

STRUCTURAL NOTES:
ELECTRONIC VERSIONS OF STRUCTURAL DRAWINGS ARE THE SOLE, COPYRIGHTED PROPERTY OF BENNETT & PLESS, INC. ELECTRONIC VERSIONS OF DRAWINGS ARE NOT TO BE USED OR TRANSFERRED WITHOUT THE EXPRESS, WRITTEN PERMISSION OF BENNETT & PLESS, INC.

DO NOT SCALE OFF THESE DRAWINGS. LAYOUT THE BUILDING USING APPLICABLE CONTRACT DRAWINGS. BUILDING DIMENSIONS AND THE LOCATION OF ALL STRUCTURAL ELEMENTS INCLUDING BUT NOT LIMITED TO SLAB DEGS, WALLS, COLUMNS, OPENINGS, AND DEPRESSIONS SHALL BE COORDINATED WITH THE ARCHITECTURAL DRAWINGS PRIOR TO LAYOUT OF THE BUILDING. RESOLVE ANY DISCREPANCIES.

TO THE BEST OF THE ENGINEER-OF-RECORD'S KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE DOCUMENTS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.

GENERAL NOTES:
STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.

THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCES TO INSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS.

DESIGN LOADS:
GENERAL SYSTEM FOR THIS BUILDING HAS BEEN DESIGNED IN ACCORDANCE WITH THE 2023 FLORIDA BUILDING CODE, 8TH EDITION (2021 IBC), THE FOLLOWING SUPERIMPOSED LOADINGS HAVE BEEN UTILIZED:

ROOF (JOISTS):
LIVE LOAD - 20 psf
DEAD LOAD - 25 psf. (5 PSF ALLOTTED FOR FUTURE SOLAR PANELS.)
THE ROOF DESIGN COMPLIES WITH FBC SECTION 1607.12.5.
WHEN THE FUTURE SOLAR PANELS ARE TO BE INSTALLED, THEY MUST BE SUBMITTED AS A SEPARATE PERMIT WITH CALCULATIONS.
DEAD LOAD - 5 psf. (AVAILABLE TO RESIST UPLIFT
WIND:
SPEED = $V_{ult} = 135$ MPH, $V_{(asd)} = 105$ MPH
EXPOSURE C, RISK CATEGORY II, INT. COEFF. ± 0.18
THIS STRUCTURE IS DESIGNED AS AN ENCLOSED STRUCTURE.
SNOW/SEISMIC:
NOT REQUIRED PER 2023 FLORIDA BUILDING CODE 8TH ED. SECTION 101.2 EXCEPTION 2

SHOP DRAWING REVIEW:
SHOP DRAWINGS WILL BE PROVIDED FOR ALL WORK AND WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC. SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR AND OWNER'S REP PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. DRAWINGS SUBMITTED WITHOUT REVIEW WILL BE RETURNED UNCHECKED. SHOP DRAWING SUBMITTALS, IF NOT SUBMITTED ELECTRONICALLY, SHALL INCLUDE THREE SETS OF PRINTS. ONE SET OF PRINTS WILL BE RETAINED BY THE ENGINEER, ONE BY THE ARCHITECT, ONE BY THE LOCAL BUILDING DEPARTMENT (WHERE REQUIRED) AND THE CONTRACTOR SHALL MAKE PRINTS AS REQUIRED FOR DISTRIBUTION. IN ALL INSTANCES THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED IN WRITING BY THE ENGINEER.

SHOP DRAWINGS FOR ALL STRUCTURAL ELEMENTS SHALL BE SUBMITTED TO B&P WITH 10 BUSINESS DAYS AS A MINIMUM TIME TO BE REVIEWED (MAXIMUM SHEET OR PAGE COUNT OF 30). IN THE CASE OF A LARGE SUBMITTAL, OR MORE THAN 1 SUBMITTAL FOR THE SAME PROJECT, AN ADDITIONAL BUSINESS DAY SHALL BE REQUIRED FOR EVERY 5 PAGES/SHEETS OVER 30. THE TIME INDICATED ABOVE IS FOR B&P'S REVIEW ONLY. IN ORDER TO MEET PROJECT SCHEDULES, THE CONTRACTOR MUST INCLUDE ENOUGH TIME FOR DELIVERY, ARCHITECTURAL REVIEW, AND OWNER'S REVIEW.

THERE SHALL BE NO DEVIATION FROM THESE STRUCTURAL CONSTRUCTION DOCUMENTS. IF THE CONTRACTOR OR PROVIDER OF THE SHOP DRAWINGS PROPOSE ANY CHANGES, THOSE CHANGES SHALL BE CLEARLY INDICATED, AND SIGNED AND SEALED DRAWINGS AND CALCULATIONS BY A REGISTERED SOUTH CAROLINA PROFESSIONAL ENGINEER SUBMITTED. ANY CHANGES NOT APPROVED OR SUBMITTED WITHOUT THE PROPER DOCUMENTATION INDICATED PREVIOUSLY, WILL RESULT IN REVISIONS BY THE ENGINEER OR ARCHITECT-OF-RECORD. THE COST FOR SAID REVISIONS, INCLUDING ENGINEERING AND ARCHITECTURAL FEES SHALL BE BORNE BY THE CONTRACTOR.

SHOP DRAWINGS FOR SPECIALTY ENGINEERED PRODUCTS:
ALL SPECIALTY ENGINEERED PRODUCTS OR SYSTEMS REQUIRE SIGNED & SEALED CALCULATIONS AND FABRICATION AND ERECTION DRAWINGS PREPARED BY A DELEGATED ENGINEER.

THIS INCLUDES BUT IS NOT LIMITED TO CANOPIES AND THEIR FOUNDATIONS, PRE-ENGINEERED TRUSSES, ICF WALL SYSTEMS, ALL LIGHT GAUGE METAL STUD FRAMING SYSTEMS, ALUMINUM WALL SYSTEMS, GLAZED CURTAIN WALLS, PREFABRICATED STEEL STAIRS & RAILINGS, ARCHITECTURAL PRECAST CONCRETE ELEMENTS, STRUCTURAL PRECAST OR TILT-UP SYSTEMS, GLASS FIBER REINFORCED CONCRETE PANEL SYSTEMS, OPEN WEB STEEL JOISTS, STRUCTURAL STEEL CONNECTIONS REQUIRING ENGINEERING, TILT-WALL ERECTION DRAWINGS, GLULAM BEAMS, TECTUM PLANKS, PEMB, ETC.

SUBMITTALS SHALL CLEARLY IDENTIFY THE SPECIFIC PROJECT AND APPLICABLE CODES, LIST THE DESIGN CRITERIA, AND SHOW ALL DETAILS AND PLANS NECESSARY FOR PROPER FABRICATION AND INSTALLATION. CALCULATIONS AND SHOP DRAWINGS SHALL IDENTIFY SPECIFIC PRODUCT UTILIZED. GENERIC PRODUCTS WILL NOT BE ACCEPTED.

SHOP DRAWINGS AND CALCULATIONS SHALL BE PREPARED UNDER THE DIRECT SUPERVISION AND CONTROL OF THE DELEGATED ENGINEER. SHOP DRAWINGS AND CALCULATIONS REQUIRE THE IMPRESSED SEAL, DATE AND SIGNATURE OF THE DELEGATED ENGINEER.

COMPUTER PRINTOUTS ARE AN ACCEPTABLE SUBSTITUTE FOR MANUAL COMPUTATIONS PROVIDED THEY ARE ACCOMPANIED BY SUFFICIENT DESCRIPTIVE INFORMATION TO PERMIT THEIR PROPER EVALUATION. SUCH DESCRIPTIVE INFORMATION SHALL BEAR THE IMPRESSED SEAL AND SIGNATURE OF THE DELEGATED ENGINEER AS AN INDICATION THAT HE/SHE HAS ACCEPTED RESPONSIBILITY FOR THE RESULTS. SEPIAS DO NOT REQUIRE SIGNATURE AND SEAL. THE STRUCTURAL ENGINEER WILL RETAIN ONE SIGNED AND SEALED BLUELINE PRINT FOR RECORD.

DRAWINGS PREPARED SOLELY TO SERVE AS A GUIDE FOR FABRICATION AND INSTALLATION (SUCH AS REINFORCING STEEL SHOP DRAWINGS OR STRUCTURAL STEEL ERECTION DRAWINGS) AND REQUIRING NO ENGINEERING DO NOT REQUIRE THE SEAL OF A DELEGATED ENGINEER.

CATALOG INFORMATION ON STANDARD PRODUCTS DOES NOT REQUIRE THE SEAL OF A DELEGATED ENGINEER.

REVIEW BY THE STRUCTURAL ENGINEER OF RECORD OF SUBMITTALS IS LIMITED TO VERIFYING THE FOLLOWING:

- THAT THE SPECIFIED STRUCTURAL SUBMITTALS HAVE BEEN FURNISHED.
- THAT THE STRUCTURAL SUBMITTALS HAVE BEEN SIGNED AND SEALED BY THE DELEGATED ENGINEER.
- THAT THE DELEGATED ENGINEER HAS UNDERSTOOD THE DESIGN INTENT AND HAS USED THE SPECIFIED STRUCTURAL CRITERIA. (NO DETAILED CHECK OF CALCULATIONS WILL BE MADE).
- THAT THE CONFIGURATION SET FORTH IN THE STRUCTURAL SUBMITTALS IS CONSISTENT WITH THE CONTRACT DOCUMENTS. (NO DETAILED CHECK OF DIMENSIONS OR QUANTITIES WILL BE MADE).

SUBMITTALS NOT MEETING THE CRITERIA LISTED IN THIS SECTION WILL NOT BE REVIEWED.

FOUNDATIONS:
SEE THE FOLLOWING REPORT FOR COMPLETE GEOTECHNICAL RECOMMENDATIONS AND INSTALLATION PROCEDURES.

REPORT No.: 41976
PREPARED BY: GEOVIEW ASSOCIATES, INC.
TITLED: FINAL REPORT GEOPHYSICAL INVESTIGATION, US HIGHWAY 27 AND CR25 LADY LAKE, FLORIDA
DATED: OCTOBER 15, 2024

THIS REPORT SHALL BE CONSIDERED PART OF THE CONTRACT DOCUMENTS
FOUNDATION DESIGN IS BASED ON A SOIL BEARING PRESSURE OF 2000 PSF.

FOUNDATION DESIGN BASED ON 1" TOTALS AND 1/2" DIFFERENTIAL SETTLEMENT. REFER TO GEOTECHNICAL REPORT FOR NECESSARY SITE REMEDIATION.

FORMWORK AND SHORING:
NO STRUCTURAL CONCRETE SHALL BE STRIPPED UNTIL IT HAS REACHED AT LEAST TWO THIRDS OF THE 28 DAY DESIGN STRENGTH. DESIGN, ERECTION AND REMOVAL OF ALL FORMWORK, SHORES AND RESHORES SHALL MEET THE REQUIREMENTS SET FORTH IN ACI STANDARDS 347 AND 301.

PLUMBING SLEEVE:
MINIMUM SLEEVE SPACING SHALL BE THREE DIAMETERS CENTER TO CENTER OF THE LARGER SLEEVE OR 6" CLEAR BETWEEN SLEEVES, WHICHEVER IS GREATER. PRIOR TO CONSTRUCTION SLEEVE LOCATIONS AND SIZES SHALL BE APPROVED BY THE ENGINEER.

EMBEDDED CONDUITS:
LOCATIONS AND SIZES OF CONDUIT MUST BE APPROVED BY THE ENGINEER PRIOR TO PLACEMENT. WITHIN SLABS, BEAMS OR WALLS, CONDUIT SHALL OCCUPY ONLY THE MIDDLE ONE THIRD OF THE MEMBER DEPTH OR THICKNESS. MAXIMUM CONDUIT O.D. FOR SINGLE CONDUITS OR SUM OF O.D.'S FOR MULTIPLE CONDUITS THAT CROSS SHALL BE NO LARGER THAN ONE THIRD THE SLAB DEPTH. PARALLEL CONDUITS SHALL BE SPACED WITH A MINIMUM OF 3 DIAMETERS CLEAR. CONDUITS SHALL BE A MINIMUM OF ONE DIAMETER AWAY FROM AND SHALL NOT INTERFERE WITH OR DISPLACE ANY REINFORCING. CONDUIT SHALL NOT BE TIED TO REINFORCING. CONDUITS SHALL NOT OCCUR WITHIN COLUMN ZONES OF SLABS AND OR TRANSFER GIRDERS. CONDUIT PLACEMENT SHALL NOT IMPAIR THE STRENGTH OF THE CONSTRUCTION AS JUDGED BY THE ENGINEER.

REINFORCING STEEL:
SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. SECURE APPROVAL OF SHOP DRAWINGS PRIOR TO COMMENCING FABRICATION.

