

GENERAL NOTES:

(THESE SPECIFICATIONS ARE IN ADDITION TO AND DO NOT EXCLUDE ANY FOUND IN THE GENERAL SPECIFICATIONS FOR THE PROJECT)

1. CONTRACTOR SHALL BE RESPONSIBLE FOR BRACING AND SHORING OF STRUCTURE DURING CONSTRUCTION. FOUNDATION WALLS WHICH ARE TIED TO SLABS OR FLOORFLOOR FRAMING SHALL BE BRACED AGAINST BACKFILL MOVEMENT UNTIL SLABFRAMING (WITH DECK ATTACHMENT) IS COMPLETED. CONTRACTOR SHALL PROVIDE ALL TEMPORARY SAFETY ENCLOSURES TO PROTECT ALL PERSONNEL INVOLVED IN PROJECT.

2. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR INITIATING, MAINTAINING AND SUPERVISING ALL SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK. KEYSTONE STRUCTURAL ENGINEERING, P.C. IS NOT RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION OR FOR RELATED SAFETY PRECAUTIONS AND PROGRAMS.

3. SHOP DRAWINGS AND SUBMITTALS:
A. CONTRACTOR SHALL FURNISH COMPLETE AND DETAILED SHOP DRAWINGS. ALL SHOP DRAWINGS SHALL BE PREPARED UNDER THE DIRECT SUPERVISION OF A PROFESSIONAL ENGINEER REGISTERED IN THE APPLICABLE STATE.

B. CONTRACTOR SHALL REVIEW AND VERIFY ALL DIMENSIONS AND STAMP ALL SHOP DRAWINGS PRIOR TO SUBMITTING DRAWINGS TO ARCHENG. FOR REVIEW. CONTRACTOR IS RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS SUCH AS, MEMBER SIZES, DETAILS AND DIMENSIONS SPECIFIED IN THE CONSTRUCTION DOCUMENTS.

C. SHOP DRAWINGS SHALL SHOW MEMBERS SIZES, LOCATION, TYPE AND EXTENT OF ALL MEMBERS, BOLTS AND WELDS.

D. CONTRACTOR SHALL HAVE AN APPROVED SHOP DRAWINGS AND PROOF OF WELDER CERTIFICATION AT JOB SITE AT ALL TIMES.

E. PROVIDE SETTING DRAWINGS, TEMPLATES AND DIRECTIONS FOR INSTALLATION OF ANCHOR BOLTS AND OTHER ANCHORAGES TO BE INSTALLED BY OTHERS.

F. COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

4. DESIGN LOADS:
THE BUILDING STRUCTURE DESCRIBED IN THESE PLANS SHALL BE CONSTRUCTED IN COMPLIANCE WITH THE 2020 FLORIDA BUILDING CODE AND ALL AMENDMENTS IN CONJUNCTION WITH ASCE 7-16.

A. GRAVITY LOADS
DEAD LOADS:
ROOF: 20 PSF
FLOOR: 60 PSF

LIVE LOADS:
ROOF: 20 PSF
FLOOR: 125 PSF
STAIRS/CORRIDOR: 100 PSF

B. SNOW LOADS:
GROUND SNOW LOAD (Pg): 0.0 PSF
BALANCED ROOF SNOW LOAD (PH-RAIN): 0.0 PSF
SNOW EXPOSURE FACTOR (Ce): 1.0
SNOW IMPORTANCE FACTOR (Is): 1.0
THERMAL FACTOR (Ct): 1.0

C. WIND LOADS:
BASIC WIND SPEED(3 SEC. GUST): 160 MPH (ULTIMATE DESIGN WIND SPEED)
RISK CATEGORY: I
EXPOSURE CATEGORY: B
INTERNAL PRESSURE (GCp): +/-0.18

REFER TO THE COMPONENT & CLADDING WIND LOAD SCHEDULE ON S0.1 & ASCE-7-16

D. SEISMIC DESIGN CRITERIA:
SEISMIC IMPORTANCE FACTOR (Ie): 1.0
RISK CATEGORY: II

MAPPED SPECTRAL RESPONSE ACCELERATIONS:
Ss: 0.0592
SITE CLASS: D
SDS: 0.061g
SDI: 0.048g
SITE COEFFICIENT
Fa = 1.6
Fv = 2.4

E. SEISMIC DESIGN CATEGORY: A
BASIC SEISMIC FORCE RESISTING SYSTEM: LIGHT FRAMED WALLS SHEATHED WITH STEEL SHEETS
SEISMIC RESPONSE COEFFICIENT (Cs): 0.010
RESPONSE MODIFICATION FACTORS (R): 6.5
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

FOUNDATION NOTES:

1. SEE ARCHITECTURAL AND CIVIL DRAWINGS FOR BUILDING LOCATION AND ORIENTATION. COORDINATE ALL DIMENSIONS WITH ARCH. DRAWINGS. DO NOT SCALE DRAWING

3. SEE ARCHITECTURAL DRAWINGS FOR SIDE WALK EXTENTS, PLANTER, AND PAVEL LOCATIONS AND DETAILS

4. COORDINATE FINISHED FLOOR ELEVATIONS (F.F.E.) WITH ARCH. AND CIVIL DRAWINGS.

5. FINISHED FLOOR ELEVATION = 31.0' (SEE CIVIL)

6. ALL FOOTINGS HAVE BEEN DESIGNED BASED UPON A SOIL BEARING PRESSURE OF 3000 PSF. FOLLOW ALL RECOMMENDATIONS OUTLINED IN THE GEOTECHNICAL ENGINEERING REPORT BY PARTNER ENGINEERING AND SCIENCE, INC., PARTNER PROJECT NO. 22-390207, D. DATED 12-09-2022.

7. ALL FOUNDATION EXCAVATIONS SHALL BE EVALUATED BY THE GEOTECHNICAL ENGINEER OR TESTING AGENCY PRIOR TO POURING ANY FOUNDATION CONCRETE. CONTACT STRUCTURAL ENGINEER IF SOIL BEARING PRESSURE IS LESS THAN 3000 PSF.

8. ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 2' - 0" BELOW F.F.E. AND A MINIMUM OF 1' - 0" BELOW ADJACENT EXTERIOR FINISH GRADE (TYP. UNO)

9. TOP OF INTERIOR FOOTINGS SHALL BE A MINIMUM OF 0 - 8" BELOW F.F.E. (UNO)

10. PRIOR TO COMMENCING ANY FOUNDATION WORK, COORDINATE WORK WITH ANY EXISTING OR NEW UTILITIES. LOWER FOUNDATION AS REQUIRED TO AVOID INTERFERENCE WITH UTILITIES.

11.  INDICATES FOOTING STEP. SEE PLANS

SPECIALTY ENGINEERED PRODUCTS:

1. THE GENERAL CONTRACTOR IS RESPONSIBLE TO COORDINATE THE PROPER SUBMISSION OF SPECIALTY ENGINEERED SHOP DRAWINGS WHICH SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ASSURE THAT THE SPECIALTY ENGINEERED SHOP DRAWINGS ARE SUBMITTED IN A TIMELY MANNER SO AS TO ALLOW REVIEWS AND RESUBMISSIONS AS REQUIRED. ALL SPECIALTY ENGINEERED PRODUCTS SHALL BE DESIGNED FOR THE APPROPRIATE GRAVITY LOADS AND WIND LOADS INCLUDING UPLIFT AND LATERAL LOADS. INTERIOR SPECIALTY PRODUCTS SHALL BE DESIGNED FOR LATERAL LOADS TO ASSURE STABILITY. SPECIALTY ENGINEERED PRODUCTS SHALL BE, BUT ARE NOT LIMITED TO, THE FOLLOWING:

A. LIGHT GAUGE METAL FRAMING INCLUDING, BUT NOT LIMITED TO, SOFFITS, CLADDING, CEILINGS, ETC.

B. MISCELLANEOUS METALS INCLUDING STEEL STAIRS, MECHANICAL EQUIPMENT SUPPORTS, FRAMES THAT SUPPORT MACHINES, PIPES OR OTHER STRUCTURAL METAL USED FOR SUPPORT OF MECHANICAL SYSTEMS.

C. MISCELLANEOUS HANGARS, METAL FRAMES, LADDERS, RIGGING, HANGING WALLS, METAL RAILINGS, GLAZING FRAMES, CLADDING SUCH AS STONE, PRECAST, ALUMINUM METAL PANELS, CABLE BARRIER SYSTEMS, ETC. OR ANY OTHER MISCELLANEOUS PRODUCT REQUIRED BY THE ARCHITECTURAL OR MECHANICAL CONSTRUCTION DOCUMENTS.

CONCRETE SLAB ON GRADE NOTES:

1. FLOOR SLABS ON GRADE SHALL BE 7" THICK 3000 PSI CONCRETE, REINFORCED WITH #4's @ 18" o.c. EACH WAY, LOCATED 2" CLEAR FROM BOTTOM OF SLAB (LOWEST BAR PERPENDICULAR TO LOAD BEARING WALLS). SEE CIVIL PLANS FOR FINISHED FLOOR ELEVATIONS. COORDINATE VAPOR BARRIER REQUIREMENTS WITH ARCHITECTURAL AND GEOTECHNICAL ENGINEER REQUIREMENTS. PROVIDE 15 MIL (MIN) POLYETHYLENE VAPOR BARRIER WITH JOINTS LAPPED 6" AND TAPPED UNDER ALL INTERIOR SLABS. REFER TO GEOTECHNICAL ENGINEER FOR BELOW SLAB ON GRADE, SUBGRADE PREPARATION REQUIREMENTS.

2. CONTROL/CONSTRUCTION JOINTS SHALL BE LOCATED SUCH THAT NO AREA EXCEEDS 441 SQUARE FEET NOR SHALL THE LENGTH EXCEED 1.5 TIMES THE WIDTH. CONSTRUCTION JOINTS MAY BE SUBSTITUTED FOR CONTROL JOINTS. METAL "KEYHOLD" JOINTS SHALL BE USED AT ALL CONSTRUCTION JOINTS. LOCATE ALL CONTROL JOINT EQUAL DISTANCE BETWEEN LOAD BEARING WALL AND 5'-0" MIN FROM THE END OF LOAD BEARING WALLS (INCLUDING OPENINGS ETC.)

3. CONDUITS AND PIPES EMBEDDED IN SLABS SHALL NOT BE LARGER IN OUTSIDE DIMENSION THAN ONE-THIRD THE OVERALL THICKNESS OF THE SLAB. SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS OR WIDTHS ON CENTER. A MINIMUM SLAB THICKNESS OF 41/2" MUST BE MAINTAINED OVER THE EMBEDDED CONDUITS OR PIPES.

4. THICKEN SLAB AT LOCATIONS INDICATED ON FOUNDATION PLAN SEE 10/S3.1A

STEEL NOTES:

1. STRUCTURAL STEEL:
A. SHALL CONFORM TO THE LATEST STANDARDS OF ASTM:
WIDE FLANGE BEAMS: A992
MISC. STRUCTURAL STEEL SHAPES, PLATES AND BARS: A36
HOLLOW STRUCTURAL STEEL SECTIONS (ROUND AND RECTANGULAR): A500 GRADE B
STRUCTURAL STEEL PIPE: A53 GRADE B

B. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERRECTED IN ACCORDANCE WITH THE STANDARDS AND SPECIFICATIONS OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC 360-05) USING ALLOWABLE STRESS DESIGN

C. PROVIDE 1" (MINIMUM) 5000 PSI 28-DAY STRENGTH NON-SHRINK GROUT PER ASTM C 1107 UNDER ALL BASE PLATES

D. SHOP OR FIELD SPLICES BETWEEN SUPPORTS THAT ARE NOT REQUIRED BY DESIGN WILL NOT BE ALLOWED. ANY MEMBERS CONTAINING SUCH SPLICES FOUND IN THE FIELD SHALL BE REMOVED AND REPLACED WITH UNSPICED MEMBERS AT THE FABRICATOR'S EXPENSE.

2. STEEL CONNECTIONS:
A. WHERE BEAM REACTIONS OR DETAILS ARE NOT SHOWN IN THE CONSTRUCTION DOCUMENTS, CONNECTIONS SHALL BE DESIGNED FOR ONE-HALF THE MAXIMUM (SIMPLE SPAN) UNIFORM LOAD WHICH THE MEMBER WILL SUPPORT FOR THE SPAN SHOWN ON THE DRAWINGS.

B. BOLTS SHALL BE HIGH STRENGTH A-325 BOLTS OF SAME SIZE AND NUMBER AS SHOWN ON DRAWINGS. CONNECTIONS SHALL CONFORM TO THE SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A-325 OR A-490 BOLTS. CONNECTIONS ARE BEARING TYPE.

C. BOLTS SHALL BE TIGHTENED TO THE SNUG TIGHT CONDITION UNLESS OTHERWISE NOTED ON THE DRAWINGS.

3. WELDS:
A. MINIMUM SIZE OF WELD IS 3/16" AND (E70XX) UNLESS NOTED OTHERWISE.

B. ALL WELDING SHALL CONFORM TO THE LATEST "STRUCTURAL WELDING CODE" BY THE AMERICAN WELDING SOCIETY. ALL WORK SHALL BE PERFORMED BY CERTIFIED WELDERS EXPERIENCED IN THE TYPE OF CONSTRUCTION INVOLVED. PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE.

4. PROVIDE ALL SUPPORTING STEEL NOT INDICATED ON PLAN AS REQUIRED FOR THE INSTALLATION OF MECHANICAL EQUIPMENT AND MATERIALS, INCLUDING ANGLES, CHANNELS, BEAMS, HANGERS, ETC. DO NOT SUPPORT EQUIPMENT OR PIPING FROM METAL DECKING.

5. STEEL STAIRS:
ENGINEERED CONCRETE FILLED STEEL STAIR SYSTEM AND ALL CONNECTIONS OF THE SAME TO THIS STRUCTURE SHALL BE DESIGNED BY AN ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED. SUBMIT SHOP DRAWINGS BEARING THE SEAL AND THE SIGNATURE OF THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. THE CONFIGURATION OF THE STEEL STAIR SYSTEM SHALL BE SHOWN ON THE ARCHITECTURAL DRAWINGS. STEEL STAIR SYSTEM AND ALL CONNECTIONS SHALL BE DESIGNED FOR ALL APPLICABLE LOADS AS INDICATED ON THE PLANS AND IN ALL APPLICABLE BUILDING CODES. THE LOADS SHALL BE CLEARLY INDICATED ON ALL SHOP DRAWINGS. SHOP DRAWINGS SHALL SHOW AND SPECIFY ALL CONNECTIONS UTILIZED WITHIN THE STEEL STAIR SYSTEM AS WELL AS CONNECTIONS TO AND LOADS IMPOSED UPON THE STRUCTURAL SYSTEM SHOWN ON THESE PLANS.

COLD FORMED METAL FRAMING NOTES:

1. METAL FRAMING:
NON-STANDARD SSMA METAL FRAMING MEMBER (CEE (C), TRACK, ZEE (Z) & HAT CHANNELS) DIMENSIONS, CONFIGURATIONS AND SECTION PROPERTIES SHALL COMPLY WITH THE 2007 AISI. COLD-FORMED STEEL DESIGN SPECIFICATIONS

2. USE STEEL TO FORM ZINC COATED (GALVANIZED) NON-STANDARD SSMA METAL FRAMING MEMBERS TO MEET THE PHYSICAL AND CHEMICAL PROPERTIES OF ASTM A653, GRADE 55 (FY=55 KSI), AND G60 COATING DESIGNATION AS DESCRIBED IN ASTM A 924

3. USE STEEL TO FORM PRIMED NON-STANDARD SSMA METAL FRAMING MEMBERS TO MEET THE PHYSICAL AND CHEMICAL PROPERTIES OF ASTM A1011, GRADE 55 (FY=55 KSI)

4. SCREWS SHALL BE SELF DRILLING TYPE SCREWS OF THE OF THE MINIMUM DIAMETER INDICATED ON PLANS AND DETAILS WITH A MIN PENETRATION THROUGH JOINED MATERIAL OF NOT LESS THAN (3) EXPOSED THREADS.

5. SCREW SPACING AND EDGE DISTANCES SHALL BE (3) SCREW DIAMETERS WITH A MIN 1/2" DIMENSION.

6. SCREW SHALL HAVE A PROTECTIVE COATING OF NOT LESS THAN THE JOINED MEMBER PROTECTIVE COATING.

7. ALL BOLTS FOR CONNECTIONS SHALL CONFORM TO ASTM A325

8. DO NOT ALTER, CUT OR REMOVE MEMBERS OR CONNECTIONS OF MEMBERS WITH PRIOR WRITTEN APPROVAL OF ENGINEER.

9. FIELD TOUCH UP ALL FIELD WELDS AND ABRASIONS OF GALVANIZED MATERIALS WITH ZINC RICH PAINT IN ACCORDANCE WITH ASTM A 780, ANNEX A2. TOUCH UP WORK SHALL BE COMPLETED PRIOR TO ATTACHMENT OF THE WORK OF ANY OTHER SECTIONS TO THE LIGHT GAUGE STEEL FRAMING

10. STANDARD SSMA METAL STUD AND TRACK FRAMING (NON-LOAD BEARING IN-FILL FRAMING) (OPTIONAL)

A. METAL STUD STRENGTH CRITERIA:
1. 18 MIL TO 43 MIL - 33 KSI MIN. YIELD STRESS
2. 54 MIL TO 97 MIL - 50 KSI MIN. YIELD STRESS
3. RUNNER TRACK - 33 KSI MIN. YIELD STRESS (UNO)
DO NOT USE STUDS LESS THAN 43 MIL FOR ANY EXTERIOR WALL USED TO BACK-UP BRICK VENEER.

B. ATTACH METAL FRAMING TO PRIMARY STRUCTURE WITH A DEFLECTION TRACK OR A STEEL NETWORK INC. VERTICAL DEFLECTION CAPABLE OF ALLOWING 1" OF VERTICAL DEFLECTION.

11. METAL STUD FASTENERS:
A. SCREW CONNECTIONS
USE #10-16 KWIK-FLEX SELF-DRILLING SCREWS OR APPROVED EQUAL UNLESS NOTED OTHERWISE.
MIN. EMBEDMENT = 1 1/4" MIN. EDGE DISTANCE = 2"
MIN. SPACING = 3" MAX. SPACING = 12"
B. POWER DRIVEN FASTENERS:

1. FASTENING TO CONCRETE:
USE 0.145" DIA. DOME HEAD NAIL TYPE "X-ZP" BY HILTI OR APPROVED EQUAL UNLESS NOTED OTHERWISE.
MIN. EMBEDMENT = 1 1/4" MIN. EDGE DISTANCE = 2"
MIN. SPACING = 3" MAX. SPACING = 12"

2. FASTENING TO STEEL:
USE 0.145" DIA. DOME HEAD KNURLED SHANK FASTENER TYPE "X-EDNP" BY HILTI OR APPROVED EQUAL UNLESS NOTED OTHERWISE.
MIN. EMBEDMENT = FULL PENETRATION
MIN. EDGE DISTANCE = 1 1/2" MIN. SPACING = 1 1/2" MAX. SPACING 12"

ROOF FRAMING NOTES:

1. SEE COMPONENT & CLADDING TABLE S0.1 FOR NET UPLIFT (LRFD) (UNO). NO 1/3 STRESS INCREASE ALLOWED.

2. 1.5" TYPE B ROOF DECK (ALL CANOPIES UNO.O.)

36/5 1 1/2" TYPE B X222GA. (02957) (3 SPAN MIN.)

ATTACH CANOPY DECK TO SUPPORTS WITH (#12 SCREWS) IN A 36/4 PATTERN WITH 6 SIDELAP FASTENERS (#10 SCREWS) PER SPAN. ATTACHMENT AT PERIMETER OF DECK SHALL BE EQUAL TO ATTACHMENT AT DECK SHEET LAPS AND DECK SHEET ENDS. ANY PARTIAL OR SKEWED SHEETS SHALL BE ATTACHED AT EVERY FLUTE REGARDLESS OF ATTACHMENT PATTERN.

3. 3" TYPE N ROOF DECK. (MAIN ROOF)

24/8 3" TYPE N X222GA. (02957) (3 SPAN MIN.)

ATTACH ROOF DECK TO SUPPORTS WITH (#12 SCREWS) IN A 24/8 PATTERN WITH 7 SIDELAP FASTENERS (#10 SCREWS) PER SPAN. ATTACHMENT AT PERIMETER OF DECK SHALL BE EQUAL TO ATTACHMENT AT DECK SHEET LAPS AND DECK SHEET ENDS. ANY PARTIAL OR SKEWED SHEETS SHALL BE ATTACHED AT EVERY FLUTE REGARDLESS OF ATTACHMENT PATTERN.

4. ROOF SYSTEM IS DESIGNED FOR MECHANICAL EQUIPMENT SHOWN. NOTIFY ARCHITECT/ENGINEER IF ADDITIONAL EQUIPMENT REQUIRED OR IF HEAVIER UNITS ARE SUPPLIED.

5. COORDINATE THE EXACT LOCATION AND EXTENT OF ROOF HATCH OPENINGS WITH ARCH. DRAWINGS.

FLOOR FRAMING NOTES:

1. FLOOR SLAB SHALL BE 5 1/2" (TOTAL THICKNESS) WITH NORMAL WEIGHT CONCRETE (F=3000 PSI) ON 3" 20 GAUGE COMPOSITE METAL DECK (50 FY) (NEW MILLIUMIN 3.0 CD OR EQUAL). REINFORCE SLAB WITH MACRO SYNTHETIC FIBERS MEETING THE CRITERIA OF ASTM D7508 AT A DOSAGE RATE DETERMINED BY THE FIBER MANUFACTURER FOR THE APPLICATION, BUT NOT LESS THAN 4 LBS/CU YD PER SD.

2. ATTACHMENT OF FLOOR DECK TO SUPPORTS WITH (#12 TEK SCREWS) IN A 36/4 PATTERN WITH A SIDE LAPS PER SPAN (#10 SCREWS). ATTACHMENT AT PERIMETER OF DECK SHALL BE EQUAL TO ATTACHMENT AT DECK SIDE LAPS AND DECK SHEET ENDS. ANY PARTIAL OR SKEWED SHEETS SHALL BE ATTACHED AT EVERY FLUTE REGARDLESS OF ATTACHMENT PATTERN.

36/4 3" TYPE S 3CD 20GA. (03548) (3 SPAN MIN.) (G60 GALVANIZED.)

3. SUSPENDED CEILINGS, LIGHT FIXTURES, DUCTS AND OTHER PERMANENT SUSPENDED LOADS SHALL NOT BE SUPPORTED BY THE METAL DECKING.

4. PROVIDE 51/2"x4" LVL #4GA CONT. POURSTOUR ANGLE AT ALL DECK EDGES AND AROUND ALL OPENINGS IN FLOOR SLAB.

5. PROVIDE 3 1/2"x3 1/2"x1/4" ANGLE AT ALL DECK EDGES SUPPORTED ON CMU WALLS.

SPECIAL INSPECTIONS:

A. THE SPECIAL INSPECTOR SHALL BE ENGAGED BY THE OWNER. SPECIAL INSPECTOR SHALL BE FULLY QUALIFIED, APPROVED BY THE BUILDING OFFICIAL, REGISTERED BY APPLICABLE REGISTRATION BOARD IF REQUIRED BY THE LOCAL BUILDING OFFICIAL, AND SHALL BE ACCEPTABLE TO THE ARCHITECT.

B. THE SPECIAL INSPECTOR SHALL PROVIDE VERIFICATION OF CONSTRUCTION QUALITY CONTROL INSPECTIONS AND TESTING. THE SPECIAL INSPECTOR SHALL CERTIFY THAT ALL WORK REQUIREMENTS SPECIFICATION IS PERFORMED IN COMPLIANCE WITH ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS, BUILDING CODE REQUIREMENTS AND LOCAL BUILDING DEPARTMENT REQUIREMENTS.

