General Notes

GN. GENERAL

- THE STRUCTURAL DRAWINGS AND SPECIFICATIONS ARE A PORTION OF THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR AND SUBCONTRACTORS SHALL REFERENCE, AND COORDINATE WITH ALL OTHER DISCIPLINE'S DRAWINGS. ANY DISCREPANCIES OR OMISSIONS SHALL BE REPORTED TO THE STRUCTURAL ENGINEER AND ARCHITECT.
- DESIGN CRITERIA:
 - A. GENERAL BUILDING CODE: 2023 FLORIDA BUILDING CODE, 8TH EDITION
 - B. DESIGN LOAD CRITERIA: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, AMERICAN SOCIETY OF CIVIL ENGINEERS, ASCE 7
 - C. FLOOD LOAD CRITERIA: FLOOD RESISTANT DESIGN AND CONSTRUCTION, AMERICAN SOCIETY OF CIVIL ENGINEER, ASCE 24
 - BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, AMERICAN CONCRETE INSTITUTE, ACI 318
 - E. STRUCTURAL STEEL SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS. AMERICAN INSTITUTE OF STEEL CONSTRUCTION, AISC 360
 - F. STEEL JOISTS: STANDARD SPECIFICATIONS FOR K-SERIES, LH-SERIES, AND DLH-SERIES OPEN WEB STEEL JOISTS AND FOR JOIST GIRDERS, STEEL JOIST INSTITUTE, SJI-100
 - G. STEEL DECK: STANDARD FOR STEEL ROOF DECK, STANDARD FOR NON-COMPOSITE STEEL FLOOR DECK, STANDARD FOR COMPOSITE STEEL FLOOR DECK SLAB, STEEL DECK INSTITUTE, SDI RD, SDI NC, SDI C
 - BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES, THE MASONRY SOCIETY, TMS 402/602
- DESIGN LOADS:
 - A. DEAD LOADS (PSF): ANY CHANGES IN CONSTRUCTION MATERIALS FROM THOSE SHOWN ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS SHALL BE REPORTED BY THE CONTRACTOR THE STRUCTURAL ENGINEER OF RECORD FOR VERIFICATION OF LOAD-CARRYING CAPACITY.
 - B. LIVE LOADS (PSF) ROOF (REDUCIBLE)-
 - ULTIMATE DESIGN WIND SPEED, Vult (MPH)-----162 ALLOWABLE DESIGN WIND SPEED, Vasd (MPH)----RISK CATEGORY---EXPOSURE CATEGORY--INTERNAL PRESSURE COEFFICIENT-----±0.18

FOR WALL COMPONENT AND CLADDING PRESSURES - SEE TYPICAL DETAILS

- D. RAIN LOADS: 15-MIN RAIN INTENSITY, I (IN/HR)-----8.94 60-MIN RAIN INTENSITY, I (IN/HR)------4.51
- E. FLOOD LOADS: FLOOD ZONE--
- EXISTING CONDITIONS:

CONTRACTOR TO VERIFY ALL EXISTING DIMENSIONS, ELEVATIONS, AND STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES PRIOR TO FABRICATION AND CONSTRUCTION.

- SUBMITTALS:
 - A. REVIEW OF SHOP DRAWINGS AND OTHER SUBMITTALS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL ENGINEER. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF THE SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. ALL SHOP DRAWINGS MUST BE REVIEWED AND "APPROVED" BY THE CONTRACTOR PRIOR TO
- B. ELECTRONIC SHOP DRAWINGS ARE TO BE SUBMITTED TO THE STRUCTURAL ENGINEER IN .PDF FORMAT.
- C. RESUBMITTED SHOP DRAWINGS SHALL HAVE ALL NOTED CHANGES SINCE THE PREVIOUS SUBMISSION IDENTIFIED BY CLOUDING OR OTHERWISE CLEARLY IDENTIFIED.
- D. SHOP DRAWINGS: THE CONTRACTOR SHALL SUBMIT FOR REVIEW ALL SHOP DRAWINGS AND DELEGATED DESIGN DRAWINGS REFERENCED ON THE SUBMITTAL SCHEDULE SHOWN IN TYPICAL DETAILS. DELEGATED DESIGN DRAWINGS ARE TO BE DESIGNED BY A LICENSED PROFESSIONAL ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED. CALCULATIONS SIGNED AND SEALED BY THE LICENSED ENGINEER SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW.
- 6. ALL DETAILS SHOWN IN THE DRAWINGS ARE TYPICAL. SIMILAR DETAILS APPLY TO SIMILAR CONDITIONS.
- 7. THE CONTRACTOR IS RESPONSIBLE FOR MEANS, METHODS, TECHNIQUES, SEQUENCING, AND PROCEDURE OF CONSTRUCTION.
- CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED FLOORS/ROOFS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT CONSTRUCTION LOADS DO NOT EXCEED THE NOTED DESIGN LIVE LOADS.
- FD. FOUNDATIONS
- GEOTECHNICAL ENGINEER: UES GEOTECHNICAL REPORT TITLE: GEOTECHNICAL EXPLORATION KANNER HIGHWAY PUD - RESIDENTIAL & COMMERCIAL DEVELOPMENT SOUTH KANNER HIGHWAY STUART. FLORIDA JOB NUMBER: 3330.2100102.0000 REPORT DATE: OCTOBER 3, 2023
- SHALLOW FOUNDATION ALLOWABLE BEARING PRESSURES (PSF): STRIP FOUNDATIONS-----2,500 SPREAD FOUNDATIONS-------2,500
- ALL FOUNDATION BEARING SURFACES SHALL BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE TO ENSURE COMPLIANCE WITH NOTED PRESSURES.
- FINAL BEARING ELEVATIONS MAY VARY AS REQUIRED TO PROVIDE PROPER BEARING CAPACITY IN AN APPROVED BEARING STRATUM AS DETERMINED BY THE GEOTECHNICAL ENGINEER.
- FOOTINGS SHALL BE NEATLY EXCAVATED WHERE POSSIBLE AND FREE OF LOOSE OR WET MATERIALS. WHERE SOFT OR UNSUITABLE BEARING SURFACES ARE ENCOUNTERED, THE AREA SHALL BE UNDERCUT AS REQUIRED AND REPLACED WITH LEAN CONCRETE OR COMPACTED. DENSE GRADED STONE AS DIRECTED BY THE GEOTECHNICAL ENGINEER.
- FOOTINGS SHALL BE PLACED THE SAME DAY AS INSPECTED BY THE GEOTECHNICAL ENGINEER UNLESS OTHERWISE DIRECTED BY THE GEOTECHNICAL ENGINEER.

- COMPACTED FILL SHALL MEET THE REQUIREMENTS NOTED IN THE GEOTECHNICAL REPORT. EXCAVATED MATERIAL MAY BE USED AS BACKFILL WITH WRITTEN APPROVAL FROM THE GEOTECHNICAL ENGINEER STATING THAT SUCH MATERIAL IS SUITABLE AS BACKFILL AND INSTRUCTIONS ARE GIVEN FOR PROPER MOISTURE CONTENT AND COMPACTION.
- FOUNDATION AND RETAINING WALLS SHALL NOT BE BACKFILLED UNTIL CONCRETE HAS REACHED NOTED 28-DAY COMPRESSIVE STRENGTH.
- PROVIDE 4" OF COMPACTED GRANULAR FILL BENEATH ALL SLABS ON GRADE. PROVIDE 10 MIL VAPOR RETARDER BETWEEN BOTTOM OF SLAB AND TOP OF GRANULAR FILL.
- CN. CONCRETE
- CONCRETING OPERATIONS SHALL COMPLY WITH ACI STANDARDS.
- MINIMUM CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS (PSI), TYPE OF CONCRETE, MAXIMUM W/C (WATER/CEMENTITIOUS MATERIALS RATIO), TOTAL AIR CONTENT, SLUMP AND CONCRETE USE:

W/C AIR SLUMP USE NORMAL WT. 0.57 --- 3" TO 5" FOOTINGS NORMAL WT. 0.53 4-6% 3" TO 5" UNLESS NOTED

- REINFORCING BARS USED IN CONCRETE SHALL MEET ASTM A615, GRADE 60.
- WELDED WIRE REINFORCEMENT (WWR) USED IN CONCRETE SHALL MEET ASTM A185. MINIMUM LAB AND EMBEDMENT TO BE THE GREATER OF ONE CROSS WIRE SPACING + 2", OR 6".
- CONCRETE COVERAGE OF REINFORCEMENT, UNLESS NOTED:
- FOOTINGS-----2" TOP & 3" BOTTOM & SIDES
- REINFORCING BARS SHOWN IN SECTIONS AND DETAILS ARE A SCHEMATIC REPRESENTATION OF THE MEMBER REINFORCEMENT. SEE SCHEDULES, SECTION NOTES, AND GENERAL NOTES FOR ACTUAL REINFORCEMENT REQUIRED. DO NOT SCALE FROM SECTIONS.
- DETAILING, FABRICATION, AND PLACEMENT OF REINFORCEMENT AND ANY ACCESSORIES SHALL BE IN ACCORDANCE WITH ACI 315, GUIDE TO PRESENTING REINFORCING STEEL DESIGN DETAILS. FABRICATION OF REINFORCEMENT STEEL CANNOT BEGIN TILL ENGINEER OF RECORD HAS REVIEWED AND APPROVED SHOP DRAWINGS. SEE SUBMITTAL SCHEDULE FOR SHOP DRAWING REQUIREMENTS.
- SPLICES SHALL BE CLASS "B" TENSION LAP SPLICES UNLESS NOTED. REINFORCEMENT MARKED CONTINUOUS SHALL BE SPLICED WITH A CLASS "B" TENSION LAP SPLICE. BARS SHALL BE IN CONTACT WHEN MAKING A LAP SPLICE UNLESS OTHERWISE NOTED.
- DO NOT WELD REINFORCEMENT WITHOUT PRIOR APPROVAL FROM THE ENGINEER OF RECORD.
- STRUCTURAL STEEL
- FABRICATE AND ERECT ALL STRUCTURAL STEEL IN ACCORDANCE WITH AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
- THE STEEL FRAME IS "NON-SELF-SUPPORTING". ADEQUATE TEMPORARY SUPPORT MUST BE PROVIDED BY THE CONTRACTOR TILL THE LATERAL FORCE RESISTING SYSTEM ELEMENTS AND STABILITY OF THE STRUCTURE IS IN PLACE.
- STRUCTURAL STEEL AND STRUCTURAL STEEL CONNECTIONS MUST CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS, UNLESS NOTED ELSEWHERE IN THE

WIDE FLANGE (W & WT) SHAPES	ASTM A992
CHANNELS (C & MC), ANGLES (L)	ASTM A36
PLATES	ASTM A572 GR. 50
HOLLOW STRUCTURAL SECTION (HSS)	ASTM A500, GR.C
HEADED STUDS	•
BOLTS	ASTM F3125
NUTS	ASTM A56
ANCHOR RODS	ASTM F1554 GR. 36
THREADED ROD	ASTM A36
WASHERS (FOR STRUCTURAL BOLTS)	ASTM F436
WASHERS (FOR ANCHOR BOLTS)	
WELDED CONNECTIONS	
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- BEAMS ARE TO BE SPACED EQUALLY IN BAYS, UNLESS DIMENSIONED ON PLAN.
- BEAMS SHALL BE ERECTED WITH NATURAL CAMBER ORIENTED UPWARDS.
- FABRICATE BRACING MEMBERS WITH SUFFICIENT DRAW TO PREVENT SAGGING.
- 7. HSS MEMBERS SHALL HAVE 1/4" CLOSURE PLATE.
- GROUT UNDER BASE PLATES SHALL BE NON-SHRINK, NON-METALLIC TYPE. GROUT SHALL HAVE SPECIFIED DESIGN COMPRESSIVE STRENGTH TWO TIMES THAT OF THE
- STRUCTURAL STEEL MEMBERS SHALL NOT BE CUT, SPLICED, OR MODIFIED IN THE FIELD UNLESS NOTED ON THE STRUCTURAL DRAWINGS OR APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
- STRUCTURAL STEEL NOT EXPOSED TO VIEW SHALL BE PRIMED WITH THE MANUFACTURER'S STANDARD SHOP PRIMER. STRUCTURAL STEEL EXPOSED TO WEATHER IN ITS FINAL POSITION SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A123. STRUCTURAL STEEL EXPOSED TO VIEW, REFER TO PROJECT SPECIFICATIONS FOR FINISHED COATING SYSTEM.
- CONNECTION WORK LINES SHOWN IN THE TYPICAL DETAILS ARE ALONG THE MEMBERS NEUTRAL AXIS UNLESS NOTED.
- 12. ALL WELD SHALL CONFORM TO THE AMERICAN WELDING SOCIETY STANDARDS AND MUST BE PERFORMED BY A CERTIFIED WELDER.
- 13. ALL WELD SIZES SHOWN IN THE DRAWINGS ARE CONSIDERED EFFECTIVE WELD SIZES.
- 14. ALL BOLTED CONNECTIONS SHALL BE BEARING TYPE CONNECTIONS.
- 15. ALTERNATE CONNECTION DETAILS FROM THOSE SHOWN IN THE TYPICAL DETAILS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD FOR APPROVAL.
- 16. ALL STRUCTURAL STEEL CONNECTIONS ARE TO BE DESIGNED UNDER DIRECT SUPERVISION OF A LICENSED PROFESSIONAL ENGINEER IN THE STATE IN WHICH THE PROJECT IS LOCATED.
- 17. SEE PLAN NOTES FOR BEAM REACTIONS.
- SJ. STEEL JOISTS
- DESIGN, FABRICATE, AND ERECT STEEL JOISTS IN ACCORDANCE WITH SJI STANDARDS.
- PROVIDE MINIMUM END BEARING AT SUPPORTS AS REQUIRED BY SJI. STAGGER ENDS OF JOISTS AS NECESSARY.
- DECK STEEL DECK SPLICES TO BE OVER JOISTS.
- PROVIDE HORIZONTAL AND DIAGONAL BRIDGING IN ACCORDANCE WITH SJI TO PROVIDE ADEQUATE BOTTOM CHORD BRACING.
- DESIGN ROOF JOISTS FOR NET WIND PRESSURES AND ADDITIONAL LOADING SHOWN ON PLAN OR IN SECTIONS. SEE COMPONENTS AND CLADDING TABLES FOR WIND PRESSURES.
- JOISTS SHALL BE EQUALLY SPACED IN BAYS.