WELDED WIRE FABRIC:
TO CONFORM TO ASTM A-185, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. MINIMUM LAP SHALL BE ONE SPACE PLUS TWO INCHES. USE OF FLAT MANUFACTURED SHEETS IS RECOMMENDED.

CONCRETE:
SHALL BE PER AN APPROVED MIX DESIGN PROPORTIONED TO ACHIEVE A STRENGTH AT 28 DAYS AS LISTED BELOW WITH A PLASTIC AND WORKABLE MIX:

4000psi FOR ALL CONCRETE

CONCRETE SHALL BE PLACED AND CURED ACCORDING TO ACI STANDARDS AND SPECIFICATIONS.

SUBMIT PROPOSED MIX DESIGN WITH RECENT FIELD CYLINDER OR LAB TESTS FOR REVIEW PRIOR TO USE. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. MIX SHALL MEET THE REQUIREMENTS OF ASTM C33 FOR COARSE AGGREGATE. CONCRETE SHALL COMPLY WITH THE REQUIREMENTS OF ASTM STANDARD C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME STAMPED WHEN CONCRETE IS BATCHED. THE MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE HALF (1-1/2) HOURS. IF FOR ANY REASON THERE IS A LONGER DELAY THAN THAT STATED ABOVE, THE CONCRETE SHALL BE DISCARDED. IT SHALL BE THE RESPONSIBILITY OF THE TESTING LAB TO NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NONCOMPLIANCE WITH THE ABOVE. SLABS SHALL BE CURED USING A DISPERSING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE 1-D AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE. SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAILY. CALCIUM CHLORIDES SHALL NOT BE UTILIZED; OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE ENGINEER.

CONSTRUCTION OR CONTROL JOINTS SHALL BE PROVIDED IN SLABS ON GRADE SO THAT THE MAXIMUM AREA OF THE SLAB BETWEEN JOINTS SHALL BE 225 SQUARE FEET, OR AS SHOWN ON THE PLANS. SAW CUT CONTROL JOINTS SHALL BE MADE AS SOON AS SLAB WILL SAFELY SUPPORT MEN AND EQUIPMENT AND THE SLAB WILL NOT BE DAMAGED BY EQUIPMENT, BUT NO LATER THAN 24 HOURS. ASPECT RATIO (LONGSIDE TO SHORTSIDE OF CONCRETE AREA) SHALL NOT EXCEED 1.5. NO EMBEDDED ANGLES OR OTHER FIXED METAL ITEMS SHALL EXTEND THROUGH JOINTS, UNLESS OTHERWISE NOTED. EMBEDDED ANGLES AND OTHER FIXED METAL ITEMS SHALL BE CONTINUOUS BETWEEN CONCRETE JOINTS, UNLESS OTHERWISE NOTED. ENGINEER SHALL APPROVE LOCATION OF ALL JOINTS NOT SHOWN ON DRAWINGS. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR APPROVAL.

CONCRETE MIX DESIGNS SHALL INCLUDE A WRITTEN DESCRIPTION INDICATING WHERE EACH PARTICULAR MIX IS TO BE PLACED WITHIN THE STRUCTURE. IF ACCEPTED, PEA ROCK PUMP MIX USE IS LIMITED TO VERTICAL ELEMENT POURS AND BEAM POURS LESS THAN 60 LINEAL FEET PER POUR.

CONCRETE DESIGN MIX SUBMITTALS SHALL INCLUDE TESTED, STATISTICAL BACK-UP DATA AS PER CHAPTER 4 OF ACI 301.

WATER/CEMENT RATIO FOR CONCRETE AT EXTERIOR BALCONIES OR CONCRETE EXPOSED TO WEATHER SHALL NOT EXCEED 0.40 BY WEIGHT.

CONCRETE COVER FOR REINFORCING		
CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH		3"
CONCRETE EXPOSED TO EARTH OR WEATHER	#6 THRU #18 BARS	2"
	#5 BAR, W31 OR D31 WIRE, AND SMALLER	1 1/2"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER:		
SLABS AND WALLS:		
	#11 & SMALLER	3/4"
BEAMS, COLUMNS:		
	PRIMARY REINF., TIES, STIRRUPS	1 1/2"

CONCRETE TESTING:
AN INDEPENDENT TESTING LABORATORY SHALL PERFORM THE FOLLOWING TESTS ON CAST IN PLACE CONCRETE:

- ASTM C143 - "STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE." SLUMP RANGE SHALL BE 3 TO 4 INCHES.
- ASTM C39 - "STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS." A SEPARATE TEST SHALL BE CONDUCTED FOR EACH CLASS, FOR EVERY 50 CUBIC YARDS (OR FRACTION THEREOF), PLACED PER DAY. CYLINDER(S) QUANTITIES AND TEST AGE AS FOLLOWS:
1 AT 7 DAYS
2 AT 28 DAYS

ONE ADDITIONAL RESERVE CYLINDER TO BE TESTED UNDER THE DIRECTION OF THE ENGINEER, IF REQUIRED. IF 28 DAY STRENGTH IS ACHIEVED, THE ADDITIONAL CYLINDER(S) MAY BE DISCARDED.

PENETRATIONS:
NO PENETRATIONS SHALL BE MADE IN ANY STRUCTURAL MEMBERS OTHER THAN THOSE LOCATED ON THESE DRAWINGS WITHOUT PREVIOUS APPROVAL OF THE ENGINEER.

MASONRY WALLS:
MASONRY UNITS SHALL MEET ASTM C 90 FOR HOLLOW LOAD BEARING TYPE MASONRY WITH UNIT STRENGTH OF 1900 psi ON THE NET AREA ($f_m = 1900$ psi). MORTAR SHALL BE TYPE "M" OR "S" AND MEET ASTM C 270. GROUT SHALL BE 2000 psi MINIMUM COMPRESSIVE STRENGTH AND MEET ASTM C 476. PROVIDE HOOKED DOWELS IN FOOTINGS FOR VERTICAL REINFORCING ABOVE. LAP SPLICES 48 BAR DIAMETERS.

BLOCK CELLS SHALL BE GROUT FILLED WITH VERTICAL REINFORCING BARS AT CORNERS, INTERSECTIONS, EACH SIDE OF OPENINGS AND WALL CONTROL JOINTS AND AS SHOWN ON THE PLANS. DOWELS SHALL BE USED TO PROVIDE CONTINUITY INTO THE STRUCTURE ABOVE AND/OR BELOW, UNLESS NOTED OTHERWISE. USE METAL LATH, MORTAR, OR SPECIAL UNITS TO CONFIN CONCRETE AND GROUT TO AREA REQUIRED. MASONRY SHALL BE LAID IN RUNNING BOND PATTERN UNLESS NOTED OTHERWISE.

PROVIDE 9 GAGE GALVANIZED HORIZONTAL JOINT REINFORCING (DUR O WALL OR ENGINEER APPROVED SUBSTITUTION) AT ALTERNATE BLOCK COURSES. OVERLAP JOINT REINF. @ CORNERS.

SUBMIT PROPOSED GROUT MIX DESIGN FOR REVIEW PRIOR TO USE. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. GROUT SLUMP SHALL BE BETWEEN 8 AND 11 INCHES. USE OF SUPERPLASTICIZER IS PROHIBITED.

CELLS TO BE GROUT FILLED SHALL HAVE VERTICAL ALIGNMENT SUFFICIENT TO MAINTAIN A CLEAR, UNOBSTRUCTED, CONTINUOUS VERTICAL GROUT SPACE. CLEANOUT OPENINGS SHALL BE PROVIDED AT THE BOTTOM OF CELLS TO BE GROUT FILLED IN EACH POUR IN EXCESS OF 5 FEET IN HEIGHT. ANY OVERHANGING MORTAR OR OTHER OBSTRUCTION OR DEBRIS SHALL BE REMOVED FROM THE INSIDES OF SUCH CELL WALLS. THE CLEANOUTS SHALL BE SEALED BEFORE GROUTING, AFTER INSPECTION.

VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 192 BAR DIAMETERS. CELLS CONTAINING REINFORCEMENT SHALL BE FILLED SOLIDLY WITH GROUT. GROUT SHALL BE POURED IN LIFTS OF 4 FEET MAXIMUM HEIGHT. GROUT SHALL BE CONSOLIDATED AT TIME OF PLACING BY VIBRATING AND RECONSOLIDATED LATER BY VIBRATING BEFORE PLASTICITY IS LOST.

WHEN TOTAL GROUT POUR EXCEEDS 5 FEET IN HEIGHT, THE GROUT SHALL BE PLACED IN 4 FOOT LIFTS. MINIMUM CELL DIMENSION SHALL BE IN ACCORDANCE WITH TABLE 5 OF ACI 530.1 (3" X 3" FOR COARSE GROUT, 12 FT. MAXIMUM POUR HEIGHT).

WHEN THE GROUTING IS STOPPED FOR ONE HOUR OR LONGER, HORIZONTAL CONSTRUCTION JOINTS SHALL BE MADE BY STOPPING THE POUR OF GROUT NOT LESS THAN 1-1/2 INCH BELOW THE TOP OF THE UPPERMOST UNIT GROUTED.

WALL CONSTRUCTION JOINTS SHALL BE SPACED @ 24'-0" OC MAX. WITH TYP. VERT. REINF. AT EA. SIDE OF JOINT. HORIZONTAL JOINT REINF. SHALL TERMINATE 2' FROM EA. SIDE OF JOINT. BOND BEAM/TIE BEAM REINF. SHALL BE CONTINUOUS THROUGH WALL C.J.

LINTELS FOR MASONRY:
OPENINGS NOT PROVIDED WITH CONCRETE BEAMS SHALL BE SPANNED WITH PRECAST CONCRETE LINTELS WITH A WIDTH TO MATCH WALL WIDTH AND WITH 2#5 REINF. BARS, MIN. ALL PRECAST LINTELS SHALL BEAR A MINIMUM OF 8" AT EACH END. PRECASTER TO DESIGN PRECAST LINTELS FOR LOADS NOTED ON THIS SHEET.

STRUCTURAL STEEL:
WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A-572 OR A-992 GRADE 50 AND OTHER SHAPES SHALL CONFORM TO ASTM A36 AND THE SPECIFICATION FOR DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC. SHOP CONNECTIONS TO BE WELDED (UTILIZING E70XX ELECTRODES) AND FIELD CONNECTIONS TO BE BOLTED, UNLESS OTHERWISE NOTED ON STRUCTURAL DRAWINGS. STEEL SHALL RECEIVE ONE SHOP COAT AND ONE FIELD TOUCH UP COAT OF APPROVED PAINT, EXCEPT WHERE GALVANIZING IS INDICATED ON THE DRAWINGS.

STRUCTURAL TUBING SHALL CONFORM TO ASTM A-500, GRADE B, $F_y = 46$ ksi. STRUCTURAL PIPE SHALL CONFORM TO ASTM A-53 GRADE B, TYPE E OR S, $F_y = 35$ ksi. BEAM CONNECTIONS TO TUBE COLUMNS SHALL BE A.I.S.C. THRU-PLATE TYPE UNLESS SHOWN OTHERWISE.

BOLTED CONNECTIONS SHALL CONSIST OF MINIMUM 3/4 INCH DIAMETER ASTM A-325H HIGH STRENGTH BOLTS. BEAM CONNECTIONS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF SOUTH CAROLINA FOR THE REACTIONS SHOWN ON THE PLANS. IF NOT SHOWN, THE ENGINEER SHALL DESIGN THE BEAM CONNECTIONS TO SUPPORT AN END REACTION OF 1/2 KIPS FROM THE TABLES 3-6 "ALLOWABLE UNIFORM LOADS IN KIPS FOR BEAMS LATERALLY SUPPORTED" OF THE STEEL CONSTRUCTION MANUAL (15TH EDITION), BUT CONNECTIONS SHALL NOT HAVE LESS THAN 2 ROWS OF BOLTS. ANCHOR BOLTS SHALL CONFORM TO ASTM F1554 GRADE 36 (THREADED ROD). A SIGNED & SEALED CALCULATION SUBMITTAL SHALL BE ISSUED WITH SHOP DRAWINGS FOR REVIEW BY THE ENGINEER-OF-RECORD.

STRUCTURAL STEEL JOISTS AND JOIST GIRDERS SHALL BE FABRICATED AND ERECTED IN STRICT CONFORMANCE TO THE LATEST EDITION OF THE STEEL JOIST INSTITUTE STANDARDS. JOIST AND JOIST GIRDER ERECTION PROCEDURE SHALL CONFORM STRICTLY TO S.J.I. STANDARDS. CONTRACTOR IS TO SUBMIT DESIGN CALCULATIONS FOR ALL JOIST AND JOIST GIRDERS WITH CONCENTRATED AND STRUTTED LOAD COMPONENTS AND UNIFORM AND UPLIFT LOADS. PROVIDE SPECIAL MARKINGS FOR THESE SPECIAL JOISTS. DESIGN CALCULATIONS ARE TO BE SUBMITTED WITH SHOP DRAWINGS. DESIGN FOR UPLIFT USING COMPONENTS & CLADDING TABLE T3 S1.01.

