

General Notes

GN. GENERAL

GN.1	THE STRUCTURAL DRAWINGS AND SPECIFICATIONS ARE A PORTION OF THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR AND SUBCONTRACTORS SHALL REFERENCE AND COORDINATE WITH ALL OTHER DISCIPLINES' DRAWINGS. ANY DISCREPANCIES OR OMISSIONS SHALL BE REPORTED TO THE STRUCTURAL ENGINEER AND ARCHITECT.
GN.2	DESIGN CRITERIA: <div><div>A. CODES AND SPECIFICATIONS:<div><div>1. GENERAL BUILDING CODE:<div>FLORIDA BUILDING CODE, 2020 (7TH EDITION).</div></div><div>2. DESIGN LOAD CRITERIA:<div>MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, AMERICAN SOCIETY OF CIVIL ENGINEERS, ASCE 7</div></div><div>3. CONCRETE:<div>BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, AMERICAN CONCRETE INSTITUTE, ACI 318.</div></div><div>4. STRUCTURAL STEEL:<div>SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, AMERICAN INSTITUTE OF STEEL CONSTRUCTION, AISC 360.</div></div><div>5. STEEL JOISTS:<div>STANDARD SPECIFICATIONS, LOAD TABLES AND WEIGHT TABLES FOR STEEL JOISTS AND JOIST GIRDERS, STEEL JOIST INSTITUTE, SJI.</div></div><div>6. STEEL DECK:<div>STEEL DECK INSTITUTE DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS, ROOF DECKS AND CELLULAR METAL FLOOR DECK WITH ELECTRICAL DISTRIBUTION.</div></div><div>7. COLD-FORMED METAL FRAMING:<div>NORTH AMERICAN SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, AMERICAN IRON AND STEEL INSTITUTE.</div></div><div>8. MASONRY:<div>BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES, TMS 402/ACI 530/ASCE 5. SPECIFICATION FOR MASONRY STRUCTURES, TMS 602/ACI 530.1/ASCE 6.</div></div></div></div><div>B. DESIGN LOADS (PSF):<div>RISK CATEGORY-----II<div><div>1. DEAD LOADS:<div>ANY CHANGES IN CONSTRUCTION MATERIALS FROM THOSE SHOWN ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS SHALL BE REPORTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER FOR VERIFICATION OF LOAD-CARRYING CAPACITY OF THE STRUCTURE.</div></div><div>2. LIVE LOADS:<div>ROOF (REDUCIBLE)-----20</div><div>LIVE LOAD REDUCTIONS HAVE BEEN APPLIED IN ACCORDANCE WITH THE BUILDING CODE, UNLESS NOTED.</div></div><div>3. WIND LOADS:<div>ULTIMATE DESIGN WIND SPEED, VuIt-----161 MPH (3 – SECOND GUST)<div>NOMINAL DESIGN WIND SPEED, Vasd-----125 MPH (3 – SECOND GUST)<div>WIND EXPOSURE CATEGORY-----B INTERNAL PRESSURE COEFFICIENT-----.0.18</div></div><div>WALL COMPONENT AND CLADDING WIND PRESSURE-SEE DRAWINGS</div></div><div>4. SEISMIC LOADS:<div>SEISMIC IMPORTANCE FACTOR (Ie)-----1.0</div><div>MAPPED SPECTRAL RESPONSE ACCELERATIONS:<div>Ss-----0.048 S1-----0.027</div><div>ASSUMED SITE CLASS-----D (DEFAULT)<div>DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETERS:<div>Sds-----0.052 SD1-----0.043</div></div><div>SEISMIC DESIGN CATEGORY-----A BASIC SEISMIC-FORCE-RESISTING SYSTEM:<div>EXEMPT – CONFORMS TO ASCE 7-16 SECTION 1.4.</div></div><div>REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR SEISMIC SUPPORT AND ATTACHMENT REQUIREMENTS FOR UTILITIES.</div></div><div>5. SNOW LOAD:<div>GROUND SNOW LOAD (Pg)-----0.0</div></div><div>6. RAIN LOADS:<div>DESIGN RAIN LOAD-----15 PSF RAIN INTENSITY-----4.43 IN/HR</div></div></div></div></div></div></div></div></div>
GN.3	CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS PRIOR TO FABRICATION/CONSTRUCTION. NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES PRIOR TO FABRICATION/CONSTRUCTION.

GN.4 SPECIAL INSPECTIONS/STRUCTURAL ENGINEER'S SITE VISITS:

A.	SPECIAL INSPECTIONS ARE REQUIRED FOR THIS PROJECT IN ACCORDANCE WITH INTERNATIONAL BUILDING CODE. REFER TO DRAWINGS.
B.	SITE VISITS BY STRUCTURAL ENGINEER: <div><div>1. STRUCTURAL ENGINEER'S SITE VISITS ARE FOR VISUAL OBSERVATION OF THE IN-PLACE STRUCTURE FOR GENERAL CONFORMANCE TO THE APPROVED CONSTRUCTION DOCUMENTS AT THE TIME OF THE OBSERVATION.</div><div>2. CONTRACTOR SHALL NOTIFY STRUCTURAL ENGINEER AND ARCHITECT, PER THE SCHEDULE STATED BELOW, WHEN SUCH ITEMS HAVE PROGRESSED TO THE POINT WHERE THEY WILL BE IN PLACE AND READY FOR REVIEW. FAILURE TO NOTIFY MAY REQUIRE REMOVAL OF COMPLETED CONSTRUCTION.</div><div>NOTIFY PRIOR TO THE REQUIRED DAYS FOLLOWING SCHEDULED TASKS NOTIFICATION</div><div>COVERING METAL ROOF DECK-----10 DAYS</div></div>
C.	SITE VISITS BY THE STRUCTURAL ENGINEER'S OFFICE DO NOT REPLACE INSPECTIONS AND TESTING BY THE TESTING AGENCY OR SPECIAL INSPECTOR.
GN.5	SUBMITTALS: <div><div>A. REVIEW OF SHOP DRAWINGS AND OTHER SUBMITTALS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL ENGINEER. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. ALL SHOP DRAWINGS MUST BE REVIEWED AND "APPROVED" BY THE CONTRACTOR PRIOR TO SUBMITTAL.</div><div>B. ELECTRONIC SHOP DRAWING SUBMITTALS: SUBMIT ALL ELECTRONIC SHOP DRAWINGS IN .PDF FORMAT. REVIEWED SHOP DRAWINGS WILL BE RETURNED IN .PDF FORMAT. ALL PRINTS REQUIRED BY THE CONTRACTOR ARE THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE MADE AFTER APPROVED SHOP DRAWINGS ARE RETURNED.</div><div>C. RESUBMITTED SHOP DRAWINGS: RESUBMITTED SHOP DRAWINGS SHALL HAVE ALL CHANGES SINCE THE PREVIOUS SUBMISSION IDENTIFIED BY CLOUDING OR OTHER CLEAR COMMUNICATION. RE-REVIEWED SHOP DRAWINGS WILL ONLY BE REVIEWED FOR IDENTIFIED CHANGES.</div><div>D. SHOP DRAWINGS: THE CONTRACTOR SHALL SUBMIT FOR STRUCTURAL ENGINEER REVIEW SHOP DRAWINGS FOR THE FOLLOWING ITEMS.<div><div>1. CONCRETE MIX DESIGNS</div><div>2. CONCRETE REINFORCING</div><div>3. STRUCTURAL STEEL</div><div>4. STEEL JOIST</div><div>5. STEEL DECK</div><div>6. COLD-FORMED METAL STUD DETAILS AND PLAN LAYOUT</div><div>7. MASONRY MORTAR MIX DESIGNS</div><div>8. MASONRY GROUT MIX DESIGNS</div><div>9. MASONRY REINFORCING</div></div></div></div>
GN.6	ALL DETAILS SHOWN ARE TYPICAL. SIMILAR DETAILS APPLY TO SIMILAR CONDITIONS, UNLESS NOTED.
GN.7	THE CONTRACTOR IS RESPONSIBLE FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
GN.8	CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED FLOORS/ROOFS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT LOADS DO NOT EXCEED THE DESIGN LIVE LOAD.
FD. FOUNDATION	
FD.1	A GEOTECHNICAL ENGINEER, EMPLOYED BY THE CONTRACTOR, SHALL PROVIDE COMPACTED FILL REQUIREMENTS FOR THE BUILDING PAD AND REVIEW THE FOUNDATION BEARING SURFACE TO VERIFY THE BASIS OF DESIGN BEARING PRESSURE NOTED. DO NOT PLACE CONCRETE PRIOR TO GEOTECHNICAL ENGINEER'S APPROVAL.
FD.2	ASSUMED DESIGN BEARING PRESSURES:-----2000 PSF
FD.3	ALL FOUNDATION BEARING SURFACES SHALL BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE TO ENSURE COMPLIANCE WITH PRESSURES NOTED. THE FINAL BEARING ELEVATIONS MAY VARY AS REQUIRED TO PROVIDE PROPER BEARING CAPACITY IN AN APPROVED BEARING STRATUM AS DETERMINED BY THE GEOTECHNICAL ENGINEER.
FD.4	FOOTINGS SHALL BE PLACED THE SAME DAY AS INSPECTION BY THE GEOTECHNICAL ENGINEER UNLESS EXTENDED TIME IS APPROVED BY THE GEOTECHNICAL ENGINEER.
FD.5	FOOTINGS SHALL BE NEATLY EXCAVATED WHERE POSSIBLE WITH SIDES AND TOP EDGES FREE OF LOOSE OR WET MATERIALS. WHERE NEAT EXCAVATION IS NOT POSSIBLE, FOOTING EXCAVATION SHALL BE FILLED WITH CONCRETE TO THE TOP OF FOOTING. THE BOTTOM EXCAVATION SHALL BE CLEAN AND DRY WITH ALL LOOSE MATERIAL REMOVED FOR AN ESSENTIALLY FLAT BEARING SURFACE. WHERE SOFT OR UNSUITABLE BEARING SURFACES ARE ENCOUNTERED, THE AREA SHALL BE UNDERCUT AS REQUIRED AND REPLACED WITH LEAN CONCRETE OR COMPACTED DENSE GRADED CRUSHED STONE AS DIRECTED BY THE GEOTECHNICAL ENGINEER.

FD.6	COMPACTED FILL SHALL MEET THE REQUIREMENTS NOTED IN THE GEOTECHNICAL REPORT. EXCAVATED MATERIAL MAY BE USED AS BACKFILL MATERIAL WITH WRITTEN APPROVAL FROM THE GEOTECHNICAL ENGINEER STATING THAT SUCH MATERIAL IS SUITABLE AS BACKFILL AND INSTRUCTIONS ARE GIVEN FOR PROPER MOISTURE CONTENT AND COMPACTION.
FD.7	PROVIDE 4" OF COMPACTED GRANULAR FILL BENEATH ALL SLABS ON GRADE. PROVIDE 10 MIL VAPOR RETARDER BETWEEN BOTTOM OF SLAB AND TOP OF GRANULAR FILL.
FD.8	FOUNDATIONS SHALL BE CENTERED ABOUT COLUMN LINES, UNLESS NOTED.
GN. CONCRETE	
GN.1	CONCRETING OPERATIONS SHALL COMPLY WITH ACI STANDARDS.
GN.2	MINIMUM CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS (PSI), TYPE OF CONCRETE, MAXIMUM W/C (WATER/CEMENTITIOUS MATERIALS RATIO), TOTAL AIR CONTENT, SLUMP AND CONCRETE USE: <div><div>STRENGTH TYPE W/C AIR SLUMP USE</div><div>3000 NORMAL WT. 0.57 --- 3" TO 5" FOOTINGS</div><div>3000 NORMAL WT. 0.53 4-6% 3" TO 5" UNLESS NOTED</div></div>
GN.3	REINFORCING BARS: ASTM A615 GRADE 60.
GN.4	WELDED WIRE REINFORCEMENT (WWR): ASTM A185. MINIMUM LAP AND EMBEDMENT TO BE THE GREATER OF ONE CROSS WIRE SPACING PLUS 2" OR 6".
GN.5	REINFORCING STEEL SHOWN IN SECTIONS AND DETAILS IS A SCHEMATIC INDICATION THAT REINFORCING EXISTS. SEE SCHEDULES, SECTION NOTES AND GENERAL NOTES FOR ACTUAL REINFORCING REQUIRED.
GN.6	REINFORCING BAR PLACING ACCESSORIES TO BE INSTALLED IN ACCORDANCE WITH ACI MANUAL OF STANDARD PRACTICE. WHERE CONCRETE IS EXPOSED IN FINISHED BUILDING, PROVIDE ACCESSORIES WITH RUSTPROOF LEGS.
GN.7	DETAIL REINFORCEMENT IN ACCORDANCE WITH ACI 315. REINFORCEMENT SHALL NOT BE WELDED UNLESS NOTED OR APPROVED BY THE STRUCTURAL ENGINEER.
GN.8	SPLICES SHALL BE CLASS "B" TENSION LAP SPLICE, UNLESS NOTED.
GN.9	REINFORCING MARKED "CONTINUOUS" SHALL BE SPLICED WITH CLASS "B" TENSION LAP SPLICE, UNLESS NOTED.
GN.10	CONCRETE COVERAGE OF REINFORCEMENT, UNLESS NOTED: <div>FOOTINGS-----2' TOP & 3" BOTTOM & SIDES EXTERIOR SLABS-----1 1/2' TOP</div>
GN.11	SLABS ON GRADE: 4" THICK, REINFORCED WITH 6X6 W2.9/W2.9 WWR AT MID-DEPTH OF SLAB, UNLESS NOTED.
MA. MASONRY	
MA.1	MASONRY CONSTRUCTION SHALL CONFORM TO TMS 402/ACI 530/ASCE 5 AND TMS 602/ACI 530.1/ASCE 6 SPECIFICATIONS.
MA.2	CONCRETE MASONRY UNITS (CMU) SHALL BE LIGHTWEIGHT (DENSITY = 105 PCF), CONFORMING TO ASTM C90, UNLESS NOTED.
MA.3	COMPRESSIVE STRENGTH OF MASONRY (F'm): 2500 PSI AT 28 DAYS.
MA.4	GROUT SHALL CONFORM TO ASTM C476 WITH COMPRESSIVE STRENGTH (F'g) OF 2500 PSI AT 28 DAYS. GROUT SHALL BE PLACED ACCORDING TO TMS 602/ACI 530.1/ASCE 6 SECTION 3.5.
MA.5	MORTAR SHALL CONFORM TO ASTM C270, TYPE S OR M FOR TYPICAL CONDITIONS, TYPE M FOR BASEMENT AND RETAINING WALLS.
MA.6	ALL MASONRY SHALL BE RUNNING BOND, UNLESS NOTED.
MA.7	ALL BLOCK CELLS AND CAVITIES BELOW GRADE SHALL BE FILLED WITH CONCRETE OR GROUT.
MA.8	SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF MASONRY CONTROL JOINTS AND OPENINGS.
MA.9	REINFORCING BARS: ASTM A615 GRADE 60. LAP REINFORCING BARS ACCORDING TO TYPICAL DETAILS.
MA.10	HORIZONTAL JOINT REINFORCING: LADDER TYPE, 9 GAGE SPACED VERTICALLY AT 16", UNLESS NOTED. PLACE REINFORCING ACCORDING TO MANUFACTURER'S RECOMMENDATIONS. LAP REINFORCING A MINIMUM OF 6".
MA.11	WHEN REINFORCING BARS ARE SPECIFIED, PROVIDE AT EACH SIDE OF CONTROL JOINTS, OPENINGS AND WALL ENDS ACCORDING TO TYPICAL DETAILS. REINFORCING BARS TO BE CENTERED IN WALL, UNLESS NOTED.


MA.12	CONDUIT, PIPING, AND SLEEVES OF ANY MATERIAL TO BE EMBEDDED IN MASONRY SHALL COMPLY WITH THE FOLLOWING REQUIREMENTS: <div><div>A. CONDUIT, PIPING, AND SLEEVES OF ALUMINUM SHALL NOT BE EMBEDDED IN MASONRY.</div><div>B. CONDUIT, PIPING, AND SLEEVES SHALL NOT PASS THROUGH JAMBS, LINTELS, BOND BEAMS, OR SHEAR WALLS WITHOUT APPROVAL BY THE STRUCTURAL ENGINEER.</div><div>C. REINFORCING SHALL NOT BE CUT, BENT, OR DISPLACED FOR PLACEMENT OF CONDUIT, PIPING, AND SLEEVES.</div><div>D. CONDUIT, PIPING, AND SLEEVES SHALL BE NO CLOSER THAN 3 DIAMETERS ON CENTER. MINIMUM SPACING OF DIFFERENT DIAMETERS SHALL BE DETERMINED USING THE LARGER DIAMETER.</div></div>
MA.13	TEMPORARY BRACING OF CMU WALLS IS THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL REMAIN IN PLACE UNTIL PERMANENT RESTRAINT IS PROVIDED.
SS. STRUCTURAL STEEL	
SS.1	FABRICATE AND ERECT ALL STRUCTURAL STEEL IN ACCORDANCE WITH AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
SS.2	THE STEEL FRAME IS "NON-SELF-SUPPORTING". ADEQUATE TEMPORARY SUPPORT MUST BE PROVIDED BY THE CONTRACTOR UNTIL THE LATERAL FORCE RESISTING SYSTEM AND STABILITY OF THE COMPLETED STRUCTURE IS IN PLACE.
SS.3	LATERAL FORCE RESISTING SYSTEM AND STABILITY OF THE BUILDING IN THE COMPLETED STRUCTURE IS PROVIDED AS FOLLOWS: <div><div>A. ROOF DIAPHRAGM: STEEL ROOF DECKING</div><div>B. COLLECTOR ELEMENTS/DRAW STRUTS: NONE</div><div>C. LATERAL FORCE RESISTING SYSTEM: MASONRY SHEAR WALLS</div></div>
SS.4	STRUCTURAL STEEL AND STRUCTURAL STEEL CONNECTIONS SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS UNLESS NOTED OTHERWISE: <div><div>W AND WT SHAPES ASTM A992</div><div>STIFFENER PLATES, BASE PLATES, CAP PLATES, CONNECTION PLATES, ANGLES AND CHANNELS ASTM A36</div><div>HOLLOW STRUCTURAL SECTIONS ASTM A500, GRADE C</div><div>WELDED CONNECTIONS E70XX ELECTRODES, MINIMUM SIZE FILLET WELD 3/16"</div><div>HEADED ANCHOR RODS ASTM F1554 GRADE 36 ANCHOR AND HEAVY HEX NUT, UNLESS INDICATED.</div><div>SHEAR CONNECTORS ASTM A108, GRADE 1015 THROUGH 1020, HEADED-STUD TYPE, COLD FINISHED CARBON STEEL; AWS D1.1, TYPE B.</div><div>BOLTS ASTM A325 OR A490</div><div>NUTS ASTM A563</div><div>WASHERS ASTM F436</div></div>
SS.5	FABRICATE BRACING MEMBERS WITH SUFFICIENT DRAW TO PREVENT SAGGING.
SS.6	WHERE NO CAMBER IS INDICATED, BEAMS SHOULD BE ERECTED WITH NATURAL CAMBER ORIENTED UPWARD.
SS.7	BEAMS SHALL BE EQUALLY SPACED IN BAYS, UNLESS NOTED.
SS.8	HSS MEMBERS SHALL HAVE A 1/4" CLOSURE PLATE.
SS.9	FOUR ANCHOR RODS MINIMUM FOR BASE PLATES UNDER COLUMNS.
SS.10	GROUT UNDER BEARING PLATES SHALL BE NON-SHRINK, NON-METALLIC TYPE. GROUT SHALL HAVE A SPECIFIED DESIGN COMPRESSIVE STRENGTH TWO TIMES THAT OF THE SUPPORTING CONCRETE.
SS.11	STRUCTURAL STEEL MEMBERS SHALL NOT BE CUT, SPLICED, OR MODIFIED IN THE FIELD UNLESS NOTED ON THE STRUCTURAL DRAWINGS OR APPROVED BY THE STRUCTURAL ENGINEER.
SS.12	STRUCTURAL STEEL NOT EXPOSED TO VIEW SHALL BE PRIMED WITH MANUFACTURER'S STANDARD SHOP PRIMER. STRUCTURAL STEEL EXPOSED TO WEATHER IN ITS FINAL POSITION SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A123. FOR STRUCTURAL STEEL EXPOSED TO VIEW, REFER TO PROJECT SPECIFICATIONS FOR FINISHED COATING SYSTEM.
SS.13	DRAIN HOLES SHALL BE PROVIDED IN ALL STEEL AS REQUIRED TO PREVENT WATER ACCUMULATION. HOLES THROUGH STRUCTURAL STEEL MEMBERS SHALL BE GROUND SMOOTH AND NOT EXCEEDING 1/2" DIAMETER. DRAIN HOLES SHALL BE LEFT CLEAN AND UNOBSTRUCTED.