SD. STEEL DECK

- ROOF DECK: WIDE RIB TYPE B, STEEL ROOF DECK, 20 GAGE, 1-1/2" DEEP.
- DECK PROPERTIES, ACCESSORIES, AND ATTACHMENTS SHALL BE ACCORDANCE WITH THE STEEL DECK INSTITUTE (SDI)
- DECKS SHALL BE CONTINUOUS OVER THREE SPANS.
- DO NOT SHORE DECK.
- SHEET STEEL FOR DECKING SHALL BE GRADE A653 SS GR50, MINIMUM.
- DECK SHALL HAVE A G60 GALVANIZED FINISH.
- STEEL DECK SHALL HAVE A MINIMUM 1-1/2" BEARING AT END CONDITIONS.
- LIGHT GAGE FRAMING, SUSPENDED CEILING SYSTEMS, MEP SYSTEMS, AND OTHER UTILITIES SHALL NOT BE SUPPORTED BY THE STEEL ROOF DECK.
 - MASONRY
 - MASONRY CONSTRUCTION SHALL CONFORM TO TMS 402/602.
- CONCRETE MASONRY UNITS (CMU) SHALL BE NORMAL WEIGHT (DENSITY = 125 PCF), CONFORMING TO ASTM C90, UNLESS NOTED.

STEEL DECK SHALL HAVE A MINIMUM 3" BEARING AT INTERMEDIATE CONDITIONS.

COMPRESSIVE STRENGTH OF MASONRY (f'm) IS 2000 PSI AT 28 DAYS.

CONFORM TO ASTM C476 AND BE PLACED IN ACCORDANCE WITH TMS 602.

- COMPRESSIVE STRENGTH OF GROUT (f'g) IS 2500 PSI AT 28 DAYS. GROUT SHALL
- MORTAR SHALL CONFORM TO ASTM C270, TYPE S OR M FOR TYPICAL CONDITIONS (TYPE M
- MASONRY WALLS SHALL BE LAID IN RUNNING BOND, UNLESS NOTED.
- ALL BLOCK CELLS BELOW GRADE SHALL BE FILLED WITH CONCRETE OR GROUT.
- SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF CONTROL JOINTS AND OPENINGS.
- REINFORCING BARS USED IN MASONRY SHALL MEET ASTM A615, GRADE 60. CONDUIT, PIPING, AND SLEEVES OF ANY MATERIAL TO BE EMBEDDED IN MASONRY SHALL
- COMPLY WITH THE FOLLOWING REQUIREMENTS: A. CANNOT BE MADE OF ALUMINUM.

THE LARGER DIAMETER.

- B. CANNOT PASS THROUGH JAMBS, LINTELS, BOND BEAMS, OR SHEAR WALLS WITHOUT
- APPROVAL BY THE STRUCTURAL ENGINEER. C. REINFORCEMENT BARS CANNOT BE CUT, BENT, OR DISPLACED.

SHALL REMAIN IN PLACE UNTIL PERMANENT RESTRAINT IS PROVIDED.

- SPACING OF CONDUIT, PIPE, OR SLEEVE SHALL NOT BE CLOSER THAN 3 DIAMETERS ON CENTER. MINIMUM SPACING OF DIFFERENT DIAMETERS SHALL BE DETERMINED BY
- TEMPORARY BRACING OF MASONRY WALLS IS THE RESPONSIBILITY OF THE CONTRACTOR AND
- HORIZONTAL JOINT REINFORCING: LADDER TYPE, 9 GAGE SPACED VERTICALLY AT 16", UNLESS NOTED. PLACE REINFORCING ACCORDING TO MANUFACTURER'S RECOMMENDATIONS. LAP REINFORCING A MINIMUM OF 6".
- COLD-FORMED METAL FRAMING
- DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS (CFS) AND ACCESSORIES IS THE RESPONSIBILITY OF THE COLD-FORMED STEEL MANUFACTURER. THE CFS MANUFACTURER IS RESPONSIBLE FOR THE DESIGN OF THE CONNECTIONS BETWEEN MEMBERS AND THEIR CONNECTION TO THE BUILDINGS PRIMARY STRUCTURAL FRAME.
- ANY COLD-FORMED STEEL SIZES NOTED ARE FOR PRELIMINARY PRICING INFORMATION ONLY. THE COMPLETE DESIGN OF COLD-FORMED STEEL FRAMING SYSTEM AND PREPARATION OF ERECTION DRAWINGS ARE BY THE ENGINEER RESPONSIBLE FOR THEIR DESIGN.
- COLD-FORMED STEEL FABRICATION AND ERECTION SHALL CONFORM TO AISI S100 "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".
- COLD-FORMED STEEL STUDS SHALL CONFORM TO THE FOLLOWING, UNLESS OTHERWISE NOTED IN THE STRUCTURAL PLANS AND SECTIONS:

GALVANIZED STUDS	
33 & 43 (FY=33KSI)ASTM	A1003/A1003M
	,
54, 68, & 97 (FY=50KSI)ASTM	,
TRACKS, BRIDGING, ACCESSORIESASTM	A1003/A1003M

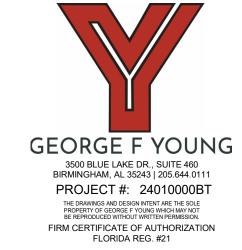
CONNECTIONS (WELDED & BOLTED)-----AWS D1.3

- PROVIDE COLD-FORMED STEEL CAPABLE OF WITHSTANDING DESIGN LOADS WITHIN
 - A. DESIGN LOADS AS INDICATED IN SECTION ON OF THESE GENERAL NOTES.
- B. DEFLECTION LIMITS AS INDICATED BY THE FLORIDA BUILDING CODE.

LIMITS AND UNDER CONDITIONS INDICATED.

WOOD SHEATHING

- ALL MANUFACTURED WOOD FRAMING CONNECTORS TO BE BY SIMPSON STRONG-TIE COMPANY, INC. OR APPROVED EQUAL. ALL CONNECTORS SHALL BE FASTENED TO FRAMING MEMBERS FILLING THE REQUIRED NUMBER OF CONNECTOR HOLES WITH THE TYPE AND SIZE FASTENERS SPECIFIED BY THE MANUFACTURER.
- CANOPY ROOF SHEATHING: 3/4" PLYWOOD, APA STRUCTURAL I RATED SHEATHING, EXPOSURE I. PANEL IDENTIFICATION INDEX 48/24. PROVIDE ONE H-CLIP AT ALL ADJOINING PANEL JOINTS MIDWAY BETWEEN SUPPORTS. LONG DIMENSION OF PANEL PERPENDICULAR TO SUPPORTS.
- CANOPY ROOF SHEATHING ATTACHMENT PATTERN: PROVIDE SSMA #10 FLAT-HEAD SELF-DRILLING SCREWS WITH A MINIMUM HEAD DIAMETER OF 0.333" AT 12 INCHES AT ALL FOUR PANEL EDGES AND 12 INCHES IN THE FIELD OF THE PANEL.



For For Signed & Sealed Signed &

Approval | Record | Shop Dwgs | Sealed Calcs

Structural Submittal Schedule

Χ

Description

MIX DESIGN, NOTE THE USAGE FOR EACH

PROVIDED DESIGN. PROVIDE MIX RECIPE,

BREAK DATE, AND PRODUCT DATA AS

CONCRETE MIX DESIGN SUBMIT MIX DESIGN FOR EACH CONCRETE

REQUIRED BY ACI 301.

EMBEDDED ITEMS EMBED PLATES, BEARING PLATES, ETC.

MILD REINFORCEMENT VERTICAL REINF, BOND BEAM REINF, LITEL

MASONRY ACCESSORIES BAR SPASCERS, HORIZONTAL JOINT REINF,

REINF, BENT BAR SCHEDULE

STEEL DECK SUBMIT PLANS SHOWING SPANS, DECK

STRUCTURAL MEMBERS

AND/OR DECK TYPE.

NEW ROOF CURB SYSTEM MATERIAL AND LAYOUT OF NEW

SIZE, AND MATERIAL FOR EACH ROOF

MECHANICAL CURB AND ATTACHMENT TO

CONCRETE ACCESSORIES VAPOR BARIER, REBAR CHAIRS, ETC.

GROUT MIX DESIGNS MIX DESIGN AND BREAK DATA

Submittal Type

33000 STRUCTURAL CONCRETE

WELDED WIRE REINFORCEMENT

MILD REINFORCEMENT

MILD REINFORCEMENT

42000 CONCRETE MASONRY UNITS (CMU)

MASONRY PRODUCT

MORTAR MIX DESIGNS

PRECAST LINTEL DATA

STRUCTURAL STEEL

DECK ATTACHMENT

STEEL JOISTS AND JOIST

51200 STRUCTURAL STEEL

77200 ROOF ACCESSORIES

52100 STEEL JOISTS

53100 STEEL DECK

INFORMATION

31600 FOUNDATIONS



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BANK

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CONTRACTOR IS RESPONSIBLE FOR THE COORDINATION, SCHEDULING AND CONFORMANCE OF ITS WORK AND ALL WORK PERFORMED BY SUBCONTRACTORS OR OTHER TRADES WITH THE DESIGN DOCUMENTS.

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ISSUE BY DATE DESCRIPTION 3/24/25 | PERMIT SET

PROJECT INFORMATION BLOCK JOB# 230651 DATE: 3/24/25 DRAWN BY: SSC CHECKED BY:

SHEET TITLE

GENERAL NOTES

Abbreviations

EACH FACE

EACH SIDE

EXPANSION

EACH WAY

EDGE OF DECK

EDGE OF SLAB

ELECTRICAL

EMBEDMENT

FINISH FACE

ENGINEER

EQUAL

EQ

EXP

EW

EOD

EOS

ELEC

EMBED

ENGR

EXPANSION JOINT

AND

PERCENT

DIAMETER

CONSTRUCTION

ABOVE FINISH FLOOR

STRUCTURAL STEEL

AIR HANDLING UNIT

MATERIALS

DEGREE

ASD

ACI

AISC

AISI

AWS

AFF

ARCH.

AESS

ALT

APPRV

BRG

BOF

BOT

BLDG

BTWN

CIP

CONC

CMU

CRSI

CONT

CJ

CLR

COL

COMP

CONN

CONTR

DIA

DIM

DBL

DEG

DP

EΑ

DWLS

ASTM

NUMBER OR POUND

ALLOWABLE STRESS DESIGN

AMERICAN CONCRETE INSTITUTE

AMERICAN INSTITUTE OF STEEL

AMERICAN IRON AND STEEL INSTITUTE





400 N Ashley Drive, P: 813 - 323 - 9233 Suite. 600 Lic. #: AA - 0003590 Tampa, FL 33602 W: www.bdgllp.com



ANNER

S KANNER HIGHWA ART, FLORIDA 34994 THIRD

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> ISSUE BY DATE DESCRIPTION 3/24/25 | PERMIT SET

PROJECT INFO	RMATION BLOCK
JOB#	230651
DATE:	3/24/25
DRAWN BY:	SSC
CHECKED BA-	IDB

CHECKED BY: SHEET TITLE

> COMPONENTS & CLADDING

S-102

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	3					3

Flat Roof Components and Cladding Wind Pressures

Flat Roof Ultimate Wind Pressures

Interior Zone 1'

Interior Zone 1

Perimeter Zone 2

Corner Zone 3

RELIABLE DEAD LOAD FOR UPLIFT ON ROOF

Max - Pressure

(psf)

-33.2

-33.2

-33.2

-33.2

-57.8

-54.0

-48.9

-45.1

-76.2

-71.3

-64.8

-59.9

-103.9

-94.1

-81.1

-71.3

Max + Pressure

(psf)

16.0

16.0

16.0

16.0

16.0

16.0

16.0

16.0

33.2

31.7

29.8

28.3

33.2

31.7

29.8

28.3

(sq.ft)

10

20

50

≥100

10

20

50

≥100

10

20

50

≥100

20

50

≥100

h = 14' - 0"

b = 0.2h = 2' - 10"

c = 0.6h = 8' - 5"

STRUCTURE = 6 PSF

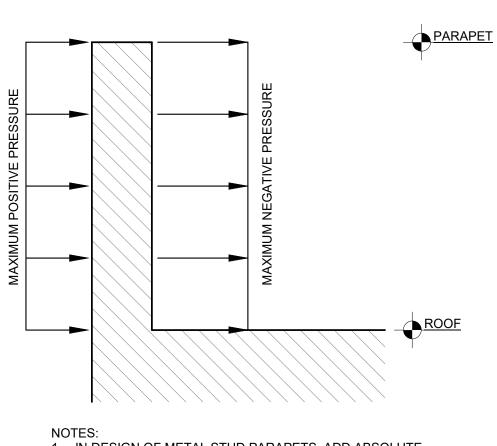
ı	N.	T.S

Wall Ultimate Wind Pressures						
Area Max + Pressure Max - Pressure (sq.ft) (psf) (psf)						
	Interior Zone	4				
10	33.2	-36.0				
20	31.7	-34.5				
50	29.8	-32.5				
100	28.3	-31.1				
≥200	24.9	-27.7				
	Corner Zone	5				
10	33.2	-44.2				
20	31.7	-41.3				
50	29.8	-37.4				
100	28.3	-34.5				
≥200	24.9	-27.7				
a = 4' - 1	a = 4' - 11"					

			TOP OF PARAPET
(5p)	4 p	(5p)	ROOF ELEVATION
(5)	4	(5)	
			FINISH FLOOR
a		a	

2 Wall Components and Cladding Wind Pressures N.T.S.

Parapet Ultimate Wind Pressures (Windward + Leeward)		
Area (SQ.FT)	Max + Pressure (PSF)	
Int	erior Zone 4p	
10	104.2	
20	97.5	
50	88.6	
100	81.8	
Co	orner Zone 5p	
10	104.2	
20	97.5	
50	88.6	
100	81.8	



1. IN DESIGN OF METAL STUD PARAPETS, ADD ABSOLUTE VALUES OF MAXIMUM POSITIVE AND NEGATIVE PARAPET PRESSURES IN APPLICABLE ZONE.

2. IN DESIGN OF EXTERIOR FINISHES ATTACHMENT, USE THE LARGEST ABSOLUTE VALUE OF LISTED PRESSURES FOR THE APPLICABLE ZONE.

Parapet Components and Cladding Wind Pressures N.T.S.

