GENERAL REQUIREMENTS

A. THE GENERAL STRUCTURAL NOTES EMPLOY THE FOLLOWING DEFINITIONS AND ABBREVIATIONS:
   1. CONTRACT DOCUMENTS – THE LATEST SET OF DRAWINGS, SPECIFICATIONS, AND RECORDED ADDENDA AND AMENDMENTS ISSUED FOR BID OR CONSTRUCTION.
   2. LICENSED PROFESSIONAL (STRUCTURAL) ENGINEER – AN ENGINEER LICENSED IN THE STATE IN WHICH THE PROJECT IS LOCATED AND QUALIFIED TO PERFORM THE WORK REQUIRED.
   3. STRUCTURAL ENGINEER OF RECORD – LICENSED PROFESSIONAL ENGINEER WHO IS IN RESPONSIBLE CHARGE FOR THE PREPARATION, SIGNING, DATING, SEALING, AND ISSUING OF STRUCTURAL ENGINEERING DOCUMENTS FOR ENGINEERING SERVICE OR CREATIVE WORK.
   4. DELEGATED ENGINEER – A LICENSED PROFESSIONAL ENGINEER WHO PROVIDES SERVICES OR CREATIVE WORK REGARDING A PORTION OF THE ENGINEERING PROJECT. THE DELEGATED ENGINEER IS THE ENGINEER OF RECORD FOR THAT PORTION OF THE ENGINEERING PROJECT. TYPICALLY, DELEGATED ENGINEERS FALL INTO ONE OF THE FOLLOWING CATEGORIES:
      a. AN INDEPENDENT CONSULTANT
      b. AN EMPLOYEE OR OFFICER OF AN ENTITY SUPPLYING COMPONENTS TO A FABRICATOR OR CONTRACTOR
      c. AN EMPLOYEE OR OFFICER OF A FABRICATOR OR CONTRACTOR
   5. DELEGATED ENGINEERING DOCUMENTS – ENGINEERING DOCUMENTS THAT ARE PREPARED BY A DELEGATED ENGINEER.
   6. DESIGN TEAM – DESIGN PROFESSIONALS INCLUDING THE ARCHITECT, STRUCTURAL ENGINEER, CIVIL ENGINEER, MEP ENGINEER, AND ANY OTHER CONSULTANT THAT ISSUES CONTRACT DOCUMENTS.
   7. CONTRACTOR – GENERAL CONTRACTOR, CONSTRUCTION MANAGER, DESIGN BUILDER, OR ANY OTHER ENTITY CONTRACTED BY THE OWNER TO PERFORM THE WORK.
   8. SHOP DRAWINGS – DRAWINGS DEPICTING INSTALLATION MEANS AND METHODS AND CATALOG INFORMATION ON STANDARD PRODUCTS. SHOP DRAWINGS SHALL BE PREPARED BASED ON ENGINEERING DIRECTION CONTAINED IN CONTRACT DOCUMENTS BY A CONTRACTOR, FABRICATOR, MANUFACTURER, OR LICENSED PROFESSIONAL ENGINEER, FOR INCORPORATION INTO THE PROJECT.
   9. ESTABLISHED CHANNELS – AT THE ONSET OF THE PROJECT, ARCHITECT, OWNER, AND CONTRACTOR SHALL ESTABLISH DESIRED LINES OF COMMUNICATION BETWEEN ALL PROJECT PARTIES. THESE AGREED UPON LINES OF COMMUNICATION ARE THE ESTABLISHED CHANNELS.
B. GENERAL STRUCTURAL NOTES ARE APPLICABLE TO THE DESIGN AND CONSTRUCTION OF THE ENTIRE PROJECT AND THUS ARE APPLICABLE TO EVERY SHEET WITHIN THIS SET.
C. WHERE A DETAIL, TYPICAL DETAIL, SECTION, OR PLAN NOTE IS SHOWN FOR ONE CONDITION, IT SHALL APPLY FOR ALL SIMILAR OR LIKE CONDITIONS, UNLESS NOTED OTHERWISE.
D. ISOMETRIC VIEWS ARE FOR VISUALIZATION PURPOSES ONLY AND DO NOT CONVEY ALL OF THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.
E. SHOULD THE CONTRACTOR ENCOUNTER A CONFLICT BETWEEN THESE DRAWINGS AND ANY OTHER CONTRACT DOCUMENT OR APPLICABLE CODE OR STANDARD OF PRACTICE DURING BIDDING, THE PROVISION RESULTING IN THE GREATER COST APPLIES. SHOULD THE CONTRACTOR ENCOUNTER A CONFLICT DURING CONSTRUCTION, THE CONTRACTOR SHALL SUBMIT A WRITTEN REQUEST FOR CLARIFICATION TO THE DESIGN TEAM, WHO WILL PROVIDE A WRITTEN RESPONSE IN RETURN.
F. SPECIFICATIONS HAVE NOT BEEN ISSUED ON THIS PROJECT BY THE STRUCTURAL ENGINEER OF RECORD AND THUS REFERENCES TO SPECIFICATIONS WITHIN THE GENERAL STRUCTURAL NOTES SHALL BE IGNORED. SHOULD THE CONTRACTOR REQUIRE FURTHER INFORMATION ON THE EXECUTION OF THE WORK, THEY MUST SUBMIT A WRITTEN REQUEST FOR CLARIFICATION.
G. THE CONTRACTOR SHALL SUPERVISE AND DIRECT ALL WORK AND SHALL BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, AND SEQUENCE. THE CONTRACTOR HAS SOLE RESPONSIBILITY FOR THE QUALITY AND CORRECTNESS OF THE WORK.
H. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR COORDINATION OF THE STRUCTURAL WORK WITH OTHER TRADES INCLUDING, BUT NOT LIMITED TO: ARCHITECTURAL, CIVIL, AND MEP FOR FLOOR SLAB STEPS, SLOPES AND CURBS, FLOOR SLAB FINISH, OPENINGS IN STRUCTURAL FLOORS, ROOFS AND WALLS, ETC.
I. THE BUILDING HAS BEEN DESIGNED BY THE STRUCTURAL ENGINEER OF RECORD TO RESIST THE CODE REQUIRED VERTICAL AND LATERAL FORCES IN ITS FULLY COMPLETED CONDITION. THE CONTRACTOR SHALL PROVIDE ALL REQUIRED BRACING, SHORING, AND OTHER CONSTRUCTION SUPPORTS NECESSARY TO ENSURE THE BUILDING'S STABILITY AND SAFETY THROUGHOUT THE DURATION OF CONSTRUCTION. FURTHER, THE CONTRACTOR SHALL NOT OVERLOAD THE STRUCTURE DURING CONSTRUCTION. THE CONTRACTOR SHALL RETAIN A LICENSED PROFESSIONAL ENGINEER TO PROVIDE THE ANALYSIS AND DESIGN NECESSARY TO DETERMINE POTENTIALLY OVERLOADED, UNSTABLE, OR HAZARDOUS CONDITIONS THAT MAY OCCUR AT ANY STAGE DURING CONSTRUCTION.
J. THE CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS AND COORDINATE WITH THE CONTRACT DOCUMENTS AND SHOP DRAWINGS.
K. THE CONTRACTOR SHALL NOT EMPLOY CONSTRUCTION MEANS OR METHODS THAT MAY DAMAGE UTILITIES, ADJACENT BUILDINGS, OR PROPERTY. DOCUMENTATION OF ADJACENT CONDITIONS PRIOR TO CONSTRUCTION IS RECOMMENDED. FURTHER, THE CONTRACTOR SHALL EITHER ADEQUATELY CONFINE THE SITE OR PROTECT ADJACENT PROPERTY FROM DAMAGE.
L. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR PROJECT SAFETY AND OSHA REQUIREMENTS. SHOULD THE STRUCTURAL ENGINEER OF RECORD NOTIFY THE CONTRACTOR OF A POTENTIALLY UNSAFE CONDITION, IT IS SOLELY AS A COURTESY FROM ONE PROFESSIONAL TO ANOTHER. IT SHOULD NOT BE INTERPRETED AS THE STRUCTURAL ENGINEER OF RECORD ASSUMING ANY RESPONSIBILITY FOR PROJECT SAFETY.
M. ALL STRUCTURES REQUIRE PERIODIC MAINTENANCE TO EXTEND LIFE SPAN AND ENSURE STRUCTURAL INTEGRITY FROM EXPOSURE TO THE ENVIRONMENT. A PLANNED PROGRAM OF MAINTENANCE SHALL BE ESTABLISHED BY THE BUILDING OWNER. THIS PROGRAM SHALL INCLUDE, BUT NOT BE LIMITED TO: PAINTING OF STRUCTURAL STEEL, PROTECTIVE COATINGS FOR CONCRETE, SEALANTS, CAULKED JOINTS, EXPANSION JOINTS, CONTRACTION JOINTS, SPALLS AND CRACKS IN CONCRETE, AND PRESSURE WASHING OF EXPOSED STRUCTURAL ELEMENTS EXPOSED TO A SALINE OR OTHER HARSH CHEMICAL ENVIRONMENT.
N. THE USE OF DE-ICING CHEMICALS ON ANY EXPOSED STRUCTURAL ELEMENT IS DISCOURAGED AND WILL ACCELERATE DETERIORATION OF STRUCTURAL ELEMENTS.
O. THE BUILDING OWNER SHALL NOT ALTER OR MODIFY ANY STRUCTURAL ELEMENT WITHOUT CONSULTING A LICENSED PROFESSIONAL ENGINEER. FURTHER, BUILDING OWNER SHALL NOT RENOVATE, REPURPOSE, ADD-ON TO, OR OTHERWISE MODIFY THE EXISTING STRUCTURAL SYSTEMS WITHOUT CONSULTING A LICENSED PROFESSIONAL ENGINEER.
P. CONTRACT DRAWINGS SHOW MAJOR OPENINGS IN FLOORS AND WALLS AND DO NOT NECESSARILY SHOW ALL OPENINGS REQUIRED. THE CONTRACTOR SHALL COORDINATE ALL OPENING SIZES AND LOCATIONS BETWEEN ALL DISCIPLINES AND TRADES. ADDITIONAL OPENINGS, BLOCKOUTS, AND SLEEVES MAY BE REQUIRED AND SHALL BE CONSTRUCTED USING THE TYPICAL DETAILS AND/OR REQUIREMENTS WITHIN THE CONTRACT DOCUMENTS. OPENINGS REQUIRED, BUT NOT SHOWN ON THE STRUCTURAL DRAWINGS, MUST BE APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
Q. THE CONTRACTOR SHALL COORDINATE PIPING AND CONDUIT EMBEDDED IN OR ATTACHED TO SLABS, SLABS-ON-DECK, BEAMS, AND COLUMNS. ANY REQUIRED MODIFICATIONS TO STRUCTURAL MEMBERS OR THEIR REINFORCEMENT AS A RESULT OF EMBEDMENT OR ATTACHMENT SHALL BE SUBMITTED TO THE DESIGN TEAM FOR THEIR REVIEW. SEE GENERAL STRUCTURAL NOTES SECTION "DESIGN CRITERIA" FOR LIMITATIONS OF MEP LOADING ON STRUCTURAL SYSTEMS.
R. THE STRUCTURAL ENGINEER OF RECORD'S ROLE DURING CONSTRUCTION
   1. THE STRUCTURAL ENGINEER OF RECORD SHALL NOT ASSUME CONTROL OF, OR RESPONSIBILITY FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, PROJECT SAFETY, THE ACTS AND OMISSIONS OF THE CONTRACTOR, OR THEIR FAILURE TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
   2. STRUCTURAL ENGINEER OF RECORD SHALL NOT HAVE AUTHORITY TO STOP THE WORK OR AUTHORIZE CHANGES TO ANY CONTRACT SUM.
   3. PERIODIC SITE VISITS BY REPRESENTATIVES OF THE STRUCTURAL ENGINEER OF RECORD ARE SOLELY FOR THE PURPOSE OF BECOMING GENERALLY FAMILIAR WITH THE PROGRESS AND QUALITY OF THE WORK AND DETERMINING, IN GENERAL, IF THE WORK OBSERVED IS BEING PERFORMED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THIS LIMITED OBSERVATION SHOULD NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS AND THAT OBSERVATIONS ARE QUALITATIVE, NOT QUANTITATIVE. THIS LIMITED INFORMATION WILL BE USED TO ADVISE THE OWNER/CONTRACTOR/ARCHITECT OF POTENTIAL DEFICIENCIES.
S. CLARIFICATION OF POSITION OF STRUCTURALLY FRAMING ELEMENTS
   1. USE ONLY DIMENSIONS INDICATED ON THE DRAWINGS, DO NOT SCALE ANY DIMENSIONS.
   2. IF NOT INDICATED ON DRAWINGS, ASSUME EQUAL SPACING BETWEEN ESTABLISHED DIMENSIONS.
   3. CENTER LINES OF COLUMNS AND FOUNDATIONS SHALL COINCIDE WITH GRID LINE INTERSECTION, UNLESS NOTED OTHERWISE.
   4. CENTER LINES OF FOUNDATIONS, GRADE BEAMS, AND WALLS SHALL COINCIDE WITH CENTER LINES OF FOUNDATIONS, UNLESS NOTED OTHERWISE.
   5. CENTER LINES OF FRAMING MEMBERS SHALL COINCIDE WITH COLUMN CENTER LINES, UNLESS NOTED OTHERWISE.
   6. ELEVATIONS SHOWN ARE TO TOP OF FOUNDATIONS, SLABS, OR BEAMS, UNLESS NOTED OTHERWISE.
T. SEE ARCHITECTURAL, CIVIL, MEP, AND VERTICAL TRANSPORTATION CONTRACT DOCUMENTS FOR ADDITIONAL INFORMATION RELATING TO THE COORDINATION OF STRUCTURAL COMPONENTS INCLUDING, BUT NOT LIMITED TO:
   1. CIVIL
      a. SITING OF BUILDING GRID LINES WITH RESPECT TO CITY BENCHMARKS
      b. SITE PREPARATION
      c. BACKFILLING MATERIALS AND REQUIREMENTS INCLUDING DRAINAGE ADJACENT TO RETAINING WALLS
      d. SITE ELEMENTS OUTSIDE OF BUILDING ENVELOPE
      e. NEW AND EXISTING SITE UTILITIES
   2. ARCHITECTURAL
      a. PLAN DIMENSIONS AND PROJECT DATUM
      b. SLAB EDGE DIMENSIONS AND FINISH ELEVATIONS
      c. WATERPROOFING AND DAMP PROOFING DETAILS
      d. SLAB SLOPES, STEPS AND DEPRESSIONS, RAMPS, TRENCHES
      e. EMBEDMENTS, INSERTS, BLOCKOUTS, ETC.
      f. CONCRETE FINISHES AND TOPPING SLABS
      g. CONCRETE CURBS AND HOUSEKEEPING PADS
      h. INTERIOR NON-STRUCTURAL MASONRY PARTITIONS
      i. LIFE SAFETY, FIRE RATING
      j. METAL PAN STAIRS AND SUPPORTS
      k. OPERABLE PARTITIONS
   3. MEP
      a. PIPE AND DUCT SIZES FOR OPENING AND SLEEVE COORDINATION
      b. FLOOR DRAINS
      c. UNDERFLOOR AND PERIMETER DRAINAGE SYSTEMS
      d. EQUIPMENT CURBS
      e. CONDUITS AND EMBEDMENTS IN WALLS AND SLABS
   4. VERTICAL TRANSPORTATION
      a. INSERTS, HANGERS, TRENCHES, PITS, CONDUITS IN WALLS AND SLAB

ELECTRONIC DATA/REPRODUCTION

A. ALL INFORMATION CONTAINED IN THE ELECTRONIC FILES OF THE CONTRACT DOCUMENTS ARE INSTRUMENTS OF SERVICE OF THE ARCHITECT/STRUCTURAL ENGINEER OF RECORD AND SHALL NOT BE USED FOR OTHER PROJECTS, ADDITIONS TO THE PROJECT, OR THE COMPLETION OF THE PROJECT BY OTHERS. ELECTRONIC FILES OF THE STRUCTURAL DOCUMENTS REMAIN THE PROPERTY OF JEZERINAC GROUP AND IN NO CASE SHALL THEIR TRANSFER BE CONSIDERED A SALE.
B. THE USE OF ELECTRONIC FILES OR REPRODUCTIONS OF THESE CONTRACT DOCUMENTS BY ANY CONTRACTOR, SUBCONTRACTOR, ERECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFIES THEIR ACCEPTANCE OF ALL INFORMATION SHOWN HEREIN AS CORRECT AND OBLIGATES THEMSELVES TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS OR OMISSIONS THAT MAY OCCUR HEREIN. THE USE OF ELECTRONIC FILES DOES NOT RELIEVE THE CONTRACTOR'S RESPONSIBILITY FOR PROPER CHECKING AND COORDINATION OF DIMENSIONS, DETAILS, SIZE, AND QUANTITIES.
C. DIMENSIONS AND ELEMENT SIZES AND LOCATIONS IN THE ELECTRONIC FILES MAY NOT BE PRECISE AND, IN SOME CASES, HAVE BEEN INTENTIONALLY ALTERED FOR PRESENTATION PURPOSES. DO NOT SCALE DIMENSIONS ELECTRONICALLY OR OTHERWISE.
D. WHEN USED FOR THE PREPARATION OF SHOP DRAWINGS, ALL INFORMATION NOT APPLICABLE TO THE SUBCONTRACT SHALL BE REMOVED FROM THE DRAWINGS, INCLUDING, BUT NOT LIMITED TO: SHEET NUMBERS, SECTION MARKS, TITLE BLOCKS, AND REFERENCES TO THE CONTRACT DOCUMENTS.

GOVERNING CODES & STANDARDS

BUILDING CODE:	FBC 2023	FLORIDA BUILDING CODE, BUILDING
STANDARDS:	ASCE 7	AMERICAN SOCIETY OF CIVIL ENGINEERS: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
	ACI 318	AMERICAN CONCRETE INSTITUTE: BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
	TMS 402	THE MASONRY SOCIETY: BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES
	AISC 360	AMERICAN INSTITUTE OF STEEL CONSTRUCTION: SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
	AWS D1.1	AMERICAN WELDING SOCIETY: STRUCTURAL WELDING CODE - STEEL
	AWS D1.3	AMERICAN WELDING SOCIETY: STRUCTURAL WELDING CODE - SHEET STEEL
	AWS D1.4	AMERICAN WELDING SOCIETY: STRUCTURAL WELDING CODE - REINFORCEMENT STEEL

SUBMITTALS

A. REFER TO DIVISION 01 OF SPECIFICATIONS FOR SUBMITTAL PROCEDURES AND REQUIREMENTS. REFER TO THE APPLICABLE SPECIFICATION SECTIONS FOR TECHNICAL CONTENT.
B. SUBMIT SPECIFIC COMPONENTS SUCH AS COLUMNS, FOUNDATIONS, ETC, IN A SINGLE PACKAGE. SUBMIT SIMILAR FLOORS TOGETHER.
C. **[TEN]** WORKING DAYS PRIOR TO SUBMITTING SHOP DRAWINGS, THE CONTRACTOR SHALL SUBMIT, FOR REVIEW AND COMMENT BY THE STRUCTURAL ENGINEER OF RECORD, A SCHEDULE WHICH DETAILS THE ESTIMATED QUANTITY OF SHOP DRAWINGS AND THE DATE THE SHOP DRAWINGS WILL BE RECEIVED BY THE STRUCTURAL ENGINEER OF RECORD. THE STRUCTURAL ENGINEER OF RECORD SHALL HAVE THE OPPORTUNITY TO REVIEW THE PROPOSED SCHEDULE AND SUBMIT COMMENTS TO THE CONTRACTOR. THE FINAL SHOP DRAWING SCHEDULE SHALL BE DEVELOPED AND SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD. IN ACCORDANCE WITH THE SHOP DRAWING SCHEDULE, THE STRUCTURAL ENGINEER OF RECORD WILL RETURN THE SHOP DRAWING ITEMS WITHIN TEN WORKING DAYS AFTER HAVING RECEIVED THE REPRODUCIBLE SHOP DRAWING.
D. THE CONTRACTOR SHALL REVIEW EACH SUBMITTAL PRIOR TO FORWARDING TO ARCHITECT AND STRUCTURAL ENGINEER OF RECORD. THE CONTRACTOR SHALL STAMP EACH SUBMITTAL VERIFYING THAT THE FOLLOWING IS ADDRESSED:
   1. THE SUBMITTAL IS REQUESTED.
   2. THE SUBMITTAL IS BASED ON THE LATEST DESIGN.
   3. THE SUBMITTAL IS CLEARLY CLOUDED FOR ALL THE DIFFERENCES FROM THE CONTRACT DOCUMENTS ON THE FIRST SUBMITTAL.
   4. THE SUBMITTAL IS CLEARLY CLOUDED FOR ALL CHANGES AND ADDITION FROM PREVIOUS SUBMITTAL.
   5. THE ARCHITECT'S AND STRUCTURAL ENGINEER OF RECORD'S COMMENTS FROM ANY PREVIOUS SUBMITTALS ARE ADDRESSED.
   6. THE WORK IS COORDINATED AMONGST ALL CONSTRUCTION TRADES.
   7. THE SUBMITTAL IS COMPLETE.
   8. THE SUBMITTAL SHALL INCLUDE A STAMP INDICATING PROJECT NAME AND LOCATION, SUBMITTAL NUMBER, AND SPECIFICATION SECTION NUMBER.
E. THE STRUCTURAL ENGINEER OF RECORD'S REVIEW OF SUBMITTALS SHALL BE FOR GENERAL CONFORMANCE WITH THE DESIGN INTENT.
F. THE STRUCTURAL ENGINEER OF RECORD SHALL RETURN, WITHOUT COMMENT, SUBMITTALS WHICH THE CONTRACTOR HAS NOT STAMPED OR WHICH DO NOT MEET THE ABOVE REQUIREMENTS.
G. FOR THE COMPONENTS DESIGNED BY A DELEGATED ENGINEER; PROVIDE SHOP DRAWINGS, DESIGN CALCULATIONS, AND A COVER LETTER SIGNED AND SEALED BY THE DELEGATED ENGINEER. LETTER SHALL INDICATE THAT THE SHOP DRAWINGS ARE IN CONFORMANCE WITH THE DELEGATED ENGINEER'S CALCULATIONS. REFER TO APPLICABLE SPECIFICATION SECTIONS FOR ADDITIONAL REQUIREMENTS.
H. DEFERRED SUBMITTALS ARE MANUFACTURER OR CONTRACTOR DESIGNED COMPONENTS PER THE CONTRACT DOCUMENTS. THESE ELEMENTS OF THE DESIGN ARE DEFERRED SUBMITTAL COMPONENTS AND HAVE NOT BEEN PERMITTED UNDER THE BASE BUILDING APPLICATION. DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE ARCHITECT/STRUCTURAL ENGINEER OF RECORD, WHO SHALL REVIEW THEM FOR GENERAL CONFORMANCE TO THE DESIGN OF THE BUILDING. THE CONTRACTOR SHALL SUBMIT THESE REVIEWED DEFERRED SUBMITTAL DOCUMENTS TO THE BUILDING OFFICIAL FOR APPROVAL. THESE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DESIGN TEAM HAS REVIEWED AND THE BUILDING OFFICIAL HAS APPROVED. SEE BELOW FOR THE LIST OF DEFERRED SUBMITTALS.
I. THE FOLLOWING SUBMITTALS ARE REQUIRED TO BE SUBMITTED FOR STRUCTURAL ENGINEER OF RECORD REVIEW AS OUTLINED IN THE SPECIFICATIONS:

031000	CONCRETE FORMWORK	(SS, CALC)
032000	CONCRETE REINFORCEMENT LAYOUT	(S)
033000	CONCRETE MIX DESIGNS	(CALC, TA)
033000	CONCRETE CONSTRUCTION JOINT LAYOUT	(S)
042200	MASONRY REINFORCEMENT LAYOUT	(S)
051200	STRUCTURAL STEEL	(S)
051200	STRUCTURAL STEEL CONNECTIONS	(DF, S, CALC)
051200	SHEAR STUD LAYOUT	(S)
051400	STRUCTURAL ALUMINUM FRAMING	(DF, SS, CALC)
052100	STEEL JOISTS, BRIDGING, AND CONNECTIONS	(DF, SS, CALC)
053100	STEEL COMPOSITE DECK	(S)
053100	STEEL FORM DECK	(S)
053100	STEEL ROOF DECK	(S)
	SHORING AND RESHORING	(DF, SS, CALC)

S = SHOP DRAWING REQUIRED
DF = DEFERRED SUBMITTAL
SS = SIGNED AND SEALED SHOP DRAWINGS PREPARED BY A LICENSED DELEGATED ENGINEER IN THE STATE IN WHICH THE PROJECT IS LOCATED
CALC = SUPPORTING CALCULATIONS REQUIRED, SIGNED AND SEALED BY A LICENSED DELEGATED ENGINEER IN THE STATE IN WHICH THE PROJECT IS LOCATED.
REC = ITEMS SUBMITTED FOR RECORD ONLY AND WILL NOT HAVE STRUCTURAL ENGINEER OF RECORD SHOP DRAWING STAMP AFFIXED.
TA = ITEMS SUBMITTED TO OWNER'S TESTING AGENCY FOR THEIR REVIEW.

ABBREVIATIONS

ADDL	ADDITIONAL
ADJ	ADJACENT
AFF	ABOVE FINISHED FLOOR
ALT	ALTERNATE
APPROX	APPROXIMATE
ARCH	ARCHITECT OR ARCHITECTURAL
ASD	ALLOWABLE STRESS DESIGN
B/	BOTTOM OF
B/B	BACK-TO-BACK
BLDG	BUILDING
BLKG	BLOCKING
BRG	BEARING
BOT	BOTTOM
BTWN	BETWEEN
C	COMPRESSION
CFS	COLD-FORMED STEEL
CIP	CAST-IN-PLACE
CJ	CONTRACTION JOINT
CL	CENTER LINE
CLR	CLEAR OR CLEARANCE
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONN(S)	CONNECTION(S)
CONST	CONSTRUCTION
CONT	CONTINUOUS
COORD	COORDINATE
db	REINFORCING BAR DIAMETER
DBA	DEFORMED BAR ANCHOR
DCW	DEMAND CRITICAL WELD
°	DEGREE(S)
Ø	DIAMETER
DIAG	DIAGONAL
DIM(S)	DIMENSION(S)
DL	DEAD LOAD
DWG(S)	DRAWING(S)
EA	EACH
EF	EACH FACE
EJ	EXPANSION JOINT
EL	ELEVATION
ELEV	ELEVATOR
EOS	EDGE-OF-SLAB
EQ	EQUAL
EQUIP	EQUIPMENT
EW	EACH WAY
EXIST	EXISTING
EXP	EXPANSION
EXT	EXTERIOR
F/F	FACE-TO-FACE
FD	FLOOR DRAIN
FF	FINISH FLOOR
FND	FOUNDATION
FS	FAR SIDE
FT	FEET
FTG	FOOTING
GA	GAGE, GAUGE
GALV	GALVANIZED
GB	GRADE BEAM
GC	GENERAL CONTRACTOR
GDR	GIRDER
GEN	GENERAL
GYP	GYPSUM
HCA	HEADED CONCRETE ANCHORS
HORIZ	HORIZONTAL
HSS	HOLLOW STRUCTURAL SECTION
ID	INSIDE DIAMETER
IF	INSIDE FACE
IN	INCH
INFO	INFORMATION
INT	INTERIOR
JST(S)	JOIST(S)
K	KIPS (1,000 POUNDS)
KLF	KIP PER LINEAR FOOT
KSF	KIP PER SQUARE FOOT

ABBREVIATIONS

KSI	KIPS PER SQUARE INCH
L	LENGTH
LB(S)	POUND(S)
LL	LIVE LOAD
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG	LONGITUDINAL
LRFD	LOAD RESISTANCE AND FACTOR DESIGN
LSH	LONG SIDE HORIZONTAL
LSV	LONG SIDE VERTICAL
LTS	LAP TENSION SPLICE
LW	LIGHT WEIGHT
LWC	LIGHT WEIGHT CONCRETE
M	MOMENT
MAX	MAXIMUM
MC	MOMENT CONNECTION(S)
MECH	MECHANICAL
MEP	MECHANICAL, ELECTRICAL, PLUMBING, FIRE PROTECTION
MFR	MANUFACTURER
MID	MIDDLE
MIN	MINIMUM
MISC	MISCELLANEOUS
NIC	NOT IN CONTRACT
NS	NEAR SIDE
NTS	NOT TO SCALE
NWC	NORMAL WEIGHT CONCRETE
NSFC	NOT SHOWN FOR CLARITY
OC	ON CENTER
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
OH	OPPOSITE HAND
OPNG(S)	OPENING(S)
OPP	OPPOSITE
OSL	OUTSTANDING LEG
PAF	POWDER ACTUATED FASTENER
PERP	PERPENDICULAR
PJF	PREFORMED JOINT FILLER
PL	PLATE
PLF	POUNDS PER LINEAR FOOT
PCST	PRECAST
PREFAB	PRE-FABRICATED
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	POST-TENSIONED
REF	REFERENCE
REINF	REINFORCE(D) (ING) OR (MENT)
REQ(D)	REQUIRE(D)
REV	REVISION
RTU	ROOF TOP UNIT
SCHED	SCHEDULE(D)
SDL	SUPERIMPOSED DEAD LOAD
SER	STRUCTURAL ENGINEER OF RECORD
SF	SQUARE FOOT (FEET)
SIM	SIMILAR
SLRS	SEISMIC LOAD RESISTING SYSTEM
SOG	SLAB-ON-GROUND
SP	SPACE
SPEC(S)	SPECIFICATION(S)
SS	STAINLESS STEEL
STD	STANDARD
STIFF	STIFFENER
STR	STRUCTURE OR STRUCTURAL
SYM	SYMMETRICAL
T	TENSION
T&B	TOP AND BOTTOM
T&G	TONGUE & GROOVE
T/	TOP OF
TEMP	TEMPERATURE OR TEMPORARY
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
V	SHEAR
VERT	VERTICAL
VIF	VERIFY IN FIELD
W/	WITH
W/O	WITHOUT
WP	WORK POINT
WWR	WELDED WIRE REINFORCEMENT

Grand total: 172

WILLIAM JOHN RAIOLA — LICENSE — No. 80234 — William J Raiola — STATE OF FLORIDA — PROFESSIONAL ENGINEER
(I) approving this document
2026.05.29 12:09:55-04'00'
05/29/2026

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

JEZERINAC GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 286 9004
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS

ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date

CONTRACT DOCUMENTS FOR:
SLPS C.A. MOORE GYMNASIUM
ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION

GENERAL NOTES

PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W.JR/TSB

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

S001

5/29/2026 3:58:00 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR_JG-CA Moore_R26.rvt

DESIGN CRITERIA

A. STRUCTURE LOCATION:
LONGITUDE: -80.356
LATITUDE: 27.458

B. LOADING:
1. SUPERIMPOSED DEAD & LIVE LOADS:
ALL DEAD LOADS LISTED BELOW ARE IN ADDITION TO THE STRUCTURE'S SELF-WEIGHT.

