DESIGN CRITERIA

D1 ALL WORK SHALL CONFORM TO AT LEAST THE MINIMUM STANDARDS OF THE FLORIDA BUILDING CODE (FBC), EIGHTH EDITION (2023).

D2 DESIGN LOAD VALUES ARE INDICATED ON THE APPROPRIATE SHEETS AS FOLLOWS:
- FOUNDATION DESIGN - SEE FOUNDATION NOTE F2 ON THIS SHEET
- ROOF FRAMING DESIGN - SHEET S120
- WIND DESIGN - SHEET S0.4
- RAIN DESIGN:
  - 15 MINUTE RAINFALL INTENSITY: 9.64 IN/HR
  - 60 MINUTE RAINFALL INTENSITY: 4.74 IN/HR

D3 TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE STRUCTURAL PLANS AND SPECIFICATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE BUILDING CODE.

D4 EVERY REASONABLE EFFORT HAS BEEN MADE TO ENSURE COORDINATION BETWEEN THESE DRAWINGS AND THE BOUND STRUCTURAL SPECIFICATIONS. SHOULD THERE BE ANY DISCREPANCIES, THE CONTRACTOR SHALL THEN REQUEST A CLARIFICATION IN WRITING.

SPECIALTY ENGINEERING REQUIREMENTS

SE1 STEEL PAN STAIRS, SHIPS LADDERS AND OTHER ROOF ACCESS LADDERS SHALL BE DESIGNED BY THE FABRICATOR'S SPECIALTY ENGINEER AND SHALL INCLUDE STRINGERS, TREADS, HAND RAILINGS, PLATFORMS (AS REQUIRED), PAN INSERTS AND MISCELLANEOUS SUPPORTS AND CONNECTIONS. SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. MINIMUM DESIGN LIVE LOAD SHALL BE 100 PSF FOR STEEL PAN STAIRS. SUBMITTALS SHALL INCLUDE THE JOINTING IN THE CONCRETE FILL AS REQUIRED TO MITIGATE PLASTIC SHRINKAGE CRACKING.

SE2 GUARDRAILS, HANDRAILS, POSTS AND SUPPORT CONNECTIONS SHALL BE DESIGNED BY THE FABRICATOR'S SPECIALTY ENGINEER. SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY A ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. DUE CONSIDERATION SHALL BE GIVEN TO EXPANSION & CONTRACTION BY PROVIDING SLIP JOINTS AS REQUIRED. DESIGN LOADING(S) SHALL CONFORM TO ALL REQUIREMENTS OF THE BUILDING CODE (SEE DESIGN CRITERIA FOR THE APPLICABLE BUILDING CODE).

SE3 EXTERIOR CURTAIN WALLS SHALL BE DESIGNED BY THE VENDOR'S SPECIALTY ENGINEER AND SHALL INCLUDE FRAME, GLASS, GLAZING AND CONNECTIONS. SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. DESIGN LOADING(S) SHALL CONFORM TO ALL REQUIREMENTS OF THE BUILDING CODE (SEE DESIGN CRITERIA FOR APPLICABLE BUILDING CODE). THE VENDOR SHALL PROVIDE WINDOW WALL REACTIONS TO THE ARCHITECT.

SE4 FLAGPOLES AND SITE LIGHTING POLES SHALL BE DESIGNED BY THE POLE VENDOR'S SPECIALTY ENGINEER AND SHALL INCLUDE POLES, FOUNDATIONS AND CONNECTIONS. SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. MINIMUM DESIGN LOADS SHALL CONFORM TO NAAMM FP1001-11 "SPECIFICATIONS FOR DESIGN LOADS OF METAL FLAGPOLES".

SE5 EXTERIOR LIGHT STEEL FRAMING, INCLUDING BUT NOT LIMITED TO: WALLS, EXTERIOR CEILINGS, FASCIAS AND SOFFITS SHALL BE DESIGNED BY A SPECIALTY ENGINEER. STRUCTURAL ELEMENTS HAVE BEEN PROVIDED FOR THE ATTACHMENT OF THE LIGHT STEEL FRAMING. THE LIGHT STEEL SYSTEM SUPPLIER SHALL DESIGN AND DETAIL ALL CONNECTIONS TO THESE ELEMENTS. ANY FURTHER ELEMENTS REQUIRED FOR THE SUPPORT SHALL BE DESIGNED AND SUPPLIED AS PART OF THE LIGHT STEEL SYSTEM. SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY AN ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. DESIGN LOADING(S) SHALL CONFORM TO ALL REQUIREMENTS OF THE BUILDING CODE (SEE DESIGN CRITERIA FOR THE APPLICABLE BUILDING CODE) AND MAXIMUM DEFLECTIONS SHALL BE AS FOLLOWS: L/600 (BRICK VENEER), L/360 (STUCCO) & L/240 (FLEXIBLE FINISHES). A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.

SE6 PROVIDE ENGINEERED SUBMITTALS, SIGNED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION, FOR ALL MECHANICAL, ELECTRICAL AND PLUMBING SUPPORTS OR ATTACHMENTS NOT INCLUDED WITHIN THE CONSTRUCTION DOCUMENTS FOR ALL ITEMS THAT REQUIRE ANYTHING OTHER THAN THE MANUFACTURER'S STANDARD HARDWARE OR ARE EXPOSED TO WIND LOADS. AN EXAMPLE OF POSSIBLE ITEMS THAT THIS WOULD APPLY TO ARE, INCLUDING BUT NOT LIMITED TO, ROOFTOP AND/OR WALL MOUNTED DUCTS, PIPES AND TRANSFORMERS.

FOUNDATIONS

F1 THE FOUNDATIONS AND SLAB-ON-GRADE DESIGN CONTAINED HEREIN IS BASED SOLELY UPON THE PROJECT'S GEOTECHNICAL REPORT (REPORT NUMBER 2408005-b, DATED 10/08/2024, AS PREPARED BY KSM ENGINEERING & TESTING). BBM HAS RELIED EXCLUSIVELY ON THE CONTENTS AND RECOMMENDATIONS WITHIN THIS REPORT, BUT ACCEPTS NO RESPONSIBILITY WHATSOEVER FOR ITS CONTENTS OR ACCURACY. THE GENERAL CONTRACTOR/CONSTRUCTION MANAGER SHALL MAKE THEMSELVES FAMILIAR WITH THE REPORT BEFORE THE COMMENCEMENT OF CONSTRUCTION.

F2 FOUNDATION DESIGN IS BASED ON A NET ALLOWABLE SOIL BEARING PRESSURE OF 2,500 PSF. COMPACTION UNDER ALL FOUNDATIONS SHALL ALSO BE AS STATED IN NOTE SOG1.

F3 A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY-STANDARD SOIL DENSITY TESTS TO ENSURE CONFORMANCE WITH GEOTECHNICAL SOILS REPORT. SUBMIT REPORTS TO ARCHITECT AND ENGINEER.

F4 CONTRACTOR, IN CONJUNCTION WITH GEOTECHNICAL FIELD REPRESENTATIVE, SHALL DETERMINE IF ANY UNSUITABLE CONDITIONS ARE DISCOVERED DURING EXCAVATION WHICH WOULD PREVENT ATTAINMENT OF THE DESIGN SOIL PRESSURE RECOMMENDED BY THE SOILS REPORT.

F5 FOOTINGS SHALL BE CAST TO THE SCHEDULED SIZE AND SHALL NOT BE OVERSIZED BY MORE THAN 6" ON ANY SIDE FOR FOOTING WIDTH OF AT LEAST 6'-0". FOR FOOTINGS LESS THAN 6'-0" IN WIDTH THE MAXIMUM OVERSIZING SHALL BE 3".

F6 CONTRACTOR SHALL BE PREPARED FOR AND SHALL INCLUDE COST OF FORMING FOUNDATIONS SHOULD THE EARTH NOT PROVIDE ADEQUATE BANK STABILITY.

GENERAL

G1 THE GENERAL CONTRACTOR SHALL REVIEW AND DETERMINE THAT DIMENSIONS ARE COORDINATED BETWEEN ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO FABRICATION OR START OF CONSTRUCTION.

G2 THE GENERAL CONTRACTOR SHALL PROVIDE ALL MEASURES AS NECESSARY TO PROTECT THE STRUCTURE, THE WORK PERSONNEL AND OTHER PEOPLE DURING CONSTRUCTION. HE SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION.

G3 NO STRUCTURAL MEMBER SHALL BE CUT, NOTCHED OR OTHERWISE REDUCED IN STRENGTH.

G4 THE GENERAL CONTRACTOR SHALL COORDINATE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR ANCHORED, EMBEDDED AND SUPPORTED ITEMS WHICH AFFECT THE STRUCTURAL DRAWINGS AND NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES.

G5 ALL SHOP DRAWING SUBMITTALS SHALL BE SUBMITTED VIA ELECTRONIC MEDIA (i.e. PDF OR DWF FORMAT). HARD COPY SUBMITTALS WILL NOT BE ACCEPTED.

1. SUBMITTALS SHALL NOT BE SECURED IN ANY FORMAT THAT WILL PREVENT COMMENTS FROM BEING ADDED.
2. SUBMITTALS THAT ARE REQUIRED TO BE SIGNED AND SEALED SHALL BE SUBMITTED WITH A CERTIFIED DIGITAL SIGNATURE CONSISTENT WITH REQUIREMENTS OF THE STATE LAW WHERE THE PROJECT IS LOCATED. INVALID OR BLANK SIGNATURE FIELDS WILL NOT BE ACCEPTED.
3. SUBMITTALS THAT ARE MARKED PRELIMINARY OR ARE OTHERWISE INCOMPLETE WILL BE RETURNED REJECTED AND OR NOT REVIEWED. SUBMITTALS SHALL BE A FINAL OR COMPLETED STATE PRIOR TO SUBMISSION TO BBM.

G6 ANY SUBMITTALS RECEIVED BY ARCH/ENG THAT HAVE NOT BEEN CHECKED BY THE GC AND HIS SUBCONTRACTOR SHALL BE RETURNED WITHOUT REVIEW.

G7 ALL SECTIONS AND DETAILS SHALL BE CONSTRUED TO BE TYPICAL OR SIMILAR UNLESS ANOTHER SECTION OR DETAIL IS NOTED.

G8 ANY CONFLICTS NOTICED, OR OBSERVED, BETWEEN THE WRITTEN SPECIFICATIONS AND THE CONSTRUCTION DOCUMENTS DURING PROJECT BIDDING OR PROJECT CONSTRUCTION SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE STRUCTURAL ENGINEER-OF-RECORD. IF SUCH DISCREPANCY IS NOT NOTICED OR BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER-OF-RECORD FOR WRITTEN CLARIFICATION, THE CONTRACTOR/SUBCONTRACTOR SHALL PROVIDE, AT NO COST TO THE OWNER DURING PROJECT CONSTRUCTION, THE MORE STRINGENT AND/OR MORE COSTLY OF THE TWO ITEMS IN THE BID AND/OR FINAL INSTALLATION.

G9 "BBM STRUCTURAL ENGINEERS" ASSUMES NO RISK OR LIABILITY FOR THE SITE SAFETY OR WELL-BEING OF ANY CONTRACTOR, SUB-CONTRACTOR NOR THEIR EMPLOYEES DURING THE CONSTRUCTION OF THE PROJECT CONTAINED IN THESE DOCUMENTS.

G10 GENERAL CONTRACTOR/CONSTRUCTION MANAGER SHALL SUPPLY ALL SUB-CONTRACTORS WITH THE STRUCTURAL GENERAL NOTE SHEETS AS WELL AS THE STRUCTURAL DRAWINGS.

G11 THE CONTRACTOR'S MEANS AND METHODS SHALL FULLY CONFORM TO THE REQUIREMENTS OF SEI/ASCE 37 (DESIGN LOADS ON STRUCTURES DURING CONSTRUCTION) UNTIL ALL OF THE STRUCTURAL ELEMENTS ARE IN PLACE AND HAVE RECEIVED THE INSPECTOR'S APPROVAL.

G12 REFER TO ARCHITECTURAL DRAWINGS FOR ROOF COVERINGS. ROOFS SHALL BE INSPECTED BY A LICENSED ENGINEER/ ARCHITECT AND A REPRESENTATIVE OF THE ROOFING MANUFACTURER AND REPORTS SHALL BE SUBMITTED TO THE OWNER AND ARCHITECT.

FLORIDA PRODUCT APPROVAL / NOTICE OF ACCEPTANCE

FPA1 THE STRUCTURAL ROOF DECK(S) FOR THIS PROJECT HAVE BEEN DESIGNED TO BE COMPLIANT WITH THE CODES AND PRESSURES LISTED HEREIN. ANY PRODUCTS INSTALLED OVER THE STRUCTURAL DECK(S) SHALL HAVE BEEN TESTED AS AN ASSEMBLY, INCLUSIVE OF THE STRUCTURAL DECK, AND SHALL BE SUBMITTED FOR APPROVAL WITH THE FPA/NOA CLEARLY IDENTIFIED. FAILURE TO PROVIDE THIS INFORMATION SHALL BE CAUSE FOR REJECTION BY THE ARCHITECT AND BBM STRUCTURAL ENGINEERS, INC. ALL WARRANTIES MUST BE PART OF THE ROOFING SUBMITTAL. ANY WARRANTY CONTAINING LANGUAGE WHICH "EXCLUDES GALE AND/OR HURRICANE FORCE WIND DAMAGE", OR "EXCLUDES DAMAGE FROM GALE AND/OR HURRICANE FORCE WINDS", OR ANY OTHER SIMILAR VERBIAGE, SHALL BE REJECTED AND THE ENTIRE PRODUCT ASSEMBLY SHALL NOT BE PERMITTED FOR USAGE ON THE PROJECT.

FPA2 RATIONAL ANALYSIS OF THE ROOF SYSTEM SHALL NOT BE PERMITTED WITHOUT THE WRITTEN CONSENT OF THE OWNER, ARCHITECT AND ENGINEER, AND THEN, ONLY WHERE PERMITTED BY THE LOCAL JURISDICTION HAVING DIRECT AUTHORITY.

SLAB ON GRADE

SOG1 UNLESS NOTED OTHERWISE IN THE GEOTECHNICAL REPORT, COMPACT INTERIOR FILL TO 95% OF MODIFIED PROCTOR MAXIMUM DRY DENSITY (ASTM D1557). SOIL COMPACTION SHALL BE FIELD-CONTROLLED BY A REPRESENTATIVE TECHNICIAN OF A QUALIFIED LABORATORY. EACH LAYER OF FILL SHALL NOT EXCEED 12" THICK AND SHALL BE COMPACTED PRIOR TO PLACEMENT OF NEXT LAYER.

SOG2 MAXIMUM SPACING OF CONTROL JOINTS (i.e. SAWCUT JOINT OR CONSTRUCTION JOINT) SHALL BE AS SET IN THE TABLE BELOW, OR AS NOTED ON PLANS. THE MORE STRINGENT SHALL APPLY. PATTERNS SHALL BE APPROXIMATELY SQUARE WITH A RATIO OF LONG SIDE TO SHORT SIDE NOT EXCEEDING 1.5 TO 1. SEE SLAB-ON-GRADE DETAILS FOR ADDITIONAL INFORMATION.

SLAB THICKNESS (IN)	* 3/4" OR LARGER AGGREGATE SPACING (FT)
4	12
5	13
6	14
7 AND GREATER	15

* MIX DESIGNS CONTAINING AGGREGATE LESS THAN 3/4" ARE NOT ACCEPTABLE.

SOG3 GENERAL CONTRACTOR SHALL COORDINATE EXACT LOCATION OF SJ'S AND CJ'S WITH ARCHITECTURAL FLOOR FINISHES TO ENSURE SLAB JOINTS DO NOT READ THROUGH.

POST-INSTALLED ANCHORS

PIA1 POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER-OF-RECORD (SEOR) PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REBAR. HOLES SHALL BE DRILLED AND CLEANED AND PRODUCT SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII). SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE SPECIFIED BELOW, SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER-OF-RECORD ALONG WITH CALCULATIONS THAT ARE PREPARED & SEALED BY A REGISTERED PROFESSIONAL ENGINEER IN THE SAME STATE AS PROJECT LOCATION. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING EQUIVALENT OR BETTER PERFORMANCE VALUES OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S) AS REQUIRED BY THE BUILDING CODE.

PIA2 THE CONTRACTOR SHALL ARRANGE FOR AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS. ADDITIONALLY, INSTALLATION OF ADHESIVE ANCHORS IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION SHALL BE DONE BY CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS CERTIFIED THROUGH ACI AND IN ACCORDANCE WITH ACI 318-19 (SECTION 17.2.3). PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED PRIOR TO COMMENCEMENT OF INSTALLATION.

PIA3 CONCRETE ANCHORS:

A. MECHANICAL ANCHORS FOR USE IN CRACKED AND UNCRACKED CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.2 AND/OR ICC-ES AC193. PRE-APPROVED MECHANICAL ANCHORS INCLUDE:
   1. DEWALT "POWER-STUD+ SD1" (ICC-ES ESR-2818)
   2. DEWALT "POWER-STUD SD2, SD4 (3/4&5) AND SD6 (3/8&5)" (ICC-ES ESR-2502)
   3. DEWALT "SCREW-BOLT+" (ICC-ES ESR-3889)
   4. DEWALT "SNAKE+" (ICC-ES ESR-2272)
   5. DEWALT "MINI-UNDERCUT+" (ICC-ES ESR-3912)
   6. HILTI KWIK BOLT-TZ2 EXPANSION ANCHOR SAFE SET SYSTEM WITH AT MODULE FOR APPLICABLE SIZES (ICC-ES ESR-4266)
   7. HILTI KWIK BOLT 1 EXPANSION ANCHOR SAFE SET SYSTEM WITH HOLLOW DRILL BIT AND VACUUM AT MODULE FOR APPLICABLE SIZES PER IAPMO UES ER-678
   8. HILTI KWIK HUS-EZ (KH-EZ), KH-EZ CRC, KH-EZ S&316, KH-EZ C, KH-EZ E, KH-EZ I, AND KH-EZ P SCREW ANCHOR SAFE SET SYSTEM WITH HOLLOW DRILL BIT AND VACUUM PER ICC-ES ESR-3027
   9. SIMPSON STRONG-TIE "STRONG-BOLT 2" (ICC-ES ESR-3037)
   10. SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR-2713)
   11. SIMPSON STRONG-TIE "STAINLESS STEEL TITEN HD" (IAPMO UES ER-493)

B. ADHESIVE ANCHORS FOR USE IN CRACKED AND UNCRACKED CONCRETE SHALL BE INSPECTED AS FOLLOWS: AT THE ONSET OF EACH APPLICATION, A MANUFACTURER'S REPRESENTATIVE MUST BE PRESENT TO WITNESS AT LEAST FIVE COMPLETE INSTALLATIONS. INSTALLERS MUST BE TRAINED BY THE MANUFACTURER AND MANUFACTURER SHALL SUBMIT DOCUMENTATION TO THE CONTRACTOR INDICATING TRAINING HAS TAKEN PLACE. TRAINED INSTALLERS SHALL PROVIDE WRITTEN DOCUMENTATION TO THE CONTRACTOR THAT ALL ANCHORS HAVE BEEN INSTALLED PER THE MPII. CONTRACTOR SHALL SUBMIT THIS DOCUMENTATION TO THE SEOR. ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE HAVING MINIMUM AGE OF 21 DAYS. HOLES SHALL BE DRY AT THE TIME OF INSTALLATION. ADHESIVES SHALL HAVE MAX IN-SERVICE SHORT-TERM TEMPERATURE OF 150°F, AND MAX IN-SERVICE LONG-TERM TEMPERATURE OF 110°F. PRIOR TO INSTALLATION OF ADHESIVE ANCHORS IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATIONS RESISTING SUSTAINED TENSION LOADS, INSTALLERS ARE REQUIRED TO BE CERTIFIED IN ACCORDANCE WITH THE ACI / CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM OR MUST BE CONTINUOUSLY INSPECTED. ANCHORS SHALL ALSO HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC308. PRE-APPROVED ADHESIVE ANCHORS INCLUDE THE FOLLOWING PRODUCTS:
   1. DEWALT
      a. "PURE 220+" WITH DUSTX+ HOLLOW DRILL BIT SYSTEM OR USING TRADITIONAL BLOW OUT METHOD PER ICC-ES ESR-5144.
      b. AC200+ WITH DUSTX+ HOLLOW DRILL BIT SYSTEM OR USING TRADITIONAL BLOW OUT METHOD PER ICC-ES ESR-4027.
   2. DEWALT AC200+ GOLD (ICC-ES ESR-4027)
   3. HILTI
      a. HILTI HIT-HY 200 V3 SAFE SET SYSTEM WITH THE HILTI HIT-Z ROD PER ICC ESR-4868.
      b. HILTI HIT-HY 200 V3 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT (TE-CD OR TE-YD) AND VACUUM WITH HAS-E THREADED ROD PER ICC ESR-4868.
      c. HILTI HY 200 V3 ADHESIVE ANCHOR USING TRADITIONAL CLEANING METHOD, 2 BLOWS OF COMPRESSED AIR (90 PSI MINIMUM, OIL FREE COMPRESSOR), 2 BRUSHES WITH SAME SIZE AS HOLE WIRE BRUSH, AND 2 MORE BLOWS, (2x2x2 CLEANING METHOD) PER ICC ESR-4868.
      d. HILTI HIT-RE 500-V3 EPOXY RESIN ANCHORING SYSTEM WITH HAS-E THREADED ROD PER ICC ESR-3814 FOR SLOW CURE APPLICATIONS.
      e. HILTI KWIK-X DUAL ACTION ANCHOR SAFESET SYSTEM WITH KHC CAPSULE ADHESIVE AND KWIK-HUS EZ PER ICC ESR-5065
   4. SIMPSON STRONG-TIE "SET-3G" (ICC-ES ESR-4057)
   5. SIMPSON STRONG-TIE "ET-3G" (ICC-ES ESR-5334)
   6. SIMPSON STRONG-TIE "AT-XP" (IAMPO UES ER-263)
   7. SIMPSON STRONG-TIE "ET-HP" (ICC-ES ESR-3372)

