

STRUCTURAL SPECIFICATIONS (cont.), LOAD SCHEDULE, & WIND DESIGN DATA:

SUBMITTALS:

1. ALL SHOP DRAWINGS SHALL BE SUBMITTED AND APPROVED PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL PLAN AND ALLOW (10) WORKING DAYS FOR APPROVAL FROM FULLONE STRUCTURAL GROUP'S OFFICE. THIS SHALL BE FIGURED INTO THE CONTRACTORS SCHEDULE BEFORE CONSTRUCTION BEGINS.
2. SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC.
3. SHOP DRAWINGS WILL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. DRAWINGS SUBMITTED WITHOUT REVIEW WILL BE RETURNED UNCHECKED.
4. SHOP DRAWING SUBMITTALS SHALL INCLUDE FOUR SETS OF PRINTS. ONE SET OF PRINTS WILL BE RETAINED BY THE ENGINEER, AND THE CONTRACTOR SHALL MAINTAIN ONE SET FOR THE OWNER.
5. THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED IN WRITING BY THE ENGINEER.
6. CHANGES AND ADDITIONS MADE ON RE-SUBMITTALS SHALL BE CLEARLY FLAGGED AND NOTED. THE PURPOSE OF RE-SUBMITTALS SHALL BE CLEARLY NOTED ON THE LETTER OF TRANSMITTAL. ARCHITECT/ENGINEER REVIEW WILL BE LIMITED TO THOSE ITEMS CAUSING THE RE-SUBMITTAL.
7. THE FOLLOWING SHALL BE INCLUDED, AS A MINIMUM, FOR SHOP DRAWING AND SUBMITTAL REVIEW:
- 7.1 REINFORCING STEEL
- 7.2 CONCRETE MIX DESIGN
- 7.3 STRUCTURAL STEEL ERECTION DRAWINGS
- 7.4 METAL DECK
- 7.5 COLD FORMED STEEL (LIGHT GAUGE STEEL)
- 7.6 CONCRETE MASONRY AND MORTAR MIX
- 7.7 MISCELLANEOUS STEEL, STEEL EMBEDS, STEEL ANGLES, BOLTS, ETC.
- 7.8 CONCRETE CONSTRUCTION JOINT LOCATION FOR ALL STRUCTURAL CONCRETE ELEMENTS
- 7.9 CONTROL JOINT LOCATIONS (CMU CONSTRUCTION)
8. SHOP DRAWINGS SHALL SHOW ALL MEMBER SIZES, WELDS, BOLTS, CONNECTION DETAILS, LAYOUTS, BRIDGING, ACCESSORIES, EMBEDS, ETC., AS REQUIRED TO FABRICATE AND ERECT THE PROPOSED CONSTRUCTION IMPROVEMENTS.
9. THE GENERAL CONTRACTOR SHALL REVIEW AND APPROVE ALL SHOP DRAWINGS AND SUBMITTALS PRIOR TO SUBMITTING TO THE ENGINEER.
10. THE MANUFACTURER'S LITERATURE, INCLUDING A PROJECT CONTACT, AND OTHER GENERAL CONTACT INFORMATION, SHALL BE SUBMITTED FOR ALL MATERIALS AND PRODUCTS PROPOSED FOR CONSTRUCTION.
11. SHOP DRAWINGS, CALCULATIONS AND SUBMITTALS SHALL CLEARLY IDENTIFY THE SPECIFIC PROJECT AND APPLICABLE CODES, LIST DESIGN CRITERIA, AND SHOW ALL DETAILS AND PLANS NECESSARY FOR PROPER FABRICATION AND INSTALLATION. CALCULATIONS AND DRAWINGS SHALL IDENTIFY SPECIFIC PRODUCT UTILIZED. GENERIC PRODUCTS WILL NOT BE ACCEPTED.
12. SHOP DRAWINGS AND CALCULATIONS REQUIRE THE IMPRESSED SEAL, DATE AND SIGNATURE OF THE DELEGATED ENGINEER. COMPUTER PRINTOUTS ARE AN ACCEPTABLE SUBSTITUTE FOR MANUAL COMPUTATIONS PROVIDED THEY ARE ACCOMPANIED BY SUFFICIENT DESCRIPTIVE INFORMATION TO PERMIT THEIR PROPER EVALUATION. SUCH DESCRIPTIVE INFORMATION SHALL BEAR THE IMPRESSED SEAL AND SIGNATURE OF THE DELEGATED ENGINEER AS AN INDICATION THAT HE/SHE HAS ACCEPTED RESPONSIBILITY FOR THE RESULTS. THE STRUCTURAL ENGINEER WILL RETAIN ONE SIGNED AND SEALED PRINT FOR RECORD.
13. DRAWINGS PREPARED TO SERVE AS A GUIDE FOR FABRICATION AND INSTALLATION (SUCH AS REINFORCING STEEL SHOP DRAWINGS OR STRUCTURAL STEEL ERECTION DRAWINGS) AND REQUIRING NO ENGINEERING DO NOT REQUIRE THE SEAL OF A DELEGATED ENGINEER.
14. CATALOG INFORMATION ON STANDARD PRODUCTS DOES NOT REQUIRE THE SEAL OF A DELEGATED ENGINEER.
15. REVIEW BY THE STRUCTURAL ENGINEER OF RECORD OF SUBMITTALS IS LIMITED TO VERIFY THE FOLLOWING:
- 15.1 THAT THE SPECIFIED STRUCTURAL SUBMITTALS HAVE BEEN FURNISHED.
- 15.2 THAT THE STRUCTURAL SUBMITTALS HAVE BEEN SIGNED AND SEALED BY THE DELEGATED ENGINEER.
- 15.3 THAT THE DELEGATED ENGINEER HAS UNDERSTOOD THE DESIGN INTENT AND HAS USED THE SPECIFIC STRUCTURAL CRITERIA. (NO DETAILED CHECK OF CALCULATIONS WILL BE MADE.)
- 15.4 THAT THE CONFIGURATION SET FORTH IN THE STRUCTURAL SUBMITTALS IS CONSISTENT WITH THE CONTRACT DOCUMENTS. (NO DETAILED CHECK OF DIMENSIONS OR QUANTITIES WILL BE MADE.)
16. SUBMITTALS NOT MEETING THE ABOVE CRITERIA WILL BE REJECTED.

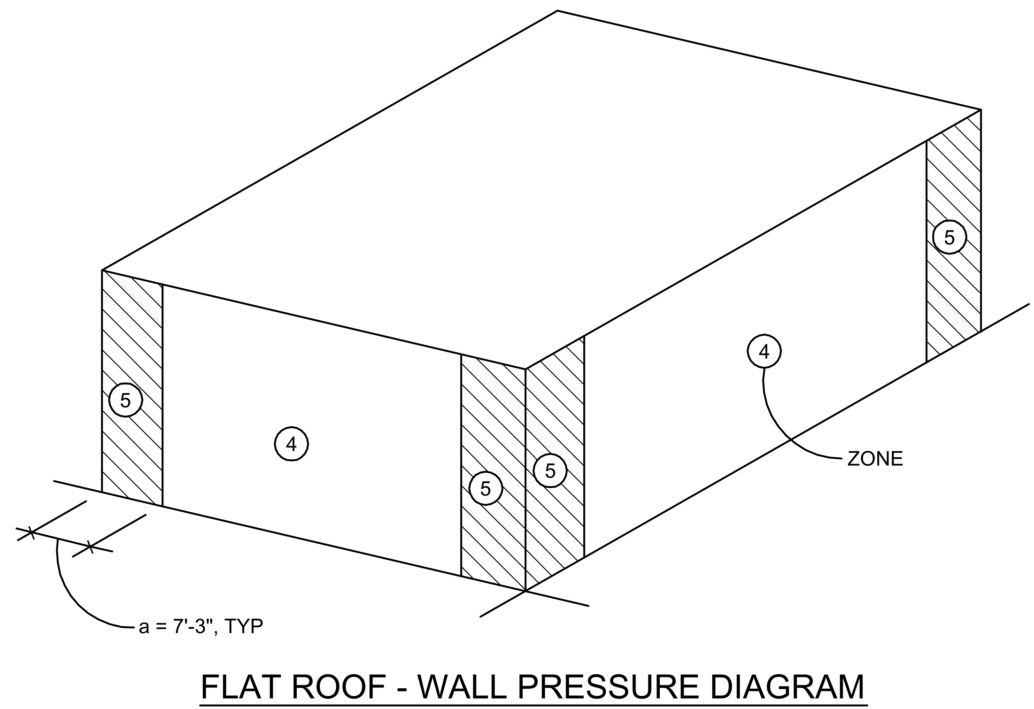
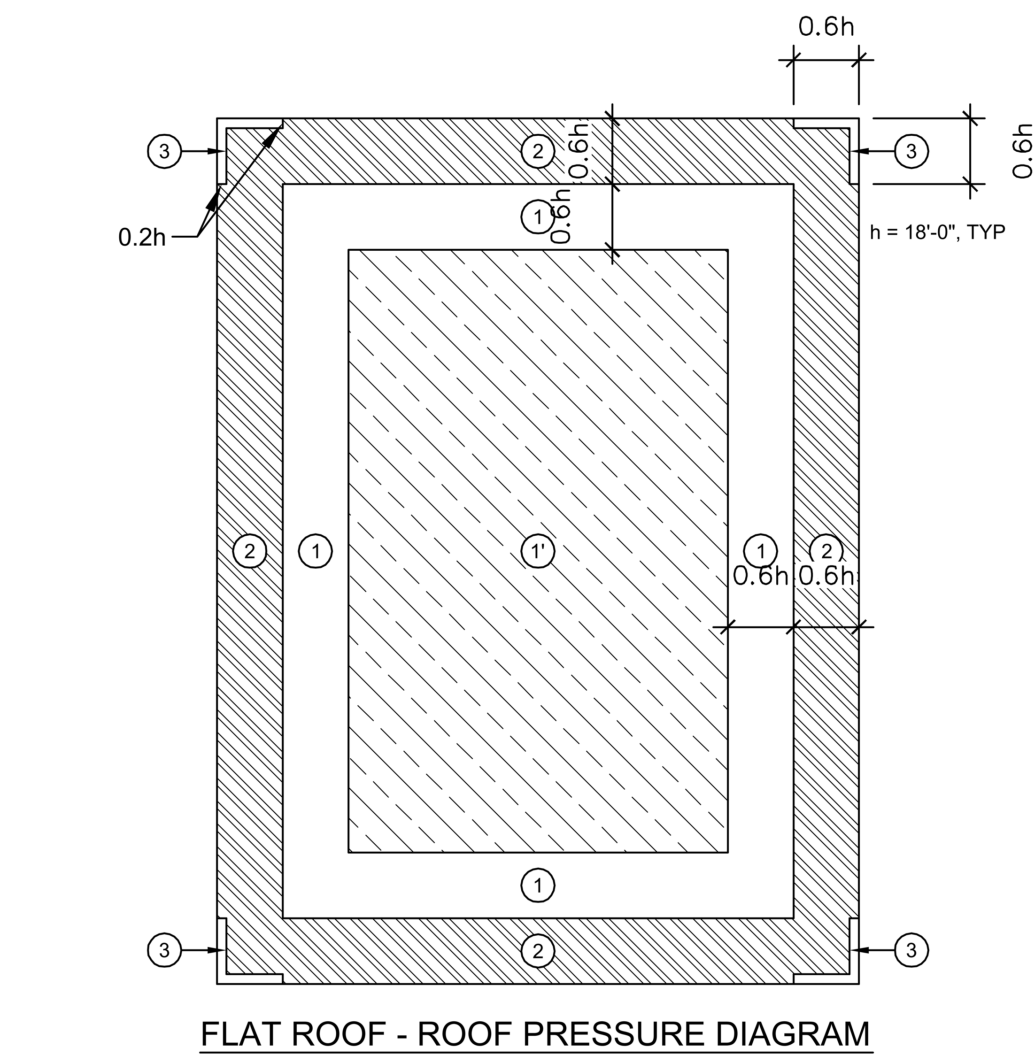
DELEGATED ENGINEER (SPECIALTY ENGINEER) RESPONSIBILITY:

1. THE CONTRACTOR SHALL REVIEW AND UNDERSTAND THE STRUCTURAL COMPONENTS AND/OR ASSEMBLIES THAT REQUIRE THE SERVICES OF A DELEGATED ENGINEER (SPECIALTY ENGINEER), PER RULE 61G15-30 OF THE FLORIDA BOARD OF PROFESSIONAL ENGINEERS.
2. THE FOLLOWING STRUCTURAL ELEMENTS THAT SHALL REQUIRE THE SERVICES OF A SPECIALTY ENGINEER INCLUDE, BUT NOT LIMITED TO, THE FOLLOWING:
- 2.1 HANDRAILS/GUARDRAILS, THEIR ASSEMBLIES, AND THEIR CONNECTIONS TO THE SUPPORT STRUCTURE. THESE ELEMENTS SHALL BE DESIGNED BY THE FABRICATOR'S SPECIALTY ENGINEER.
- 2.2 OPEN WEB STEEL JOISTS AND JOIST GIRDERS SHALL BE DESIGNED BY THE JOIST MANUFACTURER AND SHALL MEET THE REQUIREMENTS OF THE STEEL JOIST INSTITUTE.
- 2.3 HVAC ROOF TOP EQUIPMENT TO BE ENGINEERED FOR FLORIDA BUILDING CODE REQUIRED WIND LOADS. SPECIALTY ENGINEER IS TO PROVIDE SIGNED AND SEALED SHOP DRAWINGS AND CALCULATIONS FOR CONNECTION OF UNITS TO CURB/RACK AND CONNECTION OF CURB/RACK TO STEEL SUPPORT MEMBERS (ANGLES/CHANNELS) PROVIDED ON THESE DRAWINGS.
- 2.4 EXTERIOR CANOPIES AND WINDOW AWINGS. DESIGN SHALL INCLUDE CONNECTIONS TO THE BUILDING STRUCTURE. COORDINATE WITH STRUCTURAL ENGINEER OF RECORD FOR ANY MODIFICATIONS THAT MAY BE REQUIRED TO THE BUILDING STRUCTURE TO SATISFY THE CONNECTION, I.E. BREAK AWAY FROM STRUCTURE.
- 2.5 DESIGN OF STRUCTURAL STEEL SYSTEMS - DESIGN OF STRUCTURAL STEEL CONNECTIONS BASED UPON THE DESIGN CRITERIA OF THE ENGINEER OF RECORD. IT IS THE RESPONSIBILITY OF THE STEEL FABRICATOR TO IMPLEMENT THE DESIGN AS SPECIFIED AND FOR MAINTAINING FABRICATION AND ERECTION TOLERANCES, AS WELL AS, FOR ENSURING THE FIT AND ERECTABILITY OF THE STRUCTURE.
- 2.6 DESIGN OF LIGHT-GAGE METAL STUD FRAMING - DESIGN OF LIGHT-GAGE METAL STUD FRAMING MEMBERS AND CONNECTION TO STRUCTURE BASED UPON THE DESIGN CRITERIA OF THE ENGINEER OF RECORD. PROVIDE SIGNED AND SEALED DRAWINGS AND CALCULATIONS PREPARED BY A LICENSED PROFESSIONAL ENGINEER IN THE STATE OF FLORIDA. SEE "COLD-FORM STEEL FRAMING" NOTES THIS SHEET FOR ADDITIONAL INFORMATION AND REQUIREMENTS.
- 2.7 EXTERIOR CHILLER SCREEN WALL. DESIGN OF STRUCTURE AND CONNECTIONS TO CONCRETE THICKENED SLAB EDGE / FOUNDATION SHALL COORDINATE WITH STRUCTURAL ENGINEER OF RECORD FOR ANY MODIFICATIONS THAT MAY BE REQUIRED TO THE SLAB / FOUNDATIONS TO SATISFY THE CONNECTION.
3. THE ABOVE SPECIALTY STRUCTURAL ELEMENTS SHALL BE SIGNED AND SEALED BY A FLORIDA LICENSED ENGINEER. THE ENGINEERED ELEMENTS AND SYSTEMS SHALL INCLUDE SHOP DRAWINGS AND SIGNED AND SEALED PLANS AND STRUCTURAL CALCULATIONS SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL.