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ISSUE	BY	DATE	DESCRIPTION
-		11/15/22	Permit Set
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PROJECT INFORMATION BLOCK

JOB #

220666

DATE:

08/07/23

DRAWN BY:

ABG

CHECKED BY:

JDP

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

S01.0

General Notes (Continued)

SC. STRUCTURAL STEEL CONNECTIONS

SC.1	ALL LOADS GIVEN ON THE DRAWINGS FOR THE DESIGN OF STRUCTUAL STEEL CONNECTIONS ARE IN ACCORDANCE WITH "LOAD AND RESISTANCE FACTOR DESIGN" (LRFD).
SC.2	CONNECTION DETAILS SHOWN ON THE DRAWINGS ARE CONCEPTUAL UNLESS COMPLETELY DETAILED.
SC.3	ALL STRUCTURAL STEEL CONNECTIONS NOT COMPLETELY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY THE CONTRACTOR TO RESIST FORCES INDICATED. THE CONTRACTOR'S CONNECTION DESIGN SHALL BE UNDER THE DIRECT SUPERVISION OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED. LBYP CAN CONTRACT WITH THE CONTRACTOR TO PROVIDE CONNECTION DESIGN SERVICES IF REQUESTED.
SC.4	ALTERNATE CONNECTION DETAILS MAY BE UTILIZED BY THE CONTRACTOR WITH PRIOR APPROVAL BY THE ARCHITECT AND STRUCTURAL ENGINEER. THE CONTRACTOR'S ALTERNATE CONNECTION DESIGN SHALL BE UNDER THE DIRECT SUPERVISION OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.
SC.5	ALL NON-COMPOSITE BEAM CONNECTIONS SHALL BE 'SIMPLE SHEAR CONNECTIONS', UNLESS NOTED. WHERE BEAM REACTIONS AND/OR DESIGN FORCES ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS, THE CONNECTIONS SHALL BE DESIGNED TO SUPPORT A REACTION EQUAL TO ONE-HALF THE TOTAL UNIFORM LOAD CAPACITY FROM THE MAXIMUM TOTAL UNIFORM LOAD TABLE MULTIPLIED BY A FACTOR OF 1.2 FOR GIVEN SHAPE, SPAN, AND GRADE OF STEEL.
SC.6	TO THE REACTIONS ABOVE, ADD ANY LOADS OR REACTIONS OF MEMBERS SUPPORTED BY THE BEAM WITHIN THREE FEET OF BEAM END AND THE VERTICAL COMPONENTS OF FORCES IN BRACE MEMBERS FRAMING INTO THE BEAM.
SC.7	WHERE BEAM REACTIONS ARE SHOWN ON THE DRAWINGS, THE CONNECTIONS SHALL DEVELOP THE REACTIONS SHOWN. WHERE CONNECTIONS ARE SUBJECT TO ECENTRICITY, SUCH ECENTRICITY SHALL BE TAKEN INTO ACCOUNT WHEN DESIGNING AND DETAILING THE CONNECTION.
SC.8	ERECTION AIDS ARE NOT SHOWN ON THESE DRAWINGS. CONTRACTOR IS TO PROVIDE ERECTION AIDS AS REQUIRED AND REMOVE THEM ONCE WORK IS COMPLETE.
SC.9	AXIAL LOADS AND MOMENTS ARE TO BE CONSIDERED REVERSIBLE AND CONCURRENT WITH SHEAR REACTIONS, UNLESS NOTED.
SC.10	FOR CONNECTION DESIGN AND DETAILING, MEMBER WORK LINES ARE TO BE CONSIDERED ALONG THE MEMBERS' NEUTRAL AXES, UNLESS NOTED.
SC.11	ALL WELDS SHALL CONFORM TO THE AMERICAN WELDING SOCIETY (ANSI/AWS D1.1) STANDARDS AND MUST BE PERFORMED BY AN ANSI/AWS CERTIFIED WELDER.
SC.12	ALL WELD SIZES ARE TO BE CONSIDERED AS EFFECTIVE WELD SIZES AND MUST BE INCREASED TO ACCOUNT FOR ANY GAPS OR SKEWS BETWEEN MEMBERS AS REQUIRED BY ANSI/AWS D1.1.
SC.13	BOLTED CONNECTIONS SHALL USE BEARING TYPE A325-N OR A490-N IN ACCORDANCE WITH AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS".
SC.14	ALL BOLTS SHALL BE 3/4" DIAMETER OR GREATER, UNLESS NOTED. USE SNUG TIGHT BEARING CONNECTIONS FOR ALL BOLTED CONNECTIONS UNLESS NOTED.
SC.15	BOLTS THROUGH 4" WIDE BEAM FLANGES SHALL BE 5/8" DIAMETER.
SC.16	BOLTS LOADED IN TENSION SHALL BE FULLY PRETENSIONED ACCORDING TO RCSC.
SC.17	DO NOT REUSE PRETENSIONED BOLTS.

SJ. STEEL JOISTS

SJ.1	DESIGN, FABRICATE, AND ERECT STEEL JOISTS IN ACCORDANCE WITH THE SJI.
SJ.2	PROVIDE A MINIMUM END BEARING ON STEEL SUPPORTS AS REQUIRED BY SJI. STAGGER THE ENDS OF JOIST IF NECESSARY. CONTRACTOR COORDINATE METAL DECK SPLICE LOCATION TO CENTER OVER JOIST.
SJ.3	PROVIDE HORIZONTAL AND DIAGONAL BRIDGING IN ACCORDANCE WITH SJI TO PROVIDE ADEQUATE JOIST CHORD BRACING.
SJ.4	PROVIDE SLOPED BEARING ENDS WHERE JOIST SLOPE EXCEEDS 1/4" PER FT.
SJ.5	AT JOISTS PARALLEL TO BEAMS, ANCHOR BRIDGING ROWS BY WELDING TO BEAMS.
SJ.6	DESIGN ROOF JOISTS TO RESIST NET WIND UPLIFT PRESSURES. SEE THE COMPONENT AND CLADDING WIND LOAD TABLE SHOWN ON THE DRAWINGS.
SJ.7	PROVIDE JOIST REINFORCEMENT AT ANY CONCENTRATED LOADS NOT LOCATED AT A JOIST PANEL POINT.
SJ.8	CAMBER AND DEFLECTION SHOULD BE CONSIDERED WHEN DETAILING / CONSTRUCTING FRAMING ADJACENT TO OR ATTACHING TO JOISTS.
SJ.9	JOISTS SHALL BE EQUALLY SPACED IN BAYS, UNLESS NOTED.

SD. STEEL DECK

SD.1	DECK PROPERTIES AND ATTACHMENTS SHALL BE IN ACCORDANCE WITH THE STEEL DECK INSTITUTE.
SD.2	DECK SHALL BE CONTINUOUS OVER THREE OR MORE SPANS.
SD.3	SIDELAP AND PERIMETER DECK EDGE FASTENERS ARE TO BE INSTALLED BETWEEN SUPPORTS.
SD.4	ROOF DECK: WIDE RIB TYPE "WR", STEEL ROOF DECK, 22 GAGE, 1-1/2" DEEP, GALVANIZED. GALVANIZED SHEET STEEL FOR DECK SHALL HAVE A MINIMUM YIELD STRENGTH OF 33 KSI.
SD.5	COLD-FORMED METAL FRAMING, SUSPENDED CEILINGS, LIGHT FIXTURES AND DUCTS OR OTHER UTILITIES SHALL NOT BE SUPPORTED BY THE METAL ROOF DECK.
SD.6	DO NOT ALLOW EXTRANEOUS MATERIALS AND SYSTEMS TO BE INCORPORATED INTO REFERENCED TESTED FIRE-RATED DESIGN ASSEMBLIES (TYPICALLY U.L. DESIGNS). THIS INCLUDES CASTING EMBEDDED CONDUITS AND PIPING IN CONCRETE SLABS ON METAL DECK. REFER TO THE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR ASSEMBLY DESCRIPTIONS.

CF. COLD-FORMED METAL FRAMING

CF.1	DESIGN OF COLD-FORMED METAL FRAMING COMPONENTS AND ACCESSORIES IS THE RESPONSIBILITY OF THE COLD-FORMED METAL FRAMING MANUFACTURER. COLD-FORMED METAL FRAMING INCLUDES ANY BUILDING COMPONENT WHICH UTILIZES COLD-FORMED STEEL FRAMING MEMBERS, THEIR CONNECTION TO EACH OTHER AND THEIR CONNECTION TO THE BUILDINGS PRIMARY STRUCTURAL FRAME.
CF.2	ANY COLD-FORMED MEMBER SIZES NOTED ARE MINIMUM TYPICAL SIZES. THE COMPLETE DESIGN OF COLD-FORMED METAL FRAMING SYSTEM AND PREPARATION OF ERECTION DRAWINGS ARE BY THE ENGINEER RESPONSIBLE FOR THEIR DESIGN.
CF.3	SUBMIT THE FOLLOWING: <div><div>A. PRODUCT DATA: FOR EACH TYPE OF COLD-FORMED METAL FRAMING PRODUCT AND ACCESSORY UTILIZED.</div><div>B. SHOP DRAWINGS: SHOW LAYOUT, SPACINGS, SIZES, THICKNESS, AND TYPES OF COLD-FORMED METAL FRAMING; FABRICATIONS; AND FASTENING AND ANCHORAGE DETAILS, INCLUDING MECHANICAL FASTENERS. SHOW REINFORCING CHANNELS, OPENING FRAMING, SUPPLEMENTAL FRAMING, STRAPPING, BRACING, BRIDGING, SPLICES, ACCESSORIES, CONNECTION DETAILS, AND ATTACHMENT TO ADJOINING WORK.</div><div>C. CALCULATIONS: COLD-FORMED METAL FRAMING DESIGN CALCULATIONS FOR THE FILES OF THE STRUCTURAL ENGINEER AND ARCHITECT. CALCULATIONS SHALL BEAR THE SEAL OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.</div></div>
CF.4	PROVIDE COLD-FORMED METAL FRAMING CAPABLE OF WITHSTANDING DESIGN LOADS WITHIN LIMITS AND UNDER CONDITIONS INDICATED. <div><div>A. DESIGN LOADS AS INDICATED IN "GN" SECTION OF THESE GENERAL NOTES.</div><div>B. DEFLECTION LIMITS: DESIGN FRAMING SYSTEMS TO WITHSTAND DESIGN LOADS WITHOUT DEFLECTIONS GREATER THAN THE FOLLOWING:<div><div>1. EXTERIOR NON-LOAD-BEARING FRAMING: HORIZONTAL DEFLECTION OF 1/600 OF THE WALL HEIGHT.</div></div></div></div>

Tension Lap Splice Lengths		
BAR SIZE	f _C = 3000	
	TOP BARS	OTHER BARS
#3	28"	22"
#4	37"	29"
#5	47"	36"
#6	56"	43"
#7	81"	63"
#8	93"	72"

1. TOP BARS ARE HORIZONTAL REINFORCEMENT WITH MORE THAN 12" OF CONCRETE CAST BELOW THE REINFORCEMENT.

Abbreviations


ABOVE FINISH FLOOR	- AFF	JOINT	- JT
ADJACENT	- ADDNL	JOIST(S)	- JST(S)
AIR CONDITIONER	- ADJ	JOIST GIRDER	- JG
AIR HANDLING UNIT	- A/C		
ALTERNATE	- AHU	KIPS (1000 LBS)	- K
ANCHOR	- ALT	KIPS PER LINEAL FOOT	- KLF
ANCHOR ROD	- ANC	KIPS PER SQUARE INCH	- KSI
AND	- AR	KIPS PER SQUARE FOOT	- KSF
APPROVED	- &		
APPROXIMATE	- APPRV	LIGHTWEIGHT CONCRETE	- LWT CONC
ARCHITECTURAL	- APPROX	LIVE LOAD	- LL
ARCHITECTURALLY EXPOSED	- ARCH.	LONGITUDINAL	- LONG.
STRUCTURAL STEEL	- AESS	LONG LEG HORIZONTAL	- LLH
AT (WHEN INDICATING SPACING ONLY)	- @	LONG LEG VERTICAL	- LLV
BACK TO BACK	- B TO B	MANUFACTURE(R)	- MFR
BALANCE	- BAL	MASONRY	- MAS
BASEMENT	- BSMT	MASONRY OPENING	- MO
BASE PLATE	- BSPL	MATERIAL	- MATL
BEAM	- BM	MAXIMUM	- MAX
BEARING	- BRG	MECHANICAL	- MECH
BELOW FINISH FLOOR	- BRF	MEZZANINE	- MEZZ
BETWEEN	- BTWN	MIDDLE	- MID
BLOCK	- BLK	MINIMUM	- MIN
BLOCKING	- BLKG	MISCELLANEOUS	- MISC
BOTTOM	- BOT	MOMENT	- M
BOTTOM CHORD EXTENSION	- BCX	MOMENT CONNECTION(S)	- MC
BRICK	- BRK		
BRIDGING	- BRDG	NEAR FACE	- NF
BUILDING	- BLDG	NEAR SIDE	- NS
		NOMINAL	- NOM
		NOT IN CONTRACT	- NIC
		NOT TO SCALE	- NTS
		NUMBER	- NO. or #
CAST IN PLACE	- CIP	ON CENTER	- OC
CENTER	- CTR	OPENING(S)	- OPNG(S)
CENTERLINE	- CL	OPPOSITE	- OPP
CENTER TO CENTER	- C TO C	OUTSIDE FACE	- O.F.
CHANNEL	- C	OUTSIDE DIAMETER	- OD
CLEAR OR CLEARANCE	- CLR	OUTSTANDING LEG	- OSL
COLUMN	- COL		
COMPLETE JOINT PENETRATION	- CJP		
COMPRESSION	- COMP	PARALLEL	- PAR
CONCRETE	- CONC	PARTITION(S)	- PARTN(S)
CONCRETE MASONRY UNIT	- CMU	PENETRATION	- PEN
CONNECTION(S)	- CONN(S)	PERMANENT	- PERM
CONTINUOUS	- CONT	PERPENDICULAR	- PERP
CONTRACTOR	- CONTR	PLATE	- PL
CONSTRUCTION	- CONST	PLUMBING	- PLBG
CONTROL JOINT	- CJ	PNEUMATIC	- PNEU
CORNER	- COR	POST-TENSION	- PT
COORDINATE	- COORD	POUNDS	- LBS
COVER PLATE	- COV PL	POUNDS PER LINEAL FOOT	- PLF
		POUNDS PER SQUARE INCH	- PSI
		POUNDS PER SQUARE FT	- PSF
DEGREE	- DEG OR °	POUNDS PER CUBIC YARD	- PCY
DEGREE CELSIUS	- C°	POUNDS PER CUBIC INCH	- PCI
DEGREE FAHRENHEIT	- F°	PRECAST CONCRETE	- PC
DETAIL	- DET	PREENGINEERED METAL BUILDING	- PEMB
DEAD LOAD	- DL	PREFABRICATED	- PREFAB
DIAGONAL	- DIAG	PRELIMINARY	- PRELIM
DIAMETER	- DIA or Ø	PRESSURE INJECTED FOOTING	- PIF
DIMENSION(S)	- DIM(S)	PROJECTION	- PROJ
DRAWING(S)	- DWG(S)		
DRILLED PIER	- DP		
DOUBLE	- DBL	RADIUS	- R
DOUBLE EXTRA STRONG	- XXS	REFERENCE	- REF
DOWEL(S)	- DWL(S)	REINFORCED CONC PIPE	- RCP
DOWN	- DN	REINFORCING	- REINF
		REQUIRED	- REQD
EACH	- EA	RISER	- RIS
EACH FACE	- EF	ROOF	- RF
EACH WAY	- EW	ROOF DRAIN	- RD
EDGE OF DECK	- EOD	ROOF TOP UNIT	- RTU
EDGE OF SLAB	- EOS	ROUND	- RND
ELECTRICAL	- ELEC		
ELEVATION	- EL	SCHEDULE	- SCHED
ELEVATOR	- ELEV	SECTION	- SECT
EMBEDMENT	- EMBED.	SHEAR	- V
ENGINEER	- ENGR	SHEET	- SHT
EQUAL	- EQ	SIMILAR	- SIM
EQUIPMENT	- EQUIP.	SPACE	- SP
EXISTING	- EXIST.	SPECIFICATION(S)	- SPEC(S)
EXPANSION	- EXP	SPECIFIED	- SPEC'D
EXPANSION ANCHOR	- EXP ANC	SQUARE	- SQ
EXPANSION JOINT	- EJ	STANDARD	- STD
EXTENSION	- EXTN	STEEL	- STL
EXTERIOR	- EXT	STIFFENER	- STIFF.
EXTRA STRONG	- XS	STRAIGHT	- STR
		STIRRUPS	- STIR.
FABRICATOR	- FABR	STRUCTURE OR STRUCT'L	- STRUCT
FACE TO FACE	- F TO F	SYMMETRICAL	- SYM
FACE OF STUD	- FOS	SUPPORT(S)	- SUPT(S)
FAR SIDE	- FS		
FASTENER	- FAS	TEMPERATURE	- TEMP
FIELD VERIFY	- FV	TENSION	- T
FINISH (ED)	- FIN.	THICK	- THK
FINISHED FLOOR	- FF	TONGUE AND GROOVE	- T&G
FLANGE	- FLG	TOP AND BOTTOM	- T&B
FLOOR	- FLR	TOP CHORD EXTENSION	- TCX
FLOOR DRAIN	- FD	TOP OF BEAM	- TOB
FOOT	- FT	TOP OF FOOTING	- TOF
FOOTING	- FTG	TOP OF GRADE BEAM	- TOGB
FOUNDATION	- FDN	TOP OF JOIST	- TOJ
FRAMING	- FRMG	TOP OF PIER	- T.O.P.
		TOP OF PILE CAP	- TOPC
GAGE OR GAUGE	- GA	TOP OF STEEL	- TOS
GALVANIZED	- GALV	TOP OF WALL	- TOW
GENERAL	- GEN	TREAD	- TR
GENERAL CONTRACTOR	- GC	TYPICAL	- TYP
GOVERNMENT	- GOVT		
GRADE	- GR	UNLESS NOTED	- U.N.
GRADE BEAM	- GB		
GROUND	- GRD	VERTICAL	- VERT
HARD ROCK	- HD RK	WATERPROOFING	- WPGF
HEADED STUD(S)	- H. STUD(S)	WATERSTOP	- WS
HEIGHT	- HT	WELDED WIRE REINFORCEMENT	- WWR
HIGH STRENGTH	- HS	WIND LOAD	- WL
HOOK	- HK	WIND MOMENT CONNECTION	- WC
HORIZONTAL	- HORZ	WINDOW	- WDW
		WITH	- W/
INFORMATION	- INFO	WITHOUT	- W/O
INSIDE DIAMETER	- ID	WOOD	- WD
INSIDE FACE	- I.F.	WORK POINT	- WP
INTERIOR	- INT	WEIGHT	- WT
INTERMEDIATE	- INTM	WIDE FLANGE	- WF



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ISSUE	BY	DATE	DESCRIPTION
-		11/15/22	Permit Set
-			

