GENERAL STRUCTURAL NOTES

2" 1" 1/2" 0

PART I - DESIGN CRITERIA

A. GENERAL BUILDING CODE

1. THE CONSTRUCTION DOCUMENTS ARE BASED ON THE REQUIREMENTS OF THE FLORIDA BUILDING CODE EIGHTH EDITION (2023).

B. DEAD AND LIVE LOADS

1. REFER TO THE LOAD KEY PLANS ON SHEETS S-021 AND S-022 FOR ALL DEAD AND LIVE LOADING USED ON THE PROJECT.

2. PARTITIONS: AN ALLOWANCE FOR PARTITIONS IS NOT REQUIRED FOR UNIFORM FLOOR LIVE LOADS OF 80 PSF OR GREATER.

3. REDUCTION OF LIVE LOADS:
   a. LIVE LOADS HAVE BEEN REDUCED USING THE STANDARD PROCEDURE FROM THE BUILDING CODE.
   b. FOR LIVE LOADS EXCEEDING 100 POUNDS PER SQUARE FOOT, NO REDUCTION HAS BEEN MADE, EXCEPT THAT THE DESIGN LIVE LOAD ON MEMBERS SUPPORTING TWO OR MORE FLOORS HAS BEEN REDUCED A MAXIMUM OF 20 PERCENT.

C. ELEVATOR LOADS

1. THERE WILL BE NO ELEVATORS FOR THIS PROJECT, BUT ACCOMMODATIONS HAVE BEEN MADE FOR FUTURE ELEVATORS WHEN THE BUILDING IS EXPANDED.

D. WIND LOADS

1. WIND PRESSURES ARE BASED ON THE PROVISIONS OF THE FLORIDA BUILDING CODE WHICH REFER TO THE AMERICAN SOCIETY OF CIVIL ENGINEERS, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, ASCE 7-22 AND THE FOLLOWING CRITERIA:
   a. ULTIMATE DESIGN WIND SPEED (V_{ult}): 165 MPH (3 SECOND GUST)
   b. NOMINAL DESIGN WIND SPEED (V_{ASD}): 128 MPH (3-SECOND GUST)
   c. TORNADO SPEED (V_T): 89 MPH
   d. BUILDING RISK CATEGORY: IV
   e. EFFECTIVE PLAN AREA FOR TORNADO DESIGN (A_e): 250,000 SF
   f. WIND EXPOSURE CATEGORY: C
   g. INTERNAL PRESSURE COEFFICIENTS:
      1) INTERNAL PRESSURE COEFFICIENT (GC_{Pi}): +0.18/-0.18
      2) TORNADO INTERNAL PRESSURE COEFFICIENT (GC_{PiT}): +0.55/-0.18

2. THIS STRUCTURE IS LOCATED IN A WIND-BORNE DEBRIS REGION AND HAS BEEN DESIGNED AS ENCLOSED. IMPACT-RESISTANT EXTERIOR GLAZING OR IMPACT-RESISTANT COVERING OVER EXTERIOR GLAZING IS REQUIRED FOR GLAZING PER SECTION 1609.1.2 OF THE FLORIDA BUILDING CODE. REFER TO THE ARCHITECTURAL DRAWINGS FOR DETAILS FOR ATTACHMENT OF THE IMPACT-RESISTANT EXTERIOR GLAZING OR IMPACT-RESISTANT COVERING OVER EXTERIOR GLAZING TO THE STRUCTURE.

3. WIND PRESSURES TO BE USED IN THE DESIGN OF THE CLADDING AND ROOFTOP EQUIPMENT ARE SHOWN IN 3/S-003.

E. RAIN LOAD DESIGN CRITERIA

1. RAIN LOAD DESIGN CRITERIA IS BASED ON THE PROVISIONS OF THE FLORIDA BUILDING CODE WHICH REFER TO THE AMERICAN SOCIETY OF CIVIL ENGINEERS, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, ASCE 7-22 AND THE FOLLOWING CRITERIA:
   a. DESIGN RAINFALL INTENSITY, i (15 MINUTE): 10.4 IN/HR
   b. DESIGN RAINFALL INTENSITY, i (60 MINUTE): 5.21 IN/HR

F. SEISMIC DESIGN CRITERIA

1. THIS BUILDING IS IN SEISMIC DESIGN CATEGORY A, AND PER SECTION 101.2 EXCEPTION 2 OF THE FLORIDA BUILDING CODE, SEISMIC DESIGN IS NOT REQUIRED.

G. FUTURE EXPANSION

1. THE STRUCTURE HAS BEEN DESIGNED WITH THE CAPACITY TO EXPAND VERTICALLY TO BECOME A FUTURE FIVE-STORY BED TOWER PLUS A MECHANICAL PENTHOUSE. THE NORTHEAST CORNER OF THE BUILDING WILL ALSO EXPAND HORIZONTALLY, AND THERE WILL BE A FUTURE HORIZONTAL EXPANSION ON THE NORTH SIDE OF THE BUILDING SEPARATED BY AN EXPANSION JOINT. REFER TO OVERALL PLANS FOR AREAS WITH PLANNED FUTURE EXPANSION.

H. BUILDING MOVEMENT AND DEFLECTIONS

1. THE BUILDING MOVEMENT SPECIFIED HEREIN IS ANTICIPATED TO OCCUR AND SHOULD BE CONSIDERED BY THE CONTRACTOR IN THE PERFORMANCE OF THE WORK.

2. SPANDREL BEAM DEFLECTIONS. THE FOLLOWING PROVISION SHOULD BE MADE FOR SUPERIMPOSED LOAD DEFLECTION OF ONE FLOOR RELATIVE TO AN ADJACENT FLOOR IN THE DESIGN, FABRICATION, AND ERECTION OF THE BUILDING CLADDING:
   a. TYPICAL SPANDREL BEAM: 3/8"

   SUPERIMPOSED LOAD DEFLECTION IS DEFINED AS ALL DEFLECTION ANTICIPATED TO OCCUR AFTER THE CLADDING HAS BEEN ERECTED. THE RELATIVE DEFLECTION ABOVE DOES NOT INCLUDE THE FLOOR DEFLECTION CAUSED BY THE WEIGHT OF THE CLADDING ITSELF OR THE DEAD LOAD DEFLECTION OF THE FLOOR FRAMING ITSELF, NOR DOES IT INCLUDE ANY ALLOWANCE FOR FABRICATION OR ERECTION TOLERANCES.

3. FLOOR DEFLECTIONS: THE FOLLOWING PROVISION FOR SUPERIMPOSED LOAD DEFLECTIONS SHALL BE MADE IN THE DESIGN, FABRICATION, AND INSTALLATION OF ALL PARTITIONS, GLASS WALLS, AND OTHER ELEMENTS SUPPORTED BY AND ATTACHED TO THE STRUCTURE:
   a. TYPICAL FLOOR MEMBERS: SPAN/360 BUT NOT LESS THAN 3/8"
   b. TYPICAL ROOF MEMBERS: SPAN/360 BUT NOT LESS THAN 3/8"

4. LATERAL FRAME WIND DEFLECTION (DRIFT): THE FOLLOWING PROVISION FOR LATERAL FRAME DEFLECTION IN THE PLANE OF THE WALL OF ONE FLOOR RELATIVE TO AN ADJACENT FLOOR SHALL BE MADE IN THE DESIGN, FABRICATION AND INSTALLATION OF THE BUILDING CLADDING:
   a. TYPICAL FLOOR TO FLOOR: FLOOR HEIGHT/400

   THE DEFLECTIONS LISTED ABOVE ARE BASED ON WIND LOADS WITH A 25-YEAR MEAN RECURRENCE INTERVAL.

I. RESTRAINED CONSTRUCTION CLASSIFICATION

1. IN ACCORDANCE WITH ASTM E 119, ALL FLOOR CONSTRUCTION IS CLASSIFIED AS RESTRAINED CONSTRUCTION.

J. HANDRAILS AND GUARDS

1. THE HANDRAIL ASSEMBLIES AND GUARDS SHALL BE DESIGNED FOR 50 PLF OR A CONCENTRATED LOAD OF 200 POUNDS AT ANY POINT APPLIED IN ANY DIRECTION AT THE TOP AND TO TRANSFER THIS LOAD THROUGH THE SUPPORTS TO THE STRUCTURE. THESE LOADS NEED NOT BE ASSUMED TO ACT CONCURRENTLY.

K. ROOFTOP EQUIPMENT ANCHORAGE

1. ALL ROOF TOP EQUIPMENT CURBS, ROOF TOP MECHANICAL EQUIPMENT, EQUIPMENT TIE DOWNS, AND CONNECTIONS OF ALL EQUIPMENT TO BUILDING STRUCTURE FOR WIND LOADING ARE TO BE DESIGNED AND ENGINEERED BY A REGISTERED SPECIALTY ENGINEER RETAINED BY THE MECHANICAL EQUIPMENT SUPPLIER. SIGNED AND SEALED DRAWINGS AND CALCULATIONS ARE TO BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL. THE EQUIPMENT MANUFACTURER SHALL PROVIDE THE ATTACHMENT OF THE UNIT TO THE STRUCTURE AND SUBMIT TO THE ENGINEER LOADS, LOCATIONS, AND METHODS OF ATTACHMENT. THE STRUCTURAL ENGINEER WILL MAKE PROVISIONS IN THE DESIGN OF THE PRIMARY STRUCTURAL FRAME TO ACCOMMODATE THE LOADS AND ATTACHMENTS SUBMITTED BY THE MANUFACTURER.

PART II - FOUNDATION

A. GEOTECHNICAL REPORT

1. FOUNDATION DESIGN IS BASED ON THE GEOTECHNICAL INVESTIGATION REPORT "DESIGN LEVEL GEOTECHNICAL SERVICES, PROPOSED ORLANDO HEALTH FSED - CONFIDENTIAL, LAKE ANDREW DRIVE, VIERA, BREVARD COUNTY, FLORIDA" BY INTERTEK PSI DATED DECEMBER 2, 2025 (PSI PROJECT NO. 07573442).

2. THE GEOTECHNICAL REPORT IS AVAILABLE TO THE GENERAL CONTRACTOR UPON REQUEST TO THE OWNER. THE INFORMATION INCLUDED THEREIN MAY BE USED BY THE GENERAL CONTRACTOR FOR HIS GENERAL INFORMATION ONLY. THE ARCHITECT AND ENGINEER WILL NOT BE RESPONSIBLE FOR THE ACCURACY OR APPLICABILITY OF SUCH DATA THEREIN.

PART II - FOUNDATION (CONTINUED)

B. DEEP FOUNDATIONS

1. PILE FOUNDATIONS (FOR MAIN BUILDING)
   a. PILE TYPE: DRILLED PRESSURE GROUTED DISPLACEMENT PILES
   a. PILE SIZE: 18 DIAMETER
   b. PILE LENGTH (BELOW EXISTING GRADE): 90 FEET MINIMUM
   c. PILE CAPACITY (ALLOWABLE):
      1) COMPRESSION: 140 TONS
      2) TENSION: 65 TONS
      3) LATERAL: 8 TONS
   d. GROUT VOLUME OVERRUN IN PILES: BASED ON HISTORY OF DEEP FOUNDATIONS, GROUT OVERAGES CAN BE ANTICIPATED TO BE 150% TO 200% OVER THEORETICALLY CALCULATED VOLUME.

C. SHALLOW FOUNDATIONS

1. SPREAD FOOTINGS (FOR ADJACENT LIGHTLY LOADED STRUCTURES)
   a. ALLOWABLE BEARING PRESSURE: 3,000 PSF NET PRESSURE

2. GRADE BEAMS
   a. EXCEPT AS NOTED ON THE DRAWINGS, NON-EXPOSED GRADE BEAM SIDES MAY BE EARTH-FORMED.
   b. GRADE BEAM BOTTOM STEEL SHALL BE CHAIRED AT 5'-0" MAXIMUM CENTERS USING BEAM BOLSTERS PROVIDING 3" BOTTOM COVER TO REINFORCING STEEL. BEAM BOLSTERS USED SHALL BE DESIGNED AND MANUFACTURED FOR SUPPORT ON SOIL.

D. RETAINING WALLS

1. DESIGN CRITERIA
   a. ACTIVE SOIL PRESSURE: 40 PSF PER FOOT OF DEPTH.
   b. AT-REST SOIL PRESSURES: 60 PSF PER FOOT OF DEPTH.
   c. PASSIVE SOIL PRESSURES: 360 PSF PER FOOT OF DEPTH.
   d. SURCHARGE: WALLS HAVE BEEN DESIGNED FOR A SURCHARGE LOAD OF 100 PSF EQUIVALENT TO AN ADDITIONAL LATERAL PRESSURE OF 33 PSF FOR ACTIVE SOIL CONDITIONS AND 50 PSF FOR AT-REST SOIL CONDITIONS.
   e. THE DESIGN PRESSURES SPECIFIED ABOVE ARE BASED ON DRAINED BACKFILL WITH NO BUILDUP OF WATER PRESSURE BEHIND THE WALLS.

E. CONSTRUCTION DEWATERING

1. THE CONTRACTOR SHALL DETERMINE THE EXTENT OF CONSTRUCTION DEWATERING REQUIRED FOR THE EXCAVATION. THE CONTRACTOR SHALL SUBMIT TO THE GEOTECHNICAL ENGINEER FOR REVIEW THE PROPOSED PLAN FOR CONSTRUCTION DEWATERING, PRIOR TO BEGINNING THE EXCAVATION.

PART III - REINFORCED CONCRETE

A. CLASSES OF CONCRETE

1. ALL CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF THE "CLASSES OF CONCRETE MATRIX" IN 1/S-003 UNLESS NOTED OTHERWISE ON THE DRAWINGS.

B. HORIZONTAL CONSTRUCTION JOINTS IN CONCRETE POURS

1. THERE SHALL BE NO HORIZONTAL CONSTRUCTION JOINTS IN ANY CONCRETE POURS UNLESS SHOWN ON THE DRAWINGS. THE ARCHITECT/ENGINEER SHALL APPROVE ALL DEVIATIONS OR ADDITIONAL JOINTS IN WRITING.

C. REINFORCING STEEL

1. ALL REINFORCING STEEL SHALL BE ASTM A 615 GRADE 60 UNLESS NOTED OTHERWISE ON THE DRAWINGS OR IN THESE NOTES.

2. PROVIDE REINFORCING STEEL CONFORMING TO ASTM A 706 GRADE 60 FOR ALL REINFORCING STEEL REQUIRED TO BE WELDED AND WHERE NOTED ON THE DRAWINGS.

3. DEFORMED BAR ANCHORS: 3/8" TO 5/8" DIAMETER AWS D1.1 TYPE C (TABLE 7.1) STUDS MANUFACTURED IN CONFORMANCE WITH SPECIFICATION ASTM A 1064 WITH A MINIMUM YIELD STRENGTH 70,000 PSI. 3/4" OR LARGER DIAMETER, ASTM A706 BARS OF EQUAL SIZE WITH WELDS TO STEEL SUBSTRATE THAT DEVELOP FULL STRENGTH OF ANCHOR. ASTM A 615 REINFORCING BARS SHALL NOT BE SUBSTITUTED FOR DEFORMED BAR ANCHORS.

4. HEADED STUD ANCHORS FOR EMBEDDED PLATES: AWS D1.1 TYPE A STUDS MANUFACTURED IN CONFORMANCE WITH SPECIFICATION ASTM A 29 WITH A MINIMUM TENSILE STRENGTH OF 61,000 PSI.

5. HEADED SHEAR STUDS FOR SHEAR REINFORCEMENT: ASTM A 1044 WITH A MINUMUM YIELD STRENGTH OF 51,000 PSI.

6. SMOOTH WELDED WIRE REINFORCEMENT: ASTM A 1064, YIELD STRENGTH 65,000 PSI.

D. REINFORCING STEEL COVERAGE

1. REINFORCING STEEL COVERAGE SHOULD CONFORM TO THE REQUIREMENTS SPECIFIED IN 2/S-003 UNLESS NOTED OTHERWISE ON THE DRAWINGS. COVER SPECIFIED SHALL BE CONSIDERED MINIMUMS THAT MAY REQUIRE INCREASING WHERE REINFORCING STEEL INTERSECTS FOR DIFFERENT MEMBER TYPES. COVER IN STRUCTURAL MEMBERS NOT SPECIFIED IN THE DETAILS SHALL CONFORM TO THE REQUIREMENTS OF ACI 318 UNLESS SPECIFIED OTHERWISE ON THE DRAWINGS. THE REINFORCING STEEL DETAILER SHALL ADJUST REINFORCING STEEL CAGE SIZES AT INTERSECTING STRUCTURAL MEMBERS AS REQUIRED TO ALLOW CLEARANCE FOR INTERSECTING REINFORCING BAR LAYERS WITH MINIMUM SPECIFIED COVER.

E. HEADED SHEAR STUD REINFORCEMENT COVERAGE

1. CONCRETE COVER FOR THE HEADS AND BASE RAILS OF HEADED SHEAR STUD REINFORCEMENT IN CONCRETE SLABS SHALL CONFORM TO THE REQUIREMENTS SPECIFIED FOR THE REINFORCING STEEL COVER. THE CONCRETE COVER FOR THE HEADS OR BASE RAILS SHALL NOT BE LESS THAN THAT REQUIRED FOR THE REINFORCEMENT IN THE TYPE OF MEMBER IN WHICH THE HEADED SHEAR STUD REINFORCEMENT IS PLACED. COVER IN STRUCTURAL MEMBERS NOT SPECIFIED SHALL CONFORM TO THE REQUIREMENTS OF ACI 318 UNLESS SPECIFIED OTHERWISE ON THE DRAWINGS.

F. WALL AND COLUMN DOWELS

1. DOWELS BETWEEN FOUNDATIONS AND WALLS OR COLUMNS SHALL MATCH SIZE, SPACING, QUANTITY AND GRADE OF THE SPECIFIED VERTICAL REINFORCING, UNLESS NOTED OTHERWISE ON THE DRAWINGS. ALL BARS SHALL HAVE A CLASS B TENSION SPLICE, UNLESS NOTED OTHERWISE.

F. SPLICES AND HOOKS IN REINFORCING STEEL

1. UNSCHEDULED BEAMS AND SLABS, INCLUDING GRADE BEAMS, SHALL HAVE CONTINUOUS TOP BARS LAPPED AT MIDSPAN BETWEEN SUPPORTS WITH A CLASS B TENSION SPLICE. BOTTOM BARS SHALL BE LAPPED AT THE SUPPORTS WITH A CLASS B TENSION SPLICE. ALL BEAM BARS SHALL BE HOOKED AT DISCONTINUOUS END, UNLESS NOTED OTHERWISE.

PART IV - ARCHITECTURAL PRECAST CONCRETE

A. DESIGN RESPONSIBILITY

1. DESIGN: THE DESIGN AND DETAILING OF ALL ARCHITECTURAL PRECAST CONCRETE AND ITS CONNECTION TO THE STRUCTURAL FRAME SHALL BE THE RESPONSIBILITY OF THE PRECAST FABRICATOR.

B. CONNECTIONS

1. TYPE: SUGGESTED GRAVITY AND LATERAL TYPICAL PANEL CONNECTIONS ARE SHOWN ON THE DRAWINGS. THE PRECAST MANUFACTURER MAY CONSIDER OTHER CONNECTION TYPES PROVIDED PRELIMINARY WRITTEN APPROVAL IS OBTAINED FROM THE ENGINEER PRIOR TO THE SUBMISSION OF SHOP DRAWINGS. THE LOCATION OF GRAVITY BEARING CONNECTIONS AND LATERAL CONNECTIONS SHALL BE AS SPECIFIED HEREIN AND IN THE DETAILS ON THE DRAWINGS UNLESS WRITTEN APPROVAL IS OBTAINED FOR ALTERNATE LOCATIONS FROM THE ENGINEER. CONNECTIONS USED SHALL BE ADJUSTABLE TYPE AND SHALL NOT RESULT IN ADDITIONAL FORCES ON THE SUPPORTING STRUCTURAL MEMBERS.

2. SUGGESTED DETAILS ON THE DRAWINGS SHALL IN NO WAY RELIEVE THE PRECASTER FROM THE FULL AND TOTAL RESPONSIBILITY FOR THE PROPER DESIGN AND DETAILING OF ALL PRECAST PANELS AND THEIR CONNECTIONS.

PART V - CONCRETE MASONRY

A. SCOPE

1. REFER TO THE STRUCTURAL SPECIFICATIONS AND DRAWINGS FOR ADDITIONAL INFORMATION AND REQUIREMENTS.

2. REFER TO ARCHITECTURAL DRAWINGS FOR THE EXTENT OF MASONRY WALLS. NON-LOADBEARING WALLS MAY NOT BE SHOWN ON THE STRUCTURAL DRAWINGS.

B. CONCRETE MASONRY ASSEMBLY STRENGTH

1. NET AREA COMPRESSIVE DESIGN STRENGTH OF CONCRETE MASONRY ASSEMBLY (F'M) SHALL BE 2,000 PSI, UNLESS NOTED OTHERWISE.

C. CONCRETE MASONRY UNITS

1. COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNITS (BASED ON NET AREA) SHALL BE 2,000 PSI MINIMUM.

2. UNITS SHALL CONFORM TO ASTM C 55 OR ASTM C 90 AND SAMPLED IN ACCORDANCE WITH ASTM C 140.

D. MORTAR

1. USE ONLY PORTLAND CEMENT/LIME, TYPE M OR S, MORTAR CONFORMING TO ASTM C 270.

E. GROUT

1. MIX DESIGNS:
   a. GROUT SHALL BE PROPORTIONED PER ASCE C 476. MINIMUM COMPRESSIVE STRENGTH OF GROUT SHALL BE EQUAL TO F'M BUT NOT LESS THAN 2,000 PSI.
   b. FOR FILLING SPACES LESS THAN 4" IN ONE OR BOTH HORIZONTAL DIRECTIONS, USE FINE GROUT.
   c. FOR FILLING SPACES BETWEEN 4" AND 10" IN BOTH HORIZONTAL DIRECTIONS, USE COARSE GROUT.
   d. FOR FILLING SPACES 10" AND LARGER IN BOTH HORIZONTAL DIRECTIONS, USE 3,000 PSI NORMALWEIGHT CONCRETE.
   e. FOR ANY GROUT WITH DESIGN STRENGTH GREATER THAN 2,000 PSI, MIX DESIGN SUBMITTALS SHALL INCLUDE THE RESULTS OF THE TESTS PERFORMED IN ACCORDANCE WITH ASTM C 1019.
   f. SLUMP RANGE AT POINT OF FINAL DISCHARGE: 8" TO 11".
   g. THE USE OF AIR-ENTRAINING ADMIXTURES IS NOT PERMITTED.

2. EXTENTS OF GROUTING:
   a. FULLY GROUTED WALLS SHALL BE PROVIDED AT ALL PORTIONS OF WALLS LOCATED ADJACENT TO EARTH FILL. EXTEND FULL GROUTING AT LEAST 8" ABOVE TOP OF SOIL IN CONTACT WITH WALL.
   b. FOR ALL OTHER LOCATIONS, PROVIDE PARTIALLY GROUTED WALLS. GROUT ONLY CELLS WITH VERTICAL OR HORIZONTAL REINFORCEMENT AND ANY OTHER CELLS SHOWN TO BE GROUTED IN DETAILS OR OTHERWISE.

F. REINFORCEMENT FOR CONCRETE MASONRY UNITS

1. ALL REINFORCING STEEL IN CONCRETE MASONRY CONSTRUCTION SHALL BE ASTM A 615 GRADE 60 UNLESS NOTED OTHERWISE ON THE DRAWINGS OR IN THESE NOTES.

2. REFER TO THE DRAWINGS FOR REINFORCEMENT PLACEMENT REQUIREMENTS.

G. CONTROL JOINTS

1. CONTROL JOINTS SHALL BE PROVIDED IN ALL CONCRETE MASONRY CONSTRUCTION AT LOCATIONS INDICATED ON THE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS. SPECIFIC CONTROL JOINT DETAILS ARE AS SHOWN IN THE ARCHITECTURAL DRAWINGS.

2. REFER TO THE DRAWINGS FOR ADDITIONAL MINIMUM REQUIREMENTS AND PLACEMENT GUIDELINES.

H. STABILITY AND BRACING OF MASONRY WALLS DURING CONSTRUCTION

1. ALL MASONRY WALLS AND WALL COMPONENTS SHOWN ON THE ARCHITECTURAL AND STRUCTURAL DRAWINGS HAVE BEEN DESIGNED TO RESIST THE REQUIRED CODE VERTICAL AND LATERAL FORCES APPLIED TO THEM IN THE FINAL CONSTRUCTED CONFIGURATION ONLY ASSUMING FULL BRACING TOP, BOTTOM, AND/OR SIDE OF WALL AS SHOWN. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROPERLY AND ADEQUATELY BRACE ALL MASONRY WALLS AND SHORE ALL MASONRY LINTELS AT ALL STAGES DURING CONSTRUCTION TO RESIST ERECTION LOADS AND LATERAL LOADS THAT COULD POSSIBLY BE APPLIED PRIOR TO COMPLETION OF CONSTRUCTION. WALLS ARE NOT IN THEIR FINAL CONSTRUCTED STATE UNTIL THEY REACH THEIR FULL DESIGN STRENGTH NOTED IN THESE CONSTRUCTION DOCUMENTS.

PART VI - STRUCTURAL STEEL

A. MATERIAL

1. HOT ROLLED STRUCTURAL MEMBERS: ALL HOT ROLLED STEEL PLATES, SHAPES, SHEET PILING, AND BARS SHALL BE NEW STEEL CONFORMING TO ASTM SPECIFICATION A 6.

2. ASTM SPECIFICATION AND GRADE: CLEARLY MARK THE GRADE OF STEEL ON EACH PIECE, WITH A DISTINGUISHING MARK VISIBLE FROM FLOOR SURFACES, FOR THE PURPOSE OF FIELD INSPECTION OF PROPER GRADE OF STEEL. UNLESS NOTED OTHERWISE ON THE DRAWINGS, STRUCTURAL STEEL SHALL BE AS FOLLOWS:
   a. W- AND WT-SHAPES: ASTM A 992.
   b. M- AND S-SHAPES: ASTM A 572, GRADE 50.
   c. C-SHAPES: ASTM A 572, GRADE 50.
   d. L-SHAPES: ASTM A 572, GRADE 50.
   e. ROUND HSS: ASTM A 500, GRADE C (FY=50 KSI).
   f. RECTANGULAR HSS: ASTM A 500, GRADE C (FY=50 KSI).
   g. BASE PLATES: ASTM A 572, GRADE 50.
   h. BENT PLATES: ASTM A 572, GRADE 50.
   i. ALL CONNECTION MATERIAL, EXCEPT AS NOTED OTHERWISE HEREIN OR ON THE DRAWINGS, INCLUDING BEAM COLUMN CONTINUITY PLATES AND DOUBLER PLATES, BEARING PLATES, GUSSET PLATES, STIFFENER PLATES, FILLER PLATES, ETC. SHALL CONFORM TO ASTM A 572, GRADE 50 UNLESS A HIGHER GRADE OF STEEL IS REQUIRED BY STRENGTH AND PROVIDED THE RESULTING SIZES ARE COMPATIBLE WITH THE CONNECTED MEMBERS.
   j. OTHER STEEL: ANY OTHER STEEL NOT INDICATED OTHERWISE SHALL CONFORM TO ASTM A 992 OR ASTM A 572, GRADE 50.

B. CONNECTIONS

1. CONNECTION DETAILS NOT COMPLETELY DETAILED ON THE DRAWINGS INCLUDING MATERIAL GRADE AND SIZES, WELD SIZES, AND NUMBER OF BOLTS SHALL BE DESIGNED BY THE CONTRACTOR PER THE SPECIFICATIONS. CONCEPTUAL CONNECTION DETAILS WITH THE REQUIRED MEMBER DESIGN FORCES ARE SHOWN ON THE DRAWINGS AND ARE APPLICABLE TO ALL CONNECTIONS NOT DESIGNED AND FULLY DETAILED ON THE DRAWINGS. THE CONCEPTUAL DETAILS ARE PROVIDED ONLY TO INDICATE THE CONNECTION TYPE REQUIRED AND MAY NOT FULLY REPRESENT THE COMPLEXITY OF THE CONNECTION AS REQUIRED BY THE FINAL CONNECTION DESIGN FOR THE FORCES THEY MUST RESIST. ADDITIONAL CONNECTION ELEMENTS MAY NOT BE SPECIFICALLY SHOWN IN THE CONCEPTUAL DETAILS BUT MAY BE REQUIRED BY THE FINAL CONNECTION DESIGN, SUCH AS STIFFENER PLATES, DOUBLER PLATES, SUPPLEMENT/ REINFORCING PLATES OR OTHER CONNECTION MATERIAL. THE FABRICATOR IS RESPONSIBLE FOR ENGAGING THE SERVICES OF A CONNECTION SPECIALTY ENGINEER TO PREPARE A FINAL CONNECTION DESIGN FOR SUBMISSION THAT MEETS THE REQUIREMENTS OF THE CONCEPTUAL CONNECTION DETAILS AND RESISTS THE INDICATED DESIGN FORCES.

2. REFER TO THE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

3. REACTIONS NOTED ON THE PLANS ARE BASED ON FACTORED LOADS AND ARE INTENDED FOR USE WITH THE LOAD AND RESISTANCE FACTOR DESIGN METHOD.

C. STRUCTURAL BOLTS AND THREADED FASTENERS

1. A 325 BOLTS: ALL BOLTS IN STRUCTURAL CONNECTIONS SHALL CONFORM TO ASTM A 325 TYPE 1, UNLESS INDICATED OTHERWISE ON THE DRAWINGS.

D. WELDING

1. UNLESS NOTED OTHERWISE, ELECTRODES FOR WELDING SHALL CONFORM TO E70XX (SMAW), F7XX-EXXX (SAW), ER70S-X (GMAW), OR E7XT-X (FCAW).

E. ANCHOR RODS

1. UNLESS INDICATED OTHERWISE IN THE COLUMN SCHEDULE OR ON THE DRAWINGS, ANCHOR RODS SHALL CONFORM TO ASTM F 1554 GRADE 55 (WITH SUPPLEMENTARY REQUIREMENT S1) AND THE SIZE SHALL BE 3/4" DIAMETER AND SHALL EMBED INTO THE CONCRETE FOUNDATION A DISTANCE OF 1'-0" WITH A HEAVY HEX NUT AT THE EMBEDDED END. STRIKE BOLT THREADS AT THE EMBEDDED END AT TWO PLACES BELOW THE NUT.

F. GROUT

1. GROUT BELOW STRUCTURAL STEEL BASE PLATES SHALL BE NON-METALLIC, NON-SHRINK GROUT WITH A MINIMUM STRENGTH OF 8,000 PSI.

PART VII - STEEL DECKS

A. ROOF DECK

1. TYPICAL ROOF DECK SHALL BE 1 1/2" DEEP 20 GAGE WIDE RIB, TYPE B GALVANIZED (G90), UNLESS NOTED OTHERWISE ON THE DRAWINGS.

2. ATTACHMENT OF DECK:
   a. MINIMUM ATTACHMENT AT SUPPORTS: STEEL DECK UNITS SHALL BE FASTENED TO ALL SUPPORT MEMBERS AT EACH RIB (36/7) WITH 3/4" DIAMETER PUDDLE WELDS, UNLESS NOTED OTHERWISE. AT MEMBERS PARALLEL TO DECK SPAN, SPACING OF FASTENERS SHALL BE 6".
   b. MINIMUM ATTACHMENT AT SIDE LAPS: SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BY WELDING, SHEET METAL SCREWS, OR BUTTON PUNCHING AT A MAXIMUM OF 12", UNLESS NOTED OTHERWISE.

3. FLAT, RIDGE, AND VALLEY PLATES:

   UNLESS NOTED OTHERWISE, CONTRACTOR SHALL PROVIDE FLAT PLATES (20 GAGE MINIMUM) AT ALL LOCATIONS WHERE ROOF DECK CHANGES DIRECTION AND RIDGE OR VALLEY PLATES (20 GAGE MINIMUM) AT ALL LOCATIONS WHERE ROOF SLOPE EXCEEDS 1/2" PER FOOT.

PART VIII - SPECIAL INSPECTIONS

A. THRESHOLD BUILDINGS

1. THIS PROJECT IS CLASSIFIED AS A "THRESHOLD BUILDING". SPECIAL INSPECTION OF THE CONSTRUCTION OF "THRESHOLD BUILDINGS" IS REQUIRED BY THE STATE OF FLORIDA IN ACCORDANCE WITH CHAPTERS 471 AND 553 OF THE FLORIDA STATUTES. THE OWNER SHALL PAY FOR THIS INSPECTION. REFER TO THE THRESHOLD INSPECTION PLAN FOR ADDITIONAL REQUIREMENTS.

PART IX - SUBMITTALS

A. SUBMITTAL LIST AND SCHEDULE

1. THE GENERAL CONTRACTOR SHALL PREPARE A DETAILED LIST AND SCHEDULE OF ALL SUBMITTAL ITEMS TO BE SENT TO THE STRUCTURAL ENGINEER PRIOR TO THE START OF CONSTRUCTION. THIS LIST SHALL BE UPDATED AND REVISED AND KEPT CURRENT AS THE JOB PROGRESSES. THE SUBMITTAL LIST SHALL BE ORGANIZED AS SHOWN BELOW:
   a. SHOP DRAWINGS
   b. DESIGN CALCULATIONS
   c. PRODUCT DATA, CERTIFICATES, REPORTS, AND OTHER LITERATURE

B. SUBMITTALS TO BE PROVIDED TO STRUCTURAL ENGINEER

1. STRUCTURAL SUBMITTALS: IN ADDITION TO THE SUBMITTALS REQUIRED BY THE STRUCTURAL SPECIFICATIONS, THE FOLLOWING SUBMITTALS SHALL BE PROVIDED:
   a. LAYOUT OF EMBEDDED ITEMS (PLATES, ANGLES, BOLTS, ETC.) OR ITEMS ATTACHED TO THE STRUCTURAL FRAME FOR BUILDING CLADDING ATTACHMENT OR FOR ATTACHMENT OF OTHER ITEMS.
   b. LAYOUT OF MECHANICAL, ELECTRICAL, AND PLUMBING OPENINGS IN CONCRETE SLABS.
   c. LAYOUT OF PENETRATIONS IN CONCRETE BEAMS.

2. DELGATED SUBMITTALS:
   a. THE FOLLOWING ITEMS ARE CONSIDERED DELEGATED SUBMITTALS BY THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE:
      1) COLD-FORMED METAL FRAMING (REC)
      2) EXTERIOR WINDOW WALL SYSTEM (S&S, REC)
      3) FORMWORK, SHORING, AND BACKSHORING (S&S, REC)
      4) PILE LOAD TEST FRAME (S&S, REC)
      5) PRECAST ARCHITECTURAL CONCRETE (S&S, REC)
      6) METAL FABRICATIONS, RAILINGS, AND GRATINGS (S&S)
      7) ROOF TOP EQUIPMENT AND ANCHORAGES (S&S)
      8) STEEL STAIRS (S&S)
      9) STRUCTURAL STEEL CONNECTIONS (S&S)
      10) WINDOW WASHING AND FALL ARREST SYSTEMS (S&S)
      11) MEP HANGERS AND SUPPORTS (S&S, REC)

   NOTES:

   (S&S) ITEMS MARKED THUS SHALL HAVE THE SHOP DRAWINGS AND DELEGATED DESIGN SUBMITTALS (INCLUDING CALCULATIONS) SEALED PER THE PROJECT SPECIFICATIONS BY AN ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.

   (REC) ITEMS MARKED THUS SHALL BE SUBMITTED TO ENGINEER FOR RECORD ONLY AND WILL NOT HAVE THE ENGINEER'S SHOP DRAWING STAMP AFFIXED.

3. SUBMITTALS WITH IMPACT TO STRUCTURE:
   a. ELEVATORS
   b. MECHANICAL EQUIPMENT WEIGHTS

4. OTHER SUBMITTALS:
   a. PILE LOAD TEST RECORDS (S&S, REC)

5. SUBMITTAL REQUIREMENTS:
   a. ALL SHOP DRAWINGS MUST BE REVIEWED AND ELECTRONICALLY STAMPED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL.
   b. CONTRACTOR SHALL PROVIDE THE SUBMITTAL IN ELECTRONIC PORTABLE DOCUMENT FORMAT (PDF) PER THE SPECIFICATIONS.
   c. THE OMISSION FROM THE SHOP DRAWINGS OF ANY MATERIALS REQUIRED BY THE CONTRACT DOCUMENTS TO BE FURNISHED SHALL NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY OF FURNISHING AND INSTALLING SUCH MATERIALS, REGARDLESS OF WHETHER THE SHOP DRAWINGS HAVE BEEN REVIEWED AND APPROVED.

C. REPRODUCTION

1. THE USE OF ELECTRONIC FILES OR REPRODUCTIONS OF THESE CONTRACT DOCUMENTS BY ANY CONTRACTOR, SUBCONTRACTOR, ERECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFIES THEIR ACCEPTANCE OF ALL INFORMATION SHOWN HEREON AS CORRECT, AND OBLIGATES THEMSELVES TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS THAT MAY OCCUR HEREON.

PART X - MISCELLANEOUS

A. CONTRACT DOCUMENTS

1. IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR T

STRUCTURAL SYMBOLS AND NOTATIONS

MATERIAL IDENTIFICATION GRAPHICS

THE FOLLOWING MATERIAL IDENTIFICATION SYMBOLS MAY BE USED IN THE SECTIONS AND DETAILS:

ALUMINUM | CAST-IN-PLACE CONCRETE | EARTH | EARTH (UNDISTURBED) | EXISTING CONSTRUCTION | GRAVEL | GROUT

MASONRY | POLYSTYRENE | RIGID INSULATION | ROCK | STEEL | STRUCTURAL PRECAST CONCRETE | WOOD

THE FOLLOWING MATERIAL IDENTIFICATION SYMBOLS MAY BE USED IN THE PLANS:

STEEL COLUMN | CONCRETE COLUMN | CONCRETE WALL | LOADBEARING MASONRY WALL | NON-LOADBEARING MASONRY WALL | STRUCTURAL PRECAST WALL | GRATING

EX W14x22

BOUNDARY OF EXISTING CONSTRUCTION

DEMO W14x22

BOUNDARY OF EXISTING CONSTRUCTION TO BE DEMOLISHED

BOUNDARY OF FUTURE EXPANSION

BOUNDARY OF CONCRETE TOPPING SLAB POURED ON SUBSTRUCTURE

SYMBOLIC LINEWORK

THE FOLLOWING SYMBOLIC LINEWORK MAY BE USED THROUGHOUT THE DRAWINGS:

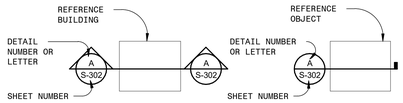
- CENTERLINE
- GRIDLINE
- MATCHLINE
- NOT IN CONTRACT
- OPENING
- STEEL JOIST
- WELDED WIRE REINFORCEMENT

VIEW REFERENCE SYMBOLS

THESE FOLLOWING SYMBOLS MAY BE USED THROUGHOUT THE DRAWINGS TO REFER TO OTHER VIEWS:

FRAMING ELEVATION

CALLOUT

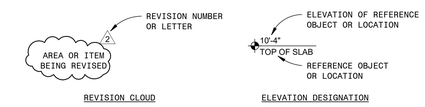


OVERALL BUILDING

PARTIAL SECTION OR DETAIL

UNIVERSAL SYMBOLS

THESE FOLLOWING SYMBOLS MAY BE USED THROUGHOUT THE DRAWINGS:



REVISION CLOUD

ELEVATION DESIGNATION

REFERENCE GRID INDICATOR

WORK POINT

FLAT FACE BREAK MARK

CURVED FACE BREAK MARK

LINE BREAK MARK

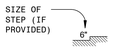
PLAN SYMBOLS

THE FOLLOWING INDICATOR SYMBOLS MAY BE USED THROUGHOUT THE PLANS:

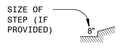
SLOPE TRANSITION

RAMP SLOPE

SLOPE PITCH



STEP

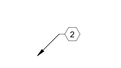


STEP TO SLOPE

RIDGE

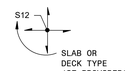
PROJECT NORTH ARROW

BORING LOCATION

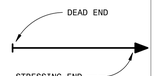


KEYNOTE INDICATOR

ONE-WAY SLAB OR DECK SPAN



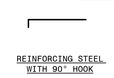
TWO-WAY SLAB SPAN



POST-TENSIONING

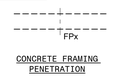
SMALL PENTRATIONS IN STRUCTURAL SLAB OR DECK

LARGE AREA OPENING IN STRUCTURAL SLAB OR DECK

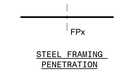


REINFORCING STEEL WITH 90° HOOK

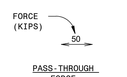
REINFORCING STEEL WITH 180° HOOK



CONCRETE FRAMING PENETRATION

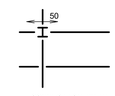


STEEL FRAMING PENETRATION

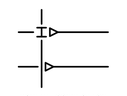


PASS-THROUGH FORCE

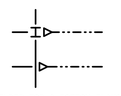
BRACE/KICKER DIRECTION



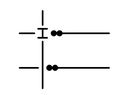
CONNECTION WITH PASS THROUGH FORCE



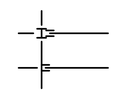
MOMENT CONNECTION (NON-SEISMIC)



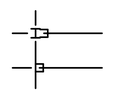
WELDED TOP & BOTTOM FLANGE (SEISMIC COLLECTOR)



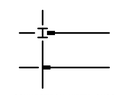
CONNECTION WITH TWO COLUMNS OF BOLTS



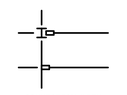
CONNECTION WITH DOUBLE SHEAR PLATE



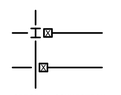
CONNECTION WITH FULL DEPTH SHEAR PLATE



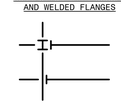
CONNECTION WITH GUSSET AND WELDED FLANGES



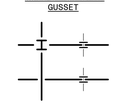
CONNECTION WITH GUSSET



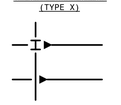
SPECIAL CONNECTION (TYPE X)



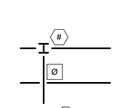
CONNECTION WITH WELDED TOP FLANGE



BEAM SPLICE CONNECTION (SHEAR ONLY)

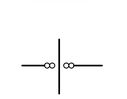


MOMENT CONNECTION (SEISMIC)

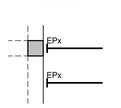


PROVIDE # NUMBER OF BOLTS IN CONNECTION

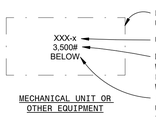
PROVIDE Ø BOLT DIAMETER IN PLACE OF SCHED BOLT



CONNECTION WITH TWO COLUMNS OF BOLTS AND EXTENDED SHEAR PLATE



CONNECTION TO CONCRETE WITH EMBEDDED PLATE



MECHANICAL UNIT OR OTHER EQUIPMENT

STRUCTURAL ELEMENTS TAGS

THE FOLLOWING STRUCTURAL ELEMENT TAGS, USED IN CONJUNCTION WITH SCHEDULES, MAY APPEAR ON THE DRAWINGS:

Bxx DENOTES SLAB BOTTOM REINFORCEMENT TYPE "xx"
BMxx DENOTES CONCRETE BEAM TYPE "xx"
BPxx DENOTES BASE PLATE TYPE "xx"
Cxx DENOTES COLUMN TYPE "xx"
CFxx DENOTES CONTINUOUS FOOTING TYPE "xx"
DPxx DENOTES DRILLED PIER TYPE "xx"
EPxx DENOTES EMBEDDED PLATE TYPE "xx"
Fxx DENOTES SPREAD FOOTING TYPE "xx"
FPxx DENOTES FRAMING PENETRATION TYPE "xx"
GBxx DENOTES GRADE BEAM TYPE "xx"
MKxx DENOTES CMU WALL TYPE "xx"
Pxx DENOTES PIER TYPE "xx"
PCxx DENOTES PILE OR PIER CAP TYPE "xx"
Sxx DENOTES ONE-WAY SLAB TYPE "xx"
SRxx DENOTES HEADED SHEAR STUD REINFORCEMENT TYPE "xx"
SFxx DENOTES STRIP FOOTING TYPE "xx"
Txx DENOTES SLAB TOP REINFORCEMENT TYPE "xx"
WFxx DENOTES WALL FOOTING TYPE "xx"

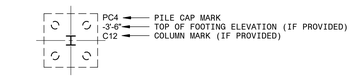
ABBREVIATIONS

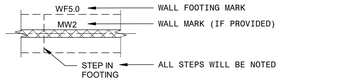
THE FOLLOWING ABBREVIATIONS MAY BE USED IN THE DRAWINGS:

@	AT	LLH	LONG LEG HORIZONTAL
&	AND	LLV	LONG LEG VERTICAL
#	NUMBER	LONG	LONGITUDINAL
ø	ROUND, DIAMETER	LSH	LONG SIDE HORIZONTAL
2L	DOUBLE ANGLE	LSLP	LONG-SLOTTED HOLE PARALLEL
2C	DOUBLE CHANNEL	LSLT	LONG-SLOTTED HOLE TRANSVERSE
ADDL	ADDITIONAL	LSV	LONG SIDE VERTICAL
AESS	ARCHITECTURAL EXPOSED STRUCTURAL STEEL	LWC	LIGHTWEIGHT CONCRETE
		M##	MOMENT
AFF	ABOVE FINISHED FLOOR	MAS	MASONRY
AHR	ANCHOR	MAX	MAXIMUM
AHU	AIR HANDLING UNIT	MC	MOMENT CONNECTION
ALT	ALTERNATE	MECH	MECHANICAL
APPROX	APPROXIMATE	MEZZ	MEZZANINE
AR	ANCHOR ROD	MFR	MANUFACTURER
ARCH	ARCHITECTURAL	MH###	HORIZONTAL (MOMENT)
BC	BOTTOM OF COLUMN	MIN	MINIMUM
BF	BRACED FRAME	MISC	MISCELLANEOUS
BLDG	BUILDING	MTL	METAL
BM	BEAM	NF	NEAR FACE
BO	BOTTOM OF	NLB CMU	NON-LOADBEARING CMU
BOD	BOTTOM OF DECK	NIC	NOT IN CONTRACT
BOT	BOTTOM	NS	NEAR SIDE
BRDG	BRIDGING	NTS	NOT TO SCALE
BRG	BEARING	NWC	NORMALWEIGHT CONCRETE
BTWN	BETWEEN	OC	ON CENTER
C=	CAMBER	OD	OUTSIDE DIAMETER
CANTIL	CANTILEVER	OPH	OPPOSITE HAND
CFSF	COLD-FORMED STEEL FRAMING	OPNG	OPENING
CIP	CAST-IN-PLACE	OPP	OPPOSITE
CJ	CONSTRUCTION/CONTROL JOINT	OVS	OVERSIZED HOLE
CJP	COMPLETE JOINT PENETRATION	P###	AXIAL LOAD
CL	CENTERLINE	PAF	POWDER ACTUATED FASTENER
CMU	CONCRETE MASONRY UNIT	PAR	PARALLEL
COL	COLUMN	PCC	PRECAST CONCRETE
CONC	CONCRETE	PCF	POUNDS PER CUBIC FOOT
CONN	CONNECTION	PCY	POUNDS PER CUBIC YARD
CONSTR	CONSTRUCTION	PERP	PERPENDICULAR
CONT	CONTINUOUS	PL	PLATE
COORD	COORDINATE	PLF	POUNDS PER LINEAR FOOT
COV	COVER	PJP	PARTIAL JOINT PENETRATION
CPRS	COMPRESSIBLE	PRELIM	PRELIMINARY
CTRS	CENTERS	PROP	PROPERTY
db	BAR DIAMETER	PSF	POUNDS PER SQUARE FOOT
DBA	DEFORMED BAR ANCHOR	PSI	POUNDS PER SQUARE INCH
DCW	DEMAND CRITICAL WELD	PT	POST-TENSION(ED)
DET	DETAIL	QTY	QUANTITY
DIA	DIAMETER	R##	REACTION
DIM	DIMENSION	RAD	RADIUS
DWA	DEFORMED WIRE ANCHOR	REF	REFERENCE
DWG	DRAWING	REINF	REINFORCEMENT
EA	EACH	REM	REMAINDER
EF	EACH FACE	REQD	REQUIRED
EJ	EXPANSION JOINT	REV	REVISION
EL	ELEVATION	RS	ROCK SOCKET
ELEV	ELEVATOR	RTU	ROOF TOP UNIT
EMBED	EMBEDMENT, EMBEDDED	SC	SLIP CRITICAL
ENGR	ENGINEER	SCHED	SCHEDULE(D)
EQ	EQUAL	SDS	SELF-DRILLING SCREW
EQUIP	EQUIPMENT	SECT	SECTION
EQUIV	EQUIVALENT	SHT	SHEET
EW	EACH WAY	SIM	SIMILAR
EXIST	EXISTING	SLBB	SHORT LEG BACK TO BACK
EXP	EXPANSION	SLRS	SEISMIC LOAD RESISTING SYSTEM
EXT	EXTERIOR	SOG	SLAB-ON-GRADE
FAB	FABRICATE(D)	SPA	SPACING
f'c=	28 DAY CONCRETE STRENGTH=	SPEC	SPECIFICATION
f'm=	28 DAY MASONRY STRENGTH=	SPRT	SUPPORT
FD	FLOOR DRAIN	SQ	SQUARE
FDTN	FOUNDATION	SS	STAINLESS STEEL
FF	FAR FACE	SSLP	SHORT-SLOTTED HOLE PARALLEL
FIN	FINISH(ED)	SSLT	SHORT-SLOTTED HOLE TRANSVERSE
FLR	FLOOR	STD	STANDARD
FS	FAR SIDE	STIF	STIFFENER
FTG	FOOTING	STIR	STIRRUP
FUT	FUTURE	STL	STEEL
FV	FIELD VERIFY	STRUCT	STRUCTURE, STRUCTURAL
Fy=	YIELD STRENGTH=	SW	SHEAR WALL
GALV	GALVANIZE(D)	SYMM	SYMMETRIC, SYMMETRICAL
GEN	GENERAL	T##	TORSION
GR	GRADE	TC	TOP OF COLUMN
H##	HORIZONTAL (REACTION)	TH###	PASS THRU FORCE
HGR	HANGER	THD	THREADED
HORIZ	HORIZONTAL	TO	TOP OF
HSA	HEADED STUD ANCHOR	TOC	TOP OF CONCRETE
HSS	HOLLOW STRUCTURAL SECTION	TOM	TOP OF MASONRY
ID	INSIDE DIAMETER	TOS	TOP OF STEEL, TOP OF SLAB
INFO	INFORMATION	TRANS	TRANSVERSE
INT	INTERIOR	TYP	TYPICAL
JT	JOINT	UNO	UNLESS NOTED OTHERWISE
K	KIPS (1000 LBS)	VERT	VERTICAL
KSF	KIPS PER SQUARE FOOT	WF	WIDE FLANGE
KSI	KIPS PER SQUARE INCH	WP	WORK POINT
L	ANGLE	WS	WATERSTOP
LB CMU	LOADBEARING CMU	WT	WEIGHT
LBS	POUNDS	WT	SPLIT WIDE FLANGE
Ld	DEVELOPMENT LENGTH	WWR	WELDED WIRE REINFORCEMENT
LLBB	LONG LEG BACK TO BACK	XS	EXTRA STRONG
		XXS	DOUBLE EXTRA STRONG

FOUNDATION ANNOTATION STYLES

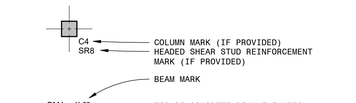
THE FOLLOWING STRUCTURAL ANNOTATIONS MAY APPEAR ON THE FOUNDATION PLANS:

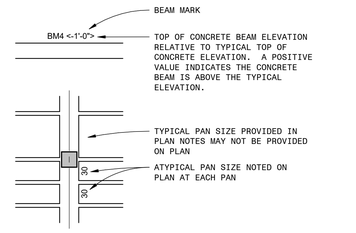


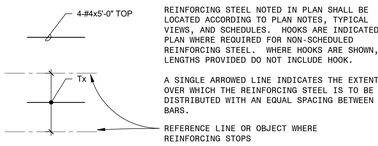


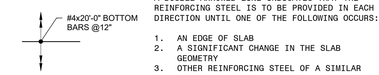
CONCRETE ANNOTATION STYLES

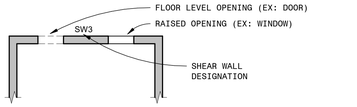
THE FOLLOWING STRUCTURAL ANNOTATIONS MAY APPEAR ON THE CONCRETE FRAMING PLANS

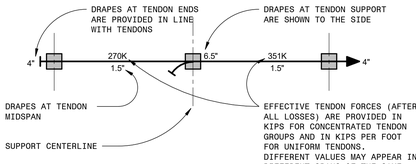




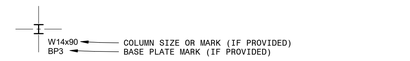




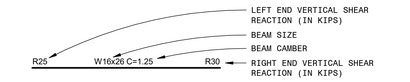




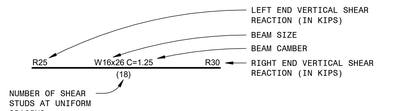
STEEL ANNOTATION STYLES



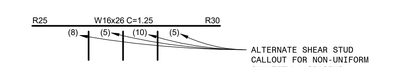
THE FOLLOWING STRUCTURAL ANNOTATIONS MAY APPEAR ON THE STEEL FRAMING PLANS. ALL LOADS INDICATED ARE FACTORED:



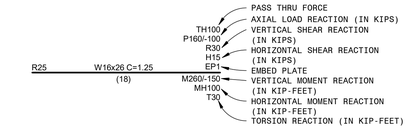
NON-COMPOSITE BEAM



COMPOSITE BEAM

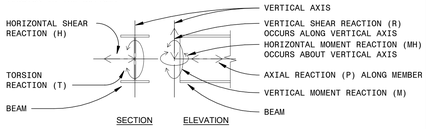


ADDITIONAL INFORMATION MAY BE SPECIFIED ON STEEL BEAMS FOR THE PURPOSES OF DELEGATED CONNECTION DESIGN. IN THOSE CASES, FORCES AND MOMENTS WILL BE PROVIDED AS APPROPRIATE AT THE ENDS OF THE BEAMS AS INDICATED BELOW.

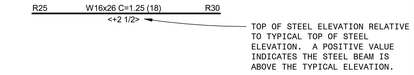


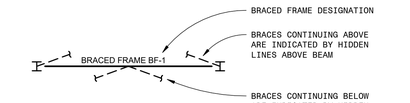
THE FOLLOWING DIAGRAM PROVIDES INFORMATION REGARDING THE ORIENTATION OF THE REACTIONS PROVIDED. THE DIAGRAM BELOW APPLIES TO ALL NON-SLOPING MEMBERS REGARDLESS OF THE INDIVIDUAL MEMBER ORIENTATION (FOR EXAMPLE, AN HSS ORIENTED LONG-SIDE HORIZONTAL). REFER TO DETAILS FOR ADDITIONAL INFORMATION ON LOADING OF SLOPED MEMBERS.

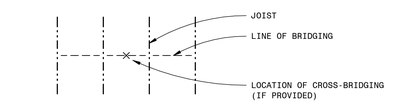
FOR ALL LOADING, REACTIONS ARE TO BE APPLIED AT THE CENTERLINE OF THE MEMBER AT THE WORK POINT, UNLESS NOTED OTHERWISE. IF ONLY A SINGLE VALUE IS SHOWN, THE LOAD SHALL BE ASSUMED TO ACT IN BOTH DIRECTIONS WITH EQUAL MAGNITUDE. FOR AXIAL LOAD REACTIONS, A POSITIVE VALUE INDICATES TENSION. FOR VERTICAL MOMENTS, A POSITIVE VALUE WILL CAUSE COMPRESSION IN THE TOP OF THE BEAM AND TENSION IN THE BOTTOM.

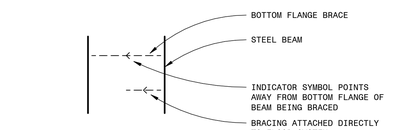


SECTION ELEVATION









HDR ARCHITECTURE, INC.
4830 W KENNEDY BLVD #400
TAMPA, FL 33609
FL FIRM REG CERTIFICATION
AA-C000457

Walter P Moore and Associates, Inc.
201 East Kennedy Blvd, Suite 700
Tampa, Florida 33602
WPM Project No. S05-25024-00

813.221.2424
Florida DBPR License No. 3818

FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
TO THE BEST OF THE ARCHITECTS OR ENGINEERS KNOWLEDGE. THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH THIS SECTION AND CHAPTER 633, FLORIDA STATUES.

ORLANDO HEALTH
MELBOURNE NEW FREESTANDING EMERGENCY CARE FACILITY

LAKE ANDREW DR & WILLET PLACE
MELBOURNE, FL 32940

ORLANDO HEALTH®

PROJECT MANAGER	HDR ARCHITECTURE, INC.
PROJECT DESIGNER	HDR ARCHITECTURE, INC.
PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
LANDSCAPE ARCHITECT	MCKINTOSH ASSOCIATES, an LJA COMPANY
CIVIL ENGINEER	BSE CONSULTANTS, an LJA COMPANY
STRUCTURAL ENGINEER	WALTER P. MOORE & ASSOCIATES, INC.
MECHANICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
ELECTRICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
PLUMBING ENGINEER	I.C. TOMASSON ASSOCIATES, INC
INTERIOR DESIGNER	HDR ARCHITECTURE, INC.
EQUIPMENT PLANNER	IMEG CORP.
WAYFINDING	HDR ARCHITECTURE, INC.

SHEET REVIEWER	

MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

CORY ANN DAYTON LICENSE No. 57441 STATE OF FLORIDA PROFESSIONAL ENGINEER

Cory Ann Dayton, State of Florida, Professional Engineer, License No. 57441. This item has been digitally signed and sealed by Cory Ann Dayton, PE on the date indicated. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.
Email: cdayton@walterpmoore.com

CORY ANN DAYTON, PE NO. 57441

Sheet Name

STRUCTURAL SYMBOLS AND NOTATIONS

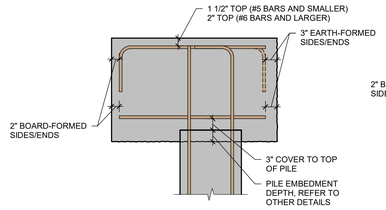
Sheet Number

S-002

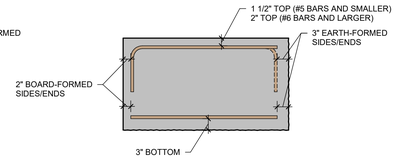
Project Status

CONSTRUCTION DOCUMENTS

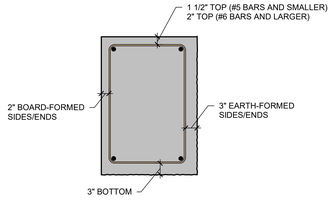
3/4/2026 2:17:59 PM Autodesk Docs://10427132_Orlando_Health_Sunrise/905-25024-01 - OH Vero FSED - Structural - R2026.rvt



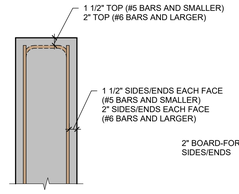
PILE CAPS



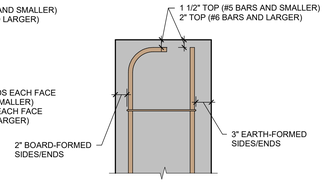
SPREAD FOOTINGS



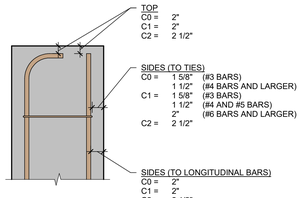
GRADE BEAMS



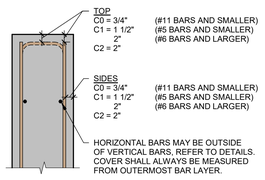
FOUNDATION WALLS



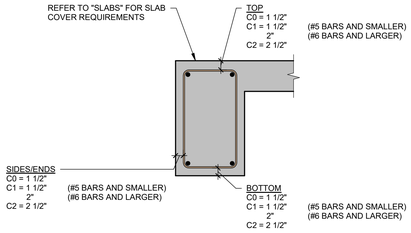
PLINTHS/PILASTERS



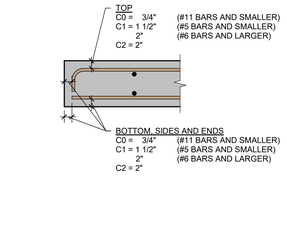
COLUMNS



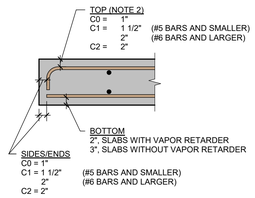
WALLS



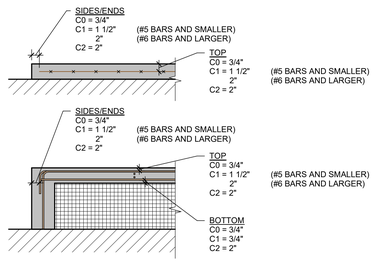
BEAMS AND GIRDERS



SLABS



SLABS-ON-GRADE

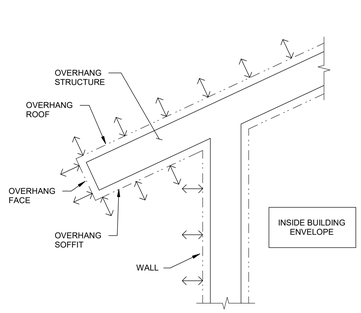


TOPPING SLABS

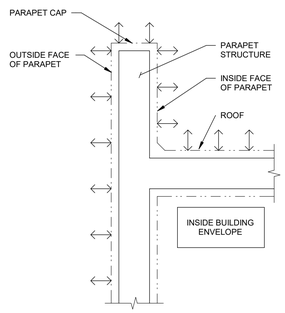
NOTES:
1. CONCRETE PROFILES AND REINFORCING STEEL CONFIGURATIONS ARE SCHEMATIC AND ARE PROVIDED FOR ESTABLISHING TYPICAL CLEAR CONCRETE COVERS ONLY. REFER TO STRUCTURAL PLANS AND DETAILS FOR ALL OTHER INFORMATION.
2. ALL COVERS SHOWN ARE CLEAR FROM THE OUTERMOST SURFACE OF REINFORCING STEEL TO THE CLOSEST OUTER SURFACE OF THE CONCRETE, INCLUDING REVEALS, DRIP GROOVES, OR RUSTICATIONS.
3. WHERE COVERS ARE DIFFERENT AS A FUNCTION OF BAR SIZE, DETAILER SHALL ADJUST LOCATION OF TRANSVERSE REINFORCING STEEL AS REQUIRED SUCH THAT CLEAR COVERS ARE MET FOR BOTH TRANSVERSE AND LONGITUDINAL STEEL.
4. C0, C1, AND C2 REFER TO THE CORROSION EXPOSURE CLASS OF THE CONCRETE ELEMENT. REFER TO THE CLASSES OF CONCRETE MATRIX AND GENERAL NOTES FOR ADDITIONAL INFORMATION.

2 TYPICAL CLEAR CONCRETE COVER REQUIREMENTS FOR REINFORCING STEEL

NO SCALE



OVERHANG



PARAPET

NOTES:
1. REFER TO LOADING DIAGRAMS FOR LOCATION OF EACH CLADDING TYPE.
2. FOR EACH OF THE SPECIFIED OVERHANG CLADDING ELEMENTS, USE THE FOLLOWING VALUES FROM THE TABLES IN 3/S-003 :
   a. OVERHANG SOFFIT CLADDING - USE INTERIOR OR EDGE WALL PRESSURES.
   b. OVERHANG FACE CLADDING - USE EDGE OR CORNER ROOF PRESSURES.
   c. OVERHANG ROOF CLADDING - USE INTERIOR, EDGE, OR CORNER ROOF PRESSURES.
   d. OVERHANG STRUCTURE (SUPPORTING BOTH SOFFIT AND ROOF COMPONENTS)
      1) POSITIVE (DOWNWARD) PRESSURE = [POSITIVE ROOF PRESSURE + -(NEGATIVE WALL PRESSURE)]
      2) NEGATIVE (UPWARD) PRESSURE = [-(NEGATIVE ROOF PRESSURE) + POSITIVE WALL PRESSURE]
3. FOR EACH OF THE SPECIFIED PARAPET CLADDING ELEMENTS, USE THE FOLLOWING VALUES FROM THE TABLES IN 3/S-003:
   a. PARAPET OUTSIDE FACE CLADDING - USE INTERIOR OR EDGE WALL PRESSURES.
   b. PARAPET INSIDE FACE CLADDING - USE THE EDGE OR CORNER ROOF PRESSURES.
   c. PARAPET CAP CLADDING - USE THE EDGE OR CORNER ROOF PRESSURES.
   d. PARAPET STRUCTURE (SUPPORTING BOTH FACES OF THE PARAPET) - USE THE TABULATED PARAPET PRESSURES.
4. THE LOADING DIAGRAMS INDICATED ARE SCHEMATIC IN NATURE ONLY AND ARE NOT INTENDED TO PROVIDE CONSTRUCTION-RELATED INFORMATION. REFER TO DETAILS FOR INFORMATION REGARDING STRUCTURAL FRAMING AND CONNECTIONS.

4 APPLICATION OF WIND PRESSURE FOR PARAPETS AND OVERHANGS

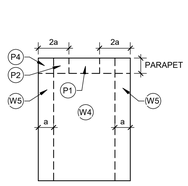
NTS

ASCE 7-22 ULTIMATE WALL PRESSURES

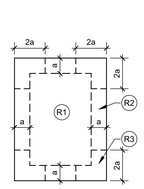
CLADDING TYPE	LOCATION	EFFECTIVE AREA (SF)	POSITIVE PRESSURES (PSF)	NEGATIVE PRESSURES (PSF)
WALL (W4)	ZONE 4 INTERIOR	10 20 50 100 500	+82 +82 +75 +71 +59	-82 -82 -78 -74 -67
WALL (W5)	ZONE 5 EDGE 33FT WIDE	10 20 50 100 500	+82 +82 +75 +71 +59	-150 -150 -132 -119 -89
PARAPET (H<3 FT) (P1)	WALL ZONE 4 + ROOF ZONE 2	10 20 50 100 500	+243 +233 +214 +200 +167	-137 -137 -126 -118 -99
PARAPET (H<3 FT) (P2)	WALL ZONE 4 + ROOF ZONE 3	10 20 50 100 500	+311 +299 +276 +259 +220	-137 -137 -126 -118 -99
PARAPET (H<3 FT) (P3)	WALL ZONE 5 + ROOF ZONE 2	10 20 50 100 500	+243 +233 +214 +200 +167	-205 -205 -181 -163 -122
PARAPET (H<3 FT) (P4)	WALL ZONE 5 + ROOF ZONE 3	10 20 50 100 500	+311 +299 +276 +259 +220	-205 -205 -181 -163 -122
PARAPET (H>3 FT) (P5)	WALL ZONE 4 + ROOF ZONES 2&3	10 20 50 100 500	+243 +233 +215 +200 +167	-137 -137 -126 -118 -99
PARAPET (H>3 FT) (P6)	WALL ZONE 5 + ROOF ZONES 2&3	10 20 50 100 500	+243 +137 +120 +107 +76	-205 -205 -181 -163 -122

ASCE 7-22 ULTIMATE ROOF PRESSURES

CLADDING TYPE	LOCATION	EFFECTIVE AREA, Ae (SF)	POSITIVE PRESSURES (PSF)	NEGATIVE PRESSURES (PSF)
ROOF (R1)	ZONE 1 INTERIOR	10 20 50 100 500	+16 +16 +16 +16 +16	-119 -113 -104 -97 -82
ROOF (PARAPET <3FT) (R2)	ZONE 2 EDGE 33FT WIDE	10 20 50 100 500	+16 +16 +16 +16 +16	-187 -178 -166 -156 -134
ROOF (PARAPET <3FT) (R3)	ZONE 3 CORNER 33FT WIDE X 66FT LONG	10 20 50 100 500	+16 +16 +16 +16 +16	-255 -243 -227 -215 -187
ROOF (PARAPET >3FT) (R2/3)	ZONES 2&3	10 20 50 100 500	+16 +16 +16 +16 +16	-187 -178 -166 -156 -135



ELEVATION



ROOF PLAN

NOTES:
1. WIDTH OF EDGE OR CORNER ZONE, a = 33'-0".
2. PRESSURES ACT NORMAL TO THE CLADDING SURFACE. POSITIVE PRESSURES ACT TOWARDS THE SURFACE AND NEGATIVE PRESSURES ACT AWAY FROM THE SURFACE.
3. DESIGN PRESSURE SHALL NOT BE LESS THAN 16 PSF, ACTING IN EITHER DIRECTION NORMAL TO THE SURFACE.
4. THE EFFECTIVE WIND AREA IS THE SPAN LENGTH MULTIPLIED BY AN EFFECTIVE WIDTH THAT NEED NOT BE LESS THAN ONE-THIRD THE SPAN LENGTH. FOR CLADDING FASTENERS, THE EFFECTIVE WIND AREA SHALL NOT BE GREATER THAN THE AREA THAT IS TRIBUTARY TO AN INDIVIDUAL FASTENER.
5. TABULATED PARAPET STRUCTURE PRESSURES INCLUDE THE PRESSURE FROM BOTH EXPOSED SURFACES OF THE ELEMENT. REFER TO DETAIL 4/S-003 TO DETERMINE THE APPROPRIATE PRESSURE TO BE APPLIED TO THE INDIVIDUAL SURFACES.
6. FOR ROOF CORNER ZONES WITH PARAPETS AT LEAST 3 FEET HIGH ON BOTH SIDES, EDGE ZONE PRESSURES MAY BE USED.
7. THE DESIGN PRESSURES LISTED ARE ULTIMATE DESIGN PRESSURES AS DETERMINED BY ASCE 7-22. TO OBTAIN SERVICE LEVEL PRESSURES, USE THE ALLOWABLE STRESS DESIGN BASIC LOAD COMBINATIONS IN ASCE 7-22, SECTION 2.4.1, WHICH MULTIPLY TABULATED ULTIMATE WIND PRESSURES BY A FACTOR OF 0.6.
8. ULTIMATE LATERAL WIND PRESSURE TO BE USED FOR THE DESIGN OF ROOFTOP EQUIPMENT IS +/-144 PSF. ULTIMATE UPLIFT PRESSURE TO BE USED FOR THE DESIGN OF ROOFTOP EQUIPMENT IS 113 PSF OR THE TABULATED ROOF UPLIFT PRESSURE, WHICHEVER IS GREATER.
9. REFER TO S-031 AND S-032 FOR WIND PRESSURE DIAGRAMS INDICATING APPLICABLE WIND ZONES.

