

STRUCTURAL NOTES

GENERAL NOTES:

1. TYPICAL DETAILS AND GENERAL NOTES APPLY TO ALL PARTS OF THE WORK EXCEPT WHERE SPECIFICALLY DETAILED OR UNLESS OTHERWISE NOTED.
2. DRAWINGS ARE NOT TO BE SCALED.
3. FOR DIMENSIONS NOT SHOWN, COORDINATE WITH ARCHITECTURAL DRAWINGS.
4. THE CONTRACTOR SHALL CAREFULLY REVIEW THE DRAWINGS TO IDENTIFY THE SCOPE OF WORK REQUIRED, VISIT THE SITE TO RELATE THE SCOPE OF WORK TO EXISTING CONDITIONS, AND DETERMINE THE EXTENT OF WHICH THOSE CONDITIONS AND PHYSICAL SURROUNDINGS WILL IMPACT THE WORK.
5. THE CONTRACTOR SHALL ASSUME THE MOST STRINGENT REQUIREMENTS APPLY IN CASE OF CONFLICT AMONG SPECIFICATIONS, STANDARDS, CODES AND DRAWINGS. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER IMMEDIATELY TO RESOLVE THE CONFLICT.
6. ANY DEVIATION, MODIFICATION, OR SUBSTITUTION FROM THE BID SET OF STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER FOR REVIEW/APPROVAL, PRIOR TO ITS USE OR INCLUSION ON THE SHOP DRAWINGS. WITHOUT SUCH PRIOR APPROVAL, DEVIATIONS, MODIFICATIONS, OR SUBSTITUTIONS WILL BE REJECTED. COSTS FOR DEMOLITION AND REWORK OF SUCH ITEMS WILL BE BORNE BY THE CONTRACTOR.
7. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED FOR IN-SERVICE LOADS ONLY. THE MEANS, METHODS, PROCEDURES, AND SEQUENCES OF CONSTRUCTION ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR, WHICH INCLUDE THE DETERMINATION OF ALLOWABLE CONSTRUCTION LOADING OF THE STRUCTURE. THE CONTRACTOR SHALL PROVIDE, DESIGN, MONITOR, AND MAINTAIN ALL NECESSARY TEMPORARY AND PERMANENT SYSTEMS (SHORING, BRACING, GUYS, FALSEWORK, FORMWORK, SHEETING, ETC.) TO ENSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION. ANY SYSTEMS SHOWN ON THE DOCUMENTS ARE PARTIAL AND SCHEMATIC IN NATURE AND EXTENTS ARE NOT ALL INCLUSIVE. ALL WORK SHALL BE PERFORMED WITHOUT DAMAGE TO ADJACENT EXISTING WORK. SHORING SYSTEMS SHALL BE DESIGNED, SIGNED, AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE JURISDICTION WHERE THE PROJECT IS LOCATED.
8. THE CONTRACTOR IS RESPONSIBLE FOR SITE SAFETY. THE CONTRACTOR AND ALL SUBCONTRACTORS SHALL REVIEW THE STRUCTURAL CONTRACT DOCUMENTS AND SHALL NOTIFY THE STRUCTURAL ENGINEER OF ANY CONFLICTS BETWEEN THOSE DOCUMENTS AND ANY SAFETY REGULATIONS. SUCH REVIEW AND NOTIFICATION SHALL OCCUR PRIOR TO PRODUCTION OF SHOP DRAWINGS.
9. THE CONTRACTOR SHALL PROTECT ALL WORK, MATERIALS, AND EQUIPMENT FROM DAMAGE AND SHALL PROVIDE PROPER STORAGE FACILITIES FOR MATERIALS AND EQUIPMENT DURING CONSTRUCTION.
