GENERAL NOTES

1. ALL STRUCTURAL ITEMS FOR THIS PROJECT HAVE BEEN DESIGNED IN ACCORDANCE WITH APPROPRIATE PROVISIONS OF EACH OF THE FOLLOWING:

A. BUILDING CODE: 2023 FLORIDA BUILDING CODE, EIGHTH EDITION. B. STRUCTURAL STEEL: THE A.I.S.C. "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS ANSI/AISC 360." A.C.I. "BUILDING CODE REQUIREMENTS FOR C. CONCRETE: STRUCTURAL CONCRETE", ACI 318-19.

2. THE MASONRY STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE SPECIFICATIONS AND ALL OTHER DISCIPLINES DRAWINGS (INCLUDING ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL). IF THERE ARE QUESTIONS BETWEEN DRAWINGS, IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ARCHITECT AND/OR STRUCTURAL ENGINEER IN WRITING PRIOR TO PERFORMING WORK.

3. IN ANY CASE OF QUESTIONS BETWEEN THE NOTES, DETAILS, AND SPECIFICATIONS, THE MOST RIGID REQUIREMENTS SHALL GOVERN.

4. DETAILS DESIGNATED AS "TYPICAL" APPLY TO ALL AREAS OF SIMILAR CONDITIONS UNLESS OTHERWISE NOTED. 5. MECHANICAL/PLUMBING/ELECTRICAL OPENINGS SHALL BE COORDINATED BY CONTRACTOR. FINAL SIZES AND LOCATIONS TO BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL

6. CONTRACTOR IS RESPONSIBLE FOR AND SHALL VERIFY AND COORDINATE ALL DIMENSIONS. DETAILS, AND EXISTING CONDITIONS BEFORE PROCEEDING WITH WORK. ANY DISCREPANCIES SHALL BE BROUGHT IN WRITING

TO THE IMMEDIATE ATTENTION OF THE ARCHITECT AND/OR STRUCTURAL ENGINEER 7. CONTRACTOR SHALL FULLY BRACE AND OTHERWISE PROTECT ALL WORK IN PROGRESS UNTIL THE STRUCTURE

8. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR CONCRETE AND STRUCTURAL STEEL A MINIMUM OF TWO (2) WEEKS PRIOR TO THE START OF FABRICATION.

9. THE OWNER SHALL ENGAGE AN INDEPENDENT TESTING AND INSPECTION AGENCY ACCEPTABLE TO THE ARCHITECT AND/OR STRUCTURAL ENGINEER TO INSPECT THE FOLLOWING:

B. STEEL - HIGH STRENGTH BOLTED CONNECTIONS AND WELDED CONNECTIONS IN THE SHOP AND FIELD

C. CONCRETE - INSPECT REINFORCING PLACEMENT, INSPECT AND TEST CONCRETE QUALITY D. MASONRY - IN ACCORDANCE WITH TMS402/602 LEVEL B QUALITY ASSURANCE.

CONTRACTOR SHALL COORDINATE INSPECTIONS REQUIRED FOR THE ABOVE AGENCIES. ALL REQUESTS FOR SUBSTITUTIONS OF MATERIALS OR DETAILS SHOWN IN THE CONTRACT DOCUMENTS SHALL BE SUBMITTED FOR APPROVAL DURING THE BIDDING PERIOD. ONCE BIDS ARE ACCEPTED, PROPOSED SUBSTITUTIONS WILL BE CONSIDERED ONLY WHEN THEY ARE OFFICIALLY SUBMITTED WITH AN IDENTIFIED SAVINGS TO BE DEDUCTED FROM THE CONTRACT.

DESIGN CRITERIA

THERMAL FACTOR, Ct = 1.1

1. SEE PLANS FOR ROOF DEAD AND LIVE LOADS

2. SNOW LOADS: GROUND SNOW LOAD, Pg = 0 PSF FLAT ROOF SNOW LOAD, Pf = 0 PSF MINIMUM SNOW LOAD USED FOR DESIGN = 0 PSF SNOW EXPOSURE FACTOR, Ce = 1.0

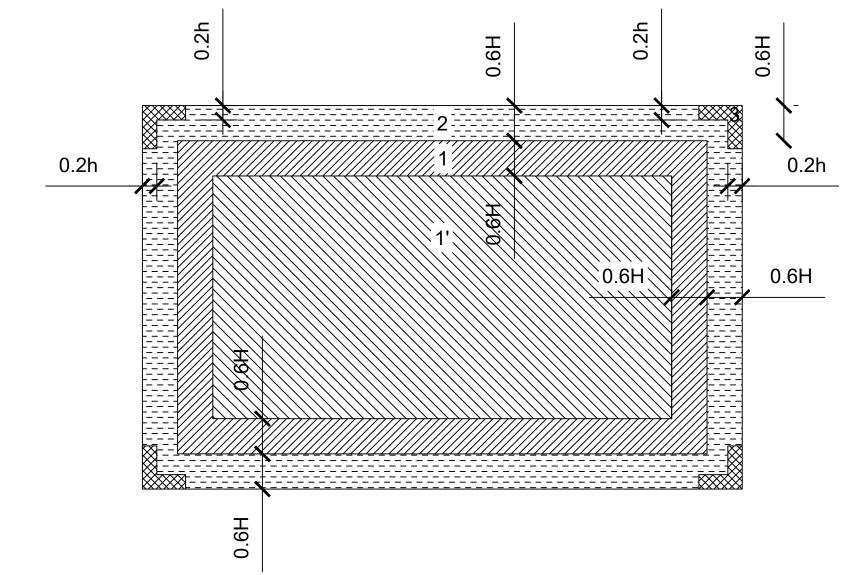
SNOW LOAD IMPORTANCE FACTOR, I = 1.0

WIND LOADS:

BASIC WIND SPEED VT = NOT APPLICABLE FOR RISK CATEGORY II

RISK CATEGORY - II WIND EXPOSURE - C INTERNAL PRESSURE COEFFICIENT GCpi = ±0.18 COMPONENTS AND CLADDING PRESSURE: h = 15.25 FT (Mean Roof Height = 13'-5") ROOF 10 SQFt EFFECTIVE WIND AREA:

ZONE 1 P = +17.1 PSF, -38.5 PSF \square P = +17.1 PSF, -67.1 PSF ZONE ZONE 2 P = +17.1 PSF, -88.5 PSF ZONE 3 ☑ P = +17.1 PSF, -120.6 PSF P = +33.7 PSF, -36.9 PSFCORNERS P = +33.7 PSF, -41.6 PSF



FOUNDATION CONSTRUCTION NOTES

1. FOUNDATIONS FOR THIS PROJECT CONSIST OF SPREAD AND STRIP FOOTINGS DESIGNED TO BEAR ON UNDISTURBED VIRGIN SOIL OR STRUCTURAL COMPACTED FILL PLACED OVER UNDISTURBED VIRGIN SOIL HAVING APRESUMED BEARING CAPACITY OF 2,500 POUNDS PER SQUARE FOOT PER UES, ORLANDO, FLORIDA, PREPARED JUNE 14. 2024. UES REPORT NO. 2092572. CAPACITY OF FOOTING SUBGRADE SHALL BE INSPECTED AND VERIFIED BY A GEOTECHNICAL ENGINEER LICENSED IN THE STATE OF FLORIDA PRIOR

2. ELEVATIONS SHOWN ON THE DRAWINGS ARE TO THE BOTTOM OF FOUNDATIONS AND ARE MINIMUM DEPTHS. IF BEARING MATERIALS AT THE SPECIFIED ELEVATIONS ARE FOUND TO HAVE LOWER BEARING CAPACITIES THAN REQUIRED, MATERIALS SHALL BE REMOVED AND REPLACED WITH LEAN CONCRETE. 3. DESIGN, FURNISH, AND PLACE ALL TEMPORARY OR PERMANENT SUPPORTS, WHETHER SHORING, SHEETING, OR BRACING, SO THAT NO HORIZONTAL MOVEMENT OR VERTICAL SETTLEMENT OCCURS TO EXISTING STRUCTURES, STREETS, OR UTILITIES ADJACENT TO PROJECT SITE.

