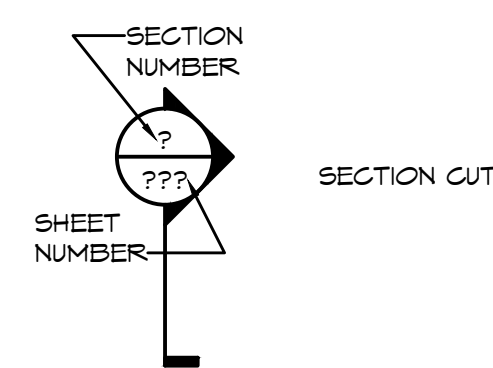


GENERAL NOTES

SYMBOLS & LEGENDS



INDICATES STEEL BEAM SIZE
INDICATES UPWARD CAMBER
INDICATES NUMBER OF SHEAR CONNECTORS
INDICATES CURVED STEEL MEMBER
INDICATES EXISTING MEMBER
INDICATES NEW MEMBER
INDICATES GALVANIZED STEEL MEMBER
INDICATES ASTM A53 STD WEIGHT CLASS PIPE (NPS)
INDICATES ASTM A53 XS WEIGHT CLASS PIPE (NPS)
INDICATES ASTM A43 XS WEIGHT CLASS PIPE (NPS)

ABBREVIATIONS

ACI	AMERICAN CONCRETE INSTITUTE	LLV	LONG LEG VERTICAL
ADJ	AUXILIARY DRAIN	LP	LONG POINT
AFF	ADJACENT	LVL	LEVEL (ING)
AFS	ABOVE FINISH FLOOR	M	MOMENT
ASG	AMERICAN SOCIETY OF STEEL BUILDING	MAX	MAXIMUM
ATC	AMERICAN INSTITUTE OF TIMBER CONSTRUCTION	MB	MACHINE BOLT
ASI	AMERICAN IRON & STEEL INSTITUTE	MBMA	MANUFACTURERS ASSOCIATION
ALT	ANCHOR	MECH	MECHANICAL
ANCH	ANCHORAGE	MEP	MECHANICAL, ELECTRICAL, PLUMBING
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE	MEZZ	MEZZANINE
APA	AMERICAN PLYWOOD ASSOCIATION	MFR	MANUFACTURER (R) (ED)
APPROX	APPROXIMATE	MIA	MASONRY INSTITUTE OF AMERICA
ARCH	ARCHITECT	MIN	MINIMUM
ASTM	AMERICAN SOCIETY FOR TESTING MATERIALS	MISC	MISCELLANEOUS
AT	AT	MO	MASONRY OPENING
AVS	AMERICAN VELDING SOCIETY	NCA	NATIONAL CONCRETE MASONRY ASSOCIATION
BFF	BELOW FINISH FLOOR	NG	NOT IN CONTRACT
BFG	BELOW FINISH SLAB	NOM	NOMINAL PIPE SIZE
BLDG	BUILDING	NTS	NOT TO SCALE
BLK	BLOCK	OD	OUTSIDE DIAMETER
BLN	BELONG	OPNS	OPENINGS
BM	BEAM	OPH	OPPOSITE HAND
BOF	BOTTOM OF FOOTING	OPP	OPPOSITE
BRG	BRACING	OSL	OUTSTANDING LEG
BS	BOTH SIDES	P	AXIAL FORCE
BTM	BOTTOM	PCA	PORTLAND CEMENT ASSOCIATION
BTWN	BETWEEN	PCC	PRECAST CONCRETE
C/C	CENTER TO CENTER	PCF	POUNDS PER CUBIC FOOT
CDL	COLLATERAL DEAD LOAD	PG	PRESTRESSED CONCRETE
CG	CENTER OF GRAVITY	PL	PROPERTY LINE
CH	CLEAR	PLF	POUNDS PER LINEAR FOOT
CLR	CLEAR (ANCE)	PSF	POUNDS PER SQUARE FOOT
CM	CONCRETE MASONRY UNIT	PSI	POUNDS PER SQUARE INCH
COMP	COMPRESSION	PT	POST-TENSIONING INSTITUTE
CONC	CONCRETE	PVC	POLYVINYL CHLORIDE
CONN	CONNECTION	PVMT	PAVEMENT
CONT	CONTINUOUS	PLY	PLYWOOD
CJP	COMPLETE JOINT PENETRATION	QTY	QUANTITY
CRSI	CONCRETE REINFORCING STEEL INSTITUTE	REF	REFERENCE
D	DEPTH, DEEP	REIN	REINFORCEMENT (D), REINFORCING
DBA	DEFORMED BAR ANCHOR	REQ	REQUIRED
DBL	DOUBLE	RO	ROUGH OPENING
DET	DETAIL	R	RADIUS, RADIAL (LY)
DIAG	DIAGONAL	SCH	SCHEDULE (D)
DIM	DIMENSION (ED)	SDI	STEEL DESIGN DEAD LOAD
DL	DEAD LOAD	SECT	SECTION
DN	DOWN	SHT	SHEET
DN	DOWN	SH	SHIM
DNL	DOWN	SJ	STEEL JOIST INSTITUTE
E	MODULUS OF ELASTICITY	SL	SNOW LOAD
E	EACH	SL	SLAB ON GRADE
EF	EACH FACE	SP	SPACE, SPACING
EJ	EXPANSION JOINT	SPEC	SPECIFICATION
EL	ELEVATION	SPB	SOUTHERN PINE INSPECTION BUREAU
ELC	ELECTRICAL	SQ	SQUARE
EOR	ENGINEER OF RECORD	SSMA	STEEL STUD MNFR ASSOCIATION
EQ	EQUAL	SSPC	STEEL STRUCTURES PAINTING COUNCIL
EQUIP	EQUIPMENT	STAG	STAGGERED
EXP	EXPANSION	STD	STANDARD
EXT	EXTERIOR	STRUCT	STRUCTURAL
FD	FLOOR DRAIN	SYM	SYMMETRY, SYMMETRICAL
FDN	FOUNDATION	T	TOP
FF	FINISH FLOOR	T&B	TOP & BOTTOM
FIN	FINISH (ED)	TEMP	TEMPORARY
FLR	FLOOR	TEN	TENSION, TENSILE
FOC	FACE OF CONCRETE	T&G	TONGUE & GROOVE
FP	FRAMING	THD	THREAD (ED)
FRMS	FRAMING	THK	THICKNESS
FT	FOOT, FEET	TL	TOTAL LOAD
FTG	FOOTING	TOC	TOP OF CONCRETE (SLAB, BEAM, COLUMN, ETC)
FUT	FUTURE	TOP	TOP OF FOOTING
GA	GAUGE	TOF	TOP OF FLOOR
GALV	GALVANIZED	TOL	TOP OF LEDGER
GC	GENERAL CONTRACTOR	TOP	TOP OF PIER
GLB	GULLIM BEAM	TOP	TOP OF STEEL
GWB	Gypsum WALL BOARD	TOW	TOP OF WALL
GYP	GYP	TRI	TRUSS PLATE INSTITUTE
H	HIGH	TYP	TYPICAL
HORIZ	HORIZONTAL	UL	UNDERWRITERS LABORATORIES
HSL	HIGH STRENGTH BOLT	UDL	UNIFORM DEAD LOAD
HT	HEIGHT	UL	UNIFORM LIVE LOAD
ID	INSIDE DIAMETER	USL	UNIFORM SNOW LOAD
INT	INTERIOR	UNO	UNLESS NOTED OTHERWISE
JST	JOIST	VER	VERTICAL
JO	JOINT	VIF	VERIFY IN FIELD
KIP	KIPS	W	WIDTH, WIDE
KSI	KIPS PER SQUARE INCH	W	WIDTH
LB	LONG, LENGTH	WCLB	WEST COAST LUMBER
LD	LOAD	WCLB	WEST COAST LUMBER
LD	LOAD	WCLB	WEST COAST LUMBER
LL	LIVE LOAD	WCLB	WEST COAST LUMBER
LLH	LONG LEG HORIZONTAL	WCLB	WEST COAST LUMBER

GRAVITY DESIGN LOADS

- DEAD LOADS
 - FLOOR = 75 PSF
 - ROOF = 22 PSF
- LIVE LOADS
 - CORRIDORS (ABOVE 1ST FLOOR) = 20 PSF
 - OPERATING ROOMS, LABS = 60 PSF
 - FATENT ROOMS = 60 PSF
 - ROOF = 20 PSF
 - PARTITIONS = 20 PSF
- RAIN LOADS
 - RAIN LOADS, R = 32 PSF
 - RAIN INTENSITY = 10.1 IN/HR

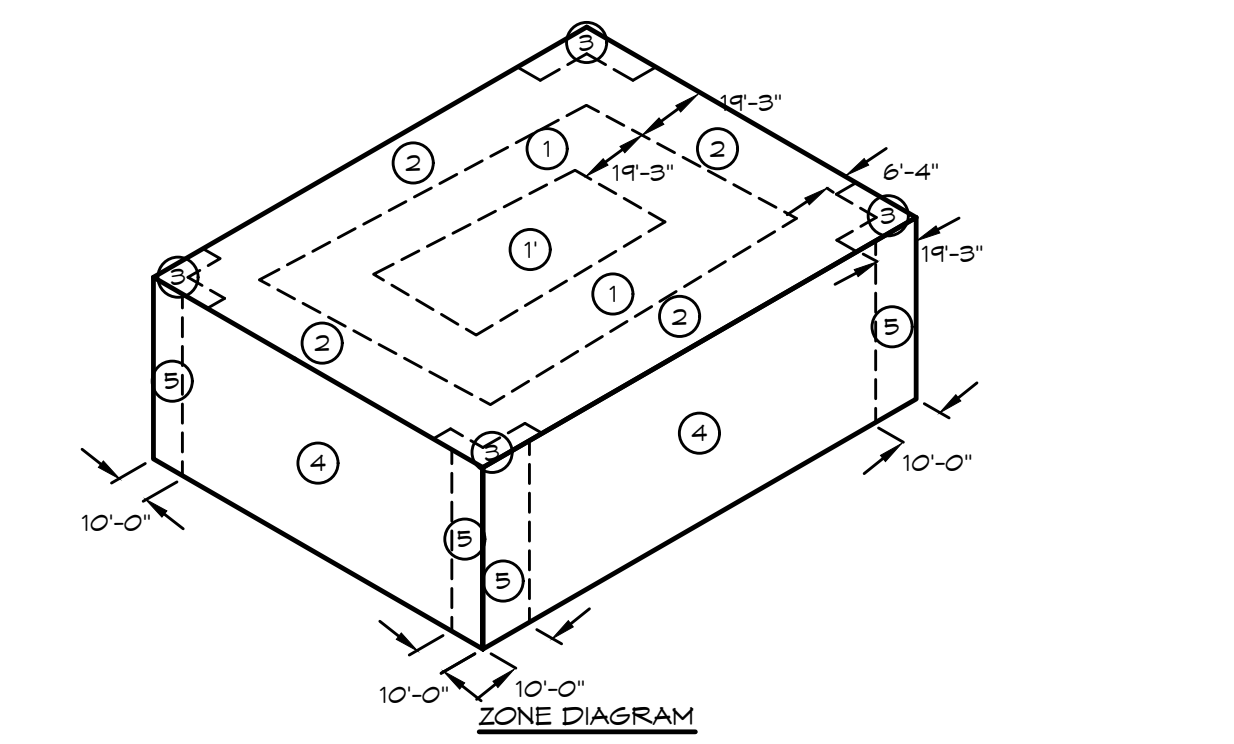
LATERAL DESIGN LOADS

- WIND LOADS
 - BUILDING RISK CATEGORY = IV
 - BASIC WIND VELOCITY, V_{100} = 120 mph
 - EXPOSURE CATEGORY = B
 - INTERNAL PRESSURE COEFFICIENT = ± 0.10
 - COMPONENTS AND CLADDING PRESSURES = ASCE 7 WIND LOAD PROVISIONS AS INDICATED IN TABLE BELOW
- TORNADO LOADS
 - TORNADO WIND SPEED = 82 MPH
 - EFFECTIVE PLAN AREA = 45800 SQ FT

COMPONENTS/CLADDING SERVICE LEVEL
DESIGN WIND PRESSURES (PSF)

ZONE	MINIMUM EFFECTIVE WIND AREA				
	10 SQ FT	50 SQ FT	100 SQ FT	200 SQ FT	500 SQ FT
FLAT ROOF					
1	+16/-36	+15/-36	+13/-36	+13/-29	+13/-19
2	+16/-62	+15/-53	+13/-53	+13/-44	+13/-34
3	+16/-62	+15/-70	+13/-64	+13/-54	+13/-52
4	+16/-112	+15/-87	+13/-77	+13/-66	+13/-52
WALL					
4	+34/-42	+35/-38	+33/-36	+31/-35	+29/-32
5	+34/-52	+35/-44	+33/-40	+31/-37	+29/-32

- NOTES:
- ALL EXTERIOR DOOR, WINDOW AND ROOF ASSEMBLIES SHALL SATISFY THE REQUIREMENTS OF THE FLORIDA BUILDING CODE INCLUDING CONNECTIONS TO THE MAIN BUILDING STRUCTURE THAT HAVE THE CAPACITY TO RESIST THE WIND LOADS INDICATED.
 - NEGATIVE WIND PRESSURE DESIGN VALUES INDICATED WIND SUCTION ACTING ON THE COMPONENT OR CLADDING AREA FROM THE COMPONENT AND POSITIVE DESIGN VALUES INDICATE WIND PRESSURES ACTING ON THE COMPONENT OR CLADDING TOWARD THE COMPONENT.
 - VALUES INDICATED ARE SERVICE LEVEL DESIGN WIND PRESSURES BASED ON V_{100} .
 - SEE ZONE DIAGRAM FOR LOCATIONS WHERE PRESSURES ARE TO BE APPLIED.
 - SEE DETAILS FOR WIND PRESSURE AT ROOF OVERHANGS AND PARAPETS NOT INCLUDED IN TABLE ABOVE.



GENERAL

- THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS BEFORE STARTING WORK. THE OWNER'S REPRESENTATIVE SHALL BE NOTIFIED OF ANY DISCREPANCY.
- ALL MATERIAL AND WORKMANSHIP SHALL CONFORM TO THE 2023 FLORIDA BUILDING CODE.
- MEANS AND METHODS OF CONSTRUCTION ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- THE DESIGN, ADEQUACY, AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC. IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- THE CONTRACTOR SHALL COORDINATE THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL WORKS WITH THE STRUCTURAL DRAWINGS. THE OWNER'S REPRESENTATIVE SHALL BE NOTIFIED OF ANY DISCREPANCIES OR OMISSIONS.
- THE CONTRACTOR SHALL VERIFY ALL FLOOR AND ROOF MOUNTED MECHANICAL EQUIPMENT WEIGHTS, FLOOR AND/OR ROOF OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- WEIGHTS OF ROOF SUPPORTED EQUIPMENT CONSIDERED IN THE DESIGN ARE INDICATED ON THE DRAWINGS. NOTIFY THE CONTRACTOR OF ANY CHANGES TO THE WEIGHTS OF EQUIPMENT PRIOR TO STARTING WORK.
- THICKENED SLABS, DEPRESSIONS, TRENCHES, AND UTILITY PENETRATIONS ARE SCHEMATICALLY INDICATED ON THE STRUCTURAL DRAWINGS. CONTRACTOR SHALL COORDINATE LOCATIONS AND ANY ADDITIONAL REQUIREMENTS WITH THE FINAL ARCHITECTURAL, MECHANICAL, AND PLUMBING DRAWINGS AND WITH THE SPECIFIC REQUIREMENTS OF THE VENDOR'S DRAWINGS FOR SPECIALTY EQUIPMENT. NOTIFY ARCHITECT AND STRUCTURAL ENGINEER WHERE DISCREPANCIES EXIST BETWEEN THIS INFORMATION AND THE STRUCTURAL DRAWINGS PRIOR TO STARTING WORK.
- THE CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF ANY CONDITIONS ENCOUNTERED IN THE FIELD CONTRADICTORY TO THOSE SHOWN ON THE STRUCTURAL DRAWINGS.
- THE STRUCTURE WILL BE UNSTABLE UNTIL ALL STEEL MEMBERS ARE ERECTED. CONNECTIONS ARE COMPLETELY BOLTED AND/OR WELDED AND INSPECTED, AND THE FLOOR DECK ATTACHED TO THE STEEL FRAMING AND THE CONCRETE PLACED AND CURED. UNTIL SUCH TIME, TEMPORARY BRACING IS REQUIRED AND MUST REMAIN IN PLACE.
- SHOULD ANY OF THE INSTRUCTIONS SHOWN ON THE DRAWINGS CONFLICT WITH THE GENERAL STRUCTURAL NOTES, THE SPECIFIC INSTRUCTIONS SHALL GOVERN.
- ARCHITECTURAL BACKGROUNDS ARE SHOWN FOR REFERENCE ONLY. FINAL STRUCTURAL DOCUMENTS SHOULD BE COORDINATED WITH ARCHITECTURAL CONSTRUCTION DOCUMENTS.

PROTECTION OF SYSTEMS AND UTILITIES

- SYSTEMS AND UTILITIES IDENTIFIED IN FBC 449.4.2 SHALL BE PROTECTED FROM DEBRIS IMPACT BY AN EQUIPMENT PROTECTION SYSTEM. EQUIPMENT PROTECTION SHALL BE DESIGNED TO RESIST THE WIND LOADS DUE TO ULTIMATE DESIGN WIND SPEED, V_{100} , FOR COMPONENTS AND CLADDING. GLASS IN GLAZED CURTAIN WALLS, GLAZED STOREFRONTS AND GLAZED PARTITIONS SHALL MEET THE SEISMIC REQUIREMENTS OF ASCE 7, SECTION 13.5.4. THE LOAD RESISTANCE OF GLASS UNDER UNIFORM WIND LOAD SHALL BE DETERMINED IN ACCORDANCE WITH ASTM E1300.
- EXHAUST OPENINGS IN BUILDINGS SHALL COMPLY WITH AMCA STANDARD 950.

COMPONENTS ANCHORED TO THE BUILDING

- ELEMENTS OF STRUCTURES AND THEIR ATTACHMENTS, PERMANENT NON-STRUCTURAL COMPONENTS AND THEIR ATTACHMENTS, AND EQUIPMENT SUPPORTED BY THE BUILDING WHICH ARE NOT DETAILED ON THESE DRAWINGS SHALL BE DESIGNED BY OTHERS, PRIOR TO FABRICATION AND INSTALLATION, SUBMIT DESIGN PLANS, DETAILS AND CALCULATIONS, PERFORMED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED, TO THE ARCHITECT FOR REVIEW AND TO THE AUTHORITY HAVING JURISDICTION FOR REVIEW AND APPROVAL.
- CONNECTIONS OF THESE ELEMENTS TO THE MAIN STRUCTURAL MEMBERS SHALL NOT RESULT IN TORSION IN THE SUPPORTING STRUCTURAL MEMBERS.

FOUNDATION

- THE GEOTECHNICAL REPORT PROVIDED BY UNIVERSAL ENGINEERING SERVICES DATED MAY 15, 2024, (UES PROJECT NO. 3330 2200069 0000), WAS USED FOR THE DESIGN OF THE FOUNDATIONS. SITE PREPARATION, GRADING, FOOT-VEHICLE IMPROVEMENT VERIFICATION PROGRAM REQUIREMENTS AND SOIL REMEDIATION SHALL BE IN ACCORDANCE TO THE RECOMMENDATIONS IN THIS DOCUMENT.
- THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS THAT VARY FROM THOSE ASSUMED FOR DESIGN.
- BUILDING COLUMN FOUNDATIONS SHALL CONSIST OF SHALLOW FOOTINGS BEARING ON IMPROVED SOIL SUBGRADE.
- PROVIDE AN VIBRO-REPLACEMENT (VIBRO STONE COLUMNS) SUBGRADE MEETING THE FOLLOWING PERFORMANCE CRITERIA:
 - MAXIMUM ALLOWABLE BEARING PRESSURE = 6000 PSF
 - ESTIMATED TOTAL LONG-TERM SETTLEMENT FOR FOUNDATIONS = 1"
 - ESTIMATED LONG-TERM DIFFERENTIAL SETTLEMENT BETWEEN FOUNDATIONS = $\frac{1}{8}"$
 - MINIMUM IMPROVEMENT DEPTH TO BE 2X THE WIDTH OF THE COLUMN FOOTING OR 15'-0" MINIMUM.
- SEE SPECIFICATION SECTION 31.2260 FOR ADDITIONAL PERFORMANCE CRITERIA FOR SUBGRADE IMPROVEMENT WITH VIBRO-REPLACEMENT (VIBRO STONE COLUMNS).
- SUBMIT THE FOLLOWING TO THE ARCHITECT (STRUCTURAL ENGINEER) FOR APPROVAL: ENGINEERING CALCULATIONS AND CONSTRUCTION DOCUMENTS BEARING THE SEAL OF A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED FOR SUBGRADE IMPROVEMENT SYSTEM WITH VIBRO-REPLACEMENT (VIBRO STONE COLUMNS).
- SHALLOW SCREENING FOUNDATIONS MAY BE FOUNDATIONS ON EXISTING SOILS OR ON NEWLY PLACED ENGINEERED FILL WITH A NET ALLOWABLE BEARING PRESSURE OF 2500 PSF.
- CONTRACTOR SHALL VERIFY BY AN INDEPENDENT TESTING LABORATORY THAT THE ASSUMED BEARING CAPACITY IS OBTAINED.

CONCRETE

- CONCRETE WORK SHALL CONFORM TO THE ACI 318 AND CRSI STANDARDS.
- CONCRETE SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH UTILIZING TYPE I/IV CEMENT UNLESS NOTED OTHERWISE:

LOCATION	STRENGTH (PSI)	REMARKS
1. FOOTINGS	4000 PSI	NORMAL WEIGHT (SEE NOTE 3 FOR SULFATE RESISTANCE REQUIREMENTS)
2. CONCRETE SCREENING	4000 PSI	NORMAL WEIGHT (SEE NOTE 3 FOR SULFATE RESISTANCE REQUIREMENTS)
3. ELEVATOR PIT	4000 PSI	NORMAL WEIGHT w/ INTEGRAL WATERPROOFING ADMIXTURE (SEE NOTE 3 FOR SULFATE RESISTANCE REQUIREMENTS)
4. PEDESTALS	3000 PSI	NORMAL WEIGHT w/ MYRA
2.2 SLAB ON GROUND	3000 PSI	NORMAL WEIGHT w/ MYRA
2.3 SLAB ON METAL DECK	3500 PSI	NORMAL WEIGHT w/ MYRA
2.4 EXTERIOR CONCRETE	4000 PSI	(SEE NOTE 3 FOR SULFATE RESISTANCE REQUIREMENTS)
2.5 CURBS AT EXT WALL	3000 PSI	NORMAL WEIGHT
3. PROVIDE CONCRETE THAT IS IN CONTACT w/ THE SOIL WITH SULFATE RESISTANCE.		
3.1. PROVIDE CONCRETE w/ A MAXIMUM W/C RATIO < 0.50 USING ASTM C150 TYPE I/IV CEMENTS.		
3.2. ALTERNATIVELY PROVIDE CONCRETE w/ A MAXIMUM W/C RATIO < 0.50 USING ASTM C595 CEMENTS w/ MODERATE SULFATE RESISTANCE AS INDICATED BY THE M/S DESIGNATION.		
4. WATER/CEMENT RATIO, FLY ASH CONTENT AND MAXIMUM SLABS ARE SPECIFIED IN SECTION 03 31 00 OF THE PROJECT SPECIFICATIONS. SUBSTANTIATING DATA FOR CONCRETE MIX DESIGN ARE ALSO DESCRIBED IN THIS SECTION.		
5. THE CONCRETE MIX DESIGN FOR ALL INTERIOR SLABS SHALL CONTAIN A CONCRETE MOISTURE VAPOR REDUCTION ADMIXTURE (MYRA) PER SECTION 033100 OF THE PROJECT SPECIFICATIONS.		
6. SUBMIT SHOP DRAWINGS INDICATING LOCATIONS OF PROPOSED SLAB CONTROL JOINTS AND CONSTRUCTION JOINTS TO THE ARCHITECT FOR REVIEW PRIOR TO SLAB CONSTRUCTION.		
7. PIPES OR DUCTS EXCEEDING ONE-THIRD THE SLAB OR WALL THICKNESS SHALL NOT BE PLACED IN STRUCTURAL CONCRETE UNLESS SPECIFICALLY DETAILED. SEE MECHANICAL AND/OR ELECTRICAL DRAWINGS FOR LOCATION OF SLEEVES, ACCESSORIES, ETC.		
8. REFER TO ARCHITECTURAL DRAWINGS FOR ITEMS REQUIRED TO BE ENCASED IN CONCRETE AND LOCATION OF FLOOR FINISHES AND SLAB DEPRESSIONS.		

STRUCTURAL STEEL

- STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING DESIGNATIONS:
 - ALL COLUMNS, BEAMS, AND GIRDERS UNLESS OTHERWISE NOTED = ASTM A992
 - PLATES, RODS, ANGLES CHANNELS AND BARS UNLESS OTHERWISE NOTED = ASTM A36
 - HSS SECTIONS = ASTM A500 (GRADE B)
 - STRUCTURAL STEEL PIPES = ASTM A53 (GRADE B)
- STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED ACCORDING TO THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".
- STANDARD SHEAR CONNECTIONS ARE DETAILED ON THE STRUCTURAL DRAWINGS. ANY DEVIATION BY THE FABRICATOR WILL REQUIRE A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED TO REDESIGN AND SIGN CALCULATIONS AND SHOP DRAWINGS FOR REVIEW BY THE ENGINEER OF RECORD. SEE FRAMING PLANS FOR FACTORED BEAM OR GIRDER END REACTIONS FOR CONNECTION DESIGN. MINIMUM CONNECTION SHALL CONSIST OF TWO BOLTS. WHERE NO VALUE IS SHOWN, FACTORED REACTION IS LESS THAN OR EQUAL TO 20 KIPS. CONNECTIONS MAY BE WELDED OR BOLTED. WELDED CONNECTIONS SHALL BE BEARING TYPE WITH MINIMUM $\frac{1}{4}"$ DIAMETER A528 OR F1552 BOLT. FIELD WELDING SHALL BE BY CERTIFIED WELDERS IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY.
- CAMBER: PROVIDE UPWARD CAMBER AT MIDSPAN BETWEEN SUPPORTS WHERE INDICATED ON DRAWINGS (<1"). THE AMOUNT OF CAMBER WILL VARY DUE TO MILLING AND SOME MEMBERS (SUCH AS THOSE USED IN ANCHORAGE) SHALL BE CALCULATED INITIAL DEFLECTION OF LESS THAN 1" BUT NOT CAMBERED. THE AMOUNT OF ACTUAL DEFLECTION FOR EACH STRUCTURAL STEEL MEMBER MAY VARY DUE TO MILL OR SHOP TOLERANCES.
- ANCHOR RODS SHALL BE PROVIDED WITH THE MINIMUM PROPERTIES OF ASTM F1554 AS FOLLOWS:
 - NON-FRAME COLUMN ANCHORS - ASTM F1554 GR50 OR GR55 WITH FORGED HEX HEADS OR WITH SINGLE HEAVY HEX NUT TACK WELDED TO ROD.
 - FRAME COLUMN ANCHORS - ASTM F1554 GR55 WITH FORGED HEAVY HEX HEADS.
- WHERE NON-GALVANIZED STRUCTURAL STEEL IS EXPOSED TO EARTH OR WEATHER, PROVIDE A PROTECTIVE COVER CONSISTING OF 3" MINIMUM CONCRETE COVER, A COAT OF BITUMINOUS MASTIC OR SIMILAR.

SHEAR CONNECTORS (WELDED STUDS)

- SHEAR CONNECTORS SHALL BE NELSON SHEAR CONNECTOR STUDS PER ICC ER-2056 OR APPROVED EQUAL.
- COMPOSITE BEAMS: CONNECT SLAB TO BEAM WITH $\frac{1}{4}"$ INCH DIAMETER WELDED STUD SHEAR CONNECTORS. EXTEND STUD TO 1" BELOW TOP OF SLAB. SEE PLAN FOR REQUIRED NUMBER OF SHEAR CONNECTORS SPACED EQUALLY ALONG BEAM INDICATED ON PLAN.
- WHEN THE NUMBER OF SHEAR CONNECTIONS IS NOT SHOWN, BEAM SHALL RECEIVE SHEAR CONNECTORS AT 12" CENTER-TO-CENTER SPACING.
- STUDS SHALL BE AUTOMATICALLY END WELDED IN ACCORDANCE WITH THE MNFR'S RECOMMENDATIONS IN SUCH A MANNER TO PROVIDE COMPLETE FUSION BETWEEN THE END OF THE STUD AND THE STEEL MEMBER.

REINFORCING STEEL

- REINFORCING STEEL SHALL CONFORM TO ASTM A615 - GRADE 60.
 - REINFORCING STEEL THAT IS WELDED SHALL CONFORM TO ASTM A708 - GRADE 60.
 - WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064 AND HAVE A MINIMUM LAP OF 6".
 - REINFORCEMENT SHALL BE SPLICED ONLY AS SHOWN OR NOTED. SPLICES AT OTHER LOCATIONS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER.
 - PROVIDE DOVELLS IN FOOTINGS THE SAME SIZE AND NUMBER AS VERTICAL WALL OR COLUMN REINFORCING. DOVELLS SHALL HAVE A MINIMUM STANDARD LAP SPICE UNLESS OTHERWISE SHOWN.
 - PROVIDE THE FOLLOWING PROTECTIVE COVERINGS OF REINFORCING BARS:
 - CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:

ALL REINFORCEMENT SIZES	3" CLEAR
2. CONCRETE EXPOSED TO EARTH OR WEATHER:	
#6 THROUGH #10 BARS	2" CLEAR
#5 BAR, #31 OR #31 WIRE, AND SMALLER	1" CLEAR
 - CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:

SLABS, WALLS, JOISTS, #14 AND #10 BARS	1" CLEAR
#11 BAR AND SMALLER	3/4" CLEAR
 - BEAMS, COLUMNS:

ALL REINFORCEMENT SIZES	3" CLEAR
2. CONCRETE EXPOSED TO EARTH OR WEATHER:	
#6 THROUGH #10 BARS	2" CLEAR
#5 BAR, #31 OR #31 WIRE, AND SMALLER	1" CLEAR
- ALL REINFORCING BARS TO BE SPLICED WITH THE FOLLOWING STANDARD LAP LENGTHS:

BAR NO.	BOTTOM BAR LENGTH (INCHES)	TOP BAR * LENGTH (INCHES)
#3	19	25
#4	25	33
#5	31	41
#6	37	48
#7	44	56
#8	51	64
#9	58	72
#10	70	84
#11	85	102

- *TOP BAR IS HORIZONTAL REINFORCEMENT IN FLEXURAL MEMBERS SO PLACED THAT MORE THAN 12 INCHES OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE SPLICE.
- REINFORCING BARS MARKED CONTINUOUS TO BE SPLICED WITH THE STANDARD LAP LENGTH.
 - PROVIDE CORNER BARS TO MATCH ALL LONGITUDINAL REINFORCEMENT AT ALL CORNERS AND INTERSECTIONS OF ALL CONCRETE MEMBERS (FOOTINGS, GRADE BEAMS, WALLS, ETC.) UNLESS OTHERWISE DETAILED.

METAL STAIRS AND INTERMEDIATE STAIR LANDINGS

- STEEL FRAMED STAIRS AND INTERMEDIATE LANDINGS ARE DETAILED ON THE STRUCTURAL DRAWINGS. ANY DEVIATIONS BY THE FABRICATOR WILL REQUIRE THE FOLLOWING TO BE SUBMITTED TO THE ARCHITECT FOR REVIEW BY THE ARCHITECT (STRUCTURAL ENGINEER) FOR APPROVAL: ENGINEERING CALCULATIONS AND CONSTRUCTION DOCUMENTS BEARING THE SEAL OF A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED FOR ALL METAL STAIRS AND ACCESSORIES. METAL STAIR ACCESSORIES INCLUDE ALL STAIR FRAMING MEMBERS, TREADS, CONNECTIONS, RAILINGS AND RELATED MISCELLANEOUS METALS.
- DESIGN METAL STAIRS FOR THE FOLLOWING MINIMUM LOADS:
 - SELF WEIGHT DEAD LOADS
 - LIVE LOAD OF 100 PSF
 - CONCENTRATED LIVE LOAD ON STAIR TREADS OF 300 LB ON AN AREA OF 4 IN²
- DESIGN METAL STAIR HANDRAIL AND GUARDRAIL SYSTEMS FOR THE FOLLOWING MINIMUM LOADS:
 - SINGLE CONCENTRATED LOAD OF 200 LB APPLIED IN ANY DIRECTION AT ANY POINT ALONG THE TOP OF THE RAIL AND TRANSFER THIS LOAD THROUGH THE SUPPORTS TO THE STRUCTURE
 - DISTRIBUTED LOAD OF 80 LB/FT APPLIED IN ANY DIRECTION AT THE TOP OF THE RAIL AND TRANSFER THIS LOAD TO THE STRUCTURE (APPLIED NON-CONCURRENTLY WITH THE 200 LB CONCENTRATED LOAD)
 - CONCENTRATED LOAD OF 80 LB ON AN AREA NOT TO EXCEED 1 FT² INCLUDING OPENINGS AND SPACE BETWEEN RAILS FOR INTERMEDIATE HANDRAILS (EXCEPT HANDRAILS), BALUSTERS AND PANEL FINISHS (APPLIED NON-CONCURRENTLY WITH DISTRIBUTED LOAD OF 80 LB/FT)
- SEE CONTRACT DRAWINGS FOR METAL STAIR DIMENSIONS AND FOR INTENT OF ATTACHMENT TO MAIN STRUCTURAL MEMBERS.

MASONRY

- MASONRY WORK SHALL CONFORM TO ACI-530.
- PROVIDE MINIMUM NET COMPRESSIVE STRENGTH OF MASONRY ASSEMBLAGE, $F_m = 2000$ PSI.
- VERIFY COMPRESSIVE STRENGTH OF MASONRY ASSEMBLAGE BY EITHER THE UNIT STRENGTH METHOD OR THE PRISM TEST METHOD.
- PROVIDE CONCRETE MASONRY UNITS OF MEDIUM WEIGHT CLASSIFICATION COMPLYING WITH ASTM C90 WITH MINIMUM COMPRESSIVE STRENGTH AS NEEDED TO MEET THE SPECIFIED COMPRESSIVE STRENGTH OF THE MASONRY ASSEMBLAGE, F_m .
- PROVIDE TYPE M OR TYPE S MORTAR COMPLYING WITH ASTM C270 FOR MASONRY WALLS.
- PROVIDE REINFORCING STEEL REQUIREMENTS FOUND IN THE GENERAL NOTES.
- PROVIDE 48 BAR DIMETERS FOR DEVELOPMENT LENGTHS AND SPLICE LENGTHS OF REINFORCING IN MASONRY UNO.
- USE 4"-0" GROUT LIFTS FOR BAR SIZES #3 THRU #6 AND 5"-0" GROUT LIFTS FOR BAR SIZES #7 AND LARGER.
- CONCRETE MASONRY UNITS SHALL BE GROUTED WITH 5,000 PSI COARSE GROUT AS SHOWN ON PLANS. ALL MASONRY WITH REINFORCING BARS SHALL BE GROUTED SOLID.
- GROUT SHALL BE CONSOLIDATED BY MECHANICAL VIBRATION ACCORDING TO THE METHOD OUTLINED IN THE SPECIFICATIONS.
- PROVIDE THE TYPE OF MASONRY UNITS SHOWN ON PLANS.
- PROVIDE MASONRY IN A RUNNING BOND PATTERN.
- PROVIDE DUR-O-WALL HORIZONTAL JOINT REINFORCEMENT AND SPECIAL DUR-O-WALL ACCESSORIES FOR CORNERS, INTERSECTIONS, ETC. HORIZONTAL JOINT REINFORCING SHALL CONSIST OF AT LEAST TWO WIRES OF #11. HORIZONTAL JOINT REINFORCING SHALL BE SPACED NOT MORE THAN 16" ON CENTER.
- SUPPORT AND FASTEN REINFORCEMENT TOGETHER PRIOR TO GROUTING USING ASTM A618 WIRE OR ASTM A618 STEEL WIRE TO PREVENT DISPLACEMENT BEYOND ALLOWABLE TOLERANCES.
- INSPECTIONS, TESTS, AND SUBMITTALS FOR THE CONSTRUCTED MASONRY SHALL COMPLY WITH THE LEVEL 2 QUALITY ASSURANCE REQUIREMENTS OF ACI 530 AND THE GOVERNING BUILDING CODE. SEE TABLE OF STRUCTURAL INSPECTIONS THIS SHEET.

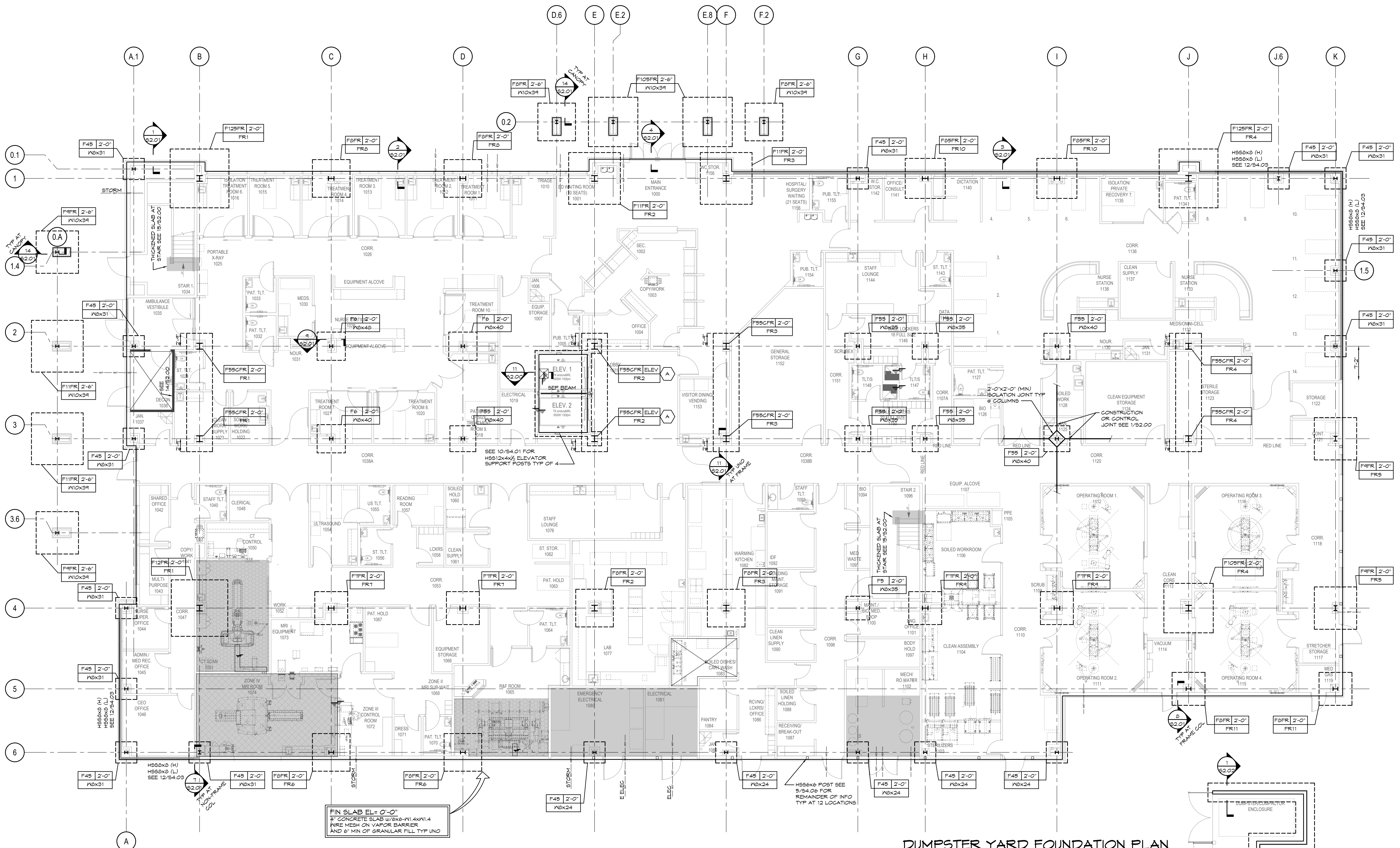
LIGHT GAGE METAL FRAMING (NON-LOAD-BEARING) - DESIGN BUILD PERFORMANCE SPECIFICATION

- ALL COLD-FORMED METAL STUDS AND ACCESSORIES SHALL BE OF THE TYPE, SIZE, AND GAGE SHOWN ON PLANS. WHERE SIZES ARE NOT SHOWN, THE DESIGN SHALL BE THE RESPONSIBILITY OF THE MANUFACTURER.
- A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED SHALL DESIGN THE EXTERIOR WALL FRAMING INCLUDING THE ANCHORAGE TO THE PRIMARY STRUCTURAL FRAME. THE PROFESSIONAL ENGINEER SHALL PROVIDE SEALED CALCULATIONS, REVIEW THE SHOP DRAWINGS AND PROVIDE A SEALED LETTER CERTIFYING THAT THE INFORMATION SHOWN ON THE SHOP DRAWINGS IS IN CONFORMANCE WITH THE DESIGN.
- ALL MEMBERS SHALL BE DESIGNED IN ACCORDANCE TO AISI "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".
- METAL STUDS SHALL BE FROM A SPSA APPROVED SUPPLIER.
- ALL STRUCTURAL MEMBERS SHALL BE FORMED FROM CORROSION-RESISTANT STEEL, CORRESPONDING TO THE REQUIREMENTS OF ASTM A446, WITH A MINIMUM YIELD STRENGTH OF 33 KSI.
- ALL STRUCTURAL MEMBERS SHALL BE DESIGNATED IN CONFORMANCE TO ASTM A525. EXTERIOR MEMBERS #60, ALL OTHER MEMBERS #60, MIN.
- FASTENING OF COMPONENTS SHALL BE WITH SELF-DRILLING SCREWS OF SUFFICIENT SIZE TO ENSURE THE STRENGTH OF THE CONNECTION. WIRE TYING OF COMPONENTS IS NOT PERMITTED.
- SPLICES IN FRAMING COMPONENTS OTHER THAN THE RUNNER TRACK IS NOT PERMITTED.
- NON-LOAD-BEARING EXTERIOR WALL STUDS SHALL COMPLY WITH THE FOLLOWING:
 - CONTRIBUTION OF SHEATHING MATERIALS TO THE PROVISION OF PLANE DEFLECTION TO 4/800 (BRICK/STONE VENEER) AND 4/800 (METAL PANEL) AT SERVICE LEVEL WIND PRESSURES INDICATED BELOW.
- STUDS SHALL BE SECURED TO CONTINUOUS RUNNER TRACKS UNLESS THE STUD TERMINATES AT A DEFLECTION TRACK.
- INSTALLATION OF EXTERIOR WALL FRAMING SHALL ACCOMMODATE DEFLECTION OF PRIMARY STRUCTURAL FRAMING USING VERTICAL SLIDE CLIPS AND/OR DEFLECTION SLIP TRACKS WHERE NECESSARY. DESIGN FRAMING SHALL BE PROVIDED TO SUPPORT THE EXTERIOR WALL STUDS. TESTING SHALL BE PERFORMED AS REQUIRED BY THE EOR. CONTACT EOR FOR ADHESIVE AND MECHANICAL ANCHOR INSTALLATIONS AS REQUIRED BY THE EOR. CONTACT EOR FOR ADHESIVE AND MECHANICAL ANCHORS REQUIRED TO BE TESTED AND REQUIRED PROOF LOAD MAGNITUDE.
- SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE LISTED BELOW, SHALL BE SUBMITTED TO THE EOR WITH CALCULATIONS SHOWING THAT THE SUBSTITUTED PRODUCT WILL ACHIEVE AN EQUIVALENT CAPACITY USING THE APPROPRIATE DESIGN PROCEDURE REQUIRED BY THE BUILDING CODE. PRODUCT ICC-ES CODE REPORTS SHALL BE INCLUDED WITH SUBMITTAL PACKAGE.
- ADHESIVE ANCHORS IN CONCRETE SHALL BE SIMPSON SET-XP CONFORMING TO ICC ESR-2508 OR HILTI HIT-RE200 CONFORMING TO ICC ESR-2932 OR APPROVED EQUAL.
- ADHESIVE ANCHORS IN GROUT FILLED CMU SHALL BE HILTI HY-210 MAX CONFORMING TO ICC ESR-4143 OR APPROVED EQUAL.
- EXPANSION ANCHORS IN CONCRETE SHALL BE SIMPSON STRONG-BOLT CONFORMING TO ICC ESR-1771 OR HILTI Kwik Bolt TZ CONFORMING TO ICC ESR-1911 OR APPROVED EQUAL.
- EXPANSION ANCHORS IN GROUT FILLED CMU SHALL BE HILTI Kwik Bolt 3 CONFORMING TO ICC ESR-1935 OR APPROVED EQUAL.
- POWER DRIVEN FASTENERS (PDFS) SHALL CONFORM TO ICC ESR-2130 OR APPROVED EQUAL.

POST INSTALLED FASTENERS

- POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE DRAWINGS. CONTRACTOR SHALL OBTAIN APPROVAL FROM ENGINEER OF RECORD (EOR) PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED

NO.	DATE	DESCRIPTION

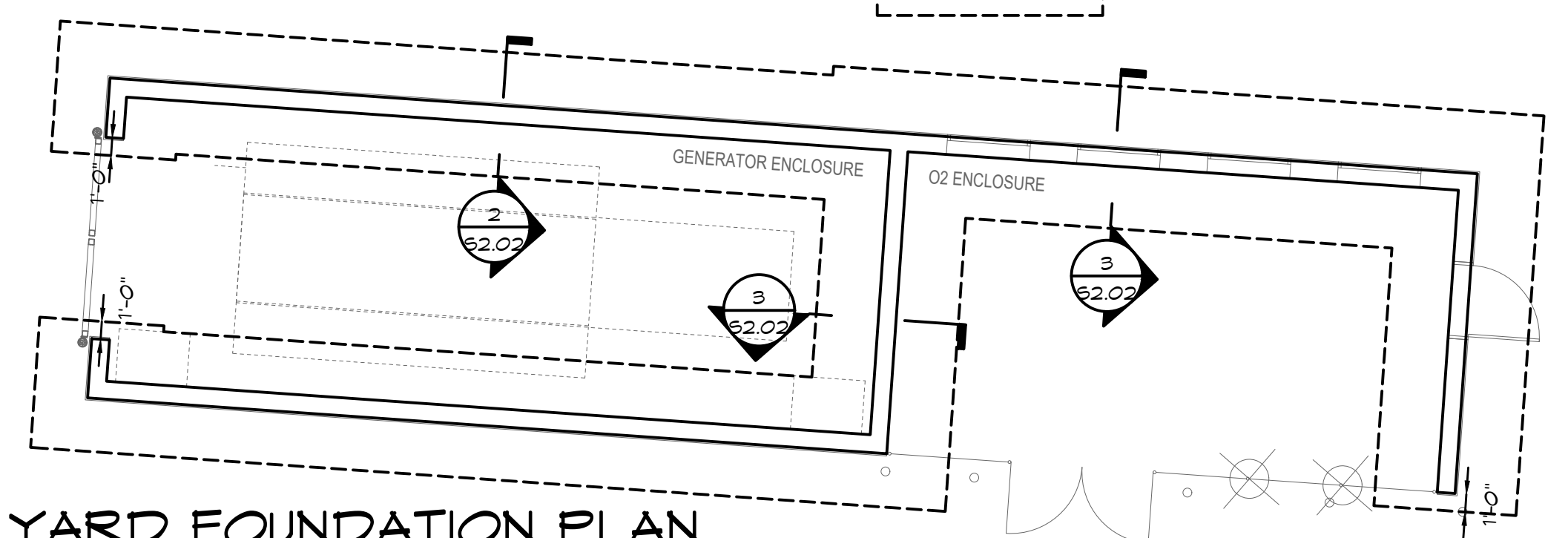


DUMPSTER YARD FOUNDATION PLAN

SCALE: 1/8"=1'-0"

NOTES:

- SEE SHEET S1.00 FOR GENERAL NOTES.
- SEE ARCH AND CIVIL FOR LOCATION.