10. SITE VISITS PERFORMED BY THE ARCHITECT/ENGINEER DO NOT INCLUDE INSPECTIONS OF MEANS AND METHODS OF CONSTRUCTION PERFORMED BY THE CONTRACTOR.
11. STRUCTURAL OBSERVATIONS PERFORMED BY THE ARCHITECT/ENGINEER DURING CONSTRUCTION ARE NOT THE CONTINUOUS AND SPECIAL INSPECTION SERVICES AND DO NOT WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS REQUIRED OF THE BUILDING DEPARTMENT INSPECTOR OR THE TESTING AGENCY. ALSO, OBSERVATIONS DO NOT GUARANTEE THE CONTRACTORS PERFORMANCE AND SHALL NOT BE CONSIDERED AS SUPERVISION OF CONSTRUCTION.
12. ALL WALLS, FLOORS, AND ROOF MEMBERS SHALL BE SECURELY SHORED AND BRACED AT ALL TIMES DURING CONSTRUCTION.
13. NO PIPES OR DUCTS SHALL BE EMBEDDED INTO STRUCTURAL MEMBERS UNLESS SO SHOWN ON THE PLANS OR APPROVED BY THE ENGINEER.
14. NO STRUCTURAL ELEMENTS ARE TO BE CUT UNLESS SPECIFICALLY APPROVED BY THE ENGINEER.
- DESIGN CRITERIA:**
1. STRUCTURAL DRAWINGS HAVE BEEN PREPARED IN ACCORDANCE WITH THE 2023 FLORIDA BUILDING CODE.
2. LIVE LOADS:
- | | |
|------------------------|--------------------------------|
| ROOF LOAD = | 20 PSF (REDUCIBLE AS PER CODE) |
| FLOOR SLAB-ON-GRADE: | |
| RETAIL (FIRST FLOOR) = | 100 PSF |
| ELECTRICAL ROOM = | 150 PSF |
3. DEAD LOADS:
- | |
|--|
| ROOF LOAD = 20 PSF (SUPERIMPOSED + ACTUAL MAT'L WEIGHT) |
| FLOOR LOAD = 45 PSF (SUPERIMPOSED + ACTUAL MAT'L WEIGHT) |
4. SNOW LOADS:
- | | |
|---------------------------|-------|
| GROUND SNOW LOAD (Ps) | 3 PSF |
| SNOW EXPOSURE FACTOR (Ce) | 1.0 |
| THERMAL FACTOR (Ct) | 1.0 |
| SNOW IMPORTANCE FACTOR: | 1.0 |
| FLAT ROOF SNOW LOAD (Pi) | 3 PSF |
5. WIND LOADS:
- ULTIMATE WIND SPEED, Vult (3 sec. GUST) = 165 MPH
- WIND IMPORTANCE FACTOR (Iw) = 1.0
- RISK CATEGORY: II
- WIND EXPOSURE = C
- ENCLOSED BUILDING INTERNAL PRESSURE COEFFICIENT = +0.18 or -0.18
6. SEISMIC LOADS:
- | | |
|-----------------------------------|---|
| SEISMIC IMPORTANCE FACTOR: | 1.0 |
| RISK CATEGORY: | II |
| MAPPED ACCELERATIONS: | |
| Ss | 0.056g |
| S1 | 0.025g |
| SITE CLASS DEFINITION: | D (ASSUMED) |
| SPECTRAL RESPONSE COEF: | |
| Sds | 0.050 |
| Sd1 | 0.035 |
| SEISMIC DESIGN CATEGORY: | A |
| RESPONSE MODIFICATION FACTOR (R): | 2 |
| SEISMIC FORCE RESISTING SYSTEM: | ORDINARY REINFORCED MASONRY SHEAR WALLS |
- ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE

FOUNDATION NOTES:

- GEOTECHNICAL EXPLORATION REPORT: UNIVERSAL ENGINEERING SCIENCES. PREPARED FOR: DKC COCONUT CROSSING, LLC
PROJECT NUMBER: 0630 2100222 0000
DATE: MARCH 8, 2022, REVISED DATE: MARCH 21, 2022
1. FOUNDATIONS HAVE BEEN DESIGNED BASED ON AN ALLOWABLE SOIL BEARING CAPACITY OF 2500 PSF.
2. THE OWNER SHOULD PROVIDE FINAL STRUCTURAL AND CIVIL DRAWINGS TO UES (GEOTECH) PRIOR TO CONSTRUCTION TO CONFIRM GEOTECHNICAL RECOMMENDATIONS PER GEOTECHNICAL REPORT.
3. A SOILS TESTING LABORATORY SHALL BE RETAINED BY THE OWNER TO PROVIDE CONSTRUCTION REVIEW TO ENSURE CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS DURING THE EXCAVATIONS, BACKFILL, AND FOUNDATION PHASES OF THE PROJECT.
4. THE CONTRACTOR SHALL STUDY THE GEOTECHNICAL INVESTIGATION REPORT AND VISIT THE SITE PRIOR TO THE START OF ANY WORK. THE CONTRACTOR SHALL VERIFY ANY EXISTING FIELD CONDITION THAT MAY AFFECT THE INSTALLATION OF THE FOUNDATION SYSTEM.
5. BEARING ELEVATIONS ARE ESTIMATED FROM SOIL BORING DATA INDICATED IN THE GEOTECHNICAL SUBSURFACE INVESTIGATION REPORT. DETERMINATION OF FINAL BEARING ELEVATIONS, TOPSOIL AND EXCAVATION STRIPPING DEPTH, INSPECTION OF ALL SUBSOIL EXPOSED DURING STRIPPING, SITE GRADING, EXCAVATION OPERATIONS, APPROVAL OF FILL MATERIALS, DENSITY TESTING OF FILLS TO ENSURE PLACEMENT PER SPECIFICATION REQUIREMENTS, INSPECT FOUNDATION BEARING SURFACES, AND VERIFY ALLOWABLE BEARING PRESSURES ARE THE TESTING LABORATORY'S RESPONSIBILITY.
6. AFTER STRIPPING, SOILS SHOULD BE DENSIFIED BY EXCAVATING TWO FEET OF EXISTING SOILS WITHIN BUILDING FOOTPRINT AND EXTENDING A MINIMUM 5'-0" OUTSIDE BUILDING LINE. GROUNDWATER CONTROL WILL BE NECESSARY. REFER TO GEOTECH REPORT.
7. CONTRACTOR SHALL REFER TO SITE PREP RECOMMENDATION IN GEOTECH FOR ALL REQUIREMENTS IN SUBGRADE PREPARATION INCLUDING SLAB AND FOOTING SUBGRADE PREPARATION NOT SHOWN HEREIN.
8. NEW FOOTINGS PLACED ADJACENT TO EXISTING FOOTINGS SHALL BEAR AT THE SAME ELEVATION, UNLESS NOTED OTHERWISE.
9. STEP FOOTINGS AT A RATIO OF ONE (1) VERTICAL TO TWO (2) HORIZONTAL WITH A MAXIMUM VERTICAL STEP OF 2'-0" UNLESS NOTED OTHERWISE.
10. INUNDATION AND LONG TERM EXPOSURE OF BEARING SURFACES, WHICH WILL RESULT IN DETERIORATION OF BEARING FORMATIONS, SHALL BE PREVENTED. FOOTINGS SHALL BE PLACED IMMEDIATELY FOLLOWING FOOTING EXCAVATIONS AND BEARING SURFACE INSPECTION.
11. PRIOR TO PLACING CONCRETE, ALL DEBRIS, WATER, AND LOOSE EARTH SHALL BE REMOVED FROM THE FOUNDATION BED.
12. COLUMN FOOTINGS AND WALL FOOTINGS SHALL BE PLACED MONOLITHICALLY WITH ADJACENT FOOTINGS AT THE SAME ELEVATION.
13. WHERE A UTILITY LINE PASSES UNDER A FOOTING, PROVIDE A STEEL OR CAST-IRON SLEEVE WITH MINIMUM 2" CLEAR ON ALL SIDES OF PIPE.
14. THE CONTRACTOR SHALL EXERCISE GREAT CARE DURING EXCAVATION, UNDERGROUND UTILITY LOCATIONS, IF SHOWN, ARE APPROXIMATE. THE CONTRACTOR SHALL PREDETERMINE UTILITY LOCATIONS AND NOTIFY THE ENGINEER IMMEDIATELY IF DEVIATION FROM PLANS EXIST. THE CONTRACTOR IS RESPONSIBLE FOR THE SAFE SUPPORT OF UTILITIES ACROSS EXCAVATIONS.
15. SHEETING, SHORING, AND DEWATERING IS THE RESPONSIBILITY OF THE CONTRACTOR.
16. BOTTOM OF ALL EXTERIOR FOOTINGS/GRADE BEAMS SHALL BEAR A MINIMUM OF 1'-6" BELOW ADJACENT FINAL GRADE FOR FROST PROTECTION.