3 WIND COMPONENT AND CLADDING PRESSURES - ASCE 7-22

NO SCALE

CLASSES OF CONCRETE MATRIX

CONCRETE USAGE	MINIMUM COMPRESSIVE STRENGTH (f'c)	CONCRETE TYPE	EXPOSURE CLASS	MAXIMUM W/CM RATIO	PERMISSIBLE AIR CONTENT	REQUIRED CEMENT REPLACEMENT	MAXIMUM AGGREGATE SIZE	ADDITIONAL REMARKS
AUGERCAST PILES	5,500 PSI AT 56 DAYS	NWC	C1			25% - 50%	3/4"	
PILE CAPS	5,000 PSI AT 56 DAYS	NWC	C1			40% - 70%	1 1/2"	
WALL FOOTINGS AND GRADE BEAMS	5,000 PSI AT 56 DAYS	NWC	C1			40% - 70%	1"	
RETAINING WALLS	4,000 PSI AT 28 DAYS	NWC	C1			25% - 50%	1"	
INTERIOR SLAB-ON-GRADE	4,000 PSI AT 28 DAYS	NWC		0.45		10% - 50%	1 1/2"	
EXTERIOR SLAB-ON-GRADE	4,000 PSI AT 28 DAYS	NWC	C1	0.45		10% - 50%	1 1/2"	
INTERIOR CURBS	4,000 PSI AT 28 DAYS	NWC				25% - 50%	1"	
EXTERIOR CURBS	4,000 PSI AT 28 DAYS	NWC	C1			25% - 50%	1"	
ELEVATED FLOOR SYSTEMS	5,000 PSI AT 28 DAYS	NWC		0.45		15% - 50%	1"	
COLUMNS	5,000 PSI AT 28 DAYS	NWC				25% - 50%	1"	
SHEAR WALLS	5,000 PSI AT 28 DAYS	NWC				25% - 50%	1"	

NOTES:
1. ALL CONCRETE SHALL BE CONSIDERED TO BE IN EXPOSURE CLASS F0, S0, W0, AND C0 ACCORDING TO ACI 318-19 UNLESS NOTED OTHERWISE IN THE TABLE ABOVE, IN NOTES BELOW, OR ELSEWHERE ON THE STRUCTURAL DRAWINGS.
2. CONCRETE SHALL BE PROPORTIONED TO COMPLY WITH ACI 318-19 TABLES 19.3.2.1, 19.3.3.1, AND 26.4.2.2b IN ADDITION TO THE NOTATIONS IN THE TABLE ABOVE AND THE STRICTER REQUIREMENTS SHALL GOVERN. REFER TO THE SPECIFICATIONS FOR OTHER REQUIREMENTS FOR VARIOUS EXPOSURE CLASSES RELATIVE TO CEMENT TYPE, AIR ENTRAINMENT REQUIREMENTS, CHLORIDE ION LIMITS, AND POZZOLAN LIMITS.

1 CLASSES OF CONCRETE MATRIX

NO SCALE

HDR ARCHITECTURE, INC.
4830 W KENNEDY BLVD #400
TAMPA, FL 33609
FL FIRM REG CERTIFICATION
AA-C000457

Walter P Moore and Associates, Inc.
201 East Kennedy Blvd, Suite 700
Tampa, Florida 33602
WPM Project No. S05-25024-00

813.221.2424
Florida DBPR License No. 3818

FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
TO THE BEST OF THE ARCHITECTS OR ENGINEERS KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH THIS SECTION AND CHAPTER 633, FLORIDA STATUES.

ORLANDO HEALTH MELBOURNE NEW FREESTANDING EMERGENCY CARE FACILITY

LAKE ANDREW DR & WILLET PLACE
MELBOURNE, FL 32940

ORLANDO HEALTH®

PROJECT MANAGER	HDR ARCHITECTURE, INC.
PROJECT DESIGNER	HDR ARCHITECTURE, INC.
PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
LANDSCAPE ARCHITECT	MCKINTOSH ASSOCIATES, an LJA COMPANY
CIVIL ENGINEER	BSE CONSULTANTS, an LJA COMPANY
STRUCTURAL ENGINEER	WALTER P. MOORE & ASSOCIATES, INC.
MECHANICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
ELECTRICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
PLUMBING ENGINEER	I.C. TOMASSON ASSOCIATES, INC
INTERIOR DESIGNER	HDR ARCHITECTURE, INC.
EQUIPMENT PLANNER	IMEG CORP.
WAYFINDING	HDR ARCHITECTURE, INC.
SHEET REVIEWER	

MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

CORY ANN DAYTON LICENSE No. 57441 STATE OF FLORIDA PROFESSIONAL ENGINEER

Cory Ann Dayton, State of Florida, Professional Engineer, License No. 57441. This item has been digitally signed and sealed by Cory Ann Dayton, PE on the date indicated. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.
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CORY ANN DAYTON, PE NO. 57441

Sheet Name

GENERAL STRUCTURAL INFORMATION

Sheet Number

S-003

Project Status

CONSTRUCTION DOCUMENTS

3/4/2026 2:18:01 PM Autodesk Docs://10427102_Orlando_Health_Sunrise/S05-25024-01 - OH Viera FSED_Structural - R22Dh.rvt

0 1/2" 1" 2"

SLAB TENSION DEVELOPMENT AND LAP SPLICE LENGTHS GRADE 60 REINFORCEMENT, NORMALWEIGHT CONCRETE													
BAR SIZE	LAP CLASS	f'c = 3000 PSI		f'c = 4000 PSI		f'c = 5000 PSI		f'c = 6000 PSI		f'c = 7000 PSI		f'c = 8000 PSI	
		BOTTOM BARS	OTHER BARS	BOTTOM BARS	OTHER BARS	BOTTOM BARS	OTHER BARS	BOTTOM BARS	OTHER BARS	BOTTOM BARS	OTHER BARS	BOTTOM BARS	OTHER BARS
#3	A	12	13	12	12	12	12	12	12	12	12	12	12
	B	16	17	16	16	16	16	16	16	16	16	16	16
#4	A	17	22	15	19	13	17	12	16	12	14	12	14
	B	23	29	20	25	17	23	16	21	16	19	16	19
#5	A	25	32	21	28	19	25	18	23	16	21	15	20
	B	33	42	28	37	25	33	24	30	21	28	20	26
#6	A	33	43	29	37	26	34	24	31	22	28	21	27
	B	43	56	38	49	34	45	32	41	29	37	28	36
#7	A	53	69	46	60	42	54	38	49	35	46	33	43
	B	69	90	60	78	55	71	50	64	46	60	43	56
#8	A	66	86	57	74	51	67	47	61	44	56	41	53
	B	86	112	75	97	67	88	62	80	58	73	54	69
#9	A	80	104	69	90	62	81	57	74	53	68	49	64
	B	104	136	90	117	81	106	75	97	69	89	64	84
#10	A	96	125	83	108	75	97	68	88	63	82	59	77
	B	125	163	108	141	98	127	89	115	82	107	77	101
#11	A	113	146	98	127	87	114	80	104	74	96	69	90
	B	147	190	128	166	114	149	104	136	97	125	90	117

NOTES:
1. ALL SPLICE LENGTHS ARE IN INCHES.
2. THIS TABLE SHALL BE USED FOR SLABS ONLY. REFER TO OTHER DEVELOPMENT LENGTH TABLES FOR OTHER MEMBERS.
3. THE TENSION DEVELOPMENT LENGTH (Ld) IS EQUAL TO THE SCHEDULED "CLASS A" LAP SPLICE LENGTH.
4. A BOTTOM BAR IS DEFINED AS ANY BAR THAT DOES NOT HAVE MORE THAN 12" OF FRESH CONCRETE BELOW THE BAR.
5. OTHER BARS INCLUDE TOP BARS AND ALL OTHER BARS THAT HAVE MORE THAN 12" OF FRESH CONCRETE BELOW THE BAR. FOR TOP REINFORCEMENT IN SLABS THAT ARE 12" THICK OR LESS, TABULATED SPLICE LENGTHS FOR BOTTOM BARS SHALL BE USED.
6. FOR EPOXY-COATED BARS, MULTIPLY THE TABULATED SPLICE LENGTHS OF BOTTOM BARS BY 1.5 AND THE TABULATED SPLICE LENGTHS OF OTHER BARS BY 1.3.
7. WHEN LAP SPLICING BARS OF DIFFERENT SIZES, THE LAP LENGTH IS DETERMINED BY THE SMALLER BAR BUT MAY NOT BE LESS THAN THE "CLASS A" SPLICE LENGTH OF THE LARGER BAR.
8. FOR CONCRETE STRENGTHS IN BETWEEN THOSE TABULATED HERE, USE DEVELOPMENT AND LAP SPLICE LENGTHS OF LOWER CONCRETE STRENGTH.

15 TYPICAL SLAB TENSION DEVELOPMENT AND LAP SPLICE LENGTHS

NO SCALE

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BEAM AND GIRDER TENSION DEVELOPMENT AND LAP SPLICE LENGTHS GRADE 60 REINFORCEMENT, NORMALWEIGHT CONCRETE													
BAR SIZE	LAP CLASS	f'c = 3000 PSI		f'c = 4000 PSI		f'c = 5000 PSI		f'c = 6000 PSI		f'c = 7000 PSI		f'c = 8000 PSI	
		BOTTOM BARS	OTHER BARS	BOTTOM BARS	OTHER BARS	BOTTOM BARS	OTHER BARS	BOTTOM BARS	OTHER BARS	BOTTOM BARS	OTHER BARS	BOTTOM BARS	OTHER BARS
#3	A	12	13	12	12	12	12	12	12	12	12	12	12
	B	16	17	16	16	16	16	16	16	16	16	16	16
#4	A	16	20	14	18	12	16	12	15	12	14	12	13
	B	21	26	19	24	16	21	16	20	16	19	16	17
#5	A	22	28	19	25	17	22	16	20	14	19	14	17
	B	29	37	25	33	23	29	21	26	19	25	19	23
#6	A	31	40	27	35	24	31	22	29	21	27	19	25
	B	41	52	36	46	32	41	29	38	28	36	25	33
#7	A	48	62	41	53	37	48	34	44	31	41	29	38
	B	63	81	54	69	49	63	45	58	41	54	38	50
#8	A	56	73	49	63	44	57	40	52	37	48	35	45
	B	73	95	64	82	58	75	52	68	49	63	46	59
#9	A	66	85	57	74	51	66	47	60	43	56	40	52
	B	86	111	75	97	67	86	62	78	56	73	52	68
#10	A	71	93	62	80	55	72	51	66	47	61	44	57
	B	93	121	81	104	72	94	67	86	62	80	58	75
#11	A	82	106	71	92	64	83	58	75	54	70	50	65
	B	107	138	93	120	84	108	76	98	71	91	65	85

NOTES:
1. ALL SPLICE LENGTHS ARE IN INCHES
2. THIS TABLE SHALL BE USED FOR BEAMS AND GIRDERS ONLY. REFER TO OTHER DEVELOPMENT LENGTH TABLES FOR OTHER MEMBERS.
3. THE TENSION DEVELOPMENT LENGTH (Ld) IS EQUAL TO THE SCHEDULED "CLASS A" LAP SPLICE LENGTH.
4. A BOTTOM BAR IS DEFINED AS ANY BAR THAT DOES NOT HAVE MORE THAN 12" OF FRESH CONCRETE BELOW THE BAR.
5. OTHER BARS INCLUDE TOP BARS, FACE BARS, AND ALL OTHER BARS THAT HAVE MORE THAN 12" OF FRESH CONCRETE BELOW THE BAR.
6. FOR EPOXY-COATED BARS, MULTIPLY THE TABULATED SPLICE LENGTHS OF BOTTOM BARS BY 1.5 AND THE TABULATED SPLICE LENGTHS OF OTHER BARS BY 1.3.
7. WHEN LAP SPLICING BARS OF DIFFERENT SIZES, THE LAP LENGTH IS DETERMINED BY THE SMALLER BAR BUT MAY NOT BE LESS THAN THE "CLASS A" SPLICE LENGTH OF THE LARGER BAR.
8. FOR CONCRETE STRENGTHS IN BETWEEN THOSE TABULATED HERE, USE DEVELOPMENT AND LAP SPLICE LENGTHS OF LOWER CONCRETE STRENGTH.

13 TYPICAL BEAM AND GIRDER TENSION DEVELOPMENT AND LAP SPLICE LENGTHS

NO SCALE

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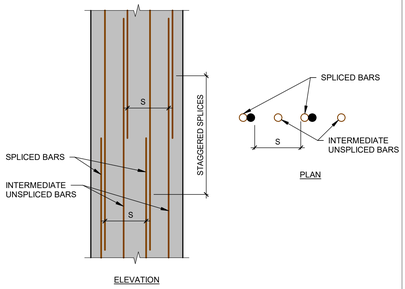

NOTES:
1. "S" = CLEAR BAR SPACING TO BE USED FOR DETERMINATION OF TENSION SPLICE LENGTH CATEGORY

11 TYPICAL CLEAR SPACING CRITERIA OF LAP SPLICED BARS, STAGGERED SPLICES

NO SCALE

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WALL HORIZONTAL BAR TENSION DEVELOPMENT AND LAP SPLICE LENGTHS GRADE 60 REINFORCEMENT, NORMALWEIGHT CONCRETE							
BAR SIZE	LAP CLASS	f'c = 3000 PSI	f'c = 4000 PSI	f'c = 5000 PSI	f'c = 6000 PSI	f'c = 7000 PSI	f'c = 8000 PSI
#3	A	13	12	12	12	12	12
	B	17	16	16	16	16	16
#4	A	22	19	17	16	14	14
	B	29	25	23	21	19	19
#5	A	32	28	25	23	21	20
	B	42	37	33	30	28	26
#6	A	43	37	34	31	28	27
	B	56	49	45	41	37	36
#7	A	69	60	54	49	46	43
	B	90	78	71	64	60	56
#8	A	86	74	67	61	56	53
	B	112	97	88	80	73	69
#9	A	104	90	81	74	68	64
	B	136	117	106	97	89	84
#10	A	125	108	97	88	82	77
	B	163	141	127	115	107	101
#11	A	146	127	114	104	96	90
	B	190	166	149	136	125	117

NOTES:
1. ALL SPLICE LENGTHS ARE IN INCHES.
2. THIS TABLE SHALL BE USED FOR WALL HORIZONTAL BARS ONLY. REFER TO OTHER DEVELOPMENT LENGTH TABLES FOR WALL VERTICAL BARS AND OTHER MEMBERS.
3. THE TENSION DEVELOPMENT LENGTH (Ld) IS EQUAL TO THE SCHEDULED "CLASS A" LAP SPLICE LENGTH.
4. FOR EPOXY-COATED BARS, MULTIPLY THE TABULATED SPLICE LENGTHS BY 1.3.
5. WHEN LAP SPLICING BARS OF DIFFERENT SIZES, THE LAP LENGTH IS DETERMINED BY THE SMALLER BAR BUT MAY NOT BE LESS THAN THE "CLASS A" SPLICE LENGTH OF THE LARGER BAR.
6. FOR CONCRETE STRENGTHS IN BETWEEN THOSE TABULATED HERE, USE DEVELOPMENT AND LAP SPLICE LENGTHS OF LOWER CONCRETE STRENGTH.

10 TYPICAL WALL HORIZONTAL BAR TENSION DEVELOPMENT AND LAP SPLICE LENGTHS

NO SCALE

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WALL VERTICAL BAR TENSION DEVELOPMENT AND LAP SPLICE LENGTHS GRADE 60 REINFORCEMENT, NORMALWEIGHT CONCRETE													
BAR SIZE	LAP CLASS	f'c = 3000 PSI		f'c = 4000 PSI		f'c = 5000 PSI		f'c = 6000 PSI		f'c = 7000 PSI		f'c = 8000 PSI	
		CATEGORY		CATEGORY		CATEGORY		CATEGORY		CATEGORY		CATEGORY	
		1	2	1	2	1	2	1	2	1	2	1	2
#4	A	17	17	15	15	15	15	15	15	15	15	15	15
	B	22	22	19	19	17	17	16	16	15	15	15	15
#5	A	21	25	19	21	19	19	19	19	19	19	19	19
	B	27	32	24	28	21	25	19	23	19	21	19	20
#6	A	25	33	23	29	23	26	23	24	23	23	23	23
	B	33	43	28	37	25	34	23	31	23	28	23	27
#7	A	41	48	35	42	32	38	29	34	27	32	27	30
	B	53	63	46	54	41	49	38	45	35	41	33	39
#8	A	51	55	44	48	40	43	36	39	34	36	31	34
	B	66	72	57	62	51	56	47	51	44	47	41	44
#9	A	62	62	54	54	48	48	44	44	41	41	38	38
	B	81	81	70	70	63	63	57	57	53	53	50	50
#10	A	71	71	61	61	55	55	50	50	47	47	44	44
	B	92	92	80	80	71	71	65	65	60	60	56	56
#11	A	84	84	73	73	65	65	60	60	55	55	52	52
	B	109	109	95	95	85	85	77	77	72	72	67	67

NOTES:
1. ALL SPLICE LENGTHS ARE IN INCHES.
2. THIS TABLE SHALL BE USED FOR WALL VERTICAL BARS ONLY. REFER TO OTHER DEVELOPMENT LENGTH TABLES FOR WALL HORIZONTAL BARS AND OTHER MEMBERS.
3. THE TENSION DEVELOPMENT LENGTH (Ld) IS EQUAL TO THE SCHEDULED "CLASS A" LAP SPLICE LENGTH.
4. FOR DETERMINING THE BAR CATEGORY, "db" IS DEFINED AS THE DIAMETER OF THE LARGER BAR BEING SPLICED.
5. THE "CATEGORY 1" LAP LENGTH SHALL BE USED WHEN THE CLEAR SPACING BETWEEN BARS AT THE SPLICE IS EQUAL TO OR GREATER THAN 3db. THE "CATEGORY 2" LAP LENGTH SHALL BE USED WHEN THE CLEAR SPACING BETWEEN BARS AT THE SPLICE IS LESS THAN 3db AND GREATER THAN 2db.
6. WHEN THE CLEAR SPACING BETWEEN BARS AT THE SPLICE IS EQUAL TO OR LESS THAN 2db, SPLICES SHALL BE STAGGERED SO THAT NO MORE THAN 50% OF BARS ARE SPLICED AT ANY GIVEN LOCATION. REFER TO 11/S-004 FOR CRITERIA TO DETERMINE CLEAR SPACING "S" FOR BARS AT STAGGERED SPLICES.
7. FOR EPOXY-COATED BARS, MULTIPLY THE TABULATED SPLICE LENGTHS BY 1.5.
8. WHEN LAP SPLICING BARS OF DIFFERENT SIZES, THE LAP LENGTH IS DETERMINED BY THE SMALLER BAR BUT MAY NOT BE LESS THAN THE "CLASS A" SPLICE LENGTH OF THE LARGER BAR.
9. FOR CONCRETE STRENGTHS IN BETWEEN THOSE TABULATED HERE, USE DEVELOPMENT AND LAP SPLICE LENGTHS OF LOWER CONCRETE STRENGTH.

8 TYPICAL WALL VERTICAL BAR TENSION DEVELOPMENT AND LAP SPLICE LENGTHS

NO SCALE

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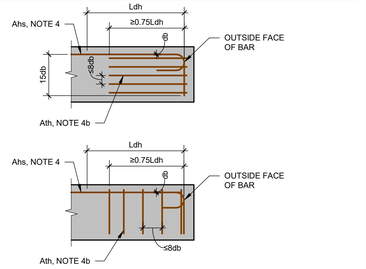
DEVELOPMENT LENGTHS OF STANDARD HOOKS IN TENSION TABLE 1 - UNCONFINED HOOKS GRADE 60 REINFORCEMENT, NORMALWEIGHT CONCRETE														
BAR SIZE	f'c=3000 PSI		f'c=4000 PSI		f'c=5000 PSI		f'c=6000 PSI		f'c=7000 PSI		f'c=8000 PSI		BAR SIZE
	Lhb	0.8 Lhb	Lhb	0.8 Lhb	Lhb	0.8 Lhb	Lhb	0.8 Lhb	Lhb	0.8 Lhb	Lhb	0.8 Lhb	
#3	8	6	7	6	7	6	7	6	6	6	6	6	#3
#4	12	10	11	9	11	9	10	8	10	8	9	7	#4
#5	16	13	15	12	15	12	14	12	13	11	13	10	#5
#6	21	17	20	16	19	15	19	15	17	14	16	13	#6
#7	27	21	25	20	24	19	24	19	22	18	20	16	#7
#8	32	26	30	24	29	24	29	23	27	21	25	20	#8
#9	39	31	36	29	35	28	34	27	32	25	30	24	#9
#10	46	37	43	35	42	33	41	33	38	30	35	28	#10
#11	54	43	51	41	49	39	48	38	44	35	41	33	#11
#14	71	-	66	-	64	-	63	-	58	-	54	-	#14
#18	109	-	102	-	98	-	96	-	89	-	83	-	#18

DEVELOPMENT LENGTHS OF STANDARD HOOKS IN TENSION TABLE 2 - CONFINED HOOKS GRADE 60 REINFORCEMENT, NORMALWEIGHT CONCRETE														
BAR SIZE	f'c=3000 PSI		f'c=4000 PSI		f'c=5000 PSI		f'c=6000 PSI		f'c=7000 PSI		f'c=8000 PSI		BAR SIZE
	Lhb	0.8 Lhb	Lhb	0.8 Lhb	Lhb	0.8 Lhb	Lhb	0.8 Lhb	Lhb	0.8 Lhb	Lhb	0.8 Lhb	
#3	6	6	6	6	6	6	6	6	6	6	6	6	#3
#4	8	6	7	6	7	6	7	6	6	6	6	6	#4
#5	10	8	10	8	9	8	9	7	9	7	8	7	#5
#6	13	11	13	10	12	10	12	10	11	9	10	8	#6
#7	17	14	16	13	15	12	15	12	14	11	13	10	#7
#8	20	16	19	15	18	15	18	15	17	14	16	13	#8
#9	24	20	23	18	22	18	22	17	20	16	19	15	#9
#10	29	23	27	22	26	21	26	21	24	19	22	18	#10
#11	34	27	32	26	31	25	30	24	28	22	26	21	#11
#14	44	-	42	-	40	-	39	-	36	-	34	-	#14
#18	68	-	64	-	62	-	60	-	56	-	52	-	#18



NOTES:
1. Ldh = DEVELOPMENT LENGTH OF STANDARD HOOKS IN TENSION (INCHES).
2. Ldh = Lhb UNLESS CONDITIONS OF NOTE 3 ARE SATISFIED.
3. Ldh = 0.8Lhb FOR #11 BARS AND SMALLER WHEN HOOKS TERMINATING INSIDE COLUMN CORE WITH SIDE COVER (NORMAL TO PLANE OF HOOK) IS NOT LESS THAN 2 1/2 INCHES OR WITH SIDE COVER (NORMAL TO PLANE OF HOOK) IS NOT LESS THAN 6db.
4. FOR #11 BARS AND SMALLER, TABLE 2 SHALL BE USED PER ACI 318-19 IF ONE OF THE FOLLOWING CONDITIONS ARE MET:
   a. THE SPACING OF HOOKED BARS IS GREATER OR EQUAL TO 6db.
   b. CONFINING REINFORCEMENT AS SHOWN IS PROVIDED: Ath ≥ 0.4*Ahs
5. Ath = TOTAL CROSS-SECTIONAL AREA OF TIES OR STIRRUPS CONFINING HOOKED BARS. Ahs = TOTAL CROSS-SECTIONAL AREA OF HOOKED BARS BEING DEVELOPED AT A CRITICAL SECTION.
6. HOOKS ARE NOT CONSIDERED EFFECTIVE FOR DEVELOPING BARS IN COMPRESSION.
7. Ldh SHALL BE MULTIPLIED BY 1.2 FOR EPOXY-COATED HOOKED REINFORCING BARS.

COLUMN TENSION DEVELOPMENT AND LAP SPLICE LENGTHS GRADE 60 REINFORCEMENT, NORMALWEIGHT CONCRETE													
BAR SIZE	LAP CLASS	f'c = 3000 PSI		f'c = 4000 PSI		f'c = 5000 PSI		f'c = 6000 PSI		f'c = 7000 PSI		f'c = 8000 PSI	
		CATEGORY		CATEGORY		CATEGORY		CATEGORY		CATEGORY		CATEGORY	
		1	2	1	2	1	2	1	2	1	2	1	2
#5	A	19	20	19	19	19	19	19	19	19	19	19	19
	B	23	26	20	23	19	21	19	19	19	19	19	19
#6	A	23	29	23	25	23	23	23	23	23	23	23	23
	B	29	37	25	32	23	29	23	26	23	24	23	23
#7	A	33	43	29	37	27	34	27	31	27	28	27	27
	B	43	56	38	49	34	43	31	40	28	37	27	34
#8	A	39	51	34	44	30	39	30	36	30	33	30	31
	B	50	66	44	57	39	51	36	46	33	43	31	40
#9	A	44	58	38	50	34	45	34	41	34	38	34	36
	B	58	75	50	65	45	58	41	53	38	49	35	46
#10	A	51	66	44	57	39	51	39	47	39	43	39	40
	B	66	85	57	74	51	66	47	60	43	56	41	52
#11	A	57	72	49	62	44	56	43	51	43	47	43	44
	B	74	93	64	81	57	72	52	66	49	61	46	57

NOTES:
1. ALL SPLICE LENGTHS ARE IN INCHES.
2. THIS TABLE SHALL BE USED FOR COLUMNS ONLY. REFER TO OTHER DEVELOPMENT LENGTH TABLES FOR OTHER MEMBERS.
3. THE TENSION DEVELOPMENT LENGTH (Ld) IS EQUAL TO THE SCHEDULED "CLASS A" LAP SPLICE LENGTH.
4. FOR DETERMINING THE BAR CATEGORY, "db" IS DEFINED AS THE DIAMETER OF THE LARGER BAR BEING SPLICED.
5. THE "CATEGORY 1" LAP LENGTH SHALL BE USED WHEN THE CLEAR SPACING BETWEEN BARS AT THE SPLICE IS EQUAL TO OR GREATER THAN 3db. THE "CATEGORY 2" LAP LENGTH SHALL BE USED WHEN THE CLEAR SPACING BETWEEN BARS AT THE SPLICE IS LESS THAN 3db AND GREATER THAN 2db.
6. WHEN THE CLEAR SPACING BETWEEN BARS AT THE SPLICE IS EQUAL TO OR LESS THAN 2db, SPLICES SHALL BE STAGGERED SO THAT NO MORE THAN 50% OF BARS ARE SPLICED AT ANY GIVEN LOCATION. REFER TO 11/S-004 FOR CRITERIA TO DETERMINE CLEAR SPACING "S" FOR BARS AT STAGGERED SPLICES.
7. IN CASES WHERE BAR SPACING IS NOT THE SAME AT DIFFERENT COLUMN FACES, THE SMALLER SPACING SHALL BE USED TO DETERMINE THE APPLICABLE SPLICE CATEGORY.
8. FOR EPOXY-COATED BARS, MULTIPLY THE TABULATED SPLICE LENGTHS BY 1.5.
9. WHEN LAP SPLICING BARS OF DIFFERENT SIZES, THE LAP LENGTH IS DETERMINED BY THE SMALLER BAR BUT MAY NOT BE LESS THAN THE "CLASS A" SPLICE LENGTH OF THE LARGER BAR.
10. FOR CONCRETE STRENGTHS IN BETWEEN THOSE TABULATED HERE, USE DEVELOPMENT AND LAP SPLICE LENGTHS OF LOWER CONCRETE STRENGTH.

5 TYPICAL COLUMN TENSION DEVELOPMENT AND LAP SPLICE LENGTHS

NO SCALE

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MAT FOUNDATION LAP SPLICE LENGTHS GRADE 60 REINFORCEMENT, NORMALWEIGHT CONCRETE													
BAR SIZE	LAP CLASS	f'c = 3000 PSI		f'c = 4000 PSI		f'c = 5000 PSI		f'c = 6000 PSI		f'c = 7000 PSI		f'c = 8000 PSI	
		BOTTOM BARS	OTHER BARS	BOTTOM BARS	OTHER BARS	BOTTOM BARS	OTHER BARS	BOTTOM BARS	OTHER BARS	BOTTOM BARS	OTHER BARS	BOTTOM BARS	OTHER BARS
#5	A	17	22	15	19	13	17	12	16	12	14	12	14
	B	22	28	19	25	17	22	16	20	16	19	16	18
#6	A	20	26	18	23	16	20	14	19	13	17	13	16
	B	26	34	23	29	20	26	19	24	17	22	16	21
#7	A	29	38	25	33	23	29	21	27	19	25	18	23
	B	38	49	33	43	29	38	27	35	25	32	23	30
#8	A	33	43	29	37	26	34	24	31	22	28	21	27
	B	43	56	37	49	34	44	31	40	28	37	27	35
#9	A	41	54	36	46	32	42	29	38	27	35	25	33
	B	54	69	46	60	42	54	38	49	35	46	33	43
#10	A	51	66	44	57	39	51	36	47	33	43	31	41
	B	66	85	57	74	51	66	47	61	43	56	41	53
#11	A	61	79	53	68	47	61	43	56	40	52	37	49
	B	79	103	68	89	61	80	56	73	52	67	49	63

NOTES:
1. ALL SPLICE LENGTHS ARE IN INCHES.
2. THIS TABLE SHALL ONLY BE USED FOR MAT FOUNDATION LAP SPLICE LENGTHS OF BARS MEETING THE REQUIREMENTS IN NOTES 3 THROUGH 5. REFER TO 15/S-004 FOR OTHER CONDITIONS AND FOR MAT FOUNDATION TENSION DEVELOPMENT LENGTHS. REFER TO OTHER DEVELOPMENT LENGTH TABLES FOR OTHER MEMBERS.
3. CENTER TO CENTER SPACING OF BARS SHALL NOT BE LESS THAN 4".
4. WHERE BAR SPACING IS LESS THAN 8", LAP SPLICES AT ALL LOCATIONS SHALL BE STAGGERED.
5. CONCRETE COVER TO THE SPLICED BARS SHALL NOT BE LESS THAN 2" (CLEAR).
6. A BOTTOM BAR IS DEFINED AS ANY BAR THAT DOES NOT HAVE MORE THAN 12" OF FRESH CONCRETE BELOW THE BAR.
7. OTHER BARS INCLUDE TOP BARS AND ALL OTHER BARS THAT HAVE MORE THAN 12" OF FRESH CONCRETE BELOW THE BAR.
8. FOR EPOXY-COATED BARS, MULTIPLY THE TABULATED SPLICE LENGTHS OF BOTTOM BARS BY 1.5 AND THE TABULATED SPLICE LENGTHS OF OTHER BARS BY 1.3.
9. WHEN LAP SPLICING BARS OF DIFFERENT SIZES, THE LAP LENGTH IS DETERMINED BY THE SMALLER BAR BUT MAY NOT BE LESS THAN THE "CLASS A" SPLICE LENGTH OF THE LARGER BAR.
10. FOR CONCRETE STRENGTHS IN BETWEEN THOSE TABULATED HERE, USE DEVELOPMENT AND LAP SPLICE LENGTHS OF LOWER CONCRETE STRENGTH.

3 TYPICAL MAT FOUNDATION LAP SPLICE LENGTHS

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1 TYPICAL DEVELOPMENT LENGTHS OF STANDARD HOOKS IN TENSION

NO SCALE

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FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
TO THE BEST OF THE ARCHITECTS OR ENGINEERS KNOWLEDGE. THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH THIS SECTION AND CHAPTER 633, FLORIDA STATUTES.

ORLANDO HEALTH MELBOURNE NEW FREESTANDING EMERGENCY CARE FACILITY

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MELBOURNE, FL 32940

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PROJECT DESIGNER	HDR ARCHITECTURE, INC.
PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
LANDSCAPE ARCHITECT	MCKINTOSH ASSOCIATES, an LJA COMPANY
CIVIL ENGINEER	BSE CONSULTANTS, an LJA COMPANY
STRUCTURAL ENGINEER	WALTER P. MOORE & ASSOCIATES, INC.
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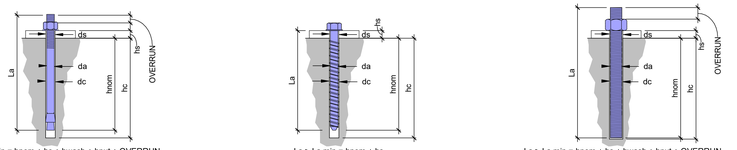
INTERIOR DESIGNER	HDR ARCHITECTURE, INC.
EQUIPMENT PLANNER	IMEG CORP.
WAYFINDING	HDR ARCHITECTURE, INC.
SHEET REVI



La ≥ La,min = hnom + hs + hwash + hnut + OVERRUN

EXPANSION AND UNDERCUT

La ≥ La,min = hnom + hs

SCREW

La ≥ La,min = hnom + hs + hwash + hnut + OVERRUN

ADHESIVE

ANCHOR SIZE FOR ORDER:	"da" DIAMETER x "La" ANCHOR LENGTH
DRILLED HOLE:	"dc" DIAMETER x "hc" DEEP

DEFINITIONS:

- da = DIAMETER OF ANCHOR (IN)
- dc = DIAMETER OF HOLE IN CONCRETE = DIAMETER OF DRILL BIT (IN)
- ds = DIAMETER OF HOLE IN STEEL ATTACHMENT (IN)
- La,min = MINIMUM LENGTH OF ANCHOR (IN)
- La = ORDERED LENGTH OF ANCHOR (IN)
- hnom = NOMINAL EMBEDMENT DEPTH (IN)
- hc = DEPTH OF HOLE IN CONCRETE (IN)
- hs = THICKNESS OF STEEL ATTACHMENT (IN)
- hwash = THICKNESS OF WASHER (IN)
- hnut = HEIGHT OF HEX NUT (IN)
- OVERRUN = 1/4" UNLESS NOTED OTHERWISE

CONTRACTOR AND INSTALLER NOTES:

1. ONLY POST-INSTALLED ANCHOR PRODUCTS SPECIFIED IN THE CONTRACT DOCUMENTS SHALL BE USED WHERE SPECIFIED IN THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL PROVIDE SIGNED AND SEALED CALCULATIONS TO THE ENGINEER OF RECORD (EOR) FOR ANCHOR PRODUCTS SUBSTITUTED FOR THOSE INDICATED IN THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE EOR PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN ANCHORS AND REINFORCEMENT.
2. ANCHOR LENGTHS SPECIFIED IN THE CONTRACT DOCUMENTS INDICATE THE *NOMINAL EMBEDMENT DEPTH*. REFER TO THE ANCHOR TYPE FOR THE DEFINITION OF NOMINAL EMBEDMENT DEPTH "hnom". IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO DETERMINE THE CORRECT ANCHOR LENGTH "La" FOR ORDER BASED ON THE SPECIFIED NOMINAL EMBEDMENT DEPTH, ATTACHMENT THICKNESS AND OTHER ANCHOR CHARACTERISTICS NOTED.
3. MINIMUM ANCHOR LENGTH "La,min" IS DETERMINED AS SHOWN FOR EACH ANCHOR. ORDER AND INSTALL AN ANCHOR LENGTH EQUAL TO OR GREATER THAN THIS VALUE. INSTALLED ANCHOR LENGTHS SHALL NOT HAVE NOMINAL EMBEDMENT DEPTHS THAT EXCEED THEIR CORRESPONDING MINIMUM CONCRETE THICKNESS LIMITS. REFER TO ANCHOR'S ICC-ES EVALUATION SERVICE REPORT (ESR).
4. REFER TO THE ANCHOR'S ICC-ES EVALUATION SERVICE REPORT (ESR) FOR DRILL BIT TYPE AND DIAMETER, AND DEPTH OF HOLE TO BE DRILLED IN THE CONCRETE.
5. FOLLOW THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII).
6. ALL DRILLED HOLES FOR ADHESIVE ANCHORS SHALL BE WITHIN SIX DEGREES OF PERPENDICULAR TO THE SURFACE OF THE CONCRETE MEMBER.

7 TYPICAL POST-INSTALLED ANCHOR INFORMATION

NO SCALE

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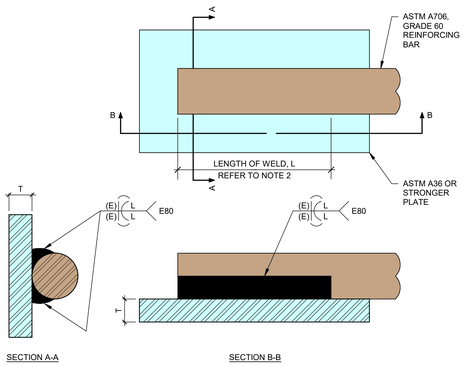
MINIMUM LENGTH OF WELDABLE REINFORCEMENT, L (INCHES) GRADE 60 REINFORCEMENT, E80 ELECTRODE					
BAR SIZE	PLATE THICKNESS, T				
	1/4"	5/16"	3/8"	7/16"	1/2"
#3	2"	2"	2"	2"	2"
#4	2 1/2"	2 1/2"	2 1/2"	2 1/2"	2 1/2"
#5	3"	3"	3"	3"	3"
#6	4"	3 1/2"	3 1/2"	3 1/2"	3 1/2"
#7	5 1/4"	4 1/4"	4 1/4"	4 1/4"	4 1/4"
#8	6 3/4"	5 1/2"	4 3/4"	4 3/4"	4 3/4"
#9	8 1/2"	7"	6"	5 1/4"	5 1/4"
#10	10 3/4"	8 3/4"	7 1/2"	6 1/2"	6"
#11	13"	10 1/2"	9"	7 3/4"	7"

NOTES:

1. E = 0.2db
2. WELD LENGTH IS CONSTRUCTED WELD LENGTH WHICH INCLUDES AN ALLOWANCE FOR WELD START-UP AND TERMINATION.



SECTION A-A SECTION B-B

5 TYPICAL DEVELOPMENT OF WELDABLE REINFORCEMENT

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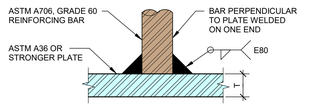

DEVELOPMENT OF WELDABLE REINFORCEMENT GRADE 60 REINFORCEMENT, E80 ELECTRODE		
BAR SIZE	NOMINAL WELD SIZE	MINIMUM PLATE THICKNESS
#3	3/16"	1/4"
#4	1/4"	1/4"
#5	5/16"	5/16"
#6	5/16"	7/16"
#7	3/8"	1/2"
#8	7/16"	9/16"
#9	1/2"	5/8"
#10	9/16"	11/16"
#11	5/8"	3/4"

3 TYPICAL DEVELOPMENT OF WELDABLE REINFORCEMENT

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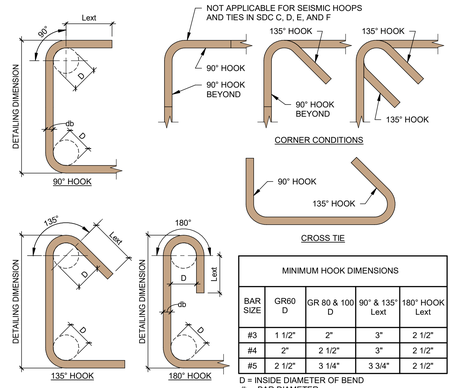

MINIMUM HOOK DIMENSIONS				
BAR SIZE	GR60 D	GR 80 & 100 D	90° & 135° Lext	180° HOOK Lext
#3	1 1/2"	2"	3"	2 1/2"
#4	2"	2 1/2"	3"	2 1/2"
#5	2 1/2"	3 1/4"	3 3/4"	2 1/2"

D = INSIDE DIAMETER OF BEND
db = BAR DIAMETER

2 TYPICAL STIRRUP AND TIE HOOKS

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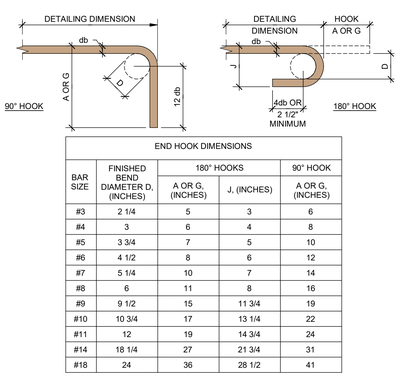

END HOOK DIMENSIONS					
BAR SIZE	FINISHED BEND DIAMETER D, (INCHES)	180° HOOKS		90° HOOK	
		A OR G, (INCHES)	J, (INCHES)	A OR G, (INCHES)	
#3	2 1/4	5	3	6	
#4	3	6	4	8	
#5	3 3/4	7	5	10	
#6	4 1/2	8	6	12	
#7	5 1/4	10	7	14	
#8	6	11	8	16	
#9	9 1/2	15	11 3/4	19	
#10	10 3/4	17	13 1/4	22	
#11	12	19	14 3/4	24	
#14	18 1/4	27	21 3/4	31	
#18	24	36	28 1/2	41	

D = INSIDE DIAMETER OF BEND

1 TYPICAL END HOOK TYPES

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EQUIPMENT PLANNER	IMEG CORP.
WAYFINDING	HDR ARCHITECTURE, INC.

SHEET REVIEWER

MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

CORY ANN DAYTON LICENSE No. 57441 STATE OF FLORIDA PROFESSIONAL ENGINEER

Cory Ann Dayton, State of Florida, Professional Engineer, License No. 57441. This item has been digitally signed and sealed by Cory Ann Dayton, PE on the date indicated. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.
Email: cdayton@walterpmoore.com

CORY ANN DAYTON, PE NO. 57441

Sheet Name

GENERAL REINFORCEMENT INFORMATION

Sheet Number

S-005

Project Status

CONSTRUCTION DOCUMENTS

3/4/2026 2:18:07 PM Autodesk Docs://10427132_Orlando_Health_Sunrise/S05-25024-01_OH Vero FSED - Structural - R2025.rvt

THRESHOLD INSPECTION PLAN

PART I - GENERAL

A. PURPOSE

1. THE PURPOSE OF THIS THRESHOLD INSPECTION PLAN, AS REQUIRED BY FLORIDA STATUTE CHAPTER 553 AND THE FLORIDA BUILDING CODE, IS TO PROVIDE SPECIFIC INSPECTION PROCEDURES AND SCHEDULES SO THAT THE BUILDING CAN BE ADEQUATELY INSPECTED FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS.

B. QUALIFICATIONS

1. ONLY ENGINEERS CERTIFIED BY THE STATE OF FLORIDA AS SPECIAL INSPECTORS OF THRESHOLD BUILDINGS, IN ACCORDANCE WITH FLORIDA STATUTE CHAPTER 471, CAN ASSUME THE RESPONSIBILITIES OF A SPECIAL INSPECTOR.

2. IN ACCORDANCE WITH FLORIDA STATUTE CHAPTER 553, THE ARCHITECT OR ENGINEER OF RECORD MAY ACT AS THE SPECIAL INSPECTOR PROVIDED SHE OR HE IS ON THE BOARD OF PROFESSIONAL ENGINEERS' OR THE BOARD OF ARCHITECTURE AND INTERIOR DESIGN'S LIST OF PERSONS QUALIFIED TO BE SPECIAL INSPECTORS.

3. THE SPECIAL INSPECTOR SHALL BE PERMITTED TO SEND AN AUTHORIZED REPRESENTATIVE TO THE JOB SITE TO PERFORM THE NECESSARY INSPECTIONS PROVIDED ALL REQUIRED WRITTEN REPORTS ARE PREPARED BY AND BEAR THE SEAL OF THE SPECIAL INSPECTOR.

4. THE SPECIAL INSPECTOR'S AUTHORIZED REPRESENTATIVE SHALL BE QUALIFIED BY EDUCATION OR LICENSURE TO PERFORM THE DUTIES ASSIGNED BY THE SPECIAL INSPECTOR. THE QUALIFICATIONS SHALL INCLUDE ONE OF THE FOLLOWING: LICENSURE AS A PROFESSIONAL ENGINEER OR ARCHITECT; GRADUATION FROM AN ENGINEERING EDUCATION PROGRAM IN CIVIL OR STRUCTURAL ENGINEERING; GRADUATION FROM AN ARCHITECTURAL EDUCATION PROGRAM; SUCCESSFUL COMPLETION OF THE NCEES FUNDAMENTALS EXAMINATION; LICENSURE AS A BUILDING INSPECTOR PER CHAPTER 468, F.S.; OR LICENSURE AS A GENERAL CONTRACTOR PER CHAPTER 489, F.S.

5. THE TERM "SPECIAL INSPECTOR" IN THE BALANCE OF THIS INSPECTION PLAN REFERS TO THE SPECIAL INSPECTOR AND/OR THE SPECIAL INSPECTOR'S AUTHORIZED REPRESENTATIVE.

C. SPECIAL INSPECTOR'S RESPONSIBILITIES

1. THE SPECIAL INSPECTOR MUST DEDICATE THEIR TIME ON-SITE TO THE INSPECTION REQUIREMENTS OF THIS INSPECTION PLAN. THEY SHALL COORDINATE WITH THE CONTRACTOR TO ALLOW ENOUGH TIME TO PERFORM THE INSPECTIONS AS OUTLINED IN THIS PLAN.

2. THE SPECIAL INSPECTOR SHALL CONTACT THE ENGINEER AT LEAST ONCE A WEEK TO DISCUSS DETAILS ON THE PROJECT AND TO CONFIRM THEY HAVE REVIEWED ALL APPROVED RFIS, SUPPLEMENTAL SKETCHES, ETC.

3. THE SPECIAL INSPECTOR SHALL COOPERATE WITH THE CONTRACTOR AND SHALL REFRAIN FROM DIRECTING WORK, AS THIS IS EXPRESSLY NOT PART OF THE THRESHOLD INSPECTION FUNCTION.

4. THE SPECIAL INSPECTOR MAY NOT SERVE AS A SURROGATE IN CARRYING OUT THE RESPONSIBILITIES OF THE BUILDING OFFICIAL, ARCHITECT, OR ENGINEER OF RECORD.

5. THE SPECIAL INSPECTOR SHALL NOT PERFORM OTHER CONSTRUCTION RELATED WORK ON THE PROJECT, INCLUDING MATERIALS TESTING SERVICES.

D. CONTRACTOR'S RESPONSIBILITIES

1. THE CONTRACTOR SHALL BE FAMILIAR WITH THE SCOPE OF THE INSPECTIONS THAT WILL BE PERFORMED AND ALSO THE REQUIREMENTS OF THE CONTRACTOR AS DESCRIBED HEREIN, PARTICULARLY REGARDING FORMWORK DESIGN AND INSPECTIONS.

2. THE CONTRACTOR SHALL COOPERATE WITH AND ASSIST THE SPECIAL INSPECTOR IN PERFORMING THEIR INSPECTION DUTIES AND SHALL PROVIDE FREE ACCESS TO THE PROJECT AT ALL TIMES.

3. THE CONTRACTOR SHALL COORDINATE THE SCHEDULE OF WORK TO ACCOMMODATE THE REQUIRED INSPECTIONS. THEY SHALL ADVISE THE SPECIAL INSPECTOR IN ADVANCE OF CONSTRUCTION SCHEDULES AND PLANNED OPERATIONS IN ORDER TO ASSURE TIMELY AND APPROPRIATE INSPECTION. A MINIMUM OF TWENTY-FOUR (24) HOURS NOTICE IS REQUIRED.

4. THESE INSPECTIONS SHALL NOT RELIEVE THE CONTRACTOR OF THEIR RESPONSIBILITIES TO CARRY OUT THEIR OWN QUALITY CONTROL INSPECTIONS AND TESTING. THE CONTRACTOR'S CONTRACTUAL OR STATUTORY OBLIGATIONS ARE NOT RELIEVED BY ANY ACTION OF THE SPECIAL INSPECTOR.

E. OWNER'S RESPONSIBILITIES

1. THE OWNER SHALL RETAIN A SPECIAL INSPECTOR TO PERFORM THE INSPECTION SERVICES AS DESCRIBED HEREIN.

2. THE OWNER SHALL PAY ALL COSTS OF EMPLOYING A SPECIAL INSPECTOR, BUT THE SPECIAL INSPECTOR SHALL BE RESPONSIBLE TO THE ENFORCEMENT AGENCY.

3. THE OWNER SHALL RETAIN A GEOTECHNICAL ENGINEER TO PERFORM ALL SOILS RELATED TESTS AND INSPECTIONS, INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
   a. SOIL EXCAVATION, FILL, AND COMPACTION.
   b. AUGERCAST PILE, DRILLED PIER, OR DRIVEN PILE INSTALLATION:
      1) MONITOR DRILLING.
      2) VERIFY DIMENSIONS (DIAMETER, LENGTH, EMBEDMENT DEPTH, ETC.)
      3) MONITOR CONCRETE PLACEMENT.
      4) MONITOR LOAD TESTS.

F. FREQUENCY OF INSPECTION

1. THE SPECIAL INSPECTOR SHALL PERFORM SITE VISITS AT A FREQUENCY DETERMINED BY HIM TO SATISFY HIMSELF THAT THE INSPECTIONS ARE BEING PERFORMED IN ACCORDANCE WITH THIS PLAN.

G. DISCREPANCIES AND DEVIATIONS

1. THE SPECIAL INSPECTOR SHALL IMMEDIATELY ALERT THE CONTRACTOR OF ALL DISCREPANCIES AND DEVIATIONS FROM THE CONTRACT DOCUMENTS.

2. THE CONTRACTOR SHALL, UPON BEING INFORMED BY THE SPECIAL INSPECTOR, IMMEDIATELY CAUSE TO ELIMINATE SUCH DISCREPANCIES AND DEVIATIONS.

PART II - REPORTING

A. WRITTEN REPORTS

1. THE SPECIAL INSPECTOR SHALL SUBMIT WRITTEN REPORTS TO THE ENFORCING AGENCY HAVING JURISDICTION FOR THE PROJECT EACH WEEK DURING CONSTRUCTION. COPIES OF THE REPORT SHALL BE SUBMITTED TO THE OWNER, ARCHITECT, ENGINEER OF RECORD AND CONTRACTOR.

2. THE REPORT SHALL BEAR THE SIGNATURE AND SEAL OF THE SPECIAL INSPECTOR.

3. THE REPORT SHALL INCLUDE:
   a. A DESCRIPTION OF CONSTRUCTION PROGRESS FOR THE WEEK.
   b. WORKING CONDITIONS INCLUDING WEATHER, TEMPERATURE, TIME OF DAY, TYPE AND LOCATION OF WORK BEING PERFORMED.
   c. CHANGES IN WORKING SEQUENCE OR MATERIALS AND ANY UNUSUAL CIRCUMSTANCES AFFECTING THE PERFORMANCE OF WORK.
   d. ANY DISCREPANCIES OR DEVIATIONS FROM THE CONTRACT DOCUMENTS AND WHETHER ANY MEASURES WERE TAKEN BY THE CONTRACTOR TO CORRECT THESE CONDITIONS.
   e. THE NON-CONFORMANCE AND CORRECTIONS LOG AS INDICATED BELOW.

B. NON-CONFORMANCE AND CORRECTIONS LOG

1. THE SPECIAL INSPECTOR SHALL KEEP AN EXCEPTIONS AND CORRECTIONS LOG FOR FOLLOW-UP.

2. THE LOG SHALL INCLUDE, FOR EACH NON-CONFORMANCE ITEM, THE DATE IT WAS NOTED, THE CORRECTIVE ACTION TAKEN, AND THE DATE IT WAS RECTIFIED.

3. THIS LOG SHALL BE REVIEWED ON A DAILY BASIS AND UPDATED AS EXCEPTIONS ARE RECTIFIED.

C. LETTER OF COMPLETION

1. IN ACCORDANCE WITH SECTION 553.79 (7)A OF THE FLORIDA STATUTES, THE SPECIAL INSPECTOR SHALL, UPON COMPLETION OF THE BUILDING AND PRIOR TO THE ISSUANCE OF A CERTIFICATE OF OCCUPANCY, FILE A SIGNED AND SEALED STATEMENT WITH THE ENFORCEMENT AGENCY IN SUBSTANTIALLY THE FOLLOWING FORM: TO THE BEST OF MY KNOWLEDGE AND BELIEF, THE CONSTRUCTION OF ALL STRUCTURAL LOAD-BEARING COMPONENTS DESCRIBED IN THE THRESHOLD INSPECTION PLAN COMPLIES WITH THE CONTRACT DOCUMENTS, AND THE SPECIALTY SHORING DESIGN PROFESSIONAL ENGINEER HAS ASCERTAINED THAT THE SHORING AND RESHORING CONFORMS WITH THE SHORING AND RESHORING PLANS SUBMITTED TO THE ENFORCEMENT AGENCY.

PART III - FOUNDATIONS

A. AUGERCAST PILES

1. THE SPECIAL INSPECTOR MUST BE PRESENT A MINIMUM OF 2 HOURS PER DAY TO BE GENERALLY FAMILIAR WITH THE PILE INSTALLATION THAT IS BEING CONTINUOUSLY MONITORED BY THE GEOTECHNICAL ENGINEER.

2. VERIFY AUGERCAST PILE DIAMETER.

3. VERIFY ANCHOR RODS AND/OR DOWELS ARE INSTALLED WITH THE PROPER LAP SPLICE LENGTHS AND IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

4. VERIFY PLACEMENT OF REINFORCEMENT AND OBSERVE CONCRETING OPERATIONS AS OUTLINED IN THE REINFORCED CONCRETE SECTION OF THIS INSPECTION PLAN.

B. PILE CAPS

1. THE SPECIAL INSPECTOR MUST BE PRESENT FULL TIME DURING THE ENTIRE PLACEMENT OF THE FIRST TWO MAJOR CAP CONCRETE POURS AND THEN MUST BE PRESENT AT THE START OF ALL OTHER CONCRETE POURS.

2. VERIFY CAP DIMENSIONS.

3. VERIFY ANCHOR RODS AND/OR DOWELS ARE INSTALLED WITH THE PROPER LAP SPLICE LENGTHS AND IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

4. VERIFY PLACEMENT OF REINFORCEMENT AND OBSERVE CONCRETING OPERATIONS AS OUTLINED IN THE REINFORCED CONCRETE SECTION OF THIS INSPECTION PLAN.

C. SHALLOW FOUNDATIONS

1. THE SPECIAL INSPECTOR MUST BE PRESENT FULL TIME DURING THE ENTIRE PLACEMENT OF THE FIRST TWO MAJOR FOUNDATION CONCRETE POURS AND THEN MUST BE PRESENT AT THE START OF ALL OTHER CONCRETE POURS.

2. OBSERVE THAT THE OWNER'S GEOTECHNICAL ENGINEER IS MONITORING THE SUB-SURFACE DENSIFICATION.

3. OBSERVE THAT FORMS ARE PLUMB AND STRAIGHT, BRACED AGAINST MOVEMENT, AND LUBRICATED FOR REMOVAL.

4. VERIFY FOUNDATION DIMENSIONS.

5. VERIFY ANCHOR RODS AND/OR DOWELS ARE INSTALLED WITH THE PROPER LAP SPLICE LENGTHS AND IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

6. VERIFY PLACEMENT OF REINFORCEMENT AND OBSERVE CONCRETING OPERATIONS AS OUTLINED IN THE REINFORCED CONCRETE SECTION OF THIS INSPECTION PLAN.

D. FOUNDATION WALLS AND PITS

1. THE SPECIAL INSPECTOR MUST BE PRESENT FULL TIME DURING THE ENTIRE PLACEMENT OF THE FIRST TWO MAJOR WALL OR PIT CONCRETE POURS AND THEN MUST BE PRESENT AT THE START OF ALL OTHER CONCRETE POURS.

2. OBSERVE THAT FORMS ARE PLUMB AND STRAIGHT, BRACED AGAINST MOVEMENT, AND LUBRICATED FOR REMOVAL.

3. VERIFY FOUNDATION WALL AND PIT DIMENSIONS.

4. VERIFY PLACEMENT OF REINFORCEMENT AND OBSERVE CONCRETING OPERATIONS AS OUTLINED IN THE REINFORCED CONCRETE SECTION OF THIS INSPECTION PLAN.

5. VERIFY THAT WALLS ARE NOT BACKFILLED UNTIL FLOOR CONSTRUCTION AT TOP OF WALL IS COMPLETE OR TEMPORARY BRACING IS PROVIDED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

E. SLABS-ON-GRADE

1. THE SPECIAL INSPECTOR MUST BE PRESENT FULL TIME DURING THE ENTIRE PLACEMENT OF THE FIRST TWO MAJOR SLAB-ON-GRADE CONCRETE POURS AND THEN MUST BE PRESENT AT THE START OF ALL OTHER CONCRETE POURS.

2. OBSERVE SUBGRADE PREPARATION INCLUDING BACKFILLING, COMPACTION, AND PERFORMANCE OF COMPACTION TESTS BY THE TESTING LABORATORY PRIOR TO CONCRETE PLACEMENT.

3. NOTE ANY ALTERATION AND SUBSEQUENT REPLACEMENT OF SUBGRADE MATERIALS REQUIRED BY OTHER TRADES.

4. VERIFY THAT THE MOISTURE RETARDER OR VAPOR BARRIER IS PROVIDED, IS LAPPED PROPERLY, AND IS NOT TORN OR PUNCTURED.

5. OBSERVE THAT FORMWORK AT TURNDOWNS AND SLAB EDGES IS PLUMB AND STRAIGHT, BRACED AGAINST MOVEMENT AND LUBRICATED FOR REMOVAL.

6. OBSERVE PLACEMENT OF SCREEDS TO OBTAIN PROPER LEVEL AND THICKNESS OF SLABS. OBSERVE LOCATION OF SLAB DEPRESSIONS AND STEPS IN SLAB WHILE MAINTAINING REQUIRED SLAB THICKNESS.

7. VERIFY THE POUR AREA IS FREE OF STANDING WATER OR OTHER DEBRIS.

8. VERIFY PLACEMENT OF REINFORCEMENT AND OBSERVE CONCRETING OPERATIONS AS OUTLINED IN THE REINFORCED CONCRETE SECTION OF THIS INSPECTION PLAN.

9. CHECK THAT THE LOCATION AND TYPE OF SLAB CONTROL JOINTS AND CONSTRUCTION JOINTS CONFORM TO THE CONTRACT DOCUMENTS.

10. VERIFY THAT SAWCUT CONTROL JOINTS ON SLABS-ON-GRADE ARE CUT WITHIN 12 HOURS OF PLACEMENT.

PART IV - REINFORCED CONCRETE

A. FORMWORK, SHORING, RESHORING, AND BACKSHORING

1. FOR FORMWORK, SHORING, RESHORING, AND BACKSHORING OBTAIN DETAILED DRAWINGS WHICH SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA.

2. REVIEW THE GENERAL ARRANGEMENT OF FORMS, SHORES, RESHORES, AND BACKSHORES FOR COMPLIANCE WITH THE SHOP DRAWINGS.

3. DETERMINE THAT A PROFESSIONAL ENGINEER, REGISTERED IN THE STATE OF FLORIDA, WHO SPECIALIZES IN SHORING DESIGN HAS INSPECTED THE SHORING AND RESHORING OR BACKSHORING FOR CONFORMANCE WITH THE SHORING AND RESHORING PLANS SUBMITTED TO THE ENFORCING AGENCY AND THAT THEY HAVE SUBMITTED A REPORT OF THE INSPECTION BEFORE EACH POUR.

4. VERIFY THAT FORMS ARE PLUMB AND STRAIGHT, BRACED AGAINST MOVEMENT, AND LUBRICATED FOR REMOVAL.

5. REVIEW THE TIME SEQUENCE OF FORM REMOVAL AND RESHORING/BACKSHORING PROCEDURES FOR COMPLIANCE WITH THE FORMWORK, SHORING, RESHORING, AND BACKSHORING SHOP DRAWINGS.

6. REVIEW RESULTS OF FIELD CURED CONCRETE TEST CYLINDERS WITH THE TESTING LABORATORY TO CHECK TIMING OF STRIPPING OF FORMS.

7. OBSERVE THAT THE CONTRACTOR IS MONITORING FORMS DURING CONCRETE PLACEMENT FOR ABNORMAL DEFLECTIONS OR SIGNS OF IMMINENT FAILURE WHICH COULD BE CORRECTED.

B. REINFORCING STEEL

1. VERIFY THAT REINFORCEMENT SURFACES ARE FREE OF EXCESS RUST OR OTHER COATINGS THAT MAY ADVERSELY AFFECT BONDING CAPACITY. IF OILING OF FORMS IS REQUIRED, VERIFY THAT IT IS APPLIED BEFORE REINFORCING IS PLACED.

2. VERIFY ALL REINFORCING BARS FOR COMPLIANCE WITH CONTRACT DOCUMENTS AND APPROVED SHOP DRAWINGS AS FOLLOWS:
   a. MATERIAL GRADE,
   b. REINFORCEMENT SIZE,
   c. QUANTITY, SPACING, AND LAYERING,
   d. PROPER HOOK TYPE AND LOCATION.
   e. SPLICE LOCATIONS AND REQUIRED LENGTH OF LAP.
   f. VERIFY MINIMUM CLEARANCE AND COVER REQUIREMENTS FROM CONCRETE SURFACES.
   g. SUFFICIENT SPACING BETWEEN REINFORCEMENT FOR CONCRETE PLACEMENT.

3. VERIFY THAT UNSCHEDULED REINFORCING BARS SHOWN ON PLAN, IN DETAILS, OR SPECIFIED IN NOTES ARE PROVIDED AND ARE IN COMPLIANCE WITH CONTRACT DOCUMENTS AND APPROVED SHOP DRAWINGS.

PART IV - REINFORCED CONCRETE (CONTINUED)

4. MECHANICAL SPLICES:
   a. PROVIDE VISUAL INSPECTION OF 100% OF THE MECHANICAL SPLICES (TENSION AND/OR COMPRESSION) ON THE PROJECT.
   b. VERIFY COMPLIANCE WITH SPECIFICATIONS AND CONFORMANCE WITH THE MANUFACTURER'S RECOMMENDATIONS FOR INSTALLATION.
   c. OBSERVE THAT THE MANUFACTURER IS PRESENT FOR THE FIRST INSTALLATION OF EACH TYPE OF SPLICE ON THE PROJECT.

5. VERIFY THAT WELDED WIRE REINFORCEMENT IS COMPOSED OF FLAT SHEETS, HAS PROPER WIRE GAGE AND SPACING, IS PROPERLY SUPPORTED, AND IS PROPERLY LAPPED.

6. INSPECT HEADED SHEAR STUD REINFORCEMENT TO ENSURE THAT IT CONFORMS TO THE STRUCTURAL DETAILS.

7. SUPPORT OF REINFORCEMENT:
   a. REVIEW TYPE AND SPACING OF SUPPORT FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS.
   b. VERIFY THAT REINFORCING IS ADEQUATELY SUPPORTED TO RESIST DISPLACEMENT OR SHIFTING DURING CONCRETE PLACEMENT.

8. OBSERVE WELDING OF REINFORCEMENT HAS BEEN DONE ACCORDING TO AWS REQUIREMENTS AND THAT IT IS INSPECTED BY THE TESTING LABORATORY.

9. REPORT ANY NOTED CONFLICTS OR DEVIATIONS FROM THE CONTRACT DOCUMENTS BEFORE CONCRETE IS POURED SO THAT CORRECTIONS MAY BE MADE.

C. CONCRETE PLACEMENT

1. THE SPECIAL INSPECTOR SHALL BE PRESENT DURING THE PLACEMENT OF CONCRETE AT A FREQUENCY AS INDICATED IN OTHER SECTIONS OF THIS INPSECTION PLAN.

2. VERIFY THE FOLLOWING WITH REGARD TO THE TESTING LABORATORY:
   a. THE TESTING LABORATORY SUPPORT STAFF HAS BEEN INFORMED AND WILL BE AVAILABLE TO MAKE TESTS AS REQUIRED.
   b. SLUMP IS MEASURED AT THE POINT OF DISCHARGE FOR EACH DAY AND CLASS OF CONCRETE.
   c. CONCRETE TEST CYLINDERS HAVE BEEN TAKEN IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

3. VERIFY THAT DEBRIS AND FOREIGN MATERIALS HAVE BEEN REMOVED BEFORE CONCRETE IS PLACED.

4. PERIODICALLY INSPECT CONCRETE UPON ARRIVAL TO VERIFY THE FOLLOWING:
   a. PROPER CONCRETE MIX NUMBER, TYPE OF CONCRETE, AND CONCRETE STRENGTH FOR THE PLACEMENT LOCATION.
   b. REPORT CONCRETE NOT MEETING THE SPECIFIED REQUIREMENTS AND IMMEDIATELY NOTIFY THE CONTRACTOR, BATCH PLANT INSPECTOR, ARCHITECT, ENGINEER OF RECORD, AND OWNER.

5. VERIFY THAT THE CONTRACTOR IS FOLLOWING APPROPRIATE CONCRETING PRACTICES AT THE POINT OF PLACEMENT AS DEFINED BELOW:
   a. VERIFY THAT THE CONCRETE IS NOT OVER 90 MINUTES OLD AT THE TIME OF PLACEMENT.
   b. VERIFY THAT HOT-WEATHER OR COLD-WEATHER TECHNIQUES ARE BEING APPLIED AS REQUIRED.
   c. VERIFY THAT CONCRETE BEING DEPOSITED IS UNIFORM, THAT THE VERTICAL DROP DOES NOT EXCEED SIX FEET, AND THAT IT IS NOT PERMITTED TO DROP FREELY OVER REINFORCEMENT CAUSING SEGREGATION.
   d. VERIFY THAT THE CONCRETE IS PROPERLY VIBRATED.
   e. VERIFY THAT EMBEDDED ITEMS AND REINFORCING STEEL ARE NOT ADVERSELY ALTERED DURING PLACEMENT. NOTE IF ANYTHING WAS DISPLACED OR OTHERWISE ALTERED DURING PLACEMENT.
   f. VERIFY THAT THERE ARE NO COLD JOINTS WITHIN THE AREA OF THE POUR.

6. VERIFY THAT THE CURING PROCESS IS AS SPECIFIED IN THE CONTRACT DOCUMENTS AND THAT ANY CURING COMPOUND USED IS APPLIED IN ACCORDANCE WITH MANUFACTURER'S PRINTED APPLICATION INSTRUCTIONS.

D. COLUMNS, WALLS, AND SHEAR WALLS

1. THE SPECIAL INSPECTOR SHALL PERIODICALLY INSPECT 25% OF THE CONCRETE PLACEMENT AS IT OCCURS.

2. CHECK ALL REINFORCEMENT BEFORE VERTICAL FORMS ARE PLACED AS OUTLINED IN THE REINFORCING STEEL SECTION.

3. OBSERVE THAT FORMS ARE PLUMB AND STRAIGHT, BRACED AGAINST MOVEMENT, LUBRICATED FOR REMOVAL, AND CONFORM TO APPROVED SHOP DRAWINGS.

4. VERIFY PROPER DIMENSIONS AND ORIENTATION.

5. FOR FLAT SLAB CONSTRUCTION, VERIFY THAT TOP OF COLUMN OR WALL ELEVATION IS SET 1/2 INCH BELOW THE FUTURE SLAB SOFFIT, OR AS SHOWN ON THE CONTRACT DOCUMENTS.

6. VERIFY WALL OPENINGS AND SLEEVES AS FOLLOWS:
   a. CORRECT SIZE AND LOCATION AS SHOWN ON THE CONTRACT DOCUMENTS.
   b. CHECK PLACEMENT OF ADDITIONAL REINFORCEMENT AROUND OPENINGS.
   c. REPORT ALL WALL OPENINGS LARGER THAN 12" THAT ARE NOT SHOWN ON THE CONTRACT DOCUMENTS.

7. VERIFY THAT ALL DEBRIS IS REMOVED FROM WITHIN THE FORMS PRIOR TO CONCRETE PLACEMENT.

E. HORIZONTAL FRAMING

1. THE SPECIAL INSPECTOR SHALL BE PRESENT DURING THE PLACEMENT OF ALL CONCRETE ON HORIZONTAL FORMWORK. CHECK ALL REINFORCEMENT AS OUTLINED IN THE REINFORCING STEEL SECTION.

2. VERIFY THE PROPER DIMENSIONS OF GIRDERS, BEAMS AND JOISTS.

3. VERIFY THAT THE SLAB THICKNESS AND TOP OF SLAB ELEVATION IS CORRECT.

4. VERIFY OPENINGS AND SLEEVES AS FOLLOWS:
   a. CORRECT SIZE AND LOCATION AS SHOWN ON THE CONTRACT DOCUMENTS.
   b. CHECK PLACEMENT OF ADDITIONAL REINFORCEMENT AROUND OPENINGS.
   c. REPORT ALL SLAB OPENINGS LARGER THAN 12" THAT ARE NOT SHOWN ON THE STRUCTURAL CONTRACT DOCUMENTS.
   d. NO SLEEVES OR OPENINGS WILL BE PERMITTED IN BEAMS WITHOUT THE PRIOR APPROVAL OF THE ENGINEER OF RECORD.

5. CHECK THAT ALL DEBRIS AND FOREIGN MATERIAL HAS BEEN REMOVED BEFORE CONCRETE IS PLACED.

F. CONSTRUCTION JOINTS

1. VERIFY THE LOCATION OF VERTICAL AND HORIZONTAL CONSTRUCTION JOINTS FOR COMPLIANCE WITH THE CONSTRUCTION JOINT LOCATION PLAN SUBMITTED BY THE CONTRACTOR TO THE ARCHITECT AND ENGINEER OF RECORD.

2. VERIFY THAT REINFORCEMENT, DOWELS, KEYS, AND BULKHEADS AT CONSTRUCTION JOINTS ARE IN CONFORMANCE WITH THE CONTRACT DOCUMENTS.

G. EMBEDDED ITEMS

1. VERIFY THAT CONDUITS PLACED IN THE CONCRETE ARE REASONABLY SPACED TO ENSURE STRUCTURAL INTEGRITY AND COMPLY WITH THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. VERIFY CONDUIT PLACEMENT ALLOWS FOR PROPER CONCRETE COVER OF THE REINFORCEMENT.

2. REVIEW LOAD CARRYING EMBEDDED ITEMS AS PLACED FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS. RELOCATION OF EMBEDDED ITEMS IN CONFLICT WITH REINFORCING SHALL NOT BE PERMITTED WITHOUT THE PRIOR APPROVAL OF THE ENGINEER OF RECORD.

PART V - ARCHITECTURAL PRECAST CONCRETE

A. GENERAL

1. OBTAIN A COPY OF THE APPROVED PRECAST SHOP DRAWINGS THAT ARE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA.

2. VISUALLY INSPECT PRECAST FOR DAMAGE SUSTAINED DURING SHIPPING.

3. VERIFY THAT THE PRECAST ERECTOR IS MAINTAINING THE PROPER JOINT TOLERANCES.

B. CONNECTIONS

1. VERIFY PROPER INSTALLATION OF CONNECTIONS BETWEEN PRECAST MEMBERS AND SUPPORT STRUCTURE AND REPORT ANY DEVIATIONS FROM THE SIGNED, SEALED, AND APPROVED SHOP DRAWINGS.

2. VISUALLY VERIFY BOLTS AND WELDS FOR COMPLIANCE WITH THE SIGNED, SEALED, AND APPROVED SHOP DRAWINGS AND ERECTION DRAWINGS.

3. FOR CONNECTIONS REQUIRED TO ALLOW MOVEMENT BETWEEN THE PRECAST AND SUPPORTING MEMBERS, VERIFY THAT THE DESIGN MOVEMENT CAN OCCUR.

4. WELD QUALITY SHALL BE REVIEWED AND APPROVED BY THE TESTING LABORATORY.

PART VI - CONCRETE MASONRY

A. GENERAL

1. VERIFY THAT MATERIALS ARE STORED PROPERLY BEFORE PLACEMENT IN THE STRUCTURE.

2. VERIFY WALL LOCATIONS AND THICKNESSES.

3. VERIFY THE PROPER INSTALLATION OF HORIZONTAL JOINT REINFORCEMENT.

4. VERIFY THE PROPER INSTALLATION OF CONTROL JOINTS, TYPE AND LOCATION.

5. VERIFY THE PROPER INSTALLATION OF LINTELS, SILLS, AND DOOR OR WINDOW JAMBS.

6. VERIFY THAT THE MASONRY IS PROPERLY CONNECTED TO THE SUPPORTING STRUCTURE(S).

B. MORTAR AND GROUT

1. VERIFY THAT MORTAR AND GROUT MATERIALS COMPLY WITH THE CONTRACT DOCUMENTS AND APPROVED SUBMITTALS.

2. OBSERVE THE MORTAR IS MIXED AND PLACED ACCORDING TO THE MANUFACTURER'S INSTRUCTIONS.

3. GROUT
   a. BAG MIX: OBSERVE THAT THE GROUT IS MIXED ACCORDING TO THE MANUFACTURER'S INSTRUCTIONS.
   b. READY MIX: VERIFY THE MIX NUMBER AND GROUT STRENGTH.
   c. PRIOR TO ANY GROUTING PROCEDURE, THE GROUT SPACE SHALL BE INSPECTED TO VERIFY THAT IT IS CLEAN AND THAT CLEANOUTS, IF REQUIRED, ARE IN PLACE AND CONFORM TO REQUIREMENTS OF THE CONTRACT DOCUMENTS.
   d. VERIFY THE PROPER GROUT PLACEMENT AND CONSOLIDATION.
   e. VERIFY THAT GROUT TESTING IS IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

C. REINFORCING STEEL

1. VERIFY THAT THE SIZE, SPACING, LOCATION, HOOKS, AND LAP SPLICE LENGTHS OF VERTICAL AND HORIZONTAL REINFORCEMENT ARE INSTALLED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

2. NOTIFY THE ENGINEER OF RECORD WHEN PLACEMENT OF CONDUIT OR PIPING INTERFERES WITH CELL GROUTING AND/OR VERTICAL REINFORCING STEEL.

