

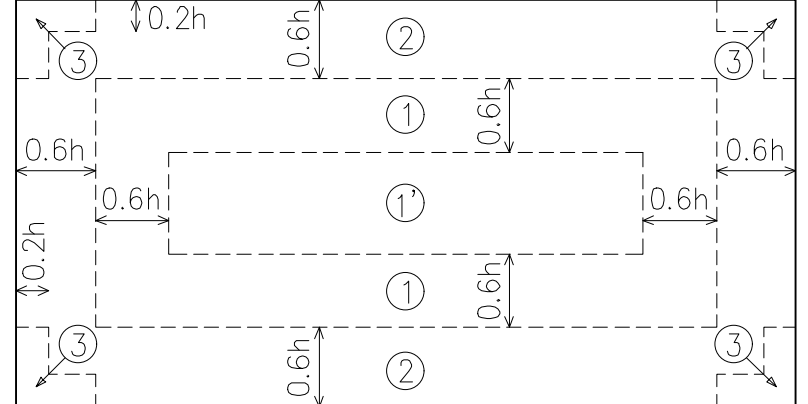
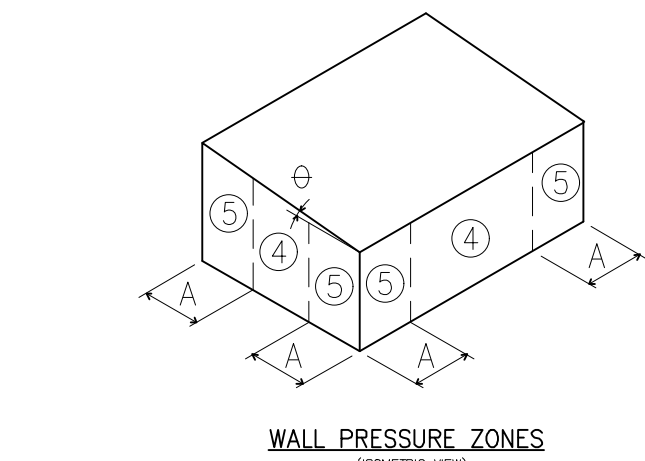
BUILDING DATA			
ULTIMATE WIND SPEED (mph): (FUTURE 1609)	140	INTERNAL PRESSURE COEFFICIENT: (Exposed Building per ASCE 7-22)	+/- 0.18
WIND LOAD FACTOR:	1.0	WIND EXPOSURE CATEGORY:	B
RISK CATEGORY: (TABLE 1604.5)	II	WIDTH OF WALL EDGE STRIP (A):	13.63
ROOF ANGLE (α): (Degrees)	< 7.0	DEAD LOAD RESISTING UPLIFT (1.00 in psf):	3.0
MEAN ROOF HEIGHT (h):	34.33	EDGE DISTANCE (0.6h) 0.6 X MEAN ROOF HEIGHT (Feet)	20.6
		EDGE DISTANCE (0.2h) 0.2 X MEAN ROOF HEIGHT (Feet)	6.87

DESIGN WIND LOADS – COMPONENTS & CLADDING (ULTIMATE)

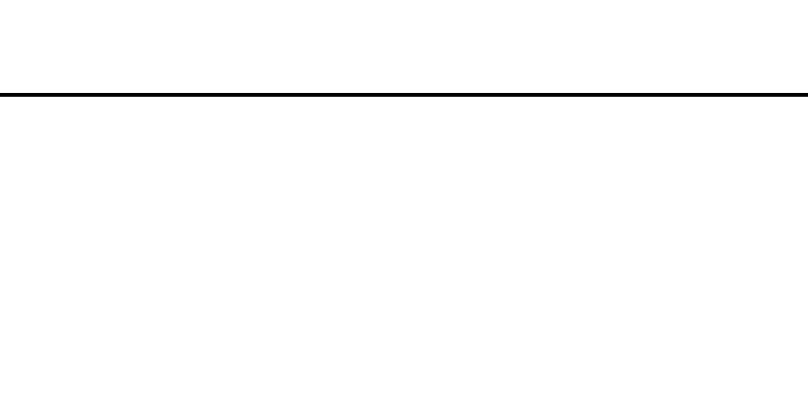
ZONE	AREA (sf)	ROOF			ZONE	AREA (sf)	WALL		
		Positive	Negative	Net Uplift			Positive	Negative	Net Uplift
1	10.0	16.00	-57.22	-54.22	4	10.0	32.87	-35.61	-32.33
1	50.0	16.00	-48.46	-45.46	4	50.0	29.49	-32.23	-29.23
1	100.0	16.00	-44.68	-41.68	4	100.0	28.04	-30.77	-27.77
1'	10.0	16.00	-32.87	-29.87	5	10.0	32.87	-43.83	-40.83
1'	50.0	16.00	-32.87	-29.87	5	50.0	29.49	-37.07	-34.07
1'	100.0	16.00	-32.87	-29.87	5	100.0	28.04	-34.16	-31.16
2	10.0	16.00	-75.48	-72.48					
2	50.0	16.00	-64.21	-61.21					
2	100.0	16.00	-59.36	-56.36					
3	10.0	16.00	-102.88	-99.88					
3	50.0	16.00	-80.34	-77.34					
3	100.0	16.00	-70.63	-67.63					

N.O.T.E.
1. FOR EFFECTIVE AREA BETWEEN THOSE GIVEN ABOVE THE LOAD MAY BE INTERPOLATED, OTHERWISE USE THE LOAD ASSOCIATED WITH THE LOWER EFFECTIVE AREA.
2. GUST AND WIND ZONE SOFTEN PRESSURES ACTING TOWARD AND AWAY FROM THE BUILDING SURFACES, RESPECTIVELY.
3. SEE PRESSURE ZONE DIAGRAMS FOR CORRESPONDING ZONES.
4. EXTERIOR GLAZING OPENINGS IN BUILDINGS SHALL COMPLY WITH FLORIDA BUILDING CODE 8TH EDITION (2023) BY EITHER BEING DESIGNED FOR IMPACT RESISTANCE OR BEING PROTECTED BY IMPACT PROTECTIVE SYSTEMS WHEN ULTIMATE WIND SPEED IS 140 MPH OR MORE.

1 DESIGN WIND CRITERIA (ULTIMATE)
NOT TO SCALE



WALL PRESSURE ZONES
ASCE 7-22



ROOF PRESSURE ZONES
ASCE 7-22

1. THE STRUCTURAL SYSTEM IS UNSTABLE UNTIL ALL CONNECTIONS HAVE BEEN MADE AND ALL CONNECTIONS HAVE REACHED THEIR MINIMUM DESIGN STRENGTH, AS SHOWN IN THE STRUCTURAL DOCUMENTS.

2. THE STRUCTURAL DOCUMENTS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OR MEANS OF CONSTRUCTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES AND SEQUENCES.

3. CONTRACTOR TO SUPPORT, BRACE AND SECURE EXISTING STRUCTURE AS REQUIRED. CONTRACTOR IS SOLELY RESPONSIBLE FOR THE SAFETY OF THE BUILDING DURING CONSTRUCTION.

4. APPLICABLE BUILDING CODE: FLORIDA BUILDING CODE 8TH EDITION (2023).

5. ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 8TH EDITION (2023).

6. DESIGN GRAVITY LOADS:

AREA	DEAD LOAD
ROOF	10 PSF (ASSUMED; REFER TO METAL BUILDING DRAWINGS)
FLOOR	20 PSF (ASSUMED; REFER TO METAL BUILDING DRAWINGS)
FLOOR	125 PSF (LIGHT STORAGE)

7. ULTIMATE DESIGN WIND SPEED = 140 MPH (FBC, FIGURE 1609.3.1) ASD NOMINAL DESIGN WIND SPEED = 108 MPH (FBC, TABLE 1609.3.1) RISK CATEGORY I (FBC, TABLE 1604.5) REFER TO 1.6.1.1 FOR WIND PRESSURES AND MORE INFO.

8. RAIN INTENSITY: 7.02 IN/H (15-MIN), 3.78 IN/H (60-MIN)

9. COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS.

10. CONTACT ENGINEER WITH ANY QUESTIONS OR DISCREPANCIES FOUND ON DRAWINGS.

11. CONTRACTOR SHALL NOTIFY THIS OFFICE WHEN THE STRUCTURAL SYSTEM IS SUBSTANTIALLY COMPLETED, AND BEFORE SHEATHING, CEILING, OR ROOFING IS INSTALLED.

12. IF THIS PROJECT IS CLASSIFIED AS A THRESHOLD BUILDING BY THE BUILDING DEPARTMENT, A SPECIAL INSPECTOR SHALL BE RETAINED IN ACCORDANCE WITH FLORIDA STATUTES.

13. THE SPECIAL INSPECTOR SHALL BE LICENSED BY THE FLORIDA BOARD OF PROFESSIONAL ENGINEERS.

14. ALL CONCRETE SHALL BE CURED IMMEDIATELY AFTER FINISHING OPERATIONS IN ACCORDANCE WITH ONE OF THE FOLLOWING METHODS:

- A) APPLY A 30% SOLIDS LIQUID MEMBRANE FORMING CHEMICAL CURING COMPOUND IN ACCORDANCE WITH ASTM C-309.
- B) PROVIDE CONTINUOUS MOISTURE TO CONCRETE IN ACCORDANCE WITH ACI 308.

15. GENERAL CONTRACTOR IS RESPONSIBLE FOR THE PROPER DESIGN AND CONSTRUCTION OF ALL FORMWORK, SHORING, AND RESHORING DESIGN SHALL BE PERFORMED BY A LICENSED FLORIDA ENGINEER.

16. A QUALIFIED TESTING LABORATORY SHALL BE RETAINED TO PERFORM THE FOLLOWING CONCRETE TESTS ON SITE:

- A) CYLINDER STRENGTH TESTS – ASTM C39, ONE SET OF FOUR CYLINDERS FOR EACH 50 CUBIC YARDS OR FRACTION THEREOF. TEST ONE CYLINDER AT 7 DAYS AND TWO AT 28 DAYS. HOLD THE FINAL CYLINDER IN RESERVE.
- B) SLUMP TESTS – ASTM C143

17. RESTRICT THE ADDITION OF MIX WATER AT THE JOB SITE. DO NOT ADD WATER WITHOUT THE APPROVAL OF THE GENERAL CONTRACTOR AND DO NOT EXCEED SLUMP LIMITATIONS OF TOTAL ALLOWABLE WATER TO CEMENT RATIO. USE COLD WATER FROM THE TRUCK TANK AND REMIX TO ACHIEVE CONSISTENCY. TEST REPORTS SHALL INDICATE QUANTITY OF WATER ADDED AT THE JOB SITE. ALL TESTS SHALL BE PREPARED AFTER THE ADDITION OF WATER TO THE MIX.

18. MAXIMUM WATER TO CEMENT RATIO WHEN NO BACK-UP DATA IS AVAILABLE:

- A) 4000 PSI, 28-DAY COMPRESSIVE STRENGTH; W/C RATIO, 0.44 MAXIMUM (NON-AIR-ENTRAINED); 0.36 MAXIMUM (AIR-ENTRAINED).
- B) 3000 PSI, 28-DAY COMPRESSIVE STRENGTH; W/C RATIO, 0.48 MAXIMUM (NON-AIR-ENTRAINED); 0.47 MAXIMUM (AIR-ENTRAINED).

19. REINFORCING BAR COVER:

- A) FOOTINGS: 3"
- B) COLUMNS: 1-1/2"
- C) BEAMS: 1-1/2"
- D) SLABS: 3/4" (INTERIOR) 1-1/2" (EXTERIOR)

20. CONCRETE SHALL BE PLACED WITHIN 90 MINUTES OF BATCH TIME.

21. WHERE BAR LENGTHS ARE GIVEN ON DRAWINGS, LENGTH OF HOOK, IF REQUIRED, IS NOT INCLUDED.

22. PROVIDE COMMERCIAL FORM COATING COMPOUNDS THAT WILL NOT BOND, STAIN, OR ADVERSELY AFFECT CONCRETE SURFACES. SET FORMS BEFORE PLACING CONCRETE.

23. ALL CONCRETE SHALL BE CONSOLIDATED IN PLACE USING INTERNAL VIBRATORS.

24. REPAIR AND PATCH DEFECTIVE AREAS WITH CEMENT MORTAR IMMEDIATELY AFTER REMOVAL OF FORMS. EXCEPT WHERE REINFORCING IS VISIBLE, CONTACT STRUCTURAL ENGINEER FOR EVALUATION.

25. PROVIDE 3/4" DIAMETERS ON ALL EXPOSED CORNERS OF COLUMNS, BEAMS, AND WALLS UNLESS NOTED OTHERWISE ON ARCHITECTURAL DRAWINGS.

26. PROVIDE CORNER BARS AT ALL BEAM AND WALL FOOTING CORNERS TO MATCH HORIZONTAL BARS.

27. SUBMITTALS:

- A) SUBMIT PROPOSED CONCRETE MIX DESIGN PRIOR TO CONSTRUCTION, INCLUDING BACKUP DATA IN ACCORDANCE WITH ACI 308-20 CHAPTER 4, SECTION 4.2.3, INCLUDING SECTION 4.2.3.4B, INDICATING WHERE CONCRETE IS TO BE PLACED WITHIN THE STRUCTURE.
- B) SUBMIT DETAILED SHOP DRAWINGS OF REINFORCING BARS SHOWING NUMBER, SIZE, AND LOCATION. INCLUDE BAR LISTS AND BEND DIAGRAM.
- C) SUBMIT FORMWORK AND SHORING DRAWINGS TO LOCAL BUILDING DEPARTMENT WHEN REQUIRED BY FLORIDA THRESHOLD LAW.

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Electrical Review - Jeff Morrison PX3515 - Reviewed - 3/28/2026

Plumbing Review - Alfonso Zacour Otero AR91999 - Reviewed - 3/29/2026

Fire Review - Mike Hudkins - NA to Private Provider - 3/30/2026

CONCRETE SHALL BE PROVIDED IN THE BOTTOM COURSE OF MASONRY IN EACH GROUT FOUR WHEN THE POUR HEIGHT EXCEEDS 30'. CLEANOUTS TO

CONCRETE SHALL BE READ/MIX PER ASTM C394:

- A) PORTLAND CEMENT – ASTM C 150
- B) AGGREGATES – ASTM C33 (3/4" MAX.)
- C) NO CALCIUM CHLORIDE
- D) AIR ENTRAINING – ASTM C260
- E) WATER REDUCING – ASTM C494
- F) FLYASH – ASTM C618 CLASS F (20% MAXIMUM BY WEIGHT)
- G) WATER – CLEAN AND POTABLE

REINFORCING STEEL: ASTM A615 GRADE 60.

REQUIRED SLUMP RANGE = 3" TO 5".

WELDED WIRE REINFORCEMENT: ASTM A-185, FURNISH IN SHEETS, NOT ROLLS.

MOISTURE/VAPOR RETARDER: 10 MIL POLYETHYLENE, LAP 6" AND TAPE ALL JOINTS.

CODES AND STANDARDS: (CURRENT EDITION)

- ACI 301 "SPEC FOR STRUCTURAL CONCRETE FOR BUILDINGS"
- ACI 308 "RECOMMENDED PRACTICE FOR HOT WEATHER CONCRETING"
- ACI 318 "BLOG CODE REQUIREMENTS FOR REINF. CONCRETE"
- ACI 315 "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT"

MINIMUM LAP SPlice = 30 BAR DIAMETERS UNLESS NOTED OTHERWISE.

PROVIDE PROPERLY TIED SPACERS, CHAIRS, BOLSTERS, ETC. AS REQUIRED AND NECESSARY TO ASSEMBLE, PLACE AND SUPPORT ALL REINFORCING IN PLACE. USE WIRE BAR TYPE SUPPORTS COMPLYING WITH CRS RECOMMENDATIONS. USE PLASTIC TIE LEGS ON ALL EXPOSED SURFACES.

CONTRACTOR SHALL VERIFY LOCATIONS OF ALL OPENINGS, SLEEVES, AND SLAB RECESSES AS REQUIRED BY OTHER TRADES BEFORE CONCRETE IS PLACED. NO SLEEVE, OPENING, OR INSERT MAY BE PLACED IN BEAMS, JOISTS, OR COLUMNS UNLESS APPROVED BY THE ENGINEER.

CONTRACTOR SHALL VERIFY EMBEDDED ITEMS, INCLUDING BUT NOT LIMITED TO ANCHOR BOLTS, BOLT CLUSTERS, WELD PLATES, ETC., BEFORE PLACING CONCRETE. NOTIFY ENGINEER OF ANY CONFLICTS WITH REBAR.

SEE ARCHITECTURAL DRAWINGS FOR REQUIRED CONCRETE FINISHES.

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ONE COPY OF ALL TEST REPORTS SHALL BE SENT DIRECTLY TO OWNER, ARCHITECT, STRUCTURAL ENGINEER, AND GENERAL CONTRACTOR.

RESTRICT THE ADDITION OF MIX WATER AT THE JOB SITE. DO NOT ADD WATER WITHOUT THE APPROVAL OF THE GENERAL CONTRACTOR AND DO NOT EXCEED SLUMP LIMITATIONS OF TOTAL ALLOWABLE WATER TO CEMENT RATIO. USE COLD WATER FROM THE TRUCK TANK AND REMIX TO ACHIEVE CONSISTENCY. TEST REPORTS SHALL INDICATE QUANTITY OF WATER ADDED AT THE JOB SITE. ALL TESTS SHALL BE PREPARED AFTER THE ADDITION OF WATER TO THE MIX.