4. CONTROL SURFACE AND SUBSURFACE WATER DURING CONSTRUCTION SO THAT FOUNDATION WORK WILL BE PERFORMED IN DRY CONDITIONS AND ON UNDISTURBED SOIL

EXCAVATIONS FOR FOOTINGS SHALL BE FINISHED BY HAND. 6. FOUNDATION CONCRETE SHALL NOT BE PLACED IN WATER OR ON FROZEN GROUND. ". ALL STRUCTURAL COMPACTED FILL SHALL CONSIST OF CLEAN, WELL- GRADED GRANULAR MATERIAL

CONTAINING NO MORE THAN 12% NOR LESS THAN 5% BY WEIGHT OF MATERIAL PASSING THE #200 SIEVE. MATERIAL SHALL BE FREE FROM CLAY LUMPS, ORGANICS AND DELETERIOUS MATERIAL. EXISTING ON SITE FILL/EXCAVATED MATERIAL MAY BE USED FOR BACKFILLING PROVIDED IT IS INSPECTED BY THE SOILS ENGINEER AND MEETS THE CRITERIA ABOVE.

ALL STRUCTURAL COMPACTED FILL AND BACKFILL IN BUILDING AND WITHIN 5'-0" OF BUILDING SHALL BE PLACED IN 12" MAXIMUM LOOSE LIFTS AND COMPACTED WITH A HEAVY VIBRATORY COMPACTOR TO AT LEAST 95% OF THE MAXIMUM MODIFIED PROCTOR DENSITY AS PER ASTM D-1557 TO A DEPTH OF TWO

FEET UNDER THE SUPERVISION OF A LICENSED SOILS ENGINEER. 9. THE BUILDING AND THE ADJASCENT AREAS SHOULD BE PROOF ROLLED PRIOR TO PLACING FILL.THE ROLLER SHOULD HAVE DRUM WEIGHT OF AT LEAST 10 TONS. PROOF ROLLING SHOULD CONSIST OF A MINIMUM OF TEN OVERLAPPING COVERAGES OF THE ROLLER IN A CRISS-CROSS PATTERN WITH MAXIMUM

TRAVEL SPEED OF 2 FT PER SECOND. 10. ALL FILL AND BACKFILL SHALL BE PLACED ON VIRGIN SOIL THAT DOES NOT CONTAIN ANY ORGANIC MATERIAL. STRIP ALL TOP SOIL. PRIOR TO PLACING FILL OR BACKFILL, PROOF-COMPACT SUBGRADE WITH A HEAVY VIBRATORY COMPACTOR TO AT LEAST 95% OF THE MAXIMUM MODIFIED PROCTOR DENSITY AS

PER ASTM D-1557 TO A DEPTH OF TWO FEET UNDER THE SUPERVISION OF A LICENSED SOILS ENGINEER. 11. FOUNDATION ELEMENTS SHALL BE CONSIDERED CENTERED UNDER COLUMN CENTERLINES UNLESS OTHERWISE NOTED.

12. NO FOOTINGS SHALL BE PLACED ABOVE 1 VERTICAL ON 2 HORIZONTAL SLOPE EXTENDED FROM THE CLOSEST EDGE OF ANY UNDISTURBED SOIL OR OTHER FOUNDATION STRUCTURE. BOTTOM OF EXTERIOR FOOTINGS SHALL NOT BE LESS THAN 1'-6" BELOW FINISHED GRADE 13. WHERE SOFT AREAS ARE ENCOUNTERED, THE AREA SHALL BE UNDERCUT AS DIRECTED BY THE SOILS

ENGINEER AND REPLACED WITH COMPACTED FILL OR LEAN CONCRETE. 14. WHERE SHALLOW ROCK IS ENCOUNTERED AT FOOTING BEARING ELEVATION, THE ROCK SHALL BE REMOVED A MINIMUM OF 12" BELOW THE BOTTOM OF FOOTING AND REPLACED WITH CONTROLLED

CONCRETE CONSTRUCTION NOTES

1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE A.C.I. "BUILDING

CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (ACI 318). 2. CONCRETE DESIGN MIXES SHALL CONFORM WITH ASTM C94, AND HAVE PROPERTIES AS INDICATED BELOW:

FOUNDATIONS, WALLS, AND PIERS:

SLABS-ON-GRADE

MAX. W/C RATIO: 0.50 AIR CONTENT: 5% ± 1 1/2%

f'c=4,000 psi AT 28 DAYS

f'c=4,000 psi AT 28 DAYS MAX. W/C RATIO: 0.50 AIR CONTENT: 3% MAX

SLUMP SHALL BE LIMITED TO 4 INCHES. FOR CONCRETE WITH HRWR (SUPER-P), SLUMP SHALL BE LIMITED TO 2-4 INCHES PRIOR TO ADDITION OF HRWR, AND A MAXIMUM OF 8 INCHES AFTER ADDITION OF HRWR. 4. ADMIXTURES USED IN CONCRETE SHALL BE AS ALLOWED BY THE SPECIFICATIONS AND ONLY WITH LABORATORY DESIGN MIX APPROVAL. ALL ADMIXTURES SHALL CONTAIN NO MORE CHLORIDE IONS THAN

ARE PRESENT IN MUNICIPAL DRINKING WATER. 5. CONCRETE MATERIALS SHALL BE AS INDICATED BELOW:

A. PORTLAND CEMENT: ASTM C150, TYPE I/II ASTM C618 - 15% - 25% OF CEMENTITOUS MATERIAL

C. NORMAL-WEIGHT AGGREGATES: ASTM C33, 3/4" MAXIMUM **ASTM C94 AND POTABLE**

ADMIXTURES SHALL BE AS INDICATED BELOW:

A. AIR-ENTRAINING ADMIXTURE: ASTM C260 WATER REDUCING ADMIXTURE: ASTM C494, TYPE A

C. WATER REDUCING AND RETARDING ADMIXTURE: ASTM C494 TYPE D D. WATER-REDUCING, ACCELERATING ADMIXTURE: ASTM C494 TYPE E

HIGH RANGE WATER REDUCING ADMIXTURE (SUPER-PLASTICIZER): ASTM C494, TYPE F HIGH RANGE WATER REDUCING AND RETARDING ADMIXTURE: ASTM C494 TYPE G 7. EPOXY JOINT FILLER SHALL BE A TWO-COMPONENT SEMI RIGID RESIN, 100% SOLIDS, AND HAVE A MINIMUM

SHORE A HARDNESS OF 80 WHEN MEASURED IN ACCORDANCE WITH ASTM D 2240. 8. ALL REINFORCING STEEL SHALL BE INTERMEDIATE GRADE, NEW BILLET STEEL, DEFORMED BARS, CONFORMING TO ASTM A-615, GRADE 60. ALL BARS SHALL BE SECURELY SUPPORTED AND WIRED IN PLACE PRIOR TO CONCRETE PLACEMENT.

9. ALL WELDED WIRE FABRIC (W.W.F.) SHALL CONFORM TO ASTM A-185. 10. FIBER REINFORCING SHALL BE MONOFILAMENT POLYPROPYLENE FIBERS FOR SECONDARY

REINFORCEMENT, ASTM C1116, TYPE III. VAPOR RETARDER SHALL CONFORM TO ASTM E1745, CLASS C, WITH MINIMUM 10 MIL. THICKNESS 12. REINFORCING STEEL SHOWN IN SECTIONS ARE SCHEMATIC INDICATIONS THAT REINFORCING EXISTS. SEE

SECTION NOTES, SCHEDULES, PLAN NOTES, ETC. FOR ACTUAL REINFORCING REQUIRED. 13. UNLESS OTHERWISE NOTED, ALL BARS MARKED CONT. SHALL BE SPLICED AT ALL LAP POINTS AND CORNERS AND DEVELOPED AT NON-CONTINUOUS ENDS AS TYPICAL DETAILS. SPLICE CONTINUOUS TOP BARS AT CENTER BETWEEN SUPPORTS AND SPLICE CONTINUOUS BOTTOM BARS AT SUPPORTS. WELDED WIRE FABRIC SHALL BE LAPPED 12 INCHES OR TWO SPACES, WHICHEVER IS LONGER. SHEETS SHALL BE WIRED TOGETHER.