	DEAD LOAD	LIVE LOAD
ROOF:		
TYPICAL	12 PSF	20 PSF REDUCIBLE
MECHANICAL WELL	60 PSF	100 PSF NON-REDUCIBLE
MECHANICAL EQUIPMENT	SEE PLAN	N/A

2. RAIN LOAD:
DESIGN RAINFALL: 5.40"/HOUR (100-YEAR, 1-HOUR RAINFALL)
RAINWATER AT LOWEST POINT OF ROOF SHALL NOT POND DURING DESIGN RAINFALL
DESIGN RAIN LOAD, R: 20 PSF
3. WIND LOAD:
ULTIMATE DESIGN WIND SPEED, V_{ult}: 170 MPH
NOMINAL DESIGN WIND SPEED, V_{asd}: 132 MPH
RISK CATEGORY: III
EXPOSURE CATEGORY: B
ENCLOSURE CLASSIFICATION: ENCLOSED
INTERNAL PRESSURE COEFFICIENT: +/- 0.18
COMPONENTS & CLADDING DESIGN PRESSURES: SEE WIND PRESSURE DIAGRAMS
4. TORNADO LOAD:
RISK CATEGORY: III
ULTIMATE DESIGN TORNADO SPEED, V_T: 50 MPH
NOMINAL DESIGN WIND SPEED, V_{Tasd}: 39 MPH
EXPOSURE CATEGORY: B
PER 32.5.2 OF ASCE 7, V_T < *60 MPH. DESIGN FOR TORNADO LOADS IS NOT REQUIRED.*
5. SEISMIC LOAD:
RISK CATEGORY: III
SEISMIC IMPORTANCE FACTOR, I_e: 1.25
SITE CLASS: D
MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER, S_s: 0.062 g
MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER, S_1: 0.028 g
SPECTRAL RESPONSE ACCELERATION PARAMETER, S_{DS}: 0.056 g
SPECTRAL RESPONSE ACCELERATION PARAMETER, S_{D1}: 0.040 g
SEISMIC DESIGN CATEGORY: A
SEISMIC FORCE RESISTING SYSTEM: NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE

C. HANDRAIL AND GUARDRAIL LOADS:
CONCENTRATED AND DISTRIBUTED LOADS ARE TO BE APPLIED AT THE HANDRAIL OR TOP RAIL IN ANY DIRECTION. CONCENTRATED AND DISTRIBUTED LOADS ARE NOT TO BE APPLIED CONCURRENTLY.
CONCENTRATED LOAD 200 LB
DISTRIBUTED LOAD 50 PLF

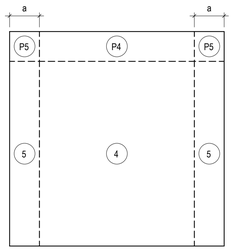
D. FUTURE EXPANSION:
NO PROVISIONS HAVE BEEN MADE FOR FUTURE VERTICAL OR HORIZONTAL EXPANSION OF THE STRUCTURE.

E. SERVICEABILITY:
1. DEFLECTION LIMITS: TOTAL LOAD DEFLECTION ONLY APPLIES TO THE DEFLECTION DUE TO THE CREEP COMPONENT OF LONG-TERM DEAD LOAD DEFLECTION PLUS THE SHORT-TERM DEFLECTION. LONG-TERM DEFLECTION OF WOOD STRUCTURAL MEMBERS SHALL BE CALCULATED IN ACCORDANCE WITH THE AWC NDS. IT IS PERMITTED TO ESTIMATE THE CREEP-COMPONENT OF THE LONG-TERM DEFLECTION AS THE IMMEDIATE DEAD LOAD DEFLECTION.
a. ROOF MEMBERS
- TOTAL LOAD DEFLECTION: L/240
- TRANSITORY LOAD DEFLECTION: L/360

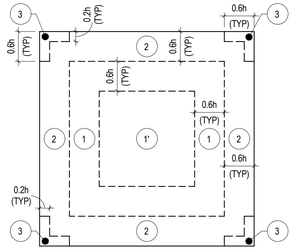
b. FLOOR MEMBERS
- TOTAL LOAD DEFLECTION: L/240
- LIVE LOAD DEFLECTION: L/360

c. EXTERIOR WALLS & CLADDING
- WIND LOAD DEFLECTION: L/360

d. INTERIOR PARTITIONS
- LIVE LOAD DEFLECTION: L/360



WALL ELEVATION



MONOSLOPE ROOF PLAN (θ ≤ 3°)

NOTES:

1. FOR COMPONENTS HAVING EFFECTIVE AREAS IN BETWEEN TABULATED VALUES, DESIGN LOADS MAY BE INTERPOLATED. OTHERWISE DESIGN LOAD SHALL BE TAKEN FROM THE NEXT LOWEST TABULATED EFFECTIVE AREA.
2. DESIGN VALUES SHOWN IN THIS TABLE ARE ULTIMATE VALUES FOR USE WITH ASD DESIGN. REFER TO THE BUILDING CODE FOR APPLICABLE LOAD COMBINATIONS.
3. a = 9'-8". SEE ROOF PLAN MAP BELOW FOR LOCATION OF a-ZONES. WALL a-ZONE LOCATIONS TO MATCH ROOF a-ZONES.
4. h = 27'-4". SEE ROOF PLAN MAP BELOW FOR LOCATION OF h_zZONES.
5. POSITIVE PRESSURE VALUES REFER TO FORCES ACTING TOWARDS BUILDING OR COMPONENT FACE, NEGATIVE PRESSURE VALUES REFER TO FORCES ACTING AWAY FROM BUILDING OR COMPONENT FACE.
6. EACH COMPONENT AND ITS CONNECTION SHALL BE DESIGNED FOR MAXIMUM POSITIVE AND NEGATIVE FORCES.
7. PARAPET COMPONENTS AND CLADDING ARE THOSE ELEMENTS WHICH EXIST ABOVE THE HORIZONTAL PLANE OF THE ROOF AND SHALL BE DESIGNED FOR:
- POSITIVE AND NEGATIVE PRESSURES 4 OR 5 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON OUTSIDE FACE.
- POSITIVE PRESSURES 4 OR 5 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON ROOF SIDE FACE.
- NEGATIVE PRESSURES 2 OR 3 APPLIED TO THE SHEATHING OR PANELING AND ITS CONNECTION ON ROOF SIDE FACE.
- P4/5 SHALL BE APPLIED TO THE DESIGN OF THE STRUCTURAL ELEMENT OF THE PARAPET AND ITS CONNECTION, INCLUDING BUT NOT LIMITED TO THE STUD FRAMING OF THE PARAPET.

8. ROH# : DENOTES DESIGN WIND PRESSURE VALUES WHICH SHALL BE APPLIED AT ROOF OVERHANGS TO TOP SURFACE CLADDING OR SHEATHING AND ITS CONNECTION. SOFFIT CLADDING OR SHEATHING SHALL BE DESIGNED FOR SIMILAR PRESSURE TO THE ADJACENT WALL PRESSURE. A COMBINATION OF THESE FORCES SHALL BE APPLIED TO THE STRUCTURAL ELEMENT OF THE OVERHANG AND ITS CONNECTION, INCLUDING BUT NOT LIMITED TO THE STUD FRAMING OF THE OVERHANG.
9. ALL DOORS TO BE RATED TO RESIST DESIGN WIND PRESSURES SPECIFIED.

COMPONENT & CLADDING EXTERNAL WIND PRESSURE LOADS (PER ASCE 7-22)

ULTIMATE WIND SPEED, Vult		170 mph	RISK CATEGORY:		III		EDGE DISTANCE, a (ft)		10.8				
NOMINAL WIND SPEED, Vasd		132 mph	EXPOSURE CATEGORY:		C		ROOF SLOPE (DEGREES)		0.25/12				
			DIRECTIONALITY FACTOR, Kd =		0.85								
COMPONENT LOCATION			POSITIVE PRESSURES (psf)						NEGATIVE PRESSURES (psf)					
EFFECTIVE AREA, Ae (ft²)			10	20	50	100	200	500	10	20	50	100	200	500
ROOFS	ZONE 1		25.9	24.2	22.1	20.5	20.5	20.5	-101.2	-94.6	-85.7	-79.1	-72.4	-63.6
	ZONE 1'		25.9	24.2	22.1	20.5	20.5	20.5	-58.2	-58.2	-58.2	-58.2	-50.1	-39.3
	ZONE 2		63.5	60.7	56.9	54.0	51.2	47.4	-133.6	-125.0	-113.6	-105.0	-96.4	-85.1
	ZONE 3		63.5	60.7	56.9	54.0	51.2	47.4	-133.6	-125.0	-113.6	-105.0	-96.4	-85.1
WALLS	ZONE 4		63.5	60.7	56.9	54.0	51.2	47.4	-68.9	-66.1	-62.3	-59.4	-56.6	52.8
	ZONE 5		63.5	60.7	56.9	54.0	51.2	47.4	-85.1	-79.4	-71.8	-66.1	-60.3	-52.8

NOTES:

1. "Ae" INDICATES EFFECTIVE AREA AS DEFINED BY SECTION 26.2 OF ASCE 7.
2. PRESSURE VALUES IN ABOVE TABLE ARE BASED ON THE PARAMETERS LISTED AT THE TOP OF THE TABLE.
3. PRESSURES VALUES IN THE ABOVE TABLE ARE FOR ENCLOSED BUILDING CONDITION WITH INTERNAL PRESSURE COEFFICIENT OF +/-0.18.
4. GLAZED OPENINGS SHALL BE PROTECTED IN ACCORDANCE WITH SECTION 26.10.3 OF ASCE 7.
5. POSITIVE OR NEGATIVE DESIGN PRESSURES SHALL NOT BE TAKEN LESS THAN 16PSF (ULTIMATE VALUE) OR 10PSF (NOMINAL VALUE).
6. POSITIVE WIND PRESSURES ACT TOWARD THE SURFACE, AND NEGATIVE PRESSURES ACT AWAY FROM THE SURFACE.
7. PRESSURES IN TABLE ARE ALLOWABLE PRESSURES. NO FURTHER REDUCTIONS PERMITTED.

WILLIAM JOHN RAIOLA LICENSE No. 80234 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/29/2026

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JEZERINAC GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 286 9004
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS

ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date

CONTRACT DOCUMENTS FOR:
SLPS C.A. MOORE GYMNASIUM
ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION

LOADING CRITERIA

PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W./PETER

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

S002

5/29/2026 3:58:01 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR-JG-CA Moore_R26.rvt

EARTHWORK & FOUNDATIONS

A. GEOTECHNICAL INVESTIGATION REPORT
   1. FOUNDATION DESIGN IS BASED ON THE GEOTECHNICAL INVESTIGATION REPORT AS FOLLOWS:
      a. *REPORT TITLE: GEOTECHNICAL EXPLORATION - C.A. MOORE K8 ACADEMY OF SUCCESS NEW GYMNASIUM PROJECT (REPORT No. A24153.00166.001)*
      b. *PREPARED BY: UES, 607 NW COMMODITY COVE, FL*
      c. *DATED: 09/18/2025*
   2. THE GEOTECHNICAL INVESTIGATION REPORT IS AVAILABLE TO THE CONTRACTOR UPON REQUEST TO THE OWNER. THE INFORMATION HEREIN MAY BE USED BY THE CONTRACTOR FOR HIS GENERAL REFERENCE ONLY. THE GEOTECHNICAL INVESTIGATION REPORT RECOMMENDATIONS SHALL SUPERSEDE THE MINIMUM CRITERIA STATED IN THE STRUCTURAL GENERAL NOTES.

B. SHALLOW FOUNDATIONS
   1. FOUNDATIONS ARE DESIGNED IN ACCORDANCE WITH THE GEOTECHNICAL INVESTIGATION REPORT.
   2. FOUNDATION SIZES AND REINFORCEMENT ARE BASED ON AN ALLOWABLE BEARING PRESSURE OF 2500 PSF PER THE GEOTECHNICAL INVESTIGATION REPORT.
   3. FOUNDATIONS SHALL BEAR A MINIMUM OF 1'-0" BELOW ADJACENT EXTERIOR GRADE.
   4. FOUNDATIONS SHALL BEAR ON COMPACTED STRUCTURAL FILL, NATURAL SOILS, OR ROCK PREPARED PER THE GEOTECHNICAL INVESTIGATION REPORT.
   5. PRIOR TO PLACEMENT OF CONCRETE, A QUALIFIED GEOTECHNICAL ENGINEER SHALL VERIFY SOILS CONFORMANCE TO THE RECOMMENDATIONS AND ASSUMPTIONS IN THE GEOTECHNICAL INVESTIGATION REPORT. ALL ADVERSE CONDITIONS SHALL BE REPORTED TO THE ARCHITECT/ STRUCTURAL ENGINEER OF RECORD.
   6. SOILS BELOW FOUNDATIONS NOT MEETING DESIGN BEARING PRESSURE SHALL BE REMEDIATED PER THE GEOTECHNICAL INVESTIGATION REPORT AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF THE FOUNDATIONS.
   7. CENTER ALL FOUNDATIONS UNDER THEIR RESPECTIVE COLUMNS OR WALLS, UNLESS NOTED OTHERWISE.
   8. TOP OF FOUNDATION ELEVATIONS PROVIDED ON THE CONTRACT DRAWINGS ARE FOR PURPOSE OF THE CONTRACT AND SHALL BE ADJUSTED, AS REQUIRED, AT THE TIME OF EXCAVATION TO BEAR ON PROPERLY PREPARED SUPPORT SUBGRADE (PER THE GEOTECHNICAL ENGINEER'S RECOMMENDATIONS).

C. EARTHWORK AND EXCAVATION
   1. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING, BUT NOT LIMITED TO: LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS, AND UTILITIES IN ACCORDANCE WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT AND OSHA REGULATIONS.
   2. EXCAVATION SHALL NOT OCCUR WITHIN ONE FOOT OF THE ANGLE OF REPOSE OF ANY SOIL BEARING FOUNDATION UNLESS THE FOUNDATION IS PROTECTED AGAINST SETTLEMENT.
   3. THE EXTENT OF SUBGRADE PREPARATION SHALL EXTEND A MINIMUM OF 5'-0" BEYOND THE BUILDING PERIMETER.
   4. THE CONTRACTOR SHALL PROVIDE A SUBGRADE BENEATH THE SLAB-ON-GROUND PER THE GEOTECHNICAL ENGINEER'S RECOMMENDATIONS.
   5. UNLESS NOTED IN THE GEOTECHNICAL INVESTIGATION REPORT, COMPACT FILL TO 95% OF MAXIMUM DRY DENSITY AS DETERMINED BY MODIFIED PROCTOR ASTM D-1557. EACH LAYER SHALL NOT EXCEED 8" LOOSE THICKNESS. COMPACT PRIOR TO THE PLACEMENT OF THE NEXT LAYER. COMPACTION SHALL MEET ALL RECOMMENDATIONS OF THE GEOTECHNICAL INVESTIGATION REPORT.
   6. PLACEMENT OF FILL AND COMPACTION SHALL BE MONITORED AND ACCEPTED BY A RETAINED TESTING AGENCY. PERFORM A MINIMUM OF ONE FIELD DENSITY TEST (ASTM D-1556 OR D-6938) FOR EVERY 2,500 SQUARE FEET OF EACH LAYER. THE TESTING AGENCY SHALL RANDOMLY SELECT TEST LOCATIONS.
   7. THE CONTRACTOR SHALL DETERMINE THE EXTENT OF THE CONSTRUCTION DEWATERING SYSTEMS REQUIRED FOR THE EXCAVATION. AT A MINIMUM, THE CONTRACTOR SHALL PROVIDE POSITIVE DRAINAGE AWAY FROM THE BUILDING SITE.
   8. THE CONTRACTOR SHALL SUBMIT CONSTRUCTION DEWATERING PLAN TO THE GEOTECHNICAL ENGINEER FOR APPROVAL PRIOR TO BEGINNING EXCAVATION.
   9. THE CONTRACTOR SHALL INSTALL ALL NECESSARY DEWATERING SYSTEMS.

CONCRETE

A. ALL CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH DIVISION 03 OF THE SPECIFICATIONS.
B. FOR CONCRETE MIXTURE REQUIREMENTS SEE SCHEDULE ON THIS SHEET.
C. THE USE OF RECYCLED CONCRETE IS PROHIBITED WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD.
D. NORMAL WEIGHT CONCRETE SHALL BE USED FOR ALL CONCRETE MEMBERS UNLESS NOTED OTHERWISE. NORMAL WEIGHT CONCRETE SHALL HAVE A CURED DENSITY OF 145 PCF ±5 PCF. WHERE LIGHT WEIGHT CONCRETE IS SPECIFIED THE CURED DENSITY SHALL BE 112 PCF ±3 PCF.
E. EACH MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER AND THE INTENDED LOCATION OF PLACEMENT ON THE SPECIFIC PROJECT SHALL BE CLEARLY STATED.
F. ALL PROPOSED CONSTRUCTION JOINT LOCATIONS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL. HORIZONTAL CONSTRUCTION JOINTS SHALL NOT BE PERMITTED IN BEAMS, WALLS, AND SLABS UNLESS SPECIFICALLY SHOWN ON STRUCTURAL DRAWINGS OR BY WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD. FOR MILD REINFORCED MEMBERS, CONSTRUCTION JOINTS SHALL OCCUR WITHIN THE MIDDLE THIRD OF A MEMBER'S SPAN. ALL APPROVED CONSTRUCTION JOINTS SHALL BE INDICATED, DIMENSIONED, AND DETAILED ON THE CONCRETE REINFORCEMENT SHOP DRAWINGS.
G. GIRDERS, BEAMS, HAUNCHES, DROP PANELS, DROP CAPS, AND CAPITALS SHALL BE POURED MONOLITHICALLY AS PART OF THE SLAB SYSTEM UNLESS NOTED OTHERWISE.
H. PROVIDE A ¾ INCH CHAMFER AT ALL EXPOSED CORNERS OF BEAMS, WALLS, ETC UNLESS NOTED OTHERWISE.
I. CONCRETE CORING AND INSTALLATION OF DRILLED ANCHORS IS NOT PERMITTED WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD.
J. REFER TO THE ARCHITECTURAL DRAWINGS FOR ALL CONCRETE DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL COORDINATE BETWEEN THE ARCHITECTURAL, STRUCTURAL, AND MEP DRAWINGS TO FURNISH DIMENSIONED DRAWINGS THAT LOCATE AND SIZE ALL SLAB EDGES, OPENINGS, AND PENETRATIONS. THESE DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL.
K. EMBEDDED CONDUITS, PIPES, AND SLEEVES
   1. THE OUTSIDE DIAMETER OF CONDUITS, PIPES, AND SLEEVES SHALL NOT EXCEED ONE-THIRD THE THICKNESS OF THE SLAB, WALL OR BEAM IN WHICH THEY ARE EMBEDDED. EMBEDMENTS SHALL NOT SIGNIFICANTLY REDUCE THE CAPACITY OF THE MEMBERS THEY PENETRATE.
   2. THE MINIMUM CLEAR COVER FOR CONDUITS, PIPES, AND SLEEVES SHALL BE 1 ½" FOR CONCRETE EXPOSED TO EARTH OR WEATHER AND ¾" FOR CONCRETE NOT EXPOSED TO EARTH OR WEATHER.
   3. ALUMINUM EMBEDMENTS AND EMBEDMENTS MADE OF ANY OTHER MATERIAL HARMFUL TO THE CONCRETE OR REINFORCEMENT ARE PROHIBITED.
   4. EMBEDMENTS NOT SHOWN ON THE CONTRACT DOCUMENTS SHALL BE DESIGNED TO RESIST THE EFFECTS OF MATERIAL, PRESSURE, AND TEMPERATURE THAT THEY WILL BE SUBJECTED TO. THE WORK SHALL BE COORDINATED AMONGST ALL CONSTRUCTION TRADES.
   5. THE CONTENTS OF EMBEDDED PIPES SHALL NOT FLOW UNTIL THE CONCRETE HAS REACHED ITS SPECIFIED DESIGN STRENGTH.
   6. CONDUITS, PIPES, AND SLEEVES SHALL BE PLACED BETWEEN TOP AND BOTTOM LAYERS OF REINFORCEMENT IN SLABS AND BETWEEN INNER AND OUTER LAYERS OF REINFORCEMENT IN WALLS.
   7. EMBEDDED ITEMS SHALL BE FABRICATED AND INSTALLED SUCH THAT CUTTING, BENDING, OR DISPLACEMENT OF REINFORCEMENT FROM ITS SPECIFIED LOCATION IS NOT REQUIRED.

CONCRETE REINFORCEMENT

A. ALL CONCRETE REINFORCEMENT SHALL BE INSTALLED IN ACCORDANCE WITH DIVISION 03 OF THE SPECIFICATIONS.
B. ALL REINFORCING STEEL SHALL BE ASTM A615, GRADE 60 UNLESS NOTED OTHERWISE.
C. WHERE WELDS ARE INDICATED FOR REINFORCING STEEL ON THE DRAWINGS, REINFORCING STEEL SHALL BE A706, GRADE 60 UNLESS OTHERWISE NOTED.
D. WELDED WIRE REINFORCEMENT SHALL CONFORM TO THE MATERIAL REQUIREMENTS OF ASTM A1064.
E. ALL 90°, 135°, AND 180° HOOKED REINFORCEMENT SPECIFIED AND GRAPHICALLY DEPICTED IN THE CONTRACT DOCUMENTS SHALL BE DETAILED IN ACCORDANCE WITH ACI 318 STANDARD HOOK GEOMETRY FOR DEFORMED BARS IN TENSION AND FOR STIRRUPS, TIES, AND HOOPS.
F. FOR EVERY VERTICAL OR HORIZONTAL BAR DISCONTINUED BY AN OPENING, ONE BAR (MINIMUM OF 2 BARS) SHALL BE ADDED AT SIDE OF OPENING (HALF TO EACH SIDE, TYPICAL).
G. FOR CONCRETE CLEAR COVER TO REINFORCEMENT SEE SCHEDULE ON THIS SHEET, UNLESS NOTED OTHERWISE. CLEAR COVER IN PARENTHESES () DENOTES CLEAR COVER WHEN THE AS-BUILT APPLICATION IS EXPOSED TO WEATHER.
H. ALL LAP SPLICES SHALL BE CLASS B TENSION LAP SPLICES IN ACCORDANCE WITH ACI 318 UNLESS NOTED OTHERWISE. SEE LAP SPLICE SCHEDULE ON THIS SHEET FOR LAP SPLICE LENGTHS. UNLESS NOTED AS CONTINUOUS, REINFORCEMENT SHALL ONLY BE SPLICED AT LOCATIONS SHOWN ON THE CONTRACT DOCUMENTS. SPLICES AT NON-SPECIFIED LOCATIONS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL.
I. A MINIMUM LAP SPLICE OF 8" SHALL BE PROVIDED AT ALL END AND SIDE LAP CONDITIONS FOR WELDED WIRE REINFORCEMENT UNLESS NOTED OTHERWISE.
J. MECHANICAL SPLICES ARE REQUIRED WHERE SPECIFIED ON THE CONTRACT DOCUMENTS. MECHANICAL SPLICES ARE ALSO REQUIRED TO SPLICE #14 AND #18 BARS. MECHANICAL SPLICES MAY ALSO BE USED AT THE CONTRACTOR'S OPTION, PROVIDED THE MECHANICAL SPLICES HAVE A CURRENT ICC-ES REPORT DEMONSTRATING THEY CAN DEVELOP 125% OF THE SPECIFIED YIELD STRENGTH OF THE BAR IN TENSION OR COMPRESSION. MECHANICAL SPLICES SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL.
K. THE USE OF WELDED SPLICES IS PROHIBITED UNLESS NOTED OTHERWISE. THE CONTRACTOR SHALL SUBMIT THE LOCATIONS OF WELDED SPLICES TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL. IF APPROVED, WELDED SPLICES SHALL CONFORM TO THE REQUIREMENTS OF AWS D1.
L. DOWELS SHALL MATCH SIZE AND SPACING OF PRIMARY REINFORCEMENT UNLESS NOTED OTHERWISE.
M. SEE TYPICAL DETAILS FOR REINFORCEMENT REQUIRED AT OPENINGS AND PENETRATIONS.
N. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE REINFORCEMENT BAR SIZES AND PLACEMENT. WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS, ELEVATIONS, AND DETAILS IS NOT ACCEPTABLE.

