GENERAL

1. THE GENERAL CONTRACTOR SHALL REVIEW AND DETERMINE THAT DIMENSIONS ARE COORDINATED BETWEEN ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO FABRICATION OR START OF CONSTRUCTION.
2. THE GENERAL CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, THE WORK PERSONS AND OTHER PEOPLE DURING CONSTRUCTION. HE SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION.
3. NO STRUCTURAL MEMBER SHALL BE CUT, NOTCHED OR OTHERWISE REDUCED IN STRENGTH.
4. THE GENERAL CONTRACTOR SHALL COORDINATE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR ANCHORED, EMBEDDED AND SUPPORTED ITEMS WHICH AFFECT THE STRUCTURAL DRAWINGS AND NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES.
5. ALL SHOP DRAWING SUBMITTALS SHALL BE SUBMITTED VIA ELECTRONIC MEDIA (I.E. PDF OR DWF FORMAT). HARD COPY SUBMITTALS WILL NOT BE ACCEPTED. SUBMITTALS SHALL NOT BE SECURED IN ANY FORMAT THAT WILL PREVENT COMMENTS FROM BEING ADDED.
6. ANY SUBMITTALS RECEIVED BY ARCHITECT/ENGINEER THAT HAVE NOT BEEN CHECKED BY THE GC AND HIS SUBCONTRACTOR SHALL BE RETURNED WITHOUT REVIEW.
7. ALL SECTIONS AND DETAILS SHALL BE CONSTRUED TO BE TYPICAL OR SIMILAR UNLESS ANOTHER SECTION OR DETAIL IS NOTED.
8. ANY CONFLICTS NOTICED, OR OBSERVED, BETWEEN THE WRITTEN SPECIFICATIONS AND THE CONSTRUCTION DOCUMENTS DURING PROJECT BIDDING OR PROJECT CONSTRUCTION SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE STRUCTURAL ENGINEER OF RECORD. IF SUCH DISCREPANCY IS NOT NOTICED OR BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER OF RECORD FOR WRITTEN CLARIFICATION, THE CONTRACTOR/SUBCONTRACTOR SHALL PROVIDE, AT PROJECT BID OR DURING PROJECT CONSTRUCTION, THE MORE STRINGENT AND/OR MORE COSTLY OF THE TWO ITEMS IN THE BID AND/OR FINAL INSTALLATION.
9. GENERAL CONTRACTOR/CONSTRUCTION MANAGER SHALL SUPPLY ALL SUB-CONTRACTORS WITH THE STRUCTURAL GENERAL NOTE SHEETS AS WELL AS THE STRUCTURAL DRAWINGS.
10. THE CONTRACTORS MEANS AND METHODS SHALL FULLY CONFORM TO THE REQUIREMENTS OF SEI/ASCE 37 (DESIGN LOADS ON STRUCTURES DURING CONSTRUCTION) UNTIL ALL OF THE STRUCTURAL ELEMENTS ARE IN PLACE AND HAVE RECEIVED THE INSPECTORS APPROVAL.
11. REFER TO ARCHITECTURAL DRAWINGS FOR ROOF COVERINGS. ROOF COVERINGS FOR ENHANCED HURRICANE PROTECTION AREA (EHPA) FACILITIES SHALL BE PROVIDED IN ACCORDANCE WITH THE LATEST ASTM AND FACTORY MUTUAL STANDARDS FOR MATERIALS AND WIND UPLIFT FORCES. ROOFS SHALL BE INSPECTED BY A LICENSED ENGINEER/ARCHITECT AND A REPRESENTATIVE OF THE ROOFING MANUFACTURER AND REPORTS SHALL BE SUBMITTED TO THE OWNER AND ARCHITECT.

FOUNDATIONS

1. A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY-STANDARD SOIL DENSITY TESTS TO ENSURE CONFORMANCE WITH GEOTECHNICAL SOILS REPORT. SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
2. CONTRACTOR, IN CONJUNCTION WITH GEOTECHNICAL FIELD REPRESENTATIVE, SHALL DETERMINE IF ANY UNSUITABLE CONDITIONS ARE DISCOVERED DURING EXCAVATION WHICH WOULD PREVENT ATTAINMENT OF THE DESIGN SOIL PRESSURE RECOMMENDED BY THE SOILS REPORT.
3. FOR FOUNDATION DESIGN VALUES, SEE FOUNDATION SCHEDULE.
4. FOOTINGS SHALL BE CAST TO THE SCHEDULED SIZE AND SHALL NOT BE OVERSIZED BY MORE THAN 6" ON ANY SIDE FOR FOOTING WIDTH OF AT LEAST 6'-0". FOR FOOTINGS LESS THAN 6'-0" IN WIDTH THE MAXIMUM OVERSIZING SHALL BE 3".
5. CONTRACTOR SHALL BE PREPARED FOR AND SHALL INCLUDE COST OF FORMING FOUNDATIONS SHOULD THE EARTH NOT PROVIDE ADEQUATE BANK STABILITY.

SLAB ON GRADE

1. UNLESS NOTED OTHERWISE IN THE GEOTECHNICAL REPORT, COMPACT INTERIOR FILL TO 95% OF MODIFIED PROCTOR MAXIMUM DRY DENSITY (ASTM D1557). SOIL COMPACTION SHALL BE FIELD-CONTROLLED BY A REPRESENTATIVE TECHNICIAN OF A QUALIFIED LABORATORY. EACH LAYER OF FILL SHALL NOT EXCEED 12" THICK AND SHALL BE COMPACTED PRIOR TO PLACEMENT OF NEXT LAYER.
2. MAXIMUM SPACING OF CONTROL JOINTS (I.E. SAWCUT JOINT OR CONSTRUCTION JOINT) SHALL BE AS SET IN THE TABLE BELOW, OR AS NOTED ON PLANS. THE MORE STRINGENT SHALL APPLY. PATTERNS SHALL BE APPROXIMATELY SQUARE WITH A RATIO OF LONG SIDE TO SHORT SIDE NOT EXCEEDING 1.5 TO 1. SEE SLAB-ON-GRADE DETAILS FOR ADDITIONAL INFORMATION.

SLAB THICKNESS (IN.)	3/4" OR LARGER AGGREGATE SPACING (FT.)
4"	12'
5"	13'
6"	14'
7" AND GREATER	15'

MIX DESIGNS CONTAINING AGGREGATE LESS THAN 3/4" ARE NOT ACCEPTABLE

3. GENERAL CONTRACTOR SHALL COORDINATE EXACT LOCATION OF SJ'S AND CJ'S WITH ARCHITECTURAL FLOOR FINISHES TO ENSURE SLAB JOINTS DO NOT READ THROUGH.

CONCRETE AND REINFORCING

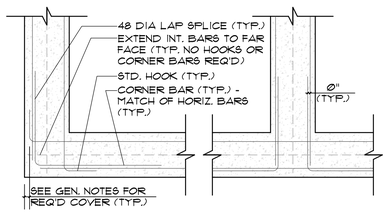
1. A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD TESTING INCLUDING SLUMP TESTS AND CYLINDER BREAKS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
2. CONCRETE WORK SHALL CONFORM TO ACI 318-11 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE.
3. ALL CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES.