PART VII - STRUCTURAL STEEL

A. FRAMING

1. VISUALLY INSPECT STEEL AS IT IS RECEIVED FOR POSSIBLE DAMAGE IN SHIPPING, WORKMANSHIP, AND PIECE MARKING.

2. VERIFY THAT STEEL MEMBER SIZES AND STEEL GRADE CONFORM TO THE CONTRACT DOCUMENTS AND APPROVED SHOP DRAWINGS.

3. CHECK THE INSTALLATION OF ANCHOR RODS FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS AND APPROVED SHOP DRAWINGS FOR THE FOLLOWING:
   a. LOCATION.
   b. SIZE, QUANTITY, AND MATERIAL GRADE.
   c. EMBEDMENT LENGTH INTO THE FOUNDATION OR SUPPORTING MEMBER.

4. CHECK THE INSTALLATION OF BASE PLATES FOR PROPER LEVELING. VERIFY THE PROPER GROUT TYPE AND INSTALLATION PROCEDURES ARE FOLLOWED.

5. INSPECT 100% OF THE COLUMN COMPRESSION AND BASE JOINTS FOR VERIFICATION THAT GAPS IN CONTACT BEARING DO NOT EXCEED 1/16 INCH.
   a. GAPS GREATER THAN 1/16 INCH BUT LESS THAN 1/4 INCH SHALL BE REPORTED TO THE ENGINEER OF RECORD FOR ASSESSMENT.
   b. ALL GAPS GREATER THAN 1/4 INCH SHALL BE SHIMMED WITH NON-TAPERED MILD STEEL SHIMS.

6. REVIEW THAT THE CAMBER OF STEEL BEAMS COMPLIES WITH THE CONTRACT DOCUMENTS AND APPROVED SHOP DRAWINGS.

7. PERIODICALLY INSPECT THE STEEL FRAME FOR SUCH ITEMS AS BRACING, STIFFENERS, MEMBER LOCATIONS, AND JOINT DETAILS AT EACH CONNECTION FOR COMPLIANCE WITH CONTRACT DOCUMENTS AND APPROVED SHOP DRAWINGS.

8. ENDEAVOR TO GUARD THE OWNER AGAINST THE CONTRACTOR CUTTING, GRINDING, REAMING, OR MAKING ANY OTHER FIELD MODIFICATION TO STRUCTURAL STEEL WITHOUT THE PRIOR APPROVAL OF THE ENGINEER OF RECORD. REPORT ANY NOTED UNAUTHORIZED MODIFICATIONS TO THE ENGINEER OF RECORD.

B. CONNECTIONS

1. VISUALLY CHECK THAT ALL CONNECTIONS ARE COMPLETED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS AND THE APPROVED SHOP DRAWINGS.

2. WELDED CONNECTIONS
   a. OBSERVE THAT PROPER WELDING PROCEDURES ARE BEING IMPLEMENTED.
   b. VERIFY THAT WELDS AR INSPECTED AND TESTED BY THE TESTING LABORATORY.

3. BOLTED CONNECTIONS
   a. VERIFY BOLT AND WASHER SIZES AND TYPES THAT ARE BEING INSTALLED.
   b. VERIFY THE TIGHTENING METHOD FOR HIGH STRENGTH BOLTS AND THAT IMPACT WRENCHES ARE BEING PROPERLY CALIBRATED.
   c. VISUALLY CHECK THAT BOLTS ARE BEING TIGHTENED PROPERLY.

C. ERECTION

1. OBTAIN A COPY OF THE APPROVED ERECTION PLAN THAT HAS BEEN SIGNED AND SEALED BY A PROFESSIONAL ENGINEER IN THE STATE OF FLORIDA.

2. VERIFY THAT STEEL IS ERECTED IN ACCORDANCE WITH THE APPROVED ERECTION PLAN.

3. VERIFY THAT TEMPORARY BRACING IS INSTALLED AND IS IN CONFORMANCE WITH THE APPROVED ERECTION DRAWINGS.

4. VISUALLY CHECK PLUMBNESS AND FRAME ALIGNMENT AS ERECTION PROCEEDS.

5. VERIFY THAT SURVEYS ARE OCCURRING AS REQUIRED BY THE CONTRACT DOCUMENTS TO CHECK PLUMBNESS AND FRAME ALIGNMENT AS ERECTION PROGRESSES. REVIEW THE SUBMITTED SURVEY REPORT.

D. GRATING

1. VISUALLY INSPECT THE GRATING FOR DAMAGE DURING SHIPPING.

2. VERIFY THAT THE GRATING DEPTH, TYPE OR PROPERTIES, AND FINISH COMPLY WITH THE CONTRACT DOCUMENTS AND/OR APPROVED SHOP DRAWINGS.

3. VERIFY ALL GRATING ATTACHMENT TO THE SUPPORTING CONCRETE, STEEL, AND/OR MASONRY AS SPECIFIED IN THE CONTRACT DOCUMENTS AND/OR APPROVED SHOP DRAWINGS.

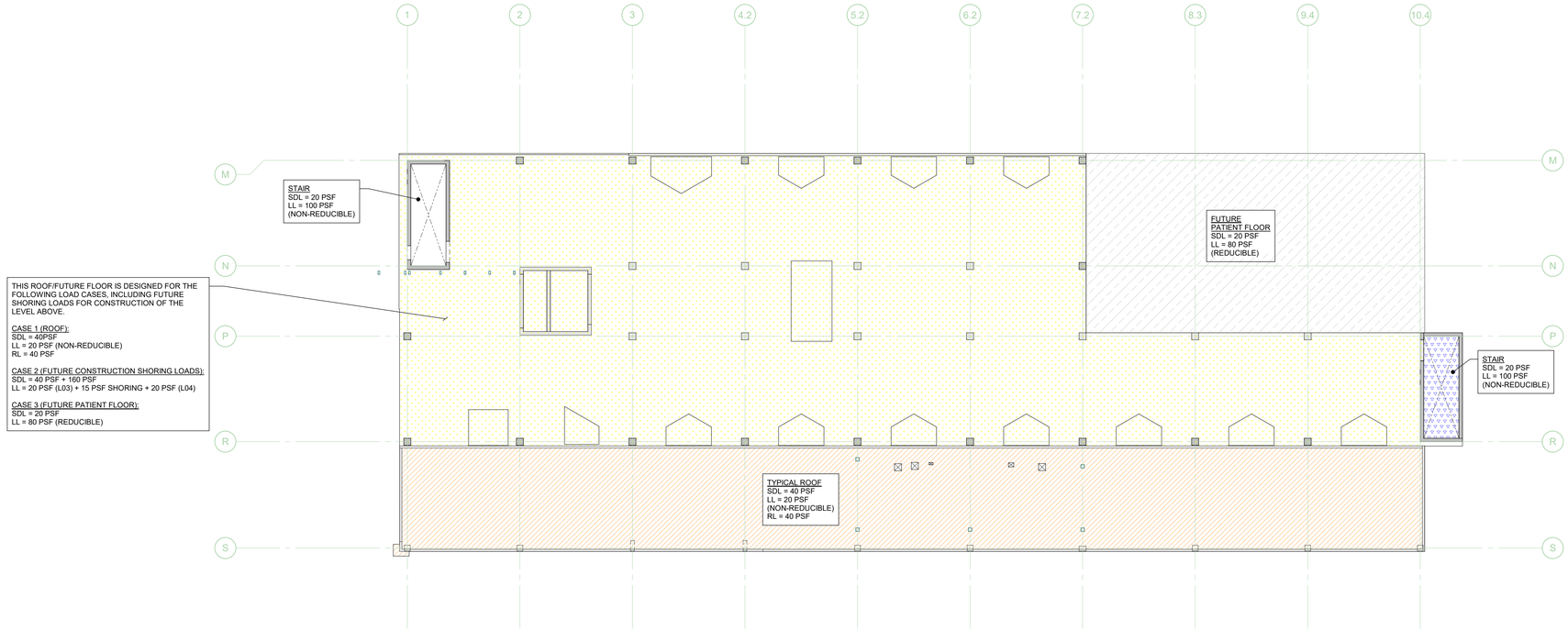
PART VIII - STEEL DECK

A. ROOF DECK

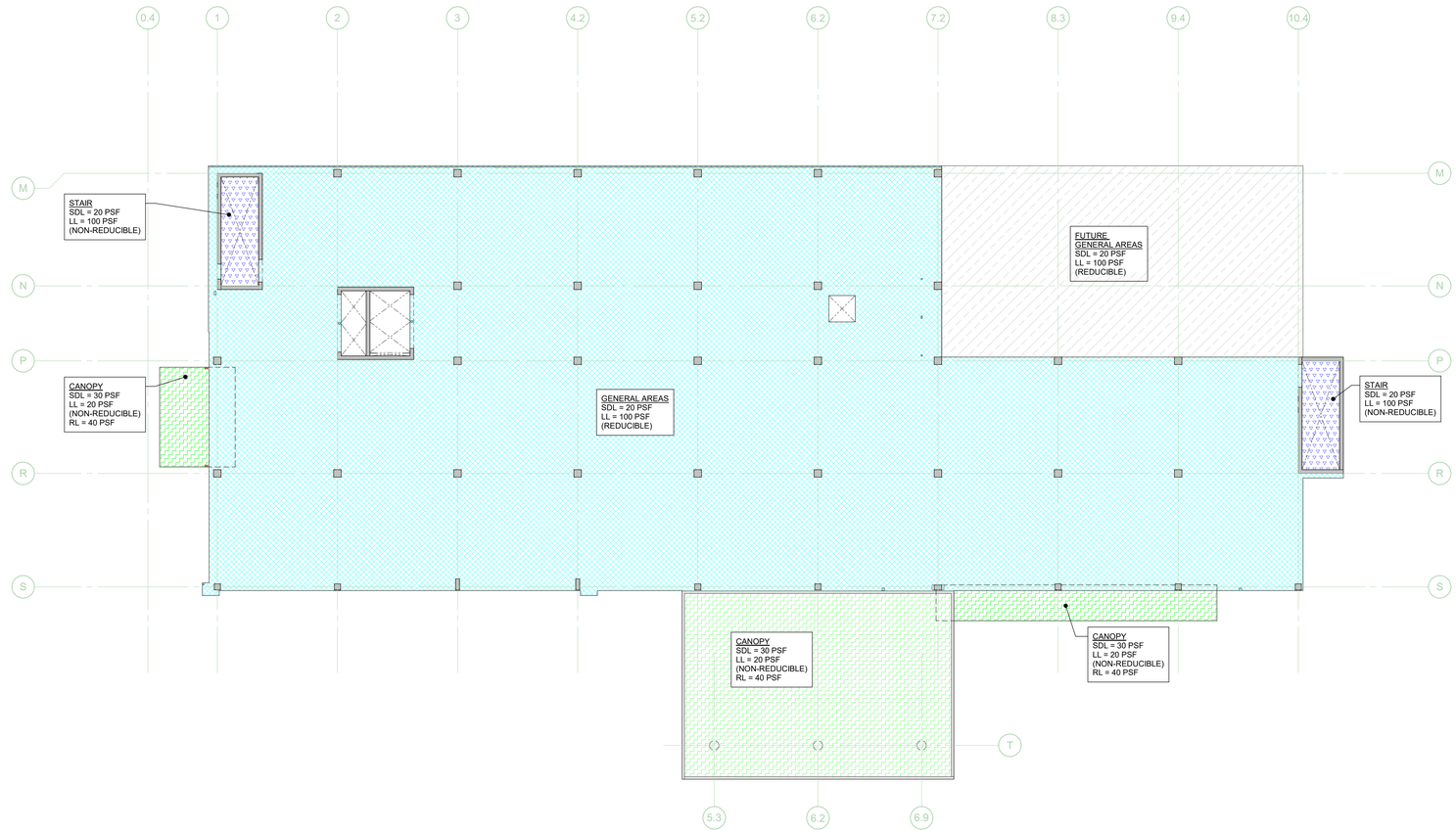
1. VISUALLY INSPECT THE DECK FOR DAMAGE DURING SHIPPING.

2. VERIFY THAT THE DECK DEPTH, GAGE, TYPE OR PROPERTIES, AND FINISH COMPLY WITH THE CONTRACT DOCUMENTS.

3. VERIFY THAT THE DECK ATTACHMENT TO THE SUPPORTING STEEL IS AS SPECIFIED IN THE CONTRACT DO



2 LOAD KEY PLAN - ROOF (FUTURE LEVEL 03)
1/16" = 1'-0"



1 LOAD KEY PLAN - LEVEL 02
1/16" = 1'-0"

0 1/2" 1" 2"

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WPM Project No. S05-25024-00

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FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
TO THE BEST OF THE ARCHITECTS OR ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH THIS SECTION AND CHAPTER 633, FLORIDA STATUES.

ORLANDO HEALTH MELBOURNE NEW FREESTANDING EMERGENCY CARE FACILITY

LAKE ANDREW DR & WILLET PLACE
MELBOURNE, FL 32940

ORLANDO HEALTH®

PROJECT MANAGER	HDR ARCHITECTURE, INC.
PROJECT DESIGNER	HDR ARCHITECTURE, INC.
PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
LANDSCAPE ARCHITECT	MCKINTOSH ASSOCIATES, an LJA COMPANY
CIVIL ENGINEER	BSE CONSULTANTS, an LJA COMPANY
STRUCTURAL ENGINEER	WALTER P. MOORE & ASSOCIATES, INC.
MECHANICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
ELECTRICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
PLUMBING ENGINEER	I.C. TOMASSON ASSOCIATES, INC
INTERIOR DESIGNER	HDR ARCHITECTURE, INC.
EQUIPMENT PLANNER	IMEG CORP.
WAYFINDING	HDR ARCHITECTURE, INC.
SHEET REVIEWER	

MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

CORY ANN DAYTON LICENSE No. 57441 STATE OF FLORIDA PROFESSIONAL ENGINEER

Cory Ann Dayton, State of Florida, Professional Engineer, License No. 57441. This item has been digitally signed and sealed by Cory Ann Dayton, PE on the date indicated. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.
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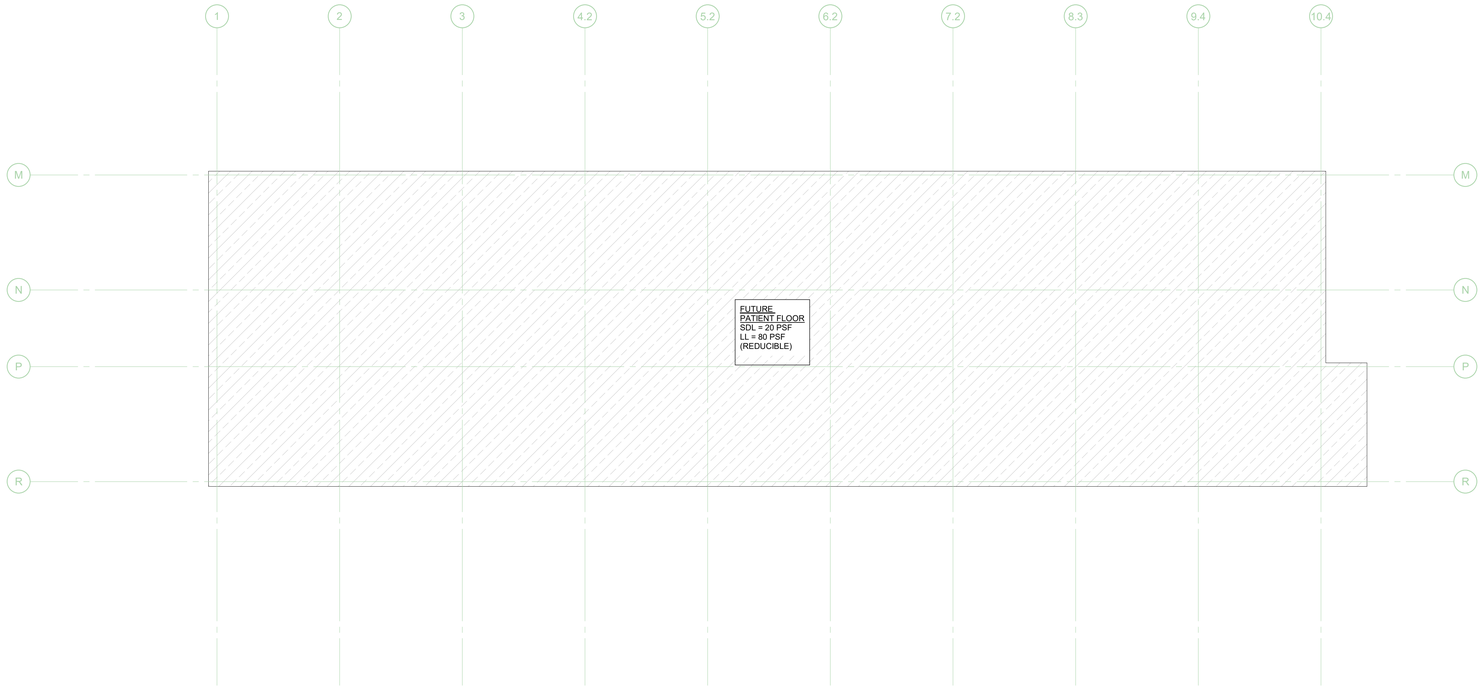
CORY ANN DAYTON, PE NO. 57441

Sheet Name
LOAD KEY PLANS

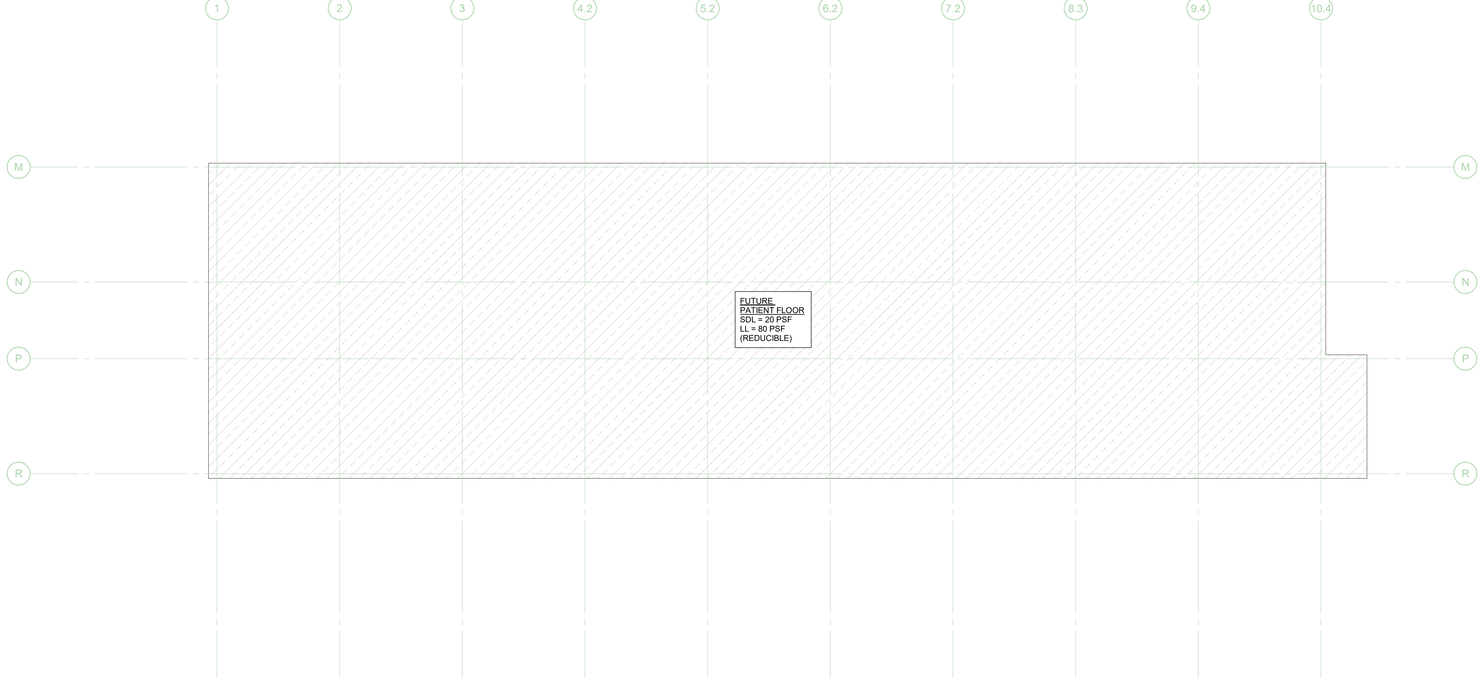
Sheet Number
S-021

Project Status
CONSTRUCTION DOCUMENTS



2 LOAD KEY PLAN - FUTURE LEVEL 05

1/16" = 1'-0"



1 LOAD KEY PLAN - FUTURE LEVEL 04

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CORY ANN DAYTON, PE NO. 57441

Sheet Name

LOAD KEY PLANS

Sheet Number

S-022

Project Status

CONSTRUCTION DOCUMENTS

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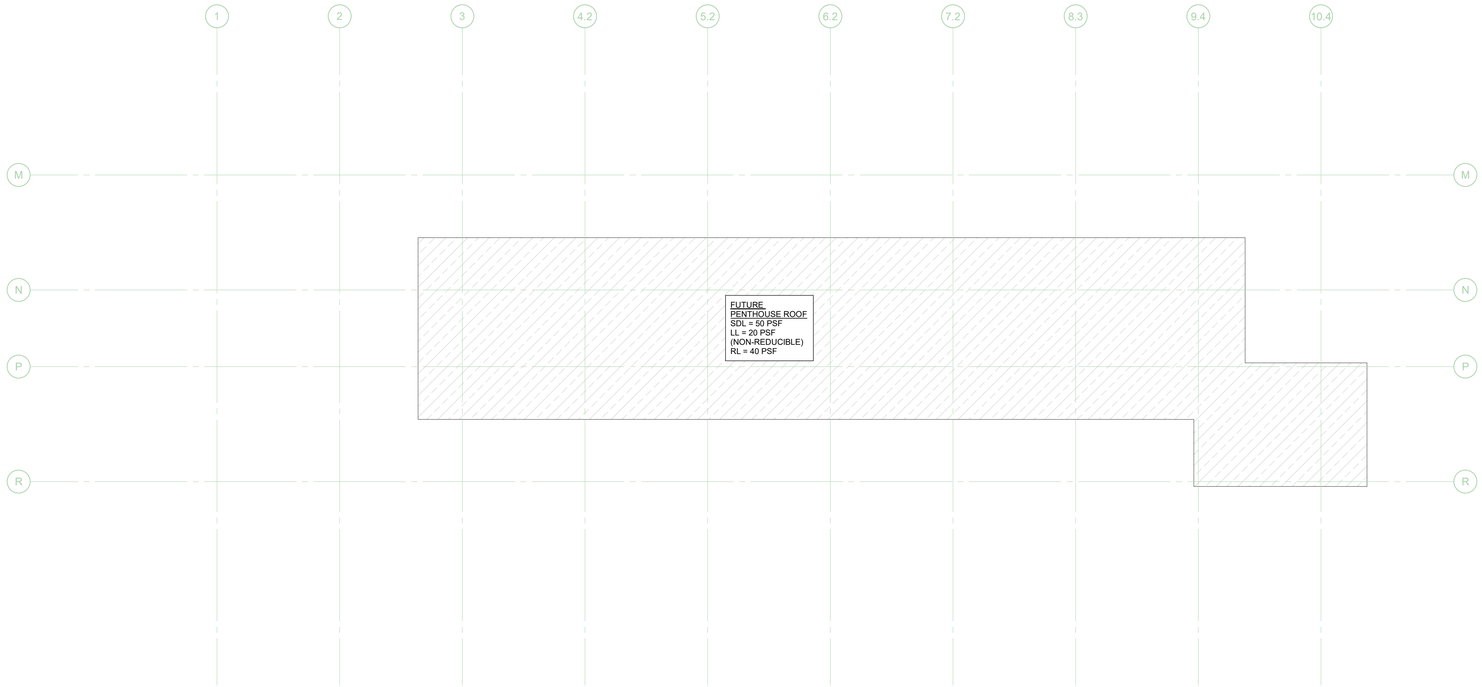
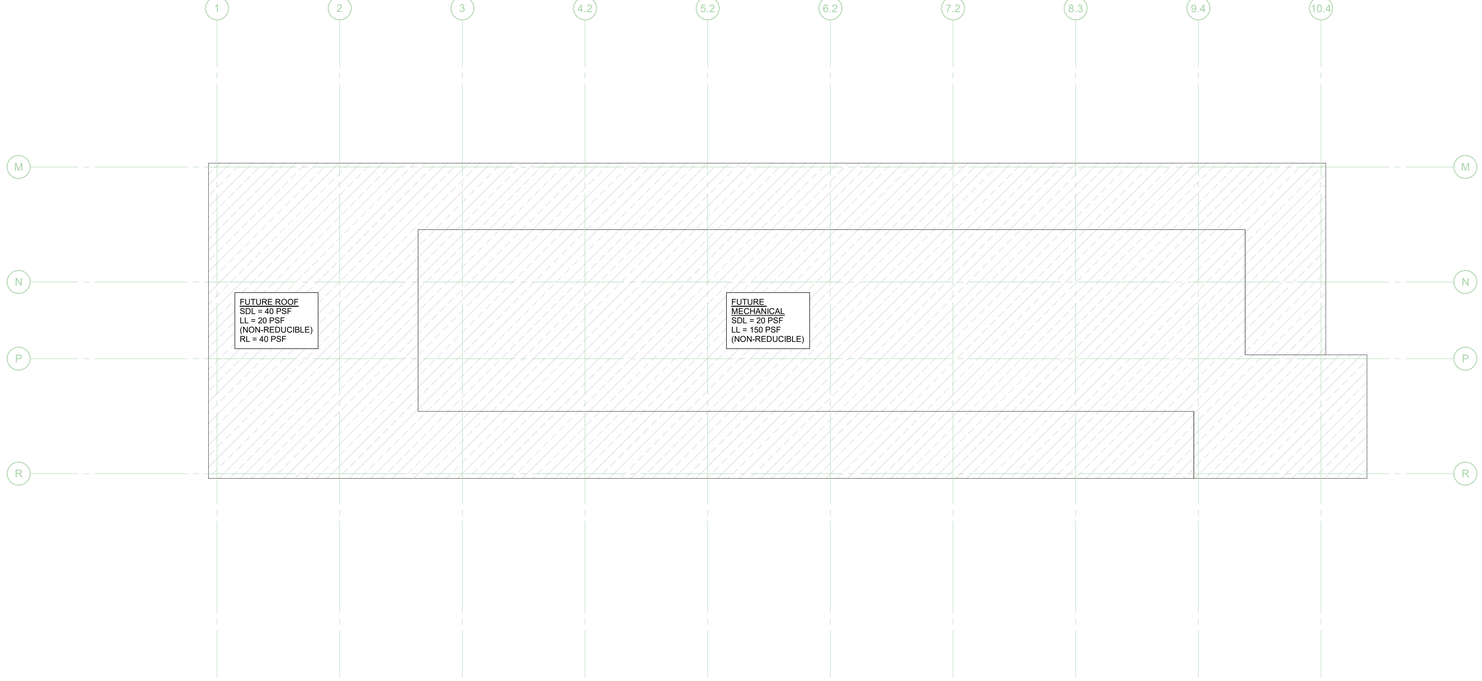


2 LOAD KEY PLAN - FUTURE PENTHOUSE ROOF
1/16" = 1'-0"



1 LOAD KEY PLAN - FUTURE LEVEL 06 PENTHOUSE
1/16" = 1'-0"

0 1/2" 1" 2"

3/4/2026 2:18:11 PM Autodesk Docs://10427132_Orlando_Health_Sunrise/305-29024-01 - OH Viera FSED - Structural - R2025.rvt

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WPM Project No. S05-25024-00

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FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
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EQUIPMENT PLANNER	IMEG CORP.
WAYFINDING	HDR ARCHITECTURE, INC.
SHEET REVIEWER	

MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

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Sheet Name

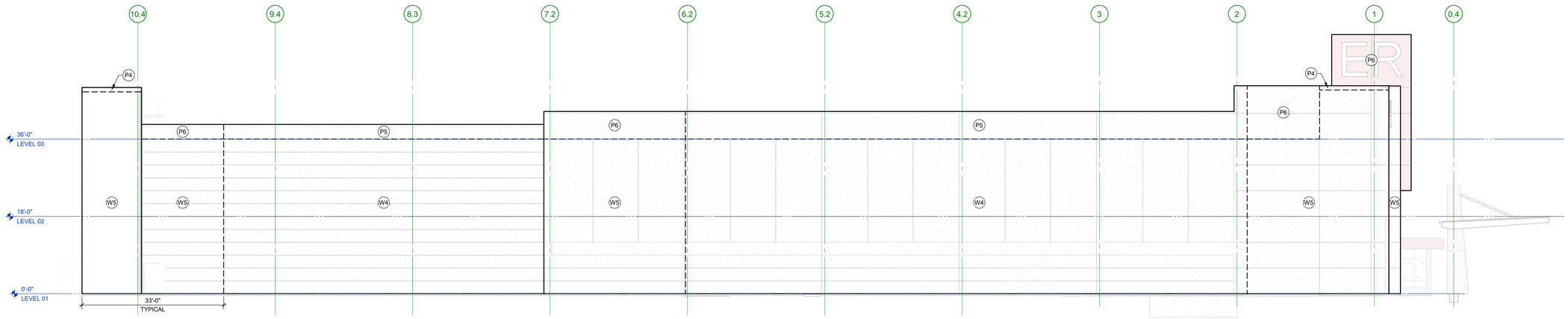
LOAD KEY PLANS

Sheet Number

S-023

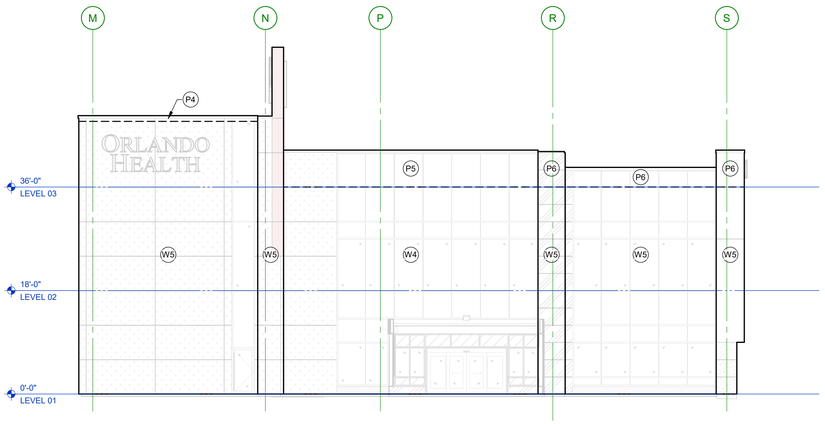
Project Status

CONSTRUCTION DOCUMENTS



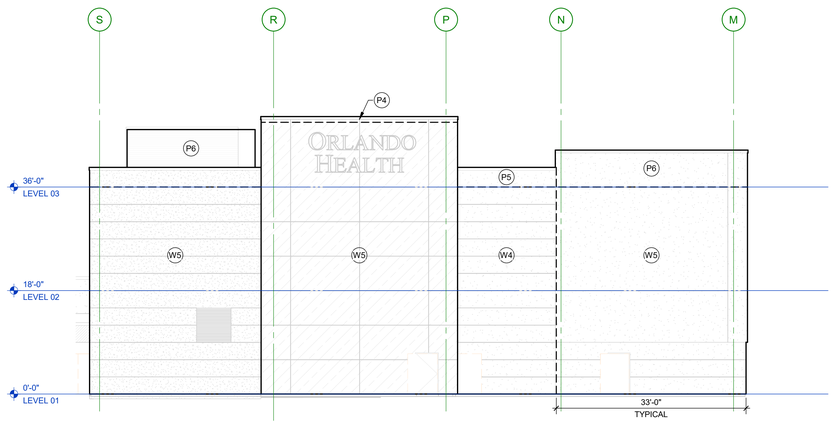
4 WIND PRESSURE DIAGRAM - NORTH ELEVATION

3/32" = 1'-0"



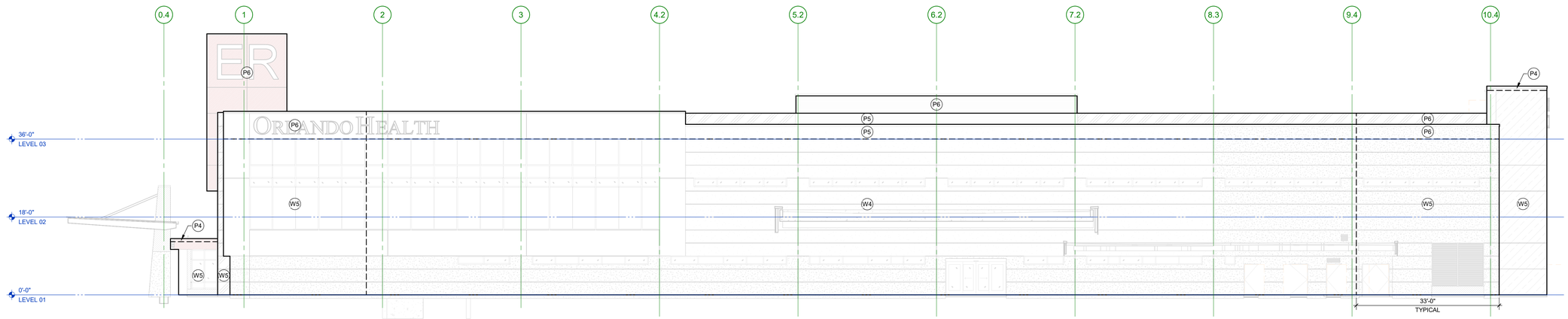
3 WIND PRESSURE DIAGRAM - WEST ELEVATION

3/32" = 1'-0"



2 WIND PRESSURE DIAGRAM - EAST ELEVATION

3/32" = 1'-0"



1 WIND PRESSURE DIAGRAM - SOUTH ELEVATION

3/32" = 1'-0"

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PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

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Sheet Name

WIND PRESSURE DIAGRAMS

Sheet Number

S-031

Project Status

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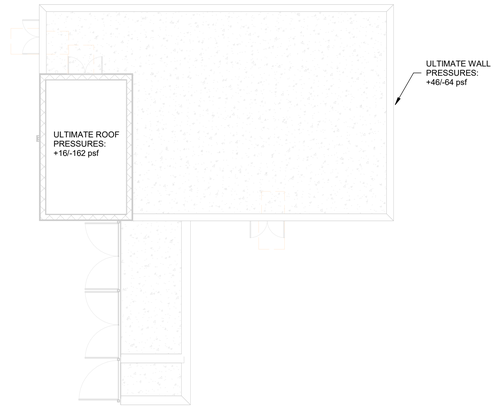
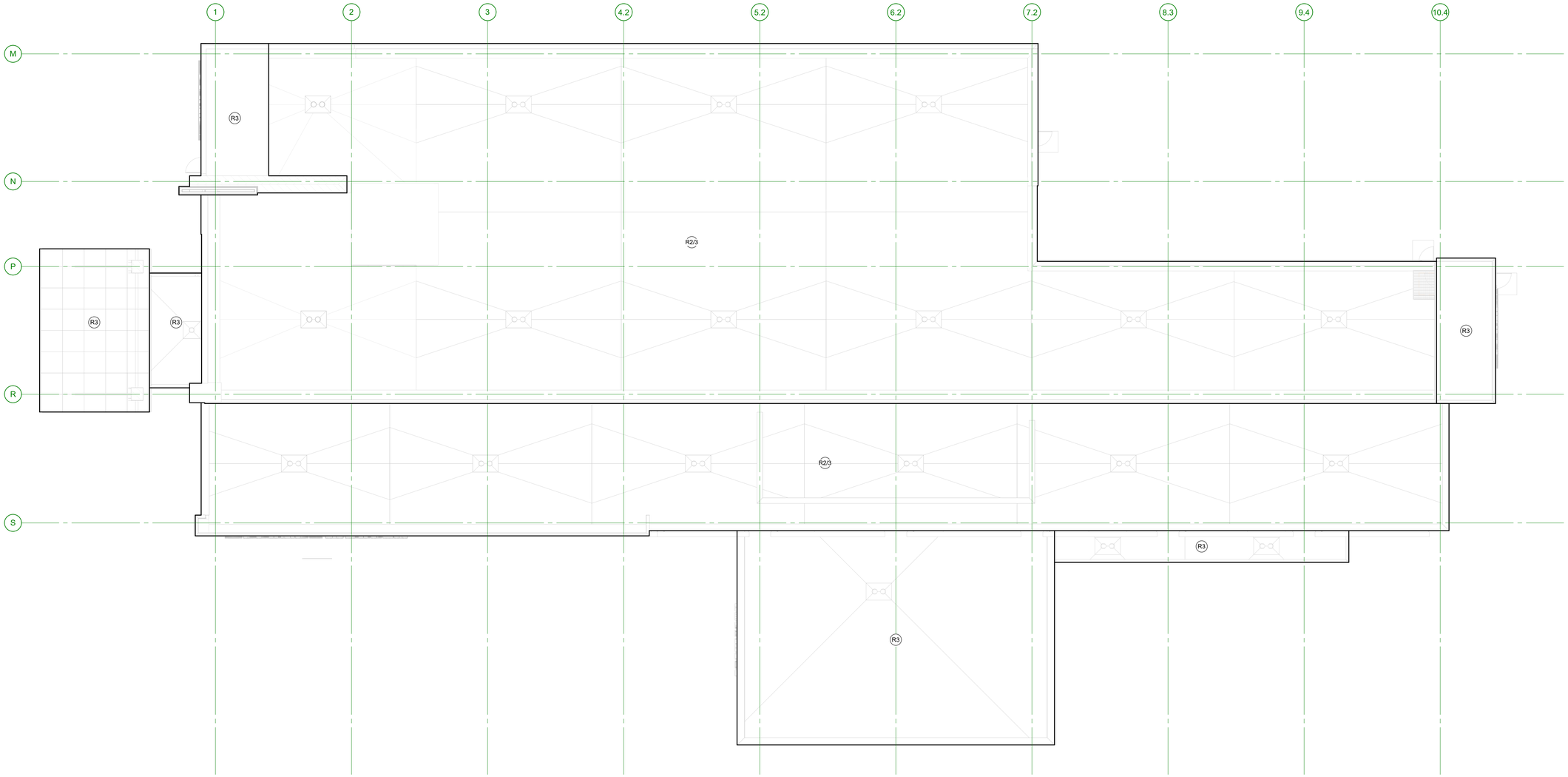


2 WIND PRESSURE DIAGRAM - ROOF PLAN - CHILLER YARD
3/32" = 1'-0"



1 WIND PRESSURE DIAGRAM - ROOF PLAN
3/32" = 1'-0"

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Sheet Name

WIND PRESSURE DIAGRAMS

Sheet Number

S-032

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CONSTRUCTION DOCUMENTS

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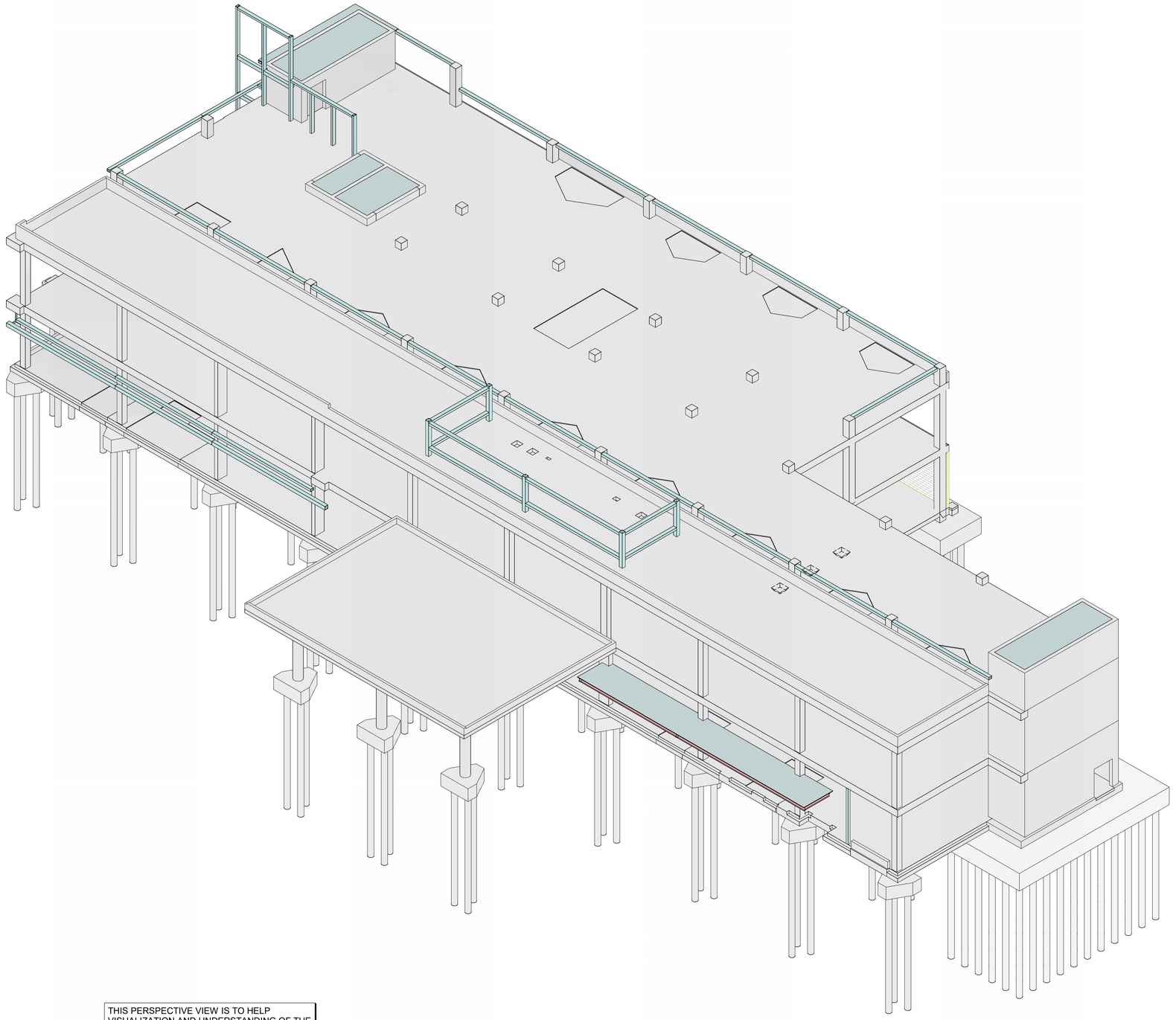
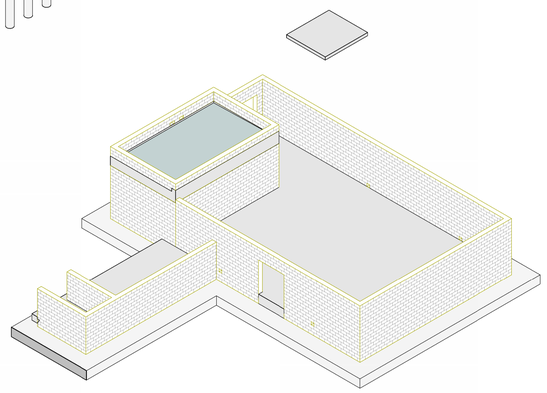
0 1/2" 1" 2"



THIS PERSPECTIVE VIEW IS TO HELP VISUALIZATION AND UNDERSTANDING OF THE STRUCTURE AND SHOULD NOT BE USED TO DEFINE SCOPE OR DETERMINE QUANTITIES



1 ISOMETRIC

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Sheet Name

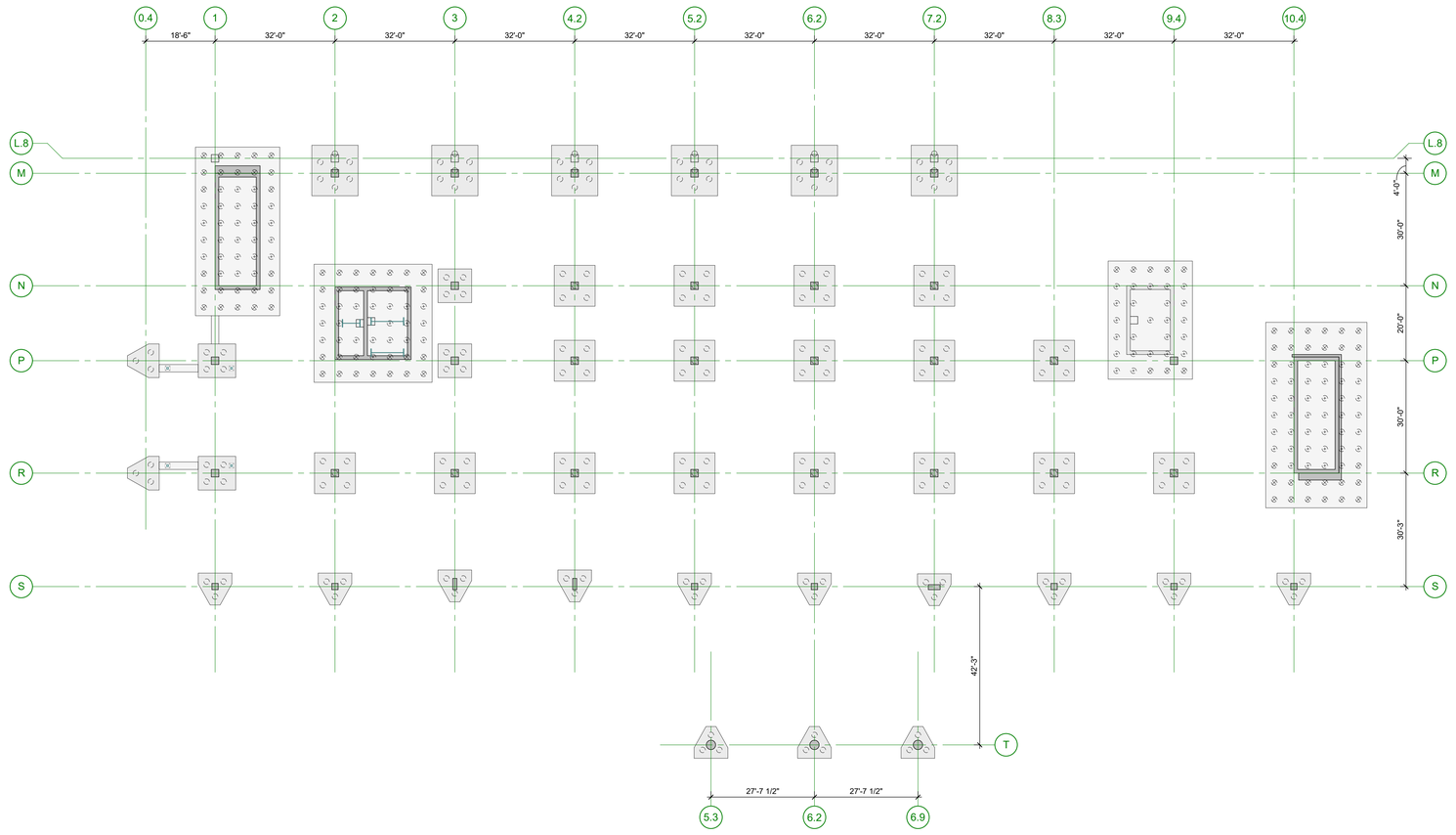
BUILDING ISOMETRIC

Sheet Number

S-051

Project Status

CONSTRUCTION DOCUMENTS

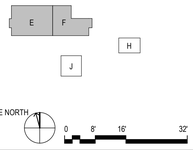


1 OVERALL FOUNDATION PLAN
1/16" = 1'-0"

FOUNDATION PLAN NOTES:

1. CIVIL DRAWINGS ELEVATION 30.82' = DATUM ELEVATION 0'-0".
2. REFER TO PLAN FOR TOP OF FOUNDATION ELEVATIONS.
3. FOUNDATION STRUCTURE IS ASSUMED TO BE AUGERCAST PILES ON CAST-IN-PLACE CONCRETE PILE CAPS. THIS STILL NEEDS TO BE CONFIRMED.
4. COLUMN FOUNDATIONS SHALL BE CENTERED ON THE CENTROID OF SUPPORTED COLUMN UNLESS NOTED OTHERWISE.
5. WALL FOOTINGS SHALL BE CENTERED ON THE SUPPORTED WALL, UNLESS NOTED OTHERWISE.
6. ALIGN TOP OF GRADE BEAMS WITH TOP OF ADJACENT FOUNDATIONS, UNLESS NOTED OTHERWISE.
7. REFER TO S-101 SERIES FOR SLAB-ON-GRADE INFORMATION.
8. REFER TO S-300 SERIES FOR FOUNDATION AND SLAB-ON-GRADE TYPICAL DETAILS AND SCHEDULES.
9. REFER TO S-500 SERIES FOR CONCRETE COLUMN ELEVATIONS.
10. REFER TO S-600 SERIES FOR SHEAR WALL ELEVATIONS.
11. REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR SLAB PENETRATIONS AND UNDERGROUND UTILITIES.
12. CONTRACTOR TO FIELD VERIFY ALL EXISTING CONDITIONS AND DIMENSIONS. NOTIFY ARCHITECT AND STRUCTURAL ENGINEER OF ANY DISCREPANCIES.
13. UNLESS NOTED OTHERWISE, AT ALL COLUMN STUBS PROVIDE TAPERED THREADED MECHANICAL COUPLERS AT THE TOP OF ALL COLUMN VERTICAL REINFORCEMENT TO ACCOMMODATE FUTURE EXPANSION. REFER TO 1/S-311.
14. PROVIDE TAPERED THREADED MECHANICAL COUPLERS AT THE TOP OF ALL SHEAR WALL VERTICAL REINFORCEMENT TO ACCOMMODATE FUTURE EXPANSION. REFER TO 2/S-311.

KEY PLAN



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Sheet Name

OVERALL FOUNDATION PLAN

Sheet Number

S-100

Project Status

CONSTRUCTION DOCUMENTS

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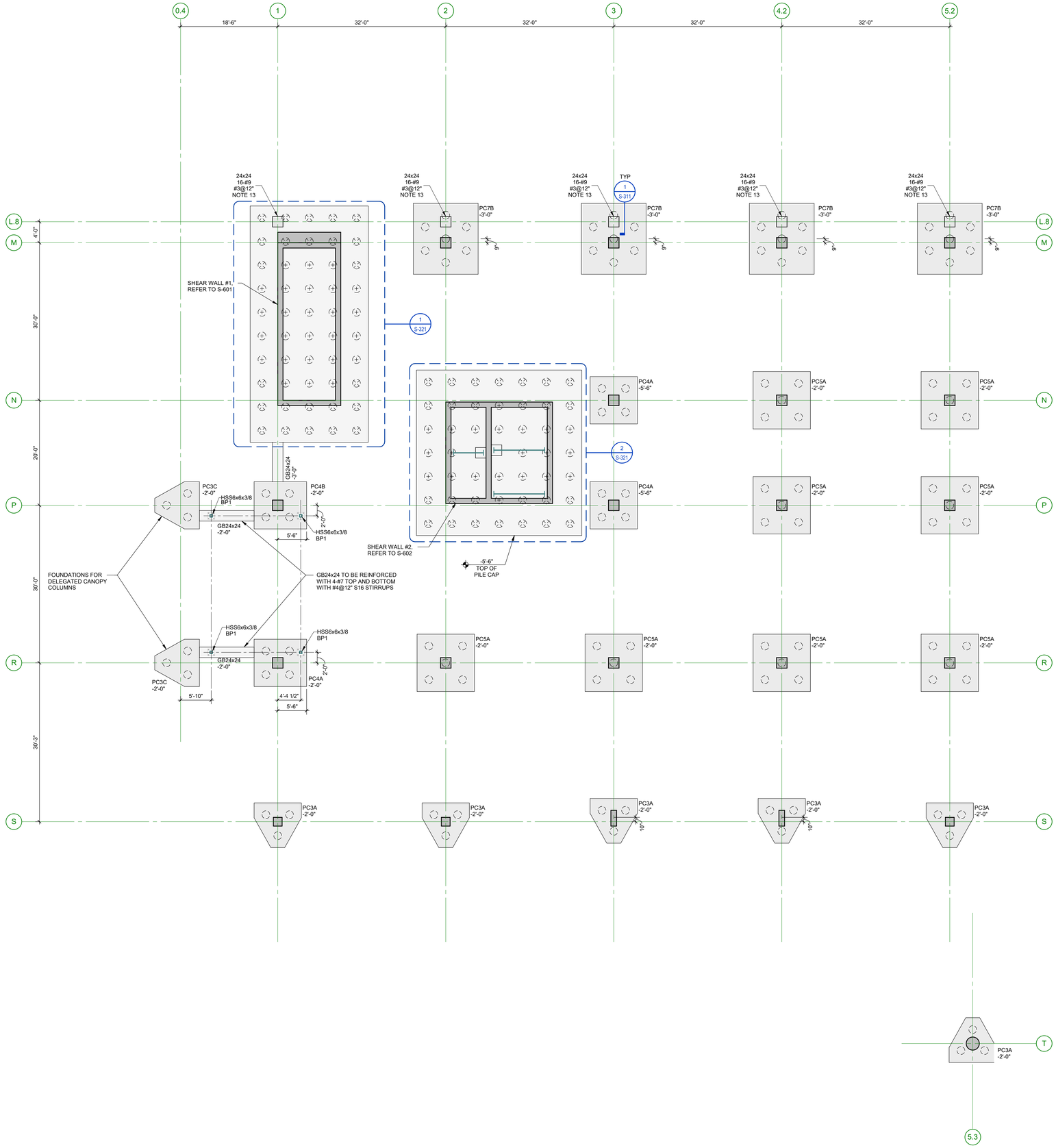
REFER TO S-100 FOR PLAN NOTES



1 FOUNDATION PLAN - AREA E
1/8" = 1'-0"

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Sheet Name

FOUNDATION PLAN - AREA E

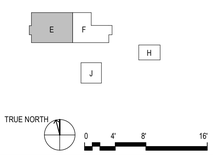
Sheet Number

S-100E

Project Status

CONSTRUCTION DOCUMENTS

KEY PLAN



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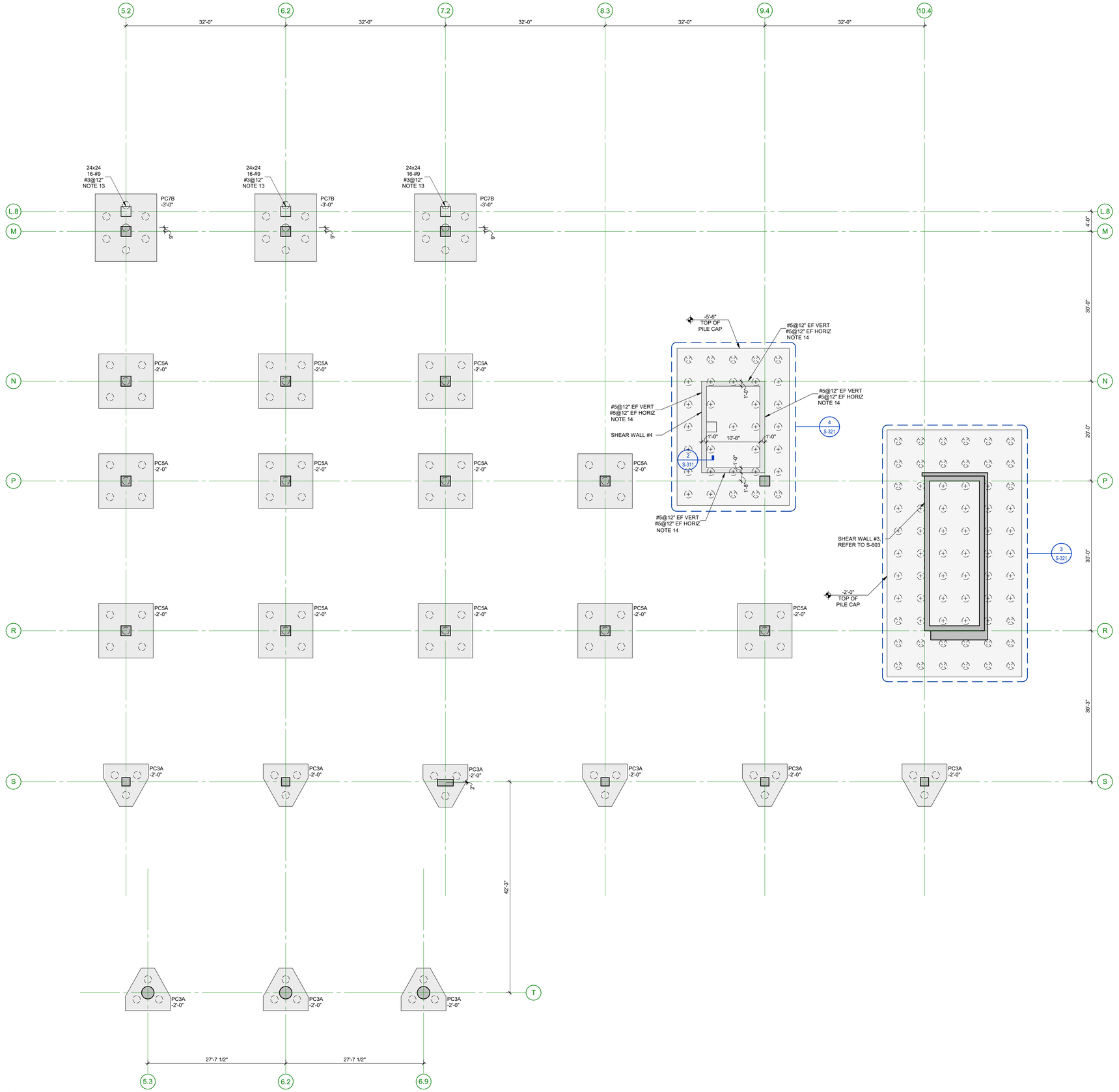
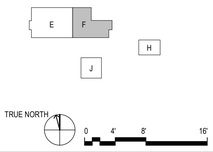
REFER TO S-100 FOR PLAN NOTES



1 FOUNDATION PLAN - AREA F
1/8" = 1'-0"

KEY PLAN



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ORLANDO HEALTH MELBOURNE NEW FREESTANDING EMERGENCY CARE FACILITY

LAKE ANDREW DR & WILLET PLACE
MELBOURNE, FL 32940

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PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
LANDSCAPE ARCHITECT	MCKINTOSH ASSOCIATES, an LJA COMPANY
CIVIL ENGINEER	BSE CONSULTANTS, an LJA COMPANY
STRUCTURAL ENGINEER	WALTER P. MOORE & ASSOCIATES, INC.
MECHANICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
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PLUMBING ENGINEER	I.C. TOMASSON ASSOCIATES, INC
INTERIOR DESIGNER	HDR ARCHITECTURE, INC.
EQUIPMENT PLANNER	IMEG CORP.
WAYFINDING	HDR ARCHITECTURE, INC.
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MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

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Sheet Name

FOUNDATION PLAN - AREA F

Sheet Number

S-100F

Project Status

CONSTRUCTION DOCUMENTS

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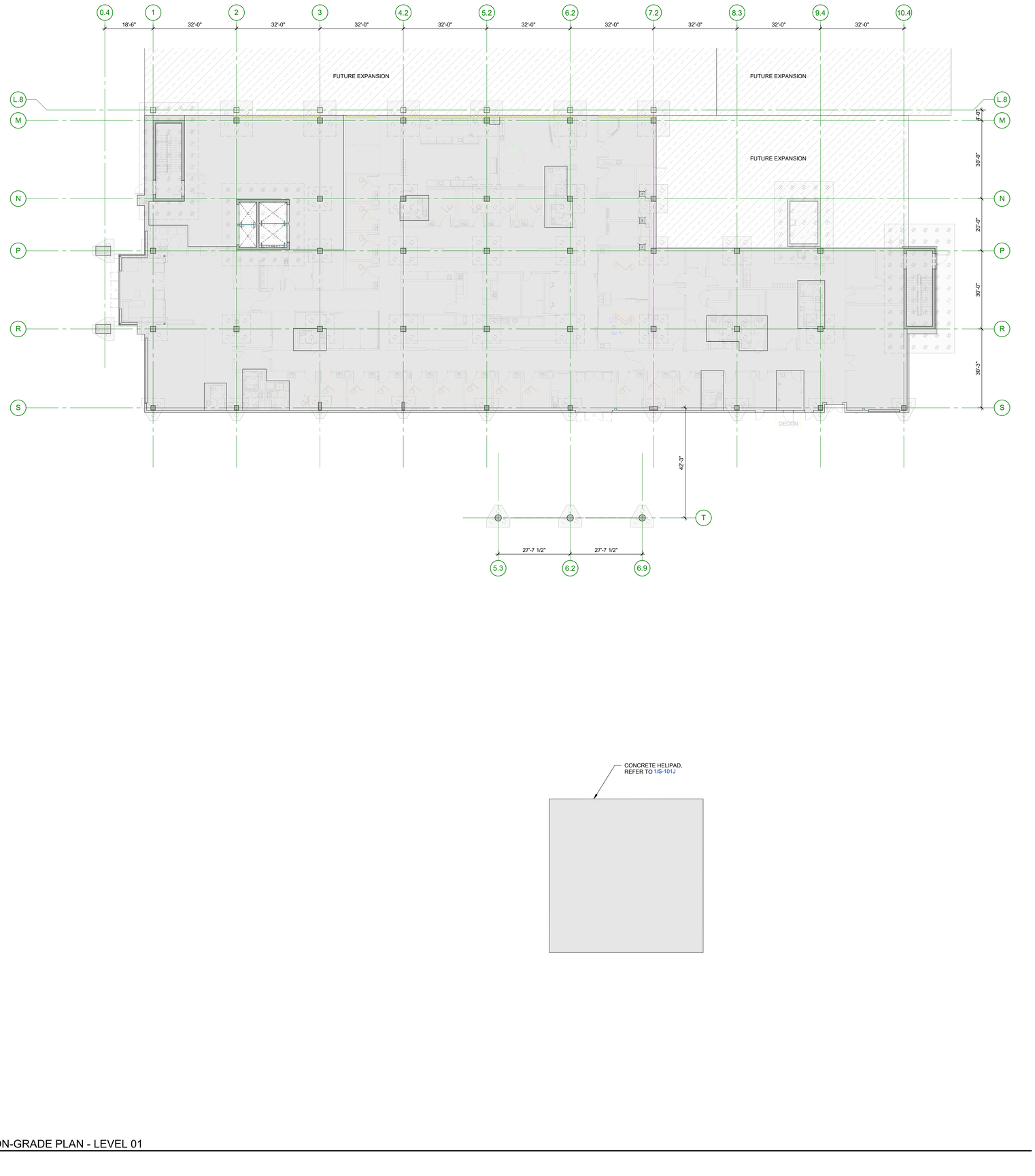
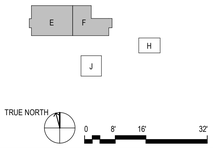


1 OVERALL SLAB-ON-GRADE PLAN - LEVEL 01
1/16" = 1'-0"

SLAB-ON-GRADE PLAN NOTES:

1. CIVIL DRAWINGS ELEVATION 30.82' = DATUM ELEVATION 0'-0".
2. REFER TO PLAN FOR TOP OF SLAB ELEVATIONS.
3. REFER TO PLAN FOR SLAB-ON-GRADE THICKNESS. SLAB-ON-GRADE SHALL BE REINFORCED AS FOLLOWS:
   a. 5" SLAB-ON-GRADE: WWR6x6-W2.9x2.9
   b. 8" SLAB-ON-GRADE: 2 LAYERS WWR4x4-W4xW4
   c. 10" SLAB-ON-GRADE: 2 LAYERS #5@12" EACH DIRECTION
   d. 12" SLAB-ON-GRADE: 2 LAYERS #4@12" EACH DIRECTION
4. PROVIDE 2-#4x4'-0" ADDITIONAL SLAB REINFORCEMENT AT ALL RE-ENTRANT CORNERS.
5. REFER TO S-100 SERIES FOR FOUNDATION PLANS.
6. REFER TO S-300 SERIES FOR FOUNDATION AND SLAB-ON-GRADE TYPICAL DETAILS AND SCHEDULES.
7. REFER TO S-500 SERIES FOR CONCRETE COLUMN ELEVATIONS.
8. REFER TO S-600 SERIES FOR SHEAR WALL ELEVATIONS.
9. REFER TO S-700 SERIES FOR CLADDING ELEVATIONS.
10. ALL STEEL THAT IS PERMANENTLY EXPOSED TO THE EXTERIOR OR IS PERMANENTLY IN UNCONDITIONED SPACE SHALL BE HOT-DIPPED GALVANIZED, EXCEPT WHERE THE ARCHITECT HAS SPECIFIED AN EXTERIOR GRADE HIGH-PERFORMANCE PAINT.
11. REFER TO ARCHITECTURAL DRAWINGS FOR EXTENTS AND DIMENSIONS OF RAISED OR DEPRESSED SLAB AREAS, SLOPES, CURBS, AND DRAINS. REFER TO TYPICAL DETAILS FOR REINFORCEMENT REQUIREMENTS.
12. REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR SLAB PENETRATIONS AND UNDERGROUND UTILITIES.
13. CONFIRM ALL HOUSEKEEPING PAD DIMENSIONS WITH MECHANICAL DRAWINGS. REFER TO 14/S-303 FOR HOUSEKEEPING PAD DETAIL.
14. CONTRACTOR TO FIELD VERIFY ALL EXISTING CONDITIONS AND DIMENSIONS. NOTIFY ARCHITECT AND STRUCTURAL ENGINEER OF ANY DISCREPANCIES.

