A. GENERAL

- CONTRACT DOCUMENTS INCLUDE, BUT ARE NOT LIMITED TO, THE STRUCTURAL DOCUMENTS (DRAWINGS AND SPECIFICATIONS), BUT DO NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR MATERIAL PREPARED AND SUBMITTED BY THE CONTRACTOR.
- REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION, OR ASSOCIATION OR TO CODES OF LOCAL OR STATE AUTHORITIES, SHALL MEAN THE LATEST STANDARD, CODE, SPECIFICATION OR TENTATIVE SPECIFICATION ADOPTED AT THE DATE OF TAKING BIDS, UNLESS SPECIFICALLY STATED OTHERWISE.
- 4. CONTRACT DOCUMENTS SHALL GOVERN IN THE EVENT OF A CONFLICT WITH THE CODE OF PRACTICE OR SPECIFICATIONS OF ACI, PCI, AISC, SJI OR OTHER STANDARDS. WHERE A CONFLICT OCCURS WITHIN THE CONTRACT DOCUMENTS, THE STRICTEST REQUIREMENT SHALL
- 5. MATERIAL, WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE REFERENCED BUILDING
- 6. CONTRACTOR SHALL COORDINATE THE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DOCUMENTS. ARCHITECT/STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS SEE THE ARCHITECTURAL DRAWINGS.
- CONTRACTOR SHALL OBTAIN AND COORDINATE EDGE OF SLAB DIMENSIONS, OPENING LOCATIONS AND DIMENSIONS, DEPRESSED SLAB LOCATIONS AND EXTENTS, SLAB SLOPES, CURB LOCATIONS, AND CMU WALL LOCATIONS. ARCHITECT/STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION.
- CONTRACTOR SHALL VERIFY EXISTING DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS BEFORE STARTING WORK. ARCHITECT/STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY
- CONTRACTOR SHALL VERIFY THAT MISCELLANEOUS FRAMING SHOWN ON THE STRUCTURAL DRAWINGS FOR MECHANICAL EQUIPMENT, OWNER-FURNISHED ITEMS, PARTITIONS, ETC. IS CONSISTENT WITH THE REQUIREMENTS OF SUCH ITEMS. CONTRACTOR SHALL VERIFY EQUIPMENT WEIGHTS, REQUIRED OPENING SIZES AND LOCATIONS IDENTIFIED ON THE STRUCTURAL DRAWINGS ARE IN AGREEMENT WITH FINAL ARCHITECTURAL AND MECHANICAL SHOP DRAWINGS AND SUBMITTALS.
- 10. CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, SAFETY, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION. CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA REGULATIONS.
- 11. THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED FORM. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED. FURNISHED. AND INSTALLED BY THE CONTRACTOR. CONTRACTOR IS RESPONSIBLE FOR CONSTRUCTABILITY ANALYSIS, AND ERECTION PROCEDURES, INCLUDING DESIGN AND ERECTION OF FALSEWORK, TEMPORARY BRACING, ETC.
- 12. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED.
- 13. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE STRUCTURAL ELEMENTS AND CONNECTIONS SHOWN IN THE CONTRACT DOCUMENTS. REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS REGARDING ARRANGEMENT AND SIZES OF MEMBERS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGN LOADS AND CONTRACT DOCUMENT DETAILS. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE ARCHITECT / STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL ENGINEER. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE ARCHITECT / STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR FRRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE
- 14. WHERE A SECTION OR DETAIL IS SHOWN OR DETAILED FOR ONE CONDITION. IT SHALL APPLY TO ALL SIMILAR AND LIKE CONDITIONS. DETAILS LABELED "TYPICAL" OR "TYP." ON THE STRUCTURAL DRAWINGS APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR. THE CONTRACTOR SHALL CONSIDER ALL OF THE CONTRACT DOCUMENTS IN DETERMINING SIMILAR AND LIKE CONDITIONS.
- 15. STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR THE DESIGN OF STEEL STAIRS, HANDRAILS, CURTAIN WALL/WINDOW WALL SYSTEMS, COLD-FORMED METAL FRAMING, OR OTHER SYSTEMS NOT SHOWN IN THE STRUCTURAL DOCUMENTS. STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR THE CONNECTION OF THESE SYSTEMS TO THE STRUCTURE. SUCH SYSTEMS SHALL BE DESIGNED BY AN ENGINEER LICENSED IN THE PROJECT STATE AND SHALL BE FURNISHED AND INSTALLED BY OTHERS AS REQUIRED BY OTHER PORTIONS OF THE CONTRACT DOCUMENTS.
- 16. USE ONLY DIMENSIONS INDICATED ON THE CONTRACT DOCUMENTS. DO NOT SCALE DRAWINGS OR MEASURE OBJECTS IN ELECTRONIC FILES. NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES.
- 17. THE OWNER SHALL ESTABLISH A PERIODIC MAINTENANCE PROGRAM TO PROTECT THE STRUCTURE FROM DETERIORATION. THE MAINTENANCE PROGRAM IS THE RESPONSIBILITY OF THE OWNER AND SHOULD INCLUDE, BUT IS NOT LIMITED TO, THE FOLLOWING:

PAINTING OF EXPOSED STEEL THAT IS NOT GALVANIZED. INSPECTION AND MAINTENANCE OF PROTECTIVE COATINGS, SEALANTS, CAULKED JOINTS, EXPANSION JOINTS, AND CONTROL JOINTS. REPAIR OF SPALLS AND CRACKS IN CONCRETE ELEMENTS

REPAIR AND RESTORATION OF CORRODED ELEMENTS. CLEANOUT OF DRAINS INCLUDING ALL ROOF AND TRENCH DRAINS AND SCUPPERS CLEANING OF STRUCTURAL ELEMENTS EXPOSED TO HARSH CHEMICALS INCLUDING DE-ICING CHEMICALS. REPLACEMENT OF WORN BEARING PADS

- 18. THE USE OF STRUCTURAL BIM OR CAD FILES IS PROHIBITED WITHOUT WRITTEN CONSENT FROM THE STRUCTURAL ENGINEER.
- B. CODE/DESIGN CRITERIA
- 1. STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE, SEVENTH EDITION (2020).
- 2. GRAVITY LOADS
- 2.1 UNIFORM FLOOR LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE)
- GENERAL AREAS
- 2.2 UNIFORM ROOF LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE)
- 20 PSF RAIN INTENSITY (15-MIN. STORM DURATION), i 8.98 IN/HR
- PONDING AND DRIFT EFFECTS HAVE BEEN INCLUDED IN THE DESIGN. 2.3 CONCENTRATED FLOOR LOADS - DISTRIBUTED OVER AN AREA OF 2.5 FEET SQUARE, UNLESS NOTED OTHERWISE:

PARKING: FLOOR GUARD RAIL

IMPACT RESISTANCE.

3000 LB (ON AREA OF 4.5"x4.5") 6000 LB (BETWEEN 18 AND 27 ABOVE FLOOR OVER 1 SQUARE FOOT)

SCALE: 1"=1'-0"

WIND LOADS:

BASIC DESIGN WIND SPEED. V ALLOWABLE STRESS DESIGN WIND SPEED, Vasd 125 MPH RISK CATEGORY

INTERNAL PRESSURE COEFFICIENT, GCpi +/-0.18 THE PROJECT IS LOCATED IN A WIND-BORNE DEBRIS REGION AND REQUIRED MISSILE

SCALE: 1-1/2"=1'-0"

B. CODE/DESIGN CRITERIA (CONT.)

- 4. MAXIMUM ESTIMATED DEFLECTIONS LISTED BELOW ARE EXPECTED TO OCCUR AND SHALL BE CONSIDERED BY THE CONTRACTOR AND CLADDING DESIGNERS IN THE PERFORMANCE OF THE
- 4.1 SPANDREL BEAM DEFLECTIONS: 1/2" RELATIVE SUPERIMPOSED LOAD DEFLECTION BETWEEN LEVELS. RELATIVE SUPERIMPOSED LOAD DEFLECTION IS THE DEFLECTION ANTICIPATED TO OCCUR BETWEEN ADJACENT FLOORS AFTER THE CLADDING HAS BEEN ERECTED. THE RELATIVE SUPERIMPOSED DEFLECTION DOES NOT INCLUDE DEFLECTIONS DUE TO THE WEIGHT OF THE FLOOR FRAMING ITSELF, NOR DOES IT
- 4.2 MAXIMUM ESTIMATED DEFLECTIONS (IN INCHES) ARE AS FOLLOWS:

INCLUDE AN ALLOWANCE FOR CONSTRUCTION TOLERANCE.

LIVE LOAD DEAD + LIVE LOAD ROOF MEMBERS: L/360 L/240 FLOOR MEMBERS: L/360 L/240

WHERE, L = SPAN LENGTH (IN INCHES) BETWEEN CENTERLINES OF SUPPORTS. FOR CANTILEVERS, L IS TWICE THE LENGTH OF THE CANTILEVER.

4.3 MAXIMUM LATERAL INTER-STORY DRIFT DUE TO WIND LOADING (10-YEAR MEAN RECURRENCE INTERVAL) ARE AS FOLLOWS:

H/400 AT THE CENTER OF MASS MAXIMUM OF 0.6" AT THE BUILDING

5. SPECIAL INSPECTIONS:

- 5.1 SPECIAL INSPECTIONS IN ACCORDANCE WITH CHAPTER 17 OF THE BUILDING CODE AND THE REQUIREMENTS OF THE FLORIDA STATUTES FOR THRESHOLD BUILDINGS ARE REQUIRED. INSPECTIONS AS DEFINED IN THE THRESHOLD LETTER SHALL BE MADE BY A QUALIFIED PERSON AN APPROVED AS A THRESHOLD INSPECTOR. INSPECTOR SHALL OBSERVE WORK FOR CONFORMANCE WITH THE APPROVED DRAWINGS AND SPECIFICATIONS.
- 5.2 INSPECTION REPORTS SHALL BE FURNISHED TO THE BUILDING OFFICIAL, ARCHITECT, AND STRUCTURAL ENGINEER. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR, AND, IF NOT CORRECTED, SHALL BE REPORTED TO THE BUILDING OFFICIAL, ARCHITECT, AND STRUCTURAL ENGINEER.
- 5.3 SPECIAL INSPECTOR SHALL SUBMIT A FINAL REPORT STATING THAT THE STRUCTURAL WORK WAS, TO THE BEST OF THEIR KNOWLEDGE, PERFORMED IN ACCORDANCE WITH THE APPROVED DRAWINGS, SPECIFICATIONS, AND THE BUILDING CODE.
- 5.4 THE TYPES OF WORK LISTED IN THE THRESHOLD LETTER REQUIRE SPECIAL INSPECTION: (REFER TO BUILDING CODE AND SPECIFICATIONS FOR DETAILED INSPECTION REQUIREMENTS.) THE THRESHOLD INSPECTION PLAN IS PROVIDED BY THE STRUCTURAL ENGINEER IN SPECIFICATION SECTION 01410.
- 6. NO PROVISIONS HAVE BEEN MADE FOR FUTURE HORIZONTAL OR VERTICAL EXPANSION.
- 7. IN THE PROFESSIONAL OPINION OF STANLEY D. LINDSEY & ASSOCIATES. LTD.. THE STRUCTURAL CONTRACT DOCUMENTS FOR THIS PROJECT HAVE BEEN PREPARED IN ACCORDANCE WITH THE DESIGN CRITERIA AS SET FORTH IN THE ABOVE REFERENCED BUILDING CODE.

C. DEFERRED STRUCTURAL SUBMITTALS

- 1. DEFERRED SUBMITTALS, AS DEFINED BY THE BUILDING CODE, SHALL BE SUBMITTED TO THE BUILDING OFFICIAL BY THE CONTRACTOR. THE DEFERRED SUBMITTALS SHALL BE SIGNED AND SEALED BY A LICENSED ENGINEER IN THE PROJECT STATE.
- 2. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR THE DESIGN OF THE DEFERRED SUBMITTAL COMPONENTS OR THE CONNECTION TO THE STRUCTURE. THE STRUCTURAL DESIGN OF THE COMPONENTS AND THE CONNECTION TO THE STRUCTURE IS DELEGATED TO A SPECIALTY ENGINEER WHO SHALL BE ENGAGED BY THE CONTRACTOR, VENDOR, AND / OR SUPPLIER OF THE COMPONENTS AS PART OF THE DEFERRED SUBMITTAL PROCESS.
- THE CONTRACTOR SHALL SUBMIT THE DEFERRED SUBMITTAL TO THE ARCHITECT / STRUCTURAL ENGINEER FOR REVIEW. AFTER REVIEW BY THE ARCHITECT / STRUCTURAL ENGINEER THE CONTRACTOR SHALL SUBMIT THE REVIEWED SUBMITTAL TO THE BUILDING OFFICIAL PER SECTION 107.3 OF THE BUILDING CODE.
- 4. THE ITEMS LISTED BELOW ARE IDENTIFIED AS DEFERRED SUBMITTALS. REFER TO THE ARCHITECTURAL. MECHANICAL. ELECTRICAL. PLUMBING. AND CIVIL DRAWINGS AND SPECIFICATIONS FOR ADDITIONAL DEFERRED SUBMITTAL COMPONENTS. ALL COSTS ASSOCIATED WITH THE PREPARATION OF THE DEFERRED SUBMITTAL, INCLUDING THE SPECIALTY ENGINEERS DESIGN FEES, SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
 - GROUND IMPROVEMENT STRUCTURAL PRECAST ELEMENTS
 - ARCHITECTURAL PRECAST ELEMENTS TEMPORARY BRACING / SHORING FOR THE STABILITY OF THE STRUCTURE DURING CONSTRUCTION
 - CONCRETE FORMWORK STRUCTURAL STEEL CONNECTIONS
 - METAL STAIRS, LANDINGS, HANDRAILS, AND GUARDRAILS EXTERIOR BUILDING SIGNAGE
- ANCHORAGE OF EXTERIOR ARCHITECTURAL, MECH., ELEC., AND PLUMBING EQUIPMENT ATTACHMENT OF ROOFTOP EQUIPMENT, PIPING, & DUCTWORK TO THE UNDERLYING STRUCTURE GLAZING SYSTEMS (e.g., CURTAIN WALL, WINDOW WALL, AND STOREFRONT SYSTEMS)
- PREFABRICATED / PRE-ENGINEERED ARCHITECTURAL ELEMENTS AND STRUCTURES OTHER ELEMENTS INDICATED SPECIFICALLY IN THE CONTRACT DOCUMENTS BUILDING MAINTENANCE SYSTEMS
- 5. DEFERRED SUBMITTAL COMPONENTS SHALL BE DESIGNED FOR THE LOADS AS DEFINED BY THE APPLICABLE BUILDING CODE WITH DESIGN DATA DEFINED IN THE SECTION B OF THE STRUCTURAL NOTES.
- 6. THE DESIGN OF ITEMS LISTED BELOW ARE THE RESPONSIBILITY OF THE CONTRACTOR BUT ARE NOT CONSIDERED A DEFERRED SUBMITTAL AND ARE NOT TO BE SUBMITTED TO THE DESIGN TEAM. ALL COSTS ASSOCIATED WITH THE DESIGN OF THESE ELEMENTS, INCLUDING THE SPECIALTY ENGINEERS DESIGN FEES, SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
 - TEMPORARY SUPPORT OF EXCAVATION SYSTEMS
 - BLASTING PLANS TEMPORARY BRACING / SHORING FOR THE STABILITY OF THE STRUCTURE DURING

SCALE: 3/4"=1'-0"

CONSTRUCTION ALL OTHER ELEMENTS IDENTIFIED IN THE CONTRACT DOCUMENTS.