JOISTS SHALL BE THE SIZE AND SPACING AS SHOWN ON THE STRUCTURAL DRAWINGS AND SHALL BE DESIGNED, FABRICATED, INSTALLED AND BRIDGED IN ACCORDANCE WITH THE STEEL JOIST INSTITUTE SPECIFICATIONS. ENDS OF BRIDGING LINES TERMINATING AT WALLS OR BEAMS SHALL BE ANCHORED THERETO AT TOP AND BOTTOM CHORDS. MINIMUM JOIST BRIDGING TERMINATION CONNECTIONS TO MASONRY SHALL BE L3 X 3 X 1/4 X 3" LONG WITH (1) 1/2" DIAMETER ANCHOR BOLT SIDE 1/4 X 4 X 4 WITH (1) 1/2" X 5" HEADED STUD TO CONCRETE. BRIDGING SHALL BE WELDED OR BOLTED AT 4 POINTS OF CONTACT. WELD SHALL NOT DAMAGE THE JOIST. CROSS BRIDGING SHALL BE WELDED OR BOLTED AT ITS CENTER POINT.

K SERIES JOISTS SHALL BEAR A MINIMUM OF 2 1/2" ON STEEL BEAMS AND 4" ON CONCRETE BEAMS. JOIST BEARING PLATES TO BE MINIMUM 3/8" X 4" X 7 1/2" WITH (2) 1/2" DIAMETER X 5" SHEAR STUD CONNECTORS. BEARING PLATES FOR BACK TO BACK SINGLE JOISTS SHALL BE MINIMUM 3/8" X 7 1/2" X 7 1/2" WITH (4) 1/2" DIAMETER X 5" SHEAR STUD CONNECTORS. BEARING PLATES SHALL BE CAST INTEGRALLY WITH THE CONCRETE BEAM. WELD JOISTS TO BEARING PLATES WITH A MINIMUM OF (2) 1/8" FILLET WELDS, UNLESS

SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWING SUBMITTAL SHALL INCLUDE LAYOUT, COMPONENT DESIGNATION, BRIDGING, AND PERTINENT SECTIONS AND DETAILS. SUBMITTALS FOR JOISTS, OTHER THAN STANDARD SJI CATALOG SELECTIONS WHICH HAVE BEEN CHECKED BY SJI, SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A SOUTH CAROLINA REGISTERED PROFESSIONAL ENGINEER.

PROVIDE CEILING EXTENSIONS AT CONTACT CEILINGS-SEE ARCHITECTURAL DRAWINGS. JOISTS SHALL BE CAMBERED FOR DEAD LOAD.

ALL DAMAGED JOISTS DELIVERED TO THE JOB SITE SHALL BE REJECTED OR REPAIRED BY THE JOIST MANUFACTURER. REPAIR METHODS SHALL BE SUBMITTED FOR APPROVAL AND SEALED BY A PROFESSIONAL ENGINEER.

WHERE BAR JOISTS ARE UTILIZED, AND COLUMNS ARE NOT FRAMED IN AT LEAST TWO DIRECTIONS WITH STRUCTURAL STEEL MEMBERS, A BAR JOIST NEAREST TO THE COLUMN SHALL BE FIELD BOLTED (EACH END OF JOIST) TO PROVIDE LATERAL STABILITY DURING CONSTRUCTION. EXTEND BOTTOM CHORDS OF ALL JOISTS AT COLUMN LINES OR NEAR COLUMN IF JOIST IS NOT ON CENTERLINE COLUMN, AND WELD TO COLUMNS OR BEAM AFTER ALL DEAD LOAD IS IN PLACE. PROVIDE JOIST BRIDGING AND JOIST GIRDER BRACING PER ASTC AND S.J.I. REQUIREMENTS. NOTHING SHALL BE SUSPENDED FROM THE DECK AND BRIDGING.

WELDING:
WELDING SHALL BE DONE BY WELDERS WITH CURRENT CERTIFICATION USING ASTM E70 SERIES ELECTRODE FOR SHOP WELDING A36 STEEL, AND E70 SERIES LOW HYDROGEN ELECTRODES FOR ALL WELDING OF HIGH STRENGTH STEELS AND FOR FIELD WELDING.

WELDS SHOWN ON STRUCTURAL DRAWINGS ARE MINIMUM DESIGN REQUIREMENTS. THE FABRICATOR'S SHOP DRAWINGS SHALL REFLECT WELDS IN ACCORDANCE WITH AWS REQUIREMENTS.

ALL FULL PENETRATION GROOVE WELDS SHALL BE INSPECTED BY ULTRASONIC TESTING. TWENTY-FIVE PERCENT OF ALL OTHER WELDS SHALL BE INSPECTED AT RANDOM UNLESS NOTED OTHERWISE. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

UNLESS NOTED OTHERWISE ON THE DRAWINGS, GROOVE WELDS SHALL BE FULL PENETRATION.

PROVIDE FILLET WELDS AT CONTACT POINTS BETWEEN STEEL MEMBERS SUFFICIENT TO DEVELOP THE ALLOWABLE TENSILE STRENGTH OF THE SMALLER MEMBER AT THE JOINT UNLESS DETAILED OTHERWISE ON THE DRAWINGS.

STEEL ROOF DECK:
SHALL BE AS NOTED ON PLAN AND SHALL CONFORM TO PROVISIONS OF THE STEEL DECK INSTITUTE (SDI) SPECIFICATIONS FOR STEEL ROOF DECK. WELD PATTERN SHALL BE A MINIMUM 36/17 UNLESS NOTED OTHERWISE. FASTEN SIDE LAPS WITH (5) #10 SCREW PER SPAN U.N.O. ON PLAN.

METAL STUDS:
LIGHT GAUGE STEEL FRAMING SYSTEM, INCLUDING TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH ACCEPTED ENGINEERING PRINCIPLES AND GOVERNING CODES. THE DESIGN SHALL BE PERFORMED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF SOUTH CAROLINA. SHOP DRAWINGS SHALL BE SUBMITTED WHICH BEARS THE SIGNATURE, DATE, AND EMBOSSED SEAL OF THE ENGINEER. SHOP DRAWINGS SHALL CLEARLY INDICATE CONNECTIONS AND MATERIALS USED.

DESIGN OF LIGHT GAGE METAL FRAMING NOT SPECIFICALLY DETAILED ON DRAWINGS SHALL BE PERFORMED BY A LICENSED STRUCTURAL ENGINEER IN THE STATE IN WHICH THE PROJECT WILL BE CONSTRUCTED. DESIGN CALCULATIONS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL PRIOR TO FABRICATION. DESIGN CALCULATIONS SHALL BE SIGNED AND SEALED BY THE DESIGN ENGINEER.

LIGHT GAGE METAL FRAMING SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL PRIOR TO FABRICATION SHOWING WALL SECTIONS COORDINATED WITH DRAWINGS SHOWING FRAMING, ACCESSORIES, ANCHORAGE AND CONNECTION DETAILS.

MATERIAL SPECIFICATIONS FOR LIGHT-GAGE STEEL:
16 GA. OR HEAVIER: ASTM A-446, $F_y = 50$ KSI MIN.
18 GA. OR LIGHTER: ASTM A-446, $F_y = 33$ KSI MIN.

GALVANIZING: MINIMUM G-60 COATING

CONNECTION MATERIAL GAGE SHALL MATCH STUD GAGE U.N.O. CLIP ANGLES SHALL BE 14 GA. MINIMUM.

BUILT-UP MEMBERS FASTEN TOGETHER WITH 1" LONG STITCH WELDS OR #12 SCREWS AT 12" OC MAXIMUM, EACH FLANGE, AND EACH TRACK.

PROVIDE MINIMUM BRIDGING AT 4' MAXIMUM VERTICAL SPACING IN WALLS.

STUDS SHALL BE INSTALLED TO SEAT SQUARELY (WITHIN 1/16") AGAINST THE WEB PORTION OF THE TOP AND BOTTOM TRACKS. TRACKS SHALL REST ON A CONTINUOUS, UNIFORM BEARING SURFACE.

TEMPORARY BRACING SHALL BE PROVIDED AND LEFT IN PLACE UNTIL WORK IS PERMANENTLY STABILIZED.

SPLICING OF MEMBERS SPANNING BETWEEN SUPPORTS SHALL NOT BE PERMITTED.

VERTICAL ALIGNMENT (PLUMBNESS) OF STUDS SHALL BE WITHIN 1/960TH (1/8" IN 10'-0") OF THE SPAN.

HORIZONTAL ALIGNMENT (LEVELNESS) OF WALLS SHALL BE WITHIN 1/960TH (1/8" IN 10'-0") OF THEIR RESPECTIVE LENGTHS.

SPACING OF STUDS SHALL NOT BE MORE THAN + 1/8" FROM THE DESIGNED SPACING PROVIDING THAT THE CUMULATIVE ERROR DOES NOT EXCEED THE REQUIREMENTS OF THE FINISHED MATERIALS.

PROVIDE DEEP TRACK ASSEMBLY AT TOPS OF ALL NON-LOAD BEARING STUD WALLS TO ALLOW FOR MOVEMENT OF STRUCTURE. ARCHITECT SHALL REVIEW IN PLACE METAL STUD CONSTRUCTION PRIOR TO THE INSTALLATION OF GYPSUM WALL BOARD OR SHEATHING.

POST-INSTALLED ANCHORS:
POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER-OF-RECORD PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REBAR. HOLES SHALL BE DRILLED AND CLEANED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE SPECIFIED BELOW, SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER-OF-RECORD ALONG WITH CALCULATIONS THAT ARE PREPARED & SEALED BY A REGISTERED PROFESSIONAL ENGINEER IN SOUTH CAROLINA. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING EQUIVALENT OR BETTER PERFORMANCE VALUES OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S) AS REQUIRED BY THE BUILDING CODE.

CONCRETE ANCHORS:
MECHANICAL ANCHORS FOR USE IN CRACKED AND UNCRACKED CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 308.2 AND ICC-ES AC193. PRE-APPROVED MECHANICAL ANCHORS INCLUDE:

- SIMPSON STRONG-TIE 'STRONG-BOLT 2' (ICC-ES ESR-3037)
- SIMPSON STRONG-TIE 'TITEN-HD' (ICC-ES ESR-2713)

ADHESIVE ANCHORS FOR USE IN CRACKED AND UNCRACKED CONCRETE SHALL BE INSPECTED AS FOLLOWS AT THE ONSET OF EACH APPLICATION. A MANUFACTURER'S REPRESENTATIVE MUST BE PRESENT TO WITNESS AT LEAST FIVE COMPLETE INSTALLATIONS. INSTALLERS MUST BE TRAINED BY THE MANUFACTURER EACH CERTIFIED INSTALLER WILL BE ISSUED A CERTIFICATION CARD TO VERIFY THEIR TRAINING AND SHALL BE REQUIRED TO CARRY THEIR CERTIFICATION CARD ON THEIR PERSON. CERTIFIED INSTALLERS SHALL PROVIDED WRITTEN DOCUMENTATION THAT ALL ANCHORS HAVE BEEN INSTALLED PER THE MANUFACTURER'S INSTRUCTIONS. ANCHORS SHALL ALSO HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC306. PRE-APPROVED ADHESIVE ANCHORS INCLUDE:

- SIMPSON STRONG-TIE 'SET-XP' (ICC-ES ESR-2508)
- HILTI HIT-RE 500 V3 (ICC-ES ESR-3814)

MASONRY ANCHORS:
ANCHORAGE TO SOLID-GROUTED CONCRETE MASONRY.

MECHANICAL AND CONCRETE SCREW ANCHORS FOR USE IN SOLID-GROUTED CONCRETE MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC101 OR AC106, RESPECTIVELY. PRE-APPROVED MECHANICAL AND CONCRETE SCREW ANCHORS INCLUDE:

SIMPSON STRONG-TIE 'STRONG-BOLT 2' (ICC-ES ESR-3037)
SIMPSON STRONG-TIE 'TITEN-HD' (ICC-ES ESR-1056)

ADHESIVE ANCHORS FOR USE IN SOLID-GROUTED CONCRETE MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC58. PRE-APPROVED ADHESIVE ANCHORS INCLUDE:

SIMPSON STRONG-TIE 'XP' (ICC-ES ESR-2508)
HILTI HIT HY270 (ICC-ES ER-4143)

ANCHORAGE TO HOLLOW CONCRETE MASONRY/UNREINFORCED CLAY BRICK MASONRY

SCREW ANCHORS FOR USE IN HOLLOW CONCRETE MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC106. PRE-APPROVED SCREW ANCHORS INCLUDE:

SIMPSON STRONG-TIE 'TITEN-HD' (ICC-ES ESR-1056)

ADHESIVE ANCHORS WITH SCREEN TUBES FOR USE IN HOLLOW CONCRETE MASONRY/UNREINFORCED CLAY BRICK MASONRY SHALL BE TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC58 OR AC60, AS APPROPRIATE. THE APPROPRIATE SCREEN TUBE SHALL BE USED AS RECOMMENDED BY THE ADHESIVE MANUFACTURER. PRE-APPROVED ADHESIVE ANCHORS WITH SCREEN TUBES INCLUDE:

SIMPSON STRONG-TIE '3G' (ICC-ES ESR-4844)

POWER-ACTUATED FASTENERS (PAF) SHALL BE BY SIMPSON STRONG-TIE (ICC-ES ESR-2138) OR ENGINEER-APPROVED EQUAL.