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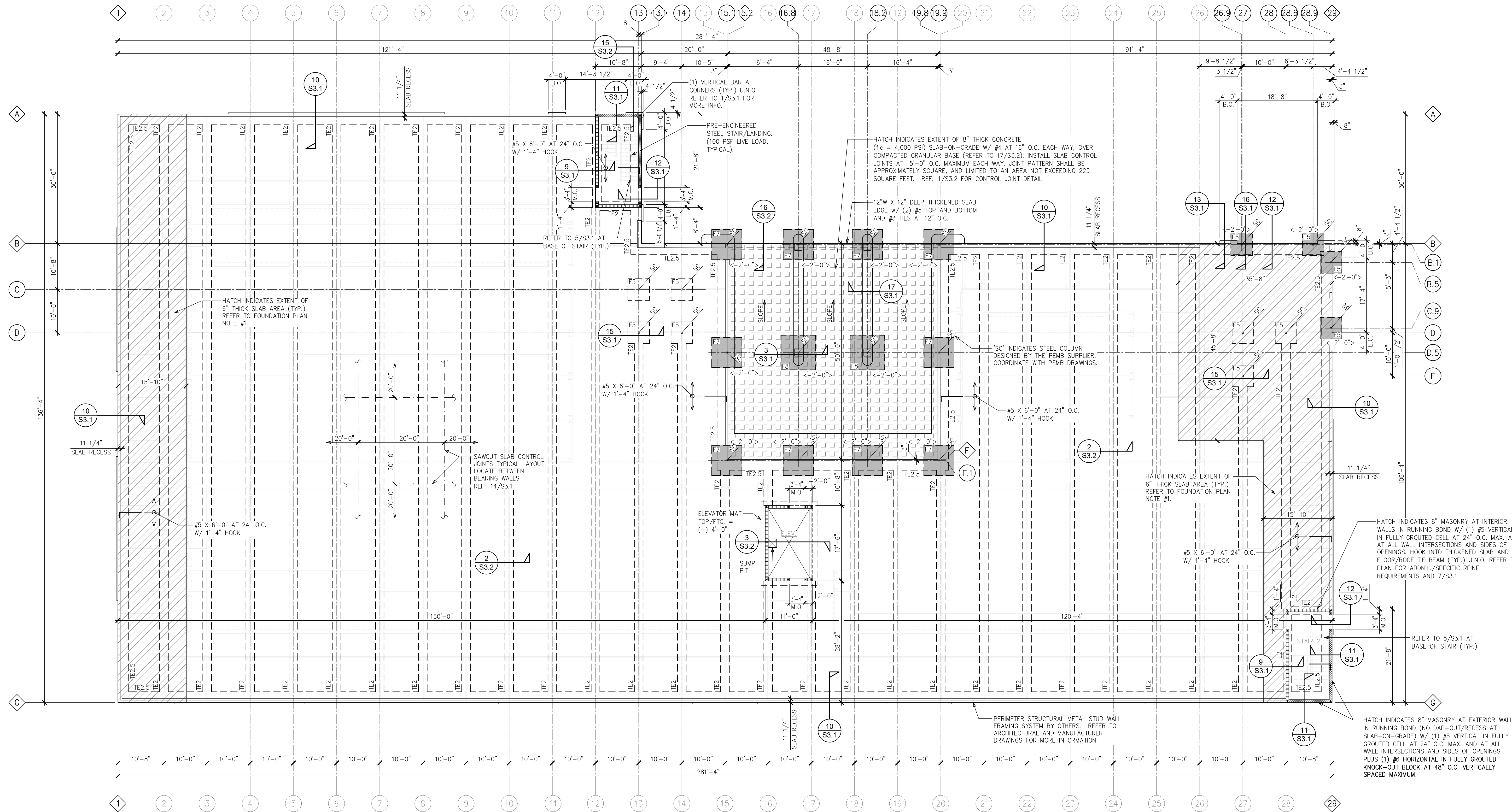
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FOUNDATION PLAN
SCALE: 3/32" = 1'-0"

CONCRETE MIX DESIGN SUBMITTALS:
CONCRETE MIX DESIGNS SHALL BE SUBMITTED FOR CONCRETE SLAB-ON-GRADE (BY THE STRUCTURAL ENGINEER) AND THE UPPER LEVEL, COMPOSITE CONCRETE FLOOR SLABS (BY THE PEMB STRUCTURAL ENGINEER).
THE METHODOLOGY FOR THE CURING OF ALL CONCRETE SHALL BE INCLUDED WITH CONCRETE MIX DESIGN SUBMITTALS. CONCRETE CURING SHALL BE COMPATIBLE WITH THE CONCRETE FINISHING PRODUCTS SPECIFIED IN THE ARCHITECTURAL DRAWINGS.
THE CONCRETE MIX DESIGNS AND CURING SUBMITTALS MUST BE REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER OF RECORD AND/OR THE PEMB STRUCTURAL ENGINEER PRIOR TO ORDER AND PLACEMENT OF CONCRETE.

INTERIOR MASONRY WALLS HAVE BEEN ASSUMED TO BE LATERALLY BRACED BY PRE-ENGINEERED DIAPHRAGM ROOF/FLOOR SYSTEM AND INTERIOR LIGHT GAGE WALL SYSTEM. METHOD OF ATTACHMENT OF THE INTERIOR METAL BUILDING POSTS/SHEAR WALLS AND ROOF PURLIN SYSTEM SHALL BE DESIGNED BY METAL BUILDING SPECIALTY ENGINEER TO TRANSFER THE BUILDING LATERAL WALL FORCES THROUGH THE ROOF PANEL/WALLS INTO A LATERAL RESISTING SYSTEM DESIGNED BY THE BUILDING ENGINEER.

CONCRETE SLAB-ON-GRADE:
THE DESIGN OF CONCRETE SLAB-ON-GRADE SHALL BE THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER OF RECORD FOR THE BUILDING. REFER TO THE FOUNDATION PLAN, DETAILS AND SPECIFICATIONS FOR CONCRETE THICKNESS, COMPRESSIVE STRENGTH AND REINFORCEMENT. SEE ALSO CONCRETE MIX DESIGN SUBMITTALS NOTE.

FOOTING SCHEDULE

MARK	SIZE	DEPTH	REINFORCING	COMMENTS
TE2	2'-0" WIDE X CONT.	1'-4"	(3) #6 CONT. TOP AND BOTTOM W/ #6 TRANSV. AT 12" O.C. BOTTOM	THICKENED SLAB AT INTERIOR
TE2.5	2'-6" WIDE X CONT.	2'-0"	(3) #5 CONT. BOTTOM W/ #5 TRANSV. AT 12" O.C. BOTTOM	THICKENED SLAB EDGE AT EXTERIOR
F5	5'-0" X 5'-0"	1'-0"	(5) #5 EACH WAY, BOTTOM	COLUMN FOOTING
F6	6'-0" X 6'-0"	1'-4"	(7) #5 EACH WAY, BOTTOM	COLUMN FOOTING
F7	7'-0" X 7'-0"	1'-6"	(8) #6 EACH WAY, BOTTOM	COLUMN FOOTING
F8	8'-0" X 8'-0"	1'-10"	(9) #6 EACH WAY, BOTTOM	COLUMN FOOTING

FOUNDATION PLAN NOTES:

- SLAB-ON-GRADE TO BE 6" THICK MINIMUM AT HATCHED AREAS (4,000 PSI) W/ #4 AT 14" O.C. EACH WAY (MID.) ON 10 MIL VAPOR RETARDER. ALL OTHER AREAS TO BE 4" THICK (4,000 PSI) W/ #6X6 - W2.9XW2.9 W.W.F. ON 10 MIL VAPOR RETARDER. REFER TO GEOTECHNICAL REPORT FOR SLAB SUBGRADE PREPARATION REQUIREMENTS. REFER TO ARCHITECT DRAWINGS FOR TERMITE TREATMENT REQUIREMENTS.
- SLAB SAWCUT 1/8" X 1" DEEP CONTROL JOINTS AT 20'-0" X 20'-0" CENTERED BETWEEN COLUMN GRIDS. COORDINATE CONSTRUCTION JOINT W/ 14/S3.1
- TOP/SLAB ELEVATION = 0'-0" U.N.O.
< > INDICATES TOP OF FOOTING ELEVATION.
- CENTER ALL FOOTINGS BELOW WALL OR COLUMN U.N.O.
- REFER TO ARCHITECTURAL/CIVIL FOR EXTERIOR SLAB AND SIDEWALK INFORMATION.
- REFER TO ARCHITECT DRAWINGS FOR ADDITIONAL DIMENSIONS NOT SHOWN.
- REFER TO ARCHITECT/CIVIL DRAWINGS FOR BUILDING'S LOCATION AND ORIENTATION ON THE SITE.
- REFER TO ARCHITECTURAL/PLUMBING DRAWINGS FOR FIXTURE/DRAIN LOCATIONS AND REQUIREMENTS.
- PROVIDE CORNER BARS AT ALL CORNERS AND INTERSECTIONS OF CONTINUOUS FOOTINGS. REFER TO DETAIL 2/S3.1 FOR MORE INFO.
- REFER TO 4/S3.1 FOR PIPE/CONDUIT PENETRATION DETAIL.
- REFER TO 1/S1.1 FOR DESIGN WIND CRITERIA.
- "B.O." - INDICATES SLAB BUMP-OUT/EXTENSION. COORDINATE W/ ARCHITECT DWG'S.
- CONTRACTOR TO SUBMIT COLUMN/WALL REACTIONS AND FINAL SHOP DRAWINGS/CALCULATIONS FROM PRE-ENGINEERED METAL BUILDING SYSTEMS DESIGNER FOR REVIEW PRIOR TO ERECTION OR ANY FOUNDATION WORK. FOUNDATIONS ARE SUBJECT TO CHANGE BASED ON REACTIONS PER FINAL DOCUMENTS FOR CONSTRUCTION.



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Ph. 727. 515. 3336
FABER@FWHARCHITECTS.COM

TO THE BEST OF THE KNOWLEDGE OF THE
ARCHITECTS AND ENGINEERS, SAID PLANS AND
SPECIFICATIONS COMPLY WITH THE APPLICABLE
MINIMUM BUILDING CODES AND THE
APPLICABLE MINIMUM SAFETY STANDARDS

ALAN C. GUENTHER, P.E. FL PER. 53308

revision descriptions

no. date

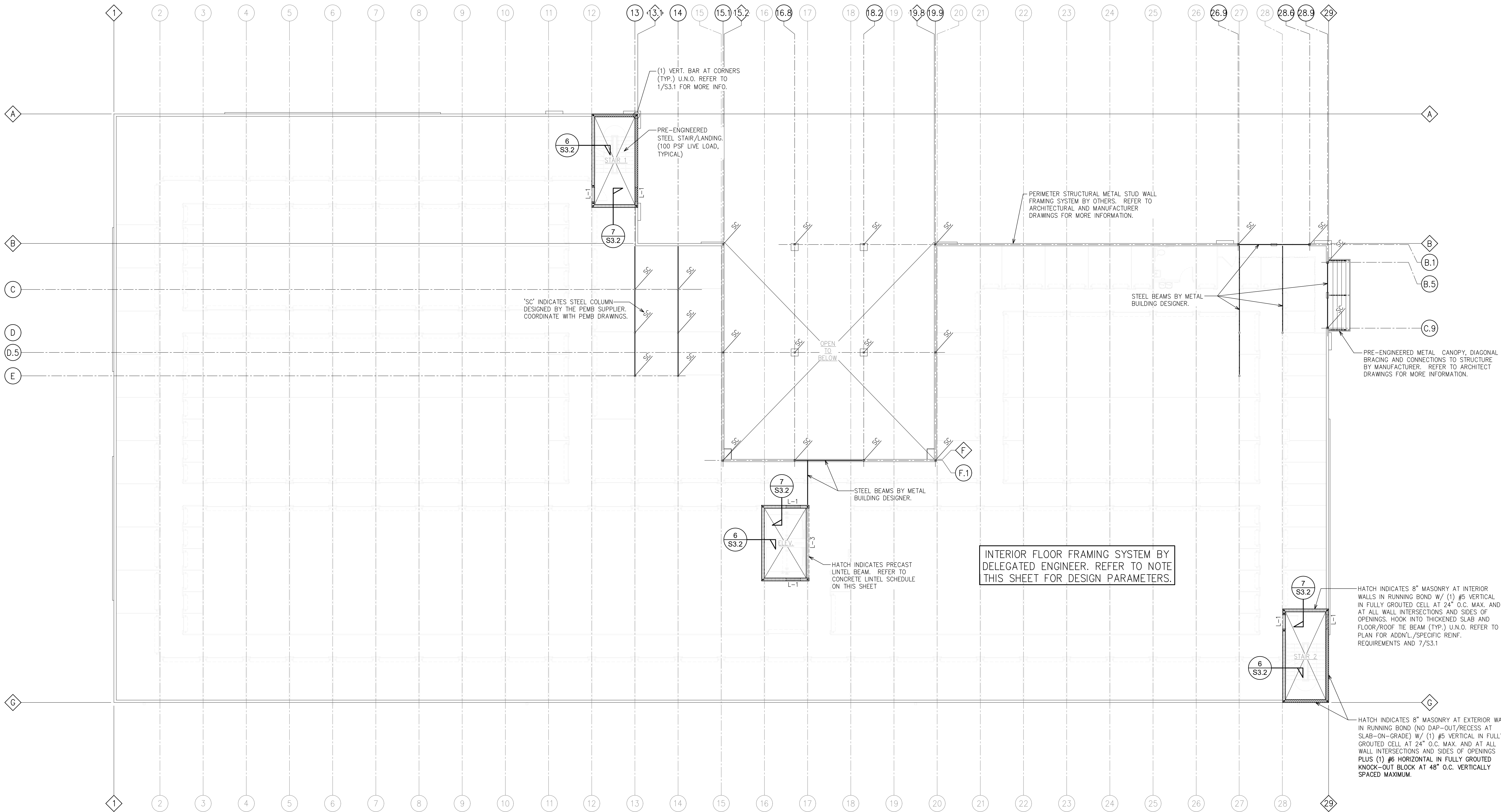
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SELF-STORAGE
DAVENPORT
OSCEOLA POLK LINE ROAD
DAVENPORT, FLORIDA 33896

100% PERMIT SET
02.23.26
date

25035
comm. no.

SECOND FLOOR
FRAMING PLAN

S2.2



CONCRETE LINTEL SCHEDULE			
MARK	SIZE	REINFORCING BARS	COMMENTS
L-1	8" X 8"	(1) #5 CONT.	TYPE A
L-2	8" X 16"	(2) #5 CONT.	TYPE B
L-3	8 X 24	(3) #5 CONT.	TYPE C

(TYPE A)

(TYPE B)

(TYPE C)

- REFER TO 8/S3.1 FOR ADDITIONAL LINTEL REQUIREMENTS.
- ALL LINTELS AND KNOCK-OUT BLOCKS TO BE GROUTED SOLID.
- ALL PRECAST LINTELS EXCEEDING OPENING SPAN OF 14'-0" TO BE PRESTRESSED U.N.O.

SECOND FLOOR FRAMING PLAN
SCALE: 3/32" = 1'-0"



- SECOND FLOOR FLOOR FRAMING PLAN NOTES:
- TOP OF 2ND FLOOR SLAB ELEVATION = 12'-6 1/2" U.N.O.
 - REFER TO SHEET S2.1 FOR MASONRY DIMENSIONS.
 - REFER TO 1/S1.1 FOR DESIGN WIND CRITERIA.

INTERIOR MASONRY WALLS HAVE BEEN ASSUMED TO BE Laterally BRACED BY PRE-ENGINEERED DIAPHRAGM FLOOR SYSTEM AND INTERIOR LIGHT GAGE WALL SYSTEM. METHOD OF ATTACHMENT OF THE INTERIOR METAL BUILDING POSTS/SHEAR WALLS AND FLOOR PURLIN SYSTEM SHALL BE DESIGNED BY METAL BUILDING SPECIALTY ENGINEER TO TRANSFER THE BUILDING LATERAL WALL FORCES THROUGH THE ROOF PANEL/WALLS INTO A LATERAL RESISTING SYSTEM DESIGNED BY THE BUILDING ENGINEER.

CONCRETE MIX DESIGN SUBMITTALS:
CONCRETE MIX DESIGNS SHALL BE SUBMITTED FOR CONCRETE SLAB-ON-GRADE (BY THE STRUCTURAL ENGINEER) AND THE UPPER LEVEL, COMPOSITE CONCRETE FLOOR SLABS (BY THE PEMB STRUCTURAL ENGINEER).
THE METHODOLOGY FOR THE CURING OF ALL CONCRETE SHALL BE INCLUDED WITH CONCRETE MIX DESIGN SUBMITTALS. CONCRETE CURING SHALL BE COMPATIBLE WITH THE CONCRETE FINISHING PRODUCTS SPECIFIED IN THE ARCHITECTURAL DRAWINGS.
THE CONCRETE MIX DESIGNS AND CURING SUBMITTALS MUST BE REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER OF RECORD AND/OR THE PEMB STRUCTURAL ENGINEER PRIOR TO ORDER AND PLACEMENT OF CONCRETE.

- UPPER LEVEL CONCRETE FLOOR SLABS:
THE DESIGN OF ALL UPPER LEVEL, COMPOSITE CONCRETE FLOOR SLABS SHALL BE THE RESPONSIBILITY OF THE PROFESSIONAL ENGINEER (P.E.) DELEGATED BY THE PRE-ENGINEERED METAL BUILDING (PEMB) MANUFACTURER AND DOCUMENTED WITHIN THE SEALED ENGINEERED SHOP DRAWINGS. THE FOLLOWING STANDARDS OUTLINE THE MINIMUM DESIGN AND PERFORMANCE REQUIREMENTS:
- DESIGN RESPONSIBILITY AND DOCUMENTATION:
THE PEMB DELEGATED ENGINEER IS RESPONSIBLE FOR THE COMPREHENSIVE DESIGN OF THE COMPOSITE FLOOR SYSTEM.
SUBMITTALS MUST EXPLICITLY DETAIL:
 - METAL DECKING: PROFILE DEPTH, MATERIAL GAUGE, AND YIELD STRESS.
 - CONCRETE PROPERTIES: TOTAL SLAB THICKNESS, SPECIFIED COMPRESSIVE STRENGTH, AND AGGREGATE TYPE.
 - REINFORCEMENT: DETAILED SPECIFICATIONS FOR TEMPERATURE AND SHRINKAGE REINFORCEMENT (E.G. WELDED WIRE REINFORCEMENT OR FIBER MESH).
 - CURING PROTOCOLS: NECESSARY METHODS AND DURATIONS TO ENSURE PROPER HYDRATION AND STRUCTURAL INTEGRITY.
 - BASE DESIGN SPECIFICATION FOR SPANS NOT EXCEEDING 10'-0" UNDER LIGHT STORAGE LOADING CONDITIONS, THE MINIMUM DESIGN SHALL CONSIST OF:
 - DECKING: 2" DEEP, 18-GAUGE COMPOSITE METAL DECK (50 KSI).
 - CONCRETE: 2" NORMAL WEIGHT CONCRETE (NWC) COVER OVER THE RIBS.
 - PERFORMANCE: 3,000 PSI COMPRESSIVE STRENGTH AT 28 DAYS.
 - TOTAL THICKNESS: 4" OVERALL SLAB DEPTH.
 - ALTERNATE DESIGN RECOMMENDATION (PERFORMANCE OPTIMIZED) TO MITIGATE DECK DEFLECTION DURING THE WET CONCRETE POUR AND MINIMIZE SUBSEQUENT CONCRETE SHRINKAGE CRACKING, THE PEMB SHALL PROVIDE AN ALTERNATE BID FOR THE FOLLOWING ASSEMBLY:
 - DECKING: 3" DEEP, 18-GAUGE COMPOSITE METAL DECK (50 KSI).
 - CONCRETE: 2" NORMAL WEIGHT CONCRETE (NWC) COVER OVER THE RIBS.
 - PERFORMANCE: 3,000 PSI COMPRESSIVE STRENGTH AT 28 DAYS.
 - TOTAL THICKNESS: 5" OVERALL SLAB DEPTH.