AMERICAN SOCIETY FOR TESTING PSI FOOT OR FEET POUNDS PER SQUARE INCH PCI FTG FOOTING POUNDS PER CUPIC FOOT AMERICAN WELDING SOCIETY FDN FOUNDATION PLBG PLUMBING PLF POUNDS PER LINEAR FOOT FIELD VERIFY ARCHITECT OR ARCHITECTURAL FAR SIDE POST-TENSIONING ARCHITECTURALLY EXPOSED GALV GALVANIZED RADIUS REINF REINFORCEMENT GAUGE GRADE SECT SECTION HEADED STUDS SC SHEAR CONNECTOR H. STUD SHT

NORMAL WEIGHT CONCRETE

POUNDS PER SQUARE FOOT

NOT IN CONTRACT

NOT TO SCALE

ON CENTER

OPENING

OPPOSITE

PILE CAP

PLATE

NS

NWC

NIC

N.T.S

OC

OPNG

OPP

PC

PSF

ALTERNATE APPROX APPROXIMATE APPROVED BOTTOM SLV INCH SHORT LEG VERTICAL **BEARING** INVERT SIM SIMILAR BOTTOM OF FOUNDATION SOG INFORMATION SLAB ON GRADE BOTTOM STD STANDARD BUILDING JOINT STL STEEL BETWEEN JOIST SDI STEEL DECK INSTITUTE JOIST GIRDER SQUARE FOOT CAST-IN-PLACE STIFF STIFFENER CONCRETE SUP SUPPORT KIP (1000 LBS) CONCRETE MASONRY UNIT KIPS PER SQUARE FOOT SYM SYMMETRICAL CONCRETE REINFORCING STEEL KSI KIPS PER SQUARE INCH STD STANDARD INSTITUTE KLF KIPS PER LINEAR FOOT CONTINUOUS TOP CONTROL JOINT TO TOP OF LW LIGHT WEIGHT CLEAR TOC LWC TOP OF CONCRETE LIGHT WEIGHT CONCRETE COLUMN TOF LRFD LOAD AND RESISTANCE FACTOR TOP OF FOOTING CHANNEL DESIGN TOS TOP OF STEEL

COMPRESSION LLV LONG LEG VERTICAL TOW TOP OF WALL CONNECTION LLH LONG LEG HORIZONTAL TOPC TOP OF PILE CAP CONTRACTOR LBS POUNDS TYP **TYPICAL ANGLE** TENSION DIAMETER DIMENSION METAL UNO UNLESS NOTED OTHERWISE DOWELS MINIMUM UNDERSIDE US DOUBLE MAXIMUM

DEGREE MANUFACTURER WELDED WIRE REINFORCEMENT WWR DRILLED PIER MOMENT WITH MECH MECHANICAL WP **WORK POINT** EACH MISC MISCELLANEOUS WITHOUT EACH END

> **DESCRIPTION SYMBOL** ■ NEW COLUMN GRID LINE

■ EXISTING COLUMN GRID LINE VIEW NUMBER — PLAN TITLE

■ PLAN TITLE

VIEW NUMBER SECTION TITLE - REFERENCE SHEET

VIEW NUMBER

REFERENCE SHEET

■ SECTION TAG

Symbol Legend N.T.S.

■ SECTION TITLE

■ ELEVATION TAG REFERENCE SHEET

VIEW NUMBER

Special Inspection General Notes

- SI.1 SPECIAL INSPECTIONS ARE NOT REQUIRED PER THE FLORIDA BUILDING CODE.
 THE SPECIAL INSPECTIONS LISTED ARE FOR THE OWNER'S QUALITY CONTROL PLANS.
 THESE INSPECTIONS ARE IN ACCORDANCE WITH AND REFERENCE THE
 2021 INTERNATIONAL BUILDING CODE. THE FLORIDA BUILDING CODE DOES NOT HAVE
 SPECIFIC REFERENCES FOR THESE SECTIONS. ALL SPECIAL INSPECTIONS SHALL BE
 PERFORMED IN CONFORMANCE WITH THE APPLICABLE INTERNATIONAL BUILDING CODE
 AND ITS REFERENCED SPECIFICATIONS.
- SI.2 THE SPECIAL INSPECTOR SHALL BE EMPLOYED BY THE OWNER OR THE OWNER'S AGENT AND NOT BY THE CONTRACTOR OR SUBCONTRACTOR WHOSE WORK IS TO BE INSPECTED OR TESTED. ANY CONFLICT OF INTEREST MUST BE DISCLOSED TO THE BUILDING OFFICIAL PRIOR TO COMMENCING WORK.
- THE SPECIAL INSPECTOR SHALL BE QUALIFIED PER THE INTERNATIONAL BUILDING CODE AND SHALL BE EDUCATED IN THE TASKS REQUIRED TO CONDUCT, SUPERVISE, AND EVALUATE THE INSPECTIONS. THE SPECIAL INSPECTOR MUST ALSO BE OBJECTIVE, COMPETENT, AND HAVE ACCESS TO THE APPROPRIATE TESTING EQUIPMENT WHICH SHALL BE MAINTAINED AND PERIODICALLY CALIBRATED. THE QUALIFICATIONS OF THE SPECIAL INSPECTOR MAY BE SUBJECT TO THE APPROVAL OF THE BUILDING OFFICIAL.
- SI.4 SPECIAL INSPECTION AGENTS:
 - ATA: APPROVED TESTING AGENCY
 - GEOR: GEOTECHNICAL ENGINEER OF RECORD:
 UES
 607 NW COMMODITY COVE
 PORT ST. LUCIE, FL 34986
 - SEOR: STRUCTURAL ENGINEER OF RECORD:

 GEORGE F. YOUNG, INC.

 3500 BLUE LAKE DR, SUITE 460
 BIRMINGHAM, AL 35243
- SI.5 THE SPECIAL INSPECTIONS SHALL BE PERFORMED IN ADDITION TO ANY OBSERVATIONS PERFORMED BY THE ENGINEER OF RECORD AND ANY INSPECTIONS PERFORMED BY THE BUILDING OFFICIAL.
- SI.6 THE SPECIAL INSPECTOR SHALL MAINTAIN RECORDS AND PROVIDE THE REQUIRED DOCUMENTATION AS PRESCRIBED IN THE INTERNATIONAL BUILDING CODE, INCLUDING THE SUBMITTAL OF REPORTS TO THE BUILDING OFFICIAL AND THE DESIGNER OF RECORD.
- SI.7 THE CONTRACTOR SHALL COORDINATE THE CONSTRUCTION SCHEDULE WITH THE SPECIAL INSPECTOR TO ALLOW FOR SPECIAL INSPECTIONS.
- SI.8 CONSTRUCTION WHICH REQUIRES SPECIAL INSPECTIONS SHALL BE MAINTAINED IN SUCH A STATE AS TO ALLOW ACCESS FOR THE SPECIAL INSPECTOR UNTIL THE REQUIRED INSPECTIONS OR TESTS HAVE BEEN COMPLETED.
- SI.9 ANY DEVIATIONS FOUND DURING THE SPECIAL INSPECTION PROCESS SHALL IMMEDIATELY BE BROUGHT TO THE ATTENTION OF THE DESIGNER OF RECORD. ALL DEVIATIONS MUST BE ADDRESSED PRIOR TO COMPLETION OF THE WORK.
- SI.10 INSPECTION FREQUENCY:
 - A. CONTINUOUS SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS PRESENT WHEN AND WHERE THE WORK TO BE INSPECTED IS BEING PERFORMED.
 - B. PERIODIC SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS INTERMITTENTLY PRESENT WHERE THE WORK TO BE INSPECTED HAS BEEN OR IS BEING PERFORMED.
 - C. OBSERVE OBSERVE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS.
 - D. PERFORM PERFORM TASKS FOR EACH JOINT, MEMBER, AND CONNECTION.
- SI.11 SPECIAL INSPECTIONS FOR STRUCTURAL, LOAD-BEARING, OR LATERAL LOAD BEARING FABRICATED ITEMS SHALL BE PERFORMED FOR THE FABRICATED ITEMS AT THE FABRICATOR'S SHOP. SPECIAL INSPECTIONS FOR FABRICATED ITEMS MAY BE WAIVED WHEN THE FABRICATOR IS REGISTERED AND HAS APPROVAL TO PERFORM THE WORK WITHOUT SPECIAL INSPECTIONS. IF THE INSPECTIONS ARE WAIVED, THE FABRICATOR MUST SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL SHOWING COMPLIANCE WITH THE APPROVED STRUCTURAL DRAWINGS.

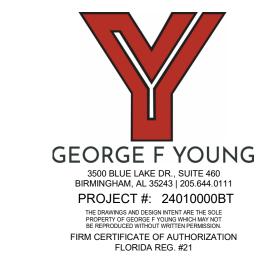
	Soils					
NO.	INSPECTION TASK	FREQUENCY	REFERENCE FOR CRITERIA	AGENT		
1.00	VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	PERIODIC		GEOR		
2.00	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	PERIODIC		GEOR		
3.00	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	PERIODIC		GEOR		
4.00	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT, AND COMPACTION OF COMPACTED FILL.	CONTINUOUS		GEOR		
5.00	PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	PERIODIC		GEOR		

Wind Resistance					
NO.	INSPECTION TASK	FREQUENCY	REFERENCE FOR CRITERIA	AGENT	
1.00	COLD FORMED STEEL LIGHT-FRAMED CONSTRUCTION		IBC 1705.11.2		
1.01	WELDING OPERATIONS OF ELEMENTS OF THE MAIN WIND FORCE-RESISTING SYSTEM	CONTINUOUS		ATA	
1.02	SCREW ATTACHMENT, BOLTING, ANCHORING, OR OTHER FASTENING OF ELEMENTS OF THE MAIN WIND FORCE-RESISTING SYSTEM, INCLUDING SHEAR WALLS, DIAPHRAGMS, DRAG STRUTS, BRACES, AND HOLD DOWNS.	PERIODIC		ATA	
2.00	WIND RESISTING COMPONENTS		IBC 1705.11.3		
2.01	ROOF COVERINGS, ROOF DECK, AND ROOF FRAMING CONNECTIONS	PERIODIC		ATA	
2.02	EXTERIOR WALL COVERING AND CONNECTIONS TO ROOF AND FLOOR DIAPHRAGMS AND FRAMING	PERIODIC		ATA	

	Steel Joists			
NO.	INSPECTION TASK	FREQUENCY	REFERENCE FOR CRITERIA	AGENT
1.00	INSTALLATION OF OPEN-WEB STEEL JOISTS AND JOIST GIRDERS.			
1.01	END CONNECTIONS - WELDED OR BOLTED.	PERIODIC	SJI SPECIFICATIONS LISTED IN IBC 2207.1	ATA
1.02	HORIZONTAL OR DIAGONAL STANDARD BRIDGING OR BRIDGING THAT DIFFERS FROM SJI SPECIFICATIONS LISTED IN SECTION 2207.1.	PERIODIC	SJI SPECIFICATIONS LISTED IN IBC 2207.1	ATA

	Concrete			
NO.	INSPECTION TASK	FREQUENCY	REFERENCE STANDARD	Agent
1.00	INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	PERIODIC	ACI 318 CH 20, 25.2, 25.3, 26.5.1-26.5.3; IBC 1908.4	ATA
2.00	INSPECT ANCHORS CAST IN CONCRETE.	PERIODIC	ACI 318: 17.8.2	ATA
3.00	INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS.			ATA
3.01	INSPECT ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.	CONTINUOUS	ACI 318: 17.8.2.4	ATA
3.02	INSPECT MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.01.	PERIODIC	ACI 318: 17.8.2	ATA
4.00	VERIFY USE OF REQUIRED DESIGN MIX.	PERIODIC	ACI 318: CH 19, 26.4.3, 26.4.4; IBC 1904.1, 1904.2, 1908.2, 1908.3	ATA
5.00	PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE. DETERMINE UNIT WEIGHT OF LIGHTWEIGHT CONCRETE.	CONTINUOUS	ASTM C 172; ASTM C 31; ACI 318:26.4.5, 26.12; IBC 1908.10	ATA
6.00	INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	CONTINUOUS	ACI 318: 26.4.5; IBC 1908.6, 1908.7, 1908.8	ATA
7.00	VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	PERIODIC	ACI 318: 26.4.7-26.4.9; IBC 1908.9	ATA
8.00	INSPECT FORMWORK FOR SHAPE, LOCATION, AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	PERIODIC	ACI 318: 26.10.1(B)	ATA
9.00	ISOLATED CONCRETE FOOTINGS OF BUILDINGS THREE STORIES OR LESS ARE EXCEPTED FROM INSPECTIONS BUT NOT FROM MATERIALS TESTING.		IBC 1705.3 (1)	ATA
10.00	CONTINUOUS CONCRETE FOOTINGS SUPPORTING WALLS OF LIGHT-FRAME CONSTRUCTION OR THOSE THAT ARE DESIGNED IN ACCORDANCE WITH IBC 2015 TABLE 1809.7 ARE EXCEPTED FROM INSPECTIONS BUT NOT FROM MATERIALS TESTING.		IBC 1705.3 (2)	ATA
11.00	SLABS ON GRADE ARE EXCEPTED FROM INSPECTIONS BUT NOT FROM MATERIALS TESTING.		IBC 1705.3 (3)	ATA
12.00	CONCRETE FOUNDATION WALLS CONSTRUCTED IN ACCORDANCE WITH IBC 2015 TABLE 1807.1.6.2 ARE EXCEPTED FROM INSPECTIONS BUT NOT FROM MATERIALS TESTING.		IBC 1705.3 (4)	ATA

	Masonry - Level B			
NO.	INSPECTION TASK	FREQUENCY	REFERENCE FOR CRITERIA TMS 402/ACI 530/ASCE 5 TMS 602/ACI 530.1/ASCE 6	AGENT
1.00	VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) AS DELIVERED TO THE PROJECT SITE FOR SELF-CONSOLIDATING GROUT.	PERIODIC	TMS 602 ART. 1.5 B.1.B.3	ATA
2.00	VERIFICATION OF F'M AND F'AAC PRIOR TO CONSTRUCTION, EXCEPT WHERE SPECIFICALLY EXEMPTED BY TMS 402/ACI 530/ASCE 5.	PERIODIC	TMS 602 ART. 1.4 B	ATA
3.00	VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS.	PERIODIC	TMS 602 ART. 1.5	ATA
4.00	AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:			
4.01	PROPORTIONS OF SITE-PREPARED MORTAR	PERIODIC	TMS 602 ART. 2.1, 2.6 A	ATA
4.02	CONSTRUCTION OF MORTAR JOINTS	PERIODIC	TMS 602 ART. 3.3 B	ATA
5.00	PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:			
5.01	GROUT SPACE	PERIODIC	TMS 602 ART. 3.2 D, 3.2 F	ATA
5.02	GRADE, TYPE, AND SIZE OF REINFORCEMENT AND ANCHOR BOLT	PERIODIC	TMS 402 SEC. 6.1; TMS 602 ART. 2.4, 3.4	ATA
5.03	PLACEMENT OF REINFORCEMENT, CONNECTORS	PERIODIC	TMS 402 SEC. 6.1, 6.2.1, 6.2.6, 6.2.7; TMS 602 ART. 3.2 E, 3.4, 3.6 A	ATA
5.04	PROPORTIONS OF SITE-PREPARED GROUT	PERIODIC	TMS 602 ART. 2.6 B, 2.4 G.1.b	ATA
5.05	CONSTRUCTION OF MORTAR JOINTS	PERIODIC	TMS 602 ART. 3.3 B	ATA
6.00	VERIFY DURING CONSTRUCTION:			
6.01	SIZE AND LOCATION OF STRUCTURAL ELEMENTS	PERIODIC	TMS 602 ART. 3.3 F	ATA
6.02	TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION	PERIODIC	TMS 402 SEC. 1.2.1(e), 6.1.4.3, 6.2.1	ATA
6.03	PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°F)	PERIODIC	TMS 602 ART. 1.8 C, 1.8 D	ATA
7.00	OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS.	PERIODIC	TMS 602 ART. 1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3, 1.4 B.4	ATA
NOTES:	(a) CONTINUOUS REQUIRED FOR THE FIRST 5,000 SQUARE FEET OF AAC MASONRY. PERIODIC REQUIRED AFTER THE FIRST 5,000 SQUARE FEET OF AAC MASONRY.			





