRAWINGS.
2. GC SHALL COORDINATE STEEL BEAM MEMBER SIZES SUCH THAT THE STEEL BEAM FLANGE WIDTH IS WIDER THAN THE HSS POST WIDTH + 1/2" EACH SIDE. IF THIS IS NOT ACHIEVED, THE STEEL BEAM SHALL BE UPSIZED TO THE NEXT HEAVIEST BEAM OF MATCHING DEPTH THAT ACHIEVES THIS BEAM WIDTH. IF THIS OCCURS, SUBMIT THESE LOCATIONS TO EOR FOR REVIEW.
3. GC SHALL COORDINATE THE EXACT ELEVATION OF THE POST SPLICE PLATE WITH THE BOTTOM OF SCREEN WALL, ROOFING THICKNESS, AND OTHER RELEVANT CRITERIA.
4. EXPOSED STRUCTURAL STEEL SHALL BE HOT-DIP GALVANIZED.
5. GC OPTION: WHERE BOLTED SPLICES ARE SHOWN, IT IS PERMITTED TO PROVIDE A SINGLE CONTINUOUS MEMBER IN LIEU OF THE BOLTED SPLICE.

TYPICAL CANTILEVER STEEL SCREEN WALL (ON STEEL BEAM) - HEIGHTS UP TO 8'-0" (SCREEN WALL AT CORNERS)

C4 / S327 SCALE: 1" = 1'-0"



THICKENED EDGE DETAIL

A4 / S327 SCALE: 1" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
www.mcsal.com FL CA#26616

Straticon
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Elie El Zghayar, PhD, PE on **06/12/2026**
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TAMBONE COMPANIES

SEACOAST BANK HEADQUARTERS

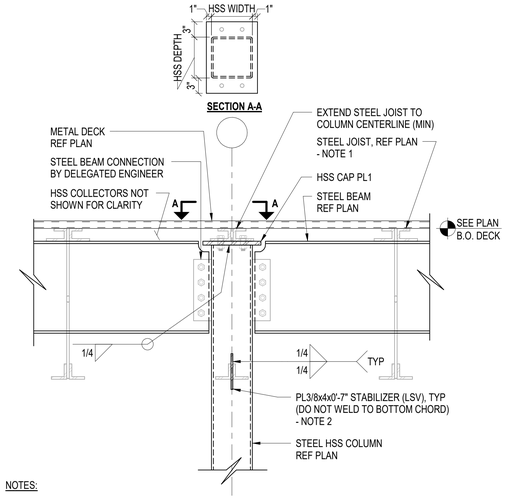
815 SOUTH COLORADO AVE
STUART, FL 34994

PROJECT NO:
240089.00

STEEL TYPICAL DETAILS

SHEET NUMBER:
S327

THE LINE SHOWN ABOVE IS 3" LONG ON ORIGINAL IF NOT SHEET IS ORIGINAL PAPER SIZE

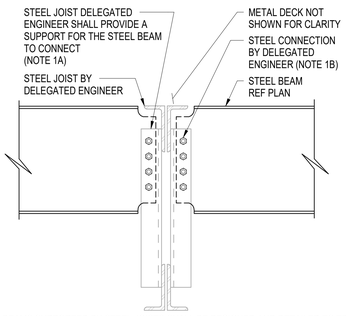


NOTES:

1. STEEL JOIST CONNECTIONS TO STEEL BEAMS AND TO CAP PLATES REQUIRED FOR ERECTION AND THE FINAL DESIGN STATE SHALL BE PROVIDED BY THE STEEL JOIST SUPPLIER/ERECTOR AND THEIR DELEGATED ENGINEER.
2. THE JOIST SUPPLIER SHALL SIZE THE GAP BETWEEN THE JOIST BOTTOM CHORD ELEMENTS TO ENGAGE THE STABILIZER PLATE IN THE OUT-OF-PLANE DIRECTION ONLY. THE STABILIZER PLATE SIZE SHOWN IS THE MINIMUM REQUIREMENT, GC SHALL COORDINATE THE EXACT SIZE AND PLACEMENT WITH THE JOIST SUPPLIER/ERECTOR.

JOIST BEARING CONNECTION AT WIDE FLANGE GIRDERS AND HSS COLUMN

D1 / S331 SCALE: 1" = 1'-0"

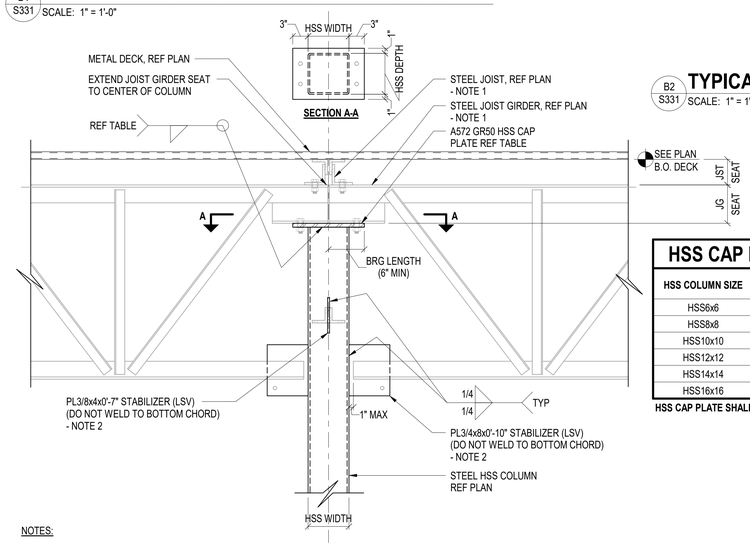


NOTES:

1. STEEL BEAMS SUPPORTED BY STEEL JOISTS SHALL BE DESIGNED AND DETAILED TO TRANSFER THE REACTIONS INDICATED WITHIN THE CONTRACT DOCUMENTS. GC SHALL COORDINATE BETWEEN THE STEEL JOIST DELEGATED ENGINEER AND THE STEEL CONNECTIONS DELEGATED ENGINEER TO PROVIDE THE FULL CONNECTION DESIGN.
   A. THE DESIGN OF THE JOIST WEB MEMBER AND ITS FORCE TRANSFER WITHIN THE STEEL JOIST, INCLUDING ANY LOCALIZED EFFECTS OF JOIST WEB BOLT HOLES AND OTHER WEB MODIFICATIONS REQUIRED TO ACHIEVE THE CONNECTION, SHALL BE DESIGNED BY THE STEEL JOIST DELEGATED ENGINEER.
   B. THE CONNECTION DESIGN FROM THE STEEL BEAM TO THE STEEL JOIST, INCLUDING BOLT/WELD SPECIFICATIONS, COPING/BEAM MODIFICATION DESIGNS, AND OTHER RELEVANT REQUIREMENTS, SHALL BE DESIGNED BY THE STEEL CONNECTION DELEGATED ENGINEER.

STEEL BEAMS SUPPORTED BY STEEL JOISTS

B1 / S331 SCALE: 1" = 1'-0"



HSS CAP PLATE TABLE

HSS COLUMN SIZE	CAP PLATE THICKNESS	WELD SIZE
HSS6x6	1"	1/4
HSS8x8	1"	1/4
HSS10x10	1 1/4"	5/16
HSS12x12	1 1/4"	5/16
HSS14x14	1 1/2"	5/16
HSS16x16	1 1/2"	5/16

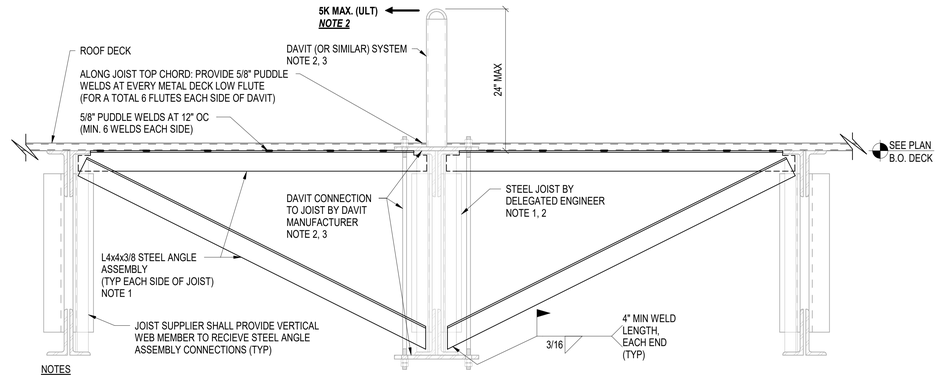
HSS CAP PLATE SHALL BE A572 GR50 MATERIAL

NOTES:

1. STEEL JOIST CONNECTIONS TO JOIST GIRDERS AND JOIST GIRDER CONNECTIONS TO CAP PLATE REQUIRED FOR ERECTION AND THE FINAL DESIGN STATE SHALL BE PROVIDED BY THE STEEL JOIST SUPPLIER/ERECTOR AND THEIR DELEGATED ENGINEER.
2. THE JOIST SUPPLIER SHALL SIZE THE GAP BETWEEN THE JOIST BOTTOM CHORD ELEMENTS TO ENGAGE THE STABILIZER PLATE IN THE OUT-OF-PLANE DIRECTION ONLY. THE STABILIZER PLATE SIZE SHOWN IS THE MINIMUM REQUIREMENT, GC SHALL COORDINATE THE EXACT SIZE AND PLACEMENT WITH THE JOIST SUPPLIER/ERECTOR.