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TO THE BEST OF THE KNOWLEDGE OF THE
ARCHITECTS AND ENGINEERS, SAID PLANS AND
SPECIFICATIONS COMPLY WITH THE APPLICABLE
MINIMUM BUILDING CODES AND THE
APPLICABLE MINIMUM SAFETY STANDARDS

ALAN C. GUENTHER, P.E. FL PER. 53308

revision descriptions

no. date

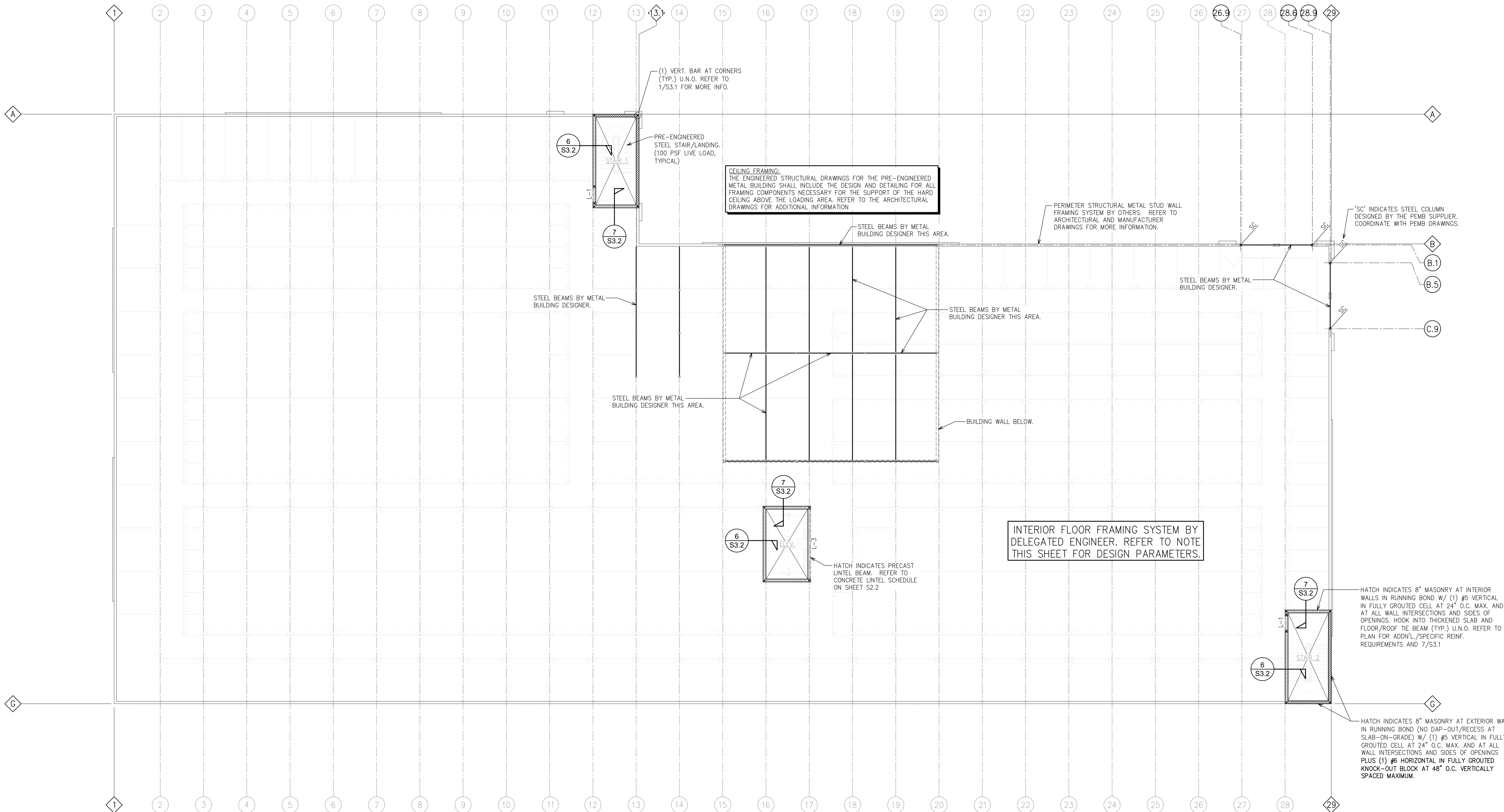
REUNION
SELF-STORAGE
DAVENPORT
OSCEOLA POLK LINE ROAD
DAVENPORT, FLORIDA 33896

100% PERMIT SET
02.23.26
date

25035
comm. no.

THIRD FLOOR
FRAMING PLAN

S2.3



THIRD FLOOR FRAMING PLAN
SCALE: 3/32" = 1'-0"

THIRD FLOOR FLOOR FRAMING PLAN NOTES:

1. TOP OF 3RD FLOOR SLAB ELEVATION = 23'-2 1/2" U.N.O.
2. REFER TO SHEET S2.1 FOR MASONRY DIMENSIONS.
3. REFER TO 1/S1.1 FOR DESIGN WIND CRITERIA.

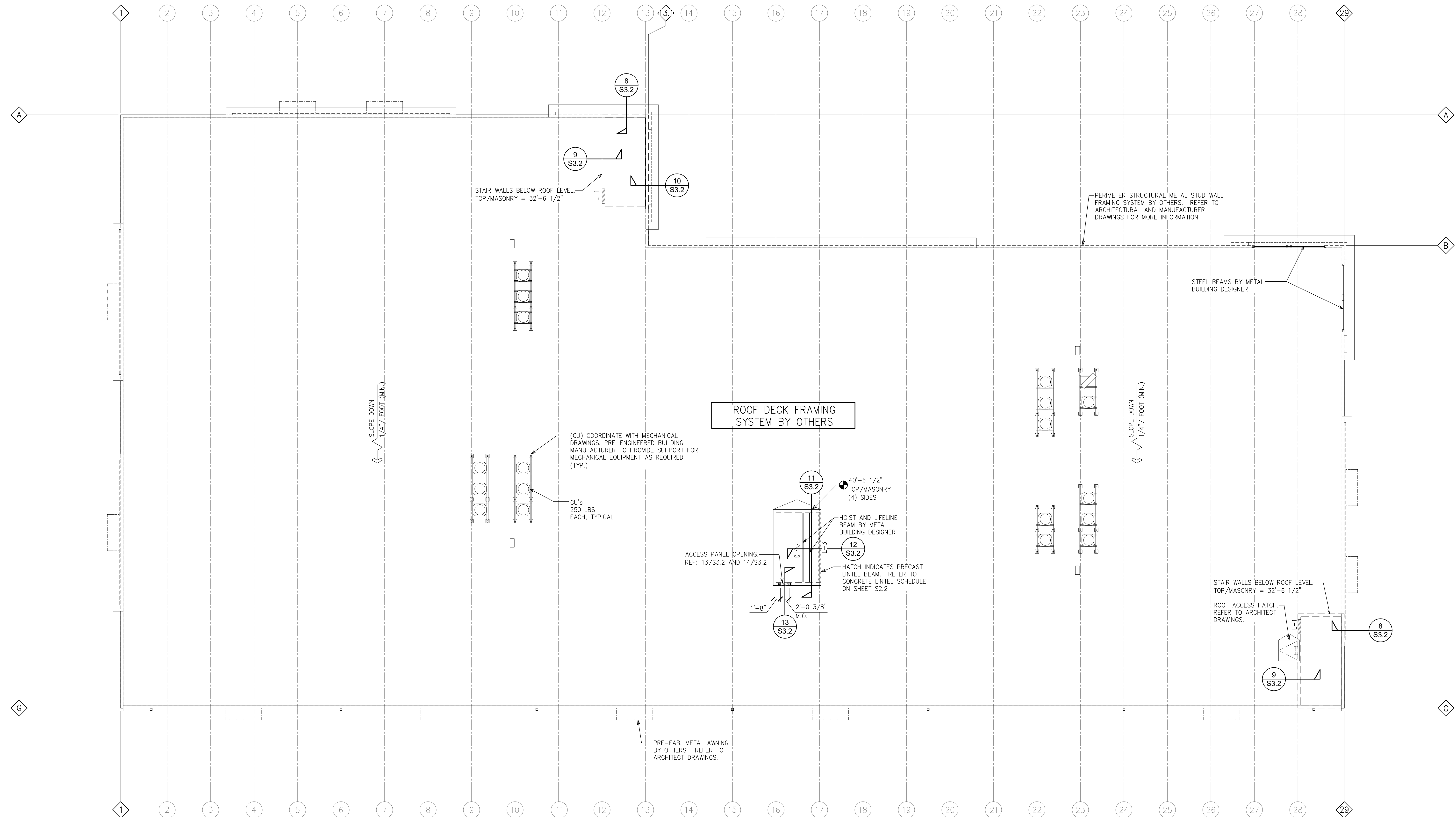
INTERIOR MASONRY WALLS HAVE BEEN ASSUMED TO BE Laterally BRACED BY
PRE-ENGINEERED DIAPHRAGM FLOOR SYSTEM AND INTERIOR LIGHT GAGE WALL
SYSTEM. METHOD OF ATTACHMENT OF THE INTERIOR METAL BUILDING
POSTS/SHEAR WALLS AND FLOOR FURLIN SYSTEM SHALL BE DESIGNED BY
METAL BUILDING SPECIALTY ENGINEER TO TRANSFER THE BUILDING LATERAL
WALL FORCES THROUGH THE ROOF PANEL/WALLS INTO A LATERAL RESISTING
SYSTEM DESIGNED BY THE BUILDING ENGINEER.

CONCRETE MIX DESIGN SUBMITTALS:
CONCRETE MIX DESIGNS SHALL BE SUBMITTED FOR CONCRETE SLAB-ON-GRADE
(BY THE STRUCTURAL ENGINEER) AND THE UPPER LEVEL, COMPOSITE CONCRETE
FLOOR SLABS (BY THE PEMB STRUCTURAL ENGINEER).
THE METHODOLOGY FOR THE CURING OF ALL CONCRETE SHALL BE INCLUDED
WITH CONCRETE MIX DESIGN SUBMITTALS. CONCRETE CURING SHALL BE
COMPATIBLE WITH THE CONCRETE FINISHING PRODUCTS SPECIFIED IN THE
ARCHITECTURAL DRAWINGS.
THE CONCRETE MIX DESIGNS AND CURING SUBMITTALS MUST BE REVIEWED AND
APPROVED BY THE STRUCTURAL ENGINEER OF RECORD AND/OR THE PEMB
STRUCTURAL ENGINEER PRIOR TO ORDER AND PLACEMENT OF CONCRETE.

UPPER LEVEL CONCRETE FLOOR SLABS:

THE DESIGN OF ALL UPPER LEVEL, COMPOSITE CONCRETE FLOOR SLABS SHALL BE THE
RESPONSIBILITY OF THE PROFESSIONAL ENGINEER (P.E.) DELEGATED BY THE
PRE-ENGINEERED METAL BUILDING (PEMB) MANUFACTURER AND DOCUMENTED WITHIN THE
SEALED ENGINEERED SHOP DRAWINGS. THE FOLLOWING STANDARDS OUTLINE THE MINIMUM
DESIGN AND PERFORMANCE REQUIREMENTS:

1. DESIGN RESPONSIBILITY AND DOCUMENTATION:
THE PEMB DELEGATED ENGINEER IS RESPONSIBLE FOR THE COMPREHENSIVE DESIGN
OF THE COMPOSITE FLOOR SYSTEM.
SUBMITTALS MUST EXPLICITLY DETAIL:
 - METAL DECKING: PROFILE DEPTH, MATERIAL GAUGE, AND YIELD STRESS.
 - CONCRETE PROPERTIES: TOTAL SLAB THICKNESS, SPECIFIED COMPRESSIVE
STRENGTH, AND AGGREGATE TYPE.
 - REINFORCEMENT: DETAILED SPECIFICATIONS FOR TEMPERATURE AND SHRINKAGE
REINFORCEMENT (E.G. WELDED WIRE REINFORCEMENT OR FIBER MESH).
 - CURING PROTOCOLS: NECESSARY METHODS AND DURATIONS TO ENSURE
PROPER HYDRATION AND STRUCTURAL INTEGRITY.
2. BASE DESIGN SPECIFICATION FOR SPANS NOT EXCEEDING 10'-0" UNDER LIGHT
STORAGE LOADING CONDITIONS, THE MINIMUM DESIGN SHALL CONSIST OF:
 - DECKING: 2" DEEP, 18-GAUGE COMPOSITE METAL DECK (50 KSI).
 - CONCRETE: 2" NORMAL WEIGHT CONCRETE (NWC) COVER OVER THE RIBS.
 - PERFORMANCE: 3,000 PSI COMPRESSIVE STRENGTH AT 28 DAYS.
 - TOTAL THICKNESS: 4" OVERALL SLAB DEPTH.
3. ALTERNATE DESIGN RECOMMENDATION (PERFORMANCE OPTIMIZED) TO MITIGATE DECK
DEFLECTION DURING THE WET CONCRETE POUR AND MINIMIZE SUBSEQUENT
CONCRETE SHRINKAGE CRACKING, THE PEMB SHALL PROVIDE AN ALTERNATE BID
FOR THE FOLLOWING ASSEMBLY:
 - DECKING: 3" DEEP, 18-GAUGE COMPOSITE METAL DECK (50 KSI).
 - CONCRETE: 2" NORMAL WEIGHT CONCRETE (NWC) COVER OVER THE RIBS.
 - PERFORMANCE: 3,000 PSI COMPRESSIVE STRENGTH AT 28 DAYS.
 - TOTAL THICKNESS: 5" OVERALL SLAB DEPTH.

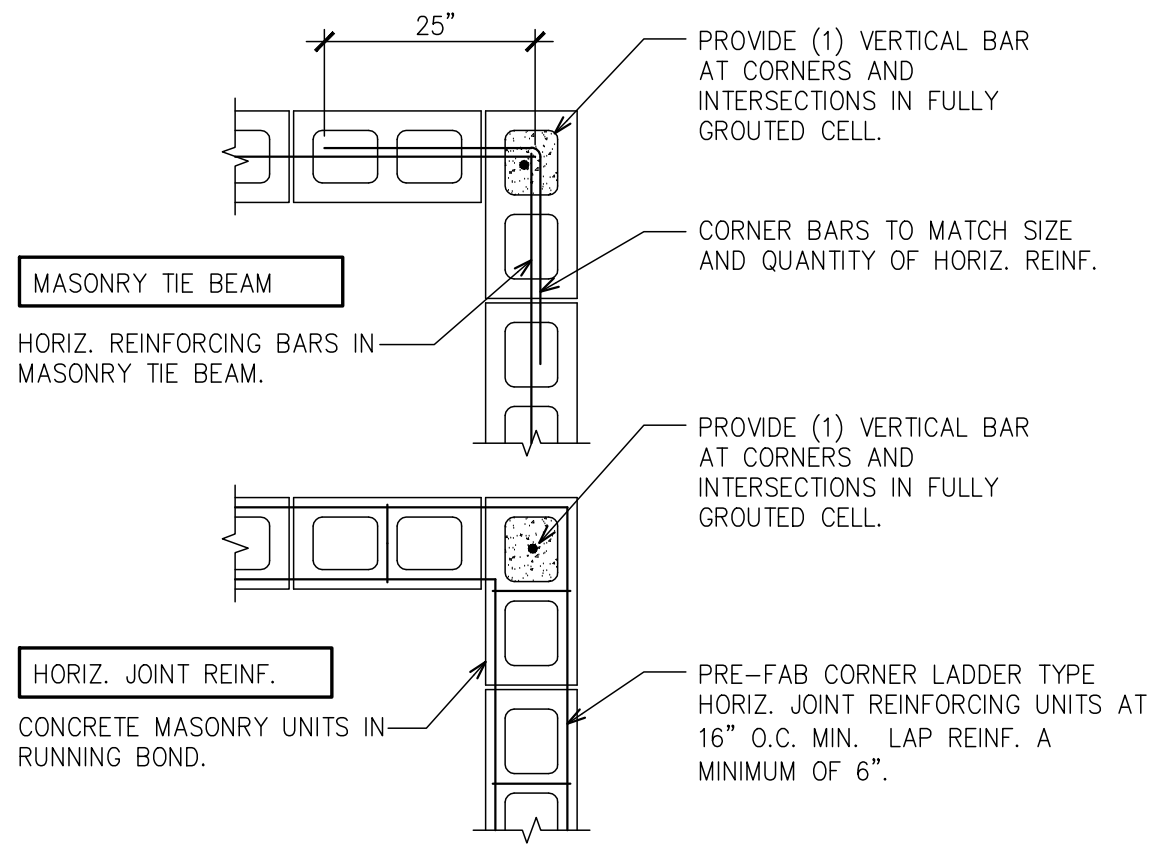


ROOF FRAMING PLAN
SCALE: 3/32" = 1'-0"

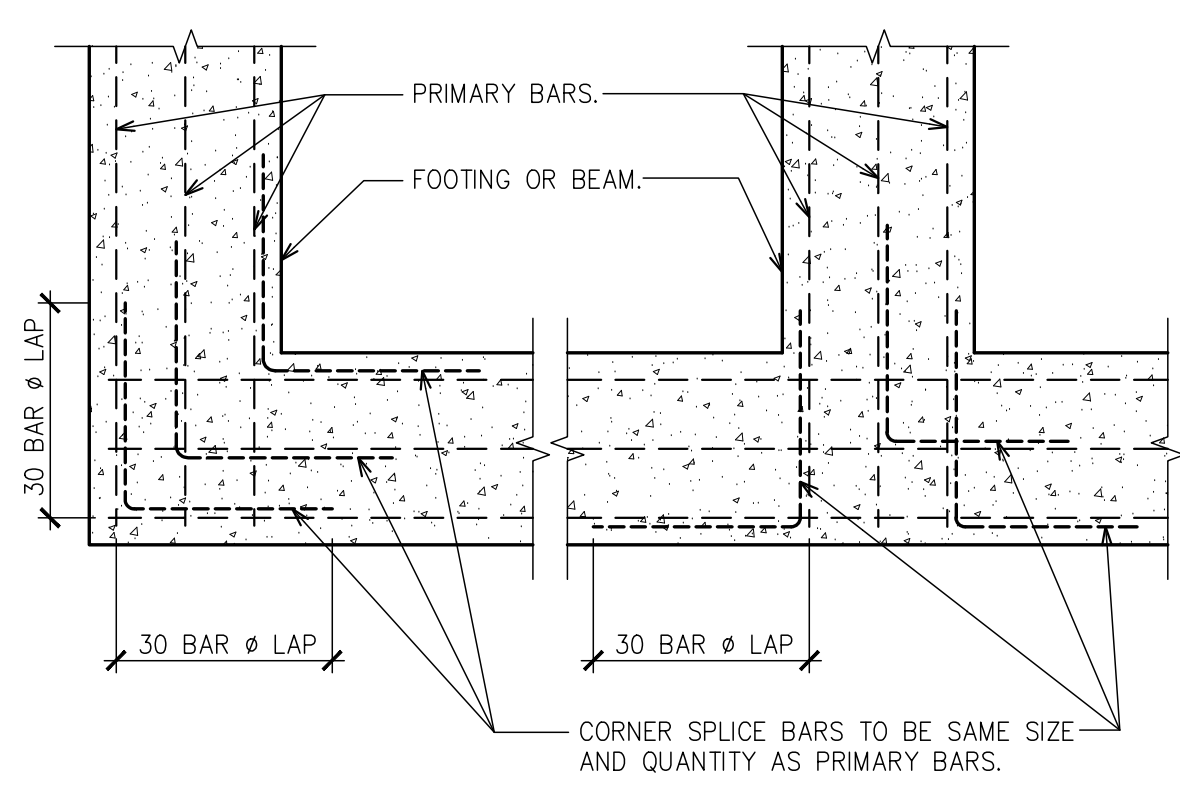
EXTERIOR MASONRY WALLS HAVE BEEN ASSUMED TO BE Laterally Braced By
PRE-ENGINEERED DIAPHRAGM ROOF SYSTEM AND INTERIOR LIGHT GAGE WALL
SYSTEM. METHOD OF ATTACHMENT OF THE INTERIOR METAL BUILDING
POSTS/SHEAR WALLS AND ROOF PURLIN SYSTEM SHALL BE DESIGNED BY METAL
BUILDING SPECIALTY ENGINEER TO TRANSFER THE BUILDING LATERAL WALL
FORCES THROUGH THE ROOF PANEL/WALLS INTO A LATERAL RESISTING SYSTEM
DESIGNED BY THE BUILDING ENGINEER.