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FIFTH THIRD TPLACE AT KANNER

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SEA

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY JOHN D. PERDUE, ON THE DATE ADJACENT TO THE SEAL. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

PROJECT INFORMATION BLOCK

JOB # 230651

DATE: 3/24/25

DRAWN BY: SSC

CHECKED BY: JDP

SHEET TITLE

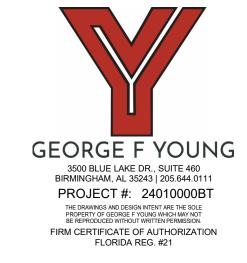
SPECIAL INSPECTIONS

SHEET NUMBER

S-103

1.00 IMPRIENT STRUCTURE AND STRUCTURE AND STRUCTURE IN PERFORMS FOR INSERTION IN PRIENT BY AND STRUCTURE AND S	ERIA AGENT
NEW PROPERTY AND ADDRESS OF THE PRABMENT DE CALE SEEL OR BEEC/TED STEEL PRABME AS APPROPRIATE TO VERDING COMPLIANCE WITH THE DETAILS STEACH CONNECTION. PROPERTY AND ADDRESS OF THE PRABMENT OF VERDING CONNECTION.	7 ATA
MISS 280 SEC N	ATA
VELDING PROCEDURE SPECIFICATIONS (WPS) AVAILABLE PERFORM	ATA
AMUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE MATERIAL IDENTIFICATION (PTPECRADE) METAL PROPERTY	1
ASC 380 SEC. N ATERNAL IDENTIFICATION SYSTEMM () ASSERVE DESERVE DESER	ATA
MELDER IDENTIFICATION SYSTEM [a] OSSERWE	ATA
TACHING TICKX WELD DULING JOINT GEOMETRY). JOINT PREPARATION, DIMENSIONS (ALIGNMENT, ROOT PACE, BEVEL). CLEANLINESS (CONDITION OF STEEL SURFACES). OBSERVE CONFIGURATION AND FINISH OF ACCESS HOLES OBSERVE CONFIGURATION AND FINISH OF ACCESS HOLES OBSERVE CONFIGURATION AND FINISH OF ACCESS HOLES OBSERVE AISC 300 SEC. N OBSERVE AISC 300 SEC. N OBSERVE O	ATA
TACKING (TACK WELD QUALITY AND LOCATION), AND BACKING TYPE AND FIT (IF APPLICABLE) OCONFIDENTION AND FIRST DISHO OF ACCESS HOLES OCHECK WELDING COUPMENT ORSERVE	ATA
OBSERVE OBSERVE AISC 360 SEC. N OBSERVE OBSE	ATA
3.01 USE OF QUALIFIED WELDING: 3.02 CONTROL AND HANDLING OF WELDING CONSUMABLE PACKAGING AND EXPOSURE CONTROL. 3.03 CONTROL AND HANDLING OF WELDING CONSUMABLE PACKAGING AND EXPOSURE CONTROL. 3.04 CONTROL AND HANDLING OF WELDING CONSUMABLE PACKAGING AND EXPOSURE CONTROL. 3.05 NO WELDING SUFFLINGS AND WELDING SUPPLIES WELDING WIND SPEED WITH ILLIMITS PRECIPITATION. AND TEMPERATURE 3.06 ENVIRONMENTAL CONDITIONS INCLIDINGS WIND SPEED WITH ILLIMITS, PRECIPITATION. AND TEMPERATURE 3.07 WELDING SETTINGS ON WELDING SUPPLINGS. THE AND THE PACKAGING AND PROPER POSITION (F. V. H., O.H.) 3.08 WELDING TECHNIQUES INSTAURAS, AND PROPER POSITION (F. V. H., O.H.) 3.09 WELDING TECHNIQUES INSTAURAS, AND PROPER POSITION (F. V. H., O.H.) 4.00 INSPECTION TASKS AFTER WELDING. 4.00 WELDING TECHNIQUES INSTAURAS, AND PROPER POSITION (F. V. H., O.H.) 5.00 SERVE 1. SEC. N. SPECTION TASKS AFTER WELDING. 5.00 SERVE 1. SEC. N. SPECTION TASKS AFTER WELDING. 5.00 SERVE 1. SEC. N. SPECTION TASKS AFTER WELDING. 5.00 SERVE 1. SEC. N. SPECTION TASKS AFTER WELDING. 5.00 SERVE 1. SEC. N. SPECTION TASKS AFTER WELDING. 5.00 SERVE 1. SEC. N. SPECTION TASKS AFTER WELDING. 5.00 SERVE 1. SEC. N. SPECTION TASKS AFTER WELDING. 5.00 SERVE 1. SEC. N. SPECTION TASKS AFTER WELDING. 5.00 SERVE 1. SEC. N. SPECTION TASKS AFTER WELDING. 5.00 SERVE 1. SEC. N. SPECTION TASKS AFTER WELDING. 5.00 SERVE 1. SEC. N. SPECTION TASKS AFTER WELDING. 5.00 SERVE 1. SEC. N. SPECTION TASKS AFTER WELDING. 5.00 SERVE 1. SEC. N. SPECTION TASKS AFTER WELDING. 5.00 SEC. N. SPECTION TASKS AFTER DEAD TO THE THE THREADS ARE TO BE EXCLUDED FROM SHAR	ATA
301 USE OF QUALIFIED WELDERS 302 CONTROL AND HANDLING OF WELDING CONSUMABLE PACKAGING AND EXPOSURE CONTROL 303 NO WELDING OVER CRACKED TACK WELDS 304 ENVIRONMENTAL COMDITIONS INCLIDINGS WITHIN LIMITS, PRECIPITATION, AND TEMPERATURE 305 WES POLICIANDED INCLIDING SETTINGS ON WELDING EQUIPMENT, TRAVEL SPEED, SELECTED WELDING MATERIALS, SHIELDING GAS TYPE-FLOW RATE, PREHEAT APPLIED, INTERPASS 306 WELDING TECHNIQUES INCLIDING SETTINGS ON WELDING EQUIPMENT, TRAVEL SPEED, SELECTED WELDING MATERIALS, SHIELDING GAS TYPE-FLOW RATE, PREHEAT APPLIED, INTERPASS 307 WELDING TECHNIQUES INCLIDING: INTERPASS AND FINAL CLEANING, EACH PASS WITHIN PROFILE LIMITATIONS, EACH PASS MEETS QUALITY REQUIREMENTS 308 WELDING TECHNIQUES INCLIDING: INTERPASS AND FINAL CLEANING, EACH PASS WITHIN PROFILE LIMITATIONS, EACH PASS MEETS QUALITY REQUIREMENTS 309 SERVE 300 INSPECTION TASKS AFTER WELDING: 300 SERVE 300 SERVE PERFORM 301 WELDS LEANED 302 SEZE LEASTED 303 WELDIN ARE USUAL ACCEPTANCE CRITERIA FOR: CRACK PROHIBITION, WELDISASE-METAL FUSION, CRATER CROSS SECTION, WELD PROFILES, WELD SIZE, UNDERCUT, AND POROSITY 304 PERFORM 305 WELDIS MEET VISUAL ACCEPTANCE CRITERIA FOR: CRACK PROHIBITION, WELDISASE-METAL FUSION, CRATER CROSS SECTION, WELD PROFILES, WELD SIZE, UNDERCUT, AND POROSITY 305 PERFORM 306 SECTION SERVE SHAPE AND WELD TASS REMOVED (IF REQUIRED) 307 REPERFORM 308 SECTION SERVE SHAPE AND WELD TASS REMOVED (IF REQUIRED) 308 PERFORM 309 SECTION TASKS PRICE TO BOLITING: 309 PERFORM 300 SERVE 300	ATA
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NOTES: (a) THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM BY WHICH A WELDER WHO HAS WELDED A JOINT OR MEMBER CAN BE IDENTIFIED. STAMPS, IF USED, SHALL BE THE LOW-STRESS TYPE. (b) WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 IN. (75MM) OF	

	Steel Deck			
NO.	INSPECTION TASK	FREQUENCY	REFERENCE FOR CRITERIA	AGENT
1.00	INSPECTION OR EXECUTION TASKS PRIOR TO DECK PLACEMENT:		SDI STD QA/QC TABLE 1.1	
1.01	VERIFY COMPLIANCE OF MATERIALS (DECK AND ALL DECK ACCESSORIES) WITH CONSTRUCTION DOCUMENTS, INCLUDING PROFILES, MATERIAL PROPERTIES, AND BASE METAL THICKNESS.	PERFORM		ATA
1.02	DOCUMENT ACCEPTANCE OR REJECTION OF DECK AND DECK ACCESSORIES.	PERFORM		ATA
2.00	INSPECTION OR EXECUTION TASKS AFTER DECK PLACEMENT:		SDI STD QA/QC TABLE 1.2	
2.01	VERIFY COMPLIANCE OF DECK AND ALL DECK ACCESSORIES INSTALLATION WITH CONSTRUCTION DOCUMENTS.	PERFORM		ATA
2.02	VERIFY DECK MATERIALS ARE REPRESENTED BY THE MILL CERTIFICATIONS THAT COMPLY WITH THE CONSTRUCTION DOCUMENTS.	PERFORM		ATA
2.03	DOCUMENT ACCEPTANCE OR REJECTION OF INSTALLATION OF DECK AND DECK ACCESSORIES.	PERFORM		ATA
3.00	INSPECTION OR EXECUTION TASKS PRIOR TO WELDING:		SDI STD QA/QC TABLE 1.3	
3.01	WELDING PROCEDURE SPECIFICATIONS (WPS) AVAILABLE.	OBSERVE		ATA
3.02	MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE.	OBSERVE		ATA
3.03	MATERIAL INDENTIFICATION (TYPE/GRADE).	OBSERVE		ATA
3.04	CHECK WELDING EQUIPMENT.	OBSERVE		ATA
4.00	INSPECTION OR EXECUTION TASKS DURING WELDING:		SDI STD QA/QC TABLE 1.4	
4.01	USE QUALIFIED WELDERS.	OBSERVE		ATA
4.02	CONTROL AND HANDLING OF WELDING CONSUMABLES.	OBSERVE		ATA
4.03	ENVIRONMENTAL CONDITIONS (WIND SPEED, MOISTURE, TEMPERATURE).	OBSERVE		ATA
4.04	WPS FOLLOWED.	OBSERVE		ATA
5.00	INSPECTION OR EXECUTION TASKS AFTER WELDING:		SDI STD QA/QC TABLE 1.5	
5.01	VERIFY SIZE AND LOCATION OF WELDS, INCLUDING SUPPORT, SIDELAP, AND PERIMETER WELDS.	PERFORM		ATA
5.02	WELDS MEET VISUAL ACCEPTANCE CRITERIA.	PERFORM		ATA
5.03	VERIFY REPAIR ACTIVITIES.	PERFORM		ATA
5.04	DOCUMENT ACCEPTANCE OR REJECTION OF WELDS.	PERFORM		ATA
6.00	INSPECTION OR EXECUTION TASKS PRIOR TO MECHANICAL FASTENING:		SDI STD QA/QC TABLE 1.6	
6.01	MANUFACTURER INSTALLATION INSTRUCTIONS AVAILABLE FOR MECHANICAL FASTENERS.	OBSERVE		ATA
6.02	PROPER TOOLS AVAILABLE FOR FASTENER INSTALLATION.	OBSERVE		ATA
6.03	PROPER STORAGE FOR MECHANICAL FASTENERS.	OBSERVE		ATA
7.00	INSPECTION OR EXECUTION TASKS DURING MECHANICAL FASTENING:		SDI STD QA/QC TABLE 1.7	
7.01	FASTENERS ARE POSITIONED AS REQUIRED.	OBSERVE		ATA
7.02	FASTENERS ARE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.	OBSERVE		ATA
8.00	INSPECTION OR EXECUTION TASKS AFTER MECHANICAL FASTENING:		SDI STD QA/QC TABLE 1.8	
8.01	CHECK SPACING, TYPE, AND INSTALLATION OF SUPPORT FASTENERS.	PERFORM		ATA
8.02	CHECK SPACING, TYPE, AND INSTALLATION OF SIDELAP FASTENERS.	PERFORM		ATA
8.03	CHECK SPACING, TYPE, AND INSTALLATION OF PERIMETER FASTENERS.	PERFORM		ATA
8.04	VERIFY REPAIR ACTIVITIES.	PERFORM		ATA
8.05	DOCUMENT ACCEPTANCE OR REJECTION OF MECHANICAL FASTENERS.	PERFORM		ATA
		•	 	





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AT KANNER

3301 S KANNER HIGHWAY STUART, FLORIDA 34994 THIRD SE AT K

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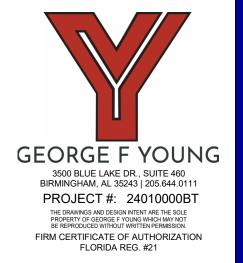
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SPECIAL INSPECTIONS





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ANNER

HIRD

Branch Foundation and Floor Plan

1/4" = 1'-0" 1. FINISH FLOOR (TOP OF SLAB) ELEVATION 0'-0", UNLESS NOTED.

2. TOP OF FOOTING: -2'-0", UNLESS NOTED.

3. FOR SLAB ON GRADE CONSTRUCTION, SEE GENERAL NOTES AND TYPICAL DETAILS. FOR SLAB EDGES NOT DIMENSIONED, SEE ARCHITECTURAL DRAWINGS.

4. FOR SLAB RECESS, SEE ARCHITECTURAL DRAWINGS. 5. GENERAL CONTRACTOR SHALL COORDINATE TILE JOINT LOCATIONS WITH CONTROL JOINTS.

6. FOOTING STEP ELEVATIONS AND LOCATION ARE APPROXIMATE. GENERAL CONTRACTOR SHALL COORDINATE ALL FOOTING STEPS WITH CIVIL, PLUMBING AND UTILITY DRAWINGS.