C. SPECIAL INSPECTIONS ARE REQUIRED FOR THE ITEMS NOTED IN THE STATEMENT OF WORK TO MEET THE PHYSICAL AND CHEMICAL PROPERTIES OF ASTM A1011, GRADE 55 (FY=55 KSI).

D. FAILURE TO NOTIFY THE SPECIAL INSPECTOR PRIOR TO OBSCURING AN ITEM REQUIRING INSPECTION MAY RESULT IN THE CONTRACTOR REMOVING OTHER WORK TO ALLOW INSPECTION. THIS WORK WILL BE REMOVED AND REPLACED AT THE CONTRACTORS EXPENSE. FAILURE TO HAVE REQUIRED ITEMS INSPECTED IS REASON FOR REJECTION OF THE WORK.

E. PREMATURE NOTIFICATION FOR INSPECTIONS WILL RESULT IN ADDITIONAL INSPECTION WITH ALL EXPENSES AND FEES PAID FOR BY THE CONTRACTOR.

SHEAR WALL AND WALL PANEL NOTES

1. 26 GAUGE PBU WALL PANELS WHERE NOTED, ALL OTHER PANELS SHALL BE 29 GAUGE PBU WALL PANELS. ALL WALL PANELS SHALL BE GRADE 60

2. ATTACH ALL PBU PANEL ENDS TO SUPPORTS WITH #12 TEK SCREWS AT 6" o.c. AND TO INTERMEDIATE SUPPORTS AT 12" o.c.

3. SIDELAP ATTACHMENT FOR WALLS DESIGNATED AS SHEAR WALLS ON THE PLANS SHALL BE #12 TEK SCREWS AT 12" o.c. (20" o.c. FOR ALL OTHER WALL). FOR WALLS DESIGNATED AS SHEAR WALLS ON THE PLANS, FASTEN BOTTOM EDGE OF PANEL TO BOTTOM TRACK AND TOP EDGE OF PANEL TO THE CONT. ANGLE/TOP TRACK WITH #12x3/4" TEK SCREW PANEL FASTENERS @ 12" o.c. (20" o.c. FOR ALL OTHER WALL).

4. ALL PERIMETER PANELS (EXCEPT ROOF LEVEL) SHALL BE FULL HEIGHT 26 GAUGE PBU WALL PANELS WITH #12 TEK SCREW SIDELAP FASTENER AT 12" o.c. ROOF LEVEL PERIMETER PANELS SHALL BE FULL HEIGHT 29 GAUGE PBU WALL PANELS WITH #12 TEK SCREW SIDELAP FASTENER AT 12" o.c. FOR ALL GAUGES, FASTEN BOTTOM EDGE OF PANEL TO BOTTOM TRACK AND TOP EDGE OF PANEL THE CONT. ANGLE/TOP TRACK WITH #12x3/4" TEK SCREW PANEL FASTENERS @ 12" o.c.

5. ALL INTERIOR PANELS SHALL BE 8'-4" HIGH. COORDINATE EXACT PANEL HEIGHT WITH ARCHITECTURAL.

6. THERE SHALL BE A DOUBLE STUD AT EACH END OF EACH SHEAR WALL AT THE LEVEL OF THE SHEAR WALL AND ALL LEVELS BELOW WITH THE EXCEPTION OF SHEAR WALLS AT THE ROOF LEVEL. THE DOUBLE STUD SIZE, TYPE AND GAUGE SHALL BE EQUAL TO THE SHEAR WALL FRAMING MEMBERS AT EACH LEVEL.

7. SCREWS SHALL BE SELF DRILLING TYPE SCREWS WITH A MIN PENETRATION THROUGH JOINED MATERIAL OF NOT LESS THAN (3) EXPOSED THREADS.

8. SHEAR WALLS DESIGNATED ON A LEVEL ARE THE WALLS BELOW THAT LEVEL.

SW-1 29 GAUGE PBU PANEL ON ONE SIDE OF WALL TO 8'-4" MINIMUM WITH STRAP BRACES ABOVE TOP OF PANEL

SW-2 26 GAUGE PBU PANEL ON ONE SIDE OF WALL TO 8'-4" MINIMUM WITH STRAP BRACES ABOVE TOP OF PANEL

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REINFORCED MASONRY NOTES:

IMPORTANT:
SPECIAL INSPECTION IS REQUIRED FOR ALL MASONRY CONSTRUCTION.

1. ALL MASONRY WALLS SHALL HAVE CELLS REINFORCED AND DOWELED INTO FOUNDATION AS NOTED ON THE DRAWINGS. ALL MASONRY BELOW GRADE SHALL BE GROUTED SOLID.

2. REINFORCED CELLS WITH GROUT SHALL BE CONTINUOUS FROM FOUNDATION TO THE TOP OF THE WALL.

3. FILL REINFORCED CELLS WITH MECHANICALLY MIXED GROUT (2,500 PSI MIN.).

A. GROUT SHALL CONFORM TO ASTM C-476 (PROPORTION SPECIFICATION).
B. GROUT SHALL BE MIXED FOR AT LEAST 5 MINUTES IN MECHANICAL MIXER AND PLACED WITHIN 1 1/2 HOURS FROM THE INITIAL INTRODUCTION OF WATER AND PRIOR TO INITIAL SET.
C. BETWEEN GROUT POURS, A HORIZONTAL CONSTRUCTION JOINT SHALL BE FORMED BY STOPPING ALL CMU AT THE SAME ELEVATION AND WITH THE GROUT STOPPING A MINIMUM OF 1 1/2" BELOW A MORTAR JOINT. EXCEPT AT THE TOP OF THE WALL, WHERE BOND BEAMS OCCUR, THE GROUT POUR SHALL BE STOPPED A MINIMUM OF 1/2" BELOW THE TOP OF THE MASONRY.
D. CMU WALLS SHALL BE CONSTRUCTED USING LOW-LIFT GROUTING (6'-0" MAX. POUR HEIGHTS).
E. ALL EXTERIOR WALLS AND INTERIOR SHEAR WALLS ARE MARKED ON THE PLANS AS (SW). SEE SCHEDULE ON THIS SHEET FOR REINFORCEMENT.

4. ALL CONCRETE MASONRY SHALL HAVE A MINIMUM NET AREA COMPRESSIVE STRENGTH (F'm) OF 1500 PSI. (MSJC S1.4B.28 TABLE 2.)

5. MORTAR SHALL BE TYPE S-(ASTM 270 TABLE 1 MORTAR PROPORTION SPECIFICATION REQUIREMENTS).
A. THICKNESS OF BED JOINTS SHALL BE 3/8" EXCEPT THAT THE THICKNESS OF THE BED JOINT OF THE STARTING COURSE PLACED OVER FOUNDATIONS SHALL NOT BE LESS THAN 1/4" AND NOT MORE THAN 3/4".
B. MORTAR SHALL BE MIXED UNTIL MIXTURE IS UNIFORM THOUGHOUT.
C. UNUSED MORTAR SHALL BE DISCARDED WITHIN 2 1/2 HOURS AFTER INITIAL MIXING.

6. ALL MASONRY UNITS SHALL CONFORM TO ASTM C-90 AND HAVE A NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNITS EQUAL TO 1900 PSI (MSJC S1.4B.28 TABLE 1).
A. ALL WALLS SHALL BE CONSTRUCTED USING RUNNING BOND (U.N.O.)

7. REINFORCING STEEL SHALL CONFORM TO ASTM A615-GRADE 60. FOR WELDED REINFORCING USE ASTM A706-GRADE 60.

8. ALL MASONRY WALLS SHALL HAVE STANDARD HORIZONTAL (9 GA.) LADDER TYPE REINFORCING @ 16" o.c. JOINT REINFORCEMENT SHALL CONFORM TO ASTM A951.

9. COORDINATE REQUIRED TYPE, SIZE, GAUGE, AND SPACING OF STEEL ANCHORS FOR ATTACHMENT OF MASONRY VENEER WITH ARCH. DRAWINGS.

10. CONTROL JOINTS IN CONCRETE MASONRY SHALL HAVE A MAXIMUM SPACING OF 25'-0" o.c. (UNO). COORDINATE THE LOCATION OF BRICK EXPANSION JOINTS WITH ARCH. DWGS.

11. SLEEVE ALL PLUMBING OR FIRE PROTECTION PIPING THRU CMU WALLS.

12. REINFORCING BARS FOR VERTICAL FILLED CELLS SHALL BE LAPPED ACCORDING TO MASONRY REINFORCING LAP LENGTH SCHEDULE ON THIS SHEET.

13. COORDINATE EXACT LOCATION AND EXTENT OF ALL OPENINGS IN MASONRY WALLS WITH ARCH. DRAWINGS.

14. MINIMUM MASONRY COVER FOR REINFORCING STEEL:

A. MASONRY EXPOSED TO WEATHER OR EARTH:
2" FOR BARS LARGER THAN #5
1 1/2" FOR #5 AND SMALLER BARS
B. MASONRY NOT EXPOSED TO WEATHER OF EARTH: 1 1/2"

REINFORCED CONCRETE NOTES:

1. STRUCTURAL MEMBERS OF REINFORCED CONCRETE SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI318

2. ALL CONCRETE SHALL HAVE A SLUMP OF 4" +/- 1" (SUPER PLASTICIZER/WATER REDUCING ADMIXTURES MAY BE ADDED TO ANY MIX PER THE PROJECT SPECIFICATIONS AND THE APPROVED MANUFACTURERS SPECIFICATIONS TO INCREASE WORKABILITY AND/OR TO IMPROVE THE FLOW CHARACTERISTICS OF CONCRETE. KEYSTONE STRUCTURAL ENGINEERING, P.C. HAS NO CONTROL OVER OR PREDICTION OF THE IMPACT ON SLUMP OF ADDING SUCH SUPER PLASTICIZER/WATER REDUCING ADMIXTURES) AND A MINIMUM 28 DAY COMPRESSIVE STRENGTH:
A. FOOTINGS: 3000 PSI (0.50 MAXIMUM WATER/CEMENT RATIO)
B. WALLS: 3000 PSI (0.50 MAXIMUM WATER/CEMENT RATIO)
C. SLAB ON GRADE: 3000 PSI (540 LBS/CUBIC YARD MINIMUM CEMENTITIOUS MATERIAL & 0.50 MAXIMUM WATER/CEMENT RATIO)

3. LIMIT PERCENTAGE, BY WEIGHT, OF CEMENTITIOUS MATERIALS OTHER THAN PORTLAND CEMENT IN CONCRETE TO 25%.

4. AIR CONTENT: 6% (+/- 1.5%) AT POINT OF DELIVERY FOR 1" NOMINAL MAXIMUM AGGREGATE SIZE.

5. AIR CONTENT: DO NOT ALLOW AIR CONTENT OF TROWEL-FINISHED FLOORS TO EXCEED 3%.

6. ALL STEEL REINFORCEMENT SHALL BE ASTM 615-GRADE 60. ALL WELDED STEEL REINFORCEMENT SHALL BE ASTM A706-GRADE 60. WELDED WIRE REINFORCEMENT SHALL BE ASTM A185. ALL WELDED REINFORCEMENT SHALL BE IN ACCORDANCE WITH AWS D1.4.

7. MINIMUM CONCRETE COVER FOR REINFORCING STEEL: (UNO)

A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"

B. CONCRETE EXPOSED TO EARTH AND WEATHER:
#5 REBAR AND SMALLER: 1 1/2"
#6 REBAR AND LARGER: 2"

C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH.

SLABS, WALLS, AND JOISTS:
#14 OR #18 REBARS: 1 1/2"
#14 REBAR AND SMALLER: 3/4"

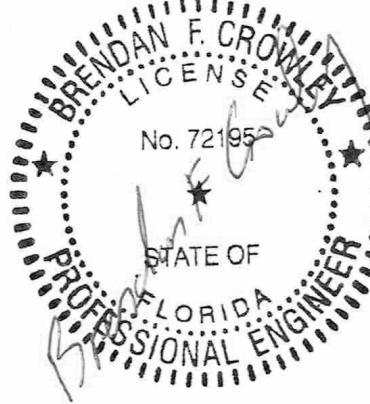
BEAMS AND COLUMNS: 1 1/2"

8. UNLESS NOTED OTHERWISE, CAST IN PLACE CONCRETE SHALL HAVE THE FOLLOWING STEEL ADDED AROUND ALL OPENINGS:
2-#5 LENGTH OF OPENING - 48" ALONG EACH SIDE OF OPENING AND TWO (2)-#5x5'-0" DIAGONALLY AT EACH CORNER.

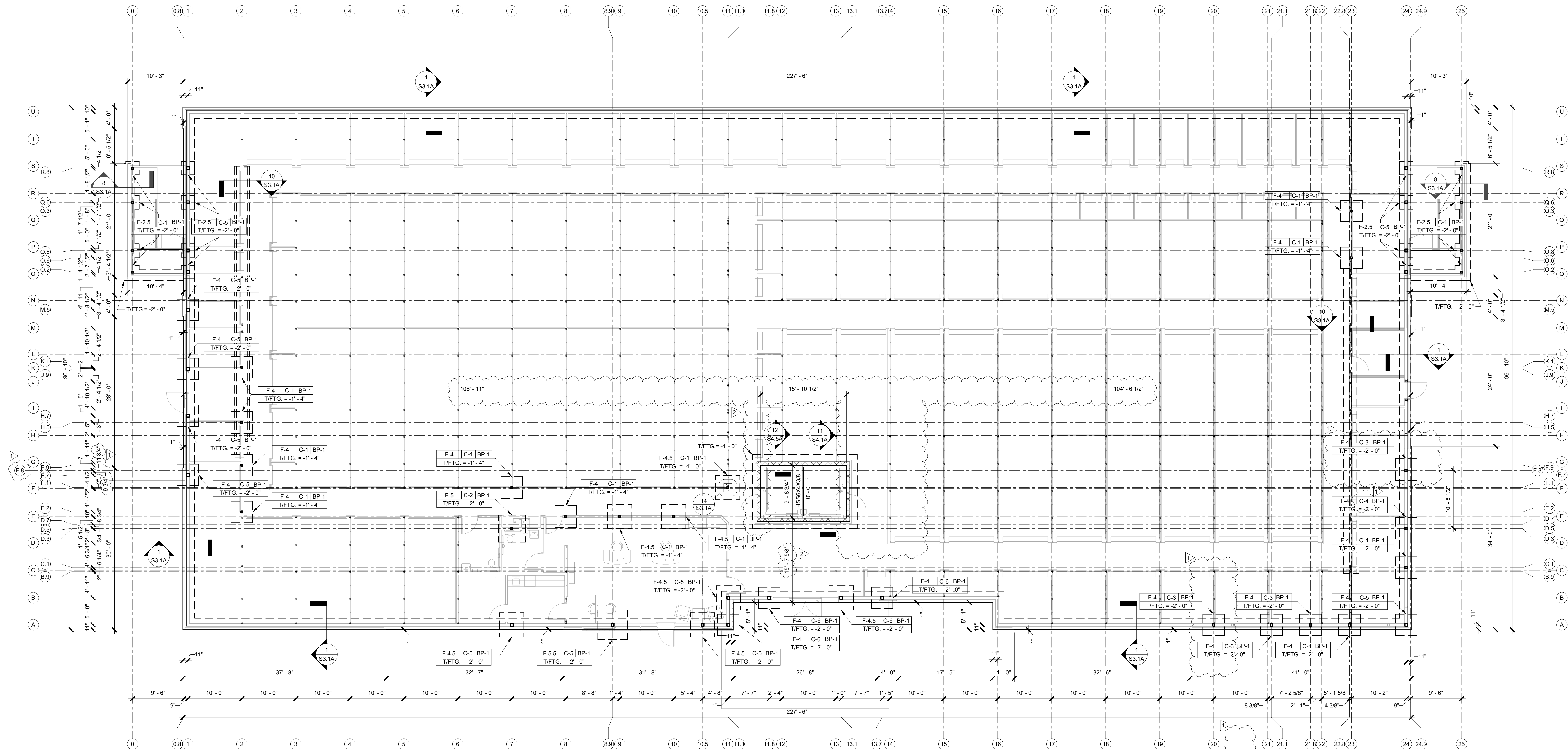
9. ALL CONCRETE COMPRESSIVE STRENGTH TESTS SHALL BE AVAILABLE AT JOB SITE.

10. ALL LAP SPLICE LENGTHS SHALL BE AS SHOWN IN TABLE 2 THIS SHEET

ULTIMATE DESIGN VALUES		COMPONENT AND CLADDING (MAIN ROOF)	
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Rev. No.	Date	Name
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3 Foundation Plan
1/8" = 1'-0"

FIRST FLOOR F.F.E. = 31.00' NGVD
SECOND FLOOR F.F.E. = 41.67' NGVD
THIRD FLOOR F.F.E. = 52.33' NGVD
TOP OF FOOTINGS REFERENCE = F.F.E. = 0' - 0"
(FIRST FLOOR - REFERENCE ELEVATION)

DO NOT LOCATE FLOOR SLAB ON GRADE CONTROL JOINTS
IN THE CORRIDORS, PARALLEL TO CORRIDOR DIRECTION
(TRANSVERSE CONTROL JOINTS ACCEPTABLE)
COORDINATE CONTROL JOINT LAYOUT WITH CORRIDOR
LAYOUT

Structural Column Schedule		
Type Mark	Type	Type Comments
C-1	HSS4X4X1/4	
C-2	HSS4X4X3/8	
C-3	HSS6X4X1/4	
C-4	HSS6X4X5/16	
C-5	HSS6X6X1/4	
C-6	HSS6X6X1/2	

Structural Foundation Schedule			
Type Mark	Footings Dimensions	Reinforcing	Remarks
F-2.5	2'-6"x2'-6"x1'-0"	(3) #5 EACH WAY	
F-4	4'-0"x4'-0"x1'-0"	(4) #5 EACH WAY	
F-4.5	4'-6"x4'-6"x1'-0"	(5) #5 EACH WAY	
F-5	5'-0"x5'-0"x1'-2"	(5) #5 EACH WAY	
F-5.5	5'-6"x5'-6"x1'-2"	(6) #5 EACH WAY	

- MASONRY WALL REINFORCING NOTES:
- SEE PLAN FOR REQUIRED VERTICAL REINFORCING.
 - LOCATE REINFORCING IN CENTER OF WALL AND GROUT SOLID
 - HORIZONTAL JOINT REINFORCING SHALL BE HOT DIPPED GALVANIZED REINFORCING WITH (2) 9 GA. RODS AND 9 GA. CROSS WIRES AT 16" O.C. IN ALL EXTERIOR WALLS.
 - INTERIOR WALL HORIZONTAL JOINT REINFORCING SHALL BE (2) 9 GA. WIRE RODS & 9 GA. CROSS WIRES AT 16" o.c.
 - ADDITIONAL VERTICAL REINFORCING REQUIRED AT:
(COORD. WITH DETAIL 2/S4.5A)
-EACH SIDE OF OPENINGS
-WALL INTERSECTIONS
-ENDS OF WALLS
-EACH SIDE OF CONTROL JOINTS (COORD W/ ARCH).
-PLASTERS AS DETAILED.
-AS NOTED AND DETAILED ON DRAWINGS.
 - PROVIDE BOND BEAMS:
(COORD. WITH DETAIL 2/S4.5A)
-BOTTOM AND TOP OF OPENINGS.
-JOIST AND DECK BEARING (CONTINUOUS)
-TOP COURSE OF MASONRY WALLS
-AS NOTED AND DETAILED ON DRAWINGS.
-PROVIDE CORNER BARS AT ALL BOND BEAM INTERSECTIONS
 - PROVIDE MATCHING DOWEL FOR VERTICAL REINFORCING INTO FOUNDATION AND BOND BEAM AT ROOF.
 - PROVIDE CORNER BARS AT ALL BOND BEAMS.
 - FILL ALL MASONRY CELLS BELOW FINISHED FLOOR. WHERE GRADE IS ABOVE FINISHED FLOOR, FILL ALL CELLS BELOW GRADE. FILL MATERIAL SHALL BE 2000 PSI (MIN) GROUT.
 - ALL MASONRY STRUCTURES SHALL BE REINFORCED WITH #5 BARS VERTICAL AT 16" O.C. MAX. IN CENTER OF GROUT FILLED CELLS.
 - PROVIDE A 8" WIDE x 16" DEEP BOND BEAM w/ HOLES PROVIDED SO THAT THE VERTICAL REINF. MAY PASS CONTINUOUSLY THROUGH. REINFORCED w/ (2) #5 CONT. AROUND THE ENTIRE PERIMETER OF EACH MASONRY STRUCTURE. TOP OF BOND BEAM ELEVATIONS OF 10'-8", 21'-4", 32'-0" ROOF DECK BEARING AND TOP OF WALL

ISSUED FOR CONSTRUCTION

PROJECT

19th Lane
Vero Beach
Storage
Buildings

ADDRESS

19th Vane,
Vero Beach, FL 32966

CLIENT



Shamrock Building
Systems, Inc.