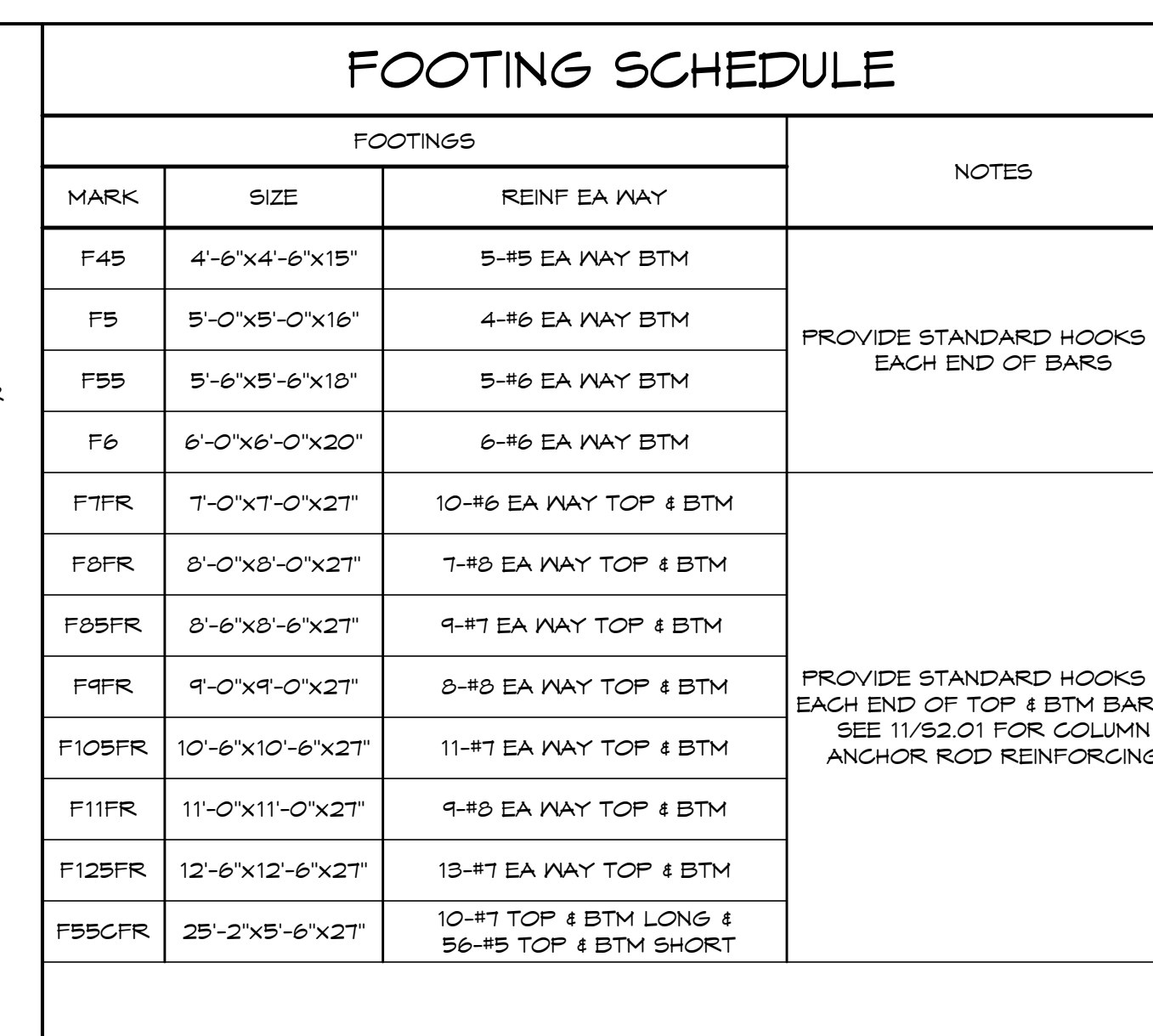


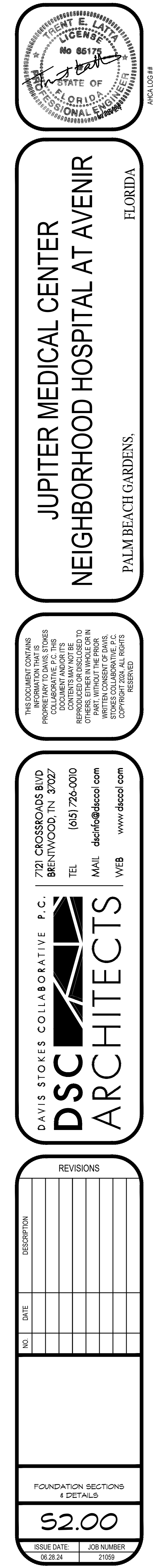
GENSET & BULK O2 YARD FOUNDATION PLAN

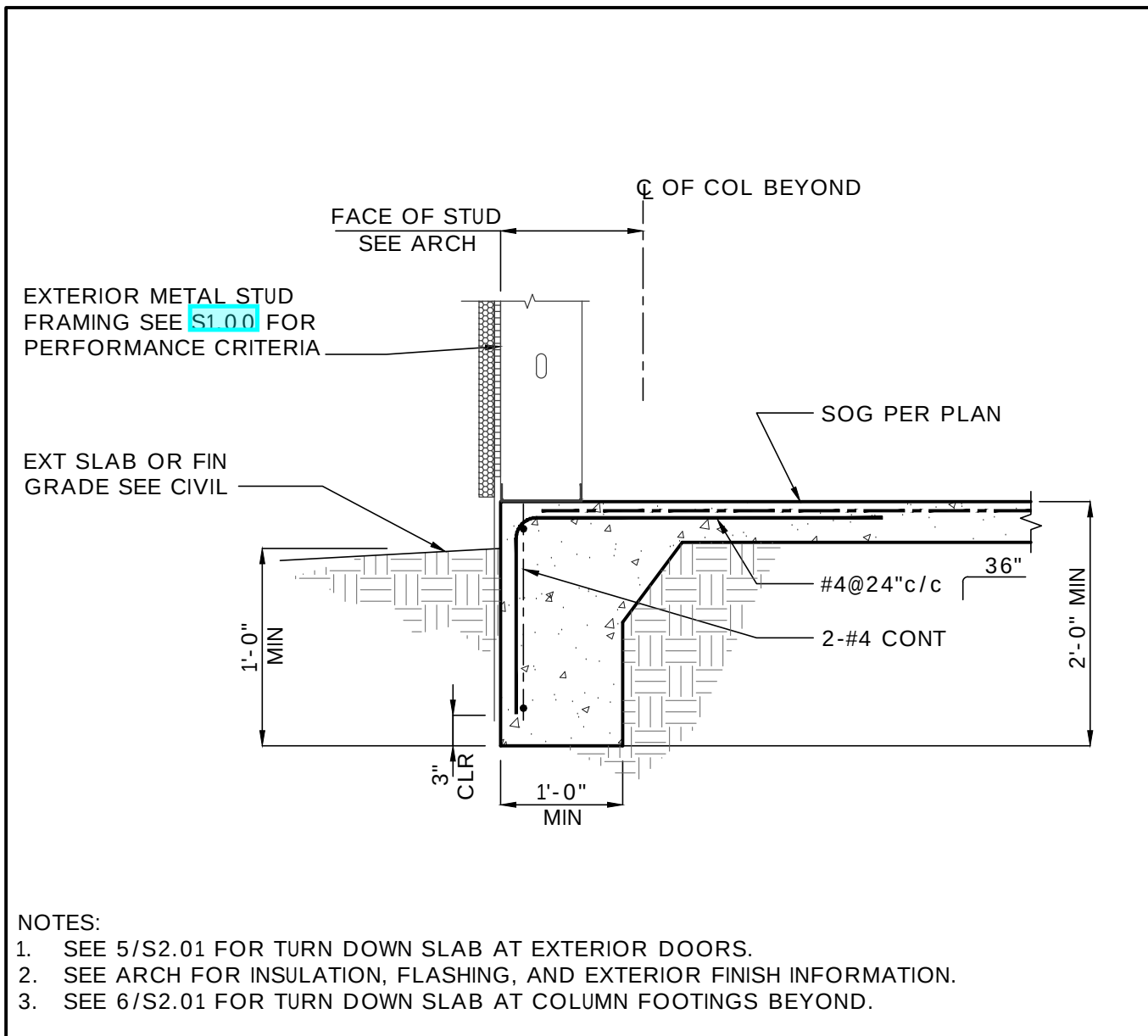
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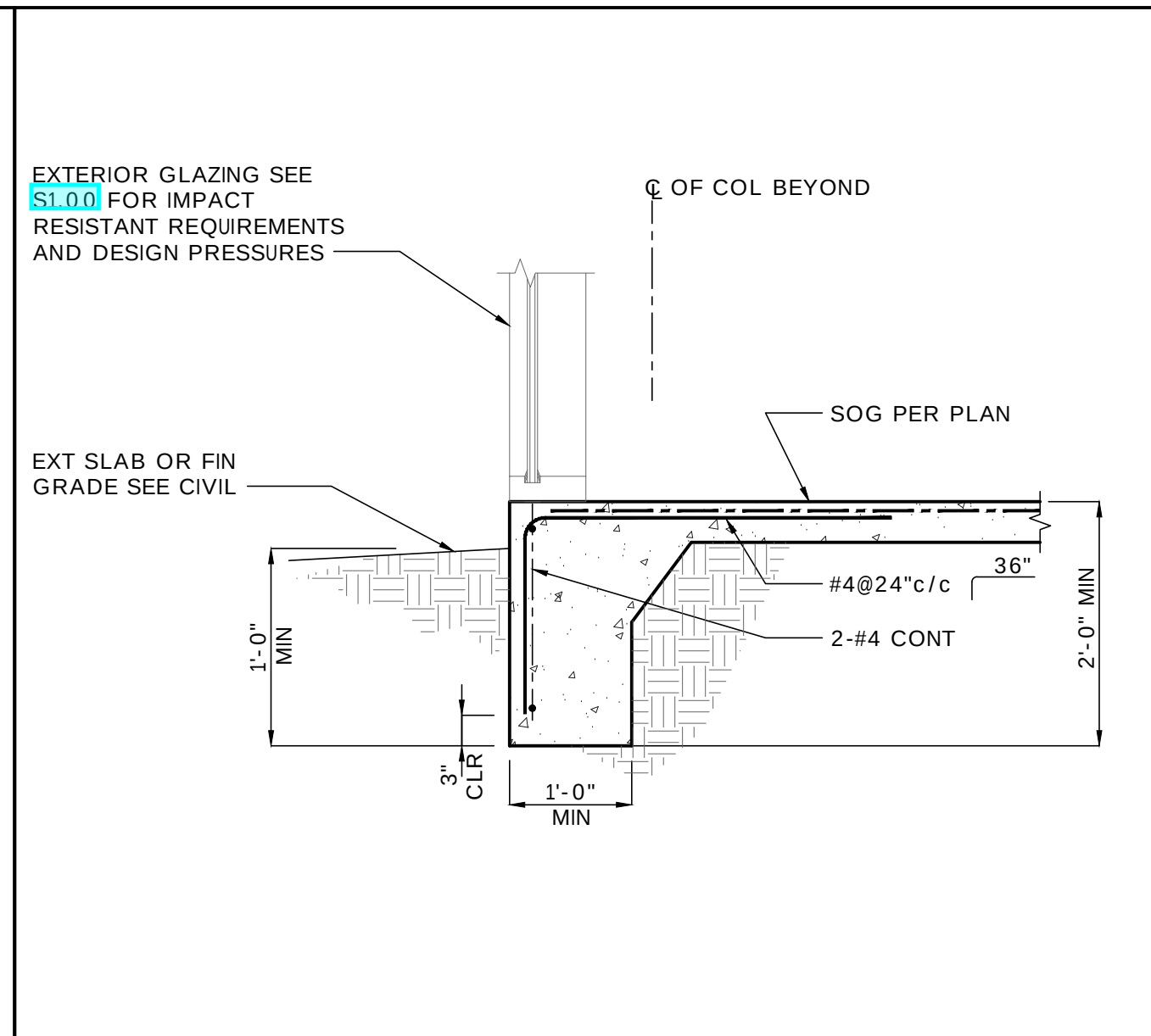
- SEE SHEET S1.00 FOR GENERAL NOTES.
- SEE ARCH AND CIVIL FOR LOCATION.
- SEE PROTECTION OF SYSTEMS AND UTILITIES NOTES ON S1.00 FOR REMAINDER OF INFO.

[illegible]

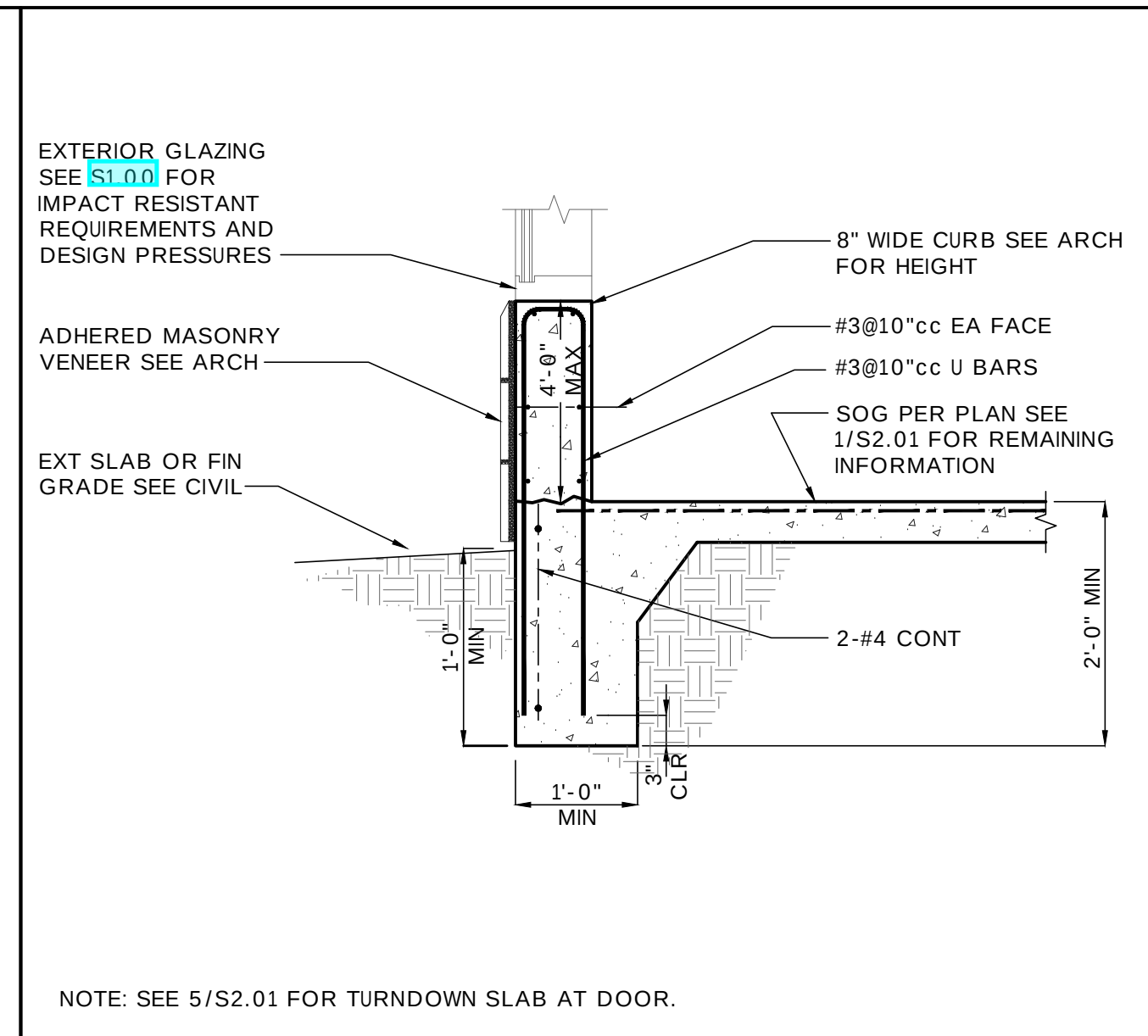




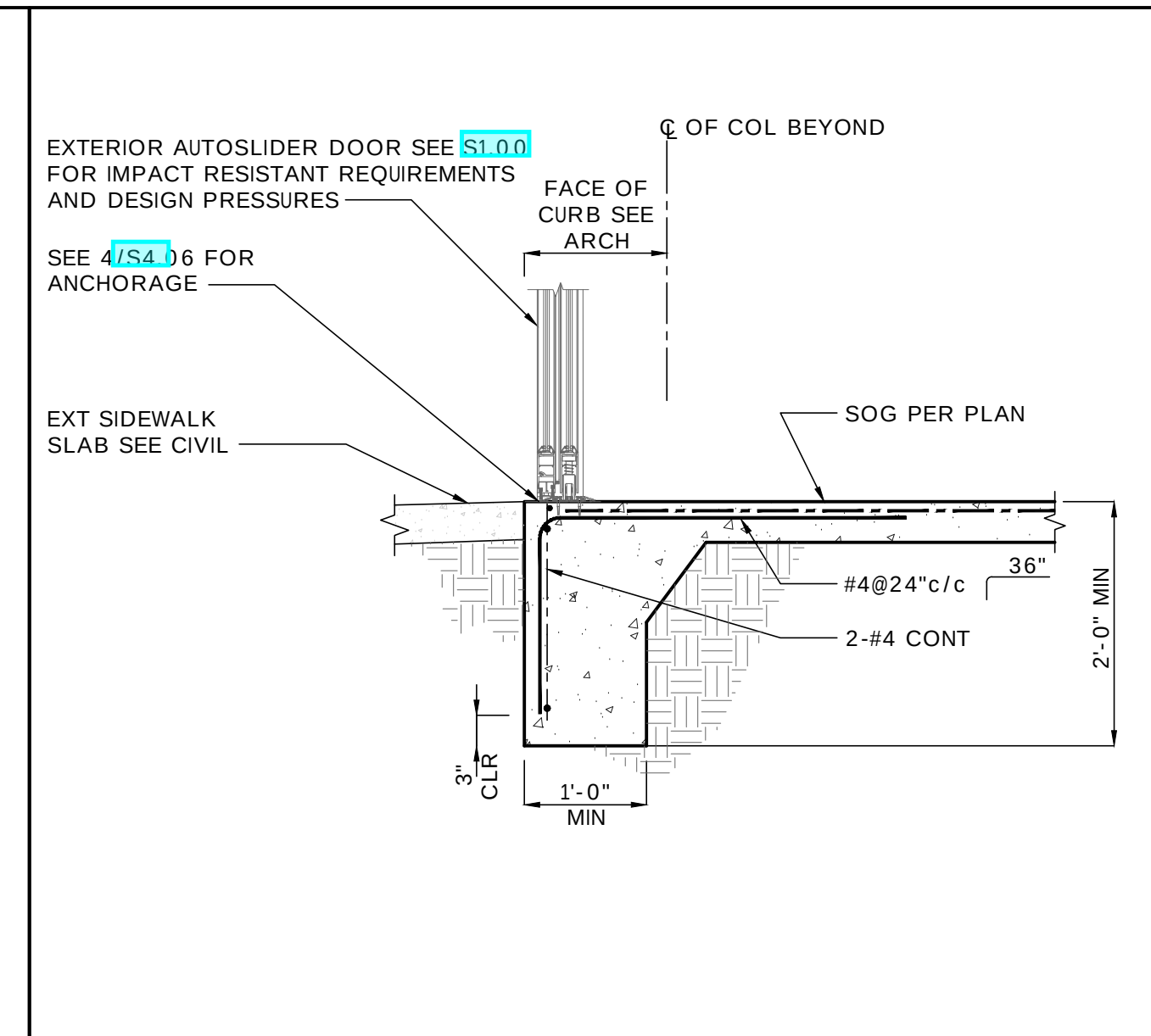
1 TYP PERIMETER TURNDOWN SLAB EDGE



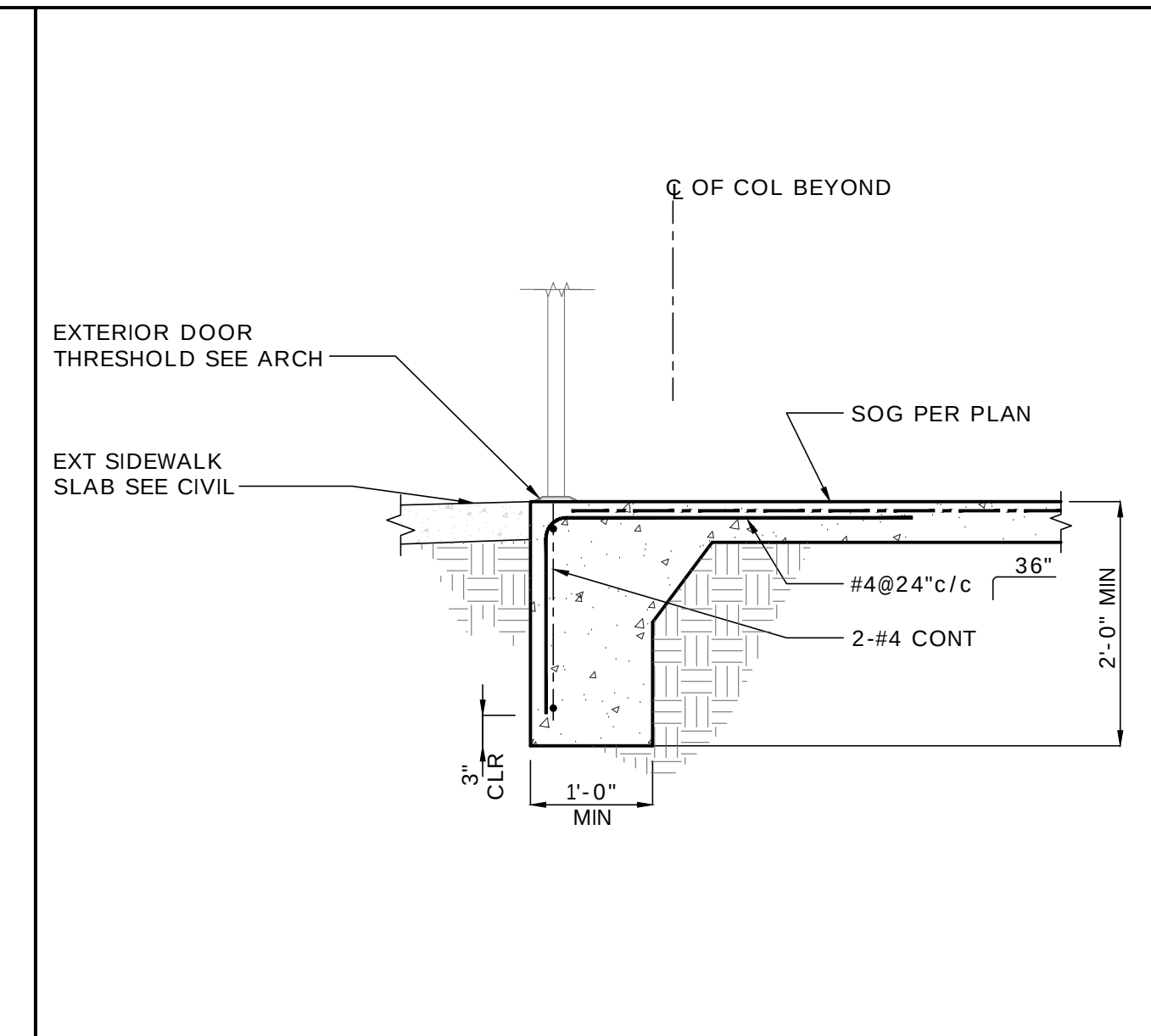
2 TYP PERIMETER TURNDOWN SLAB EDGE AT CURTAINWALL



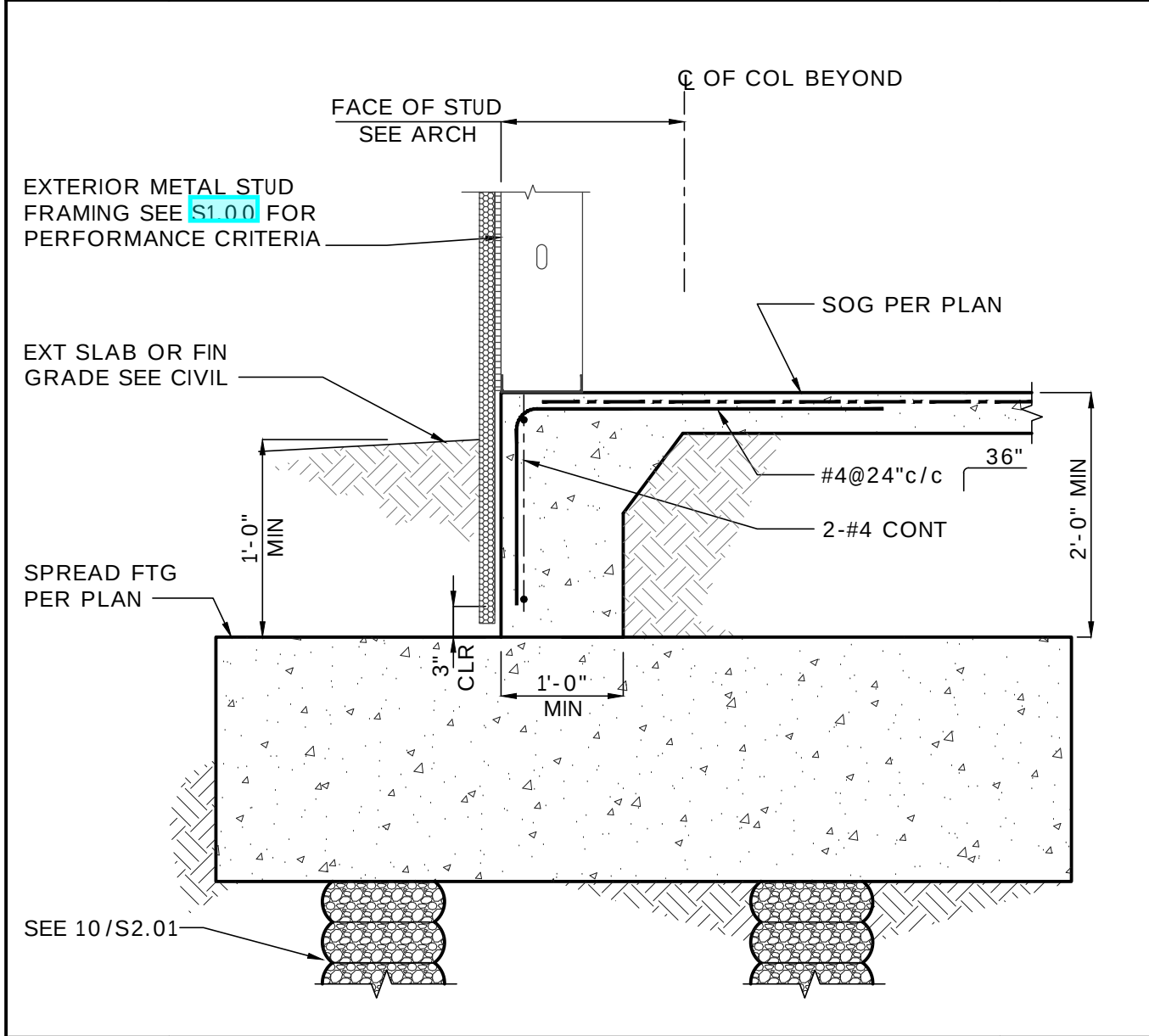
3 TYP PERIMETER TURNDOWN & CURB AT CURTAINWALL



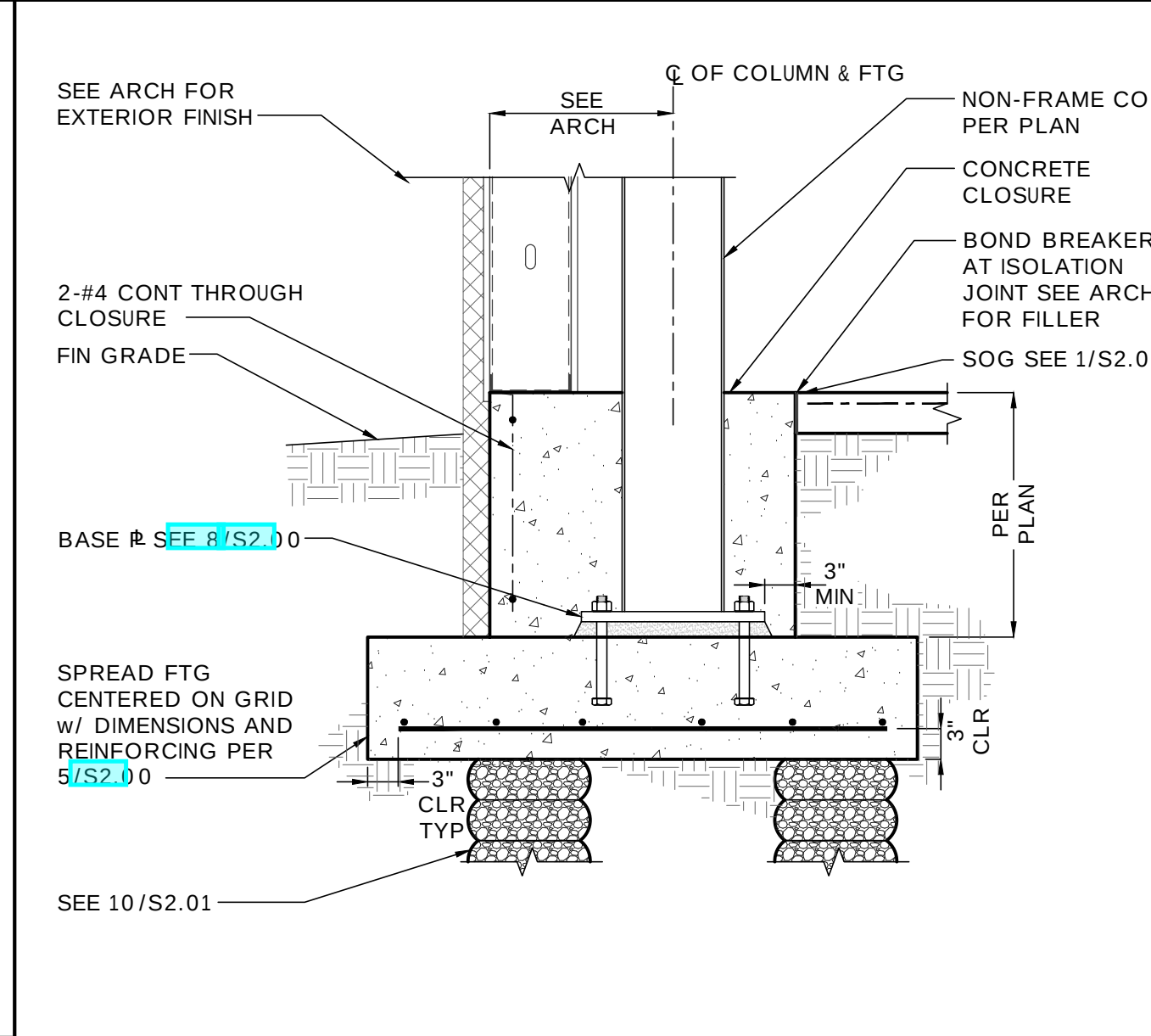
4 PERIMETER TURNDOWN AT AUTO-SLIDER EXTERIOR DOOR



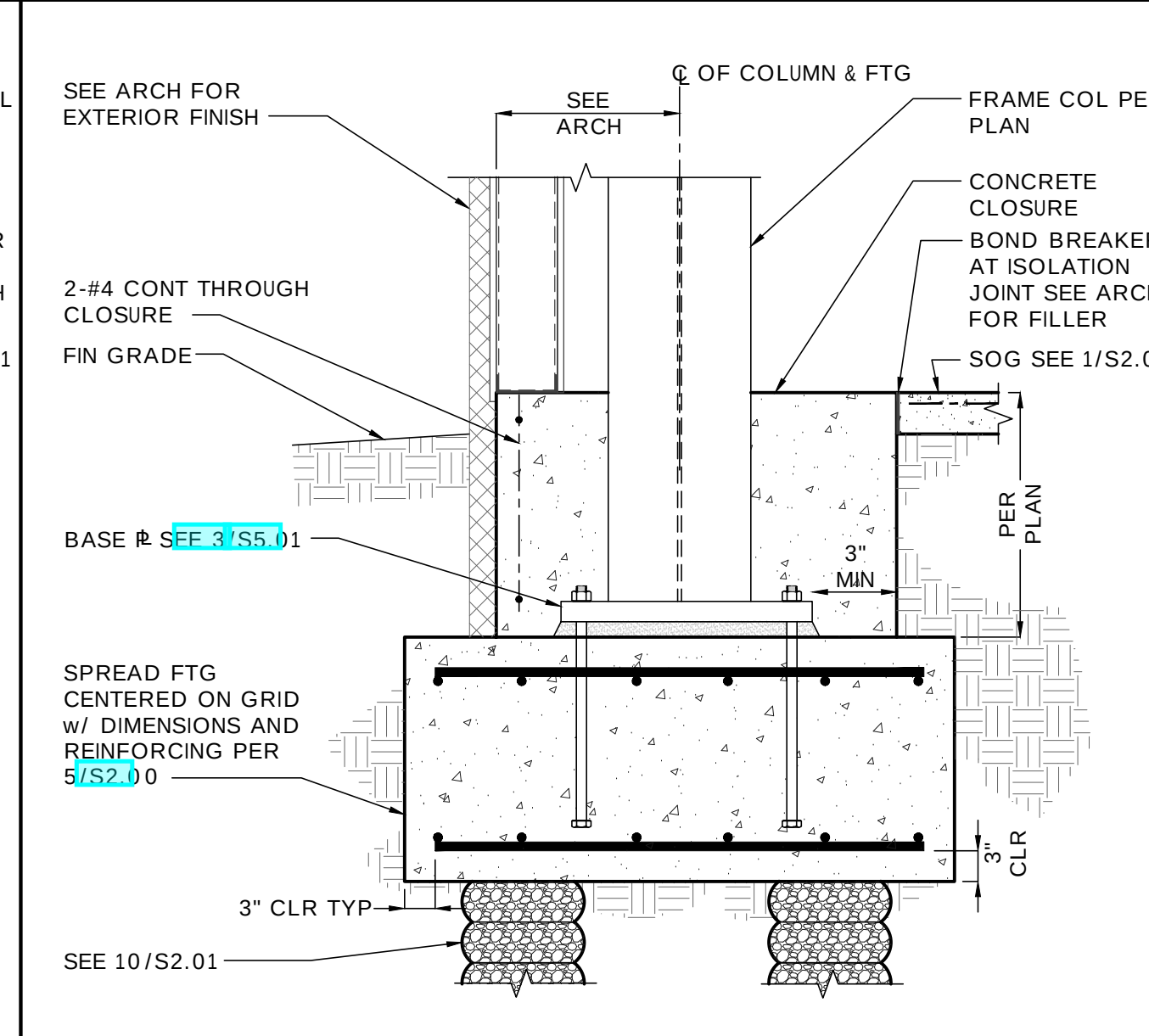
5 PERIMETER TURNDOWN AT EXTERIOR DOOR



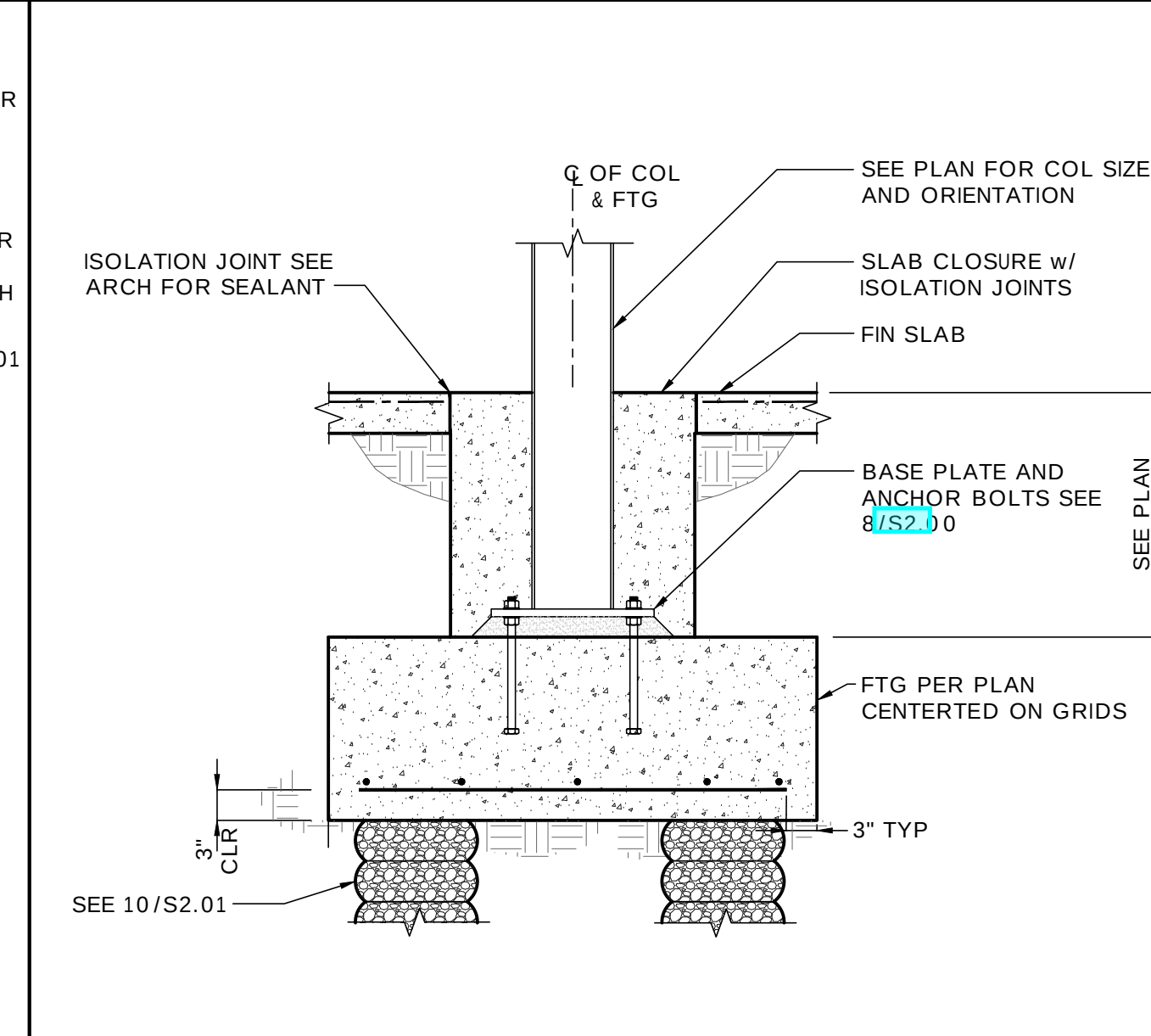
6 TURN DOWN SLAB EDGE AT COLUMN FOOTING



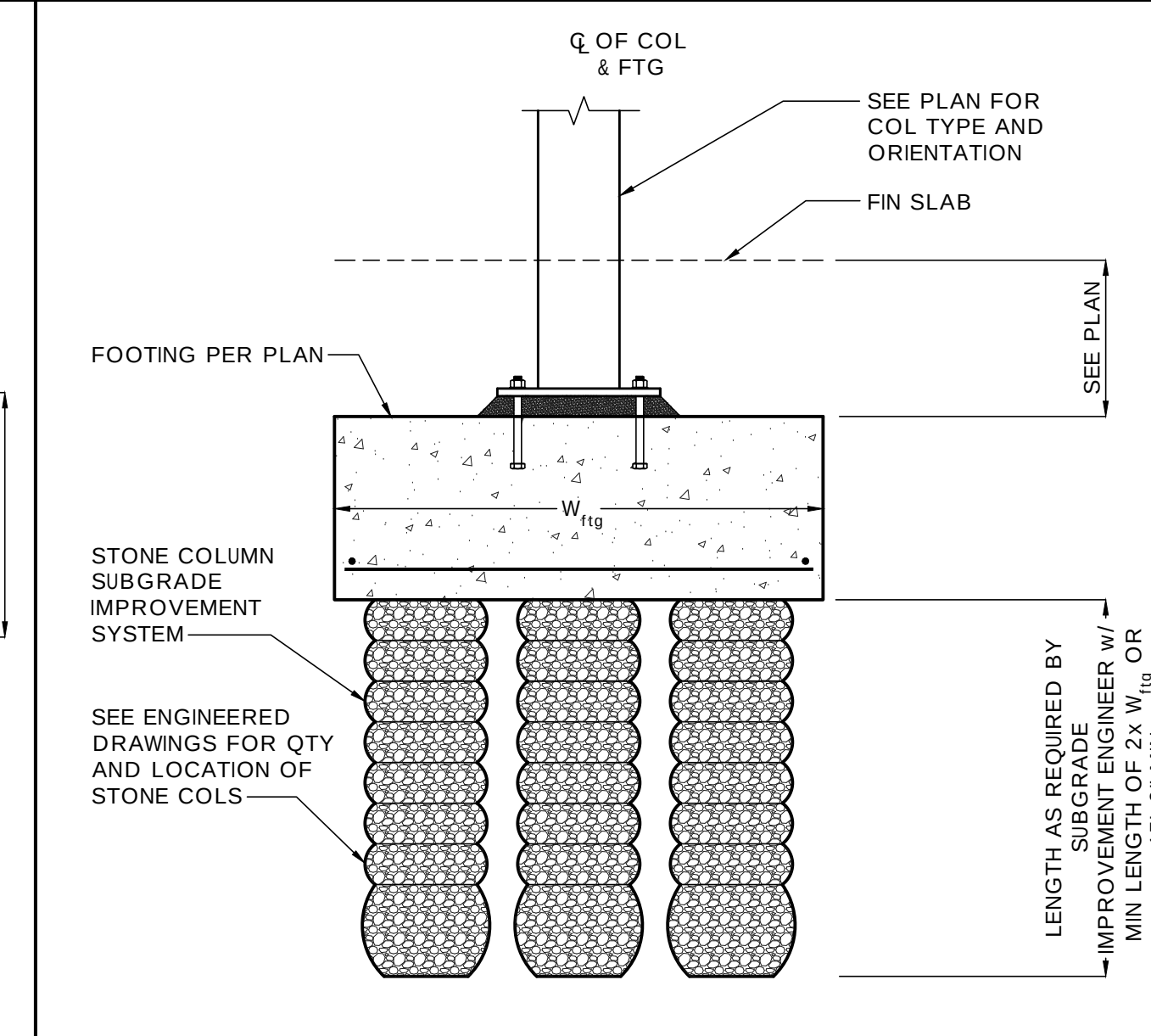
7 PERIMETER NON-FRAME COLUMN AT FOUNDATION



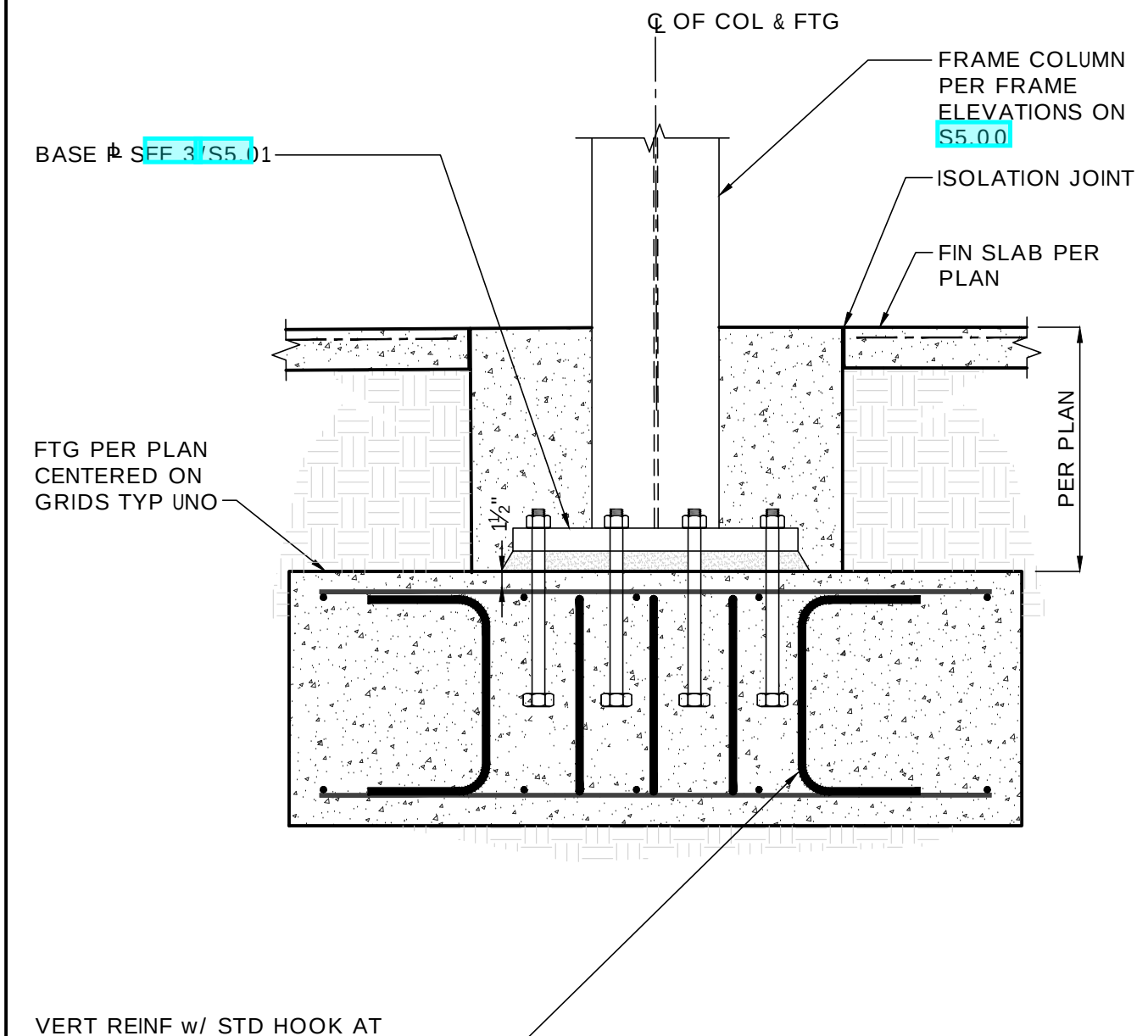
8 PERIMETER FRAME COLUMN AT FOUNDATION



9 TYPICAL INTERIOR NON-FRAME COLUMN FOOTING



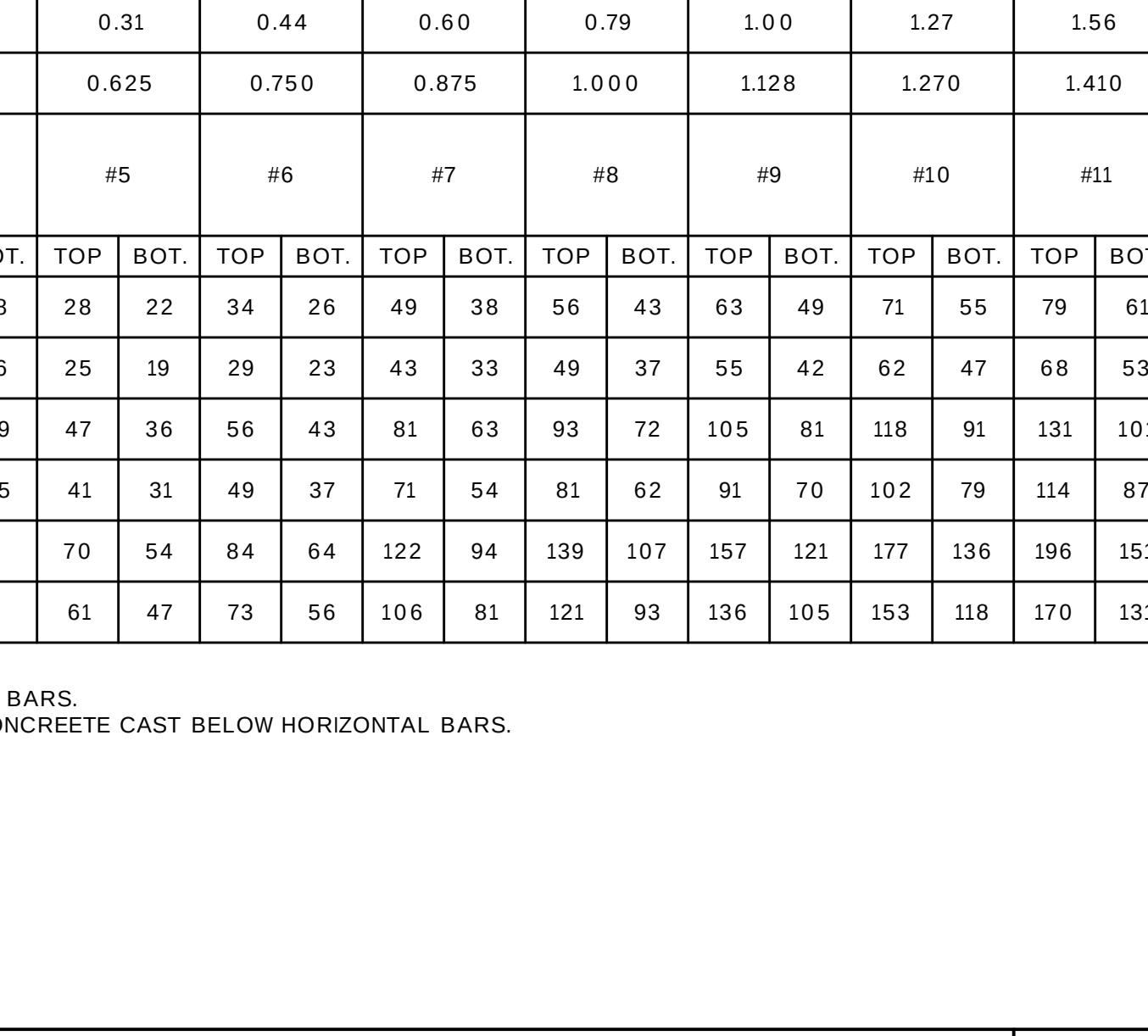
10 TYPICAL COLUMN FOOTING w/ IMPROVED SOIL