CONCRETE NOTES:

1. MINIMUM CONCRETE COMPRESSIVE STRENGTH, (fc), AT 28 DAYS:
- A. BUILDING FOUNDATIONS: 4000 PSI
- B. FOR SITE PAVING REQUIREMENTS REFER TO CIVIL.
- C. BUILDING SLAB, SIDEWALKS AND EQUIPMENT PAD: 4000 PSI
2. CONCRETE PROPORTIONS SHALL CONSIST OF THE FOLLOWING MATERIALS TO ACHIEVE A MIX THAT WILL YIELD A FINISHED PRODUCT THAT WILL PRODUCE THE REQUIRED COMPRESSIVE STRENGTHS LISTED IN NOTE #1 ABOVE.
- A. CEMENTITIOUS MATERIALS:
- a. PORTLAND CEMENT, ASTM C150, TYPE 1.
- b. FLYASH OR SLAG CEMENT ARE NOT ALLOWED.
- B. AGGREGATES: NORMAL WEIGHT, ASTM C33, LIGHT WEIGHT, ASTM C330.
- C. PROVIDE ALL AGGREGATE FROM A SINGLE SOURCE.
- D. WATER: ASTM C94, POTABLE.
- E. AIR ENTRAINMENT ADMIXTURE: ASTM C260.
- F. WATER REDUCING ADMIXTURE: ASTM C494, TYPE F OR G.
- G. CALCIUM CHLORIDE: ANY ADMIXTURE CONTAINING MORE THAN 0.1% CHLORIDE IONS, CONTENT BY WEIGHT, ARE NOT PERMITTED.
- CONCRETE MIX DESIGNS MUST BE SUBMITTED TO THE E.O.R. FOR HIS APPROVAL BEFORE CONSTRUCTION BEGINS. MIX DESIGNS MUST BE SUBMITTED WITH COMPRESSIVE STRENGTH INFORMATION, FOR THAT SPECIFIC MIX, AS PER THE REQUIREMENTS OF CHAPTER 5 OF ACI-318-14. SUBMISSIONS WITHOUT THIS INFORMATION WILL BE STAMPED "REJECTED" & RETURNED TO THE CONTRACTOR WITHOUT E.O.R. REVIEW. TEST DATA SHALL BE NO OLDER THAN 12 MONTHS. ALL CONCRETE COMPRESSIVE STRENGTH TESTS SHALL BE AVAILABLE ON THE JOB SITE FOR REVIEW BY THE INSPECTOR.

3. STRUCTURAL MEMBERS OF REINFORCED CONCRETE SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE CODE REFERENCED ACI 318-14.
4. SLAB ON GRADE CONTROL OR CONSTRUCTION JOINTS SHALL BE LOCATED AT COLUMN LINES AND INTERMEDIATELY SO THAT NO JOINT SPACING SHALL EXCEED 12'-0". NOR SHALL THE LENGTH OF ANY PANEL EXCEED ONE AND A HALF, (1 1/2), TIMES THE WIDTH OF THAT PANEL. REFER TO TYPICAL DETAILS FOR SLAB JOINT CONSTRUCTION.
5. FLOOR SLAB CONSTRUCTION JOINTS MUST BE DOWELED.
6. EXTERIOR SLABS SHALL DRAIN FREELY AWAY FROM THE BUILDING. COORDINATE ELEVATIONS WITH CIVIL ENGINEER AND ARCHITECT.
7. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SLAB FINISHES, SLOPES AND DEPRESSIONS OF INTERIOR SLABS.

REINFORCING STEEL NOTES:

1. SHALL BE DETAILED, FABRICATED AND PLACED ACCORDING TO THE STANDARDS OF THE AMERICAN CONCRETE INSTITUTE (ACI) AND THE CONCRETE REINFORCING STEEL INSTITUTE (CRSI).
2. MATERIALS:
- REINFORCING BARS SHALL COMPLY WITH ASTM A615 GRADE 60, WELDED WIRE FABRIC SHALL COMPLY WITH ASTM A62 AND A1064, FLAT SHEETS ONLY.
- REINFORCING BARS FOR WELDING SHALL COMPLY WITH ASTM A706.
3. CLEAR MINIMUM COVER OF CONCRETE OVER REINFORCING BARS SHALL BE AS FOLLOWS:
- | | |
|----------------------------------|------|
| CONCRETE PLACED AGAINST EARTH | 3" |
| FORMED SURFACES EXPOSED TO EARTH | 3" |
| TOP OF SLABS | 3/4" |
4. ALL BARS DENOTED CONTINUOUS ON PLANS, SECTIONS AND DETAILS SHALL HAVE CLASS "B" TENSION SPICE LAPS AND CORNER BARS AND HOOKS AT DISCONTINUOUS ENDS. SPICED BARS SHALL BE SECURELY WIRED TOGETHER. SPICES OF ADJACENT REINFORCING BARS SHALL BE STAGGERED 24" WHEREVER POSSIBLE.
5. WELDED WIRE FABRIC, WHERE REQUIRED, SHALL BE PLACED IN THE CENTER OF THE SLAB UNLESS NOTED OTHERWISE. LAP JOINTS ONE WIRE SPACING PLUS 2" OR A MINIMUM OF 6", EXTEND FABRIC TO WITHIN 1" OF EDGES OF SLABS ON GRADE.
6. PROVIDE PRECAST DOBIES, WIRE OR PLASTIC CHAIRS, SUPPORT BARS, ETC. TO MAINTAIN SPECIFIED CLEARANCES FOR THE ENTIRE LENGTH OF ALL REINFORCING BARS AND WELDED WIRE FABRIC. DO NOT USE REBAR STAKES TO SUPPORT REINFORCING. ALL REINFORCEMENT SHALL BE FASTENED AND SECURED TO PREVENT DISPLACEMENT BY THE PLACING OF CONCRETE.
7. ALL CONCRETE WALLS TO BE DETAILED IN ELEVATION ON SHOP DRAWINGS. NO MORE THAN 50% OF HORIZONTAL WALL REINFORCING SHALL LAP IN A SINGLE VERTICAL PLANE.
8. PROVIDE (2) #4 REINFORCING BARS, 3'-0" LONG, IN CONCRETE SLABS AT ALL RE-ENTRANT CORNERS. PLACE BARS AT MID DEPTH OF SLAB WITH 2" MINIMUM HORIZONTAL CLEARANCE FROM SLAB CORNER.
9. REBAR FOR WELDED CONNECTIONS MUST MEET ASTM A706. STANDARD ASTM A615 GRADE 60 REBARS ARE NOT ACCEPTABLE FOR WELDING. WELDING PROCEDURE SHALL CONFORM TO ANSII/AWS D1.4 "STRUCTURAL WELDING CODE - REINFORCING STEEL". DO NOT TACK WELD.
10. REINFORCING DOWELS, WHERE REQUIRED, SHALL MATCH THE SIZE, SPACING AND QUANTITY OF MAIN REINFORCING, UNLESS NOTED OTHERWISE.

STRUCTURAL STEEL NOTES:

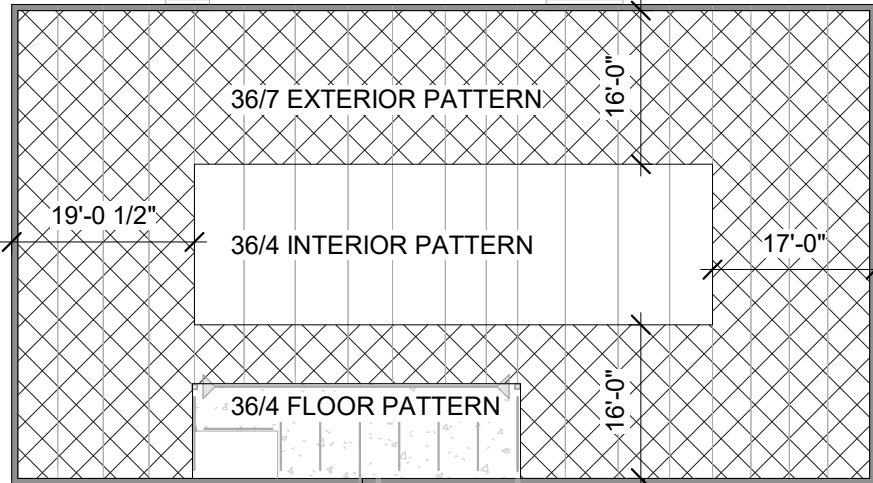
1. STRUCTURAL STEEL MATERIALS:
- WIDE FLANGE BEAMS: ASTM A992, Fy = 50 KSI
- STRUCTURAL TUBING: ASTM A500 Fy=46 KSI OR A1085, Fy = 50 KSI
- ASTM A36, Fy = 36 KSI
- OTHER SHAPES & PLATES: ASTM A53, GRADE B, Fy = 35 KSI
- STRUCTURAL PIPE: ASTM A325, UNLESS NOTED OTHERWISE
- CONNECTION BOLTS: ASTM F1554, A307 OR A36, Fy = 36 KSI.
- ANCHOR BOLTS: "PROVIDE 3 NUTS AND 2 WASHERS WITH EACH ANCHOR BOLT."
- E70-XX, UNLESS NOTED OTHERWISE.
- WELDING ELECTRODES:
2. STRUCTURAL STEEL DETAILING, FABRICATION, AND ERECTION SHALL BE IN ACCORDANCE WITH THE "2016 AISC SPECIFICATIONS"
3. SHOP OR FIELD SPICES BETWEEN SUPPORTS THAT ARE NOT REQUIRED BY DESIGN WILL NOT BE ALLOWED. ANY MEMBERS CONTAINING SUCH SPICES SHALL BE REMOVED AND REPLACED WITH UNSPLICED MEMBERS AT THE FABRICATOR'S EXPENSE.
4. PROVIDE BOLTS AND PUNCHED HOLES IN STRUCTURAL AND MISCELLANEOUS STEEL FOR ATTACHMENTS OF WOOD NAILERS AS REQUIRED ON THE ARCHITECTURAL, MECHANICAL AND STRUCTURAL DRAWINGS.
5. MINIMUM SIZE OF WELD IS 3/16" UNLESS NOTED OTHERWISE. ALL WELDING SHALL CONFORM TO THE PROVISIONS OF AWS D1.1, STRUCTURAL WELDING CODE BY THE AMERICAN WELDING SOCIETY. ALL WORK SHALL BE PERFORMED BY CERTIFIED WELDERS EXPERIENCED IN THE TYPE OF CONSTRUCTION INVOLVED. PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE.
6. SHOP DRAWINGS SHALL SHOW COMPLETE WELDING INFORMATION, BOTH SHOP AND FIELD, USING AMERICAN WELDING SOCIETY SYMBOLS UNLESS OTHERWISE INDICATED.
7. PROVIDE 1 1/2" NON-METALLIC, SHRINKAGE RESISTANT GROUT CONFORMING TO ASTM C1107 UNDER ALL COLUMN BASEPLATES.
8. ALL STEEL SHALL HAVE A PRIME COAT OF PAINT EXCEPT AREAS TO BE FIELD WELDED.
9. PRIME AND PAINT ALL FIELD WELDS AFTER INSPECTION.
10. STEEL FABRICATOR TO BE AN AISC CERTIFIED FABRICATOR.
11. ALL PLAN DIMENSIONS ARE TO CENTERLINE OF STEEL MEMBERS EXCEPT FOR STEEL CHANNELS. CHANNEL DIMENSIONS ARE TO THE BACK FACE OF THE WEB.
12. REFER TO ARCHITECTURAL DRAWINGS FOR MISCELLANEOUS STEEL, BRACKETS, BRACES, SUPPORTS, ETC. NOT INDICATED ON THE STRUCTURAL DRAWINGS.

STEEL JOISTS:

1. BAR JOISTS SHALL BE DESIGNED AND FABRICATED ACCORDING TO THE REQUIREMENTS OF THE STEEL JOIST INSTITUTE (SJI) UNLESS OTHERWISE NOTED. JOIST FABRICATOR SHALL BE A MEMBER OF THE STEEL JOIST INSTITUTE.
2. BAR JOISTS SHALL BE WELDED TO SUPPORTING BEAMS, OR WELD PLATES WITH 2" OF 3/16" WELD ON EACH SIDE OF BAR JOIST, UNLESS NOTED OTHERWISE.
3. PROVIDE JOIST BRIDGING TO MEET THE REQUIREMENTS OF SJI. PROVIDE UPLIFT BRIDGING FOR THE NET UPLIFT SHOWN IN THE DESIGN LOADING NOTES.
4. BRIDGING TERMINATING AT WALLS OR STEEL BEAMS SHALL BE ANCHORED TO WALL OR BEAM. BRIDGING SHALL BE ANCHORED AT ENDS PRIOR TO APPLYING ROOF OR FLOOR LOADS.
5. BAR JOISTS AT COLUMN LOCATIONS TO BE BOLTED TO SUPPORTING BEAM AT TIME OF ERECTION.
6. ALL JOIST SHALL BE DESIGNED TO RESIST THE NET UPLIFT BASED ON THE WIND PRESSURES SHOWN IN THE C&C WIND PRESSURE TABLE. REFER TO DESIGN CRITERIA FOR DESIGN DEAD LOAD TO BE USED.
7. MINIMUM BEARING REQUIREMENTS:
- BAR JOISTS:
- 2 1/2" ON STRUCTURAL STEEL
- 4" ON STEEL BEARING PLATES ON MASONRY OR CONCRETE
8. NO LIGHT GAUGE FRAMING, MECHANICAL, ELECTRICAL, OR OTHER EQUIPMENT SHALL BE SUSPENDED FROM OR ATTACHED TO ANY INTERIOR BRIDGING.
9. GENERAL CONTRACTOR SHALL VERIFY ALL STRUCTURAL STEEL JOIST LOCATIONS, ROOF EQUIPMENT WEIGHTS, LOCATIONS, AND OPENING SIZES.