7. PROVIDE 2#5x6'-0" AT ALL RE-ENTRANT CORNERS AND DISCONTINUOUS CONTROL JOINTS. 8. PROVIDE ISOLATION JOINT AROUND COLUMN THROUGH SLAB AND CURB, TYPICAL AT

DRIVE-THRU. 9. BUILD MASONRY CONNECTION ACROSS WALLS AT THREE LOCATIONS.

10. AT BEAM BEARING ABOVE, PROVIDE (3) CELLS REINFORCED WITH 1 #5 EACH AND GROUTED SOLID FROM BEAM BEARING TO FOUNDATION.

11. CONTRACTOR TO COORDINATE PIPE AND CONDUIT LOCATIONS IN CMU WALLS TO BE IN UNREINFORCED CELLS ONLY.

Structural Column Schedule								
Designation	Size	Base Plate	Anchor Rods	Notes				
C1	HSS6x6x3/8	3/4x12x1'-0"	(4) 3/4"Ø, 9" EMBEDMENT					
C2	HSS8x8x3/8	3/4x14x1'-2"	(4) 3/4"Ø, 9" EMBEDMENT					

1. ALL COLUMNS ARE C1, UNLESS NOTED. 2. PROVIDE 1 1/2" EDGE DISTANCE FOR ANCHOR RODS.

Structural Foundation Schedule							
Designation	Length	Width	Depth	Bottom Reinforcement	Top Reinforcement	Notes	
F4	4' - 0"	4' - 0"	1' - 2"	5#5 EW	5#5 EW		

1. ALL FOOTINGS ARE F4, UNLESS NOTED.

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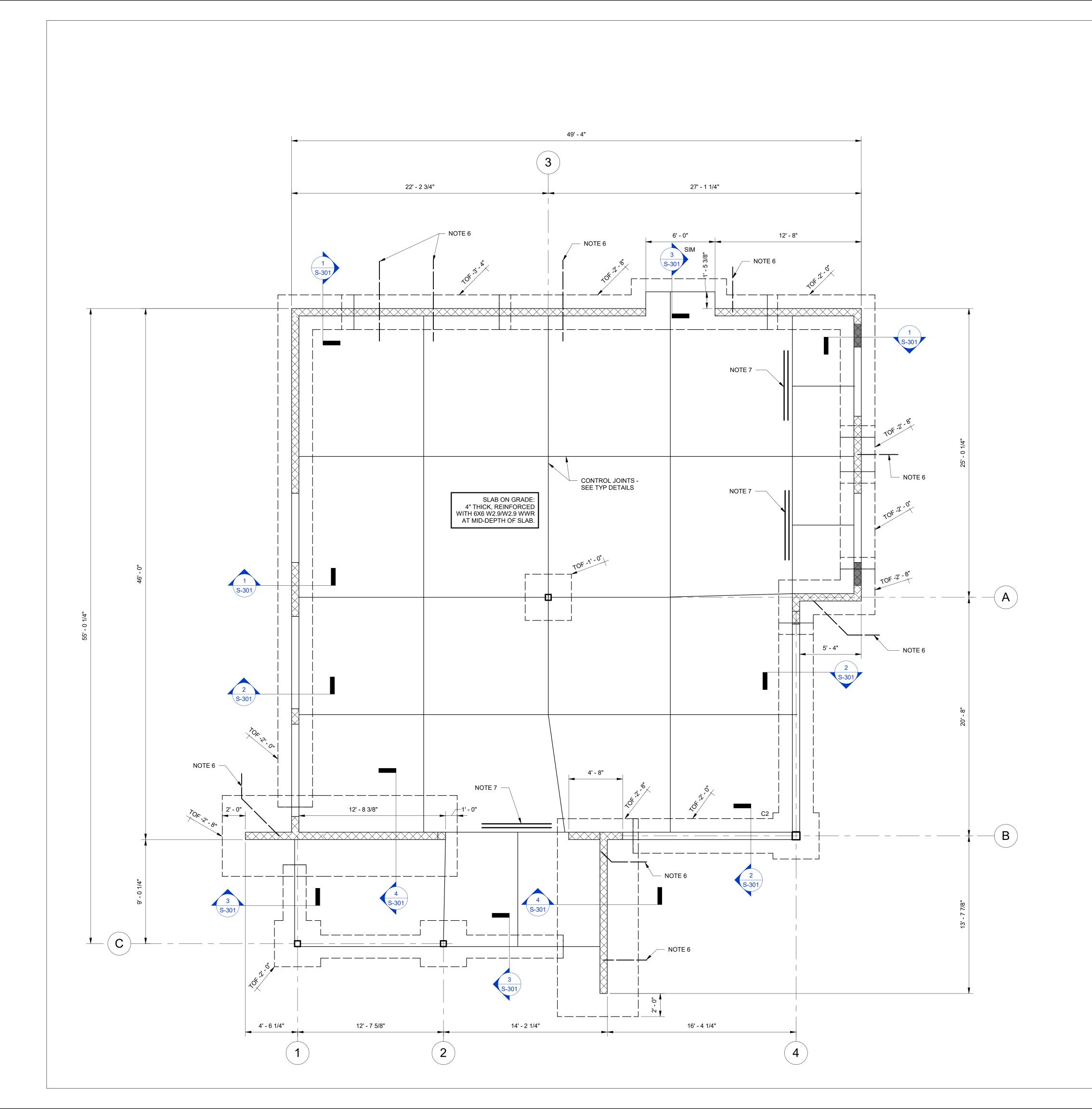
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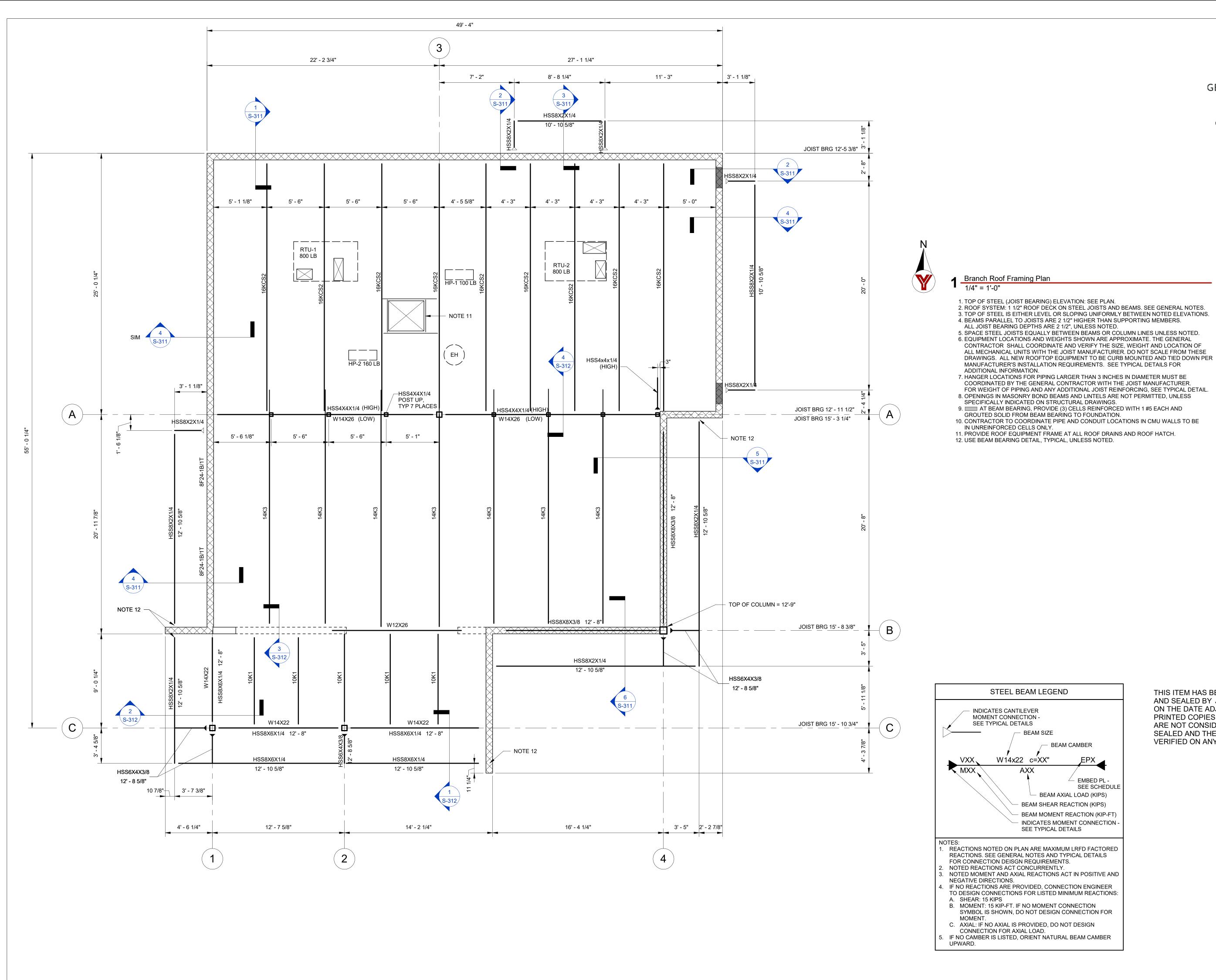
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BRANCH FOUNDATION AND FLOOR PLAN

SHEET NUMBER

S-201









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THIRD

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Branch Roof Framing Plan

1/4" = 1'-0"

- 1. TOP OF STEEL (JOIST BEARING) ELEVATION: SEE PLAN. 2. ROOF SYSTEM: 1 1/2" ROOF DECK ON STEEL JOISTS AND BEAMS. SEE GENERAL NOTES.
- 3. TOP OF STEEL IS EITHER LEVEL OR SLOPING UNIFORMLY BETWEEN NOTED ELEVATIONS. 4. BEAMS PARALLEL TO JOISTS ARE 2 1/2" HIGHER THAN SUPPORTING MEMBERS.
- ALL JOIST BEARING DEPTHS ARE 2 1/2", UNLESS NOTED. 5. SPACE STEEL JOISTS EQUALLY BETWEEN BEAMS OR COLUMN LINES UNLESS NOTED. 6. EQUIPMENT LOCATIONS AND WEIGHTS SHOWN ARE APPROXIMATE. THE GENERAL CONTRACTOR SHALL COORDINATE AND VERIFY THE SIZE, WEIGHT AND LOCATION OF ALL MECHANICAL UNITS WITH THE JOIST MANUFACTURER. DO NOT SCALE FROM THESE
- ADDITIONAL INFORMATION. 7. HANGER LOCATIONS FOR PIPING LARGER THAN 3 INCHES IN DIAMETER MUST BE
- COORDINATED BY THE GENERAL CONTRACTOR WITH THE JOIST MANUFACTURER. FOR WEIGHT OF PIPING AND ANY ADDITIONAL JOIST REINFORCING, SEE TYPICAL DETAIL. 8. OPENINGS IN MASONRY BOND BEAMS AND LINTELS ARE NOT PERMITTED, UNLESS
- SPECIFICALLY INDICATED ON STRUCTURAL DRAWINGS. 9. AT BEAM BEARING, PROVIDE (3) CELLS REINFORCED WITH 1 #5 EACH AND
- GROUTED SOLID FROM BEAM BEARING TO FOUNDATION. 10. CONTRACTOR TO COORDINATE PIPE AND CONDUIT LOCATIONS IN CMU WALLS TO BE

STEEL BEAM LEGEND

- BEAM SIZE

W14x22 c=XX"

BEAM CAMBER

BEAM AXIAL LOAD (KIPS)

BEAM SHEAR REACTION (KIPS) BEAM MOMENT REACTION (KIP-FT) INDICATES MOMENT CONNECTION -

SEE TYPICAL DETAILS

- EMBED PL -

SEE SCHEDULE

INDICATES CANTILEVER

IN UNREINFORCED CELLS ONLY. 11. PROVIDE ROOF EQUIPMENT FRAME AT ALL ROOF DRAINS AND ROOF HATCH. 12. USE BEAM BEARING DETAIL, TYPICAL, UNLESS NOTED.

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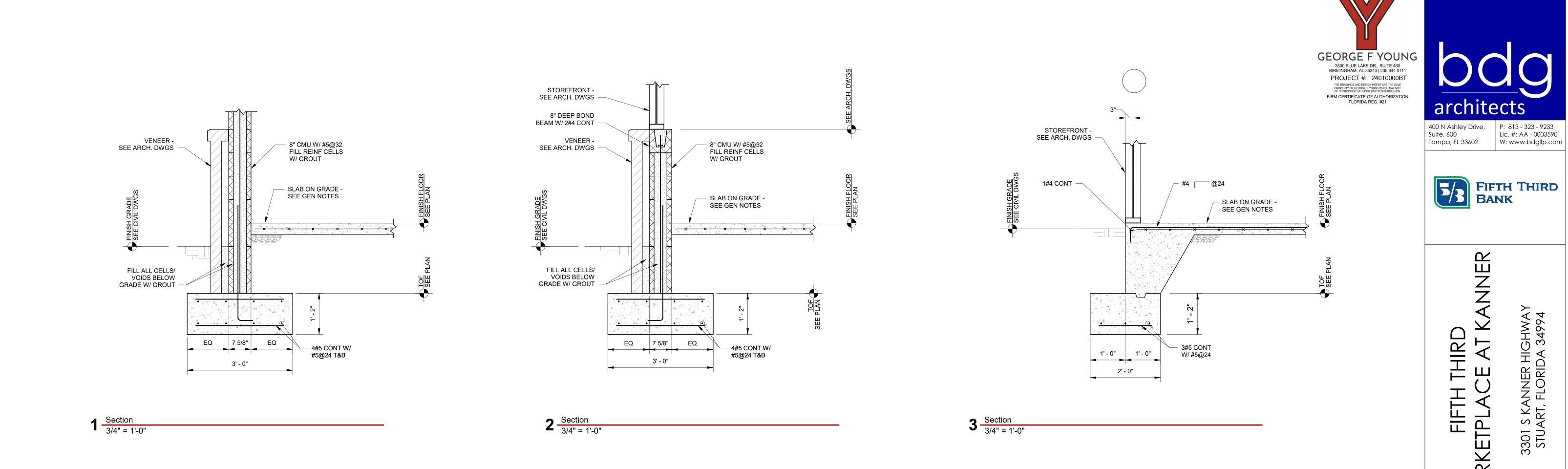
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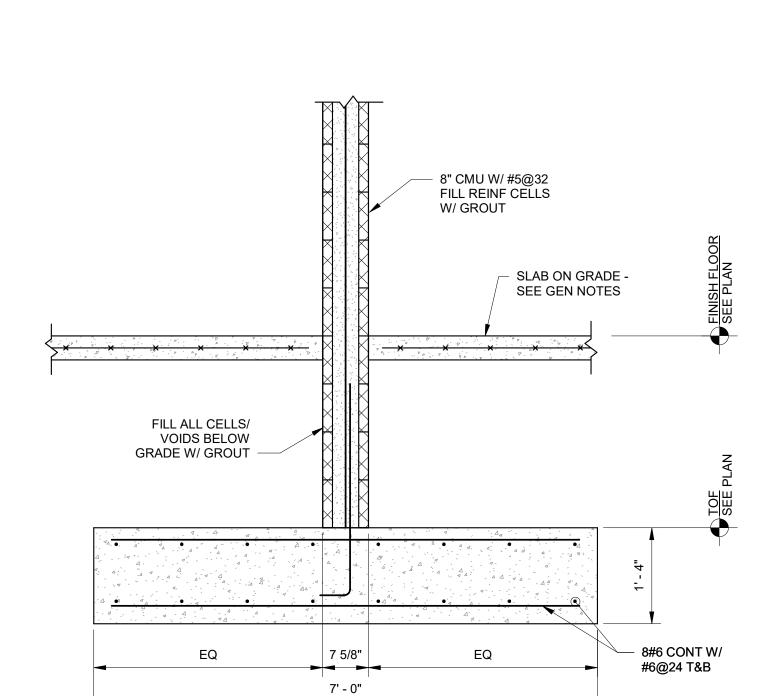
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BRANCH ROOF FRAMING PLAN