CONCRETE COVER FOR REINFORCING BARS SHALL BE AS SHOWN IN DETAILS. 15. SLAB ON GRADE SHALL BE WET CURED WITH A MOISTURE RETAINING COVER CONFORMING TO ASTM C171 WITH SIDES AND ENDS LAPPED AT LEAST 12", SEALED WITH WATERPROOF TAPE. MOISTURE RETAINING

COVER SHALL REMAIN IN PLACE FOR A MINIMUM OF 7 DAYS. AT OPENINGS IN CONCRETE WALLS, PROVIDE ADDED REINFORCEMENT IN ACCORDANCE WITH THE

TYPICAL DETAILS UNLESS OTHERWISE NOTED. REINFORCEMENT SHALL NOT BE WELDED OR HEATED IN ANY WAY.

18. SLEEVES, MECHANICAL OPENINGS, CONDUITS, PIPES, RECESSES, DEPRESSIONS, CURBS, AND ALL EMBEDDED ITEMS SHALL BE PROVIDED FOR AS SHOWN ON THE ARCHITECTURAL AND MECHANICAL DRAWINGS AND AS REQUIRED BY EQUIPMENT MANUFACTURERS. MINIMUM CONCRETE BETWEEN SLEEVES SHALL BE 6". INSTALLATION OF THESE ITEMS SHALL BE COORDINATED WITH SHOP DRAWINGS OF TRADES REQUIRING THESE ITEMS

 SET FORMS TO FOLLOW SLOPES AND GRADES DEFINED ON PLAN, KEEPING MEMBER DEPTHS CONSTANT AS DETAILED OR SCHEDULED, UNLESS NOTED OTHERWISE. SLOPE UNIFORMLY BETWEEN ELEVATIONS

20. REINFORCING, INCLUDING WELDED WIRE FABRIC, FOR SLABS ON GRADE AND FOOTINGS SHALL BE SUPPORTED ON SOLID CONCRETE BLOCKS AT 5'-0" ON CENTER MAXIMUM EACH WAY. 21. PROVIDE SHEAR KEY IN ALL CONSTRUCTION JOINTS IN WALLS.

22. ALL CONSTRUCTION JOINTS SHALL BE THOROUGHLY CLEANED AND TREATED WITH THE SPECIFIED BONDING COMPOUND JUST BEFORE PLACING NEW CONCRETE.

23. SEE ARCHITECTURAL DRAWINGS FOR DETAILS OF WEEPHOLES, FLASHING REGLETS, FASCIA DETAILS, 24. UNDER NO CIRCUMSTANCES SHALL CONCRETE BE PUMPED THROUGH ALUMINUM PIPES. CONCRETE

SHALL NOT BE PLACED IN CONTACT WITH ALUMINUM, ALUMINUM MIXING DRUMS, TRUCK MIXERS,

BUGGLES, CHUTES, CONVEYORS, TREMIE PIPES, AND OTHER EQUIPMENT MADE OF ALUMINUM SHALL NOT 25. WHERE CONCRETE ABUTS MASONRY, PROVIDE VERTICAL METAL SLOTS TO RECEIVE GALVANIZED METAL

DOVETAIL ANCHORS. SLOTS SHALL BE SPACED AT 24" ON CENTER.

26. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR FABRICATION, BENDING, AND PLACEMENT OF CONCRETE REINFORCEMENT. SHOP DRAWINGS SHALL COMPLY WITH ACI 315 "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES".

CONCRETE PLACEMENT. CONTRACTOR SHALL NOTIFY ENGINEER OF ANY DISCREPANCY FROM ACI 315 "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES". 28. COLD OR HOT WEATHER CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH THE APPLICABLE

CODE REQUIREMENTS. 29. INSTALLATION OF POST-INSTALLED ADHESIVE ANCHORS MUST BE INTO CONCRETE THAT HAS A MINIMUM AGE OF 21 DAYS AT THE TIME OF INSTALLATION.

STEEL CONSTRUCTION NOTES

- 1. ALL STRUCTURAL STEEL WORK SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST AISC CODE OF STANDARD PRACTICE. STRUCTURAL STEEL SHALL BE NEW. CLEAN. AND STRAIGHT. AND SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:
- A. WIDE FLANGE ROLLED SHAPES: ASTM A992, GRADE 50 (Fy = 50 KSI). B. PLATES, ANGLES, BARS, CHANNELS, AND S SHAPES: ASTM A36 (Fy = 36 KSI).
- C. RECTANGULAR HSS: ASTM A500, GRADE B (Fy = 46 KSI).
- D. ROUND HSS: ASTM A500, GRADE B (Fy = 42 KSI).
- E. PIPE: ASTM A53, TYPE E OF S, GRADE B (Fy = 35 KSI). 2. ALL ANCHOR RODS, UNLESS OTHERWISE NOTED, SHALL BE ASTM F1554, GRADE 36
- 3. ALL BOLTED CONNECTIONS, UNLESS OTHERWISE NOTED, SHALL BE 3/4"Ø A325 HIGH STRENGTH BOLTS, IN BEARING TYPE CONNECTIONS AND SHALL BE PROVIDED WITH HARDENED WASHERS UNDER THE TURNED **ELEMENT (NUT OR BOLT THREAD)**
- 4. ALL STRUCTURAL STEEL SHALL BE PAINTED WITH ONE COAT OF SHOP PRIMER, EXCEPT WHERE FIELD WELDING OR SLIP CRITICAL BOLTING IS TO BE DONE, STEEL TO RECEIVE SPRAY-ON FIREPROOFING, STEEL
- TO BE EMBEDDED IN CONCRETE, AND STEEL TO BE HOT-DIPPED GALVANIZED. 5. STRUCTURAL STEEL EXPOSED TO WEATHER, EXCESSIVE MOISTURE, OR CORROSIVE ENVIRONMENT AND AS INDICATED ON CONSTRUCTION DOCUMENTS, SHALL BE HOT-DIPPED GALVANIZED, MEETING REQUIREMENTS OF ASTM A123 AND A153 AS APPLICABLE.
- 6. INSTALLATION AND TIGHTENING OF ALL HIGH STRENGTH BOLTS SHALL CONFORM TO THE AISC
- "SPECIFICATION FOR THE STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS." 7. CONNECTIONS MAY BE WELDED OR HIGH STRENGTH BOLTED. ALL CONNECTIONS SHALL CONFORM TO THE TYPICAL CONNECTION DETAILS SHOWN ON THE DRAWINGS.
- 8. ALL WELDING SHALL CONFORM TO THE AMERICAN WELDING SOCIETY STRUCTURAL WELDING CODE STEEL (AWS D1.1) AND SHALL BE DONE BY A.W.S. QUALIFIED WELDERS USING E70XX ELECTRODES. ALL CONTACT SURFACES WITHIN HIGH STRENGTH BOLTED CONNECTIONS AND WELDING AREAS SHALL BE FREE OF OIL, PAINT, AND LACQUER.
- THE CONTRACTOR SHALL COORDINATE THE SIZE AND LOCATION OF ALL ROOF OPENINGS SHOWN ON THE STRUCTURAL, ARCHITECTURAL AND/OR MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS. ANY STEEL WHICH IS NOT SHOWN ON THE CONTRACT DRAWINGS AS FURNISHED BY THE STRUCTURAL STEEL CONTRACTOR AND WHICH IS REQUIRED BY THE MECHANICAL, PLUMBING, AND ELECTRICAL TRADES FOR OPENINGS AND/OR TO SUPPORT THEIR WORK SHALL BE SUPPLIED AND INSTALLED BY THE CONTRACTOR REQUIRING SUCH STEEL, UNLESS OTHERWISE NOTED.
- 11. CUTS, HOLES, COPING, ETC. REQUIRED IN STRUCTURAL STEEL MEMBERS FOR THE WORK OF OTHER TRADES SHALL BE SHOWN ON THE STRUCTURAL STEEL SHOP DRAWINGS AND BE MADE IN THE SHOP. HOLES SHALL BE REINFORCED AND APPROVED BY THE STRUCTURAL ENGINEER.
- 12. BURNING OF HOLES, CUTS, ETC. IN STRUCTURAL STEEL MEMBERS IN THE FIELD WILL NOT BE PERMITTED, EXCEPT WITH THE SPECIFIC WRITTEN PERMISSION OF THE ENGINEER
- 13. FOR MISCELLANEOUS STEEL, SEE ARCHITECTURAL DRAWINGS. SUBMIT ALL STRUCTURAL STEEL SHOP DRAWINGS FOR REVIEW PRIOR TO ANY FABRICATION A MINIMUM OF TWO (2) WEEKS PRIOR TO THE START OF FABRICATION. STEEL SHOP DRAWINGS SHALL INCLUDE CALCULATIONS FOR DESIGNS OF ALL CONNECTIONS, SIGNED AND SEALED BY AN ENGINEER LICENSED IN