METAL DECK:

1. AT ROOF FRAMING: 1 1/2" DEEP, TYPE "B" (WIDE RIB), GRADE 80, 20 GAUGE, GALVANIZED SHEET WIDTH = 36". ROOF DECK WELD PATTERN TO SUPPORTING MEMBERS SHALL BE 36" PATTERN OF 5/8" DIA. PUDDLE WELDS AT THE EXTERIOR REGION AND A 364 PATTERN OF 5/8" DIA. PUDDLE WELDS AT THE INTERIOR REGION. DECK SIDELAP FASTENERS SHALL BE (5) #12 TEK SCREWS PER SPAN. ATTACH DECK TO SUPPORTS WITH 5/8" DIA. PUDDLE WELDS AT 6" O.C. FASTEN DECK AT OPENINGS FRAMES WITH #12 TEK SCREWS AT 6" O.C. REFER TO TYPICAL DECK ATTACHMENT DETAIL.
2. AT FLOOR RAIMING: 1" DEEP, TYPE "C", GRADE 80, 20 GAUGE, GALVANIZED SHEET WIDTH = 36". FLOOR DECK WELD PATTERN TO SUPPORTING MEMBERS SHALL BE 364 PATTERN OF 5/8" DIA. DECKS SIDELAP FASTENERS SHALL BE (3) #12 TEK SCREWS PER SPAN. ATTACH DECK TO PERIMETER SUPPORTS WITH 5/8" DIA. PUDDLE WELDS AT 6" O.C. FASTEN DECK AT OPENINGS FRAMES WITH #12 TEK SCREWS AT 6" O.C. REFER TO TYPICAL DECK ATTACHMENT DETAIL.
3. PREPARE AND REPAIR DAMAGED GALVANIZED COATING ON BOTH SURFACES OF DECK WITH GALVANIZING REPAIR PAINT ACCORDING TO ASTM 780 AND MANUFACTURER'S INSTRUCTIONS.
4. METAL DECK MANUFACTURER SHALL BE A MEMBER OF THE STEEL DECK INSTITUTE (SDI). ALL PRODUCTS SHALL BE FACTORY MUTUAL APPROVED.
5. STEEL DECK SHALL BE CONTINUOUS OVER A MINIMUM OF 3 SUPPORTS.
6. ALL METAL DECK WELDING IS ONLY TO BE PERFORMED BY AN AWS D1.3 CERTIFIED WELDER. CERTIFICATIONS ARE TO BE AVAILABLE ON SITE FOR VERIFICATION.
7. DECK ATTACHMENT PATTERN SHALL BE AS INDICATED ON THE DRAWINGS.
8. PROVIDE 1 1/2" MINIMUM DECK BEARING AT ALL SUPPORTS. DECK SHALL BE PLACED AT THE PERIMETER WITH A COMPLETE RIB BEARING ON THE STEEL SUPPORT.
9. NO LIGHT GAUGE FRAMING, MECHANICAL, ELECTRICAL, OR OTHER EQUIPMENT SHALL BE SUSPENDED FROM OR ATTACHED TO ANY METAL ROOF DECK.
10. OPENINGS SMALLER THAN 12" SQUARE ARE TO BE CUT BY INDIVIDUAL TRADE AND FLASHED BY ROOFING/SIDING CONTRACTOR. ALL LARGE OPENINGS TO BE CUT AND FLASHED BY ROOFING/SIDING CONTRACTOR WITH OPEN EDGES SUPPORTED BY STRUCTURAL STEEL.



