

THRESHOLD BUILDING STRUCTURAL SPECIAL INSPECTION STATEMENT

STRUCTURAL SPECIAL INSPECTION STATEMENT

THIS STATEMENT OF SPECIAL INSPECTIONS IS SUBMITTED AS A CONDITION FOR PERMIT ISSUANCE IN ACCORDANCE WITH THE SPECIAL INSPECTION AND STRUCTURAL TESTING REQUIREMENTS OF THE BUILDING CODE. IT INCLUDES A *SCHEDULE OF SPECIAL INSPECTION SERVICES* APPLICABLE TO THIS PROJECT. THIS STATEMENT OF SPECIAL INSPECTIONS ENCOMPASSES THE STRUCTURAL DISCIPLINE.

SPECIAL INSPECTOR'S QUALIFICATIONS: THE SPECIAL INSPECTOR SHALL BE CERTIFIED TO ACT AS A SPECIAL INSPECTOR BY THE FLORIDA BOARD OF BUILDING CODES AND STANDARDS OR IS A LICENSED ARCHITECT OR REGISTERED ENGINEER WHO IS CERTIFIED UNDER CHAPTER 471 OR CHAPTER 481 TO CONDUCT INSPECTIONS OF THRESHOLD BUILDINGS.

THE SPECIAL INSPECTOR IS RESPONSIBLE FOR OBSERVING THAT THE CONSTRUCTION OF THE STRUCTURAL SYSTEM COMPLIES WITH THE OFFICIAL CONTRACT DOCUMENTS AND APPROPRIATE BUILDING CODES. THEY SHALL ENSURE THAT THE SHORING OF THE CONSTRUCTION IS IN ACCORDANCE WITH THE SHORING DRAWINGS PROVIDED BY THE CONTRACTOR TO THE ENFORCING AGENCY. THE INTERPRETATION OF THE PERMITTED DOCUMENTS WILL BE PROVIDED BY THE ARCHITECT-OF-RECORD OR ENGINEER-OF-RECORD.

THE SPECIAL INSPECTOR IS RESPONSIBLE TO THE ENFORCING AGENCY.

THE SPECIAL INSPECTOR WILL MEET WITH REPRESENTATIVES OF THE ENFORCING AGENCY, OWNER, CONTRACTOR, SUBCONTRACTORS, TESTING AGENCIES, AND OTHER INTERESTED PARTIES TO REVIEW SPECIFIC THRESHOLD INSPECTION REQUIREMENTS AND PROCEDURES. THIS ORIENTATION WILL BE ACCOMPLISHED AT A STRUCTURAL PRECONSTRUCTION CONFERENCE.

THE SPECIAL INSPECTOR OR HIS AUTHORIZED REPRESENTATIVE WILL VISIT THE PROJECT SITE AS DEMANDED BY THE CONSTRUCTION ACTIVITY TO INSPECT THE VARIOUS STRUCTURAL ELEMENTS DURING THE PROGRESS OF THE WORK, BUT NOT LESS THAN ONCE A WEEK. THE CONTRACTOR SHALL NOTIFY THE SPECIAL INSPECTOR A MINIMUM OF 24 HOURS IN ADVANCE OF ANY CONCRETE POUR OR OTHER CRITICAL CONSTRUCTION ACTIVITY.

THE SPECIAL INSPECTOR WILL REPORT OBSERVATION OF MATERIALS, WORKMANSHIP, OR OTHER FEATURES OF CONSTRUCTION DEVIATING FROM THE REQUIREMENTS OF THE OFFICIAL CONTRACT DOCUMENTS. GUIDELINES FOR OBSERVATIONS COVERING SPECIFIC AREAS OF CONSTRUCTION ARE LISTED WITHIN THE SCHEDULES INCLUDED IN THIS PLAN.

QUALITY CONTROL, INSPECTIONS AND TESTING BY AN INDEPENDENT TESTING AGENCY ARE REQUIRED BY THE SPECIFICATIONS FOR SOME STRUCTURAL ELEMENTS. REPORTS OF THESE INSPECTIONS AND TESTS WILL BE FURNISHED TO THE SPECIAL INSPECTOR TO SUPPLEMENT HIS OWN INSPECTIONS. IT IS THE RESPONSIBILITY OF THE SPECIAL INSPECTOR TO ESTABLISH A CLEAR METHOD FOR MARKING ALL TESTED AND INSPECTED ITEMS.

THE SPECIAL INSPECTOR SHALL IMMEDIATELY NOTIFY THE CONTRACTOR, ENFORCING AGENCY, ARCHITECT-OF-RECORD, AND ENGINEER-OF-RECORD WHEN ANY NONCONFORMING ITEM IS FOUND. THE SPECIAL INSPECTOR SHALL COMPLETE INSPECTION REPORTS AFTER EACH INSPECTION, AND SIGN, SEAL, AND FORWARD EACH REPORT TO THE ENFORCING AGENCY, THE ARCHITECT-OF-RECORD, STRUCTURAL ENGINEER-OF-RECORD, AND CONTRACTOR. REPORTS SHALL BE IN A FORM APPROVED BY THE ENGINEER-OF-RECORD.

AT THE COMPLETION OF ALL STRUCTURAL WORK AND AFTER THE CORRECTION OF ALL DEFICIENCIES, THE SPECIAL INSPECTOR SHALL SUBMIT A SIGNED AND SEALED STATEMENT TO THE BUILDING OFFICIAL CONFORMING TO THE FLORIDA STATUTES. COPIES WILL BE SENT TO THE BUILDING OFFICIAL, OWNER, ARCHITECT-OF-RECORD, STRUCTURAL ENGINEER-OF-RECORD, AND CONTRACTOR.

JOB SITE SAFETY AND MEANS AND METHODS OF CONSTRUCTION ARE SOLELY THE RESPONSIBILITY OF THE CONTRACTOR.

THIS STATEMENT OF SPECIAL INSPECTIONS IS NOT INTENDED TO INCORPORATE ALL ASPECTS OF THE SPECIAL INSPECTION PROGRAM AS DEFINED IN THE SPECIAL INSPECTION AND STRUCTURAL TESTING REQUIREMENTS OF THE BUILDING CODE. REFER TO OTHER DISCIPLINES FOR REMAINING SPECIAL INSPECTIONS THAT MAY BE REQUIRED.

THE STRUCTURAL ENGINEER OF RECORD IS NOT RESPONSIBLE FOR ENSURING THE SPECIAL INSPECTION PROGRAM IS CONDUCTED IN ACCORDANCE WITH THIS PLAN.

THIS STATEMENT OF SPECIAL INSPECTIONS INCLUDES THE FOLLOWING INSPECTION CATEGORIES:

☒ FABRICATORS

☒ SOILS

☒ DRIVEN DEEP FOUNDATIONS

☒ CAST-IN-PLACE DEEP FOUNDATIONS

☒ HELICAL PILE FOUNDATIONS

☒ CONCRETE CONSTRUCTION

☐ MASONRY CONSTRUCTION - LEVEL 1

☐ MASONRY CONSTRUCTION - LEVEL 2

☐ MASONRY CONSTRUCTION - LEVEL 3

☐ STRUCTURAL STEEL CONSTRUCTION

☐ OPEN-WEB STEEL JOISTS AND JOIST ORDERS

☐ COLD-FORMED STEEL DECK

☐ COLD-FORMED STEEL TRUSSES

☐ COLD-FORMED STEEL LIGHT FRAME

☐ CONSTRUCTION

☐ WOOD CONSTRUCTION

| STRUCTURAL SPECIAL INSPECTION SCHEDULE: FABRICATORS | | | | |
|-----------------------------------------------------|-----------------------------|------------|---------------------|---|
| VERIFICATION AND INSPECTION TASK | APPLICABLE TO THIS PROJECT? | FREQUENCY | REFERENCED STANDARD | |
| | | CONTINUOUS | PERIODIC | |
| VERIFY FABRICATION AND IMPLEMENTATION PROCEDURES | | | | |
| STEEL CONSTRUCTION | X | - | X | - |
| CONCRETE CONSTRUCTION (REBAR FABRICATION) | X | - | X | - |
| WOOD CONSTRUCTION | - | - | X | - |
| COLD FORM METAL CONSTRUCTION | - | - | X | - |
| OTHER CONSTRUCTION | - | - | X | - |

| STRUCTURAL SPECIAL INSPECTION SCHEDULE: SOILS | | | | |
|-------------------------------------------------------------------------------------------------------------------|-----------------------------|------------|---------------------|---|
| VERIFICATION AND INSPECTION TASK | APPLICABLE TO THIS PROJECT? | FREQUENCY | REFERENCED STANDARD | |
| | | CONTINUOUS | PERIODIC | |
| VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY. | | | | |
| VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL. | X | - | X | - |
| PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS. | X | - | X | - |
| VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL. | X | X | - | - |
| PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY. | X | - | X | - |

STRUCTURAL SPECIAL INSPECTION SCHEDULE: STRUCTURAL STEEL CONSTRUCTION

| VERIFICATION AND INSPECTION TASK | APPLICABLE TO THIS PROJECT? | QC | QA | AISC REFERENCE |
|-----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|-----------------------------|----|----|----------------|
| INSPECTION TASKS PRIOR TO WELDING | | | | |
| WELDER QUALIFICATION RECORDS AND CONTINUITY RECORDS | X | P | O | |
| WPS AVAILABLE | X | P | P | |
| MATERIAL CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE | X | P | P | |
| MATERIAL IDENTIFICATION (TYPE / GRADE) | X | O | O | |
| WELDER IDENTIFICATION SYSTEM | X | O | O | |
| FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY) | | | | |
| • JOINT PREPARATIONS | | | | |
| • DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL) | X | O | O | |
| • CLEANLINESS (CONDITION OF STEEL SURFACES) | | | | |
| • TACKING (TACK WELD QUALITY AND LOCATION) | | | | |
| • BACKING TYPE AND FIT (IF APPLICABLE) | | | | |
| FIT-UP OF CUP GROOVE WELDS OF HSS T-, Y- AND K-JOINTS WITHOUT BACKING (INCLUDING JOINT GEOMETRY) | | | | |
| • JOINT PREPARATIONS | X | P | O | |
| • DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL) | | | | |
| • CLEANLINESS (CONDITION OF STEEL SURFACES) | | | | |
| • TACKING (TACK WELD QUALITY AND LOCATION) | | | | |
| CONFIGURATION AND FINISH OF ACCESS HOLES | X | O | O | |
| FIT-UP OF FILLET WELDS | | | | |
| • DIMENSIONS (ALIGNMENT, GAPS AT ROOT) | X | O | O | |
| • CLEANLINESS (CONDITION OF STEEL SURFACES) | | | | |
| • TACKING (TACK WELD QUALITY AND LOCATION) | | | | |
| CHECK WELDING EQUIPMENT | X | O | - | |
| INSPECTION TASKS DURING WELDING | | | | |
| CONTROL AND HANDLING OF WELDING CONSUMABLES | | | | |
| • PACKAGING | X | O | O | |
| • EXPOSURE CONTROL | | | | |
| NO WELDING OVER CRACKED TACK WELDS | X | O | O | |
| ENVIRONMENTAL CONDITIONS | | | | |
| • WIND SPEED WITHIN LIMITS | X | O | O | |
| • PRECIPITATION AND TEMPERATURE | | | | |
| WPS FOLLOWED | | | | |
| • SETTING OF WELDING EQUIPMENT | | | | |
| • TRAVEL SPEED | | | | |
| • SELECTED WELDING MATERIALS | X | O | O | |
| • SHIELDING GAS TYPE / FLOW RATE | | | | |
| • PREHEAT APPLIED | | | | |
| • INTERPASS TEMPERATURE MAINTAINED (MIN. / MAX.) | | | | |
| • PROPER POSITION (F, V, H, OH) | | | | |
| WELDING TECHNIQUES | | | | |
| • INTERPASS AND FINAL CLEANING | X | O | O | |
| • EACH PASS WITHIN PROFILE LIMITATIONS | | | | |
| • EACH PASS MEETS QUALITY REQUIREMENTS | | | | |
| PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS | X | P | P | |
| INSPECTION TASKS AFTER WELDING | | | | |
| WELDS CLEANED | X | O | O | |
| SIZE, LENGTH AND LOCATION OF WELDS | X | P | P | |
| WELDS MEET VISUAL ACCEPTANCE CRITERIA | | | | |
| • CRACK PROHIBITION | | | | |
| • WELD / BASE-METAL FUSION | X | P | P | |
| • CRATER CROSS SECTION | | | | |
| • WELD PROFILES | | | | |
| • WELD SIZE | | | | |
| • UNDERCUT | | | | |
| • POROSITY | | | | |
| ARC STRIKES | X | P | P | |
| K-AREA - WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS IN RISK CATEGORY III OR IV, UT ALL COMPLETE-JOINT-PENETRATION (CJP) WELDS IN MATERIALS 5/16" OR THICKER. | X | P | P | |
| STRUCTURES IN RISK CATEGORY II, UT ALL COMPLETE-JOINT-PENETRATION (CJP) WELDS IN MATERIALS 5/16" OR THICKER. | X | P | P | |
| STRUCTURES IN RISK CATEGORY III OR IV, UT ALL COMPLETE-JOINT-PENETRATION (CJP) WELDS IN MATERIALS 5/16" OR THICKER. | X | P | P | |
| NONDESTRUCTIVE TESTING OF WELDED JOINTS | | | | |
| STRUCTURES IN RISK CATEGORY III OR IV, UT ALL COMPLETE-JOINT-PENETRATION (CJP) WELDS IN MATERIALS 5/16" OR THICKER. | X | P | P | |
| STRUCTURES IN RISK CATEGORY II, UT ALL COMPLETE-JOINT-PENETRATION (CJP) WELDS IN MATERIALS 5/16" OR THICKER. | X | P | P | |
| STRUCTURES IN RISK CATEGORY III OR IV, UT ALL COMPLETE-JOINT-PENETRATION (CJP) WELDS IN MATERIALS 5/16" OR THICKER. | X | P | P | |
| INSPECTION TASKS PRIOR TO BOLTING | | | | |
| MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS | X | O | P | |
| FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS | X | O | O | |
| CORRECT FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE) | X | O | O | |
| CORRECT BOLTING PROCEDURE SELECTED FOR JOINT DETAIL | X | O | O | |
| CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS | X | O | O | |
| PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED | X | P | O | |
| PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS | X | O | O | |
| INSPECTION TASKS DURING BOLTING | | | | |
| FASTENER ASSEMBLIES PLACED IN ALL HOLES AND WASHERS AND NUTS ARE POSITIONED AS REQUIRED | X | O | O | |
| JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION | X | O | O | |
| FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING | X | O | O | |
| FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES | X | O | O | |
| INSPECTION TASKS AFTER BOLTING | | | | |
| DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS | X | P | P | |

J. ELEVATORS

1. THE BASIS OF DESIGN FOR THE ELEVATOR SUPPORTS AND SHAFT IS OTIS LVM 4500 HOLED HYDRAULIC 4,500 LB CAPACITY ELEVATOR.

2. THE CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER IF FINAL ELEVATOR SELECTION DIFFERS FROM THE BASIS OF DESIGN.

3. THE STRUCTURE HAS BEEN DESIGNED TO SUPPORT THE FOLLOWING ELEVATOR REACTIONS. CONTRACTOR SHALL NOTIFY STRUCTURAL ENGINEER FOR ANY DEVIATIONS FROM THE LOADS LISTED.

HOIST BEAM

7,500 LB

ELEVATOR MACHINE SUPPORT

SEE PLAN

PIT FLOOR BUFFER REACTIONS

XX,XXX LB

PIT FLOOR RAIL REACTIONS

XX,XXX LB

REACTION AT RAIL SUPPORTS

XXX LB

4. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS DEPICTING SUPPORT REACTIONS, RAIL LOADS, MAXIMUM GUIDE RAIL SPANS, PIT DEPTH, REQUIRED SHAFT DIMENSIONS AND CLEARANCES, AND OVERHEAD CLEARANCES.

5. ELEVATOR GUIDE RAIL SUPPORTS

5.1 ELEVATOR GUIDE RAIL SUPPORT TUBES HAVE NOT BEEN INCLUDED IN THE CONTRACT DOCUMENTS FOR FLOOR HEIGHTS OF 11'-00" OR LESS. THE ELEVATOR RAILS SUPPLIED BY THE ELEVATOR SUPPLIER SHALL SPAN 11'-00" WITHOUT ADDITIONAL SUPPORT. IF THE RAILS PROVIDED CANNOT SPAN THE REQUIRED DISTANCE BETWEEN FLOORS, THE CONTRACTOR SHALL SUPPLY ADEQUATE SUPPORT IN ACCORDANCE WITH ANSI/ASME A17.1 DESIGNED BY AN ENGINEER LICENSED IN THE PROJECT STATE. SUBMIT SHOP DRAWINGS CLEARLY INDICATING THE GUIDE RAIL SUPPORTS, THE DESIGN CRITERIA USED, AND THE ATTACHMENT TO THE STRUCTURE.

5.2 ELEVATOR GUIDE RAIL SUPPORT TUBES HAVE BEEN INCLUDED IN THE CONTRACT DOCUMENTS FOR FLOOR HEIGHTS OF XX'-XX" OR GREATER. IF THE ELEVATOR RAILS PROVIDED CAN SPAN THE REQUIRED DISTANCE WITHOUT ADDITIONAL SUPPORT, THE ELEVATOR RAIL SUPPORT TUBES MAY BE OMITTED.

K. POST-INSTALLED ANCHORS AND REINFORCING STEEL

1. POST-INSTALLED ANCHORS AND REINFORCING STEEL SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER PRIOR TO INSTALLING POST-INSTALLED ANCHORS OR REINFORCING STEEL IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS OR REINFORCING STEEL.

2. ANCHORS AND REINFORCING STEEL SHALL BE INSTALLED PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPI).

3. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE SPECIFIED BELOW SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER ALONG WITH CALCULATIONS THAT ARE SIGNED AND SEALED BY A LICENSED PROFESSIONAL ENGINEER. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING EQUIVALENT PERFORMANCE VALUES (MINIMUM) OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S) AS REQUIRED BY THE BUILDING CODE.

4. THE CONTRACTOR SHALL ARRANGE ONSITE INSTALLATION TRAINING BY THE MANUFACTURER FOR EACH PRODUCT TO BE INSTALLED. SUBMIT TO THE STRUCTURAL ENGINEER DOCUMENTATION CONFIRMING TRAINING OF ALL PERSONNEL WHO WILL BE INSTALLING PRODUCTS. TRAINING AND DOCUMENTATION SHALL OCCUR PRIOR TO COMMENCEMENT OF PRODUCT INSTALLATION. INSTALLATION OF ADHESIVE ANCHOR PRODUCTS IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATION RESISTING SUSTAINED TENSION LOADS SHALL BE CONDUCTED BY AN INSTALLER CERTIFIED IN ACCORDANCE WITH THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM. PROOF OF CERTIFICATION SHALL BE MAINTAINED AT THE JOB SITE.

5. ANCHOR CAPACITY IS DEPENDENT UPON SPACING BETWEEN ADJACENT ANCHORS, CONCRETE STRENGTH AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS. IF NO SPACING OR EDGE DISTANCES ARE SPECIFIED ON THE STRUCTURAL DRAWINGS, REFER TO APPLICABLE EVALUATION REPORT FOR CRITICAL SPACING AND EDGE DISTANCES.

6. EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR OR REINFORCING LOCATIONS. THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TO LOCATE THE POSITION OF THE REINFORCING BARS AT THE LOCATIONS OF THE CONCRETE ANCHORS OR REINFORCING, BY FERROSCAN, GPR, X-RAY, CHIPPING OR OTHER MEANS IN ORDER TO AVOID CONFLICT WITH INSTALLATION. THE CONTRACTOR SHALL NOT DAMAGE ANY REINFORCING STEEL PRIOR TO CONSULTING WITH THE STRUCTURAL ENGINEER.

7. UNLESS NOTED OTHERWISE, THE MINIMUM EMBEDMENT OF EXPANSION AND SCREW ANCHORS SHALL BE 6 TIMES THE ANCHOR DIAMETER, THE MINIMUM EMBEDMENT FOR ADHESIVE ANCHORS SHALL BE 12 TIMES THE ANCHOR DIAMETER, AND THE MINIMUM EMBEDMENT FOR ADHESIVE REINFORCING SHALL BE 12 BAR DIAMETERS.

8. ADHESIVE ANCHOR INSERT SHALL BE:

INTERIOR NON-CORROSIVE ENVIRONMENTS: ASTM F1554 Gr. 36 ALL-THREAD ROD

EXTERIOR NON-CORROSIVE ENVIRONMENTS: ASTM F1554 Gr. 36 ALL-THREAD ROD

HOT-DIPPED GALVANIZED PER ASTM B695, CLASS 65, TYPE I

CORROSIVE ENVIRONMENTS: ASTM A193 Gr. B8M TYPE 316 ALL-THREAD ROD

9. MECHANICAL AND SCREW ANCHORS IN EXTERIOR NON-CORROSIVE ENVIRONMENTS SHALL BE HOT-DIPPED GALVANIZED PER ASTM B695, CLASS 65, TYPE I

10. ADHESIVE ANCHORS AND REINFORCING STEEL CAPACITY IS DEPENDENT UPON INSTALLATION CONDITIONS. THE FOLLOWING INSTALLATION CONDITIONS HAVE BEEN ASSUMED:

HOLES DRILLED WITH HAMMER DRILL WITH CARBIDE TIPPED DRILL BIT

DRY HOLE

CONCRETE CURED FOR A MINIMUM OF 21 DAYS

TEMPERATURE CATEGORY B (110 DEG. F LONG TERM AND 130 DEG. F SHORT TERM)

IF THESE CONDITIONS DO NOT EXIST, THE CONTRACTOR SHALL SUBMIT ALTERNATE DRILLING METHODS AND MANUFACTURER BOND DATA FOR THE CONDITIONS BEING INSTALLED.