JOIST GIRDER BEARING CONNECTION TO STEEL HSS COLUMN

A1 / S331 SCALE: 1" = 1'-0"

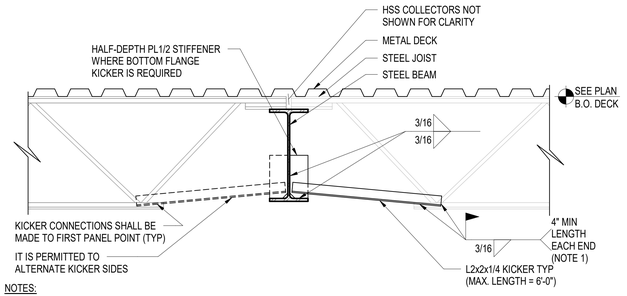


NOTES

1. THE FIELD INSTALLED STEEL ANGLE ASSEMBLY SHOWN WITHIN THIS DETAIL IS PROVIDED TO RESOLVE STEEL JOIST OUT-OF-PLANE FORCES INTO THE METAL DECK DIAPHRAGM ONLY. THE STEEL JOIST DELEGATED ENGINEER IS RESPONSIBLE FOR RESOLVING VERTICAL SHEAR/BENDING FORCES IMPOSED BY THE PROPOSED SYSTEM AS WELL AS IN PLANE AXIAL FORCES ALONG THE JOIST TOP CHORD. JOISTS THAT ARE NOT CAPABLE OF WITHSTANDING THESE ADDITIONAL LOADS SHALL BE RETROFITTED AS REQUIRED. THE RETROFIT DESIGN SHALL BE PROVIDED BY THE JOIST MANUFACTURER.
2. DAVIT LOADS SHALL BE PROVIDED BY THE DAVIT SUPPLIER AND THEIR DELEGATED ENGINEER. THESE LOADS SHALL BE SENT TO THE STEEL JOIST DELEGATED ENGINEER BY THE GENERAL CONTRACTOR. GC SHALL CONFIRM THAT THE DAVIT LOADING DOES NOT EXCEED THE CRITERIA SET FORTH WITHIN THIS DETAIL.
3. THE DAVIT SUPPLIER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE CONNECTION TO THE JOIST/BEAM. AT A MINIMUM, THE DAVIT SUPPLIER AND THEIR DELEGATED ENGINEER SHALL SPECIFY THE TOP PLATE CONNECTION TO THE JOIST TOP CHORD WITH A 3/16" ALL AROUND FILLET WELD. GENERAL CONTRACTOR SHALL COORDINATE BETWEEN DAVIT AND JOIST MANUFACTURER TO ENSURE THAT ALL IMPOSED LOADS ARE ACCOUNTED FOR AND PROPERLY RESOLVED.

TYPICAL DAVIT (OR SIMILAR) DETAIL AT STEEL JOIST

D2 / S331 SCALE: 1" = 1'-0"

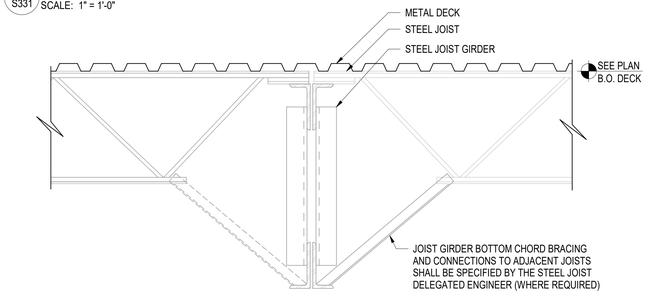


NOTES:

1. KICKERS SHALL NOT BE INSTALLED UNTIL ALL SELF-WEIGHT AND SUPERIMPOSED DEAD LOADS ARE APPLIED TO THE STRUCTURE.

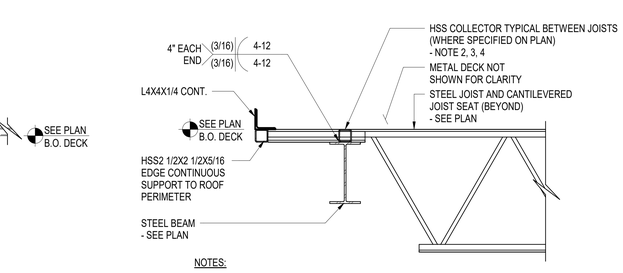
TYPICAL STEEL JOIST CONNECTION AT INTERIOR WIDE FLANGE BEAM

C2 / S331 SCALE: 1" = 1'-0"



TYPICAL STEEL JOIST CONNECTION AT INTERIOR JOIST GIRDER

B2 / S331 SCALE: 1" = 1'-0"

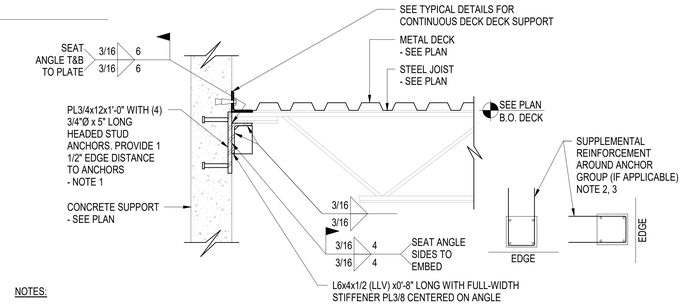


NOTES:

1. FASTEN DECK TO COLLECTOR STEEL AS REQUIRED PER DECK FASTENING REQUIREMENTS.
2. HSS COLLECTOR DEPTH SHALL MATCH THE DEPTH OF THE JOIST SEAT. TYPICAL HSS-JOIST RELATIONSHIPS SHALL BE AS FOLLOWS:
   A. K SERIES: HSS2 1/2x 2 1/2x 3/16
   B. LH/DLH SERIES: HSS5x3x3/16 (LSV)
3. HSS COLLECTORS SHALL BE CENTERED BETWEEN EACH JOIST AND SHALL TERMINATE 3" FROM THE JOIST SEAT. CONTRACTOR SHALL COORDINATE BETWEEN TRADES AS REQUIRED.
4. HSS COLLECTORS ARE REQUIRED AT ALL STEEL BEAMS WITH AN AXIAL LOAD REACTION LISTED ON PLAN (UNO).

TYPICAL HSS COLLECTORS AT JOIST BEARING ON STEEL GIRDER

D4 / S331 SCALE: 1" = 1'-0"

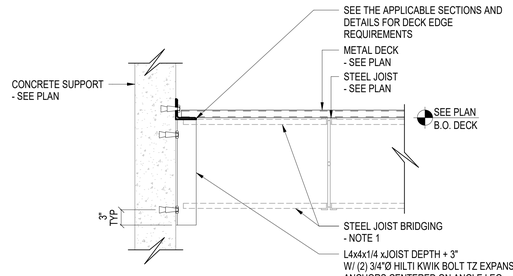


NOTES:

1. PLATE SHALL BE CENTERED HORIZONTALLY ON SEAT ANGLE. TOP OF PLATE SHALL EQUAL BOTTOM OF DECK ELEVATION.
2. EMBED PLATES NEAR VERT/HORIZ EDGES: IF DISTANCE FROM STUD TO A CONCRETE EDGE IS LESS THAN 1'-8", PROVIDE 1#5 U-BAND WITH 36" LEGS.
3. EMBED PLATES NEAR CORNERS: IF DISTANCE FROM STUD TO EACH CONCRETE EDGE AT A CORNER IS LESS THAN 3'-0" FOR EACH EDGE PROVIDE 1#5 U-BAND WITH 36" LEGS IN EACH DIRECTION.

TYPICAL K-SERIES STEEL JOIST CONNECTION TO CONCRETE SUPPORT

C4 / S331 SCALE: 1" = 1'-0"

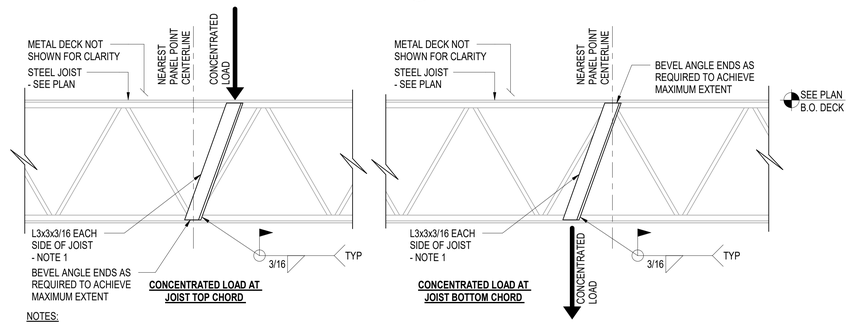


NOTES:

1. STEEL JOIST BRIDGING SIZES, QUANTITIES, LOCATIONS AND CONNECTIONS TO THE STEEL ANGLE SHALL BE DETERMINED/PROVIDED BY THE STEEL JOIST SUPPLIER AND DELEGATED ENGINEER.
2. GC SHALL COORDINATE WITH THE JOIST SUPPLIER TO DETERMINE THE STEEL ANGLE LENGTHS AND REQUIRED BRIDGING ANGLE PLAN LOCATIONS AND ELEVATIONS.

TYPICAL JOIST BRIDGING CONNECTION AT CONCRETE SUPPORT

B4 / S331 SCALE: 1" = 1'-0"



NOTES:

1. THE CONTRACTOR SHALL COORDINATE THE INSTALLATION OF THESE STEEL ANGLES BASED ON THE FOLLOWING RULES:
   A. A FIELD INSTALLED JOIST WEB IS *REQUIRED* IF MORE THAN 100 LBS PLACED BETWEEN TWO GIVEN PANEL POINTS.
   B. FOR SPECIAL JOISTS AND OTHER UNIQUE LOADING CONDITIONS SHOWN WITHIN THE PLANS, THE JOIST SUPPLIER SHALL COORDINATE THE PANEL POINT LOCATIONS TO RECIEVE THE SPECIFIED LOADS.

FIELD INSTALLED JOIST WEB MEMBERS AT CONCENTRATED LOADS

A4 / S331 SCALE: 1" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
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ELIE EL ZGHAYER
LICENSE
No.78483
STATE OF FLORIDA
PROFESSIONAL ENGINEER

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SEACOAST BANK HEADQUARTERS

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STUART, FL 34994

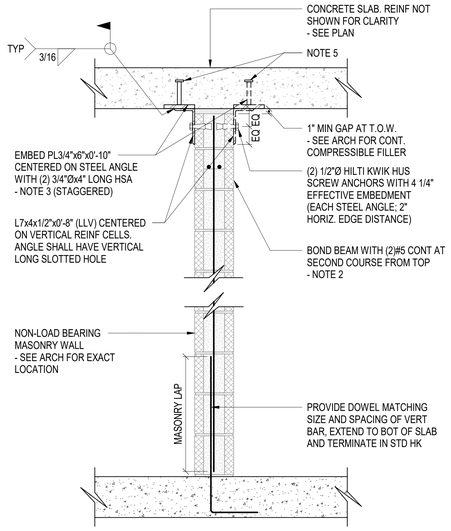
PROJECT NO.
240089.00

STEEL JOIST TYPICAL DETAILS

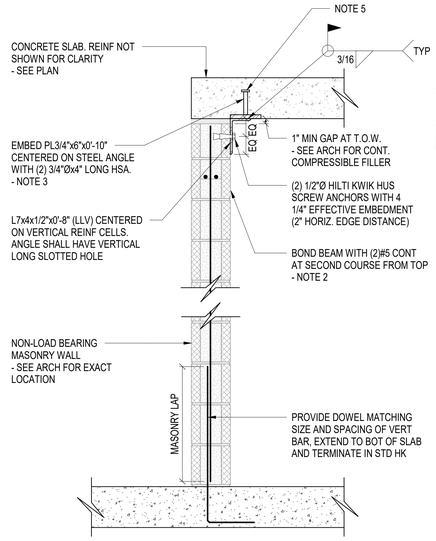
SHEET NUMBER:
S331

NON-LOAD BEARING MASONRY WALL SCHEDULE

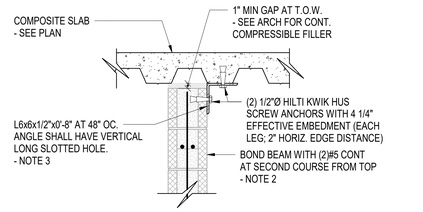
CONDITION		CMU TYPE		GROUTED CELLS / VERTICAL REINFORCEMENT		
LOCATION	HEIGHT	f'm (PSI)	THICKNESS	LAYERS	SIZE	SPACING
INTERIOR	16'-0" AND LESS	1,900	8"	1	#5	48"
EXTERIOR	8'-0" AND LESS	1,900	8"	1	#5	48"
	12'-0" AND LESS	1,900	8"	1	#5	32"
	14'-0" AND LESS	1,900	8"	1	#5	16"
	16'-0" AND LESS	1,900	8"	1	#5	8"
WALLS SUBJECT TO VEHICLE IMPACT (NOTE 4)	12'-0" AND LESS	1,900	8"	1	#6	16"