CONCRETE MIXTURE REQUIREMENTS

APPLICATION	EXPOSURE CLASS	f'c (PSI)	TEST AGE	MODULUS OF ELASTICITY (KSI)	MAXIMUM W/CM	AIR CONTENT	NOMINAL MAXIMUM AGGREGATE	MAXIMUM CONCRETE WEIGHT (PCF)
FOUNDATIONS	F0, S1, C2, W1	3000	28 DAYS	3122	SEE NOTE 2	SEE NOTE 3	1"	150
EXTERIOR SLAB-ON-GROUND	F0, S1, C1, W1	4500	28 DAYS	3824	0.55	4.5% ± 1.5%	1"	150
SLAB-ON-GROUND	F0, S0, C0, W0	3000	28 DAYS	3122	SEE NOTE 2	SEE NOTE 3	1"	150
ELEVATED SLABS ON STEEL DECK	F0, S0, C1, W2	4000	28 DAYS	3605	SEE NOTE 2	SEE NOTE 3	3/4"	145
BEAMS	F0, S0, C1, W2	5000	28 DAYS	4031	SEE NOTE 2	SEE NOTE 3	3/4"	150
COLUMNS	F0, S0, C1, W1	5000	28 DAYS	3605	SEE NOTE 2	SEE NOTE 3	3/4"	150
CMU FILLED CELLS	GROUT MIX SHALL BE USED FOR CMU FILLED CELLS - SEE CONCRETE UNIT MASONRY GENERAL NOTES, DETAILS, & SPECIFICATION							

NOTES:
1. EXPOSURE CATEGORIES AND CLASSES FOR SULFATES, PERMEABILITY, AND CORROSION PROTECTION OF REINFORCEMENT IS CLASS ZERO UNLESS NOTED OTHERWISE.
2. WATER/CEMENT RATIO SHALL BE AS REQUIRED FOR THE SPECIFIED CONCRETE MIX DESIGN. THERE IS NO MAXIMUM WATER/CEMENT RATIO REQUIREMENT FOR THE EXPOSURE CLASSIFICATION ASSOCIATED WITH THIS APPLICATION. MAXIMUM WATER/CEMENT RATIO IS NOT APPLICABLE FOR DURABILITY REQUIREMENTS IN LIGHTWEIGHT CONCRETE.
3. THERE IS NO MANDATORY TARGET AIR CONTENT FOR THIS APPLICATION. THE CONTRACTOR MAY CHOOSE TO ADD AIR ENTRAINMENT TO IMPROVE THE WORKABILITY AND FINISHING PROPERTIES OF THE MIX. AIR CONTENT SHALL BE AS REQUIRED FOR THE SPECIFIED CONCRETE MIX.

CLASS B TENSION LAP SPLICE LENGTHS (INCHES)

	f'c = 3000 PSI				f'c = 4000 PSI				f'c = 5000 PSI			
	BEAMS/COLUMNS		OTHERS		BEAMS/COLUMNS		OTHERS		BEAMS/COLUMNS		OTHERS	
BAR SIZE	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS
#3	28	21	42	32	24	19	36	28	22	17	32	25
#4	37	29	56	43	32	25	48	37	29	22	43	33
#5	46	36	69	53	40	31	60	46	36	28	54	41
#6	56	43	83	64	48	37	72	56	43	33	65	50
#7	81	62	122	94	70	54	105	81	63	48	91	72
#8	93	71	139	107	80	52	120	93	72	55	108	83
#9	104	80	156	120	90	70	135	104	81	62	121	93
#10	116	89	174	134	100	77	150	116	90	69	134	103
#11	127	98	191	147	110	85	165	127	99	76	148	114

NOTES:
1. TABULATED VALUES ARE BASED ON MINIMUM YIELD STRENGTH OF 60 KSI. LENGTHS ARE IN INCHES.
2. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF FRESH CONCRETE PLACED BELOW THE DEVELOPMENT OR SPLICE.
3. REBAR IS ASSUMED TO BE UNCOATED (NO EPOXY COATING). INCREASE DEVELOPMENT LENGTHS SHOWN BY 1.3 FOR TOP, AND 1.5 FOR OTHER EPOXY COATED BARS.
4. FOR LIGHTWEIGHT CONCRETE, MULTIPLY TABULATES VALUES BY 1.3.
5. LAP SPLICE LENGTHS SHALL BE AS SHOWN IN THE TABLE ABOVE, UNLESS NOTED OTHERWISE.
6. WHERE BARS OF DIFFERENT SIZES ARE LAP SPLICES, THE LAP SPLICE LENGTH SHALL BE THE LARGER OF THE TENSION DEVELOPMENT LENGTH OF THE LARGER BAR AND THE TENSION LAP SPLICE OF THE SMALLER BAR.

DEVELOPMENT LENGTH (INCHES)

	f'c = 3000 PSI					f'c = 4000 PSI					f'c = 5000 PSI					f'c = 6000 PSI				
	Ld					Ld					Ld					Ld				
	BEAM/COLUMN		OTHERS		Ldh	BEAM/COLUMN		OTHERS		Ldh	BEAM/COLUMN		OTHERS		Ldh	BEAM/COLUMN		OTHERS		Ldh
BAR SIZE	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS		TOP BARS	OTHER BARS	TOP BARS	OTHER BARS		TOP BARS	OTHER BARS	TOP BARS	OTHER BARS		TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	
#3	21	16	32	25	9	19	14	28	21	8	17	13	25	19	7	15	12	23	17	6
#4	29	22	43	33	11	25	19	37	29	10	22	17	33	26	9	20	16	30	23	8
#5	36	27	53	41	14	31	24	46	36	12	28	21	41	32	11	25	19	38	29	10
#6	43	33	64	49	17	37	29	56	43	15	33	26	50	38	13	30	23	45	35	12
#7	62	48	94	72	20	54	42	81	62	17	48	37	72	56	15	44	34	66	51	14
#8	71	55	107	82	22	62	48	93	71	19	55	42	83	64	17	50	39	46	58	16
#9	80	62	120	92	25	69	53	104	80	22	62	47	93	72	20	57	44	85	65	18
#10	89	69	134	103	28	77	59	116	89	25	69	53	103	80	22	53	48	94	73	21
#11	98	75	147	113	31	85	65	127	98	27	76	58	114	88	25	69	53	104	80	24

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 286 9004
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS

ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date

CONTRACT DOCUMENTS FOR:

SLPS C.A. MOORE GYMNASIUM

ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION

CONCRETE GENERAL NOTES & SCHEDULES

PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W./RJT/EB

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

S003A

WILLIAM JOHN RAIOLA LICENSE No. 80234 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/29/2026

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JEZERINAC GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

5/29/2026 3:58:01 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR-JG-CA Moore_R26.rvt

SLAB-ON-GROUND

A. THE SLAB-ON-GROUND HAS BEEN DESIGNED IN ACCORDANCE WITH THE GEOTECHNICAL INVESTIGATION REPORT.
B. SLAB THICKNESSES AND REINFORCEMENT ARE BASED ON A MODULUS OF SUBGRADE REACTION OF 150 PCI PER THE GEOTECHNICAL INVESTIGATION REPORT.
C. SUBGRADE PREPARATION SHALL BE PERFORMED IN ACCORDANCE WITH THE GEOTECHNICAL INVESTIGATION REPORT.
D. FOR INTERIOR SLABS, PLACE A 10-MIL (MINIMUM) VAPOR RETARDER BETWEEN THE SOIL AND BOTTOM OF SLAB. SEE CAST-IN-PLACE CONCRETE SPECIFICATIONS FOR APPROVED VAPOR RETARDER PRODUCTS/MANUFACTURERS. DO NOT USE VAPOR RETARDERS AT EXTERIOR SLABS. SEE ARCHITECTURAL CONTRACT DOCUMENTS FOR PROJECT SPECIFIC REQUIREMENTS.
E. IF THE SLAB-ON-GROUND HAS BEEN DESIGNATED AS A STRUCTURAL SLAB-ON-GROUND IN THE CONTRACT DOCUMENTS, NO SAW CUTTING OF THE SLAB IS PERMITTED.
F. CONTRACTION JOINTS SHALL BE CUT INTO THE SURFACE OF THE SLAB, IN EACH DIRECTION. SEE THE TYPICAL SAW CUT JOINT DETAIL FOR TIME, DEPTH, AND SPACING OF JOINT REQUIREMENTS UNLESS NOTED OTHERWISE. CONTRACTION JOINTS SHALL BE CONSTRUCTED SUCH THAT THE AREA CONTAINED BY THE CONTRACTION JOINTS HAS A MAXIMUM RATIO OF LONG SIDE TO SHORT SIDE OF 1.5 TO 1 UNLESS NOTED OTHERWISE. DO NOT CONSTRUCT CONTRACTION JOINTS SUCH THAT L-SHAPED SLAB PANELS ARE CREATED.
G. COLUMN ISOLATION JOINTS SHALL BE CONSTRUCTED PER THE TYPICAL COLUMN ISOLATION JOINT DETAIL IN ORDER TO PROVIDE ADEQUATE SPACE FOR COLUMN INSTALLATION.
H. CONSTRUCTION JOINT LOCATIONS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL. SLAB CONSTRUCTION JOINTS SHALL BE DOWELED.
I. WHERE SPECIFIED ON PLAN, WELDED WIRE REINFORCEMENT SHALL BE INSTALLED. WELDED WIRE REINFORCEMENT SHALL BE PROPERLY CHAIRED SUCH THAT IT IS LOCATED AT A DEPTH OF 1 1/2" FROM THE TOP OF SLAB.
J. REFERENCE ARCHITECTURAL AND MEP DOCUMENTS FOR SLAB FINISHES AND SLOPES NOT REFERENCED ON THE STRUCTURAL DOCUMENTS. THE MINIMUM SLAB THICKNESS SPECIFIED IN THE CONTRACT DOCUMENTS MUST BE MAINTAINED.
K. REFERENCE ARCHITECTURAL DOCUMENTS FOR VAPOR RETARDER AND SLAB AND CONTRACTION JOINT SEALANT REQUIREMENTS.
L. CONDUITS SHALL NOT BE PLACED WITHIN THE SLAB. CONDUITS SHALL BE PLACED BENEATH THE SLAB.

POST-INSTALLED ANCHORS

A. SEE POST-INSTALLED ANCHORS SPECIFIED PRODUCTS BY APPLICATION AND INSTALLATION REQUIREMENTS ON THIS SHEET.
B. THE GENERAL CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.
C. SUBSTITUTION REQUESTS FOR ALTERNATIVE PRODUCTS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD WITH CALCULATIONS THAT ARE PREPARED AND SEALED BY A LICENSED PROFESSIONAL ENGINEER. CALCULATIONS SHALL SHOW THAT THE SUBSTITUTED PRODUCT WILL ACHIEVE AN EQUIVALENT CAPACITY USING THE APPROPRIATE DESIGN PROCEDURE REQUIRED BY THE REFERENCED BUILDING CODE.
D. ALTERNATIVE PRODUCTS SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL SHALL HAVE A VALID RESEARCH REPORT, ALSO KNOWN AS EVALUATION REPORT, INDICATING COMPLIANCE WITH APPROPRIATE ACCEPTANCE CRITERIA REQUIRED BY THE REFERENCED BUILDING CODE FOR THE INTENDED LOAD TYPE AND USE (E.G. WIND, SEISMIC, SUSTAINED TENSION, ETC). ALTERNATIVE PRODUCTS SUBMITTED SHALL INDICATE THAT THE ANCHOR IS PERMITTED FOR RESISTING LOADS IN CRACKED CONCRETE. RESEARCH REPORTS SHALL BE ISSUED BY A SOURCE APPROVED BY THE AUTHORITY HAVING JURISDICTION.
E. LOCATE, BY NON-DESTRUCTIVE MEANS, ALL EXISTING REINFORCEMENT, AND AVOID DURING INSTALLATION OF ANCHORS. IF EXISTING REINFORCEMENT LAYOUT PROHIBITS THE INSTALLATION OF ANCHORS AS INDICATED ON THE STRUCTURAL DRAWINGS, THE GENERAL CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF RECORD IMMEDIATELY.
F. HOLES SHALL BE DRILLED AND CLEANED, AND ANCHORS SHALL BE INSTALLED PER THE MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS. DEFECTIVE OR ABANDONED HOLES SHALL BE FILLED WITH NON-SHRINK GROUT OR AN INJECTABLE ADHESIVE MATCHING THE ADJACENT CONCRETE COMPRESSIVE STRENGTH.
G. HOT DIPPED GALVANIZED STEEL ANCHORS SHALL BE USED AT ALL EXTERIOR LOCATIONS AND WHERE SPECIFICALLY INDICATED ON THE DRAWINGS.
H. MASONRY ANCHORS SHALL NOT BE INSTALLED IN HOLLOW CORE MASONRY. IF INSTALLATION INTO HOLLOW CORE MASONRY IS DESIRED, SUBMIT ALTERNATIVE PRODUCT FOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER OF RECORD.
I. MASONRY ANCHORS SHALL NOT BE INSTALLED IN HEAD JOINTS.
J. IN ADDITION TO THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS, THE FOLLOWING GUIDELINES SHALL BE FOLLOWED FOR INSTALLATION OF ADHESIVE ANCHORS:
   1. ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE HAVING A MINIMUM AGE OF 21 DAYS AT TIME OF ANCHOR INSTALLATION. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI AT THE TIME OF INSTALLATION UNLESS HIGHER STRENGTH IS REQUIRED PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.
   2. ADHESIVE ANCHORS SHALL BE INSTALLED IN DRY CONCRETE, AND DURING DRY CONDITIONS.
   3. ADHESIVE ANCHORS SHALL BE INSTALLED IN HOLES PREDRILLED WITH A CARBIDE TIPPED DRILL BIT.
   4. ADHESIVE ANCHORS SHALL BE INSTALLED WITHIN THE TEMPERATURE RANGE SPECIFIED IN THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS, BUT NOT OUTSIDE OF THE DESIGN TEMPERATURE RANGE. LOADS SHALL NOT BE APPLIED TO ADHESIVE ANCHORS UNTIL THE FULL CURING TIME ASSOCIATED WITH THE INSTALLATION TEMPERATURE HAS ELAPSED.
K. INSTALLATION OF ADHESIVE ANCHORS SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY AN APPLICABLE CERTIFICATION PROGRAM. CERTIFICATION SHALL INCLUDE WRITTEN AND PERFORMANCE TESTS IN ACCORDANCE WITH THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT.
L. SPECIAL INSPECTIONS SHALL BE PROVIDED FOR POST-INSTALLED ANCHORS IN ACCORDANCE WITH THE ANCHOR MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS AND/OR EVALUATION REPORTS, UNLESS MORE SPECIFIC REQUIREMENTS ARE SPECIFIED IN THE CONSTRUCTION DOCUMENTS.
M. INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY INCLINED TO RESIST SUSTAINED TENSILE LOADS SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM. THESE ANCHORS ARE DESIGNATED ON THE DRAWINGS WITH (CERT).
N. CONTINUOUS INSPECTION SHALL BE PROVIDED FOR ADHESIVE ANCHORS INSTALLED IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSILE LOADS.
O. ADHESIVE ANCHORS SHALL BE PROOF TESTED AS REQUIRED IN THE SPECIFICATIONS. EACH TYPE AND SIZE OF ANCHOR SHALL BE PROOF TESTED IN TENSION BY AN INDEPENDENT TESTING LABORATORY. TENSION TESTING SHALL BE PERFORMED IN ACCORDANCE WITH ASTM E488 AND ACI 355.4. THE INDEPENDENT TESTING LABORATORY SHALL SUBMIT AN ANCHORAGE TESTING PLAN TO THE STRUCTURAL ENGINEER OF RECORD TO ENSURE THE TESTING REQUIREMENTS ARE FULFILLED.
P. HOLE DRILLING AND INSTALLATION OF ADHESIVE ANCHORS SHALL BE IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS. ANCHORS SHALL BE INSTALLED IN CONCRETE DRY CONDITION.

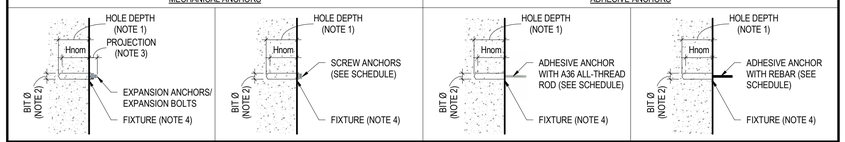
POST-INSTALLED ANCHORS SPECIFIED PRODUCTS BY APPLICATION

	ANCHOR TYPE		CONCRETE	CONCRETE MASONRY
MECHANICAL	EXPANSION ANCHORS/ EXPANSION BOLTS		HILTI KWIK BOLT TZ SIMPSON STRONG-BOLT 2 DEWALT/POWERS POWER-STUD+ SD2	HILTI KWIK BOLT 3 SIMPSON STRONG-BOLT 2 DEWALT/POWERS POWER-STUD+ SD1
MECHANICAL	SCREW ANCHORS		HILTI HUS-EZ SIMPSON TITEN HD DEWALT/POWERS SCREW-BOLT+	HILTI HUS-EZ SIMPSON TITEN HD DEWALT/POWERS SCREW-BOLT+
ADHESIVE	ADHESIVE ANCHORS (EPOXY ANCHORS) WITH A36 ALL-THREAD ROD		HILTI HIT-HY200 SIMPSON SET-3G DEWALT/POWERS PURE 110+	HILTI HIT-HY270 SIMPSON SET-3G DEWALT/POWERS AC100+ GOLD
ADHESIVE	ADHESIVE ANCHORS (EPOXY ANCHORS) WITH REBAR		HILTI HIT-HY200 SIMPSON SET-3G DEWALT/POWERS PURE 110+	

NOTES:

1. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE GENERAL CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE GIVEN TO AVOID CONFLICTS WITH EXISTING REINFORCEMENT. HOLES SHALL BE DRILLED AND CLEANED PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS. ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.
2. SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE SPECIFIED, SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD WITH CALCULATIONS THAT ARE PREPARED AND SEALED BY A REGISTERED DESIGN PROFESSIONAL LICENSED IN THE STATE IN WHICH THE PROJECT IS LOCATED. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT WILL ACHIEVE AN EQUIVALENT CAPACITY USING THE APPROPRIATE DESIGN PROCEDURE REQUIRED BY THE REFERENCED BUILDING CODE.
3. ALTERNATE PRODUCTS SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL SHALL HAVE A VALID RESEARCH REPORT, ALSO KNOWN AS AN EVALUATION REPORT, INDICATING COMPLIANCE WITH APPROPRIATE ACCEPTANCE CRITERIA REQUIRED BY THE REFERENCED BUILDING CODE FOR THE INTENDED LOAD TYPE AND USE (E.G., WIND, SEISMIC, SUSTAINED TENSION, ETC.). RESEARCH REPORTS SHALL BE ISSUED BY A SOURCE APPROVED BY THE AUTHORITY HAVING JURISDICTION.
4. SPECIAL INSPECTIONS SHALL BE PROVIDED FOR POST-INSTALLED ANCHORS IN ACCORDANCE WITH THE ANCHOR MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS AND/OR EVALUATION REPORT, UNLESS MORE SPECIFIC REQUIREMENTS ARE SPECIFIED IN THE CONSTRUCTION DOCUMENTS.

POST-INSTALLED ANCHORS INSTALLATION REQUIREMENTS



NOTES:

1. POST-INSTALLED ANCHORS ARE SPECIFIED BY THE NOMINAL EMBEDMENT (Hnom) INDICATED IN MANUFACTURER'S LITERATURE. INCREASE THE HOLE DEPTH AS REQUIRED BY THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS FOR THE SPECIFIED ANCHOR SIZE AND EMBEDMENT.
2. COORDINATE BIT DIAMETER WITH MANUFACTURER REQUIREMENTS.
3. TOTAL ANCHOR LENGTH SHALL BE COORDINATED TO PROVIDE ADEQUATE PROJECTION LENGTH FOR FIXTURE THICKNESS, WASHER(S) AS REQUIRED (SEE NOTE 5), AND FULL ENGAGEMENT OF NUT.
4. FIXTURE CONSIDERATIONS SHALL BE COORDINATED WITH MANUFACTURER REQUIREMENTS:
   A. CONSIDERATION FOR "THROUGH-SET" VS "PRESET" INSTALLATION IN DETERMINING FIXTURE HOLE DIAMETER AND WASHER REQUIREMENTS (SEE NOTE 5).
   B. INCREASE NOMINAL EFFECTIVE EMBEDMENT IF REQUIRED WHEN FIXTURE THICKNESS IS LESS THAN MINIMUM FOR EXPANSION ANCHORS.
5. PROVIDE DOUBLE WASHERS WHEN "THROUGH-SET" INSTALLATION IS USED FOR ADHESIVE ANCHORS AS REQUIRED BY MANUFACTURER REQUIREMENTS.
6. COORDINATE OTHER REQUIREMENTS WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS INCLUDING (BUT NOT LIMITED TO) TEMPERATURE, HOLE DRILLING/CLEANING/PREPARATION, AND INSTALLATION TORQUE.
7. ADDITIONAL NOTES FOR ADHESIVE ANCHORS:
   A. ADHESIVE ANCHOR DESIGN TEMPERATURE RANGE IS 110°F (LONG TERM) AND 130°F (SHORT TERM).
   B. IN ADDITION TO THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS, THE FOLLOWING GUIDELINES SHALL BE FOLLOWED FOR INSTALLATION OF ADHESIVE ANCHORS:
      a. ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE HAVING A MINIMUM AGE OF 21 DAYS AT TIME OF ANCHOR INSTALLATION.
      b. ADHESIVE ANCHORS SHALL BE INSTALLED IN DRY CONCRETE, AND DURING DRY CONDITIONS.
      c. ADHESIVE ANCHORS SHALL BE INSTALLED IN HOLES PREDRILLED WITH A CARBIDE TIPPED DRILL BIT.
      d. ADHESIVE ANCHORS SHALL BE INSTALLED WITHIN THE TEMPERATURE RANGE SPECIFIED IN THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS, BUT NOT OUTSIDE OF THE DESIGN TEMPERATURE RANGE.
      e. LOAD SHALL NOT BE APPLIED TO ADHESIVE ANCHORS UNTIL THE FULL CURING TIME ASSOCIATED WITH THE INSTALLATION TEMPERATURE HAS ELAPSED.
   C. INSTALLATION OF ADHESIVE ANCHORS SHALL BE PERFORMED BY CERTIFIED PERSONNEL. CERTIFICATION SHALL INCLUDE WRITTEN AND PERFORMANCE TESTS IN ACCORDANCE WITH THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT.
   D. WHEN ANCHORING TO CONCRETE MASONRY WITH VOIDS, THE APPROPRIATE SCREEN TUBE SHALL BE USED AS RECOMMENDED BY THE ADHESIVE MANUFACTURER.

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 286 9004
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS

ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date

WILLIAM JOHN RAIOLA LICENSE No. 80234 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/29/2026

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JEZERINAC GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

CONTRACT DOCUMENTS FOR:
SLPS C.A. MOORE GYMNASIUM
ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION

CONCRETE GENERAL NOTES & SCHEDULES

PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W.JR/TEB

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

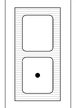
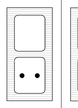
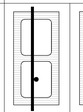
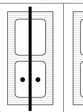
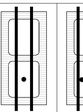
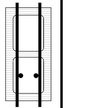
S003B

5/29/2026 3:58:02 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR-JG-CA Moore_R26.rvt