STRUCTURAL NOTES

D. FOUNDATION

- 1. FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT PREPARED BY NOVA. REPORT NUMBER 10116-2023123 DATED JULY 26. 2023. STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT FROM THOSE ASSUMED FOR DESIGN.
- 2. STRUCTURAL TESTING/INSPECTION AGENCY SHALL CERTIFY THE BEARING MEDIUM.
- 3. INDIVIDUAL SPREAD FOOTINGS AND CONTINUOUS FOOTINGS SHALL BEAR ON SOIL CAPABLE OF SUPPORTING 6000 PSF AND 6000 PSF, RESPECTIVELY AFTER RECEIVING SOIL
- IMPROVEMENT FROM RAMMED AGGREGATE PIER SYSTEM. 3.1 NO FOOTINGS SHALL BEAR ON ROCK. UNDERCUT ROCK A MINIMUM OF 2 FEET BELOW
- 4. FOUNDATION WALLS ARE DESIGNED FOR LATERAL PRESSURES DUE TO THE FOLLOWING EQUIVALENT FLUID DENSITIES:
- WALLS SUPPORTED AT TOP (AT-REST CONDITION): 60 PCF WALLS FREE TO DISPLACE AT TOP (ACTIVE CONDITION): 40 PCF

BOTTOM OF FOOTING AND REPLACE WITH STRUCTURAL FILL.

- 5. PROOFROLL BUILDING AREAS WITH TWO COMPLETE COVERAGES OF A LOADED DUMP-TRUCK OR SCRAPER. REPLACE SOFT AREAS WITH COMPACTED STRUCTURAL FILL AS REQUIRED BY THE
- 6. DENSIFY BUILDING AREAS AND A MINIMUM OF 10'-0" OUTSIDE THE BUILDING PERIMETER USING A VIBRATORY ROLLER (SEE SPECIFICATIONS).
- 7. UNDERCUT THE ENTIRE BUILDING AREA TO THE EXTENT SHOWN ON THE STRUCTURAL DOCUMENTS AND REPLACE WITH COMPACTED STRUCTURAL FILL AS REQUIRED BY THE SPECIFICATIONS.

E. REINFORCEMENT

- 1. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE. 2. WELDED WIRE REINFORCEMENT SHALL CONFORM TO ASTM A1064 AND HAVE MINIMUM SIDE
- AND END LAPS OF ONE CROSS WIRE SPACING PLUS 2", BUT NOT LESS THAN 6. 3. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE REINFORCING BAR SIZES AND PLACEMENT. WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS,
- ELEVATIONS, AND DETAILS IS NOT ACCEPTABLE. 4. PROVIDE DOWELS FROM FOUNDATIONS THE SAME SIZE AND NUMBER AS THE VERTICAL WALL
- OR COLUMN REINFORCING, UNLESS NOTED OTHERWISE.
- 5. PLACE REINFORCEMENT AS FOLLOWS. UNLESS NOTED OTHERWISE:

•	,	L HEIM ONOLMEN	7.0 1 0220	one, oneless n	OTED OTHERWISET
	5.1	CAST-IN-PLACE	CONCRETE	REINFORCEMEN	T COVER

PERMANENTLY EXPOSED TO EARTH:

BEAM/GIRDER STIRRUPS

CAST AGAINST THE EARTH	3"	CLEAR	
EXPOSED TO EARTH OR WEATHER:			
FOR BARS LARGER THAN A NO. 5 BAR	2"	CLEAR	
NO. 5 BARS OR SMALLER	1-1/2"	CLEAR	
PARKING SLABS:			
TOP	1-1/2"	CLEAR	
BOTTOM	3/4"	CLEAR	
NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH:			
WALLS	3/4"	CLEAR	
SLABS	3/4"	CLEAR	
#3 COLUMN TIES	1-5/8"	CLEAR	
#4 AND LARGER COLUMN TIES	1-1/2"	CLEAR	

5.2 PRECAST CONCRETE REINFORCEMENT COVER (MANUFACTURED UNDER PLANT CONTROL

1-1/2" CLEAR

1-1/2" CLEAR

1-1/4" CLEAR

EXPOSED TO EARTH OR WEATHER: WALL PANELS: REINFORCING BARS LARGER THAN NO. 11 AND PRESTRESSING TENDONS LARGER THAN 1-1/2" 1-1/2" CLEAR REINFORCING BARS NO. 11 AND SMALLER AND PRESTRESSING TENDONS 1-1/2" AND SMALLER 3/4" CLEAR OTHER MEMBERS: REINFORCING BARS LARGER THAN NO. 11 AND 2" CLEAR PRESTRESSING TENDONS LARGER THAN 1-1/2" REINFORCING BARS NO. 6 TO NO. 11 AND

PRESTRESSING TENDONS 5/8" AND SMALLER, AND, W31 AND D31 AND SMALLER WIRE

5.3 MASONRY CONCRETE REINFORCEMENT COVER

TO 1-1/2"

PRESTRESSING TENDONS LARGER THAN 5/8"

REINFORCING BARS NO. 5 AND SMALLER,

MASONRY REINFORCING STEEL SHALL BE PLACED IN THE CENTER OF CMU CELLS, UNLESS NOTED OTHERWISE. IF REINFORCING STEEL IS NOTED TO BE PLACED OTHER THAN THE CENTER OF THE CELL, PLACE BAR TO ACHIEVE 2" CLEAR COVER BETWEEN REINFORCING AND OUTSIDE FACE OF CMU BLOCK.

6. REINFORCEMENT SHALL BE SPLICED ONLY AT LOCATIONS SHOWN OR NOTED IN THE STRUCTURAL DOCUMENTS, EXCEPT REINFORCEMENT MARKED "CONTINUOUS" CAN BE SPLICED AT LOCATIONS DETERMINED BY CONTRACTOR. SPLICES AT OTHER LOCATIONS SHALL BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER. REINFORCING STEEL SPLICES SHALL BE AS FOLLOWS, UNLESS NOTED OTHERWISE:

CONCRETE: CLASS B TENSION LAP - SEE REINFORCING LAP LENGTH SCHEDULE MASONRY: SEE REINFORCING LAP LENGTH SCHEDULE

F. CAST-IN-PLACE CONCRETE

SCALE: 1/2"=1'-0"

- 1. CONCRETE WORK SHALL CONFORM TO ACI 318 AND CRSI STANDARDS
- 2. CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES:

2.1	NORMAL WEIGHT	STRUCTURAL	CONCRETE:			
			EXPOSURE CLASS	28-DAY MIN. COMPRESSIVE STRENGTH, F'C	,	MAX. AGGREGAT SIZE
	FOOTINGS		C1	4,000 PSI	0.55	1 1/2"
	DEDECTALO		0.4	4 000 DOT	0 55	4 11

PEDESTALS 4.000 PSI 0.55 INTERIOR FOUNDATION WALLS 4,500 PSI 0.45 EXTERIOR FOUNDATION WALLS 4,500 PSI 0.45 INTERIOR SLABS-ON-GRADE 4,000 PSI 0.48 EXTERIOR SLABS-ON-GRADE F2,C1 4,500 PSI 0.45 CURBS, PARAPETS, F1,C1 5,000 PSI 0.40 3/4" IMPACT BARRIERS INTERIOR WALLS 5,000 PSI 0.48 EXTERIOR WALLS F1,C1 5,000 PSI 0.40

ALL NORMAL WEIGHT CONCRETE SHALL BE CONSIDERED TO BE IN EXPOSURE CLASS FO, SO, WO, AND CO ACCORDING TO ACI 318 UNLESS NOTED OTHERWISE ABOVE OR ELSEWHERE ON THE STRUCTURAL DRAWINGS

0" 2' 4'

SCALE: 1/4"=1'-0"

F. CAST-IN-PLACE CONCRETE (CONT.)

3. CONCRETE MIX REQUIREMENTS

- 3.1 ALL CONCRETE SHALL BE PROPORTIONED TO COMPLY WITH ACI 318 CHAPTER 19 IN ACCORDANCE WITH THE EXPOSURE CLASS INDICATED. WHERE REQUIREMENTS SHALL APPLY. REFER TO THE SPECIFICATIONS FOR OTHER REQUIREMENTS FOR VARIOUS EXPOSURE CLASSES RELATIVE TO THE CEMENT TYPE, AIR ENTRAINMENT REQUIREMENTS, CHLORIDE ION LIMITS, POZZOLAN LIMITS, AND SHRINKAGE LIMITS.
- 3.2 CONCRETE SHALL BE CONSIDERED EXTERIOR CONCRETE IF THE CONCRETE IS PERMANENTLY EXPOSED TO THE WEATHER OR MOISTURE OR IF IT IS IN AN UNCONDITIONED SPACE IN ITS COMPLETED CONFIGURATION.
- 3.3 ALL CONCRETE SHALL SATISFY BOTH THE SPECIFIED MAXIMUM WATER TO CEMENT RATIO AND THE MINIMUM COMPRESSIVE STRENGTH, F'C, REQUIREMENTS.
- 4. PIPES OR DUCTS SHALL NOT EXCEED ONE-THIRD THE SLAB OR WALL THICKNESS UNLESS SPECIFICALLY DETAILED. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION OF SLEEVES, ACCESSORIES, ETC.
- 5. REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS. GROOVES. ORNAMENTS. CLIPS OR GROUNDS REQUIRED TO BE ENCASED IN CONCRETE AND FOR LOCATION OF ALL CONCRETE FINISHES AND SLAB DEPRESSIONS.
- 6.1 LOCATIONS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER
- 6.2 NO HORIZONTAL JOINTS ARE PERMITTED EXCEPT THOSE SHOWN ON THE STRUCTURAL
- 6.3 JOINTS SHALL BE LOCATED WITHIN THE MIDDLE THIRD OF SPANS OF SLABS, BEAMS, AND GIRDERS.
- 6.4 JOINTS IN GIRDERS SHALL BE OFFSET A MINIMUM DISTANCE OF TWO TIMES THE WIDTH OF INTERSECTING BEAMS.
- 7. DEFECTIVE AREAS IN CONCRETE INCLUDING, BUT NOT LIMITED TO, HONEY-COMBING, SPALLS, AND CRACKS WITH WIDTHS EXCEEDING 0.01 INCH SHALL BE REPAIRED. EXTENT OF DEFECTIVE AREA TO BE DETERMINED BY THE STRUCTURAL ENGINEER.

G. STRUCTURAL PRECAST CONCRETE

6. CONSTRUCTION JOINTS

- 1. PRECAST CONCRETE DESIGN, MANUFACTURE AND ERECTION SHALL CONFORM TO ACI 318 AND: PCI MNL-116, MANUAL FOR QUALITY CONTROL FOR PLANTS AND PRODUCTION OF PRECAST PRESTRESSED CONCRETE PRODUCTS.
- PCI MNL-120, DESIGN HANDBOOK: PRECAST AND PRESTRESSED CONCRETE. PCI MNL-123, MANUAL ON DESIGN AND TYPICAL DETAILS OF CONNECTIONS FOR PRECAST
- PRESTRESSED CONCRETE. PCI MNL-127, ERECTORS' MANUAL: STANDARDS AND GUIDELINES FOR THE ERECTION OF PRECAST CONCRETE PRODUCTS. PCI MNL-129, PRECAST PRESTRESSED PARKING STRUCTURES: RECOMMENDED PRACTICE
- PCI MNL-135, TOLERANCE MANUAL FOR PRECAST AND PRESTRESSED CONCRETE CONSTRUCTION.

FOR DESIGN AND CONSTRUCTION.

- 2.1 PRECAST MEMBERS SHALL BE CAPABLE OF SUPPORTING THE SUPERIMPOSED LOADS AS GIVEN IN THE CONTRACT DOCUMENTS, IN ADDITION TO THE SELF-WEIGHT OF THE PRECAST MEMBER, TOPPINGS, ETC.
- 2.2 PRECAST SHEAR WALLS AND THEIR CONNECTIONS SHALL BE DESIGNED TO RESIST, IN ADDITION TO GRAVITY LOADS, THE LOADS INDICATED IN THE SHEAR WALL SCHEDULE.
- 2.3 EFFECTS TO BE CONSIDERED BY THE PRECAST DESIGN ENGINEER IN THE DESIGN OF OF THE PRECAST ELEMENTS AND CONNECTIONS INCLUDE, BUT ARE NOT LIMITED TO, THE FOLLOWING:

GRAVITY WIND SEISMIC

ERECTION LOADS

DIFFERENTIAL TEMPERATURE BETWEEN PANEL FACES ECCENTRICITY OF APPLIED LOADS LOADS FROM MATERIALS SUPPORTED BY PANELS VOLUMETRIC CHANGES DUE TO TEMPERATURE, CREEP, & SHRINKAGE HANDRAILS LOADS IMPACT ON PANELS DESIGNED AS GUARDRAILS

LOAD COMBINATIONS SHALL BE IN ACCORDANCE WITH THE BUILDING CODE. 3. DESIGN OF STRUCTURAL PRECAST ELEMENTS AND THEIR CONNECTIONS SHALL BE THE SOLE

- RESPONSIBILITY OF THE CONTRACTOR. SUBMIT SHOP DRAWINGS. DESIGN LOAD DATA. AND SUPPORT REACTIONS OF PRECAST ELEMENTS AND THEIR CONNECTIONS SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE. REVIEW SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS REGARDING ARRANGEMENT AND SIZES OF MEMBERS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGN LOADS AND CONTRACT DOCUMENT DETAILS. SUCH REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF THE FULL RESPONSIBILITY FOR THE DESIGN OF THE PRECAST AND PRECAST CONNECTIONS NOT SPECIFIED IN THE CONTRACT DOCUMENTS.
- 4. SUBMIT REACTIONS OF ALL PRECAST ELEMENTS SUPPORTED BY STRUCTURAL ELEMENTS SHOWN ON THE DRAWINGS.
- 5. CONCRETE FOR PRECAST MEMBERS SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 5000 PSI.
- 6. FOR OPENINGS AND EMBEDDED ITEMS, CONTRACTOR SHALL COORDINATE WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING CONTRACT DOCUMENTS.

H. CONCRETE MASONRY

FOR CONCRETE MASONRY.

SCALE: 1/8"=1'-0"

- 1. CONCRETE MASONRY WORK SHALL CONFORM TO TMS 402/602, BUILDING CODE REQUIREMENTS
- AND SPECIFICATIONS FOR MASONRY STRUCTURES. 2. MINIMUM COMPRESSIVE STRENGTH OF CONCRETE MASONRY SHALL BE F'M = 2,000 PSI.
- 3. MORTAR SHALL BE TYPE S AND SHALL COMPLY WITH THE BUILDING CODE REQUIREMENTS
- 4. CONCRETE MASONRY UNITS SHALL BE GROUTED WITH 2.500 PSI COARSE GROUT AS SHOWN
- IN THE STRUCTURAL DOCUMENTS. GROUT SHALL CONFORM TO ASTM C476. 5. PROVIDE HORIZONTAL JOINT REINFORCEMENT WITH NO. 9 GAGE LONGITUDINAL WIRES AT
- 16" VERTICALLY, UNLESS NOTED OTHERWISE. PROVIDE SPECIAL ACCESSORIES FOR CORNERS, INTERSECTIONS, ETC. 6. PROVIDE OPEN BOTTOM BEAM BLOCK UNITS WITH 3" DEEP MINIMUM WEB OPENINGS AT HORIZONTAL REINFORCEMENT LOCATIONS. A MINIMUM CLEAR SPACE OF ONE BAR DIAMETER
- SHALL BE PROVIDED BETWEEN THE REINFORCING BARS AND THE FACE OF MASONRY UNITS. 7. PROVIDE CONTRACTION (CONTROL) JOINTS IN ALL CONCRETE MASONRY WALLS AT LOCATIONS APPROVED BY THE ARCHITECT AT A MAXIMUM SPACING OF 1.5 TIMES THE WALL HEIGHT OR 25'-0", WHICHEVER IS LESS. THE DISTANCE TO THE FIRST CONTRACTION

JOINT FROM CORNERS OR INTERSECTING WALLS IS LIMITED TO HALF OF THE ABOVE

- 8. PROVIDE DOVETAIL ANCHORS AT 16" C/C, UNLESS NOTED OTHERWISE, WHERE MASONRY
- WALLS ABUT CONCRETE SURFACES OR STRUCTURAL STEEL MEMBERS. 9. SUBMIT WRITTEN CONSTRUCTION PROCEDURES PRIOR TO THE START OF MASONRY

#4@24", FOR EXTERIOR WALLS, UNLESS NOTED OTHERWISE.