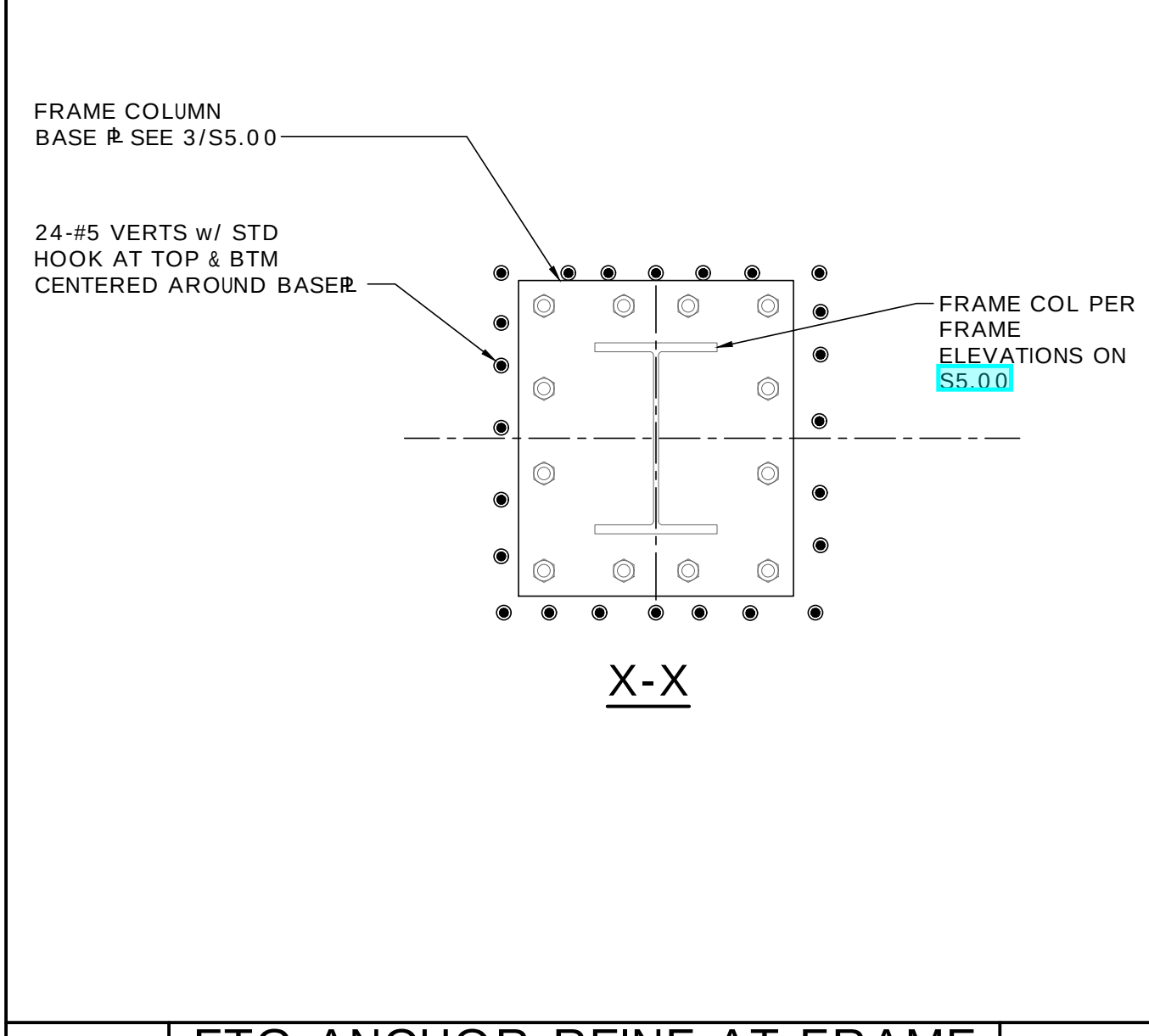
11 FTG ANCHOR REINF AT FRAME COL

BAR			AREA (in ²)	0.11	0.20		
			DIAMETER (d _b)	0.375	0.500		
LAP CLASS	CATEGORY	DESCRIPTION	NORMAL WEIGHT CONCRETE f _c (PSI)	#3	#4		
CLASS "B"	1	COVER ≥ 2d _b & CLR SPC ≥ 4 d _b	3000	TOP	BOT.	TOP	BOT.
			4000	21	16	21	16
	2	ALL OTHERS	3000	28	22	38	29
			4000	25	19	33	25
	3	COVER < d _b OR CLR SPC < 2 d _b	3000	-	-	-	-
			4000	-	-	-	-
	NOTES:						
	1. TOP BARS ARE HORIZONTAL BARS w/ MORE THAN 12" OF CONCRETE CAST BELOW THE						
2. BOTTOM BARS ARE ALL VERTICAL BARS & HORIZONTAL BARS w/ LESS THAN 12" OF CO							

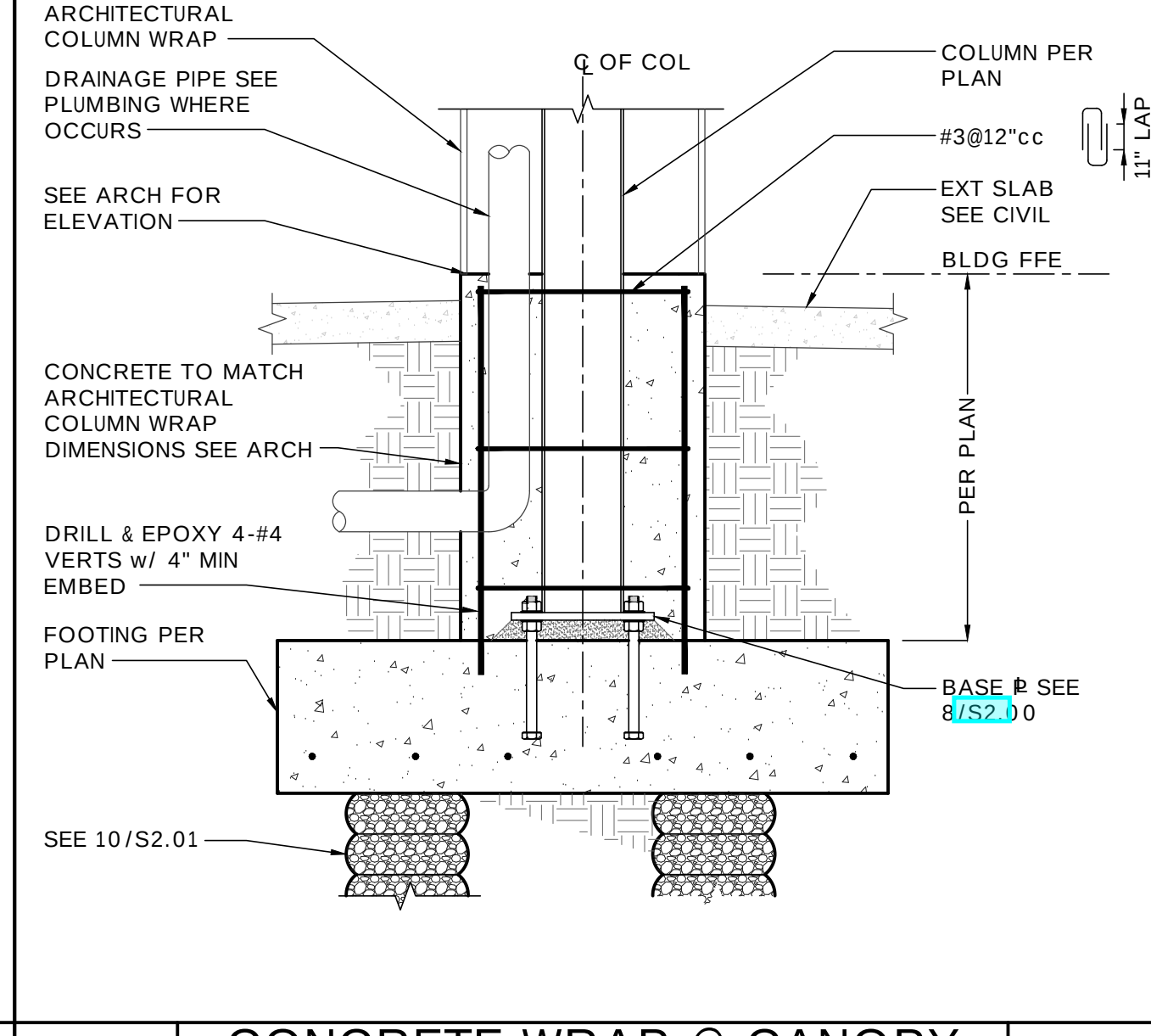
12 TYPICAL REINFORCING GRADE 60 SPLICE SCHEDULE



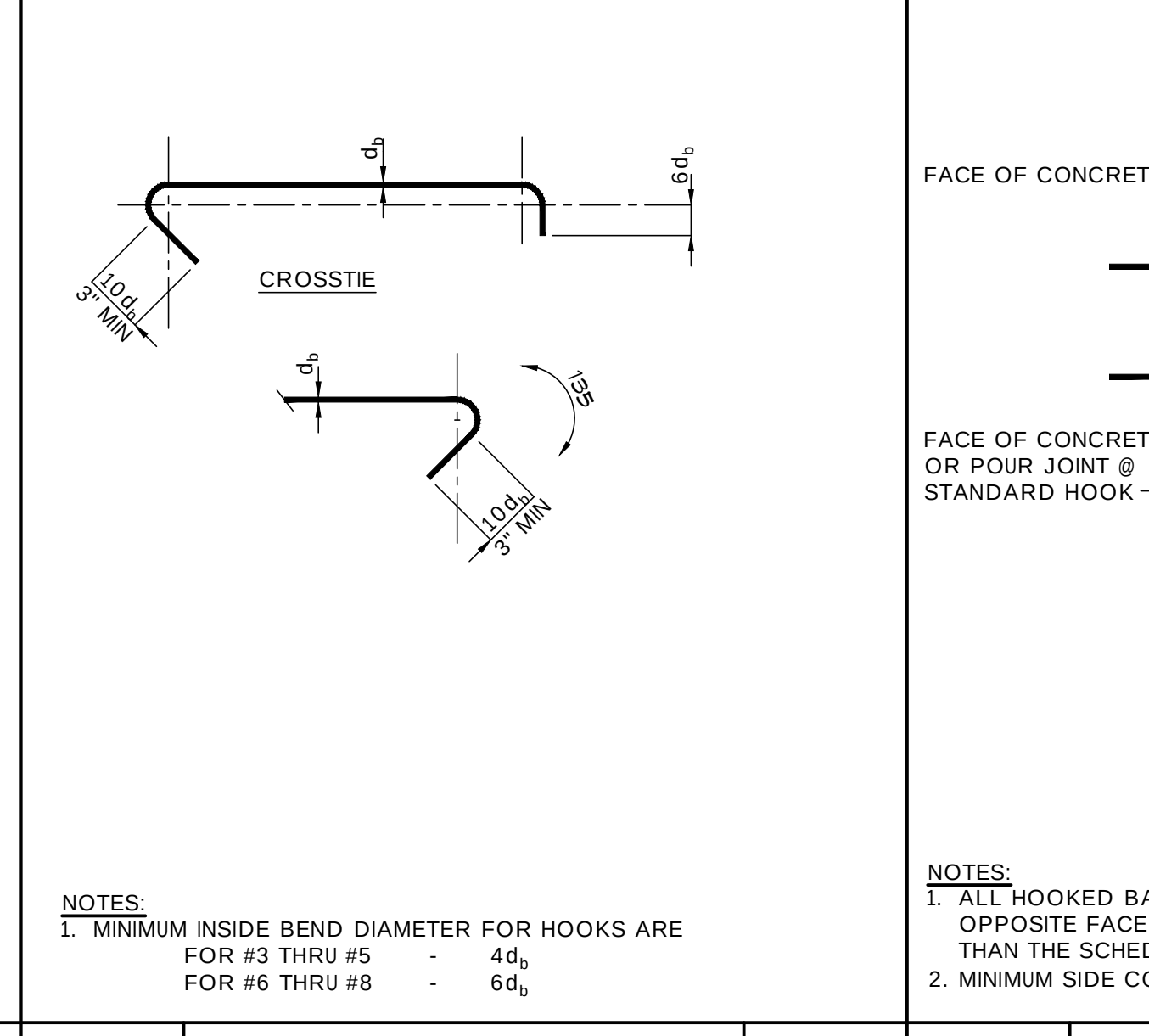
13 TYP STRAIGHT AND HOOKED EMBEDMENT LENGTH SCHEDULE



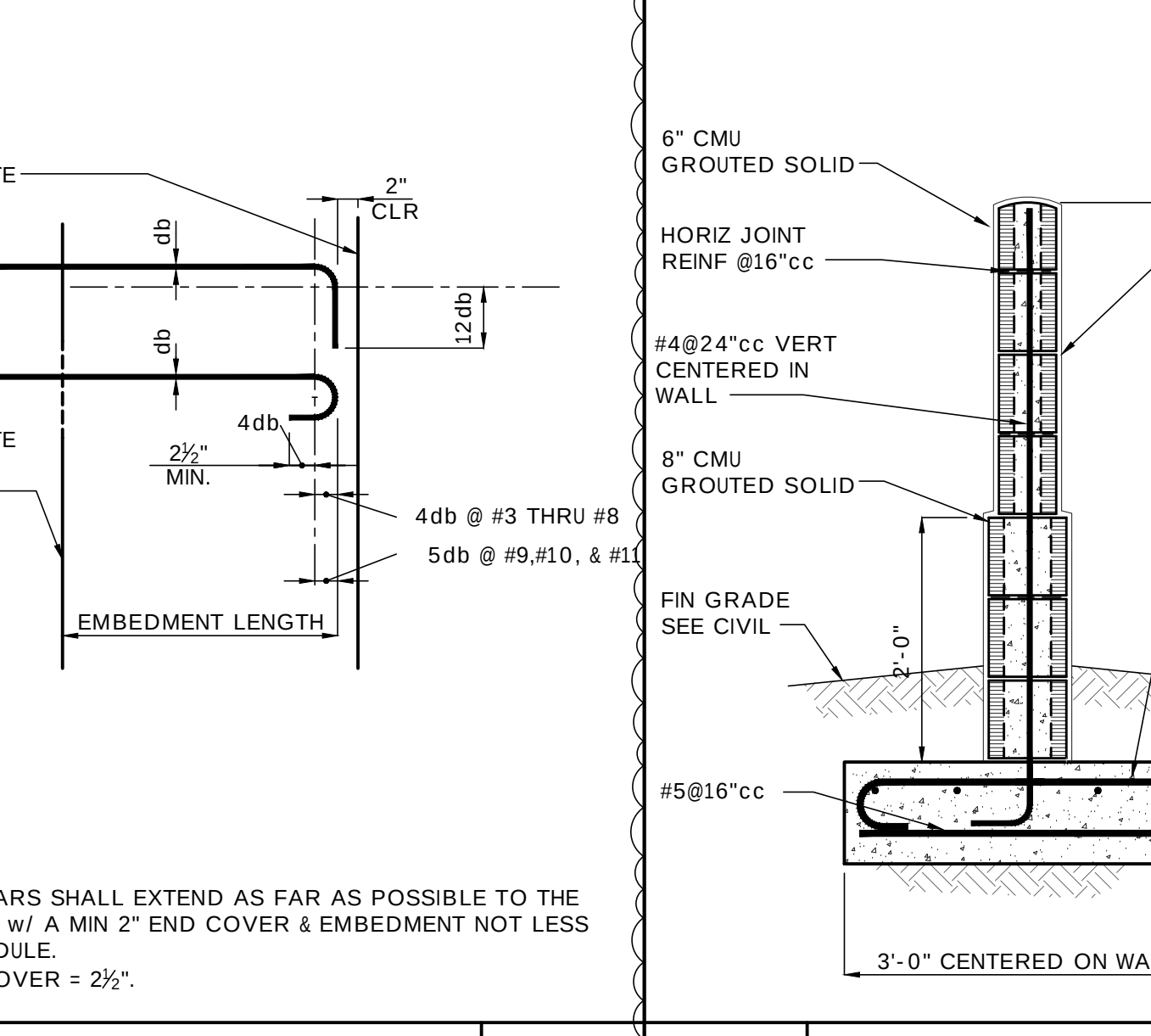
14 CONCRETE WRAP @ CANOPY COLUMN



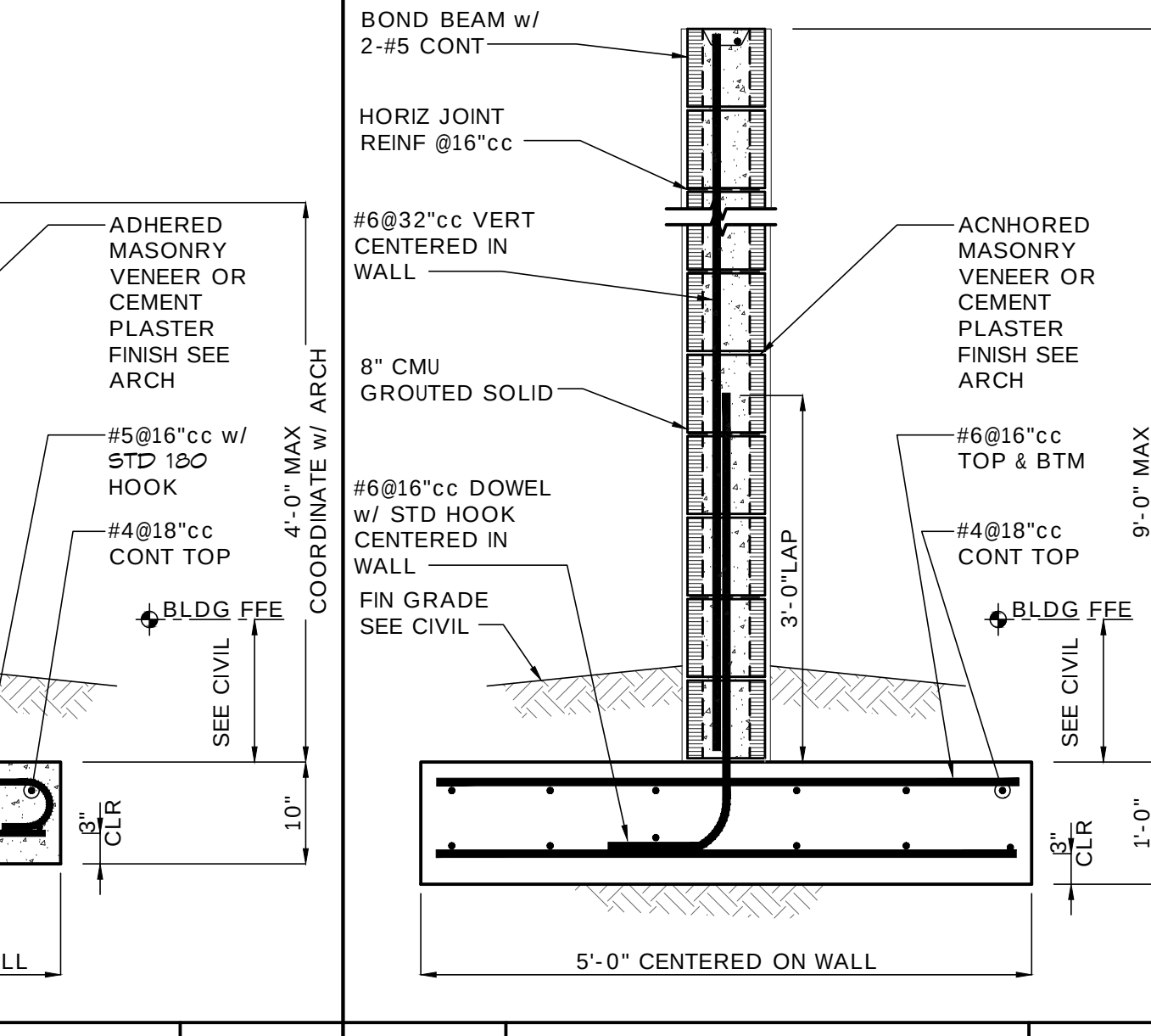
15 TYP STD HOOK DETAILS FOR STIRRUPS, CROSSTIES & TIES



16 TYP HOOK DETAILS



17 ARCHED SIGN



18 SIGN

FLORIDA

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NEIGHBORHOOD HOSPITAL AT AVENIR
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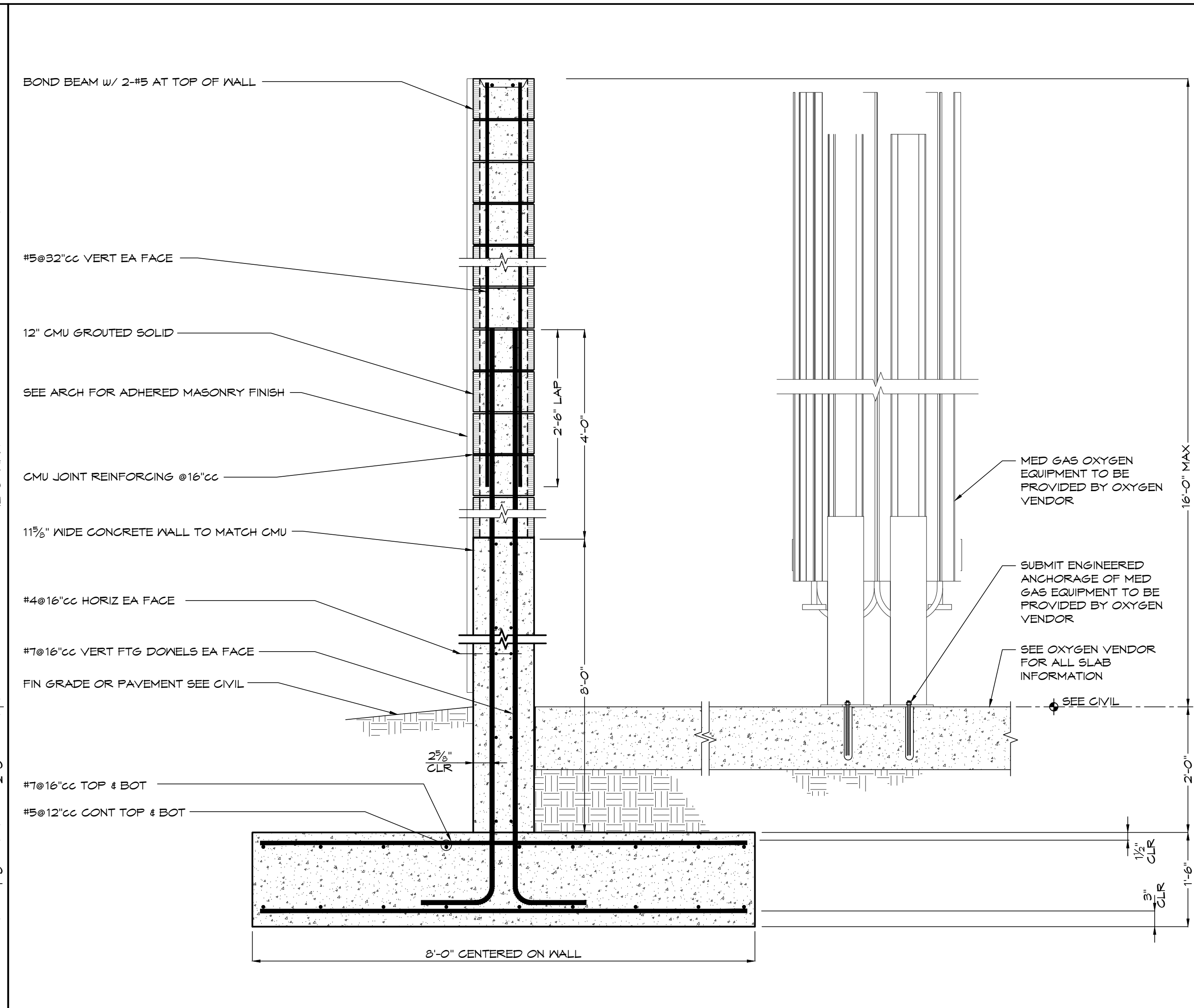
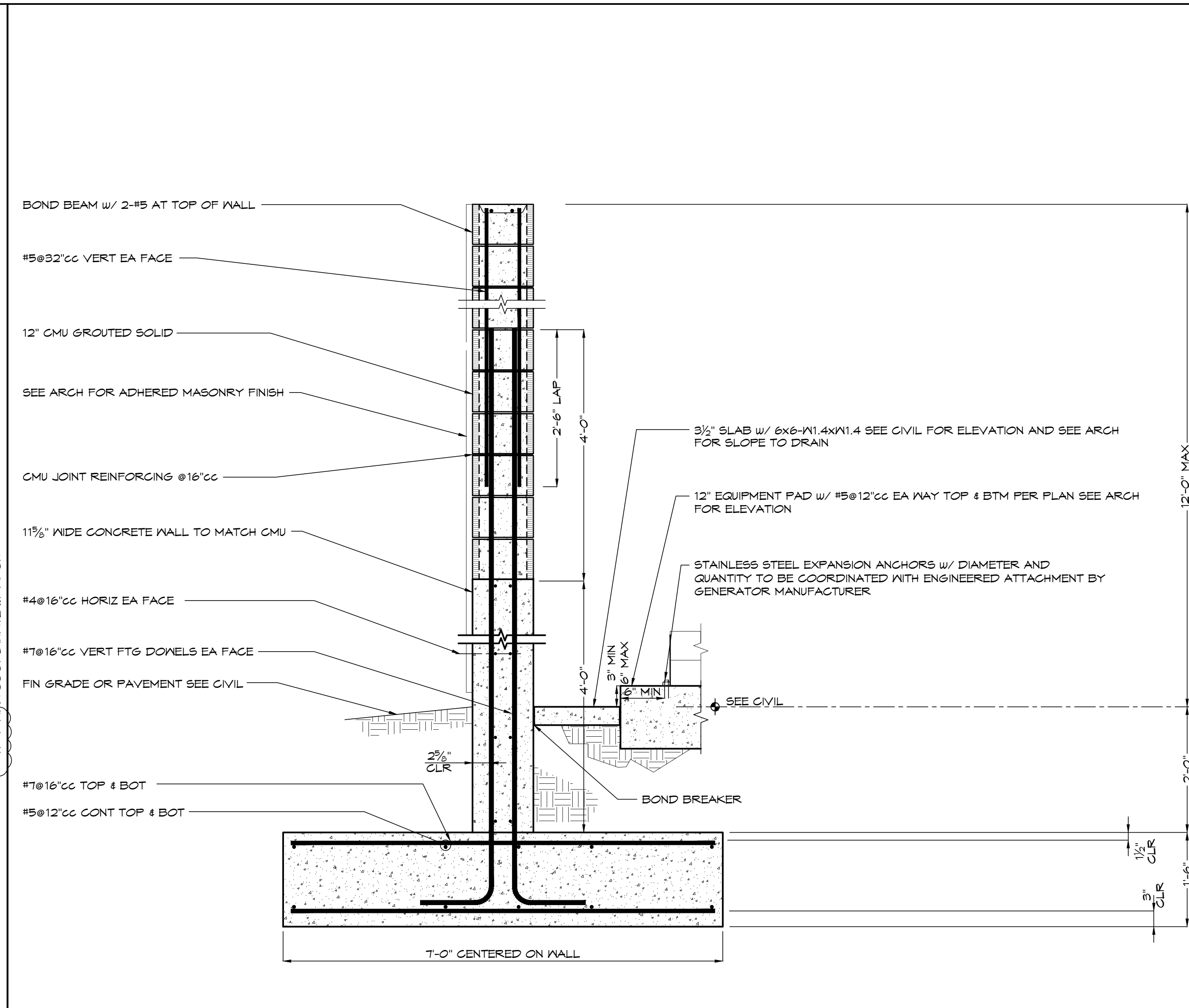
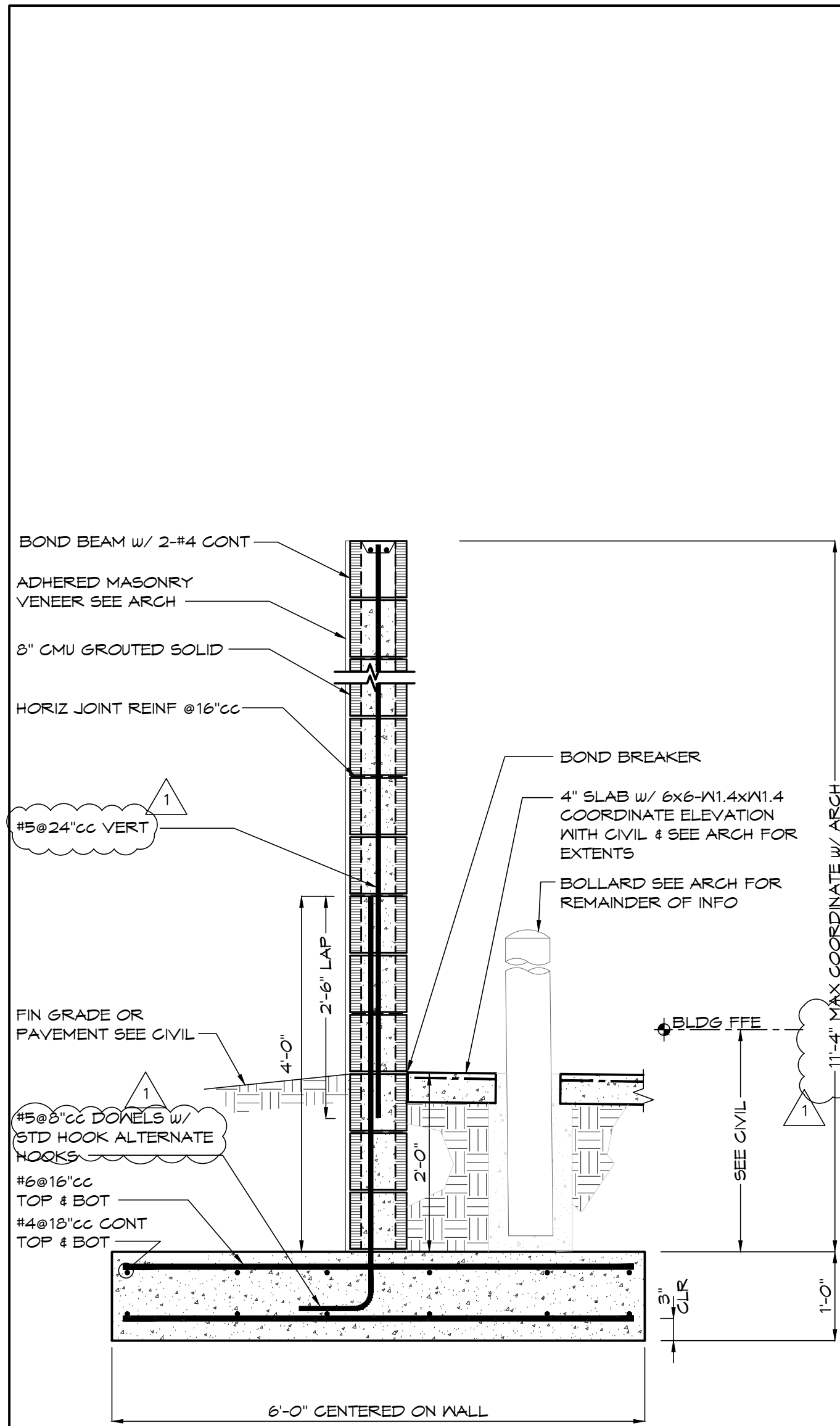
REVISIONS

NO.	DATE	DESCRIPTION
3	11/24	CCD 1002

FOUNDATION SECTIONS & DETAILS

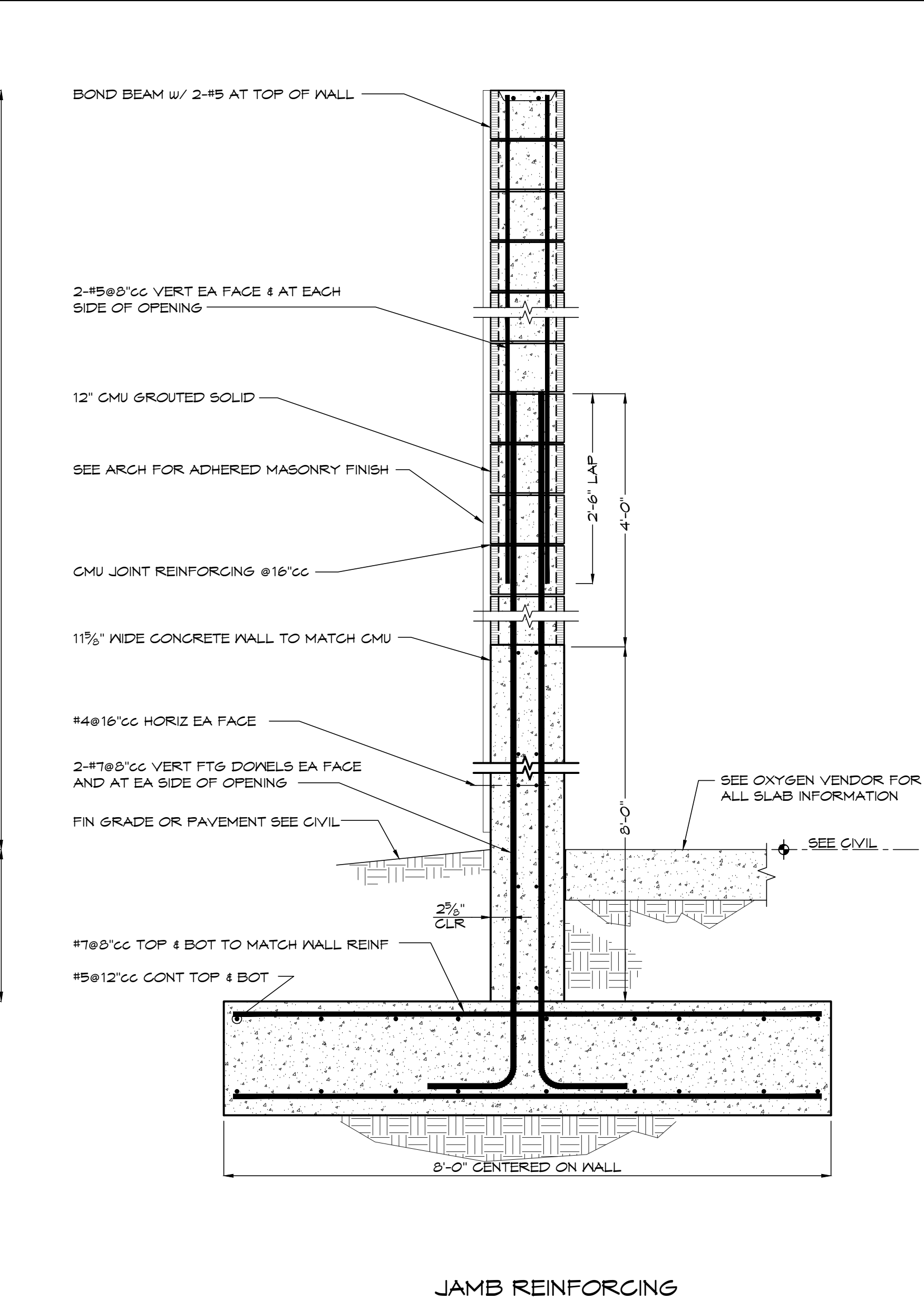
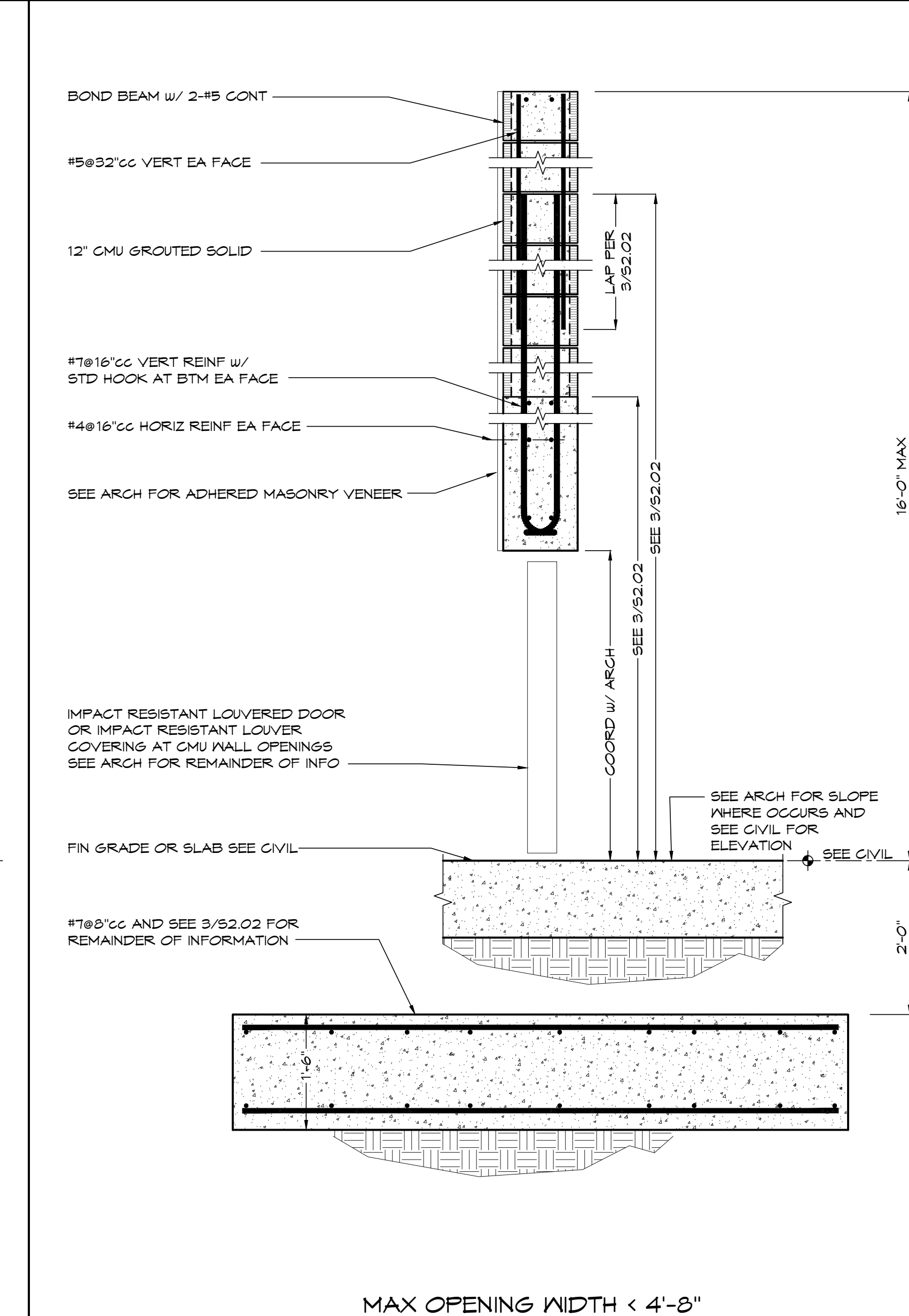
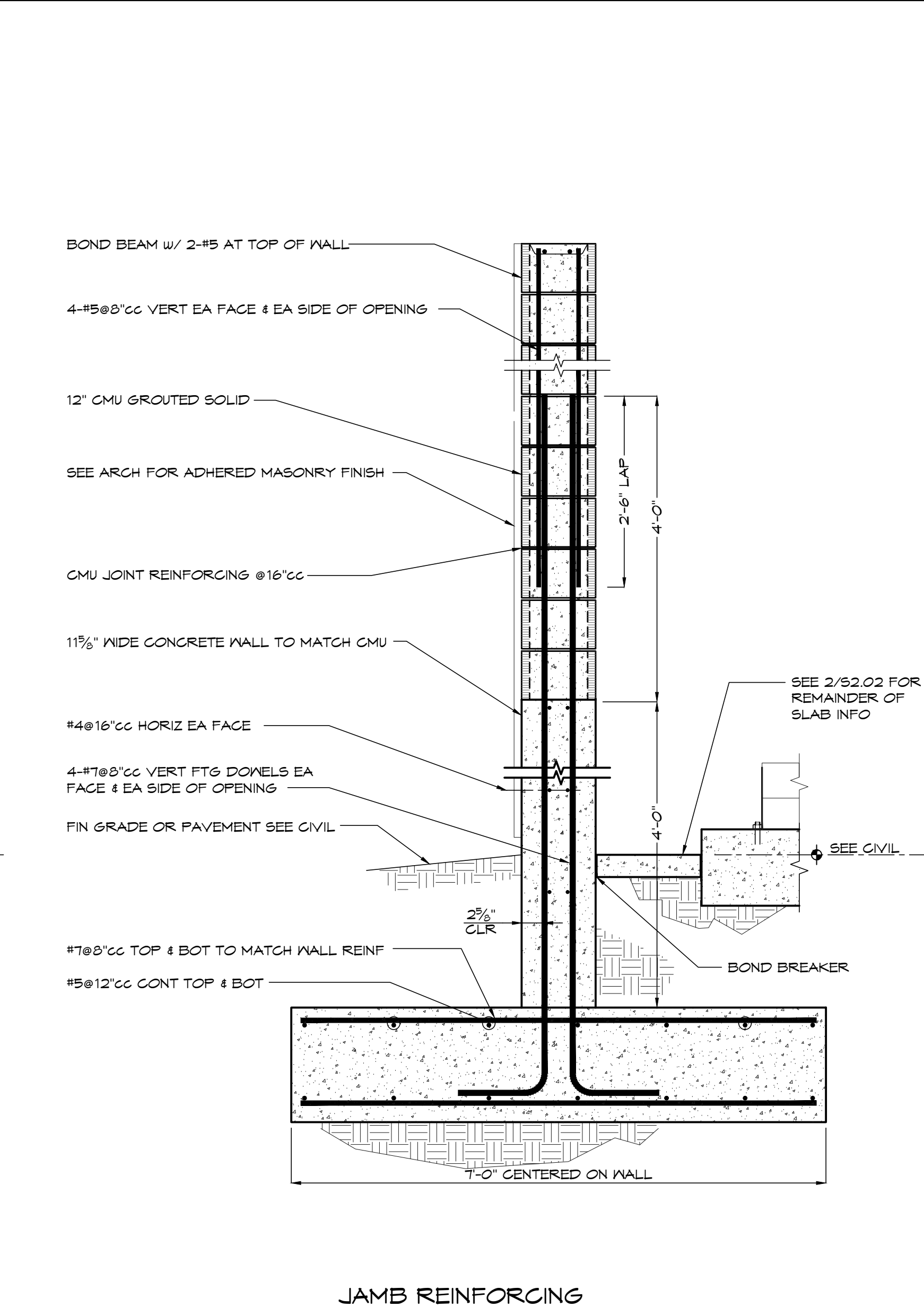
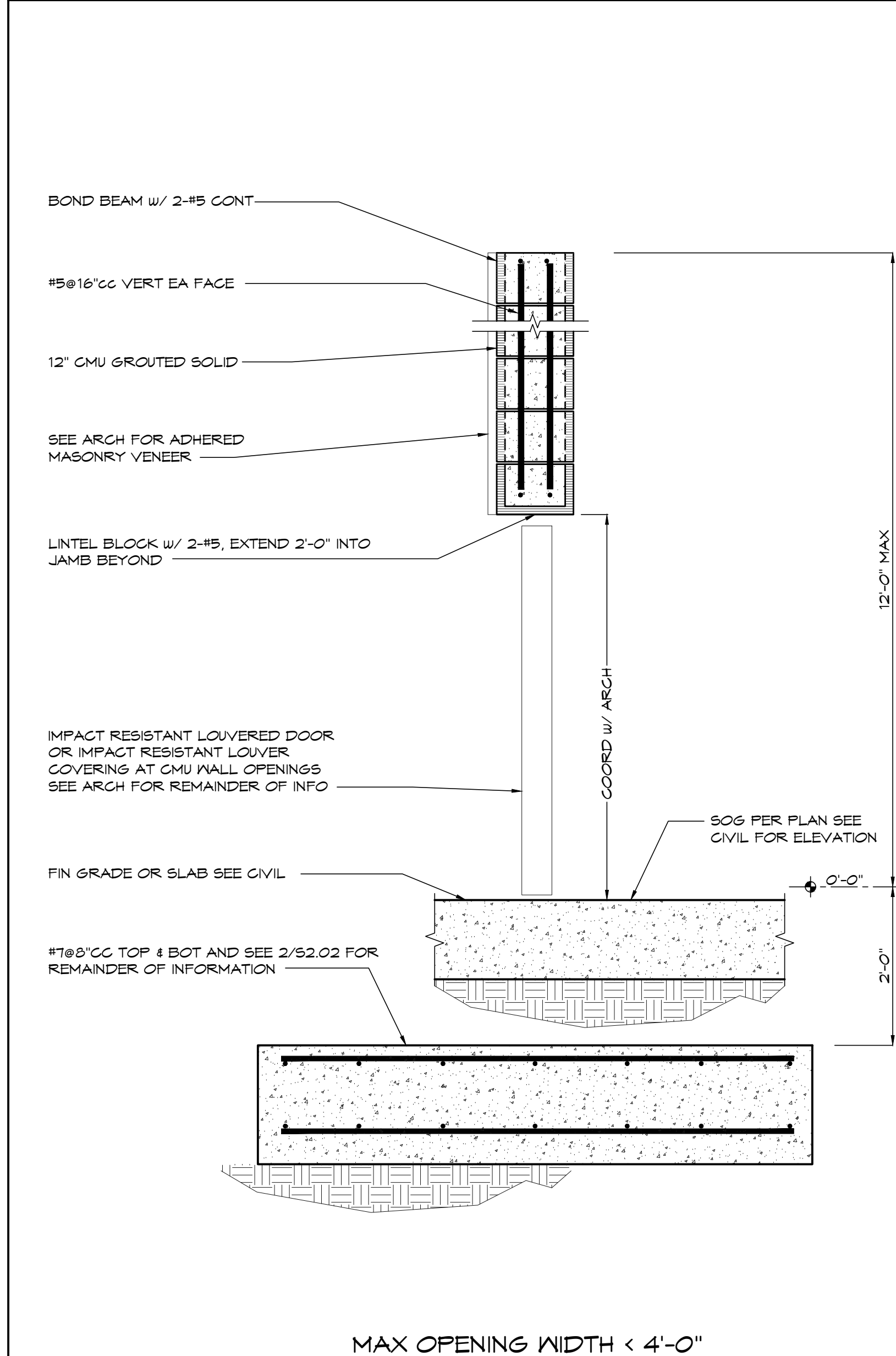
S2.01