GAS-ACTUATED FASTENERS (GAF) SHALL BE BY SIMPSON STRONG-TIE (ICC-ES ESR-2811) OR ENGINEER-APPROVED EQUAL.

Sheet Number	Sheet Name
S1.0	GENERAL NOTES
S1.01	WIND DESIGN DATA SHEET
S1.1	FOUNDATION PLAN
S1.2	ROOF FRAMING PLAN
S2.0	WALL SECTIONS
S2.1	WALL SECTIONS
S2.2	SECTIONS & DETAILS
S2.3	WALL SECTIONS
S3.0	TYPICAL DETAILS
S3.1	TYPICAL DETAILS
S3.2	TYPICAL DETAILS
S3.3	TYPICAL DETAILS

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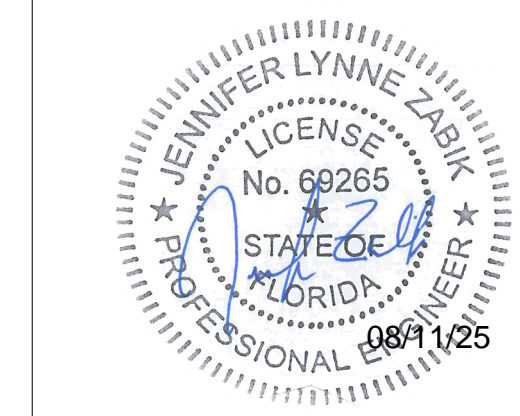
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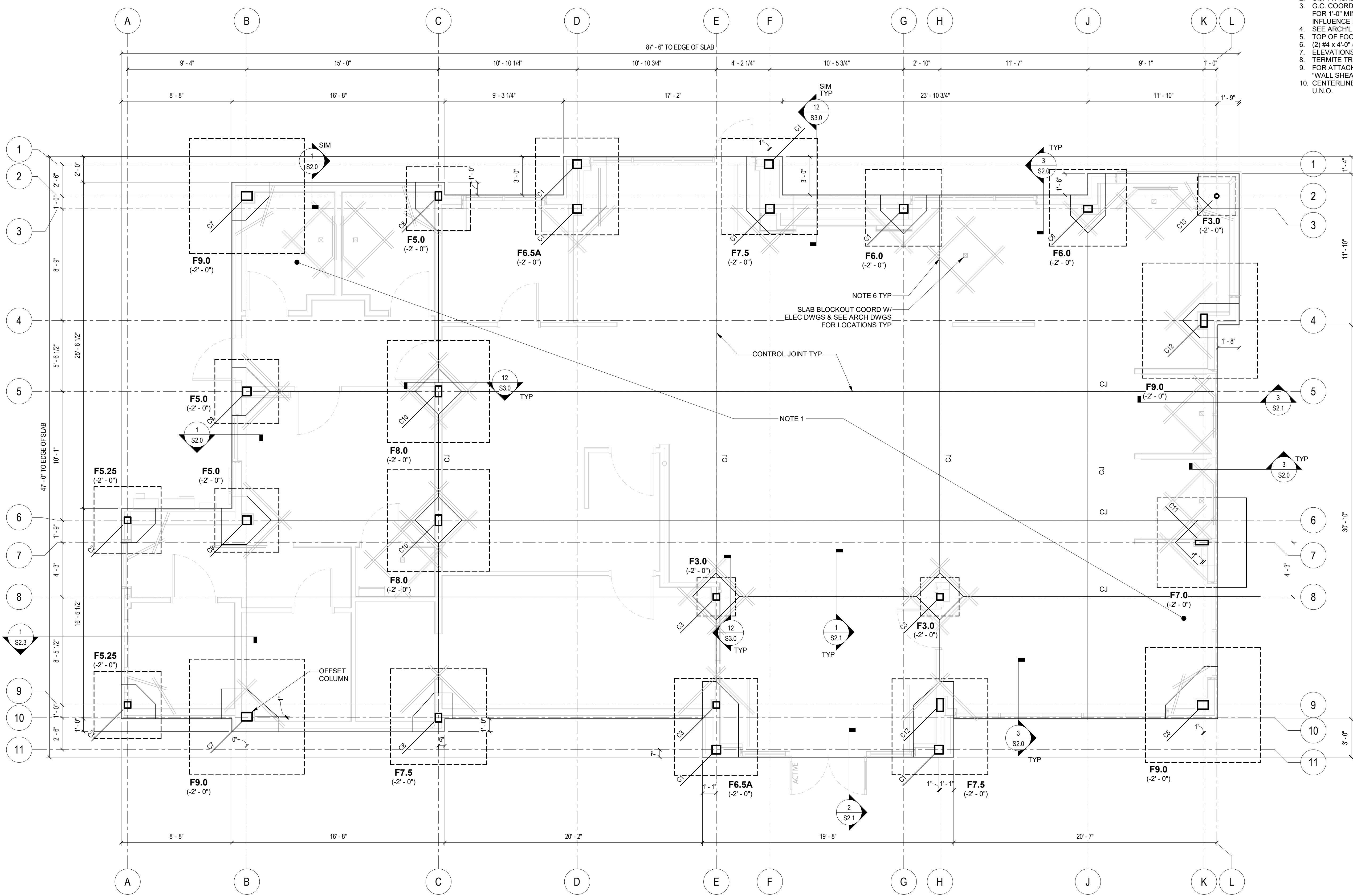
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PROJECT INFORMATION BLOCK	
JOB #:	244041
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DRAWN BY:	
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SHEET TITLE	
FOUNDATION PLAN	
SHEET NUMBER	
S1.1	

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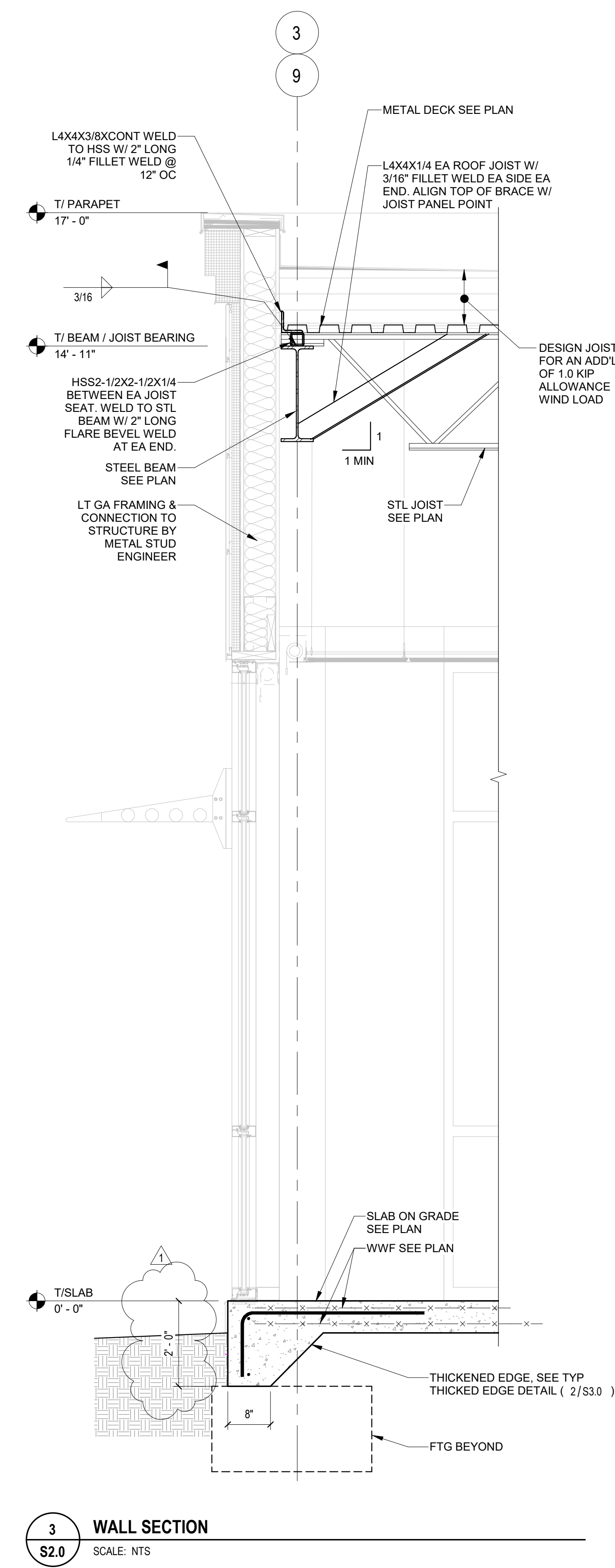
FOUNDATION PLAN NOTES:

- 6" CONC. SLAB ON GRADE REINFORCED WITH (2) LAYERS OF 6x6 W14xW1.4 W.W.F. ON 15 MIL VAPOR BARRIER ON PREPARED SUBGRADE U.N.O. SEE ARCH. FOR DIMENSIONS AND LOCATIONS.
- C.J. TYPICALLY DENOTES CONTROL JOINT PER DETAIL ON S3.1.
- G.C. COORDINATE W/ CIVIL DRAWINGS TOP OF FOOTING ELEVATIONS WILL ALLOW FOR 1" MINIMUM OF SOIL ABOVE TOP OF FOOTING. SEE 'TYP. FOUNDATION INFLUENCE DETAIL' ON SHEET S3.0.
- SEE ARCH. FOR ALL DIMENSIONS.
- TOP OF FOOTING ELEV. = -1'-6" BELOW SLAB U.N.O.
- (2) #4 x 4'-0" @ 1' OC RE-ENTRANT BARS CENTERED IN SLAB.
- ELEVATIONS REFERENCE T.O. SLAB-ON-GRADE ELEV. = 0'-0".
- TERMITE TREATMENT REQ'D AS PART OF SUBGRADE PREPARATION. SEE SPEC'S.
- FOR ATTACHMENT OF WOOD SHEATHING TO LIGHT GAUGE METAL STUDS SEE 'WALL SHEATHING FASTENING SCHEDULE' ON SHEET S3.3.
- CENTERLINES OF FOUNDATIONS SHALL COINCIDE WITH COLUMN CENTERLINES, U.N.O.