ROOF FRAMING PLAN

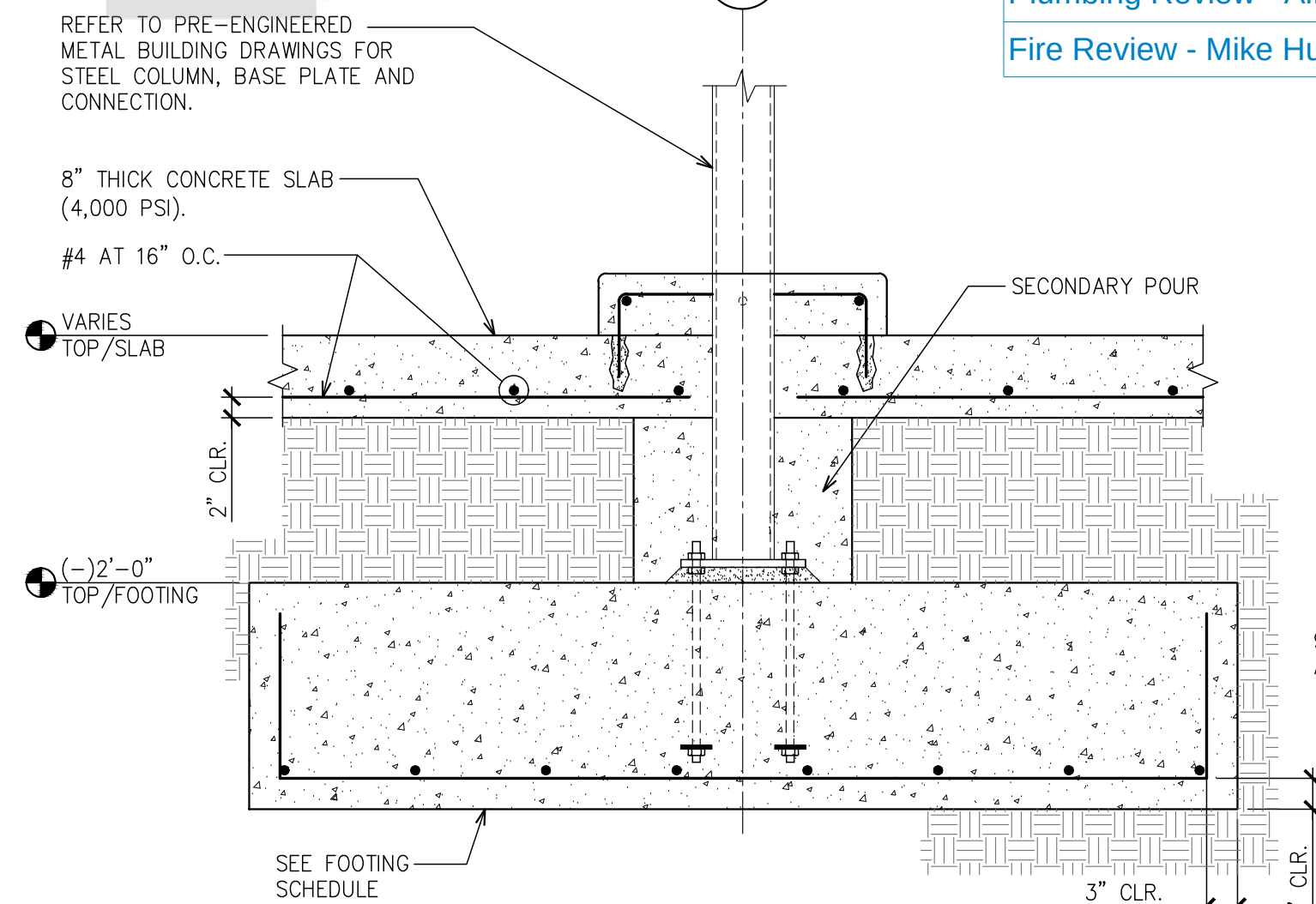
1. REFER TO SHEET S2.2 FOR CONCRETE LINTEL SCHEDULE.
2. REFER TO 1/S1.1 FOR DESIGN WIND CRITERIA.



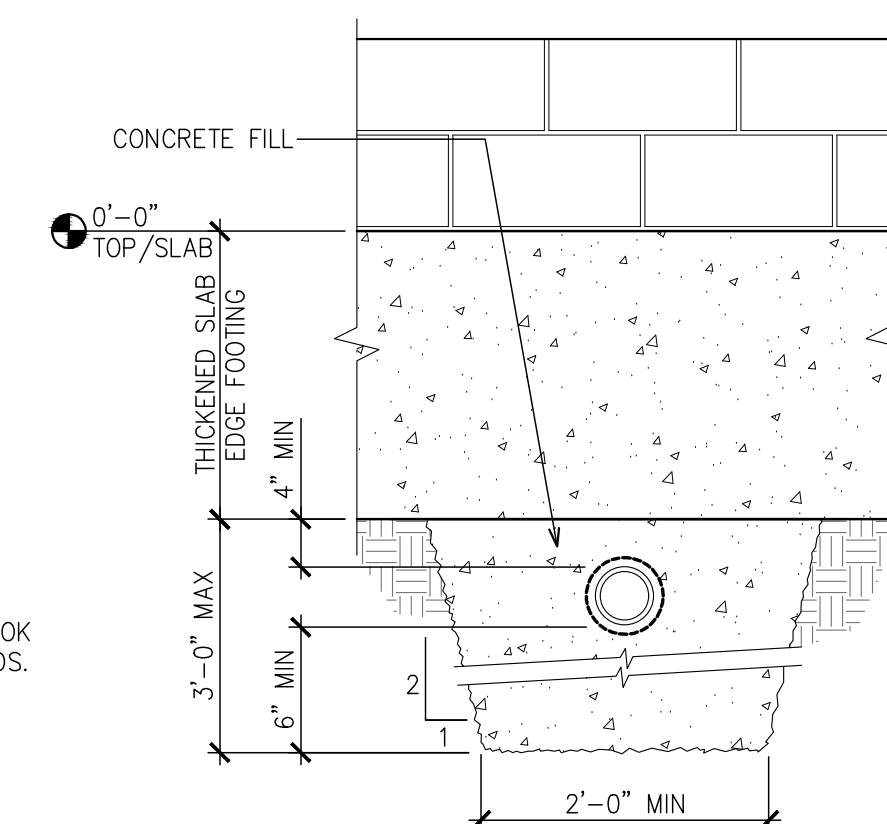
1 TYPICAL REINF. DETAIL
AT CORNERS AND INTERSECTIONS
SCALE: N.T.S.



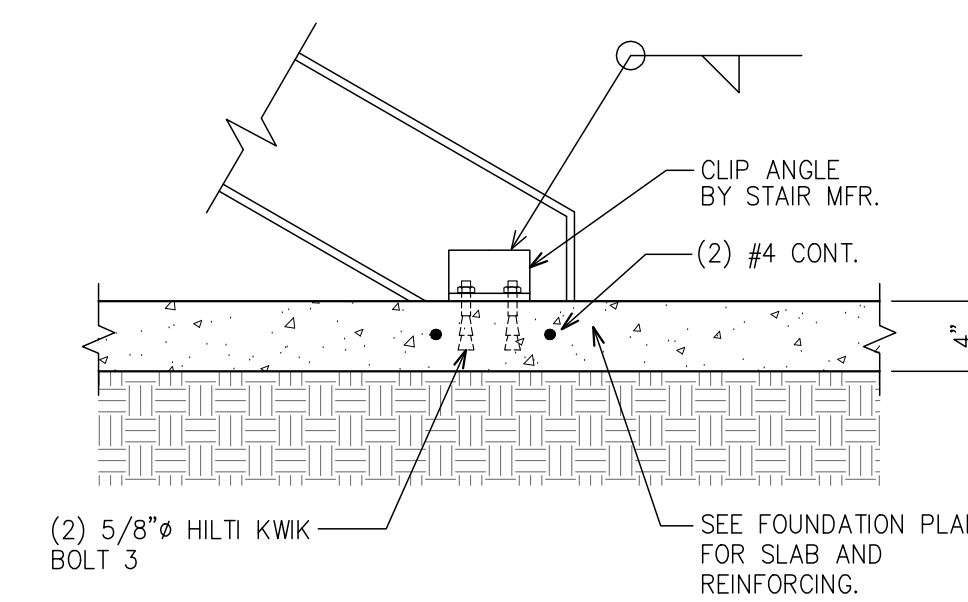
2 CORNER BAR DETAIL
NOT TO SCALE



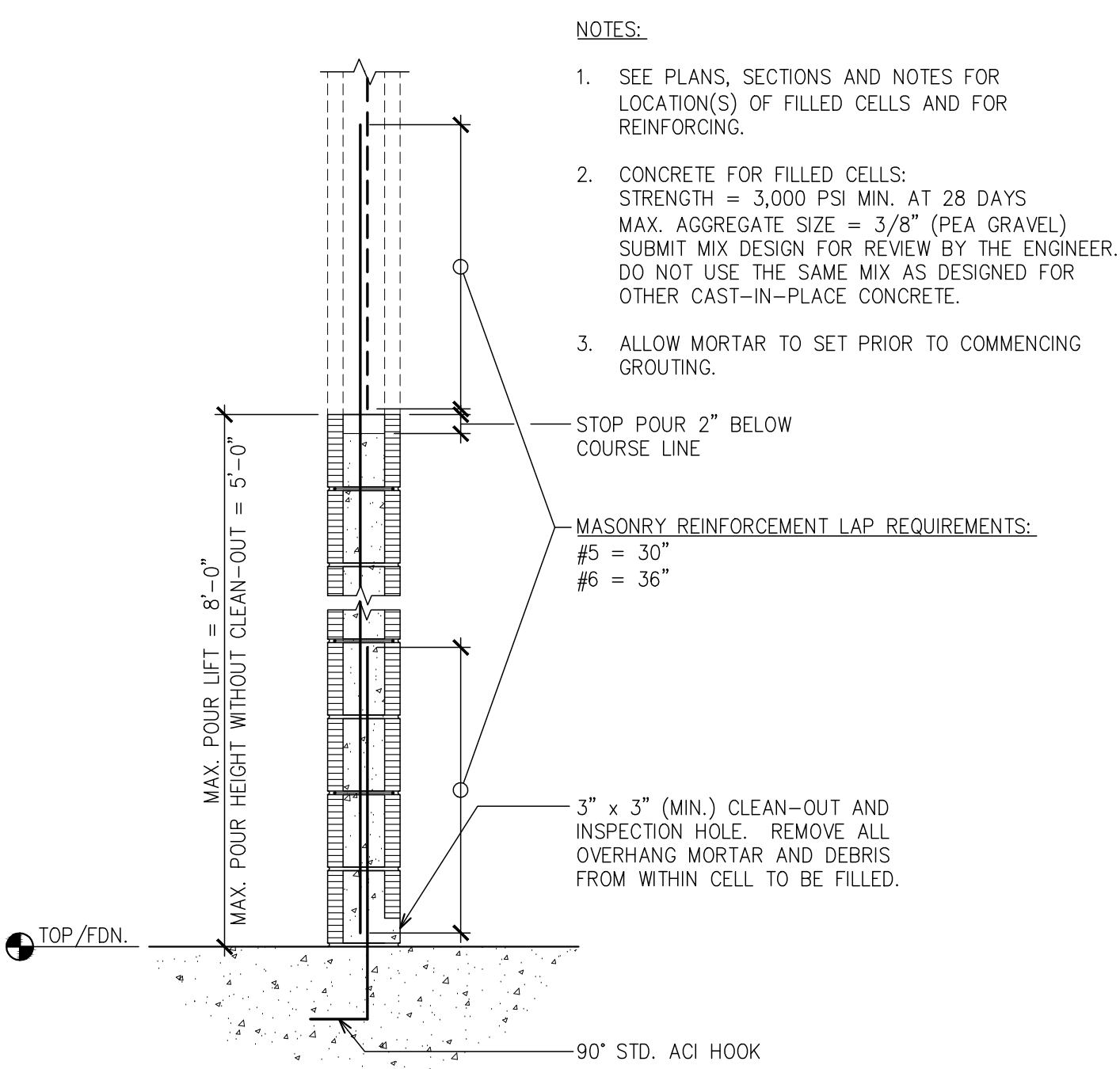
3 SECTION AT STEEL COLUMN
SCALE: 3/4" = 1'-0"



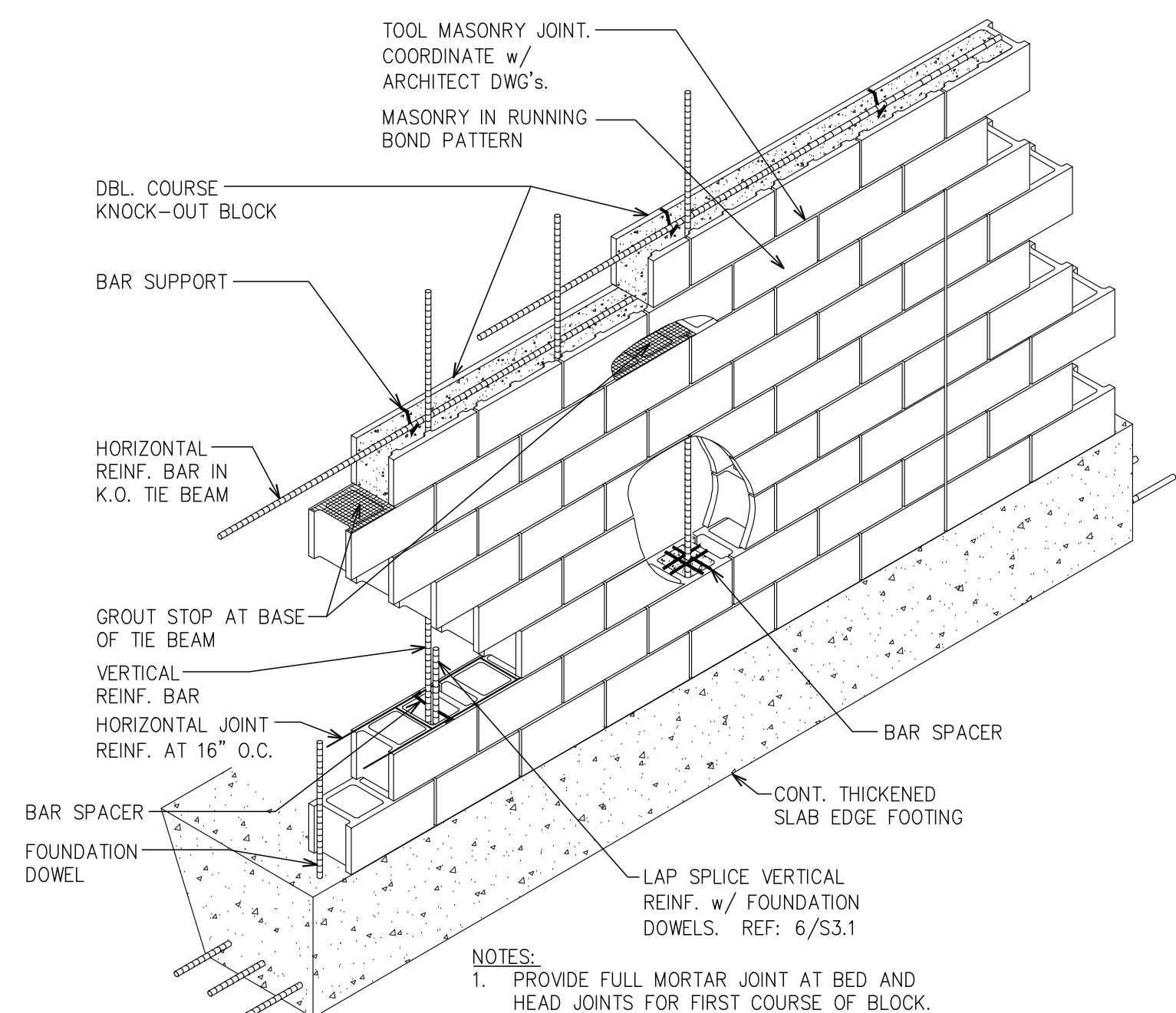
4 FOUNDATION DETAIL AT CONDUIT/PIPE
NOT TO SCALE



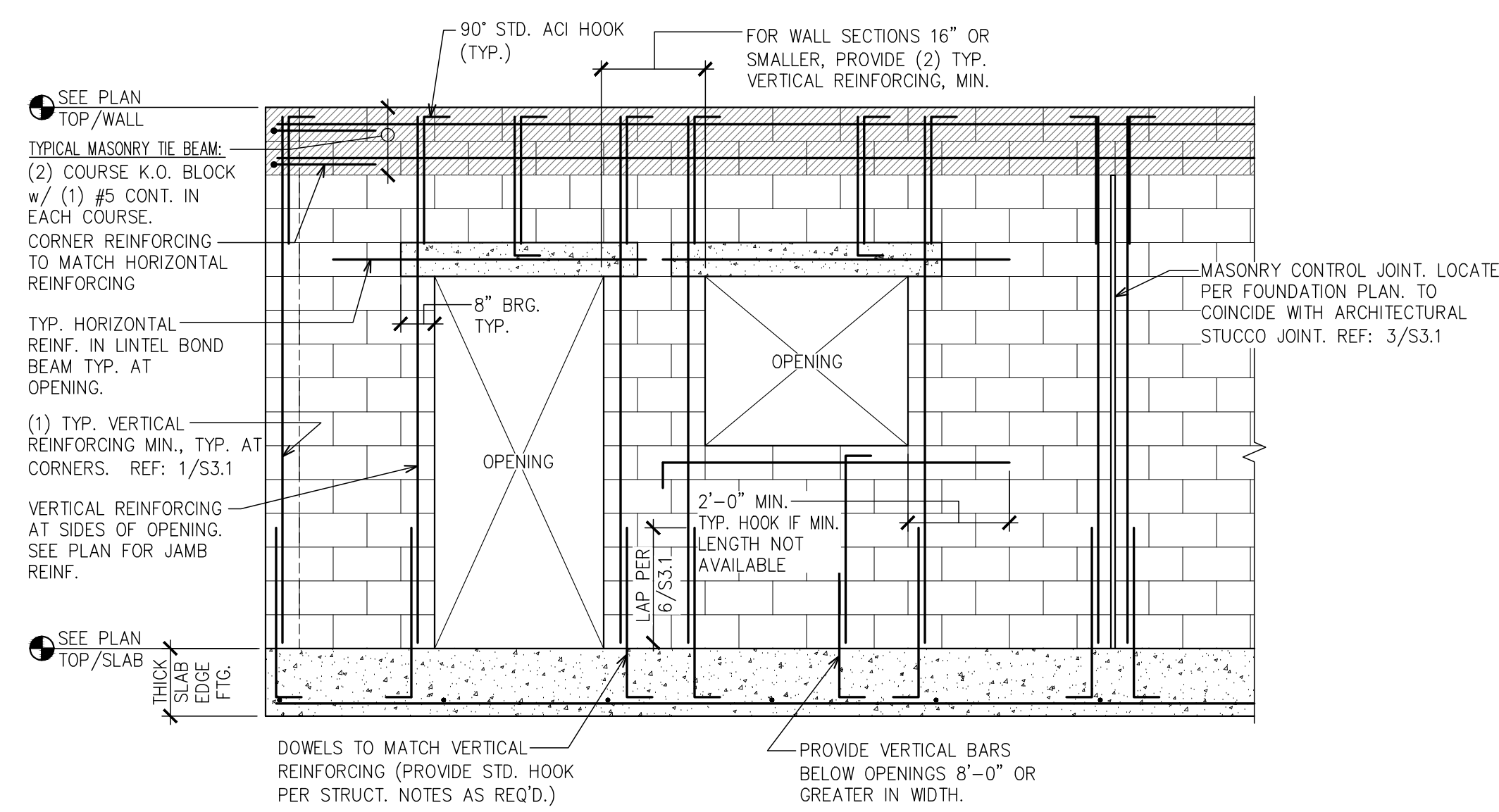
5 BASE OF METAL STAIR
SCALE: 1" = 1'-0"