3/24/25

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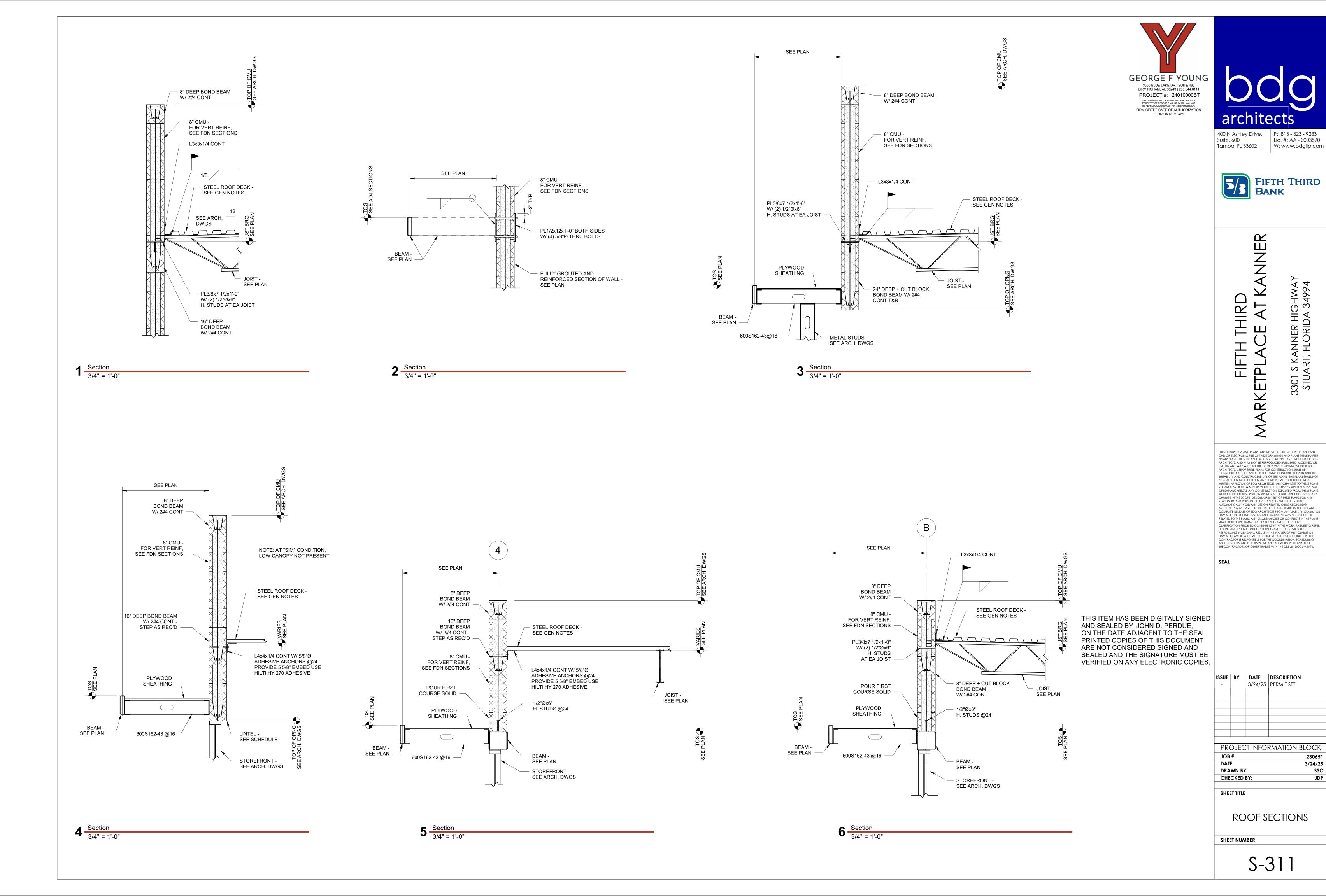
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FOUNDATION SECTIONS



230651

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SEE PLAN

— 8" DEEP + CUT BLOCK BOND BEAM W/ 2#4 CONT T&B



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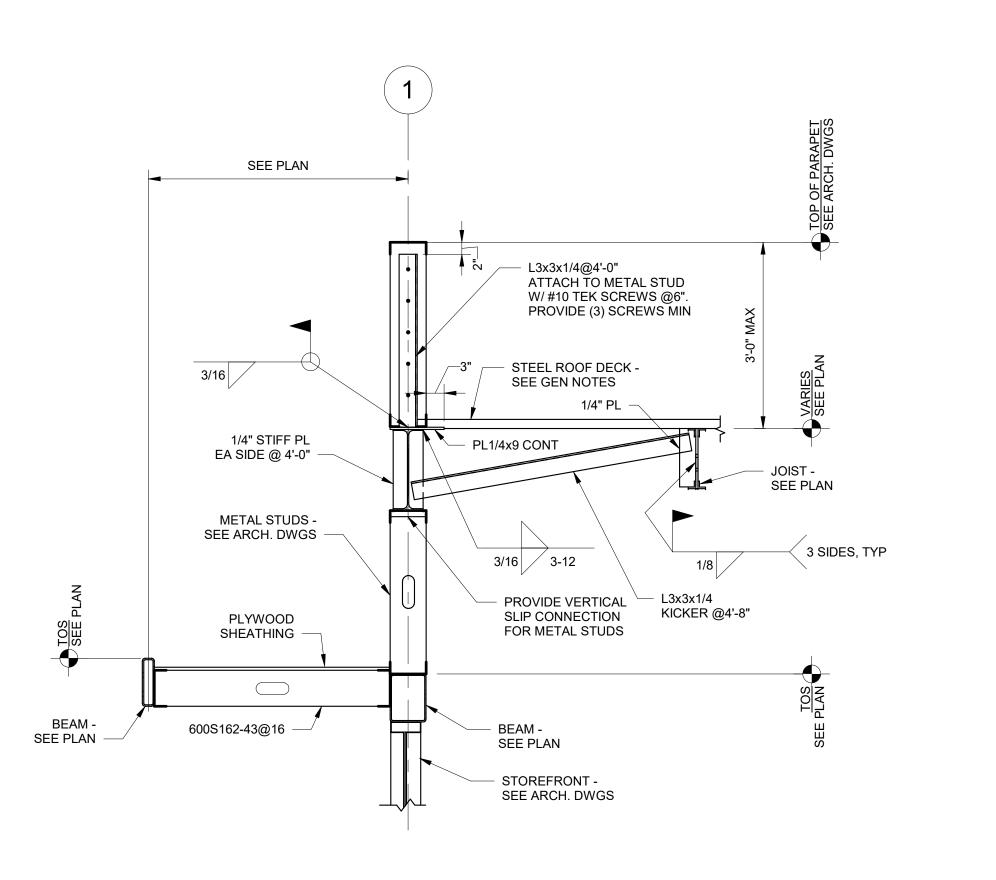
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ROOF SECTIONS

JDP

SHEET NUMBER

S-312





TOP OF PARAPET SEE ARCH. DWGS

- L3x3x1/4@4'-0" ATTACH TO METAL STUD

W/#10 TEK SCREWS @6". PROVIDE (3) SCREWS MIN

STEEL ROOF DECK -

JOIST -

SEE PLAN

SEE GEN NOTES

L3x3x1/4 CONT

L5x3 1/2x5/16 LLV CONT BTWN JOISTS

KICKER @4'-8"

PROVIDE VERTICAL

SLIP CONNECTION

FOR METAL STUDS

L3x3x1/4

SEE PLAN

STOREFRONT -SEE ARCH. DWGS



4 Section 3/4" = 1'-0"

SEE PLAN

3/16

SEE PLAN —

1/4" STIFF PL

EA SIDE @ 4'-0"

METAL STUDS -

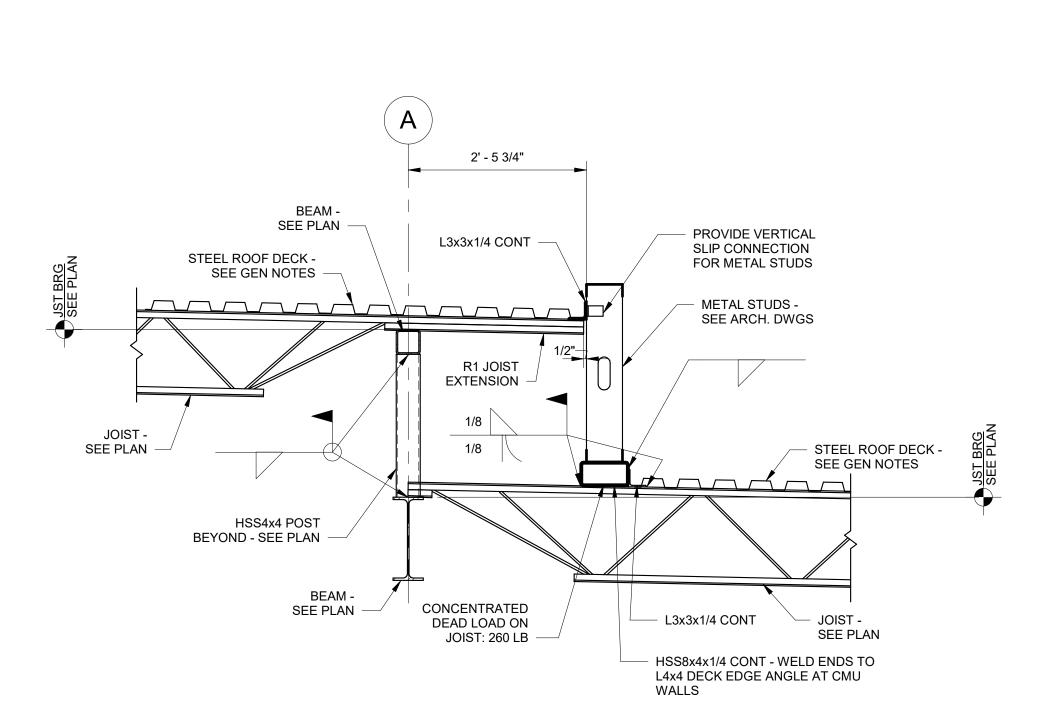
SEE ARCH. DWGS -

600S162-43 @16

TOS

BEAM -SEE PLAN PLYWOOD

SHEATHING -



Section 3/4" = 1'-0"

STEEL ROOF DECK -SEE GEN NOTES -

PL3/8x7 1/2x1'-0"

8" CMU -FOR VERT REINF,

SEE FDN SECTIONS

W/ (2) 1/2"Øx6" H. STUDS AT EA JOIST

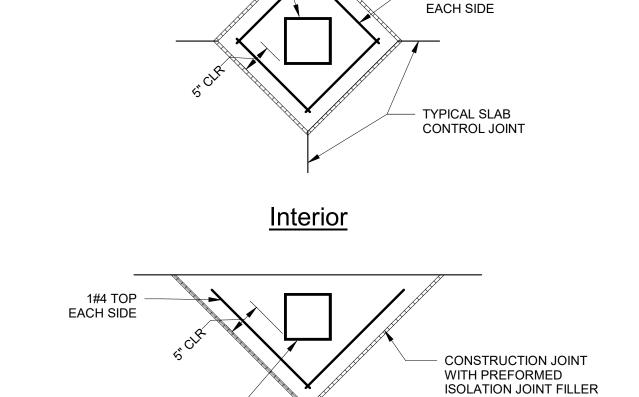
JOIST -

SEE PLAN

HSS Column Base and Footing Detail N.T.S.

COLUMN

COLUMN



CONSTRUCTION JOINT WITH

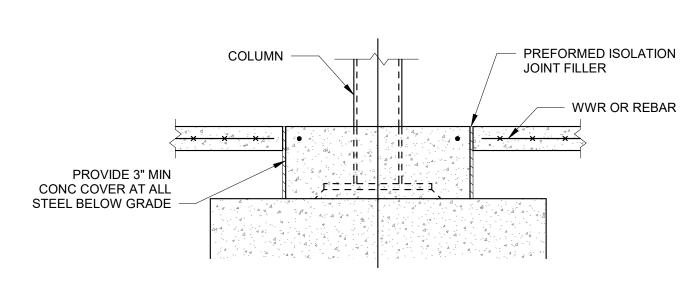
PREFORMED ISOLATION

JOINT FILLER

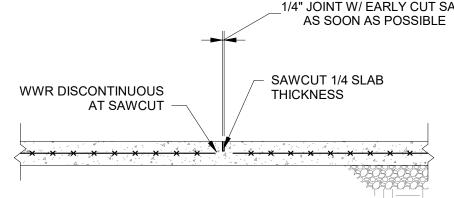
Isolation Joint Detail

Exterior

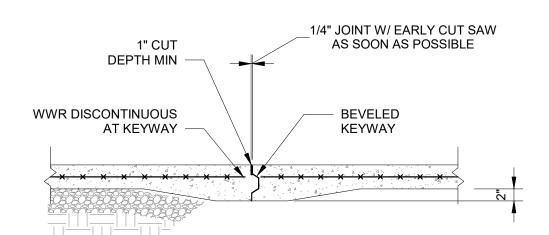
TYPICAL SLAB CONTROL JOINT



Section at Column



Control Joint

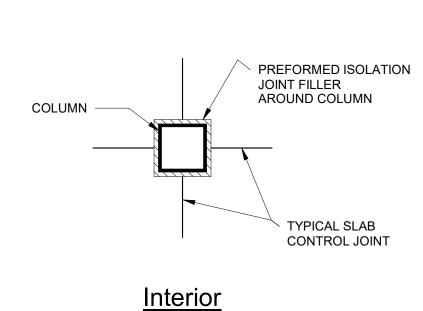


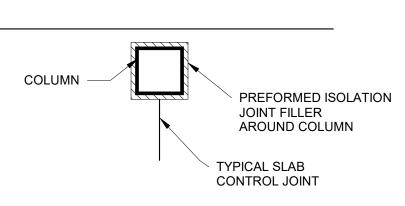
Construction Joint

1. CONTRACTOR'S OPTION TO USE EITHER JOINT TYPE. 2. CONTROL JOINT FILLER SEMI-RIGID EPOXY. 3. SAWCUT TO TAKE PLACE WITHIN 4-12 HOURS OF FINISHING CONCRETE: 4 HOURS IN HOT WEATHER,

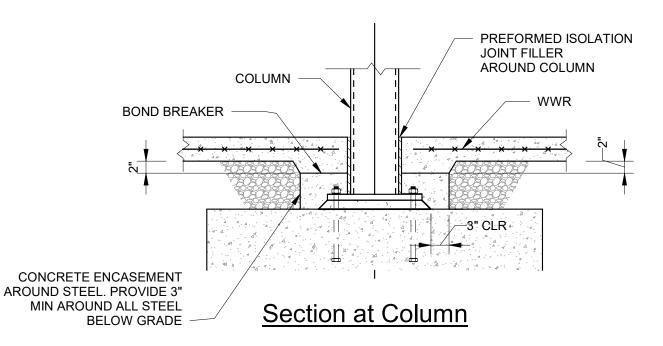
12 HOURS IN COLD WEATHER.

