

GENERAL STRUCTURAL NOTES:

1. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ALL PROJECT DOCUMENTS AND SPECIFICATIONS. THE CONTRACTOR SHALL VERIFY THE REQUIREMENTS OF OTHER TRADES AS TO ADDITIONAL ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK.
2. THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE PROVISIONS OF THE FLORIDA BUILDING CODE, 8TH EDITION (2023).
3. THE WORK IS SUBJECT TO SPECIAL INSPECTIONS AS DESCRIBED IN THE SCHEDULE OF SPECIAL INSPECTIONS AND THE PROJECT SPECIFICATIONS.
4. THE CONTRACTOR SHALL PROVIDE TEMPORARY SHORING AND BRACING REQUIRED TO ERECT AND HOLD THE STRUCTURE IN PROPER ALIGNMENT UNTIL PERMANENT SUPPORTS AND LATERAL BRACING ARE IN PLACE.
5. DESIGN CRITERIA:

RISK CATEGORY	II
<u>LIVE LOADS - UNIFORM:</u>	
SLAB ON GRADE	100 PSF
ROOF	20 PSF
LIVE LOAD REDUCTION OF THE UNIFORMLY DISTRIBUTED ROOF LIVE LOADS HAS BEEN UTILIZED.	
<u>LIVE LOADS - CONCENTRATED:</u>	
ROOFS	300#
<u>WIND:</u>	
ULTIMATE DESIGN WIND SPEED	156 MPH
EXPOSURE CATEGORY	B
INTERNAL PRESSURE COEFFICIENT	±0.18
COMPONENT AND CLADDING PRESSURES:	
WALLS, ZONE 5 (10 SF)	49.9 PSF
ROOF, ZONE 3 (10 SF)	106.7 PSF
PARAPET, END/CORNER (10 SF)	132.6 PSF
<u>RAIN LOADS:</u>	
15 MINUTE DURATION	8.7 INCHES/HOUR
60 MINUTE DURATION	4.3 INCHES/HOUR

FOUNDATION NOTES:

1. FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE RECOMMENDATIONS IN THE REPORT OF GEOTECHNICAL EXPLORATION DATED NOVEMBER 2018 PREPARED BY NUTTING ENGINEERS.
2. FOUNDATIONS HAVE BEEN DESIGNED FOR A NET ALLOWABLE SOIL BEARING PRESSURE OF 4,000 PSF.
3. PRIOR TO PLACING FOUNDATION CONCRETE, ALL FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY THE SPECIAL INSPECTOR TO EXPLORE THE EXTENT OF LOOSE, SOFT, EXPANSIVE, OR OTHERWISE UNSATISFACTORY SOIL MATERIAL AND TO VERIFY DESIGN BEARING PRESSURE. DIRECTION FOR CORRECTIVE ACTION WILL BE PROVIDED WHERE REQUIRED.
4. NO UNBALANCED BACKFILLING SHALL BE DONE AGAINST MASONRY OR CONCRETE WALLS UNLESS WALLS ARE SECURELY BRACED AGAINST OVERTURNING, EITHER BY TEMPORARY CONSTRUCTION BRACING OR BY PERMANENT CONSTRUCTION.
5. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR CONTROL OF GROUNDWATER AND SURFACE RUNOFF THROUGHOUT THE CONSTRUCTION PROCESS. INUNDATION AND LONG TERM EXPOSURE OF BEARING SURFACES WHICH RESULT IN DETERIORATION OF BEARING SHALL BE PREVENTED.

CAST-IN-PLACE CONCRETE NOTES:

1. CONCRETE SHALL BE IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE (ACI) 301 AND 318.
2. CONCRETE SHALL BE NORMAL WEIGHT AND SHALL OBTAIN 28 DAY COMPRESSIVE STRENGTHS AS FOLLOWS:
- | | |
|---------------------------------|-----------|
| A. SLAB-ON-GRADE | 3,500 PSI |
| B. LINAC / PET CT VAULT | 4,000 PSI |
| C. CONCRETE NOT OTHERWISE NOTED | 3,000 PSI |
3. REINFORCING MATERIALS SHALL BE AS FOLLOWS:
- | |
|--|
| A. REINFORCING BARS - ASTM A 615, GRADE 60, DEFORMED. |
| B. WELDED REINFORCING BARS - ASTM A 706, GRADE 60. |
| C. WELDED WIRE REINFORCEMENT - ASTM A 185, WELDED STEEL WIRE REINFORCEMENT; PROVIDE SHEET TYPE, ROLL TYPE IS NOT ACCEPTABLE. |
4. ALL REINFORCING STEEL AND EMBEDDED ITEMS SUCH AS ANCHOR RODS AND WELD PLATES SHALL BE ACCURATELY PLACED AND ADEQUATELY TIED AND SUPPORTED BEFORE CONCRETE IS PLACED TO PREVENT DISPLACEMENT BEYOND PERMITTED TOLERANCES.
5. CONCRETE COVER TO REINFORCING BARS SHALL CONFORM TO THE MINIMUM COVER RECOMMENDATIONS IN ACI 318-14, UNLESS THE DRAWINGS SHOW GREATER COVER REQUIREMENTS.
6. LAP CONTINUOUS REINFORCING BARS 57 X BAR DIAMETER, TYPICAL UNLESS OTHERWISE NOTED.

CONCRETE MASONRY NOTES:

1. CONCRETE MASONRY MATERIALS AND CONSTRUCTION SHALL CONFORM TO THE AMERICAN CONCRETE INSTITUTE (ACI) 530.
2. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C 90 AND SHALL BE MADE WITH NORMAL WEIGHT AGGREGATE. MINIMUM NET AREA COMPRESSIVE STRENGTH OF MASONRY UNITS SHALL BE 2,000 PSI AT 28 DAYS.
3. COMPRESSIVE STRENGTH OF MASONRY SHALL BE DETERMINED BY THE UNIT STRENGTH METHOD AS SET FORTH IN ACI 530.1. THE NET AREA COMPRESSIVE STRENGTH OF MASONRY, F_m, SHALL BE 2,000 PSI AT 28 DAYS.
4. MORTAR SHALL BE TYPE M OR S AND SHALL COMPLY WITH ASTM C270, PROPORTIONS OR PROPERTIES SPECIFICATION.
5. GROUT SHALL COMPLY WITH ASTM C 476 PROPERTIES SPECIFICATION, AND SHALL BE PROPORTIONED TO OBTAIN A 28 DAY COMPRESSIVE STRENGTH OF 2,000 PSI.
6. REINFORCING STEEL SHALL COMPLY WITH ASTM A 615, GRADE 60. SHOP FABRICATE REINFORCING BARS WHICH ARE SHOWN TO BE BENT OR HOOKED.
7. ALL BOND BEAMS, REINFORCED CELLS AND CELLS WITH EXPANSION BOLTS, EMBED PLATES OR OTHER ANCHORS AND ALL CELLS BELOW AREA SHALL BE GROUTED SOLID. GROUT PROCEDURE SHALL COMPLY WITH ACI 530.1.
8. PROVIDE REINFORCING BARS OF THE GIVEN SIZE AND SPACING SHOWN. LAP CONTINUOUS REINFORCING STEEL 48 BAR DIAMETERS UNLESS OTHERWISE NOTED.
9. PROVIDE ONE VERTICAL BAR EACH SIDE OF ALL OPENINGS AND CONTROL JOINTS, AND AT CORNERS AND INTERSECTIONS OF ALL MASONRY WALLS. SHOW CONTROL JOINT LOCATIONS ON THE REINFORCING STEEL SHOP DRAWINGS.
10. PROVIDE REINFORCING STEEL DOWELS OF THE SAME SIZE AND SPACING AS VERTICAL REINFORCING FROM THE SUPPORTING STRUCTURE. DOWELS SHALL HAVE STANDARD ACI HOOKS.
11. PROVIDE STANDARD 9 GAGE LADDER TYPE HORIZONTAL JOINT REINFORCING IN CMU WALLS AT 16 INCHES ON CENTER.
12. PROVIDE HORIZONTAL BOND BEAMS WITH CONTINUOUS REINFORCING AS SHOWN IN THE SECTIONS AND DETAILS.

STRUCTURAL STEEL NOTES:

1. STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 360.
2. STRUCTURAL STEEL SHALL COMPLY WITH THE FOLLOWING SPECIFICATIONS:
- | |
|---|
| A. STRUCTURAL STEEL SHAPES, PLATES AND BARS (EXCEPT W-SHAPES) -ASTM A 36, Fy = 36 KSI |
| B. STRUCTURAL STEEL W-SHAPES - ASTM A 992/A572, GRADE 50, Fy = 50 KSI |
| C. HOLLOW STRUCTURAL SHAPES (HSS): SQUARE AND RECTANGULAR - ASTM A 500, GRADE C, Fy = 50 KSI ROUND - ASTM A 500, GRADE B, Fy = 42 KSI |
| D. ANCHOR RODS - ASTM F 1554, GRADE 36 OR 105 AS INDICATED |
| E. HIGH STRENGTH BOLTS - ASTM A325 (TYPICAL UON) |
3. UNLESS OTHERWISE NOTED CONNECTIONS SHALL BE AISC TYPE 2 PR "STANDARD FRAMED BEAM CONNECTIONS" WITH ASTM A 325 BOLTS, UNLESS OTHERWISE NOTED. DESIGNED FOR ONE-HALF THE UNIFORM LOAD CONSTANTS FOR LATERALLY SUPPORTED BEAMS GIVEN IN PART 3 OF THE "STEEL CONSTRUCTION MANUAL".
4. HIGH STRENGTH BOLTS MAY BE TIGHTENED TO THE "SNUG TIGHT" CONDITION IN LIEU OF FULL PRETENSIONING, EXCEPT FOR THE FOLLOWING CONNECTIONS WHICH SHALL BE FULLY PRETENSIONED:
- | |
|---|
| A. BOLTED CONNECTIONS USING NON-STANDARD HOLES. |
|---|
5. PROVIDE L3X3X1/4 ANGLE FRAMING AROUND OPENINGS LARGER THAN 6 INCHES IN ANY DIMENSION (INCLUDING ROOF DRAINS AND MULTIPLE PIPES) TO SUPPORT STEEL DECK, TYPICAL UNLESS OTHERWISE NOTED OR DETAILED.
6. WELDING SHALL BE IN ACCORDANCE WITH AWS D1.1, "STRUCTURAL WELDING CODE - STEEL". WELD ELECTRODES SHALL BE E70XX LOW HYDROGEN, UNLESS OTHERWISE NOTED. PROVIDE CONTINUOUS FILLET WELDS WITH MINIMUM SIZE REQUIRED BY TABLE J2.4, PART 4 OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 360.
7. COORDINATE ALL MEMBER LOCATIONS, UNIT WEIGHTS, OPENING SIZES, AND CURB DIMENSIONS FOR MECHANICAL EQUIPMENT WITH THE ACTUAL EQUIPMENT FURNISHED.
8. HOT DIP GALVANIZE AFTER FABRICATION THE FOLLOWING:
- | |
|--|
| A. SHELF ANGLES SUPPORTING MASONRY IN EXTERIOR WALLS. |
| B. LINTELS AND LINTEL ASSEMBLIES SUPPORTING MASONRY IN EXTERIOR WALLS. |
| C. ALL STEEL EXPOSED TO WEATHER IN THE FINAL CONSTRUCTION. |
| D. ITEMS IDENTIFIED AS GALVANIZED ON ARCHITECTURAL OR STRUCTURAL DRAWINGS. |
| E. ALL CANOPY COLUMNS AND ANCHORAGE. |
9. ALL MEMBERS AND WELDS EXPOSED TO VIEW IN THE FINISHED CONSTRUCTION SHALL BE CONSIDERED ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS), WELDS SHALL BE GROUND SMOOTH.
10. STEEL MEMBERS SHALL BE SPLICED ONLY WHERE INDICATED. CONTINUOUS MEMBERS SHALL BE SPLICED OVER SUPPORTS, UNLESS OTHERWISE NOTED.
11. ALL EXPOSED STEEL AND ALL CANOPY STEEL SHALL BE PRIME PAINTED UNLESS OTHERWISE NOTED (COORDINATE WITH ARCHITECTURAL DRAWINGS). PRIME PAINTING OF REMAINING STRUCTURAL STEEL SHALL BE AN ADD ALTERNATE.

STEEL DECK NOTES:

1. STEEL DECK SHALL BE IN ACCORDANCE WITH THE AMERICAN IRON AND STEEL INSTITUTE (AISI), "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" AND THE STEEL DECK INSTITUTE (SDI), "DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS, AND ROOF DECKS."
2. STEEL DECK INSTALLATION SHALL COMPLY WITH THE FOLLOWING:
- | |
|---|
| A. ROOF DECK: 1 1/2" X 22 GAGE TYPE B GALVANIZED UNLESS OTHERWISE NOTED, ATTACH DECK TO SUPPORTS WITH 5/8 INCH DIAMETER PUDDLE WELDS IN ALL RIBS WHERE SIDELAPS OCCUR AND AT 6 INCHES ON CENTER ALONG SUPPORTS WITH A 36/7 PATTERN AND 6" OC AT ROOF PERIMETER FRAMING WITH A 36/7 PATTERN. FASTEN SIDE LAPS WITH 5- # 10 SELF-TAPPING HEX HEAD SCREWS EQUALLY SPACED BETWEEN SUPPORTS. FASTEN EDGEMOST DECK PANEL TO STEEL FRAMING WITH 5/8 INCH DIAMETER PUDDLE WELDS AT 6" OC. |
|---|
3. STEEL DECK SHALL BE INSTALLED PERPENDICULAR TO SUPPORTS AND SHALL HAVE A MINIMUM OF THREE CONTINUOUS SPANS. ENDLAPS SHALL ONLY OCCUR AT SUPPORTS.
4. WELDING SHALL BE IN ACCORDANCE WITH AWS D1.3 "STRUCTURAL WELDING CODE - SHEET STEEL".
5. PERMANENT SUSPENDED LOADS SHALL NOT BE SUPPORTED BY STEEL ROOF DECK.

STEEL JOIST NOTES:

1. STEEL JOISTS SHALL BE IN ACCORDANCE WITH THE STEEL JOIST INSTITUTE (SJI) STANDARD SPECIFICATIONS.
2. STEEL JOISTS DESIGNED AS 'SP' ON PLANS ARE SPECIAL JOISTS WHICH SHALL BE DESIGNED FOR SPECIAL CRITERIA INDICATED.
3. JOIST BRIDGING SHALL CONFORM TO SJI SPECIFICATIONS, INCLUDING BRIDGING REQUIRED FOR JOISTS SUBJECTED TO UPLIFT LOADS. PROVIDE CROSS-BRIDGING AT ENDS OF BRIDGING LINES AND CHANGES IN JOIST DEPTHS AND AT ROLLED STEEL SHAPES RUNNING PARALLEL TO JOISTS. BRIDGING SHOWN SHALL BE PROVIDED, IN ADDITION TO THE REQUIRED STANDARD BRIDGING. ENDS OF ALL BRIDGING LINES SHALL BE ANCHORED TO WALLS OR BEAMS. EXTEND BRIDGING LINES THROUGH AREAS WITH ROLLED STEEL SHAPES.
4. ROOF JOISTS SHALL BE DESIGNED FOR A NET UPLIFT LOADS OF 15 PSF (ASD).
5. ALL JOIST CHORDS SHALL BE DESIGNED FOR A MINIMUM CONCENTRATED LOAD OF 300 LBS. HUNG FROM THE JOIST TOP OR BOTTOM CHORD AT ANY POINT ALONG THE SPAN.
6. PERMANENT SUSPENDED LOADS SHALL NOT BE SUPPORTED BY JOIST BRIDGING.
7. COMPLY WITH OSHA SAFETY STANDARDS FOR THE ERECTION OF STEEL JOISTS.

COLD-FORMED METAL FRAMING NOTES:

1. COLD-FORMED METAL FRAMING SHALL BE IN ACCORDANCE WITH THE AMERICAN IRON AND STEEL INSTITUTE (AISI) "NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS".
2. WELDING SHALL BE IN ACCORDANCE WITH AWS D1.3, "STRUCTURAL WELDING CODE - SHEET STEEL". TOUCH UP ALL WELDS WITH SPECIFIED COATING SYSTEMS.
3. COLD-FORMED METAL FRAMING MEMBERS SHALL CONFORM TO ASTM C 955, AND BE FORMED OF CORROSION-RESISTANT STEEL CONFORMING TO ASTM A 653 AND ASTM C 955 WITH A MINIMUM YIELD STRENGTH OF 33 KSI FOR 43 MIL AND THINNER MEMBERS AND 50 KSI FOR ALL OTHER MEMBERS.
4. MEMBER SECTION PROPERTIES SHALL CONFORM TO PART 'V' OF THE "COLD-FORMED STEEL DESIGN MANUAL".
5. PROVIDE BRIDGING LINES AT 4'-0" MAXIMUM ON CENTER IN ALL WALLS UNLESS OTHERWISE INDICATED. BRIDGING SHALL BE FULLY INSTALLED AND ANCHORED AT ENDS BEFORE SUPERIMPOSING LOADS ONTO THE STUDS. COORDINATE BRIDGING LINE LAYOUT WITH METAL PANEL FASTENING LOCATIONS. REFER TO ARCHITECTURAL DRAWINGS.

FOUNDATION PLAN NOTES:

1. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO NONBEARING WALLS, WALL CONTROL JOINTS AND OPENINGS.
2. UNLESS OTHERWISE NOTED, ALL ELEVATIONS ARE BASED ON A FINISHED GROUND FLOOR REFERENCE OF 0'-0". ACTUAL FINISHED FLOOR ELEVATION IS 28.00' (NAVD). REFER TO ARCHITECTURAL DRAWINGS FOR FINISHED FLOOR MATERIAL.
3. TOP OF ALL COLUMN AND WALL FOOTINGS SHALL BE AT -1'-4" UON.

4. UTILITY ARE NOT SHOWN ON PLAN. THE CONTRACTOR SHALL COORDINATE THE LOCATIONS, SIZES, AND INVERTS OF UTILITIES. AT LOCATIONS WHERE UTILITIES PASS BELOW THE TOP OF FOOTING ELEVATION, STEP THE TOP OF FOOTING DOWN ON EACH SIDE PER THE "STEPPED FOOTING DETAIL" AND SLEEVE THE UTILITY THROUGH THE FOUNDATION WALL. THE CONTRACTOR MAY, AT HIS OPTION, SLEEVE THE UTILITY THROUGH THE FOUNDATION PER THE "UTILITY SLEEVE DETAIL."
5. UNLESS OTHERWISE INDICATED, EXTEND WALL FOOTINGS A MINIMUM OF 6 INCHES BEYOND ENDS OF WALLS.

FOUNDATION PLAN KEY NOTES:

- N1. PROVIDE ALTERNATE PRICE TO SUBSTITUTE HSS6X8X1/4 TYPICAL HSS6X6X3/8 AT BRACED FRAMED COLUMNS FOR ALL W10X33 COLUMNS UNLESS OTHERWISE NOTED.
- N2. PROVIDE ALTERNATE PRICE TO SUBSTITUTE HSS8X8X1/4 COLUMNS FOR W-SHAPE AT THIS LOCATION.
- N3. REFER TO S201 FOR 'U' REINFORCING AT BRACED FRAME COLUMN ANCHORAGE.
- N4. COORDINATE ALL DIMENSIONS, LAYOUT, AND CONSTRUCTION TOLERANCES WITH EQUIPMENT MANUFACTURER.
- N5. REFER TO THE TYPICAL CONCRETE WALL OPENING DETAIL ON S501. COORDINATE LAYOUT WITH ARCHITECTURAL AND MEP.
- N6. 1/2" EXPANSION JOINT MATERIAL BETWEEN SLAB ON GRADE AND VAULT.

SLAB-ON-GRADE PLAN NOTES:

1. SLAB-ON-GRADE JOINTS SHALL BE SAWED JOINTS OR KEYED CONSTRUCTION JOINTS UNLESS SPECIFICALLY DENOTED TO BE KEYED CONSTRUCTION JOINTS. CONTRACTOR SHALL COORDINATE ALL SLAB JOINTS WITH JOINTS IN BONDED FLOOR FINISHES. REFER TO ARCHITECTURAL DRAWINGS FOR FLOOR FINISH JOINT LOCATIONS.
2. PLACE 1-#4 x 3'-0" IN MIDDLE OF SLAB AT REENTRANT CORNERS WHERE A SLAB CONTROL JOINT DOES NOT OCCUR.
3. REFER TO ARCHITECTURAL DRAWINGS FOR EXACT LIMITS OF SLAB DEPRESSIONS.
4. FLOOR DRAINS AND FLOOR SINKS ARE NOT SHOWN ON PLAN. REFER TO PLUMBING DRAWINGS FOR LOCATIONS.
5. REFER TO CIVIL DRAWINGS FOR EXTERIOR CONCRETE SLABS AND PAVING.

SLAB-ON-GRADE KEY NOTES:

- S1. 4" CONCRETE SLAB-ON-GRADE OVER VAPOR RETARDER AND COMPACTED STRUCTURAL FILL, UNLESS OTHERWISE INDICATED. REINFORCE SLAB WITH 6x6 W2.9xW2.9 WELDED WIRE REINFORCING PLACED 1 1/2" CLEAR BELOW TOP OF SLAB. MAINTAIN REINFORCEMENT IN POSITION ON BOLSTERS, CHAIRS OR SPACERS DURING CONCRETE PLACEMENT.
- S2. 6" CONCRETE SLAB-ON-GRADE OVER VAPOR RETARDER AND COMPACTED STRUCTURAL FILL, UNLESS OTHERWISE INDICATED. REINFORCE SLAB WITH 6x6 W2.9xW2.9 WELDED WIRE REINFORCING PLACED 1 1/2" CLEAR BELOW TOP OF SLAB. MAINTAIN REINFORCEMENT IN POSITION ON BOLSTERS, CHAIRS OR SPACERS DURING CONCRETE PLACEMENT.

ROOF FRAMING PLAN NOTES:

1. REFER TO FOUNDATION PLAN AND ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN.
2. TOP OF DECK ELEVATIONS ARE SHOWN ON PLAN. INTERMEDIATE ELEVATIONS SHALL BE STRAIGHT LINES BETWEEN GIVEN ELEVATIONS. INTERPOLATE AS REQUIRED FOR INTERMEDIATE BEARING ELEVATIONS, UNLESS OTHERWISE NOTED.
3. ROOF FRAMING SHALL BE EQUALLY SPACED BETWEEN POINTS INDICATED NOT TO EXCEED 5'-6" OC TO SUPPORT STEEL ROOF DECK.
4. COORDINATE AND VERIFY ALL MEMBER LOCATIONS, DIMENSIONS, WEIGHTS, OPENING SIZES, AND CURB DIMENSIONS FOR ALL MECHANICAL EQUIPMENT WITH THE ACTUAL EQUIPMENT FURNISHED. INCLUDE THIS INFORMATION ON THE JOIST AND STRUCTURAL STEEL SHOP DRAWINGS.

ROOF FRAMING PLAN KEY NOTES:

- R1. DESIGN BEAM TO COLUMN OR BEAM TO BEAM CONNECTION FOR TENSION / COMPRESSION LOAD T=XK (ASD) IN ADDITION TO ALL OTHER LOADS.
- R2. L3X3X1/4 'X' BRIDGING BETWEEN COLUMN AND FIRST JOIST.
- R3. L3X3X3/8 KNEE BRACE REFER TO TYPICAL KNEE BRACE DETAIL ON S503.
- R4. HSS5X2 1/2X3/16 (LSH) ON TOP OF BEAM BETWEEN EACH JOIST SEAT FASTEN DECK TO HSS WITH 5/8"Ø PUDDLE WELDS AT 6" OC.
- R5. VERTICAL SLIP CONNECTION BETWEEN HSS GIRT AND EACH WALL STUD.
- R6. REFER TO THE TYPICAL CONCRETE WALL OPENING DETAIL ON S501. COORDINATE LAYOUT WITH ARCHITECTURAL AND MEP.
- R7. EXTEND DECK BEARING ANGLE TO BEAM ON LINE C.7.

