

STRUCTURAL NOTES

GOVERNING CODES:

- This design is based on the following codes:
A. Florida Building Code, Seventh Edition (2020).
B. Building Code Requirements for Structural Concrete ACI 318 (2014 EDITION).
C. Specification for the Design, Fabrication, and Erection of Structural Steel for Building, ASD Design method.
D. Structural Welding Code D1.1-D1.1M-2010.

DESIGN LOADS:

- A. Wind load based on using the ASCE 7-16 "Minimum Design Loads and Associated Criteria for Buildings and Other Structures" methods of calculation for wind pressures and the following factors:
1. Mean Roof Height of 39'-0"
2. Wind speed of 157 (Vult), 122 MPH (ASD Level) nominal.
3. Exposure Category 'C'.
4. Risk Category II.
5. Internal Pressure Coefficient of +/- 0.18 ("Enclosed" building).
This building is IN NOT A WIND BORNE DEBRIS REGION as defined by Florida Building Code.
B. Roof live load of 20 PSF with allowable load reductions based on area as outlined in the Florida Building Code.
C. Roof dead load of 20 PSF.
D. Handrails and guard loads: 50 PLF linear load and 200 LB concentrated load anywhere along rail.

SHOP DRAWINGS:

- A. When shop drawings or submittals are required (1) electronic copy (combined PDF format) shall be submitted along with a transmittal. GC shall review submittals for subcontractor coordination items prior to our review. Contractor shall allow for (10) ten business days for shop drawing and submittal review. The following shop drawings and/or submittals are required:
1. Concrete mix design submittals of all concrete used in structural applications clearly identifying their intended use (i.e. "slab-on-grade").
2. Field density testing reports.
3. Concrete mix design test reports.
4. Concrete reinforcing layout shop drawings.
5. Structural steel shop drawings.
6. Tilt panel embed and erecting shop drawings.
7. Roofing submittal only to be reviewed for structural purposes. Assemblies must comply with Florida Building Code product approval and meet project design wind pressures. If assembly does not meet the design wind pressures than zone enhancement calculations must be provided, or a different assembly must be provided that complies with corner and edge zone wind pressures.
8. Awning/canopy submittals must include design loads, material grade, and stamp from a registered Florida professional Engineer.
9. Overhead door submittals must include Florida Building Code Product Approval number.

DRAWINGS AND SPECIFICATIONS:

- A. Do not scale these drawings for dimensions not given. Advise Project Architect of any conflicts between these drawings and the Architectural drawings. Verify all field conditions and confirm column locations in respect to building wall alignment prior to the start of work.
B. These drawings are to be used in combination with the architectural, mechanical, plumbing, electrical, and civil drawings. Refer to these other drawings for details that relate to structural components.
C. These construction documents have been prepared from the most complete information available to the engineer. All data on existing construction conditions are approximate & shall be verified prior to commencing work.
D. The contractor shall comply with the manufacturer's instructions & recommendations to the extent-printed information are more detailed or stringent than the requirements contained in the plans.
E. The plans show the location of all fixtures & equipment & are intended to convey the general intent of the work in scope & layout. They are not intended to show in minute detail all of the accessories intended for the purpose of execution of the work, but it is understood that such details are part of this work.
F. The Contractor shall perform no portion of the work at any time without Contract Documents or, where required, approved shop drawings, product data or supplemental details for such portion of the work.
G. The Contractor is responsible for means and methods of construction to ensure the safety of the building until the structural system is completed. The structural system is unstable until all connections have been made and all concrete has reached the minimum design strength as specified in these drawings.

CONCRETE:

- A. Concrete strength requirement:
1. 3000 psi: Foundations
2. 4000 psi: Tilt wall panels, slab on grade.
B. Maximum Slump: 4" (+/- 1") for all concrete, except use 8"-11" for filling cells in block. Fly ash, if used, shall not exceed 20% by weight of total cementitious content. Slump limits shall be strictly adhered to.
C. Ready mix design:
1. Type I or III portland cement per ASTM C150 (fly ash, if used, shall not exceed 20% by weight of total cementitious content). The ready-mix shall use only one brand of cement throughout project.
2. Water-cement ratio (by weight) of 0.52 max for 4,000 psi mixes and 0.57 for 3,000 psi mixes. Water shall be potable and clean conforming to ASTM C 94.
3. Graded aggregate with a maximum aggregate size of 1 1/2". Course aggregate shall be a blend conforming to AASHTO #4, #57 and #89 grading requirements (1 1/2" to 3/8" nominal size). Aggregates shall be natural, non-reactive, and uniformly washed before use and shall conform to ASTM C33.
D. The concrete shall contain the maximum size aggregate permitted by ACI Code up to 3/4" maximum. The guidelines for maximum aggregate size are not greater than 1/5 the narrowest opening in the forms, and/or 1/3 the depth of the slab. Course aggregate shall meet #467 gradation as defined in ASTM C33, with at least 3% retained on the 1/2" sieve and at least 8% retained on the 1" sieve.
E. Small rock mixes are not permitted for slab-on-grade and tilt wall panel applications. When used in ACI blended mix, small rock is permitted but shall not exceed 20% by weight of total course aggregate content.
F. Concrete formwork: Concrete formwork shall be in a clean condition for use as finished surface forms. Forms shall not be removed until the concrete is of sufficient strength to support its own weight and proposed construction loads.
G. Minimum Concrete Cover:
1. Concrete permanently exposed to earth = 3 inches.
2. Concrete exposed to earth or weather = 1-1/2 inches
3. Concrete not exposed to earth or weather = 3/4 inches
H. Concrete must be batched, mixed, and transported in accordance with The Specifications for Ready-Mixed Concrete ASTM C94. Concrete shall be placed with-in 90 minutes of batch time.
I. Expansion and Contraction Joints: Floor (contraction) joints shall be saw cut or prefabricated at a maximum spacing of 2 to 3 times the slab thickness in feet (i.e. 8'-12" o/c for a 4" thick slab). These contraction joints shall be approximately 1/8" wide and extend to a minimum depth of 1/4 the slab thickness. A 1/4" perimeter expansion (isolation) joint shall be used at floors adjoining walls and around all columns. These isolation joints shall be formed by installing an asphalt impregnated fiber sheet that extends the entire thickness of the slab.
J. The concrete supplier shall provide mix designs of all concrete used in structural applications clearly identifying their intended use (i.e. "slab-on-grade").
K. When pouring against earth, the embankment shall be wetted first and all pours on direct sun days will be misted at least twice following initial set. Isolation joints shall be provided at all columns and expansion felt at the perimeter of the slab.
L. Finish on the surface of floor shall be machined troweled followed by hand finishing as required.
M. Exterior walks and parking areas to be light brushed finished.
N. Curing of concrete shall be in accordance with ACI 318.
O. Provide vapor barrier at slab on grade areas. Provide seam tape and pipe boots at all penetrations.
P. Air-entrained concrete shall not be used at interior floor slab.
Q. Floor flatness:
1. Specified overall values: FF45/FL35
2. Minimum local values FF45/FL35

CONCRETE REINFORCEMENT:

- A. Reinforcing steel bars shall be Grade 60 (60 KSI ASTM A615) steel and tied with drawn steel wire.
B. Welded Wire Fabric shall conform to ASTM A185.
C. Placement of reinforcing steel shall be as shown on plans and per the "Manual of Standard Practice for Detailing Reinforced Concrete Structures", ACI 315 manual on placing steel reinforcing.
D. Provide minimum lap splice of 48 bar diameters, but not less than 18 inches, for all reinforcing bars, unless noted otherwise. Stagger splices in adjacent bars at least 24 inches, except in strip footings, masonry tie beams (top of wall) and as otherwise noted.
E. Provide a minimum length of 16 bar diameters of standard hook wall reinforcement into footing or slab, unless noted otherwise. Length of standard hook shall be 12 bar diameters.
F. In wall footings, grade beams and bond beams, provide bent bars at corners and intersections of the same number and size as the straight bars.
G. Use #5 transverse bars @ 48" oc max at any continuous footing w/ (3) or more bars.

TERMITE TREATMENT OF SOIL & STRUCTURE:

- A. All buildings shall have pre-construction treatment protection against subterranean termites. A certification of compliance must be issued to the building department by a licensed pest control company before a certificate of occupancy will be issued. The certificate of compliance shall state: "The building has received a complete treatment for the prevention of subterranean termites. The treatment is in accordance with the rules and laws of Florida Department of Agriculture and Consumer Services." All of this work shall be in conformance with Florida Building Code Section 1816.
B. A permanent sign that identifies the termite treatment provider and the need for re-inspection and treatment contract renewal shall be provided. The sign shall be posted near the water heater or electrical panel. The sign and the posting method shall be in accordance with Florida Building Code Section 105.11.
C. All condensate and roof downspouts shall discharge at least 1'-0" away from the structure sidewall, whether by underground piping, tail extensions, or splash blocks. Gutter with downspouts are required on all buildings with eaves of less than 6" horizontal projection (except gable end rakes or a roof above another roof). Irrigation/sprinkler systems and risers for spray heads shall not be installed within one foot of the building sidewall. This placement of these water sources close to the building sidewall shall be in accordance with Florida Building Code Section 1503.6.
D. The exterior wall covering shall terminate not closer than 6" from the final earth grade to allow for inspection for termite infestation. The exception to this is paint or decorative cementitious finishes less than 5/8" thick adhered directly to the masonry foundation sidewall. The termination of exterior wall coverings shall be in accordance with Florida Building Code Section 1403.8.
E. If soil treatment is used for subterranean termite protection, the initial treatment shall be performed after ALL excavation, backfilling and compaction is complete. Any soil area that is disturbed after initial soil treatment shall be retreated with chemical treatment including spaces boxed or formed. This is required under Florida Building Code Section 1816.1.1.
F. If soil treatment is used for subterranean termite protection, space in concrete floors boxed out or formed for subsequent installation of plumbing traps, drains, or any other purpose shall be created by using plastic or metal permanently placed forms of sufficient depth to eliminate any planned soil disturbance after initial chemical soil treatment. The placement and construction of the permanent forms shall be in accordance with Florida Building Code Section 1816.1.3.
G. If soil treatment is used for subterranean termite protection, the chemical treated soil shall be protected against rainfall dilution by a 6 mil (min.) vapor barrier. If rainfall occurs prior to placement of the vapor barrier, chemical re-treatment is required.
H. If soil treatment is used for subterranean termite protection, concrete overpour and mortar accumulation along the foundation perimeter shall be removed prior to exterior soil treatment.
I. If soil treatment is used for subterranean termite protection, chemical soil treatments shall also be applied under all exterior concrete or grade within one foot of primary structure sidewalls. Also, a vertical chemical treatment barrier shall be applied promptly after construction is completed, including initial landscaping and irrigation / sprinkler installation. Any soil disturbed after the chemical vertical barrier is applied shall be promptly retreated. This exterior chemical treatment shall be in accordance with Florida Building Code Section 1816.1.6.
J. After all work is completed, loose wood and debris shall be completely removed from under the building and within one foot of the structure. All wood forms and supports shall be completely removed. This includes, but is not limited to: wooden grade stakes, forms, contraction spacers, tub trap boxes, plumbing supports, bracing, shoring, forms, or other cellulose-containing material place in any location where such materials are not clearly visible and readily removal prior to completion of the work.

FOUNDATIONS:

- A. Maximum soil bearing pressure used for design 3,000 PSF.
B. Prepare subgrade in accordance with Geotechnical Report No. HD215037 dated December 13, 2021, prepared by Terracon Consultants, Inc.
C. Notify Engineer if footing excavation reveals unsuitable or unstable soils, or materials or conditions not anticipated in the original Report.
D. Concrete placement shall occur immediately after footing excavation & placement of reinforcing steel. Any freestanding wall shall be pumped out of footing excavation prior to concrete placement.
E. A qualified testing laboratory shall be retained to perform the following minimum in-place density tests: This suggested testing shall be used in absence of a testing program suggested by a soil engineer as contained in a soil report performed for this project. Compaction beneath all floor slab should be verified for a depth of 12 inches and meet the 95 percent criteria.
1. One density test for each 2,500 square feet of compacted subgrade.
2. One density test at each column.
3. One density test per 50 feet of wall footing.

TILT WALL PANELS:

- A. Shop drawings are required for work under this section. Shop drawings shall include the following:
1. Reinforcing layout
2. Embed layout
3. Additional reinforcement required for panel erection
4. Erection hardware placement and lift engineering
5. Slab layout showing location of panel casting.
B. The concrete supplier shall provide mix designs of all concrete used in the tilt wall panels.
C. Chairs as required shall be all plastic. Steel chairs with plastic tips are not acceptable.
D. No allowance was provided for the stresses created in the panels during erection. Contractor shall have panels investigated by a Professional Engineer licensed in the State of Florida to design panel lifting system. The results of this investigation shall be provided to this Engineer prior to the start of casting of the tilt wall panels.
E. Rustication forming material shall be new and in unused condition unless form work is designed for reuse.
F. Stack casting of tilt wall panels shall not be permitted without prior written approval of the engineer. This approval shall limited to specific panels.
G. All burrs, honeycomb and pockets shall be removed from panels after erection of same. This work shall begin immediately after the erection of the panels is complete.
H. Panel Finishing: Unless otherwise indicated on plans, all panels shall receive a steel troweled finish on the interior surface. The exterior surface of the panels shall be cast against the floor slab, which shall have a steel troweled finish.
I. Holes created by erection hardware and lifting devices shall be patched smooth. Prefabricated insert plugs are not acceptable except at interior.

STEEL BAR JOISTS:

- A. The design, fabrication and erection of steel joists shall conform to the "American Institute of Steel Construction Standard Specification for Open Web Steel Joists" (latest edition).
B. Joists supplied under this section shall be fabricated by a recognized manufacturer, using cord or web sections fabricated from steel having a yield strength of at least 36 KSI but not exceeding 50 KSI.
C. Bridging per the applicable Steel Joist Institute specification (latest edition) shall be used and shall be installed before construction loads are applied to the joists. The ends of all bridging lines terminating at walls or beams shall not be anchored until all the roof dead loads are applied.
D. Ends of joists resting on steel supports shall be connected with the equivalent of two 1/8-inch fillet welds 1" long or with two 1/2" diameter bolts. Field welding shall not damage the joists.
E. All joists on column centerlines shall be secured by 1/2" inch diameter bolts at the top chord bearing. The bottom chord shall be extended to the column.
F. Joists shall bear 4" minimum on masonry and 2 1/2" minimum on steel unless noted otherwise. Joists bearing on masonry shall bear on an embedded steel plate.
G. Steel joists shall be primed painted with one coat of light gray primer meeting the minimum requirements of Federal Specification TT-P-636 or Steel Structures Painting Council Specification 15-68T, Type I.
H. Joist girders shall be proportioned such that they can be erected without bridging. The strutted ends of the bottom cord shall be restrained from lateral movement to brace the girder from overturning.
I. Bar joists and truss girders shall have manufacturer's standard beveled ends or sloped shoes if joist slope exceeds 1/4 inches per 12 inches.
J. Steel joists shall be designed to resist net uplift as shown on wind diagram shown in contract drawings. Maximum joist bearing top chord rollover forces at joist bearing walls to be less than 4,100 lb (ULT) or 2,450 lb (ASD).
K. Steel bar joist and joist girders submittals:
1. Detailed shop drawings including layouts of steel bar joists, joist girders, bridging, connections, and accessories.
2. Signed and sealed calculations for special joists (SP-joist) carried out by a delegate registered professional engineer in the state of Florida shall be submitted for review of general conformance with the design criteria specified by the engineer of record.

STRUCTURAL STEEL:

- A. Fabrication and erection of structural steel shall be in accordance with AISC "Specification for the Design, Fabrication and Erection of Structural Steel for Buildings" (latest edition).
B. Structural steel shapes (used as beams and columns) shall conform to ASTM A572 Grade 50 KSI unless otherwise noted on the contract drawings.
C. Plates, channels, rods, anchor bolts and angles shall conform to ASTM A36 unless otherwise noted of the contract drawings.
D. Steel pipe shall conform to ASTM A53 Grade B or ASTM A501.
E. Structural tubing shall conform to ASTM A500 Grade B (46 KSI minimum).
F. All bolts (except anchor bolts) shall be high strength (HSB) shall conform to ASTM A325, 3/4" diameter unless noted otherwise. High strength bolts shall be used unless specifically noted on the drawings.
G. All welding shall be performed by certified welders in accordance with AWS "Code for Arc and Gas Welding in Building Construction" (latest edition). The minimum electrode used shall be E70xx Low Hydrogen electrodes unless otherwise specified.
H. All welded plate connections and field welds to be painted grey primer.
I. Structural steel shall be primed painted with one coat of light gray primer meeting the minimum requirements of Federal Specification TT-P-636 or Steel Structures Painting Council Specification 15-68T, Type I.
J. All beams shall be fabricated and erected natural camber up.
K. Connections may be single shear plate (or double angle) framed beam connections per AISC unless noted otherwise. All bolts shall be 3/4" diameter A-325-N unless noted otherwise. Shop connections may be bolted or welded, with welded connections equal to bolted connections. All bolts shall be tightened snug tight with suitable nuts and hardened washers unless noted otherwise. Design connections for the larger of either the shear value shown on the drawings or 75% of the maximum shear listed in the tables for "Allowable uniform load in Kips for beams laterally supported" at the bottom of each page in the "Properties And Reaction Values" Part 2 of the latest edition of the AISC "Manual Of Steel Construction".
L. Splicing of structural steel where not detailed is not permitted without prior written approval of the structural engineer.
M. Structural openings, supports, anchors, etc. around or affected by mechanical, electrical and plumbing equipment shall be verified with the equipment purchased before proceeding with structural work affected.

METAL ROOF DECK:

- A. Metal roof deck shall be 1-1/2" thick, 22 Ga. Type B (as identified by the Steel Deck Institute) primed white bottom / gray top steel deck conforming to ASTM A1008 with a minimum yield strength of 33 ksi.
B. The deck shall be placed on the supporting framework with a minimum end lap of two inches centered over supports. The deck shall be attached to the supports, and the side lap of adjacent units in the pattern shown on the contract drawings.
C. All roof deck openings 12" diameter or larger are to have support angles per typical deck opening detail, including openings for roof sump pans.
D. Roof deck shall be laid out such that decking shall span three spans without interruption wherever possible.
E. Deck and supporting members damaged by excess welding heat shall be repaired or replaced as determined by Engineer.
F. Arc puddle welds shall be at least 5/8" in effective diameter or an elongated weld having an equal perimeter. Fillet and seam welds when used shall be a minimum of 1 1/2" long. Weld metal shall penetrate all layers of deck material at end laps and side joints and have good fusion to the supporting members.
G. HILTI X-HSN 24 pins may be used instead of arc puddle welds with approved shop drawings and calculations.

POST-INSTALLED ANCHORS:

- The below Products are the design basis for this project. Substitution requests for products other than those listed below may be submitted by the contractor to the Engineer-of-Record (EOR) for review. Substitutions will only be considered for products having a code Report recognizing the product for the appropriate application and project building code. Substitution submittals shall demonstrate that the substituted product is capable of achieving the equivalent performance values of the design basis product.
A. Adhesive Anchors (Epoxy):
1. Into Concrete: Adhesive for rebar and anchors shall have been tested in accordance with ACI 355.4 and ICC-ES AC308 for cracked concrete and seismic applications. Pre-approved products include:
a) HILTI HIT-HY 200 (ICC-ES ESR-3187)
b) SIMPSON STRONG-TIE "SET-XP" (ICC-ES ESR-2508)
2. Anchoring System shall utilize traditional preparation of the anchor hole (Blowing and brushing) per the manufactures requirements.
3. Anchoring adhesive shall be a two-part component 100% solid epoxy based system supplied through a static-mixing nozzle supplied by the manufacturer. This requirement shall be met regardless of which epoxy product or manufacturer that is used on this project.
4. The threaded rods to be used in combination with Epoxy system shall be fabricated from steel meeting or exceeding the properties of ASTM A36.
B. Mechanical Anchors (Expansion/Screw Anchors):
1. Into Concrete: Anchors shall have been tested in accordance with ACI 355.2 and ICC-ES AC193 for cracked and seismic applications. Adhesive anchors shall be installed by a certified adhesive anchor installer where designated on the contract documents. Pre-approved products include:
a) Screw Anchor- HILTI "KWIK HUS-EZ" (ICC-ES ESR 3027)
b) Expansion Anchor- HILTI "KWIK BOLT TZ" (ICC-ES ESR 1917)
c) Screw Anchor- SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR 2713)
d) Expansion Anchor- SIMPSON STRONG-TIE "STRONG-BOLT 2" (ICC-ES ESR-3037)

STRUCTURAL SHEET INDEX

Table with 2 columns: SHEET # and DESCRIPTION. Lists sheets 8001 through 8514 including structural notes, wind zone diagrams, foundation plans, and elevations.

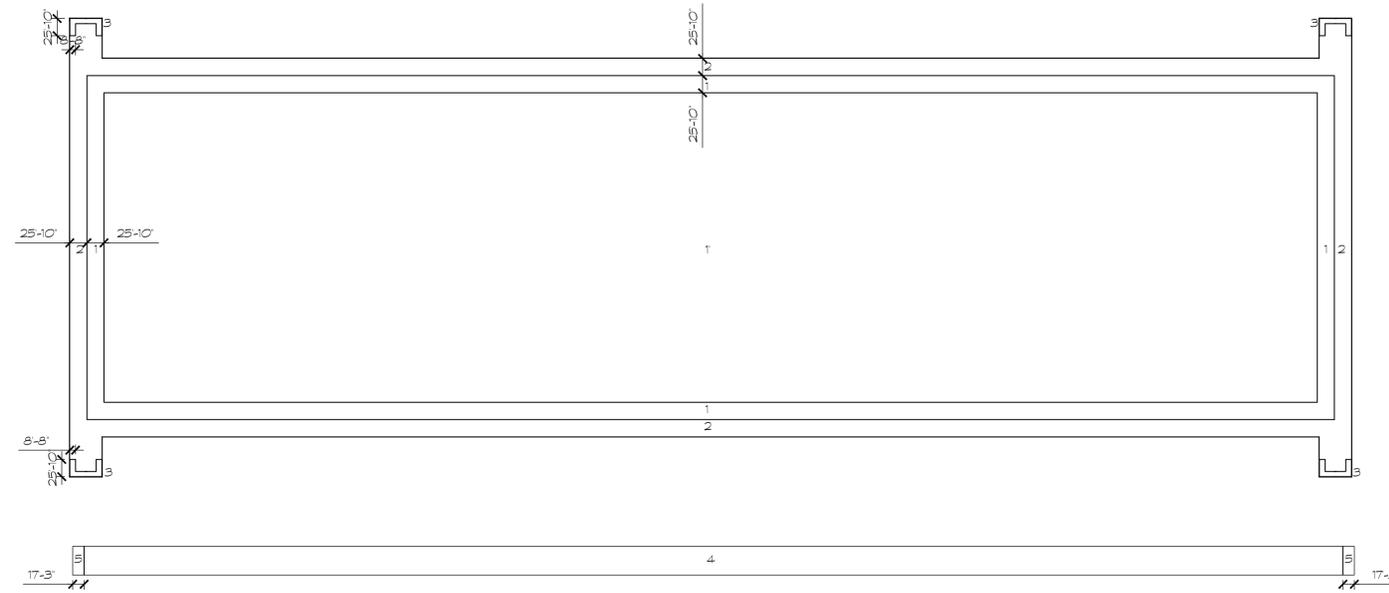
AVANTI CONSULTING ENGINEERS logo and contact information: 1345 N. Falkenberg Rd., Tampa, Florida 33619. P (813) 655-2884 www.avantitampa.com. FLORIDA CERTIFICATE OF AUTHORIZATION No. 24637.

STONEMONT FT PIERCE BLDG. 1 ORANGE AVENUE FORT PIERCE, FLORIDA 34945

Table with 2 columns: PA/PM: DMS, DRAWN BY.: DNR, JOB NO.: MIA21-0040-00. Includes a REMARKS table with columns for DATE and ISSUED FOR PERMIT.

STRUCTURAL NOTES SHEET S100-001

THESE DRAWINGS AND SPECIFICATIONS ARE THE PROPERTY AND COPYRIGHT OF WASE MALCOLM AND SHALL NOT BE USED ON ANY OTHER WORK EXCEPT BY AGREEMENT WITH WASE MALCOLM. WRITTEN DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALED DIMENSIONS AND SHALL BE VERIFIED ON THE JOB SITE. ANY DISCREPANCY SHALL BE BROUGHT TO THE NOTICE OF WASE MALCOLM PRIOR TO THE COMMENCEMENT OF ANY WORK.



POSITIVE WIND PRESSURE ON GLAZING & WALL COMPONENTS

LOCATION ON BUILDING	POS. PRESSURE < 10 SQ. FT.	POS. PRESSURE < 20 SQ. FT.	POS. PRESSURE < 50 SQ. FT.	POS. PRESSURE < 100 SQ. FT.	POS. PRESSURE < 500 SQ. FT.
FIELD AREA [4]	60.4	57.8	55.2	52.7	45.0
CORNER AREA [5]	60.4	57.8	55.2	52.7	45.0

NEGATIVE WIND PRESSURE ON GLAZING & WALL COMPONENTS

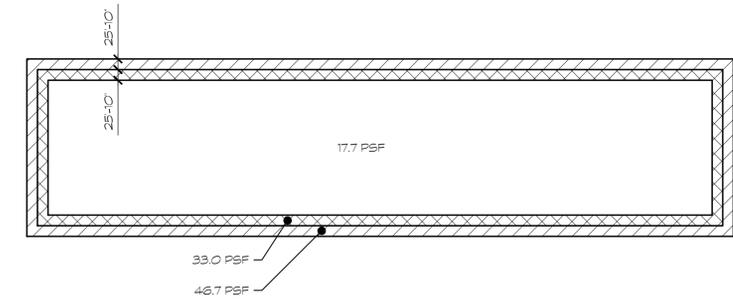
LOCATION ON BUILDING	NEG. PRESSURE < 10 SQ. FT.	NEG. PRESSURE < 20 SQ. FT.	NEG. PRESSURE < 50 SQ. FT.	NEG. PRESSURE < 100 SQ. FT.	NEG. PRESSURE < 500 SQ. FT.
FIELD AREA [4]	65.5	62.9	60.4	57.8	50.1
CORNER AREA [5]	80.8	75.7	68.0	62.9	50.1

ROOF WIND UPLIFT PRESSURES

ZONE	UPLIFT PRESSURE < 10 SQ. FT.	UPLIFT PRESSURE < 20 SQ. FT.	UPLIFT PRESSURE < 50 SQ. FT.	UPLIFT PRESSURE < 100 SQ. FT.	UPLIFT PRESSURE < 500 SQ. FT.
[1]	61.4	61.4	61.4	61.4	41.5
[1]	106.8	98.3	89.8	84.1	67.1
[2]	140.9	132.4	121.1	109.7	89.8
[3]	192.1	172.2	149.5	132.4	89.8

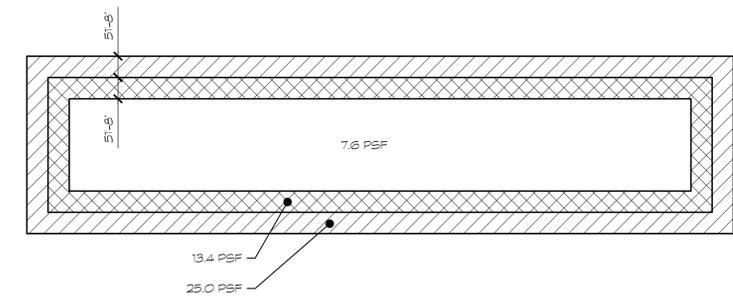
GENERAL NOTES:

- WIND LOADS ARE BASED ON ASCE 7-16 'MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES' USING THE FOLLOWING CRITERIA:
 - DESIGN CRITERIA: WIND SPEED: $V_{ult} = 157$ MPH (ULTIMATE), $V_{nom} = 122$ MPH (NOMINAL).
 - BUILDING RISK CATEGORY II.
 - 'ENCLOSED' BUILDING CLASSIFICATION ($GCFI = +/- 0.18$).
 - BUILDING EXPOSURE CATEGORY C.
 - MEAN ROOF HEIGHT LESS THAN 43'-0".
 - ROOF ANGLE OF LESS THAN 7 DEGREES.
- ALL WIND PRESSURES SHOWN ABOVE ARE IN PSF (LB/SQ. FT).
- NOTE WIND PRESSURES SHOWN ARE 'ULTIMATE' AND CAN BE CONVERTED TO NOMINAL (ASD) BY MULTIPLYING BY 0.6.
- WALL WIND PRESSURES SHOWN HAVE BEEN REDUCED 10% SINCE ROOF ANGLE LESS THAN 10 DEGREES PER ASCE 7-16.
- GROSS PRESSURES ARE FOR WINDOWS, DOORS, VENEER, ROOFING, ROOFING ACCESSORIES AND OTHER BUILDING COMPONENTS AND GLAZING.
- GROSS PRESSURES SHALL BE LINEARLY INTERPOLATED FOR (A) NOT SHOWN IN TABLE.
- POSITIVE PRESSURES INDICATE PRESSURES ACTING TOWARD A PROJECTED SURFACE. NEGATIVE PRESSURES INDICATE PRESSURES ACTING AWAY FROM A PROJECTED SURFACE.
- JOIST/TRUSS NET UPLIFT LOAD COMBINATION IS $0.6D + 0.6W$. 'W' IS AS SHOWN ABOVE AND 'D' = 12 PSF.
- BUILDING IS NOT IN A WIND-BORNE DEBRIS REGION.



JOIST NET UPLIFT DIAGRAM NTS

NOTE: NET UPLIFT LOADS ARE BASED ON ASD LOAD COMBINATION $0.6D + 0.6W$



JOIST GIRDER NET UPLIFT DIAGRAM NTS

NOTE: NET UPLIFT LOADS ARE BASED ON ASD LOAD COMBINATION $0.6D + 0.6W$

ABBREVIATIONS (NOT ALL ARE USED)

AB	ANCHOR BOLT	LL	LIVE LOAD
AFF	ABOVE FINISH FLOOR	LLBB	LONG LEG BACK-TO-BACK
ARCH	ARCHITECT(URAL)	LLH	LONG LEG HORIZONTAL
		LLV	LONG LEG VERTICAL
BLDG	BUILDING	(N)	NEW
BRDG	BRIDGING	OC	ON CENTER
BRG	BEARING	PAF	POWDER-ACTUATED FASTENER(S)
CJ	CONTRACTION JOINT (SLAB SAW CUT)	PL	PLATE
CL	CENTERLINE	PLF	POUNDS PER LINEAR FOOT
CMU	CONCRETE MASONRY UNIT	PNL	PANEL
CONC	CONCRETE	PSF	POUNDS PER SQUARE FOOT
CONN	CONNECTION	PSI	POUNDS PER SQUARE INCH
CONT	CONTINUOUS	REQ	REQUIRED
COORD	COORDINATE	SDI	STEEL DECK INSTITUTE
DIA	DIAMETER	SM	SIMILAR
DL	DEAD LOAD	SJI	STEEL JOIST INSTITUTE
DWG	DRAWING	SOG	SLAB ON GRADE
EA	EACH	SPECS	SPECIFICATIONS
EF	EACH FACE	SQ	SQUARE
ENG	ENGINEER	STD	STANDARD
EL	ELEVATION	STL	STEEL
EQ	EQUAL	TOS	TOP OF STEEL
EW	EACH WAY	TOT	TOP OF BEAM
EXT	EXTERIOR	TOF	TOP OF FOOTING
(E)	EXISTING	TOV	TOP OF WALL
FBC	FLORIDA BUILDING CODE	TET	TOP AND BOTTOM
FF	FINISHED FLOOR	TRANS	TRANSVERSE
FS	FAR SIDE	TYP	TYPICAL
FTG	FOOTING	UNO	UNLESS NOTED OTHERWISE
GA	GAUGE	VIF	VERIFY IN FIELD
GC	GENERAL CONTRACTOR	VERT	VERTICAL
HC	HOLLOW-CORE	WL	WIND LOAD
HORIZ	HORIZONTAL	WWM	WELDED WIRE MESH
HS	HEADED STUD	W	WIND
HSS	HOLLOW STRUCTURAL SECTION		
INT	INTERIOR		

Avanti Project No. 21-177

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FLORIDA CERTIFICATE OF AUTHORIZATION No. 24537

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WIND PRESSURES & WIND ZONE DIAGRAMS

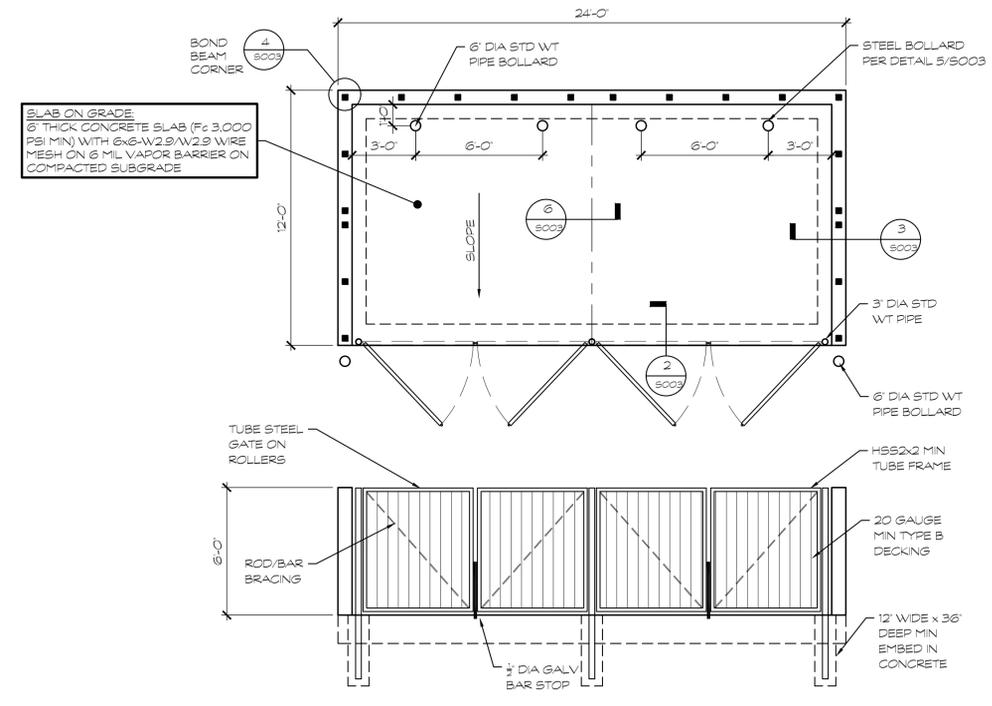
DATE	REMARKS
07-13-2022	ISSUED FOR PERMIT

PA/PM:	DMS
DRAWN BY.:	DNR
JOB NO.:	MIA21-0040-00

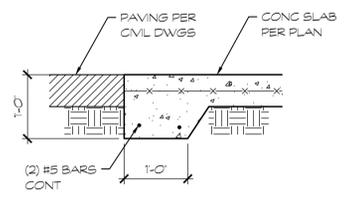
SHEET
S100-002

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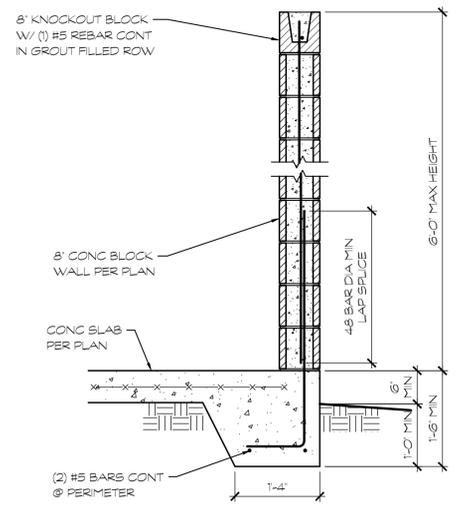
- MASONRY WALL REINFORCING**
1. USE (1) #5 BAR IN FILLED CELL @ 32" OC MAX.
 2. GROUT FILL ALL CELLS SOLID.
 3. SEE DETAIL 4/S003 FOR CORNER REINFORCING IN BOND BEAM.