10. MINIMUM VERTICAL WALL REINFORCEMENT SHALL BE #4@24", FOR INTERIOR WALLS AND

SCALE: 3/32"=1'-0"

I. STRUCTURAL STEEL

- 1. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED ACCORDING TO THE ANSI/AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND THE AISC 303 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
- INDICATED DIFFER FROM REQUIREMENTS OF CHAPTER 19, THE STRICTER REQUIREMENT 2. STRUCTURAL STEEL SHALL BE OF THE FOLLOWING GRADE UNLESS NOTED OTHERWISE ON DRAWINGS:

ASTM A992

ASTM A36

ASTM A572, GRADE 50

ASTM A572, GRADE 50

W, WT, C, AND MC SHAPES L, M, S, MT, AND ST SHAPES ASTM A572, GRADE 50 ASTM A500, GRADE C HSS SHAPES STEEL PIPE SHAPES ASTM A53, GRADE B

3. BOLTS, ANCHOR RODS, AND HEADED STUDS:

PLATES AND BARS

ROUND ROD

HP-SHAPES

3.1 ALL BOLTS SHALL BE GROUP A OR GROUP B HIGH STRENGTH BOLTS WITH A 3/4" MINIMUM DIAMETER. THE FOLLOWING CONNECTIONS SHALL BE SLIP CRITICAL:

> SHEAR CONNECTIONS IN MOMENT CONNECTIONS WITH WELDED JOINTS (MOMENT FRAMES, CANTILEVERS, ETC.) FIELD MODIFIED CONNECTIONS

CONNECTIONS IN BRACED FRAMES OTHER CONNECTIONS AS NOTED IN THE DRAWINGS

ALL OTHER CONNECTIONS MAY BE BEARING TYPE. BOLT SHEAR STRENGTH SHALL BE DETERMINED IN ACCORDANCE WITH TABLES 7-1 OR 7-3 AS APPLICABLE IN THE AISC "STEEL CONSTRUCTION MANUAL".

3.2 ANCHOR RODS SHALL CONFORM TO ASTM F1554, GRADE 55, UNLESS NOTED OTHERWISE. 4. CONNECTIONS:

4.1 STEEL CONNECTIONS SHALL BE DETAILED BASED ON THE DESIGN INFORMATION

PROVIDED IN THE CONTRACT DOCUMENTS. DEVIATION FROM THE CONNECTION DETAILS DEPICTED IN THE CONTRACT DOCUMENTS SHALL NOT BE PERMITTED WITHOUT ADVANCE WRITTEN PERMISSION FROM THE STRUCTURAL ENGINEER. 4.2 THE SERVICES OF A CONNECTION DESIGN SPECIALTY ENGINEER (CDSE) SHALL BE INCLUDED IN THE CONTRACTOR'S SCOPE OF SERVICES. THE CDSE SHALL BE

LICENSED IN THE PROJECT STATE. THE CDSE IS RESPONSIBLE FOR REVIEWING THE STEEL SHOP DRAWINGS TO ENSURE THAT ALL CONNECTION DESIGN DETAILS HAVE BEEN CORRECTLY SHOWN ON THE SHOP DRAWINGS; AND, THE CDSE SHALL SUBMIT A SIGNED AND SEALED LETTER, WITH EACH SHOP DRAWING SUBMITTAL, CONFIRMING THE ABOVE REVIEW.

4.3 STANDARD SHEAR CONNECTIONS:

4.3.1 STANDARD SHEAR CONNECTIONS SHALL BE DETAILED AS DOUBLE-ANGLE, SINGLE-PLATE, SINGLE-ANGLE, OR TEE CONNECTIONS IN ACCORDANCE WITH CONNECTION TABLES IN PART 10 OF THE "STEEL CONSTRUCTION MANUAL". THE CDSE SHALL SUBMIT SIGNED AND SEALED CALCULATIONS FOR ALL SHEAR CONNECTIONS THAT CANNOT BE DETAILED COMPLETELY FROM THE ABOVE MANUAL FOR REVIEW BY THE STRUCTURAL ENGINEER.

0.05 X THE REQUIRED SHEAR DESIGN LOAD FOR THE CONNECTION 4.4 FOR WELDED CONNECTIONS, USE PREQUALIFIED WELDED JOINTS IN ACCORDANCE WITH

4.3.2 CONNECTIONS SHALL BE DETAILED TO CARRY A MINIMUM AXIAL LOAD OF

AISC AND THE STRUCTURAL WELDING CODE OF THE AMERICAN WELDING SOCIETY.

"NON-PREQUALIFIED JOINTS" SHALL BE QUALIFIED PRIOR TO FABRICATION. 4.5 THE REQUIRED REACTION USING LRFD LOAD COMBINATIONS SHALL BE AS SHOWN ON THE STRUCTURAL DRAWINGS. IF REACTIONS ARE NOT SHOWN:

FOR BEAMS W10 AND SMALLER: 10 KIP SHEAR CAPACITY

FOR BEAMS W12 AND LARGER: CONTACT THE STRUCTURAL ENGINEER 4.6 STEEL CONNECTIONS NOT SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY THE CDSE WITH THE EXCEPTION OF SIMPLE SHEAR CONNECTIONS AS DESCRIBED IN SECTION 4.3. THE CDSE SHALL SUBMIT SIGNED AND SEALED CALCULATIONS FOR ALL SUCH CONNECTIONS. THE CDSE SHALL USE THE UNIFORM FORCE METHOD WHEN APPLICABLE FOR ALL BRACE AND TRUSS TYPE

4.7 REVIEW OF THE SHOP DRAWINGS AND/OR CONNECTION CALCULATIONS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR AND CDSE OF THE FULL

5. ALL STEEL EXPOSED TO WEATHER OR MOISTURE SHALL BE GALVANIZED, UNLESS OTHERWISE DIRECTED BY THE ARCHITECT.

RESPONSIBILITY FOR THE DESIGN AND ADEQUACY OF SUCH CONNECTIONS.

6. STEEL BAR GRATING SHALL CONFORM TO THE FOLLOWING, UNLESS NOTED OTHERWISE:

6.1 STEEL BAR GRATING SHALL BE 1-1/4 x 3 /16 19-W-4 STEEL BAR GRATING.

STEEL BAR MATERIAL SHALL CONFORM TO ASTM A1011 CS TYPE B. 6.2 GRATING SURFACE SHALL BE PLAIN.

6.3 GRATING FINISH TO BE GALVANIZED IN ACCORDANCE WITH ASTM A123 OR A385.

6.4 FASTEN GRATING TO STEEL SUPPORTS WITH SADDLE CLIP AND SELF-DRILLING FASTENER AT EVERY SIXTH BEARING BAR ALONG SUPPORT (MIN. OF 2 CLIPS PER

DRAWING INDEX S001 STRUCTURAL NOTES S002 STRUCTURAL INSPECTIONS, NOTES CONT. AND WIND PRESSURES S101 FOUNDATION PLAN S102 LEVEL P2 PLAN S103 LEVEL P3 PLAN S104 LEVEL P4 PLAN S105 LEVEL P5 PLAN S106 LEVEL P6 PLAN S107 ROOF FRAMING PLAN FOUNDATION SECTIONS AND DETAILS FOUNDATION SECTIONS AND DETAILS FOUNDATION SECTIONS AND DETAILS S204 FOUNDATION SECTIONS AND DETAILS S210 MASONRY SECTIONS AND DETAILS S301 FRAMING SECTIONS AND DETAILS

CONSTRUCTION INFO GENERAL CONTRACTOR PRECAST CONCRETE **BRASFIELD & GORRIE** METROMONT JIM BATCHER WALKER COX HCA DESIGN MANAGER: HCA CONSTRUCTION MANAGER:

SCALE: 1/32"=1'-0"

GREG ATKINSON

RYAN ROHE

0" 8' 16'

SCALE: 1/16"=1'-0"

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Project No. 24024.00 COA No. 1329

REVISIONS

PROJ. 07/30/2024 DATE

DR. BY CG

CK. BY

STRUCTURAL SPECIAL INSPECTION STATEMENT

THIS STATEMENT OF SPECIAL INSPECTIONS IS SUBMITTED AS A CONDITION FOR PERMIT ISSUANCE IN ACCORDANCE WITH THE SPECIAL INSPECTION AND STRUCTURAL TESTING REQUIREMENTS OF THE BUILDING CODE. IT INCLUDES A SCHEDULE OF SPECIAL INSPECTION SERVICES APPLICABLE TO THIS PROJECT. THIS STATEMENT OF SPECIAL INSPECTIONS ENCOMPASSES THE STRUCTURAL DISCIPLINE.

SPECIAL INSPECTOR'S QUALIFICATIONS: THE SPECIAL INSPECTOR SHALL BE CERTIFIED TO ACT AS A SPECIAL INSPECTOR BY THE FLORIDA BOARD OF BUILDING CODES AND STANDARDS OR IS A LICENSED ARCHITECT OR REGISTERED ENGINEER WHO IS CERTIFIED UNDER CHAPTER 471 OR CHAPTER 481 TO CONDUCT INSPECTIONS OF THRESHOLD BUILDINGS.

THE SPECIAL INSPECTOR IS RESPONSIBLE FOR OBSERVING THAT THE CONSTRUCTION OF THE STRUCTURAL SYSTEM COMPLIES WITH THE OFFICIAL CONTRACT DOCUMENTS AND APPROPRIATE BUILDING CODES. THEY SHALL ENSURE THAT THE SHORING OF THE CONSTRUCTION IS IN ACCORDANCE WITH THE SHORING DRAWINGS PROVIDED BY THE CONTRACTOR TO THE ENFORCING AGENCY. THE INTERPRETATION OF THE PERMITTED DOCUMENTS WILL BE PROVIDED BY THE ARCHITECT-OF-RECORD OR ENGINEER-OF-RECORD.

THE SPECIAL INSPECTOR IS RESPONSIBLE TO THE ENFORCING AGENCY.

THE SPECIAL INSPECTOR WILL MEET WITH REPRESENTATIVES OF THE ENFORCING AGENCY, OWNER, CONTRACTOR, SUBCONTRACTORS, TESTING AGENCIES, AND OTHER INTERESTED PARTIES TO REVIEW SPECIFIC THRESHOLD INSPECTION REQUIREMENTS AND PROCEDURES. THIS ORIENTATION WILL BE ACCOMPLISHED AT A STRUCTURAL PRECONSTRUCTION CONFERENCE.

THE SPECIAL INSPECTOR OR HIS AUTHORIZED REPRESENTATIVE WILL VISIT THE PROJECT SITE AS DEMANDED BY THE CONSTRUCTION ACTIVITY TO INSPECT THE VARIOUS STRUCTURAL ELEMENTS DURING THE PROGRESS OF THE WORK, BUT NOT LESS THAN ONCE A WEEK. THE CONTRACTOR SHALL NOTIFY THE SPECIAL INSPECTOR A MINIMUM OF 24 HOURS IN ADVANCE OF ANY CONCRETE POUR OR OTHER CRITICAL CONSTRUCTION ACTIVITY.

THE SPECIAL INSPECTOR WILL REPORT OBSERVATION OF MATERIALS, WORKMANSHIP, OR OTHER FEATURES OF CONSTRUCTION DEVIATING FROM THE REQUIREMENTS OF THE OFFICIAL CONTRACT DOCUMENTS. GUIDELINES FOR OBSERVATIONS COVERING SPECIFIC AREAS OF CONSTRUCTION ARE LISTED WITHIN THE SCHEDULES INCLUDED IN THIS

QUALITY CONTROL INSPECTIONS AND TESTING BY AN INDEPENDENT TESTING AGENCY ARE REQUIRED BY THE SPECIFICATIONS FOR SOME STRUCTURAL ELEMENTS. REPORTS OF THESE INSPECTIONS AND TESTS WILL BE FURNISHED TO THE SPECIAL INSPECTOR TO SUPPLEMENT HIS OWN INSPECTIONS. IT IS THE RESPONSIBILITY OF THE SPECIAL

INSPECTOR TO ESTABLISH A CLEAR METHOD FOR MARKING ALL TESTED AND INSPECTED ITEMS. THE SPECIAL INSPECTOR SHALL IMMEDIATELY NOTIFY THE CONTRACTOR, ENFORCING AGENCY, ARCHITECT-OF-RECORD, AND ENGINEER-OF-RECORD WHEN ANY NONCONFORMING ITEM IS FOUND. THE SPECIAL INSPECTOR SHALL COMPLETE INSPECTION REPORTS AFTER EACH INSPECTION; AND SIGN, SEAL, AND FORWARD EACH REPORT TO THE ENFORCING AGENCY, THE ARCHITECT-OF-RECORD, STRUCTURAL ENGINEER-OF-RECORD, AND CONTRACTOR. REPORTS SHALL BE IN A

FORM APPROVED BY THE ENGINEER-OF-RECORD. AT THE COMPLETION OF ALL STRUCTURAL WORK AND AFTER THE CORRECTION OF ALL DEFICIENCIES, THE SPECIAL INSPECTOR SHALL SUBMIT A SIGNED AND SEALED STATEMENT TO THE BUILDING OFFICIAL CONFORMING TO THE FLORIDA STATUTES. COPIES WILL BE SENT TO THE BUILDING OFFICIAL, OWNER, ARCHITECT-OF-RECORD, STRUCTURAL ENGINEER-OF-RECORD, AND CONTRACTOR.

JOB SITE SAFETY AND MEANS AND METHODS OF CONSTRUCTION ARE SOLELY THE RESPONSIBILITY OF THE CONTRACTOR. THIS STATEMENT OF SPECIAL INSPECTIONS IS NOT INTENDED TO INCORPORATE ALL ASPECTS OF THE SPECIAL INSPECTION PROGRAM AS DEFINED IN THE SPECIAL INSPECTION AND STRUCTURAL TESTING REQUIREMENTS OF THE BUILDING CODE. REFER TO OTHER DISCIPLINES FOR REMAINING SPECIAL INSPECTIONS THAT MAY BE REQUIRED.

THE STRUCTURAL ENGINEER OF RECORD IS NOT RESPONSIBLE FOR ENSURING THE SPECIAL INSPECTION PROGRAM IS CONDUCTED IN ACCORDANCE WITH THIS PLAN.

THIS STATEMENT OF SPECIAL INSPECTIONS INCLUDES THE FOLLOWING INSPECTION CATEGORIES:

DRIVEN DEEP FOUNDATIONS ☐ HELICAL PILE FOUNDATIONS □ CONCRETE CONSTRUCTION

CONCRETE CONSTRUCTION (REBAR FABRICATION)

WOOD CONSTRUCTION

OTHER CONSTRUCTION

PROPERLY.