LOCATION	28 DAY STRENGTH	SLUMP	COARSE AGGREGATE(S)
SLAB-ON-GRADE	4,000 PSI	4"+-1"	1 1/2"
RETAINING WALL	4,000 PSI	4"+-1"	1"
TIE BEAMS	4,000 PSI	4"+-1"	3/4"
TIE COLUMNS	3,000 PSI	4"+-1"	3/4"
CAST-IN-PLACE BEAMS	4,000 PSI	4"+-1"	1"
CAST-IN-PLACE COLUMNS	4,000 PSI	4"+-1"	1"
EQUIPMENT SUPPORTS	4,000 PSI	4"+-1"	1"
TILT-UP PANELS	4,000 PSI	4"+-1"	1 1/2"
GROUT UNDER TILT-UP PANELS	5,000 PSI	8"+-1"	3/8"
ELEVATED SLABS FORMED AND POURED	4,000 PSI	4"+-1"	1"
ELEVATED SLABS FORMED W/ METAL DECK	3,000 PSI	4"+-1"	1"
FILLED CELL, PRECAST LINTELS & BOND BEAM GROUT	2,500 PSI	8"+-1"	3/8"

NOTES:
1. SLUMP FOR RAMPS AND SLOPING SURFACES SHALL NOT EXCEED 4"
2. ALL SLAB MIXES SHALL HAVE A MAXIMUM SAND TO TOTAL AGGREGATE RATIO Ø.50.
3. A 2" OR 3" PUMP SHALL BE ACCEPTABLE FOR COLUMNS, CELL FILL AND TIE BEAMS BUT WILL NOT BE ALLOWED FOR FOUNDATIONS, SLABS, TILT-UP PANELS AND CONCRETE BEAMS.
4. READY MIX SUPPLIER SHALL DESIGN THE MIXES THAT CONTAIN MULTIPLE AGGREGATES TO BE WELL GRADATED.
5. SLABS SHALL NOT BE AIR ENTRAINED.
6. FOR SLABS THAT SHALL RECEIVE MOISTURE SENSITIVE FLOORING:
- CONTRACTOR SHALL WORK WITH THE READY MIX SUPPLIER TO PROVIDE A MIX DESIGN THAT WILL BE AT OR BELOW 75% RELATIVE HUMIDITY AT THE TIME THE FLOORING IS SCHEDULED TO BE INSTALLED.
- DO NOT USE LIGHTWEIGHT AGGREGATES.
- PROVIDE A MIX WITH GOOD SELF-DESICCATING PROPERTIES. CONSIDER ADDING 2%-4% SILICA FUME.
- DO NOT HARD TROWEL THE SURFACE BUT INSTEAD PROVIDE A LIGHTLY TROWELLED SURFACE.

1. CONCRETE MIX DESIGN SUBMITTALS
- EACH MIX DESIGN SHALL BE LABELED TO INDICATE THE AREA IN WHICH THE CONCRETE IS TO BE PLACED (I.E. FOUNDATIONS, SLAB-ON-GRADE, COLUMNS, ETC.) FAILURE TO DO SO WILL CAUSE DELAY AND/OR REJECTION OF SUBMITTALS.
- PROPOSED MIX DESIGN SHALL BE IN ACCORDANCE WITH METHOD 1 OR METHOD 2 OF ACI 301. PROVIDE SUPPORTING DATA IN TABULAR FORM FOR EACH SEPERATE PROPOSED MIX.
- SUBMIT CONCRETE MIX DESIGN FOR EACH PROPOSED CLASS OF CONCRETE.

2. REBAR SHALL CONFORM TO ASTM-615 GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO A-185 AND SHALL BE LAPPED MINIMUM ONE MESH +2" WHERE SPLICED. ALL REINFORCING SHALL BE DOMESTICALLY PRODUCED. ALL REBAR THAT IS TO BE WELDED SHALL BE LOW ALLOY ASTM A706 GRADE 60.
3. SPLICES AND ANCHORAGE OF REINFORCING SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE.
- WELDED WIRE FABRIC: 8"
- ALL OTHER:
- #3 BAR & SMALLER: 48 DIA (12" MIN)
- #4 BAR & LARGER: 60 DIA
4. REINFORCEMENT IN WALLS, FOOTINGS AND BEAMS SHALL BE CONTINUOUS AND LAPPED 48 BAR DIA AT SPLICE UNLESS NOTED OTHERWISE. HOOK AND LAP ALL CORNER AND INTERSECTING BARS. (SEE REINF DEVELOPMENT DETAIL BELOW).



REINF. DEVELOPMENT DETAIL

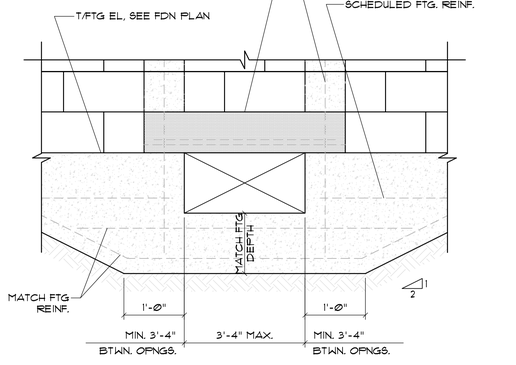
SCALE: N.T.S.

5. COVER FOR REINFORCING SHALL BE AS FOLLOWS

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"
CONCRETE EXPOSED TO EARTH OR WEATHER:
#6 THRU #18 BARS: 2" #5 BAR, W31 OR D31 WIRE AND SMALLER: 1 1/2"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER:
SLABS, WALLS, JOISTS: #14 AND #18 BARS: 1 1/2" #11 BAR AND SMALLER: 3/4"
BEAMS, COLUMNS: PRIMARY REINF., TIES, STIRRUPS, SPIRALS: 1 1/2"

6. FOOTING PENETRATION DETAILS

NOTES:
1. GC SHALL CONTACT ENGINEER IF OPENING SIZE AND/OR SPACING EXCEEDS THAT SHOWN.
2. WRAP PIPE(S) WITH A MINIMUM 1" THICK COMPRESSIBLE PIPE INSULATION FOR ENTIRE WIDTH OF FTG. +4" MIN. EA. SIDE. GROUT VOID SOLID.

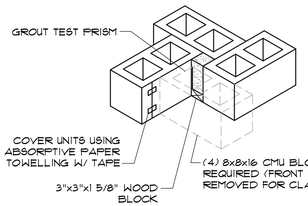


FOOTING PENETRATION DETAIL

SCALE: N.T.S.