METAL DECK CONSTRUCTION NOTES

- 1. ALL STEEL DECK SHALL CONFORM TO THE LATEST EDITION OF THE FOLLOWING CODES AND SPECIFICATIONS:
- A. AMERICAN IRON AND STEEL INSTITUTE (AISI) SPECIFICATION FOR DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS, LATEST EDITION.
- B. AMERICAN WELDING SOCIETY (AWS) D1.3 STRUCTURAL WELDING CODE/ SHEET STEEL.
- C. STEEL DECK INSTITUTE (S.D.I.) DESIGN MANUAL, LATEST EDITION.
- D. AMERICAN INSTITUTE OF STEEL CONSTRUCTION SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
- 2. STEEL ROOF DECK SHALL BE AS INDICATED ON PLANS. MATERIAL SHALL CONFORM TO ASTM A653, GRADE 33, WITH MINIMUM YIELD STRENGTH OF 33,000 PSI. 3. ALL STEEL ROOF DECK SHALL BE GALVANIZED AND CONFORM TO ASTM A924 WITH A MINIMUM COATING OF
- G60 AS DEFINED IN ASTM A653. 4. CONSTRUCTION LOADS SHALL BE PLACED SUCH THAT THEY DO NOT EXCEED DESIGN LIVE LOADS. ALL
- STEEL ROOF DECK SHALL BE FULLY INSTALLED AND CAPABLE OF SUPPORTING LOADS SHOWN ON PLAN AND FOR THE INDICATED SPANS AND ALL CONSTRUCTION LOADS PRIOR TO LOADING. 5. ALL STEEL ROOF DECK SHALL BE CONTINUOUS OVER FOUR OR MORE STRUCTURAL SUPPORTS, (i.e.: DECK
- SHOULD BE DETAILED FOR A THREE-SPAN CONTINUOUS CONDITION). 6. ANCHOR STEEL ROOF AND FLOOR DECK AT ENDS AND AT ALL INTERMEDIATE SUPPORTING MEMBERS WITH 5/8" DIAMETER PUDDLE WELDS AT 12" O.C. AT THE BOTTOM OF THE RIB ACROSS THE WIDTH OF THE
- DECK UNIT UNLESS NOTED OTHERWISE. 3" ROOF DECK SHALL BE FASTENED AT 8" O.C. 7. SIDE LAPS OF STEEL ROOF DECK AND FLOOR DECK SHALL BE FASTENED AT 18" O.C. MAXIMUM WITH #10
- SELF-DRILLING SCREWS, 5/8" DIAMETER PUDDLE WELDS, OR 1 INCH LONG FILLET WELD. 8. ALL ELECTRICAL, MECHANICAL AND PIPING EQUIPMENT, PIPING, DUCTWORK, ETC. SHALL BE HUNG FROM STEEL BEAMS ONLY. WORK SHALL NOT BE HUNG FROM METAL DECK.
- 9. CUT OUT METAL DECK WHERE BOLT OR PLATE PROJECTIONS INTERFERE WITH METAL DECK BEARING.

OPEN WEB STEEL JOIST CONSTRUCTION NOTES

- 1. STEEL JOISTS SHALL CONFORM TO THE LATEST STEEL JOIST INSTITUTE (S.J.I.) STANDARD
- SPECIFICATIONS FOR OPEN WEB JOISTS. 2. JOISTS SHALL BEAR A MINIMUM OF 4" ON CONCRETE AND MASONRY WALLS, AND A MINIMUM OF 2 1/2" ON
- STEEL BEAMS, UNLESS OTHERWISE NOTED ON DRAWINGS. 3. JOISTS FRAMING TO COLUMNS SHALL HAVE THEIR BOTTOM CHORDS EXTENDED TO COLUMN AND SHALL HAVE THEIR TOP AND BOTTOM CHORDS BOLTED TO COLUMN. DO NOT CONNECT BOTTOM CHORDS OD
- JOISTS TO COLUMNS UNTIL THE ROOF DEAD LOAD HAS BEEN APPLIED. 4. DIAGONAL AND HORIZONTAL BRIDGING FOR ALL JOISTS SHALL BE AS SHOWN ON PLAN AND BE FURNISHED BY THE STEEL JOIST SUPPLIER. INSTALLATION SHALL MEET THE DESIGN AND SPACING REQUIREMENTS OF THE STEEL JOIST INSTITUTES STANDARD SPECIFICATIONS, UNLESS OTHERWISE NOTED ON DRAWINGS.
- PROVIDE ADDITIONAL UPLIFT BRIDGING AS REQUIRED BY S.J.I. ALL CLIPS AND CONNECTION PLATES SHALL BE SHOP-WELDED TO JOISTS. 6. FIELD WELDING OF ANY MISCELLANEOUS ITEMS TO JOISTS IS NOT PERMITTED, EXCEPT IF SUCH ITEMS ARE SPECIFICALLY SHOWN ON STRUCTURAL DRAWINGS.
- JOISTS SHALL NOT BE FIELD SPLICED. 8. JOISTS SHALL HAVE POSITIVE CAMBER AS RECOMMENDED BY S.J.I FOR GIVEN SPANS.
- MAXIMUM DEFLECTION OF STEEL JOISTS DUE TO LIVE LOAD = L/360. 10. STEEL JOISTS SHALL RECEIVE SHOP COAT OF PRIMER PAINT, UNLESS OTHERWISE NOTED ON DRAWINGS...
- 11. JOIST MANUFACTURER SHALL SUBMIT, WITH THE SHOP DRAWINGS, A CATALOG USED FOR THE MANUFACTURE OF JOISTS INDICATING THE LOADING TABLES AND SIZES OF ALL MEMBERS USED. 12. ALL JOISTS SHALL BE DESIGNED FOR A NET UPLIFT LOAD PER COMPONENTS AND CLADDING PRESSURES. THE JOIST MANUFACTURER SHALL SUBMIT, WITH THE SHOP DRAWINGS, CERTIFICATION THAT THE CHORD
- AND WEB MEMBERS HAVE BEEN INVESTIGATED FOR REVERSE STRESSES DUE TO ALL UPLIFT FORCES. 13. ALL CONCENTRATED LOADS AT EITHER TOP OR BOTTOM CHORD MUST BE APPLIED AT PANEL POINTS, OR FIELD STRUT MEMBER MUST BE UTILIZED AS SHOWN IN THE TYPICAL DETAILS.