LOAD SCHEDULE (FLAT ROOF):

ROOF:	DEAD LOAD (FLAT ROOF)	
	ROOFING & INSULATION	= 7 PSF
	METAL DECK	= 3 PSF
	STEEL FRAMING	= 5 PSF
	MECH. / ELEC. / PLUMBING	= 6 PSF
	FIRE SPRINKLER *	= 5 PSF
	MISCELLANEOUS	= 4 PSF
	TOTAL ROOF DEAD LOAD	= 30 PSF
ROOF:	LIVE LOAD (FLAT ROOF)	= 20 PSF

* SPRINKLER LOADING IS DEFINED AS DEAD LOAD ONLY. SYSTEM IS CONSIDERED FULLY PRESSURIZED AND THEREFORE HAS A MAXIMUM POSSIBLE LOADING.



WIND DESIGN DATA:

- CODE USED:
- 8th EDITION (2023) FLORIDA BUILDING CODE
 - EXPOSURE CLASSIFICATION: "B"
 - BUILDING RISK CATEGORY: "II"
 - ULTIMATE WIND SPEED (Vult): 137 MPH
 - DESIGN WIND SPEED (Vasd): 106.2 MPH
 - INTERNAL PRESSURE COEFFICIENT: ±0.18 - FULLY ENCLOSED

FLAT ROOF - ROOF PRESSURE DIAGRAM

GROSS UPLIFT SCHEDULE COMPONENTS AND CLADDING				
ZONE	EFFECTIVE WIND AREA			
	10 SF	20 SF	50 SF	100 SF
1	+14 / -31 PSF	+12 / -31 PSF	+12 / -31 PSF	+11 / -31 PSF
1	+14 / -54 PSF	+12 / -51 PSF	+12 / -45 PSF	+11 / -42 PSF
2	+14 / -71 PSF	+12 / -65 PSF	+12 / -59 PSF	+11 / -55 PSF
3	+14 / -97 PSF	+12 / -88 PSF	+12 / -77 PSF	+11 / -65 PSF

FLAT ROOF - ROOF OVERHANG PRESSURE DIAGRAM

GROSS UPLIFT SCHEDULE COMPONENTS AND CLADDING				
ZONE	EFFECTIVE WIND AREA			
	10 SF	20 SF	50 SF	100 SF
1	-54 PSF	-52 PSF	-52 PSF	-51 PSF
2	-71 PSF	-65 PSF	-57 PSF	-51 PSF
3	-97 PSF	-85 PSF	-71 PSF	-62 PSF

FLAT ROOF - WALL PRESSURE DIAGRAM

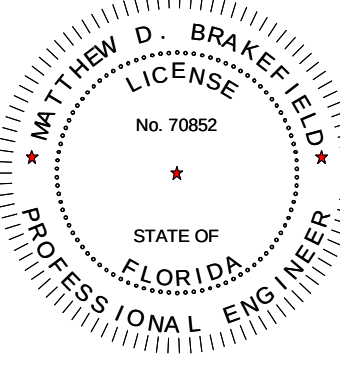
DESIGN COMPONENTS AND CLADDING WIND PRESSURE FOR WALLS, DOORS, & WINDOWS				
ZONE	EFFECTIVE WIND AREA			
	10 SF	20 SF	50 SF	100 SF
4	+34 / -37 PSF	+32 / -35 PSF	+29 / -34 PSF	+29 / -32 PSF
5	+34 / -45 PSF	+32 / -42 PSF	+29 / -38 PSF	+29 / -35 PSF

NOTE: ALL EXTERIOR DOOR & WINDOW ASSEMBLIES SHALL SATISFY THE REQUIREMENTS IN THE 8th EDITION (2023) FLORIDA BUILDING CODE, WITH ANY AND ALL REVISIONS. ALL CONNECTIONS TO THE BUILDING STRUCTURE SHALL HAVE THE CAPACITY TO WITHSTAND THE PRESSURES INDICATED IN THIS SCHEDULE.

NOTE: THIS TABLE IS BASED ON THE ULTIMATE WIND SPEED (Vult) FOUND IN THE WIND DESIGN DATA TABLE.

SEAL

This item has been digitally signed and sealed by Matthew D. Brakefield, PE, on the date 05/19/2026. The digital signature can be verified on any electronic copies.



Surgery Center at Millenia, LLC

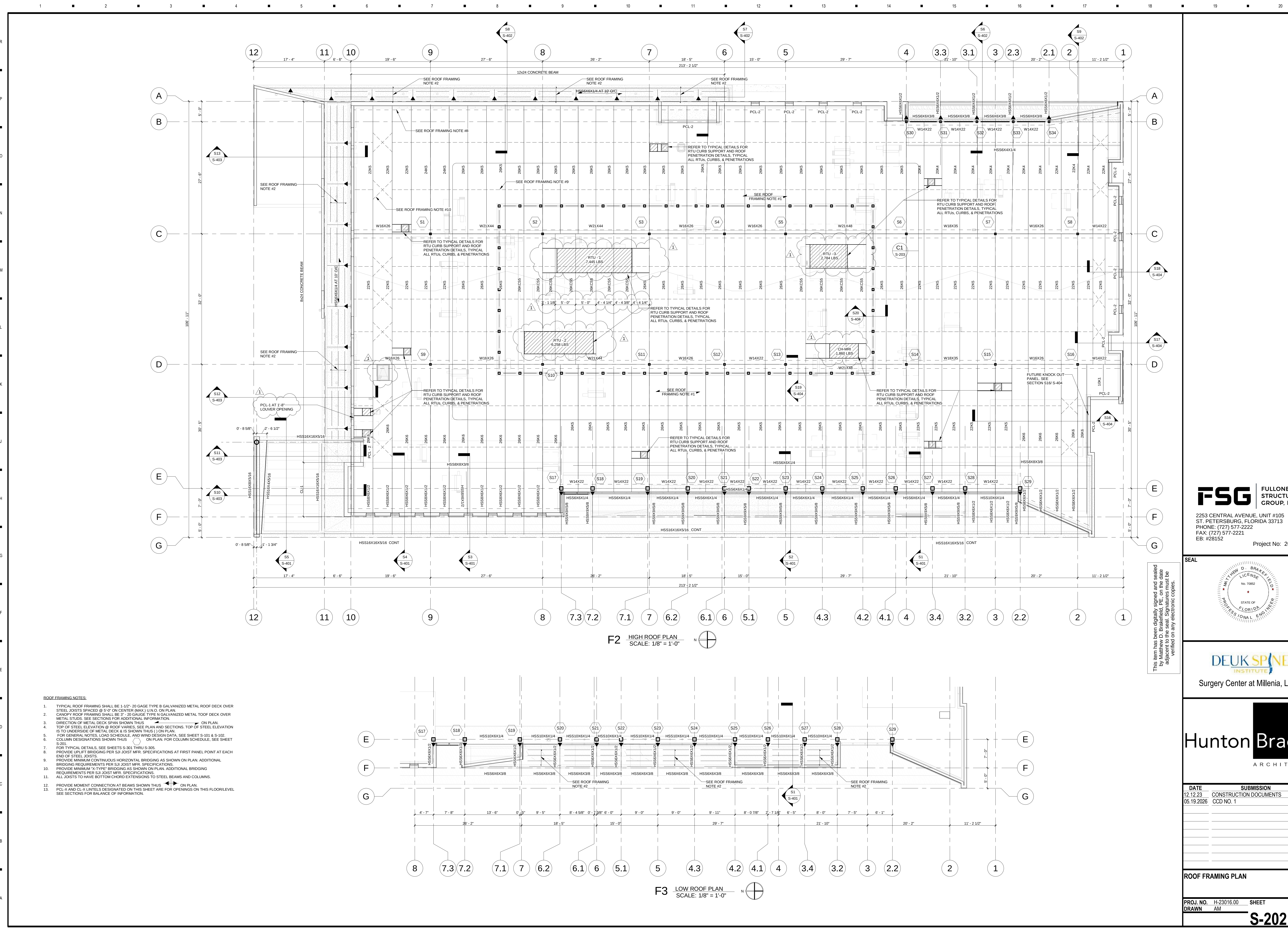


DATE	SUBMISSION	NO
12.12.23	CONSTRUCTION DOCUMENTS	
05.19.2026	CCD NO. 1	1

LOAD & WIND DATA

PROJ. NO. H-23016.00 SHEET
DRAWN AM

S-102



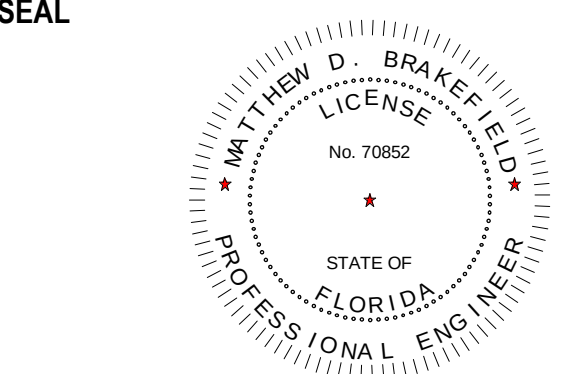
- ROOF FRAMING NOTES:**
1. TYPICAL ROOF FRAMING SHALL BE 1-1/2" x 20 GAUGE TYPE B GALVANIZED METAL ROOF DECK OVER STEEL JOISTS SPACED @ 5'-0" ON CENTER (MAX.) UNO, ON PLAN.
 2. CANOPY ROOF FRAMING SHALL BE 5" x 20 GAUGE TYPE B GALVANIZED METAL ROOF DECK OVER METAL STUDS. SEE SECTIONS FOR ADDITIONAL INFORMATION.
 3. DIRECTION OF METAL DECK SPAN SHOWN THUS ON PLAN.
 4. TOP OF STEEL ELEVATION @ ROOF VARIES. SEE PLAN AND SECTIONS. TOP OF STEEL ELEVATION IS TO UNDERSIDE OF METAL DECK & IS SHOWN THUS () ON PLAN.
 5. FOR GENERAL NOTES, LOAD SCHEDULE, AND WIND DESIGN DATA, SEE SHEET S-101 & S-102. COLUMN DESIGNATIONS SHOWN THUS ON PLAN. FOR COLUMN SCHEDULE, SEE SHEET S-301.
 6. FOR TYPICAL DETAILS, SEE SHEETS S-301 THRU S-305.
 7. PROVIDE UP/LIFT BRIDGING PER SJI JOIST MFR. SPECIFICATIONS AT FIRST PANEL POINT AT EACH END OF STEEL JOISTS.
 8. PROVIDE MINIMUM CONTINUOUS HORIZONTAL BRIDGING AS SHOWN ON PLAN. ADDITIONAL BRIDGING REQUIREMENTS PER SJI JOIST MFR. SPECIFICATIONS.
 9. PROVIDE MINIMUM "X" TYPE BRIDGING AS SHOWN ON PLAN. ADDITIONAL BRIDGING REQUIREMENTS PER SJI JOIST MFR. SPECIFICATIONS.
 10. ALL JOISTS TO HAVE BOTTOM CHORD EXTENSIONS TO STEEL BEAMS AND COLUMNS.
 11. PROVIDE MOMENT CONNECTION AT BEAMS SHOWN THUS ON PLAN.
 12. PCL-X AND CL-X UNITS DESIGNATED ON THIS SHEET ARE FOR OPENINGS ON THIS FLOOR LEVEL. SEE SECTIONS FOR BALANCE OF INFORMATION.
 13. SEE SECTIONS FOR BALANCE OF INFORMATION.

This item has been digitally signed and sealed by Matthew D. Brakenfield, P.E. on the date 05/19/2026. The digital signature can be verified on any electronic copies.