ABBREVIATIONS:

ABBREVIATION	DEFINITION	ABBREVIATION	DEFINITION
AFF	ABOVE FINISHED FLOOR	INT	INTERIOR
ARCH	ARCHITECT / ARCHITECTURAL	JBE	JOIST BEARING ELEVATION
BEJ	BUILDING EXPANSION JOINT	JT	JOINT
		KCJ	KEYED CONSTRUCTION JOINT
BLDG	BUILDING	L	LOW
BOD	BOTTOM OF DECK	LLH	LONG LEG HORIZONTAL
BOT	BOTTOM	LLV	LONG LEG VERTICAL
BRG	BEARING	LSH	LONG SIDE HORIZONTAL
BM	BEAM	LSV	LONG SIDE VERTICAL
BTWN	BETWEEN	LTWT	LIGHTWEIGHT
CJ	CONTROL JOINT	LWC	LIGHT WEIGHT CONCRETE
CLR	CENTERLINE	MAS	MASONRY
CMU	CLEAR	MATL	MATERIAL
COL	CONCRETE MASONRY UNIT	MAX	MAXIMUM
CONC	COLUMN	MECH	MECHANICAL
CONST	CONCRETE	MFR	MANUFACTURER
CONT	CONSTRUCTION	MID	MIDDLE
DBL	DOUBLE	MIN	MINIMUM
DC	DIAPHRAGM CHORD	MOD	MODIFY
DCJ	DOWELED	MOS	MIDDLE OF SLAB
		MOW	MIDDLE OF WALL
DJ	DOUBLE JOIST	NOM	NOMINAL
DWGS	DRAWINGS	NTS	NOT TO SCALE
EA	EACH	OC	ON CENTER
EF	EACH FACE	OPH	OPPOSITE HAND
EJ	EXPANSION JOINT	OPNG	OPENING
EL	ELEVATION	PAF	POWDER ACTUATED FASTENER
ELEV	ELEVATION	PEMB	PRE ENGINEERED
EOD	EDGE OF DECK		METAL BUILDING
EOS	EDGE OF SLAB	PL	PLATE
EQ	EQUAL	R	RADIUS
EW	EACH WAY	REF	REFER TO
EXIST	EXISTING	REINF	REINFORCED
EXT	EXTERIOR	REQD	REQUIRED
FD	FLOOR DRAIN		STEPPED FOOTING
FDN	FOUNDATION	SF	STEPPED GRADE BEAM
FO	FACE OF	SIM	SIMILAR
FF EL	FINISHED FLOOR ELEVATION	SJ	SAWED JOINT
		SL	SLOPE
FIN	FINISHED	STD	STANDARD
FIN FLR	FINISHED FLOOR	T&B	TOP AND BOTTOM
FOB	FACE OF BUILDING	THK	THICK
FOM	FACE OF CONCRETE	TF	TOP OF FOOTING
FOS	FACE OF MASONRY	TOM	TOP OF MASONRY
FRMG	FACE OF STUD	TOS	TOP OF STEEL
FTG	FRAMING	TS	THICKENED SLAB
FTG	FOOTING	TYP	TYPICAL
GALV	GALVANIZED	UON	UNLESS OTHERWISE
GEN	GENERAL		NOTED
GR BM	GRADE BEAM	VERT	VERTICAL
H	HIGH	WWR	WELDED WIRE REINFORCING
HK	HOK		
HORIZ	HORIZONTAL		
HSS	HOLLOW STRUCTURAL SHAPE		
HT	HEIGHT		
HVY	HEAVY		

PLAN LEGEND:

TOS = +X'-X"	=	TOP OF STEEL ELEVATION MEASURED FROM REFERENCED FINISHED FIRST FLOOR ELEVATION = 0'-0"
TOD = +X'-X"	=	TOP OF DECK ELEVATION MEASURED FROM REFERENCED FINISHED FIRST FLOOR ELEVATION = 0'-0"
XXx#	=	MECHANICAL UNIT SUPPORTED ABOVE FRAMING (WEIGHT IN POUNDS) - COORDINATE W/ MECH DWGS
(-X'-X")	=	TOP OF FOOTING ELEVATION MEASURED FROM REFERENCED FINISHED FIRST FLOOR ELEVATION = 0'-0"
± X"	=	TOP OF SLAB OR DECK ELEVATION MEASURED FROM REFERENCED FINISHED FIRST FLOOR ELEVATION = 0'-0" OR REFERENCES TOP OF DECK ELEVATION
	=	FLOOR / ROOF OPENING
	=	CHANGE IN ELEVATION - REF ARCH DWGS FOR DIMENSIONS
SL	=	DIRECTION OF SLOPE
	=	KCJ, SJ LINE ON PLAN
	=	WARP LINE OF ROOF DECK
	=	MOMENT CONNECTION
	=	BEAM SPLICE
	=	HORIZONTAL BRIDGING
	=	CROSS BRIDGING
X	=	PLAN KEY NOTE MARK
X	=	COLUMN GRID MARK
X	=	SECTION/DETAIL NUMBER/LETTER
X SX	=	SECTION/DETAIL MARK
	=	SHEET NUMBER WHERE SECTION/DETAIL MARK IS DRAWN
WFX	=	WALL FOOTING MARK
CFX	=	COLUMN FOOTING MARK
T=Xk	=	BEAM TO COLUMN OR BEAM TO BEAM CONNECTION TENSION / COMPRESSION LOAD

REVISIONS	DATE	DESCRIPTION
No.	0	ISSUED FOR PERMIT

SCHEDULE OF SPECIAL INSPECTION SERVICES				
Per IBC Section 1704 of the Florida Building Code 8th Edition the following items require Special Inspections. Special inspectors must be employed by the Owner or registered design professional in responsible charge acting as the owner's agent.				
MATERIAL / ACTIVITY	SERVICE	APPLICABLE TO THIS PROJECT		
		Y/N	EXTENT	AGENT*
1704.2.5 Inspection of Fabricators		Y		1
Verify fabrication/quality control procedures	In-plant review	Y	Periodic	1
1705.2 Steel Construction		Y		1
1. Fabricator and erector documents (Verify reports and certificates as listed in AISC 360, chapter N, paragraph 3.2 for compliance with construction documents) Submittal Review	Submittal Review	Y	Each submittal	1
2. Material verification of structural steel	Shop (3) and field inspection	Y	Periodic	
3. Embedments (Verify diameter, grade, type, length, embedment. See 1705.3 for anchors)	Field Inspection	Y	Continuous	1
4. Verify member locations, braces, stiffeners, and application of joint details at each connection comply with construction documents	Field Inspection	Y	Periodic	1
5. Structural Steel Welding				1
a. Inspection tasks Prior to Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-1)	Shop (3) and field inspection	Y	Observe or Perform as noted (4)	1
b. Inspection tasks During Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-1)	Shop (3) and field inspection	Y	Observe (4)	1
c. Inspection tasks After Welding (Observe, or perform for each welded joint or member, the QA tasks listed In AISC 360, Table N5.43)	Shop (3) and field inspection	Y	Observe or Perform as noted (4)	1
d. Nondestructive testing (NDT) of welded joints: <i>see Commentary</i>		Y		1
1) Complete penetration groove welds 5/16" or greater in <i>risk category</i> III or IV	Shop or field ultrasonic testing - 100%	Y	Periodic	1
2) Complete penetration groove welds 5/16" or greater in <i>risk category</i> II	Shop or field ultrasonic testing - 10%	Y	Periodic	1
3) Thermally cut surfaces of access holes when material t > 2"	Shop or field magnetic Particle or Penetrant testing	N	Periodic	
4) Welded joints subject to fatigue when required by AISC 360, Appendix 3, Table A-3.1	Shop or field radiographic or Ultrasonic testing	N	Periodic	
5) Fabricator's NDT reports when fabricator performs NDT	Verify reports	Y	Each submittal (5)	1
6. Structural steel bolting:	Shop and field inspection	Y		1
a. Inspection tasks Prior to Bolting (Observe, or perform tasks for each bolted connection, in accordance with QA tasks listed in AISC 360, Table N5.6-1)	Shop and field inspection	Y	Observe or Perform as noted (4)	1
b. Inspection tasks During Bolting (Observe the QA tasks listed in AISC 360, Table N5.6-2)	Shop and field inspection	Y	Observe (4)	1
1) Pre-tensioned and slip-critical joints		N		
a) Turn-of-nut with matching markings		N	Periodic	
b) Direct tension indicator		N	Periodic	
c) Twist-off type tension control bolt		N	Periodic	
d) Turn-of-nut without matching markings		N	Continuous	
e) Calibrated wrench		N	Continuous	

SCHEDULE OF SPECIAL INSPECTION SERVICES				
Per IBC Section 1704 of the Florida Building Code 8th Edition the following items require Special Inspections. Special inspectors must be employed by the Owner or registered design professional in responsible charge acting as the owner's agent.				
MATERIAL / ACTIVITY	SERVICE	Y/N	EXTENT	AGENT*
2) Snug-tight joints		Y	Periodic	1
c. Inspection tasks After Bolting (Perform tasks for each bolted connection in accordance with QA tasks listed in AISC 360, Table N5.6-3)	Shop and field inspection	Y	Perform (4)	1
7. Inspection of steel elements of composite construction prior to concrete placement in accordance with QA tasks listed in AISC 360, Table N6.1	Shop and field inspection and testing	Y	Observe or Perform as noted (4)	1
1705.2.2 Cold-formed Steel Deck		Y		1
1. Material verification of cold-formed steel deck:	Submittal Review	Y		1
a. Identification markings	Field inspection	Y	Periodic	1
b. Manufacturer's certified test reports	Submittal Review	Y	Each submittal	1
2. Connection of cold-formed steel deck to supporting structure:	Field inspection	Y		1
a. Welding	Field inspection	Y	Periodic	1
b. Other fasteners (In accordance with AISC 360,Section N6)	Field inspection	Y		1
1) Verify fasteners are in conformance with approved	Field inspection	Y	Periodic	1
2) Verify fastener Installation is in conformance with approved submittal and manufacturer's recommendations	Field inspection	Y	Periodic	1
1705.2.3 Open-web Steel Joists and Joist Girders		Y		1
1. End Connections – Welding or Bolted	Field inspection	Y	Periodic	1
2. Bridging – Horizontal or Diagonal	Field inspection	Y	Periodic	1
1705.3 Concrete Construction		Y		1
1. inspection of reinforcing steel Installation (see 1705.2.2 for welding)	Shop and field inspection	Y	Periodic	1
2. Inspection of prestressing steel installation	Shop and field inspection	N	Periodic	
3. Inspection of anchors cast in concrete where allowable loads have been increased per section 1908.5 or where strength design is used	Shop and field inspection	N	Continuous	
4. Inspection of anchors and reinforcing steel post-installed in hardened concrete: Per research reports including verification of anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, concrete minimum thickness, anchor embedment and tightening torque	Field Inspection	Y	Periodic or as required by the research report Issued by an approved source	1
5. Verify use of approved design mix	Shop and field inspection	Y	Periodic	1
6. Fresh concrete sampling, perform slump and air content tests and determine temperature of concrete	Shop and field inspection	Y	Continuous	1
7. Inspection of concrete and shotcrete placement for proper application techniques	Shop and field inspection	Y	Continuous	1
8. Inspection for maintenance of specified curing temperature	Shop and field inspection	Y	Periodic	1
9. Inspection of prestressed concrete	Shop and field inspection	N		
a. Application of prestressing force		N	Continuous	
b. Grouting of bonded prestressing tendons in the seismic-force-resisting system		N	Continuous	
10. Erection of precast concrete members		N		
a. Inspection in accordance with construction documents	Field inspection	N	In accordance with construction documents	
b. Perform inspections of welding and bolting in accordance with section 1705.2	Field inspection	N	In accordance with section 1705.2	
11. Verification of in-situ concrete strength, prior to stressing of tendons in post tensioned concrete and prior to removal of shores and forms from beams and structural slabs	Review field testing and laboratory reports	N	Periodic	
12. Inspection of formwork for shape, lines, location and dimensions	Field inspection	Y	Periodic	1
13. Concrete strength testing and verification of compliance with construction documents	Field testing and review of laboratory reports	Y	Periodic	1

SCHEDULE OF SPECIAL INSPECTION SERVICES				
Per IBC Section 1704 of the Florida Building Code 8th Edition the following items require Special Inspections. Special inspectors must be employed by the Owner or registered design professional in responsible charge acting as the owner's agent.				
MATERIAL / ACTIVITY	SERVICE	Y/N	EXTENT	AGENT*
1705.4 Masonry Construction				
1. Verify compliance with approved submittals	Field inspection	Y	Periodic	1
2. Verification of Pm and fAAC prior to construction and for every 5,000 SF during construction	Testing by unit strength method or prism test method	Y	Periodic	1
3. Verification of proportions of materials in premixed or preblended mortar, prestressing grout, and grout other than self-consolidating grout, as delivered to the project site	Field inspection	Y	Continuous	1
4. Verify placement of masonry units	Field inspection	Y	Periodic	1
5. Verify proportions of site-mixed mortar, grout and prestressing grout for bonded tendons	Field inspection	Y	Periodic	1
6. Verify grade, type, and size of reinforcement and anchor bolts, and prestressing tendons and anchorages	Field inspection	Y	Periodic	1
7. Verify construction of mortar joints	Field inspection	Y	Periodic	1
8. Verify grout space prior to grouting	Field inspection	Y N	Level B - Periodic Level C - Continuous	1 1
9. Verify size and location of structural masonry elements	Field inspection	Y	Periodic	1
10. Verify welding of reinforcement (see 1705.2.2)	Field inspection	Y	Continuous	1
11. Verify preparation, construction, and protection of masonry during cold weather (temperature below 40°F) or hot weather (temperature above 90°F)	Field inspection	Y	Periodic	1
12. Prepare grout and mortar specimens	Field inspection	Y N	Level B - Periodic Level C - Continuous	1 1
1705.6 Soils				
1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	Field Inspection	Y	Periodic	1
2. Verify excavations are extended to proper depth and have reached proper material.	Field Inspection	Y	Periodic	1
3. Perform classification and testing of controlled fill materials.	Field Inspection	Y	Periodic	1
4. Verify use of proper materials, densities, and lift thicknesses during placement and compaction of controlled fill	Field Inspection	Y	Continuous	1
5. Prior to placement of controlled till, observe subgrade and verify that site has been prepared properly	Field Inspection	Y	Periodic	1
1705.11.3 Wind-resisting Components				
1. Roof cladding	Shop and field inspection	Y	Periodic	1
2. Wall cladding	Shop and field inspection	Y	Periodic	1
1705.16 Exterior Insulation and Finish Systems (EIFS)				
1. Verify materials, details and installations are per the approved construction documents	Field inspection	Y	Periodic	1
2. Inspection of water-resistive barrier over sheathing substrate	Field inspection	Y	Periodic	1
Special inspection reports are to be kept on the job for Building inspector Verification.				
All discrepancies must be brought to the immediate attention of the contractor for correction. If not corrected discrepancies must be brought to the immediate attention of the building official, and design professional in responsible charge before completion of that state of work.				
A final special inspection report, from the special inspector(s), documenting the required special Inspections were performed, correction of discrepancies, and compliance with construction documents shall be submitted before a Certificate of Occupancy is issued.				
* INSPECTION AGENTS FIRM	ADDRESS			
1. TO BE DETERMINED.				

© 2025 WEB Engineering, P.C. All Rights Reserved

BBLMedical Facilities

302 Washington Ave. Ext. Albany, New York 12203 518-452-2500 fax 518-452-2888

WEB Engineering Inc.

P.O. Box 308, Old Forge NY 13420 315-363-1041

NOTE: IT IS A VIOLATION OF THE FLORIDA BOARD OF PROFESSIONAL ENGINEERS' LAW FOR ANY PERSON, UNLESS ACTING UNDER THE DIRECTION OF A LICENSED ARCHITECT, TO ALTER AN ITEM AS INDICATED ON DRAWING, IN ANY WAY

WALTER EDGAR BEECHER, P.E.

FLORIDA PROFESSIONAL ENGINEER

No. 65538

Florida Cencer Specialists

Proposed Medical Office Building

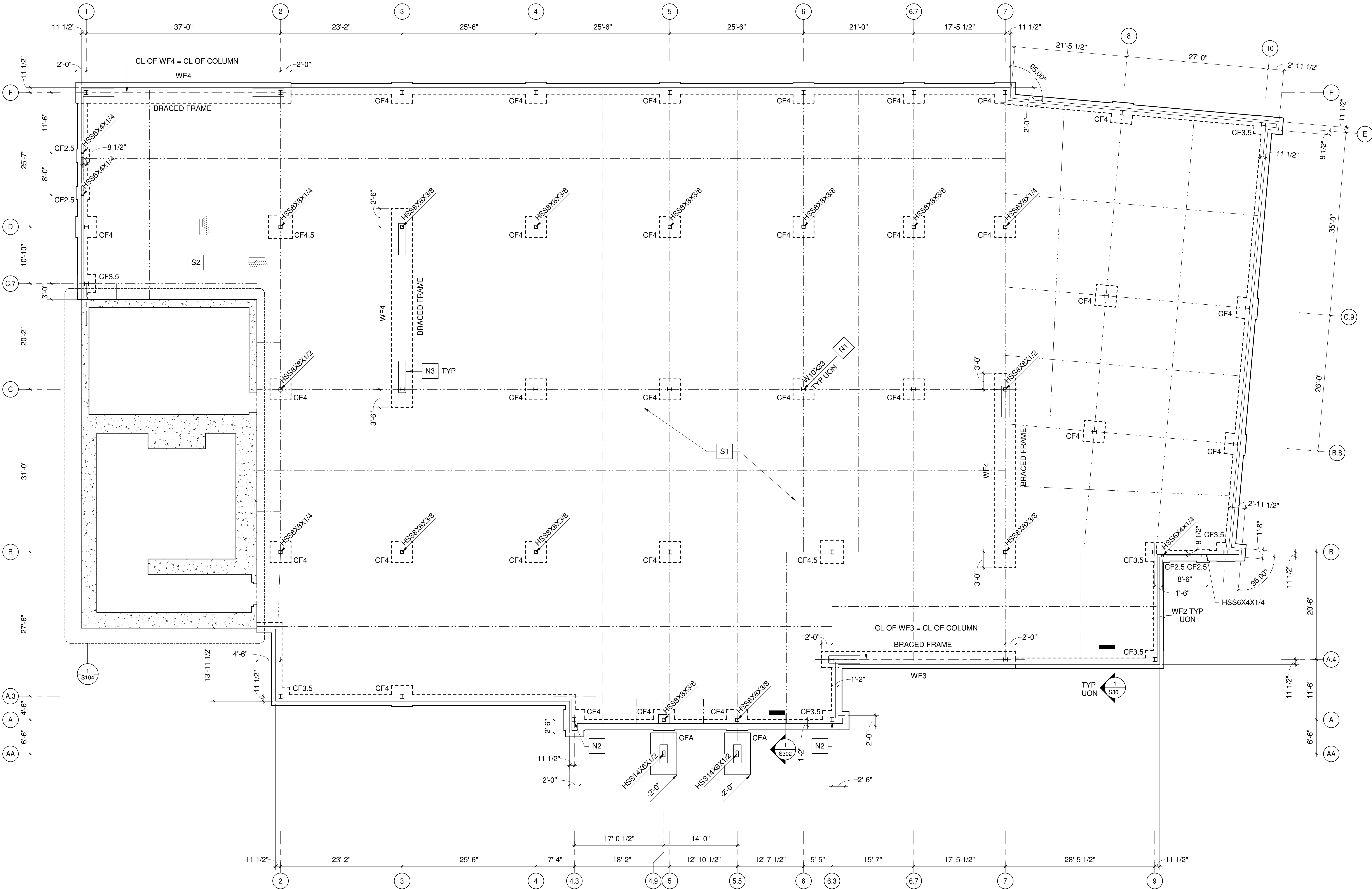
Innovation Way, Port St. Lucie, FL

REVISIONS	DATE								
	DESCRIPTION								
ISSUED FOR PERMIT									
No.									
0									

WEB JOB NO.: 24081

DRAWING TITLE:
SCHEDULE OF SPECIAL INSPECTIONS

DRAWING NUMBER:
S002

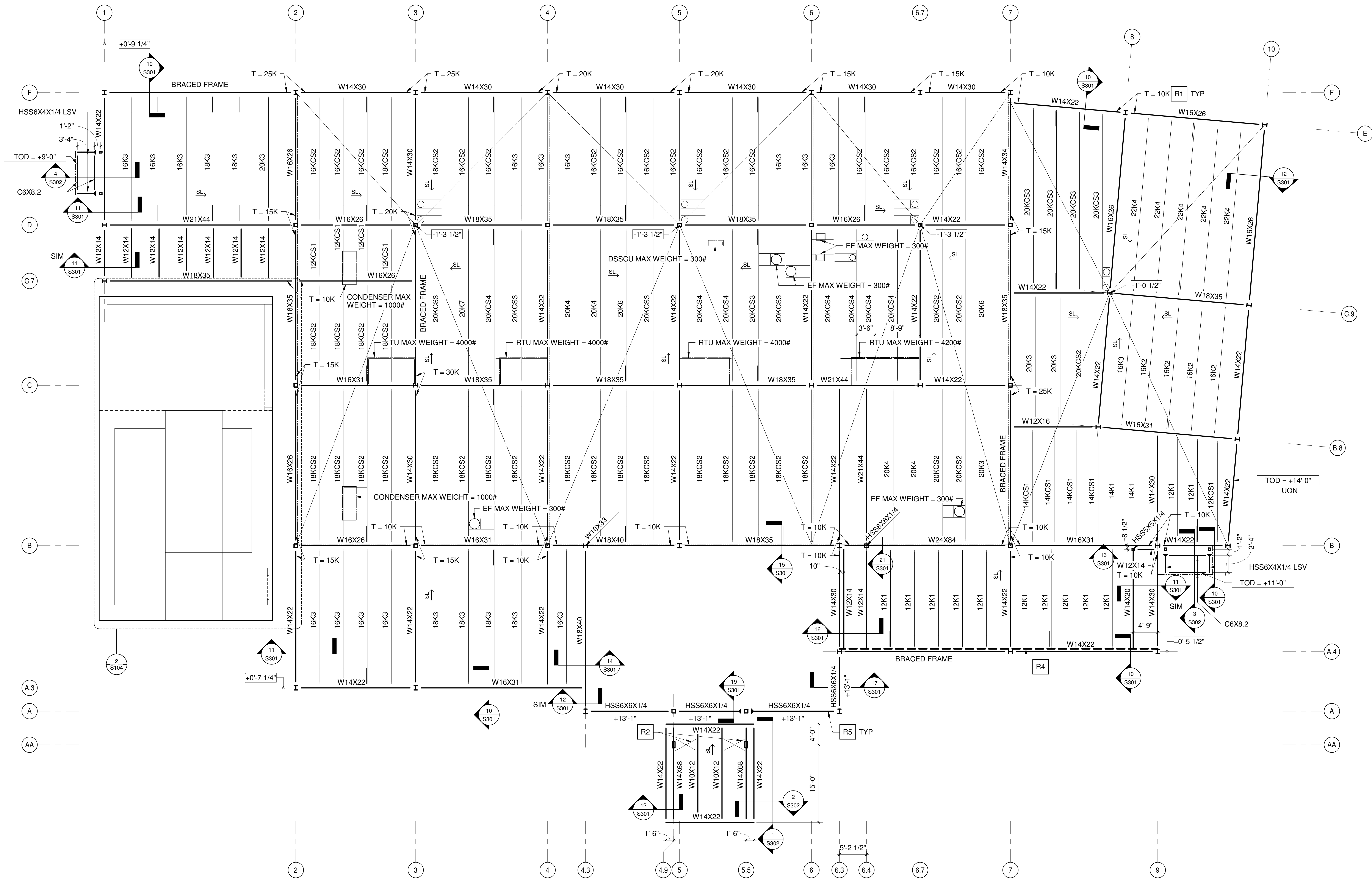


1 FOUNDATION PLAN
1/8" = 1'-0"

WALL FOOTING SCHEDULE				
MARK	SIZE		REINFORCING	
	WIDTH	DEPTH	CONTINUOUS	TRANSVERSE
WF2	2'- 0"	1'-0"	3 - #4 BOT	#4 TIES AT 8'-0" OC BOT
WF3	3'- 0"	1'-4"	6 - #5 TOP, 5 - #6 BOTTOM	#5 AT 12" OC TOP AND BOTTOM
WF4	4'- 0"	1'-9"	5 - #7 TOP, 5 - #8 BOTTOM	#5 AT 12" OC TOP AND BOTTOM

COLUMN FOOTING SCHEDULE				
MARK	SIZE		REINFORCING	
	LENGTH	WIDTH	DEPTH	EW BOTTOM
CF2.5	2'- 6"	2'- 6"	1'-0"	3 - #4
CF3.5	3'- 6"	3'- 6"	1'-0"	4 - #4
CF4	4'- 0"	4'- 0"	1'-0"	5 - #4
CF4.5	4'- 6"	4'- 6"	1'-1"	5 - #4
CFA	8'- 0"	5'- 0"	1'-8"	8 - #6

REVISIONS	DESCRIPTION	DATE	ISSUED FOR PERMIT
No.			
0			



1 ROOF FRAMING PLAN
1/8" = 1'-0"

© 2025 WEB Engineering,
P.C. All Rights Reserved

BBLMedical Facilities
302 Washington Ave. Ext.
Albany, New York 12203
518-452-2500 fax 518-452-2888

WEB
Engineering Inc.
P.O. Box 308, Old Forge NY 13420
515-353-1041

NOTE: IT IS A VIOLATION
OF THE FLORIDA
BOARD OF PROFESSIONAL
ENGINEERS
LAW FOR ANY PERSON,
UNLESS ACTING UNDER THE
DIRECTION OF A LICENSED
ARCHITECT, TO ALTER AN ITEM
AS INDICATED ON DRAWING,
IN ANY WAY
WITHOUT THE
WRITTEN CONSENT OF
THE ENGINEER.
No. 65538
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

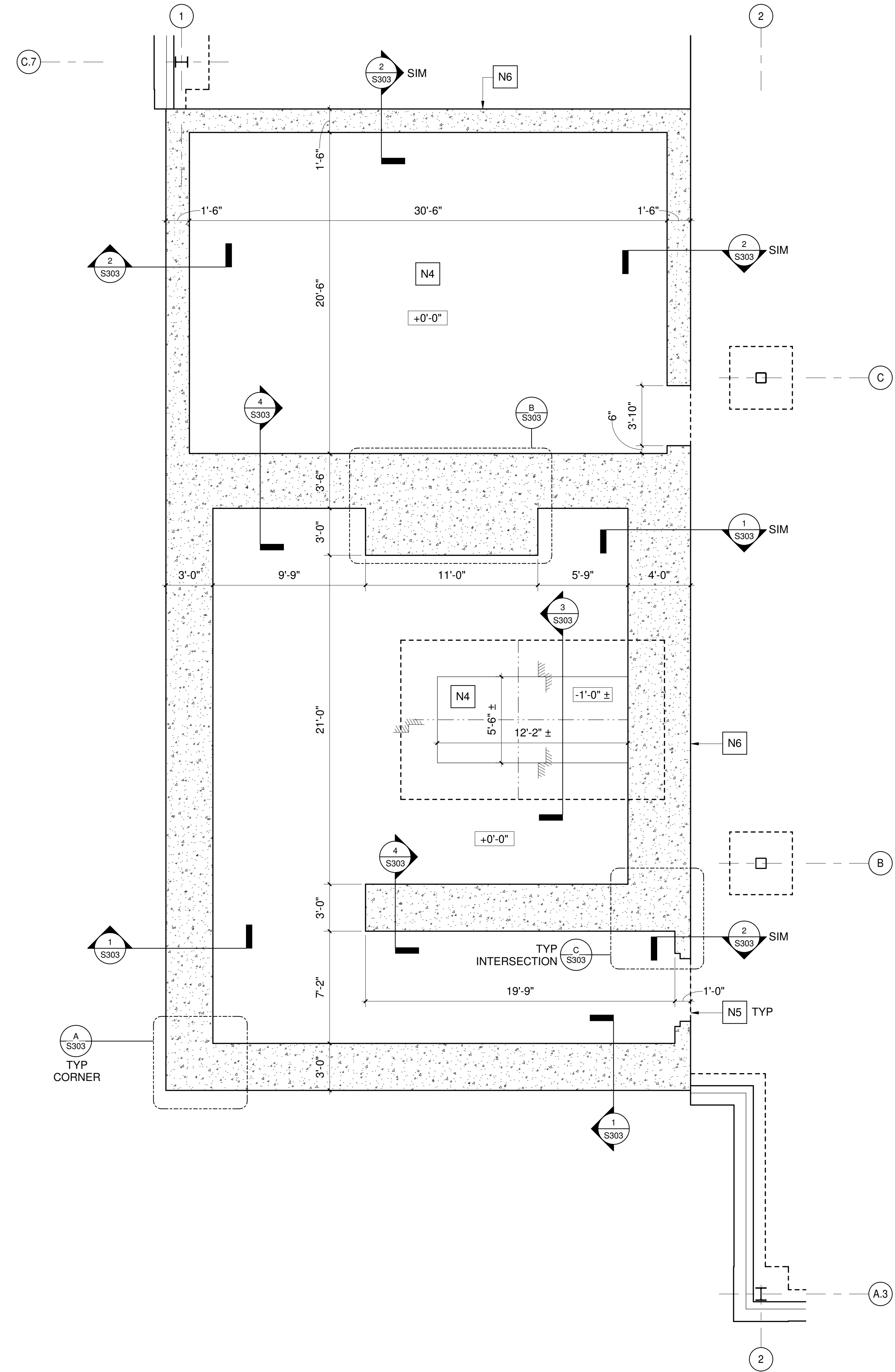
Florida Center Specialists
Proposed Medical Office Building
Innovation Way, Port St. Lucie, FL