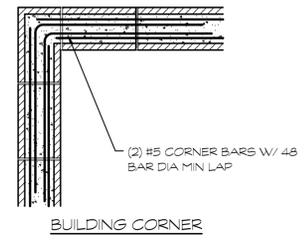
1 DUMPSTER ENCLOSURE
SCALE: 1/4" = 1'-0"



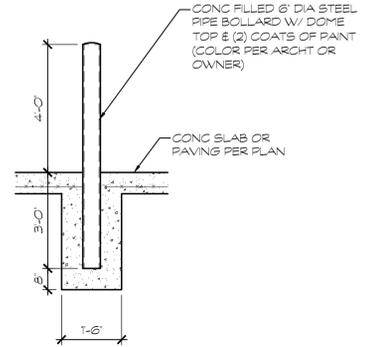
2 SLAB EDGE
SCALE: 3/4" = 1'-0"



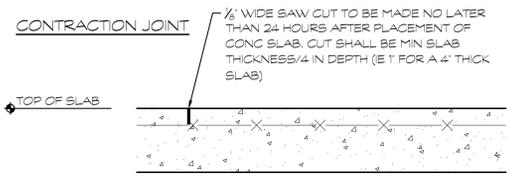
3 DUMPSTER WALL SECTION
SCALE: 3/4" = 1'-0"



4 BOND BEAM CORNER CONTINUITY
SCALE: 3/4" = 1'-0"



5 BOLLARD
SCALE: 3/8" = 1'-0"



6 CONTRACTION JOINT
SCALE: 1/2" = 1'-0"

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BLDG. 1
ORANGE AVENUE
FORT PIERCE, FLORIDA 34945

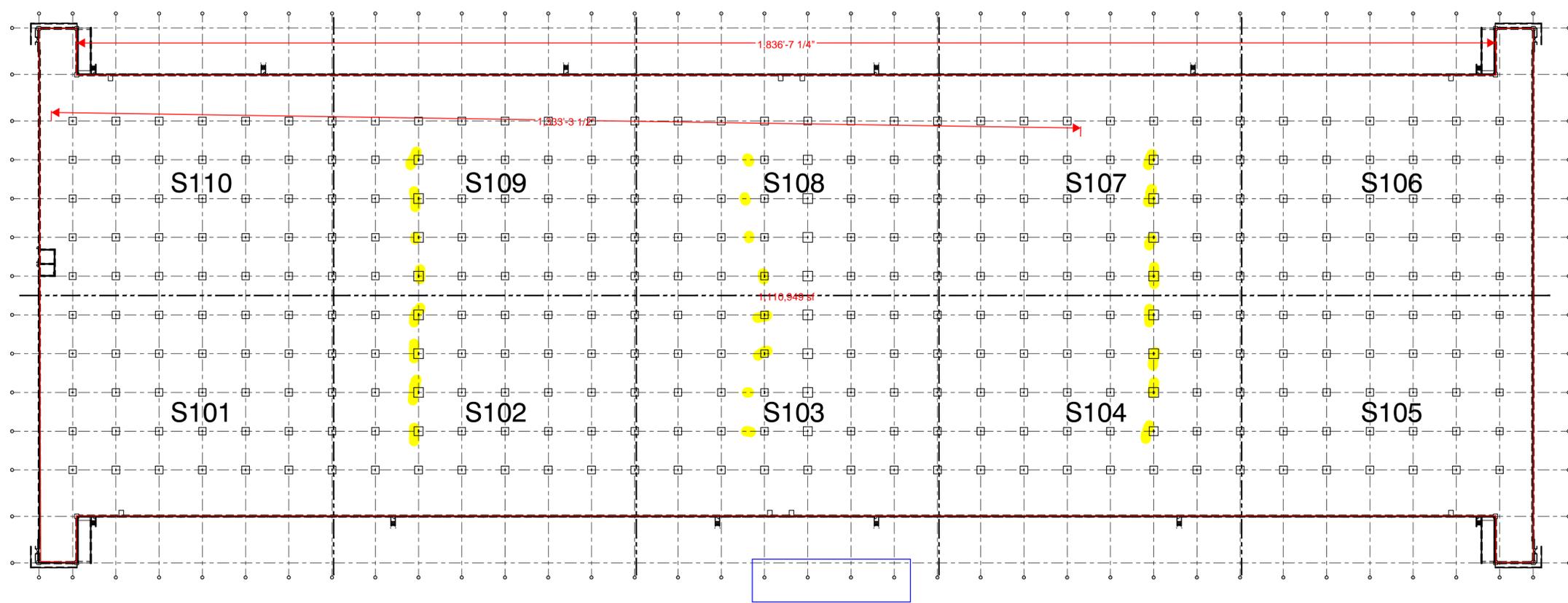
DUMPSTER PLAN & STRUCTURAL DETAILS

DATE	REMARKS
07-13-2022	ISSUED FOR PERMIT

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OVERALL FOUNDATION PLAN
SCALE: 1" = 80'-0"

- REFER TO STRUCTURAL NOTES FOR SOIL COMPACTION REQUIREMENTS. REINFORCING 'CHAIRS' ARE REQUIRED PER FLORIDA BUILDING CODE.
- THE CONCRETE SLAB SHALL BE CAST ON A VAPOR BARRIER ON WELL-COMPACTED, CLEAN, TERMITES-TREATED FILL. SLAB CURING IS REQUIRED.
- THE CONCRETE SLAB ON GRADE HAS NOT BEEN SPECIFICALLY DESIGNED TO SAFELY SUPPORT PANEL BRACE REACTIONS, NOR HAS IT BEEN SPECIFICALLY DESIGNED TO SAFELY SUPPORT ANY CRANE LOADS. IF USED AS SUCH, SUBMIT SIGNED AND SEALED CALCULATIONS VERIFYING SLAB'S CAPACITY TO BE UTILIZED IN SUCH A MANNER.
- CENTERLINES OF WALLS AND COLUMNS SHALL COINCIDE WITH THE FOUNDATION CENTERLINES, UNLESS SPECIFICALLY SHOWN OTHERWISE.
- ELEVATIONS SHOWN ON THESE PLANS REFER TO 0'-0" REFERENCE ELEVATION (TOP OF GROUND FLOOR SLAB).
- SEE DETAIL 7/S301 FOR PLUMBING CHASE AT FOOTING IF REQUIRED.
- SEE DRAWING S001 FOR STRUCTURAL NOTES.
- VERIFY ALL DIMENSIONS, ELEVATIONS, SLAB STEPS AND SLAB FINISHES WITH BUILDING DRAWINGS PRIOR TO COMMENCING CONSTRUCTION. FOR ADDITIONAL DIMENSIONAL INFORMATION NOT GIVEN HERE, REFER TO BUILDING DRAWINGS.
- SLAB ON GRADE CONTRACTION JOINTS (CJ) SHALL BE TOOLED OR SAWCUT WITHIN 6 HOURS OF POURING. SEE DETAIL 1/S301. JOINT PATTERN SHALL BE SHOWN ON PLAN. CONTRACTOR SHALL PROVIDE DIAMOND SHAPED 'LEAVE-OUTS' AT ALL COLUMNS AND AT ALL ROOF DRAIN LOCATIONS (HALF-DIAMONDS).
- SIDE FORMS SHALL BE USED FOR ALL FOOTINGS UNLESS IT CAN BE DEMONSTRATED THAT SOILS ARE STABLE ENOUGH TO MAINTAIN VERTICAL FORM-CUT DURING CONSTRUCTION. INSPECTOR TO VERIFY AND ADVISE.
- PROVIDE 8" WIDE x 8" DEEP MINIMUM THICKENED CONCRETE EDGE REINFORCED WITH (1) #4 BAR CONTINUOUS AT ALL DOOR OPENINGS.
- COORDINATE FINAL GRADE ELEVATIONS WITH TOP/FOOTING ELEVATIONS AND CIVIL PLANS PRIOR TO COMMENCING CONSTRUCTION. VERIFY 4" MINIMUM FROM TOP OF FOOTING TO FINISHED GRADE.
- SEE 5/S301 FOR CONSTRUCTION JOINT DETAILS (SLAB COLD JOINTS, IF NEEDED).
- PANEL DIMENSIONS SHOWN ON THESE PLANS ARE NOMINAL. ACTUAL DIMENSIONS WILL VARY. CONTRACTOR TO COORDINATE.

FOOTING SCHEDULE

MARK	SIZE	REINFORCEMENT				REMARKS
		LENGTH	WIDTH	DEPTH	TOP CENTER BOTTOM	
F10	CONT	1'-0"	1'-0"	-	-	(2) #5 BARS CONT
F14	CONT	1'-4"	0'-10"	-	-	(2) #5 BARS CONT
F22	CONT	2'-0"	1'-0"	-	-	(3) #5 BARS CONT
F22	CONT	2'-8"	1'-0"	-	-	(4) #5 BARS CONT
F23	CONT	3'-0"	1'-0"	-	-	(4) #5 BARS CONT
F27	5'-0"	5'-0"	1'-0"	-	-	(5) #5 BARS EW
F29	5'-6"	5'-6"	1'-0"	-	-	(6) #5 BARS EW
F30	9'-0"	9'-0"	1'-9"	-	-	(10) #6 BARS EW
F30	9'-6"	9'-6"	2'-0"	-	-	(9) #7 BARS EW
F32	10'-0"	10'-0"	2'-0"	-	-	(8) #6 BARS EW
F16	11'-6"	11'-6"	1'-6"	-	-	(11) #6 BARS EW
F29	12'-6"	12'-6"	2'-0"	-	-	(15) #6 BARS EW

GENERAL NOTES:

- THE BOTTOM OF ALL FOOTINGS SHALL BE (1'-0") MIN BELOW FINISHED EARTH GRADE.
- TOP OF ALL INTERIOR FOOTINGS SHALL BE 8" BELOW FINISHED FLOOR (UNLESS NOTED OTHERWISE).
- SEE DETAIL 8/S301 FOR FOOTING STEP (IF APPLICABLE).
- USE #5 TRANS BARS @ 48" OC MAX AT ANY CONTINUOUS FOOTING w/ (3) OR MORE BARS.

COLUMN SCHEDULE

MARK	TYPE	BASE PLATE	NOTES
C1	HSS 10x10 ³	16" x 16" x 1" THICK BASE PLATE W/ (4) 1" DIA ANCHOR BOLTS	-
C2	W30x124	12" x 32" x 1 1/2" THICK BASE PLATE W/ (4) 1" DIA ANCHOR BOLTS	-

- GENERAL NOTES:
1. SEE DETAIL 6/S301 FOR ANCHOR BOLT SPECIFICATIONS.

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OVERALL FOUNDATION PLAN

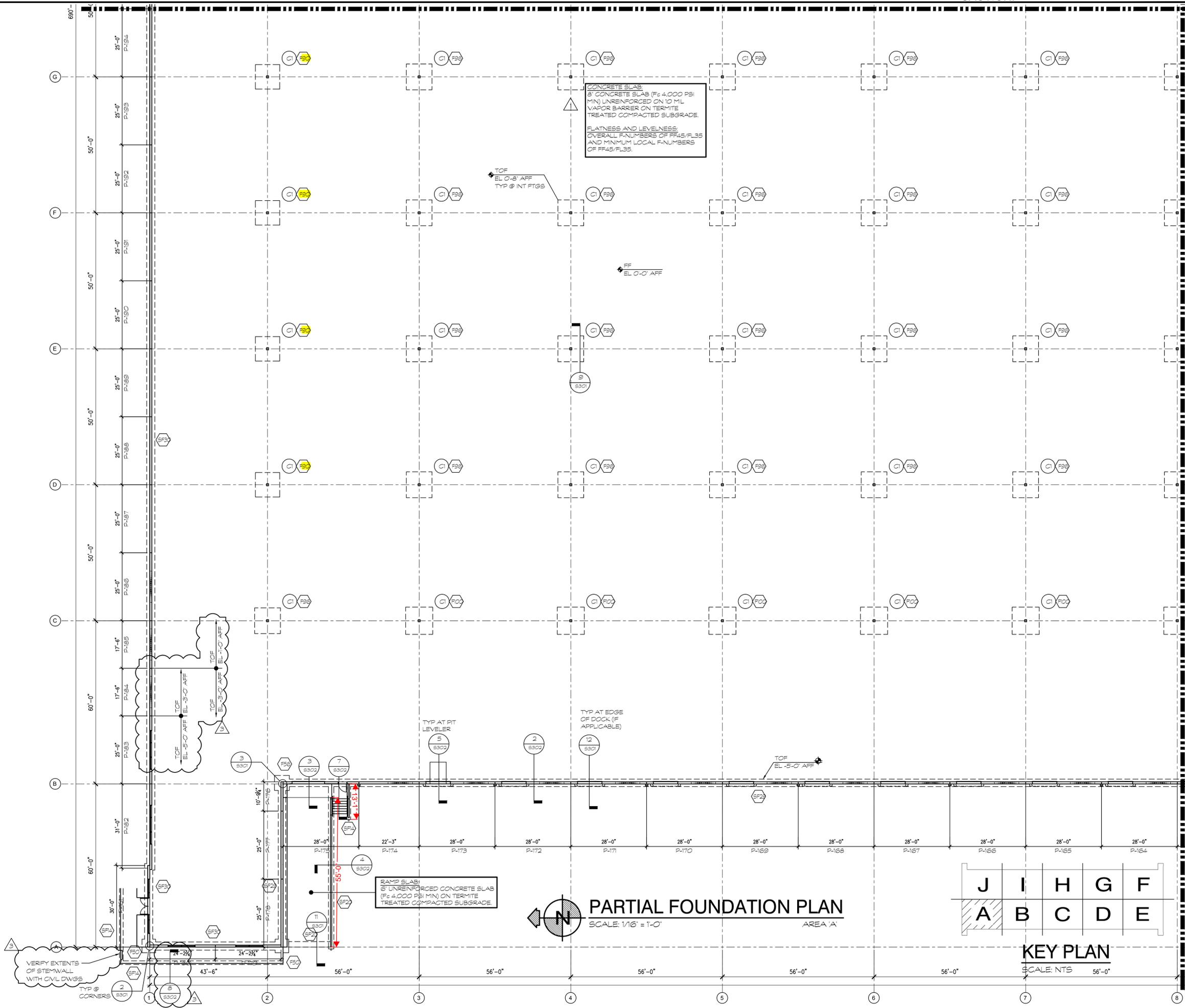
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JOB NO.:	MIA21-0040-00

SHEET
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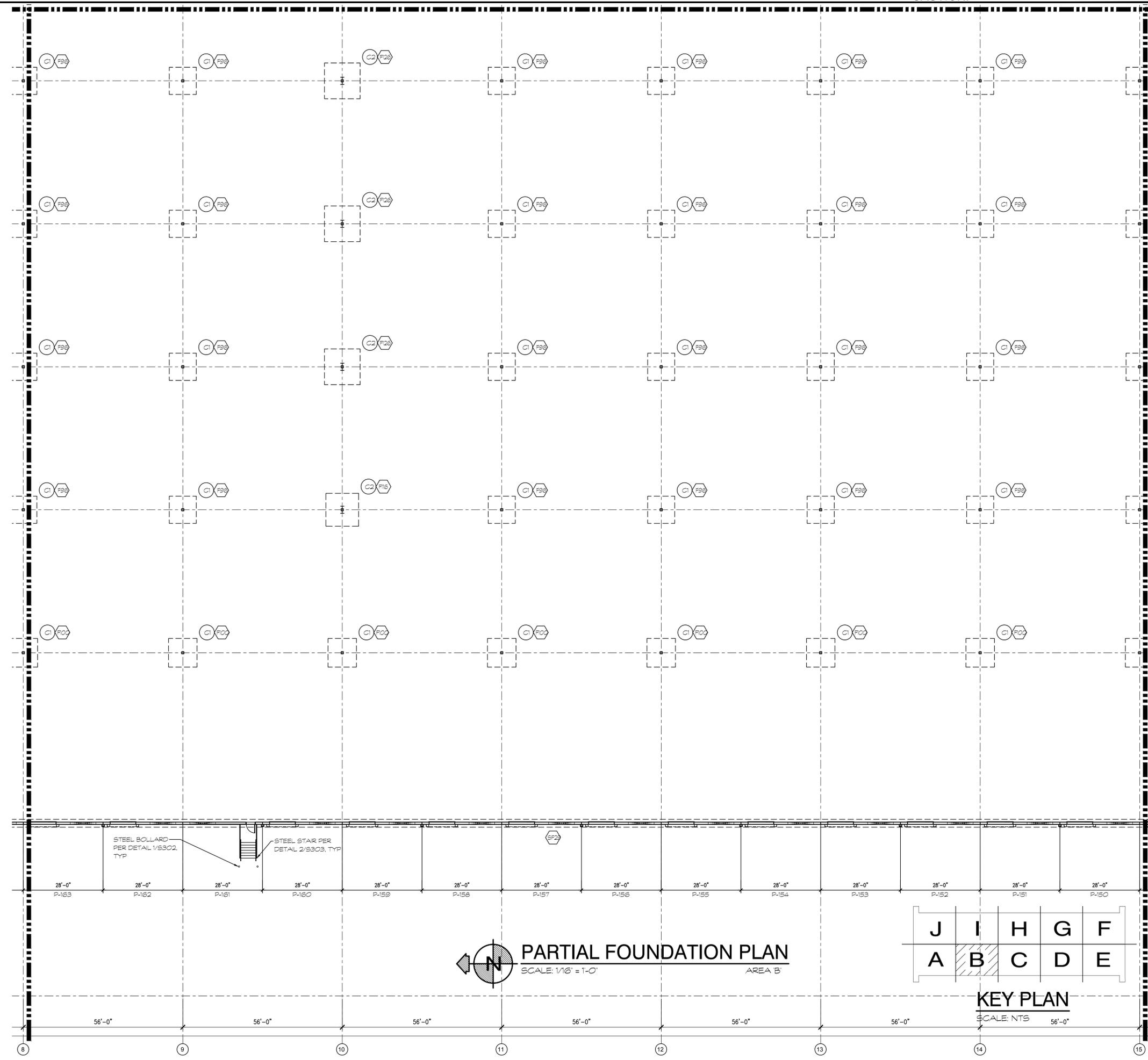
PARTIAL FOUNDATION PLAN AREA 'A'

DATE	REMARKS
07-13-2022	ISSUED FOR PERMIT
08-18-2022	OWNER REVISION
09-08-2022	CLIENT COORDINATION

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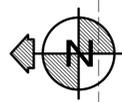
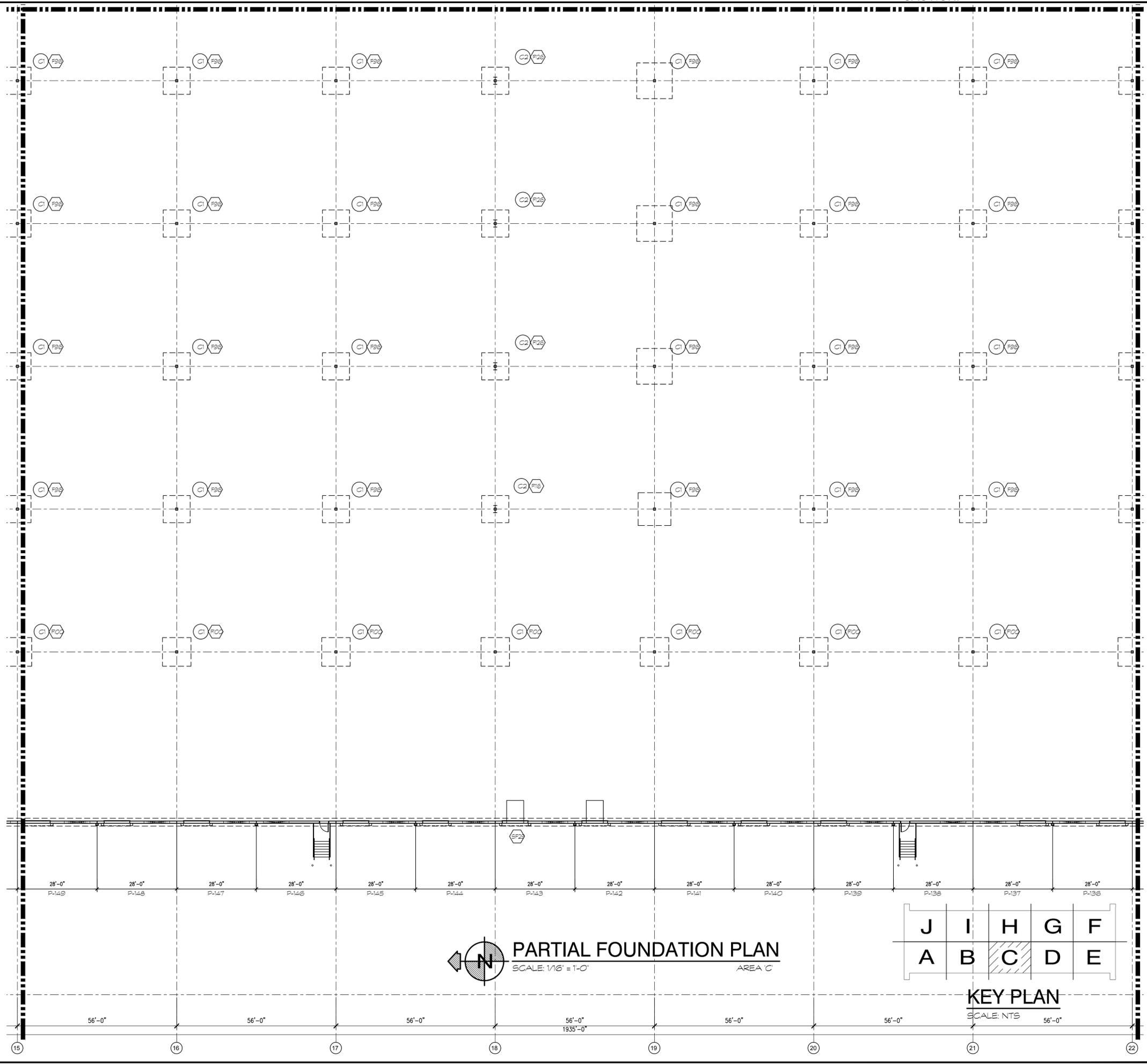
PARTIAL FOUNDATION PLAN AREA 'B'

DATE	REMARKS
07-13-2022	ISSUED FOR PERMIT

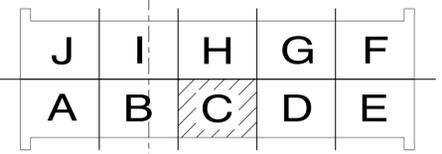
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PARTIAL FOUNDATION PLAN
SCALE: 1/16" = 1'-0"
AREA 'C'



KEY PLAN
SCALE: NTS

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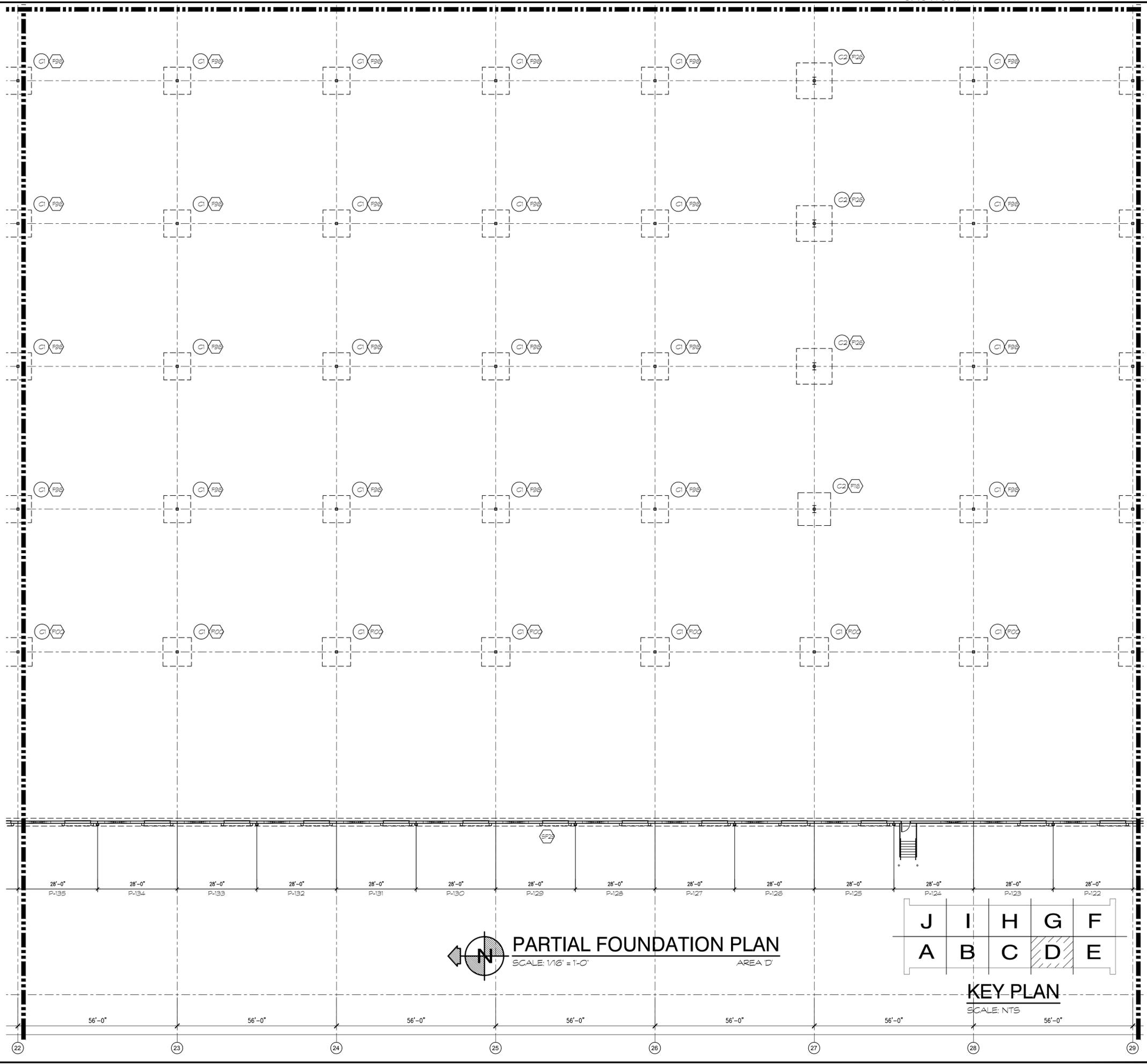
PARTIAL FOUNDATION PLAN AREA 'C'

DATE	REMARKS
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SHEET
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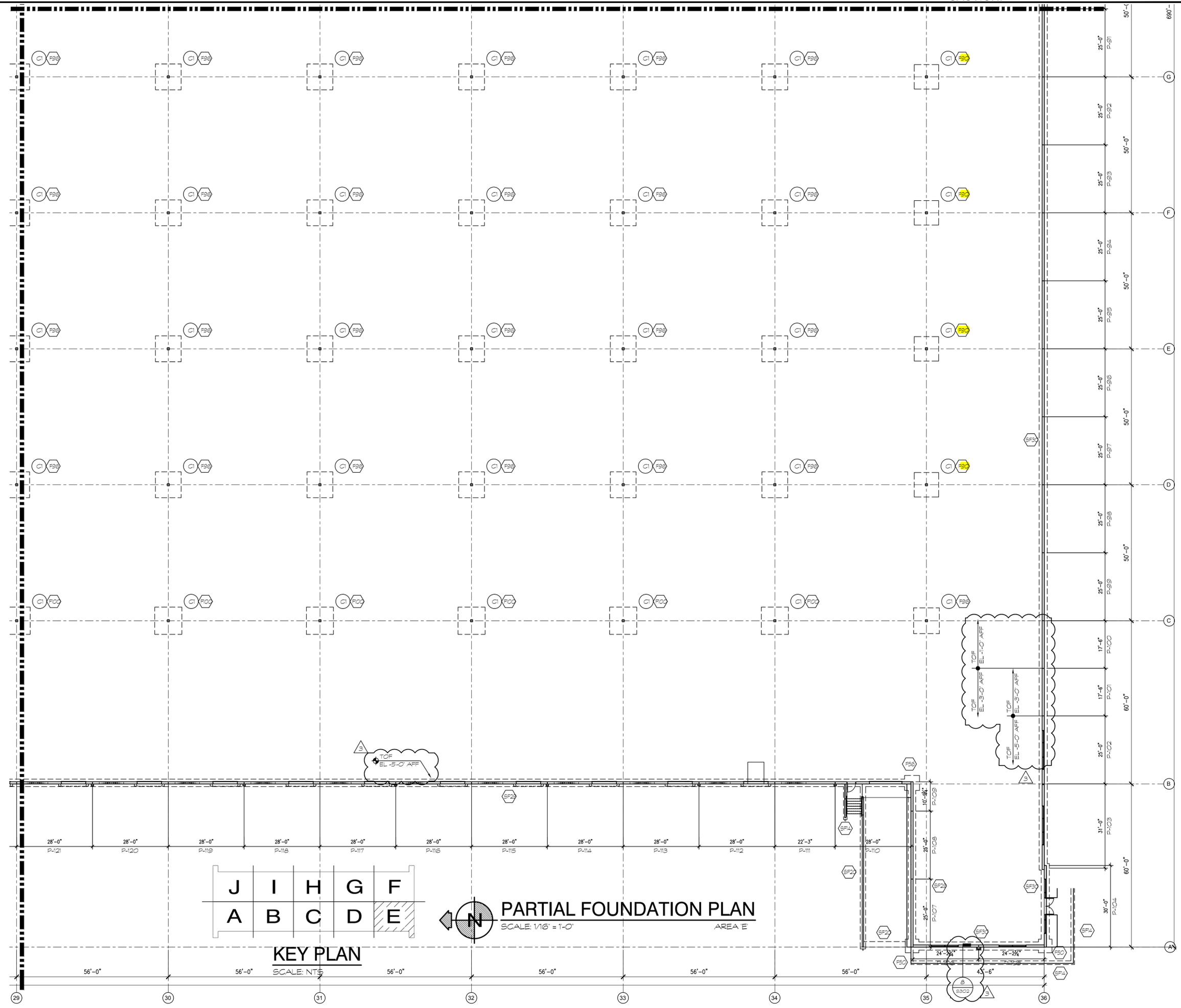
PARTIAL FOUNDATION PLAN AREA 'D'

DATE	REMARKS
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SHEET
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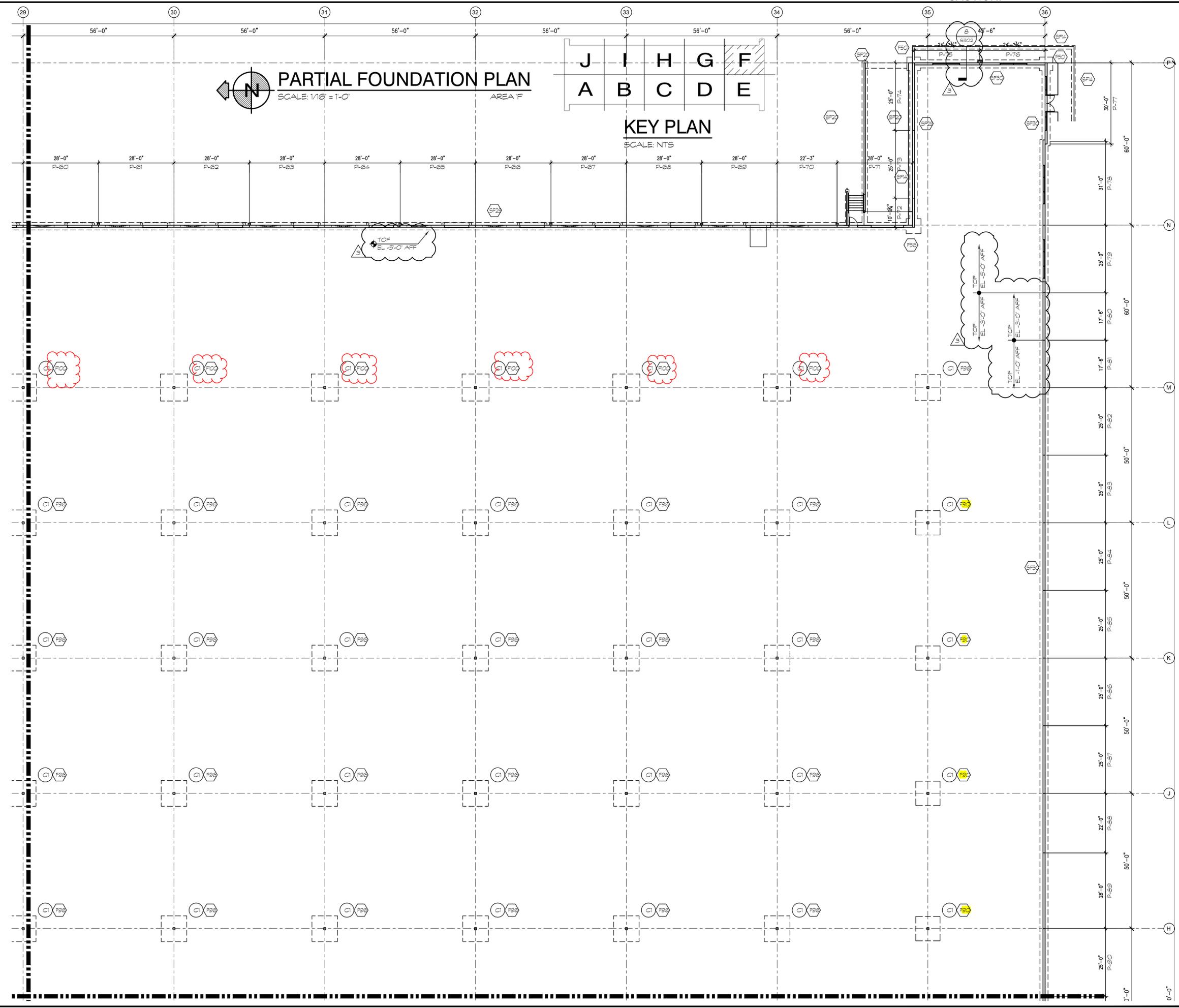
PARTIAL FOUNDATION PLAN AREA 'E'

DATE	REMARKS
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09-08-2022	CLIENT COORDINATION
3	

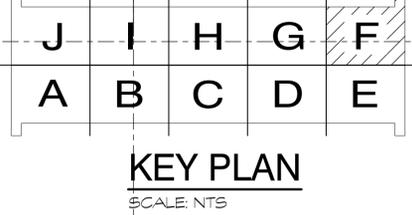
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PARTIAL FOUNDATION PLAN
SCALE: 1/8" = 1'-0"
AREA F



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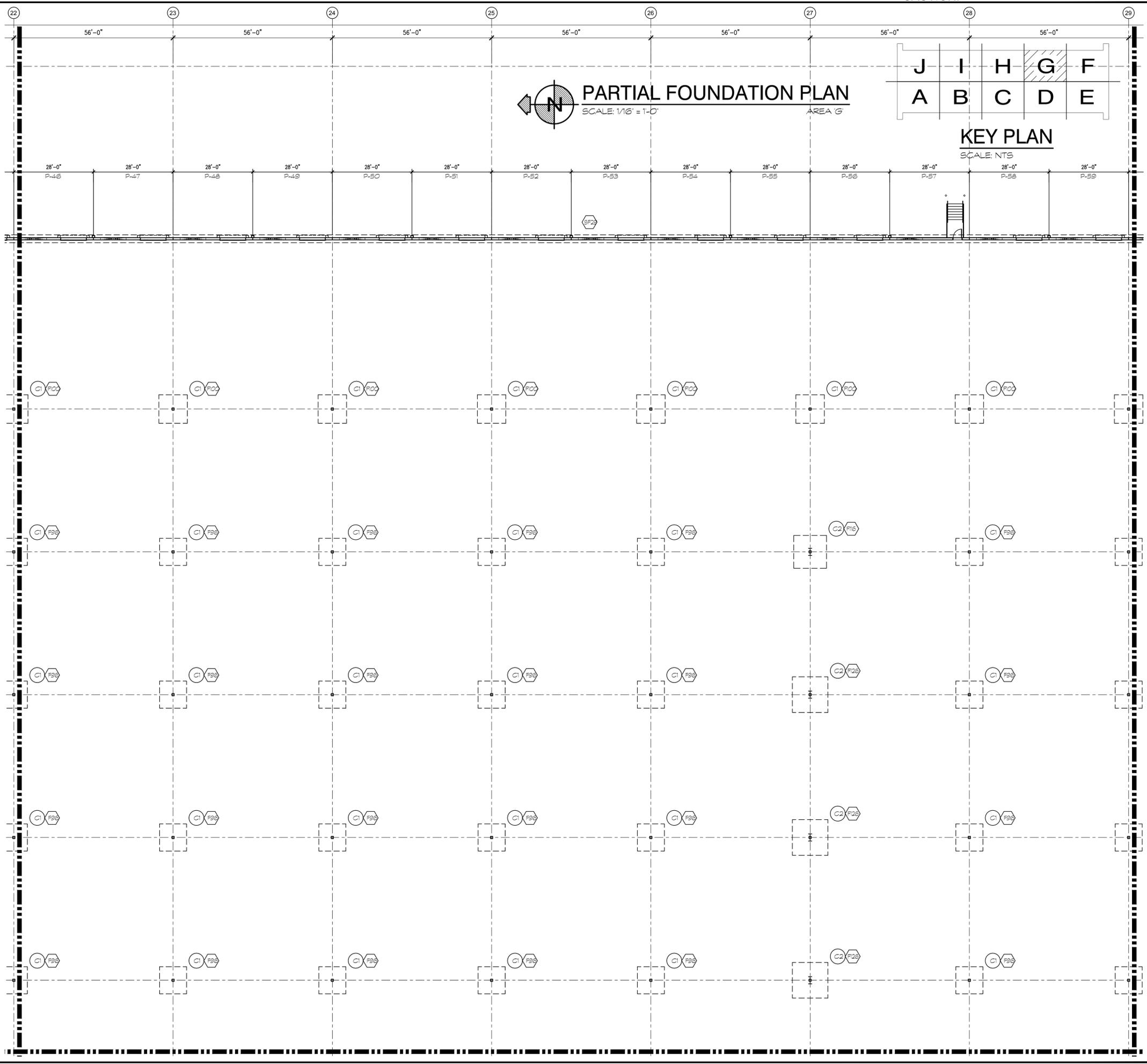
PARTIAL FOUNDATION PLAN AREA 'F'

DATE	REMARKS
07-13-2022	ISSUED FOR PERMIT
09-08-2022	CLIENT COORDINATION

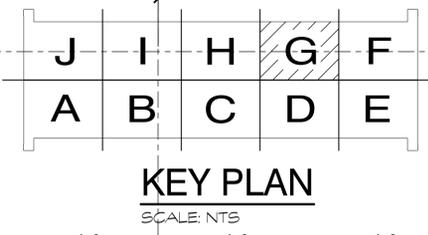
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SHEET
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PARTIAL FOUNDATION PLAN
SCALE: 1/16" = 1'-0"
AREA 'G'



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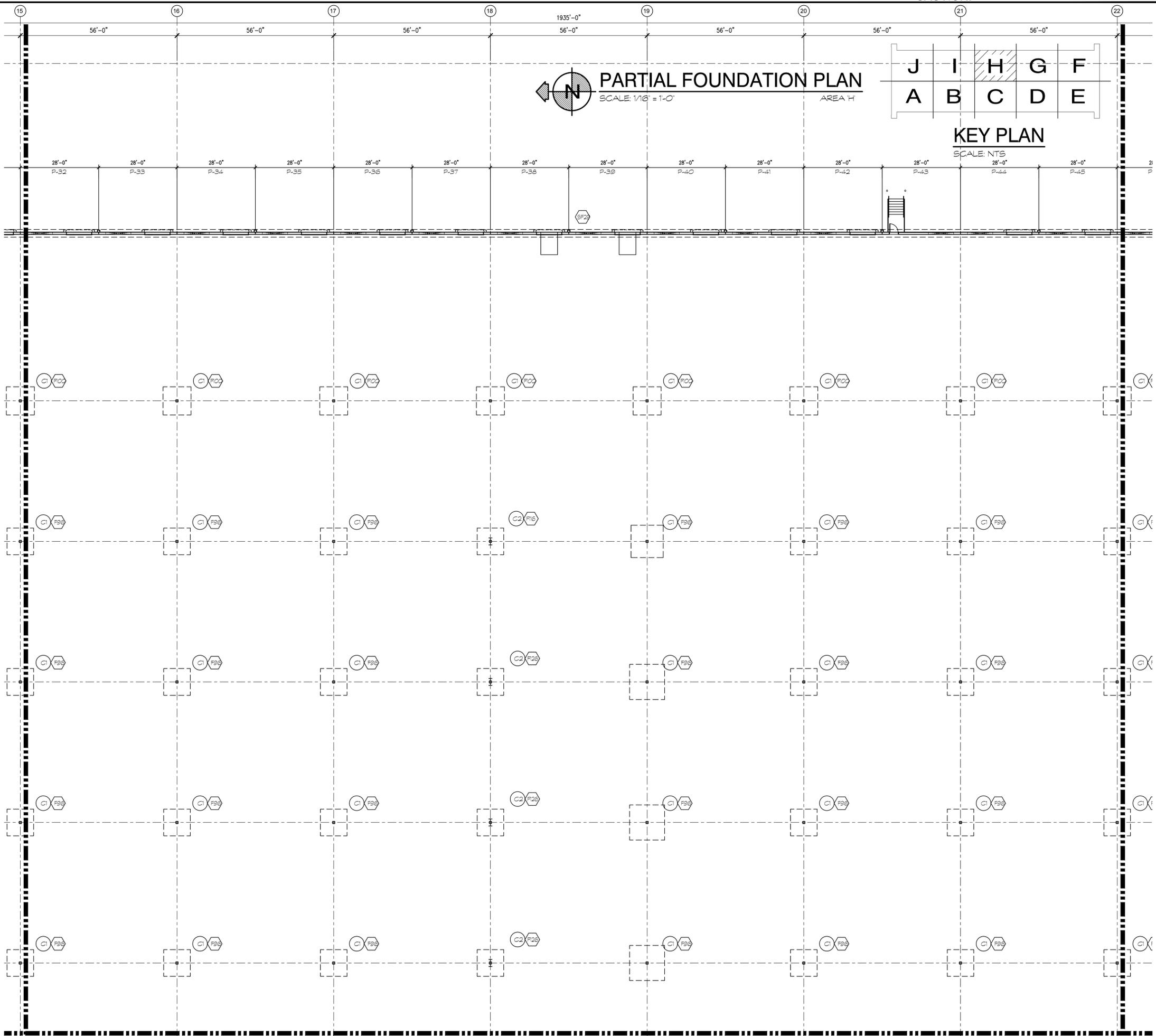
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PARTIAL FOUNDATION PLAN AREA 'G'	
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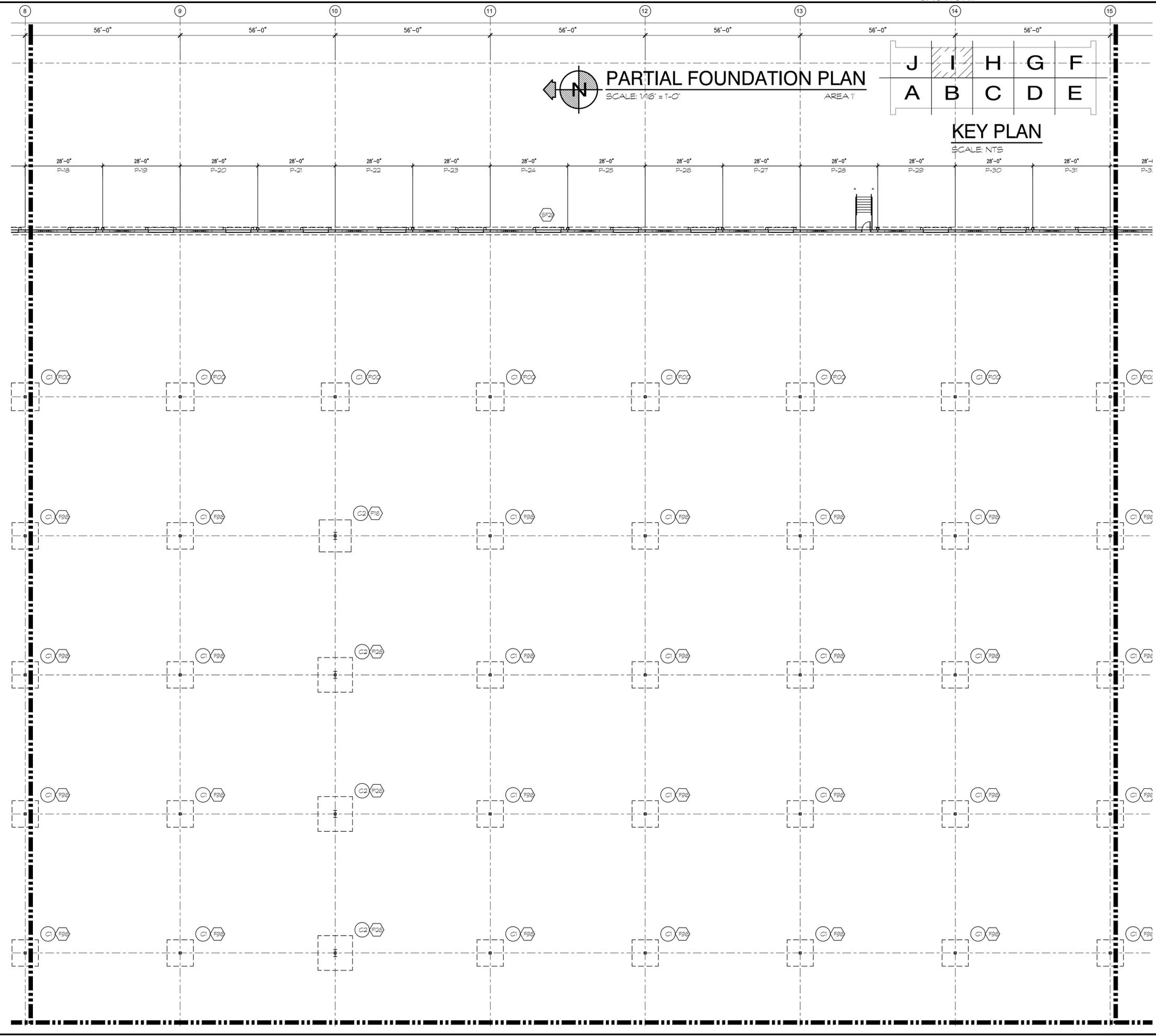
PARTIAL FOUNDATION PLAN AREA 'H'