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ST. PETERSBURG, FLORIDA 33713
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Project No: 2035-001



DEUK SPINE INSTITUTE

Surgery Center at Millenia, LLC

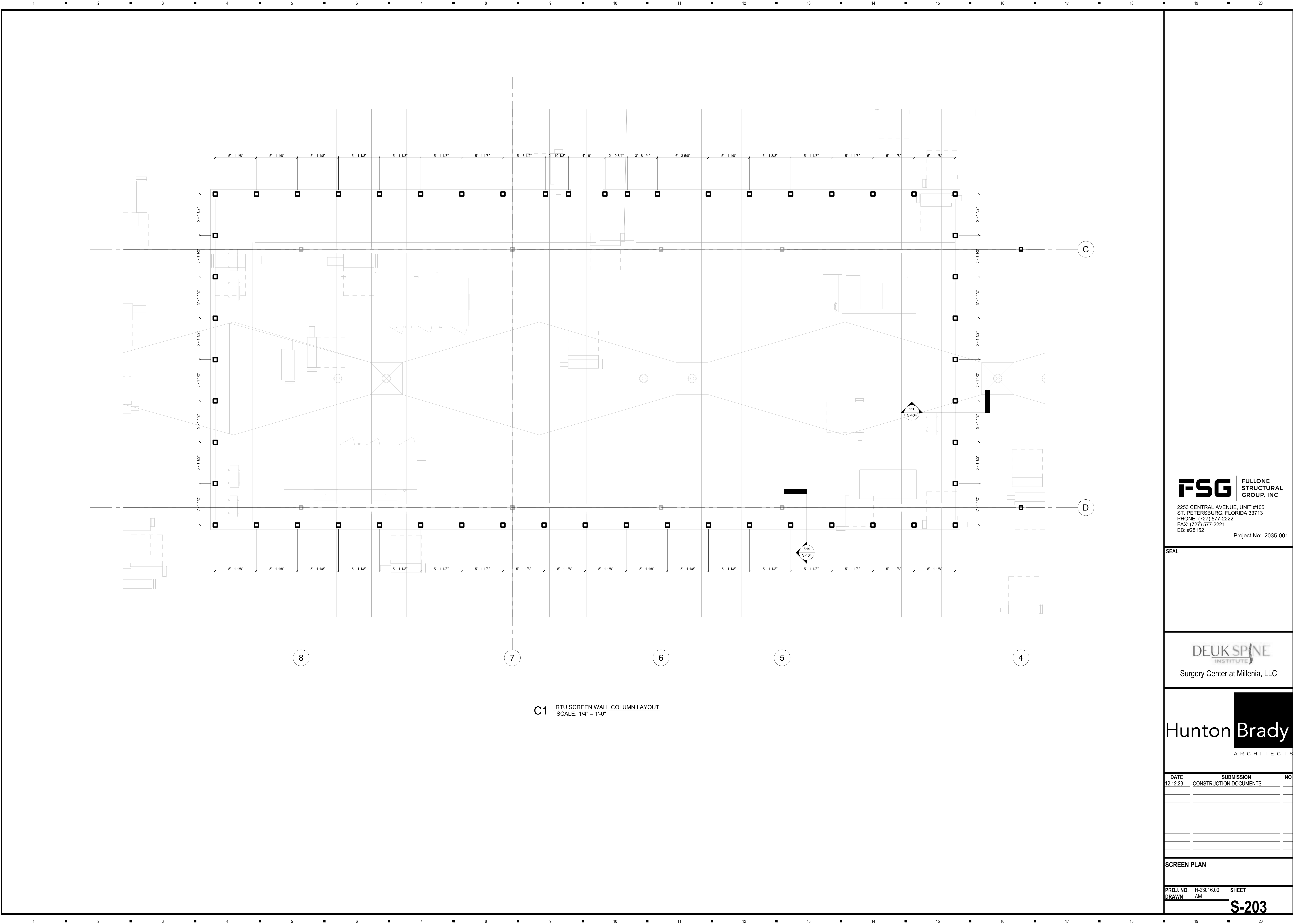
Hunton Brady

ARCHITECTS

DATE	SUBMISSION	NO
12.12.23	CONSTRUCTION DOCUMENTS	
05.19.2026	CCD NO. 1	1

ROOF FRAMING PLAN

PROJ. NO. H-23016.00 SHEET
DRAWN AM
S-202



C1 RTU SCREEN WALL COLUMN LAYOUT
SCALE: 1/4" = 1'-0"

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Project No: 2035-001

SEAL

DEUK SPINE
INSTITUTE

Surgery Center at Millenia, LLC

Hunton Brady
ARCHITECTS

DATE	SUBMISSION	NO
12.12.23	CONSTRUCTION DOCUMENTS	

SCREEN PLAN

PROJ. NO. H-23016.00 SHEET
DRAWN AM

S-203

NOTES:

1. CONCRETE FOR SLAB ON GRADE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AS INDICATED ON THE "CONCRETE AND REINFORCING" NOTES.

2. SLAB ON GRADE CONCRETE MIX SHALL HAVE A WATER-CEMENT RATIO AS INDICATED IN SPECS AND/OR NOTES.

3. CONSTRUCTION JOINTS SHALL BE LOCATED A MINIMUM OF 5'-0" AWAY FROM ANY OTHER JOINTS TO WHICH THEY ARE PARALLEL.

4. SAW CUTTING SHALL BE DONE WITH A POWER SAW WITH A MASONRY CUTTING BLADE. CUTTING SHALL BE DONE AS SOON AS CONCRETE HARDENS ENOUGH SO THAT THE BLADE DOES NOT DISCLOSE THE AGGREGATES.

5. WHERE SAW-CUT IS DISCONTINUED AT A TRANSVERSE JOINT, STOP CUT 2" SHORT.

6. SLAB REINFORCING NOT SHOWN FOR CLARITY.

SAW JOINT TO BE FILLED WITH FIELD MOLDED FILLER. SEE STRUCTURAL SPECIFICATION SHEET.

1/4" MAX.

T/4 MIN.

VAPOR BARRIER

COMPACTED FILL

SAWED CONTROL JOINT DETAIL

NOT TO SCALE

EDGE BEFORE REMOVING BULKHEAD TO MATCH CONSTRUCTION JOINT

T/10 (0.75" MIN.)

T/4 (1.5" MIN.)

2" X 4" LUMBER BULKHEAD

METAL KEY

VAPOR BARRIER

COMPACTED FILL

APPLY CURING COMPOUND TO SIDES OF EARLIER POUR TO BREAK BOND OF LATER POUR.

BULKHEAD CONTROL JOINT DETAIL

NOT TO SCALE

15'-0" MAX. (U.N.O.)

15'-0" MAX. (U.N.O.)

SAWED CONTROL JOINT PATTERN

SLAB ON GRADE SAW-CUT (S.J.) LAYOUT

NOT TO SCALE

CONSTRUCTION OR CONTROL JOINT

COLUMN ISOLATION JOINT

FOUNDATION NOTES:

0'-0" REFERENCE ELEVATION FOR ACTUAL ELEVATION. SEE CIVIL DRAWINGS.

CONCRETE SLAB JOINT DETAIL

NOT TO SCALE

NOTE:

IN CASE OF CONFLICT BETWEEN INFORMATION SHOWN ON THIS DRAWING, SECTION DRAWINGS, OR ARCHITECTURAL DRAWINGS, IT IS THE CONTRACTOR'S RESPONSIBILITY TO INFORM THE ARCHITECT AND ENGINEER OF SUCH CONFLICT IN ORDER TO RECEIVE A CLARIFICATION BEFORE PROCEEDING TO WORK. DIMENSIONS, SIZES, AND REINFORCEMENT OF STRUCTURAL ELEMENTS SHOWN IN THIS DRAWING ARE MINIMUM TO BE USED UNLESS NOTED OTHERWISE (U.N.O.) IN PLANS.

NOTES:

1. SEE FOUNDATION NOTES FOR SLAB THICKNESS AND REINFORCING.

2. SLABS SHALL BEAR ON COMPACTED FILL.

3. COMPACTED FILL SHALL BE COVERED WITH VAPOR BARRIER. SEE SPECS.

4. UNDER MACHINE EQUIPMENT DEEPEN SLAB TO 8" AND ADD TWO LAYERS OF WELDED WIRE FABRIC TOP AND BOTTOM, UNLESS NOTED OTHERWISE.

5. FOR DEPRESSIONS, SEE ARCHITECTURAL DRAWINGS.

6. AT CONSTRUCTION JOINTS USE WOOD FORMS WITH SHEAR KEYS.

7. STOP WIRE MESH AT CONSTRUCTION JOINTS. CUT EVERY OTHER WIRE AT SAW-CUT JOINT.

8. WHERE SLAB IS DOWELED INTO WALL, FIRST SLAB CONSTRUCTION JOINT TO BE NOT MORE THAN 15 FEET FROM WALL.

9. FOR EXPANSION JOINT LOCATION SEE PLAN. FOR DETAILS SEE ARCHITECTURAL DRAWINGS.

STAIR STRINGER

W.W.F.

1" MIN. 2" MAX.

12"

8"

8"

1

NOTE 1

NOTE 2

NOTE 3

(2) #5 CONTINUOUS

AT STAIR LANDING

NOT TO SCALE

8" OR LARGER MASONRY NON-BEARING PARTITION

1" MIN. 2" MAX.

W.W.F.

NOTE 1

NOTE 2

NOTE 3

(2) #5 CONTINUOUS

8"

8"

1

HAUNCH AT PARTITION (NO JOINT)

NOT TO SCALE

1" MIN. 2" MAX.

W.W.F.

NOTE 1

NOTE 2

NOTE 3

NOTE 5

2'-0" DIM OR T DIM MIN.

AT DEPRESSION

NOT TO SCALE

1" MIN. 2" MAX.

W.W.F.

NOTE 1

NOTE 2

NOTE 3

(1) #5 CONTINUOUS

(2) #5 CONTINUOUS

12"

1'-4" MIN.

EXTERIOR PLATFORM ADJACENT TO FINISHED GRADE

NOT TO SCALE

1" MIN. 2" MAX.

W.W.F.

NOTE 1

NOTE 2

NOTE 3

(1) #5 CONTINUOUS

(2) #5 CONTINUOUS

12"

1'-4" MIN.

CONCRETE PAVEMENT

SEE SITE DRAWINGS

EXTERIOR PLATFORM ADJACENT TO CONCRETE PAVEMENT

NOT TO SCALE

NOTES:

1. FOOTING, COLUMN, AND SLAB REINFORCEMENT NOT SHOWN FOR CLARITY. SEE PLAN AND SCHEDULE.

STEEL COLUMN

NOTES 1 AND 2

BASE PLATE

NOTE 2

CONCRETE SLAB

NOTE 2

1/2" NON-SHRINK GROUT

JOINT FILLER TO EXTEND FULL DEPTH OF JOINT

1/4" Ø VENT HOLE FOR GROUT, TYP.

ANCHOR BOLTS

NOTE 2

(1) #3 TOP (TYP 4 SIDES)

1/2" PRE-MOLDED JOINT FILLER AND SEALANT

KEYED CONSTRUCTION JOINT (TYP)

NO KEY AT COLUMN POCKET (TYP)

PIER OR FOOTING BELOW

NOTE 1

TYPICAL COLUMN ISOLATION JOINT DETAIL

NOT TO SCALE

CONCRETE COLUMN

NOTES 1 AND 2

CONCRETE SLAB

NOTE 2

1/2" NON-SHRINK GROUT

JOINT FILLER TO EXTEND FULL DEPTH OF JOINT

(1) #3 TOP (TYP 4 SIDES)

1/2" PRE-MOLDED JOINT FILLER AND SEALANT

KEYED CONSTRUCTION JOINT (TYP)

NO KEY AT COLUMN POCKET (TYP)

PIER OR FOOTING BELOW

NOTE 1

TYPICAL CONCRETE COLUMN ISOLATION JOINT DETAIL

NOT TO SCALE

NOTES:

1. DIMENSION "A" TO BE KEPT IN BLOCK COURSING NOT TO EXCEED 1'-4" AT MASONRY WALLS OR 1'-0" AT CONCRETE WALLS.

2. FOOTING REINFORCING IN ALL BLOCK FOUNDATION WALLS ARE (2) #5 CONTINUOUS (EXTEND THROUGH SPREAD FOOTINGS UNLESS NOTED OTHERWISE).

3. THIS DIMENSION IS AT LEAST THE MINIMUM LAP TYPE FOR THE SIZE OF REINFORCING. DOWELS AS REQUIRED.

NOTE 3

1'-0"

2'-4" MIN.

NOTE 1

NOTE 4

CONTINUOUS WALL FOOTING

NOTE 2

NOTE 3

TYPICAL STEPPED FOOTING DETAIL

NOT TO SCALE

NOTES:

1. FOR BOLTS LESS THAN 2" Ø, "A" DIMENSION IS 1-1/2" TYPICAL, UNLESS NOTED OTHERWISE.

2. FOR BOLTS EQUAL TO OR GREATER THAN 2" Ø, "A" DIMENSION IS 3" TYPICAL, UNLESS NOTED OTHERWISE.

3. INCREASE THE LENGTH OF THE THREADED RODS BY THE PIER LENGTH WHEN REQUIRED.

4. TACK WELD NUTS AND PLATE TO THREADED RODS.

5. SEE BASE PLATE SCHEDULE FOR DIMENSIONS.

STEEL COLUMN

NOTES 1 AND 2

BASE PLATE

NOTE 2

CONCRETE SLAB

NOTE 2

1/2" NON-SHRINK GROUT

JOINT FILLER TO EXTEND FULL DEPTH OF JOINT

1/4" Ø VENT HOLE FOR GROUT, TYP.

ANCHOR BOLTS

NOTE 2

(4) 3/4" Ø A36 THREADED RODS

1 1/2" NON-SHRINK GROUT

NOTE 3 TYP.

STEEL BEAM

SEE PLAN

FOOTING

SEE PLAN

BASE PLATE WITH THREADED RODS

NOT TO SCALE

NOTES:

1. TABULATED VALUES ARE BASED ON A MINIMUM YIELD STRENGTH OF 60,000 PSI. LENGTHS ARE IN INCHES.

2. TENSION DEVELOPMENT LENGTHS AND TENSION LAP SPICE LENGTHS ARE CALCULATED PER ACI 318-19, SECTIONS 25.4.2.2 AND 25.5.1, RESPECTIVELY.

3. TABULATED VALUES FOR BEAMS OR COLUMNS ARE BASED ON TRANSVERSE REINFORCEMENT AND CONCRETE COVER MEETING MINIMUM CODE REQUIREMENTS.

4. CASES ONE AND TWO, WHICH DEPEND ON THE TYPE OF STRUCTURAL MEMBER, CONCRETE COVER, AND CENTER-TO-CENTER SPACING OF THE BARS, ARE DEFINED AS:

- BEAMS, COLUMNS - CASE ONE: CONCRETE COVER AT LEAST 1 BAR DIAMETER AND CENTER-TO-CENTER SPACING AT LEAST 2 BAR DIAMETERS.
- BEAMS, COLUMNS - CASE TWO: CONCRETE COVER LESS THAN 1 BAR DIAMETER OR CENTER-TO-CENTER SPACING LESS THAN 2 BAR DIAMETERS.
- ALL OTHERS - CASE ONE: CONCRETE COVER AT LEAST 1 BAR DIAMETER AND CENTER-TO-CENTER SPACING AT LEAST 3 BAR DIAMETERS.
- ALL OTHERS - CASE TWO: CONCRETE COVER LESS THAN 1 BAR DIAMETER OR CENTER-TO-CENTER SPACING LESS THAN 3 BAR DIAMETERS.

5. LAP SPICE LENGTHS (MINIMUM OF 12 INCHES) ARE MULTIPLES OF TENSION DEVELOPMENT LENGTHS, CLASS B = 1.3L_d (ACI 12.15.1). WHEN DETERMINING THE LAP SPICE LENGTH, L_s IS CALCULATED WITHOUT THE 12-INCH MINIMUM OF ACI 25.4.2.1.

6. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS.

7. FOR GRADE 75 (MINIMUM YIELD STRENGTH OF 75,000 PSI) REINFORCING BARS, LENGTHS MUST BE MULTIPLIED BY 1.25. FOR GRADE 80 REINFORCING BARS, MULTIPLE THE TABULATED VALUES BY 1.33.