2 Slab Control Joint Details N.T.S.



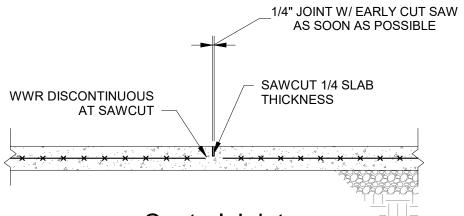


Exterior Isolation Joint Detail



4 Column/Slab-Joint Details N.T.S.

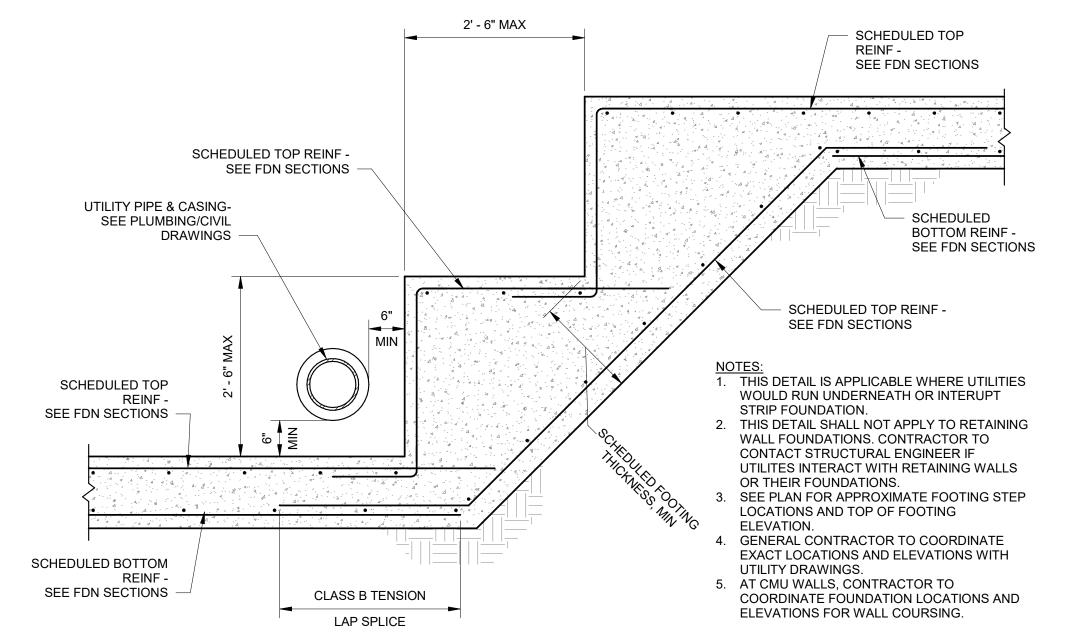
CONTRACTOR'S OPTION TO USE EITHER JOINT DETAIL



Tension Lap Splice Lengths BAR SIZE TOP BARS OTHER BARS #3 22" 29" 36" #5 43" 63" #7 72"

1. TOP BARS ARE HORIZONTAL REINFORCEMENT WITH MORE THAN 12" OF CONCRETE CAST BELOW THE REINFORCEMENT.

3 Concrete Tension Lap Splice Lengths N.T.S.



5 Stepped Strip Footing Detail N.T.S.

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GEORGE F YOUNG 3500 BLUE LAKE DR., SUITE 460 BIRMINGHAM, AL 35243 | 205.644.0111 PROJECT #: 24010000BT FIRM CERTIFICATE OF AUTHORIZATION FLORIDA REG. #21 architects

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DATE:	3/24/25
DRAWN BY:	SSC
CHECKED BY:	JDP

SHEET TITLE

TYPICAL DETAILS

SHEET NUMBER

S-401

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3/24/25

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TYPICAL DETAILS

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INTERMEDIATE SUPPORT MATCH SIDE LAP NUMBER AT PERIMETER INTERMEDIATE SUPPORT - PUDDLE WELDS AT END AND INTERMEDIATE SUPPORTS (5)#10 TEK SCREW SIDE LAPS -END SUPPORT 36/5 PATTERN 5/8"Ø PUDDLE WELD

3-SIDES NOTE 3

1 1/2" Type B Roof Deck Attachment Detail N.T.S.

FULL DEPTH CONNECTION FULL DEPTH CONNECTION PER AISC PER AISC SINGLE SHEAR PLATE SINGLE SHEAR PLATE 3/8"x5"xL 3/8"x5"xL COLUMN -SEE SCHEDULE — **ERECTION SEAT** (OPTIONAL) Beam to Column Beam to Beam

3 HSS Beam - HSS Column Moment Connection N.T.S.

3/4" CAP PLATE

1. SEE PLAN FOR MOMENT CONNECTION LOCATIONS. 2. INCREASE FILLET WELD SIZE FOR GAP.

MAXIMUM GAP PERMITTED = 3/16". 3. OPTIONAL GROOVE WELD AS REQUIRED BY LOADING. USE FLARE-BEVEL GROOVE WELD (BTC-P10) ON VERTICAL

SIDES IF WIDTHS OF BEAM AND COLUMN ARE EQUAL. 4. CONNECTION DESIGNER TO CHECK CONCENTRATED LOADS ON SUPPORTING MEMBER & SIZE WELDS.

4 Beam/Column and Beam/Beam Connection Details N.T.S.

6 HSS Beam - HSS Column Connection N.T.S. NOTES: 1. AT HSS BEAMS DEEPER THAN 8", PROVIDE L4x4 AT TOP AND BOTTOM. IF COLUMN DOES NOT EXTEND ABOVE BEAM, USE ALTERNATIVE.

1/2" MAX.-PL1/4x4x1'-0" CAP PL L4x4x1/4x0'-4" - L4x4x1/4x0'-4" **Typical Alternative**

JOIST MANUFACTURER TO VERIFY 3" DIMENSION AND MAXIMUM

- CL PANEL POINT

CONCENTRATED

3" OR MORE-

LOAD

L2x2x1/4 (EXTEND FROM LOAD TO NEAREST PANEL

POINT ON OPPOSITE CHORD) -

CONCENTRATED

JOIST —— SEE PLAN

3/16

2 Joist Reinforcement Detail N.T.S.

CONCENTRATED LOAD FOR TOP AND BOTTOM CHORD REINFORCEMENT REQUIREMENTS. GENERAL CONTRACTOR

AND JOIST MANUFACTURER TO COORDINATE

PROVIDE DETAILS ON JOIST SHOP DRAWINGS.

CL PANEL POINT

∕─3" OR MORE

DISTANCE AND CONCENTRATED LOAD AND



RTU CURB -SEE MECHANICAL DWGS

STEEL DECK -

SEE PLAN

- STEEL JOIST -SEE PLAN



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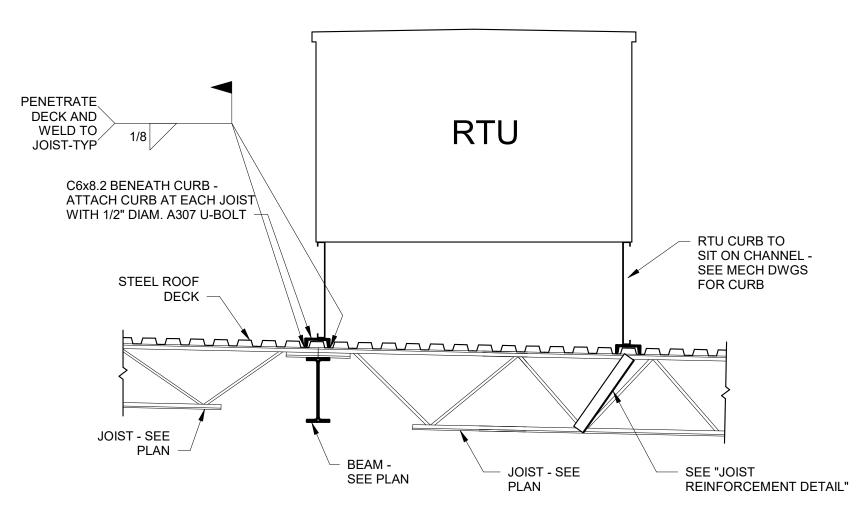
DATE: 3/24/25 **DRAWN BY:** SSC CHECKED BY:

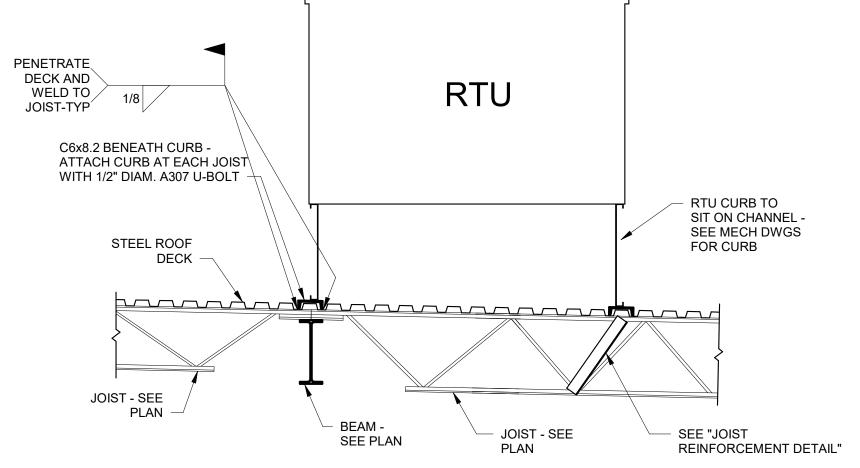
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SHEET TITLE

TYPICAL DETAILS

SHEET NUMBER









S-403

DWGS

BEAM

DUCT OPENING -

SEE MECHANICAL

CHANNEL BENEATH CURB AT EDGE OF RTU SEE SECTION

CHANNEL BENEATH CURB AT EDGE OF

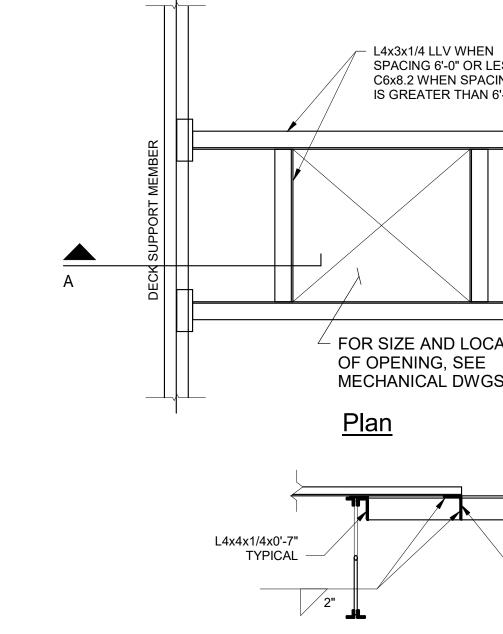
RTU SEE SECTION

PLAN

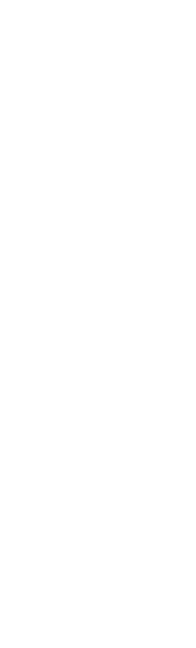
SEE "ROOF OPENING FRAME

DETAIL" FOR STEEL ANGLE REINF AT DUCT OPENING









A. AT METAL ROOF DECK, USE #10 TEK SCREWS @ 6" SPACING.

VARIES, CONTACT STRUCTURAL ENGINEER.

1. SECURE CURB TO ROOF AS NOTED BELOW. IF ROOF CONSTRUCTION

RTU

DUCT

OPENING

WOOD NAILER

CURB

PREMANUFACTURERED

STEEL ANGLES -

FRAME DETAIL" -

3 Section N.T.S.

SEE "ROOF OPENING

6 Equipment Support Curb N.T.S.

COUNTER FLASHING

ROOFING TO EXTEND OVER

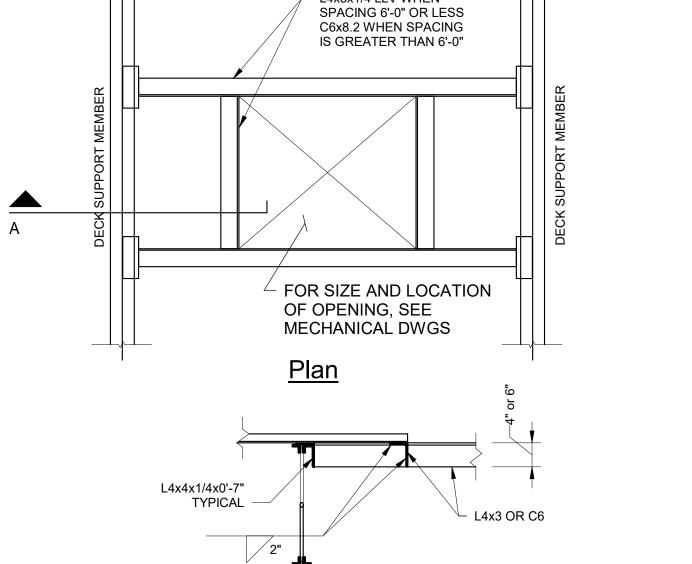
INSULATION ON METAL DECK -

NOTES:

CURB

		٦
STEEL ANGLES - SEE "ROOF OPENING FRAME DETAIL"	RTU	
CHANNEL BENEATH CURB SEE SECTION —		RTU CURB- SEE MECHANICAL DWGS — STEEL DECK
JOIST - SEE PLAN BEAM - SEE PLAN		JOIST - SEE PLAN

1	Section				
4	N.T.S.				



SPACING



STANDARD BLOCK

REINF INTO LINTEL.