MASONRY

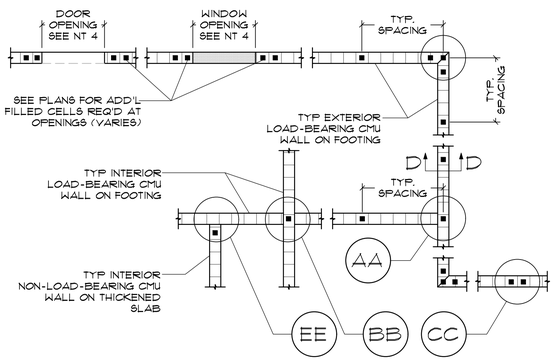
1. MASONRY CONSTRUCTION MATERIALS AND INSPECTIONS SHALL CONFORM TO THE LATEST EDITION OF THE ACI BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES (ACI 530-11, ASCE 5-11, TMS 402-11), SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530.1-11, ASCE 6-11, TMS 602-11) ASTM C476-02, ASTM C1019-09 AND NCMA TEK 10-1.
2. CONCRETE BLOCKS SHALL CONFORM TO ASTM C-90 (f'm = 1500 PSI) (1900 PSI ON THE NET AREA).
3. MORTAR SHALL COMPLY WITH ASTM C270, TYPE "M" FOR RETAINING WALLS AND WALLS BELOW GRADE, TYPE "S" FOR TYPICAL WALLS. (COMPRESSIVE STRENGTH = 2500 PSI AND 1800 PSI, RESPECTIVELY. SITE TESTED MORTAR CUBES SHALL ACHIEVE A MINIMUM OF 80% OF THE DESIGN COMPRESSIVE STRENGTH).
4. BLOCK SHALL NOT BE MOISTENED BEFORE GROUTING.
5. ALL MASONRY CROSS WEBS SHALL BE FULLY BEDDED IN MORTAR AROUND CELLS TO BE GROUTED.
6. REINFORCE WALLS WITH LADDER TYPE (ASTM A-153, #9 GAUGE WIRE) DEFORMED REINFORCEMENT EQUAL TO DUR-O-WAL IN BED JOINTS AT 16" O.C. U.N.O, MEASURED VERTICALLY. PLACE PER THE MFR'S INSTRUCTIONS. LAP ALL HORIZONTAL JOINT REINFORCING 6" MIN.
7. VERTICAL REINFORCING MUST HAVE A MINIMUM CLEARANCE OF 1/2" TO INSIDE FACE. VERTICAL REINFORCEMENT IN WALLS SHALL BE SECURED AND LATERALLY SUPPORTED AGAINST DISPLACEMENT AT INTERVALS NOT EXCEEDING 192 X (BAR DIAMETER) OR 10 FT (WHICHEVER IS LESS) WHENEVER A CLEAN-OUT IS REQUIRED. SEE GROUTING DETAIL NOTE FOR CLEAN-OUT REQUIREMENTS.
8. GROUT PLACEMENT STOPPED FOR (1) HOUR OR MORE SHOULD BE STOPPED 1 1/2" BELOW THE TOP OF THE MASONRY UNIT TO PROVIDE A KEY FOR SUBSEQUENT GROUTING.
9. TYPICAL VERTICAL REINFORCING SIZE AND SPACING SHALL BE ABOVE AND BELOW ALL WALL OPENINGS.
10. TEMPORARY BRACING AND SHORING OF WALLS TO PROVIDE STABILITY DURING CONSTRUCTION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
11. REINFORCE MASONRY OPENINGS LESS THAN 2'-0" WIDE, WITH HORIZ. JOINT REINF PLACED IN (2) HORIZ JOINTS APPROXIMATELY 8" APART, IMMEDIATELY ABOVE THE GROUT FILLED LINTEL AND IMMEDIATELY BELOW THE GROUT FILLED SILL. EXTEND REINFORCING A MINIMUM OF 2'-0" BEYOND JAMBS OF THE OPENING EXCEPT AT CONTROL JOINTS.
12. PROVIDE FILLED PRECAST U-LINTELS AS MANUFACTURED BY CAST-CRETE OR APPROVED EQUAL WITH (2) #5 CONT. AT ALL OPENINGS WHERE BEAMS ARE NOT SHOWN, SCHEDULED OR NOTED GREATER THAN 2'-0" WIDE. LINTELS SHALL HAVE MINIMUM UNFILLED CAPACITY OF 400 LB/LF AND BEAR NOMINAL 6" (MIN 6") EACH END ON A GROUT FILLED CELL. AVOID PRE-CAST LINTEL MFR'S STANDARD TABULATED LOAD TABLES AS EVIDENCE THAT THE MINIMUM CAPACITIES AS LISTED IN THE BEAM SCHEDULE ARE SATISFIED. REFER TO MASONRY WALL BEAM SCHEDULE FOR TYPICAL PRECAST LINTEL SPANS AND DETAILS.
13. STOPPING AND RESUMING WORK: RACK BACK 1/2-UNIT LENGTH IN EACH COURSE. DO NOT TOOTH. CLEAN EXPOSED SURFACES OF SET MASONRY. REMOVE LOOSE MASONRY UNITS AND MORTAR PRIOR TO LAYING FRESH MASONRY.
14. DO NOT APPLY UNIFORM LOADS TO MASONRY WALLS FOR (3) DAYS.
15. DO NOT APPLY CONCENTRATED LOADS TO MASONRY WALLS FOR (7) DAYS.
16. EXTEND VERTICAL WALL REINFORCEMENT TO WITHIN 2" OF TOP OF WALL OR BEAM UNLESS NOTED OTHERWISE. TERMINATE REINFORCING WITH STANDARD ACI 90 DEGREE HOOK IF ROOF JOISTS AND/OR TRUSSES BEAR ON TOP OF WALL AND THERE IS NO PARAPET. IF PARAPET EXISTS, HOOK IS NOT REQUIRED.
17. REFER TO ARCHITECTURAL DRAWINGS FOR WATERPROOFING DETAILS AT MASONRY CONTROL JOINTS.
18. JOB SITE MIXING OF GROUT SHALL NOT BE PERMITTED. TESTING SHALL CONFORM TO ASTM C1019. SEE TEST MOLD DETAIL BELOW. SEE THE SCHEDULE UNDER CONCRETE NOTES FOR COMPRESSIVE STRENGTH AND SLUMP REQUIREMENTS.



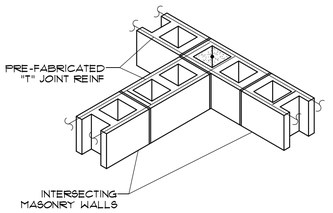
MASONRY GROUT DETAIL - TEST MOLD (ASTM C-1019)

SCALE: N.T.S.

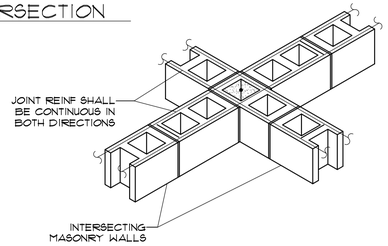
19. TYPICAL MASONRY DETAILS



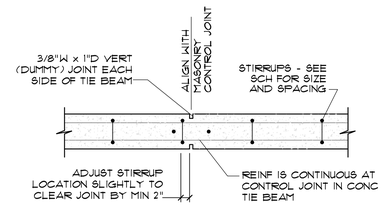
NOTES:
1. SEE DETAIL "CC" FOR LOCATING MASONRY CONTROL JOINTS. CONTRACTOR SHALL SUBMIT MCJ PLAN TO ARCHITECT FOR APPROVAL.
2. SEE ARCHITECTURAL DRAWINGS FOR OPENING SIZES AND LOCATIONS.
3. SEE FDN PLAN NOTES FOR REINFORCED FILLED CELL SIZE AND SPACING.
4. MULTIPLE FILLED CELLS MAY BE REQUIRED AT JAMBS. ADDITIONAL BARS WILL BE SHOWN ON PLANS. IF NOONE ARE SHOWN, THEN A SINGLE TYPICAL REINFORCED JAMB IS SUFFICIENT.
5. SEE MASONRY NOTES ON GENERAL NOTE SHEETS FOR HORIZONTAL JOINT REINFORCING AND OTHER ADDITIONAL INFORMATION.