ADDRESS

1298 Concord Road SE
Smyrna, GA 30080

SHEET TITLE

FOUNDATION
PLAN
(BUILDING A)

Date:

04-09-2025

PROJECT NUMBER

23-131

SHEET NUMBER

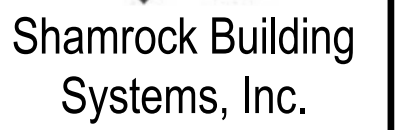
S1.1A

ISSUED FOR CONSTRUCTION

PROJECT

19th Vane,
Vero Beach, FL 32966

CLIENT



ADDRESS
1298 Concord Road SE
Smyrna, GA 30080

SHEET TITLE

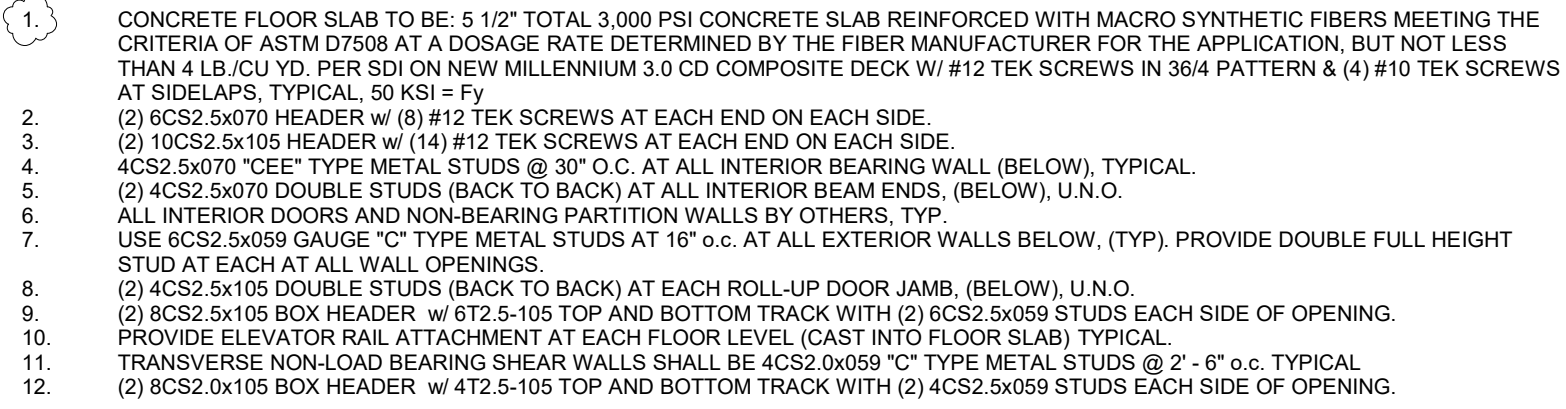
Date: 04-09-2025

PROJECT NUMBER

SHEET NUMBER

CC 1 A

S2.1A



**IMPACT DRIVERS ARE NOT ALLOWED FOR ALL METAL
STUD SCREW INSTALLATIONS**

**USE SCREW GUN WITH TORQUE SETTINGS CALIBRATED
TO SCREW MANUFACTURERS SPECIFICATIONS.**

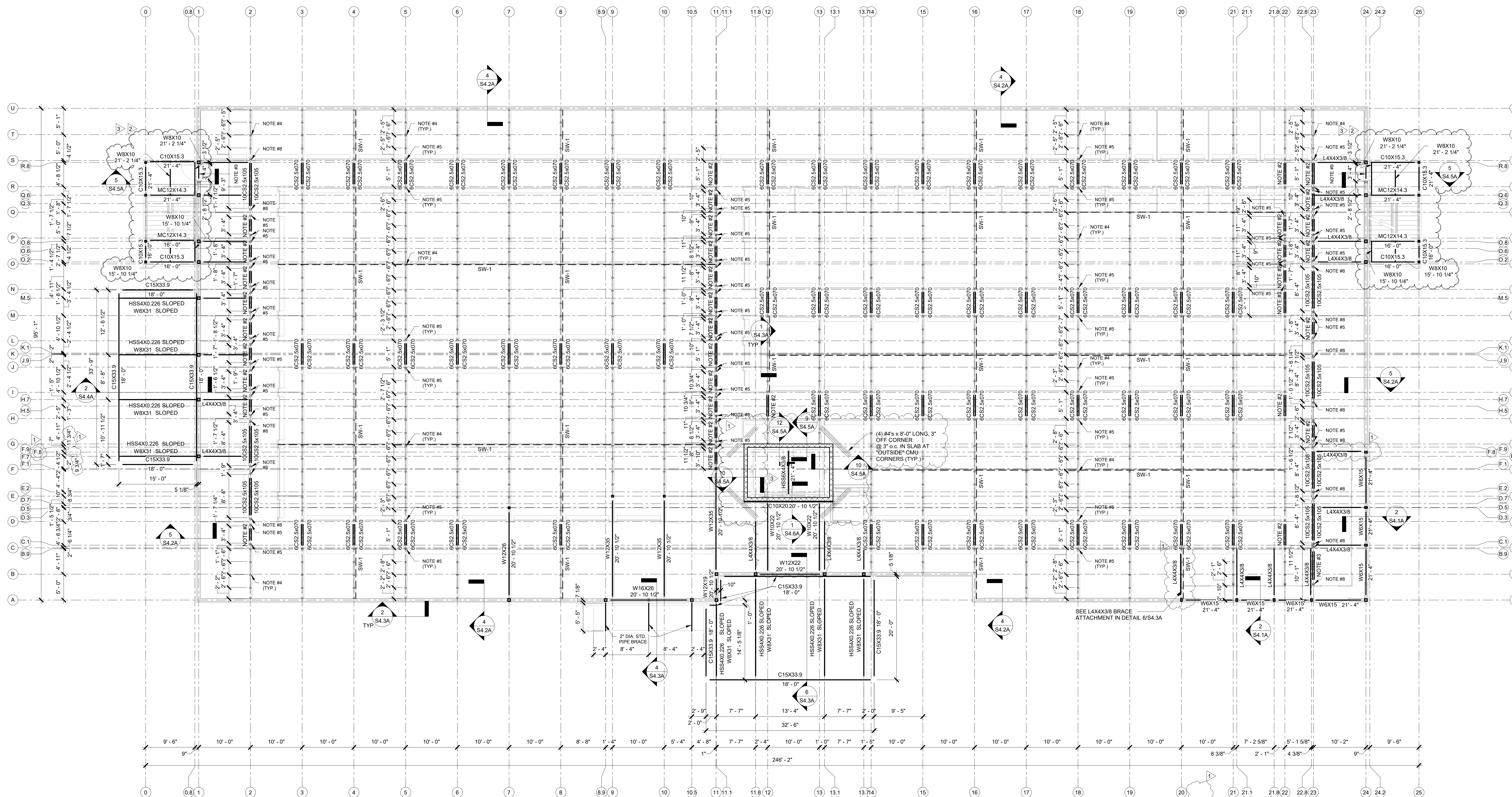
**DO NOT PLACE OR OPERATE RIDE-ON TROWELS WITH
TOTAL WEIGHT OVER 1000 LBS (TROWEL + RIDER) AND
NO OTHER ADJACENT CONSTRUCTION LOAD ON FLOOR.
FLOORS ALWAYS MAINTAIN 30 MIN. SPACING
BETWEEN ADJACENT TROWELS OR ANY OTHER
CONSTRUCTION LOAD.**

Rev. No.	Date	Name
1	04-09-2025	Contractor Coordination
2	05-30-2025	Contractor Coordination
3	06-19-2025	Contractor Coordination

SHEET TITLE

SHEET NUMBER

S2.2A



2 Third Floor Framing Plan
S2.2A 1/8" = 1'-0"

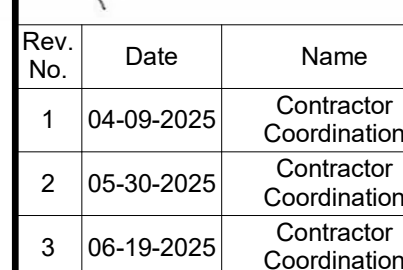
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[illegible]

IMPACT DRIVERS ARE NOT ALLOWED FOR ALL METAL STUD SCREW INSTALLATIONS

USE SCREW GUN WITH TORQUE SETTINGS CALIBRATED TO SCREW MANUFACTURERS SPECIFICATIONS.

DO NOT PLACE OR OPERATE RIDE-ON TROWELS WITH TOTAL WEIGHT OVER 1000 LBS. (TROWEL + RIDER) AND NO OTHER ADJACENT CONSTRUCTION LOAD ON FRAMED FLOORS. ALWAYS MAINTAIN 30 MIN. SPACING BETWEEN ADJACENT TROWELS OR ANY OTHER CONSTRUCTION LOAD.

[illegible]

PROJECT

19th Lane
Vero Beach
Storage
Buildings

ADDRESS
19th Vane,
Vero Beach, FL 32966

CLIENT



Shamrock Building
Systems, Inc.

ADDRESS
1298 Concord Road SE
Smyrna, GA 30080

SHEET TITLE

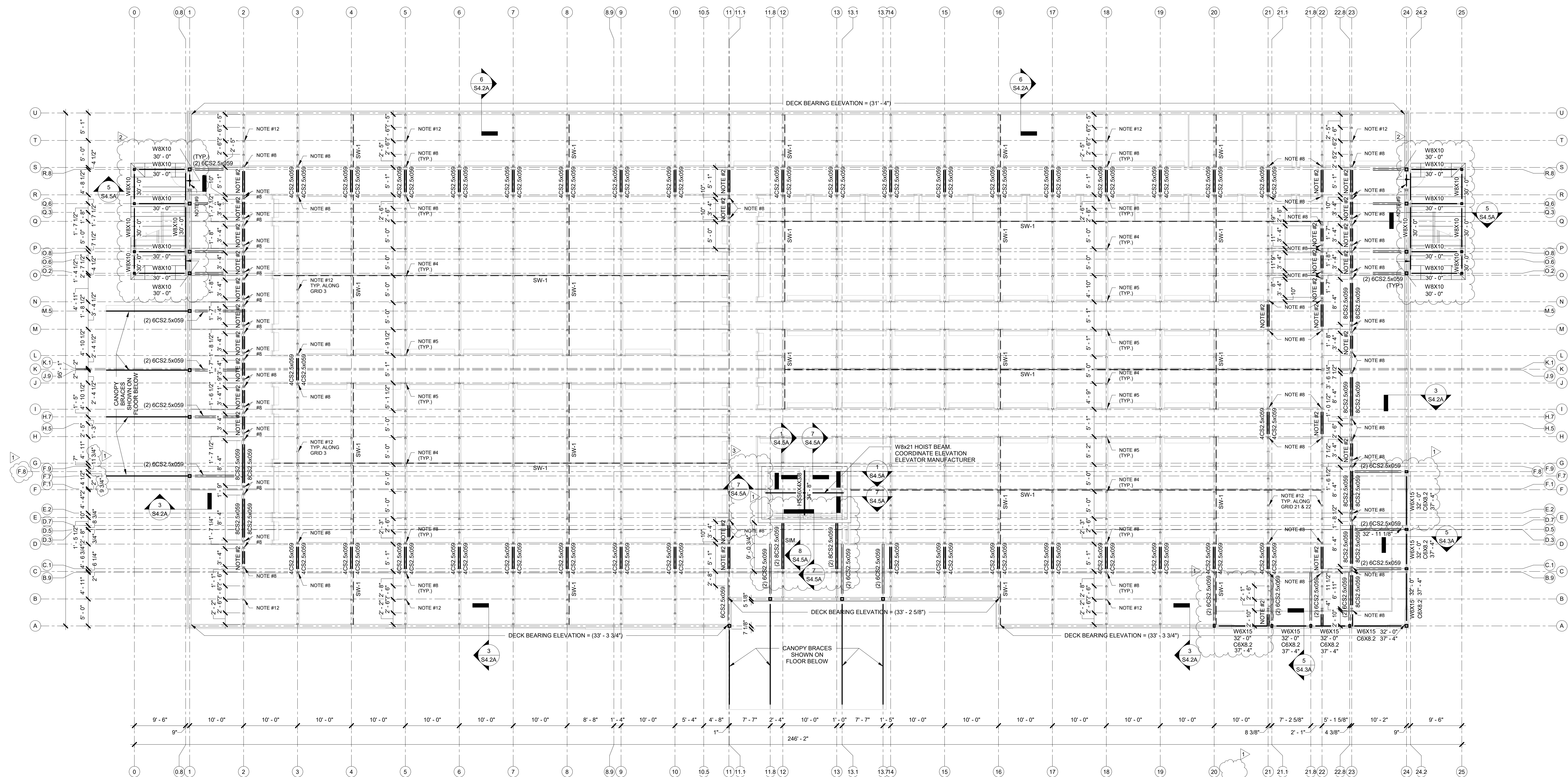
ROOF
FRAMING
PLAN
(BUILDING A)

Date: 04-09-2025

PROJECT NUMBER	00-104
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SHEET NUMBER

S2.3A

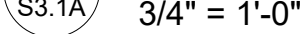
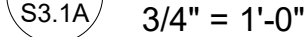
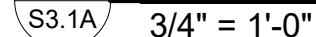
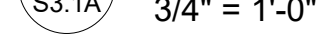
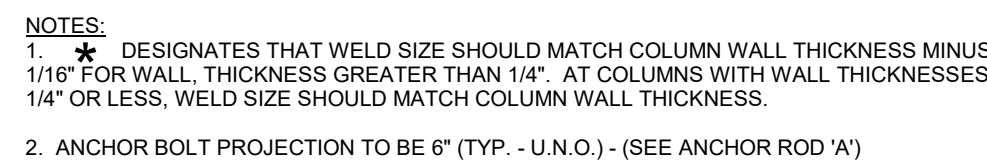
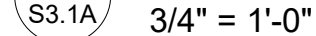
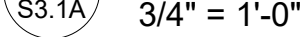
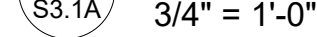
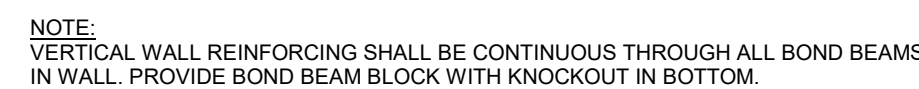
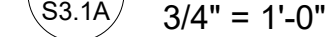
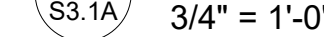
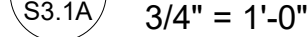
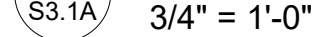


2 Roof Framing Plan
S2.3A 1/8" = 1'-0"

1. ROOF DECK TO BE 3" TYPE "N", 22 GA. W/ #12 TIE SCREWS IN 24" PATTERN & 7" #10 TIE SCREWS AT SIDELAPS, TYP. (2) 4CS2.6509 HEDGER
2. (2) 6CS2.6509 HEDGER
3. 4CS2.5059 "S" TYPE METAL STUDS @ 5'-0" O.C. AT ALL INTERIOR BEARING WALLS U/N (BELOW), TYP.
4. PROVIDE SINGLE 4CS2.6509 "S" TYPE STUD AT EACH ROLLUP DOOR JAMB AND AT EACH SIDE OF CORRIDORS FROM ALL INTERIOR DOORS AND NON-BEARING PARTITION WALLS BY OTHERS, TYP.
5. PROVIDE SINGLE 4CS2.6509 "S" TYPE STUD AT EACH ROLLUP DOOR JAMB AND AT EACH SIDE OF CORRIDORS FROM ALL INTERIOR DOORS AND NON-BEARING PARTITION WALLS BY OTHERS, TYP. PROVIDE DOUBLE FLIGHT STUD AT EACH AT ALL WALL OPENINGS.
6. 4CS2.6509 HEDGER @ EACH ROLL-UP DOOR JAMB U/N UNDER ALL DOUBLE "S" STUDS AT THIS LEVEL. PROVIDE DOUBLE "S" STUDS (BACK TO BACK) TO FOUNDATION BELOW.
7. 4CS2.6509 HEDGER @ 6'-0" O.C. @ 15'-0" TO 16'-0" TO 17'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
8. TRANSVERSE NONLOAD BEARING SHEAR WALLS @ 6'-0" O.C. SHALL BE 4CS2.6509 "C" TYPE STUDS @ 5'-0" O.C. TYPICAL.
9. 4CS2.6509 HEDGER @ 6'-0" O.C. @ 17'-0" TO 18'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
10. 4CS2.6509 HEDGER @ 6'-0" O.C. @ 18'-0" TO 19'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
11. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 19'-0" TO 20'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
12. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 20'-0" TO 21'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
13. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 21'-0" TO 22'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
14. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 22'-0" TO 23'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
15. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 23'-0" TO 24'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
16. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 24'-0" TO 25'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
17. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 25'-0" TO 26'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
18. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 26'-0" TO 27'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
19. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 27'-0" TO 28'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
20. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 28'-0" TO 29'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
21. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 29'-0" TO 30'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
22. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 30'-0" TO 31'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
23. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 31'-0" TO 32'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
24. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 32'-0" TO 33'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
25. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 33'-0" TO 34'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
26. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 34'-0" TO 35'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
27. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 35'-0" TO 36'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
28. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 36'-0" TO 37'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
29. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 37'-0" TO 38'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
30. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 38'-0" TO 39'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
31. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 39'-0" TO 40'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
32. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 40'-0" TO 41'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
33. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 41'-0" TO 42'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
34. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 42'-0" TO 43'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
35. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 43'-0" TO 44'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
36. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 44'-0" TO 45'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
37. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 45'-0" TO 46'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
38. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 46'-0" TO 47'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
39. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 47'-0" TO 48'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
40. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 48'-0" TO 49'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
41. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 49'-0" TO 50'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
42. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 50'-0" TO 51'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
43. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 51'-0" TO 52'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
44. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 52'-0" TO 53'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
45. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 53'-0" TO 54'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
46. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 54'-0" TO 55'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
47. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 55'-0" TO 56'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
48. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 56'-0" TO 57'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
49. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 57'-0" TO 58'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
50. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 58'-0" TO 59'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
51. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 59'-0" TO 60'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
52. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 60'-0" TO 61'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
53. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 61'-0" TO 62'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
54. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 62'-0" TO 63'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
55. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 63'-0" TO 64'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
56. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 64'-0" TO 65'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
57. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 65'-0" TO 66'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
58. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 66'-0" TO 67'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
59. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 67'-0" TO 68'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
60. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 68'-0" TO 69'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
61. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 69'-0" TO 70'-0" @ 6CS2.6509 STUDS EACH SIDE OF OPENING.
62. (2) 6CS2.6509 105 BOX HEDGER @ 6'-0" O.C. @ 70'-0

IMPACT DRIVER ARE NOT ALLOWED FOR ALL
METAL STUD SCREW INSTALLATIONS

USE SCREW GUN WITH TORQUE SETTINGS
CALIBRATED TO SCREW MANUFACTURERS
SPECIFICATIONS.



NOTES:

1. PRIMED STEEL MEMBERS SHALL MEET THE PHYSICAL AND CHEMICAL PROPERTIES OF ASTM A 1011, GRADE 55
2. ZINC-COATED (GALVANIZED) MEMBERS MEET THE PHYSICAL AND CHEMICAL OF ASTM A 653, GRADE 55 AND G60 COATING DESIGNATION AS DESCRIBED IN ASTM A 924.

Rev. No.	Date	Name
1	04-09-2025	Contractor Coordination
2	06-19-2025	Contractor Coordination

ISSUED FOR CONSTRUCTION

PROJECT

19th Lane Vero Beach Storage Buildings

ADDRESS
19th Lane,
Vero Beach, FL 32966

CLIENT


Shamrock Building Systems, Inc.

ADDRESS
1298 Concord Road SE
Smyrna, GA 30080

SHEET TITLE

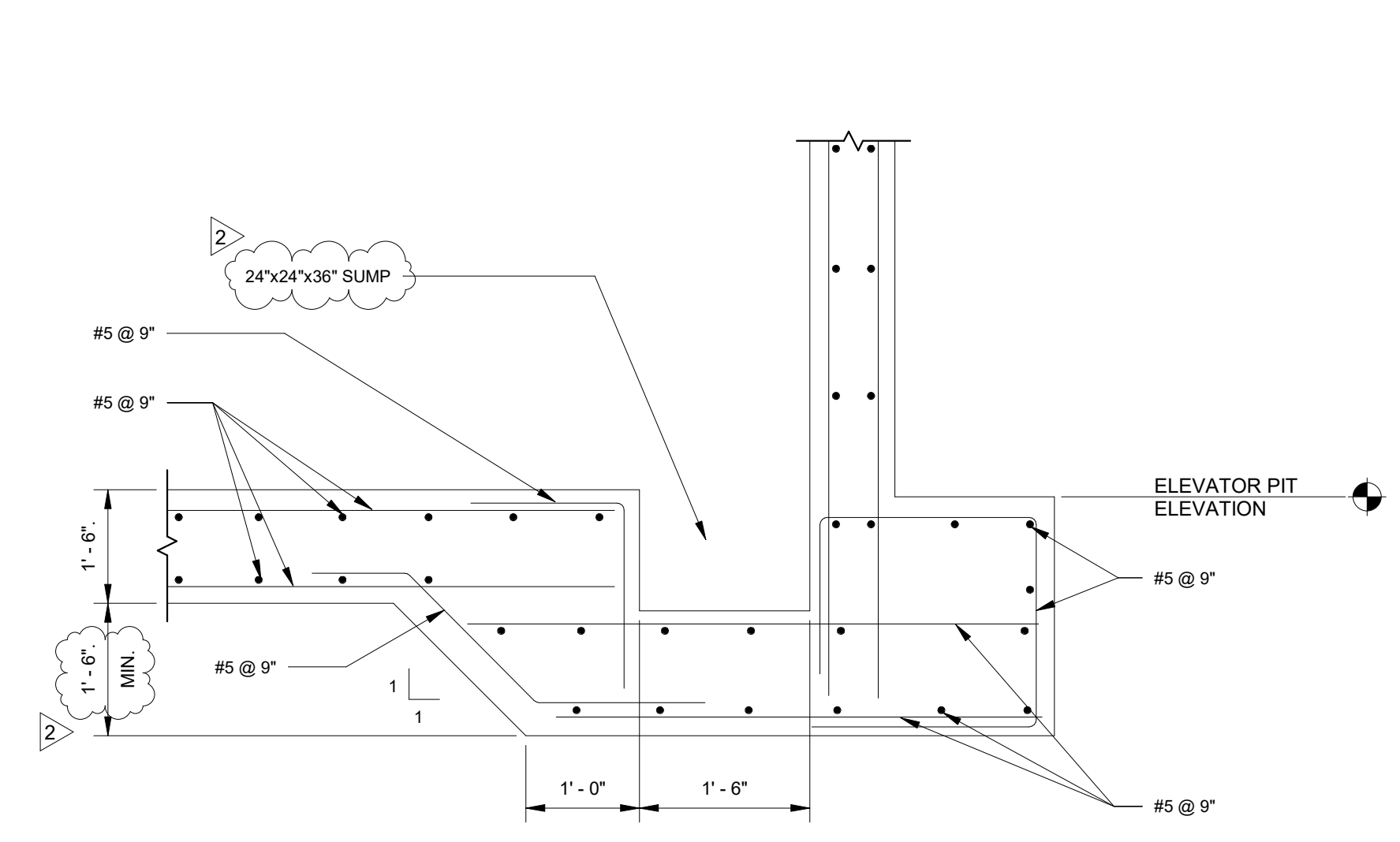
FRAMING DETAILS (BUILDING A)