DATE	REMARKS
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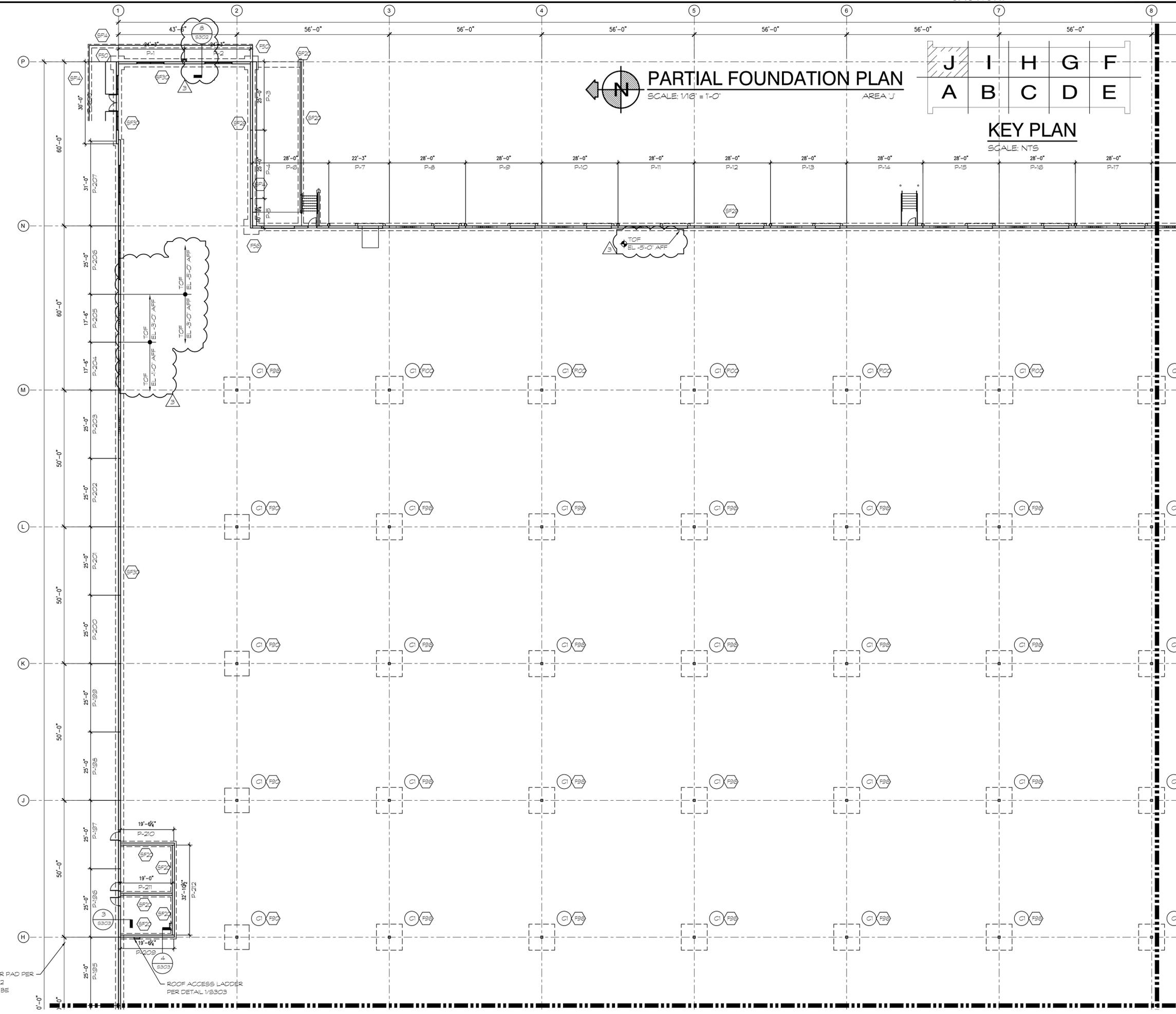
PARTIAL FOUNDATION PLAN AREA '1'

DATE	REMARKS
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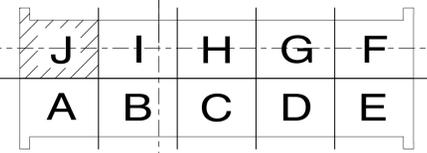
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PARTIAL FOUNDATION PLAN
SCALE: 1/16" = 1'-0"
AREA 'J'



KEY PLAN
SCALE: NTS

TRANSFORMER PAD PER
DETAIL 6/S302
LOCATION TO BE
DETERMINED

ROOF ACCESS LADDER
PER DETAIL 1/S303

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PARTIAL FOUNDATION PLAN AREA 'J'		REMARKS
DATE	07-13-2022	ISSUED FOR PERMIT
NO.	3	CLIENT COORDINATION

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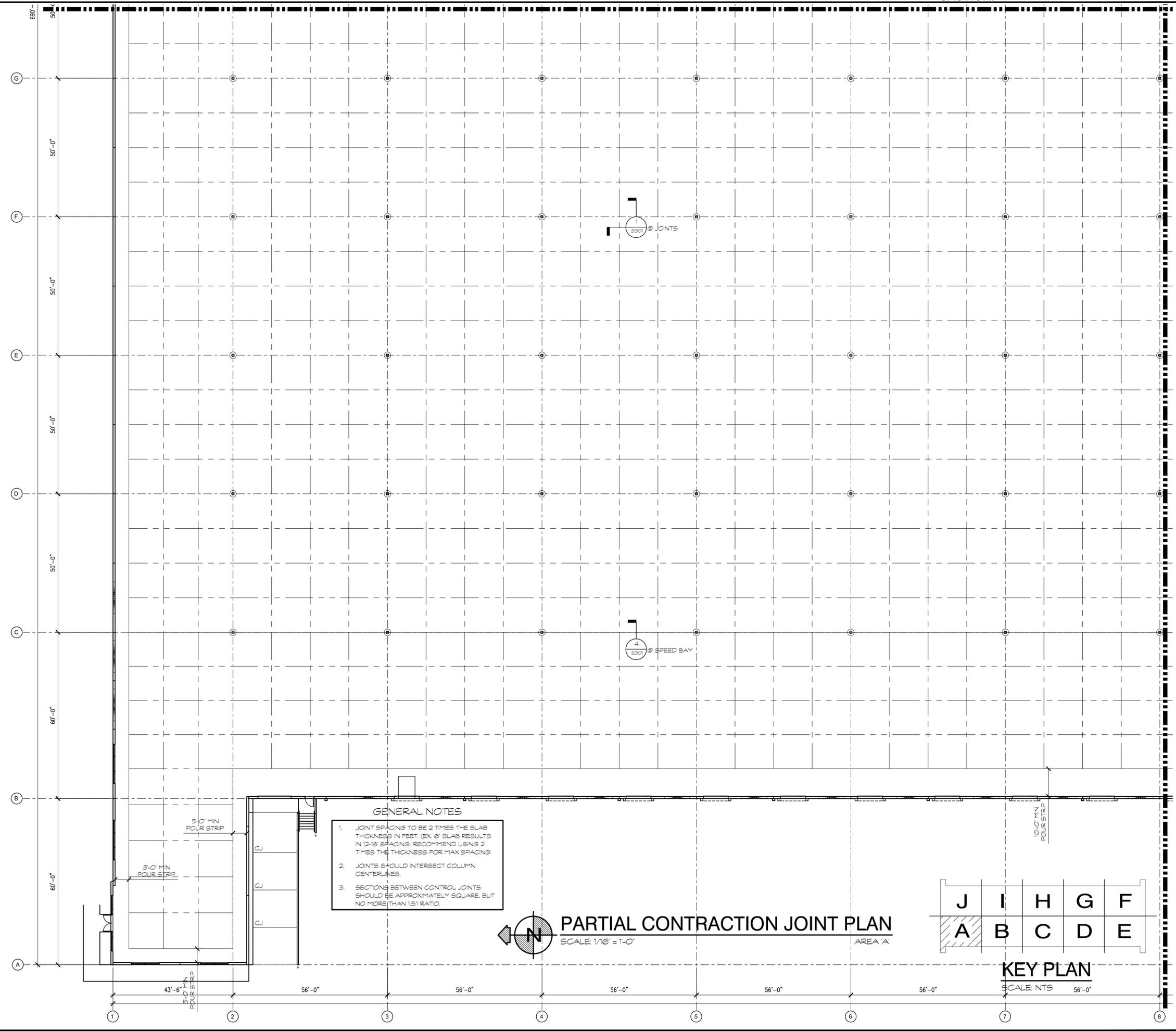
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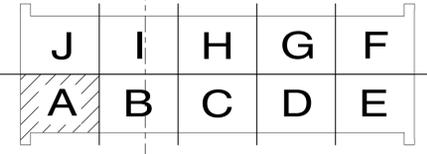
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- GENERAL NOTES**
1. JOINT SPACING TO BE 2 TIMES THE SLAB THICKNESS IN FEET. (EX. 6" SLAB RESULTS IN 12'-0" SPACING, RECOMMEND USING 2 TIMES THE THICKNESS FOR MAX SPACING.)
 2. JOINTS SHOULD INTERSECT COLUMN CENTERLINES.
 3. SECTIONS BETWEEN CONTROL JOINTS SHOULD BE APPROXIMATELY SQUARE, BUT NO MORE THAN 1.5:1 RATIO.



PARTIAL CONTRACTION JOINT PLAN
SCALE: 1/16" = 1'-0"
AREA 'A'



KEY PLAN
SCALE: NTS
56'-0"

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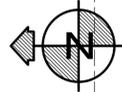
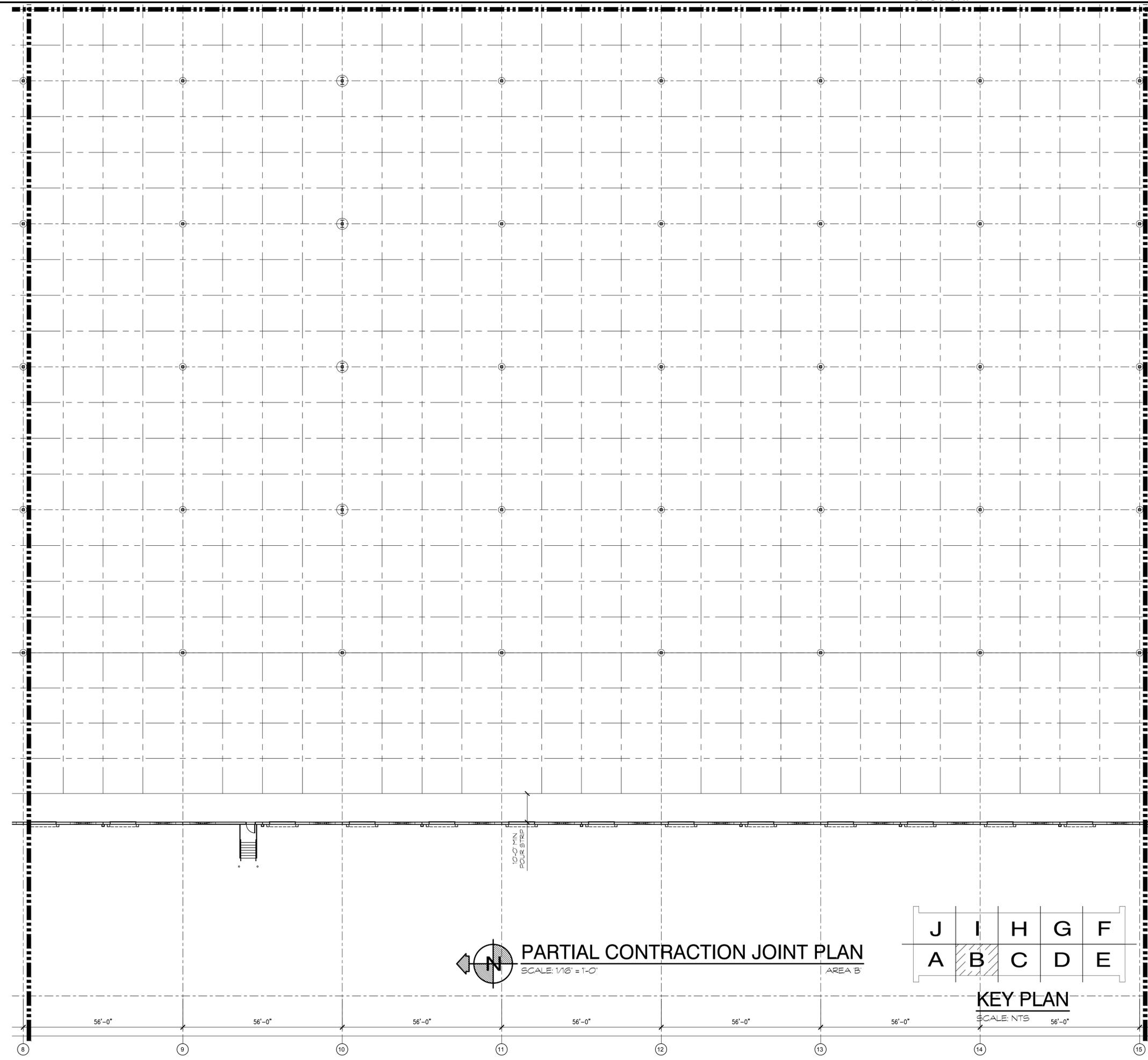
PARTIAL CONTRACTION JOINT PLAN AREA 'A'

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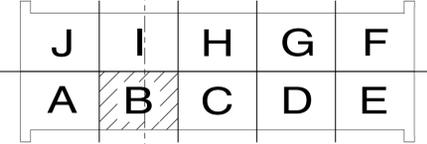
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PARTIAL CONTRACTION JOINT PLAN
SCALE: 1/16" = 1'-0" AREA B



KEY PLAN
SCALE: NTS 56'-0"

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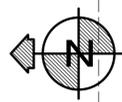
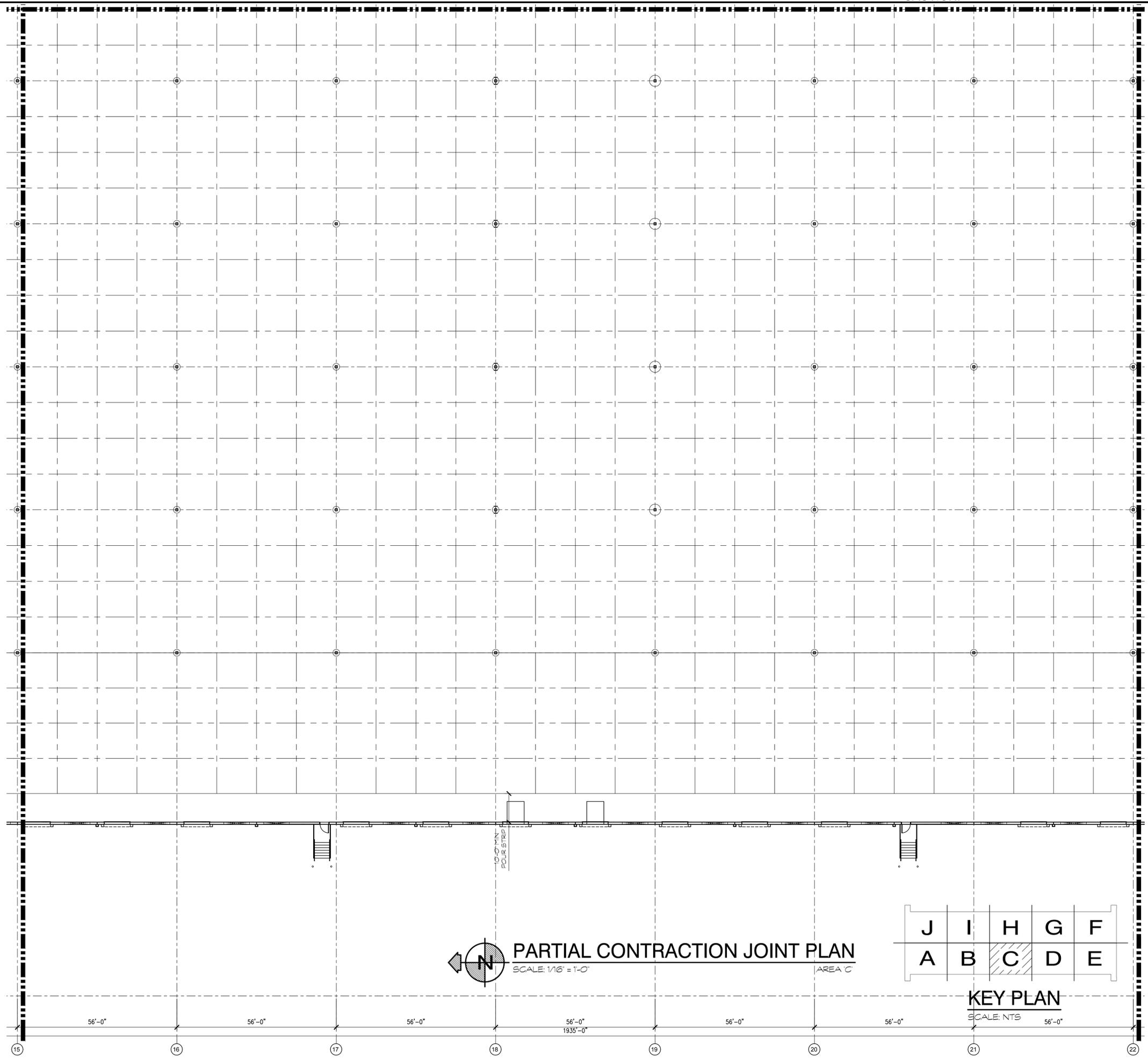
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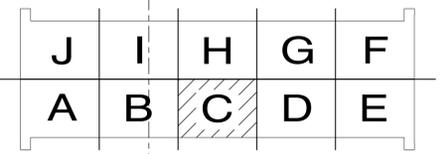
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PARTIAL CONTRACTION JOINT PLAN
SCALE: 1/16" = 1'-0" AREA C



KEY PLAN
SCALE: NTS

56'-0" 56'-0" 56'-0" 56'-0" 56'-0" 56'-0" 56'-0"

15 16 17 18 19 20 21 22

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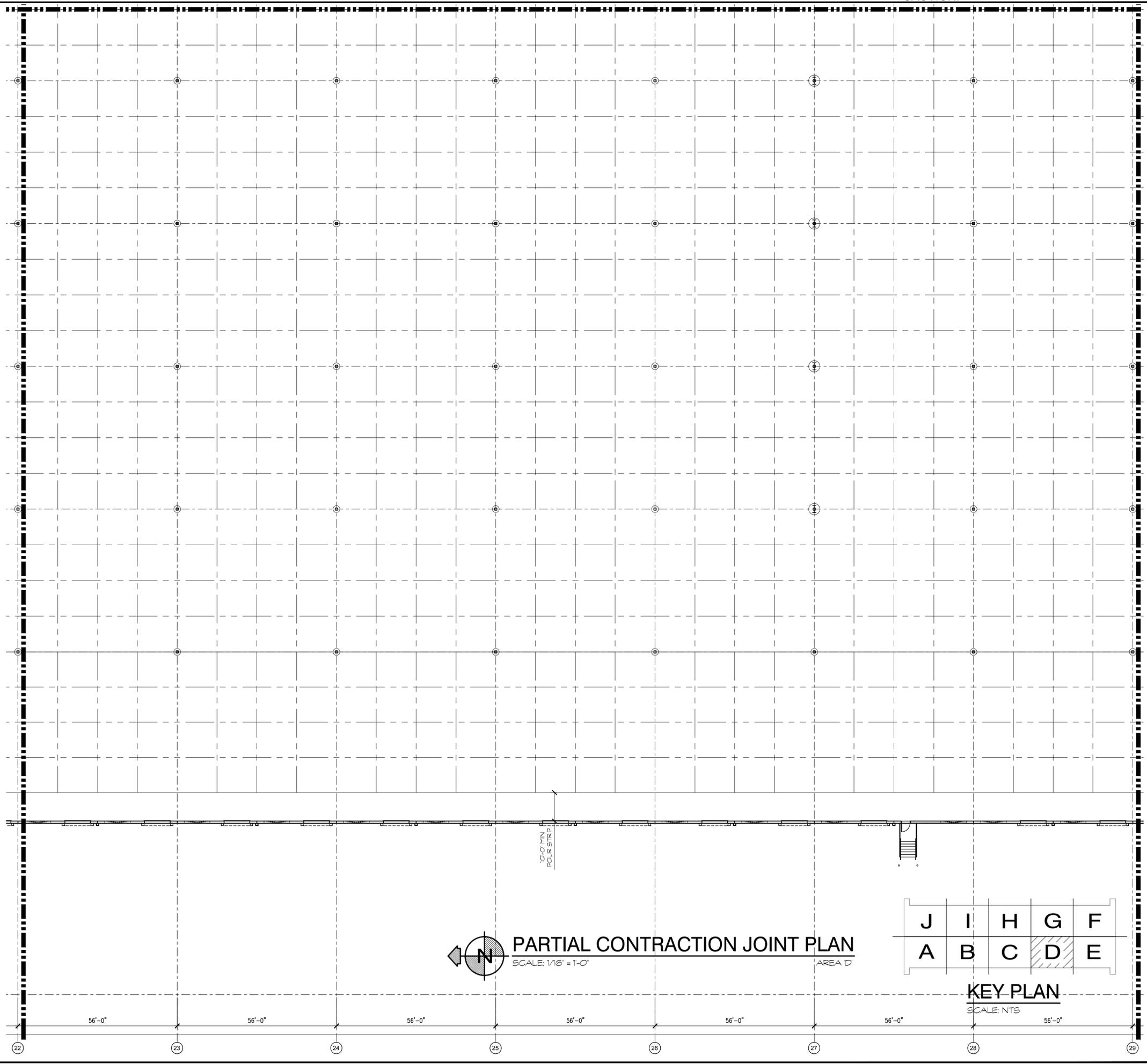
PARTIAL CONTRACTION JOINT PLAN AREA 'C'

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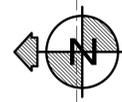
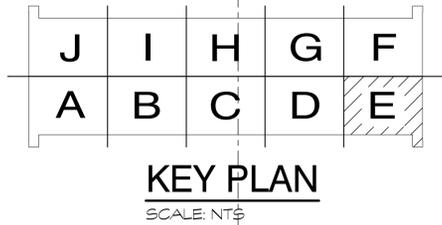
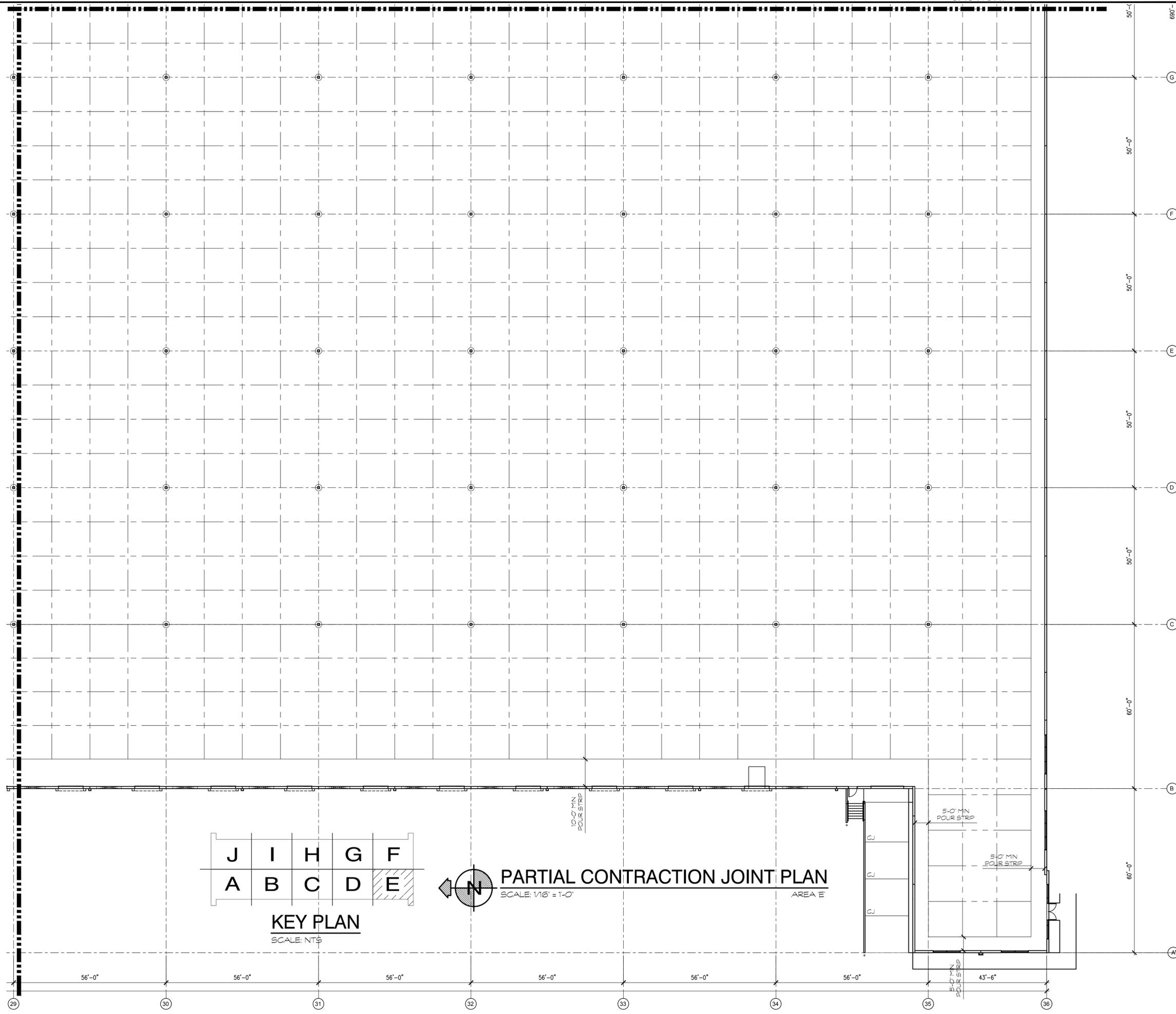
PARTIAL CONTRACTION JOINT PLAN AREA 'D'

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PARTIAL CONTRACTION JOINT PLAN
SCALE: 1/16" = 1'-0"
AREA E'

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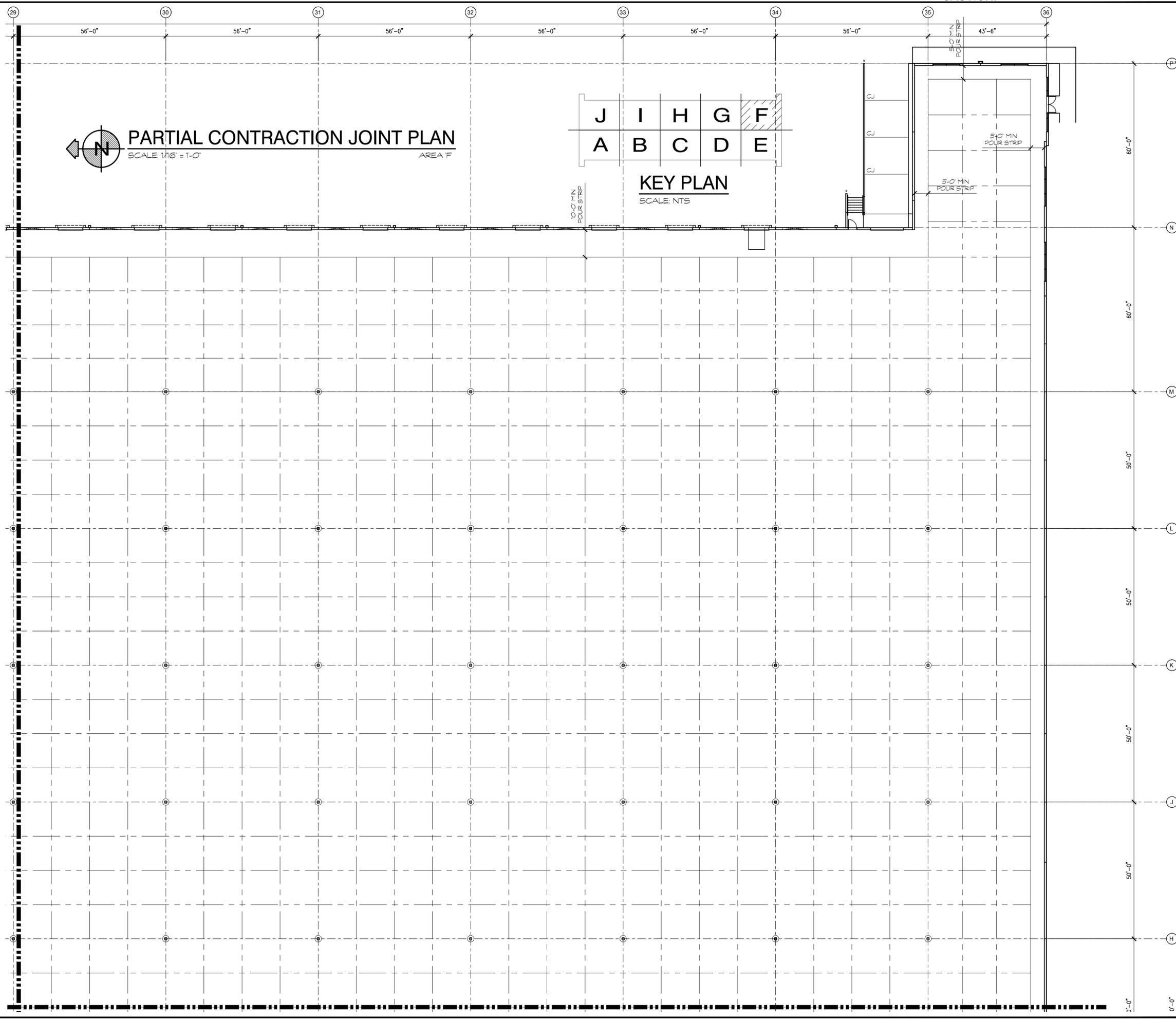
PARTIAL CONTRACTION JOINT PLAN AREA E'

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PARTIAL CONTRACTION JOINT PLAN
 SCALE: 1/16" = 1'-0"
 AREA 'F'

KEY PLAN
 SCALE: NTS

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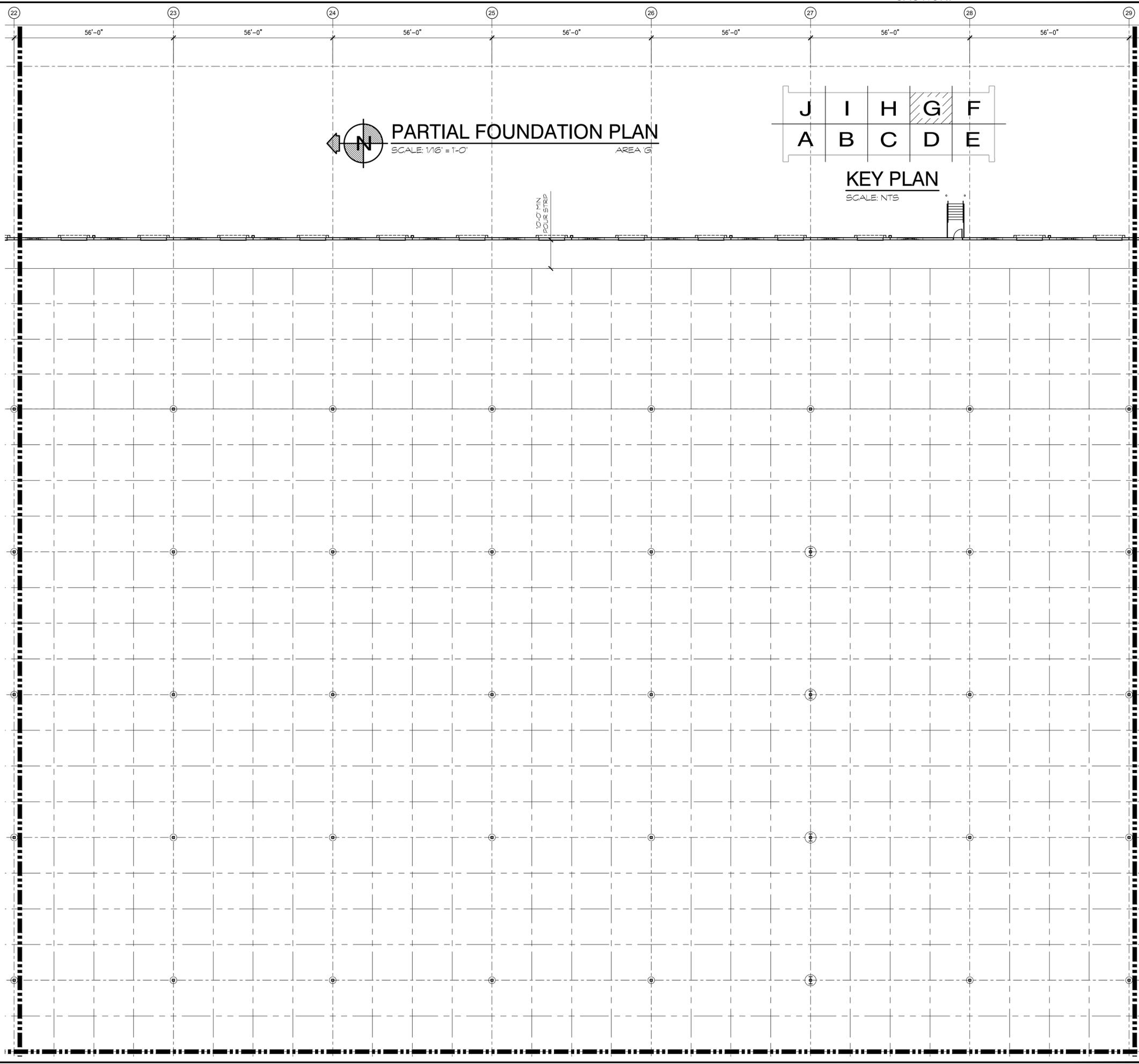
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PARTIAL CONTRACTION JOINT PLAN AREA 'F'	
DATE	REMARKS
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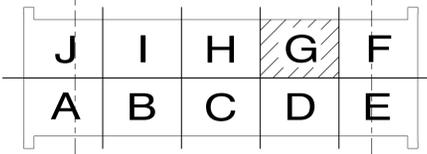
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PARTIAL FOUNDATION PLAN
 SCALE: 1/16" = 1'-0"
 AREA 'G'



KEY PLAN
 SCALE: NTS

12'-0" MIN
FOUR STRIP

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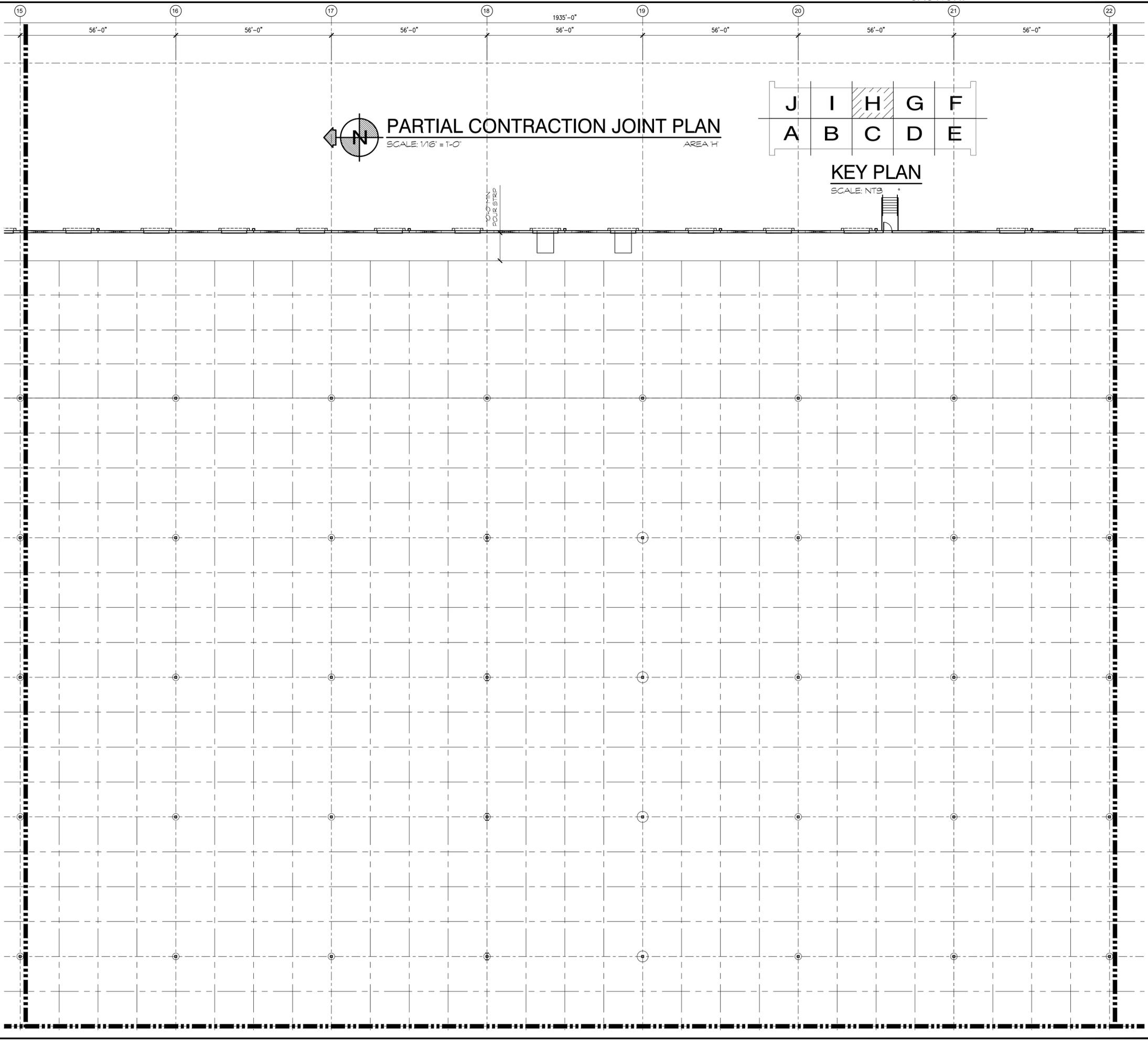
PARTIAL CONTRACTION JOINT PLAN AREA 'G'