PROJECT INFORMATION BLOCK

JOB #	220666
DATE:	08/07/23
DRAWN BY:	ABG
CHECKED BY:	JDP

SHEET TITLE

GENERAL NOTES
(CONTINUED) AND
TYPICAL DETAILS

SHEET NUMBER

S01.1

<h1>Components and Cladding</h1> <h2>Gross Ultimate Wind Pressures</h2> <h3>(Vult per ASCE 7-16)</h3>			
ZONE	EFFECTIVE WIND AREA (SQ FT)	MAX POSITIVE PRESSURE (PSF)	MAX NEGATIVE PRESSURE (PSF)
ZONE 1' ROOF INTERIOR ZONE	10	16.0	-35.0
	20	16.0	-35.0
	50	16.0	-35.0
	100	16.0	-35.0
ZONE 1 ROOF INTERMEDIATE ZONE	10	16.0	-60.9
	20	16.0	-56.9
	50	16.0	-51.6
	≥100	16.0	-47.6
ZONE 2 ROOF EDGE ZONE	10	35.0	-80.4
	20	33.5	-75.2
	50	31.4	-68.4
	≥100	29.9	-63.2
ZONE 3 ROOF CORNER ZONE	10	35.0	-109.6
	20	33.5	-99.2
	50	31.4	-85.6
	≥100	29.9	-75.2
ZONE 4 WALL INTERIOR ZONE	10	35.0	-37.9
	20	33.5	-36.4
	50	31.4	-34.3
	100	29.9	-32.8
ZONE 5 WALL EDGE ZONE	≥500	26.3	-29.2
	10	35.0	-46.7
	20	33.5	-43.6
	50	31.4	-39.5
ZONE 5 WALL EDGE ZONE	100	29.9	-36.4
	≥500	26.3	-29.2

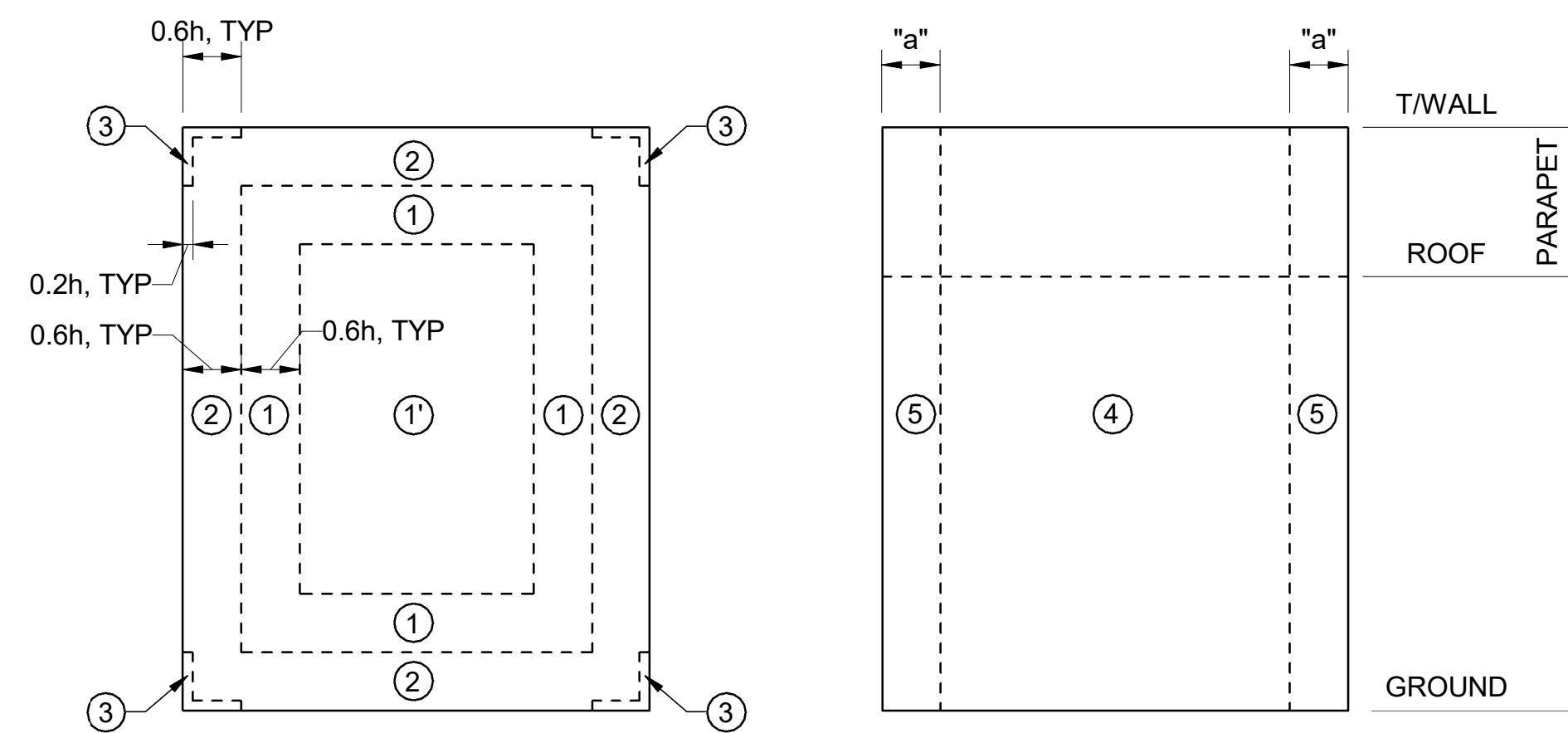
INTERNAL PRESSURE COEFFICIENT = ±0.18

WIDTH OF EDGE STRIP, a = 4'-8"

RELIABLE ROOF DEAD LOAD FOR UPLIFT = 6 PSF

WORST CASE ROOF DEAD LOAD = 25 PSF

Components and Cladding Ultimate Wind Pressures (Vult per ASCE 7-16)		
ZONE	EFFECTIVE WIND AREA (SQ FT)	MAX (NET) PRESSURE (PSF)
4	10	111.4
	20	104.2
	50	94.6
	100	87.4
5	10	142.7
	20	130.0
	50	113.1
	100	100.3



Roof Plan

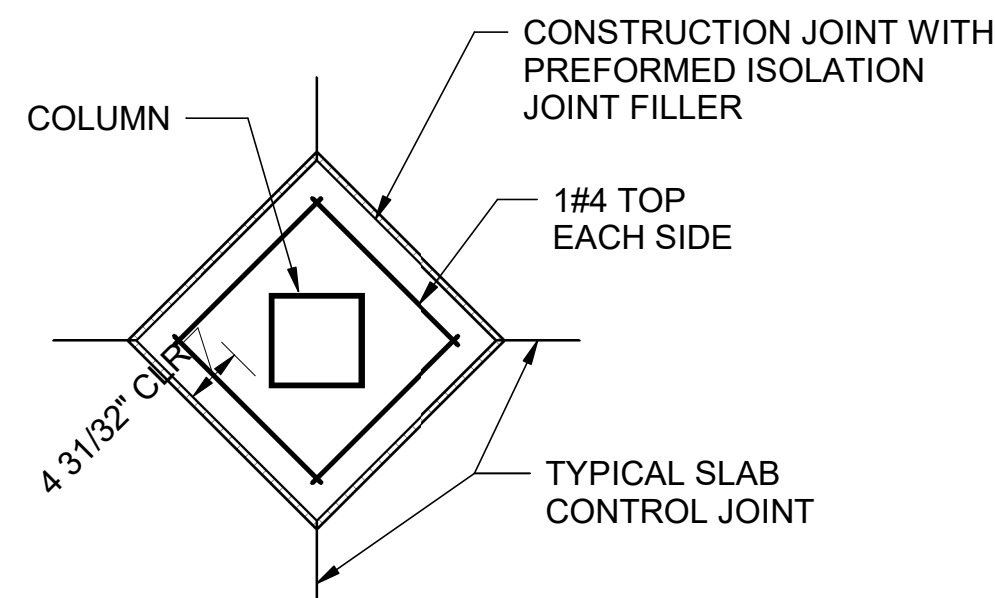
Wall Elevation

Components and Cladding Windows

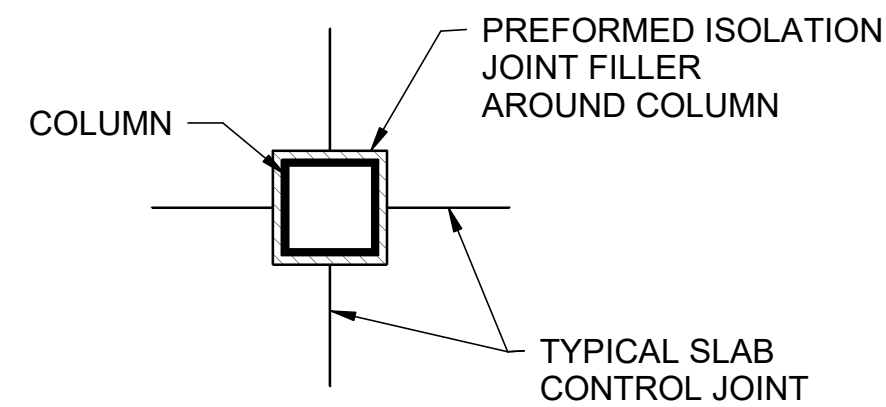
36/5 PATTERN
W/ 6 SIDELAPS
ALL ROOF DECK

36" COVERAGE

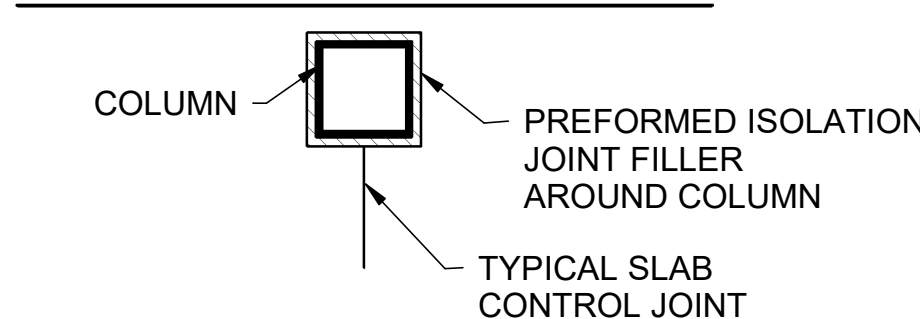
1. AT SUPPORT FASTENERS, USE 5/8"Ø PUDDLE WELDS.
2. AT SIDELAP FASTENERS, USE #10 TEK SCREWS.



Interior

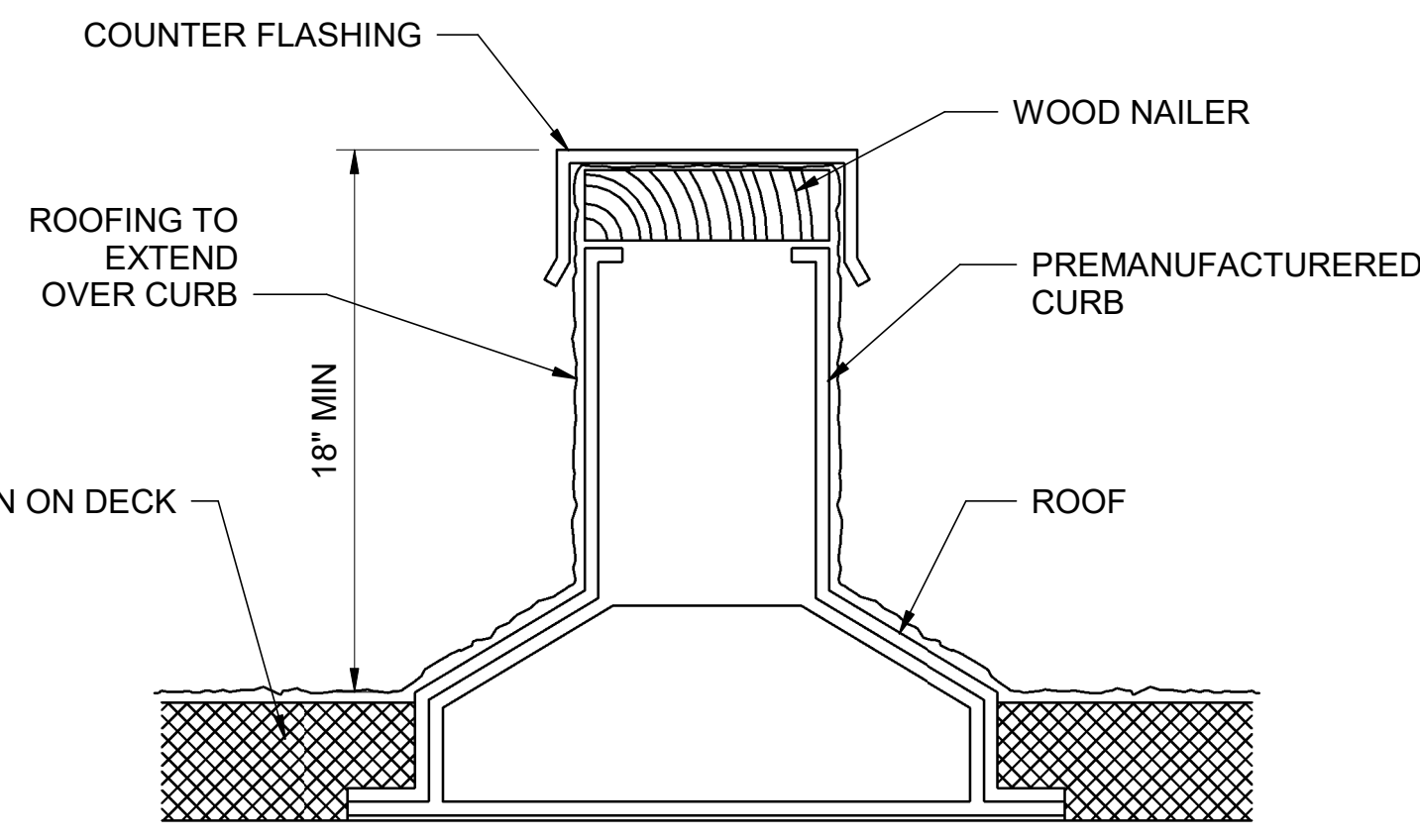
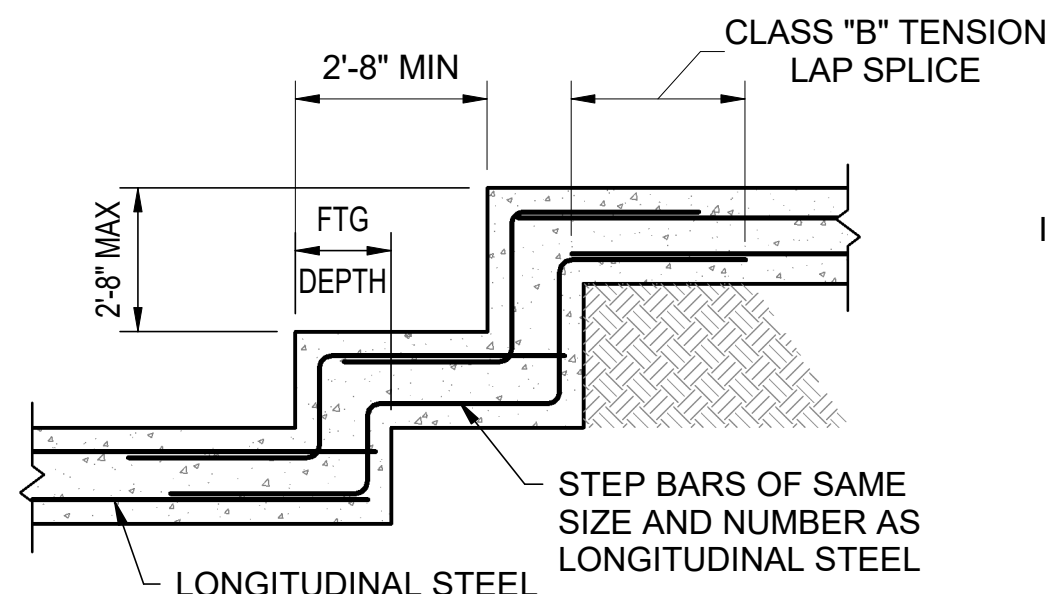


Interior



Exterior

Isolation Joint Detail



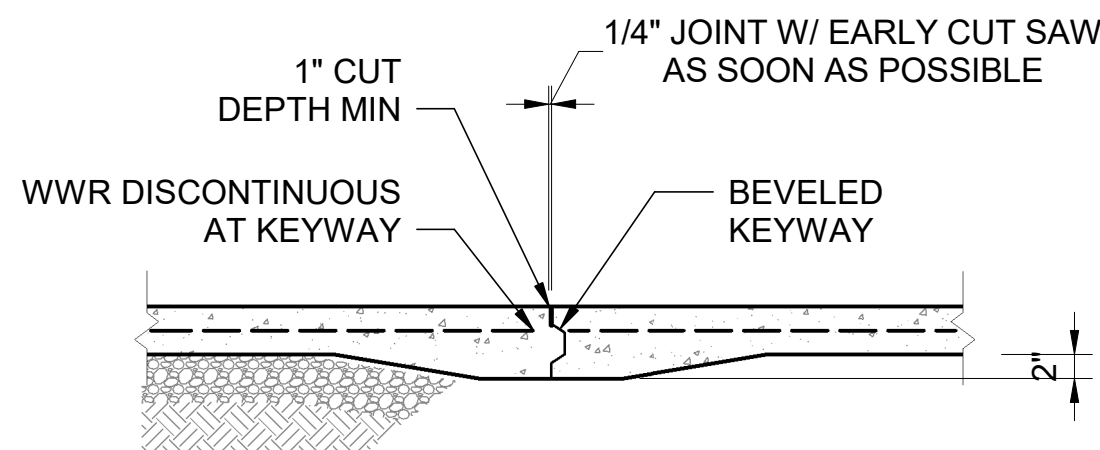
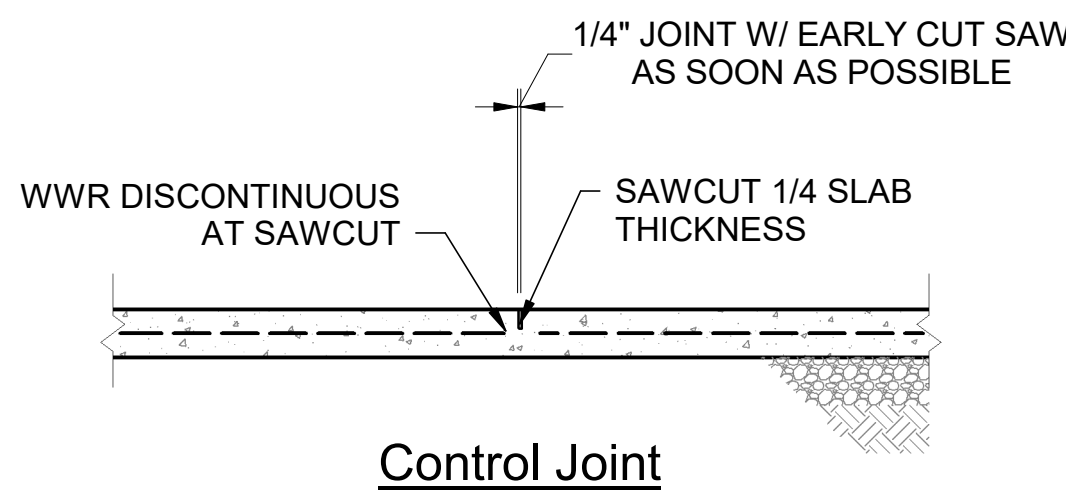
NOTES:

1. CONTROL JOINT FILLER SEMI-RIGID EPOXY.
2. SAWCUT TO TAKE PLACE WITHIN 4-12 HOURS OF FINISHING CONCRETE: 4 HOURS IN HOT WEATHER, 12 HOURS IN COLD WEATHER.

JOINT TYPE IS OPTIONAL

TYPICAL FOOTING REINF - SEE SECTIONS
 2'-0" MIN
 ADDITIONAL 2#5 TOP
 2'-0" MIN
 TYPICAL WALL FOOTING - SEE SECTIONS
 TOF
 SEE PLAN
 COMPACTED FILL OR
 DISTURBED EARTH AS REQD
 SLEEVE AT EA UTILITY LINE
 CAST INTO LEAN CONCRETE,
 1'-0" MINETER THAN LINE PASSING
 REINFORCING STEEL SLEEVES
 MUST BE ORIENTED
 LONGITUDINAL TO FOOTING LENGTH
 4" MIN
 6" MIN WHERE MULTIPLE
 SLEEVES OCCUR
 LEAN CONCRETE FILL AS SHOWN AND
 1'-0" MIN BEYOND BOTH SIDES OF
 FOOTING BEARING LOCATION (INSTALL
 AND CURE PRIOR TO FOOTING PLACEMENT)

1. CONTRACTOR'S OPTION TO STEP FOOTINGS BELOW UTILITIES IN LIEU OF THIS DETAIL
2. COORDINATE UTILITY LOCATIONS W/ CIVIL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS.
3. UTILITIES SHALL NOT PASS BELOW COLUMN FOOTINGS.