COLD FORM METAL CONSTRUCTION

☐ MASONRY CONSTRUCTION - LEVEL 1 ☐ COLD-FORMED STEEL DECK MASONRY CONSTRUCTION - LEVEL 2 COLD-FORMED STEEL TRUSSE ☐ MASONRY CONSTRUCTION - LEVEL 3 ☐ COLD-FORMED STEEL LIGHT FRAME ☐ CAST-IN-PLACE DEEP FOUNDATIONS ☐ STRUCTURAL STEEL CONSTRUCTION CONSTRUCTION ☐ OPEN-WEB STEEL JOISTS AND JOIST ☐ WOOD CONSTRUCTION

- - X

- - X

-

STRUCTURAL SPECIAL INSPECTION SCHEDULE: **FABRICATORS** FREQUENCY VERIFICATION AND INSPECTION TASK REFERENCED STANDARD PROJECT? CONTINUOUS PERIODIC **VERIFY FABRICATION AND IMPLEMENTATION PROCEDURES** STEEL CONSTRUCTION

STRUCTURAL SPECIAL INSPECTION SCHEDULE: SOILS										
VERIFICATION AND INSPECTION TASK	APPLICABLE TO THIS	FREQUENCY		REFERENCED STANDARD						
		CONTINUOUS	PERIODIC							
VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	Х	-	Х	-						
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	Х	-	Х	-						
PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	Х	-	Х	-						
VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	Х	×	-	-						
PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED	Х	-	Х	-						

SCALE: 1-1/2"=1'-0"

SCALE: 1"=1'-0"

STRUCTURAL SPECIAL INSPECT	TION S	CHED	ULE:	
STRUCTURAL STEEL CON	STRUC	CTION		

VERIFICATION AND INSPECTION TASK	APPLICABLE TO THIS PROJECT?	QC	QA	AISC REFERENC
INSPECTION TASKS PRIOR TO WELDING				
WELDER QUALIFICATION RECORDS AND CONTINUITY RECORDS	X	Р	0	
WPS AVAILABLE	X	Р	Р	
MATERIAL CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	X	Р	Р	
MATERIAL IDENTIFICATION (TYPE / GRADE)	X	0	0	
WELDER IDENTIFICATION SYSTEM	X	0	0	
FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY) • JOINT PREPARATIONS • DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL) • CLEANLINESS (CONDITION OF STEEL SURFACES) • TACKING (TACK WELD QUALITY AND LOCATION) • BACKING TYPE AND FIT (IF APPLICABLE)	X	0	0	AISC 360
FIT-UP OF CJP GROOVE WELDS OF HSS T-, Y- AND K-JOINTS WITHOUT BACKING (INCLUDING JOINT GEOMETRY) • JOINT PREPARATIONS • DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL) • CLEANLINESS (CONDITION OF STEEL SURFACES) • TACKING (TACK WELD QUALITY AND LOCATION)	X	Р	0	N5.4-1
CONFIGURATION AND FINISH OF ACCESS HOLES	X	0	0	
FIT-UP OF FILLET WELDS • DIMENSIONS (ALIGNMENT, GAPS AT ROOT) • CLEANLINESS (CONDITION OF STEEL SURFACES) • TACKING (TACK WELD QUALITY AND LOCATION)	х	0	0	
CHECK WELDING EQUIPMENT	Х	0	-	

CHECK WELDING EQUIPMENT	Х	O	-	
INSPECTION TASKS DURING TO WELDING				
CONTROL AND HANDLING OF WELDING CONSUMABLES • PACKAGING • EXPOSURE CONTROL	Х	0	0	
NO WELDING OVER CRACKED TACK WELDS	Х	0	0	
ENVIRONMENTAL CONDITIONS • WIND SPEED WITHIN LIMITS • PRECIPITATION AND TEMPERATURE	Х	0	0	
WPS FOLLOWED • SETTING OF WELDING EQUIPMENT • TRAVEL SPEED • SELECTED WELDING MATERIALS • SHIELDING GAS TYPE / FLOW RATE • PREHEAT APPLIED • INTERPASS TEMPERATURE MAINTAINED (MIN. / MAX.) • PROPER POSITION (F, V, H, OH)	X	0	0	AISC 360 N5.4-2
WELDING TECHNIQUES INTERPASS AND FINAL CLEANING EACH PASS WITHIN PROFILE LIMITATIONS EACH PASS MEETS QUALITY REQUIREMENTS	Х	0	0	
PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS	Х	Р	Р	

PLACEMENT AND INSTALLATION OF STEEL READED STUD ANCHORS	^	Р	Р	
INSPECTION TASKS AFTER WELDING				
WELDS CLEANED	Х	0	0	
SIZE, LENGTH AND LOCATION OF WELDS	Х	Р	Р	
WELDS MEET VISUAL ACCEPTANCE CRITERIA CRACK PROHIBITION WELD / BASE-METAL FUSION CRATER CROSS SECTION WELD PROFILES WELD SIZE UNDERCUT POROSITY	X	Р	Р	
ARC STRIKES	Х	Р	Р	AISC 360,
<i>k</i> -AREA - WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS HAS BEEN PERFORMED IN THE <i>k</i> -AREA, VISUALLY INSPECT THE WEB <i>k</i> -AREA FOR CRACKS WITHIN 3 in. OF THE WELD	Х	Р	Р	N5.4-3
VISUALLY INSPECT THE WELD ACCESS HOLE FOR CRACKS AFTER WELDING ROLLED HEAVY SHAPES AND BUILT-UP HEAVY SHAPES	Х	Р	Р	-
BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	Х	Р	Р	
REPAIR ACTIVITIES	Х	Р	Р	
DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	Х	Р	Р	
NO PROHIBITED WELDS HAVE BEEN ADDED WITHOUT THE APPROVAL OF THE	V	0	0	

STRUCTURAL ENGINEER	X	0	0	
NONDESTRUCTIVE TESTING OF WELDED JOINTS				
STRUCTURES IN RISK CATEGORY III OR IV, UT ALL COMPLETE-JOINT- PENETRATION (CJP) WELDS IN MATERIALS 5/16" OR THICKER.	Х	Р	Р	AISC 360,
STRUCTURES IN RISK CATEGORY II, UT ALL COMPLETE-JOINT-PENETRATION (CJP) WELDS IN MATERIALS 5/16" OR THICKER.	Х	Р	Р	N5.5

STRUCTURES IN RISK CATEGORY II, UT ALL COMPLETE-JOINT-PENETRATION (CJP) WELDS IN MATERIALS 5/16" OR THICKER.	Χ	Р	Р	C.G/II
INSPECTION TASKS PRIOR TO BOLTING				
MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS	Χ	0	Р	
FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	Χ	0	0	
CORRECT FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE)	Х	0	0	
CORRECT BOLTING PROCEDURE SELECTED FOR JOINT DETAIL	Χ	0	0	
CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	Х	0	0	AISC 360, N5.6
PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	X	Р	0	
PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS	Х	0	0	
INSPECTION TASKS DURING BOLTING				

PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS	Х	0	0	
INSPECTION TASKS DURING BOLTING				
FASTENER ASSEMBLIES PLACED IN ALL HOLES AND WASHERS AND NUTS ARE POSITIONED AS REQUIRED	Х	0	0	
JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION	Х	0	0	AISC 260
FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING	Х	0	0	AISC 360, N5.6
FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES	Х	0	0	
			•	<u> </u>

SCALE: 1/2"=1'-0"

SCALE: 1/4"=1'-0"

INSPECTION TASKS AFTER BOLTING

SCALE: 3/4"=1'-0"

DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS

STRUCTURAL NOTES CONT.

J. ELEVATORS

1. THE BASIS OF DESIGN FOR THE ELEVATOR SUPPORTS AND SHAFT IS OTIS LVM 4500 HOLED HYDRAULIC 4,500 LB CAPACITY ELEVATOR.

SELECTION DIFFERS FROM THE BASIS OF DESIGN.

2. THE CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER IF FINAL ELEVATOR

3. THE STRUCTURE HAS BEEN DESIGNED TO SUPPORT THE FOLLOWING ELEVATOR REACTIONS. CONTRACTOR SHALL NOTIFY STRUCTURAL ENGINEER FOR ANY DEVIATIONS FROM THE LOADS

> HOIST BEAM 7,500 LB SEE PLAN ELEVATOR MACHINE SUPPORT PIT FLOOR BUFFER REACTIONS XX,XXX LB PIT FLOOR RAIL REACTIONS XX,XXX LB REACTION AT RAIL SUPPORTS XXX I B

4. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS DEPICTING SUPPORT REACTIONS, RAIL LOADS, MAXIMUM GUIDE RAIL SPANS, PIT DEPTH, REQUIRED SHAFT DIMENSIONS AND CLEARANCES, AND OVERHEAD CLEARANCES.

5. ELEVATOR GUIDE RAIL SUPPORTS

5.1 ELEVATOR GUIDE RAIL SUPPORT TUBES HAVE NOT BEEN INCLUDED IN THE CONTRACT DOCUMENTS FOR FLOOR HEIGHTS OF 11'-00" OR LESS. THE ELEVATOR RAILS SUPPLIED BY THE ELEVATOR SUPPLIER SHALL SPAN 11'-00" WITHOUT ADDITIONAL SUPPORT. IF THE RAILS PROVIDED CANNOT SPAN THE REQUIRED DISTANCE BETWEEN FLOORS, THE CONTRACTOR SHALL SUPPLY ADEQUATE SUPPORT IN ACCORDANCE WITH ANSI/ASME A17.1 DESIGNED BY AN ENGINEER LICENSED IN THE PROJECT STATE. SUBMIT SHOP DRAWINGS CLEARLY INDICATING THE GUIDE RAIL SUPPORTS, THE DESIGN CRITERIA USED, AND THE ATTACHMENT TO THE STRUCTURE.

5.2 ELEVATOR GUIDE RAIL SUPPORT TUBES HAVE BEEN INCLUDED IN THE CONTRACT DOCUMENTS FOR FLOOR HEIGHTS OF XX'-XX" OR GREATER. IF THE ELEVATOR RAILS PROVIDED CAN SPAN THE REQUIRED DISTANCE WITHOUT ADDITIONAL SUPPORT, THE ELEVATOR RAIL SUPPORT TUBES MAY BE OMITTED.

K. POST-INSTALLED ANCHORS AND REINFORCING STEEL

1. POST-INSTALLED ANCHORS AND REINFORCING STEEL SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER PRIOR TO INSTALLING POST-INSTALLED ANCHORS OR REINFORCING STEEL IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS OR REINFORCING STEEL.

2. ANCHORS AND REINFORCING STEEL SHALL BE INSTALLED PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII)

3. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE SPECIFIED BELOW SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER ALONG WITH CALCULATIONS THAT ARE SIGNED AND SEALED BY A LICENSED PROFESSIONAL ENGINEER. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING EQUIVALENT PERFORMANCE VALUES (MINIMUM) OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S) AS REQUIRED BY THE BUILDING CODE.

4. THE CONTRACTOR SHALL ARRANGE ONSITE INSTALLATION TRAINING BY THE MANUFACTURER FOR EACH PRODUCT TO BE INSTALLED. SUBMIT TO THE STRUCTURAL ENGINEER DOCUMENTATION CONFIRMING TRAINING OF ALL PERSONNEL WHO WILL BE INSTALLING PRODUCTS. TRAINING AND DOCUMENTATION SHALL OCCUR PRIOR TO COMMENCEMENT OF PRODUCT INSTALLATION. INSTALLATION OF ADHESIVE ANCHOR PRODUCTS IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATION RESISTING SUSTAINED TENSION LOADS SHALL BE CONDUCTED BY AN INSTALLER CERTIFIED IN ACCORDANCE WITH THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM. PROOF OF CERTIFICATION SHALL BE MAINTAINED AT THE JOB SITE.

5. ANCHOR CAPACITY IS DEPENDENT UPON SPACING BETWEEN ADJACENT ANCHORS, CONCRETE STRENGTH AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS. IF NO SPACING OR EDGE DISTANCES ARE SPECIFIED ON THE STRUCTURAL DRAWINGS, REFER TO APPLICABLE EVALUATION REPORT FOR CRITICAL SPACING AND EDGE DISTANCES.

6. EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR OR REINFORCING LOCATIONS. THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TO LOCATE THE POSITION OF THE REINFORCING BARS AT THE LOCATIONS OF THE CONCRETE ANCHORS OR REINFORCING, BY FERROSCAN, GPR, X-RAY, CHIPPING OR OTHER MEANS IN ORDER TO AVOID CONFLICT WITH INSTALLATION. THE CONTRACTOR SHALL NOT DAMAGE ANY REINFORCING STEEL PRIOR TO CONSULTING WITH THE STRUCTURAL ENGINEER.

7. UNLESS NOTED OTHERWISE, THE MINIMUM EMBEDMENT OF EXPANSION AND SCREW ANCHORS SHALL BE 6 TIMES THE ANCHOR DIAMETER, THE MINIMUM EMBEDMENT FOR ADHESIVE ANCHORS SHALL BE 12 TIMES THE ANCHOR DIAMETER, AND THE MINIMUM EMBEDMENT FOR ADHESIVE REINFORCING SHALL BE 12 BAR DIAMETERS.

8. ADHESIVE ANCHOR INSERT SHALL BE:

INTERIOR NON-CORROSIVE ENVIRONMENTS: ASTM F1554 Gr. 36 ALL-THREAD ROD EXTERIOR NON-CORROSIVE ENVIRONMENTS: ASTM F1554 Gr. 36 ALL-THREAD ROD HOT-DIPPED GALVANIZED PER ASTM B695, CLASS 65, TYPE I CORROSIVE ENVIRONMENTS: ASTM A193 GR. B8M TYPE 316 ALL-THREAD ROD

9. MECHANICAL AND SCREW ANCHORS IN EXTERIOR NON-CORROSIVE ENVIRONMENTS SHALL BE HOT-DIPPED GALVANIZED PER ASTM B695, CLASS 65, TYPE I

10. ADHESIVE ANCHORS AND REINFORCING STEEL CAPACITY IS DEPENDENT UPON INSTALLATION CONDITIONS. THE FOLLOWING INSTALLATION CONDITIONS HAVE BEEN ASSUMED:

> HOLES DRILLED WITH HAMMER DRILL WITH CARBIDE TIPPED DRILL BIT CONCRETE CURED FOR A MINIMUM OF 21 DAYS

IF THESE CONDITIONS DO NOT EXIST, THE CONTRACTOR SHALL SUBMIT ALTERNATE

SCALE: 1/8"=1'-0"

SCALE: 3/32"=1'-0"

TEMPERATURE CATEGORY B (110 DEG. F LONG TERM AND 130 DEG. F SHORT TERM)

DRILLING METHODS AND MANUFACTURER BOND DATA FOR THE CONDITIONS BEING INSTALLED.

K. POST-INSTALLED ANCHORS AND REINFORCING STEEL (CONT.)

11. POST-INSTALLED ANCHORS AND REINFORCING INSTALLED INTO CONCRETE SHALL BE AS

11.1 MECHANICAL AND SCREW ANCHORS FOR USE IN CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN CRACKED CONCRETE IN ACCORDANCE WITH ACI 355.2

> 11.1.1 EXPANSION ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.: SIMPSON STRONG-TIE STRONG-BOLT 2 (ICC ESR-3037) HILTI KWIK BOLT-TZ2 (ICC ESR-4266)

> > DEWALT SCREW-BOLT+ (ICC ESR-3889)

DEWALT POWER-STUD+ SD2 (ICC ESR-2502) 11.1.2 SCREW ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.: SIMPSON STRONG-TIE TITEN-HD (ICC ESR-2713) HILTI KWIK HUS-EZ (ICC ESR-3027)

11.1.3 SHALLOW EMBEDMENT ANCHORS (<3/4") SHALL BE ONE OF THE FOLLOWING, HILTI HDI-P-TZ (ICC ESR-4236)

DEWALT MINI-UNDERCUT+ (ICC ESR-3912) 11.2 ADHESIVE ANCHORS FOR USE IN CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED

FOR USE IN CRACKED CONCRETE IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308 AND THE ICC-ES EVALUATION SERVICES REPORT SHALL SHOW COMPLIANCE WITH THE BUILDING CODE STATED IN SECTION B OF THESE STRUCTURAL NOTES. 11.2.1 ADHESIVE ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.:

SIMPSON STRONG-TIE SET-XP (ICC ESR-2508) HILTI HIT-RE 500 v3 SLOW CURE (ICC ESR-3814) HILTI HIT-HY 200 FAST CURE (ICC ESR-3187) DEWALT PURE110+ (ICC ESR-3298)

11.3 ADHESIVE FOR INSTALLING REINFORCING STEEL IN EXISTING CONCRETE SHALL BE QUALIFIED IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308 AND THE ICC-ES EVALUATION SERVICES REPORT SHALL SHOW COMPLIANCE WITH THE BUILDING CODE STATED IN SECTION B OF THESE STRUCTURAL NOTES.

11.3.1 ADHESIVE SHALL BE ONE OF THE FOLLOWING, U.N.O.: SIMPSON STRONG-TIE "SET-XP" (ICC-ES ESR-2508) HILTI HIT-RE 500 v3 SLOW CURE (ICC ESR-3814) DEWALT PURE110+ (ICC ESR-3298)

12. POST-INSTALLED ANCHORS INSTALLED INTO SOLID GROUTED CONCRETE MASONRY CELLS SHALL BE AS INDICATED BELOW. DO NOT INSTALL ANCHORS INTO HOLLOW HEAD JOINTS OR TEE JOINTS BETWEEN MASONRY BLOCKS.