PIA4 MASONRY ANCHORS:

A. ANCHORAGE TO SOLID-GROUTED CONCRETE MASONRY
   1. MECHANICAL AND CONCRETE SCREW ANCHORS FOR USE IN SOLID-GROUTED CONCRETE MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC01 OR AC106, RESPECTIVELY. PRE-APPROVED MECHANICAL AND CONCRETE SCREW ANCHORS INCLUDE:
      a. DEWALT "POWER-STUD+ SD1" (ICC-ES ESR-2966)
      b. DEWALT "SCREW-BOLT+" (ICC-ES ESR-4042)
      c. HILTI KWIK BOLT-TZ2 EXPANSION ANCHOR PER ICC-ES ESR-4561
      d. HILTI KWIK BOLT-1 EXPANSION ANCHOR PER IAPMO UES ER-617
      e. HILTI KH-EZ, KH-EZ CRC, KH-EZ S&316, KH-EZ C, AND KH-EZ P SCREW ANCHORS PER ICC-ES ESR-3056
      f. SIMPSON STRONG-TIE "STRONG-BOLT 2" (IAPMO UES ER-240)
      g. SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR-1056)
   2. ADHESIVE ANCHORS FOR USE IN SOLID-GROUTED CONCRETE MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC58. PRE-APPROVED ADHESIVE ANCHORS INCLUDE:
      a. DEWALT AC100+ GOLD (ICC-ES ESR-3200)
      b. HILTI HIT-HY 270 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT AND VACUUM PER ICC-ES ESR-4143
      c. HILTI HIT-HY 200 V3 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT AND VACUUM PER ICC ESR-4878
      d. SIMPSON STRONG-TIE "SET-XP" (IAPMO UES ER-265)
      e. SIMPSON STRONG-TIE "AT-XP" (IAPMO UES ER-281)
      f. SIMPSON STRONG-TIE "ET-HP" (IAPMO UES ER-241)

B. ANCHORAGE TO HOLLOW CONCRETE MASONRY / UNREINFORCED CLAY BRICK MASONRY
   1. SCREW ANCHORS FOR USE IN HOLLOW CONCRETE MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC106. PRE-APPROVED SCREW ANCHORS INCLUDE:
      a. SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR-1056)
   2. ADHESIVE ANCHORS WITH SCREEN TUBES FOR USE IN HOLLOW CONCRETE MASONRY / UNREINFORCED CLAY BRICK MASONRY SHALL BE TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC58 OR AC60, AS APPROPRIATE. THE APPROPRIATE SCREEN TUBE SHALL BE USED AS RECOMMENDED BY THE ADHESIVE MANUFACTURER. PRE-APPROVED ADHESIVE ANCHORS WITH SCREEN TUBES INCLUDE:
      a. DEWALT AC100+ GOLD (ICC-ES ESR-3200) - HOLLOW CONCRETE MASONRY
      b. DEWALT AC100+ GOLD (ICC-ES ESR-4105) - CLAY BRICK MASONRY
      c. HILTI HY 270 ADHESIVE ANCHOR (ICC-ES ESR-4143)
      d. SIMPSON STRONG-TIE "SET-XP" (IAPMO UES ER-265)
      e. SIMPSON STRONG-TIE "ACRYLIC-TIE" (ICC-ES ESR-5191)

PRE-FABRICATED METAL PLATE CONNECTED WOOD TRUSSES

WT1 A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.

WT2 "BBM STRUCTURAL ENGINEERS" WILL REQUIRE THAT THE ENGINEERED DRAWINGS FOR THE WOOD COMPONENTS ON THE REFERENCED PROJECT BE REVIEWED FOR COMPATIBILITY WITH THE DESIGN INTENT OF THE STRUCTURE PRIOR TO FABRICATION.

WT3 ALL WOOD TRUSSES SHALL BE DESIGNED AND CERTIFIED BY TRUSS MANUFACTURER'S REGISTERED ENGINEER. ALL TRUSS-TO-TRUSS & TRUSS-TO-STRUCTURE CONNECTIONS SHALL BE DESIGNED AND DETAILED BY THE TRUSS MFR. CONNECTIONS SHALL BE DESIGNED FOR DEAD, LIVE AND WIND (BOTH UPLIFT & LATERAL) LOADS. THE WIND PRESSURE AND SUCTION EXERTED ON THE WALLS MAY BE FOUND IN THE WIND SCHEDULES. CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT.

WT4 DESIGN, HANDLING, ERECTION AND TEMPORARY AND PERMANENT BRACING OF METAL PLATE CONNECTED WOOD TRUSSES SHALL BE IN ACCORDANCE WITH THE FOLLOWING DOCUMENTS:

A. APPLICABLE BUILDING CODE IDENTIFIED IN THE GENERAL NOTES.

B. NATIONAL FOREST PRODUCTS ASSOCIATION (NFPA). NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS) LATEST EDITION.

C. TRUSS PLATE INSTITUTE (TPI) & WOOD TRUSS COUNCIL OF AMERICA (WTCA). BUILDING COMPONENT SAFETY INFORMATION BCSI 1-03 "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING AND BRACING OF METAL PLATE CONNECTED WOOD TRUSSES".

WT5 TRUSS REPAIRS REQUIRED DURING THE PROGRESS OF WORK ARE TO BE DESIGNED BY THE TRUSS ENGINEER WITH REQUIRED MATERIALS AND HARDWARE TO BE PROVIDED BY THE TRUSS MANUFACTURER. FIELD REPAIRS MADE TO TRUSSES ARE TO BE SUPERVISED AND INSPECTED BY THE TRUSS MANUFACTURER'S REPRESENTATIVE. WRITTEN CONFIRMATION OF REPAIR ACCEPTANCE BY THE TRUSS MANUFACTURER IS REQUIRED ON ALL REPAIRS AND MUST BE SUBMITTED TO ARCHITECT FOR ACCEPTANCE. NAIL-ON TYPE TRUSS REPAIR PLATES WILL NOT BE ALLOWED IN A BOTTOM CHORD TENSION SPLICE.

WT6 SHOP DRAWINGS SHALL BE SUBMITTED WHICH INDICATE DESIGN LOADS, DURATION FACTOR, TRUSS LAYOUT, TRUSS CONFIGURATIONS AND TRUSS-TO-TRUSS CONNECTIONS (USE OF NAIL-ON TYPE TRUSS PLATES FOR FIELD SPLICES IN TENSION MUST BE APPROVED BY ENGINEER OF RECORD). SHOP DRAWINGS SHALL SHOW PIECE MARKS, MEMBER SIZES, GRADES AND CONNECTION DETAILS. PERMANENT BRACING SHALL BE INDICATED IN THE TRUSS LAYOUT DRAWINGS AND SHALL BE SUPPLIED AND INSTALLED BY THE FRAMING CONTRACTOR.

WT7 REVIEW ARCHITECTURAL REFLECTED CEILING PLANS AND SECTIONS FOR SPECIAL CEILING CONDITIONS INCLUDING CEILING SLOPES, TROFFERS, COFFERS, TRAYS, STEPS AND OTHER SPECIAL FEATURES.

WT8 REVIEW ARCHITECTURAL FLOOR PLANS FOR OPERABLE WALLS THAT ARE SUSPENDED FROM THE TRUSSES. ANALYSIS FOR CLOSED, PARTIALLY OPEN AND OPEN POSITION LOAD CURVES ARE REQUIRED. SEE WALL MFR FOR SPECIAL DEFLECTION CRITERIA.

WT9 REVIEW MECHANICAL PLANS FOR ROOF TOP UNITS, EXHAUST FANS, KITCHEN HOODS AND OTHER LOADS SUSPENDED OR SUPPORTED BY TRUSSES FOR LOCATION, LOADS AND PHYSICAL SIZE. TRUSS SPACING AND MULTI-PLY TRUSS CONFIGURATIONS TO ACCOMMODATE OPENINGS AND SPECIAL LOADING SHALL BE PROVIDED BY THE TRUSS MANUFACTURER, UNLESS NOTED OTHERWISE ON PLANS.

WT10 TRUSSES SHALL BE DESIGNED FOR THE LOADS AS INDICATED ON THE FRAMING PLANS. ALLOWABLE STRESS INCREASES SHALL BE AS IDENTIFIED IN NFPA NDS LATEST EDITION UNLESS SUPERSEDED BY REQUIREMENTS OF THE APPLICABLE BUILDING CODE. PRE-FABRICATED WOOD TRUSSES SHALL BE FABRICATED FROM SYP#1 FINE KNOT DENSE DRIED "S" GRADE OR BETTER FOR CHORDS AND "S" GRADE OR BETTER FOR WEBS. NO WANE, KNOTS, SKIPS OR OTHER DEFECTS SHALL OCCUR IN THE PLATE CONTACT AREA OR SCARFED AREA OF WEB MEMBERS. PLATES SHALL BE CENTERED WITH ONE REQUIRED EACH SIDE OF TRUSS.

WT11 BEARING LOCATIONS MUST BE MARKED ON TRUSSES BY FABRICATOR TO ENSURE PROPER INSTALLATION. TRUSS ENGINEER TO UTILIZE ONLY LOAD BEARING WALLS INDICATED ON PLAN FOR BEARING AND TIE-DOWN. TRUSS SUPPLIER SHALL PROVIDE GALVANIZED MOISTURE BARRIER WHERE THE TRUSS BEARS ON EITHER MASONRY OR CONCRETE SUPPORTS.

WT12 BOTTOM CHORD BRACING: WHERE BOTTOM CHORDS OF TRUSSES DO NOT HAVE SOLID SHEATHING (REVIEW ARCHITECTURAL REFLECTED CEILING PLAN FOR SUSPENDED CEILING AND/OR OPEN TRUSS WORK), PROVIDE PERMANENT 2x4 x 10'-0" (MIN LENGTH) LATERAL BRACES AT 10'-0" OC (MAX) UNLESS CLOSER SPACING IS REQUIRED BY TRUSS ENGINEER'S ANALYSIS. STAGGER SPLICES MIN 4'-0" AND ATTACH WITH (2) 12d NAILS AT EACH TRUSS. TERMINATE BRACING RUNS AT WALLS, OR WITH DIAGONAL CROSS BRACE EACH END. CONFORM TO TRUSS PLATE INSTITUTE (TPI) HIB-91, "HANDLING, INSTALLING AND BRACING OF METAL PLATE CONNECTED WOOD TRUSSES".

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CONCRETE AND REINFORCING

C1 A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD TESTING INCLUDING SLUMP TESTS AND CYLINDER BREAKS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.

C2 CONCRETE WORK SHALL CONFORM TO ACI 318-19 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE.

C3 ALL CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES:

LOCATION	28 DAY STRENGTH	SLUMP	COARSE AGGREGATE(S)	COMMENTS	EXPOSURE CATEGORIES & CLASSES (PER ACI 318-19 SECTION 19.3.1)			
					F	S	W	C
FOUNDATIONS	3000 PSI	4" +/- 1"	1"		F0	S0	W0	C0
SLAB-ON-GRADE: FOOT TRAFFIC	3000 PSI	4" +/- 1"	1"		F0	S0	W0	C0
TIE BEAMS AND TIE COLUMNS (MAX 50' BETWEEN COLD JOINTS)	3000 PSI	6" +/- 1"	3/8"		F0	S0	W0	C0

NOTES:
1. SLUMP FOR RAMPS AND SLOPING SURFACES SHALL NOT EXCEED 4".
2. ALL CONC MIXES SHALL HAVE A MAXIMUM SAND TO TOTAL AGGREGATE RATIO OF Ø.50.
3. A 2" OR 3" PUMP SHALL BE ACCEPTABLE FOR COLUMNS AND TIE BEAMS BUT WILL NOT BE ALLOWED FOR FOUNDATIONS, SLABS, AND CONCRETE BEAMS.
4. READY MIX SUPPLIER SHALL DESIGN THE MIXES THAT CONTAIN MULTIPLE AGGREGATES TO BE WELL GRADATED.
5. SLABS SHALL NOT BE AIR ENTRAINED.
6. FOR SLABS THAT SHALL RECEIVE MOISTURE SENSITIVE FLOORING:
   a. CONTRACTOR SHALL WORK WITH THE READY MIX SUPPLIER TO PROVIDE A MIX DESIGN THAT WILL BE AT OR BELOW 75% RELATIVE HUMIDITY AT THE TIME THE FLOORING IS SCHEDULED TO BE INSTALLED.
   b. DO NOT USE LIGHTWEIGHT AGGREGATES.
   c. PROVIDE A MIX WITH GOOD SELF-DESICCATING PROPERTIES. CONSIDER ADDING 2%-4% SILICA FUME.
   d. DO NOT HARD TROWEL THE SURFACE BUT INSTEAD PROVIDE A LIGHTLY TROWELLED SURFACE.

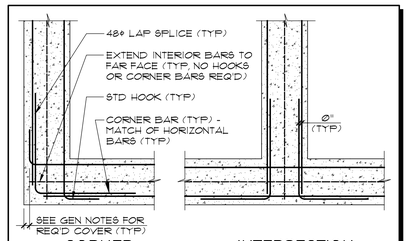
C4 CONCRETE MIX DESIGN SUBMITTALS:
1. EACH MIX DESIGN SHALL BE LABELED TO INDICATE THE AREA IN WHICH THE CONCRETE IS TO BE PLACED (I.E. FOUNDATIONS, SLAB-ON-GRADE, COLUMNS, ETC.). FAILURE TO DO SO WILL CAUSE DELAY AND/OR REJECTION OF SUBMITTALS.
2. PROPOSED MIX DESIGN SHALL BE IN ACCORDANCE WITH METHOD 1 OR METHOD 2 OF ACI 301. PROVIDE SUPPORTING DATA IN TABULAR FORM FOR EACH SEPARATE PROPOSED MIX.
3. SUBMIT CONCRETE MIX DESIGN FOR EACH PROPOSED CLASS OF CONCRETE.

C5 REBAR SHALL CONFORM TO ASTM A615 GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064 AND SHALL BE LAPPED MINIMUM ONE MESH + 2" WHERE SPLICED. ALL REINFORCING SHALL BE DOMESTICALLY PRODUCED. ALL REBAR THAT IS TO BE WELDED SHALL BE LOW ALLOY ASTM A706 GRADE 60.

C6 SPLICES AND ANCHORAGE OF REINFORCING SHALL BE AS FOLLOWS (UNLESS OTHERWISE NOTED):

WELDED WIRE FABRIC: 8"
ALL OTHER (UNLESS SCHEDULED OTHERWISE: #6 BAR & SMALLER: 48 DIA (12" MIN)
#7 BAR & LARGER: 60 DIA

C7 REINFORCEMENT IN WALLS, FOOTINGS AND BEAMS SHALL BE CONTINUOUS AND LAPPED 48 BAR DIA AT SPLICE UNLESS OTHERWISE NOTED. HOOK AND LAP ALL CORNER AND INTERSECTING BARS. (SEE REINF DEVELOPMENT DETAIL).



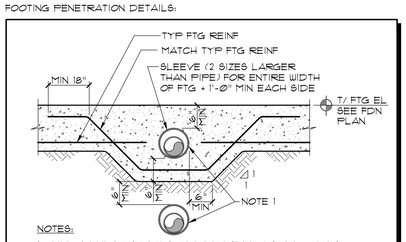
CORNER INTERSECTION

REINF DEVELOPMENT DETAIL

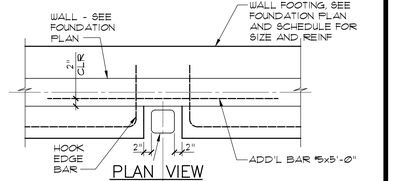
C8 COVER FOR REINFORCING SHALL BE AS FOLLOWS:

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	3"
CONCRETE EXPOSED TO EARTH OR WEATHER:	
#6 THRU #18 BARS:	2"
#5 BAR, W31 OR D31 WIRE AND SMALLER:	1 1/2"

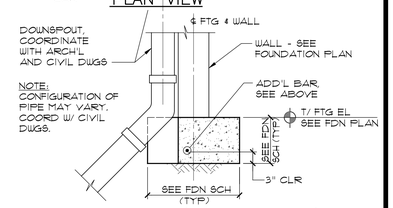
C9 FOOTING PENETRATION DETAILS:



OPTION 1 (SLEEVED)



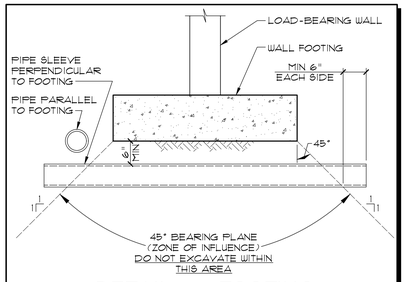
PLAN VIEW



OPTION 2 (NOTCH IN FTG)

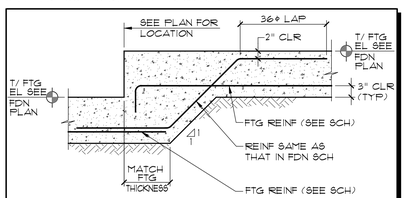
DETAIL – FOOTING PENETRATIONS

C10 FOOTING EXCAVATION DETAIL:



DETAIL – FOOTING EXCAVATIONS

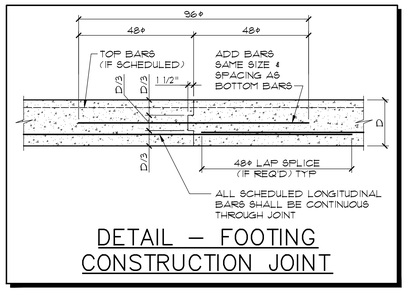
C11 STEPPED FOOTING DETAIL (IF REQUIRED):



DETAIL – STEPPED FOOTING

C12 BASE CONSTRUCTION PRICE SUBMITTED BY CONTRACTOR/SUBCONTRACTOR SHALL INCLUDE 1000# OF BOTH #5 AND #6 REINFORCING STEEL AND 100 MAN HOURS (TOTAL) OF LABOR FOR INSTALLATION OF THIS REINFORCING AS DIRECTED BY THE ARCHITECT/ENGINEER.

C13 FOOTING CONSTRUCTION JOINT (IF REQUIRED):



DETAIL – FOOTING CONSTRUCTION JOINT

WOOD

WD1 WOOD CONSTRUCTION SHALL CONFORM TO AITC, NATIONAL DESIGN SPECIFICATIONS AND THE APPLICABLE CODE. SEE NOTE D1 FOR APPLICABLE BUILDING CODE.

WD2 HURRICANE CLIPS, JOIST HANGERS, POST CAPS AND BASES, BEAM HANGERS AND OTHER STANDARD CONNECTIONS SHALL BE ADEQUATE FOR LOADS. METAL CONNECTORS AND FASTENERS SHALL BE G185 HOT-DIPPED GALVANIZED.

WD3 PROVIDE MEMBERS AS SCHEDULED:

VISUALLY GRADED DIMENSION LUMBER
(PER SOUTHERN FOREST PRODUCTS ASSOCIATION SUPPLEMENT No. 13)

JOIST, STUD & HEADERS		Fb* (PSI)	Fv (PSI)	Fc⊥* (PSI)	E (PSI)	COMMERCIAL GRADE AND SPECIES
THICKNESS (in)	WIDTH (in)					
2 TO 4	4	1100	175	1450	1,400,000	NO. 2 SYP
2 TO 4	6	1000	175	1400	1,400,000	NO. 2 SYP
2 TO 4	8	925	175	1350	1,400,000	NO. 2 SYP
2 TO 4	10	800	175	1300	1,400,000	NO. 2 SYP
2 TO 4	12	750	175	1250	1,400,000	NO. 2 SYP

* INCLUDES SIZE FACTOR (CF)

WD4 PROVIDE MEMBERS AS SCHEDULED:

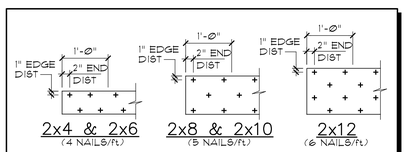
VISUALLY GRADED TIMBERS
(PER SOUTHERN FOREST PRODUCTS ASSOCIATION SUPPLEMENT No. 13)

BEAMS & COLUMNS	Fb* (PSI)	Fv (PSI)	Fc⊥* (PSI)	E (PSI)	COMMERCIAL GRADE AND SPECIES
5x5 & LARGER	850	165	525	1,200,000	NO. 2 SYP
	1500	165	950	1,500,000	SELECT STRUCTURAL SYP

* INCLUDES SIZE FACTOR (CF)

WD5 WOOD IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE-TREATED.

