

STRUCTURAL NOTES

(REFER TO PROJECT MANUAL FOR ADDITIONAL INFORMATION)

1. FOOTINGS & FOUNDATION EXCAVATION:

- A. A GEOTECHNICAL ANALYSIS HAS BEEN PERFORMED ON THIS SITE. SEE PROJECT MANUAL FOR GEOTECHNICAL INFORMATION. APPROPRIATE RECOMMENDATIONS STATED IN THE GEOTECHNICAL REPORT ISSUED BY UNIVERSAL ENGINEERING SCIENCES INC DATED FEBRUARY 15, 2024 SHALL BE FOLLOWED.
- B. THESE FOUNDATIONS HAVE BEEN DESIGNED FOR A SOIL BEARING OF 3000 PSF FOR CONTINUOUS AND FOR ISOLATED FOOTINGS.
- C. FOUNDATIONS AND SLAB SHALL BEAR ON COMPACTED SUITABLE NATIVE SOILS OR ON PROPERLY PLACED AND COMPACTED STRUCTURAL BACKFILL. SEE GEOTECHNICAL REPORT FOR SPECIFIC REQUIREMENTS REGARDING EXCAVATION AND PREPARATION OF SUBGRADE. A GEOTECHNICAL ENGINEER SHALL BE PRESENT TO DIRECT THE REMOVAL OF UNSUITABLE SOILS AND TO DETERMINE THE ADEQUACY OF THE BEARING SURFACE PRIOR TO PLACEMENT OF THE REINFORCEMENT AND CONCRETE.
- D. FOOTING WIDTHS TO BE AS SHOWN ON PLANS AND DETAILS. BOTTOM OF FOOTING IS TO BE EXCAVATED SQUARE AND TRUE.
- E. WHERE ANY OPEN TRENCH HAS BEEN EXPOSED TO RAIN, SNOW OR ICE PRIOR TO POURING CONCRETE, ALL REINFORCING OR THAT TRENCH SHALL BE REMOVED AND THE BOTTOM OF THE TRENCH SHALL BE DRAINED OF ALL WATER AND CLEANED OF MUD, SNOW OR ICE. A GEOTECHNICAL ENGINEER OR HIS TECHNICAL REPRESENTATIVE SHALL INSPECT THE BOTTOM OF THE TRENCH AND OBSERVE THE RE-COMPACTATION OF SOILS PRIOR TO PLACING REINFORCEMENT AND POURING OF CONCRETE.
- F. ALL STRIP FOOTINGS SHALL BE CENTERED UNDER WALLS BEING SUPPORTED AND ALL ISOLATED FOOTINGS SHALL BE CENTERED UNDER COLUMNS, UNLESS NOTED OTHERWISE.
- G. MINIMUM EXTERIOR FOOTING DEPTH SHALL BE AS INDICATED ON THE FOUNDATION PLAN SHEET S2.1.
- H. IN THE EVENT THAT ORGANIC SOIL OR UNCOMPACTED FILL IS FOUND BELOW FOOTINGS OR FLOOR SLABS, IT SHALL BE REMOVED AND REPLACED WITH SELECT FILL, COMPACTED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT.
- I. STRUCTURAL FILL SHALL BE PLACED IN NO GREATER THAN 8" LOOSE LIFTS AND COMPACTED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT. ADEQUATE DENSITY AND MOISTURE CONTENT TESTS SHALL BE PERFORMED TO INSURE COMPLIANCE WITH PROJECT SPECIFICATIONS. SUBGRADE INSPECTION AND FILL TESTING UNDER CONTROLLED CONDITIONS IS CONSIDERED ESSENTIAL IF THE FOOTINGS ARE TO BE FOUND IN FILL. A TESTING FREQUENCY OF AT LEAST ONE FIELD DENSITY TEST FOR EACH 2500 SQUARE FEET OF LIFT, BUT NOT LESS THAN 3 TESTS PER LIFT IS RECOMMENDED WITHIN THE BUILDING AREAS.

2. CONCRETE:

- A. ALL READY MIX CONCRETE SHALL BE 4000 psi FOR ALL CONCRETE PLACEMENT. DO NOT ADD WATER TO THE MIX DESIGN AFTER DELIVERY TO THE PROJECT SITE.
- B. EXPOSED EXTERIOR CONCRETE SHALL BE AIR-ENTRAINED. INTERIOR CONCRETE SHALL NOT BE AIR-ENTRAINED.
- C. UNLESS NOTED OTHERWISE, CONCRETE COVER OVER STEEL REINFORCEMENT SHALL CONFORM TO THE MINIMUM REQUIREMENT BY ACI 318.
- D. REINFORCEMENT DETAILING AND PLACEMENT SHALL CONFORM TO ACI 318 AND ACI 315, EXCEPT WHERE OTHERWISE INDICATED.
- E. HOT OR COLD WEATHER CONCRETING SHALL BE IN ACCORDANCE WITH ACI 305-89 AND ACI 306-1-90, RESPECTIVELY.
- F. ANY CONCRETE PLACED BY MEANS OF PUMPING SHALL BE DONE IN ACCORDANCE WITH ACI 304.2R (82).
- G. CEMENT SHALL CONFORM TO ASTM C-150 TYPE I OR 1/II AND LIMESTONE CEMENT SHALL BE ASTM C-595 TYPE II.
- H. AGGREGATES SHALL CONFORM TO ASTM C-33 FOR NORMAL WEIGHT CONCRETE & ASTM C-330 FOR LIGHTWEIGHT CONCRETE.
- I. READY MIX CONCRETE SHALL BE MIXED AND DELIVERED IN ACCORDANCE WITH ASTM C-94.
- J. ADMIXTURES MAY BE USED WITH THE APPROVAL OF THE STRUCTURAL ENGINEER. ADMIXTURES USED TO INCREASE THE WORKABILITY OF THE CONCRETE SHALL NOT BE CONSIDERED TO REDUCE THE CEMENT CONTENT. NO CALCIUM CHLORIDE ADMIXTURES ALLOWED.

3. SLABS ON GRADE:

- A. FLOOR SLABS ARE TO BE PLACED AND FINISHED IN ACCORDANCE WITH ACI 302 (SEE PROJECT MANUAL FOR ADDITIONAL INFORMATION).
- B. THICKNESS TOLERANCE FOR ALL SLABS IS TO BE PER ACI 117 AND IS TO BE NO MORE THAN +3/8" (THICKER) AND NO MORE THAN -1/2" (THINNER) FROM THE DESIGN THICKNESS.

4. REINFORCING:

- A. REINFORCING BARS SHALL BE BILLET STEEL, ASTM A 615, GRADE 60. PROVIDE CONTINUOUS BENT BARS AT FOOTING STEPS AND 90 DEGREE BENT TIES AT CORNERS, UNLESS OTHERWISE NOTED. LAP SPLICES OR EMBEDMENT LENGTHS SHALL CONFORM TO CLASS B SPLICE (SEE SPLICE TABLE). ADJACENT BAR SPLICES IN WALLS AND FOOTINGS TO BE ALTERNATED. ALL FOOTINGS SHALL REQUIRED HOOKED REINFORCING PROJECTED INTO WALLS, PILASTERS OR COLUMNS. THE SIZE AND SPACING OF DOWELS ARE TO MATCH VERTICAL REINFORCING.
- B. WELDED WIRE FABRIC (WWF) SHALL CONFORM TO THE CURRENT ASTM SPECIFICATION FOR COLD DRAWN STEEL REINFORCEMENT WIRE. LAP END AND EDGES MINIMUM 6".
- C. REINFORCING DETAILING, BENDING, AND PLACING SHALL CONFORM TO ACI 315.
- D. MINIMUM CONCRETE COVERAGE: THE MINIMUM CLEAR DISTANCES BETWEEN REINFORCING STEEL AND FACE OF CONCRETE SHALL BE MAINTAINED UNLESS NOTED OTHERWISE:

SLABS ON EARTH.....	2"
CONCRETE BELOW GRADE, FORMED.....	2"
CONCRETE BELOW GRADE, UNFORMED AND POURED AGAINST EARTH.....	3"

5. LUMBER:

- A. ALL LUMBER SHALL BE FIRE TREATED IN ACCORDANCE WITH PROJECT SPECIFICATIONS. REFER TO ARCHITECTURAL AND PROJECT MANUAL FOR MORE INFORMATION AND SPECIFIC REQUIREMENTS.
- B. TREATED LUMBER: IN LOCATIONS WHERE TREATED LUMBER IS SHOWN ON DRAWINGS, THE APPROVED PRESSURE TREATED WOODS ARE ACQ-D(CARBONATE) OR CA-B TREATED WOODS WITHOUT AMMONIA CARRIERS. THE CHEMICAL RETENTION LEVELS ARE TO BE NO GREATER THAN 0.4 PCF FOR ACQ-D, 0.21 PCF FOR CA-B. ALL METAL CONNECTORS ARE TO HAVE A GALVANIZED COATING OF NO LESS THAN 1.85 OUNCES OF ZINC PER SQUARE FOOT PER ASTM A653. ALL BOLTS, SCREWS NAILS AND OTHER FASTENERS ARE TO BE GALVANIZED PER ASTM A153. WHERE TREATED LUMBER IS SHOWN IN EXTERIOR INSTALLATIONS WITH NO ROOF COVERINGS TO PREVENT DIRECT EXPOSURE TO RAIN, USE HOT DIP GALVANIZED CONNECTORS PER ASTM A123.

6. STRUCTURAL STEEL:

- A. FABRICATOR QUALIFICATIONS: FABRICATOR MUST PARTICIPATE IN THE AISC QUALITY CONTROL PROGRAM AND BE DESIGNATED AN AISC-CERTIFIED PLANT.
- B. MATERIALS (UNLESS NOTED OTHERWISE):

STRUCTURAL STEEL	ASTM A992, GRADE 50
PLATES, ANGLES, CHANNELS, AND MISCELLANEOUS STEEL	ASTM A36
ANCHOR RODS	ASTM F1554, GRADE 36
HIGH STRENGTH BOLTS	ASTM F3125
WELDING ELECTRODES	AWS A5.1 (E70XX)
PIPE	ASTM A53, GRADE B
SQUARE AND RECTANGULAR HOLLOW STRUCTURAL SECTIONS (HSS).....	ASTM A500, GRADE B

7. LIGHT GAUGE STEEL (COLD-FORMED STEEL OR CFS):

- A. LIGHT GAUGE STEEL MEMBERS, ASSEMBLIES AND PROTECTIVE COATINGS SHALL COMPLY WITH ACCEPTED INDUSTRY PRACTICES AS DETERMINED BY THE AISI AND SSMMA.
- B. SLIP TRACK ASSEMBLIES SHALL BE CONSTRUCTED TO CONFORM TO THE GUIDELINES SET FORTH IN SSMA TECHNICAL NOTE #1 (JAN 2001).
- C. CONSTRUCTED ASSEMBLIES SHALL USE POSITIVE CONNECTIONS WITH APPROVED FASTENERS (NOT FRICTION FIT) UNLESS NOTED OTHERWISE.
- D. STUDS SHALL SEAT SQUARELY WITHIN A METAL TRACK OF SAME SIZE AND GAUGE WITH 1/4" MAX GAP. FASTEN EACH STUD FLANGE TO TRACK W/ (1) #10 SDS.
- E. CFS WALLS SHALL BE LATERALLY BRACED AT BOTH FLANGES WITH A COMBINATION OF EITHER CONTINUOUS SHEATHING AND/OR STRAP BRACING (OR EQUIV). WALLS COMPRISED OF MULTIPLE STUD LAYERS SHALL HAVE BRACING ELEMENTS LOCATED BETWEEN EACH LAYER OF STUDS.
 - SHEATHING: THICKNESS AND MATERIAL TO BE AS PER ARCHITECTURAL. FASTENERS TO BE #8 CORROSION-RESISTANT SCREWS @ 12" O.C. ALONG SUPPORTS.
 - STRAP BRACING: 2"x20ga FLAT METAL STRAPPING ALIGNED WITH FULL DEPTH BLOCKING-IN MEMBERS AT ENDS OF WALL RUN, ADJACENT TO OPENINGS, AND @ 8'-0" O.C. ALONG WALL RUN. SPACE VERTICALLY @ 6'-0" O.C. FOR INTERIOR WALLS AND FOR PERIMETER WALLS.
- F. BASE TRACK ANCHORS - MIN. (2) PER EACH WALL RUN:
 - END-OF-RUN AND ADJACENT TO OPENINGS: 3/4" WITH 4" EMBED. EITHER CAST-IN-PLACE HEADED BOLTS OR POST-INSTALL USING SIMPSON TITEN HD SCREW ANCHORS.
 - WITHIN EACH WALL RUN: EITHER ANCHORS EQUALING END-OF-RUN ANCHORS PLACED @ 48" O.C. OR IN LOW SEISMIC REGIONS (SEISMIC DESIGN CATEGORY OF A, B, OR C) INSTALL (2) HILTI X-U 32 PAF'S (OR EQUAL) @ EACH STUD.
- G. SPECIFIED FASTENERS AND CONNECTORS (OR EQUIVALENT) TO BE INSTALLED IN ACCORDANCE WITH ACCEPTED INDUSTRY STANDARDS AND THE MANUFACTURER PROVIDED INSTALLATION INSTRUCTIONS (MPI). INSTALLATION LOCATIONS TO BE LIMITED TO EXPOSURE, LOADING TYPE AND MATERIALS SPECIFIED IN CORRESPONDING CODE REPORTS.

8. CONCRETE MASONRY UNITS (CMU):

- A. CONCRETE MASONRY UNITS SHALL BE 2-CELL BLOCK THAT CONFORMS WITH THE PROVISIONS OF THE BUILDING CODE STATED IN THE DESIGN CRITERIA.
- B. CELLS TO RECEIVE GROUT SHALL BE SUFFICIENTLY ALIGNED SO AS TO MAINTAIN A CLEAR, UNOBSTRUCTED, CONTINUOUS CELL OF NOT LESS THAN 2 1/2"x3".
- C. FULLY GROUT ALL CELLS BELOW GRADE, WITHIN 4'-8" OF CORNERS, ABOVE INTERMEDIATE BOND BEAM, AND THOSE WHICH CONTAIN REINFORCEMENT.
- D. VERTICAL REINFORCEMENT FOR CMU WALLS SHALL BE #5 @ 16" O.C. WITHIN THE AREA LABELED "MASONRY WALL SPECIAL REINFORCEMENT AREA" ALONG GRIDLINE "A" (SEE SHEETS S2.1 AND S4.1). ALL OTHER WALL AREAS SHALL BE #5 @ 24" O.C. WITH AN ADDITIONAL VERTICALLY REINFORCED CELL LOCATED AT EITHER SIDE OF CONTROL JOINT LOCATIONS AS WELL AS AT WALL ENDS AND CHANGES IN DIRECTION OR ELEVATION. SEE LITEL AND JAMB SCHEDULE, S4.1, FOR REINFORCEMENT AT OPENINGS.
- E. PROVIDE A FOUNDATION DOWEL ALIGNED WITH AND OF EQUAL SIZE TO EACH VERTICAL WALL REINFORCING BAR. FOUNDATION DOWEL SHALL EXTEND INTO MASONRY WALL A SUFFICIENT DISTANCE TO DEVELOP A LAP SPLICE WITH WALL REINFORCEMENT.
- F. LAP SPLICE AND DEVELOPMENT LENGTH FOR REINFORCING BARS #5 AND SMALLER SHALL BE 30".
- G. VERTICAL WALL REINFORCEMENT SHALL BE CENTERED IN CELLS WITH 9ga HDG REBAR POSITIONERS @ 48" O.C. AND EXTEND CONTINUOUSLY FROM THE FOUNDATION TO TERMINATE IN THE TOPMOST BOND BEAM. HORIZONTAL BOND BEAM REINFORCEMENT SHALL EXTEND CONTINUOUSLY, EXCEPT WHERE INTERRUPTED BY WALL OPENINGS, AND INCLUDE BENT BARS AT CORNER LOCATIONS.
- H. PROVIDE 9ga HDG LADDER TYPE HORIZONTAL JOINT REINFORCEMENT AT EVERY COURSE BELOW GRADE AND AT 16" O.C. ABOVE GRADE.
- I. DOUBLE WYTHE CMU MUST BE CONTINUOUSLY TIED TOGETHER WITH 9ga HDG HORIZONTAL JOINT REINFORCEMENT AT EACH BED JOINT. REFER TO ARCHITECTURAL DRAWINGS AND PROJECT SPECIFICATIONS FOR INFORMATION REGARDING VENEER ANCHORAGE AND THE REQUIREMENTS.

9. GENERAL:

- A. CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA REGULATIONS.
- B. THE STRUCTURAL DESIGN OF THE BUILDING IS BASED UPON THE FULL INTERACTION OF ALL ITS COMPONENT PARTS, WITH NO PROVISION MADE FOR CONDITIONS OCCURRING DURING CONSTRUCTION. THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED FORM. THE CONTRACTOR SHALL PROVIDE ADEQUATE BRACING DURING CONSTRUCTION. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED AND INSTALLED BY THE CONTRACTOR. THE CONTRACTOR IS RESPONSIBLE FOR CONSTRUCTIBILITY ANALYSIS AND ERECTION PROCEDURES, INCLUDING DESIGN AND ERECTION OF FALSEWORK, TEMPORARY BRACING, ETC. THE STRUCTURAL ENGINEER ASSUMES NO LIABILITY FOR THE STRUCTURE DURING CONSTRUCTION.
- C. CORRECTIONS DUE TO UNFORESEEN FIELD CONDITIONS OR DIMENSIONAL DISCREPANCIES ON CONSTRUCTION DOCUMENTS MUST BE BROUGHT TO THE ATTENTION OF THE PROJECT ARCHITECT FOR REVIEW AND AUTHORIZATION PRIOR TO CORRECTIVE MEASURES BEING IMPLEMENTED.
- D. STRUCTURAL DRAWINGS ARE TO BE USED IN CONJUNCTION WITH ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS.
- E. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER THESE GENERAL NOTES.
- F. ALL SHOP DRAWINGS SHALL BE REVIEWED AND APPROVED BY THE PROJECT ARCHITECT/ENGINEER PRIOR TO SUBMITTING TO THE BUILDING DEPARTMENT FOR REVIEW AND APPROVAL.

10. ADHESIVE / EPOXY POST-INSTALLED ANCHORS AND BARS:

- A. ADHESIVE USED IN STRUCTURAL APPLICATIONS SHALL BE SIMPSON AT-3G (ESR-5026) OR SET-3G (ESR-4057, ESR-4844). SUBSTITUTION FOR STRUCTURAL APPLICATIONS REQUIRES PRIOR WRITTEN APPROVAL OF THE PROJECT ENGINEER OF RECORD.
- B. ADHESIVE USED IN NON-STRUCTURAL APPLICATIONS SHALL BE AT THE CONTRACTOR'S DISCRETION AND RATED FOR APPROPRIATE EXPOSURE AND MATERIALS.
- C. CONTRACTOR TO FOLLOW ALL MANUFACTURERS INSTALLATION INSTRUCTIONS FOR SPECIFIC ADHESIVE/EPOXY APPLICATION.
- D. SPECIAL INSPECTIONS ARE REQUIRED FOR POST-INSTALLED ANCHORS USED IN STRUCTURAL APPLICATIONS. REFER TO SPECIAL INSPECTIONS REQUIREMENTS.

11. SPECIAL INSPECTIONS REQUIREMENTS:

- A. OWNER SHALL ENGAGE ONE OR MORE QUALIFIED SPECIAL INSPECTORS AND/OR TESTING AGENCIES TO CONDUCT STRUCTURAL TESTS, CONSTRUCTION MATERIAL TESTING, AND SPECIAL INSPECTIONS SPECIFIED IN THE "STATEMENT OF SPECIAL INSPECTIONS".
- B. FOR THE SPECIFIC RESPONSIBILITY OF THE OWNER, CONTRACTOR, AND SPECIAL INSPECTOR REFER TO THE PROJECT MANUAL.

DESIGN CRITERIA

BUILDING CODE: 2023 FLORIDA BUILDING CODE

DESIGN LOADS:

ROOF DEAD LOAD.....	SEE PEMB
ROOF COLLATERAL LOAD.....	2.5 psf
ADDITIONAL CEILING SYSTEM COLLATERAL AT PARTS AREA "A".....	2.0 psf
ROOF LIVE LOAD.....	20.0 psf
SPRINKLER LOADS	
BRANCH PIPE (UNIFORM).....	0.0 psf
LINEAL LOOP/TEE MAIN PIPE.....	0.0 pf

SNOW LOAD: (RESERVED)

WIND CALCULATION METHOD: ENVELOPE

V = 132 mph V_{std} = 102.2 mph
 EXPOSURE = C
 RISK CATEGORY = II
 BUILDING, ENCLOSED GC_p = ±0.18

WIND COMPONENTS & CLADDING: (ASD)

WALL = ±28.4 psf
 ROOF = -48.9 psf

BASE SHEAR:

V_x: WIND = 51.6 k
 V_y: WIND = 40.1 k

SEISMIC DESIGN: (RESERVED)

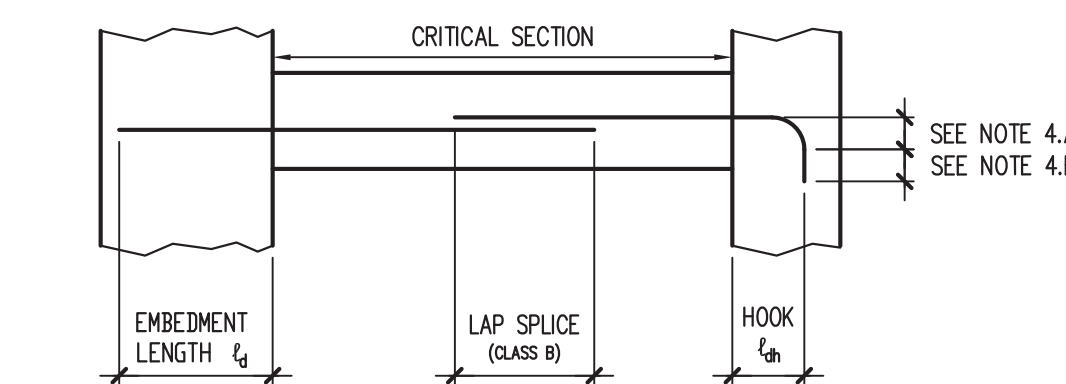


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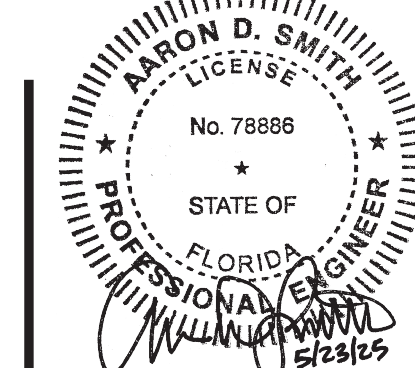
SPLICE TABLE ¹					
(UNLESS NOTED OTHERWISE)					
BAR SIZE	LAP SPLICES (in.) ²		EMBED LENGTH (in.)		
	TOP BARS ³ Class B	OTHERS Class B	TOP BARS ³ ld	OTHERS ld	HOOKS ⁵ ldh
#3	25	19	19	15	8
#4	33	25	25	19	10
#5	41	31	31	24	12
#6	49	37	37	29	15
#7	71	54	54	42	17

1. SPLICE TABLE IS BASED ON THE FOLLOWING:

- A. CONCRETE f_c = 4000 psi
- B. GRADE 60 REBAR
- C. BAR SPACING NOT LESS THAN 2 BAR DIAMETERS OR 1"
- D. CONCRETE COVER NOT LESS THAN ONE BAR DIAMETER
2. LAP LENGTHS SHOWN ARE FOR CLASS "B" TENSION SPLICES PER ACI 318-14 CHAPTER 12.
3. TOP BARS ARE DEFINED AS HORIZONTAL REINFORCEMENT PLACED SO THAT MORE THAN 12" OF CONCRETE IS CAST BELOW THE REINFORCEMENT IN THAT MEMBER.
4. STANDARD 90° HOOKS:
 - A. RADIUS = 4 BAR DIAMETERS FOR #3 THRU #8
 - B. LENGTH = 12 BAR DIAMETERS
5. HOOK LENGTH MAY BE REDUCED IN ACCORDANCE WITH ACI 318-14 CHAPTER 12.5



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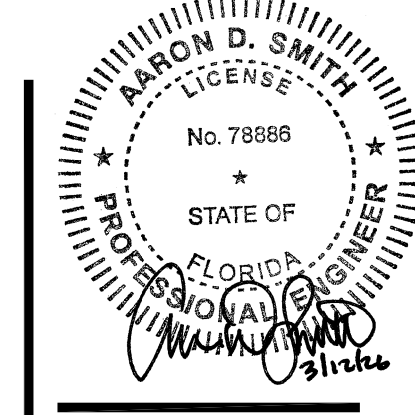


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PROJECT:
NEW O'REILLY AUTO PARTS STORE
HWY FL 44
EUSTIS, FL #2

O'Reilly AUTO PARTS
 CORPORATE OFFICES
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 (417) 862-2674 TELEPHONE

COMM # 4902
 DATE: 5/23/25
 REVISION
 DATE:



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PROJECT:
NEW O'REILLY AUTO PARTS STORE
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EUSTIS, FL #2
SIGN FOUNDATION DETAILS

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COMM # 4902
 DATE: 1/30/26
 REVISION
 DATE: 3/12/26

S1.2

GENERAL NOTES

- A. REFER TO PROJECT MANUAL FOR ADDITIONAL REQUIREMENTS.
- B. CONTACT OWNER'S OR TENANT'S DESIGNATED PROJECT ADMINISTRATOR FOR QUESTIONS OR CLARIFICATIONS.
- C. SITE SIGN FOUNDATIONS AND ROUGH-IN ELECTRICAL TO BE PROVIDED BY CONTRACTOR.
- D. REQUIRED GOVERNMENTAL PERMITS FOR PERMANENT TYPE SIGNAGE TO BE PROVIDED BY OWNER OR OTHERS (N.I.C.).