REVISIONS	DATE
DESCRIPTION	6/13/25
ISSUED FOR PERMIT	
No.	0

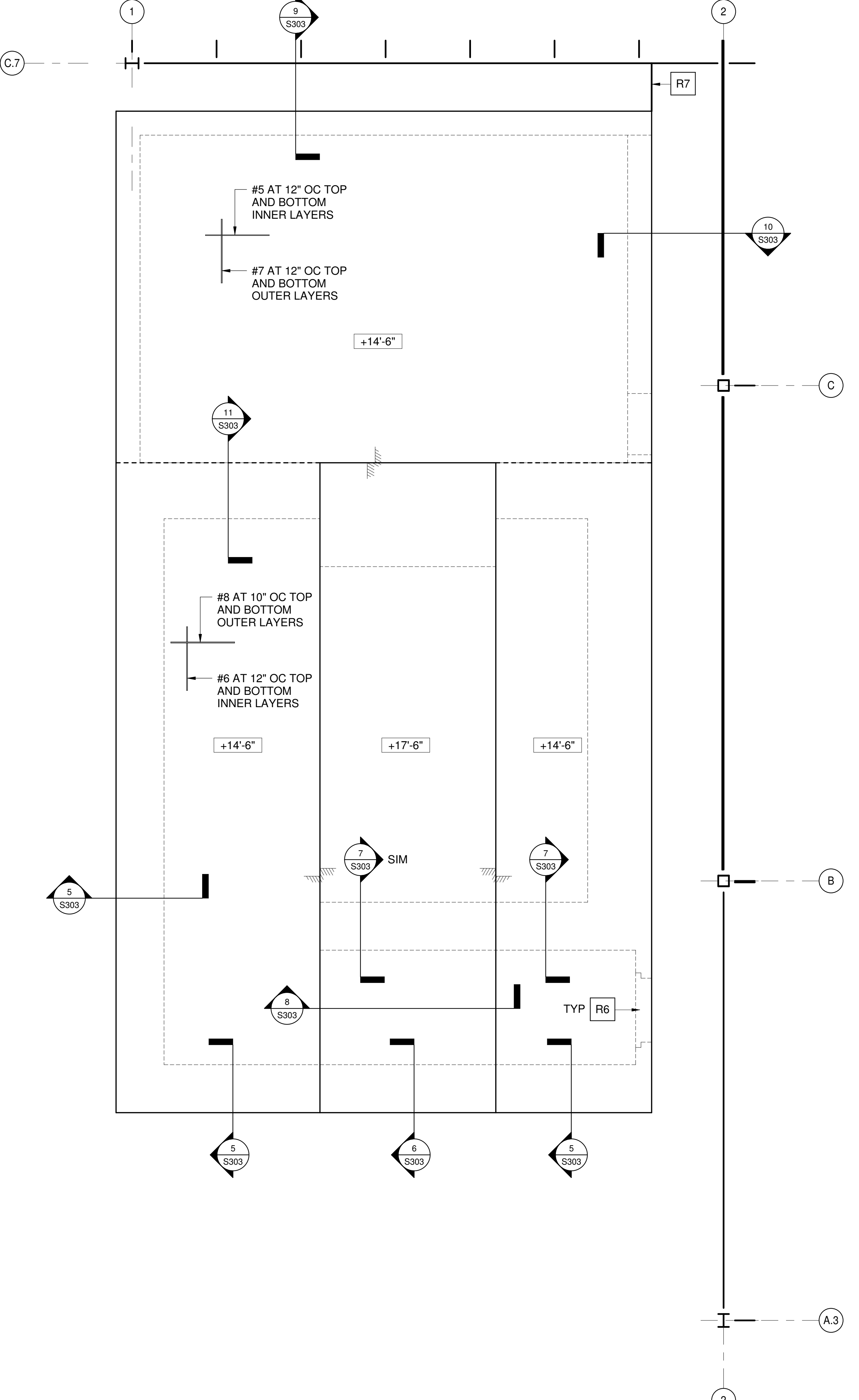
WEB JOB NO.: 24081

DRAWING TITLE:
ROOF FRAMING PLAN

DRAWING NUMBER:
S102



1 PARTIAL FOUNDATION PLAN LINAC
1/4" = 1'-0"



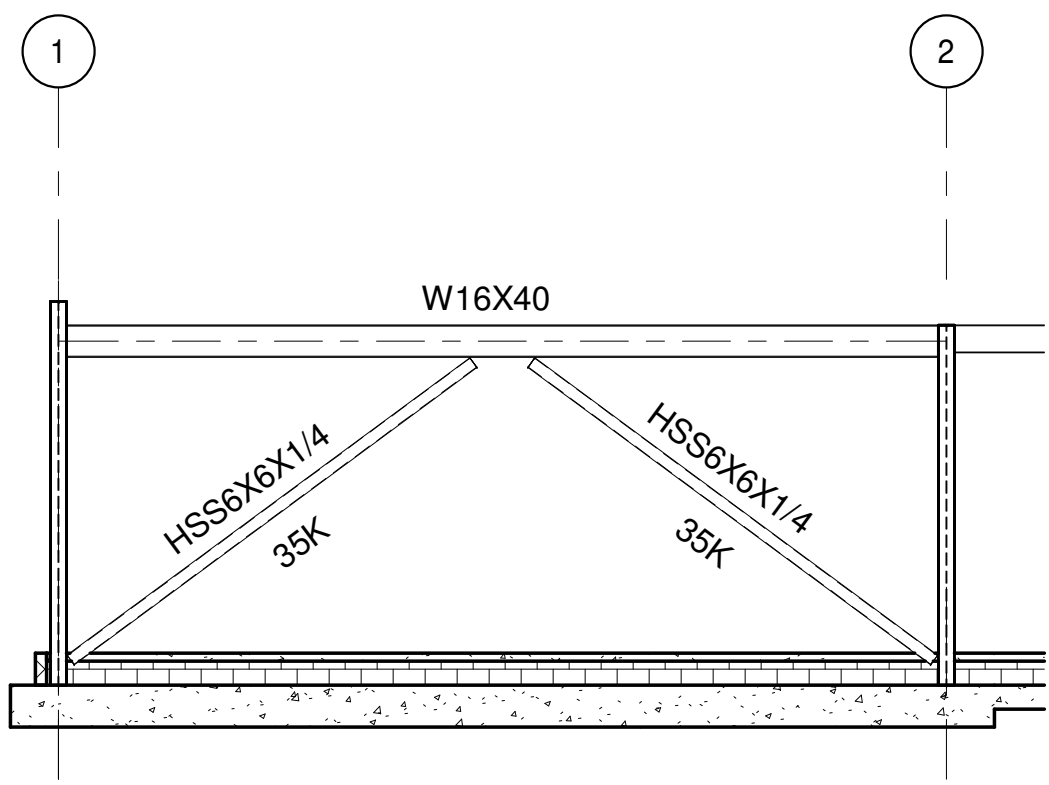
2 PARTIAL ROOF FRAMING PLAN LINAC
1/4" = 1'-0"

REVISIONS		DATE
DESCRIPTION	ISSUED FOR PERMIT	
No.		
0		

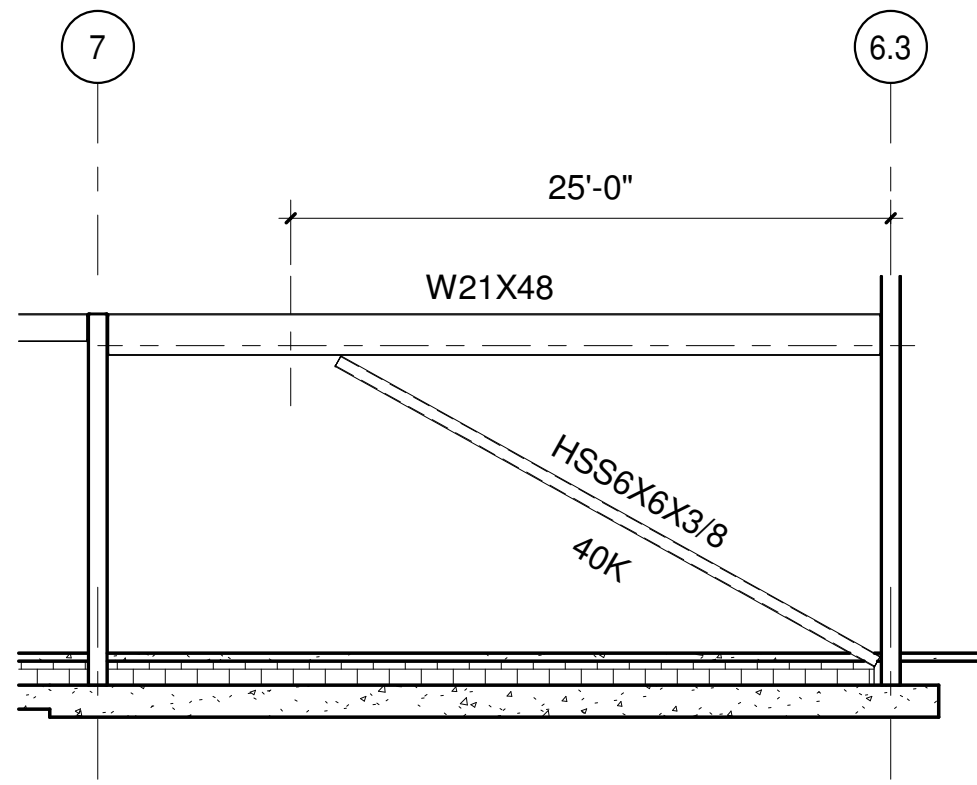
WEB JOB NO.: 24081

DRAWING TITLE:
LINAC PLANS

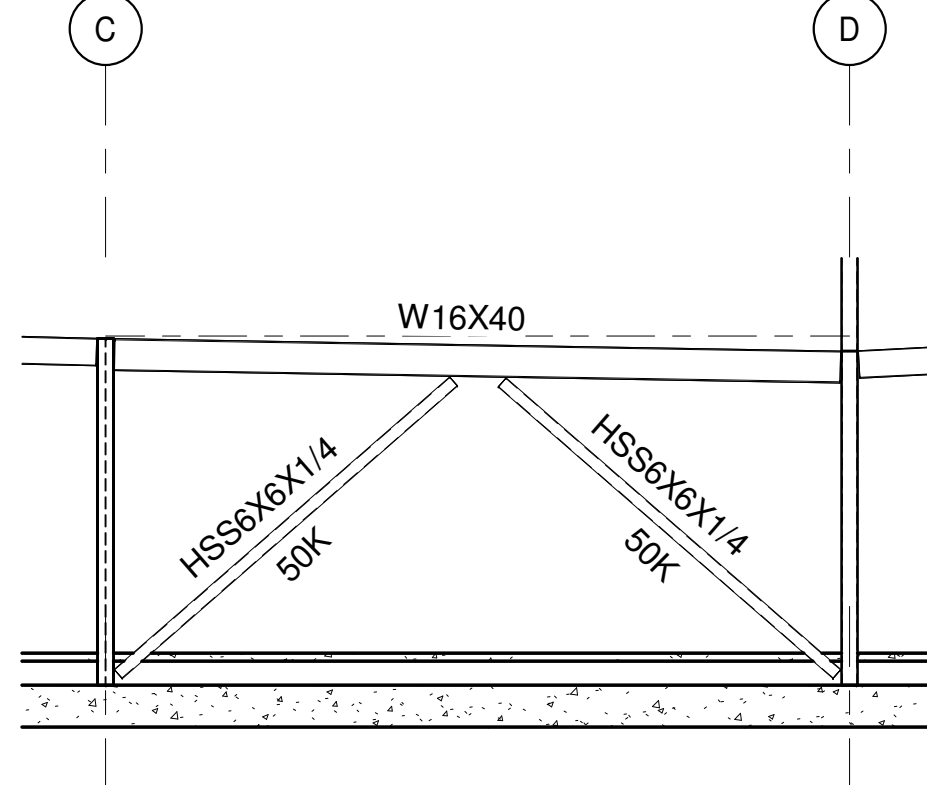
DRAWING NUMBER:
S104



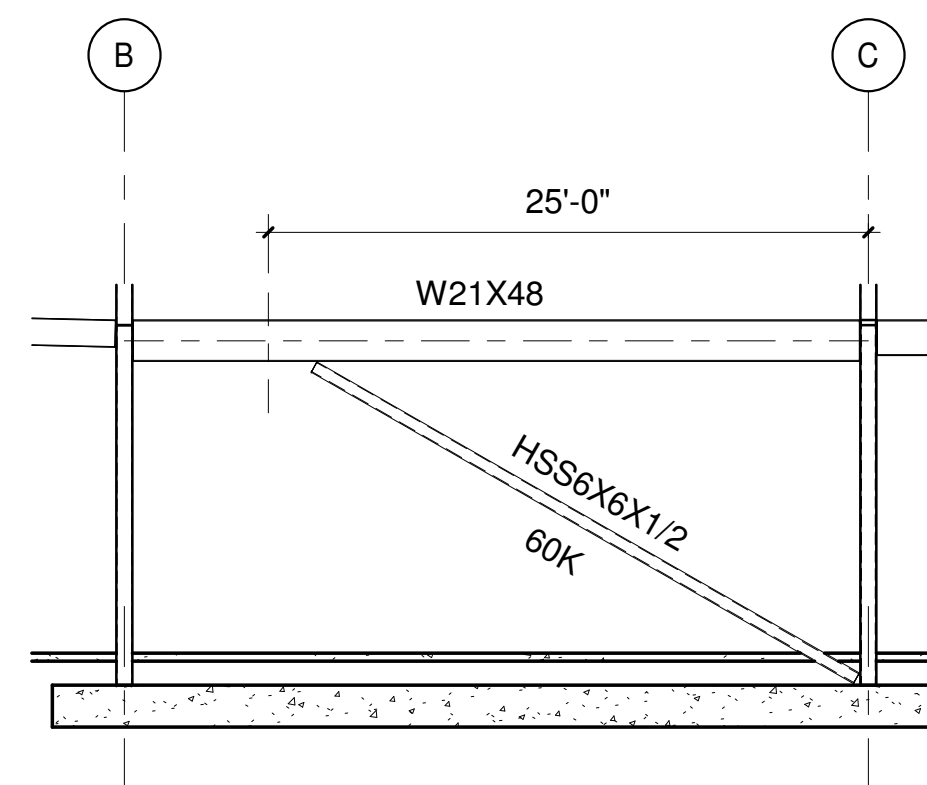
1 BRACED FRAME ELEVATION F/1-2
1/8" = 1'-0"



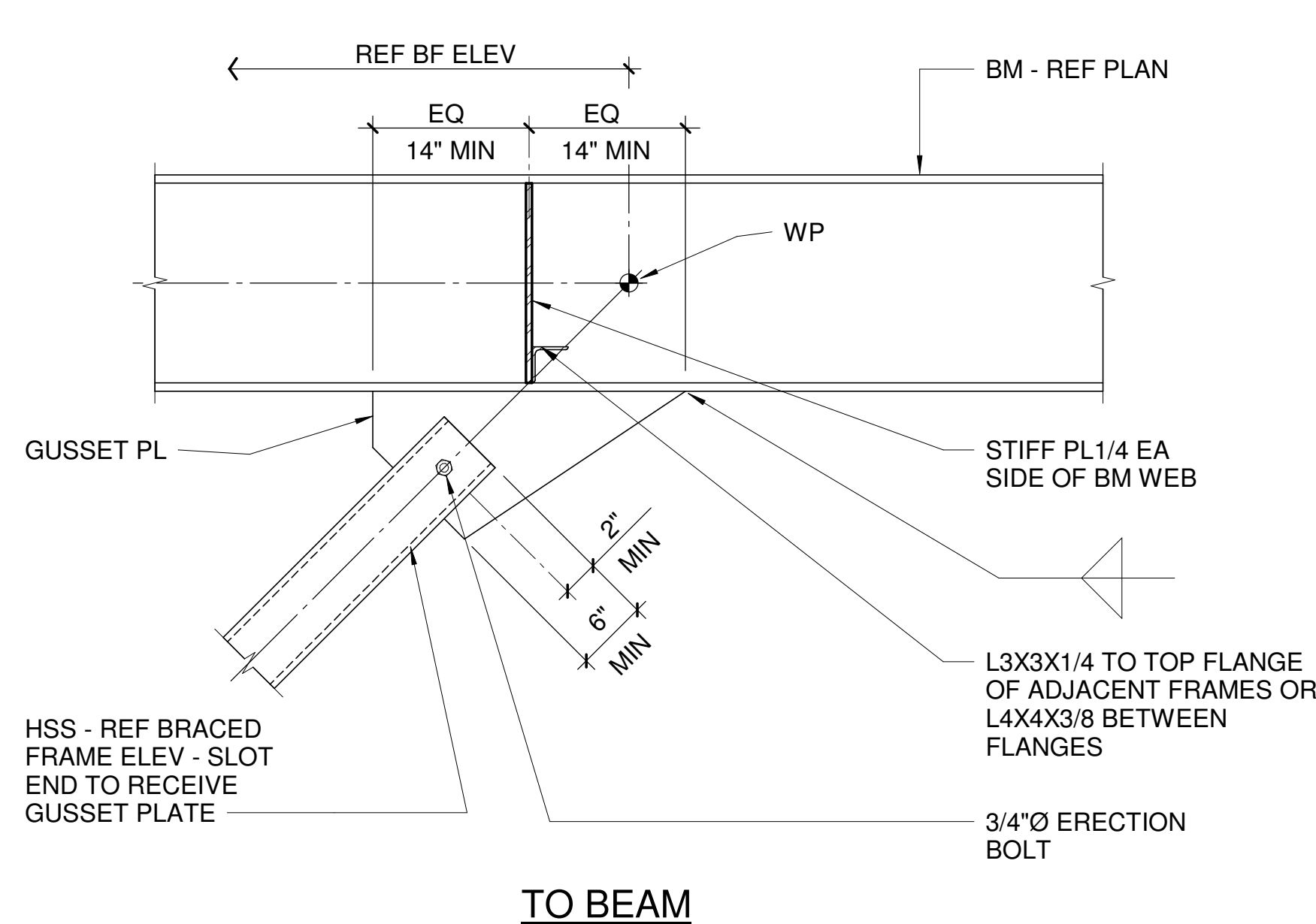
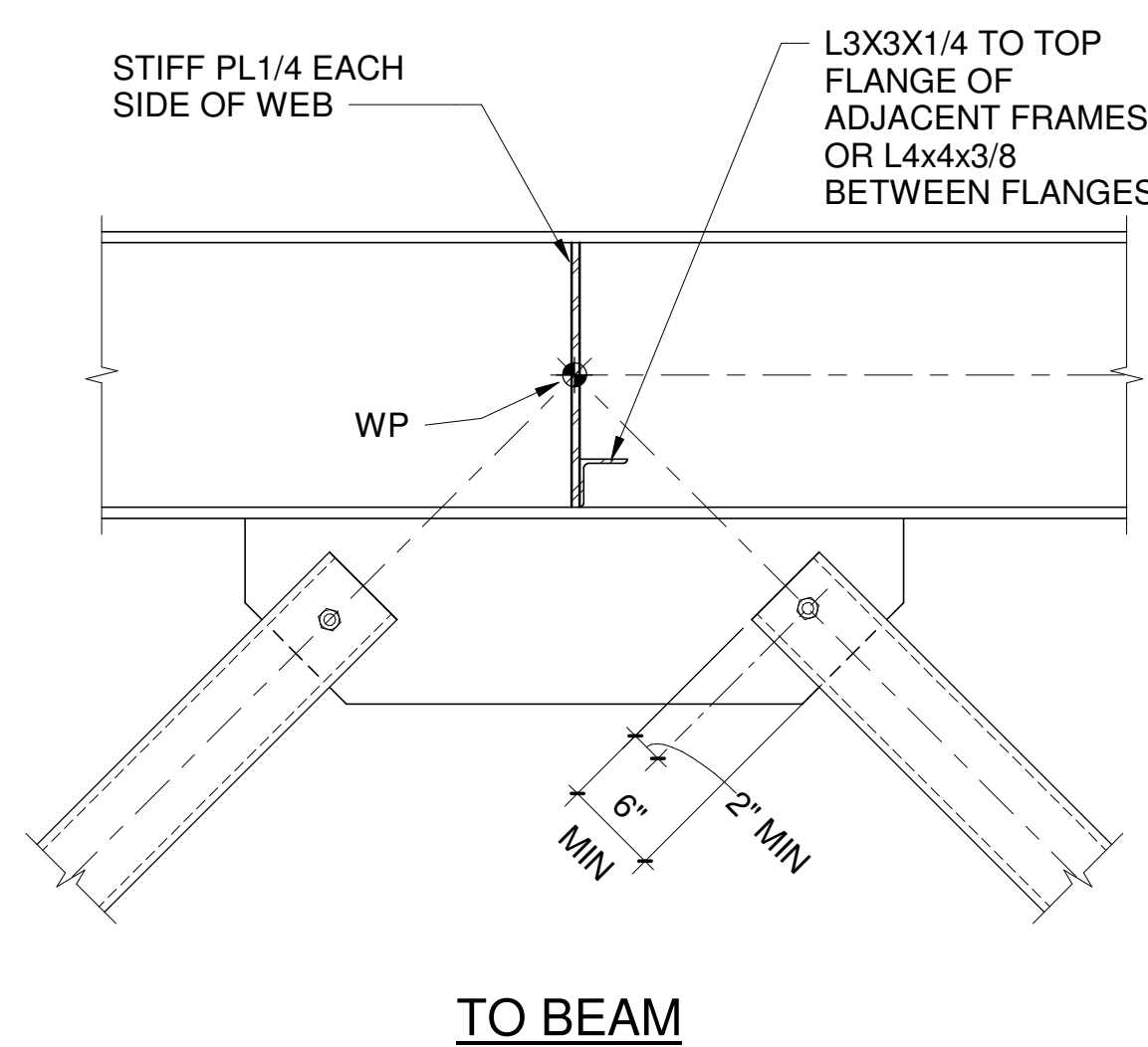
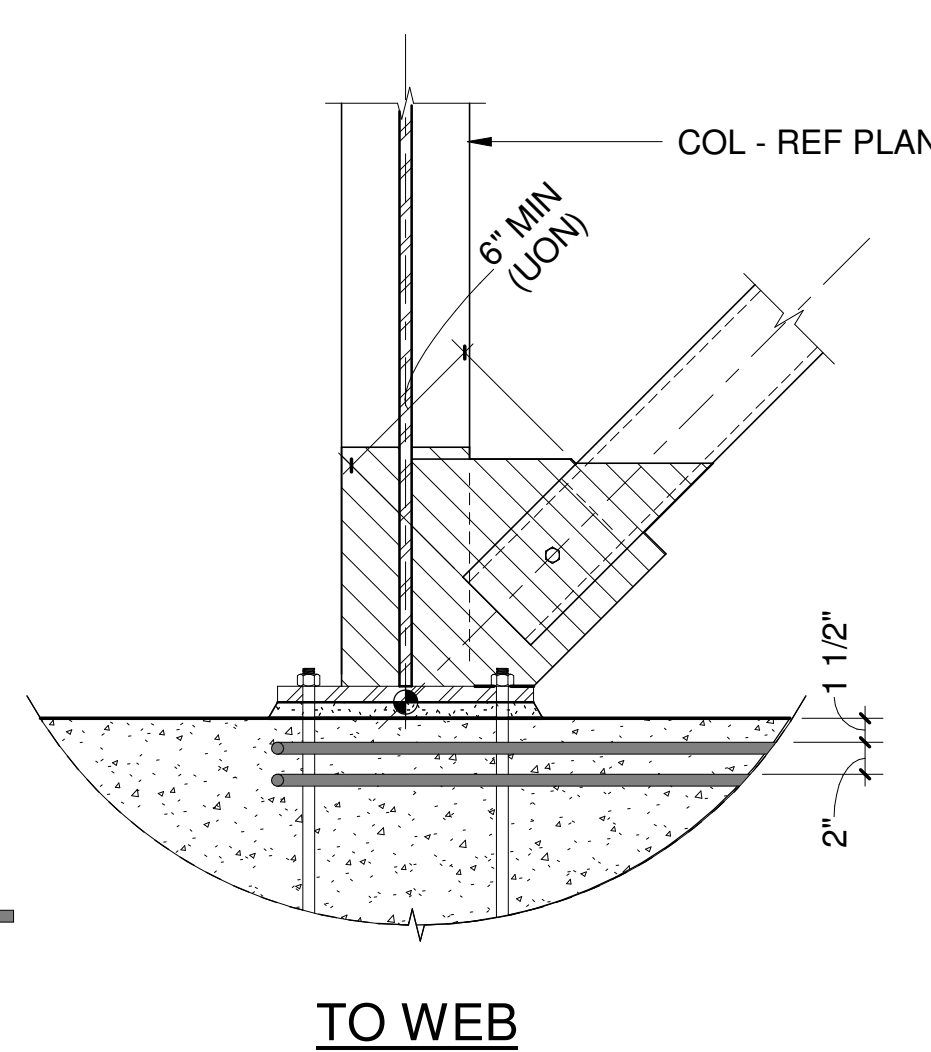
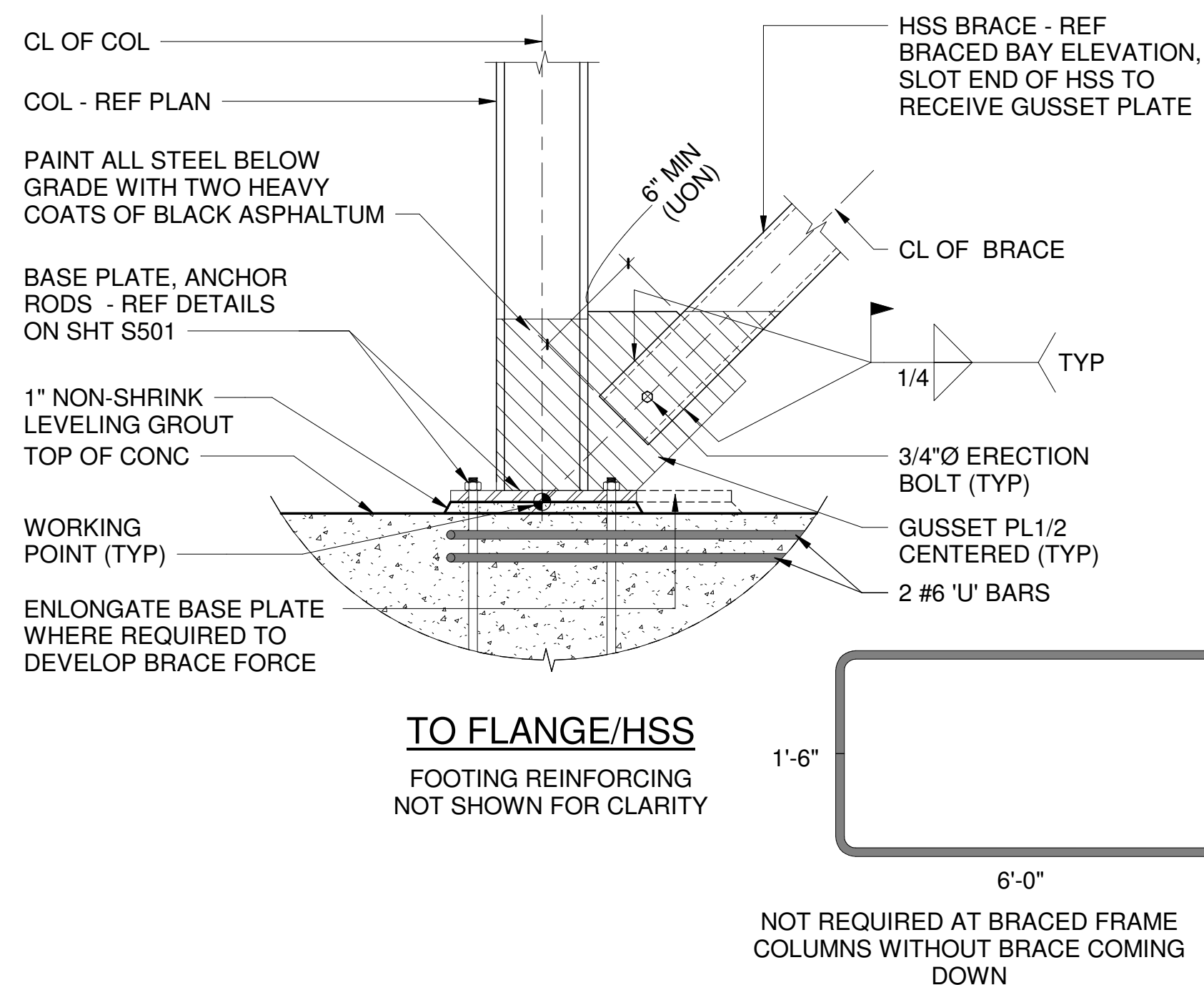
2 BRACED FRAME ELEVATION A.4/6.2-7
1/8" = 1'-0"



3 BRACED FRAME ELEVATION 3/C-D
1/8" = 1'-0"



4 BRACED FRAME ELEVATION 7/B-C
1/8" = 1'-0"



BRACED FRAME NOTES:

1. FRAMED BEAM CONNECTIONS AT BRACED FRAMES SHALL BE CAPABLE OF RESISTING A TOTAL VERTICAL REACTION EQUAL TO ONE HALF (1/2) OF THE MAXIMUM TOTAL UNIFORM LOAD FOR LATERALLY SUPPORTED BEAMS GIVEN IN TABLE 3-6 OF THE "STEEL CONSTRUCTION MANUAL," PLUS THE VERTICAL AND HORIZONTAL COMPONENTS OF THE DIAGONAL BRACE FORCES SHOWN ON THE BRACED FRAME ELEVATIONS. SUBMIT CALCULATIONS FOR ALL BRACED FRAMES AT TENSION (REFER TO PLAN) CONNECTIONS PREPARED BY A FLORIDA PROFESSIONAL ENGINEER.
2. THE DIAGONAL FORCE SHOWN IN KIPS (ASD) ON THE BRACED FRAME ELEVATIONS DENOTES TENSION FORCE AND COMPRESSION FORCE IN THE MEMBER UNLESS OTHERWISE NOTED. BRACE CONNECTIONS SHALL BE DESIGNED TO RESIST BOTH TENSION AND COMPRESSION FORCES.
3. ALL DIAGONAL HSS MEMBERS SHALL BE LONG SIDE VERTICAL UNLESS OTHERWISE NOTED.