DATE	REMARKS
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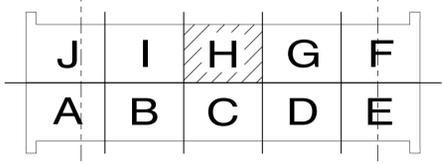
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PARTIAL CONTRACTION JOINT PLAN
 SCALE: 1/16" = 1'-0"
 AREA 'H'



KEY PLAN
 SCALE: NTS

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PARTIAL CONTRACTION JOINT PLAN AREA 'H'

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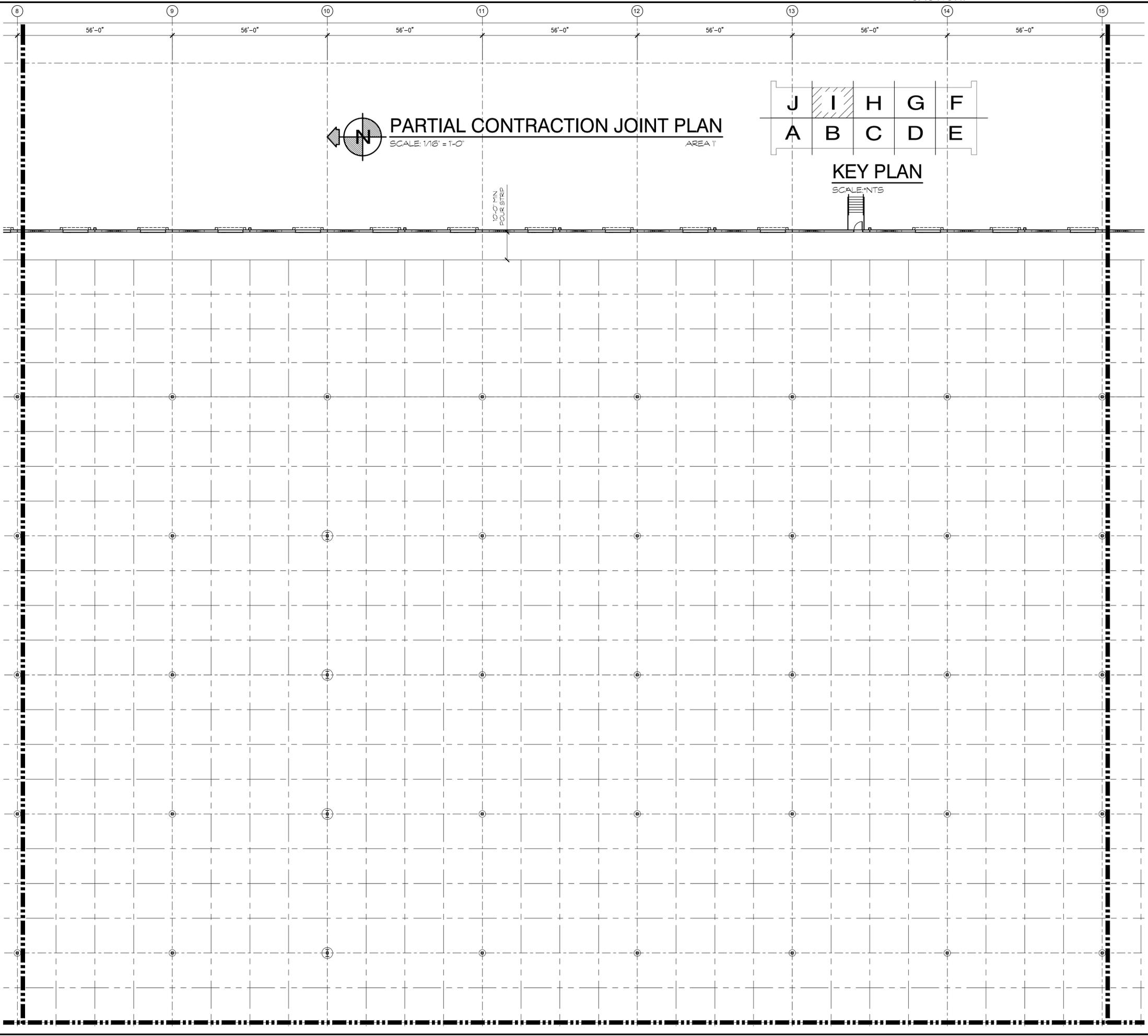
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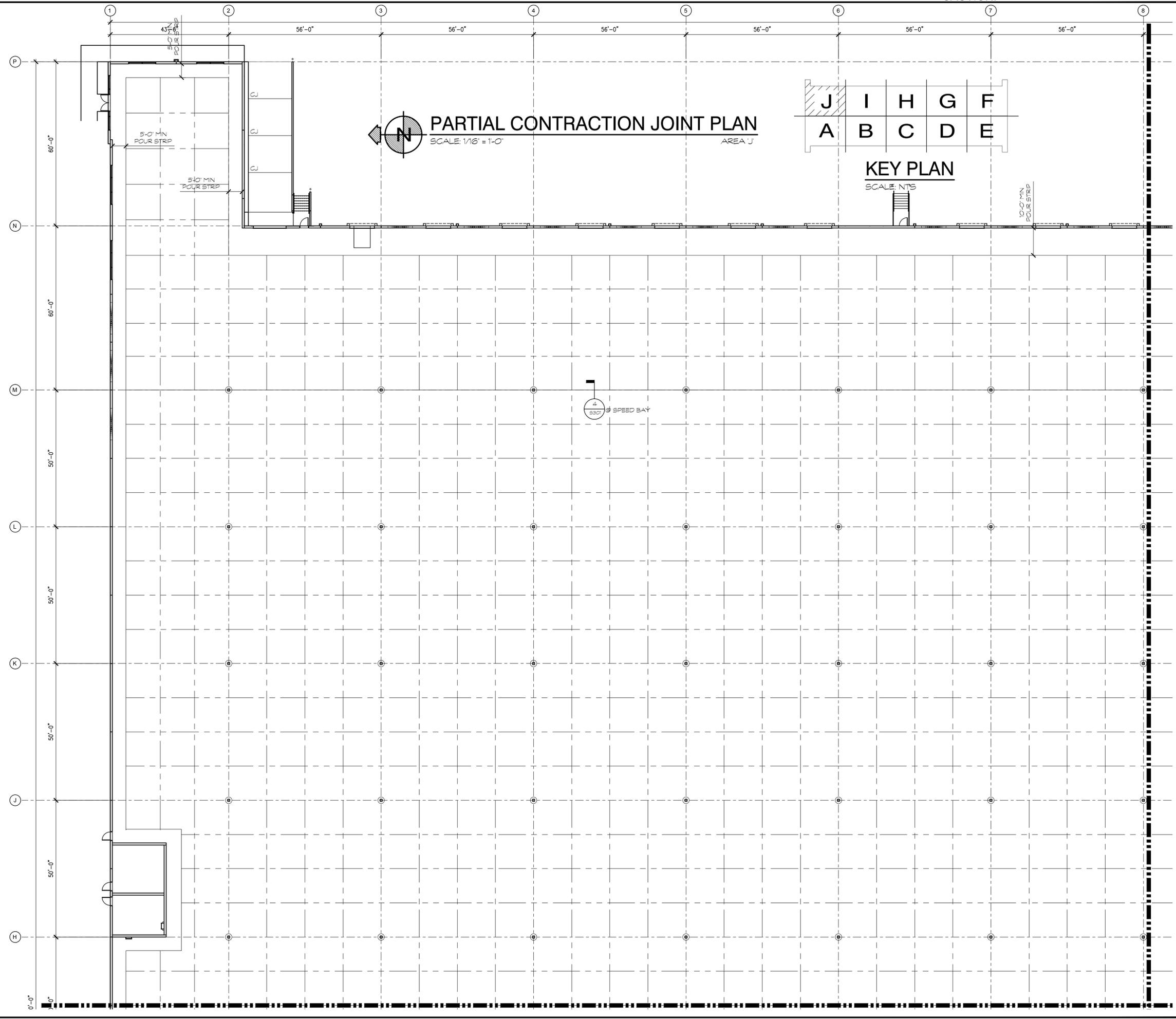
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PARTIAL CONTRACTION JOINT PLAN AREA '1'	
DATE	REMARKS
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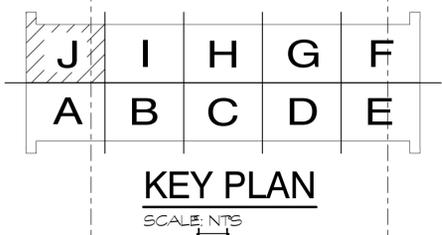
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PARTIAL CONTRACTION JOINT PLAN
 SCALE: 1/16" = 1'-0"
 AREA J



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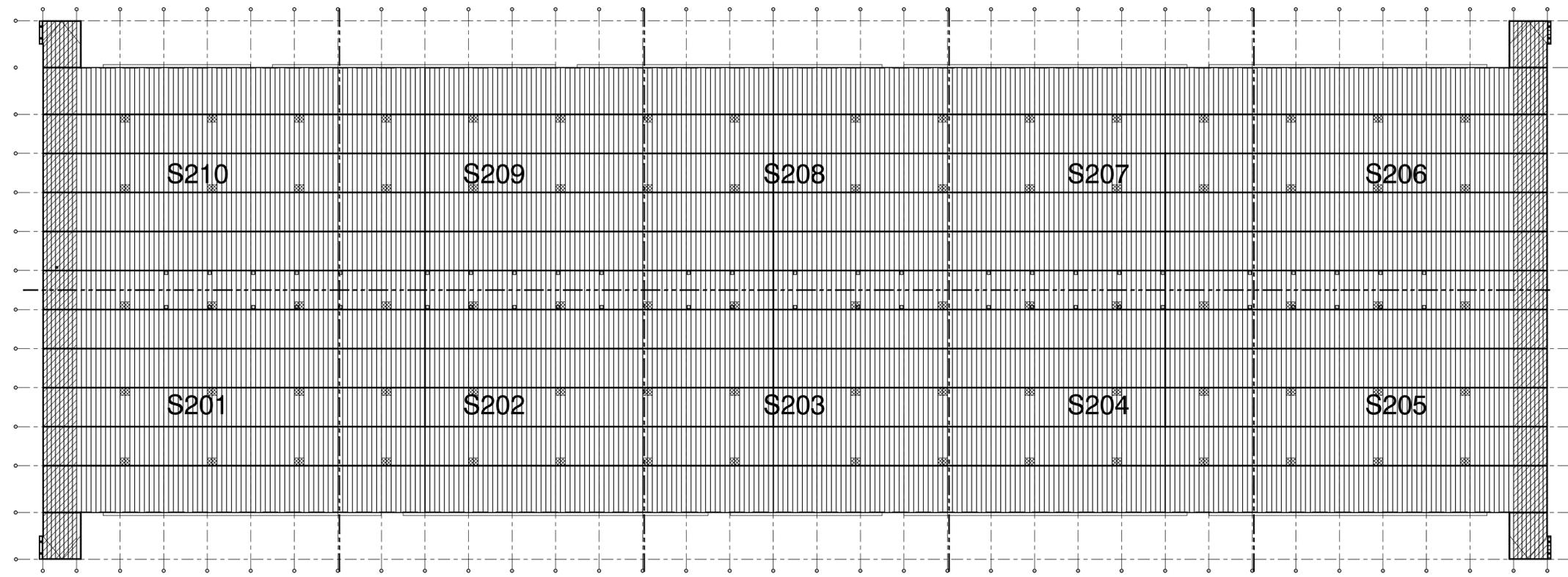
PARTIAL CONTRACTION JOINT PLAN AREA 'J'

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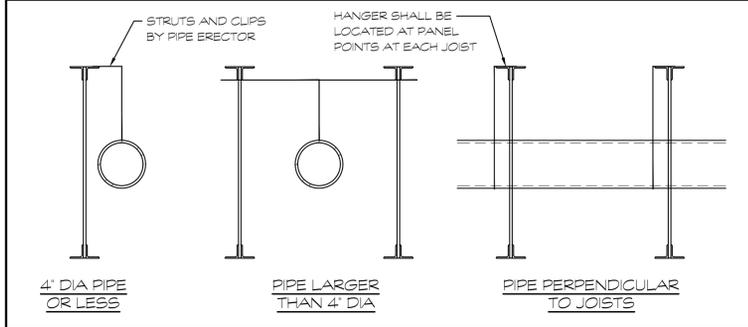


OVERALL ROOF FRAMING PLAN
SCALE: 1" = 80'-0"

METAL DECKING ATTACHMENT NOTES

1. WHEN POWDER DRIVEN OR PNEUMATIC FASTENERS ARE SHOWN ON STRUCTURAL DRAWINGS ACCEPTABLE FASTENER TYPES ARE PNUJTEK K AND SDK SERIES AND HILTI X-ENP19, X-HSN24, X-EDN19, AND X-EDN22. OTHER FASTENER TYPES MAY BE APPROVED IF FASTENER DESIGN INFORMATION IS PROVIDED. CONTRACTOR SHALL COORDINATE FASTENER TYPE WITH THE THICKNESS OF STEEL SUBSTRATE THAT THE FASTENER IS ATTACHED TO.
2. WHERE SIDELAP FASTENERS ARE SPECIFIED ON THE STRUCTURAL DRAWINGS ACCEPTABLE MANUFACTURERS ARE BUILDEX, ELCO, HILTI, SIMPSON STRONG TIE OR TRIANGLE. NO SUBSTITUTION WILL BE APPROVED. IF A SPECIFIC TYPE OF SIDELAP SCREW IS SHOWN ON THE STRUCTURAL DRAWINGS, IT MUST BE USED UNLESS AN ALTERNATE IS APPROVED BY THE ENGINEER.
3. GC TO PROVIDE CALCULATIONS PREPARED BY MANUFACTURER OF FASTENING SYSTEM FOR EOR REVIEW PRIOR TO DECKING INSTALLATION.
4. SEE DECKING DIAPHRAGM SKETCH FOR REQUIRED VALUES AND ZONES.

WATER PIPE SUPPORT SCHEDULE



DIA (IN)	WEIGHT (PLF)	MAX PIPE SUPPORT SPACING (FT)	NOTES:
2 1/2	8.5	12	1. PIPES IN TABLE ARE SCHEDULE 40 OR STANDARD (S) TYPE. 2. PIPE WEIGHT INCLUDES PIPE + INSULATION + WATER. 3. EXACT PIPE LOCATION TO BE COORDINATED WITH FIRE PROTECTION DESIGN DWGS. 4. PIPES RUNNING PARALLEL TO JOIST w/ DIA GREATER THAN 4" OR RUNNING IN COMBINATION w/ OTHER PIPES SHALL BE DISTRIBUTED TO A MINIMUM OF TWO JOISTS. ONE JOIST SHOULD NOT SUPPORT (2) OR MORE PIPES GREATER THAN 4". 5. MEMBER SIZES ON PLAN HAVE BEEN ADJUSTED TO SUPPORT WATER PIPING LOADS IN THIS TABLE. 6. ANY PIPE OR COMBINATION OF PIPES WITH TOTAL DIA GREATER THAN 8" SHALL BE HUNG PER THE DIRECTION OF THE ENGINEER (NOTIFY ENGINEER PRIOR TO PROCEEDING WITH WORK).
3	11.5	12	
4	17.0	12	
5	24.5	12	
6	32.5	6	
8	52.0	6	

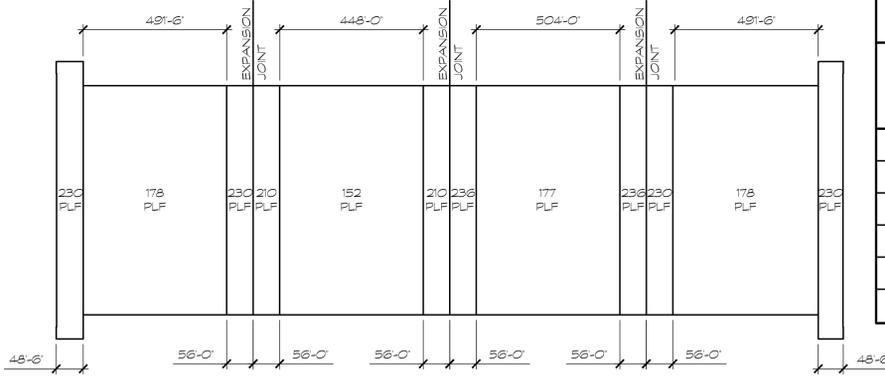
ROOF FRAMING GENERAL NOTES

1. STRUCTURAL CLEAR HEIGHT SHALL BE 40'-0" AFF FROM GROUNDLINE G.M (UPHILL OF GIRDER).
2. BAR JOISTS & GIRDERS SHOWN IN FUTURE RTU ZONE ON ROOF PLAN HAVE BEEN DESIGNED INCLUDING DEAD LOADS ASSOCIATED WITH FUTURE ROOF TOP UNIT AND CURB LOCATED ANYWHERE ALONG JOISTS. SUPPORTED BY TWO JOISTS MINIMUM (LIMIT ONE UNIT PER JOIST SPAN).
3. SEE DETAIL 1/8402 FOR PERIMETER ANGLE SPLICE. SPLICE EVERY THREE PANELS MAXIMUM.
4. SEE DETAIL 2/8401 FOR TYP DECK OPENING WHERE REQUIRED (ie ROOF HATCH).
5. SEE DETAIL 1/8401 @ FUTURE RTU SUPPORT FRAMING & OPENING.
6. SEE ARCHT DWGS FOR REQUIRED ROOFING & ROOFING DETAILS (ie CRICKETS, SCUPPERS, DRAINS, SLOPES, FLASHING, ETC).
7. SEE DETAIL 8/8401 & 2/8402 FOR JOIST BEARING AT TILT PANEL JOINT.
8. SEE DETAIL 3/8401 FOR BAR JOIST STIFFENER REQUIREMENTS.

DESIGN LOAD SCHEDULE (OFFICE AREAS)

TPO ROOFING	3.0
ROOF INSULATION	2.0
METAL ROOF DECK	2.0
BAR JOIST FRAMING	2.5
FUTURE OFFICE CEILING	1.5
MECHANICAL/ELECTRICAL	2.0
MISCELLANEOUS	1.0
SPRINKLER SYSTEM	3.0
TOTAL DEAD LOAD	17.0
TOTAL LIVE LOAD	20.0
TOTAL LOAD	37.0

- GENERAL NOTES:**
1. CODE PERMITTED LIVE LOAD REDUCTIONS BASED ON AREA SHALL BE TAKEN.
 2. ALL LOADS SHOWN ABOVE ARE IN PSF.



DECKING DIAPHRAGM ZONES NTS

NOTE: DIAPHRAGM LOADS ARE BASED ON ASD LOAD COMBINATION 0.6D+0.6W

DESIGN LOAD SCHEDULE (WAREHOUSE AREAS)

TPO ROOFING	3.0
ROOF INSULATION	2.0
METAL ROOF DECK	2.0
BAR JOIST FRAMING	2.5
FUTURE OFFICE CEILING	0.0
MECHANICAL/ELECTRICAL	1.0
MISCELLANEOUS	0.5
SPRINKLER SYSTEM	3.0
TOTAL DEAD LOAD	14.0
TOTAL LIVE LOAD	20.0
TOTAL LOAD	34.0

- GENERAL NOTES:**
1. CODE PERMITTED LIVE LOAD REDUCTIONS BASED ON AREA SHALL BE TAKEN.
 2. ALL LOADS SHOWN ABOVE ARE IN PSF.

LEGEND

- ADDITIONAL GIRDER POINT LOAD ABOVE DESIGNATION (IF APPLICABLE)
- FUTURE RTU ZONE (RTU + CURB WEIGHT = 1,500 LB)
- FUTURE RTU ZONE (RTU + CURB WEIGHT = 2,500 LB)
- ROOF EXHAUST FAN PER MECH DWGS WEIGHT = 900 LB

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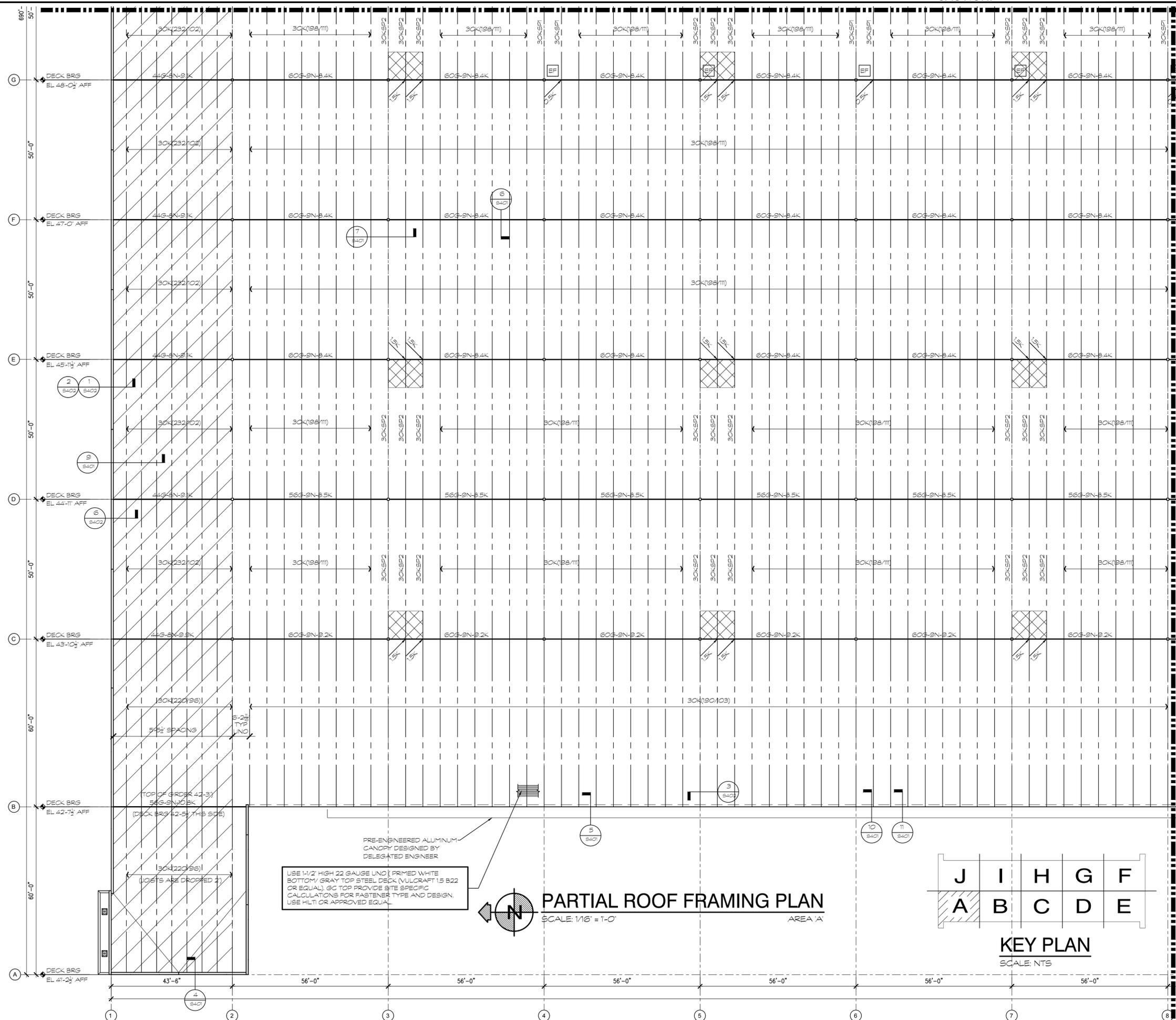
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OVERALL ROOF FRAMING PLAN

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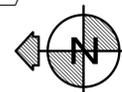
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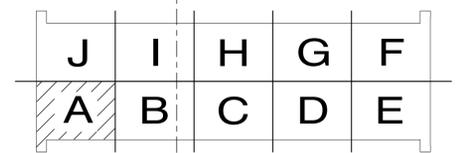


PRE-ENGINEERED ALUMINUM CANOPY DESIGNED BY DELEGATED ENGINEER

USE 1 1/2" HIGH 22 GAUGE UNO (PRIMED WHITE BOTTOM/ GRAY TOP STEEL DECK (NULCRAFT 1.5 B22 OR EQUAL), GC TOP PROVIDE SITE SPECIFIC CALCULATIONS FOR FASTENER TYPE AND DESIGN. USE HILTI OR APPROVED EQUAL.



PARTIAL ROOF FRAMING PLAN
SCALE: 1/16" = 1'-0"
AREA 'A'



KEY PLAN
SCALE: N.T.S.

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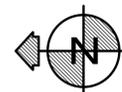
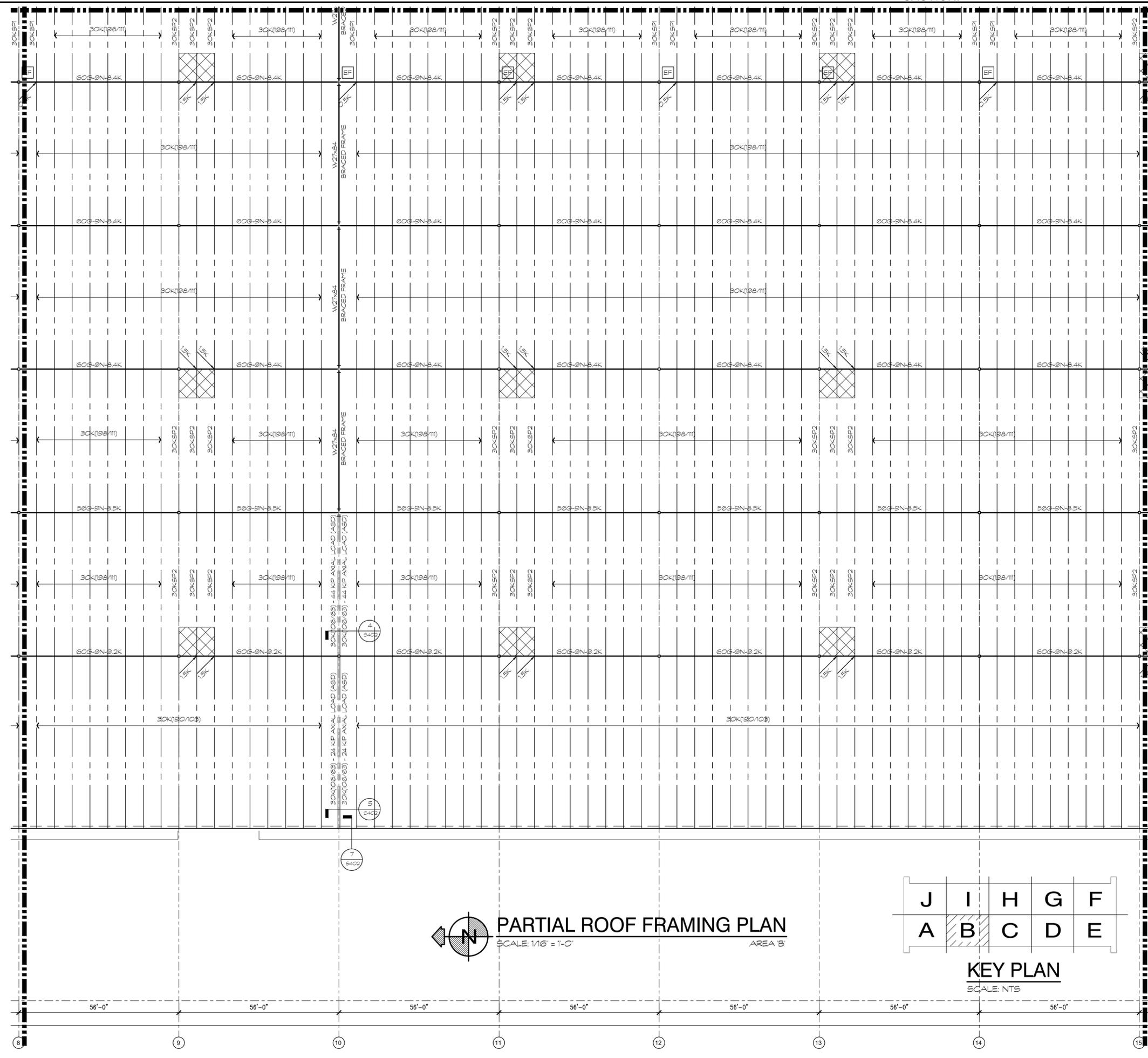
PARTIAL ROOF FRAMING PLAN AREA 'A'

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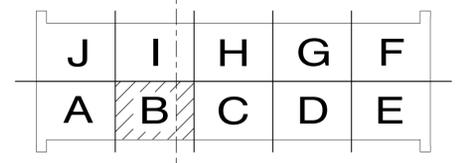
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PARTIAL ROOF FRAMING PLAN
SCALE: 1/16" = 1'-0" AREA 'B'



KEY PLAN
SCALE: N.T.S.

PARTIAL ROOF FRAMING PLAN AREA 'B'

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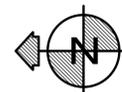
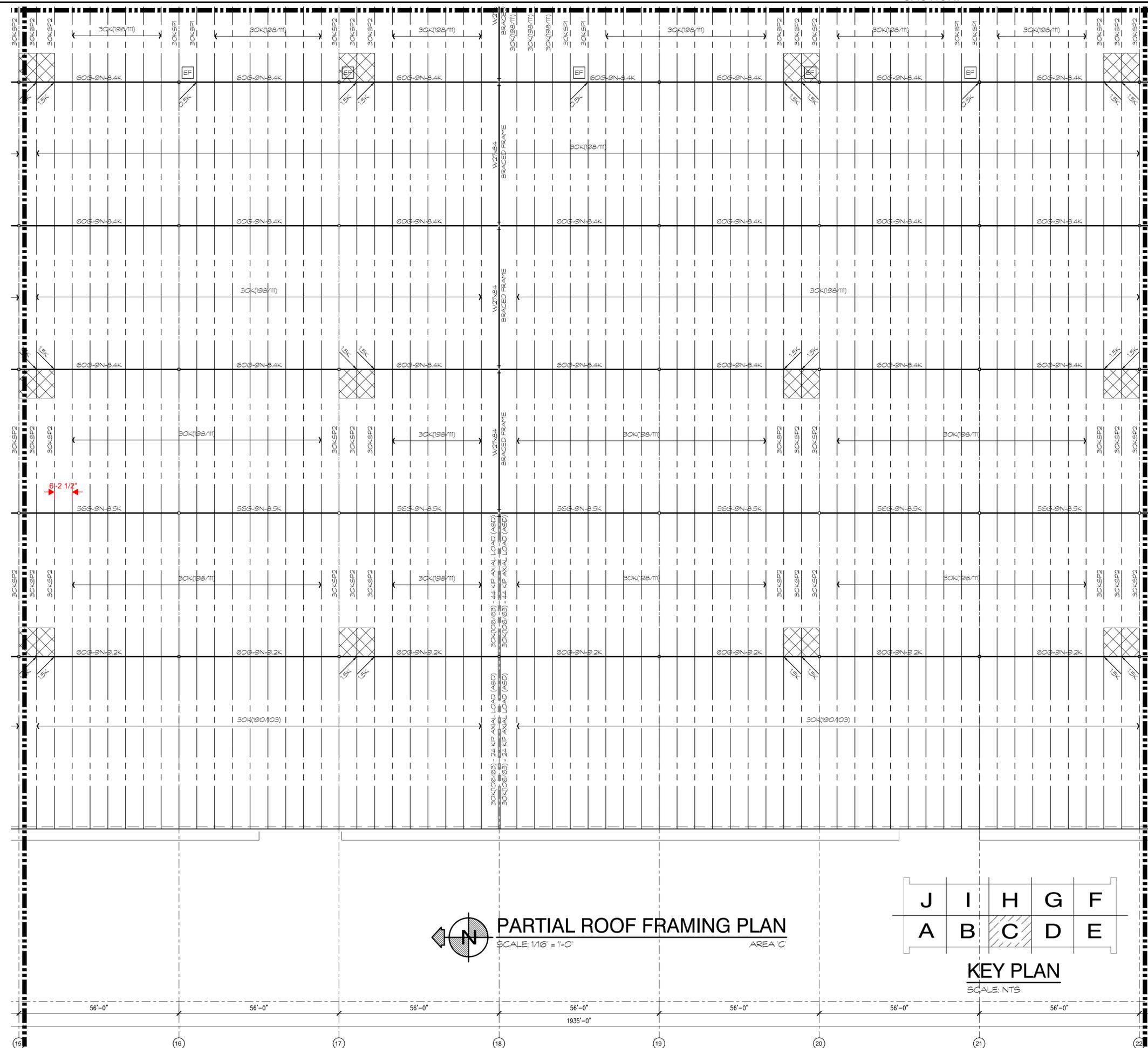
Avanti Project No. 21-177

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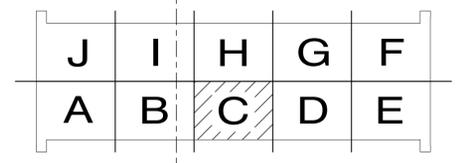
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PARTIAL ROOF FRAMING PLAN
SCALE: 1/16" = 1'-0" AREA 'C'



KEY PLAN
SCALE: N.T.S.