SEQUENCE OF WORK

- A. CONTRACTOR TO PROVIDE MONUMENT SIGN ROUGH-IN ELECTRICAL POINT OF CONNECTION, CONCRETE FOUNDATION, AND ANCHOR BOLT CONSTRUCTION (COORDINATE WITH SIGN CONTRACTOR).
- B. SIGN CONTRACTOR (BY OTHERS) TO INSTALL POST & SIGN CONSTRUCTION.
- C. CONTRACTOR TO PROVIDE MASONRY SIGN BASE AND SEALANT CONSTRUCTION.

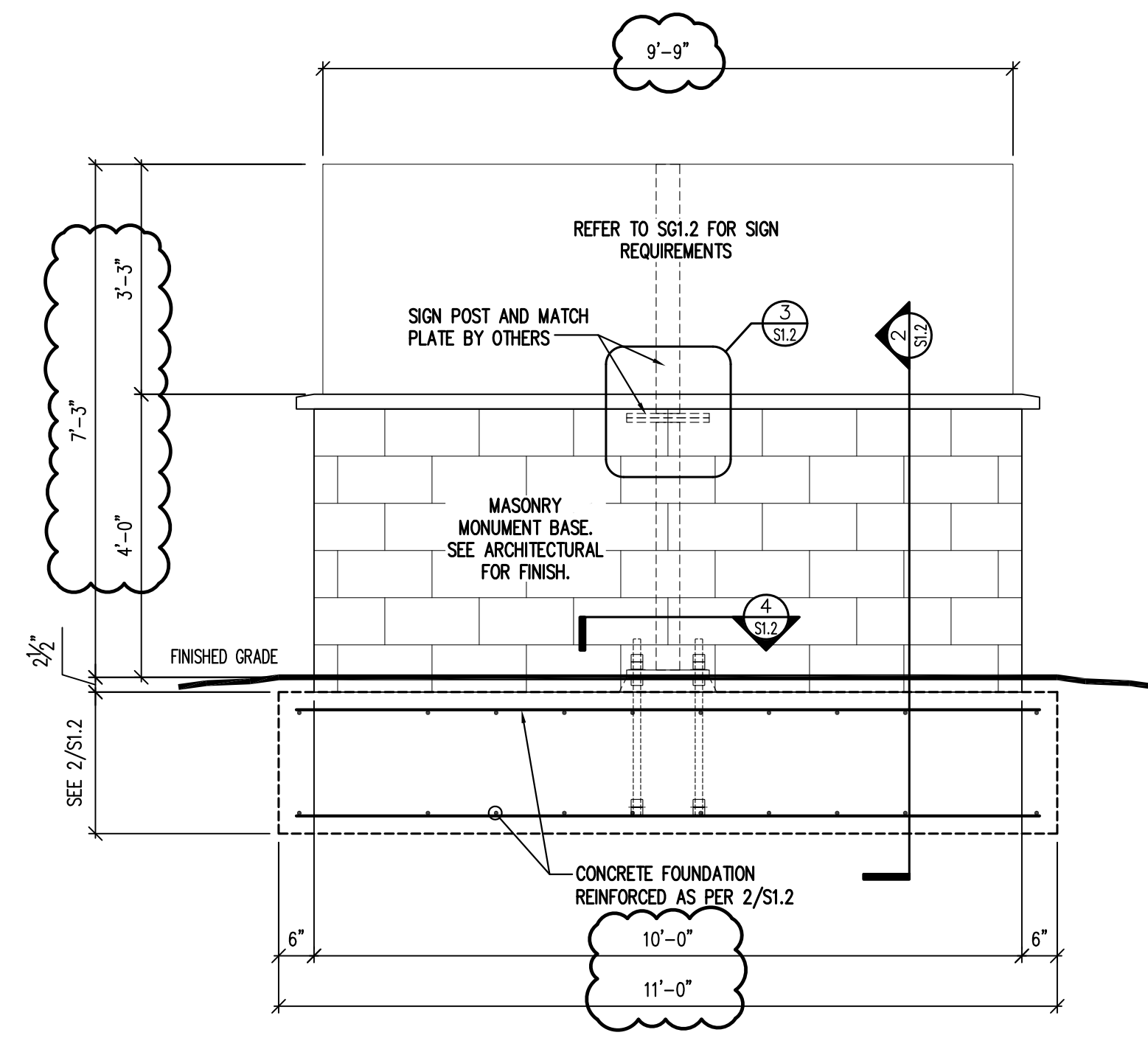
DESIGN CRITERIA

- A. GOVERNING BUILDING CODE: 2023 FLORIDA BUILDING CODE
 - B. WIND CRITERIA: RISK CATEGORY = II
 - V_W = 132 mph
 - V_W = 102.2 mph
 - EXPOSURE "C"
 - C. SEISMIC CRITERIA: (RESERVED)
 - D. GEOTECHNICAL PARAMETERS:
 - REPORT AUTHOR: UNIVERSAL ENGINEERING SCIENCES INC
 - DATED: FEBRUARY 15, 2025
 - ALLOWABLE BEARING PRESSURE: 3000 psf
 - ALLOWABLE LATERAL RESISTANCE: 175 psf/ft
- *INCREASED PER PROVISIONS FOR TRANSIENT LOADS ON ISOLATED FOUNDATIONS**

STRUCTURAL NOTES

- A. BRACE POLE SIGN AGAINST LATERAL FORCES UNTIL CONCRETE HAS REACHED SPECIFIED COMPRESSIVE DESIGN STRENGTH.
- B. AIR-ENTRAIN CONCRETE LOCATED IN REGIONS SUBJECTED TO FREEZE-THAW CYCLES.
- C. CENTER CMU VERTICAL REINFORCEMENT WITHIN GROUTED CELLS THAT ARE ALIGNED TO PROVIDE MIN. 2"x3" VERTICAL CELL SHAFT.
- D. EVERY OTHER CMU COURSE TO CONTAIN 9ga LADDER TYPE HORIZONTAL JOINT REINFORCEMENT.
- E. MASONRY WALLS SHALL CONTAIN FULL DEPTH WEEPS @ 32"o.c.
- F. STANDARDS:
 - CONCRETE AIR ENTRAINMENT..... ACI 318-19
 - AGGREGATES FOR NORMAL WEIGHT CONCRETE..... ASTM C-33
 - HOT WEATHER CONCRETING..... ACI 305-89
 - COLD WEATHER CONCRETING..... ACI 305-1-90
 - PUMPED CONCRETE PLACEMENT..... ACI 304.2R-82
 - CONCRETE MIXTURE AND DELIVERY..... ASTM C-94
 - STEEL FABRICATOR QC CERTIFICATION..... IAS AC472
- H. MATERIALS:
 - CONCRETE 28-DAY STRENGTH (f_c)..... 4000psi
 - PORTLAND CEMENT TYPE..... 1/II
 - FOUNDATION EXPOSURE CLASS..... F1
 - CONCRETE ADMIXTURES (NO CaCl₂)..... SEE PROJECT MANUAL
 - HOT ROLLED STEEL SHAPES..... ASTM A992 (50ksi)
 - PLATES, ANGLES, AND MISC STEEL..... ASTM A36 (36ksi)
 - ANCHOR RODS..... ASTM F1554 (36ksi)
 - HIGH STRENGTH BOLTS..... ASTM F3125
 - WELDING ELECTRODES..... AWS A5.1 (E70XX)
 - PIPE..... ASTM A53, Gr. B (36ksi)
 - ROUND HSS..... ASTM A500, Gr. C (50ksi)
 - RECTANGULAR HSS..... ASTM A500, Gr. C (50ksi)

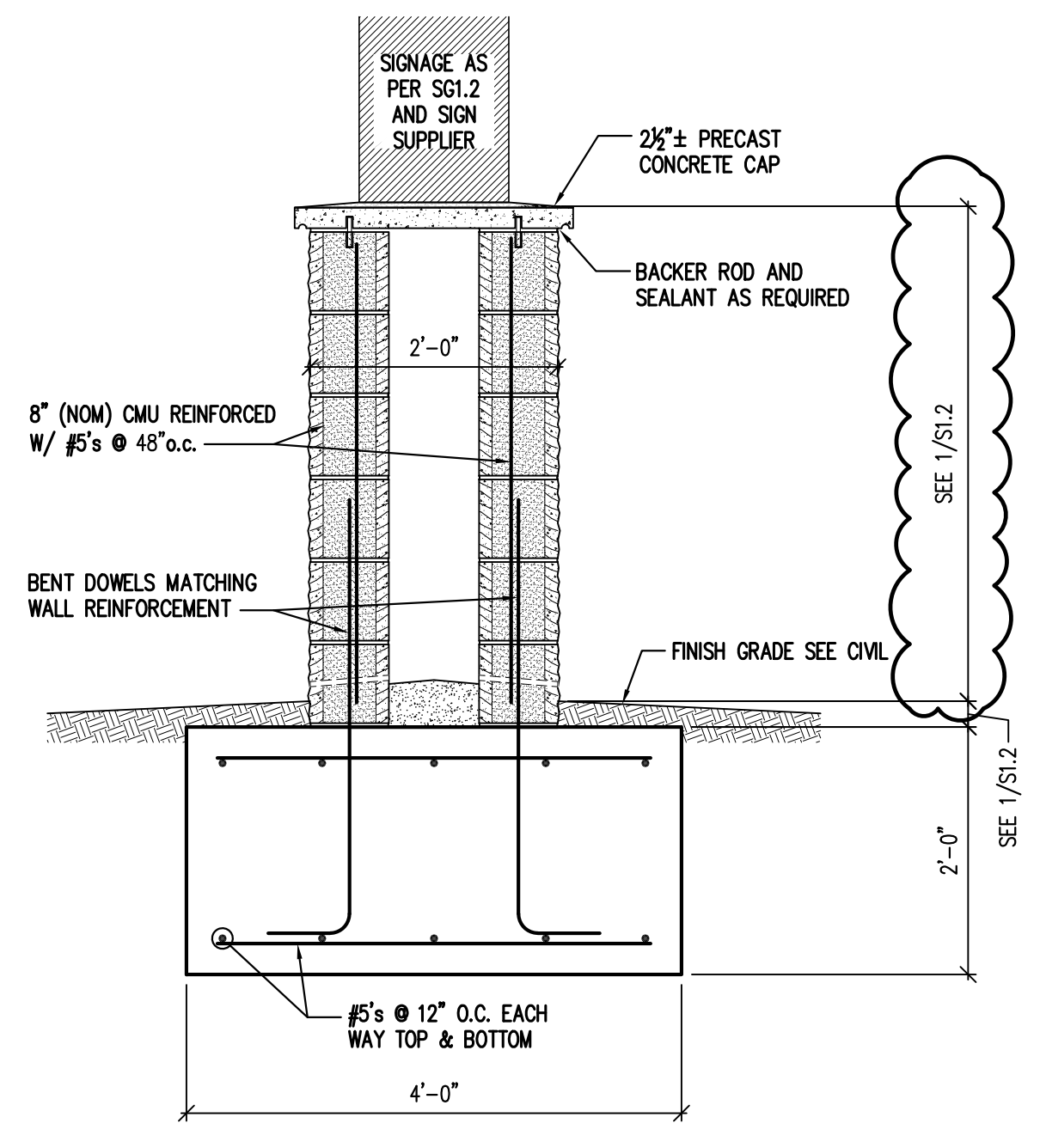
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1 SIGN ELEVATION

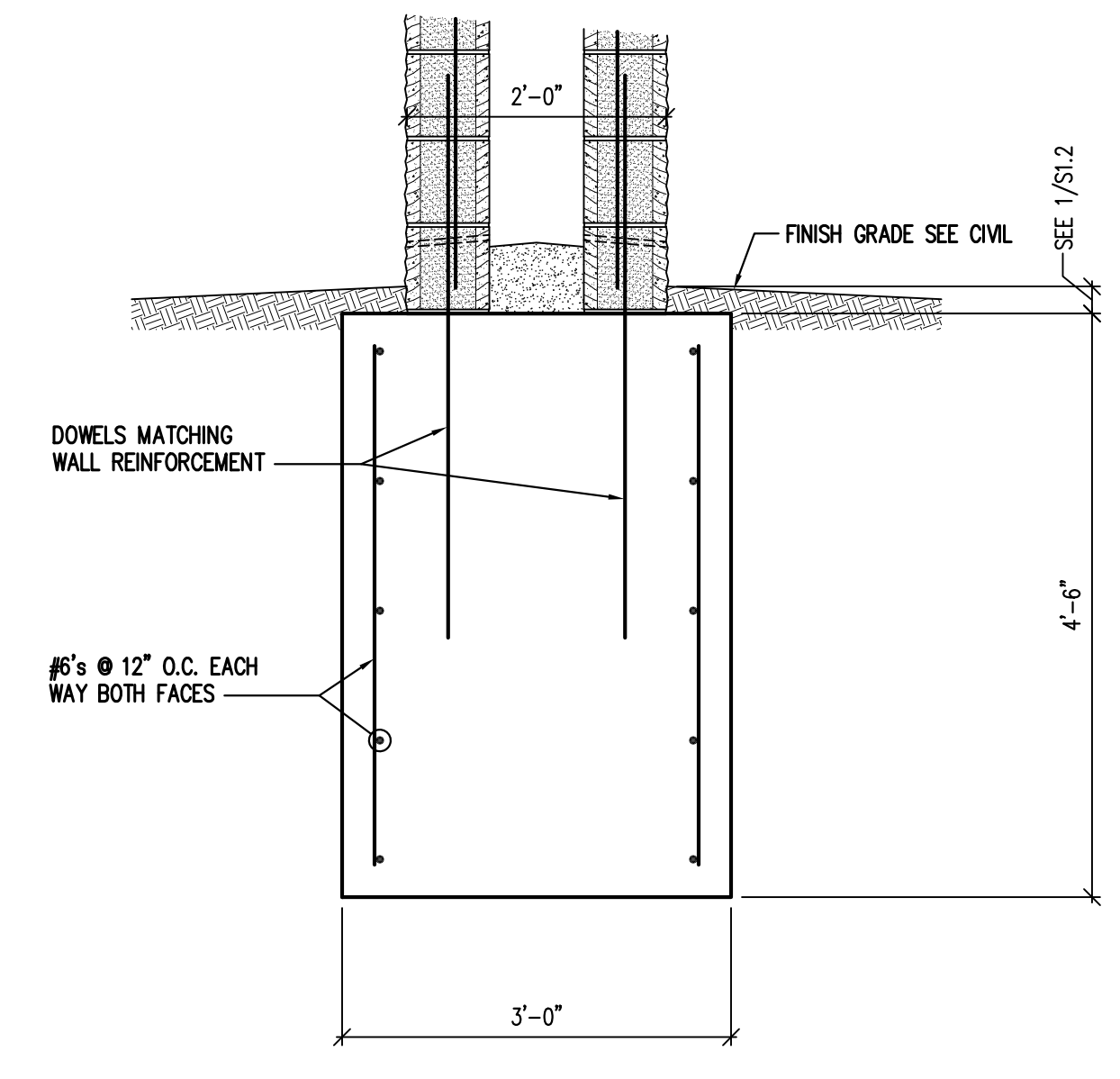
S1.2 SCALE: 1/2" = 1'-0"

NOTE: COORDINATE FOUNDATION WITH SITE UTILITIES



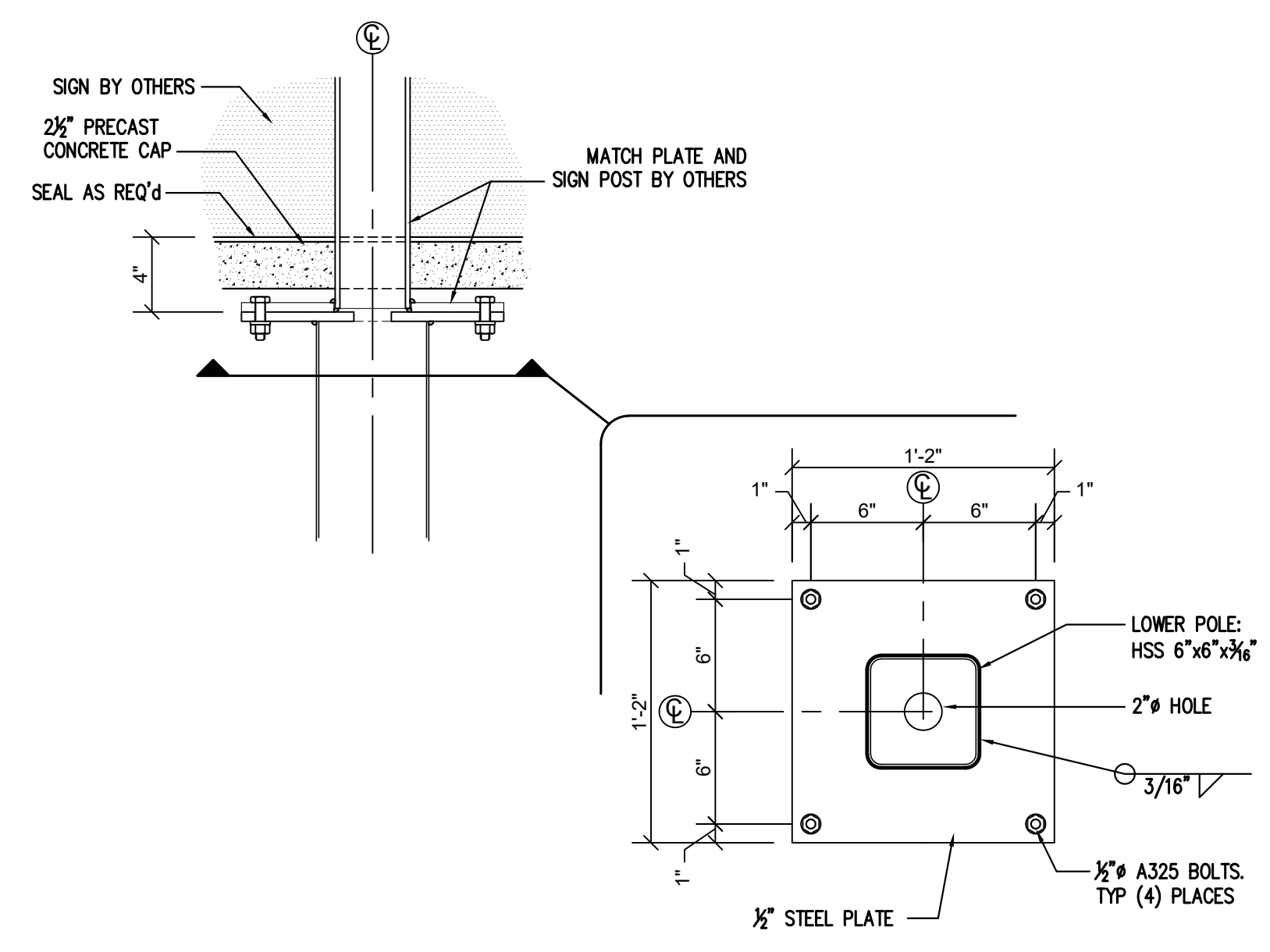
2 MONUMENT SIGN FOUNDATION SECTION

S1.2 SCALE: 3/4" = 1'-0"



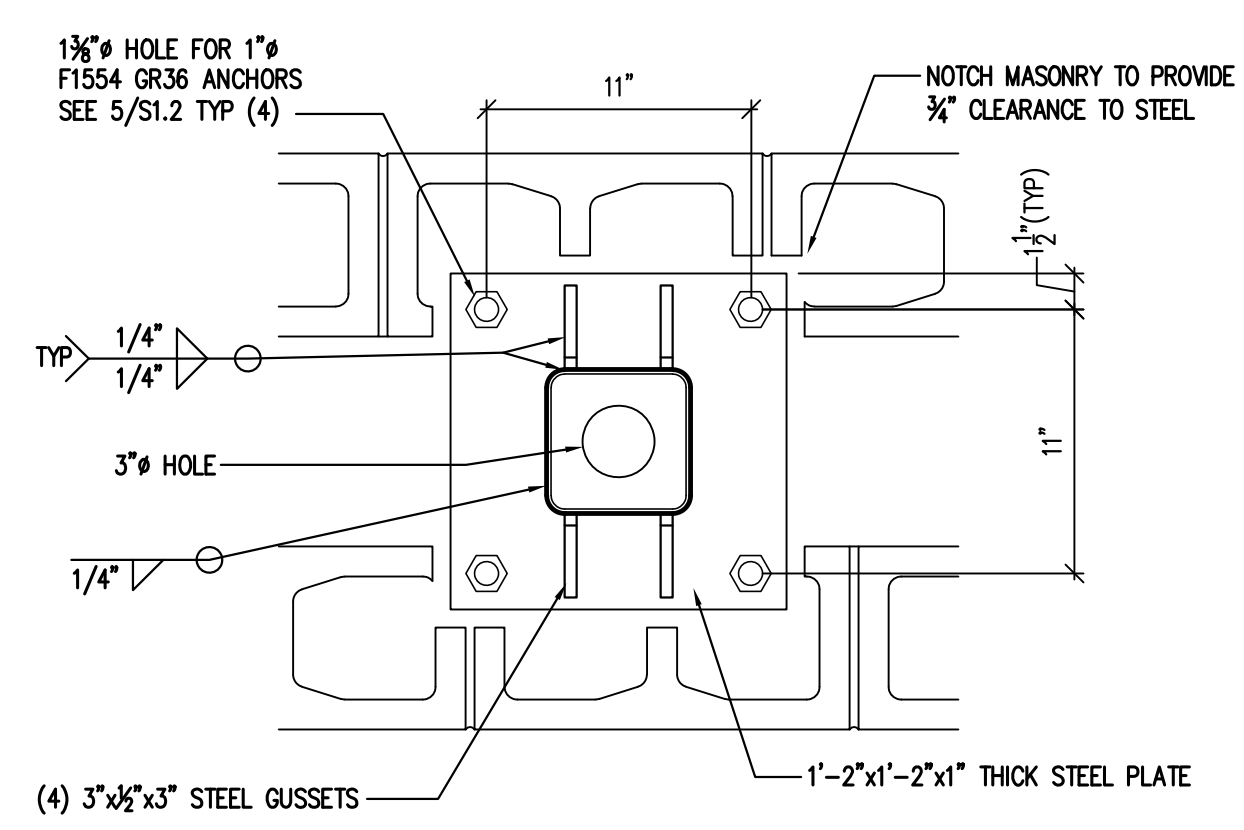
2b ALTERNATE "KNIFED" FOUNDATION SECTION

S1.2 SCALE: 3/4" = 1'-0"



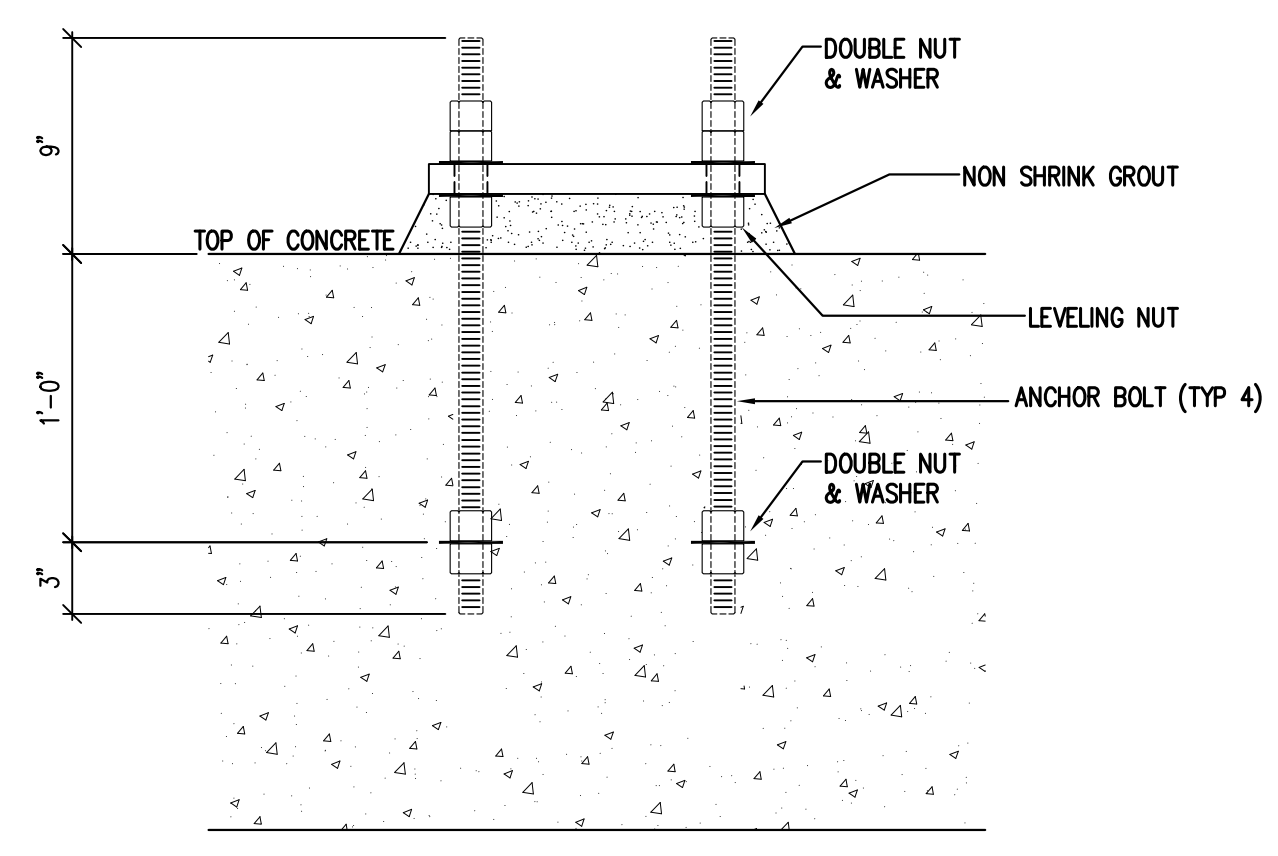
3 COLUMN CAP PLATE DETAIL

S1.2 SCALE: 1 1/2" = 1'-0"