KEY PLAN



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walter p moore

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SHEET REVIEWER	

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Sheet Name

OVERALL SLAB-ON-GRADE PLAN - LEVEL 01

Sheet Number

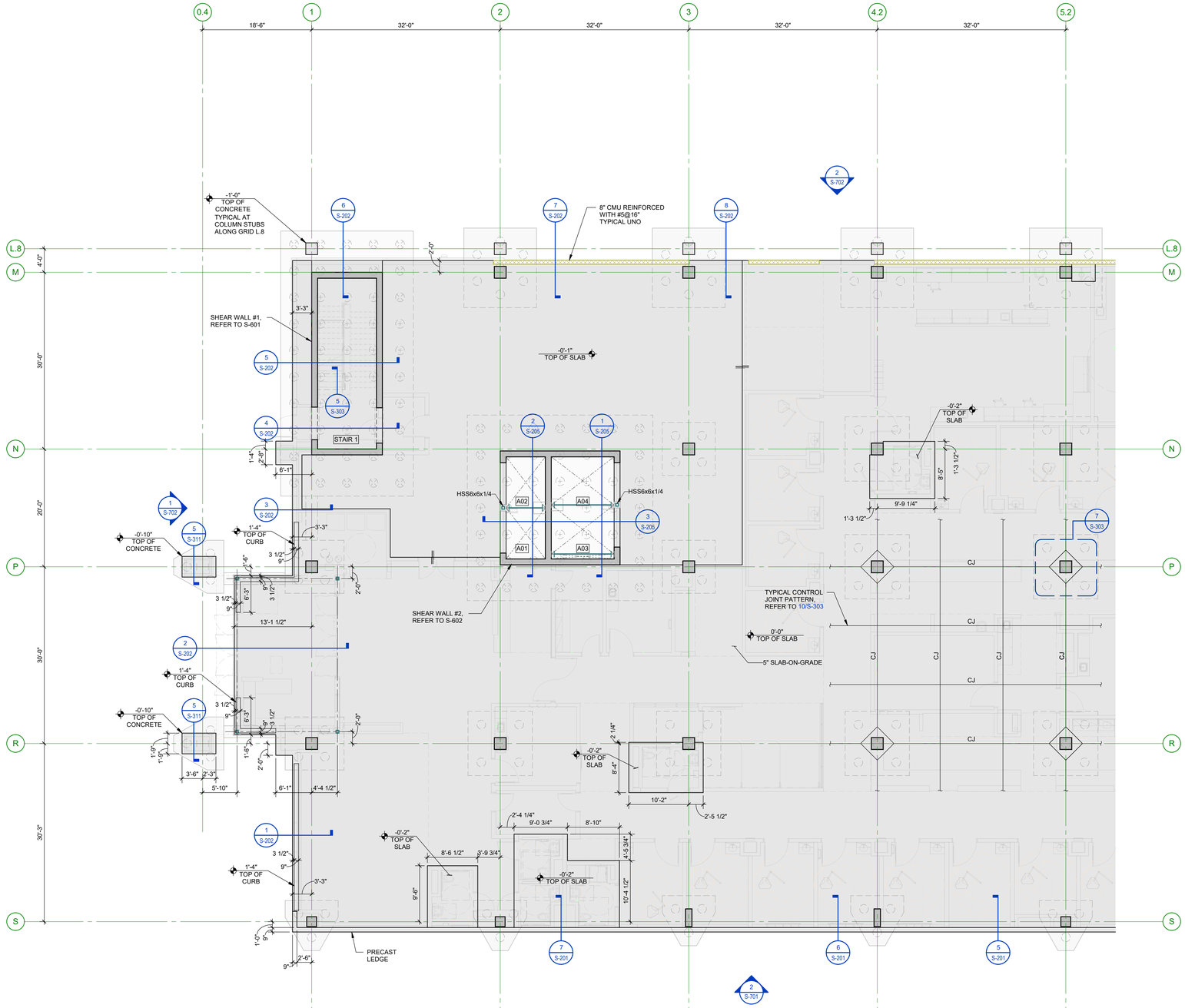
S-101

Project Status

CONSTRUCTION DOCUMENTS

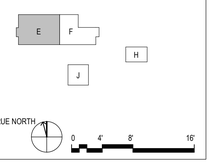
3/4/2026 2:19:16 PM Autodesk Docs://10427132_Orlando_Health_Sunrise/S05-25024-01 - OH View FSED_Structural - R2026.rvt

REFER TO S-101 FOR PLAN NOTES



1 SLAB-ON-GRADE PLAN - LEVEL 01 - AREA E
1/8" = 1'-0"

KEY PLAN



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Sheet Name

SLAB-ON-GRADE PLAN - LEVEL 01 - AREA E

Sheet Number

S-101E

Project Status

CONSTRUCTION DOCUMENTS

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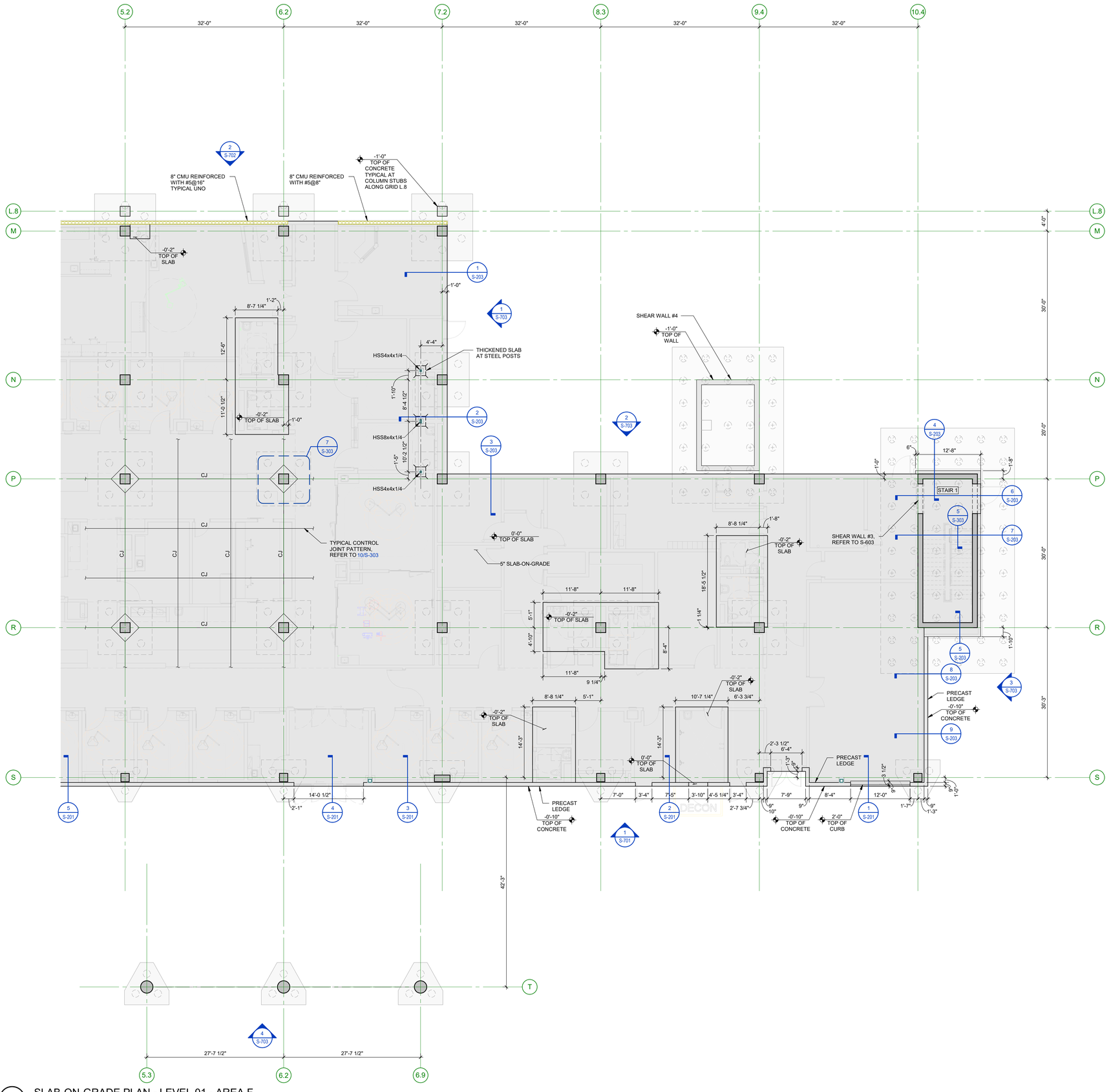
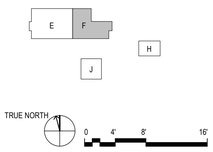
REFER TO S-101 FOR PLAN NOTES



1 SLAB-ON-GRADE PLAN - LEVEL 01 - AREA F
1/8" = 1'-0"

KEY PLAN



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Sheet Name

SLAB-ON-GRADE PLAN - LEVEL 01 - AREA F

Sheet Number

S-101F

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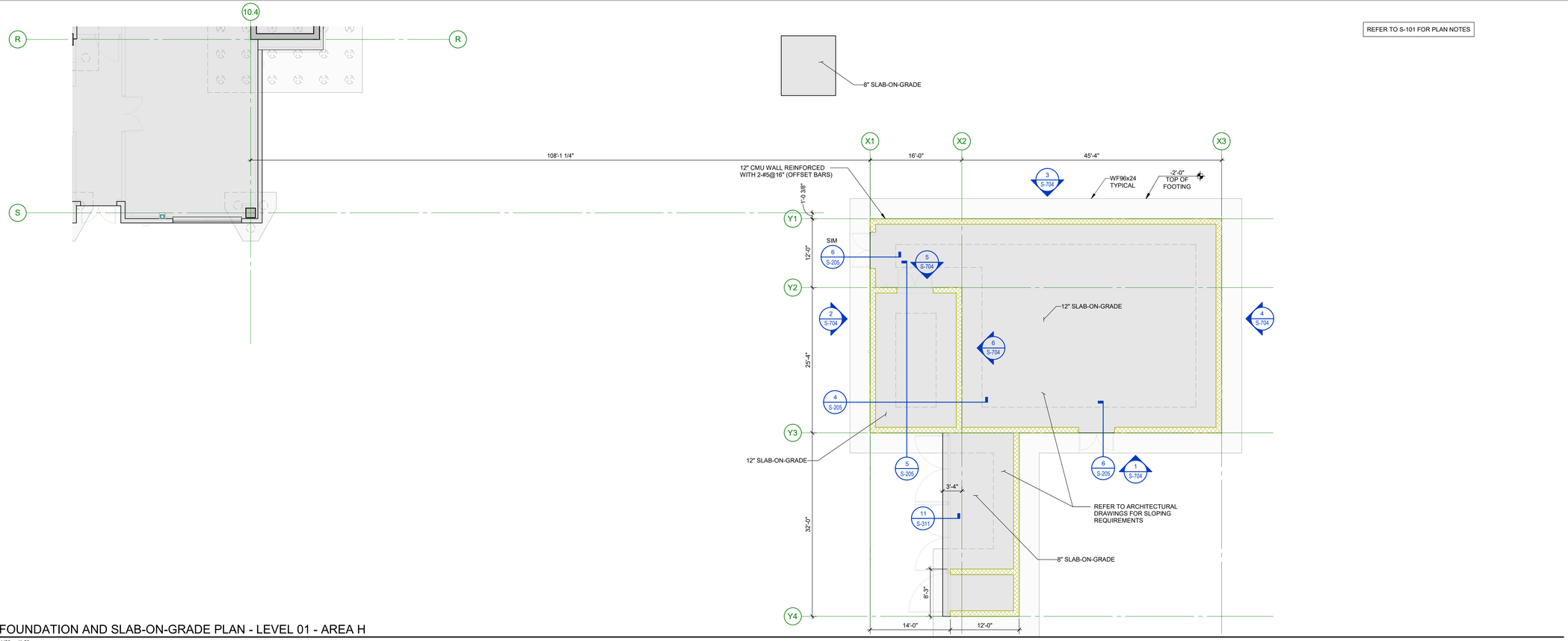
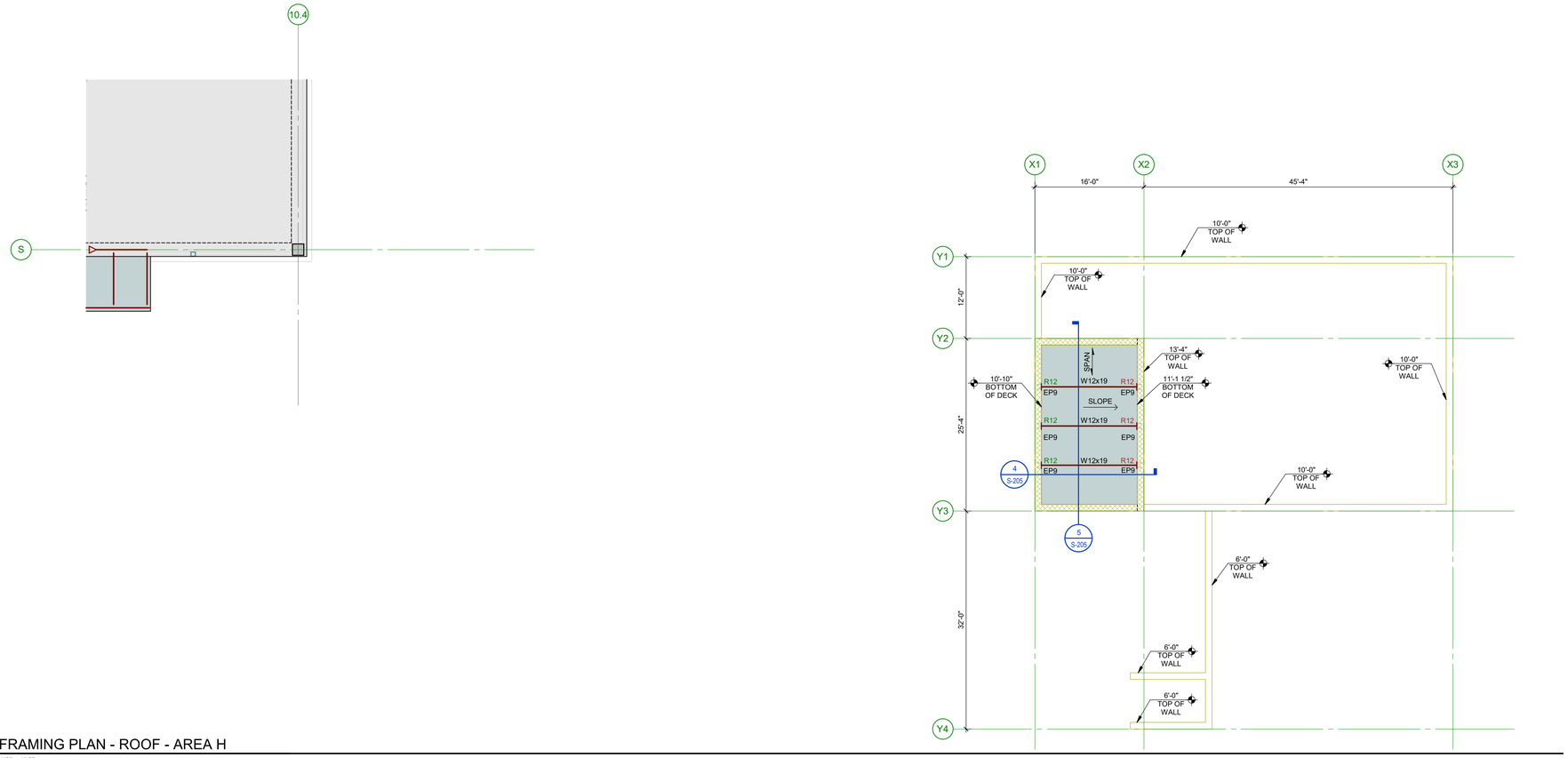


1 FOUNDATION AND SLAB-ON-GRADE PLAN - LEVEL 01 - AREA H

1/8" = 1'-0"



2 FRAMING PLAN - ROOF - AREA H

1/8" = 1'-0"

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Sheet Name

CHILLER YARD PLANS

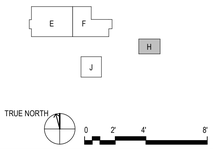
Sheet Number

S-101H

Project Status

CONSTRUCTION DOCUMENTS

KEY PLAN



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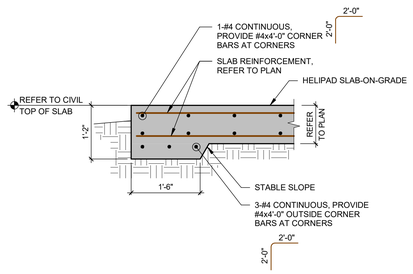
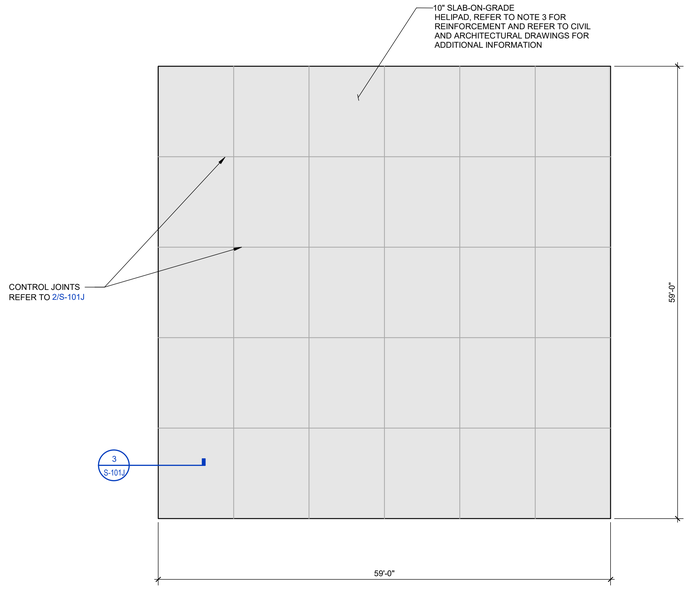
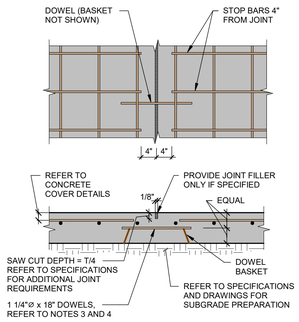


3 TYPICAL HELIPAD EDGE OF SLAB

3/4" = 1'-0"



REFER TO S-101 FOR PLAN NOTES



NOTES:

1. REFER TO PLAN FOR SLAB THICKNESS (T) AND REINFORCEMENT.
2. SLAB REINFORCEMENT SHALL BE CHAIRED BY SOIL SUPPORTED SLAB BOLSTERS.
3. DOWELS SHOULD BE SMOOTH, ALIGNED, AND SUPPORTED SO THEY WILL REMAIN PARALLEL IN BOTH THE HORIZONTAL AND VERTICAL PLANES. DOWELS SHALL BE PLACED NO CLOSER THAN 12 INCHES FROM INTERSECTION OF JOINTS.
4. TO PREVENT A BOND BETWEEN THE DOWEL AND THE CONCRETE, LIGHTLY COAT THE EXPOSED END OF THE DOWELS WITH A PARAFFIN-BASED LUBRICANT, ASPHALT EMULSION, FORM OIL, OR GREASE IMMEDIATELY BEFORE PLACING CONCRETE ON THE SECOND SIDE OF THE JOINT. ALTERNATIVELY, USE A PLASTIC OR METAL SLEEVE SPECIFICALLY MANUFACTURED FOR THIS PURPOSE.
5. MAKE SAW CUT AS SOON AS SLAB IS ABLE TO SUPPORT WEIGHT OF WORKERS AND SAWING EQUIPMENT WITHOUT DAMAGE TO FINISH SURFACE OF SLAB. REFER TO SPECIFICATIONS.

2 TYPICAL CONTROL JOINTS IN HELIPAD SLAB

NO SCALE

10303-LPA-SCH.dgn
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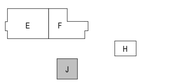
1 SLAB-ON-GRADE PLAN - LEVEL 01 - AREA J - HELIPAD

1/8" = 1'-0"



TRUE NORTH

0 4' 8' 16'

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Sheet Name

HELIPAD PLAN AND DETAILS

Sheet Number

S-101J

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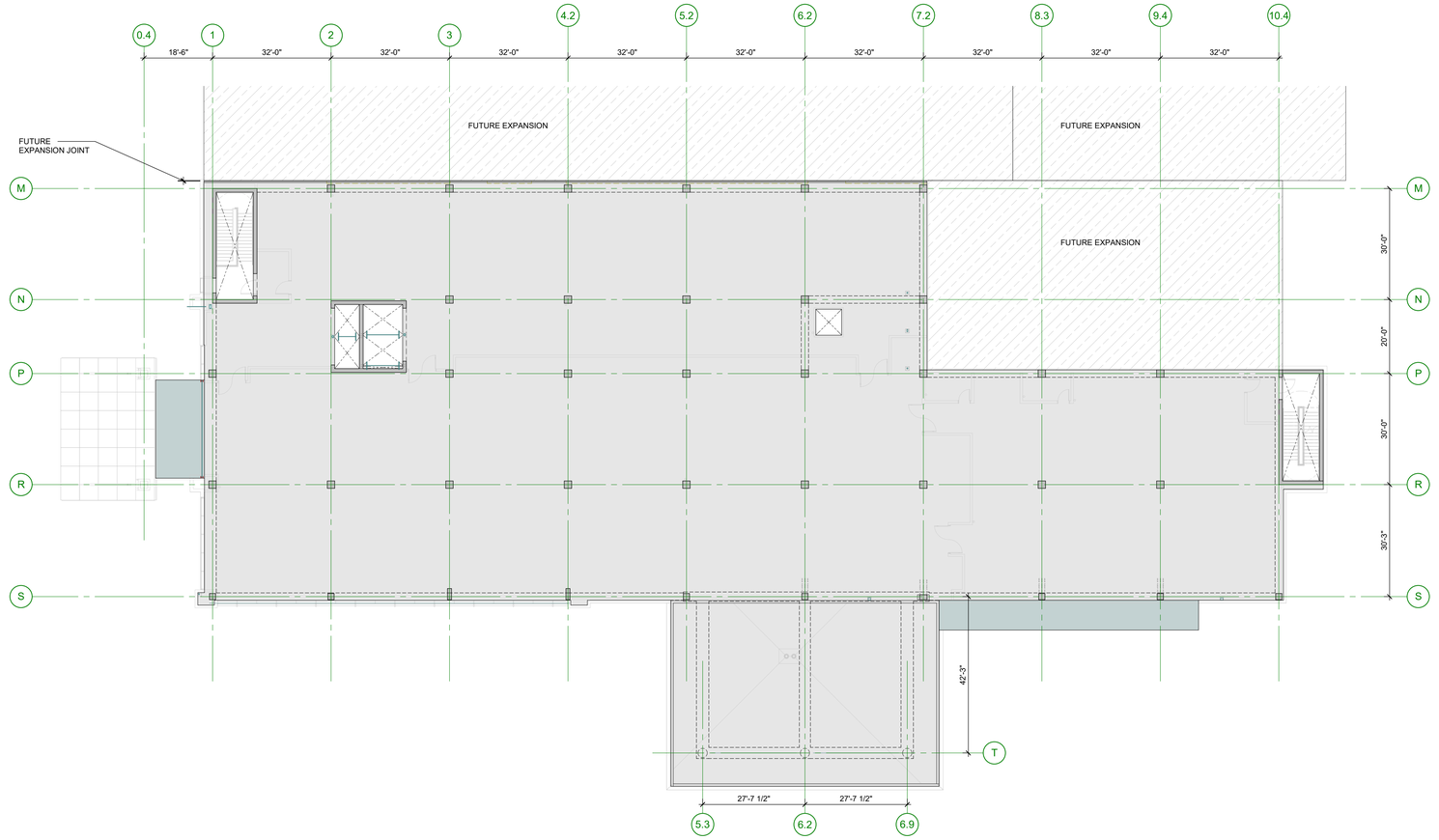


1 OVERALL FRAMING PLAN - LEVEL 02
1/16" = 1'-0"

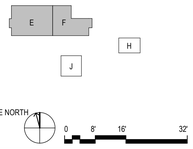
TYPICAL FRAMING PLAN NOTES:

1. REFER TO PLAN FOR TOP OF CONCRETE ELEVATIONS.
2. FLOOR STRUCTURE IS A NORMALWEIGHT CONCRETE SLAB. REFER TO PLAN FOR SLAB TYPES AND 13/S-451 FOR SLAB SCHEDULE AND REINFORCEMENT PLACEMENT GUIDELINES. ADDITIONAL REINFORCEMENT BEYOND TYPICAL SCHEDULED REINFORCEMENT IS SHOWN ON PLAN.
3. PROVIDE STUD RAILS AS SHOWN ON PLAN. REFER TO 11/S-403 FOR STUD RAIL SCHEDULE AND 13 AND 15/S-403 FOR STUD RAIL PLACEMENT DETAILS.
4. REFER TO S-400 SERIES FOR FRAMING TYPICAL DETAILS.
5. REFER TO S-450 SERIES FOR BEAM AND SLAB SCHEDULES.
6. REFER TO S-500 SERIES FOR CONCRETE COLUMN ELEVATIONS.
7. REFER TO S-600 SERIES FOR SHEAR WALL ELEVATIONS.
8. REFER TO S-700 SERIES FOR CLADDING ELEVATIONS.
9. REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR PENETRATIONS NOT SHOWN. REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS AT OPENINGS. CONTRACTOR MUST SUBMIT PLAN SHOWING ALL PENETRATIONS TO ENGINEER-OF-RECORD PRIOR TO CONSTRUCTION OF SLAB.
10. CONFIRM ALL HOUSEKEEPING PAD DIMENSIONS WITH MECHANICAL DRAWINGS. REFER TO 10/S-403 FOR HOUSEKEEPING PAD DETAIL. UNLESS NOTED OTHERWISE, MAXIMUM HOUSEKEEPING PAD THICKNESS SHALL BE 4".
11. ALL STEEL THAT IS PERMANENTLY EXPOSED TO THE EXTERIOR OR IS PERMANENTLY IN UNCONDITIONED SPACE SHALL BE HOT-DIPPED GALVANIZED, EXCEPT WHERE THE ARCHITECT HAS SPECIFIED AN EXTERIOR GRADE HIGH-PERFORMANCE PAINT.
12. UNLESS NOTED OTHERWISE, AT ALL COLUMN STUBS PROVIDE TAPERED THREADED MECHANICAL COUPLERS AT THE TOP OF ALL COLUMN VERTICAL REINFORCEMENT TO ACCOMMODATE FUTURE EXPANSION.
13. PROVIDE TAPERED THREADED MECHANICAL COUPLERS AT THE TOP OF ALL SHEAR WALL VERTICAL REINFORCEMENT TO ACCOMMODATE FUTURE EXPANSION.

STEEL ROOF PLAN NOTES:

1. REFER TO PLAN FOR BOTTOM OF DECK ELEVATION.
2. ROOF SYSTEM IS ROOF DECK SUPPORTED BY STEEL BEAMS. UNLESS NOTED OTHERWISE, ROOF DECK SHALL BE 1 1/2" DEEP, TYPE B, 20 GAGE GALVANIZED (G90) STEEL ROOF DECK. ROOF DECK SHALL BE PLACED WITH A TWO SPAN CONDITION MINIMUM. NO SINGLE SPANS ARE ALLOWED WITHOUT WRITTEN APPROVAL OF ENGINEER-OF-RECORD.
3. PROVIDE A CONTINUOUS 1/4" BENT PLATE AT ALL ROOF EDGES AND OPENINGS, UNLESS NOTED OTHERWISE.
4. ALL STEEL THAT IS PERMANENTLY EXPOSED TO THE EXTERIOR OR IS PERMANENTLY IN UNCONDITIONED SPACE SHALL BE HOT-DIPPED GALVANIZED.
5. REFER TO S-405 FOR STEEL FRAMING TYPICAL DETAILS.
6. REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR PENETRATIONS NOT SHOWN. REFER TO TYPICAL DETAILS FOR ADDITIONAL STEEL REQUIREMENTS AT OPENINGS.

KEY PLAN



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FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
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EQUIPMENT PLANNER	IMEG CORP.
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MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

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Sheet Name

OVERALL FRAMING PLAN - LEVEL 02

Sheet Number

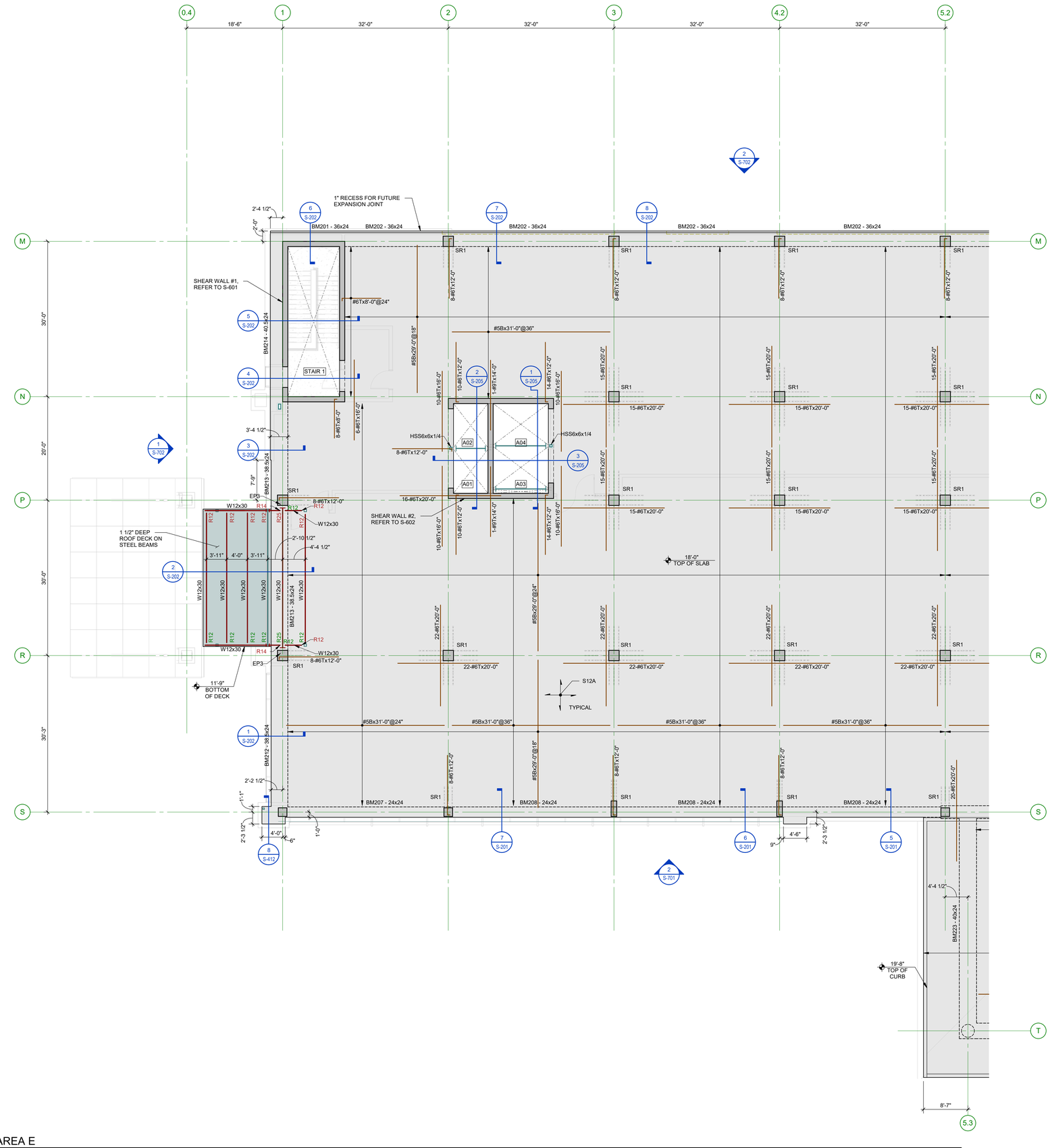
S-102

Project Status

CONSTRUCTION DOCUMENTS

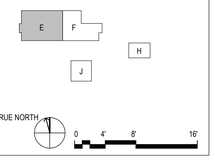
3/4/2026 2:19:39 PM Autodesk Docs://10427132_Orlando_Health_Sunrise/S05-25024-01 - OH View FSED_Structural - R2025.rvt

REFER TO S-102 FOR PLAN NOTES



1 FRAMING PLAN - LEVEL 02 - AREA E
1/8" = 1'-0"

KEY PLAN



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Sheet Name
FRAMING PLAN - LEVEL 02 - AREA E

Sheet Number
S-102E

Project Status
CONSTRUCTION DOCUMENTS

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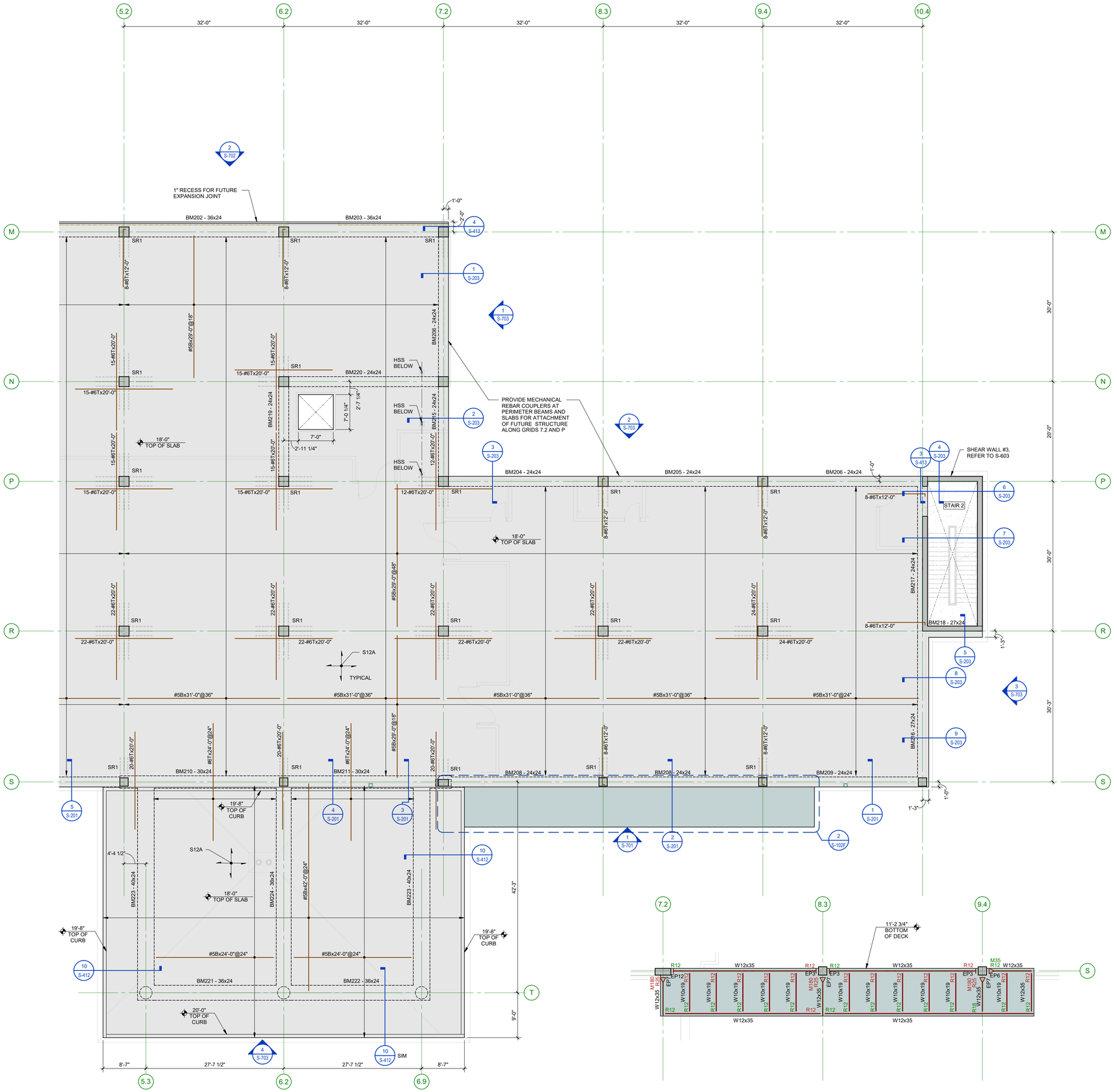
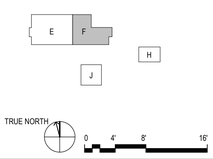


1 FRAMING PLAN - LEVEL 02 - AREA F
1/8" = 1'-0"

2 CANOPY PARTIAL PLAN
1/8" = 1'-0"

KEY PLAN



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Sheet Name

FRAMING PLAN - LEVEL 02 - AREA F

Sheet Number

S-102F

Project Status

CONSTRUCTION DOCUMENTS

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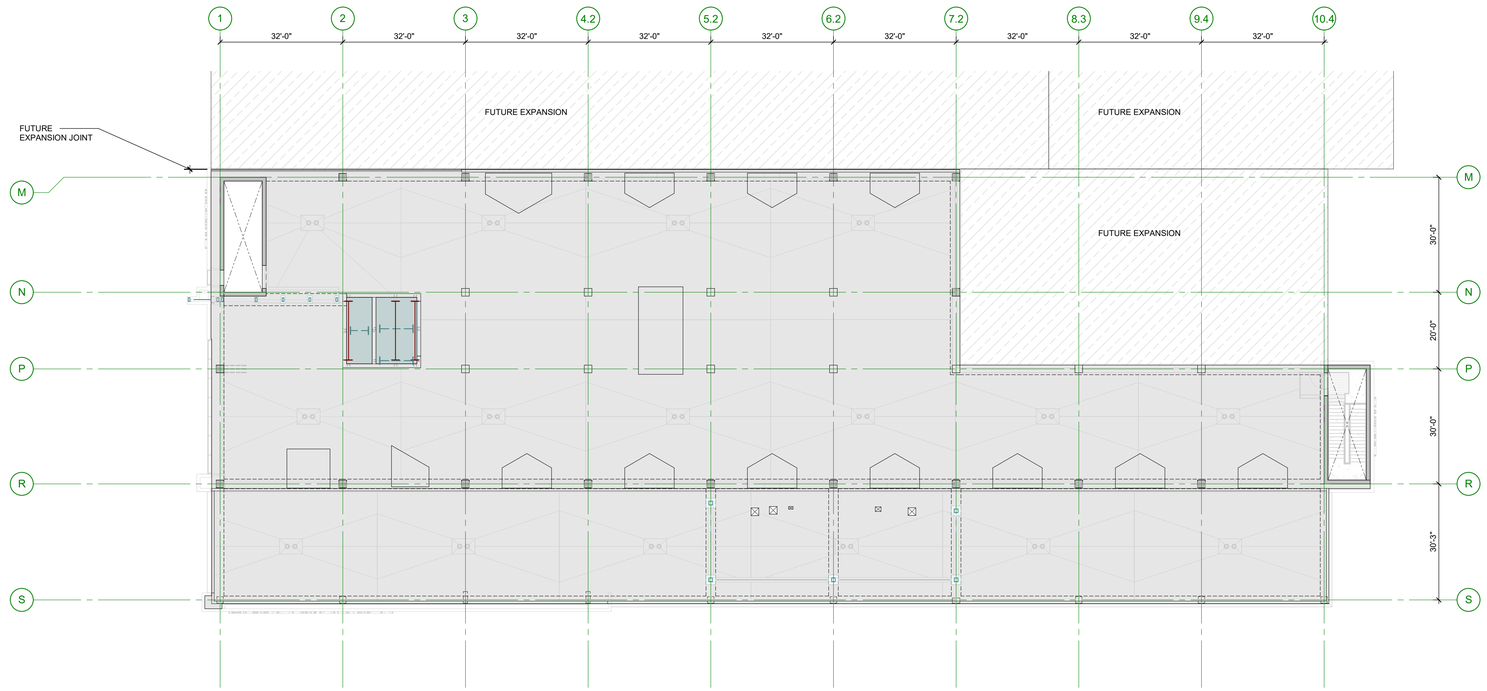


1 OVERALL FRAMING PLAN - ROOF (FUTURE LEVEL 3)
1/16" = 1'-0"

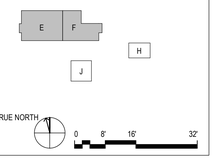
TYPICAL FRAMING PLAN NOTES:

1. REFER TO PLAN FOR TOP OF CONCRETE ELEVATIONS.
2. FLOOR STRUCTURE IS A NORMALWEIGHT CONCRETE SLAB. REFER TO PLAN FOR SLAB TYPES AND 13/S-451 FOR SLAB SCHEDULE AND REINFORCEMENT PLACEMENT GUIDELINES. ADDITIONAL REINFORCEMENT BEYOND TYPICAL SCHEDULED REINFORCEMENT IS SHOWN ON PLAN.
3. PROVIDE STUD RAILS AS SHOWN ON PLAN. REFER TO 11/S-403 FOR STUD RAIL SCHEDULE AND 13 AND 15/S-403 FOR STUD RAIL PLACEMENT DETAILS.
4. REFER TO S-400 SERIES FOR FRAMING TYPICAL DETAILS.
5. REFER TO S-450 SERIES FOR BEAM AND SLAB SCHEDULES.
6. REFER TO S-500 SERIES FOR CONCRETE COLUMN ELEVATIONS.
7. REFER TO S-600 SERIES FOR SHEAR WALL ELEVATIONS.
8. REFER TO S-700 SERIES FOR CLADDING ELEVATIONS.
9. REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR PENETRATIONS NOT SHOWN. REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS AT OPENINGS. CONTRACTOR MUST SUBMIT PLAN SHOWING ALL PENETRATIONS TO ENGINEER-OF-RECORD PRIOR TO CONSTRUCTION OF SLAB.
10. CONFIRM ALL HOUSEKEEPING PAD DIMENSIONS WITH MECHANICAL DRAWINGS. REFER TO 10/S-403 FOR HOUSEKEEPING PAD DETAIL. UNLESS NOTED OTHERWISE, MAXIMUM HOUSEKEEPING PAD THICKNESS SHALL BE 4".
11. ALL STEEL THAT IS PERMANENTLY EXPOSED TO THE EXTERIOR OR IS PERMANENTLY IN UNCONDITIONED SPACE SHALL BE HOT-DIPPED GALVANIZED, EXCEPT WHERE THE ARCHITECT HAS SPECIFIED AN EXTERIOR GRADE HIGH-PERFORMANCE PAINT.
12. UNLESS NOTED OTHERWISE, AT ALL COLUMN STUBS PROVIDE TAPERED THREADED MECHANICAL COUPLERS AT THE TOP OF ALL COLUMN VERTICAL REINFORCEMENT TO ACCOMMODATE FUTURE EXPANSION.
13. PROVIDE TAPERED THREADED MECHANICAL COUPLERS AT THE TOP OF ALL SHEAR WALL VERTICAL REINFORCEMENT TO ACCOMMODATE FUTURE EXPANSION.

STEEL ROOF PLAN NOTES:

1. REFER TO PLAN FOR BOTTOM OF DECK ELEVATION.
2. ROOF SYSTEM IS ROOF DECK SUPPORTED BY STEEL BEAMS. UNLESS NOTED OTHERWISE, ROOF DECK SHALL BE 1 1/2" DEEP, TYPE B, 20 GAGE GALVANIZED (G90) STEEL ROOF DECK. ROOF DECK SHALL BE PLACED WITH A TWO SPAN CONDITION MINIMUM. NO SINGLE SPANS ARE ALLOWED WITHOUT WRITTEN APPROVAL OF ENGINEER-OF-RECORD.
3. PROVIDE A CONTINUOUS 1/4" BENT PLATE AT ALL ROOF EDGES AND OPENINGS, UNLESS NOTED OTHERWISE.
4. ALL STEEL THAT IS PERMANENTLY EXPOSED TO THE EXTERIOR OR IS PERMANENTLY IN UNCONDITIONED SPACE SHALL BE HOT-DIPPED GALVANIZED.
5. REFER TO S-405 FOR STEEL FRAMING TYPICAL DETAILS.
6. REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR PENETRATIONS NOT SHOWN. REFER TO TYPICAL DETAILS FOR ADDITIONAL STEEL REQUIREMENTS AT OPENINGS.

KEY PLAN



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Sheet Name

OVERALL FRAMING PLAN - ROOF

Sheet Number

S-103

Project Status

CONSTRUCTION DOCUMENTS

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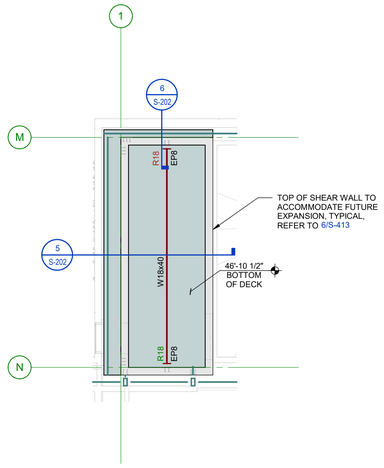
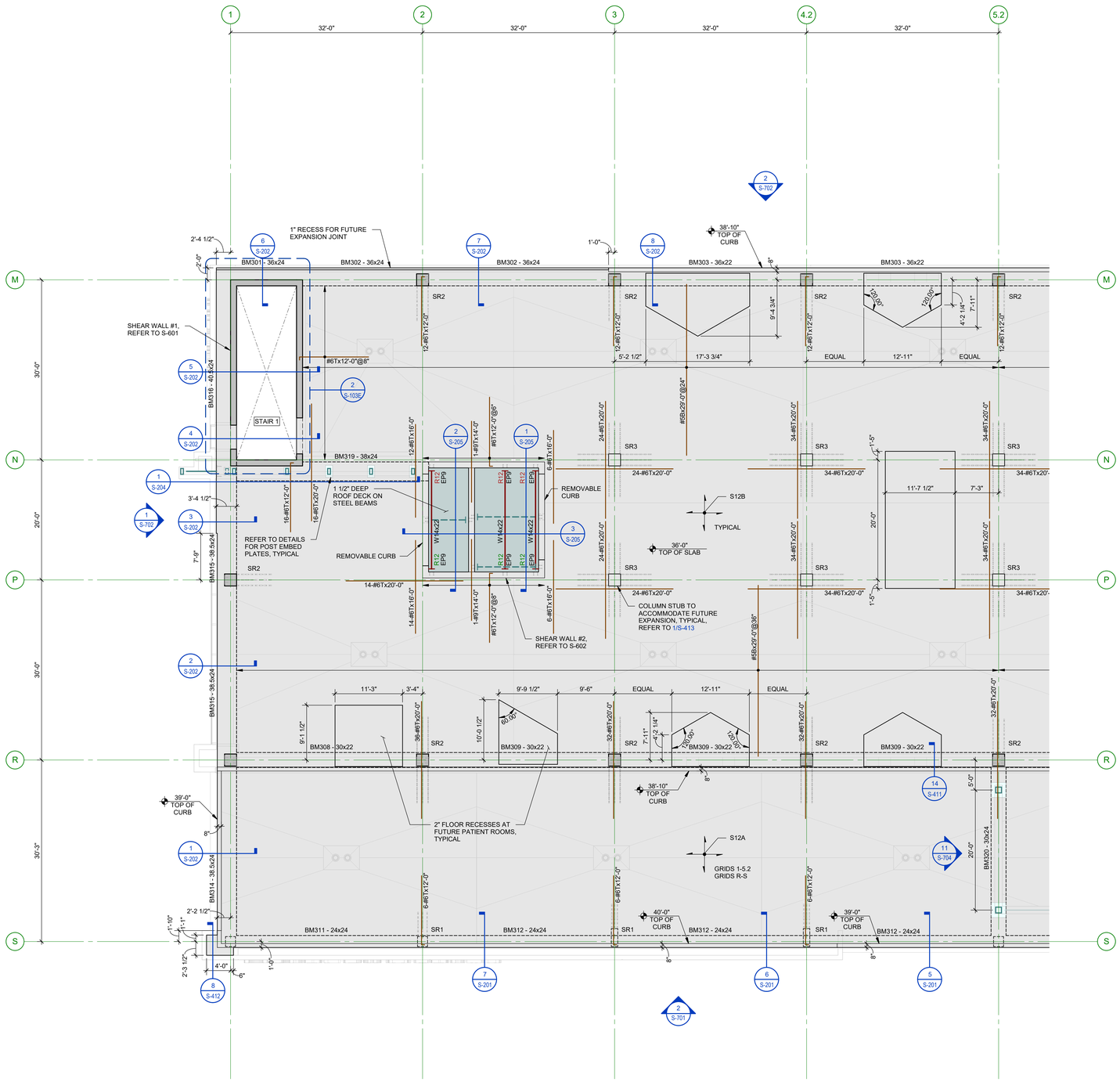
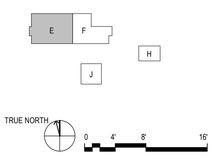


2 FRAMING PLAN - WEST STAIR ROOF
1/8" = 1'-0"



1 FRAMING PLAN - ROOF (FUTURE LEVEL 03) - AREA E
1/8" = 1'-0"

KEY PLAN



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Sheet Name

FRAMING PLAN - ROOF - AREA E

Sheet Number

S-103E

Project Status

CONSTRUCTION DOCUMENTS

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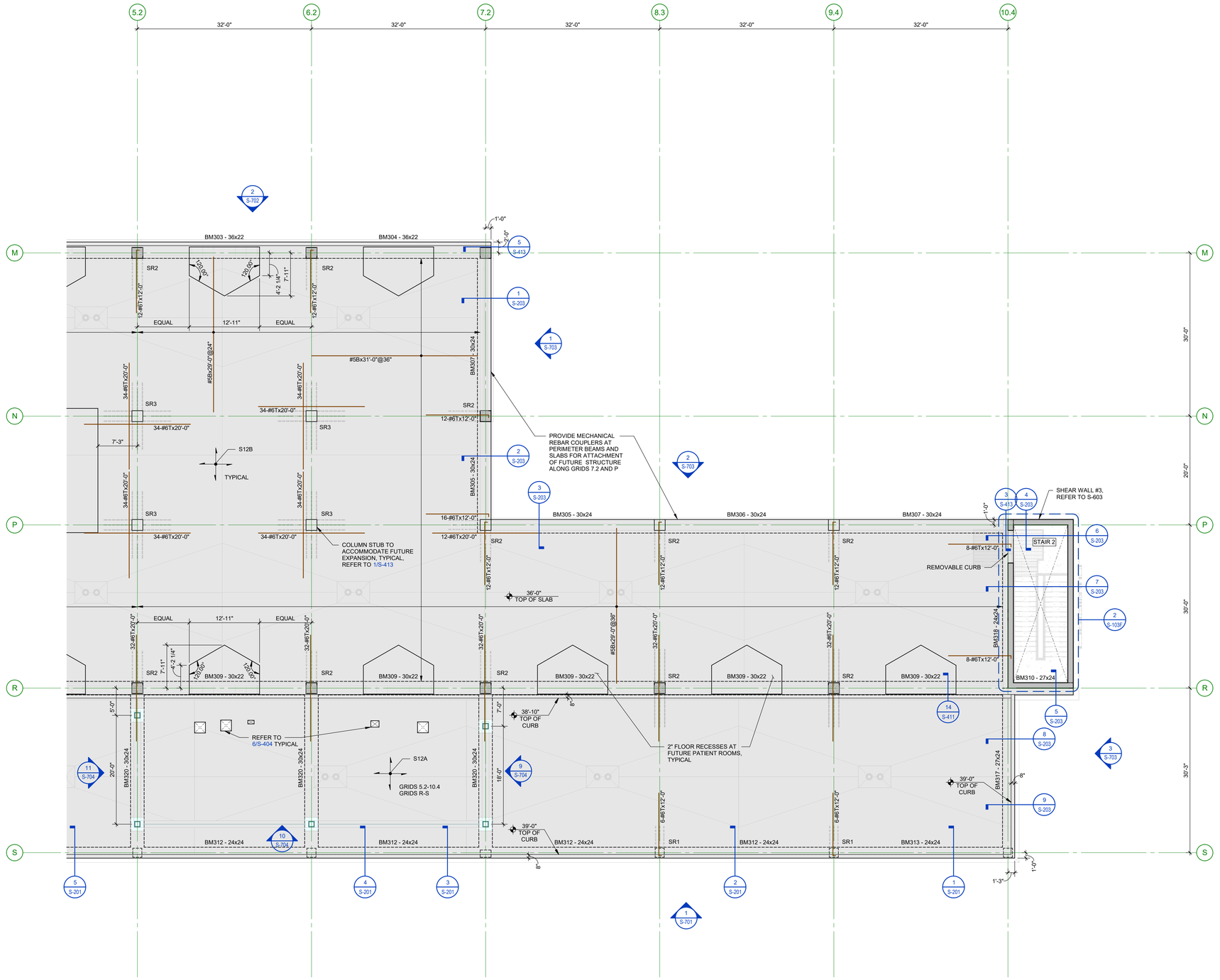
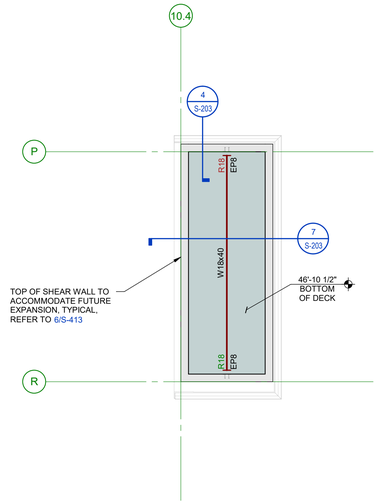


1 FRAMING PLAN - ROOF (FUTURE LEVEL 03) - AREA F
1/8" = 1'-0"

REFER TO S-103 FOR PLAN NOTES



2 FRAMING PLAN - EAST STAIR ROOF
1/8" = 1'-0"

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ORLANDO HEALTH MELBOURNE NEW FREESTANDING EMERGENCY CARE FACILITY

LAKE ANDREW DR & WILLET PLACE
MELBOURNE, FL 32940

ORLANDO HEALTH®

PROJECT MANAGER	HDR ARCHITECTURE, INC.
PROJECT DESIGNER	HDR ARCHITECTURE, INC.
PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
LANDSCAPE ARCHITECT	MCKINTOSH ASSOCIATES, an LJA COMPANY
CIVIL ENGINEER	BSE CONSULTANTS, an LJA COMPANY
STRUCTURAL ENGINEER	WALTER P. MOORE & ASSOCIATES, INC.
MECHANICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
ELECTRICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
PLUMBING ENGINEER	I.C. TOMASSON ASSOCIATES, INC
INTERIOR DESIGNER	HDR ARCHITECTURE, INC.
EQUIPMENT PLANNER	IMEG CORP.
WAYFINDING	HDR ARCHITECTURE, INC.

SHEET REVIEWER	

MARK	**DATE**	**DESCRIPTION**
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

CORY ANN DAYTON LICENSE No. 57441 STATE OF FLORIDA PROFESSIONAL ENGINEER

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Email: cdayton@walterpmoore.com

CORY ANN DAYTON, PE NO. 57441

Sheet Name

FRAMING PLAN - ROOF - AREA F

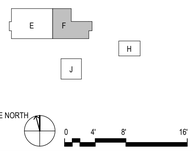
Sheet Number

S-103F

Project Status

CONSTRUCTION DOCUMENTS

KEY PLAN



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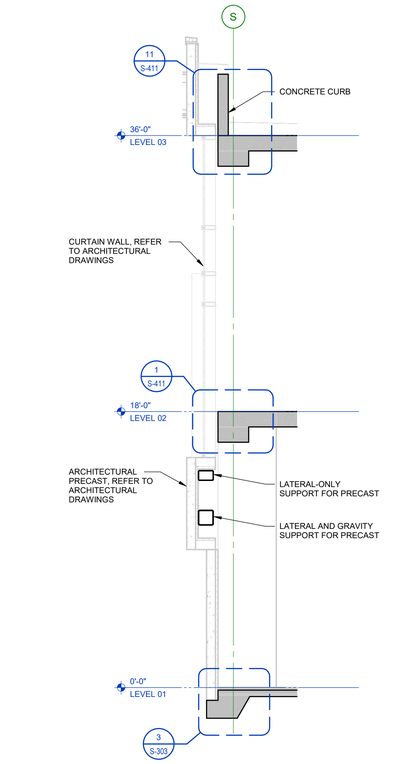

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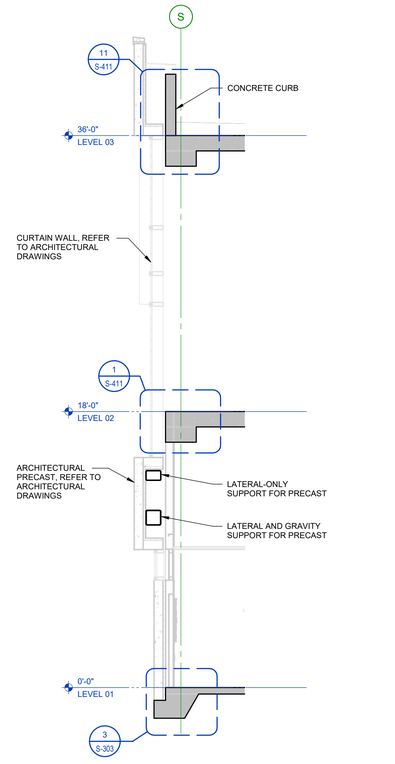
1/4" = 1'-0"



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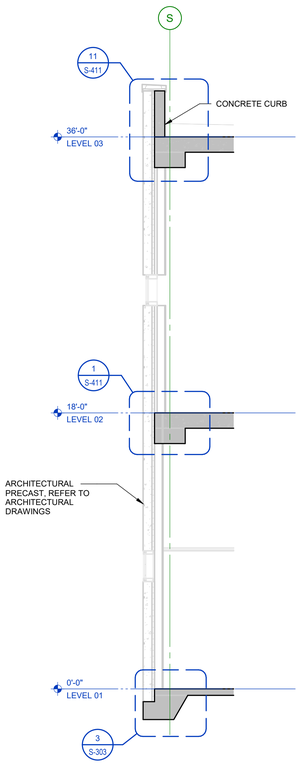

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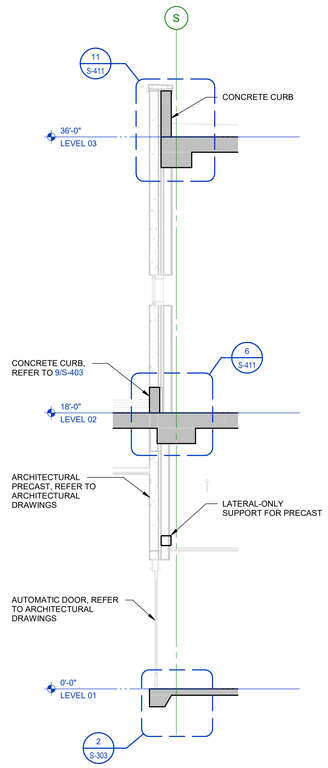
1/4" = 1'-0"



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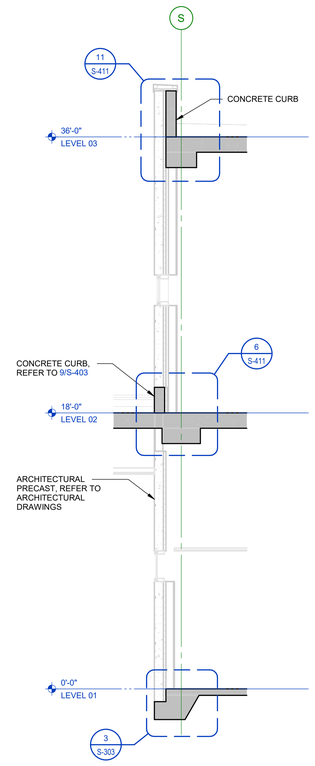
1/4" = 1'-0"



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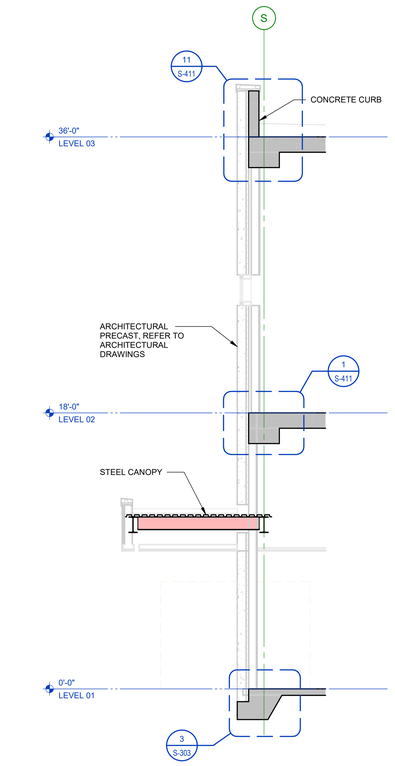
1/4" = 1'-0"



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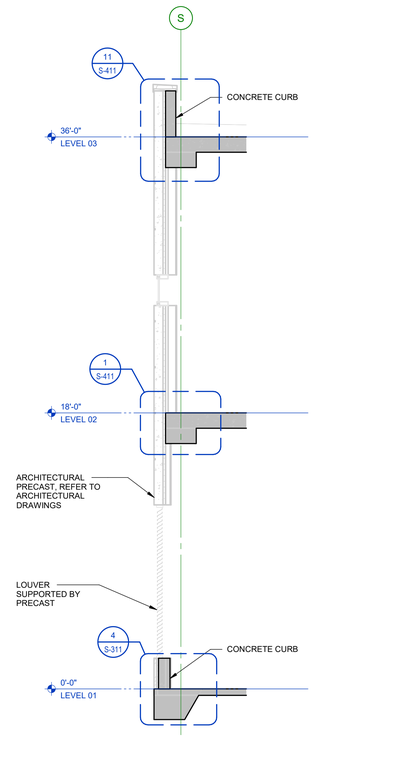
1/4" = 1'-0"



1 PARTIAL BUILDING SECTION
1/4" = 1'-0"

HDR ARCHITECTURE, INC.
4830 W KENNEDY BLVD #400
TAMPA, FL 33609
FL FIRM REG CERTIFICATION
AA-C000457

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201 East Kennedy Blvd, Suite 700
Tampa, Florida 33602
WPM Project No. S05-25024-00

813.221.2424
Florida DBPR License No. 3818

FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
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CORY ANN DAYTON, PE NO. 57441

Sheet Name

PARTIAL BUILDING SECTIONS

Sheet Number

S-201

Project Status

CONSTRUCTION DOCUMENTS

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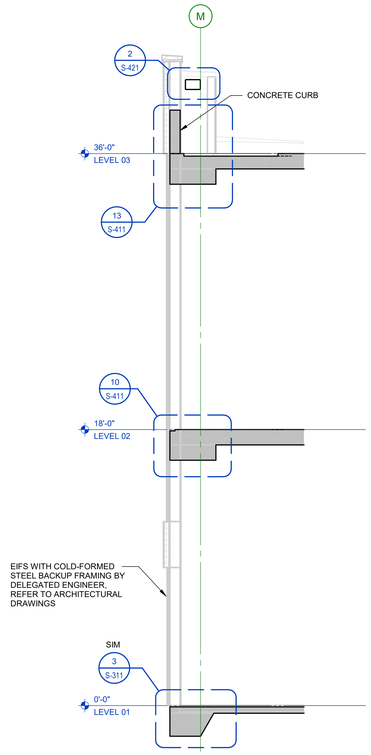
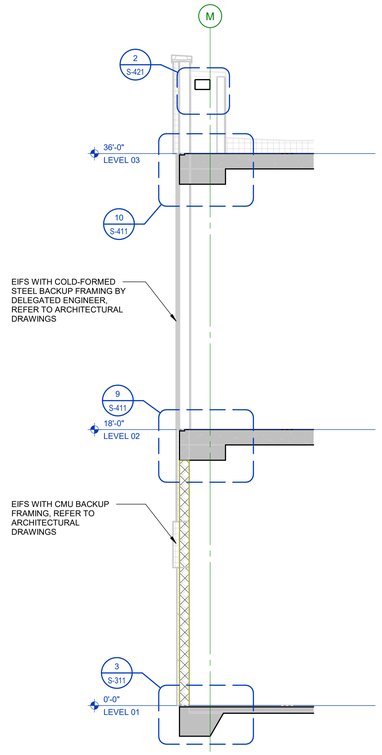


8 PARTIAL BUILDING SECTION
1/4" = 1'-0"



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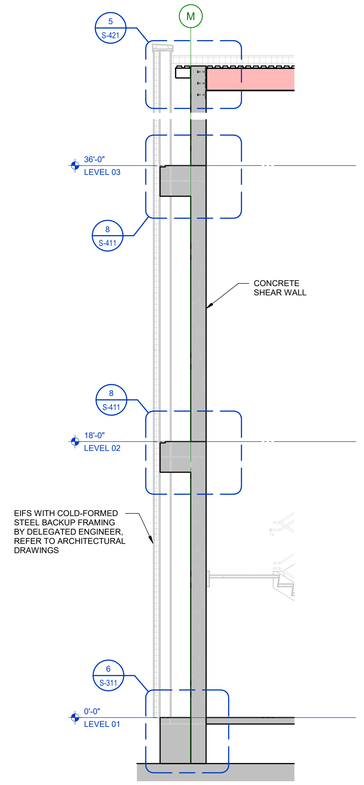
1/4" = 1'-0"



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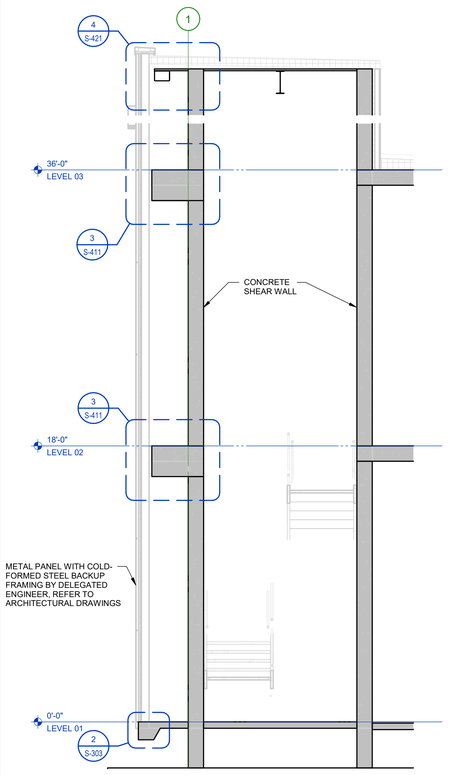

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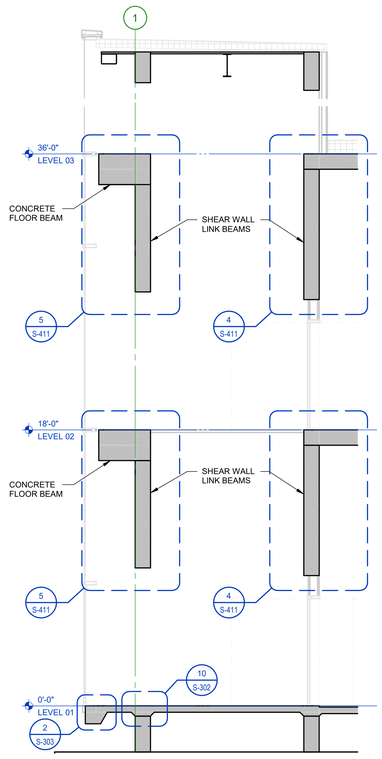
1/4" = 1'-0"



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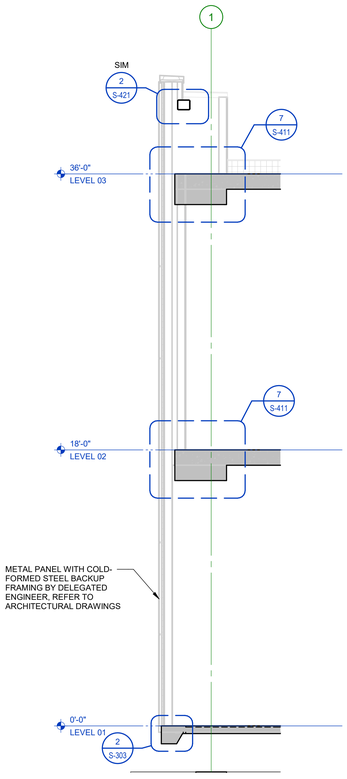

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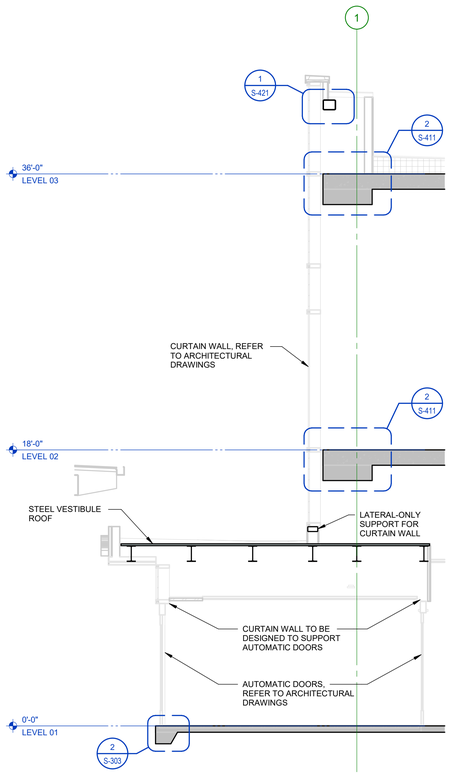

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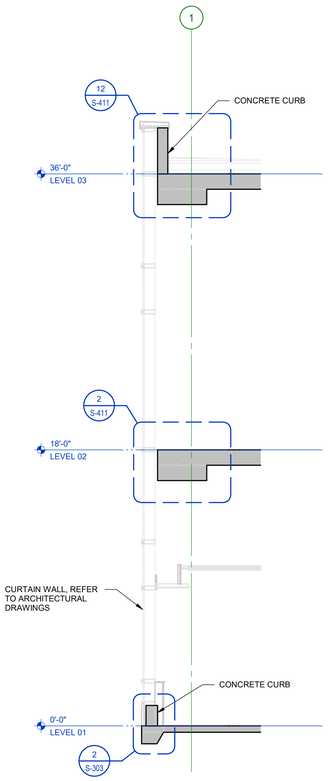

1 PARTIAL BUILDING SECTION
1/4" = 1'-0"

HDR ARCHITECTURE, INC.
4830 W KENNEDY BLVD #400
TAMPA, FL 33609
FL FIRM REG CERTIFICATION
AA-C000457

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201 East Kennedy Blvd, Suite 700
Tampa, Florida 33602
WPM Project No. S05-25024-00

813.221.2424
Florida DBPR License No. 3818

FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
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LAKE ANDREW DR & WILLET PLACE
MELBOURNE, FL 32940

ORLANDO HEALTH®

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PROJECT DESIGNER	HDR ARCHITECTURE, INC.
PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
LANDSCAPE ARCHITECT	MCKINTOSH ASSOCIATES, an LJA COMPANY
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SHEET REVIEWER	

MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
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CORY ANN DAYTON, PE NO. 57441

Sheet Name

PARTIAL BUILDING SECTIONS

Sheet Number

S-202

Project Status

CONSTRUCTION DOCUMENTS

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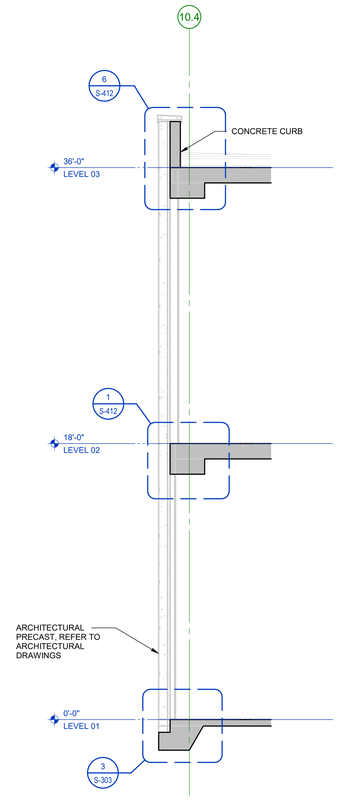
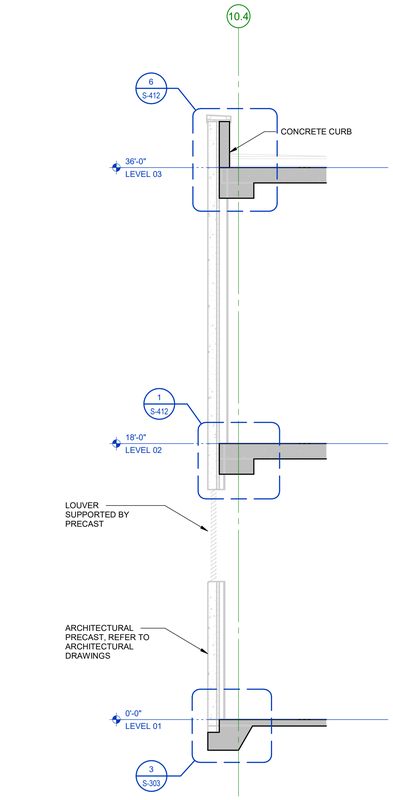


9 PARTIAL BUILDING SECTION
1/4" = 1'-0"



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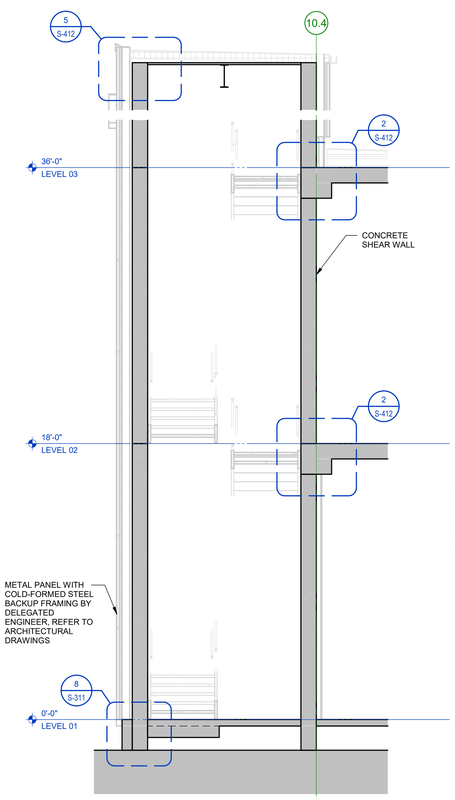

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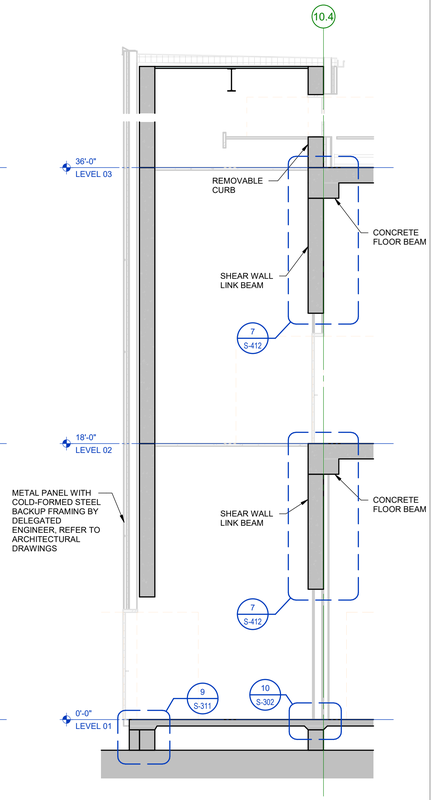

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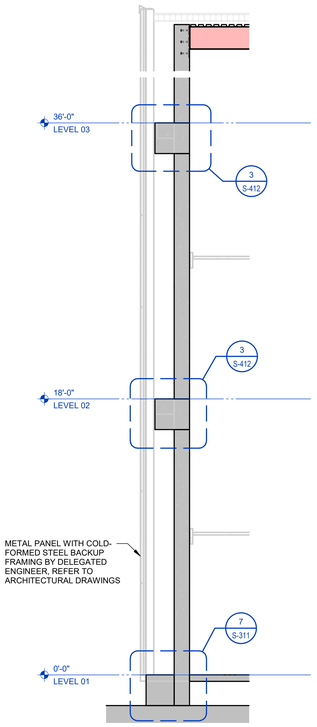

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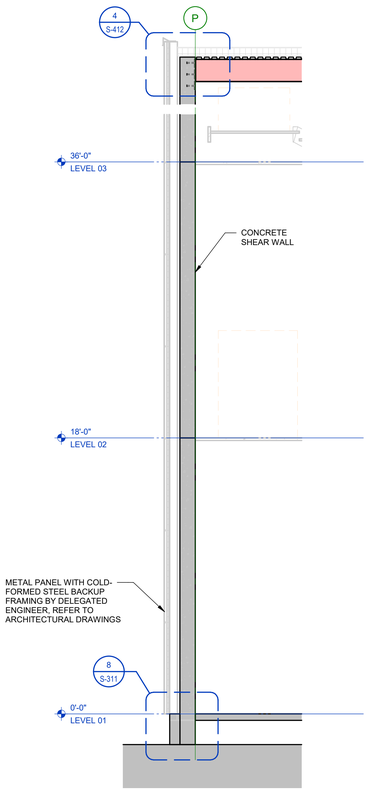

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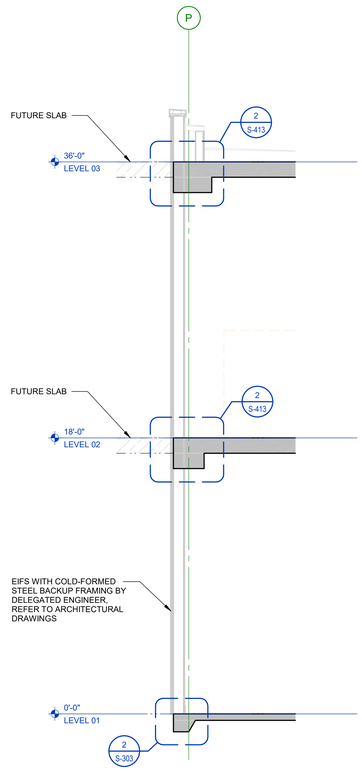