STONEMONT FT PIERCE
BLDG. 1
ORANGE AVENUE
FORT PIERCE, FLORIDA 34945

PARTIAL ROOF FRAMING PLAN AREA 'C'	
DATE	REMARKS
07-13-2022	ISSUED FOR PERMIT

PA/PM:	DMS
DRAWN BY.:	DNR
JOB NO.:	MIA21-0040-00

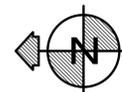
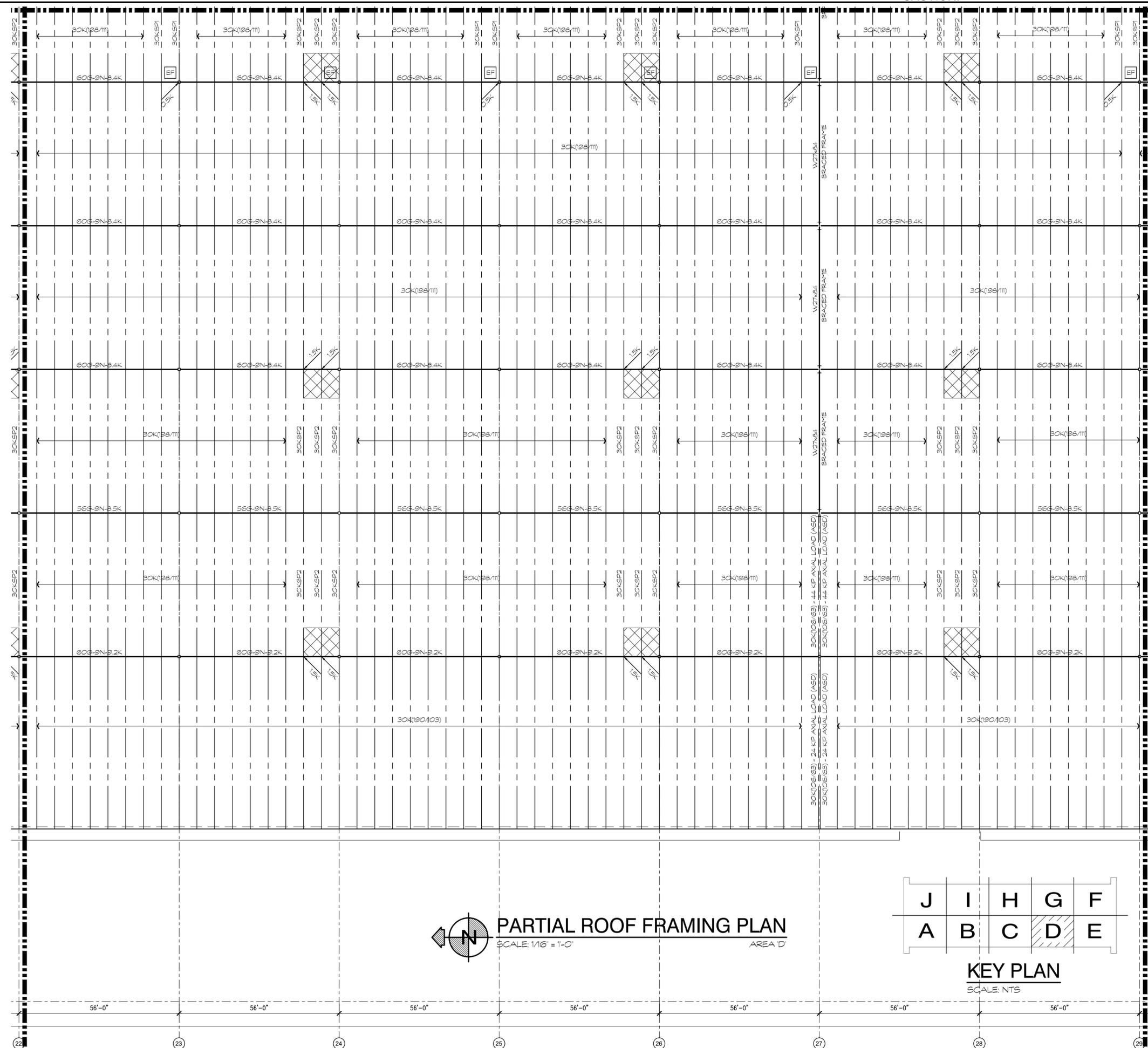
SHEET
S100-203

Avanti Project No. 21-177

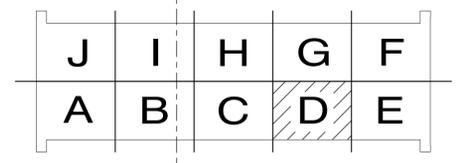
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PARTIAL ROOF FRAMING PLAN
SCALE: 1/16" = 1'-0" AREA D



KEY PLAN
SCALE: NTS

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BLDG. 1
ORANGE AVENUE
FORT PIERCE, FLORIDA 34945

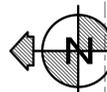
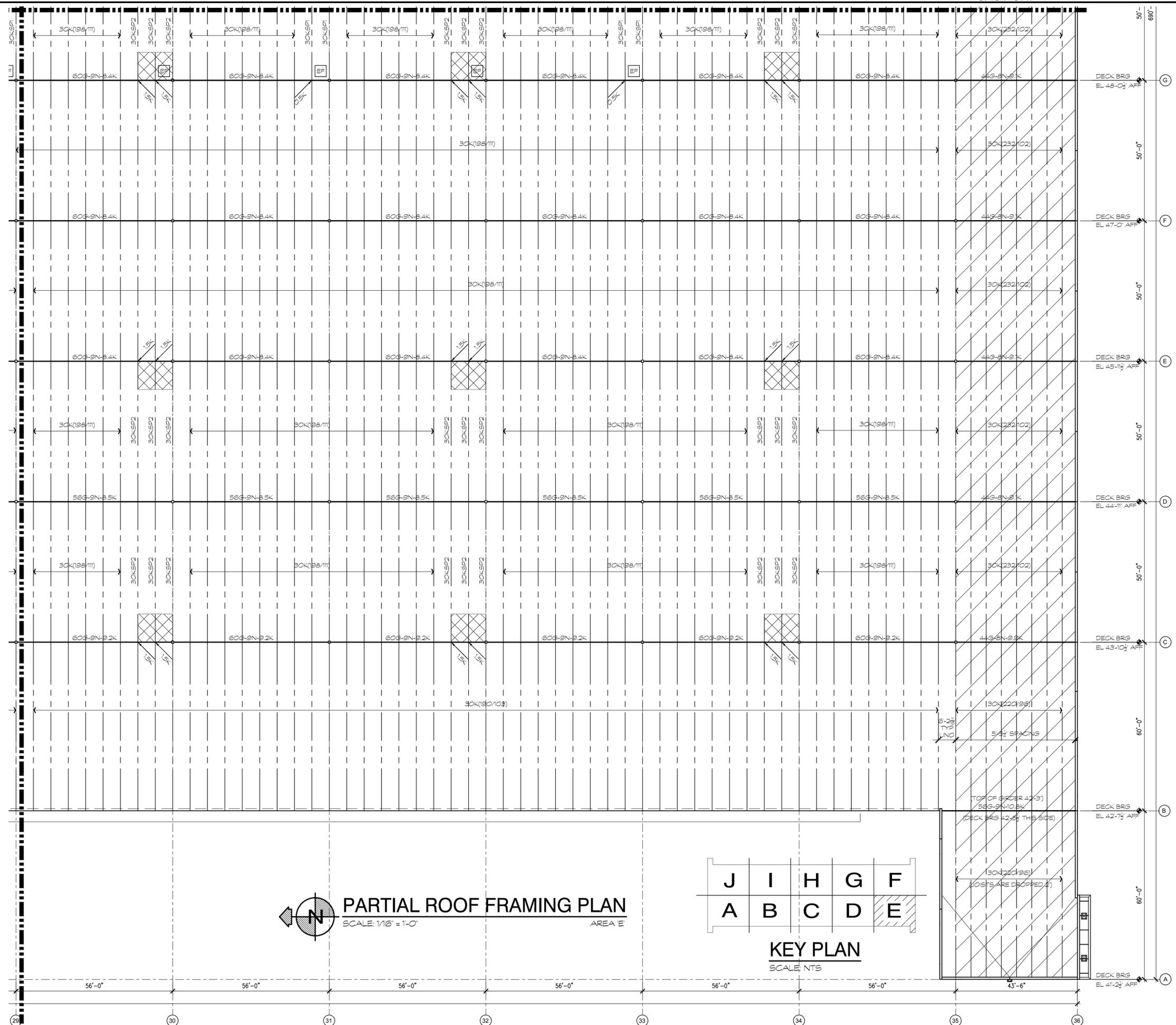
PARTIAL ROOF FRAMING PLAN AREA 'D'

DATE	REMARKS
07-13-2022	ISSUED FOR PERMIT

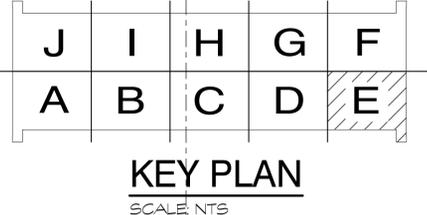
PA/PM:	DMS
DRAWN BY.:	DNR
JOB NO.:	MIA21-0040-00

SHEET
S100-204

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PARTIAL ROOF FRAMING PLAN
SCALE: 1/16" = 1'-0"



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BLDG. 1
ORANGE AVENUE
FORT PIERCE, FLORIDA 34945

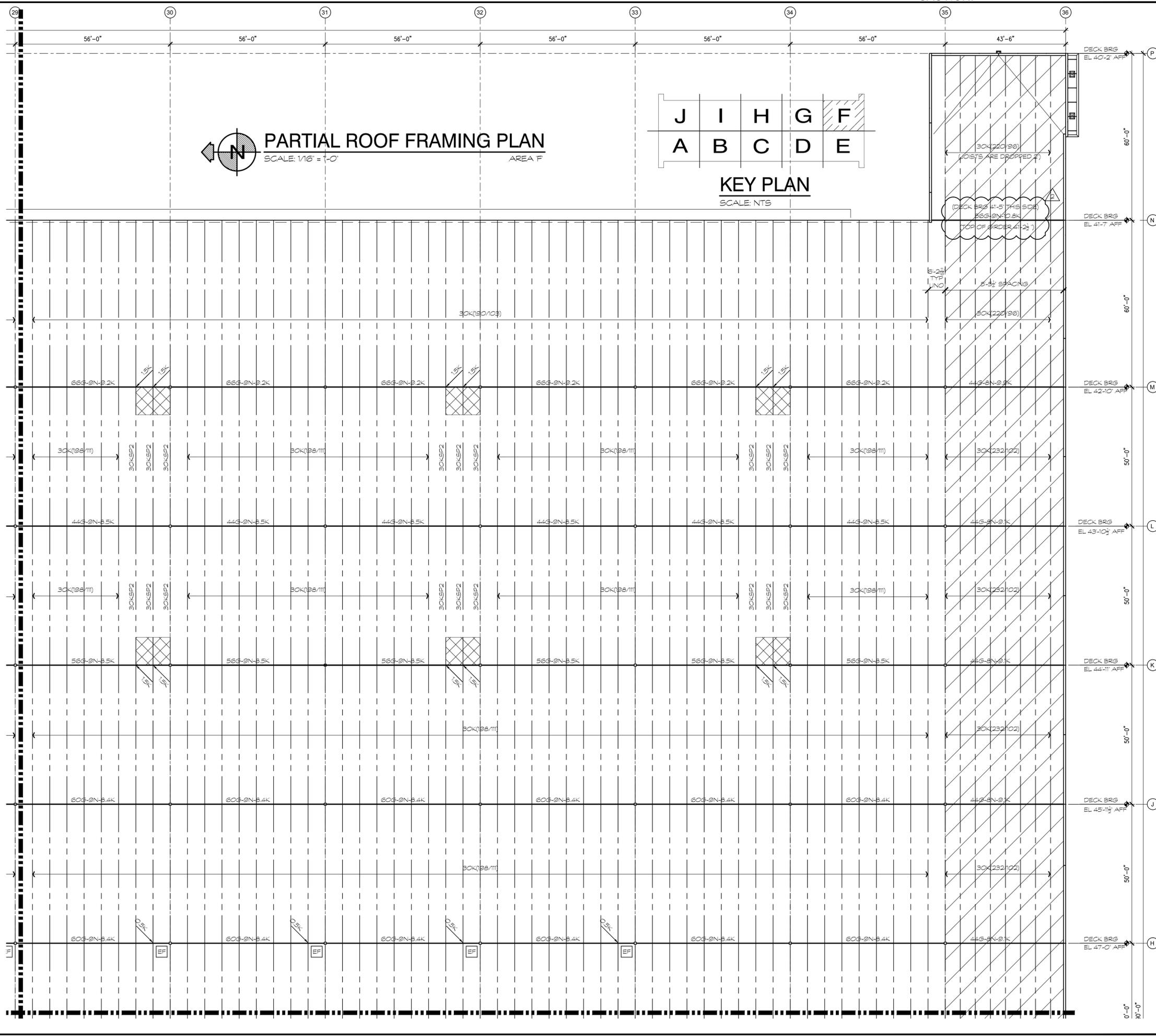
PARTIAL ROOF FRAMING PLAN AREA 'E'

DATE	REMARKS
07-13-2022	ISSUED FOR PERMIT

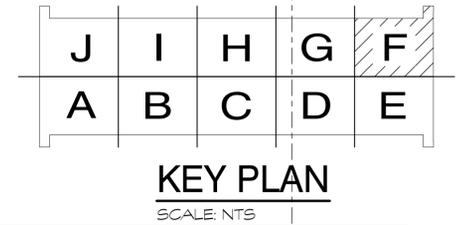
PAI/PM:	DMS
DRAWN BY.:	DNR
JOB NO.:	MIA21-0040-00

SHEET
S100-205

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PARTIAL ROOF FRAMING PLAN
SCALE: 1/16" = 1'-0"
AREA F



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ORANGE AVENUE
FORT PIERCE, FLORIDA 34945

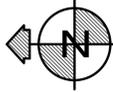
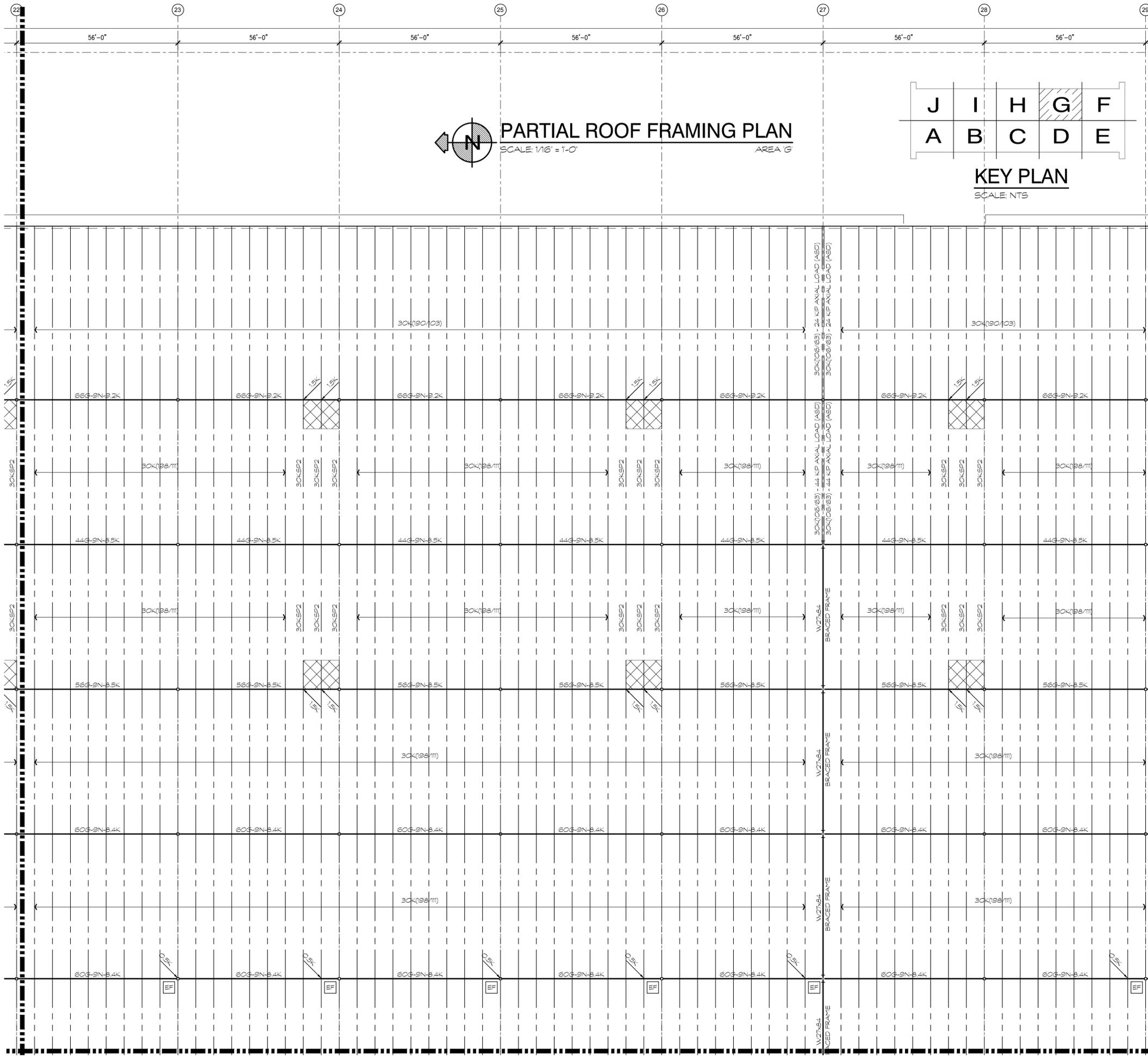
PARTIAL ROOF FRAMING PLAN AREA 'F'

DATE	REMARKS
07-13-2022	ISSUED FOR PERMIT
09-02-2022	CLIENT COORDINATION

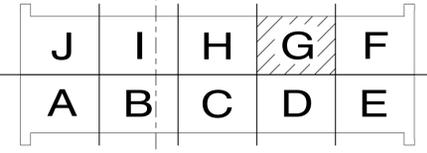
PA/PM:	DMS
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SHEET
S100-206

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PARTIAL ROOF FRAMING PLAN
SCALE: 1/16" = 1'-0"
AREA 'G'



KEY PLAN
SCALE: NTS

Avanti Project No. 21-177

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FLORIDA CERTIFICATE OF AUTHORIZATION No. 24537

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BLDG. 1
ORANGE AVENUE
FORT PIERCE, FLORIDA 34945

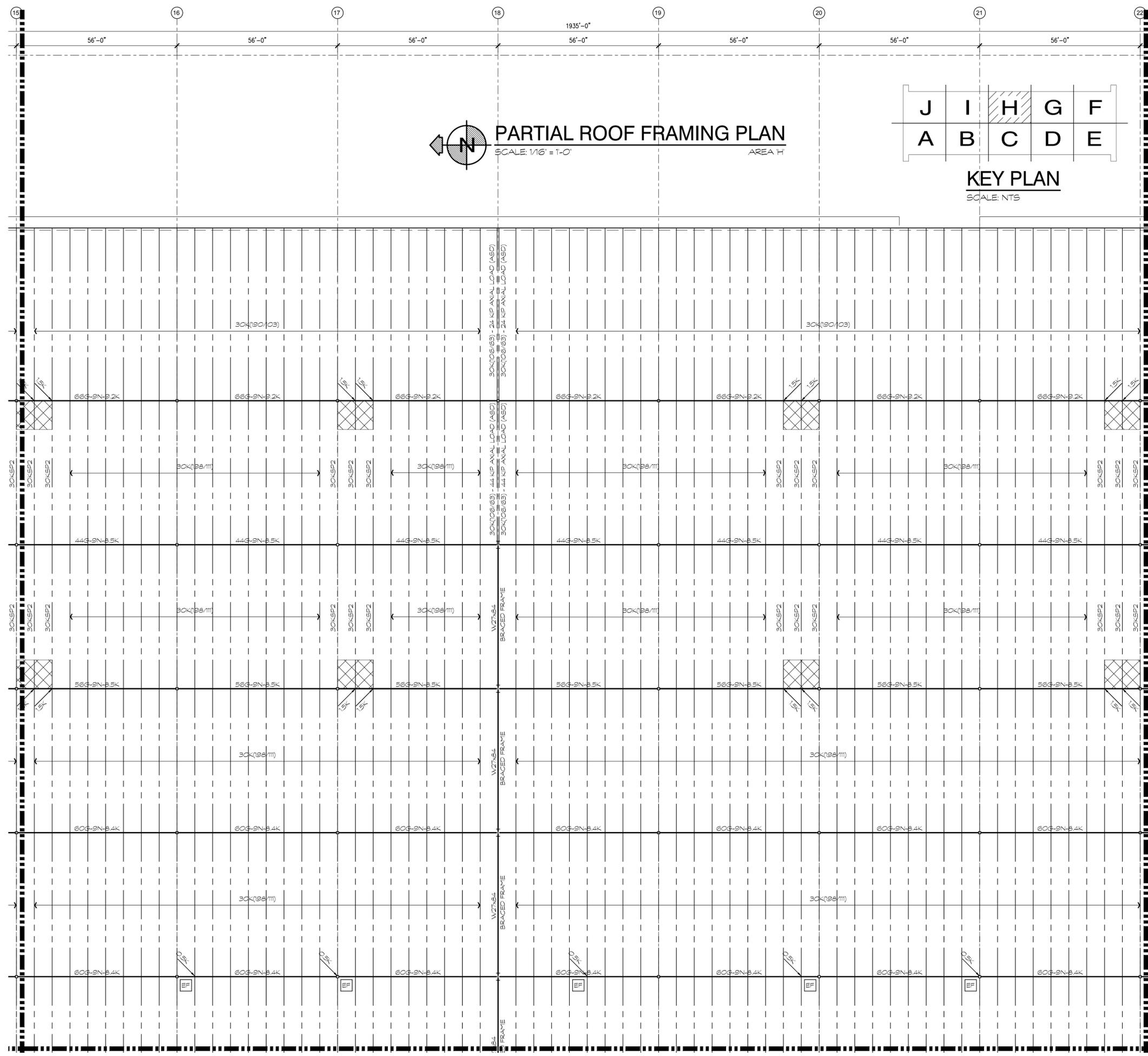
PARTIAL ROOF FRAMING PLAN AREA 'G'

DATE	REMARKS
07-13-2022	ISSUED FOR PERMIT

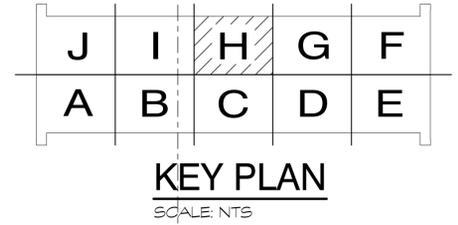
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JOB NO.:	MIA21-0040-00

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S100-207

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PARTIAL ROOF FRAMING PLAN
 SCALE: 1/16" = 1'-0"
 AREA 'H'



STONEMONT FT PIERCE
BLDG. 1
 ORANGE AVENUE
 FORT PIERCE, FLORIDA 34945

PARTIAL ROOF FRAMING
PLAN AREA 'H'

DATE	REMARKS
07-13-2022	ISSUED FOR PERMIT

PA/PM:	DMS
DRAWN BY.:	DNR
JOB NO.:	MIA21-0040-00

SHEET
S100-208

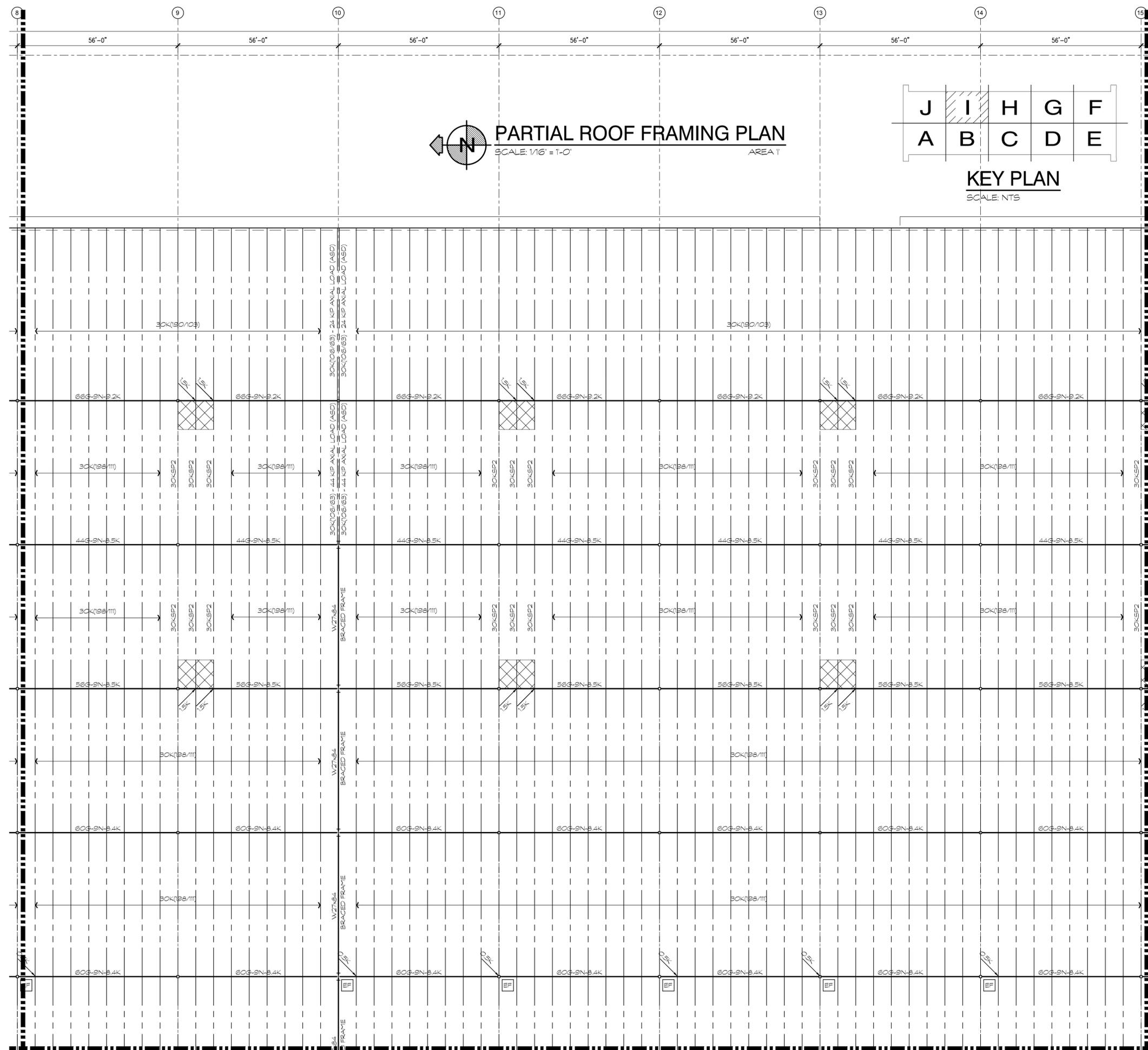
Avanti Project No. 21-177

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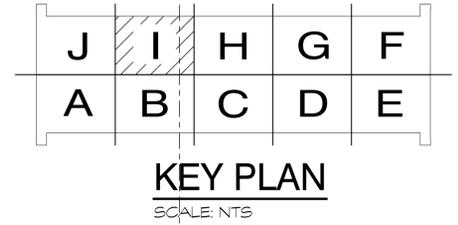
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PARTIAL ROOF FRAMING PLAN
 SCALE: 1/16" = 1'-0"
 AREA T



STONEMONT FT PIERCE
BLDG. 1
 ORANGE AVENUE
 FORT PIERCE, FLORIDA 34945

PARTIAL ROOF FRAMING PLAN AREA 'I'	
DATE	REMARKS
07-13-2022	ISSUED FOR PERMIT

PA/PM:	DMS
DRAWN BY.:	DNR
JOB NO.:	MIA21-0040-00

SHEET
S100-209

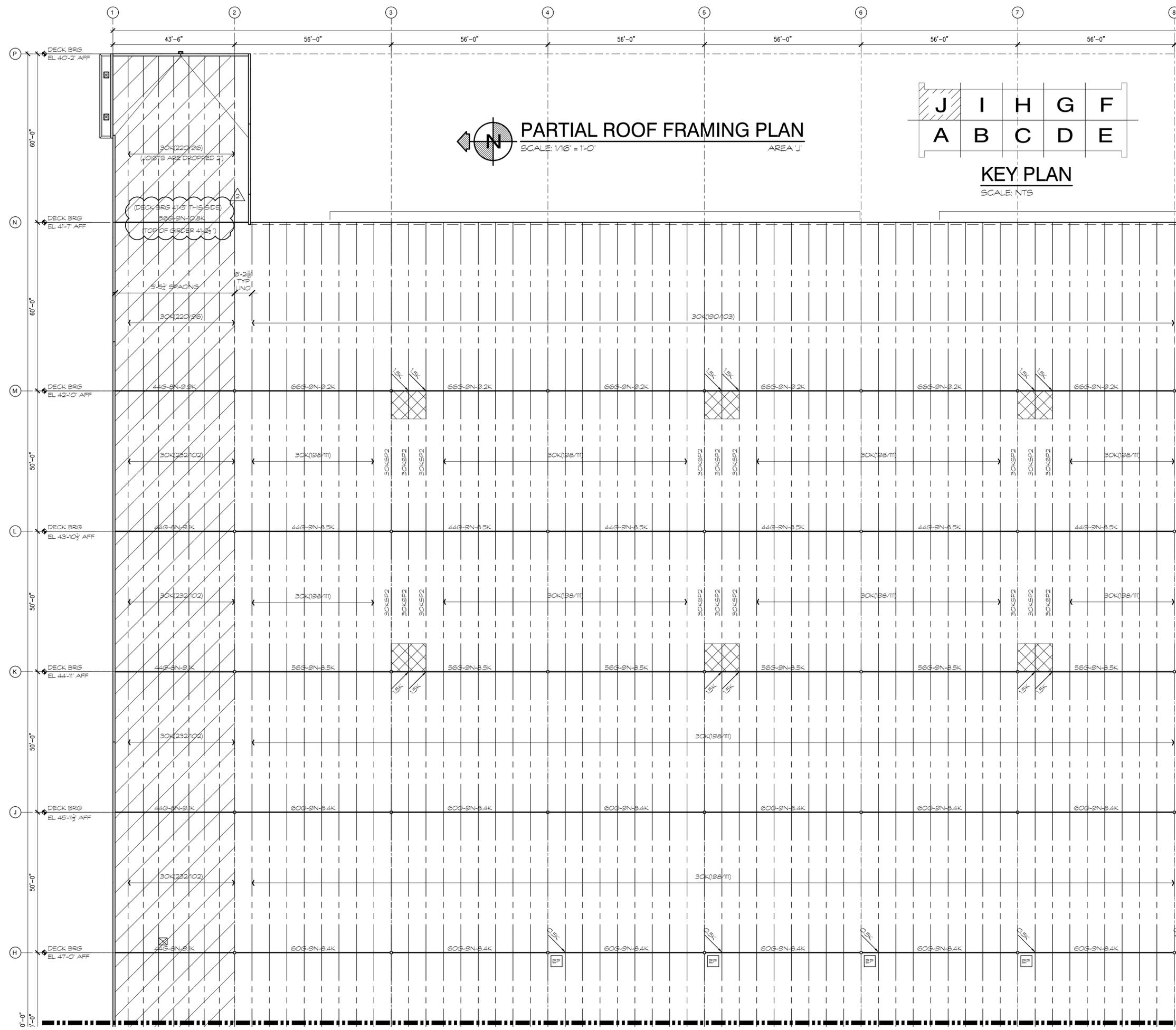
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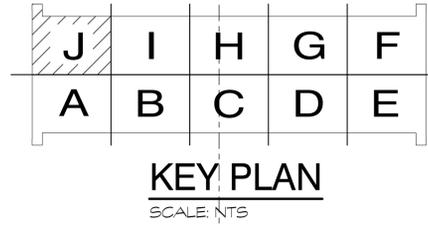
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PARTIAL ROOF FRAMING PLAN
SCALE: 1/16" = 1'-0"
AREA J



STONEMONT FT PIERCE
BLDG. 1
ORANGE AVENUE
FORT PIERCE, FLORIDA 34945

PARTIAL ROOF FRAMING PLAN AREA J	
DATE	REMARKS
07-13-2022	ISSUED FOR PERMIT
09-02-2022	CLIENT COORDINATION

PA/PM:	DMS
DRAWN BY.:	DNR
JOB NO.:	MIA21-0040-00

SHEET
S100-210

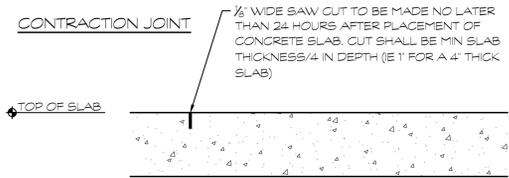
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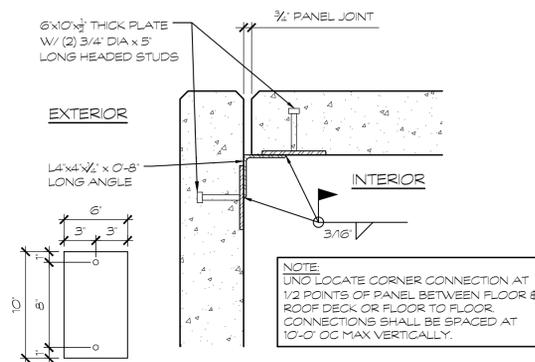
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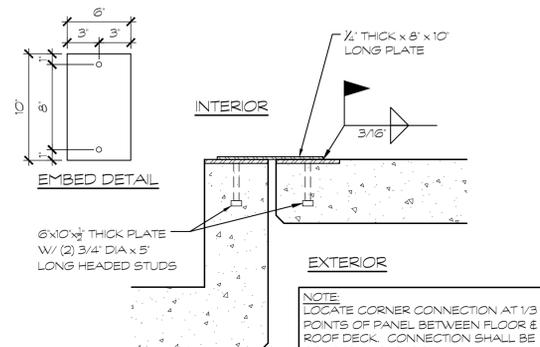
FLORIDA CERTIFICATE OF AUTHORIZATION No. 24537



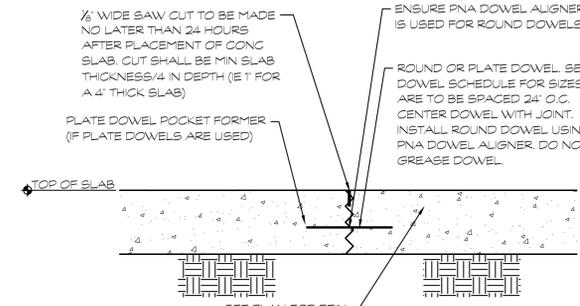
1 CONTRACTION JOINT SCALE: 1 1/2" = 1'-0"



2 CORNER CONNECTION @ PANEL BUTTED CORNER SCALE: 1 1/2" = 1'-0"



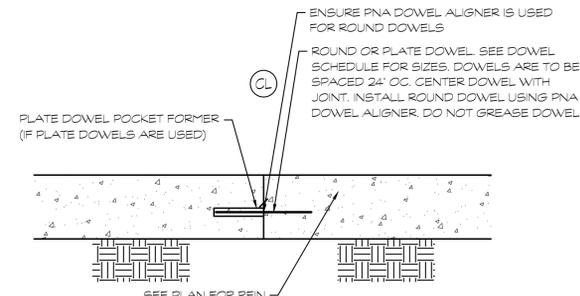
3 TILT WALL CORNER CONNECTION INSIDE BUTTED CORNER SCALE: 1 1/2" = 1'-0"



4 FREE CONTRACTION JOINT OWNER OPTION SCALE: 1 1/2" = 1'-0"

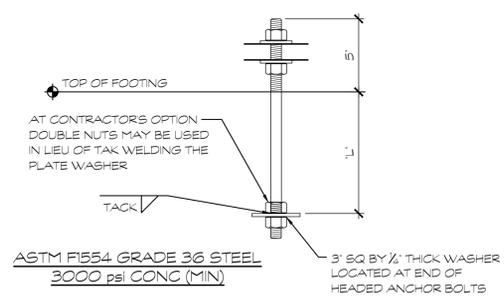
SLAB DEPTH (N)	DIAMETER (N)	TOTAL LENGTH (N)	ALTERNATE PLATE DOWELS (N)
5	5/8	10	1/4 x 4 1/2 x 4 1/2
6	3/4	10	1/4 x 4 1/2 x 4 1/2
7	7/8	13	3/8 x 4 1/2 x 4 1/2
8	1	13	3/8 x 4 1/2 x 4 1/2
9	1 1/8	15	3/4 x 4 1/2 x 4 1/2
10	1 1/4	15	3/4 x 4 1/2 x 4 1/2

NOTE: PLATE DOWELS ARE TO BE DIAMOND DOWELS BY PNA.

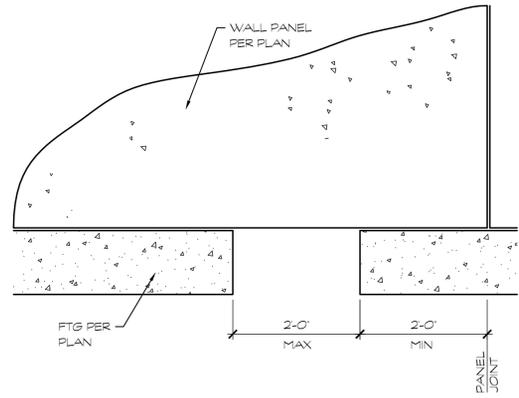


5 CONSTRUCTION JOINT IF APPLICABLE SCALE: 1 1/2" = 1'-0"

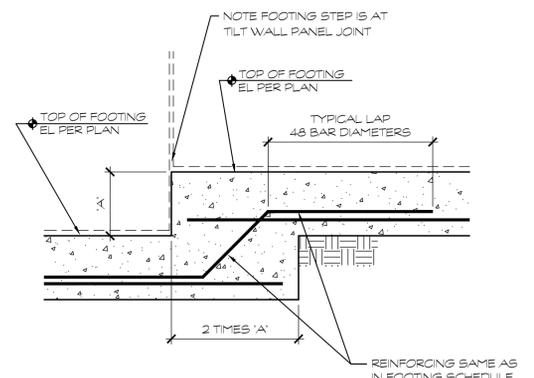
COLUMN MARK	ANCHOR BOLT	LENGTH (L)	REMARKS
C1	(4) - 3/4" DIA	9'	
C2	(4) - 1" DIA	12'	



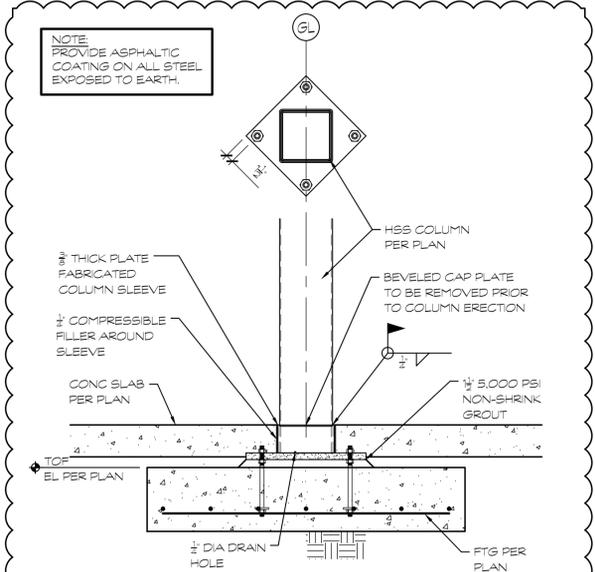
6 ANCHOR BOLT SCHEDULE SCALE: NTS



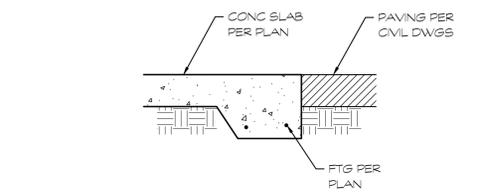
7 PLUMBING CHASE AT FOOTING SCALE: 3/4" = 1'-0"



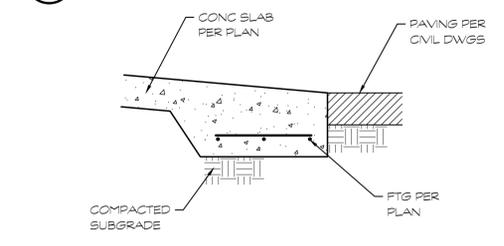
8 FOOTING STEP AT PANEL SCALE: 3/4" = 1'-0"



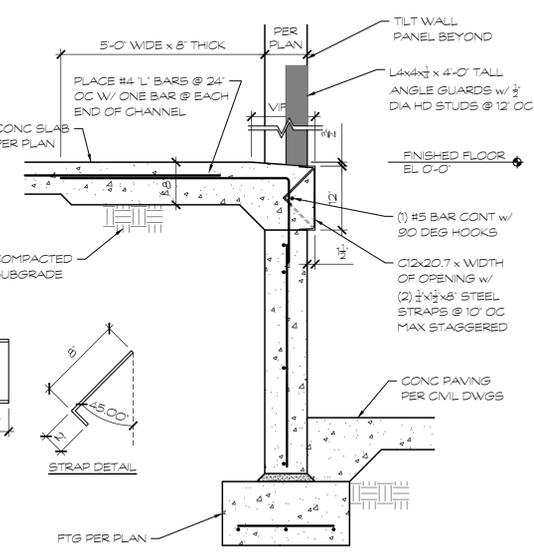
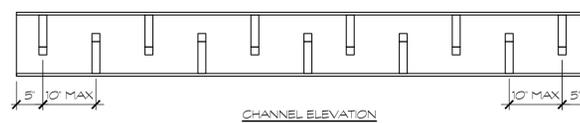
9 COLUMN FOOTING @ SLEEVE SCALE: 3/4" = 1'-0"



10 SLAB EDGE SCALE: 3/4" = 1'-0"



11 RAMP EDGE SCALE: 3/4" = 1'-0"



12 EDGE OF DOCK CHANNEL SCALE: 3/4" = 1'-0"

SLAB DEPTH (N)	DIAMETER (N)	TOTAL LENGTH (N)	PLATE DOWELS (N)
5	5/8	10	1/4 x 4 1/2 x 4 1/2
6	3/4	10	1/4 x 4 1/2 x 4 1/2
7	7/8	13	3/8 x 4 1/2 x 4 1/2
8	1	13	3/8 x 4 1/2 x 4 1/2
9	1 1/8	15	3/4 x 4 1/2 x 4 1/2
10	1 1/4	15	3/4 x 4 1/2 x 4 1/2

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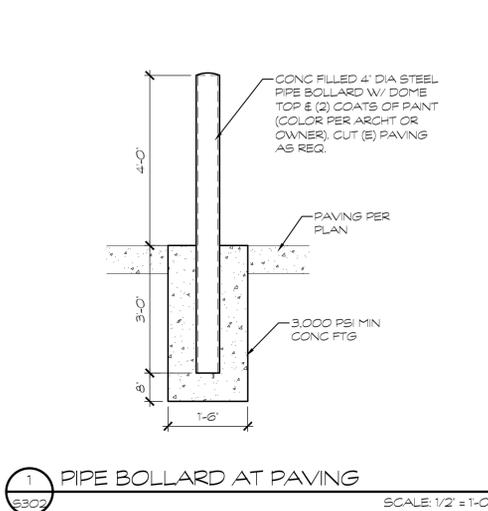
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FORT PIERCE, FLORIDA 34945

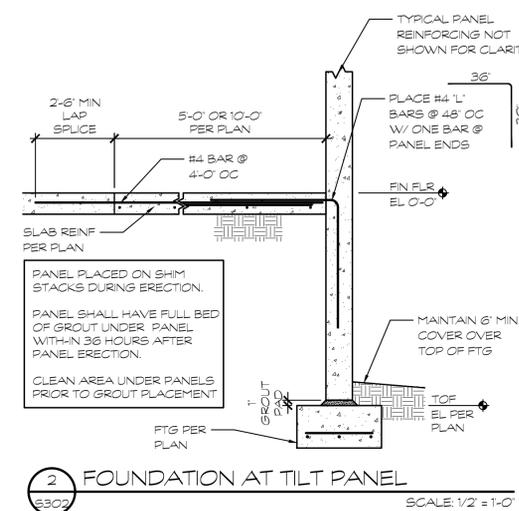
DATE	ISSUED FOR PERMIT	RFI RESPONSE
07-13-2022		
02-09-2023		