4 COLUMN BASE DETAIL

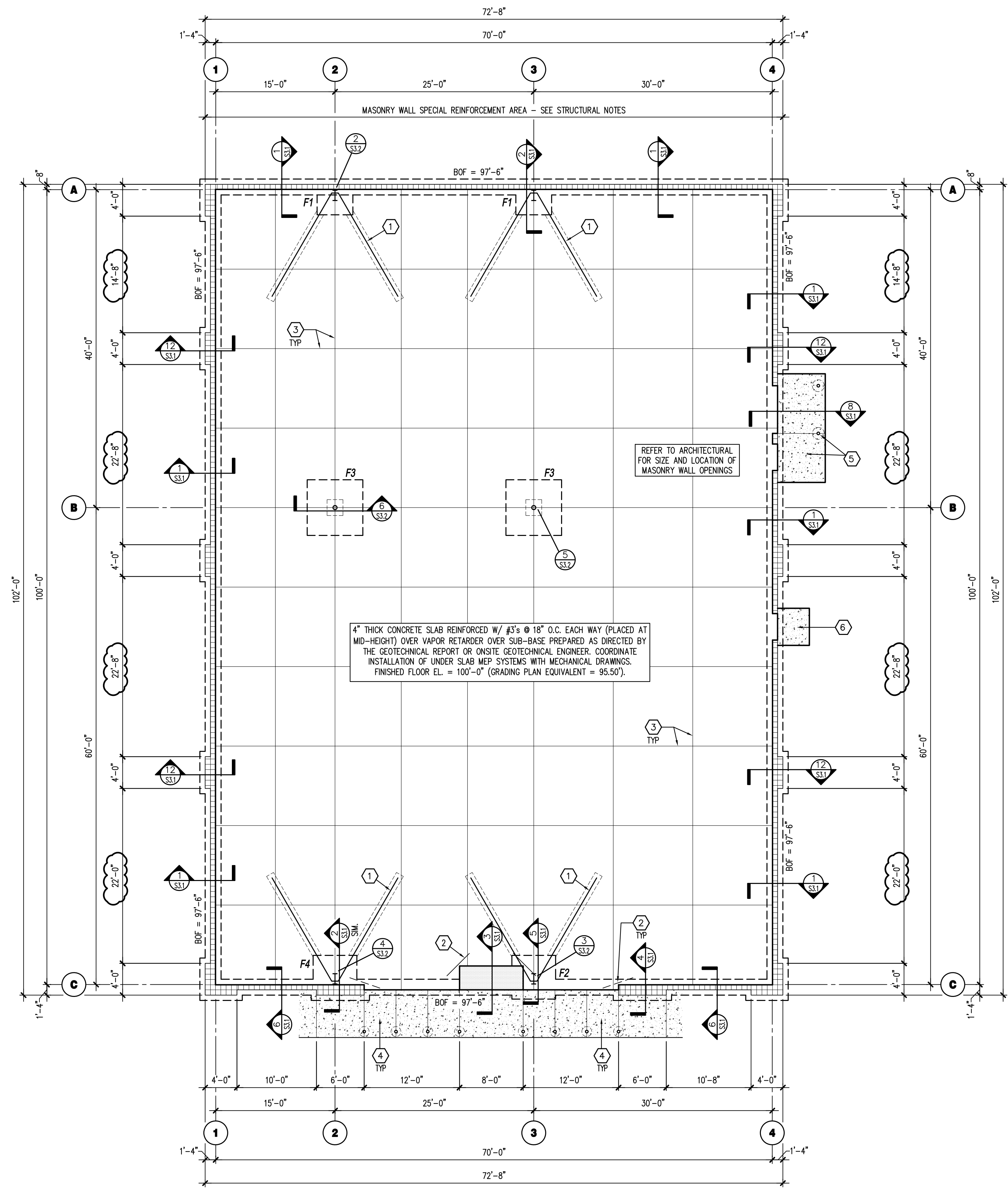
S1.2 SCALE: 1 1/2" = 1'-0"



5 ANCHOR BOLT SECTION

S1.2 SCALE: 1 1/2" = 1'-0"

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FOUNDATION PLAN
SCALE: 1/8" = 1'-0"
NORTH

KEYNOTES

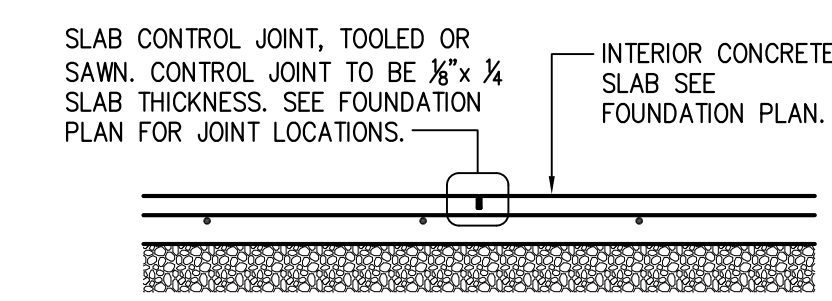
- 1 SLAB HAIRPIN. SEE DETAIL 9/S3.1
- 2 #4x4'-0" ANGLED DOWEL AT SLAB INSIDE CORNERS
- 3 CONTROL OR CONSTRUCTION JOINT SEE DETAILS 1 & 2/S2.1
- 4 SIDEWALK: 4" THICK CONCRETE OVER SUB-BASE MATERIAL PREPARED AS DIRECTED BY THE GEOTECHNICAL REPORT OR ONSITE GEOTECHNICAL ENGINEER. SEE CIVIL DRAWINGS FOR EXTENTS OF SIDEWALK AND ELEVATIONS. PROVIDE REINFORCEMENT AND JOINTS IN CONCRETE AS INDICATED BY CIVIL. (SEE ALSO 1/S2.1 & 3/S2.1). INSTALL BOLLARDS ALIGNED WITH STOREFRONT MULLIONS AS DETAILED BY CIVIL.
- 5 6" WIDE DOOR PAD: 4" THICK CONCRETE REINFORCED W/ #3'S @ 18" O.C. EACH WAY (PLACED MID-HEIGHT) OVER SUB-BASE PREPARED AS DIRECTED BY THE GEOTECHNICAL REPORT OR ONSITE GEOTECHNICAL ENGINEER. EXTEND DOOR PAD MIN. 1'-6" PAST DOOR JAMBS. SEE CIVIL DRAWINGS FOR ELEVATIONS AND BOLLARD DETAILS (ALIGN BOLLARDS WITH OVERHEAD DOOR JAMBS).
- 6 4"x4" (MIN.) DOOR PAD: 4" THICK CONCRETE REINFORCED W/ #3'S @ 18" O.C. EACH WAY (PLACED MID-HEIGHT) OVER SUB-BASE PREPARED AS DIRECTED BY THE GEOTECHNICAL REPORT OR ONSITE GEOTECHNICAL ENGINEER. CENTER PAD ON DOOR. SEE CIVIL DRAWINGS FOR ELEVATIONS.

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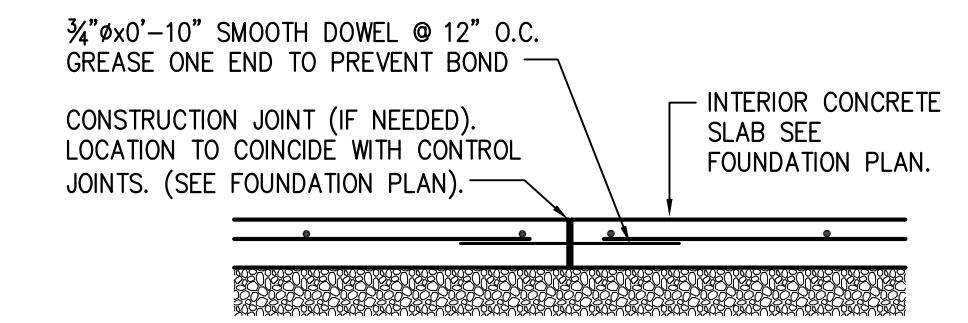
FOUNDATION SCHEDULE

MARK	FOOTING SIZE (W x L x T)	FOOTING REINFORCEMENT		BOTTOM OF FOOTING ELEVATION	PILASTER SIZE	PILASTER REINFORCEMENT		TOP OF PILASTER ELEVATION	MARK
		LONG.	TRANS.			VERT.	TIES		
F1	4'-6"x4'-6"x1'-6"	(6) #5's	(6) #5's	97'-6"					F1
F2	5'-6"x5'-6"x1'-6"	(7) #5's	(7) #5's	97'-6"					F2
F3	7'-0"x7'-0"x1'-6"	(7) #5 EACH WAY TOP & BOT.		96'-0"	2'-6"x2'-6"	(12) #6	6/S3.2	99'-0"	F3
F4	5'-6"x5'-6"x1'-6"	(7) #5's	(7) #5's	97'-6"					F4

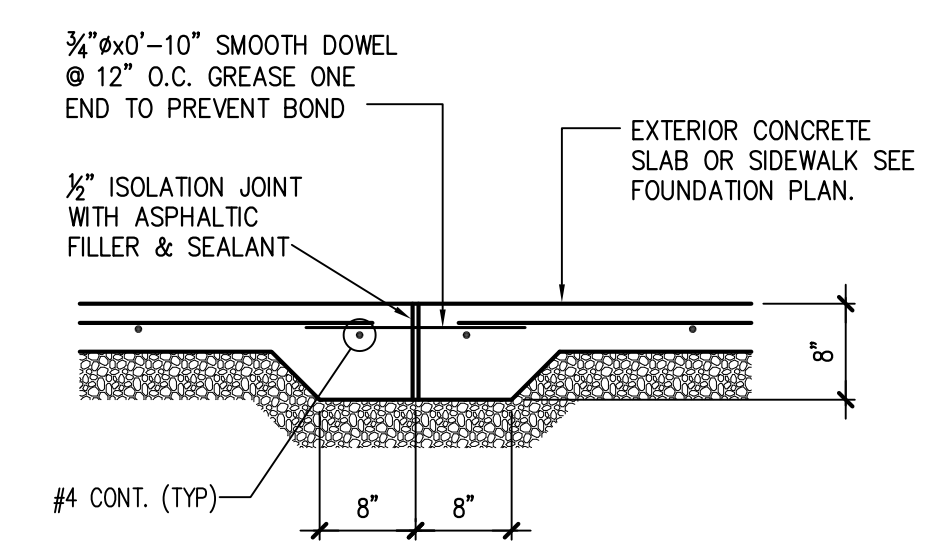
- NOTES:
- ALL ANCHOR BOLTS SHALL BE SIZE, QUANTITY AND SPACING AS SPECIFIED BY THE PRE-ENGINEERED METAL BUILDING MANUFACTURER. MAINTAIN 3" MINIMUM CONCRETE COVER AROUND BOLTS. TIES SHALL WRAP AROUND ANCHOR BOLTS.
 - COLUMN BASE PLATES ARE TO REST ON TOP OF SLAB. PROVIDE LEVEL BEARING SURFACE FOR EVEN CONTACT.
 - COLUMN FOOTING REINFORCEMENT TO BE INTEGRAL WITH CONTINUOUS FOUNDATION REINFORCEMENT (WHERE APPLICABLE).
 - PROVIDE ANCHOR BOLT TEMPLATES AT COLUMN.
 - ALL SPREAD FOOTINGS ARE TO BE CENTERED BENEATH COLUMNS UNLESS NOTED OTHERWISE.



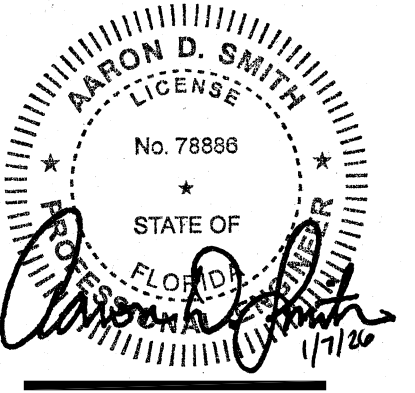
1 TYP. CONTROL JOINT DETAIL
SCALE: 3/4" = 1'-0"



2 TYP. CONSTR. JOINT DETAIL
SCALE: 3/4" = 1'-0"



3 SIDEWALK ISOLATION JOINT
SCALE: 3/4" = 1'-0"



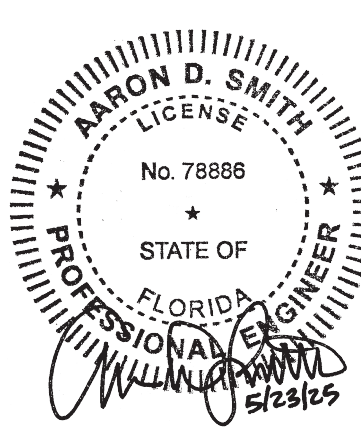
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PROJECT:
NEW O'REILLY AUTO PARTS STORE
HWY FL 44
EUSTIS, FL #2

O'Reilly AUTO PARTS
CORPORATE OFFICES
243 SOUTH PATTERSON
SPRINGFIELD, MISSOURI 65802
(417) 862-2674 TELEPHONE

COMM #	4902
DATE:	5/23/25
REVISION	
DATE:	1/7/26

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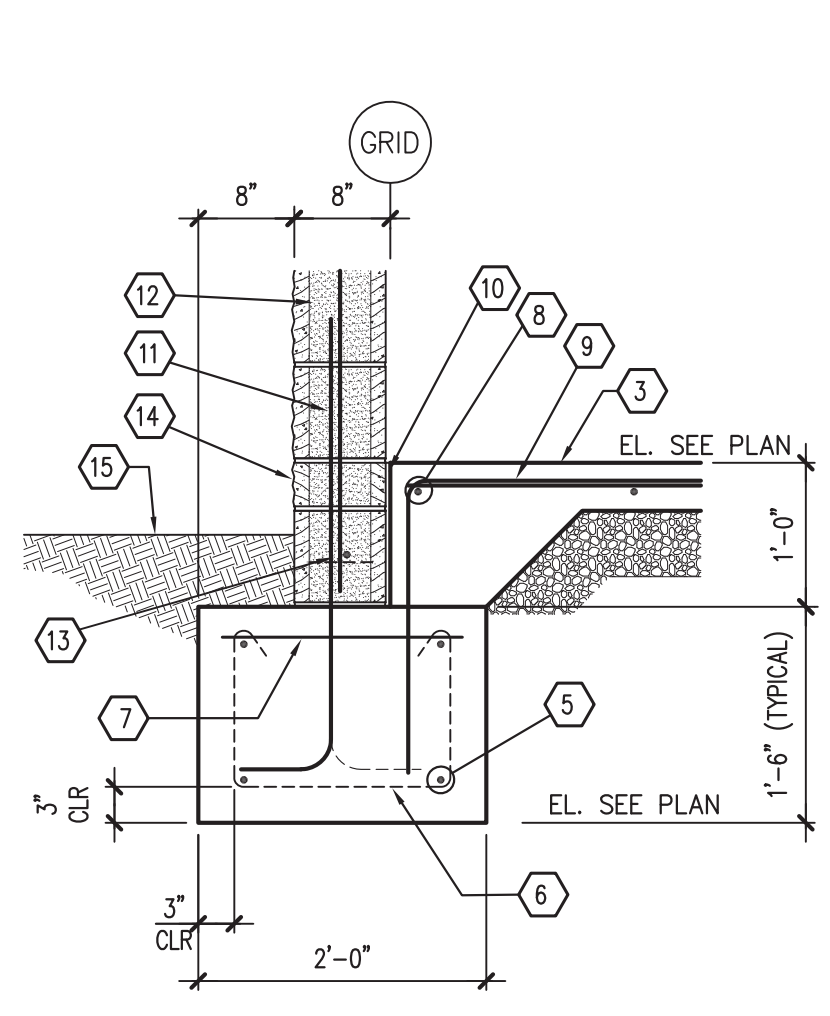
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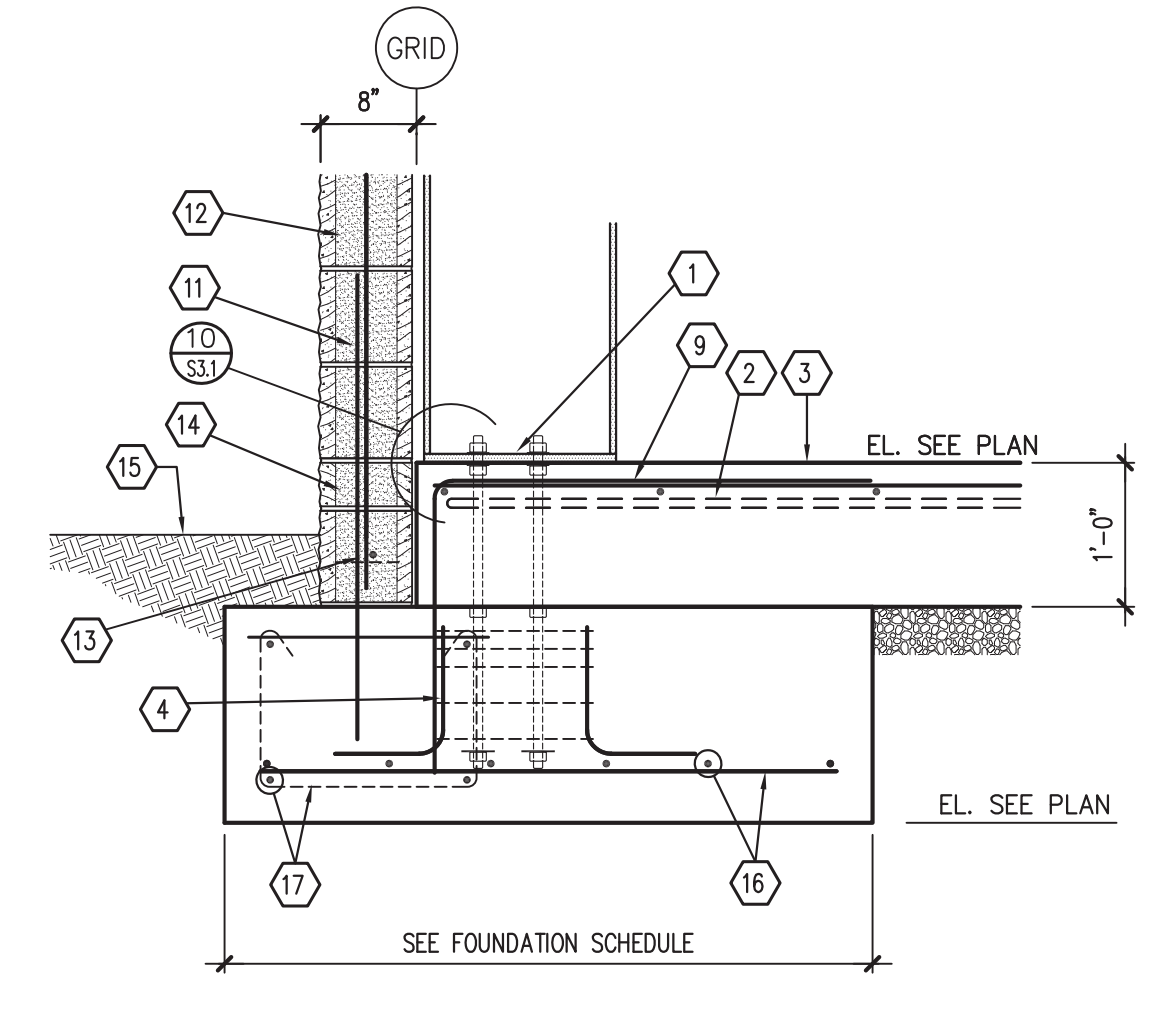
KEYNOTES

- 1 PRE-ENGINEERED METAL BUILDING (PEMB) COLUMN AND ANCHOR BOLTS. REFER TO PEMB ERECTION/PERMIT DRAWINGS FOR ANCHOR BOLT SIZE, QUANTITY, SPACING AND PLAN LOCATION. SEE DETAIL 1/S3.2 FOR CONTRACTOR PROVIDED ANCHOR BOLT ASSEMBLY. SEE PEMB ERECTION/PERMIT DRAWINGS FOR ACTUAL PEMB COLUMN SHAPE AND ARRANGEMENT.
- 2 SLAB HAIRPIN. SEE DETAIL 7/S3.1 FOR GEOMETRIC SHAPE AND DIMENSIONS
- 3 INTERIOR CONCRETE SLAB. SEE FOUNDATION PLAN AND STRUCTURAL NOTES FOR ADDITIONAL INFORMATION
- 4 SEE DETAIL 11/S3.1 FOR VERTICAL ANCHOR BOLT CONFINEMENT REINFORCEMENT
- 5 (2) #5's CONTINUOUS TOP & BOTTOM.
- 6 #3 STIRRUPS @ 48" O.C.
- 7 #3 DOWELS SPACED TO MATCH MASONRY WALL VERTICAL REINFORCEMENT
- 8 #4 CONTINUOUS
- 9 #3x5'-0" DOWEL @ 36" O.C. FIELD BEND AS SHOWN 24"
- 10 1/2" ISOLATION JOINT WITH ASPHALTIC FILLER & SEALANT
- 11 HOOKED DOWEL, MATCHING WALL REINFORCEMENT, PROJECTING INTO MASONRY WALL. SEE STRUCTURAL NOTES, SHEET S1.1
- 12 8" (NOM) CMU. REFER TO STRUCTURAL NOTES ON SHEET S1.1 FOR WALL REINFORCEMENT AND ADDITIONAL REQUIREMENTS
- 13 FIRST COURSE TO BE CONTINUOUS 8" (NOM) BOND BEAM REINFORCED W/ (1) #5 CONTINUOUS, SEE STRUCTURAL NOTES, SHEET S1.1
- 14 4" TALL CMU COURSE
- 15 FOR FINISHED GRADES, PAVING AND EXTERIOR CONCRETE SIDEWALK/DOOR PADS REFER TO CIVIL AND SITE DEVELOPMENT DRAWINGS. REFER ALSO TO SECTION 8/S3.1
- 16 COLUMN FOOTING REINFORCEMENT - SEE FOUNDATION SCHEDULE.
- 17 CONTINUOUS FOOTING REINFORCEMENT TO RUN THROUGH COLUMN FOUNDATIONS. SEE SECTION 1/S3.1 FOR REINFORCEMENT INFORMATION.
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- 19 AT EXTERIOR DOOR LOCATIONS, DRILL 4" DEEP HOLES AND INSTALL #4x1'-4" DOWELS @ 24" O.C. AND ADHERE IN PLACE. REFER TO STRUCTURAL NOTES FOR ADHESIVE ANCHOR INSTALLATION
- 20 STEEL BOLLARD SEE SITE PLAN.
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- 22 #3's @ 18" O.C. TOP & BOTTOM
- 23 ADDITIONAL #4 CONTINUOUS TOP & BOTTOM
- 24 (3) #3 TIES @ 12" O.C. (TOP) AND AT 12" O.C. FOR REMAINING HEIGHT OF PLASTER