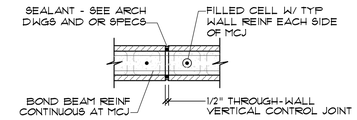
AA WALL INTERSECTION



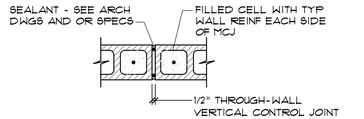
BB WALL INTERSECTION



MCJ @ TIE BEAM

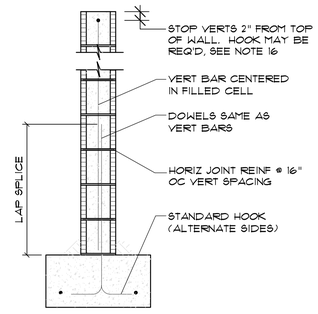


MCJ @ BOND BEAM



NOTES:
1. THROUGH-WALL JOINT SHALL BE CONTINUOUS WITHOUT INTERRUPTION FROM FOUNDATION TO TOP OF WALL.
2. TERMINATE TYPICAL HORIZONTAL JOINT REINFORCING 2" FROM JOINT.
3. MAXIMUM SPACING OF CONTROL JOINTS SHALL BE 1 1/2 x (WALL HEIGHT) OR 25'-0", WHICHEVER IS LESS.

CC MAS CONTROL JOINT (MCJ)



DD TYPICAL FILLED CELL DETAILS

BELLÓN ARCHITECTURE

BELLON ARCHITECTURE, INC
12930 S.W. 128 STREET
SUITE 203
MIAMI, FL 33186
T. (305) 278-7776
F. (305) 278-7473
WWW.BELLONARCHITECTURE.COM
ARCHITECTURE
LAND PLANNING
INTERIORS
CONSTRUCTION
MANAGEMENT

bennett&pless

BENNETT & PLESS INC.
JENNIFER ZABIK, P.E. S.E.
1024 N FULLERS CROSS ROAD
WINTER GARDEN, FL. 34787
JZABIK@BENNETT-PLESS.COM
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SEALS

RAPID BUILDINGS EAST

9000 WEST COLONIAL DR.
SUITE 401
OCOEE, FL. 34761
PHONE: (407) 347-9614

INFO@
RAPIDBUILDINGSOLUTIONS.COM

JOB NUMBER:
RBE-040-25-CLE-FL

HOOKS ST. STORAGE
13125 HOOKS STREET
CLERMONT, FL. 34711

REV.	DATE	DESCRIPTION	BY
1	03/10/25	ELEVATION UPDATES	DJM
2	05/28/25	RISER ROOM ADDED	DCH

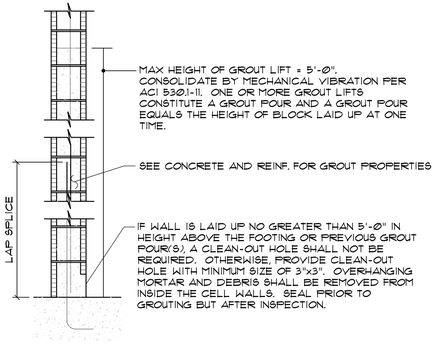
SHEET TITLE

FOUNDATION SPECIFICATIONS

DATE	01-23-25
DESIGNED BY	RBX
CHECKED BY	DJM
DRAWN BY	DJM
SCALE	AS NOTED
SHEET	

F0.1

GROUT FOR FILLED CELLS SHALL BE PLACED IN CONFORMANCE WITH ACI 530.1-11 AND AS INDICATED BELOW:



NOTES:

1. BUTTER AROUND ALL CELLS THAT ARE TO BE FILLED WITH GROUT TO ENSURE GROUT DOES NOT LEAK OUT.
2. DO NOT GROUT UNTIL MORTAR HAS SET SUFFICIENTLY TO WITHSTAND THE PRESSURE OF THE GROUT. WAIT NOT LESS THAN 24 HOURS.
3. PLACE GROUT WITHIN 90 MINUTES FROM INTRODUCING WATER IN THE MIXTURE AND PRIOR TO INITIAL SET.
4. MAXIMUM WALL HEIGHT FROM TOP OF FOOTING OR PREVIOUS GROUT POURS LAID UP AT ONE TIME SHALL BE 4'-0".
5. THE MINIMUM CONTINUOUS UNOBSTRUCTED CLEAR AREA IN CELL TO RECEIVE GROUT MUST BE NOT LESS THAN 3"x3". MORTAR FINS MUST BE REMOVED AS BLOCK PLACEMENT PROCEEDS. MORTAR DROPPINGS MUST BE KEPT OUT OF CELLS WHICH ARE TO BE GROUTED.

GROUTING DETAIL

SCALE: N.T.S.

MINIMUM LAP SPLICES FOR REINFORCED CMU.

CMU WALL WITH CENTERED VERTICAL REINFORCING					
#4	#5	#6	#7	#8	
13"	21"	31"	42"	63"	

SPECIALTY ENGINEERING REQUIREMENTS

1. STEEL PANS STAIRS SHALL BE DESIGNED BY FABRICATORS SPECIALTY ENGINEER AND SHALL INCLUDE STRINGERS, TREADS, HAND RAILINGS, FLATFORMS (AS REQUIRED) PAN INSERTS AND MISCELLANEOUS SUPPORTS AND CONNECTIONS. SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. MINIMUM DESIGN LIVE LOAD SHALL BE 100 PSF. SUBMITTALS SHALL INCLUDE THE JOINTING IN THE CONCRETE FILL AS REQUIRED TO MITIGATE PLASTIC SHRINKAGE CRACKING.
2. GUARDRAILS, HANDRAILS, POSTS AND SUPPORT CONNECTIONS SHALL BE DESIGNED BY THE FABRICATORS SPECIALTY ENGINEER. SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY AN ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. DUE CONSIDERATION SHALL BE GIVEN TO EXPANSION AND CONTRACTION BY PROVIDING SLIP JOINTS AS REQUIRED. DESIGN LOADING SHALL CONFORM TO ALL REQUIREMENTS OF THE BUILDING CODE (SEE DESIGN CRITERIA FOR APPLICABLE BUILDING CODE).
3. FLAG POLES AND SITE LIGHTING POLES SHALL BE DESIGNED BY THE POLE VENDORS SPECIALTY ENGINEER AND SHALL INCLUDE POLES, FOUNDATIONS AND CONNECTIONS. SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. MINIMUM DESIGN LOADS SHALL CONFORM TO ANSI/NAAM FPI00 "SPECIFICATIONS FOR DESIGN LOADS OF METAL FLAGPOLES".