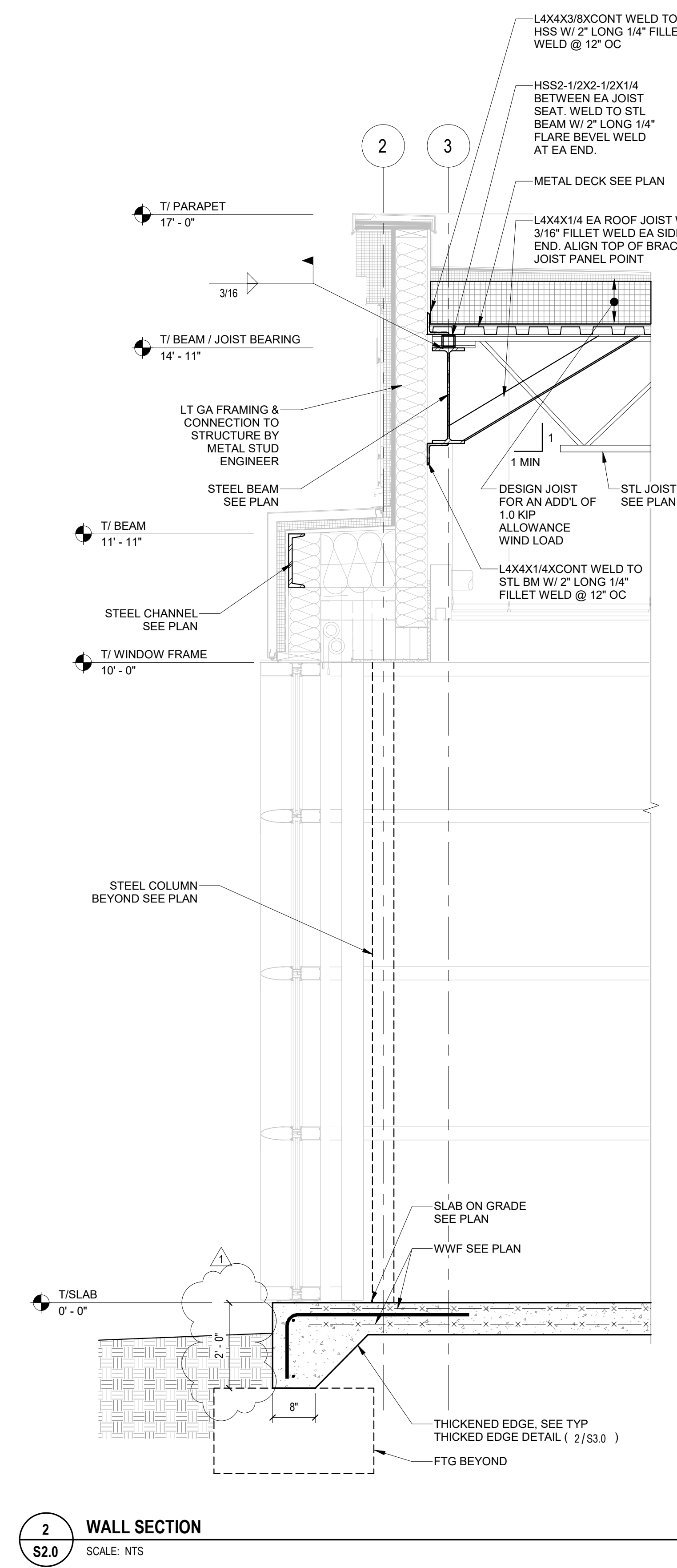


1
S1.1 SOG - FOUNDATION
SCALE: 1/4" = 1'-0"

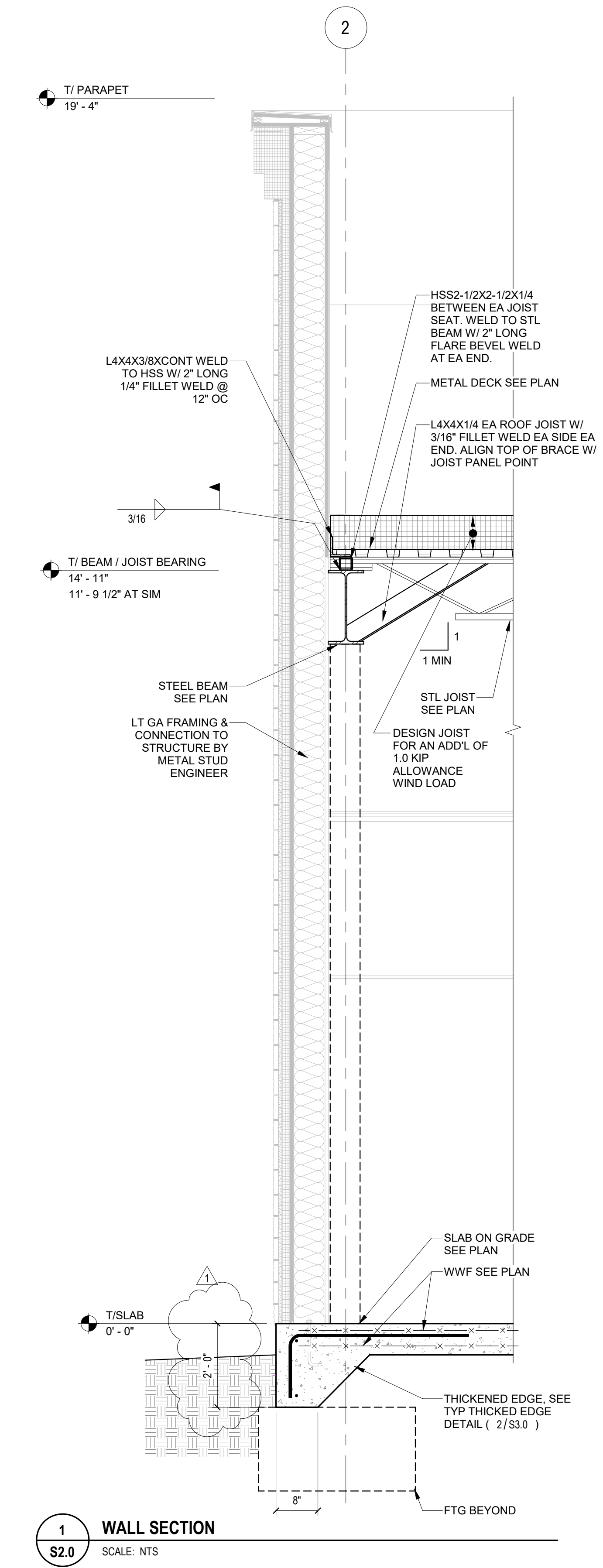
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3 WALL SECTION
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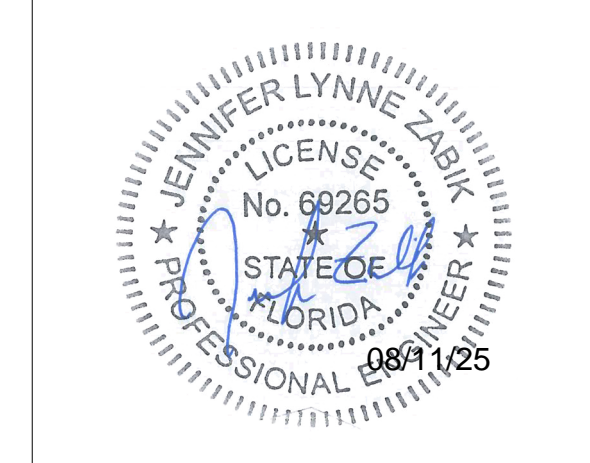
2 WALL SECTION
SCALE: NTS



1 WALL SECTION
SCALE: NTS

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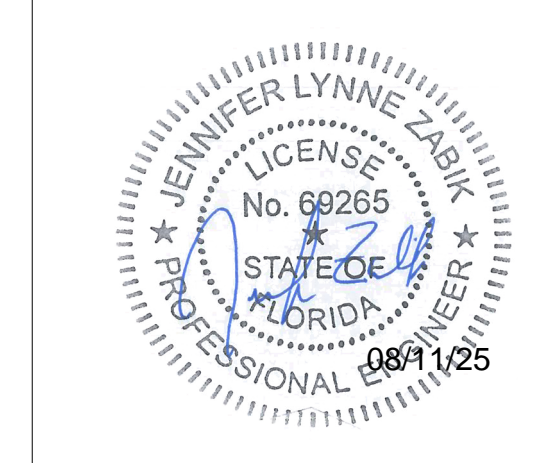
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SHEET TITLE	WALL SECTIONS
SHEET NUMBER	S2.0

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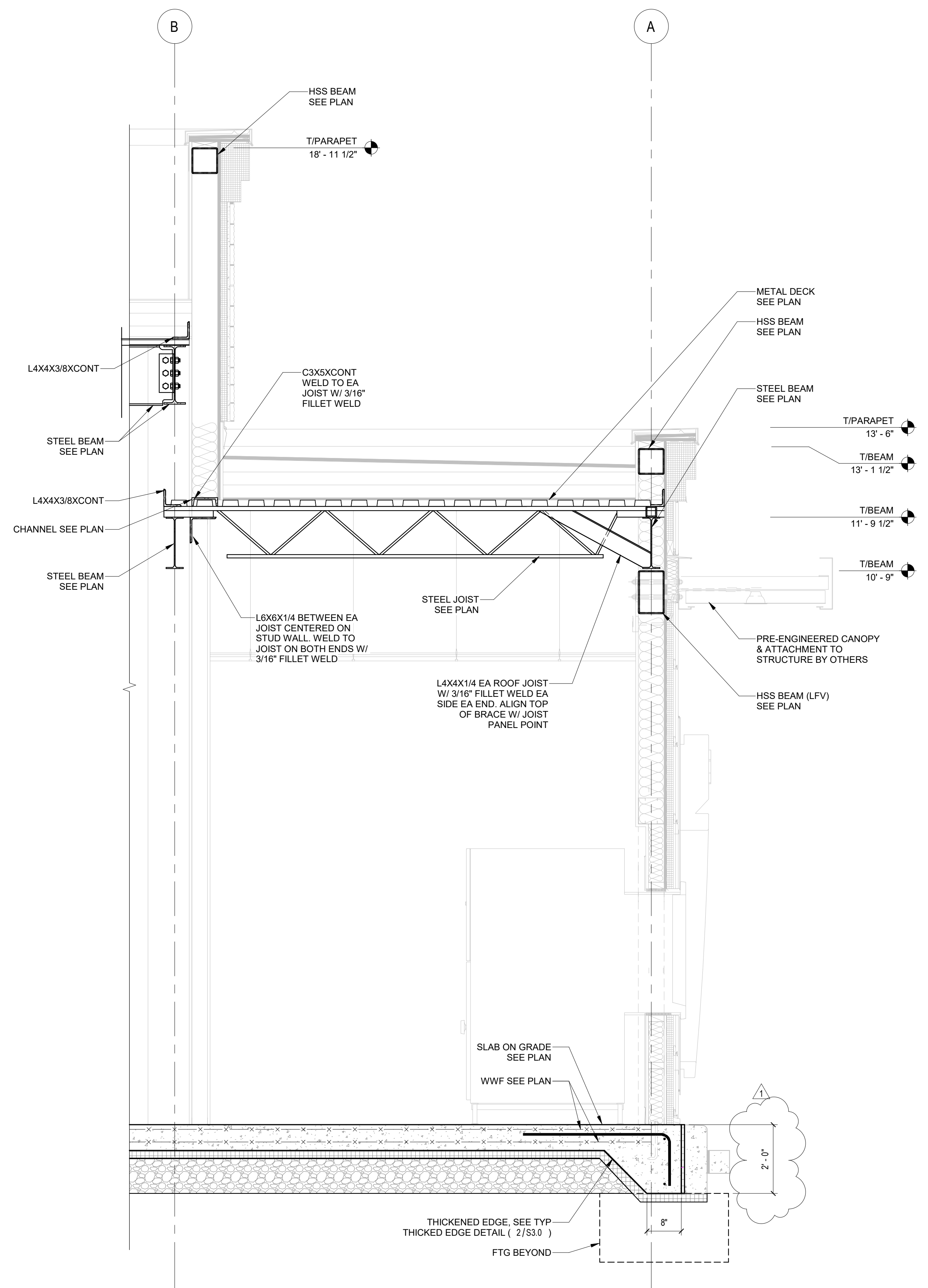
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WALL SECTIONS

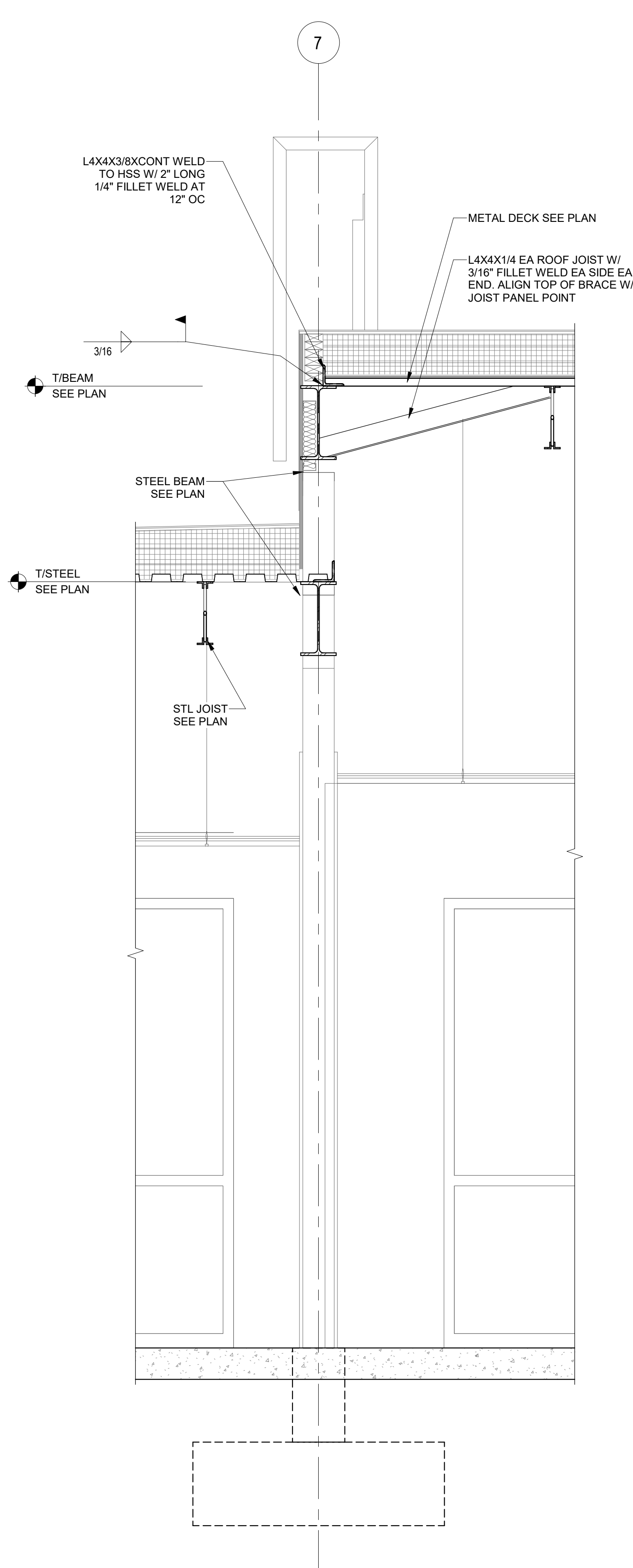
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S2.3