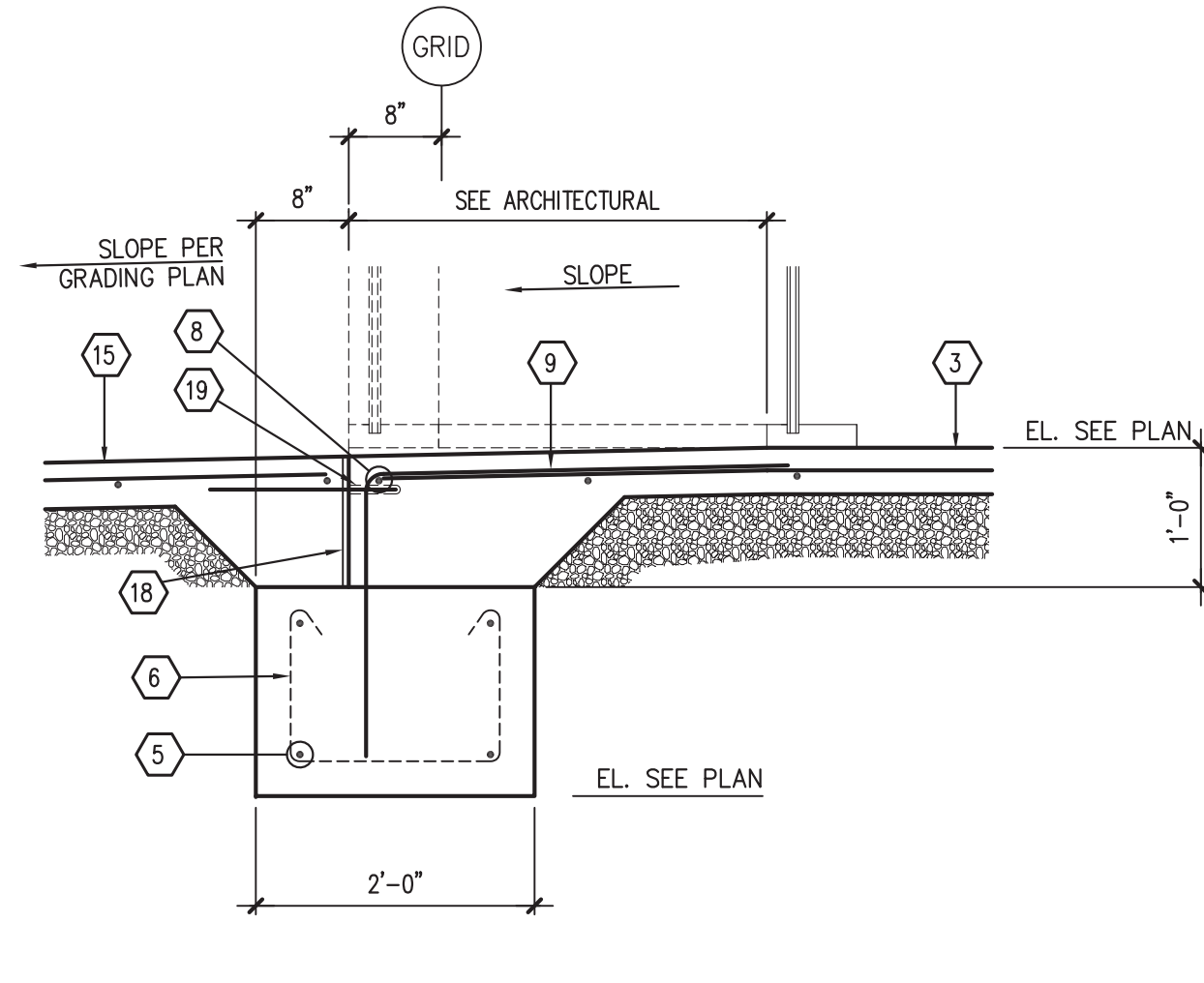
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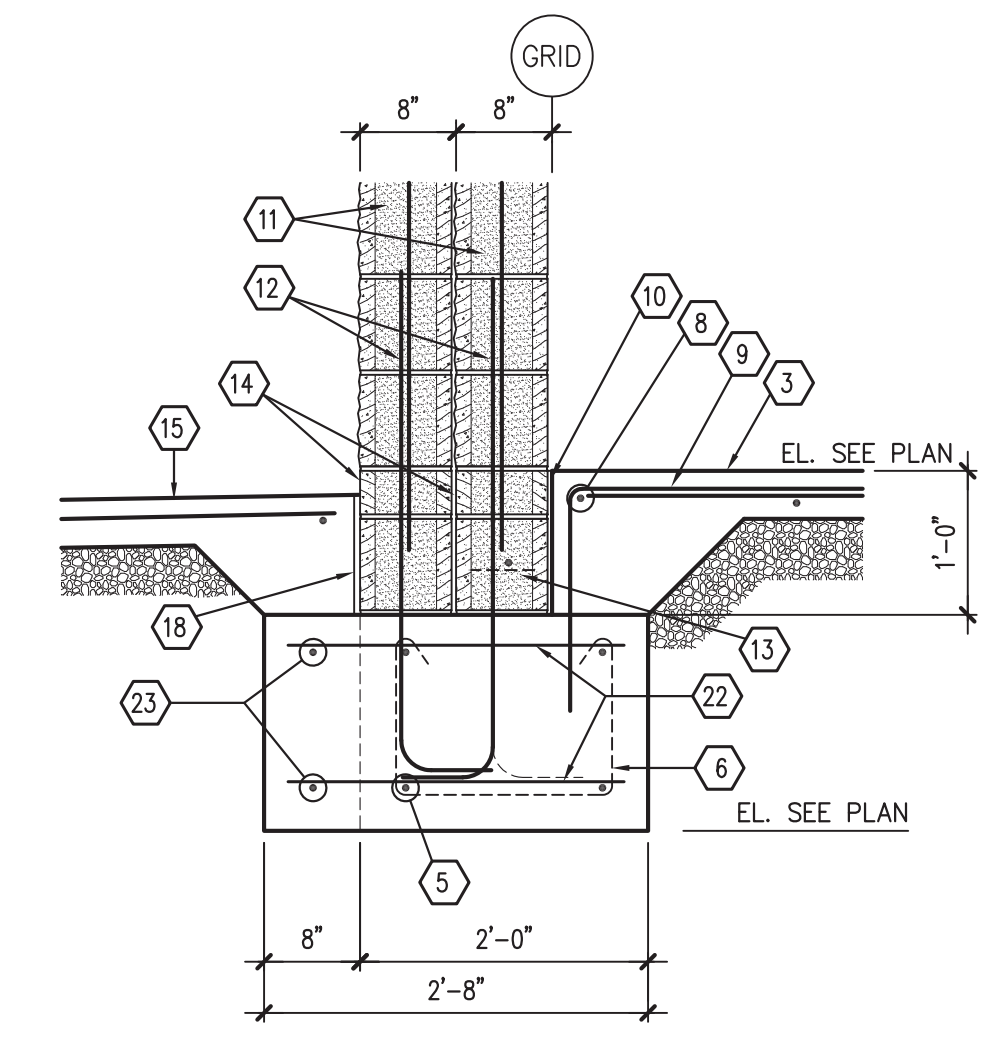
1 SECTION
 S3.1 SCALE: 3/4" = 1'-0"



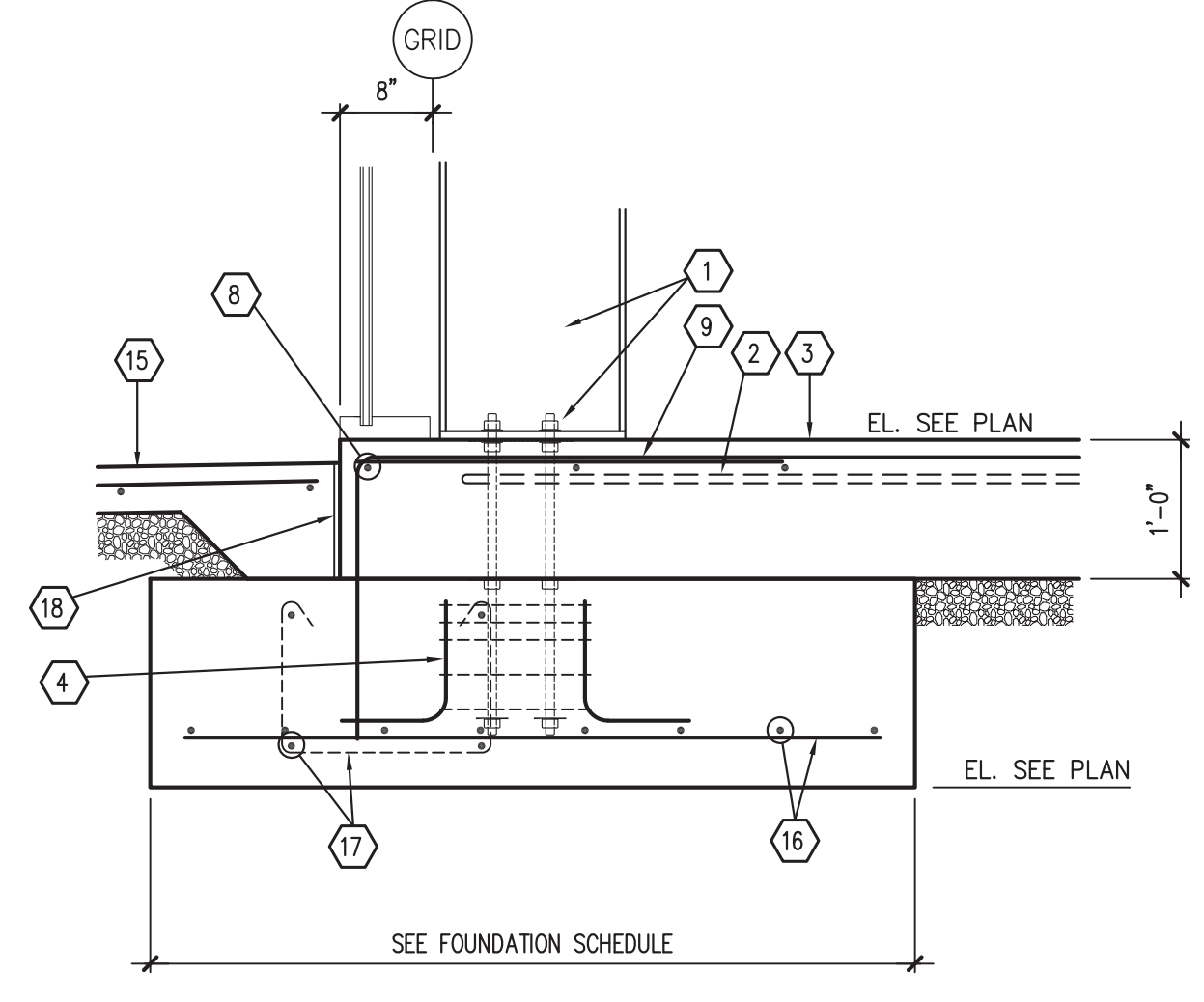
2 SECTION
 S3.1 SCALE: 3/4" = 1'-0"



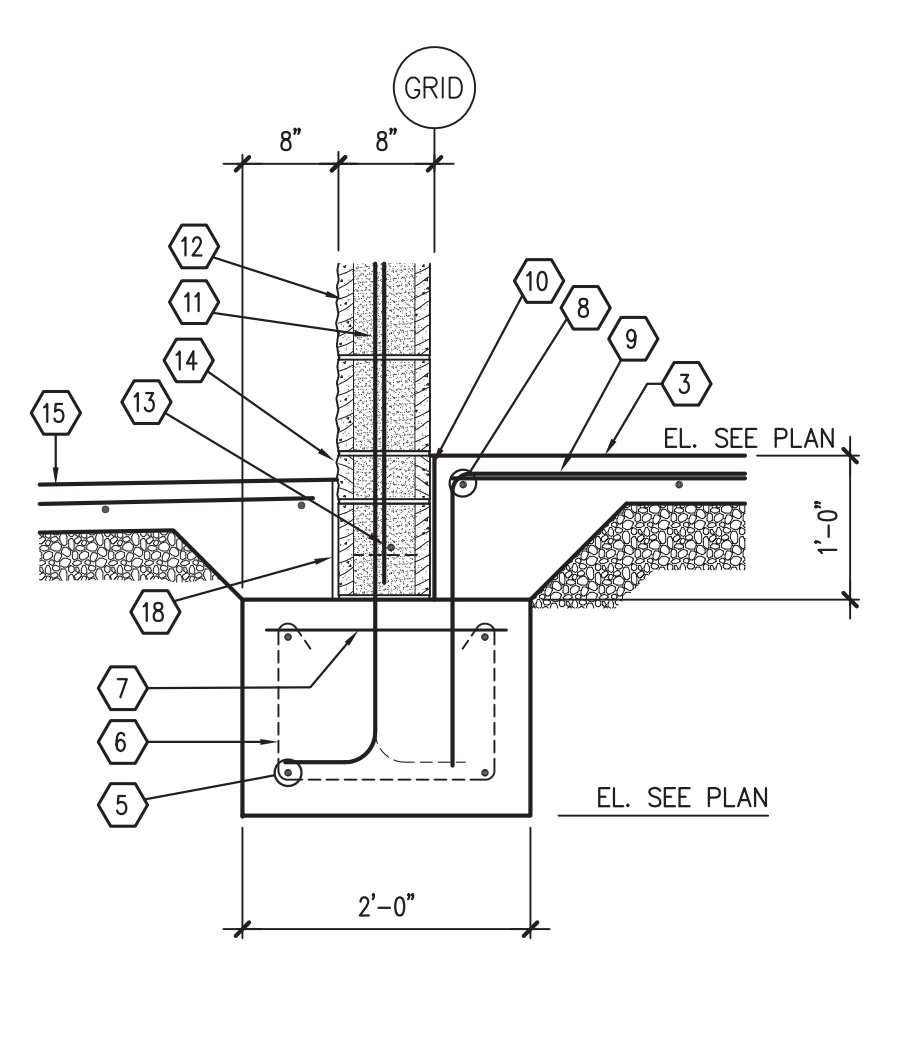
3 SECTION
 S3.1 SCALE: 3/4" = 1'-0"



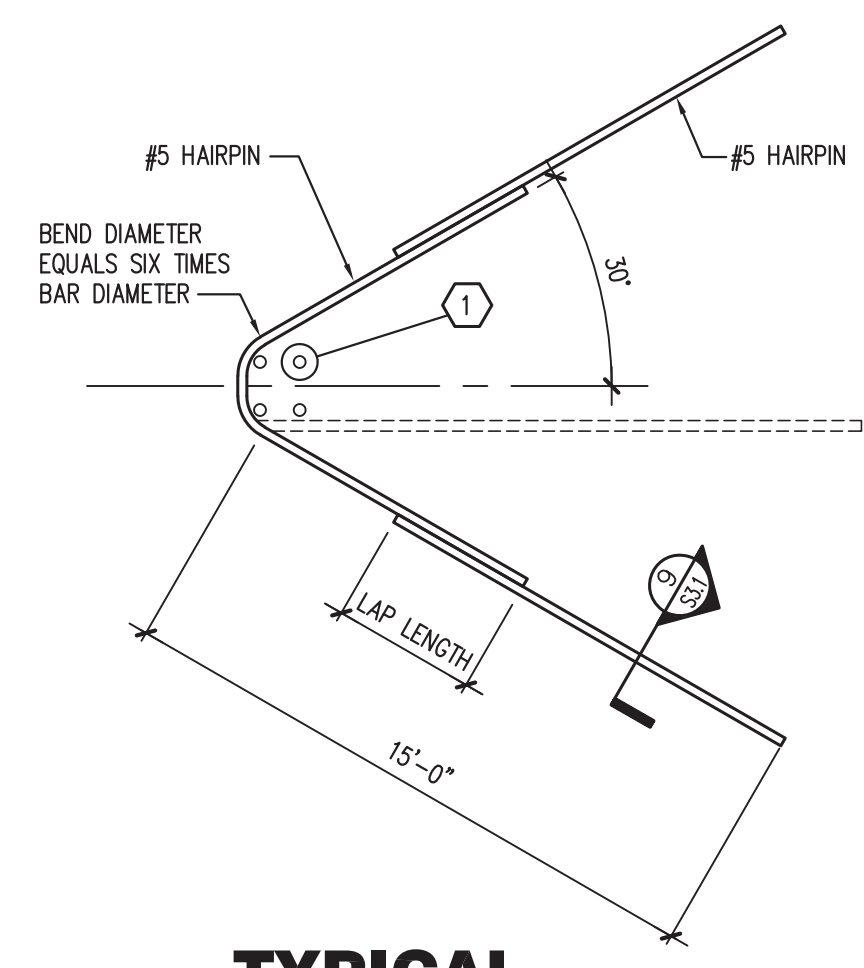
4 SECTION
 S3.1 SCALE: 3/4" = 1'-0"



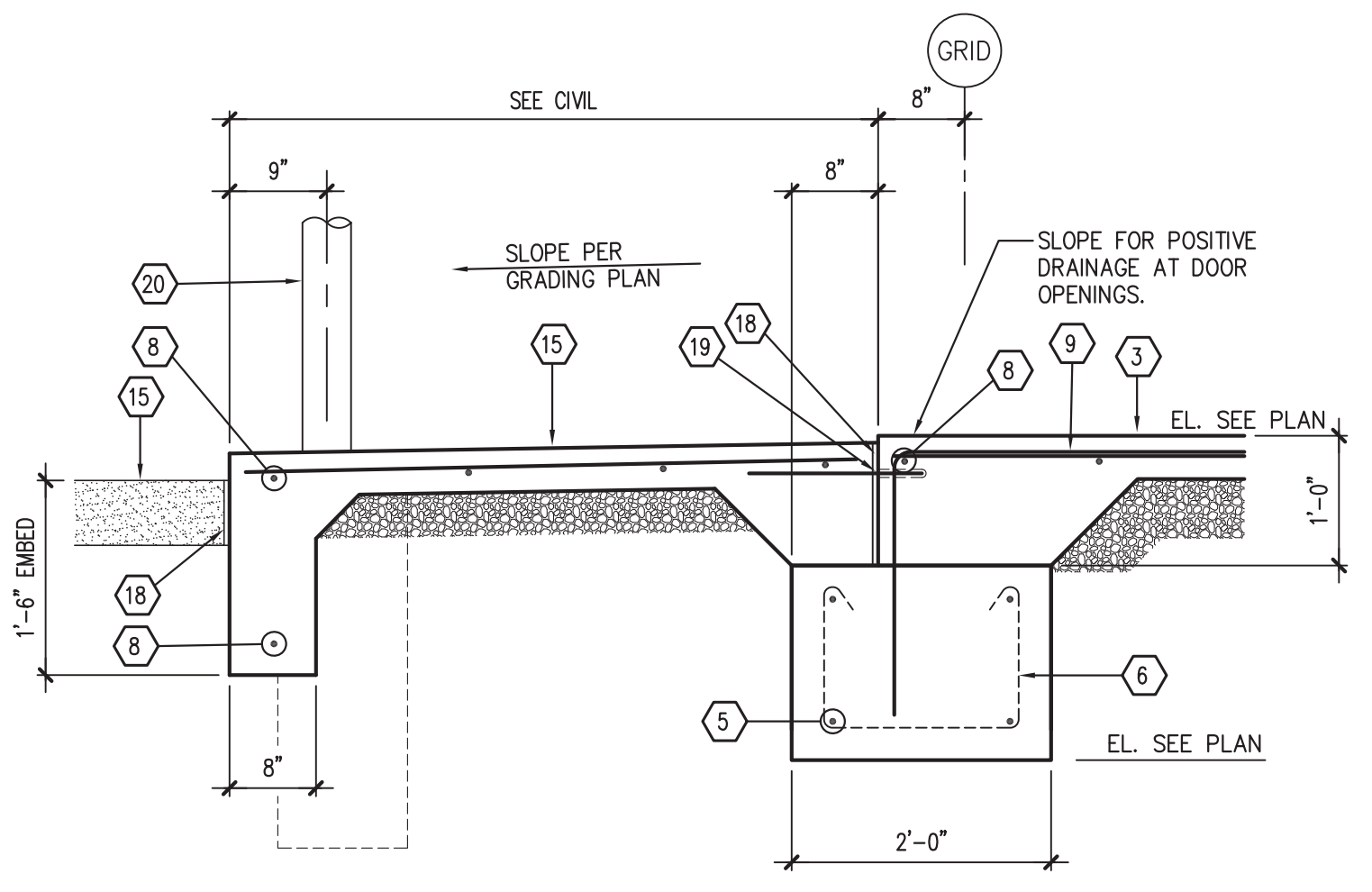
5 SECTION
 S3.1 SCALE: 3/4" = 1'-0"



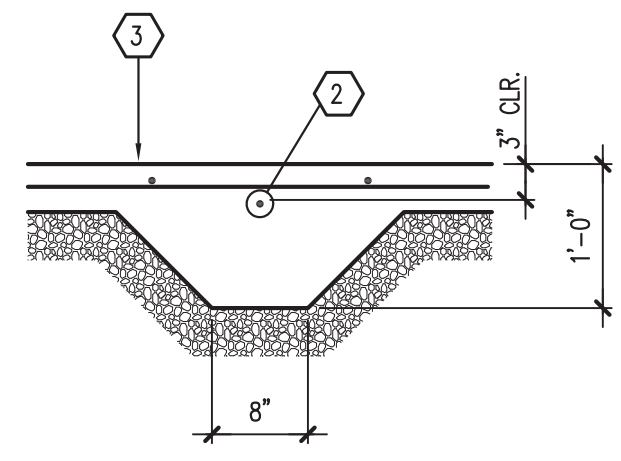
6 SECTION
 S3.1 SCALE: 3/4" = 1'-0"



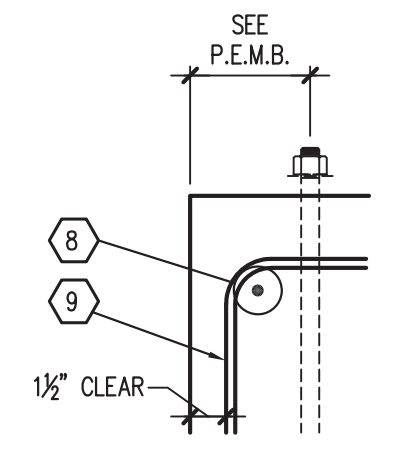
7 TYPICAL HAIRPIN DETAIL
 S3.1 SCALE: NONE



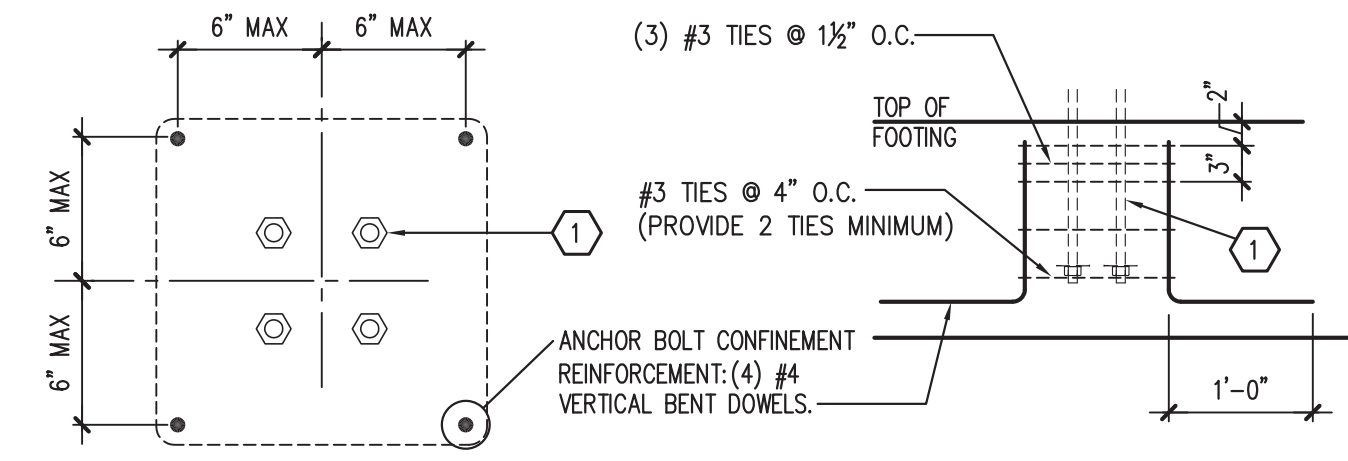
8 SECTION
 S3.1 SCALE: 3/4" = 1'-0"



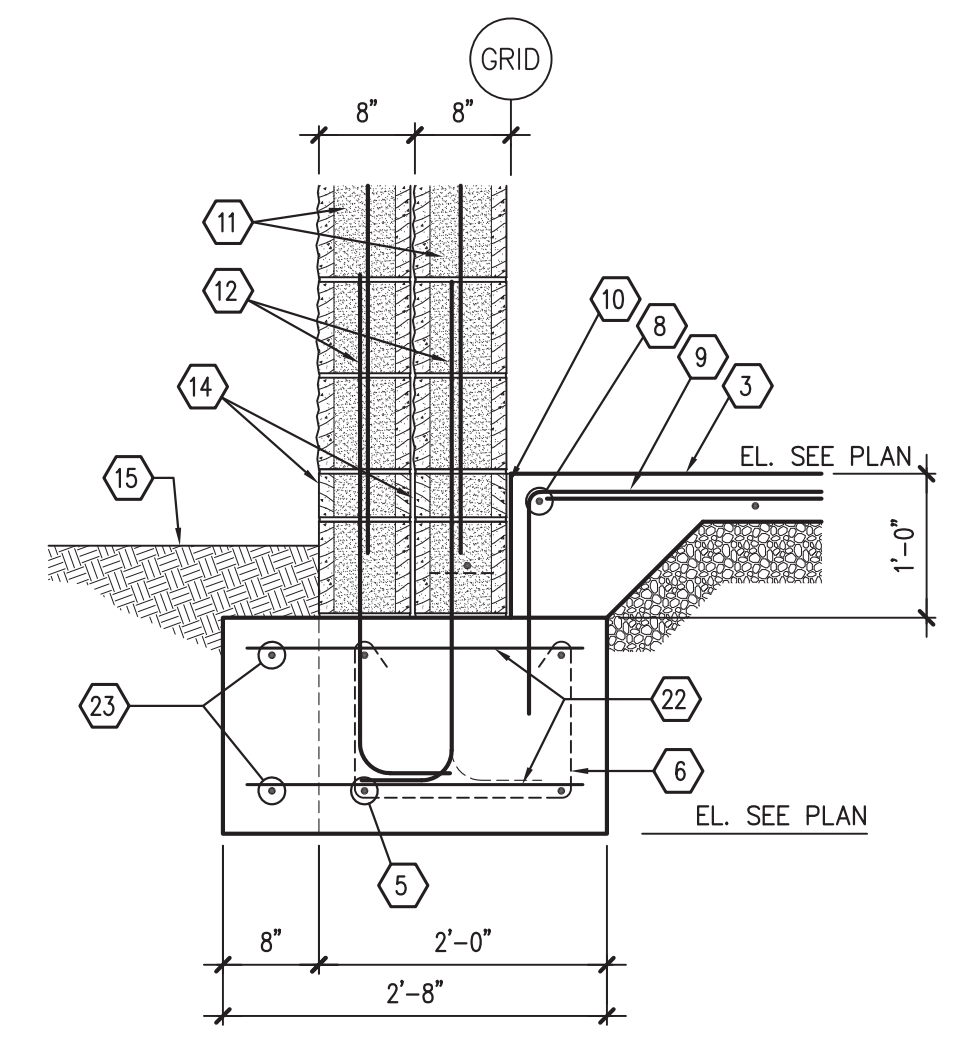
9 SECTION
 S3.1 SCALE: 3/4" = 1'-0"