Date: 04-09-2025

PROJECT NUMBER
23-131

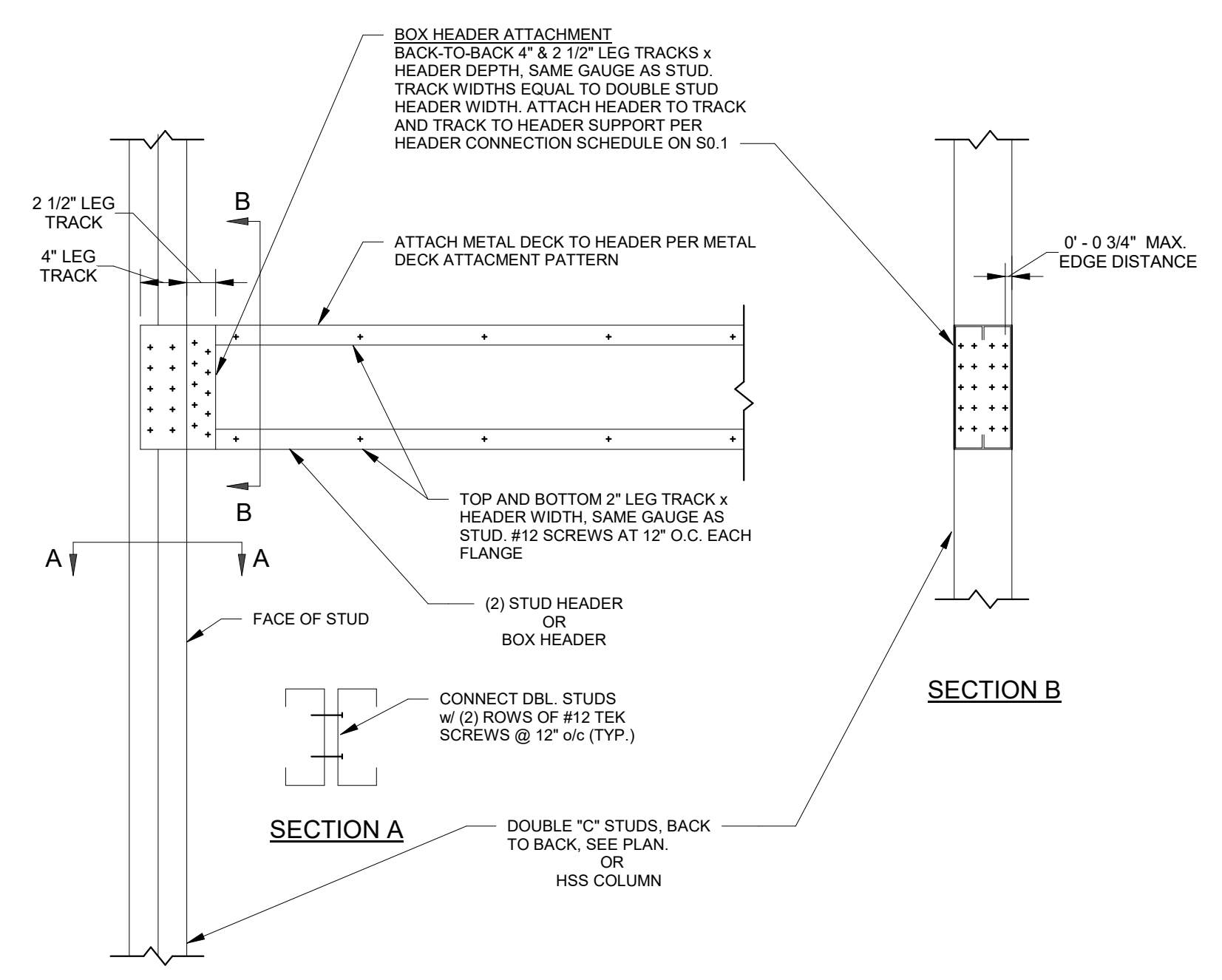
SHEET NUMBER

S4.1A

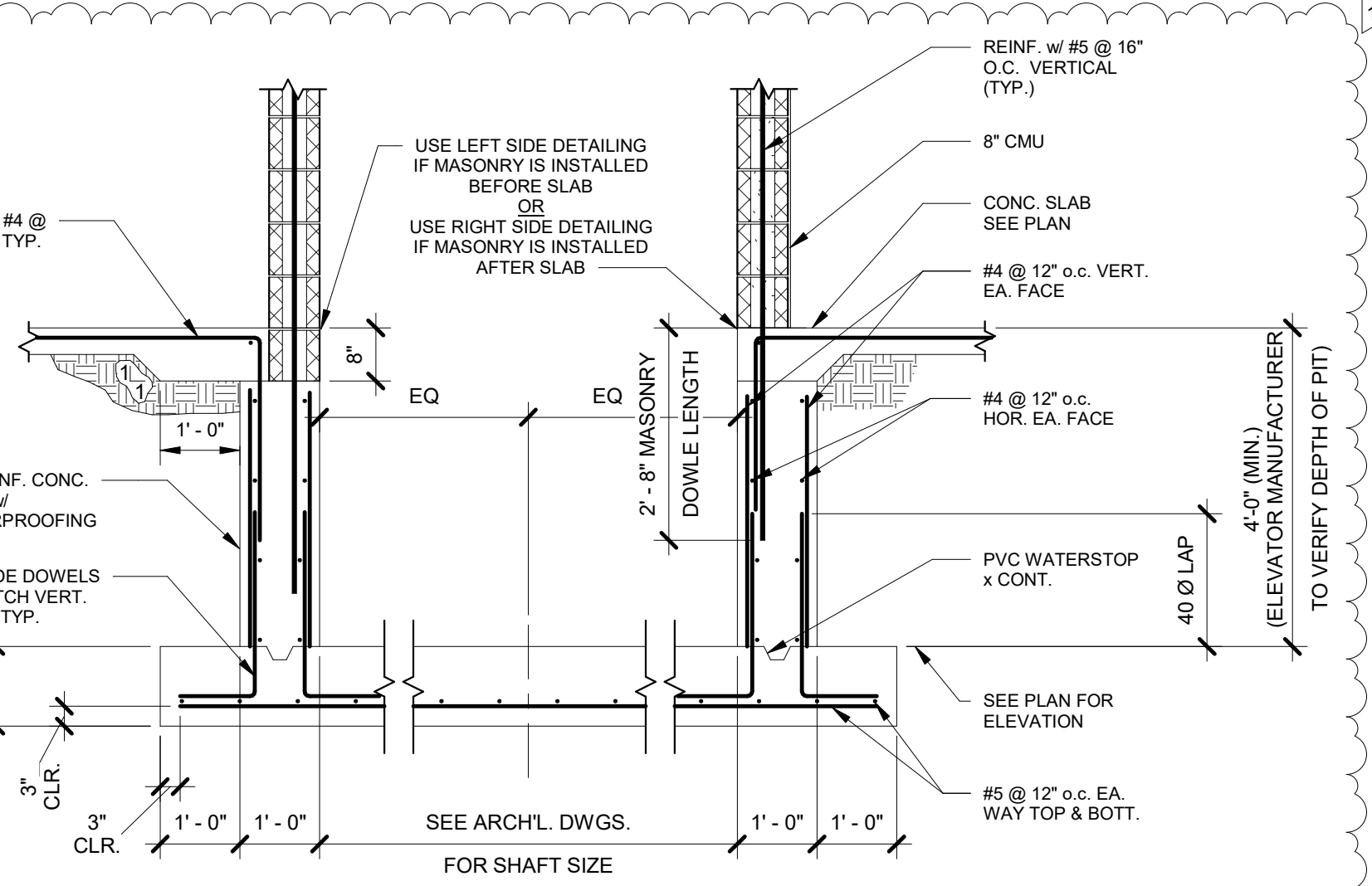


TYPICAL SUMP DETAIL
(COORDINATE LOCATION WITH ELEVATOR MANUFACTURER)

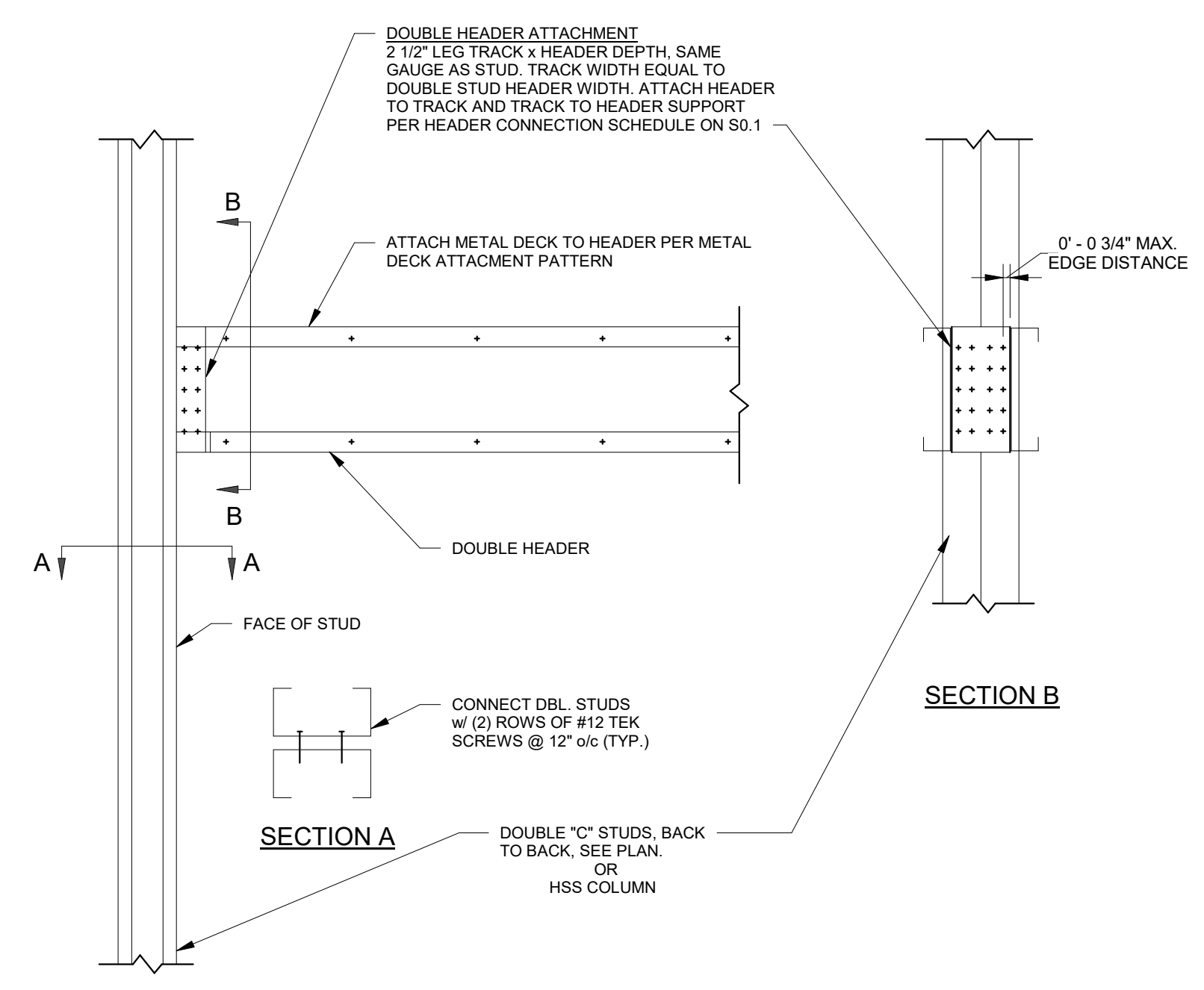
9 SUMP DETAIL
S4.1A 3/4" = 1'-0"



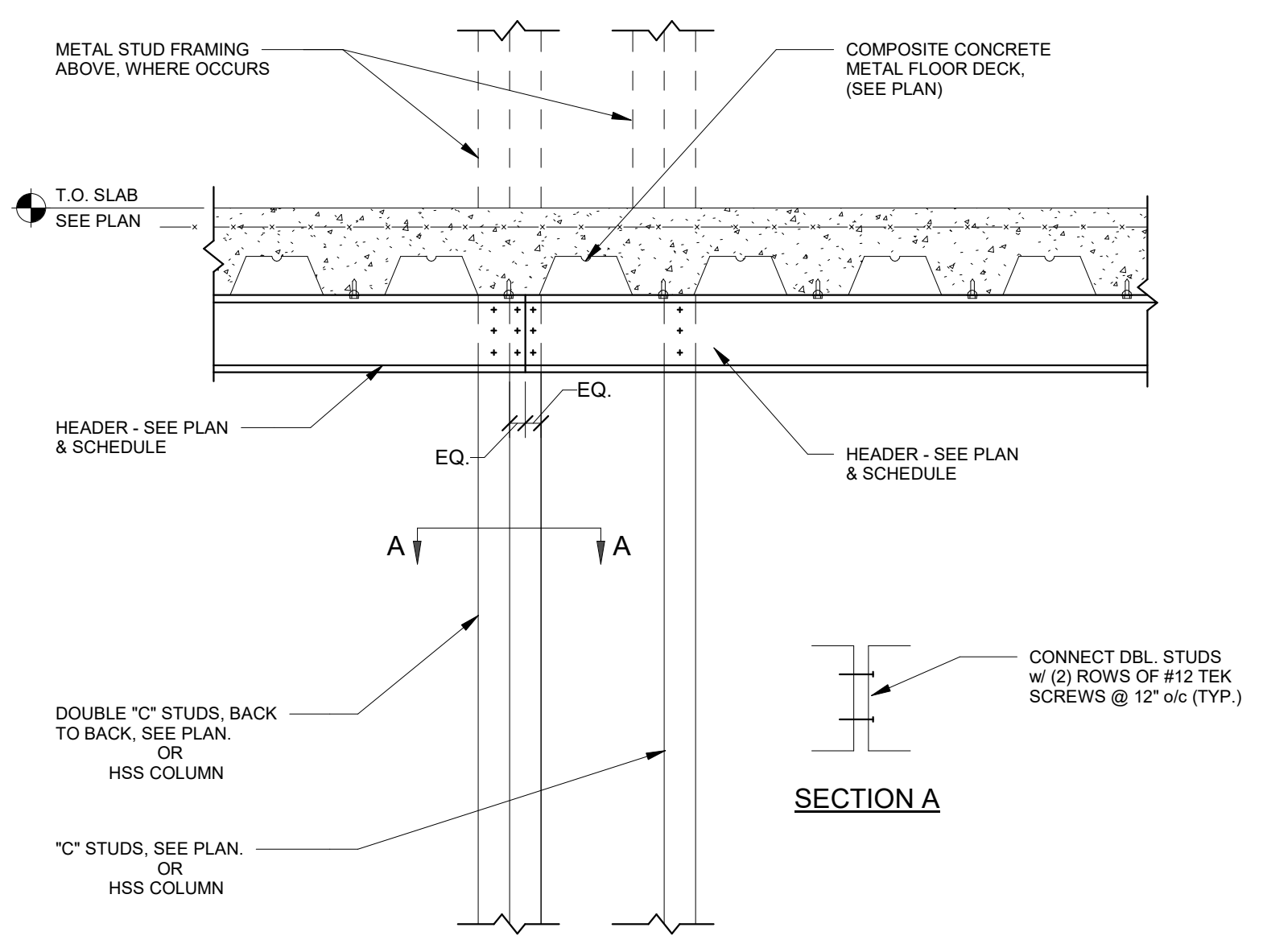
12 BOX HEADER BEARING DETAIL
S4.1A 3/4" = 1'-0"



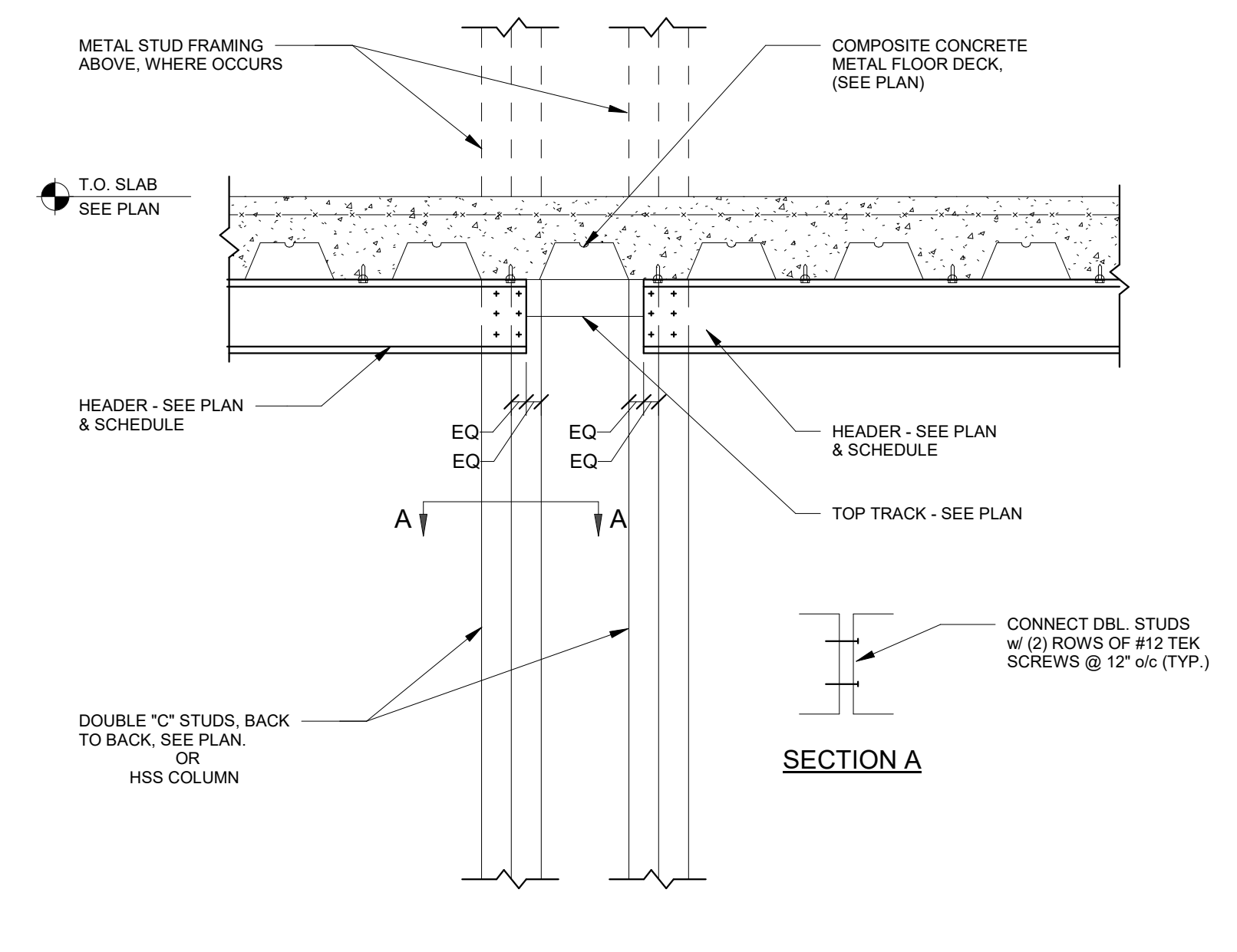
11 ELEVATOR PIT
S4.1A 1/2" = 1'-0"



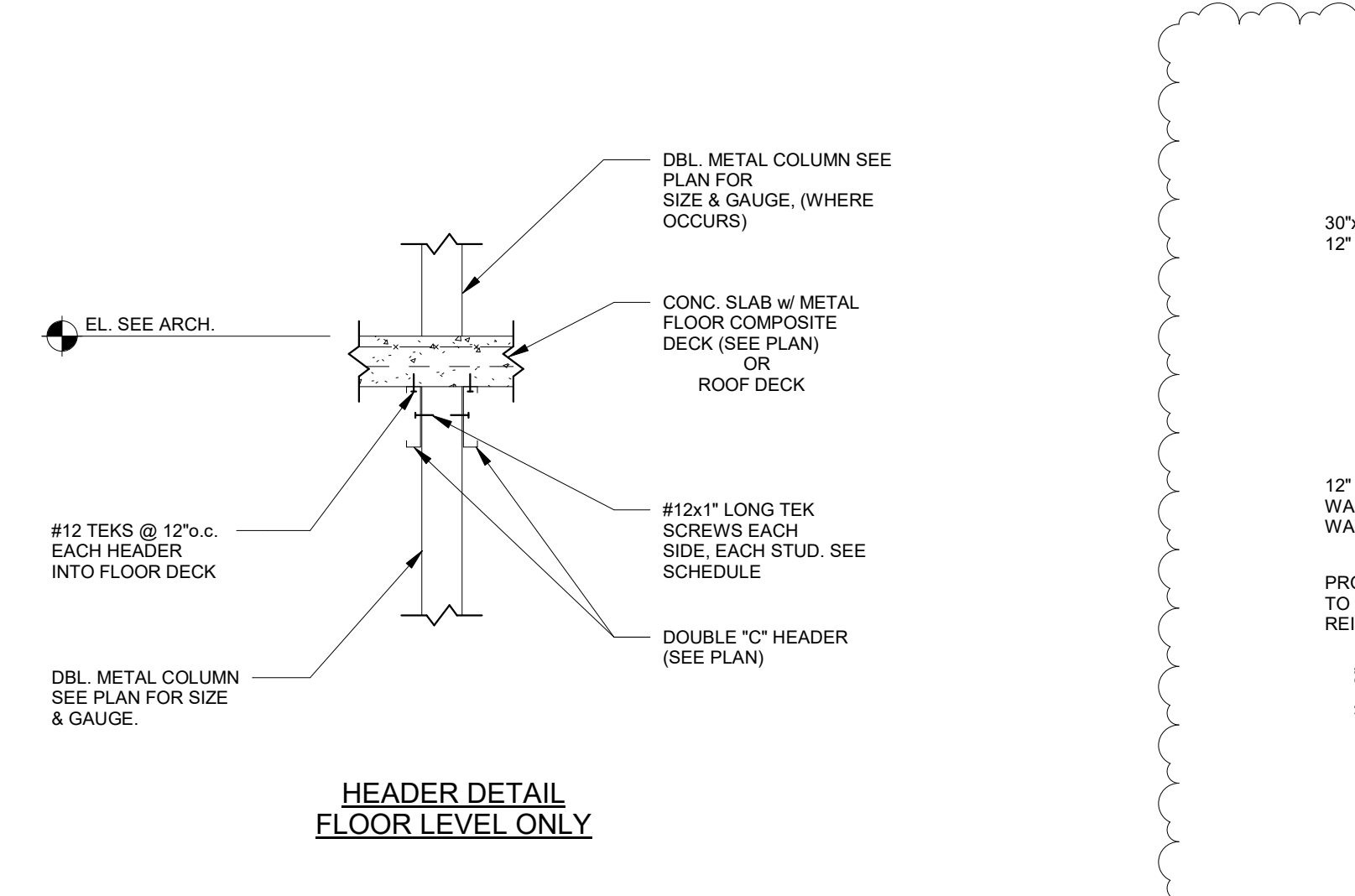
14 DOUBLE HEADER BEARING DETAIL
S4.1A 3/4" = 1'-0"



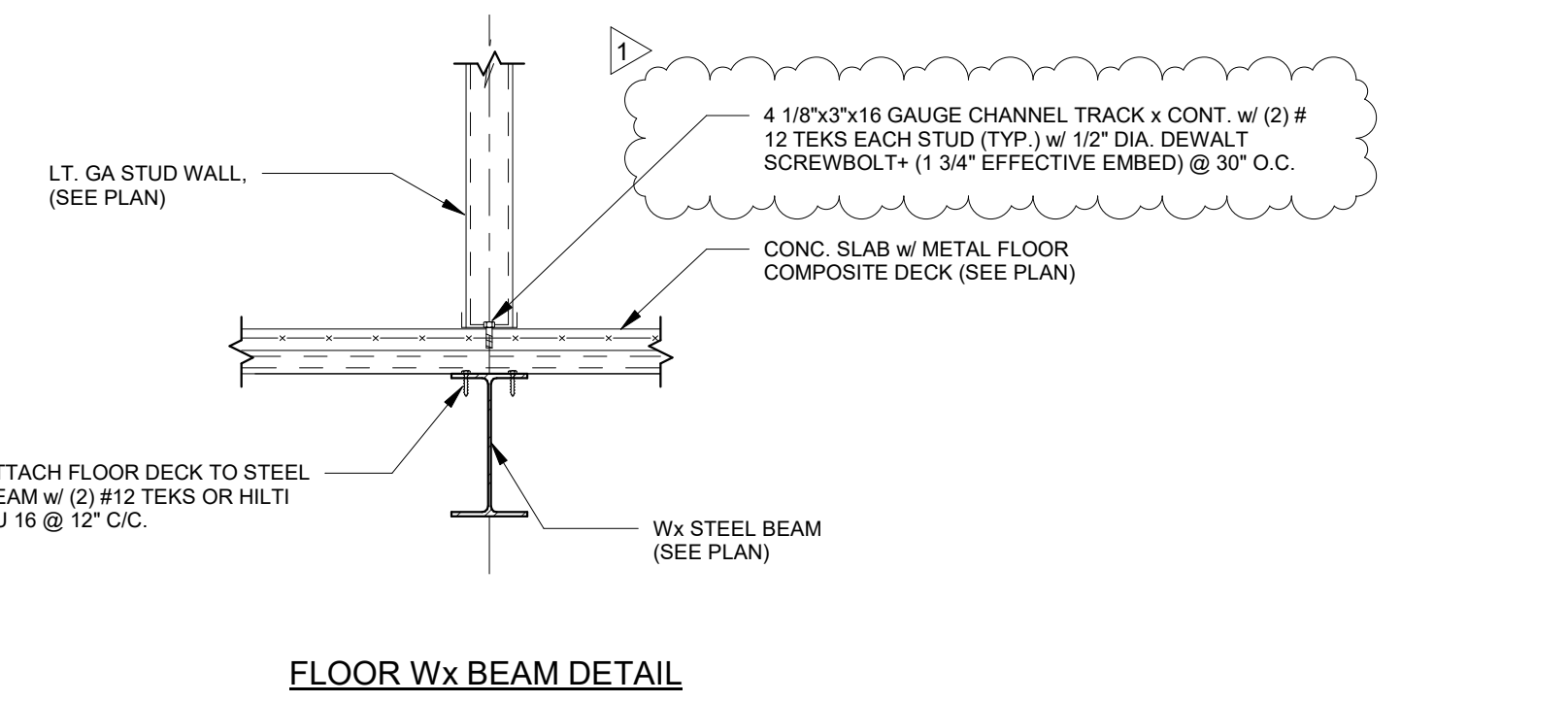
10 CONTINUOUS HEADER BEARING DETAIL
S4.1A 3/4" = 1'-0"