WD6 UNLESS NOTED OTHERWISE, MULTI-MEMBER WOOD RAFTERS, BEAMS AND LEDGERS SHALL BE FASTENED TOGETHER WITH 10d COMMON NAILS (Ø.131"x3" PNEUMATIC FRAMING NAILS ARE AN ACCEPTABLE ALTERNATE) IN EVENLY SPACED STAGGERED PATTERNS AS SHOWN BELOW:



FOR THREE (OR MORE) PLY MULTI MEMBERS, THE NAILS SHALL BE INSTALLED SUCCESSIVELY. FOR EXAMPLE, PLACE THE FIRST AND SECOND MEMBERS TOGETHER AND ATTACH PER THE SPECIFIED NAIL SPACING, THEN PLACE THE THIRD MEMBER AGAINST THE SECOND AND REPEAT NAILING, AND SO ON. EXCEPT FOR LEDGERS, EACH MEMBER SHALL BE FULL LENGTH.

NAILING PATTERN DETAILS

WD7 JOISTS OR RAFTERS SHALL BE LATERALLY SUPPORTED AT ENDS BY SOLID BLOCKING OR OTHER MEANS TO PREVENT ROTATION.

WD8 STANDARD WASHERS SHALL BE USED BETWEEN WOOD AND BOLT HEADS AND NUTS. BOLTS AND SCREWS SHALL BE ASTM A307. ALL FASTENERS IN CONTACT WITH TREATED WOOD SHALL BE G185 HOT-DIPPED GALVANIZED.

WD9 REVIEW ARCHITECTURAL REFLECTED CEILING PLANS AND SECTIONS FOR SPECIAL CEILING CONDITIONS INCLUDING CEILING SLOPES, TROFFERS, COFFERS, TRAYS, STEPS AND OTHER SPECIAL FEATURES.

ABBREVIATIONS

AAC	AUTOCLAVED AERATED CONCRETE
AB	ANCHOR BOLT
ABV	ABOVE
A.C.I.	AMERICAN CONCRETE INSTITUTE
ADDL	ADDITIONAL
AFF	ABOVE FINISH FLOOR
AGGR	AGGREGATE
A.I.S.C.	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
A.I.S.I.	AMERICAN IRON AND STEEL INSTITUTE
AL	ALUMINUM
ALT	ALTERNATE
AR	ANCHOR ROD
ARCH	ARCHITECT(URAL)
ASD	ALLOWABLE STRESS DESIGN
ASTM	AMERICAN SOCIETY OF TESTING MATERIALS
A.W.S.	AMERICAN WELDING SOCIETY
B/	BOTTOM OF
BB	BOND BEAM
BLDG	BUILDING
BLKG	BLOCKING
BLW	BELOW
BM	BEAM
BOT	BOTTOM
BP	BASE PLATE
BRDG	BRIDGING
BRG	BEARING
BRK	BRICK
BS	BOTH SIDES
BTJ	BOLTED TIE JOIST
BTWN	BETWEEN
C/C	CENTER TO CENTER
CANT	CANTILEVER
CB	CONCRETE BEAM
CC	CONCRETE COLUMN
CFMF	COLD FORMED METAL FRAMING
CFS	COLD-FORMED STEEL
CIP	CAST-IN-PLACE
CJ	CONSTRUCTION JOINT (OR CONTROL JOINT)
℄, CL	CENTERLINE
CLR	CLEAR(ANCE)
CM	CONSTRUCTION MANAGER
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONT	CONTINUOUS
CONTR	CONTRACTOR
COORD	COORD(INATE
CSK	COUNTER SINK
CTR	CENTER
CTRD	CENTERED
CY	CUBIC YARD
DBA	DEFORMED BAR ANCHOR
DIA	DIAMETER
DIAG	DIAGONAL
DL	DEAD LOAD
DN	DOWN
DTL	DETAIL
DWG	DRAWING
DWL	DOWEL
EA	EACH
EE	EACH END
EF	EACH FACE
EJ	EXPANSION JOINT
ENG	ENGINEER
EOS	EDGE OF SLAB
EL	ELEVATION
EQ	EQUAL
EQ SP	EQUAL SPACE(S)(ING)
ES	EACH SIDE
EW	EACH WAY
EXIST	EXISTING
EXP	EXPANSION
EXT	EXTERIOR
F/	FACE OF
FBC	FLORIDA BUILDING CODE
FD	FLOOR DRAIN
FDN	FOUNDATION
FF	FINISHED FLOOR
FIN	FINISH
FLR	FLOOR
FLG	FLANGE
FMG	FRAMING
FS	FAR SIDE
FT	FOOT
FTG	FOOTING
ga	GAUGE
GALV	GALVANIZE(D)
GB	GRADE BEAM
GC	GENERAL CONTRACTOR
GFC	GROUT FILLED CELL(S) / COURSE
GLB	GLU-LAM BEAM
GR	GRADE
GT	GIRDER TRUSS
HC	HOLLOW-CORE
HDG	HOT DIPPED GALVANIZED
HK	HOOK
HORIZ	HORIZONTAL
HP	HIGH POINT
HS	HEADED STUD
HSS	HOLLOW STRUCTURAL SECTION
ID	INSIDE DIAMETER
IF	INSIDE FACE
INT	INTERIOR
JST	JOIST
JT	JOINT
K	KIP
KO	KNOCK-OUT
lb, #	POUND(S)
LG	LONG
LL	LIVE LOAD
LLBB	LONG LEG BACK-TO-BACK
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LNTL	LINTEL
LSL	LONG SLOTTED
LOC	LOCATION
LONG	LONGITUDINAL
LP	LOW POINT
LRFD	LOAD & RESISTANCE FACTOR DESIGN
LW	LONG WAY
MAS	MASONRY
MATL	MATERIAL
MAX.	MAXIMUM
MB	MACHINE BOLT
MBM	METAL BUILDING MFR
MC	MASONRY COLUMN
MCJ	MASONRY CONTROL JT
MCN	MOMENT CONNECTION
ME	MONOLITHIC EDGE
MECH	MECHANICAL
MEZZ	MEZZANINE
MF	MONOLITHIC FOOTING
MFR	MANUFACTURE(ER)
MIN	MINIMUM
MISC	MISCELLANEOUS
MO	MASONRY OPENING
MS	METAL STUD
MTL	METAL
MUF	MONOLITHIC WALL FOOTING
NIC	NOT IN CONTRACT
NS	NEAR SIDE
NTS	NOT TO SCALE
OA	OVERALL
OC	ON CENTER
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
OH	OPPOSITE HAND
OHD	OVERHEAD
OPNG	OPENING
OPP	OPPOSITE
PAF	POWDER-ACTUATED FASTENER(S)
PB	PRE-CAST BEAM
PEMB	PRE-ENGINEERED METAL BUILDING
PERP	PERPENDICULAR
PC	PRECAST
PL, PLT	PLATE
PLS	PLACES
PLF	POUNDS PER LINEAR FOOT
PLYWD	PLYWOOD
PNL	PANEL
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PTN	PARTITION
R	RADIUS
RC	REINFORCED CONCRETE
REF	REFERENCE
REINF	REINFORCE(D)(ING)
REQ	REQUIRE
REQ'D	REQUIRED
RF	ROOF
RM	ROOM
RO	ROUGH OPENING
RP	RADIUS POINT
RTN	RETURN
R/W	REINFORCED WITH
SCH	SCHEDULE
SDI	STEEL DECK INSTITUTE
SECT	SECTION
SF	STEPPED FOOTING
SHT	SHEET
SHW	SHEAR WALL
SIM	SIMILAR
SJ	SAWCUT JOINT
SJI	STEEL JOIST INSTITUTE
SL	SLOPE
SLBB	SHORT LEG BACK-TO-BACK
SOG	SLAB ON GRADE
SP	SPACE(S)
SPECS	SPECIFICATIONS
SQ	SQUARE
SS	STAINLESS STEEL
SSL	SHORT SLOTTED
STD	STANDARD
STIFFR	STIFFENER
STL	STEEL
STR	STRENGTH
STRL	STRUCTURAL
SW	SHORT WAY
SYMM	SYMMETRICAL
SYP	SOUTHERN YELLOW PINE
T&B	TOP AND BOTTOM
TB	TIE BEAM
TC	TIE COLUMN
TEMP	TEMPERATURE
THK	THICK
TOPG	TOPPING
TRANS	TRANSVERSE
TYP	TYPICAL
T/	TOP OF
UNO	UNLESS NOTED OTHERWISE
VIF	VERIFY IN FIELD
VERT	VERTICAL
WD	WOOD
WF	WALL FOOTING
WO	WINDOW OPENING (MASONRY)
WP	WORKING POINT
WS	WATERSTOP
WWF	WELDED WIRE FABRIC
W/	WITH

ABBREVIATIONS

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STATE OF FLORIDA
PROFESSIONAL ENGINEER

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ASTON HEALTH

PROPOSED:

OCEANCREST HEALTH AND REHABILITATION CENTER

117-BED NURSING & MEMORY CARE FACILITY

MINTON RD. @ HERITAGE OAKS BLVD.
WEST MELBOURNE, FL 32904

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PUBLICATION	DATE
SCHEMATIC DESIGN:	
DESIGN DEVELOPMENT:	04-02-25
AHCA CD REVIEW SET:	04-14-25
PERMIT SET:	
BID SET:	
FOR CONSTRUCTION:	
REVISIONS:	

TO THE BEST OF THE ARCHITECT'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH THIS SECTION AND CHAPTER 633, FLORIDA STATUTES.

PLOT DATE:

FILE LOCATION:

SHEET DESCRIPTION:

STRUCTURAL GENERAL NOTES AND ABBREVIATIONS

PROJECT NO.	SCALE:
1966	AS NOTED

DRAWING NO.

S0.2

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MASONRY

M1 MASONRY CONSTRUCTION MATERIALS AND INSPECTIONS SHALL CONFORM TO THE LATEST EDITION OF THE ACI BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES (TMS 402-2016), SPECIFICATIONS FOR MASONRY STRUCTURES (TMS 602-2016) ASTM C476-19, ASTM C1019-20 AND NCMA TEK 10-7.

M2 CONCRETE BLOCKS SHALL CONFORM TO ASTM C-90. (f'm = 2000 PSI) (2000 PSI ON THE NET AREA).

M3 MORTAR SHALL COMPLY WITH ASTM C270, TYPE 'M' FOR RETAINING WALLS AND WALLS BELOW GRADE, TYPE 'S' FOR TYPICAL WALLS. (COMPRESSIVE STRENGTH = 2500 PSI AND 1800 PSI, RESPECTIVELY. SITE TESTED MORTAR CUBES SHALL ACHIEVE A MINIMUM OF 80% OF THE DESIGN COMPRESSIVE STRENGTH)

M4 BLOCK SHALL NOT BE MOISTENED BEFORE GROUTING.

M5 ALL MASONRY CROSS WEBS SHALL BE FULLY BEDDED IN MORTAR AROUND CELLS TO BE GROUTED.

M6 REINFORCE WALLS WITH LADDER TYPE (ASTM A-153, #3 GAGE WIRE) DEFORMED REINFORCEMENT EQUAL TO DUR-O-WAL IN BED JOINTS AT 16" OC UNO, MEASURED VERTICALLY. PLACE PER MFR'S INSTRUCTIONS. LAP ALL HORIZONTAL JOINT REINFORCING 6" MIN.

M7 VERTICAL REINFORCING MUST HAVE A MINIMUM CLEARANCE OF 1/2" TO INSIDE FACE. VERTICAL REINFORCEMENT IN WALLS SHALL BE SECURED AND LATERALLY SUPPORTED AGAINST DISPLACEMENT AT INTERVALS NOT EXCEEDING 192 x (BAR DIAMETER) OR 10 FT (WHICHEVER IS LESS) WHENEVER A CLEANOUT IS REQUIRED. SEE GROUTING DETAIL NOTE FOR CLEANOUT REQUIREMENTS.

M8 ANY MASONRY THAT IS BELOW GRADE SHALL BE FULLY GROUTED UP TO FINISHED FLOOR.

M9 GROUT PLACEMENT STOPPED FOR (1) HOUR OR MORE SHOULD BE STOPPED 1 1/2" BELOW THE TOP OF THE MASONRY UNIT TO PROVIDE A KEY FOR SUBSEQUENT GROUTING.

M10 TYPICAL VERTICAL REINFORCING SIZE AND SPACING SHALL BE ABOVE AND BELOW ALL WALL OPENINGS.

M11 TEMPORARY BRACING AND SHORING OF WALLS TO PROVIDE STABILITY DURING CONSTRUCTION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

M12 REINFORCE MASONRY OPENINGS LESS THAN 2'-0" WIDE, WITH HORIZ JOINT REINF PLACED IN (2) HORIZ JOINTS APPROXIMATELY 8" APART, IMMEDIATELY ABOVE THE GROUT FILLED LINTEL AND IMMEDIATELY BELOW THE GROUT FILLED SILL. EXTEND REINFORCING A MINIMUM OF 2'-0" BEYOND JAMBS OF THE OPENING EXCEPT AT CONTROL JOINTS.

M13 PROVIDE FILLED PRECAST U-LINTELS AS MANUFACTURED BY CAST-CRETE OR APPROVED EQUAL WITH (2) #5 CONT AT ALL OPENINGS WHERE BEAMS ARE NOT SHOWN, SCHEDULED OR NOTED 2'-0" WIDE AND GREATER. LINTELS SHALL HAVE MINIMUM UNFILLED CAPACITY OF 400 lb/LF AND BEAR NOMINAL 8" (MIN 8") EACH END ON A GROUT FILLED CELL. PROVIDE PRE-CAST LINTEL MFR'S STANDARD TABULATED LOAD TABLES AS EVIDENCE THAT THE MINIMUM CAPACITIES AS LISTED IN THE BEAM SCHEDULE ARE SATISFIED. REFER TO MASONRY WALL BEAM SCHEDULE FOR TYPICAL PRECAST LINTEL SPANS AND DETAILS.

M14 STOPPING AND RESUMING WORK: RACK BACK 1/2-UNIT LENGTH IN EACH COURSE. DO NOT TOOTH. CLEAN EXPOSED SURFACES OF SET MASONRY. REMOVE LOOSE MASONRY UNITS AND MORTAR PRIOR TO LAYING FRESH MASONRY.

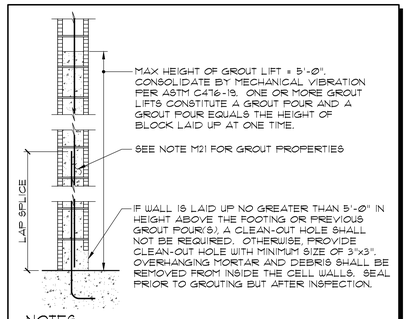
M15 DO NOT APPLY UNIFORM LOADS TO MASONRY WALLS FOR (3) DAYS.

M16 DO NOT APPLY CONCENTRATED LOADS TO MASONRY WALLS FOR (7) DAYS.

M17 EXTEND ALL VERTICAL WALL REINFORCEMENT TO WITHIN 2" OF TOP OF WALL OR BEAM UNLESS NOTED OTHERWISE. TERMINATE REINFORCING WITH STANDARD ACI 90 DEGREE HOOK IF ROOF JOISTS AND/OR TRUSSES BEAR ON TOP OF WALL AND THERE IS NO PARAPET. IF PARAPET EXISTS, HOOK IS NOT REQUIRED.

M18 REFER TO ARCHITECTURAL DRAWINGS FOR WATERPROOFING DETAILS AT MASONRY CONTROL JOINTS.

M19 GROUT FOR FILLED CELLS SHALL BE PLACED IN CONFORMANCE WITH ASTM C476-19 AND AS INDICATED BELOW:

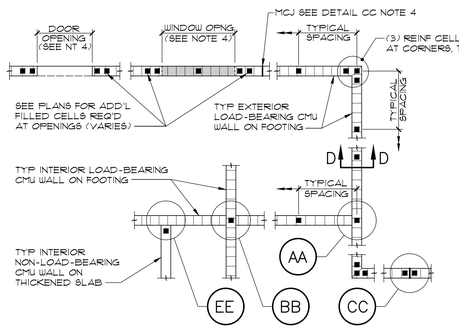


NOTES:

1. BUTTER AROUND ALL CELLS THAT ARE TO BE FILLED WITH GROUT TO ENSURE GROUT DOES NOT LEAK OUT.
2. DO NOT GROUT UNTIL MORTAR HAS SET SUFFICIENTLY TO WITHSTAND THE PRESSURE OF THE GROUT. WAIT NOT LESS THAN 24 HOURS.
3. PLACE GROUT WITHIN 90 MINUTES FROM INTRODUCING WATER IN THE MIXTURE AND PRIOR TO INITIAL SET.
4. MAXIMUM WALL HEIGHT FROM TOP OF FOOTING OR PREVIOUS GROUT POURS LAID UP AT ONE TIME SHALL BE 4'-0".
5. THE MINIMUM CONTINUOUS UNOBSTRUCTED CLEAR AREA IN CELL TO RECEIVE GROUT MUST BE NOT LESS THAN 3"x3". MORTAR FINS MUST BE REMOVED AS BLOCK PLACEMENT PROCEEDS. MORTAR DROPPINGS MUST BE KEPT OUT OF CELLS WHICH ARE TO BE GROUTED.

GROUTING DETAIL

M20 TYPICAL MASONRY DETAILS:

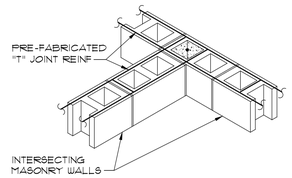


NOTES:

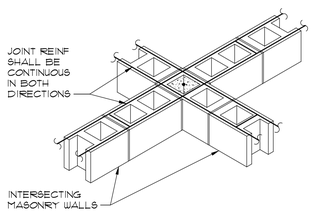
1. SEE DETAIL "CC" FOR LOCATING MASONRY CONTROL JOINTS. CONTRACTOR SHALL SUBMIT MCJ PLAN TO ARCHITECT FOR APPROVAL.
2. SEE ARCHITECTURAL DRAWINGS FOR OPENING SIZES AND LOCATIONS.
3. SEE FDN PLAN NOTES FOR REINFORCED FILLED CELL SIZE & SPACING.
4. MULTIPLE FILLED CELLS MAY BE REQUIRED AT JAMBS. ADDITIONAL BARS WILL BE SHOWN ON PLAN(S). IF NONE ARE SHOWN, THEN A SINGLE TYPICAL REINFORCED JAMB CELL IS SUFFICIENT.
5. SEE MASONRY NOTES ON GENERAL NOTE SHEETS FOR HORIZONTAL JOINT REINFORCING AND OTHER ADDITIONAL INFORMATION.

ILLUSTRATIVE PLAN OF VARIOUS CMU WALL CONDITIONS

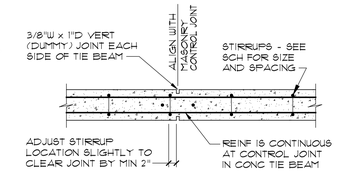
NTS



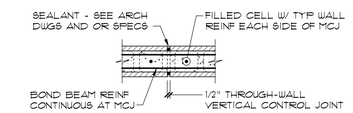
AA WALL INTERSECTION



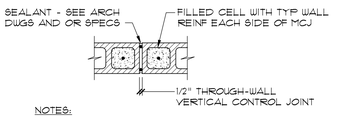
BB WALL INTERSECTION



MCJ @ TIE BEAM



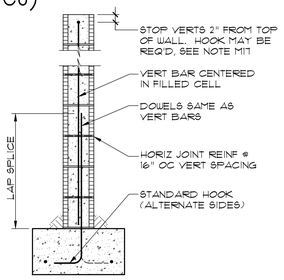
MCJ @ BOND BEAM



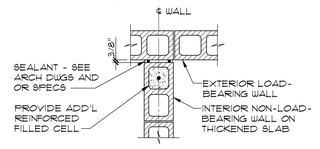
NOTES:

1. THROUGH-WALL JOINT SHALL BE CONTINUOUS WITHOUT INTERRUPTION FROM FOUNDATION TO TOP OF WALL.
2. TERMINATE TYPICAL HORIZONTAL JOINT REINFORCING 2" FROM JOINT.
3. MAXIMUM SPACING OF CONTROL JOINTS SHALL BE 1 1/2 x (WALL HEIGHT) OR 25'-0", WHICHEVER IS LESS.
4. MASONRY CONTROL JOINTS IN CMU WALLS SHALL NOT BE LOCATED WITHIN 16" OF ANY JAMB.