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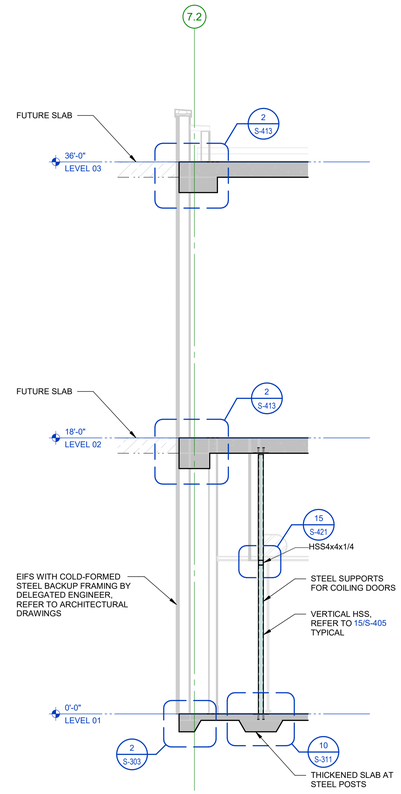

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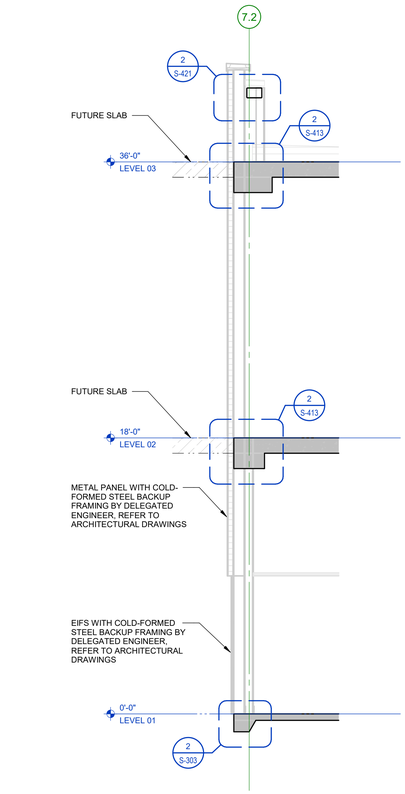

1 PARTIAL BUILDING SECTION
1/4" = 1'-0"

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CORY ANN DAYTON, PE NO. 57441

Sheet Name

PARTIAL BUILDING SECTIONS

Sheet Number

S-203

Project Status

CONSTRUCTION DOCUMENTS

0 1/2" 1" 2"

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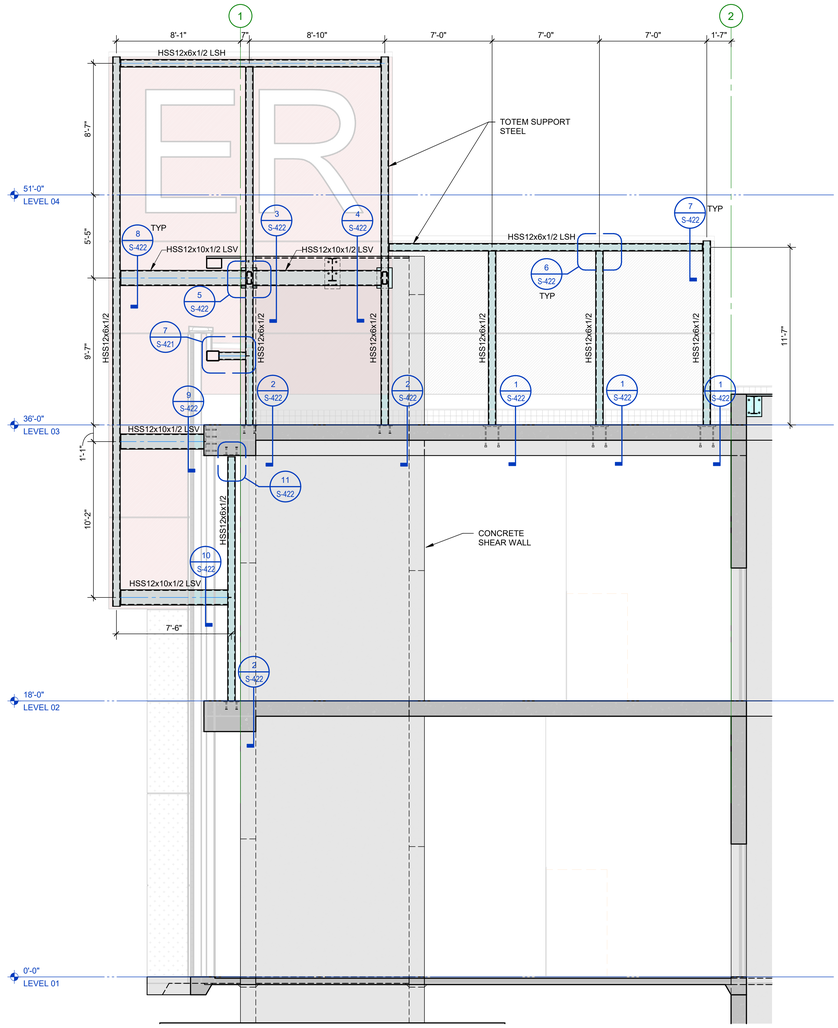

1 PARTIAL BUILDING SECTION

1/4" = 1'-0"

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Sheet Name

PARTIAL BUILDING SECTIONS

Sheet Number

S-204

Project Status

CONSTRUCTION DOCUMENTS

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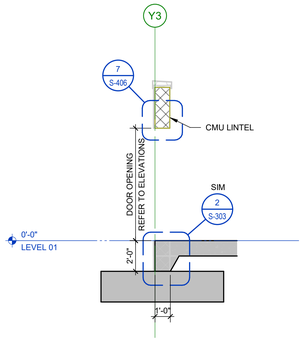
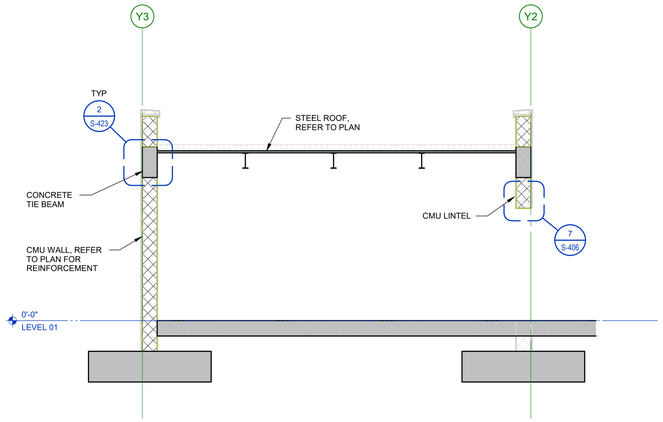
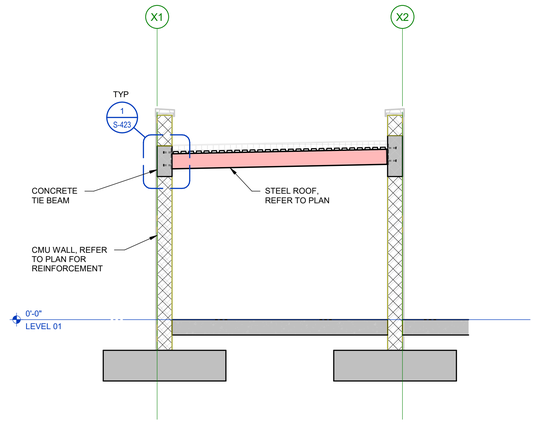


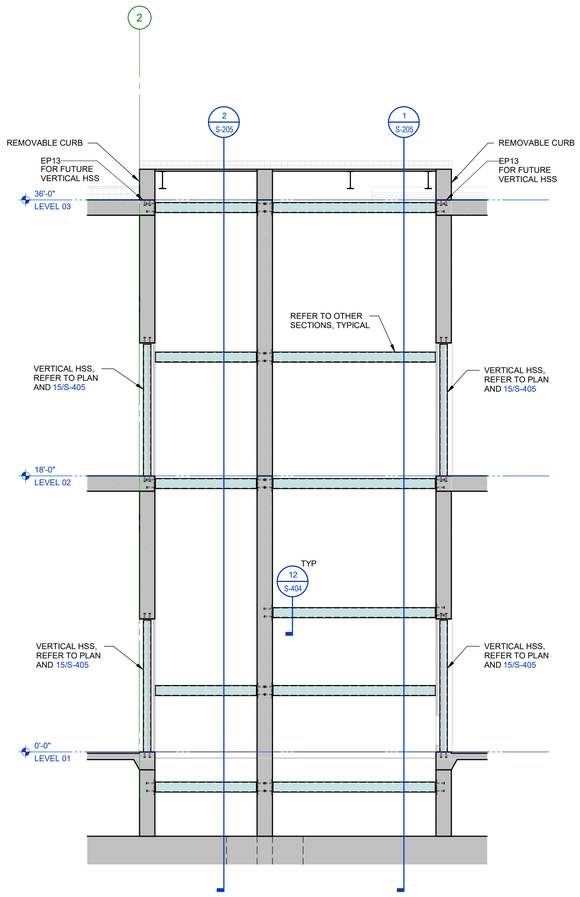
6 PARTIAL BUILDING SECTION - CHILLER YARD
1/4" = 1'-0"



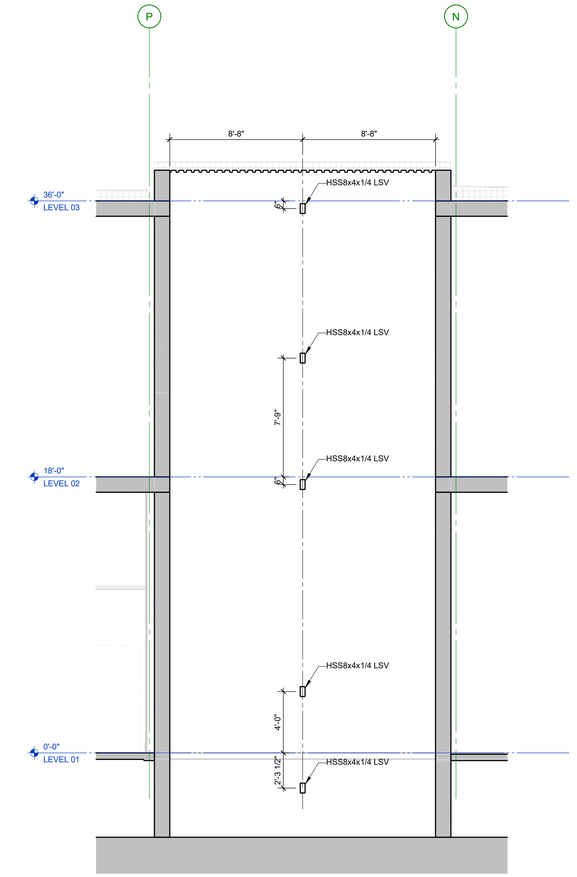
5 PARTIAL BUILDING SECTION - CHILLER YARD
1/4" = 1'-0"



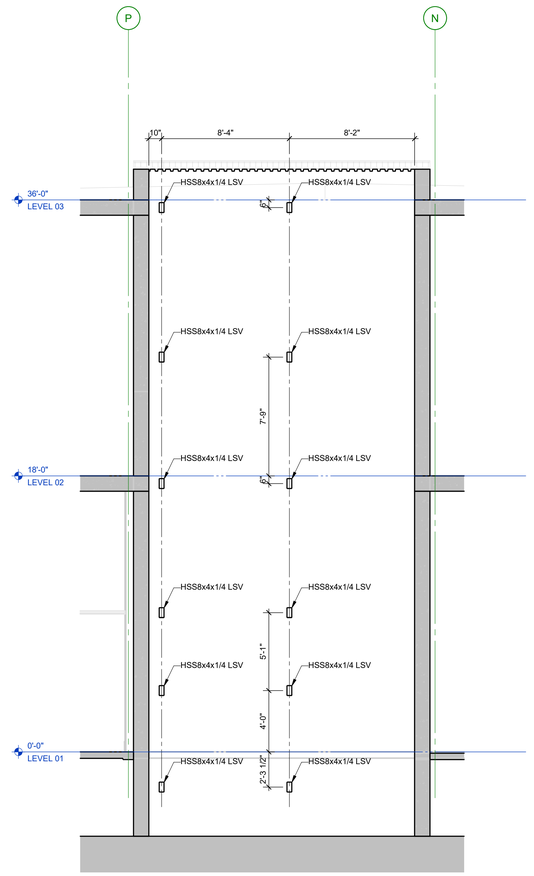
4 PARTIAL BUILDING SECTION - CHILLER YARD
1/4" = 1'-0"



3 PARTIAL BUILDING SECTION - ELEVATORS
1/4" = 1'-0"



2 PARTIAL BUILDING SECTION - PASSENGER ELEVATOR
1/4" = 1'-0"



1 PARTIAL BUILDING SECTION - SERVICE ELEVATOR
1/4" = 1'-0"

HDR ARCHITECTURE, INC.
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FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
TO THE BEST OF THE ARCHITECTS OR ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH THIS SECTION AND CHAPTER 633, FLORIDA STATUES.

ORLANDO HEALTH MELBOURNE NEW FREESTANDING EMERGENCY CARE FACILITY

LAKE ANDREW DR & WILLET PLACE
MELBOURNE, FL 32940

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PROJECT MANAGER	HDR ARCHITECTURE, INC.
PROJECT DESIGNER	HDR ARCHITECTURE, INC.
PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
LANDSCAPE ARCHITECT	MCKINTOSH ASSOCIATES, an LJA COMPANY
CIVIL ENGINEER	BSE CONSULTANTS, an LJA COMPANY
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INTERIOR DESIGNER	HDR ARCHITECTURE, INC.
EQUIPMENT PLANNER	IMEG CORP
WAYFINDING	HDR ARCHITECTURE, INC.

SHEET REVIEWER	

MARK	**DATE**	**DESCRIPTION**
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

CORY ANN DAYTON LICENSE No. 57441 STATE OF FLORIDA PROFESSIONAL ENGINEER

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Email: cdayton@walterpmoore.com

CORY ANN DAYTON, PE NO. 57441

Sheet Name

PARTIAL BUILDING SECTIONS

Sheet Number

S-205

Project Status

CONSTRUCTION DOCUMENTS

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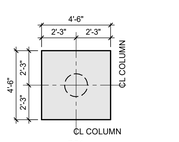
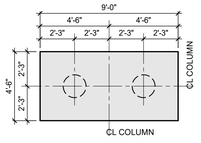


PC1
(1 PILE)



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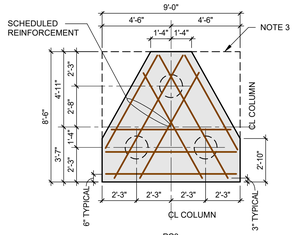
(2 PILES)



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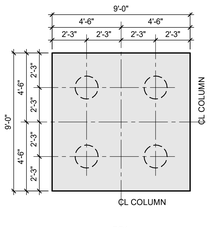
(3 PILES)



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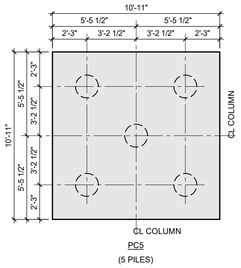
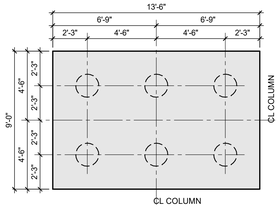
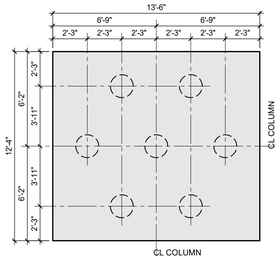
(4 PILES)



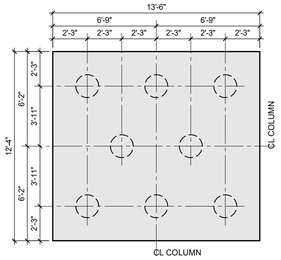
PC5
(5 PILES)



PC6
(6 PILES)



PC7
(7 PILES)



PC8
(8 PILES)

PILE CAP SCHEDULE

MARK	THICKNESS	REINFORCEMENT	PILE TYPE
PC3A	3'-8"	8-#7H 3-WAYS BOTTOM	TYPE A
PC3C	3'-8"	10-#7H 3-WAYS TOP AND BOTTOM	TYPE C
PC4A	3'-8"	17-#7H EACH DIRECTION BOTTOM	TYPE A
PC4B	3'-8"	17-#7H EACH DIRECTION BOTTOM	TYPE B
PC5A	3'-8"	17-#9H EACH DIRECTION BOTTOM	TYPE A
PC7B	4'-9"	16-#9H LONG DIRECTION TOP & BOTTOM 17-#9H SHORT DIRECTION TOP & BOTTOM	TYPE A

NOTES:
1. REFER TO PILE CAP SCHEDULE FOR CAP THICKNESS AND REINFORCEMENT.
2. REFER TO FOUNDATION PLAN FOR LOCATION OF COLUMNS. PILE AND CAP LAYOUT SHALL BE BASED ON CENTER LINE OF COLUMN UNLESS SHOWN OTHERWISE ON PLAN.
3. CORNERS MAY BE EXTENDED AS SHOWN DASHED WITHOUT REINFORCEMENT AT CONTRACTOR'S OPTION EXCEPT AT BUILDING EXTERIOR.
4. PLAN DETAILS WHICH ARE NOT SCHEDULED OR CALLED OUT ON PLAN ARE NOT TO BE USED ON THIS PROJECT.

3 TYPICAL PILE CAP PLANS - 18"Ø PILES SPACED AT 4'-6" MAXIMUM WITH 2'-3" EDGE DISTANCE

NO SCALE

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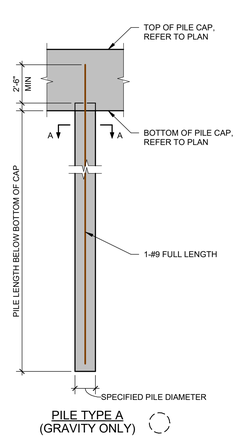
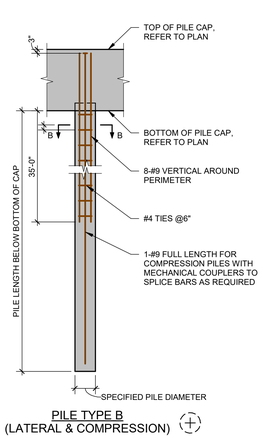
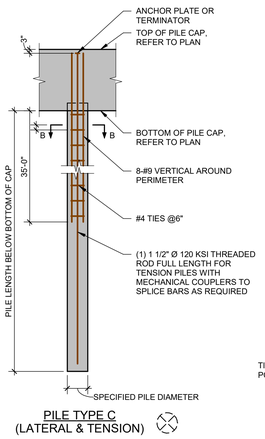


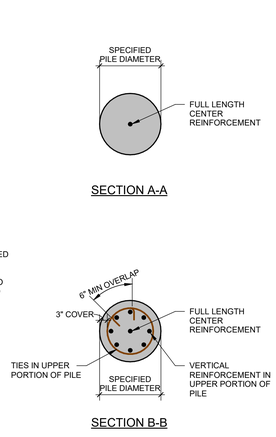
PILE TYPE A
(GRAVITY ONLY)



PILE TYPE B
(LATERAL & COMPRESSION)



PILE TYPE C
(LATERAL & TENSION)

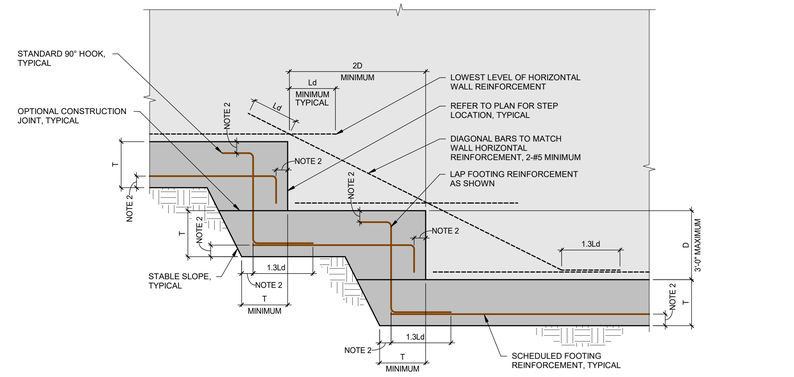


SECTION A-A

SECTION B-B

5 TYPICAL AUGERCAST PILES

NO SCALE



NOTES:
1. "T" IS THE THICKNESS OF THE CONTINUOUS WALL FOOTING. REFER TO PLAN OR DETAILS.
2. REFER TO OTHER DETAILS FOR CONCRETE CLEAR COVER REQUIREMENTS.

12 TYPICAL STEPPED CONCRETE WALL FOOTING

NO SCALE

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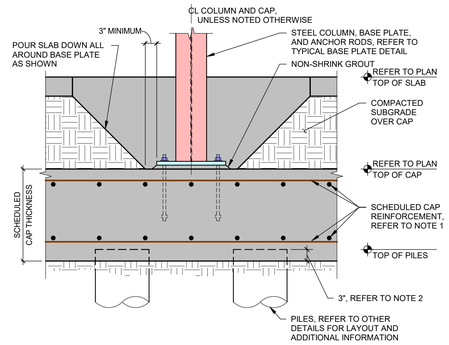
WALL FOOTING SCHEDULE

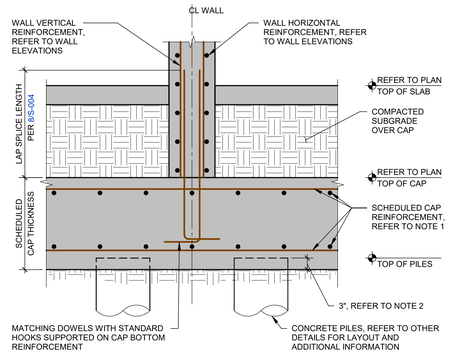
MARK	WIDTH	THICKNESS	REINFORCEMENT
WF96x24	8'-0"	2'-0"	#7H@12" EACH DIRECTION TOP AND BOTTOM



NOTES:
1. UNLESS NOTED OTHERWISE, PLACE LONG DIRECTION REINFORCEMENT IN OUTSIDE LAYER. REFER TO SCHEDULE FOR HOOK REQUIREMENTS.
2. CAP THICKNESS MUST BE INCREASED AS REQUIRED TO MAINTAIN A MINIMUM 3" EMBEDMENT FOR ALL DEEP FOUNDATION ELEMENTS. CONCRETE CONTRACTOR SHALL INCLUDE AN ALLOWANCE FOR AN ADDITIONAL 3" OF CAP THICKNESS TO ACCOMMODATE PILE CUT-OFF TOLERANCE OF -3".

7 WALL FOOTING SCHEDULE

3/4" = 1'-0"



NOTES:
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6 TYPICAL STEEL COLUMN ON CAP

NO SCALE

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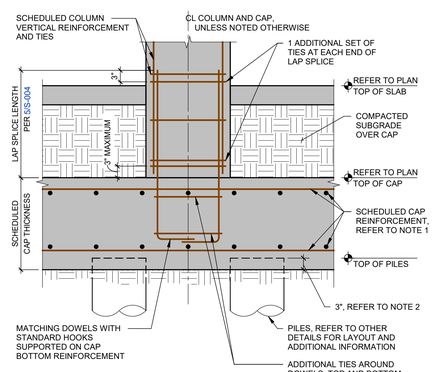

NOTES:
1. UNLESS NOTED OTHERWISE, PLACE LONG DIRECTION REINFORCEMENT IN OUTSIDE LAYER. REFER TO SCHEDULE FOR HOOK REQUIREMENTS.
2. CAP THICKNESS MUST BE INCREASED AS REQUIRED TO MAINTAIN A MINIMUM 3" EMBEDMENT FOR ALL DEEP FOUNDATION ELEMENTS. CONCRETE CONTRACTOR SHALL INCLUDE AN ALLOWANCE FOR AN ADDITIONAL 3" OF CAP THICKNESS TO ACCOMMODATE PILE CUT-OFF TOLERANCE OF -3".

2 TYPICAL CONCRETE WALL ON PILE CAP

NO SCALE

1 TYPICAL CONCRETE COLUMN ON CAP

NO SCALE

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MELBOURNE, FL 32940

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SHEET REVIEWER

MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

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CORY ANN DAYTON, PE NO. 57441

Sheet Name

FOUNDATION TYPICAL DETAILS

Sheet Number

S-301

Project Status

CONSTRUCTION DOCUMENTS

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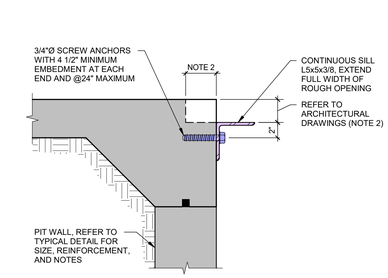

NOTES:
1. PROVIDE CONTINUOUS ELEVATOR SILL ANGLE AT THRESHOLD OF ELEVATOR OPENINGS IF REQUIRED BY ELEVATOR MANUFACTURER. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS.
2. VERIFY SIZE AND APPLICABILITY OF BLOCKOUT WITH ARCHITECTURAL AND ELEVATOR MANUFACTURER'S DRAWINGS.

13 TYPICAL ELEVATOR SILL ANGLE CONNECTION TO PIT WALL
NO SCALE

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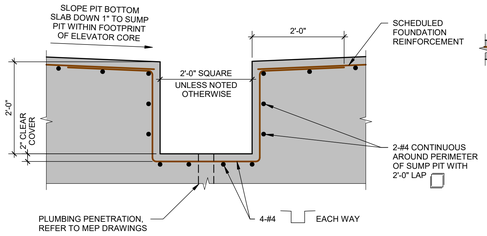
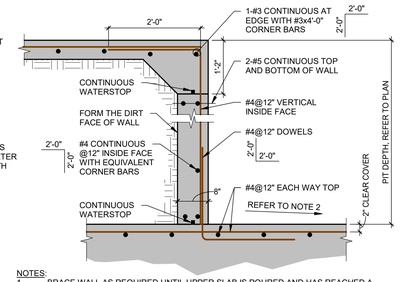


NOTES:
1. REFER TO PLAN FOR LOCATION AND SIZE.
2. REFER TO MEP AND ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION AND FINAL SIZE / LOCATION.

12 TYPICAL SUMP PIT IN FOUNDATION
NO SCALE



NOTES:
1. BRACE WALL AS REQUIRED UNTIL UPPER SLAB IS POURED AND HAS REACHED A STRENGTH OF 2500 PSI MINIMUM.
2. REFER TO PLAN FOR SUMP PIT, WHERE REQUIRED. SLOPE PIT BOTTOM SLAB 1" TO SUMP PIT.
3. REFER TO OTHER DETAILS FOR ELEVATOR SILL ANGLE AND DIVIDER BEAM CONNECTIONS, WHERE APPLICABLE.
4. GENERAL CONTRACTOR TO COORDINATE ELEVATOR PIT SIZE, DEPTH, SILL DETAIL, AND BLOCKOUT REQUIREMENTS WITH ELEVATOR MANUFACTURER'S APPROVED SHOP DRAWINGS.
5. WHERE ELEVATOR PIT SLAB IS BLOCKED OUT TO ALLOW FOR PLACEMENT OF HYDRAULIC ELEVATOR JACK, FILL BLOCKOUT WITH CONCRETE AFTER JACK IS SET.

11 TYPICAL ELEVATOR PIT
NO SCALE

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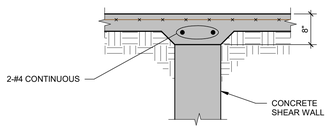
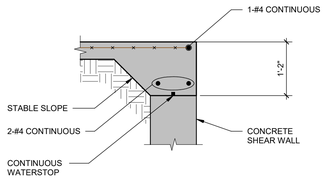
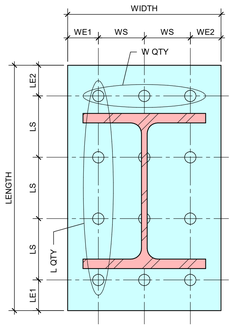




10 TYPICAL SLAB-ON-GRADE TURNDOWN AT SHEAR WALL OPENING
NO SCALE

9 TYPICAL SLAB-ON-GRADE TURNDOWN AT SHEAR WALL OPENING
NO SCALE

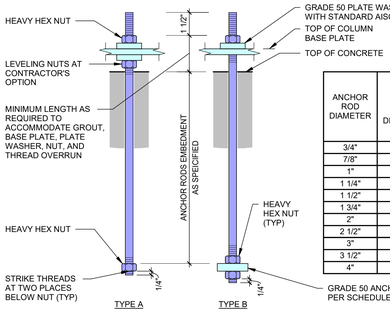


BASE PLATE SCHEDULE

MARK	BASE PLATE WIDTH	BASE PLATE LENGTH	BASE PLATE THICKNESS	ANCHORS DIAMETER	ANCHORS EMBEDMENT	ANCHORS ANCHOR GRADE	ANCHORS TYPE	ANCHORS TOTAL QTY	WIDTH SIDE WE1	WIDTH SIDE WS	WIDTH SIDE WE2	WIDTH SIDE W QTY	LENGTH SIDE LE1	LENGTH SIDE LS	LENGTH SIDE LE2	LENGTH SIDE L QTY	GROUT THICKNESS
BP1	1'-4"	1'-4"	3/4"	3/4"	1'-0"	55	A	4	2"	12"	2"	2	2"	12"	2"	2	0'-1"

NOTES:
1. ALL BASE PLATES SHALL BE A-572 GRADE 50, UNLESS NOTED OTHERWISE.
2. ALL ANCHOR RODS SHALL BE ASTM F 1554 GRADE 55, UNLESS NOTED OTHERWISE.
3. REFER TO 3/S-302 FOR GROUT THICKNESS SCHEDULE.

8 BASE PLATE SCHEDULE
NO SCALE



ANCHOR ROD DIAMETER	BASE PLATE HOLE DIAMETER	CIRCULAR OR SQUARE PLATE WASHER SIZE	F1554 GRADE 36 PLATE WASHER THICKNESS	F1554 GRADE 36 ANCHOR PLATE SIZE	F1554 GRADE 55 PLATE WASHER THICKNESS	F1554 GRADE 55 ANCHOR PLATE SIZE	F1554 GRADE 105 PLATE WASHER THICKNESS	F1554 GRADE 105 ANCHOR PLATE SIZE
3/4"	1 5/16"	2"	1/4"		1/4"	3/8"x1 1/2"x1 1/2"	3/8"	5/8"x1 3/4"x1 3/4"
7/8"	1 9/16"	2 1/2"	3/8"		3/8"	3/8"x1 3/4"x1 3/4"	1/2"	3/4"x2"x2"
1"	1 7/8"	3"	3/8"		3/8"	1/2"x2"x2"	1/2"	7/8"x2 1/4"x2 1/4"
1 1/4"	2 1/8"	3 1/2"	3/8"		1/2"	5/8"x2 1/2"x2 1/2"	5/8"	1"x2 3/4"x2 3/4"
1 1/2"	2 3/8"	4"	3/8"	ANCHOR PLATE NOT NEEDED	1/2"	3/4"x2 3/4"x2 3/4"	5/8"	1 1/4"x3 1/4"x3 1/4"
1 3/4"	2 7/8"	4 1/2"	3/4"		3/4"	7/8"x3 1/4"x3 1/4"	3/4"	1 1/2"x3 3/4"x3 3/4"
2"	3 1/4"	5"	3/4"		3/4"	1"x3 3/4"x3 3/4"	3/4"	1 3/4"x4 1/4"x4 1/4"
2 1/2"	3 3/4"	5 1/2"	3/4"		3/4"	1 1/4"x4 1/2"x4 1/2"	7/8"	2 1/4"x5 1/4"x5 1/4"
3"	4 1/4"	6 1/2"	1"		1 1/4"	1 1/2"x5 1/2"x5 1/2"	1 1/4"	2 3/4"x6 1/2"x6 1/2"
3 1/2"	4 3/4"	7"	1 1/4"	1 1/4"x6 1/4"x6 1/4"	1 1/2"	1 3/4"x6 1/4"x6 1/4"	N/A	N/A
4"	5 1/4"	8"	1 1/2"	1 1/4"x7 1/4"x7 1/4"	1 3/4"	2"x7 1/4"x7 1/4"	N/A	N/A

NOTES:
1. REFER TO COLUMN OR BASE PLATE SCHEDULE FOR ANCHOR ROD SIZE, TYPE, AND EMBEDMENT. ALL ANCHOR RODS SHALL BE TYPE A UNLESS NOTED OTHERWISE.
2. UNLESS NOTED OTHERWISE, ALL ANCHOR ROD NUTS SHALL BE TIGHTENED TO A "SNUG-TIGHT" CONDITION AS DEFINED BY AISC AFTER THE CONCRETE IS AT LEAST 14 DAYS OLD.
3. ANCHOR PLATE IS NOT REQUIRED AT TYPE A ANCHOR RODS.

4 TYPICAL ANCHOR ROD TYPE A AND TYPE B (f'c>=4000 PSI)
NO SCALE

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GROUT THICKNESS SCHEDULE		
MINIMUM BASE PLATE PLAN DIMENSION	MINIMUM GROUT THICKNESS	MAXIMUM THICKNESS
18" OR LESS	1"	2"
19" TO 24"	1 1/2"	2"
25" TO 30"	2"	2 1/2"
31" TO 36"	2 1/2"	3"
37" TO 42"	3"	3 1/2"
43" TO 48"	3 1/2"	4"
GREATER THAN 48"	REFER TO NOTE 2	4"

NOTES:
1. REFER TO SPECIFICATIONS AND GENERAL NOTES FOR GROUT REQUIREMENTS.
2. ADD 1/2" GROUT THICKNESS FOR EACH ADDITIONAL 12" OF MINIMUM PLATE DIMENSION UP TO MAXIMUM THICKNESS SPECIFIED.

3 TYPICAL GROUT THICKNESS SCHEDULE
NO SCALE

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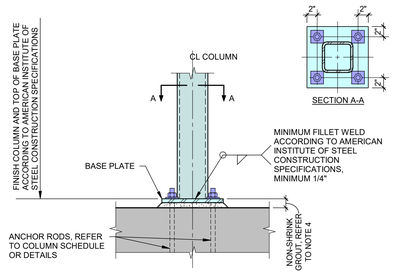

NOTES:
1. USE OVERSIZED HOLES FOR ANCHOR RODS ACCORDING TO AMERICAN INSTITUTE OF STEEL CONSTRUCTION.
2. PLATE WASHERS WITH STANDARD HOLES SHALL BE INSTALLED OVER OVERSIZED HOLES.
3. CONTRACTOR SHALL PROVIDE TEMPLATE FOR ANCHOR ROD INSTALLATION.
4. MINIMUM GROUT THICKNESS IS 1" UNLESS NOTED OTHERWISE ON PLANS, DETAILS, OR IN GROUT THICKNESS SCHEDULE.

2 TYPICAL RECTANGULAR/SQUARE HSS COLUMN BASE CONNECTION
NO SCALE

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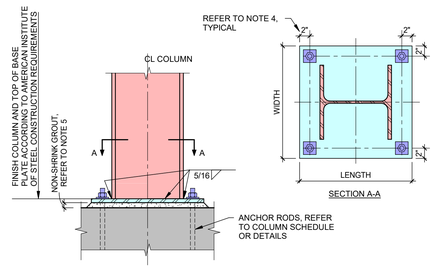

NOTES:
1. USE OVERSIZED HOLES FOR ANCHOR RODS ACCORDING TO AMERICAN INSTITUTE OF STEEL CONSTRUCTION.
2. PLATE WASHERS WITH STANDARD HOLES SHALL BE INSTALLED OVER OVERSIZED HOLES.
3. CONTRACTOR SHALL PROVIDE TEMPLATE FOR ANCHOR ROD INSTALLATION.
4. INCREASE EDGE DISTANCE TO 3" MINIMUM FOR ANCHOR ROD SIZES OF 1 1/2"Ø OR GREATER. CONFORM TO MINIMUM EDGE DISTANCE REQUIREMENTS.
5. MINIMUM GROUT THICKNESS IS 1" UNLESS NOTED OTHERWISE ON PLANS, DETAILS, OR IN GROUT THICKNESS SCHEDULE.

1 TYPICAL WIDE FLANGE COLUMN BASE CONNECTION
NO SCALE

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FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
TO THE BEST OF THE ARCHITECTS OR ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH THIS SECTION AND CHAPTER 633, FLORIDA STATUES.

ORLANDO HEALTH MELBOURNE NEW FREESTANDING EMERGENCY CARE FACILITY

LAKE ANDREW DR & WILLET PLACE
MELBOURNE, FL 32940

ORLANDO HEALTH®

PROJECT MANAGER	HDR ARCHITECTURE, INC.
PROJECT DESIGNER	HDR ARCHITECTURE, INC.
PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
LANDSCAPE ARCHITECT	MCKINTOSH ASSOCIATES, an LJA COMPANY
CIVIL ENGINEER	BSE CONSULTANTS, an LJA COMPANY
STRUCTURAL ENGINEER	WALTER P. MOORE & ASSOCIATES, INC.
MECHANICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
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INTERIOR DESIGNER	HDR ARCHITECTURE, INC.
EQUIPMENT PLANNER	IMEG CORP.
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SHEET REVIEWER	

MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/20226 AHCA STAGE III

CORY ANN DAYTON, STATE OF FLORIDA, LICENSE No. 57441, PROFESSIONAL ENGINEER

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CORY ANN DAYTON, PE NO. 57441

Sheet Name
FOUNDATION TYPICAL DETAILS

Sheet Number
S-302

Project Status
CONSTRUCTION DOCUMENTS

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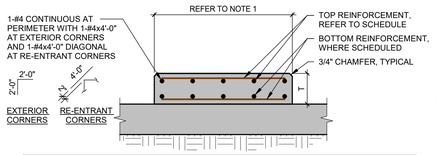

HOUSEKEEPING PAD REINFORCEMENT SCHEDULE		
PAD THICKNESS	TOP REINFORCEMENT	BOTTOM REINFORCEMENT
T = 4"	WWR4x4-W2.9xW2.9 OR #3@12" EACH WAY	NONE
4" < T <= 6"	WWR4x4-W4xW4 OR #4@18" EACH WAY	NONE
6" < T <= 8"	WWR4x4-W6xW6 OR #4@12" EACH WAY	NONE
8" < T <= 12"	#4@18" EACH WAY	#4@18" EACH WAY
12" < T <= 16"	#4@12" EACH WAY	#4@12" EACH WAY

NOTES:
1. REFER TO ARCHITECTURAL, STRUCTURAL, OR MEP DRAWINGS FOR HOUSEKEEPING PAD PLAN DIMENSIONS AND THICKNESS (4" MINIMUM THICKNESS).
2. CONTRACTOR SHALL COORDINATE DIMENSIONS AND OTHER SPECIAL REQUIREMENTS WITH EQUIPMENT MANUFACTURERS AND PROVIDE WHERE REQUIRED WHETHER SHOWN ON STRUCTURAL DRAWINGS OR NOT.

14 TYPICAL HOUSEKEEPING PAD OVER SLAB-ON-GRADE

NO SCALE

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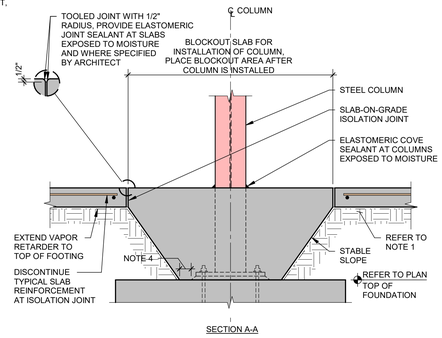
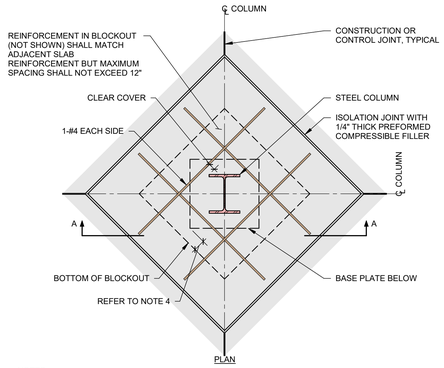


SECTION A-A



PLAN

NOTES:
1. REFER TO 1/S-303 FOR SUBGRADE PREPARATION REQUIREMENTS INCLUDING LOCATION OF VAPOR RETARDER RELATIVE TO SLAB AND GRANULAR BASE AND WHETHER FINE-GRADE GRANULAR MATERIAL IS REQUIRED.
2. REFER TO 10/S-303 FOR CONSTRUCTION AND CONTROL JOINT REQUIREMENTS.
3. CONTRACTOR TO COORDINATE REQUIRED SIZE OF BLOCKOUT FOR STRUCTURAL STEEL COLUMNS WITH STEEL ERECTOR. SUBMIT THE DESIRED BLOCKOUT SIZE TO ARCHITECT FOR APPROVAL.
4. BLOCKOUT SHALL BE SIZED TO PROVIDE 3" MINIMUM CONCRETE COVER ALL AROUND COLUMN AND BASE PLATE.

8 TYPICAL SLAB-ON-GRADE BLOCKOUT AROUND STEEL COLUMNS

NO SCALE

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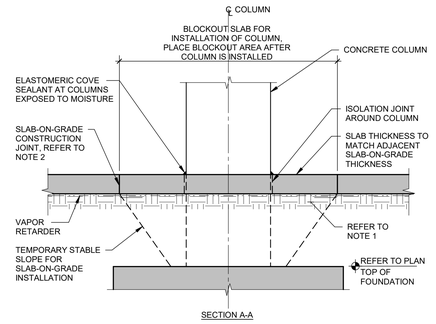

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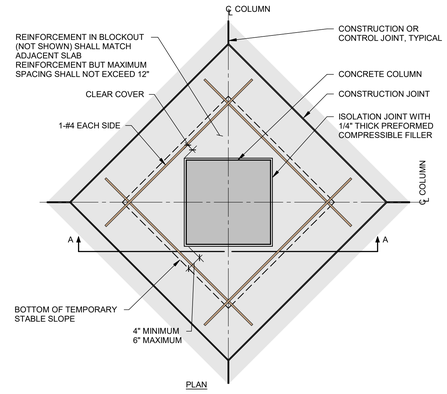

PLAN

NOTES:
1. REFER TO 1/S-303 FOR SUBGRADE PREPARATION REQUIREMENTS.
2. REFER TO 10/S-303 FOR CONSTRUCTION AND CONTROL JOINT REQUIREMENTS.

7 TYPICAL SLAB-ON-GRADE BLOCKOUT AROUND CONCRETE COLUMNS

3/4" = 1'-0"

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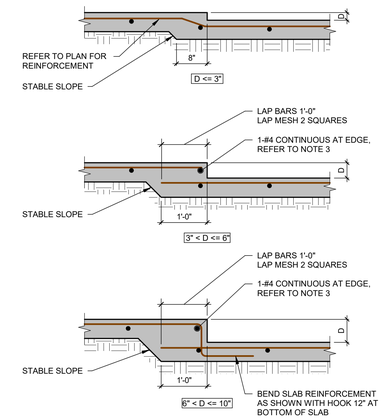

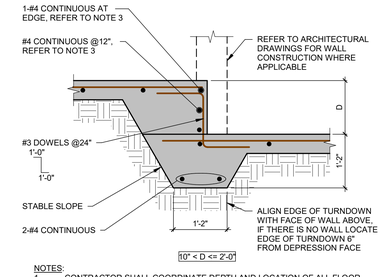

NOTES:
1. CONTRACTOR SHALL COORDINATE DEPTH AND LOCATION OF ALL FLOOR DEPRESSIONS WITH ARCHITECTURAL DRAWINGS.
2. PROVIDE ADDITIONAL 1-#4x4'-0" DIAGONAL TOP AT ALL RE-ENTRANT CORNERS.
3. PROVIDE #4x4'-0" CORNER BARS FOR ALL CONTINUOUS EDGE REINFORCEMENT THAT CAN NOT DEVELOP INTO ADJACENT SLAB.

6 TYPICAL SLAB-ON-GRADE DEPRESSIONS

NO SCALE

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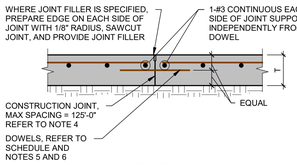
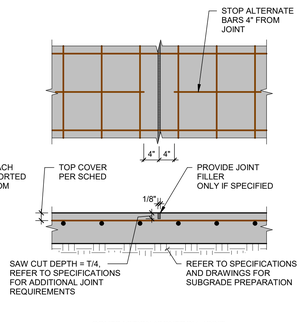
DOWEL AND JOINT SPACING SCHEDULE					
SLAB DEPTH T	TOP CLEAR COVER	DOWEL SIZE	DOWEL LENGTH	DOWEL SPACING	MAXIMUM JOINT SPACING
5"	1 3/4"	3/4"Ø	16"	12"	13'-0"
6"	2"	3/4"Ø	16"	12"	14'-6"



CONSTRUCTION JOINT



CONTRACTION (CONTROL) JOINT

NOTES:
1. REFER TO PLAN FOR SLAB THICKNESS (T) AND REINFORCEMENT.
2. SLAB REINFORCEMENT SHALL BE CHAIRED BY SOIL SUPPORTED SLAB BOLSTERS.
3. LONG STRIP CONSTRUCTION METHOD SHALL BE USED IN PLACING CONCRETE FOR ALL SLABS-ON-GRADE UNLESS NOTED OTHERWISE.
4. AT CONSTRUCTION JOINTS, BREAK BOND BETWEEN NEW AND PREVIOUSLY PLACED SLAB BY SPRAYING OR PAINTING EXPOSED SIDES OF JOINT WITH A CURING COMPOUND, BONDBREAKER, OR FORM OIL.
5. DOWELS SHOULD BE SMOOTH, ALIGNED, AND SUPPORTED SO THEY WILL REMAIN PARALLEL IN BOTH THE HORIZONTAL AND VERTICAL PLANES. DOWELS SHALL BE PLACED NO CLOSER THAN 12 INCHES FROM THE INTERSECTION OF JOINTS.
6. TO PREVENT A BOND BETWEEN THE DOWEL AND THE CONCRETE, LIGHTLY COAT THE EXPOSED END OF THE DOWELS WITH A PARAFFIN-BASED LUBRICANT, ASPHALT EMULSION, FORM OIL, OR GREASE IMMEDIATELY BEFORE PLACING CONCRETE ON THE SECOND SIDE OF THE JOINT. ALTERNATIVELY, USE A PLASTIC OR METAL SLEEVE SPECIFICALLY MANUFACTURED FOR THIS PURPOSE.
7. USE AN EARLY-ENTRY, DRY-CUT SAW TO CUT ALL SAWCUT CONTRACTION JOINTS WITHIN 1 TO 4 HOURS OF PLACEMENT, REFERENCE SPECIFICATIONS. PROVIDE CONTRACTION OR CONSTRUCTION JOINTS AT EVERY COLUMN LINE AND IN BETWEEN THE COLUMN LINES SUCH THAT THE JOINT SPACING DOES NOT EXCEED TABULATED DISTANCES.
8. FOR SLAB THICKNESS IN BETWEEN TABULATED VALUES, USE DOWEL INFORMATION FOR THICKER SLAB. MAXIMUM JOINT SPACING SHALL BE INTERPOLATED.
9.

10 TYPICAL CONSTRUCTION AND CONTROL JOINTS IN MILD REINFORCED SLAB-ON-GRADE

NO SCALE

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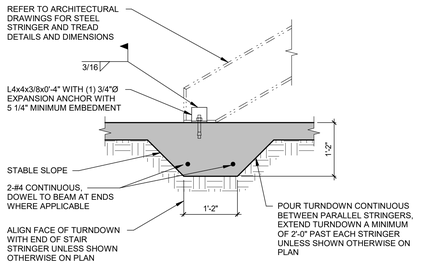

NOTES:
1. CONTRACTOR SHALL COORDINATE STAIR STRINGER LOCATIONS WITH ARCHITECTURAL DRAWINGS AND PROVIDE TURNDOWNS WHERE REQUIRED WHETHER SHOWN ON STRUCTURAL DRAWINGS OR NOT.
2. ATTACHMENT ANGLE SHALL BE HIDDEN BENEATH FIRST TREAD WHERE OPEN RISERS ARE USED. COORDINATE LOCATION WITH THE ARCHITECT.

5 TYPICAL SLAB-ON-GRADE TURNDOWN AT STEEL STAIR

NO SCALE

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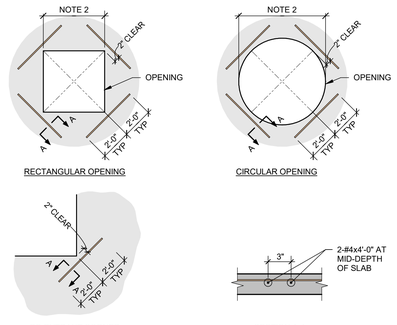

NOTES:
1. ADDITIONAL REINFORCEMENT SHALL NOT CROSS SLAB-ON-GRADE CONSTRUCTION AND CONTROL JOINTS. SKEW OR OMIT ADDITIONAL REINFORCEMENT AS REQUIRED.
2. COORDINATE OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
3. OPENINGS OR GROUPS OF OPENINGS 10" AND SMALLER DO NOT REQUIRE ADDITIONAL REINFORCEMENT. FOR RECTANGULAR OPENINGS, USE LARGER DIMENSION TO DETERMINE IF ADDITIONAL REINFORCEMENT IS REQUIRED.

4 TYPICAL SLAB-ON-GRADE OPENING AND CORNER REINFORCEMENT

NO SCALE

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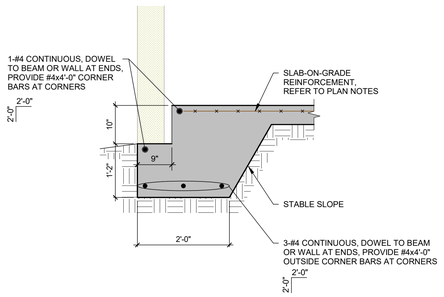

3 TYPICAL EXTERIOR SLAB-ON-GRADE TURNDOWN AT PRECAST

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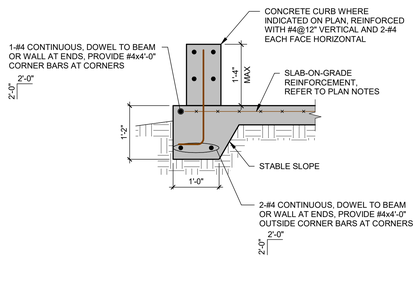

2 TYPICAL EXTERIOR SLAB-ON-GRADE TURNDOWN

NO SCALE

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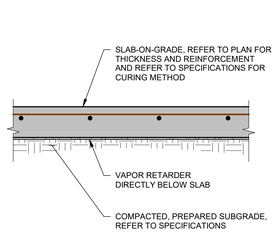

1 TYPICAL SLAB-ON-GRADE SUBGRADE PREPARATION

NO SCALE

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FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
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ORLANDO HEALTH MELBOURNE NEW FREESTANDING EMERGENCY CARE FACILITY

LAKE ANDREW DR & WILLET PLACE
MELBOURNE, FL 32940

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PROJECT DESIGNER	HDR ARCHITECTURE, INC.
PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
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SHEET REVIEWER	

MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

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Email: cdayton@walterpmoore.com

CORY ANN DAYTON, PE NO. 57441

Sheet Name

FOUNDATION TYPICAL DETAILS

Sheet Number

S-303

Project Status

CONSTRUCTION DOCUMENTS

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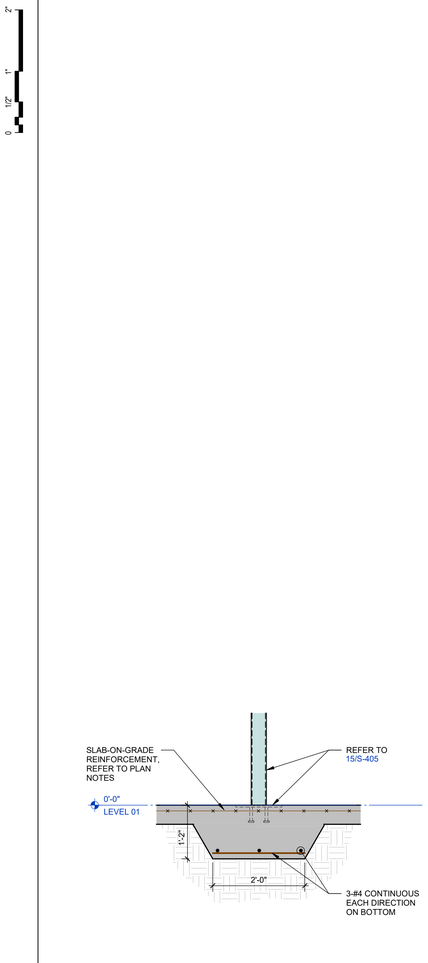
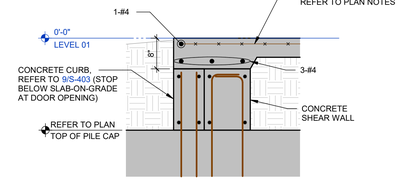
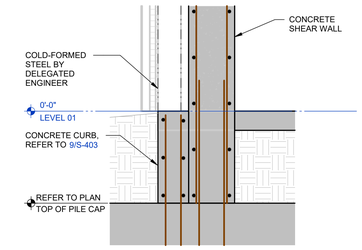


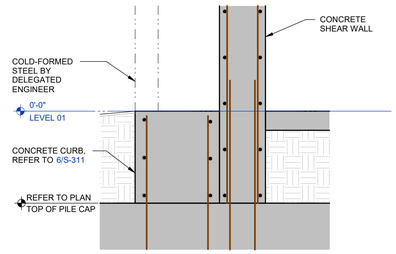
10 THICKENED SLAB-ON-GRADE TURNDOWN AT STEEL POST
3/4" = 1'-0"



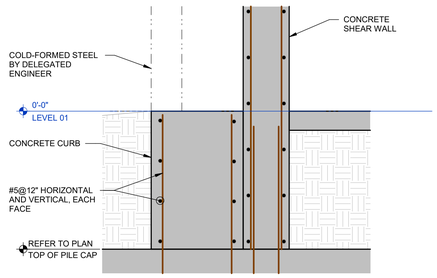
9 CURB AT SHEAR WALL DOOR OPENING
3/4" = 1'-0"



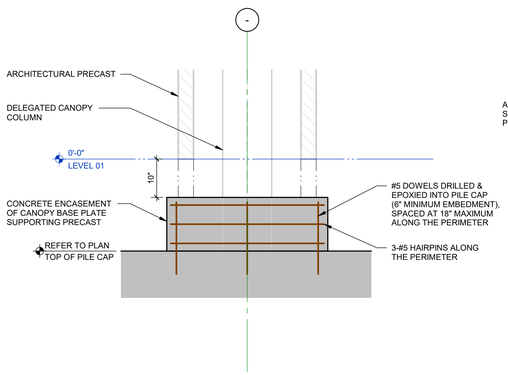
8 CURB AT SHEAR WALL
3/4" = 1'-0"



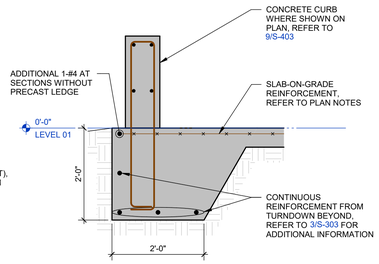
7 CURB AT SHEAR WALL
3/4" = 1'-0"



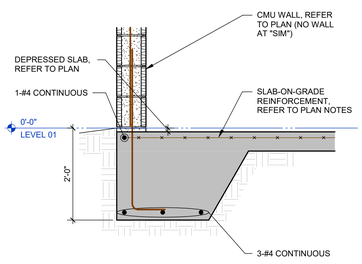
6 CURB AT SHEAR WALL
3/4" = 1'-0"



5 CONCRETE ENCASEMENT AT CANOPY BASE PLATE
3/4" = 1'-0"

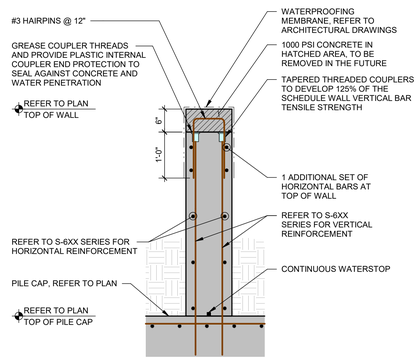


4 EXTERIOR SLAB-ON-GRADE TURNDOWN BEYOND PRECAST LEDGE
3/4" = 1'-0"

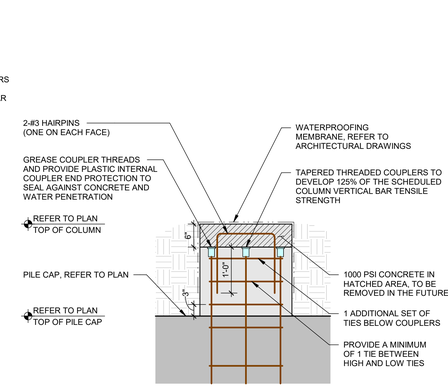


NOTES:
1. REFER TO 3/S-303 FOR ADDITIONAL INFORMATION.

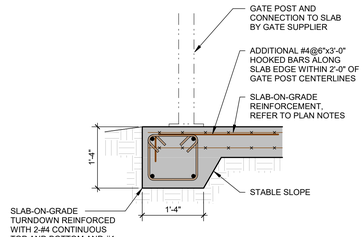
3 EXTERIOR SLAB-ON-GRADE TURNDOWN AT CMU
3/4" = 1'-0"



2 FUTURE SHEAR WALL CONNECTION
3/4" = 1'-0"



1 FUTURE COLUMN CONNECTION
3/4" = 1'-0"



11 EXTERIOR SLAB-ON-GRADE TURNDOWN FOR GATE POST
3/4" = 1'-0"

HDR ARCHITECTURE, INC.
4830 W KENNEDY BLVD #400
TAMPA, FL 33609
FL FIRM REG CERTIFICATION
AA-C000457

walter p moore

Walter P Moore and Associates, Inc.
201 East Kennedy Blvd, Suite 700
Tampa, Florida 33602
WPM Project No. S05-25024-00

813.221.2424
Florida DBPR License No. 3818

FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
TO THE BEST OF THE ARCHITECTS OR ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH THIS SECTION AND CHAPTER 633, FLORIDA STATUES.

ORLANDO HEALTH MELBOURNE NEW FREESTANDING EMERGENCY CARE FACILITY

LAKE ANDREW DR & WILLET PLACE
MELBOURNE, FL 32940

ORLANDO HEALTH®

PROJECT MANAGER	HDR ARCHITECTURE, INC.
PROJECT DESIGNER	HDR ARCHITECTURE, INC.
PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
LANDSCAPE ARCHITECT	MCKINTOSH ASSOCIATES, an LJA COMPANY
CIVIL ENGINEER	BSE CONSULTANTS, an LJA COMPANY
STRUCTURAL ENGINEER	WALTER P. MOORE & ASSOCIATES, INC.
MECHANICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
ELECTRICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
PLUMBING ENGINEER	I.C. TOMASSON ASSOCIATES, INC
INTERIOR DESIGNER	HDR ARCHITECTURE, INC.
EQUIPMENT PLANNER	IMEG CORP.
WAYFINDING	HDR ARCHITECTURE, INC.

SHEET REVIEWER	

MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

CORY ANN DAYTON LICENSE No. 57441 STATE OF FLORIDA PROFESSIONAL ENGINEER

Cory Ann Dayton, State of Florida, Professional Engineer, License No. 57441. This item has been digitally signed and sealed by Cory Ann Dayton, PE on the date indicated. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.
Email: cdayton@walterpmoore.com

CORY ANN DAYTON, PE NO. 57441

Sheet Name

FOUNDATION DETAILS

Sheet Number

S-311

Project Status

CONSTRUCTION DOCUMENTS

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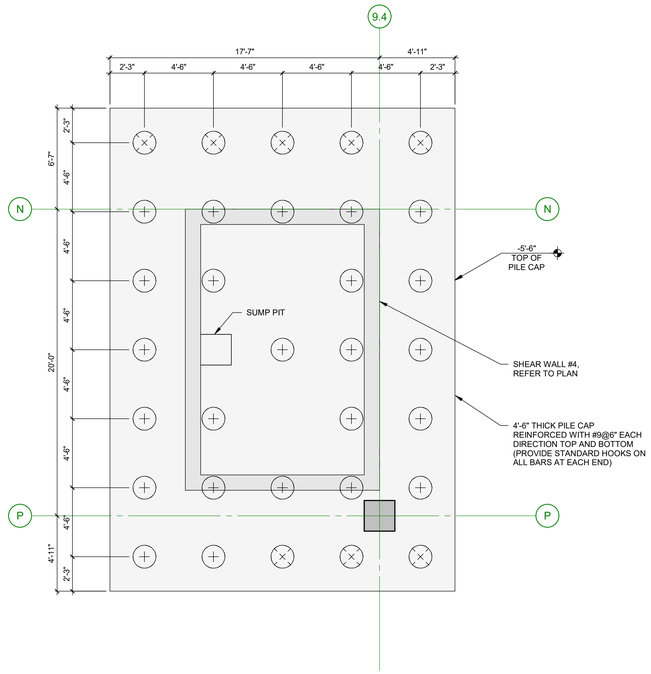
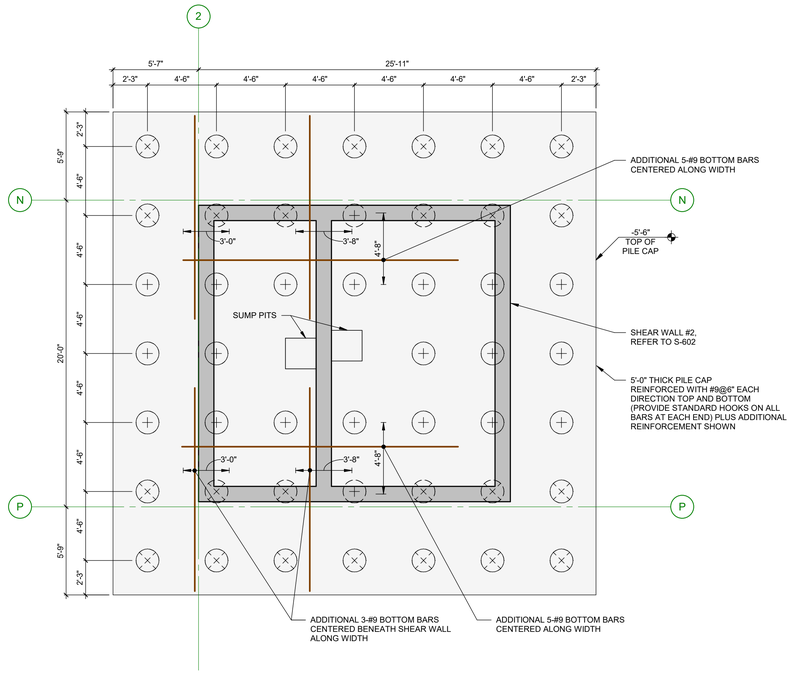


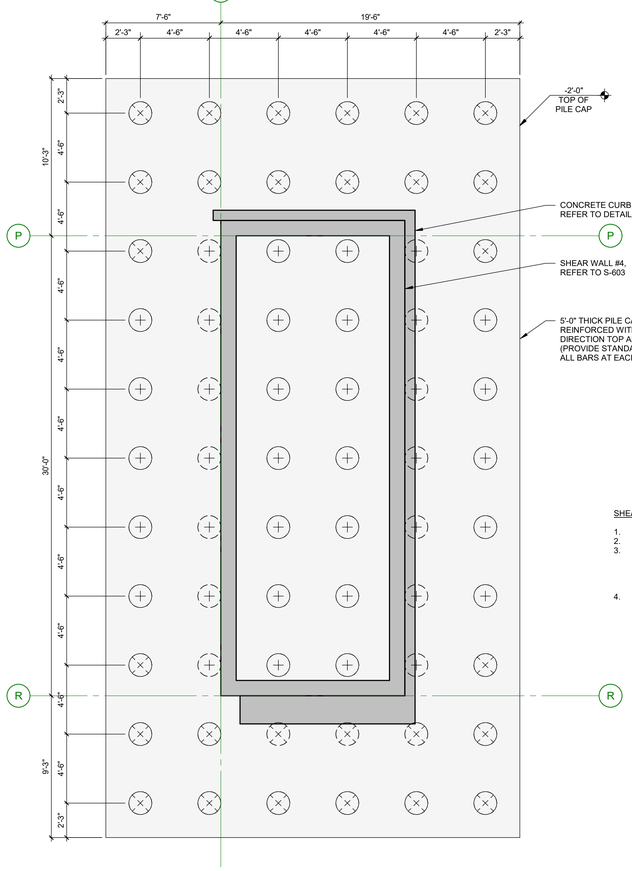
4 FOUNDATION PARTIAL PLAN - SHEAR WALL #4

1/4" = 1'-0"



2 FOUNDATION PARTIAL PLAN - SHEAR WALL #2

1/4" = 1'-0"



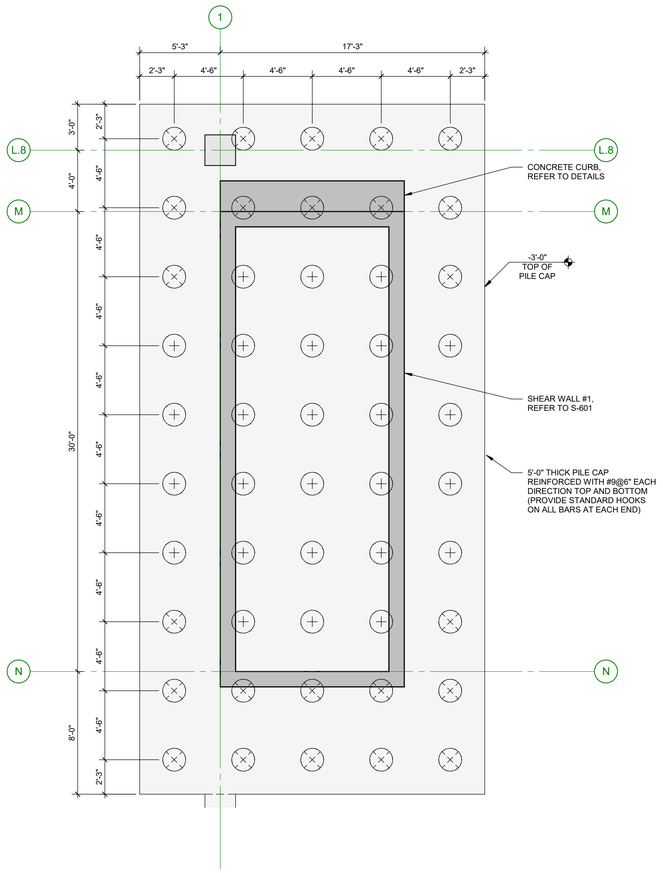
PILE LEGEND	
	PILE TYPE A, REFER TO 5/S-301 (GRAVITY ONLY)
	PILE TYPE B, REFER TO 5/S-301 (LATERAL & COMPRESSION)
	PILE TYPE C, REFER TO 5/S-301 (LATERAL & TENSION)

SHEAR WALL PILE CAP PLAN NOTES:

1. REFER TO 5/S-301 FOR TYPICAL AUGERCAST PILE DETAIL.
2. PROVIDE 90 DEGREE HOOKS FOR ALL REINFORCING STEEL BARS, UNLESS NOTED OTHERWISE.
3. PILE CAP REINFORCING STEEL PLACEMENT SEQUENCE SHALL BE AS FOLLOWS:
   A. PLAN NORTH-SOUTH BOTTOM REINFORCING STEEL
   B. PLAN EAST-WEST BOTTOM REINFORCING STEEL
   C. PLAN EAST-WEST TOP REINFORCING STEEL
   D. PLAN NORTH-SOUTH TOP REINFOCING STEEL
4. AT ELEVATOR HOISTWAYS, SLOPE TOP OF PILE CAP TO SUMP PIT AS INDICATED ON 12/S-302. THE PILE CAP THICKNESS WILL BE REDUCED UP TO 1" LOCALLY AND TOP REINFORCEMENT SHOULD BE LOWERED UP TO 1" LOCALLY TO MAINTAIN ADEQUATE COVER REQUIREMENTS.

3 FOUNDATION PARTIAL PLAN - SHEAR WALL #3

1/4" = 1'-0"



1 FOUNDATION PARTIAL PLAN - SHEAR WALL #1

1/4" = 1'-0"

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TAMPA, FL 33609
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MELBOURNE, FL 32940

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	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
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Cory Ann Dayton, State of Florida, Professional Engineer, License No. 57441. This item has been digitally signed and sealed by Cory Ann Dayton, PE on the date indicated. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.
Email: cdayton@walterpmoore.com

CORY ANN DAYTON, PE NO. 57441

Sheet Name

FOUNDATION PARTIAL PLANS

Sheet Number

S-321

Project Status

CONSTRUCTION DOCUMENTS

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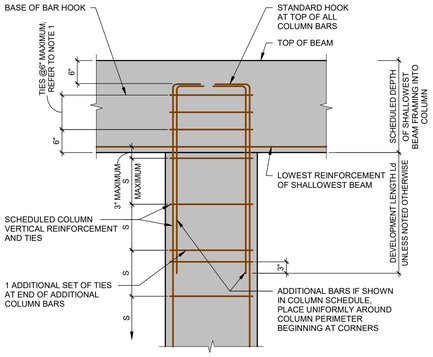

NOTES:
1. PROVIDE COLUMN TIES AT 6" MAXIMUM WITHIN THE FLOOR CONSTRUCTION AT ALL COLUMNS AS SHOWN. THE TIES WITHIN THE FLOOR CONSTRUCTION MAY BE OMITTED WITHIN THE DEPTH OF THE SHALLOWEST MEMBER FRAMING INTO THE JOINT ONLY AT INTERIOR COLUMNS WHERE BEAMS HAVING A WIDTH NO LESS THAN 1/2 OF THE COLUMN WIDTH FRAME INTO ALL FOUR SIDES OF THE COLUMN AND WHERE NO MORE THAN 12" OF COLUMN WIDTH REMAINS UNCOVERED ON EITHER SIDE OF ANY OF THE BEAMS.
2. "S" DENOTES SCHEDULED TIE SPACING.

15 TYPICAL COLUMN TERMINATION BEAM AND SLAB CONSTRUCTION

NO SCALE

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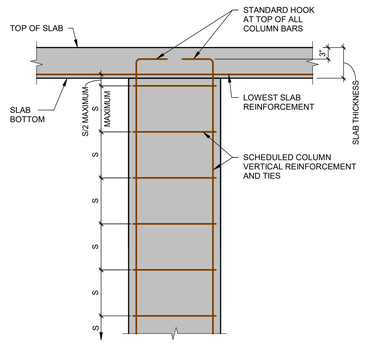

NOTES:
1. "S" DENOTES SCHEDULED TIE SPACING.