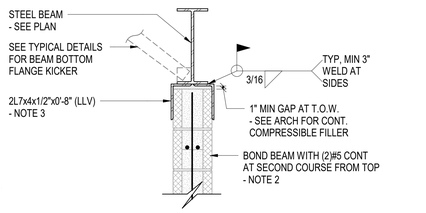
INTERIOR NON-LOAD BEARING MASONRY WALL (CONNECTION TO CONCRETE SLAB)



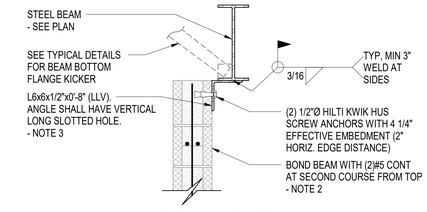
EXTERIOR NON-LOAD BEARING MASONRY WALL (CONNECTION TO CONCRETE SLAB)



INTERIOR NON-LOAD BEARING MASONRY WALL (CONNECTION TO COMPOSITE DECK)



INTERIOR NON-LOAD BEARING MASONRY WALL (CONNECTION TO STEEL BEAM)



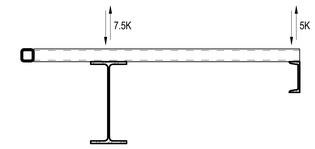
INTERIOR NON-LOAD BEARING MASONRY WALL (CONNECTION TO STEEL BEAM)

NOTES:

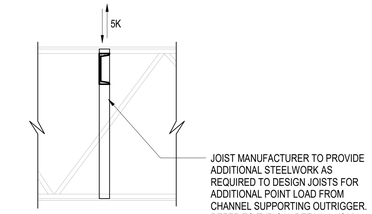
1. SEE THE GENERAL NOTES FOR HORIZONTAL JOINT REINFORCEMENT REQUIREMENTS.
2. SEE TYPICAL DETAIL REQUIREMENTS FOR ADDITIONAL BARS AT BOND BEAM CORNERS AT INTERSECTING WALLS.
3. TOP OF WALL CLIPS SHALL BE PROVIDE 8" MAX. FROM EACH END AND AT A SPACING NOT TO EXCEED THE SPACING OF THE GROUTED/VERTICAL CELLS.
4. FOR WALLS SUBJECT TO VEHICLE IMPACT, THE FIRST FOUR (4) COURSES OF MASONRY SHALL BE GROUT FILLED.
5. APPROVED SUBSTITUTION: FOR MILD-REINFORCED CONCRETE SLABS ONLY *(NOT APPLICABLE FOR POST-TENSIONED CONCRETE)* IT IS PERMITTED FOR THE STEEL ANGLE HORIZONTAL LEG TO BE CONNECTED TO THE CONCRETE SLAB ABOVE DIRECTLY WITH (2) 5/8"Ø HILTI KWIK BOLT TZ EXP ANCHORS WITH 4" EFFECTIVE EMBEDMENT (EACH STEEL ANGLE; 2" HORIZ. EDGE DISTANCE) IN LIEU OF THE EMBED PLATE.
6. NOT ALL NON-LOAD BEARING MASONRY WALLS ARE SHOWN WITHIN THE STRUCTURAL DRAWINGS. REFER TO THE ARCHITECTURAL DRAWINGS FOR ALL LOCATIONS WHERE NON-LOAD BEARING MASONRY WALLS ARE REQUIRED.

E1 / S332 **NON-LOAD BEARING MASONRY WALL SCHEDULE**

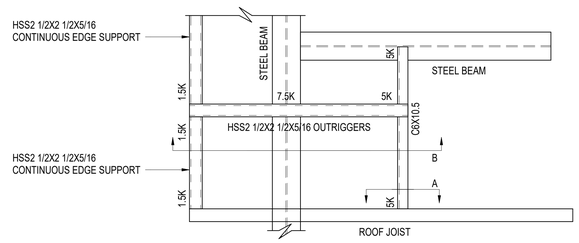
SCALE: 1" = 1'-0"



DETAIL B



DETAIL A



C5 / S332 **TYPICAL OUTRIGGER AND ROOF EDGE SUPPORT DETAILS**

SCALE: 1" = 1'-0"

8" LINTEL SCHEDULE (NON-LOAD BEARING)

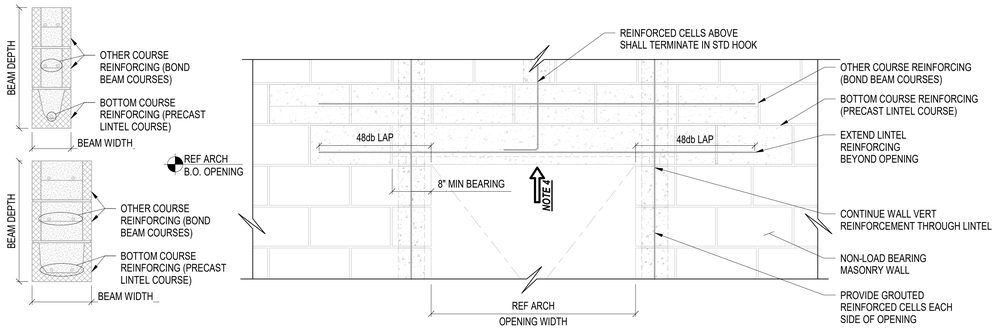
OPENING WIDTH	QUANTITY OF COURSES	BEAM WIDTH (NOMINAL)	BEAM DEPTH (NOMINAL)	BOTTOM COURSE REINFORCING	OTHER COURSE REINFORCING	REMARKS
4'-0" AND LESS	1	8"	8"	(1)#5	N/A	
9'-4" AND LESS	2	8"	16"	(1)#5	(1)#5	
14'-0" AND LESS	3	8"	24"	(1)#5	(1)#5	

12" LINTEL SCHEDULE (NON-LOAD BEARING)

OPENING WIDTH	QUANTITY OF COURSES	BEAM WIDTH (NOMINAL)	BEAM DEPTH (NOMINAL)	BOTTOM COURSE REINFORCING	OTHER COURSE REINFORCING	REMARKS
4'-0" AND LESS	1	12"	8"	(2)#5	N/A	
9'-4" AND LESS	3	12"	24"	(2)#5	(2)#5	
14'-0" AND LESS	4	12"	32"	(2)#5	(2)#5	

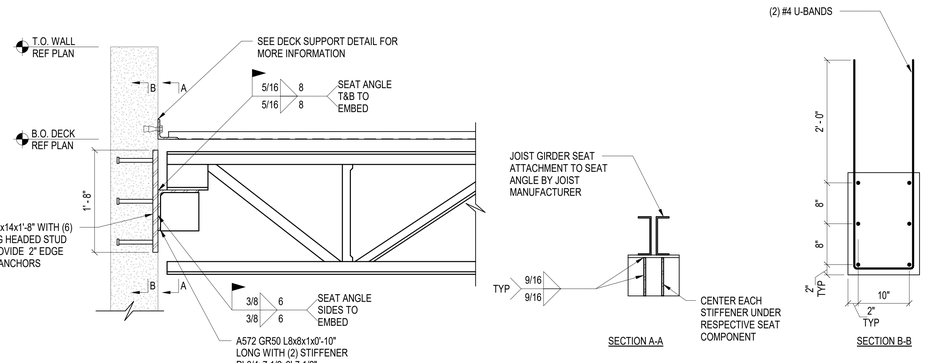
LINTEL SCHEDULE NOTES:

1. WHERE THE BOTTOM COURSE OF A TIE BEAM OR BOND BEAM MATCHES THE TOP OF OPENING ELEVATION, A LINTEL IS NOT REQUIRED.
2. PROVIDE HORIZONTAL JOINT REINFORCEMENT AT EVERY LINTEL COURSE.
3. AT OPENING SILLS, ONLY A SINGLE BOND BEAM COURSE IS REQUIRED.
4. CONTRACTOR SHALL PROVIDE SHORING AS REQUIRED UNTIL LINTEL CONSTRUCTION IS COMPLETE AND GROUT HAS CURED.
5. NOT ALL NON-LOAD BEARING MASONRY WALLS ARE SHOWN WITHIN THE STRUCTURAL DRAWINGS. REFER TO THE ARCHITECTURAL DRAWINGS FOR ALL LOCATIONS WHERE NON-LOAD BEARING MASONRY WALLS ARE REQUIRED.



