

	1	2	3	4	5	6	7	8	9	10	11	12	
	CODES / STANDARDS		SUBMITTALS		CONCRETE UNIT MASONRY		STRUCTURAL STEEL		STEEL JOISTS				
J	FLORIDA BUILDING 2023, 8TH EDITION AMERICAN SOCIETY OF CIVIL ENGINEERS (ASCE) - MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, 2022 EDITION (ASCE 7)		A. SHOP DRAWING REVIEW IS FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT. CORRECTIONS OR COMMENTS MADE ON THIS REVIEW DO NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR ERRORS AND OMISSIONS, AND FROM COMPLIANCE WITH THE PLANS AND SPECIFICATIONS. CORRECTIONS OR COMMENTS DO NOT AUTHORIZE AN INCREASE IN THE CONSTRUCTION BUDGET. B. APPROVAL OF SHOP DRAWINGS DOES NOT INDICATE ACCEPTANCE OF DEVIATIONS FROM CONTRACT DOCUMENTS OR PREVIOUS SHOP DRAWING REVIEW, UNLESS SPECIFICALLY NOTED THEREIN BY ENGINEER OF RECORD. C. ANY CHANGES TO THE DESIGN CONCEPT SHOWN IN CONTRACT DOCUMENTS SHALL BE SUBMITTED IN WRITING AND APPROVED BY THE ARCHITECT AND ENGINEER PRIOR TO SUBMITTING SHOP DRAWINGS. D. SHOP DRAWINGS SHALL BE "APPROVED", SIGNED AND DATED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL TO ENGINEER AND ARCHITECT OF RECORD. E. SHOP DRAWINGS SHALL NOT CONTAIN REPRODUCTIONS OF THE CONTRACT DRAWINGS. F. DELEGATED ENGINEERING SUBMITTALS SHALL BE SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF THE PROJECT G. SUBMITTAL REQUIREMENTS: 1. INFORMATION SUBMITTALS: CONCRETE PRODUCTS CONCRETE TEST REPORTS MASONRY PRODUCTS INCLUDING PRECAST LINTELS MASONRY GROUT TEST REPORTS 2. ACTION SUBMITTALS: CONCRETE MIX DESIGNS CONCRETE REINFORCING MASONRY GROUT MIX DESIGN MASONRY REINFORCING STRUCTURAL STEEL SHOP DRAWINGS STEEL JOIST SHOP DRAWINGS METAL DECK SHOP DRAWINGS 3. DELEGATED ENGINEERING SUBMITTALS: STEEL JOIST SHOP DRAWINGS PRE-ENGINEERED CANOPIES, AWNINGS AND MARQUEES EXTERIOR LIGHT GAUGE FRAMING		A. STANDARDS 1. THE MASONRY SOCIETY (TMS): TMS 602 2. AMERICAN CONCRETE INSTITUTE (ACI): ACI 308.1 3. AMERICAN SOCIETY OF CIVIL ENGINEERS (ASCE): ASCE 6 B. MASONRY MATERIALS 1. LOAD BEARING MASONRY, ASTM C90, NORMAL WEIGHT, COMPRESSIVE STRENGTH OF MASONRY (fm) = 2,000 PSI 2. MORTAR AND GROUT MATERIALS: PORTLAND CEMENT: ASTM C 150/C 150M, TYPE I OR II; HYDRATED LIME: ASTM C 207, TYPE S; MASONRY CEMENT: ASTM C910/C 910M; MORTAR CEMENT: ASTM C 1329/C 1329M, AGGREGATE FOR MORTAR: ASTM C 144; AGGREGATE FOR GROUT: ASTM C 404; WATER: POTABLE. 3. GROUT COMPRESSIVE STRENGTH SHALL BE 2,500 PSI AT 28 DAYS. ALL GROUTING SHALL BE LOW LIFT. C. STEEL REINFORCEMENT 1. DEFORMED AND PLAIN BARS ASTM A615 GRADE 60 2. LADDER TYPE REINFORCING ASTM A951 D. EXECUTION 1. LAYING MASONRY WALLS. UNLESS OTHERWISE INDICATED, LAY EXPOSED MASONRY IN RUNNING BOND; DO NOT USE UNITS WITH LESS-THAN-NOMINAL 4-INCH (100-MM) HORIZONTAL FACE DIMENSIONS AT CORNERS OR JAMBS. 2. ALL CMU WALLS SHALL HAVE GALVANIZED, 9 GAUGE LADDER TYPE REINFORCEMENT SPACED VERTICALLY AT 16" OC MAXIMUM. PROVIDE CORNER AND "TEE" SECTIONS OF REINFORCEMENT AT ALL WALL INTERSECTIONS. LAP ALL JOINT REINFORCEMENT EIGHT (8) INCHES MINIMUM. 3. ALL CMU WALLS SHALL BE REINFORCED AS SHOWN ON THE DRAWINGS FOR THE FULL HEIGHT OF THE WALL. 4. ALL LAP SPLICES SHALL HAVE A MINIMUM LAP EQUAL TO 48 TIMES THE BAR DIAMETER. 5. POSITION AND HOLD REINFORCING IN PLACE BY THE USE OF PREFABRICATED STEEL WIRE BAR POSITIONERS. CONSOLIDATE GROUT WHEN PLACING BY USING A MECHANICAL VIBRATOR. RECONSOLIDATE BY MECHANICAL VIBRATION AFTER INITIAL WATER LOSS AND SETTLEMENT HAS OCCURRED. 6. LINTELS BEARING ON CMU SHALL HAVE A MINIMUM BEARING LENGTH OF EIGHT (8) INCHES. 7. ALL BOND BEAMS INDICATED ON DRAWINGS SHALL BE GROUTED SOLID. 8. ALL MASONRY BELOW GRADE SHALL BE GROUTED SOLID. 9. LOCATE CONTROL AND EXPANSION JOINTS WHERE INDICATED ON PLANS. IF LOCATIONS ARE NOT INDICATED, LIMIT CONTROL JOINT SPACING TO THE LESSER OF WALL LENGTH TO HEIGHT RATIO OF 1.5 OR 25-FT MAXIMUM SPACING. 10. PROVIDE LINTELS WHERE SHOWN AND WHERE OPENINGS OF MORE THAN 24 INCHES ARE SHOWN WITHOUT STRUCTURAL STEEL OR OTHER SUPPORTING LINTELS. PROVIDE MINIMUM BEARING OF 8 INCHES AT EACH JAMB UNLESS OTHERWISE INDICATED. E. MORTAR BEDDING AND JOINTING 1. LAY HOLLOW CMU AS FOLLOWS: A. BED FACE SHELLS IN MORTAR AND MAKE HEAD JOINTS OF DEPTH EQUAL TO BED JOINTS. B. BED WEBS IN MORTAR IN ALL COURSES OF PIERS, COLUMNS, AND PLASTERES. C. BED WEBS IN MORTAR IN GROUTED MASONRY, INCLUDING STARTING COURSE ON FOOTINGS. D. FULLY BED ENTIRE UNITS, INCLUDING AREAS UNDER CELLS, AT STARTING COURSE ON FOOTINGS WHERE CELLS ARE NOT GROUTED. F. TOLERANCES 1. DIMENSIONS AND LOCATIONS OF ELEMENTS: A. FOR DIMENSIONS IN CROSS SECTION OR ELEVATION, DO NOT VARY BY MORE THAN PLUS 1/2 INCH OR MINUS 1/4 INCH. B. FOR LOCATION OF ELEMENTS IN PLAN, DO NOT VARY FROM THAT INDICATED BY MORE THAN PLUS OR MINUS 1/2 INCH. C. FOR LOCATION OF ELEMENTS IN ELEVATION, DO NOT VARY FROM THAT INDICATED BY MORE THAN PLUS OR MINUS 1/4 INCH IN A STORY HEIGHT OR 1/2 INCH TOTAL. 2. LINES AND LEVELS: A. FOR BED JOINTS AND TOP SURFACES OF BEARING WALLS, DO NOT VARY FROM LEVEL BY MORE THAN 1/4 INCH IN 10 FEET, OR 1/2-INCH MAXIMUM. B. FOR CONSPICUOUS HORIZONTAL LINES, SUCH AS LINTELS, SILLS, PARAPETS, AND REVEALS, DO NOT VARY FROM LEVEL BY MORE THAN 1/8 INCH IN 10 FEET, 1/4 INCH IN 20 FEET, OR 1/2-INCH MAXIMUM. C. FOR VERTICAL LINES AND SURFACES DO NOT VARY FROM PLUMB BY MORE THAN 1/4 INCH IN 10 FEET, 3/8 INCH IN 20 FEET, OR 1/2-INCH MAXIMUM. D. FOR CONSPICUOUS VERTICAL LINES, SUCH AS EXTERNAL CORNERS, DOOR JAMBS, REVEALS, AND EXPANSION AND CONTROL JOINTS, DO NOT VARY FROM PLUMB BY MORE THAN 1/8 INCH IN 10 FEET, 1/4 INCH IN 20 FEET, OR 1/2-INCH MAXIMUM. E. FOR LINES AND SURFACES, DO NOT VARY FROM STRAIGHT BY MORE THAN 1/4 INCH IN 10 FEET, 3/8 INCH IN 20 FEET, OR 1/2-INCH MAXIMUM. F. FOR VERTICAL ALIGNMENT OF EXPOSED HEAD JOINTS, DO NOT VARY FROM PLUMB BY MORE THAN 1/4 INCH IN 10 FEET, OR 1/2-INCH MAXIMUM. G. FOR FACES OF ADJACENT EXPOSED MASONRY UNITS, DO NOT VARY FROM FLUSH ALIGNMENT BY MORE THAN 1/16 INCH. 3. JOINTS: A. FOR BED JOINTS, DO NOT VARY FROM THICKNESS INDICATED BY MORE THAN PLUS OR MINUS 1/8 INCH, WITH A MAXIMUM THICKNESS LIMITED TO 1/2 INCH. B. FOR EXPOSED BED JOINTS, DO NOT VARY FROM BED-JOINT THICKNESS OF ADJACENT COURSES BY MORE THAN 1/8 INCH. C. FOR HEAD AND COLLAR JOINTS, DO NOT VARY FROM THICKNESS INDICATED BY MORE THAN PLUS 3/8 INCH OR MINUS 1/4 INCH. D. FOR EXPOSED HEAD JOINTS, DO NOT VARY FROM THICKNESS INDICATED BY MORE THAN PLUS OR MINUS 1/8 INCH.	