SHOP DRAWING SUBMITALS

- GENERAL CONTRACTOR SHALL SUBMIT SHOP DRAWINGS TO THE ENGINEER FOR REVIEW OF ALL REQUIRED INFORMATION AS INDICATED IN THE DRAWINGS AND SPECIFICATIONS, PRIOR TO ANY
- 2. NO PORTION OF THE STRUCTURAL DRAWINGS SHALL BE REPRODUCED FOR USE AS SHOP DRAWINGS.
- ALL DIMENSIONS SHALL BE COORDINATED BY THE CONTRACTOR AND/OR THE DETAILER.
- 4. DETAILER SHALL USE THE SAME GRID IDENTIFICATIONS AS THOSE SHOWN ON THE CONTRACT DRAWINGS 5. ALL SHOP DRAWINGS SHALL BE SUBMITTED PRIOR TO PROCEEDING WITH ANY ASSOCIATED WORK AND SHALL ALLOW FOR SUFFICIENT REVIEW TIME. SUBMIT A MINIMUM OF TWO (2) WEEKS PRIOR TO THE START
- 6. SHOP DRAWINGS SHALL BE SUBMITTED WITH CONTRACTOR'S STAMP OF APPROVAL, CERTIFYING THE CONTRACTOR HAS COORDINATED AND VERIFIED ALL DIMENSIONS, MATERIALS, AND ANY ADDITIONAL INFORMATION AFFECTING STRUCTURAL WORK. THE CONTRACTOR'S REVIEW INCLUDES BUT IS NOT LIMITED TO COORDINATION AND VERIFICATION OF ACTUAL FIELD CONDITIONS, DIMENSIONS, ELEVATIONS,
- AND SUPPORTS AND OPENINGS FOR ACTUAL EQUIPMENT PURCHASED. SHOP DRAWINGS NOT COMPLYING WITH THE ABOVE SHALL BE RETURNED FOR CORRECTION WITHOUT
- 8. RESUBMITTED SHOP DRAWINGS SHALL INCLUDE ALL CHANGES ON THE DRAWINGS CLOUDED AND
- MARKED WITH REVISION TAG NUMBER. 9. CONTRACTOR SHALL NOT PROCEED WITH ANY WORK OR FABRICATION UNTIL THE SHOP DRAWINGS HAVE BEEN REVIEWED AND APPROVED BY THE ENGINEER.

MASONRY CONSTRUCTION NOTES

- 1. ALL CONCRETE MASONRY CONSTRUCTION, INCLUDING MATERIAL AND WORKMANSHIP SHALL CONFORM TO THE "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES"
- TMS 402-2016 AND "SPECIFICATION FOR MASONRY STRUCTURES" TMS 602-2016. 2. ALL CONCRETE MASONRY SHALL BE CONSTRUCTED OF HOLLOW LOAD BEARING UNITS
- CONFORMING TO ASTM C90. 3. MORTAR SHALL CONFORM TO ASTM C270, TYPE "M" OR "S".
- 4. GROUT FOR CORES, BOND BEAMS, AND WITHIN CONCRETE MASONRY UNITS AS INDICATED ON DRAWINGS, SHALL CONFORM TO ASTM C476 AND DEVELOP A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI. ALL CONCRETE MASONRY CORES CONTAINING REINFORCING SHALL BE FILLED SOLID.
- 5. ALL CONCRETE MASONRY UNITS SHALL BE HOLLOW LOAD BEARING UNITS CONFORMING
- TO ASTM C90, WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2,000 PSI. 6. THE NET AREA COMPRESSIVE STRENGTH OF MASONRY (fm) SHALL BE A 2,000 PSI.
- CONCRETE MASONRY UNITS SHALL BE IN RUNNING BOND PATTERN.
- 8. ALL CONCRETE MASONRY WALL SHALL HAVE FULL BED MORTAR. ALL CONCRETE MASONRY WALLS SHALL HAVE LADDER TYPE HORIZONTAL JOINT REINFORCEMENT WITH 3/16" SIDE RODS, SPACED AT 8 INCHES ON CENTER BELOW GRADE AND 16 INCHES ON CENTER ABOVE GRADE, UNLESS OTHERWISE NOTED ON DRAWINGS.
- 10. ALL REINFORCING WITHIN CONCRETE MASONRY WALLS SHALL CONFORM TO ASTM A615. GRADE 60. SPLICES AND DEVELOPMENT FOR DEFORMED BARS SHALL BE IN ACCORDANCE WITH THE FOLLOWING, UNLESS NOTED OTHERWISE:

PROVIDE PREFABRICATED CORNER AND TEE SECTIONS AT ALL WALL INTERSECTIONS.

BAR SIZE SPLICE/DEVELOPMENT LENGTH

AT ALL OPENINGS, CORNERS, AND WALL INTERSECTIONS

11. GROUT SHALL NOT BE POURED IN LIFTS GREATER THAN 5 FEET AT A TIME. 12. PROVIDE VERTICAL CONTROL JOINTS AT A MAXIMUM OF 20 FT ON CENTER. TERMINATE HORIZONTAL JOINT REINFORCING AT EITHER SIDE OF CONTROL JOINT. BOND BEAM REINFORCING SHALL BE CONTINUOUS THROUGH CONTROL JOINT.

13. UNLESS OTHERWISE NOTED ON DRAWINGS, PROVIDE ADDITIONAL #5 MINIMUM VERTICAL

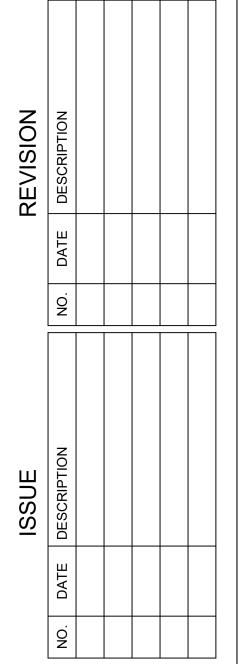
PRELIMINARY

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HITECT AND THE LEARNING EXPERIENCE FOR APPROVAL.