STRUCTURAL STEEL

1. A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
2. FABRICATE AND ERECT STRUCTURAL STEEL IN CONFORMANCE WITH THE LATEST VERSION OF AISC 360-10.
3. MATERIAL SPECIFICATIONS:
   - ALL STEEL SHALL BE PRODUCED DOMESTICALLY.
   - ROLLED SHAPES, PLATES AND BARS: ASTM A36, EXCEPT WIDE-FLANGE AND WT SECTIONS, WHICH SHALL BE ASTMA992.
   - HOLLOW STRUCTURAL SECTION (HSS): ASTM A500, GRADE B.
   - ANCHOR BOLTS, RODS, NUTS AND WASHERS: PER BASE PLATE SCHEDULE.
   - HEADED STUDS: ASTM A108, GRADE 1015 THROUGH 1020, COLD-FINISHED CARBON STEEL, AWS D1.1, TYPE B.
   - BOLT STRUCTURAL CONNECTIONS: UNLESS NOTED OTHERWISE, ALL BOLTS SHALL BE 3/4" DIA. ASTM A325, TYPE N. BOLTS INDICATED LESS THAN 5/8" DIA. SHALL BE ASTM A307.
   - WELDED CONNECTIONS: ELECTRODE - E70XX U.N.O. (LOW HYDROGEN). FILLET WELDS SHALL BE 3/16" U.N.O.
4. HIGH STRENGTH FIELD-BOLTED CONNECTIONS SHALL BE INSTALLED, TIGHTENED, TESTED AND INSPECTED ACCORDING TO "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS" BY THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (RCSC). ALL BOLTS IN STEEL TO STEEL CONNECTIONS SHALL BE BROUGHT TO A "SNUG-TIGHT" CONDITION, AS DEFINED IN THE SPECIFICATION. ALL BOLTS IN STEEL TO EMBED CONNECTIONS SHALL BE FINGER-TIGHT WITH PEENED THREADS. SLIP-CRITICAL (SC) BOLTS MUST BE FULLY TENSIONED PER SPECIFICATION.
5. ALL WIDE FLANGE FLOOR MEMBERS SHALL BE CONNECTED TO THE SUPPORTING STRUCTURE AS DETAILED IN CONNECTION SCHEDULES.
6. BRACE AND MAINTAIN ALL STEEL IN ALIGNMENT UNTIL OTHER PARTS OF CONSTRUCTION NECESSARY FOR PERMANENT SUPPORT ARE COMPLETED. CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLING TEMPORARY SHORING AS REQUIRED FOR THE STABILITY OF THE STEEL FRAME UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN COMPLETED AND BUILDING IS ENCLOSED.
7. ALL WELDING SHALL CONFORM TO THE REQUIREMENTS OF "THE STANDARD CODE FOR WELDING IN BUILDING CONSTRUCTION" OF THE AMERICAN WELDING SOCIETY.
8. GROUT FOR COLUMN BASE PLATES AND PRESET BEARING PLATES SHALL BE NON-SHRINK NON-METALLIC GROUT (5000 PSI MIN.).
9. SUBMIT SHOP DRAWINGS INDICATING ALL SHOP ERECTION DETAILS INCLUDING PROFILES, SIZES, SPACING AND LOCATIONS OF STRUCTURAL MEMBERS, CONNECTION ATTACHMENTS, FASTENERS, LOADS AND TOLERANCES.
10. STRUCTURAL STEEL SHALL RECEIVE A SHOP COAT OF PRIMER (COLOR AS DIRECTED BY ARCHITECT) EXCEPT THOSE AREAS WHICH WILL RECEIVE SPRAY-ON FIRE PROTECTION, OR WHERE HEADED STUDS ARE TO BE WELDED.
11. THE STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN FULL CONFORMANCE WITH THE "OSHA STEEL ERECTION STANDARD". IF THE CONSTRUCTION DRAWINGS DEVIATE FROM THE OSHA STANDARD THEN THE FABRICATOR SHALL PROVIDE SUBMITTALS THAT CLEARLY INDICATE THE DEVIATION WITH A REVISION CLOUD AND REQUEST APPROVAL FROM BBM TO MAKE THE CHANGE SO THAT CONFORMANCE WITH THE OSHA STANDARD IS ASSURED.
12. REFER TO SPECIALTY ENGINEERING NOTES FOR DELEGATED ENGINEERING REQUIREMENTS.

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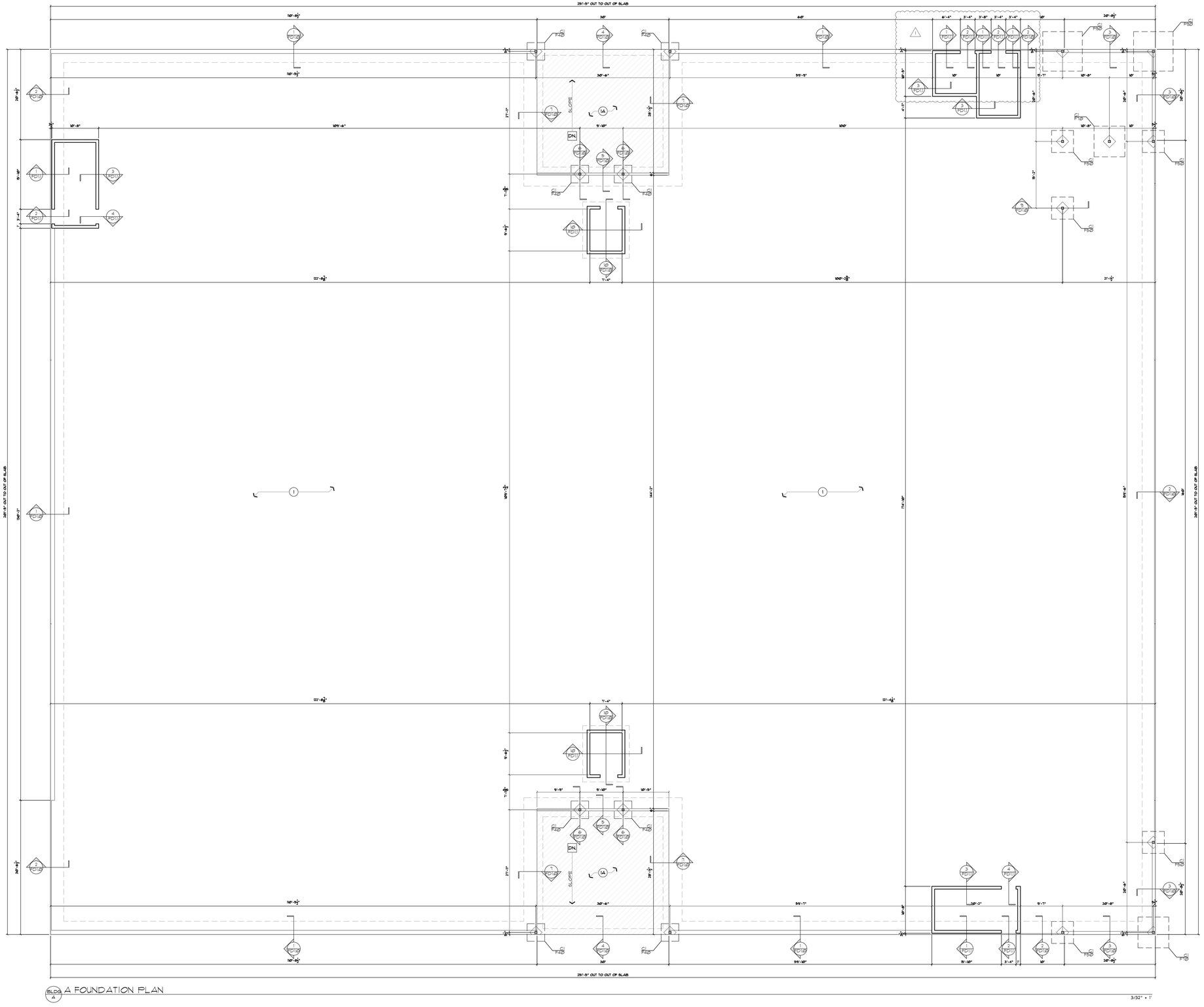
REV.	DATE	DESCRIPTION	BY
1	03/10/25	ELEVATION UPDATES	DJM
2	05/28/25	RISER ROOM ADDED	DCH

SHEET TITLE

FOUNDATION SPECIFICATIONS

DATE	01-23-25
DESIGNED BY	RBX
CHECKED BY	DJM
DRAWN BY	DJM
SCALE	AS NOTED
SHEET	

F0.1A



FOUNDATION PLAN NOTES (3 STORY)