A. STANDARDS 1. STEEL JOIST INSTITUTE (SJI) - STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOIST, K-SERIES, AMERICAN NATIONAL STANDARD (SJI-K) 2. SJI - STANDARD SPECIFICATIONS FOR LONGSPAN STEEL JOISTS, LH SERIES AND DEEP LONGSPAN STEEL JOISTS, DLH-SERIES (SJI-LHDLH) 3. SJI - STANDARD SPECIFICATIONS FOR JOIST GIRDEERS (SJI-GJ) B. MATERIALS 1. STRUCTURAL STEEL SHAPES MEETING THE ASTM SPECIFICATIONS PRESCRIBED IN SJI STANDARD SPECIFICATIONS. C. JOIST ACCESSORIES 1. BRIDGING: SCHEMATICALLY INDICATED. DETAIL AND FABRICATE ACCORDING TO SJI'S "SPECIFICATIONS". FURNISH ADDITIONAL ERECTION BRIDGING IF REQUIRED FOR STABILITY. 2. FABRICATE STEEL BEARING PLATES FROM ASTM A 36/A 36M STEEL WITH INTEGRAL ANCHORAGES OF SIZES AND THICKNESSES INDICATED. SHOP PRIME PAINT. 3. HIGH-STRENGTH BOLTS, NUTS, AND WASHERS: ASTM A 325, TYPE 1, HEAVY HEX STEEL STRUCTURAL BOLTS; ASTM A 563 HEAVY HEX CARBON-STEEL NUTS; AND ASTM F 436 HARDENED CARBON-STEEL WASHERS. 4. WELDING ELECTRODES: COMPLY WITH AWS STANDARDS. 5. FURNISH MISCELLANEOUS ACCESSORIES INCLUDING SPLICE PLATES AND BOLTS REQUIRED BY JOIST MANUFACTURER TO COMPLETE JOIST ASSEMBLY. D. QUALITY ASSURANCE 1. MANUFACTURER QUALIFICATIONS: A MANUFACTURER CERTIFIED BY SJI TO MANUFACTURE JOISTS COMPLYING WITH APPLICABLE STANDARD SPECIFICATIONS AND LOAD TABLES IN SJI'S "SPECIFICATIONS". MANUFACTURER'S RESPONSIBILITIES INCLUDE PROVIDING PROFESSIONAL ENGINEERING SERVICES FOR DESIGNING SPECIAL JOISTS TO COMPLY WITH PERFORMANCE REQUIREMENTS. 2. WELDING QUALIFICATIONS: QUALIFY FIELD-WELDING PROCEDURES AND PERSONNEL ACCORDING TO AWS D1.1/D1.1M. "STRUCTURAL WELDING CODE - STEEL". E. EXECUTION 1. EXAMINATION: EXAMINE SUPPORTING SUBSTRATES, EMBEDDED BEARING PLATES, AND ABUTTING STRUCTURAL FRAMING FOR COMPLIANCE WITH REQUIREMENTS FOR INSTALLATION TOLERANCES AND OTHER CONDITIONS AFFECTING PERFORMANCE. PROCEED WITH INSTALLATION ONLY AFTER UNSATISFACTORY CONDITIONS HAVE BEEN CORRECTED. 2. DO NOT INSTALL JOISTS UNTIL SUPPORTING CONSTRUCTION IS IN PLACE AND SECURED. 3. INSTALL JOISTS AND ACCESSORIES PLUMB, SQUARE, AND TRUE TO LINE; SECURELY FASTEN TO SUPPORTING CONSTRUCTION ACCORDING TO SJI'S "SPECIFICATIONS". 4. BOLT JOISTS TO SUPPORTING STEEL FRAMEWORK USING HIGH-STRENGTH STRUCTURAL BOLTS. COMPLY WITH RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS' "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A 325 OR ASTM A 490 BOLTS" FOR HIGH-STRENGTH STRUCTURAL BOLT INSTALLATION AND TIGHTENING REQUIREMENTS. 5. INSTALL AND CONNECT BRIDGING CONCURRENTLY WITH JOIST ERECTION. BEFORE CONSTRUCTION LOADS ARE APPLIED, ANCHOR ENDS OF BRIDGING LINES AT TOP AND BOTTOM CHORDS IF TERMINATING AT WALLS OR BEAMS. F. PROTECTION 1. TOUCHUP PAINTING: AFTER INSTALLATION, PROMPTLY CLEAN, PREPARE, AND PRIME OR REPRIME FIELD CONNECTIONS, RUST SPOTS, AND ABRASD SURFACES OF PRIME-PAINTED JOISTS, BEARING PLATES, AND ACCESSORIES. 2. PROVIDE FINAL PROTECTION AND MAINTAIN CONDITIONS, IN A MANNER ACCEPTABLE TO MANUFACTURER AND INSTALLER, THAT ENSURE THAT JOISTS AND ACCESSORIES ARE WITHOUT DAMAGE OR DETERIORATION AT TIME OF SUBSTANTIAL COMPLETION.							
H	DESIGN CRITERIA OCCUPANCY RISK CATEGORY II DESIGN LOADS DEAD LOADS ROOF MEZZANINE 20 PSF NA LIVE LOADS ROOF FLOOR ON GRADE MEZZANINE 20 PSF 100 PSF NA SNOW LOADS GROUND SNOW LOAD (Pg) SNOW LOAD IMPORTANCE FACTOR (Is) TEMPERATURE FACTOR (Ct) EXPOSURE FACTOR (Ce) FLAT ROOF SNOW LOAD (Pi) 3 PSF 1.0 1.0 1.0 8 PSF WIND LOADS BASIC WIND SPEED (V ULT) BASIC WIND SPEED (V ASD) WIND EXPOSURE INTERNAL PRESSURE COEFFICIENT COMPONENT & CLADDING DESIGN WIND PRESSURES: REFER TO WIND DESIGN SHEET TORNADO SPEED N/A FOR RISK CATEGORY II EARTHQUAKE DESIGN DATA SEISMIC OCCUPANCY CATEGORY SEISMIC IMPORTANCE FACTOR (Is) SITE CLASS SPECTRAL RESPONSE COEFFICIENTS So SEISMIC DESIGN CATEGORY SEISMIC BASIC-FORCE RESISTING SYSTEM = SEISMIC RESPONSE COEFFICIENT (Cs) RESPONSE MODIFICATION FACTOR (R) DESIGN BASE SHEAR ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE RAIN LOAD INTENSITY (60 MIN DURATION/100 YR STORM) 3.8in/Hr GENERAL REQUIREMENTS A. VERIFY EXISTING CONDITIONS AND DIMENSIONS PRIOR TO BEGINNING WORK OR FABRICATING MATERIALS. NOTIFY STRUCTURAL ENGINEER OF RECORD (SER) OF ANY DISCREPANCIES BEFORE PROCEEDING WITH ANY PHASE OF WORK. B. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR ITEMS NOT SHOWN ON THE STRUCTURAL DRAWINGS. C. DO NOT SCALE DRAWINGS FOR THE PURPOSE OF ESTABLISHING DIMENSIONS. D. DETAILS LABELED "TYPICAL DETAILS" ON DRAWINGS APPLY TO SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. SUCH DETAILS APPLY WHETHER OR NOT DETAILS ARE REFERENCED AT EACH LOCATION. NOTIFY SER OF CONFLICT REGARDING APPLICABILITY OF "TYPICAL DETAILS". E. DO NOT LOAD THE SLAB ON GRADE OR SUPPORTED SLAB WITH ERECTION CRANES OR ERECTION EQUIPMENT. THE SLABS HAVE NOT BEEN DESIGNED FOR CRANE LOADS AND WILL REQUIRE AN INCREASE IN THICKNESS AND/OR REINFORCEMENT. F. DO NOT STORE OR CURE CONSTRUCTION MATERIALS ON POURED OR ERECTED FLOORS OR ROOFS IN EXCESS OF 80 PERCENT OF LIVE LOAD. GENERAL CONTRACTOR SHALL ENSURE THAT ALL SUB-CONTRACTORS ARE INFORMED OF LOADING RESTRICTIONS. AVOID IMPACT WHEN PLACING MATERIALS ON POURED OR ERECTED FLOORS OR ROOF. G. BEFORE PROCEEDING WITH ANY WORK WITHIN THE PROJECT AREA, THE CONTRACTOR SHALL FAMILIARIZE THEMSELVES WITH EXISTING STRUCTURE AND OTHER CONDITIONS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE ALL NECESSARY BRACINGS, SHORING AND OTHER SAFEGUARDS TO MAINTAIN ALL PARTS OF EXISTING STRUCTURES AND FACILITIES IN A SAFE CONDITION DURING THE PROCESS OF DEMOLITION AND CONSTRUCTION AND TO PROTECT FROM DAMAGING THOSE PORTIONS OF EXISTING STRUCTURES AND FACILITIES WHICH ARE TO REMAIN. H. THE CONTRACT STRUCTURAL DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION. PROVIDE ALL MEASURES REQUIRED TO PROTECT THE STRUCTURE, WORKMEN AND OTHER PERSONS DURING CONSTRUCTION, INCLUDING BRACING AND SHORING FOR CONSTRUCTION EQUIPMENT, SHORING FOR THE BUILDING, FORMS AND SCAFFOLDING, SHORING OF RETAINING WALLS AND OTHER TEMPORARY SUPPORTS AS REQUIRED. COMPLY WITH APPLICABLE REQUIREMENTS OF OSHA AND OTHER GOVERNING BODIES HAVING JURISDICTION AT THE SITE. I. PRINCIPLE OPENINGS THROUGH THE FRAMING ARE SHOWN ON DRAWINGS. DEVIATIONS FROM THE OPENINGS SHOWN ON THE STRUCTURAL DRAWINGS MUST BE APPROVED BY THE SER PRIOR TO IMPLEMENTING THE CHANGES. J. LOADINGS FOR MECHANICAL EQUIPMENT ARE BASED ON THE UNITS SHOWN ON THE MECHANICAL DRAWINGS. ANY CHANGES IN TYPE, SIZE OR NUMBER OF PIECES OF EQUIPMENT SHALL BE REPORTED TO THE SER FOR VERIFICATION OF THE ADEQUACY OF SUPPORTING MEMBERS PRIOR TO THE PLACEMENT OF SUCH EQUIPMENT. EARTHWORK AND FOUNDATIONS A. EARTHWORK AND FOUNDATION DESIGN IS BASED UPON THE GEOTECHNICAL REPORT: PERFORMED BY EGS FLORIDA, LLC, PROJECT NO. 24-7482, DATED SEPTEMBER 13, 2023 B. ALLOWABLE SOIL BEARING PRESSURE = 2500 PSF C. THE CONTRACTOR SHALL PROVIDE AND OPERATE DEWATERING EQUIPMENT AND BE RESPONSIBLE FOR MAINTAINING EXCAVATIONS AND WORK AREAS IN A DRY CONDITION. D. ARRANGE FOR OWNERS INDEPENDENT TESTING AGENCY TO MONITOR CUT AND FILL OPERATIONS AND PERFORM FIELD DENSITY TESTS PRIOR TO PLACING CONCRETE. E. ALL BACKFILL AND TRENCHING OPERATIONS SHALL COMPLY WITH GEOTECHNICAL ENGINEERING REQUIREMENTS AS WELL AS ALL CURRENT AND APPLICABLE LOCAL, STATE AND FEDERAL SAFETY CODES, INCLUDING THE OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION. F. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING ADEQUATE SHORING OF THE NEW AND EXISTING CONSTRUCTION DURING CONSTRUCTION OPERATIONS IN ORDER TO PREVENT ANY DAMAGE DUE TO BACKFILLING AND TRENCHING. G. ALL BEARING AREAS FOR FOUNDATIONS SHALL BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER OR RECORD PRIOR TO ANY PLACEMENT OF CONCRETE. IF BEARING AREAS ARE NOT SUITABLE, AS DETERMINED BY THE GEOTECHNICAL ENGINEER, THE CONTRACTOR MAY BE REQUESTED TO CARRY THE EXCAVATION DEEPER TO MORE SUITABLE BEARING MATERIAL. H. DO NOT PLACE FOOTINGS OR SLABS AGAINST SUBGRADE CONTAINING FREE WATER, FROST OR ICE. I. PROTECT PIPES AND CONDUITS RUNNING THROUGH WALLS AND SLABS WITH 1/2 INCH EXPANSION MATERIAL. LOWER CONTINUOUS FOOTINGS AND GRADE BEAMS PERPENDICULAR TO PIPE RUNS TO ALLOW PIPES TO PASS ABOVE THE FOOTINGS. ALTERNATIVELY, PROVIDE A CONCRETE JACKET IF PIPES AND CONDUITS ARE LOW ENOUGH TO BE PLACES BELOW THE FOOTINGS. LOWER FOOTINGS PARALLEL TO PIPE RUNS TO AVOID SURCHARGE ONTO ADJACENT TRENCH EXCAVATION.	CAST-IN-PLACE CONCRETE A. STANDARDS 1. AMERICAN CONCRETE INSTITUTE (ACI): ACI 117, ACI 301, ACI 302.1R, ACI 306.1, ACI 308, ACI 318 2. CONCRETE REINFORCING STEEL INSTITUTE (CRSI): MANUAL OF STANDARD PRACTICE 3. WIRE REINFORCEMENT INSTITUTE, INC. (WRI) - DESIGN OF SLAB ON-GROUND FOUNDATIONS B. CONCRETE MATERIALS 1. CONCRETE MIXTURES SHALL BE THE NORMAL WEIGHT TYPE (145 PCF) AND DESIGNED PER ACI USING THE FOLLOWING PARAMETERS: FOUNDATIONS: 28 DAY COMPRESSIVE STRENGTH (fc) CATEGORY F CLASS CATEGORY S CLASS CATEGORY W CLASS CATEGORY C CLASS 3000 PSI F0 S0 W0 C0 BEAMS AND COLUMNS: 28 DAY COMPRESSIVE STRENGTH (fc) CATEGORY F CLASS CATEGORY S CLASS CATEGORY W CLASS CATEGORY C CLASS 3000 PSI F0 S0 W0 C0 ALL BUILDING ELEMENTS, UNLESS NOTED OTHERWISE: 28 DAY COMPRESSIVE STRENGTH (fc) CATEGORY F CLASS CATEGORY S CLASS CATEGORY W CLASS CATEGORY C CLASS 3000 PSI F0 S0 W0 C0 UNIT WEIGHT: NORMAL WEIGHT CONCRETE (145 PCF) AIR CONTENT: EXPOSURE CLASS F0 = 0% ±2% EXPOSURE CLASS F1, F2, F3 = 6% ±1% CEMENTITIOUS MATERIAL: LIMIT FLYASH TO 15% OF TOTAL CEMENT 2. NO CALCIUM CHLORIDE OR ADMIXTURES CONTAINING CHLORIDES SHALL BE USED IN ANY CONCRETE. MIX MATERIALS SHALL COMPLY WITH THE FOLLOWING: PORTLAND CEMENT ASTM C150 FLY ASH ASTM C618 CLASS C OR F SILICA FUME ASTM C1240 SLAG CEMENT ASTM C969 GRADE 100 OR 120 NORMAL WEIGHT AGGREGATE ASTM C33 CLASS 35 WATER ASTM C84 POTABLE AIR-ENTRAINING ASTM C494 TYPE A WATER-REDUCING ASTM C494 TYPE B RETARDING ASTM C494 TYPE D WATER-REDUCING/RETARDING ASTM C1017 TYPE II 3. CHEMICAL ADMIXTURES MUST BE CERTIFIED BY MANUFACTURER TO BE COMPATIBLE WITH OTHER ADMIXTURES AND THAT DO NOT CONTRIBUTE WATER-SOLUBLE CHLORINE IONS EXCEEDING THOSE ALLOWED IN HARDENED CONCRETE. C. CLEAR COVER 1. UNLESS NOTED OTHERWISE MAINTAIN THE FOLLOWING CONCRETE COVER FOR REINFORCEMENT: CONCRETE CAST AGAINST EARTH 3 INCHES CONCRETE EXPOSED TO THE WEATHER: #5 AND SMALLER BARS 1 1/2 INCHES #6 AND LARGER BARS 2 INCHES SLABS AND WALLS NOT EXPOSED TO WEATHER: #11 AND SMALLER BARS 3/4 INCHES BEAMS AND PEDESTALS NOT EXPOSED TO WEATHER: STIRRUPS AND TIES 3/4 INCHES D. STEEL REINFORCEMENT 1. DEFORMED AND PLAIN BARS, ASTM A615 GRADE 60 2. WELDED WIRE REINFORCEMENT (WWR), ASTM A1064, PLAIN 65,000 PSI YIELD STRESS IN FLAT SHEETS 3. ALL REINFORCEMENT SHALL BE SUPPORTED AND HELD IN PLACE BY MANUFACTURED STEEL WIRE BAR SUPPORTS IN ACCORDANCE WITH CRSI. USE OF ANY OTHER MATERIALS WITHOUT WRITTEN AUTHORIZATION BY THE SER IS PROHIBITED. 4. STEEL REINFORCING SHALL BE FABRICATED ACCORDING TO CRSI. E. VAPOR RETARDERS 1. SHEET VAPOR RETARDER: POLYETHYLENE SHEET, ASTM D 4397, NOT LESS THAN 10 MIL THICK. F. EXECUTION 1. PROVIDE CLASS "B" REINFORCEMENT SPLICES FOR CONTINUOUS REINFORCEMENT. REINFORCEMENT SPLICES AND DEVELOPMENT LENGTHS SHALL BE IN ACCORDANCE WITH ACI 318 AND THE TABLES PROVIDED WITH THESE DRAWINGS. WHERE THERE IS A CONFLICT, THE MORE STRINGENT REQUIREMENT SHALL APPLY. 2. SPLICES FOR WELDED WIRE FABRIC SHALL BE TWO (2) INCHES IN ADDITION TO ONE SPACING OF CROSS WIRES. 3. PROVIDE CONTINUOUS HORIZONTAL WALL REINFORCEMENT WITH 90° BENDS AND EXTENSIONS AT CORNERS AND INTERSECTIONS AS SHOWN ON TYPICAL REINFORCEMENT DETAILS. 4. ALL RE-ENTRANT CORNERS FOR SLAB-ON-GRADE CONSTRUCTION SHALL BE REINFORCED WITH TWO (2) #4 BY 3'-0" LONG (MIN) AT 45° FROM THE SLAB EDGES AND AT SLAB MID-DEPTH. 5. UNLESS NOTED OTHERWISE, PROVIDE DOWELS TO MATCH SIZE AND SPACING OF MAIN REINFORCEMENT. 6. DO NOT WELD OR BEND REINFORCEMENT IN THE FIELD UNLESS SPECIFICALLY SHOWN OR APPROVED BY STRUCTURAL ENGINEER. 7. ALL INTERIOR SLABS-ON-GRADE SHALL BE PLACED OVER A 10 MIL (MINIMUM) VAPOR RETARDER. ALL EDGES OF THE VAPOR RETARDER SHALL BE LAPPED A MINIMUM OF SIX (6) INCHES AND TAPED TO PREVENT ANY AND ALL PASSAGE OF MOISTURE. PLACE, PROTECT, AND REPAIR SHEET VAPOR RETARDER ACCORDING TO ASTM E 1643 AND MANUFACTURERS WRITTEN INSTRUCTIONS. 8. COORDINATE PLACEMENT OF CAST-IN-PLACE EMBEDMENTS AND ANCHOR RODS WITH TEMPLATE. SECURELY ATTACH EMBEDMENT ITEMS TO FORMWORK OR REINFORCING. 9. PLACE CONCRETE IN ONE LAYER OR IN HORIZONTAL LAYERS OF SUCH THICKNESS SO THAT NO NEW CONCRETE WILL BE PLACED ON CONCRETE THAT HAS HARDENED ENOUGH TO CAUSE SEAMS OR PLANES OF WEAKNESS (COLD JOINTS). 10. PROVIDE CONSTRUCTION, CONTRACTION AND ISOLATION JOINTS AS INDICATED ON DRAWINGS. HORIZONTAL CONSTRUCTION JOINTS ARE NOT ALLOWED UNLESS SPECIFICALLY NOTED OR APPROVED BY THE SER. PROPOSED JOINT LOCATIONS THAT ARE DIFFERENT OR IN ADDITION TO THE JOINT LOCATIONS INDICATED ON THE DRAWINGS MUST BE REVIEWED AND APPROVED BY THE SER. 11. SURFACE OF CONCRETE CONSTRUCTION JOINTS SHALL BE CLEANED AND LAITANCE REMOVED. IMMEDIATELY BEFORE NEW CONCRETE IS PLACED, ALL CONSTRUCTION JOINTS SHALL BE WETTED AND STANDING WATER REMOVED. 12. UNLESS NOTED OTHERWISE, CHAMFER ALL EXPOSED EDGES OF CONCRETE 3/4 INCH. 13. BEGIN CURING PROCEDURES IMMEDIATELY AFTER COMPLETING PLACEMENT. CONCRETE SHALL BE PROTECTED FROM PREMATURE DRYING, EXCESSIVELY HOT OR COLD TEMPERATURES AND MECHANICAL INJURY.											
G													
F													
E													
D													
C													
B													
A													