Construction Joint

NOTES:

1. CONTROL JOINT FILLER SEMI-RIGID EPOXY.
2. SAWCUT TO TAKE PLACE WITHIN 4-12 HOURS OF FINISHING CONCRETE: 4 HOURS IN HOT WEATHER, 12 HOURS IN COLD WEATHER.

Isolation Joint Detail

The diagram illustrates the construction of an isolation joint in a concrete slab and column. A vertical column is shown with a vertical crack at its center. The slab is divided into two sections by a vertical joint. The joint is filled with 'PREFORMED ISOLATION JOINT FILLER'. On either side of the joint, the slab has 'WWR OR REBAR' (Welded Wire Reinforcement or Reinforcing Bars). The column has 'REBAR' (Reinforcing Bars) extending through it. A note points to the bottom of the slab, stating 'PROVIDE 3" MIN CONC COVER AT ALL STEEL BELOW GRADE'.

COLUMN

PREFORMED ISOLATION JOINT FILLER

WWR OR REBAR

PROVIDE 3" MIN CONC COVER AT ALL STEEL BELOW GRADE

Section at Column

CONTRACTOR'S OPTION TO USE EITHER JOINT DETAIL

[illegible]

PROJECT INFORMATION BLOCK

JOB #	220666
DATE:	08/07/23
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SHEET TITLE

TYPICAL DETAILS

SHEET NUMBER

S01.2

ISSUE	BY	DATE	DESCRIPTION
-		11/15/22	Permit Set
-			
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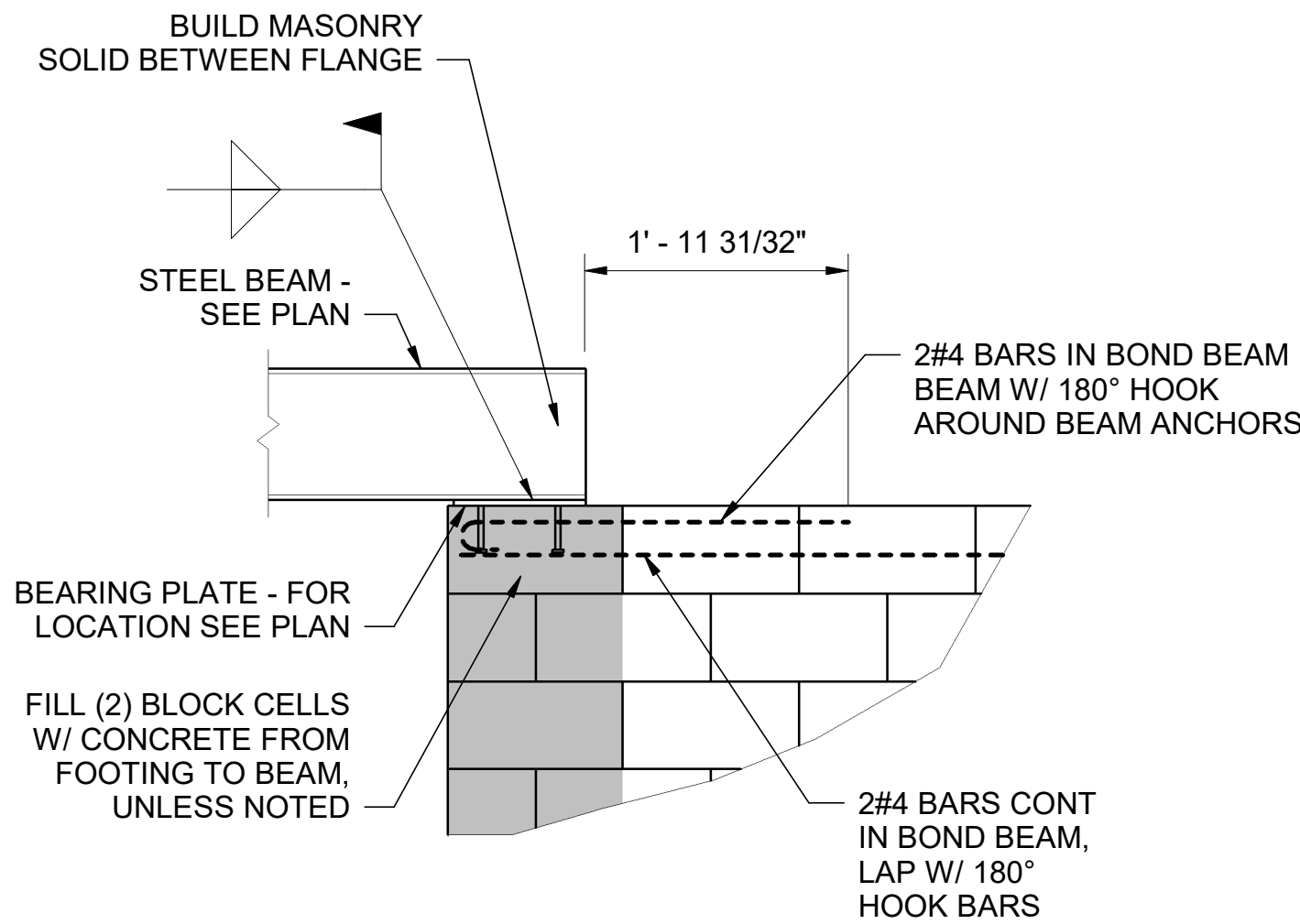
PROJECT INFORMATION BLOCK	
JOB #	220666
DATE:	08/07/23
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SHEET TITLE

TYPICAL DETAILS

SHEET NUMBER

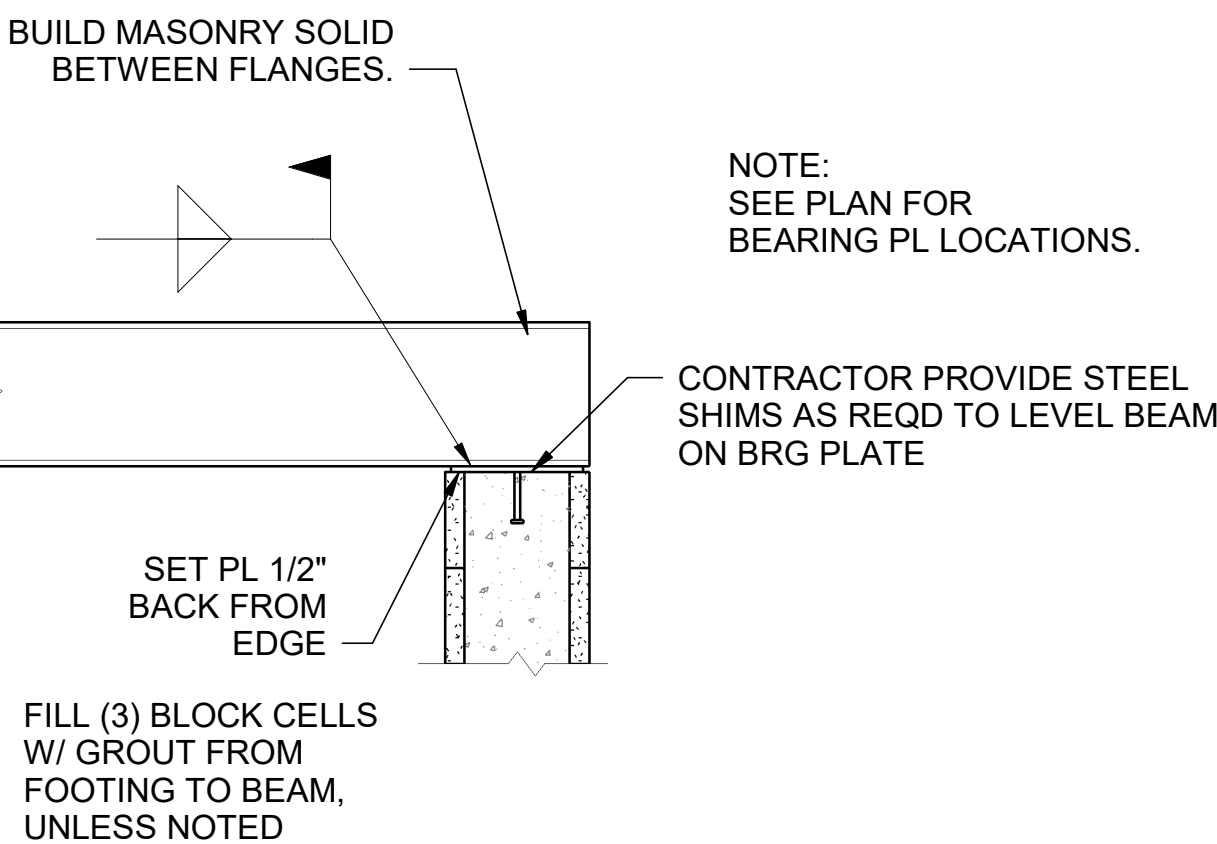
S01.4



Bearing Plate Schedule

	THICKNESS	LENGTH	WIDTH	NO. OF STUDS	LENGTH OF STUDS
TYP BP FOR 8" CMU	3/8"	1' - 4"	7 1/2"	2	6"
BP 1	3/8"	2' - 0"	7 1/2"	3	6"

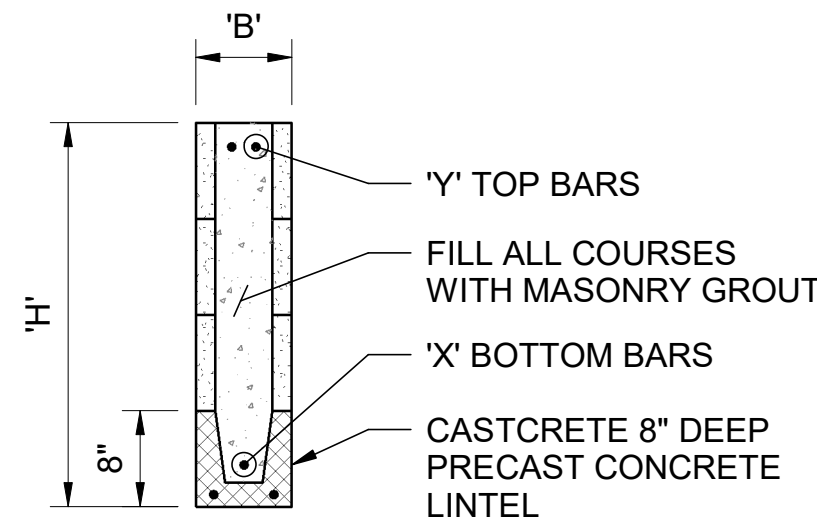
Beam Bearing Detail



Masonry Lintel Schedule

MAXIMUM OPENING WIDTH	CMU LINTEL DIMENSIONS AND REINFORCING		NOTES
	DEPTH	8" WALL	
6'-0"	8	2#4 BOT	---
13'-0"	24	8F24-1B/1T	3

- DO NOT USE THIS SCHEDULE IF CONCENTRATED LOAD IS APPLIED TO THE LINTEL AT A HEIGHT LESS THAN HALF THE SPAN ABOVE THE LINTEL, OR IF STACK BOND IS SPECIFIED.
- PROVIDE 8" MINIMUM BEARING FOR ALL LINTELS.
- THESE LINTELS ARE PRECAST AND ARE TO BE PROVIDED BY CAST-CRETE.
- DESIGNATIONS FOR THE PRECAST LINTELS SHOWN IN THE STRUCTURAL DRAWINGS ARE OF THE FORMAT 8F16-1B, WHERE 8 INDICATES THE CELL WIDTH, "F" DESIGNATES THAT CELLS ARE FILLED WITH GROUT, 16 INDICATES THE TOTAL DEPTH OF THE LINTEL, AND 1B INDICATES 1 #5 BAR IN THE BOTTOM OF THE CELL.



DESIGNATION: 8F32-1B/2T

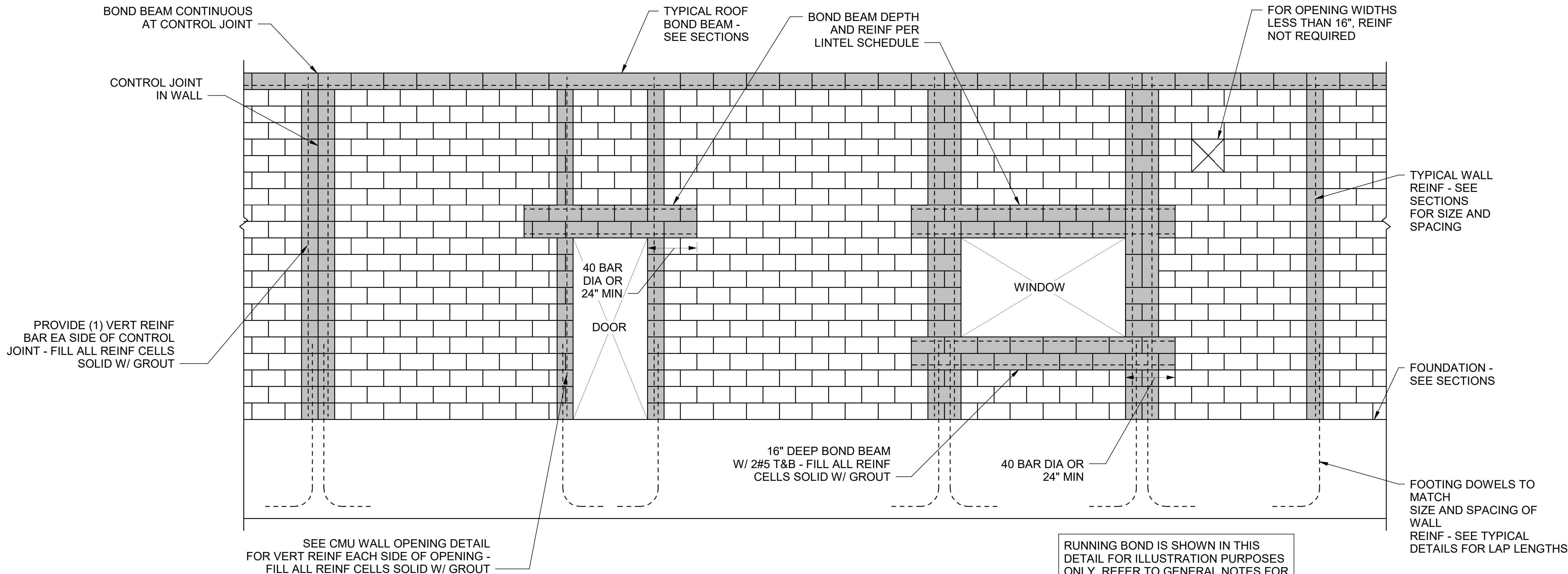
WIDTH 'B'

DEPTH 'H'

OF TOP BARS 'Y'

OF BOT BARS 'X'

CastCrete Lintel Detail



Door and Window Opening
Reinforcement at Load Bearing Masonry Walls



1. STONE VENEER NOT SHOWN FOR CLARITY.
2. ALL BOND BEAMS MADE WITH FORM BLOCKS ONLY.
3. FOR STRUCTURAL BOND BEAMS, SEE SECTIONS.



NOTE:
SEE ARCHITECTURAL DRAWINGS FOR
CONTROL JOINT LOCATIONS. CONTROL
JOINT SPACING NOT TO EXCEED 25'-0"
OR 1.5 TIMES THE WALL HEIGHT,
WHICHEVER IS LESS.



Masonry Tension Lap Splice Lengths	
BAR SIZE	$f_m = 2000$
#4	18"
#5	20"
#6	38"
#7	52"

1. THIS TABLE CONTAINS DEVELOPMENT AND SPLICE LENGTHS FOR REINFORCEMENT IN CMU ACCORDING TO ACI 530-13/TMS 402-13/ASCE 5-13.
2. FOR DEVELOPMENT AND SPLICE LENGTHS OF EPOXY COATED BARS, MULTIPLY TABULATED VALUES BY 1.5.
3. WHERE VERTICAL BAR IS INTERRUPTED BY A BEARING PLATE OR OTHER OBSTRUCTION, PROVIDE BACK DOWEL MATCHING VERTICAL REINFORCEMENT W/ 8" MAXIMUM OFFSET.

[illegible]

PROJECT INFORMATION BLOCK	
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SHEET TITLE

TYPICAL DETAILS

SHEET NUMBER

S01.5

Special Inspection General Notes

- SI.1 ALL SPECIAL INSPECTIONS SHALL BE PERFORMED IN CONFORMANCE WITH THE APPLICABLE INTERNATIONAL BUILDING CODE AND ITS REFERENCED SPECIFICATIONS.
- SI.2 THE SPECIAL INSPECTOR SHALL BE EMPLOYED BY THE OWNER OR THE OWNER'S AGENT AND NOT BY THE CONTRACTOR OR SUBCONTRACTOR WHOSE WORK IS TO BE INSPECTED OR TESTED. ANY CONFLICT OF INTEREST MUST BE DISCLOSED TO THE BUILDING OFFICIAL PRIOR TO COMMENCING WORK.
- SI.3 THE SPECIAL INSPECTOR SHALL BE QUALIFIED PER THE INTERNATIONAL BUILDING CODE AND SHALL BE EDUCATED IN THE TASKS REQUIRED TO CONDUCT, SUPERVISE, AND EVALUATE THE INSPECTIONS. THE SPECIAL INSPECTOR MUST ALSO BE OBJECTIVE, COMPETENT, AND HAVE ACCESS TO THE APPROPRIATE TESTING EQUIPMENT WHICH SHALL BE MAINTAINED AND PERIODICALLY CALIBRATED. THE QUALIFICATIONS OF THE SPECIAL INSPECTOR MAY BE SUBJECT TO THE APPROVAL OF THE BUILDING OFFICIAL.
- SI.4 SPECIAL INSPECTION AGENTS:

ATA: APPROVED TESTING AGENCY

GEOR: GEOTECHNICAL ENGINEER OF RECORD:
PSI
5801 BENJAMIN CENTER DRIVE, SUITE 112
TAMPA, FL 33634

SEOR: STRUCTURAL ENGINEER OF RECORD:
LBVD INC.
880 MONTCLAIR ROAD, SUITE 600
BIRMINGHAM, AL 35213
- SI.5 THE SPECIAL INSPECTIONS SHALL BE PERFORMED IN ADDITION TO ANY OBSERVATIONS PERFORMED BY THE ENGINEER OF RECORD AND ANY INSPECTIONS PERFORMED BY THE BUILDING OFFICIAL.
- SI.6 THE SPECIAL INSPECTOR SHALL MAINTAIN RECORDS AND PROVIDE THE REQUIRED DOCUMENTATION AS PRESCRIBED IN THE INTERNATIONAL BUILDING CODE, INCLUDING THE SUBMITTAL OF REPORTS TO THE BUILDING OFFICIAL AND THE DESIGNER OF RECORD.
- SI.7 THE CONTRACTOR SHALL COORDINATE THE CONSTRUCTION SCHEDULE WITH THE SPECIAL INSPECTOR TO ALLOW FOR SPECIAL INSPECTIONS.
- SI.8 CONSTRUCTION WHICH REQUIRES SPECIAL INSPECTIONS SHALL BE MAINTAINED IN SUCH A STATE AS TO ALLOW ACCESS FOR THE SPECIAL INSPECTOR UNTIL THE REQUIRED INSPECTIONS OR TESTS HAVE BEEN COMPLETED.
- SI.9 ANY DEVIATIONS FOUND DURING THE SPECIAL INSPECTION PROCESS SHALL IMMEDIATELY BE BROUGHT TO THE ATTENTION OF THE DESIGNER OF RECORD. ALL DEVIATIONS MUST BE ADDRESSED PRIOR TO COMPLETION OF THE WORK.
- SI.10 INSPECTION FREQUENCY:

A. CONTINUOUS – SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS PRESENT WHEN AND WHERE THE WORK TO BE INSPECTED IS BEING PERFORMED.