PPROVED SUBSTITUTIONS WILL BE REPLACED AT THE EXPENSE OF

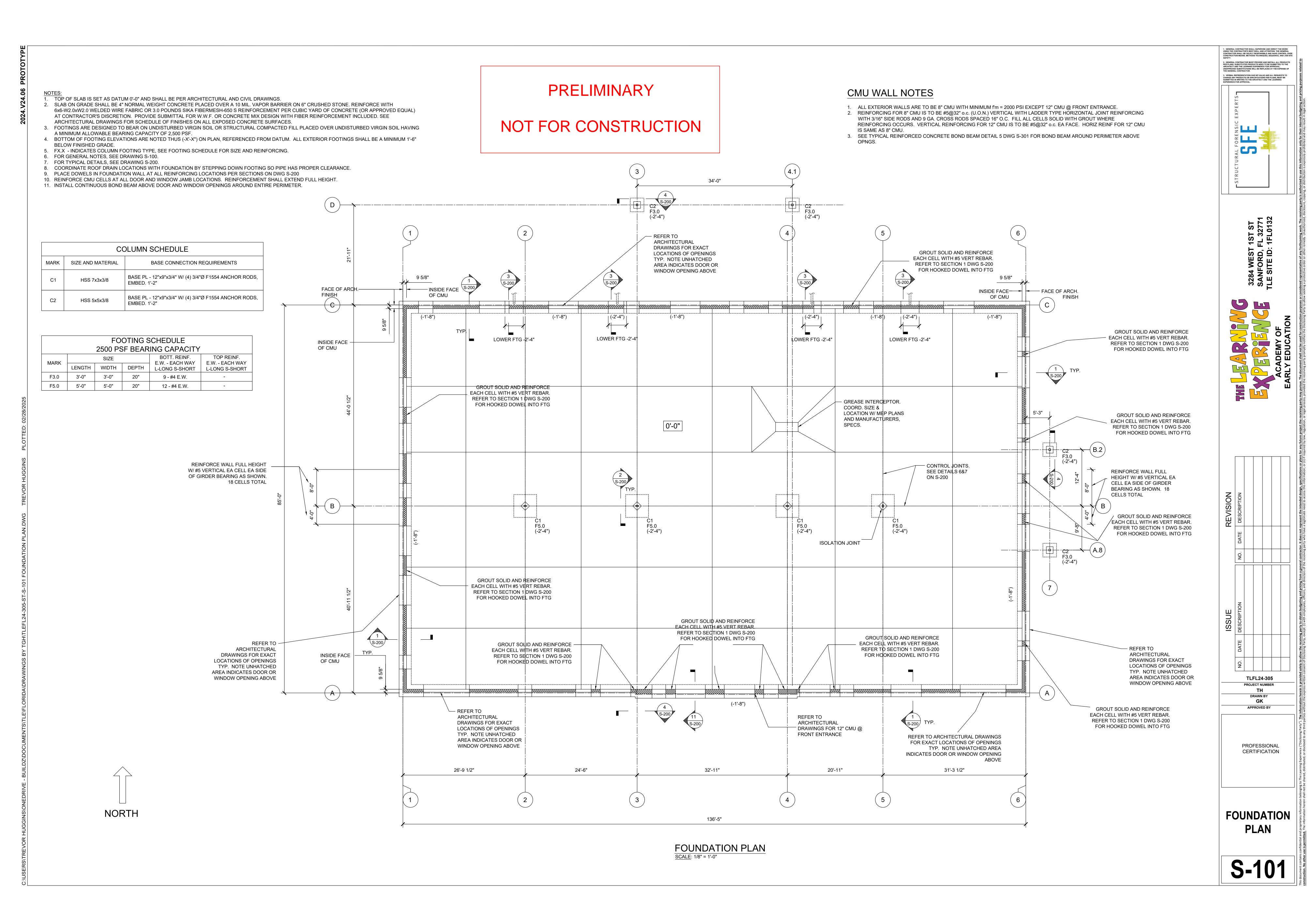


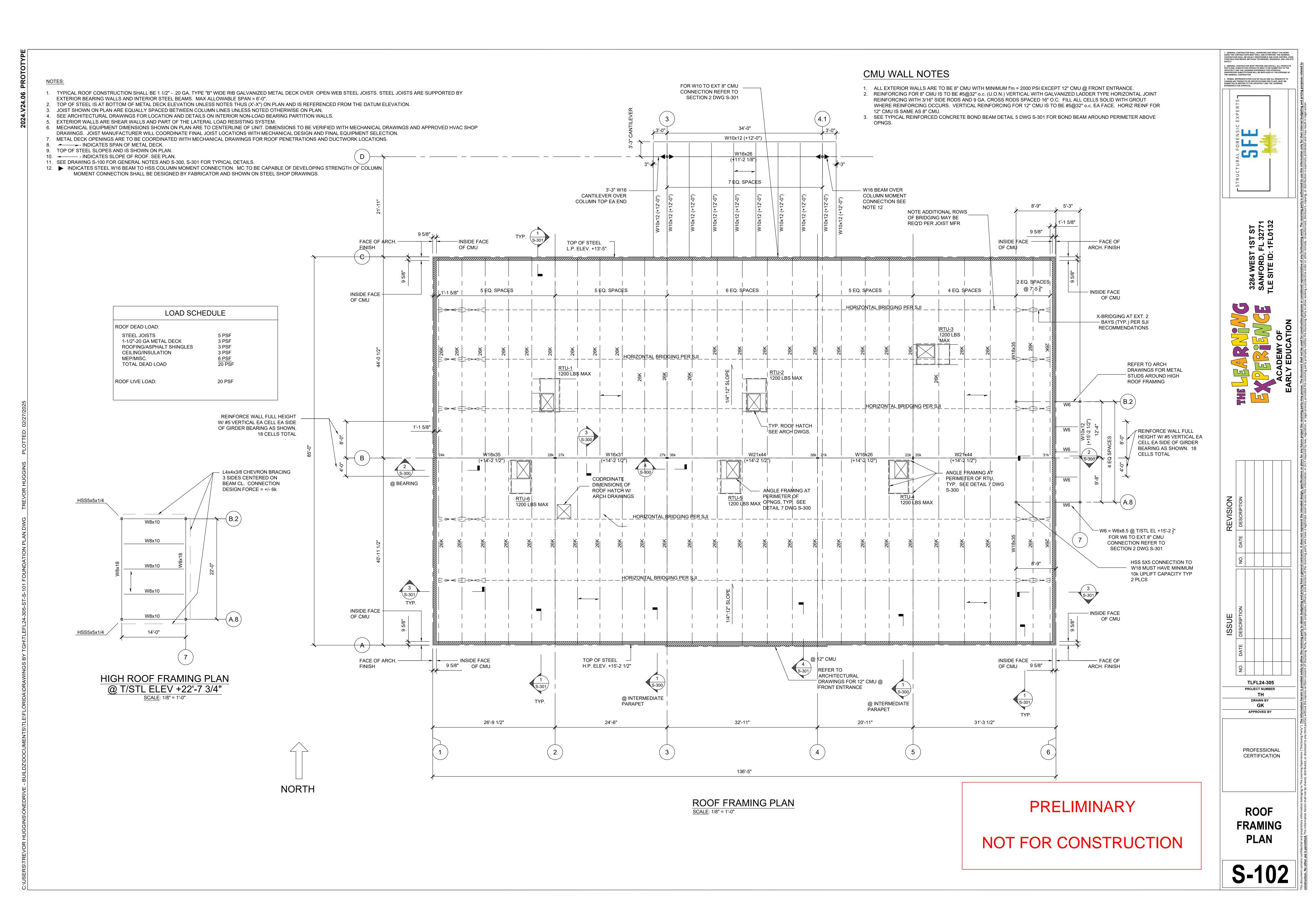
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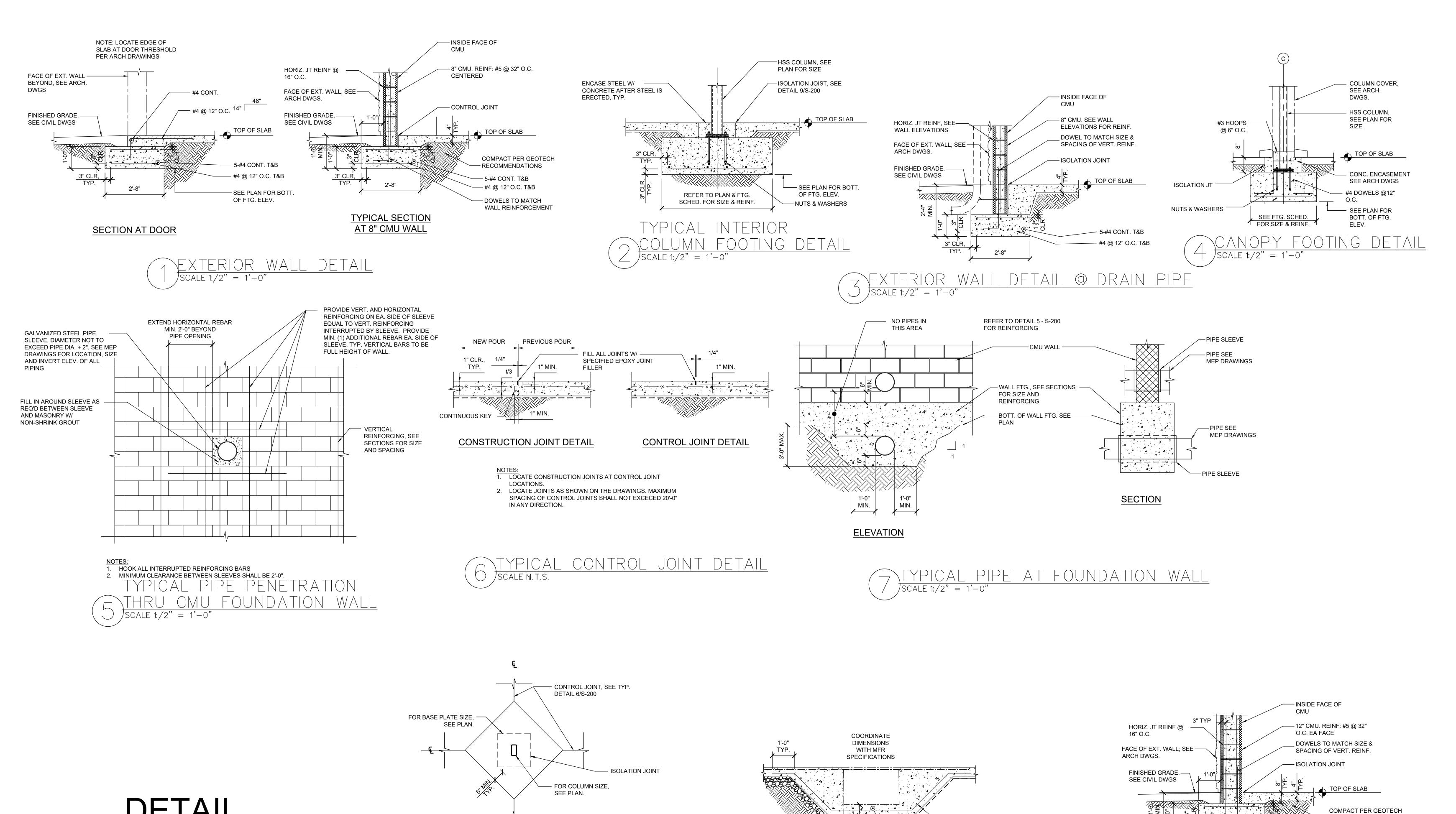
PROFESSIONAL CERTIFICATION