BRACED FRAME DETAILS

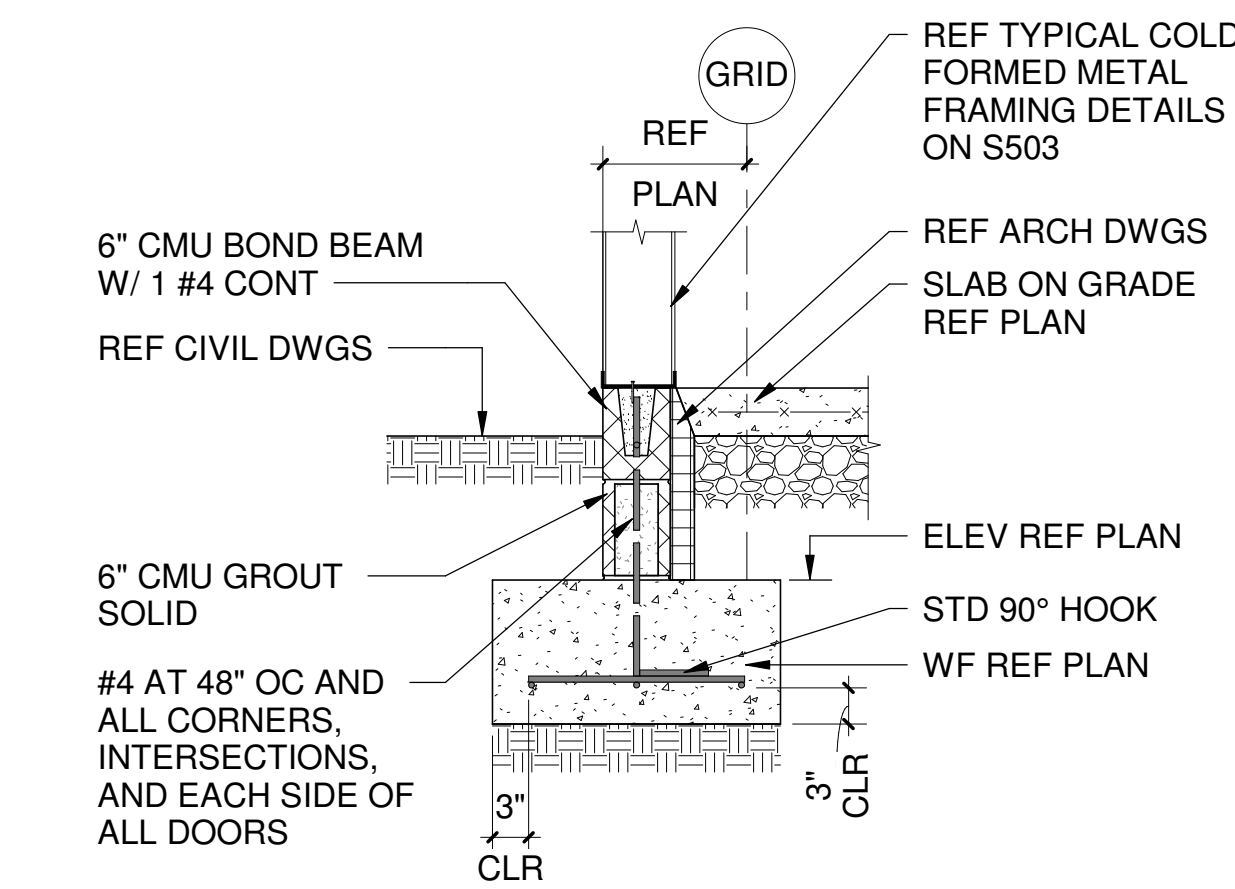
NTS

REVISIONS	DATE	DESCRIPTION
No.	0	ISSUED FOR PERMIT

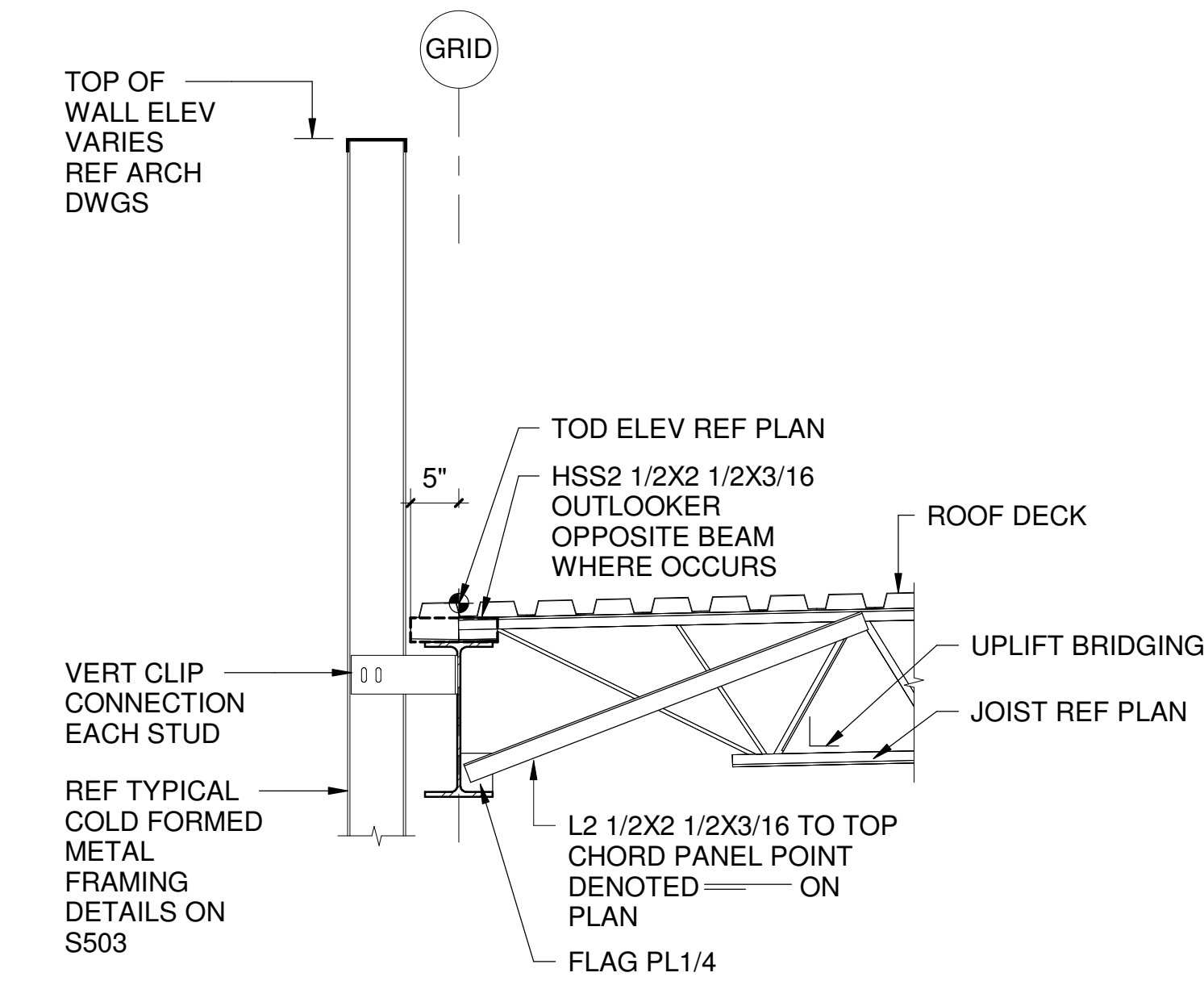
WEB JOB NO.: 24081

DRAWING TITLE:
BRACED FRAME
DETAILS

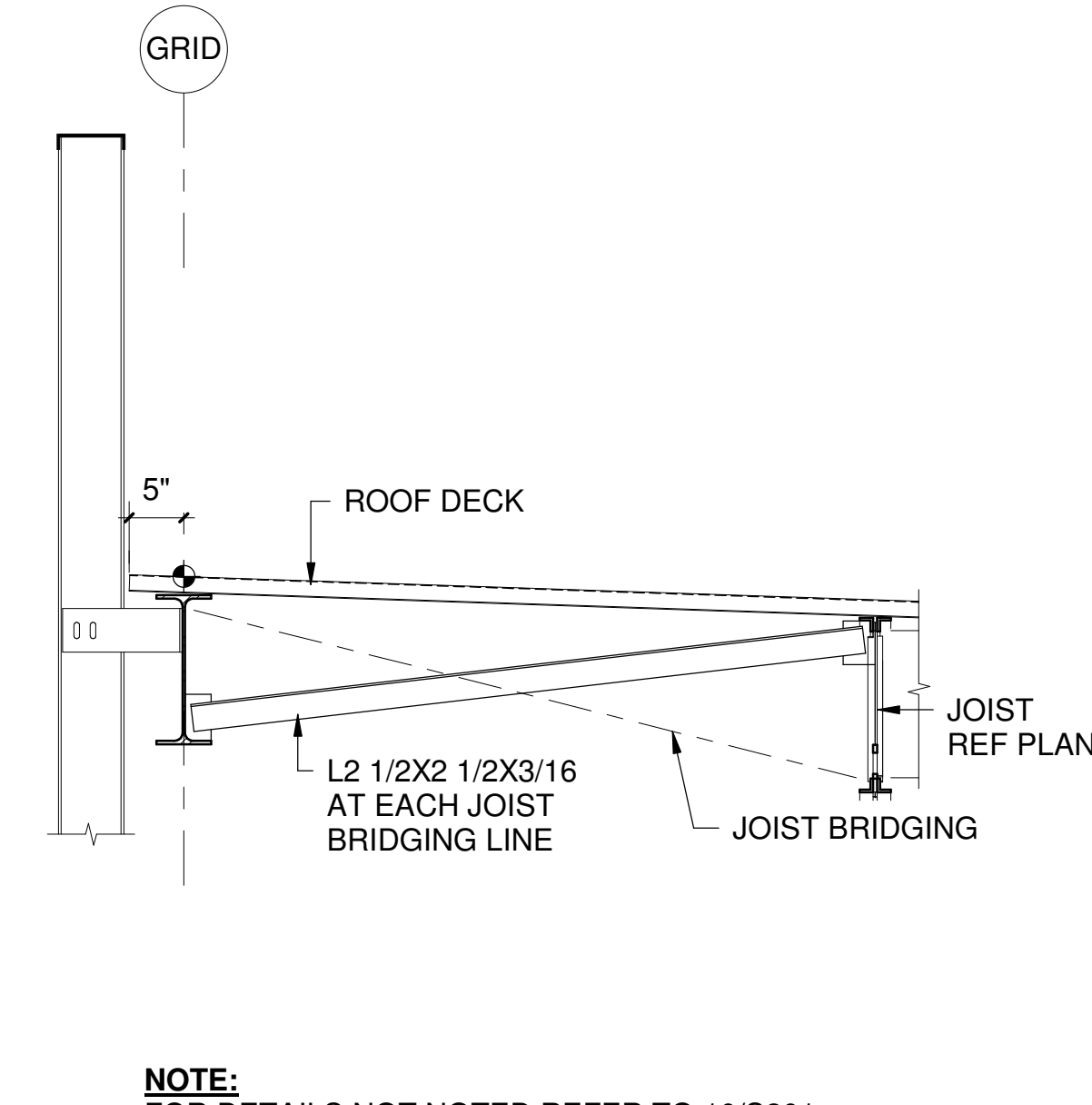
DRAWING NUMBER:
S201



1 SECTION
3/4" = 1'-0"

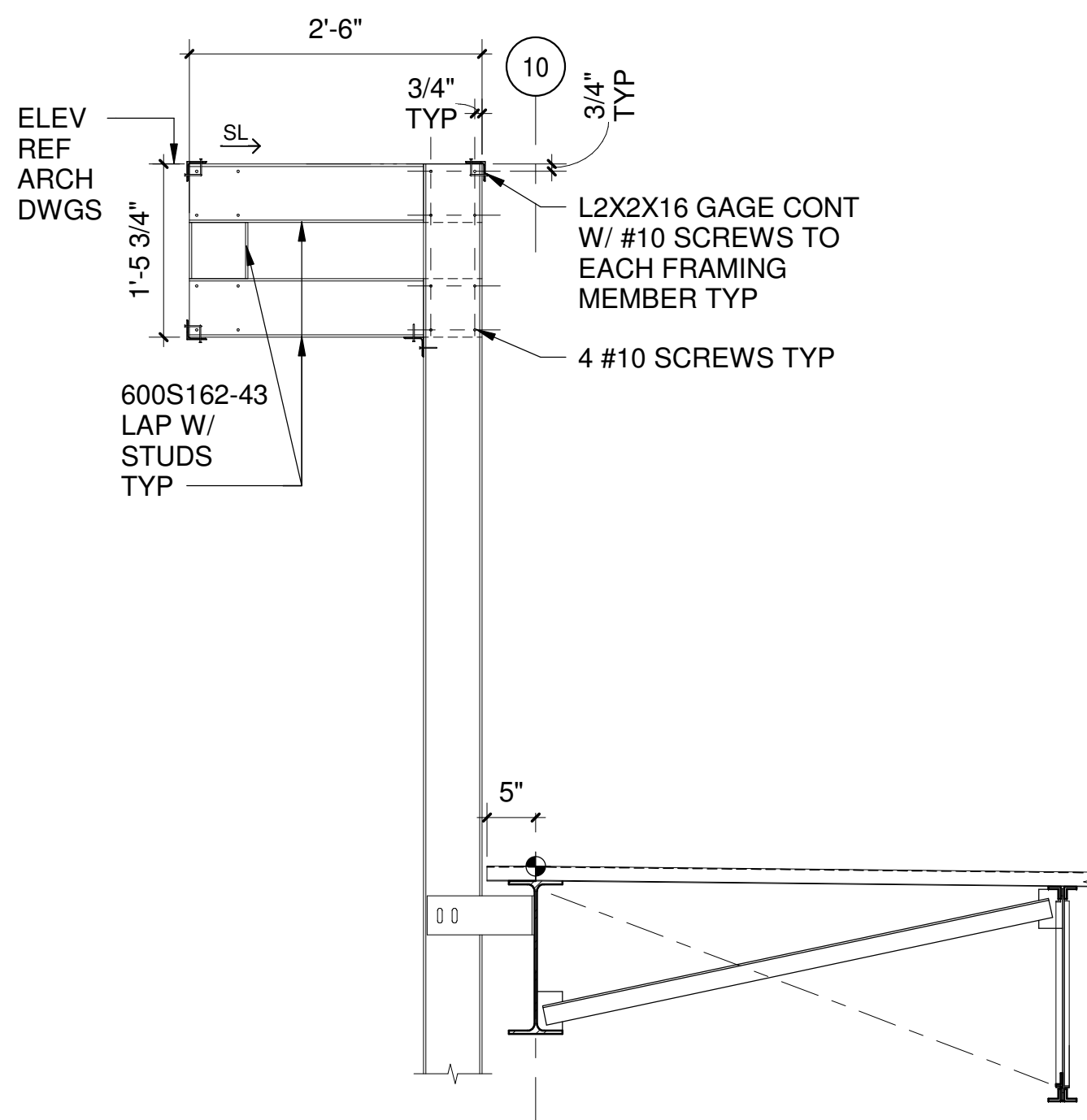


10 SECTION
3/4" = 1'-0"



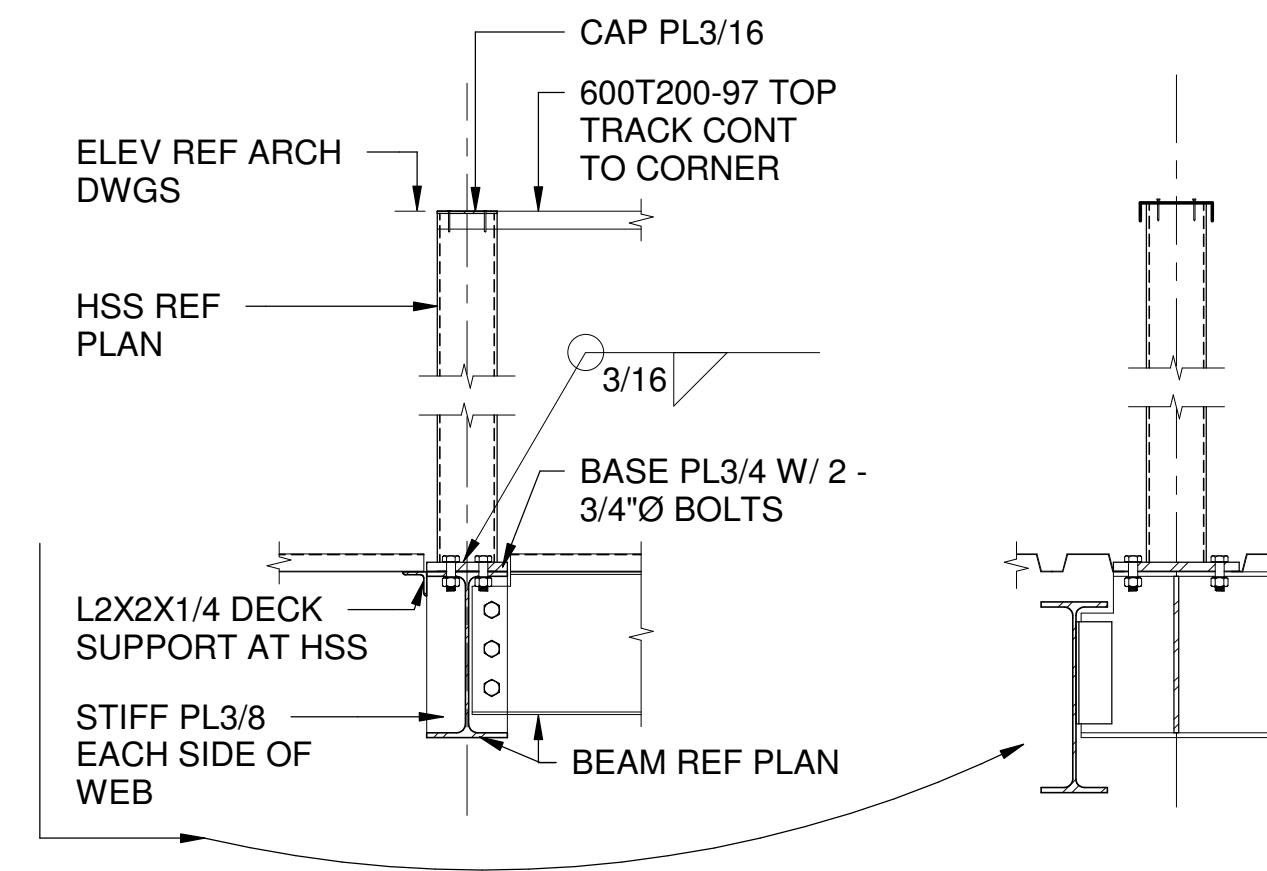
11 SECTION
3/4" = 1'-0"

NOTE:
FOR DETAILS NOT NOTED REFER TO 10/S301.

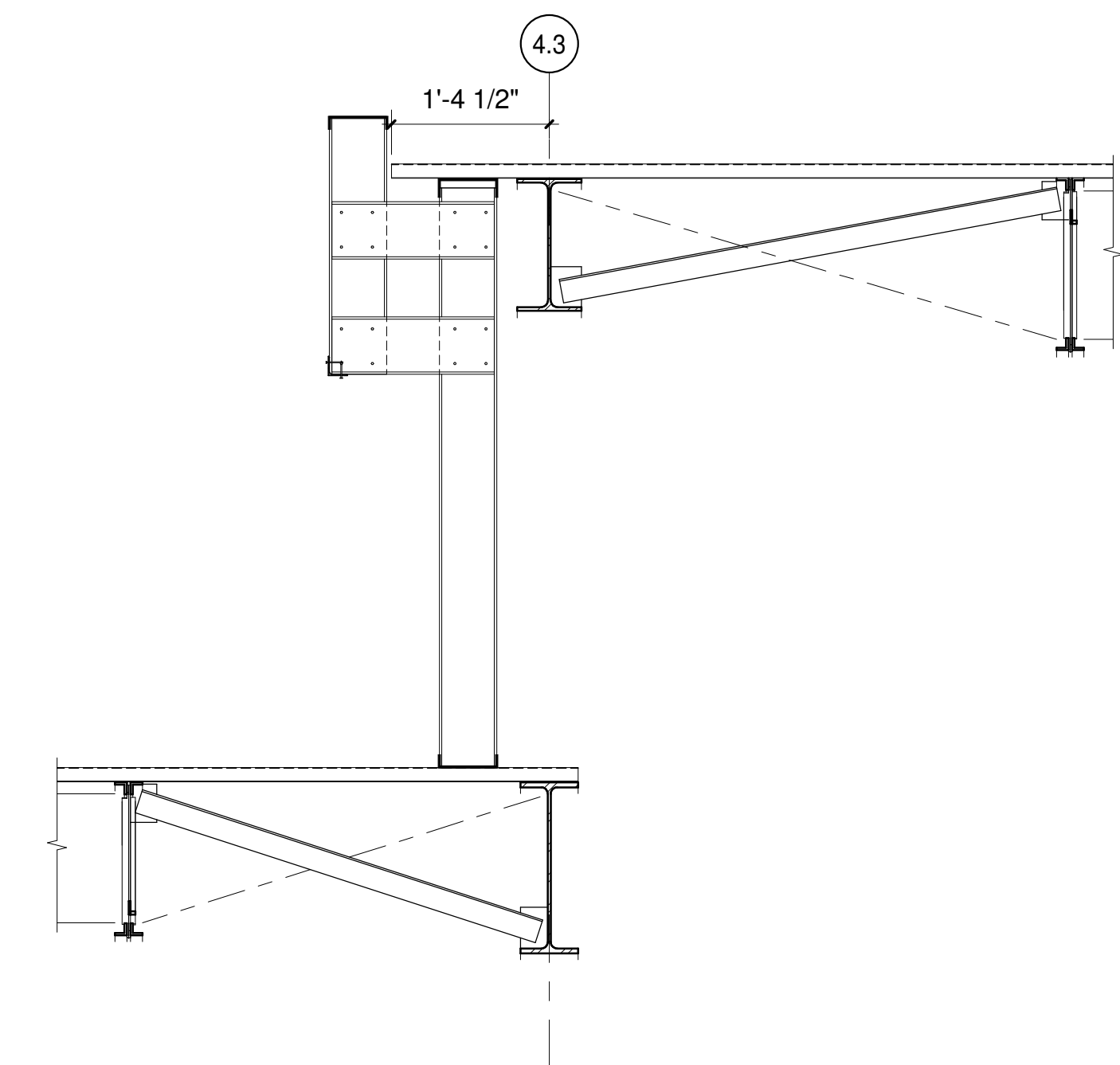


12 SECTION
3/4" = 1'-0"

NOTE:
FOR DETAILS NOT NOTED REFER TO 11/S301.