CC MAS CONTROL JOINT (MCJ)

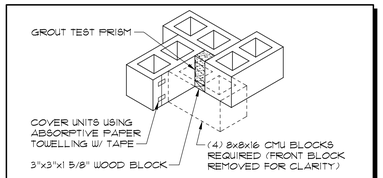


DD TYPICAL FILLED CELL DETAILS



EE INTERSECTION OF LOAD-BRG & NON-LOAD-BRG WALLS

M21 JOB SITE MIXING OF GROUT SHALL NOT BE PERMITTED. TESTING SHALL CONFORM TO ASTM C1019. SEE TEST MOLD DETAIL BELOW. SEE SCHEDULE BELOW FOR COMPRESSIVE STRENGTH AND SLUMP REQUIREMENTS.



DETAIL – MASONRY GROUT TEST MOLD (ASTM C-1019)

MASONRY GROUT SHALL HAVE THE FOLLOWING PROPERTIES:

LOCATION	28 DAY STRENGTH	SLUMP	COARSE AGGREGATE
FILLED CELL, PRECAST LINTELS & BOND BEAMS	2500 PSI	8" TO 11"	3/8"
HOLLOW METAL FRAMES, 4" PARTITION BLOCK	2500 PSI	8" TO 11"	NONE

NOTES:

1. ALL MASONRY GROUT SHALL CONFORM TO ASTM C 476.
2. COURSE AGGREGATES SHALL CONFORM TO ASTM C 404.
3. DO NOT TEST GROUT USING CYLINDERS AS THIS IS CONTRARY TO ASTM C476-19 AND IS STRICTLY PROHIBITED.

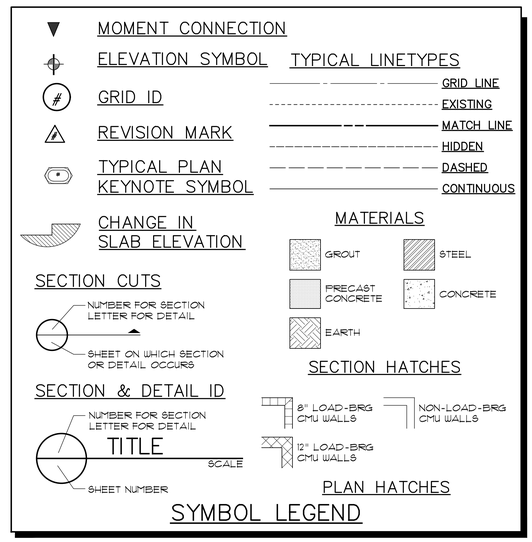
M22 MINIMUM LAP SPLICES FOR REINFORCED CMU PER EIGHTH EDITION (2023) FBC FOR LRFD DESIGNS.

CMU WALLS WITH CENTERED VERTICAL REINFORCING						
#4	#5	#6	#7	#8	#9 (NOTE 1)	#10 (NOTE 2)
15"	23"	35"	48"	72"	82"	

CMU WALLS WITH EACH FACE VERTICAL REINFORCING						
#4	#5	#6	#7	#8	#9 (NOTE 1)	#10 (NOTE 2)
23"	36"	54"	63"	72"	82"	

NOTES:

1. #9 BARS ARE NOT ALLOWED IN 8" CMU BUT ACCEPTABLE FOR 10" AND 12" CMU. MAXIMUM BAR DIAMETER SHALL NOT EXCEED ONE-EIGHTH OF THE NOMINAL WALL THICKNESS.
2. #10 BARS SHALL BE SPLICED USING MECHANICAL CONNECTORS AND SHALL ONLY BE ALLOWED IN 12" CMU.
3. EPOXY COATED BARS SHALL NOT BE USED.



SYMBOL LEGEND

SYMBOL LEGEND

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117-BED NURSING & MEMORY CARE FACILITY

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TO THE BEST OF THE ARCHITECT'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH THIS SECTION AND CHAPTER 633, FLORIDA STATUTES.

PLOT DATE:

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SHEET DESCRIPTION:

STRUCTURAL GENERAL NOTES AND SYMBOL LEGEND

PROJECT NO. 1966 | SCALE: AS NOTED

DRAWING NO.

S0.3

File=F:\2024\24200\24200_TECH\24200_SDWGS\24200_Sdwgs\24200_S0.1.dwg Tab=S0.3 Date/Time= Oct 17, 2025 – 12:31pm Plot by dbaker

STRUCTURAL STEEL

S01 A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.

S02 FABRICATE AND ERECT STRUCTURAL STEEL IN CONFORMANCE WITH THE LATEST VERSION OF AISC 360-16.

S03 MATERIAL SPECIFICATIONS:

ALL STEEL SHALL BE PRODUCED DOMESTICALLY.

ROLLED SHAPES, PLATES AND BARS: ASTM A572 GR 50, EXCEPT WIDE-FLANGE ≤ WT SECTIONS, WHICH SHALL BE ASTM A992.

HOLLOW STRUCTURAL SECTION (HSS): ASTM A500, GRADE C.

ANCHOR BOLTS, RODS, NUTS AND WASHERS: PER BASE PLATE SCHEDULE.

HEADED STUDS: ASTM A108, GRADE 1015 THROUGH 1020, COLD-FINISHED CARBON STEEL, AWS D1.1, TYPE B.

BOLTED STRUCTURAL CONNECTIONS: UNLESS NOTED OTHERWISE, ALL BOLTS SHALL BE 3/4"ø ASTM A325, TYPE N. BOLTS INDICATED LESS THAN 5/8"ø SHALL BE ASTM A307.

WELDED CONNECTIONS: ELECTRODES - E70XX UNO (LOW HYDROGEN). FILLET WELDS SHALL BE 3/16" UNO.

S04 ALL WELDING SHALL CONFORM TO THE REQUIREMENTS OF "THE STANDARD CODE FOR WELDING IN BUILDING CONSTRUCTION" OF THE AMERICAN WELDING SOCIETY.

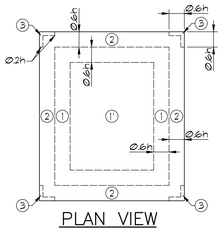
S05 GROUT FOR COLUMN BASE PLATES AND PRESET BEARING PLATES SHALL BE NON-SHRINK, NON-METALLIC GROUT (5000 PSI MIN.)

S06 SUBMIT SHOP DRAWINGS INDICATING ALL SHOP AND ERECTION DETAILS INCLUDING PROFILES, SIZES, SPACING AND LOCATIONS OF STRUCTURAL MEMBERS, CONNECTION ATTACHMENTS, FASTENERS, LOADS AND TOLERANCES.

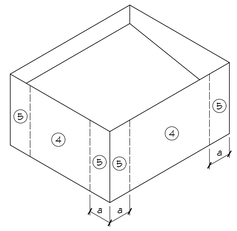
S07 ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIP GALVANIZED IN ACCORDANCE WITH ASTM A123 FOR MEMBERS AND ASTM A153 FOR CONNECTION ELEMENTS, EXCEPT THAT ALL ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS) SHALL BE BLAST CLEANED AND COATED IN ACCORDANCE WITH THE STRUCTURAL STEEL AND PAINT SPECIFICATIONS.

S08 STRUCTURAL STEEL SHALL RECEIVE A SHOP COAT OF PRIMER (COLOR AS DIRECTED BY ARCHITECT) EXCEPT THOSE AREAS WHICH WILL RECEIVE SPRAY-ON FIRE PROTECTION, OR WHERE HEADED STUDS ARE TO BE WELDED.

S09 THE STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN FULL CONFORMANCE WITH THE "OSHA STEEL ERECTION STANDARD". IF THE CONSTRUCTION DRAWINGS DEVIATE FROM THE OSHA STANDARD THEN THE FABRICATOR SHALL PROVIDE SUBMITTALS THAT CLEARLY INDICATE THE DEVIATION WITH A REVISION CLOUD AND REQUEST APPROVAL FROM BBM TO MAKE THE CHANGE SO THAT CONFORMANCE WITH THE OSHA STANDARD IS ASSURED.



PLAN VIEW
(ROOF)



NOTES:

1. A (Kd) OF Ø.85 HAS BEEN USED IN THE DEVELOPMENT OF THESE VALUES. THE USE OF THESE VALUES SHALL ONLY BE APPLIED WHEN USED IN CONJUNCTION WITH LOAD COMBINATIONS SPECIFIED IN SECTIONS 2.3 & 2.4 OF ASCE 7-22.
2. PRESSURES AND SUCTIONS ON SOFFITS SHALL BE THE SAME AS CORRESPONDING WALL ZONES 4 & 5.
3. IF THE STRUCTURE IS AN EHPA OR IS INSURED BY FACTORY MUTUAL (FM) THE GROSS UPLIFT DESIGN PRESSURES SHOWN HEREIN SHALL BE DOUBLED FOR ROOF COVERINGS.
4. THE "ULTIMATE" WIND LOADS SHOWN IN THE COMPONENTS AND CLADDING SCHEDULE SHALL BE MULTIPLIED BY Ø.6 TO REDUCE THEM DOWN TO "SERVICE" LEVEL FOR ALL TESTED ASSEMBLIES, INCLUDING BUT NOT LIMITED TO DOORS, WINDOWS AND ROOF ASSEMBLIES.
5. PARAPET WIND LOADS ARE BASED ON A "SOLID" PARAPET WITH NO INTERNAL PRESSURE. ACTUAL PARAPET CONSTRUCTION MAY DICTATE THAT INTERNAL PRESSURE BE ADDED TO THE EXTERNAL PRESSURE IN ACCORDANCE WITH ASCE 7-22 SECTION 30.6.
6. UPLIFT VALUES FOR ZONE 2 MAY BE USED IN LIEU OF ZONE 3 FOR CORNER ZONES IF PARAPET HEIGHT WITH RESPECT TO FINISHED ROOF IS GREATER THAN 3 ft.
7. FOR ATTACHED CANOPIES ON BUILDING, REFERENCE ASCE 7-22 SECTION 30.9 FOR PRESSURE COEFFICIENTS.

WIND DESIGN CRITERIA

Ultimate Design Wind Speed	159 mph
Service Design Wind Speed	123.2 mph
Risk Category	III
Exposure Category	C
Enclosure Classif.	Enclosed Building
Internal Pressure Coef.	+/-0.18

COMPONENT AND CLADDING ULTIMATE WIND PRESSURES

h = 15.0 ft a = 9.0 ft

Roof	Surface Pressure (psf)			
Area	10 sf	100 sf	500 sf	1000 sf
Negative Zone 1	-88.0	-88.7	-55.3	-55.3
Negative Zone 1'	-50.6	-50.6	-34.2	-27.2
Negative Zone 2	-116.1	-91.3	-74.0	-74.0
Negative Zone 3	-158.3	-108.7	-74.0	-74.0
Positive All Zones	22.5	17.8	17.8	17.8
Overhang Zone 1&1'	-79.6	-74.9	-46.8	-46.8
Overhang Zone 2	-107.7	-74.6	-51.5	-51.5
Overhang Zone 3	-148.8	-92.0	-51.5	-51.5

Overhang pressures in the table above assume an internal pressure coefficient (Gcpi) of 0.0
Overhang soffit pressure equals adj wall pressure (which includes internal pressure of 8.4 psf)

Parapet	Solid Parapet Pressure (psf)					
Area	10 sf	20 sf	50 sf	100 sf	200 sf	500 sf
CASE A: Zone 2 :	164.0	153.3	134.9	128.7	118.1	104.0
Zone 3 :	210.1	191.3	166.4	147.7	128.9	104.0
CASE B: Interior zone :	-96.8	-91.9	-85.5	-80.6	-75.7	-69.2
Corner zone :	-110.7	-103.3	-93.6	-86.2	-78.9	-69.2

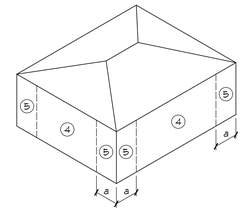
Wall	Surface Pressure (psf)			
Area	10 sf	100 sf	200 sf	500 sf
Negative Zone 4	-54.8	-47.3	-46.1	-42.1
Negative Zone 5	-67.4	-52.5	-48.1	-42.1
Positive Zone 4 & 5	50.6	43.1	40.9	37.9

TORNADO DESIGN CRITERIA

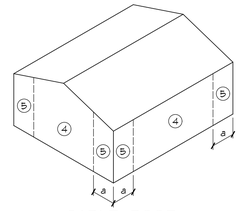
Tornado Wind Speed	85 mph
Effecting Area	163,618 sf
Internal Pressure Coef.	0.55
	-0.18

TORNADO DESIGN NOT REQUIRED

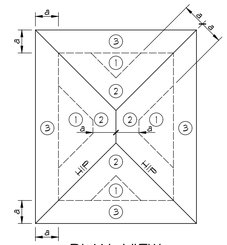
WIND SCHEDULE
MONOSLOPE



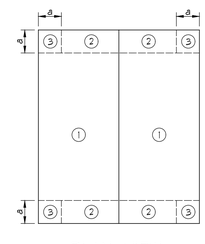
HIP ROOF



GABLE ROOF



PLAN VIEW
(HIP ROOF)



PLAN VIEW
(GABLE ROOF)

NOTES:

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4. THE "ULTIMATE" WIND LOADS SHOWN ABOVE SHALL BE MULTIPLIED BY Ø.6 TO REDUCE THEM DOWN TO "SERVICE" LEVEL FOR ALL TESTED ASSEMBLIES, INCLUDING BUT NOT LIMITED TO DOORS, WINDOWS AND ROOF ASSEMBLIES.
5. FOR ATTACHED CANOPIES ON BUILDING, REFERENCE ASCE 7-22 SECTION 30.9 FOR PRESSURE COEFFICIENTS.

WIND DESIGN CRITERIA

Ultimate Design Wind Speed	159 mph
Service Design Wind Speed	123.2 mph
Risk Category	III
Exposure Category	C
Enclosure Classif.	Enclosed Building
Internal Pressure Coef.	+/-0.18

COMPONENT AND CLADDING ULTIMATE WIND PRESSURES

h = 32.3 ft a = 13.5 ft

Roof	Surface Pressure (psf)			
Area	10 sf	100 sf	200 sf	300 sf
Negative Zone 1	-119.4	-63.8	-47.0	-37.2
Negative Zone 2	-157.7	-86.2	-64.6	-64.6
Negative Zone 3	-207.0	-109.4	-109.4	-109.4
Positive All Zones	42.7	30.1	26.3	26.3
Overhang Zone 1	-164.8	-104.5	-84.8	-73.3
verhang Zone 2 ezone 4	-208.1	-126.9	-102.4	-100.7
Overhang Zone 2 ezone 5	-224.6	-133.7	-106.3	-102.9
Overhang Zone 3	-257.4	-141.2	-146.2	-144.5

Overhang pressures in the table above assume an internal pressure coefficient (Gcpi) of 0.0
Overhang soffit pressure equals adj wall pressure (which includes internal pressure of 9.9 psf)

Wall	Surface Pressure (psf)			
Area	10 sf	100 sf	200 sf	500 sf
Negative Zone 4	-70.1	-60.4	-57.5	-53.7
Negative Zone 5	-86.5	-67.2	-61.4	-53.7
Positive Zone 4 & 5	64.6	55.0	52.0	48.2

TORNADO DESIGN CRITERIA

Tornado Wind Speed	85 mph
Effecting Area	163,618 sf
Internal Pressure Coef.	0.55
	-0.18

TORNADO DESIGN NOT REQUIRED

WIND SCHEDULE
HIP/GABLE

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PLOT DATE:

FILE LOCATION:

SHEET DESCRIPTION:

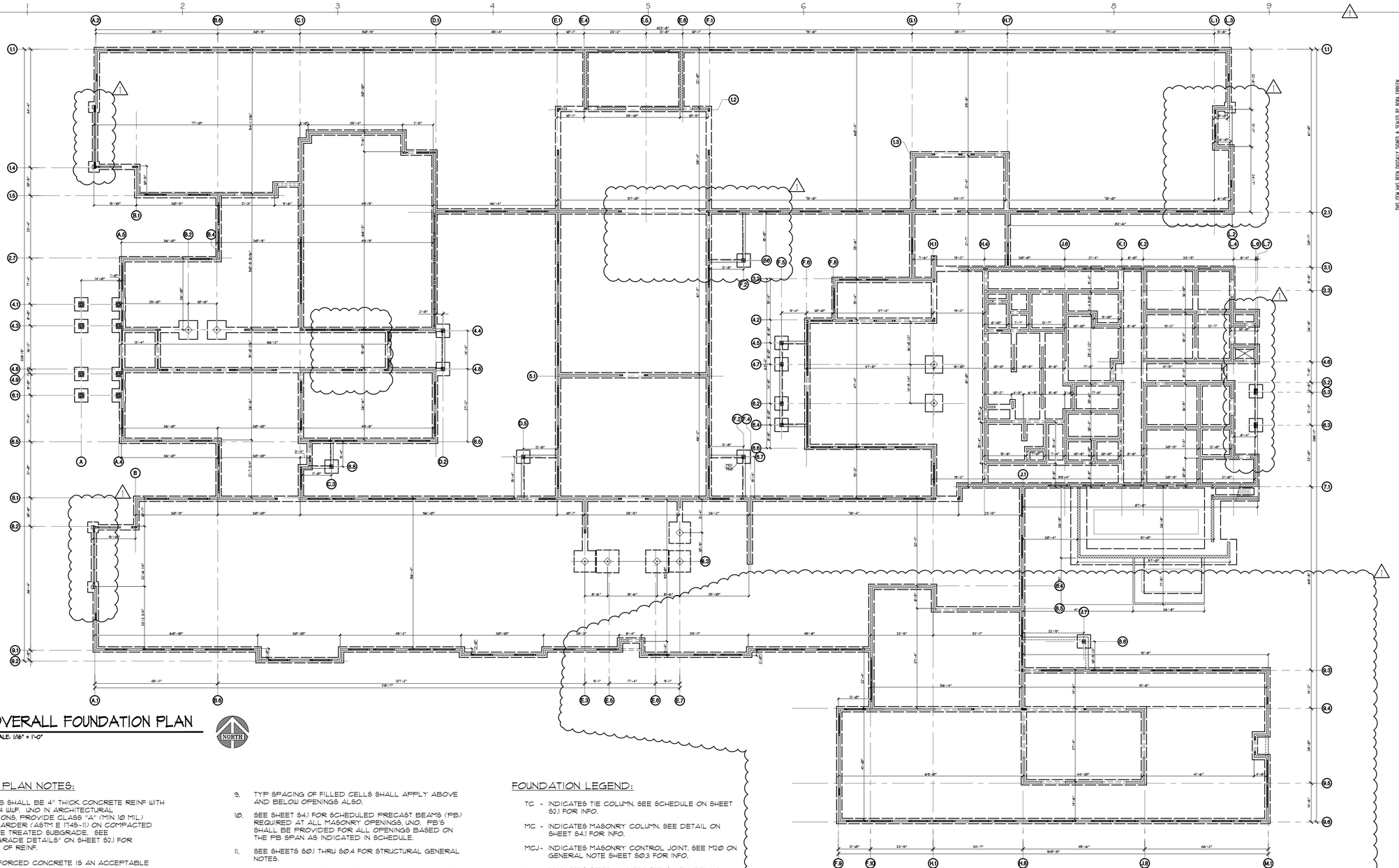
STRUCTURAL GENERAL NOTES AND WIND SCHEDULE

PROJECT NO.	SCALE:
1966	AS NOTED

DRAWING NO.

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1 OVERALL FOUNDATION PLAN

SCALE: 1/16" = 1'-0"

NORTH

FOUNDATION PLAN NOTES:

1. FLOOR SLAB SHALL BE 4" THICK CONCRETE REINF WITH 6X6-W1.4XW1.4 WWF. UNO IN ARCHITECTURAL SPECIFICATIONS, PROVIDE CLASS "A" (MIN 10 MIL) VAPOR RETARDER (ASTM E 1745-11) ON COMPACTED AND TERMITE TREATED SUBGRADE. SEE "SLAB-ON-GRADE DETAILS" ON SHEET S2.1 FOR PLACEMENT OF REINF.

1a. FIBER-REINFORCED CONCRETE IS AN ACCEPTABLE ALTERNATIVE TO WELDED-WIRE FABRIC. REINFORCED CONCRETE FIBERS SHALL BE 100% VIRGIN POLYPROPYLENE FIBRILLATED FIBERS AS MANUFACTURED BY FIBER MESH CO. (OR APPROVED EQUAL) APPLIED AT A RATE OF 1 1/2 lbs/CY.

2. T/ SLAB EL = Ø'-Ø" (TYP, UNO). REFERENCE ONLY - SEE CIVIL DWGS FOR ACTUAL ELEVATION.

3. T/ WALL FTG EL = -1'-4" (TYP, UNO).

4. T/ COL FTG EL = -1'-4" (TYP, UNO).

5. ALL CMU BEARING WALLS ARE 8" OR 12" (TYP, UNO).

6. STEP AND/OR LOWER FOUNDATIONS WHERE SHOWN AND AS NECESSARY TO AVOID INTERFERENCE WITH OTHER TRADES. SEE CONCRETE GENERAL NOTES FOR DETAILS AND SECTIONS. PARTICULAR ATTENTION SHALL BE PAID TO DOWNSPOUTS ENSURING THAT PROPER ACTIONS HAVE BEEN TAKEN TO PREVENT PIPES FROM CONFLICTING WITH THE FOUNDATION SYSTEM.