10 DETAIL
 S3.1 SCALE: 1 1/2" = 1'-0"



11 SECTION
 S3.1 SCALE: VARIES



12 SECTION
 S3.1 SCALE: 3/4" = 1'-0"

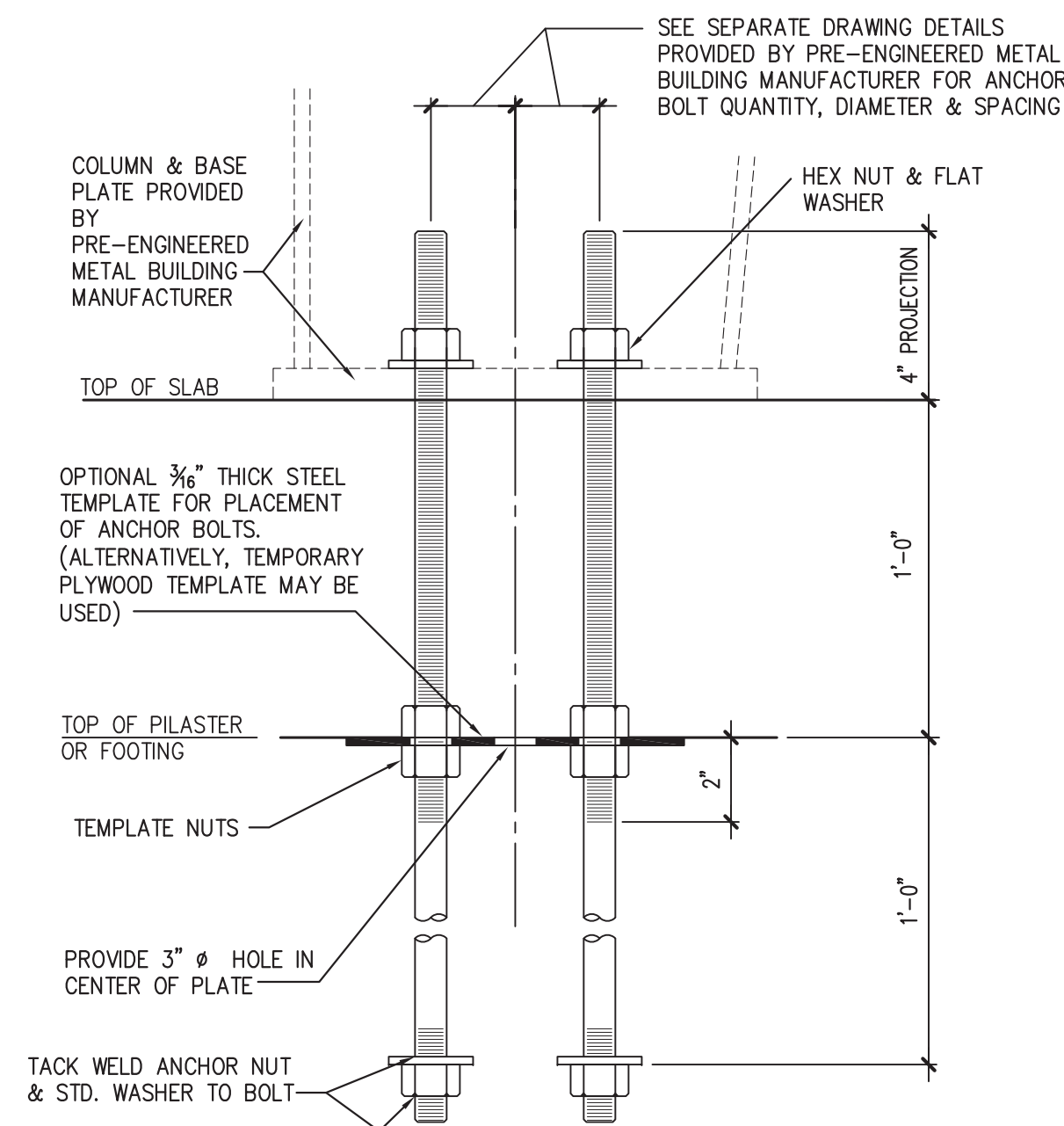
JSS JS Smith Consulting Engineers, P.C.
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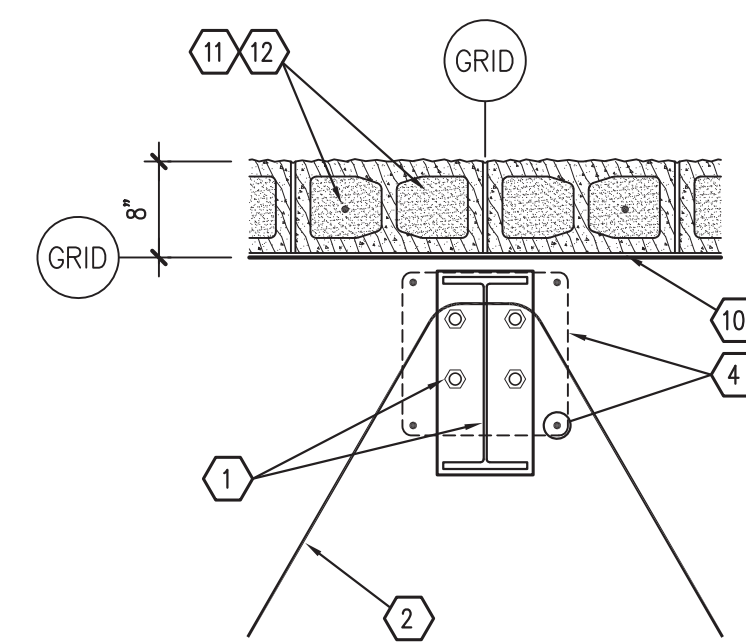


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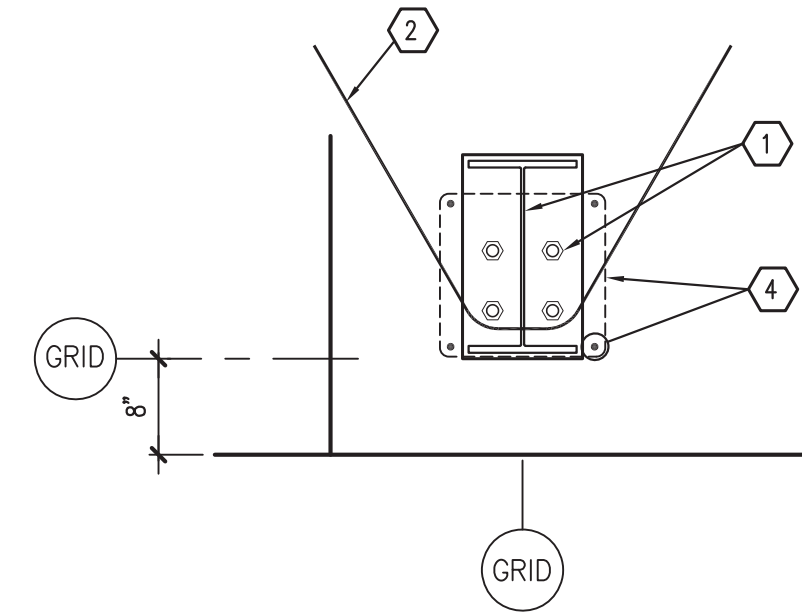


1 ANCHOR ASSEMBLY
S3.2 SCALE: NONE

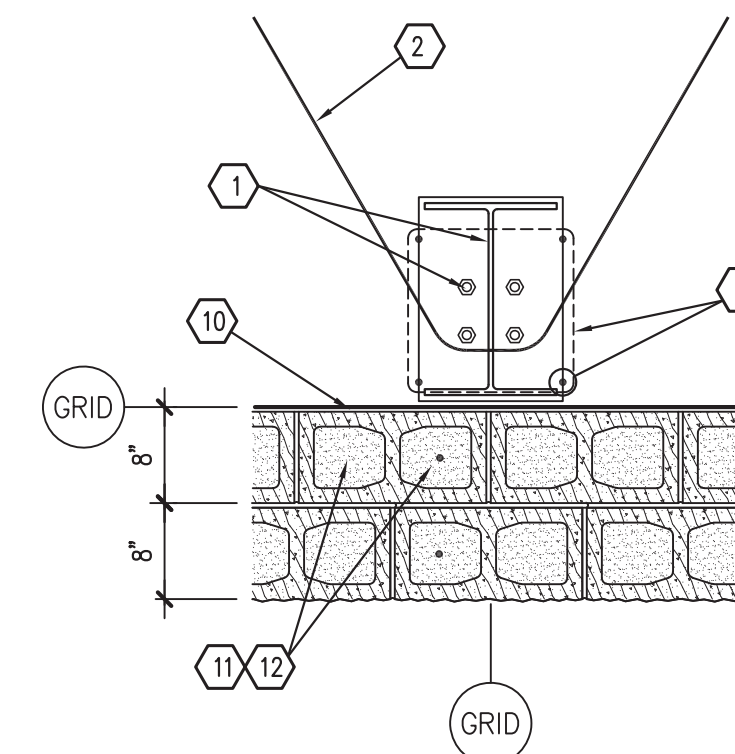
NOTE: GENERAL CONTRACTOR PROVIDED ANCHOR BOLT ASSEMBLY TO BE PLACED IN FORM PRIOR TO POURING CONCRETE. BOLT MATERIAL MAY BE F1554 GR. 36, OR CONTINUOUSLY THREADED A36 ROD. ALL ANCHOR BOLTS SHALL BE CLEAN AND FREE OF ALL OIL, GREASE, ICE, ETC.



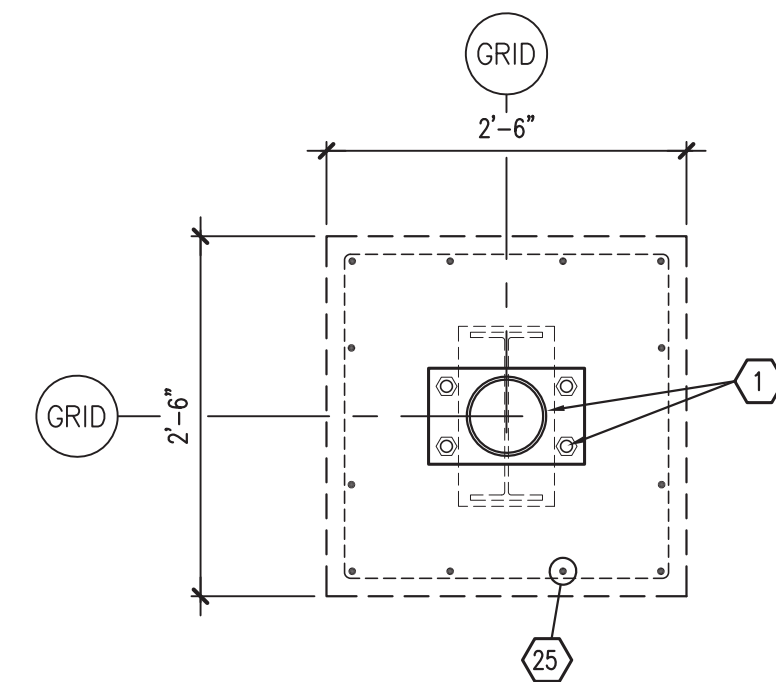
2 DETAIL
S3.2 SCALE: 3/4" = 1'-0"



3 DETAIL
S3.2 SCALE: 3/4" = 1'-0"

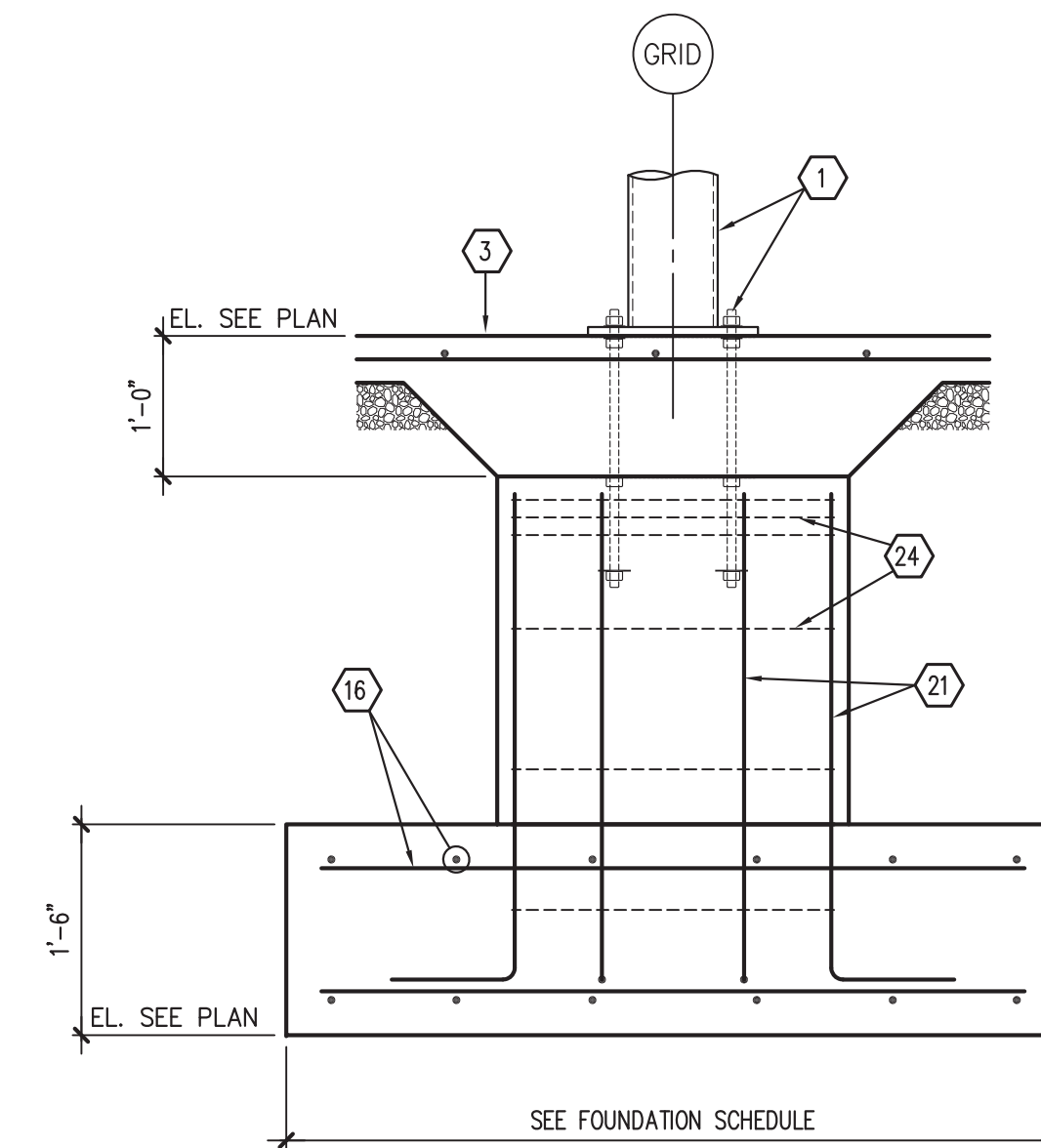


4 DETAIL
S3.2 SCALE: 3/4" = 1'-0"

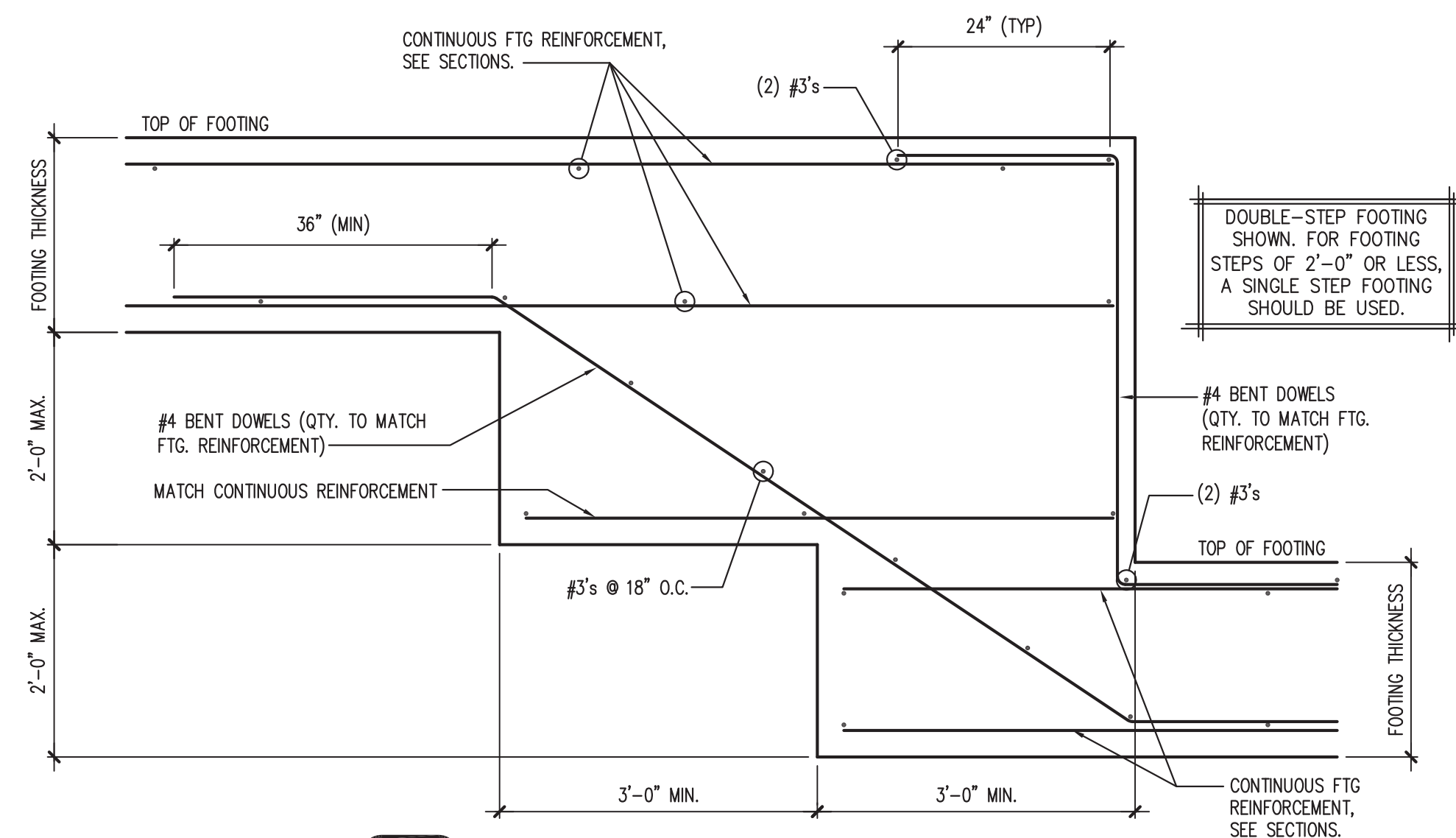


5 DETAIL
S3.2 SCALE: 3/4" = 1'-0"

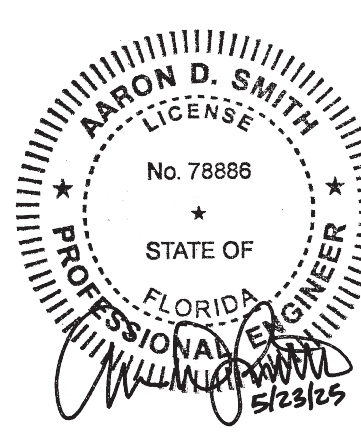
REFER TO PEMB SUPPLIER FOR COLUMN SHAPE, SIZE, AND ORIENTATION AT THIS LOCATION



6 SECTION
S3.2 SCALE: 3/4" = 1'-0"



7 TYPICAL STEPPED FOOTING
S3.2 SCALE: NONE (IF REQUIRED BY FIELD CONDITIONS)



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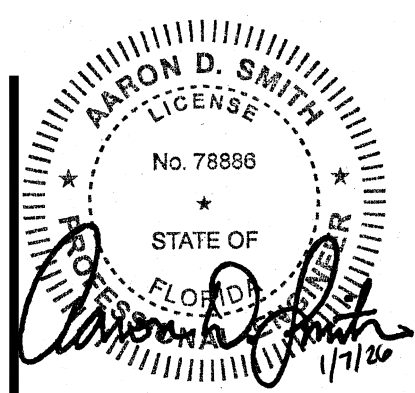
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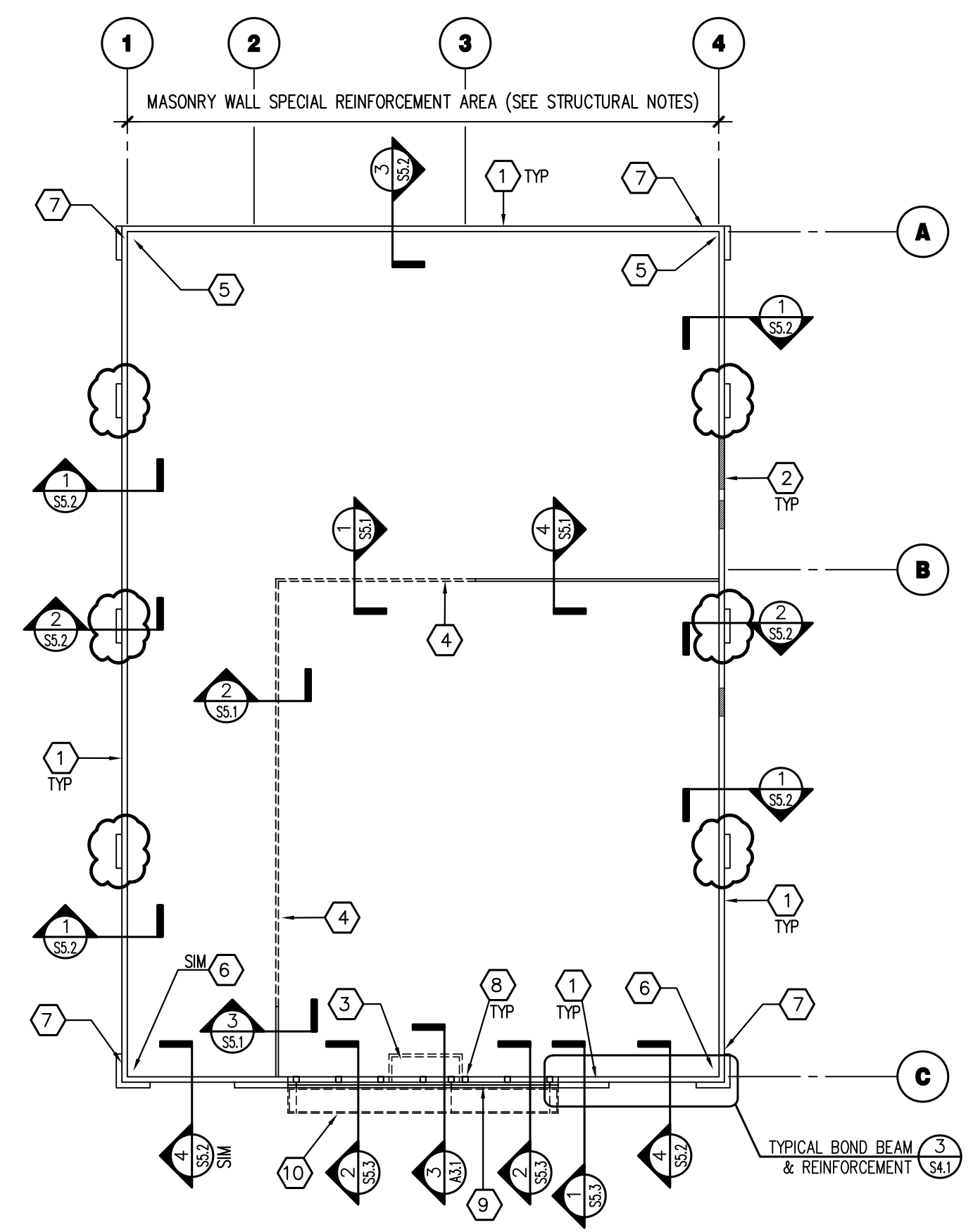
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FRAMING PLAN

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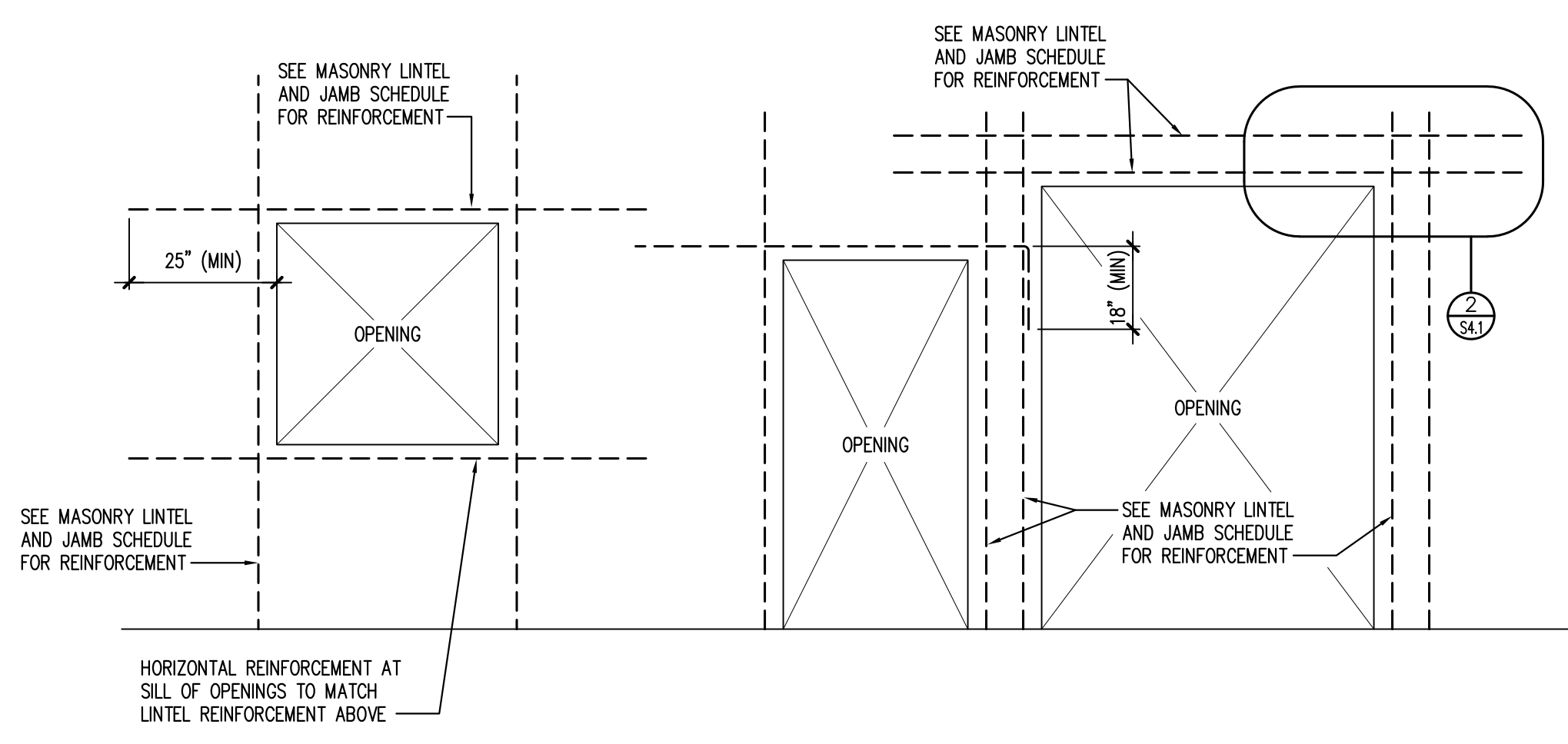
FRAMING KEY PLAN