B. PERIODIC – SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS INTERMITTENTLY PRESENT WHERE THE WORK TO BE INSPECTED HAS BEEN OR IS BEING PERFORMED.

C. OBSERVE – OBSERVE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS.

D. PERFORM – PERFORM TASKS FOR EACH JOINT, MEMBER, AND CONNECTION.
- SI.11 SPECIAL INSPECTIONS FOR STRUCTURAL, LOAD-BEARING, OR LATERAL LOAD BEARING FABRICATED ITEMS SHALL BE PERFORMED FOR THE FABRICATED ITEMS AT THE FABRICATOR'S SHOP. SPECIAL INSPECTIONS FOR FABRICATED ITEMS MAY BE WAIVED WHEN THE FABRICATOR IS REGISTERED AND HAS APPROVAL TO PERFORM THE WORK WITHOUT SPECIAL INSPECTIONS. IF THE INSPECTIONS ARE WAIVED, THE FABRICATOR MUST SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL SHOWING COMPLIANCE WITH THE APPROVED STRUCTURAL DRAWINGS.

Soils				
NO.	INSPECTION TASK	FREQUENCY	REFERENCE FOR CRITERIA	AGENT
1.00	VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	PERIODIC		GEOR
2.00	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	PERIODIC		GEOR
3.00	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	PERIODIC		GEOR
4.00	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT, AND COMPACTION OF COMPACTED FILL.	CONTINUOUS		GEOR
5.00	PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	PERIODIC		GEOR

Wind Resistance				
NO.	INSPECTION TASK	FREQUENCY	REFERENCE FOR CRITERIA	AGENT
1.00	COLD FORMED STEEL LIGHT-FRAMED CONSTRUCTION		IBC 1705.11.2	
1.01	WELDING OPERATIONS OF ELEMENTS OF THE MAIN WIND FORCE-RESISTING SYSTEM	CONTINUOUS		ATA
1.02	SCREW ATTACHMENT, BOLTING, ANCHORING, OR OTHER FASTENING OF ELEMENTS OF THE MAIN WIND FORCE-RESISTING SYSTEM, INCLUDING SHEAR WALLS, DIAPHRAGMS, DRAG STRUTS, BRACES, AND HOLD DOWNS.	PERIODIC		ATA
2.00	WIND RESISTING COMPONENTS		IBC 1705.11.3	
2.01	ROOF COVERINGS, ROOF DECK, AND ROOF FRAMING CONNECTIONS	PERIODIC		ATA
2.02	EXTERIOR WALL COVERING AND CONNECTIONS TO ROOF AND FLOOR DIAPHRAGMS AND FRAMING	PERIODIC		ATA

Steel Joists				
NO.	INSPECTION TASK	FREQUENCY	REFERENCE FOR CRITERIA	AGENT
1.00	INSTALLATION OF OPEN-WEB STEEL JOISTS AND JOIST GIRDERS.			
1.01	END CONNECTIONS - WELDED OR BOLTED.	PERIODIC	SJI SPECIFICATIONS LISTED IN IBC 2207.1	ATA
1.02	HORIZONTAL OR DIAGONAL STANDARD BRIDGING OR BRIDGING THAT DIFFERS FROM SJI SPECIFICATIONS LISTED IN SECTION 2207.1.	PERIODIC	SJI SPECIFICATIONS LISTED IN IBC 2207.1	ATA

Concrete				
NO.	INSPECTION TASK	FREQUENCY	REFERENCE STANDARD	Agent
1.00	INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	PERIODIC	ACI 318 CH 20, 25.2, 25.3, 26.5.1-26.5.3; IBC 1908.4	ATA
2.00	INSPECT ANCHORS CAST IN CONCRETE.	PERIODIC	ACI 318: 17.8.2	ATA
3.00	INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS.			ATA
3.01	INSPECT ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.	CONTINUOUS	ACI 318: 17.8.2.4	ATA
3.02	INSPECT MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.01.	PERIODIC	ACI 318: 17.8.2	ATA
4.00	VERIFY USE OF REQUIRED DESIGN MIX.	PERIODIC	ACI 318: CH 19, 26.4.3, 26.4.4; IBC 1904.1, 1904.2, 1908.2, 1908.3	ATA
5.00	PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE. DETERMINE UNIT WEIGHT OF LIGHTWEIGHT CONCRETE.	CONTINUOUS	ASTM C 172; ASTM C 31; ACI 318:26.4.5, 26.12; IBC 1908.10	ATA
6.00	INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	CONTINUOUS	ACI 318: 26.4.5; IBC 1908.6, 1908.7, 1908.8	ATA
7.00	VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	PERIODIC	ACI 318: 26.4.7-26.4.9; IBC 1908.9	ATA
8.00	INSPECT FORMWORK FOR SHAPE, LOCATION, AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	PERIODIC	ACI 318: 26.10.1(B)	ATA
9.00	ISOLATED CONCRETE FOOTINGS OF BUILDINGS THREE STORIES OR LESS ARE EXCEPTED FROM INSPECTIONS BUT NOT FROM MATERIALS TESTING.		IBC 1705.3 (1)	ATA
10.00	CONTINUOUS CONCRETE FOOTINGS SUPPORTING WALLS OF LIGHT-FRAME CONSTRUCTION OR THOSE THAT ARE DESIGNED IN ACCORDANCE WITH IBC 2015 TABLE 1809.7 ARE EXCEPTED FROM INSPECTIONS BUT NOT FROM MATERIALS TESTING.		IBC 1705.3 (2)	ATA
11.00	SLABS ON GRADE ARE EXCEPTED FROM INSPECTIONS BUT NOT FROM MATERIALS TESTING.		IBC 1705.3 (3)	ATA
12.00	CONCRETE FOUNDATION WALLS CONSTRUCTED IN ACCORDANCE WITH IBC 2015 TABLE 1807.1.6.2 ARE EXCEPTED FROM INSPECTIONS BUT NOT FROM MATERIALS TESTING.		IBC 1705.3 (4)	ATA

Masonry - Level B				
NO.	INSPECTION TASK	FREQUENCY	REFERENCE FOR CRITERIA	AGENT
1.00	VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) AS DELIVERED TO THE PROJECT SITE FOR SELF-CONSOLIDATING GROUT.	PERIODIC	TMS 602 ART. 1.5 B.1.B.3	ATA
2.00	VERIFICATION OF F'M AND F'AAc PRIOR TO CONSTRUCTION, EXCEPT WHERE SPECIFICALLY EXEMPTED BY TMS 402/ACI 530/ASCE 5.	PERIODIC	TMS 602 ART. 1.4 B	ATA
3.00	VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS.	PERIODIC	TMS 602 ART. 1.5	ATA
4.00	AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:			
4.01	PROPORTIONS OF SITE-PREPARED MORTAR	PERIODIC	TMS 602 ART. 2.1, 2.6 A	ATA
4.02	CONSTRUCTION OF MORTAR JOINTS	PERIODIC	TMS 602 ART. 3.3 B	ATA
4.03	GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES	PERIODIC	TMS 602 ART. 2.4 B, 2.4 H	ATA
4.04	LOCATION OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES	PERIODIC	TMS 602 ART. 3.4, 3.6 A	ATA
4.05	PRESTRESSING TECHNIQUE	PERIODIC	TMS 602 ART. 3.6 B	ATA
4.06	PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	CONTINUOUS AND PERIODIC(a)	TMS 602 ART. 2.1 C	ATA
5.00	PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:			
5.01	GROUT SPACE	PERIODIC	TMS 602 ART. 3.2 D, 3.2 F	ATA
5.02	GRADE, TYPE, AND SIZE OF REINFORCEMENT AND ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES	PERIODIC	TMS 402 SEC. 6.1; TMS 602 ART. 2.4, 3.4	ATA
5.03	PLACEMENT OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES	PERIODIC	TMS 402 SEC. 6.1, 6.2.1, 6.2.6, 6.2.7; TMS 602 ART. 3.2 E, 3.4, 3.6 A	ATA
5.04	PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS	PERIODIC	TMS 602 ART. 2.6 B, 2.4 G.1.b	ATA
5.05	CONSTRUCTION OF MORTAR JOINTS	PERIODIC	TMS 602 ART. 3.3 B	ATA
6.00	VERIFY DURING CONSTRUCTION:			
6.01	SIZE AND LOCATION OF STRUCTURAL ELEMENTS	PERIODIC	TMS 602 ART. 3.3 F	ATA
6.02	TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION	PERIODIC	TMS 402 SEC. 1.2.1(e), 6.1.4.3, 6.2.1	ATA
6.03	WELDING OF REINFORCEMENT	CONTINUOUS	TMS 402 SEC. 8.1.6.7.2, 9.3.3.4(c), 11.3.3.4(b)	ATA
6.04	PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°F)	PERIODIC	TMS 602 ART. 1.8 C, 1.8 D	ATA
6.05	APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE	CONTINUOUS	TMS 602 ART. 3.6 B	ATA
6.06	PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE	CONTINUOUS AND PERIODIC(a)	TMS 602 ART. 3.5, 3.6 C	ATA
6.07	PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	PERIODIC	TMS 602 ART. 3.3 B.9, 3.3 F.1.b	ATA
7.00	OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS.	PERIODIC	TMS 602 ART. 1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3, 1.4 B.4	ATA
NOTES:	(a) CONTINUOUS REQUIRED FOR THE FIRST 5,000 SQUARE FEET OF AAC MASONRY. PERIODIC REQUIRED AFTER THE FIRST 5,000 SQUARE FEET OF AAC MASONRY.			



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Project No. **101-18-371.033**
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SHEET TITLE
SPECIAL INSPECTIONS
SHEET NUMBER
S01.6

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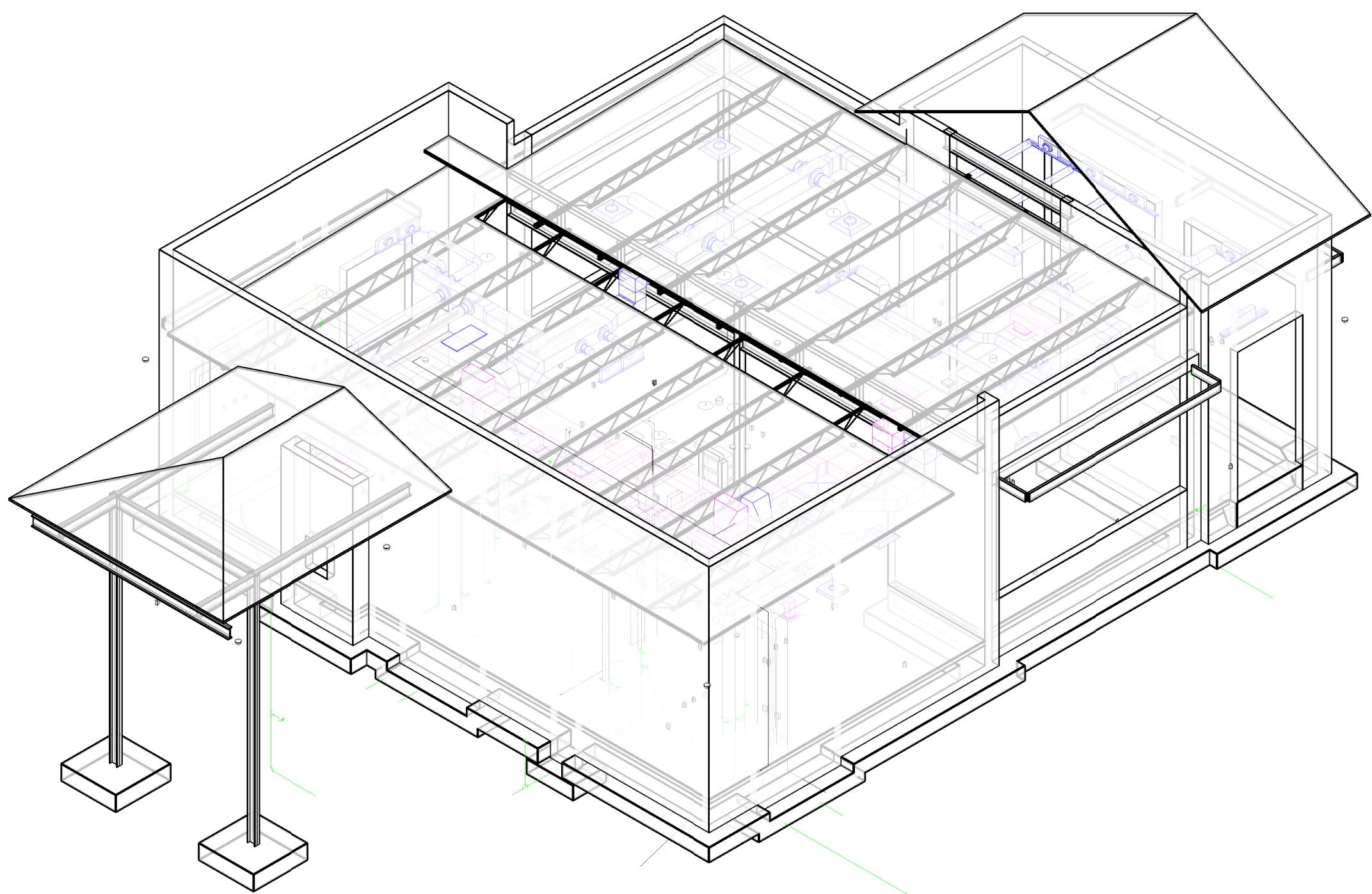
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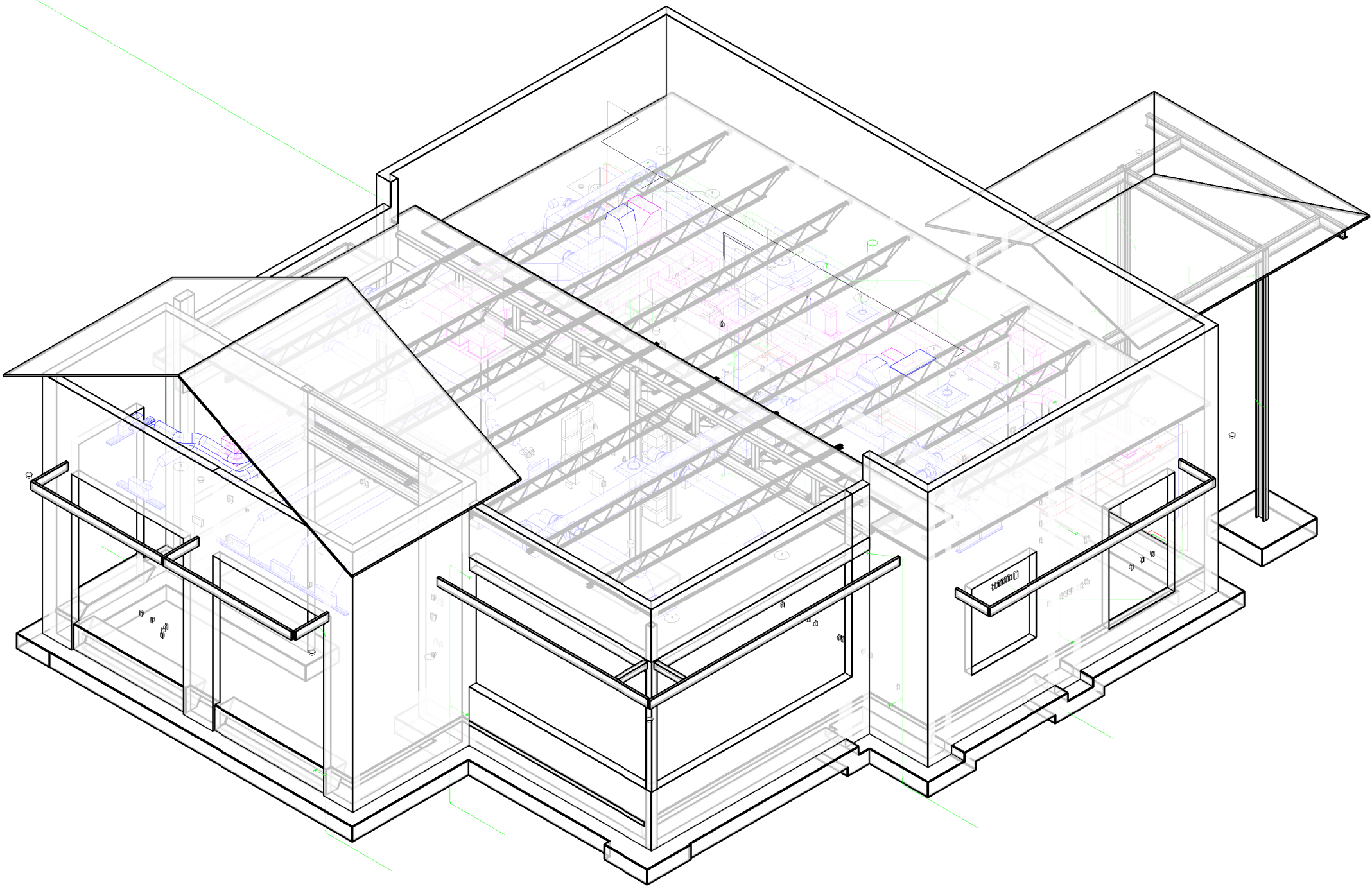
ISOMETRIC VIEWS

SHEET NUMBER

S01.8



Isometric View 1



Isometric View 2

NOTE:
ISOMETRIC IS SHOWN FOR VISUAL PURPOSES ONLY.
NOT ALL ELEMENTS AND MEMBERS HAVE BEEN SHOWN
FOR CLARITY. ISOMETRICS SHALL NOT BE USED FOR
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Branch Foundation and Floor Plan

1/4" = 1'-0"

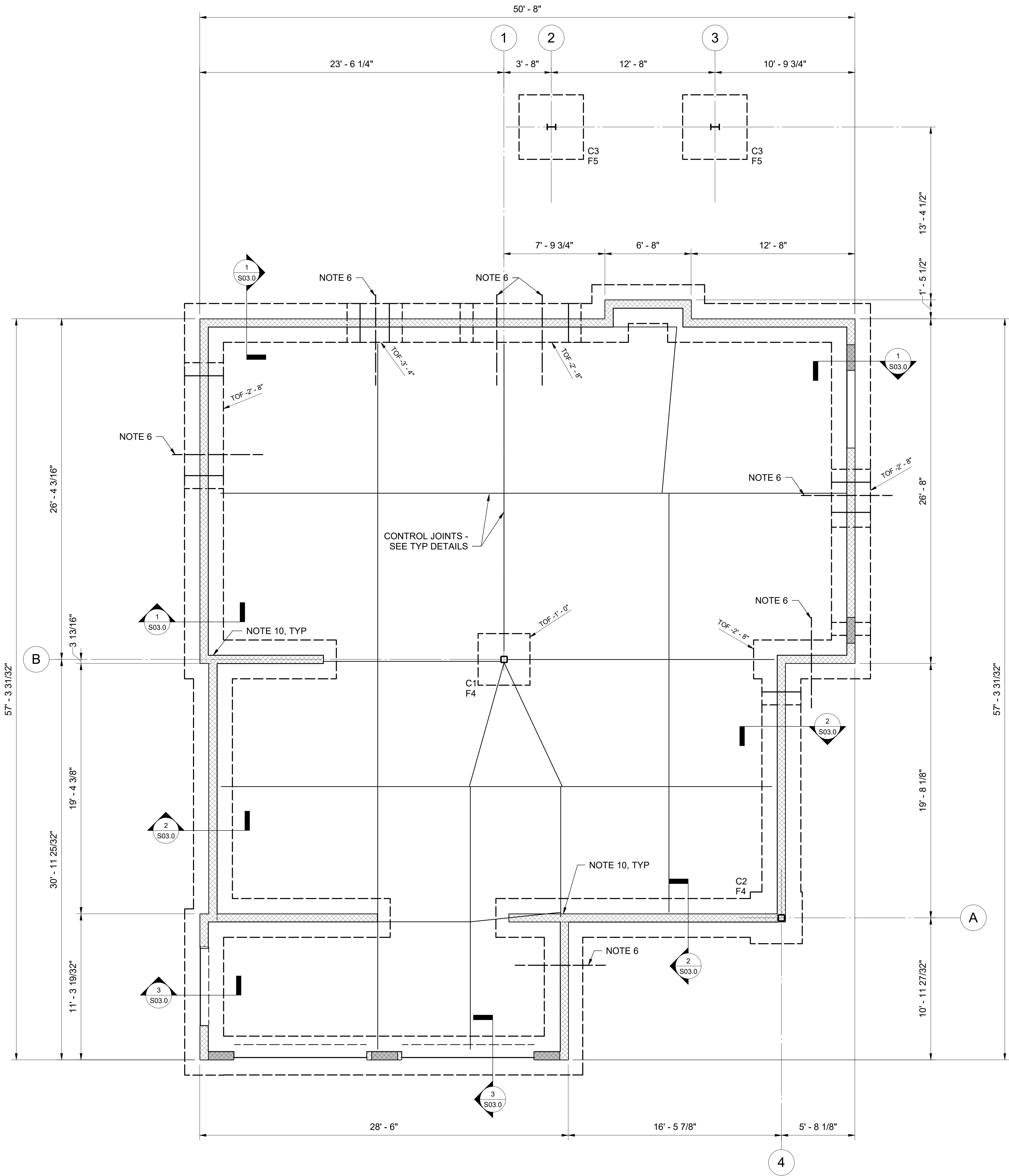
1. FINISH FLOOR (TOP OF SLAB) ELEVATION 0'-0", UNLESS NOTED.
2. TOP OF FOOTING: -2'-0", UNLESS NOTED.
3. FOR SLAB ON GRADE CONSTRUCTION, SEE GENERAL NOTES AND TYPICAL DETAILS. FOR SLAB EDGES NOT DIMENSIONED, SEE ARCHITECTURAL DRAWINGS.
4. FOR SLAB RECESS, SEE ARCHITECTURAL DRAWINGS.
5. GENERAL CONTRACTOR SHALL COORDINATE TILE JOINT LOCATIONS WITH CONTROL JOINTS.
6. FOOTING STEP ELEVATIONS AND LOCATION ARE APPROXIMATE. GENERAL CONTRACTOR SHALL COORDINATE ALL FOOTING STEPS WITH CIVIL, PLUMBING AND UTILITY DRAWINGS.
7. PROVIDE 2#5x6'-0" AT ALL RE-ENTRANT CORNERS AND DISCONTINUOUS CONTROL JOINTS.
8. PROVIDE ISOLATION JOINT AROUND COLUMN THROUGH SLAB AND CURB, TYPICAL AT DRIVE-THRU.
9. BUILD MASONRY CONNECTION ACROSS WALLS AT THREE LOCATIONS.
10. AT BEAM BEARING, PROVIDE (3) CELLS GROUTED SOLID FROM BEAM BEARING TO FOUNDATION. SEE TYPICAL DETAILS FOR ADDITIONAL INFORMATION.
11. INDICATES LOCATION WHERE SECTION 3/S04.1 APPLIES. PROVIDE (3) CELLS REINFORCED WITH 1#5 EACH AND GROUTED SOLID FROM BEAM ELEVATION TO FOUNDATION.