12.1 MECHANICAL AND SCREW ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES ACO1 AND AC106, RESPECTIVELY.

12.1.1 EXPANSION ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.: SIMPSON STRONG-TIE STRONG-BOLT2 (IAPMO-UES ER-240) HILTI KWIK BOLT-TZ2 (ICC ESR-4561) DEWALT POWER-STUD+ SD1 (ICC ESR-2966)

12.1.2 SCREW ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.: SIMPSON STRONG-TIE TITEN-HD (ICC ESR-1056) HILTI KWIK HUS-EZ (ICC ESR-3056) DEWALT SCREW-BOLT+ (ICC-ES ESR-4042)

12.2 ADHESIVE ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC58

12.2.1 ADHESIVE ANCHORS SHALL BE ONE OF THE FOLLOWING. U.N.O.: SIMPSON STRONG-TIE "SET-XP" (IAPMO-UES ER-265) HILTI HIT-HY 270 (ICC ESR-4143) DEWALT AC100+ GOLD (ICC ESR-3200)

13. POST-INSTALLED ANCHORS INSTALLED INTO HOLLOW CONCRETE MASONRY SHALL BE AS

13.1 SCREW ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC106.

13.1.1 SCREW ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.: DEWALT TAPPER + (ICC ESR 3196) SIMPSON STRONG-TIE TITEN-HD (ICC ESR-1056)

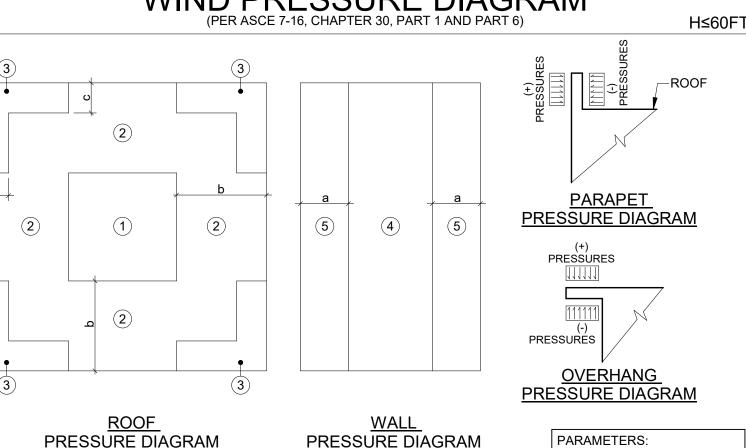
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BE USED AS RECOMMENDED BY THE ADHESIVE MANUFACTURER

HILTI KWIK-CON II (ICC ESR-NOT AVAILABLE) 13.2 ADHESIVE ANCHORS WITH SCREEN TUBES SHALL BE TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC58 OR AC60, AS APPROPRIATE. SCREEN TUBES SHALL

13.2.1 ADHESIVE ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.: SIMPSON STRONG-TIE "SET-XP" (IAPMO-UES ER-265) HILTI HIT-HY 270 (ICC ESR-4143) DEWALT AC100+ GOLD (ICC ESR-3200)





FECTIVE			PRI	ESSURE	(PSF) -	SEE NO	TES 1 - 4				
AREA t²) - SEE NOTE 2	ROOF ZONE				WALLS		0	VERHANG	S	PARAPETS	
	ZONE 1'	ZONE 1	ZONE 2	ZONE 3	ZONE 4	ZONE 5	ZONE 1'/1	ZONE 2	ZONE 3	ZONE 4	ZONE 5
<u><</u> 10	+30, -68	+30, -118	+30, -156	+30, -213	+74, -81	+74, -99	+16, -107	+16, -145	+16, -201	+211, -134	+268, -153
20	+28, -68	+28, -110	+28, -146	+28, -193	+71, -77	+71, -93	+16, -105	+16, -131	+16, -178	+197, -127	+244, -143
50	+26, -68	+26, -100	+26, -133	+26, -166	+66, -73	+66, -84	+16, -103	+16, -114	+16, -147	+179, -118	+213, -130
100	+24, -68	+24, -92	+24, -123	+24, -146	+63, -69	+63, -77	+16, -101	+16, -100	+16, -124	+166, -112	+189, -119
200	+24, -58	+24, -85	+24, -113	+24, -126	+60, -66	+60, -70	+16, -84	+16, -87	+16, -100	+152, -105	+165, -109

> 500 | +24, -46 | +24, -74 | +24, -99 | +24, -99 | +55, -62 | +55, -62 | +16, -63 | +16, -69 | +16, -69 | +134, -96 | +134, -96 (+) AND (-) SIGNS INDICATED PRESSURES ACTING TOWARDS AND AWAY FROM THE SURFACE, RESPECTIVELY. SEE THE PARAPET AND OVERHANG DIAGRAMS FOR PRESSURE DIRECTION ON THOSE ELEMENTS. USE THE PRESSURE ASSOCIATED WITH THE CLOSEST EFFECTIVE AREA THAT DOES NOT EXCEED THE ACTUAL EFFECTIVE AREA, ALTERNATIVELY, LINEAR INTERPOLATION IS PERMITTED. IF A PARAPET ≥ 3'-0" IN HEIGHT IS PROVIDED AROUND THE PERIMETER OF A FLAT ROOF:

THE POSITIVE VALUES FOR ZONE 3 SHALL BE EQUAL TO THOSE FOR WALL ZONE 5. THE POSITIVE VALUES FOR ZONE 2 SHALL BE EQUAL TO THOSE FOR WALL ZONE 4. THE WIND PRESSURES USED TO DESIGN ANY DEFERRED SUBMITTAL ELEMENT SHALL BE CALCULATED BY THE REGISTERED PROFESSIONAL ENGINEER DESIGNING THOSE ELEMENTS. NOTIFY THE STRUCTURAL ENGINEER OF

> CONSTRUCTION INFO GENERAL CONTRACTOR PRECAST CONCRETE **BRASFIELD & GORRIE** METROMONT WALKER COX JIM BATCHER HCA CONSTRUCTION MANAGER: HCA DESIGN MANAGER: RYAN ROHE **GREG ATKINSON**

> > SCALE: 1/32"=1'-0"

BASIC WIND SPEED: 161 MPH

RISK CATEGORY II

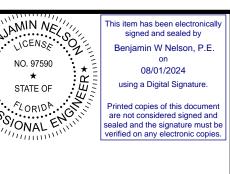
b = 33'-0" c = 11'-0"

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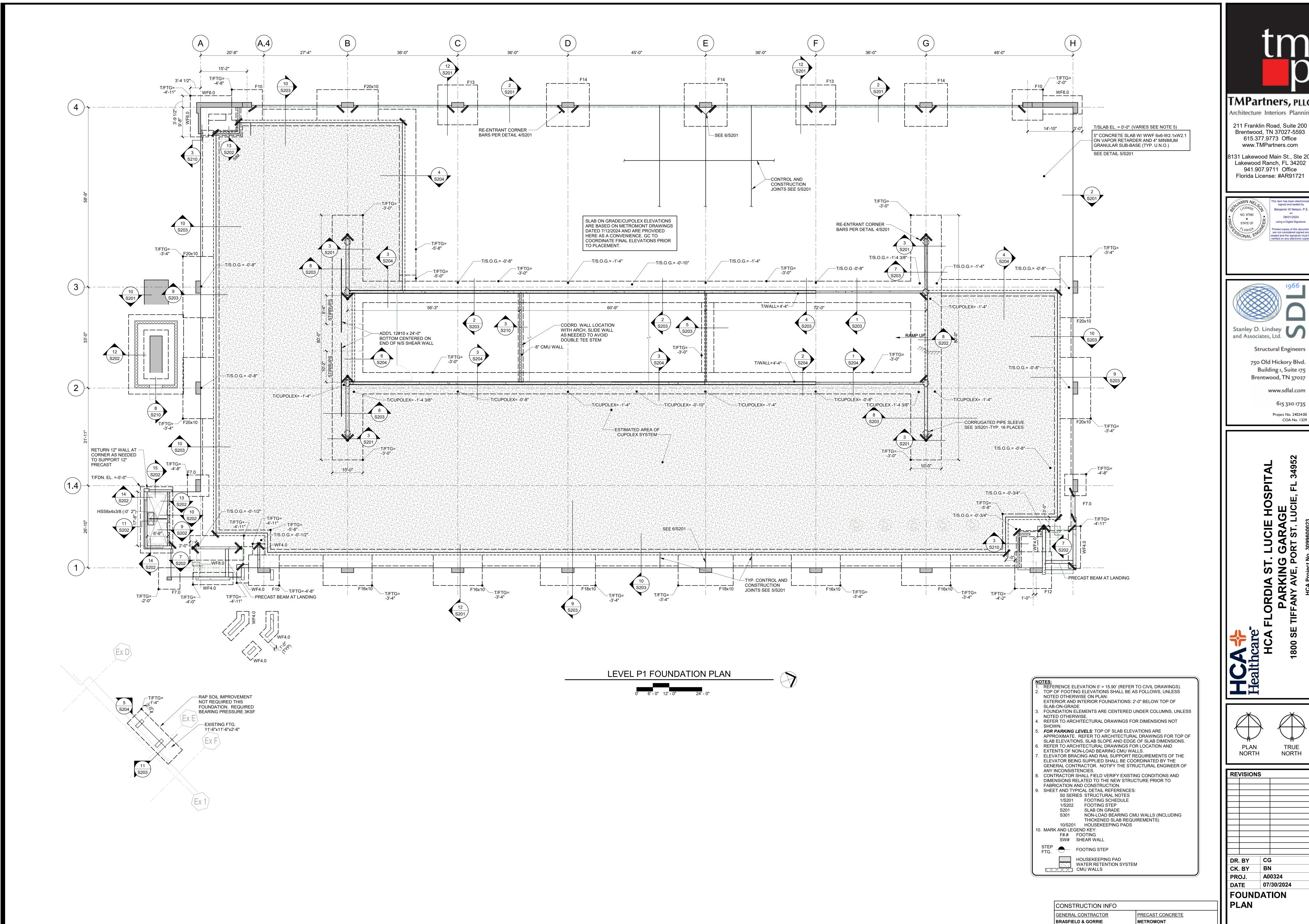
Brentwood, TN 37027

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COA No. 1329

REVISIONS DR. BY CG

CK. BY PROJ. DATE 07/30/2024 STRUCTURAL NOTES CONT. ANI WIND PRESSURES



0" 2' 4'

SCALE: 1/4"=1'-0"

0" 4' 8'

SCALE: 3/32"=1'-0"

SCALE: 1/8"=1'-0"

0" 1' 2'

SCALE: 1/2"=1'-0"

SCALE: 3/4"=1'-0"

SCALE: 1-1/2"=1'-0"

SCALE: 1"=1'-0"

SCALE: 3"=1'-0"

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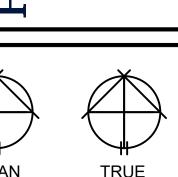
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TRUE NORTH

REVISIONS					
DR. BY		CG	;		
CK. BY		BN	I		
PROJ.		A00324			
DATE		07/30/2024			
FOUNDATION					

WALKER COX

RYAN ROHE

0" 8' 16'

SCALE: 1/16"=1'-0"

HCA DESIGN MANAGER:

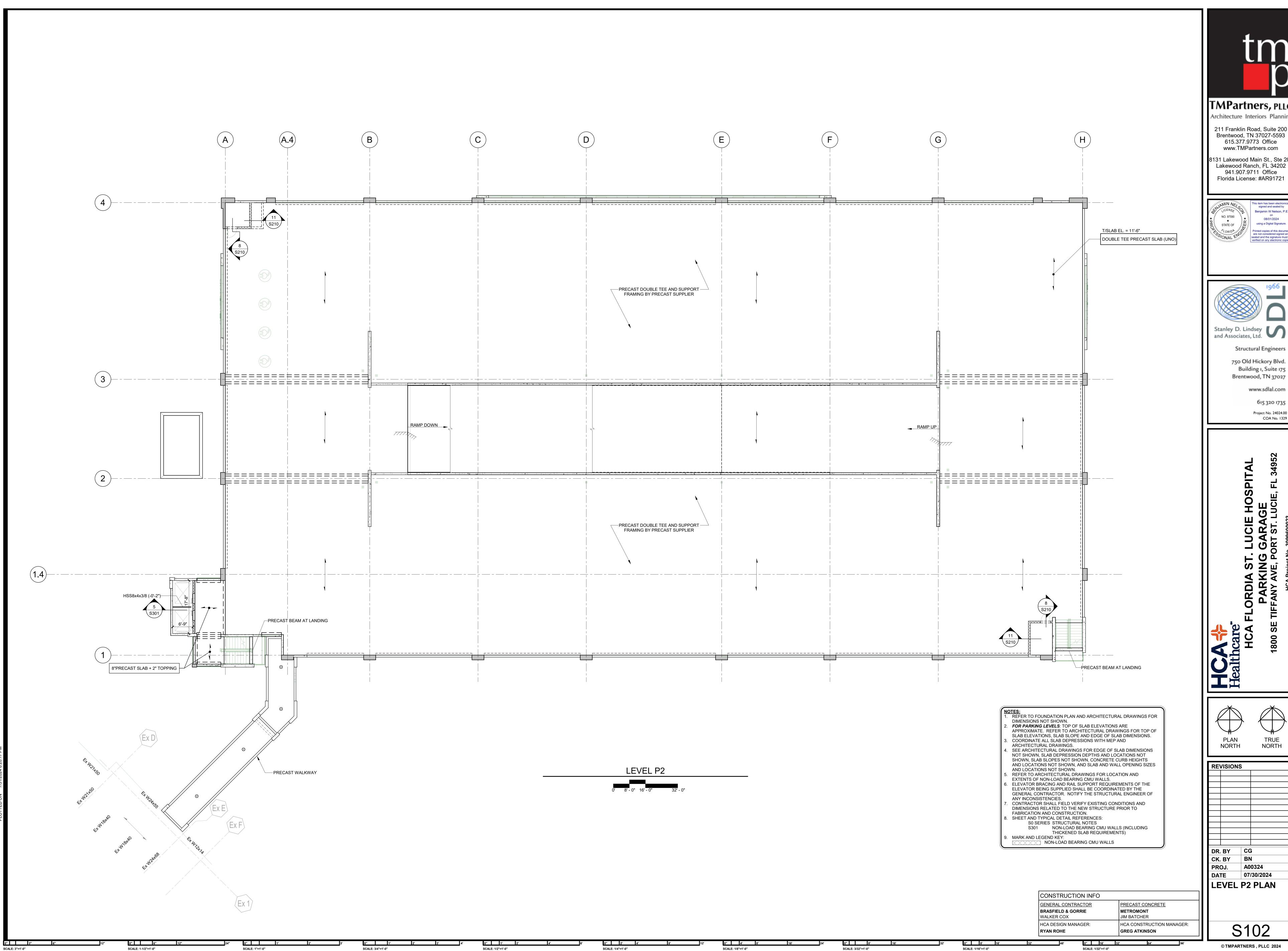
JIM BATCHER

0" 16' 32' 64'

SCALE: 1/32"=1'-0"

GREG ATKINSON

HCA CONSTRUCTION MANAGER:



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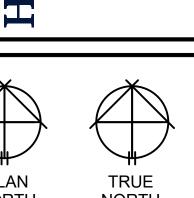
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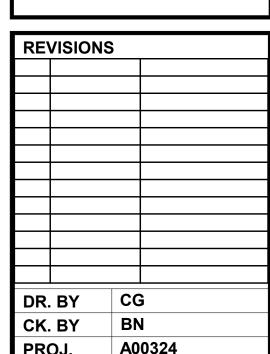