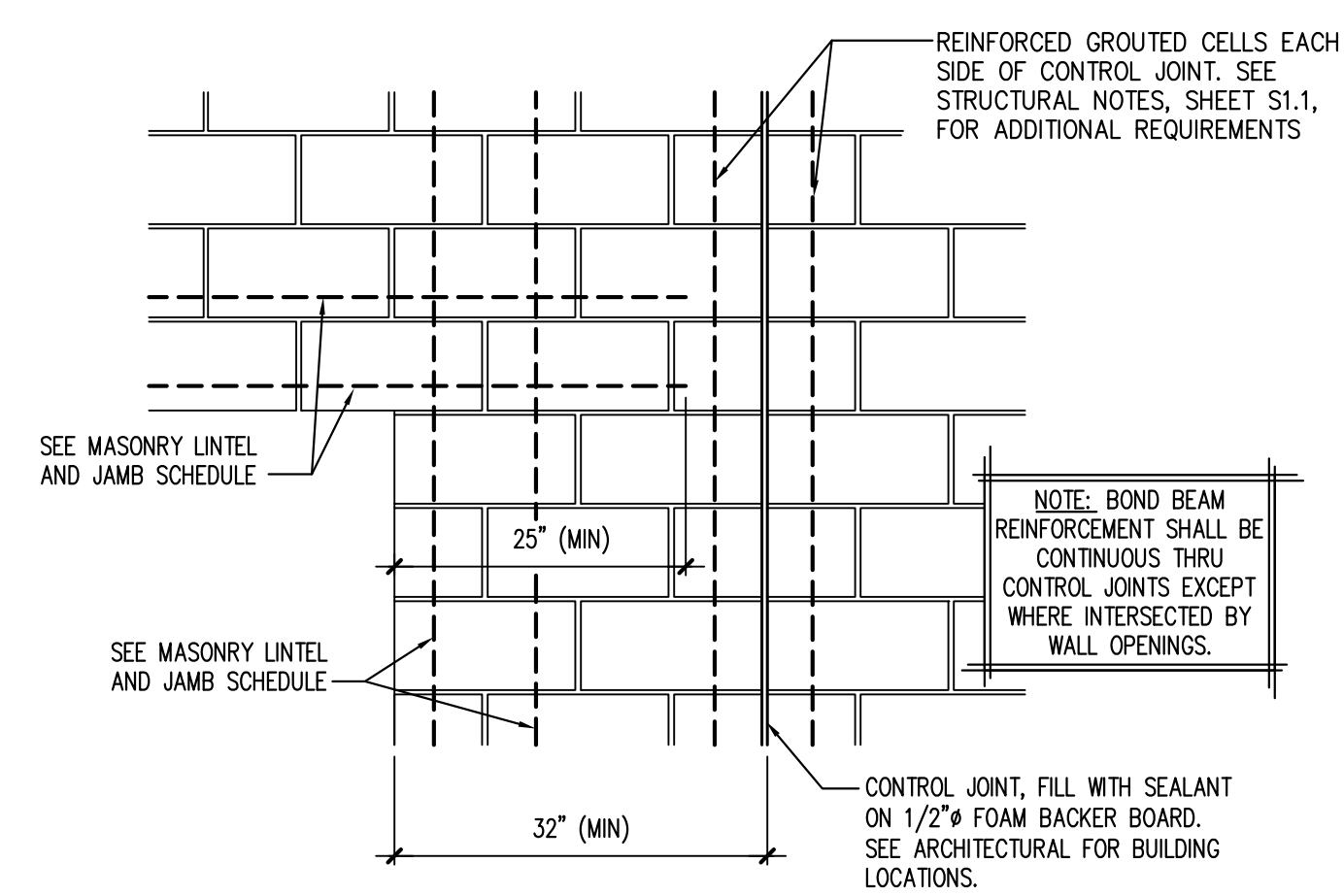
MASONRY LINTEL & JAMB SCHEDULE	
	MAXIMUM OPENING WIDTH: 4'-0" FULLY GROUTED LINTEL DEPTH: 8" HORIZONTAL REINFORCEMENT: (2) #5's JAMB: (1) #5 IN ADJACENT CELL
	MAXIMUM OPENING WIDTH: 8'-0" FULLY GROUTED LINTEL DEPTH: 1'-4" HORIZONTAL REINFORCEMENT: (4) #5's JAMB: (2) #5 IN (2) ADJACENT CELLS
FOR ADDITIONAL INFORMATION SEE STRUCTURAL NOTES AND 1/S4.1 - MASONRY WALL OPENINGS ELEVATION 1/S4.1 2/S4.1 - MASONRY LINTEL, JAMB AND CONTROL JOINT DETAIL 3/S4.1 - PLAN VIEW OF TYPICAL MASONRY BOND BEAM	

KEYNOTES

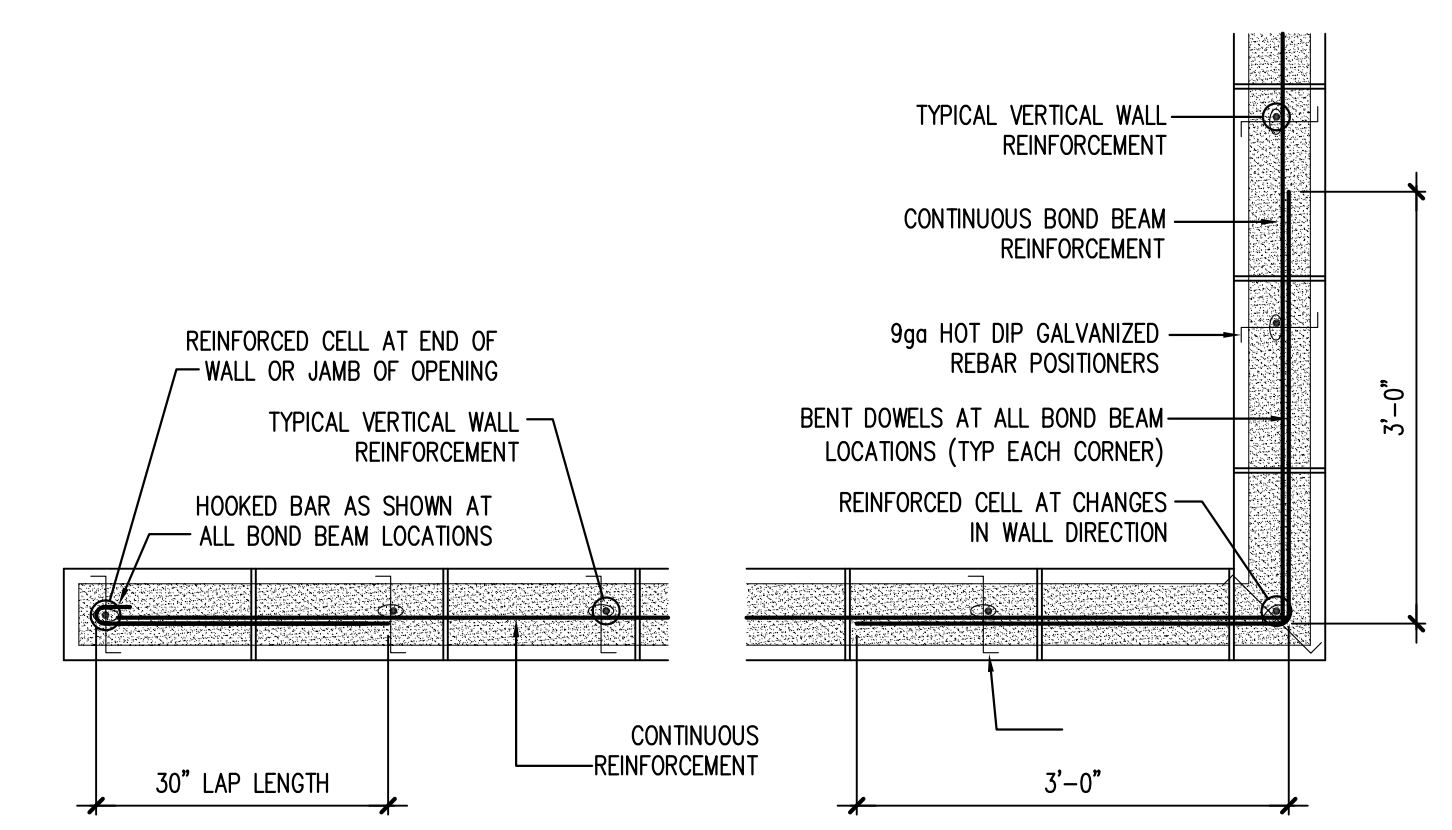
- 1 MASONRY EXTERIOR WALL CONSTRUCTION: FOR MASONRY CONTROL JOINT SEE DETAIL 2/S4.1. COORDINATE WITH ARCHITECTURAL FOR CONTROL JOINT LOCATIONS. FOR LOCATIONS OF MECHANICAL OPENINGS IN MASONRY WALLS, REFER TO MECHANICAL DRAWINGS. SEE STRUCTURAL NOTES, SHEET S1.1, FOR BOND BEAM AND ADDITIONAL REQUIREMENTS.
- 2 MASONRY LINTEL OVER DOOR OPENINGS, SEE MASONRY LINTEL SCHEDULE.
- 3 PROVIDE 16ga LIGHT GAUGE STEEL FRAMING AT ENTRY LOCATION. ALIGN FRAMING MEMBERS WITH METAL STUDS OVER STOREFRONT (SPACING NOT TO EXCEED 24" O.C.) ALL STUD-TO-STUD CONNECTIONS TO BE (3) #8 SDS (MIN). REFER TO ARCHITECTURAL SECTIONS FOR FRAMING PROFILES AND MEMBER DEPTHS.
- 4 METAL STUD FULL HEIGHT & SUSPENDED PARTITION WALL (PROVIDE MOVEMENT JOINTS AS LOCATED AND DETAILED ON THE ARCHITECTURAL DRAWINGS)
- 5 REFER TO SECTION 6/S5.2 FOR SPANDREL BEAM CONNECTIONS TO MASONRY WALL
- 6 REFER TO SECTION 5/S5.2 FOR SPANDREL BEAM CONNECTIONS TO MASONRY WALL
- 7 FULLY GROUT ALL CELLS IN MASONRY WALL A MINIMUM OF 4'-8" IN EACH DIRECTION FROM CORNER TO ALLOW FOR METAL BUILDING CONNECTIONS.
- 8 CONTRACTOR PROVIDED HSS 8"x8"x1/4" VERTICAL TUBES @ 60" O.C (MAX) FOR ENTIRE LENGTH OF STOREFRONT AND AT EACH CANOPY ROD HANGER, REFER TO SECTION 3 & 4/S5.3
- 9 CONTRACTOR PROVIDED HSS 6"x4"x1/4" HORIZONTAL TUBE SPANNING ENTIRE LENGTH OF STOREFRONT, REFER TO SECTIONS 2, 3, AND 4/S5.3
- 10 PRE-FABRICATED METAL CANOPY WITH HANGER RODS, REFER TO ARCHITECTURAL DRAWINGS FOR QUANTITY AND LOCATION OF EACH HANGER ROD.



1 MASONRY WALL OPENINGS
S4.1 SCALE: NONE

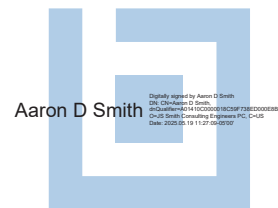


2 MASONRY LINTEL, JAMB AND CONTROL JOINT DETAIL
S4.1 SCALE: 3/4" = 1'-0"

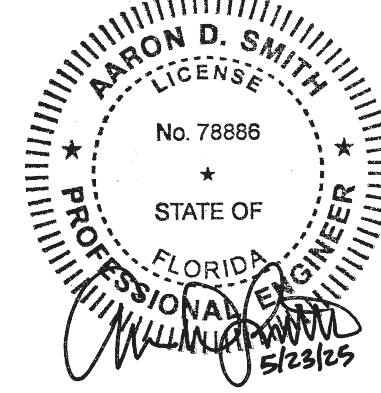


3 PLAN VIEW OF TYPICAL MASONRY BOND BEAM
S4.1 SCALE: 3/4" = 1'-0"

JS Smith Consulting Engineers, P.C.
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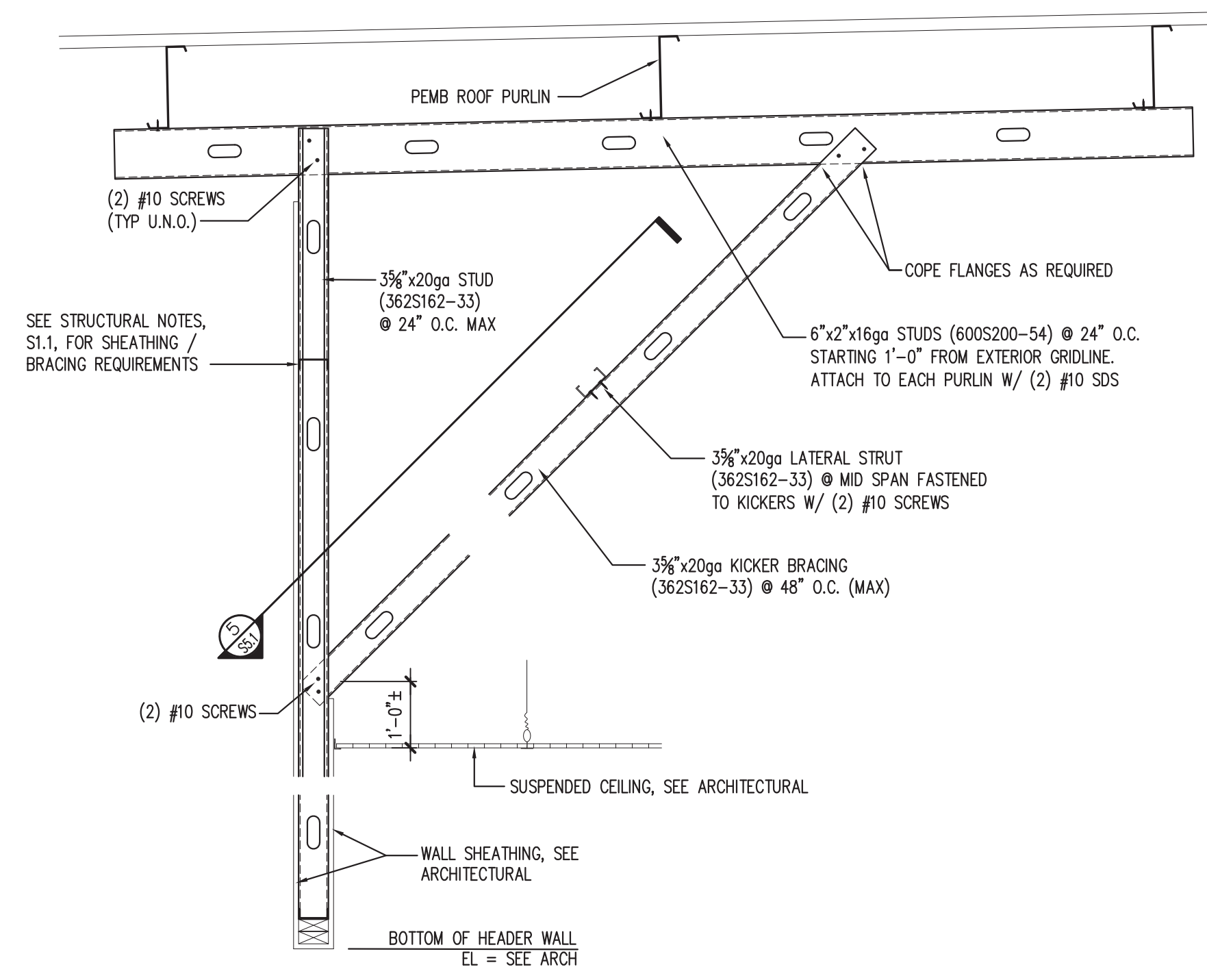
417.862.0558
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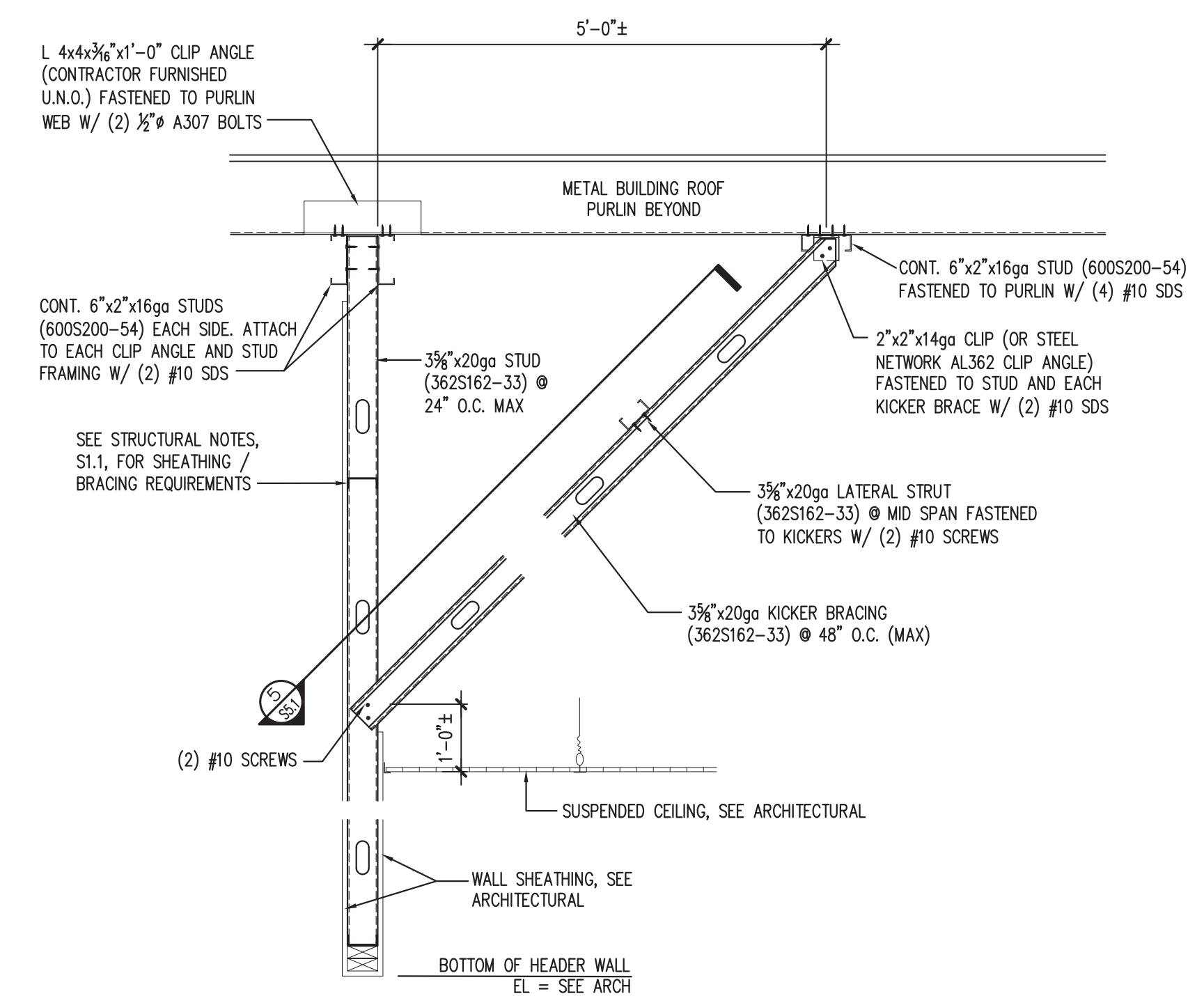
PROJECT:
NEW O'REILLY AUTO PARTS STORE
HWY FL 44
EUSTIS, FL #2

O'Reilly AUTO PARTS
 CORPORATE OFFICES
 243 SOUTH PATTERSON
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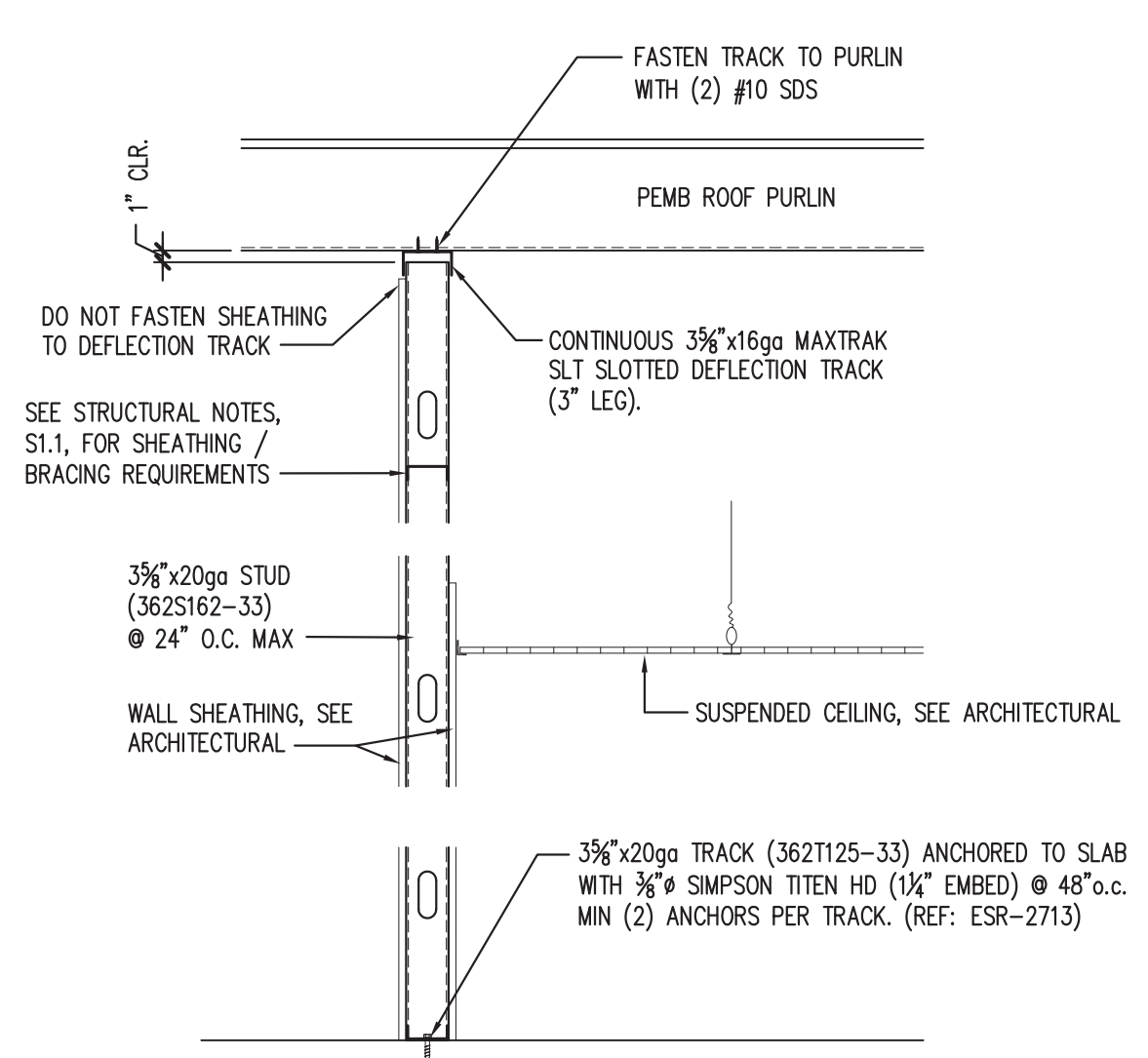
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DATE:	5/23/25
REVISION	
DATE:	