Larson Design Group

3000 WESTINGHOUSE DRIVE,
SUITE 400
CRANBURY TOWNSHIP, PA 16066

SUBCONSULTANT

This item has been digitally signed and sealed by Brent C. Parsons on 06/26/2025.
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COMMENTS	RESPONSE TO COMMENTS	DATE	MARK
		2025.06.24	1

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POINCIANA MULTI-TENANT
4425-4443 S POINCIANA BOULEVARD
KISSIMEE, FLORIDA 34758
STRUCTURAL NOTES

Date: April 25, 2025

Project No.: 13427-001

Sheet No.:

S-001

PERMIT SET

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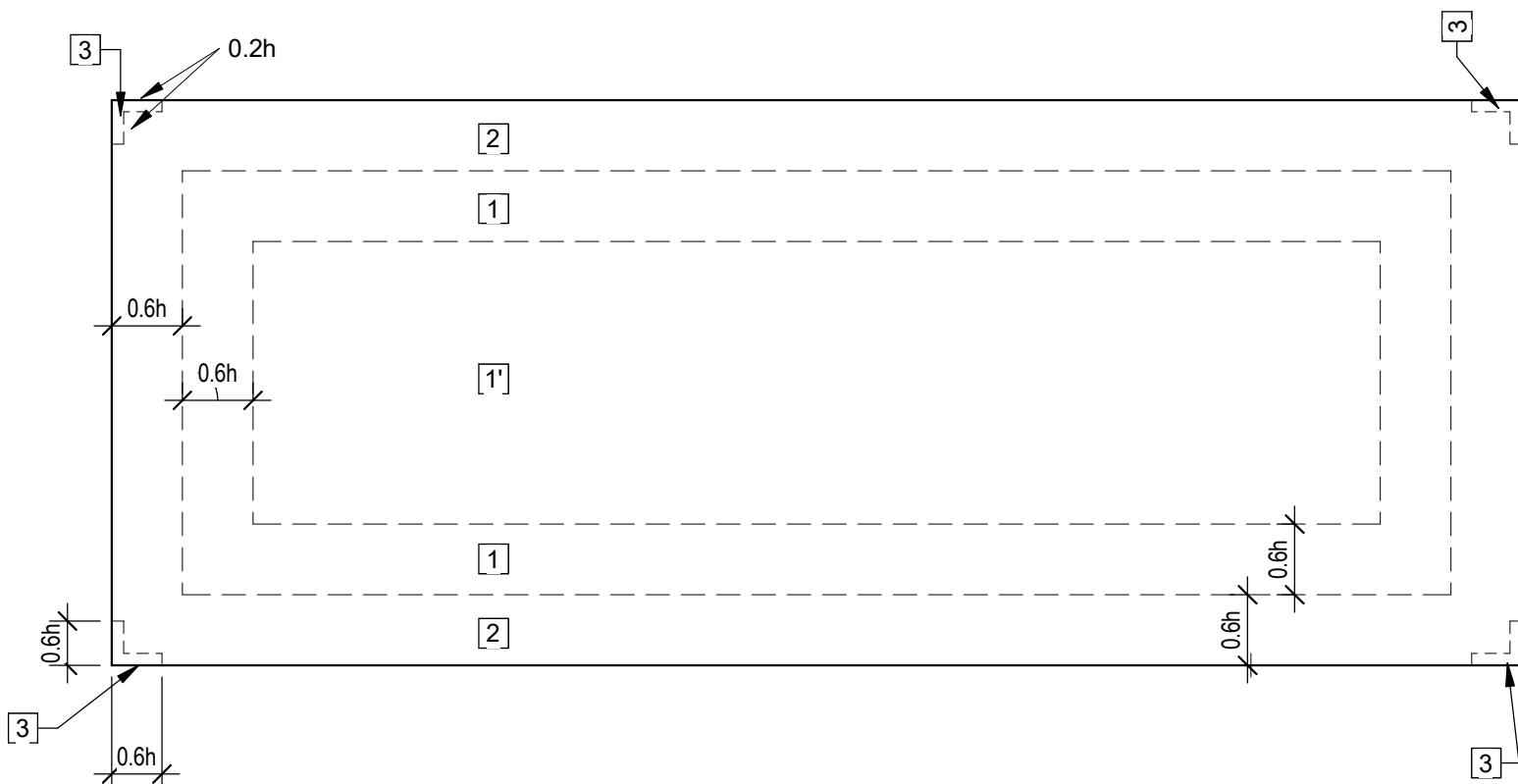
1 2 3 4 5 6 7 8 9 10 11 12

CMU REINFORCING LAP SPLICE SCHEDULE				
BAR SIZE	BAR SPACING	8" CMU	10" CMU	12" CMU
#3	CENTERED	15"	15"	15"
	EDGE	15"	15"	15"
#4	CENTERED	20"	20"	20"
	EDGE	23"	23"	23"
#5	CENTERED	25"	25"	25"
#5	EDGE	35"	35"	35"
#6	CENTERED	38"	30"	30"
	EDGE	NA	54"	54"
#7	CENTERED	52"	40"	35"
#7	EDGE	NA	NA"	63"

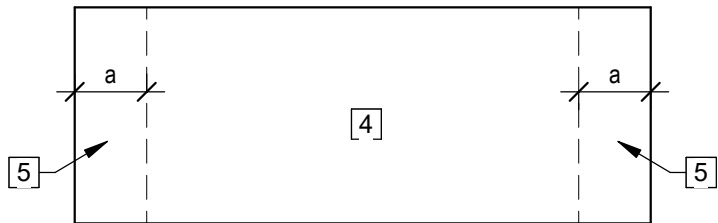
NOTES:
1. MASONRY $f_m = 2000$ PSI
2. STEEL REINFORCING $f_y = 60000$ PSI
3. APPLIES TO BOTH VERTICAL AND HORIZONTAL REINFORCING
4. WHERE HORIZONTAL REINFORCING REQUIRED 1 BAR, USE CENTER SPACING
5. WHERE HORIZONTAL REINFORCING REQUIRES 2 BARS, USE EDGE SPACING

TENSION BAR LAP SPLICE LENGTH FOR CLASS B SPLICES									
BAR SIZE	GRADE 60 BAR BAR SPACING	3000 PSI		4000 PSI		4500 PSI		5000 PSI	
		(3) Bar Dia or More	Less Than (3) Bar Dia	(3) Bar Dia or More	Less Than (3) Bar Dia	(3) Bar Dia or More	Less Than (3) Bar Dia	(3) Bar Dia or More	Less Than (3) Bar Dia
#3	TOP BARS:	25	42	25	37	23	35	22	33
3/8" Ø	OTHER BARS:	22	33	19	28	18	27	17	25
	TOP BARS:	38	56	33	49	31	46	29	44
#4	TOP BARS:	29	43	25	37	24	35	23	34
1/2" Ø	OTHER BARS:	47	70	41	61	38	57	36	54
	TOP BARS:	36	54	31	47	30	44	28	42
#5	TOP BARS:	56	84	49	73	46	69	44	65
5/8" Ø	OTHER BARS:	43	65	37	56	35	53	34	50
	TOP BARS:	81	122	71	106	67	100	63	95
#7	TOP BARS:	63	94	54	81	51	77	49	73
7/8" Ø	OTHER BARS:	93	139	81	121	76	114	72	108
	TOP BARS:	72	107	62	93	59	88	56	83
#9	TOP BARS:	105	157	91	136	86	128	81	122
1.128" Ø	OTHER BARS:	81	121	70	105	66	99	63	94
	TOP BARS:	118	177	102	153	96	144	92	137
#10	TOP BARS:	91	136	79	118	74	111	71	106
1.270" Ø	OTHER BARS:	131	196	114	170	107	160	102	152
	TOP BARS:	101	151	87	131	82	123	78	117
1.410" Ø	OTHER BARS:								

TABLE NOTES:
1. LENGTHS APPLY TO UNCOATED REINFORCEMENT IN NORMAL WEIGHT CONCRETE ONLY.
2. TOP BARS REFERS TO HORIZONTAL REINFORCEMENT SO PLACED THAT MORE THAN 12 INCHES OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE DEVELOPMENT LENGTH OR SPLICE.



Monoslope Roof



Walls

COMPONENTS & CLADDING WIND PRESSURES						
MIN PARAPET HEIGHT	1.0 ft					
ROOF ANGLE	1.2 deg		h = 18.3 ft			
TYPE OF ROOF	MONOSLOPE		a = 6.1 ft			
ULTIMATE LOADS						
			BASIC WIND SPEED = 140.0 mph			
			BASE PRESSURE = 37.8 psf		Qh	
ROOF	SURFACE PRESSURE (PSF)					
	AREA	10 sf	100 sf	500 sf	1000 sf	
	NEGATIVE ZONE 1	-71.0	-55.5	-44.6	-44.6	
	NEGATIVE ZONE 1'	-40.8	-40.8	-27.6	-21.9	
	NEGATIVE ZONE 2	-93.7	-73.7	-59.7	-59.7	
	NEGATIVE ZONE 3	-127.7	-87.7	-59.7	-59.7	
	POSITIVE ALL ZONES	18.1	16.0	16.0	16.0	
	OVERHANG ZONE 1&1'	-64.2	-60.5	-37.8	-37.8	
	OVERHANG ZONE 2	-86.9	-60.2	-41.6	-41.6	
WALLS	OVERHANG ZONE 3	-120.9	-74.2	-41.6	-41.6	
	SURFACE PRESSURE (PSF)					
	AREA	10 sf	100 sf	200 sf	500 sf	
	NEGATIVE ZONE 4	-44.2	-38.2	-36.4	-34.0	
	NEGATIVE ZONE 5	-54.4	-42.4	-38.8	-34.0	
	POSITIVE ZONE 4 AND 5	40.8	34.8	33.0	30.6	
	SURFACE PRESSURE (PSF)					
PARAPETS	Area	10 sf	20 sf	50 sf	100 sf	200 sf 500 sf
	CASE A: ZONE 2 :	122.2	114.3	103.8	95.9	88.0 77.5
	ZONE 3 :	156.6	142.6	124.1	110.1	96.1 77.5
	CASE B : INTERIOR ZONE :	-72.2	-68.5	-63.7	-60.1	-56.4 -51.6
	CORNER ZONE :	-82.5	-77.0	-69.8	-64.3	-58.8 -51.6

DEAD LOAD TO BE USED WITH JOIST UPLIFT (PSF) = 4

NET UPLIFT FOR JOISTS (SERVICE LEVEL PRESSURES USING 0.6DL+0.6WL)					
ROOF			SURFACE PRESSURE (PSF)		
	AREA	10 sf	100 sf	500 sf	1000 sf
	NEGATIVE ZONE 1	-40.2	-30.9	-24.4	-24.4
	NEGATIVE ZONE 1'	-22.1	-22.1	-14.2	-10.7
	NEGATIVE ZONE 2	-53.8	-41.8	-33.4	-33.4
	NEGATIVE ZONE 3	-74.2	-50.2	-33.4	-33.4
	POSITIVE ALL ZONES	13.3	12.0	12.0	12.0
OVERHANG ZONE 1&1'		-36.1	-33.9	-20.3	-20.3
	OVERHANG ZONE 2	-49.7	-33.7	-22.5	-22.5
	OVERHANG ZONE 3	-70.1	-42.1	-22.5	-22.5