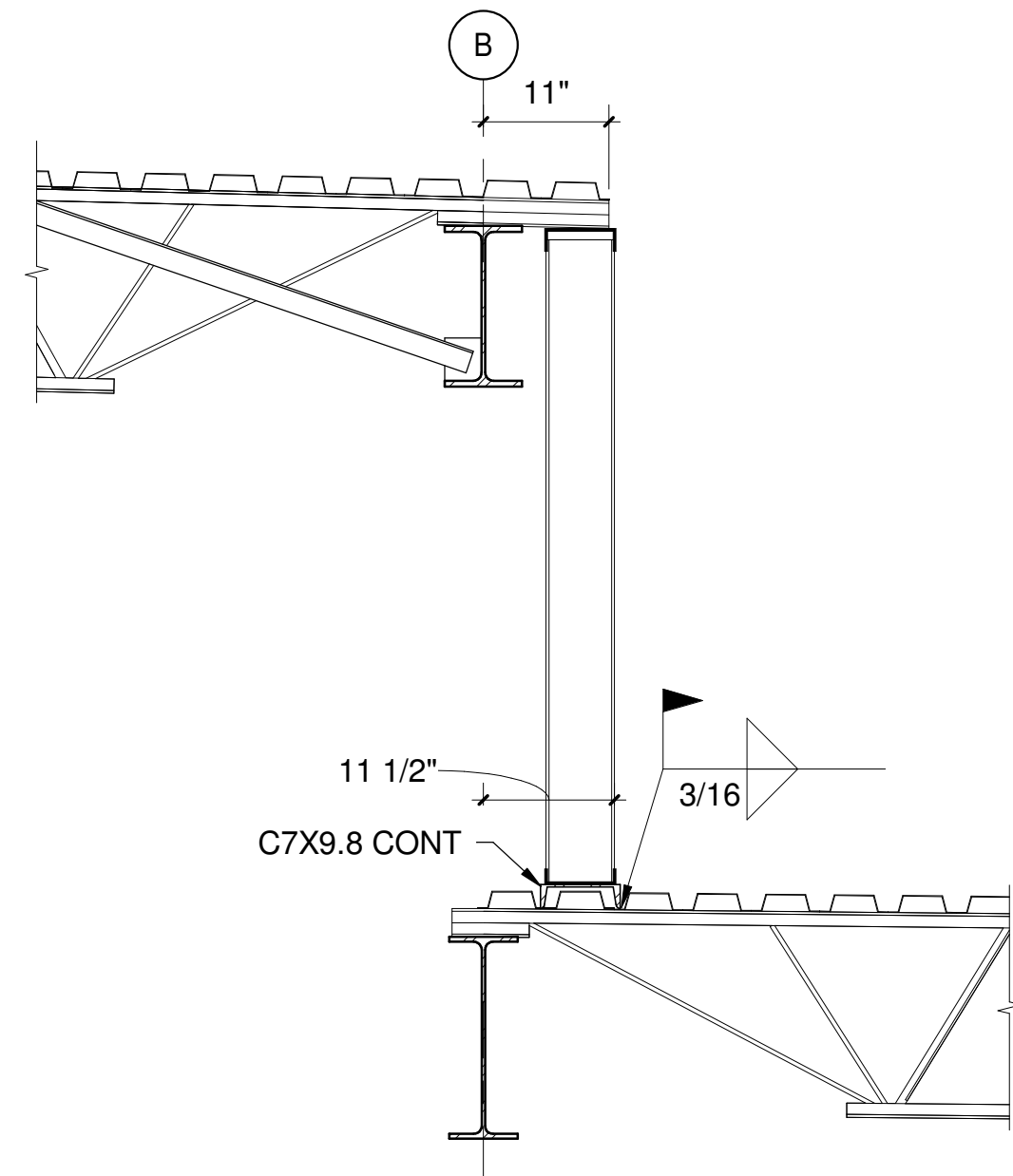


13 SECTION
3/4" = 1'-0"



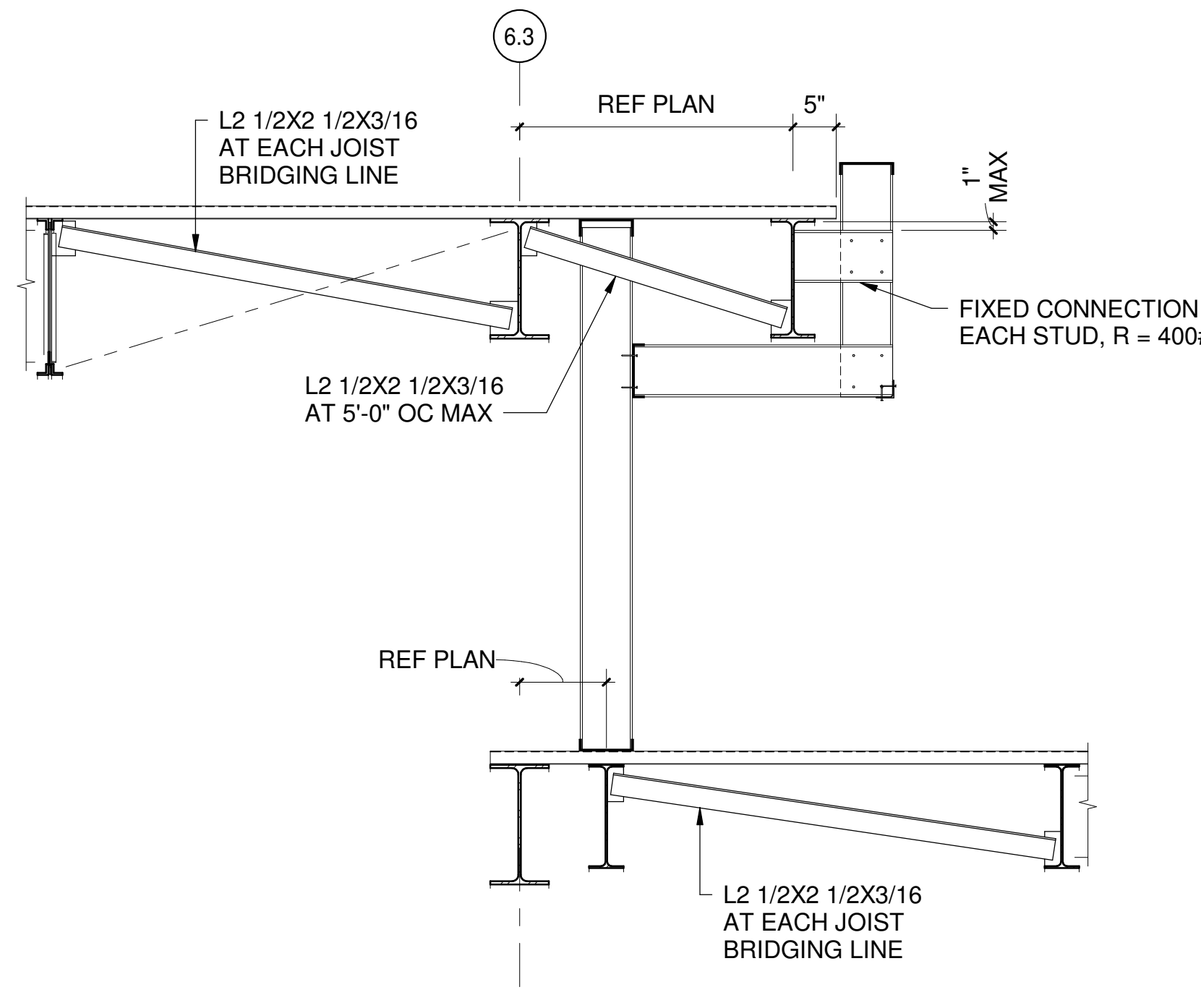
14 SECTION
3/4" = 1'-0"

NOTE:
FOR DETAILS NOT NOTED REFER TO 11 AND 12/S301.



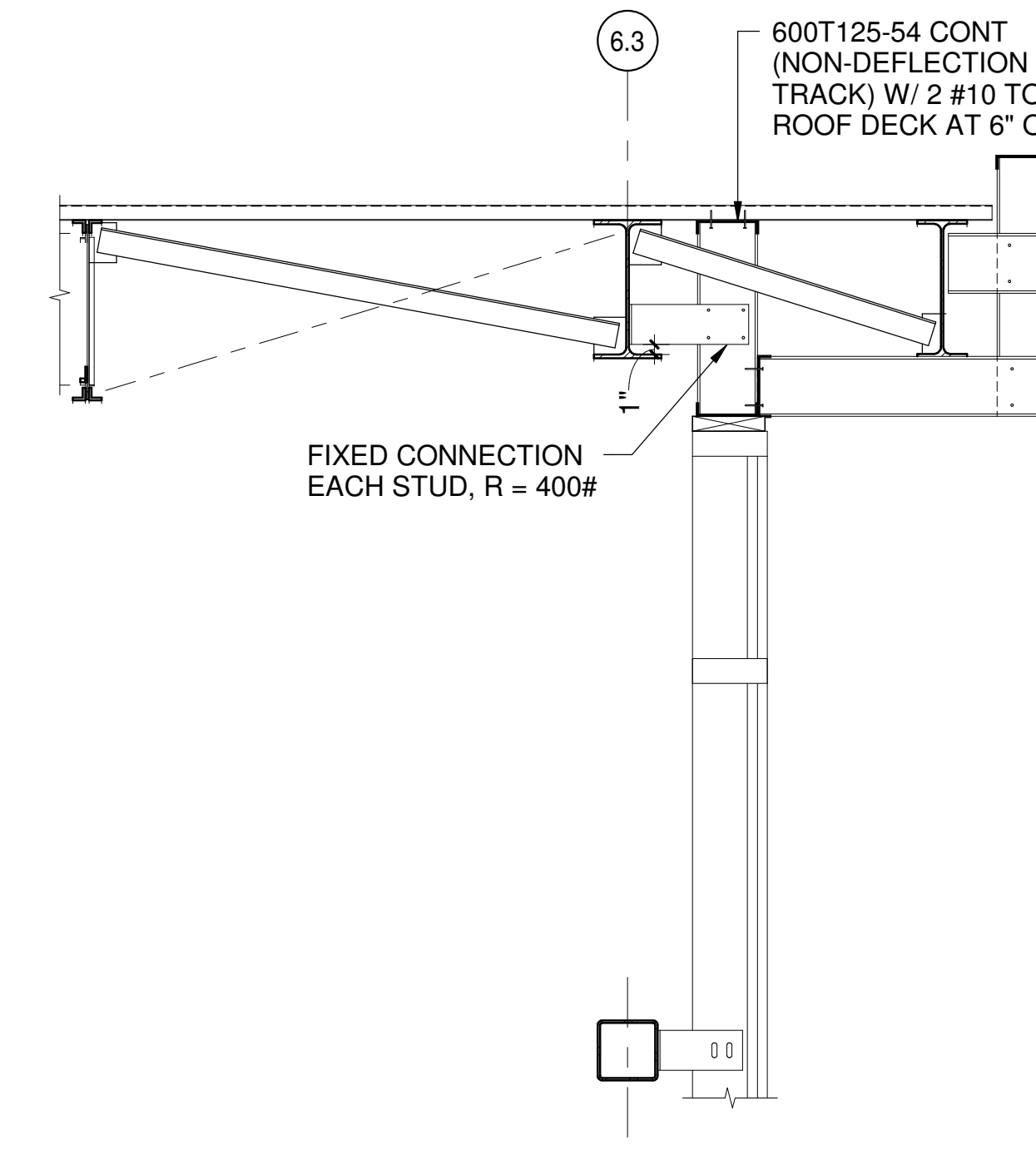
15 SECTION
3/4" = 1'-0"

NOTE:
FOR DETAILS NOT NOTED REFER TO 10/S301.



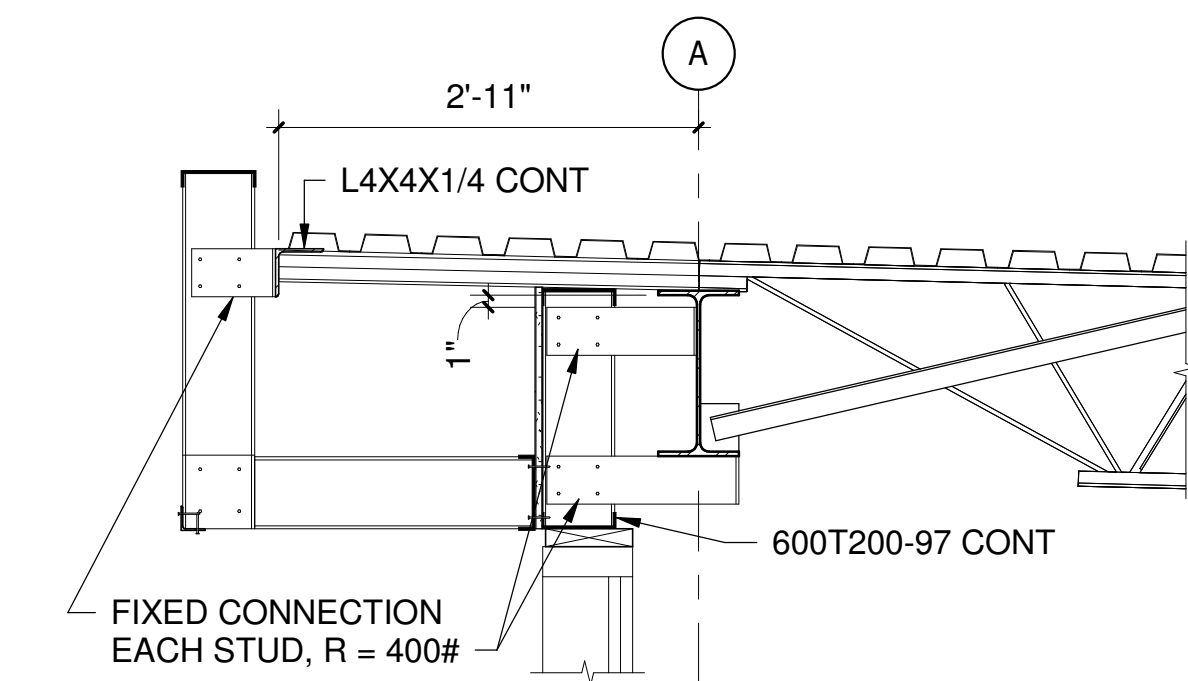
16 SECTION
3/4" = 1'-0"

NOTE:
FOR DETAILS NOT NOTED REFER TO 11, 12 AND 14/S301.



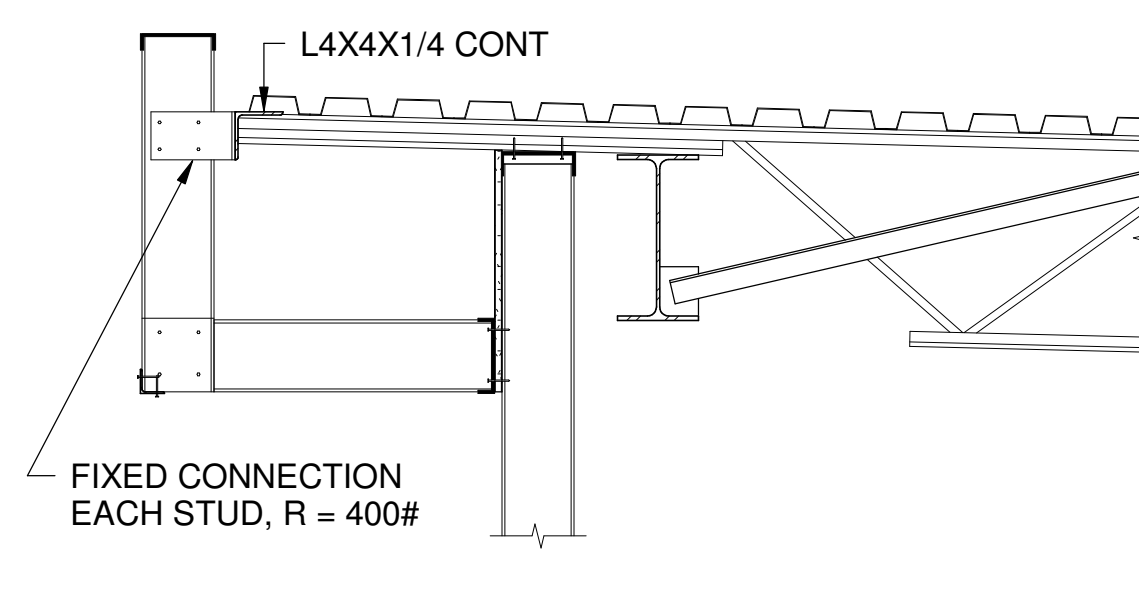
17 SECTION
3/4" = 1'-0"

NOTE:
FOR DETAILS NOT NOTED REFER TO 16 AND 19/S301.

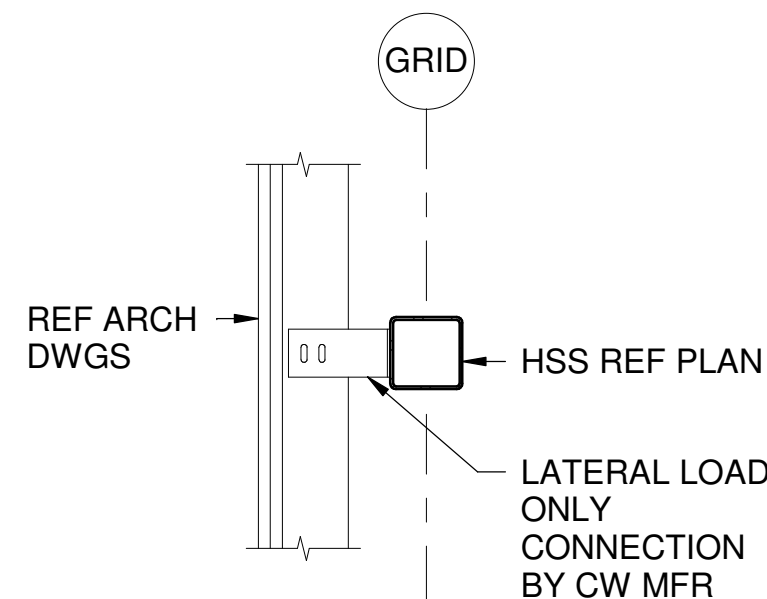


18 SECTION
3/4" = 1'-0"

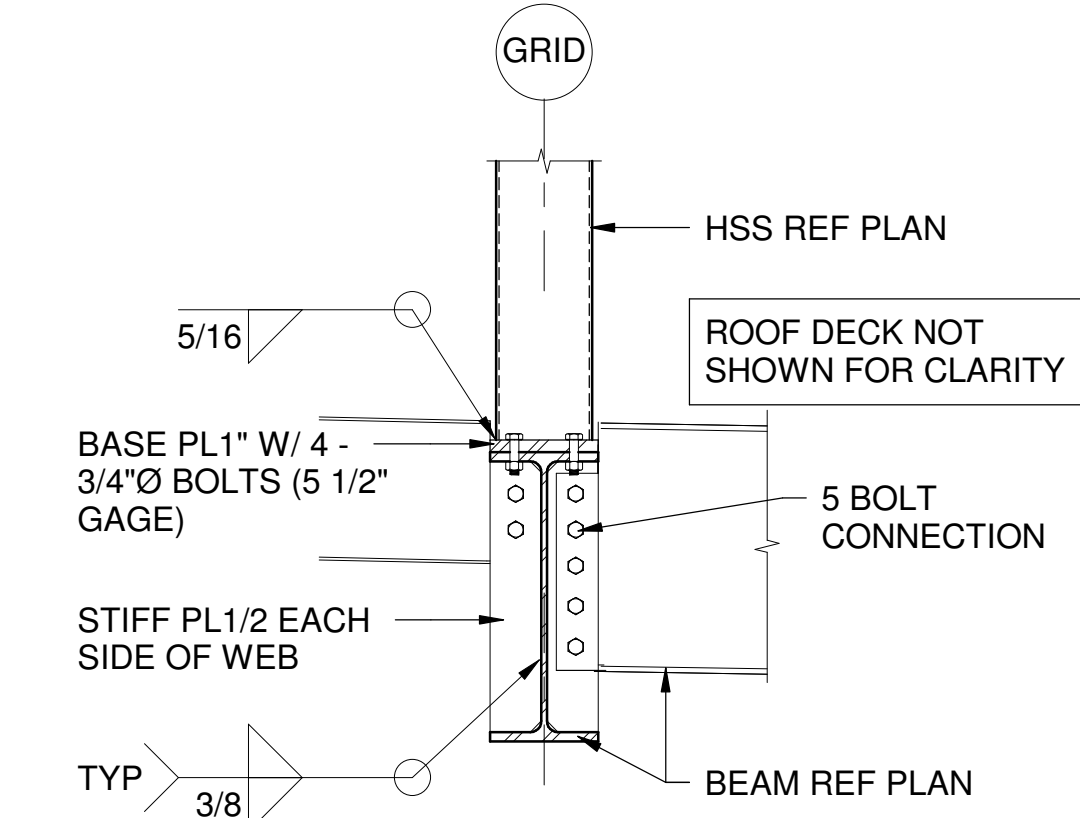
NOTE:
FOR DETAILS NOT NOTED REFER TO 12/S301.



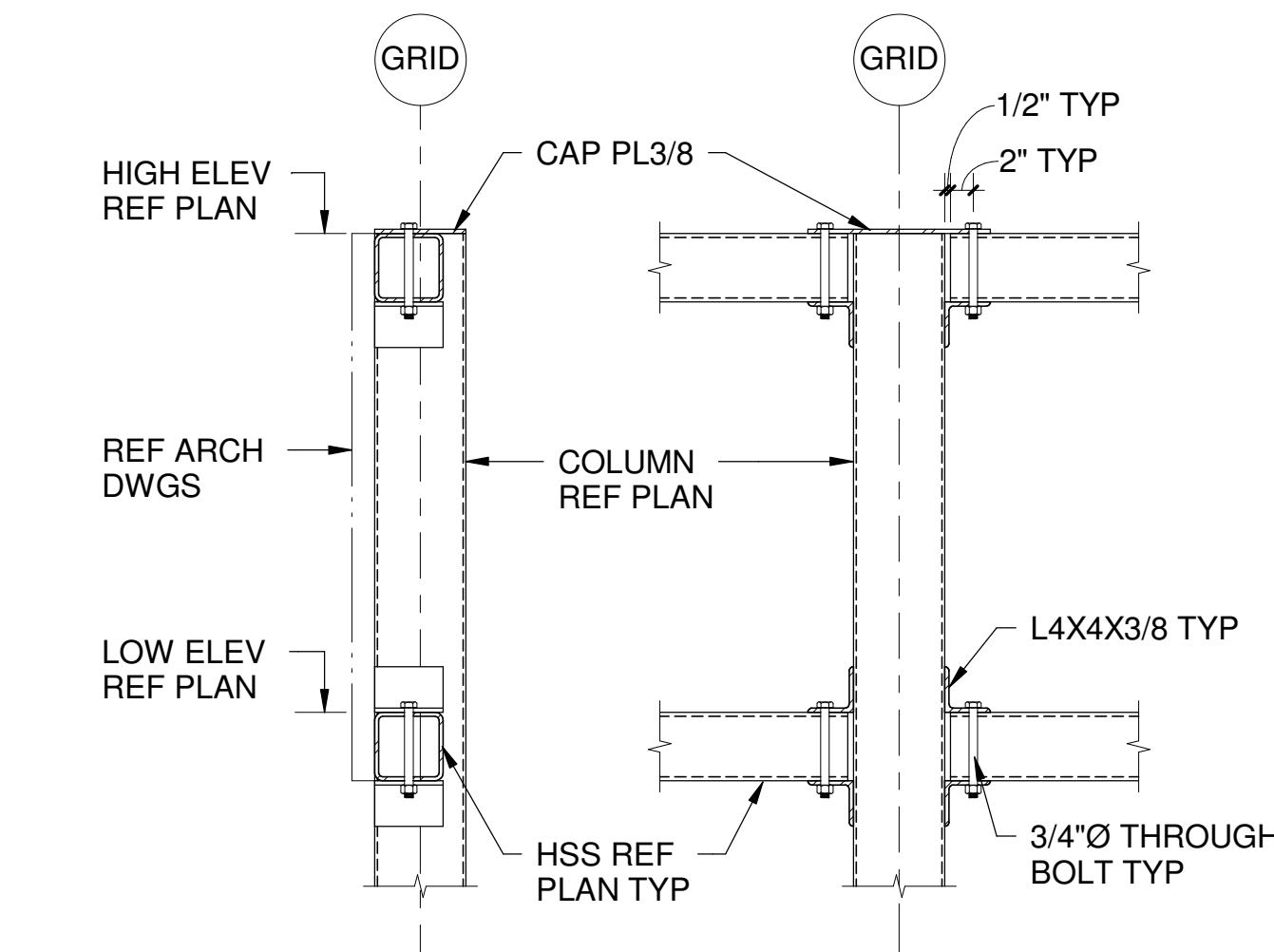
19 SECTION
3/4" = 1'-0"



21 SECTION
3/4" = 1'-0"



22 SECTION
3/4" = 1'-0"

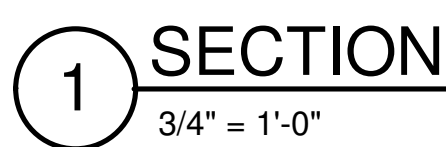


REVISIONS	DATE	DESCRIPTION	ISSUED FOR PERMIT
No.	0	6/13/25	

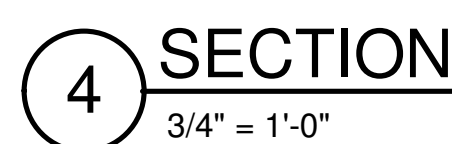
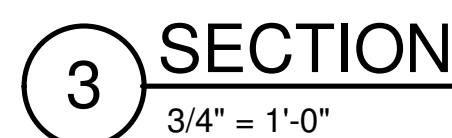
WEB JOB NO.: 24081

DRAWING TITLE:
SECTIONS

DRAWING NUMBER:
S301



2 SECTION
3/4" = 1'-0"



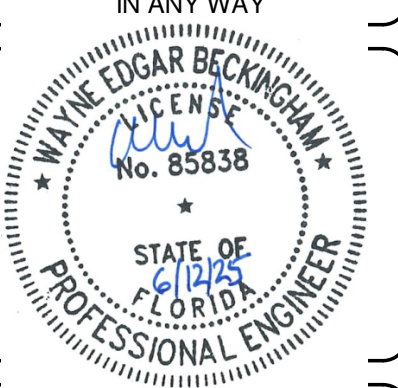
**BBLMedical
Facilities**

**302 Washington Ave. Ext.
Albany, New York 12203**

518-452-8200 fax 518-452-2898



NOTE: IT IS A VIOLATION
OF THE FLORIDA
BOARD OF PROFESSIONAL
ENGINEERS
LAW FOR ANY PERSON,
UNLESS ACTING UNDER THE
DIRECTION OF A LICENSED
ARCHITECT, TO ALTER AN ITEM
AS INDICATED ON DRAWING,
JAN AND CARRIES



Florida Cancer Specialists
Proposed Medical Office Building
Innovation Way, Port St. Lucie, FL