PA/PM:	DMS
DRAWN BY:	DNR
JOB NO.:	MIA21-0040-00

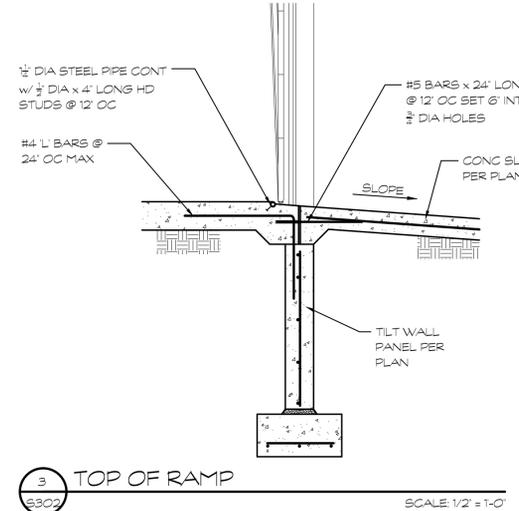
SHEET
S100-301



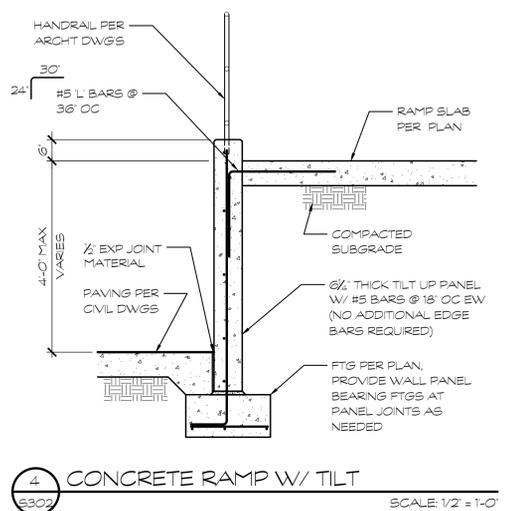
1 PIPE BOLLARD AT PAVING
SCALE: 1/2" = 1'-0"



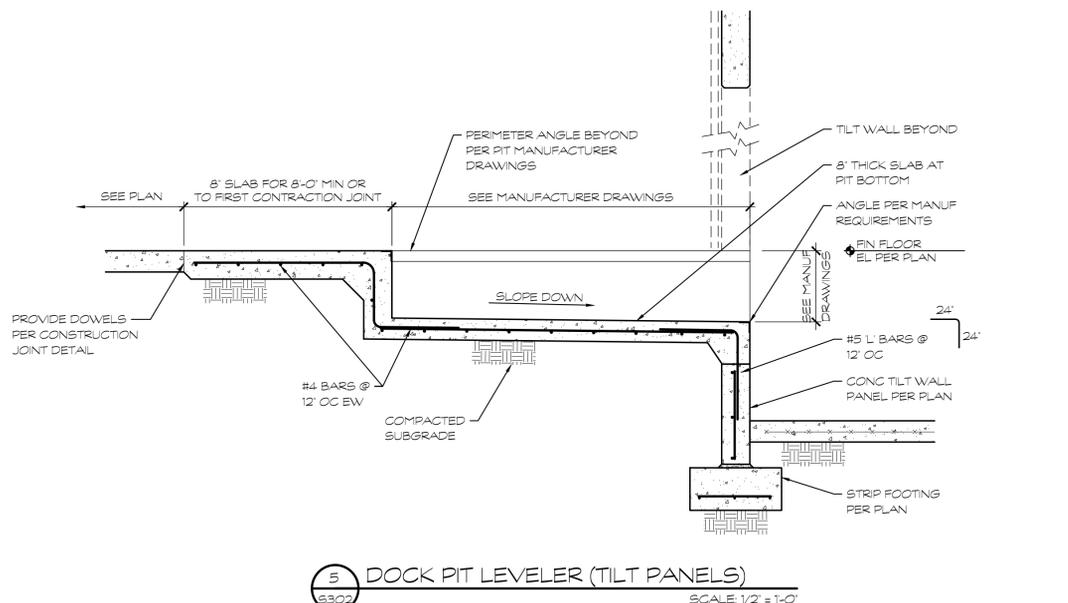
2 FOUNDATION AT TILT PANEL
SCALE: 1/2" = 1'-0"



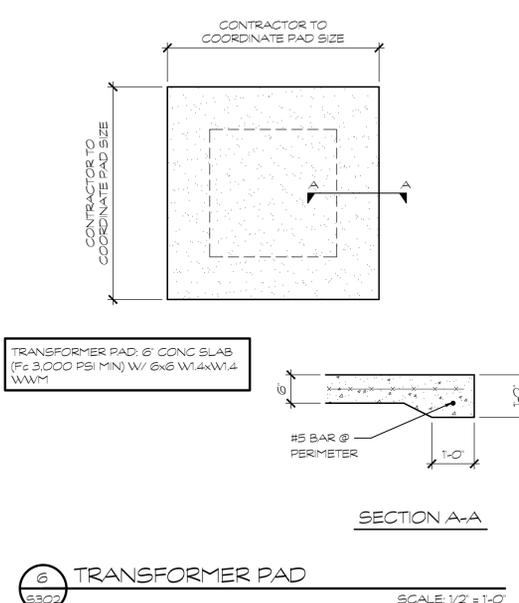
3 TOP OF RAMP
SCALE: 1/2" = 1'-0"



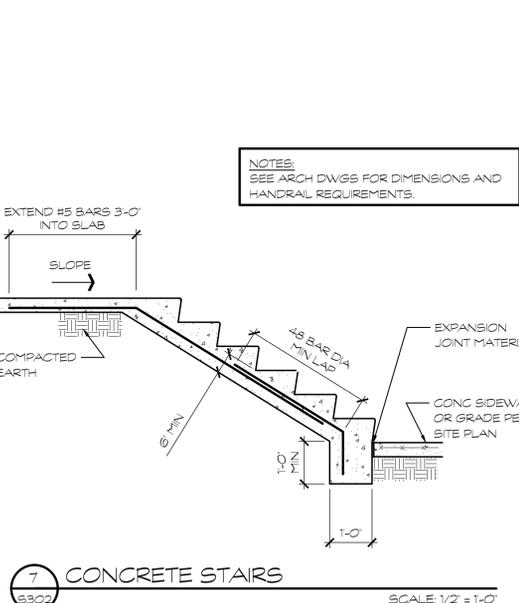
4 CONCRETE RAMP W/ TILT
SCALE: 1/2" = 1'-0"



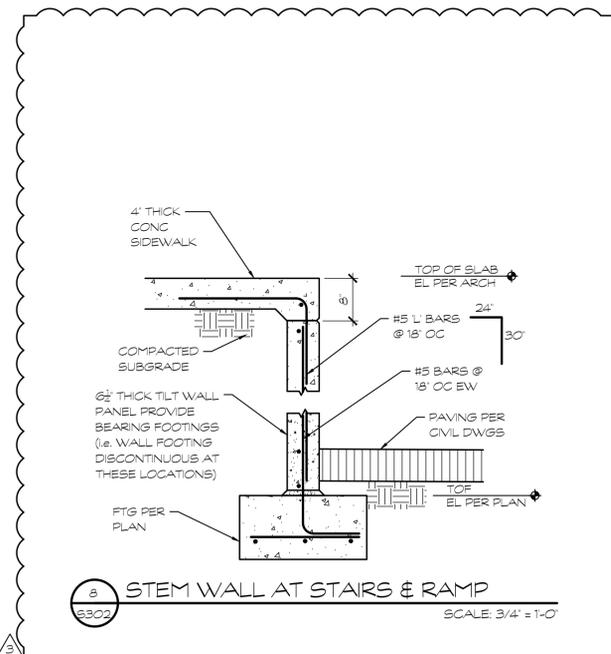
5 DOCK PIT LEVELER (TILT PANELS)
SCALE: 1/2" = 1'-0"



6 TRANSFORMER PAD
SCALE: 1/2" = 1'-0"



7 CONCRETE STAIRS
SCALE: 1/2" = 1'-0"



8 STEM WALL AT STAIRS & RAMP
SCALE: 3/4" = 1'-0"

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Avanti Project No. 21-177

STONEMONT FT PIERCE
BLDG. 1
ORANGE AVENUE
FORT PIERCE, FLORIDA 34945

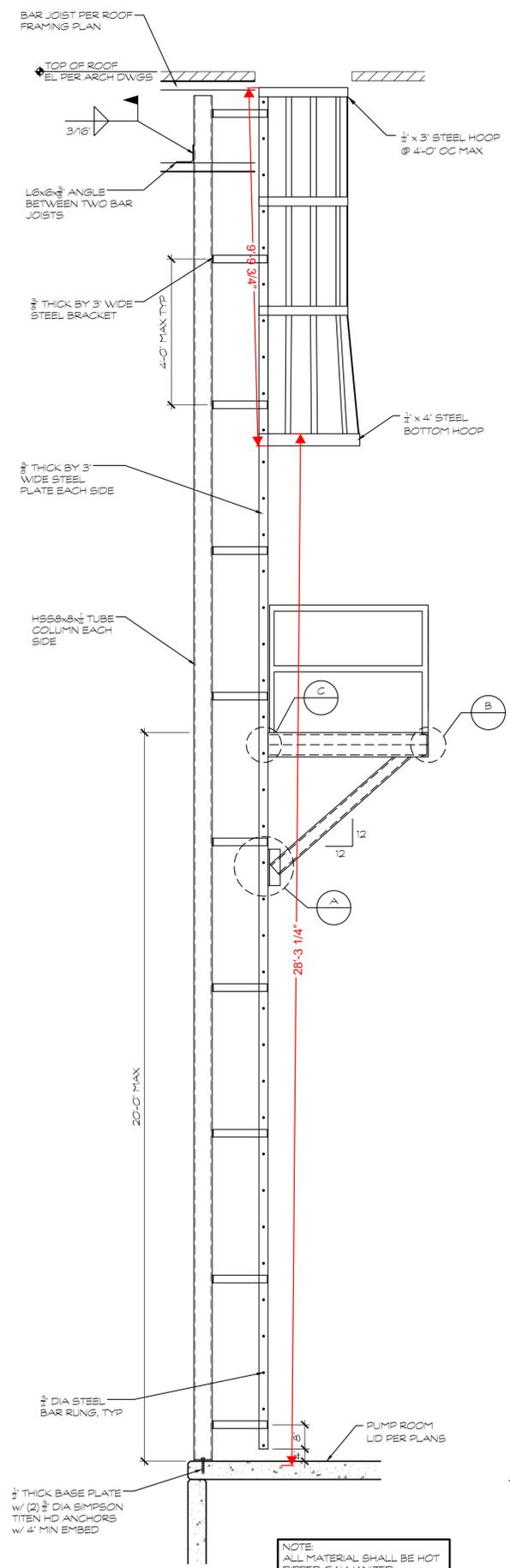
STRUCTURAL DETAILS

DATE	REMARKS
07-13-2022	ISSUED FOR PERMIT
09-08-2022	CLIENT COORDINATION
3	

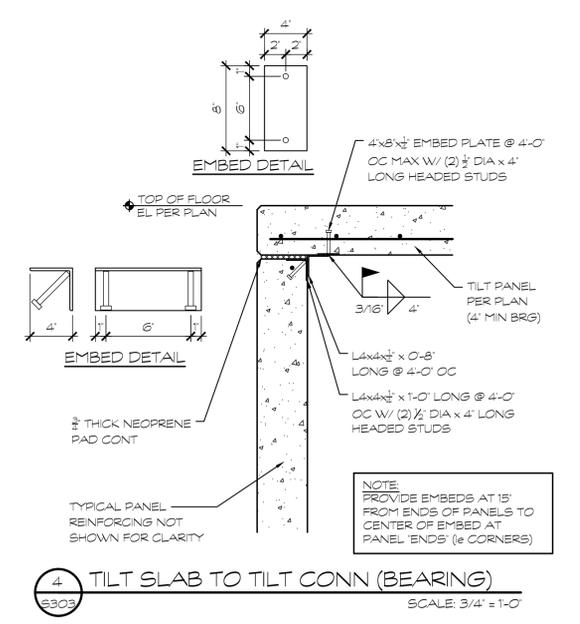
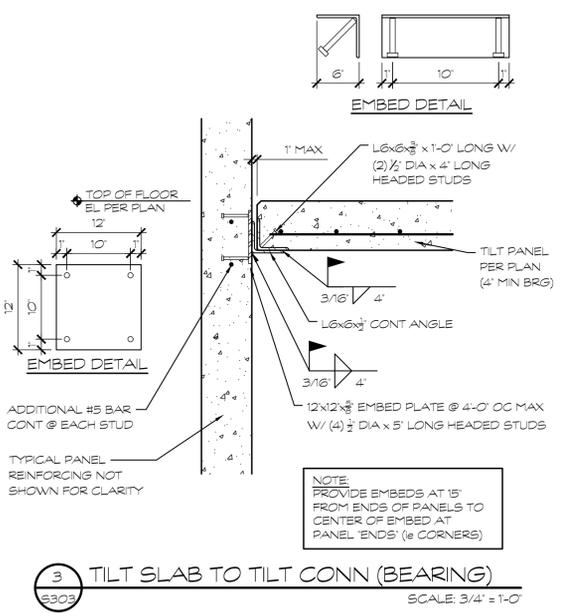
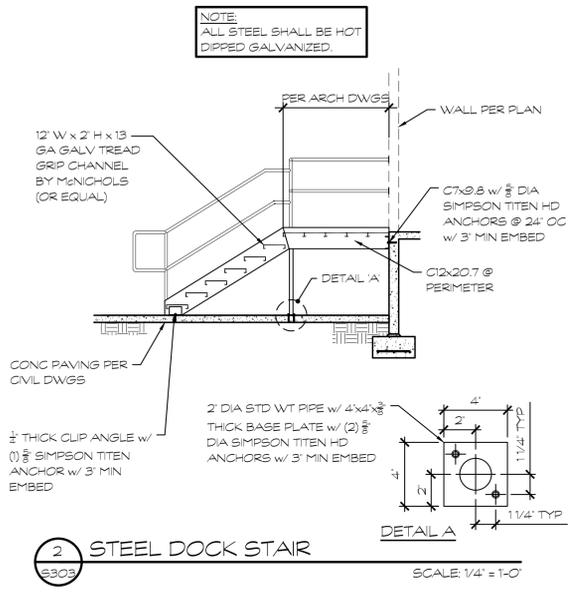
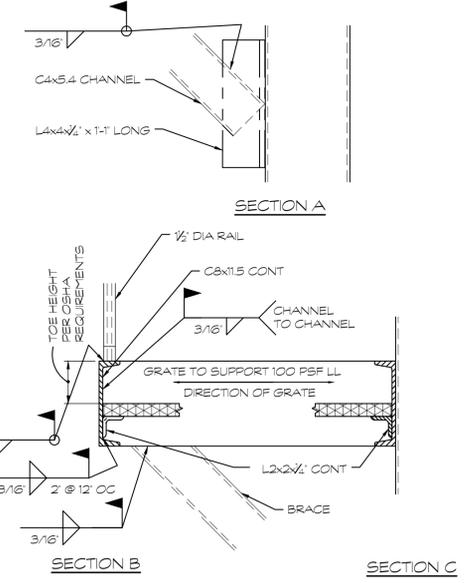
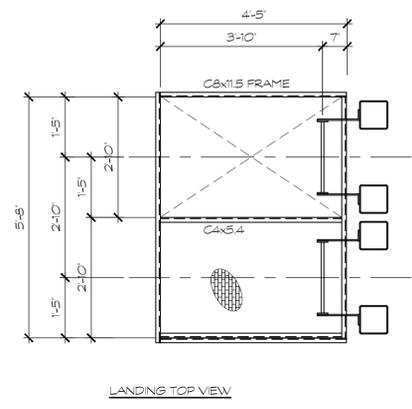
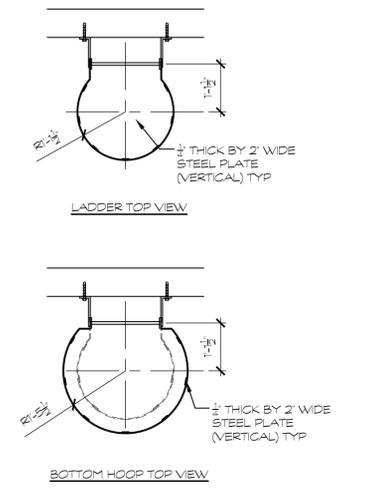
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SHEET
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1 CAGED LADDER
 1/2" LESS THAN 30 FT OVER LID
 SCALE: 1/2" = 1'-0"



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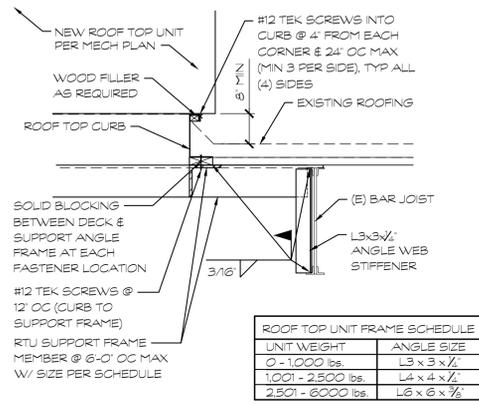
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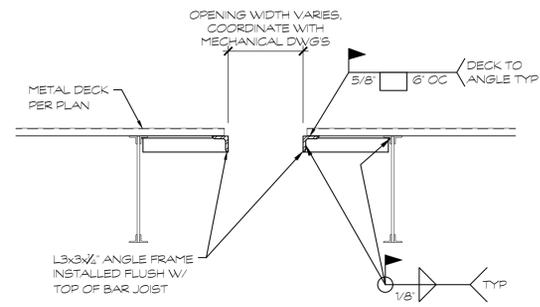
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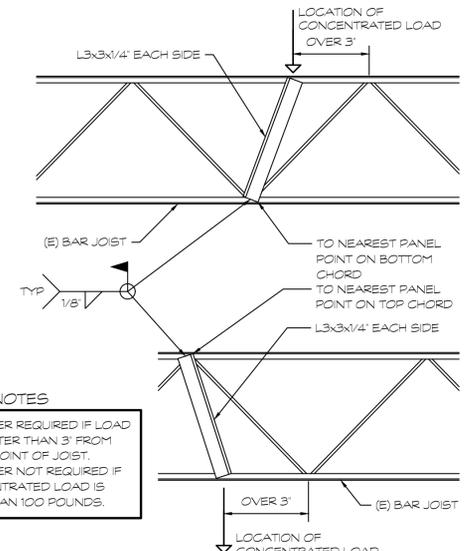
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1 RTU SUPPORT AND TIE DOWN
SCALE: 3/4" = 1'-0"

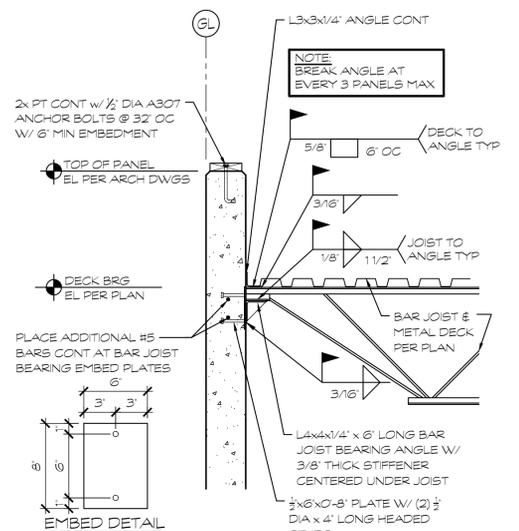


2 ROOF OPENING FRAMING
SCALE: 3/4" = 1'-0"

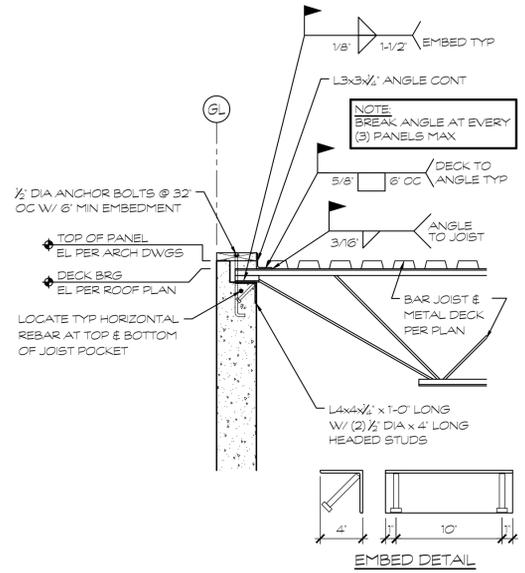


3 BAR JOIST STIFFENER REQUIREMENTS
SCALE: 3/4" = 1'-0"

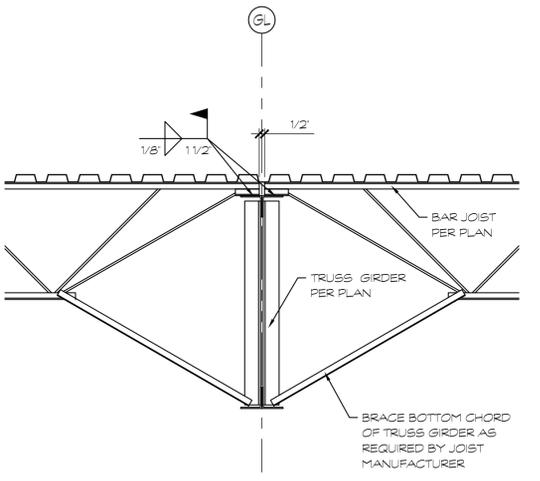
NOTES
1. STIFFENER REQUIRED IF LOAD IS GREATER THAN 3' FROM PANEL POINT OF JOIST.
2. STIFFENER NOT REQUIRED IF CONCENTRATED LOAD IS LESS THAN 100 POUNDS.



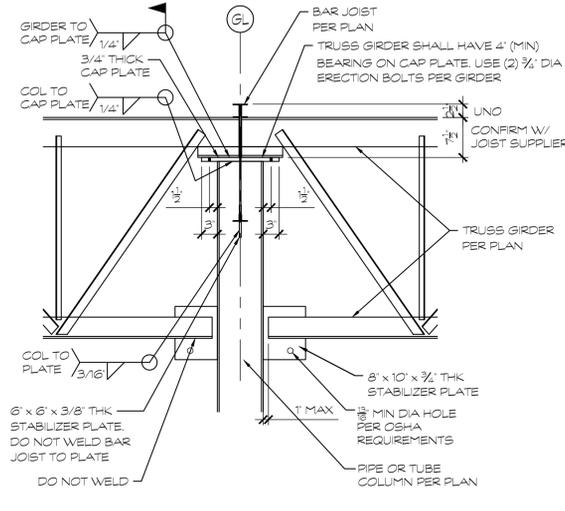
4 BAR JOIST TO TILT WALL @ PARAPET
SCALE: 3/4" = 1'-0"



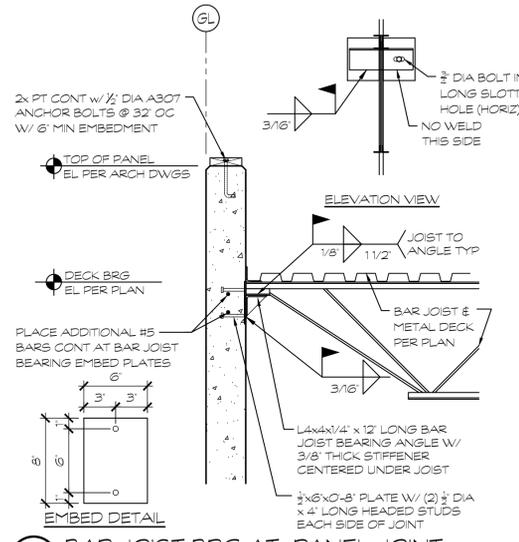
5 BAR JOIST TO TILT WALL @ EAVE
SCALE: 3/4" = 1'-0"



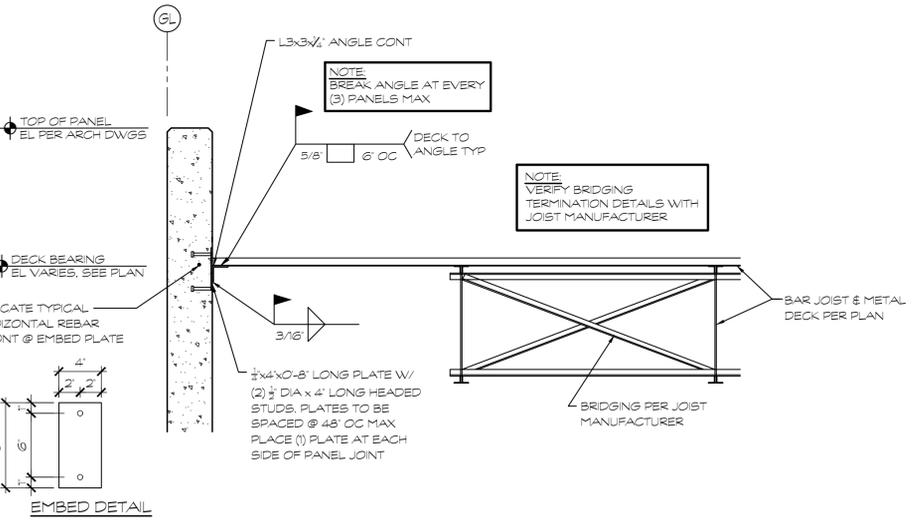
6 BAR JOIST TO TRUSS GIRDER CONN
SCALE: 3/4" = 1'-0"



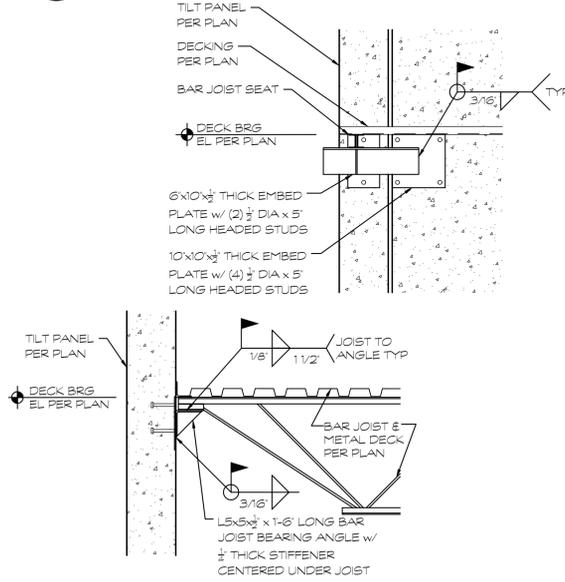
7 GIRDER TO COLUMN CONNECTION
SCALE: 3/4" = 1'-0"



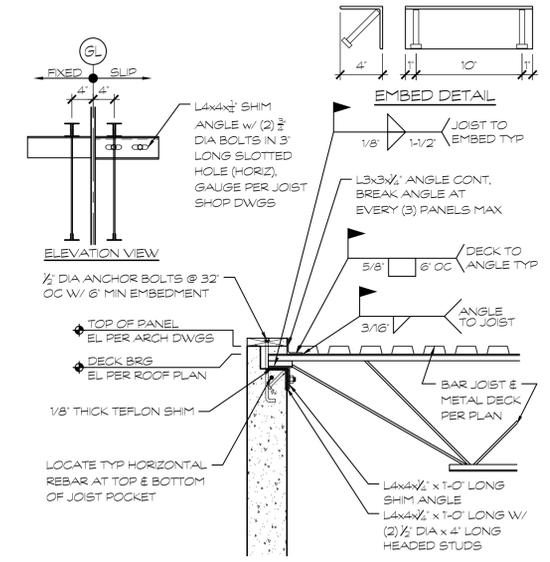
8 BAR JOIST BRG AT PANEL JOINT @ PARAPET
SCALE: 3/4" = 1'-0"



9 DECK BEARING AT WALL
SCALE: 3/4" = 1'-0"



10 JOIST BEARING AT PANEL EDGE
SCALE: 3/4" = 1'-0"



11 BAR JOIST TO TILT WALL @ EAVE @ EXPANSION JOINT
SCALE: 3/4" = 1'-0"

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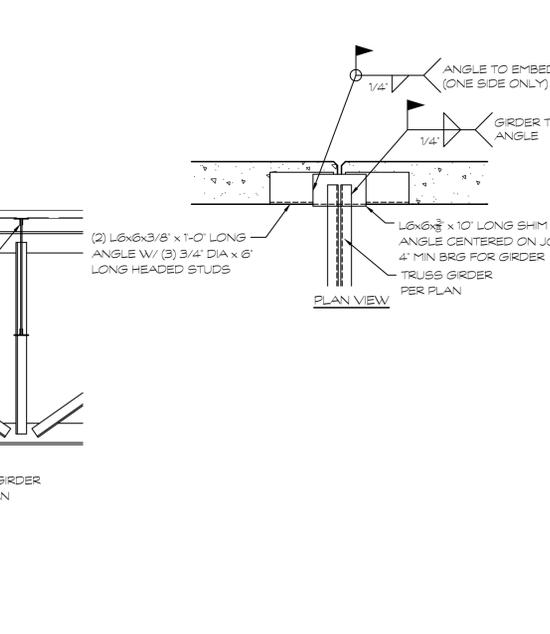
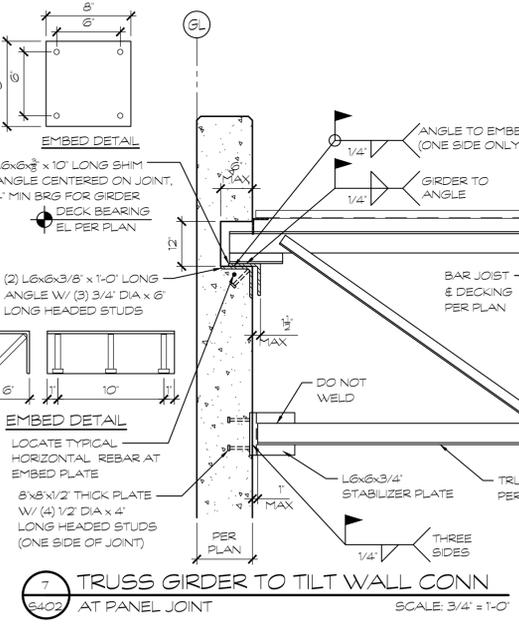
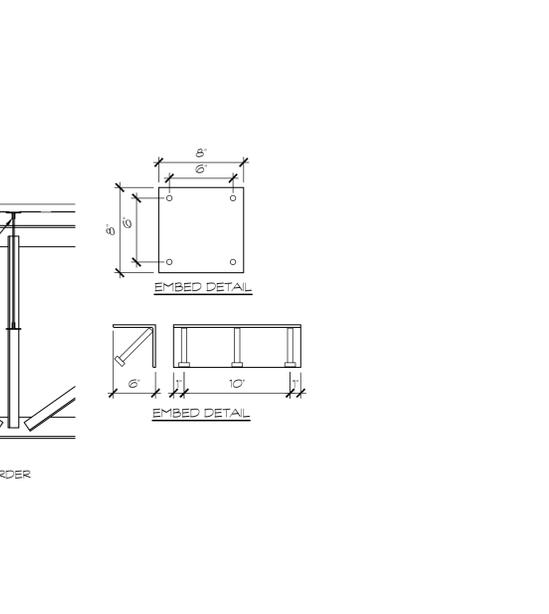
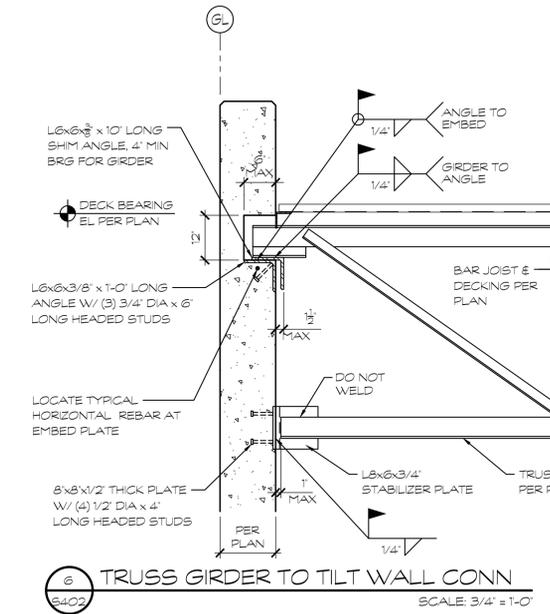
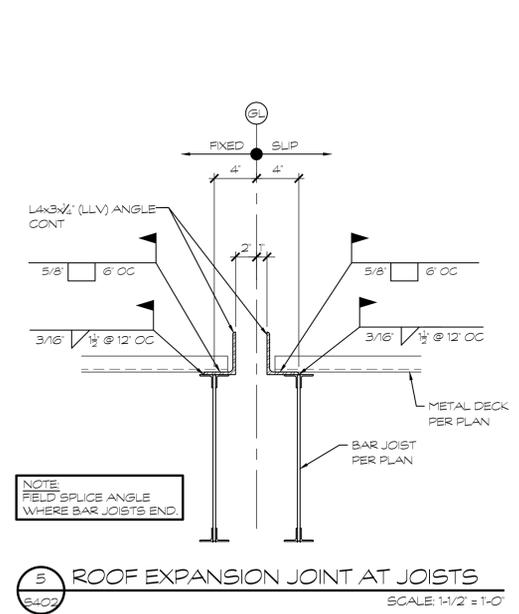
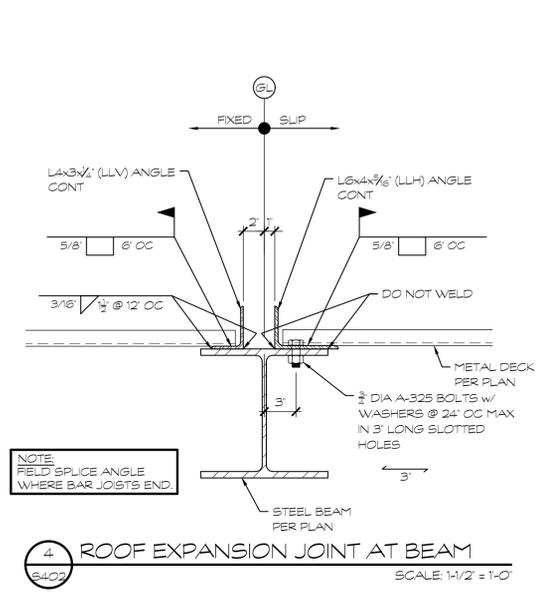
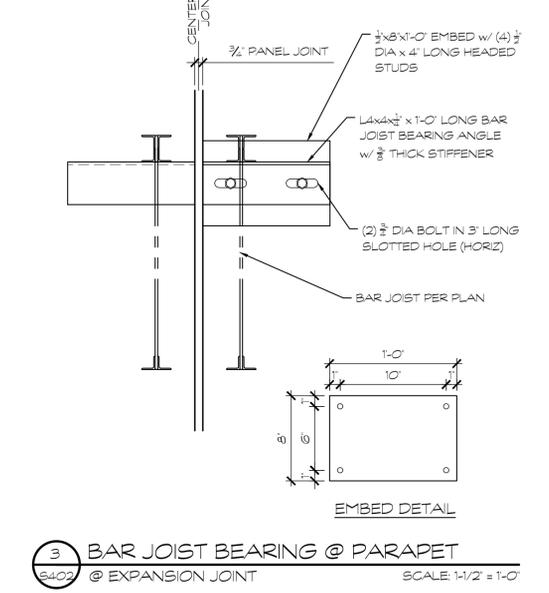
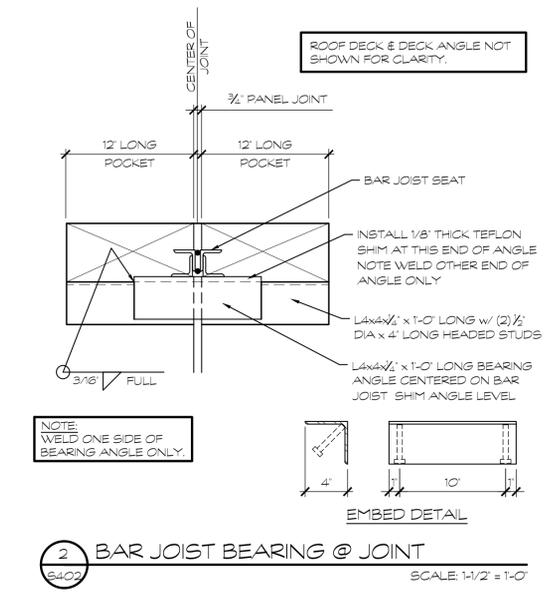
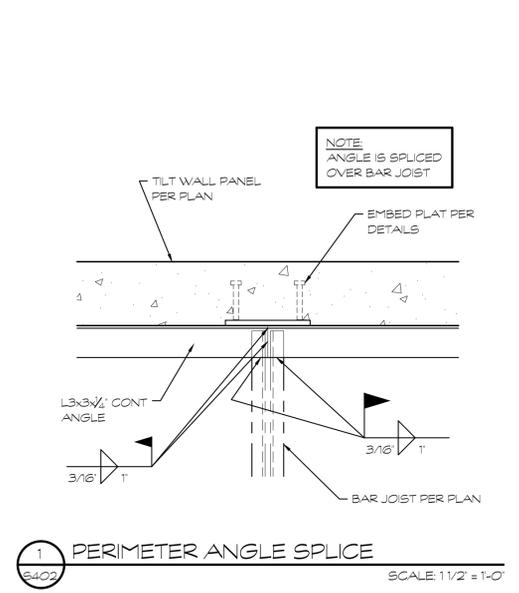
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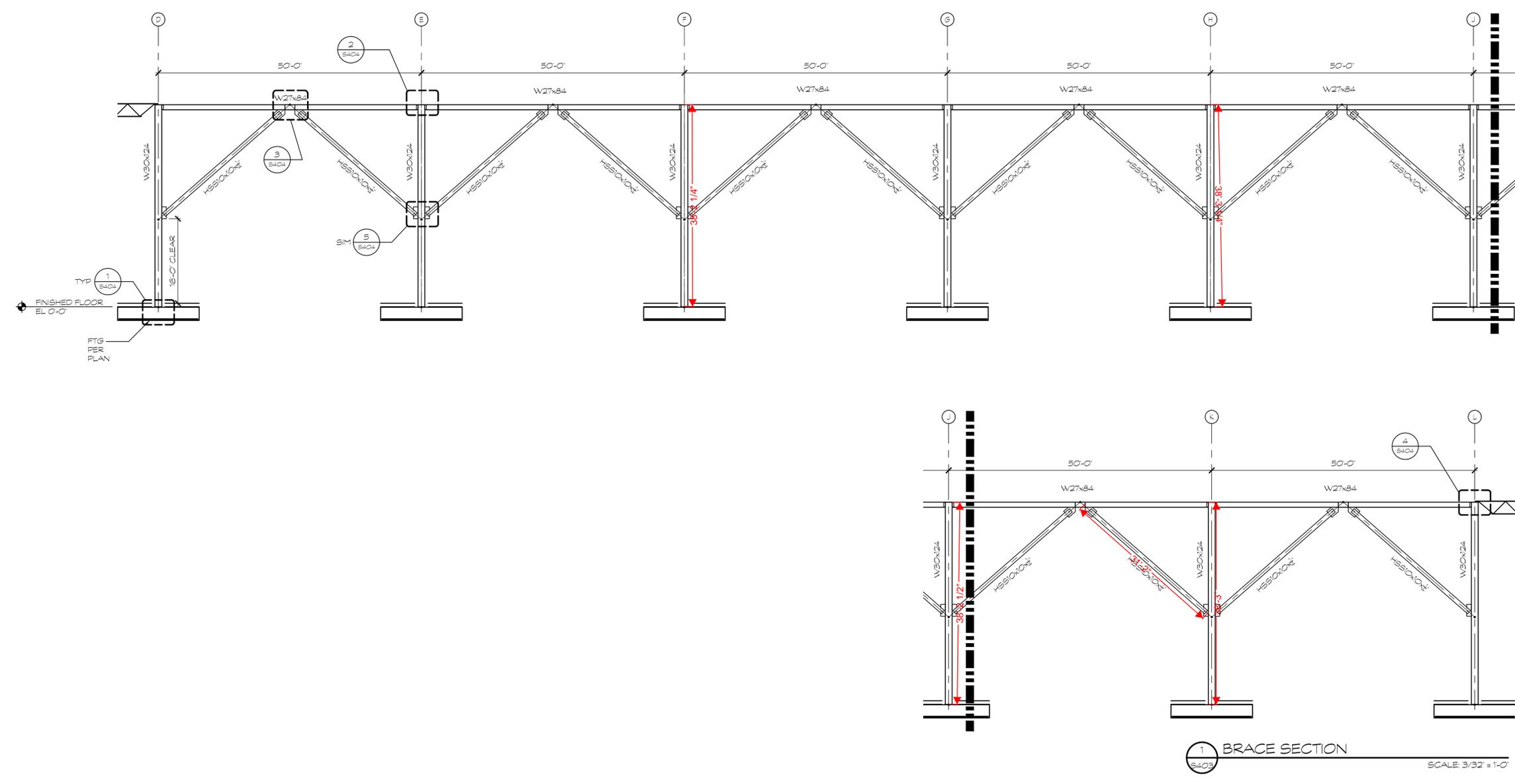
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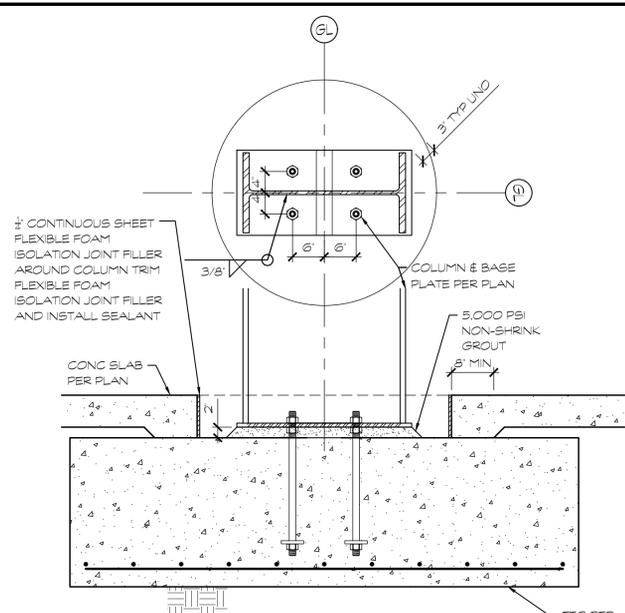
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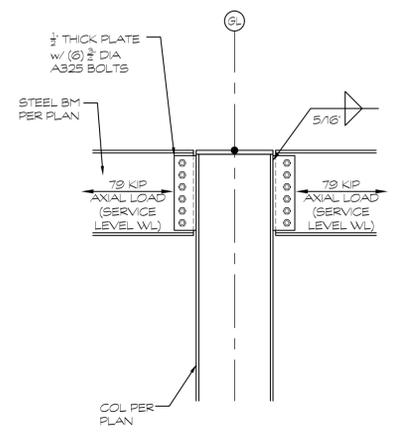
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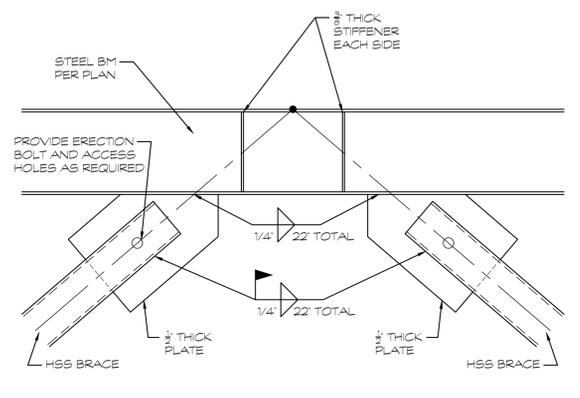
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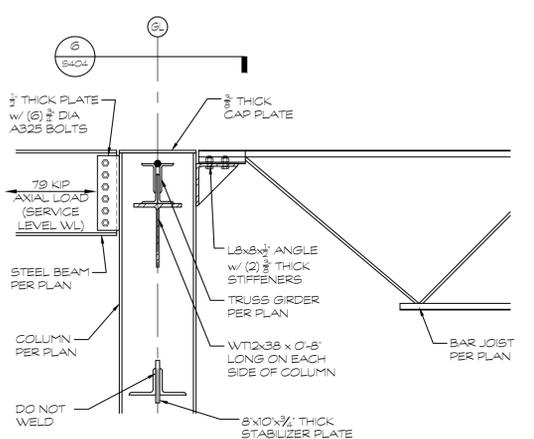
1 COLUMN FTG AT BRACED FRAME
SCALE: 3/4" = 1'-0"