Structural Column Schedule

DESIGNATION	SIZE	BASE PLATE	ANCHOR RODS	NOTES
C1	HSS6x6x3/8	3/4x12x1'-0"	(4)3/4"Ø, 9" EMBEDMENT	
C2	HSS8x8x3/8	3/4x14x1'-2"	(4)3/4"Ø, 9" EMBEDMENT	
C3	W8x18	3/4x14x1'-0"	(4)3/4"Ø, 9" EMBEDMENT	

Structural Foundation Schedule

DESIGNATION	LENGTH	WIDTH	THICKNESS	TOP REINF	BOTTOM REINF	NOTES
F4	4' - 0"	4' - 0"	1' - 2"	5#5 EW	5#5 EW	
F5	5' - 0"	5' - 0"	1' - 4"	6#5 EW	6#5 EW	



[illegible]

SHEET TITLE
BRANCH ROOF FRAMING PLAN



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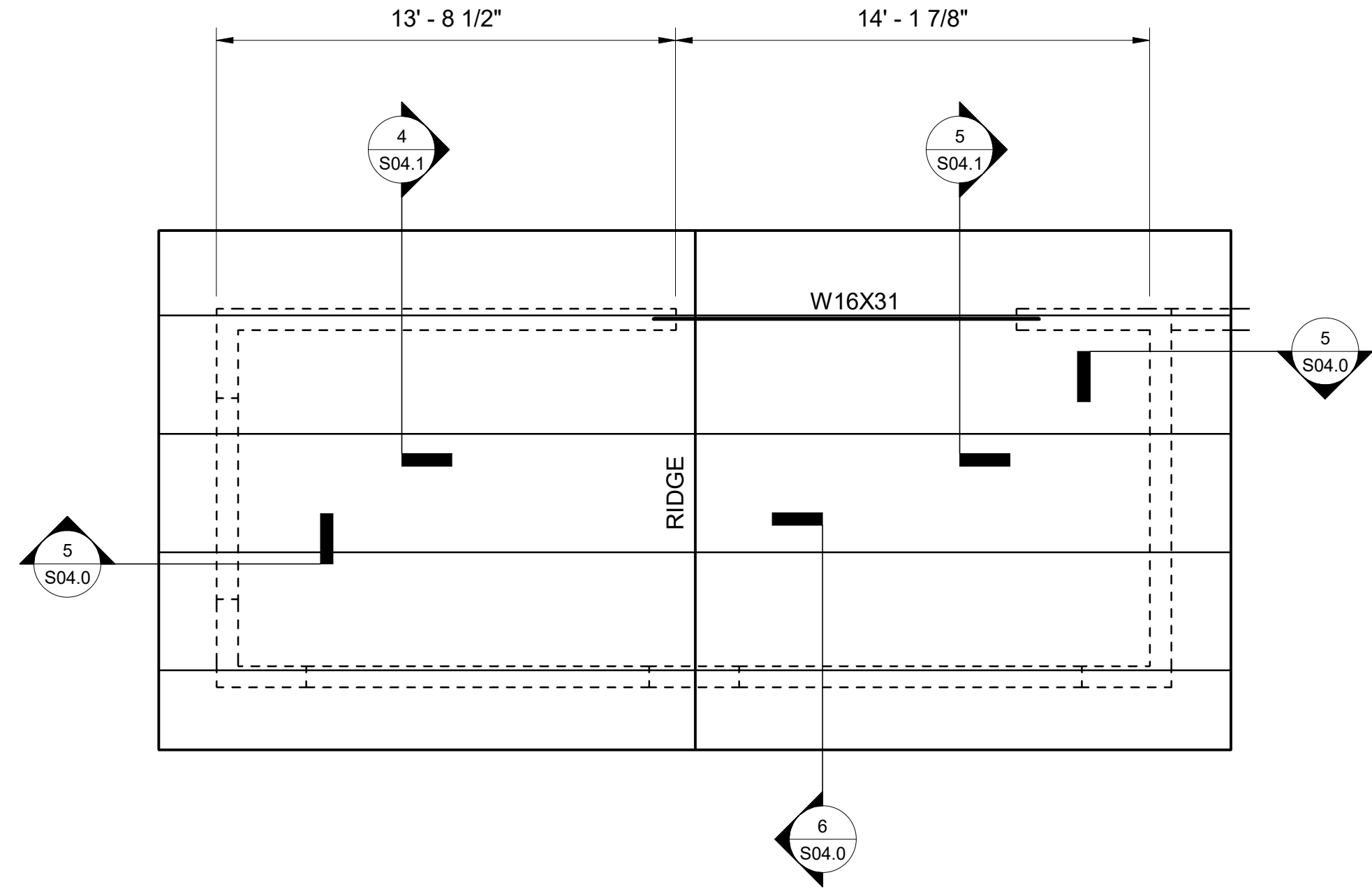
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SHEET TITLE

HIGH ROOF FRAMING
PLAN

SHEET NUMBER

S02.2



High Roof Framing Plan

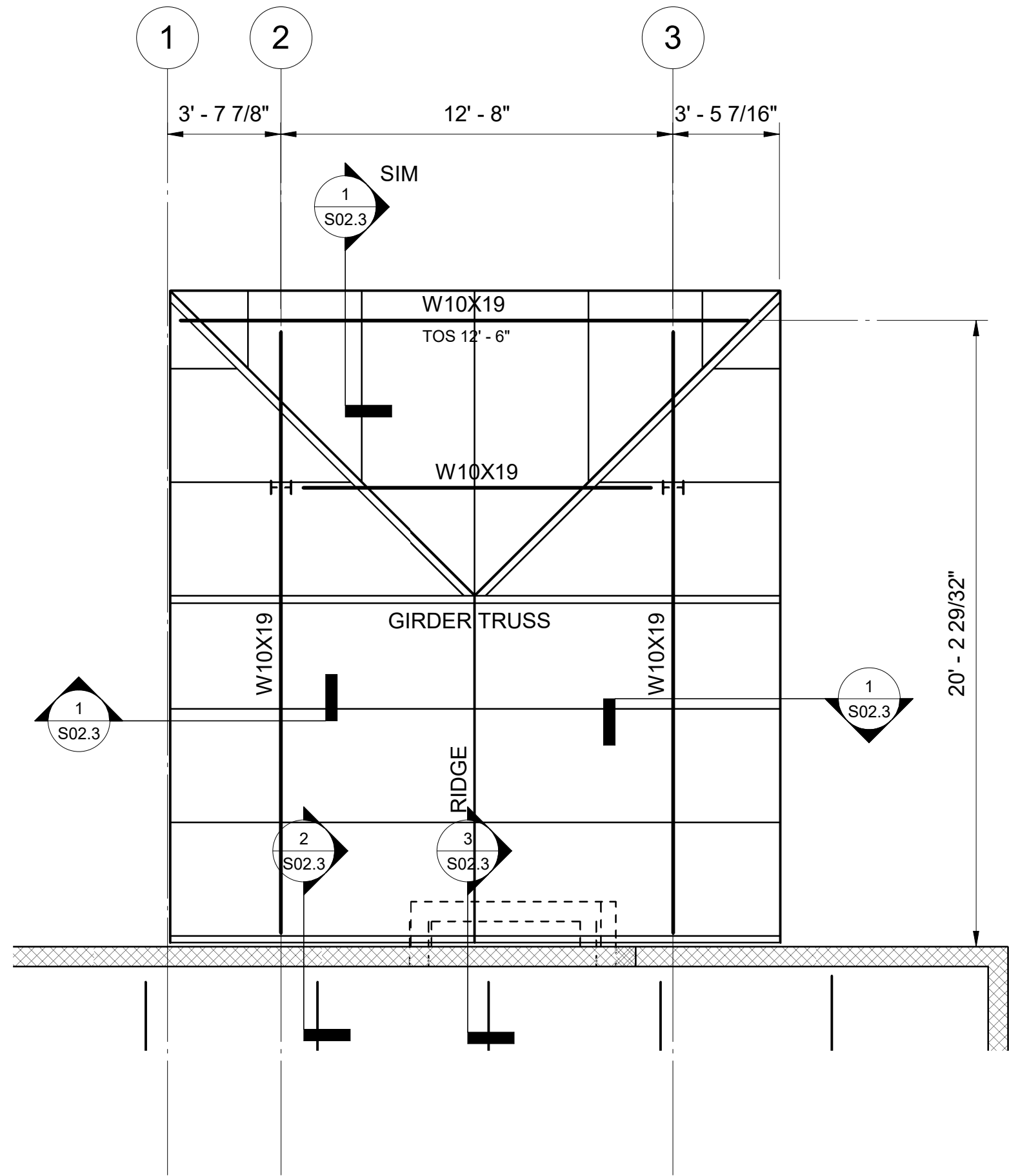
1/4" = 1'-0"

NOTES:

1. TRUSS BEARING ELEVATION: 18'-8".
2. ROOF SYSTEM: 1 1/2" ROOF DECK ON PRE-FABRICATED COLD FORMED METAL ROOF TRUSSES ON LOAD-BEARING CMU WALLS. TRUSS SPACING IS 4'-0" MAXIMUM. SEE GENERAL NOTES.
3. TRUSS ROOF SYSTEM:
 - A. TRUSS LAYOUT SHOWN FOR GENERAL INTENT. EXACT LAYOUT OF COLD-FORMED STEEL TRUSSES TO BE DETERMINED BY THE TRUSS MANUFACTURER AND SUBMITTED TO THE STRUCTURAL ENGINEER.
 - B. SEE GENERAL NOTES AND TYPICAL DETAILS FOR DEAD LOAD, LIVE LOAD, AND WIND LOAD ON TRUSSES.
 - C. TRUSS TEMPORARY AND PERMANENT LATERAL BRACING MUST BE DESIGNED BY THE TRUSS MANUFACTURER.
 - D. COLD-FORMED TRUSS MANUFACTURER TO PROVIDE BLOCKING BETWEEN TRUSSES CAPABLE OF TRANSFERRING 300 PLF LATERAL LOAD FROM ROOF DIAPHRAGM INTO SHEAR WALL.
4. HANGER LOCATIONS FOR PIPING LARGER THAN 3 INCHES IN DIAMETER MUST BE COORDINATED BY THE GENERAL CONTRACTOR WITH THE TRUSS MANUFACTURER. FOR WEIGHT OF PIPING AND ANY ADDITIONAL TRUSS REINFORCING, SEE TYPICAL DETAIL.

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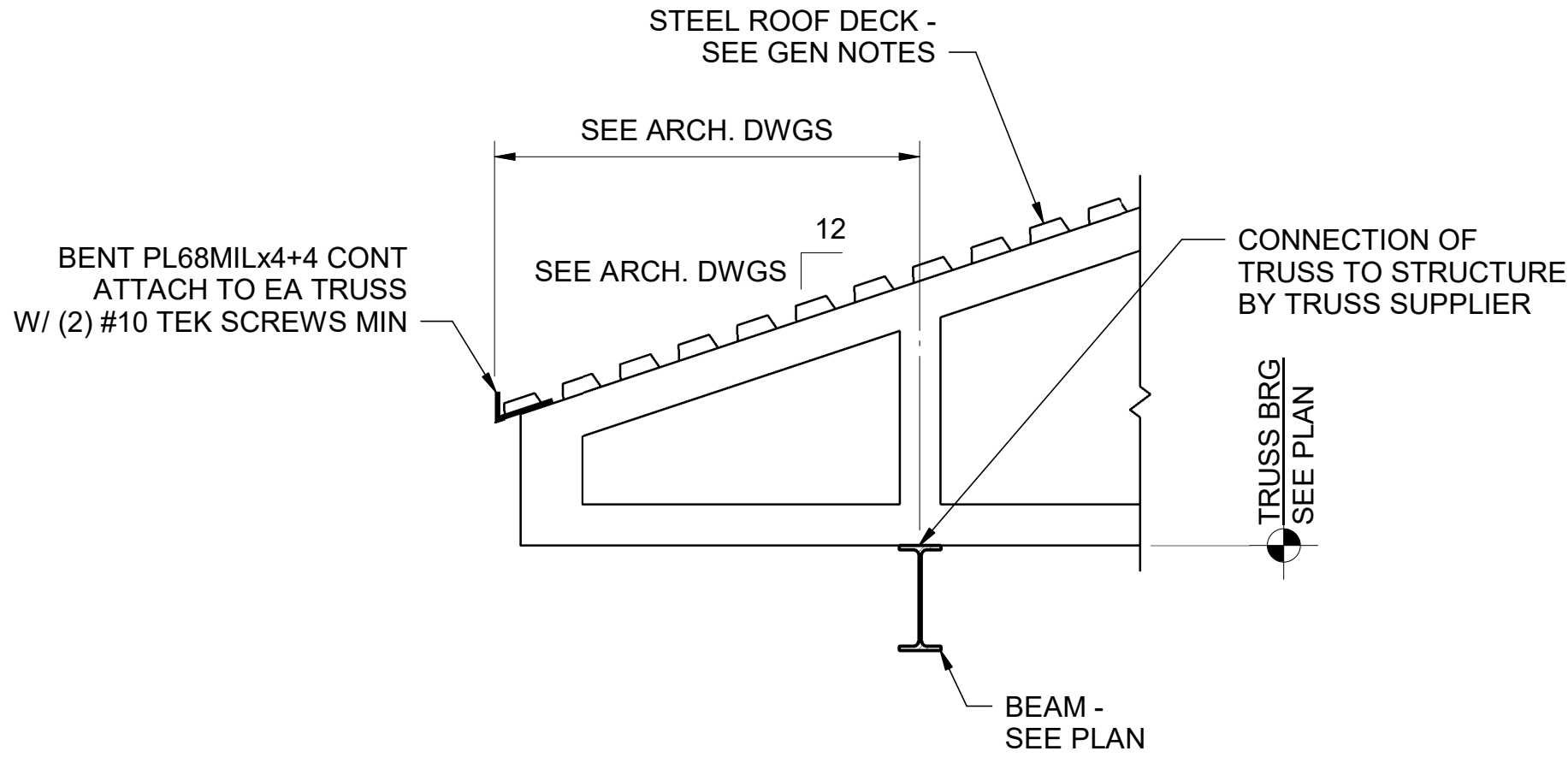


Drive Thru Roof Framing Plan

1/4" = 1'-0"

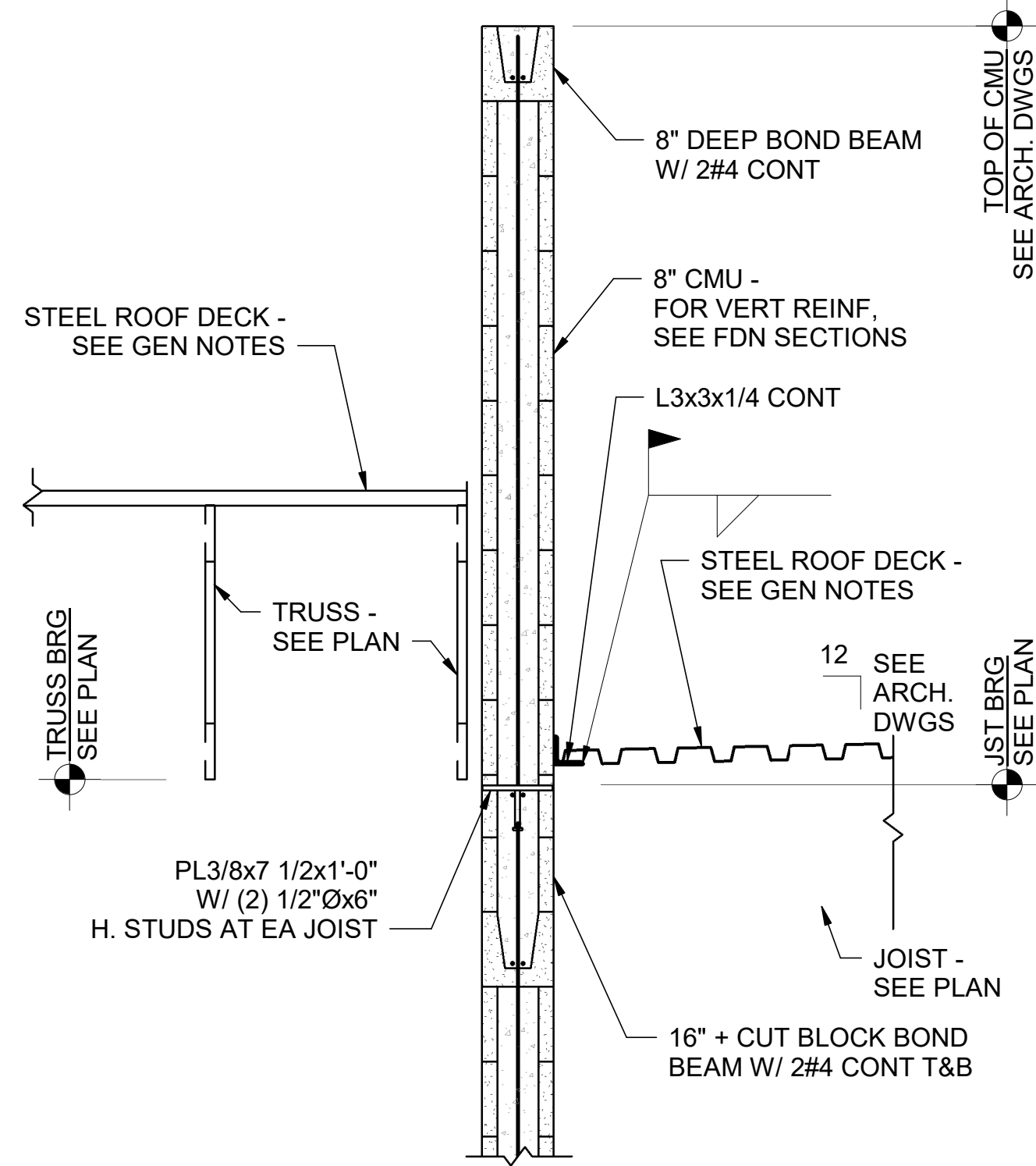
NOTES:

- TRUSS BEARING ELEVATION: 12'-6".
- ROOF SYSTEM: 1 1/2" ROOF DECK ON PRE-FABRICATED COLD FORMED METAL ROOF TRUSSES ON LOAD-BEARING CMU WALLS. TRUSS SPACING IS 4'-0" MAXIMUM. SEE GENERAL NOTES.
- TRUSS ROOF SYSTEM:
 - TRUSS LAYOUT SHOWN FOR GENERAL INTENT. EXACT LAYOUT OF COLD-FORMED STEEL TRUSSES TO BE DETERMINED BY THE TRUSS MANUFACTURER AND SUBMITTED TO THE STRUCTURAL ENGINEER.
 - SEE GENERAL NOTES AND TYPICAL DETAILS FOR DEAD LOAD, LIVE LOAD, AND WIND LOAD ON TRUSSES.
 - TRUSS TEMPORARY AND PERMANENT LATERAL BRACING MUST BE DESIGNED BY THE TRUSS MANUFACTURER.
 - COLD-FORMED TRUSS MANUFACTURER TO PROVIDE BLOCKING BETWEEN TRUSSES CAPABLE OF TRANSFERRING 300 PLF LATERAL LOAD FROM ROOF DIAPHRAGM INTO SHEAR WALL.
- HANGER LOCATIONS FOR PIPING LARGER THAN 3 INCHES IN DIAMETER MUST BE COORDINATED BY THE GENERAL CONTRACTOR WITH THE TRUSS MANUFACTURER. FOR WEIGHT OF PIPING AND ANY ADDITIONAL TRUSS REINFORCING, SEE TYPICAL DETAIL.