ISSUE DATE: 07/24/24 JOB NUMBER: 21059



- NOTES:
1. SEE 5/52.02 FOR LINTEL AND JAMB REINFORCING AT WALL OPENINGS.
 2. SEE ARCH FOR DIMENSIONS, OPENINGS AND LOCATIONS OF WALLS.
 3. THE GENERATOR, FUEL SUPPLY AND ALL FUEL LINES OF THE EMERGENCY POWER SYSTEM GENERATOR, PANEL BOARDS, TRANSFER SWITCHES, DISCONNECT SWITCHES, ENCLOSED CIRCUIT BREAKERS OR EMERGENCY SYSTEM RACEWAY SYSTEMS REQUIRED TO SUPPORT THE OCCUPIED PATIENT AREAS, PATIENT SUPPORT AREAS OR SUPPORT UTILITIES SHALL BE CONTAINED WITHIN PROTECTED AREAS DESIGNED AND CONSTRUCTED TO MEET THE STRUCTURAL REQUIREMENTS OF THE CODE AND DEBRIS IMPACT REQUIREMENTS INCLUDING LOUVERS AND DOORS.
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1 CMU SCREEN AT DUMPSTER YARD 2 CMU SCREEN AT GENERATOR 3 CMU SCREEN AT BULK OXYGEN



5 CMU SCREEN OPENING AT GENERATOR 6 CMU SCREEN OPENING AT BULK OXYGEN

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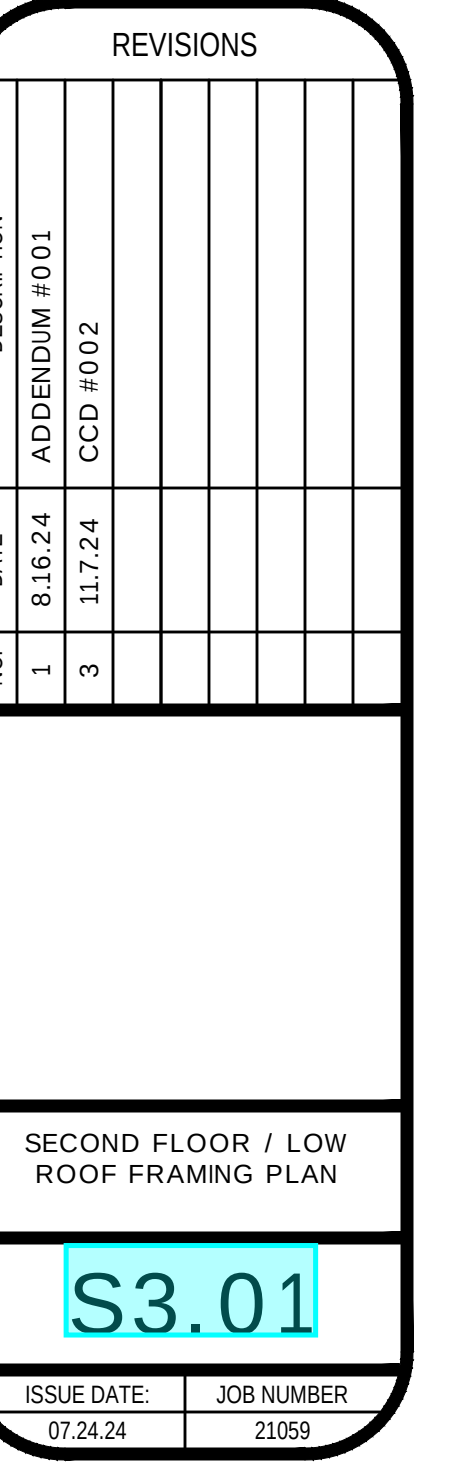
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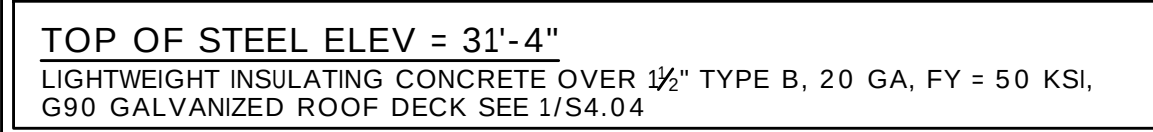
REVISIONS

NO.	DATE	DESCRIPTION
1	5/16/24	APPENDIX 1001

FOUNDATION SECTIONS & DETAILS
S2.02
 ISSUE DATE: 05.28.24 JOB NUMBER: 21059



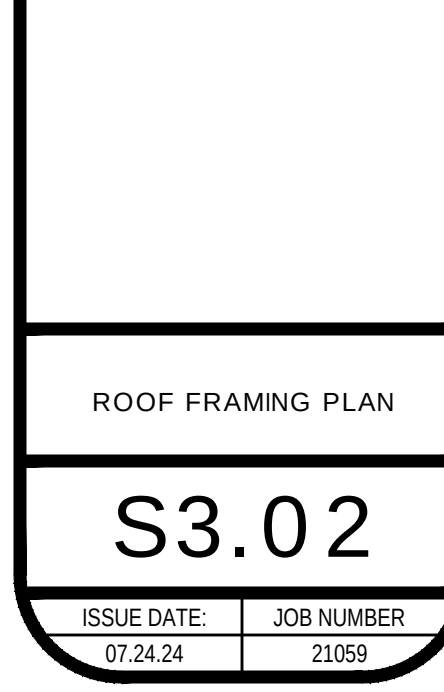
1. MAXIMUM WEIGHT INDICATED IS THE MAXIMUM OPERATING WEIGHT. SEE ARCH AND MEP FOR REMAINING REQUIREMENTS.
2. COORDINATE ANCHORAGE OR ROOF/OT MECHANICAL UNITS W/ MECH. PRIOR TO INSTALLING MECHANICAL EQUIPMENT. PROVIDE ENGINEERING CALCULATIONS DESIGNED FOR LOADS INDICATED IN THE BUILDING CODE. ANCHORAGE DETAILS FOR EQUIPMENT AND EQUIPMENT SUPPORTS, OR PROVIDE PRE-APPROVED ANCHORAGE DESIGNS. SUBMIT ITEMS FOR REVIEW AND APPROVAL TO THE ARCHITECT.
4. SEE PROTECTION OF SYSTEMS AND UTILITIES NOTES ON S1.0 FOR REMAINDER OF INFO.

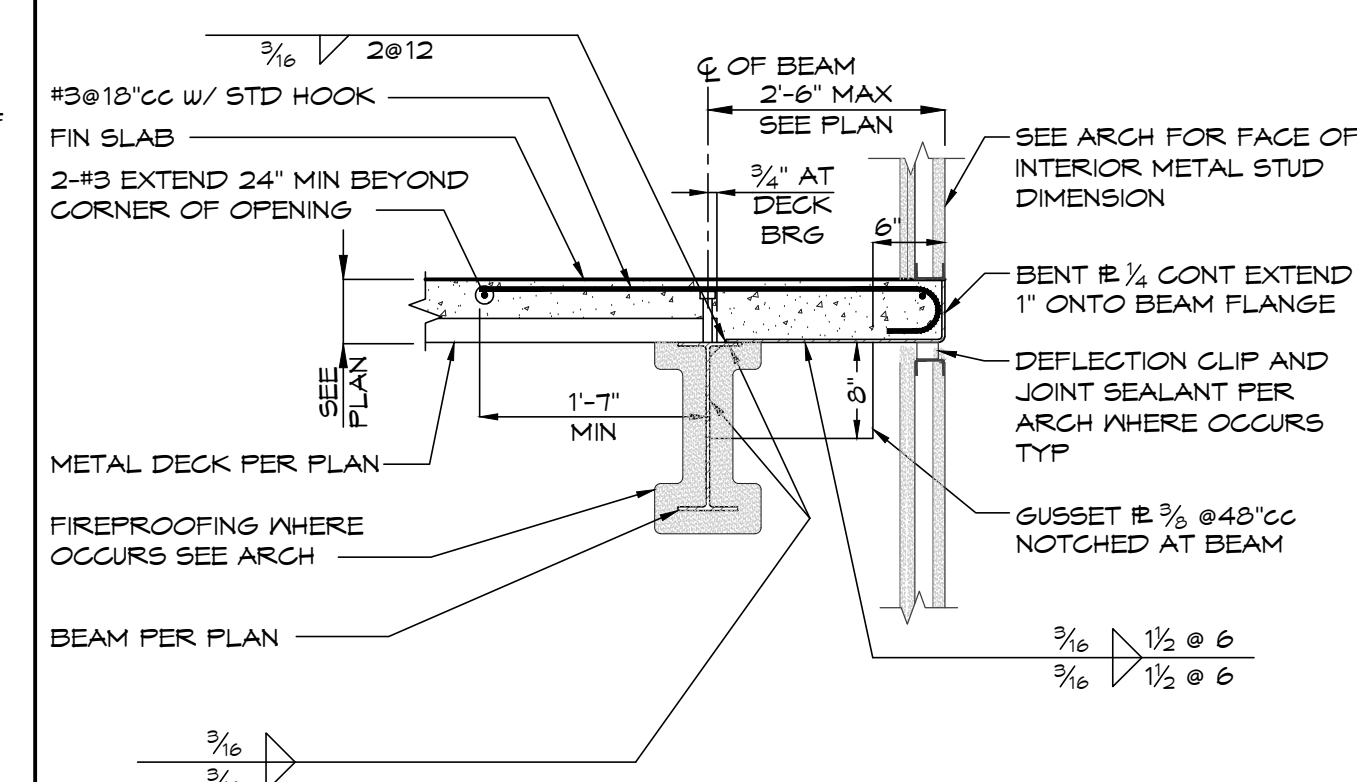
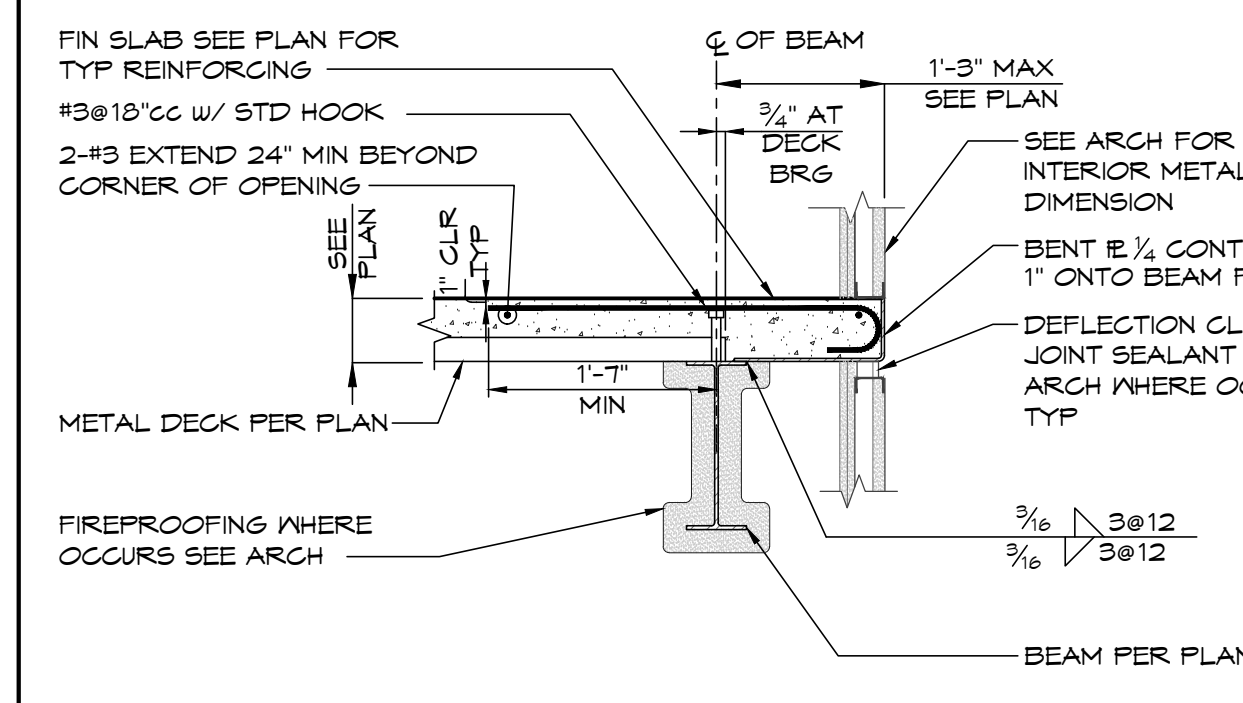
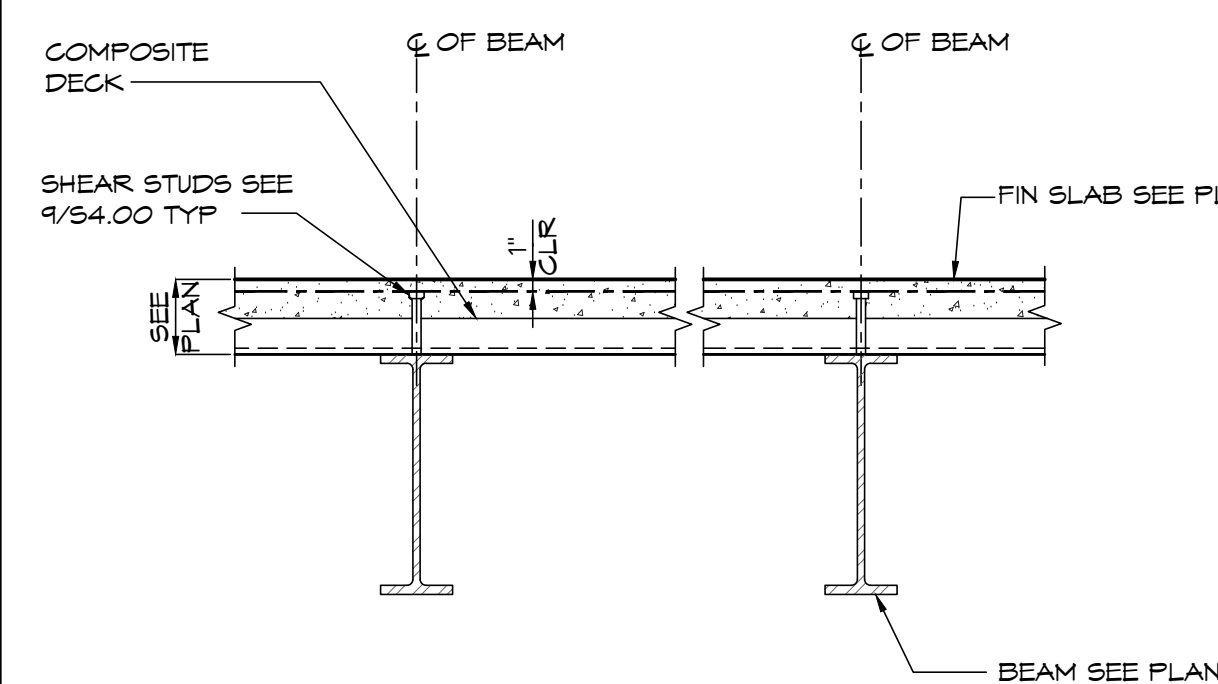
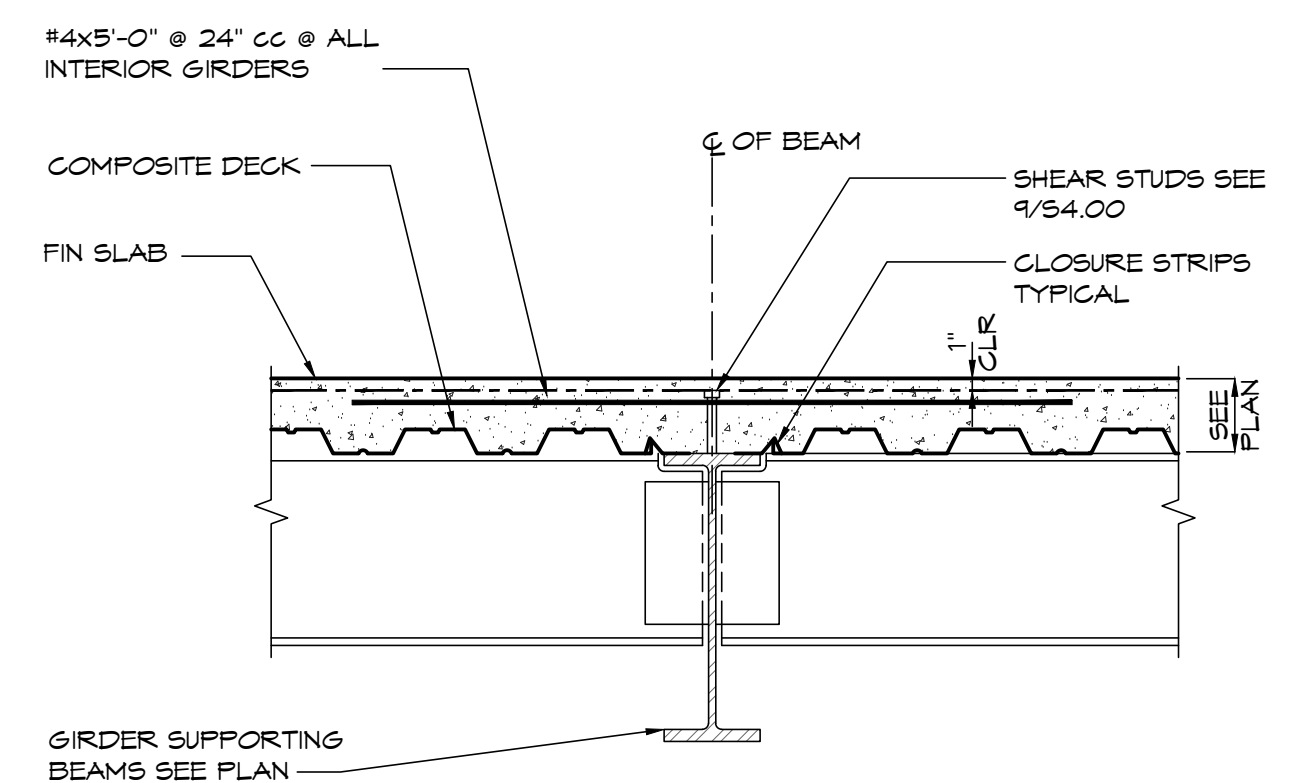
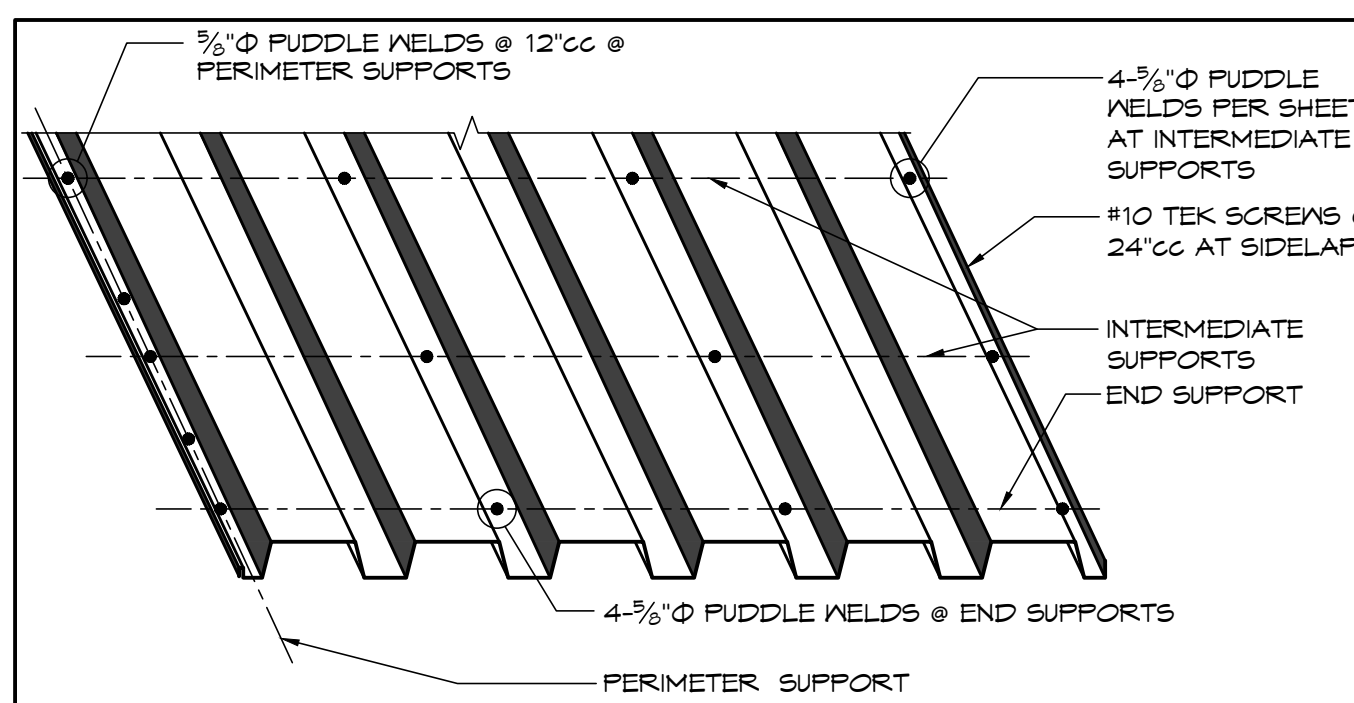


NOTES:

1. COORDINATE ANCHORAGE OF ROOFTOP MECHANICAL UNITS w/ MECH AND ARCH.
2. PRIOR TO INSTALLING MECHANICAL EQUIPMENT PROVIDE ENGINEERED CALCULATIONS DESIGNED FOR LOADS INDICATED IN THE BUILDING CODE, ANCHORAGE DETAILS FOR EQUIPMENT AND EQUIPMENT SUPPORTS, OR PROVIDE PREAPPROVED ANCHORAGE DESIGNS. SUBMIT ITEMS FOR REVIEW AND APPROVAL TO THE ARCHITECT.

- REVISÉD FRAMING FOR MECHANICAL UNITS.





STEEL DECK TYPE	GAUGE	TOTAL COMPOSITE SLAB THICKNESS	MAXIMUM UNSHORED CONSTRUCTION CLEAR SPAN		
			SINGLE	DOUBLE	TRIPLE
2VL	20	3½" NORMAL WEIGHT CONCRETE OVER 2" DECK FOR 3½" TOTAL	8'-2"	9'-0"	9'-4"

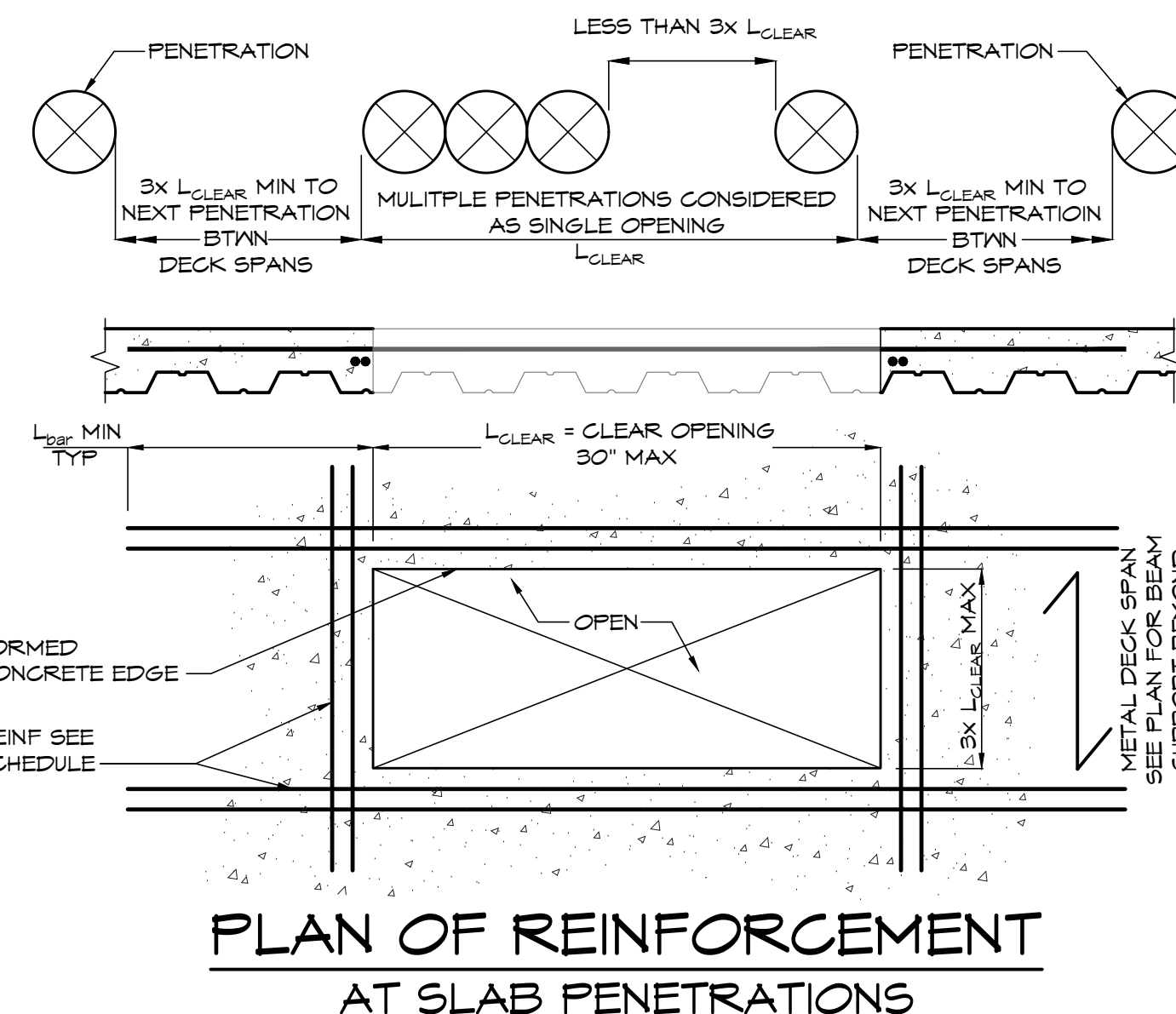
- NOTES:
1. PROVIDE STEEL SHEET MATERIAL MEETING ASTM A653 GRADE 50 WITH A G90 FINISH.
 2. EA SHEAR STUD WELDED THROUGH METAL DECK MAY REPLACE ONE WELD.
 3. PROVIDE MINIMUM BEARING LENGTH OF 2" AT ENDS OF DECK.
 4. 1½" SEAM WELDS MAY BE SUBSTITUTED FOR SIDE LAP DECK CONNECTIONS AT THE SAME SPACING INDICATED.

1	TYPICAL COMPOSITE DECK ATTACHMENT DETAIL
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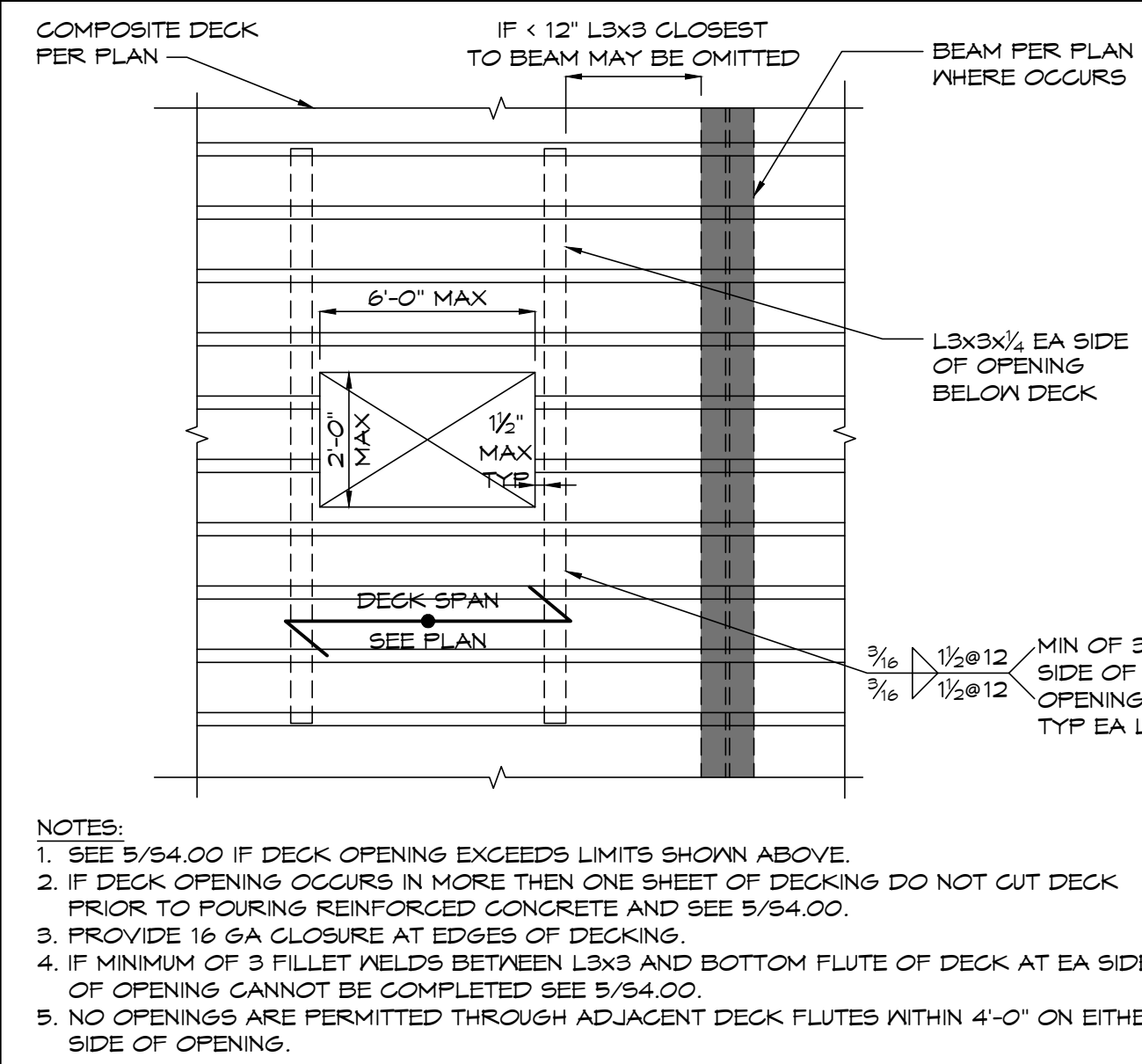
L _{clear}	REBAR	L _{lap}	REMARK
UP TO 0"	NONE	-	MINIMUM DISTANCE TO ADJACENT PENETRATION OF 3X L _{lap} = 2'-0"
0' TO 16"	2-#5 ON EA SIDE	25"	MINIMUM DISTANCE TO ADJACENT PENETRATION OF 3X L _{lap} = 7'-0"
16" TO 24"	2-#5 ON EA SIDE	25"	MINIMUM DISTANCE TO ADJACENT PENETRATION OF 3X L _{lap} = 6'-0"
24" TO 30"	2-#11 ON EA SIDE	40"	MINIMUM DISTANCE TO ADJACENT PENETRATION OF 3X L _{lap} = 7'-0"
OVER 30"	PERMITTED		NOTIFY ENGINEER FOR REQUIRED FRAMING AROUND PENETRATIONS

- NOTES:**
1. SEE PLAN FOR DECK ORIENTATION AND TYPICAL SLAB REINFORCING.
 2. PROVIDE CONCRETE FORMWORK FOR OPENING AND CUT SLAB. OPENING AFTER REINFORCED SLAB HAS REACHED DESIGN STRENGTH AND A MINIMUM OF 7 DAYS.
 3. PENETRATIONS THROUGH THE SAME DECK FLUTES ARE PERMITTED BUT MUST HAVE THE SAME FLUTE DIMENSION AND MUST BE ALIGNED.
 4. MINIMUM DISTANCE BETWEEN ADJACENT PENETRATIONS THAT ARE NOT IN THE SAME DECK FLUTE SHALL BE 12" MINIMUM.
 5. PENETRATIONS SHALL BE 6" MINIMUM IN WIDTH AND 4" MINIMUM OF $3 \times \text{SLAB THICKNESS}$.
 6. SUBMIT SHOP DRAWINGS INDICATING PENETRATION LAYOUT AND PENETRATIONS TO ARCHITECT (ENGINEER) FOR APPROVAL PRIOR TO POURING SLAB.
 7. SUBMIT TWO (2) ALTERNATE DETAILS IF DECK OPENING IS UNFRAMED AND IS CUT PRIOR TO POURING CONCRETE SLAB.

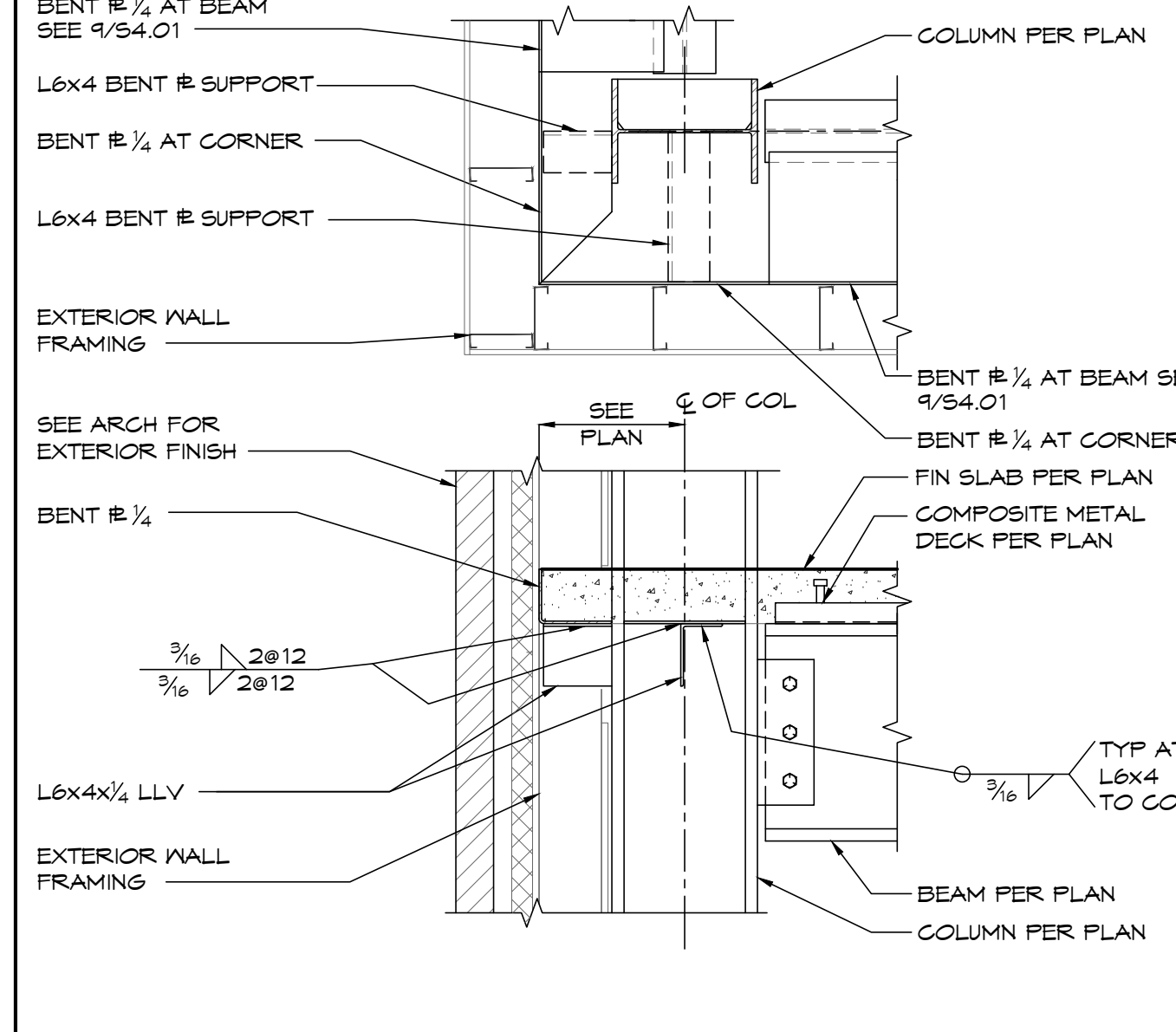
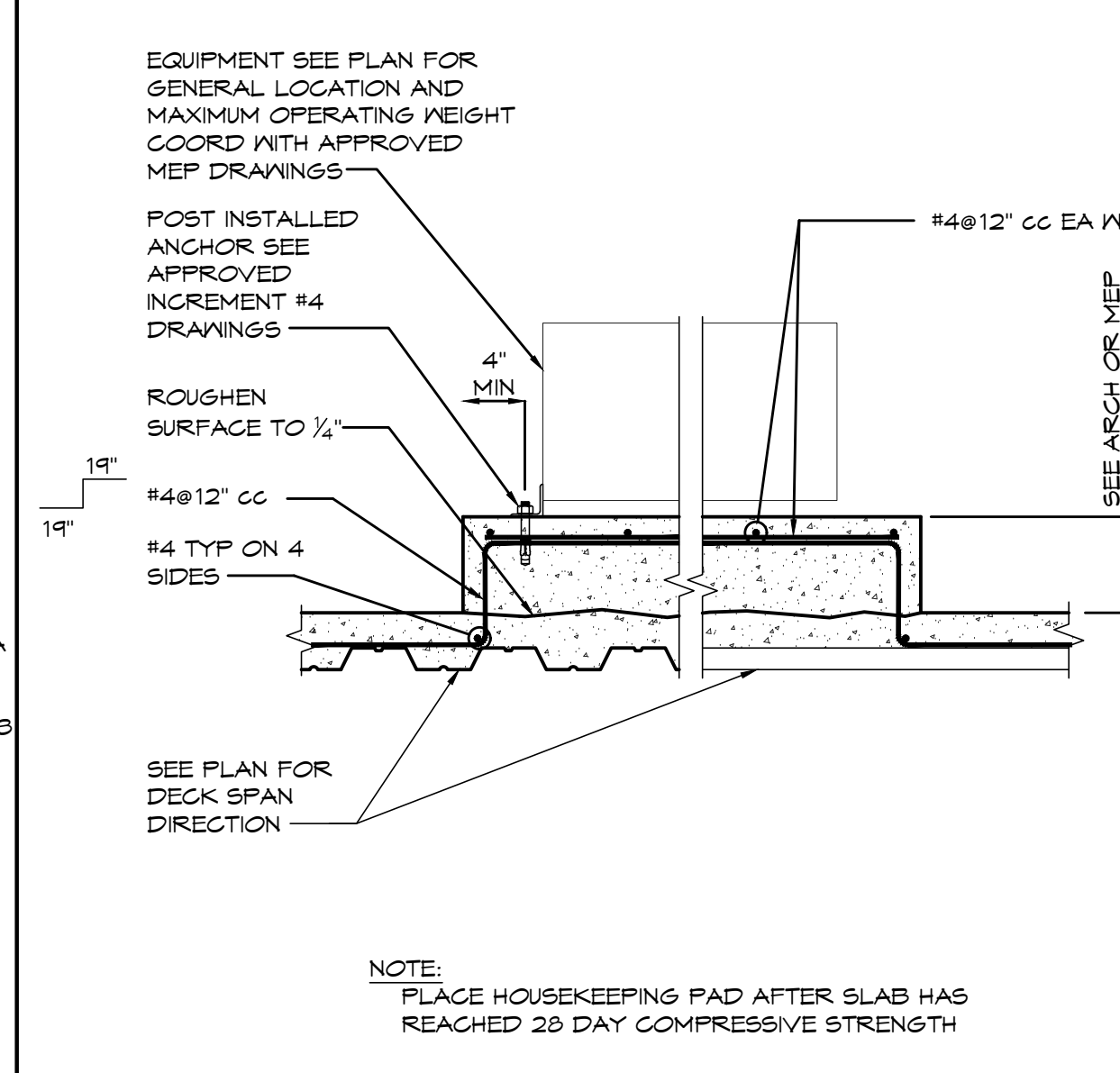
2	TYPICAL COMPOSITE SLAB DETAIL @ ALL GIRDERS
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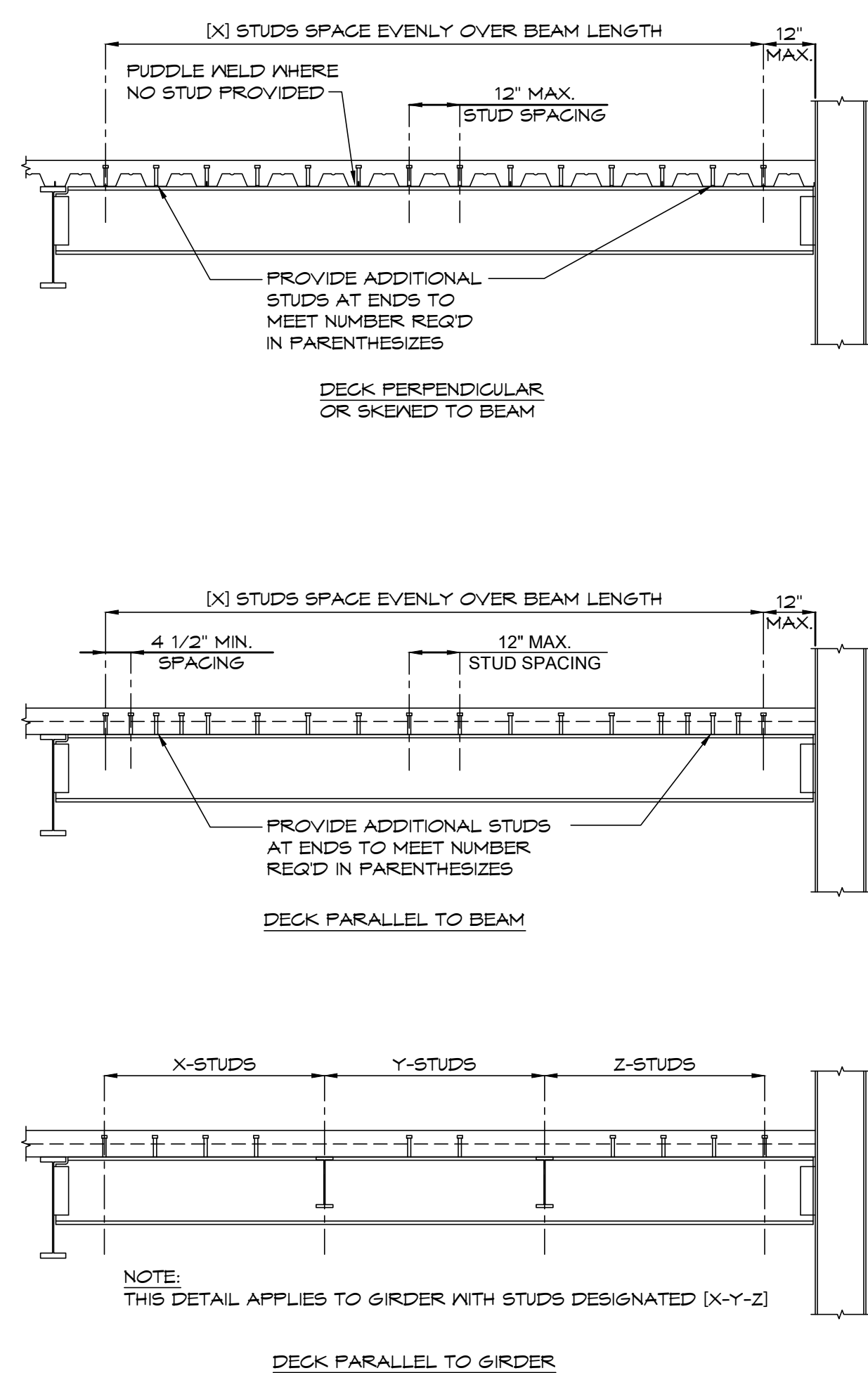
3	TYPICAL COMPOSITE SLAB DETAIL
---	----------------------------------



4	TYP INTERIOR EDGE OF FRAMED COMPOSITE SLAB
---	--

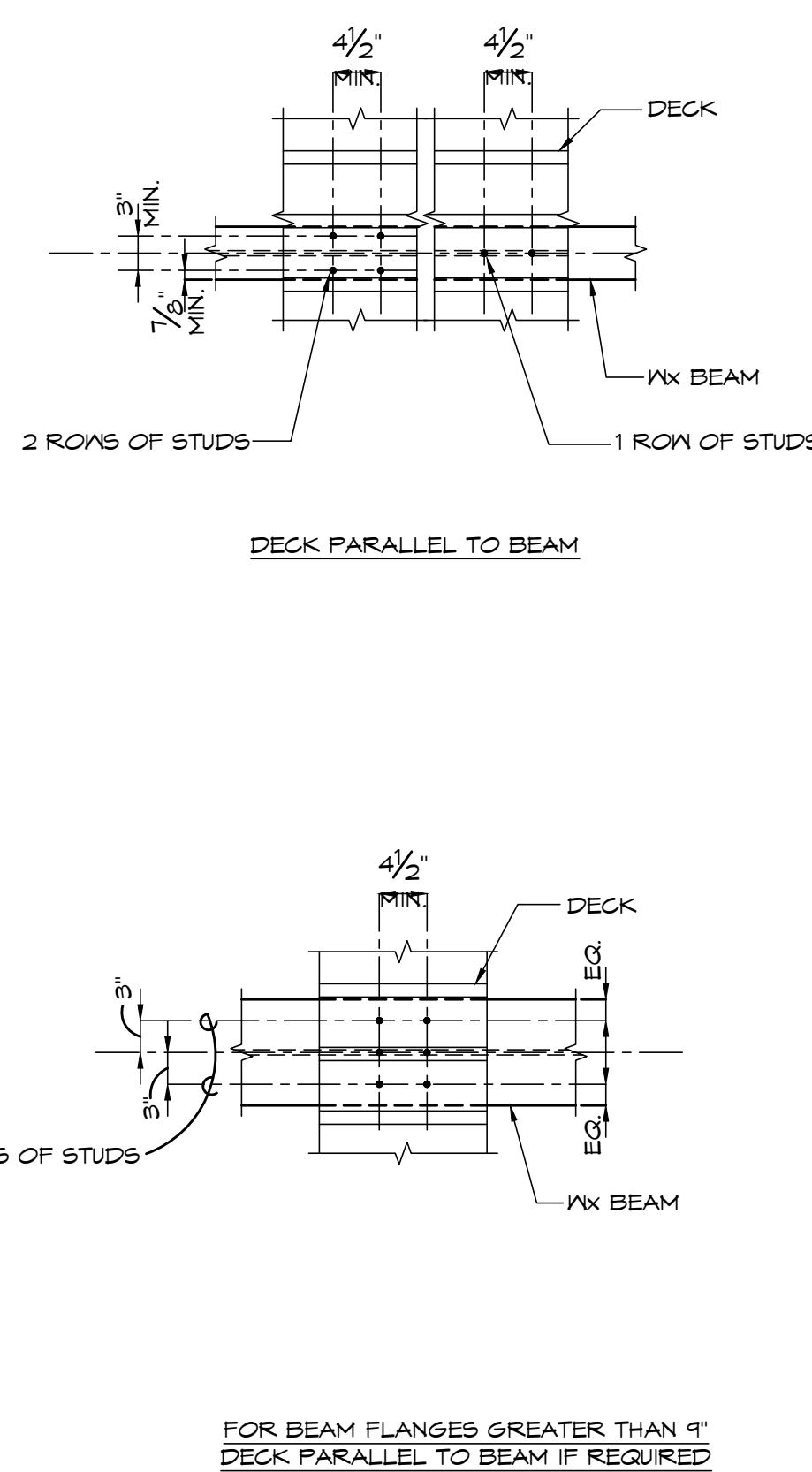


5	TYPICAL UNFRAMED SLAB ON COMPOSITE METAL DECK PENETRATION
---	---

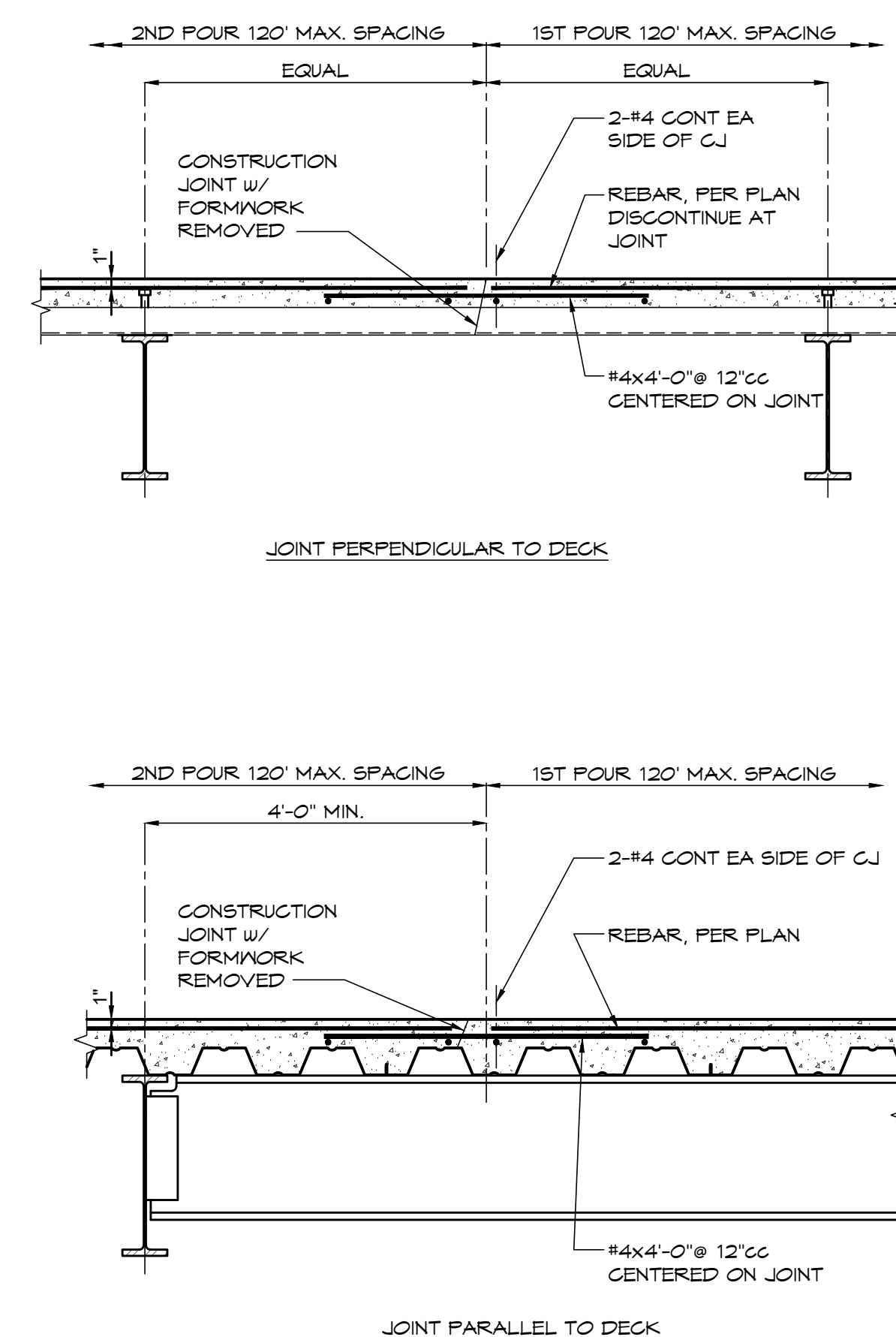


- NOTE:**
1. THE MINIMUM NUMBER OF STUDS REQUIRED IS SHOWN AS (X) ON FRAMING PLANS OR 12" o.c. MAXIMUM SPACING IF NOT CALLED OUT ON PLANS.
 2. IF TWO STUDS ARE REQUIRED IN ONE FLUTE THE TRANSVERSE SPACING SHALL BE 3" MINIMUM.
 3. ALL BEAMS AND GIRDER SHALL HAVE STUDS @12" o.c. AS A MINIMUM.