8. FOR LIGHTWEIGHT AGGREGATE CONCRETE, MULTIPLE THE TABULATED VALUES BY 1.33.

BAR SIZE	LAP CLASS	TENSION LAP SPICES								COMPRESSION LAP SPICES						
		r _c = 3000 PSI, NORMAL WEIGHT LAP LENGTH PER SPACING & COVER CASE		r _c = 4000 PSI, NORMAL WEIGHT LAP LENGTH PER SPACING & COVER CASE		r _c = 5000 PSI, NORMAL WEIGHT LAP LENGTH PER SPACING & COVER CASE		COMPRESSION LAP SPICES								
		CASE ONE	CASE TWO	CASE ONE	CASE TWO	CASE ONE	CASE TWO	r _c = 3000 PSI	r _c = 4000 PSI	r _c = 5000 PSI						
#3	B	28"	22"	42"	32"	24"	19"	36"	28"	22"	17"	33"	25"	12"	12"	12"
#4	B	37"	29"	56"	43"	32"	25"	48"	37"	29"	22"	43"	33"	15"	15"	15"
#5	B	47"	36"	70"	54"	40"	31"	60"	47"	36"	28"	54"	42"	19"	19"	19"
#6	B	56"	43"	84"	64"	48"	37"	72"	56"	43"	33"	65"	50"	23"	23"	23"
#7	B	81"	63"	122"	94"	70"	54"	106"	81"	63"	49"	94"	73"	27"	27"	27"
#8	B	93"	72"	139"	107"	80"	62"	121"	93"	72"	55"	108"	83"	30"	30"	30"
#9	B	105"	81"	157"	121"	91"	70"	136"	105"	81"	63"	122"	94"	34"	34"	34"
#10	B	118"	91"	177"	136"	102"	79"	153"	118"	91"	70"	137"	105"	38"	38"	38"
#11	B	131"	101"	196"	151"	113"	87"	170"	131"	101"	78"	152"	117"	43"	43"	43"

NOTES:

1. TABULATED VALUES ARE BASED ON A MINIMUM YIELD STRENGTH OF 60,000 PSI AND NORMAL-WEIGHT CONCRETE. LENGTHS ARE IN INCHES.

2. TENSION DEVELOPMENT LENGTHS AND TENSION LAP SPICE LENGTHS ARE CALCULATED PER ACI 318-19, SECTIONS 25.4.2.3 AND 25.5.1, RESPECTIVELY, WITH BAR SIZES LIMITED TO #3 THROUGH #11.

3. WHEN THE VARIABLE "s_v" FROM ACI 25.4.2.3 WAS CALCULATED, IT WAS ASSUMED THAT CONCRETE COVER CONTROLLED. THAT IS, CENTER-TO-CENTER SPACING WAS ASSUMED TO BE GREATER THAN 1 BAR DIAMETER PLUS TWICE THE CONCRETE COVER.

4. LAP SPICE LENGTHS (MINIMUM OF 12 INCHES) ARE MULTIPLES OF TENSION DEVELOPMENT LENGTHS, CLASS B = 1.3L_d (ACI 318 25.5.1). WHEN DETERMINING THE LAP SPICE LENGTH, L_s IS CALCULATED WITHOUT THE 12-INCH MINIMUM OF ACI 25.4.2.1.

5. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS.

6. FOR GRADE 75 (MINIMUM YIELD STRENGTH OF 75,000 PSI) REINFORCING BARS, LENGTHS MUST BE MULTIPLIED BY 1.25. FOR GRADE 80 REINFORCING BARS, MULTIPLE THE TABULATED VALUES BY 1.33.

7. FOR LIGHTWEIGHT AGGREGATE CONCRETE, MULTIPLE THE TABULATED VALUES BY 1.33.

BAR SIZE	LAP CLASS	TENSION LAP SPICES						COMPRESSION LAP SPICES							
		r _c = 3000 PSI		r _c = 4000 PSI		r _c = 5000 PSI		COMPRESSION LAP SPICES							
		CONCRETE COVER = 1.5 INCHES	CONCRETE COVER = 2.0 INCHES	CONCRETE COVER = 1.5 INCHES	CONCRETE COVER = 2.0 INCHES	CONCRETE COVER = 1.5 INCHES	CONCRETE COVER = 2.0 INCHES	r _c = 3000 PSI	r _c = 4000 PSI	r _c = 5000 PSI					
#3	B	17"	13"	17"	13"	15"	12"	15"	12"	13"	12"	12"	12"	12"	12"
#4	B	23"	17"	23"	17"	20"	15"	20"	15"	18"	14"	18"	14"	15"	15"
#5	B	28"	22"	28"	22"	24"	19"	24"	19"	22"	17"	22"	17"	19"	19"
#6	B	34"	26"	34"	26"	29"	22"	29"	22"	26"	20"	26"	20"	23"	23"
#7	B	55"	43"	49"	38"	48"	37"	42"	33"	43"	33"	38"	29"	27"	27"
#8	B	70"	54"	56"	43"	60"	47"	48"	37"	54"	42"	43"	33"	30"	30"
#9	B	86"	66"	69"	53"	74"	57"	60"	46"	67"	51"	54"	41"	34"	34"
#10	B	105"	81"	85"	66"	91"	70"	74"	57"	82"	63"	66"	51"	38"	38"
#11	B	125"	97"	102"	79"	109"	84"	89"	68"	97"	75"	79"	61"	43"	43"

NOTES:

1. FOOTING, COLUMN, AND SLAB REINFORCEMENT NOT SHOWN FOR CLARITY. SEE PLAN AND SCHEDULE.

STEEL COLUMN

NOTES 1 AND 2

BASE PLATE

NOTE 2

CONCRETE SLAB

NOTE 2

1/2" NON-SHRINK GROUT

JOINT FILLER TO EXTEND FULL DEPTH OF JOINT

1/4" Ø VENT HOLE FOR GROUT, TYP.

ANCHOR BOLTS

NOTE 2

(4) 3/4" Ø A36 THREADED RODS

1 1/2" NON-SHRINK GROUT

NOTE 3 TYP.

STEEL BEAM

SEE PLAN

FOOTING

SEE PLAN

BASE PLATE WITH THREADED RODS

NOT TO SCALE

NOTES:

1. TABULATED VALUES ARE BASED ON A MINIMUM YIELD STRENGTH OF 60,000 PSI. LENGTHS ARE IN INCHES.

2. TENSION DEVELOPMENT LENGTHS AND TENSION LAP SPICE LENGTHS ARE CALCULATED PER ACI 318-19, SECTIONS 25.4.2.2 AND 25.5.1, RESPECTIVELY.

3. TABULATED VALUES FOR BEAMS OR COLUMNS ARE BASED ON TRANSVERSE REINFORCEMENT AND CONCRETE COVER MEETING MINIMUM CODE REQUIREMENTS.

4. CASES ONE AND TWO, WHICH DEPEND ON THE TYPE OF STRUCTURAL MEMBER, CONCRETE COVER, AND CENTER-TO-CENTER SPACING OF THE BARS, ARE DEFINED AS:

- BEAMS, COLUMNS - CASE ONE: CONCRETE COVER AT LEAST 1 BAR DIAMETER AND CENTER-TO-CENTER SPACING AT LEAST 2 BAR DIAMETERS.
- BEAMS, COLUMNS - CASE TWO: CONCRETE COVER LESS THAN 1 BAR DIAMETER OR CENTER-TO-CENTER SPACING LESS THAN 2 BAR DIAMETERS.
- ALL OTHERS - CASE ONE: CONCRETE COVER AT LEAST 1 BAR DIAMETER AND CENTER-TO-CENTER SPACING AT LEAST 3 BAR DIAMETERS.
- ALL OTHERS - CASE TWO: CONCRETE COVER LESS THAN 1 BAR DIAMETER OR CENTER-TO-CENTER SPACING LESS THAN 3 BAR DIAMETERS.

5. LAP SPICE LENGTHS (MINIMUM OF 12 INCHES) ARE MULTIPLES OF TENSION DEVELOPMENT LENGTHS, CLASS B = 1.3L_d (ACI 12.15.1). WHEN DETERMINING THE LAP SPICE LENGTH, L_s IS CALCULATED WITHOUT THE 12-INCH MINIMUM OF ACI 25.4.2.1.

6. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS.

7. FOR GRADE 75 (MINIMUM YIELD STRENGTH OF 75,000 PSI) REINFORCING BARS, LENGTHS MUST BE MULTIPLIED BY 1.25. FOR GRADE 80 REINFORCING BARS, MULTIPLE THE TABULATED VALUES BY 1.33.

8. FOR LIGHTWEIGHT AGGREGATE CONCRETE, MULTIPLE THE TABULATED VALUES BY 1.33.

BAR SIZE	LAP CLASS	TENSION LAP SPICES								COMPRESSION LAP SPICES						
		r _c = 3000 PSI, NORMAL WEIGHT LAP LENGTH PER SPACING & COVER CASE		r _c = 4000 PSI, NORMAL WEIGHT LAP LENGTH PER SPACING & COVER CASE		r _c = 5000 PSI, NORMAL WEIGHT LAP LENGTH PER SPACING & COVER CASE		COMPRESSION LAP SPICES								
		CASE ONE	CASE TWO	CASE ONE	CASE TWO	CASE ONE	CASE TWO	r _c = 3000 PSI	r _c = 4000 PSI	r _c = 5000 PSI						
#3	B	28"	22"	42"	32"	24"	19"	36"	28"	22"	17"	33"	25"	12"	12"	12"
#4	B	37"	29"	56"	43"	32"	25"	48"	37"	29"	22"	43"	33"	15"	15"	15"
#5	B	47"	36"	70"	54"	40"	31"	60"	47"	36"	28"	54"	42"	19"	19"	19"
#6	B	56"	43"	84"	64"	48"	37"	72"	56"	43"	33"	65"	50"	23"	23"	23"
#7	B	81"	63"	122"	94"	70"	54"	106"	81"	63"	49"	94"	73"	27"	27"	27"
#8	B	93"	72"	139"	107"	80"	62"	121"	93"	72"	55"	108"	83"	30"	30"	30"
#9	B	105"	81"	157"	121"	91"	70"	136"	105"	81"	63"	122"	94"	34"	34"	34"
#10	B	118"	91"	177"	136"	102"	79"	153"	118"	91"	70"	137"	105"	38"	38"	38"
#11	B	131"	101"	196"	151"	113"	87"	170"	131"	101"	78"	152"	117"	43"	43"	43"

NOTES:

1. TABULATED VALUES ARE BASED ON A MINIMUM YIELD STRENGTH OF 60,000 PSI AND NORMAL-WEIGHT CONCRETE. LENGTHS ARE IN INCHES.

2. TENSION DEVELOPMENT LENGTHS AND TENSION LAP SPICE LENGTHS ARE CALCULATED PER ACI 318-19, SECTIONS 25.4.2.3 AND 25.5.1, RESPECTIVELY, WITH BAR SIZES LIMITED TO #3 THROUGH #11.

3. WHEN THE VARIABLE "s_v" FROM ACI 25.4.2.3 WAS CALCULATED, IT WAS ASSUMED THAT CONCRETE COVER CONTROLLED. THAT IS, CENTER-TO-CENTER SPACING WAS ASSUMED TO BE GREATER THAN 1 BAR DIAMETER PLUS TWICE THE CONCRETE COVER.

4. LAP SPICE LENGTHS (MINIMUM OF 12 INCHES) ARE MULTIPLES OF TENSION DEVELOPMENT LENGTHS, CLASS B = 1.3L_d (ACI 318 25.5.1). WHEN DETERMINING THE LAP SPICE LENGTH, L_s IS CALCULATED WITHOUT THE 12-INCH MINIMUM OF ACI 25.4.2.1.

5. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS.

6. FOR GRADE 75 (MINIMUM YIELD STRENGTH OF 75,000 PSI) REINFORCING BARS, LENGTHS MUST BE MULTIPLIED BY 1.25. FOR GRADE 80 REINFORCING BARS, MULTIPLE THE TABULATED VALUES BY 1.33.

7. FOR LIGHTWEIGHT AGGREGATE CONCRETE, MULTIPLE THE TABULATED VALUES BY 1.33.

BAR SIZE	LAP CLASS	TENSION LAP SPICES						COMPRESSION LAP SPICES							
		r _c = 3000 PSI		r _c = 4000 PSI		r _c = 5000 PSI		COMPRESSION LAP SPICES							
		CONCRETE COVER = 1.5 INCHES	CONCRETE COVER = 2.0 INCHES	CONCRETE COVER = 1.5 INCHES	CONCRETE COVER = 2.0 INCHES	CONCRETE COVER = 1.5 INCHES	CONCRETE COVER = 2.0 INCHES	r _c = 3000 PSI	r _c = 4000 PSI	r _c = 5000 PSI					
#3	B	17"	13"	17"	13"	15"	12"	15"	12"	13"	12"	12"	12"	12"	12"
#4	B	23"	17"	23"	17"	20"	15"	20"	15"	18"	14"	18"	14"	15"	15"
#5	B	28"	22"	28"	22"	24"	19"	24"	19"	22"	17"	22"	17"	19"	19"
#6	B	34"	26"	34"	26"	29"	22"	29"	22"	26"	20"	26"	20"	23"	23"
#7	B	55"	43"	49"	38"	48"	37"	42"	33"	43"	33"	38"	29"	27"	27"
#8	B	70"	54"	56"	43"	60"	47"	48"	37"	54"	42"	43"	33"	30"	30"
#9	B	86"	66"	69"	53"	74"	57"	60"	46"	67"	51"	54"	41"	34"	34"
#10	B	105"	81"	85"	66"	91"	70"	74"	57"	82"	63"	66"	51"	38"	38"
#11	B	125"	97"	102"	79"	109"	84"	89"	68"	97"	75"	79"	61"	43"	43"

This item has been digitally signed and sealed by Matthew D. Brady, P.E. on the date indicated below. The seal number can be verified on any electronic copies.

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Project No: 2035-001

SEAL

MATTHEW D. BRAKE, P.E.
LICENSE
No. 70852
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

DEUK SPINE INSTITUTE

Surgery Center at Millenia, LLC

Hunton Brady ARCHITECTS

DATE	SUBMISSION	NO
12.12.23	CONSTRUCTION DOCUMENTS	
05.19.2026	CCD NO. 1	1

TYPICAL DETAILS

PROJ. NO. H-23016.00 SHEET
DRAWN AM

S-301

S-302

COLUMN CONNECTIONS

NOTES:

1. ALL CONNECTIONS SHALL BE DESIGNED FOR MAXIMUM UNIFORM LOAD CARRYING CAPACITY UNLESS GREATER REACTION IS SHOWN ON PLAN. IN ANY CASE, CONNECTIONS SHALL NOT BE LESS THAN THE STANDARD A.I.S.C. CONNECTION FOR THE PARTICULAR BEAM DEPTH.

2. DETAILER SHALL SUBMIT, FOR APPROVAL, STANDARD CONNECTION DETAILS CONFORMING TO ABOVE DETAILS WITH ERECTION DRAWING SUBMISSION.

3. MILL COLUMNS TO BEAR OR PROVIDE FULL PENETRATION WELD AT BASE PLATES AND CAP PLATES AT COLUMNS CARRYING BEAMS.

4. AT CONNECTION OF HUNG BEAM TO CANTILEVERED BEAMS, PROVIDE STANDARD 2" CONNECTORS AS PER NOTE 1 FOR LOAD CAPACITY OF HUNG BEAM.