GENERAL NOTES

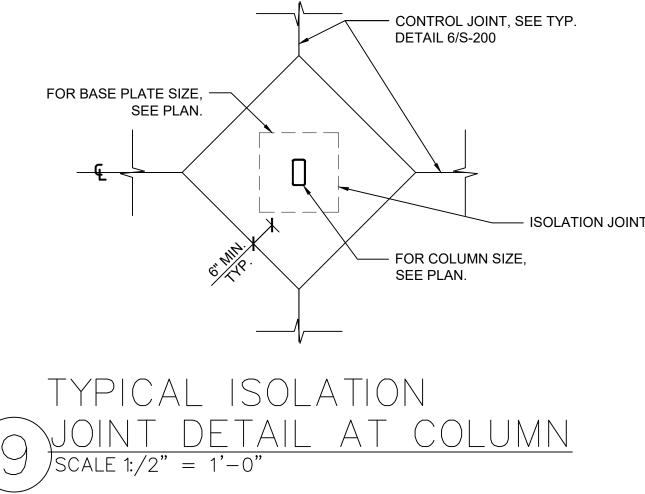
27. ALL CONCRETE REINFORCING IS SUBJECT TO INSPECTION BY THE DESIGN ENGINEER PRIOR TO

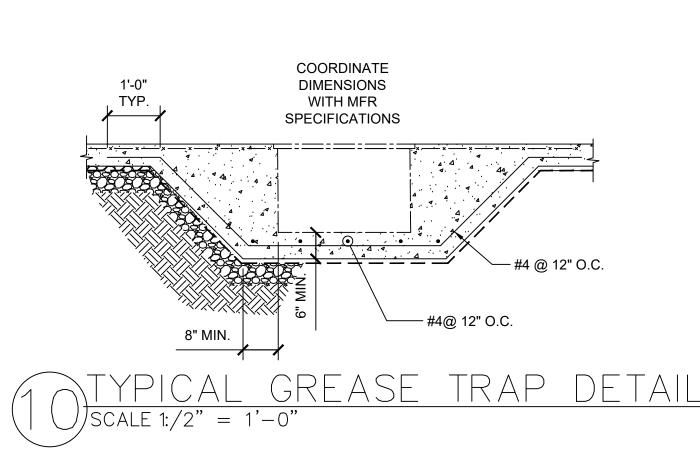


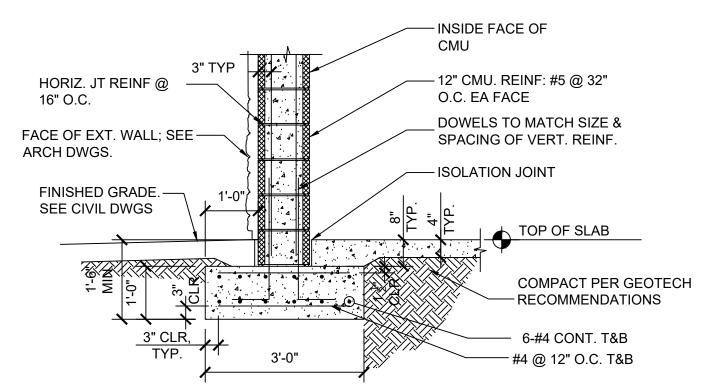




DETAIL NOT USED







1) 12" CMU EXTERIOR WALL DETAIL

SCALE 1:/2" = 1'-0"

PRELIMINARY

NOT FOR CONSTRUCTION

SCALE 1:/2" = 1'-0"

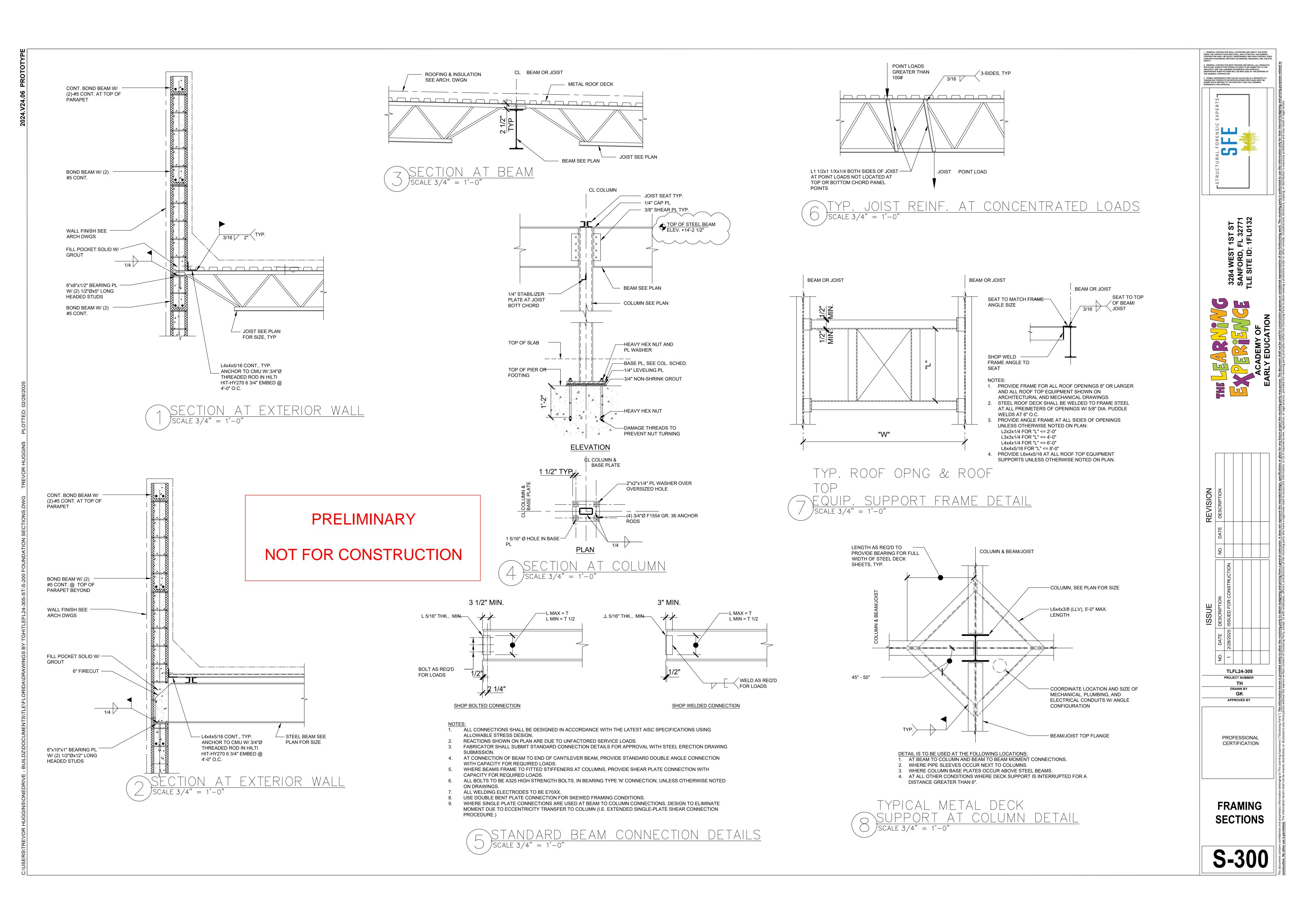
3. VERBAL REPRESENTATION HAS NO VALUE AND ALL REQUESTS TO CHANGE ANY PRODUCTS OR SPECIFICATIONS PER PLANS, MUST BE SUBMITTED IN WRITING TO THE ARCHITECT AND THE LEARNING EXPERIENCE FOR APPROVAL.