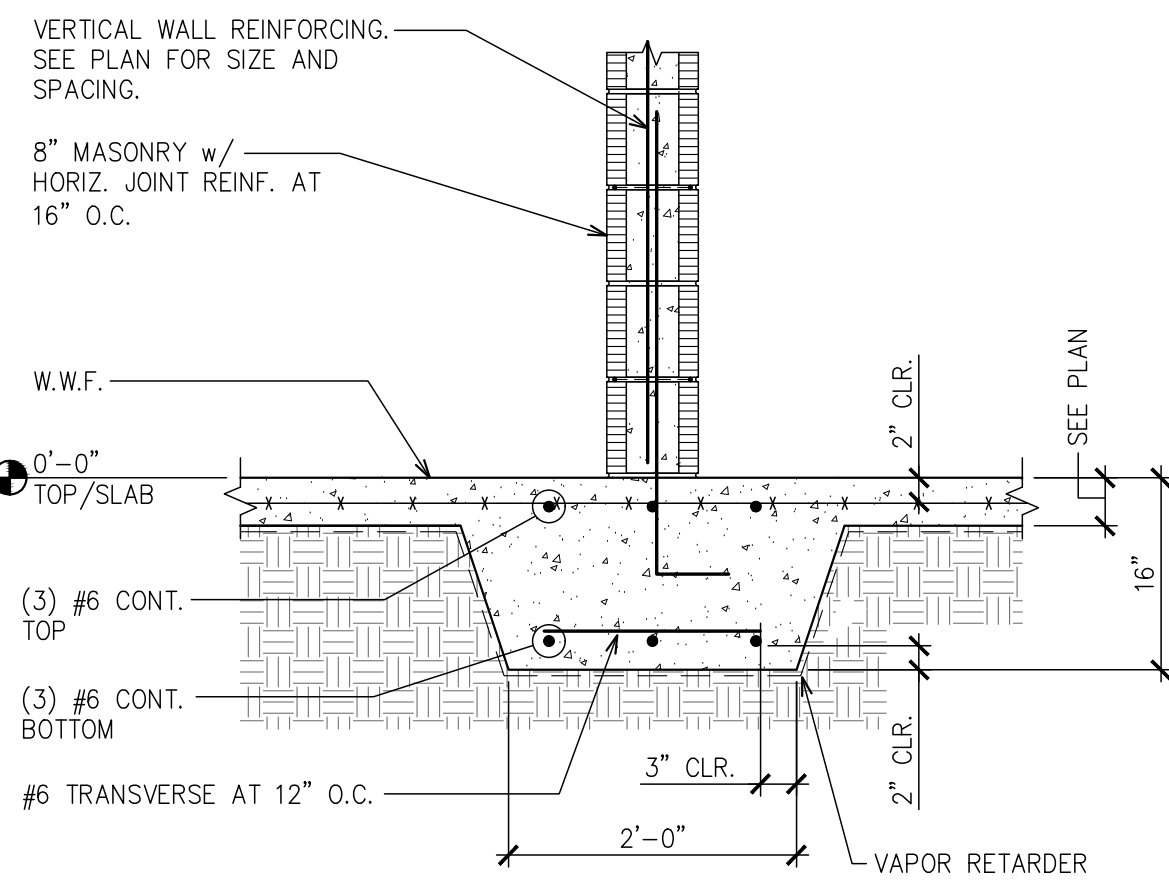
6 MASONRY LIFT DETAIL
SCALE: 3/4" = 1'-0"



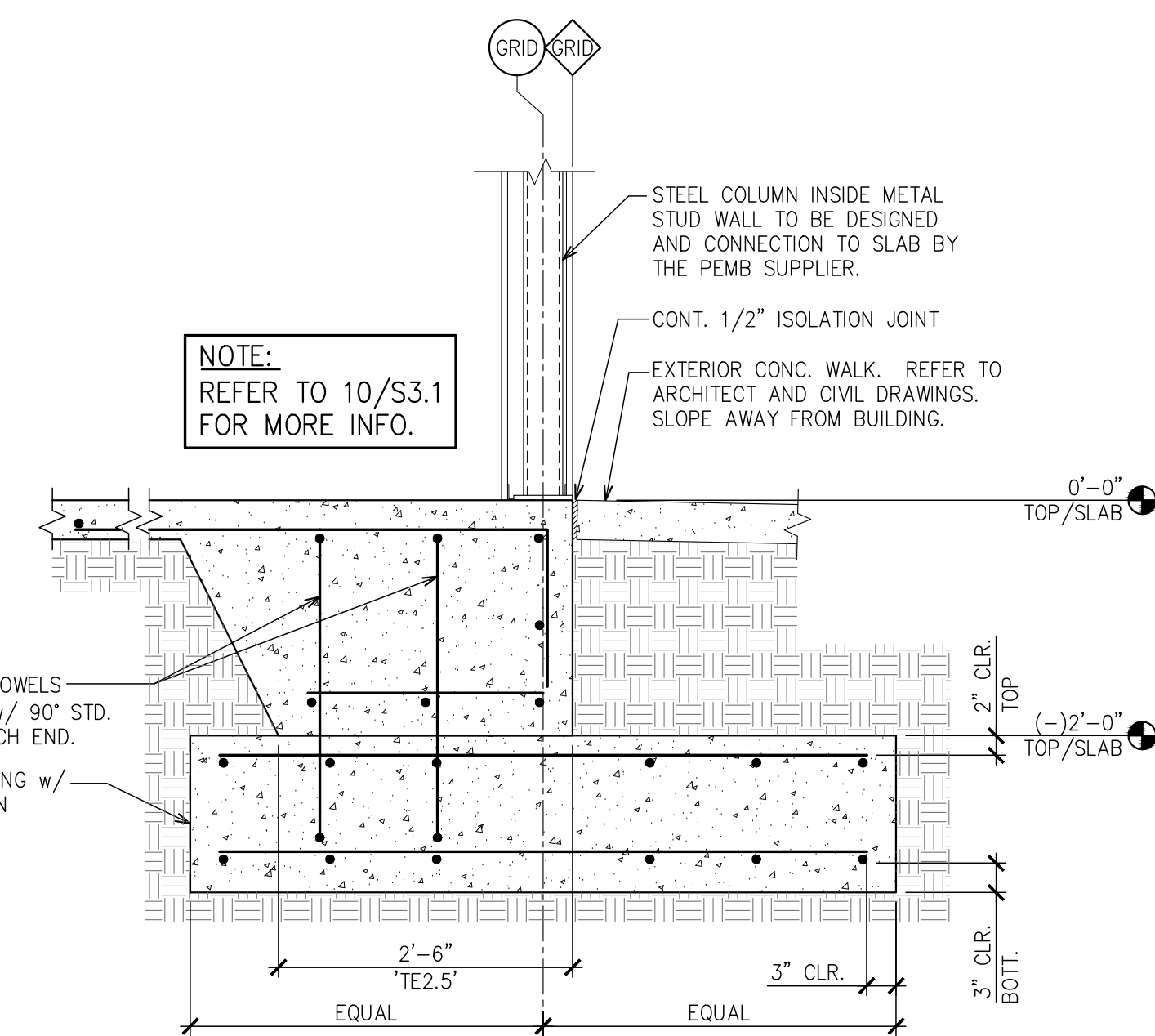
7 TYPICAL MASONRY CONSTRUCTION
NOT TO SCALE



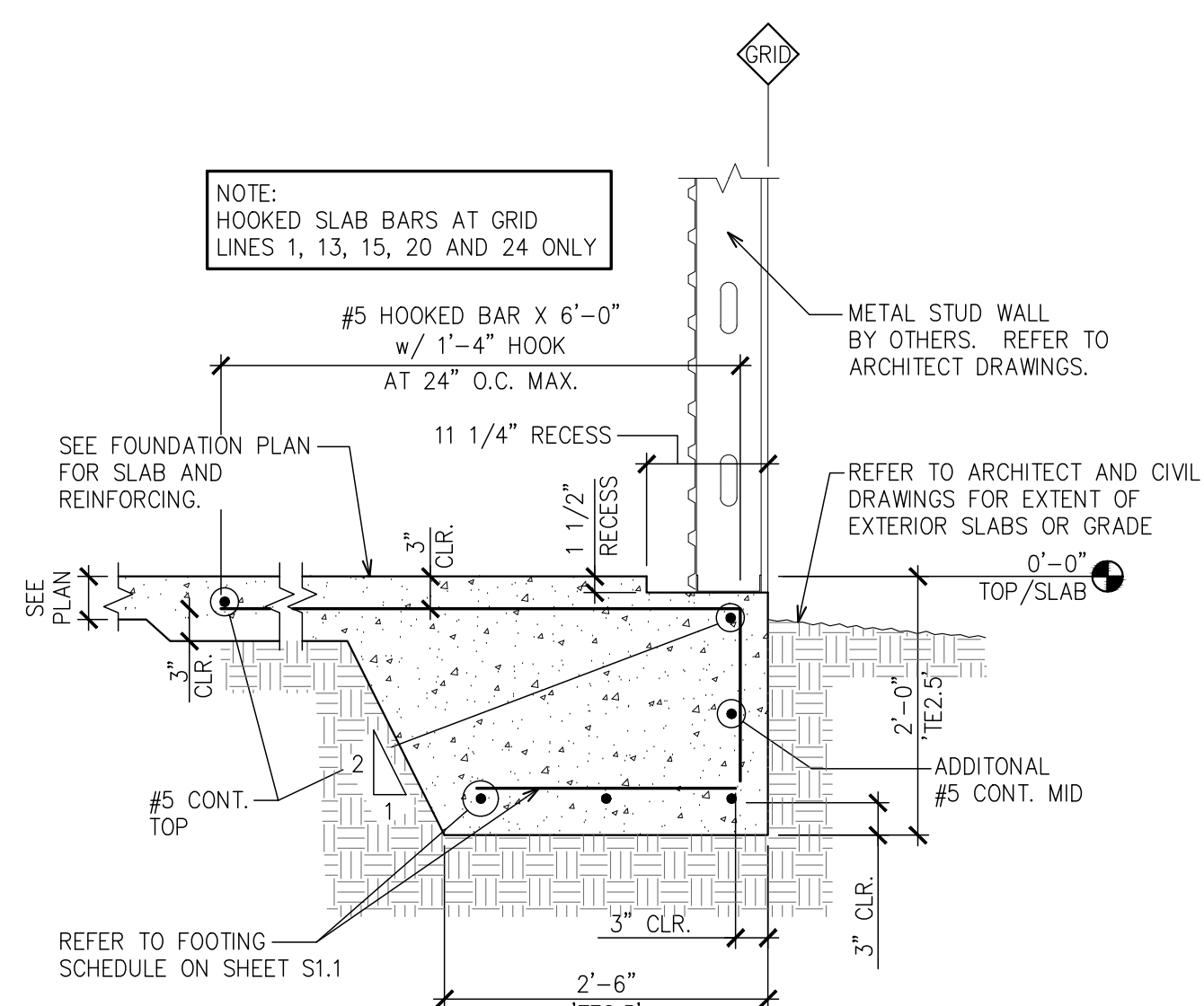
8 TYPICAL MASONRY REINFORCING AT OPENINGS -- ELEVATION
NOT TO SCALE



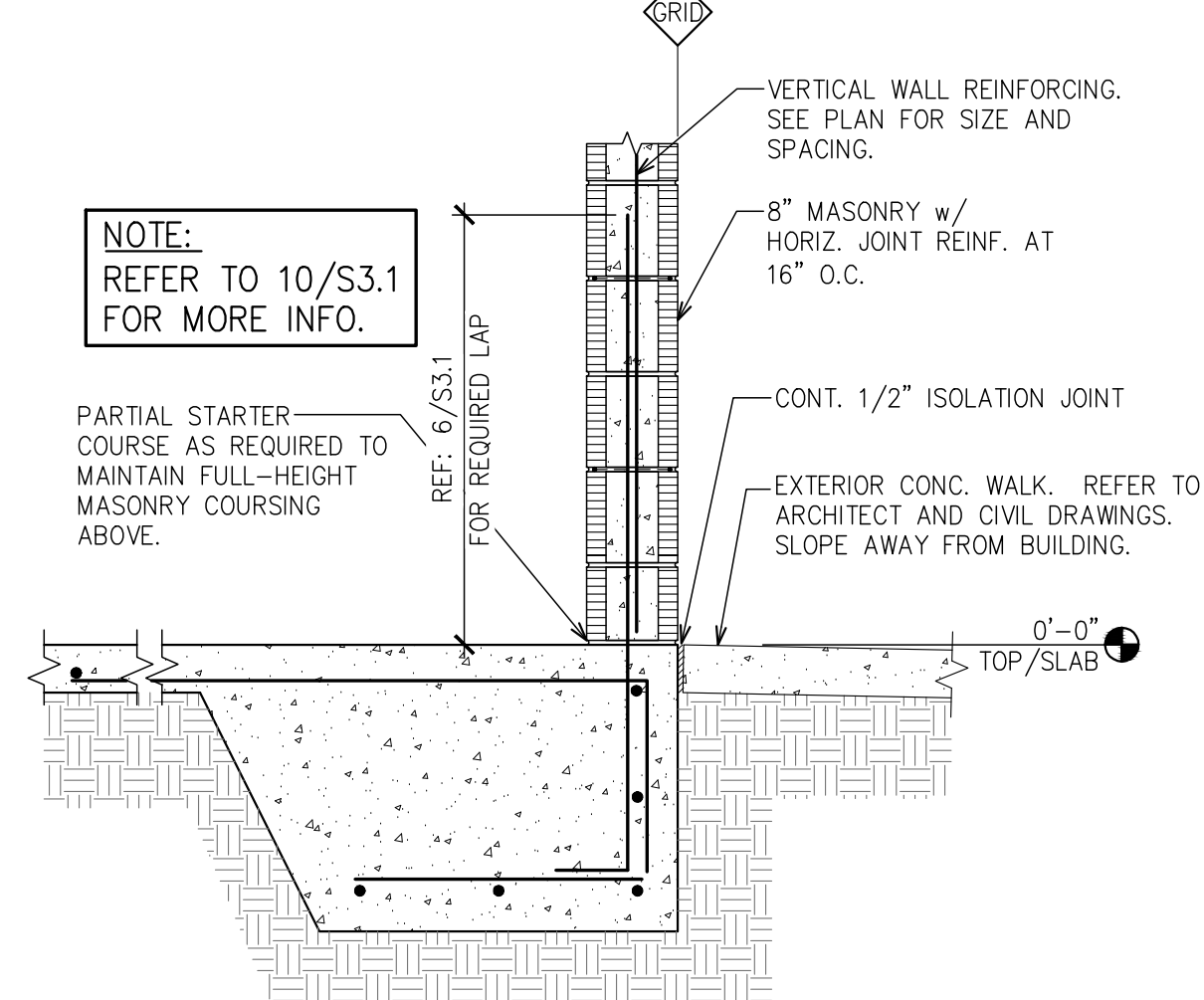
9 SECTION AT INTERIOR MASONRY WALL
SCALE: 3/4" = 1'-0"



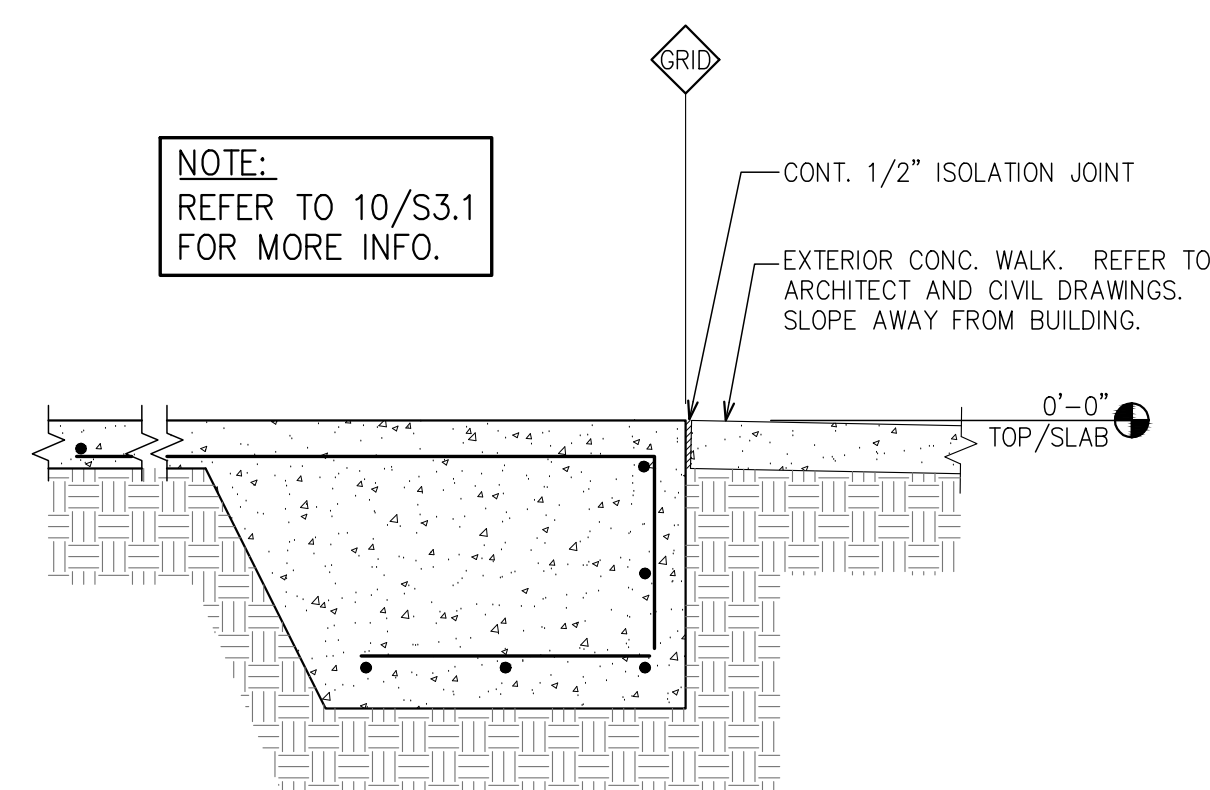
16 SECTION AT STEEL COLUMN
SCALE: 3/4" = 1'-0"