K. POST-INSTALLED ANCHORS AND REINFORCING STEEL (CONT.)

11. POST-INSTALLED ANCHORS AND REINFORCING INSTALLED INTO CONCRETE SHALL BE AS INDICATED BELOW.

11.1 MECHANICAL AND SCREW ANCHORS FOR USE IN CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN CRACKED CONCRETE IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193.

11.1.1 EXPANSION ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.:
SIMPSON STRONG-TIE STRONG-BOLT 2 (ICC ESR-3037)
HILTI KWIK BOLT-TZ2 (ICC ESR-4266)
DEWALT POWER-STUD® S02 (ICC ESR-2502)

11.1.2 SCREW ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.:
SIMPSON STRONG-TIE TITEN HD (ICC ESR-2713)
HILTI KWIK HUS-EZ (ICC ESR-3027)
DEWALT SCREW-BOLT+ (ICC ESR-3889)

11.1.3 SHALLOW EMBEDMENT ANCHORS (<3/4") SHALL BE ONE OF THE FOLLOWING, U.N.O.:
HILTI HDI-P-TZ (ICC ESR-4236)
DEWALT MINI-UNDERCUT+ (ICC ESR-3912)

11.2 ADHESIVE ANCHORS FOR USE IN CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN CRACKED CONCRETE IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308 AND THE ICC-ES EVALUATION SERVICES REPORT SHALL SHOW COMPLIANCE WITH THE BUILDING CODE STATED IN SECTION 8 OF THESE STRUCTURAL NOTES.

11.2.1 ADHESIVE ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.:
SIMPSON STRONG-TIE SET-XP (ICC ESR-2508)
HILTI HIT-RE 500 v3 SLOW CURE (ICC ESR-3814)
HILTI HIT-HY 200 FAST CURE (ICC ESR-3187)
DEWALT PURE110+ (ICC ESR-3298)

11.3 ADHESIVE FOR INSTALLING REINFORCING STEEL IN EXISTING CONCRETE SHALL BE QUALIFIED IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308 AND THE ICC-ES EVALUATION SERVICES REPORT SHALL SHOW COMPLIANCE WITH THE BUILDING CODE STATED IN SECTION 8 OF THESE STRUCTURAL NOTES.

11.3.1 ADHESIVE SHALL BE ONE OF THE FOLLOWING, U.N.O.:
SIMPSON STRONG-TIE "SET-XP" (ICC-ES ESR-2508)
HILTI HIT-RE 500 v3 SLOW CURE (ICC ESR-3814)
DEWALT PURE110+ (ICC ESR-3298)

12. POST-INSTALLED ANCHORS INSTALLED INTO SOLID GROUTED CONCRETE MASONRY CELLS SHALL BE AS INDICATED BELOW. DO NOT INSTALL ANCHORS INTO HOLLOW HEAD JOINTS OR TEE JOINTS BETWEEN MASONRY BLOCKS.

12.1 MECHANICAL AND SCREW ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC01 AND AC106, RESPECTIVELY.

12.1.1 EXPANSION ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.:
SIMPSON STRONG-TIE STRONG-BOLT2 (IAPMO-UES ER-240)
HILTI KWIK BOLT-TZ2 (ICC ESR-4561)
DEWALT POWER-STUD® SD1 (ICC ESR-2966)

12.1.2 SCREW ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.:
SIMPSON STRONG-TIE TITEN HD (ICC ESR-1056)
HILTI KWIK HUS-EZ (ICC ESR-3056)
DEWALT SCREW-BOLT+ (ICC-ES ESR-4042)

12.2 ADHESIVE ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC58

12.2.1 ADHESIVE ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.:
SIMPSON STRONG-TIE "SET-XP" (IAPMO-UES ER-265)
HILTI HIT-HY 270 (ICC ESR-4143)
DEWALT AC100+ GOLD (ICC ESR-3200)

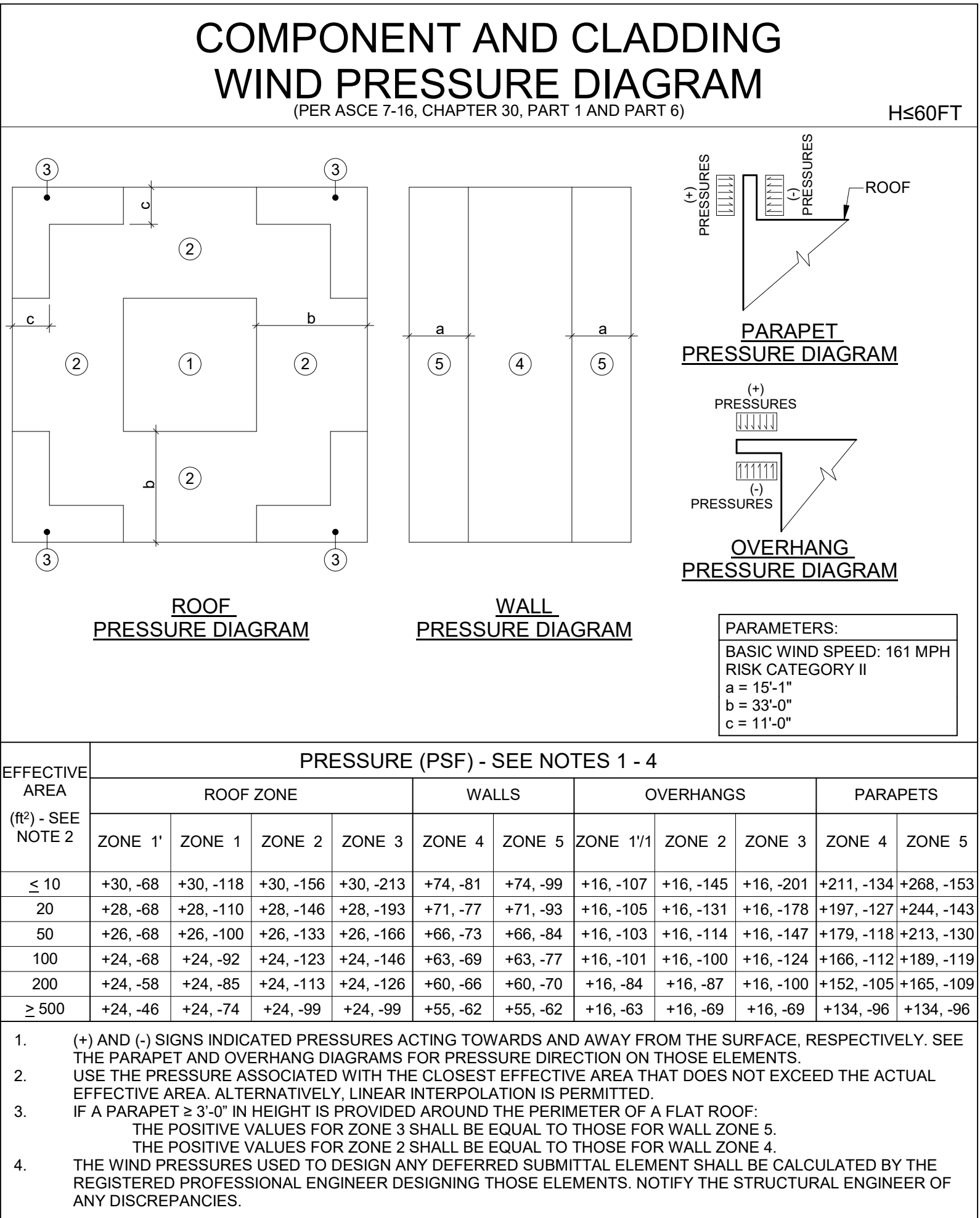
13. POST-INSTALLED ANCHORS INSTALLED INTO HOLLOW CONCRETE MASONRY SHALL BE AS INDICATED BELOW:

13.1 SCREW ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC106.

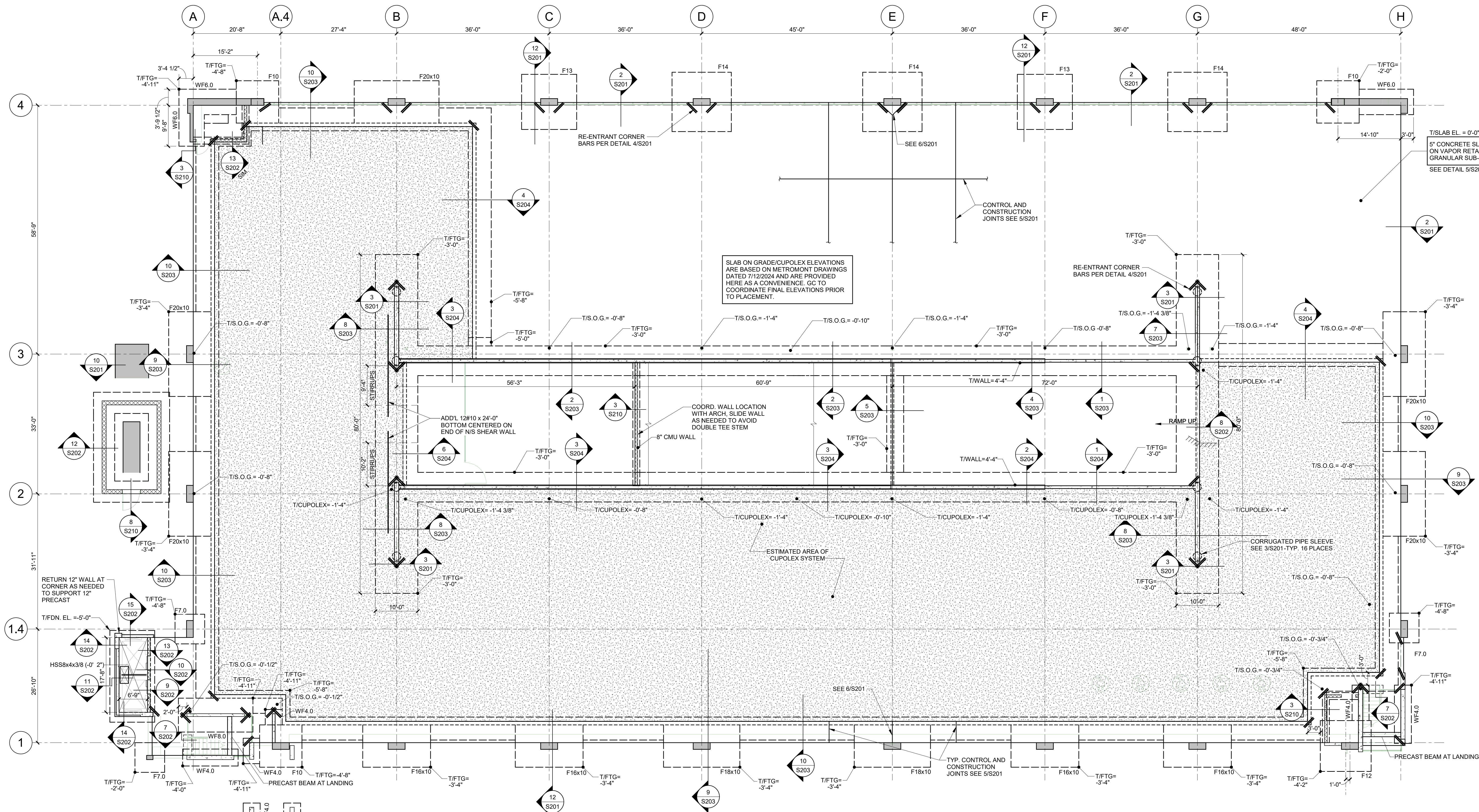
13.1.1 SCREW ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.:
DEWALT TAPPER + (ICC ESR 3196)
SIMPSON STRONG-TIE TITEN-HD (ICC ESR-1056)
HILTI KWIK-CON II (ICC ESR-NOT AVAILABLE)

13.2 ADHESIVE ANCHORS WITH SCREEN TUBES SHALL BE TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC58 OR AC60, AS APPROPRIATE. SCREEN TUBES SHALL BE USED AS RECOMMENDED BY THE ADHESIVE MANUFACTURER.

13.2.1 ADHESIVE ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.:
SIMPSON STRONG-TIE "SET-XP" (IAPMO-UES ER-265)
HILTI HIT-HY 270 (ICC ESR-4143)
DEWALT AC100+ GOLD (ICC ESR-3200)



DRAWING FILE: A:\030324 - HCA Florida St. Lucie Hospital Parking Garage\24024-HCA_Florida_St_Lucie_Parking_Garage_SDL1221.rvt
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LEVEL P1 FOUNDATION PLAN

0 6' 0" 12' 0" 24' 0"

- NOTES:**
- REFERENCE ELEVATION 0' = 15.80' (REFER TO CIVIL DRAWINGS).
 - TOP OF FOOTING ELEVATIONS SHALL BE AS FOLLOWS, UNLESS NOTED OTHERWISE ON PLAN:
EXTERIOR AND INTERIOR FOUNDATIONS: 2'-0" BELOW TOP OF SLAB-ON-GRADE.
 - FOUNDATION ELEMENTS ARE CENTERED UNDER COLUMNS, UNLESS NOTED OTHERWISE.
 - REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN.
 - FOR PARKING LEVELS:** TOP OF SLAB ELEVATIONS ARE APPROXIMATE. REFER TO ARCHITECTURAL DRAWINGS FOR TOP OF SLAB ELEVATIONS, SLAB SLOPE AND EDGE OF SLAB DIMENSIONS.
 - REFER TO ARCHITECTURAL DRAWINGS FOR LOCATION AND EXTENTS OF NON-LOAD BEARING CMU WALLS.
 - ELEVATOR BRACING AND RAIL SUPPORT REQUIREMENTS OF THE ELEVATOR BEING SUPPLIED SHALL BE COORDINATED BY THE GENERAL CONTRACTOR. NOTIFY THE STRUCTURAL ENGINEER OF ANY INCONSISTENCIES.
 - CONTRACTOR SHALL FIELD VERIFY EXISTING CONDITIONS AND DIMENSIONS RELATED TO THE NEW STRUCTURE PRIOR TO FABRICATION AND CONSTRUCTION.
 - SHEET AND TYPICAL DETAIL REFERENCES:
S0 SERIES: STRUCTURAL NOTES
1/S201: FOOTING SCHEDULE
1/S202: FOOTING STEP
S201: SLAB ON GRADE
S301: NON-LOAD BEARING CMU WALLS (INCLUDING THICKENED SLAB REQUIREMENTS)
10/S201: HOUSEKEEPING PADS
 - MARK AND LEGEND KEY:
F# #: FOOTING
SW# #: SHEAR WALL
STEP: FOOTING STEP
FTG.: HOUSEKEEPING PAD
WATER RETENTION SYSTEM
CMU WALLS

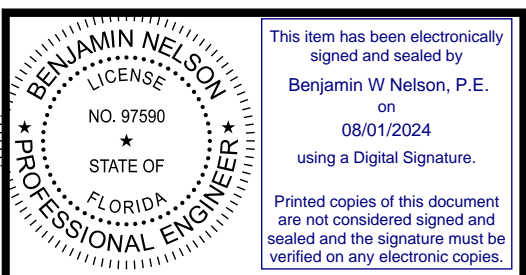
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|-------------------------------------------------------------------|----------------------------------------------------|
| GENERAL CONTRACTOR BRASFIELD & GORRIE WALKER COX | PRECAST CONCRETE METROMONT JIM BACHER |
| HCA DESIGN MANAGER: RYAN ROHE | HCA CONSTRUCTION MANAGER: GREG ATKINSON |



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Architecture Interiors Planning

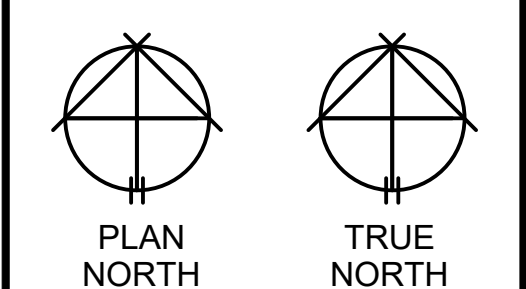
211 Franklin Road, Suite 200
Brentwood, TN 37027-5593
615.377.9773 Office
www.TMPartners.com

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PARKING GARAGE
1800 SE TIFFANY AVE, PORT ST. LUCIE, FL 34952
HCA Project No. 3099600023

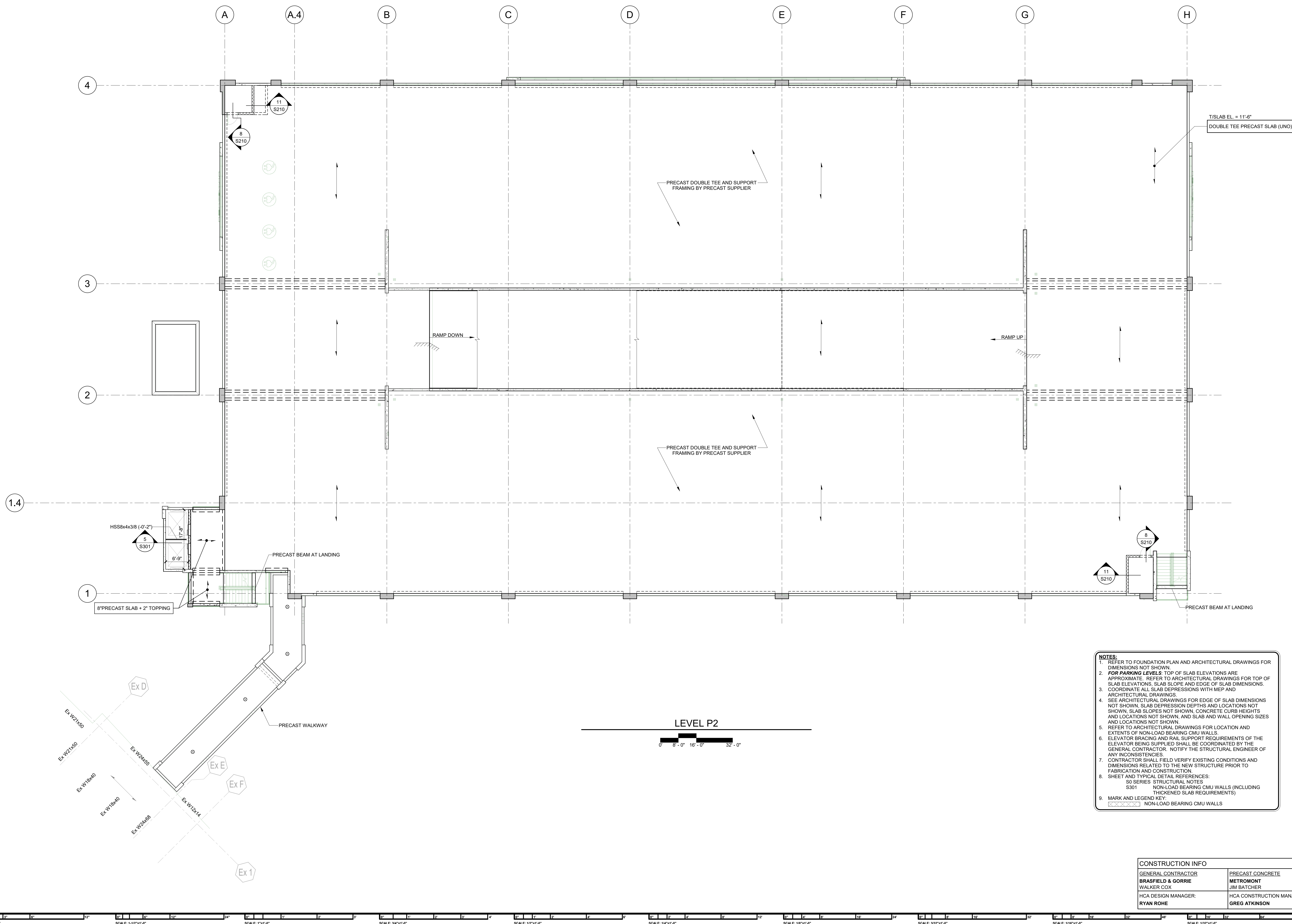


| REVISIONS | |
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DR. BY: CG
CK. BY: BN
PROJ.: A00324
DATE: 07/30/2024