CONCRETE MASONRY

A. MANUFACTURE AND INSTALL ALL CONCRETE MASONRY IN ACCORDANCE WITH DIVISION 04 SPECIFICATIONS. ALL MASONRY DESIGN SHALL CONFORM TO TMS 402 AND ALL MASONRY CONSTRUCTION SHALL CONFORM TO TMS 602.
B. ALL LOAD-BEARING, NON-LOAD-BEARING, AND BACKUP WALL CONCRETE MASONRY UNIT CONSTRUCTION SHALL CONFORM TO THE FOLLOWING MATERIAL STANDARDS:
   1. CONCRETE MASONRY UNITS: ASTM C90, NORMAL WEIGHT (135 PCF)
   2. MORTAR: ASTM C270; TYPE 'S' OR 'M' PORTLAND CEMENT/LIME ONLY (USE TYPE 'M' MORTAR WHEN MASONRY IS IN DIRECT CONTACT WITH SOIL; TYPE 'S' IS IN ALL OTHER CONDITIONS)
   3. GROUT: ASTM C476
   4. PORTLAND CEMENT: ASTM C150, TYPE I (TYPE III MAY BE USED FOR COLD-WEATHER CONSTRUCTION)
   5. HYDRATED LIME: ASTM C207, TYPE 'S'
   6. AGGREGATE: ASTM C404 (FOR GROUT)
   7. STEEL REINFORCEMENT: ASTM A615, GRADE 60
   8. JOINT REINFORCEMENT: ASTM A1064, TRUSS OR LADDER TYPE, GALVANIZE PER ASTM A153, TYPE B-2
C. CONCRETE MASONRY UNITS:
   1. F'_m SHALL BE 2000 PSI (MINIMUM NET AREA CMU COMPRESSIVE STRENGTH SHALL BE 2000 PSI).
   2. LAY CONCRETE MASONRY UNITS IN RUNNING BOND UNLESS NOTED OTHERWISE WITH UNITS DESIGNED TO ALIGN WITH WEBS IN EACH COARSE.
D. MORTAR:
   1. HEAD AND BED JOINTS SHALL BE 3/8 INCHES FOR THE THICKNESS OF THE FACE SHELL. WEBS ARE TO BE FULLY MORTARED IN ALL COURSES OF: PIERS, COLUMNS AND PILASTERS, IN THE STARTING COURSE, AND WHERE AN ADJACENT CELL IS TO BE GROUTED. REMOVE MORTAR PROTRUSIONS EXTENDING ½ INCHES OR MORE INTO CELLS TO BE GROUTED.
   2. PROVIDE FULL FACE SHELL MORTAR COVERAGE ON MASONRY UNIT HORIZONTAL AND VERTICAL (BED AND HEAD) FACE SHELL JOINTS.
E. GROUT:
   1. MASONRY GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28-DAYS.
   2. GROUT MIX SHALL CONTAIN PORTLAND CEMENT, AGGREGATE, AND A GROUT-ENHANCING SHRINKAGE-COMPENSATING ADMIXTURE.
   3. MAXIMUM SIZE OF AGGREGATE SHALL BE 3/8 INCH. SLUMP SHALL BE 8 TO 11 INCHES. WATER REDUCING ADMIXTURES MAY BE USED.
   4. GROUT ALL MASONRY CONTAINING REINFORCEMENT, ALL CELLS OF 4-HOUR RATED WALLS, AND WHERE INDICATED ON THE DRAWINGS. ALLOW MORTAR TO CURE 24 HOURS PRIOR TO GROUTING. PROVIDE CLEANOUT OPENINGS AT THE BASE OF THE CELLS CONTAINING REINFORCEMENT TO CLEAN THE CELL AND TO TIE THE VERTICAL BAR TO THE DOWEL. IN HIGH-LIFT GROUTING, USE 5'-0" (MAXIMUM) LIFTS, WITH ½ HOUR TO 1 HOUR BETWEEN LIFTS.
   5. GROUT SHALL BE VIBRATED WHILE PLACING TO ENSURE THAT CELLS ARE COMPLETELY FILLED.
F. STEEL REINFORCEMENT:
   1. PROVIDE VERTICAL REINFORCEMENT IN CELLS OF CONCRETE MASONRY UNITS (FULLY EMBEDDED IN GROUT) AS SHOWN ON THE PLANS AND OTHER DETAILS. MINIMUM REINFORCEMENT OF EXTERIOR MASONRY SHALL BE AS FOLLOWS:
      a. 1-#5 AT A MAXIMUM SPACING OF 48 INCHES
      b. 1-#5 AT EACH CORNER
      c. HEAVIER REINFORCEMENT MAY BE REQUIRED BY PLAN NOTES OR DETAILS IN THE DRAWINGS.
   2. REINFORCE WALLS WHERE INDICATED ON THE DRAWINGS AND AT ALL INTERSECTIONS, EACH SIDE OF OPENINGS AND AT THE ENDS OF WALLS. USE BAR SPACERS AT 10 FEET ON CENTER WHERE GROUT POUR HEIGHT EXCEEDS 10 FEET.
   3. ALL VERTICAL REINFORCEMENT SHALL HAVE STANDARD HOOK INTO BOND BEAM. TERMINATE AT HIGHEST BOND BEAM IF MASONRY DOES NOT EXTEND TO ROOF OR GROUTED CELL IS NOT CONTINUOUS TO ROOF. HOOK SHALL EXTEND TO THE UPPERMOST HORIZONTAL REINFORCEMENT OF THE BOND BEAM AND HAVE A MINIMUM EMBEDMENT OF 6 INCHES.
   4. ALL HORIZONTAL REINFORCEMENT AT ENDS OF BOND BEAMS SHALL HAVE STANDARD HOOK INTO VERTICAL GROUTED CELL. PROVIDE CORNER BARS SUCH THAT HORIZONTAL REINFORCEMENT IS CONTINUOUS AROUND CORNERS.
   5. COVER TO STEEL REINFORCEMENT WITHIN MASONRY ELEMENTS SHALL NOT BE LESS THAN THE FOLLOWING:
      a. EXPOSED TO EARTH OR WEATHER: 1 ½ INCHES (#5 AND SMALLER BARS), 2 INCHES (#6 AND LARGER BARS)
      b. NOT EXPOSED TO EARTH OR WEATHER: 1 ½ INCHES
G. JOINT REINFORCEMENT:
   1. JOINT REINFORCEMENT SHALL BE LADDER TYPE, 9 GAUGE, SPACED VERTICALLY AT EVERY 2 COURSES UNLESS NOTED OTHERWISE.
   2. PROVIDE JOINT REINFORCEMENT SPACED VERTICALLY AT EVERY COURSE FOR MASONRY BELOW GRADE AND IN PARAPETS AND CANTILEVERED WALLS.
   3. PROVIDE TWO ROWS OF JOINT REINFORCEMENT AT EVERY COURSE AT TOP AND BOTTOM OF OPENINGS (EXTEND 24 INCHES EACH SIDE).
   4. PROVIDE TWO ROWS OF JOINT REINFORCEMENT AT EVERY COURSE AT BOND BEAMS.
   5. OVERLAP DISCONTINUOUS JOINT REINFORCEMENT BY AT LEAST 6 INCHES.
   6. USE PREFABRICATED CORNERS AND TEES.
   7. EXTEND JOINT REINFORCEMENT A MINIMUM OF 4 INCHES INTO THE TIE BEAM.
   8. REFER TO PLANS AND DETAILS FOR BONDED JOINT REQUIREMENTS AT WALL CORNERS AND INTERSECTIONS. WHERE INDICATED ON DRAWINGS, INTERLOCK WALLS WITH METAL TIES, ANCHORS, OR PREFABRICATED JOINT REINFORCEMENT UNLESS NOTED OTHERWISE ON DRAWINGS OR SEE SPECIFICATIONS.
   9. LONGITUDINAL WIRES OF JOINT REINFORCEMENT SHALL BE FULLY EMBEDDED IN MORTAR OR GROUT WITH A MINIMUM HORIZONTAL EDGE COVER OF 5/8 INCHES WHEN EXPOSED TO EARTH AND WEATHER AND ½ INCHES WHEN NOT EXPOSED TO EARTH OR WEATHER.
H. REINFORCED MASONRY WALL CONSTRUCTION SHALL BE INSPECTED BY AN ENGINEER OR ARCHITECT IN ACCORDANCE WITH TMS 602.
I. WHERE ANCHOR BOLTS, EXPANSION ANCHORS, OR ANCHORS SET IN EPOXY ARE PLACED IN A MASONRY WALL, FILL CELLS WITH GROUT FOR BOLTED COURSE, ONE COURSE ABOVE AND TWO COURSES BELOW.
J. USE PRESSURE-TREATED WOOD FOR WOOD IN CONTACT WITH MASONRY.
K. CALCIUM CHLORIDE SHALL NOT BE USED IN MORTAR OR GROUT.
L. REFER TO ARCHITECT'S DRAWINGS FOR THE EXTENT OF MASONRY WALLS AND DIMENSIONED LOCATION OF OPENINGS. NON-LOAD BERING WALLS MAY NOT BE SHOWN ON THE STRUCTURAL DRAWINGS.
M. CONCRETE MASONRY UNITS SHALL BE CUT BELOW BEAMS, LINTELS, OR BOND BEAMS AS REQUIRED IN ORDER TO SET CONTINUOUS BEAM, LINTEL, OR BOND BEAMS AT THE PROPER ELEVATION.
N. ALL CELLS BELOW GRADE AND SLAB-ON-GROUND SHALL BE FULLY GROUTED.
O. THE FOLLOWING CRITERIA REGARDING PIPES AND CONDUITS EMBEDDED IN MASONRY SHALL BE ADHERED TO (SEE MEP DRAWINGS FOR LOCATIONS OF SLEEVES, PIPES, CONDUIT, ACCESSORIES, ETC). THESE CRITERIA WILL BE STRICTLY ENFORCED:
   1. CONDUITS, PIPES, AND SLEEVES OF ANY MATERIAL NOT HARMFUL TO MASONRY AND MEETING THE CRITERIA BELOW SHALL BE PERMITTED TO BE EMBEDDED IN MASONRY. ALL OTHER CONDUITS, PIPES, AND SLEEVES SHALL NOT BE EMBEDDED WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD.
   2. CONDUITS AND PIPES OF ALUMINUM SHALL NOT BE EMBEDDED IN STRUCTURAL MASONRY.
   3. CONDUITS, PIPES, AND SLEEVES PASSING THROUGH A WALL SHALL NOT SIGNIFICANTLY IMPAIR THE STRENGTH OF THE CONSTRUCTION. CONDUITS, PIPES, AND SLEEVES SHALL NOT PASS THROUGH JAMBS, LINTELS, BOND BEAMS, OR SHEAR WALLS WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD.
   4. CONDUITS AND PIPES SHALL NOT BE SPACED CLOSER THAN 3 DIAMETERS OR WIDTHS ON CENTER.
   5. CONDUITS AND PIPES SHALL BE FABRICATED AND INSTALLED SO THAT CUTTING, BENDING, OR DISPLACEMENT OF REINFORCEMENT FROM ITS PROPER LOCATION WILL NOT BE REQUIRED.
   6. CONDUITS AND PIPES, WITH FITTINGS, EMBEDDED WITHIN A COLUMN OR WALL SHALL NOT DISPLACE MORE THAN 2 PERCENT OF THE NET SECTION OR AS REQUIRED BY FIRE PROTECTION.
P. ALL MASONRY WALLS SHOWN ON THE STRUCTURAL DRAWINGS HAVE BEEN DESIGNED TO RESIST THE REQUIRED CODE VERTICAL AND LATERAL FORCES IN THE FINAL CONSTRUCTED CONFIGURATION ONLY ASSUMING FULL BRACING TOP, BOTTOM, AND/OR SIDE OF WALL AS SHOWN. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROPERLY AND ADEQUATELY BRACE ALL MASONRY WALLS AT ALL STAGES DURING CONSTRUCTION TO RESIST ERECTION LOADS AND LATERAL LOADS THAT COULD OCCUR PRIOR TO THE COMPLETION OF CONSTRUCTION.
Q. CONTRACTION JOINTS SHALL BE PROVIDED IN ALL CONCRETE MASONRY CONSTRUCTION. REFER TO TYPICAL CONTRACTION JOINT DETAIL FOR GUIDELINES AND SPACING.

CMU - REINFORCEMENT SPLICES[1]

BAR WITH SPLICE	VERTICAL BARS[2]		HORIZONTAL BARS[3]			
	 1 VERTICAL	 2 VERTICALS	 1 VERTICAL 1 HORIZONTAL	 2 VERTICALS 1 HORIZONTAL	 1 VERTICAL 2 HORIZONTALS	 2 VERTICALS 2 HORIZONTALS
#4	21"	26"	21"	21"	26"[4]	45"[4]
#5	26"	40"	29"	26"	40"[4]	70"[4]
#6	43"	74"	57"	43"	74"[4]	131"[4]
#7	60"	107"	81"	60"	107"[4]	NOTE 5

NOTES:

1. SEE TYPICAL REBAR LAYOUT DETAIL FOR BAR PLACEMENT. WHERE BARS OF DIFFERENT SIZES ARE TO BE SPLICED, THE SPLICE LENGTH SHALL BE THAT REQUIRED FOR THE LARGER BAR.
2. SPLICES OF VERTICAL REINFORCEMENT SHALL BE PLACED NEXT TO THE MAIN BAR AS INDICATED IN THE ILLUSTRATION.
3. SPLICES OF HORIZONTAL REINFORCEMENT SHALL BE PLACED VERTICALLY OVER THE MAIN BAR.
4. SPLICES OF HORIZONTAL REINFORCEMENT IN WALLS CONTAINING TOW BARS PER COURSE SHALL BE STAGGERED.
5. 8" CMU WALL IS NOT WIDE ENOUGH TO PERMIT THIS CONDITION.

WILLIAM JOHN RAIOLA LICENSE No. 80234 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/29/2026

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JEZERINAC GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 286 9004
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FO, FLRIDA
GEORGIA, NORTH CAROLINA
MSSBSSIPPI, TEXAS

ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date

CONTRACT DOCUMENTS FOR:
SLPS C.A. MOORE GYMNASIUM
ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION

CMU GENERAL NOTES & SCHEDULES

PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W./PETER

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

S004

5/29/2026 3:58:02 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR-JG-CA Moore_R26.rvt

STRUCTURAL STEL

A. STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS UNLESS NOTED OTHERWISE ON THE CONTRACT DOCUMENTS:
   1. ROLLED SHAPES AND CHANNELS: ASTM A572 OR A992, MIN. YIELD STRENGTH 50 KSI
   2. ANGLES FOR TRUSSES AND BRACES: ASTM A36 MIN YIELD STRENGTH 36 KSI
   3. MISCELLANEOUS ANGLES: ASTM A36
   4. HOLLOW STRUCTURAL SECTIONS: ASTM A500 GRADE C, MIN YIELD STRENGTH 46 KSI FOR ROUND AND 50 KSI FOR RECTANGULAR HSS
B. CONNECTION MATERIAL SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS OR AS NEEDED FOR CONNECTION DESIGN:
   1. ANGLES: ASTM A36
   2. WTs: ASTM A992
   3. PLATES: ASTM A572, MIN YIELD STRENGTH 50 KSI
   4. BOLTS: ASTM A325
   5. NUTS: ASTM A563
   6. WASHERS: ASTM F436
   7. ANCHOR RODS: ASTM F1554 GRADE 55 WITH WELDABILITY SUPPLEMENT S1
   8. WELD ELECTRODES: MATCH FILLER METAL TO BASE METAL PER AWS D1.1
C. WHERE NO CAMBER IS INDICATED, FABRICATE BEAMS SO THAT ANY NATURAL CAMBER IS UPWARD AFTER ERECTION.
D. CANTILEVERED BEAMS WITH NATURAL MILL CAMBER SHALL BE ERECTED SUCH THAT THE CAMBER IS ORIENTED DOWNWARD (OR CONCAVE UP).
E. SPLICES SHALL BE ALLOWED ONLY AT LOCATIONS SPECIFICALLY INDICATED ON THE STRUCTURAL DRAWINGS UNLESS APPROVED OTHERWISE BY THE SER IN WRITING.
F. FOR STEEL MEMBERS AND EMBEDMENTS EXPOSED TO WEATHER, PROVIDE HOT-DIPPED GALVANIZED FINISH.
G. PROVIDE HOLES IN ALL STEEL AS REQUIRED TO PREVENT ANY ACCUMULATION OF WATER. ALL PENETRATIONS THROUGH MAIN MEMBERS SHALL NOT EXCEED 1 1/8" DIA. AND SHALL BE GROUND SMOOTH. THESE DRAINS MUST BE KEPT CLEAN AND OPEN.
H. SHOW ALL COPES, HOLES, OPENINGS AND MODIFICATIONS REQUIRED IN STRUCTURAL STEEL MEMBERS FOR ERECTION OR THE WORK OF OTHER TRADES ON THE SHOP DRAWINGS FOR APPROVAL BY THE ARCHITECT AND STRUCTURAL ENGINEER.
I. FIELD MODIFICATIONS OF STRUCTURAL STEEL IS PROHIBITED WITHOUT PRIOR APPROVAL OF THE ARCHITECT AND STRUCTURAL ENGINEER.
J. WHERE BEAM SHEAR IS NOT NOTED, DESIGN FOR 10K.
K. ALL CONNECTIONS SHALL BE DESIGNED FOR THE SPECIFIED SHEAR, MOMENT, AND AXIAL LOADS ON THE DRAWINGS. THE CONNECTIONS SHALL BE DESIGNED FOR LOAD REVERSAL. ALL CONNECTIONS FORCES SPECIFIED ON PLAN ARE ULTIMATE LEVEL FORCES UNLESS NOTED OTHERWISE.
L. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN, DETAILING, AND FABRICATION OF ALL STEEL FRAMING CONNECTIONS UNLESS SPECIFICALLY NOTED AS 'COMPLETELY DESIGNED' ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL RETAIN A STRUCTURAL ENGINEER LICENSED TO PERFORM THE WORK IN THE JURISDICTION WHERE THE PROJECT IS LOCATED, WHO SHALL DESIGN THE CONNECTIONS. SUBMIT SIGN AND SEALED CALCULATIONS TO THE ARCHITECT FOR REVIEW AND APPROVAL PRIOR TO STARTING FABRICATION.

   CONNECTION DESIGN SHALL MEET THE REQUIREMENTS OF THE AISC SPECIFICATIONS AND THE BUILDING CODE. CONNECTIONS SHALL BE CAPABLE OF RESISTING VERTICAL AND HORIZONTAL LOADS LISTED ON THE DRAWINGS. CONNECTION DESIGN SHALL PROVIDE AN ADEQUATE LOAD PATH TO TRANSFER THE LOADS FROM EACH MEMBER, THROUGH THE CONNECTION, INTO THE SUPPORTING MEMBER, AND SHALL CONSIDER THE EFFECTS OF THE FORCES ON EACH MEMBER. PROVIDE STIFFENER PLATES, WEB DOUBLER PLATES, FLANGE CONTINUITY PLATES, ETC, AS REQUIRED. MEMBERS SHOWN ON THE DRAWINGS HAVE NOT BEEN SIZED FOR LOCAL EFFECTS AT CONNECTIONS.

   STEEL CONNECTION DETAILS SHOW GENERAL CRITERIA FOR DESIGN AND DETAILING, AND ARE NOT INTENDED TO SHOW COMPLETE CONNECTION CONFIGURATIONS OR OTHER SPECIFIC INFORMATION THAT ARE THE RESPONSIBILITY OF THE CONNECTION DESIGN ENGINEER. ALTERNATIVE CONNECTION CONFIGURATION MAY BE SUBMITTED TO THE ARCHITECT FOR REVIEW AND APPROVAL. CONNECTIONS SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS ARE TO FABRICATED AS SHOWN.

   THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ERECTION AIDS THAT INCLUDE, BUT ARE NOT LIMITED TO ERECTION ANGLES, LIFT HOLES, AND OTHER AIDS.

   STEEL BEAMS ARE EQUALLY SPACED BETWEEN DIMENSION POINTS AT THE MAXIMUM DECK SPAN LOCATION UNLESS NOTED OTHERWISE. MINIMUM CONNECTIONS SHALL BE A TWO-BOLT CONNECTION USING 3/4 INCH-DIAMETER A325 BOLTS IN SINGLE SHEAR UNLESS NOTED OTHERWISE. ALL HIGH-STRENGTH BOLTS SHALL BE INSTALLED, TIGHTENED, AND INSPECTED IN ACCORDANCE WITH THE RCSC. BOLTS IN CONNECTIONS SHALL BE INSTALLED WITH FULL PRETENSION EXCEPT WHERE "SNUG-TIGHT" INSTALLATION IS SPECIFICALLY PERMITTED ON THE DRAWINGS. WHERE CONNECTIONS ARE NOTED AS SNUG-TIGHT, THE CONTRACTOR MAY INSTALL PER THE CRITERIA FOR SNUG-TIGHT BOLTS. BOLTS IN SLIP-CRITICAL CONNECTIONS SHALL BE INSTALLED USING TURN-OF-NUT PRETENSIONING, TWIST-OFF TYPE TENSION CONTROL BOLT PRETENSIONING, OR DIRECT-TENSION-INDICATOR (DTI) PRETENSIONING. ALL BOLT HOLES SHALL BE STANDARD SIZE UNLESS NOTED OTHERWISE.

WELDING

A. ALL WELDING SHALL BE PERFORMED IN STRICT ADHERENCE TO A WRITTEN WELDING PROCEDURE SPECIFICATION PER AMERICAN WELDING SOCIETY D1.1. ALL WELDING PARAMETERS SHALL BE WITHIN THE ELECTRODE MANUFACTURER'S RECOMMENDATIONS. WELDING PROCEDURES SHALL BE SUBMITTED TO THE OWNER'S TESTING AGENCY FOR REVIEW BEFORE STARTING FABRICATION OR ERECTION. COPIES OF THE WELDING PROCEDURE SPECIFICATION SHALL BE ON SITE AND AVAILABLE TO ALL WELDERS AND THE SPECIAL INSPECTOR.
B. ALL WELDS SHALL BE MADE USING LOW HYDROGEN ELECTRODES WITH MINIMUM TENSILE STRENGTH PER AWS D1.1 (MINIMUM 70 KSI).
C. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE JOINT PREPARATIONS AND WELDING PROCEDURES THAT INCLUDE, BUT ARE NOT LIMITED TO: REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, COPES, SURFACE ROUGHNESS VALUES, TAPERS, AND TRANSITIONS OF UNEQUAL PARTS.
D. WELDING SHALL BE DONE BY WELDERS WITH CURRENT AMERICAN WELDING SOCIETY CERTIFICATION.
E. FIELD WELDING SYMBOLS HAVE NOT NECESSARILY BEEN INDICATED ON THE DRAWINGS. WHERE SHOWN, PROPER FIELD WELDING PER AMERICAN WELDING SOCIETY D1.1 SHALL BE USED. WHERE NO FIELD WELDING SYMBOLS ARE SHOWN, IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE THE USE OF SHOP AND FIELD WELDS.
F. ALL WELD SIZES SHALL BE THE LARGER OF: THE SIZE REQUIRED BY THE CONNECTION FORCES, MINIMUM SIZE PER ANSI/AWS D1.1 OR 3/16 INCH MINIMUM FILLET WELD, UNLESS NOTED OTHERWISE.
G. PROVIDE FILLET WELDS AT CONTACT POINTS BETWEEN STEEL MEMBERS SUFFICIENT TO DELEVOP THE ALLOWABLE TENSILE FORCE OF THE SMALLER MEMBER AT THE JOINT, UNLESS NOTED OTHERWISE.
H. ALL FILLET WELDS SHALL BE VISUALLY INSPECTED BY THE TESTING FIRM.
I. GROOVE WELDS SHALL BE FULL PENETRATION, UNLESS NOTED OTHERWISE.
J. ALL COMPLETE JOINT PENETRATION WELDS SHALL BE ULTRASONICALLY TESTED UPON COMPLETION OF THE CONNECTION, EXCEPT PLATES LESS THAN OR EQUAL TO 1/4-INCH-THICK SHALL BE MAGNETIC PARTICLE TESTED. REDUCTION IN TESTING MAY BE MADE IN ACCORDANCE WITH THE BUILDING CODE WITH APPROVAL OF THE ENGINEER.
K. A RUN-OFF TAB SHALL BE USED AT ALL BEVEL AND FULL PENETRATION WELDS. RUN-OFF TABS SHALL BE REMOVED BY NEAT CUTS AFTER WELD IS COMPLETED. GRIND SMOOTH WHERE REQUIRED BY DETAIL.
L. WHERE REQUIRED BY DETAIL, REMOVE WELD BACKING BARS AND GRIND SMOOTH AFTER WELD IS COMPLETED.
M. WHERE NECESSARY, REMOVE GALVANIZING OR PRIMER PRIOR TO WELDING.
N. STEEL USING COMPLETE JOINT PENETRATION GROOVE WELDS THAT FUSE THROUGH THE THICKNESS OF THE FLANGE OR WEB SHALL HAVE A MINIMUM CHARPY V-NOTCH IMPACT TESTING VALUE AS FOLLOWS:
   1. ASTM A6/A6M HOT-ROLLED SHAPES WITH A FLANGE THICKNESS EXCEEDING 2 INCHES AND BUILT-UP HEAVY SHAPES WITH PLATES EXCEEDING 2 INCHES IN THICKNESS: 20 FT-LB AT 70° FAHRENHEIT.
   2. REGARDLESS OF THICKNESS, ALL TRUSSES, LATERAL SYSTEM MEMBERS (INCLUDING COLUMNS, WIND GIRDERS, BRACES, ETC): 20 FT-LB AT 70° FAHRENHEIT.
   3. STEEL EXPOSED TO TEMPERATURES IN SERVICE BELOW 50° FAHRENHEIT: 20 FT-LB AT SERVICE TEMPERATURE + 20° FAHRENHEIT; 40° FAHRENHEIT MAXIMUM.
   4. WELD METAL: 20 FT-LB AT -20° FAHRENHEIT AND 40 FT-LB AT 70° FAHRENHEIT.
   5. TESTING IS TO BE IN ACCORDANCE WITH ASTM A6/A6M, SUPPLEMENTARY REQUIREMENT S30, CHARPY V-NOTCH IMPACT TEST FOR STRUCTURAL SHAPES – ALTERNATE CORE LOCATION, AT ROLLED SHAPES AND ASTM A673 FOR PLATES, AT ANY PERMITTED LOCATIONS. WELD METAL SHALL BE TESTED IN ACCORDANCE WITH ASTM E23, STANDARD METHODS FOR NOTCHED BAR IMPACT TESTING OF METALLIC MATERIALS FOR WELD METAL.

STEEL DECK GENERAL REQUIREMENTS

A. ALL STEEL DECK SHALL BE MANUFACTURED AND INSTALLED IN ACCORDANCE WITH THE DIVISION 05 SPECIFICATIONS.
B. STEEL DECK SHALL BE SUPPORTED BY A MINIMUM OF FOUR SUPPORT LOCATIONS (THREE SPAN CONDITION), UNLESS NOTED OTHERWISE.
C. THE CONTRACTOR SHALL COORDINATE SLAB/DECK OPENING SIZES AND LOCATIONS PER ARCHITECTURAL AND MEP CONTRACT DOCUMENTS. THE CONTRACTOR SHALL PROVIDE OPENING SUPPORT FRAMING AND/OR REINFORCEMENT AS REQUIRED PER TYPICAL DETAILS AND SUBMIT PROPOSED SLAB/DECK OPENINGS FOR REVIEW BY THE STRUCTURAL ENGINEER OF RECORD.
D. SHOP DRAWINGS SHALL BE SUBMITTED INDICATING:
   1. MATERIAL STRENGTH
   2. SECTION PROPERTIES
   3. DECK GAGE, LAYOUT
   4. FASTENER TYPE
   5. CONNECTION PATTERN
   6. CLOSURE ANGLES
E. THE CAPACITY OF THE DECK SHALL BE BASED ON CURRENT ICC-ES EVALUATION REPORTS.

COMPOSITE STEEL FLOOR DECK

A. COMPOSITE STEEL FLOOR DECK SHALL BE A MINIMUM YIELD STRENGTH OF 50 KSI AND SHALL CONFORM TO ASTM A653 WITH G90 HOT-DIPPED GALVANIZATION, UNLESS NOTED OTHERWISE.
B. WHERE COMPOSITE BEAM DESIGNATIONS OCCUR ON FRAMING PLANS THROUGH THE CALLING OUT OF A NUMBER OF REQUIRED STEEL HEADED STUD ANCHORS, FLOOR AND BEAM ARE DESIGNED TO ACT COMPOSITELY BY USE OF SHEAR STUDS ON THE BEAM TOP FLANGE.
   1. SEE DISTRIBUTED STEEL HEADED STUD ANCHOR PLACEMENT DETAIL FOR PLACEMENT REQUIREMENTS.
   2. IF LONGITUDINAL STEEL HEADED STUD ANCHOR SPACING EXCEEDS 12 INCHES, PLUG WELD DECK TO BEAM BETWEEN STUDS.
C. COMPOSITE STEEL FLOOR DECK SHALL BE ATTACHED TO SUPPORTS WITH 5/8 INCH DIAMETER PUDDLE WELDS. WHEN DECK THICKNESS IS LESS THAN 0.028 INCHES, WELDS MUST BE MADE THROUGH MINIMUM 16 GAGE WELDING WASHERS. SPACING OF WELDS SHALL BE AS FOLLOWS:
   1. AT BUTTED ENDS: AT 12 INCHES ON CENTER
   2. AT PERIMETER/EDGES OF BUILDING: AT 12 INCHES ON CENTER
   3. INTERMEDIATE SUPPORTS: AT 12 INCHES ON CENTER
   4. SIDE LAPS: AT MID-SPAN OR AT 3'-0" ON CENTER, WHICHEVER IS SMALLER. HEX HEAD SCREWS, SIZE #10, OR CRIMPING (BUTTON PUNCHING) MAY BE USED AT SIDE LAP CONNECTIONS.
D. CONCRETE BONDING-TYPE UNITS SHALL BE FORMED WITH DEFORMATIONS TO PROVIDE AN INTERLOCK BETWEEN THE CONCRETE AND STEEL.
E. DESIGN AND PROVIDE FLASHING AND CLOSURE PLATES AT WALL ENDS OF ALL UNITS, AROUND COLUMNS, AND AT ALL PERIMETER LOCATIONS REQUIRING CLOSURE. COORDINATE ALL CLOSURES WITH ELEVATOR, STAIR, ESCALATOR, AND OTHER ARCHITECTURAL DETAILS.
F. SHEAR CONNECTORS / SHEAR STUDS SHALL BE TYPE B AND CONFORM TO AWS D1.1, ASTM A108, AND ASTM A29 SPECIFICATIONS FOR 1010 THROUGH 1020 MILD STEELS. MINIMUM YIELD STRENGTH = 51 KSI AND MINIMUM ULTIMATE TENSILE STRENGTH = 65 KSI.
G. ALL STEEL HEADED STUD ANCHORS SHALL BE 3/4 INCH DIAMETER x (FLUTE HEIGHT + 1 ½ INCH) AFTER WELDING, UNLESS NOTED OTHERWISE. MINIMUM CLEARANCE FROM TOP OF STUD TO TOP OF SLAB IS 1 INCH.
H. DISTRIBUTE STEEL HEADED STUD ANCHORS UNIFORMLY OVER BEAM SPAN UNLESS NOTED OTHERWISE ON PLAN. MAXIMUM SPACING OF 3/4 INCH HEADED STUD ANCHORS SHALL NOT EXCEED 12 INCHES ON CENTER (ONE STUD PER FOOT) UNLESS NOTED OTHERWISE ON PLAN.
I. THE COMPOSITE STEEL FLOOR DECK INSTALLATION, WHEN COMPLETE, SHOULD BE READY TO RECEIVE CONCRETE. THE COMPOSITE STEEL FLOOR DECK HAS BEEN DESIGNED TO SUPPORT CONSTRUCTION LOADS UNSHORED, UNLESS NOTED OTHERWISE. IF ANY SHORING IS USED, IT SHALL BE DESIGNED AND SUPPLIED BY THE CONTRACTOR AND SHALL BE SHOWN ON THE SHOP DRAWINGS. SHORING SHALL BE APPROVED BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO PLACING CONCRETE.
J. COMPOSITE STEEL FLOOR DECKS ARE TO BE POURED LEVEL TO COLUMNS OR AS INDICATED ON THE DRAWINGS.
K. MAINTAIN INDICATED MINIMUM SLAB THICKNESS OVER BEAMS THROUGHOUT SPAN. ELEVATED SLAB SHALL BE PLACED WITH MINIMUM THICKNESS THROUGHOUT WITH NO MORE THAN ½" OF ADDITIONAL THICKNESS IN ANY LOCATION TO HELP WITH THE LEVELING OF THE SLAB.
L. COMPOSITE STEEL FLOOR DECK HANGER TAB LOADS SHALL NOT EXCEED 60 POUNDS PER HANGER TAB. IN ADDITION, LOADS ON HANGER TABS SHALL BE DISTRIBUTED IN SUCH A MANNER THAT THE TRIBUTARY LOADS FOR EACH HANGER TAB SHALL NOT EXCEED 5 POUNDS PER SQUARE FOOT. DO NOT HANG MEP EQUIPMENT AND PIPING DIRECTLY FROM STEEL DECK.