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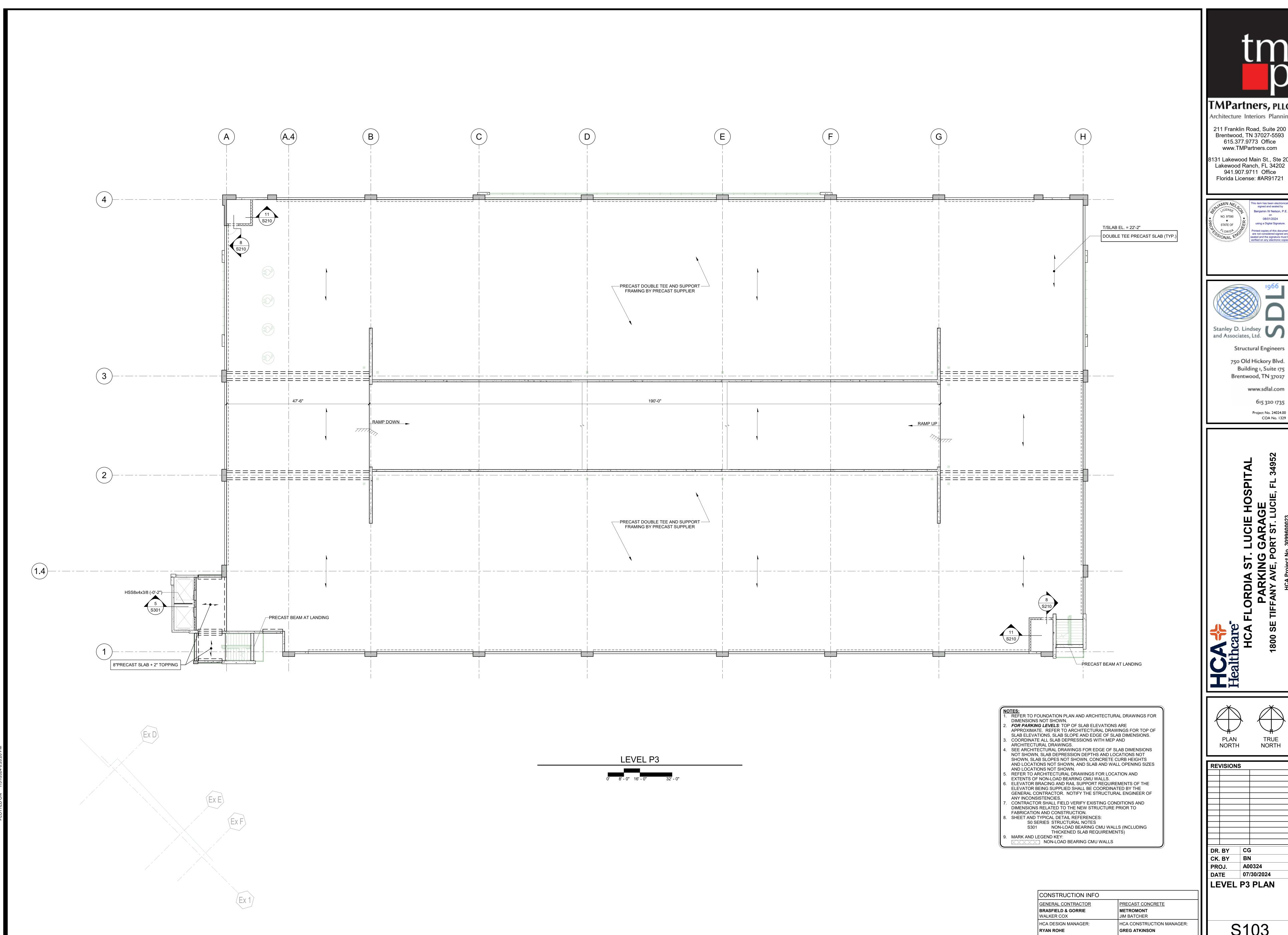
Project No. 24024.00 COA No. 1329





LEVEL P2 PLAN

07/30/2024



0" 2' 4'

SCALE: 1/4"=1'-0"

SCALE: 1-1/2"=1'-0"

SCALE: 1"=1'-0"

SCALE: 3/4"=1'-0"

SCALE: 1/2"=1'-0"

0" 4' 8'

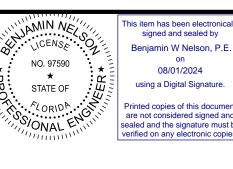
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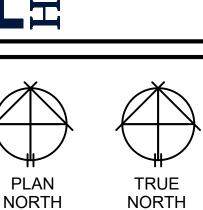


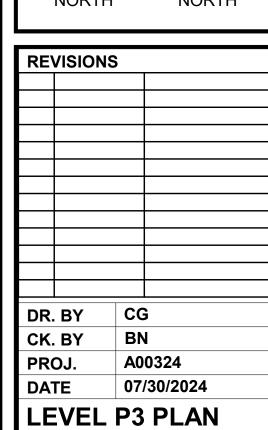


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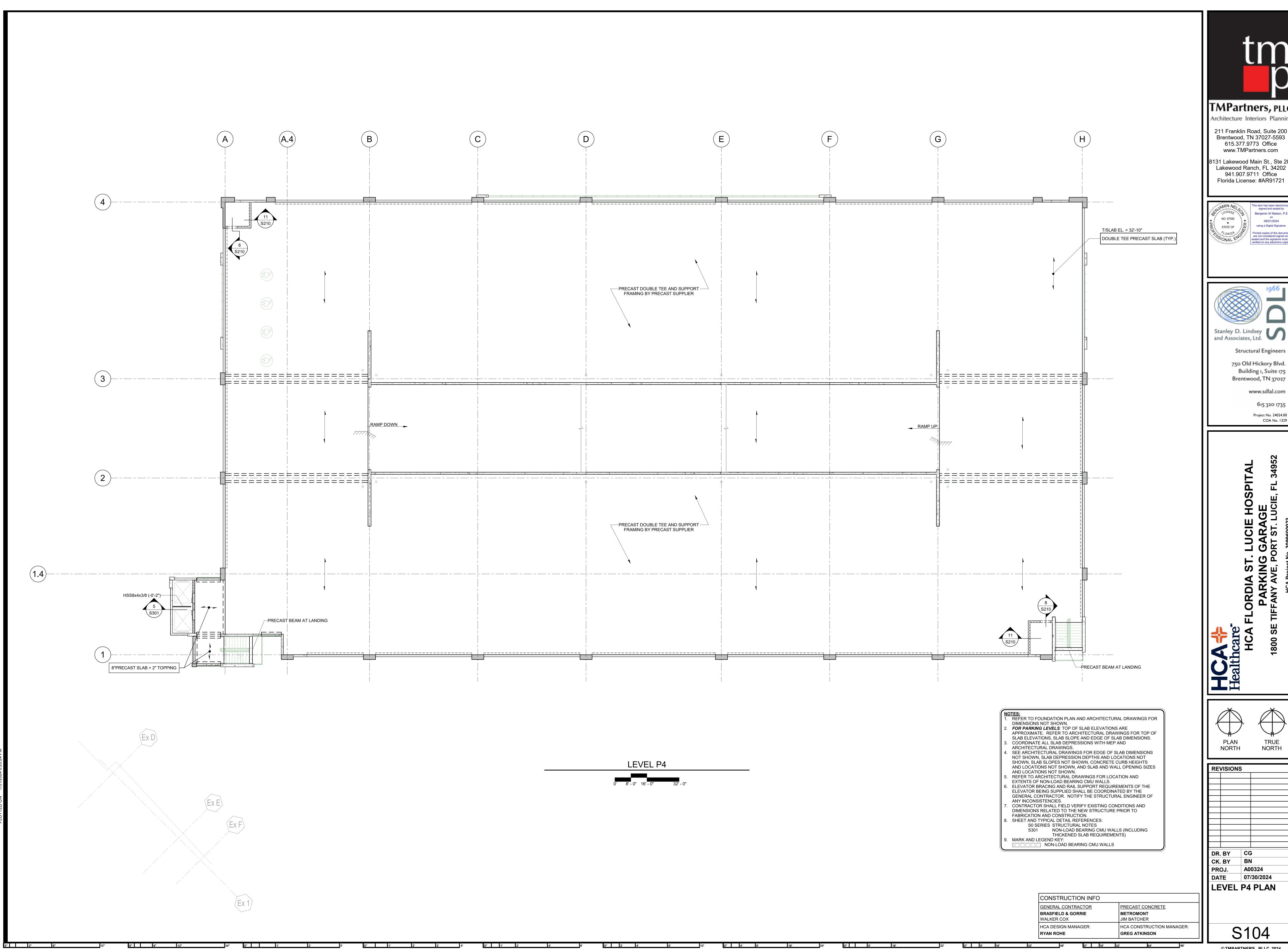


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0" 16' 32' 64'

SCALE: 1/32"=1'-0"

0" 8' 16'



SCALE: 1/8"=1'-0"

SCALE: 3/32"=1'-0"

SCALE: 1/16"=1'-0"

SCALE: 1/32"=1'-0"

SCALE: 3"=1'-0"

SCALE: 1-1/2"=1'-0"

SCALE: 1"=1'-0"

SCALE: 3/4"=1'-0"

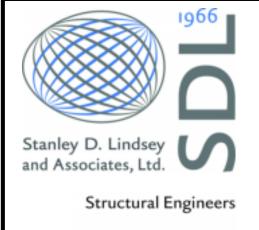
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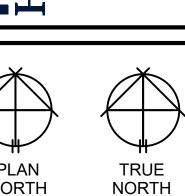
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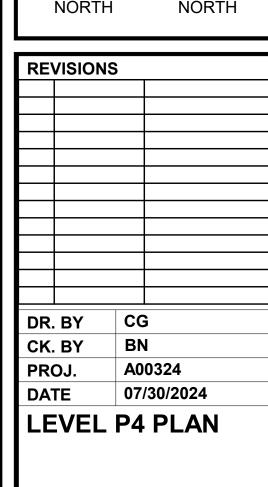


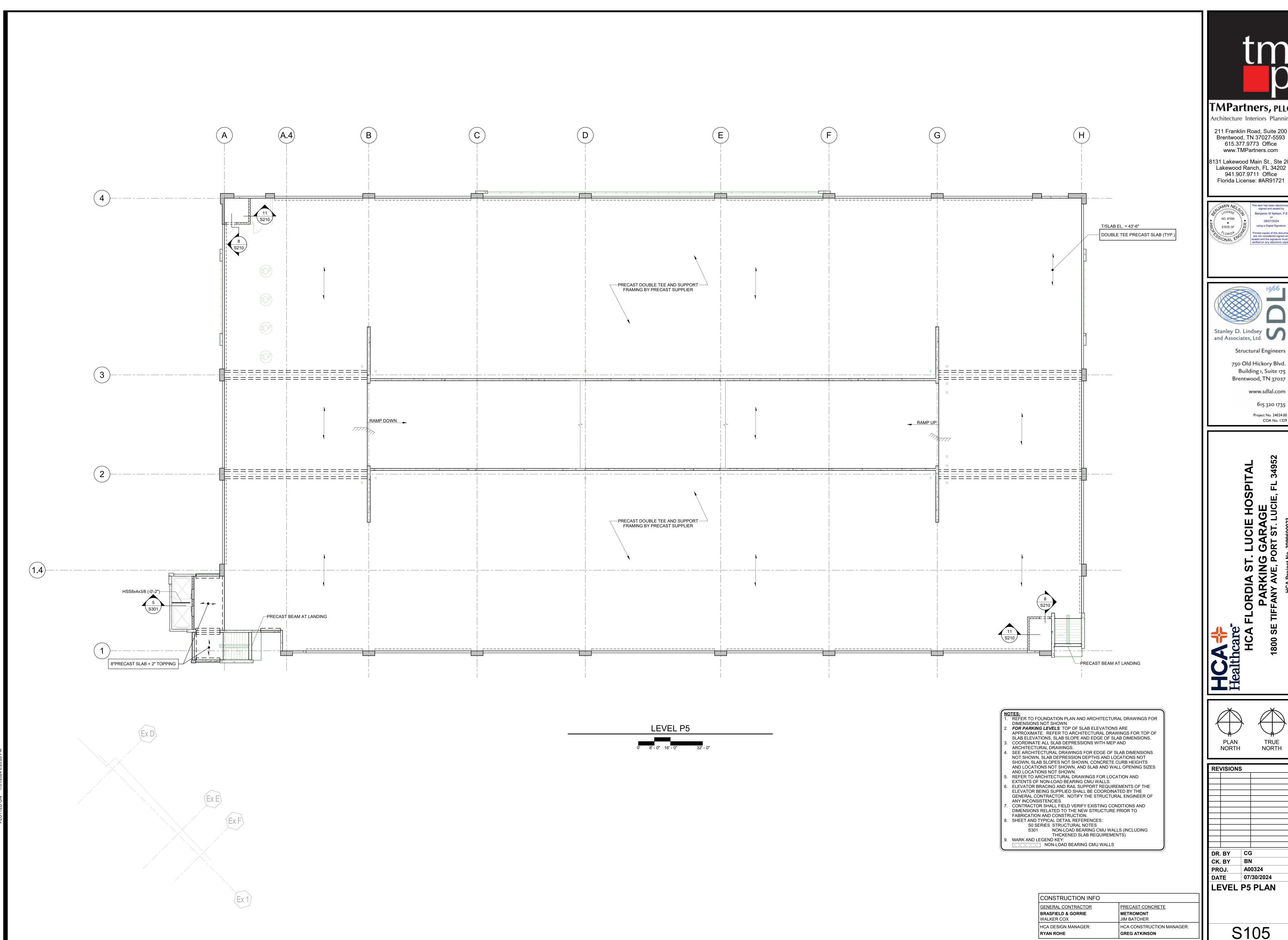


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0" 2' 4'

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SCALE: 3/4"=1'-0"

SCALE: 1/2"=1'-0"

SCALE: 3"=1'-0"

0" 4' 8'

SCALE: 1/8"=1'-0"

SCALE: 3/32"=1'-0"

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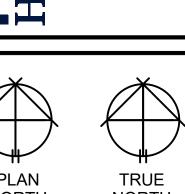
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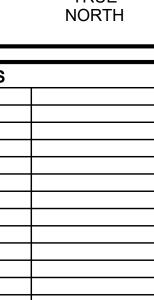


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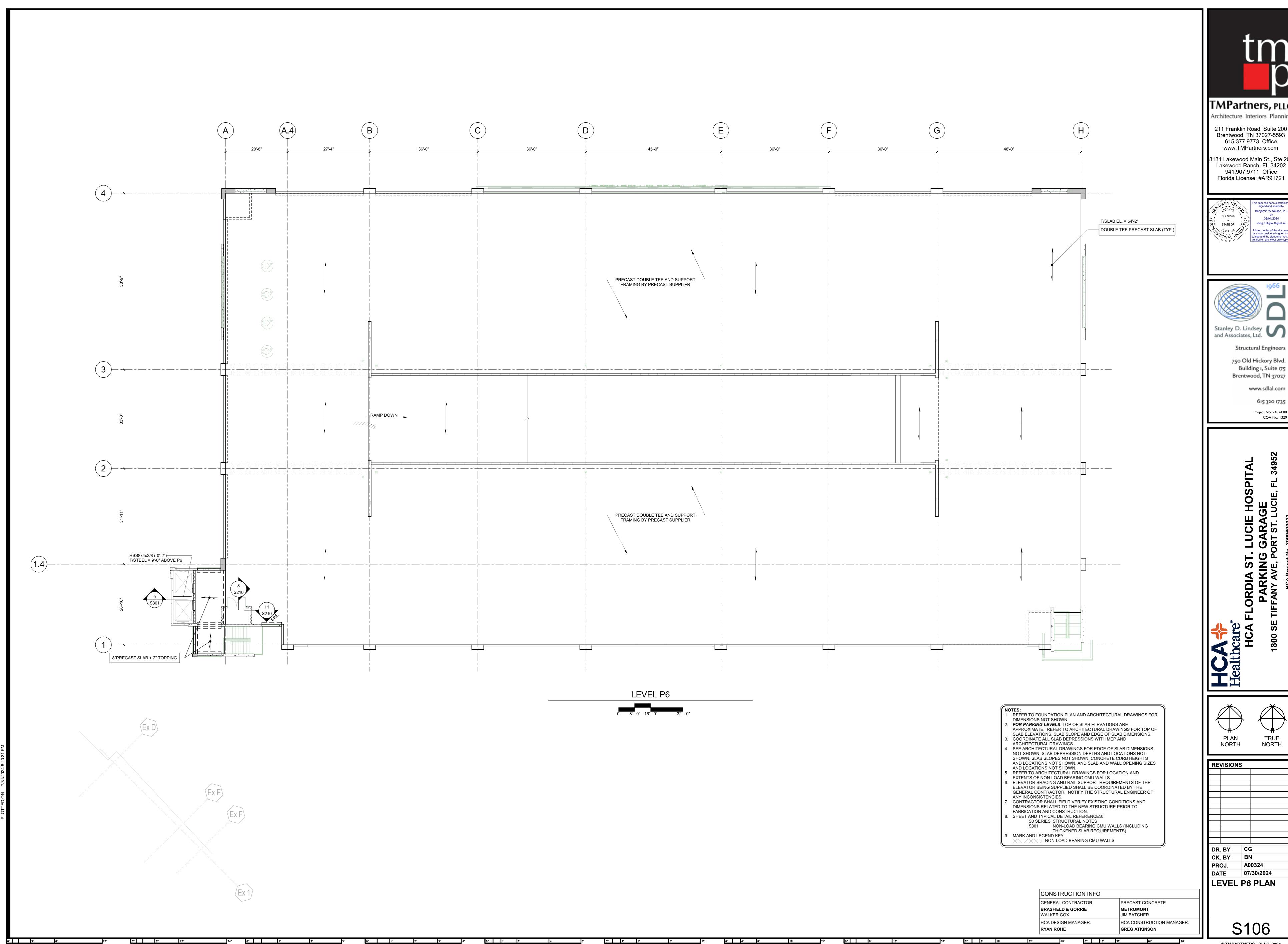
DR. BY CG CK. BY BN PROJ. 07/30/2024 DATE