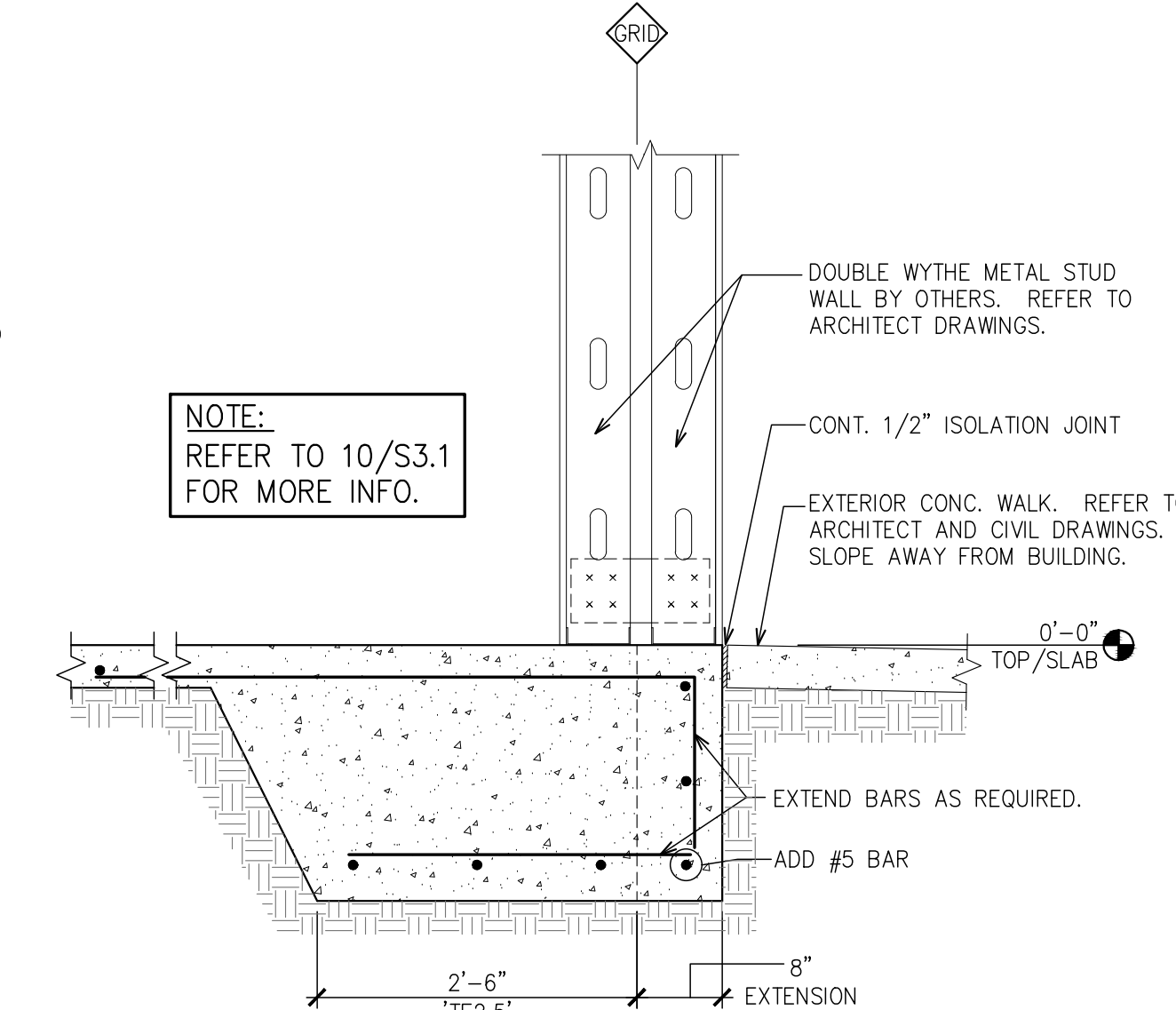
10 THICKENED SLAB AT PERIMETER
SCALE: 3/4" = 1'-0"



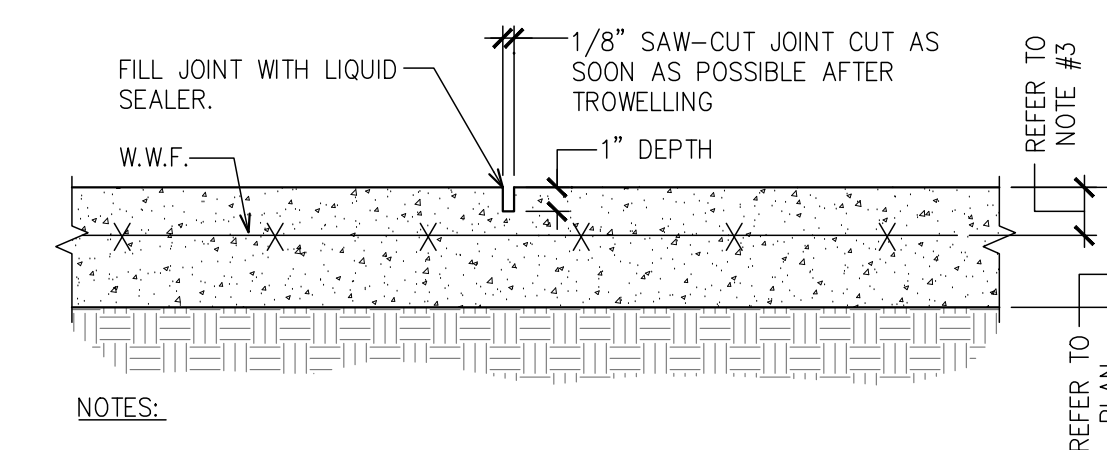
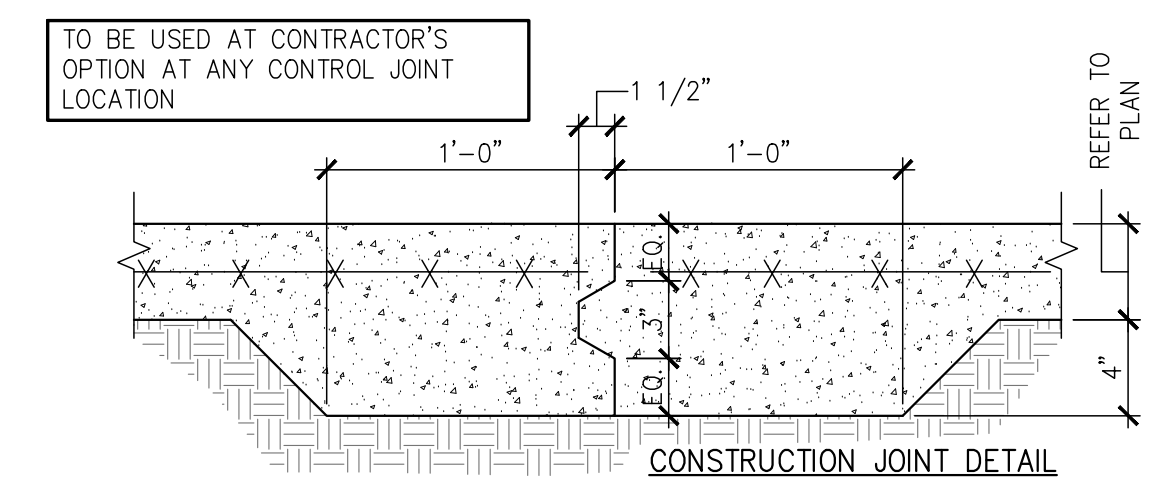
11 THICKENED SLAB AT MASONRY WALL (NO DAP)
SCALE: 3/4" = 1'-0"



12 THICK SLAB EDGE (NO DAP)
SCALE: 3/4" = 1'-0"



13 THICKENED SLAB AT METAL STUD BUMP-OUT
SCALE: 3/4" = 1'-0"

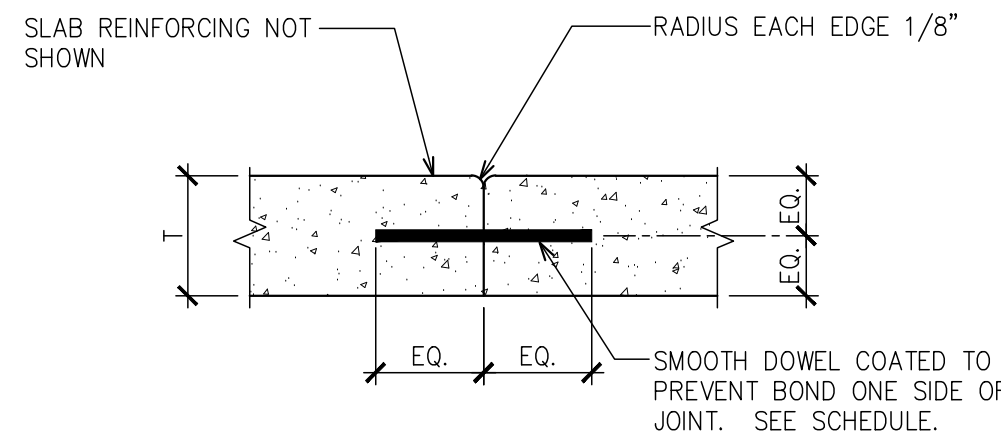


- NOTES:
- CUT SLAB AS SOON AS AGGREGATE DOES NOT DISLODGE (MUST BE WITHIN 12 HOURS OF CONCRETE PLACEMENT).
 - HAND TOOL JOINT TO FACE OF WALL WHERE SAW DOES NOT REACH.
 - PLACE REINFORCING AT MID-DEPTH FOR SLABS LESS THAN 5" THICK. PLACE REINFORCING 1/3 SLAB DEPTH (FROM TOP) FOR SLABS 5" THICK OR GREATER.

14 CONTROL AND CONSTRUCTION JOINT
SCALE: 1 1/2" = 1'-0"

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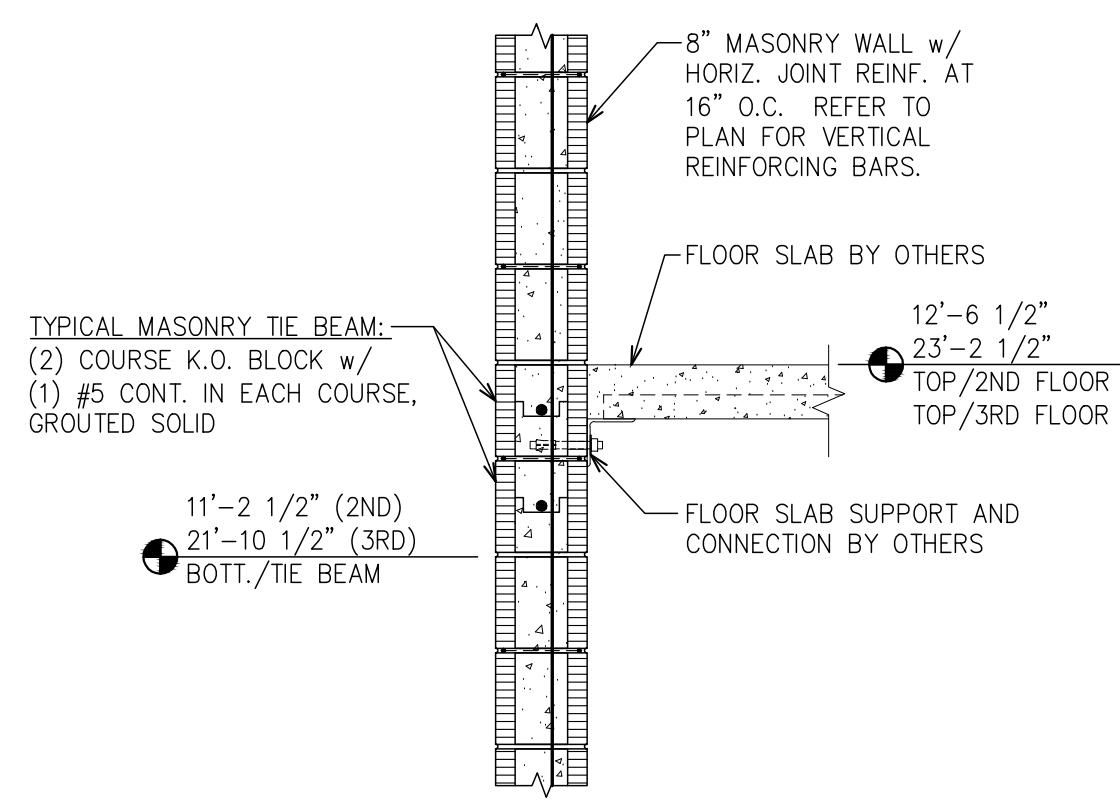
PLOT DATE: 2/20/2026



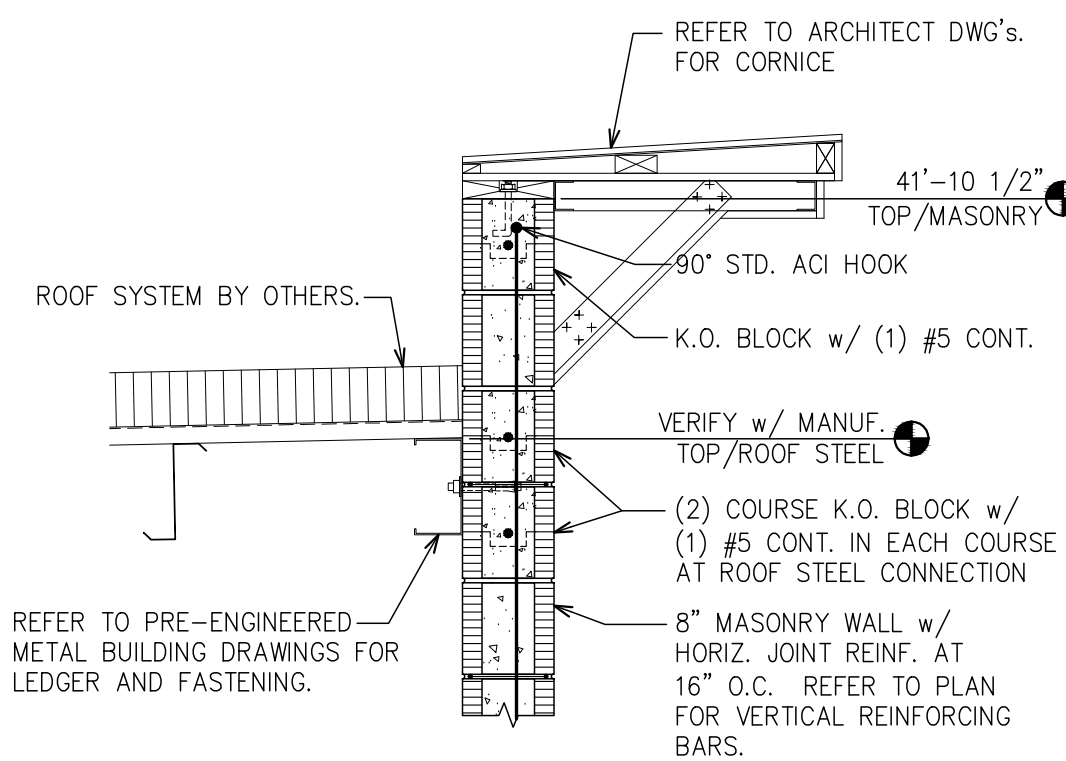
DOWEL SCHEDULE	
T	SMOOTH DOWEL SIZE AND SPACING
5"-8"	3/4"Ø X 1'-2" AT 12" O.C.
9"-12"	1 1/4"Ø X 1'-6" AT 12" O.C.

ALTERNATIVELY USE DIAMOND SHAPED LOAD PLATE (DIAMOND DOWEL SYSTEM INCLUDING INSTALLATION METHOD BY PWA-INC. OR EQUIVALENT INSTALL PER MANUFACTURER'S INSTRUCTIONS)

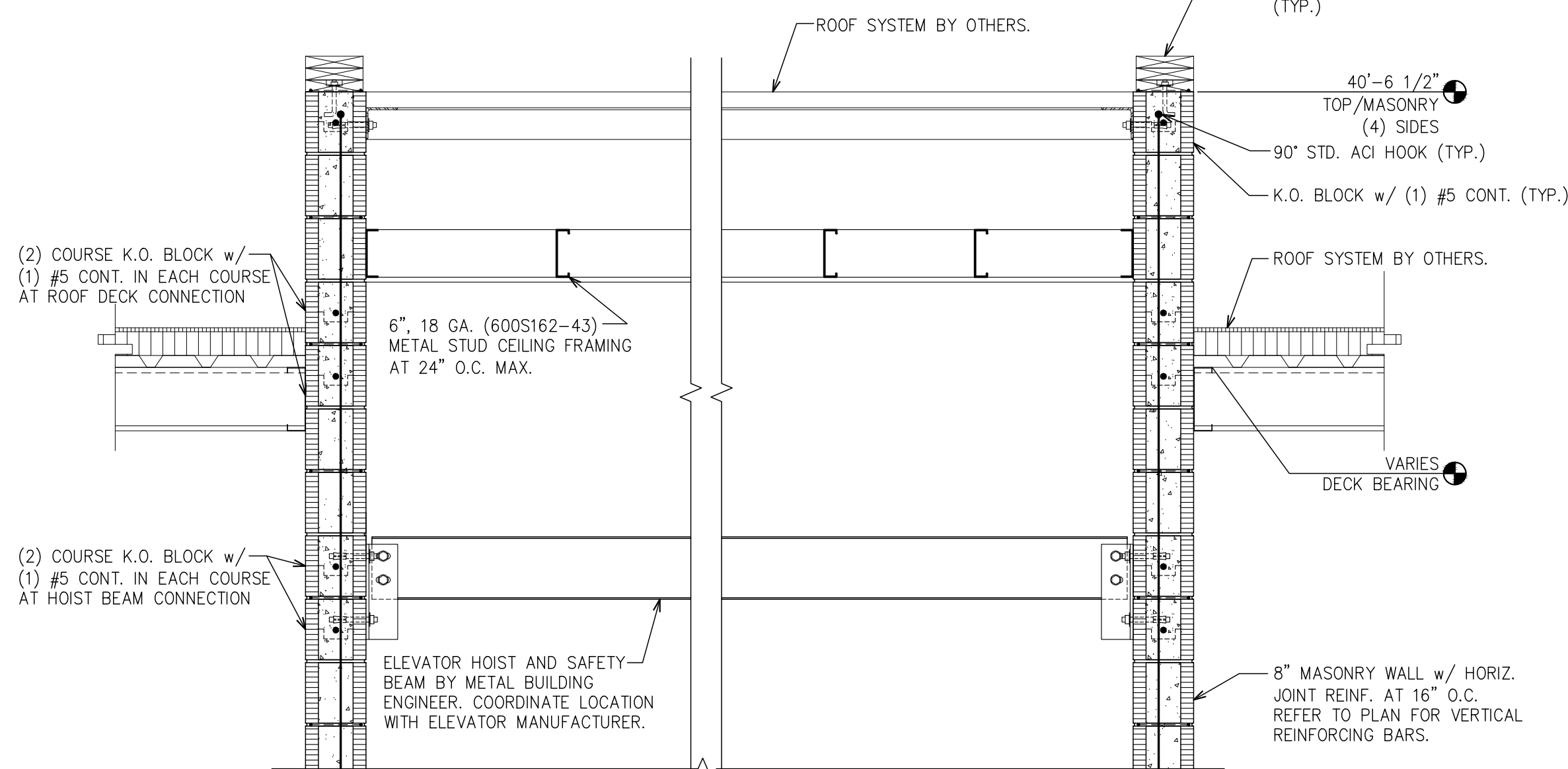
CONSTRUCTION JOINT
SLAB-ON-GRADE GREATER
THAN 4" THICK
SCALE: 3/4" = 1'-0"