FOUNDATION PLAN

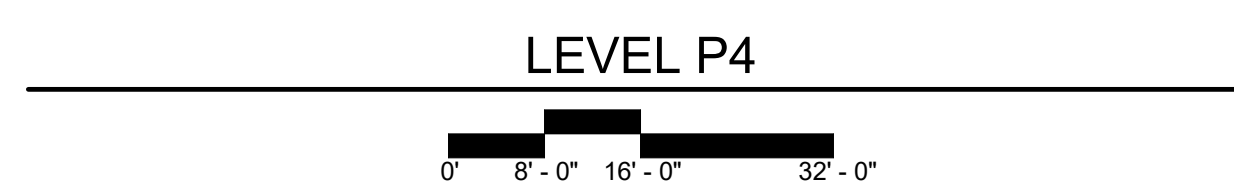
S101





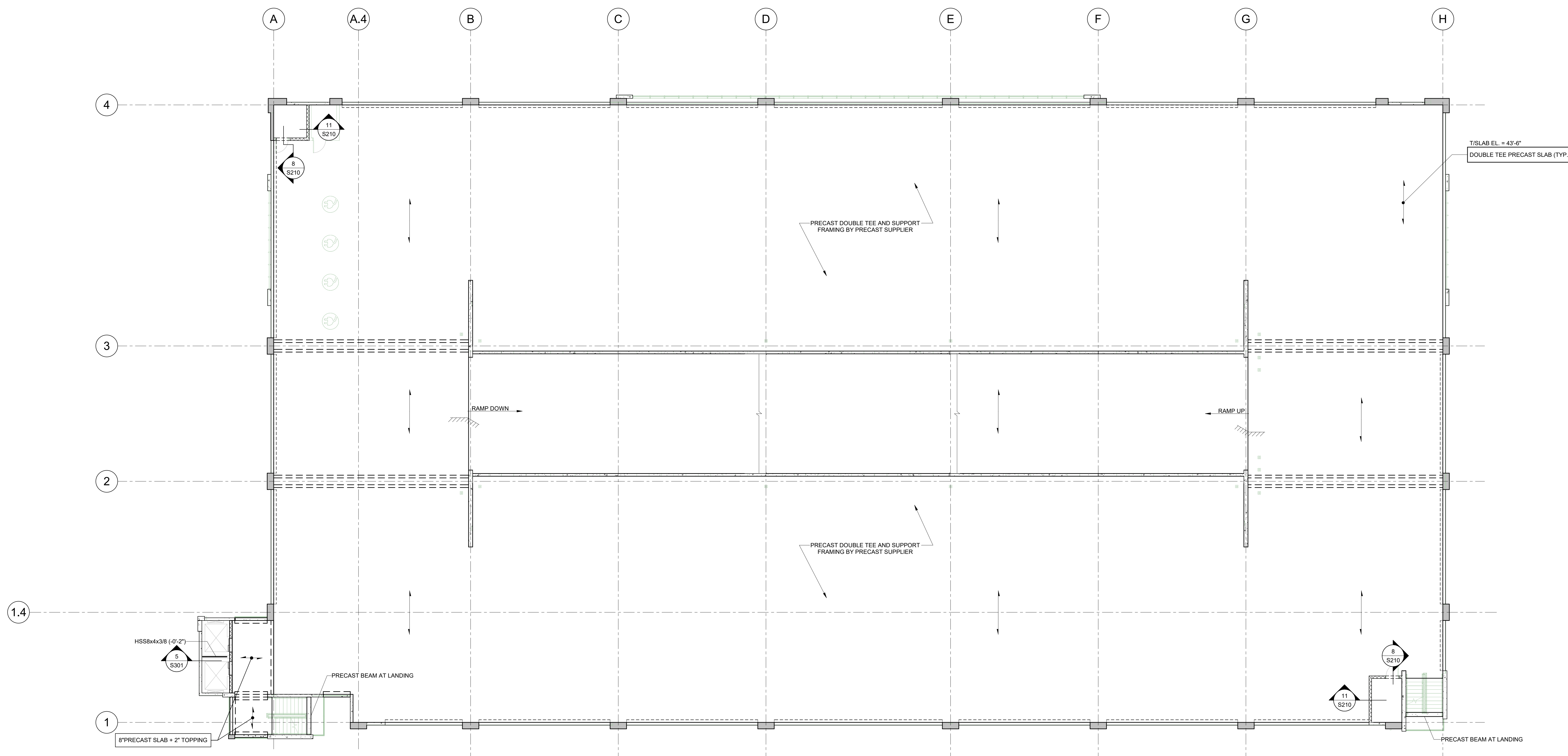
- NOTES:**
- 1. REFER TO FOUNDATION PLAN AND ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN.
 - 2. **FOUNDING LEVEL** OF ALL SLAB ELEVATIONS ARE APPROXIMATE. REFER TO ARCHITECTURAL DRAWINGS FOR TOP OF SLAB ELEVATIONS, SLAB SLOPE AND EDGE OF SLAB DIMENSIONS.
 - 3. COORDINATE ALL SLAB DEPRESSIONS WITH MEP AND ARCHITECTURAL DRAWINGS.
 - 4. SEE ARCHITECTURAL DRAWINGS FOR EDGE OF SLAB DIMENSIONS NOT SHOWN, SLAB DEPRESSION DEPTHS AND LOCATIONS NOT SHOWN, SLAB SLOPE NOT SHOWN, CONCRETE CURB HEIGHTS AND LOCATIONS NOT SHOWN, AND SLAB AND WALL OPENING SIZES AND LOCATIONS NOT SHOWN.
 - 5. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATION AND EXTENTS OF NON-LOAD BEARING CMU WALLS.
 - 6. ELEVATOR BRACING AND RAIL SUPPORT REQUIREMENTS OF THE ELEVATOR SUPPLIER SHALL BE FULLY COMMANDED BY THE GENERAL CONTRACTOR. NOTIFY THE STRUCTURAL ENGINEER OF ANY INCONSISTENCIES.
 - 7. CONTRACTOR SHALL FIELD VERIFY EXISTING CONDITIONS AND DISCREPANCIES RELATED TO NEW STRUCTURE PRIOR TO FABRICATION AND CONSTRUCTION.
 - 8. SHEET AND TYPICAL DETAIL REFERENCES:
 - SO SERIES STRUCTURAL DRAWINGS
 - S301 NON-LOAD BEARING CMU WALLS (INCLUDING THICKENED SLAB REQUIREMENTS)
 - 9. MARK AND LEGEND KEY:
 - █ NON-LOAD BEARING CMU WALLS

| CONSTRUCTION INFO | |
|--------------------------------------------------------------------------|------------------------------------------------------------|
| <u>GENERAL CONTRACTOR</u> BRASFIELD & GORRIE WALKER COX | <u>PRECAST CONCRETE</u> METROMONT JIM BATCHER |
| HCA DESIGN MANAGER: RYAN ROHE | HCA CONSTRUCTION MANAGER: GREG ATKINSON |




- NOTES:**
1. REFER TO FOUNDATION PLAN AND ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN.
 2. FORMWORK LEVEL OF SLAB ELEVATIONS ARE APPROXIMATE. REFER TO ARCHITECTURAL DRAWINGS FOR TOP OF SLAB ELEVATIONS, SLAB SLOPE AND EDGE OF SLAB DIMENSIONS.
 3. COORDINATE ALL SLAB DEPRESSIONS WITH MEP AND ARCHITECTURAL DRAWINGS.
 4. SEE ARCHITECTURAL DRAWINGS FOR EDGE OF SLAB DIMENSIONS NOT SHOWN. SLAB DEPRESSION DEPTHS AND LOCATIONS NOT SHOWN. SLAB SLOPES NOT SHOWN. CONCRETE CURB HEIGHTS AND LOCATIONS NOT SHOWN, AND SLAB AND WALL OPENING SIZES AND LOCATIONS NOT SHOWN.
 5. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATION AND EXTENTS OF NON-LOAD BEARING CMU WALLS.
 6. ELEVATOR BRACING AND RAIL SUPPORT REQUIREMENTS OF THE ELEVATOR ARE SUBJECT TO THE REQUIREMENTS SPECIFIED BY THE GENERAL CONTRACTOR. NOTIFY THE STRUCTURAL ENGINEER OF ANY INCONSISTENCIES.
 7. CONTRACTOR SHALL FIELD VERIFY EXISTING CONDITIONS AND DIMENSIONS RELATED TO THE NEW STRUCTURE PRIOR TO FABRICATION AND CONSTRUCTION.
 8. SHEET AND TYPICAL DETAIL REFERENCES:
 - SO SERIES STRUCTURAL NOTES
 - S301 NON-LOAD BEARING CMU WALLS (INCLUDING THICKENED SLAB REQUIREMENTS)
 9. MARK AND LEGEND KEY:
 - █ NON-LOAD BEARING CMU WALLS

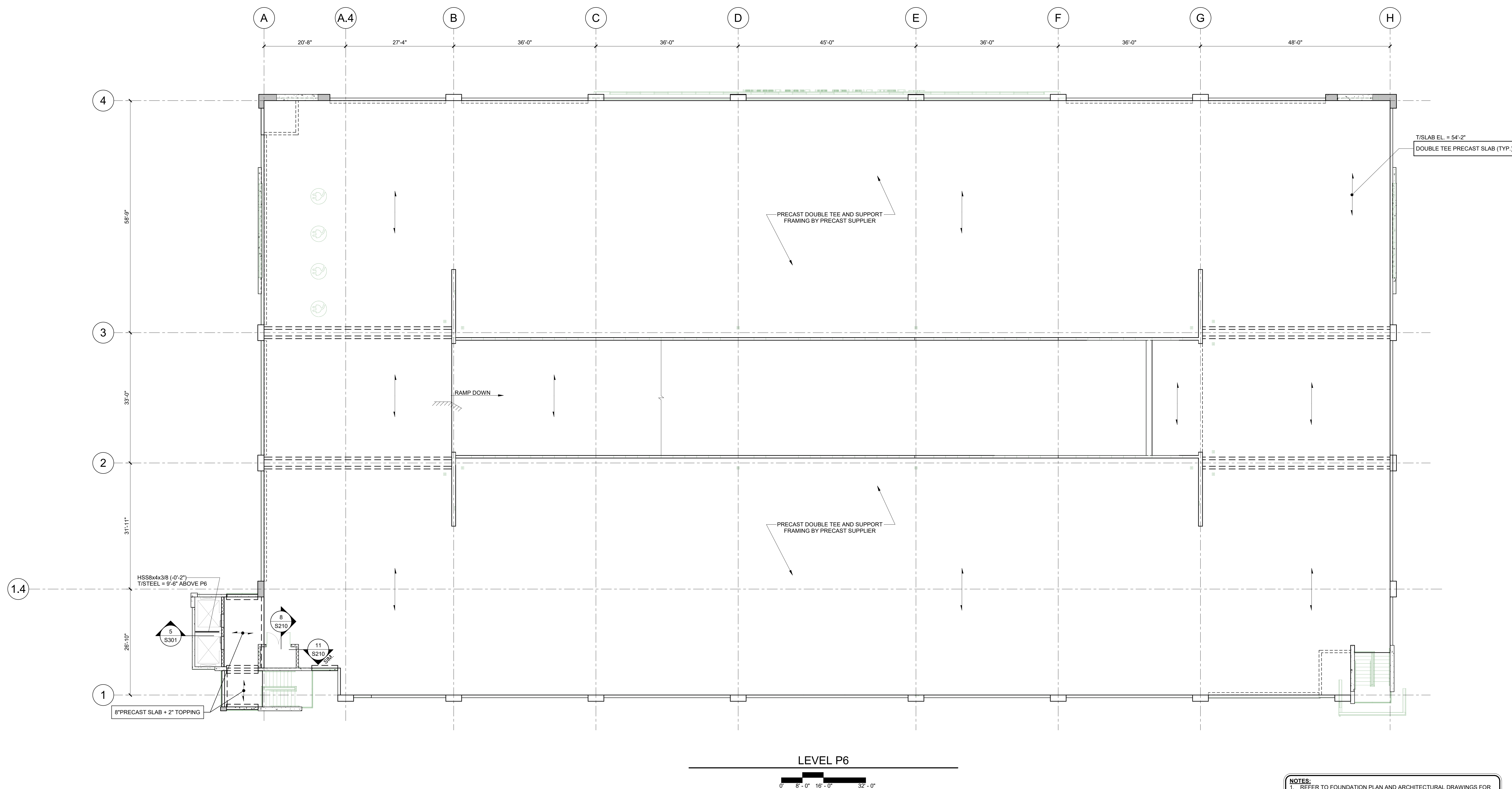
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| <u>GENERAL CONTRACTOR</u> BRASFIELD & GORRIE WALKER COX | <u>PRECAST CONCRETE</u> METROMONT JIM BATCHER |
| HCA DESIGN MANAGER: RYAN ROHE | HCA CONSTRUCTION MANAGER: GREG ATKINSON |



1. **NOTES:**
REFER TO FOUNDATION PLAN AND ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN.
2. **FOR PARKING LEVELS:** TOP OF SLAB ELEVATIONS ARE APPROXIMATE. REFER TO ARCHITECTURAL DRAWINGS FOR TOP OF SLAB ELEVATIONS, SLAB SLOPE AND EDGE OF SLAB DIMENSIONS.
3. **COORDINATE** ALL SLAB DEPRESSIONS WITH MEP AND ARCHITECTURAL DRAWINGS.
4. **SEE ARCHITECTURAL DRAWINGS** FOR EDGE OF SLAB DIMENSIONS NOT SHOWN. SLAB DEPRESSION DEPTHS AND LOCATIONS NOT SHOWN. SLAB SLOPES, CURB HEIGHTS, CURB HEIGHTS AND LOCATIONS NOT SHOWN, AND SLAB AND WALL OPENING SIZES AND LOCATIONS NOT SHOWN.
5. **REFER TO ARCHITECTURAL DRAWINGS** FOR LOCATION AND EXTENTS OF NON-LOAD BEARING GUM WALLS.
6. **ELEVATOR BRACING** AND RAIL SUPPORT REQUIREMENTS OF THE ELEVATOR BEING SUPPLIED SHALL BE COORDINATED BY THE GENERAL CONTRACTOR. NOTIFY THE STRUCTURAL ENGINEER OF ANY INCONSISTENCIES.
7. **CONTRACTOR SHALL FIELD VERIFY** EXISTING CONDITIONS AND DIMENSIONS RELATED TO ANY NEW STRUCTURE PRIOR TO FABRICATION AND CONSTRUCTION.
8. **SHEET AND TYPICAL DETAIL REFERENCES:**
S0 SERIES STRUCTURAL
S301 NON-LOAD BEARING GUM WALLS (INCLUDING THICKENED SLAB REQUIREMENTS)
9. **MARK AND LEGEND KEY:**

| | |
|---------------------------------------------------------------------------------------|----------------------------|
|  | NON-LOAD BEARING GUM WALLS |
|---------------------------------------------------------------------------------------|----------------------------|

| CONSTRUCTION INFO | |
|--------------------------------------------------------------------------|------------------------------------------------------------|
| <u>GENERAL CONTRACTOR</u> BRASFIELD & GORRIE WALKER COX | <u>PRECAST CONCRETE</u> METROMONT JIM BATCHER |
| HCA DESIGN MANAGER: RYAN ROHE | HCA CONSTRUCTION MANAGER: GREG ATKINSON |



- NOTES:**
1. REFER TO FOUNDATION PLAN AND ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN.
FOR FINISH LEVELS OF SLAB ELEVATIONS ARE APPROXIMATE. REFER TO ARCHITECTURAL DRAWINGS FOR TOP OF SLAB ELEVATIONS, SLAB SLOPE AND EDGE OF SLAB DIMENSIONS.
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 3. SEE ARCHITECTURAL DRAWINGS FOR EDGE OF SLAB DIMENSIONS NOT SHOWN, SLAB DEPRESSION DEPTHS AND LOCATIONS NOT SHOWN, SLAB SLOPES NOT SHOWN, CONCRETE CURB HEIGHTS AND LOCATIONS NOT SHOWN, AND SLAB AND WALL OPENING SIZES AND LOCATIONS NOT SHOWN.
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ELEVATOR BRACING AND RAIL SUPPORT REQUIREMENTS OF THE ELEVATOR BRACING SUBMITTAL SHALL BE OBSERVED BY THE GENERAL CONTRACTOR. NOTIFY THE STRUCTURAL ENGINEER OF ANY INCONSISTENCIES.
 5. CONTRACTOR SHALL FIELD VERIFY EXISTING CONDITIONS AND DIMENSIONS RELATED TO THE NEW STRUCTURE PRIOR TO FABRICATION AND CONNECTION.
 6. SHEET AND TYPICAL DETAIL REFERENCES:
30 SERIES STRUCTURAL NOTES
S301 NON-LOAD BEARING CMU WALLS (INCLUDING THICKENED SLAB REQUIREMENTS)
 7. MARK AND LEGEND KEY:
NON-LOAD BEARING CMU WALLS