STEEL ROOF DECK

A. STEEL ROOF DECK SHALL BE A MINIMUM YIELD STRENGTH OF 33 KSI, UNLESS NOTED OTHERWISE. ALL INTERIOR STEEL ROOF DECK SHALL CONFORM TO ASTM A1008; FACTORY PRIMED FOR PAINT. ALL EXPOSED STEEL ROOF DECK SHALL CONFORM TO ASTM A653 WITH G90 HOT-DIPPED GALVANIZATION, UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR EXTENTS.
B. STEEL ROOF DECK SHALL BE ATTACHED TO STEEL SUPPORTS WITH 5/8 INCH DIAMETER PUDDLE WELDS AND TO COLD-FORMED METAL FRAMING WITH #12 SELF-DRILLING SCREWS UNLESS NOTED OTHERWISE. WHEN DECK THICKNESS IS LESS THAN 0.028 INCHES, WELDS MUST BE MADE THROUGH MINIMUM 16 GAGE WELDING WASHERS. SPACING OF WELDS SHALL BE AS SPECIFIED IN THE DECK ATTACHMENT SCHEDULE.
C. WHERE STEEL MEMBERS ARE PARALLEL TO THE DECK FLUTES AND AT THE SAME ELEVATION OF THE BOTTOM OF THE DECK, ADJUST DECK LAYOUT AND WELD DECK TO STEEL WITH SAME WELDING AS REQUIRED FOR SIDE BOUNDARIES.
D. ERECT STEEL DECK CLOSURES AND OTHER LIGHT GAGE MATERIAL REQUIRED TO PRODUCE A COMPLETED INSTALLATION.
E. FLAT, RIDGE, AND VALLEY PLATES:
   UNLESS NOTED OTHERWISE, CONTRACTOR SHALL PROVIDE FLAT PLATES (20 GAGE MINIMUM) AT ALL LOCATIONS WHERE ROOF DECK CHANGES DIRECTION AND RIDGE OR VALLEY PLATES (20 GAGE MINIMUM) AT ALL LOCATIONS WHERE ROOF SLOPE EXCEEDS ½ INCH PER FOOT.
F. DO NOT HANG CEILINGS, DUCTS, LIGHT FIXTURES, EQUIPMENT, OR OTHER ITEMS FROM THE ROOF DECK WITHOUT PRIOR APPROVAL FROM THE DECK SUPPLIER AND REVIEW BY THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD.
G. SEE ARCHITECTURAL DRAWINGS FOR FINAL ROOF SLOPES. WHERE STRUCTURAL FRAMING DOES NOT CREATE THE SPECIFIED ROOF SLOPE, IT SHALL BE CREATED WITH RIGID INSULATION ABOVE THE DECK.

NON-COMPOSITE STEEL FORM DECK

A. NON-COMPOSITE STEEL FORM DECK SHALL BE A MINIMUM YIELD STRENGTH OF 60 KSI AND SHALL CONFORM TO ASTM A653 WITH G90 HOT-DIPPED GALVANIZATION, UNLESS NOTED OTHERWISE.
B. NON-COMPOSITE STEEL FORM DECK SHALL BE ATTACHED TO SUPPORTS WITH 5/8 INCH DIAMETER PUDDLE WELDS. WHEN DECK THICKNESS IS LESS THAN 0.028 INCHES, WELDS MUST BE MADE THROUGH MINIMUM 16 GAGE WELDING WASHERS. SPACING OF WELDS SHALL BE AS FOLLOWS:
   1. AT BUTTED ENDS: AT 10 INCHES ON CENTER
   2. AT PERIMETER/EDGES OF BUILDING: AT 10 INCHES ON CENTER
   3. INTERMEDIATE SUPPORTS: AT 10 INCHES ON CENTER
   4. SIDE LAPS: FOR FORM DECK WITH SPANS 3'-6" OR GREATER, PROVIDE TWO CONNECTIONS PER SPAN. HEX HEAD SCREWS, SIZE #10, OR CRIMPING (BUTTON PUNCHING) MAY BE USED AT SIDE LAP CONNECTIONS.

STEEL JOISTS

A. MANUFACTURE AND ERECT ALL STRUCTURAL STEEL JOISTS, JOIST GIRDERS, AND BRIDGING IN ACCORDANCE WITH SPECIFICATION SECTION 052100, SPECIFICATIONS OF THE STEEL JOIST INSTITUTE AND ALL OSHA REQUIREMENTS.
B. JOIST MANUFACTURER SHALL DESIGN JOISTS PER LISTED DESIGN CRITERIA AND ANY ADDITIONAL LOADING SHOWN ON PLAN OR IN DETAILS. AT A MINIMUM, JOIST SHALL BE DESIGNED PER STEEL JOIST INSTITUTE (SJI) LOAD TABLES.
C. JOISTS SHALL BE EQUALLY SPACED BETWEEN COLUMN LINES OR OTHER SPECIFICALLY LOCATED FRAMING MEMBERS UNLESS NOTED OTHERWISE.
D. STEEL JOISTS, JOIST GIRDERS, BRIDGING, AND THEIR CONNECTIONS SHALL BE DESIGNED FOR NET UPLIFT (NEGATIVE PRESSURE) AS INDICATED IN THE STRUCTURAL DRAWINGS. REFER TO THE APPLICABLE BUILDING CODE LISTED IN DESIGN CRITERIA FOR LOAD COMBINATIONS.
   1. DEAD LOAD (MIN) = 5 PSF
E. STEEL JOISTS, JOIST GIRDERS, BRIDGING, AND THEIR CONNECTIONS SHALL BE DESIGNED FOR ADDITIONAL DOWN FORCE RESULTING FROM UNIFORM WIND (POSITIVE PRESSURE) AS INDICATED IN THE STRUCTURAL DRAWINGS.
   1. DEAD LOAD (MAXIMUM) = 12 PSF
F. JOIST MANUFACTURER SHALL DESIGN AND DETAIL ALL BRIDGING PER SJI REQUIREMENTS AND CLEARLY INDICATE LOCATION ON STEEL JOIST ERECTION DRAWINGS.
   1. BRIDGING SHALL BE DESIGNED TO FULLY BRACE TOP CHORD OF JOISTS UNDER SERVICE LOADS FOR JOISTS NOT BRACED BY STEEL ROOF DECK.
   2. BOTTOM CHORD OF ROOF JOISTS SHALL BE DESIGNED FOR NET UPLIFT (COMPONENTS & CLADDING) SHOWN ON DIAGRAM ON S002, UNLESS NOTED OTHERWISE.
   3. BOTTOM CHORD OF ROOF JOIST GIRDERS SHALL BE DESIGNED FOR NET UPLIFT (MAIN WIND FORCE RESISTING SYSTEM) SHOWN ON DIAGRAM ON S002.
   4. PROVIDE ADDITIONAL BOTTOM CHORD BRIDGING AS REQUIRED FOR NET UPLIFT (MAIN WIND FORCE RESISTING SYSTEM) SHOWN ON DIAGRAM ON S002, FOR ROOF FRAMING.
G. AN ALLOWABLE STRESS INCREASE FOR LOAD COMBINATIONS INCLUDING WIND IS NOT PERMITTED.
H. JOIST MANUFACTURER SHALL CAMBER JOISTS PER SJI CRITERIA.
I. PROVIDE MINIMUM BEARING PER SJI REQUIREMENTS AND CONNECT TO STEEL SUPPORT AS FOLLOWS:
   1. K-SERIES: 3/16" x 1" FILLET WELD EACH SIDE
   2. LH AND DLH SERIES: ¼" x 2" FILLET WELD EACH SIDE
J. PROVIDE STANDARD DEPTH OF BEARING FOR ALL JOISTS AS SHOWN BELOW UNLESS NOTED IN DRAWINGS:
   1. K-SERIES – 2 ½"
   2. LH – 5"
   3. JOIST GIRDER – 7 ½"
   4. WHERE STEEL JOIST OR JOIST GIRDER SLOPE EXCEEDS ¼ INCH PER FOOT, PROVIDE SLOPED BEARING AS NOTED IN SLOPED SEAT REQUIREMENTS OF SJI.
K. JOIST MANUFACTURER SHALL DESIGN AND DETAIL FIELD BOLTING FOR ERECTION PER SJI REQUIREMENTS.

WILLIAM JOHN RAIOLA LICENSE No. 80234 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/29/2026

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JEZERINAC GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T. 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 286 9004
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS

ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date

CONTRACT DOCUMENT'S FOR:
SLPS C.A. MOORE GYMNASIUM
ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION

STEEL GENERAL NOTES & SCHEDULES

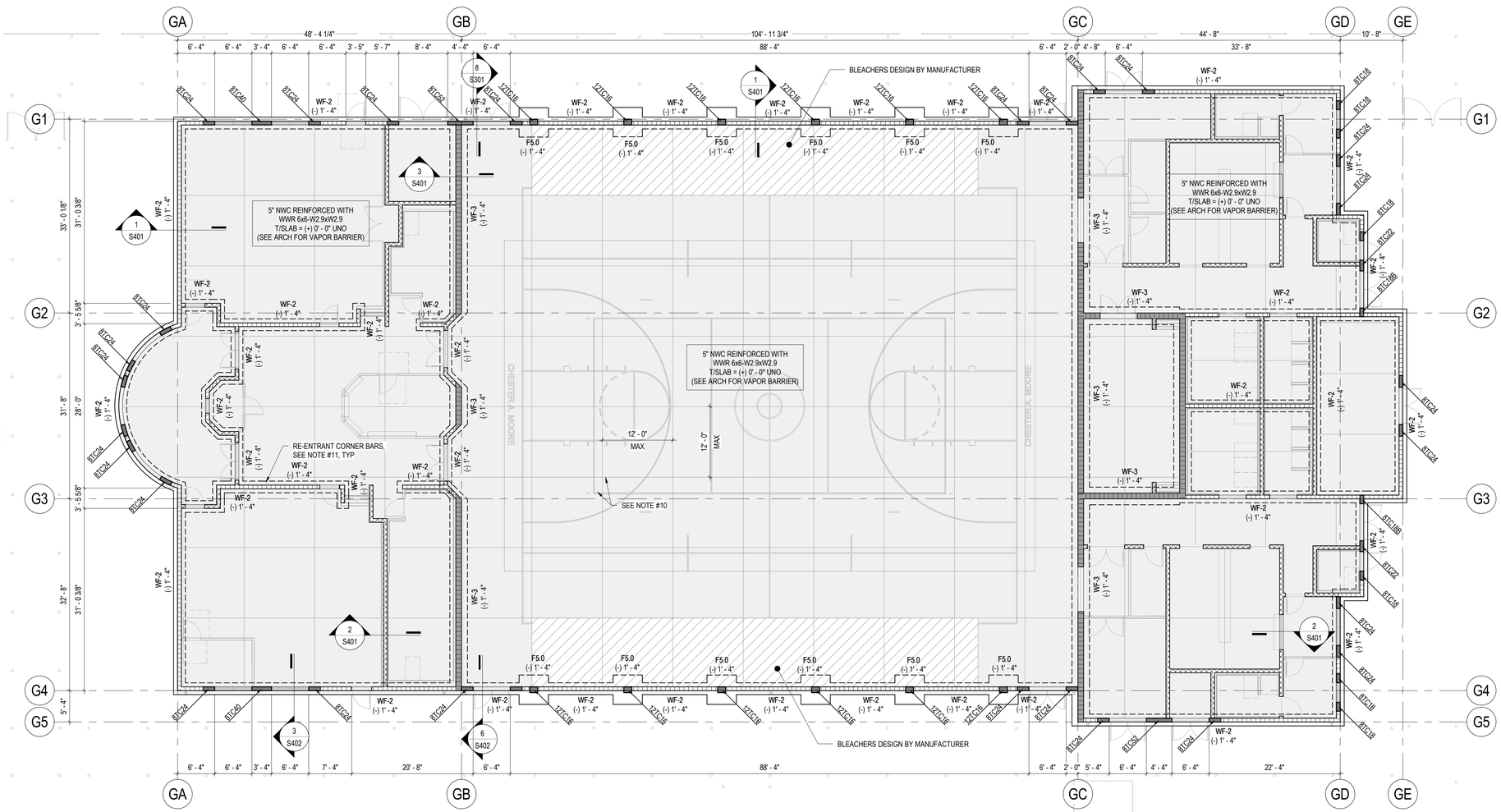
PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W.JR/TEB

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

S005

5/29/2026 3:08:03 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR-JG-CA Moore_R26.rvt



1 S100 SLAB-ON-GROUND & FOUNDATION PLAN

1" = 10'-0"

FOUNDATION PLAN NOTES:

1. F#, WF#, OR P# (-) X' - X" — FOUNDATION OR PIER MARK (SEE SCHEDULE ON THIS SHEET); TOP OF FOOTING/PIER ELEVATION
2. DENOTES CONCRETE COLUMN/PIER/WALL. FOR COLUMN SCHEDULE AND DETAILS SEE S330 SERIES. FOR WALL DETAILS SEE S300 AND 400 SERIES.
3. FOR PIPING PASSING UNDER WALL FOUNDATIONS, SEE S300. PIPING PASSING UNDER FOOTINGS SHALL BE INSPECTED BEFORE FOUNDATIONS ABOVE ARE PREPARED.
4. GENERAL CONTRACTOR SHALL COORDINATE PLUMBING AND UTILITIES LOCATIONS WITH FOUNDATIONS AS NEEDED. ADDITIONALLY, GENERAL CONTRACTOR SHALL COORDINATE FOUNDATION ELEVATIONS WITH PLUMBING AND UTILITIES AS NEEDED. FORWARD ANY FOUNDATION LOCATION CHANGE REQUESTS TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW AND APPROVAL.
5. DENOTES 8" LOAD BEARING CONCRETE MASONRY UNIT (CMU) WALL WITH #5 AT 24" OC VERTICAL REINFORCEMENT, CENTERED IN FULLY-GROUTED CELL.
6. DENOTES 12" LOAD BEARING CONCRETE MASONRY UNIT (CMU) WALL WITH #5 AT 24" OC VERTICAL REINFORCEMENT, CENTERED IN FULLY-GROUTED CELL.
7. DENOTES NON-LOAD BEARING CONCRETE MASONRY UNIT (CMU) WALL WITH #5 AT 48" OC VERTICAL REINFORCEMENT, CENTERED IN FULLY-GROUTED CELL.

SLAB-ON-GROUND PLAN NOTES:

8. REFERENCE BUILDING TOP-OF-SLAB ELEVATION = (+) 0' - 0". SEE CIVIL FOR ACTUAL ELEVATIONS.
9. SLAB-ON-GROUND INFORMATION: SEE PLAN FOR REINFORCING AND T/SLAB ELEVATION.
10. CJ: DENOTES SLAB-ON-GROUND CONTROL JOINT. FOR CONTROL JOINT REQUIREMENTS, SEE S301.
11. FOR RE-ENTRANT CORNER BARS, SEE S301.
12. FOR THICKENED SLAB UNDER NON-LOAD BEARING CONCRETE MASONRY WALLS, SEE S401.
13. GENERAL CONTRACTOR SHALL COORDINATE HOUSEKEEPING PAD LOCATIONS.
14. DENOTES STEP IN TOP OF SLAB, SEE S301.
15. SEE CIVIL DRAWINGS FOR BASE AND SUBGRADE PREPARATION INFORMATION.
16. SEE ARCHITECTURAL DRAWINGS FOR:
    - VAPOR BARRIER REQUIREMENTS AND LOCATIONS.
    - ALL SLOPED SLAB AREAS. (MAINTAIN SLAB THICKNESS NOTED ON PLAN AS A MINIMUM IN ALL AREAS).
    - ALL DEPRESSED SLAB AND/OR RAISED SLAB AREAS. (MAINTAIN SLAB THICKNESS NOTED ON PLAN AS A MINIMUM IN ALL AREAS).
    - ALL DIMENSIONS NOT SHOWN. VERIFY ALL DIMENSIONS SHOWN IN STRUCTURAL DRAWINGS WITH ARCHITECTURAL DRAWINGS AND REPORT ANY DISCREPENCIES OR DIMENSIONS NOT SHOWN ON ARCHITECTURAL DRAWINGS FOR CLARIFICATION.
    - SLAB SLOPES, DRAINS, STEPS, PENETRATIONS, FINISHES, AND ANY OTHER ADDITIONAL INFORMATION.

CONTINUOUS WALL FOOTING SCHEDULE

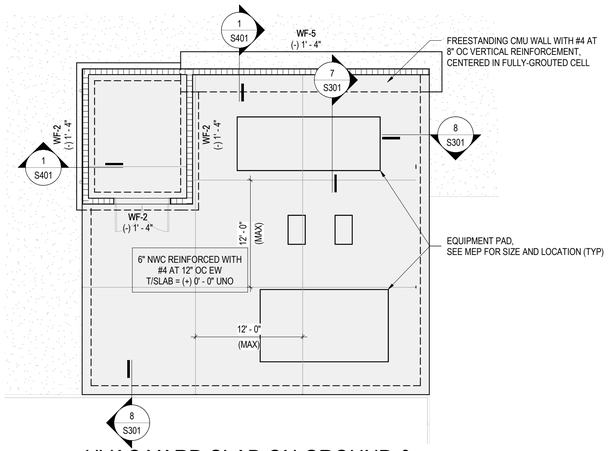
MARK	WIDTH	THICKNESS	REINFORCING
WF-2	2' - 0"	1' - 6"	(3) #5 LONG T&B WITH #5 AT 9" OC TRANS T&B
WF-3	3' - 0"	1' - 6"	(4) #5 LONG T&B WITH #5 AT 9" OC TRANS T&B
WF-5	4' - 6"	2' - 0"	(4) #5 LONG T&B WITH #5 AT 12" OC TRANS T&B

ISOLATED FOOTING SCHEDULE

MARK	LENGTH	WIDTH	THICKNESS	TOP REBAR - LONG BARS	TOP REBAR - SHORT BARS	BOTTOM REBAR - LONG BARS	BOTTOM REBAR - SHORT BARS
F5.0	5' - 0"	5' - 0"	1' - 6"	-	-	(6) #6	(6) #6

CONCRETE TIE-COLUMN SCHEDULE

MARK	COLUMN WIDTH	COLUMN DEPTH	COLUMN REINFORCEMENT	REMARKS
8TC18	7 5/8"	18"	REFERENCE 7/S330	
8TC18B	7 5/8"	17 5/8"	REFERENCE 7/S330	
8TC22	7 5/8"	30"	REFERENCE 7/S330	
8TC24	7 5/8"	24"	REFERENCE 7/S330	
8TC40	7 5/8"	40"	REFERENCE 7/S330	
8TC52	7 5/8"	52"	REFERENCE 7/S330	
12TC16	11 5/8"	16"	REFERENCE 7/S330	



2 S100 HVAC YARD SLAB ON GROUND & FOUNDATION PLAN

1/8" = 1'-0"

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 286 9004
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS

ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date

CONTRACT DOCUMENTS FOR:
SLPS C.A. MOORE GYMNASIUM
ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION

SLAB-ON-GROUND & FOUNDATION PLAN

PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W.JR/TEB

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

S100

WILLIAM JOHN RAIOLA — LICENSE — No. 80234 — STATE OF FLORIDA — PROFESSIONAL ENGINEER
05/29/2026

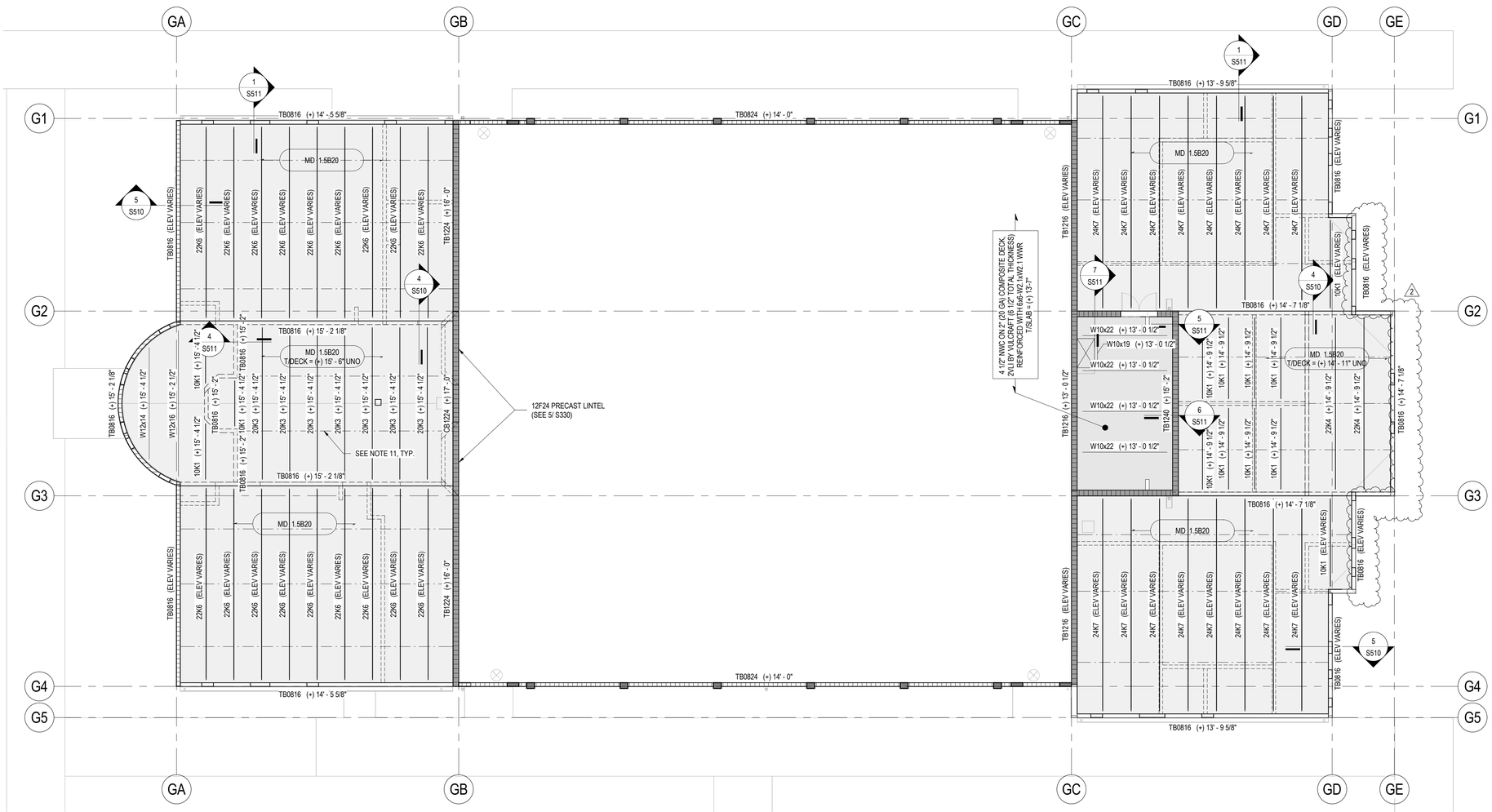
THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JEZERINAC GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

5/29/2026 3:58:06 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR_JG-CA Moore_R26.rvt



1 S101 LOW ROOF FRAMING PLAN

1" = 10'-0"

STEEL ROOF FRAMING PLAN NOTES:

1. ROOF DECK TAG:
   - ARROWS DENOTE DECK SPAN DIRECTION
   - #.#X## (#.#X##): DENOTES DECK MARK (SEE SCHEDULE ON THIS SHEET FOR DECK PROPERTIES AND ATTACHMENT PATTERNS)
2. SEE PLAN FOR TOP OF STEEL ELEVATIONS.
3. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL DIMENSIONS NOT SHOWN AND ALL SLAB ELEVATIONS.
4. JOIST SPACING SHALL NOT EXCEED 5'-0" (TYPICAL) UNLESS NOTED OTHERWISE.
5. ##K#: DENOTES STANDARD SJI OPEN WEB STEEL JOIST MARK.
   - JOIST SPACING SHALL NOT EXCEED 5'-0" (TYPICAL) UNLESS NOTED OTHERWISE.
   - JOISTS SHALL BE REINFORCED AT CONCENTRATED LOADS PER DETAIL1/ S513.
   - PIPING SUPPORTED BY JOISTS SHALL BE COORDINATED BY THE GENERAL CONTRACTOR PRIOR TO JOIST FABRICATION.
   - NOTIFY STRUCTURAL ENGINEER OF RECORD IF PIPING WEIGHT TO BE SUPPORTED BY JOISTS IS IN EXCESS OF THOSE NOTED IN SCHEDULE ON SHEET S002.
   - STEEL JOISTS ARE EQUALLY SPACED BETWEEN SUPPORTS UNLESS NOTED OTHERWISE.
6. PROVIDE DECK SUPPORT FRAMING AT ROOF OPENINGS INCLUDING DRAINS, VENTS, EXHAUST FANS, HATCHES, AND OTHER OPENINGS LARGER THAN 12" PER 4/ S513. COORDINATE SIZES AND LOCATIONS WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
7. ROOF EDGE ANGLES SHALL BE CONTINUOUS. FOR SPLICE CONNECTION SEE 7/ S513 .
8. NOTIFY STRUCTURAL ENGINEER OF RECORD IF MECHANICAL UNITS SUPPLIED EXCEED WEIGHTS NOTED ON PLAN. COORDINATE EXACT DIMENSIONS AND LOCATIONS WITH MECHANICAL DRAWINGS AND UNITS SUPPLIED.
9. PROVIDE EQUIPMENT SUPPORT FRAMING AT ROOFTOP EQUIPMENT. UNIT SUPPORT CURBS/FRAMES AND THEIR CONNECTION TO THE STRUCTURE SHALL BE DESIGNED AND DETAILED BY OTHERS, UNLESS SPECIFICALLY SHOWN.
10. GENERAL CONTRACTOR SHALL COORDINATE OPEN WEB STEEL JOIST BRIDGING AND BRACING REQUIREMENTS PER OPEN WEB STEEL JOIST SUPPLIER.
11. HORIZONTAL BRIDGING PER JOIST MANUFACTURER AS SHOWN ON PLAN, SEE 9/ S511 TYP.
12. STEEL CONNECTION DESIGN HAS BEEN DELEGATED TO BE SELECTED/COMPLETED BY STRUCTURAL STEEL FABRICATOR (SEE STRUCTURAL STEEL GENERAL NOTES & PROJECT SPECIFICATIONS).

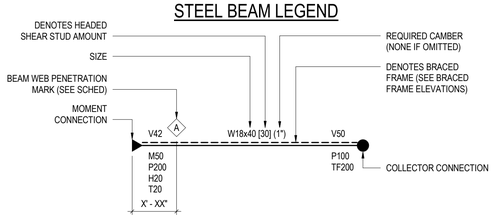
    REACTIONS SHOWN ARE IN SERVICE LEVEL (ALLOWABLE STRESS DESIGN) FOR CONNECTION DESIGN ARE NOTED AS FOLLOWS:

    ——▸ :DENOTES MOMENT CONNECTION.
    M = #K-FT :DESIGN MOMENT CONNECTIONS FOR FORCES INDICATED ON PLAN AND IN ELEVATIONS.

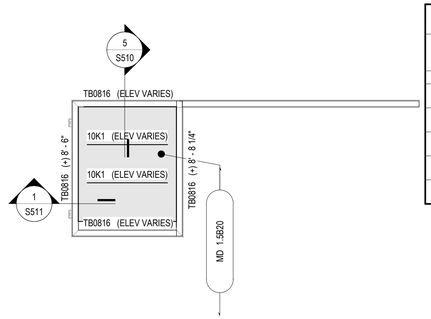
    V = #K :DENOTES SHEAR REACTION. DESIGN SHEAR CONNECTIONS FOR FORCES INDICATED. IF REACTION IS NOT SHOWN, DESIGN FOR 25 KIPS.

    ⊠ :DENOTES LATERAL BRACING. SEE DETAILS ON PLAN.