1	FLOOR SLAB SHALL BE 8" THICK, Fc'=4,000psi CONCRETE REINFORCED WITH #4 BARS @ 12" O.C. EACH WAY OVER 10 MIL VAPOR BARRIER ON COMPACTED SUB-GRADE. (SEE SLAB ON GRADE DETAILS FOR PLACEMENT OF REINFORCED)
1A	FLOOR SLAB @ LOADING AREA SHALL BE 6" THICK, Fc'=4,000psi CONCRETE REINFORCED WITH #4 BARS @ 12" O.C. EACH WAY OVER 10 MIL VAPOR BARRIER ON COMPACTED SUB-GRADE. (SEE SLAB ON GRADE DETAILS FOR PLACEMENT OF REINFORCED)
2	T/SLAB EL. = 0'-0" TYP. UNO. T/LOADING ZONE SLAB EL. = -0'-1 1/2" TYP. UNO. T/INT. STL COL FTG = -0'-8" TYP. UNO. T/EXT STL COL FTG = -2'-0" TYP. UNO. (REFERENCE ONLY - SEE CIVIL DWGS FOR ACTUAL ELEVATIONS.)
3	ALL CMU BEARING WALLS ARE 8" (TYP. UNO).
4	REINF LOAD-BEARING CMU WALLS W/ # 5 VERT BAR CENTERED IN GROUT-FILLED CELL AT ENDS, CORNERS, AND AT MAX SPACING OF 24" OC. SEE "ILLUSTRATIVE PLAN OF VARIOUS CMU WALL CONDITIONS" ON SHEET F0.1 FOR ADDITIONAL INFO. ALL LOAD BEARING CMU WALLS SHALL HAVE A BB2 AT EA FLOOR & TOP OF WALL, UNO.
5	TYP SPACING OF FILLED CELLS SHALL APPLY ABOVE AND BELOW OPENINGS ALSO.
6	SEE SHEETS S0.1 FOR FOUNDATION GENERAL NOTES.
7	MAINTAIN STRUCTURAL SLAB THICKNESS AT ALL FLOOR SLOPES AND DEPRESSIONS
8	SEE BD10 FOR COLUMN AND BASE PLATE SCHEDULES.
9	▭ INDICATES 8" CMU.
10	T/ ELEVATOR MAT -4'-0" B.F.F. COORD W/ ELEVATOR MFR.

FOUNDATION SCHEDULE

MARK	SIZE (L x W x D)	REINFORCING	REMARKS
F40	4'-0" x 4'-0" x 1'-4"	(4) #5 EA WAY, BOT	COL PAD FOOTING
F50	5'-0" x 5'-0" x 1'-4"	(5) #5 EA WAY, BOT	COL PAD FOOTING
F70	7'-0" x 7'-0" x 1'-6"	(8) #6 EA WAY, BOT	COL PAD FOOTING
F90	9'-0" x 9'-0" x 2'-0"	(16) #6 EA WAY, TOP & BOT	COL PAD FOOTING

FOUNDATION DESIGN INFORMATION

ALL FOUNDATION DESIGNED IS BASED ON AN ASSUMED NET ALLOWABLE SOIL BEARING PRESSURE OF 2000 PSF. A SOILS REPORT SHALL BE OBTAINED PRIOR TO THE PROJECT START AND THE BEARING PRESSURE SHALL BE CONFIRMED. OWNER ASSUMES ALL LIABILITY IF A SOILS REPORT IS NOT OBTAINED AND/OR THE BEARING PRESSURE IS NOT VERIFIED.

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BLDG. A
FOUNDATION PLAN

DATE	01-23-25
DESIGNED BY	RBX
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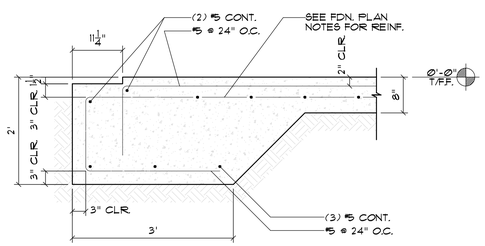
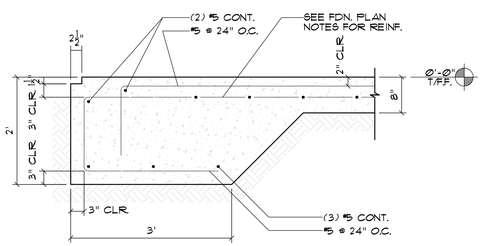
DRAWN BY	DJM
SCALE	AS NOTED
SHEET	

F1.0



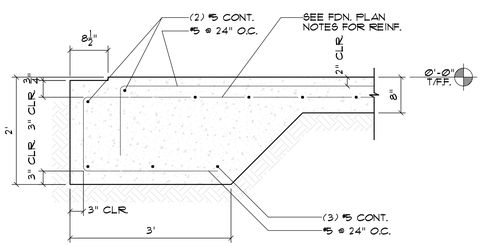
1 FD1.Ø — TYPICAL FOUNDATION DETAIL AT RAINLIP

SCALE: 3/4" = 1'



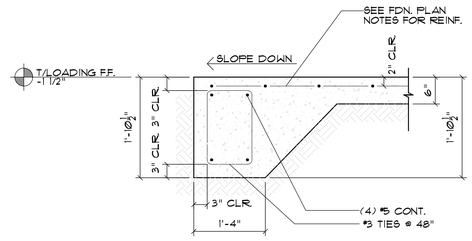
2 FD1.Ø — TYPICAL FOUNDATION DETAIL AT SHEETLEDGE

SCALE: 3/4" = 1'



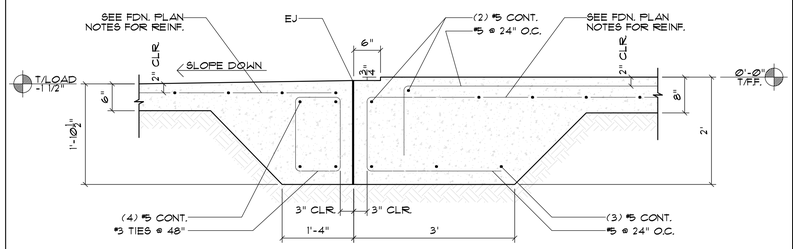
3 FD1.Ø — TYPICAL FOUNDATION DETAIL AT STORE FRONT

SCALE: 3/4" = 1'



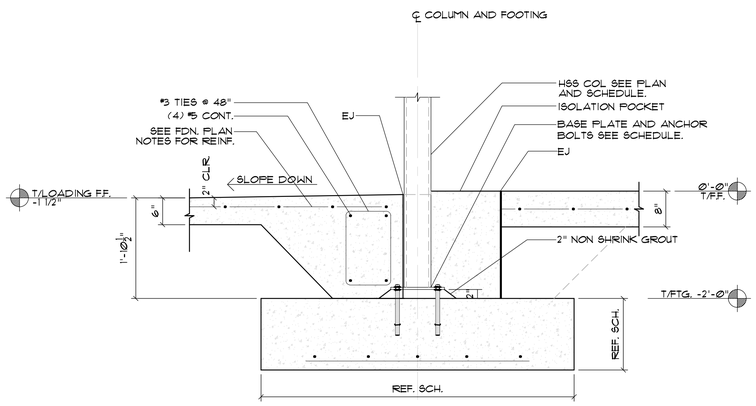
4 FD1.Ø — TYPICAL FOUNDATION DETAIL (EXTERIOR WALL LOADING ZONE)

SCALE: 3/4" = 1'