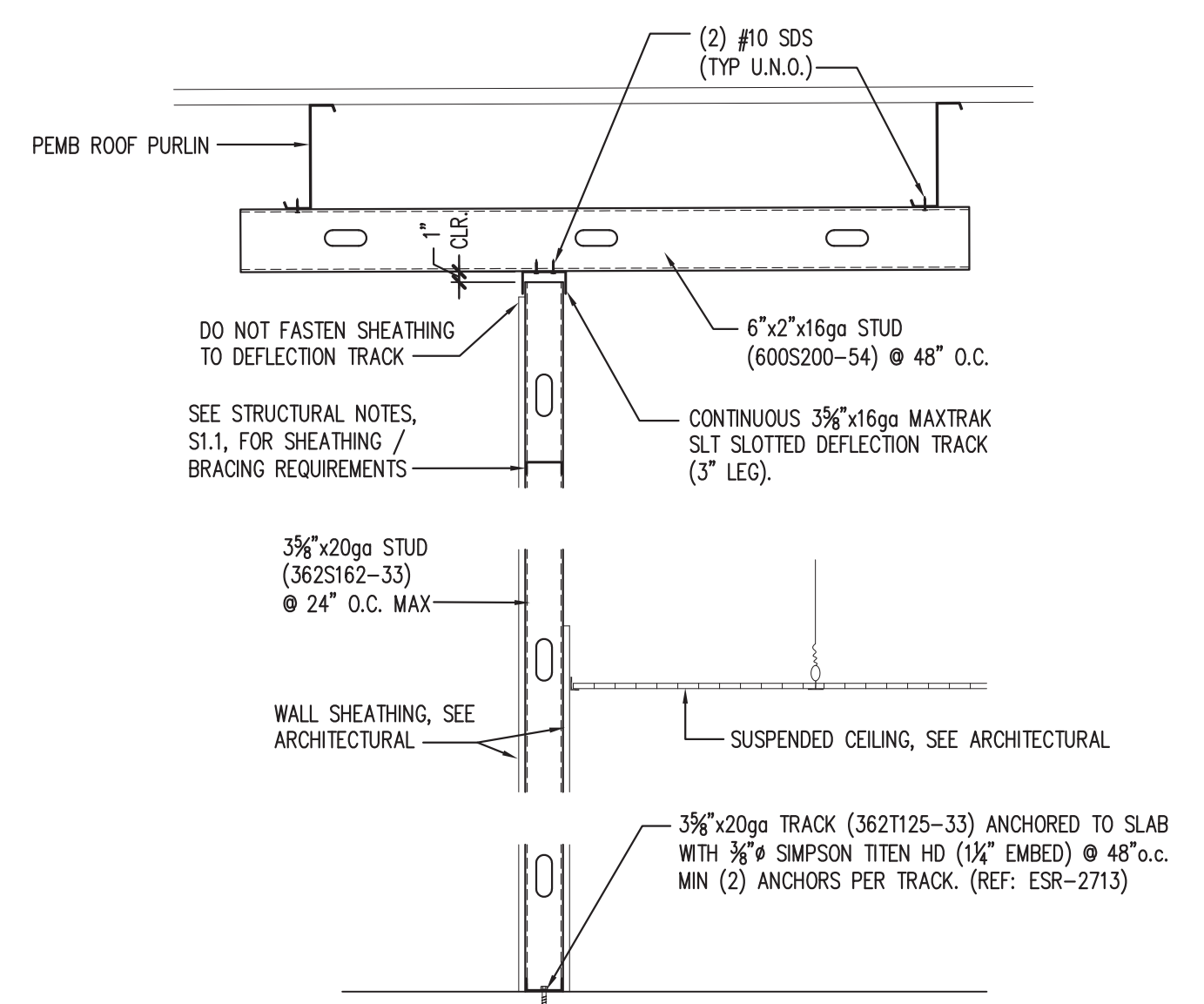
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S5.1 SCALE: 3/4" = 1'-0"



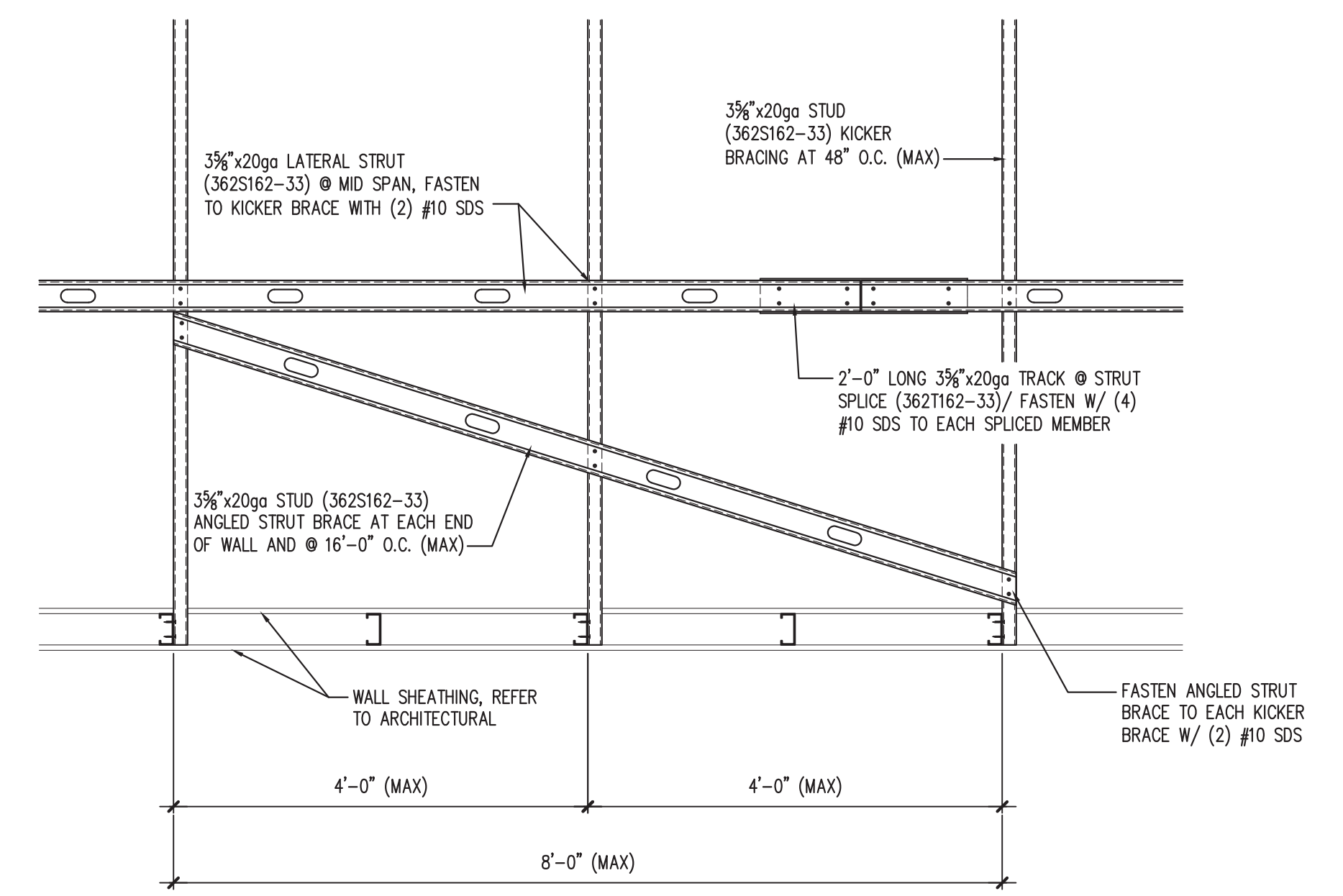
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S5.1 SCALE: 3/4" = 1'-0"



3 SECTION
S5.1 SCALE: 3/4" = 1'-0"

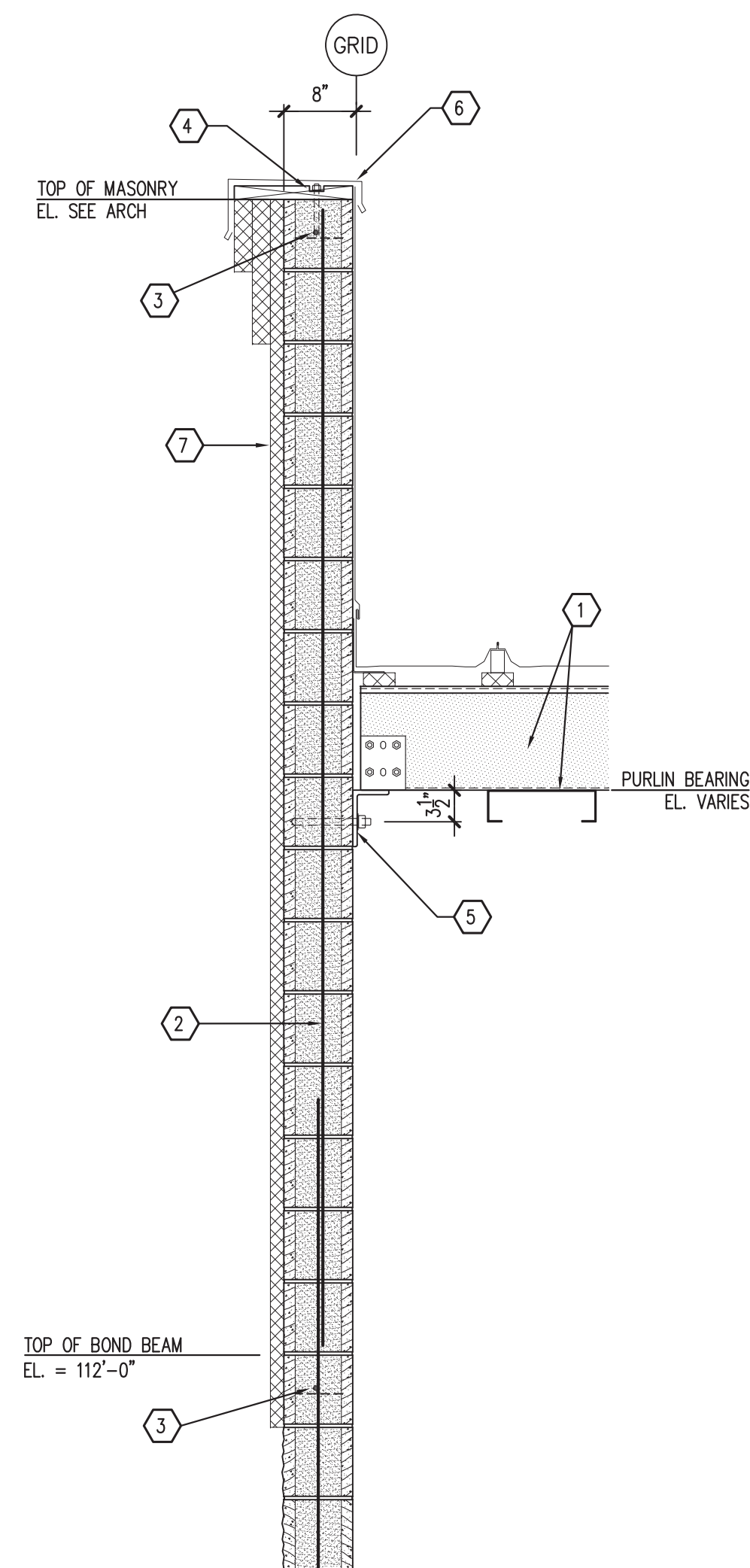


4 SECTION
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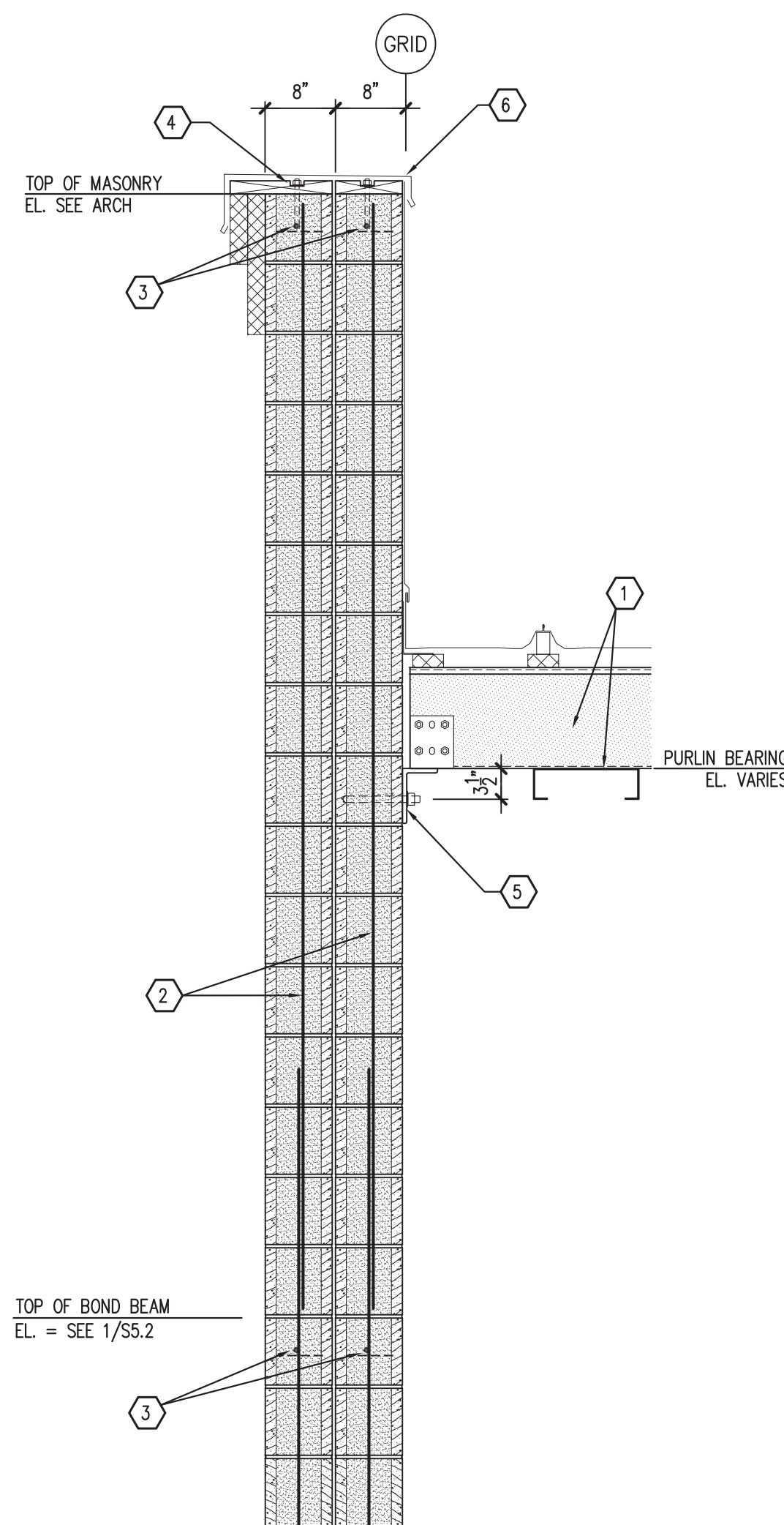


5 SECTION
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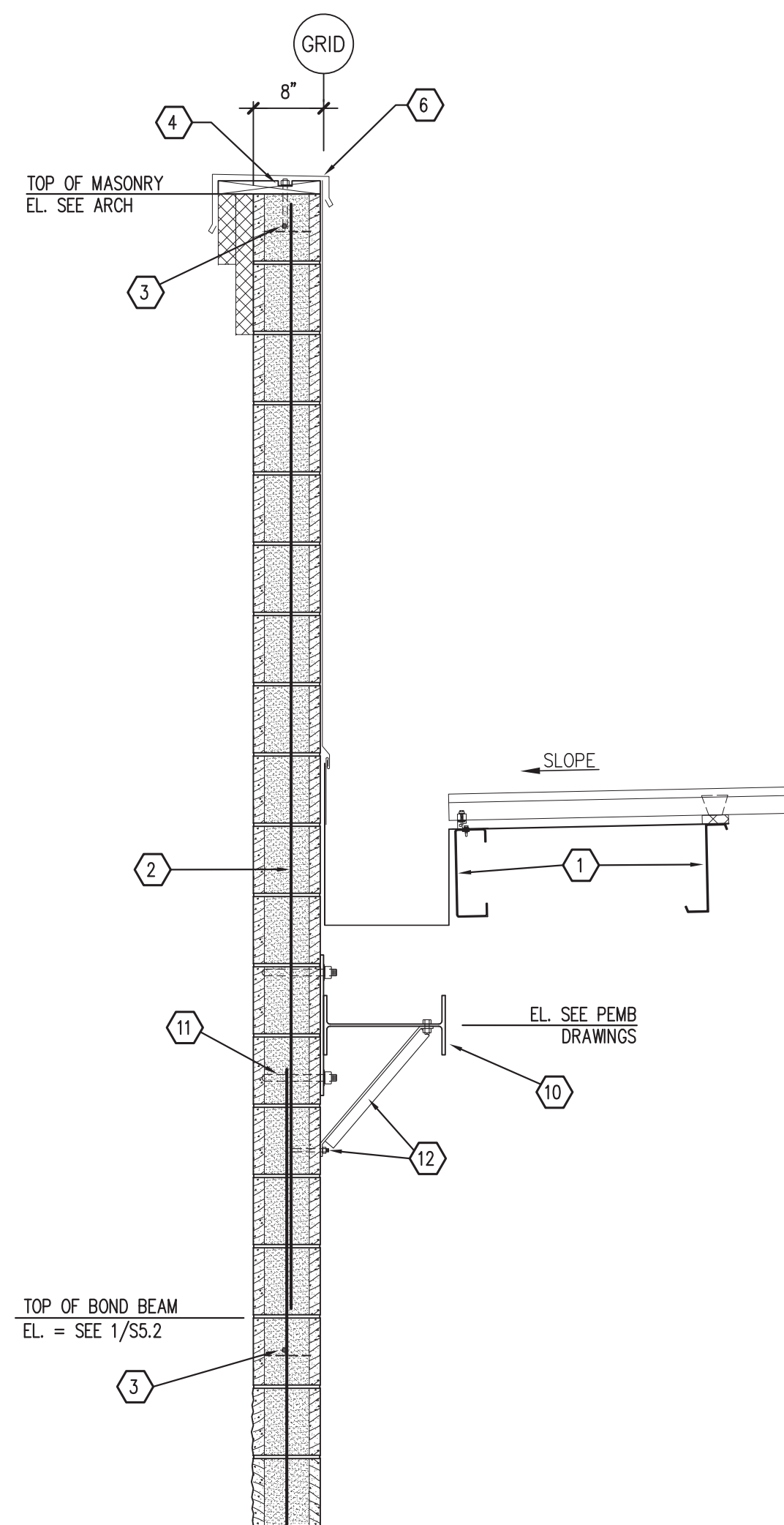
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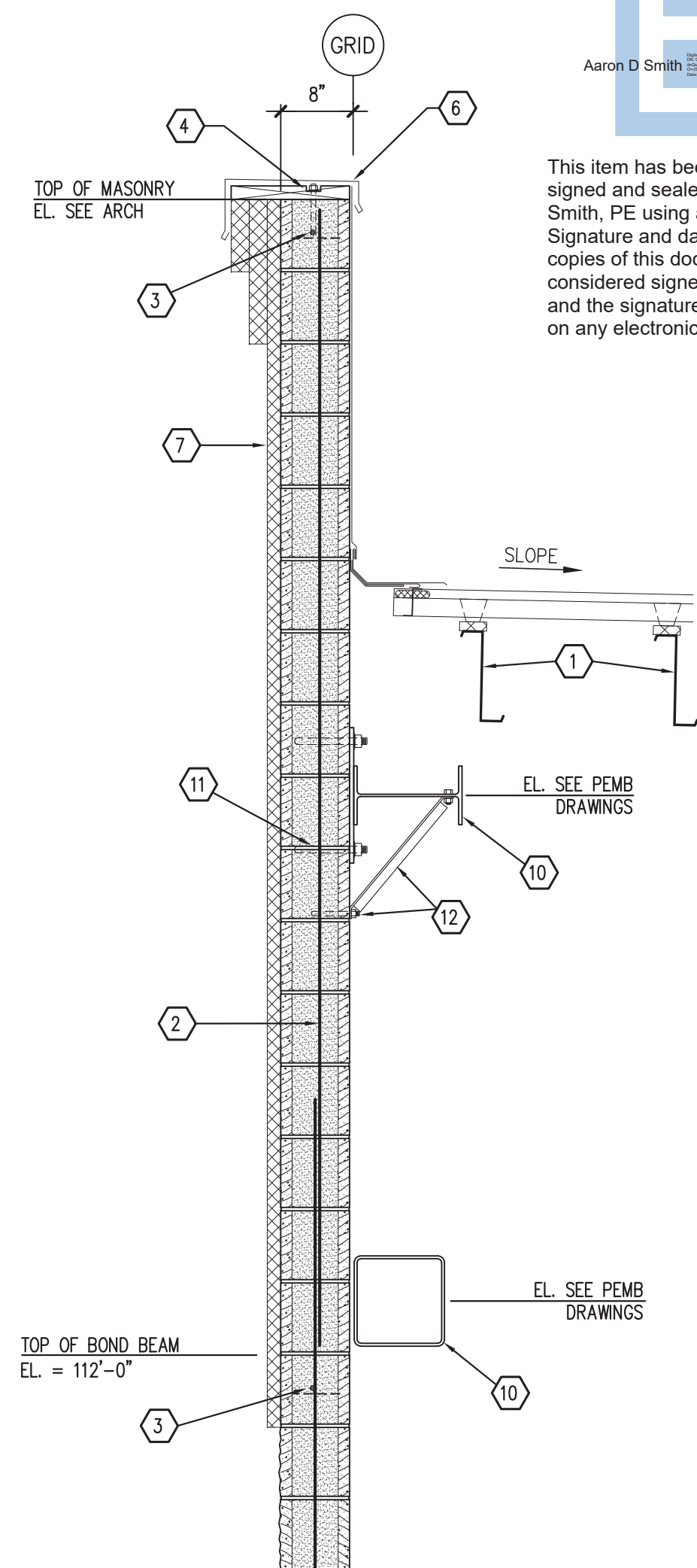
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S5.2 SCALE: 3/4" = 1'-0"



2 SECTION
S5.2 SCALE: 3/4" = 1'-0"



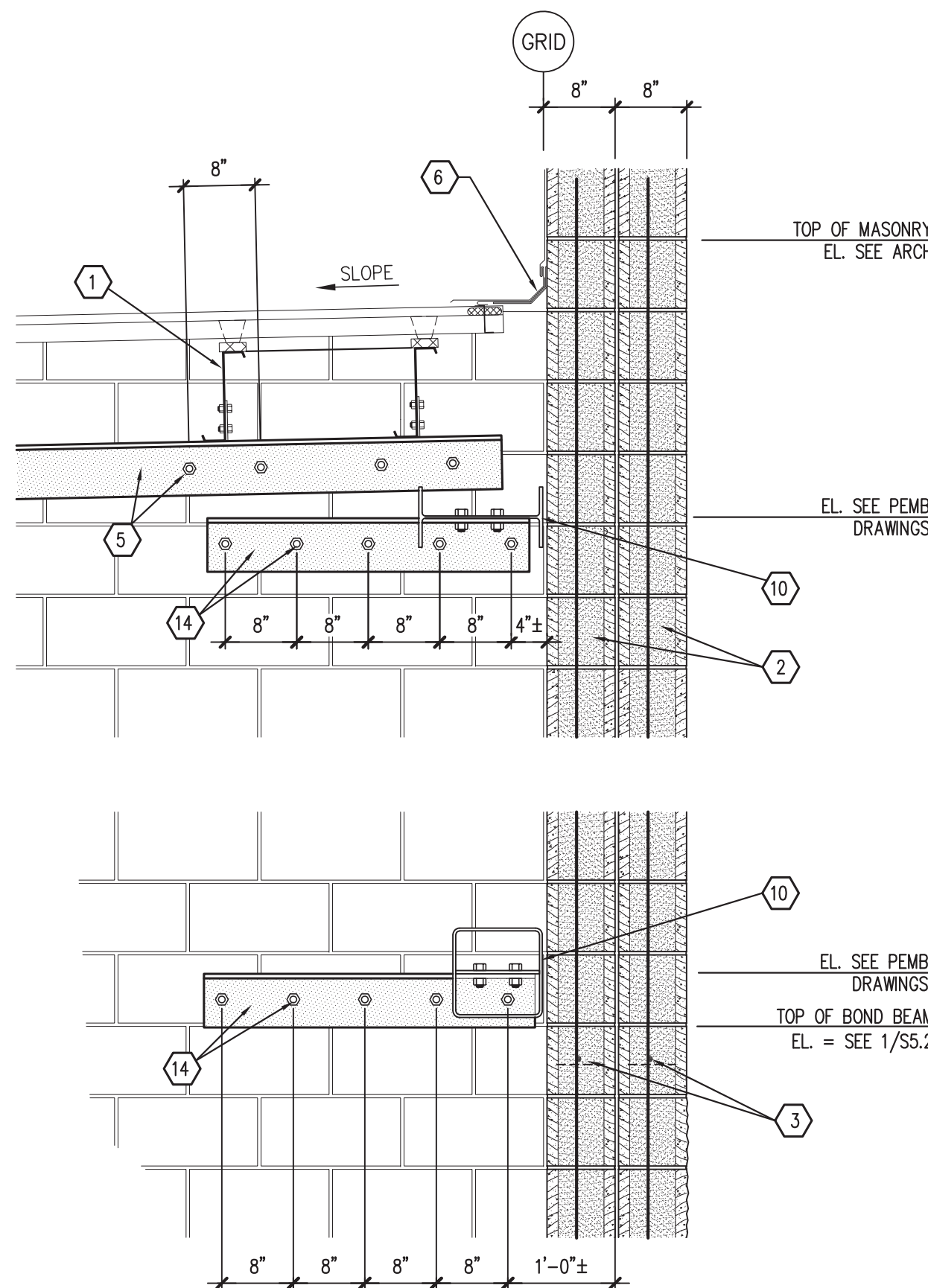
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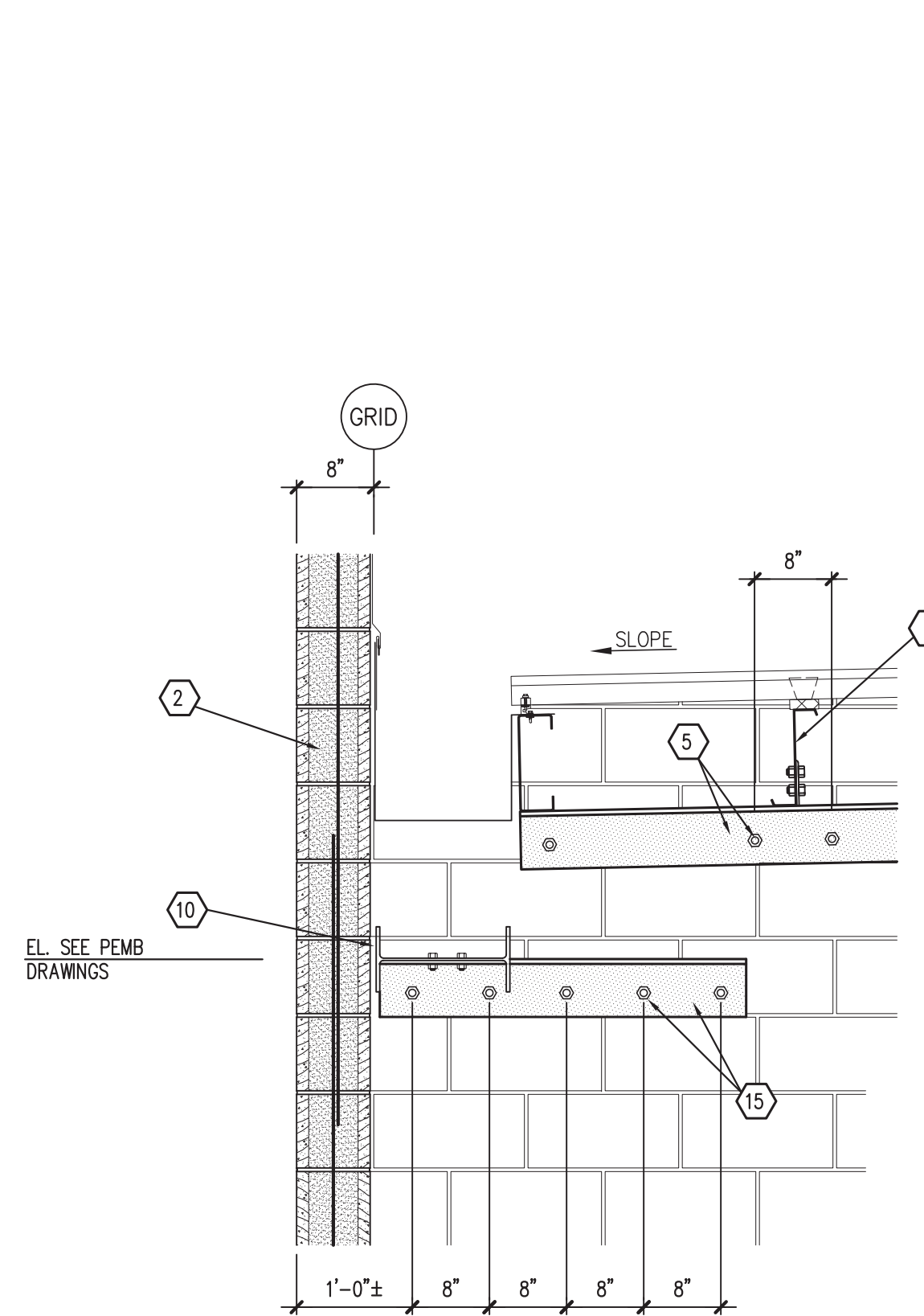
4 SECTION
S5.2 SCALE: 3/4" = 1'-0"