14 TYPICAL COLUMN TERMINATION SLAB CONSTRUCTION

NO SCALE

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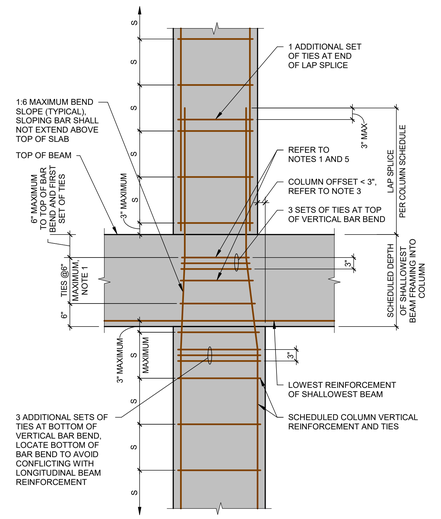

NOTES:
1. PROVIDE COLUMN TIES AT TOP OF VERTICAL BAR BEND AND AT 6" MAXIMUM WITHIN THE FLOOR CONSTRUCTION AT ALL COLUMNS AS SHOWN. THE TIES WITHIN THE FLOOR CONSTRUCTION MAY BE OMITTED WITHIN THE DEPTH OF THE SHALLOWEST MEMBER FRAMING INTO THE JOINT ONLY AT INTERIOR COLUMNS WHERE BEAMS HAVING A WIDTH NO LESS THAN 3/4 OF THE COLUMN WIDTH FRAME INTO ALL FOUR SIDES OF THE COLUMN AND WHERE NO MORE THAN 4" OF COLUMN WIDTH REMAINS UNCOVERED ON EITHER SIDE OF ANY OF THE BEAMS.
2. "S" DENOTES SCHEDULED TIE SPACING.
3. THIS TYPICAL DETAIL IS APPLICABLE FOR COLUMNS WITH OR WITHOUT OFFSETS FROM 0" TO LESS THAN 3" ON ANY FACE FROM COLUMN ABOVE THE FLOOR.
4. TERMINATE DISCONTINUED COLUMN VERTICAL REINFORCING BAR ITS SCHEDULED SPLICE LENGTH ABOVE THE FLOOR. DO NOT DISCONTINUE A CORNER BAR.
5. AT LOCATIONS WITHIN SLOPED PORTION OF VERTICAL BARS:
   A. FOR RECTANGULAR TIES, SUBSTITUTE EACH ONE-PIECE PERIMETER TIE WITH TWO-PIECE TIES.
   B. PROVIDE CROSS-TIES OF VARYING LENGTHS AS REQUIRED OR PROVIDE TWO-PIECE CROSS-TIES.
   C. LAP SPLICE TWO-PIECE TIES 15" FOR #3 TIES, 20" FOR #4 TIES, AND 24" FOR #5 TIES.

5 TYPICAL COLUMN SPLICE AT FLOOR LINE BEAM AND SLAB CONSTRUCTION

NO SCALE

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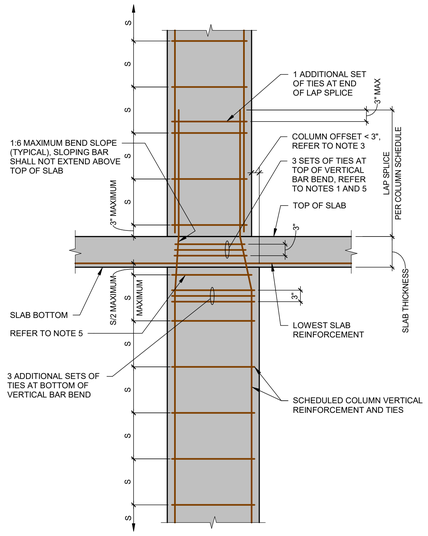

NOTES:
1. PROVIDE COLUMN TIES AT TOP OF VERTICAL BAR BEND AT ALL COLUMNS AS SHOWN. THESE TIES MAY BE OMITTED WITHIN THE DEPTH OF THE SLAB ONLY AT INTERIOR COLUMNS WITH SLAB ON ALL FOUR SIDES OF THE COLUMN.
2. "S" DENOTES SCHEDULED TIE SPACING.
3. THIS TYPICAL DETAIL IS APPLICABLE FOR COLUMNS WITH OR WITHOUT OFFSETS FROM 0" TO LESS THAN 3" ON ANY FACE FROM COLUMN ABOVE THE FLOOR.
4. TERMINATE DISCONTINUED COLUMN VERTICAL REINFORCING BAR ITS SCHEDULED SPLICE LENGTH ABOVE THE FLOOR. DO NOT DISCONTINUE A CORNER BAR.
5. AT LOCATIONS WITHIN SLOPED PORTION OF VERTICAL BARS:
   A. FOR RECTANGULAR TIES, SUBSTITUTE EACH ONE-PIECE PERIMETER TIE WITH TWO-PIECE TIES.
   B. PROVIDE CROSS-TIES OF VARYING LENGTHS AS REQUIRED OR PROVIDE TWO-PIECE CROSS-TIES.
   C. LAP SPLICE TWO-PIECE TIES 15" FOR #3 TIES, 20" FOR #4 TIES, AND 24" FOR #5 TIES.

4 TYPICAL COLUMN SPLICE AT FLOOR LINE SLAB CONSTRUCTION

NO SCALE

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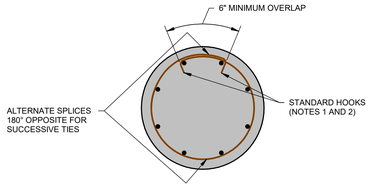

NOTES:
1. ENDS OF CIRCULAR TIES SHALL TERMINATE WITH STANDARD HOOKS THAT ENGAGE A LONGITUDINAL COLUMN BAR. AT CONTRACTOR'S OPTION, ONE ADDITIONAL #4 LONGITUDINAL BAR MAY BE ADDED AT EACH SPLICE LOCATION TO ANCHOR ONE OF THE HOOKS. THIS IS TO SIMPLIFY TIE FABRICATION WHEN THE NUMBER AND SPACING OF LONGITUDINAL COLUMN BARS VARIES THROUGHOUT THE PROJECT.
2. REFER TO 2/S-005 FOR TIE HOOK DIMENSIONS.

8 TYPICAL COLUMN TIE REQUIREMENTS CIRCULAR COLUMNS

NO SCALE

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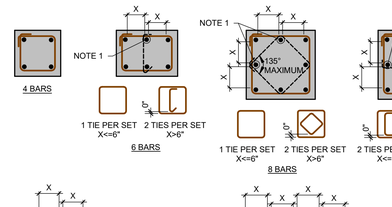

NOTES:
1. WHERE THIS NOTE IS INDICATED, BARS MUST BE TIED AS SHOWN BY DASHED LINES WHEN "X" IS GREATER THAN 6".
2. MINIMUM CONCRETE COVER SHALL BE 1 1/2" TO TIES, UNLESS MORE STRINGENT COVER REQUIREMENTS ARE SPECIFIED IN THE GENERAL NOTES OR OTHER DETAILS.
3. ALTERNATE HOOKS IN PLACING SUCCESSIVE SETS OF TIES.
4. REFER TO 2/S-005 FOR ALL HOOK REQUIREMENTS.
5. WHERE SPECIFIED BAR LAYOUTS VARY FROM CONFIGURATIONS SHOWN IN THIS DETAIL, SAME GENERAL TIE REQUIREMENTS SHALL APPLY. CONSULT ENGINEER OF-RECORD FOR CLARIFICATION AS NECESSARY.

7 TYPICAL COLUMN TIE REQUIREMENTS FOR PREASSEMBLED CAGES SQUARE OR RECTANGULAR COLUMNS

NO SCALE

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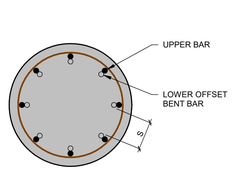

NOTES:
1. LOWER COLUMN VERTICAL BARS SHALL BE OFFSET BENT RADIALLY IN THE SHOP TOWARD THE INTERIOR OF THE COLUMN AS SHOWN.
2. "S" IS THE CLEAR BAR SPACING TO BE USED FOR DETERMINATION OF TENSION SPLICE LENGTH CATEGORY.
3. WHEN CLEAR BAR SPACING IS NOT THE SAME AT DIFFERENT COLUMN FACES, SMALLER SPACING SHALL BE USED TO DETERMINE THE APPLICABLE SPLICE CATEGORY.
4. REFER TO 11/S-004 FOR CRITERIA TO DETERMINE CLEAR SPACING FOR BARS AT STAGGERED SPLICES.

3 TYPICAL NORMAL LAP SPLICE CIRCULAR COLUMNS

NO SCALE

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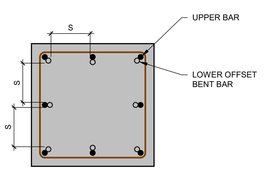

NOTES:
1. LOWER COLUMN VERTICAL BARS SHALL BE OFFSET BENT IN THE SHOP TOWARD THE INTERIOR OF THE COLUMN AS SHOWN.
2. "S" IS THE CLEAR BAR SPACING TO BE USED FOR DETERMINATION OF TENSION SPLICE LENGTH CATEGORY.
3. WHEN CLEAR BAR SPACING IS NOT THE SAME AT DIFFERENT COLUMN FACES, SMALLER SPACING SHALL BE USED TO DETERMINE THE APPLICABLE SPLICE CATEGORY.
4. REFER TO 11/S-004 FOR CRITERIA TO DETERMINE CLEAR SPACING FOR BARS AT STAGGERED SPLICES.

2 TYPICAL NORMAL LAP SPLICE SQUARE OR RECTANGULAR COLUMNS

NO SCALE

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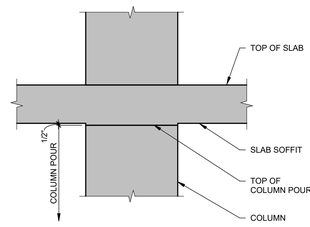

NOTES:
1. CONTRACTOR SHALL VERIFY TOP OF ALL COLUMN ELEVATIONS PRIOR TO POUR.
2. IF CONCRETE COLUMN IS POURED HIGHER THAN SLAB SOFFIT, CONTRACTOR SHALL CHIP COLUMN DOWN TO BE FLUSH WITH SLAB SOFFIT PRIOR TO PLACING SLAB REINFORCEMENT.
3. THIS DETAIL ALSO APPLIES TO FLAT SLABS WITH DROP PANELS.

1 TYPICAL CONCRETE COLUMN AT FLAT SLAB

NO SCALE

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FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
TO THE BEST OF THE ARCHITECTS OR ENGINEERS KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH THIS SECTION AND CHAPTER 633, FLORIDA STATUES.

ORLANDO HEALTH MELBOURNE NEW FREESTANDING EMERGENCY CARE FACILITY

LAKE ANDREW DR & WILLET PLACE
MELBOURNE, FL 32940

ORLANDO HEALTH®

PROJECT MANAGER	HDR ARCHITECTURE, INC.
PROJECT DESIGNER	HDR ARCHITECTURE, INC.
PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
LANDSCAPE ARCHITECT	MCKINTOSH ASSOCIATES, an LJA COMPANY
CIVIL ENGINEER	BSE CONSULTANTS, an LJA COMPANY
STRUCTURAL ENGINEER	WALTER P. MOORE & ASSOCIATES, INC.
MECHANICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
ELECTRICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
PLUMBING ENGINEER	I.C. TOMASSON ASSOCIATES, INC
INTERIOR DESIGNER	HDR ARCHITECTURE, INC.
EQUIPMENT PLANNER	IMEG CORP
WAYFINDING	HDR ARCHITECTURE, INC.

SHEET REVIEWER	

MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

CORY ANN DAYTON LICENSE No. 57441 STATE OF FLORIDA PROFESSIONAL ENGINEER

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Email: cdayton@walterpmoore.com

CORY ANN DAYTON, PE NO. 57441

Sheet Name

FRAMING TYPICAL DETAILS

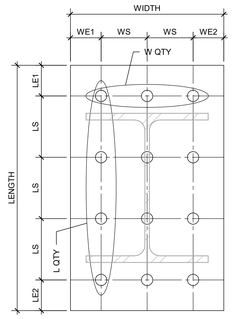
Sheet Number

S-401

Project Status

CONSTRUCTION DOCUMENTS

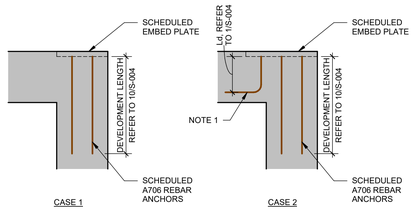
3/4/2026 2:21:27 PM Autodesk Docs://10427132_Orlando_Health_SunriseSt/s-25024-01 - CH Viera FSED_Structural - R2025.rvt



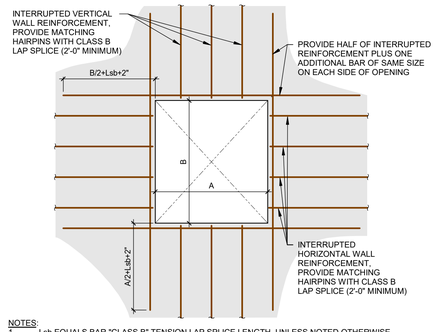
EMBED PLATE SCHEDULE

MARK	PLATE DIMENSIONS			ANCHORS REQUIRED				WIDTH SIDE				LENGTH SIDE				REMARKS
	WIDTH	LENGTH	THICKNESS	DIAMETER	EMBED LENGTH	TYPE	TOTAL QTY	WE1	WS	WE2	W QTY	LE1	LS	LE2	L QTY	
EP1	1'-0"	1'-0"	3/4"	3/4"	0'-6"	HSA	4	3"	6"	3"	2	3"	6"	3"	2	
EP2	1'-2"	2'-0"	1 1/4"	7/8"	1'-4"	HSA	6	2"	10"	2"	2	2"	10"	2"	3	
EP3	1'-0"	1'-4"	3/4"	3/4"	0'-6"	HSA	4	2"	8"	2"	2	2"	12"	2"	2	
EP4	1'-0"	1'-4"	3/4"	5/8"	NOTE 1	A706	6	3"	6"	3"	2	2"	6"	2"	3	REFER TO NOTE 1
EP5	2'-0"	1'-10"	1 1/2"	7/8"	NOTE 1	A706	8	2"	20"	2"	2	2 3/4"	5 1/2"	2 3/4"	4	REFER TO NOTE 1
EP6	1'-4"	2'-0"	3/4"	3/4"	0'-8"	HSA	8	3"	10"	3"	2	3"	6"	3"	4	
EP7	1'-4"	2'-4"	1 1/4"	3/4"	NOTE 1	A706	8	3"	10"	3"	2	2"	8"	2"	4	REFER TO NOTE 1
EP8	1'-0"	2'-0"	3/4"	3/4"	0'-6"	HSA	6	3"	6"	3"	2	3"	9"	3"	3	
EP9	1'-0"	1'-4"	3/4"	3/4"	0'-6"	HSA	4	2"	8"	2"	2	3"	10"	3"	2	
EP10	1'-6"	1'-4"	3/4"	3/4"	0'-6"	HSA	6	6"	5"	2"	3	5"	9"	2"	2	
EP11	1'-6"	1'-4"	3/4"	3/4"	0'-6"	HSA	6	4"	5"	4"	3	4"	8"	4"	2	
EP12	1'-0"	1'-4"	3/4"	3/4"	0'-6"	HSA	4	2"	6"	4"	2	2"	12"	2"	2	
EP13	1'-0"	1'-0"	1/2"	3/4"	0'-4"	HSA	4	4"	4"	4"	2	4"	4"	4"	2	
EP14	1'-0"	2'-2"	3/4"	5/8"	NOTE 1	A706	6	3"	6"	3"	2	4"	9"	4"	3	REFER TO NOTE 1
EP15	1'-0"	1'-6"	3/4"	3/4"	0'-6"	HSA	6	4"	4"	4"	2	2"	7"	2"	3	
EP16	2'-6"	2'-6"	1 1/4"	7/8"	NOTE 1	A706	16	3"	8"	3"	4	3"	8"	3"	4	REFER TO NOTE 1

NOTES:
1. AT ALL LOCATIONS SPECIFIED WITH A706 REBAR ANCHORS, DEVELOP REBAR INTO CONCRETE BEYOND WHEREVER POSSIBLE, AND REFER TO 10/S-004 FOR DEVELOPMENT LENGTH. WHERE REBAR IS UNABLE TO EXTEND THE FULL DEVELOPMENT LENGTH INTO THE CONCRETE BEYOND, REFER TO 12/S-402 FOR HOOKED BAR DETAIL.



NOTES:
1. DEVELOP REBAR ANCHORS INTO CONCRETE BEYOND WITH STRAIGHT BARS WHEREVER POSSIBLE (CASE 1). AT LOCATIONS WHERE REBAR IS UNABLE TO EXTEND THE FULL DEVELOPMENT LENGTH INTO THE CONCRETE BEYOND WITH THE APPROPRIATE CONCRETE COVER (CASE 2), PROVIDE HOOKED BARS AS SHOWN. REFER TO 1/S-005 FOR HOOK DIMENSIONS.
2. REFER TO PLANS AND EMBED PLATE SCHEDULE IN 15/S-402 FOR EMBED PLATE INFORMATION.
3. REFER TO SHEAR WALL ELEVATIONS AND TYPICAL DETAILS FOR SHEAR WALL REINFORCEMENT (NOT SHOWN).



NOTES:
1. Lsb EQUALS BAR "CLASS B" TENSION LAP SPLICE LENGTH, UNLESS NOTED OTHERWISE.
2. WALL OPENINGS ARE ONLY PERMITTED WHERE INDICATED ON THE STRUCTURAL DRAWINGS OR AS APPROVED BY THE STRUCTURAL ENGINEER.
3. THE ADDITIONAL REINFORCEMENT SPECIFIED IN THIS DETAIL IS NOT REQUIRED FOR SHEAR WALL DOOR OPENINGS WITH SCHEDULED LINK BEAMS, UNLESS SPECIFICALLY INDICATED.

15 EMBED PLATE SCHEDULE
NO SCALE

12 EMBED PLATES WITH REBAR ANCHORS
1" = 1'-0"

11 TYPICAL WALL OPENING REINFORCEMENT
NO SCALE

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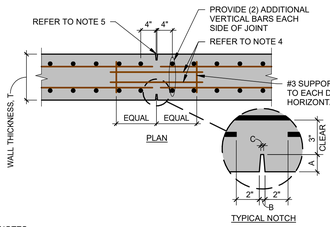
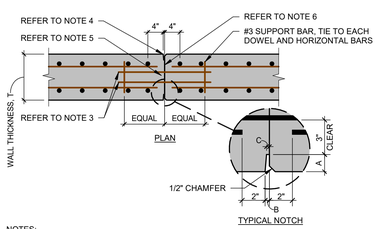


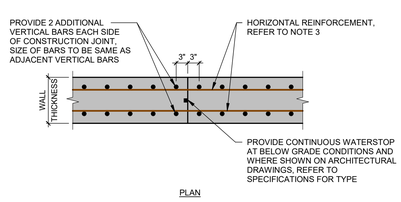
GROOVE DIMENSION SCHEDULE			
WALL THICKNESS	A	B	C
8"<=T<12"	1"	1/2"	1/4"
12"<=T<16"	1 1/2"	1/2"	1/4"
16"<=T<22"	2"	3/4"	3/8"
22"<=T<26"	2 1/2"	3/4"	3/8"
26"<=T<36"	3"	1"	1/2"
36"<=T<42"	4"	1 1/4"	1/2"
T>=42"	5"	1 3/8"	1/2"

ADDITIONAL DOWEL SCHEDULE		
HORIZONTAL BAR SIZE	DOWELS	
	T<12"	T>=12"
#4	(1) 1/2"Ø x2'-0"	(2) 3/8"Ø x2'-0"
#5	(1) 1/2"Ø x2'-0"	(2) 3/8"Ø x2'-0"
#6	(1) 3/4"Ø x3'-2"	(2) 1/2"Ø x2'-0"
#7	(1) 7/8"Ø x4'-4"	(2) 5/8"Ø x2'-6"
#8	(1) 7/8"Ø x4'-4"	(2) 5/8"Ø x2'-6"
#9	NA	(2) 3/4"Ø x3'-2"
#10	NA	(2) 7/8"Ø x4'-4"
#11	NA	(2) 1"Ø x6'-0"



GROOVE DIMENSION SCHEDULE			
WALL THICKNESS	A	B	C
8"<=T<12"	1"	3/8"	1/4"
12"<=T<16"	1 1/2"	3/8"	1/4"
16"<=T<22"	2"	9/16"	3/8"
22"<=T<26"	2 1/2"	9/16"	3/8"
26"<=T<36"	3"	3/4"	1/2"
36"<=T<42"	4"	7/8"	1/2"
T>=42"	5"	1"	1/2"

ADDITIONAL DOWEL SCHEDULE		
HORIZONTAL BAR SIZE	DOWELS	
	T<12"	T>=12"
#4	(1) 1/2"Ø x2'-0"	(2) 3/8"Ø x2'-0"
#5	(1) 1/2"Ø x2'-0"	(2) 3/8"Ø x2'-0"
#6	(1) 3/4"Ø x3'-2"	(2) 1/2"Ø x2'-0"
#7	(1) 7/8"Ø x4'-4"	(2) 5/8"Ø x2'-6"
#8	(1) 7/8"Ø x4'-4"	(2) 5/8"Ø x2'-6"
#9	NA	(2) 3/4"Ø x3'-2"
#10	NA	(2) 7/8"Ø x4'-4"
#11	NA	(2) 1"Ø x6'-0"



NOTES:
1. LOCATE VERTICAL CONTROL JOINTS ACCORDING TO THE FOLLOWING CRITERIA:
   A. WHERE SHOWN ON ARCHITECTURAL OR STRUCTURAL DRAWINGS
   B. AT 20'-0" MAXIMUM SPACING ALONG A CONTINUOUS LENGTH OF WALL
   C. WITHIN 10'-0" OF CORNERS, NO CLOSER THAN 5'-0" FROM CORNERS OR PILASTERS
   D. AT ABRUPT CHANGES IN WALL THICKNESS OR HEIGHT
2. WHERE CONTROL JOINT LOCATIONS ARE NOT SHOWN ON THE DRAWINGS, SUBMIT WALL JOINT LOCATIONS FOR APPROVAL PRIOR TO DETAILING REINFORCEMENT AND A MINIMUM OF TWO WEEKS PRIOR TO POUR.
3. THIS DETAIL IS NOT TO BE USED IN SHEAR WALLS AND WHERE WALLS SPAN HORIZONTALLY TO SUPPORT LATERAL AND/OR VERTICAL LOADS. REFER TO PLANS FOR LOCATIONS WHERE WALLS ARE SO DESIGNED.
4. PROVIDE SMOOTH DOWEL BARS AT SAME SPACING AS HORIZONTAL REINFORCEMENT. REFER TO SCHEDULE FOR NUMBER, SIZE, AND LENGTH. LIGHTLY COAT ONE END OF DOWEL WITH PARAFFIN-BASED LUBRICANT, ASPHALT EMULSION, FORM OIL, OR GREASE OR USE A PLASTIC OR METAL SLEEVE SPECIFICALLY DESIGNED FOR THE PURPOSE OF PREVENTING A BOND BETWEEN THE DOWEL AND CONCRETE. DOWELS ARE TO BE PARALLEL TO WALL SURFACE AND HORIZONTAL AND SHALL BE SECURED TO PREVENT MOVEMENT DURING CONCRETE PLACEMENT. CENTER DOWEL IN WALL WHERE ONLY ONE DOWEL IS SCHEDULED.
5. PROVIDE CONTINUOUS VERTICAL GROOVE FORMED IN EACH FACE OF WALL. FORM GROOVE WITH WOOD, RUBBER, PLASTIC, OR METAL FORMER STRIPS WITH WALL FORMS. REFER TO SCHEDULE FOR DIMENSIONS. FILL GROOVE WITH PERMANENT SEALANT AS SHOWN ON THE ARCHITECTURAL DRAWINGS AND AT BELOW GRADE CONDITIONS.
6. FORM PANELS SHALL BE SPLICED ONLY AT CONTROL JOINTS.
7. REFER TO 8/S-402 FOR CONSTRUCTION JOINT DETAIL.

NOTES:
1. WHERE CONSTRUCTION JOINT LOCATIONS ARE NOT SHOWN ON THE DRAWINGS, SUBMIT PROPOSED WALL JOINT LOCATIONS FOR APPROVAL PRIOR TO DETAILING REINFORCEMENT AND A MINIMUM OF TWO WEEKS PRIOR TO POUR. JOINTS SHALL BE SPACED NO CLOSER THAN 5'-0" FROM CORNERS AND PILASTERS. COORDINATE JOINT LOCATIONS WITH ARCHITECT.
2. THIS DETAIL IS NOT TO BE USED IN SHEAR WALLS AND WHERE WALLS SPAN HORIZONTALLY TO SUPPORT LATERAL AND/OR VERTICAL LOADS. REFER TO PLANS FOR LOCATIONS WHERE WALLS ARE SO DESIGNED.
3. PROVIDE SMOOTH DOWEL BARS AT SAME SPACING AS HORIZONTAL REINFORCEMENT. REFER TO SCHEDULE FOR NUMBER, SIZE, AND LENGTH. LIGHTLY COAT EXPOSED END OF DOWEL WITH PARAFFIN-BASED LUBRICANT, ASPHALT EMULSION, FORM OIL, OR GREASE IMMEDIATELY BEFORE PLACING CONCRETE OR USE A PLASTIC OR METAL SLEEVE SPECIFICALLY DESIGNED FOR THE PURPOSE OF PREVENTING A BOND BETWEEN THE DOWEL AND CONCRETE. DOWELS ARE TO BE PARALLEL TO WALL SURFACE AND HORIZONTAL AND SHALL BE SECURED TO PREVENT MOVEMENT DURING CONCRETE PLACEMENT. CENTER DOWEL IN WALL WHERE ONLY ONE DOWEL IS SCHEDULED.
4. PROVIDE CONTINUOUS VERTICAL GROOVE FORMED IN EACH FACE OF WALL. FORM GROOVE WITH WOOD, RUBBER, PLASTIC, OR METAL FORMER STRIPS WITH WALL FORMS. REFER TO SCHEDULE FOR DIMENSIONS. FILL GROOVE WITH PERMANENT SEALANT AS SHOWN ON THE ARCHITECTURAL DRAWINGS AND AT BELOW GRADE CONDITIONS.
5. PROVIDE CONTINUOUS WATERSTOP AT BELOW GRADE CONDITIONS AND WHERE SHOWN ON ARCHITECTURAL DRAWINGS. REFER TO SPECIFICATIONS FOR TYPE.
6. BREAK BOND BETWEEN NEW AND PREVIOUSLY PLACED CONCRETE BY SPRAYING OR PAINTING EXPOSED SIDE WITH A CURING COMPOUND, BOND BREAKER, OR FORM OIL.
7. REFER TO 10/S-402 FOR CONTROL JOINT DETAIL.

NOTES:
1. WHERE CONSTRUCTION JOINT LOCATIONS ARE NOT SHOWN ON THE DRAWINGS, SUBMIT PROPOSED WALL JOINT LOCATIONS FOR APPROVAL PRIOR TO DETAILING REINFORCEMENT AND A MINIMUM OF TWO WEEKS PRIOR TO POUR. JOINTS SHALL BE SPACED NO CLOSER THAN 5'-0" FROM CORNERS AND PILASTERS.
2. WHERE WALL SPANS HORIZONTALLY TO SUPPORT VERTICAL OR LATERAL LOADS, JOINTS SHALL BE LOCATED IN MIDDLE THIRD OF WALL SPANS, UNLESS NOTED OTHERWISE. REFER TO PLANS FOR LOCATIONS WHERE WALLS ARE SO DESIGNED.
3. SPLICES OF HORIZONTAL REINFORCEMENT ARE ALLOWED BUT NOT REQUIRED AT CONSTRUCTION JOINTS. REFER TO OTHER TYPICAL DETAILS FOR WALL REINFORCEMENT SPLICE LENGTHS.

10 TYPICAL VERTICAL CONTROL JOINT AT CONCRETE WALL (VISUALLY CRITICAL)
NO SCALE

8 TYPICAL VERTICAL CONSTRUCTION JOINT AT CONCRETE WALL (VISUALLY CRITICAL)
NO SCALE

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6 TYPICAL VERTICAL CONSTRUCTION JOINT AT CONCRETE WALL (VISUALLY NON-CRITICAL)
NO SCALE

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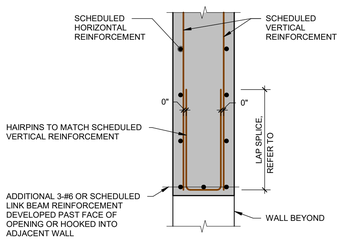
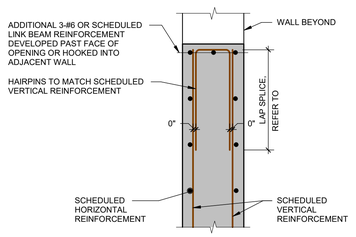


NOTES:
1. REFER TO LINK BEAM SCHEDULE IN 8/S-451 AS NECESSARY.



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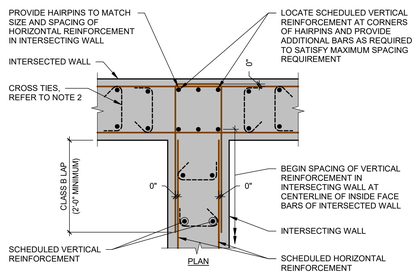
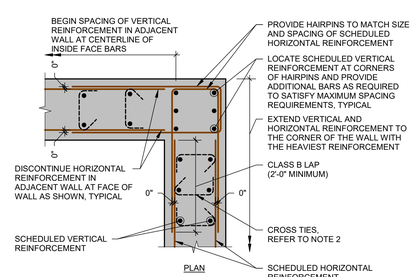


NOTES:
1. REFER TO OTHER DETAILS FOR REQUIRED WALL THICKNESSES AND REINFORCEMENT.
2. CROSS TIES ARE REQUIRED WHERE AREA OF SCHEDULED VERTICAL REINFORCEMENT EXCEEDS 0.01 TIMES THE GROSS CONCRETE AREA OF THE WALL. IF CENTER TO CENTER SPACING OF VERTICAL BARS IS 6" OR SMALLER, CROSS TIES SHALL BE PLACED AT EVERY OTHER PAIR OF VERTICAL BARS; OTHERWISE, ALL VERTICAL BARS SHALL BE CONFINED WITH CROSS TIES. CROSS TIES SHALL BE #3 BARS FOR #10 VERTICAL BARS & SMALLER AND #4 BARS FOR #11 VERTICAL BARS & LARGER.
3. THERE SHALL BE NO VERTICAL CONSTRUCTION JOINTS IN WALLS WITHIN 5'-0" OF INTERSECTIONS UNLESS SPECIFICALLY DETAILED ON THE DRAWINGS.



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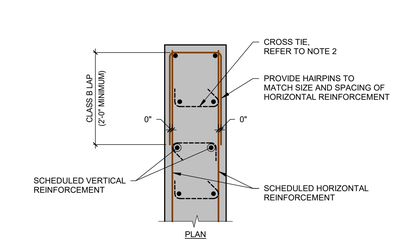

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3. THERE SHALL BE NO VERTICAL CONSTRUCTION JOINTS IN WALLS WITHIN 5'-0" OF ENDS UNLESS SPECIFICALLY DETAILED ON THE DRAWINGS.

5 TYPICAL ADDITIONAL WALL REINFORCEMENT ABOVE OPENINGS
NO SCALE

4 TYPICAL ADDITIONAL WALL REINFORCEMENT BELOW OPENINGS AND AT TOP OF WALL
NO SCALE

3 TYPICAL REINFORCEMENT SPLICE AT CONCRETE SHEAR WALL INTERSECTION
NO SCALE

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2 TYPICAL REINFORCEMENT SPLICE AT CONCRETE SHEAR WALL CORNER
NO SCALE

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1 TYPICAL REINFORCEMENT SPLICE AT CONCRETE SHEAR WALL END
NO SCALE

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HDR ARCHITECTURE, INC.
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WPM Project No. S05-25024-00

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FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
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FRAMING TYPICAL DETAILS

Sheet Number

S-402

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CONSTRUCTION DOCUMENTS

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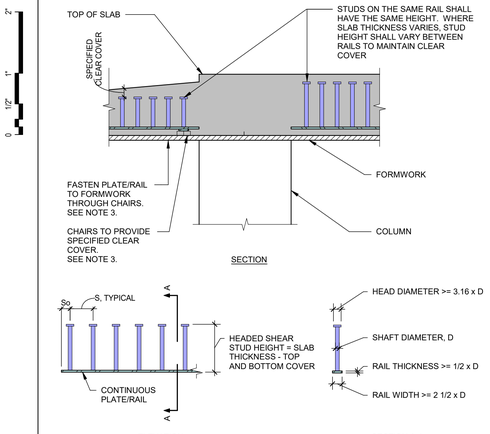

NOTES:
1. THE HEADED SHEAR STUDS SHALL BE WELDED TO THE RAILS IN CONFORMANCE WITH AWS D1.1. WELDING OF STUDS TO RAIL SHALL DEVELOP STUD TENSILE STRENGTH.
2. THE REQUIREMENTS FOR THE MATERIAL WIDTHS AND THICKNESS OF THE BASE RAIL SHOWN ARE APPLICABLE FOR SHANK DIAMETERS 0.375", 0.500", 0.625" AND 0.750".
3. USE CHAIRS AND FASTENING METHOD THAT ARE PROVIDED BY OR APPROVED BY THE HEADED SHEAR STUD REINFORCEMENT MANUFACTURER THAT INDEPENDENTLY STABILIZE THE HEADED SHEAR STUD RAILS.

15 TYPICAL HEADED SHEAR STUD REINFORCEMENT

NO SCALE

022XLFL-SHSTD.dgn
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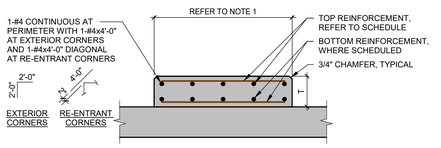

HOUSEKEEPING PAD REINFORCEMENT SCHEDULE		
PAD THICKNESS	TOP REINFORCEMENT	BOTTOM REINFORCEMENT
T = 4"	WWR4x4-W2.9xW2.9 OR #3@12" EACH WAY	NONE
4" < T <= 6"	WWR4x4-W4xW4 OR #4@18" EACH WAY	NONE
6" < T <= 8"	WWR4x4-W6xW6 OR #4@12" EACH WAY	NONE
8" < T <= 12"	#4@18" EACH WAY	#4@18" EACH WAY
12" < T <= 16"	#4@12" EACH WAY	#4@12" EACH WAY

NOTES:
1. REFER TO ARCHITECTURAL, STRUCTURAL, OR MEP DRAWINGS FOR HOUSEKEEPING PAD PLAN DIMENSIONS AND THICKNESS (4" MINIMUM THICKNESS).
2. CONTRACTOR SHALL COORDINATE DIMENSIONS AND OTHER SPECIAL REQUIREMENTS WITH EQUIPMENT MANUFACTURERS AND PROVIDE WHERE REQUIRED WHETHER SHOWN ON STRUCTURAL DRAWINGS OR NOT.

10 TYPICAL HOUSEKEEPING PAD OVER STRUCTURAL SLAB

NO SCALE

022SLFL-HPADS01
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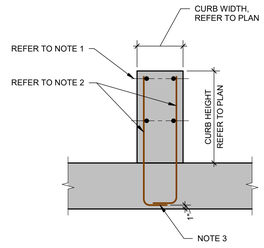
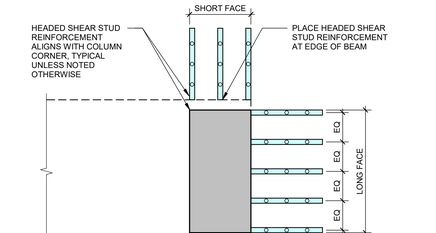


NOTES:
1. PROVIDE #4@12" CONTINUOUS HORIZONTAL EACH FACE WITH MATCHING CORNER BARS FOR TYPICAL CURBS, UNLESS NOTED OTHERWISE. FOR CURBS LESS THAN 8" WIDE, HORIZONTAL BAR(S) SHALL BE CENTERED INSTEAD OF EACH FACE.
2. FOR CURB HEIGHTS OF 4'-0" OR SHORTER PROVIDE #4@12" VERTICAL EACH FACE, AND FOR CURB HEIGHTS TALLER THAN 4'-0" UP TO 8'-0" PROVIDE #5@8" VERTICAL EACH FACE, UNLESS NOTED OTHERWISE.
3. HOOK OR EXTEND CURB VERTICAL REINFORCEMENT INTO SUPPORT BEAMS AS REQUIRED FOR FULL DEVELOPMENT OF BARS. WHEN CURB IS CONNECTED DIRECTLY TO SLAB ONLY, EXTEND HOOKED BARS TO BOTTOM BELOW SLAB REINFORCEMENT AS SHOWN.

9 TYPICAL CONCRETE CURB

3/4" = 1'-0"



NOTES:
1. REFER TO HEADED SHEAR STUD REINFORCEMENT SCHEDULE FOR STUD SPACING (S, So), NUMBER OF STUDS PER RAIL/PLATE, AND NUMBER OF RAILS/PLATES PER FACE.
2. FOR EDGE AND CORNER COLUMNS THAT ARE OFFSET FROM THE SLAB EDGE, ALIGN HEADED SHEAR STUD REINFORCEMENT WITH COLUMN CORNER, BUT NOT LESS THAN 2" CLEAR TO THE RAIL FROM THE SLAB EDGE

13 TYPICAL HEADED SHEAR STUD REINFORCEMENT LAYOUTS FOR RECTANGULAR COLUMNS

NO SCALE

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HEADED SHEAR STUD REINFORCEMENT SCHEDULE							
MARK	PLATES/RAILS PER COLUMN			STUDS PER RAIL/PLATE	PLATE/RAIL DIMENSIONS (INCHES)		REMARKS
	TOTAL	DISTRIBUTION			So	S	
		LONG FACE	SHORT FACE				
SR1	12	3	3	8	3.5	7	-
SR2	12	3	3	10	3.5	7	-
SR3	12	3	3	15	3	4	-

NOTES:
1. STUDS HAVE A SHANK DIAMETER, "D", OF 3/4" TYPICAL, UNLESS NOTED OTHERWISE.
2. REFER TO TYPICAL HEADED SHEAR STUD REINFORCEMENT DETAIL FOR INFORMATION NOT SHOWN.
3. REFER TO TYPICAL HEADED SHEAR STUD REINFORCEMENT LAYOUT DETAIL FOR PLACEMENT INFORMATION, UNLESS NOTED OTHERWISE.
4. AT COLUMN LOCATIONS WHERE A CONCRETE BEAM OR OPENING IS PRESENT, PROVIDE RAILS EQUALLY DISTRIBUTED AT THE REMAINING FACES OF COLUMN WHERE BEAM OR OPENING IS NOT PRESENT.

11 HEADED SHEAR STUD REINFORCEMENT SCHEDULE

NO SCALE

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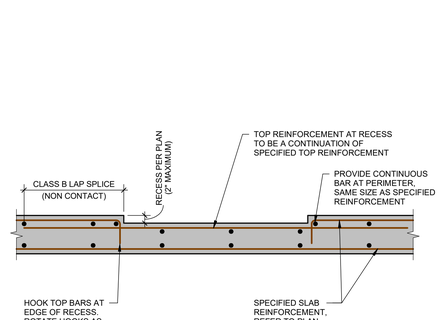
NOTES:
1. ADDITIONAL SLAB REINFORCMENT NOT SHOWN FOR CLARITY. REFER TO PLAN AND SCHEDULE.
2. UNLESS NOTED OTHERWISE, SLAB REINFORCEMENT ORIENTED IN THE EAST-WEST DIRECTION SHALL BE THE OUTERMOST LAYER.

4 TYPICAL SLAB RECESS REINFORCEMENT

3/4" = 1'-0"



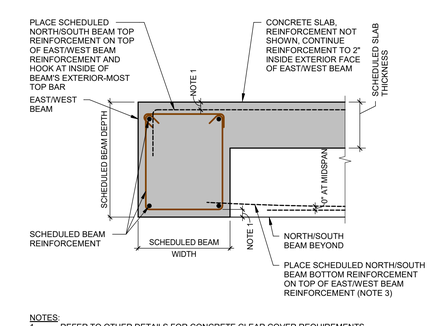
3 TYPICAL SLAB REINFORCEMENT LAYOUT

1/8" = 1'-0"

NOTES:
1. REFER TO HEADED SHEAR STUD REINFORCEMENT SCHEDULE FOR STUD SPACING (S, So), NUMBER OF STUDS PER RAIL/PLATE, AND NUMBER OF RAILS/PLATES PER FACE.
2. SIMILAR CONDITION WHEN BEAM IS CONTINUOUS THROUGH COLUMN OR AT A BEAM ALONG THE SLAB PERIMETER

8 TYPICAL HEADED SHEAR STUD REINFORCEMENT LAYOUT AT BEAMS

NO SCALE



NOTES:
1. FOR A ≤ 12". PROVIDE 1-#5 EACH SIDE.
   FOR 12" < A ≤ 24". PROVIDE 2-#5 EACH SIDE.
   FOR 24" < A ≤ 36". PROVIDE 3-#5 EACH SIDE.
2. THIS DETAIL APPLIES FOR "DEEPENED" BEAM SECTIONS ABOVE OR BELOW THE SCHEDULED BEAM.

7 TYPICAL DEEPENED CONCRETE BEAM

NO SCALE

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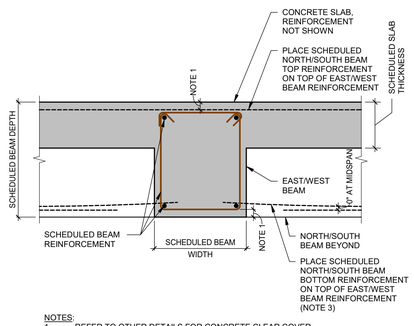

NOTES:
1. REFER TO PLAN FOR WIDENED DIMENSION "A".
2. FOR BEAMS WIDENED 3" OR MORE, PROVIDE THE FOLLOWING ADDITIONAL REINFORCEMENT (FACE BARS ARE ONLY REQUIRED WHEN DEPTH OF WIDENED BEAM SECTION EXCEEDS 22").
   - 3"≤A≤12": 1-#5 TOP, 1-#5 BOTTOM, AND #5@12" FACE BARS
   - 12"<A≤24": 2-#5 TOP, 2-#5 BOTTOM, AND #5@8" FACE BARS
   - 24"<A≤36": 3-#5 TOP, 3-#5 BOTTOM, AND #5@6" FACE BARS
   - A>36": CONSULT WITH STRUCTURAL ENGINEER

6 TYPICAL WIDENED CONCRETE BEAM

NO SCALE

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NOTES:
1. REFER TO OTHER DETAILS FOR CONCRETE CLEAR COVER REQUIREMENTS.
2. NORTH/SOUTH BEAM STIRRUPS HAVE BEEN OMITTED FOR CLARITY.
3. SECURE VERTICAL LEG OF BEAM STIRRUPS (NOT SHOWN) AT RAISED BOTTOM BAR TO PREVENT DISPLACEMENT DURING CONCRETE PLACEMENT.

2 TYPICAL EXTERIOR BEAM REINFORCEMENT PLACEMENT

NO SCALE

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NOTES:
1. REFER TO OTHER DETAILS FOR CONCRETE CLEAR COVER REQUIREMENTS.
2. NORTH/SOUTH BEAM STIRRUPS HAVE BEEN OMITTED FOR CLARITY.
3. SECURE VERTICAL LEG OF BEAM STIRRUPS (NOT SHOWN) AT RAISED BOTTOM BAR TO PREVENT DISPLACEMENT DURING CONCRETE PLACEMENT.

1 TYPICAL INTERIOR BEAM REINFORCEMENT PLACEMENT

NO SCALE"

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FL FIRM REG CERTIFICATION
AA-C000457

walter p moore

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Tampa, Florida 33602
WPM Project No. S05-25024-00

813.221.2424
Florida DBPR License No. 3818

FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
TO THE BEST OF THE ARCHITECTS OR ENGINEERS KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH THIS SECTION AND CHAPTER 633, FLORIDA STATUTES.

ORLANDO HEALTH MELBOURNE NEW FREESTANDING EMERGENCY CARE FACILITY

LAKE ANDREW DR & WILLET PLACE
MELBOURNE, FL 32940

ORLANDO HEALTH®

PROJECT MANAGER	HDR ARCHITECTURE, INC.
PROJECT DESIGNER	HDR ARCHITECTURE, INC.
PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
LANDSCAPE ARCHITECT	MCKINTOSH ASSOCIATES, an LJA COMPANY
CIVIL ENGINEER	BSE CONSULTANTS, an LJA COMPANY
STRUCTURAL ENGINEER	WALTER P. MOORE & ASSOCIATES, INC.
MECHANICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
ELECTRICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
PLUMBING ENGINEER	I.C. TOMASSON ASSOCIATES, INC
INTERIOR DESIGNER	HDR ARCHITECTURE, INC.
EQUIPMENT PLANNER	IMEG CORP.
WAYFINDING	HDR ARCHITECTURE, INC.
SHEET REVIEWER	

MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

CORY ANN DAYTON LICENSE No. 57441 STATE OF FLORIDA PROFESSIONAL ENGINEER

Cory Ann Dayton, State of Florida, Professional Engineer, License No. 57441. This item has been digitally signed and sealed by Cory Ann Dayton, PE on the date indicated. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.
Email: cdayton@walterpmoore.com

CORY ANN DAYTON, PE NO. 57441

Sheet Name

FRAMING TYPICAL DETAILS

Sheet Number

S-403

Project Status

CONSTRUCTION DOCUMENTS

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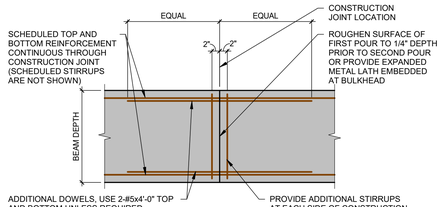

NOTES:
1. CONTRACTOR SHALL SUBMIT PROPOSED CONSTRUCTION JOINT LAYOUT AS A SHOP DRAWING FOR APPROVAL PRIOR TO DETAILING REINFORCEMENT AND A MINIMUM OF TWO WEEKS PRIOR TO FIRST POUR.
2. CONSTRUCTION JOINT LOCATIONS SHALL BE COORDINATED WITH REINFORCING STEEL SUPPLIER AND ERECTOR.
3. ACCEPTABLE JOINT LOCATIONS ARE AS FOLLOWS:
   A. FOR BEAMS NOT SUPPORTING INTERSECTING BEAMS, PLACE JOINT WITHIN MIDDLE THIRD OF SPAN.
   B. FOR BEAMS SUPPORTING INTERSECTING BEAMS, CHECK WITH STRUCTURAL ENGINEER FOR JOINT LOCATIONS AND DOWEL REQUIREMENTS.
4. SHOP DRAWINGS SHALL SHOW ALL REQUIRED ADDITIONAL REINFORCEMENT AT JOINTS.

15 TYPICAL CONSTRUCTION JOINT IN CONCRETE BEAM

NO SCALE

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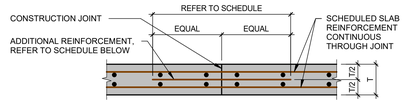

ADDITIONAL REINFORCEMENT SCHEDULE	
SLAB THICKNESS	REINFORCEMENT
T=4"	#4x2'-6" @18"
4"<T<=6"	#4x2'-6" @12"
6"<T<=8"	#5x3'-0" @12"
8<T<=12"	#6x3'-8" @12"

NOTES:
1. CONSTRUCTION JOINTS SHOULD BE LOCATED ON THE MIDDLE THIRD OF THE SLAB SPAN.
2. CONCRETE SURFACE AT CONSTRUCTION JOINT SHALL BE CLEAN AND FREE OF LAITANCE.
3. DO NOT USE FOR POST-TENSIONED SLABS.

14 TYPICAL CONSTRUCTION JOINT IN CONCRETE STRUCTURAL SLAB

NO SCALE

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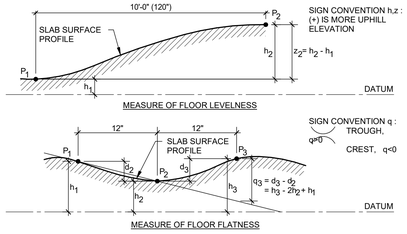

NOTES:
1. F_L = LEVELNESS F-NUMBER
   $F_L = \frac{12.5}{(3 \times Sz) + z}$ WHERE $\bar{z}$ = MEAN VALUE OF ALL z FLOOR READINGS; Sz = STANDARD DEVIATION OF ALL z FLOOR READINGS
2. F_F = FLATNESS F-NUMBER
   $F_F = \frac{4.57}{(3 \times Sq) + \bar{q}}$ WHERE $\bar{q}$ = MEAN VALUE OF ALL q FLOOR READINGS; Sq = STANDARD DEVIATION OF ALL q FLOOR READINGS
3. F_L AND F_F SHALL BE STATISTICALLY DETERMINED ACCORDING TO ASTM E 1155 "STANDARD TEST METHOD FOR DETERMINING FLOOR FLATNESS AND LEVELNESS USING THE F-NUMBER SYSTEM."
4. REFER TO CONCRETE SPECIFICATIONS FOR REQUIRED F_L AND F_F VALUES.

13 TYPICAL SLAB SURFACE TOLERANCE

NO SCALE

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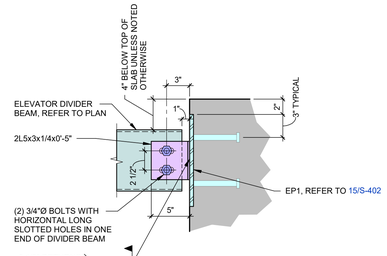

12 TYPICAL ELEVATOR DIVIDER BEAM CONNECTION TO CONCRETE BEAM OR WALL

NO SCALE

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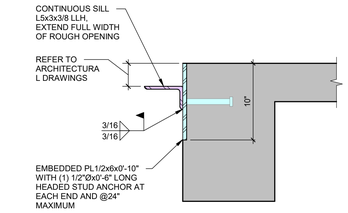

NOTES:
1. PROVIDE CONTINUOUS ELEVATOR SILL ANGLE AT THRESHOLD OF ELEVATOR OPENINGS IF REQUIRED BY ELEVATOR MANUFACTURER. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS.

11 TYPICAL ELEVATOR SILL ANGLE CONNECTION TO CONCRETE BEAM

NO SCALE

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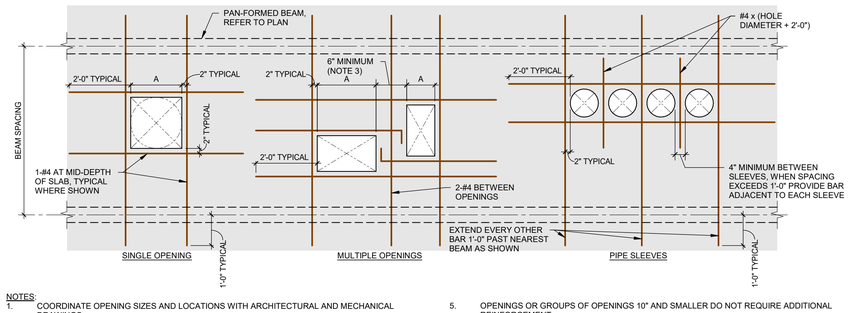

NOTES:
1. COORDINATE OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. "A" IS THE OPENING DIMENSION IN THE DIRECTION PARALLEL TO THE PAN-FORMED BEAMS. THIS DETAIL IS APPLICABLE FOR A MAXIMUM "A" DIMENSION OF 4'-0". VERIFY FRAMING WITH ENGINEER FOR OPENINGS GREATER THAN 4'-0".
3. WHEN SPACING BETWEEN MULTIPLE OPENINGS IS 2'-0" OR GREATER, REINFORCEMENT SHALL BE PLACED AS SHOWN FOR SINGLE OPENING CONDITIONS.
4. SLAB REINFORCEMENT SHALL BE CUT AROUND SLAB OPENINGS AND SHALL EXTEND TO WITHIN 2" OF OPENINGS ON ALL SIDES.
5. OPENINGS OR GROUPS OF OPENINGS 10" AND SMALLER DO NOT REQUIRE ADDITIONAL REINFORCEMENT.
6. THIS DETAIL SHOWS TYPICAL CONDITIONS. VERIFY REINFORCING STEEL PLACEMENT WITH ENGINEER FOR SPECIAL CASES.
7. PROVIDE REINFORCEMENT AS SHOWN FOR SQUARE, RECTANGULAR, OR ROUND OPENINGS.

10 TYPICAL OPENINGS IN ONE-WAY CONCRETE SLAB

NO SCALE

D2301-FL-PN(ST)5
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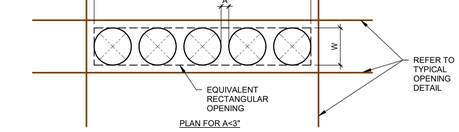

NOTES:
1. REINFORCEMENT SHOWN IS IN ADDITION TO SCHEDULED SLAB REINFORCEMENT.
2. SCHEDULED SLAB MESH REINFORCEMENT MAY BE CUT AS REQUIRED TO MISS PIPE SLEEVES.
3. REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR LOCATION AND SIZE OF SLEEVES.
4. ISOLATED PIPE SLEEVES THAT ARE SMALLER THAN 5" AND DO NOT INTERRUPT REINFORCEMENT DO NOT REQUIRE THE USE OF THIS DETAIL.
5. THIS DETAIL SHOULD NOT BE USED FOR OPENING GROUPS WITH DIAMETERS LARGER THAN 12". CONSULT STRUCTURAL ENGINEER FOR FRAMING OF SUCH CONDITIONS.
6. SLEEVES AND PENETRATIONS WITHIN 48" OF THE FACE OF ANY COLUMN IN A TWO-WAY SLAB NOT SHOWN EXPLICITLY ON THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED FOR APPROVAL TO STRUCTURAL ENGINEER.

8 ADDITIONAL REINFORCEMENT AROUND PIPE SLEEVES IN TWO-WAY CONCRETE SLAB

NO SCALE

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NOTES:
1. Lsb EQUALS BAR "CLASS B" TENSION LAP SPLICE LENGTH, UNLESS NOTED OTHERWISE. REFER TO 5/S-004.
2. SLAB OPENINGS ARE ONLY PERMITTED WHERE INDICATED ON THE STRUCTURAL PLANS OR AS APPROVED BY THE STRUCTURAL ENGINEER.
3. MAXIMUM OPENING SIZE PERPENDICULAR TO SLAB SPAN ("A") IS LIMITED TO 6 TIMES THE SLAB THICKNESS OR 4'-0" WHICHEVER IS GREATER. MAXIMUM OPENING SIZE PARALLEL TO SLAB SPAN ("B") IS LIMITED TO THE SPACE BETWEEN BEAMS.

6 TYPICAL OPENING IN TWO-WAY CONCRETE SLAB

NO SCALE

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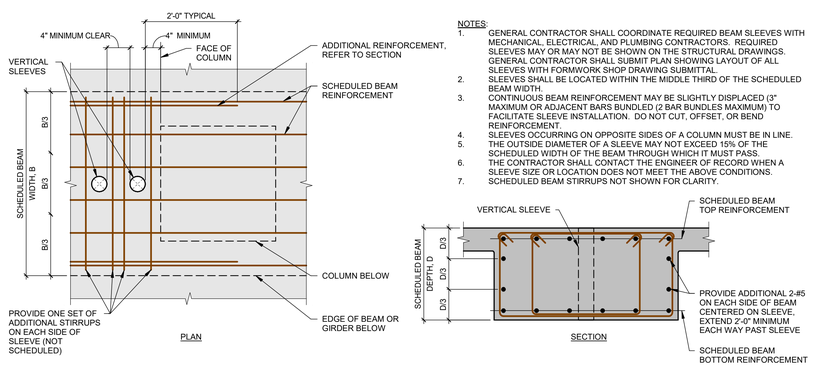

NOTES:
1. GENERAL CONTRACTOR SHALL COORDINATE REQUIRED BEAM SLEEVES WITH MECHANICAL, ELECTRICAL, AND PLUMBING CONTRACTORS. REQUIRED SLEEVES MAY OR MAY NOT BE SHOWN ON THE STRUCTURAL DRAWINGS. GENERAL CONTRACTOR SHALL SUBMIT PLAN SHOWING LAYOUT OF ALL SLEEVES WITH FORMWORK SHOP DRAWING SUBMITTAL.
2. SLEEVES SHALL BE LOCATED WITHIN THE MIDDLE THIRD OF THE SCHEDULED BEAM WIDTH.
3. CONTINUOUS BEAM REINFORCEMENT MAY BE SLIGHTLY DISPLACED (3" MAXIMUM OR ADJACENT BARS BUNDLED (2 BAR BUNDLES MAXIMUM) TO FACILITATE SLEEVE INSTALLATION. DO NOT CUT, OFFSET, OR BEND REINFORCEMENT.
4. SLEEVES OCCURRING ON OPPOSITE SIDES OF A COLUMN MUST BE IN LINE.
5. THE OUTSIDE DIAMETER OF A SLEEVE MAY NOT EXCEED 15% OF THE SCHEDULED WIDTH OF THE BEAM THROUGH WHICH IT MUST PASS. THE CONTRACTOR SHALL CONTACT THE ENGINEER OF RECORD WHEN A SLEEVE SIZE OR LOCATION DOES NOT MEET THE ABOVE CONDITIONS.
6. SCHEDULED BEAM STIRRUPS NOT SHOWN FOR CLARITY.
7. PROVIDE ADDITIONAL REINFORCEMENT AS SHOWN FOR SQUARE, RECTANGULAR, OR ROUND OPENINGS.

5 TYPICAL VERTICAL PENETRATION IN CONCRETE BEAM

NO SCALE

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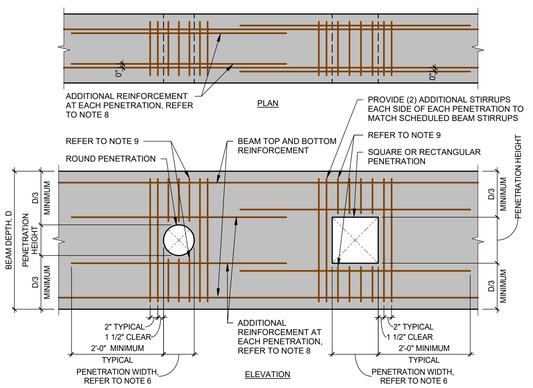

NOTES:
1. GENERAL CONTRACTOR SHALL COORDINATE LOCATION, SIZE, AND ELEVATION AND INCLUDE IN HIS CONTRACT PRICE ALL REQUIRED HORIZONTAL PENETRATIONS THROUGH CONCRETE BEAMS WHETHER SHOWN ON STRUCTURAL DRAWINGS OR NOT.
2. WHERE BEAM PENETRATIONS ARE REQUIRED BUT ARE NOT SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS, CONTRACTOR SHALL SUBMIT DRAWINGS SHOWING DIMENSIONS AND LOCATIONS OF ALL REQUIRED PENETRATIONS FOR REVIEW AND APPROVAL.
3. "D" DENOTES THE DEPTH OF BEAM.
4. CLEAR SPACING BETWEEN PENETRATIONS SHALL BE 24" MINIMUM UNLESS NOTED OTHERWISE BY THE STRUCTURAL ENGINEER.
5. PENETRATIONS SHALL BE LOCATED ACCORDING TO THE FOLLOWING CRITERIA:
   A. FOR BEAMS NOT SUPPORTING INTERSECTING BEAMS LOCATE PENETRATIONS WITHIN TWO FEET EITHER SIDE OF BEAM MIDSPAN.
   B. FOR BEAMS SUPPORTING INTERSECTING BEAMS CHECK WITH STRUCTURAL ENGINEER.
6. PENETRATION WIDTH MUST NOT EXCEED PENETRATION HEIGHT, UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS.
7. FOR LOCATIONS AND/OR SIZES OF PENETRATIONS NOT CONFORMING TO THE ABOVE CRITERIA AND NOT OTHERWISE DETAILED ON THE STRUCTURAL DRAWINGS, CONTRACTOR SHALL COORDINATE REQUIRED ADDITIONAL REINFORCEMENT WITH THE STRUCTURAL ENGINEER. PROVIDE THE FOLLOWING REINFORCEMENT AT EACH SLEEVE, UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS:
   - 1-#5 TOP AND BOTTOM AT BEAMS WITH WIDTHS LESS THAN 9".
   - 2-#5 TOP AND BOTTOM AT BEAMS WITH 2-LEG STIRRUPS.
   - 4-#5 TOP AND BOTTOM AT BEAMS WITH 4-LEG STIRRUPS.
   - "N"-#5 TOP AND BOTTOM AT BEAMS WITH "N"-LEG STIRRUPS.
8. PROVIDE ADDITIONAL STIRRUPS ABOVE AND BELOW PENETRATIONS ATSPACING NOT TO EXCEED ONE THIRD OF THE SCHEDULED STIRRUP SPACING, UNLESS DETAILED OTHERWISE ON THE STRUCTURAL DRAWINGS. SCHEDULED BEAM STIRRUPS NOT SHOWN FOR CLARITY.
9. PROVIDE ADDITIONAL REINFORCEMENT AS SHOWN FOR SQUARE, RECTANGULAR, OR ROUND OPENINGS.

3 TYPICAL HORIZONTAL PENETRATION IN CONCRETE BEAM

NO SCALE

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NOTES:
1. PLACE ALL CONDUITS, WHETHER IN SINGLE OR MULTIPLE LAYERS, WITHIN THE CENTER THIRD OF THE OVERALL SLAB THICKNESS.
2. DO NOT SPACE CONDUITS CLOSER THAN THREE DIAMETERS OR WIDTHS ON CENTER.
3. DO NOT EMBED CONDUITS AND PIPES OF ALUMINUM IN STRUCTURAL CONCRETE.
4. PLACE THE CONDUITS BETWEEN TOP AND BOTTOM SLAB REINFORCEMENT AND LOCATE AS CLOSE AS POSSIBLE TO THE MID-DEPTH OF THE SLAB.
5. PROVIDE THE SAME MINIMUM CONCRETE COVER FOR CONDUITS AS THAT OF THE SLAB MAIN REINFORCEMENT.
6. DO NOT PLACE CONDUIT ADJACENT TO NOR TIE CONDUIT TO PARALLEL REINFORCING BARS.

1 TYPICAL CONDUITS EMBEDDED IN STRUCTURAL SLABS

NO SCALE

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FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
TO THE BEST OF THE ARCHITECTS OR ENGINEERS KNOWLEDGE. THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH THIS SECTION AND CHAPTER 633, FLORIDA STATUTES.

ORLANDO HEALTH MELBOURNE NEW FREESTANDING EMERGENCY CARE FACILITY

LAKE ANDREW DR & WILLET PLACE
MELBOURNE, FL 32940

ORLANDO HEALTH®

PROJECT MANAGER	HDR ARCHITECTURE, INC.
PROJECT DESIGNER	HDR ARCHITECTURE, INC.
PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
LANDSCAPE ARCHITECT	MCKINTOSH ASSOCIATES, an LJA COMPANY
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SHEET REVIEWER

MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

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Sheet Name

FRAMING TYPICAL DETAILS

Sheet Number

S-404

Project Status

CONSTRUCTION DOCUMENTS

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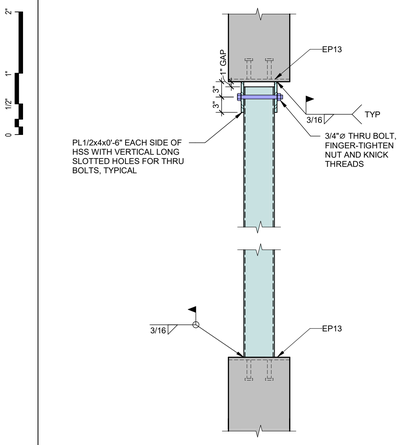
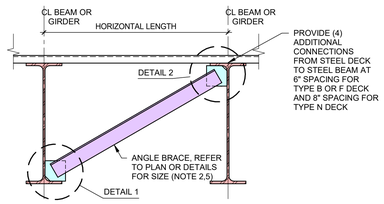
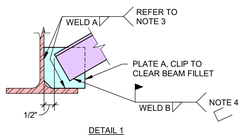


15 TYPICAL VERTICAL TUBE STEEL CONNECTIONS

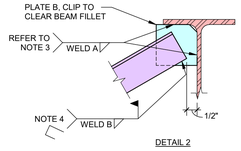
NO SCALE



CASE A: BRACE CONNECTION TO BEAM TOP FLANGE



DETAIL 1



DETAIL 2

CONNECTION SCHEDULE			
HORIZONTAL LENGTH	BRACE SIZE	PLATES A & B	WELDS A & B
UP TO 5'-0"	L2x2x1/4	4x4x1/4	3/16
5'-0" TO 8'-0"	L3x3x1/4	4x4x1/4	3/16
8'-0" TO 12'-0"	L4x4x1/4	5x5x1/4	3/16
12'-0" TO 15'-0"	L5x5x5/16	6x6x3/8	1/4

NOTES:
1. REFER TO PLAN FOR LOCATIONS OF ANGLE BRACES.
2. IF NOT SHOWN ON PLAN OR DETAILS, BRACE SIZE SHALL BE BASED ON THE HORIZONTAL LENGTH SHOWN IN THE CONNECTION SCHEDULE.
3. FIELD WELDING OF CONNECTION PLATES SHALL BE AT FABRICATOR'S OPTION. PROVIDE A MINIMUM OF 3" OF WELD ON EACH EDGE, WELD FAR SIDE IF NECESSARY.
4. WELD 3 EDGES MINIMUM WITH A MINIMUM TOTAL WELD LENGTH OF 4". WELD FAR SIDE IF NECESSARY.
5. MAXIMUM (FACTORED) FORCE IN BRACE IS 2500 LBS.

14 TYPICAL BEAM BOTTOM FLANGE BRACE (NON COMPOSITE CONSTRUCTION)

NO SCALE

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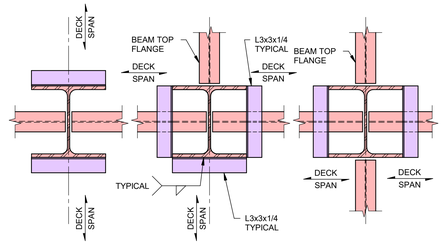

12 TYPICAL STEEL DECK SUPPORT AT STEEL COLUMNS

NO SCALE

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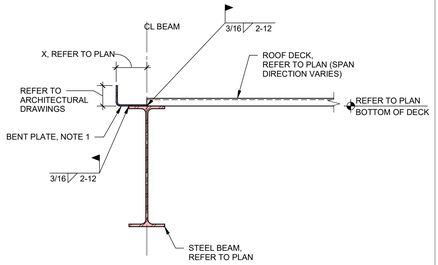
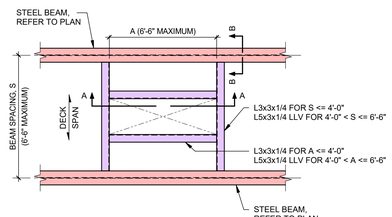


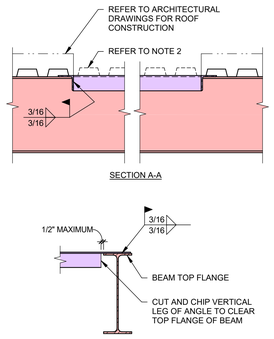
NOTES:
1. PROVIDE THE FOLLOWING BENT PLATE THICKNESS FOR THE GIVEN BENT PLATE LENGTH, X:
   A. X ≤ 1'-6": 1/4" BENT PLATE
   B. 1'-6" < X ≤ 3'-0": 3/8" BENT PLATE

11 TYPICAL STEEL ROOF DECK CLOSURE

NO SCALE



NOTES:
1. COORDINATE OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. ROOF DECK SHALL BE CONTINUOUS OVER FRAMED OPENINGS. DO NOT CUT OPENINGS UNTIL IMMEDIATELY BEFORE THEY ARE NEEDED.
3. VERIFY FRAMING WITH ENGINEER WHERE DIMENSIONS EXCEED MAXIMUM DIMENSIONS SHOWN IN THIS DETAIL.
4. REFER TO SPECIFICATIONS FOR OPENINGS WHERE THE LONGEST DIMENSION IS 10" OR LESS.



SECTION A-A

SECTION B-B

10 TYPICAL FRAMED OPENING IN ROOF DECK

NO SCALE

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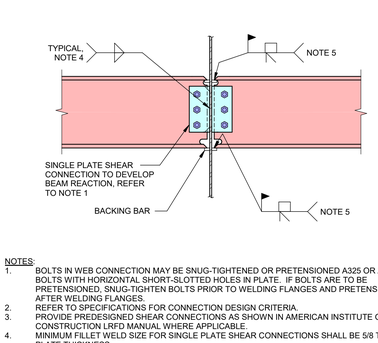

NOTES:
1. BOLTS IN WEB CONNECTION MAY BE SNUG-TIGHTENED OR PRETENSIONED A325 OR A490 BOLTS WITH HORIZONTAL SHORT-SLOTTED HOLES IN PLATE. IF BOLTS ARE TO BE PRETENSIONED, SNUG-TIGHTEN BOLTS PRIOR TO WELDING FLANGES AND PRETENSION AFTER WELDING FLANGES.
2. REFER TO SPECIFICATIONS FOR CONNECTION DESIGN CRITERIA.
3. PROVIDE PREDESIGNED SHEAR CONNECTIONS AS SHOWN IN AMERICAN INSTITUTE OF STEEL CONSTRUCTION LRFD MANUAL WHERE APPLICABLE.
4. MINIMUM FILLET WELD SIZE FOR SINGLE PLATE SHEAR CONNECTIONS SHALL BE 5/8 TIMES THE PLATE THICKNESS.
5. IF MOMENT IS SHOWN ON DRAWINGS, PARTIAL PENETRATION WELD TO DEVELOP MOMENT CAN BE USED IN LIEU OF COMPLETE JOINT PENETRATION WELD. FOR THIS CASE, WELD SHALL BE REQUIRED TO DEVELOP THE BEAM FLANGE FORCE COMPUTED AS FOLLOWS: Pu = (Mu)/(0.95D), WHERE: Mu = DESIGN MOMENT (KIP-FEET). D = BEAM DEPTH (INCHES). Pu = BEAM FLANGE FORCE (KIPS).

8 TYPICAL MOMENT CONNECTION AT COLUMN WEB

NO SCALE

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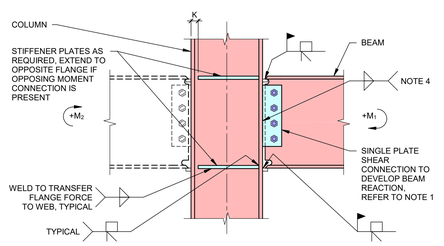

NOTES:
1. BOLTS IN WEB CONNECTION MAY BE SNUG-TIGHTENED OR PRETENSIONED A325 OR A490 BOLTS WITH HORIZONTAL SHORT-SLOTTED HOLES IN PLATE. IF BOLTS ARE TO BE PRETENSIONED, SNUG-TIGHTEN BOLTS PRIOR TO WELDING FLANGES AND PRETENSION AFTER WELDING FLANGES.
2. REFER TO SPECIFICATIONS FOR CONNECTION DESIGN CRITERIA.
3. PROVIDE PREDESIGNED SHEAR CONNECTIONS AS SHOWN IN AMERICAN INSTITUTE OF STEEL CONSTRUCTION LRFD MANUAL WHERE APPLICABLE.
4. MINIMUM FILLET WELD SIZE FOR SINGLE PLATE SHEAR CONNECTIONS SHALL BE 5/8 TIMES THE PLATE THICKNESS.
5. WHEN BEAM FRAMES ON ONLY ONE SIDE OF COLUMN OR WHEN BEAM MOMENTS ARE SIGNIFICANTLY UNBALANCED ($|M_1 + M_2| > > 0$), STRENGTHEN COLUMN WEB AS REQUIRED.