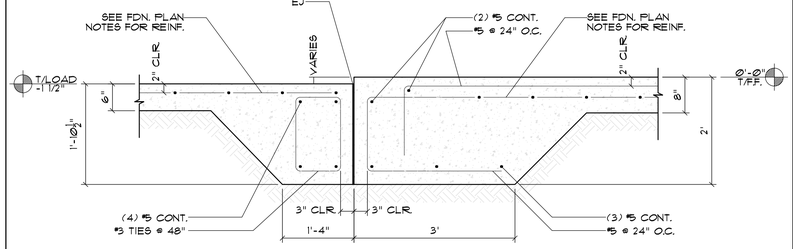
5 FD1.Ø — SECTION (ENTRANCE INTERIOR WALL LOADING ZONE)

SCALE: 3/4" = 1'



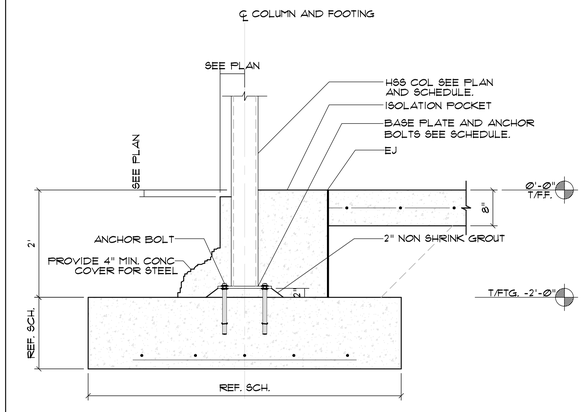
6 FD1.Ø — SECTION (ENTRANCE INTERIOR WALL LOADING ZONE @ HSS TUBE STEEL COLUMN)

SCALE: 3/4" = 1'



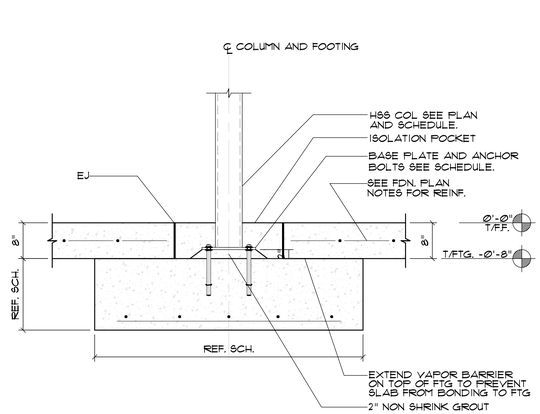
7 FD1.Ø — SECTION (INTERIOR WALL LOADING ZONE)

SCALE: 3/4" = 1'



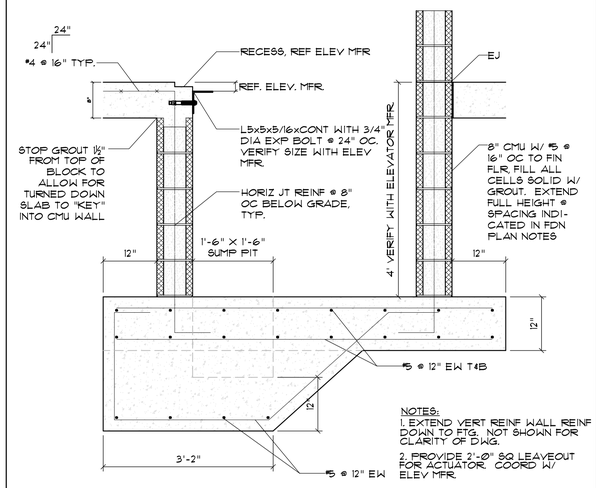
8 FD1.Ø — EXTERIOR HSS STEEL COLUMN TO FOUNDATION

SCALE: 3/4" = 1'



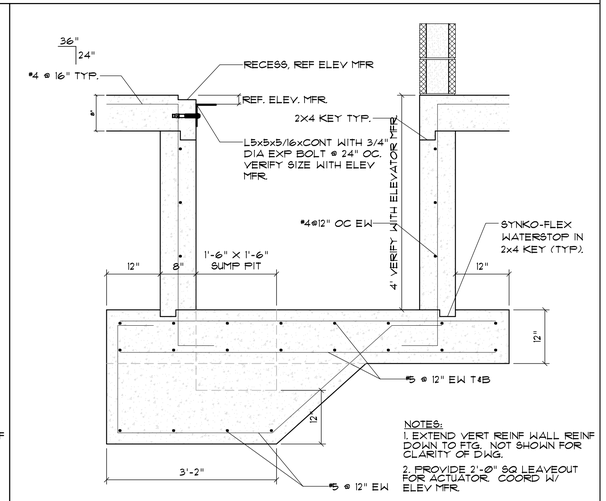
9 FD1.Ø — INTERIOR HSS STEEL COLUMN TO FOUNDATION

SCALE: 3/4" = 1'



10 FD1.Ø — SECTION AT ELEVATOR PIT

SCALE: 3/4" = 1'



10A FD1.Ø — SECTION AT ELEVATOR PIT (GC OPTION)

SCALE: 3/4" = 1'

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SEALS

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SHEET TITLE

FOUNDATION DETAILS

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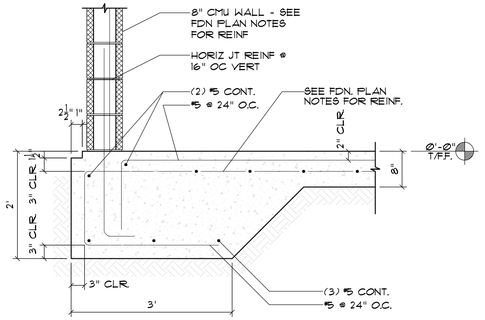
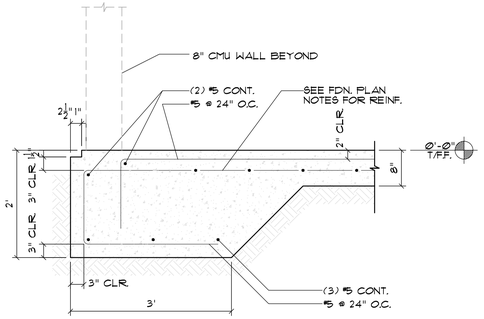
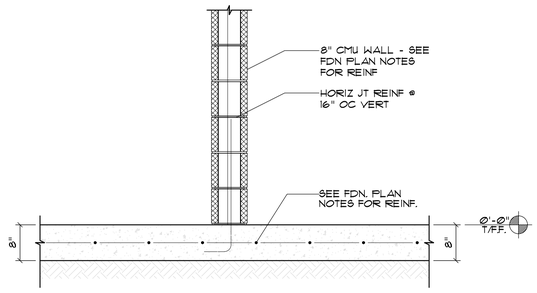
FD1.0



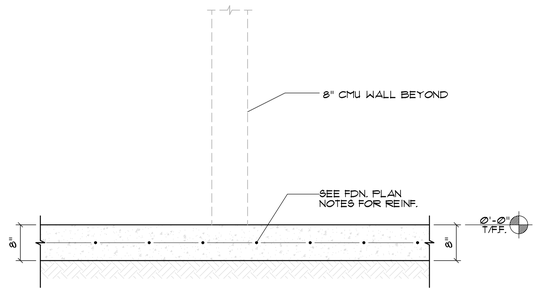
1 FD1.1 TYPICAL FOUNDATION DETAIL AT EXTERIOR CMU WALL
SCALE: 3/4" = 1'