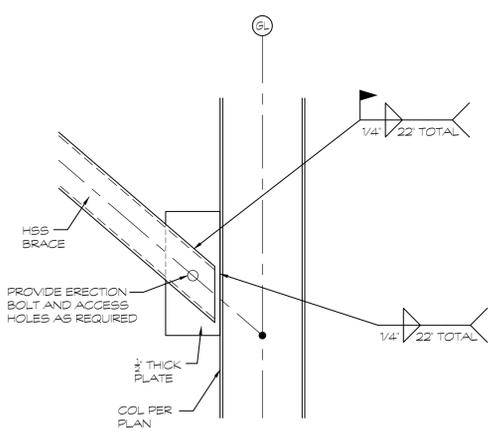
2 BEAM TO COLUMN
SCALE: 3/4" = 1'-0"



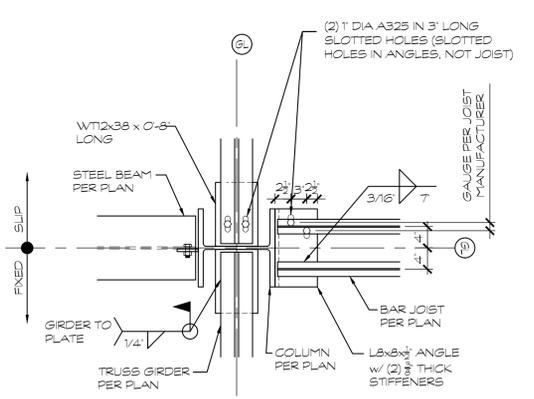
3 BRACE TO BEAM
SCALE: 3/4" = 1'-0"



4 BEAM/JOIST TO COLUMN @ EXPANSION JOINT
SCALE: 3/4" = 1'-0"



5 BRACE TO COLUMN
SCALE: 3/4" = 1'-0"



6 COLUMN AT EXPANSION JOINT
SCALE: 3/4" = 1'-0"

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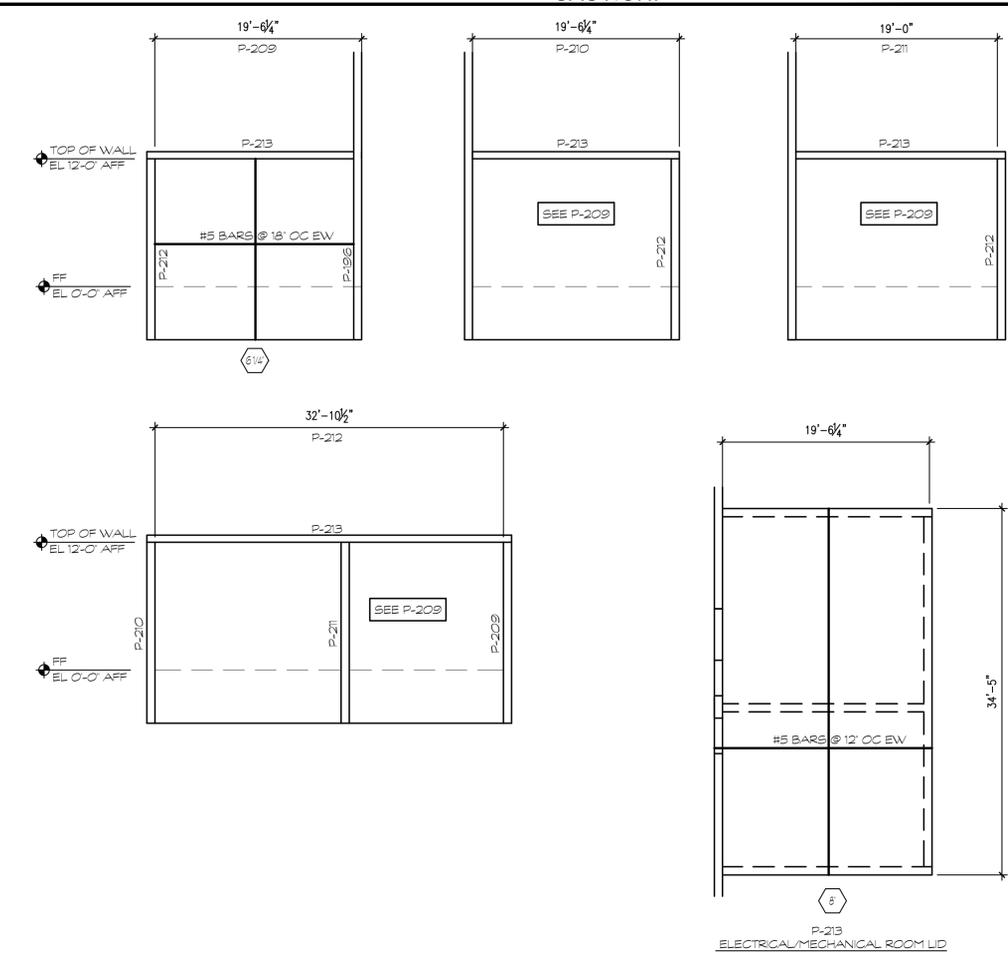
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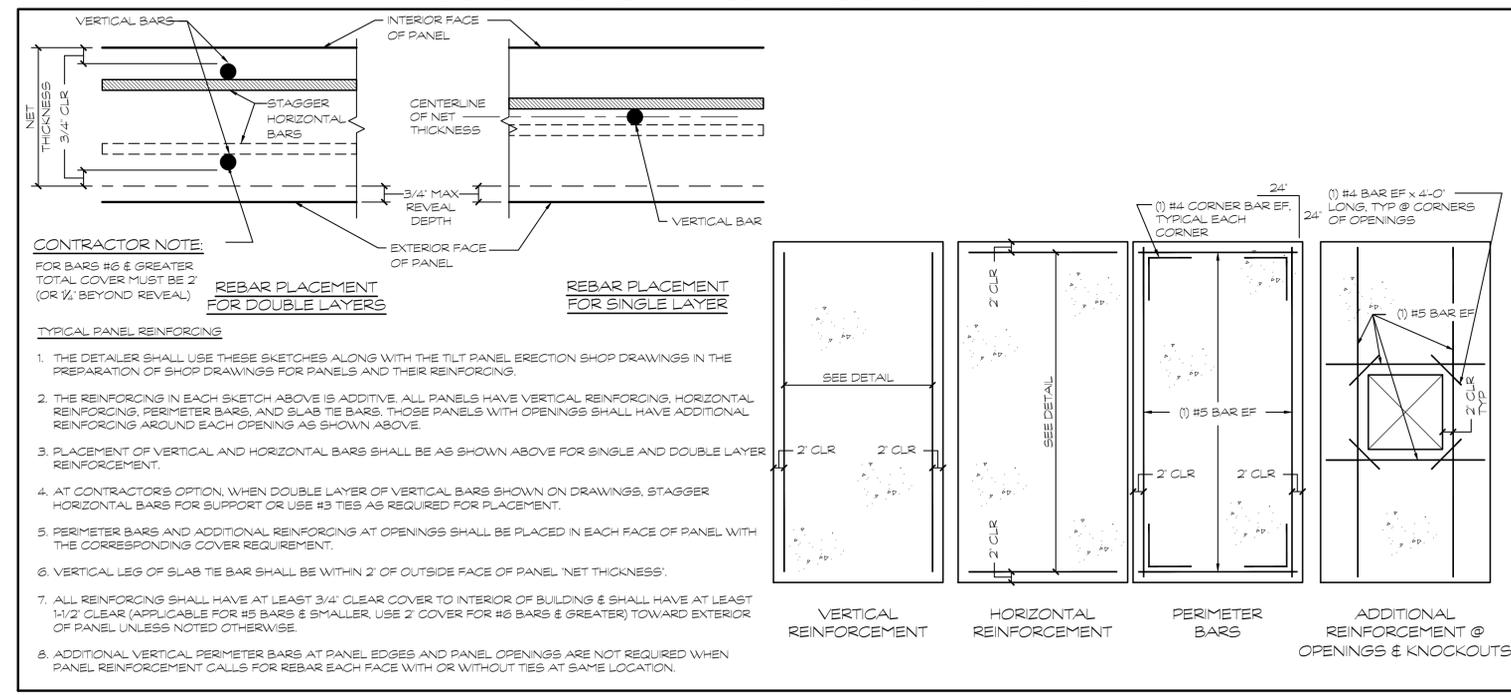
GENERAL TILT PANEL REINFORCING	
DATE	07-13-2022
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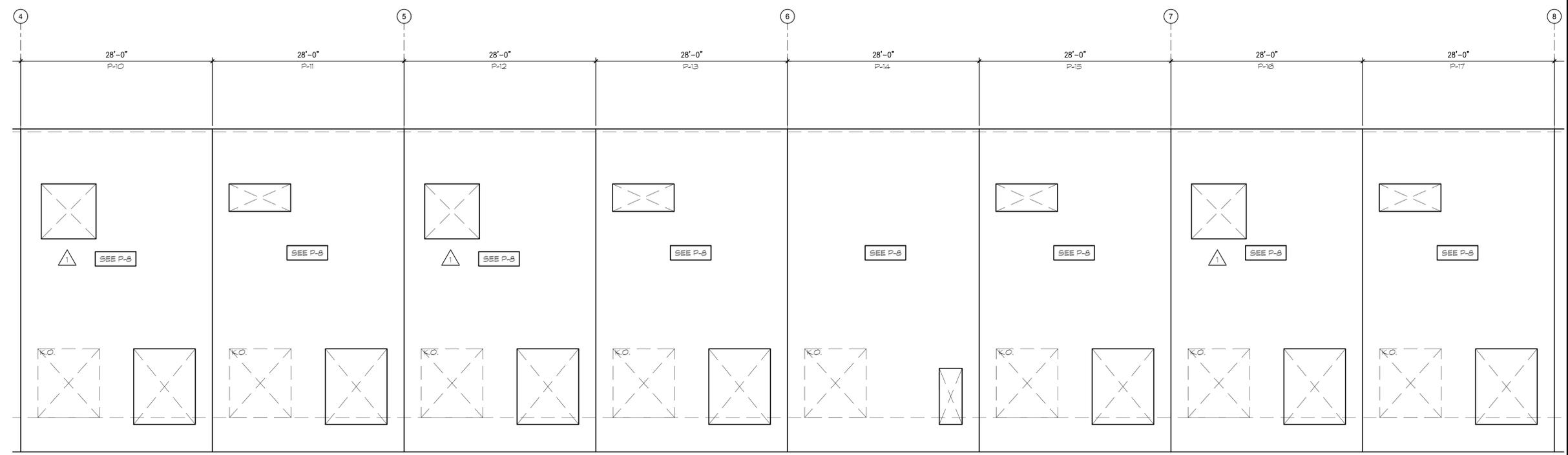
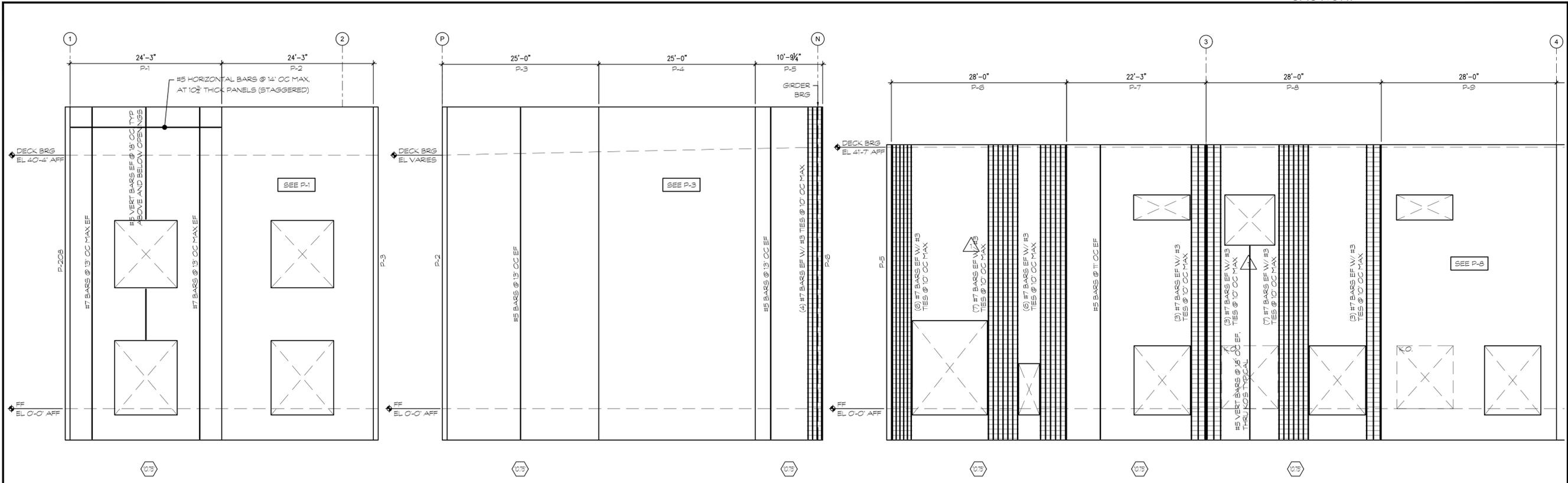
GENERAL TILT WALL PANEL REINFORCING



- CONTRACTOR NOTE:**
 FOR BARS #6 & GREATER TOTAL COVER MUST BE 2" (OR 1/2" BEYOND REVEAL)
- REBAR PLACEMENT FOR DOUBLE LAYERS**
- REBAR PLACEMENT FOR SINGLE LAYER**
- TYPICAL PANEL REINFORCING**
- THE DETAILER SHALL USE THESE SKETCHES ALONG WITH THE TILT PANEL ERECTION SHOP DRAWINGS IN THE PREPARATION OF SHOP DRAWINGS FOR PANELS AND THEIR REINFORCING.
 - THE REINFORCING IN EACH SKETCH ABOVE IS ADDITIVE. ALL PANELS HAVE VERTICAL REINFORCING, HORIZONTAL REINFORCING, PERIMETER BARS, AND SLAB TIE BARS. THOSE PANELS WITH OPENINGS SHALL HAVE ADDITIONAL REINFORCING AROUND EACH OPENING AS SHOWN ABOVE.
 - PLACEMENT OF VERTICAL AND HORIZONTAL BARS SHALL BE AS SHOWN ABOVE FOR SINGLE AND DOUBLE LAYER REINFORCEMENT.
 - AT CONTRACTOR'S OPTION, WHEN DOUBLE LAYER OF VERTICAL BARS SHOWN ON DRAWINGS, STAGGER HORIZONTAL BARS FOR SUPPORT OR USE #3 TIES AS REQUIRED FOR PLACEMENT.
 - PERIMETER BARS AND ADDITIONAL REINFORCING AT OPENINGS SHALL BE PLACED IN EACH FACE OF PANEL WITH THE CORRESPONDING COVER REQUIREMENT.
 - VERTICAL LEG OF SLAB TIE BAR SHALL BE WITHIN 2" OF OUTSIDE FACE OF PANEL 'NET THICKNESS'.
 - ALL REINFORCING SHALL HAVE AT LEAST 3/4" CLEAR COVER TO INTERIOR OF BUILDING & SHALL HAVE AT LEAST 1-1/2" CLEAR (APPLICABLE FOR #5 BARS & SMALLER, USE 2" COVER FOR #6 BARS & GREATER) TOWARD EXTERIOR OF PANEL UNLESS NOTED OTHERWISE.
 - ADDITIONAL VERTICAL PERIMETER BARS AT PANEL EDGES AND PANEL OPENINGS ARE NOT REQUIRED WHEN PANEL REINFORCEMENT CALLS FOR REBAR EACH FACE WITH OR WITHOUT TIES AT SAME LOCATION.

- PANEL ELEVATION GENERAL NOTES**
- ALL ELEVATIONS ARE AS VIEWED FROM INSIDE.
 - XX = PANEL THICKNESS (INCHES).
 - SEE 'GENERAL TILT WALL PANEL REINFORCING' DIAGRAM ON SHEET S500 FOR TYPICAL PANEL REINFORCING WHICH IS IN ADDITION TO WHAT IS SHOWN IN ELEVATION(S).
 - MAX REVEAL DEPTH SHALL BE 3/4".
 - REFER TO ARCH & MECH DWGS FOR ADDITIONAL OPENINGS NOT SHOWN HERE (ie SCUPPERS, DUCTS, LOUVERS, ETC).

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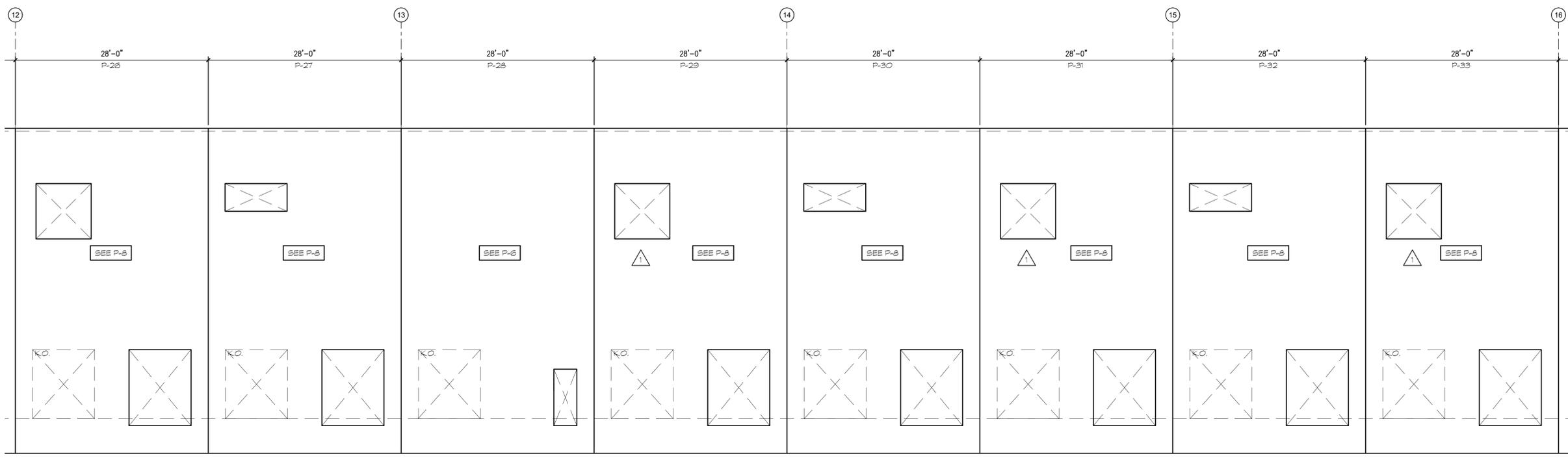
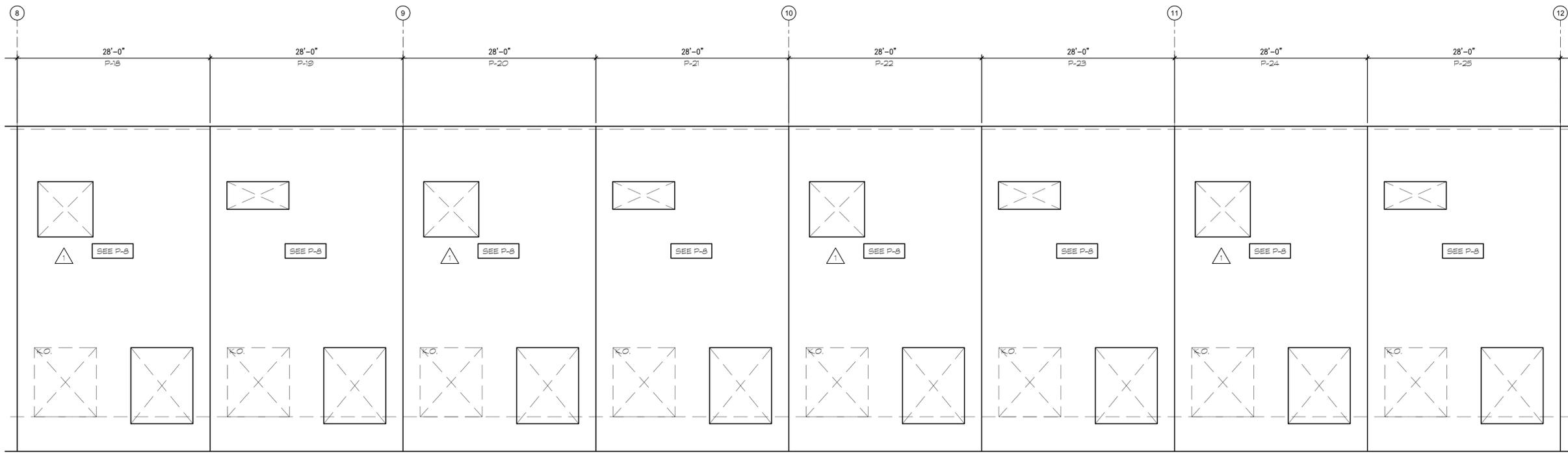
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PARTIAL TILT PANEL ELEVATIONS	
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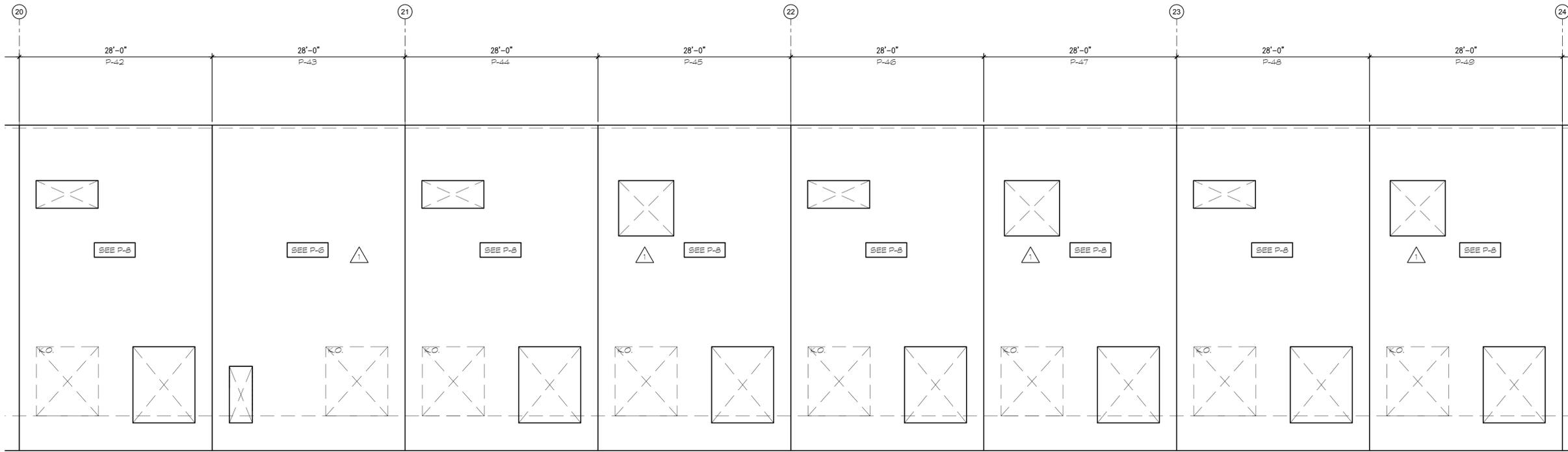
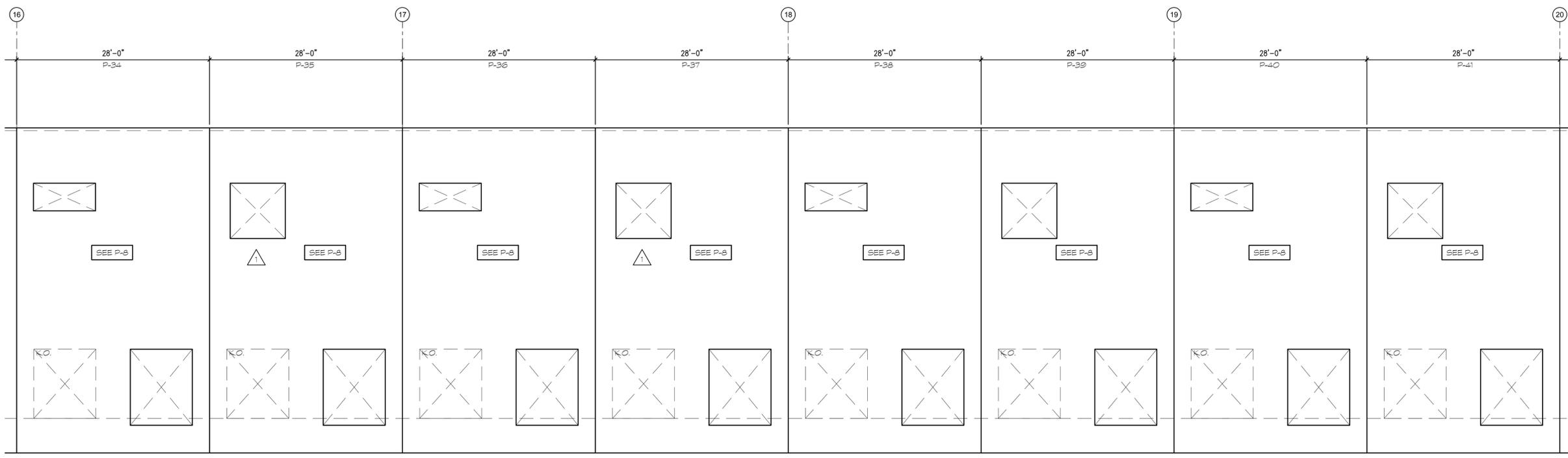
PARTIAL TILT PANEL ELEVATIONS

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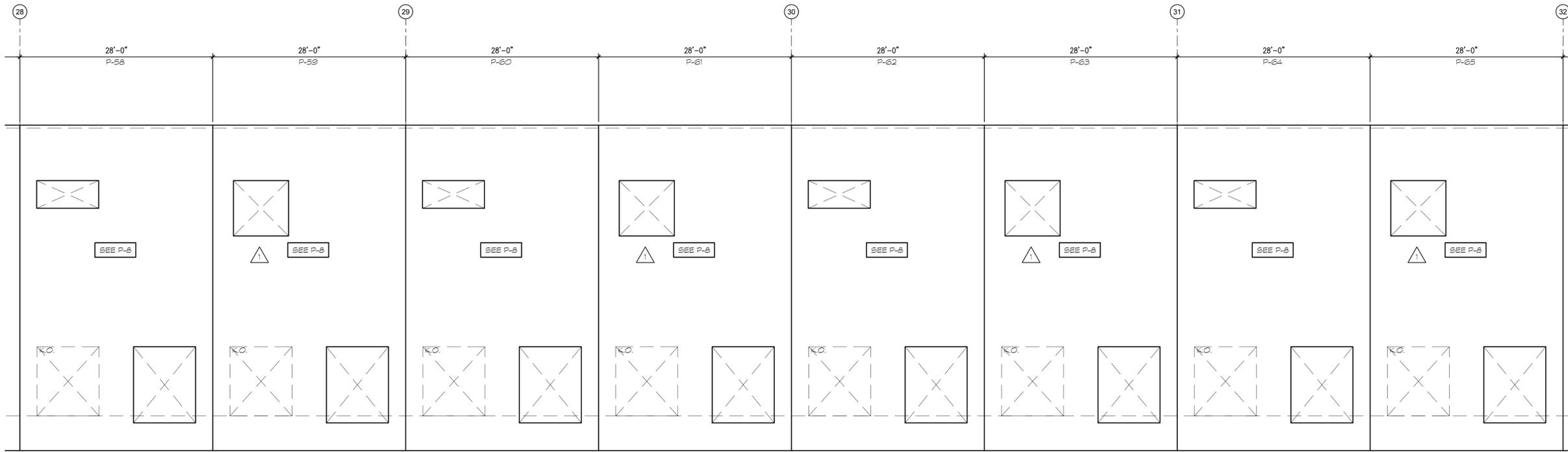
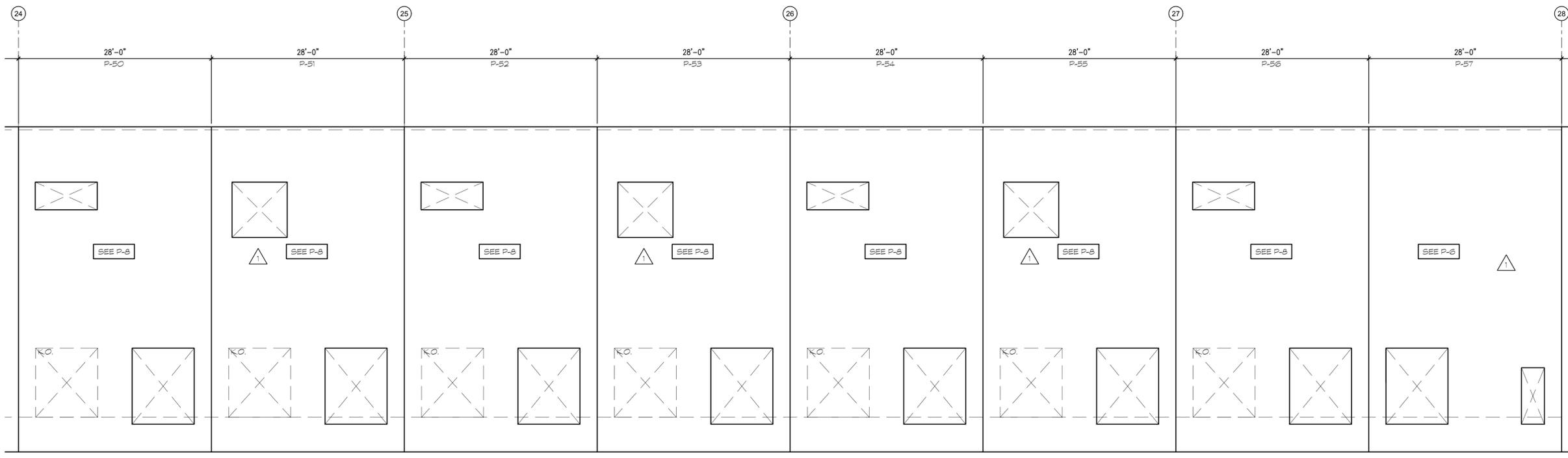
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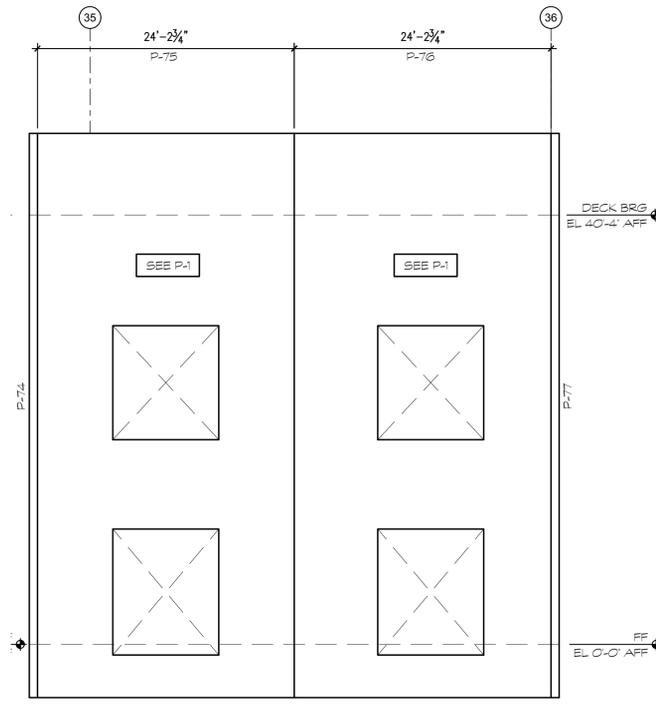
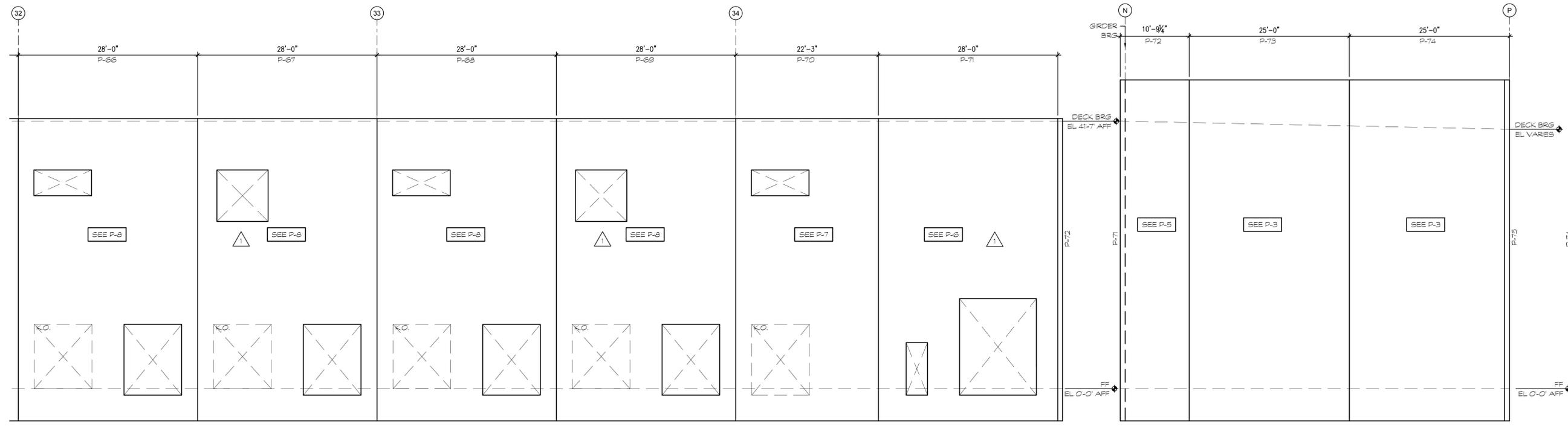
PARTIAL TILT PANEL ELEVATIONS

DATE	REMARKS
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JOB NO.:	MIA21-0040-00