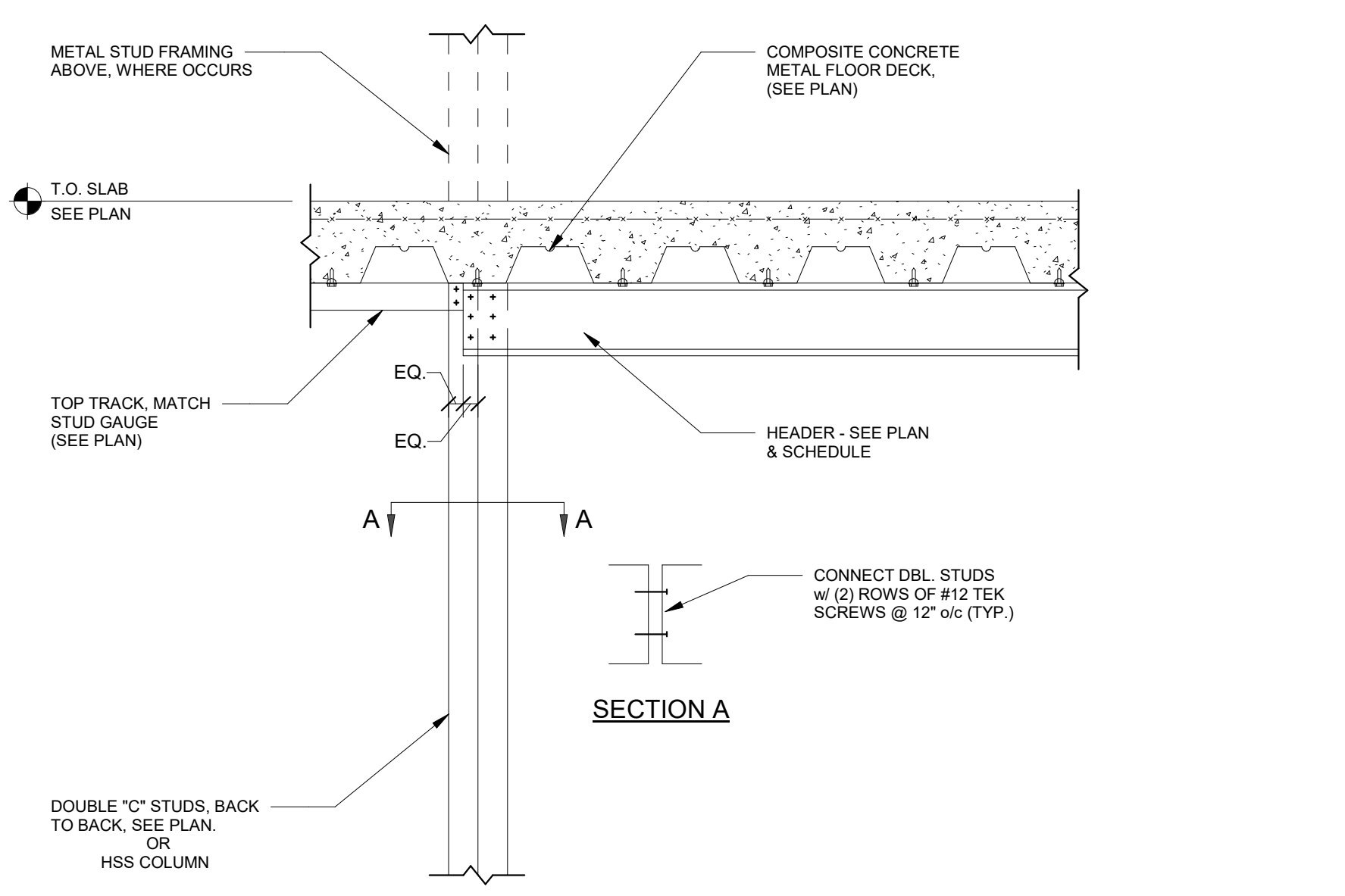
7 CONTINUOUS HEADER BEARING DETAIL
S4.1A 3/4" = 1'-0"



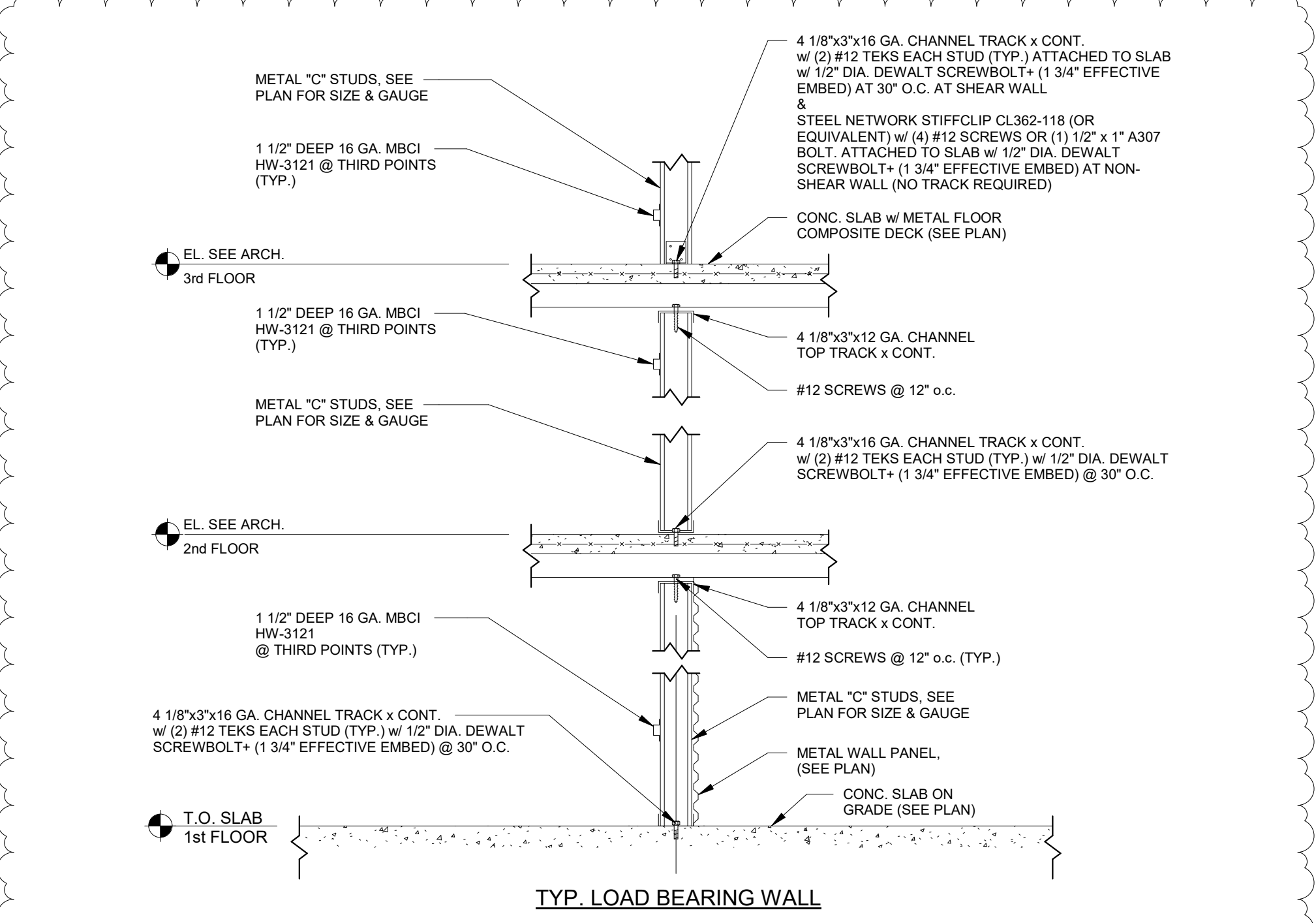
4 HEADER DETAIL
S4.1A 3/4" = 1'-0"



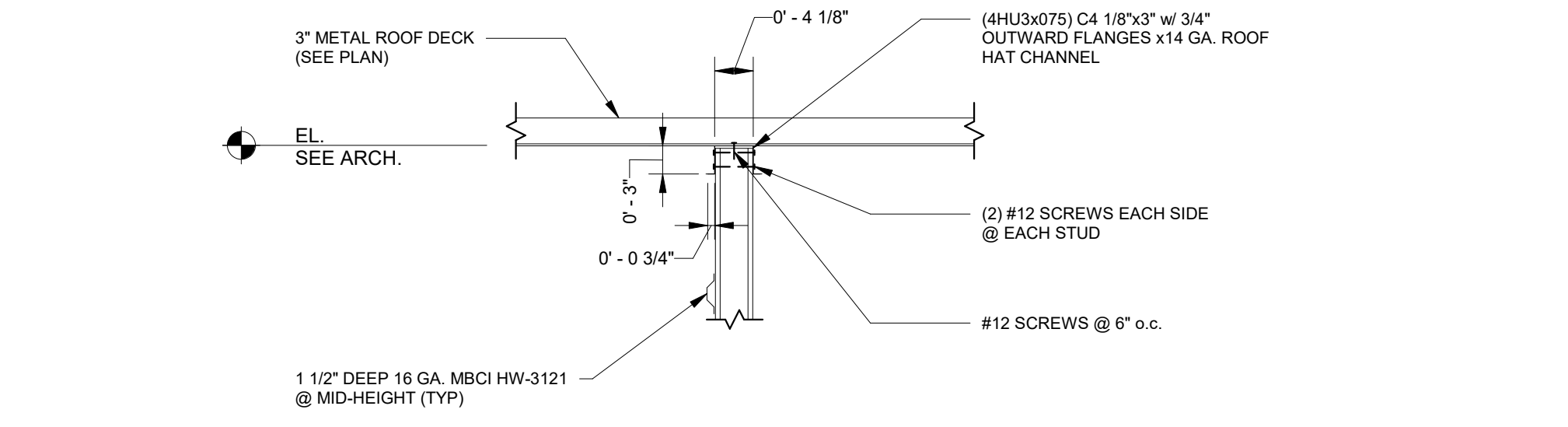
8 DECK BEARING
S4.1A 3/4" = 1'-0"



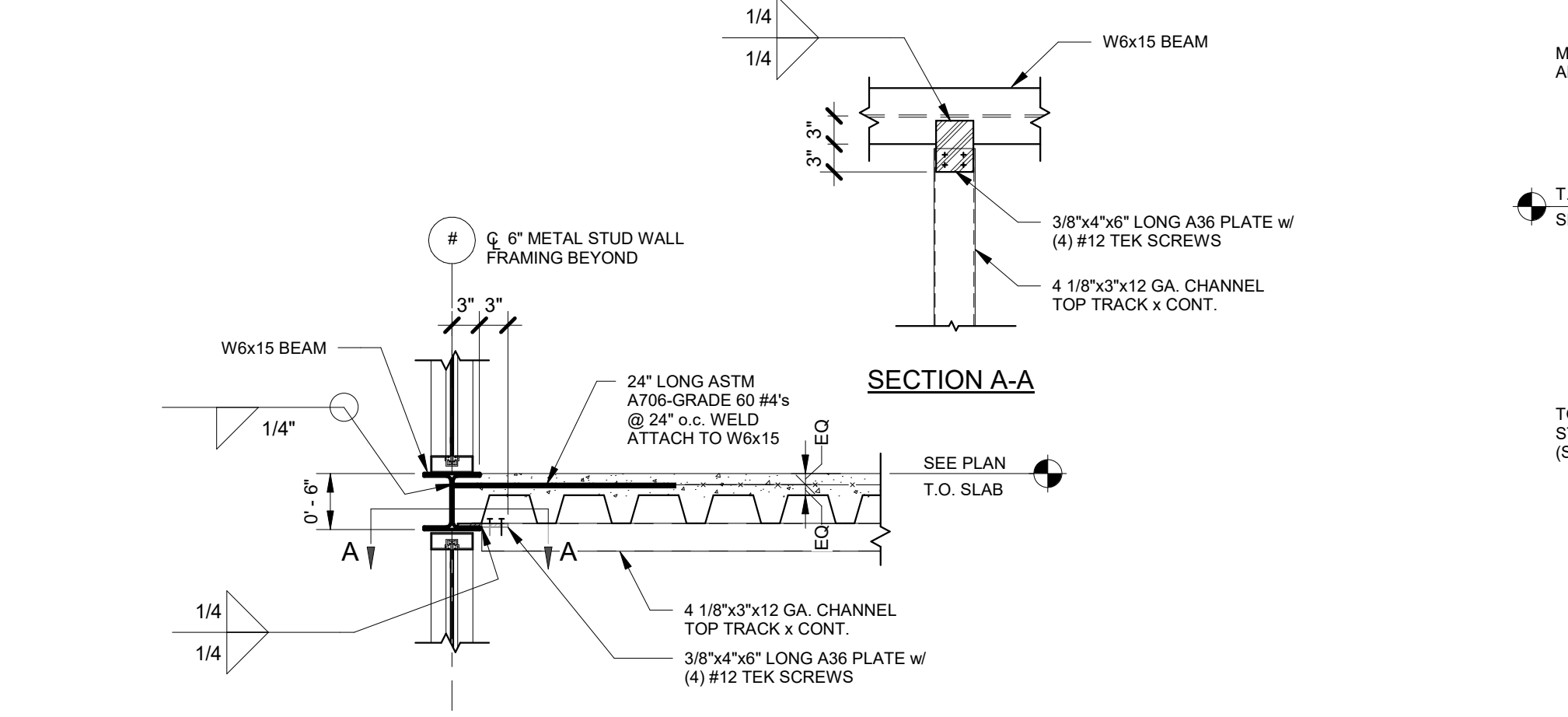
6 HEADER BEARING DETAIL
S4.1A 3/4" = 1'-0"



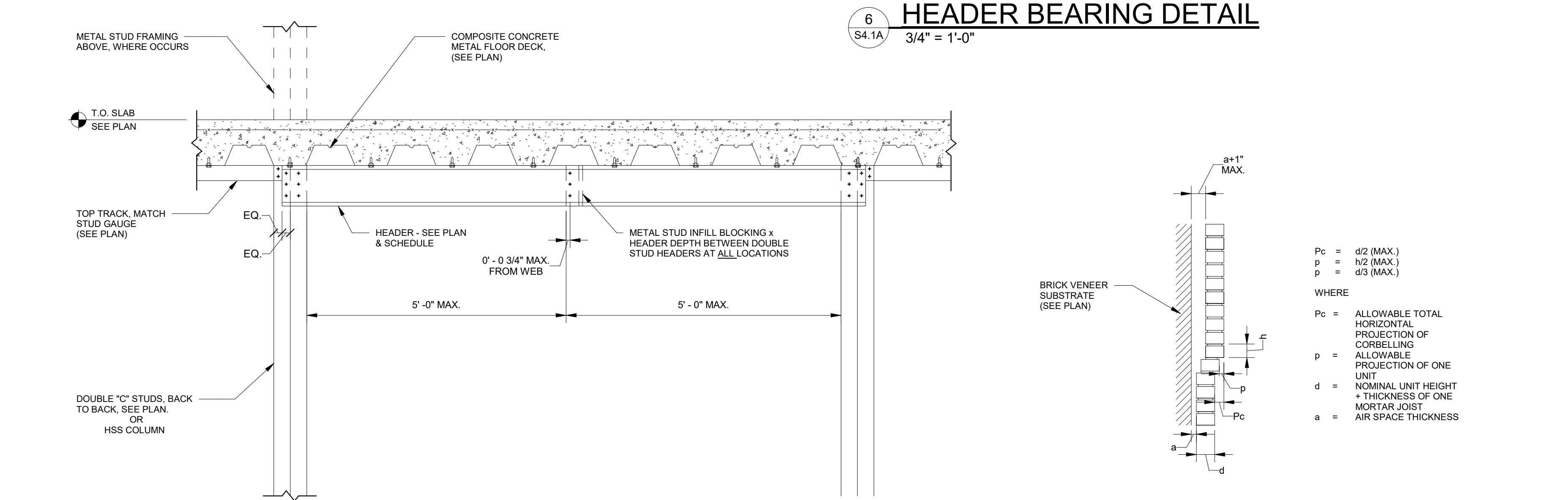
3 SECTION
S4.1A 3/4" = 1'-0"



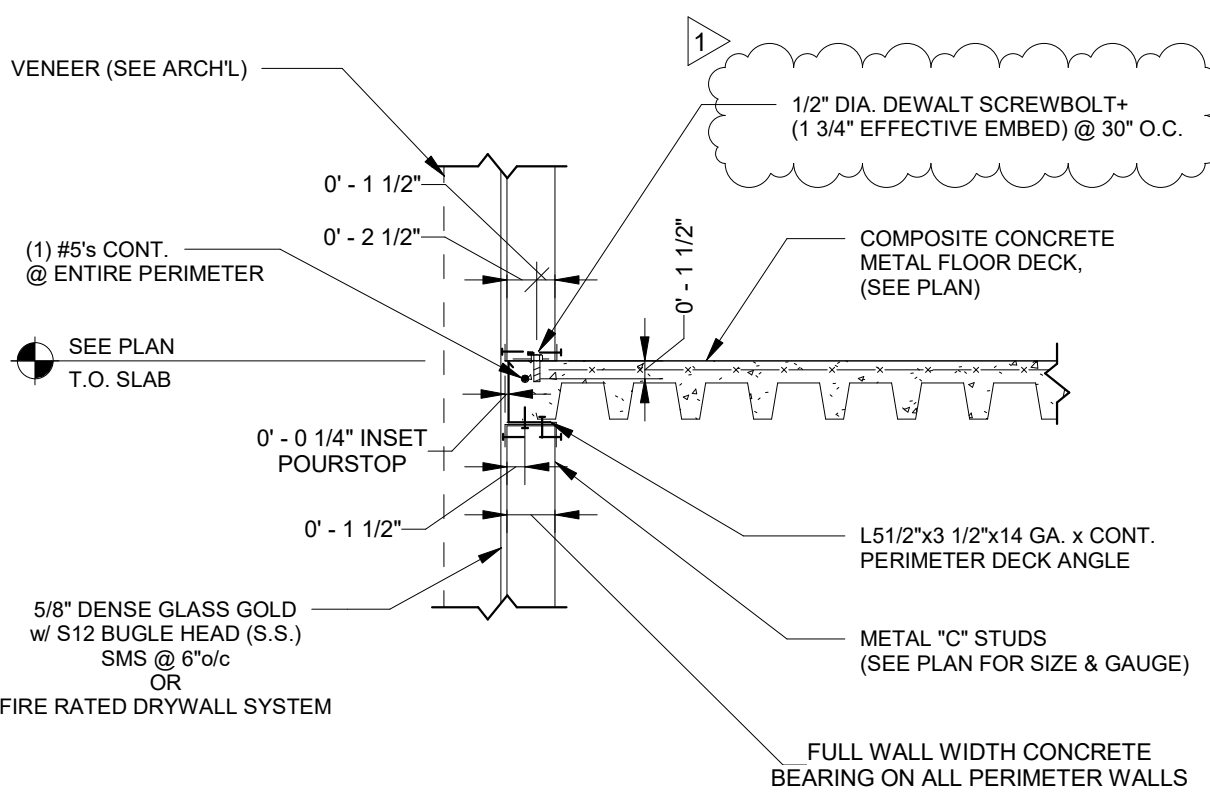
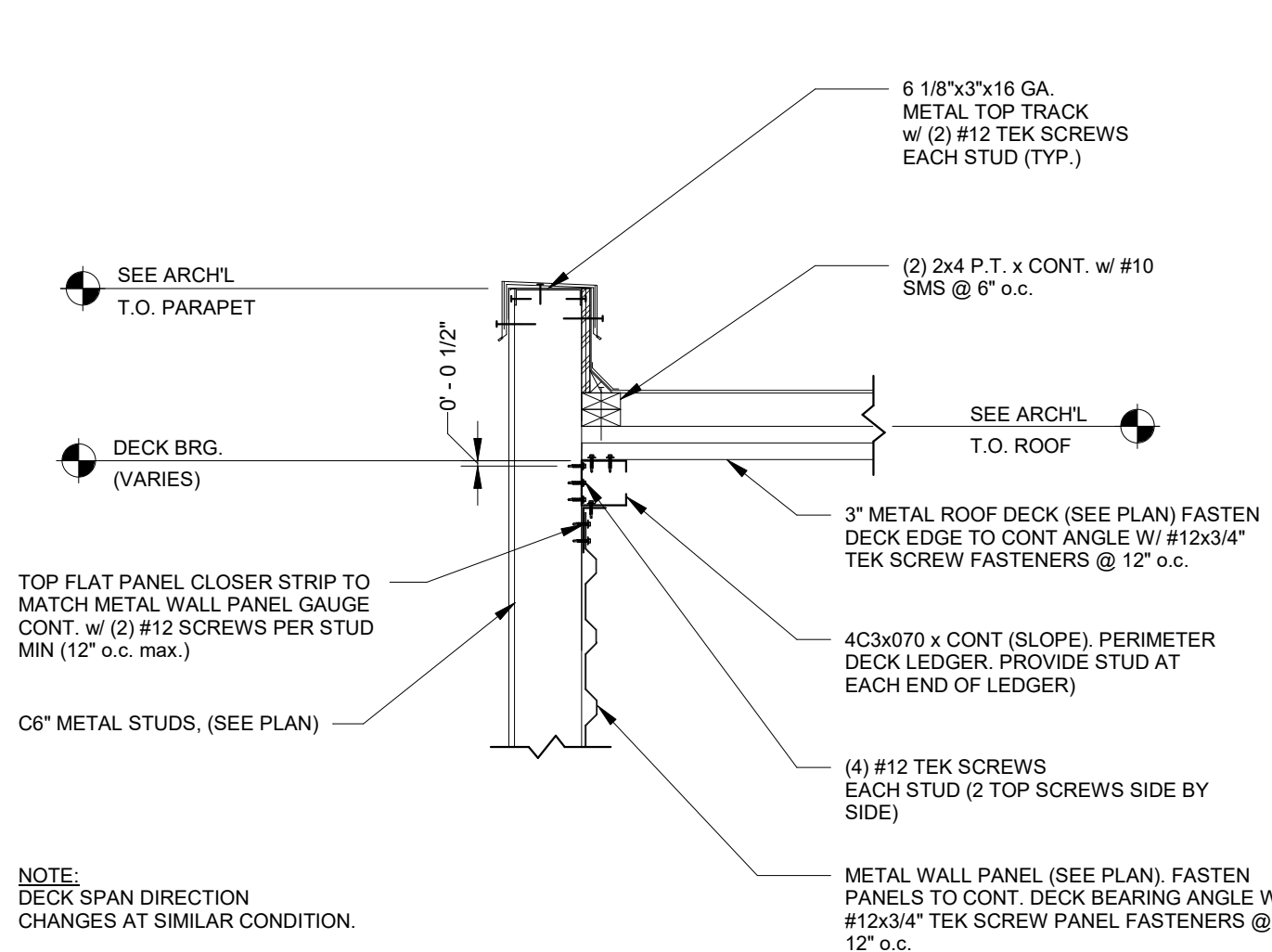
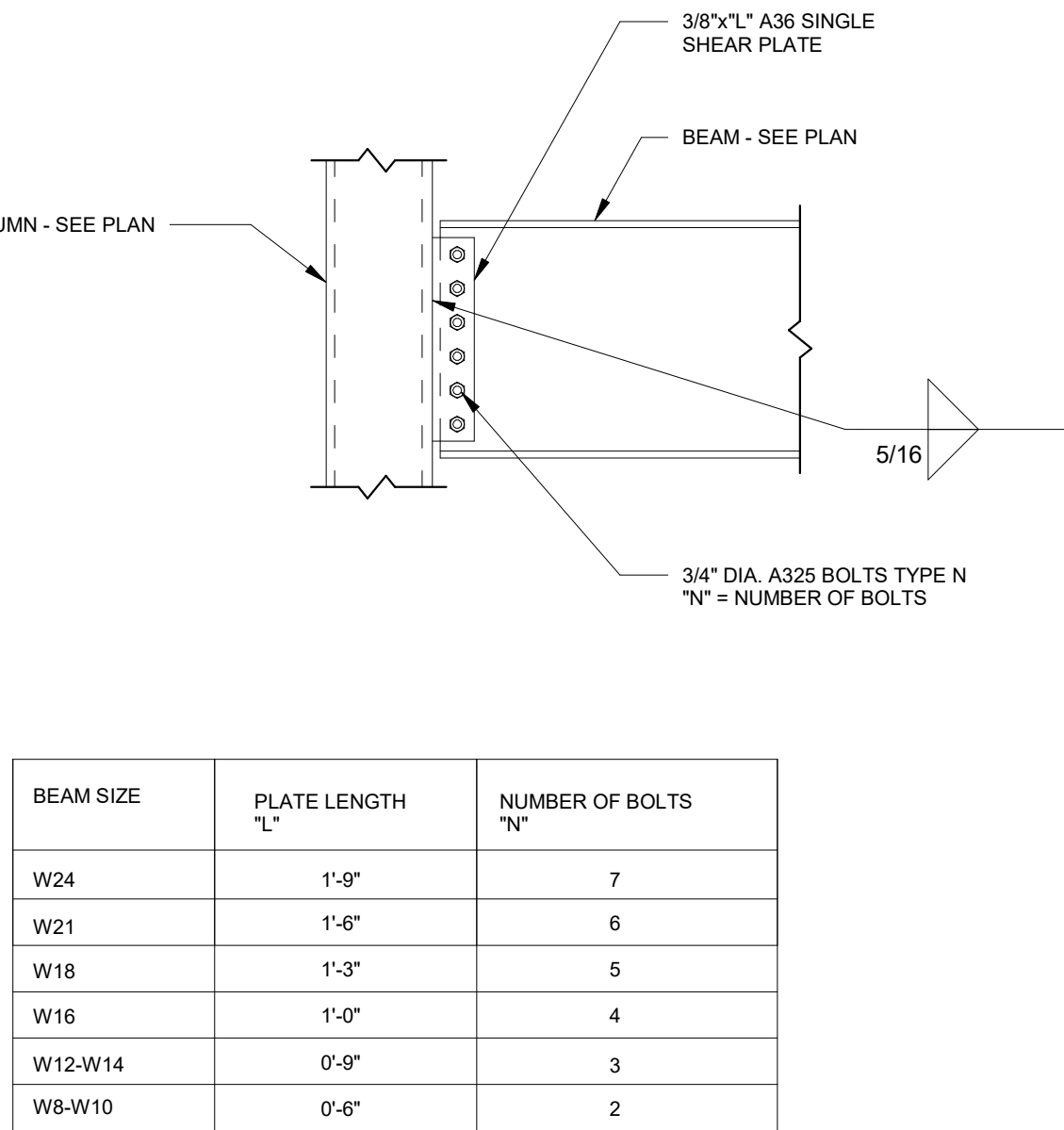
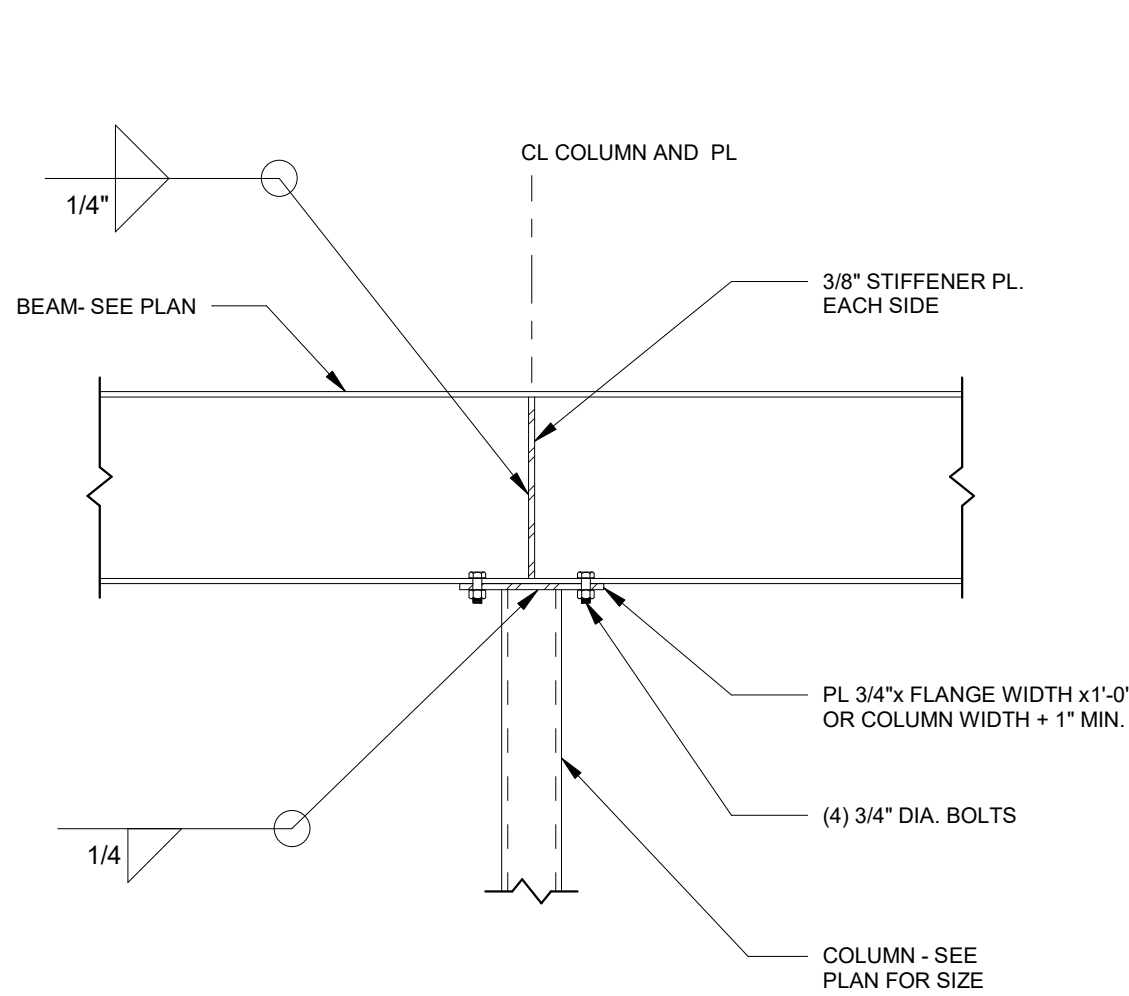
5 DECK BEARING
S4.1A 3/4" = 1'-0"