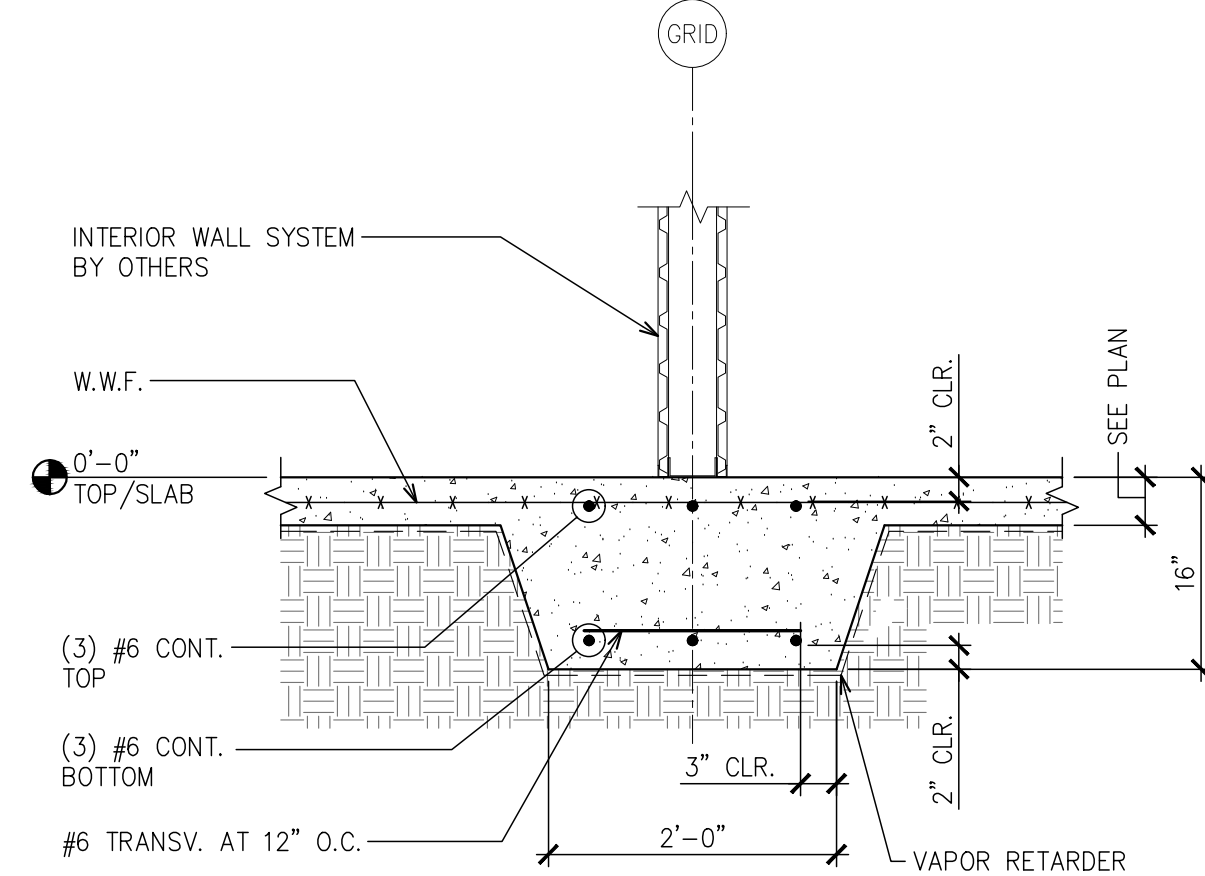
6 SECTION
SCALE: 3/4" = 1'-0"



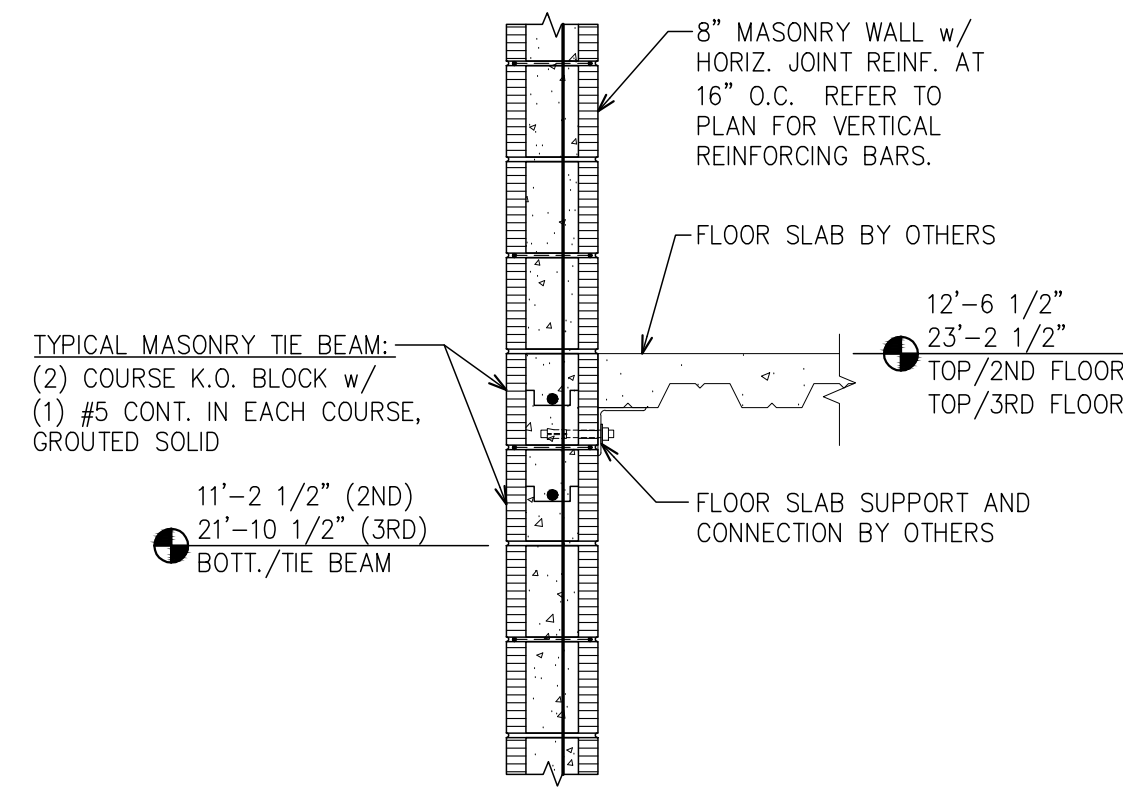
8 SECTION AT ROOF
SCALE: 3/4" = 1'-0"



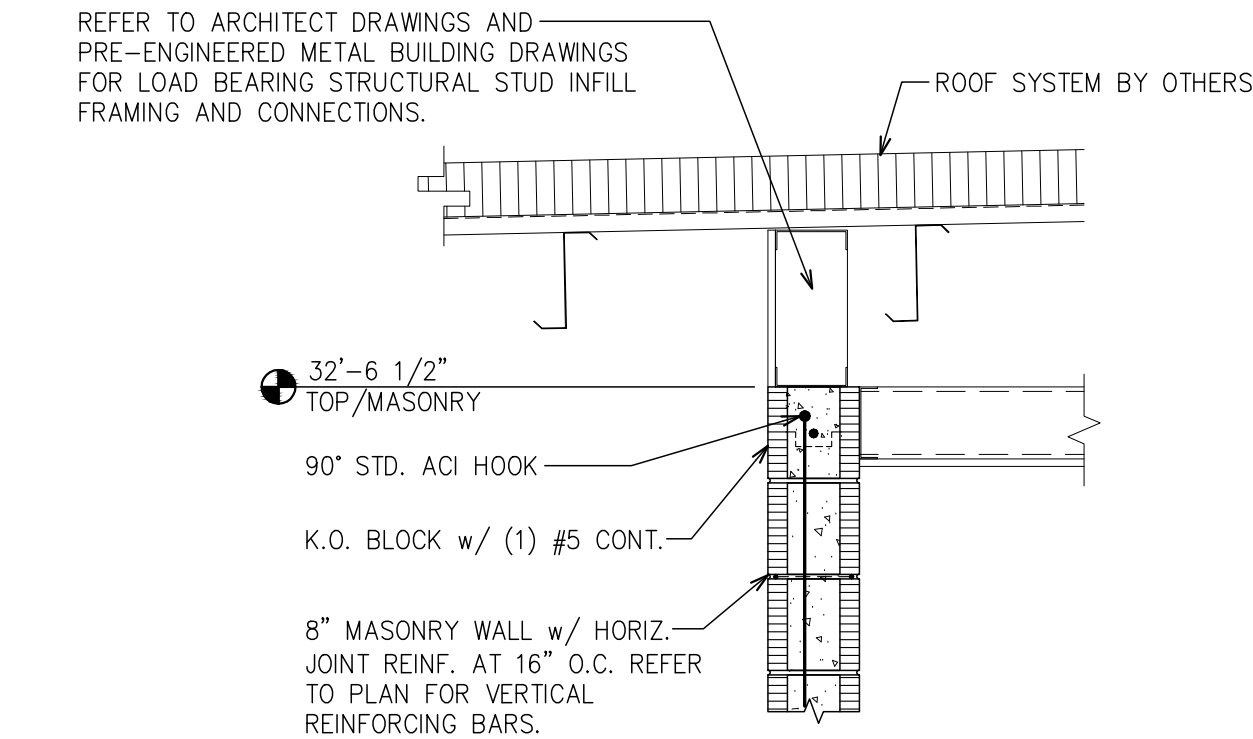
11 SECTION AT ELEVATOR ROOF
SCALE: 3/4" = 1'-0"



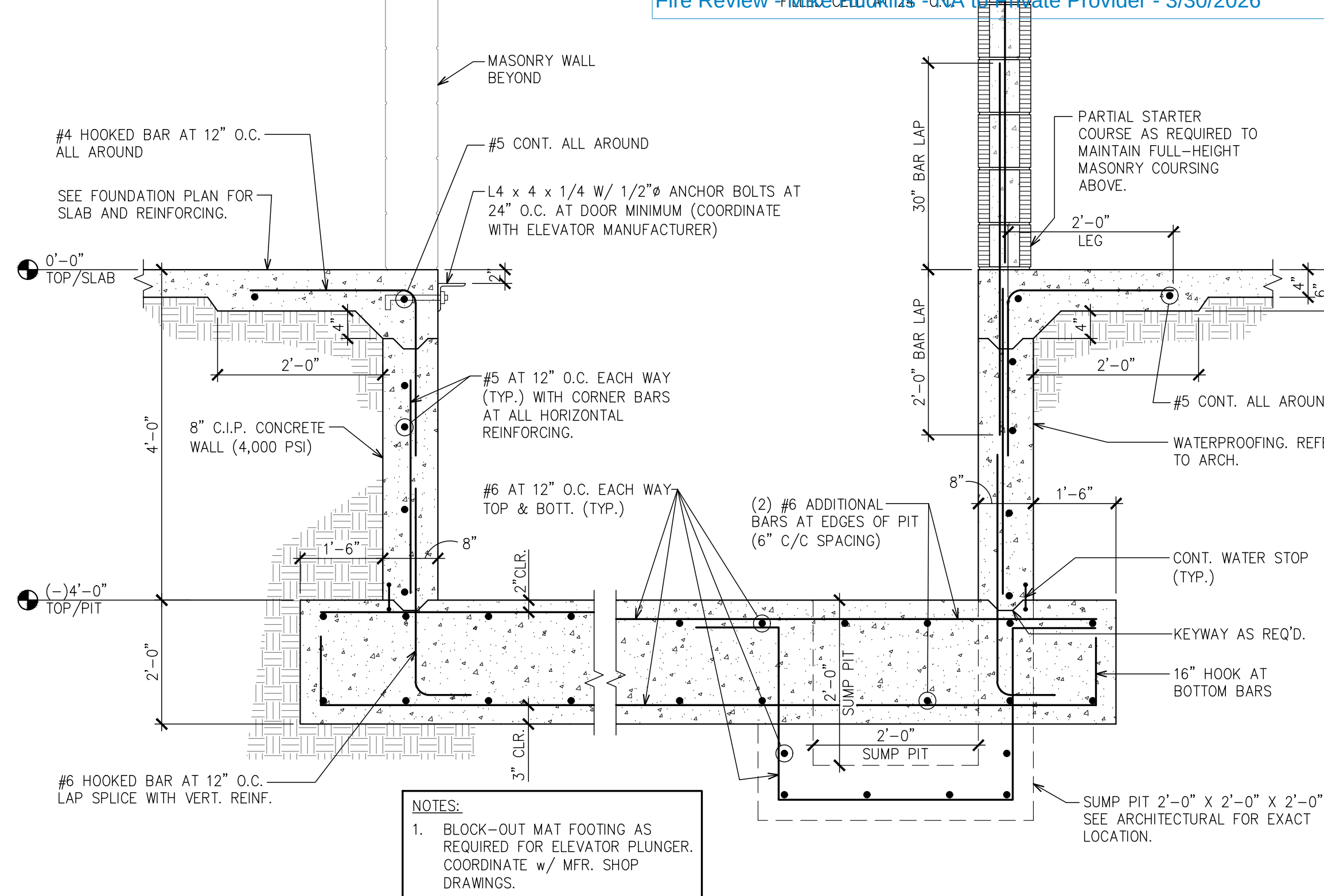
2 SECTION
SCALE: 3/4" = 1'-0"



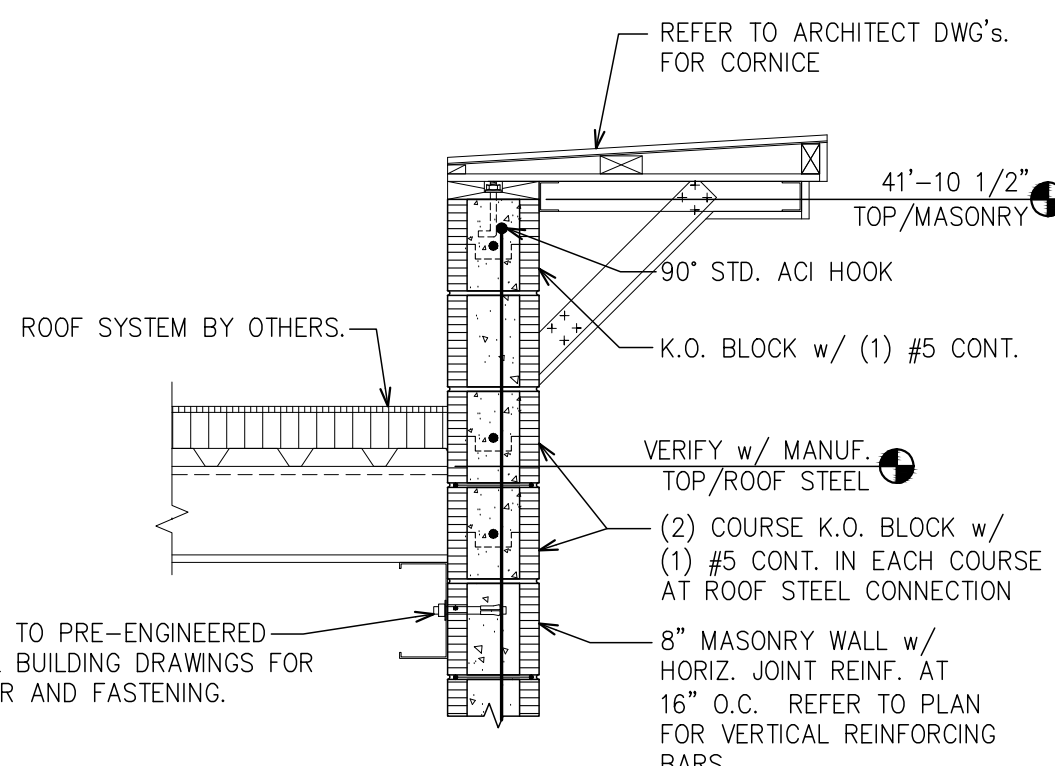
7 SECTION
SCALE: 3/4" = 1'-0"



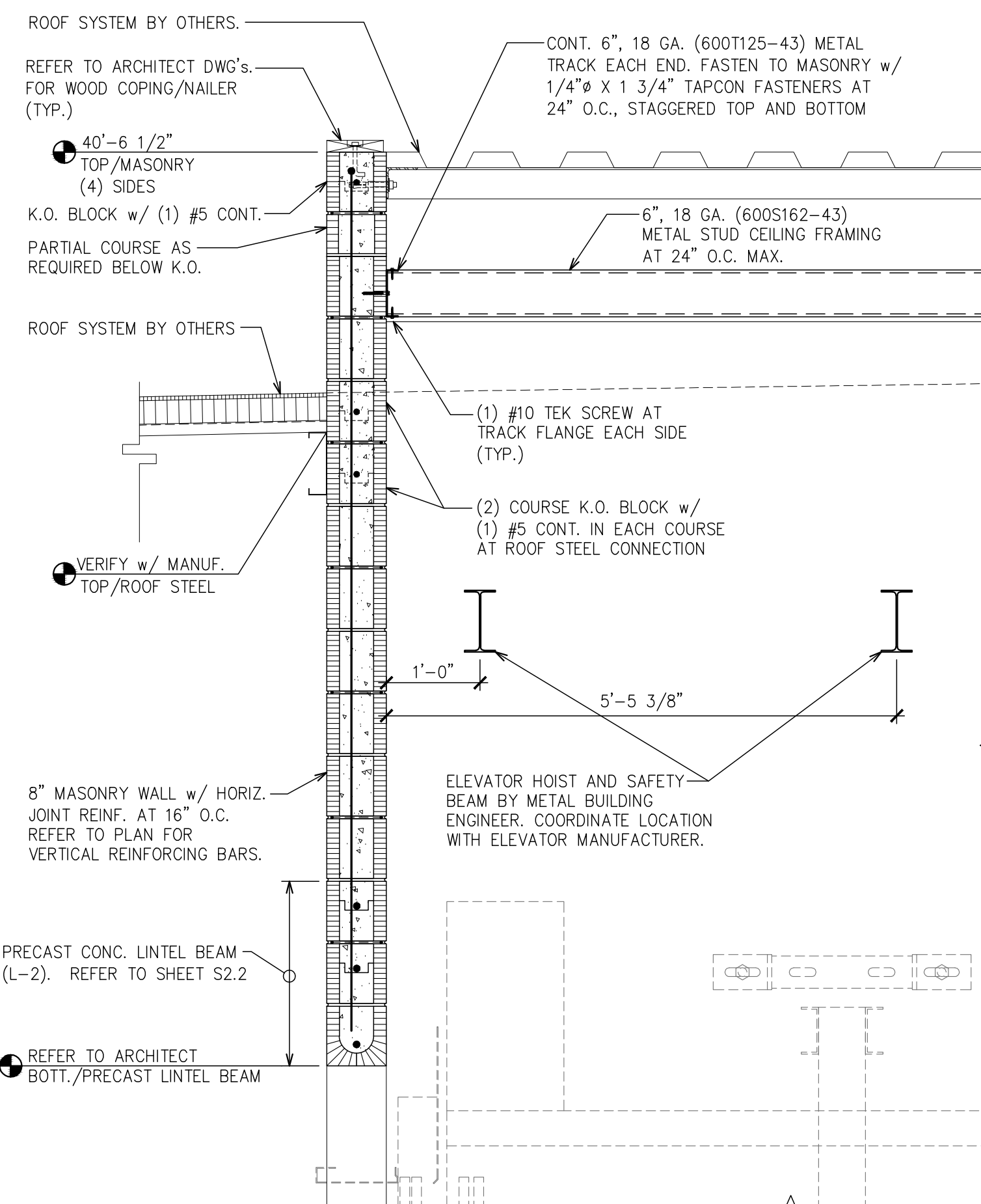
9 SECTION AT ROOF
SCALE: 3/4" = 1'-0"