5. WHERE BEAMS FRAME TO FITTED STIFFENERS AT COLUMNS, PROVIDE SHEAR PLATE CONNECTOR WITH STRENGTH CAPACITY AS PER NOTE 1.

6. ALL BOLTS TO BE MINIMUM 3/8" A-325 HIGH-STRENGTH BOLTS, FRICTION-TYPE, UNLESS NOTED OTHERWISE. ALL WELDING ELECTRODES TO BE E 70 XX.

7. DIMENSIONS SHOWN AS DEFINED BY A.I.S.C. MANUAL OF STEEL CONSTRUCTION, COLUMN MAY OCCUR ABOVE BEAM, FOR TYPE, SIZE, AND ORIENTATION, SEE PLAN AND COLUMN SCHEDULE.

9. 3/4" MINIMUM THICKNESS COLUMN CAP PLATE WELDED TO COLUMN, LENGTH OF CAP PLATE AS SHOWN, WIDTH OF CAP PLATE IS EQUAL TO THE BEAM FLANGE, OR TO THE COLUMN WIDTH + 1", WHICHEVER IS THE LARGER DIMENSION. CONNECT CAP PLATE TO BEAM WITH (4) 3/8" HIGH-STRENGTH BOLTS.

10. BASE PLATE DIMENSIONS SAME AS COLUMN CAP PLATE DIMENSIONS IN NOTE 9.

11. IF NO COLUMN ABOVE THE BEAM, USE TWO STIFFENER PLATES 3/4" MINIMUM THICKNESS UNLESS NOTED OTHERWISE. ALL STIFFENERS TO BE FITTED AND WELDED FULL LENGTH.

12. IF COLUMN ABOVE THE BEAM, USE FOUR STIFFENER PLATES WITH THE SAME SIZE AS THE COLUMN FLANGES, 3/4" MINIMUM THICKNESS. IF COLUMN WEB IS PERPENDICULAR TO BEAM WEB, USE "T" SECTION, ALL STIFFENERS TO BE FITTED AND WELDED FULL LENGTH.

3-1/2" MIN

1-1/4" MIN

1-1/4" MAX

0.5" MIN

1" MAX

BOLTS AS REQUIRED

SHOP BOLTED CONNECTION

NOT TO SCALE

3" MIN

1/2"

0.5" MIN

1" MAX

BOLTS AS REQUIRED

SHOP WELDED CONNECTION

NOT TO SCALE

NOTE 10

NOTE 8

NOTE 11

NOTE 9

NOTE 12

BEAM OVER / UNDER COLUMN

NOT TO SCALE

TYP

1/2"

MIN

WELD PLATES WHEREVER POSSIBLE

3/4"

PLATE MINIMUM THICKNESS CONTINUOUS THROUGH COLUMN

NOTE 1

1-1/4" MIN

D + 4-1/2"

MAX

1-1/4" MIN

FRAMED CONNECTION AT TUBE / PIPE COLUMN

NOT TO SCALE

NOTE:

IN CASE OF CONFLICT BETWEEN INFORMATION SHOWN ON THIS DRAWING, SECTION DRAWINGS, OR ARCHITECTURAL DRAWINGS, IT IS THE CONTRACTOR'S RESPONSIBILITY TO INFORM THE ARCHITECT AND ENGINEER OF SUCH CONFLICT IN ORDER TO RECEIVE A CLARIFICATION BEFORE PROCEEDING TO WORK. DIMENSIONS, SIZES, AND REINFORCEMENT OF STRUCTURAL ELEMENTS SHOWN IN THIS DRAWING ARE MINIMUM TO BE USED UNLESS NOTED OTHERWISE (U.N.O.) IN PLANS.

ROOF JOIST TO 12" CMU WALL CONNECTION SECTIONS

NOTES:

1. FASTEN THE DECK CLOSURE/SUPPORT ANGLE TO THE CMU WALL WITH 3/4" DIAMETER SIMPSON TITEN HD SCREWS AT 24" ON CENTER MAXIMUM. EMBED DEPTH EQUALS 5".

2. PROVIDE 3/8" THICK STIFFENER PLATES AT 24" O/C, OPPOSITE TITEN SCREW HOLES.

TOP OF PARAPET WALL
ELEVATION - SEE ARCH.

DOUBLE BOND BEAM
REINFORCED WITH (2) #5
CONTINUOUS IN
KNOCK-OUT BLOCK

BOTTOM OF BOND BEAM
STEP AS REQUIRED AT
SLOPED ROOF

VERTICAL REINFORCING
CONTINUOUS TO TOP OF WALL

CONT. L4x4x1/4 DECK
SUPPORT - SEE NOTE 1

WELD DECK TO SUPPORT ANGLE

METAL ROOF DECK
SEE PLAN

STEEL JOIST
SEE PLAN

"X-TYPE" BRIDGING
SEE PLAN

HORIZONTAL BRIDGING
SEE PLAN

L3x3x1/4 WITH (1) 3/4" DIA. X 5"
LONG SIMPSON TITEN HD
SCREW
-GROUT CELL SOLID

SECTION AT 12" CMU WALL PARALLEL TO JOIST -
BOND BEAM
NOT TO SCALE

TOP OF PARAPET WALL
ELEVATION - SEE ARCH.

DOUBLE BOND BEAM
REINFORCED WITH (2) #5
CONTINUOUS IN
KNOCK-OUT BLOCK

BOTTOM/BOND BEAM

VERTICAL REINFORCING
CONTINUOUS TO TOP OF WALL

CONT. L4x4x1/4 DECK
CLOSURE - SEE NOTE 1
(ALSO WELD TO JOISTS)

METAL ROOF DECK
SEE PLAN

STEEL JOIST - SEE PLAN
PROVIDE 5" MIN. JOIST
BEARING

PL 1/2"x8"x8" LONG WITH
(2) 1/2" DIA. X 8" LONG
HEADED STUD ANCHORS

L3x3x1/4 WITH (1) 3/4" DIA. X 5"
LONG SIMPSON TITEN HD
SCREW

SECTION AT 12" CMU WALL SINGLE BEARING -
BOND BEAM
NOT TO SCALE

TOP OF PARAPET WALL
ELEVATION - SEE ARCH.

DOUBLE BOND BEAM
REINFORCED WITH (2) #5
CONTINUOUS IN
KNOCK-OUT BLOCK

TOP OF STEEL JOISTS
ELEVATION - SEE PLAN

CONCRETE TIE BEAM -
SEE PLAN FOR BEAM
SIZE AND REINF.

VERTICAL REINFORCING
CONTINUOUS TO TOP OF WALL

CONT. L4x4x1/4 DECK
SUPPORT - SEE NOTE 1

WELD DECK TO SUPPORT ANGLE

METAL ROOF DECK
SEE PLAN

STEEL JOIST
SEE PLAN

"X-TYPE" BRIDGING
SEE PLAN

HORIZONTAL BRIDGING
SEE PLAN

L3x3x1/4 WITH (1) 3/4" DIA. X 5"
LONG SIMPSON TITEN HD
SCREW

SECTION AT 12" CMU WALL PARALLEL TO JOIST -
TIE BEAM
NOT TO SCALE

TOP OF PARAPET WALL
ELEVATION - SEE ARCH.

DOUBLE BOND BEAM
REINFORCED WITH (2) #5
CONTINUOUS IN
KNOCK-OUT BLOCK

TOP OF STEEL JOISTS
ELEVATION - SEE PLAN

CONCRETE TIE BEAM -
SEE PLAN FOR BEAM
SIZE AND REINF.

VERTICAL REINFORCING
CONTINUOUS TO TOP OF WALL

PL 1/2"x8"x8" LONG WITH
(2) 1/2" DIA. X 8" LONG
HEADED STUD ANCHORS

CONT. L4x4x1/4 DECK
CLOSURE - SEE NOTE 1
(ALSO WELD TO JOISTS)

METAL ROOF DECK
SEE PLAN

STEEL JOIST - SEE PLAN
PROVIDE 5" MIN. JOIST
BEARING

PL 1/2"x3"x4"

SECTION AT 12" CMU WALL SINGLE BEARING -
TIE BEAM
NOT TO SCALE

SECTIONS AT 12" CMU WALL SINGLE BEARING - TIE BEAM

NOTES:

1. FASTEN THE DECK CLOSURE/SUPPORT ANGLE TO THE CMU WALL WITH 3/4" DIAMETER SIMPSON TITEN HD SCREWS AT 24" ON CENTER MAXIMUM. EMBED DEPTH EQUALS 5".

2. PROVIDE 3/8" THICK STIFFENER PLATES AT 24" O/C, OPPOSITE TITEN SCREW HOLES.

TOP OF CONCRETE BEAM
ELEVATION - SEE PLAN

(1) #5 CONTINUOUS IN
CONCRETE TIE BEAM

(2) #5 CONTINUOUS IN
CONCRETE TIE BEAM
INSTALLED 2" BELOW
JOIST SEAT

CONCRETE TIE BEAM -
SEE PLAN FOR BEAM
SIZE AND REINF.

CONT. L4x4x1/4 DECK
CLOSURE - SEE NOTE 1
(ALSO WELD TO JOISTS)

STEEL JOIST - SEE PLAN
PROVIDE 5" MIN. JOIST
BEARING

METAL ROOF DECK
SEE PLAN

PL 1/2"x8"x8" LONG WITH
(2) 1/2" DIA. X 8" LONG
HEADED STUD ANCHORS

PL 1/2"x3"x4"

BETWEEN JOIST BEARING
NOT TO SCALE

TOP OF CONCRETE BEAM
ELEVATION - SEE PLAN

(1) #5 CONTINUOUS IN
CONCRETE TIE BEAM

(2) #5 CONTINUOUS IN
CONCRETE TIE BEAM
INSTALLED 2" BELOW
JOIST SEAT

CONCRETE TIE BEAM -
SEE PLAN FOR BEAM
SIZE AND REINF.

PL 1/2"x8"x8" LONG WITH
(2) 1/2" DIA. X 8" LONG
HEADED STUD ANCHORS

PL 1/2"x3"x4"

AT JOIST BEARING
NOT TO SCALE

WELD CALL-OUTS FOR K-SERIES
JOIST ONLY. SEE SJI STANDARDS
FOR LH AND DLH-SERIES JOIST
WELDING REQUIREMENTS.

BEARING ELEVATION
NOT TO SCALE

ROOF JOIST TO 8" CMU WALL CONNECTION SECTIONS

NOTES:

1. FASTEN THE DECK CLOSURE/SUPPORT ANGLE TO THE CMU WALL WITH 3/4" DIAMETER SIMPSON TITEN HD SCREWS AT 24" ON CENTER MAXIMUM. EMBED DEPTH EQUALS 5".

2. PROVIDE 3/8" THICK STIFFENER PLATES AT 24" O/C, OPPOSITE TITEN SCREW HOLES.

TOP OF PARAPET WALL
ELEVATION - SEE ARCH.

DOUBLE BOND BEAM
REINFORCED WITH (2) #5
CONTINUOUS IN
KNOCK-OUT BLOCK

BOTTOM OF BOND BEAM
STEP AS REQUIRED AT
SLOPED ROOF

VERTICAL REINFORCING
CONTINUOUS TO TOP OF WALL

CONT. L4x4x1/4 DECK
SUPPORT - SEE NOTE 1

WELD DECK TO SUPPORT ANGLE

METAL ROOF DECK
SEE PLAN

STEEL JOIST
SEE PLAN

"X-TYPE" BRIDGING
SEE PLAN

HORIZONTAL BRIDGING
SEE PLAN

L3x3x1/4 WITH (1) 3/4" DIA. X 5"
LONG SIMPSON TITEN HD
SCREW

SECTION AT 8" CMU WALL PARALLEL TO JOIST -
BOND BEAM
NOT TO SCALE

TOP OF PARAPET WALL
ELEVATION - SEE ARCH.

DOUBLE BOND BEAM
REINFORCED WITH (2) #5
CONTINUOUS IN
KNOCK-OUT BLOCK

TOP OF STEEL JOISTS
ELEVATION - SEE PLAN

CONCRETE TIE BEAM -
SEE PLAN FOR BEAM
SIZE AND REINF.

VERTICAL REINFORCING
CONTINUOUS TO TOP OF WALL

CONT. L4x4x1/4 DECK
SUPPORT - SEE NOTE 1

WELD DECK TO SUPPORT ANGLE

METAL ROOF DECK
SEE PLAN

STEEL JOIST
SEE PLAN

"X-TYPE" BRIDGING
SEE PLAN

HORIZONTAL BRIDGING
SEE PLAN

L3x3x1/4 WITH (1) 3/4" DIA. X 5"
LONG SIMPSON TITEN HD
SCREW

SECTION AT 8" CMU WALL PARALLEL TO JOIST -
TIE BEAM
NOT TO SCALE

TOP OF PARAPET WALL
ELEVATION - SEE ARCH.

DOUBLE BOND BEAM
REINFORCED WITH (2) #5
CONTINUOUS IN
KNOCK-OUT BLOCK

TOP OF STEEL JOISTS
ELEVATION - SEE PLAN

CONCRETE TIE BEAM -
SEE PLAN FOR BEAM
SIZE AND REINF.

VERTICAL REINFORCING
CONTINUOUS TO TOP OF WALL

PL 1/2"x8"x8" LONG WITH
(2) 1/2" DIA. X 8" LONG
HEADED STUD ANCHORS

CONT. L4x4x1/4 DECK
CLOSURE - SEE NOTE 1
(ALSO WELD TO JOISTS)

METAL ROOF DECK
SEE PLAN

STEEL JOIST - SEE PLAN
PROVIDE 5" MIN. JOIST
BEARING

PL 1/2"x3"x4"

SECTION AT 8" CMU WALL SINGLE BEARING -
TIE BEAM
NOT TO SCALE

TOP OF PARAPET WALL
ELEVATION - SEE ARCH.

DOUBLE BOND BEAM
REINFORCED WITH (2) #5
CONTINUOUS IN
KNOCK-OUT BLOCK

TOP OF STEEL JOISTS
ELEVATION - SEE PLAN

CONCRETE TIE BEAM -
SEE PLAN FOR BEAM
SIZE AND REINF.

VERTICAL REINFORCING
CONTINUOUS TO TOP OF WALL

PL 1/2"x8"x8" LONG WITH
(2) 1/2" DIA. X 8" LONG
HEADED STUD ANCHORS

CONT. L4x4x1/4 DECK
CLOSURE - SEE NOTE 1
(ALSO WELD TO JOISTS)

METAL ROOF DECK
SEE PLAN

STEEL JOIST - SEE PLAN
PROVIDE 5" MIN. JOIST
BEARING

PL 1/2"x3"x4"

SECTION AT 8" CMU WALL SINGLE BEARING -
TIE BEAM
NOT TO SCALE

SECTIONS AT 8" CMU WALL SINGLE BEARING - TIE BEAM

NOTES:

1. FASTEN THE DECK CLOSURE/SUPPORT ANGLE TO THE CMU WALL WITH 3/4" DIAMETER SIMPSON TITEN HD SCREWS AT 24" ON CENTER MAXIMUM. EMBED DEPTH EQUALS 5".