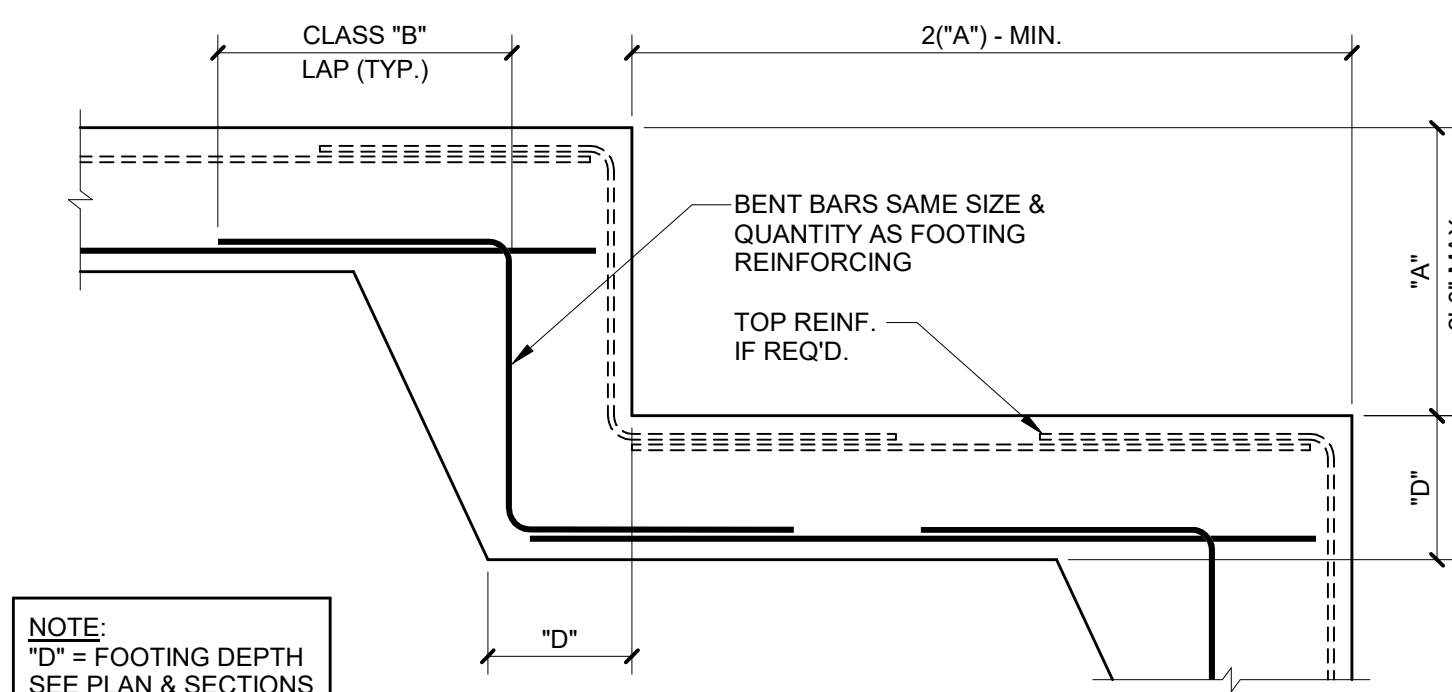
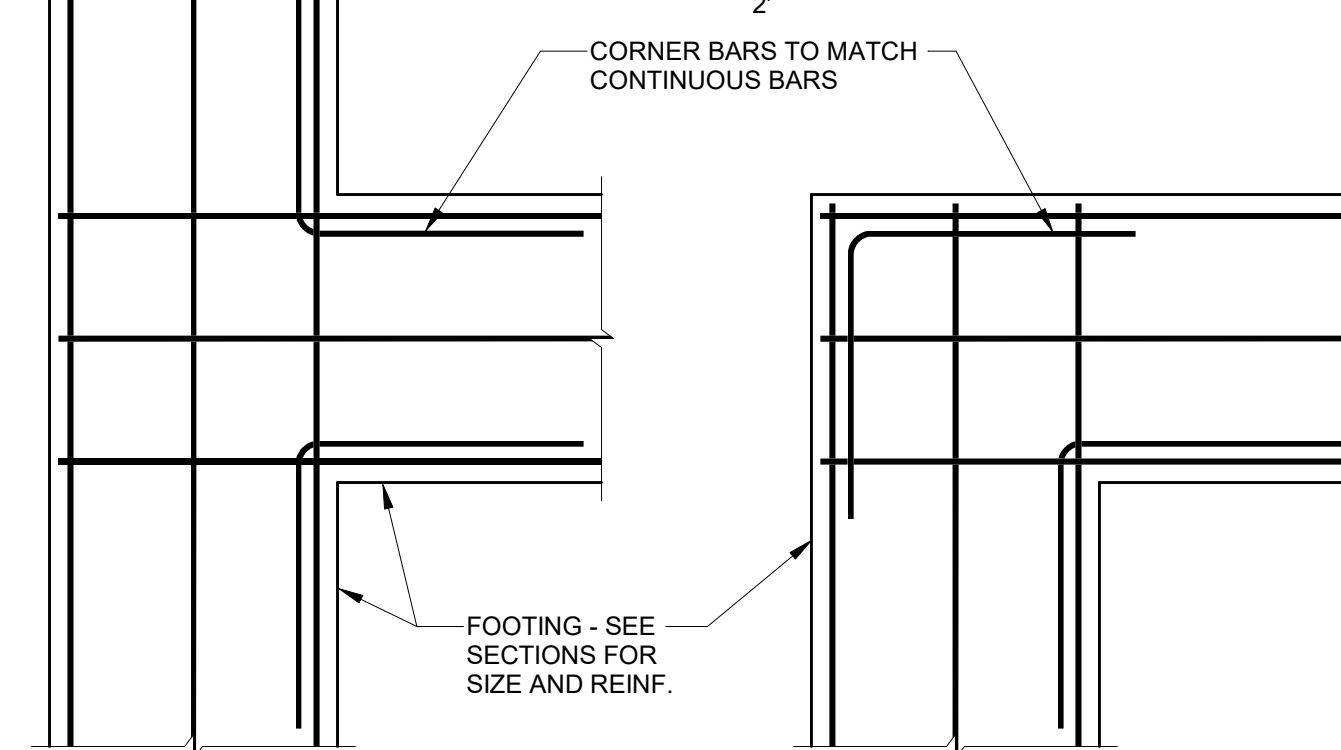
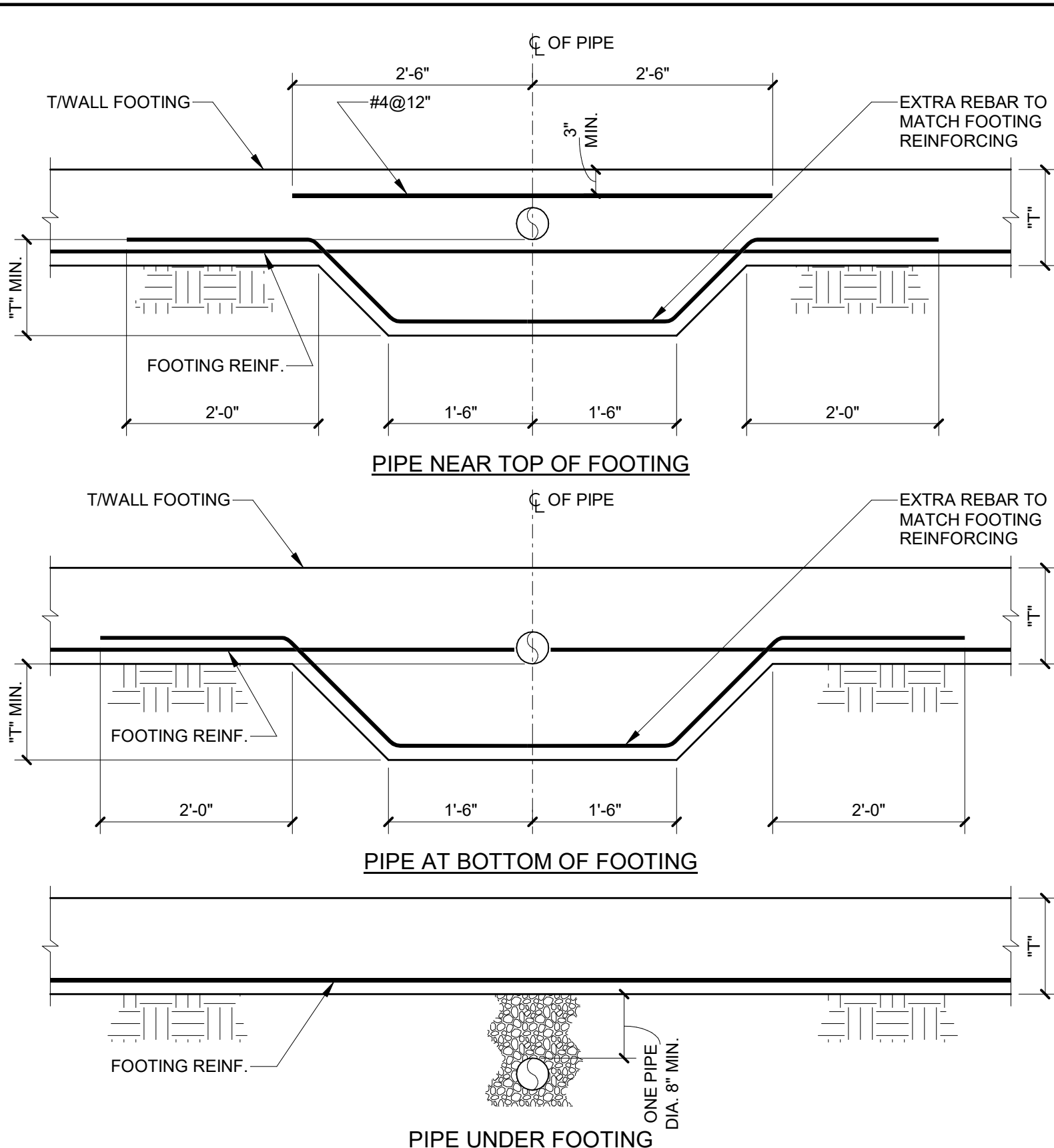
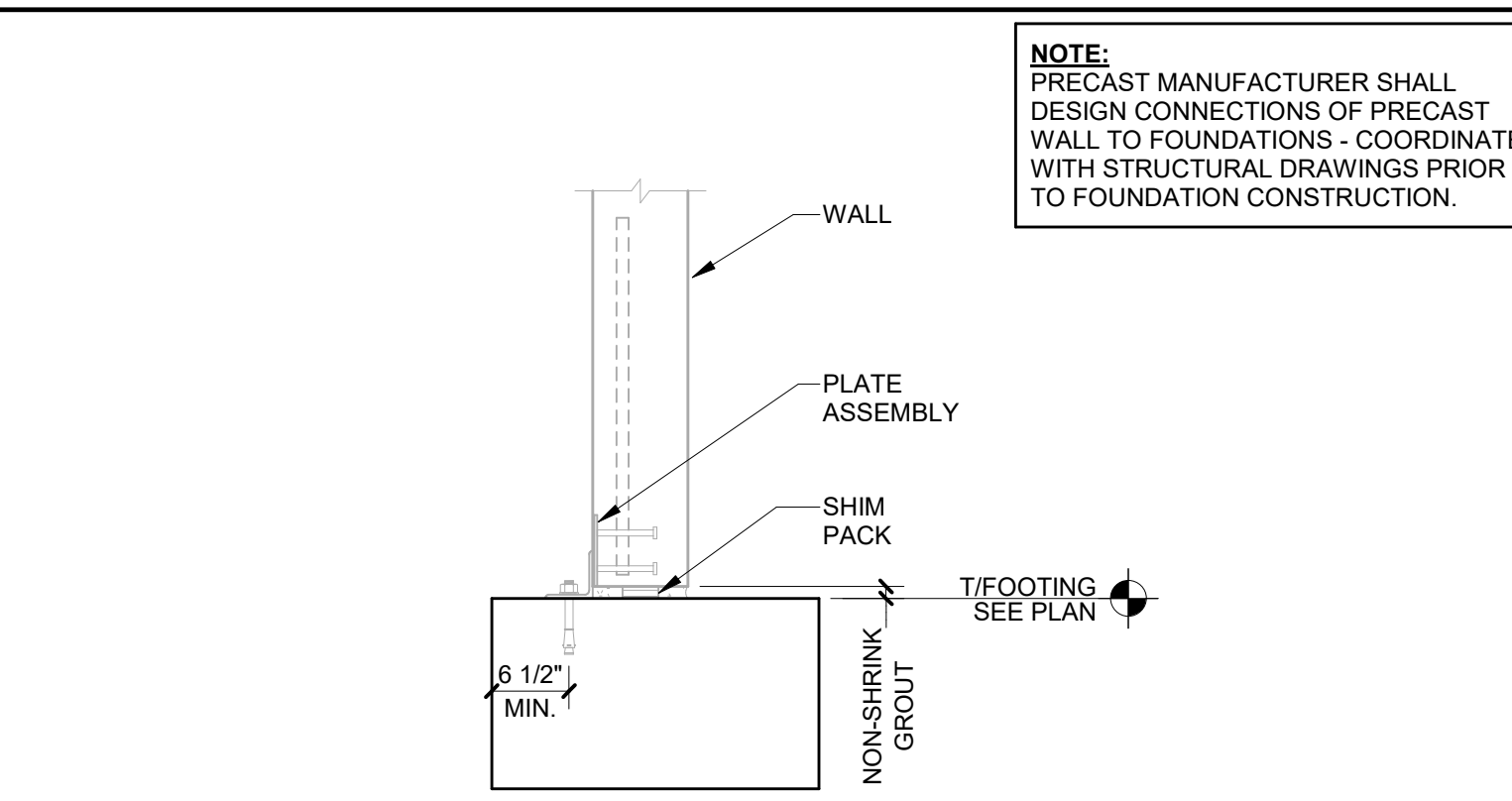
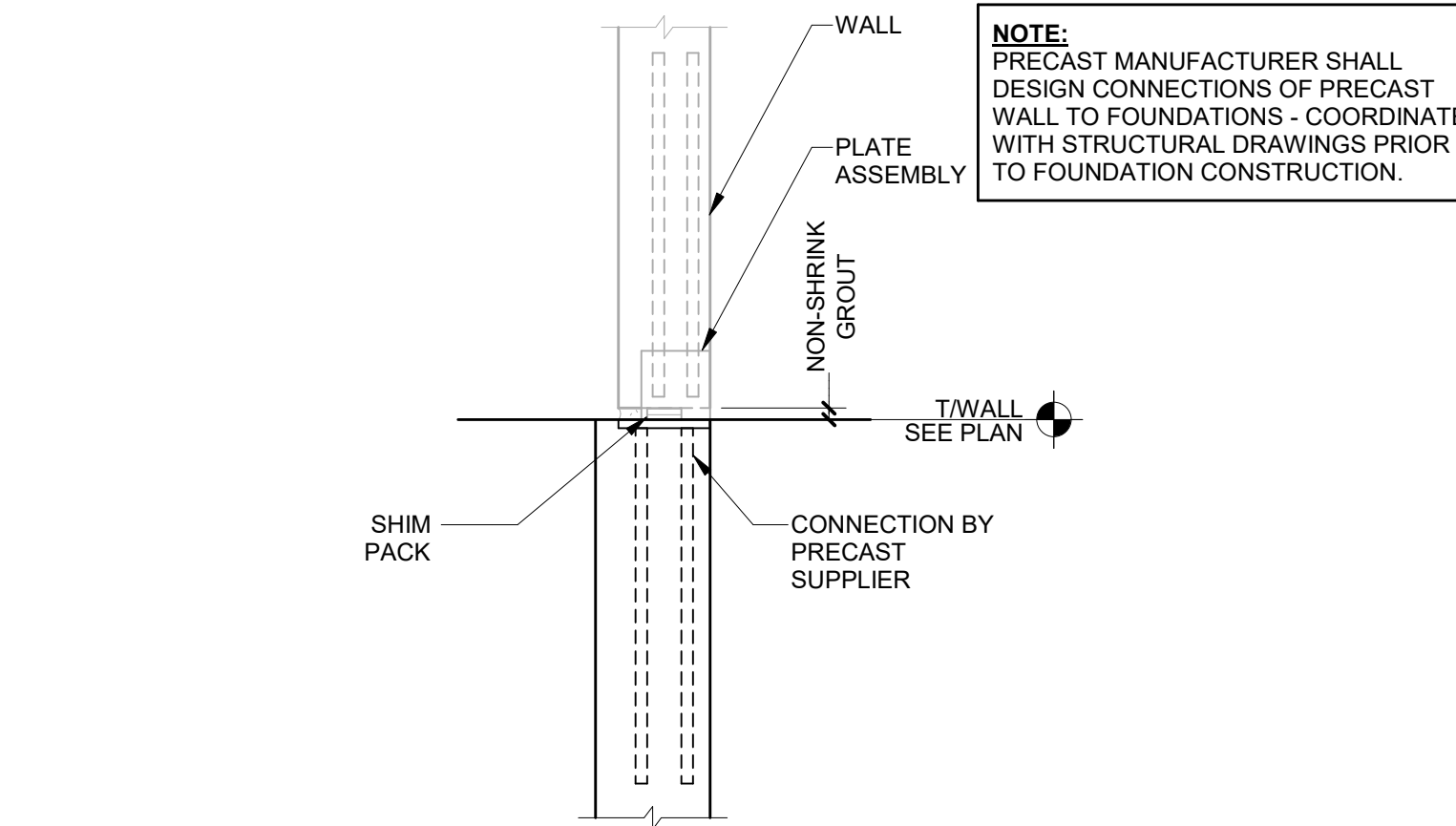
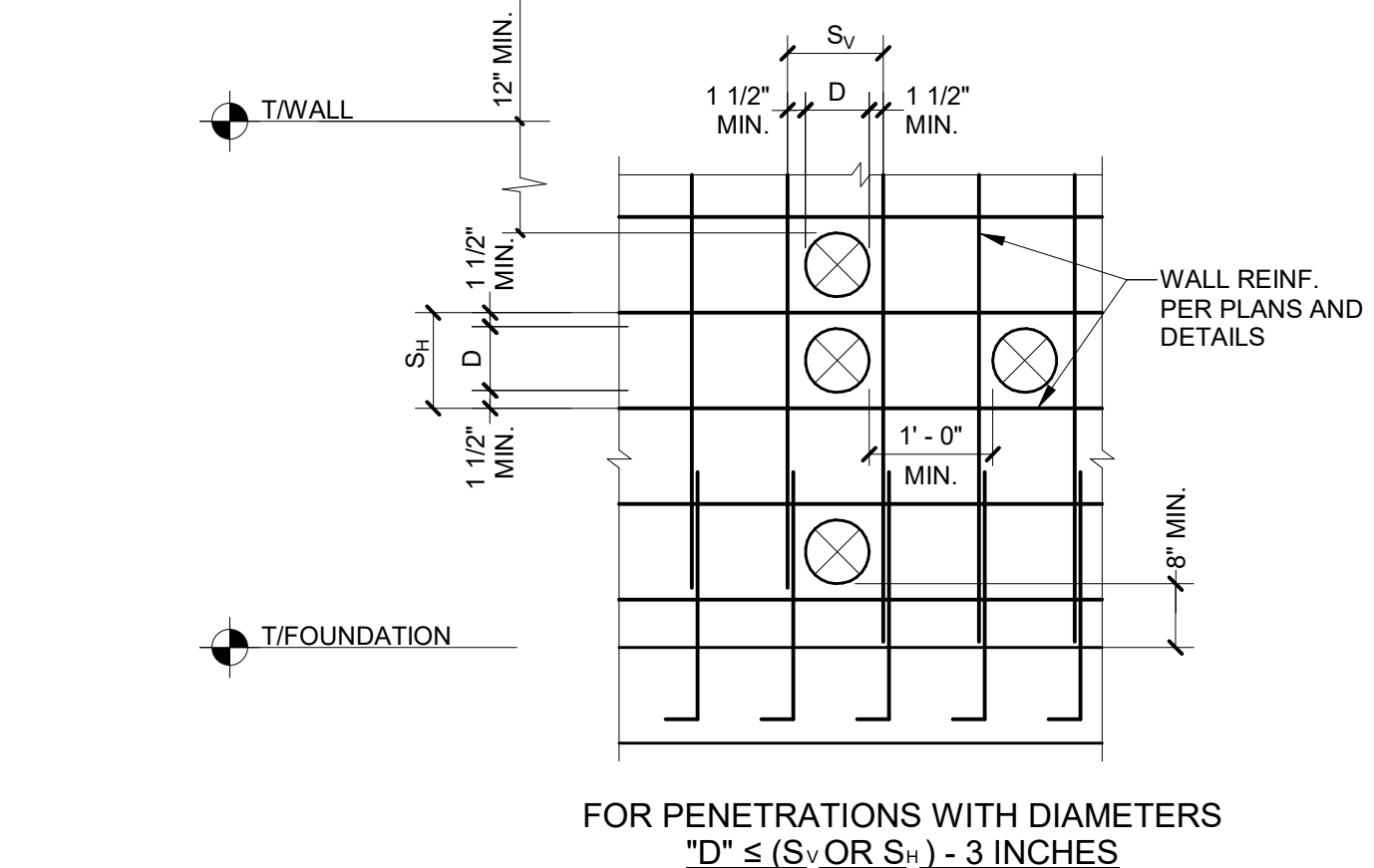
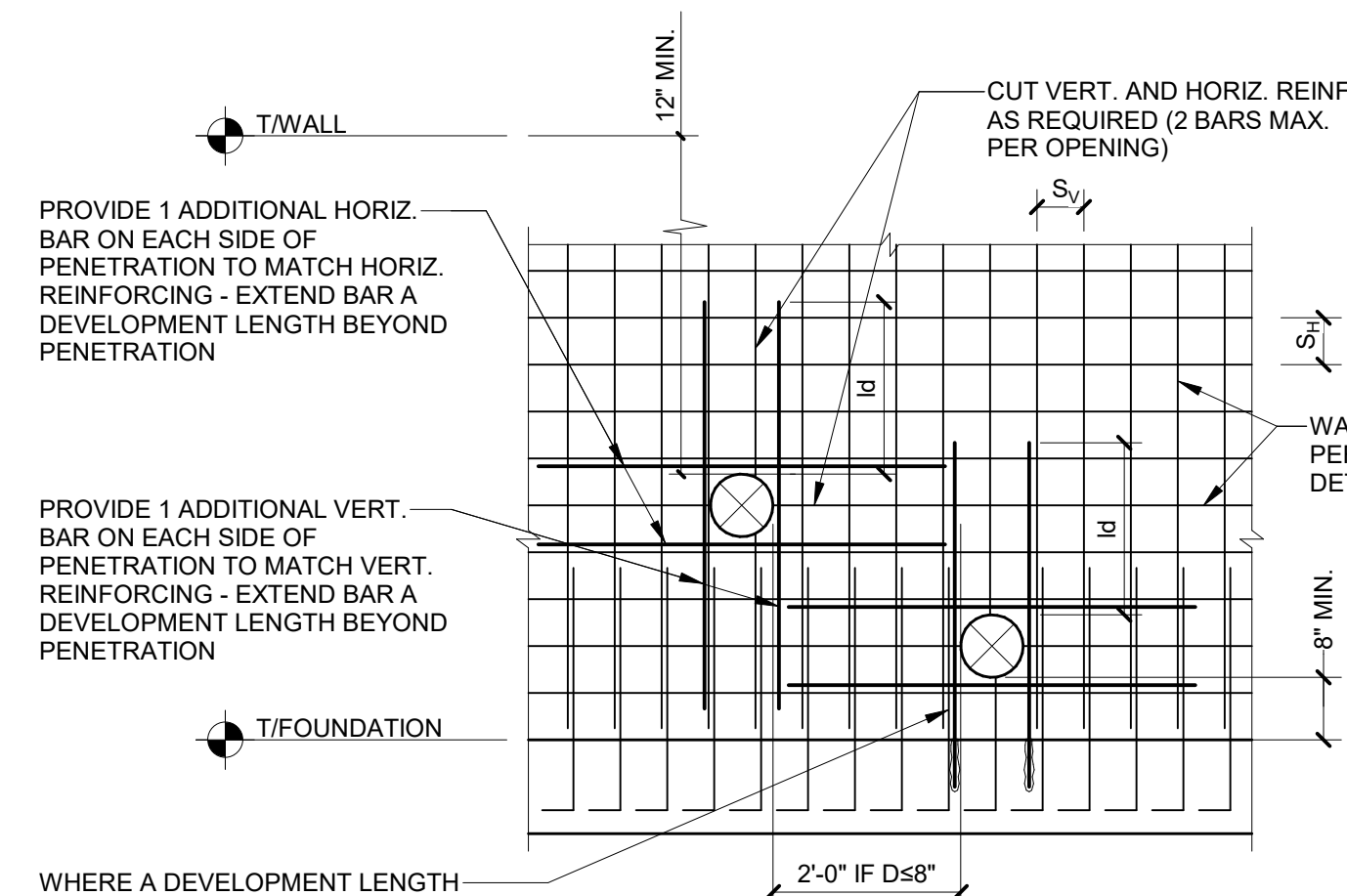
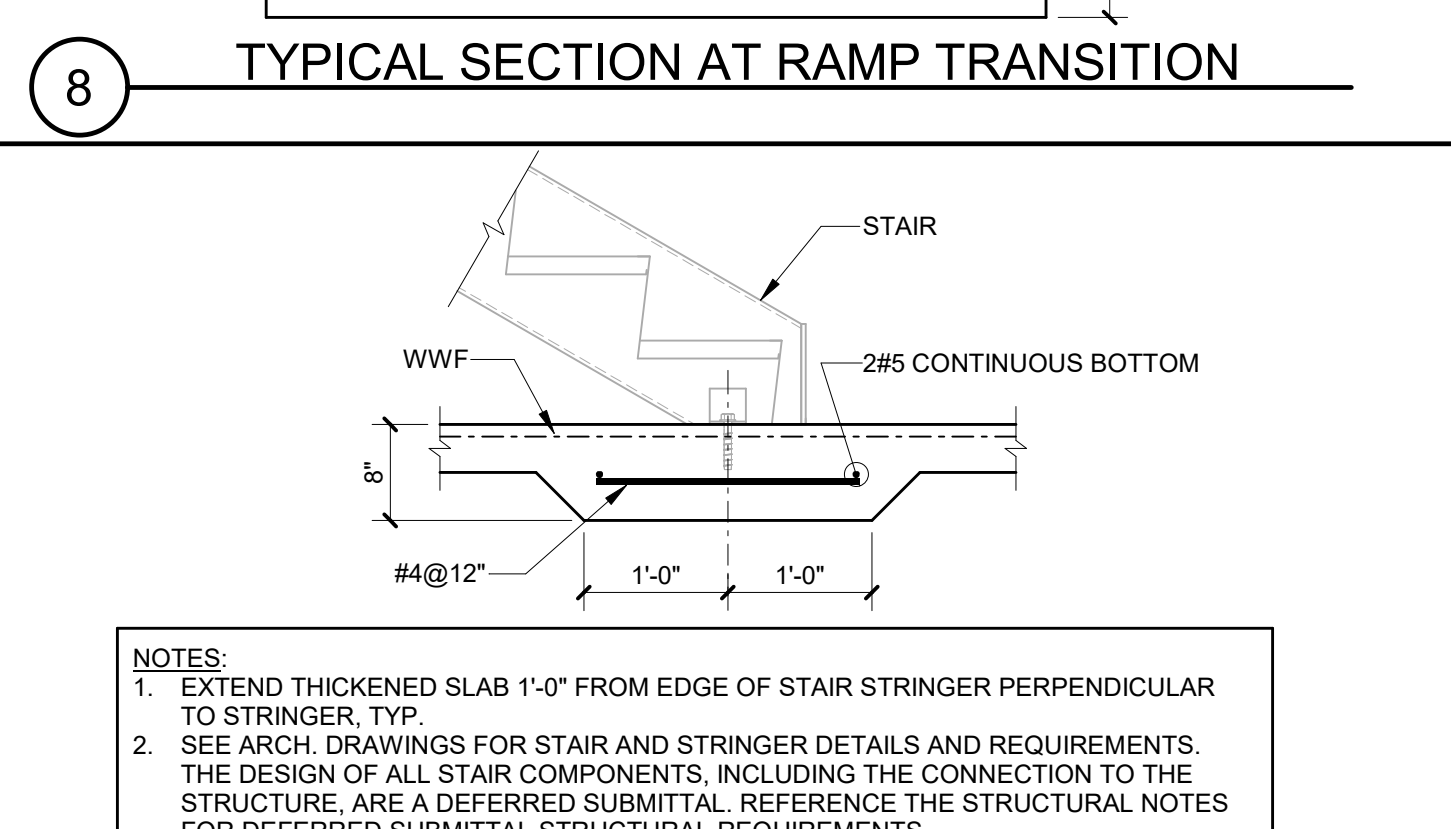
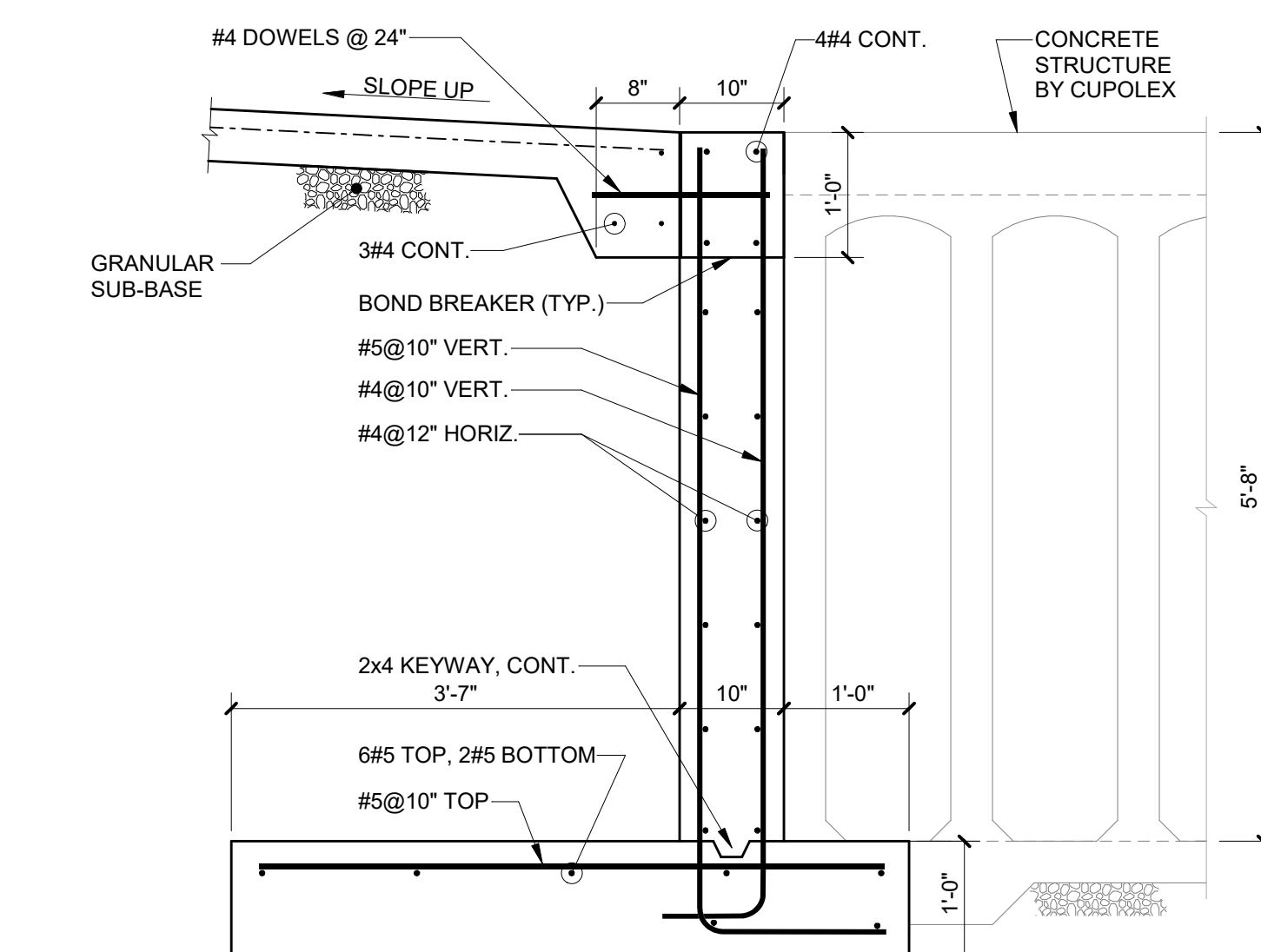
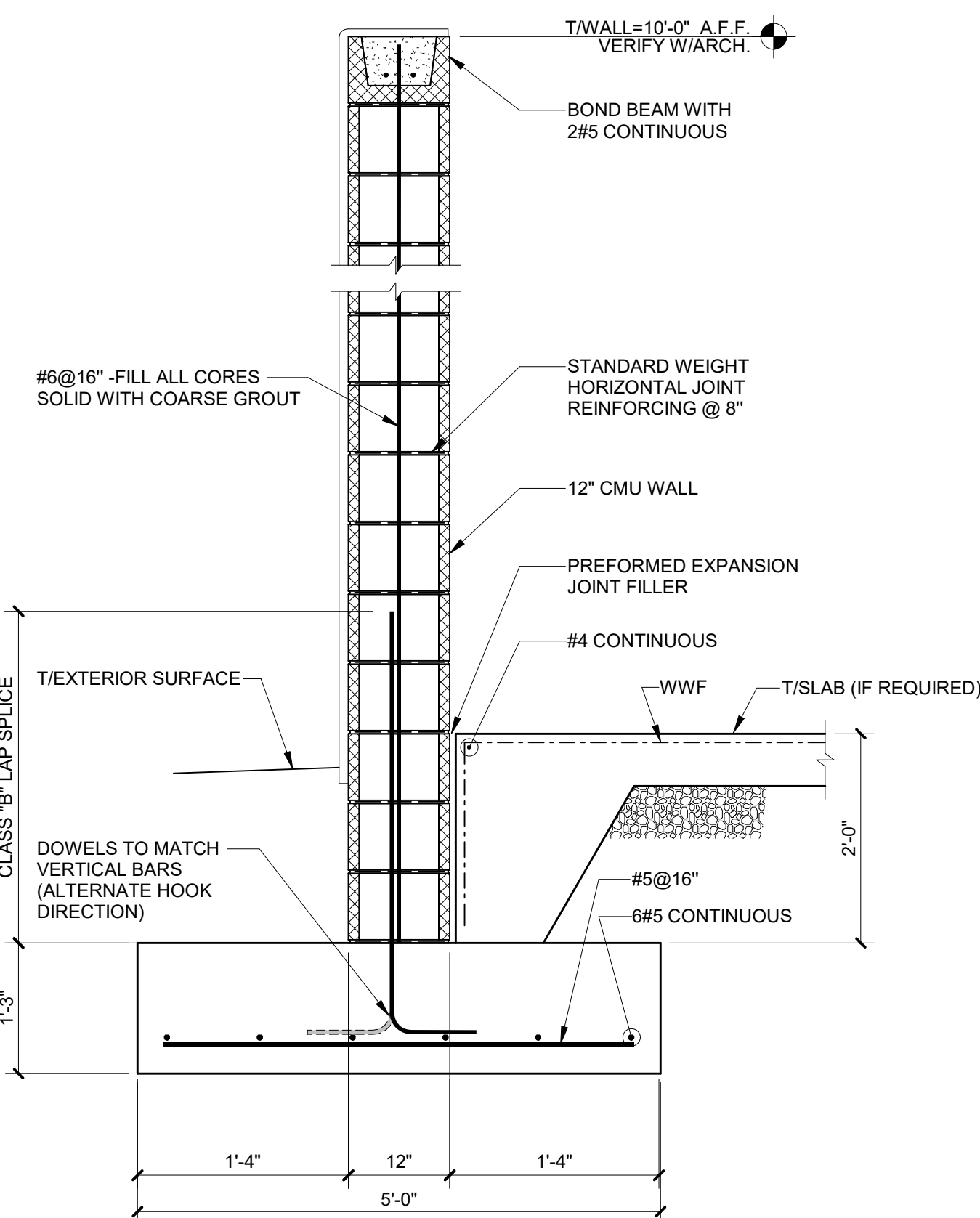
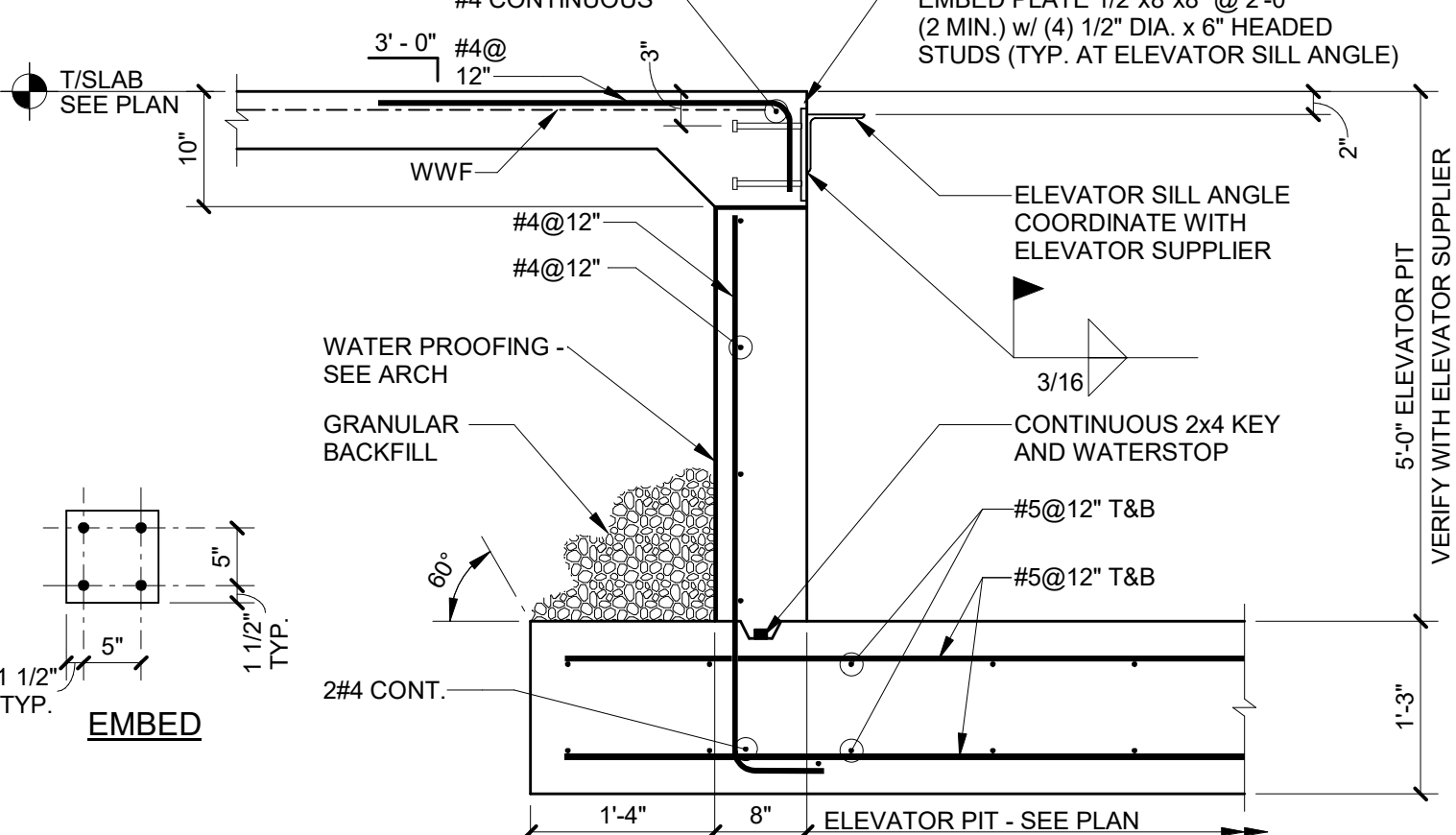
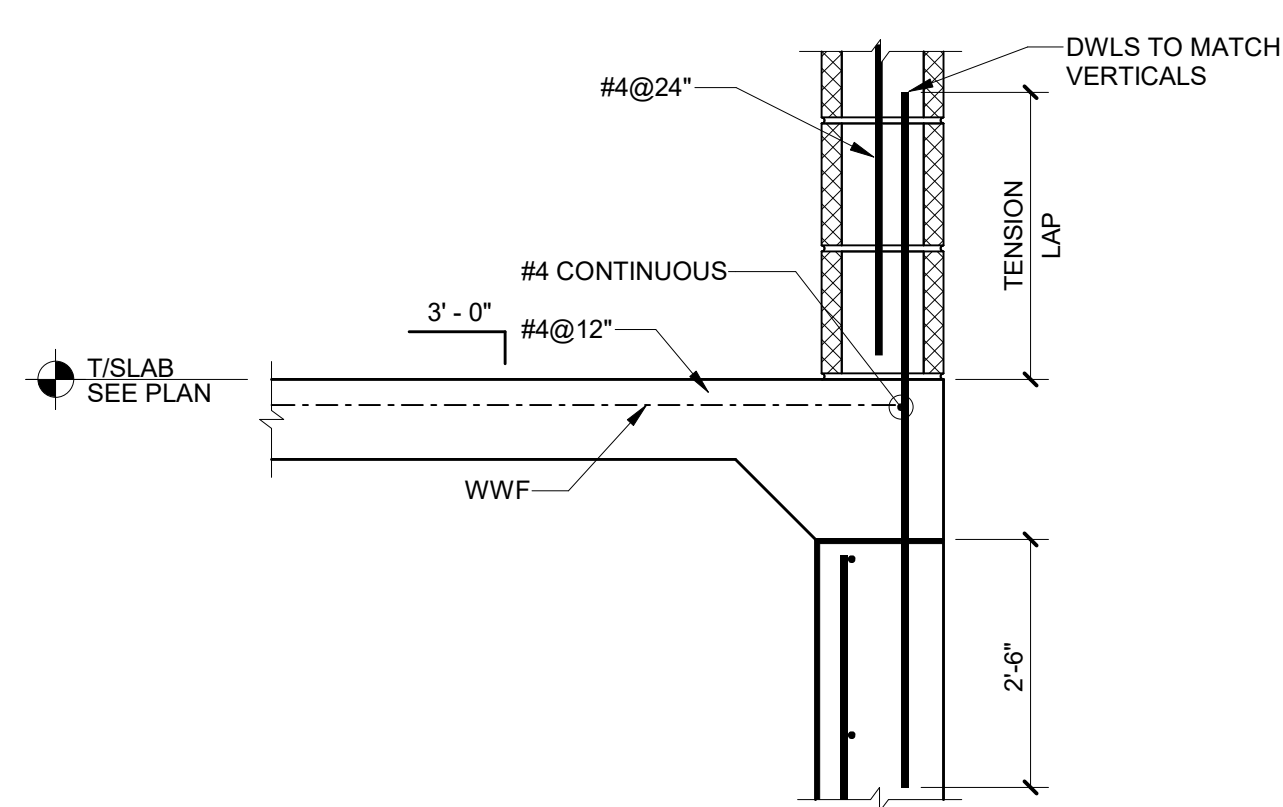
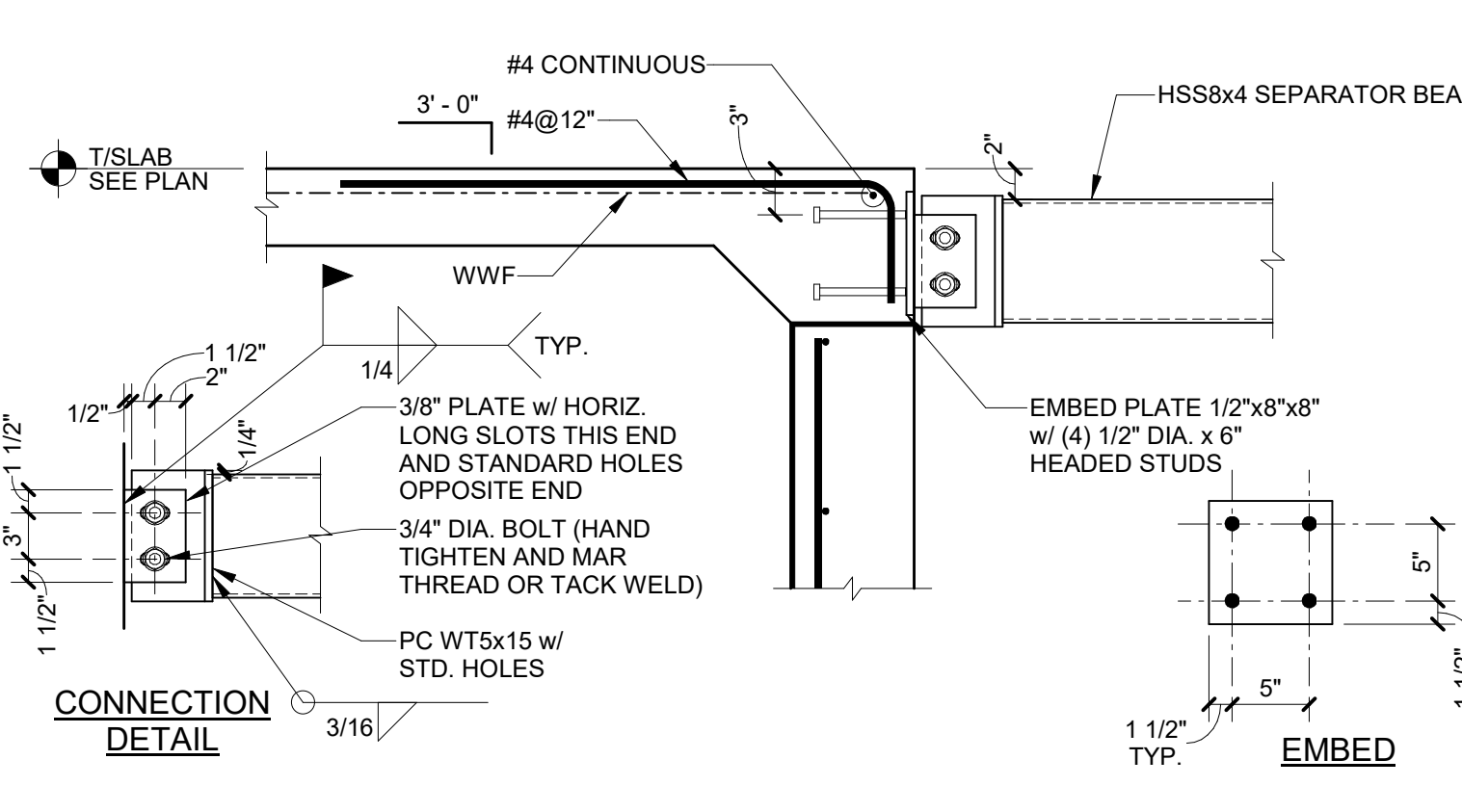