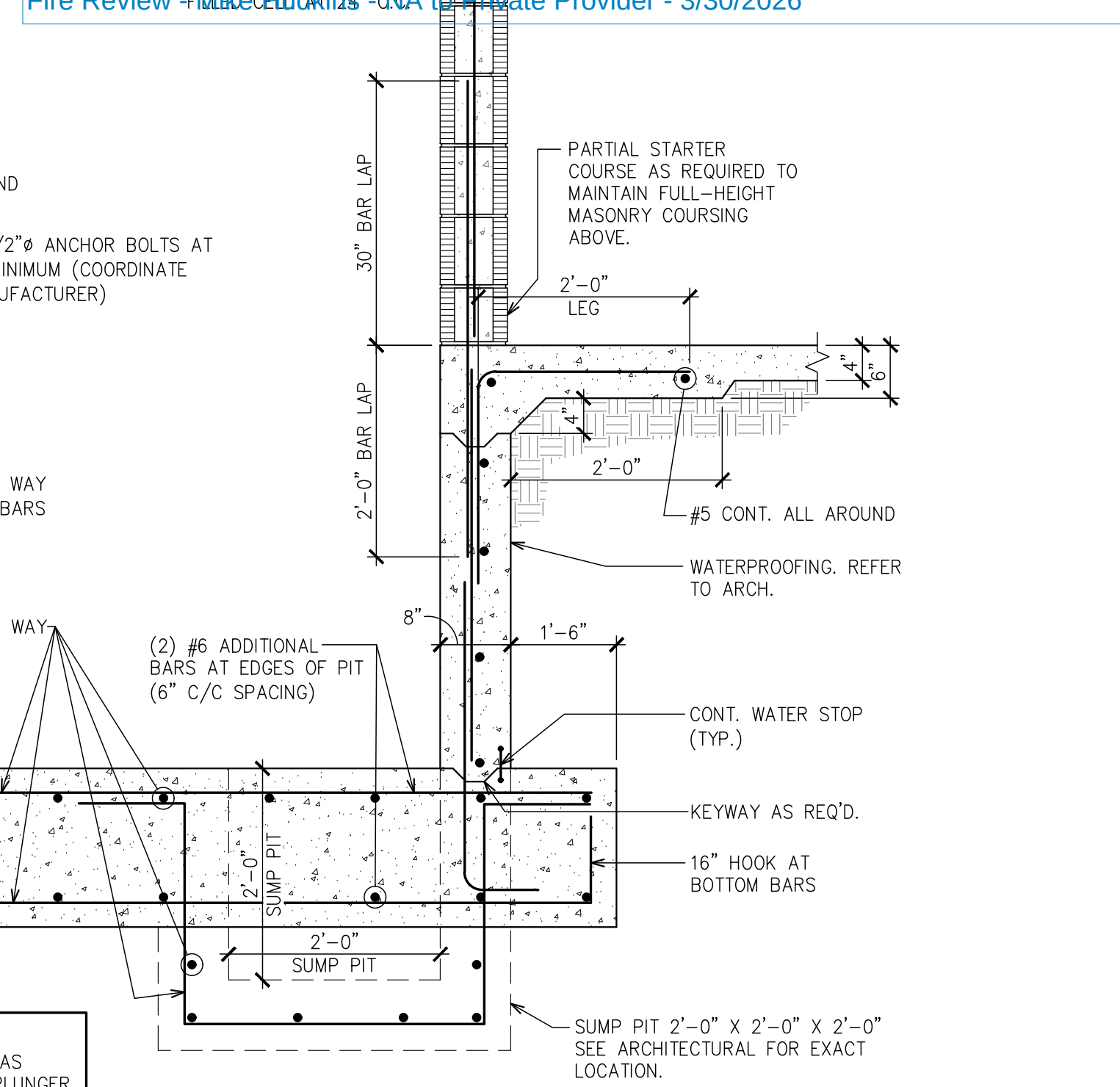
3 ELEVATOR PIT
SCALE: 3/4" = 1'-0"



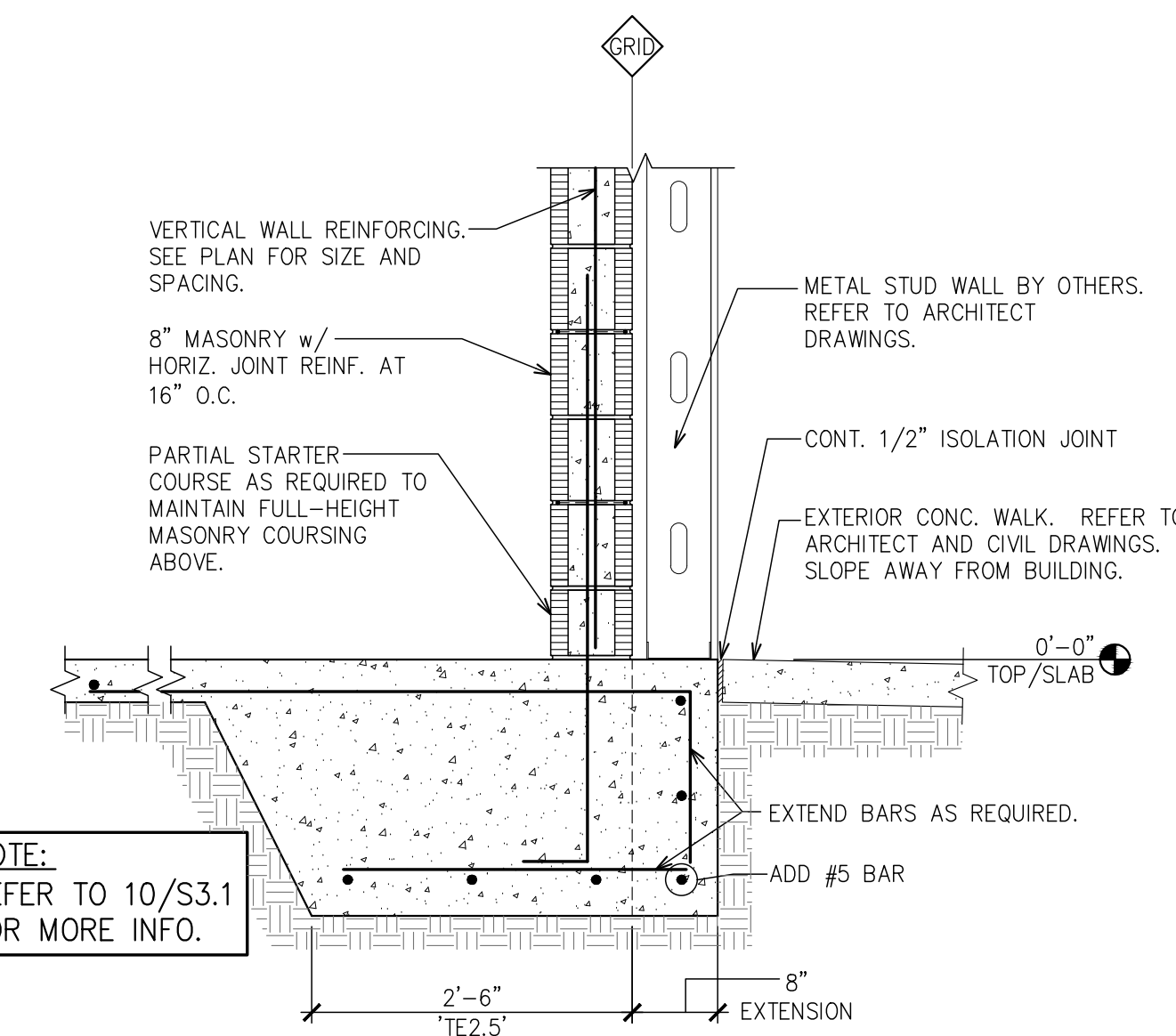
10 SECTION AT ROOF
SCALE: 3/4" = 1'-0"



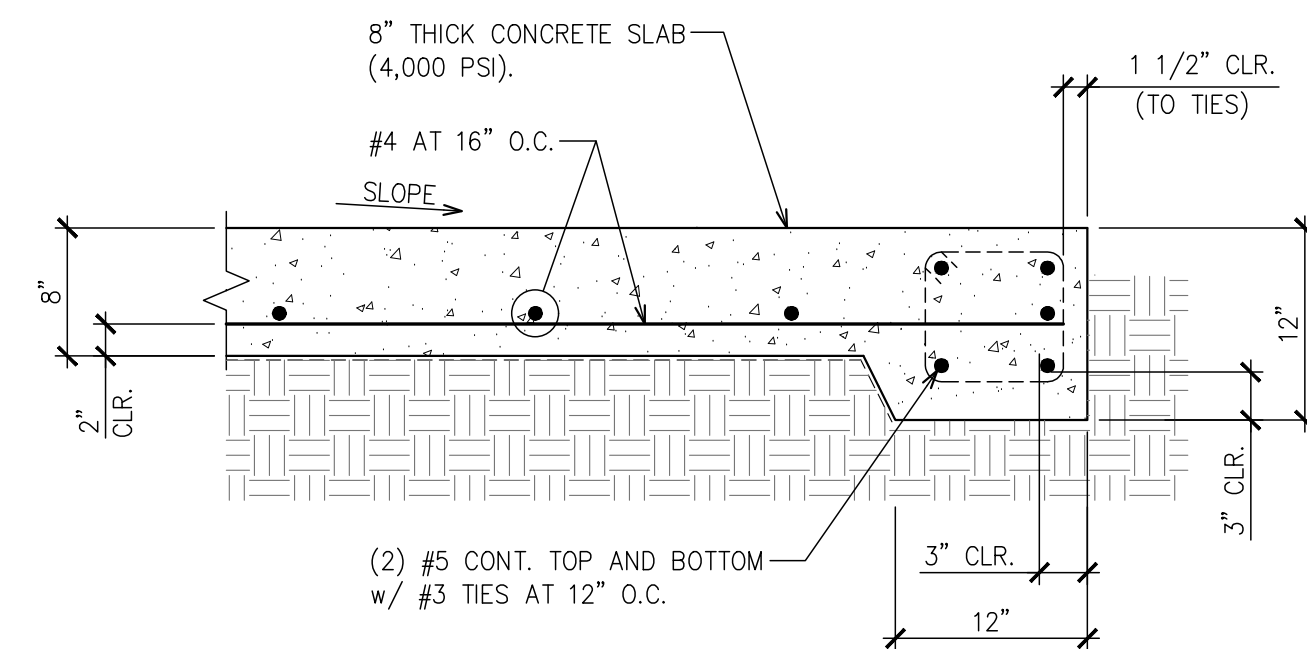
12 SECTION AT ELEVATOR ROOF
SCALE: 3/4" = 1'-0"



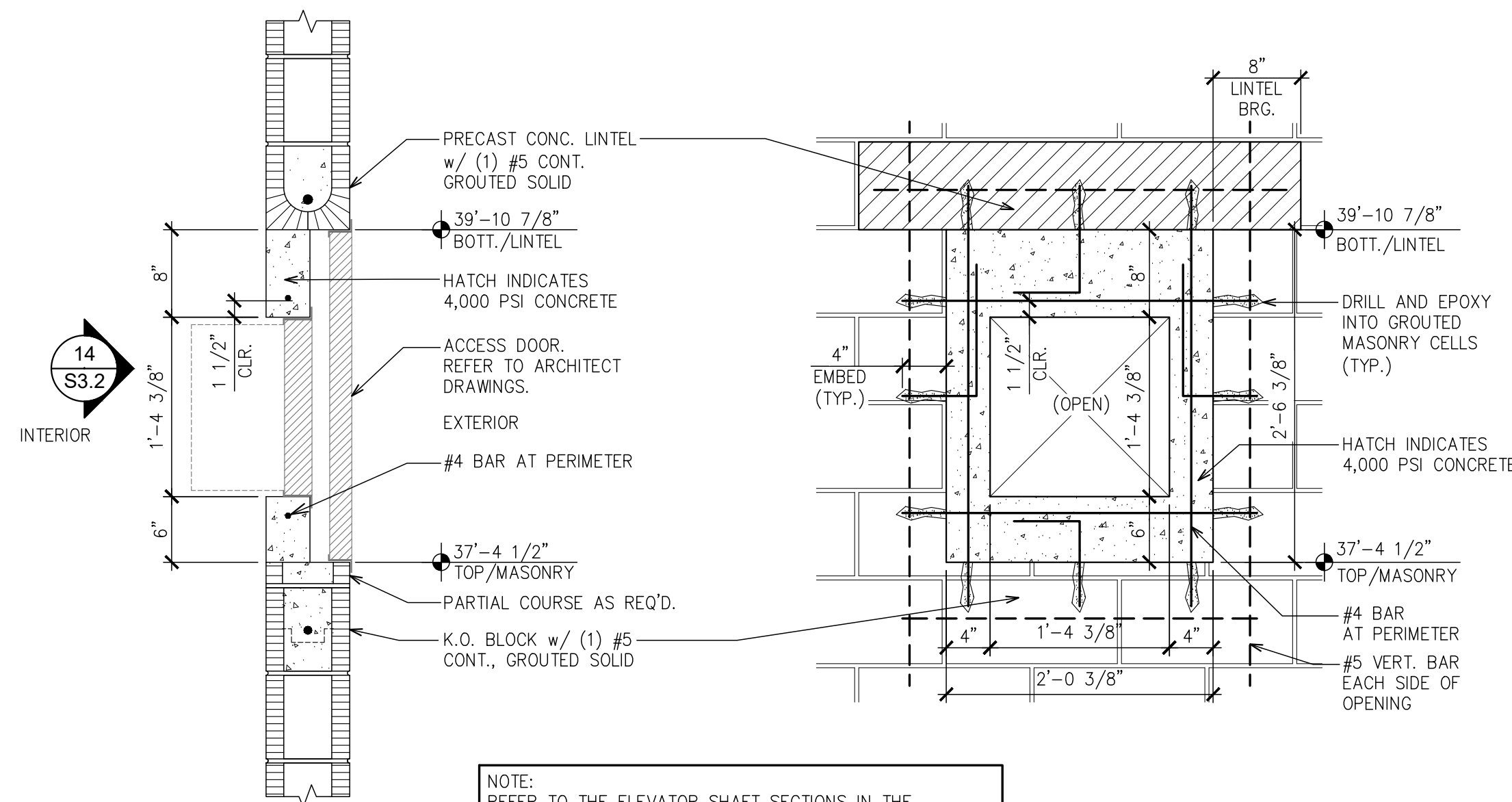
15 THICKENED SLAB AT MASONRY WALL w/ BUMP-OUT
SCALE: 3/4" = 1'-0"



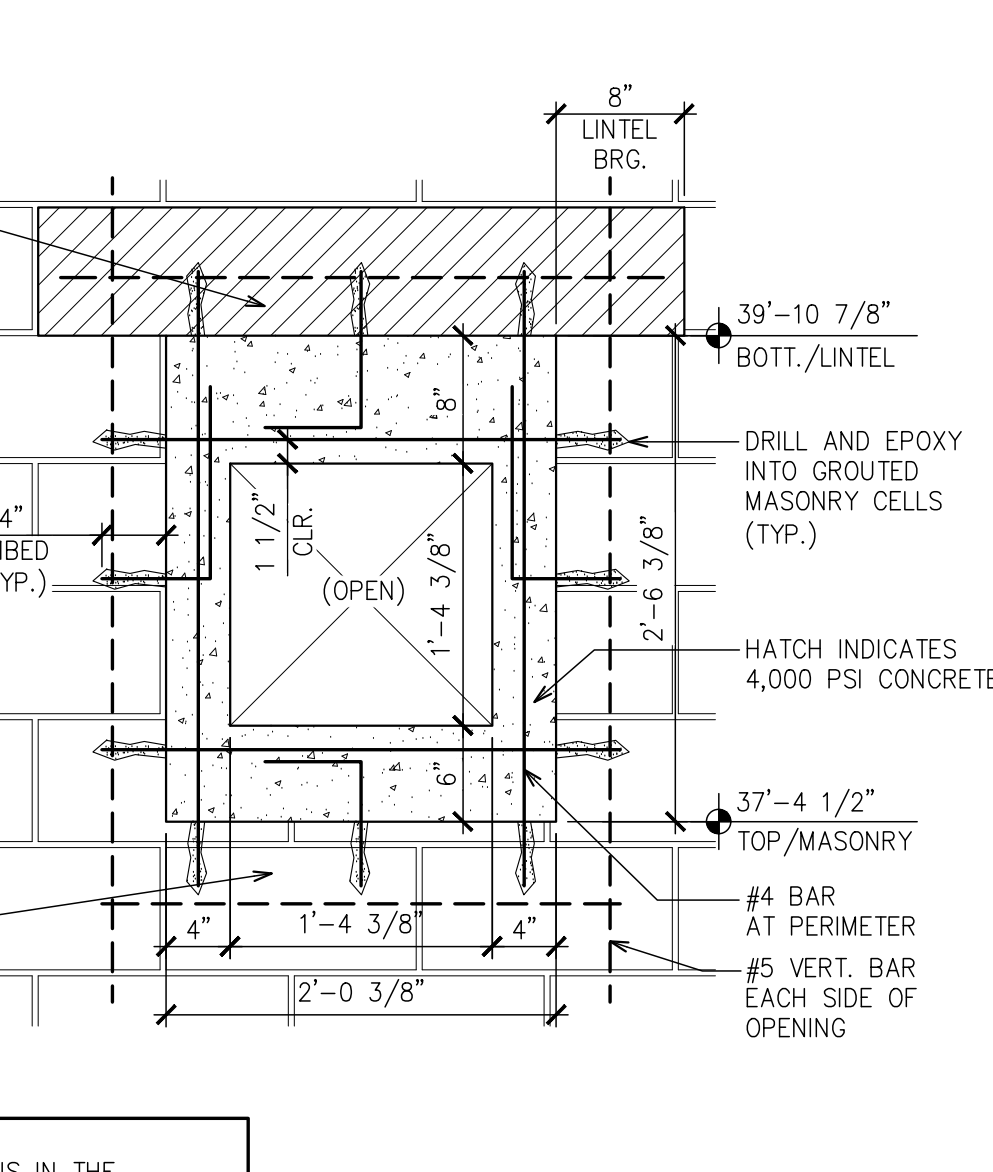
16 THICKENED SLAB EDGE
SCALE: 1" = 1'-0"



17 SLAB SECTION
SCALE: 1" = 1'-0"



13 SECTION AT ELEVATOR WALL
SCALE: 1" = 1'-0"



14 ELEVATION AT ELEVATOR WALL
SCALE: 1" = 1'-0"

revision descriptions	
no.	date