DEAD LOAD TO BE USED WITH GIRDER UPLIFT (PSF) = 5

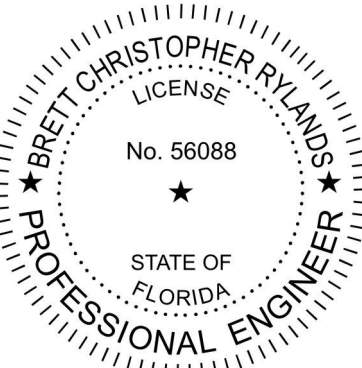
NET UPLIFT FOR GIRDERS (SERVICE LEVEL PRESSURES USING 0.6DL+0.6WL)					
ROOF			SURFACE PRESSURE (PSF)		
	AREA	10 sf	100 sf	500 sf	1000 sf
	NEGATIVE ZONE 1	-39.6	-30.3	-23.8	-23.8
	NEGATIVE ZONE 1'	-21.5	-21.5	-13.6	-10.1
	NEGATIVE ZONE 2	-53.2	-41.2	-32.8	-32.8
	NEGATIVE ZONE 3	-73.6	-49.6	-32.8	-32.8
	POSITIVE ALL ZONES	13.9	12.6	12.6	12.6
OVERHANG ZONE 1&1'		-35.5	-33.3	-19.7	-19.7
	OVERHANG ZONE 2	-49.1	-33.1	-21.9	-21.9
	OVERHANG ZONE 3	-69.5	-41.5	-21.9	-21.9



Larson Design Group

3000 WESTINGHOUSE DRIVE,
SUITE 400
CRANBERRY TOWNSHIP, PA 16006

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COMMENTS

RESPONSE TO COMMENTS

DATE

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POINCIANA MULTI-TENANT
4425 S. POINCIANA BOULEVARD
KISSIMEE, FLORIDA 34758

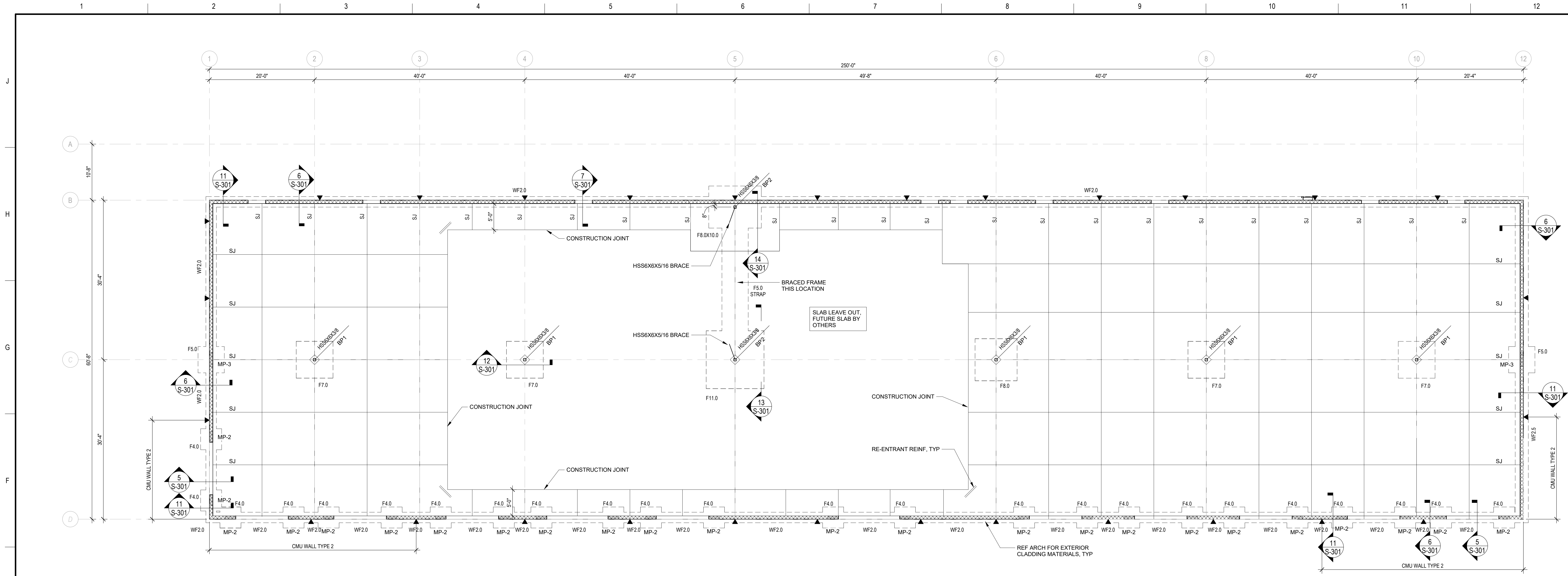
STRUCTURAL NOTES

Date: April 25, 2025
Project No.: 13427-001
Sheet No.:

S-003

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1 GROUND FLOOR PLAN
1/8" = 1'-0"

FOUNDATION SCHEDULE					
MARK	WIDTH	LENGTH	THICKNESS	TOP REINF	BTM REINF
F4.0	4'-0"	4'-0"	1'-0"	(5) #5 OC EW	(5) #5 OC EW
F5.0	5'-0"	5'-0"	1'-0"	(6) #5 OC EW	(6) #5 OC EW
F7.0	7'-0"	7'-0"	1'-4"	(8) #5 OC EW	(8) #5 OC EW
F8.0	8'-0"	8'-0"	1'-6"	(10) #5 OC EW	(10) #5 OC EW
F5.0 STRAP	5'-0"	REF PLANS	1'-8"	#7 @ 12" OC EW	#7 @ 12" OC EW
F8.0X10.0	8'-0"	10'-0"	1'-8"	#7 @ 12" OC EW	#7 @ 12" OC EW
F11.0	11'-0"	11'-0"	1'-8"	#7 @ 12" OC EW	#7 @ 12" OC EW

BRACED FRAME FOUNDATION FOOTINGS TO BE REINFORCED AS A SINGLE COMBINED FOUNDATION. REINFORCING CONTINUOUS IN LONG DIRECTION.

WALL FOUNDATION SCHEDULE				
MARK	WIDTH	THICKNESS	TOP REINF	BOT REINF
WF2.0	2'-0"	1'-0"	NA	(3) #5 CONT, #5 @ 48" TRANS
WF2.5	2'-6"	1'-0"	NA	(3) #5 CONT, #5 @ 12" TRANS

STRUCTURAL COLUMN BASE PLATE SCHEDULE				
MARK	WIDTH	LENGTH	PLATE THICKNESS	ANCHORAGE
BP1	1'-2"	1'-2"	3/4"	(4) 3/4" THREADED HEADED STUDS (ASTM F1554 GR 36), E=FTG DEPTH-4", P=5" MIN
BP2	1'-2"	1'-4"	1 1/4"	(4) 1 1/2" THREADED HEADED STUDS (ASTM F1554 GR 36), E=FTG DEPTH-4", P=6" MIN

MASONRY PILASTER SCHEDULE				
MARK	THICKNESS	WIDTH	VERT REINF	COMMENTS
MP1	7 5/8"	1'-4"	(2) #5 EA CELL	EDGE SPACING, NO HORIZ TIES
MP2	7 5/8"	2'-0"	(2) #5 EA CELL	EDGE SPACING, NO HORIZ TIES
MP3	7 5/8"	2'-8"	(2) #5 EA CELL	EDGE SPACING, NO HORIZ TIES

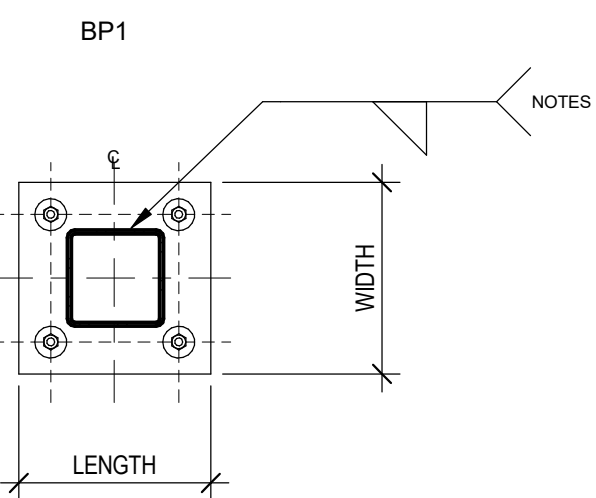
MASONRY WALL TYPE

- 8" CMU W/ (2)#5 VERT BARS @ 32" OC (EDGE SPACING) IN GROUTED CELLS AND ES OF WINDOW AND DOOR OPENINGS, TYP UNLESS OTHERWISE NOTED
- 8" CMU W/ (2)#5 VERT BARS @ 24" OC (EDGE SPACING) IN GROUTED CELLS AND ES OF WINDOW AND DOOR OPENINGS, TYP WHERE PARAPETS 25'-4" AFF