PROVIDE 90° HOOK

PRECAST U-BLOCK

BOTTOM REINF

EXTEND WALL

AT ENDS

F = FILLED WITH GROUT

QUANTITY OF #5 FIELD

BOTTOM OF U-LINTEL

- QUANTITY OF #5

FIELD ADDED REBAR

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AT TOP OF U-LINTEL

ADDED REBAR AT

U = UNFILLED

PRECAST LINTEL NAMING CONVENTION

TOP REINF

EXTEND WALL

BOTTOM REINF

NOMINAL WIDTH

NOMINAL

DEPTH

AT ENDS

REINF INTO LINTEL.

PROVIDE 90° HOOK



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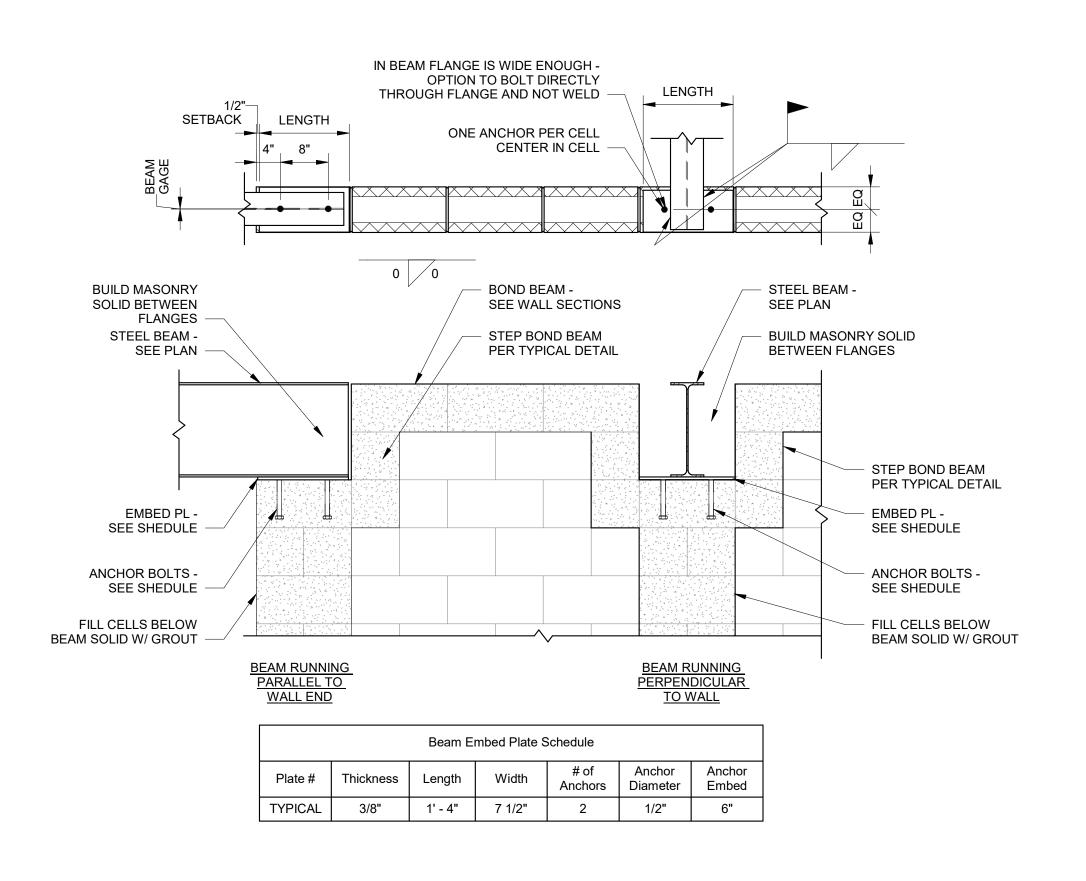
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TYPICAL DETAILS

SHEET NUMBER



Beam Embed Plate Schedule N.T.S.

Width

6'-0"

12'-0"

Wall

Thickness

8" BRG

MIN

CMU Lintel Schedule

Lintel

Depth

24"

* USE REINFORCEMENT AS INDICATED BY SPECIFIED PRECAST LINTEL.

Bottom

Reinf

2#4

Top Reinf

LINTEL -

SEE SCHEDULE

SEE SCHEDULE

MATCH SIZE IN WALL

Jamb

2 Cells

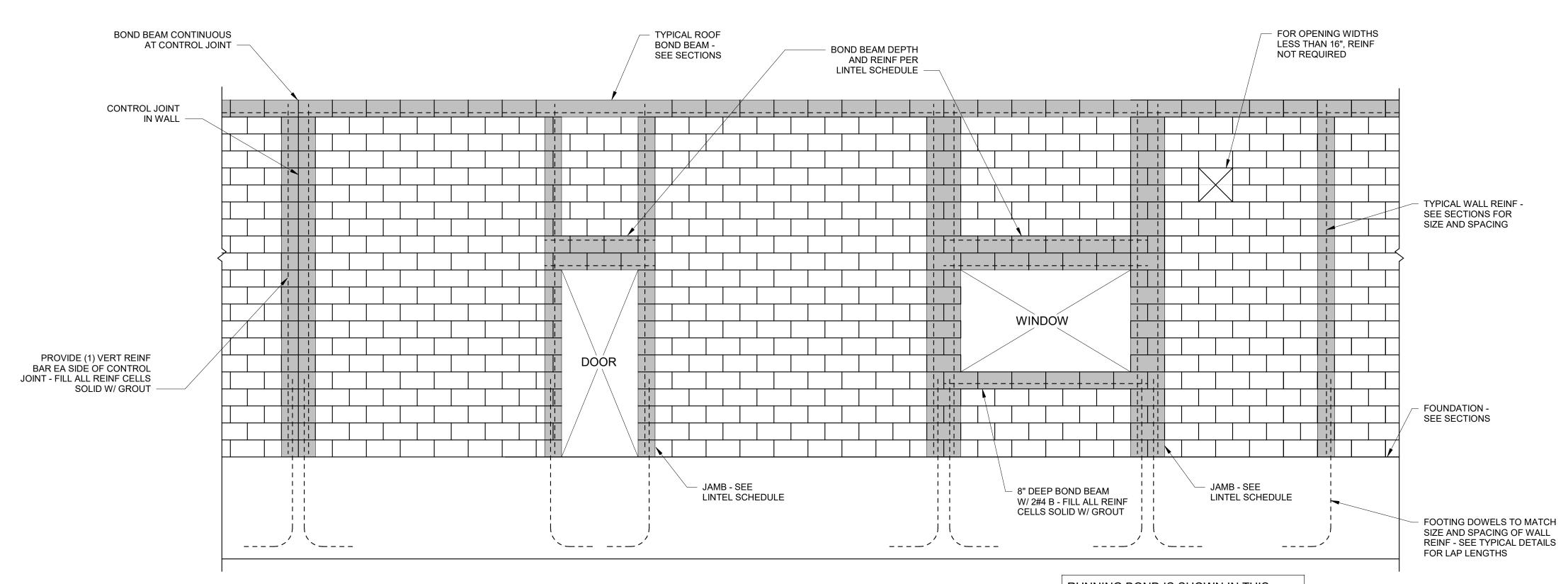
3 Cells

Precast Lintel

8F24-1B/1T

JAMB REINF -

SCHEDULED REINF 90° HOOK @ ENDS



RUNNING BOND IS SHOWN IN THIS DETAIL FOR ILLUSTRATION PURPOSES ONLY. REFER TO GENERAL NOTES FOR JOB SPECIFIC BOND PATTERN.

3 Door and Window Opening Reinforcement at Load Bearing Masonry Walls N.T.S.



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SHEET NUMBER

 FACTORY FABRICATED CORNER SECTION OF HORIZONTAL JOINT REINFORCEMENT CONTINUOUS HORIZONTAL JOINT VERTICAL REINFORCEMENT IN GROUTED CELLS -REINFORCEMENT CONT VERTICAL REINFORCEMENT IN GROUTED CELLS AT TYPICAL SPACING SEE GEN NOTES FOR LAP CONCRETE MASONRY WALL CONTINUOUS HORIZONTAL JOINT REINFORCEMENT

Plan Showing Joint Reinforcement at Wll Corner N.T.S.

Masonry Tension
Lap Splice Lengths

f'm = 2000

18"

20"

38"

52"

1. THIS TABLE CONTAINS DEVELOPMENT AND SPLICE

2. FOR DEVELOPMENT AND SPLICE LENGTHS OF EPOXY COATED BARS, MULTIPLY TABULATED VALUES BY 1.5.

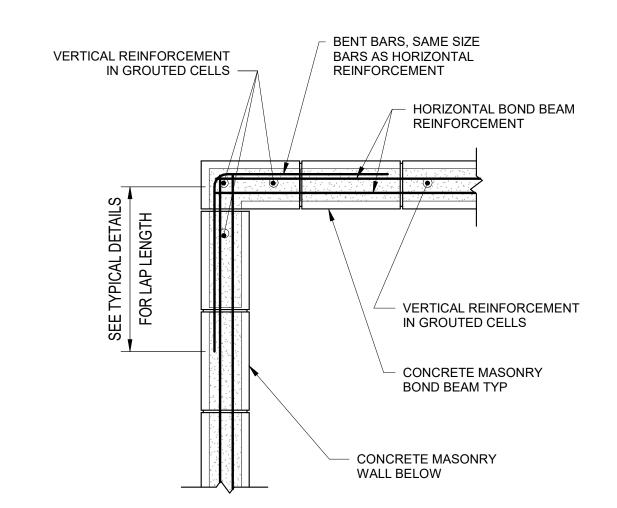
3. WHERE VERTICAL BAR IS INTERRUPTED BY A BEARING

DOWEL MATCHING VERTICAL REINFORCEMENT W/8"

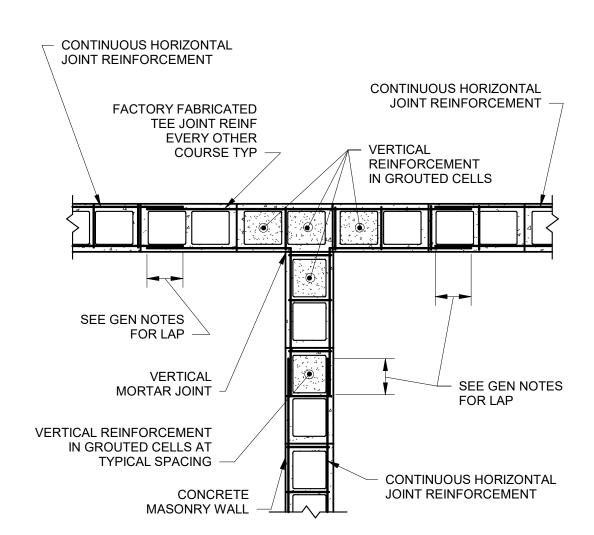
PLATE OR OTHER OBSTRUCTION, PROVIDE BACK

TO ACI 530-13/TMS 402-13/ASCE 5-13.

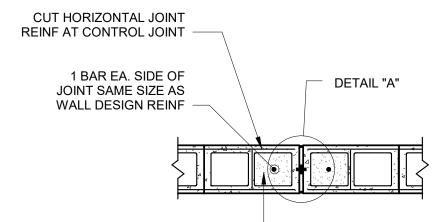
LENGTHS FOR REINFORCEMENT IN CMU ACCORDING



Plan Showing Bond Beam Reinforcement at Wall Corner N.T.S.

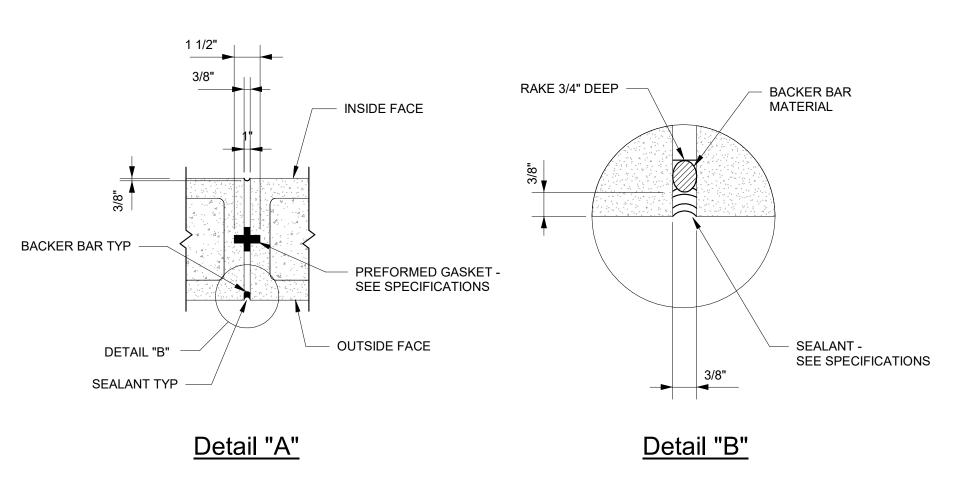


Plan Showing Joint Reinforcement at Structural Wall Intersection N.T.S.



GROUT REINF CELLS SOLID ENTIRE HEIGHT OF WALL

SEE ARCHITECTURAL DRAWINGS FOR CONTROL JOINT LOCATIONS. CONTROL JOINT SPACING NOT TO EXCEED 25'-0" OR 1.5 TIMES THE WALL HEIGHT, WHICHEVER IS LESS.



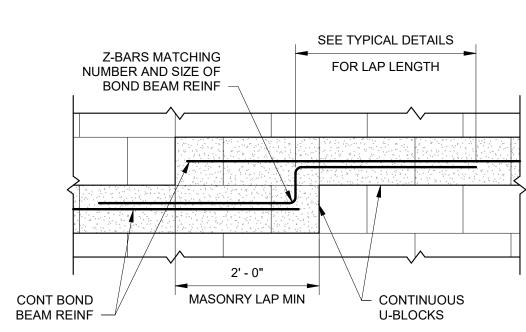
Masonry Control Joint N.T.S.

4 Masonry Tension Lap Splice Lengths N.T.S.

MAXIMUM OFFSET.

NOTES:

#5



6 Bond Beam Step Detail
N.T.S.