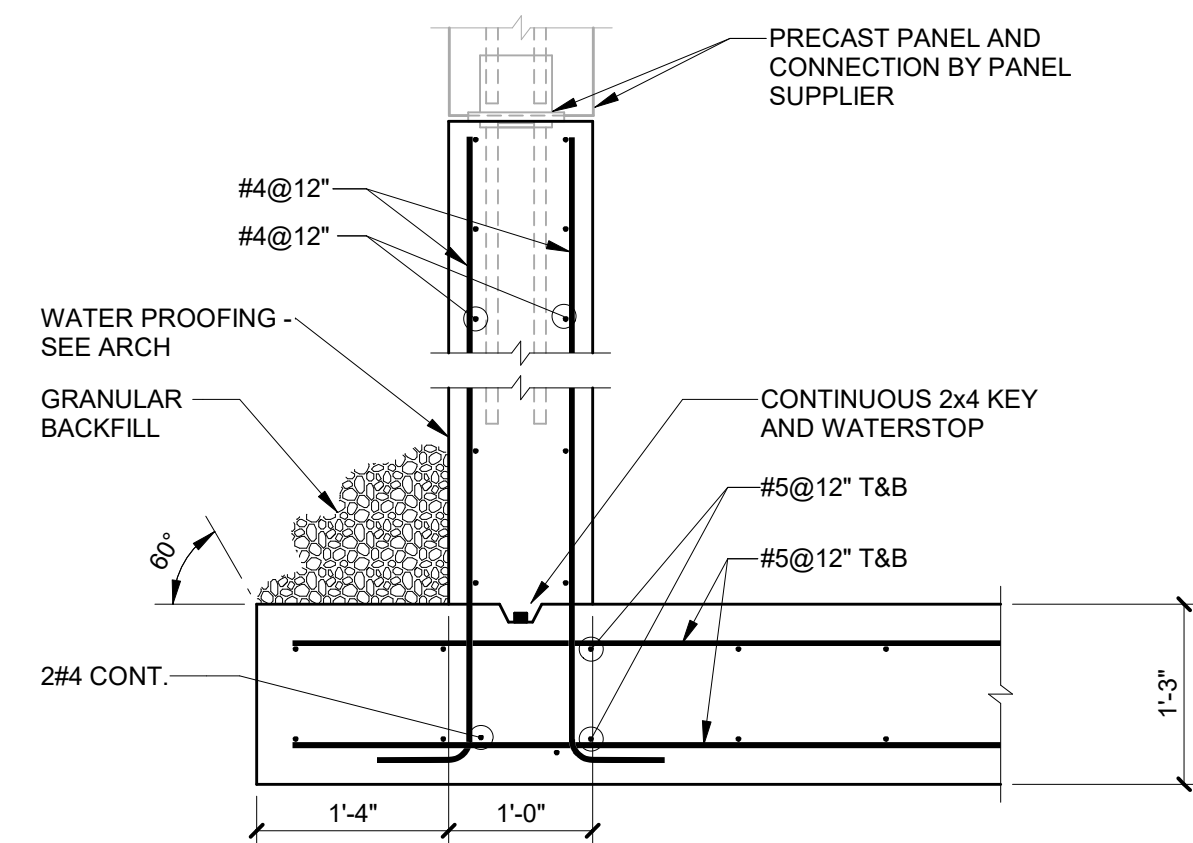
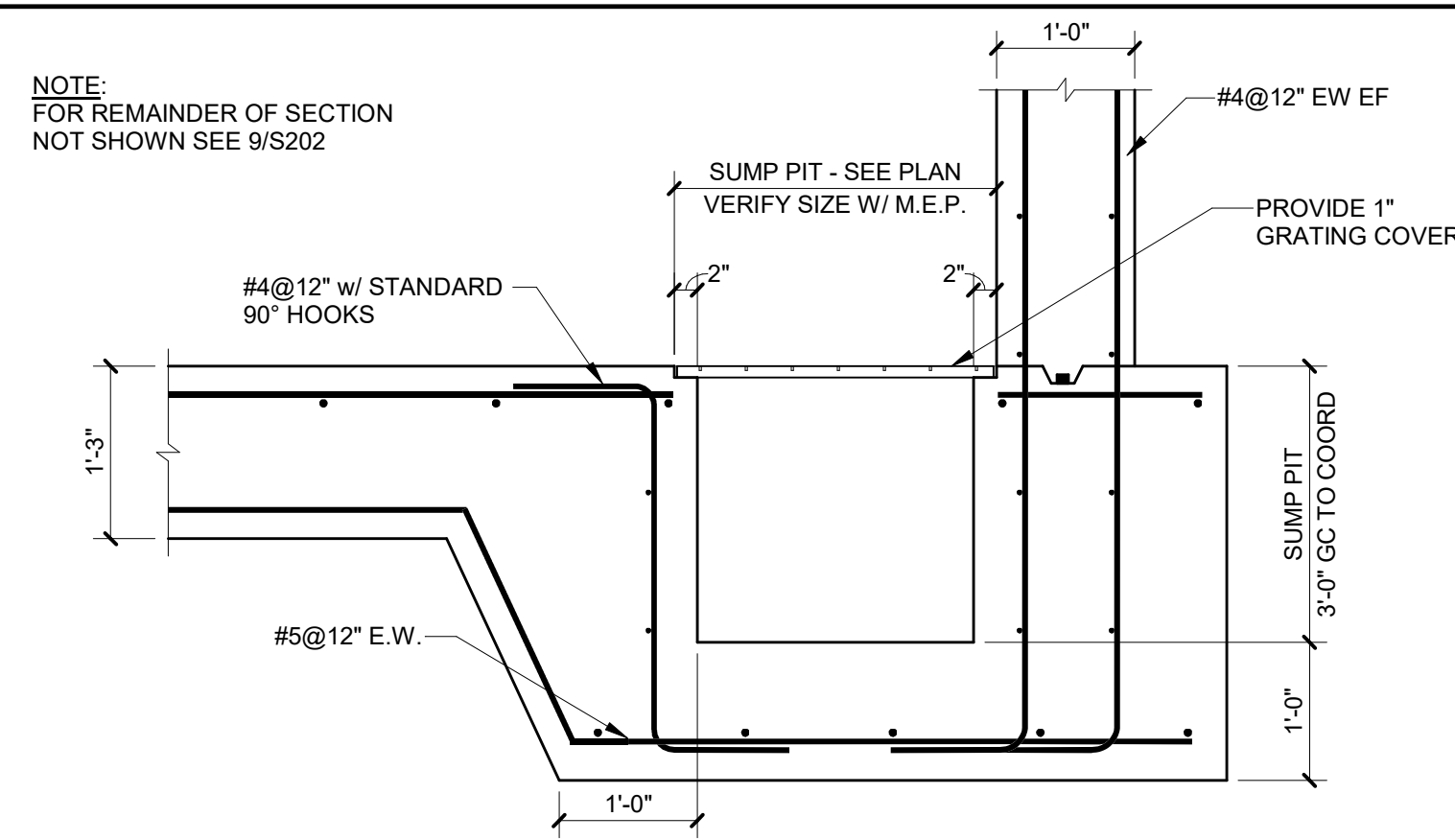
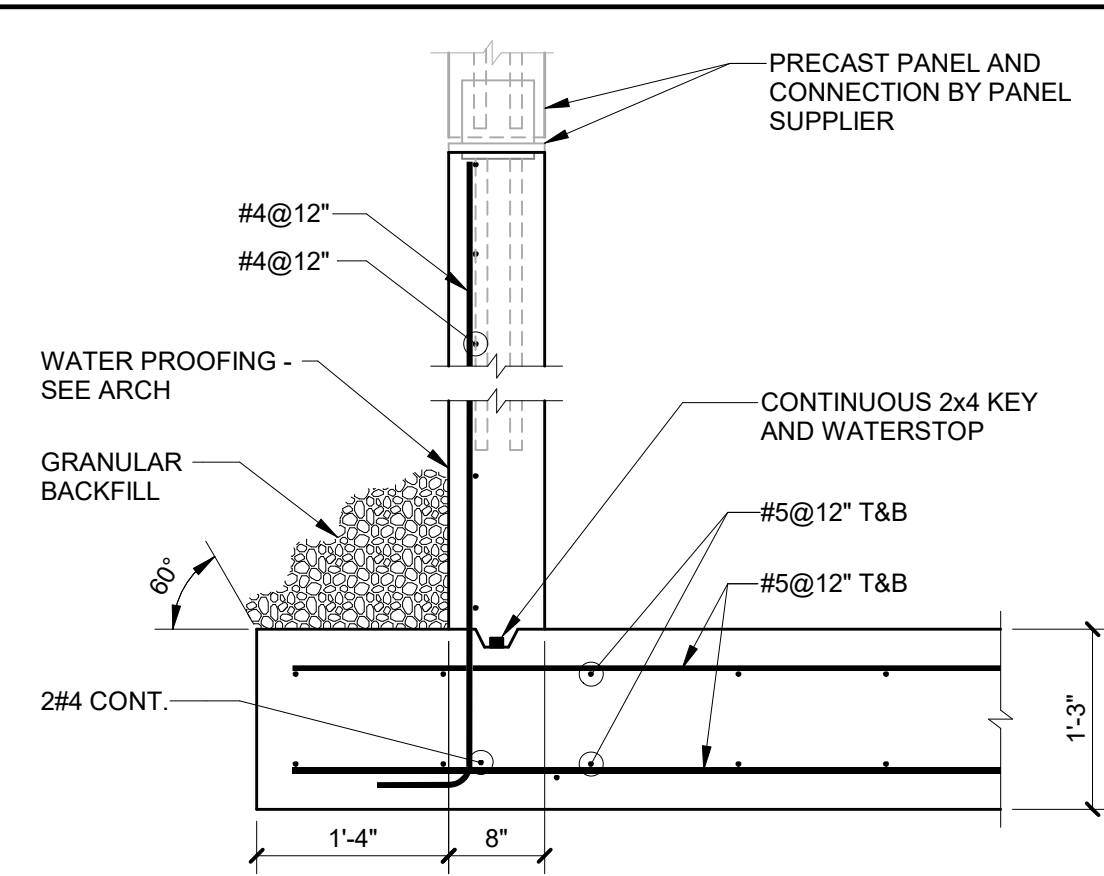
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|--------------------------------------------------------------------------|------------------------------------------------------------|
| CONSTRUCTION INFO | |
| <u>GENERAL CONTRACTOR</u> BRASFIELD & GORRIE WALKER COX | <u>PRECAST CONCRETE</u> METROMONT JIM BATCHER |
| HCA DESIGN MANAGER: RYAN ROHE | HCA CONSTRUCTION MANAGER: GREG ATKINSON |