A1 / S332 **MASONRY WALL LINTEL SCHEDULE (NON-LOAD BEARING)**

SCALE: 1" = 1'-0"



NOTES:

1. PLATE SHALL BE CENTERED HORIZONTALLY ON SEAT ANGLE. TOP OF PLATE SHALL EQUAL BOTTOM OF DECK ELEVATION.

A4 / S332 **JOIST GIRDER CONNECTION AT TILT WALL**

SCALE: 1" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
www.mcsal.com FL CA#26616

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ELIE EL ZGHAYAR LICENSE No.78483 STATE OF FLORIDA PROFESSIONAL ENGINEER

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Elie El Zghayar, PhD, PE on **06/10/2026**
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TAMBONE COMPANIES

SEACOAST BANK HEADQUARTERS

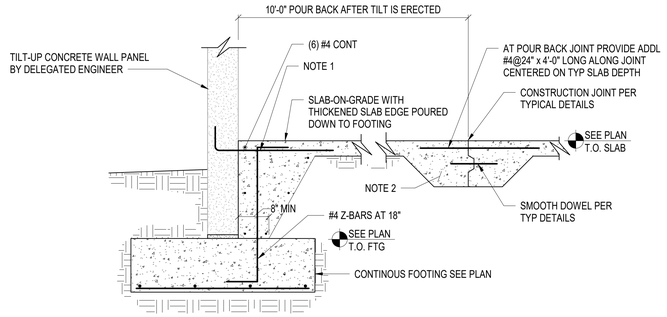
815 SOUTH COLORADO AVE
STUART, FL 34994

PROJECT NO:
240089.00

STEEL JOIST TYPICAL DETAILS

SHEET NUMBER:
S332

THE LINE SHOWN ABOVE IS 1" LONG ON ORIGINAL DRAWINGS SHEET IS ORIGINAL PAPER SIZE

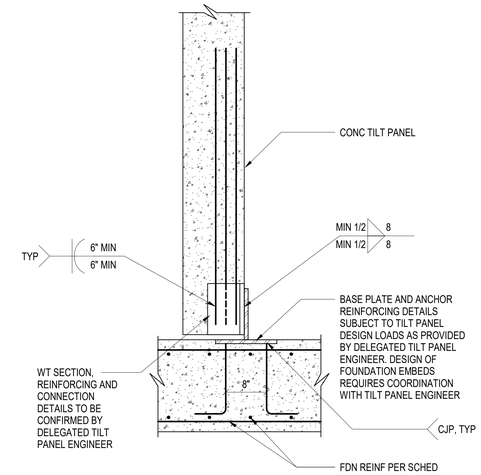


NOTES:

1. TILT PANEL CONNECTION TO SLAB-ON-GRADE DOWELS SPACED AS REQUIRED BY THE DELEGATED TILT PANEL ENGINEER. AT MINIMUM PROVIDE #4@18".
2. WITHIN POUR BACK REGION, PROVIDE #3@12" EACH WAY IN LIEU OF TYPICAL SLAB REINFORCEMENT.
3. PAD FOUNDATION SHOWN FOR ILLUSTRATION PURPOSES ONLY. FOUNDATIONS TO BE CONFIRMED UPON RECEIPT OF GEOTECHNICAL REPORT.

C1 / S361 TILT WALL CONNECTION TO SLAB-ON-GRADE

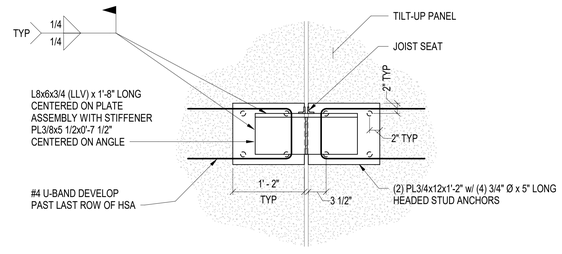
SCALE: 3/4" = 1'-0"



NOTE:
REFER TO PANEL ELEVATIONS FOR LOCATIONS

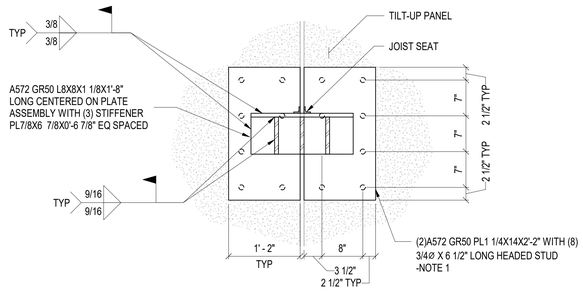
D6 / S361 TILT PANEL TIE DOWN (REQUIRED WHERE PANEL IS IN NET UPLIFT)

SCALE: 1" = 1'-0"



B1 / S361 TYPICAL JOIST SEAT AT PANEL JOINT

SCALE: 1" = 1'-0"

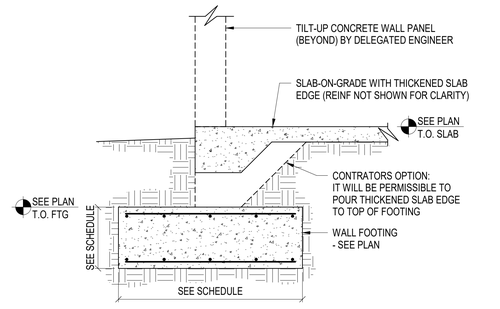


NOTES:

1. REFER TO THE EMBED PLATE EDGE/CORNER REINFORCEMENT SCHEDULE AND PROVIDE SUPPLEMENTARY REINFORCEMENT FOR THE EQUIVALENT OF W27 BETWEEN 111k AND 156k EMBED PLATE.

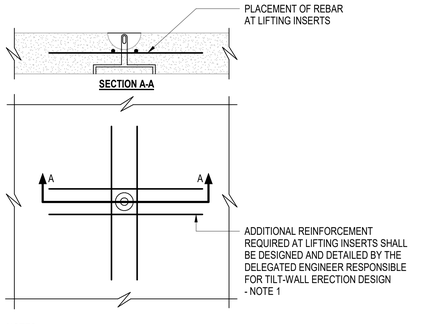
B3 / S361 BRIDGING DETAIL AT JOIST GIRDER CONNECTION AT PANEL JOINT

SCALE: 1" = 1'-0"



B5 / S361 TYPICAL TILT-UP CONCRETE WALL AT OPENING

SCALE: 3/4" = 1'-0"

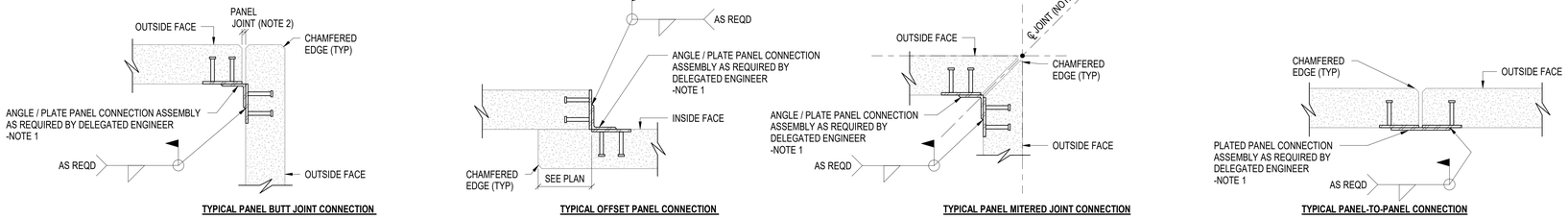


NOTES:

1. THE DELEGATED TILT-UP ERECTION ENGINEER SHALL DETAIL A MINIMUM OF (1)#4x4'-0" EACH SIDE, EACH WAY (4 TOTAL) AT A LIFTING INSERT. THE EXACT REINFORCEMENT SPECIFICATION SHALL BE DETERMINED/PROVIDED BY THE DELEGATED ENGINEER.

A1 / S361 TYPICAL ADDITIONAL REINFORCING REQUIREMENTS AT LIFTING INSERTS

SCALE: 1" = 1'-0"



NOTES:

1. PANEL TO PANEL CONNECTIONS SHALL BE DESIGNED BY THE DELEGATED TILT PANEL ENGINEER.
2. TYPICAL PANEL-TO-PANEL JOINTS SHALL BE 3/4", UNLESS NOTED OTHERWISE. REFER TO ARCHITECT FOR MORE STRINGENT PANEL JOINT REQUIREMENTS.

A2 / S361 TYPICAL CONCRETE TILT-UP WALL PANEL-TO-PANEL CONNECTION DETAILS

SCALE: 1" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
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ELIE EL ZGHAYAR — LICENSE No.78483 — STATE OF FLORIDA — PROFESSIONAL ENGINEER

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TAMBONE COMPANIES

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STUART, FL 34994

PROJECT NO:
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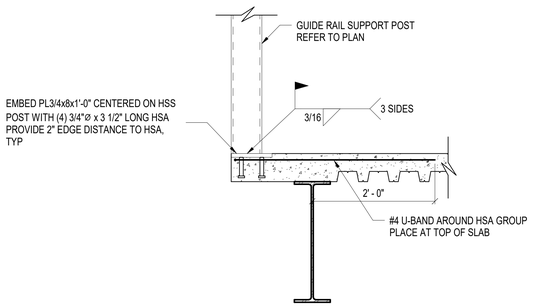
TILT-UP CONCRETE TYPICAL DETAILS

SHEET NUMBER:
S361

THE LINE SHOWN ABOVE IS 1" LONG ON ORIGINAL PLOT. IF NOT, ADJUST SCALES ACCORDINGLY SHEET'S ORIGINAL PAPER SIZE

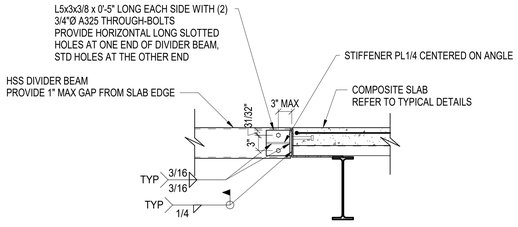


NOTES:

1. FINAL PLAN LOCATIONS, ELEVATIONS, QUANTITIES, ETC... FOR ALL ELEVATOR STEEL SHALL BE COORDINATED WITH ARCHITECTURAL & ELEVATOR MANUFACTURER DRAWINGS.
2. DETAIL ALSO APPLIES AT GUIDE RAIL POST CONNECTION TO SLAB-ON-GRADE.

D1/S401 GUIDE RAIL POST BASE CONNECTION AT ELEVATED SLAB

SCALE: 1" = 1'-0"



NOTES:

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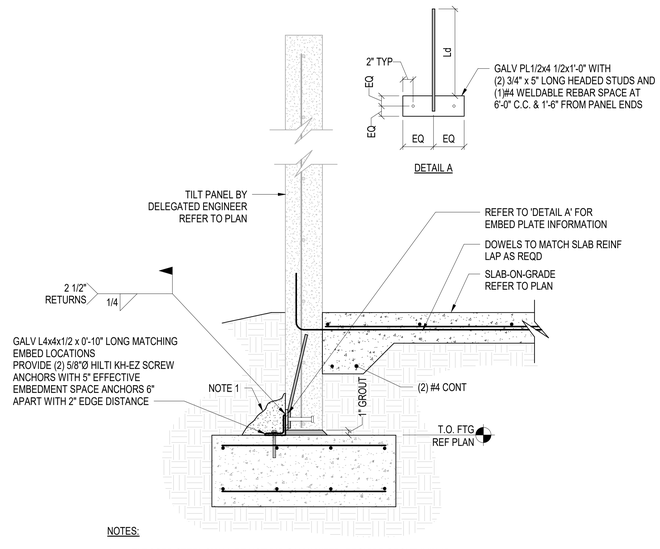
B1/S401 DIVIDER BEAM CONNECTION AT ELEVATED SLAB

SCALE: 1" = 1'-0"

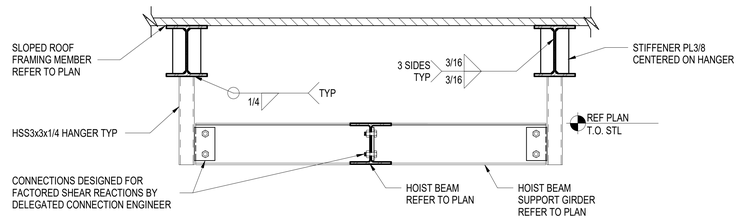


NOTES:

1. EXPOSED STEEL SHALL BE HOT-DIP GALVANIZED. PROVIDE GROUT (MIN 3" COVER) AFTER PANEL ERECTION IS COMPLETE.
2. PAD FOUNDATION SHOWN FOR ILLUSTRATION PURPOSES ONLY. FOUNDATIONS TO BE CONFIRMED UPON RECEIPT OF GEOTECHNICAL REPORT.