7 TYPICAL MOMENT CONNECTION AT COLUMN FLANGE

NO SCALE

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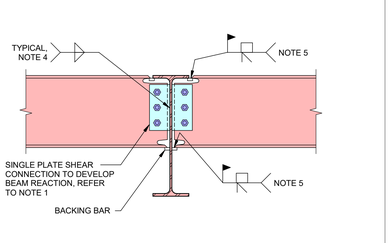

NOTES:
1. BOLTS IN WEB CONNECTION MAY BE SNUG-TIGHTENED OR PRETENSIONED A325 OR A490 BOLTS WITH HORIZONTAL SHORT-SLOTTED HOLES IN PLATE. IF BOLTS ARE TO BE PRETENSIONED, SNUG-TIGHTEN BOLTS PRIOR TO WELDING FLANGES AND PRETENSION AFTER WELDING FLANGES.
2. REFER TO SPECIFICATIONS FOR CONNECTION DESIGN CRITERIA.
3. PROVIDE PREDESIGNED SHEAR CONNECTIONS AS SHOWN IN AMERICAN INSTITUTE OF STEEL CONSTRUCTION MANUAL WHERE APPLICABLE.
4. MINIMUM FILLET WELD SIZE FOR SINGLE PLATE SHEAR CONNECTIONS SHALL BE 5/8 TIMES THE PLATE THICKNESS.
5. IF MOMENT IS SHOWN ON DRAWINGS, PARTIAL PENETRATION WELD TO DEVELOP MOMENT CAN BE USED IN LIEU OF COMPLETE JOINT PENETRATION WELD. FOR THIS CASE, WELD SHALL BE REQUIRED TO DEVELOP THE BEAM FLANGE FORCE COMPUTED AS FOLLOWS:
   - Pu = (Mu)/(0.95D), WHERE;
     - Mu = DESIGN MOMENT (KIP-FEET).
     - D = BEAM DEPTH (INCHES).
     - Pu = BEAM FLANGE FORCE (KIPS).

6 TYPICAL BEAM-TO-BEAM MOMENT CONNECTION

NO SCALE

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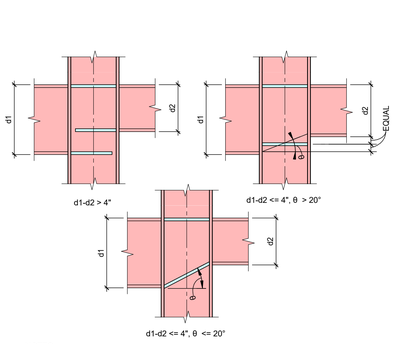

NOTES:
1. REFER TO SPECIFICATIONS FOR CONNECTION DESIGN CRITERIA.
2. PROVIDE PREDESIGNED CONNECTIONS AS SHOWN IN AMERICAN INSTITUTE OF STEEL CONSTRUCTION MANUAL WHERE APPLICABLE.
3. WHEN BEAM FRAMES ON ONLY ONE SIDE OF COLUMN OR WHEN BEAM REACTIONS ARE SIGNIFICANTLY DIFFERENT, STIFFEN COLUMN WEB AS REQUIRED.

5 TYPICAL COLUMN STIFFENER ARRANGEMENT FOR OPPOSING BEAMS OF DIFFERENT DEPTHS

NO SCALE

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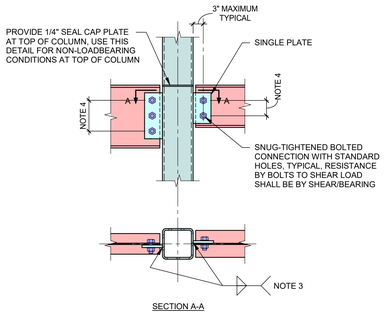

SECTION A-A

NOTES:
1. REFER TO SPECIFICATIONS FOR CONNECTION DESIGN CRITERIA.
2. REFER TO PLAN FOR CONNECTION DESIGN REACTIONS.
3. MINIMUM FILLET WELD SIZE SHALL BE 5/8 TIMES THE PLATE THICKNESS.
4. CONNECTION LENGTH SHALL BE AT LEAST 1/2 OF THE BEAM DEPTH.

4 TYPICAL SHEAR CONNECTION AT RECTANGULAR/SQUARE HSS COLUMN

NO SCALE

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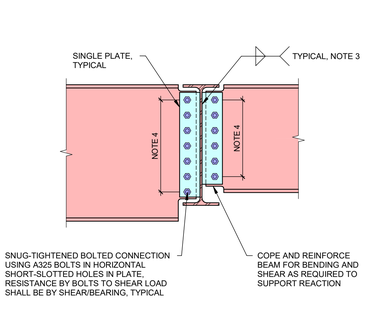

NOTES:
1. REFER TO SPECIFICATIONS FOR CONNECTION DESIGN CRITERIA.
2. PROVIDE PREDESIGNED CONNECTIONS AS SHOWN IN AMERICAN INSTITUTE OF STEEL CONSTRUCTION MANUAL WHERE APPLICABLE.
3. MINIMUM FILLET WELD SIZE SHALL BE 5/8 TIMES THE PLATE THICKNESS.
4. CONNECTION LENGTH SHALL BE AT LEAST 1/2 OF THE SUPPORTED BEAM DEPTH.

3 TYPICAL BEAM-TO-BEAM SHEAR CONNECTION

NO SCALE

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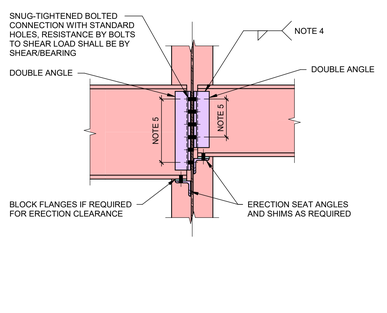

NOTES:
1. REFER TO SPECIFICATIONS FOR CONNECTION DESIGN CRITERIA.
2. REFER TO PLAN FOR CONNECTION DESIGN REACTIONS.
3. PROVIDE PREDESIGNED CONNECTIONS AS SHOWN IN AMERICAN INSTITUTE OF STEEL CONSTRUCTION MANUAL WHERE APPLICABLE.
4. SHOP WELD ON 3 SIDES TO DEVELOP BEAM REACTION, TYPICAL.
5. CONNECTION LENGTH SHALL BE AT LEAST 1/2 OF THE BEAM DEPTH.

2 TYPICAL SHEAR CONNECTION AT COLUMN WEB

NO SCALE

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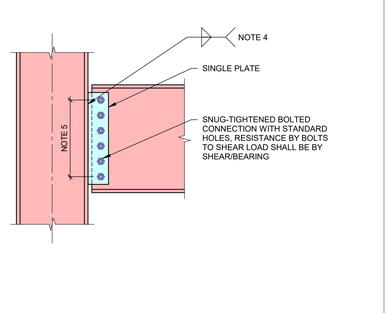

NOTES:
1. REFER TO SPECIFICATIONS FOR CONNECTION DESIGN CRITERIA.
2. REFER TO PLAN FOR CONNECTION DESIGN REACTIONS.
3. PROVIDE PREDESIGNED CONNECTIONS AS SHOWN IN AMERICAN INSTITUTE OF STEEL CONSTRUCTION MANUAL WHERE APPLICABLE.
4. MINIMUM FILLET WELD SIZE SHALL BE 5/8 TIMES THE PLATE THICKNESS.
5. CONNECTION LENGTH SHALL BE AT LEAST 1/2 OF THE BEAM DEPTH.

1 TYPICAL SHEAR CONNECTION AT COLUMN FLANGE

NO SCALE

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Florida DBPR License No. 3818

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LAKE ANDREW DR & WILLET PLACE
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ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

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Email: cdayton@walterpmoore.com

CORY ANN DAYTON, PE NO. 57441

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FRAMING TYPICAL DETAILS

Sheet Number

S-405

Project Status

CONSTRUCTION DOCUMENTS

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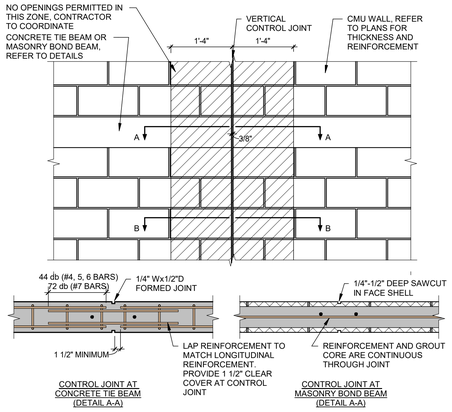
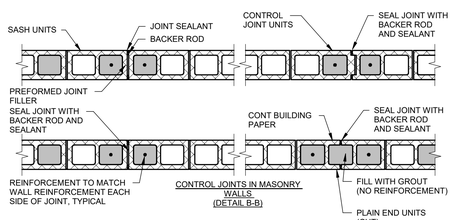
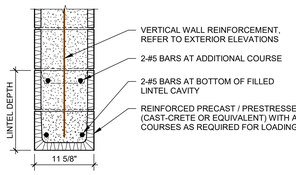




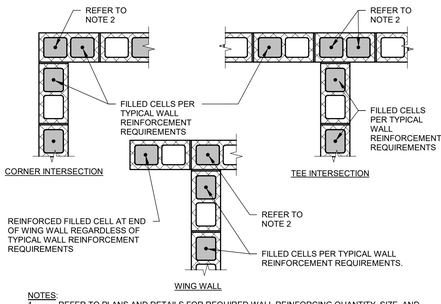
NOTES:
1. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS OF ALL CONTROL JOINTS. IF LOCATIONS ARE NOT SHOWN, CONTRACTOR SHALL SUBMIT PROPOSED LAYOUT FOR ARCHITECT AND ENGINEER APPROVAL. PLACE JOINTS IN ACCORDANCE WITH THE FOLLOWING GUIDELINES:
   - A. MAXIMUM SPACING: 20 FEET
   - B. MAXIMUM LENGTH/HEIGHT RATIO: 2 TO 1
   - C. AT ALL CHANGES IN WALL HEIGHT GREATER THAN 4'-0"
   - D. CHANGES IN WALL THICKNESS
   - E. NEAR WALL INTERSECTIONS
   - F. AT CORRESPONDING CONSTRUCTION JOINTS IN FOUNDATIONS, FLOOR, OR ROOF CONSTRUCTION
2. COORDINATE WATERPROOFING AND REQUIRED FIRE RATING OF CONTROL JOINTS WITH ARCHITECT.
3. STOP HORIZONTAL JOINT REINFORCEMENT 2" EACH SIDE OF JOINT. REFER TO DETAILS A-A FOR HORIZONTAL REINFORCEMENT AT TIE BEAMS AND BOND BEAMS.
4. USE HALF BLOCKS AT ALTERNATE COURSES SO JOINTS ARE CONSISTENT THROUGH WALLS.
5. FOUR OPTIONS ARE SHOWN FOR DETAIL B-B. ALTERNATE USED IS AT CONTRACTOR'S OPTION BUT SUBJECT TO APPROVAL BY ARCHITECT.
6. PROVIDE REINFORCED VERTICAL CELL EACH SIDE OF JOINT AS SHOWN. REINFORCEMENT TO MATCH OTHER WALL REINFORCEMENT.



NOTES:
1. LINTEL REINFORCEMENT SHALL PROJECT 40 BAR DIAMETERS (2'-0" MINIMUM) ONTO THE BEARING.

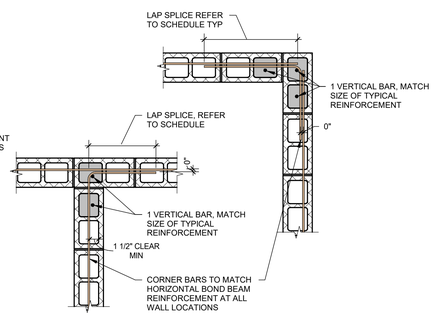
7 TYPICAL CMU LINTEL AT CHILLER YARD
NO SCALE

6 TYPICAL CMU WALL VERTICAL CONTROL JOINTS
NO SCALE

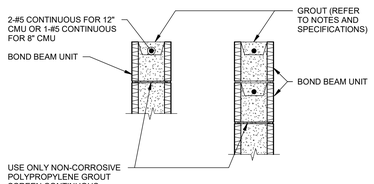


NOTES:
1. REFER TO PLANS AND DETAILS FOR REQUIRED WALL REINFORCING QUANTITY, SIZE, AND SPACING.
2. PROVIDE REINFORCED FILLED CELL COMMON TO BOTH WALLS REGARDLESS OF TYPICAL WALL REINFORCEMENT. WHEN CENTERLINE OF CELLS OF INTERSECTING WALLS DO NOT ALIGN, FILL BOTH CELLS AT INTERSECTION.
3. REFER TO TYPICAL BOND BEAM AT WALL INTERSECTION DETAIL FOR BOND BEAM REQUIREMENTS.

5 TYPICAL CMU WALL INTERSECTIONS AND WING WALLS
NO SCALE

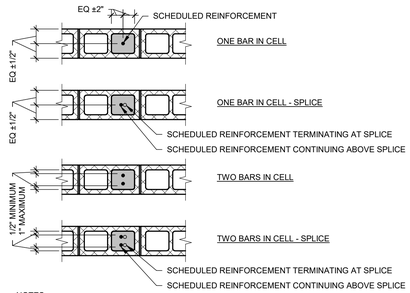


4 TYPICAL BOND BEAM AT WALL INTERSECTION
NO SCALE



NOTES:
1. GROUT CORE AND REINFORCEMENT ARE CONTINUOUS THROUGHOUT BOND BEAM.
2. TERMINATE ALL VERTICAL REINFORCEMENT INTO BOND BEAM USING STANDARD HOOK. EMBED HOOK A MINIMUM OF 6" INTO BOND BEAM. PROVIDE AT LEAST 1 1/2" CLEAR COVER FROM TOP OF BOND BEAM.
3. PROVIDE MATCHING CORNER BARS AT ALL BUILDING CORNERS. LAP WITH REINFORCEMENT SHOWN. REFER TO DETAIL 4/S-406.
4. PROVIDE LAP SPLICE PER 1/S-406.
5. REFER TO OTHER DETAILS FOR EMBEDDED ANCHORS AND COMPONENTS THAT BEAR ON BOND BEAM NOT SHOWN.

3 TYPICAL MASONRY BOND BEAM
NO SCALE



NOTES:
1. REINFORCEMENT MUST BE PLACED USING REINFORCING BAR POSITIONERS THAT LOCATE THE BAR AS SPECIFIED AND PREVENT MOVEMENT OF THE BAR DURING CONSTRUCTION.
2. SPLICED REINFORCEMENT MUST BE A CONTACT LAP SPLICE WITH SPLICED BARS ALIGNED PARALLEL TO THE WALL AS SHOWN.
3. THE ENGINEER MUST BE NOTIFIED PRIOR TO PLACEMENT OF REINFORCEMENT THAT IS REQUIRED TO BE PLACED OUTSIDE OF THE TOLERANCES OF THIS DETAIL SUCH AS TO AVOID INTERFERENCE WITH OTHER REINFORCEMENT, CONDUITS, OR EMBEDDED ITEMS.

2 TYPICAL CMU VERTICAL BAR PLACEMENT
NO SCALE

CMU WALL, PILASTER, AND COLUMN VERTICAL BAR TENSION DEVELOPMENT AND LAP SPLICE LENGTHS

GRADE 60 REINFORCEMENT, STANDARD BLOCK (f'm = 1500 PSI)

BAR SIZE	4" CMU	6" CMU		8" CMU		10" CMU		12" CMU	
	1 BAR /CELL	1 BAR /CELL	2 BARS /CELL	1 BAR /CELL	2 BARS /CELL	1 BAR /CELL	2 BARS /CELL	1 BAR /CELL	2 BARS /CELL
#3	19	16	19	16	17	16	17	16	17
#4	34	25	34	21	29	21	29	21	29
#5	NP	40	45	27	45	26	45	26	45
#6	NP	NP	NP	41	54	32	54	32	54
#7	NP	NP	NP	56	63	42	63	37	63
#8	NP	NP	NP	72	NP	64	72	51	72
#9	NP	NP	NP	NP	NP	NP	NP	65	81
#10	NP	NP	NP	NP	NP	NP	NP	NP	NP
#11	NP	NP	NP	NP	NP	NP	NP	NP	NP

NP = NOT PERMITTED

NOTES:
1. ALL DEVELOPMENT AND LAP SPLICE LENGTHS ARE IN INCHES.
2. THIS TABLE SHALL BE USED FOR ALL REINFORCED CMU WALLS, PILASTERS, AND COLUMNS UNLESS NOTED OTHERWISE IN DETAILS.
3. INCREASE TABULATED VALUES BY 50% FOR EPOXY COATED REINFORCEMENT.
4. WITH APPROVAL BY THE ENGINEER, WELDED SPLICES AND MECHANICAL SPLICES DEVELOPING AT LEAST 125% OF THE YIELD STRENGTH, F_y, OF THE BAR MAY BE SUBSTITUTED IN SOME LOCATIONS.
5. WHEN LAP SPLICING BARS OF DIFFERENT SIZES, THE LAP LENGTH IS DETERMINED BY THE SMALLER BAR.
6. REFER 2/S-406 FOR BAR POSITIONING IN CELLS.

1 TYPICAL CMU VERTICAL BAR TENSION DEVELOPMENT AND LAP SPLICE LENGTHS
NO SCALE

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FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
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MELBOURNE, FL 32940

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PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
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SHEET REVIEWER	

MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

CORY ANN DAYTON LICENSE No. 57441 STATE OF FLORIDA PROFESSIONAL ENGINEER

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Email: cdayton@walterpmoore.com

CORY ANN DAYTON, PE NO. 57441

Sheet Name

FRAMING TYPICAL DETAILS

Sheet Number

S-406

Project Status

CONSTRUCTION DOCUMENTS

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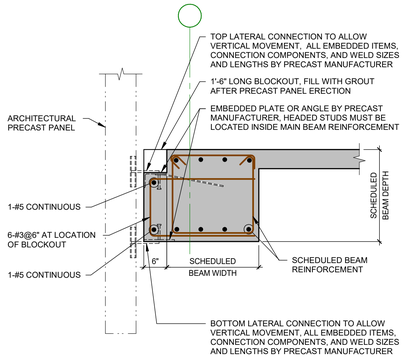

8 TYPICAL ARCHITECTURAL PRECAST PANEL LATERAL CONNECTIONS WITH BLOCKOUT

NO SCALE

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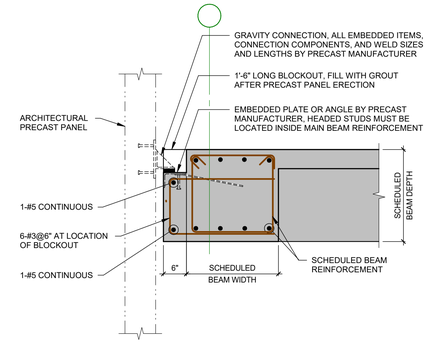

7 TYPICAL ARCHITECTURAL PRECAST PANEL GRAVITY CONNECTION WITH BLOCKOUT

NO SCALE

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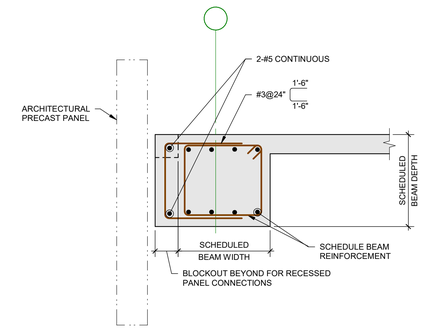

6 TYPICAL CONCRETE BEAM REINFORCEMENT BETWEEN PRECAST PANEL BLOCKOUTS

NO SCALE

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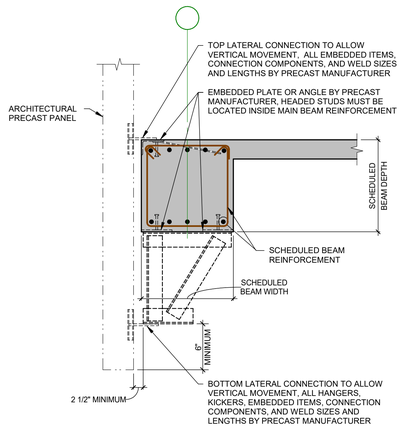

4 TYPICAL ARCHITECTURAL PRECAST PANEL LATERAL CONNECTIONS WITH DROPPED LOWER CONNECTION

NO SCALE

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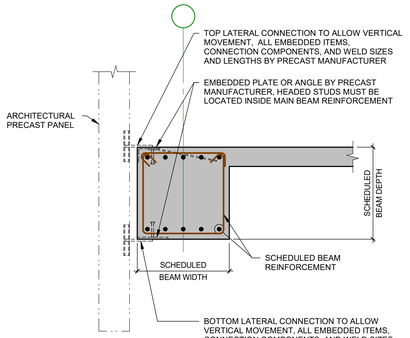

3 TYPICAL ARCHITECTURAL PRECAST PANEL LATERAL CONNECTIONS

NO SCALE

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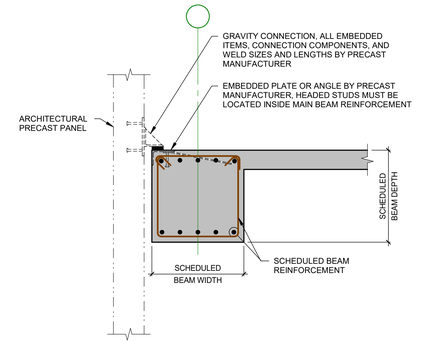

2 TYPICAL ARCHITECTURAL PRECAST PANEL GRAVITY CONNECTION

NO SCALE

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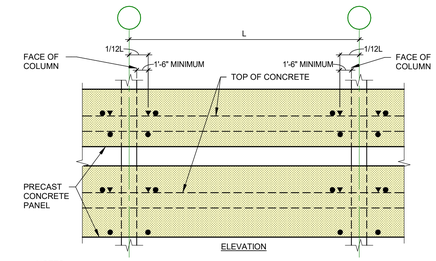

ELEVATION

NOTES:
1. ▼DENOTES GRAVITY CONNECTIONS, REFER TO OTHER DETAILS.
2. ●DENOTES LATERAL CONNECTIONS, REFER TO OTHER DETAILS.
3. PRECAST MANUFACTURER SHALL SUBMIT SHOP DRAWINGS TO THE ARCHITECT/ENGINEER SHOWING LOCATIONS OF ALL PRECAST PANEL CONNECTIONS AND LOCATIONS OF BLOCKOUT (IF APPLICABLE) PRIOR TO CONSTRUCTION OF THE FLOORS. PRECASTER SHALL DETERMINE THE EXACT NUMBER AND LOCATION OF LATERAL CONNECTIONS.
4. CONTRACTOR SHALL FURNISH AND INSTALL ALL EMBEDDED ITEMS REQUIRED FOR PANEL CONNECTION TO THE STRUCTURE. SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL EMBEDDED ITEMS SHOWING NUMBER, SIZE, AND LOCATION.
5. ALTERNATE DETAILS FOR PANEL CONNECTION TO STRUCTURE SHALL BE SUBMITTED FOR ENGINEER REVIEW AND APPROVAL.

1 TYPICAL ARCHITECTURAL PRECAST PANEL CONNECTION LAYOUT

NO SCALE

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Email: cdayton@walterpmoore.com

CORY ANN DAYTON, PE NO. 57441

Sheet Name

FRAMING TYPICAL DETAILS

Sheet Number

S-407

Project Status

CONSTRUCTION DOCUMENTS

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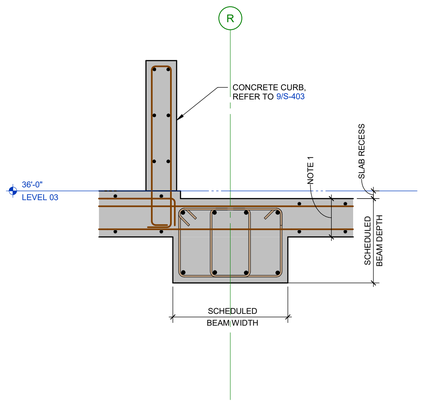
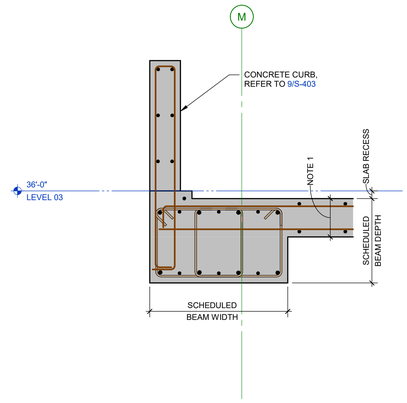


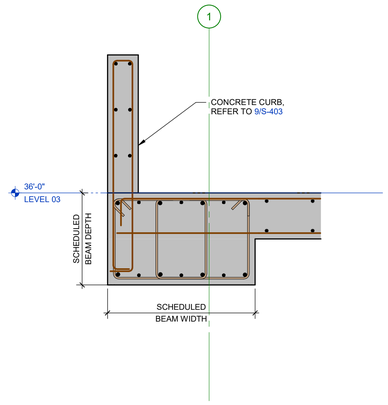
NOTES:
1. SLAB THICKNESS IS REDUCED AT SLAB RECESS, REFER TO 4/S-403 FOR REINFORCEMENT TRANSITION.

14 CONCRETE FRAMING DETAIL
3/4" = 1'-0"

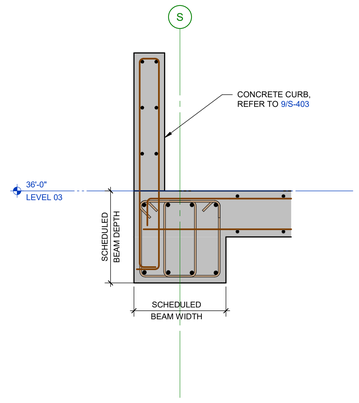


NOTES:
1. SLAB THICKNESS IS REDUCED AT SLAB RECESS, REFER TO 4/S-403 FOR REINFORCEMENT TRANSITION.

13 CONCRETE FRAMING DETAIL
3/4" = 1'-0"



12 CONCRETE FRAMING DETAIL
3/4" = 1'-0"



11 CONCRETE FRAMING DETAIL
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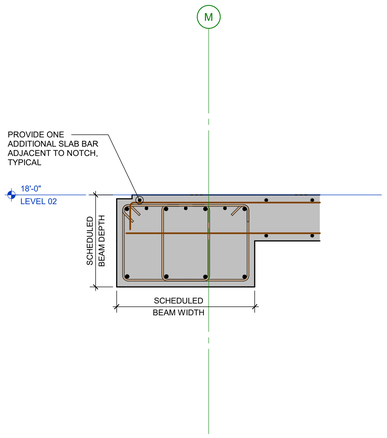

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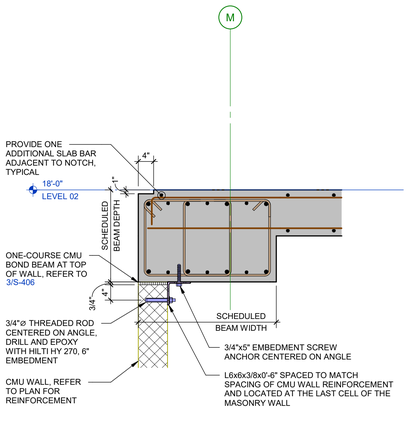
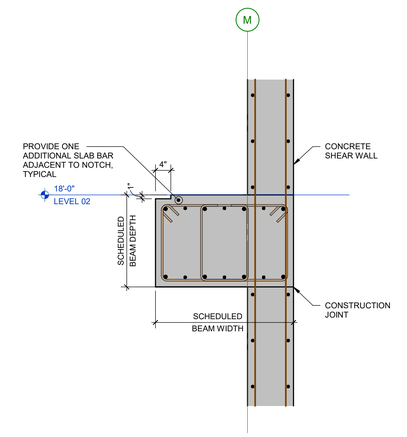
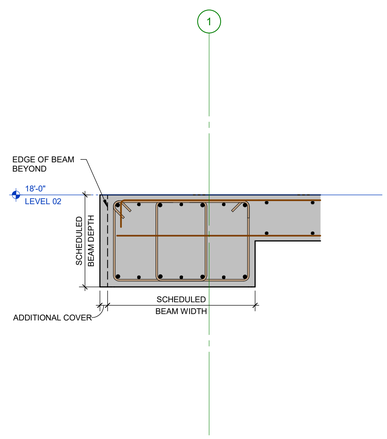


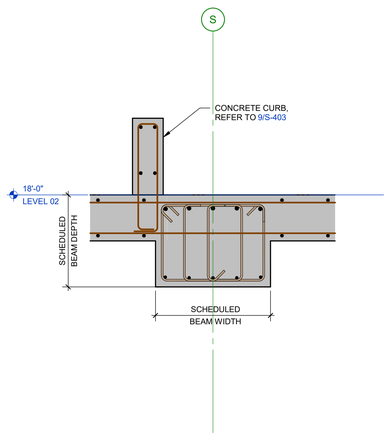
9 CONCRETE FRAMING DETAIL
3/4" = 1'-0"



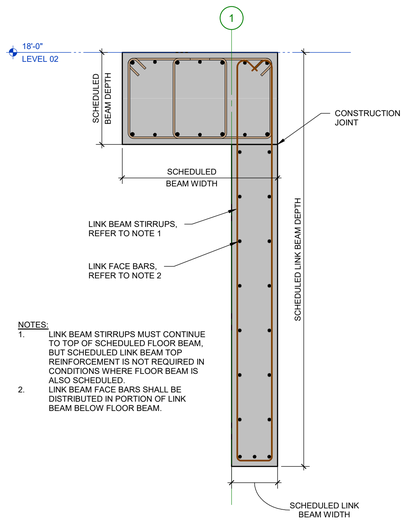
8 CONCRETE FRAMING DETAIL
3/4" = 1'-0"



7 CONCRETE FRAMING DETAIL
3/4" = 1'-0"

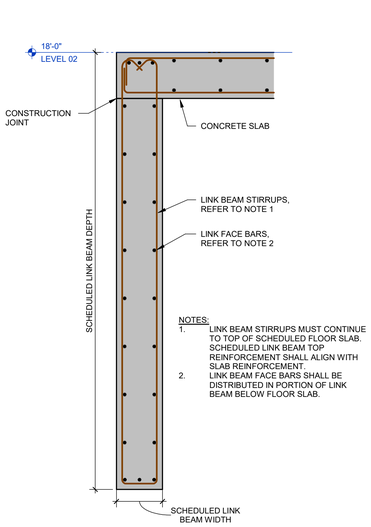


6 CONCRETE FRAMING DETAIL
3/4" = 1'-0"



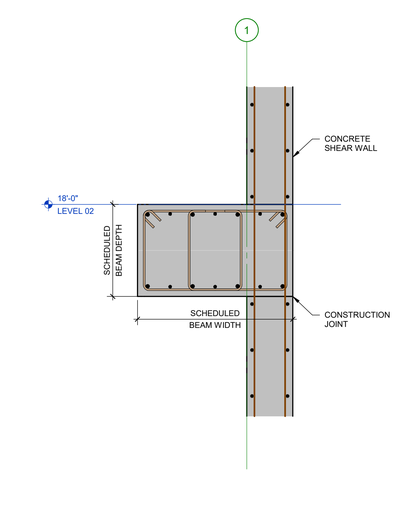
NOTES:
1. LINK BEAM STIRRUPS MUST CONTINUE TO TOP OF SCHEDULED FLOOR BEAM, BUT SCHEDULED LINK BEAM TOP REINFORCEMENT IS NOT REQUIRED IN CONDITIONS WHERE FLOOR BEAM IS ALSO SCHEDULED.
2. LINK BEAM FACE BARS SHALL BE DISTRIBUTED IN PORTION OF LINK BEAM BELOW FLOOR BEAM.

5 CONCRETE FRAMING DETAIL
3/4" = 1'-0"



NOTES:
1. LINK BEAM STIRRUPS MUST CONTINUE TO TOP OF SCHEDULED FLOOR SLAB. SCHEDULED LINK BEAM TOP REINFORCEMENT SHALL ALIGN WITH SLAB REINFORCEMENT.
2. LINK BEAM FACE BARS SHALL BE DISTRIBUTED IN PORTION OF LINK BEAM BELOW FLOOR SLAB.

4 CONCRETE FRAMING DETAIL
3/4" = 1'-0"



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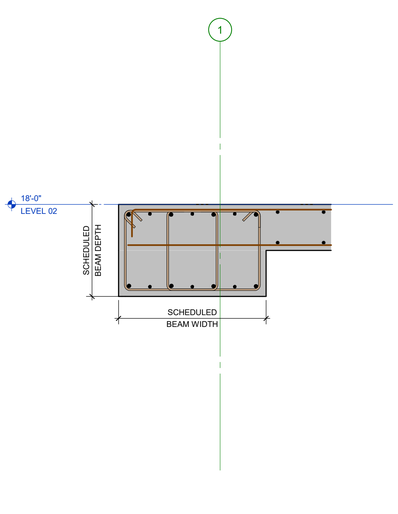
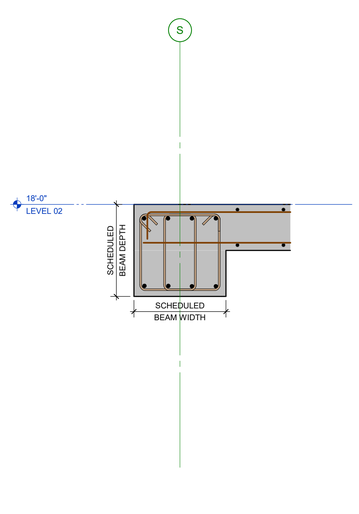


2 CONCRETE FRAMING DETAIL
3/4" = 1'-0"



1 CONCRETE FRAMING DETAIL
3/4" = 1'-0"

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PROJECT DESIGNER	HDR ARCHITECTURE, INC.
PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
LANDSCAPE ARCHITECT	MCKINTOSH ASSOCIATES, an LJA COMPANY
CIVIL ENGINEER	BSE CONSULTANTS, an LJA COMPANY
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EQUIPMENT PLANNER	IMEG CORP.
WAYFINDING	HDR ARCHITECTURE, INC.

SHEET REVIEWER

MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

CORY ANN DAYTON LICENSE No. 57441 STATE OF FLORIDA PROFESSIONAL ENGINEER

Cory Ann Dayton, State of Florida, Professional Engineer, License No. 57441. This item has been digitally signed and sealed by Cory Ann Dayton, PE on the date indicated. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.
Email: cdayton@walterpmoore.com

CORY ANN DAYTON, PE NO. 57441

Sheet Name

FRAMING DETAILS

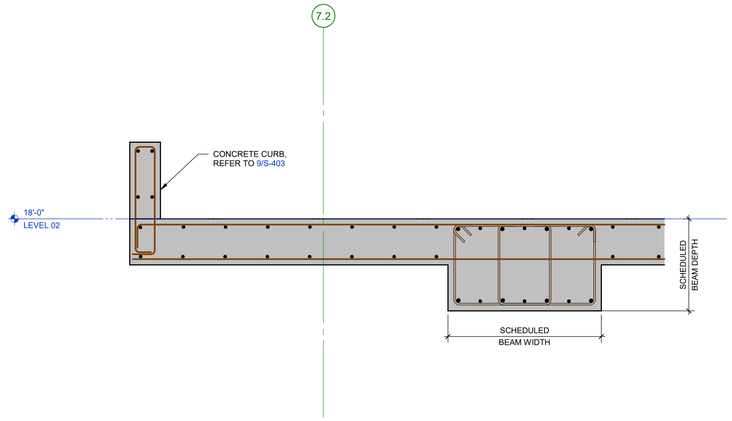
Sheet Number

S-411

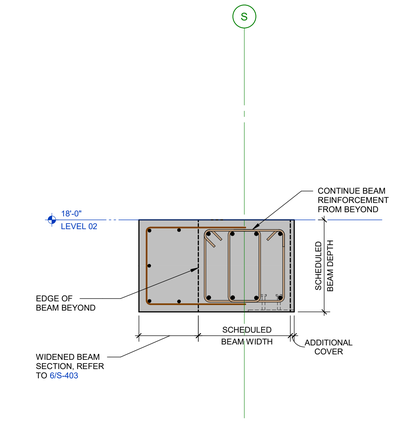
Project Status

CONSTRUCTION DOCUMENTS

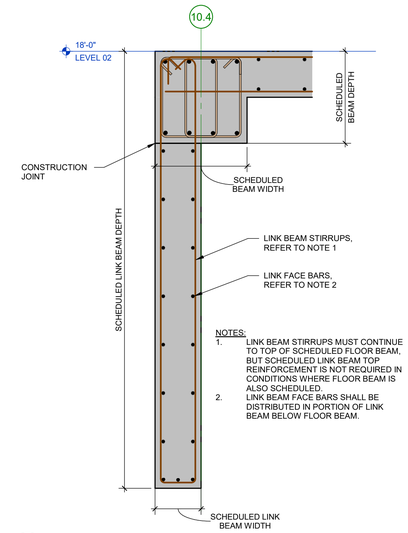
3/4/2026 2:21:59 PM Autodesk Docs://10427132_Orlando_Health_Sunrise/505-25024-01_OH Viera FSED - Structural - R2025.rvt



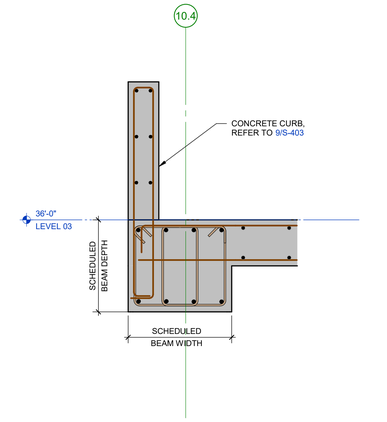
10 CONCRETE FRAMING DETAIL
3/4" = 1'-0"



8 CONCRETE FRAMING DETAIL
3/4" = 1'-0"



7 CONCRETE FRAMING DETAIL
3/4" = 1'-0"



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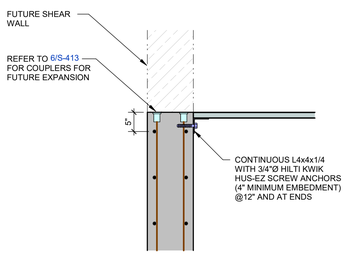
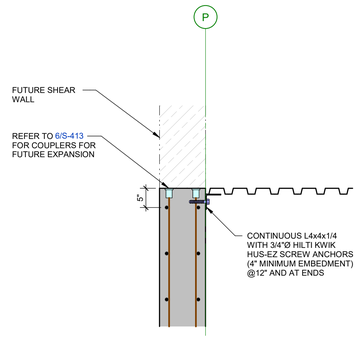
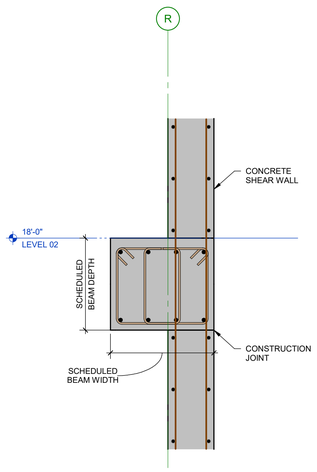
3/4" = 1'-0"



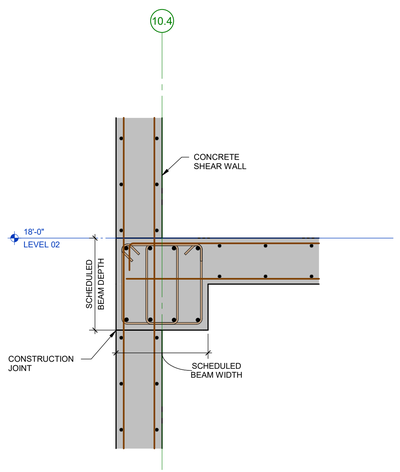
5 FRAMING DETAIL AT STEEL ROOF
3/4" = 1'-0"



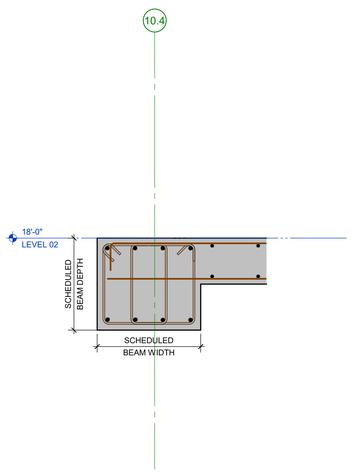
4 FRAMING DETAIL AT STEEL ROOF
3/4" = 1'-0"



3 CONCRETE FRAMING DETAIL
3/4" = 1'-0"



2 CONCRETE FRAMING DETAIL
3/4" = 1'-0"



1 CONCRETE FRAMING DETAIL
3/4" = 1'-0"

HDR ARCHITECTURE, INC.
4830 W KENNEDY BLVD #400
TAMPA, FL 33609
FL FIRM REG CERTIFICATION
AA-C000457

Walter P Moore and Associates, Inc.
201 East Kennedy Blvd, Suite 700
Tampa, Florida 33602
WPM Project No. S05-25024-00

813.221.2424
Florida DBPR License No. 3818

FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
TO THE BEST OF THE ARCHITECTS OR ENGINEERS KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH THIS SECTION AND CHAPTER 633, FLORIDA STATUES.

ORLANDO HEALTH MELBOURNE NEW FREESTANDING EMERGENCY CARE FACILITY

LAKE ANDREW DR & WILLET PLACE
MELBOURNE, FL 32940

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Email: cdayton@walterpmoore.com

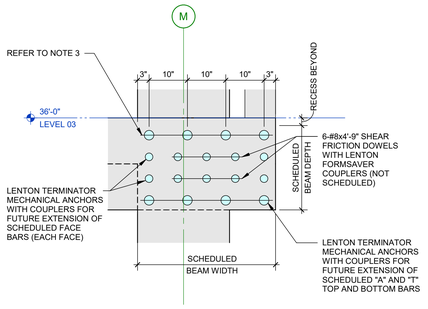
CORY ANN DAYTON, PE NO. 57441

Sheet Name
FRAMING DETAILS

Sheet Number
S-412

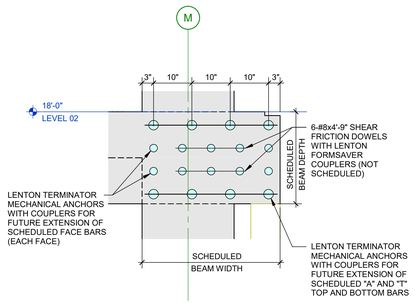
Project Status
CONSTRUCTION DOCUMENTS

3/4/2026 2:22:04 PM Autodesk Docs://10427132_Orlando_Health_Sunrise/S05-25024-01_OH Viera FSED - Structural - R2025.rvt



NOTES:
1. REINFORCEMENT COUPLERS SHALL BE TAPERED THREADED COUPLERS TO DEVELOP 125% OF THE BAR TENSILE STRENGTH.
2. GREASE COUPLER THREADS, PROVIDE PLASTIC INTERNAL COUPLER END PROTECTION, AND PROVIDE WATERPROOF COATING ALONG FACE OF SLAB TO PROTECT COUPLERS FROM CORROSION.
3. BEAM TOP REINFORCEMENT WILL BE LOWER THAN TYPICAL TO ACCOMMODATE SLAB RECESSES BEYOND.

10 FUTURE BEAM CONNECTION
3/4" = 1'-0"



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9 FUTURE BEAM CONNECTION
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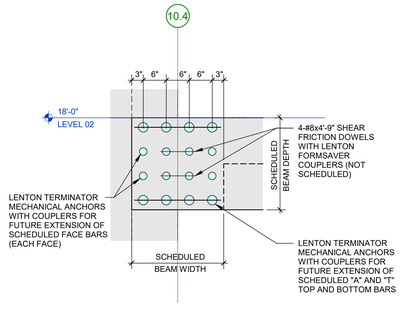

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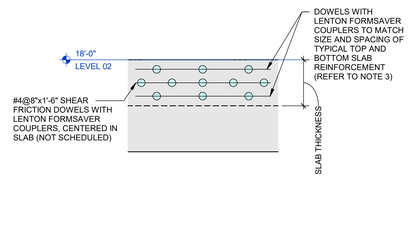
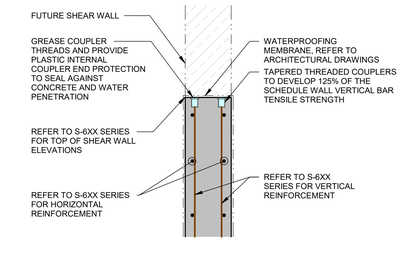
8 FUTURE BEAM CONNECTION
3/4" = 1'-0"

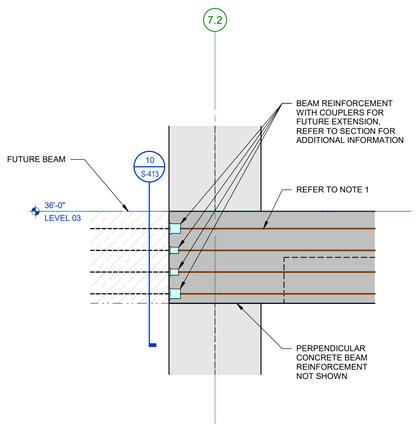


NOTES:
1. REINFORCEMENT COUPLERS SHALL BE TAPERED THREADED COUPLERS TO DEVELOP 125% OF THE BAR TENSILE STRENGTH.
2. GREASE COUPLER THREADS, PROVIDE PLASTIC INTERNAL COUPLER END PROTECTION, AND PROVIDE WATERPROOF COATING ALONG FACE OF SLAB TO PROTECT COUPLERS FROM CORROSION.
3. TOP AND BOTTOM SLAB DOWELS FOR FUTURE EXPANSION ARE IN ADDITION TO SPECIFIED TOP AND BOTTOM SLAB REINFORCEMENT IN THE CURRENT PHASE. DOWEL LENGTH SHALL BE 2'-2" FOR #5 BARS AND 2'-10" FOR #6 BARS.

7 FUTURE SLAB CONNECTION
3/4" = 1'-0"

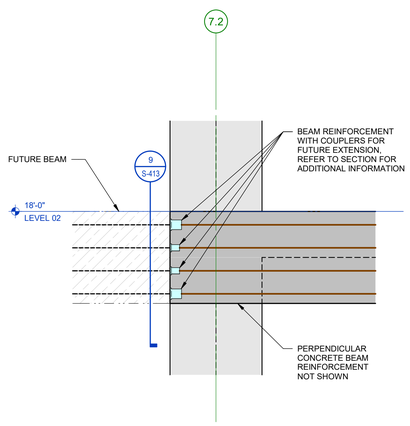


6 FUTURE SHEAR WALL CONNECTION
3/4" = 1'-0"

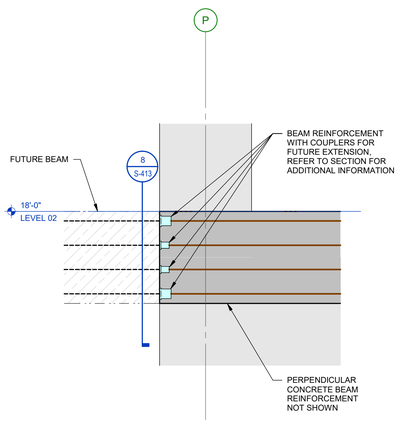


NOTES:
1. BEAM TOP REINFORCEMENT WILL BE LOWER THAN TYPICAL TO ACCOMMODATE SLAB RECESSES BEYOND.

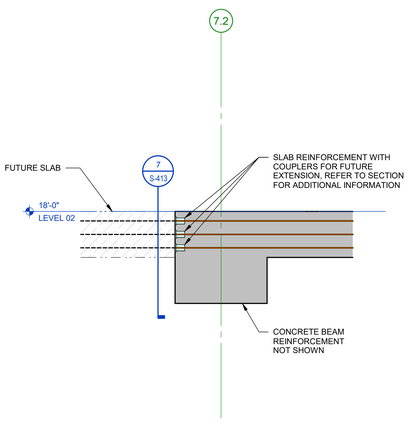
5 FUTURE BEAM CONNECTION
3/4" = 1'-0"



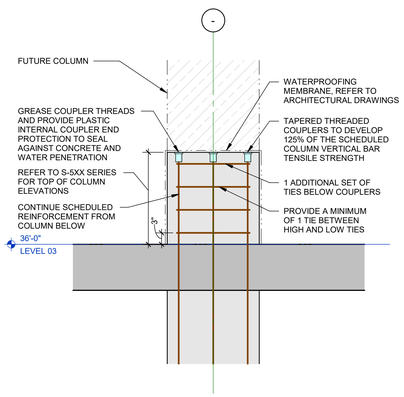
4 FUTURE BEAM CONNECTION
3/4" = 1'-0"



3 FUTURE BEAM CONNECTION
3/4" = 1'-0"



2 FUTURE SLAB CONNECTION
3/4" = 1'-0"



1 FUTURE COLUMN CONNECTION
3/4" = 1'-0"

HDR ARCHITECTURE, INC.
4830 W KENNEDY BLVD #400
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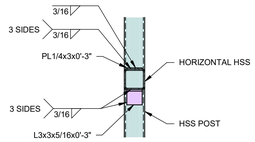
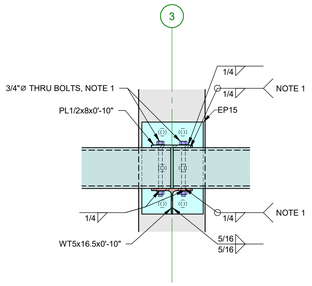
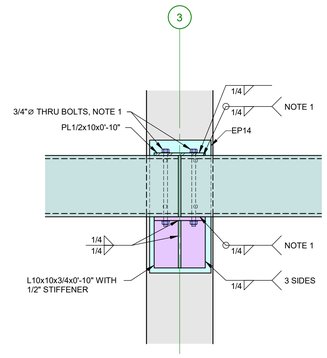


15 COILING DOOR STEEL CONNECTION
1" = 1'-0"



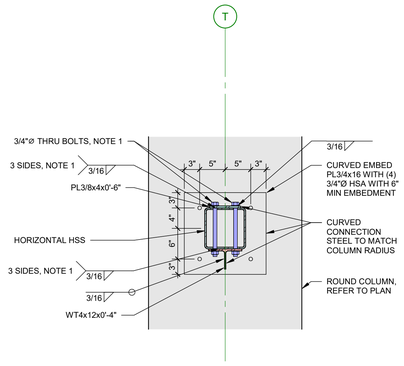
NOTES:
1. AT GRIDS 2 AND 3, PROVIDE THRU BOLTS IN LIEU OF WELDS TO HSS, AND PROVIDE LONG SLOTTED HOLES IN THE HSS PARALLEL TO THE HSS DIRECTION. DO NOT WELD HSS TO CONNECTION PLATES WHERE BOLTED CONNECTION IS PROVIDED.
2. AT GRIDS 1 AND 4.2, PROVIDE WELDS BETWEEN HSS AND CONNECTION PLATES, AND THRU BOLTS ARE NOT REQUIRED.

14 PRECAST LATERAL SUPPORT STEEL CONNECTION
1" = 1'-0"



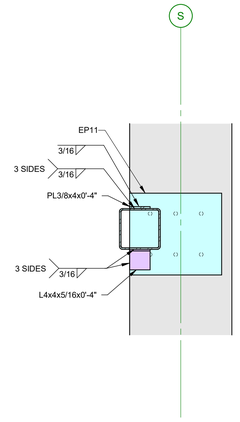
NOTES:
1. AT GRIDS 2 AND 3, PROVIDE THRU BOLTS IN LIEU OF WELDS TO HSS, AND PROVIDE LONG SLOTTED HOLES IN THE HSS PARALLEL TO THE HSS DIRECTION. DO NOT WELD HSS TO CONNECTION PLATES WHERE BOLTED CONNECTION IS PROVIDED.
2. AT GRIDS 1 AND 4.2, PROVIDE WELDS BETWEEN HSS AND CONNECTION PLATES, AND THRU BOLTS ARE NOT REQUIRED.

13 PRECAST GRAVITY SUPPORT STEEL CONNECTION
1" = 1'-0"

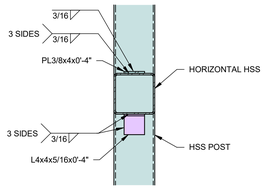


NOTES:
1. AT ONE END OF EACH HORIZONTAL HSS, PROVIDE THRU BOLTS IN LIEU OF WELDS TO HSS, AND PROVIDE LONG SLOTTED HOLES IN THE HSS PARALLEL TO THE HSS DIRECTION. DO NOT WELD HSS TO CONNECTION PLATES WHERE BOLTED CONNECTION IS PROVIDED.
2. AT THE OTHER END OF EACH HORIZONTAL HSS, PROVIDE WELDS BETWEEN HSS AND CONNECTION PLATES, AND THRU BOLTS ARE NOT REQUIRED.

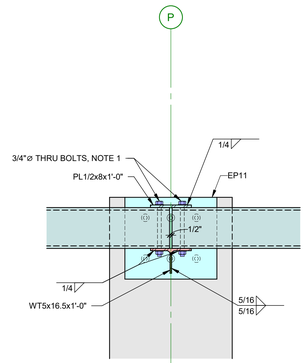
12 AMBULANCE CANOPY STEEL CONNECTION
1" = 1'-0"



11 VESTIBULE STEEL CONNECTION
1" = 1'-0"

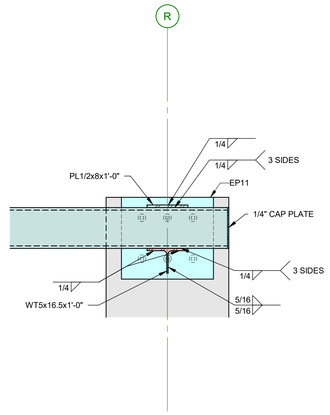


10 VESTIBULE STEEL CONNECTION
1" = 1'-0"

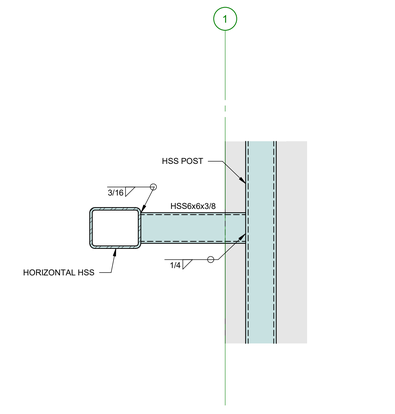


NOTES:
1. PROVIDE LONG SLOTTED HOLES IN THE HORIZONTAL HSS PARALLEL TO THE HSS DIRECTION. DO NOT WELD HSS TO CONNECTION PLATES.

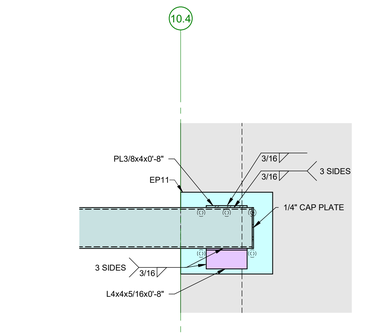
9 PARAPET STEEL CONNECTION
1" = 1'-0"



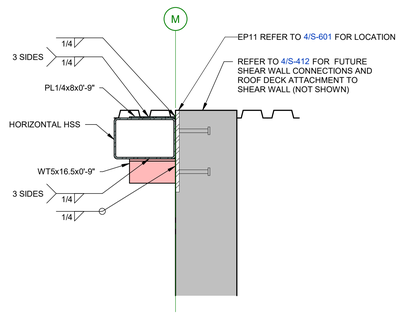
8 PARAPET STEEL CONNECTION
1" = 1'-0"



7 PARAPET STEEL CONNECTION TO TOTEM
1" = 1'-0"

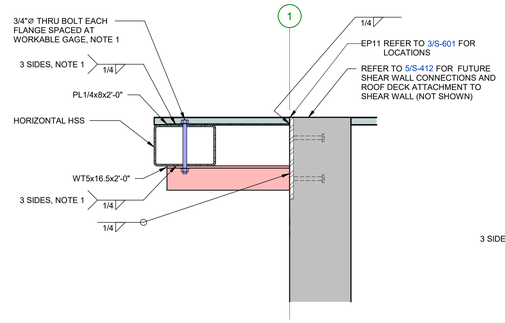


6 PARAPET STEEL CONNECTION
1" = 1'-0"



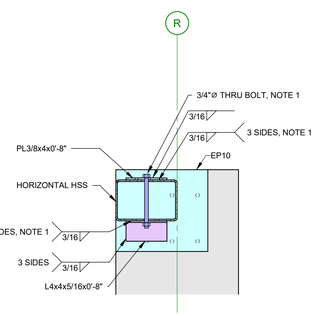
NOTES:
1. OPPOSITE END OF HORIZONTAL HSS CONNECTING TO COLUMN AT 2/M MUST BE ATTACHED USING BOLTED CONNECTION IN 3/S-421.

5 HSS CLADDING SUPPORT CONNECTION
1" = 1'-0"



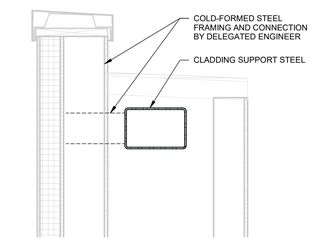
NOTES:
1. AT ONE END OF HORIZONTAL HSS, PROVIDE THRU BOLT IN LIEU OF WELDS TO HSS, AND PROVIDE LONG SLOTTED HOLES IN THE HSS PARALLEL TO THE HSS DIRECTION. DO NOT WELD HSS TO CONNECTION PLATES WHERE BOLTED CONNECTION IS PROVIDED.
2. AT THE OTHER END OF HORIZONTAL HSS, PROVIDE WELDS BETWEEN HSS AND CONNECTION PLATES, AND THRU BOLTS ARE NOT REQUIRED.

4 HSS CLADDING SUPPORT CONNECTION
1" = 1'-0"

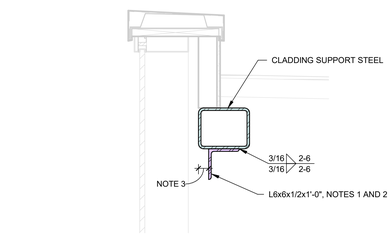


NOTES:
1. AT ONE END OF EACH HORIZONTAL HSS, PROVIDE THRU BOLT IN LIEU OF WELDS TO HSS, AND PROVIDE LONG SLOTTED HOLES IN THE HSS PARALLEL TO THE HSS DIRECTION. DO NOT WELD HSS TO CONNECTION PLATES WHERE BOLTED CONNECTION IS PROVIDED.
2. AT THE OTHER END OF EACH HORIZONTAL HSS, PROVIDE WELDS BETWEEN HSS AND CONNECTION PLATES, AND THRU BOLTS ARE NOT REQUIRED.

3 PARAPET STEEL CONNECTION
1" = 1'-0"



2 STUD WALL ATTACHMENT
1" = 1'-0"



NOTES:
1. CONTRACTOR TO COORDINATE WITH CURTAIN WALL SUPPLIER FOR LENGTH OF ANGLE REQUIRED. THE MINIMUM REQUIRED LENGTH IS 1'-0".
2. CONTRACTOR TO COORDINATE WITH CURTAIN WALL SUPPLIER FOR LOCATIONS THAT ANGLES ARE REQUIRED.
3. CONTRACTOR TO COORDINATE LOCATION OF ANGLE RELATIVE TO CURTAIN WALL WITH CURTAIN WALL SUPPLIER.

1 CURTAIN WALL ATTACHMENT
1" = 1'-0"

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SHEET REVIEWER

MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

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Sheet Name

FRAMING DETAILS

Sheet Number

S-421

Project Status

CONSTRUCTION DOCUMENTS

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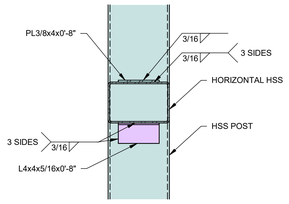
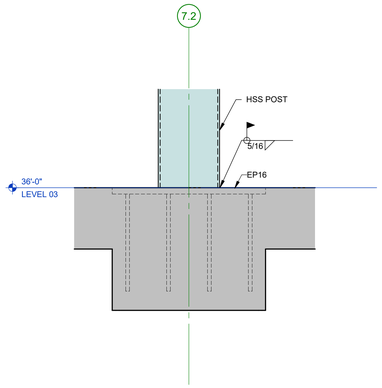
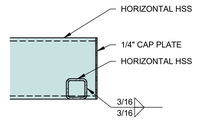


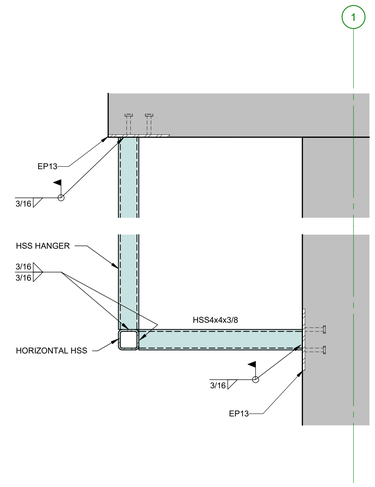
15 SCREEN WALL STEEL CONNECTION
1" = 1'-0"



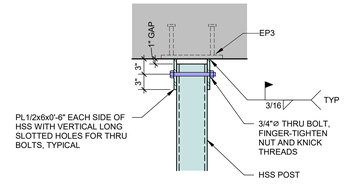
14 SCREEN WALL POST CONNECTION
1" = 1'-0"



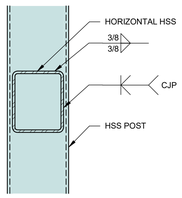
13 HSS CLADDING SUPPORT CONNECTION
1" = 1'-0"



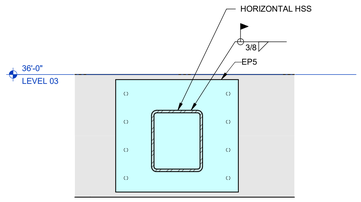
12 HSS CLADDING SUPPORT CONNECTION
1" = 1'-0"



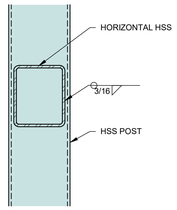
11 TOTEM STEEL CONNECTION
1" = 1'-0"



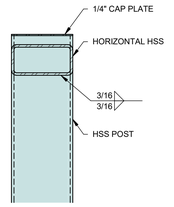
10 TOTEM STEEL CONNECTION
1" = 1'-0"



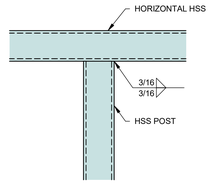
9 TOTEM STEEL CONNECTION TO BEAM
1" = 1'-0"



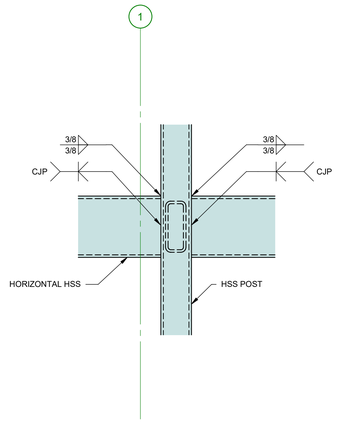
8 TOTEM STEEL CONNECTION
1" = 1'-0"



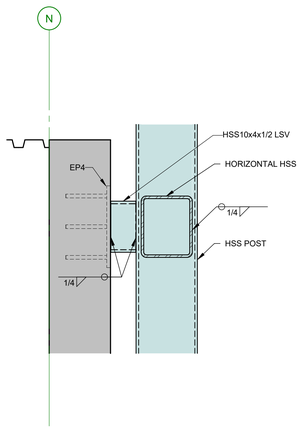
7 TOTEM STEEL CONNECTION
1" = 1'-0"



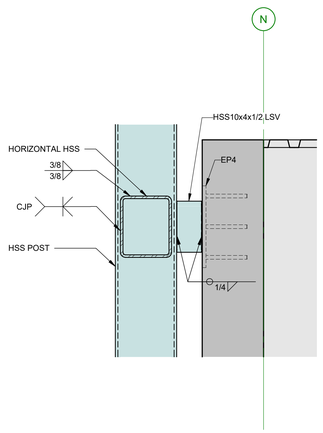
6 TOTEM STEEL CONNECTION
1" = 1'-0"



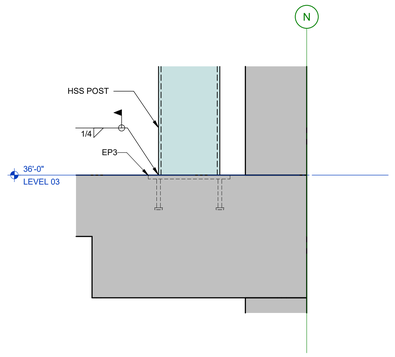
5 TOTEM STEEL CONNECTION
1" = 1'-0"



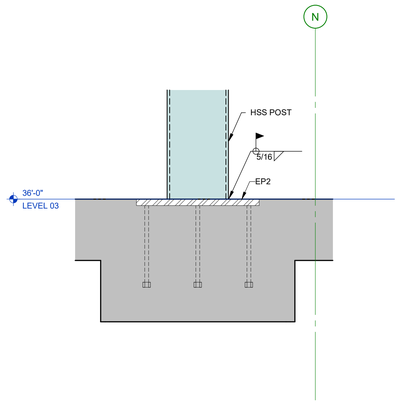
4 TOTEM STEEL CONNECTION TO SHEAR WALL
1" = 1'-0"



3 TOTEM STEEL CONNECTION TO SHEAR WALL
1" = 1'-0"



2 TOTEM POST CONNECTION
1" = 1'-0"



1 TOTEM POST CONNECTION
1" = 1'-0"

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WPM Project No. S05-25024-00

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LAKE ANDREW DR & WILLET PLACE
MELBOURNE, FL 32940

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SHEET REVIEWER	

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	3/6/2026	AHCA STAGE III

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ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

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Sheet Name
FRAMING DETAILS

Sheet Number
S-422

Project Status
CONSTRUCTION DOCUMENTS

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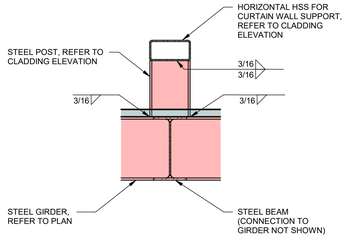
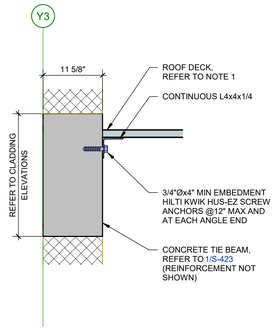
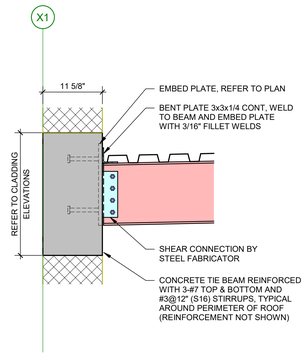


3 HSS CLADDING SUPPORT CONNECTION
1" = 1'-0"



2 DECK CONNECTION TO CONCRETE TIE BEAM
1" = 1'-0"



NOTES:
1. REFER TO GENERAL NOTES FOR DECK FASTENING PATTERN AT SUPPORTS.

1 STEEL BEAM AND DECK CONNECTION TO CONCRETE TIE BEAM
1" = 1'-0"

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Sheet Name

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Sheet Number

S-423

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0 1/2" 1" 2"

CONCRETE BEAM SCHEDULE

MARK	SIZE WIDTH	SIZE DEPTH	REINFORCEMENT TOP BARS LEFT	TOP BARS CTR	TOP BARS RIGHT	BOTTOM BARS LEFT	BOTTOM BARS CTR	BOTTOM BARS RIGHT	FACE BARS EACH SIDE	STIRRUPS BAR SIZE	STIRRUPS TYPE	STIRRUPS SPACING AT EACH END FROM FACE OF SUPPORT	REMARKS
BM201	24"	24"	5-#7D(2i)			5-#7X(2i)			2-#6	#4	S28	1@2/7@6/R@9	REINFORCEMENT IS CONTINUOUS THROUGH SHEAR WALL
BM202	36"	24"	4-#7A(2i) 3-#7B			4-#7T(2i) 1-#7W			2-#6	#4	S27	1@2/R@6	-
BM203	36"	24"	5-#7A(2i) 3-#9B		4-#9A(2i)	4-#7T(2i) 3-#9V			2-#6	#4	S27	1@2/R@6	REFER TO 4/S-413 FOR ADDITIONAL REINFORCEMENT AND COUPLERS REQUIRED FOR FUTURE EXPANSION
BM204	24"	24"	3-#7Da(2i) 3-#7Ea			4-#7X(2i)	2-#9V		3-#6	#4	S27	1@2/R@6	PROVIDE Da AND Ea BARS AT LEFT END WHERE BARS CAN EXTEND INTO ADJACENT SLAB
BM205	24"	24"	4-#9A(2i) 3-#9B				4-#7T(2i) 3-#7W		3-#6	#4	S27	1@2/13@6/R@9	-
BM206	24"	24"	4-#9A(2i) 3-#9B		3-#7D(2i) 3-#7E		2-#9V	4-#7X(2i)	3-#6	#4	S27	1@2/R@6	PROVIDE Da AND Ea BARS AT RIGHT END WHERE BARS CAN EXTEND INTO ADJACENT SHEAR WALL OR STAIR LANDING
BM207	24"	24"	4-#7D(2i)		1-#7B	4-#7X(2i)	1-#7V		2-#6	#4	S27	1@2/30@6/R@10	-
BM208	24"	24"	4-#9A(2i)				4-#9T(2i)		2-#6	#4	S27	1@2/R@5	-
BM209	24"	24"	5-#9A(2i)		4-#7D(2i)		1-#7V	4-#7X(2i)	2-#6	#4	S27	1@2/30@6/R@10	-
BM210	30"	24"	5-#9A(2i) 2-#7D 1-#9B[L2]		1-#7B 2-#9B[L2]	2-#7X	4-#9T(2i) 2-#9W		2-#6	#4	S32	1@2/4@4/6@5/R@8	PROVIDE ONE D AND ONE X BAR ON EACH SIDE OF BEAM
BM211	30"	24"	5-#9A(2i) 1-#9B[L2]		1-#7B 2-#7D 2-#9B[L2]		4-#9T(2i) 2-#9W	2-#7X	2-#6	#4	S32	1@2/4@4/6@5/R@8	PROVIDE ONE D AND ONE X BAR ON EACH SIDE OF BEAM
BM212	38 1/2"	24"	6-#7D(2i)			5-#7X(2i)	2-#7V		2-#6	#4	S27	1@2/R@6	-
BM213	38 1/2"	24"	5-#9A(2i)				5-#7T(2i) 1-#7V		2-#6	#4	S27	1@2/R@6	-
BM214	40 1/2"	24"	5-#9A(2i)		6-#7D(2i)		2-#7V	5-#7X(2i)	2-#6	#4	S28	1@2/R@6	REINFORCEMENT IS CONTINUOUS THROUGH SHEAR WALL
BM215	24"	24"	2-#7Da(2i) 2-#7Ea			4-#7X(2i)	2-#9V		3-#6	#4	S27	1@2/11@6/R@9	PROVIDE Da AND Ea BARS AT LEFT END WHERE BARS CAN EXTEND INTO ADJACENT SLAB
BM216	27"	24"	7-#7D(2i)			4-#7X(2i)	2-#9V		2-#6	#4	S27	1@2/R@6	-
BM217	24"	24"	5-#9A(2i)		4-#7A(2i)		4-#7T(2i) 1-#7W		2-#6	#4	S27	1@2/23@7/R@10	REINFORCEMENT IS CONTINUOUS THROUGH SHEAR WALL. REFER TO 3/S-413 FOR ADDITIONAL REINFORCEMENT AND COUPLERS REQUIRED FOR FUTURE EXPANSION
BM218	27"	24"	4-#7Ma(2i)				4-#7M(2i)		2-#6	#4	S28	1@2/R@10	REINFORCEMENT IS CONTINUOUS THROUGH SHEAR WALL
BM219	24"	24"			4-#7Mab		4-#7M			#4	S28	1@2/R@10	-
BM220	24"	24"	6-#7Ma 3-#7Mab[L2]				6-#7M 3-#7M[L2]			#4	S28	1@2/R@10	PROVIDE Ma BARS AT LEFT END WHERE BARS CAN EXTEND INTO ADJACENT SLAB
BM221	36"	24"	6-#7D(2i)			6-#7X(2i)			3-#6	#4	S27	1@2/11@6/R@9	-
BM222	36"	24"	6-#9A(2i)		6-#7D(2i)			6-#7X(2i)	3-#6	#4	S27	1@2/11@6/R@9	-
BM223	40"	24"		14-#9Mab(3i) 5-#9Mab[L2]			6-#7M 8-#9V(4i) 12-#9V[L2]		2-#6	#4	S27	1@2/R@6	PROVIDE Mab BARS WHERE BARS CAN EXTEND INTO ADJACENT SLAB
BM224	36"	24"		12-#9Mab			5-#7M 7-#9V 5-#9V[L2]			#4	S27	1@2/R@10	PROVIDE Mab BARS WHERE BARS CAN EXTEND INTO ADJACENT SLAB
BM301	36"	24"	5-#7D(2i)			5-#7X(2i)			2-#6	#4	S28	1@2/7@6/R@9	REINFORCEMENT IS CONTINUOUS THROUGH SHEAR WALL
BM302	36"	24"	6-#9A(2i)				5-#7T(3i) 2-#7W		2-#6	#4	S27	1@2/R@6	-
BM303	36"	22"	6-#9A(2i) 2-#9B				5-#7T(3i) 2-#9W		2-#6	#4	S27	1@2/R@6	-
BM304	36"	22"	6-#9A(2i) 3-#9B		4-#9A(2i)		4-#7T(3i) 3-#9V		2-#6	#4	S27	1@2/R@6	REFER TO 5/S-413 FOR ADDITIONAL REINFORCEMENT AND COUPLERS REQUIRED FOR FUTURE EXPANSION
BM305	30"	24"	4-#7Da(2i) 3-#7Ea			4-#7X(3i)	3-#9V		3-#6	#4	S27	1@2/R@6	PROVIDE Da AND Ea BARS AT LEFT END WHERE BARS CAN EXTEND INTO ADJACENT SLAB
BM306	30"	24"	6-#9A(2i) 3-#9B[L2]				4-#7T(3i) 3-#7W		3-#6	#4	S27	1@2/11@6/R@9	-
BM307	30"	24"	6-#9A(2i) 3-#9B[L2]		4-#7D(2i) 3-#7E		3-#9V	4-#7X(3i)	3-#6	#4	S27	1@2/R@6	PROVIDE Da AND Ea BARS AT RIGHT END WHERE BARS CAN EXTEND INTO ADJACENT SHEAR WALL OR STAIR LANDING
BM308	30"	22"	4-#7D 3-#7E		1-#7B	4-#7X	3-#9V			#4	S27	1@2/R@9	-
BM309	30"	22"	6-#9A(2i) 3-#9B[L2]				4-#9T			#4	S27	1@2/R@9	-
BM310	27"	24"	6-#9A(2i)		4-#7D(2i)			4-#7X(2i)	2-#6	#4	S28	1@2/R@8	REINFORCEMENT IS CONTINUOUS THROUGH SHEAR WALL
BM311	24"	24"	4-#7D(2i) 2-#7E			4-#7X(2i)	2-#7V		2-#6	#4	S27	1@2/R@6	-
BM312	24"	24"	4-#7A(2i) 4-#7B				4-#7T(2i) 1-#7W		2-#6	#4	S27	1@2/R@6	-
BM313	24"	24"	4-#7A(2i) 4-#7B		4-#7D(2i) 2-#7E		2-#7V	4-#7X(2i)	2-#6	#4	S27	1@2/30@6/R@9	-
BM314	38 1/2"	24"	6-#7D(2i)			5-#7X(2i)	2-#7V		2-#6	#4	S27	1@2/R@6	-
BM315	38 1/2"	24"	6-#9A(3i)				5-#7T(3i) 2-#7V		2-#6	#4	S27	1@2/R@6	-
BM316	40 1/2"	24"	6-#9A(2i)		6-#7D(2i)		2-#7V	5-#7X(2i)	2-#6	#4	S28	1@2/R@6	REINFORCEMENT IS CONTINUOUS THROUGH SHEAR WALL
BM317	27"	24"	7-#7D(2i)			4-#7X(2i)	2-#9V		2-#6	#4	S27	1@2/28@6/R@9	-
BM318	24"	24"	5-#9A(3i)		4-#7A(2i)		4-#7T(2i) 3-#7W		2-#6	#4	S27	1@2/R@6	REINFORCEMENT IS CONTINUOUS THROUGH SHEAR WALL. REFER TO 3/S-413 FOR ADDITIONAL REINFORCEMENT AND COUPLERS REQUIRED FOR FUTURE EXPANSION
BM319	38"	24"		11-#7M			11-#7M		3-#6	#4	S28	1@2/R@6	PROVIDE Ma BARS AT RIGHT END WHERE BARS CAN EXTEND INTO SHEAR WALL
BM320	30"	24"			7-#6Ma		5-#6M 4-#9V		2-#6	#4	S27	1@2/12@6/R@9	-

BEAM SCHEDULE NOTES:
1. REFER TO OTHER DETAILS ON THIS SHEET FOR BAR PLACEMENT DIAGRAM, BAR PLACEMENT NOTES, AND TYPICAL STIRRUP TYPES.
2. WHERE BEAM PLAN WIDTH IS LARGER THAN THE SCHEDULED BEAM WIDTH, REFER TO 6/S-403 FOR TYPICAL WIDENED CONCRETE BEAM DETAIL.
3. WHERE BEAM DEPTH IS LARGER THAN THE SCHEDULED BEAM DEPTH, REFER TO 7/S-403 FOR TYPICAL DEEPENED CONCRETE BEAM DETAIL.
4. WHERE THE NUMBER OF SCHEDULED TOP OR BOTTOM BARS IS LESS THAN THE NUMBER OF STIRRUP LEGS, PROVIDE ADDITIONAL #4 BARS AS REQUIRED.