6	COMPOSITE DECK AT UNFRAMED SLAB PENETRATION
---	--

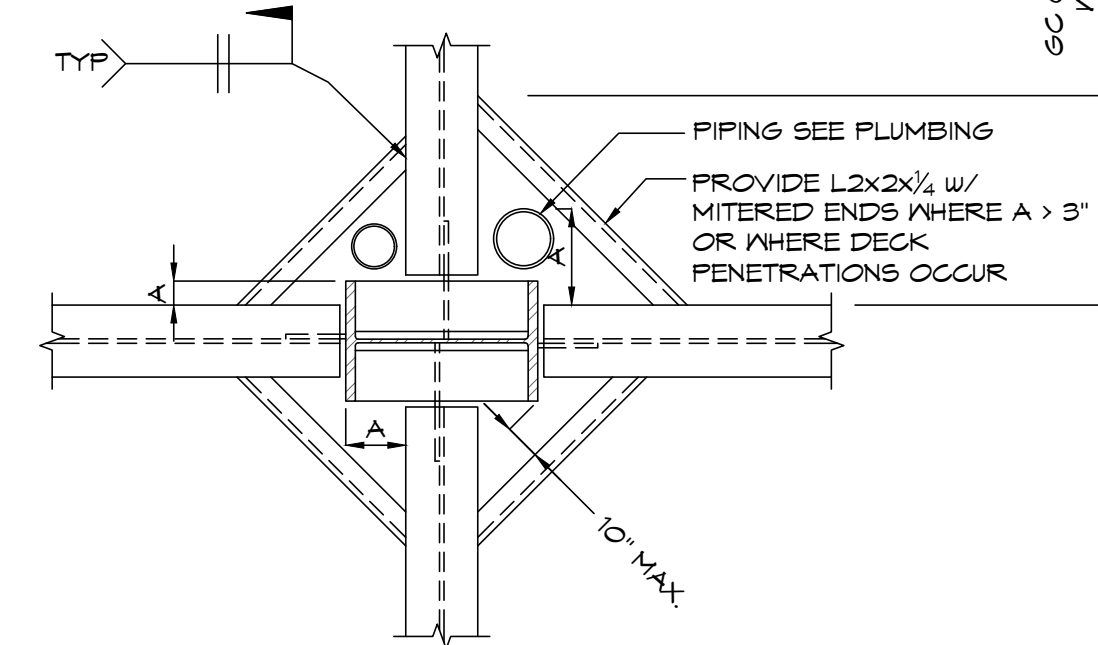


7	TYPICAL HOUSEKEEPING PAD ON DECK
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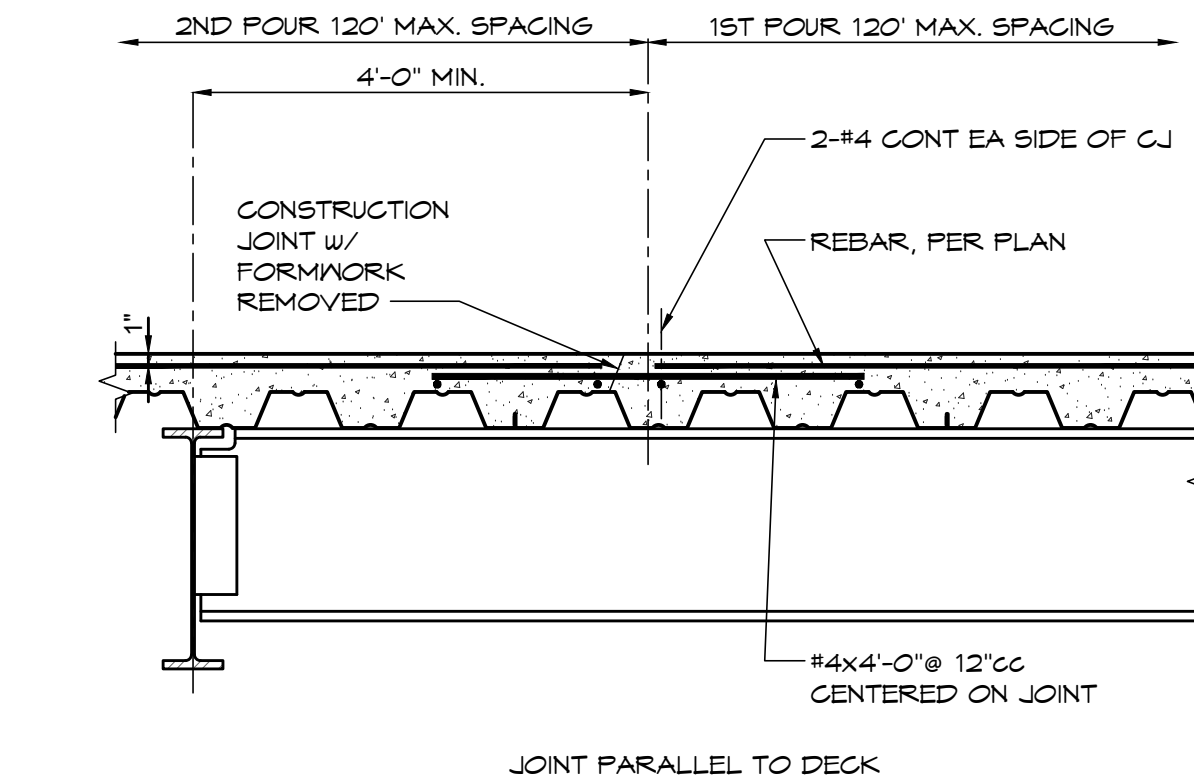
- NOTE:
1. ALLOW 8 HOURS MINIMUM BETWEEN ADJACENT POURS

8	TYPICAL BENT #2 AT CORNER COLUMN
---	----------------------------------



- NOTE:
DECK SUPPLIER TO PROVIDE DECK CLOSURES AS NEEDED

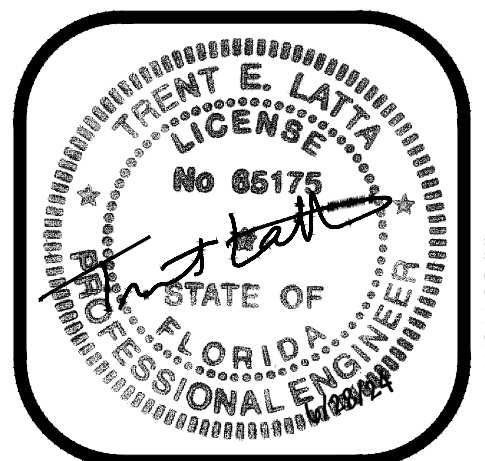
11	TYPICAL DECK SUPPORT @ COLUMN
----	----------------------------------



- NOTE:
1. ALLOW 8 HOURS MINIMUM BETWEEN ADJACENT POURS

10	TYP CONSTRUCTION JOINT FOR SLAB ON COMPOSITE DECK
----	--

9	TYPICAL COMPOSITE BEAM STUD SPACING
---	-------------------------------------



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[illegible]

1 TYPICAL STAIR HANGER DETAIL

Notes:
1. SEE ARCHITECT FOR DIMENSIONS AND LOCATIONS OF STAIRS.
2. IF DEVIATIONS FROM THE STAIR FRAMING DESIGN SHOWN ABOVE ARE MADE THEN SUBMIT TO THE ARCHITECT (STRUCTURAL ENGINEER) FOR APPROVAL CALCULATIONS AND DRAWINGS FOR THE DESIGN OF STAIR FRAMING INCLUDING STRINGERS, HEADERS, FRAMING AT THE INTERMEDIATE LANDINGS AND TREAD SUPPORTS. THE CALCULATIONS AND DRAWINGS SHALL BE DESIGNED AND SEALED BY AN ENGINEER WHO IS LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. DESIGN TO INCLUDE THE MINIMUM LOADS OF THE BUILDING CODE AS INDICATED ON S1.00.

2 BROKEN-BACK OUTSIDE STAIR STRINGER AT FLOOR LANDING

Notes:
1. SEE 1/54.01 FOR HANGER SUPPORT AT INTERMEDIATE LANDINGS.
2. SEE 3/54.01 FOR GUARDRAIL SUPPORT BEYOND AT TOP FLOOR LANDING.
3. SEE 4/54.01 FOR FRAMING AT INTERMEDIATE STAIR LANDINGS.

3 FLR @ STAIR LANDING W/ GUARDRAIL

NOTE: SEE 1/54.01 FOR REMAINING INFORMATION NOT SHOWN.

4 HEADER & STRINGER SUPPORT AT INTERMEDIATE LANDINGS

Notes:
1. SEE TYPICAL BEAM CONNECTION DETAIL FOR REMAINDER OF INFORMATION.
2. SEE 5/54.01 FOR INSIDE STAIR STRINGER SUPPORT AT HEADER.

5 STAIR STRINGER SUPPORT AT HEADER

Notes:
1. IF NOT COORDINATED PRIOR TO BIDDING THEN ASSUME 5 SEPARATOR BEAMS BETWEEN PIT AND ROOF FOR BIDDING WITH FINAL QUANTITY AND ELEVATIONS TO BE COORDINATED W/ ELEVATOR MNFR.
2. CONTRACTOR TO COORDINATE WITH ELEVATOR MANUFACTURER ANY ADDITIONAL PLATES, ANGLES, BOLTS, ETC. THAT MAY BE REQUIRED FOR ATTACHMENT OF GUIDERAIL TO HSS FRAMING.
3. ERECTOR TO COORDINATE FIELD WELDING WITH SHOP FABRICATION.

6 INTERMEDIATE STAIR LANDING

Notes:
1. SEE 1/54.01 FOR REMAINING INFORMATION NOT SHOWN.

7 FLOOR SLAB DEPRESSION

Notes:
1. SEE 1/54.01 FOR REMAINING INFORMATION NOT SHOWN.

8 EXTERIOR WALL FRAMING AT STAIR SHAFTS

Notes:
1. SEE 1/54.01 FOR REMAINING INFORMATION NOT SHOWN.

10 TYPICAL HSS12x4 ELEVATOR GUIDE RAIL SUPPORT POST

Notes:
1. SUBMIT THE DELEGATED DESIGN FOR THE EXTERIOR CURTAINWALL GLAZING INCLUDING DRAWINGS AND CALCULATIONS FOR APPROVAL SHOWING COMPLIANCE WITH PERFORMANCE CRITERIA. DELEGATED DESIGN SHALL BE COMPLETED BY AN ENGINEER WHO IS LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED.
2. FASTENERS TO SUPPORTING FRAMING SHALL BE ALUMINUM, NONMAGNETIC STEEL OR OTHER MATERIALS TO BE NON-CORROSIVE AND COMPATIBLE WITH ALUMINUM WINDOW MEMBERS, TRIM HARDWARE, ANCHORS, AND OTHER COMPONENTS.
3. COORDINATE MINIMUM GAUGE OF EXTERIOR WALL FRAMING AS REQUIRED WITH ATTACHMENTS OF CURTAINWALL FRAMING.
4. SEE ARCH FOR FLORIDA PRE-APPROVED GLAZING.

11 ELEVATOR SILL SUPPORT

Notes:
1. PROVIDE AUTOMATIC END WELDED REBAR ANCHORS AS NOTED.
2. SEE 12/54.01 FOR REMAINDER OF INFORMATION.

9 TYP EXTERIOR EDGE OF COMPOSITE SLAB

12 TYP CURTAINWALL SUPPORT W/ WOOD BUCKS AT EXTERIOR METAL WALL FRAMING

13 FLOOR AT PERIMETER OF BUILDING W/ CURTAINWALL

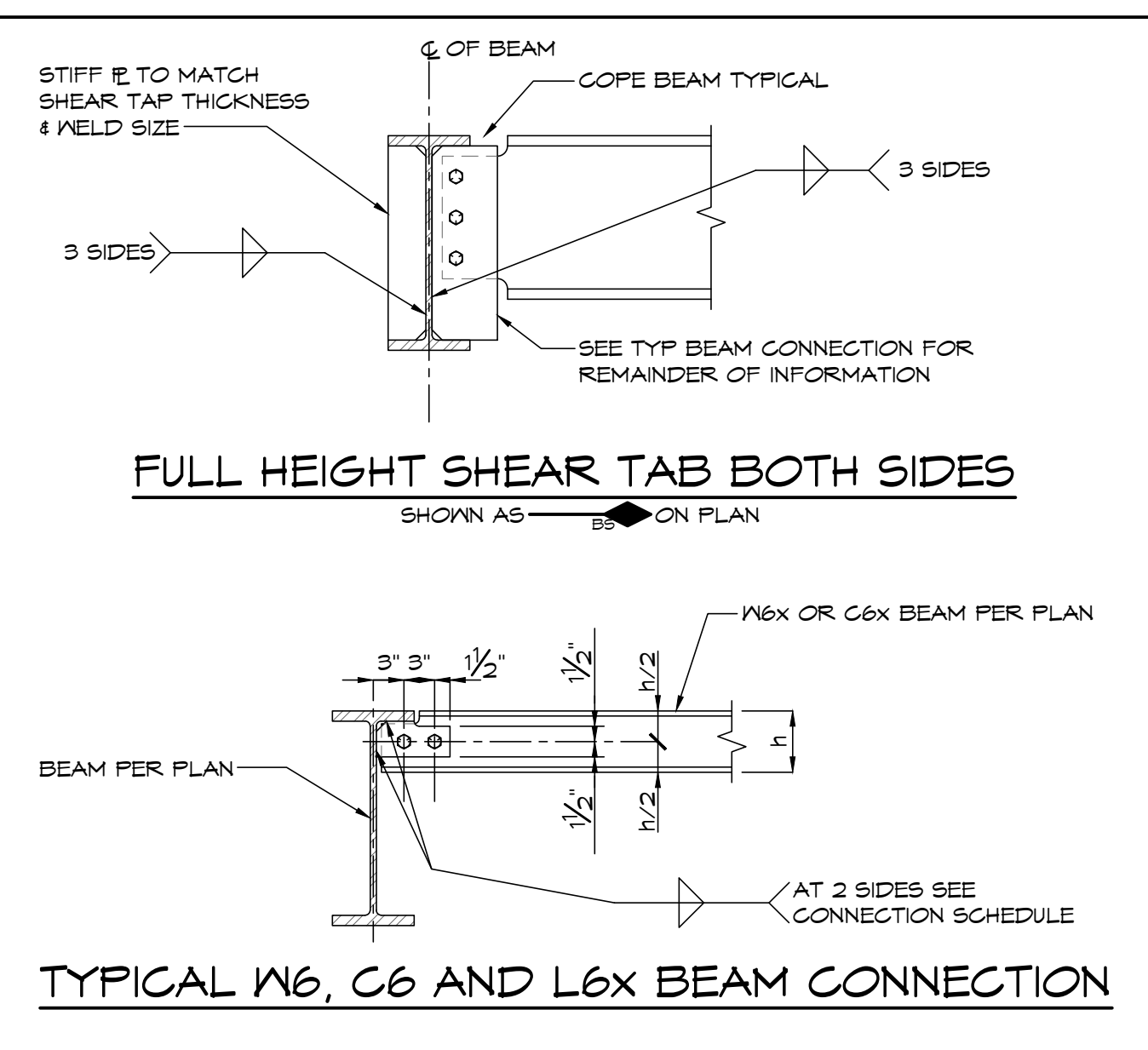
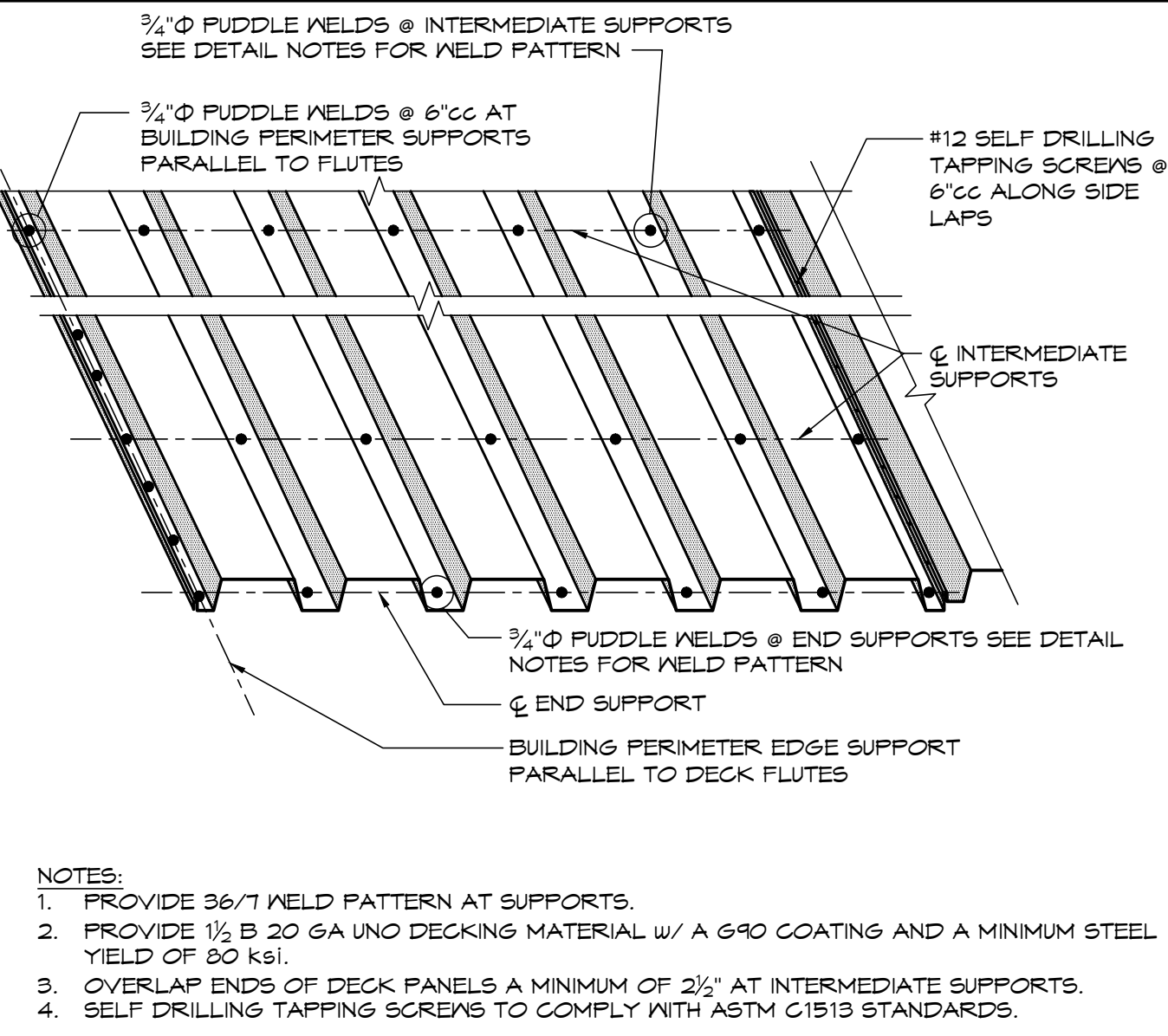
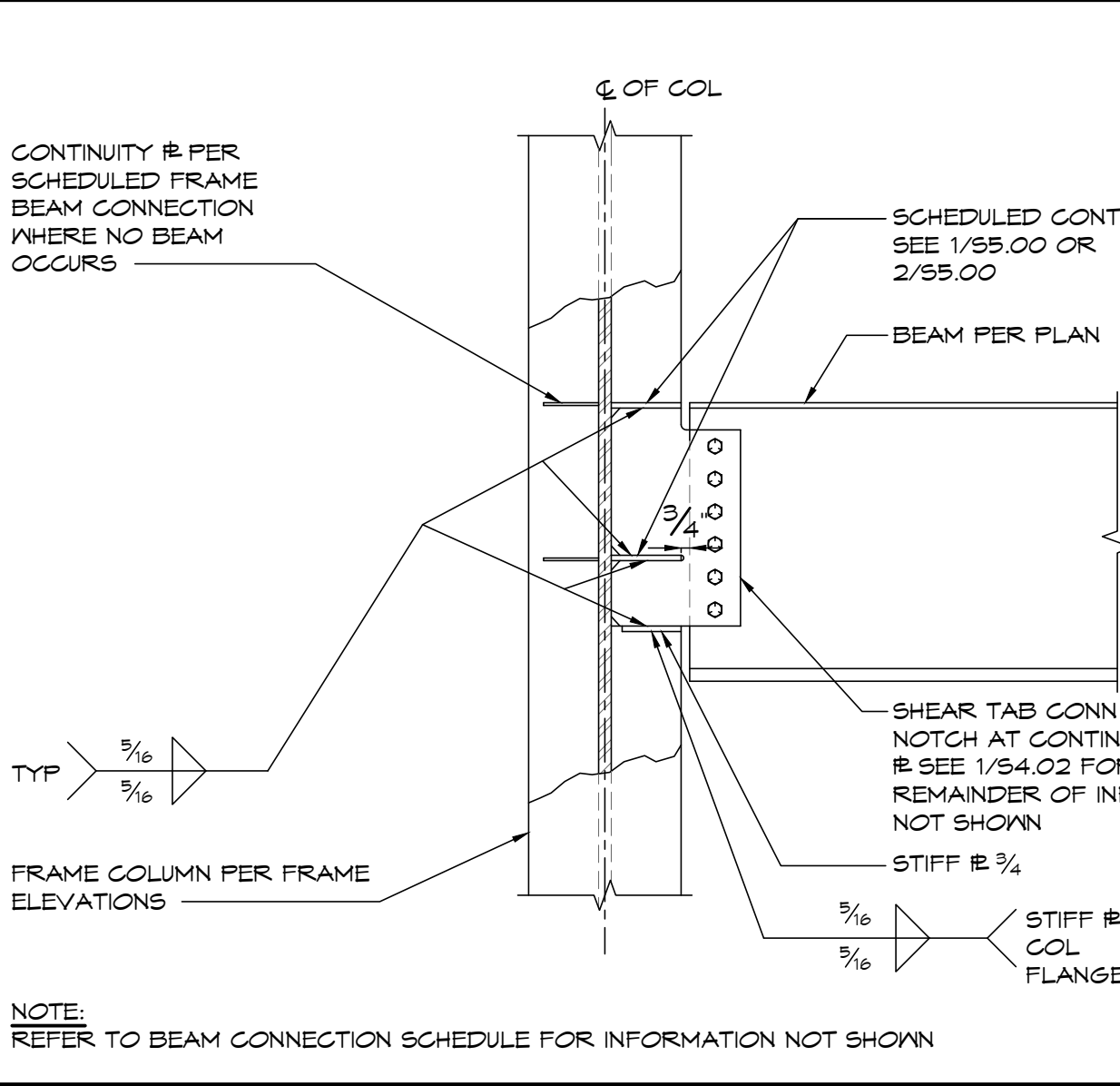
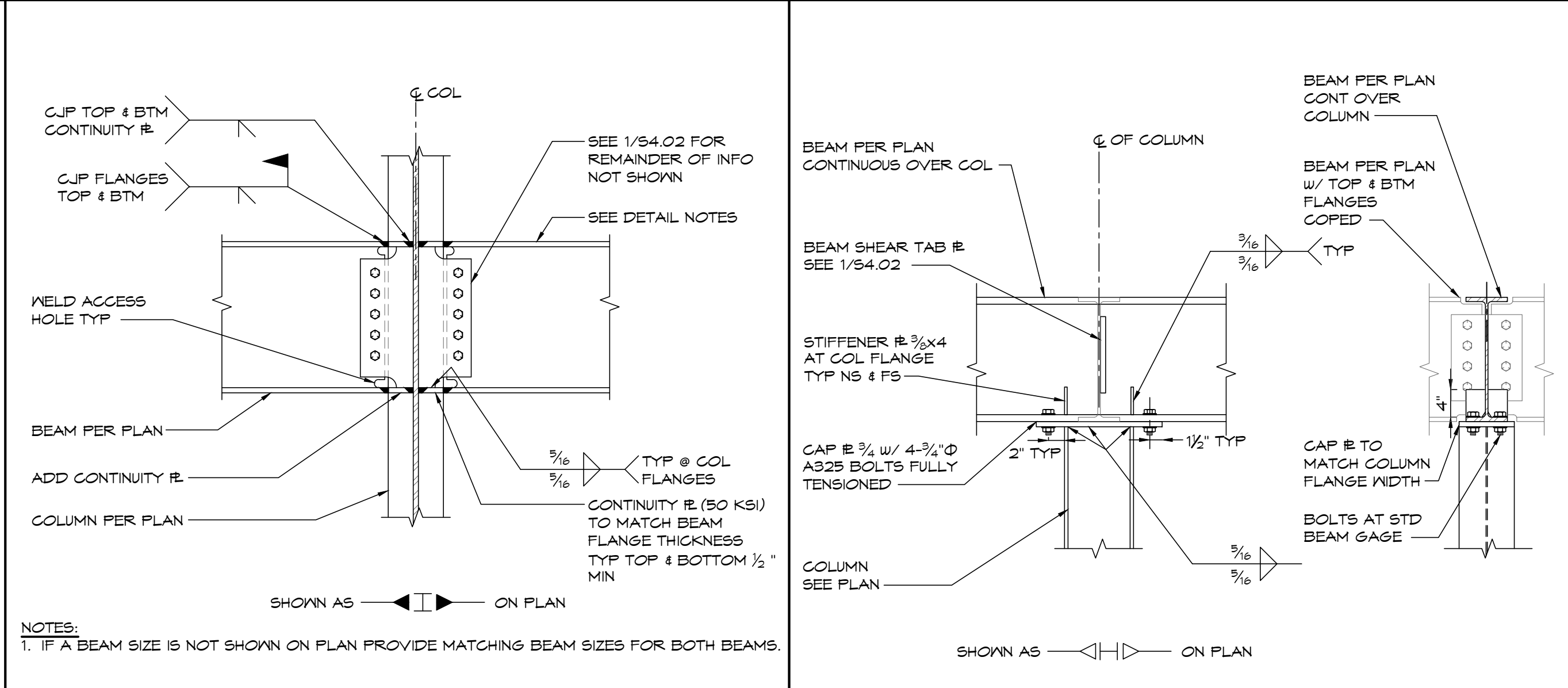
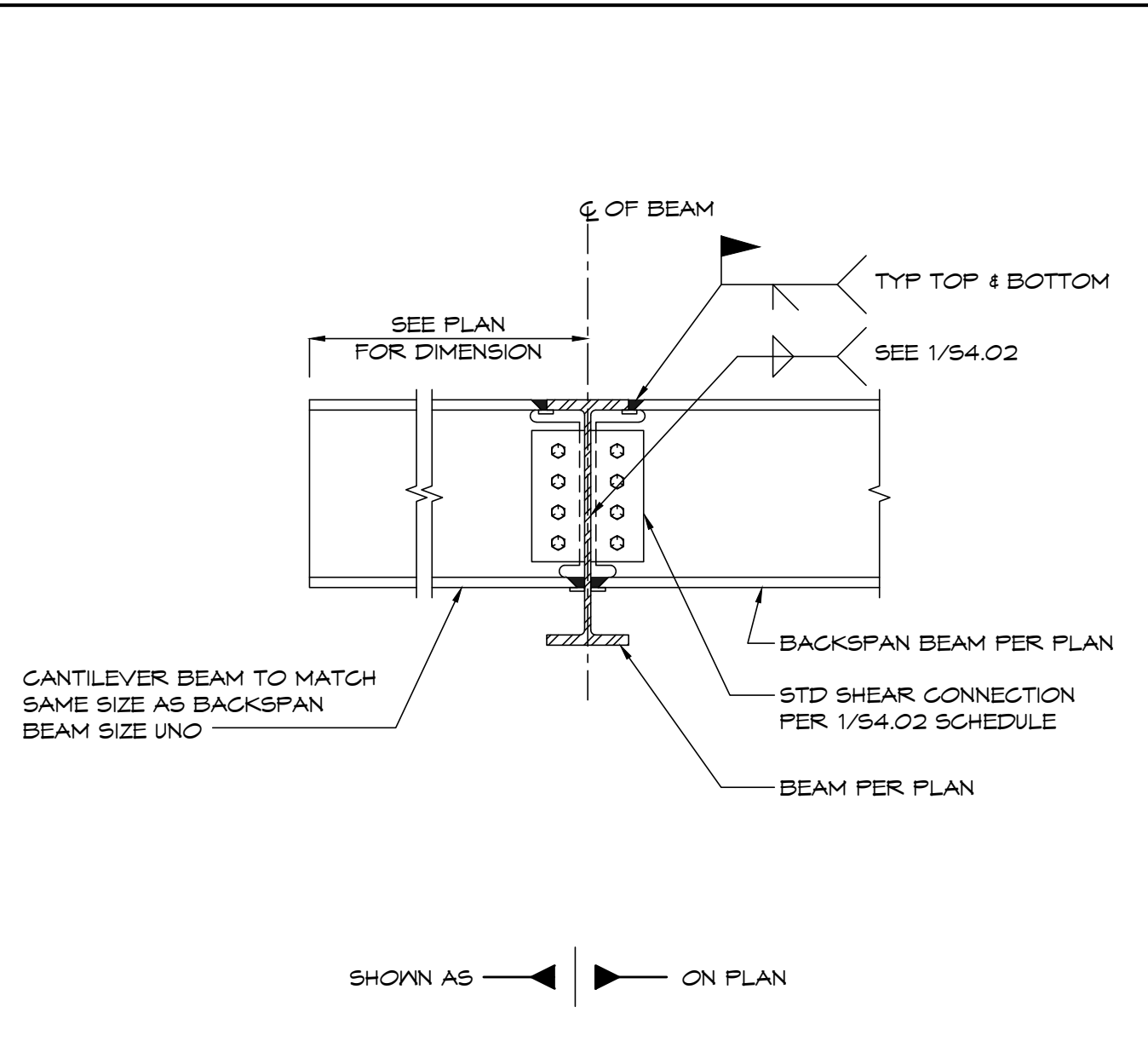
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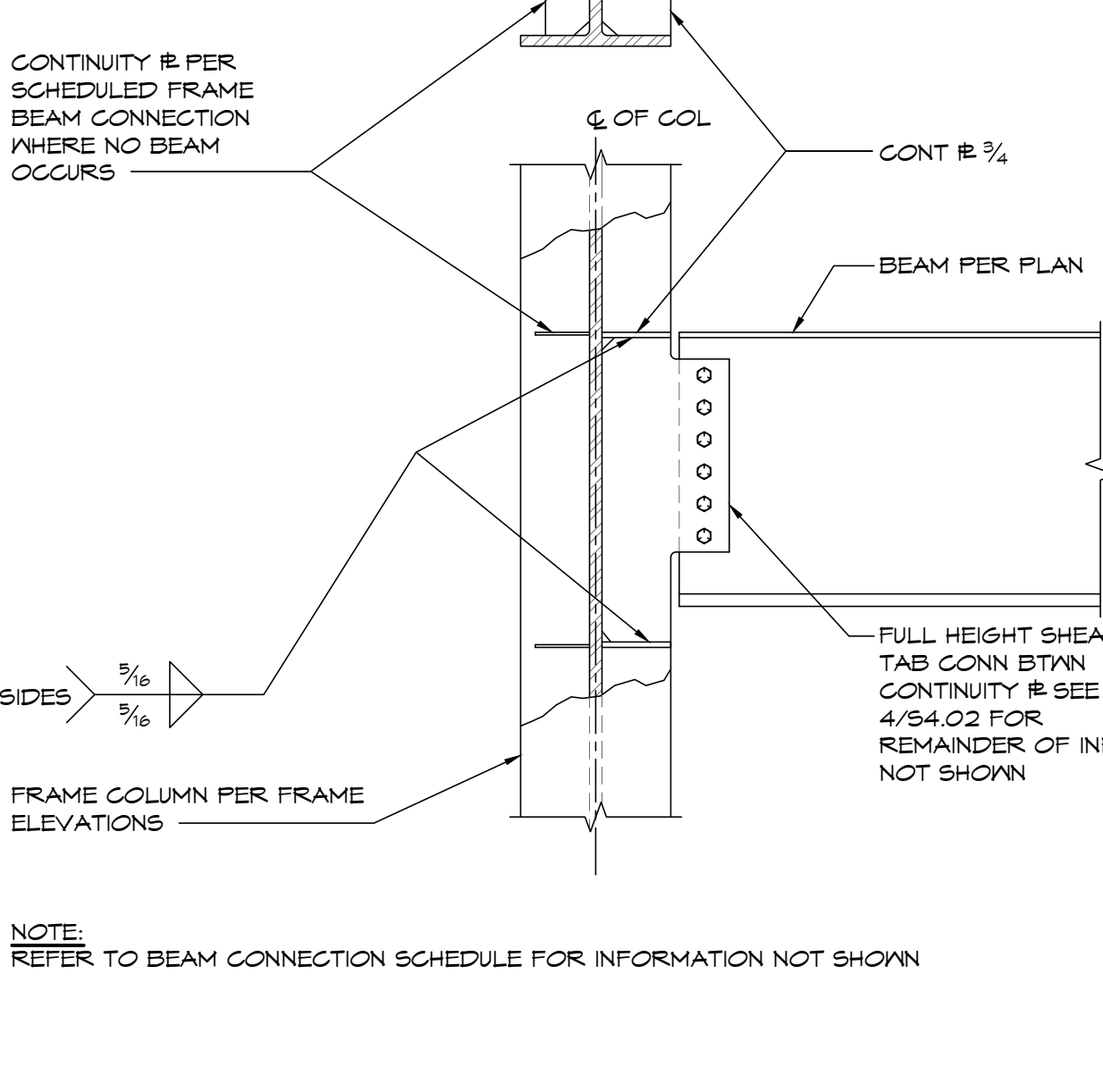
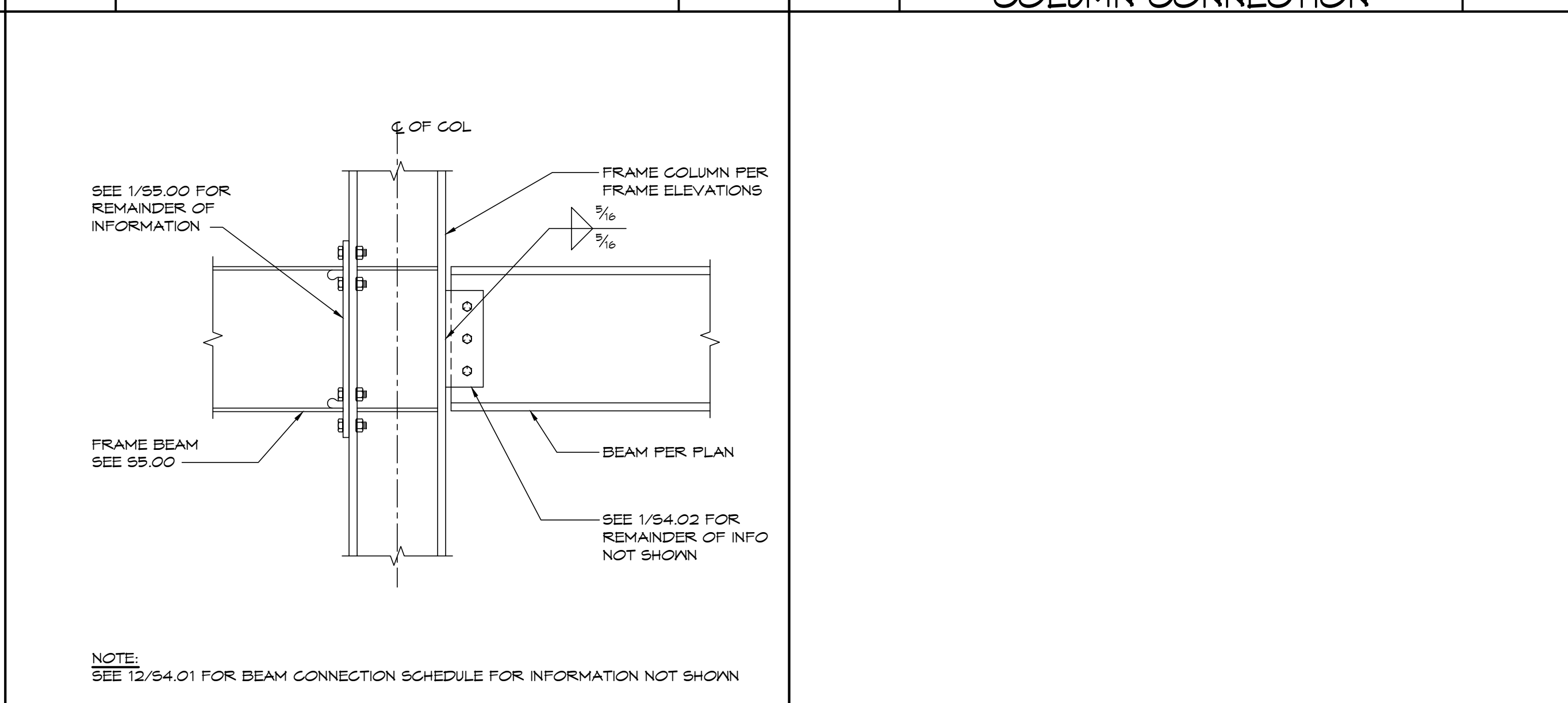
Diagram illustrating the connection of a frame beam to a column, showing both elevation and plan views.

Elevation View Labels:

- SEE 10/54.02 FOR CONTINUITY
- FRAME BEAM PER FRAME ELEVATIONS
- FRAME BEAM SEE 55.00
- C OF COL
- 45°
- TYP TOP 4 BTM
- BEAM SIZE PER PLAN

Plan View: Shows the beam's cross-section with reinforcement details.