7. ALL FTGS ARE CENTERED BENEATH BEARING WALLS AND COLUMNS (TYP, UNO).

8. REINF 8" LOAD-BEARING CMU WALLS WITH #6 VERT BAR CENTERED IN GROUT-FILLED CELL AT ENDS, CORNERS AND AT MAX SPACING OF 16" OC. REINF 12" LOAD-BEARING CMU WALLS WITH #6 VERT BAR EA FACE IN GROUT-FILLED CELL AT ENDS, CORNERS AND AT MAX SPACING OF 16" OC. PROVIDE (3) REINF CELLS AT ALL GIRDER BEARING LOCATIONS. ADDITIONAL FILLED CELLS REQUIRED AT JAMBS ARE SHOWN THUS "■" ON EACH STRUCTURAL PLAN. SEE "ILLUSTRATIVE PLAN OF VARIOUS CMU WALL CONDITIONS" ON SHEET S0.3 FOR ADDITIONAL REQUIRED FILLED CELLS AND OTHER INFORMATION.

   GC NOTE:
   INTERIOR NON-LOAD-BEARING WALLS ARE NOT SHOWN IN THEIR ENTIRETY HEREIN. SEE ARCH'L DWGS FOR LOCATIONS OF ALL INTERIOR NON-LOAD-BEARING WALLS AND THE FOUNDATION PLAN FOR THE APPROPRIATE SECTION.

9. TYP SPACING OF FILLED CELLS SHALL APPLY ABOVE AND BELOW OPENINGS ALSO.

10. SEE SHEET S4.1 FOR SCHEDULED PRECAST BEAMS (PB) REQUIRED AT ALL MASONRY OPENINGS, UNO. PB'S SHALL BE PROVIDED FOR ALL OPENINGS BASED ON THE PB SPAN AS INDICATED IN SCHEDULE.

11. SEE SHEETS S0.1 THRU S0.4 FOR STRUCTURAL GENERAL NOTES.

12. MAINTAIN STRUCTURAL SLAB THICKNESS AT ALL FLOOR SLOPES AND DEPRESSIONS.

13. THE CONTRACTOR SHALL COORDINATE ALL UNDERGROUND UTILITIES, PIPES, ETC... WITH THE FOUNDATION PLAN AND FOUNDATION ELEVATIONS. FOOTING PENETRATION DETAILS MAY BE FOUND IN THE CONCRETE AND REINFORCING SECTION OF THE STRUCTURAL GENERAL NOTES.

FOUNDATION PLAN KEY NOTES:

(1) (2) #4x4'-0" LONG @ 3" OC PLACED 2" CLR FROM CORNER, CENTERED IN SLAB (TYP. WHERE SHOWN).

(2) SLAB DEPRESSION, SEE DETAIL B/S2.1 AND REFER TO ARCH DWGS FOR EXACT SIZE, LOCATION AND DEPTH OF DEPRESSION.

FOUNDATION LEGEND:

TC - INDICATES TIE COLUMN. SEE SCHEDULE ON SHEET S2.1 FOR INFO.

MC - INDICATES MASONRY COLUMN. SEE DETAIL ON SHEET S4.1 FOR INFO.

MCJ - INDICATES MASONRY CONTROL JOINT. SEE M20 ON GENERAL NOTE SHEET S0.3 FOR INFO.

C - INDICATES STEEL COLUMN. SEE SCHEDULE ON SHEET S4.1 FOR INFO.

F - INDICATES PAD FOOTING. SEE FOUNDATION SCHEDULE ON SHEET S4.1 FOR INFO.

WF - INDICATES WALL FOOTING. SEE FOUNDATION SCHEDULE ON SHEET S4.1 FOR INFO.

ME - INDICATES MONOLITHIC EDGE. SEE FOUNDATION SCHEDULE ON SHEET S4.1 FOR INFO.

NOTE TO GC:
LOCATION OF MASONRY CONTROL JOINTS (MCJ) SHALL BE COORDINATED WITH ARCHITECTURAL DRAWINGS AND SHALL NOT EXCEED REQUIREMENTS OUTLINED IN NOTE M20 ON SHT S0.3. FOR ADDITIONAL FILLED CELLS REQUIRED AT MASONRY CONTROL JOINTS AND NOT SHOWN ON FOUNDATION PLANS, SEE "ILLUSTRATIVE PLAN OF VARIOUS CMU WALL CONDITIONS" ON SHT S0.3. SUBMIT MCJ PLAN TO ARCHITECT FOR APPROVAL.

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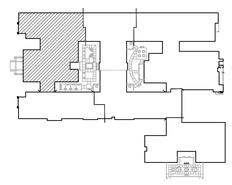
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PARTIAL FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

NORTH



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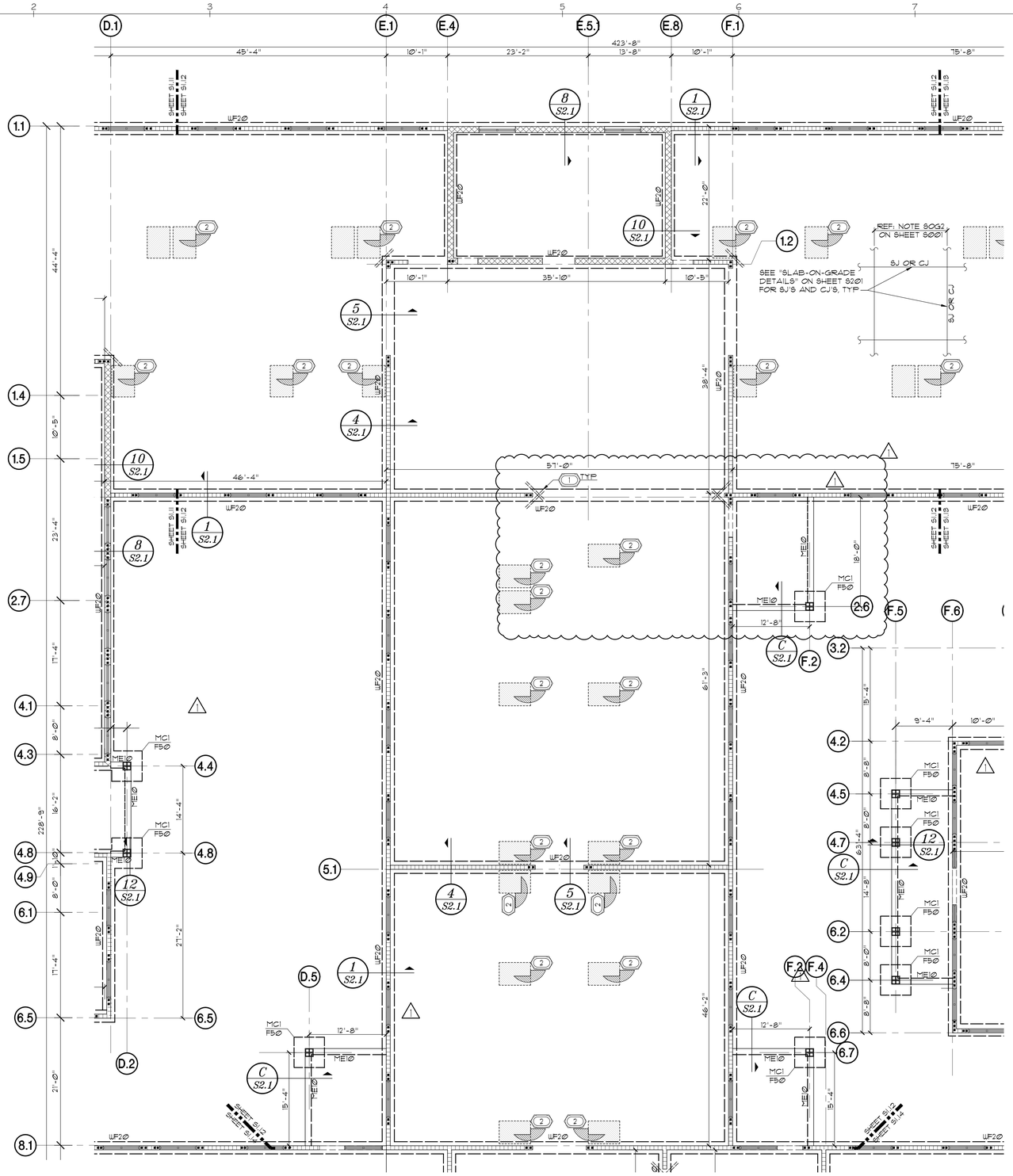
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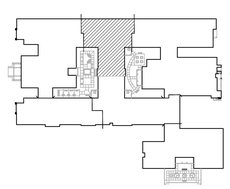
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1 PARTIAL FOUNDATION PLAN
SCALE: 1/8" = 1'-0"

NORTH



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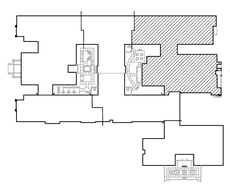
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PARTIAL FOUNDATION PLAN

1 SCALE: 1/8" = 1'-0"

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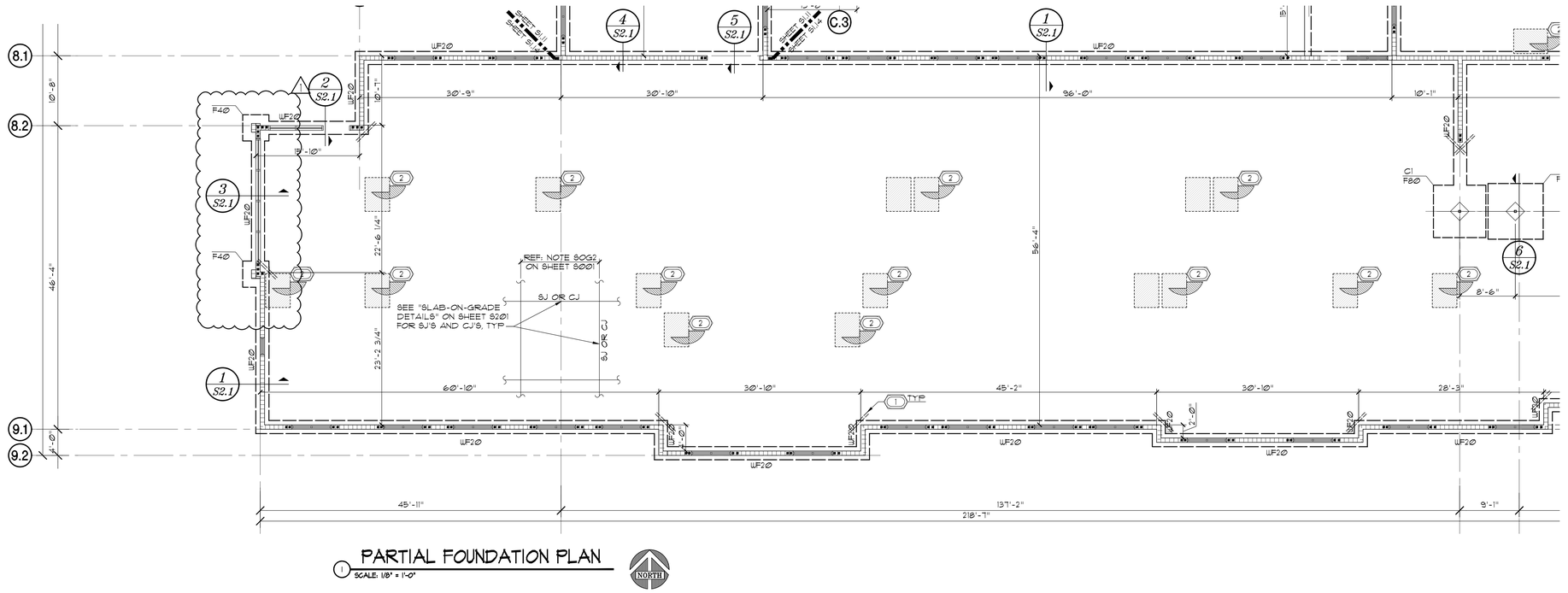
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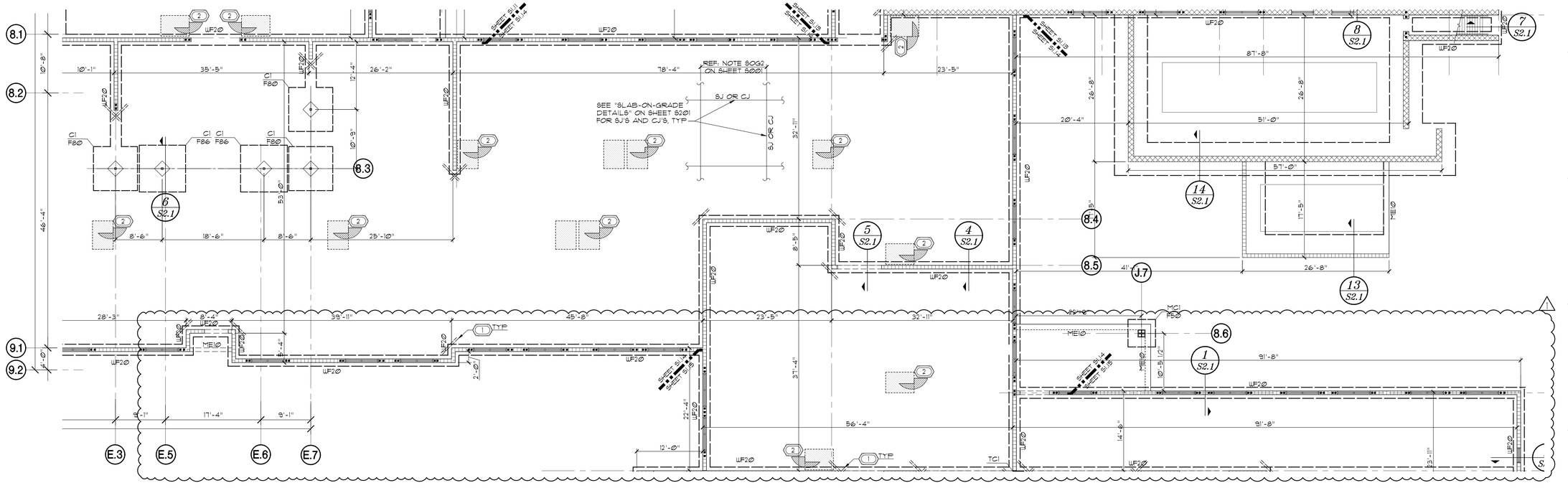
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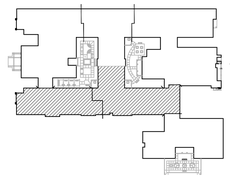
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1 PARTIAL FOUNDATION PLAN
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2 PARTIAL FOUNDATION PLAN
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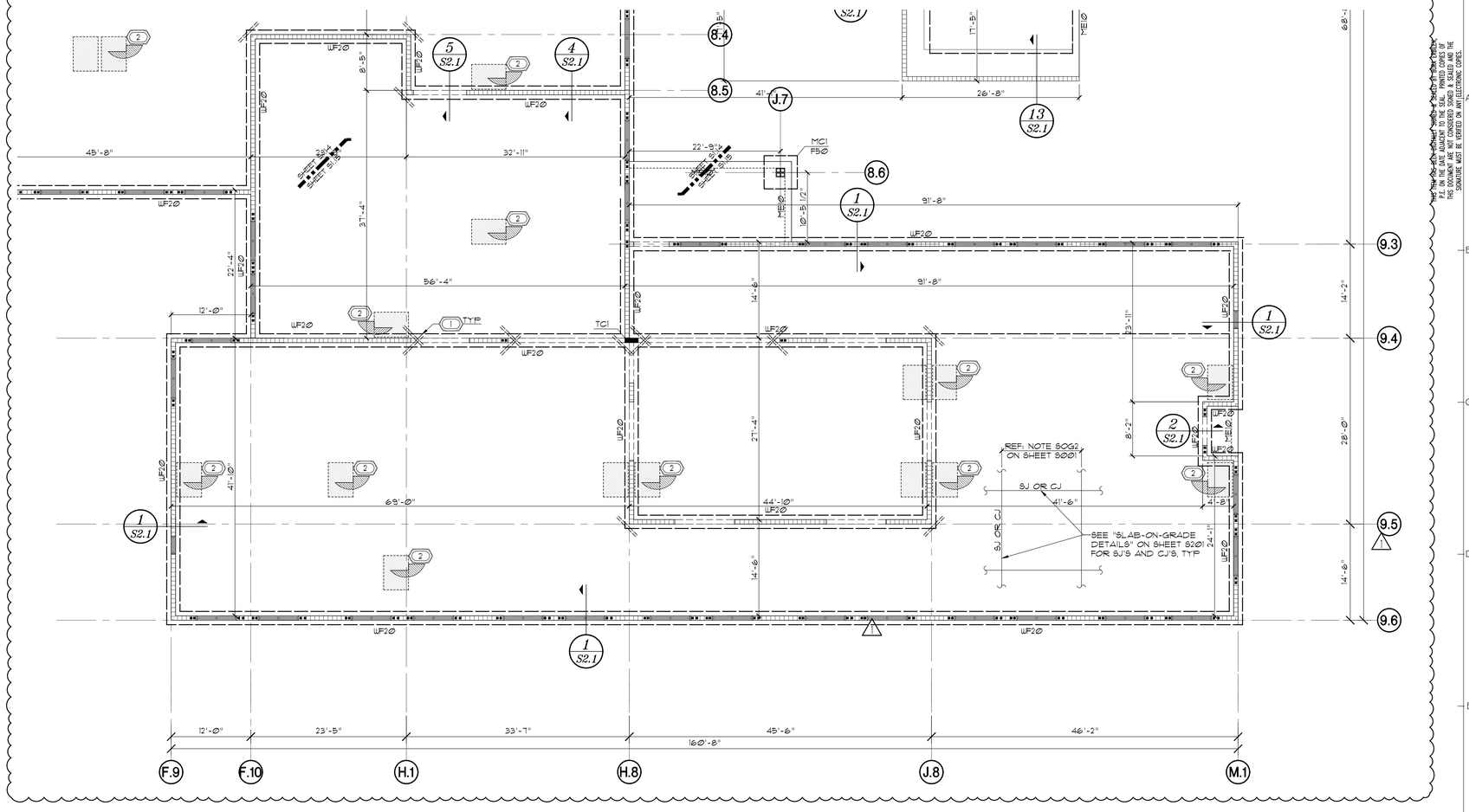
PARTIAL FOUNDATION PLAN

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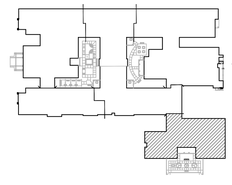
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PARTIAL FOUNDATION PLAN

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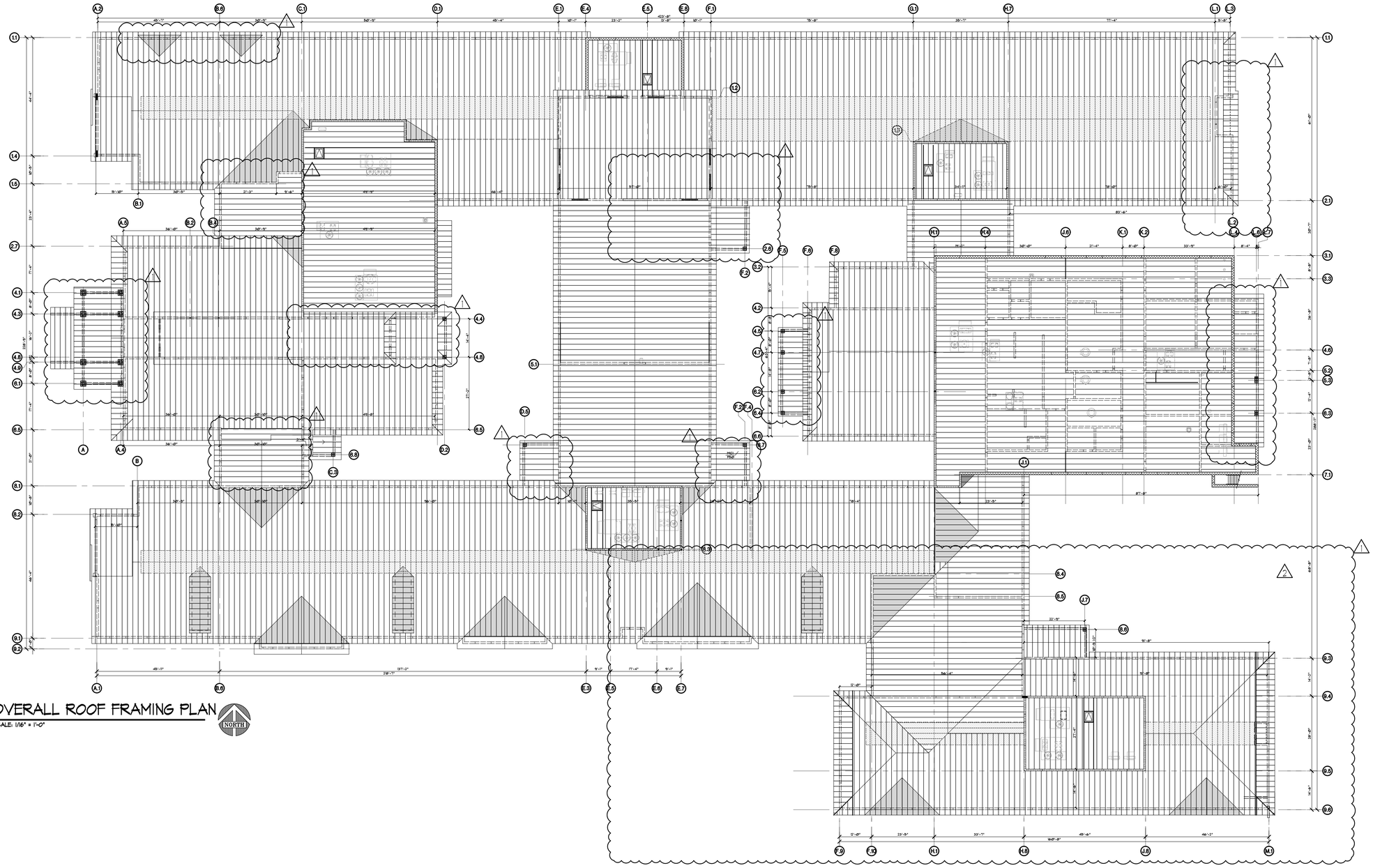
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1 OVERALL ROOF FRAMING PLAN