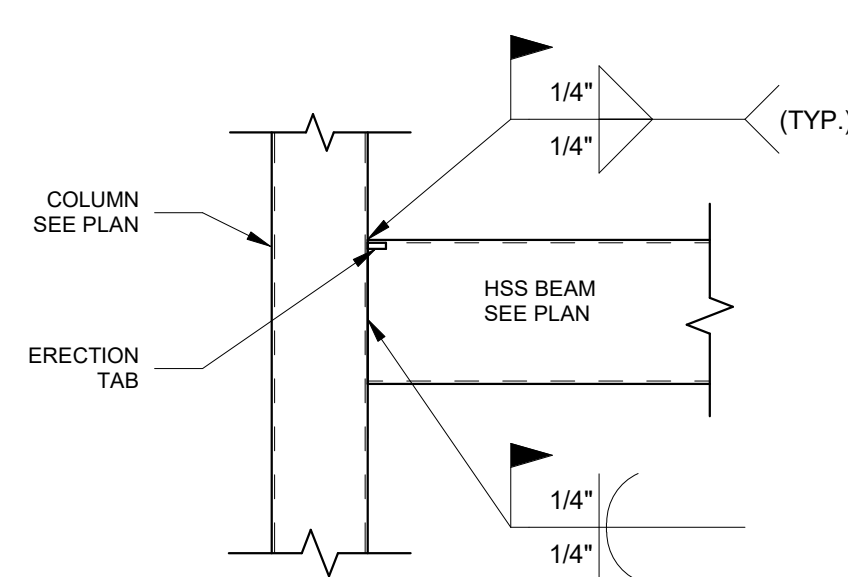
2 SLAB EDGE DETAIL @ STOREFRONT
S4.1A 3/4" = 1'-0"



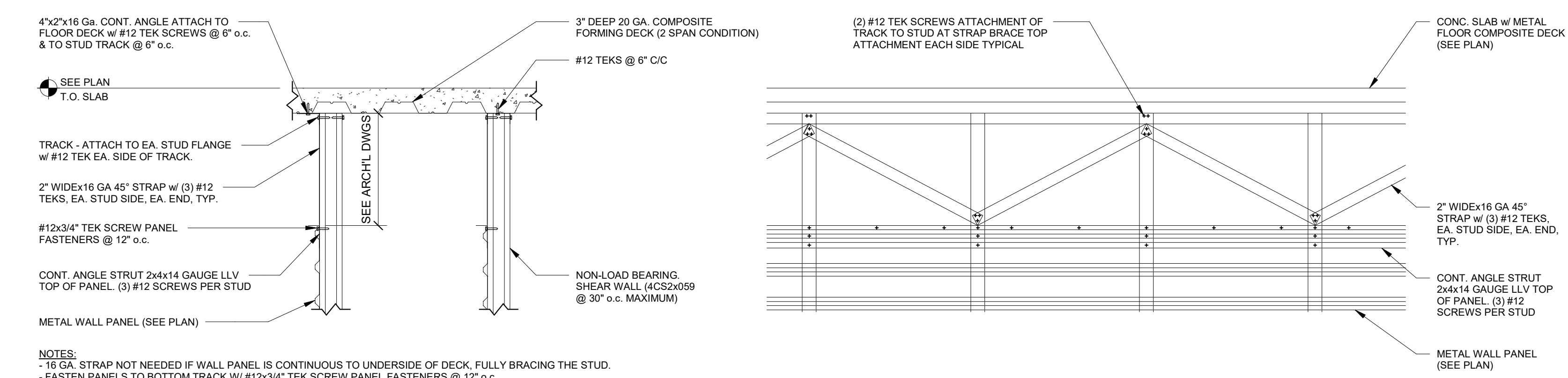
1 STUD HEADER BLOCKING DETAIL
S4.1A 3/4" = 1'-0"



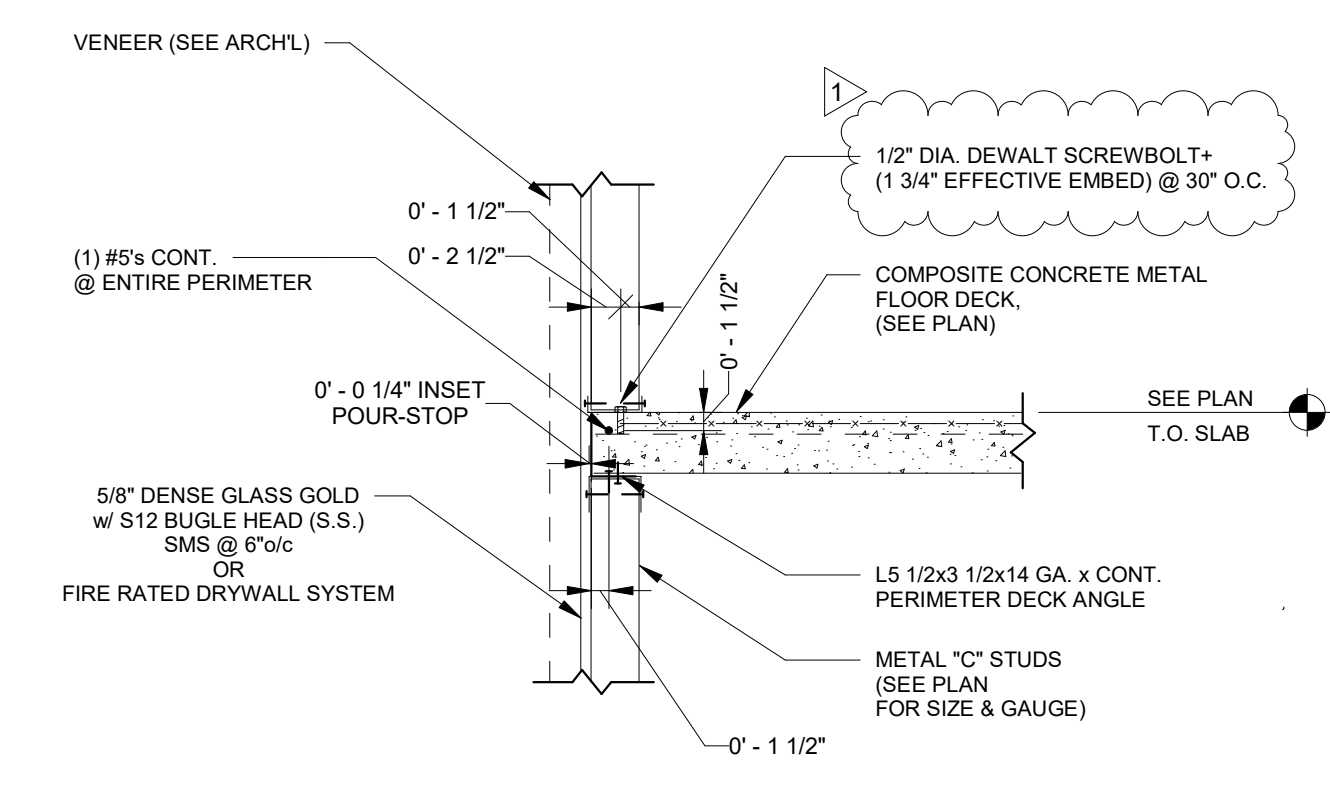
4
S4.2A **FLOOR DECK BEARING**
3/4" = 1'-0"



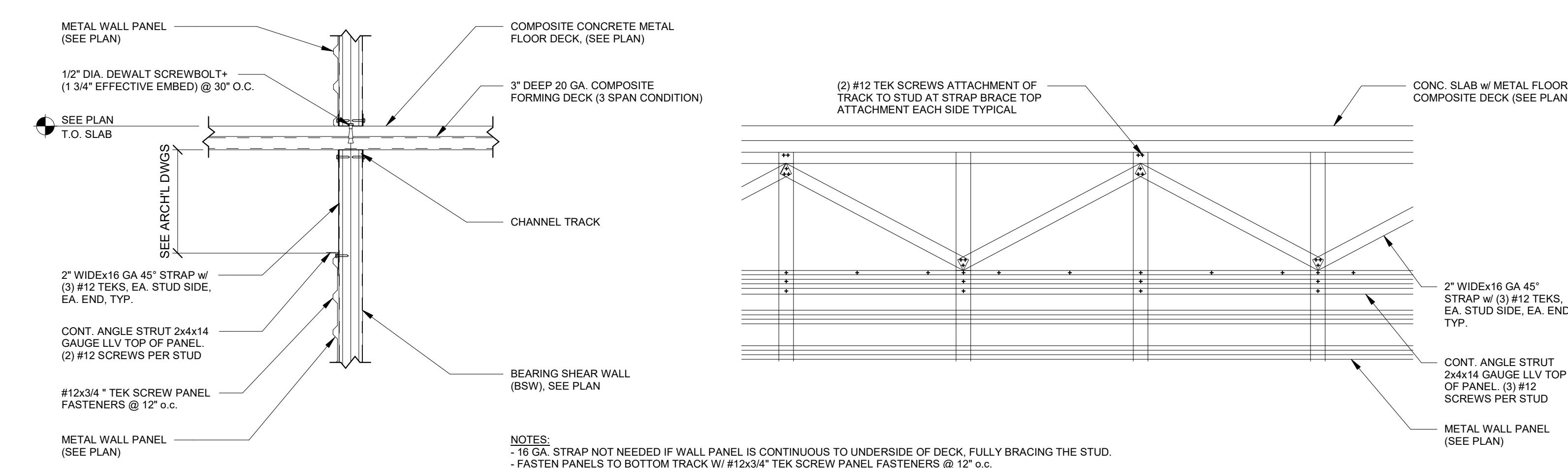
11
S4.2A **TYP. CONN. OF TUBE BM. TO COL.**
3/4" = 1'-0"



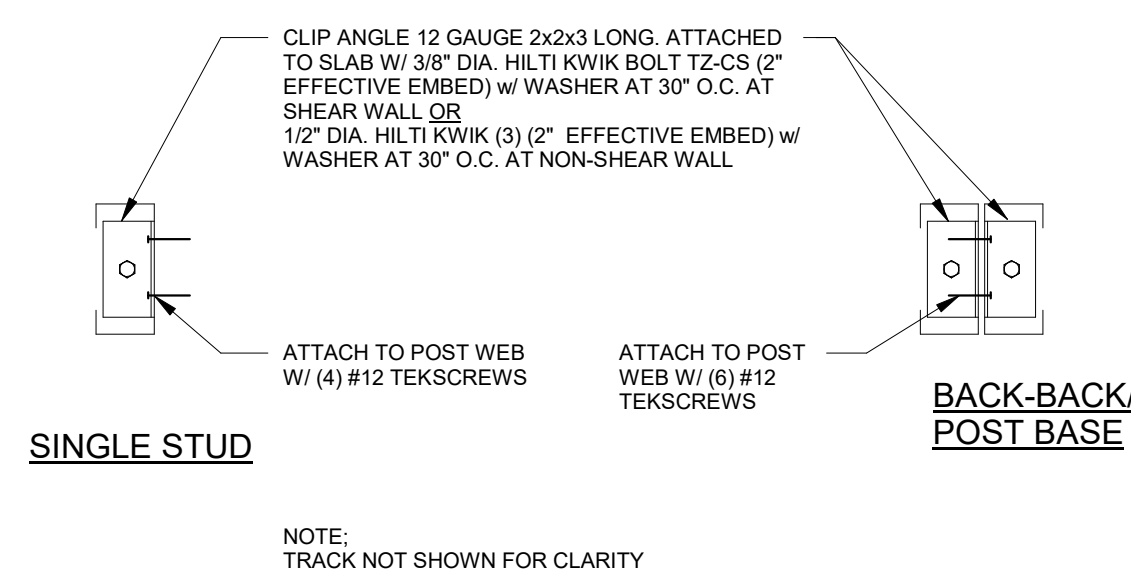
7
S4.2A **SHEAR WALL STRAP BRACING ABOVE METAL PANEL (TRANSVERSE SHEAR WALL)**
3/4" = 1'-0"



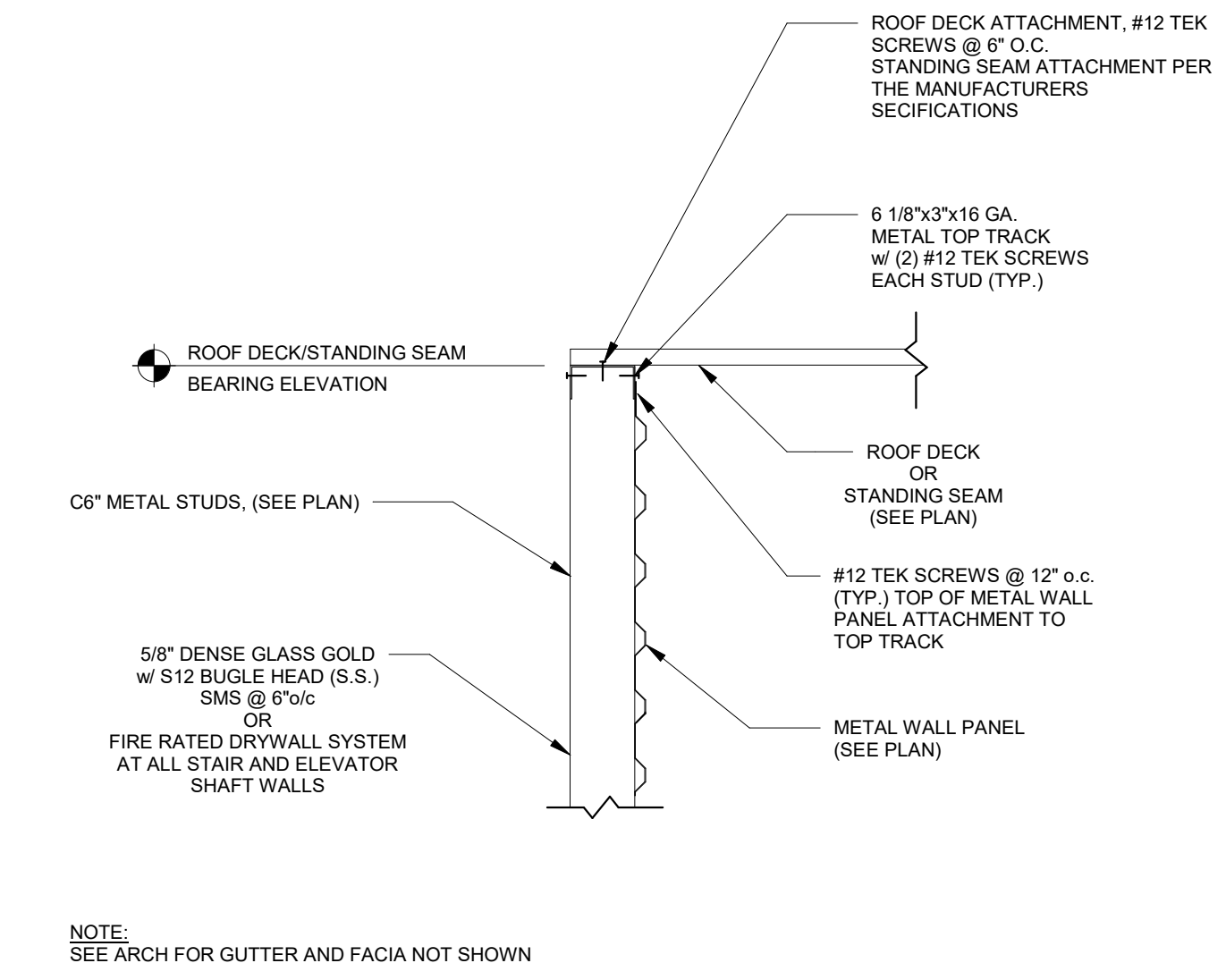
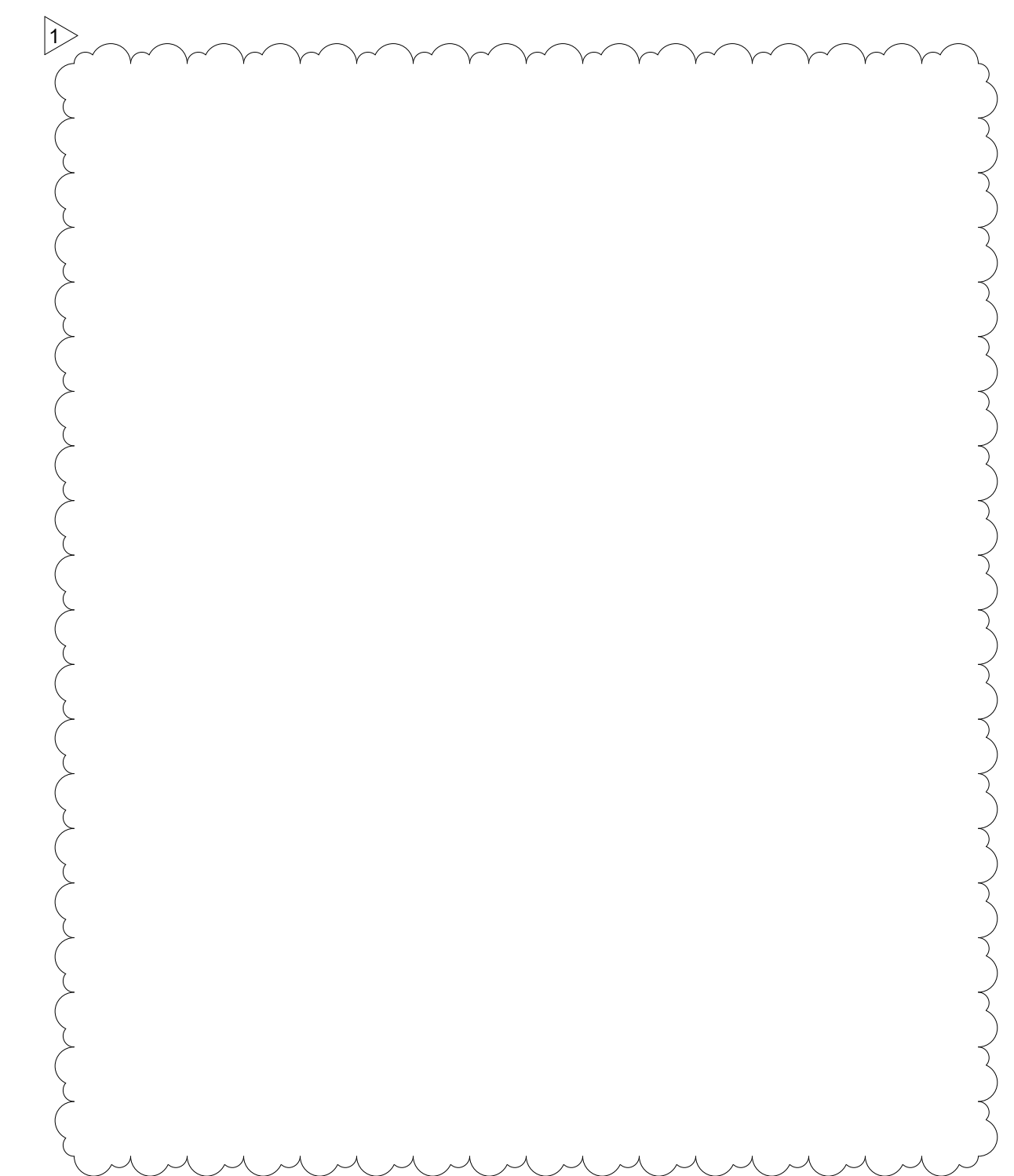
5
S4.2A **FLOOR DECK BEARING**
3/4" = 1'-0"