2. PROVIDE 3/8" THICK STIFFENER PLATES AT 24" O/C, OPPOSITE TITEN SCREW HOLES.

TOP OF CONCRETE BEAM
ELEVATION - SEE PLAN

(1) #5 CONTINUOUS IN
CONCRETE TIE BEAM

(2) #5 CONTINUOUS IN
CONCRETE TIE BEAM
INSTALLED 2" BELOW
JOIST SEAT

CONCRETE TIE BEAM -
SEE PLAN FOR BEAM
SIZE AND REINF.

CONT. L4x4x1/4 DECK
CLOSURE - SEE NOTE 1
(ALSO WELD TO JOISTS)

STEEL JOIST - SEE PLAN
PROVIDE 5" MIN. JOIST
BEARING

METAL ROOF DECK
SEE PLAN

PL 1/2"x8"x8" LONG WITH
(2) 1/2" DIA. X 8" LONG
HEADED STUD ANCHORS

PL 1/2"x3"x4"

BETWEEN JOIST BEARING
NOT TO SCALE

TOP OF CONCRETE BEAM
ELEVATION - SEE PLAN

(1) #5 CONTINUOUS IN
CONCRETE TIE BEAM

(2) #5 CONTINUOUS IN
CONCRETE TIE BEAM
INSTALLED 2" BELOW
JOIST SEAT

CONCRETE TIE BEAM -
SEE PLAN FOR BEAM
SIZE AND REINF.

PL 1/2"x8"x8" LONG WITH
(2) 1/2" DIA. X 8" LONG
HEADED STUD ANCHORS

PL 1/2"x3"x4"

AT JOIST BEARING
NOT TO SCALE

WELD CALL-OUTS FOR K-SERIES
JOIST ONLY. SEE SJI STANDARDS
FOR LH AND DLH-SERIES JOIST
WELDING REQUIREMENTS.

BEARING ELEVATION
NOT TO SCALE

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Project No: 2035-001

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Hunton Brady

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DATE
12.12.23

SUBMISSION
CONSTRUCTION DOCUMENTS

NO

TYPICAL DETAILS

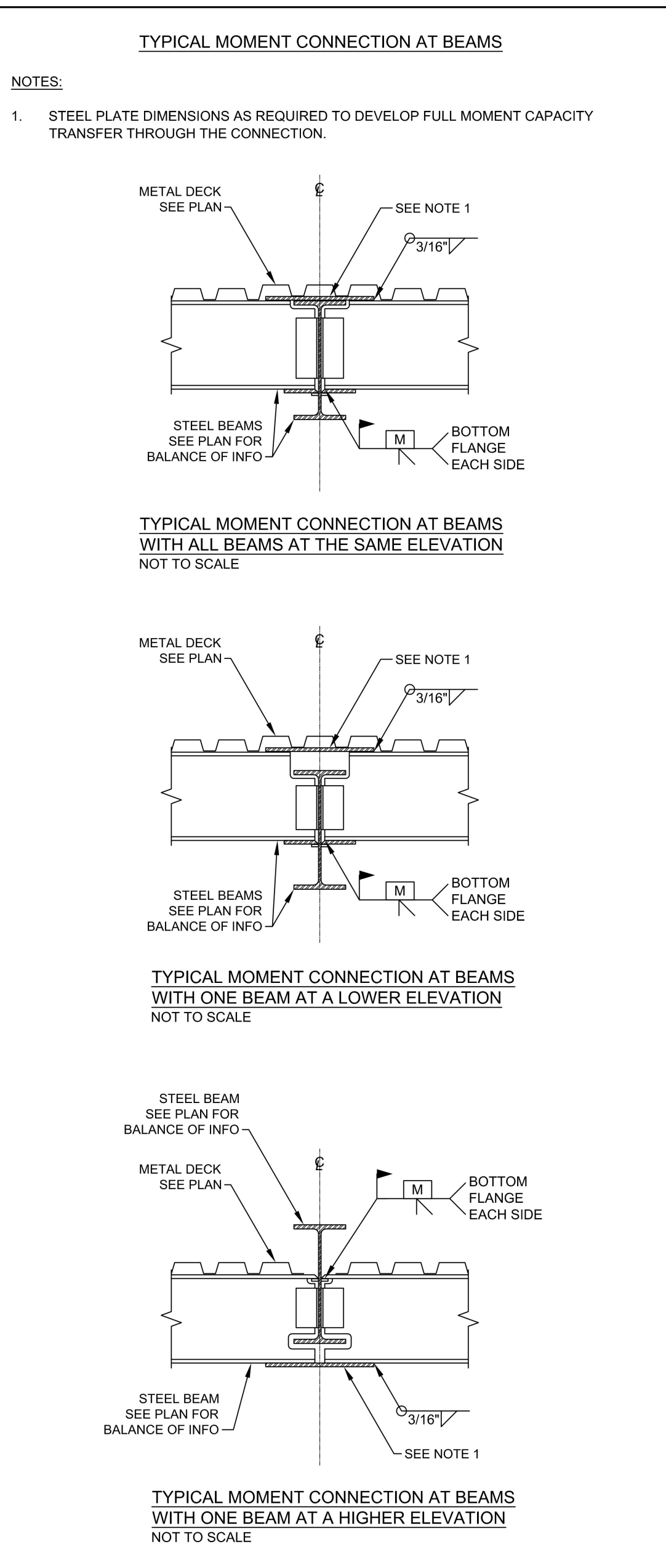
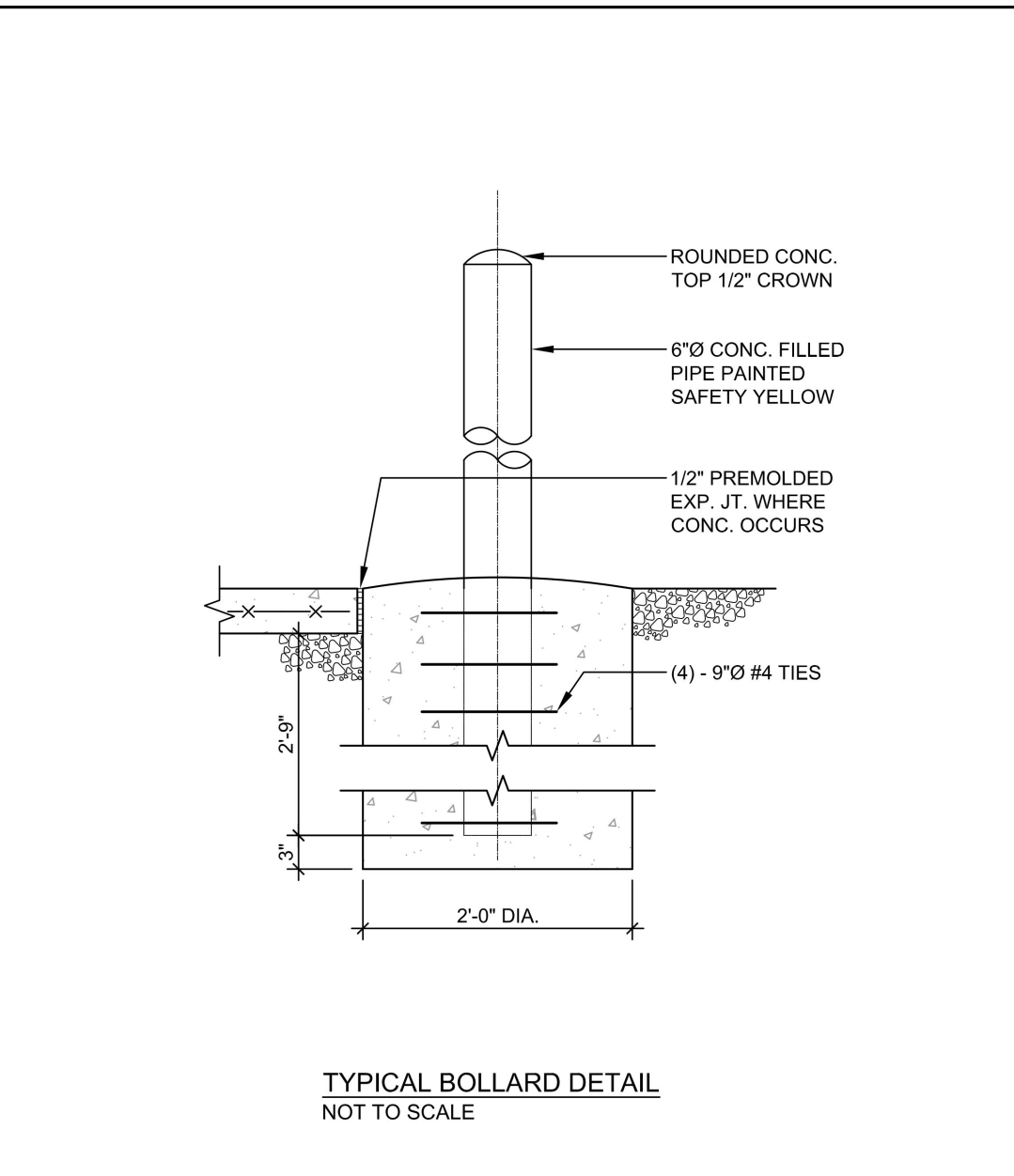
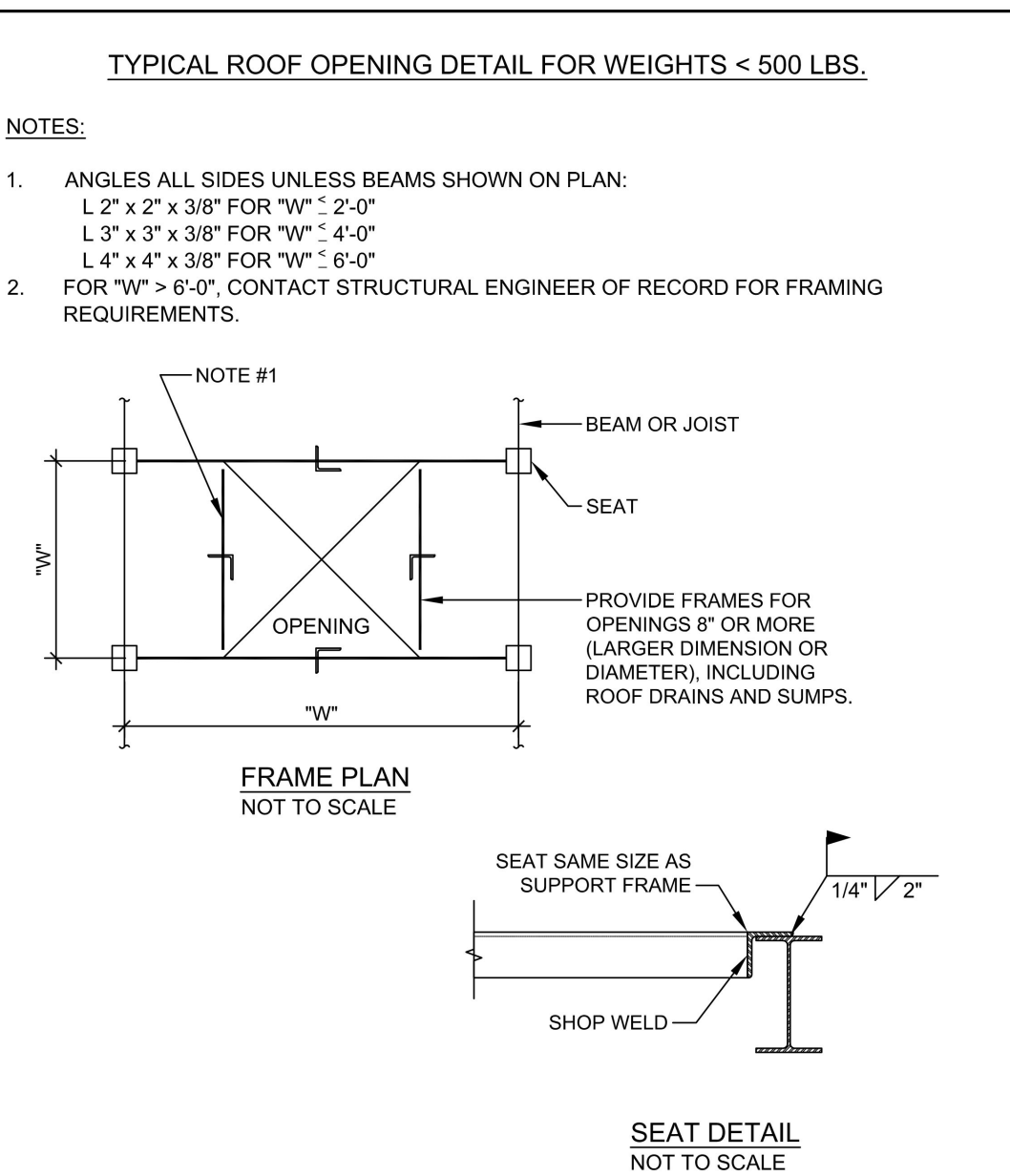
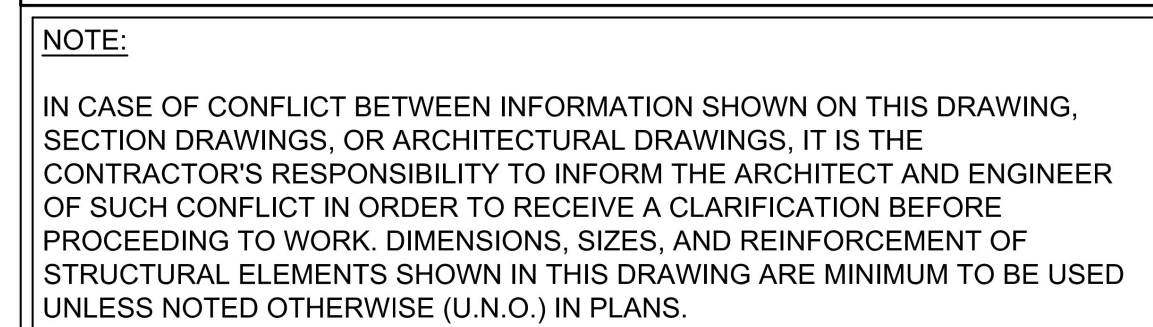
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TYPICAL DETAILS

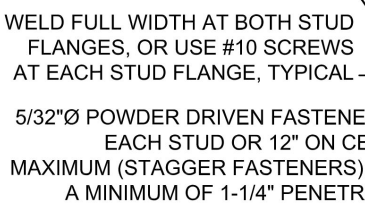
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TYPICAL STUD TO CONCRETE CONNECTION

NOTES

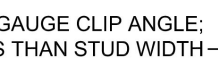
1. METAL STUD MEMBERS AND CONNECTIONS SHOWN ARE MINIMUMS. FINAL MEMBERS AND CONNECTIONS USED SHALL BE AS DESIGNED BY SPECIALTY CONTRACTOR ENGINEER. SEE LIGHT GAUGE METAL STUD FRAMING NOTES FOR MORE INFORMATION.
2. PROVIDE ONE ADDITIONAL FASTENER AT NO MORE THAN 3" FROM THE END OF THE TRACK. TERMINATE TRACKS AT MID-POINT BETWEEN STUDS SPACING SPLICE BY SCREWING A STUD SECTION BETWEEN VERTICAL STUDS TO THE RUNNER TRACK USING #10 SCREWS.