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0" 16' 32' 64'

SCALE: 1/32"=1'-0"

0" 8' 16'



SCALE: 1/8"=1'-0"

SCALE: 1/16"=1'-0"

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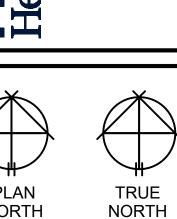




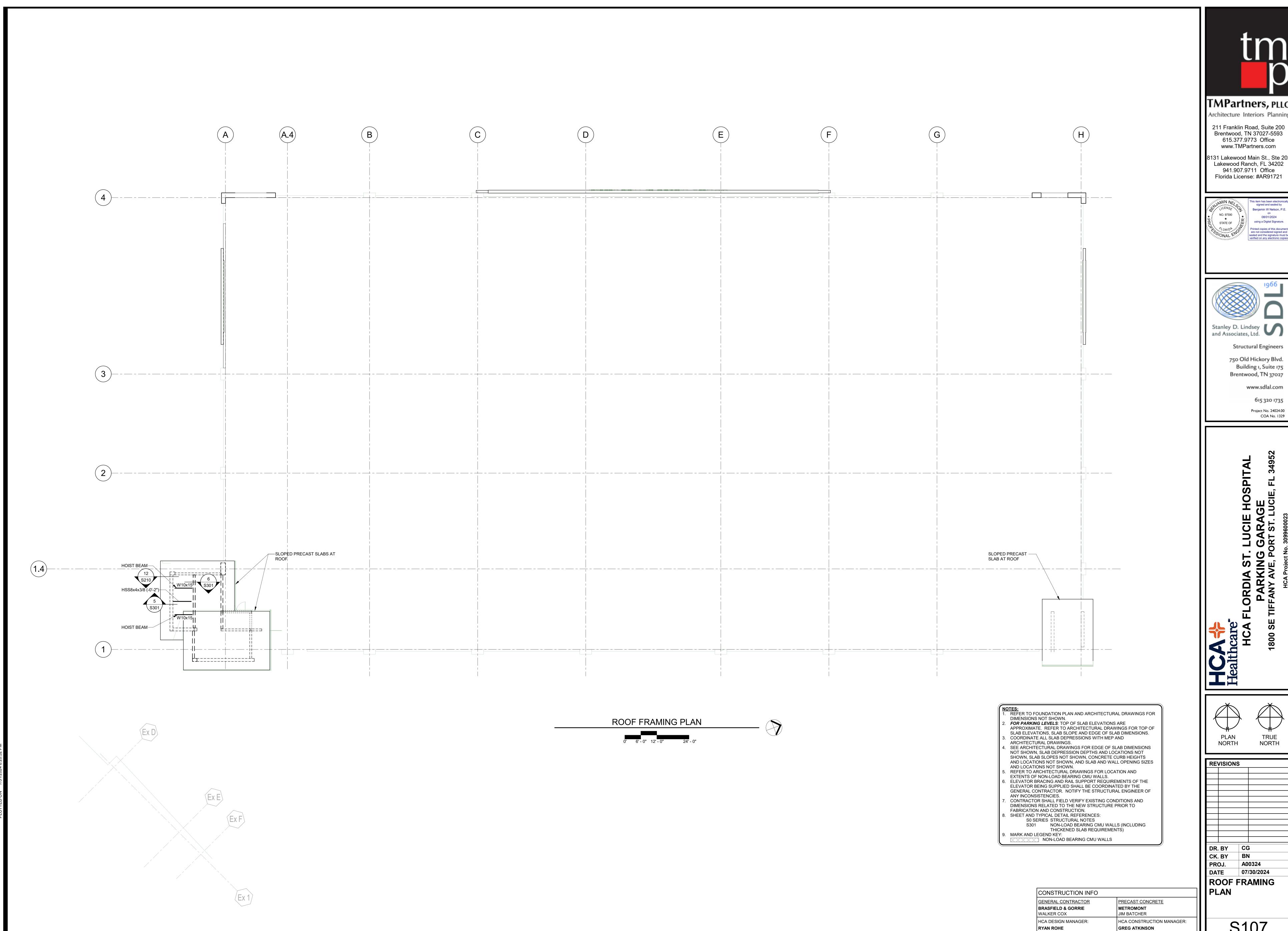
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Project No. 24024.00 COA No. 1329



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CK. BY		BN			
PROJ.		A00324			
DATE		07/30/2024			



0" 2' 4' SCALE: 1/4"=1'-0"

0" 4' 8'

SCALE: 3/32"=1'-0"

SCALE: 1/8"=1'-0"

0" 1' 2'

SCALE: 1/2"=1'-0"

0" 1' 2' 3' 4' SCALE: 3/4"=1'-0"

0" 6" 12" SCALE: 1-1/2"=1'-0"

SCALE: 1"=1'-0"

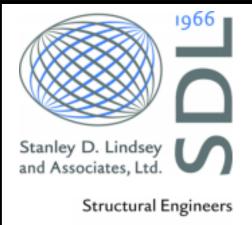
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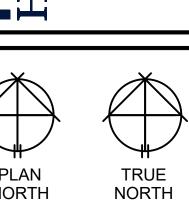




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COA No. 1329

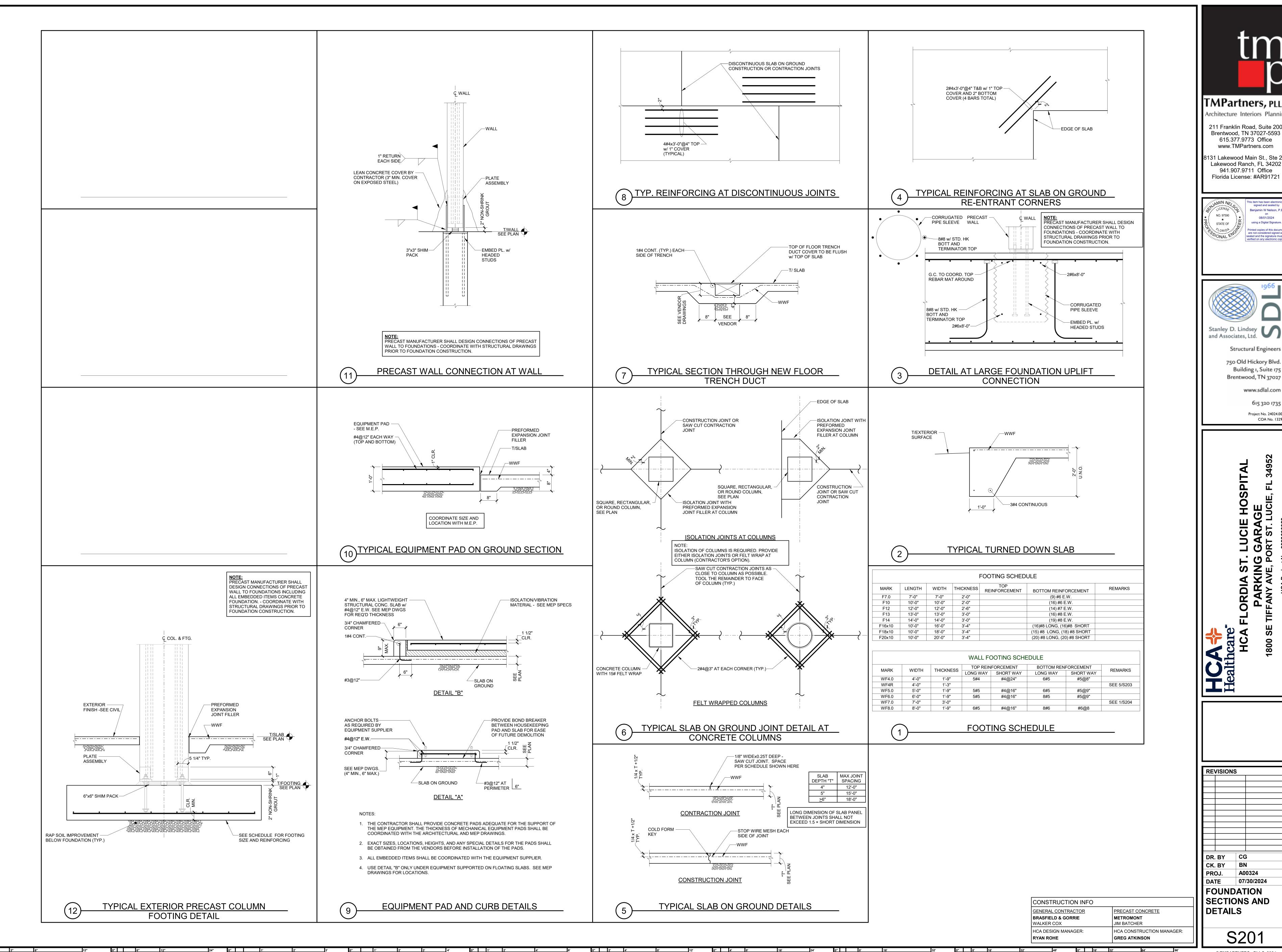


NORTH		NORTH			
REVISIONS					
DR. BY		CG			
CK. BY		BN			
PROJ.		A00324			
DATE		07/30/2024			
ROOF FRAMING					
PLAN					

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0" 16' 32' 64' SCALE: 1/32"=1'-0"

0" 8' 16'



SCALE: 1/8"=1'-0"

SCALE: 3/32"=1'-0"

SCALE: 1/16"=1'-0"

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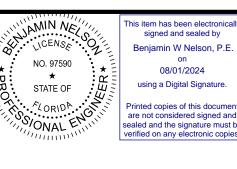
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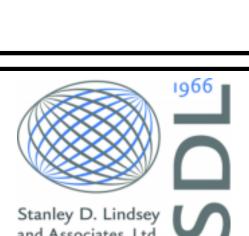
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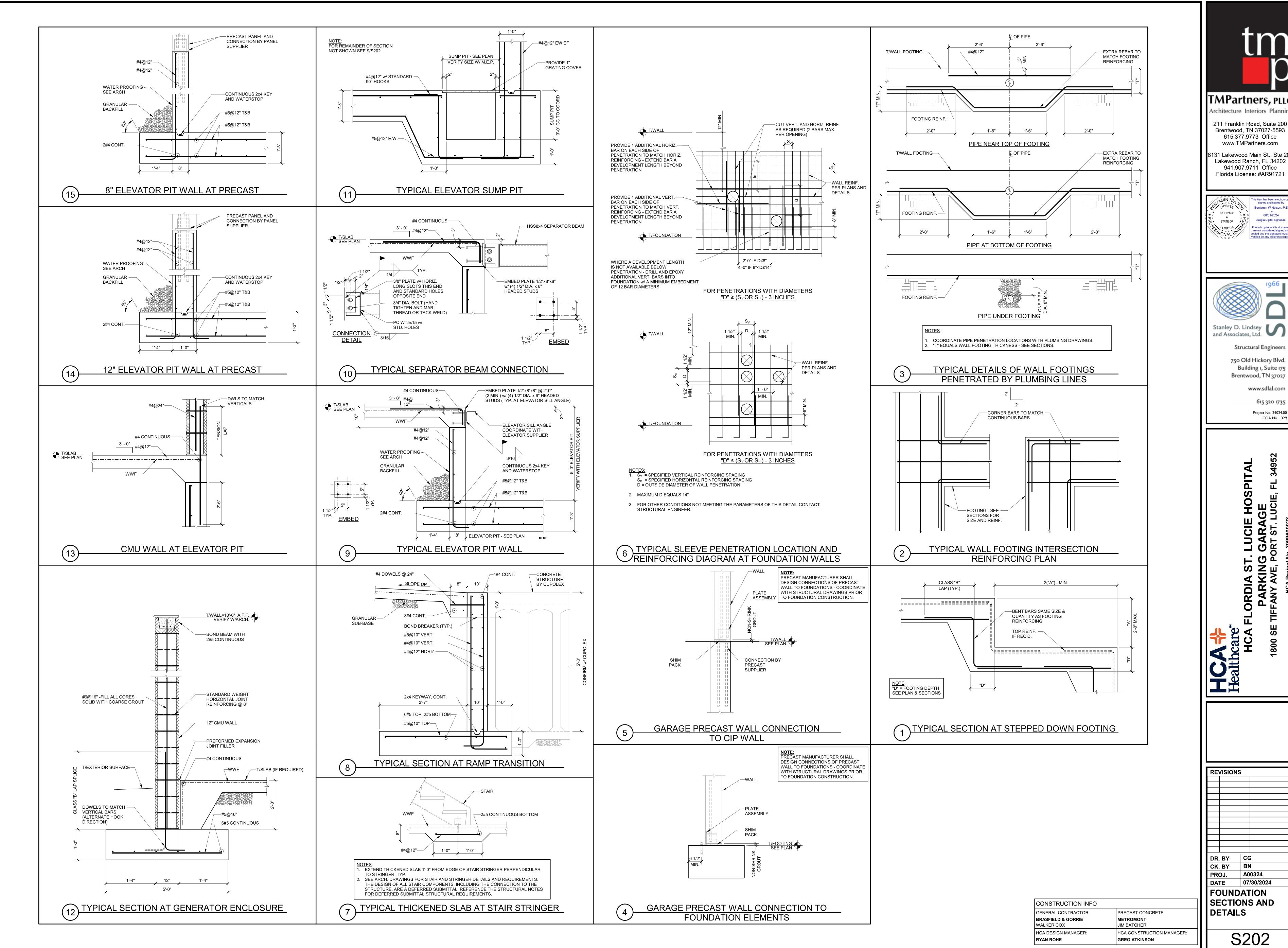
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REVISIONS DR. BY CG CK. BY A00324 07/30/2024

SECTIONS AND DETAILS



SCALE: 1/8"=1'-0"

SCALE: 3/32"=1'-0"

SCALE: 1-1/2"=1'-0"

SCALE: 1"=1'-0"