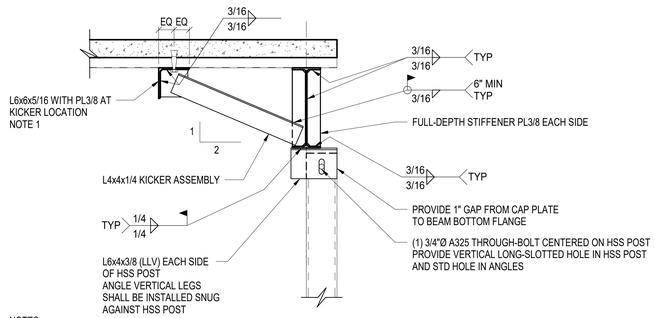
A1/S401 FOUNDATION SECTION AT CANTILEVERED TILT PANEL

SCALE: 1" = 1'-0"



D3/S401 ELEVATOR HOISTWAY FRAMING CONNECTION DETAIL

SCALE: 1" = 1'-0"

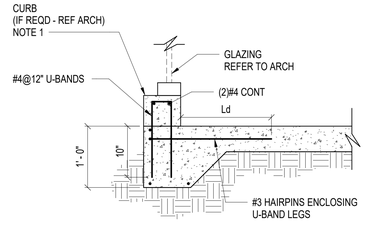


NOTES:

1. CONNECTION ANGLE SHALL SPAN (2) DECK LOW FLUTES. DETERMINE LENGTH AS REQUIRED TO ACCOMMODATE KICKER ASSEMBLY, AND PROVIDE MIN (2) 3/4"Ø HILTI KWIK BOLT TZ EXPANSION ANCHORS WITH 3 3/4" EMBEDMENT INTO (2) CONSECUTIVE DECK LOW FLUTES.
2. FINAL PLAN LOCATIONS, ELEVATIONS, QUANTITIES, ETC... FOR ALL ELEVATOR STEEL SHALL BE COORDINATED WITH ARCHITECTURAL & ELEVATOR MANUFACTURER DRAWINGS.

B3/S401 ELEVATOR STEEL LATERAL CONNECTION AT COMPOSITE SLAB

SCALE: 1" = 1'-0"

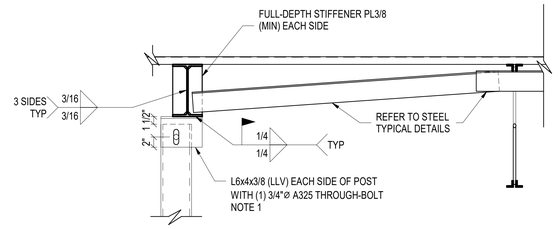


NOTES:

1. REFER TO ARCH FOR CURB EXTENTS AND PLAN LOCATIONS. CURB SHALL BE MIN 7" WIDE AND MAX 8" TALL.

D5/S401 SLAB-ON-GRADE EDGE DETAIL AT GLAZING

SCALE: 1" = 1'-0"



NOTES:

1. ANGLES SHALL BE FLUSH WITH BEAM FLANGE AT ONE END AND EDGE OF HSS POST AT OTHER END. COORDINATE WITH ELEVATOR MANUFACTURER AS REQUIRED. PROVIDE VERTICAL LONG-SLOTTED HOLES IN HSS, STD HOLES IN ANGLES. CENTER HOLES ON HSS WIDTH. INSTALL SUCH THAT ANGLES ARE SNUG TIGHT AGAINST HSS POST.
2. FINAL PLAN LOCATIONS, ELEVATIONS, QUANTITIES, ETC... FOR ALL ELEVATOR STEEL SHALL BE COORDINATED WITH ARCHITECTURAL & ELEVATOR MANUFACTURER DRAWINGS.

B5/S401 ELEVATOR STEEL LATERAL CONNECTION AT ROOF DECK

SCALE: 1" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
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TAMBONE COMPANIES

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PROJECT NO:
240089.00

FRAMING SECTIONS AND DETAILS

SHEET NUMBER:
S401

EMBED PLATE SCHEDULE

BEAM SIZE	B	L	t	# ANCHORS	# ROWS	ANCHORS PER ROW	Sh	Sv	ANCHOR DIAMETER	ANCHOR LENGTH	NOTES
WP1	12"	16"	3/4"	6	3	2	8"	6"	3/4"	6"	SEE REBAR WITH DETAIL WP1 S502
WP1A	12"	16"	3/4"	6	3	2	8"	6"	3/4"	6"	SEE REBAR WITH DETAIL WP1A S502
WP2	24"	28"	1"	9	3	3	10"	12"	3/4"	6"	SEE REBAR WITH DETAIL WP2 S502
WP2A	28"	28"	1"	9	3	3	10"	12"	3/4"	6"	SEE REBAR WITH DETAIL WP2A S502
WP3	28"	34"	1 1/4"	16	4	4	10"	8"	3/4"	6"	SEE REBAR WITH DETAIL WP3 S502
WP4	34"	34"	1 1/4"	16	4	4	10"	10"	3/4"	6"	SEE REBAR WITH DETAIL WP4 S502
WP5	SEE DETAIL WP5 S502										
WP6	SEE DETAIL WP6 S502										
WP7	20"	28"	1"	9	3	3	8"	10"	3/4"	9.25"	PLATE ASSEMBLY IS DOUBLE SIDED, WELD ANCHORS BOTH SIDES. SEE SUPPLEMENTARY REBAR WITH DETAIL WP7/WP8
WP8	20"	28"	1"	9	3	3	8"	10"	3/4"	6"	PLATE ASSEMBLY IS DOUBLE SIDED, WELD ANCHORS BOTH SIDES. SEE SUPPLEMENTARY REBAR WITH DETAIL WP7/WP8
WP7A	20"	28"	1"	9	3	3	8"	10"	3/4"	6"	SEE REBAR WITH DETAIL WP7 S502
WP8A	20"	28"	1"	9	3	3	8"	10"	3/4"	5.5"	SEE REBAR WITH DETAIL WP8 S502
WP9	8"	18"	1"	4	2	2	4"	10"	3/4"	5.5"	SEE REBAR WITH DETAIL WP9 S502
WP10	20"	22"	1"	9	3	3	8"	9"	3/4"	5.5"	SEE REBAR WITH DETAIL WP10 S502
WP11	20"	11"	1"	6	3	2	7"	8"	3/4"	6"	SEE REBAR WITH DETAIL WP11 S502
WP12	8"	24"	1"	6	3	2	10"	4"	3/4"	6"	SEE REBAR WITH DETAIL WP12 S502

EMBED PLATE SCHEDULE NOTES:

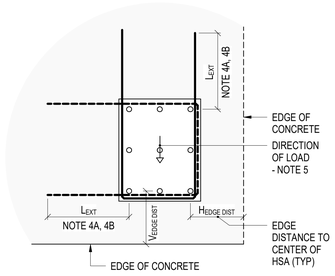
1. THE EMBED PLATES SHOWN IN THIS SCHEDULE SHALL APPLY TO STEEL BEAM END CONNECTIONS TO CONCRETE ELEMENTS, UNLESS NOTED OTHERWISE.
2. TOP OF EMBED PLATE SHALL EQUAL THE TOP OF CONNECTING BEAM, UNLESS NOTED OTHERWISE.
3. ALL EMBED PLATES SHALL BE ASTM A572 GR50.
4. EDGE/CORNER REINFORCEMENT: FOR EMBED PLATES THAT OCCUR NEAR CONCRETE EDGES AND CORNERS, PROVIDE ADDITIONAL REINFORCEMENT AS SPECIFIED WITHIN THIS SCHEDULE.
   A. IF THE SUPPLEMENTAL REINFORCEMENT CANNOT EXTEND STRAIGHT AS SHOWN, TERMINATE AT THE DISCONTINUOUS EDGE IN A STANDARD HOOK.
   B. LOCATE SUPPLEMENTAL REINFORCEMENT AS CLOSE AS PRACTICAL TO PLATE, WHILE MAINTAINING CLEAR COVER REQUIREMENTS.
5. BEAM CONNECTIONS TO EMBED PLATES SHALL BE MADE BY THE STEEL CONNECTIONS DELEGATED ENGINEER. THE CONNECTION SHALL BE MADE WITH THE REACTION RESULTANT CENTERED HORIZONTALLY ON THE PLATE WITH A MAXIMUM OUT-OF-PLANE ECCENTRICITY OF 4".
6. NP = NOT PERMITTED