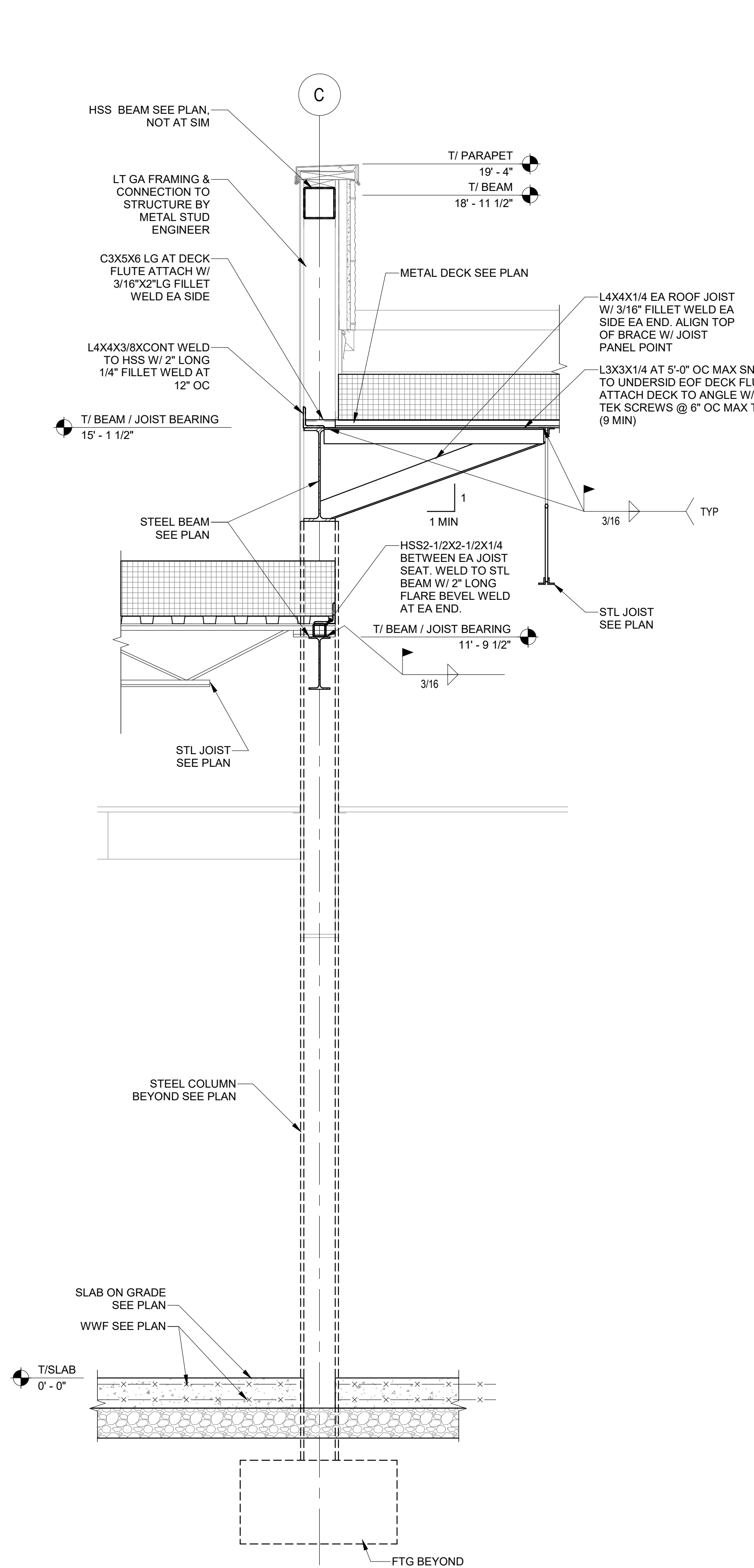
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1 WALL SECTION
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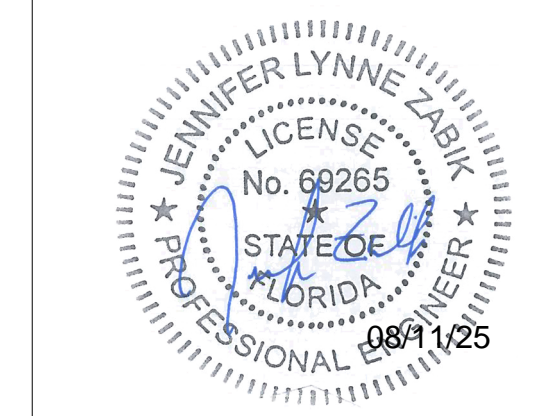


2 WALL SECTION
SCALE: 3/4\"/>



3 WALL SECTION
SCALE: NTS

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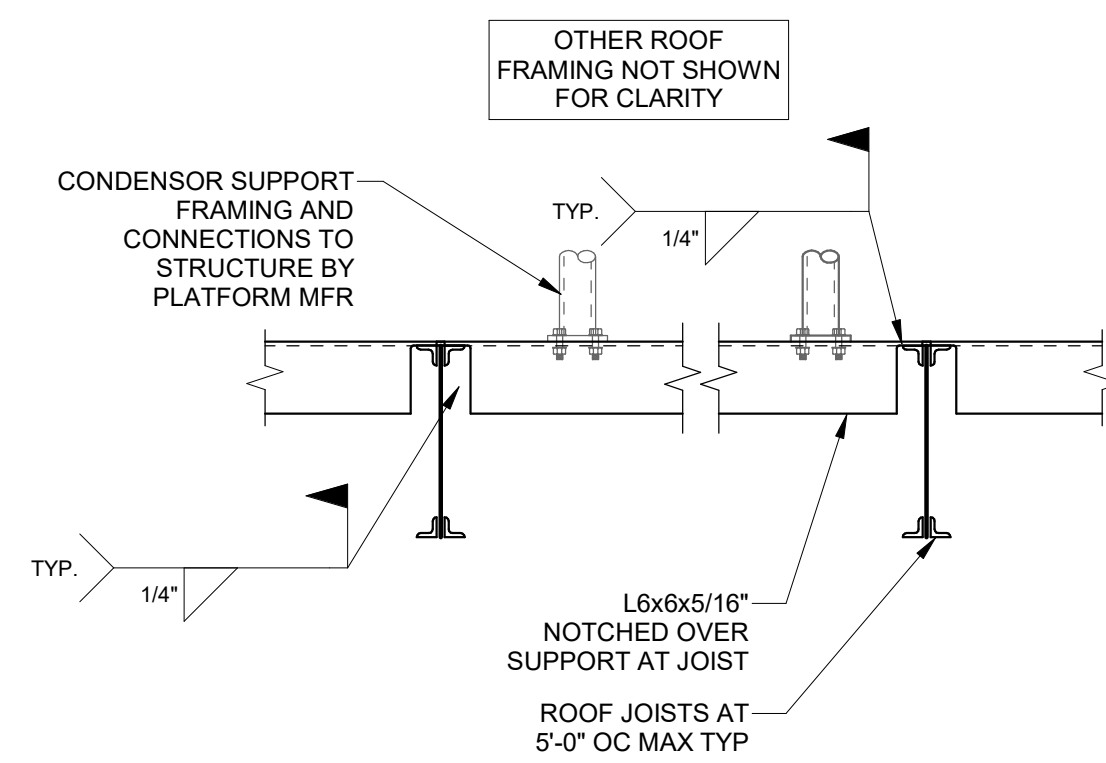
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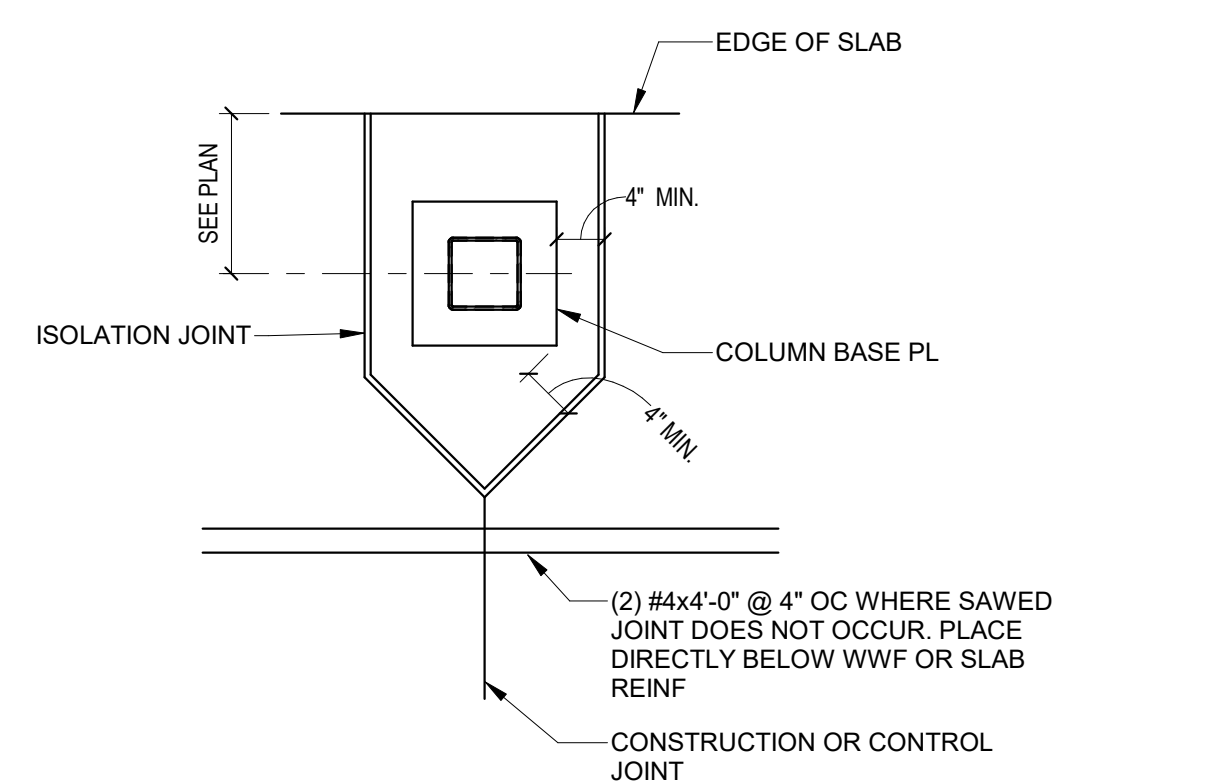
LAP SPLICE SCHEDULE

NO. BAR	TOP BARS		BOTTOM BARS	
	CASE 1	CASE 2	CASE 1	CASE 2
#3	24	37	19	28
#4	33	49	25	37
#5	41	61	31	47
#6	49	73	37	56
#7	71	106	54	81
#8	81	121	62	93