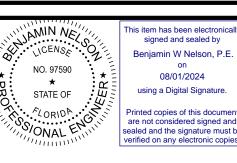
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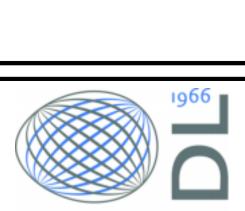
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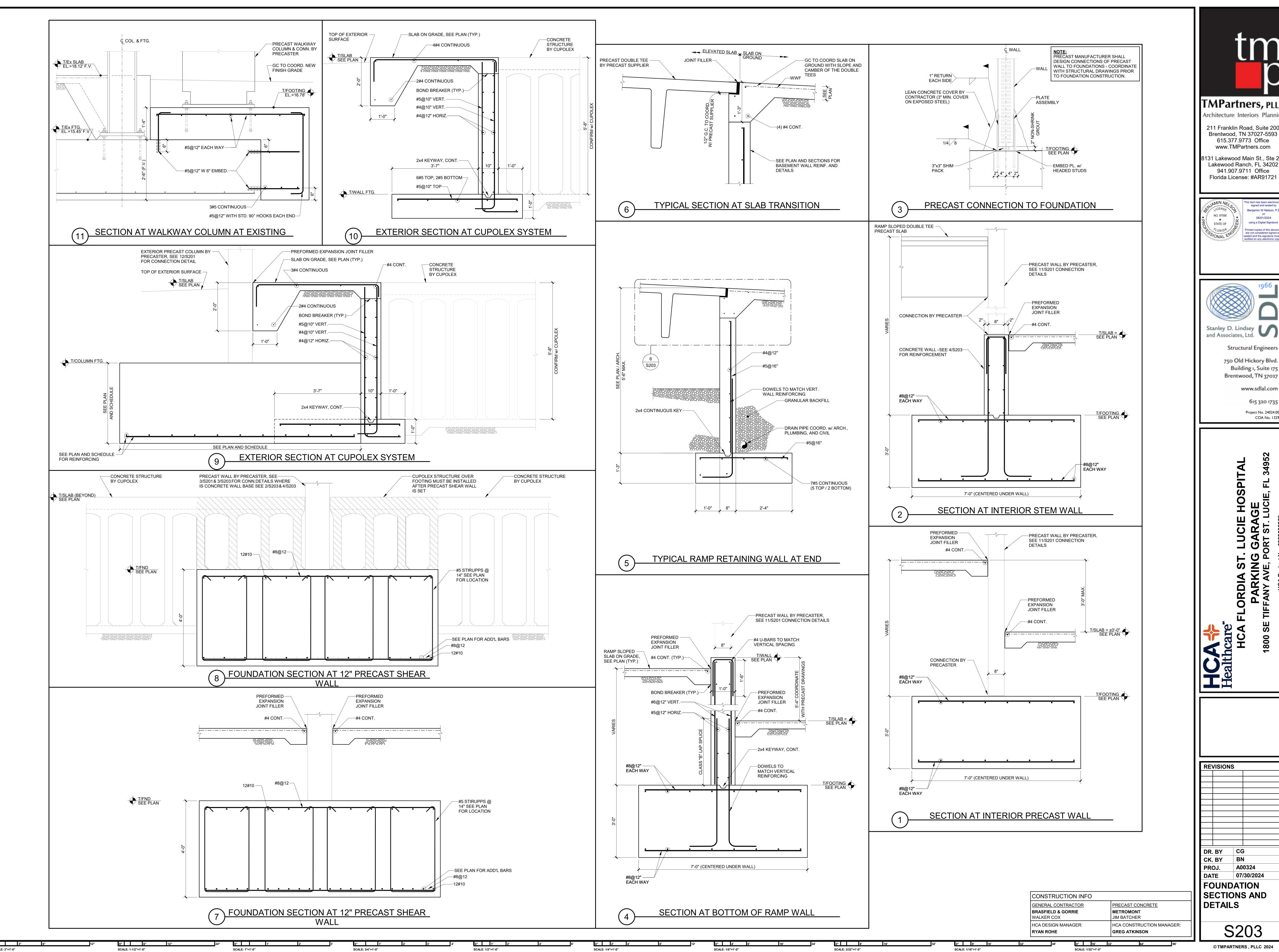
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FOUNDATION SECTIONS AND DETAILS

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0" 8' 16'

SCALE: 1/32"=1'-0"



SCALE: 1/8"=1'-0"

SCALE: 1/16"=1'-0"

SCALE: 3/32"=1'-0"

SCALE: 1/32"=1'-0"

SCALE: 1/2"=1'-0"

SCALE: 1-1/2"=1'-0"

SCALE: 1"=1'-0"

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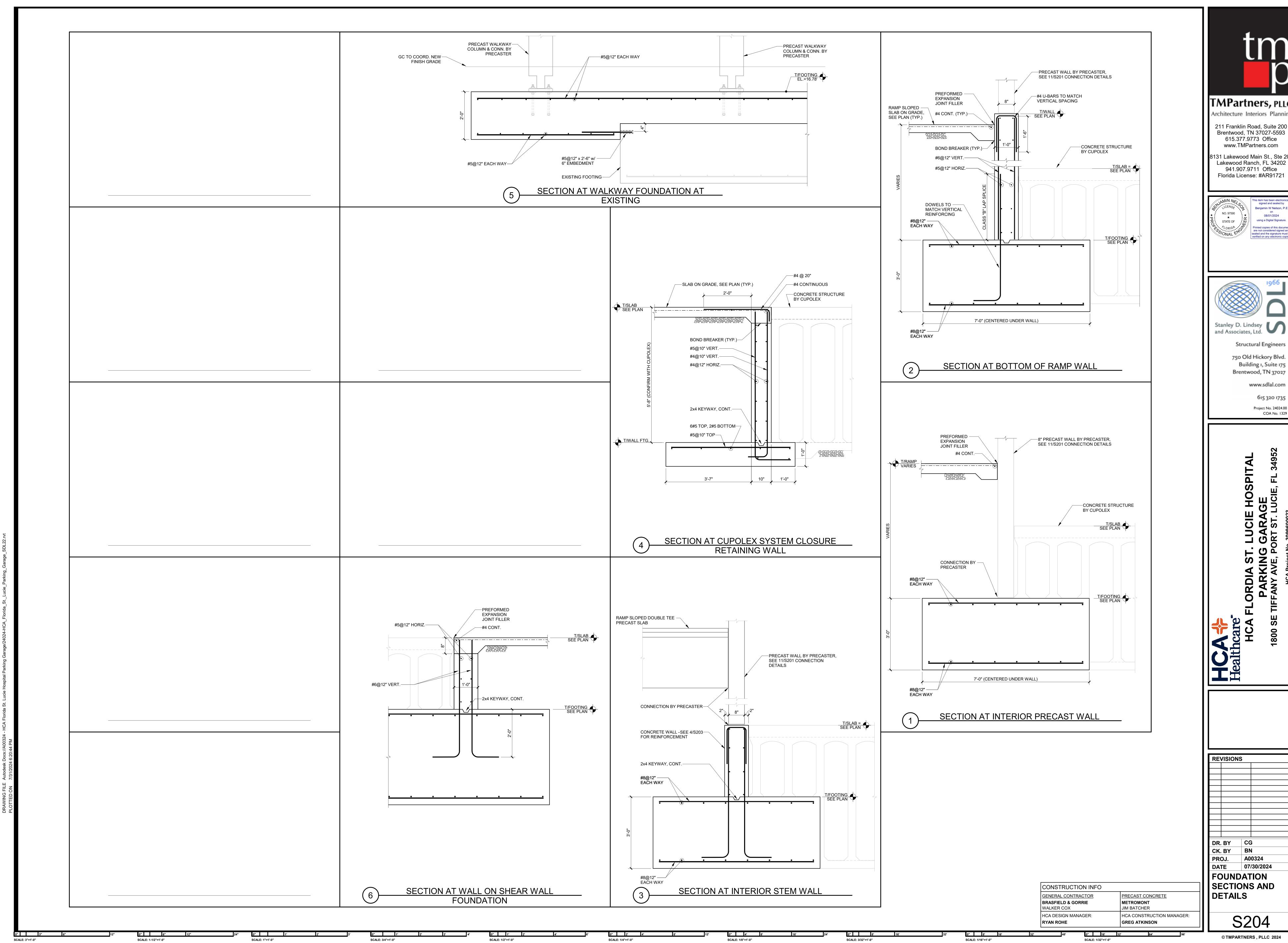
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COA No. 1329

07/30/2024

FOUNDATION SECTIONS AND

S203

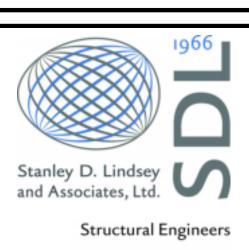


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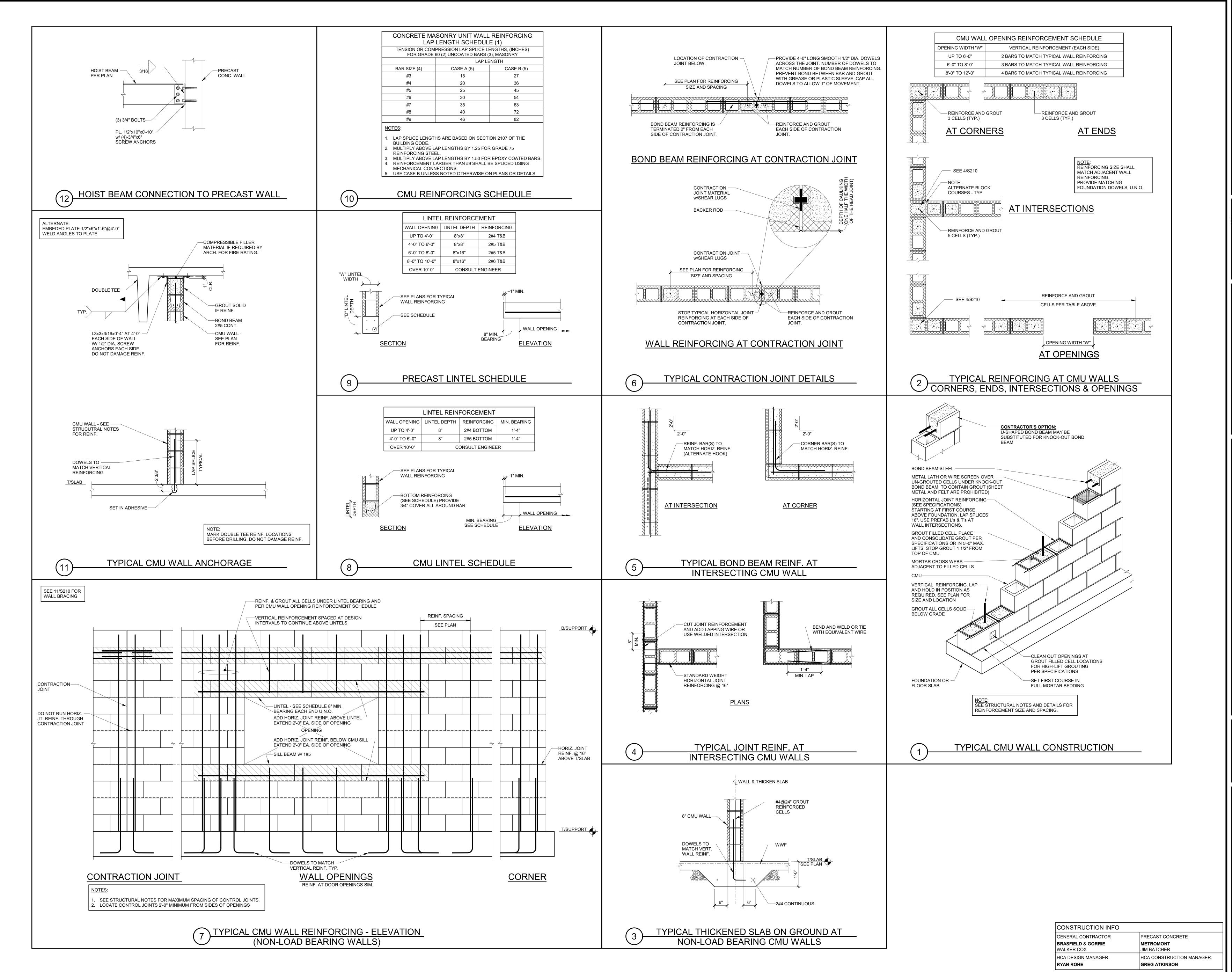
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COA No. 1329

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FOUNDATION SECTIONS AND



SCALE: 1/8"=1'-0"

SCALE: 3/32"=1'-0"

SCALE: 3/4"=1'-0"

SCALE: 1/2"=1'-0"

SCALE: 1-1/2"=1'-0"

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Architecture Interiors Planning

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Brentwood, TN 37027

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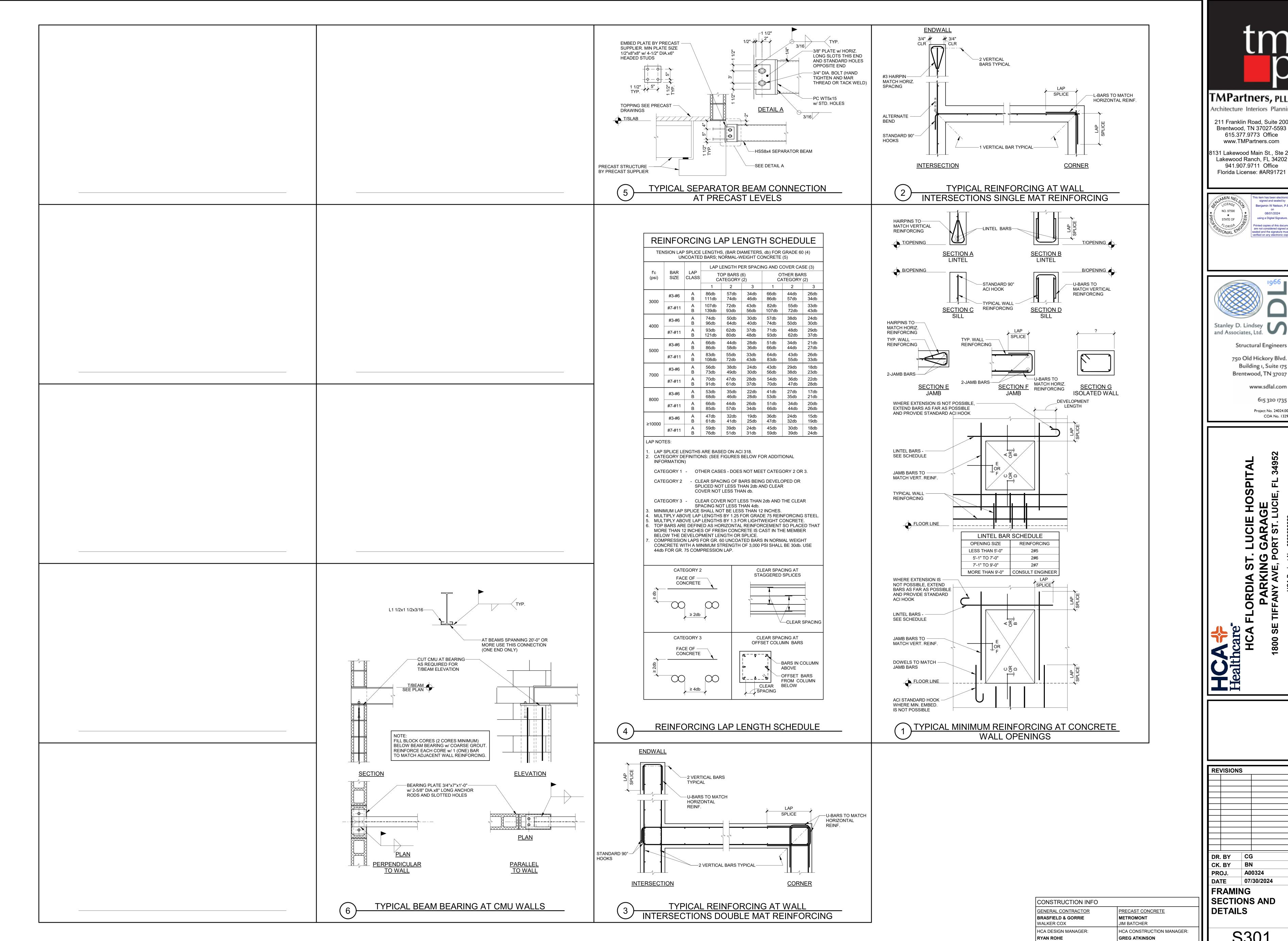
PROJ. 07/30/2024 DATE MASONRY

SECTIONS AND DETAILS

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0" 2' 4' SCALE: 1/4"=1'-0"

0" 4' 8' SCALE: 1/8"=1'-0"

0" 8' SCALE: 3/32"=1'-0"

0" 8' 16' SCALE: 1/16"=1'-0"

0" 1' 2' SCALE: 1/2"=1'-0"

0" 1' SCALE: 1"=1'-0"

0" 1' SCALE: 3/4"=1'-0"

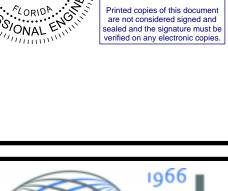
0" 6" SCALE: 1-1/2"=1'-0"

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REVISIONS DR. BY CG CK. BY PROJ. 07/30/2024 DATE FRAMING SECTIONS AND

S301

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0" 16' 32' SCALE: 1/32"=1'-0"