REVISIONS		
No.	DESCRIPTION ISSUED FOR PERMIT	DATE
0		6/13/25

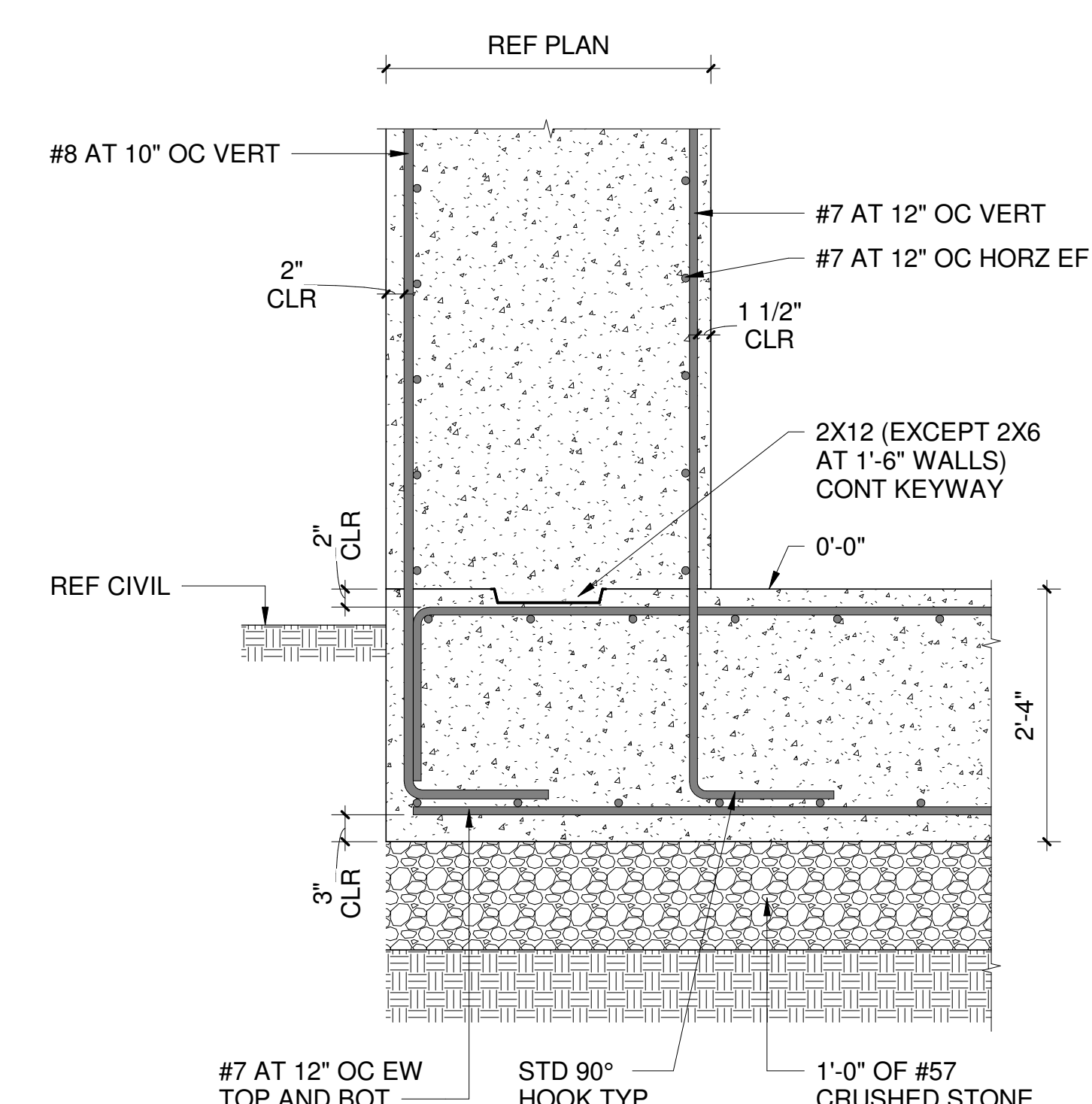
WEB JOB NO.: 24081

DRAWING TITLE:

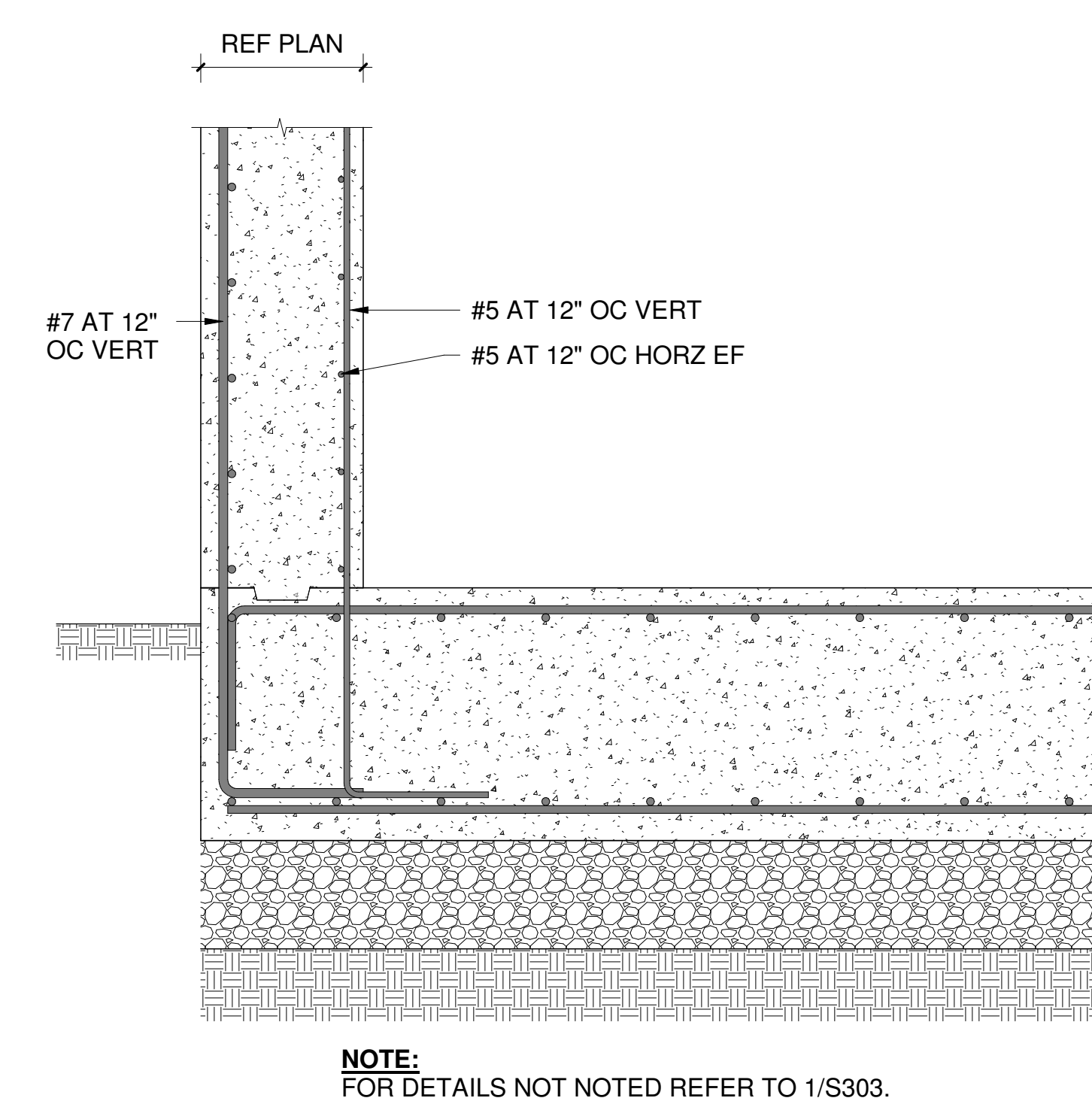
SECTIONS

DRAWING NUMBER:

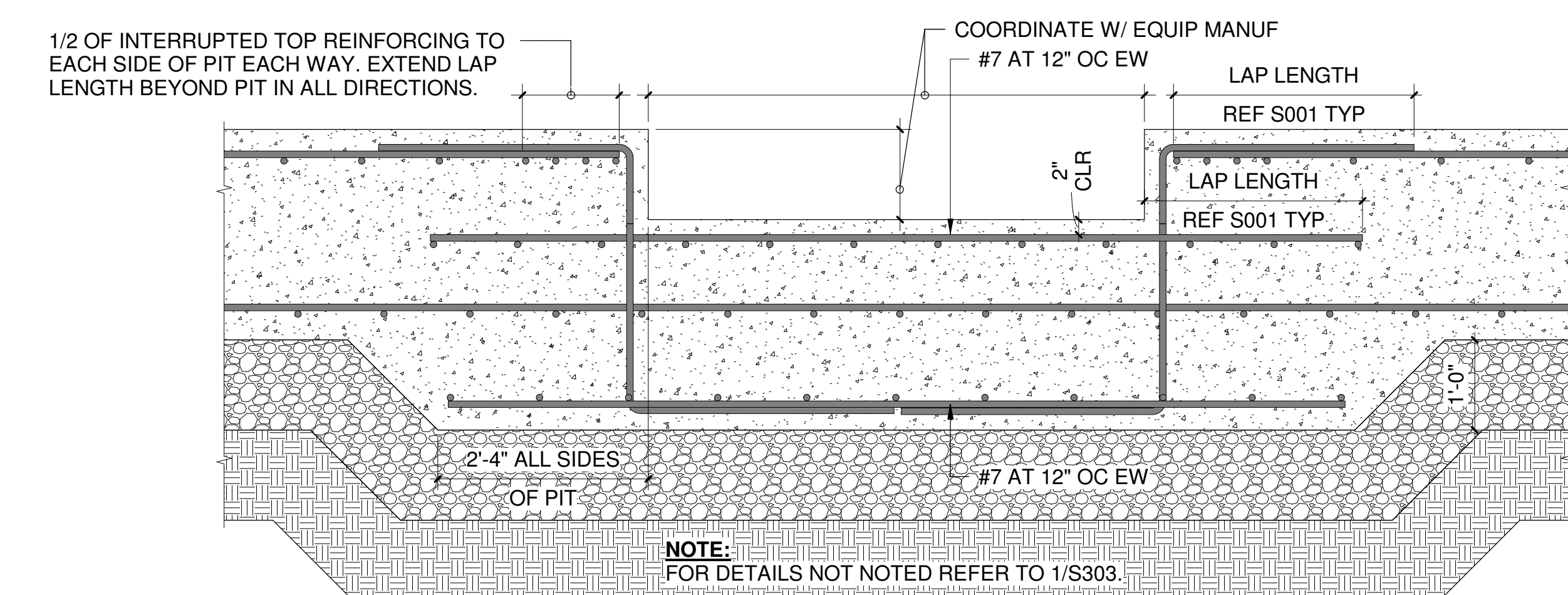
S302



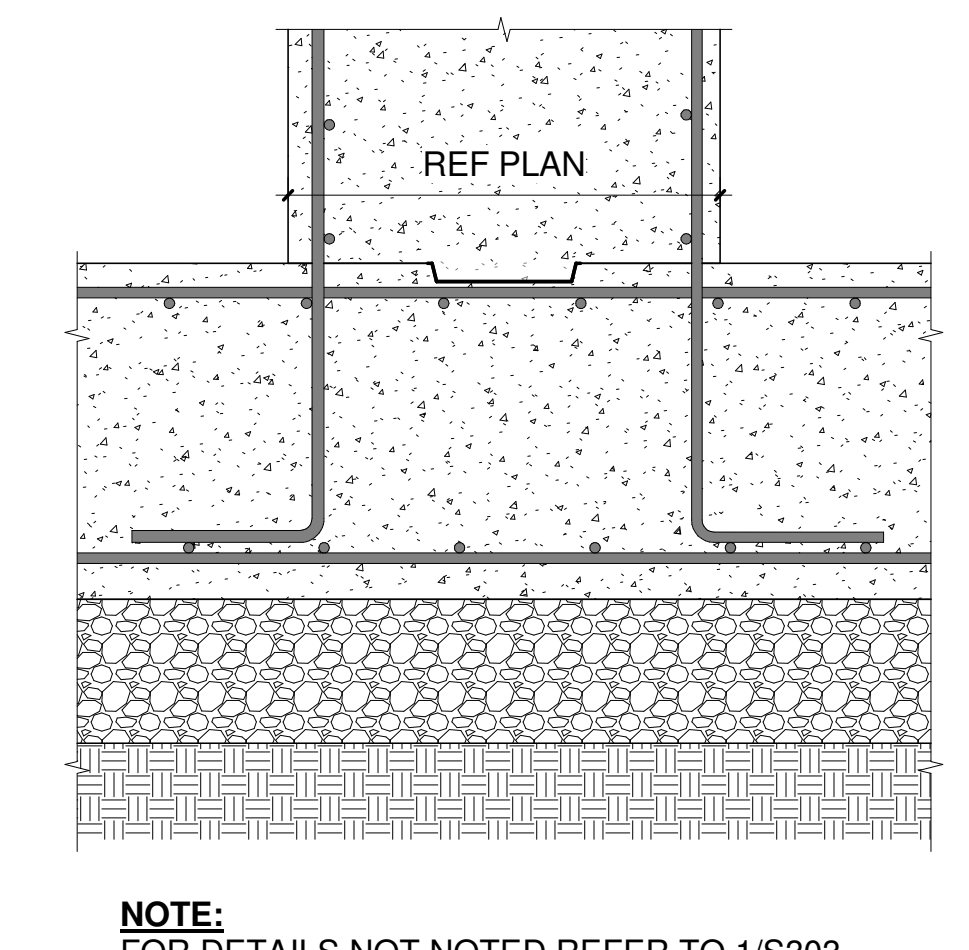
1 SECTION
3/4" = 1'-0"



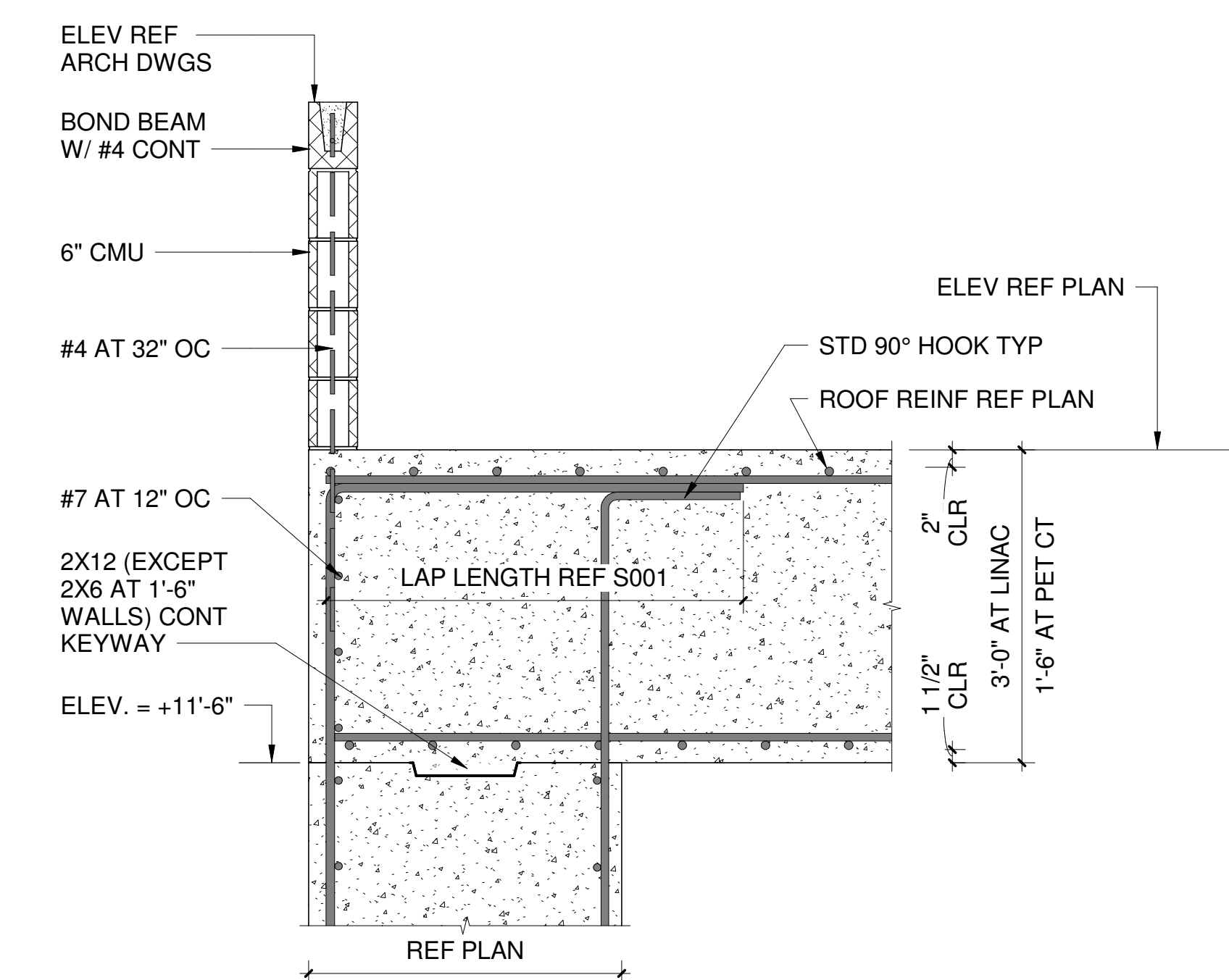
2 SECTION
3/4" = 1'-0"



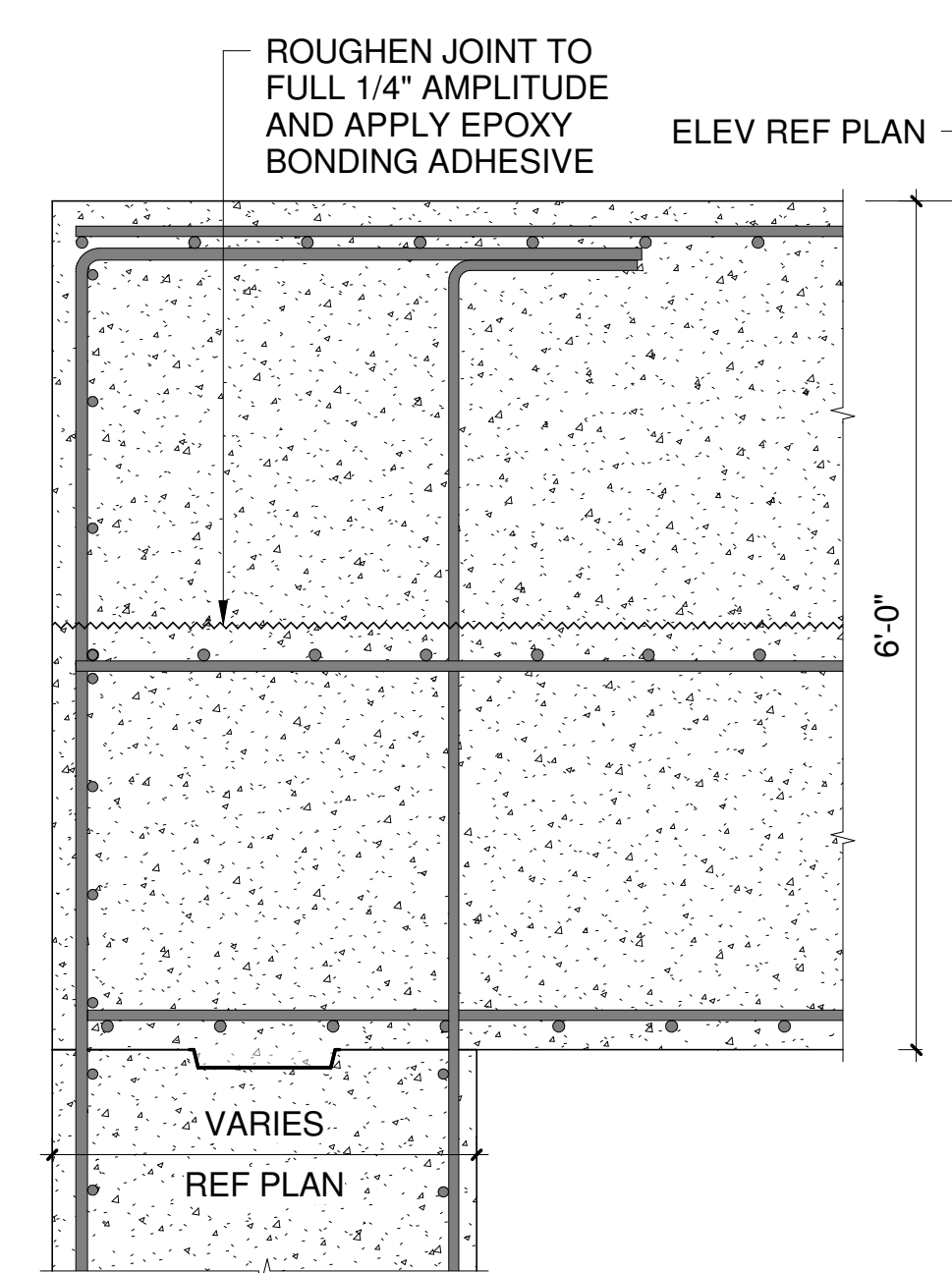
3 SECTION
3/4" = 1'-0"



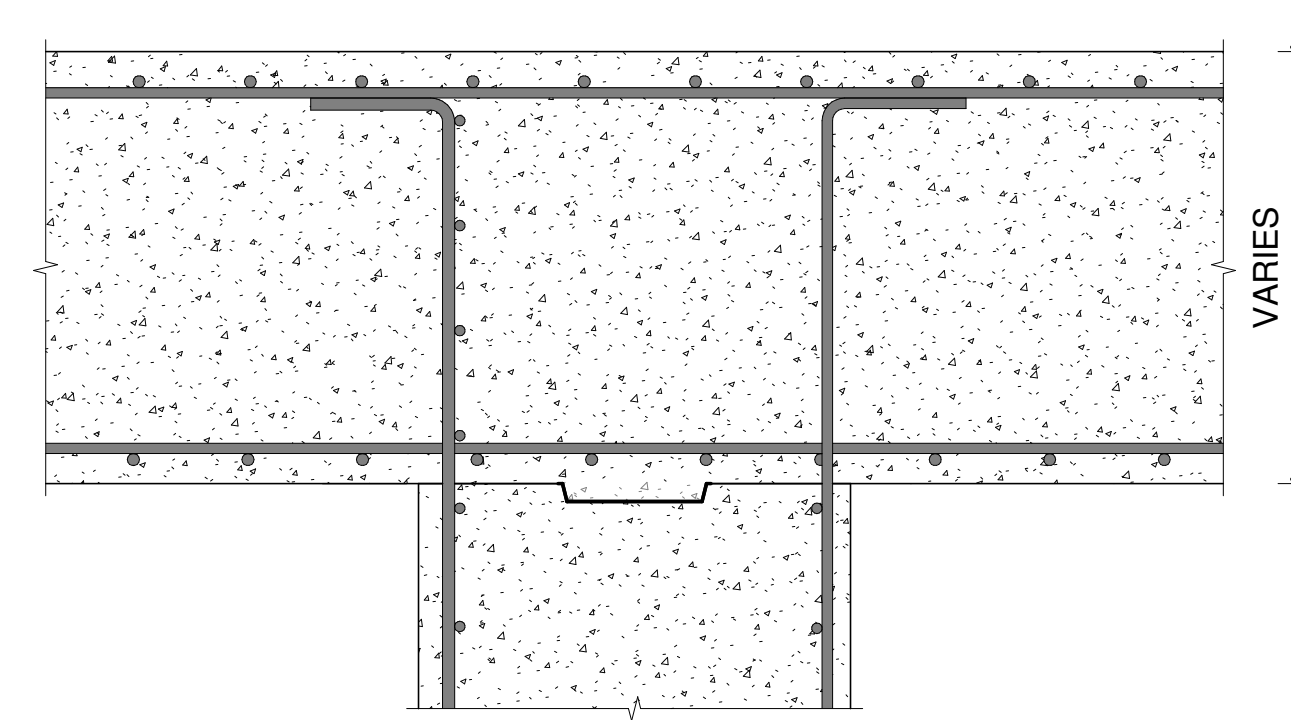
4 SECTION
3/4" = 1'-0"



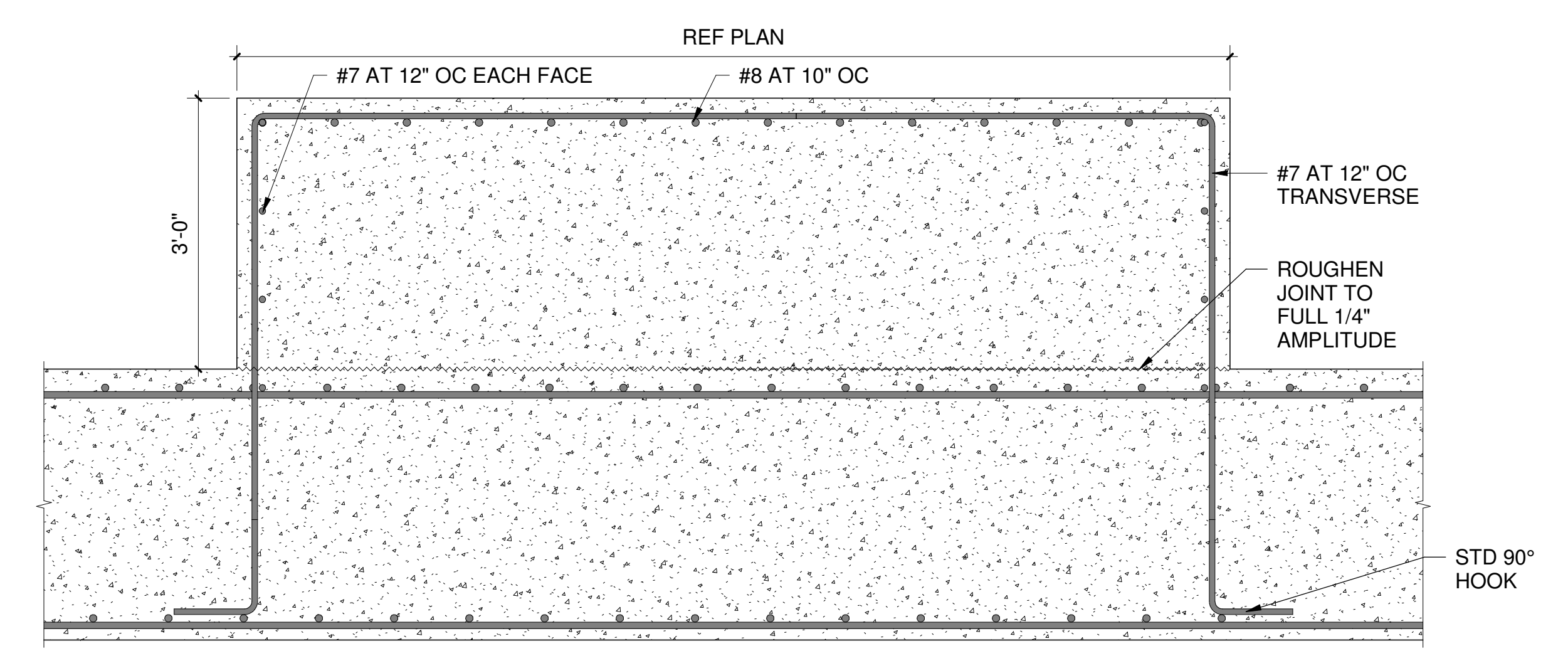
5 SECTION
3/4" = 1'-0"