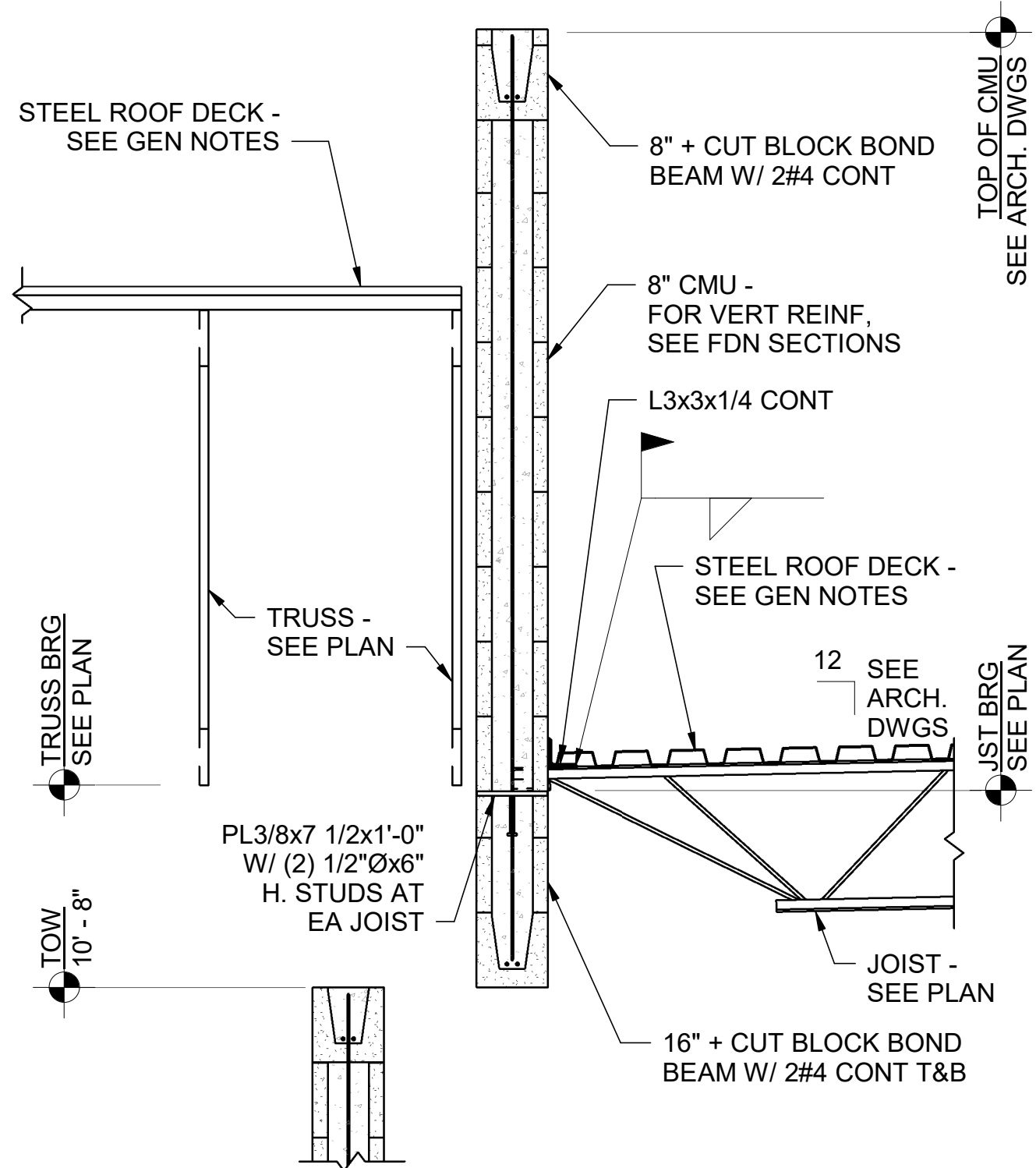
1 Section

S02.3 3/4" = 1'-0"



2 Section

S02.3 3/4" = 1'-0"



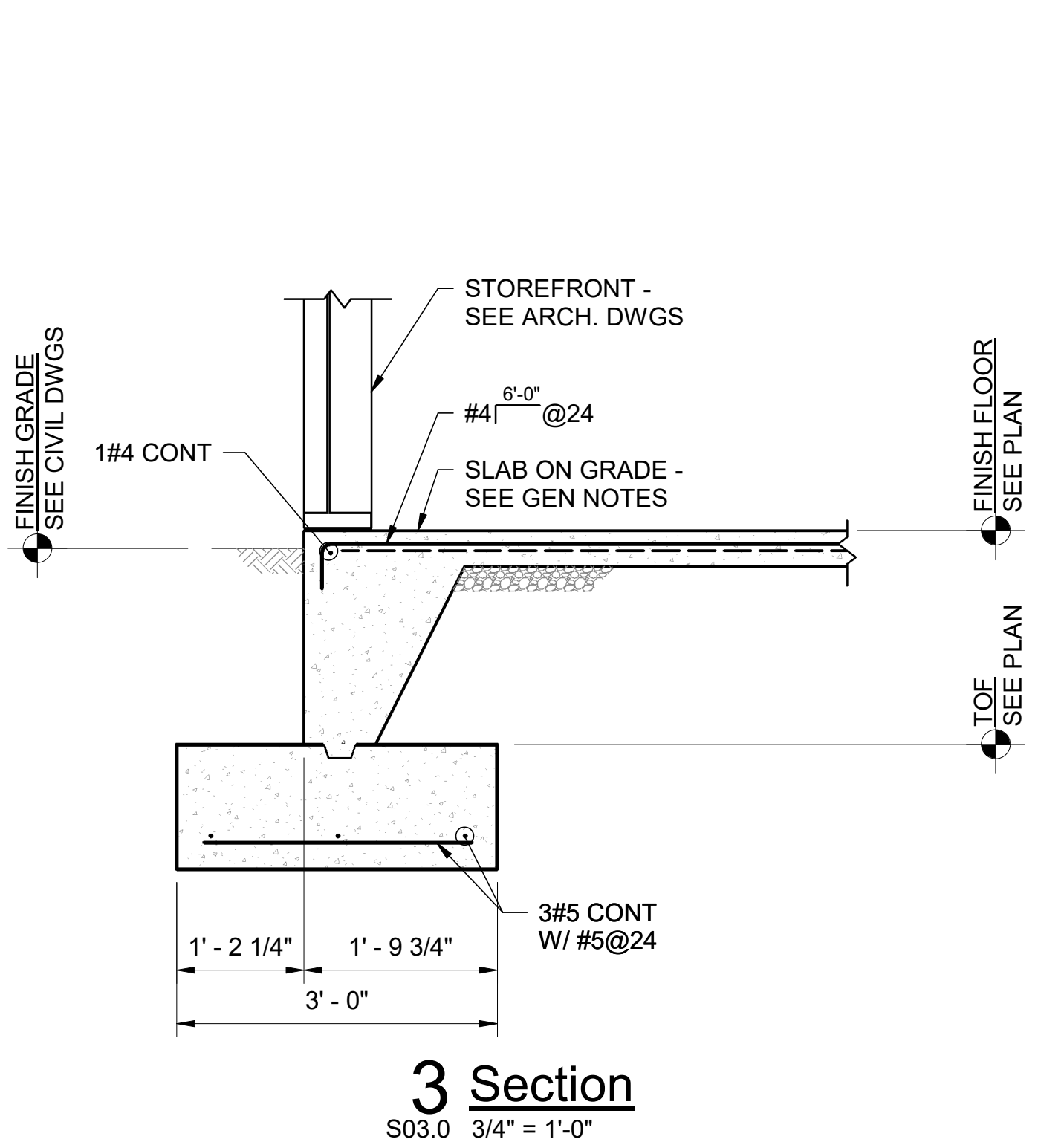
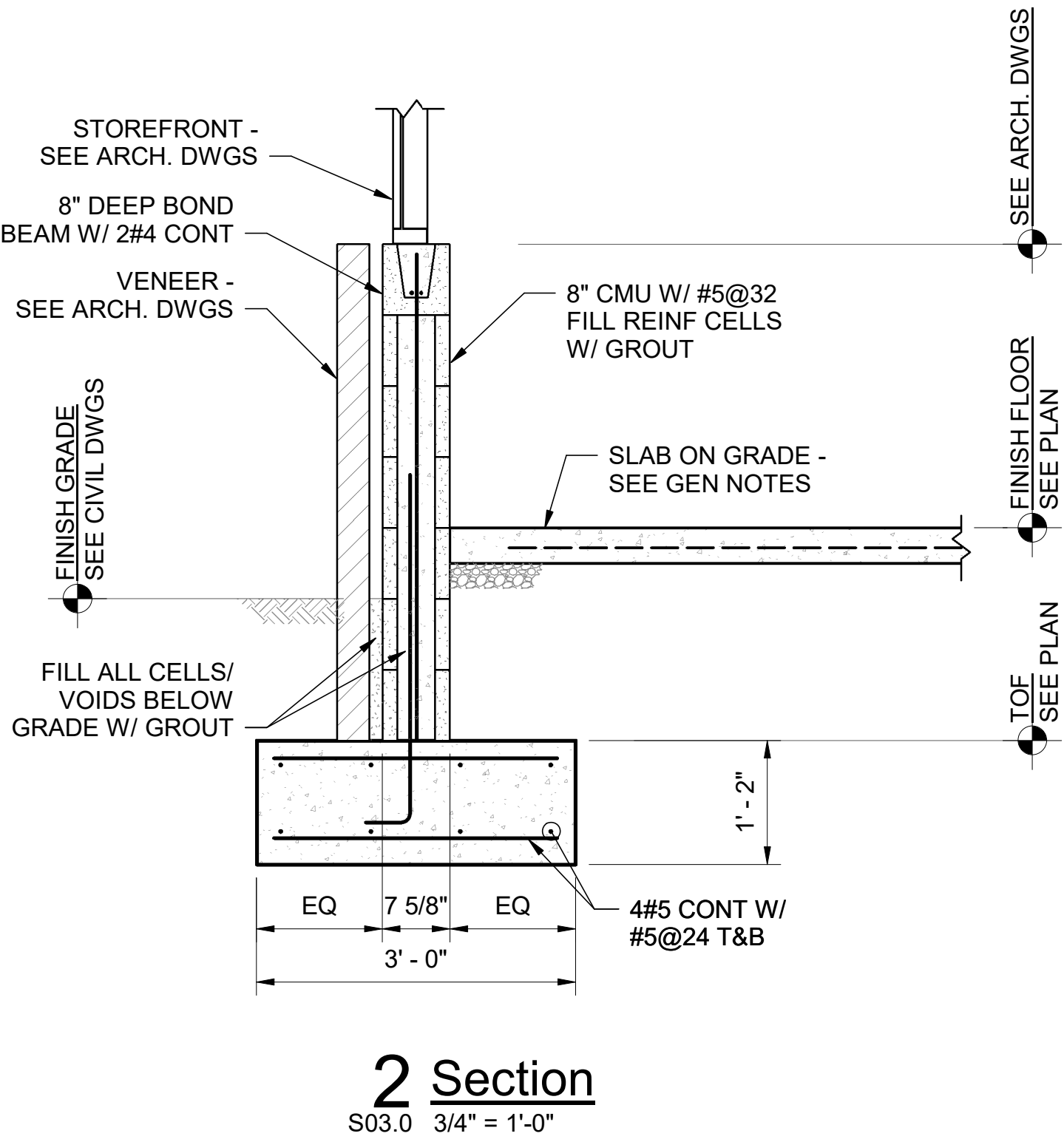
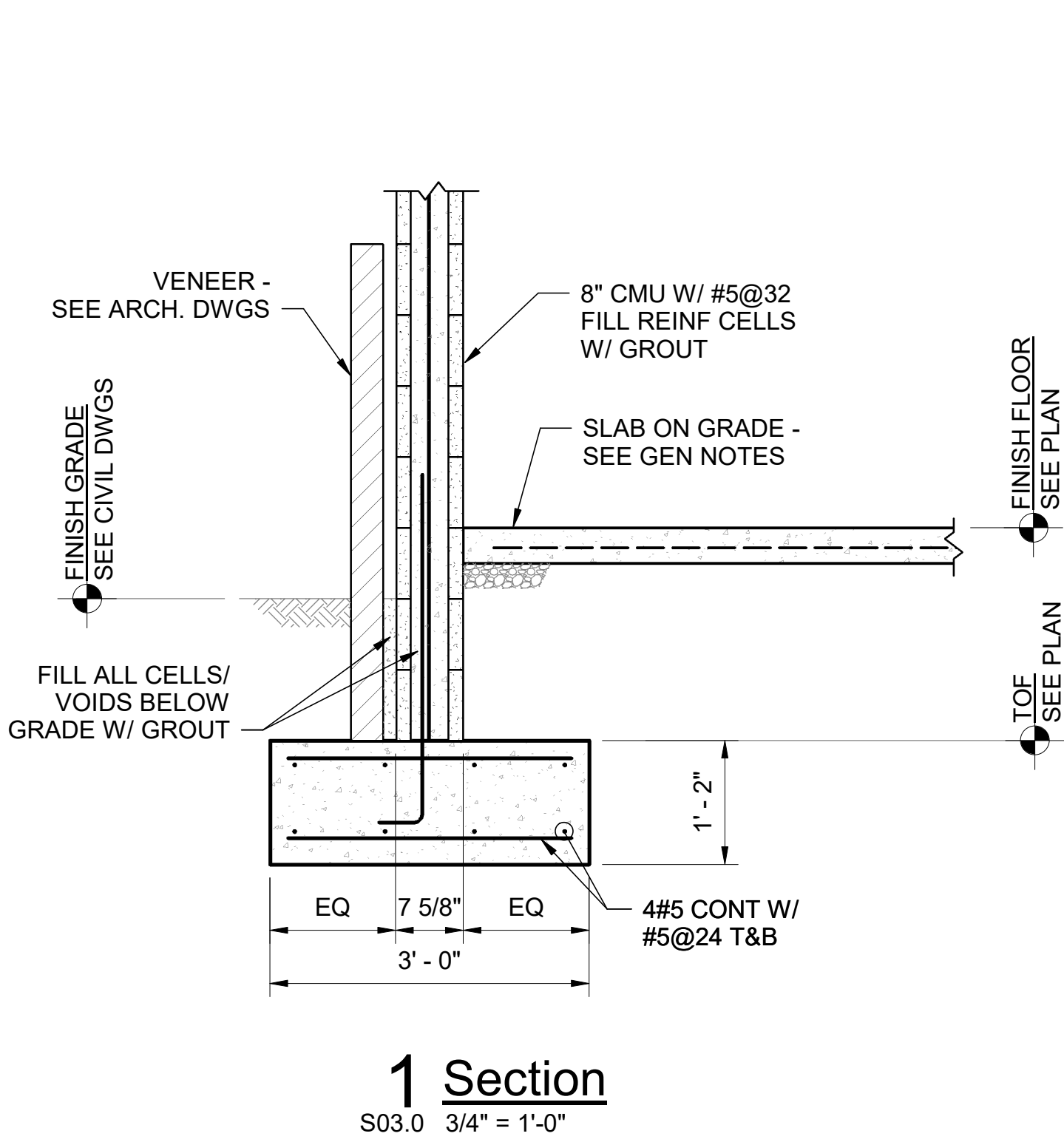
3 Section

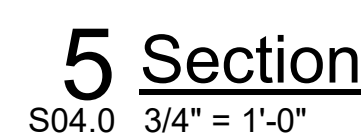
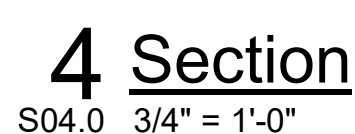
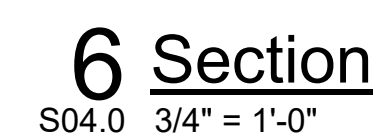
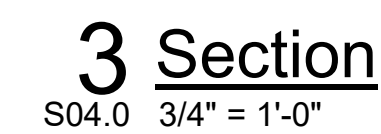
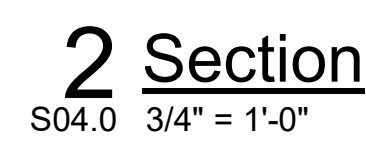
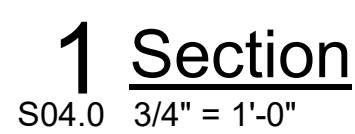
S02.3 3/4" = 1'-0"