SHEET
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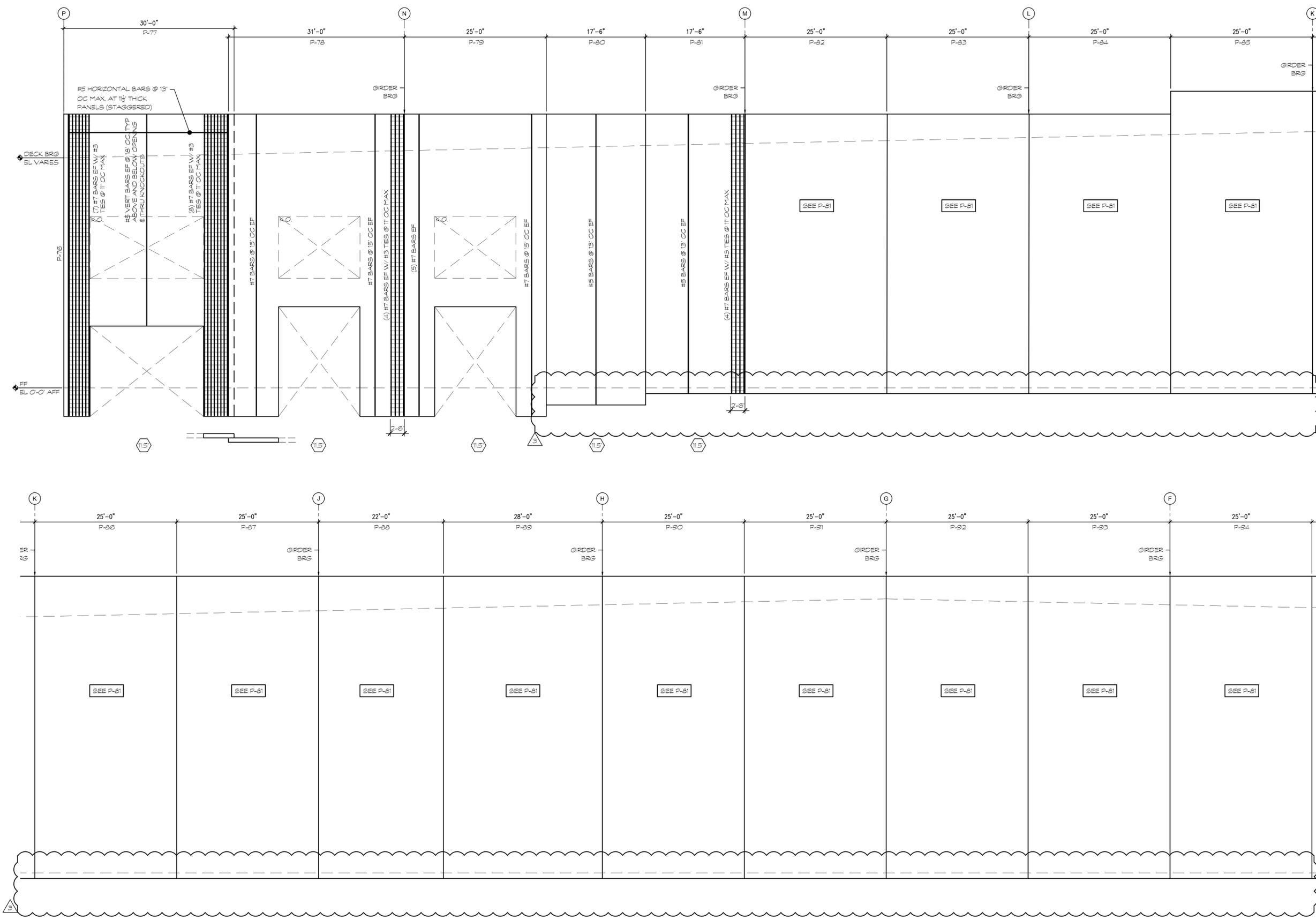
STONEMONT FT PIERCE
BLDG. 1
ORANGE AVENUE
FORT PIERCE, FLORIDA 34945

PARTIAL TILT PANEL ELEVATIONS	
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BLDG. 1
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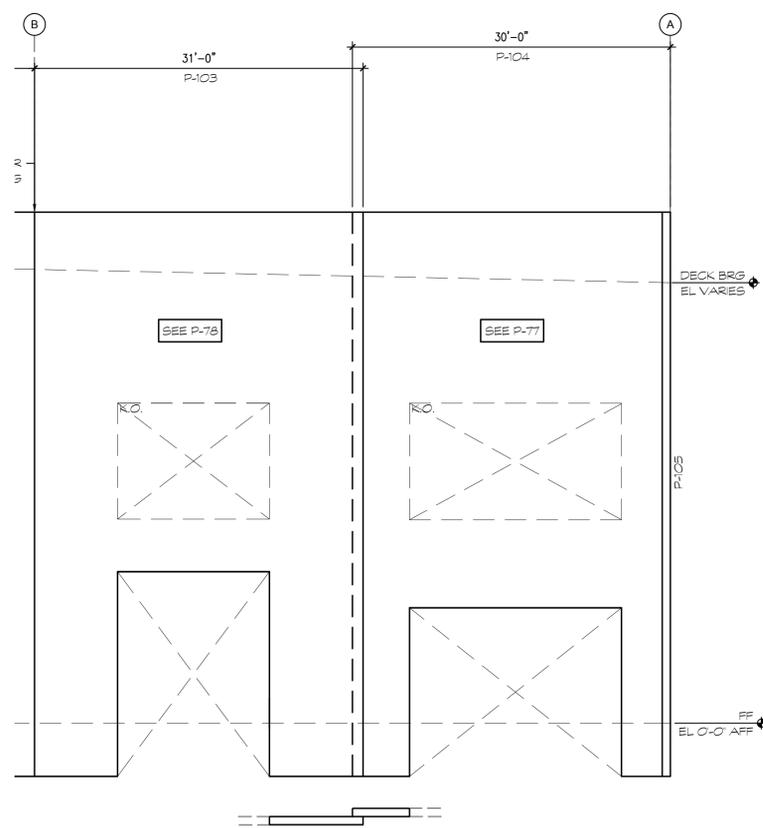
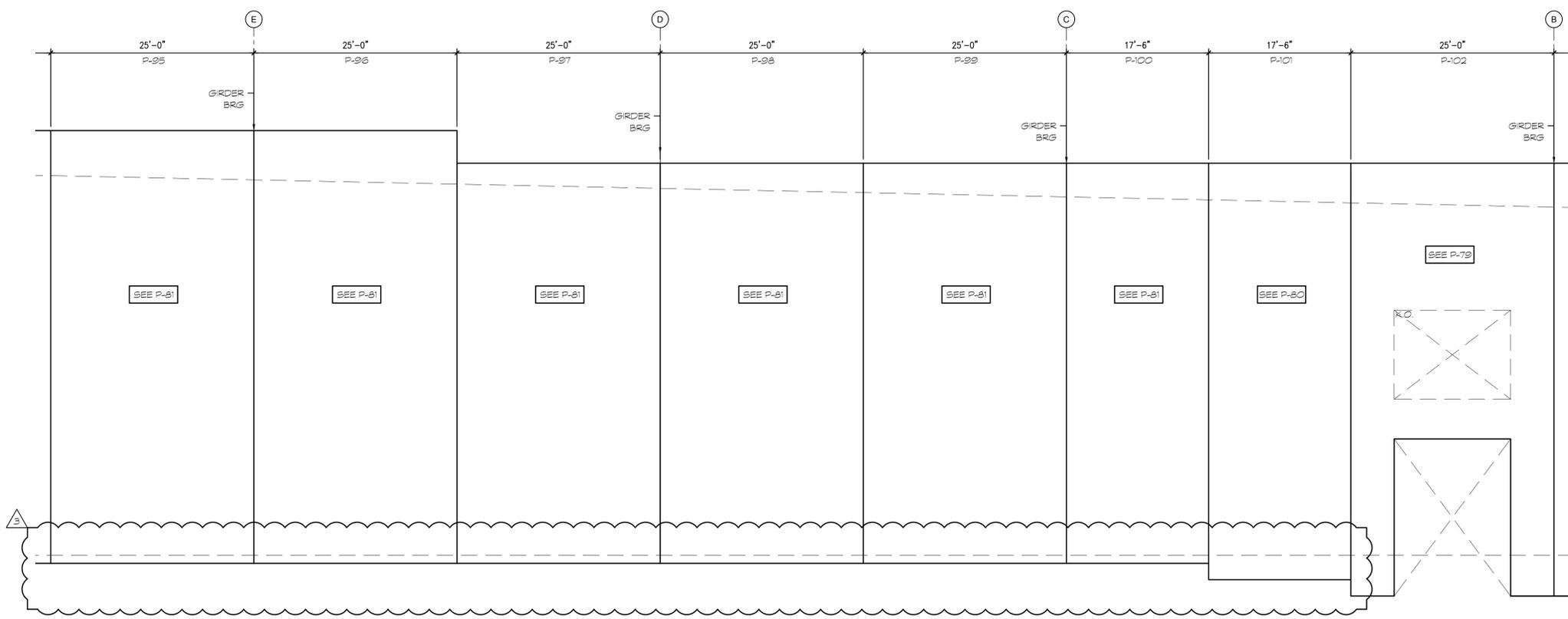
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07-13-2022	ISSUED FOR PERMIT
09-08-2022	CLIENT COORDINATION
3	

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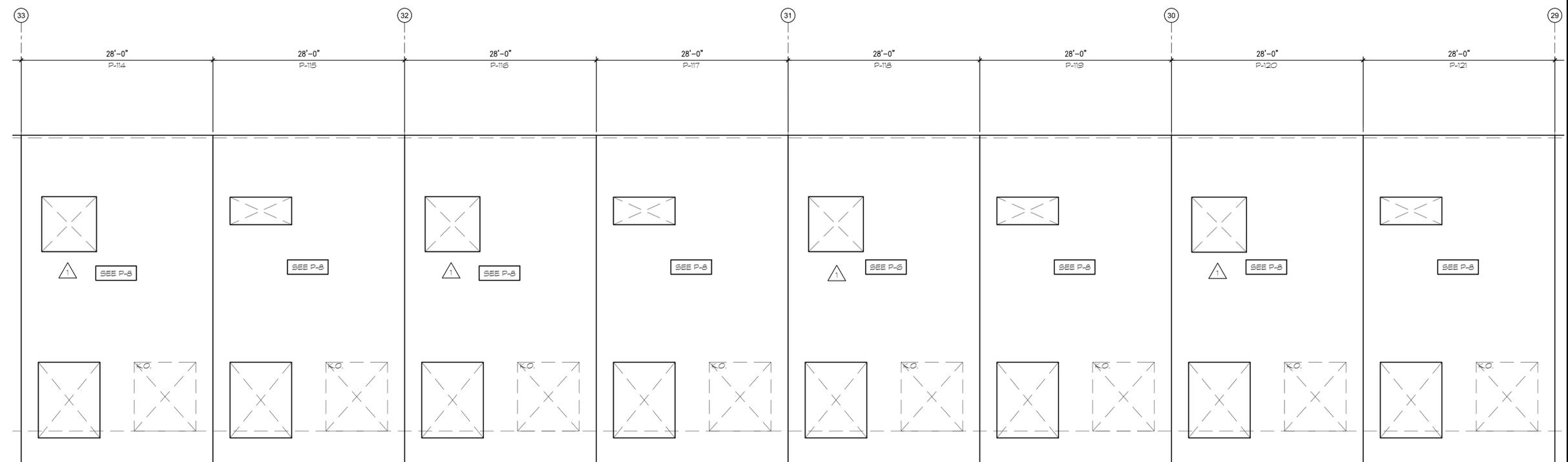
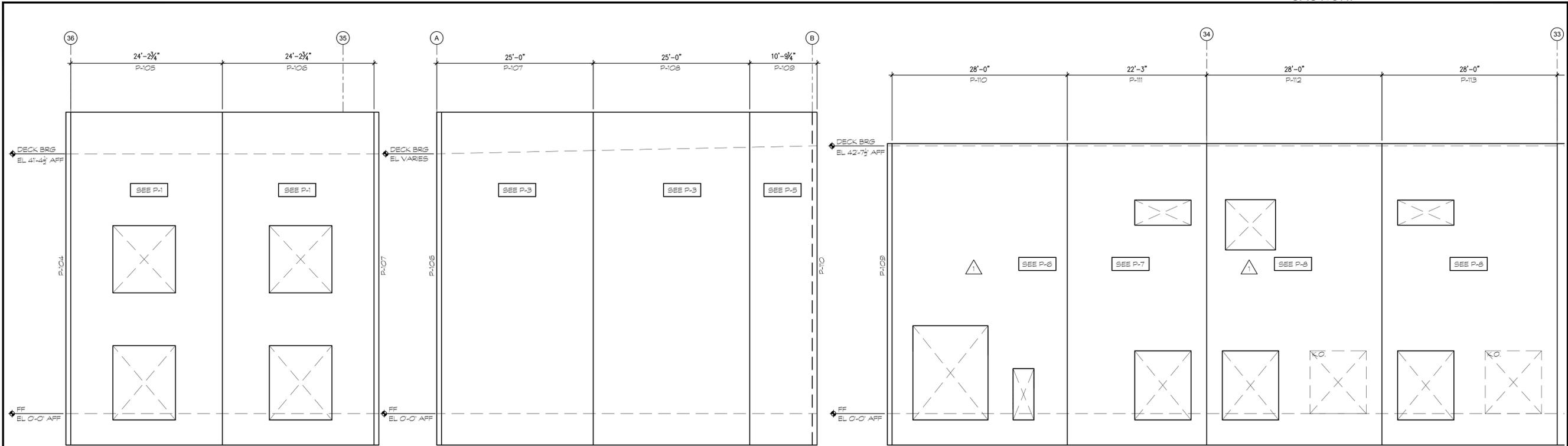
STONEMONT FT PIERCE
BLDG. 1
ORANGE AVENUE
FORT PIERCE, FLORIDA 34945

PARTIAL TILT PANEL ELEVATIONS	
DATE	REMARKS
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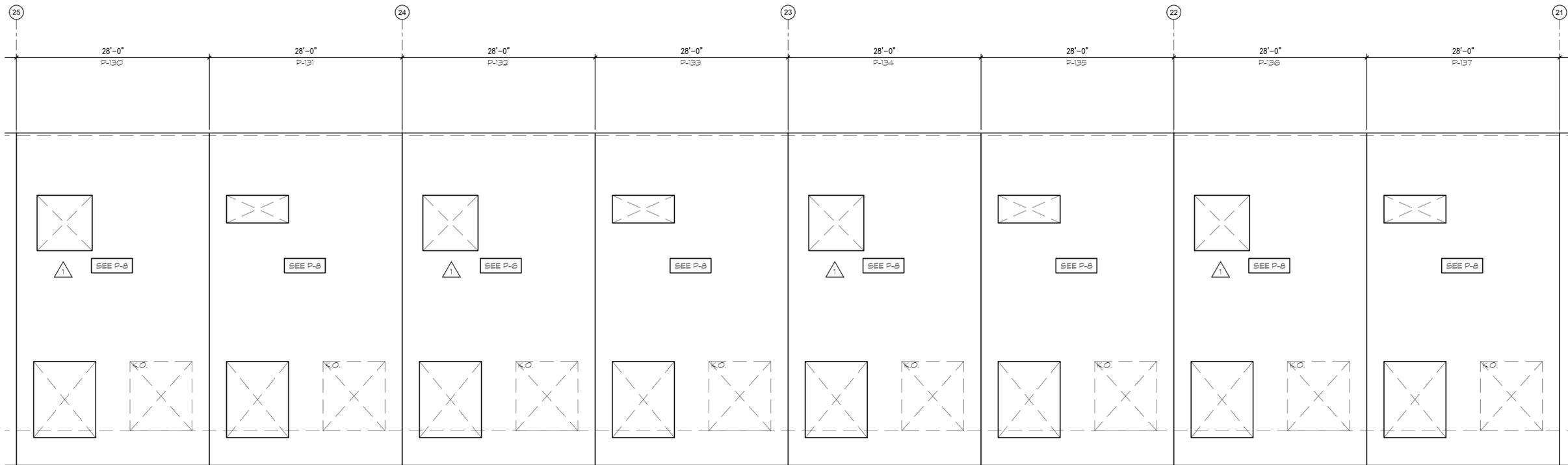
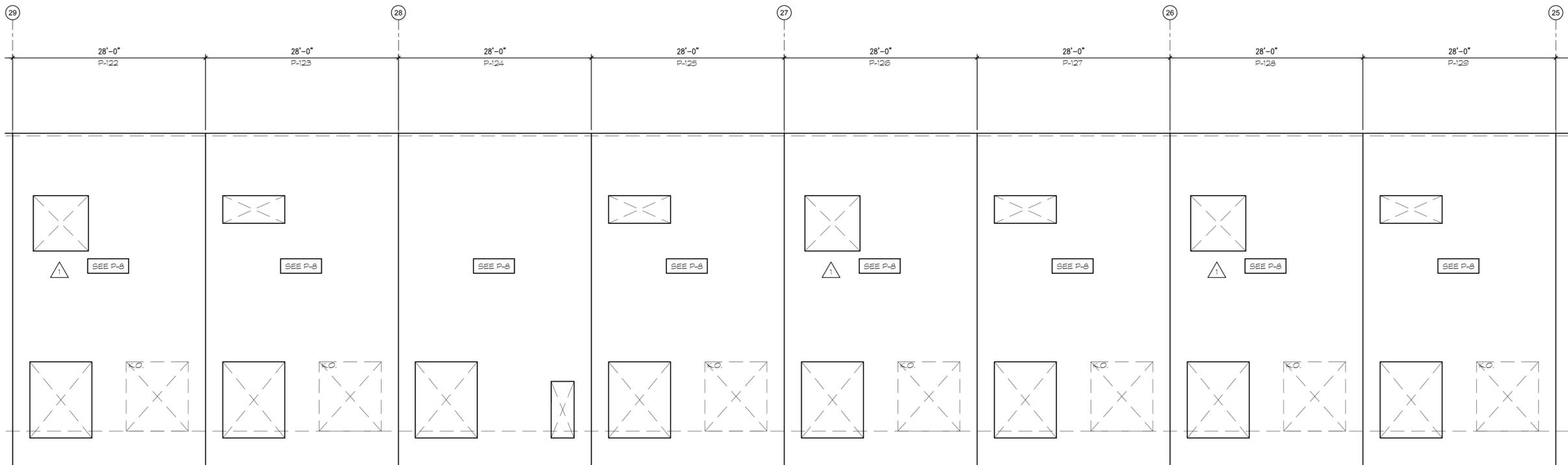
STONEMONT FT PIERCE
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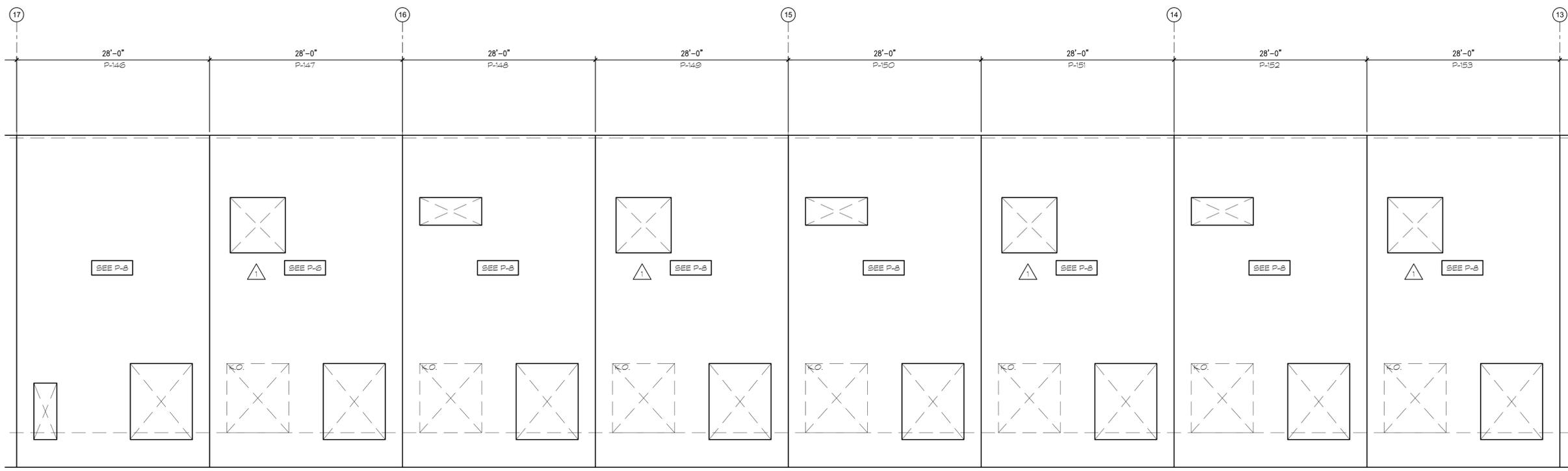
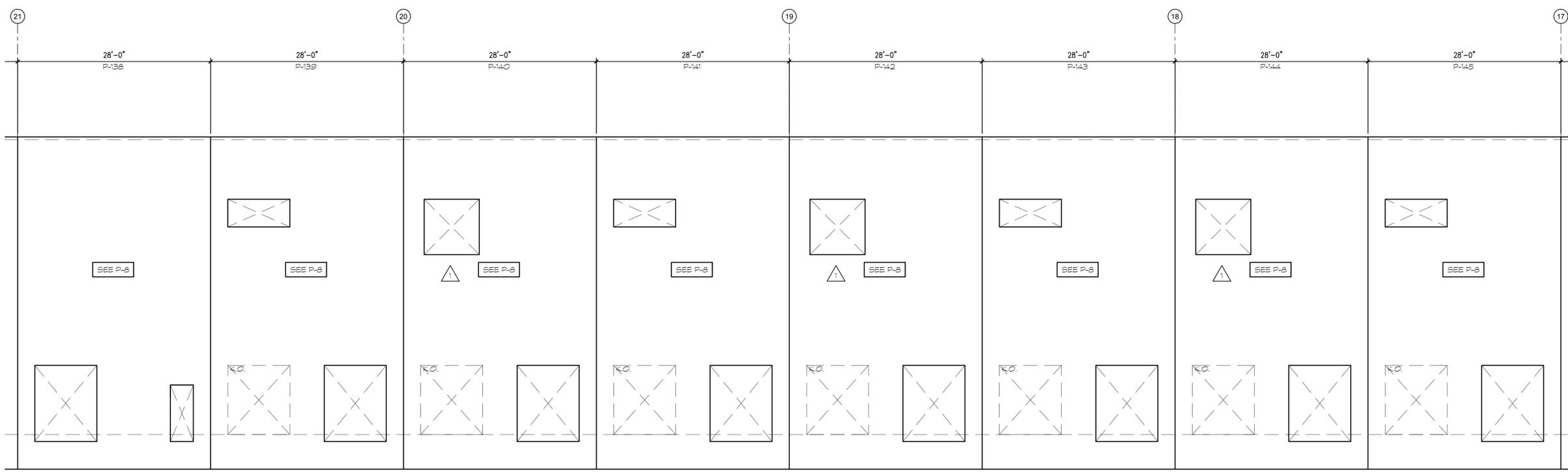
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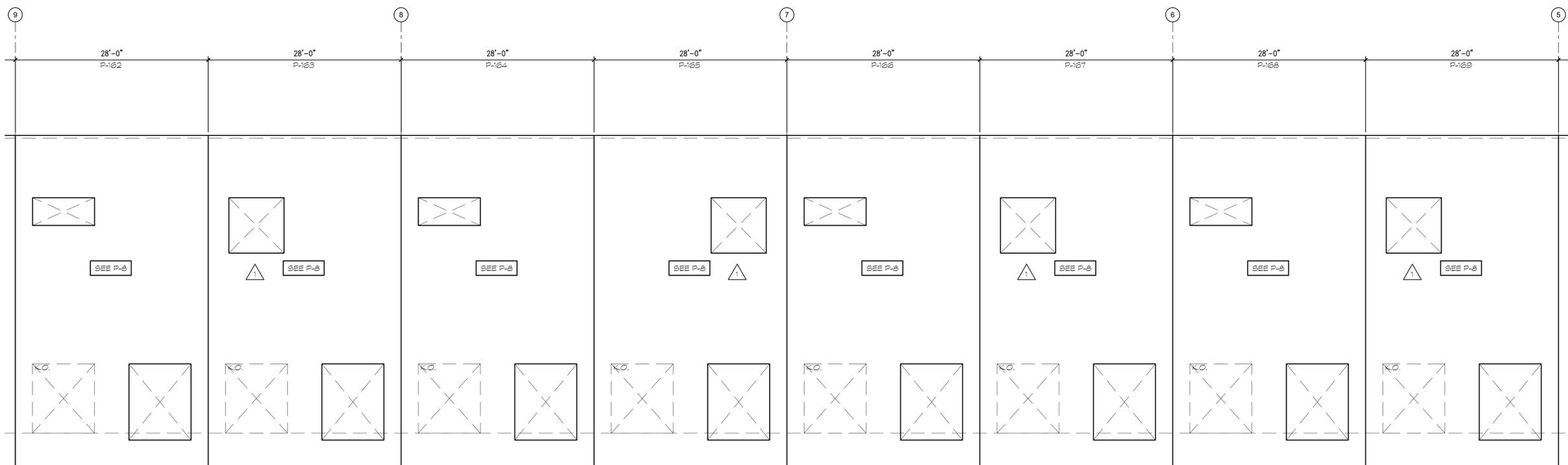
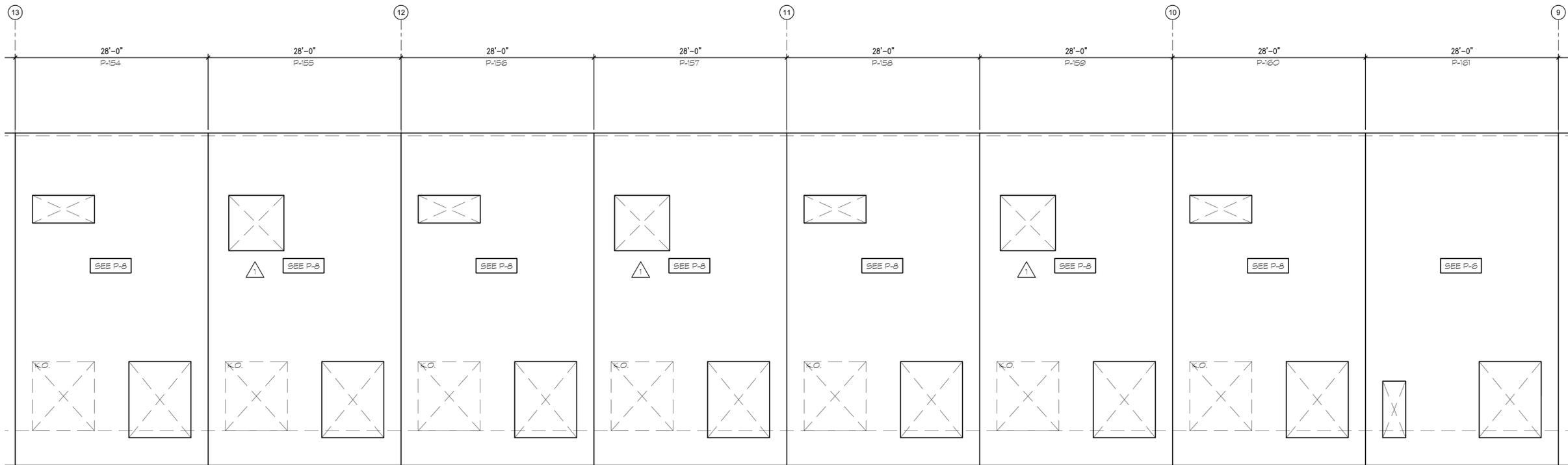
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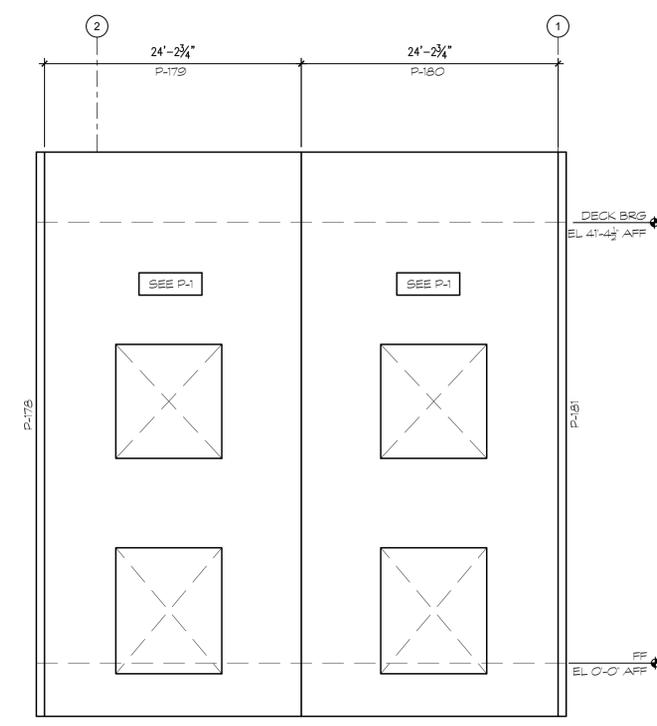
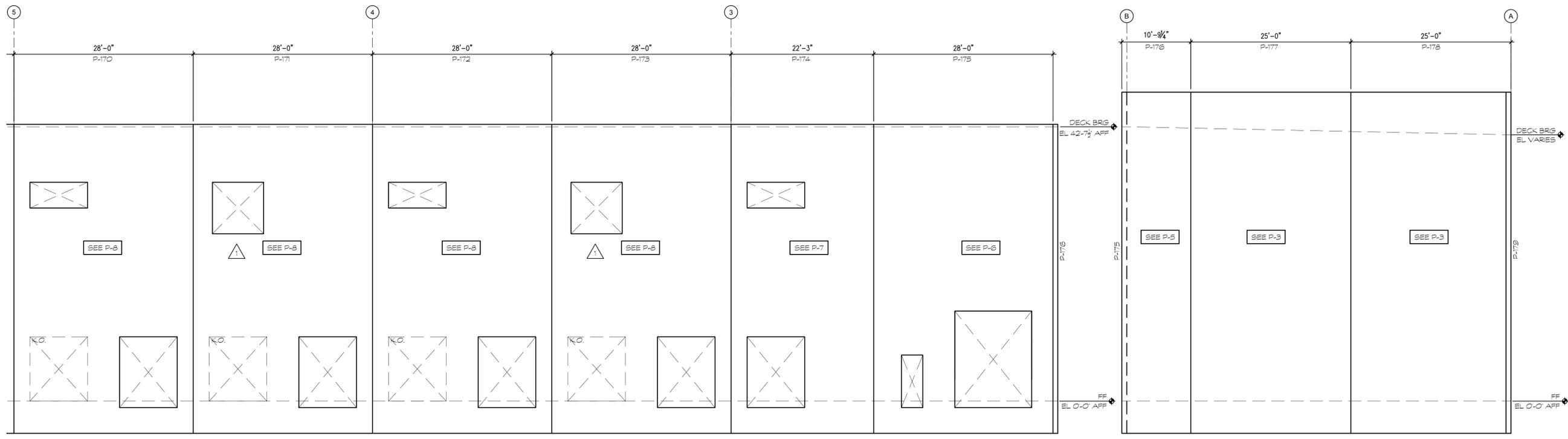
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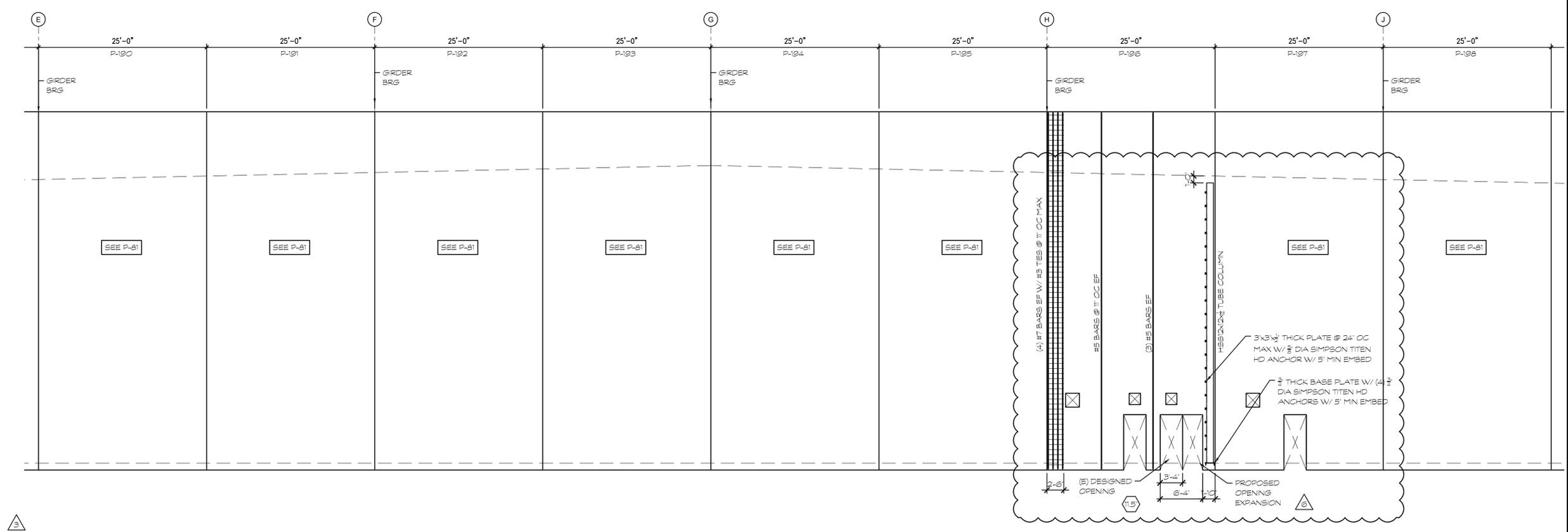
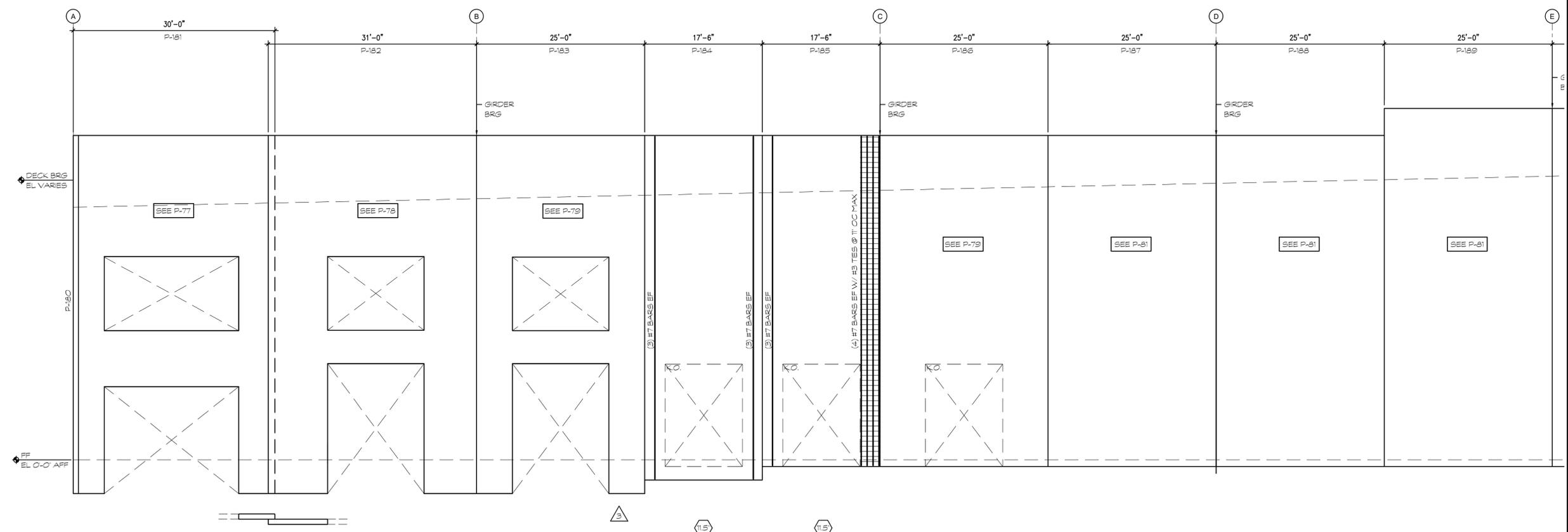
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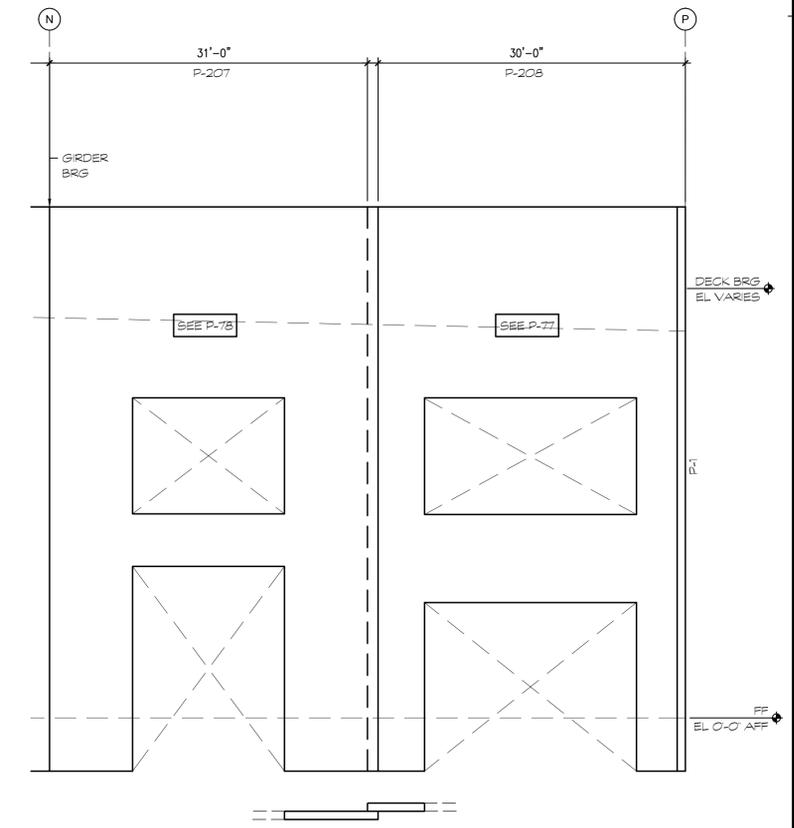
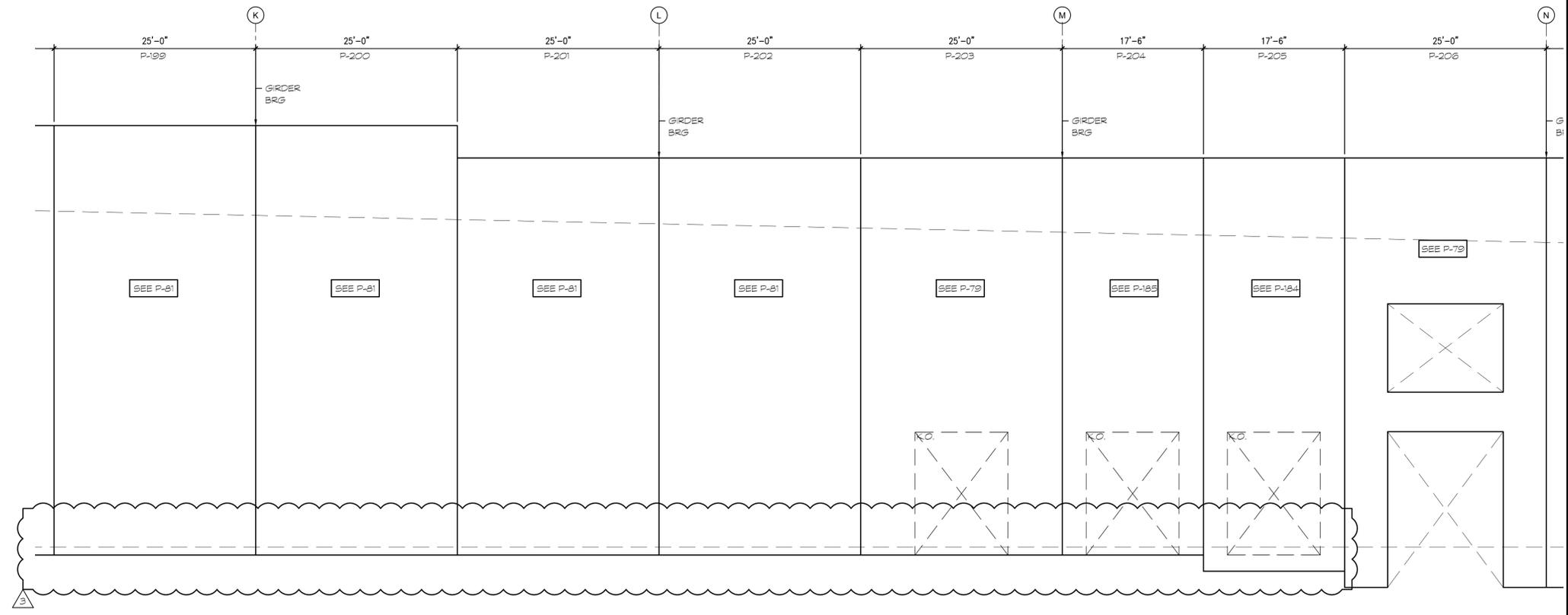
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PARTIAL TILT PANEL ELEVATIONS

DATE	REMARKS
07-13-2022	ISSUED FOR PERMIT
09-08-2022	CLIENT COORDINATION
05-15-23	FIELD CHANGE

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