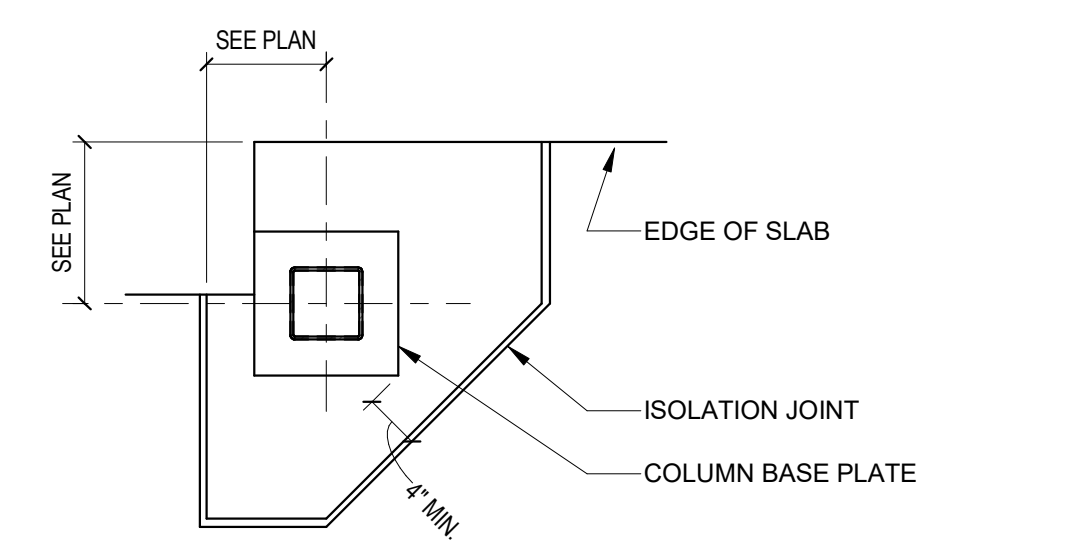
- LAP SPLICE SCHEDULE NOTES:**
- GRADE 60 REINFORCEMENT
 - $f_c = 4000$ psi
 - STIRRUPS OR TIES THROUGHOUT DEVELOPMENT LENGTH NOT LESS THAN THE CODE MINIMUM
 - CASE 1:
 - CLEARSPACING OF BARS BEING SPLICED NOT LESS THAN BAR DIAMETER, AND
 - CLEAR COVER IN ACCORDANCE WITH ACI 318 (TYP)
 - CASE 2: IF CASE 1 IS NOT MET



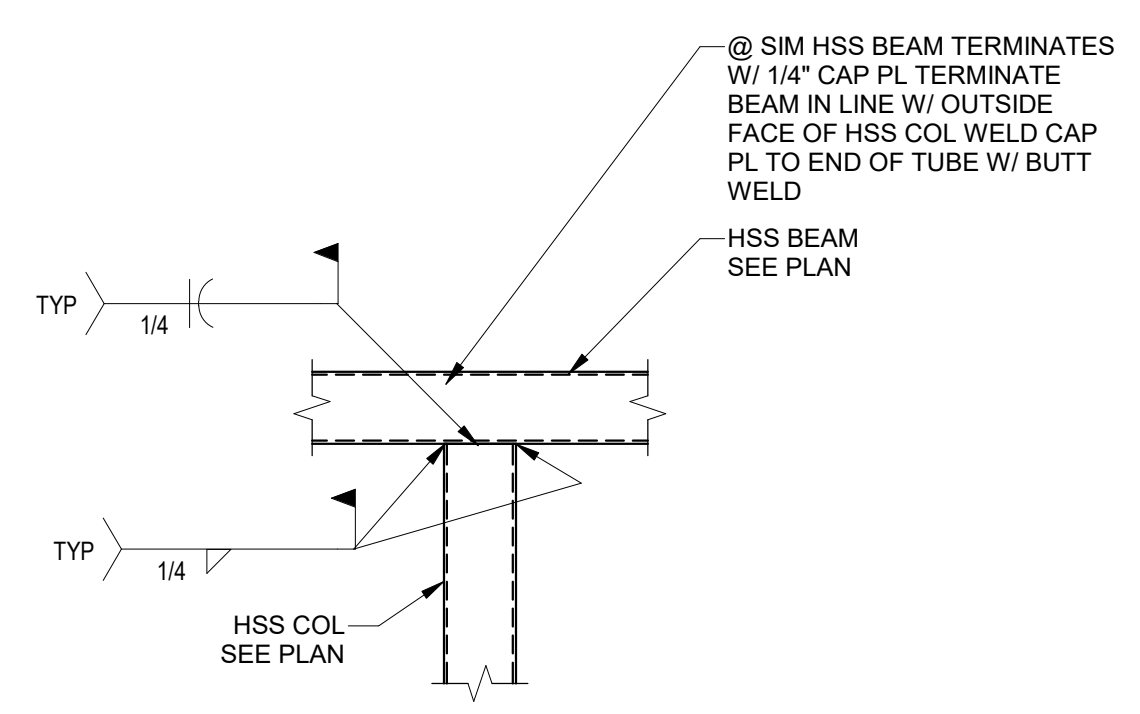
9 TYPICAL CONDENSER SUPPORT
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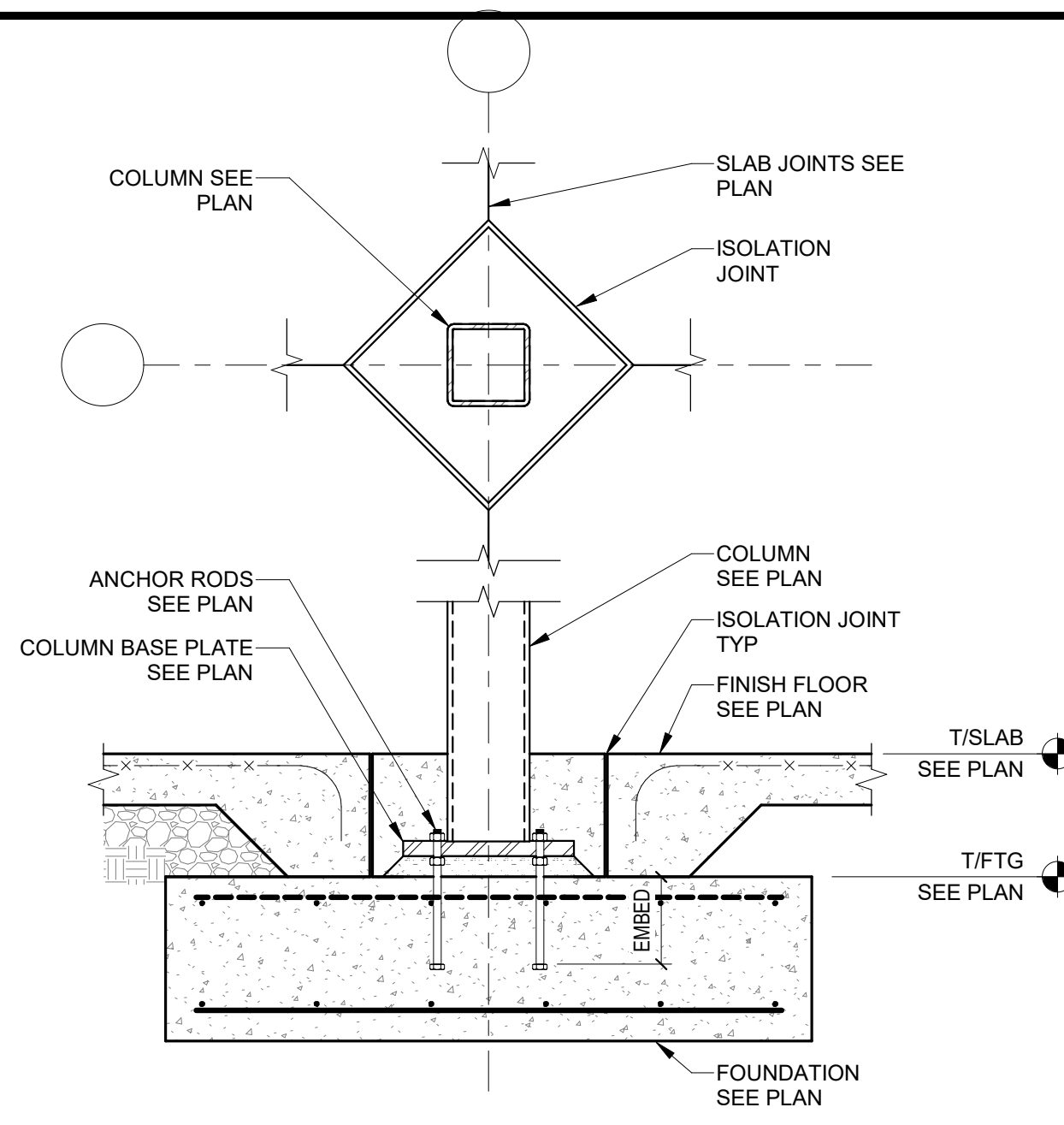
5 TYPICAL SLAB ISOLATION JOINT AT EDGE
SCALE: NTS