ISSUE			
BY	DATE	DESCRIPTION	
-	11/15/22	Permit Set	
-			

PROJECT INFORMATION BLOCK	
JOB #	220666
DATE:	08/07/23
DRAWN BY:	ABG
CHECKED BY:	JDP

SHEET TITLE
FOUNDATION SECTIONS
SHEET NUMBER
S03.0





**FIFTH THIRD
BANK**

FIFTH THIRD BANK
JENSEN BEACH
4110 NW FEDERAL HIGHWAY
JENSEN BEACH, FL 34957

SEAL

PROJECT INFORMATION BLOCK	
JOB #	220666
DATE:	08/07/23
DRAWN BY:	ABG
CHECKED BY:	JDP

SHEET TITLE

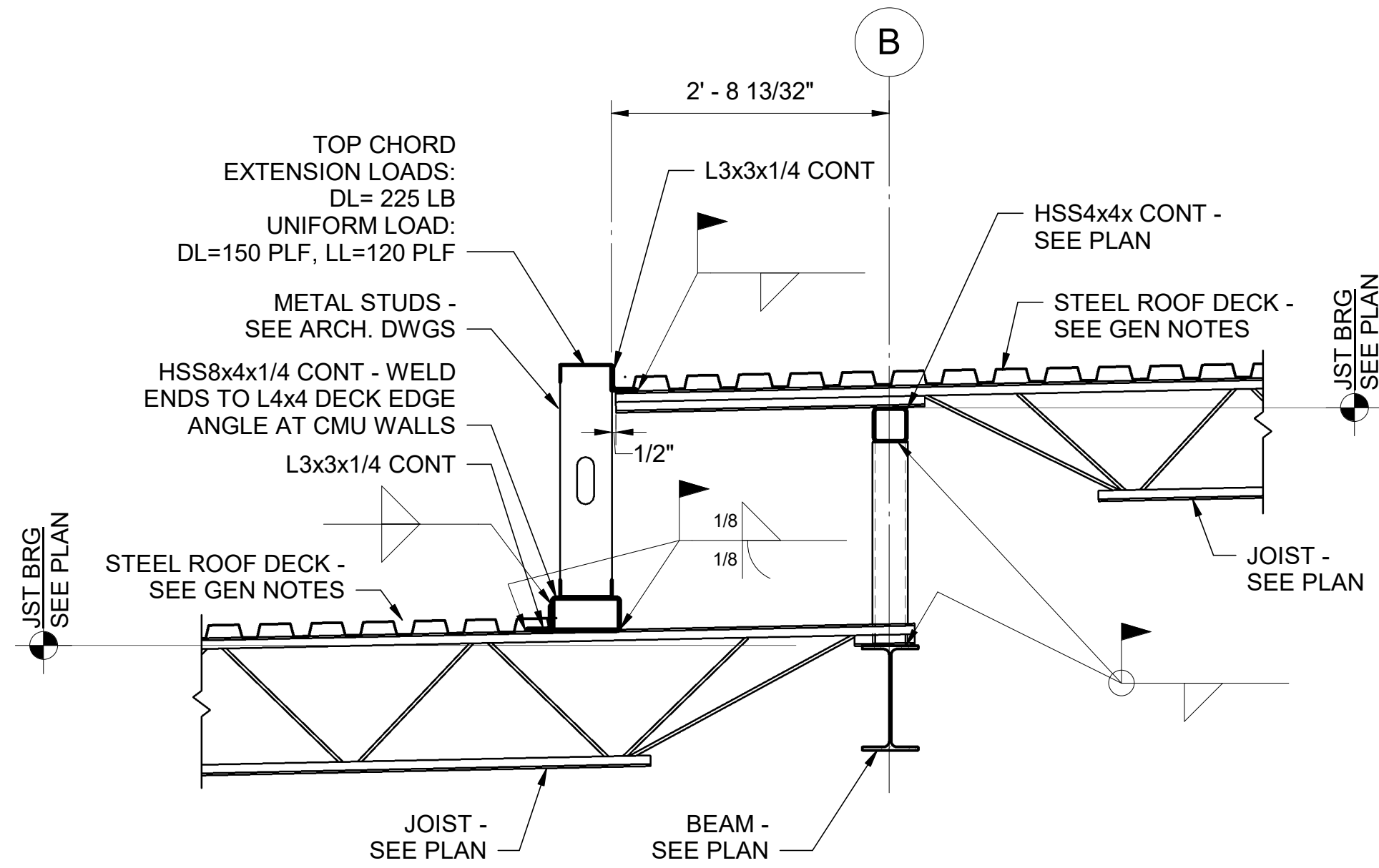
ROOF SECTIONS

SHEET NUMBER

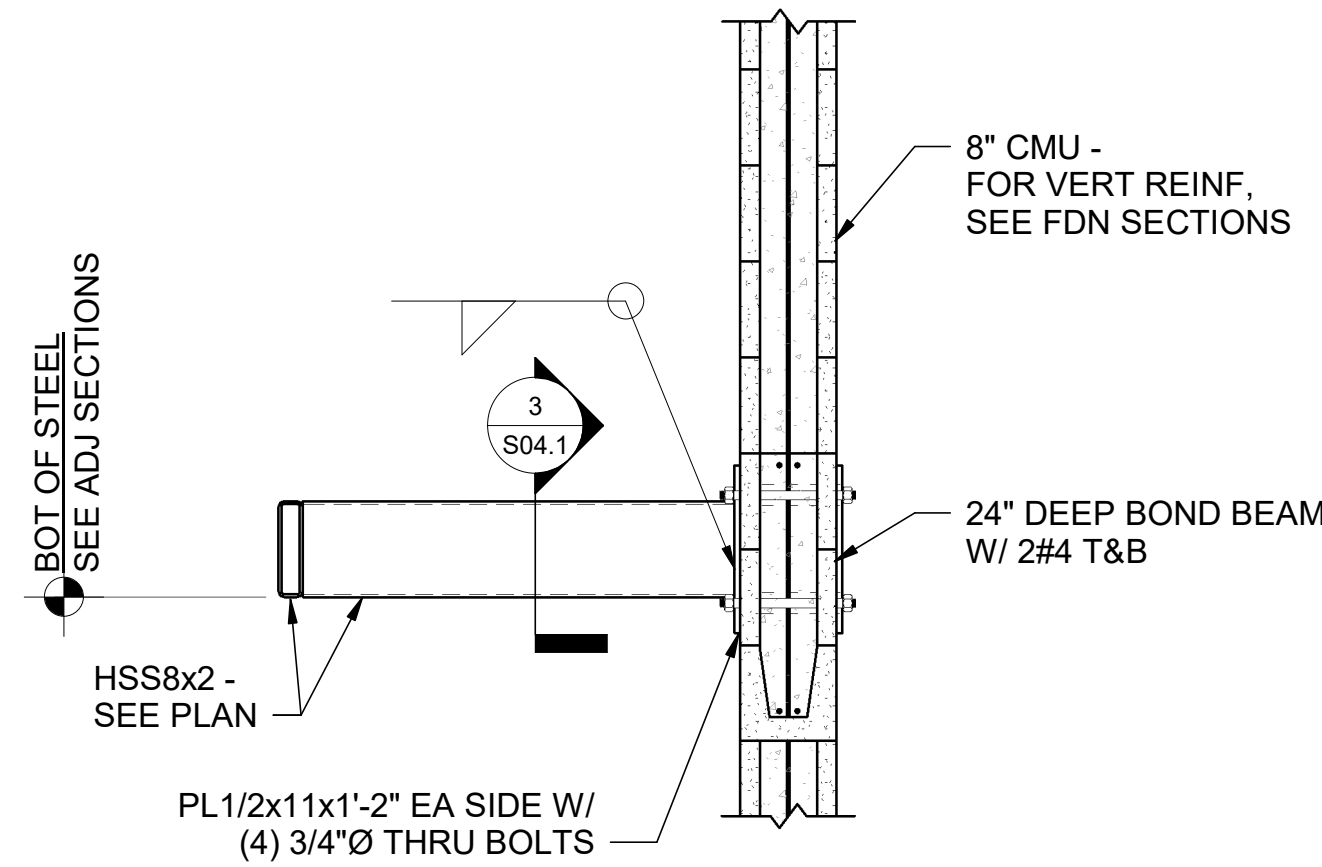
S04.0

ISSUE	BY	DATE	DESCRIPTION
-		11/15/22	Permit Set
-			
-			
-			
-			
-			
-			
-			
-			
-			

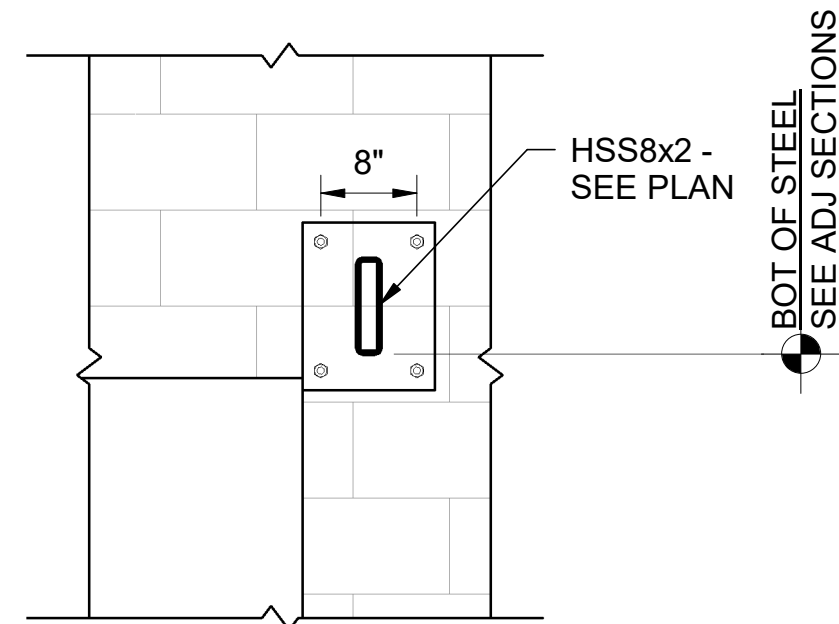
PROJECT INFORMATION BLOCK	
JOB #	220666
DATE:	08/07/23
DRAWN BY:	ABG
CHECKED BY:	JDP



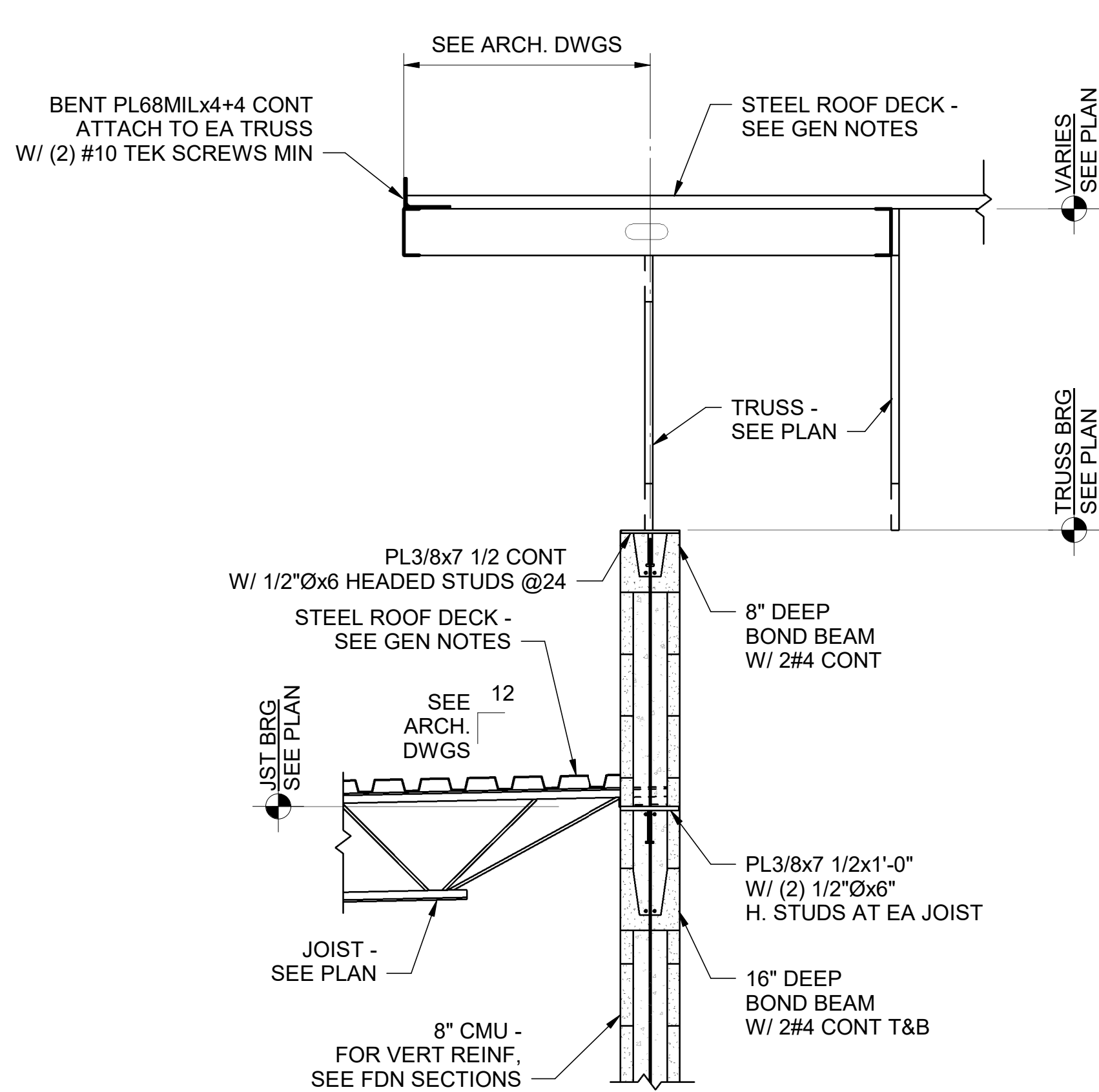
1 Section
S04.1 3/4" = 1'-0"



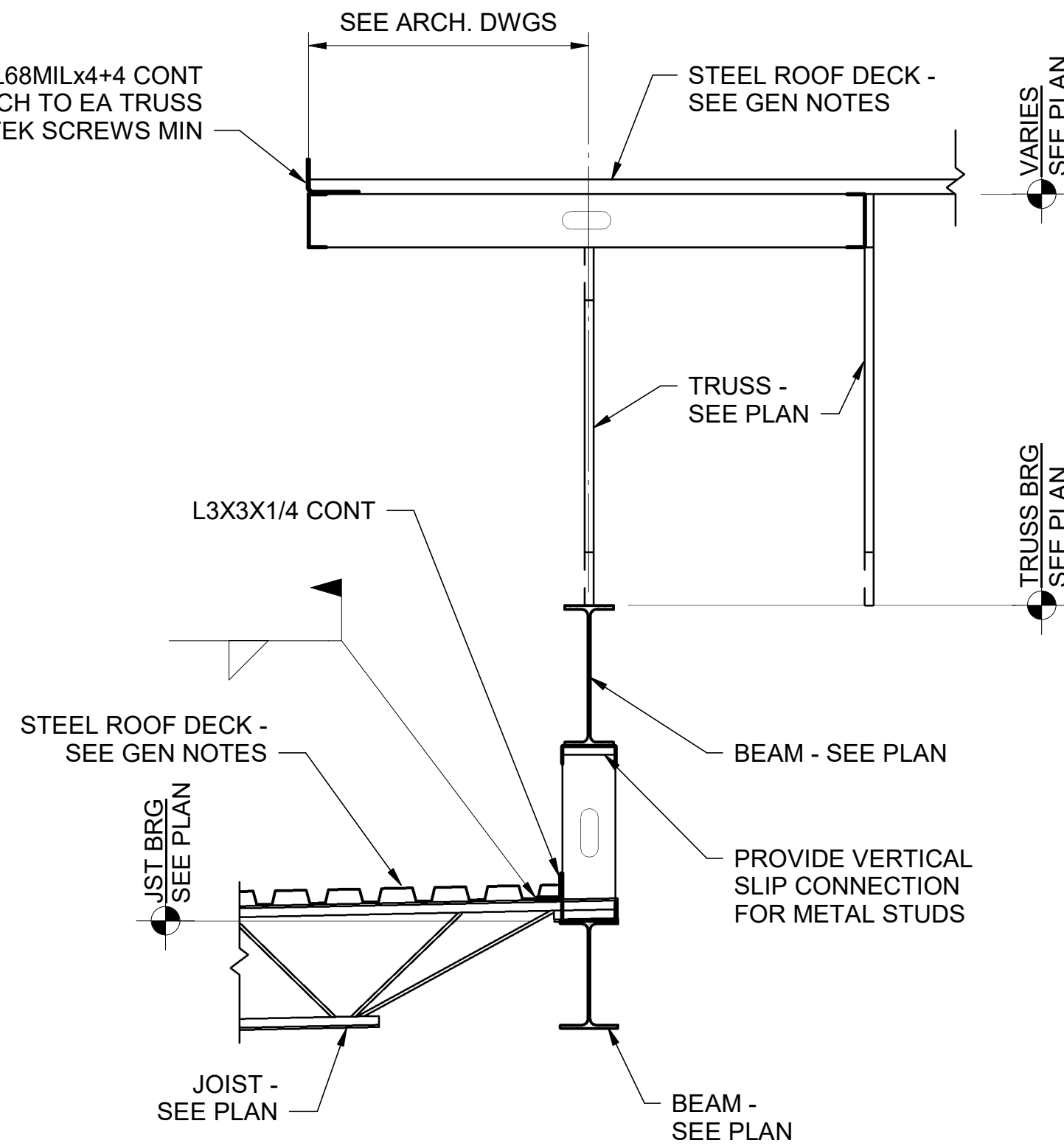
2 Section
S04.1 3/4" = 1'-0"



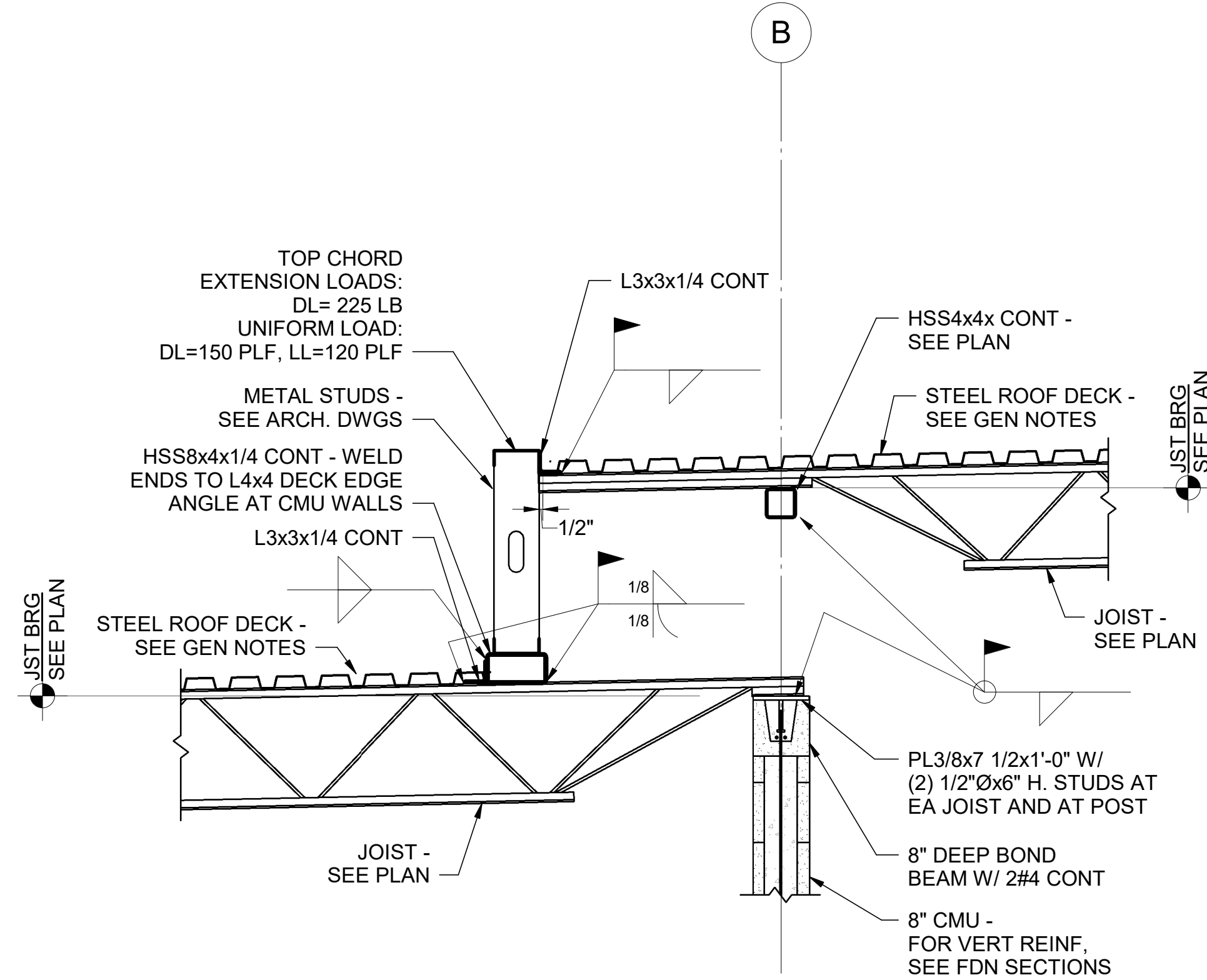
3 Section
S04.1 3/4" = 1'-0"



4 Section
S04.1 3/4" = 1'-0"



5 Section
S04.1 3/4" = 1'-0"



6 Section
S04.1 3/4" = 1'-0"