Aaron D. Smith
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STATE OF MISSOURI
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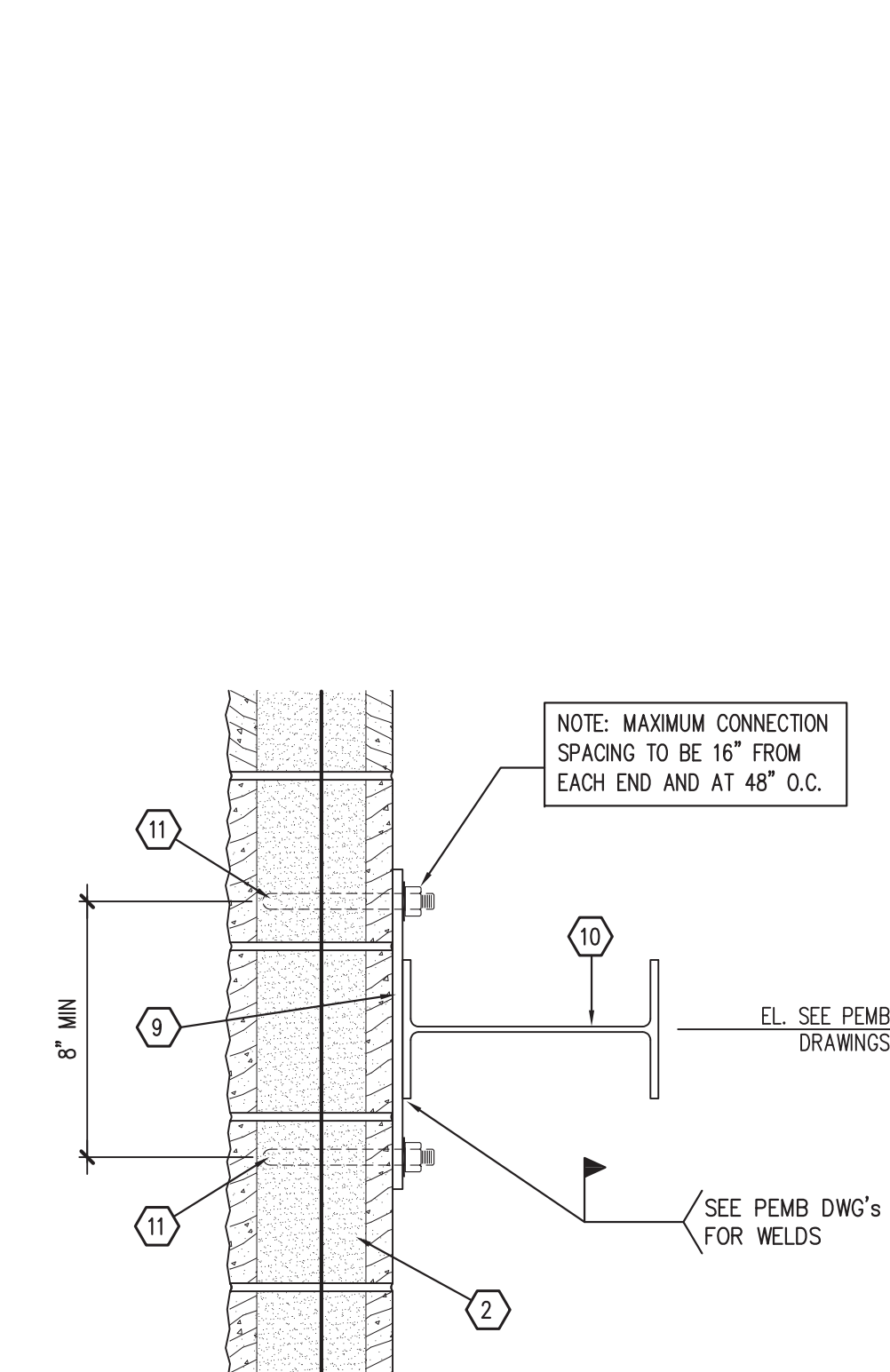
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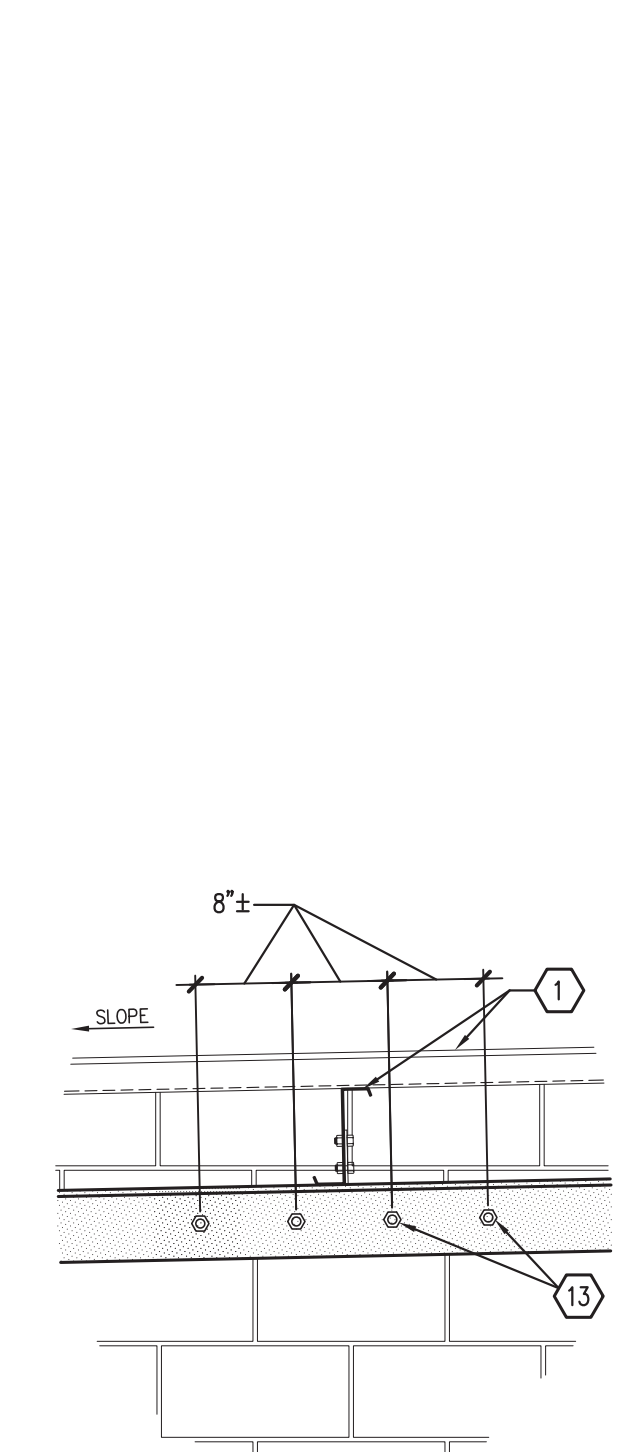
5 SECTION
S5.2 SCALE: 3/4" = 1'-0"



6 SECTION
S5.2 SCALE: 3/4" = 1'-0"



7 DETAIL
S5.2 SCALE: 1 1/2" = 1'-0"



8 DETAIL AT RTU's
S5.2 SCALE: 1 1/2" = 1'-0"

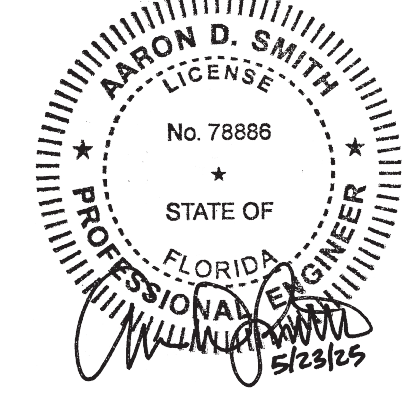
KEYNOTES

- 1 PRE-ENGINEERED METAL BUILDING (PEMB) STRUCTURAL SYSTEM. REFER TO SHOP DRAWINGS FOR ADDITIONAL INFORMATION.
- 2 8" (NOM) CMU. REFER TO STRUCTURAL NOTES, SHEET S1.1, FOR CMU WALL CONSTRUCTION REQUIREMENTS.
- 3 CONTINUOUS 8" (NOM) BOND BEAM REINFORCED W/ (1) #5
- 4 CONTINUOUS FIRE RETARDANT TREATED 2x NAILER WITH 1/2" GALVANIZED ANCHOR BOLTS @ 32" O.C.
- 5 L 6"x4"x1/2" LEDGER ANGLE BY PEMB. ANCHOR TO CMU W/ (2) 3/4"x0'-8 1/2" (6 1/2" EMBED) THREADED ROD ADHESIVE ANCHORS CENTERED UPON EACH PURLIN AND WITHIN 6" OF ANGLE END. REFER TO ADHESIVE ANCHOR NOTE. SEE 5/SS.2 & 6/SS.2 FOR TYPICAL PURLIN ANCHORAGE AND 8/SS.2 FOR ANCHORAGE OF PURLINS SUPPORTING RTU'S.
- 6 REFER TO ARCHITECTURAL FOR ALL ROOF FLASHING REQUIREMENTS
- 7 WALL FINISH SYSTEM, REFER TO ARCHITECTURAL
- 8 REFER TO ARCHITECTURAL DRAWINGS FOR EXTERIOR WALL SHEATHING. FASTEN SHEATHING @ 8" O.C. ALONG FRAMING MEMBERS W/ #6x1 1/4" LONG CORROSION RESISTANT SCREWS.
- 9 PEMB WALL CONNECTION PLATE. FIELD WELD GC PROVIDED SHIMS AS REQUIRED
- 10 PEMB WIND BEAM. SEE PEMB DRAWINGS FOR SIZE, SHAPE AND LOCATION.
- 11 ANCHOR WIND BEAM TO FULLY GROUTED CMU WITH (2) 3/4"x0'-8 1/2" (6 1/2" EMBED) THREADED ROD ADHESIVE ANCHORS AS PER 7/SS.2 AND ADHESIVE ANCHOR NOTE.
- 12 FLANGE BRACES AS PER PEMB SHOP DRAWINGS. ANCHOR TO CMU WITH 1/2"x0'-5 1/2" (4 1/2" EMBED) THREADED ROD ADHESIVE ANCHOR. SEE ADHESIVE ANCHOR NOTE.
- 13 PURLINS SUPPORTING ROOF TOP UNITS REQUIRE (4) 3/4"x0'-8 1/2" (6 1/2" EMBED) THREADED ROD ADHESIVE ANCHORS AS PER 8/SS.2. SEE ADHESIVE ANCHOR NOTE.
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- 16 SIMPSON FCB43.5 FRAMING CLIP W/ (6) #12-14 SCREWS TO STUDS AND (4) #12-24 TO PEMB WIND BEAM
- 17 4"x16ga METAL STUDS (400S162-54) @ 16" O.C.
- 18 8"x16ga METAL STUDS (800S162-54) @ 16" O.C.
- 19 3/8"x18 ga. METAL STUD TIES (362S162-43) @ 48" O.C. WITH MIN (2) #8 SCREWS TO EACH STUD
- 20 FASTEN STUDS TOGETHER WITH #10 SCREWS @ 24" O.C.
- 21 CONTRACTOR PROVIDED STEEL TUBE, SEE FRAMING PLAN SHEET S4.1
- 22 REFER TO SECTION 2/SS.3 FOR TYPICAL WALL CONSTRUCTION NOTATIONS
- 23 PRE-FABRICATED METAL CANOPY WITH HANGER RODS. REFER TO ARCHITECTURAL DRAWINGS FOR QUANTITY AND SPACING OF HANGER RODS.
- 24 FIRE RETARDANT TREATED 2x LEDGERS (SEE ARCHITECTURAL) FASTENED TO EACH METAL STUD W/ SIMPSON (2) TBG1260S SCREWS. FASTEN OUTER 2x WITH 10d NAILS @ 12" O.C. STAGGERED
- 25 WINDOW SYSTEM, REFER TO ARCHITECTURAL DRAWINGS
- 26 CONTRACTOR PROVIDED STEEL HS 8"x8" TUBE, SEE FRAMING PLAN SHEET S4.1
- 27 REFER TO SECTION 6/SS.3 FOR CONTRACTOR PROVIDED STEEL CONNECTION
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- 29 DECORATIVE COVER PLATE BY CANOPY SUPPLIER

ADHESIVE ANCHOR NOTES:

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5. ANCHOR INSTALLATION REQUIRES PERIODIC SPECIAL INSPECTIONS

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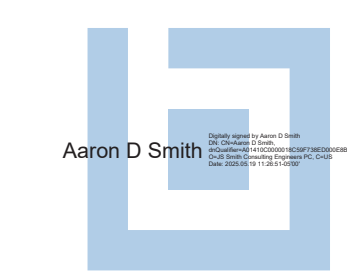


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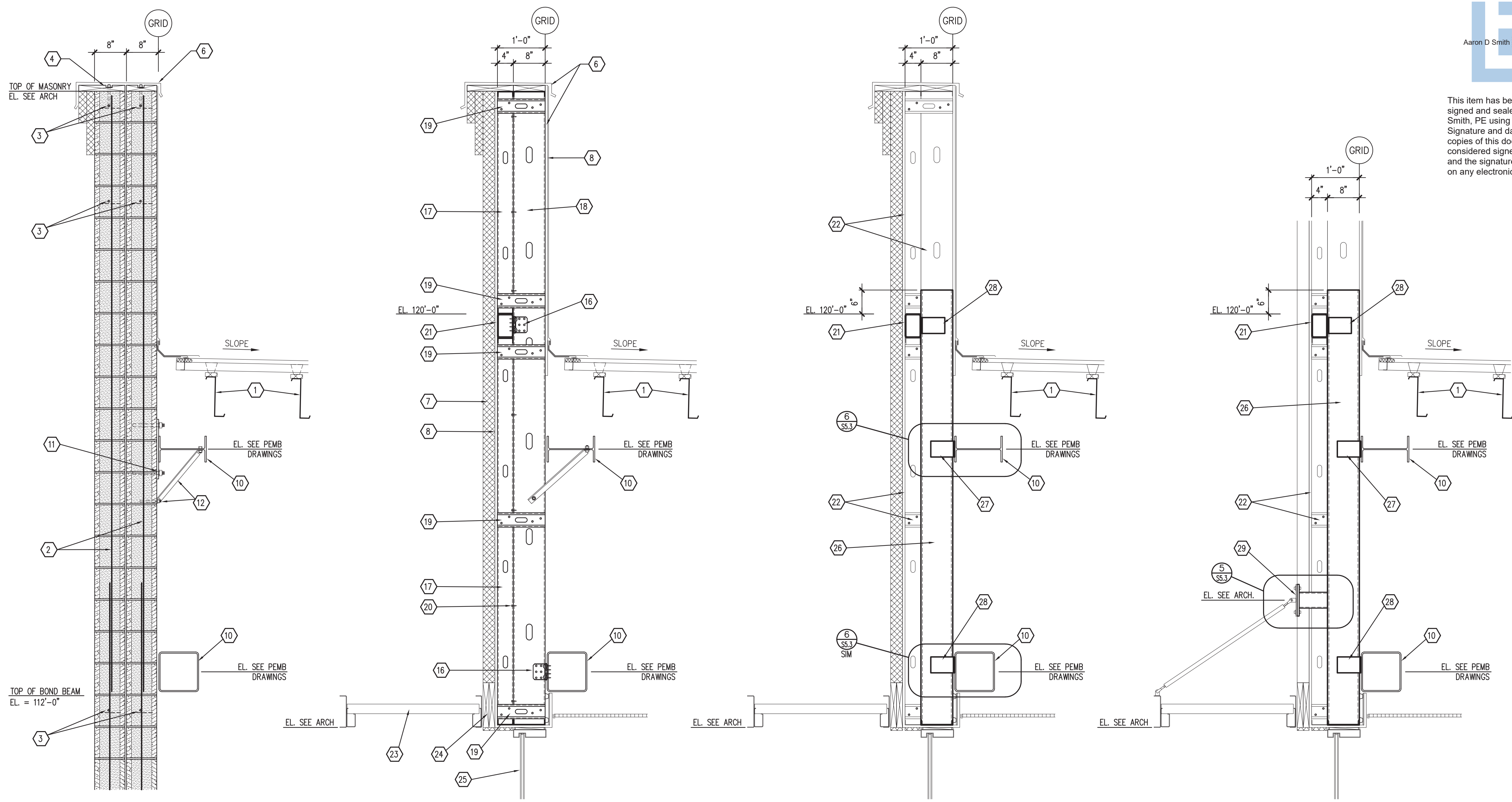
PROJECT:
NEW O'REILLY AUTO PARTS STORE
HWY FL 44
EUSTIS, FL #2

O'Reilly AUTO PARTS
CORPORATE OFFICES
243 SOUTH PATTERSON
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REVISION DATE:	



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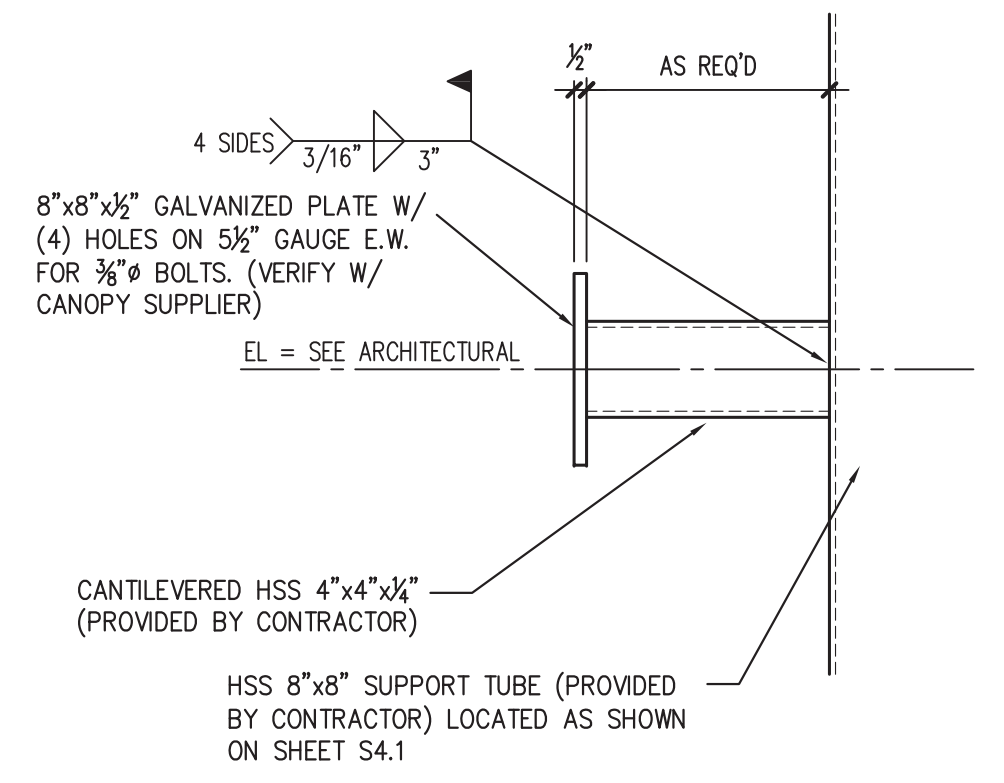
4 SECTION
S5.3 SCALE: 3/4" = 1'-0"

KEYNOTES

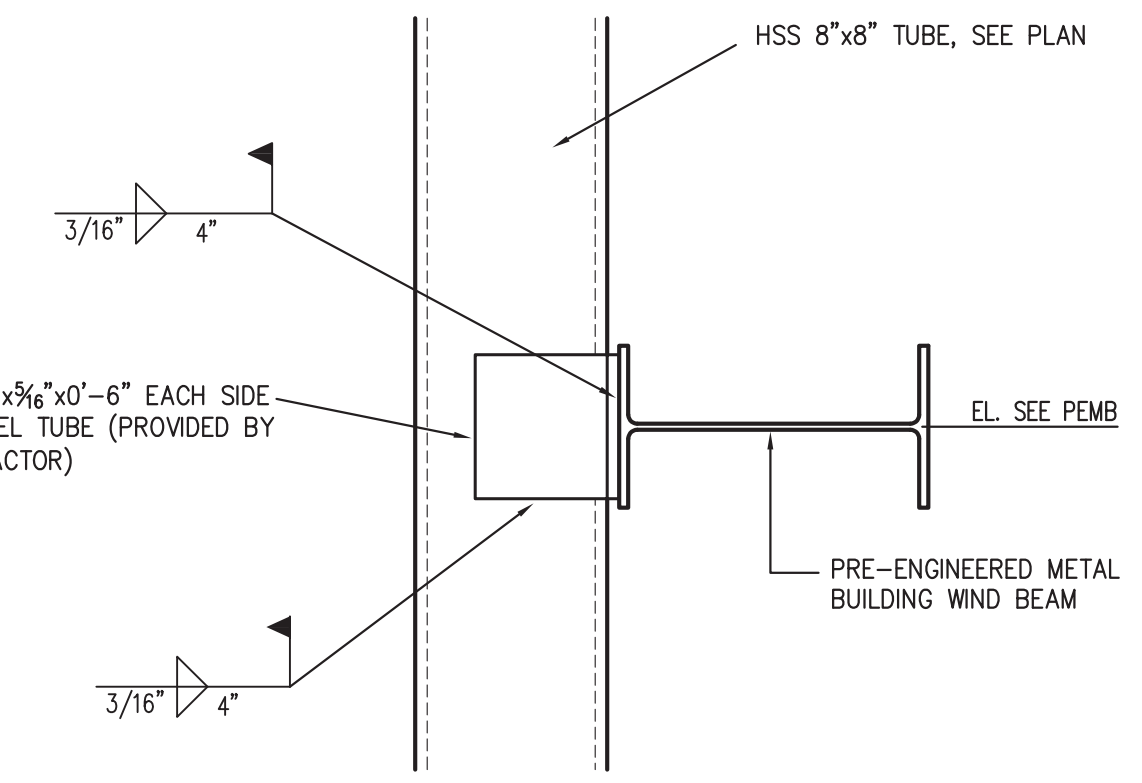
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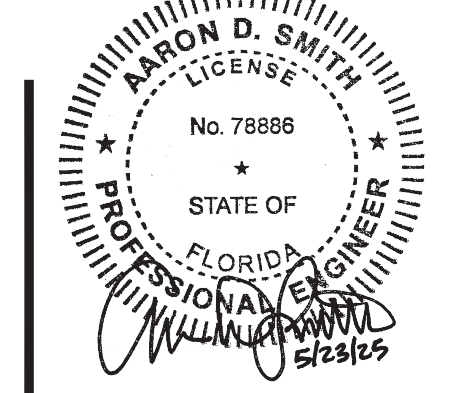


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