Legend: SHOWN AS ON PLAN



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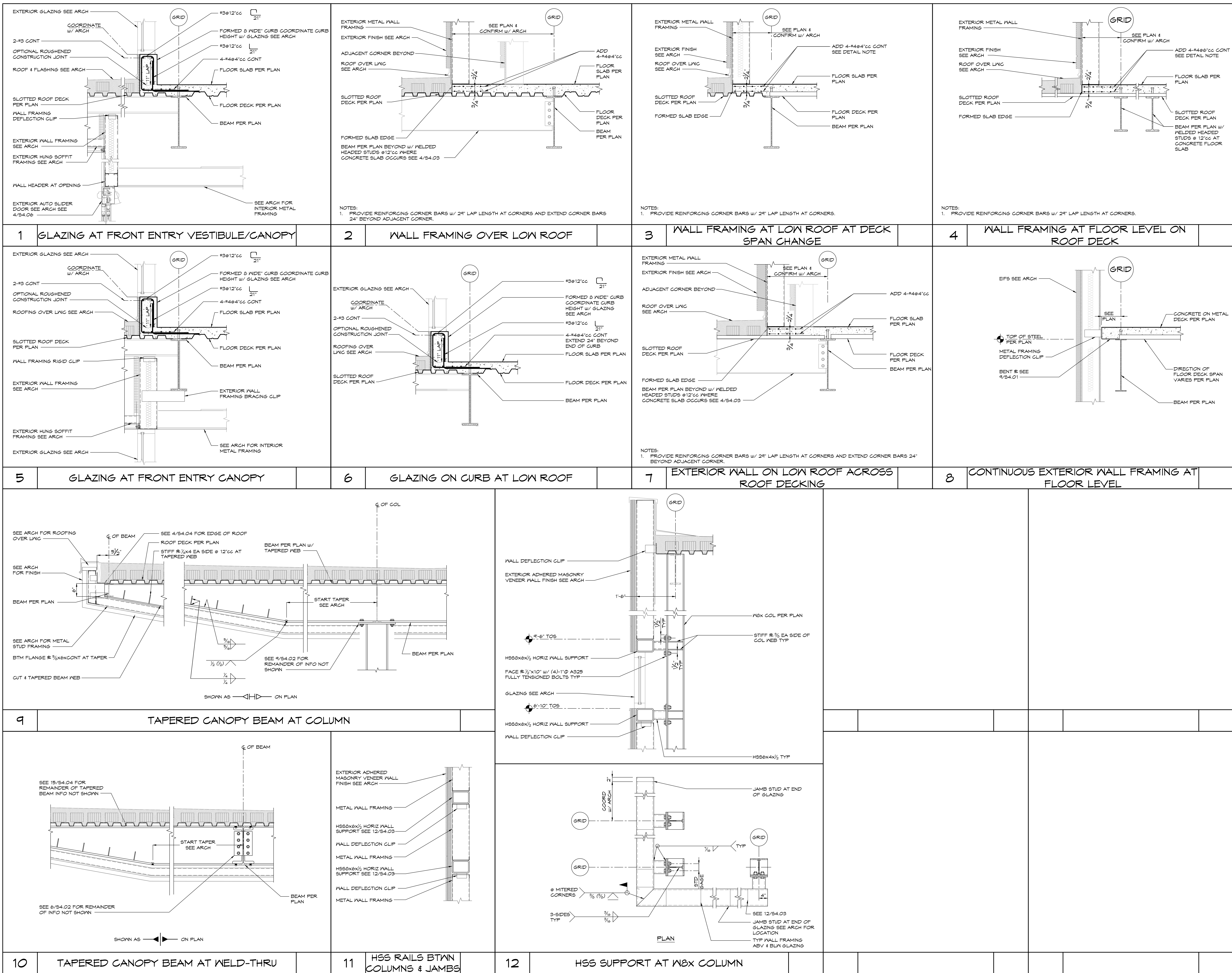
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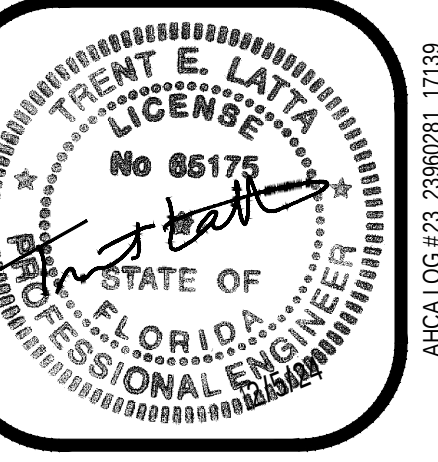
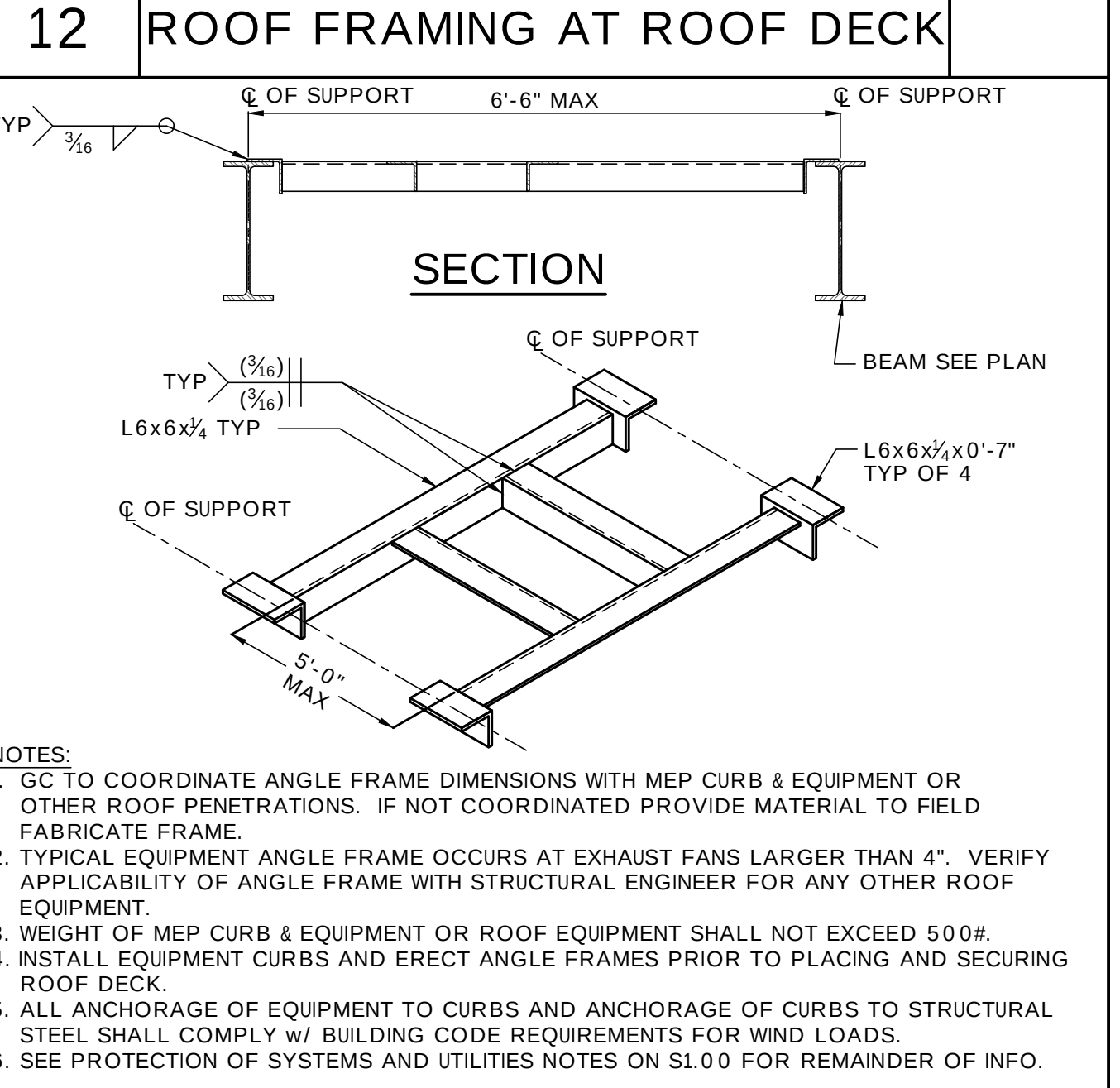
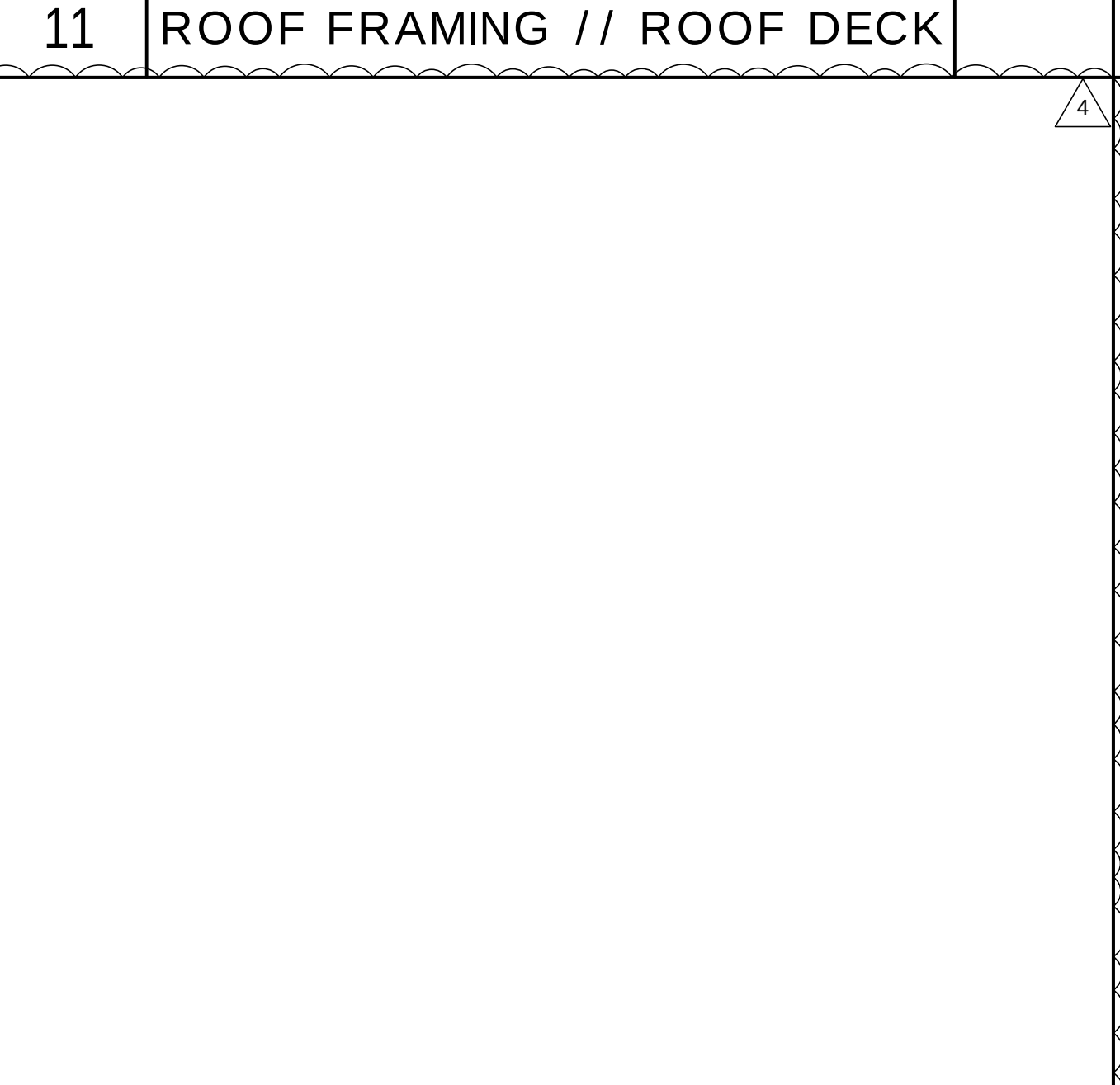
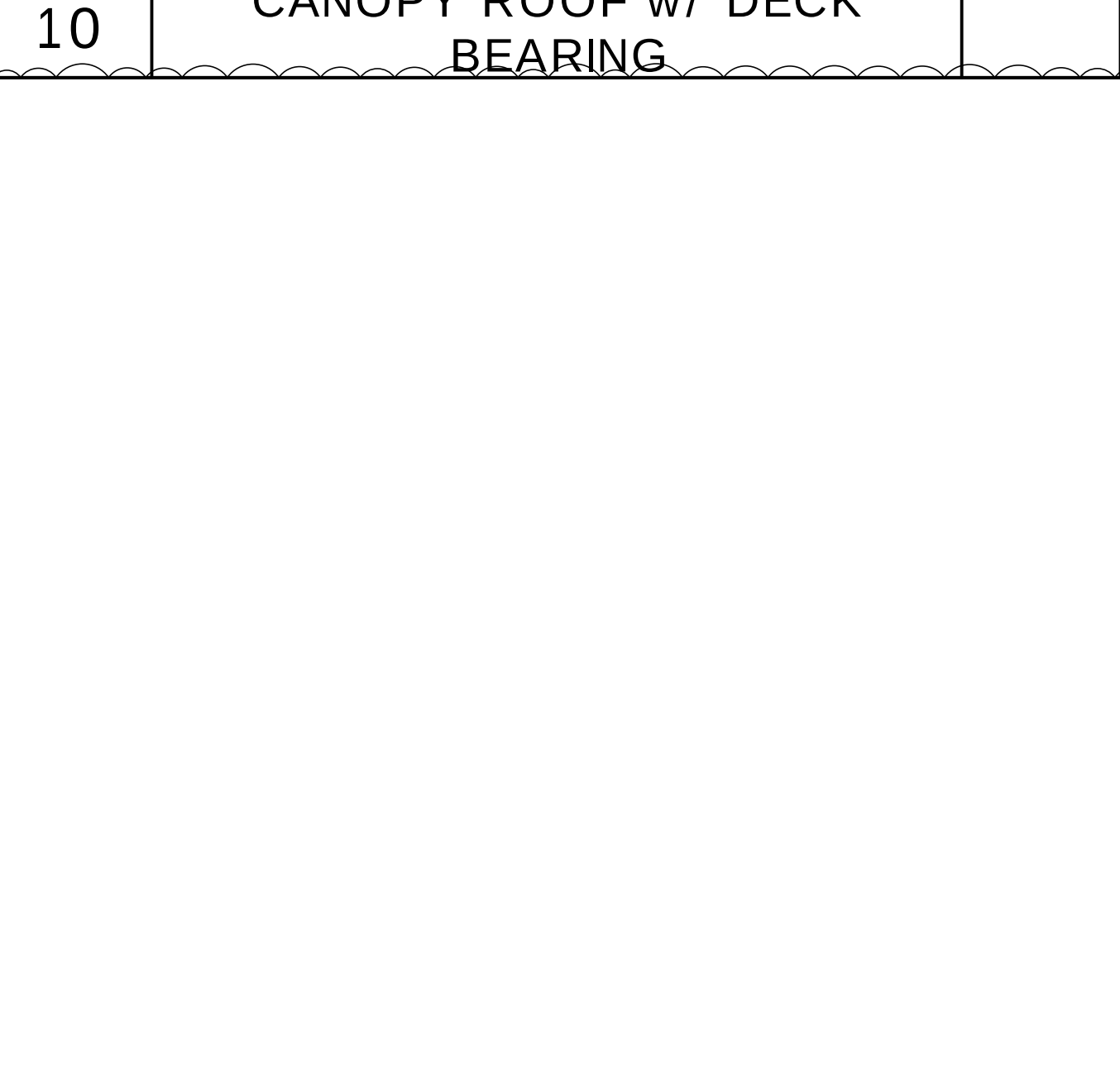
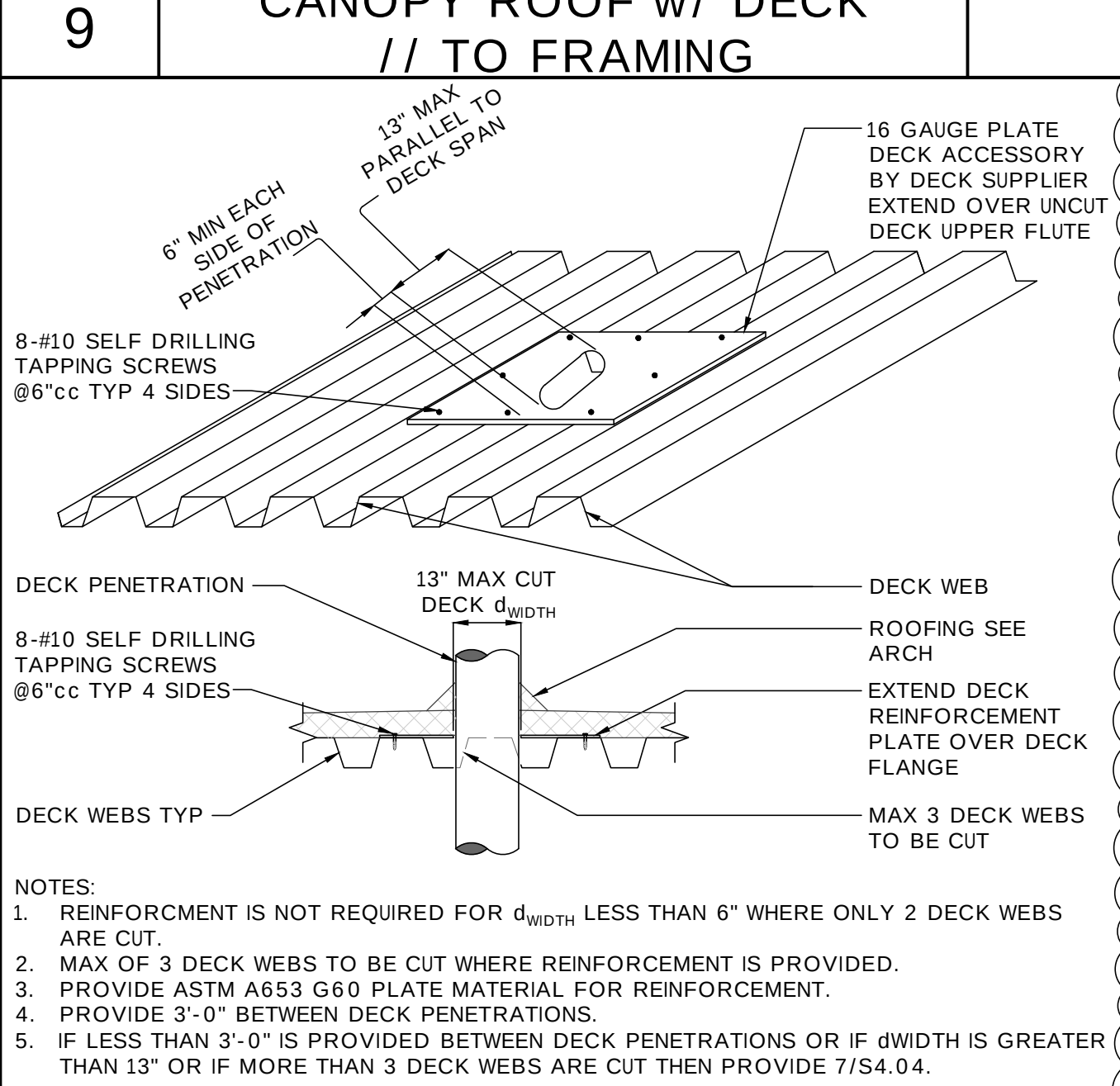
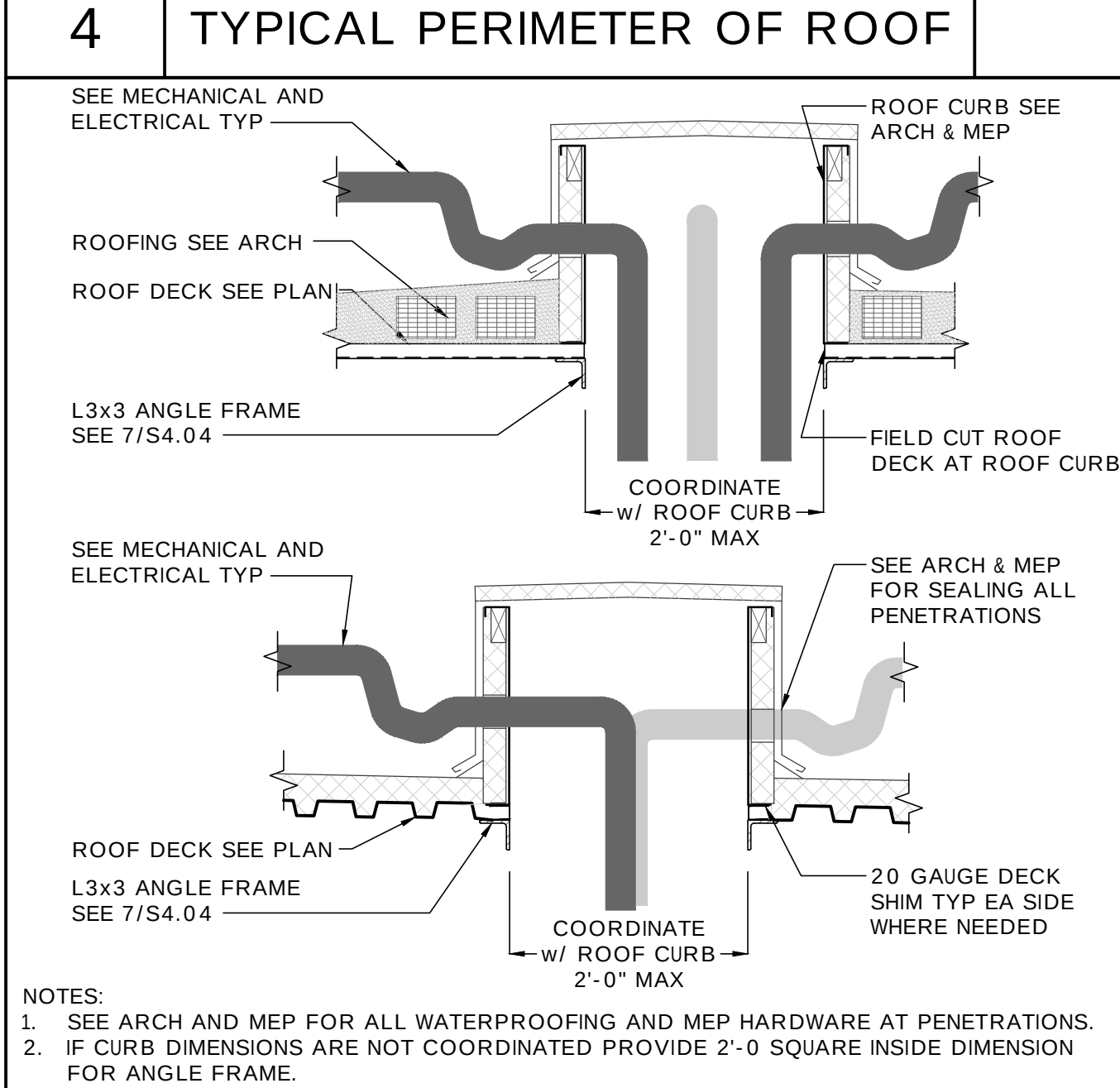
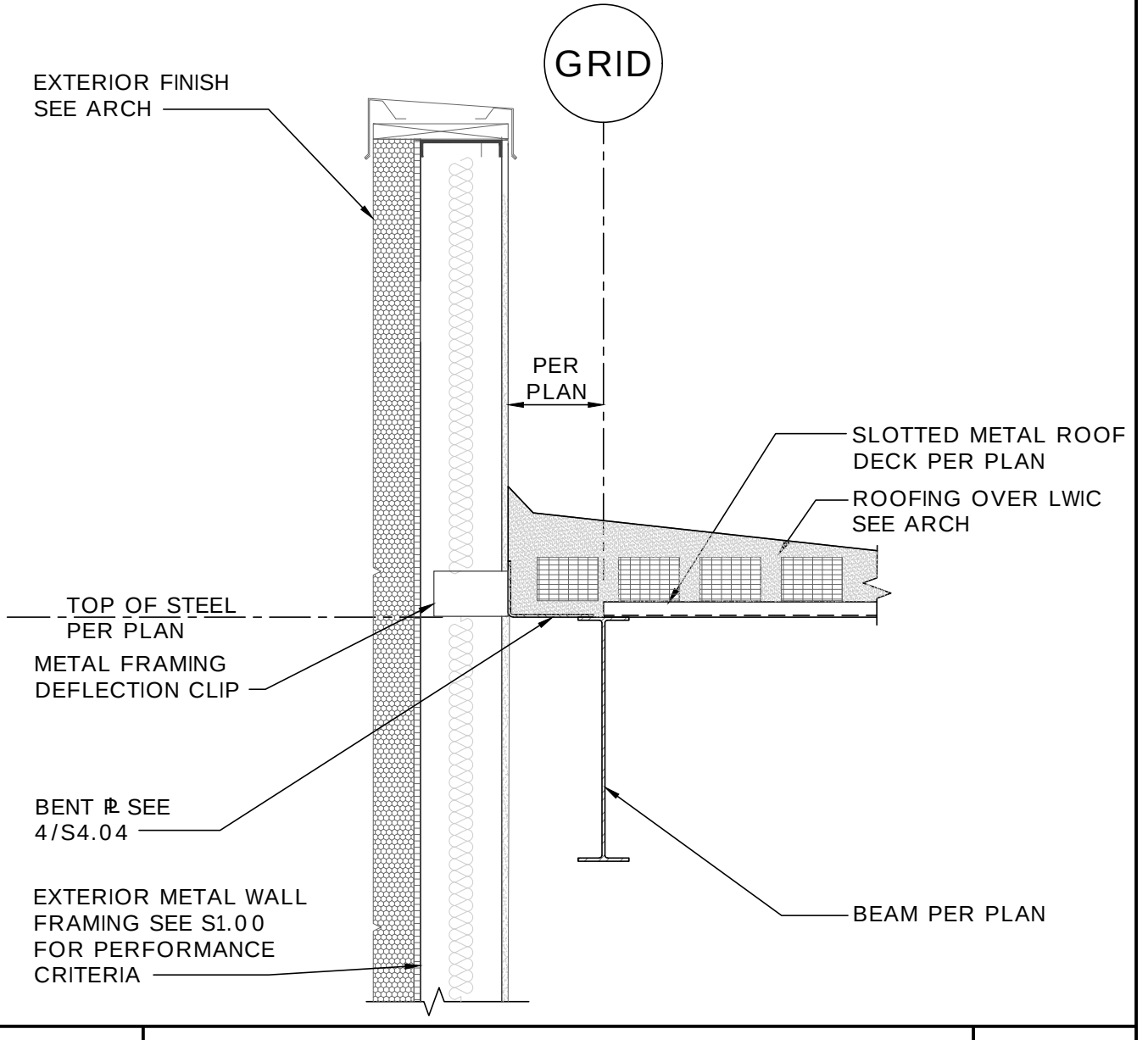
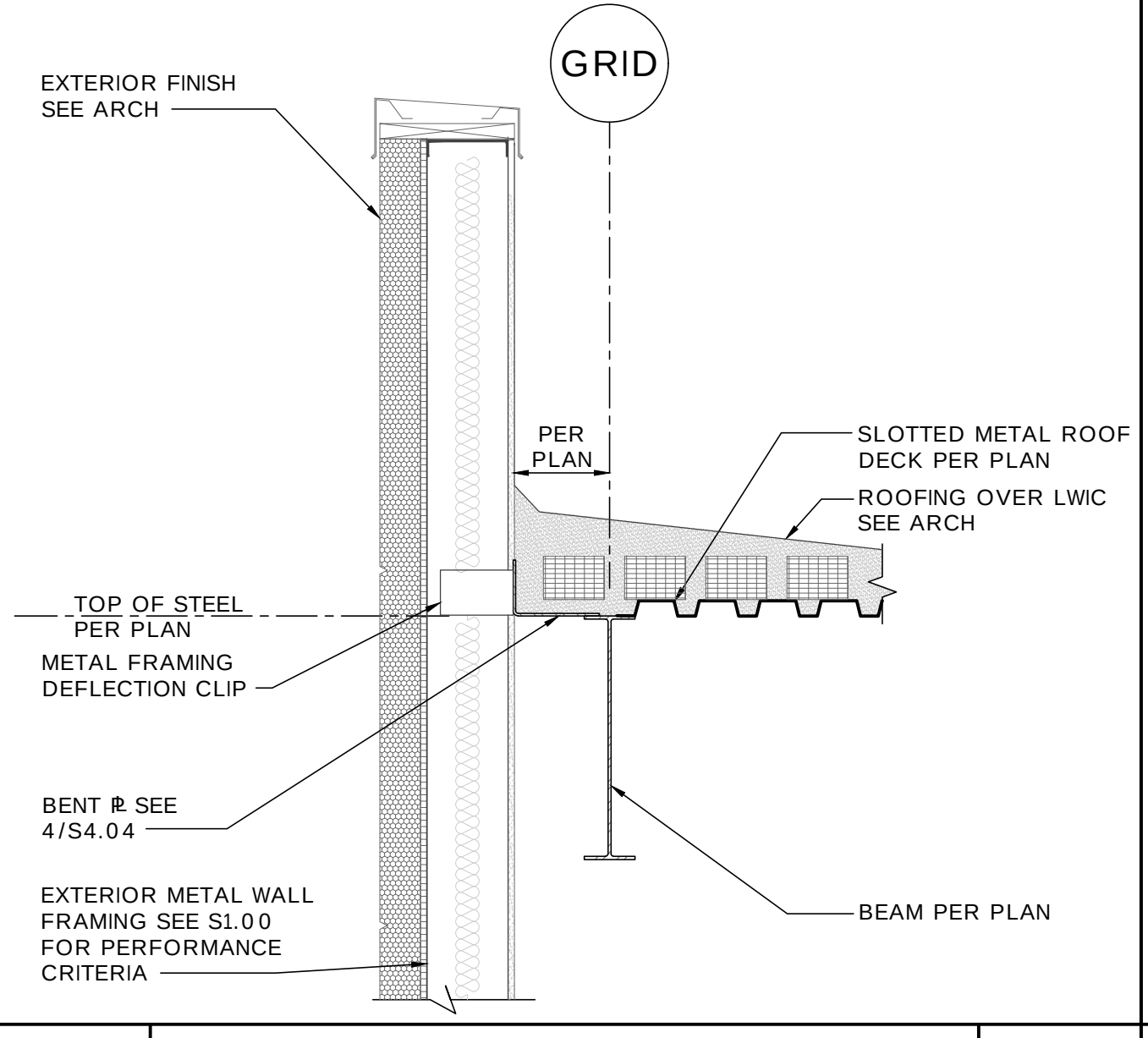
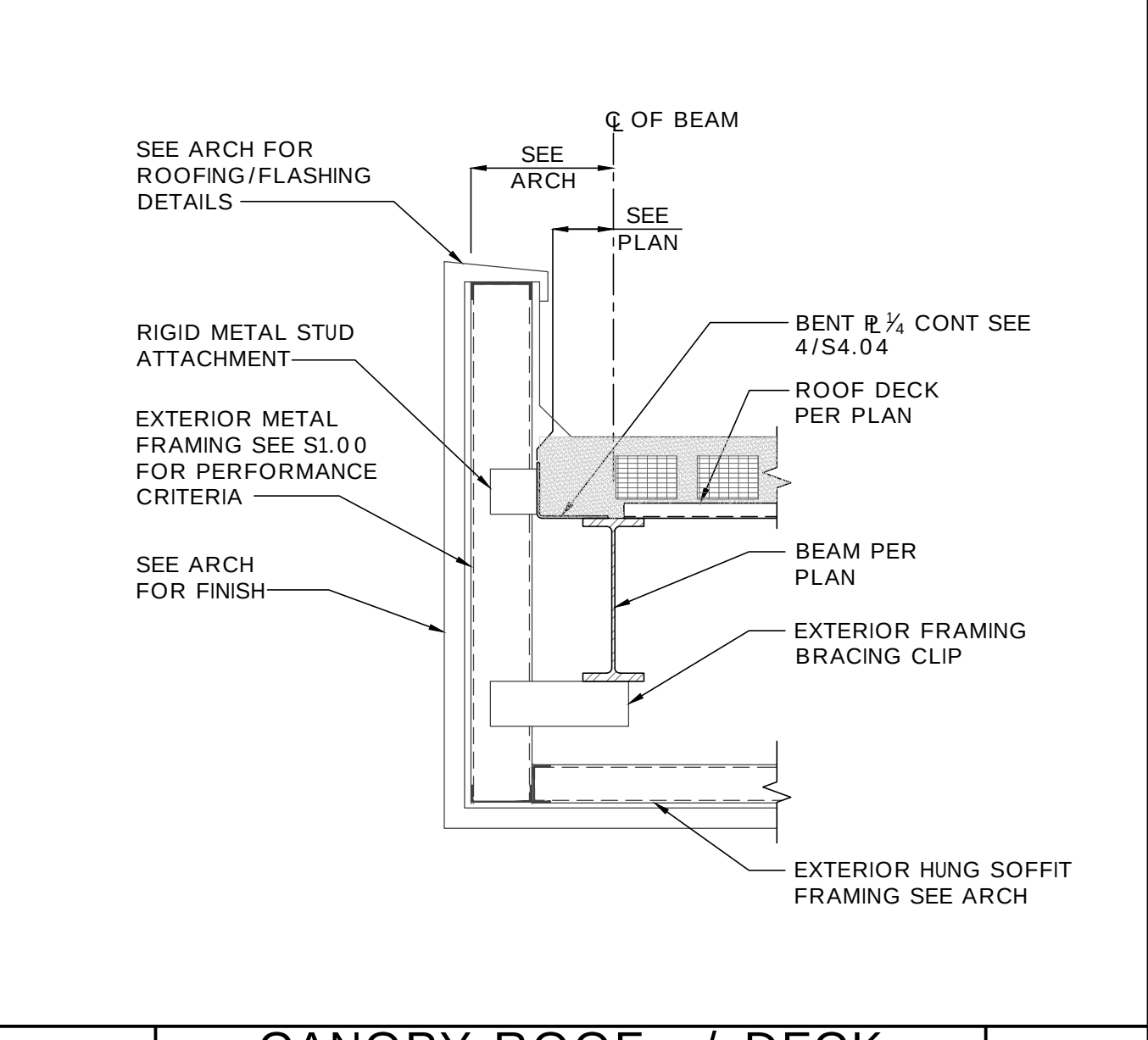
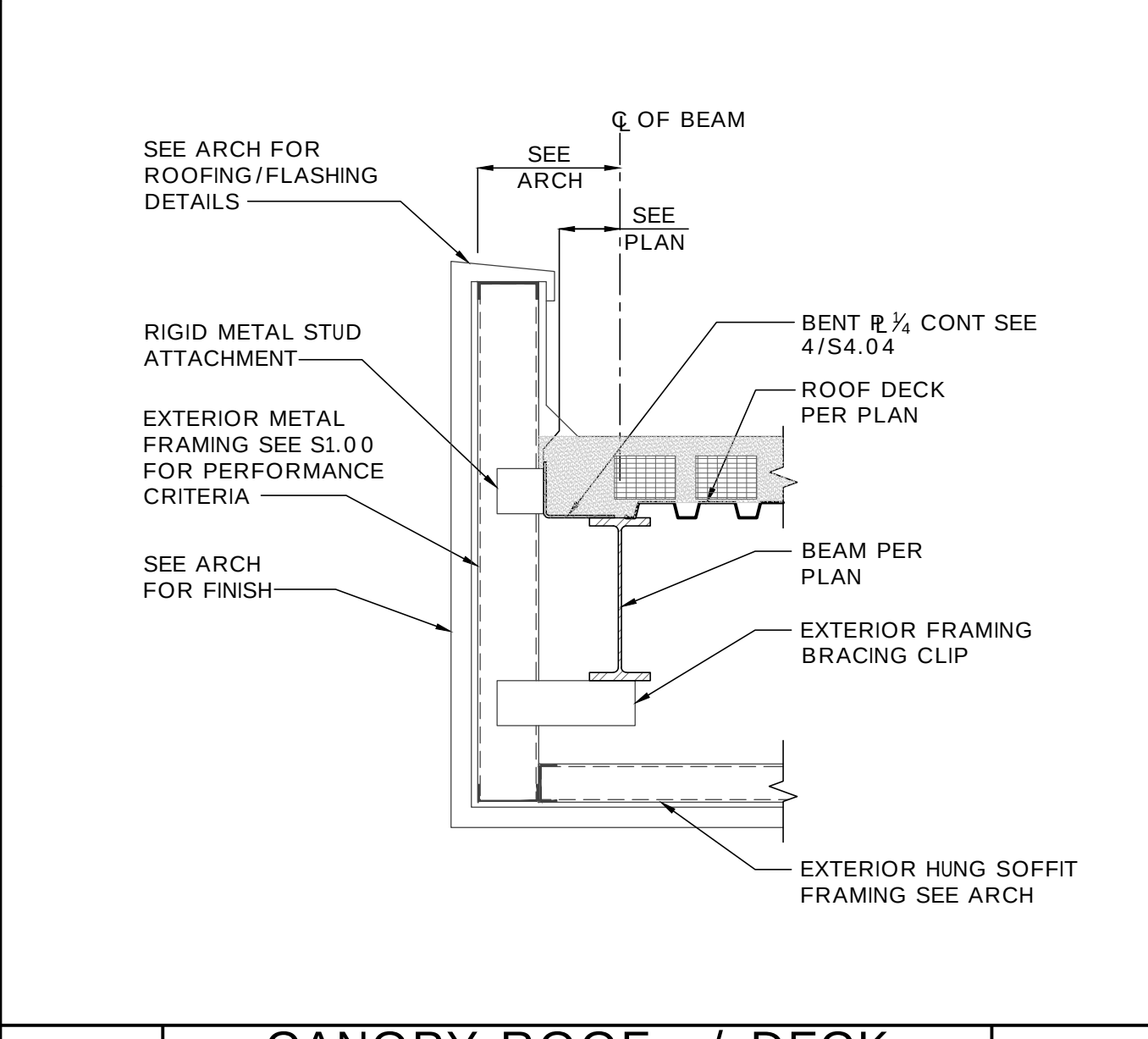
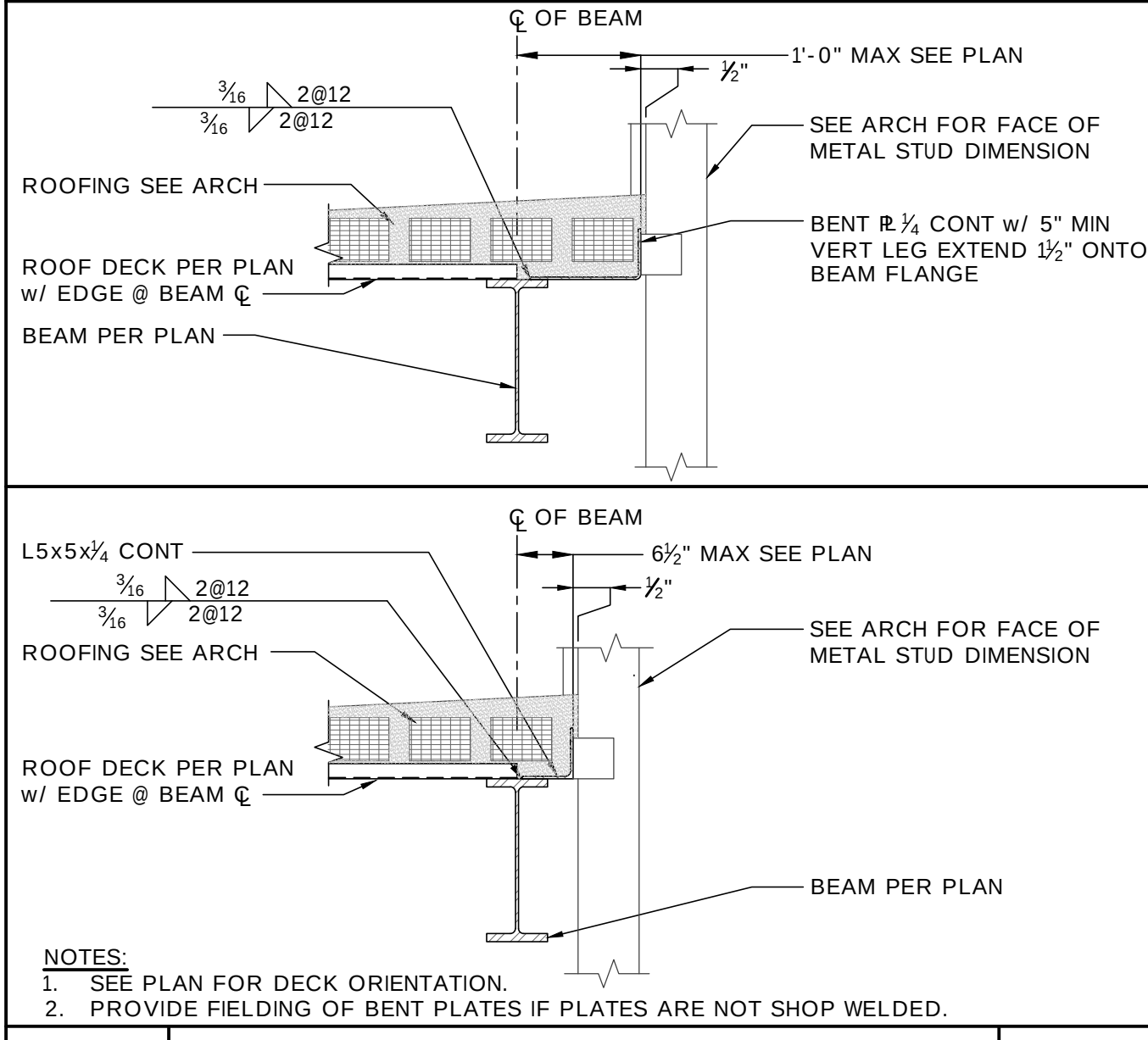
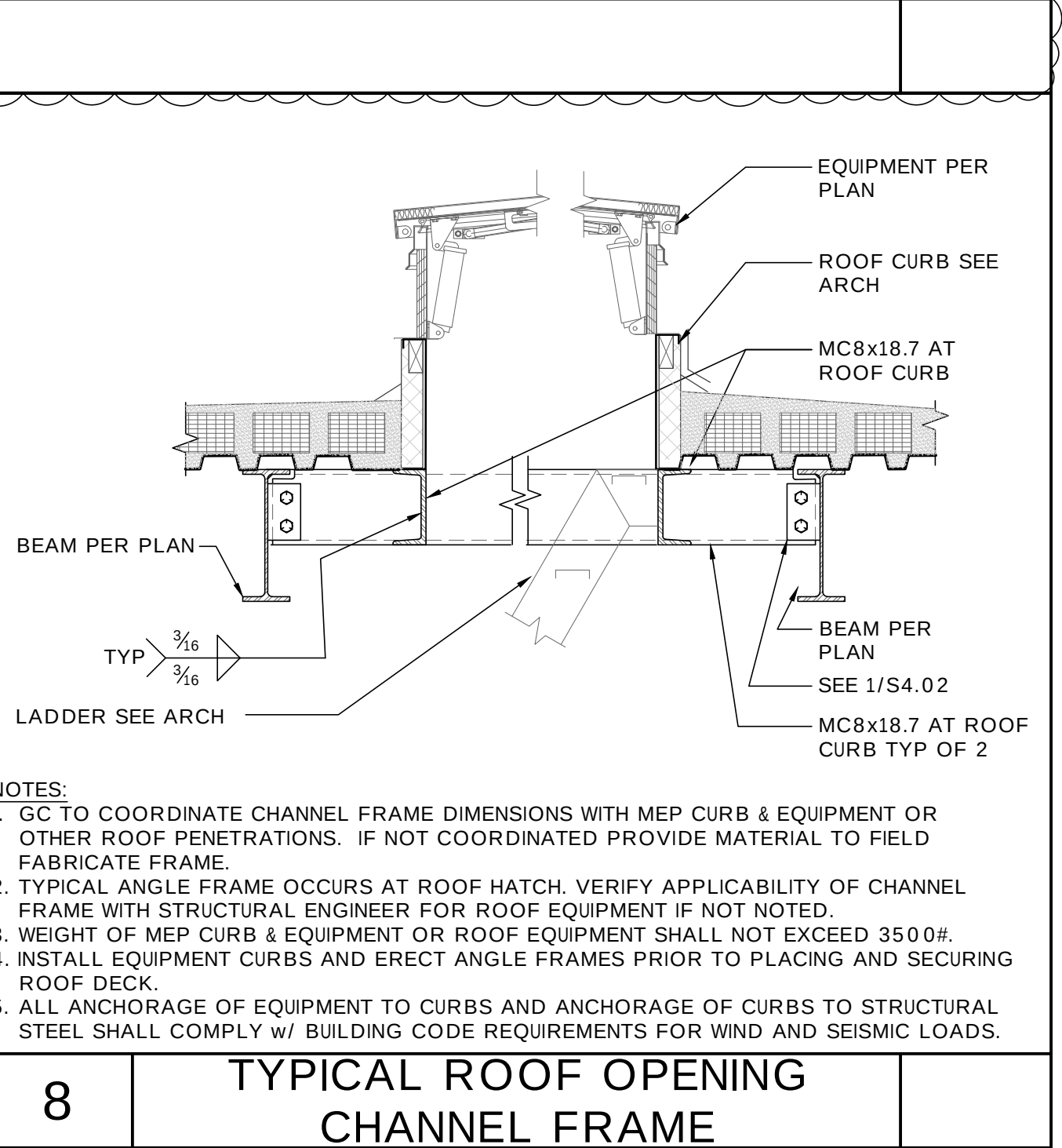
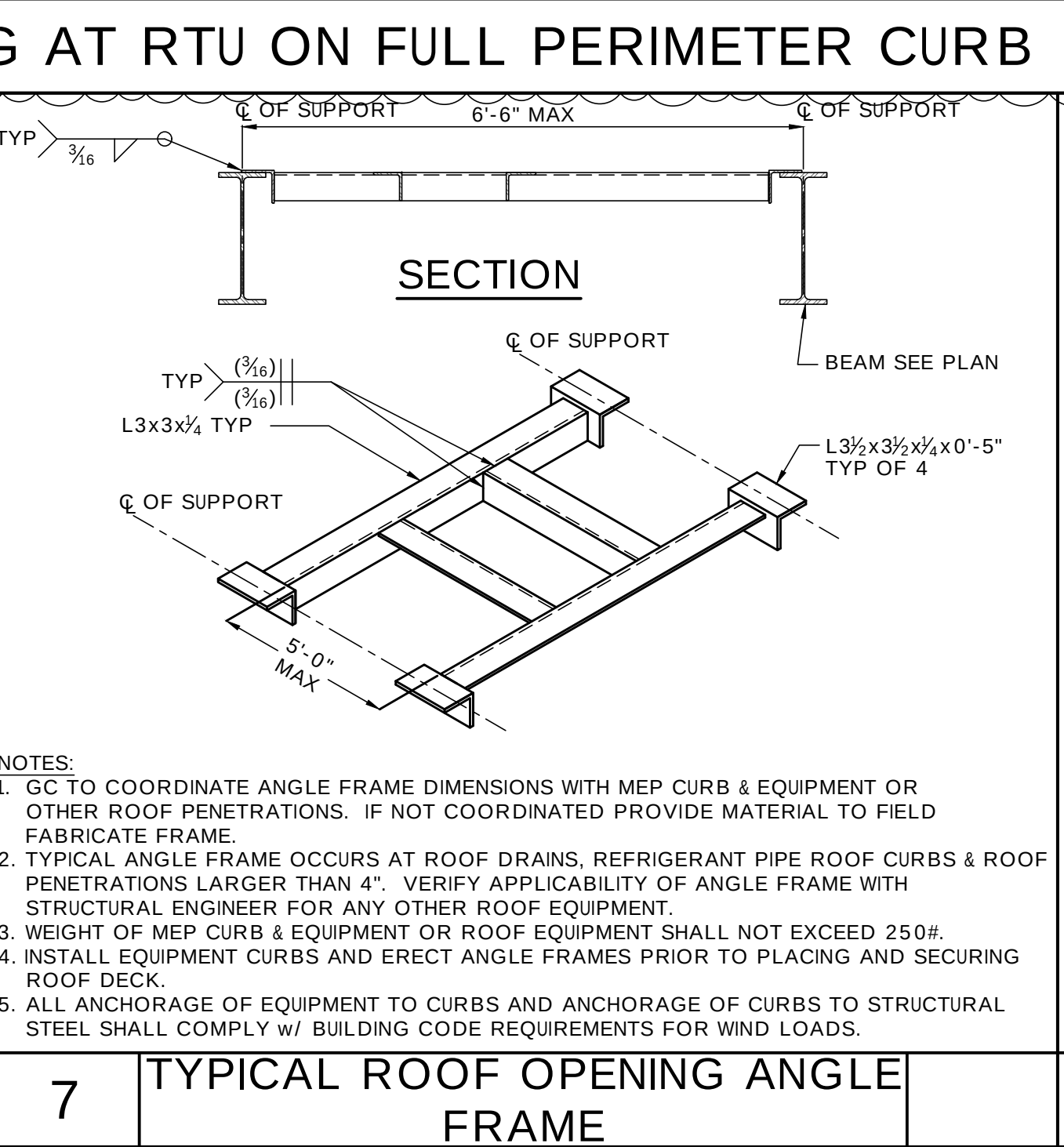
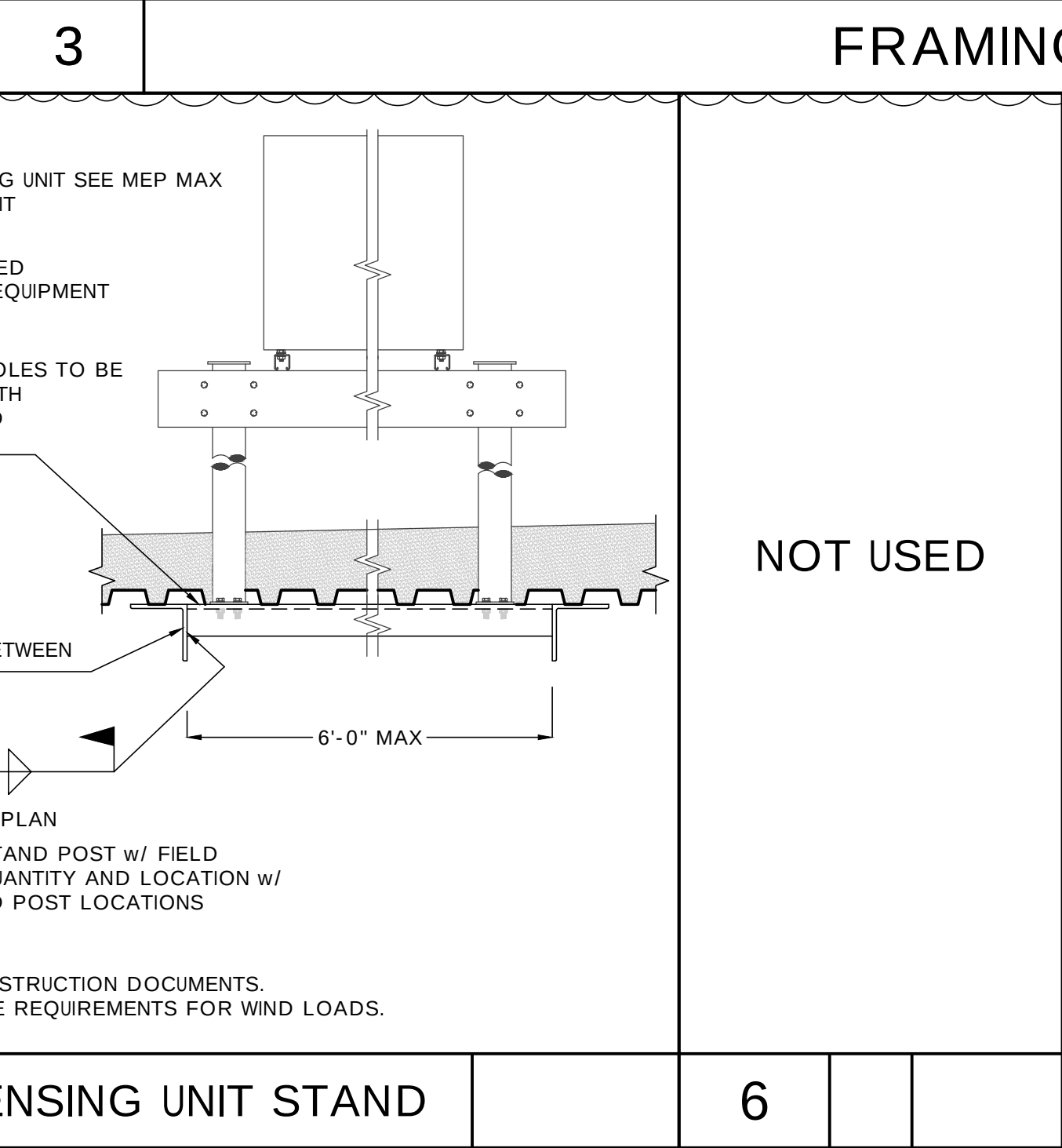
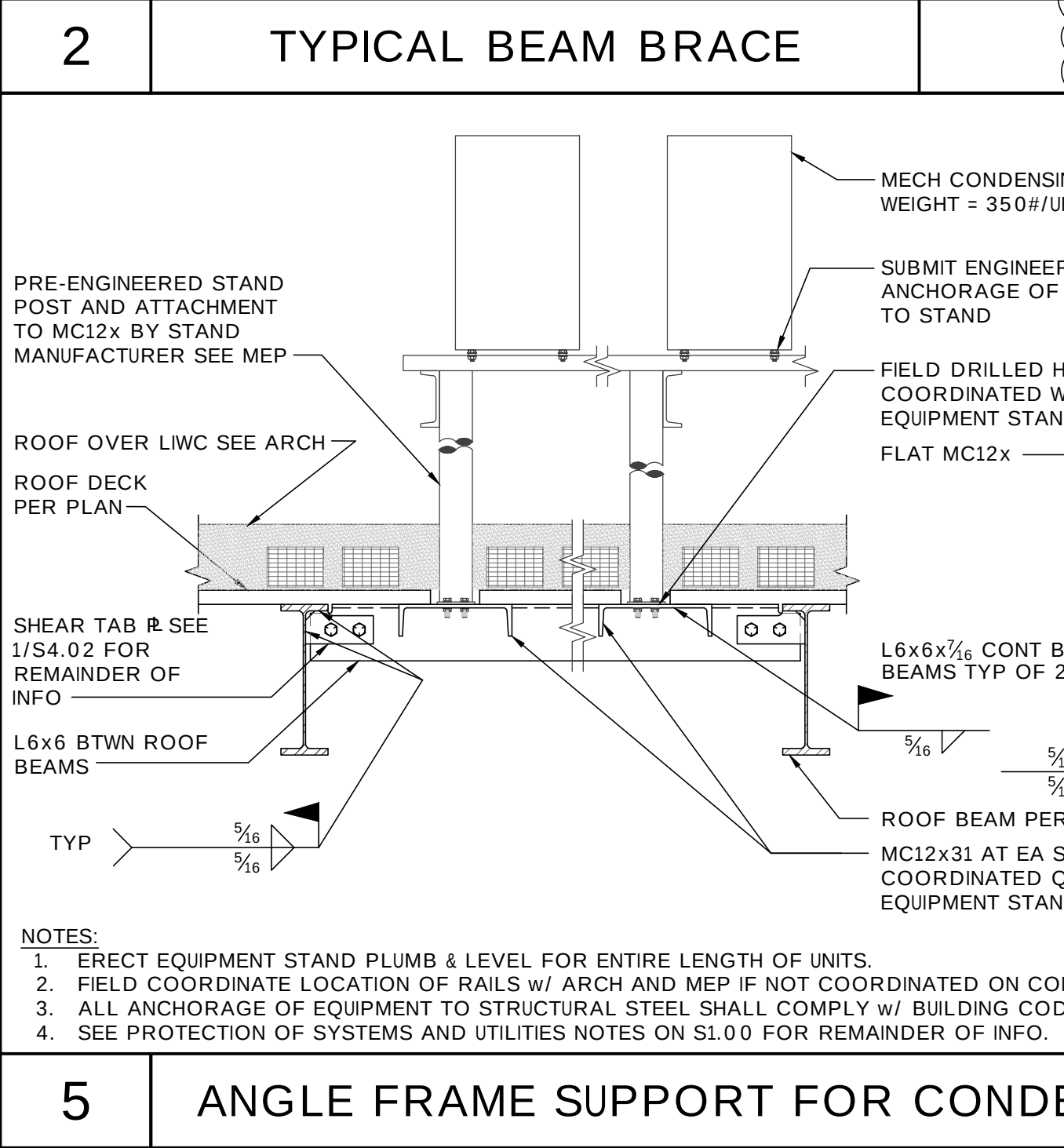
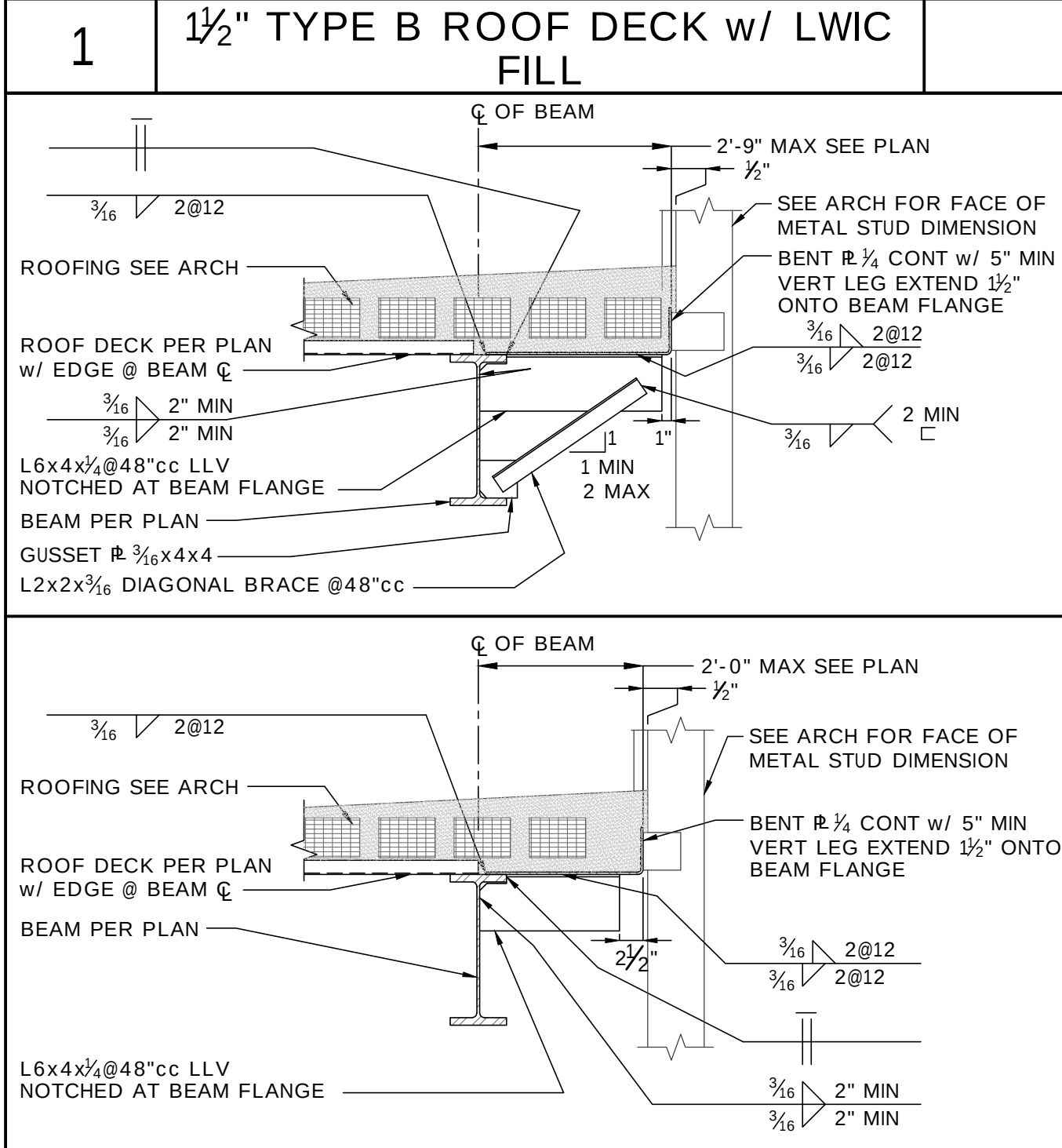
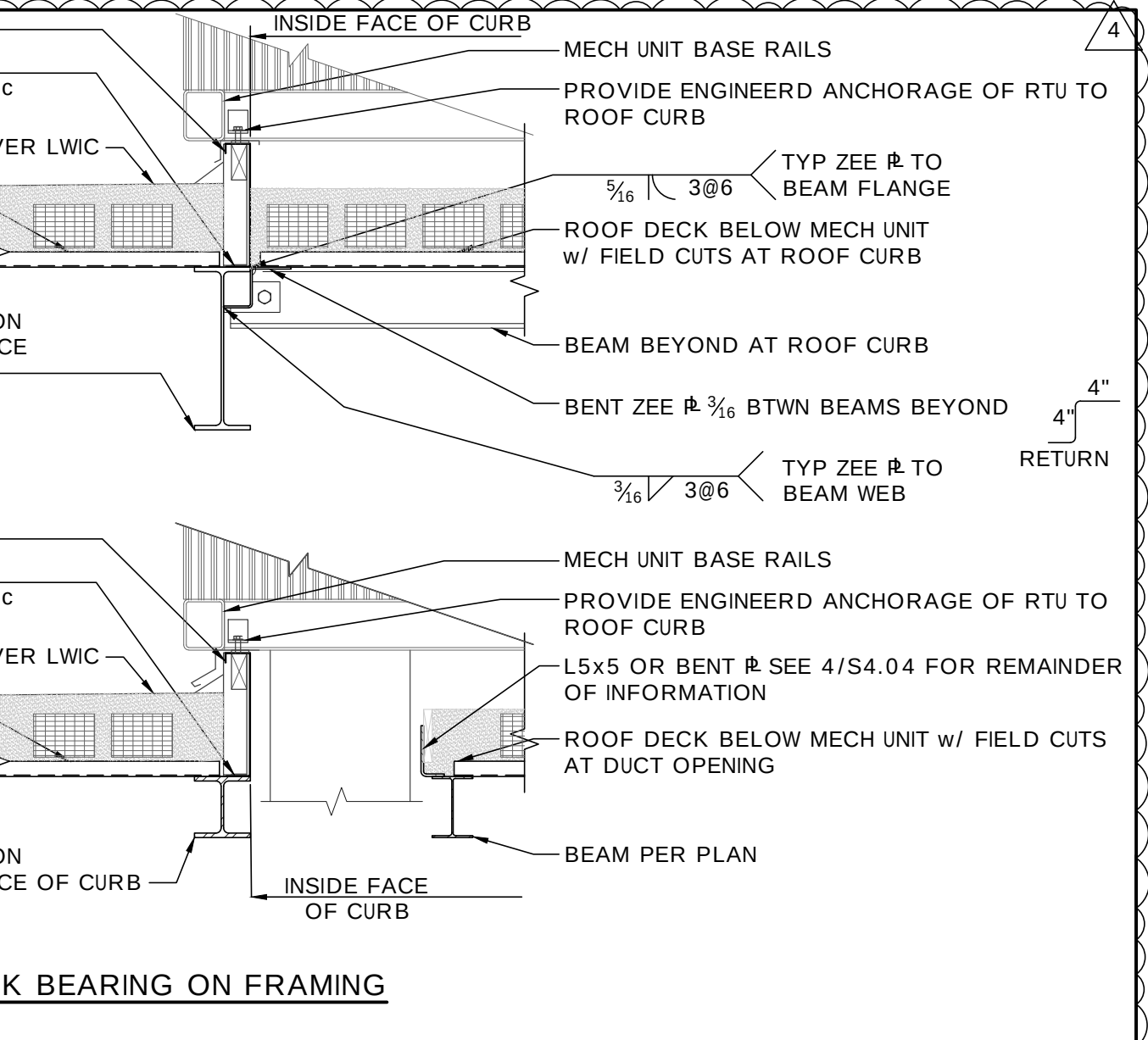
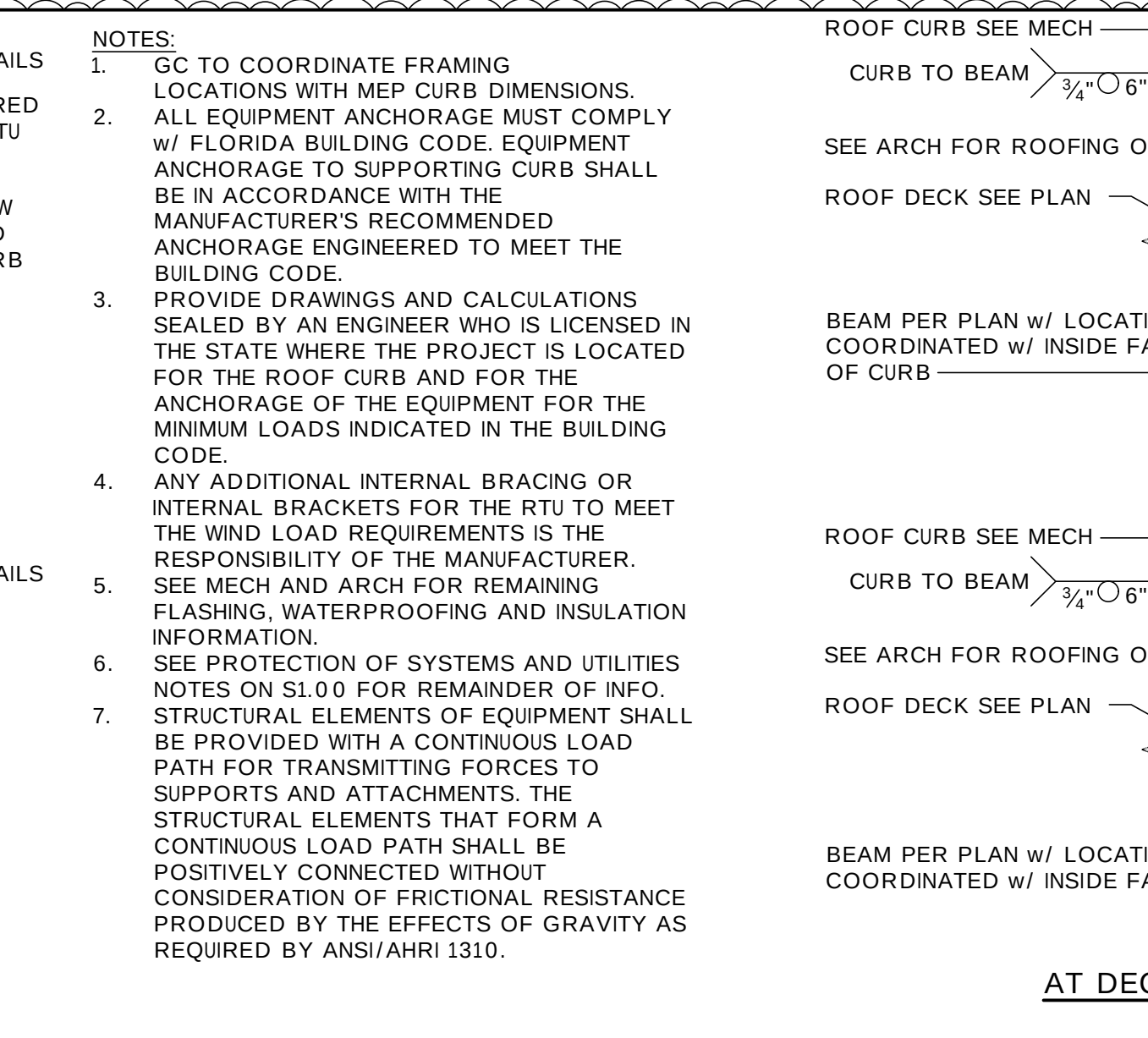
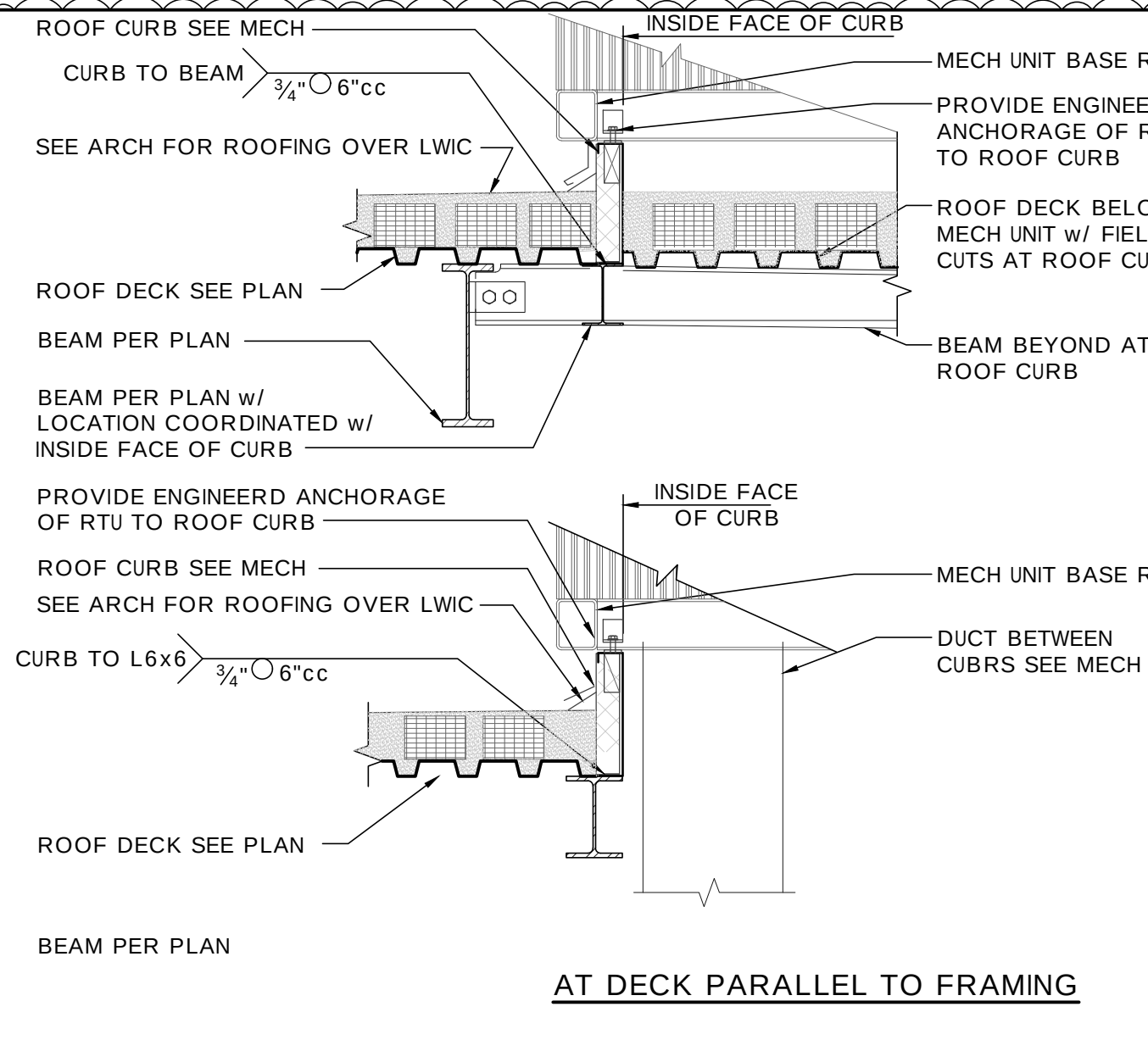
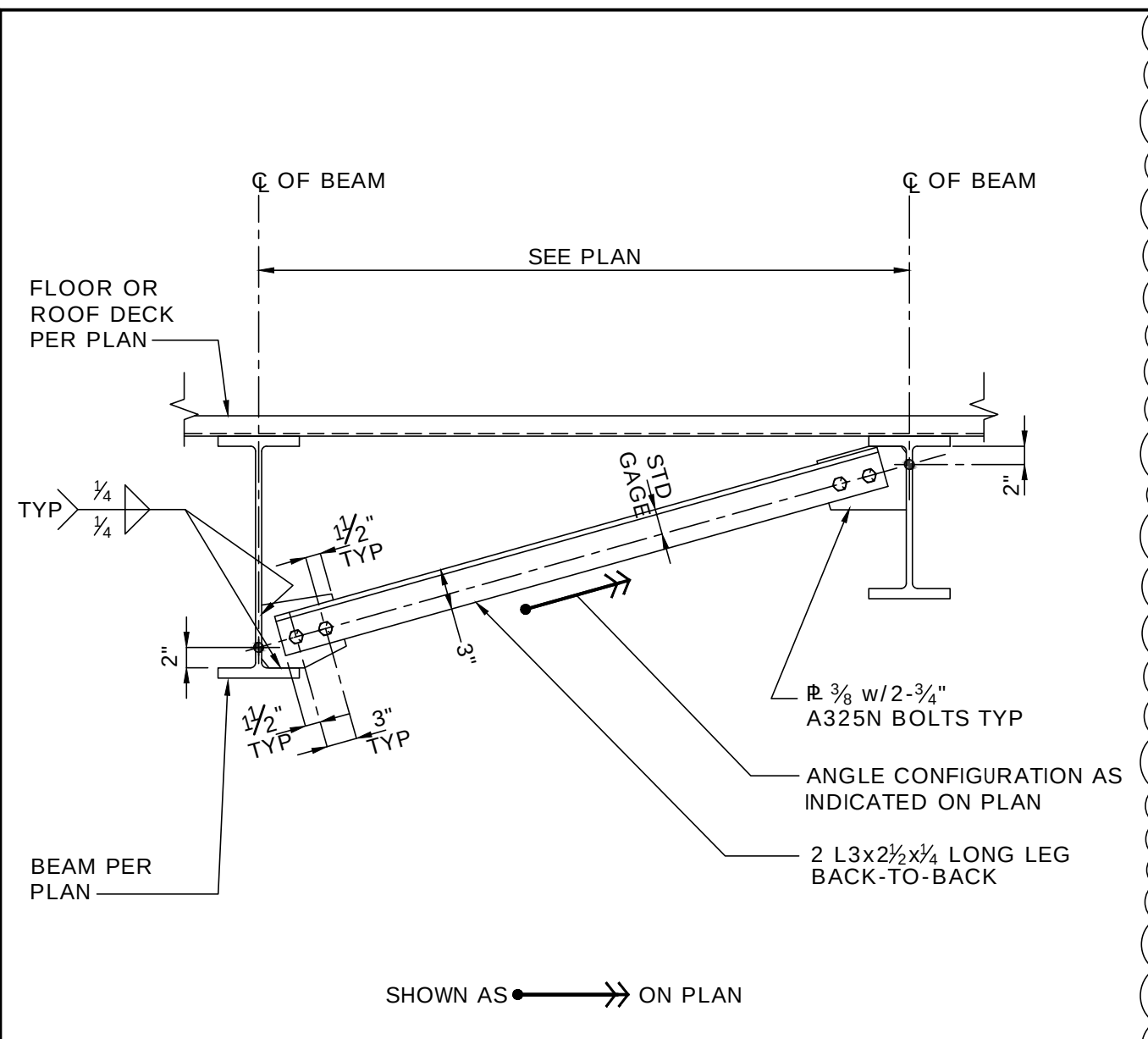
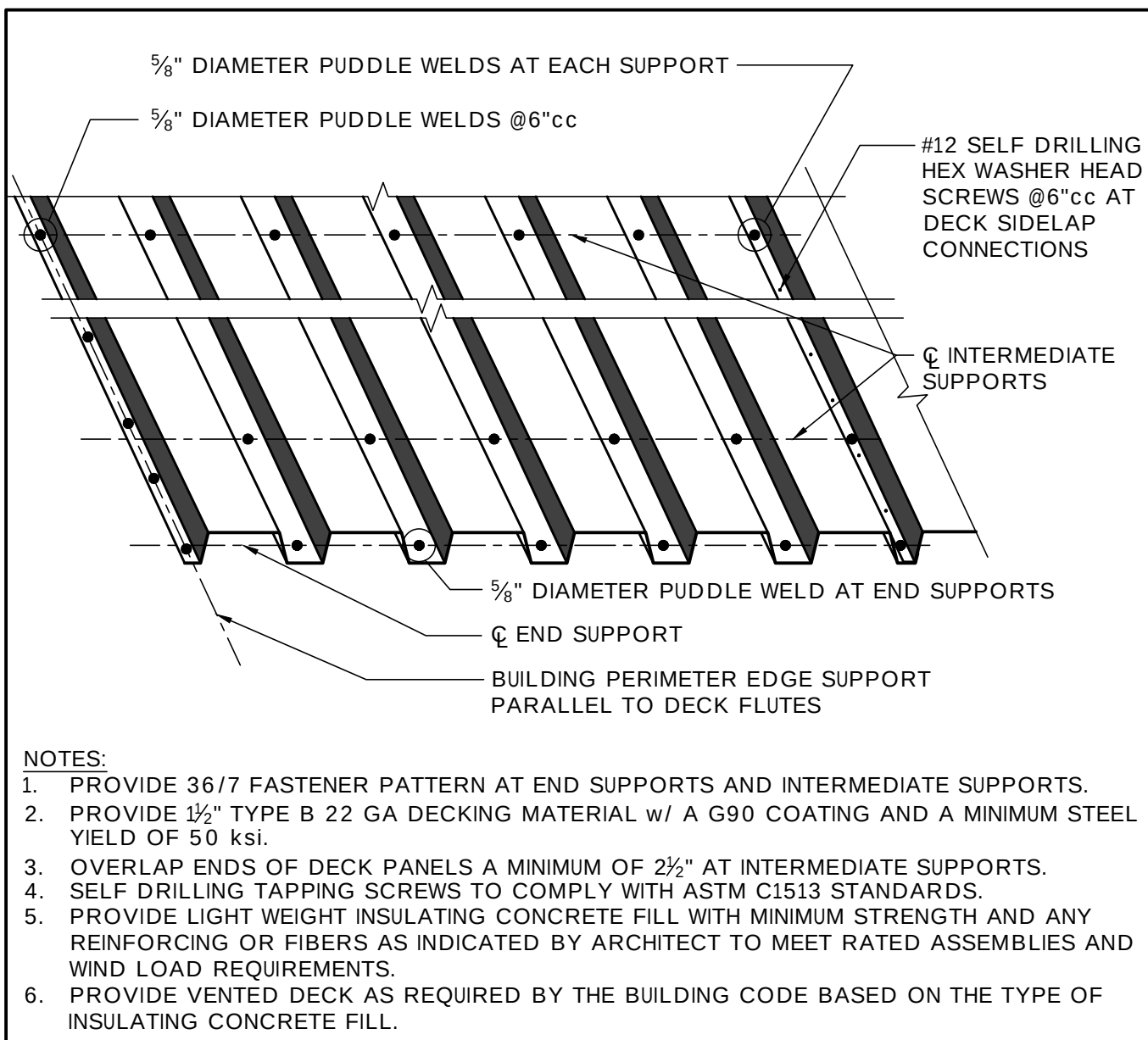
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NO.	DATE	DESCRIPTION