EMBED PLATE EDGE/CORNER REINFORCEMENT SCHEDULE WHERE WALL PLATE DESIGNATION IS NOT SPECIFIED ON PLAN

BEAM SIZE	ULT. SHEAR BEAM REACTION	CONDITION	$H_{EDGE\ DIST}$		$V_{EDGE\ DIST}$	EDGE/CORNER REINFORCEMENT	L_{EXT}
W8, W10, W12	BETWEEN 0k AND 16k	INTERIOR	AT LEAST 18"	AND	AT LEAST 18"	NOT REQ	24"
		HORIZ EDGE	LESS THAN 18"	AND	AT LEAST 18"	(1)#4 U-BAND HORIZ	
		VERT EDGE	AT LEAST 18"	AND	LESS THAN 18"	(1)#4 U-BAND VERT	
		CORNER	LESS THAN 18"	AND	LESS THAN 18"	(1)#4 U-BAND VERT AND HORIZ	
	BETWEEN 16k AND 26k	INTERIOR	AT LEAST 40"	AND	AT LEAST 40"	NOT REQ	24"
		HORIZ EDGE	LESS THAN 36"	AND	AT LEAST 48"	(2)#4 U-BAND HORIZ	
		VERT EDGE	AT LEAST 48"	AND	LESS THAN 24"	(2)#4 U-BAND VERT	
		CORNER	LESS THAN 40"	AND	LESS THAN 40"	(2)#4 U-BAND VERT AND HORIZ	
	BETWEEN 26k AND 31k	INTERIOR	AT LEAST 60"	AND	AT LEAST 60"	NOT REQ	24"
		HORIZ EDGE	LESS THAN 36"	AND	AT LEAST 90"	(2)#4 U-BAND HORIZ	
		VERT EDGE	AT LEAST 70"	AND	LESS THAN 36"	(2)#4 U-BAND VERT	
		CORNER	LESS THAN 60"	AND	LESS THAN 60"	(2)#4 U-BAND VERT AND HORIZ	
	OVER 31k	NP	NP	NP	NP	NP	
W14, W16, W18	BETWEEN 0k AND 46k	INTERIOR	AT LEAST 66"	AND	AT LEAST 66"	NOT REQ	30"
		HORIZ EDGE	LESS THAN 60"	AND	AT LEAST 100"	(2)#5 U-BAND HORIZ	
		VERT EDGE	AT LEAST 80"	AND	LESS THAN 36"	(2)#5 U-BAND VERT	
		CORNER	LESS THAN 66"	AND	LESS THAN 66"	(2)#5 U-BAND VERT AND HORIZ	
	BETWEEN 46k AND 61k	INTERIOR	AT LEAST 106"	AND	AT LEAST 106"	NOT REQ	36"
		HORIZ EDGE	LESS THAN 60"	AND	AT LEAST 160"	(2)#6 U-BAND HORIZ	
		VERT EDGE	AT LEAST 124"	AND	LESS THAN 72"	(2)#6 U-BAND VERT	
		CORNER	LESS THAN 106"	AND	LESS THAN 106"	(2)#6 U-BAND VERT AND HORIZ	
	OVER 61k	NP	NP	NP	NP	NP	
W21, W24	BETWEEN 0k AND 16k	INTERIOR	AT LEAST 18"	AND	AT LEAST 18"	NOT REQ	24"
		HORIZ EDGE	LESS THAN 18"	AND	AT LEAST 18"	(1)#4 U-BAND HORIZ	
		VERT EDGE	AT LEAST 18"	AND	LESS THAN 18"	(1)#4 U-BAND VERT	
		CORNER	LESS THAN 18"	AND	LESS THAN 18"	(1)#4 U-BAND VERT AND HORIZ	
	BETWEEN 16k AND 31k	INTERIOR	AT LEAST 30"	AND	AT LEAST 30"	NOT REQ	30"
		HORIZ EDGE	LESS THAN 18"	AND	AT LEAST 40"	(2)#5 U-BAND HORIZ	
		VERT EDGE	AT LEAST 36"	AND	LESS THAN 18"	(2)#5 U-BAND VERT	
		CORNER	LESS THAN 30"	AND	LESS THAN 30"	(2)#5 U-BAND VERT AND HORIZ	
	BETWEEN 31k AND 46k	INTERIOR	AT LEAST 55"	AND	AT LEAST 55"	NOT REQ	30"
		HORIZ EDGE	LESS THAN 36"	AND	AT LEAST 75"	(2)#5 U-BAND HORIZ	
		VERT EDGE	AT LEAST 66"	AND	LESS THAN 36"	(2)#5 U-BAND VERT	
		CORNER	LESS THAN 55"	AND	LESS THAN 55"	(2)#5 U-BAND VERT AND HORIZ	
	BETWEEN 46k AND 71k	INTERIOR	AT LEAST 105"	AND	AT LEAST 105"	NOT REQ	36"
		HORIZ EDGE	LESS THAN 36"	AND	AT LEAST 180"	(2)#6 U-BAND HORIZ	
		VERT EDGE	AT LEAST 60"	AND	LESS THAN 130"	(2)#6 U-BAND VERT	
		CORNER	LESS THAN 105"	AND	LESS THAN 105"	(2)#6 U-BAND VERT AND HORIZ	
	BETWEEN 71k AND 86k	INTERIOR	AT LEAST 140"	AND	AT LEAST 140"	NOT REQ	54"
		HORIZ EDGE	LESS THAN 40"	AND	AT LEAST 260"	(2)#7 U-BAND HORIZ	
		VERT EDGE	AT LEAST 165"	AND	LESS THAN 90"	(2)#7 U-BAND VERT	
		CORNER	LESS THAN 140"	AND	LESS THAN 140"	(2)#7 U-BAND VERT AND HORIZ	
	OVER 86k	NP	NP	NP	NP	NP	

EMBED PLATE EDGE/CORNER REINFORCEMENT SCHEDULE WHERE WALL PLATE DESIGNATION IS NOT SPECIFIED ON PLAN

BEAM SIZE	ULT. SHEAR BEAM REACTION	CONDITION	$H_{EDGE\ DIST}$		$V_{EDGE\ DIST}$	EDGE/CORNER REINFORCEMENT	L_{EXT}
W27, W30, W33	BETWEEN 0K AND 26k	INTERIOR	AT LEAST 18"	AND	AT LEAST 18"	NOT REQ	30"
		HORIZ EDGE	LESS THAN 18"	AND	AT LEAST 18"	(1)#5 U-BAND HORIZ	
		VERT EDGE	AT LEAST 18"	AND	LESS THAN 18"	(1)#5 U-BAND VERT	
		CORNER	LESS THAN 18"	AND	LESS THAN 18"	(1)#5 U-BAND VERT AND HORIZ	
	BETWEEN 26k AND 41k	INTERIOR	AT LEAST 36"	AND	AT LEAST 36"	NOT REQ	30"
		HORIZ EDGE	LESS THAN 18"	AND	AT LEAST 56"	(2)#5 U-BAND HORIZ	
		VERT EDGE	AT LEAST 48"	AND	LESS THAN 18"	(2)#5 U-BAND VERT	
		CORNER	LESS THAN 36"	AND	LESS THAN 36"	(2)#5 U-BAND VERT AND HORIZ	
	BETWEEN 41k AND 61k	INTERIOR	AT LEAST 65"	AND	AT LEAST 65"	NOT REQ	36"
		HORIZ EDGE	LESS THAN 30"	AND	AT LEAST 100"	(2)#6 U-BAND HORIZ	
		VERT EDGE	AT LEAST 80"	AND	LESS THAN 30"	(2)#6 U-BAND VERT	
		CORNER	LESS THAN 65"	AND	LESS THAN 65"	(2)#6 U-BAND VERT AND HORIZ	
	BETWEEN 61k AND 81k	INTERIOR	AT LEAST 100"	AND	AT LEAST 100"	NOT REQ	54"
		HORIZ EDGE	LESS THAN 48"	AND	AT LEAST 155"	(2)#7 U-BAND HORIZ	
		VERT EDGE	AT LEAST 120"	AND	LESS THAN 54"	(2)#7 U-BAND VERT	
		CORNER	LESS THAN 100"	AND	LESS THAN 100"	(2)#7 U-BAND VERT AND HORIZ	
	BETWEEN 81k AND 111k	INTERIOR	AT LEAST 150"	AND	AT LEAST 150"	NOT REQ	62"
		HORIZ EDGE	LESS THAN 72"	AND	AT LEAST 240"	(2)#8 U-BAND HORIZ	
		VERT EDGE	AT LEAST 180"	AND	LESS THAN 90"	(2)#8 U-BAND VERT	
		CORNER	LESS THAN 150"	AND	LESS THAN 150"	(2)#8 U-BAND VERT AND HORIZ	
	BETWEEN 111k AND 156k	INTERIOR	AT LEAST 260"	AND	AT LEAST 260"	NOT REQ	70"
		HORIZ EDGE	LESS THAN 115"	AND	AT LEAST 420"	(2)#9 U-BAND HORIZ	
		VERT EDGE	AT LEAST 290"	AND	LESS THAN 165"	(2)#9 U-BAND VERT	
		CORNER	LESS THAN 260"	AND	LESS THAN 260"	(2)#9 U-BAND VERT AND HORIZ	
	OVER 156k	NP	NP	NP	NP	NP	



EDGE/CORNER REINFORCEMENT

CLASSES OF CONCRETE SCHEDULE

CONCRETE USAGE	MIN. 28-DAY COMPRESSIVE STRENGTH (f'c)	ACI EXPOSURE CLASS	MAX. W/CM RATIO	MAX. AGGREGATE SIZE
SHALLOW FOUNDATIONS				
SPREAD FOOTINGS	4000 PSI	F0, S0, W1, C1	0.50	1 1/2"
MAT FOUNDATIONS	4000 PSI	F0, S0, W1, C1	0.50	1 1/2"
WALL FOOTINGS	4000 PSI	F0, S0, W1, C1	0.50	1 1/2"
PLINTHS	4000 PSI	F0, S0, W1, C1	0.50	1"
SLABS-ON-GRADE				
TYPICAL SLABS-ON-GRADE	4000 PSI	F0, S0, W0, C0	0.45	1"
TOPPING SLABS	3000 PSI	F0, S0, W0, C0	0.45	3/4"
ELEVATED FLOORS				
SLABS ON METAL DECK	3500 PSI	F0, S0, W0, C0	0.45	3/4"
WALLS				
PIT / RETAINING WALLS	4000 PSI	F0, S0, W1, C1	0.45	1"
MISCELLANEOUS				
CURBS (NOTE 1)	3000 PSI	F0, S0, W0, C0	0.50	3/4"
FILLER SLABS FOR METAL PAN STAIRS (NOTE 1)	3000 PSI	F0, S0, W0, C0	0.50	3/4"

CLASSES OF CONCRETE SCHEDULE NOTES:

1. FOR THIS APPLICATION, IT WILL BE PERMISSIBLE TO USE #89 STONE (PEAROCK) AS THE LARGEST AGGREGATE SIZE.
2. ALL CONCRETE SHALL BE NORMAL-WEIGHT CONCRETE, UNLESS NOTED OTHERWISE.
3. IT SHALL BE PERMISSIBLE TO PROVIDE 56-DAY COMPRESSIVE STRENGTH MIXES FOR SHALLOW FOUNDATION AND DEEP FOUNDATION ELEMENTS ONLY.

CEMENTITIOUS MATERIAL CONTENT SCHEDULE

MAX. AGGREGATE SIZE	MINIMUM CEMENTITIOUS MATERIAL CONTENT (LB/YD³)
1 1/2"	470
1"	520
3/4"	540
3/8"	610

CEMENTITIOUS MATERIAL CONTENT SCHEDULE:

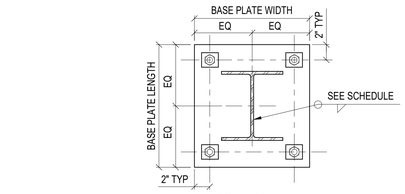
1. UNLESS NOTED OTHERWISE, AT LEAST 35% OF CEMENTITIOUS CONTENT SHALL CONSIST OF A CEMENT REPLACEMENT MATERIAL (SLAG, FLY ASH, OR SILICA FUMES). MAXIMUM AMOUNT OF CEMENT REPLACEMENT MATERIAL SHALL BE 65%.
2. FOR FLOOR FRAMING, WHEN FLY ASH IS USED AS A SUPPLEMENTARY CEMENTITIOUS METERIAL, ITS QUANTITY SHALL NOT BE LESS THAN 15% NOR MORE THAN 25% BY WEIGHT OF TOTAL CEMENTITIOUS MATERIAL, UNLESS OTHERWISE SPECIFIED.