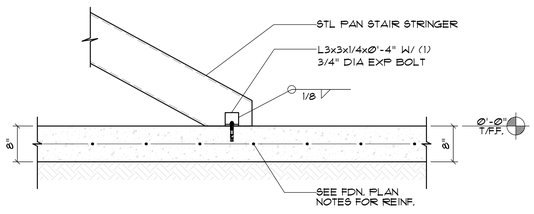
2 FD1.1 TYPICAL FOUNDATION DETAIL AT EXTERIOR CMU WALL OPENING
SCALE: 3/4" = 1'



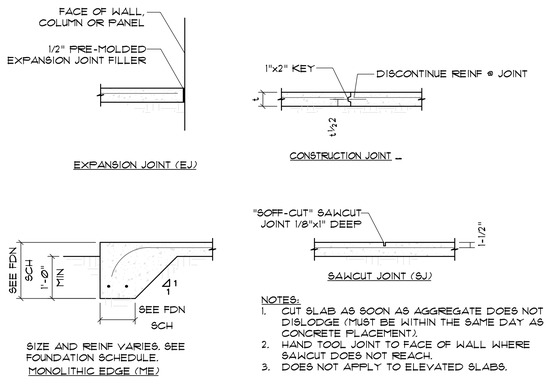
3 FD1.1 SECTION (INTERIOR CMU WALL)
SCALE: 3/4" = 1'



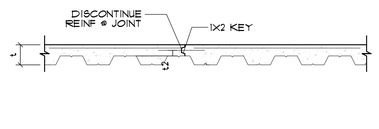
4 FD1.1 SECTION (INTERIOR CMU WALL OPENING)
SCALE: 3/4" = 1'



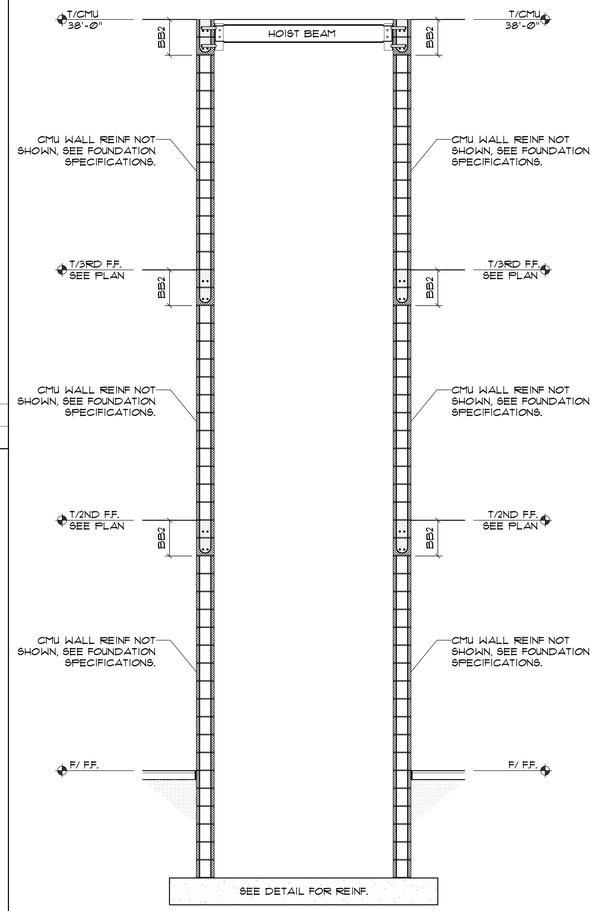
5 FD1.1 SECTION (STAIR STRINGER)
SCALE: 3/4" = 1'



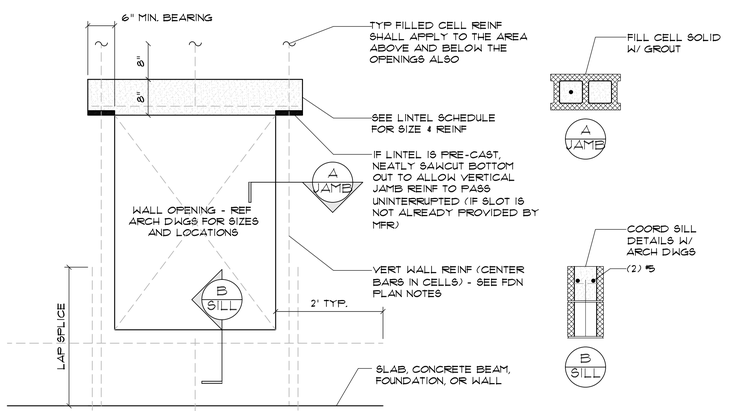
6 FD1.1 DETAILS SLAB ON GRADE
SCALE: 3/4" = 1'



7 FD1.1 DETAILS METAL DECK CONTROL JOINT
SCALE: 3/4" = 1'



10 FD1.1 SECTION @ ELEVATOR SHAFT (FOR 3 STORY)
N.T.S.

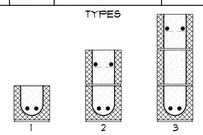


8 FD1.1 TYPICAL MASONRY WALL OPENING ELEVATION
SCALE: 3/4" = 1'

LINTEL SCHEDULE (8" CMU)

TYPE	HEIGHT	REINFORCING	MAX CLEAR SPAN	MIN. CAPACITY (NOTE 1)
1	8x8	(2) #5 CONT	UP TO 6'-Ø"	1150 PLF
2	8x16	(2) #5 CONT T4B	UP TO 10'-Ø"	1450 PLF
3	8x24	(2) #5 CONT T4B	UP TO 14'-Ø"	1600 PLF

TYPES

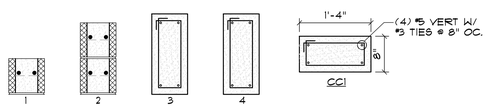


NOTE TO ENG: VERIFY BEAM CAPACITIES IF YOU HAVE FLOOR OR ROOF LOADS WITHIN THE ARCH ACTION AREA.

NOTE:
1. MIN. PB CAPACITY FOR THE INDICATED SPAN MUST BE VERIFIED BY THE PRECAST MFR AND DOCUMENTATION PROVIDED IN THE SUBMITTALS.

MASONRY WALL/CONCRETE BEAM SCHEDULE

MARK	TYPE	NOMINAL SIZE (W"xD")	REINFORCING			STIRRUPS		COMMENTS
			BOT	MID	TOP	SIZE	SPACING	
BB1	1	8x8	--	(2) #5	--	--	--	
BB2	2	8x16	(2) #5	--	(2) #5	--	--	
CB1	3	8x16	(2) #5	--	(2) #5	#3	12" OC	
CB2	4	8x24	(2) #5	(2) #5	(2) #5	#3	12" OC	
TB1	3	8x16	(2) #5	--	(2) #5	#3	24" OC	
TB2	3	8x12(MIN.)	(2) #5	--	(2) #5	#3	24" OC	MATCH COURSING



BEAM TYPES

NOTES:
1. "BB" = BOND BEAM (COMPOSED OF KNOCK-OUT BLOCK)
2. "TB" = TIE BEAM (FORMED AND POURED)
3. "CB" = CONCRETE BEAM (FORMED AND POURED)
4. "CC" = CONCRETE COLUMN (FORMED AND POURED)

9 FD1.1 MASONRY WALL BEAM SCHEDULE
SCALE: 3/4" = 1'

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FD1.1