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 - S301 NON-LOAD BEARING CMU WALLS (INCLUDING THICKENED SLAB REQUIREMENTS)
 - MARK AND LEGEND KEY:
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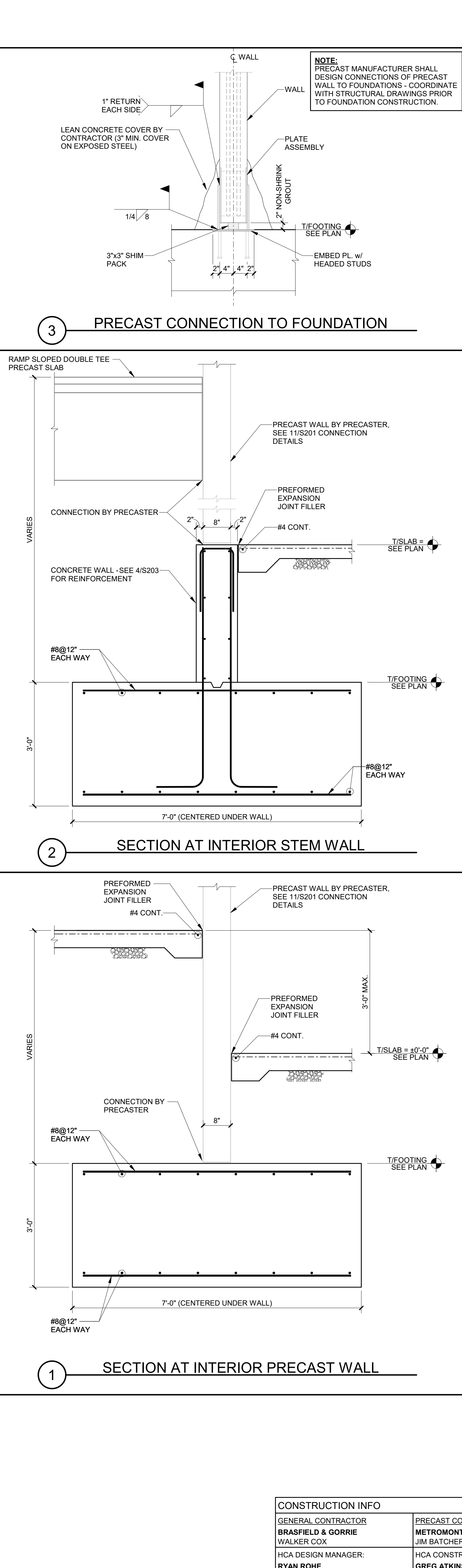
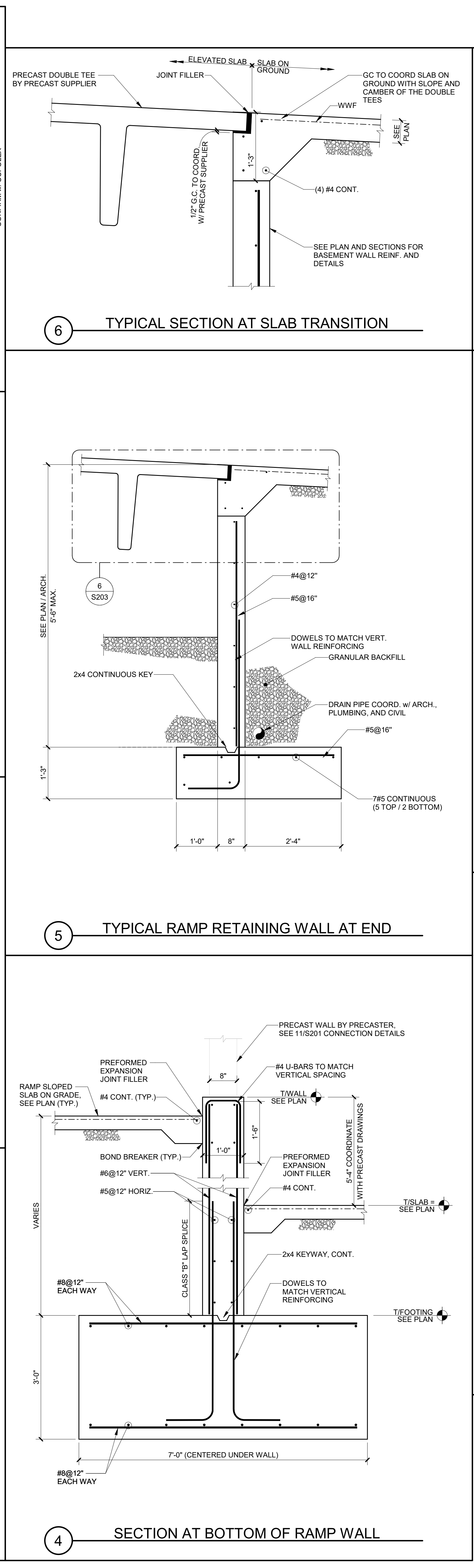
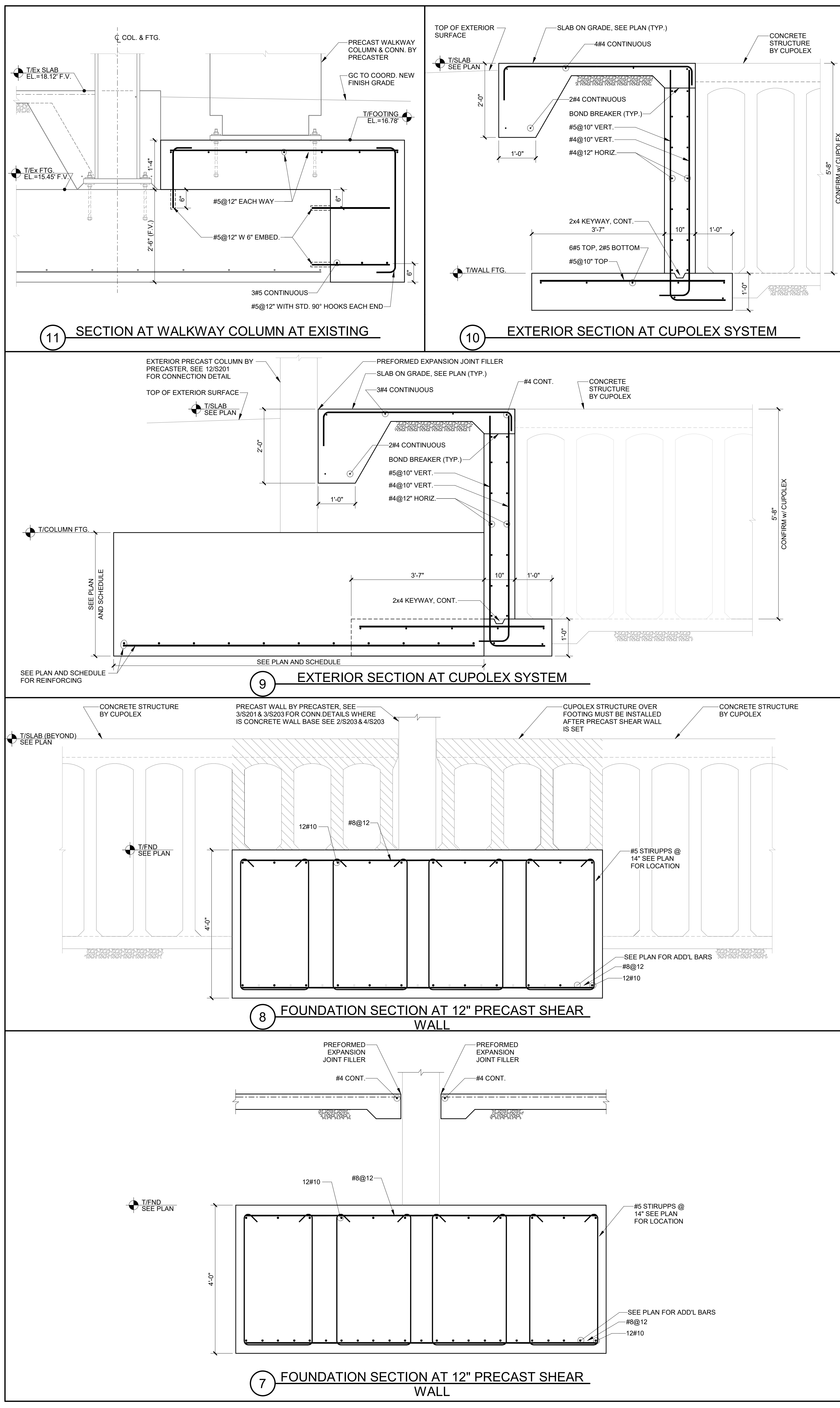
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| HCA DESIGN MANAGER: RYAN ROHE | HCA CONSTRUCTION MANAGER: GREG ATKINSON |