8
S4.2A **SHEAR WALL STRAP BRACING ABOVE METAL PANEL**
3/4" = 1'-0"



10
S4.2A **POST BASE ATTACHMENT TO SLAB**
3/4" = 1'-0"



6
S4.2A **ROOF DECK BEARING**
3/4" = 1'-0"

Keystone Structural Engineering
Professional Consultants
531 Roselane Street
Suite 150
Marietta, GA 30060
(404) 483 6921

BRANDON F. CROWLEY
No. 72169
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Rev. No.	Date	Name
1	04-09-2025	Contractor Coordination

ISSUED FOR CONSTRUCTION

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SHEET TITLE

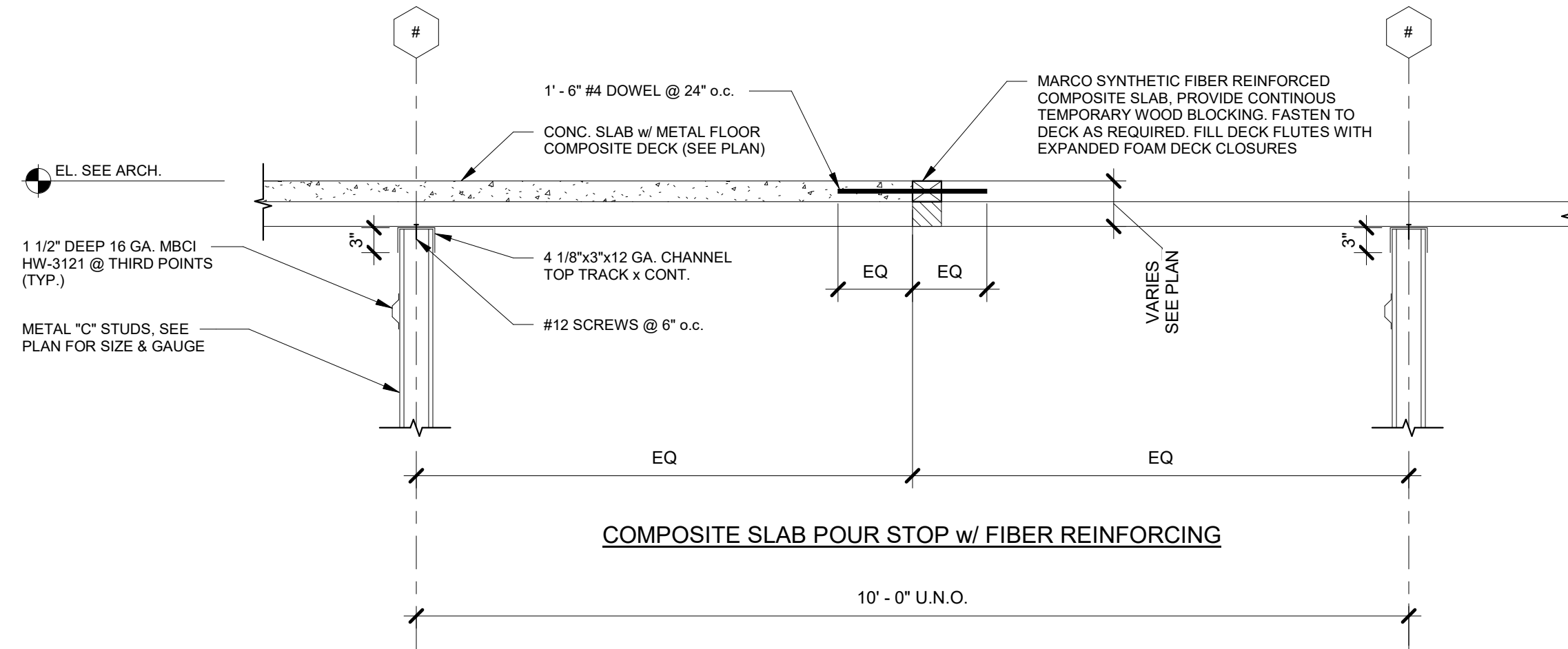
FRAMING DETAILS (BUILDING A)

Date: 04-09-2025

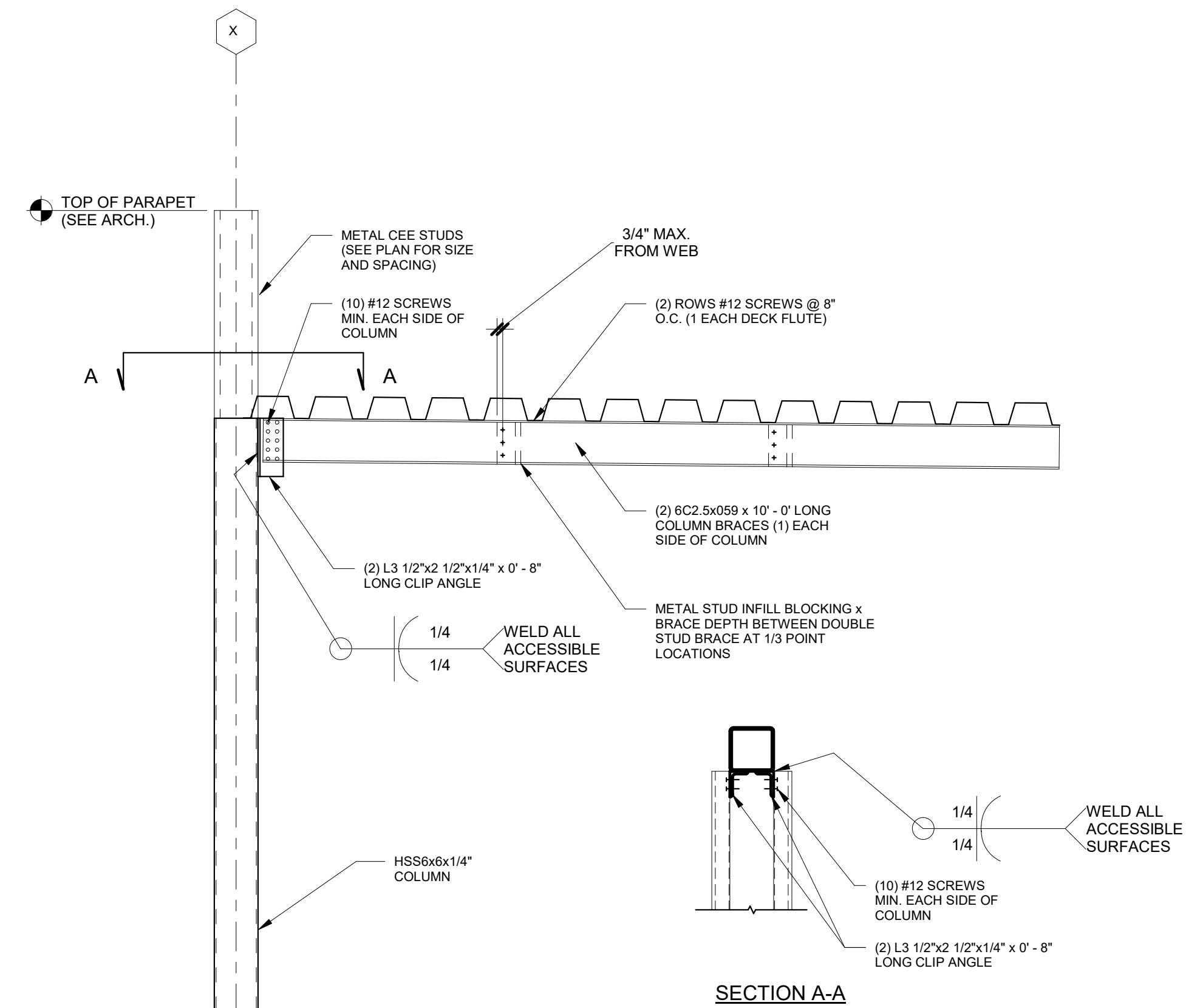
PROJECT NUMBER: 23-131

SHEET NUMBER: **S4.2A**

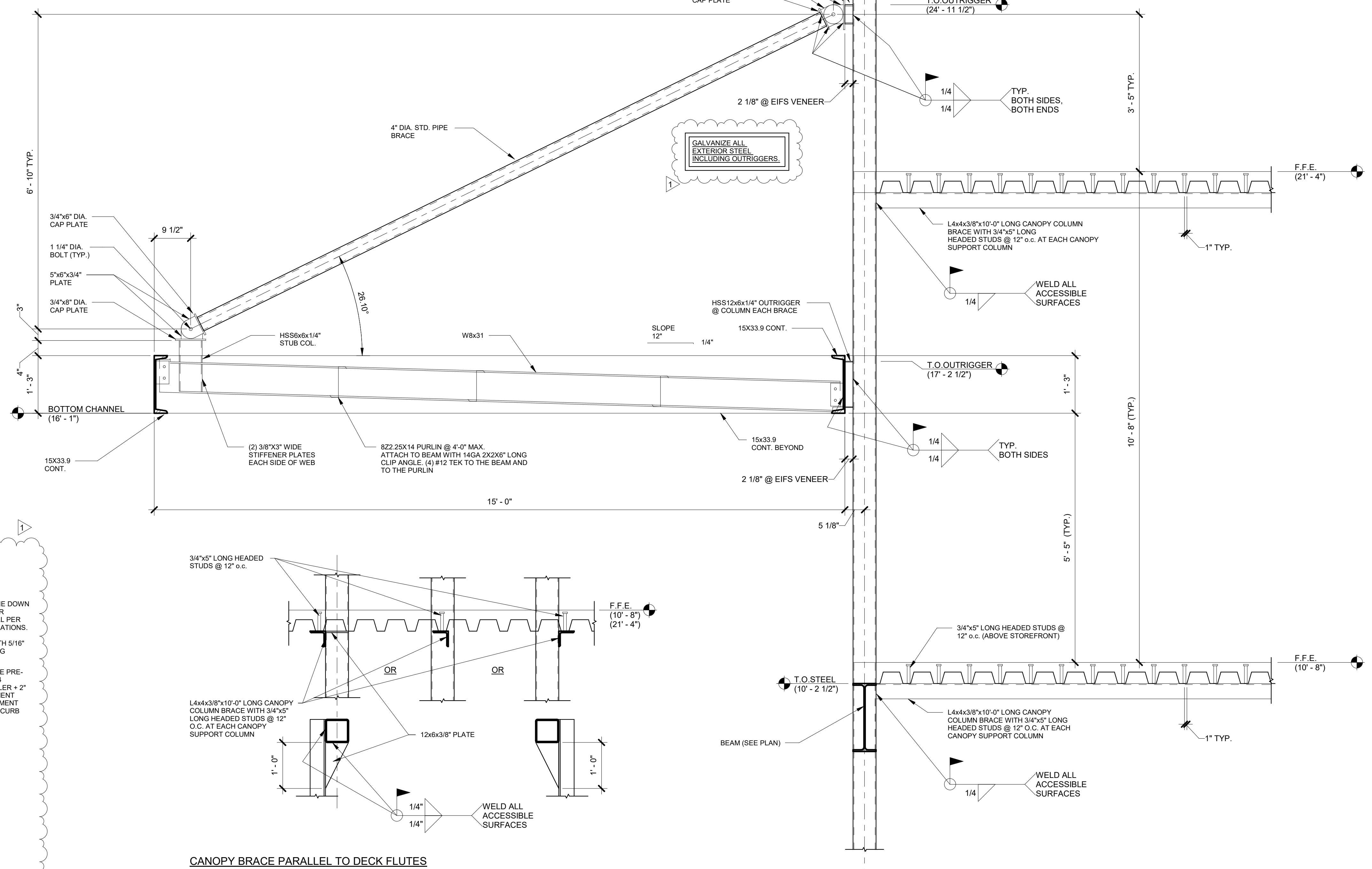




3 COMPOSITE SLAB POUR STOP DETAIL



SECTION A-A



CANOPY BRACE PARALLEL TO DECK FLUTES



1 ROOF-TOP UNIT LIGHT GAUGE CURB
S4.4A 3/4" = 1'-0"

Rev. No.	Date	Name
1	04-09-2025	Contractor Coordination
2	05-30-2025	Contractor Coordination
3	06-19-2025	Contractor Coordination

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SHEET TITLE

MASONRY DETAILS & SECTION (BUILDING A)

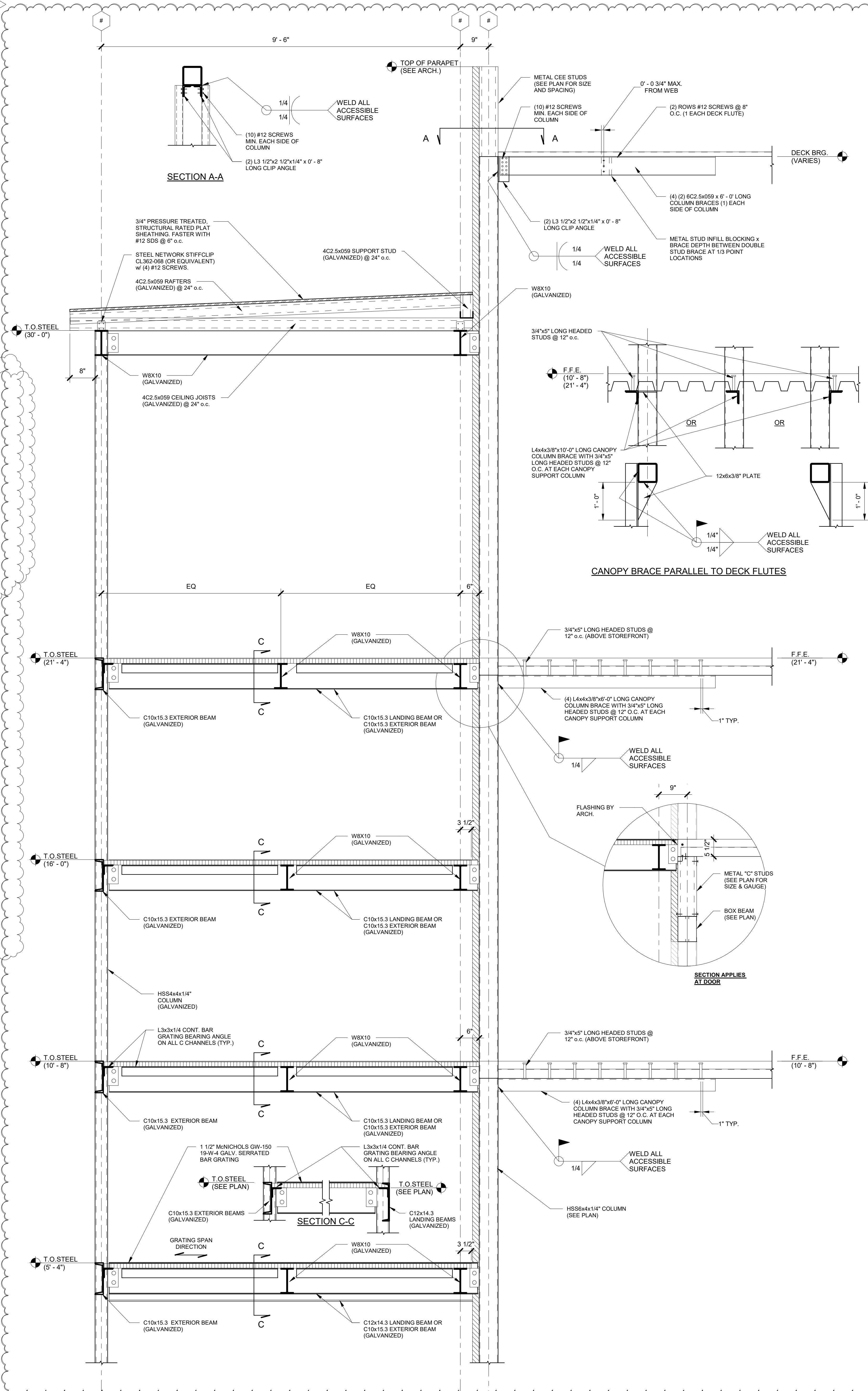
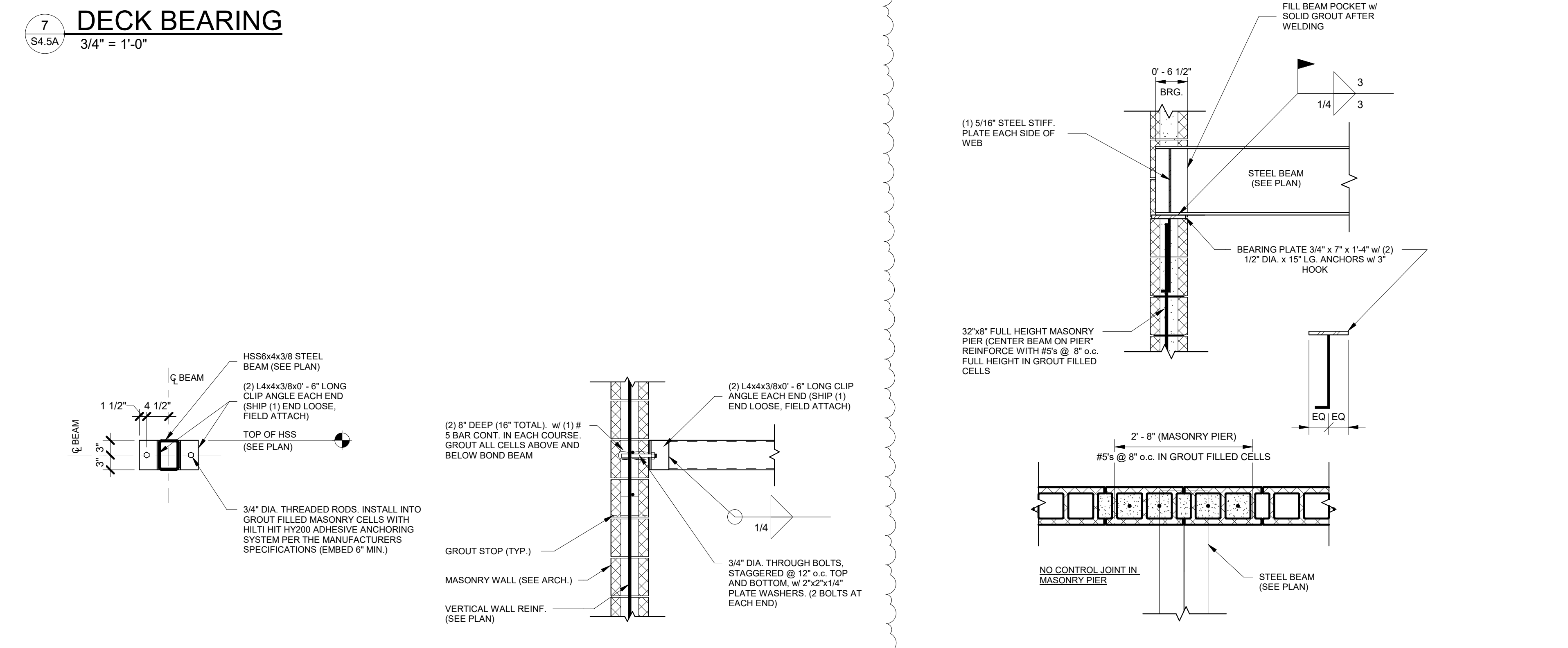
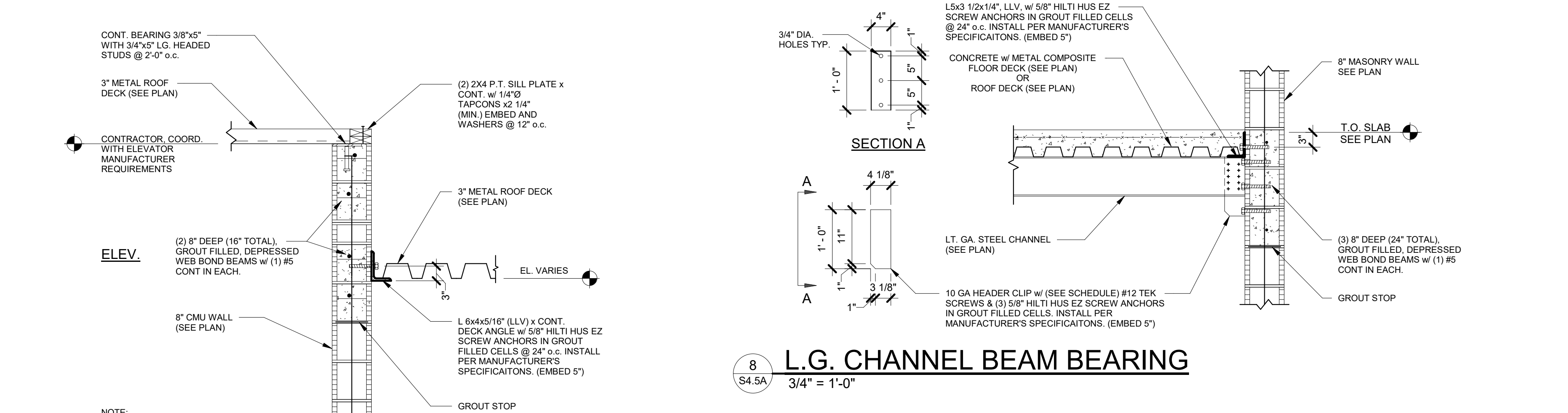
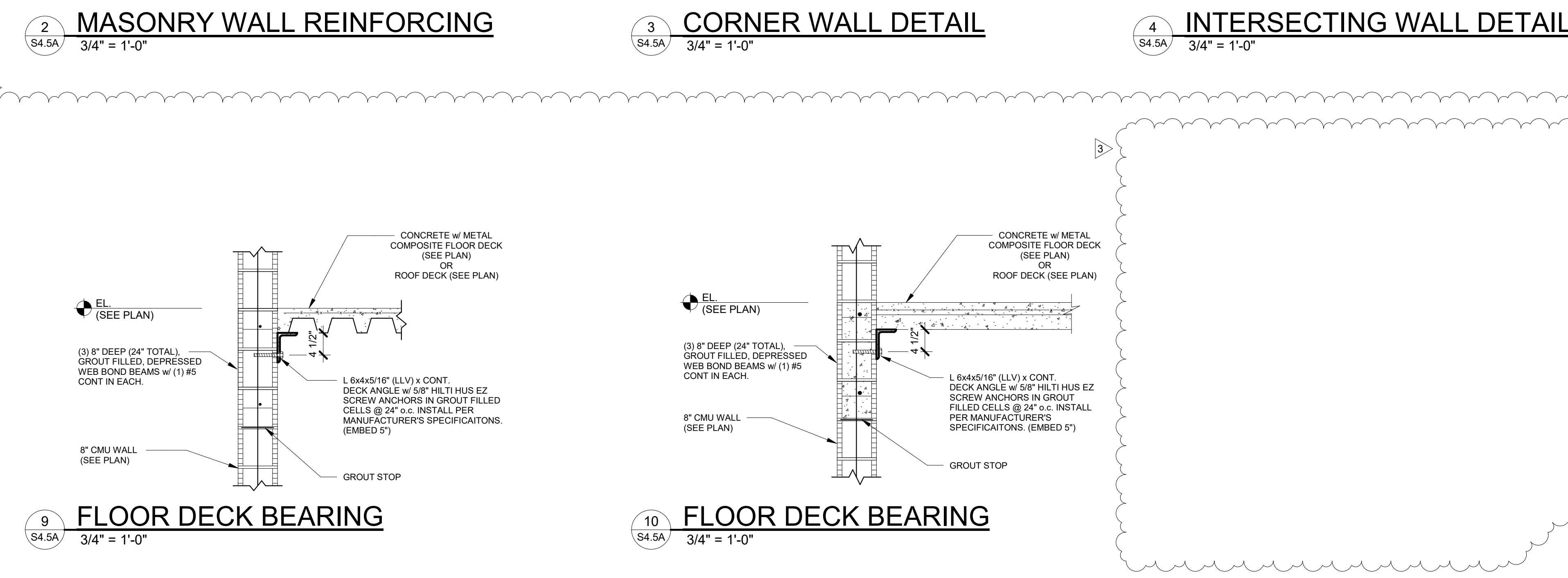
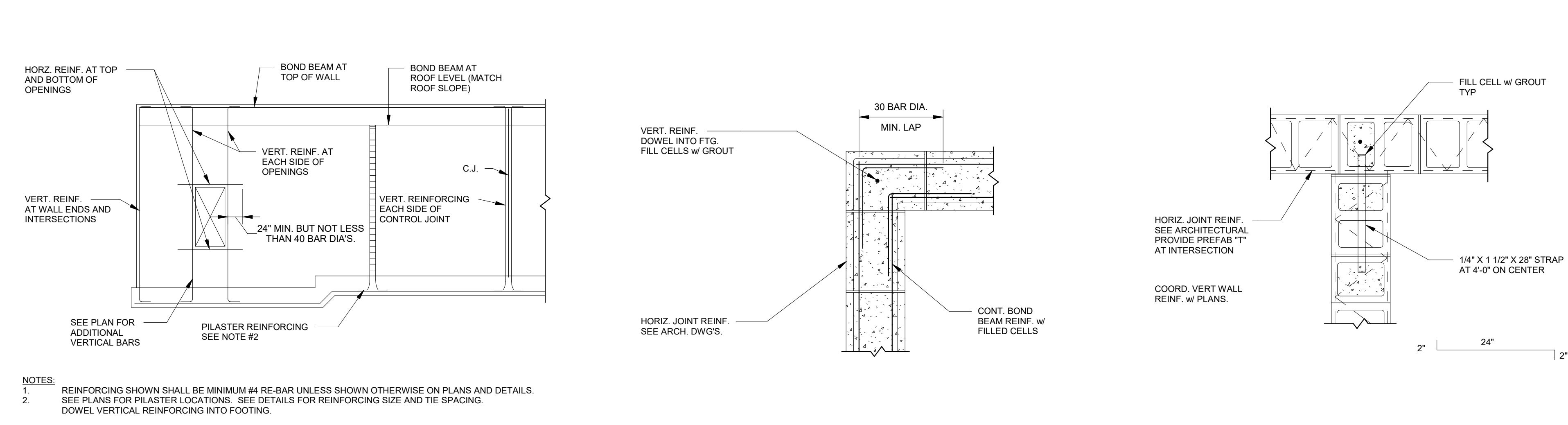
Date: 04-09-2025