SPREAD FOOTING SCHEDULE

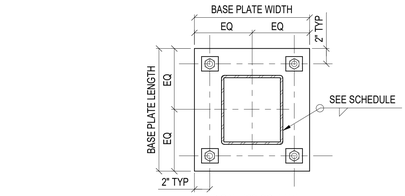
MARK	WIDTH	LENGTH	THICKNESS	REINFORCEMENT		REMARKS
				BOTTOM	TOP	
F8.0	8' - 0"	8' - 0"	24"	(10)#7 EACH WAY	(10)#7 EACH WAY	
F10.0	10' - 0"	10' - 0"	36"	(14)#8 EACH WAY		
F12.0X10.0	10' - 0"	12' - 0"	28"	(14)#8 EACH WAY	(14)#8 EACH WAY	

WALL FOOTING SCHEDULE

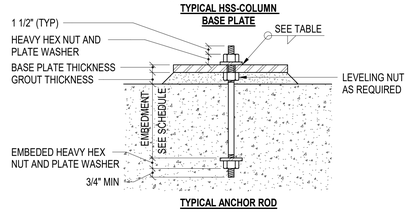
MARK	WIDTH	LENGTH	THICKNESS	REINFORCEMENT		REMARKS
				BOTTOM	TOP	
WF8.0	8' - 0"	CONT	28"	(8)#8 EACH WAY		



TYPICAL W-COLUMN BASE PLATE



TYPICAL HSS-COLUMN BASE PLATE



TYPICAL ANCHOR ROD

STEEL COLUMN AND BASE PLATE SCHEDULE

MARK	SIZE	BASE PLATE DIMENSIONS			FILLET WELD SIZE	ANCHOR RODS	REMARKS
		LENGTH	WIDTH	THICKNESS			
SC-01	HSS10X10X5/8	18"	18"	1 1/4"	5/16"	(4) 3/4"	
SC-02	HSS8X8X1/2	24"	10"	1 1/4"	5/16"	SEE DETAIL BP1/S503	

STEEL COLUMN AND BASE PLATE SCHEDULE NOTES:

1. ANCHOR RODS SHALL BE EMBEDDED A MINIMUM OF 1'-0" INTO THE FOUNDATION, UNLESS NOTED OTHERWISE.

STEEL COLUMN BASE PLATE GROUT THICKNESS SCHEDULE

SMALLER BASE PLATE PLAN DIMENSION	MINIMUM GROUT THICKNESS
24" OR LESS	1 1/2"
25" TO 30"	2"

STEEL COLUMN BASE PLATE WASHER AND HOLE DIMENSIONS SCHEDULE

ANCHOR ROD DIAMETER (NOTE 1)	MAXIMUM HOLE DIAMETER	MINIMUM PLATE WASHER PLAN DIMENSION (NOTE 2)	MINIMUM PLATE WASHER THICKNESS	MINIMUM PLATE WASHER FILLET WELD SIZE
3/4"	1 5/16"	2"	1/4"	3/16"
7/8"	1 9/16"	2 1/2"	5/16"	1/4"
1"	1 13/16"	3"	3/8"	1/4"
1 1/4"	2 1/16"	3"	1/2"	5/16"
1 1/2"	2 5/16"	3 1/2"	1/2"	5/16"
1 3/4"	2 3/4"	4"	5/8"	5/16"
2"	3 1/4"	5"	3/4"	5/16"
2 1/2"	3 1/4"	5 1/2"	7/8"	5/16"

BASE PLATE WASHER AND HOLE DIMENSIONS SCHEDULE NOTES:

1. REFER TO STEEL COLUMN AND BASE PLATE SCHEDULE FOR SPECIFIED ANCHOR ROD DIAMETER.
2. PLATE WASHER PLAN DIMENSION REFERS TO A SIDE OF A SQUARE PLATE OR THE DIAMETER OF A CIRCULAR PLATE.
3. PLATE WASHER HOLE DIAMETER SHALL NOT EXCEED ANCHOR ROD DIAMETER +1/16"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
www.mcsal.com FL CA#26616

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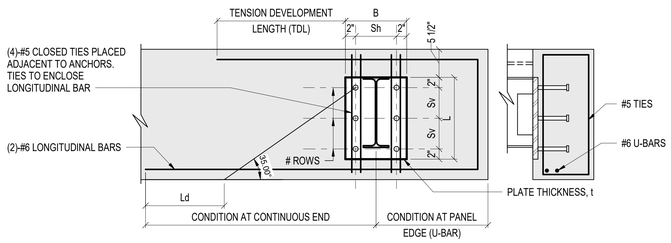
PROJECT NO:
240089.00

SCHEDULES

SHEET NUMBER:
S501



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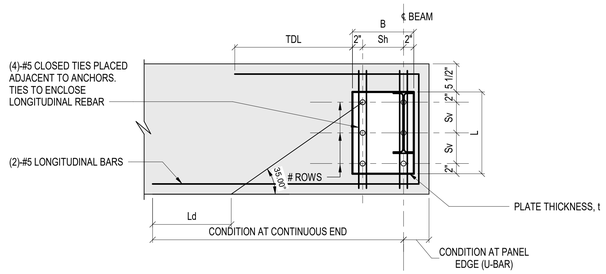

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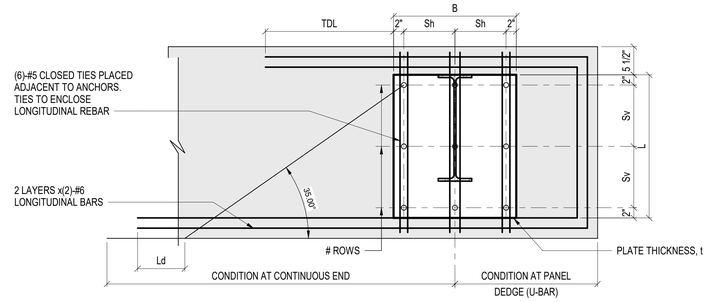

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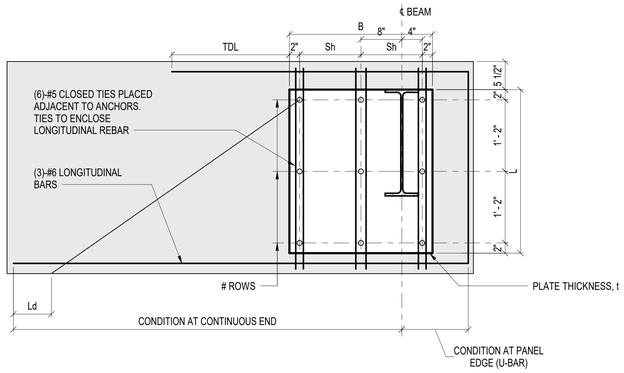

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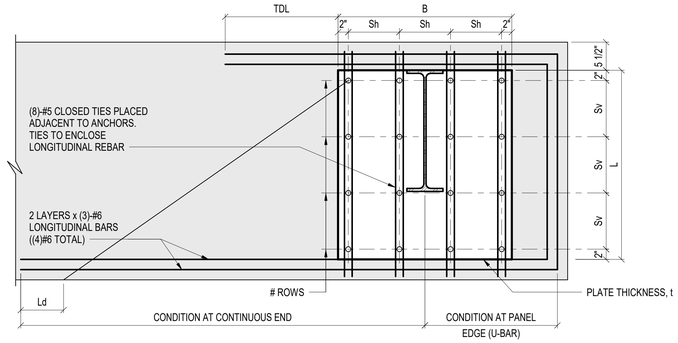

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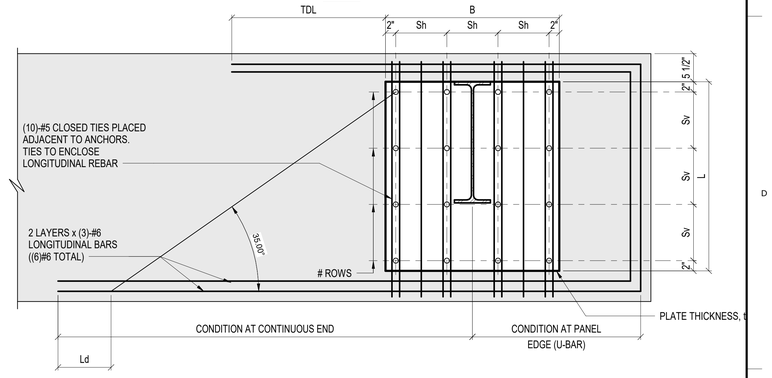

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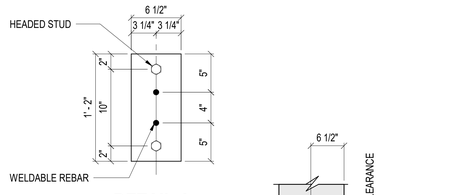
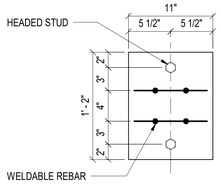


DETAIL A



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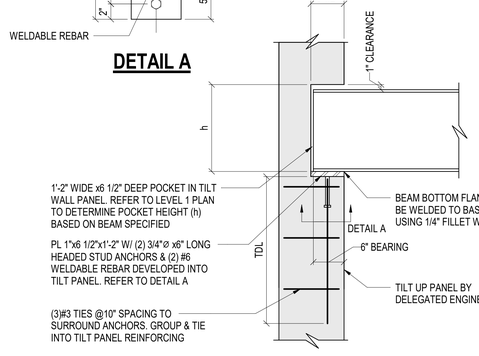

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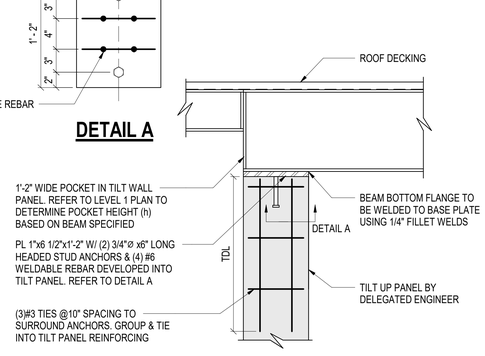