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| CONSTRUCTION INFO | |
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| GENERAL CONTRACTOR | PRECAST CONCRETE |
| BRASFIELD & GORRIE | METROMONT |
| WALKER COX | JIM BATCHER |
| HCA DESIGN MANAGER: | HCA CONSTRUCTION MANAGER: |
| RYAN ROHE | GREG ATKINSON |

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Florida License: #AR91721

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1966

Stanley D. Lindsey and Associates, Ltd.

Structural Engineers

750 Old Hickory Blvd.
Building 1, Suite 175
Brentwood, TN 37027

www.sdlal.com

615 320 1735

Project No. 24024.00
COA No. 1129

HCA Healthcare

HCA FLORIDA ST. LUCIE HOSPITAL
PARKING GARAGE
1800 SE TIFFANY AVE, PORT ST. LUCIE, FL 34952

HCA Project No. 3099600023

| REVISIONS | |
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| DR. BY | CG |
| CK. BY | BN |
| PROJ. | A00324 |
| DATE | 07/30/2024 |

FOUNDATION SECTIONS AND DETAILS

S203

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| CK. BY | BN |
| PROJ. | A00324 |
| DATE | 07/30/2024 |

**FOUNDATION
SECTIONS AND
DETAILS**

S204

TMPartners, PLLC
Architecture Interiors Planning

211 Franklin Road, Suite 200
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COA No. 1329

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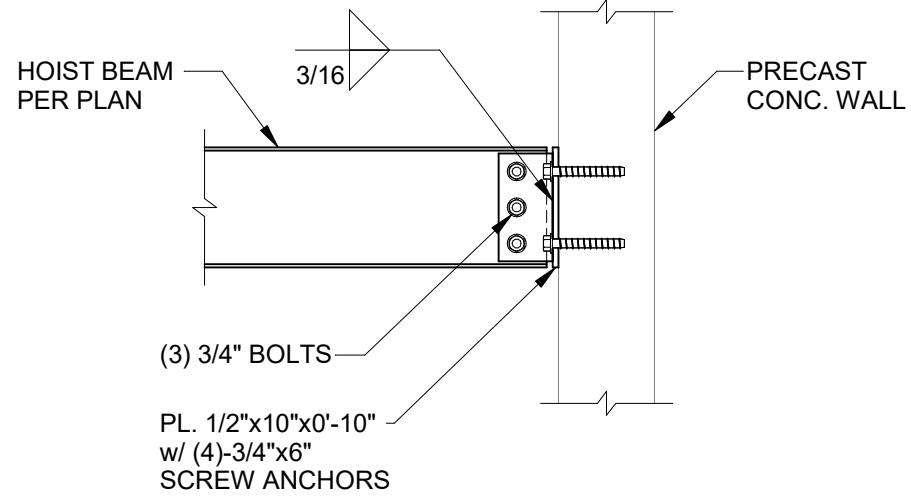
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| DATE | 07/30/2024 |

**FOUNDATION
SECTIONS AND
DETAILS**

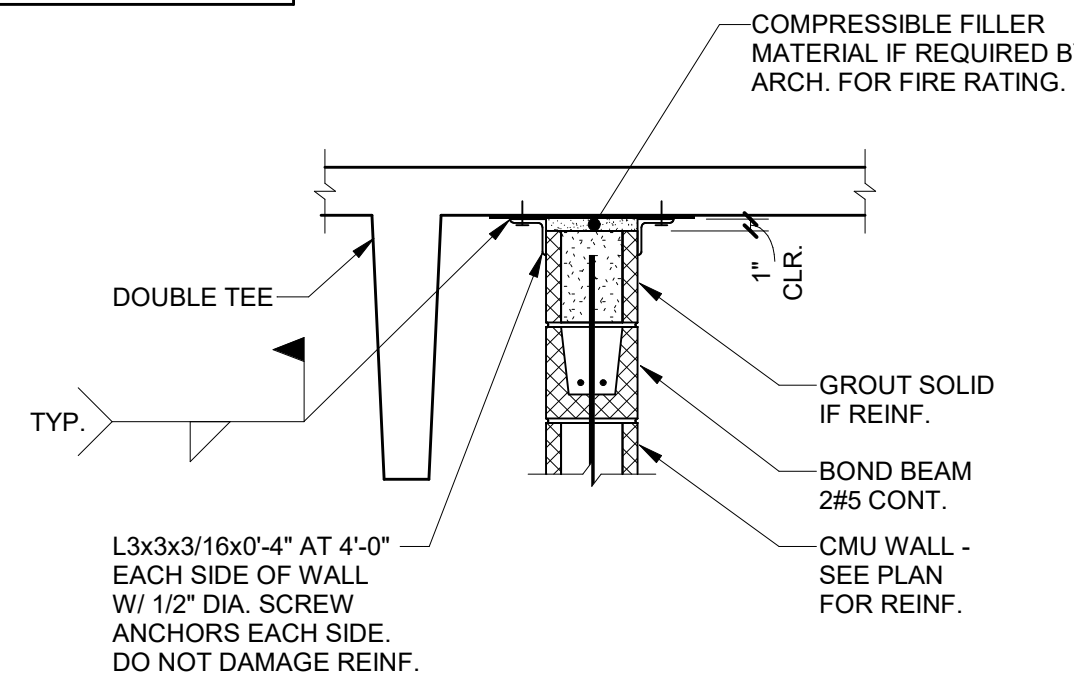
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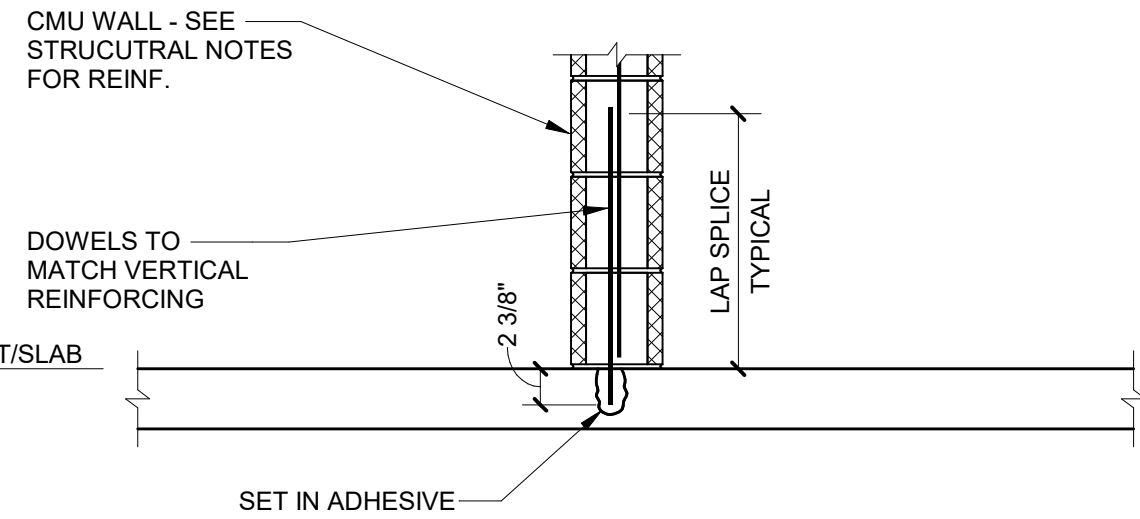


12 HOIST BEAM CONNECTION TO PRECAST WALL

ALTERNATE:
EMBED PLATE 1/2"x8"x1'-6" @ 4'-0"
WELD ANGLES TO PLATE



11 TYPICAL CMU WALL ANCHORAGE



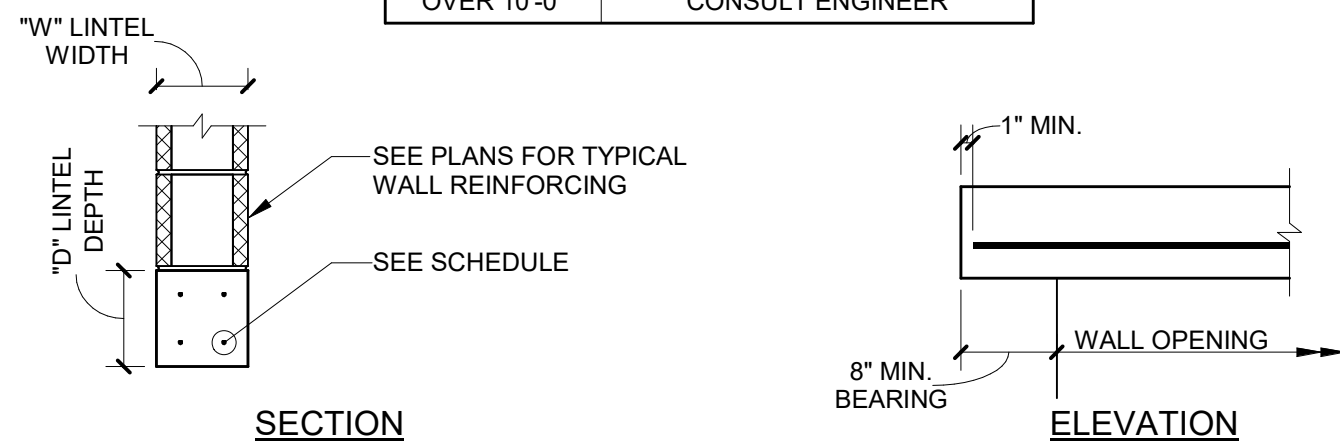
NOTE:
MARK DOUBLE TEE REINF. LOCATIONS
BEFORE DRILLING. DO NOT DAMAGE REINF.

| CONCRETE MASONRY UNIT WALL REINFORCING LAP LENGTH SCHEDULE (1) | | |
|----------------------------------------------------------------------------------------------------|------------|------------|
| TENSION OR COMPRESSION LAP SPlice LENGTHS, (INCHES) FOR GRADE 60 (2) UNCOATED BARS (3), MASONRY | | |
| LAP LENGTH | | |
| BAR SIZE (4) | CASE A (5) | CASE B (5) |
| #3 | 15 | 27 |
| #4 | 20 | 36 |
| #5 | 25 | 45 |
| #6 | 30 | 54 |
| #7 | 35 | 63 |
| #8 | 40 | 72 |
| #9 | 46 | 82 |

NOTES:
1. LAP SPlice LENGTHS ARE BASED ON SECTION 2107 OF THE BUILDING CODE.
2. MULTIPLY ABOVE LAP LENGTHS BY 1.25 FOR GRADE 75 REINFORCING STEEL.
3. MULTIPLY ABOVE LAP LENGTHS BY 1.50 FOR EPOXY COATED BARS.
4. REINFORCEMENT LARGER THAN #9 SHALL BE SPliced USING MECHANICAL CONNECTIONS.
5. USE CASE B UNLESS NOTED OTHERWISE ON PLANS OR DETAILS.

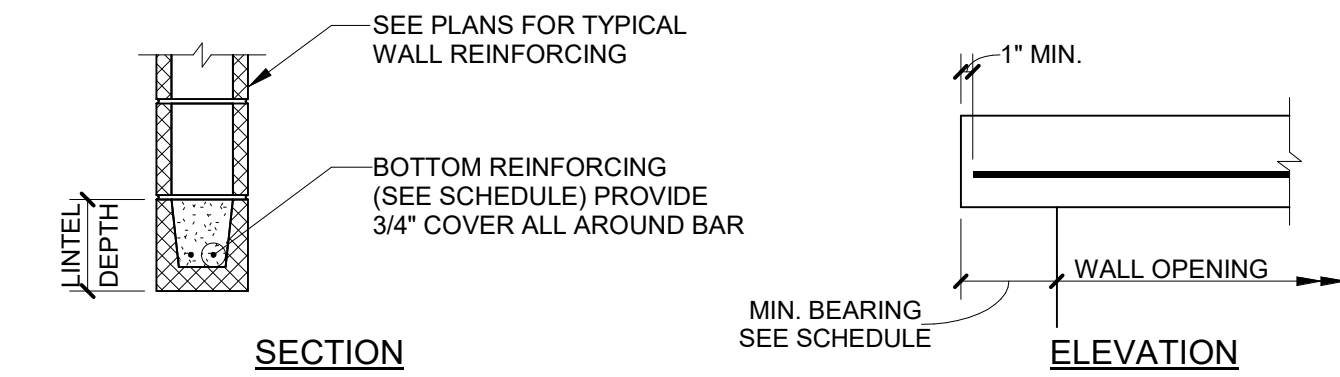
10 CMU REINFORCING SCHEDULE

| LINTEL REINFORCEMENT | | |
|----------------------|------------------|-------------|
| WALL OPENING | LINTEL DEPTH | REINFORCING |
| UP TO 4'-0" | 8"x8" | 2#4 T&B |
| 4'-0" TO 6'-0" | 8"x8" | 2#5 T&B |
| 6'-0" TO 8'-0" | 8"x16" | 2#5 T&B |
| 8'-0" TO 10'-0" | 8"x16" | 2#6 T&B |
| OVER 10'-0" | CONSULT ENGINEER | |

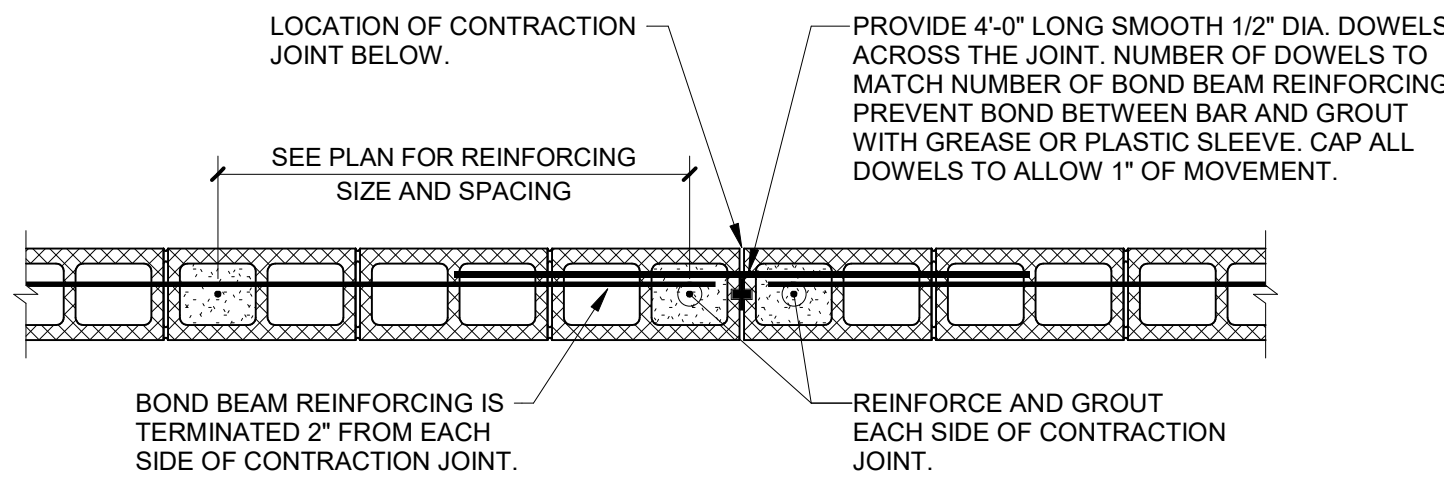


9 PRECAST LINTEL SCHEDULE

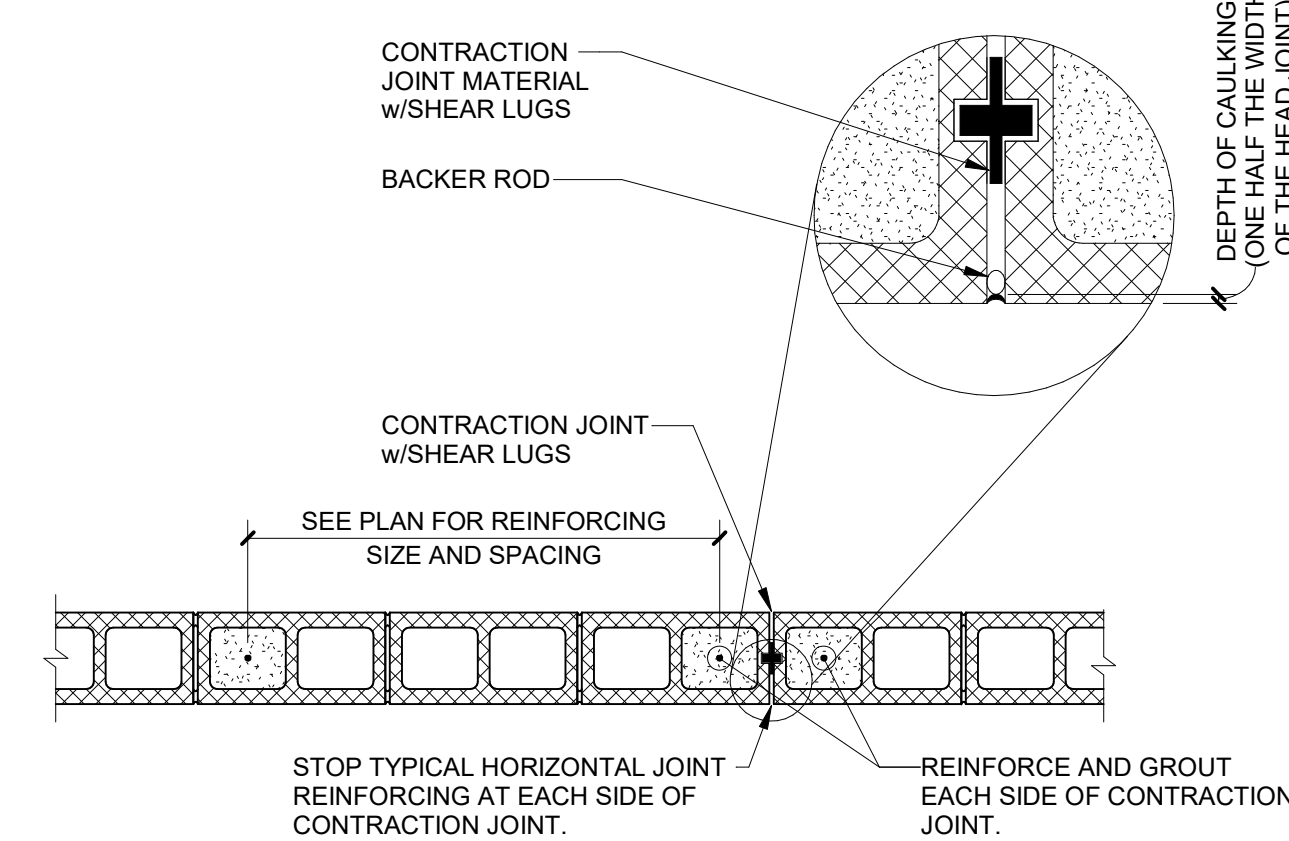
| LINTEL REINFORCEMENT | | | |
|----------------------|------------------|-------------|--------------|
| WALL OPENING | LINTEL DEPTH | REINFORCING | MIN. BEARING |
| UP TO 4'-0" | 8" | 2#4 BOTTOM | 1'-4" |
| 4'-0" TO 6'-0" | 8" | 2#5 BOTTOM | 1'-4" |
| OVER 10'-0" | CONSULT ENGINEER | | |