TYPICAL STUD TO CONCRETE CONNECTION
NOT TO SCALE

NOTES

- 
- "CSJ" STUD



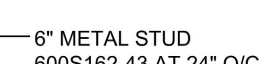
METAL STUD LATERAL BRACING - WELD ATTACHMENT
FOR MEMBERS LARGER THAN 3-1/2" INCHES
NOT TO SCALE



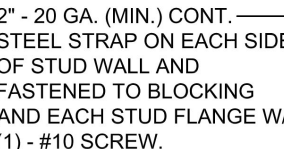
METAL STUD LATERAL BRACING - SCREW ATTACHMENT
FOR MEMBERS LARGER THAN 3-1/2" INCHES
NOT TO SCALE



METAL STUD LATERAL BRACING - WELD ATTACHMENT FOR MEMBERS
3-1/2" INCHES OR SMALLER IN 16 GAUGE OR HEAVIER THICKNESS
 NOT TO SCALE



METAL STUD LATERAL BRACING DETAIL
NOT TO SCALE



BLOCKING AND STRAPPING METAL STUD LATERAL BRACING DETAIL
NOT TO SCALE

NOTE

IN CASE OF CONFLICT BETWEEN INFORMATION SHOWN ON THIS DRAWING, SECTION DRAWINGS, OR ARCHITECTURAL DRAWINGS, IT IS THE CONTRACTOR'S RESPONSIBILITY TO INFORM THE ARCHITECT AND ENGINEER OF SUCH CONFLICT IN ORDER TO RECEIVE A CLARIFICATION BEFORE PROCEEDING TO WORK. DIMENSIONS, SIZES, AND REINFORCEMENT OF STRUCTURAL ELEMENTS SHOWN IN THIS DRAWING ARE MINIMUM TO BE USED UNLESS NOTED OTHERWISE (U.N.O.) IN PLANS.

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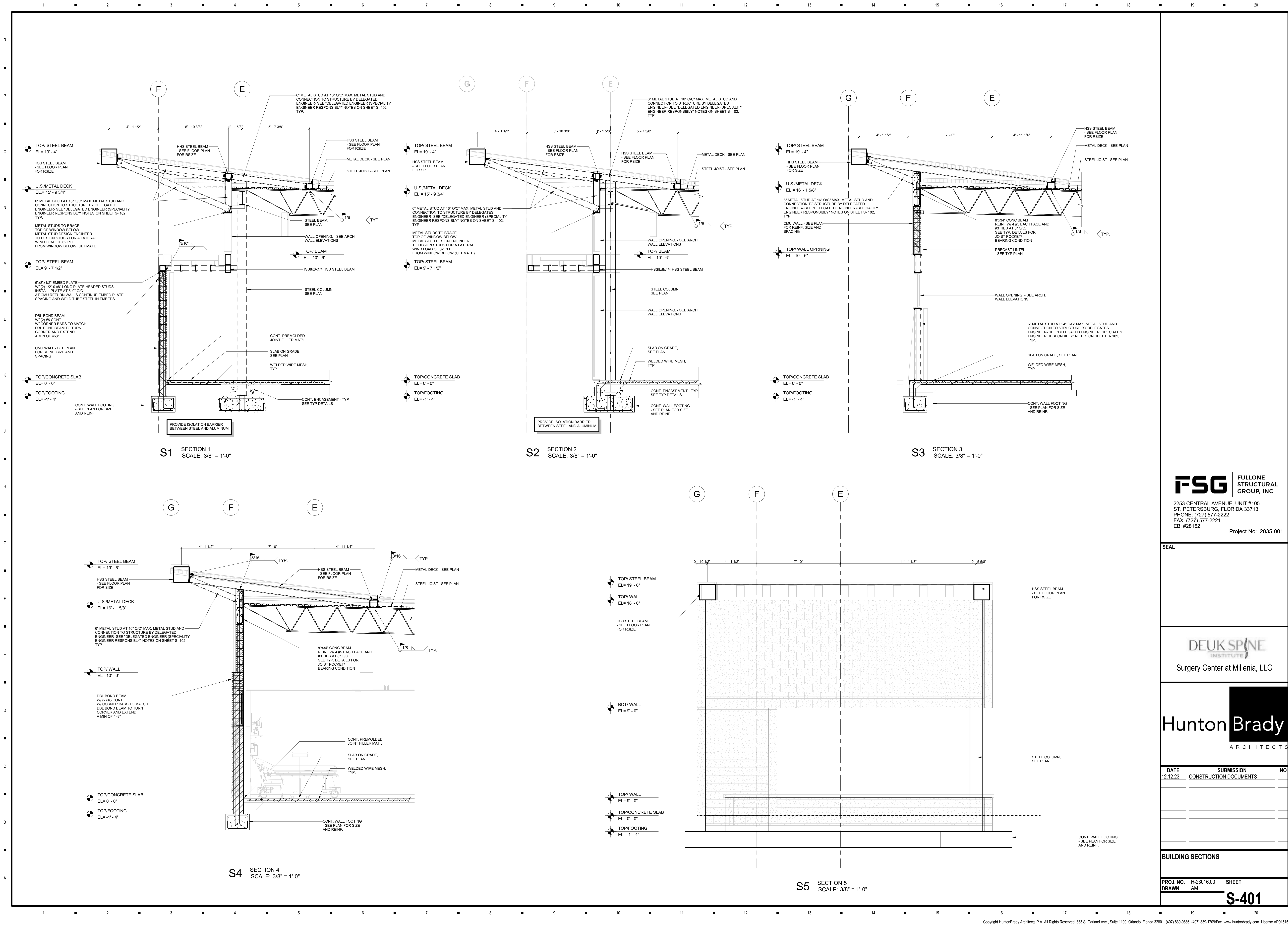
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TYPICAL DETAILS

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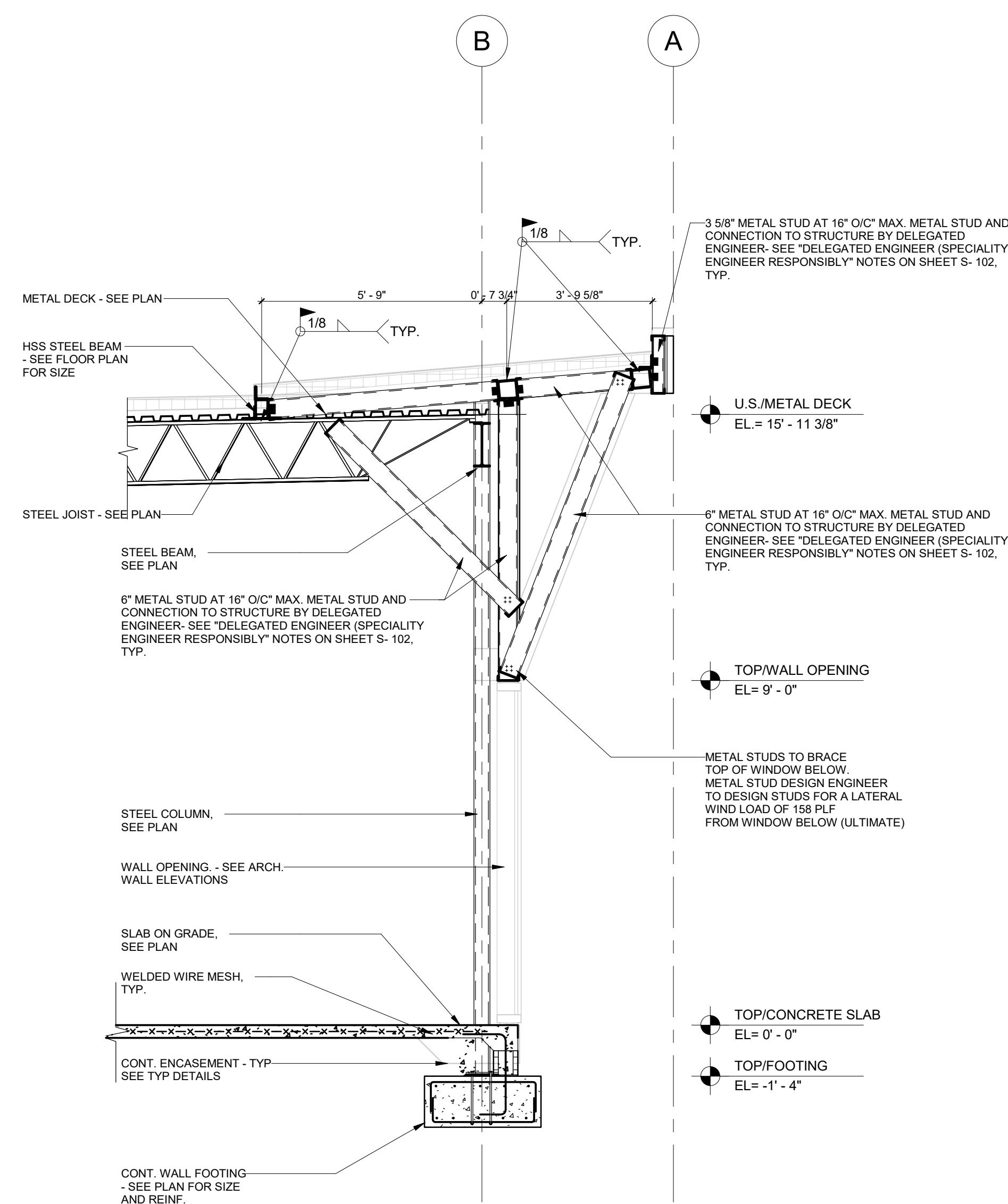
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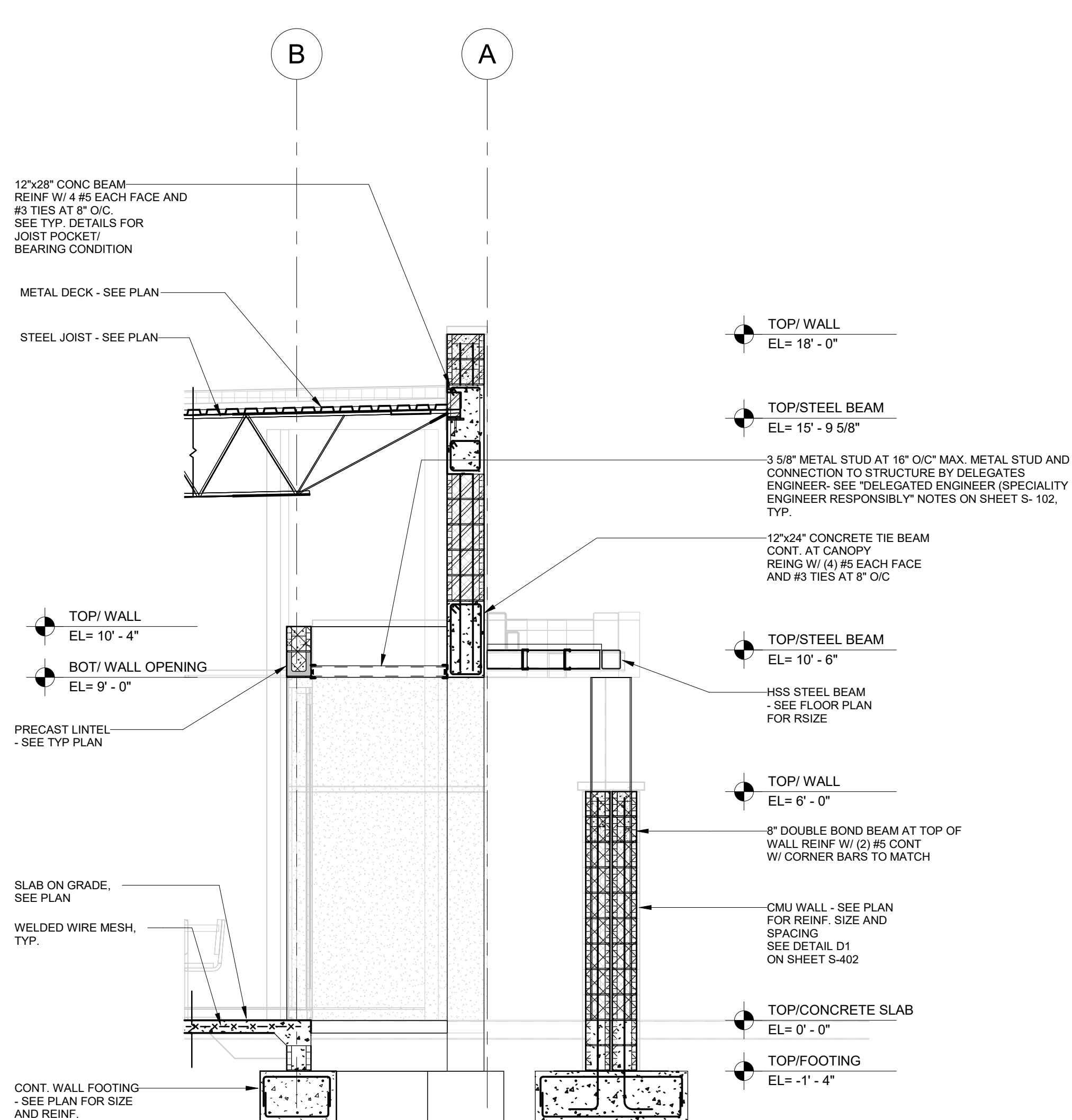
BUILDING SECTIONS