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S4.03
ISSUE DATE: 06.28.24 JOB NUMBER: 21059

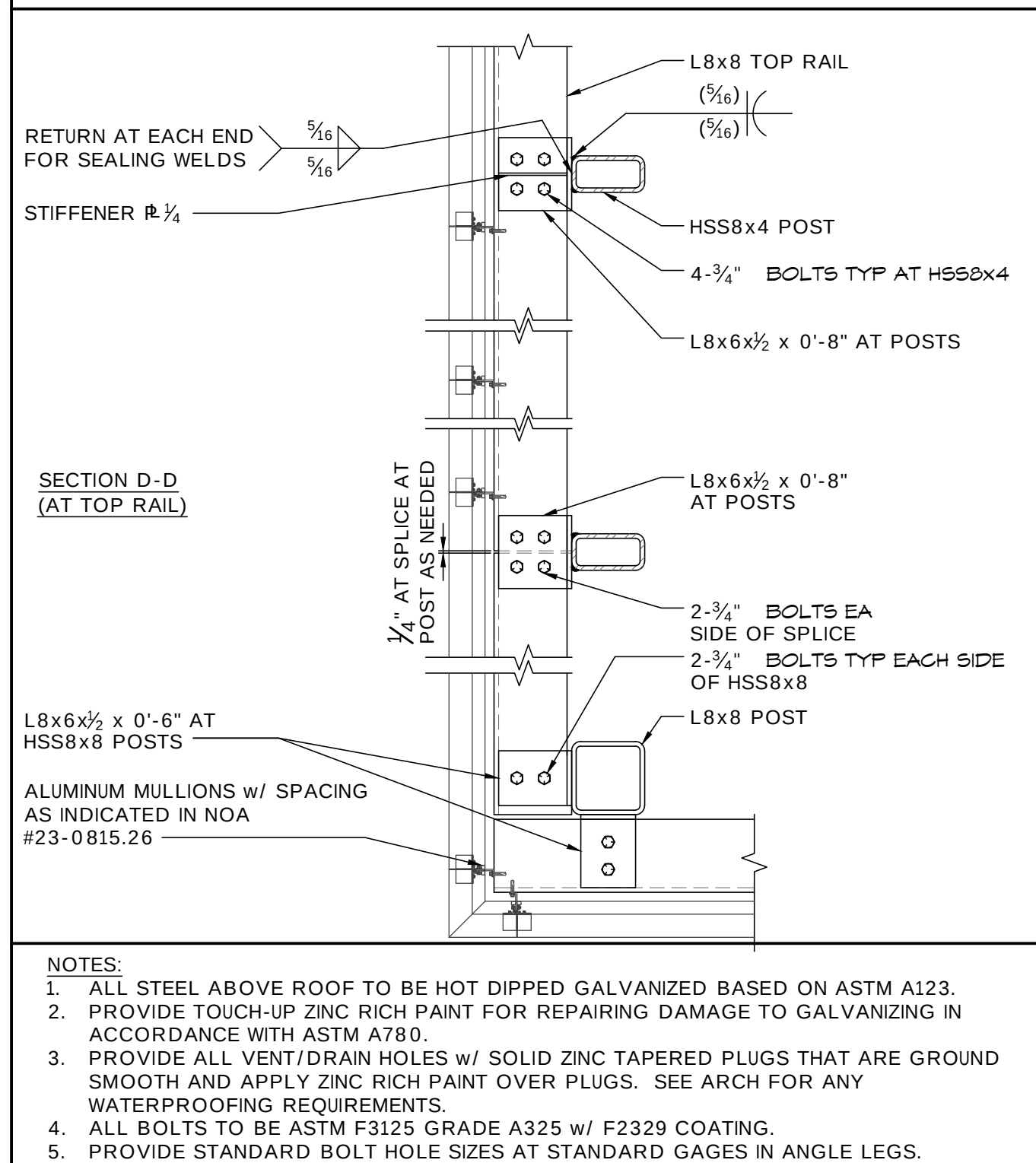
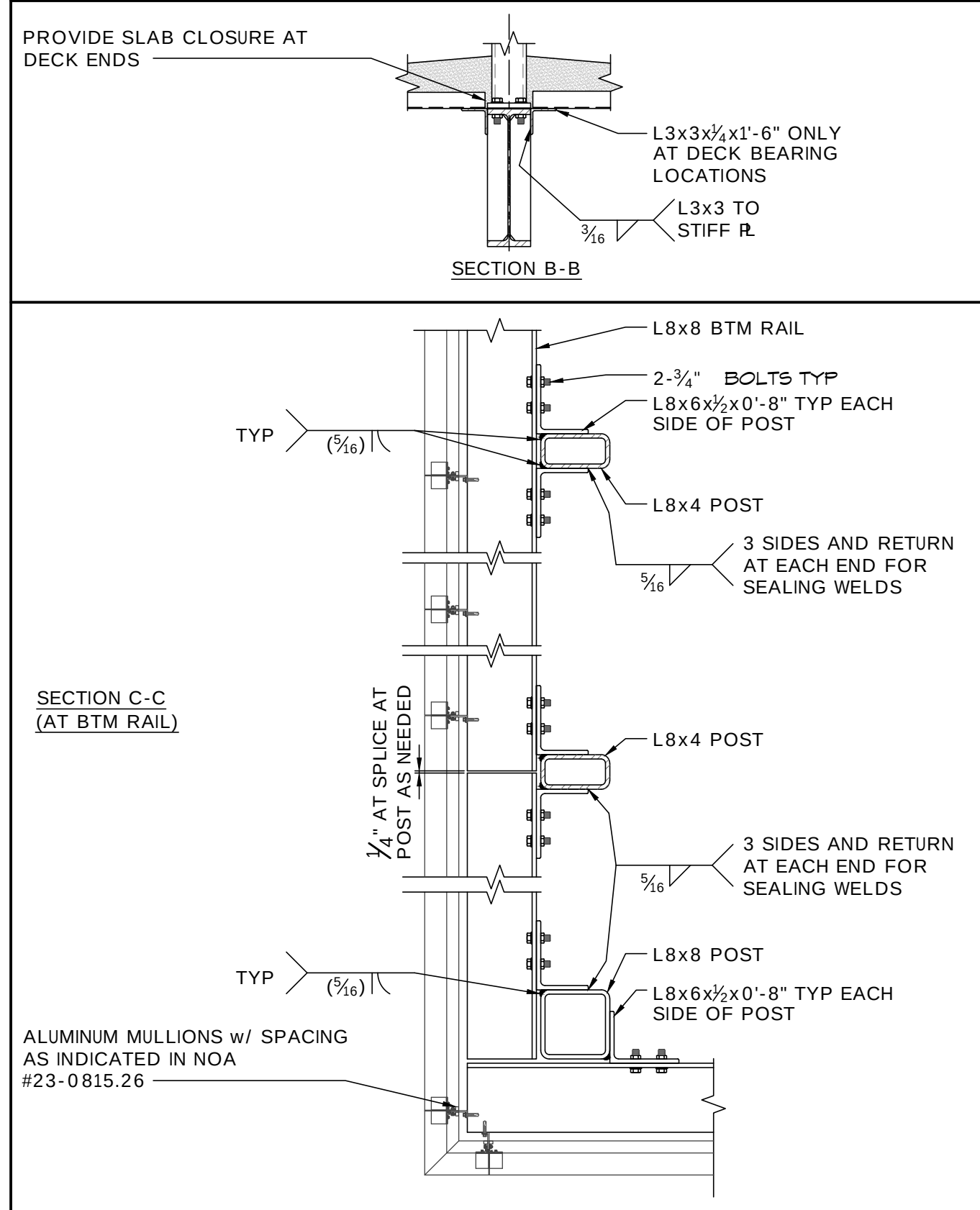
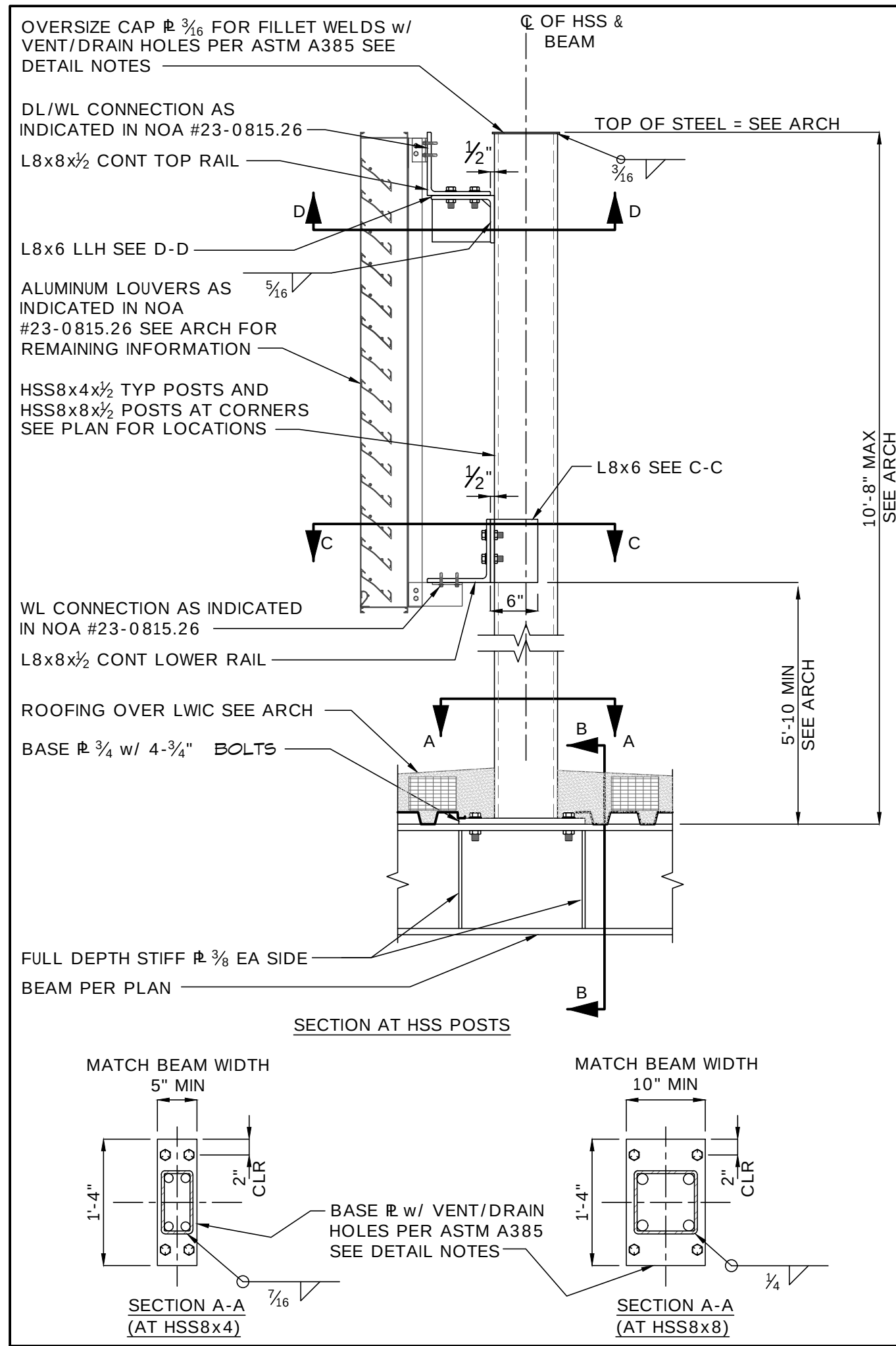


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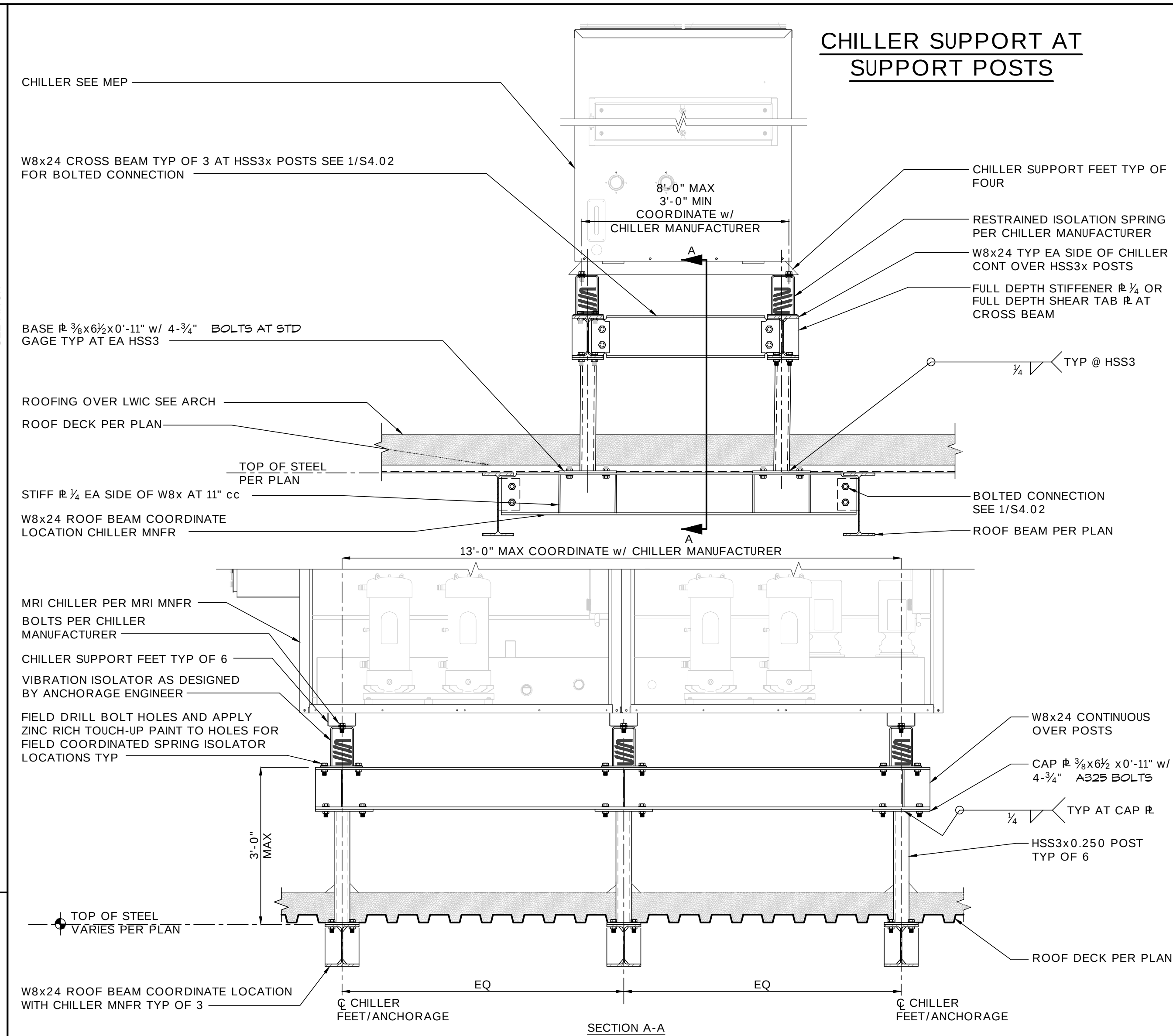
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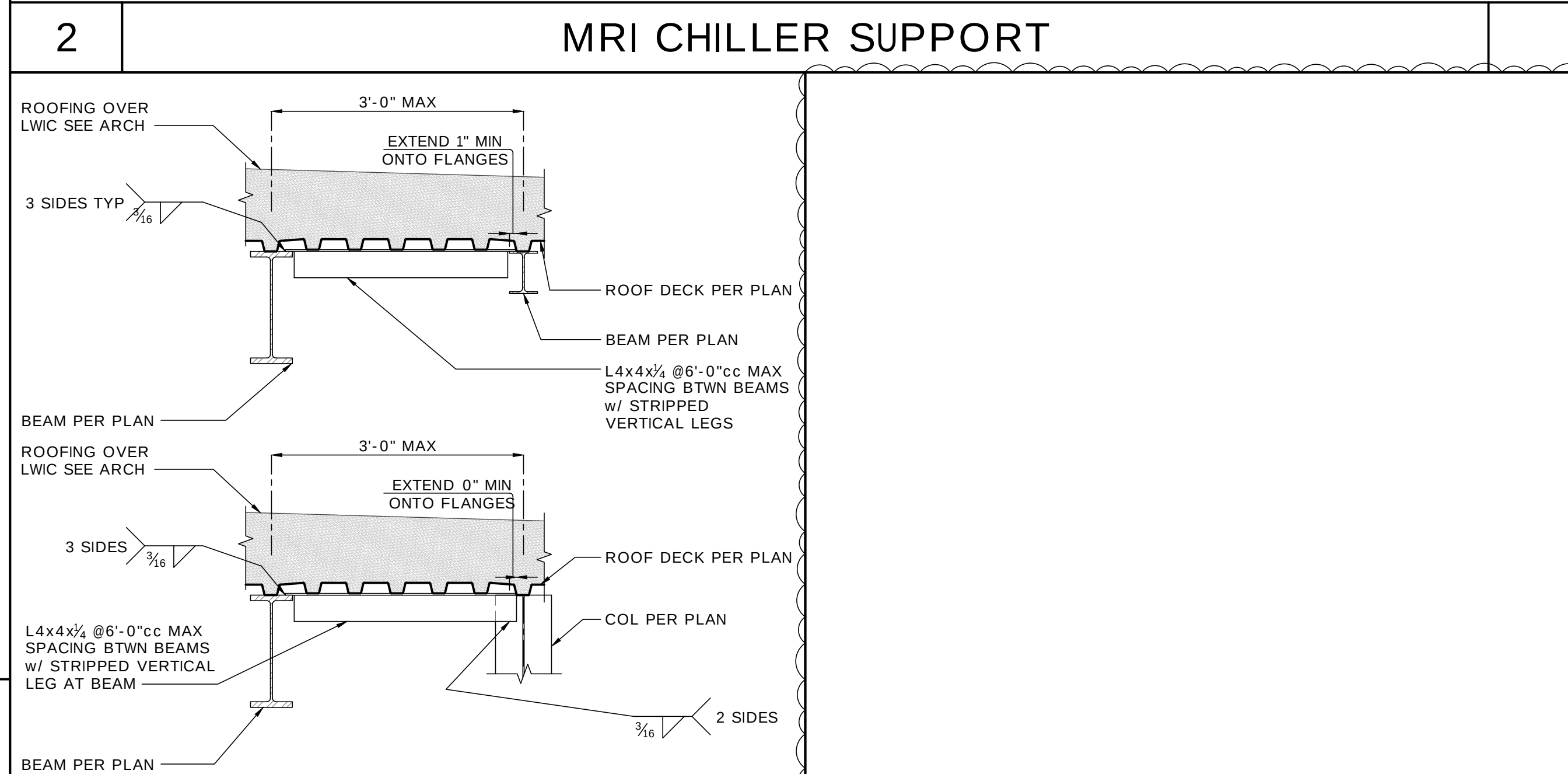
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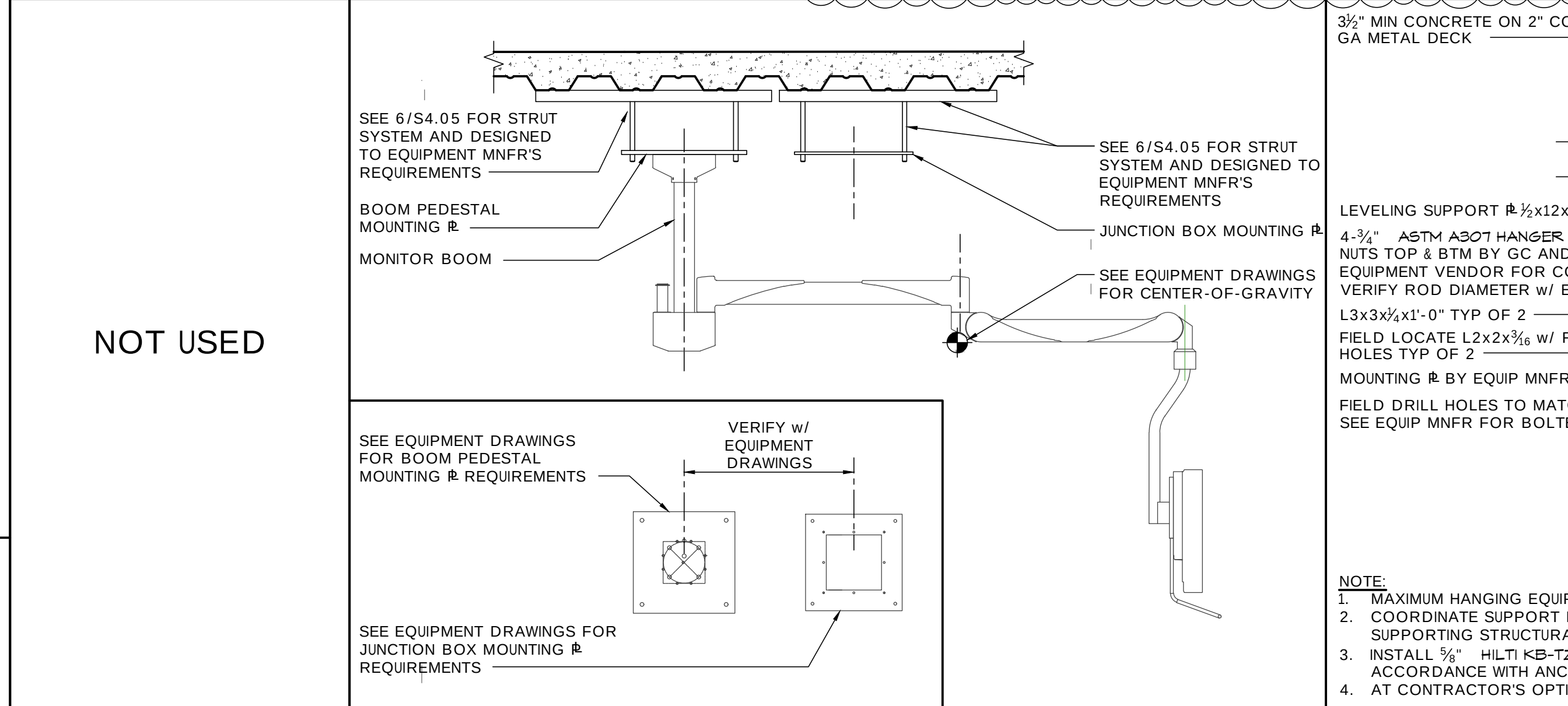
1 TYPICAL FRAMING AT ROOF SCREENING