8 CMU LINTEL SCHEDULE

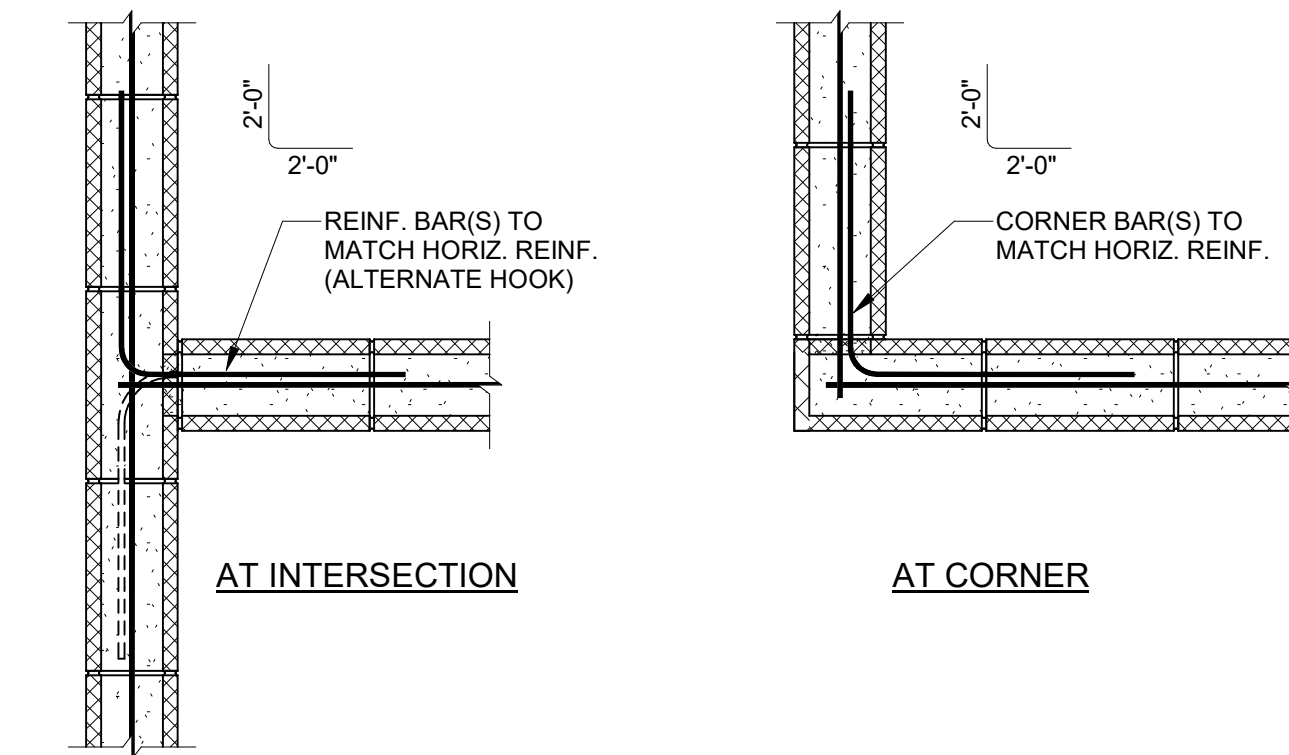


BOND BEAM REINFORCING AT CONTRACTION JOINT

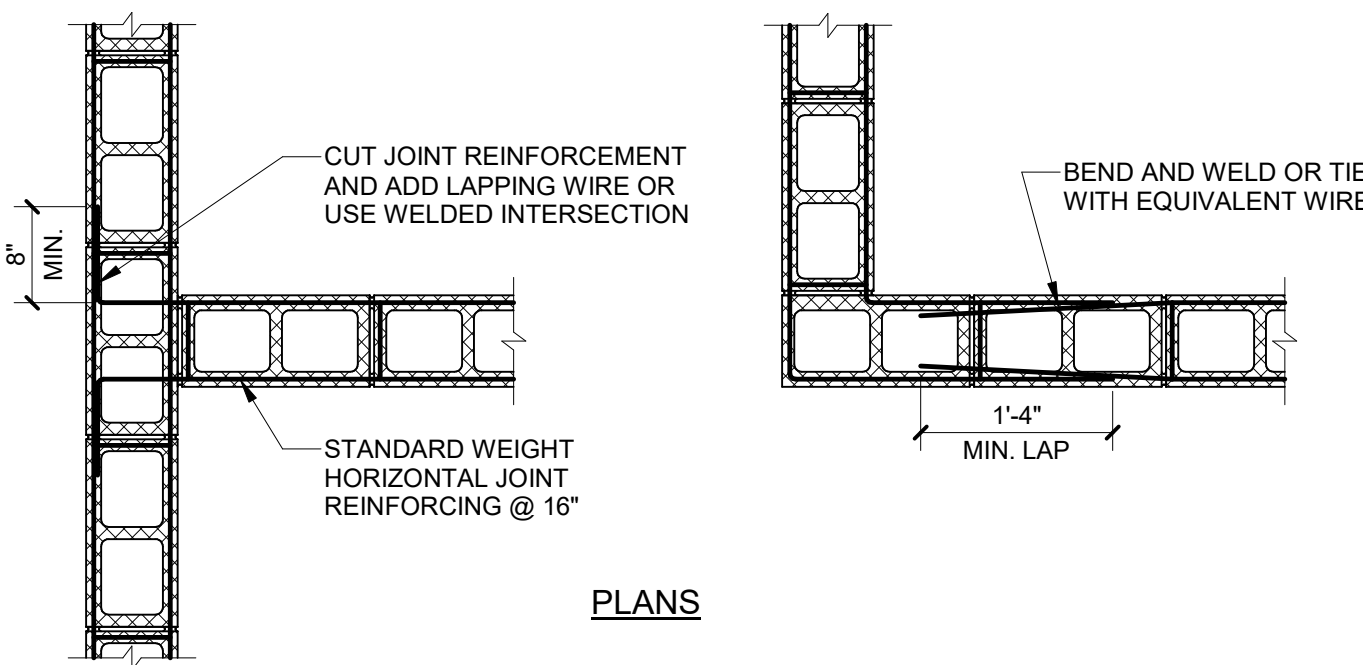


WALL REINFORCING AT CONTRACTION JOINT

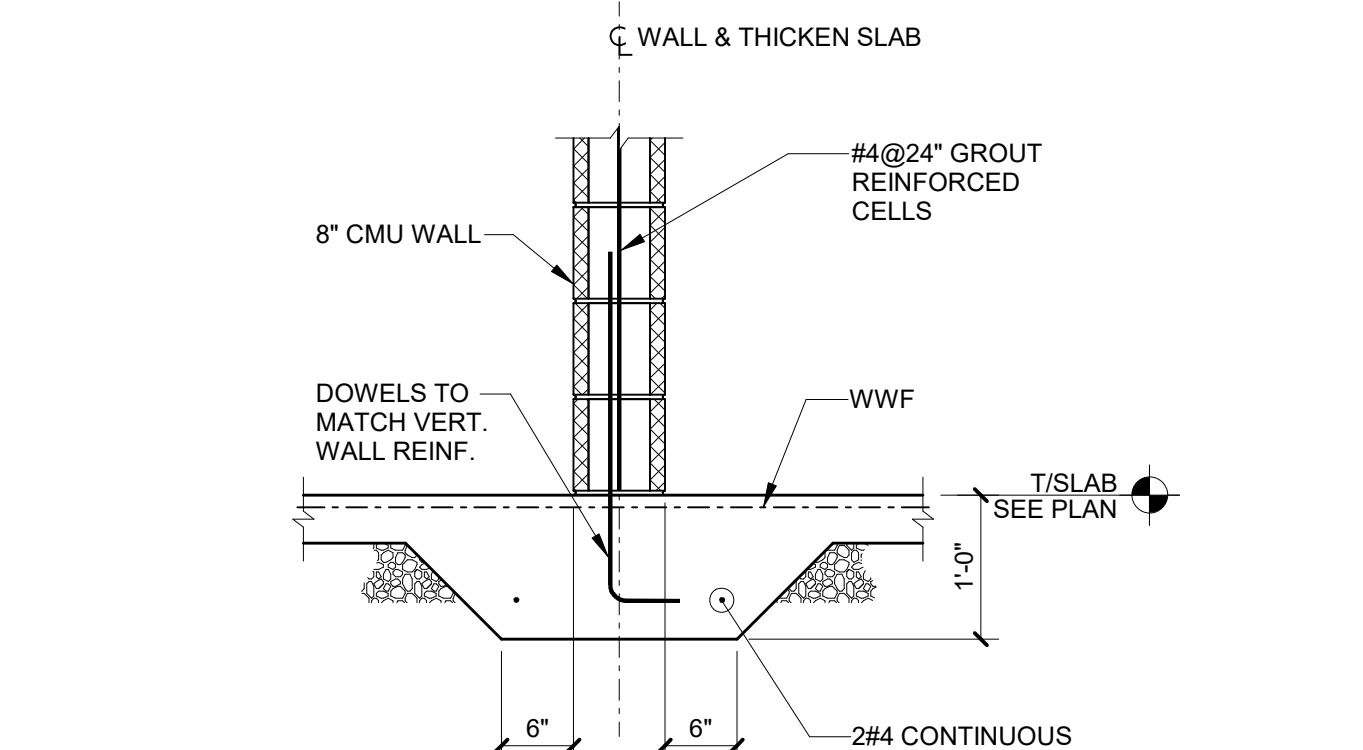
6 TYPICAL CONTRACTION JOINT DETAILS



5 TYPICAL BOND BEAM REINF. AT INTERSECTING CMU WALL

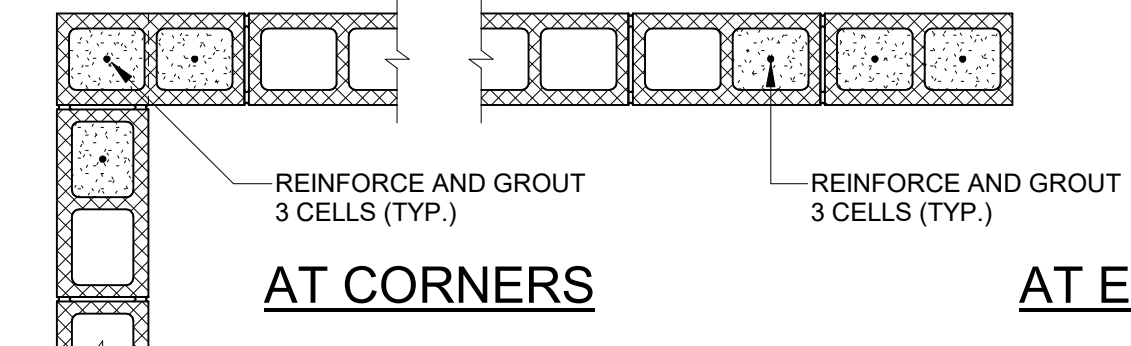


4 TYPICAL JOINT REINF. AT INTERSECTING CMU WALLS



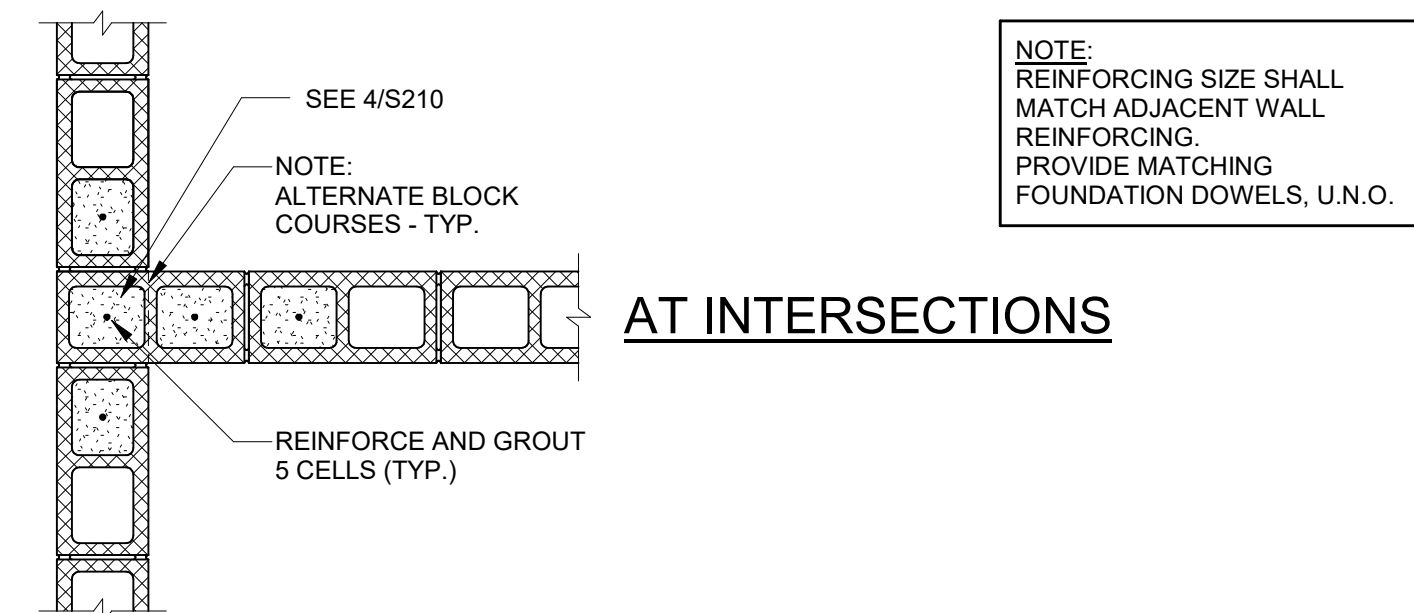
3 TYPICAL THICKENED SLAB ON GROUND AT NON-LOAD BEARING CMU WALLS

| CMU WALL OPENING REINFORCEMENT SCHEDULE | |
|-----------------------------------------|------------------------------------------|
| OPENING WIDTH "W" | VERTICAL REINFORCEMENT (EACH SIDE) |
| UP TO 6'-0" | 2 BARS TO MATCH TYPICAL WALL REINFORCING |
| 6'-0" TO 8'-0" | 3 BARS TO MATCH TYPICAL WALL REINFORCING |
| 8'-0" TO 12'-0" | 4 BARS TO MATCH TYPICAL WALL REINFORCING |

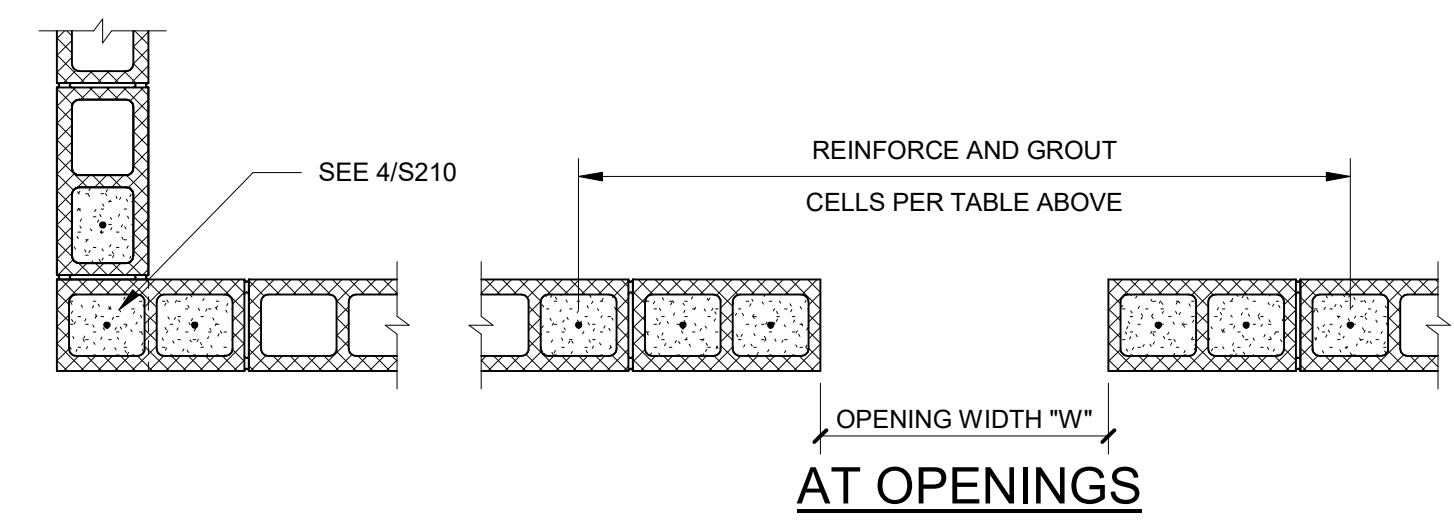


AT CORNERS

AT ENDS

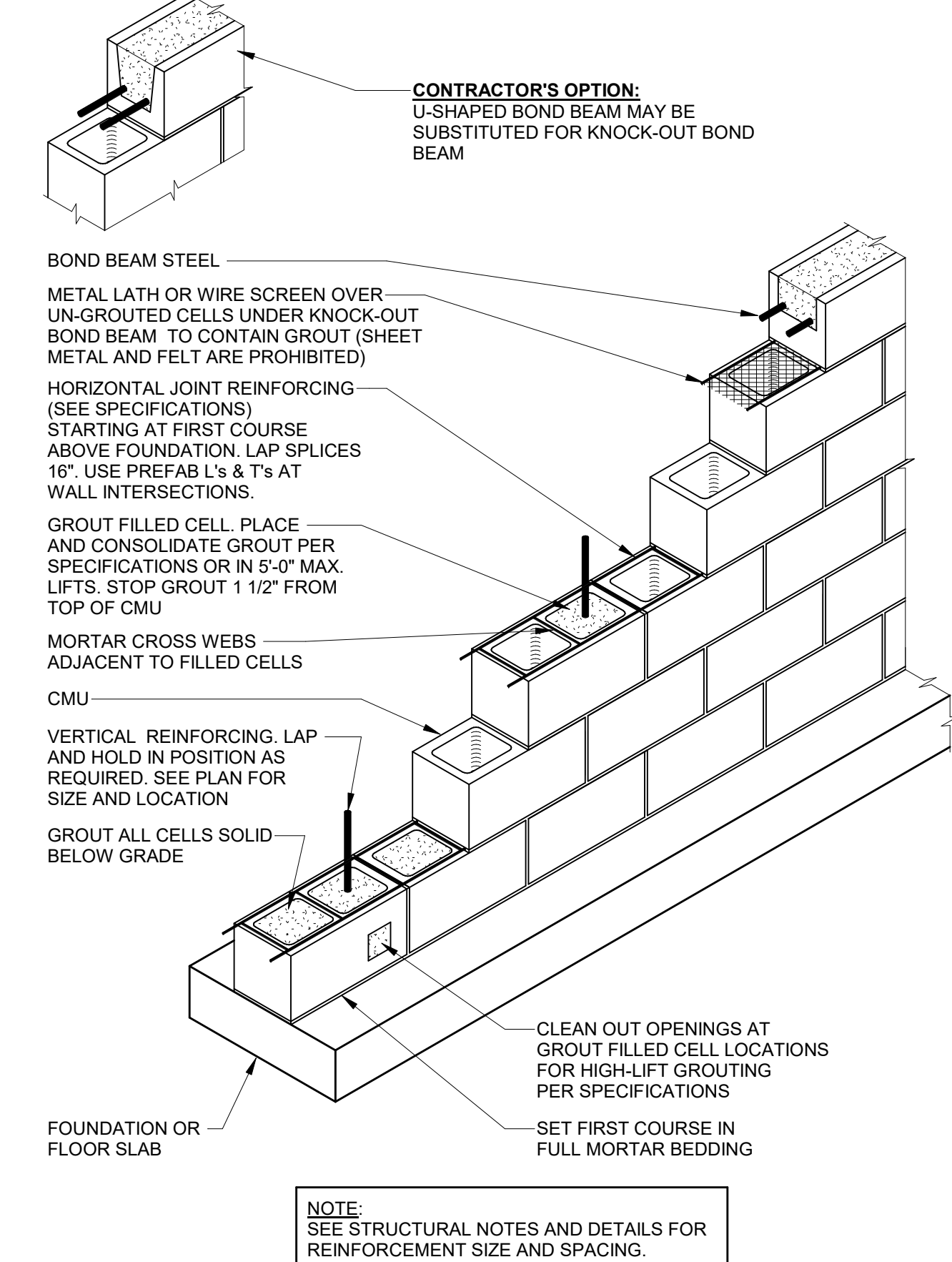


AT INTERSECTIONS

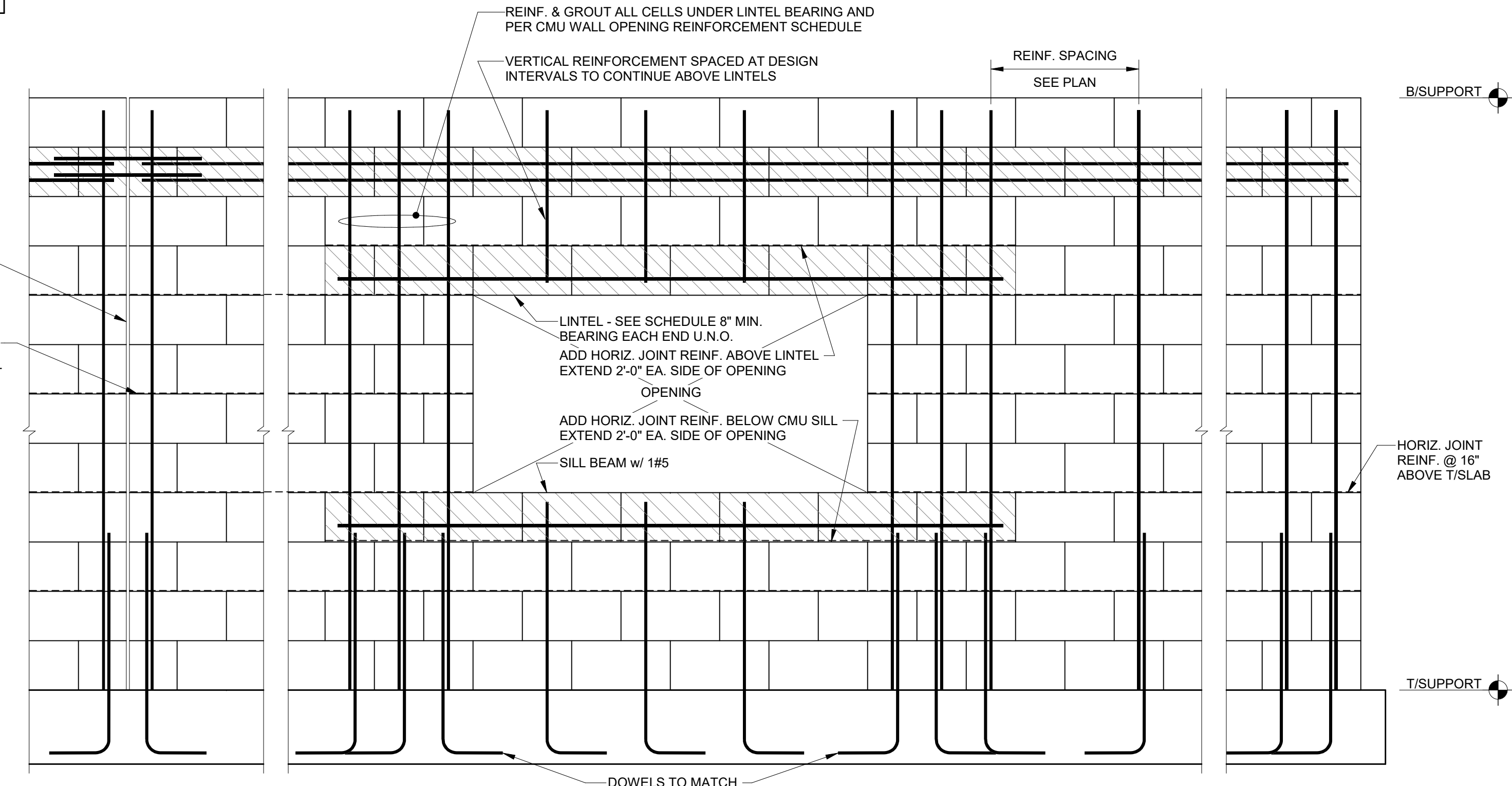


AT OPENINGS

2 TYPICAL REINFORCING AT CMU WALLS CORNERS, ENDS, INTERSECTIONS & OPENINGS



1 TYPICAL CMU WALL CONSTRUCTION



CONTRACTION JOINT

WALL OPENINGS

CORNER

NOTES:
1. SEE STRUCTURAL NOTES FOR MAXIMUM SPACING OF CONTROL JOINTS.
2. LOCATE CONTROL JOINTS 2'-0" MINIMUM FROM SIDES OF OPENINGS

7 TYPICAL CMU WALL REINFORCING - ELEVATION (NON-LOAD BEARING WALLS)

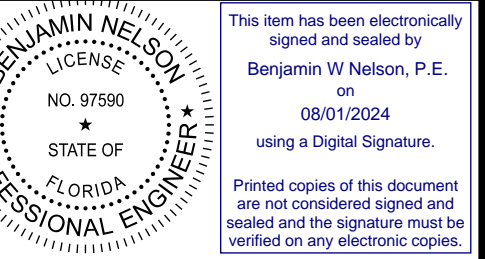
| CONSTRUCTION INFO | |
|--------------------------------------------------------|---------------------------------------------|
| GENERAL CONTRACTOR BRASFIELD & GORRIE WALKER COX | PRECAST CONCRETE METROMONT JIM BACHER |
| HCA DESIGN MANAGER: RYAN ROHE | HCA CONSTRUCTION MANAGER: GREG ATKINSON |



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| REVISIONS | |
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DR. BY CG
CK. BY BN
PROJ. A00324
DATE 07/30/2024

MASONRY
SECTIONS AND
DETAILS

S210

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1. LAP NOTES:

2. LAP SPICE LENGTHS ARE BASED ON ACI 318.

3. CATEGORY DEFINITIONS: (SEE FIGURES PROVIDED FOR ADDITIONAL INFORMATION)

CATEGORY 1 - OTHER CLASSES - DOES NOT MEET CATEGORY 2 OR 3.

CATEGORY 2 - CLEAR SPACING OF BARS BEING DEVELOPED OR SPICED NOT LESS THAN 2db AND CLEAR COVER NOT LESS THAN 4db.

CATEGORY 3 - CLEAR COVER NOT LESS THAN 2db AND THE CLEAR SPACING NOT LESS THAN 4db.

4. MINIMUM LAP SPICE SHALL NOT BE LESS THAN 12 INCHES.

5. MULTIPLY ABOVE LAP LENGTHS BY 1.25 FOR GRADE 75 REINFORCING STEEL.

6. MULTIPLY ABOVE LAP LENGTHS BY 1.3 FOR LIGHTWEIGHT CONCRETE.

7. TOP BARS ARE DEFINED AS HORIZONTAL REINFORCEMENT SO PLACED THAT MORE THAN 12 INCHES OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE DEVELOPMENT LENGTH OR SPICE.

8. COMPRESSION LAPS FOR GR. 60 UNCOATED BARS IN NORMAL WEIGHT CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI SHALL BE 30db. USE 44db FOR GR. 75 COMPRESSION LAP.



| | |
|--------------------------------------------------------------------------|------------------------------------------------------------|
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