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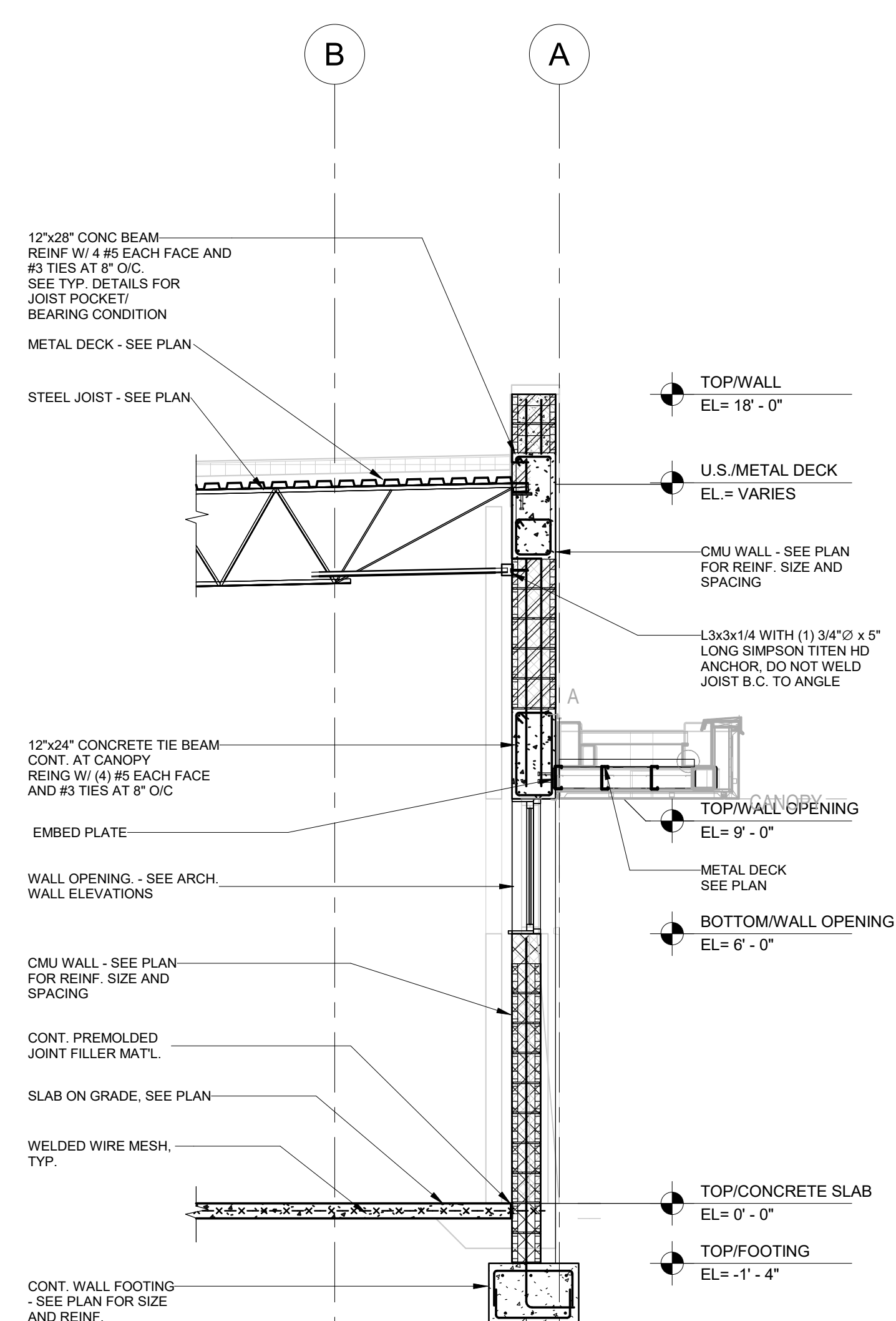
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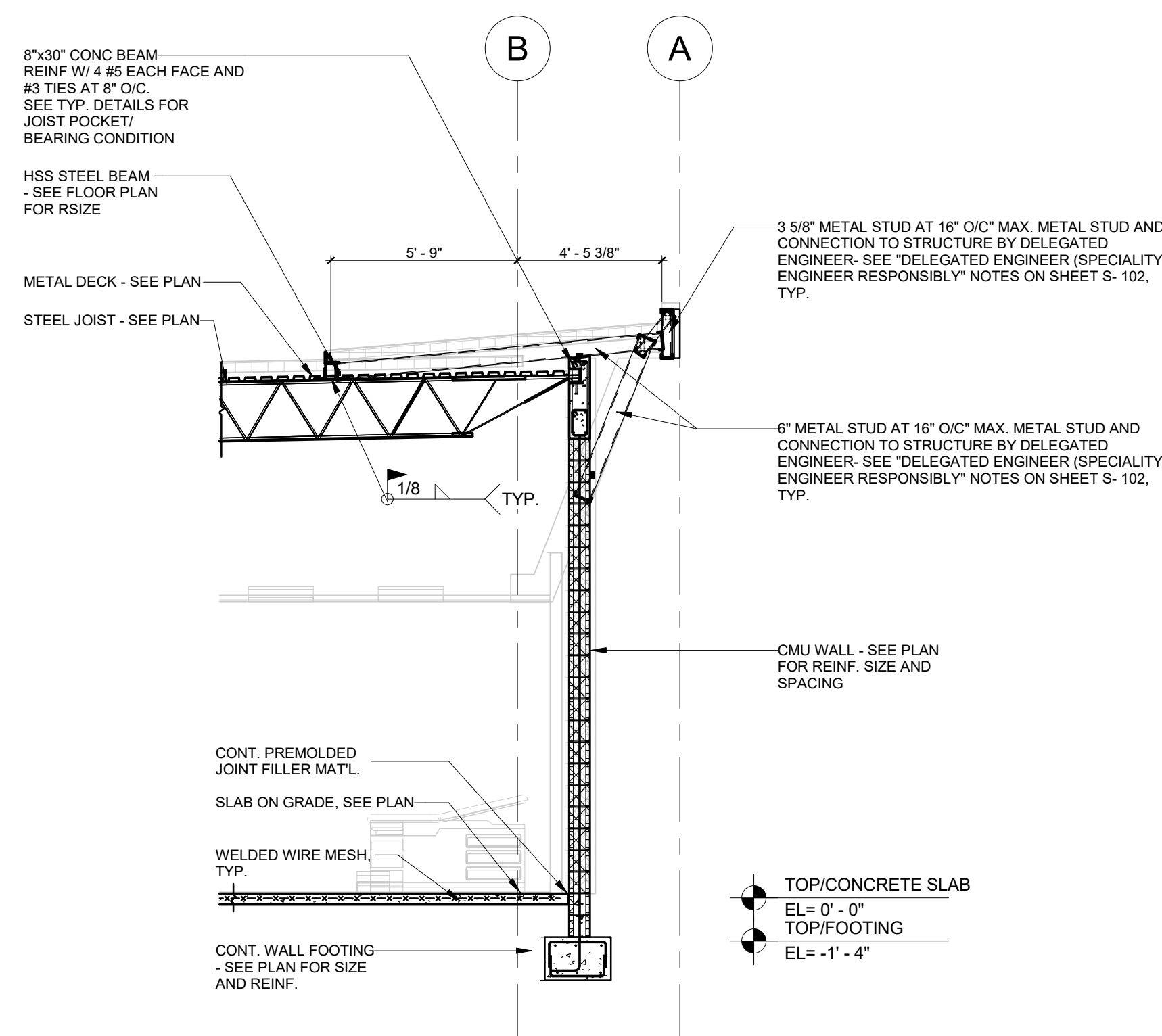
S6 SECTION 6
SCALE: 3/8" = 1'-0"



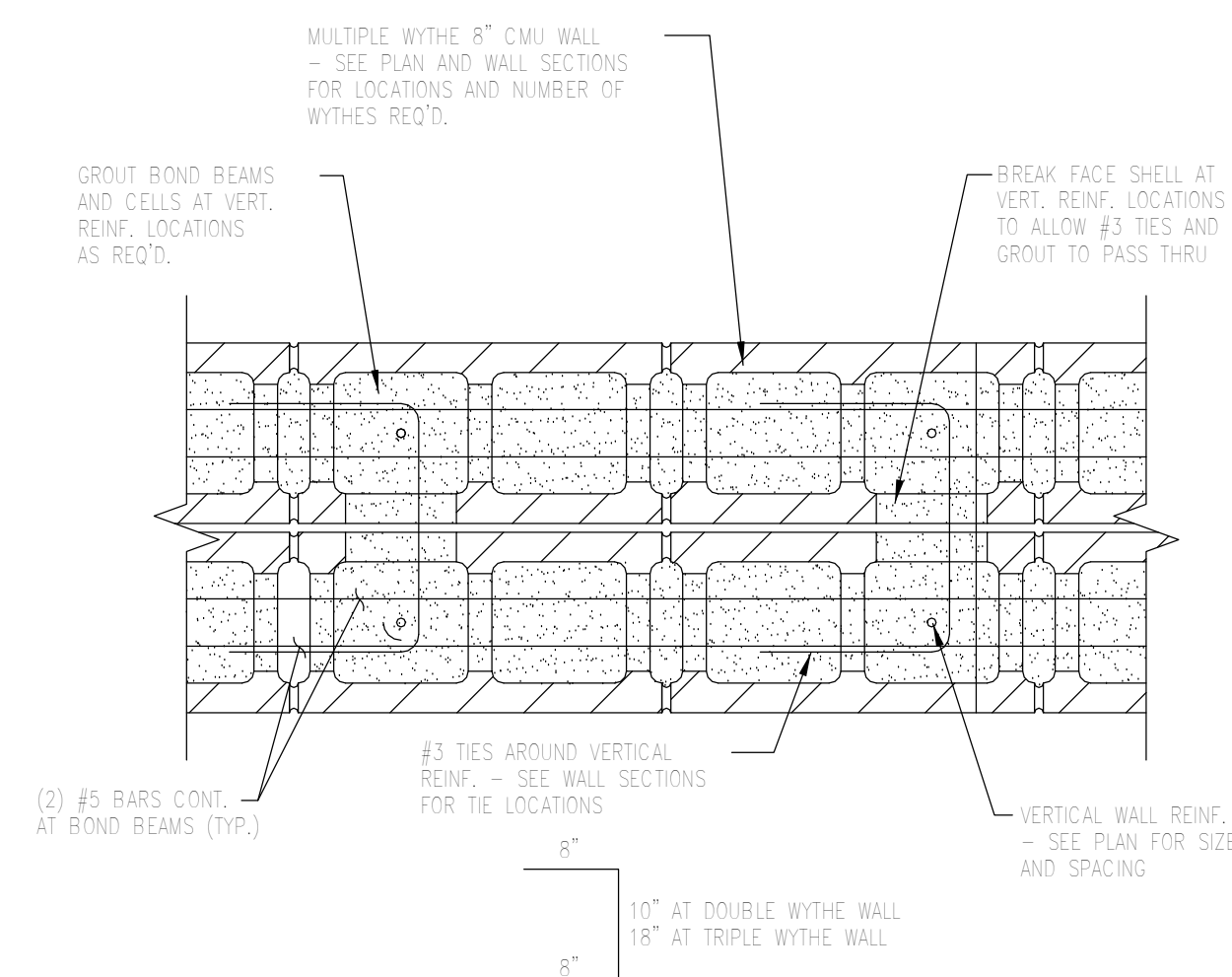
S7 SECTION 7
SCALE: 3/8" = 1'-0"



S8 SECTION 1
SCALE: 3/8" = 1'-0"



S9 SECTION 9
SCALE: 1/4" = 1'-0"



D1 Multiple Wythe CMU Wall Detail

SCALE: 1 1/2" = 1'-0"

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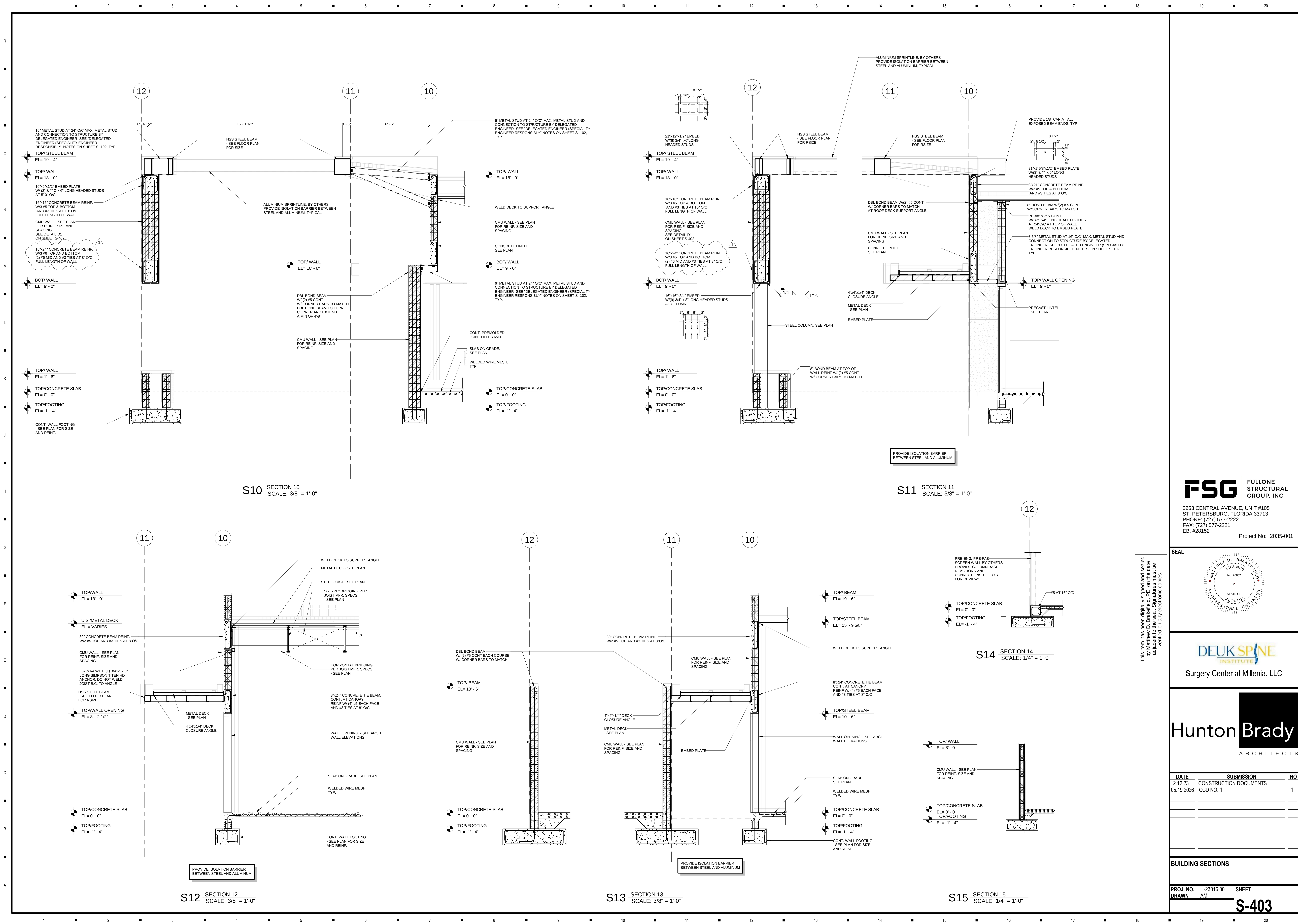
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BUILDING SECTIONS

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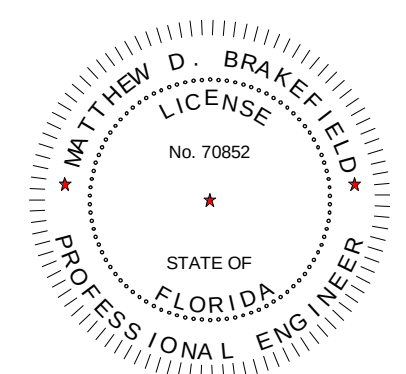
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BUILDING SECTIONS

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