STEEL BEAM LEGEND



V INDICATES FACTORED VERTICAL SHEAR IN KIPS (SEE SCHED IF OMITTED)
M INDICATES FACTORED END MOMENT IN KIP-FT (SEE SCHED IF OMITTED)
P INDICATES FACTORED END AXIAL FORCE IN KIPS
H INDICATES FACTORED HORIZONTAL SHEAR FORCE IN KIPS
T INDICATES TORSIONAL MOMENT IN KIP-FT
TF INDICATES AXIAL FORCE TRANSFERRED THROUGH THE CONNECTION JOINT IN KIPS



2 S101 HVAC YARD FRAMING PLAN

1/8" = 1'-0"

CONCRETE BEAM SCHEDULE

MARK	SIZE: BEAM WIDTH - W (IN)	SIZE: BEAM HEIGHT - H (IN)	TOP BARS	BOTTOM BARS	ADDITIONAL REINFORCEMENT	STIRRUP S1	STIRRUP S2	REMARKS
CB1224	11 5/8"	24"	(3) #9 + (3) #9*	(3) #9	-	#5 AT 5" OC	#5 AT 5" OC	
TB0816	7 5/8"	16"	(2) #6	(2) #6	-	#3 AT 8" OC	#3 AT 8" OC	HOOK TOP AND BOTTOM BARS
TB0824	7 5/8"	24"	(2) #6	(2) #6	-	#3 AT 8" OC	#3 AT 8" OC	HOOK TOP AND BOTTOM BARS
TB1216	11 5/8"	16"	(3) #6	(3) #6	-	#3 AT 8" OC	#3 AT 8" OC	HOOK TOP AND BOTTOM BARS
TB1224	11 5/8"	24"	(3) #6	(3) #6	-	#3 AT 6" OC	#3 AT 12" OC	HOOK TOP AND BOTTOM BARS
TB1240	11 5/8"	40"	(3) #6	(3) #6	(2) #4 @8" OC SKIN FRICTION BARS	#3 AT 6" OC	#3 AT 12" OC	HOOK TOP AND BOTTOM BARS

METAL DECK SCHEDULE

DECK MARK	DECK DEPTH (THICKNESS)	DECK TYPE	DECK GAUGE	MINIMUM DECK SECTION PROPERTIES: Ip (in4/ft)	In (in4/ft)	Sp (in4/ft)	Sn (in4/ft)	REMARKS
MD 1.5 B 20	1 1/2"	WIDE RIB ROOF DECK	20 GA	0.201	0.222	0.234	0.247	

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 266 9004
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS

ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date
0	ASI#1	05.29.26

CONTRACT DOCUMENTS FOR:
SLPS C.A. MOORE GYMNASIUM
ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION

LOW ROOF FRAMING PLAN

PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W.RITTER

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

WILLIAM JOHN RAIOLA
LICENSE
No. 80234
STATE OF FLORIDA
PROFESSIONAL ENGINEER
05/29/2026

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

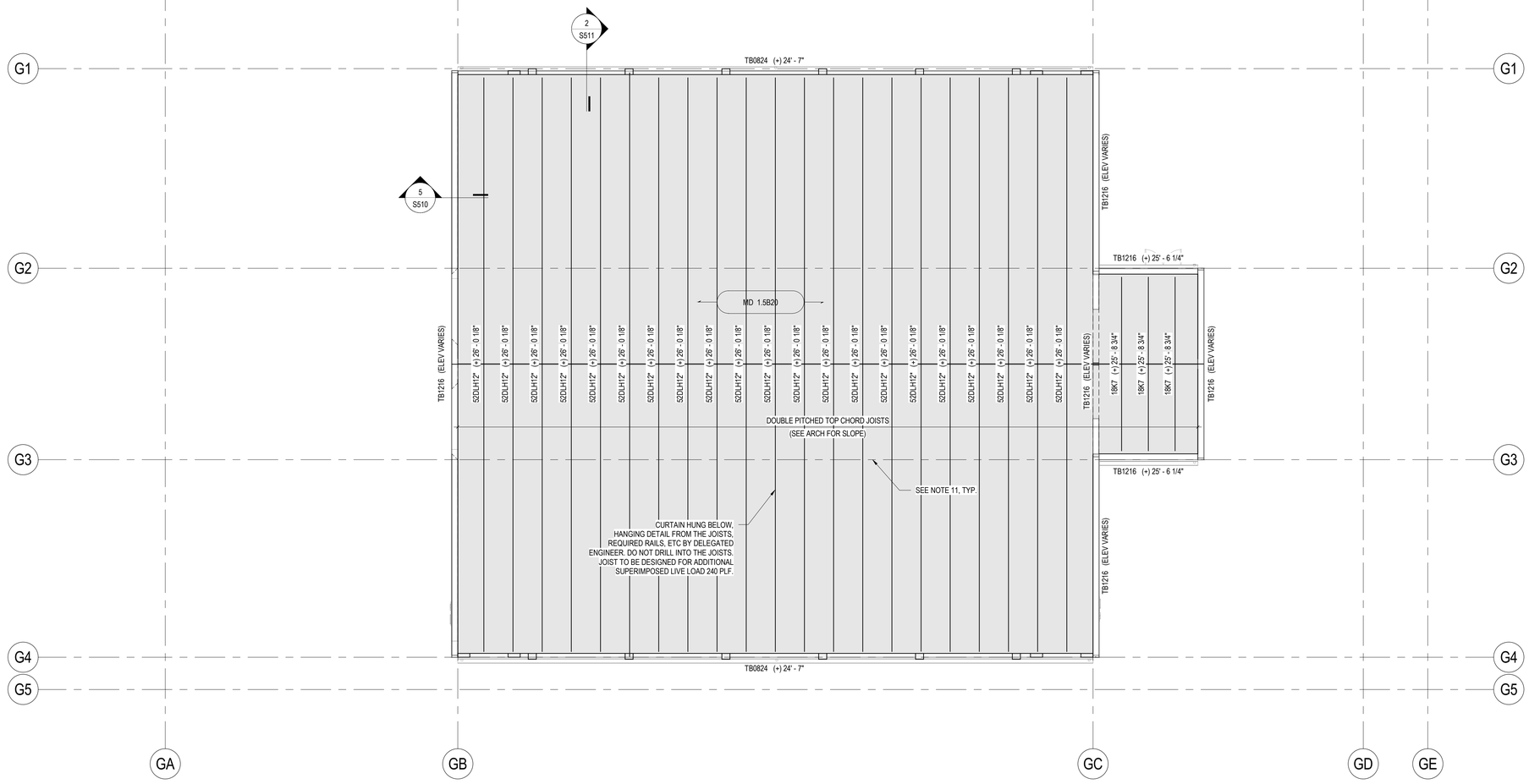
JEZERINAC
GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

JOB FILE NUMBER:
I-25129

S101

5/29/2026 3:08:08 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR-JG-GA Moore_R26.rvt



1 S102 HIGH ROOF FRAMING PLAN

1" = 10'-0"

STEEL ROOF FRAMING PLAN NOTES:

1. ROOF DECK TAG:
   - ARROWS DENOTE DECK SPAN DIRECTION
   - #.#X## (#.#X##): DENOTES DECK MARK (SEE SCHEDULE ON THIS SHEET FOR DECK PROPERTIES AND ATTACHMENT PATTERNS)
2. SEE PLAN FOR TOP OF STEEL ELEVATIONS.
3. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL DIMENSIONS NOT SHOWN AND ALL SLAB ELEVATIONS.
4. JOIST SPACING SHALL NOT EXCEED 5'-0" (TYPICAL) UNLESS NOTED OTHERWISE.
5. ##K#: DENOTES STANDARD SJI OPEN WEB STEEL JOIST MARK.
   - JOIST SPACING SHALL NOT EXCEED 5'-0" (TYPICAL) UNLESS NOTED OTHERWISE.
   - JOISTS SHALL BE REINFORCED AT CONCENTRATED LOADS PER DETAIL1/ S513.
   - PIPING SUPPORTED BY JOISTS SHALL BE COORDINATED BY THE GENERAL CONTRACTOR PRIOR TO JOIST FABRICATION.
   - NOTIFY STRUCTURAL ENGINEER OF RECORD IF PIPING WEIGHT TO BE SUPPORTED BY JOISTS IS IN EXCESS OF THOSE NOTED IN SCHEDULE ON SHEET S002.
   - STEEL JOISTS ARE EQUALLY SPACED BETWEEN SUPPORTS UNLESS NOTED OTHERWISE.
6. PROVIDE DECK SUPPORT FRAMING AT ROOF OPENINGS INCLUDING DRAINS, VENTS, EXHAUST FANS, HATCHES, AND OTHER OPENINGS LARGER THAN 12" PER 4/ S513. COORDINATE SIZES AND LOCATIONS WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
7. ROOF EDGE ANGLES SHALL BE CONTINUOUS. FOR SPLICE CONNECTION SEE 7/ S513 .
8. NOTIFY STRUCTURAL ENGINEER OF RECORD IF MECHANICAL UNITS SUPPLIED EXCEED WEIGHTS NOTED ON PLAN. COORDINATE EXACT DIMENSIONS AND LOCATIONS WITH MECHANICAL DRAWINGS AND UNITS SUPPLIED.
9. PROVIDE EQUIPMENT SUPPORT FRAMING AT ROOFTOP EQUIPMENT. UNIT SUPPORT CURBS/FRAMES AND THEIR CONNECTION TO THE STRUCTURE SHALL BE DESIGNED AND DETAILED BY OTHERS, UNLESS SPECIFICALLY SHOWN.
10. GENERAL CONTRACTOR SHALL COORDINATE OPEN WEB STEEL JOIST BRIDGING AND BRACING REQUIREMENTS PER OPEN WEB STEEL JOIST SUPPLIER.
11. HORIZONTAL BRIDGING PER JOIST MANUFACTURER AS SHOWN ON PLAN, SEE 9/ S511 TYP.
12. STEEL CONNECTION DESIGN HAS BEEN DELEGATED TO BE SELECTED/COMPLETED BY STRUCTURAL STEEL FABRICATOR (SEE STRUCTURAL STEEL GENERAL NOTES & PROJECT SPECIFICATIONS).

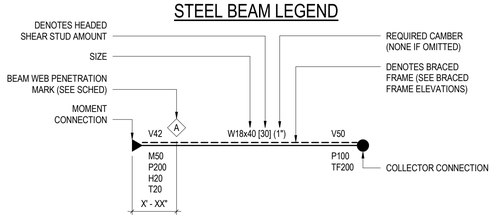
    REACTIONS SHOWN ARE IN SERVICE LEVEL (ALLOWABLE STRESS DESIGN) FOR CONNECTION DESIGN ARE NOTED AS FOLLOWS:

    ▸—— :DENOTES MOMENT CONNECTION.
    M = #K-FT DESIGN MOMENT CONNECTIONS FOR FORCES INDICATED ON PLAN AND IN ELEVATIONS.

    V = #K :DENOTES SHEAR REACTION. DESIGN SHEAR CONNECTIONS FOR FORCES INDICATED. IF REACTION IS NOT SHOWN, DESIGN FOR 25 KIPS.

    ⊠ :DENOTES LATERAL BRACING. SEE DETAILS ON PLAN.

STEEL BEAM LEGEND



V INDICATES FACTORED VERTICAL SHEAR IN KIPS (SEE SCHED IF OMITTED)
M INDICATES FACTORED END MOMENT IN KIP-FT (SEE SCHED IF OMITTED)
P INDICATES FACTORED END AXIAL FORCE IN KIPS
H INDICATES FACTORED HORIZONTAL SHEAR FORCE IN KIPS
T INDICATES TORSIONAL MOMENT IN KIP-FT
TF INDICATES AXIAL FORCE TRANSFERRED THROUGH THE CONNECTION JOINT IN KIPS

CONCRETE BEAM SCHEDULE

MARK	SIZE: BEAM WIDTH - W (IN)	SIZE: BEAM HEIGHT - H (IN)	TOP BARS	BOTTOM BARS	ADDITIONAL REINFORCEMENT	STIRRUP S1	STIRRUP S2	REMARKS
CB1224	11 5/8"	24"	(3) #9 + (3) #9*	(3) #9	-	#5 AT 5" OC	#5 AT 5" OC	
TB0816	7 5/8"	16"	(2) #6	(2) #6	-	#3 AT 8" OC	#3 AT 8" OC	HOOK TOP AND BOTTOM BARS
TB0824	7 5/8"	24"	(2) #6	(2) #6	-	#3 AT 8" OC	#3 AT 8" OC	HOOK TOP AND BOTTOM BARS
TB1216	11 5/8"	16"	(3) #6	(3) #6	-	#3 AT 8" OC	#3 AT 8" OC	HOOK TOP AND BOTTOM BARS
TB1224	11 5/8"	24"	(3) #6	(3) #6	-	#3 AT 6" OC	#3 AT 12" OC	HOOK TOP AND BOTTOM BARS
TB1240	11 5/8"	40"	(3) #6	(3) #6	(2) #4 @8" OC SKIN FRICTION BARS	#3 AT 6" OC	#3 AT 12" OC	HOOK TOP AND BOTTOM BARS

METAL DECK SCHEDULE

DECK MARK	DECK DEPTH (THICKNESS)	DECK TYPE	DECK GAUGE	MINIMUM DECK SECTION PROPERTIES: Ip (in4/ft)	In (in4/ft)	Sp (in4/ft)	Sn (in4/ft)	REMARKS
MD 1.5 B 20	1 1/2"	WIDE RIB ROOF DECK	20 GA	0.201	0.222	0.234	0.247	

WILLIAM JOHN RAIOLA LICENSE No. 80234 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/29/2026

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JEZERINAC GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 266 9004
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS
ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date

CONTRACT DOCUMENTS FOR:
SLPS C.A. MOORE GYMNASIUM
ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

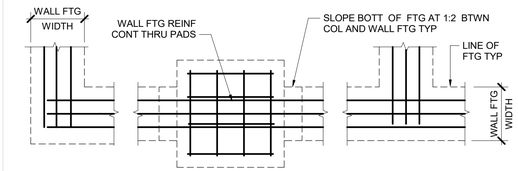
DWG IDENTIFICATION
HIGH ROOF FRAMING PLAN

PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	WJR/TEB

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

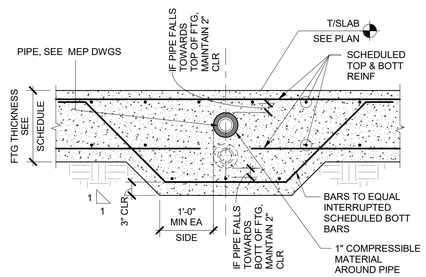
S102



FOOTING CORNER PLAN **INTERIOR FOOTING PLAN** **FOOTING INTERSECTION PLAN**

1 S300 FOOTING REINFORCING DETAILS

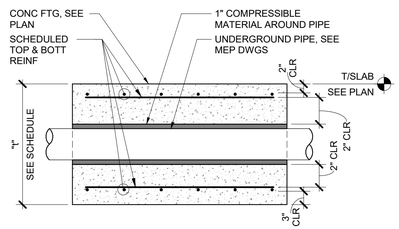
3/4" = 1'-0"



USE WHERE FOOTING THICKNESS IS LESS THAN 2'-0" THICK OR WHEN CLEARANCE TO REINFORCING AS SHOWN CANNOT BE ACHIEVED

2 S300 PIPE PENETRATION THROUGH FOOTING

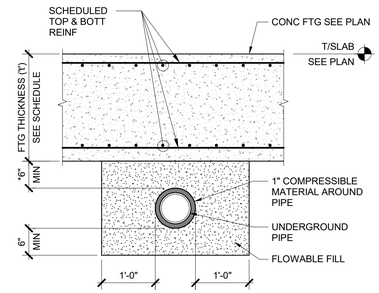
3/4" = 1'-0"



"t" DENOTES FTG THICKNESS. ADJUST THICKNESS AS REQUIRED TO PROVIDE NOTED CLEARANCES TO COMPRESSIBLE MATERIAL. PIPE TO BE LOCATED IN MIDDLE THIRD OF FOOTING THICKNESS. SEE 2 / S300 WHEN FOOTING IS 2'-0" THICK OR LESS OR WHEN CLEARANCE TO REINFORCING CANNOT BE ACHIEVED.

3 S300 PIPE PENETRATION THROUGH FOOTING

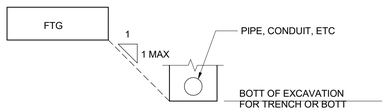
3/4" = 1'-0"



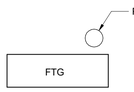
*FLOWABLE FILL SHALL BE PLACED UP TO BOTTOM OF FOOTING

4 S300 PIPE BELOW SLAB OR FOOTING

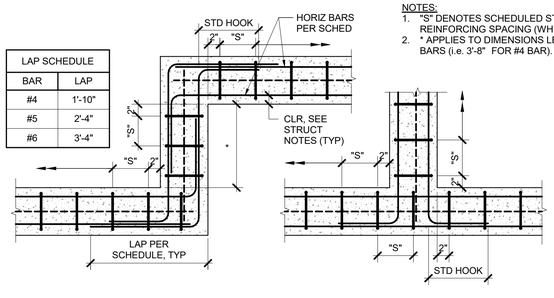
3/4" = 1'-0"



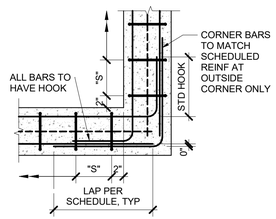
NOTE:
LOWER FOOTING AS REQUIRED TO PROVIDE MAX 1:1 SLOPE FROM BOTTOM EDGE OF FOOTING TO BOTTOM OF TRENCH EXCAVATION, OR LOWER FOOTING AS SHOWN BELOW, SO THAT TOP OF FOOTING IS BELOW PIPE, CONDUIT, ETC

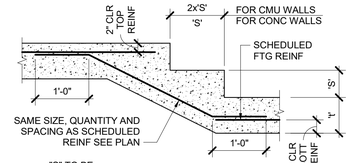


LAP SCHEDULE	
BAR	LAP
#4	1'-10"
#5	2'-4"
#6	3'-4"



NOTES:
1. "S" DENOTES SCHEDULED STIRRUP OR TRANSVERSE FOOTING REINFORCING SPACING (WHERE INDICATED)
2. * APPLIES TO DIMENSIONS LESS THAN 2 TIMES LAP SPLICE OF BARS (i.e. 3'-8" FOR #4 BAR).



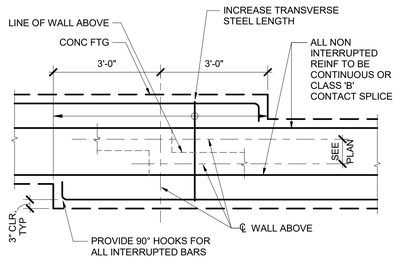


SAME SIZE, QUANTITY AND SPACING AS SCHEDULED REINF SEE PLAN

"S" TO BE:
8" FOR CMU WALLS, 16" MAX
12" FOR CONCRETE WALLS, 24" MAX UNO ON PLAN

5 S300 FOUNDATION INFLUENCE

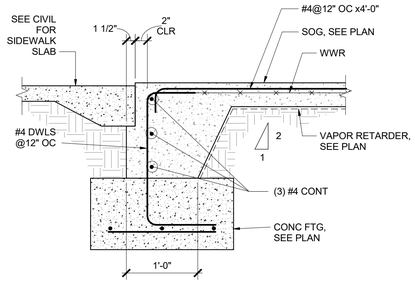
1" = 1'-0"



PLAN VIEW

6 S300 FOOTING / TIE BEAM CORNER REINFORCING DETAILS

1" = 1'-0"



7 S300 STEPPED FOOTING DETAIL

3/4" = 1'-0"

8 S300 FOOTING OFFSET DETAIL

1/2" = 1'-0"

9 S300 TURNED DOWN SLAB EDGE AT FOOTING

1" = 1'-0"

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 286 9004
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS

ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date

CONTRACT DOCUMENTS FOR:
SLPS C.A. MOORE GYMNASIUM
ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION

TYPICAL FOUNDATION DETAILS

PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W.JR/TEB

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

S300

WILLIAM JOHN RAIOLA
LICENSE
No. 80234
STATE OF FLORIDA
PROFESSIONAL ENGINEER
05/29/2026

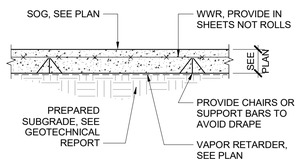
THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JEZERINAC GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

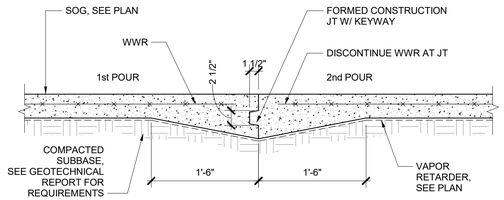
TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

5/29/2026 3:06:10 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR-JG-CA Moore_R26.rvt



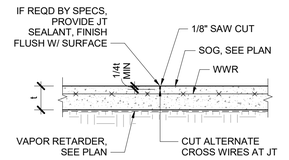
1 S301 WWR AT SOG

1" = 1'-0"



2 S301 CONSTRUCTION JOINT AT SOG

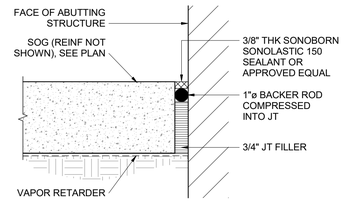
1" = 1'-0"



SAW CUT AS SOON AS POSSIBLE WITHOUT RAVELING CONCRETE (4 TO 6 HOURS AFTER POUR)

3 S301 CONTROL JOINT AT SOG (CJ)

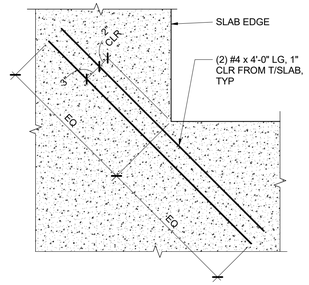
1" = 1'-0"



NOTES:
1. PROVIDE SEALANT FOR EXTERNAL JOINTS ONLY.
2. CLEAN AND PREPARE SURFACES TO RECEIVE SEALANT IN ACCORDANCE W/ MFR'S RECOMMENDATIONS.
3. GC SHALL USE NOMAFLEX OR REFLEX JT FILLER. IMPREGNATED ASPHALT FELT JT NOT PERMITTED.

4 S301 ISOLATION JOINT AT SOG (IJ)

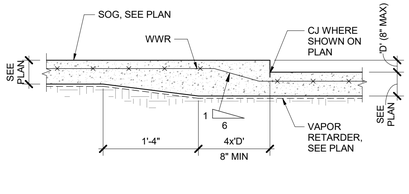
3" = 1'-0"



NOTE: DO NOT PLACE RE-ENTRANT BARS AT CONTROL JOINT LOCATIONS, UNLESS NOTED OTHERWISE

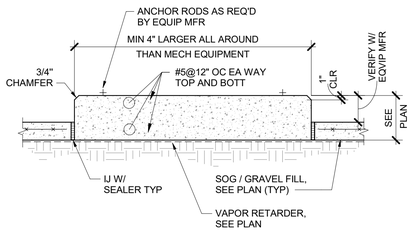
5 S301 SOG REINF AT RE-ENTRANT CORNERS

1" = 1'-0"



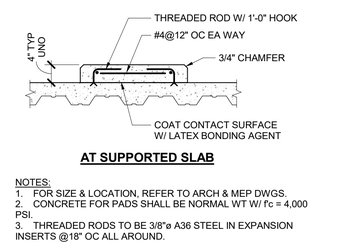
6 S301 DEPRESSED SOG

1" = 1'-0"



7 S301 EQUIPMENT PAD AT SOG

3/4" = 1'-0"

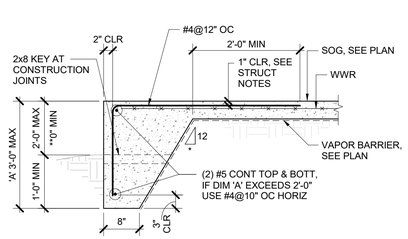


AT SUPPORTED SLAB

NOTES:
1. FOR SIZE & LOCATION, REFER TO ARCH & MEP DWGS.
2. CONCRETE FOR PADS SHALL BE NORMAL WT W/ f'c = 4,000 PSI.
3. THREADED RODS TO BE 3/8"ø A36 STEEL IN EXPANSION INSERTS @18" OC ALL AROUND.

10 S301 HOUSEKEEPING PAD AT ELEVATED SLAB

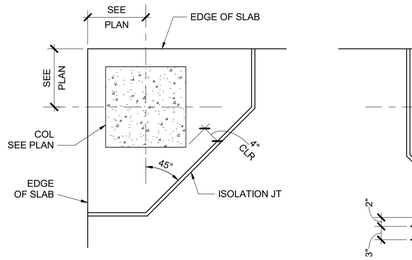
3/4" = 1'-0"



NOTES:
* AS REQUIRED NOT LESS THAN 6 UNLESS FORMED EDGE
** OR AS INDICATED ON ARCHL, COORDINATE W/ CIVIL

8 S301 TURNED DOWN SLAB EDGE

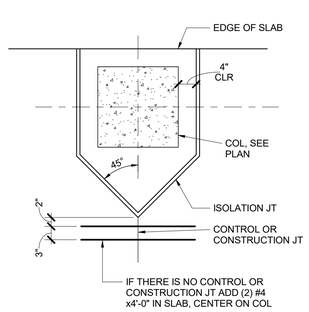
3/4" = 1'-0"



NOTE: EXTERIOR CMU WALLS NOT SHOWN FOR CLARITY

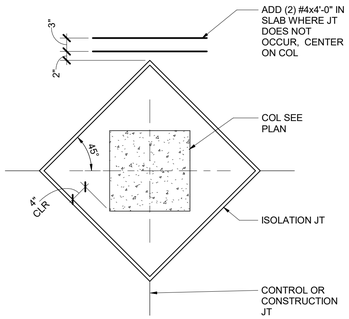
9 S301 ISOLATION JOINTS AT CONCRETE COLUMNS

3/4" = 1'-0"



CORNER

EDGE



INTERIOR

WILLIAM JOHN RAIOLA LICENSE No. 80234 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/29/2026

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JEZERINAC GROUP

480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 266 9004
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS
ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date

CONTRACT DOCUMENTS FOR:
SLPS C.A. MOORE GYMNASIUM
ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION

TYPICAL SLAB-ON-GROUND DETAILS

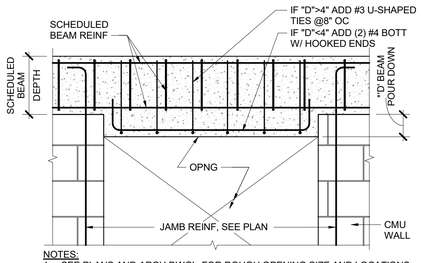
PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W.JR/TEB

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

S301

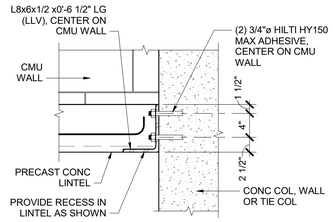
5/29/2026 3:58:19 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR_v25 C.A. Moore_R25.rvt



NOTES:
1. SEE PLANS AND ARCH DWG's FOR ROUGH OPENING SIZE AND LOCATIONS.
2. *IF 'D'>16", PROVIDE PRECAST LINTEL.

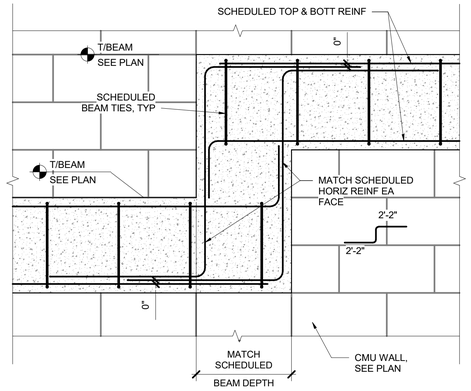
1 S330 CONCRETE BEAM POUR DOWN OVER OPENINGS

3/4" = 1'-0"



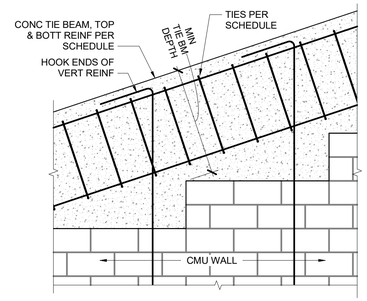
2 S330 PRECAST LINTEL SUPPORT AT CAST-IN-PLACE CONCRETE

1" = 1'-0"



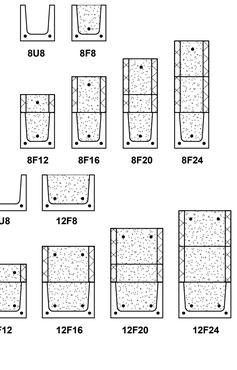
3 S330 STEP IN TIE BEAM IN CMU WALL

1" = 1'-0"



4 S330 CMU AT RAKED TIE BEAM

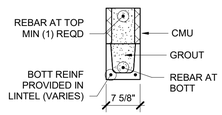
1/2" = 1'-0"



LINTEL SCHEDULE

LINTEL SPAN	8" CMU WALL	12" CMU WALL	
	LINTEL DESIGNATION	LINTEL DESIGNATION	REMARKS
UP TO 4'-0"	8 F8-1B	12 F8-2B	-
FROM 4'-1" TO 6'-0"	8 F8-1B/1T	12 F8-2B/2T	-
FROM 6'-1" TO 8'-0"	8 F16-1B/1T	12 F16-2B/2T	-
FROM 8'-1" TO 10'-0"	8 F16-2B/2T	12 F16-2B/2T	-
FROM 10'-1" TO 13'-0"	8 F24-2B/2T	12 F24-2B/2T	-
OVER 13'-0"	CAST IN PLACE BEAM	CAST IN PLACE BEAM	SEE PLANS

NOTES:
1. LINTELS SHALL BE "CAST-CRETE" OR EQUIVALENT PRECAST CONCRETE UNO.
2. LINTELS SHALL BEAR 8" MIN ON CMU/CONCRETE AND 3" MINIMUM ON STEEL ANGLES.