SCALE: 1/16" = 1'-0"

WOOD ROOF FRAMING PLAN NOTES:

1. ALL WOOD TRUSSES SHALL BE DESIGNED AND CERTIFIED BY TRUSS MANUFACTURER'S REGISTERED ENGINEER. ALL TRUSS-TO-TRUSS & TRUSS-TO-STRUCTURE CONNECTIONS SHALL BE DESIGNED AND DETAILED BY THE TRUSS MFR. CONNECTIONS SHALL BE DESIGNED FOR DEAD, LIVE AND WIND (BOTH UPLIFT & LATERAL) LOADS. THE WIND PRESSURE AND SUCTION EXERTED ON THE WALLS MAY BE FOUND IN THE WIND SCHEDULES.
2. TRUSS MANUFACTURER SHALL VERIFY ALL DIMENSIONS AND SUBMIT SHOP DRAWINGS TO ARCHITECT FOR APPROVAL.
3. SEE ARCHITECTURAL BUILDING SECTIONS, STRUCTURAL FRAMING PLANS AND TRUSS ELEVATIONS FOR ROOF PITCHES (TYP.).
4. BEARING PLATE HEIGHTS VARY. SEE ARCHITECTURAL BUILDING SECTIONS, WALL SECTIONS AND STRUCTURAL FRAMING PLAN(S) FOR BEARING HEIGHTS.
5. TRUSS SPACING SHALL BE @ 24" OC (UNO). HAND FRAMING SHALL BE @ 24" OC (UNO).
6. TRUSS MANUFACTURER SHALL PROVIDE ALL GABLE END TRUSSES WITH INTERMEDIATE STUD MEMBERS @ 16" OC AS REQUIRED TO SPAN FROM BOTTOM OF TRUSS TO ROOF.
7. OVERHANGS MAY VARY. SEE STRUCTURAL FRAMING PLAN(S) AND ARCHITECTURAL BUILDING SECTIONS AND WALL SECTIONS.
8. REFER TO ARCHITECTURAL DRAWINGS FOR FRAMED DOWN CEILINGS, VOLUME CEILINGS AND OTHER INTERIOR TREATMENTS.
9. ROOF DECK SHALL BE 19/32" A.P.A. RATED, EXPOSURE "1" TYPE PLYWOOD SHEATHING WITH 32/16 (MIN) SPAN RATING CONNECTED TO WOOD TRUSSES WITH 10d COMMON NAILS @ 6" OC EVERYWHERE.
10. BLOCKING SHALL BE PROVIDED UNDER ALL HIPS & VALLEYS WHERE DISCONTINUOUS DECK ENDS ARE NOT SUPPORTED BY A TRUSS.
11. DIAPHRAGM BLOCKING: UNO ON PLANS, PROVIDE DIAPHRAGM BLOCKING AT EDGES OF ALL OPENINGS IN THE DIAPHRAGM.
12. PROVIDE SHEATHING SUPPORT CLIPS BETWEEN TRUSSES AT UNBLOCKED PANEL EDGES (SIMPSON PSCL OR EQUAL).
13. COORDINATE SIZE AND LOCATION OF ROOFTOP UNITS WITH MECHANICAL DWGS (IF REQUIRED).
14. BEAM END REACTIONS AS SHOWN ON PLAN ARE ULTIMATE REACTIONS (ALREADY FACTORED).
15. MASONRY BOND BEAMS AND TIE BEAMS THAT ARE INDICATED ON THE PLAN SHALL BE CONTINUOUS FOR THE ENTIRE LENGTH OF THE WALL, UNO.

ROOF LEGEND:

BB - INDICATES BOND BEAM, SEE SCHEDULE ON SHEET S4.1 FOR INFO.

CB - INDICATES CONCRETE BEAM, SEE SCHEDULE ON SHEET S4.1 FOR INFO.

TB - INDICATES TIE BEAM, SEE SCHEDULE ON SHEET S4.1 FOR INFO.

PB - INDICATES PRECAST BEAM, SEE SCHEDULE ON SHEET S4.1 FOR INFO.

NOTE TO GC / TRUSS SPECIALTY ENGINEER:
ALL TEMPORARY AND PERMANENT BRACING FOR THE TRUSS SYSTEM SHALL BE DESIGNED BY A SPECIALTY ENGINEER. PROVIDE SIGNED AND SEALED SHOP DRAWINGS FOR REVIEW AND APPROVAL.

TRUSS DESIGN BASIS

DEAD LOAD (PSF)		LIVE LOAD (PSF)
TOP CHORD	BOTTOM CHORD	TOP CHORD
13	7	20 UNREDUCIBLE

NOTES:
1. SEE ROOF FRAMING PLAN FOR CONCENTRATED AND/OR SPECIAL LOADS.
2. TRUSSES SUPPORTING OVERFRAMING SHALL HAVE 5 PSF ADDED TO TOP CHORD DEAD LOAD.
3. ROOF STRUCTURE HAS BEEN DESIGNED TO SUPPORT THE WEIGHT OF PONDED WATER CAUSED BY CLOGGING OF THE PRIMARY ROOF DRAINS FOR A DEPTH OF (4) INCHES.

WOOD ROOF FRAMING PLAN KEY NOTES:

1. SHADED AREAS ON ROOF INDICATE PRE-ENGINEERED OVER-FRAMING TRUSSES.
2. HATCHED AREAS ON ROOF INDICATE RECESSES IN THE PRE-ENGINEERED WOOD TRUSSES. PLEASE SEE ARCHITECTURAL SECTIONS AND REFLECTED CEILING PLANS FOR ELEVATION CHANGES.
3. ROOF HATCH, SEE ARCHITECTURAL DRAWINGS.
4. ROOFTOP EQUIPMENT, REF MECHANICAL DRAWINGS FOR SIZE, WEIGHT LOCATIONS.

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PROJECT NO. 1966

SCALE: AS NOTED

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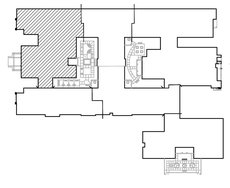
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PARTIAL ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTE TO GC:
CANTILEVERED BEAM CB4 SHALL BE SHORED/RE-SHORED FOR MINIMUM 56 DAYS



KEYPLAN

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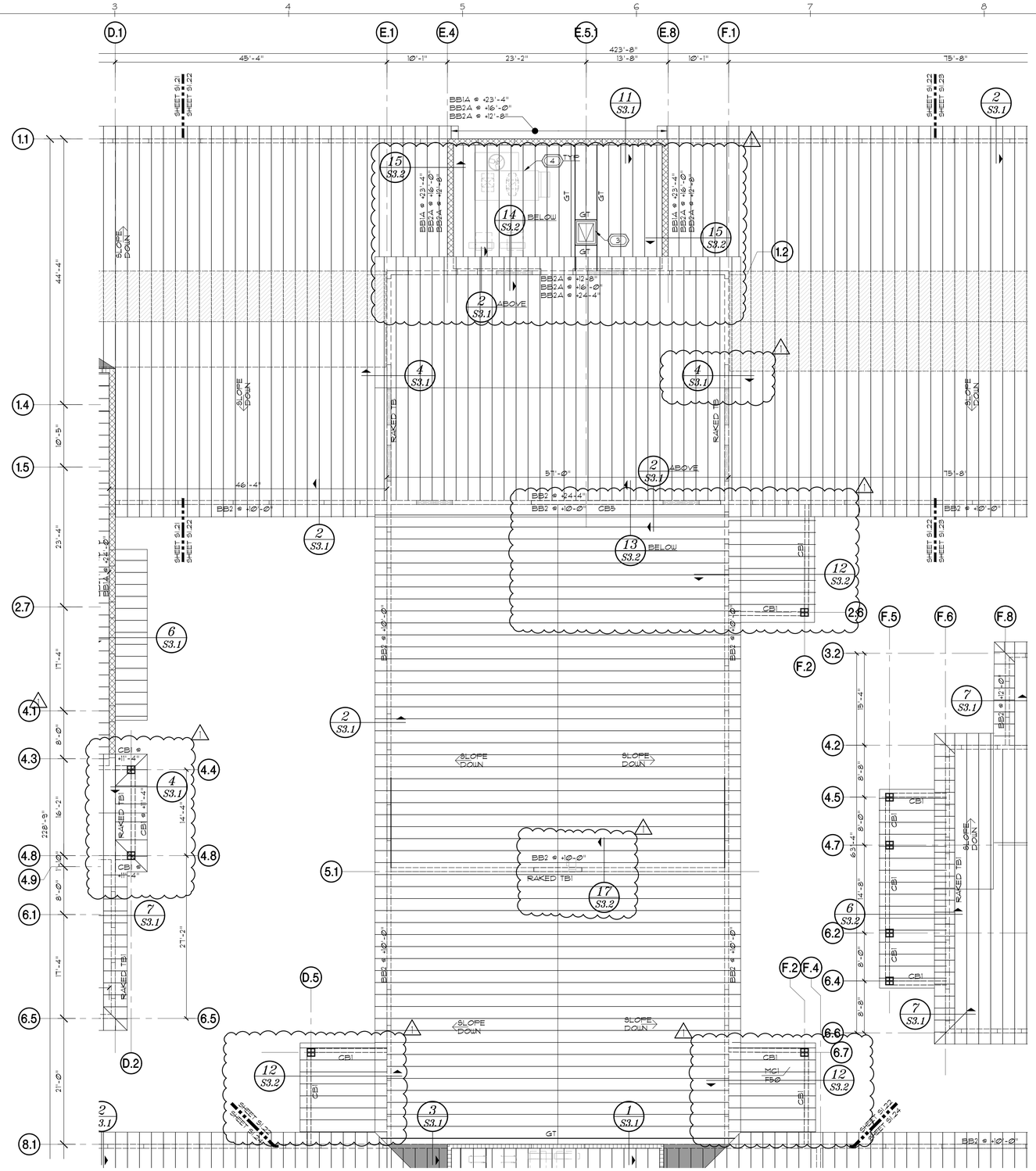
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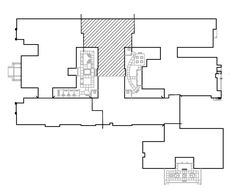
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PARTIAL ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

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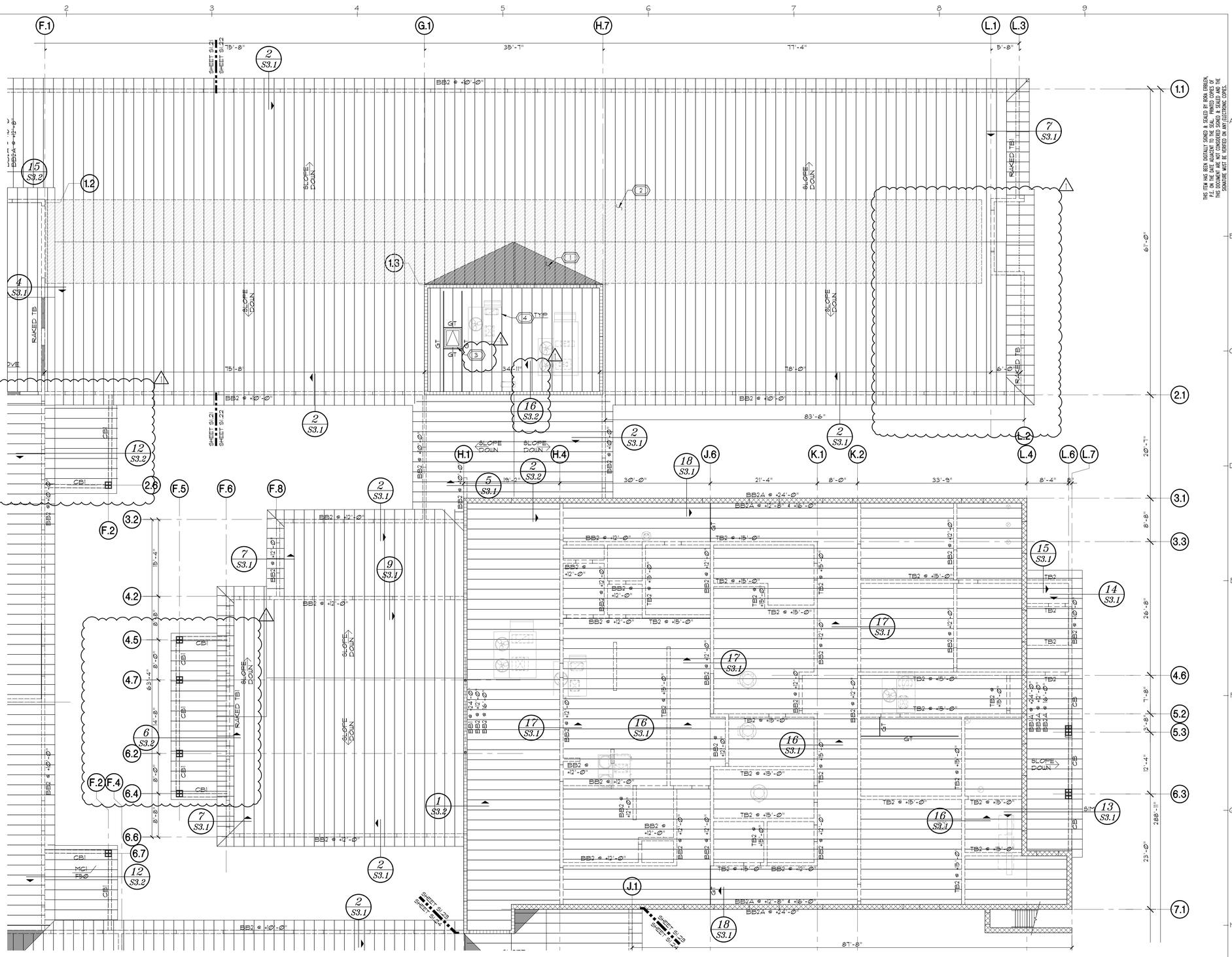
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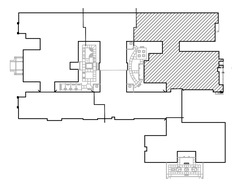
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PARTIAL ROOF FRAMING PLAN

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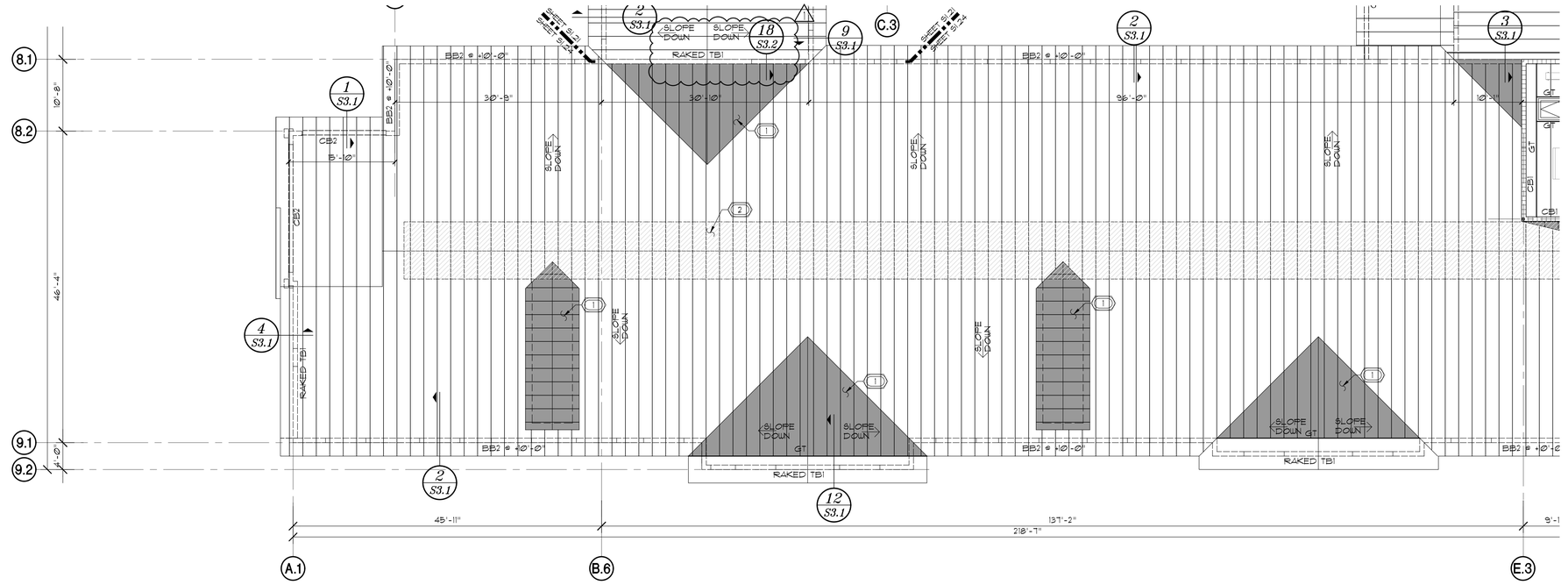
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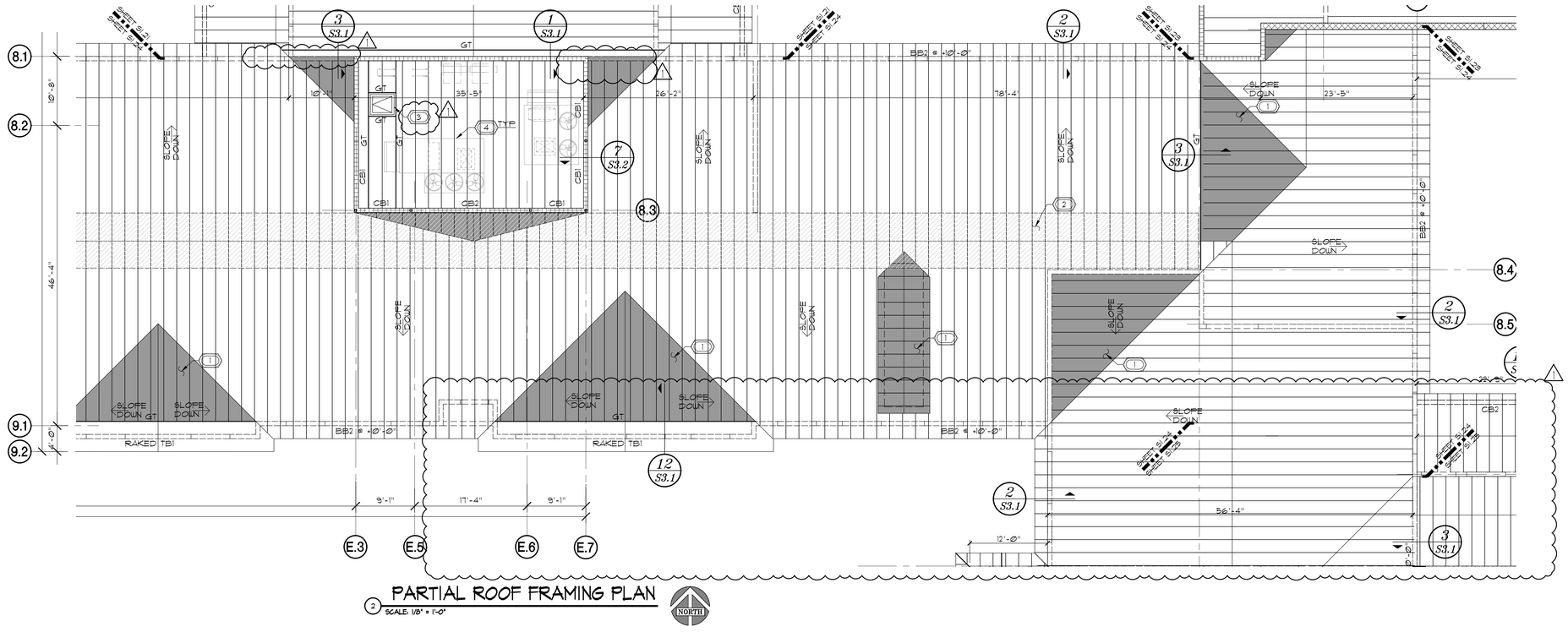
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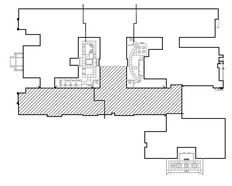
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1 PARTIAL ROOF FRAMING PLAN
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2 PARTIAL ROOF FRAMING PLAN
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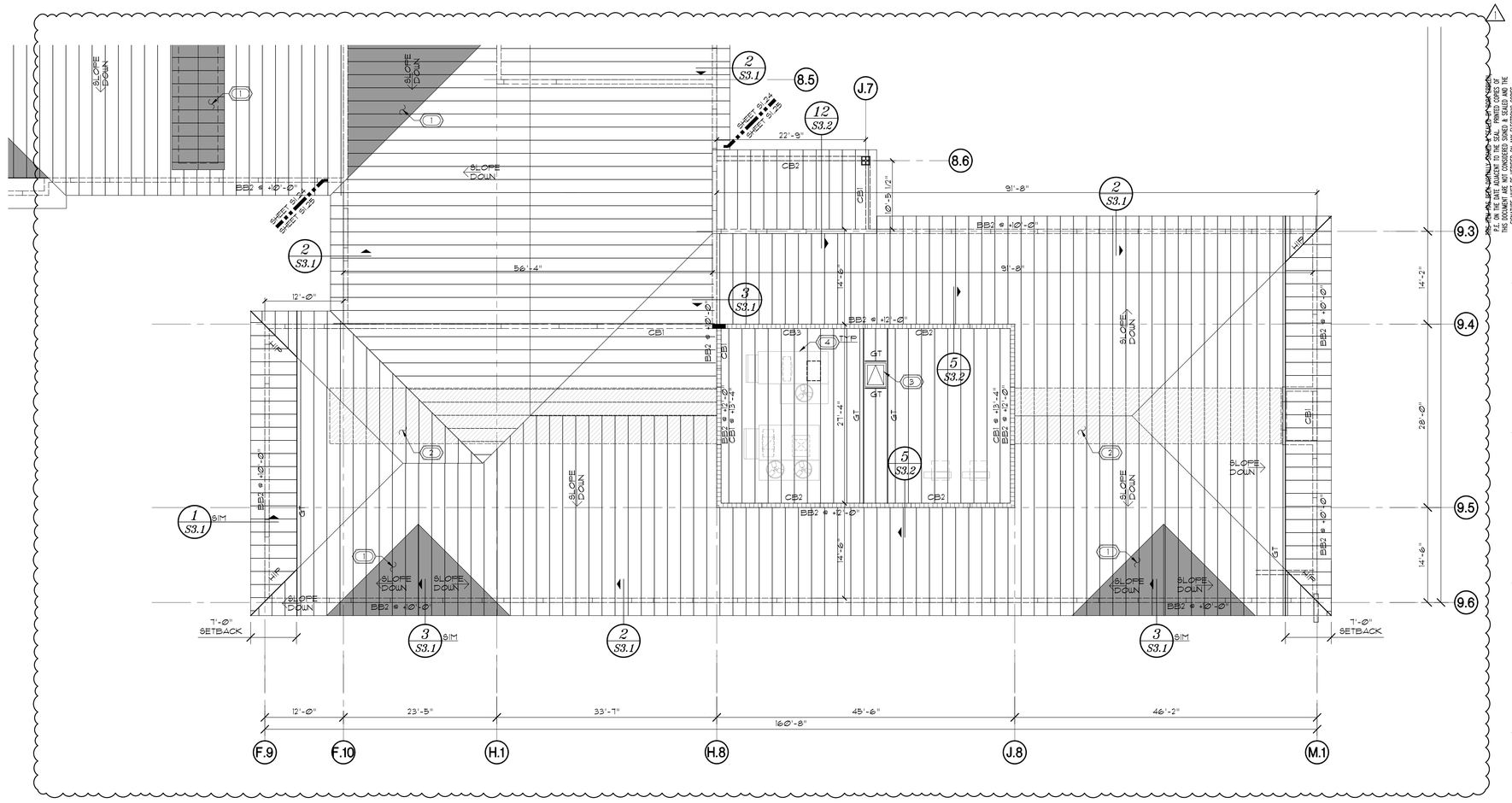
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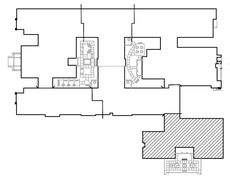
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1 PARTIAL ROOF FRAMING PLAN