FOUNDATION NOTES

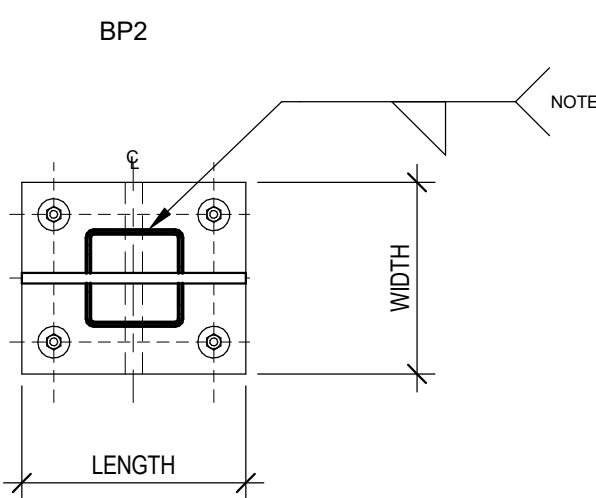
- FOR INFORMATION NOT SHOWN, REFER TO TYPICAL DETAIL SHEETS.
- REFER TO ARCHITECTURAL AND CIVIL DRAWINGS FOR LOCATIONS OF CURBS, EXTERIOR SLABS, DRAINAGE, TRASH ENCLOSURES, RAMPS, WALKS, ETC.
- REFER TO GEOTECHNICAL REPORT FOR ALL SITE AND SUB-GRADE PREPARATION, INCLUDING COMPACTED FILL AND BUILDING SLAB BASE MATERIAL AND VAPOR BARRIER.
- CONTRACTOR SHALL REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR ALL REFERENCED MECHANICAL, ELECTRICAL, AND PLUMBING EQUIPMENT, INCLUDING EXACT PIPING SIZES AND LOCATIONS.
- REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS AND CONSTRUCTION OF NON-LOAD BEARING METAL STUD WALLS, FIXTURES, AND OTHER ITEMS NOT PRESENTED HEREIN.
- FOOTINGS SHALL BE LOCATED ON CENTER LINE OF WALL, PILASTER, OR COLUMNS UNO.
- BUILDING SLAB CONSTRUCTION JOINTS AND CONTROL JOINTS SHALL NOT BE MORE THAN 10'-0". CONTROL JOINTS SHALL BE MADE WITH THE "SOFT-CUT" SAW PROCEDURE WITHIN 4 HOURS AFTER THE CONCRETE POUR.
- BUILDING SLAB ON GRADE SHALL BE 4" CONCRETE SLAB, REINFORCED WITH WWF 6x6-W2.1XW2.1 OVER 10 MIL VAPOR RETARDER ON TERMITES TREATED COMPACTED SOIL.
- AT OPENINGS/RE-ENTRANT CORNERS, PROVIDE (2) #4 x 4'-0" LONG BARS AT MID-HEIGHT OF SLAB.
- INTERIOR FOOTINGS HEREIN ARE RELATIVE TO FINISHED FLOOR ELEVATION OF 0'-0". THE TOP OF ALL INTERIOR FOOTINGS SHALL BE THUS: -2'-0", TYP. UNO.
- EXTERIOR FOOTINGS HEREIN ARE RELATIVE TO FINISHED FLOOR ELEVATION OF 0'-0". THE TOP OF ALL EXTERIOR FOOTINGS SHALL BE THUS: -2'-0", TYP. UNO.

► = MASONRY CONTROL JOINT
SJ = SAW JOINT



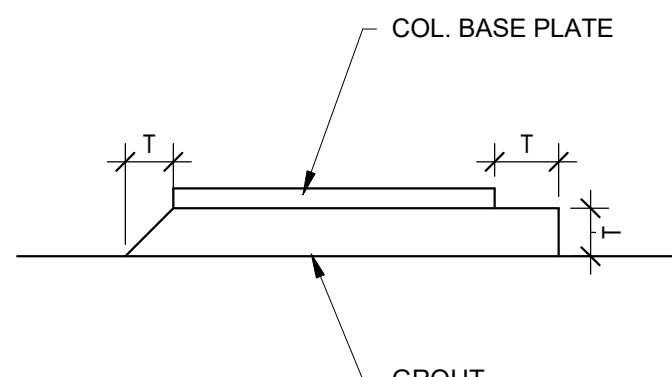
NOTES:

- SEE SCHEDULE FOR PLATE DIMENSIONS AND ANCHOR ROD SIZES
- PROVIDE MINIMUM FILLET WELD SIZE PER AISC SPECIFICATION



NOTES:

- SEE SCHEDULE FOR PLATE DIMENSIONS AND ANCHOR ROD SIZES
- REFER TO DETAIL FOR BRACE CONNECTION, SHEAR KEY, AND WELDING REQUIREMENTS



NOTES:

- GROUT MAY BE BEVELED OR FORMED
- EXTEND MINIMUM T BEYOND EDGE OF BASE PLATE
- T = 1 1/2" MAX. FOR ANCHOR BOLTS UP TO 1" DIA. AND 2" MAX. FOR ANCHOR BOLTS UP TO 1 1/2" DIA.

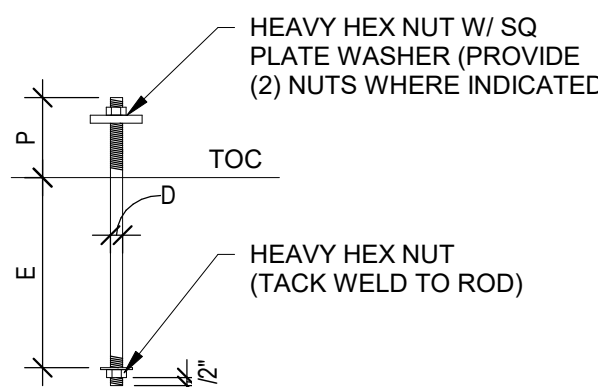
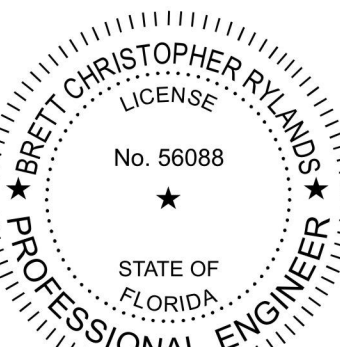


PLATE WASHER SCHEDULE			
ANCHOR BOLT DIAMETER (IN)	BASE PLATE HOLE DIA (IN)	MIN WASHER SIZE (IN)	MIN WASHER THICKNESS (IN)
3/4	1 5/16	2	1/4
1	1 7/8	3	3/8
1 1/4	2 1/8	3 1/2	1/2
1 1/2	2 3/8	4	1/2
1 3/4	2 7/8	4 1/2	5/8
2	3 1/4	5	3/4
2 1/2	3 3/4	5 1/2	7/8

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POINCIANA MULTI-TENANT
4423-4443 S POINCIANA BOULEVARD
KISSIMEE, FLORIDA 34758

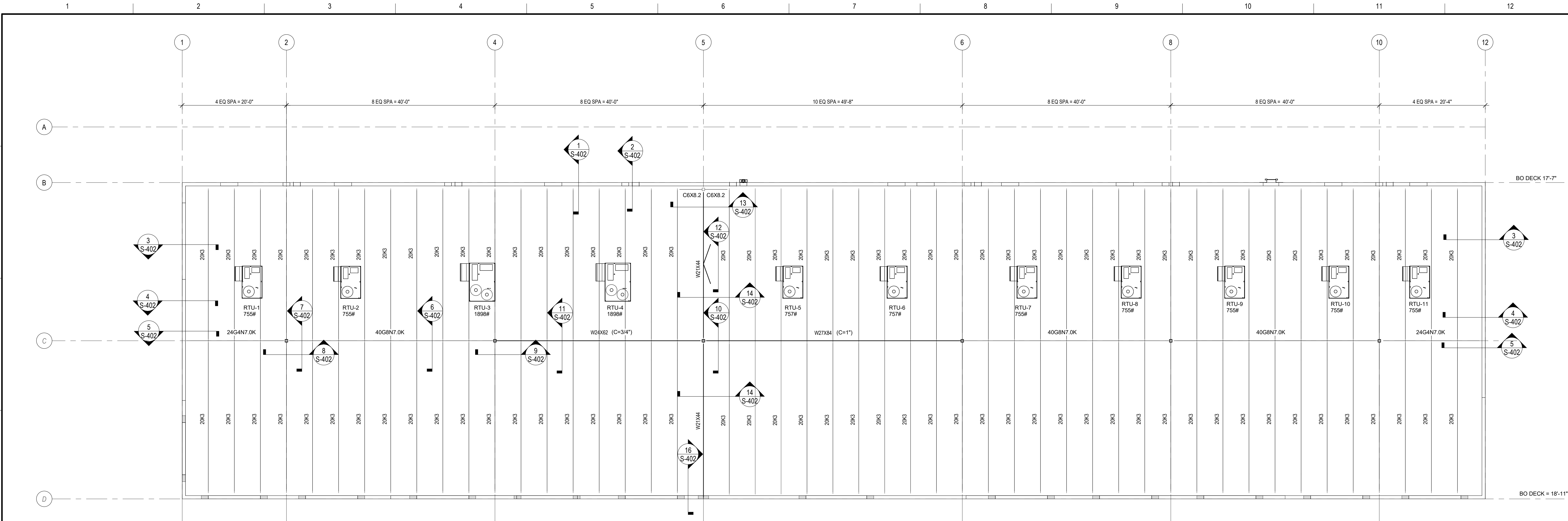
FOUNDATION PLAN

Date: April 25, 2025
Project No.: 13427-001
Sheet No.:

S-101

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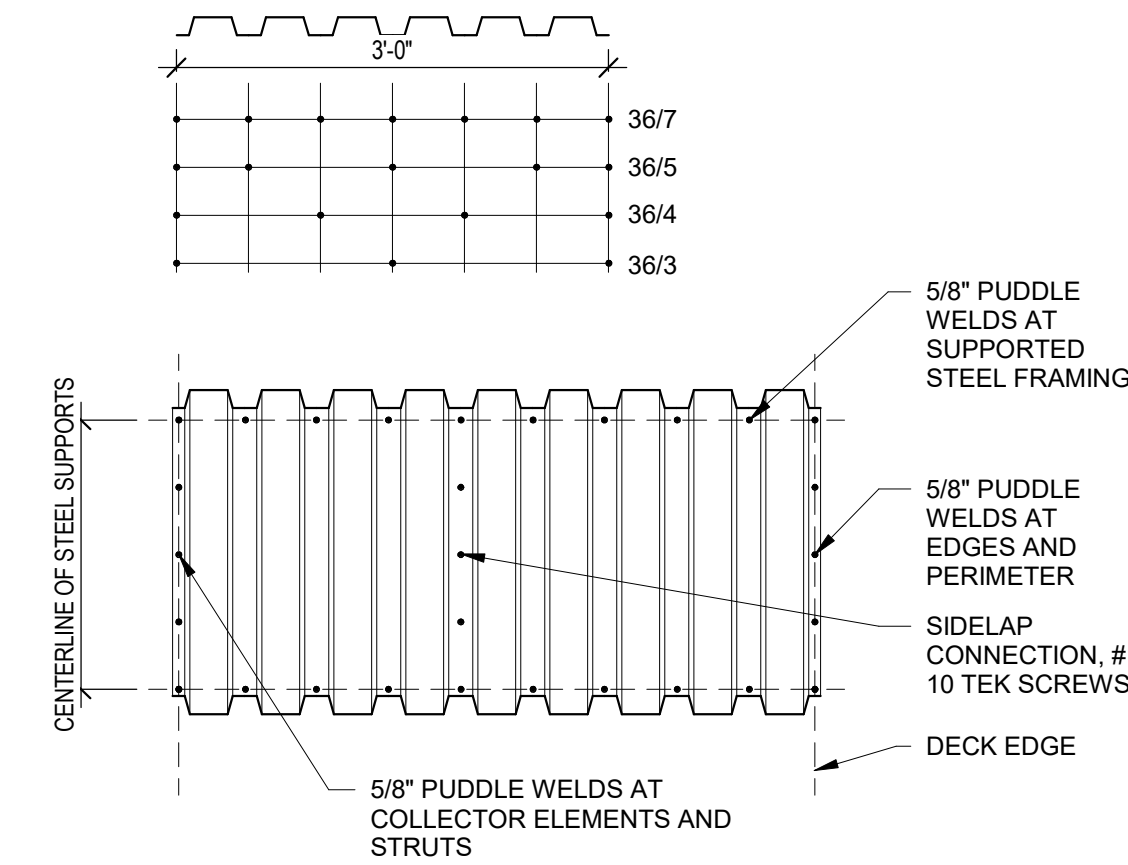
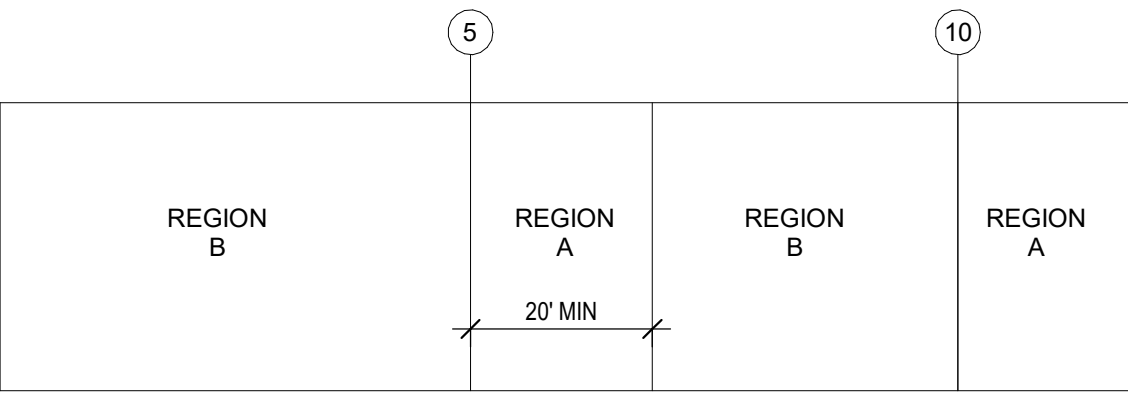


1 STRUCTURAL ROOF PLAN
1/8" = 1'-0"

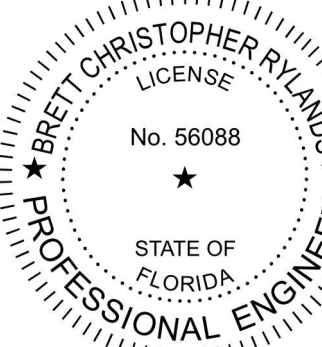
- ROOF FRAMING NOTES**
1. FOR INFORMATION NOT SHOWN, REFER TO TYPICAL DETAIL SHEETS.
 2. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS AND CONSTRUCTION OF NON-LOAD BEARING METAL STUD WALLS, FIXTURES, AND OTHER ITEMS NOT PRESENTED HEREIN.
 3. LOADING FROM ROOF TOP MECHANICAL UNITS SHALL BE TREATED AS AN ADD-LOAD IN ADDITION TO LOADS ALREADY SPECIFIED FOR K SERIES JOISTS.
 4. WHERE KCS JOISTS ARE SPECIFIED, ALL REQUIRED LOADS ARE INCLUDED IN THE JOIST SELECTION.
 5. ROOF TOP UNITS ARE SHOWN ON THIS PLANS ARE ASSUMED. REFER TO NOTE 6.
 6. MECHANICAL ZONE: ALL JOISTS BETWEEN GRID LINE B AND C SHALL BE DESIGNED WITH ADDITIONAL 1K POINT LOAD LOCATED ANYWHERE ON THE JOISTS TO ACCOMMODATE FUTURE RTUS.

DECK FASTENER SCHEDULE			
AREA	DECK	DECK TO STEEL MEMBER CONNECTORS	SIDE LAP CONNECTORS
A	1.58-20 GA	5/8" PUDDLE WELDS 36/7 PATTERN	#10 TEK SCREWS 5 SCREWS PER SPAN
B	1.58-20 GA	5/8" PUDDLE WELDS 36/7 PATTERN	#10 TEK SCREWS 3 SCREWS PER SPAN

PERIMETER, EDGES, COLLECTORS, AND STRUTS = 5/8" PUDDLE WELDS AT 6" OC UNLESS OTHERWISE NOTED



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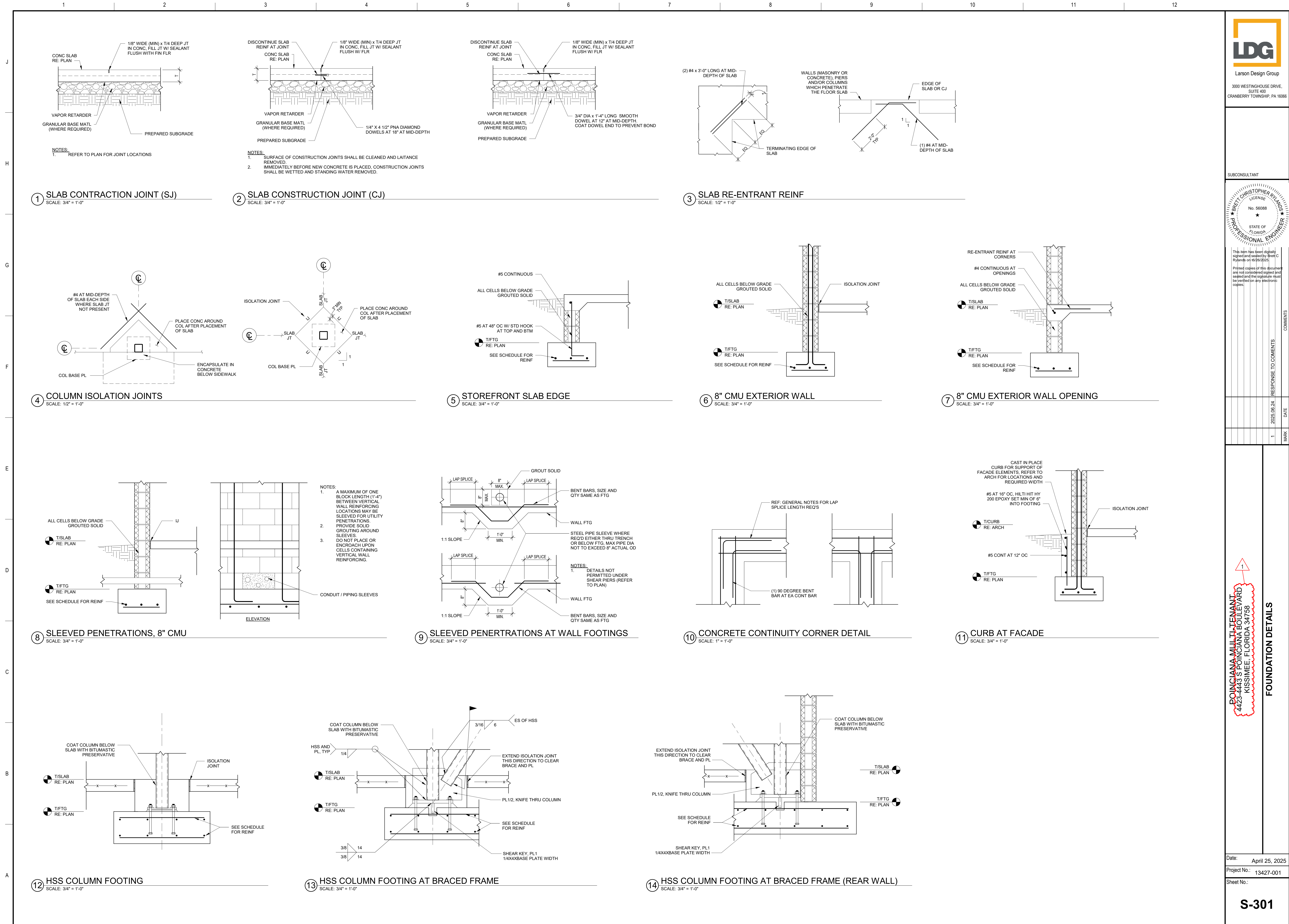
POINCIANA MULTI-TENANT
1423-4443 S POINCIANA BOULEVARD
KISSIMEE, FLORIDA 34758