PROJECT NUMBER

23-131

SHEET NUMBER

S4.5A



04/09/2025 1:14:07 PM
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3/4" = 1'-0"

Rev. No.	Date	Name
1	04-09-2025	Contractor Coordination

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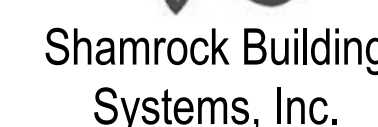
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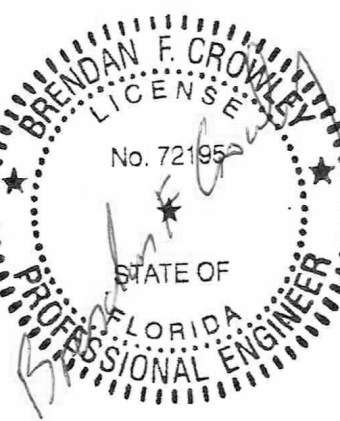
MASONRY
DETAILS &
SECTION
(BUILDING A)

Date: 04-09-2025

PROJECT NUMBER
23-131

SHEET NUMBER

S4.6A



Rev. No.	Date	Name
1	04-09-2025	Contractor Coordination

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PROJECT

19th Lane
Vero Beach
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ADDRESS
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Vero Beach, FL 32966

CLIENT



Shamrock
Building
Systems, Inc.

ADDRESS
1298 Concord Road SE
Smyrna, GA 30080

SHEET TITLE

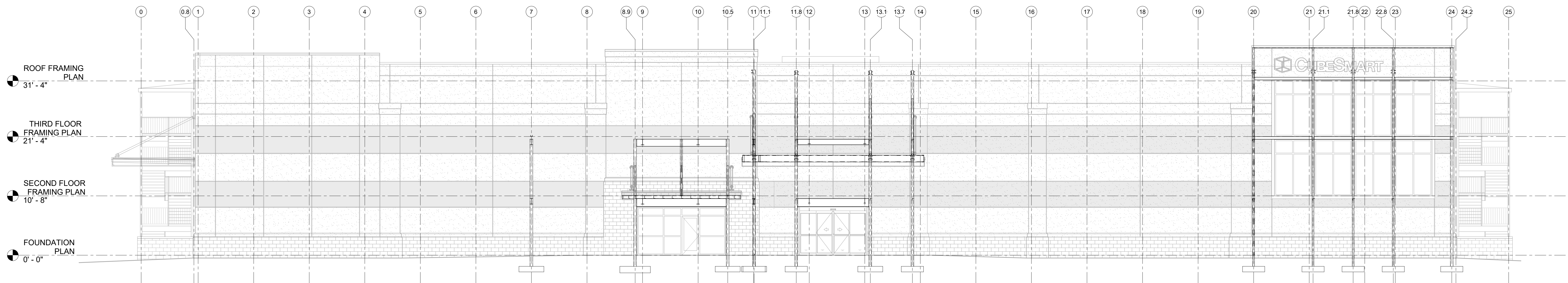
FRAMING
ELEVATIONS
(BUILDING A)

Date: 04-09-2025

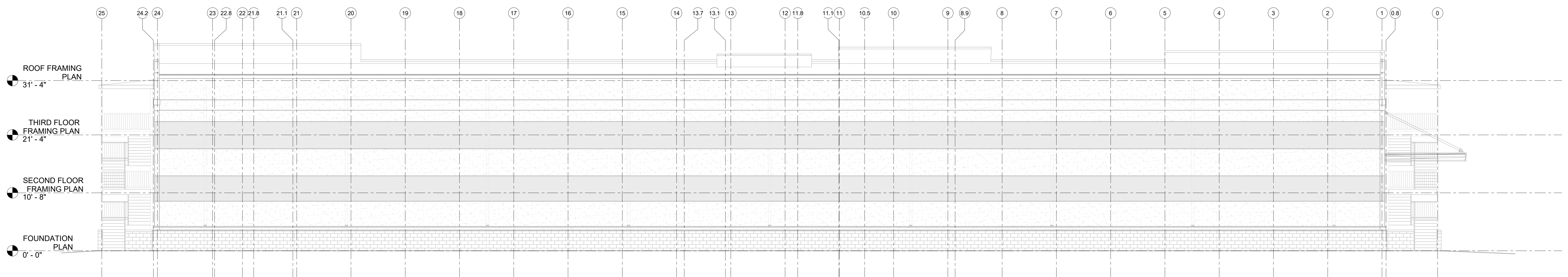
PROJECT NUMBER
23-131

SHEET NUMBER

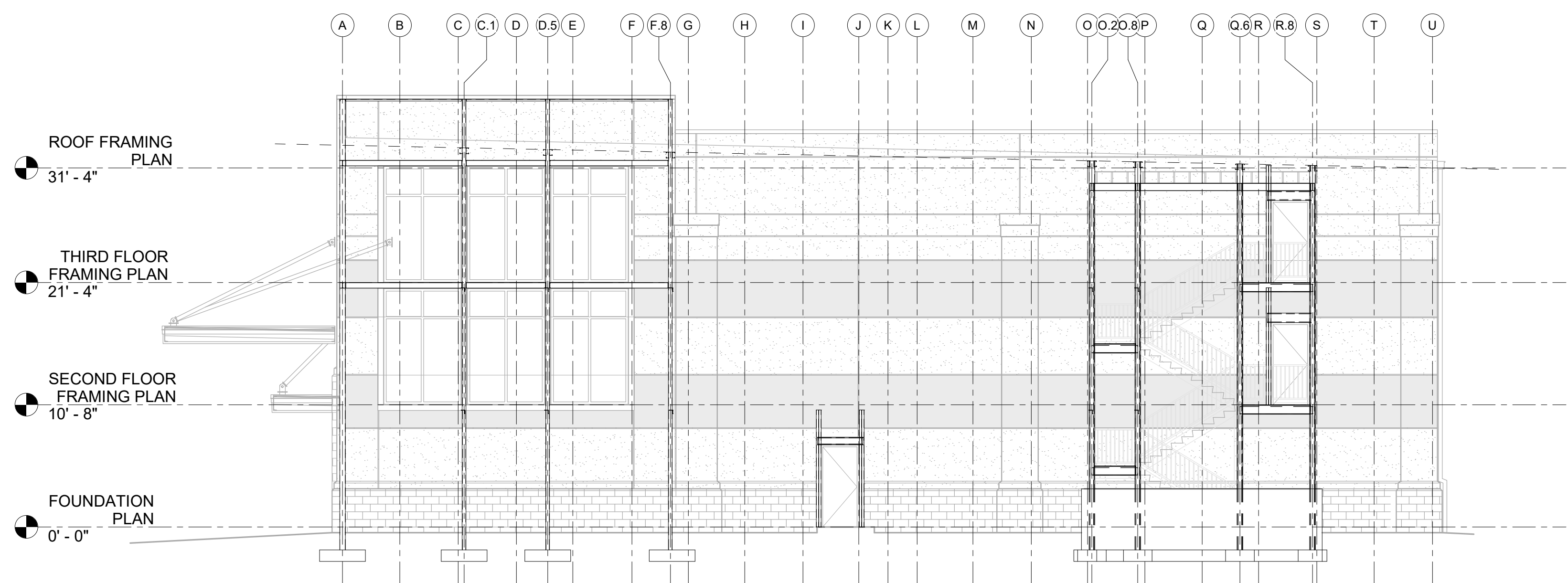
S4.7A



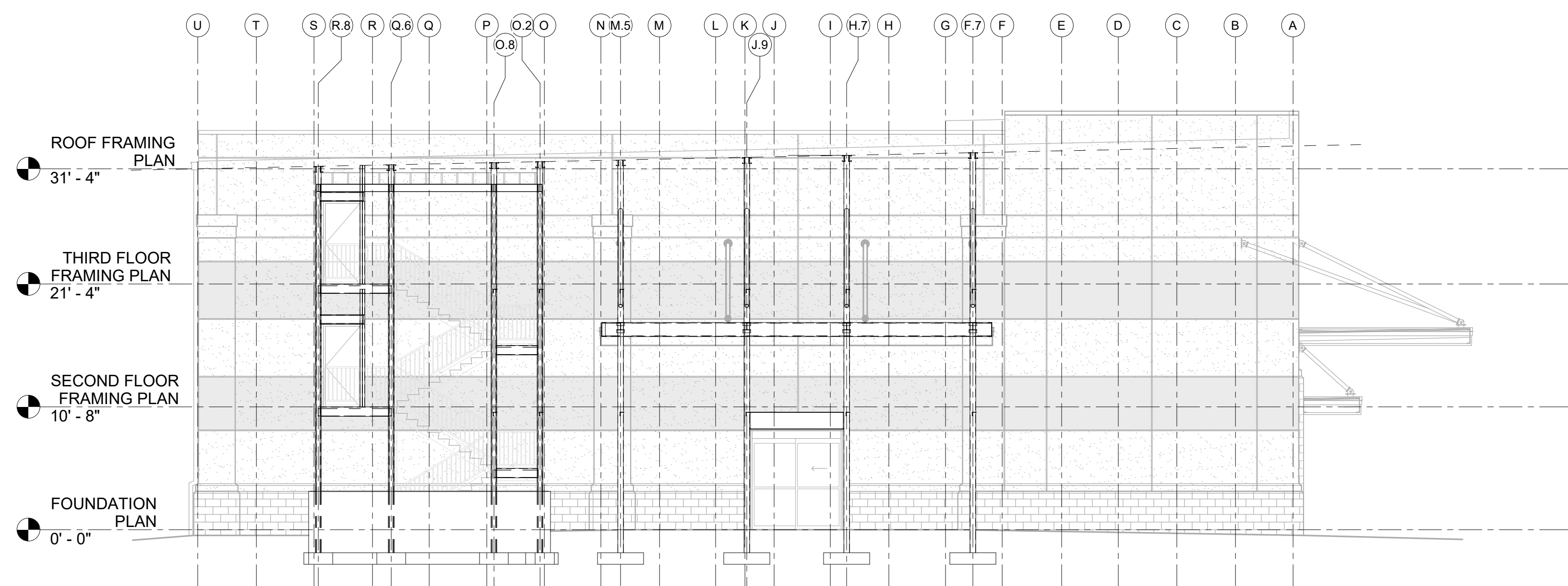
2 North
S4.7A
1/8" = 1'-0"



4 South
S4.7A
1/8" = 1'-0"



3 West
S4.7A
1/8" = 1'-0"



1 East
S4.7A
1/8" = 1'-0"

SCHEDULE OF SPECIAL INSPECTION SERVICES

SCHEDULE OF SPECIAL INSPECTION SERVICES					MATERIAL / ACTIVITY				SERVICE				APPLICABLE TO THIS PROJECT				MATERIAL / ACTIVITY				SERVICE				APPLICABLE TO THIS PROJECT				MATERIAL / ACTIVITY				SERVICE				APPLICABLE TO THIS PROJECT																							
MATERIAL / ACTIVITY					SERVICE				APPLICABLE TO THIS PROJECT				MATERIAL / ACTIVITY				SERVICE				APPLICABLE TO THIS PROJECT				MATERIAL / ACTIVITY				SERVICE				APPLICABLE TO THIS PROJECT				MATERIAL / ACTIVITY				SERVICE				APPLICABLE TO THIS PROJECT															
					Y/N				EXTENT				AGENT*				DATE COMPLETED								Y/N				EXTENT				AGENT*				DATE COMPLETED								Y/N				EXTENT				AGENT*				DATE COMPLETED			
					Y/N				EXTENT				AGENT*				DATE COMPLETED								Y/N				EXTENT				AGENT*				DATE COMPLETED								Y/N				EXTENT				AGENT*				DATE COMPLETED			
1705.1.1 Special Cases (work unusual in nature, including but not limited to alternative materials and systems, unusual design applications, materials and systems with special manufacturer's requirements)					Submittal review, shop (3) and/or field inspection				N				N/A				N/A								N				Periodic				N/A								N				Periodic				N/A											
1. Inspection of anchors post-installed in solid grouted masonry. Per research reports including verification of anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, masonry unit, grout, masonry compressive strength, anchor embedment and lightning torque					Field inspection				Y				Periodic or as required by the research report issued by an approved source				TA								Y				Periodic				as required by the research report issued by an approved source				N/A								N				Periodic				as required by the research report issued by an approved source							
2. Aggregate Pier Inspection: The special inspector's responsibilities include, but are not limited to, review of the aggregate pier designer's use of soil parameters as presented in the project soils report, and during construction, verification of aggregate properties, type and number of lifts of aggregate, hole size and depths and top elevations of the pier elements, and applied energy. Additionally, results of qualitative tests on production aggregate pier elements such as modulus load testing, split pull-out testing, bottom stabilization tests and dynamic cone penetration tests, shall be reviewed to verify compliance with design specifications.					Field inspection				N				Periodic or as required by the research report issued by an approved source				N/A								Y				Periodic				as required by the research report issued by an approved source				N/A								N				Periodic				as required by the research report issued by an approved source							
1705.2.1 Structural Steel Construction					Fabricator and erector documents (Verify reports and certificates as listed in AISC 360, Section N 3.2 for compliance with construction documents)				Submittal Review				Y				Each submittal				T/A								Y				Periodic				N/A								Y				Periodic				N/A							
2. Material verification of structural steel					Shop (3) and field inspection				Y				Periodic				T/A								Y				Periodic				N/A								Y				Periodic				N/A											
3. Structural steel welding:					Shop (3) and field inspection				Y				Observe or Perform as noted (4)				T/A								Y				Observe or Perform as noted (4)				T/A								Y				Observe or Perform as noted (4)				T/A											
a. Inspection tasks Prior to Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-1)					Shop (3) and field inspection				Y				Observe or Perform as noted (4)				T/A								Y				Observe or Perform as noted (4)				T/A								Y				Observe or Perform as noted (4)				T/A											
b. Inspection tasks During Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-2)					Shop (3) and field inspection				Y				Observe (4)				T/A								Y				Observe (4)				T/A								Y				Observe (4)				T/A											
c. Inspection tasks After Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-3)					Shop (3) and field inspection				Y				Observe or Perform as noted (4)				T/A								Y				Observe or Perform as noted (4)				T/A								Y				Observe or Perform as noted (4)				T/A											
d. Nondestructive testing (NDT) of welded joints:					Shop (3) or field ultrasonic testing - 100%				N				Periodic				N/A								Y				Periodic				N/A								Y				Periodic				N/A											
1) Complete penetration groove welds 5/16" or greater in risk category II or IV					Shop (3) or field ultrasonic testing - 100%				N				Periodic				N/A								Y				Periodic				N/A								Y				Periodic				N/A											
2) Complete penetration groove welds 5/16" or greater in risk category II					Shop (3) or field ultrasonic testing - 10% of welds minimum				Y				Periodic				TA								Y				Periodic				TA								Y				Periodic				TA											
3) Welded joints subject to fatigue when required by AISC 360, Appendix 3, Table A-3.1					Shop (3) or field radiographic or Ultrasonic testing				N				Periodic				N/A								Y				Periodic				N/A								Y				Periodic				N/A											
4) Fabricator's NDT reports when fabricator performs NDT					Verify Reports				Y				Each submittal (5)				TA								Y				Each submittal (5)				TA								Y				Each submittal (5)				TA											
4. Structural steel bolting:					Shop (3) and field inspection				Y				Observe or Perform as noted (4)				T/A								Y				Observe or Perform as noted (4)				T/A								Y				Observe or Perform as noted (4)				T/A											
a. Inspection tasks Prior to Bolting (Observe, or perform tasks for each bolted connection, in accordance with QA tasks listed in AISC 360, Table N5.6-1)					Shop (3) and field inspection				Y				Observe or Perform as noted (4)				T/A								Y				Observe or Perform as noted (4)				T/A								Y				Observe or Perform as noted (4)				T/A											
b. Inspection tasks During Bolting (Observe the QA tasks listed in AISC 360, Table N5.6-2)					Shop (3) and field inspection				Y				Observe (4)				T/A								Y				Observe (4)				T/A								Y				Observe (4)				T/A											
1) Pre-tensioned and slip-critical joints					Shop (3) and field inspection				N				Periodic				N/A								Y				Periodic				N/A								Y				Periodic				N/A											
a) Turn-of-nut with matching markings					Shop (3) and field inspection				N				Periodic				N/A								Y				Periodic				N/A								Y				Periodic				N/A											
b) Direct tension indicator					Shop (3) and field inspection				N				Periodic				N/A								Y				Periodic				N/A								Y				Periodic				N/A											
c) Twist-off type tension control bolt					Shop (3) and field inspection				N				Continuous				N/A								Y				Continuous				N/A								Y				Continuous				N/A											
d) Turn-of-nut without matching markings					Shop (3) and field inspection				N				Continuous				N/A								Y				Continuous				N/A								Y				Continuous				N/A											
e) Calibrated wrench					Shop (3) and field inspection				N				Continuous				N/A								Y				Continuous				N/A								Y				Continuous				N/A											
2) Snug-tight joints					Shop (3) and field inspection				Y				Periodic				TA								Y				Periodic				TA								Y				Periodic				TA											
c. Inspection tasks After Bolting (Perform tasks for each bolted connection in accordance with QA tasks listed in AISC 360, Table N5.6-3)					Shop (3) and field inspection				Y				Perform (4)				T/A								Y				Perform (4)				T/A								Y				Perform (4)				T/A											
5. Visual inspection of exposed cut surfaces of galvanized structural steel main members and exposed corners of the rectangular HSS for cracks subsequent to galvanizing					Shop (3) and field inspection				N				Periodic				N/A								Y				Periodic				N/A								Y				Periodic				N/A											
6. Embedments (Verify diameter, grade, type, length, embedment, See 1705.3 for anchors)					Field inspection				Y				Periodic				T/A								Y				Periodic				T/A								Y				Periodic				T/A											
7. Verify member locations, bracing, stiffeners, and application of joint details at each connection comply with construction documents					Field inspection				Y				Periodic				T/A								Y				Periodic				T/A								Y				Periodic				T/A											
1705.2.2 Cold-Formed Steel Deck					Manufacturer documents (Verify reports and certificates as listed in SDI Q/AOC, Section 2, Paragraphs 2.1 and 2.2 for compliance with construction documents)				Submittal Review				Y				Each submittal				T/A								Y				Each submittal				T/A								Y				Each submittal				T/A							
2. Material verification of steel deck, mechanical fasteners and welding materials					Shop (3) and field inspection				Y				Periodic				T/A								Y				Periodic				T/A								Y				Periodic				T/A											
3. Cold-formed steel deck placement:					Shop (3) and field inspection				Y				Perform (4)				T/A								Y				Perform (4)				T/A								Y				Perform (4)				T/A											
a. Inspection tasks Prior to Deck Placement (Perform the QA tasks listed in SDI Q/AOC, Appendix 1 Table 1.1)					Shop (3) and field inspection				Y				Perform (4)				T/A								Y				Perform (4)				T/A								Y				Perform (4)				T/A											
b. Inspection tasks After Deck Placement (Perform the QA tasks listed in SDI Q/AOC, Appendix 1 Table 1.2)					Shop (3) and field inspection				Y				Perform (4)				T/A								Y				Perform (4)				T/A								Y				Perform (4)				T/A											
4. Cold-formed steel deck welding:					Shop (3) and field inspection				N				Observe (4)				N/A								Y				Observe (4)				N/A								Y				Observe (4)				N/A											
a. Inspection tasks Prior to Mechanical Fastening (Observe the QA tasks listed in SDI Q/AOC, Appendix 1 Table 1.6)					Shop (3) and field inspection				Y				Observe (4)				T/A								Y				Observe (4)				T/A								Y				Observe (4)				T/A											
b. Inspection tasks During Mechanical Fastening (Observe the QA tasks listed in SDI Q/AOC, Appendix 1 Table 1.7)					Shop (3) and field inspection				Y				Observe (4)				T/A								Y				Observe (4)																															