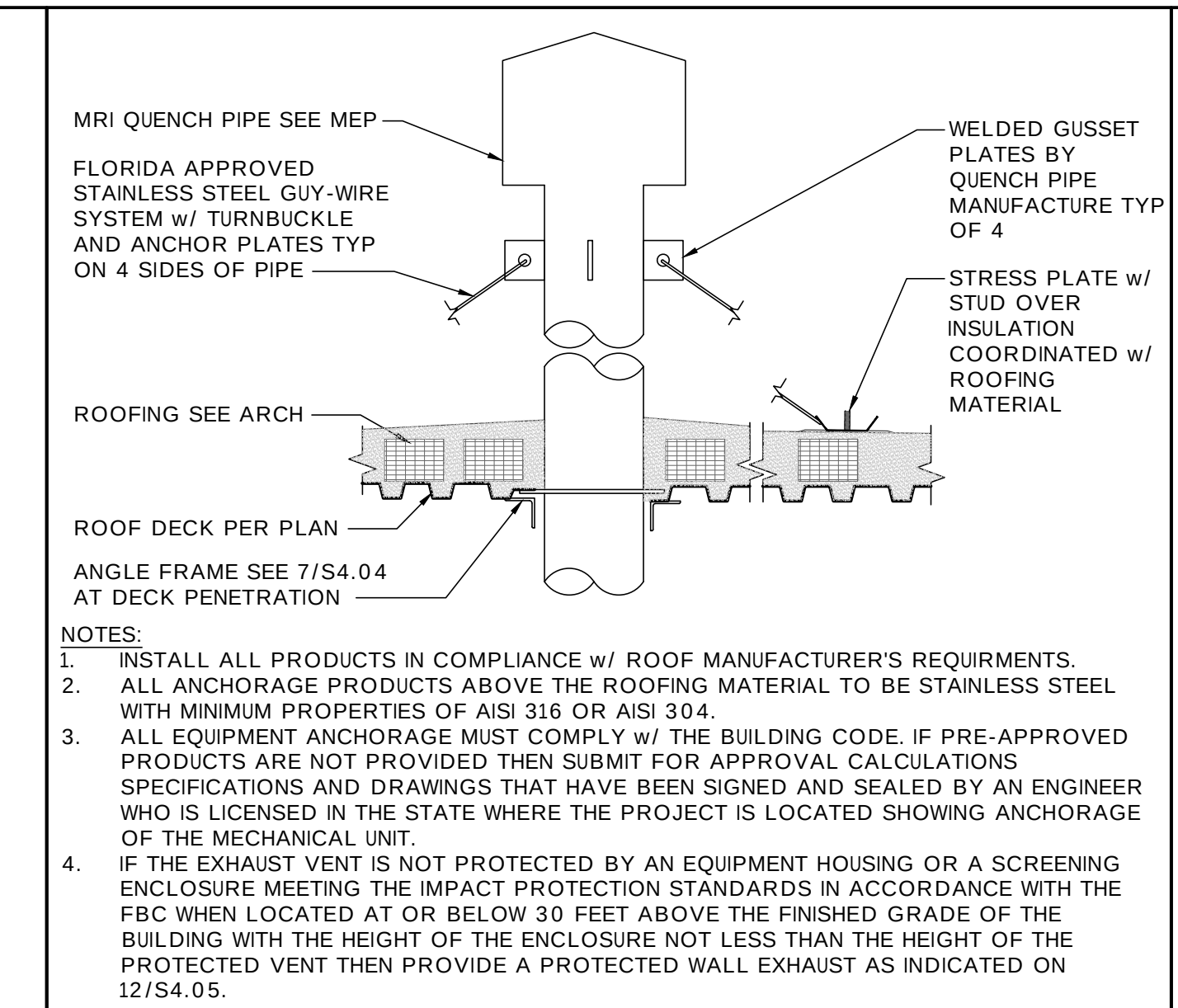
2 MRI CHILLER SUPPORT



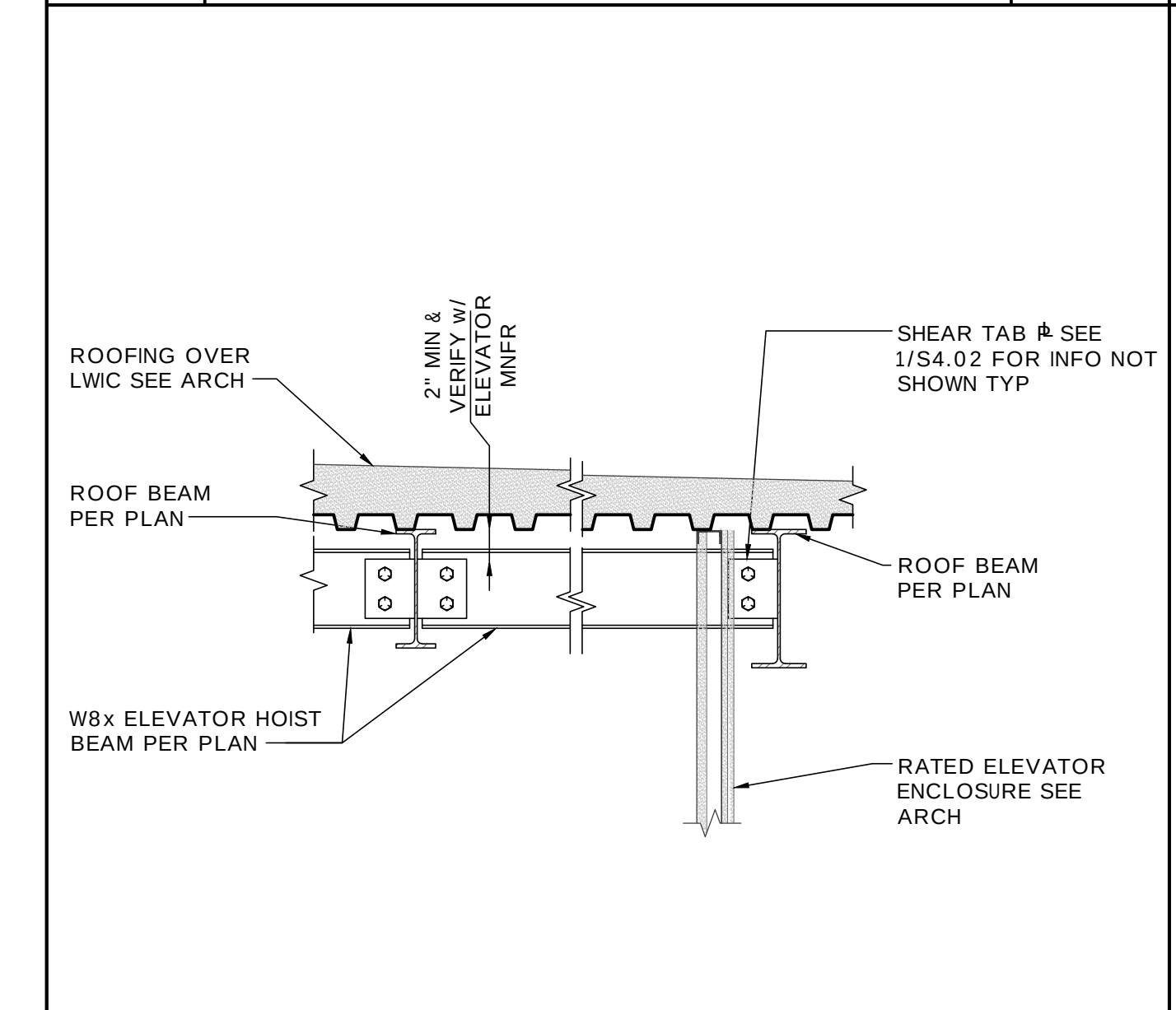
7 MISC L4x4 ROOF DECK SUPPORT



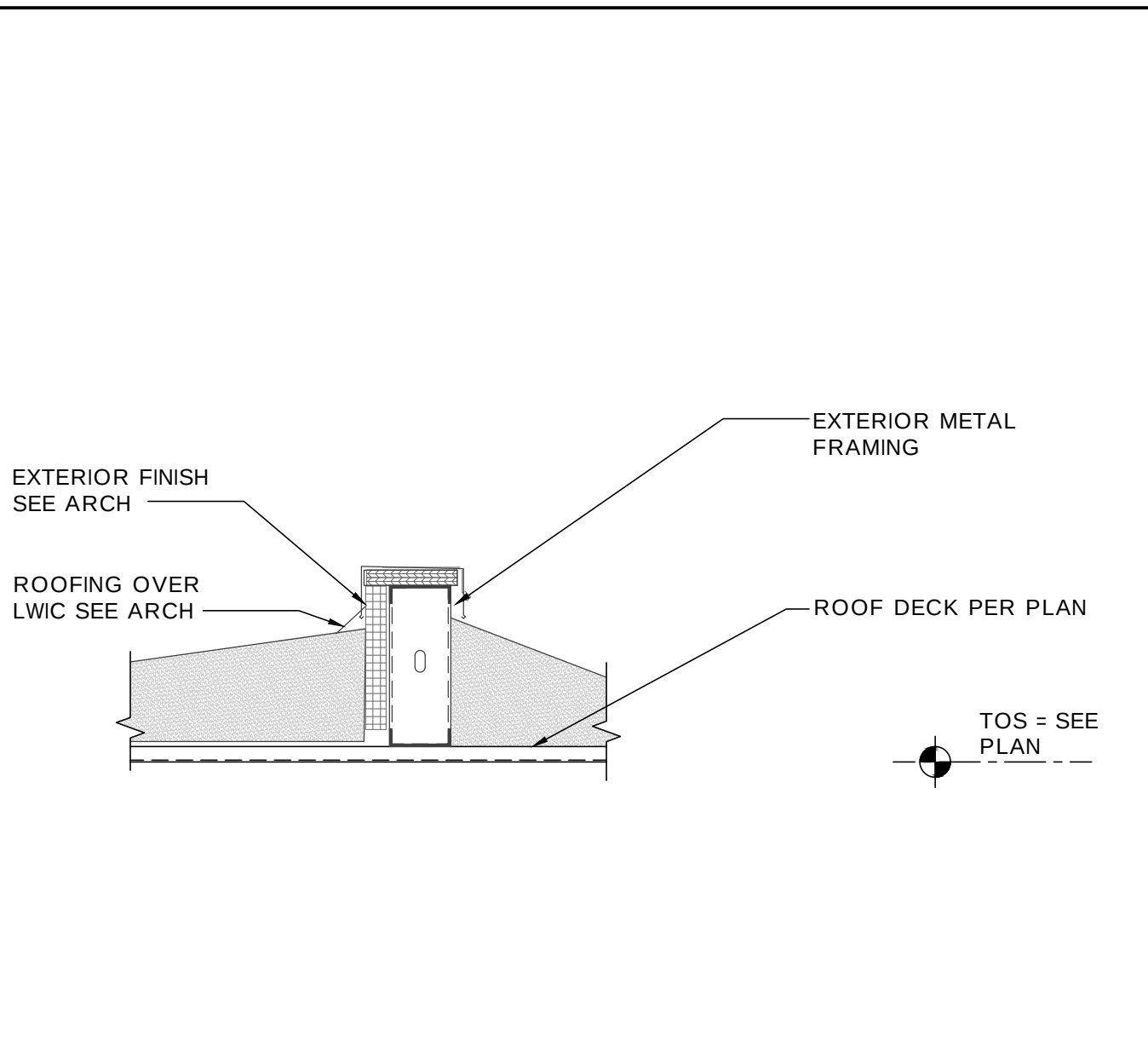
10 HANGING CT ROOM EQUIPMENT



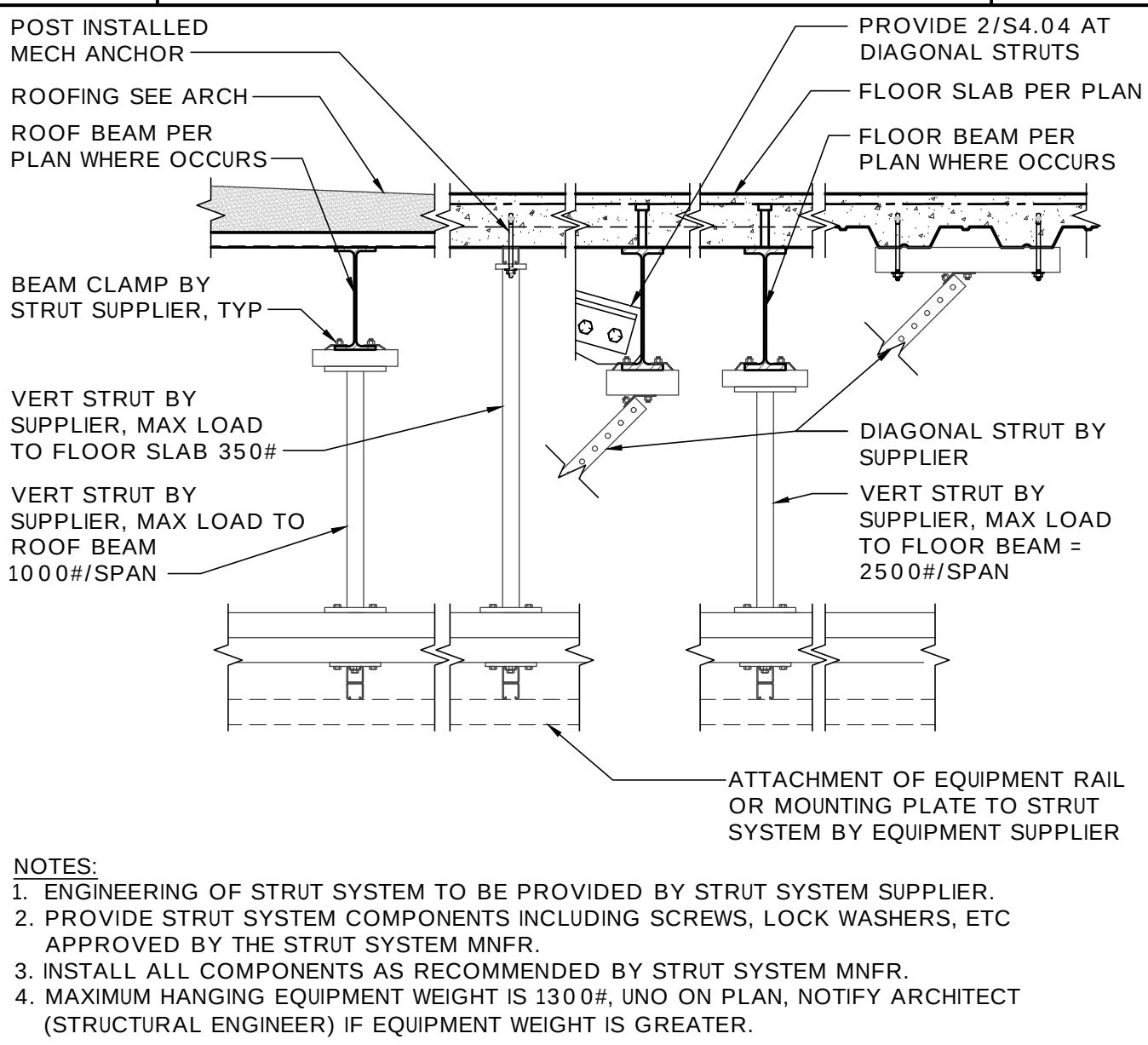
3 MRI QUENCH EXHAUST VENT GUY-WIRE ANCHOR



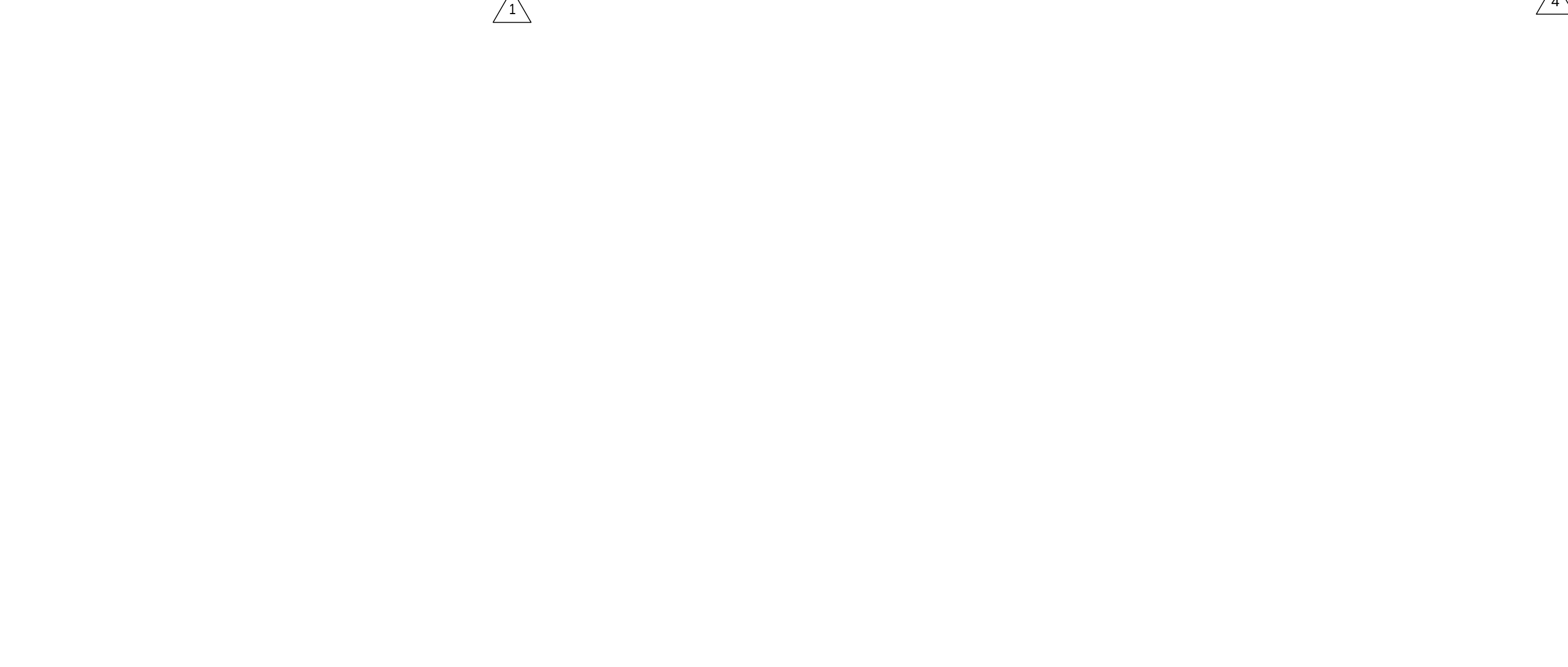
5 ELEVATOR HOIST BEAM CONNECTION



4 FREESTANDING PARAPET w/ EIFS ACROSS DECK



6 STRUT SYSTEM EQUIPMENT SUPPORT





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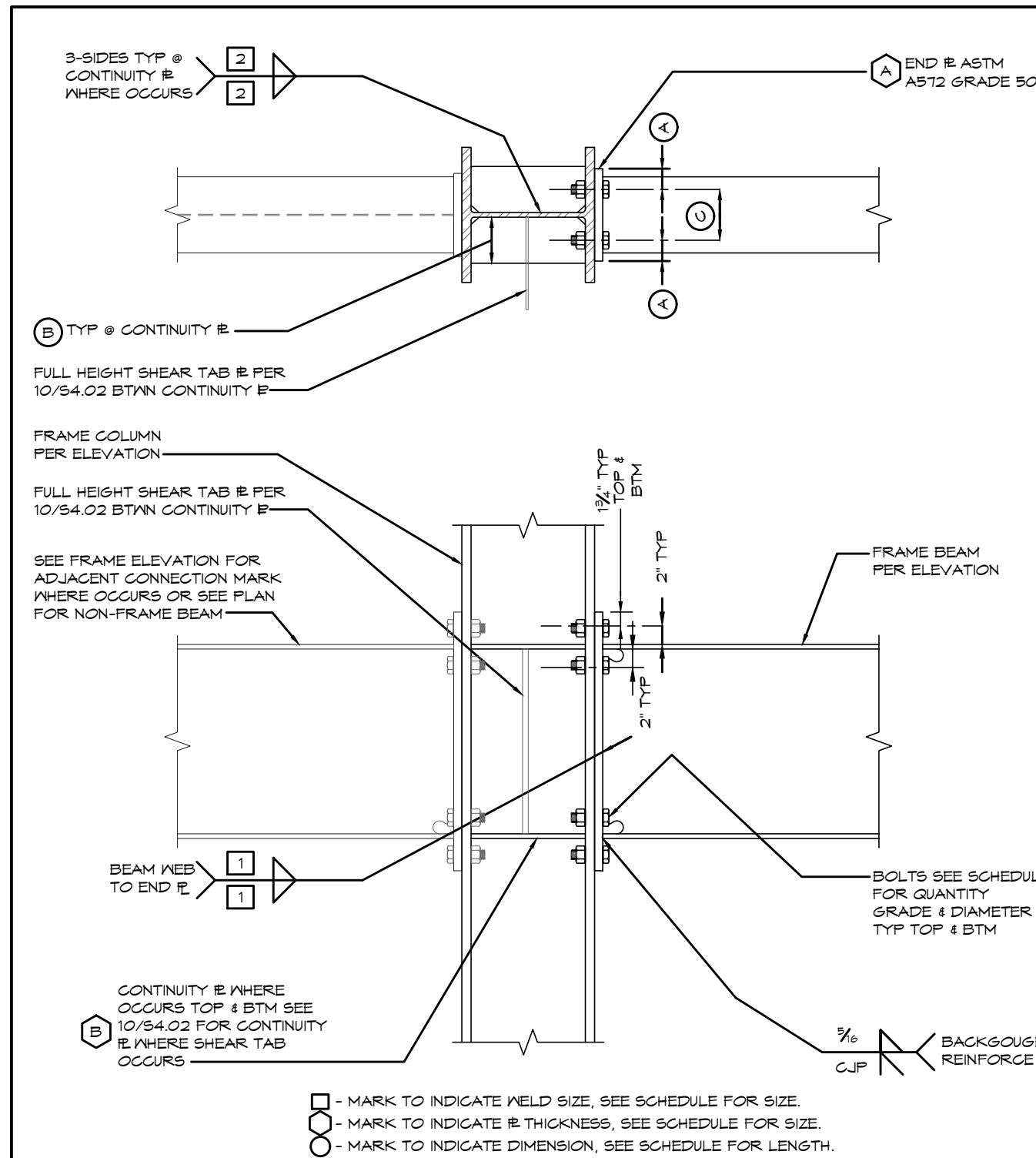
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FRAME ELEVATIONS

55.00

DUPLICATE DATE:	JOB NUMBER
06.28.24	21059



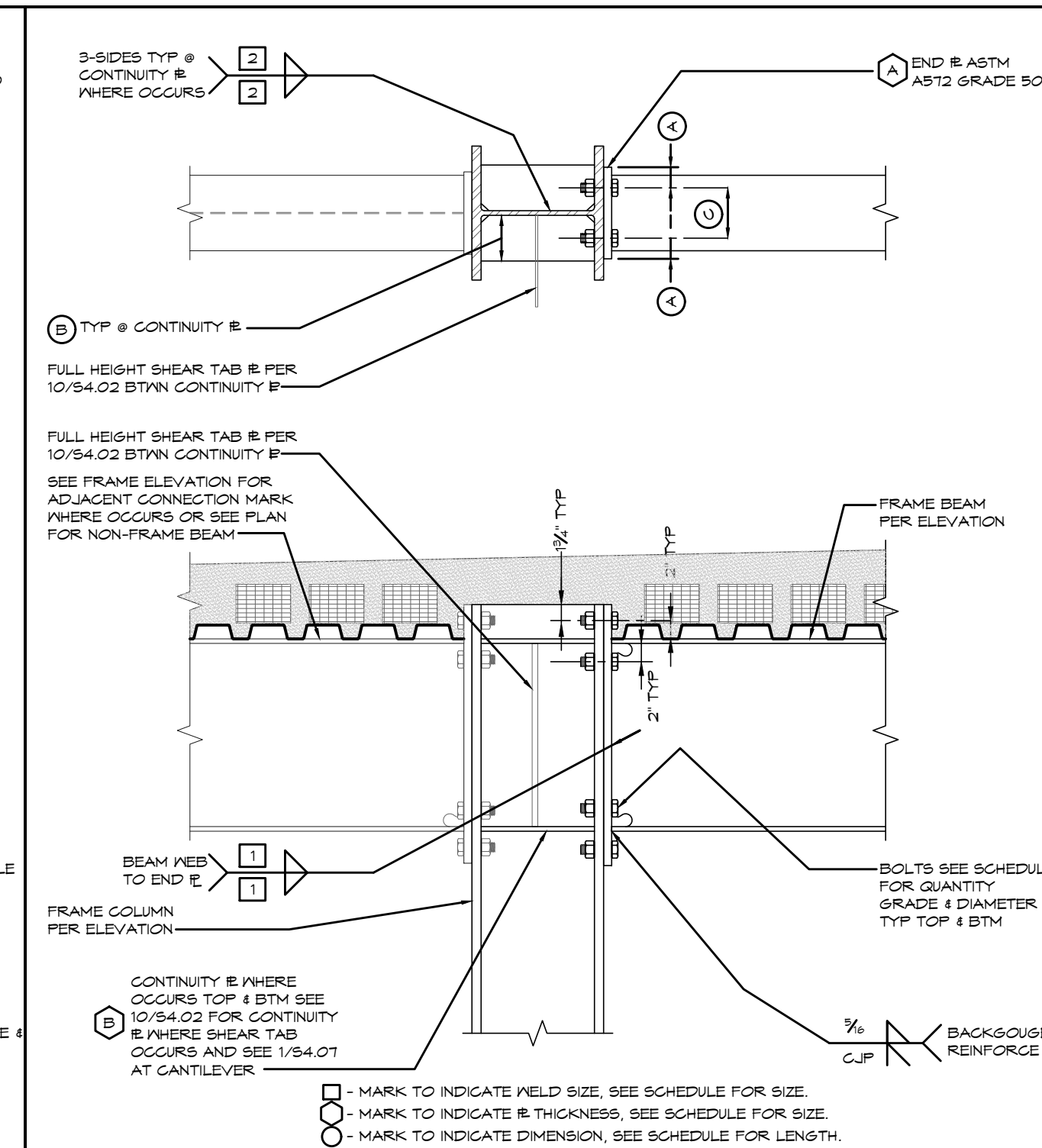


EXTENDED END-PLATE MOMENT CONNECTION SCHEDULE AT FLOOR											
CONNECTION MARK:	BOLTS			PLATE THICKNESS		PLATE DIMENSIONS		BOLT GAGE/PITCH	YIELD SIZE		
	QUANTITY	ASTM F1552 GRADE	DIAMETER	A	B	A	B		C	1	2
1A	4 TOP & 4 BTM	A490	7/8"	7/8"	7/8"	2 1/2"	5"	5 1/2"	7/8"	7/8"	
1B	4 TOP & 4 BTM	A490	7/8"	7/8"	7/8"	2 1/2"	5"	5 1/2"	7/8"	7/8"	

NOTES:

1. AT CONTRACTOR'S OPTION PROVIDE FINGER SHIM @ $\frac{3}{16}$ " MAX THICKNESS BETWEEN BEAM AND COLUMN TO ACCOUNT FOR ROLLING TOLERANCES AND ERECTION.
2. ALL BOLTS TO BE FULLY TENSIONED.
3. PROVIDE A SYSTEMATIC SNUGGING OF THE JOINT/BOLTS FOLLOWED BY A MATCHING SYSTEMATIC TENSIONING OF THE BOLTS.
4. PROVIDE STANDARD AISC WELD ACCESS HOLES FOR FLANGE GROOVE WELDS.

1	EXTENDED END-PLATE MOMENT CONNECTION AT FLOOR	
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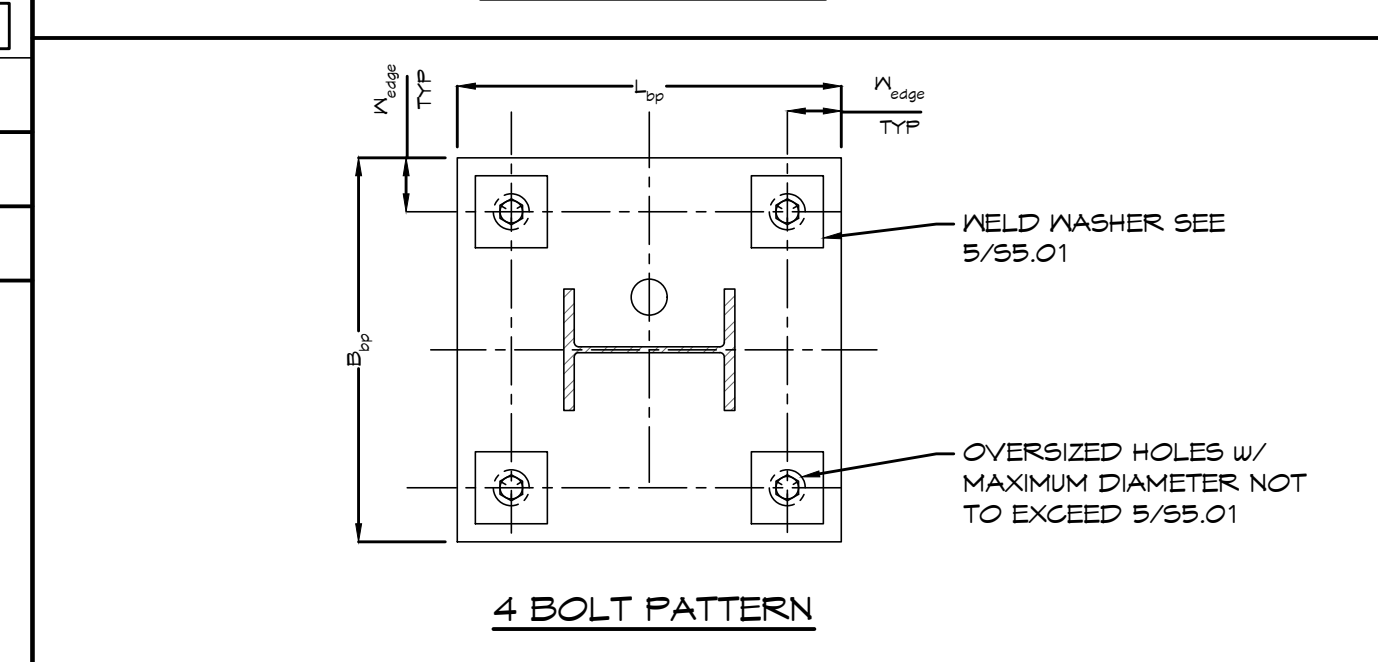
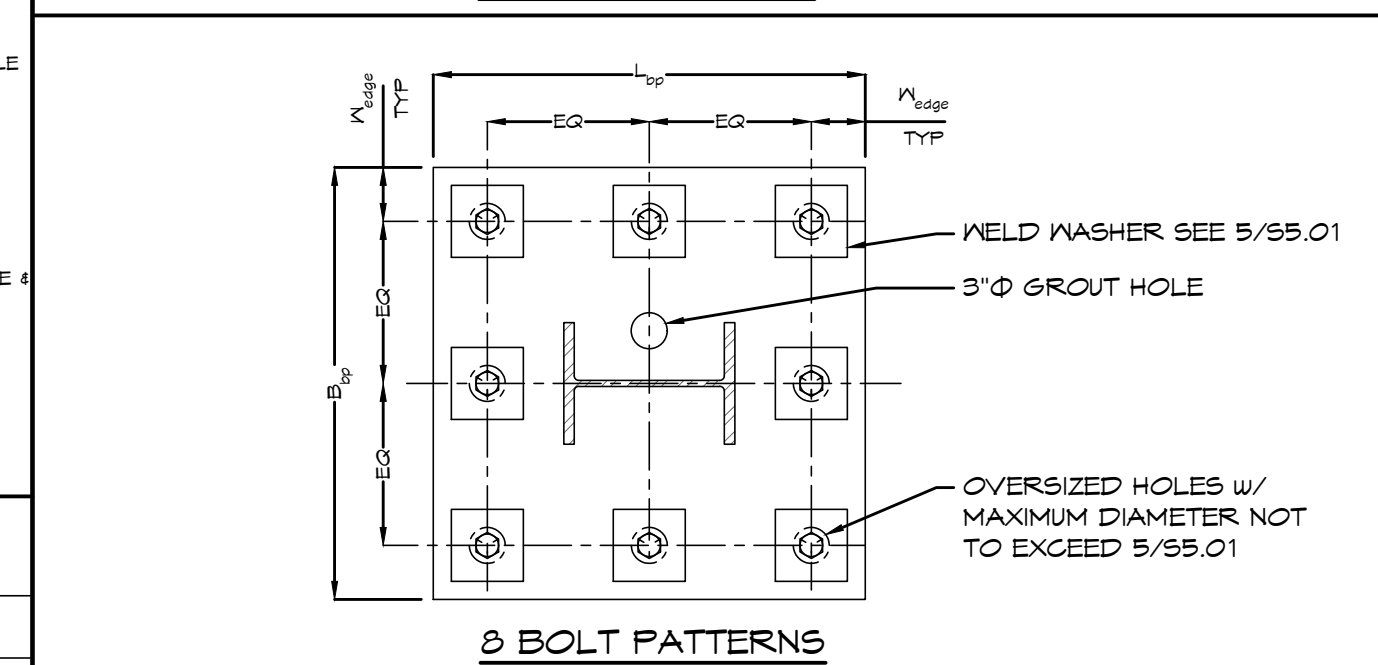
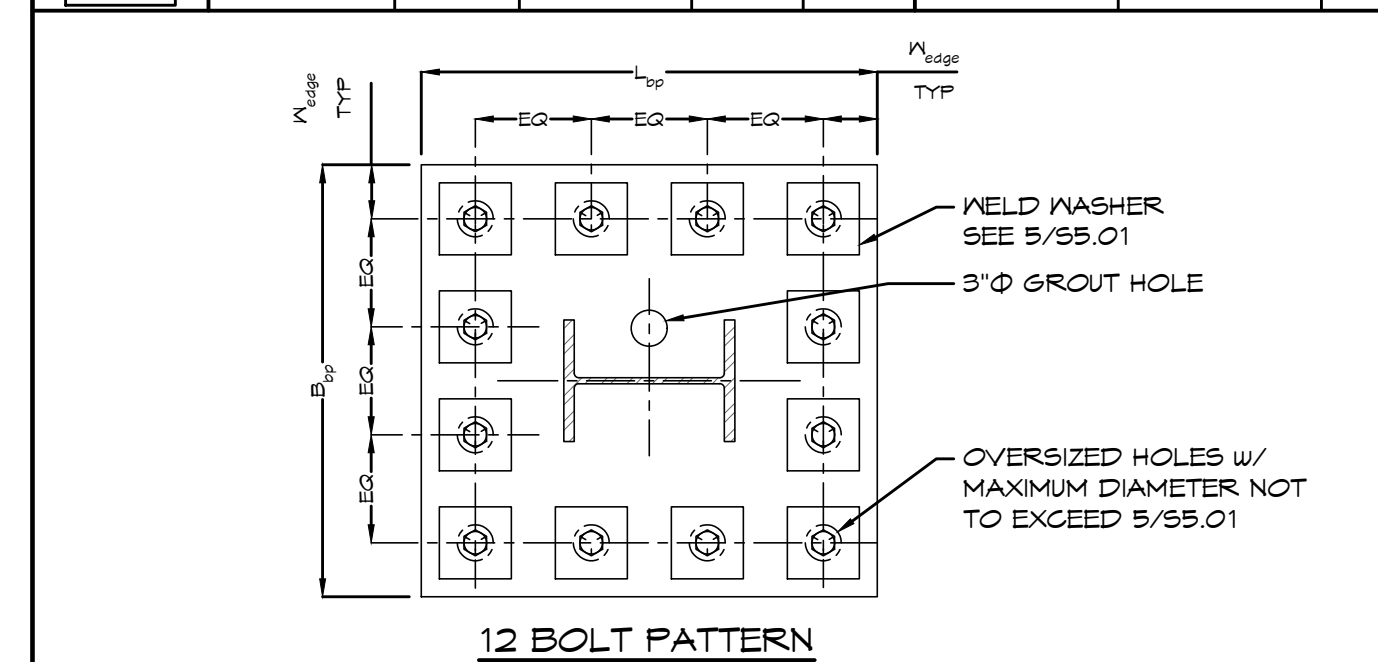
EXTENDED END-PLATE MOMENT CONNECTION SCHEDULE AT ROOF												
CONNECTION MARK		BOLTS		PLATE THICKNESS		PLATE DIMENSIONS			BOLT GAGE PITCH		WELD SIZE	
		QUANTITY	ASTM F1559 GRADE	DIAMETER	A	B	A	B	C	1	2	
END	4 TOP & 4 BTM	A490	7/8"	7/8"	3/4"	2"	4"	50%	7/8"	7/8"		
	4 TOP & 4 BTM	A490	7/8"	7/8"	3/4"	1 1/2"	4"	50%	7/8"	7/8"		

NOTES:

1. AT CONTRACTOR'S OPTION PROVIDE FINGER SHM E. $\frac{3}{8}$ " MAX THICKNESS BETWEEN BEAM AND COLUMN TO ACCOUNT FOR ROLLING TOLERANCES AND ERECTION.
2. ALL BOLTS TO BE FULLY TENSIONED.
3. PROVIDE A SYSTEMATIC SNUGGING OF THE JOINT/BOLTS FOLLOWED BY A MATCHING SYSTEMATIC TENSIONING OF THE BOLTS.
4. PROVIDE STANDARD AISC WELD ACCESS HOLES FOR FLANGE GROOVE WELDS.

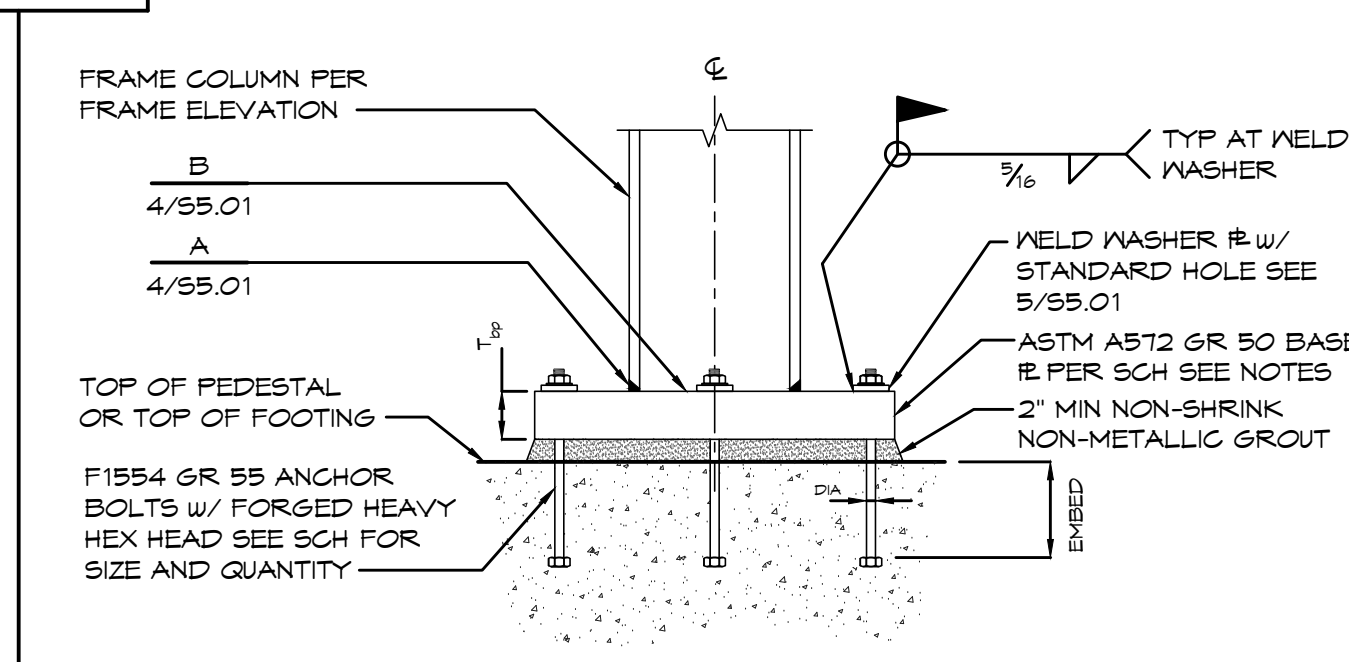
2	EXTENDED END-PLATE MOMENT CONNECTION AT ROOF	
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BASE PLATE MARK	COLUMN SIZE	λ_{edge}	BASE PLATE SIZE			F1554 GR 55 W/ FORGED HEAVY HEX HEAD ANCHORS		
			L_{top}	L_{up}	E_{up}	No	ϕ	EMBED
FBP1	W14x	3"	24"	27"	27"	12	1 1/4"	12"
FBP2	W14x	3"	2"	27"	27"	8	1 1/4"	12"
FBP3	W14x	3"	1 1/4"	27"	23"	4	1 1/4"	12"
FBP4	W12x	3"	1 1/2"	25"	21"	4	1 1/4"	12"



NOTES:
1. PROVIDE A512 GR 50 FOR BASEPLATE MATERIAL.
2. PROVIDE 2" THICK AND THICKER BASEPLATE MATERIAL WITH A MINIMUM CHARPY V-NOTCH TOUGHNESS OF 20 FT-LB AT 70°F MEASURED AT ANY LOCATION PERMITTED BY ASTM A679.
FREQUENCY P.

3	FRAME COLUMN BASE PLATE SCHEDULE
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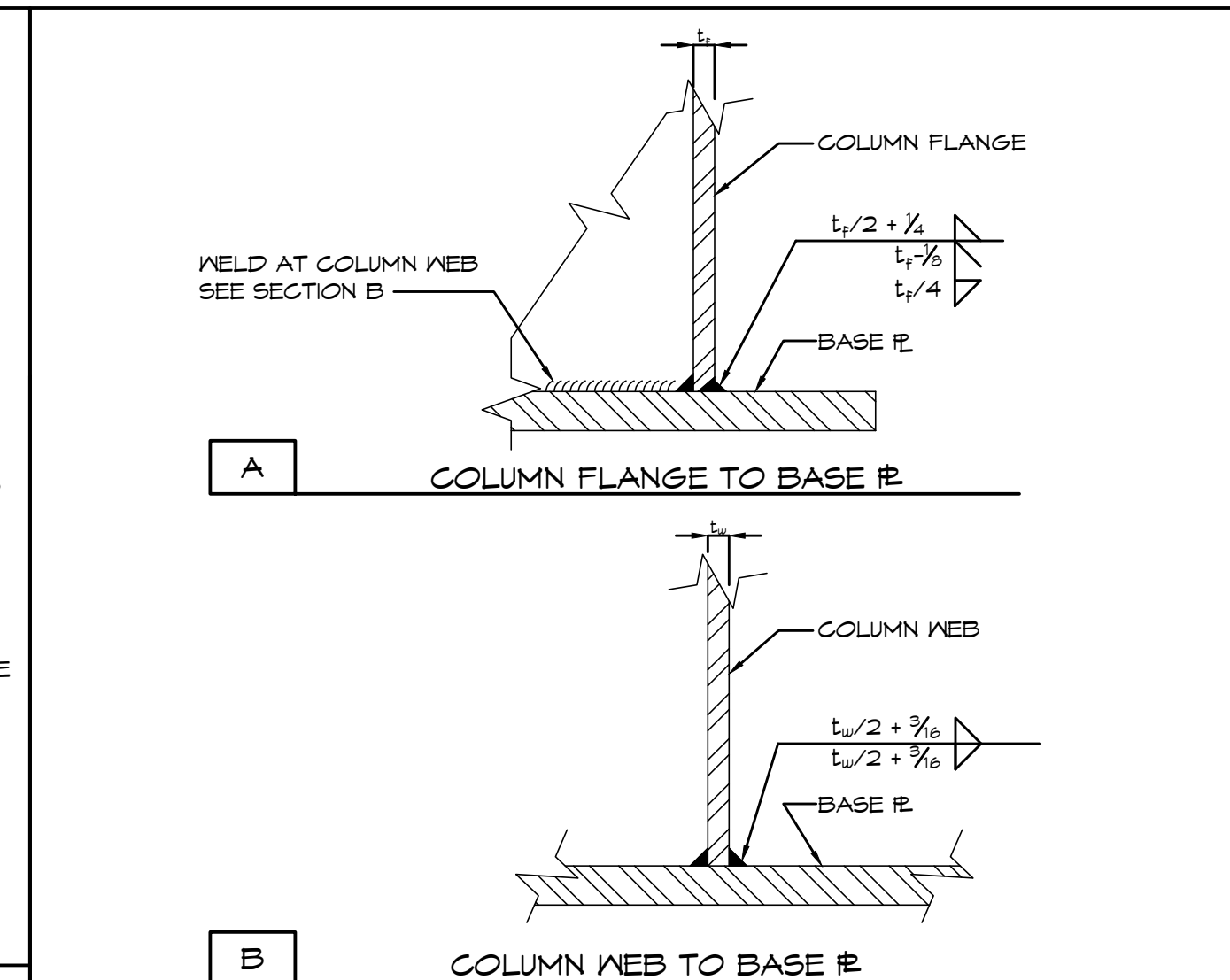


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TOUGHNESS OF 20 FT-LB AT 70°F MEASURED AT ANY LOCATION PERMITTED BY ASTM A673

USE PLATE SCHEDULE	
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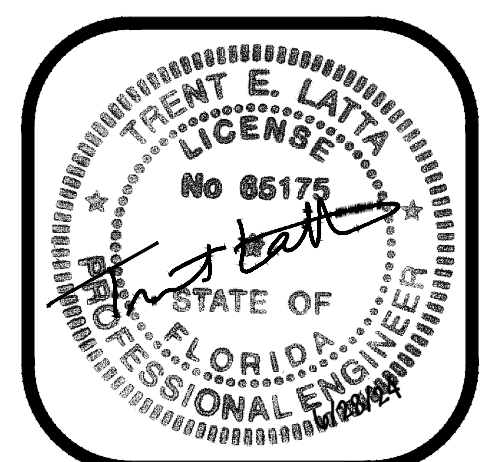


4	FRAME COLUMN TO BASE & WELDS	
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ANCHOR ROD DIAMETER	MAXIMUM BASE PLATE HOLE DIAMETER	WELD WASHER PLATE		REMARK
		MINIMUM DIAMETER OR SQUARE DIMENSION	MINIMUM THICKNESS	
3/4"	1 1/8"	2"	1/4"	
1"	1 3/8"	3"	3/8"	
1 1/4"	2 1/8"	3"	1/2"	
1 1/2"	2 3/8"	3 1/2"	1/2"	
1 3/4"	2 3/4"	4"	5/8"	

NOTES:
1. PROVIDE ASTM A36 PLATE MATERIAL FOR WELD WASHER.

5	OVERSIZED BASEPLATE HOLE WASHER SCHEDULE	
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PALM BEACH GARDENS, FLORIDA

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