5 CONCRETE BEAM SCHEDULE
12" = 1'-0"

CONCRETE SLAB SCHEDULE

TYPE	THICKNESS	REINFORCEMENT
S12A	12"	#6@12" EACH DIRECTION TOP #5@12" EACH DIRECTION BOTTOM
S12B	12"	#6@12" EACH DIRECTION TOP #5@6" EACH DIRECTION BOTTOM

NOTES:
1. ALL SCHEDULED TYPICAL SLAB REINFORCEMENT SHALL BE CONTINUOUS. WHERE SPLICING OF REINFORCEMENT IS REQUIRED, PROVIDE CLASS B LAP SPLICE AT COLUMNS FOR BOTTOM REINFORCEMENT AND AT MID-SPAN FOR TOP REINFORCEMENT. REFER TO 15/S-004 FOR SPLICE LENGTHS.
2. AT LEAST TWO BOTTOM BARS IN EACH DIRECTION MUST PASS WITHIN THE COLUMN VERTICAL REINFORCEMENT CAGE, AND THESE BARS SHALL BE HOOKED AT DISCONTINUOUS ENDS.
3. ALL TOP BARS SHALL BE HOOKED AT DISCONTINOUS ENDS. ALL BOTTOM BARS FRAMING INTO SHEAR WALLS SHALL BE HOOKED AT DISCONTINUOUS ENDS.
4. REFER TO PLAN FOR ADDITIONAL SLAB REINFORCEMENT REQUIRED BEYOND SCHEDULED REINFORCEMENT. ADDITIONAL BARS WITHOUT A SPACING DESIGNATION SHALL BE PLACED AS CLOSE TO THE COLUMN LINE AS POSSIBLE IN BETWEEN THE SCHEDULED REINFORCEMENT AT A SPACING OF 4". ADDITIONAL BARS SHALL BE DISTRIBUTED EQUALLY ON EACH SIDE OF THE INTERIOR COLUMNS.
5. TYPICAL ADDITIONAL SLAB REINFORCEMENT IS INDICATED ON PLAN AS FOLLOWS:

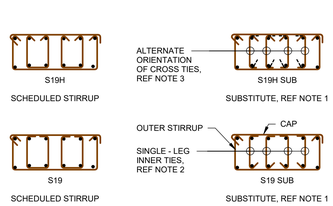
13 CONCRETE SLAB SCHEDULE
NO SCALE

SHEAR WALL LINK BEAM SCHEDULE

MARK	WIDTH (IN)	DEPTH (IN)	LONGITUDINAL REINFORCEMENT TOP BAR	LONGITUDINAL REINFORCEMENT BOTTOM BAR	LONGITUDINAL REINFORCEMENT FACE BARS (EACH FACE)	SHEAR REINFORCEMENT SIZE	SHEAR REINFORCEMENT TYPE	SHEAR REINFORCEMENT SPACING (EACH END FROM FACE OF SUPPORT)	REMARKS
LB12x22	12"	22"	3-#7	3-#7	2-#5	#4	S16	1@2/R@12	
LB12x24	12"	24"	3-#7	3-#7	2-#5	#4	S16	1@2/R@12	
LB12x30	12"	30"	3-#7	3-#7	3-#5	#4	S16	1@2/R@12	
LB12x108	12"	108"	5-#7	5-#7	9-#5	#4	S16	1@2/R@10	
LB12x112	12"	112"	3-#7	3-#7	10-#5	#4	S16	1@2/R@12	
LB12x114	12"	114"	5-#7	5-#7	10-#5	#4	S16	1@2/R@12	

NOTES:
1. REFER TO SHEAR WALL ELEVATIONS ON S-601 THROUGH S-607 FOR BEAM MARKS.
2. DEVELOP TOP AND BOTTOM REINFORCEMENT INTO ADJACENT WALLS WHERE POSSIBLE. REFER TO 13/S-004 FOR DEVELOPMENT LENGTHS. OTHERWISE, HOOK REINFORCEMENT AT DISCONTINUOUS ENDS.
3. CONTINUE BEAM FACE REINFORCEMENT TO ENTIRE EXTENT OF ADJACENT WALLS TO SERVE AS HORIZONTAL WALL REINFORCEMENT. REFER TO DETAILS 1, 2, AND 3/S-402 FOR CONDITIONS AT SHEAR WALL ENDS, CORNERS, AND INTERSECTIONS.
4. REFER TO 2/S-451 FOR STIRRUP TYPES.

8 SHEAR WALL LINK BEAM SCHEDULE
NO SCALE



SUBSTITUTE STIRRUP CONFIGURATION EXAMPLES

NOTES:
1. AT CONTRACTOR'S OPTION, INNER BEAM STIRRUP LEGS MAY BE REPLACED WITH SINGLE-LEG TIES IN LIEU OF U-SHAPED STIRRUPS. OUTER LEGS AND CAP SHALL REMAIN AS SCHEDULED/ SPECIFIED.
2. SINGLE-LEG TIES SHALL BE CROSS TIES WITH 90 DEGREE HOOKS AT ONE END AND 135 DEGREE HOOKS AT THE OPPOSITE END. REFER TO 2/S-005.
3. THE SUBSTITUTE CONFIGURATION DESCRIBED IN THIS DETAIL MAY BE USED FOR THE FOLLOWING STIRRUP TYPES: S11, S12, S19, S19H, S20, S20H, S21, S27, S27H, S28, S28H, S29, S32, S32H, S33, S33H
4. FOR ADDITIONAL REQUIREMENTS AND INFORMATION, REFER TO 2/S-451.

3 TYPICAL STIRRUP LEG SUBSTITUTION
NO SCALE

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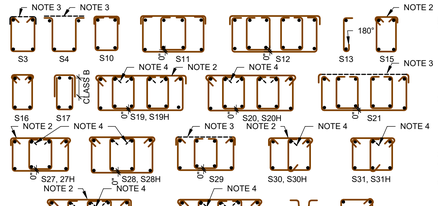

NOTES:
1. ALL HOOKS SHALL BE FABRICATED AS REQUIRED BY ACI. REFER TO 2/S-005.
2. THE TOP CAP FOR CLOSED STIRRUPS S15, S19, S27, S30, AND S32 SHALL BE PLACED SUCH THAT THE 90-DEGREE HOOK IS:
   A. CONFINED BY THE SLAB AT PERIMETER CONDITIONS (SLAB ON ONE SIDE ONLY), AND
   B. ON ALTERNATE SIDES AT ADJACENT HOOPS IN INTERIOR CONDITIONS (SLAB ON BOTH SIDES).
3. PROVIDE TOP CAP FOR OPEN STIRRUPS S3, S4, S21, AND S29 TO MATCH SCHEDULED SIZE @48".
4. STIRRUPS SCHEDULED WITH "H" SUFFIX SHALL MEET THE REQUIREMENTS OF SEISMIC HOOPS WITH STIRRUP LEGS THAT TERMINATE WITH 135° SEISMIC HOOKS AS SHOWN IN DASHED LINES (NO 90° HOOKS). CAPS MAY RETAIN 90° HOOKS WHERE SHOWN, AND SHALL BE PROVIDED PER NOTE 2. S15 AND S16 STIRRUPS MEET SEISMIC HOOP REQUIREMENTS WITHOUT MODIFICATION.
5. INTERNAL STIRRUP LEGS SHALL BE APPROXIMATELY EQUALLY SPACED ACROSS BEAM WIDTH.
6. REFER TO 3/S-451 FOR SUBSTITUTE STIRRUP CONFIGURATIONS AT CONTRACTOR'S OPTION.

2 TYPICAL STIRRUP TYPES
NO SCALE

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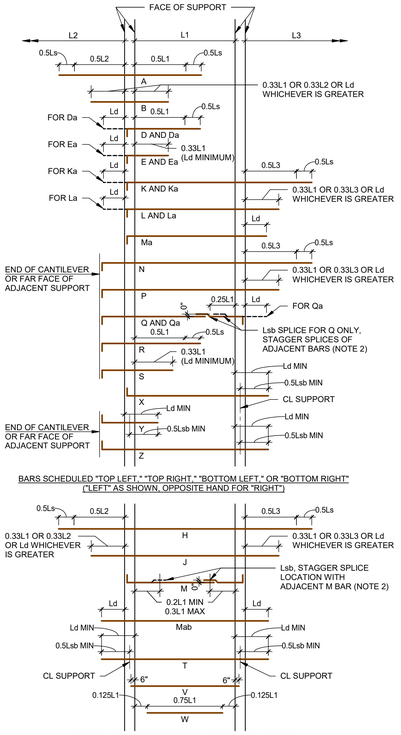

BARS SCHEDULED "TOP LEFT," "TOP RIGHT," "BOTTOM LEFT," OR "BOTTOM RIGHT" ("LEFT" AS SHOWN, OPPOSITE HAND FOR "RIGHT")

BARS SCHEDULED "TOP CENTER" OR "BOTTOM CENTER"

NOTES:
1. REFER TO TYPICAL BAR PLACEMENT NOTES.
2. WHERE OVERALL BAR LENGTH DOES NOT PERMIT SPLICE AS SHOWN, SPLICES SHALL BE OMITTED.

6 TYPICAL BAR PLACEMENT DIAGRAM
NO SCALE

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NOTES:
1. SCHEDULED LEFT (L) AND RIGHT (R) ENDS OF MEMBERS ARE RELATIVE TO THE ORIENTATION OF THE MEMBER MARK, AS SHOWN BELOW:

2. (ni) NOTED IN BEAM SCHEDULE AFTER BAR TYPE INDICATES MINIMUM NUMBER OF BARS ("n") WHICH ARE INTEGRITY BARS AND SHALL BE PLACED WITHIN THE VERTICAL COLUMN REINFORCING STEEL CAGE. REMAINDER OF BARS MAY BE PLACED WITHIN, OR OUTSIDE, THE COLUMN CAGE. FOR EXAMPLE, 4-#6A(2i)INDICATES A TOTAL OF (4) #6 "A" BARS, 2 OF WHICH ARE CLASSIFIED AS "INTEGRITY BARS" AND MUST BE PLACED WITHIN THE COLUMN CAGE. PROVIDE SUPPLEMENTAL BARS AS REQUIRED FOR STIRRUPS PER NOTE 10.
3. Ld EQUALS TENSION DEVELOPMENT LENGTH, UNLESS NOTED OTHERWISE IN SCHEDULE.
4. Lsb EQUALS BAR "CLASS B" TENSION LAP SPLICE LENGTH, UNLESS NOTED OTHERWISE IN SCHEDULE.
5. Ls REPRESENTS A LAP SPLICE WITH ADJACENT SCHEDULED BARS. WHEN LAP SPLICING BARS OF DIFFERENT SIZES, REFER TO NOTES IN TYPICAL BEAM AND GIRDER TENSION DEVELOPMENT AND LAP SPLICE LENGTHS DETAILS. THIS LAP LENGTH SHALL BE TAKEN AS:
   A. "CLASS B" TENSION LAP SPLICE LENGTH Lsb FOR BAR GROUPS SCHEDULED WITH INTEGRITY ("I") BARS. FOR EXAMPLE, 4-#6A(2i) INDICATES THE (4) #6 "A" BARS SHALL HAVE A CLASS B LAP SPLICE.
   B. "CLASS A" TENSION LAP SPLICE LENGTH FOR ALL OTHER CONDITIONS NOT OTHERWISE NOTED.
6. HOOKS SHOWN ARE GRAPHICAL REPRESENTATIONS AND MAY NOT REFLECT ACTUAL CONDITION. UNLESS NOTED OTHERWISE, TOP HOOKS SHALL HOOK DOWN AND BOTTOM HOOKS SHALL HOOK UP.
7. ALL HOOKS SHOWN ARE STANDARD HOOKS (90° OR 180°).
8. SPLICES IN SCHEDULED BARS LESS THAN 60'-0" LENGTH ARE NOT PERMITTED. SPLICES IN SCHEDULED BARS MORE THAN 60'-0" LENGTH SHALL BE REFERRED TO ENGINEER FOR APPROVAL, UNLESS A SPECIFIC BAR TYPE IS SHOWN IN THE SCHEDULE. FACE BARS SHALL BE CONTINUOUS FOR THE FULL LENGTH OF THE MEMBER. EXTEND FACE BARS A DISTANCE Ld FROM CENTERLINE OF CONTINUOUS SUPPORTS AND PROVIDE 90° HOOKS AT DISCONTINUOUS ENDS. NUMBER OF BARS SHOWN ON SCHEDULE SHALL BE PROVIDED ON EACH FACE OF MEMBER.
9. EVERY SINGLE STIRRUP LEG MUST BE HOOKED AROUND A LONGITUDINAL REINFORCING BAR WHERE THE NUMBER OF SCHEDULED TOP OR BOTTOM LONGITUDINAL REINFORCING BARS IS LESS THAN THE NUMBER OF STIRRUP LEGS AT ANY POINT WITHIN THE BEAM SPAN. PROVIDE ADDITIONAL #4 LONGITUDINAL BARS AS REQUIRED SUCH THAT EVERY STIRRUP LEG IS HOOKED AROUND A LONGITUDINAL BAR. LAP ADDITIONAL #4 BARS 2'-0" WITH SCHEDULED REINFORCEMENT WHERE APPLICABLE.
10.

1 TYPICAL BAR PLACEMENT NOTES
NO SCALE

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FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
TO THE BEST OF THE ARCHITECTS OR ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH THIS SECTION AND CHAPTER 633, FLORIDA STATUES.

ORLANDO HEALTH MELBOURNE NEW FREESTANDING EMERGENCY CARE FACILITY

LAKE ANDREW DR & WILLET PLACE
MELBOURNE, FL 32940

ORLANDO HEALTH®

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PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
LANDSCAPE ARCHITECT	MCKINTOSH ASSOCIATES, an LJA COMPANY
CIVIL ENGINEER	BSE CONSULTANTS, an LJA COMPANY
STRUCTURAL ENGINEER	WALTER P. MOORE & ASSOCIATES, INC.
MECHANICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
ELECTRICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
PLUMBING ENGINEER	I.C. TOMASSON ASSOCIATES, INC
INTERIOR DESIGNER	HDR ARCHITECTURE, INC.
EQUIPMENT PLANNER	IMEG CORP.
WAYFINDING	HDR ARCHITECTURE, INC.
SHEET REVIEWER	

MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

CORY ANN DAYTON LICENSED No. 57441 STATE OF FLORIDA PROFESSIONAL ENGINEER

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CORY ANN DAYTON, PE NO. 57441

Sheet Name

BEAM AND SLAB SCHEDULES

Sheet Number

S-451

Project Status

CONSTRUCTION DOCUMENTS

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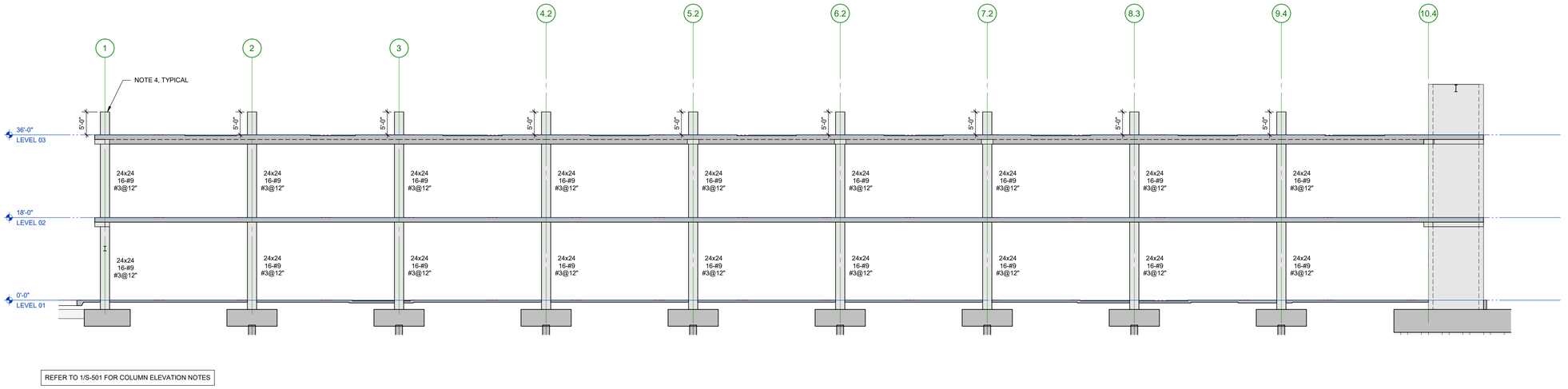
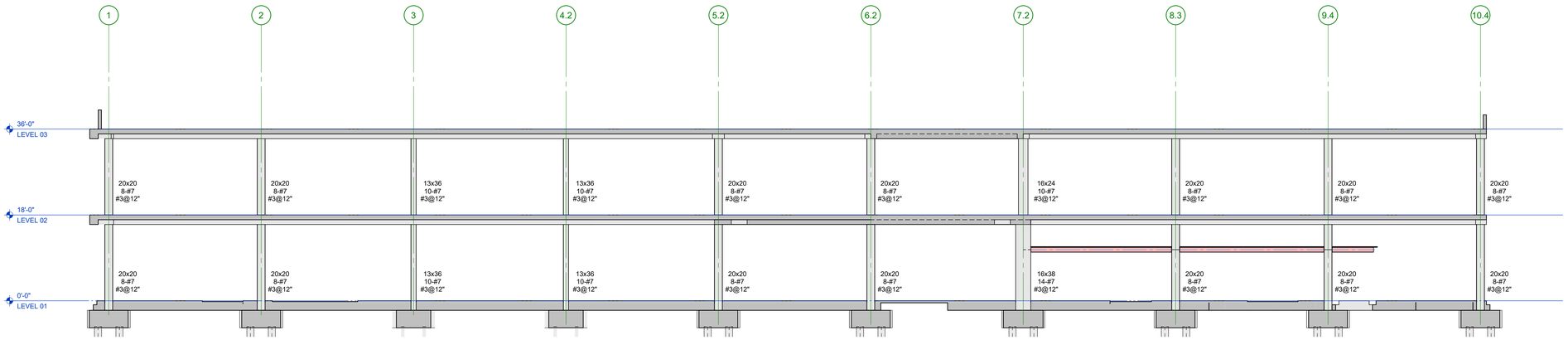


REFER TO 1/S-501 FOR COLUMN ELEVATION NOTES

3 COLUMN ELEVATION ALONG GRID R

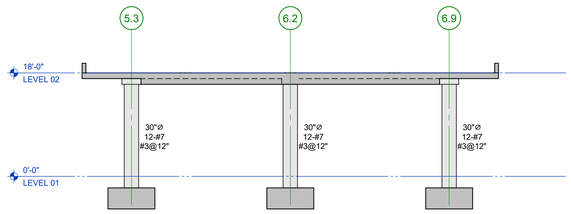
3/32" = 1'-0"



REFER TO 1/S-501 FOR COLUMN ELEVATION NOTES

2 COLUMN ELEVATION ALONG GRID S

3/32" = 1'-0"



COLUMN ELEVATION NOTES:

1. REFER TO S-401 FOR COLUMN TYPICAL DETAILS.
2. ALL COLUMN DOWELS SHALL MATCH SCHEDULED VERTICAL REINFORCEMENT, UNLESS NOTED OTHERWISE.
3. ALL LAP SPLICES SHALL BE CLASS B. REFER TO 5/S-004 FOR COLUMN LAP SPLICE LENGTHS.
4. UNLESS NOTED OTHERWISE, AT ALL COLUMN STUBS PROVIDE TAPERED THREADED MECHANICAL COUPLERS AT THE TOP OF ALL COLUMN VERTICAL REINFORCEMENT TO ACCOMMODATE FUTURE EXPANSION. REFER TO 1/S-413.

1 COLUMN ELEVATION ALONG GRID T

3/32" = 1'-0"

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Sheet Name

COLUMN ELEVATIONS

Sheet Number

S-501

Project Status

CONSTRUCTION DOCUMENTS

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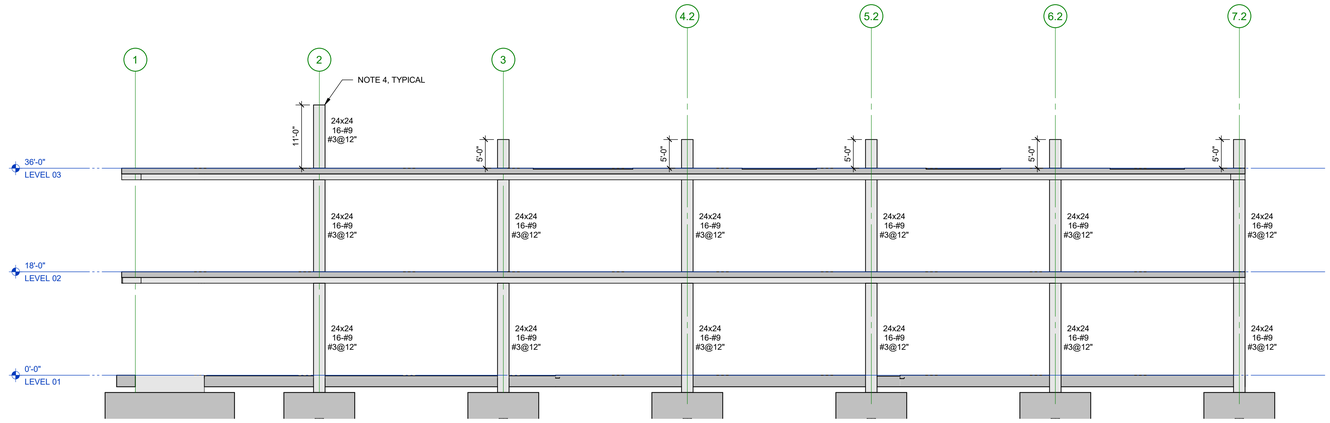
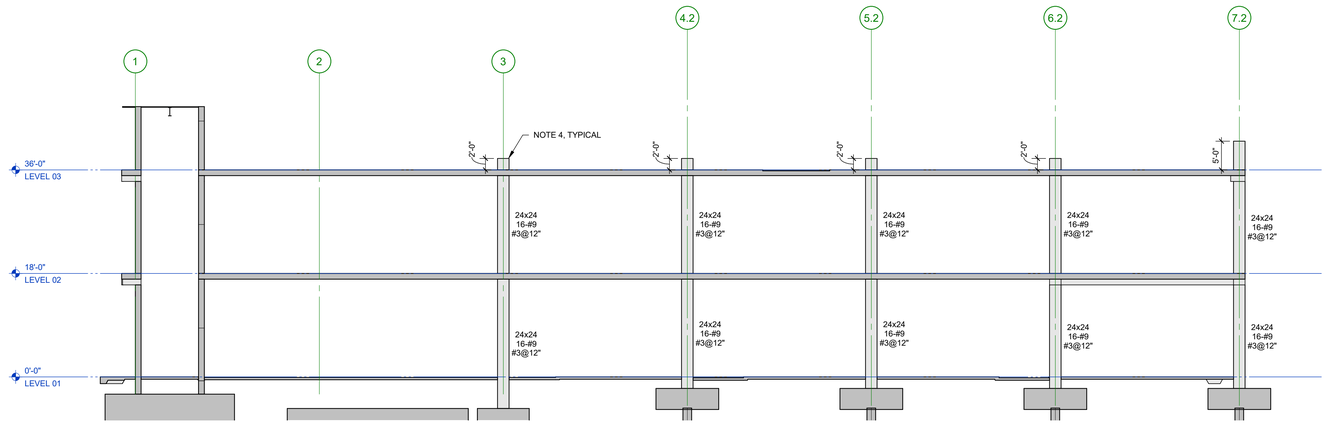


REFER TO 1/S-501 FOR COLUMN ELEVATION NOTES

3 COLUMN ELEVATION ALONG GRID M

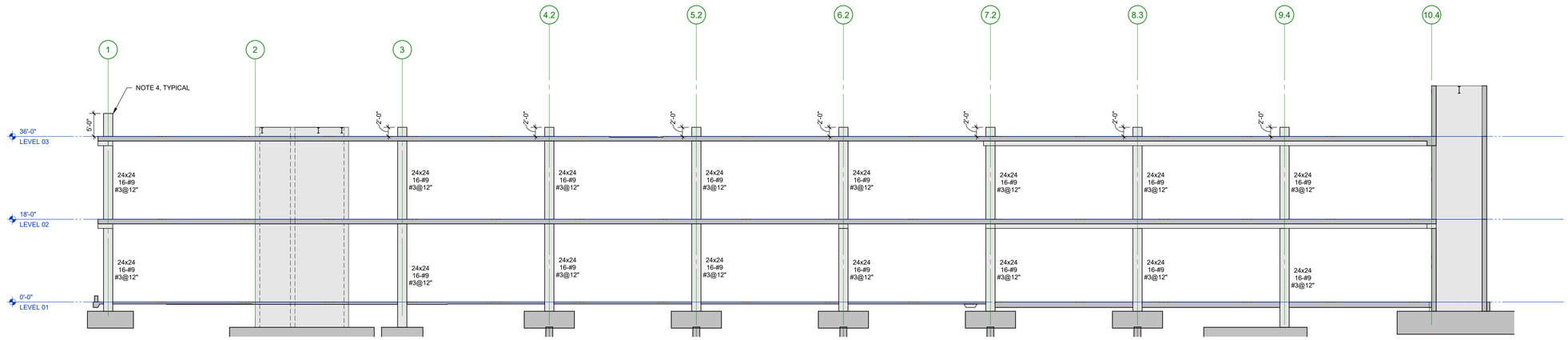
3/32" = 1'-0"



REFER TO 1/S-501 FOR COLUMN ELEVATION NOTES

2 COLUMN ELEVATION ALONG GRID N

3/32" = 1'-0"



REFER TO 1/S-501 FOR COLUMN ELEVATION NOTES

1 COLUMN ELEVATION ALONG GRID P

3/32" = 1'-0"

HDR ARCHITECTURE, INC.
4830 W KENNEDY BLVD #400
TAMPA, FL 33609
FL FIRM REG CERTIFICATION
AA-C000457

walter p moore

Walter P Moore and Associates, Inc.
201 East Kennedy Blvd, Suite 700
Tampa, Florida 33602
WPM Project No. S05-25024-00

813.221.2424
Florida DBPR License No. 3818

FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
TO THE BEST OF THE ARCHITECTS OR ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH THIS SECTION AND CHAPTER 633, FLORIDA STATUES.

ORLANDO HEALTH MELBOURNE NEW FREESTANDING EMERGENCY CARE FACILITY

LAKE ANDREW DR & WILLET PLACE
MELBOURNE, FL 32940

ORLANDO HEALTH®

PROJECT MANAGER	HDR ARCHITECTURE, INC.
PROJECT DESIGNER	HDR ARCHITECTURE, INC.
PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
LANDSCAPE ARCHITECT	MCKINTOSH ASSOCIATES, an LJA COMPANY
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SHEET REVIEWER

MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

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CORY ANN DAYTON, PE NO. 57441

Sheet Name

COLUMN ELEVATIONS

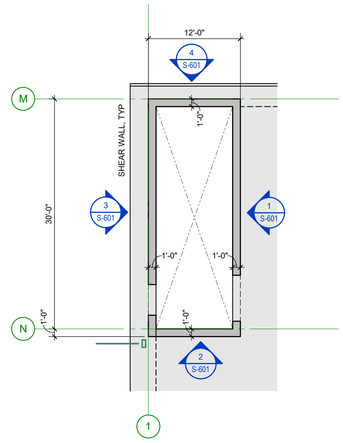
Sheet Number

S-502

Project Status

CONSTRUCTION DOCUMENTS

3/4/2026 2:22:14 PM Autodesk Docs://1P427132_Orlando_Health_SunriseS05-25024-01 - OH Viera FSED - Structural - R2025.rvt

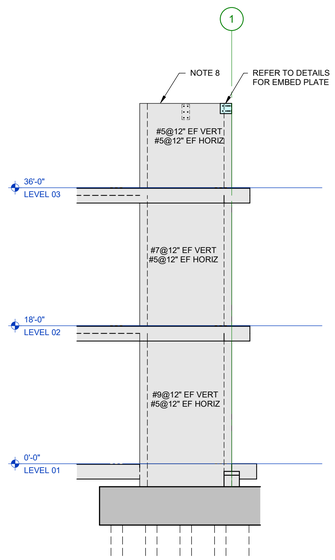


SHEAR WALL #1 KEY PLAN AND NOTES

1/8" = 1'-0"

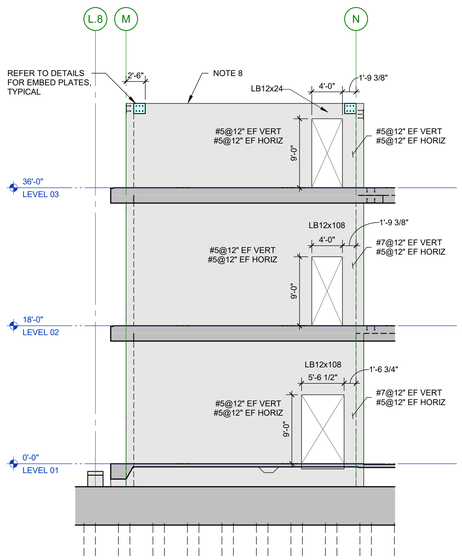
SHEAR WALL ELEVATION NOTES:

1. REFER TO S-402 FOR SHEAR WALL TYPICAL DETAILS.
2. LBx REFERS TO SHEAR WALL LINK BEAM MARK. REFER TO 8/S-451 FOR LINK BEAM SCHEDULE.
3. ALL BARS SHOWN ARE AT EACH FACE, UNLESS NOTED OTHERWISE.
4. SHEAR WALL DOWELS SHALL MATCH SCHEDULED VERTICAL REINFORCEMENT, UNLESS NOTED OTHERWISE.
5. ALL LAP SPLICES SHALL BE CLASS B. REFER TO 8 AND 10/S-004 FOR SHEAR WALL LAP SPLICE LENGTHS FOR VERTICAL AND HORIZONTAL BARS.
6. PROVIDE CROSS TIES WHERE VERTICAL REINFORCEMENT EXCEEDS 0.01 TIMES THE GROSS CONCRETE AREA OF THE WALL, TYPICAL. REFER TO 1, 2, AND 3/S-402 FOR TYPICAL DETAILS WITH CROSS TIES.
7. DO NOT PROVIDE VERTICAL CONSTRUCTION JOINTS IN SHEAR WALLS WITHOUT EOR APPROVAL.
8. PROVIDE TAPERED THREADED MECHANICAL COUPLERS AT THE TOP OF ALL SHEAR WALL VERTICAL REINFORCEMENT TO ACCOMMODATE FUTURE EXPANSION. REFER TO 2/S-412.
9. WHERE THIS NOTE IS INDICATED, PROVIDE 1/2" VERTICAL ISOLATION JOINT BETWEEN REMOVABLE CURBS AND ADJACENT SHEAR WALLS, TO FACILITATE FUTURE CURB REMOVAL WITHOUT DAMAGING SHEAR WALLS. REFER TO 9/S-403 FOR CURB REINFORCEMENT.



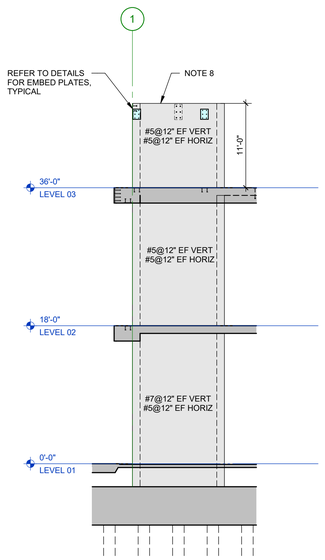
4 SHEAR WALL ELEVATION

1/8" = 1'-0"



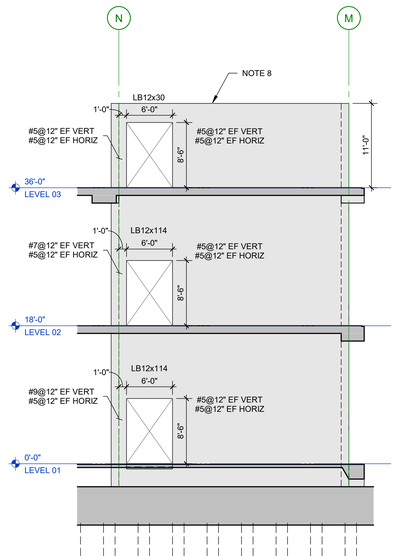
3 SHEAR WALL ELEVATION

1/8" = 1'-0"



2 SHEAR WALL ELEVATION

1/8" = 1'-0"



1 SHEAR WALL ELEVATION

1/8" = 1'-0"

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CORY ANN DAYTON, PE NO. 57441

Sheet Name

SHEAR WALL ELEVATIONS

Sheet Number

S-601

Project Status

CONSTRUCTION DOCUMENTS

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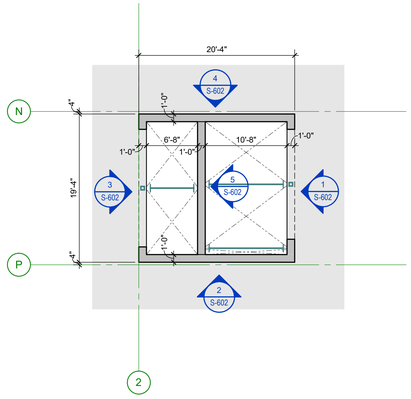

SHEAR WALL ELEVATION NOTES:

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2. LBx REFERS TO SHEAR WALL LINK BEAM MARK. REFER TO 8/S-451 FOR LINK BEAM SCHEDULE.
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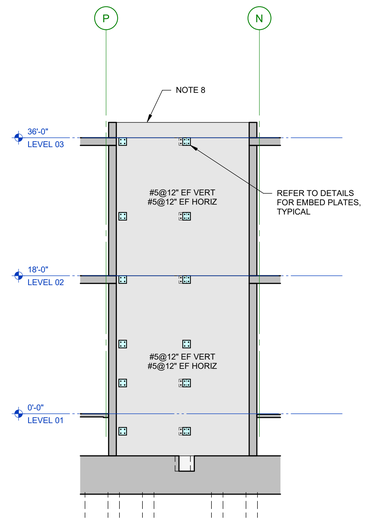
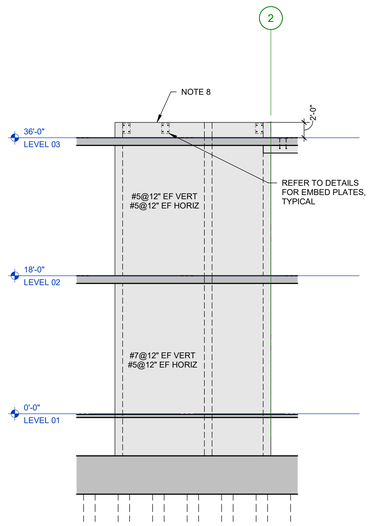
SHEAR WALL #2 KEY PLAN AND NOTES

1/8" = 1'-0"



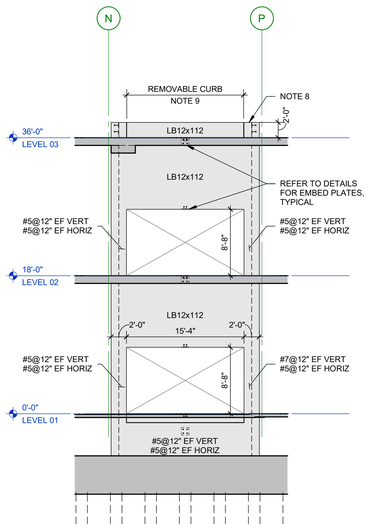
5 SHEAR WALL ELEVATION

1/8" = 1'-0"



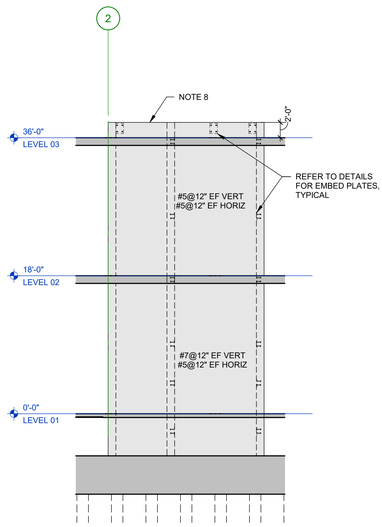
4 SHEAR WALL ELEVATION

1/8" = 1'-0"



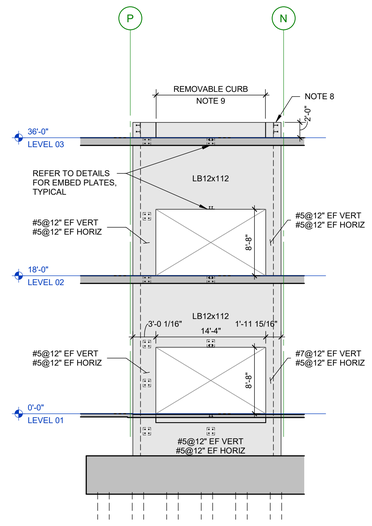
3 SHEAR WALL ELEVATION

1/8" = 1'-0"



2 SHEAR WALL ELEVATION

1/8" = 1'-0"



1 SHEAR WALL ELEVATION

1/8" = 1'-0"

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WPM Project No. S05-25024-00

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MELBOURNE, FL 32940

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SHEET REVIEWER

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AHCA NUMBER	23/23960034-19987
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ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

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Sheet Name

SHEAR WALL ELEVATIONS

Sheet Number

S-602

Project Status

CONSTRUCTION DOCUMENTS

0 1/2" 1" 2"
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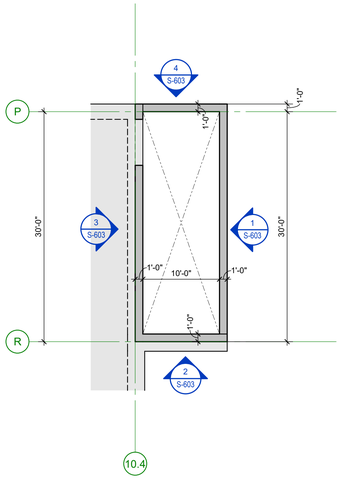

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2. LBx REFERS TO SHEAR WALL LINK BEAM MARK. REFER TO 8/S-451 FOR LINK BEAM SCHEDULE.
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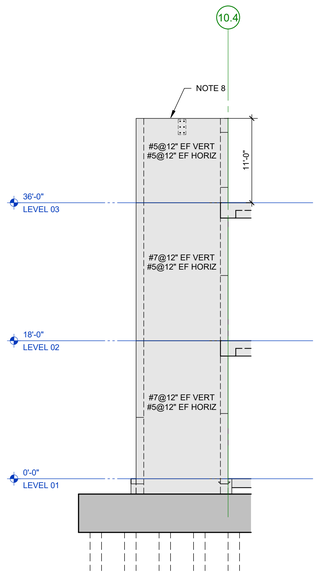
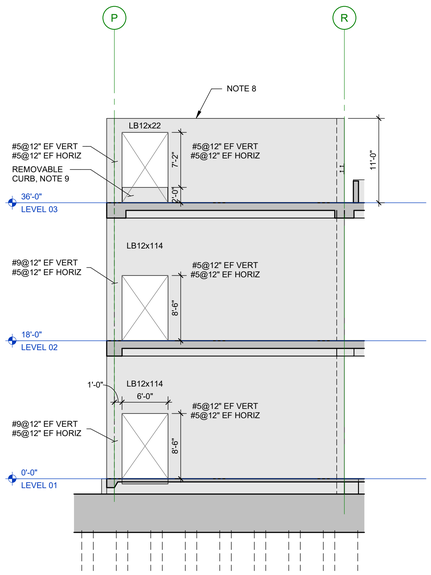
SHEAR WALL #3 KEY PLAN AND NOTES

1/8" = 1'-0"



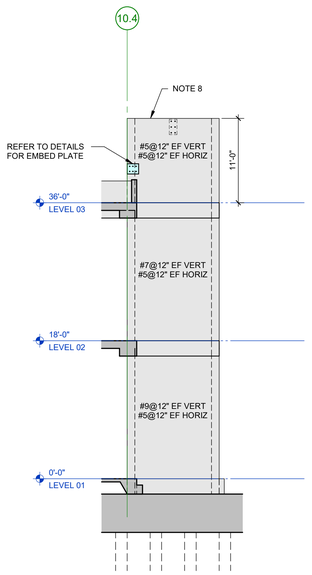
4 SHEAR WALL ELEVATION

1/8" = 1'-0"



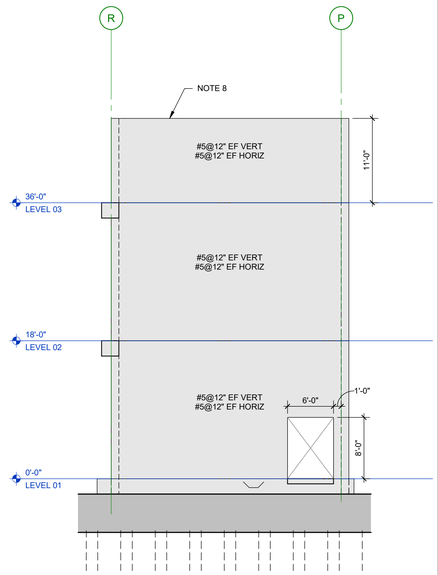
3 SHEAR WALL ELEVATION

1/8" = 1'-0"



2 SHEAR WALL ELEVATION

1/8" = 1'-0"



1 SHEAR WALL ELEVATION

1/8" = 1'-0"

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Sheet Name

SHEAR WALL ELEVATIONS

Sheet Number

S-603

Project Status

CONSTRUCTION DOCUMENTS

0 1/2" 1" 2"

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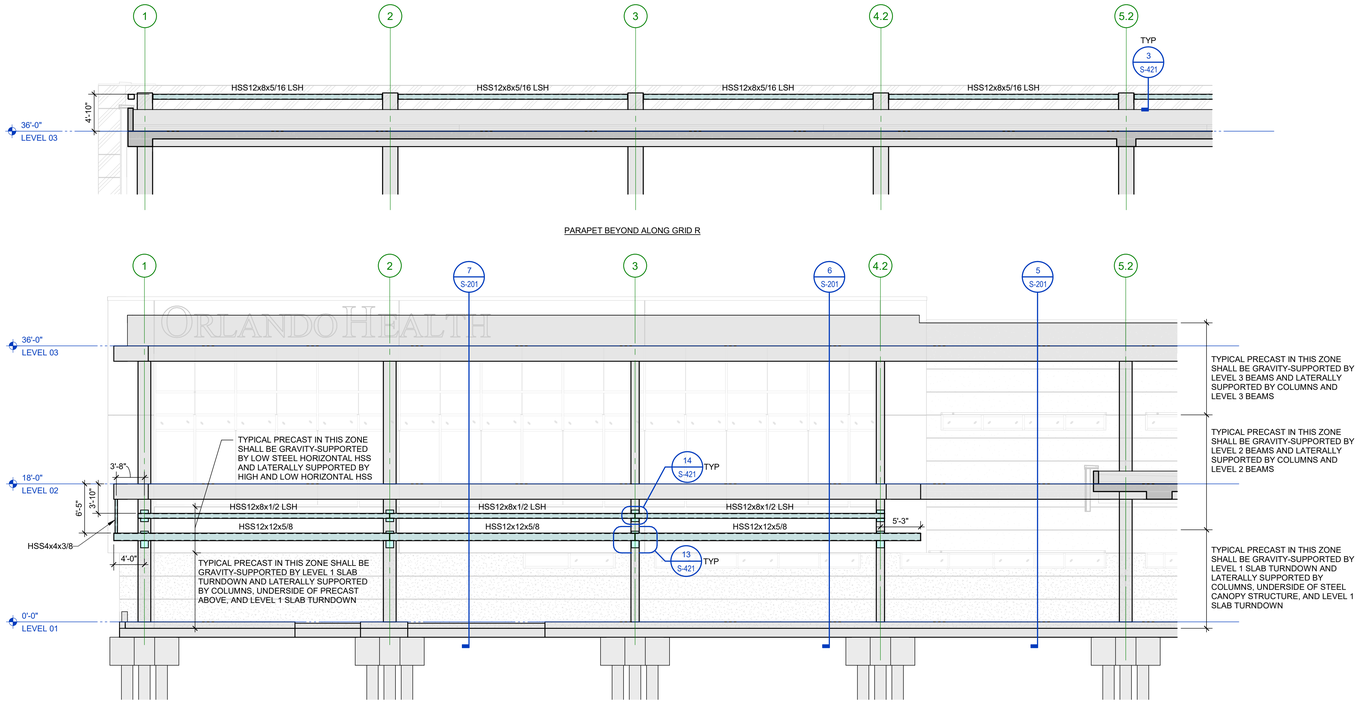
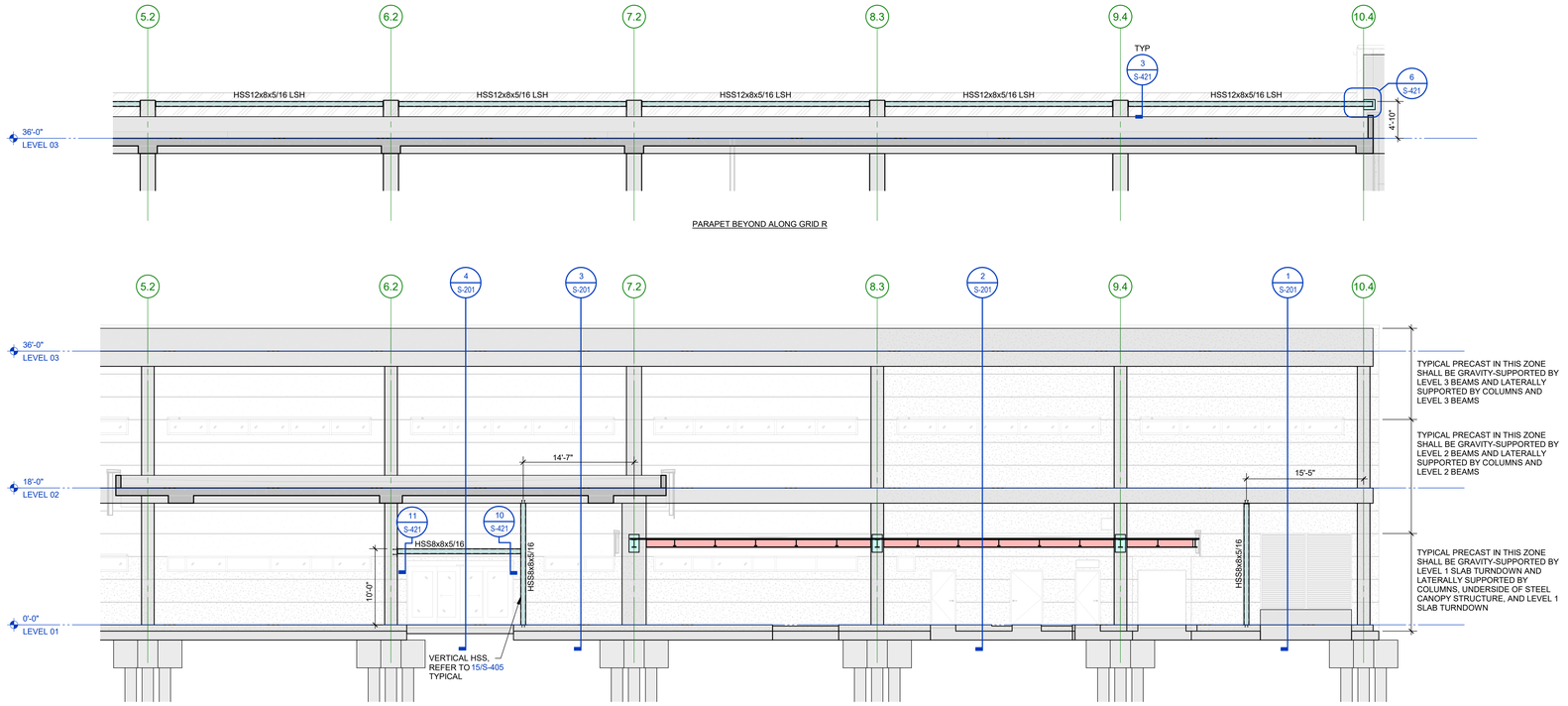


2 CLADDING ELEVATION - SOUTH

1/8" = 1'-0"



1 CLADDING ELEVATION - SOUTH

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MARK	DATE	DESCRIPTION
	3/6/2026	AHCA STAGE III

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

CORY ANN DAYTON LICENSE No. 57441 STATE OF FLORIDA PROFESSIONAL ENGINEER

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Email: cdayton@walterpmoore.com

CORY ANN DAYTON, PE NO. 57441

Sheet Name

CLADDING ELEVATIONS

Sheet Number

S-701

Project Status

CONSTRUCTION DOCUMENTS

Autodesk Docs://10427132_Orlando_Health_Sunrise/305-25024-01 - OH Viera FSED - Structural - R2025.rvt
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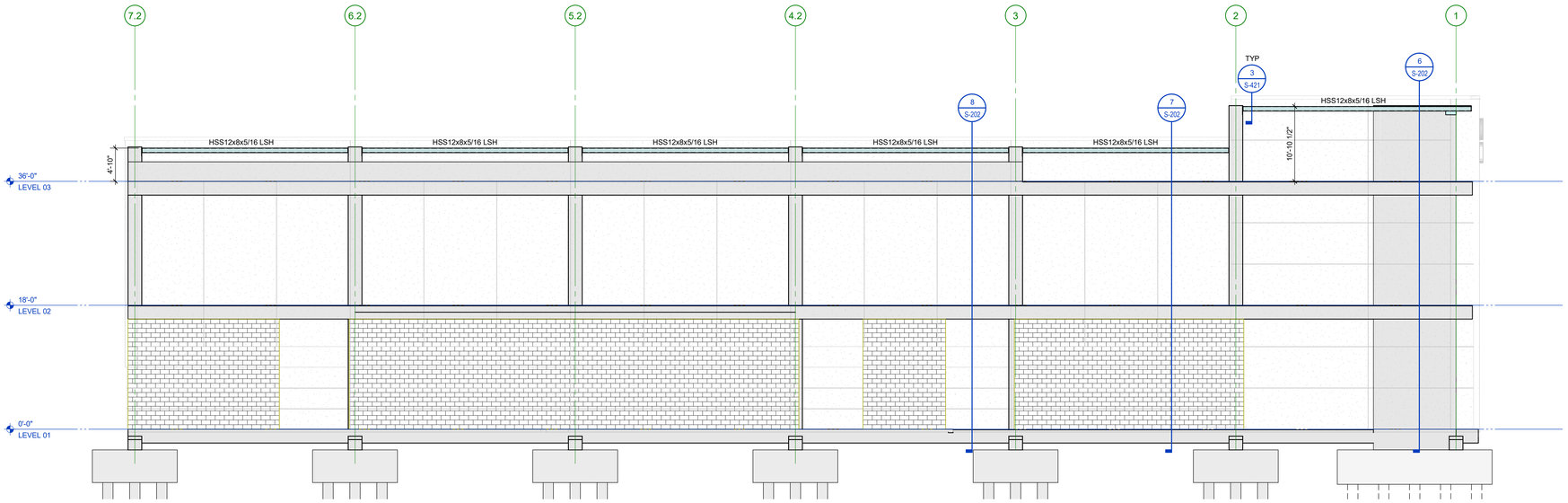
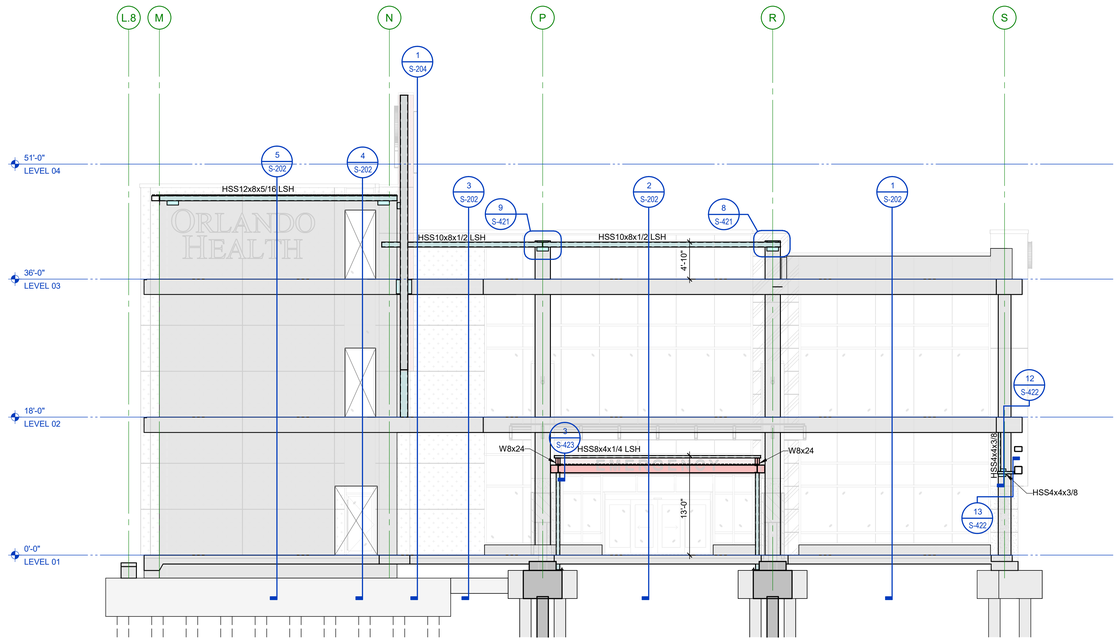


2 CLADDING ELEVATION - NORTH

1/8" = 1'-0"



1 CLADDING ELEVATION - WEST

1/8" = 1'-0"

HDR ARCHITECTURE, INC.
4830 W KENNEDY BLVD #400
TAMPA, FL 33609
FL FIRM REG CERTIFICATION
AA-C000457

Walter P Moore and Associates, Inc.
201 East Kennedy Blvd, Suite 700
Tampa, Florida 33602
WPM Project No. S05-25024-00

813.221.2424
Florida DBPR License No. 3818

FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
TO THE BEST OF THE ARCHITECTS OR ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH THIS SECTION AND CHAPTER 633, FLORIDA STATUES.

ORLANDO HEALTH MELBOURNE NEW FREESTANDING EMERGENCY CARE FACILITY

LAKE ANDREW DR & WILLET PLACE
MELBOURNE, FL 32940

ORLANDO HEALTH®

PROJECT MANAGER	HDR ARCHITECTURE, INC.
PROJECT DESIGNER	HDR ARCHITECTURE, INC.
PROJECT ARCHITECT	HDR ARCHITECTURE, INC.
LANDSCAPE ARCHITECT	MCKINTOSH ASSOCIATES, an LJA COMPANY
CIVIL ENGINEER	BSE CONSULTANTS, an LJA COMPANY
STRUCTURAL ENGINEER	WALTER P. MOORE & ASSOCIATES, INC.
MECHANICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
ELECTRICAL ENGINEER	I.C. TOMASSON ASSOCIATES, INC
PLUMBING ENGINEER	I.C. TOMASSON ASSOCIATES, INC
INTERIOR DESIGNER	HDR ARCHITECTURE, INC.
EQUIPMENT PLANNER	IMEG CORP.
WAYFINDING	HDR ARCHITECTURE, INC.

SHEET REVIEWER	

MARK	**DATE**	**DESCRIPTION**
	3/6/2026	**AHCA STAGE III**

AHCA NUMBER	23/23960034-19987
PROJECT NUMBER	10451386
ORIGINAL ISSUE	3/6/2026 AHCA STAGE III

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CORY ANN DAYTON, PE NO. 57441

Sheet Name

CLADDING ELEVATIONS

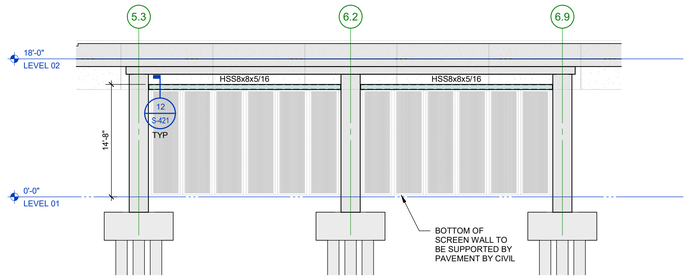
Sheet Number

S-702

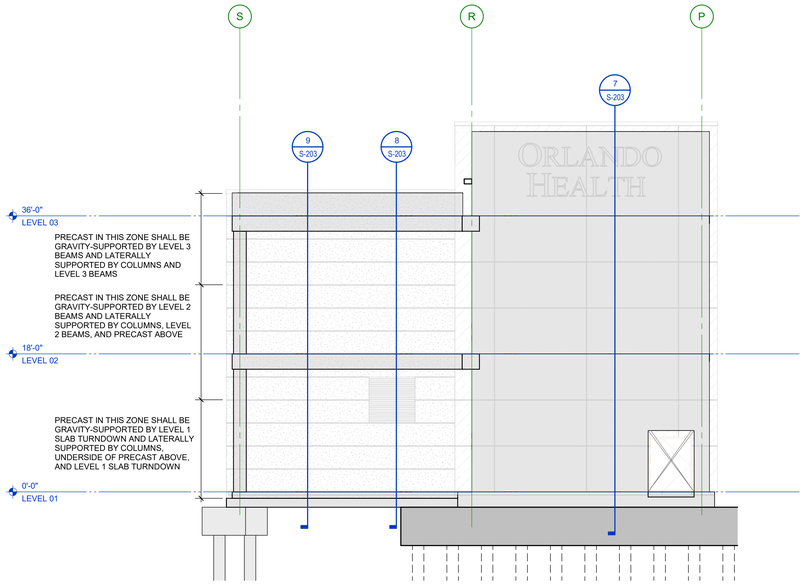
Project Status

CONSTRUCTION DOCUMENTS

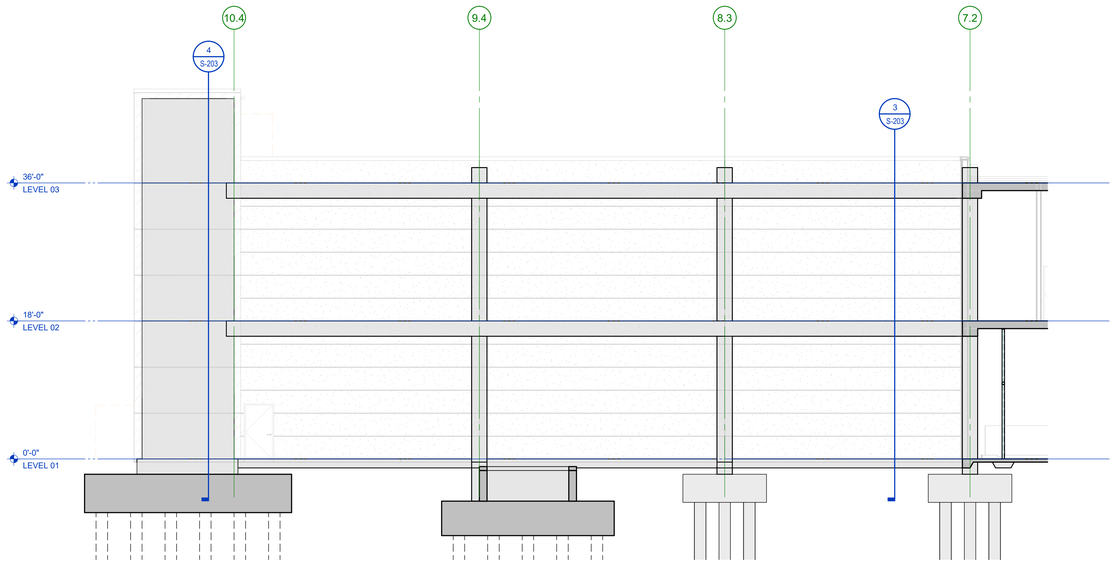
3/4/2026 2:23:08 PM Autodesk Docs://10427132_Orlando_Health_Sunrise/S05-25024-01 - OH Vera FSED - Structural - R2025.rvt



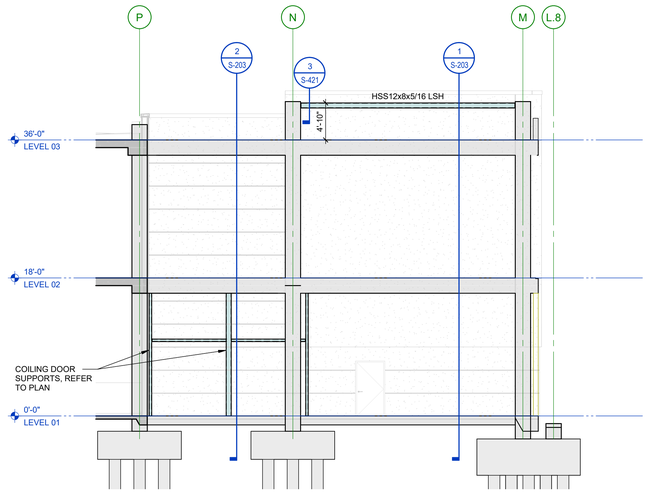
4 CLADDING ELEVATION - SOUTH AMBULANCE CANOPY
1/8" = 1'-0"



3 CLADDING ELEVATION - EAST
1/8" = 1'-0"



2 CLADDING ELEVATION - NORTH
1/8" = 1'-0"



1 CLADDING ELEVATION - EAST
1/8" = 1'-0"

HDR ARCHITECTURE, INC.
4830 W KENNEDY BLVD #400
TAMPA, FL 33609
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WPM Project No. S05-25024-00

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FBC 110.8.4.4 THRESHOLD BUILDING STATEMENT:
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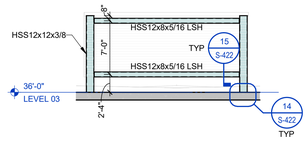
CORY ANN DAYTON, PE NO. 57441

Sheet Name
CLADDING ELEVATIONS

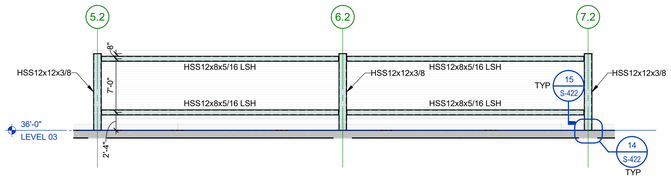
Sheet Number
S-703

Project Status
CONSTRUCTION DOCUMENTS

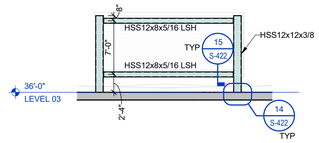
3/4/2026 2:23:14 PM Autodesk Docs://10427132_Orlando_Health_Sunrise/305-25024-01 - OH Viera FSED - Structural - R2025.rvt



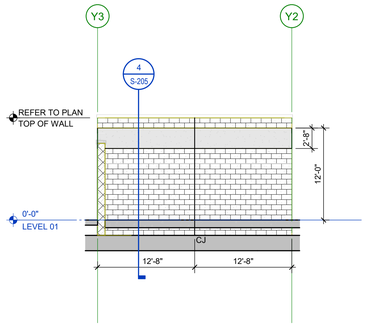
11 CLADDING ELEVATION
SCREEN WALL - WEST
1/8" = 1'-0"



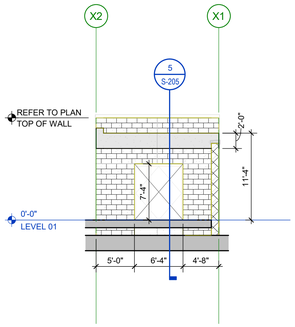
10 CLADDING ELEVATION - SCREEN WALL - SOUTH
1/8" = 1'-0"



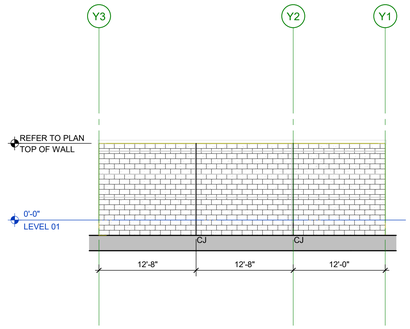
9 CLADDING ELEVATION
SCREEN WALL - EAST
1/8" = 1'-0"



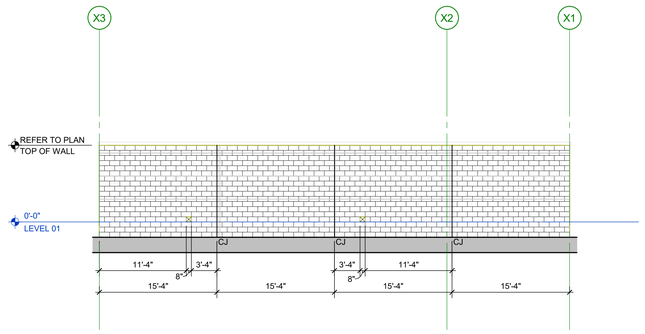
6 CLADDING ELEVATION
CHILLER YARD - EAST INTERIOR
1/8" = 1'-0"



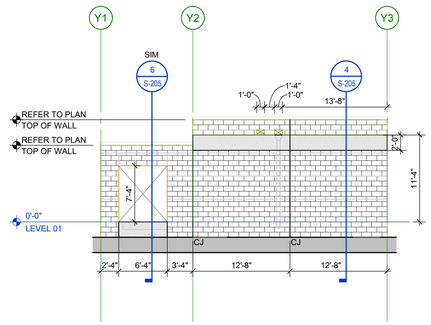
5 CLADDING ELEVATION
CHILLER YARD - NORTH INTERIOR
1/8" = 1'-0"



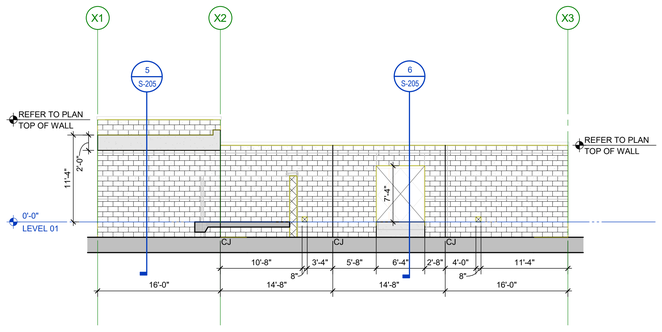
4 CLADDING ELEVATION
CHILLER YARD - EAST
1/8" = 1'-0"



3 CLADDING ELEVATION - CHILLER YARD - NORTH
1/8" = 1'-0"



2 CLADDING ELEVATION
CHILLER YARD - WEST
1/8" = 1'-0"



1 CLADDING ELEVATION - CHILLER YARD - SOUTH
1/8" = 1'-0"

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Sheet Name

CLADDING ELEVATIONS

Sheet Number

S-704

Project Status

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