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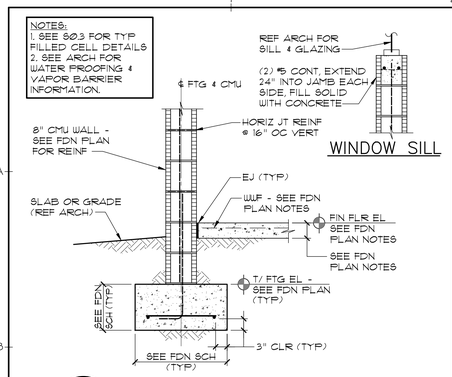
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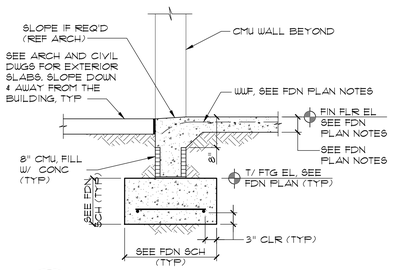
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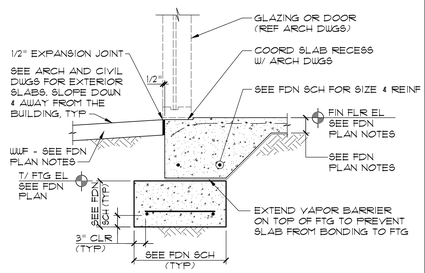
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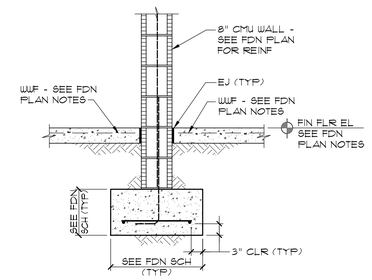
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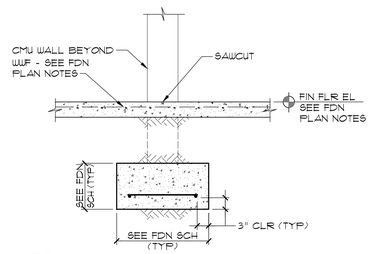
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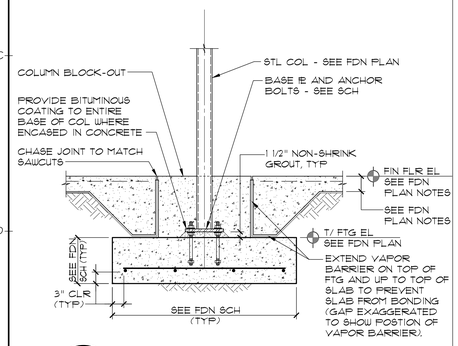
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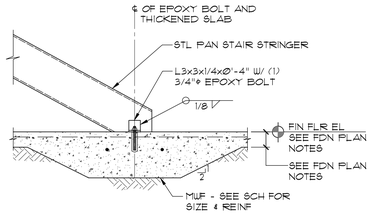
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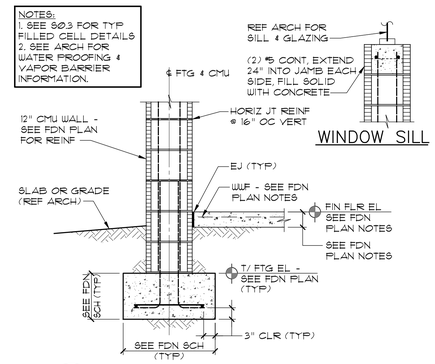
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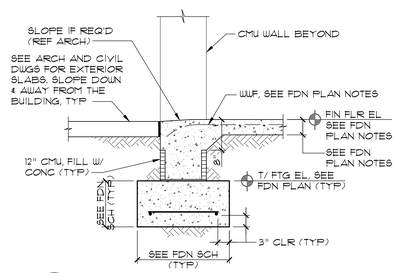
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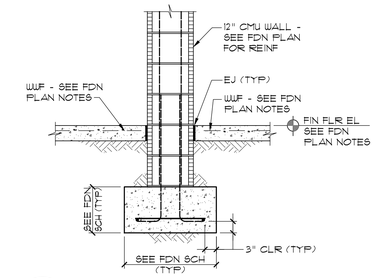
7 S2.1 SECTION @ STAIR STRINGER CONN TO SLAB



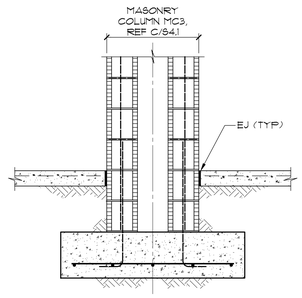
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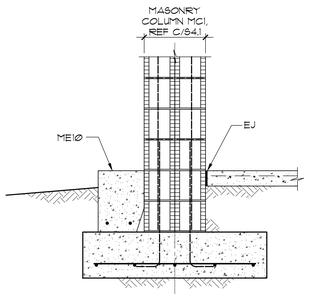
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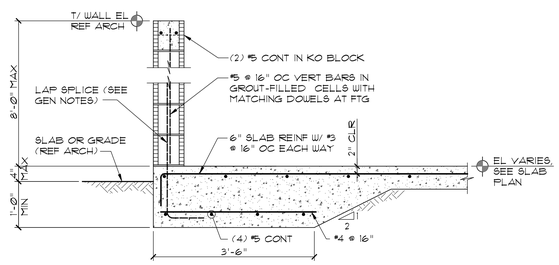
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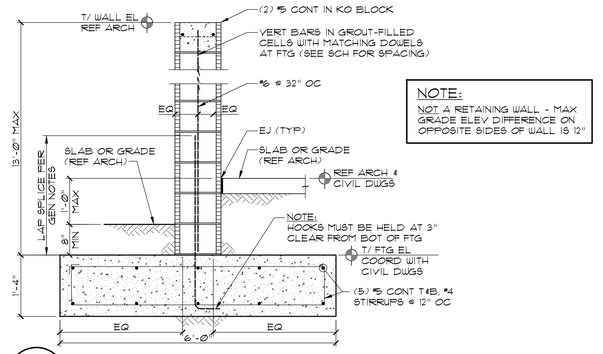
11 S2.1 SECTION @ MASONRY COLUMN



12 S2.1 SECTION @ MASONRY COLUMN

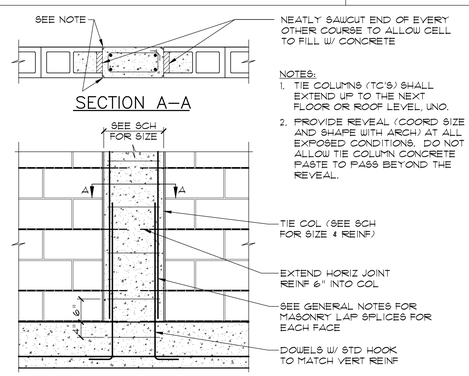


13 S2.1 SECTION @ TRASH COMPACTOR

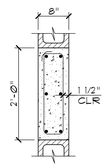


NOTE:
NOT A RETAINING WALL - MAX GRADE ELEV DIFFERENCE ON OPPOSITE SIDES OF WALL IS 12"

14 S2.1 SECTION @ GENERATOR ENCLOSURE



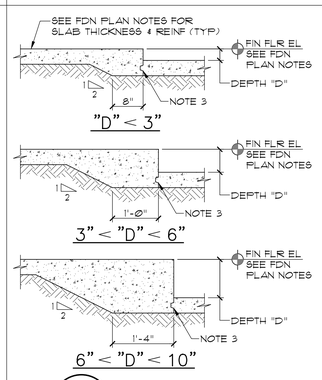
NOTES:
1. TIE COLUMNS (TC'S) SHALL EXTEND UP TO THE NEXT FLOOR OR ROOF LEVEL, UNO.
2. PROVIDE REVEAL (COORD SIZE AND SHAPE WITH ARCH) AT ALL EXPOSED CONDITIONS. DO NOT ALLOW TIE COLUMN CONCRETE PASTE TO PASS BEYOND THE REVEAL.



(6) #5 VERT & #3 TIES @ 8" OC

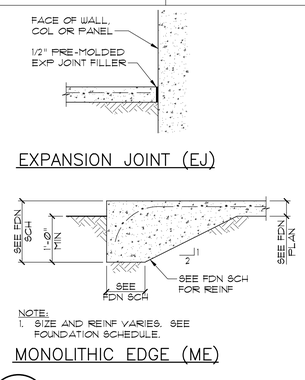
TIE COLUMN TYPES

A S2.1 DETAIL – TIE COLUMN

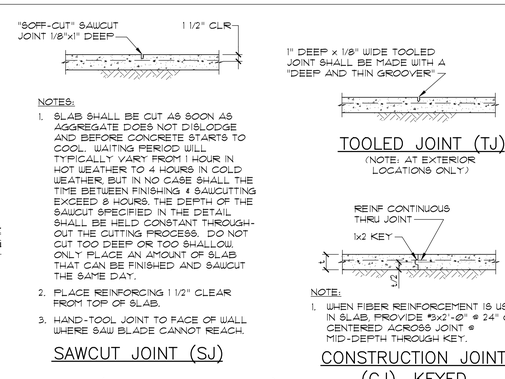


NOTES:
1. SEE FDN PLAN FOR LOCATIONS AND SLAB REINF.
2. COORD DEPRESSIONS W/ ARCH DWGS & DRAINS W/ PLUMBING DWGS.
3. 1x2 NOMINAL KEYWAY MAY BE DELETED IF SLAB IS CAST MONOLITHIC AT STEP.

B S2.1 SLAB–ON–GRADE FLR DEPRESSION DETAILS



NOTE:
1. SIZE AND REINF VARIES. SEE FOUNDATION SCHEDULE.



NOTES:
1. SLAB SHALL BE CUT AS SOON AS AGGREGATE DOES NOT DISLODGE AND BEFORE CONCRETE STARTS TO COOL. WAITING PERIOD WILL TYPICALLY VARY FROM 1 HOUR IN HOT WEATHER TO 4 HOURS IN COLD WEATHER, BUT IN NO CASE SHALL THE TIME BETWEEN FINISHING & SAWCUTTING EXCEED 8 HOURS. THE DEPTH OF THE SAWCUT SPECIFIED IN THE DETAIL SHALL BE HELD CONSTANT THROUGHOUT THE CUTTING PROCESS. DO NOT CUT TOO DEEP OR TOO SHALLOW. ONLY PLACE AN AMOUNT OF SLAB THAT CAN BE FINISHED AND SAWCUT THE SAME DAY.
2. PLACE REINFORCING 1 1/2" CLEAR FROM TOP OF SLAB.
3. HAND-TOOL JOINT TO FACE OF WALL WHERE SAW BLADE CANNOT REACH.

NOTE:
1. WHEN FIBER REINFORCEMENT IS USED IN SLAB, PROVIDE #3x2'-Ø" @ 24" OC CENTERED ACROSS JOINT @ MID-DEPTH THROUGH KEY.

C S2.1 SLAB–ON–GRADE DETAILS (FOOT TRAFFIC)

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REVISIONS:	

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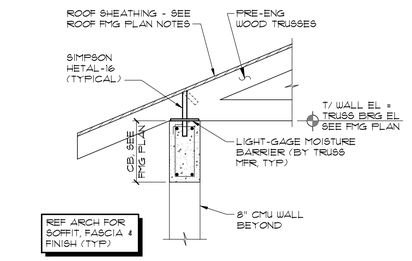
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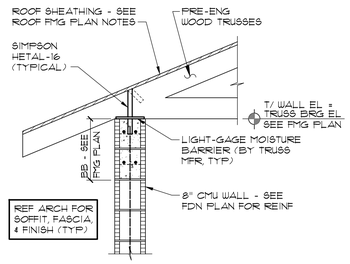
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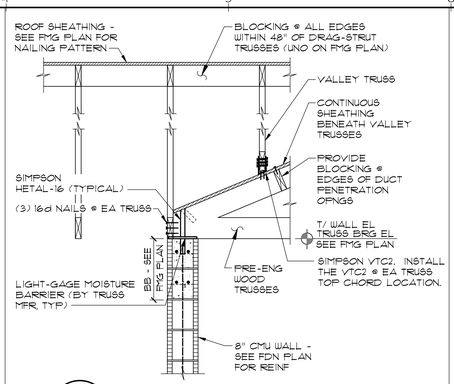
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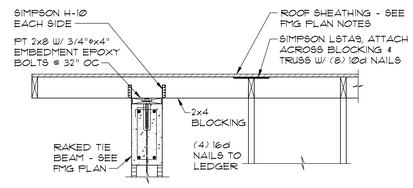
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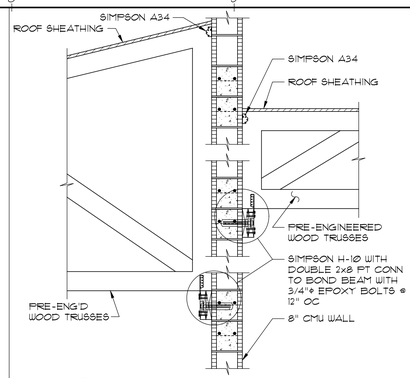
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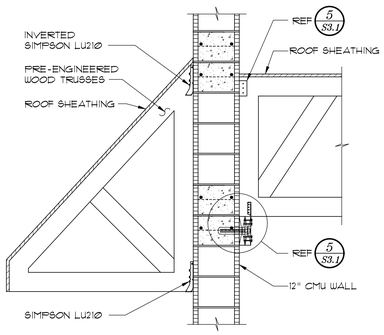
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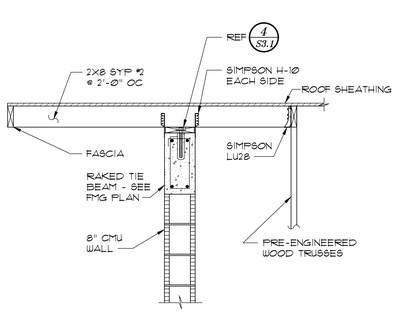
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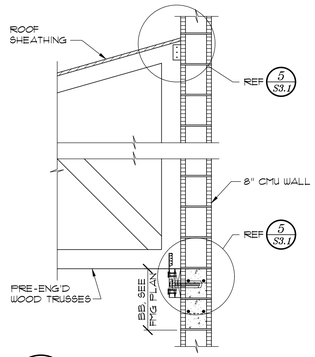
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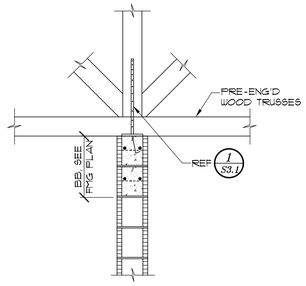
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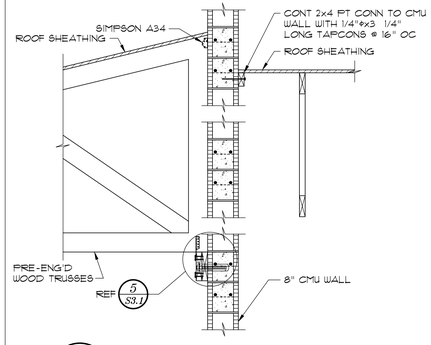
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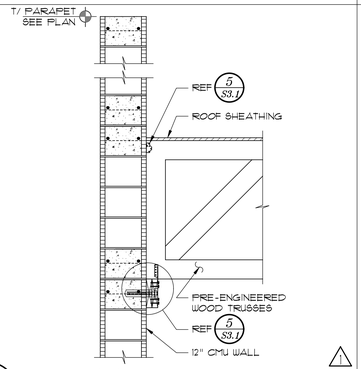
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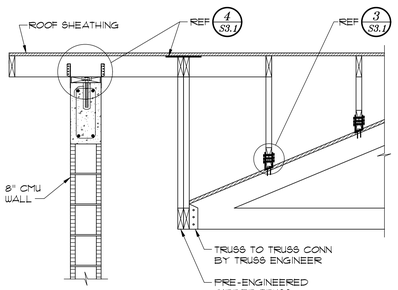
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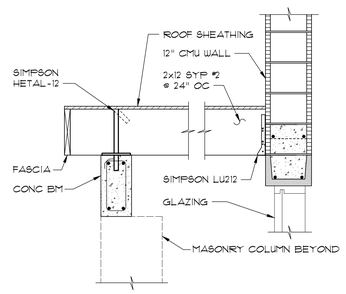
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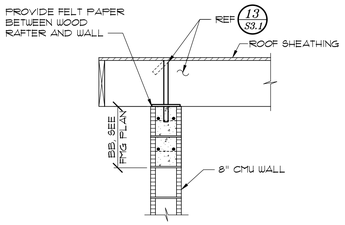
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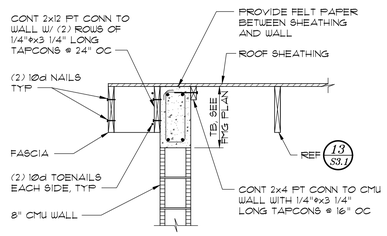
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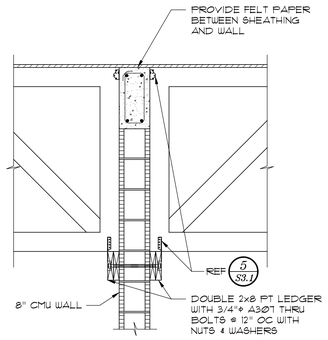
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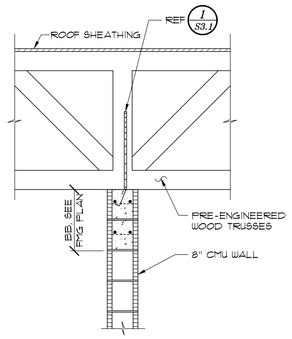
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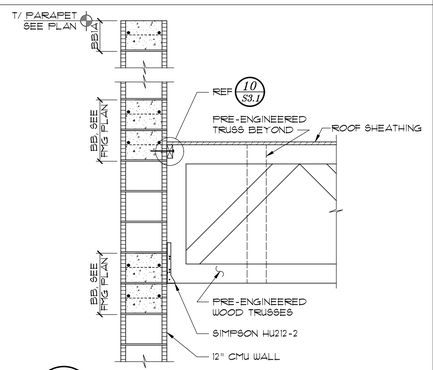
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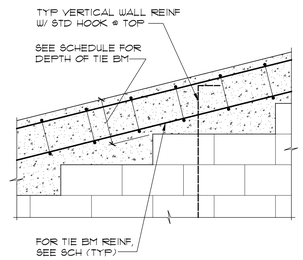
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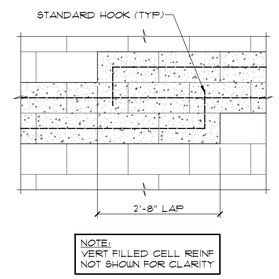
17 S3.1 SECTION @ TRUSS BEARING