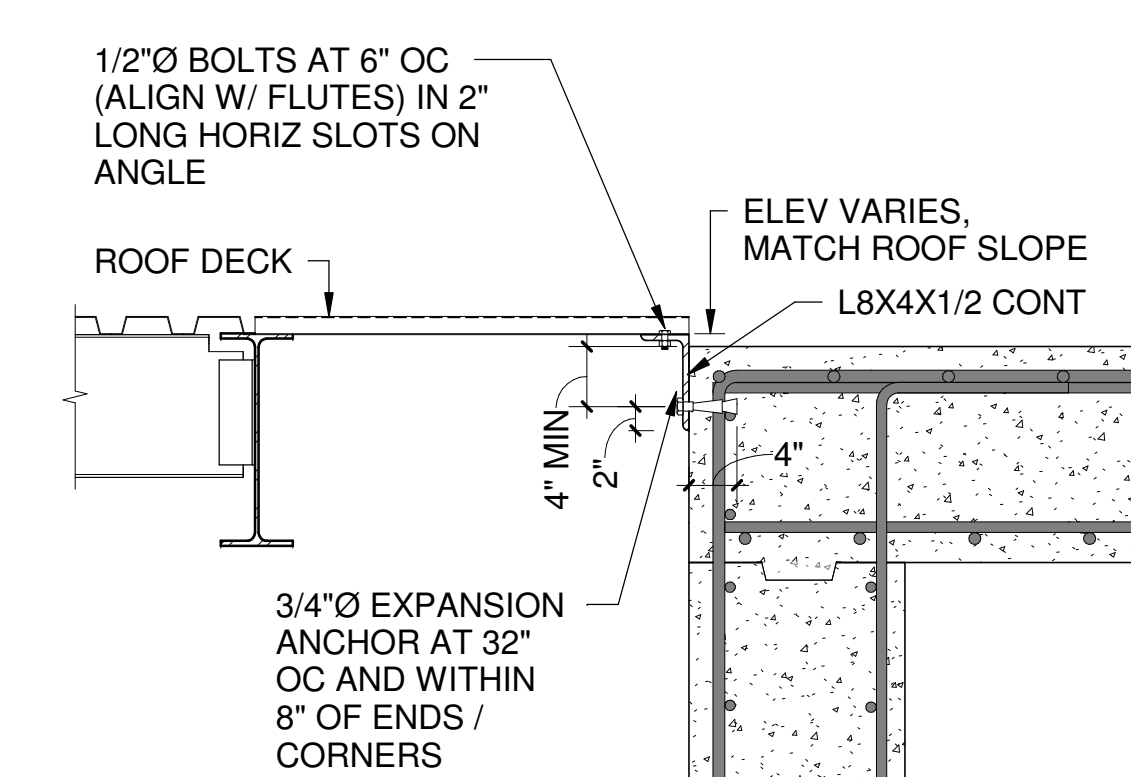
6 SECTION
3/4" = 1'-0"



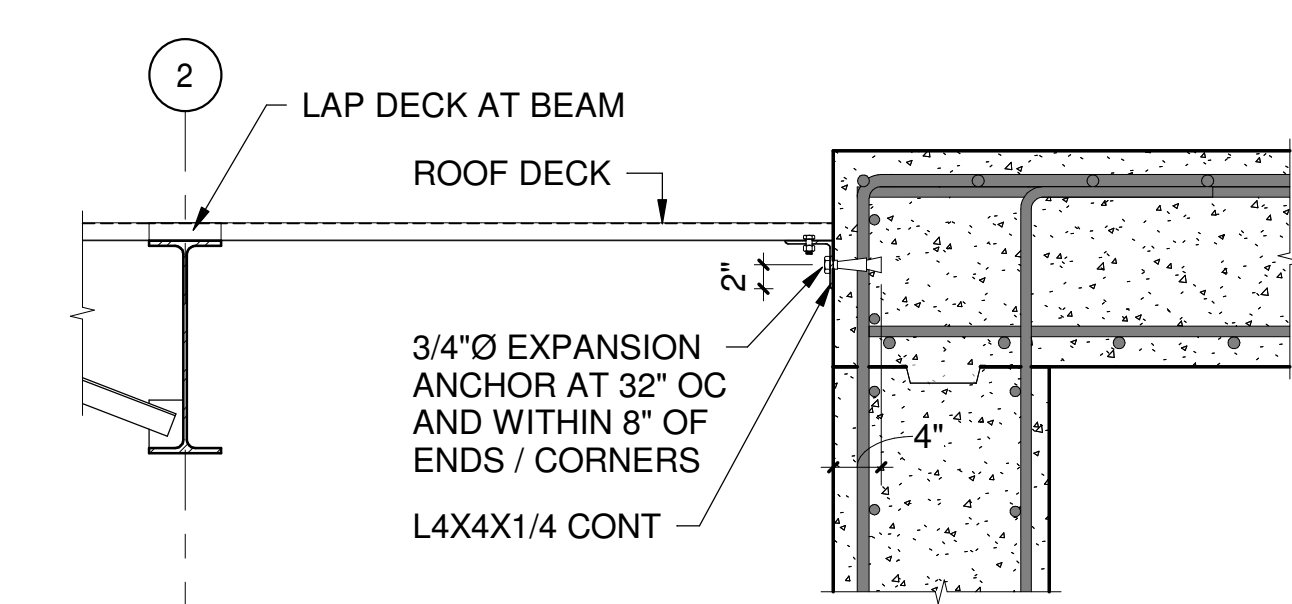
7 SECTION
3/4" = 1'-0"



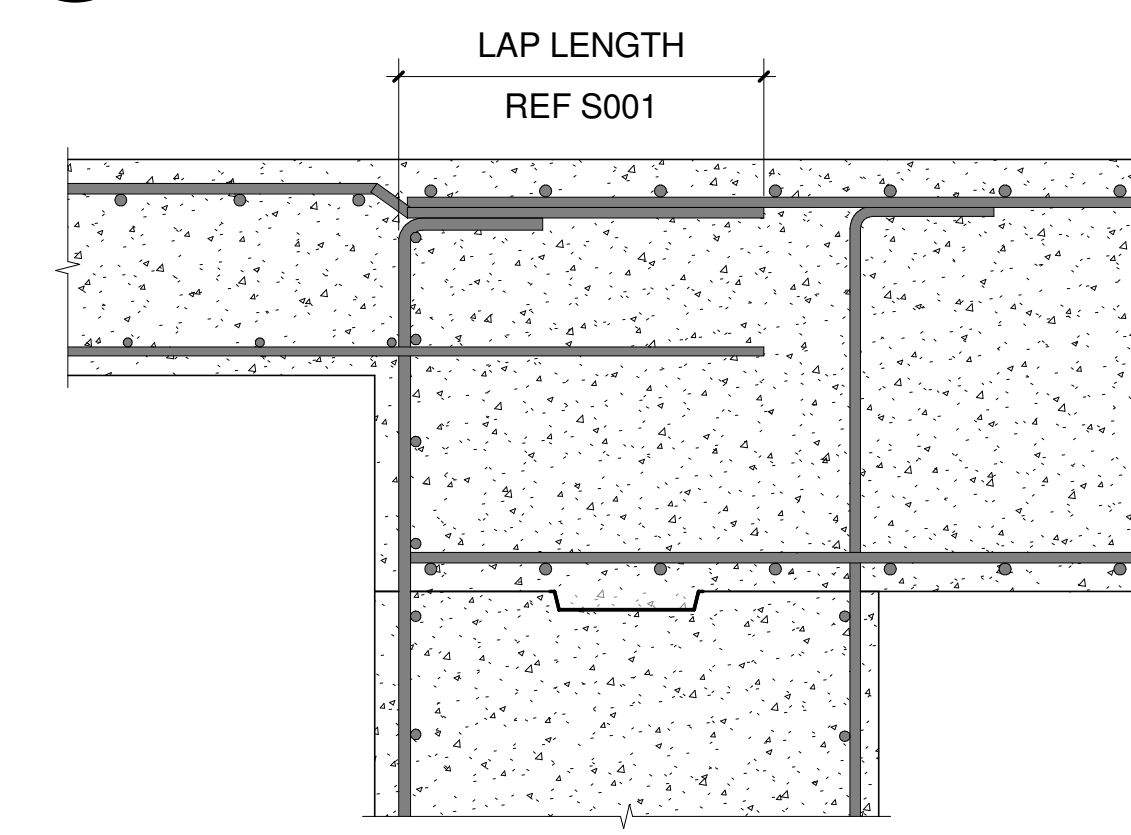
8 SECTION
3/4" = 1'-0"



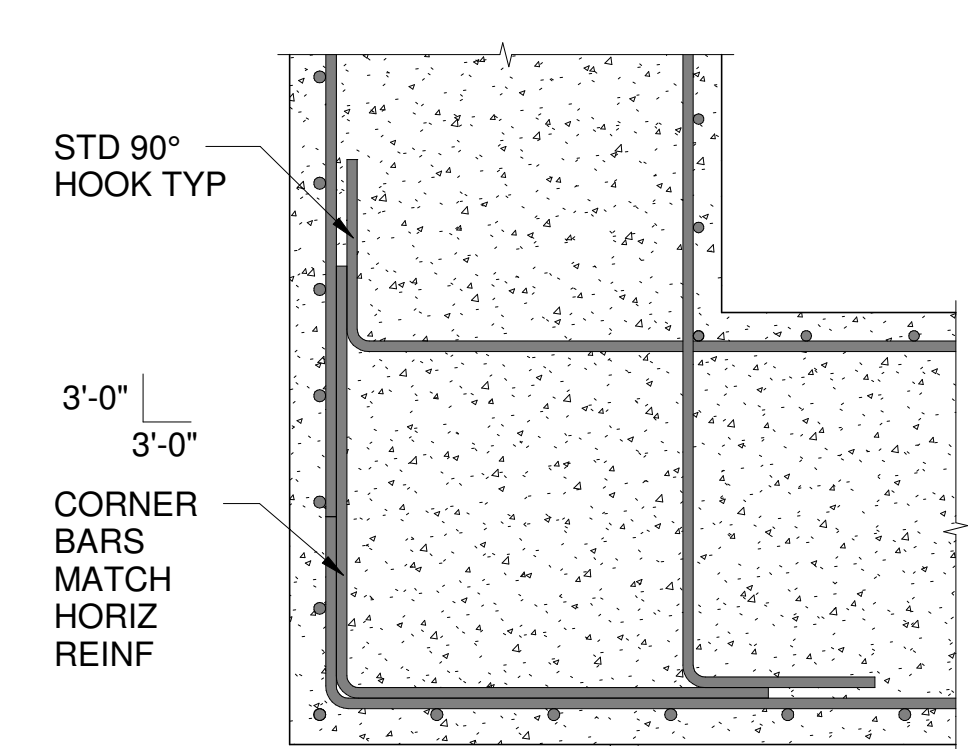
9 SECTION
3/4" = 1'-0"



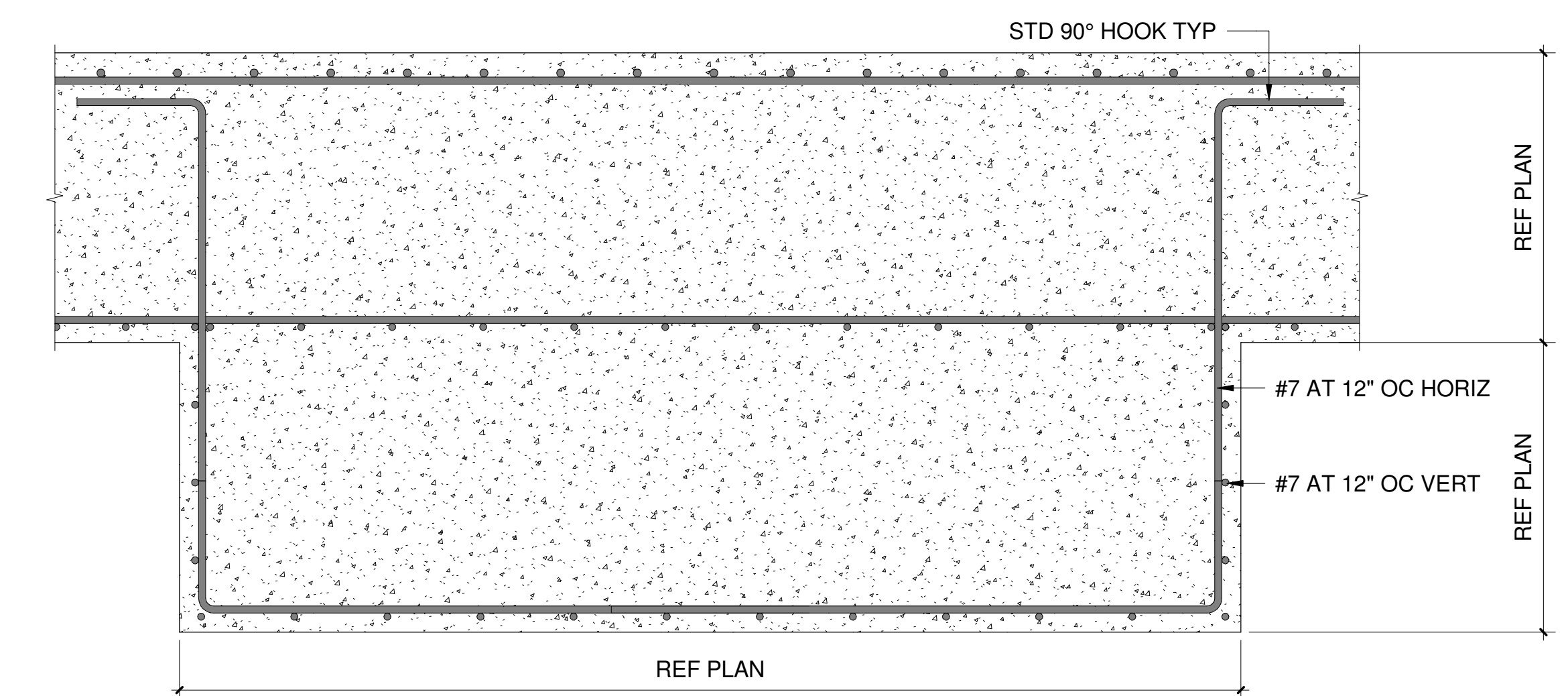
10 SECTION
3/4" = 1'-0"



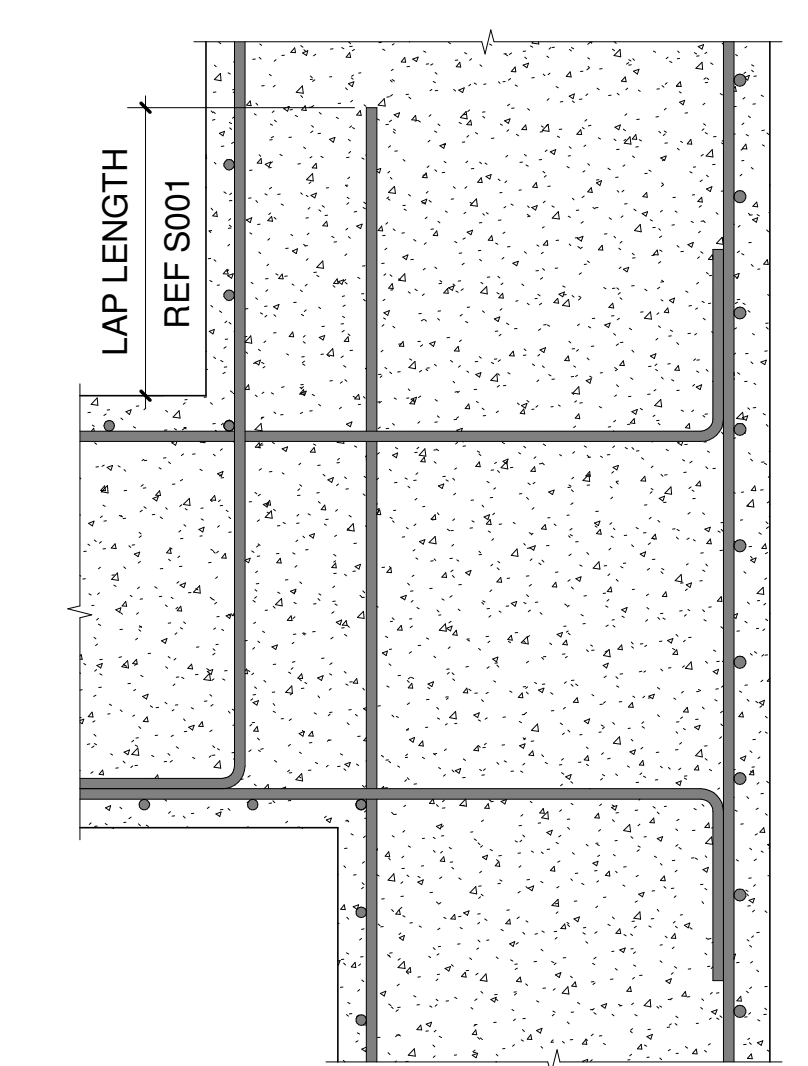
11 SECTION
3/4" = 1'-0"



A DETAIL
3/4" = 1'-0"



B DETAIL
3/4" = 1'-0"



C DETAIL
3/4" = 1'-0"

REVISIONS	DATE	DESCRIPTION
No.	0	6/13/25
1		
2		
3		
4		
5		
6		
7		
8		
9		
10		
11		
12		
13		
14		
15		
16		
17		
18		
19		
20		



NOTE:
SLEEVES SHALL NOT
BE LOCATED IN OR
UNDER COIL FTGS.



NTS

NTSNTS

WALL OPENING NOTES:

ALL OPENINGS IN STRUCTURAL WALLS REQUIRE ADDITIONAL REINFORCING EXCEPT WHERE OPENING SIZE OR LOCATION IS SUCH THAT REINFORCING STEEL IS NOT INTERRUPTED. CONDUITS, SMALL PIPES, AND OTHER SMALL SLEEVES THAT DO NOT REQUIRE PRECISE LOCATION SHALL BE SHIFTED SLIGHTLY TO CLEAR REINFORCING. WHERE RISER DUCTS OR OTHER SLEEVES ARE LARGER THAN THE SPACING BETWEEN THE BARS AND ARE SUCH THAT THE REINFORCING CAN NOT BE SHIFTED TO CLEAR.

[illegible]

FLOOR SINK

HOUSEKEEPING PAD

ISOLATION PAD

TRENCH DRAIN

PAD NOTES:

1. REF MEP DRAWINGS FOR DIMENSIONS AND LOCATION OF HOUSE KEEPING AND ISOLATION PADS. REF CIVIL DWGS FOR EXTERIOR PAD LOCATIONS AND SIZES.
2. GENERAL CONTRACTOR COORDINATE ANY REQD ANCHORAGE EMBEDS NEEDED FOR THE EQUIP PLACED ON THE PAD.

SECTION

KEYED CONSTRUCTION JOINT

SAWED JOINT

TYPICAL SLAB ON GRADE DETAILS

NTS

OPPOSITE BEAM

NOTE: WHERE POSSIBLE BUTT JOIST ENDS TOGETHER OVER BEAM CENTERLINE AND PROVIDE SPECIAL JOIST END PER SJI REQUIREMENTS. WHERE ALIGNING OF JOIST OVER BEAM IS NOT POSSIBLE, OFFSET JOIST TO PROVIDE REQUIRED MINIMUM BEARING.

NOTE: METAL ROOF DECK, EDGE ANGLE, AND ARCH FINISH NOT SHOWN FOR CLARITY

INTERIOR ROOF

EXTERIOR ROOF/OPPOSITE BEAM

TYPICAL JOIST BEARING DETAILS

NTS

SECTION

TYPICAL JOIST ON COLUMN CENTERLINE DETAILS

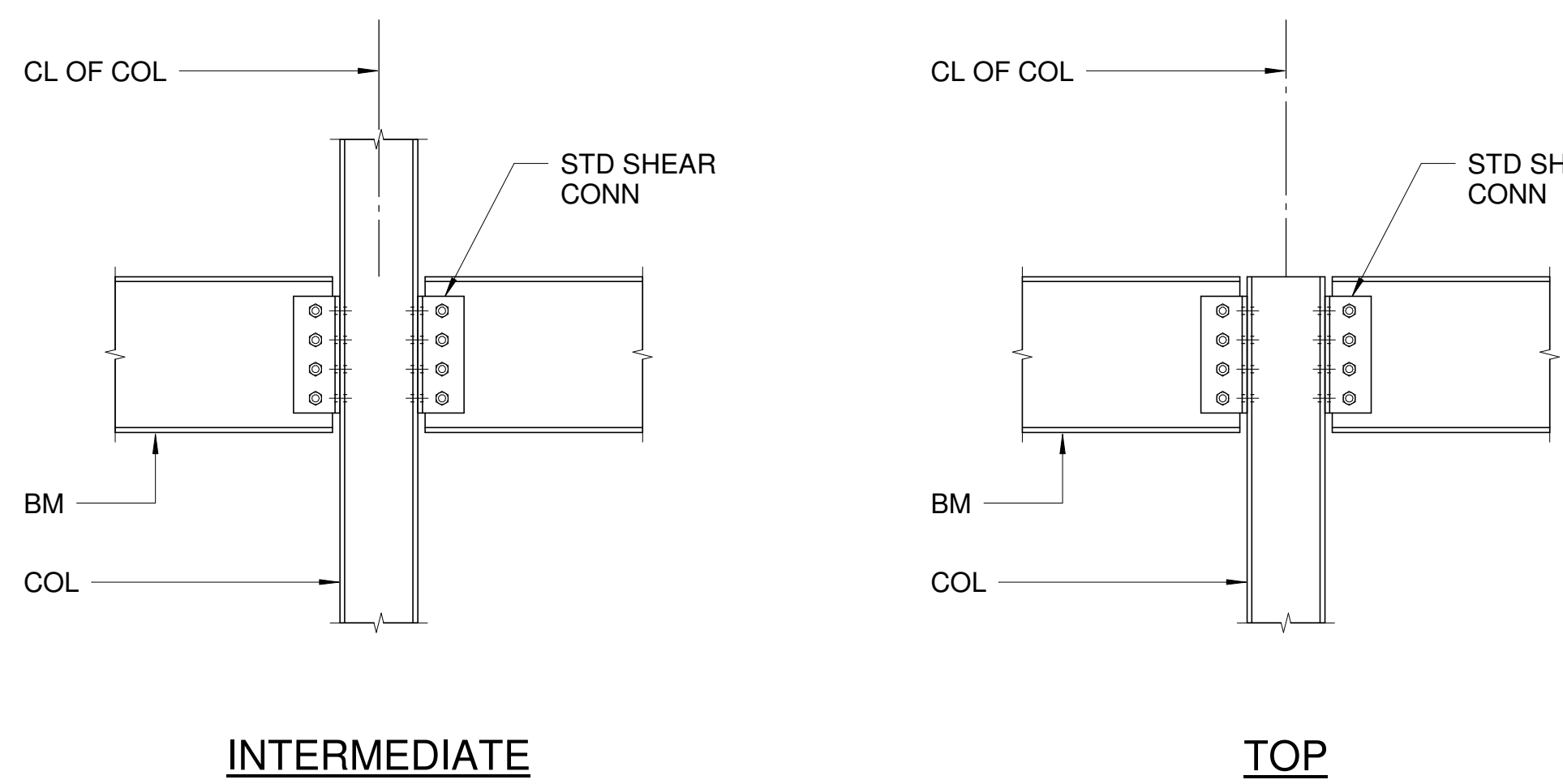
NTS

SECTION

TYPICAL ROOF TOP MECHANICAL UNIT SUPPORT DETAILS

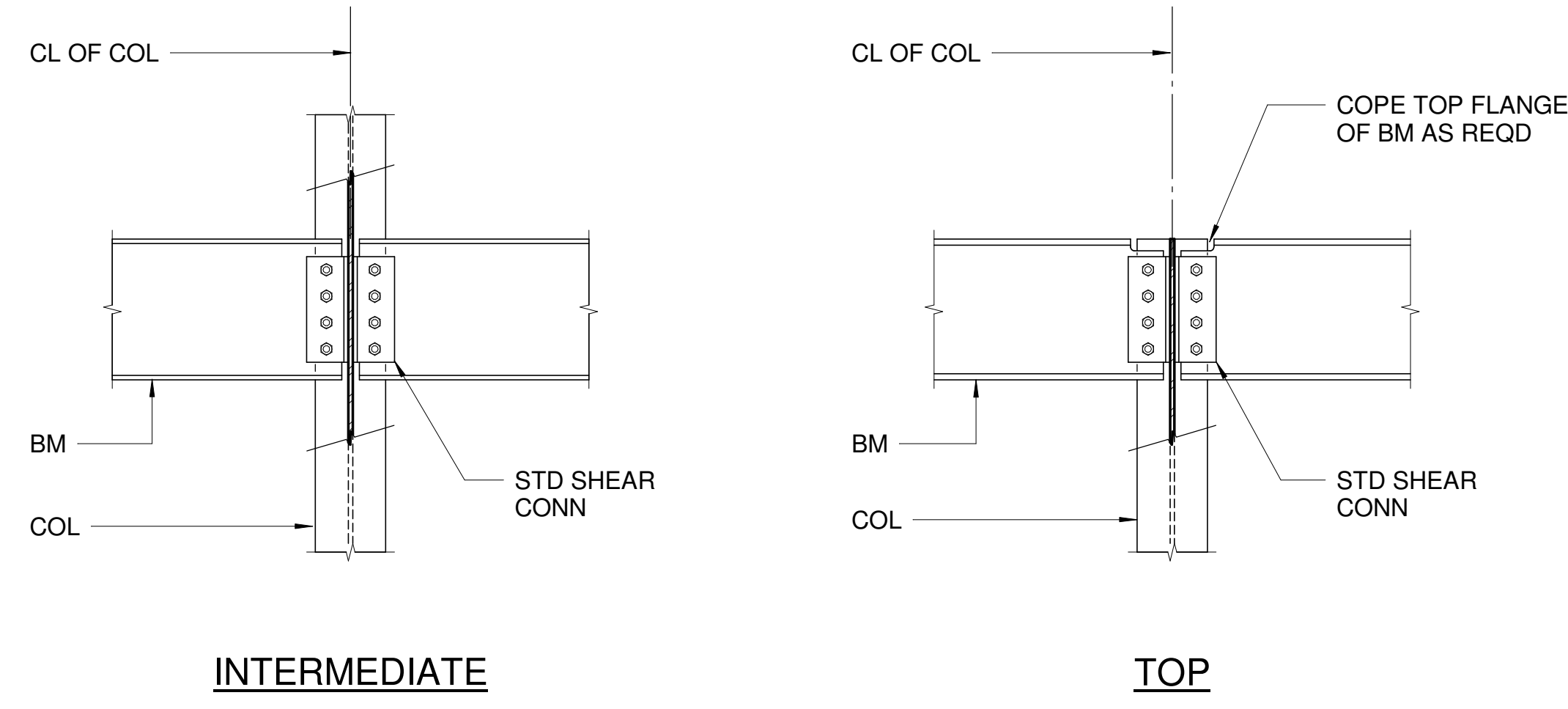
NTS

[illegible]