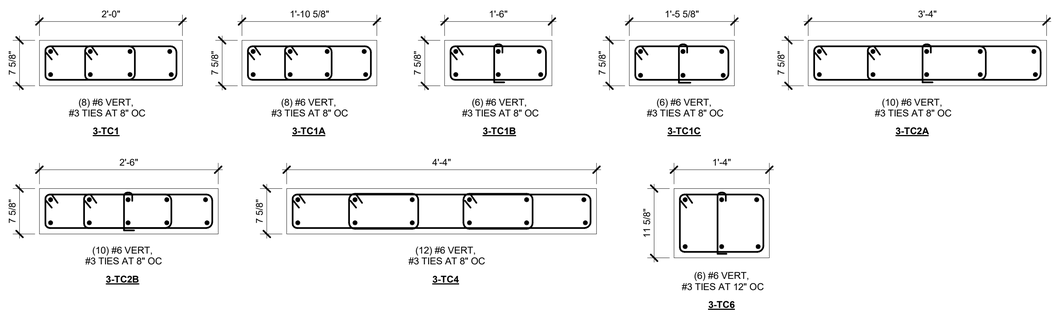


TYPE DESIGNATION

F = FILLED W/ GROUT
U = UNFILLED
8F16-1B/1T
NOMINAL WIDTH
NOMINAL HEIGHT
QTY OF #5 REBAR AT TOP
QTY OF #5 REBAR AT BOTT

5 S330 PRECAST LINTEL SCHEDULE AND DETAILS

3/4" = 1'-0"



NOTES:
1. *34 BAR DIA PLUS WALL THICKNESS LESS 2" OR 3'-0" (MINIMUM), WHICHEVER IS GREATER.
2. *34 BAR DIA PLUS 1/2 WALL THICKNESS OR 2'-6" (MINIMUM), WHICHEVER IS GREATER.
3. SEE PLAN FOR WALL THICKNESS AND REINFORCING SIZE AND SPACING.
4. NOT APPLICABLE AT SHEARWALLS. FOR SHEARWALL REINFORCING, SEE PLANS AND SCHEDULES.

6 S330 REINFORCING AT WALL ENDS, CORNERS & INTERSECTIONS - BARS ON ONE FACE

3/4" = 1'-0"

7 S330 COLUMN SCHEDULE

1" = 1'-0"

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 286 9004
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS

ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date

CONTRACT DOCUMENTS FOR:
SLPS C.A. MOORE GYMNASIUM
ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION

TYPICAL CONCRETE FRAMING DETAILS

PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W.JR/TEB

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

S330

WILLIAM JOHN RAIOLA LICENSE No. 80234 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/29/2026

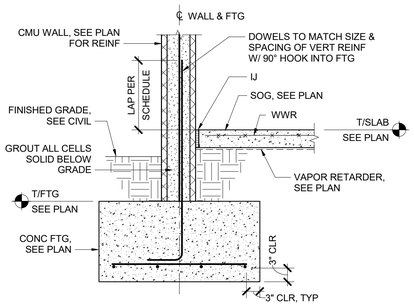
THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

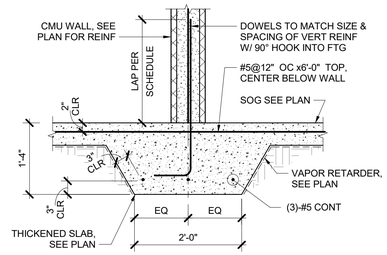
JEZERINAC GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

5/29/2026 3:58:11 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR-JG-CA Moore_R26.rvt



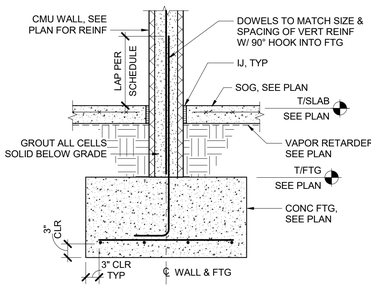
1 S401 CMU AT EXTERIOR WALL FOOTING

3/4" = 1'-0"



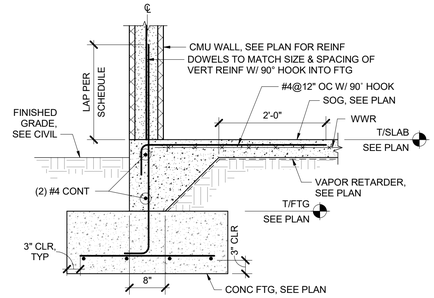
2 S401 CMU AT THICKENED SLAB FOOTING

3/4" = 1'-0"



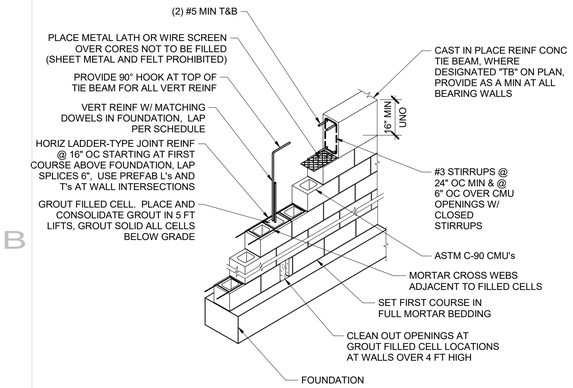
3 S401 CMU AT INTERIOR WALL FOOTING

3/4" = 1'-0"



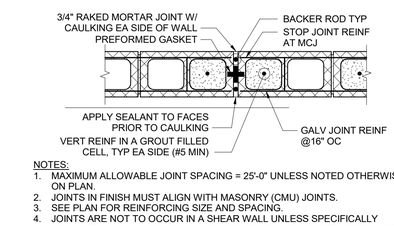
4 S401 CMU AT CONCRETE POURDOWN

3/4" = 1'-0"



5 S401 TYP MASONRY (CMU) WALL CONSTRUCTION

3/8" = 1'-0"

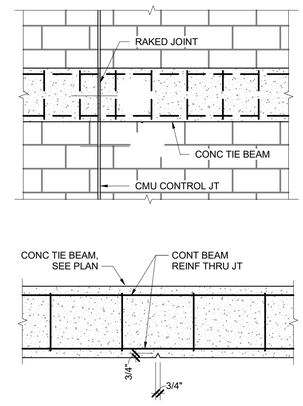


NOTES:

1. MAXIMUM ALLOWABLE JOINT SPACING = 25'-0" UNLESS NOTED OTHERWISE ON PLAN.
2. JOINTS IN FINISH MUST ALIGN WITH MASONRY (CMU) JOINTS.
3. SEE PLAN FOR REINFORCING SIZE AND SPACING.
4. JOINTS ARE NOT TO OCCUR IN A SHEAR WALL UNLESS SPECIFICALLY DETAILED ON PLAN. CONTACT THE STRUCTURAL ENGINEER OF RECORD IN THE EVENT THAT A JOINT FALLS WITHIN A SHEAR WALL.
5. DO NOT PLACE MASONRY CONTROL JOINTS WITHIN 24" OF LOAD BEARING PILASTERS OR OPENINGS.
6. CONTRACTOR TO VERIFY CONTROL JOINT LOCATIONS WITH PROJECT ARCHITECT.

6 S401 MASONRY CONTROL JOINT (MCJ) REINFORCING

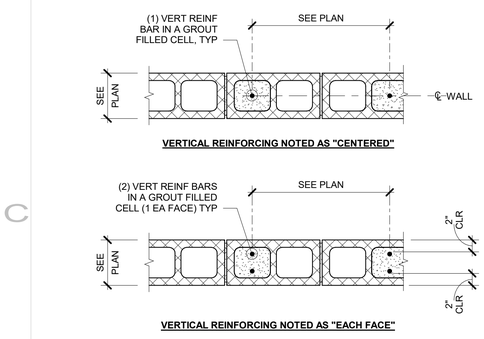
1" = 1'-0"



7 S401 CMU VERTICAL REINFORCING LAP SCHEDULE

1" = 1'-0"

SCHEDULE		
WALL THICKNESS	**BAR**	**LAP**
8"	#4	1'-8"
8"	#5	2'-1"
8"	#6	3'-7"
8"	#7	4'-11"
12"	#4	1'-8"
12"	#5	2'-1"
12"	#6	3'-3"
12"	#7	3'-10"



NOTE: SEE PLAN AND CMU WALL SCHEDULE FOR REINFORCING SIZE, SPACING AND ADDITIONAL INFORMATION.

8 S401 CMU BAR PLACEMENT REQUIREMENTS

1" = 1'-0"

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 286 9504
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS

ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date

CONTRACT DOCUMENTS FOR:

SLPS C.A. MOORE GYMNASIUM

ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION

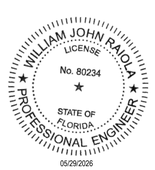
TYPICAL MASONRY FOUNDATION DETAILS

PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W.JR/TEB

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

S401



05/29/2026

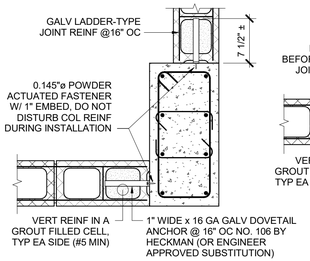
THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

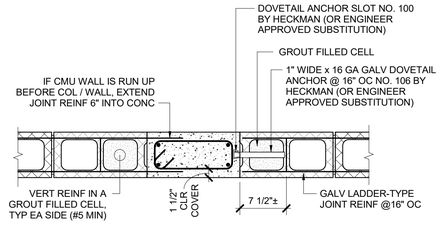
JEZERINAC
GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

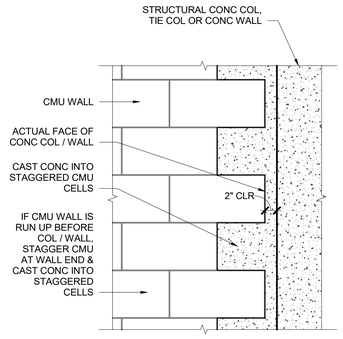
5/29/2026 3:58:12 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR-JG-CA Moore_R26.rvt



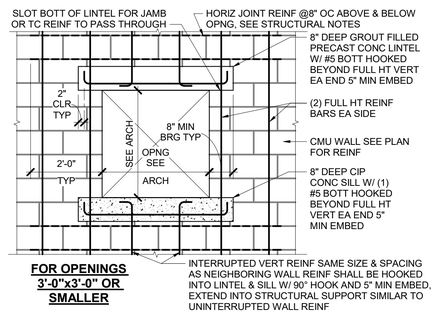
STRUCTURAL COLUMN / CONCRETE WALL



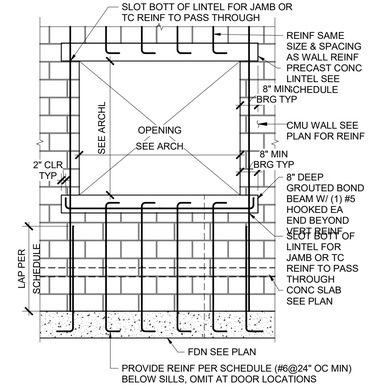
TIE COLUMN / CONCRETE WALL



ALTERNATE CONNECTION

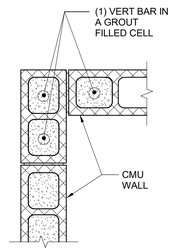


FOR OPENINGS 3'-0"x3'-0" OR SMALLER

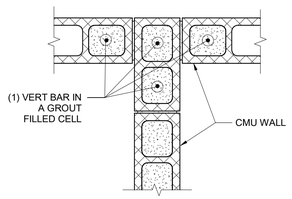


1 S402 CMU ANCHORAGE TO CONCRETE

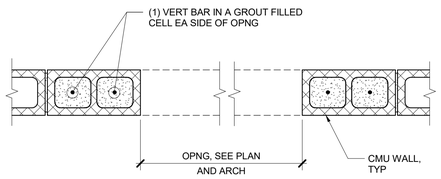
1" = 1'-0"



PLAN AT CORNER



PLAN AT INTERSECTION

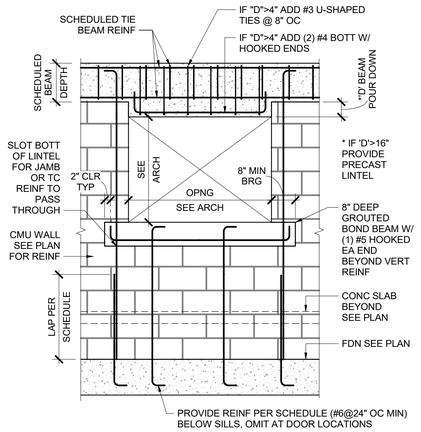


PLAN AT OPENING

NOTES:
1. SEE TYP MASONRY WALL CONSTRUCTION FOR JOINT REINFORCING NOT SHOWN HERE.
2. SEE PLAN FOR REINFORCING SIZE.

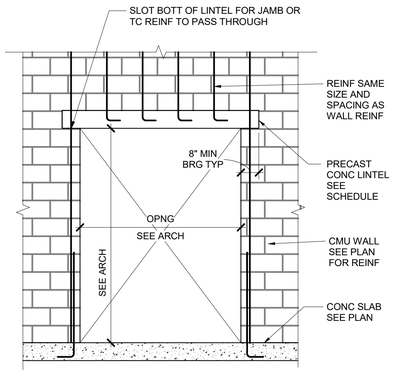
2 S402 MINOR CMU WALL OPNGS

1/2" = 1'-0"



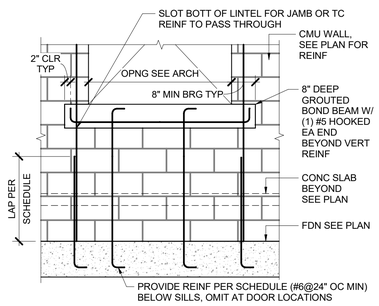
3 S402 CMU WINDOW OPNG W/ PRECAST LINTEL & BOND BM SILL

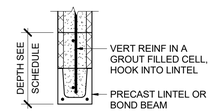
3/8" = 1'-0"



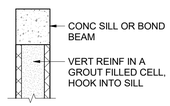
4 S402 REINFORCING AT CMU WALL CORNERS, INTERSECTIONS AND OPENINGS

1" = 1'-0"

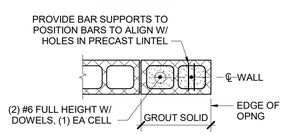




TYPICAL HEAD



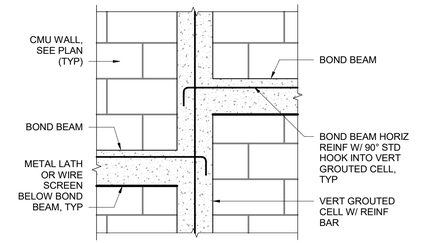
TYPICAL SILL



TYPICAL JAMB

5 S402 CMU WALL OPNG AT CONC TIE BEAM

1/2" = 1'-0"



NOTE: WHEN BOND BEAM HEIGHT MUST BE CHANGED WITHIN A WALL, SUCH AS TO ACCOMODATE OPENINGS OR A CHANGE IN BUILDING HEIGHT, THIS DETAIL WILL HELP TO ENSURE STRUCTURAL CONTINUITY. SEE TYPICAL SCHEDULE FOR STANDARD (STD) HOOK LENGTHS.

6 S402 CMU WALL OPNG W/ PRECAST LINTEL

3/8" = 1'-0"

7 S402 CMU WALL OPNG W/ BOND BEAM SILL

1/2" = 1'-0"

8 S402 CMU WALL HEAD, JAMB AND SILL DETAIL

3/4" = 1'-0"

9 S402 STEP IN BOND BEAM IN CMU WALL

3/4" = 1'-0"

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 286 9004
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS

ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date

CONTRACT DOCUMENTS FOR:

SLPS C.A. MOORE GYMNASIUM

ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION

TYPICAL MASONRY DETAILS

PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W./R/TEB

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

S402

WILLIAM JOHN RAIOLA LICENSE No. 80234 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/29/2026

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

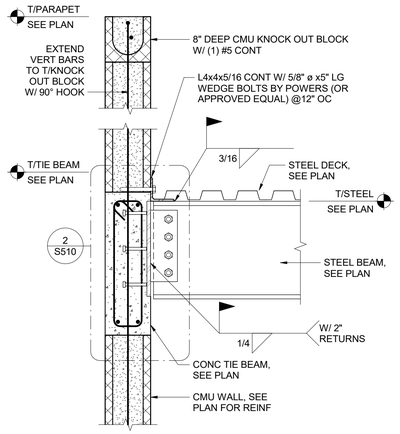
PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

JEZERINAC GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785

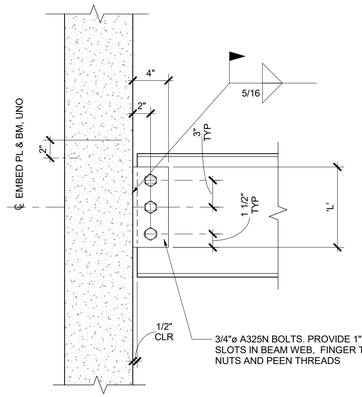
JG Project #: 25.02.027

5/29/2026 3:08:12 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR-JG-CA Moore_R26.rvt



1 S510 STEEL BEAM TO CONCRETE BEAM

1" = 1'-0"

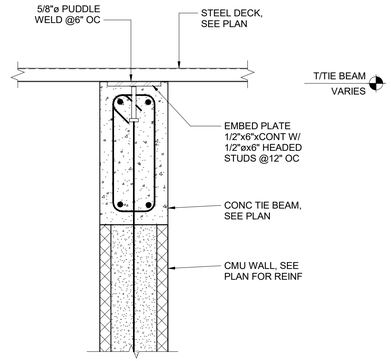


2 S510 STEEL BEAM TO CONCRETE CONNECTION SCHEDULE

1 1/2" = 1'-0"

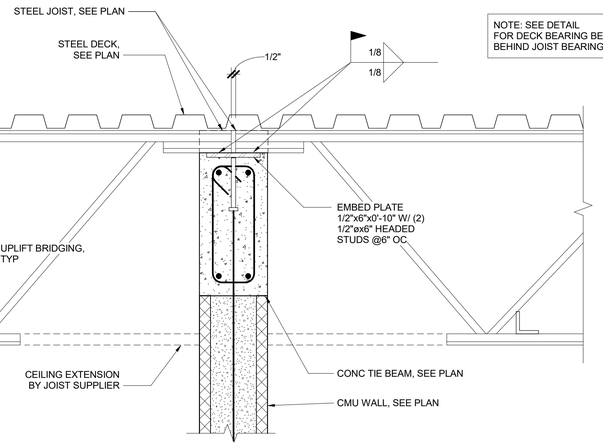
SCHEDULE					
BEAM SIZE	# ROWS OF BOLTS	EMBED PLATE (PL)		GUSSET PL THICKNESS	'L'
		PL*	STUDS		
W8, W10	2	5/8x10x0'-10"	(4) 3/4"ø x4 7/8" LG	3/8"	6"
W12, W14	3	5/8x10x1'-2"	(6) 3/4"ø x4 7/8" LG	3/8"	9"

*LONG DIMENSION VERTICAL



3 S510 DECK BRG OVER CMU WALL

1 1/2" = 1'-0"

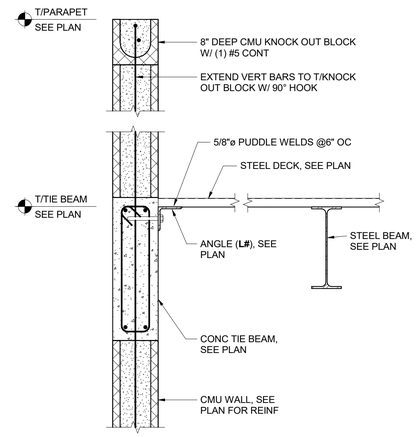


NOTE: SEE DETAIL 4 / S511 FOR DECK BEARING BEYOND AND BEHIND JOIST BEARING.

NOTE: MINIMUM BEARING ON CONCRETE BEAM = 4". STAGGER THE JOISTS WHERE NEEDED TO ACHIEVE MIN. BEARING.

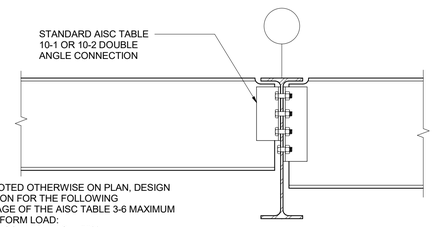
4 S510 JOIST BRG ON CMU WALL

1 1/2" = 1'-0"



5 S510 DECK SUPPORT AT CONCRETE TIE BEAM

1" = 1'-0"

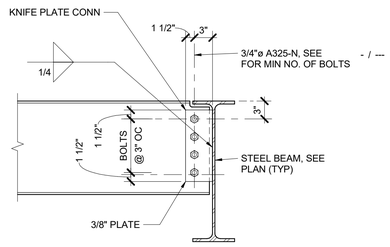


UNLESS NOTED OTHERWISE ON PLAN, DESIGN CONNECTION FOR THE FOLLOWING PERCENTAGE OF THE AISC TABLE 3-6 MAXIMUM TOTAL UNIFORM LOAD:
NON-COMPOSITE BEAMS: 50%
COMPOSITE BEAMS: 75%
MINIMUM DESIGN LOAD: 16 KIPS (FACTORED)

NOTES:
1. SEE [illegible] FOR MINIMUM NUMBER OF BOLTS.
2. DETAIL CONNECTIONS TO COMPLY W/ OSHA SAFETY AND HEALTH STANDARDS FOR THE CONSTRUCTION INDUSTRY, 29 CFR 1926 PART R SAFETY STANDARDS FOR STEEL ERECTION.

6 S510 WF BEAM TO BEAM CONN

1" = 1'-0"



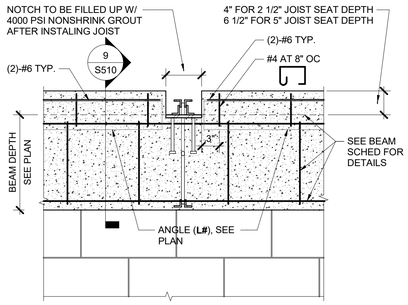
STANDARD AISC TABLE 10-9a CONNECTION

NOTE: SHORT-SLOTTED HOLES ARE PERMITTED IN BEAM WEB

UNLESS NOTED OTHERWISE ON PLAN, DESIGN CONNECTION FOR THE FOLLOWING PERCENTAGE OF THE AISC TABLE 3-6 MAXIMUM TOTAL UNIFORM LOAD:
NON-COMPOSITE BEAMS: 50%
COMPOSITE BEAMS: 75%
MINIMUM DESIGN LOAD: 16 KIPS (FACTORED)

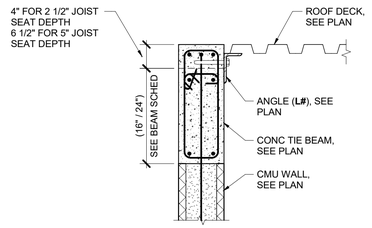
7 S510 ALTERNATE WF BEAM TO BEAM CONN

1" = 1'-0"



8 S510 SECTION

1" = 1'-0"



9 S510 SECTION

1" = 1'-0"

WILLIAM JOHN RAIOLA LICENSE No. 80234 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/29/2026

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

JEZERINAC GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 286 9004
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS

ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date

CONTRACT DOCUMENTS FOR:
SLPS C.A. MOORE GYMNASIUM
ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION

TYPICAL STEEL DETAILS

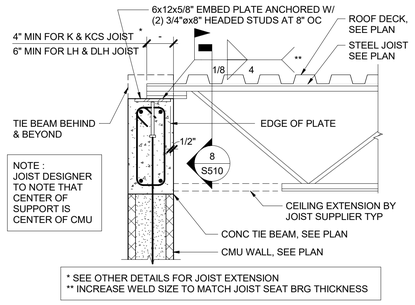
PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W.JR/TEB

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

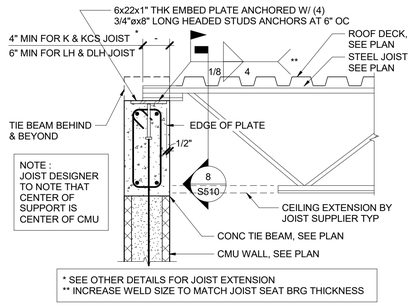
S510

5/29/2026 3:06:13 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR-JG-CA Moore_R26.rvt



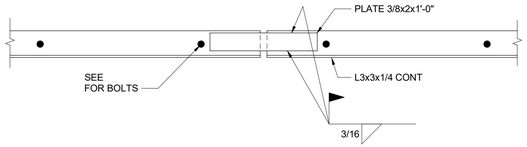
1 S511 JOIST END BRG ON CMU WALL AT LOW ROOF

1" = 1'-0"



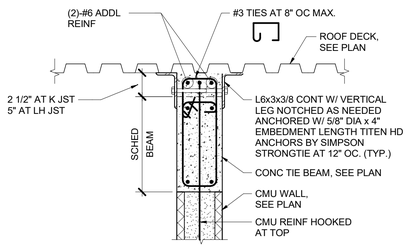
2 S511 JOIST END BRG ON CMU WALL AT HIGH ROOF

1" = 1'-0"



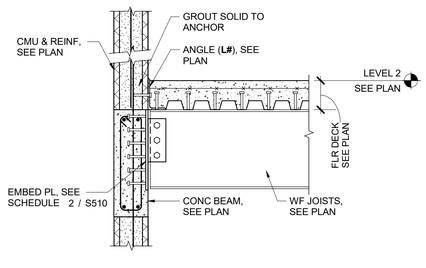
3 S511 EDGE ANGLE SPLICE DETAIL

1 1/2" = 1'-0"



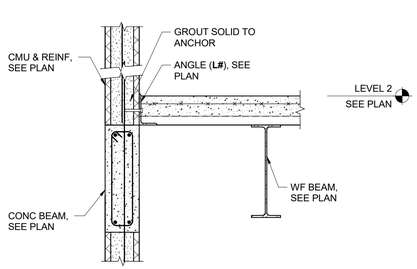
4 S511 DECK BEARING DETAIL AT INTERNAL BEARING WALL

1" = 1'-0"



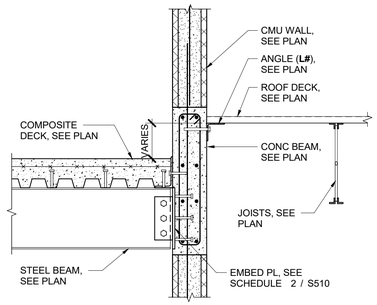
5 S511 BEAM SUPPORT AT CMU WALL

3/4" = 1'-0"



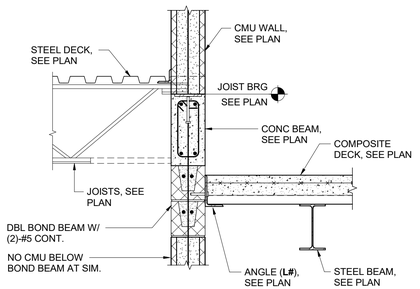
6 S511 STEEL CONN AT CONCRETE TIE BEAM

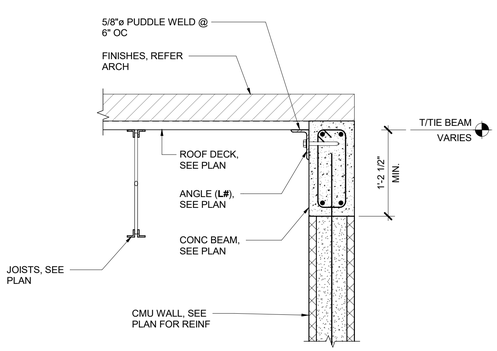
3/4" = 1'-0"