18 S3.1 SECTION @ GIRDER TRUSS



A S3.1 DETAIL – RAKED TIE BEAM



B S3.1 DETAIL – STEPPED BOND BEAM

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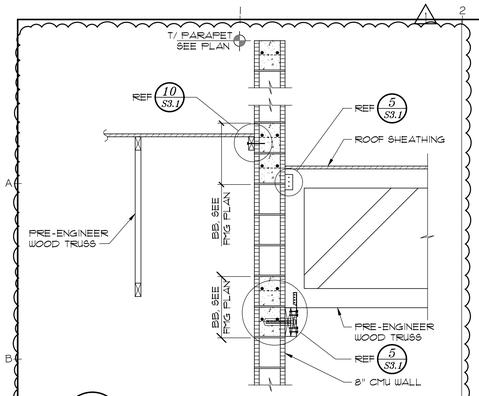
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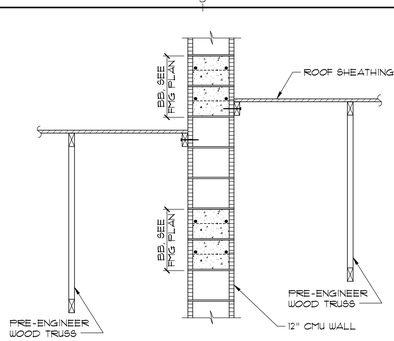
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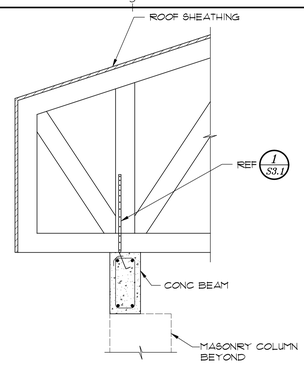
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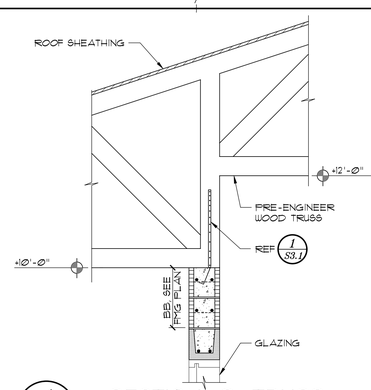
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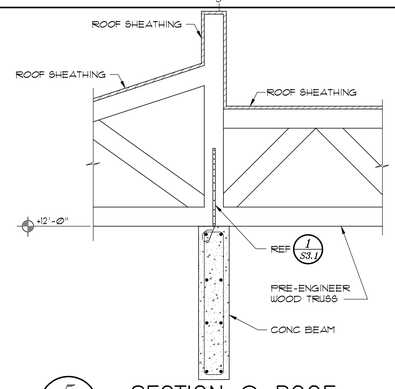
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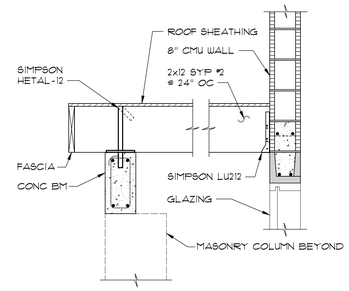
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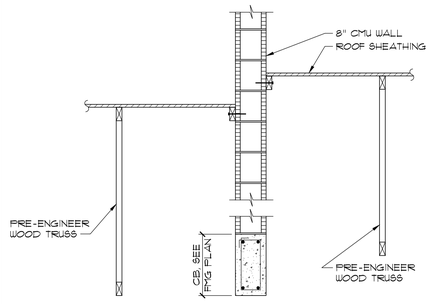
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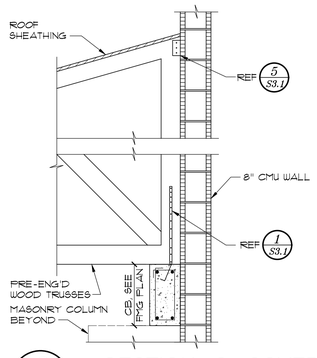
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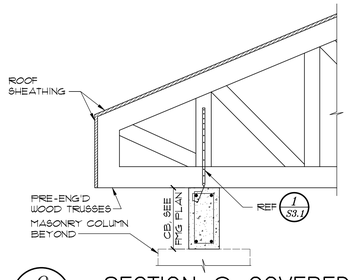
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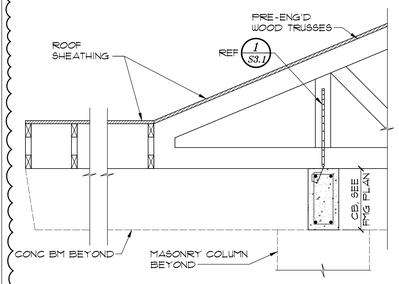
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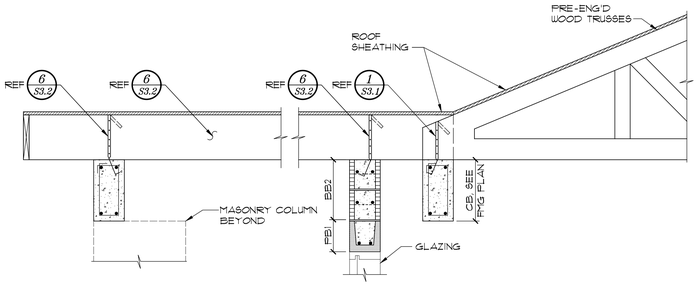
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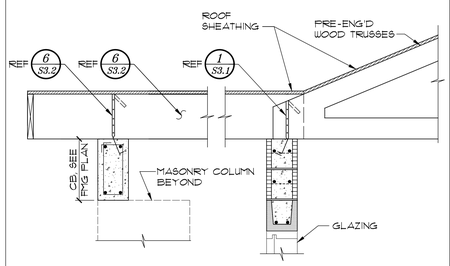
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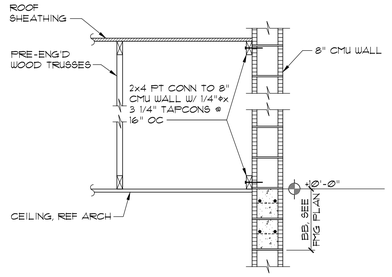
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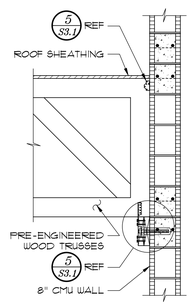
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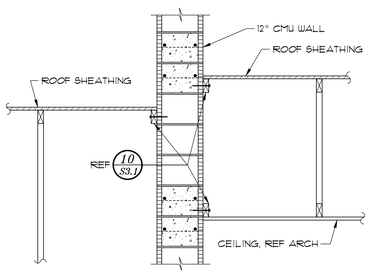
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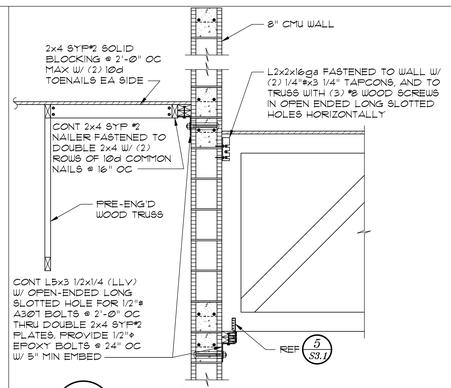
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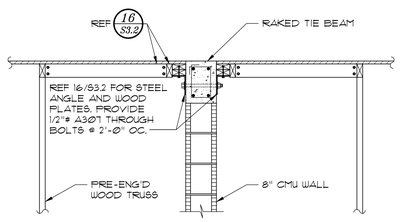
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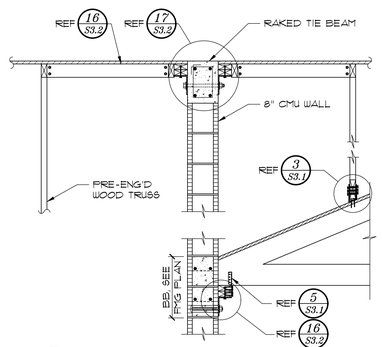
15 S3.2 SECTION @ ROOF



16 S3.2 SECTION @ FIRE WALL



17 S3.2 SECTION @ FIRE WALL



18 S3.2 SECTION @ FIRE WALL

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REVISIONS:	
1 REVISION #1	07-28-25

TO THE BEST OF THE ARCHITECT'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND THE APPLICABLE FIRE-SAFETY STANDARDS AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH THIS SECTION AND CHAPTER 633, FLORIDA STATUTES.

PLOT DATE:

FILE LOCATION:

SHEET DESCRIPTION:

ROOF FRAMING SECTIONS AND DETAILS

PROJECT NO.	SCALE:
1966	AS NOTED

DRAWING NO.

S3.2

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FOUNDATION SCHEDULE

MARK	SIZE (L x W x D)	REINFORCING	REMARKS
F5Ø	5'-Ø" x 5'-Ø" x 1'-Ø"	(5) #5 EA WAY, BOT	PAD FOOTING
F66	6'-6" x 6'-6" x 1'-2"	(7) #6 EA WAY, BOT	PAD FOOTING
F7Ø	7'-Ø" x 7'-Ø" x 1'-4"	(8) #6 EA WAY, BOT	PAD FOOTING
F76	7'-6" x 7'-6" x 1'-4"	(9) #6 EA WAY, BOT	PAD FOOTING
F8Ø	8'-Ø" x 8'-Ø" x 1'-5"	(8) #7 EA WAY, BOT	PAD FOOTING
F86	8'-6" x 8'-6" x 1'-6"	(11) #8 EA WAY, BOT	PAD FOOTING
WF2Ø	CONT x 2'-Ø" x 1'-Ø"	(2) #5 CONT & #5 @ 48" TRANS, BOT	WALL FOOTING
ME1Ø	CONT x 1'-Ø" x 1'-4"	(1) #5 CONT, BOT	MONOLITHIC EDGE

A S4.1 FOUNDATION SCHEDULE

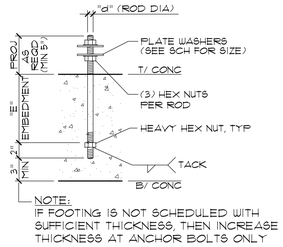
COLUMN & BASE PLATE SCHEDULE

MARK	SIZE	BASE PLATE SIZE (INCHES)			ANCHOR ROD (INCHES)		COMMENTS
		A	B	T	E	d	
C1	HSS4x4x1/4	1Ø	1Ø	3/4	1Ø	3/4	

NOTES:
1. ANCHOR RODS SHALL BE ASTM F1554 (GRADE 36) UNO, THREADED EACH END WITH NUT AT BOTTOM. PLATE WASHER AT BOTTOM NUT IS NOT REQ'D. INCREASE FOOTING THICKNESS AT ANCHOR RODS TO PROVIDE MIN 3" COVER, SEE NOTE BELOW IN ANCHOR ROD DETAIL.
2. PLATE WASHERS SHALL HAVE HOLE WITH DIAMETER EQUAL TO ROD DIAMETER + 1/16". CIRCULAR OR SQUARE WASHERS ARE ACCEPTABLE.
3. IT SHALL BE ACCEPTABLE TO PROVIDE A HEAVY HEX HEAD BOLT IN LIEU OF THE THREADED ROD WITH HEAVY HEX NUT AT BOTTOM.

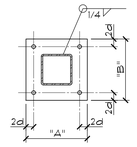
OVERSIZE HOLES AND PLATE WASHERS FOR BASE PLATES

ROD DIAMETER	HOLE DIAMETER	PLATE WASHER	ROD DIAMETER	HOLE DIAMETER	PLATE WASHER
3/4"	1 5/16"	1/4"x2"x2"	1 1/2"	2 3/8"	1/2"x4"
7/8"	1 9/16"	5/16"x2 1/2"x2 1/2"	1 3/4"	2 7/8"	5/8"x4 1/2"
1"	1 7/8"	3/8"x3"x3"	2"	3 1/4"	3/4"x5"
1 1/4"	2 1/8"	1/2"x3 1/2"x3 1/2"	2 1/2"	3 3/4"	7/8"x5 1/2"



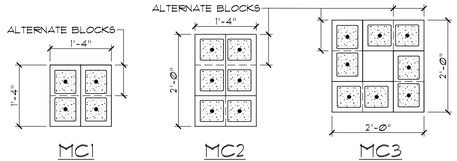
NOTE:
IF FOOTING IS NOT SCHEDULED WITH SUFFICIENT THICKNESS, THEN INCREASE THICKNESS AT ANCHOR BOLTS ONLY

ANCHOR ROD DETAIL
NOT TO SCALE



BASE PLATE DETAIL
NOT TO SCALE

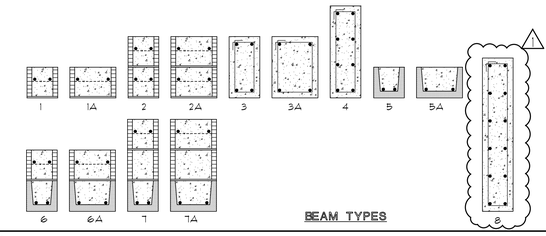
B S4.1 COLUMN & BASE PLATE SCHEDULE



C S4.1 DETAIL @ MASONRY COLUMNS

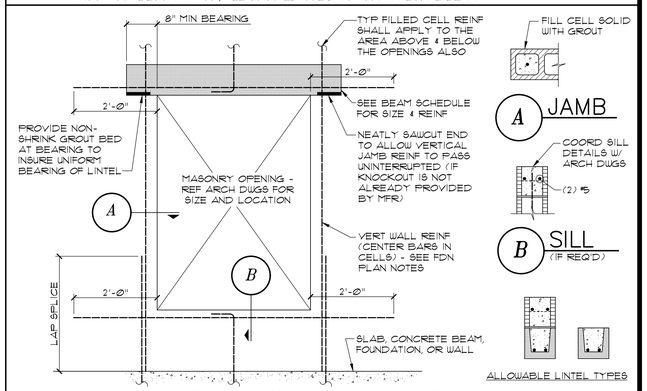
MASONRY WALL BEAM SCHEDULE

MARK	TYPE	NOMINAL SIZE (W"xD")	REINFORCING BOT	REINFORCING MID	REINFORCING TOP	STIRRUPS SIZE	STIRRUPS SPACING	COMMENTS
BB1	1	8x8	--	(2) #5	--	--	--	
BB1A	1A	12x8	--	(2) #5	--	--	--	
BB2	2	8x16	(2) #5	--	(2) #5	--	--	
BB2A	2A	12x16	(2) #5	--	(2) #5	--	--	
CB1	3	8x16	(2) #5	--	(2) #5	#3	6"	
CB2	4	8x24	(2) #5	(4) #5	(2) #5	#3	8"	
CB3	4	8x24	(2) #6	(4) #6	(2) #6	#3	6"	
CB4	3A	12x16	(3) #7	--	(3) #7	#3	6"	HOOK TOP BARS 180° AT CANTILEVERED ENDS
CB5	8	8x32	(2) #6	(8) #6	(2) #6	#3	10"	
TB1	3	8x12(MIN)	(2) #5	--	(2) #5	#3	48"	SLOPED TOP & STEPPED BOTTOM
TB2	3	8x12(MIN)	(2) #5	--	(2) #5	#3	48"	
PB1	5	8x8	(2) #5	--	--	--	--	MIN 1200 PLF CAP. FOR CLEAR SPAN UP TO 6'-Ø" (NOTE 1)
PB1A	5A	12x8	(2) #5	--	--	--	--	MIN 1900 PLF CAP. FOR CLEAR SPAN UP TO 6'-Ø" (NOTE 1)
PB2	6	8x16	(2) #5	--	(2) #5	--	--	MIN 1150 PLF CAP. FOR CLEAR SPAN UP TO 12'-Ø" (NOTE 1)
PB2A	6A	12x16	(2) #5	--	(2) #5	--	--	MIN 1350 PLF CAP. FOR CLEAR SPAN UP TO 12'-Ø" (NOTE 1)
PB3	7	8x24	(2) #5	(2) #5	(2) #5	--	--	MIN 1700 PLF CAP. FOR CLEAR SPAN UP TO 16'-Ø" (NOTE 1)
PB3A	7A	12x24	(2) #5	(2) #5	(2) #5	--	--	MIN 1500 PLF CAP. FOR CLEAR SPAN UP TO 16'-Ø" (NOTE 1)



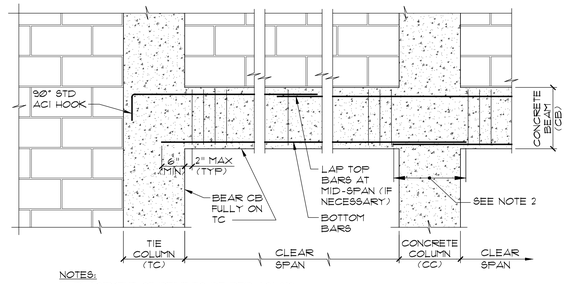
BEAM TYPES

NOTES:
1. MIN PB CAPACITY FOR THE INDICATED SPAN MUST BE VERIFIED BY THE PRECAST MFR AND DOCUMENTATION PROVIDED IN THE SUBMITTALS.
2. "BB" = BOND BEAM (COMPOSED OF KNOCK-OUT BLOCK)
3. "CB" = CONCRETE BEAM (FORMED, SHORED AND POURED)
4. "TB" = TIE BEAM (FORMED AND POURED)
5. "PB" = PRE-CAST BEAM (COMPOSED OF PRE-CAST U-LINTEL AT BOTTOM AND KNOCK-OUT BLOCK AT TOP, ALL GROUTED SOLID TO FORM INTEGRAL BEAM)



PRECAST BEAM ELEVATION
NTS

* NOTE: SOLID PRECAST LINTELS SHALL NOT BE USED.



NOTES:
1. PLACEMENT OF REINF TO BE PER ACI 318.
2. FOR BOTTOM BARS PROVIDE A CLASS "A" TENSION LAP CENTERED OVER SUPPORT OR TERMINATE BARS AS CONDITION WARRANTS.
3. SEE COLUMN SCHEDULES AND DETAILS FOR SIZE AND REINFORCING.

CONCRETE BEAM ELEVATION

D S4.1 MASONRY WALL BEAM SCHEDULE

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