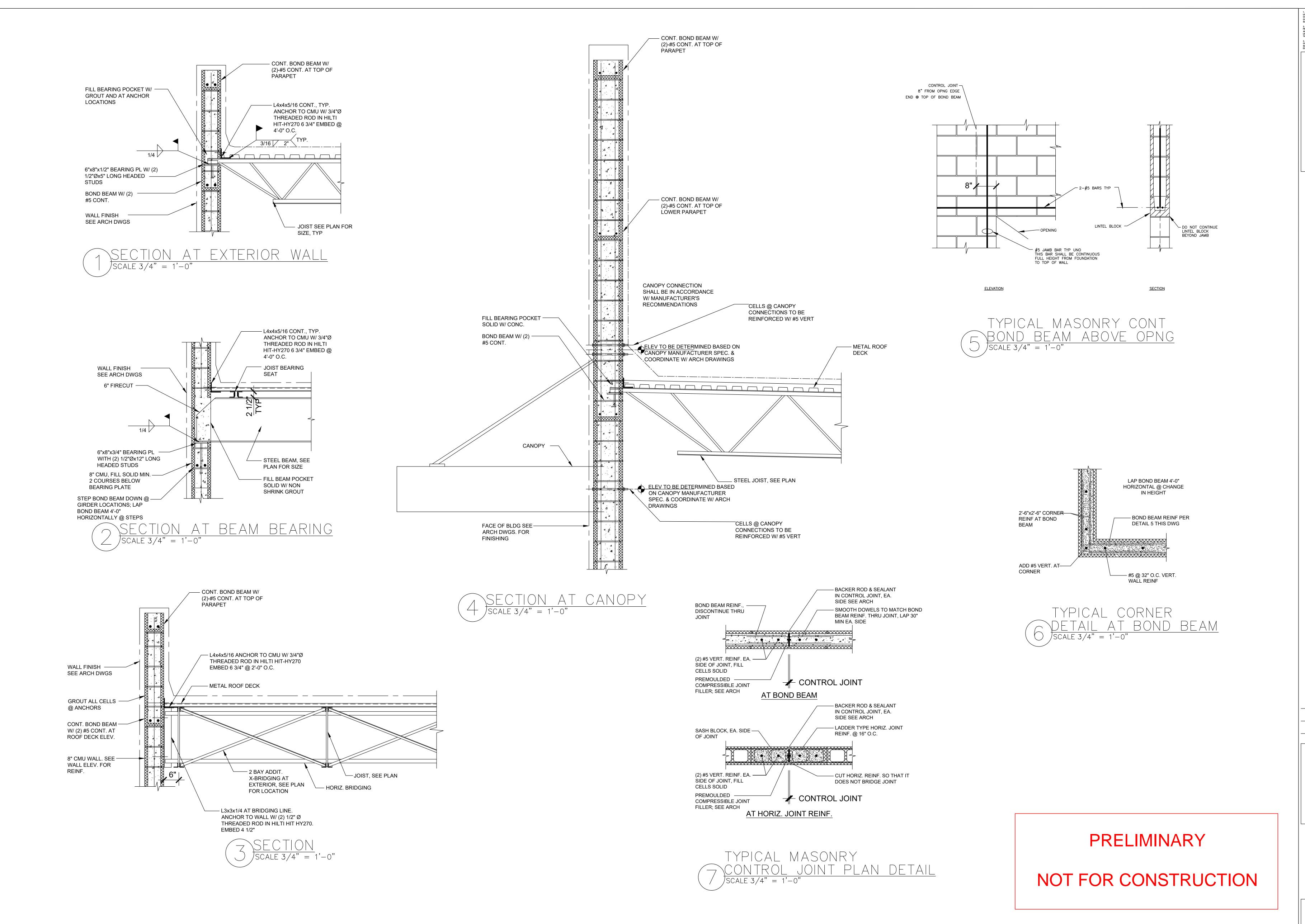
TLFL24-305 PROJECT NUMBER

TH DRAWN BY APPROVED BY

PROFESSIONAL CERTIFICATION

FOUNDATION SECTIONS





1. GENERAL CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK USING THE CONTRACTOR'S BEST SKILL AND ATTENTION. THE GENERAL CONTRACTOR'S BEST SKILL AND ATTENTION. THE GENERAL CONTRACTOR'S BEST SKILL AND ATTENTION. THE GENERAL CONTRACTOR MUST PROVIDE AND INSTALL ALL PRODUCTS SAFETY.

2. GENERAL CONTRACTOR MUST PROVIDE AND INSTALL ALL PRODUCTS PER PLANS. SUBSTITUTED PRODUCTS NEED TO BE SUBMITTED TO THE ARCHITECT AND THE LEARNING EXPERIENCE FOR APPROVAL. UNAPPROVED SUBSTITUTIONS WILL BE REPLACED AT THE EXPENSE OF THE GENERAL CONTRACTOR.

3. VERBAL REPRESENTATION HAS NO VALUE AND ALL REQUESTS TO CHANGE ANY PRODUCTS OR SPECIFICATIONS PER PLANS, MUST BE SUBMITTED IN WITHINS TO THE ARCHITECT AND THE LEARNING EXPERIENCE FOR APPROVAL.

SOLUTION OF THE CONTRACTOR.

WHEN ARCHITECT AND THE LEARNING EXPERIENCE FOR APPROVAL.

3284 WEST 1ST ST SANFORD, FL 32771 TLE SITE ID: 1FL0132

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ACADEMY OF

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ATE DESCRIPTION

8/2025 ISSUED FOR CONSTRUCTION

BY2025 ISSUED FOR CONSTRUCTION

BY2025 ISSUED FOR CONSTRUCTION

BY2025 ISSUED FOR CONSTRUCTION

BY2026 ISSUED FOR CONSTRUCTION

BY2026 ISSUED FOR CONSTRUCTION

BY2027 ISSUED FOR CONSTRUCTION

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FRAMING SECTIONS

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