7 S511 STEEL CONN AT CMU WALL

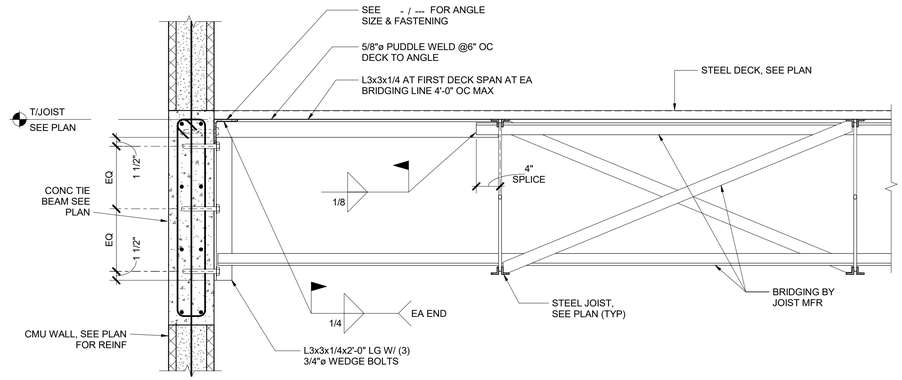
3/4" = 1'-0"





8 S511 SECTION

1" = 1'-0"



9 S511 BRIDGING TERMINATION AT CONC BEAM

1" = 1'-0"

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 266 9504
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS

ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date

CONTRACT DOCUMENTS FOR:
SLPS C.A. MOORE GYMNASIUM
ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION

STEEL DETAILS

PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W.JR/TEB

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

S511

WILLIAM JOHN RAIOLA LICENSE No. 80234 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/29/2026

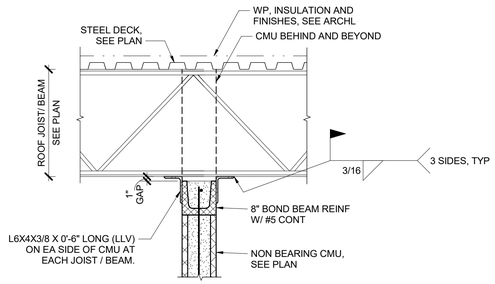
THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JEZERINAC GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

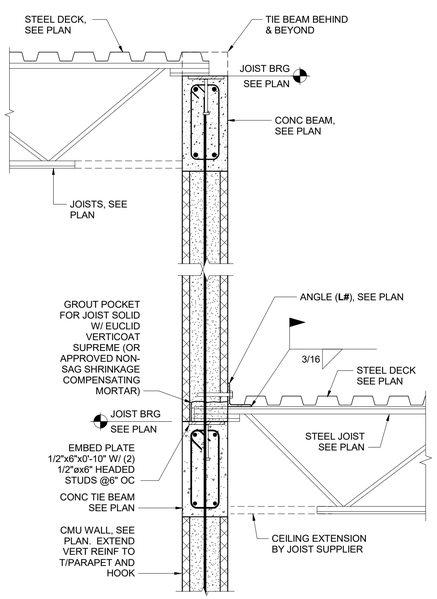
TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

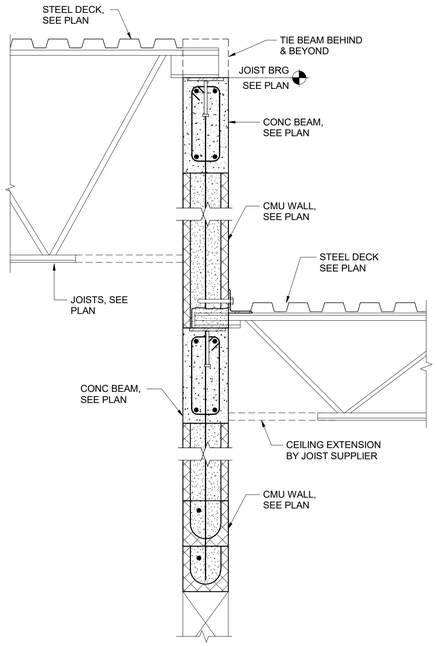
5/29/2026 3:08:14 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR-JO-CA Moore_R26.rvt

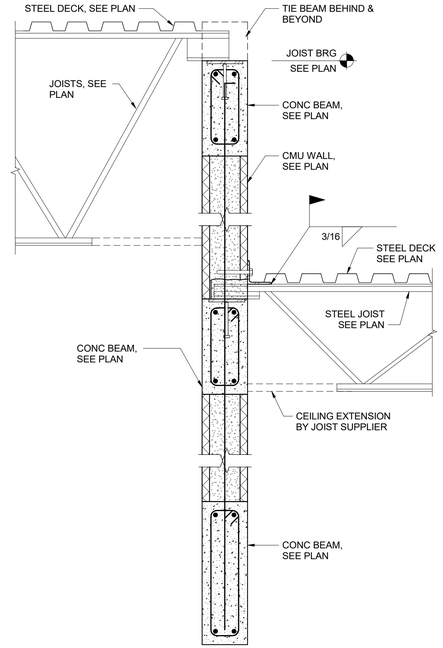


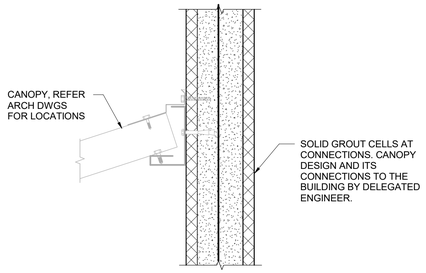
5 S512 BRACING AT NON-BEARING PARTITION WALL, PERPENDICULAR TO BEAM/ JOIST

3/4" = 1'-0"









4 S512 CANOPY AT CMU WALL

1 1/2" = 1'-0"

2 S512 JOIST END BRG AT CMU SHEAR WALL

1" = 1'-0"

3 S512 SECTION

1" = 1'-0"

1 S512 SECTION

1" = 1'-0"

WILLIAM JOHN RAIOLA LICENSE No. 80234 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/29/2026

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JEZERINAC GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 266 9004
www. ADC-ARCHITECTS.NET

REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS

ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date
1	Revision 1	Date 1

CONTRACT DOCUMENTS FOR:
SLPS C.A. MOORE GYMNASIUM
ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION
STEEL DETAILS

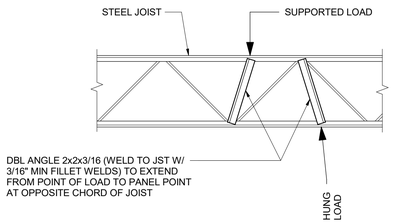
PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W.IR/TEB

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

S512

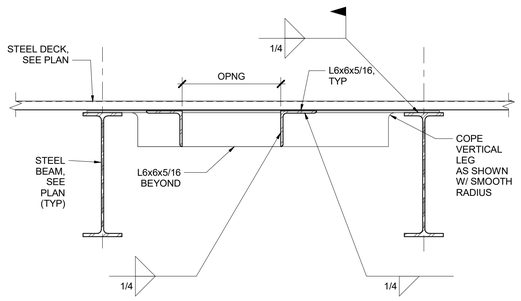
5/29/2026 3:08:14 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR-JG-CA Moore_R26.rvt



NOTES:
1. PROVIDE REINFORCING AT SUPPORTED OR HUNG LOADS >300#.
2. DETAIL NOT REQUIRED IF LOAD IS APPLIED DIRECTLY TO JOIST PANEL POINT.
3. COMPLY WITH JOIST MANUFACTURER'S REQUIREMENTS.

1 S513 JOIST REINFORCING AT POINT LOADS

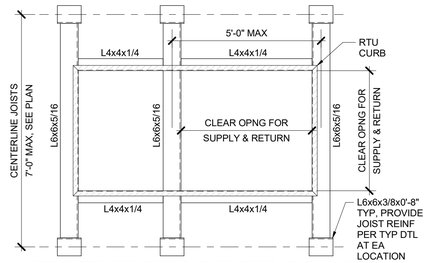
1" = 1'-0"



NOTE: PROVIDE ABOVE FRAMING AROUND ALL OPENINGS INDICATED ON STRUCTURAL, ARCHITECTURAL, AND MECH DRAWINGS. CONTRACTOR IS RESPONSIBLE FOR COORDINATING OPENING SIZES W/ PURCHASED EQUIPMENT.

2 S513 ROOF OPENING DETAIL

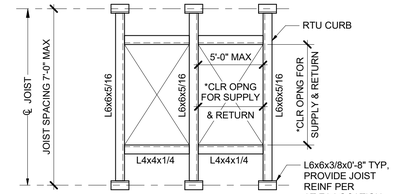
1" = 1'-0"



NOTE: COORDINATE DIMENSIONS WITH EQUIPMENT ACTUALLY FURNISHED. IF RTU UNIT, OPENING TO BE SAME AS EQUIPMENT CURB SIZE.

4 S513 ROOF OPENING/EQUIPMENT SUPPORT FRAME

1/2" = 1'-0"

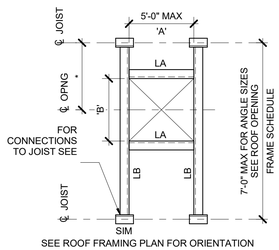


* COORDINATE DIMENSIONS W/ EQUIP ACTUALLY FURNISHED. IF RTU UNIT, OPNG TO BE SAME SIZE AS EQUIP CURB SIZE.

NOTES:
1. COORDINATE SIZE AND LOCATION OF OPENINGS W/ MEP DRAWINGS AND EQUIPMENT REQUIREMENTS.
2. FOR JOIST REINFORCING SEE 1 / S513.
3. FOR EQUIPMENT SUPPORT FRAME CONNECTION DETAILS SEE 7 / S513.

5 S513 ROOF OPENING / EQUIPMENT SUPPORT FRAME

3/4" = 1'-0"



SEE ROOF FRAMING PLAN FOR ORIENTATION

ROOF OPENING FRAME SCHEDULE			
DIM 'A'	DIM 'B'	L-SIZE 'A'	L-SIZE 'B'
2'-0"	4'-0"	L3x3x1/4	L3x3x1/4
3'-0"	5'-0"	L3x3x1/4	L3x3x5/16
4'-0"	6'-0"	L3x3x1/4	L4x4x5/16
5'-0"	7'-0"	L3x3x1/4	L6x6x5/16

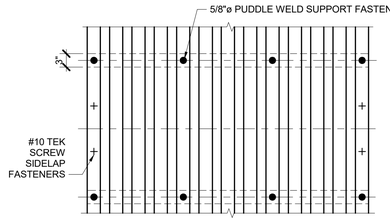
NOTE: WHEN JOIST BRIDGING INTERFERES W/ ROOF OPNG FRAMES, STOP BRIDGING AT EA SIDE OF OPNG AND 'X' BRIDGE LAST BRIDGING SPACE EA SIDE OF OPNG. ADD ADDL BRIDGING W/ 'X' BRIDGING AND HORIZONTAL BRIDGING ON EA SIDE OF CUT BRIDGING AREA. EXTEND ADDED BRIDGING PAST CUT BRIDGING

* COORDINATE DIMENSIONS W/ EQUIP ACTUALLY FURNISHED. IF RTU UNIT, OPNG TO BE SAME SIZE AS EQUIP CURB SIZE.

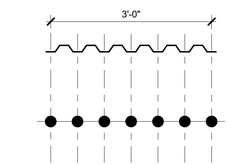
NOTES:
1. COORDINATE SIZE AND LOCATION OF OPENINGS W/ MEP DRAWINGS AND EQUIPMENT REQUIREMENTS.
2. FOR JOIST REINFORCING SEE 1 / S513.

3 S513 ROOF OPENING / EQUIP SUPPORT FRAME

3/4" = 1'-0"



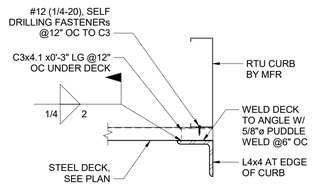
SCHEDULE		
MARK	# OF WELDS AT SUPPORTS	# OF SIDE LAP FASTENERS BETWEEN SUPPORTS
36/7	(7) 5/8" DIA WELDS	(10) SIDELAP #10 SCREWS



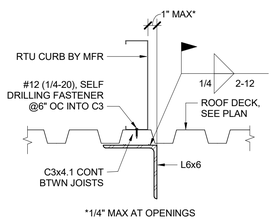
NOTES:
1. DECK SHALL BE ATTACHED TO ALL STRUCTURAL SUPPORTS W/ 5/8"ø PUDDLE WELDS.
2. SIDE LAPS BETWEEN STRUCTURAL SUPPORTS SHALL BE FASTENED BY #10 TEK SCREWS.
3. DECK SHALL BE ATTACHED TO ALL PERIMETER SUPPORTS WITH 5/8"ø PUDDLE WELDS AT 6" OC MAX.
4. END LAPS SHALL BE A MINIMUM OF 3" AND SHALL OCCUR OVER SUPPORTS.

8 S513 ROOF DECK ATTACHMENT SCHEDULE

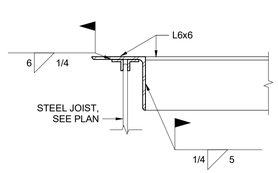
3/4" = 1'-0"



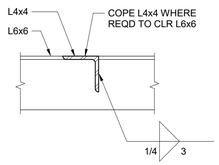
CURB AND DECK SUPPORT



CURB AT SIDE DECK BEARING



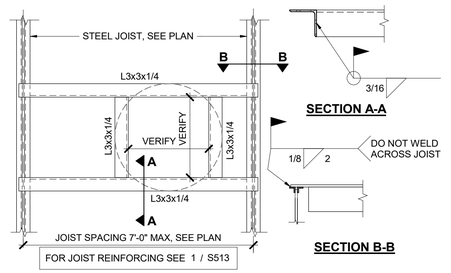
ANGLE TO JOIST CONNECTION



ANGLE TO ANGLE CONNECTION

7 S513 ROOF OPENING/EQUIPMENT SUPPORT FRAME CONNECTION DETAILS

1 1/2" = 1'-0"



6 S513 ROOF DRAIN SUPPORT FRAMING AT STEEL JOISTS

3/4" = 1'-0"

WILLIAM JOHN RAJOLA LICENSE No. 80234 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/29/2026

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAJOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

JEZERINAC GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 286 9004
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS

ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date

CONTRACT DOCUMENTS FOR:
SLPS C.A. MOORE GYMNASIUM
ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION

TYPICAL ROOF DETAILS

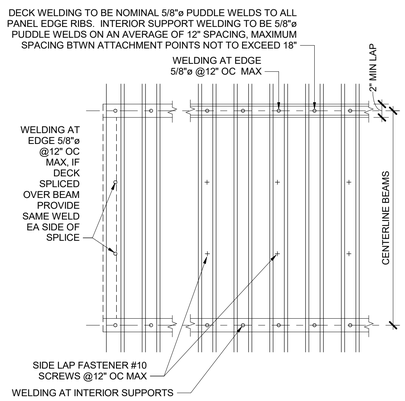
PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W./PJT/EB

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

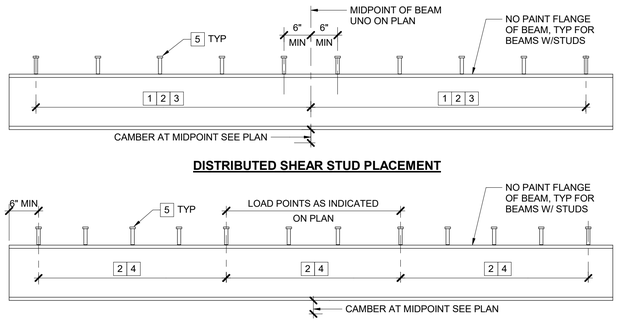
S513

5/29/2026 3:08:15 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR-JG-CA Moore_R26.rvt



1 S530 COMPOSITE FLOOR DECK FASTENING

3/4" = 1'-0"

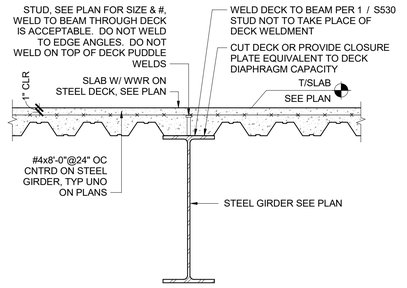


SHEAR STUD PLACEMENT

1. EQUALLY DISTRIBUTE ONE HALF OF SHEAR CONNECTORS INDICATED ON PLAN EACH SIDE OF BEAM MIDPOINT OR LOAD POINTS INDICATED ON PLANS, STARTING FROM EACH END OF SPAN. SEE PLAN FOR NUMBER OF STUDS.
2. SPACING OF SHEAR CONNECTORS SHALL NOT EXCEED 8 TIMES THE TOTAL SLAB THICKNESS AND NO GREATER THAN 36" OC. SPACING SHALL BE NO CLOSER THAN 6x STUD DIAMETER OR DISTANCE BETWEEN FLUTES FOR STEEL DECKS WHEN DECK FLUTE IS PERPENDICULAR TO BEAM. COORDINATE STUD LAYOUT WITH DECK SECTIONS USED.
3. DOUBLE OR TRIPLE STUDS IF REQUIRED. DOUBLE AND TRIPLE STUD PLACEMENT TO BE SYMMETRICAL ABOUT BEAM WEB. MAINTAIN 4x STUD DIAMETER TRANSVERSE SPACING BETWEEN STUDS IN PARALLEL ROWS, WITH A MINIMUM EDGE DISTANCE OF 1 1/2 STUD DIAMETERS, 1" MINIMUM.
4. EQUALLY DISTRIBUTE SHEAR CONNECTORS INDICATED ON PLANS FOR EACH GIRDER SEGMENT BETWEEN LOAD POINTS (BEAMS WHICH FRAME INTO GIRDER).
5. SEE PLAN FOR STUD SIZE. NUMBER OF STUDS REQUIRED PER BEAM IS INDICATED BY (#).

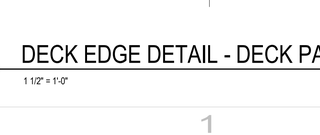
2 S530 COMPOSITE STEEL BEAM STUD PLACEMENT

3/4" = 1'-0"



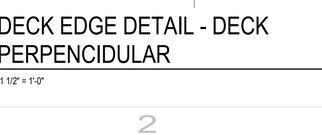
3 S530 COMPOSITE DECK AT GIRDER CONNECTION

3/4" = 1'-0"



4 S530 DECK EDGE DETAIL - DECK PARALLEL

3/4" = 1'-0"



5 S530 DECK EDGE DETAIL - DECK PERPENCIDULAR

3/4" = 1'-0"

(SIMILAR WHERE DECK SPANS IN OPPOSITE DIRECTION)

6 S530 DEPRESSED SLAB AT STEEL DECK

3/4" = 1'-0"

NOTE: MAX CONTINUOUS POUR = 100 FT IN ANY DIRECTION,

7 S530 COMPOSITE FLOOR CONSTRUCTION JOINT

1" = 1'-0"

'E'	GAGE (GA)
2"	18
3"	16
4" & 5"	14
6" & 7"	12
8" & 9"	10

8 S530 DECK EDGE DETAIL - DECK PARALLEL

1 1/2" = 1'-0"

9 S530 DECK EDGE DETAIL - DECK PERPENCIDULAR

1 1/2" = 1'-0"

VERTICAL REACTIONS FROM COOLING TOWERs, RTU

		LATERAL WIND		UPLIFT
	EQUIPMENT + CURB	WIND - X	WIND - Y	
P1	+535	-1445	+1285	-1230
P2	+380	+1445	+428	-610
P3	+535	-1445	-1285	-1230
P4	+380	+1445	-428	-610

NOTES:
1. ALL REACTION FORCES ARE IN LBS.
2. WIND REACTIONS ARE FOR ALLOWABLE STRESS DESIGN. USE APPROPRIATE LOAD FACTORS IN COMBINATIONS.
3. REACTIONS DUE TO LATERAL WIND ARE REVERSIBLE. DELEGATED ENGINEER FOR TRUSSES/ JOISTS TO DESIGN FOR THE FORCE REVERSAL IN COMBINATION WITH ALL THE OTHER FORCES ACTING ON THE TRUSSES/JOISTS INCLUDING THE UPLIFT FORCES ABOVE.

10 S530 SCHEDULE

1/8" = 1'-0"

WILLIAM JOHN RAIOLA LICENSE No. 80234 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/29/2026

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JEZERINAC GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 286 9004
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS

ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date

CONTRACT DOCUMENTS FOR:
SLPS C.A. MOORE GYMNASIUM
ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION

COMPOSITE STEEL DETAILS

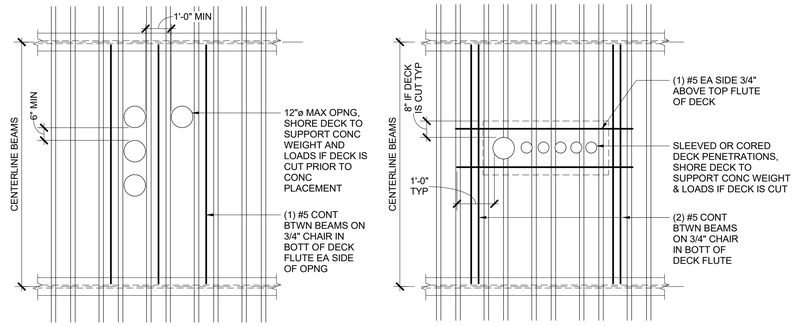
PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W.JR/TEB

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

S530

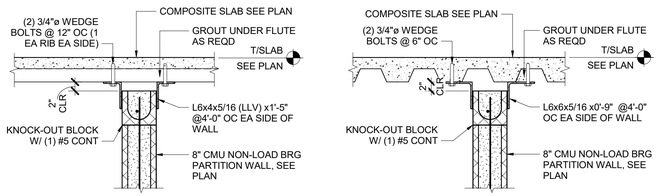
5/29/2026 3:08:16 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR-JG-CA Moore_R26.rvt



ISOLATED OPENINGS **MULTIPLE OPENINGS**

1 S531 COMPOSITE SLAB PIPE PENETRATION DETAIL

1" = 1'-0"

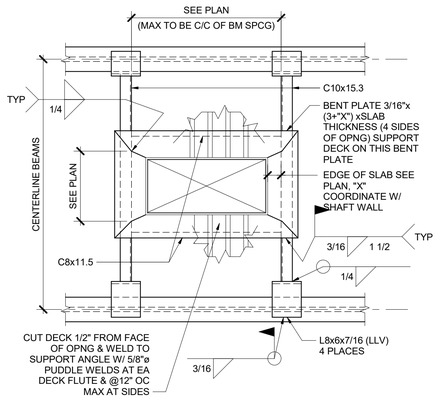


DECK PERPENDICULAR **DECK PARALLEL**

NOTE: DO NOT ATTACH ANGLES TO CMU PARTITION WALL.

4 S531 BRACING AT CMU NON LOAD BEARING PARTITION WALL

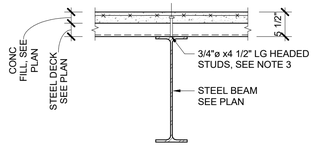
3/4" = 1'-0"



NOTE: "C" BEAMS SHOWN NOT INTENDED FOR SUPPORT OF CMU

2 S531 COMPOSITE SLAB OPENING DETAIL

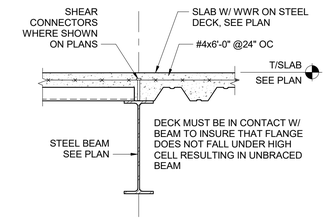
3/4" = 1'-0"



NOTES:
1. SEE PLAN FOR STEEL DECK DPTH, TYPE, GALVANIZINF REQUIREMENTS , CONCRETE THICKNESS AND TOTAL DEPTH.
2. 28 DAY COMPRESSIVE STRENGTH OF CONCRETE F'c=3,000 PSI.
3. QUANTITY OF STUDS ARE NOTED THUS [#] ON PLAN.
4. FASTEN FLOOR DECK TO SUPPORTS WITH 5/8"ø PUDDLE WELDS @12" OC. CONNECT SIDELAPS WITH #10 TEK SCREWS @3'-0" OC.

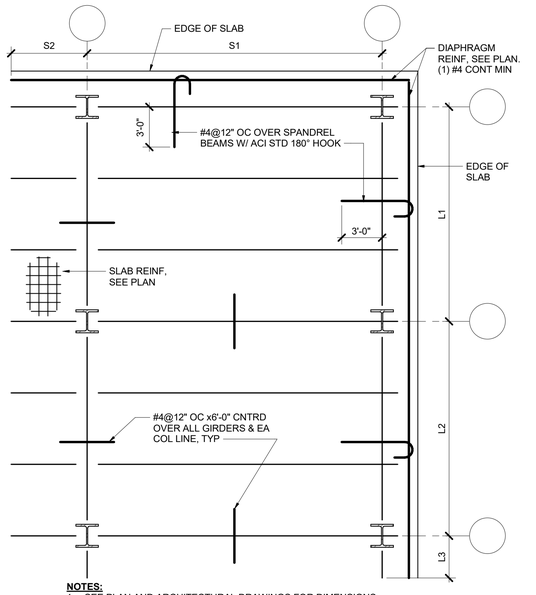
5 S531 COMPOSITE SLAB / BEAM DETAIL

3/4" = 1'-0"



3 S531 DECK TRANSITION AT STEEL BEAM

3/4" = 1'-0"



NOTES:
1. SEE PLAN AND ARCHITECTURAL DRAWINGS FOR DIMENSIONS.
2. SEE TYPICAL COMPOSITE SLAB SECTIONS FOR REINFORCING PLACEMENT.
3. REINFORCING AT INTERIOR OPENINGS SHALL BE SIMILAR TO SPANDREL BEAMS / GIRDERS.
4. SEE LOADING PLANS ON SHEET ********** FOR ALTERNATE REINFORCING AT HEAVILY LOADED AREAS.

6 S531 COMPOSITE SLAB REINFORCING AT COLUMN LINES AND EDGE OF SLAB

3/4" = 1'-0"

WILLIAM JOHN RAIOLA LICENSE No. 80234 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/29/2026

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM J. RAIOLA P.E. ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

JEZERINAC GROUP
480 HIBISCUS STREET, SUITE 107
WEST PALM BEACH, FL 33401
T 561.622.8585
www.jezerinacgroup.com
CERTIFICATE OF AUTHORIZATION FL #30785
JG Project #: 25.02.027

TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE BUILDING CODES AND MATERIAL SPECIFICATIONS.

ARCHITECTS DESIGN COLLABORATIVE, LLC
ARCHITECTURE | URBAN DESIGN
PLANNING | INTERIORS
10489 SOUTHWEST MEETING ST
TRADITION,
PORT ST. LUCIE, FL 34987
TEL. (772) 286 9004
www. ADC-ARCHITECTS.NET
REGISTRATIONS HELD IN THE FOLLOWING STATES:
CALIFORNIA, COLORADO, FLORIDA
GEORGIA, NORTH CAROLINA
MISSISSIPPI, TEXAS

ARCHITECTURE
URBAN DESIGN
PLANNING INTERIORS

SEAL

DIGITAL SIGNATURE

REVISIONS:

No.	Description	Date

CONTRACT DOCUMENTS FOR:
SLPS C.A. MOORE GYMNASIUM
ADDRESS:
827 NORTH 29TH STREET, FORT PIERCE, FL 34947

DWG IDENTIFICATION
COMPOSITE STEEL DETAILS

PROJECT PHASE:	DATE:	CHECKED / DRAWN BY:
100% CDs	12/04/25	W.JR/TEB

THIS DRAWING HAS BEEN ISSUED ON THE DATE SHOWN & FOR THE PURPOSE NOTED

JOB FILE NUMBER:
I-25129

S531

5/29/2026 3:08:17 PM
Autodesk Docs://SLPS C.A. Moore Gymnasium/STR_v25 C.A. Moore_R25.rvt