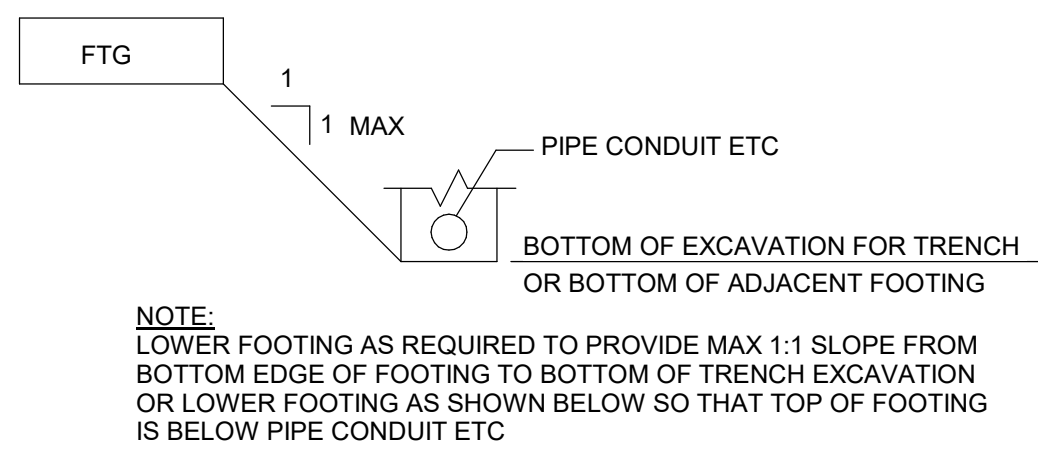
6 SLAB BLOCKOUT AT INSIDE CORNER
SCALE: NTS



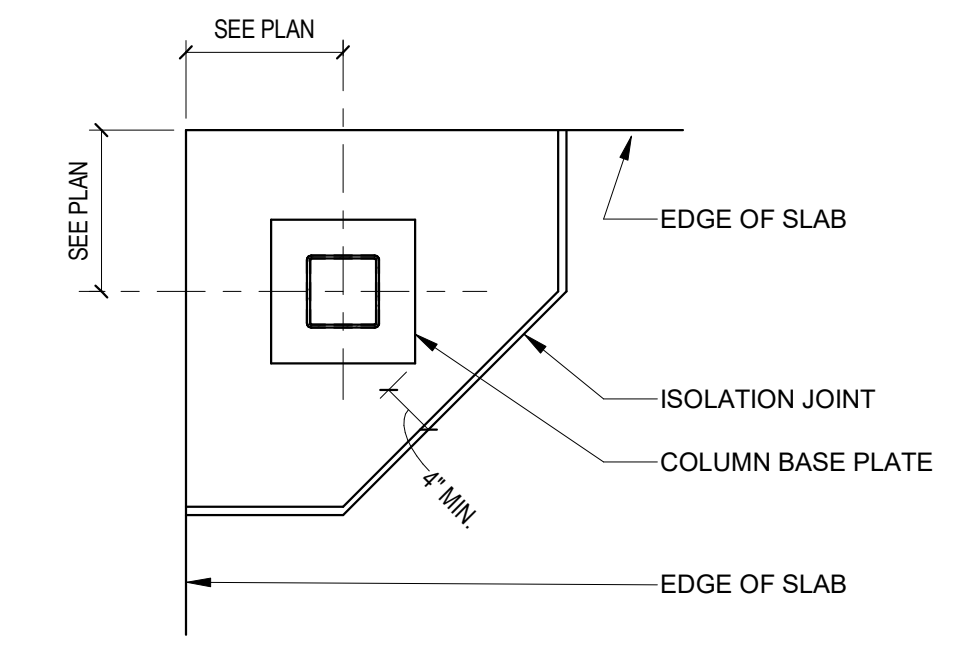
11 HSS TO HSS CONNECTION DETAIL
SCALE: NTS



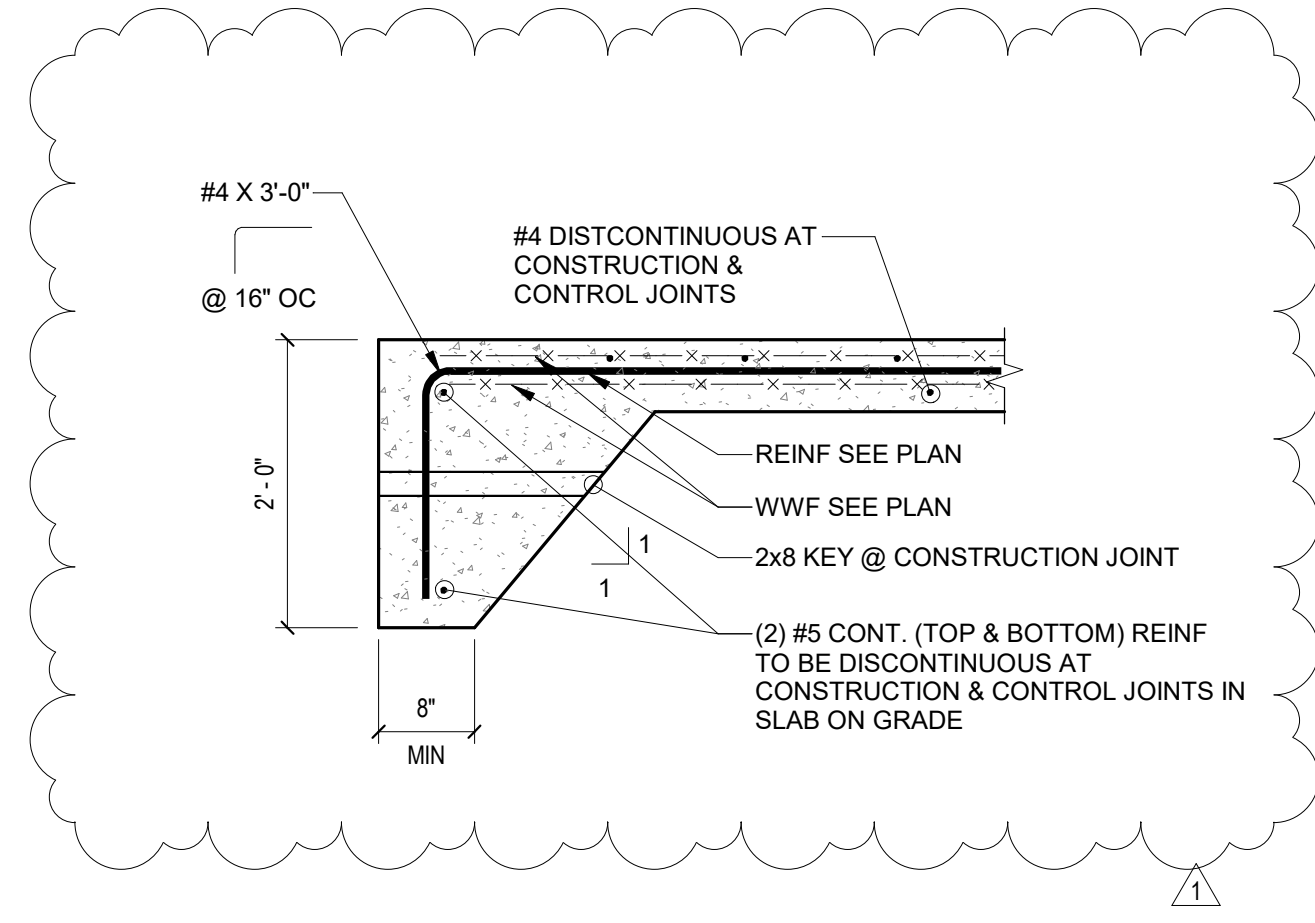
12 TYPICAL HSS COL FTG
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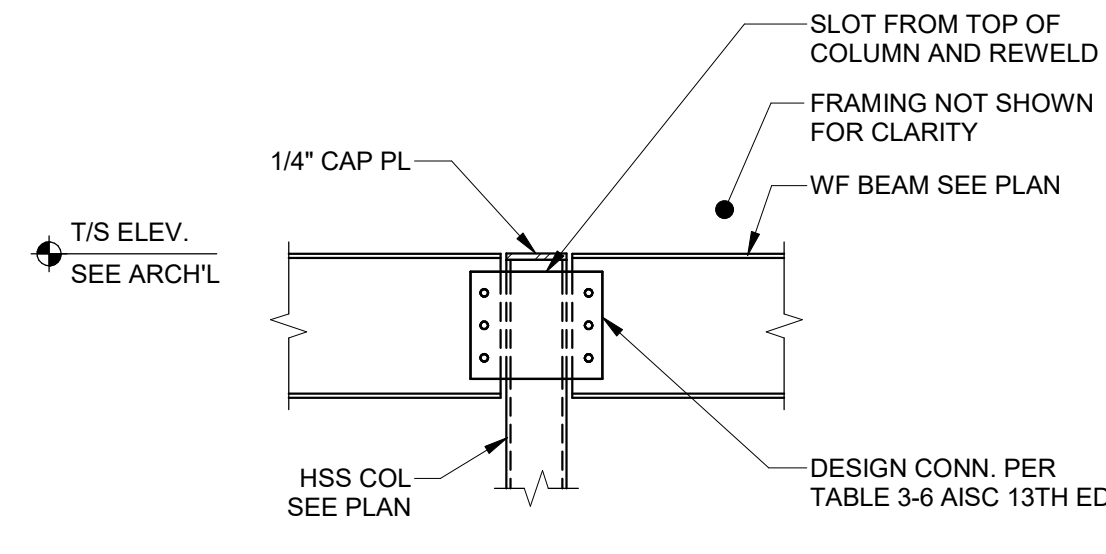
8 TYPICAL FOUNDATION INFLUENCE DETAIL
SCALE: NTS



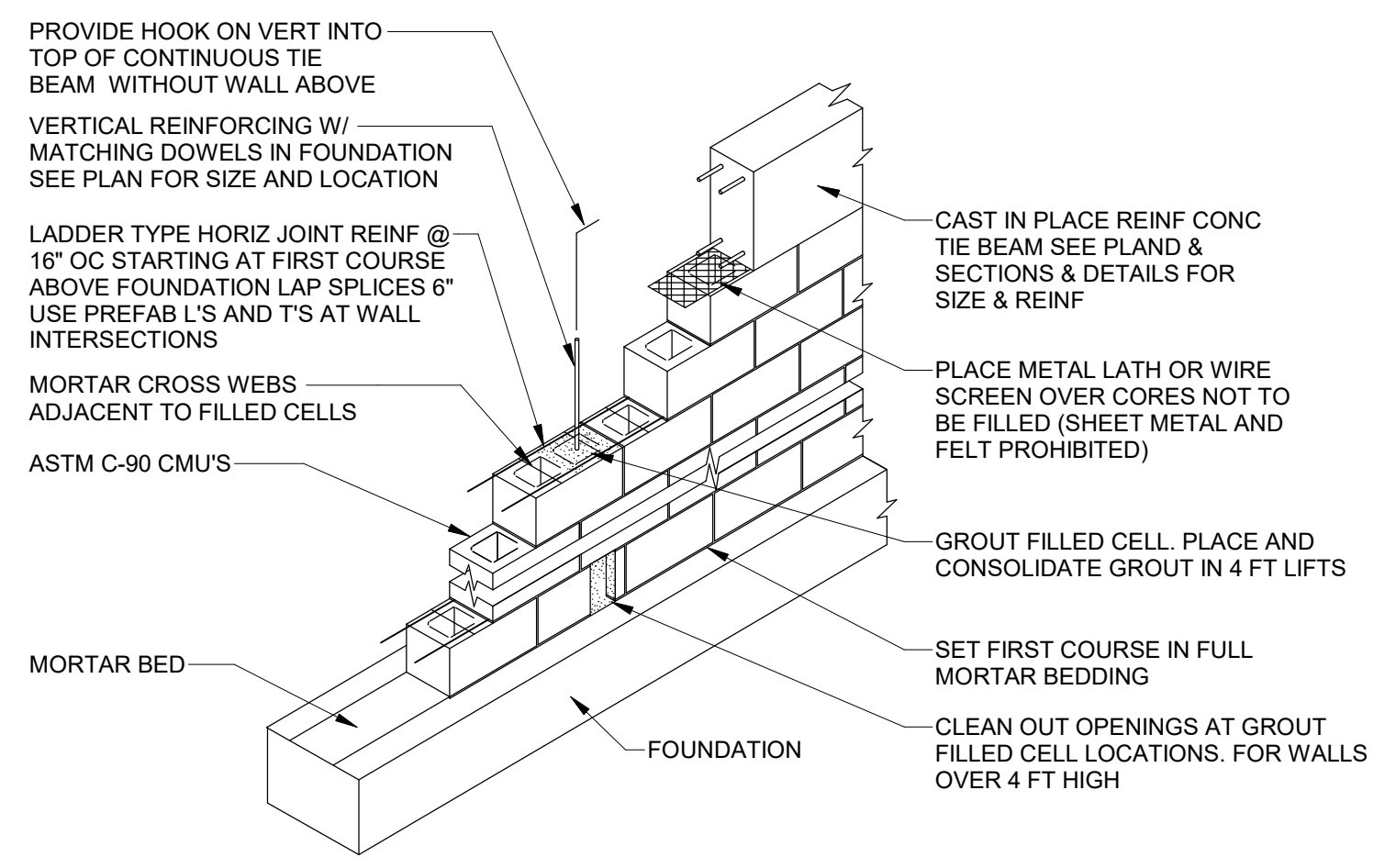
7 TYPICAL SLAB ISOLATION JOINT AT CORNER
SCALE: NTS



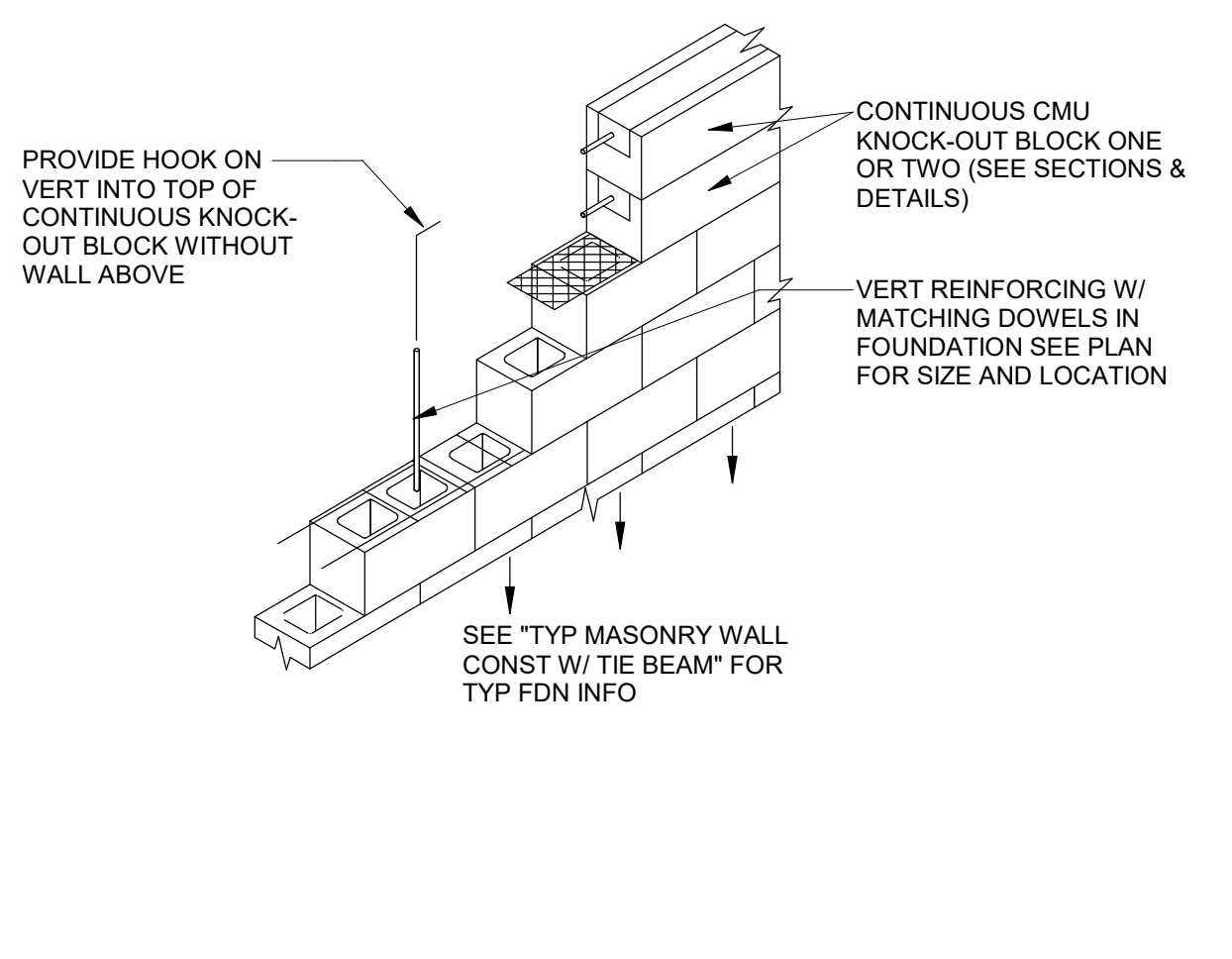
2 TYPICAL THICKENED EDGE
SCALE: NTS



1 STEEL BEAM TO STEEL COLUMN CONNECTION
SCALE: NTS



3 TYPICAL MASONRY WALL CONST. W/ TIE BEAM
SCALE: NTS



4 TYPICAL MASONRY WALL CONST. W/ KNOCK-OUT BEAM
SCALE: NTS