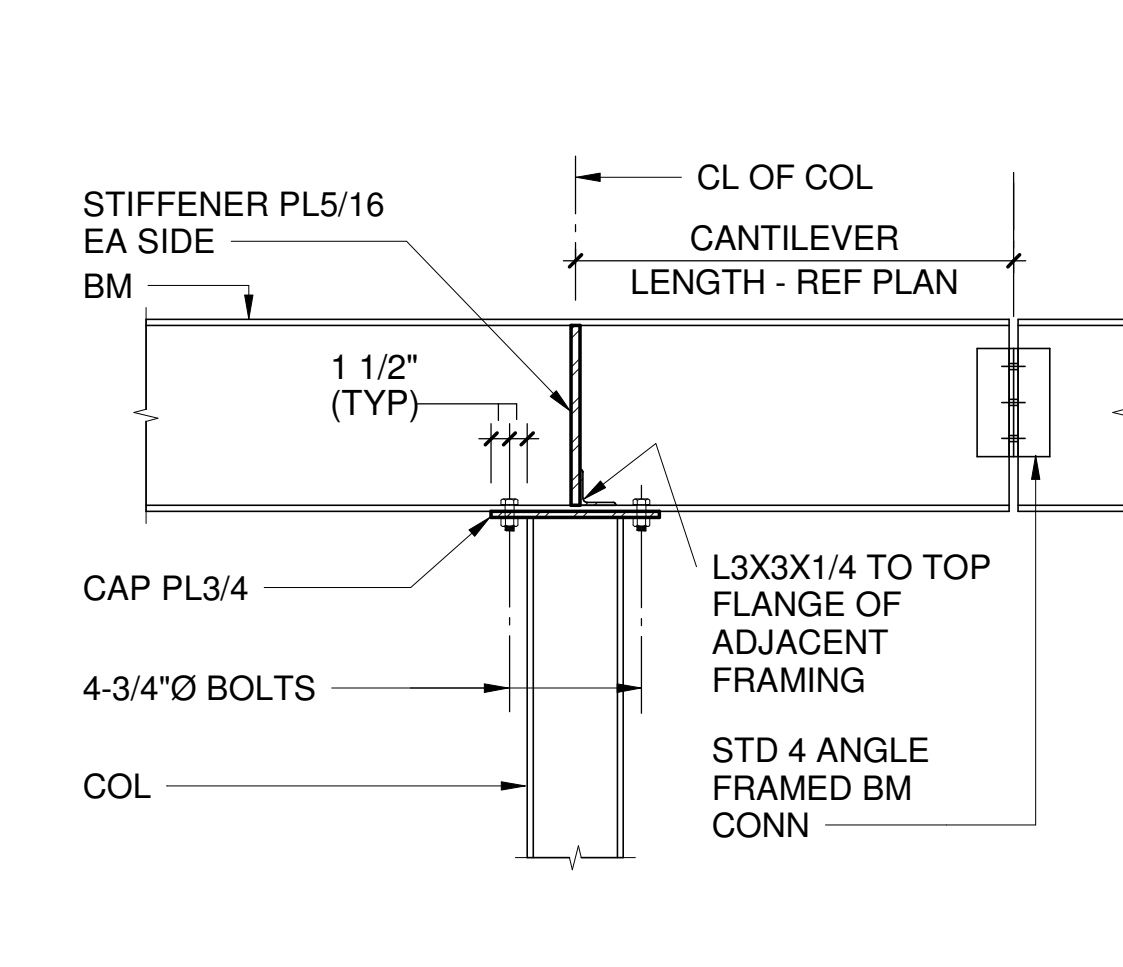
TYPICAL BEAM TO WIDE FLANGE COLUMN CONNECTION DETAILS

NTS



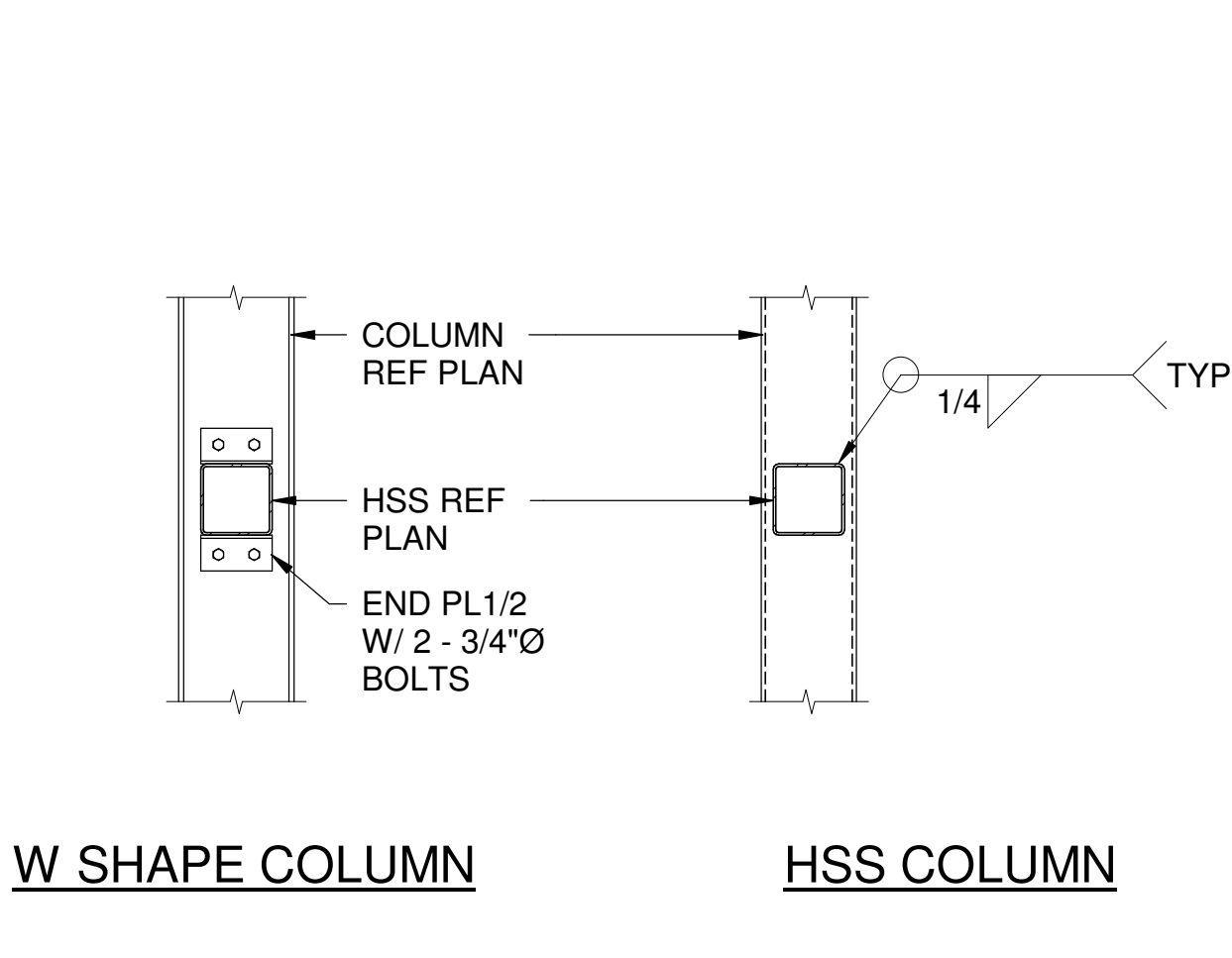
TYPICAL BEAM TO WIDE FLANGE COLUMN CONNECTION DETAILS

NTS



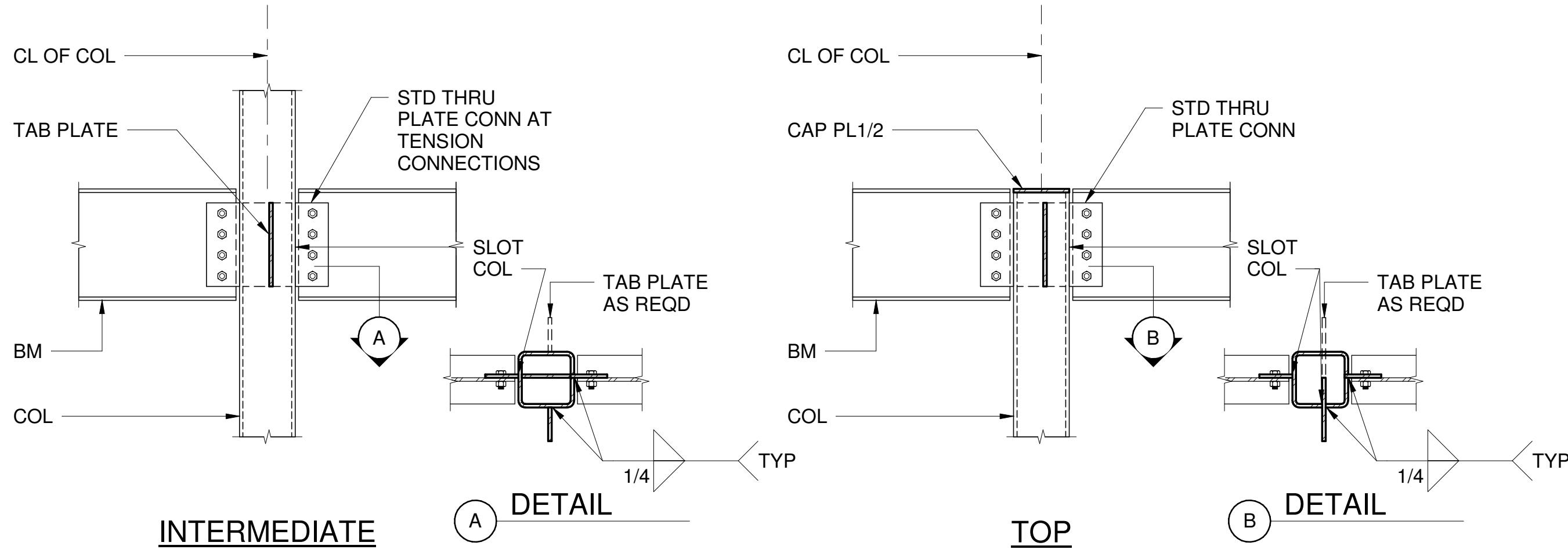
TYPICAL CANTILEVER BEAM DETAIL

NTS



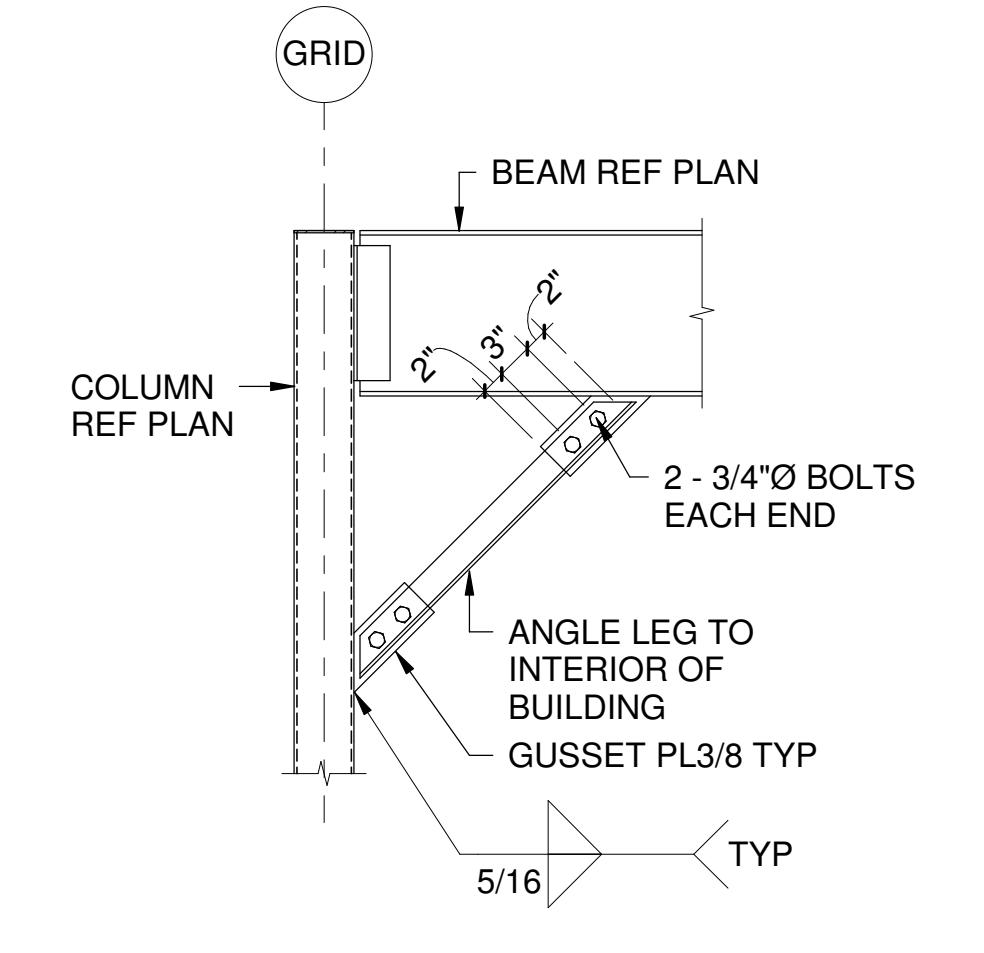
TYPICAL HSS TO COLUMN CONNECTION DETAILS

NTS



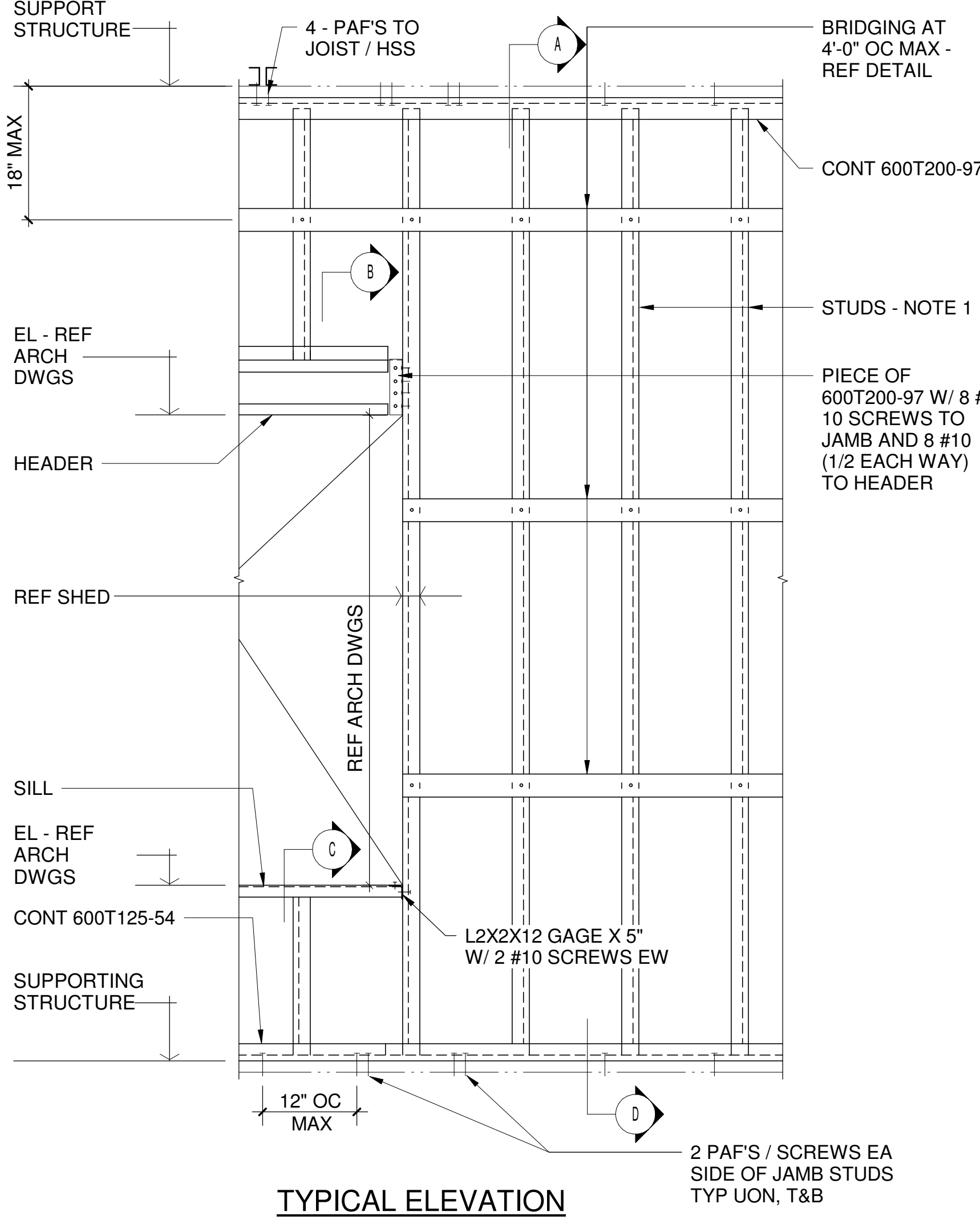
TYPICAL BEAM TO HSS COLUMN CONNECTION DETAILS

NTS



TYPICAL KNEE BRACE DETAIL

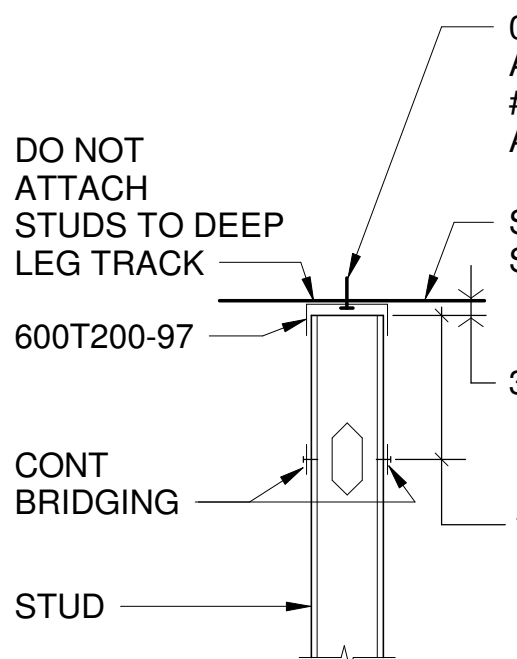
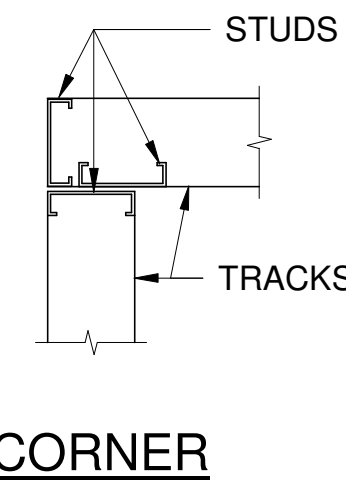
NTS



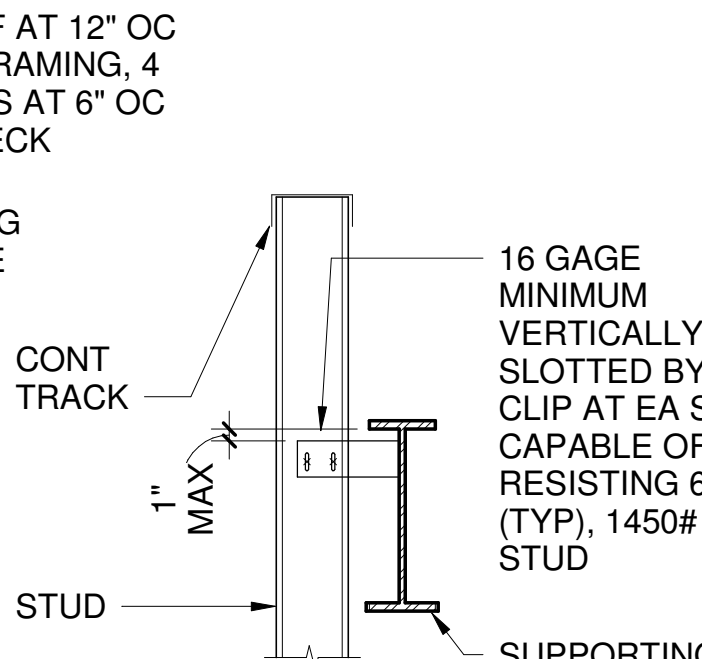
TYPICAL COLD FORMED METAL FRAMING DETAILS

NTS

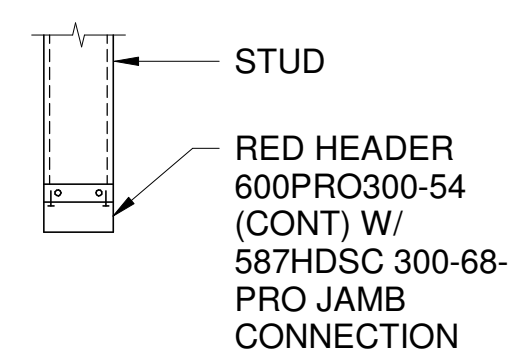
JAMB STUD SCHEDULE	
OPENING WIDTH	JAMB STUD
0 TO 4'-0"	1 - 600S200-54
TO 12'-0"	1 - 600S250-97



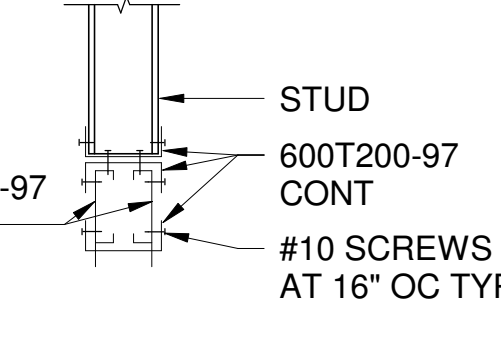
DETAIL



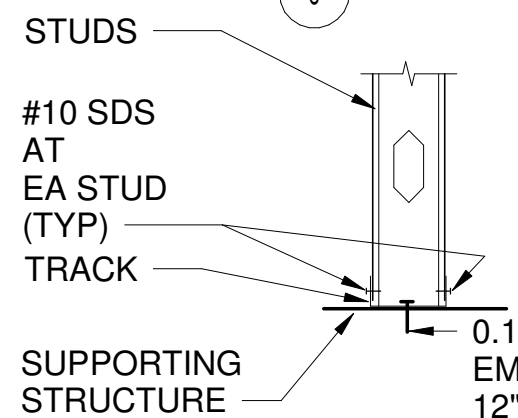
DETAIL



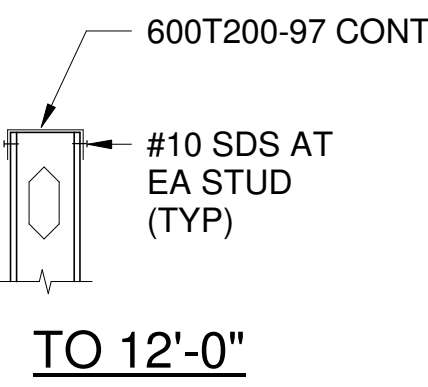
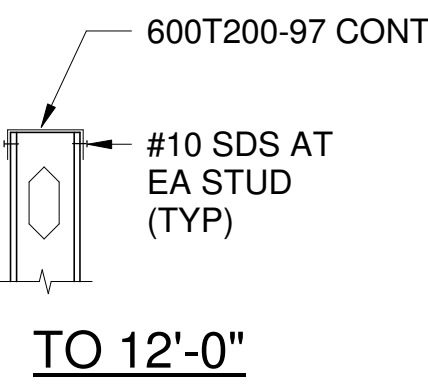
DETAIL



DETAIL



DETAIL

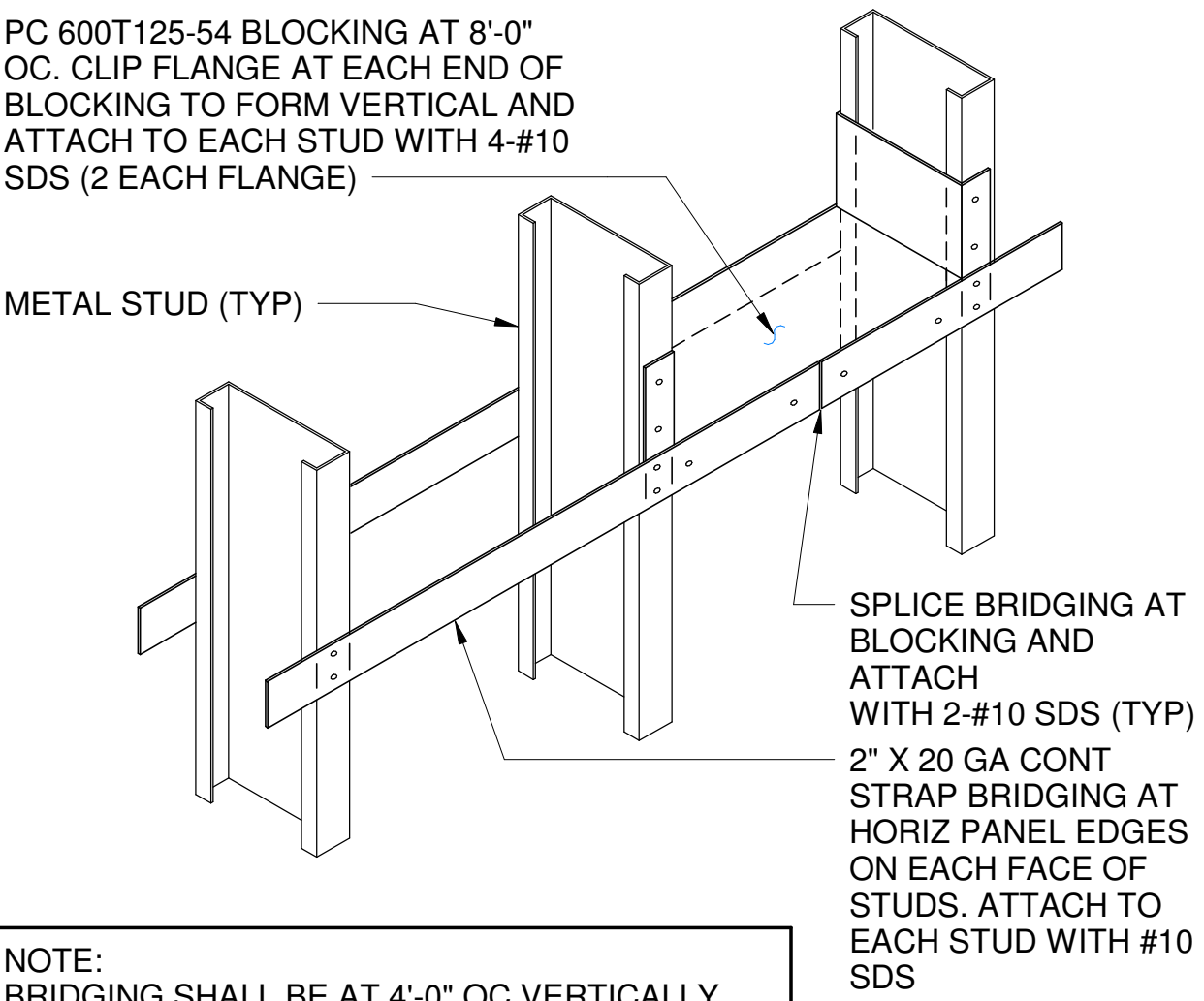


4'-1" TO 12'-0" OPENING JAMB STUDS

METAL STUD FRAMING NOTES:

- UNLESS OTHERWISE NOTED, ALL METAL STUDS SHALL BE 600S162-54 AT 16" OC EXCEPT 600S200-68 AT 16" OC AT LINES E, 10, AND B.
- PROVIDE INSULATION INDICATED ON THE ARCHITECTURAL DRAWINGS IN AREAS BETWEEN BUILT-UP MEMBERS INCLUDING HEADERS, SILLS, JAMB STUDS, ETC.
- STUD SIZES ARE STANDARD SIZES AS DEFINED BY THE STEEL STUD MANUFACTURER'S ASSOCIATION (SSMA).
- MINIMUM SCREW SPACING AND EDGE DISTANCE SHALL BE 3/4" IN ANY DIRECTION, TYPICAL.

GAGE KEY	
GAGE	MILS
18	43
16	54
14	68
12	97



REVISIONS	
DATE	DESCRIPTION
6/13/25	ISSUED FOR PERMIT