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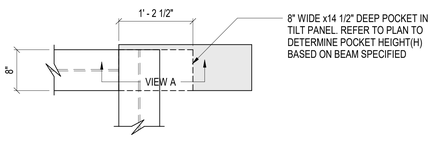

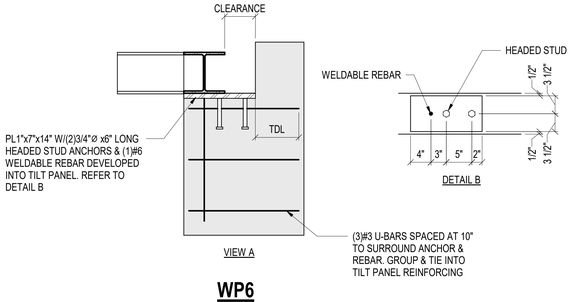

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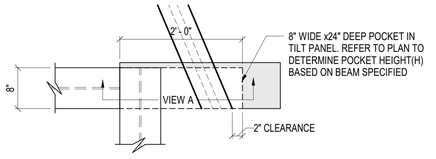

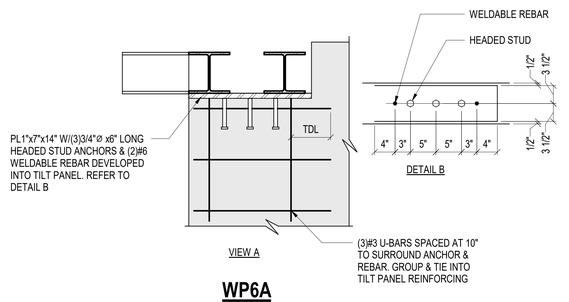

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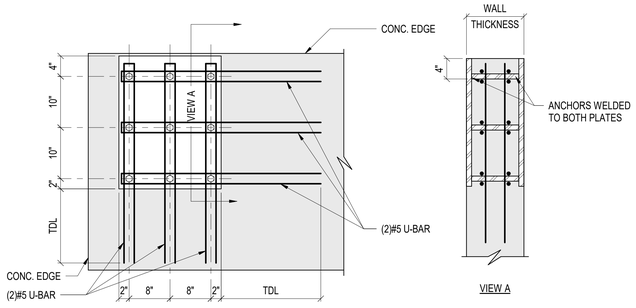
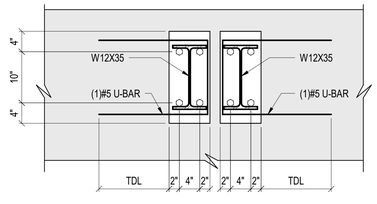
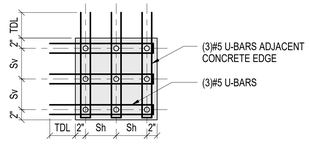


WP7/WP8 ANCHOR AND REINFORCING LAYOUT



WP9

PLATE SETTING OUT TO BE COORDINATED WITH ELEVATOR CONTRACTOR



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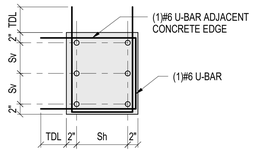

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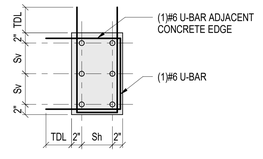

WP12

EMBED PLATE SCHEDULE

1 S502 SCALE: 1" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
www.mcsal.com FL CA#26616

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PROJECT NO:
240089.00

SCHEDULES

SHEET NUMBER:
S502

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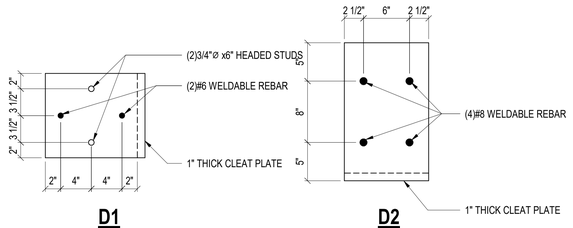

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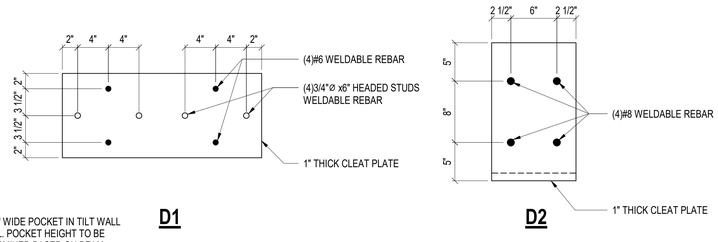

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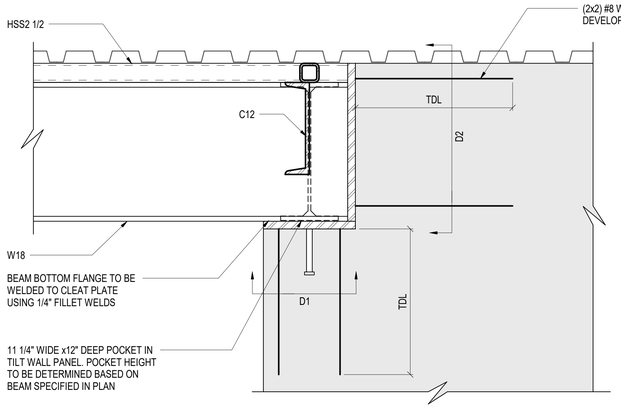

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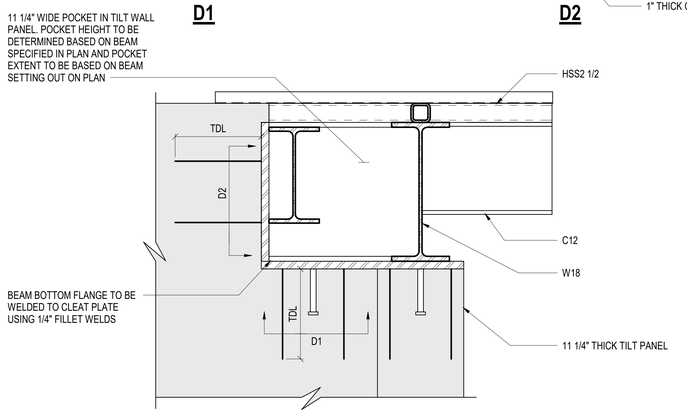

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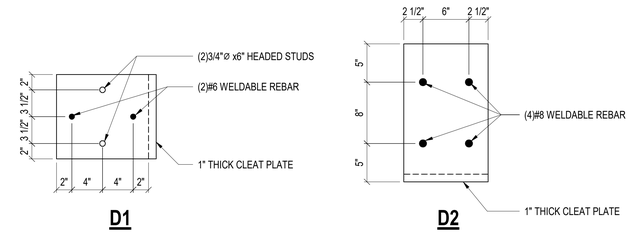

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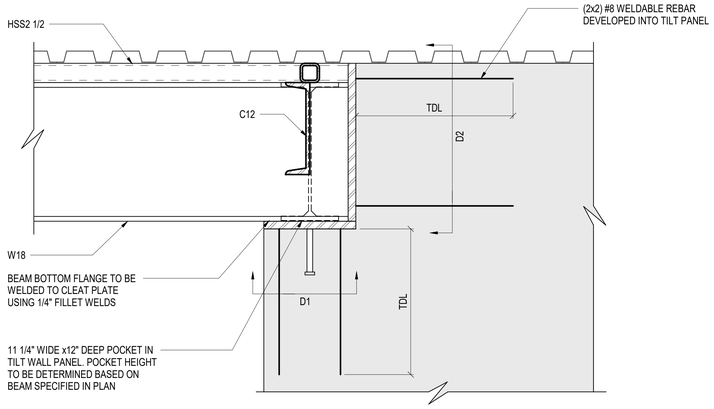
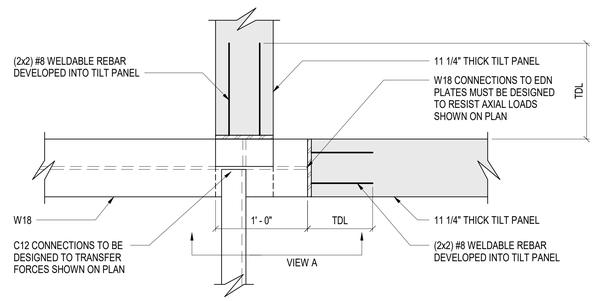
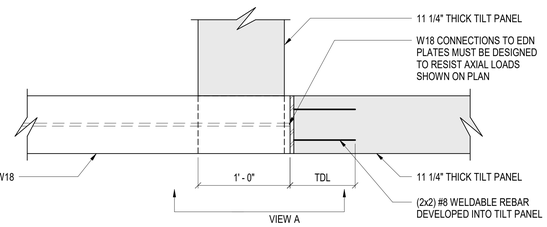


VIEW A

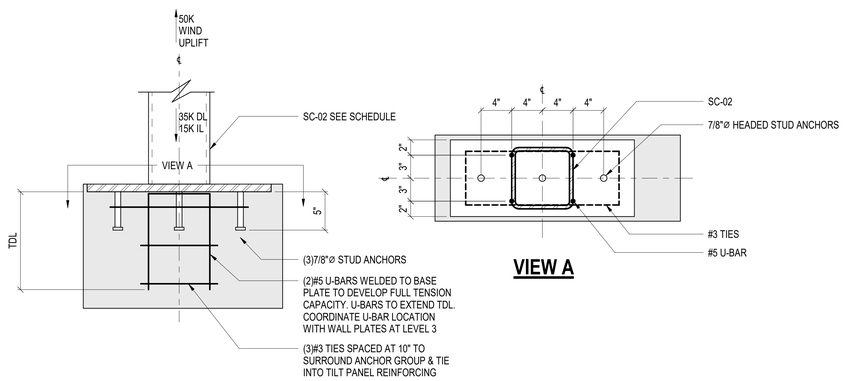
E1 S503 **DETAIL 1**
SCALE: 1 1/2" = 1'-0"



E2 S503 **DETAIL 1**
SCALE: 1 1/2" = 1'-0"



E6 S503 **DETAIL 2**
SCALE: 1 1/2" = 1'-0"



VIEW A

A1 S503 **DETAIL BP1**
SCALE: 1 1/2" = 1'-0"

Baker Barrios

ORLANDO
189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000

INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

McNamara-Salvia
Structural Engineers
201 South Orange Ave.
Suite 730
Orlando, FL 32801 407-955-5992
www.mcsal.com FL CA#26616

Straticon
Received:
06.12.2026

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	06/12/2026	PERMIT SET

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SEACOAST BANK HEADQUARTERS

815 SOUTH COLORADO AVE
STUART, FL 34994

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SHEET NUMBER:
S503