ROOF PLAN

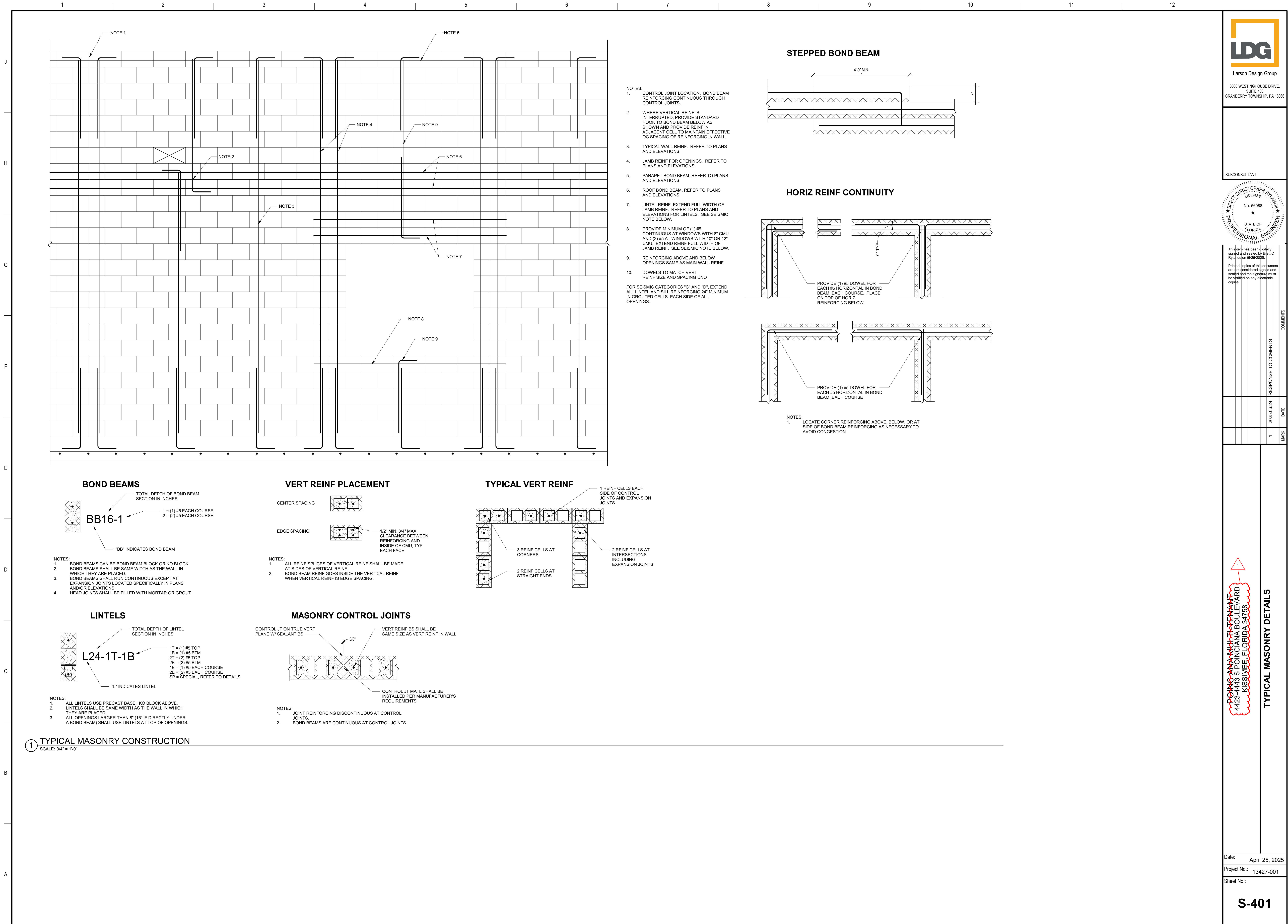
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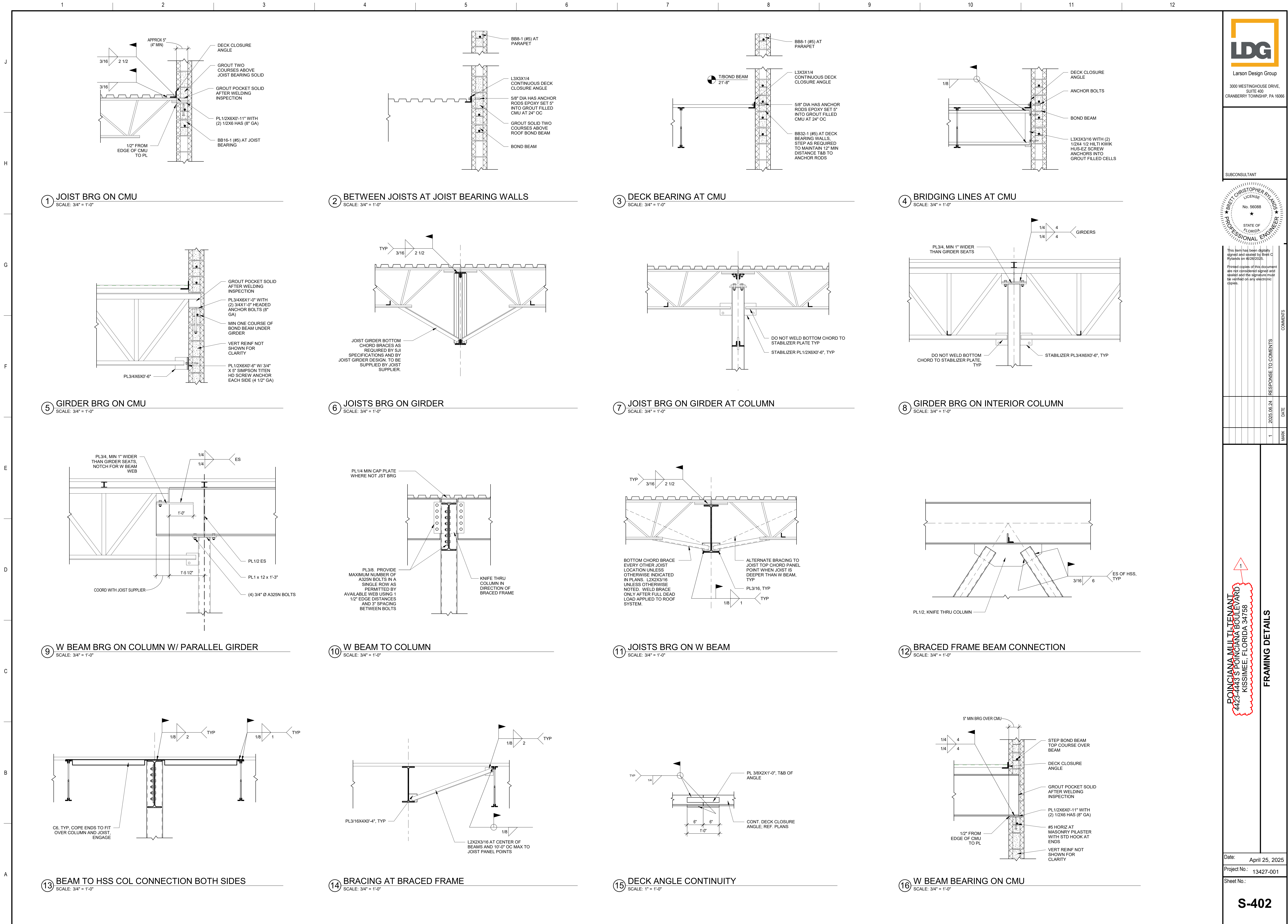
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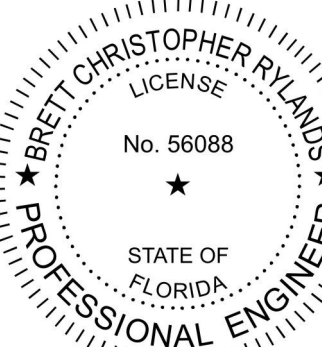
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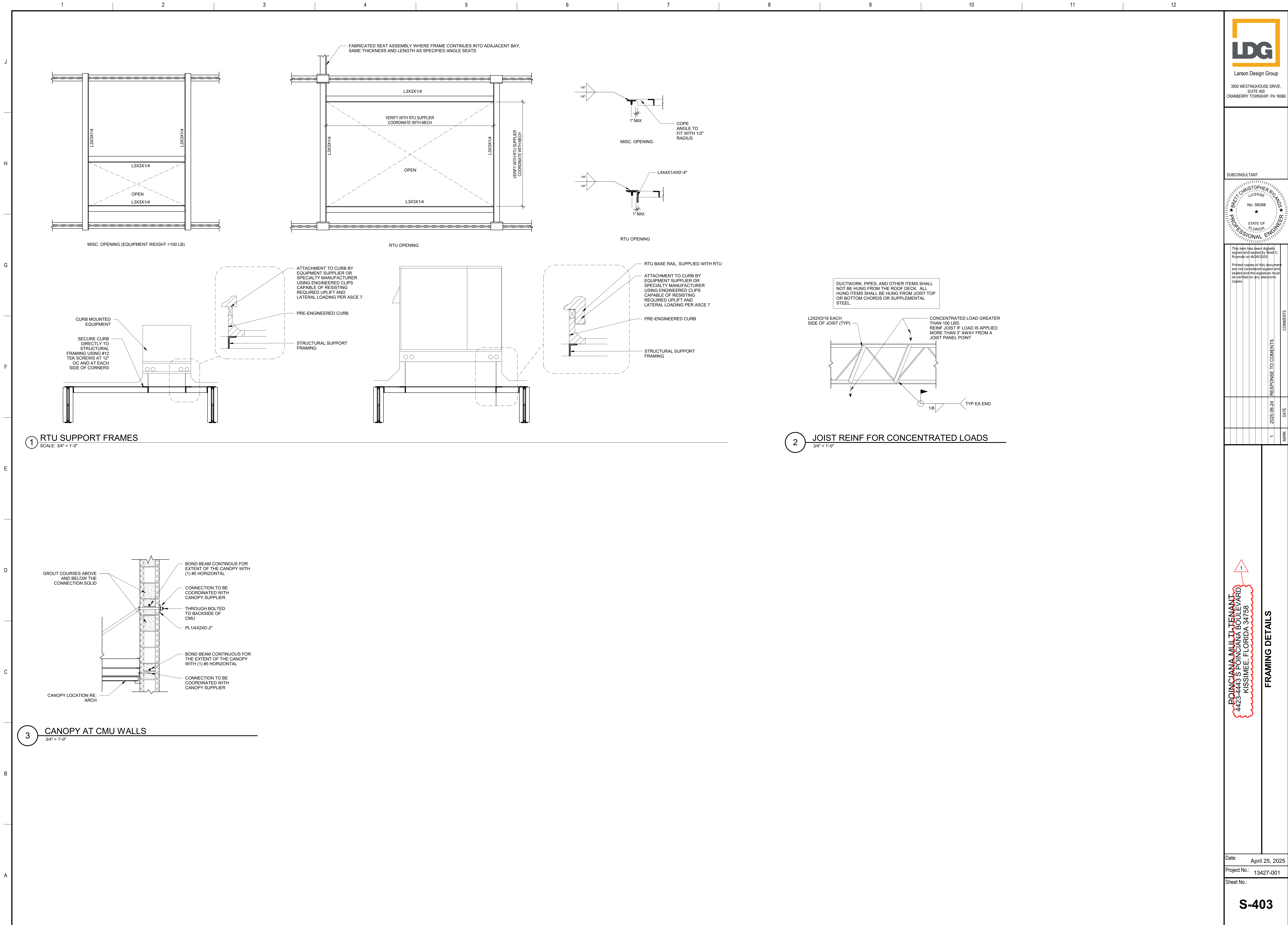
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DATE	2025.06.24
MARK	1

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4423-4443 S POINCIANA BOULEVARD
KISSIMEE, FLORIDA 34758

FRAMING DETAILS

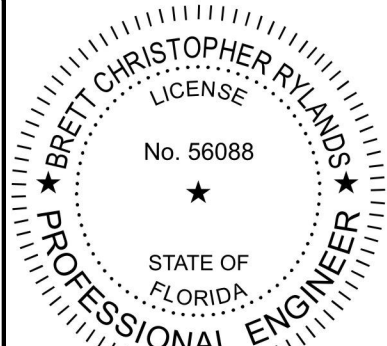
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Larson Design Group
3000 WESTINGHOUSE DRIVE,
SUITE 400
CRANBERRY TOWNSHIP, PA 16066

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STATE OF FLORIDA
PROFESSIONAL ENGINEER
No. 56088

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POINCIANA MULTI-TENANT
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KISSIMEE, FLORIDA 34758

FRAMING DETAILS

Date: April 25, 2025
Project No.: 13427-001
Sheet No.: **S-403**

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