LIGHT GAUGE STRUCTURAL FRAMING:

1. METAL STUD MANUFACTURER SHALL BE A MEMBER OF THE STEEL STUD MANUFACTURER'S ASSOCIATION (SSMA)
2. METAL STUDS AND ACCESSORIES SHALL HAVE A G60 GALVANIZED COATING UNLESS NOTED OTHERWISE.
- PRIOR TO FABRICATION, THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS INDICATING DESIGN DATA AND SHALL INCLUDE COMPLETE PLANS AND DETAILS.
4. METAL STUD STRENGTH CRITERIA:
- A. GAUGES 20 THROUGH 18 - 33 KSI MIN. YIELD STRESS
- B. GAUGES 16 THROUGH 12 - 30 KSI MIN. YIELD STRESS
- C. TRACK RUNNER - 33 KSI MIN. YIELD STRESS
5. METAL STUD MEMBER DESIGNATION: 600 S 182-33
- WELD DEPTH: 600 = 6"
- MEMBER TYPE: S = STUD
- FLANGE WIDTH: 182 = 1.825" (1 5/8")
- MINIMUM THICKNESS IN MILS
6. FASTENERS:
- A. SCREW CONNECTIONS
- FASTENING STUD TO STUD
- USE #10-16 TEKSD 3"x8" LONG BY ITW BULDEX UNLESS OTHERWISE NOTED.
- MIN. EDGE DISTANCE = 3/4"
- MIN. SPACING BETWEEN FASTENERS = 3/4"
- B. FASTENING STUD TO MASONRY:
- USE 1/4" DIA. TAPCON SCREWS BY ITW BULDEX.
- MIN. EMBEDMENT = 1" MIN. EDGE DISTANCE = 2"
- MIN. SPACING BETWEEN FASTENERS = 3"
- C. POWDER ACTUATED FASTENERS (P.A.F.):
- FASTENING TO CONCRETE OR MASONRY:
- USE 0.145" DIA. DOME HEAD NAIL BY HILTI, UNLESS NOTED OTHERWISE.
- MIN. EMBEDMENT = 1 1/4" MIN. EDGE DISTANCE = 2"
- MIN. SPACING BETWEEN FASTENERS = 3"
- FASTENING TO STEEL:
- USE 0.145" DIA. DOME HEAD KNURLED SHANK FASTENER BY HILTI, UNLESS NOTED OTHERWISE.
- MIN. EMBEDMENT = FULL PENETRATION. MIN. EDGE DISTANCE = 1/2"
- MIN. SPACING BETWEEN FASTENERS = 1"
7. FASTENING:
- A. MINIMUM FASTENING REQUIREMENTS, UNLESS NOTED OTHERWISE.
- TRACK TO CONCRETE
- (2) FASTENERS AT 16" O.C. TYP. WADDL FASTENER 3" FROM END OF TRACK AND (2) FASTENERS MINIMUM AT JAMBS.
- TRACK TO STRUCTURAL STEEL:
- 12" O.C. TYP. WADDL FASTENER 3" FROM END OF TRACK AND (2) FASTENERS MINIMUM AT JAMBS.
- STUD TO TRACK:
- (1) SCREW MINIMUM AT EACH FLANGE
- STUD TO STUD:
- (3) SCREWS MINIMUM
- STUD TO STEEL OR CONCRETE:
- USE POWDER ACTUATED FASTENERS REFERENCED ABOVE. SEE DETAILS FOR CONNECTIONS, MINIMUM 2 FASTENERS.
8. WALL OPENING FRAMING: ALL STUDS CUT TO CREATE WALL OPENING MUST BE REPLACED WITH EQUAL FULL HEIGHT KING STUDS ON EACH SIDE OF THE OPENING.
9. CURTAIN WALL AND NON LOAD BEARING PARTITIONS:
- MECHANICAL BRACING OF STUDS IS NOT NECESSARY WHERE WALL SHEATHING IS ATTACHED ON BOTH SIDES OF THE STUDS.
- WHEN ONLY ONE FACE OF THE STUDS RECEIVE SHEATHING, PROVIDE BRACING BY ONE OF THE FOLLOWING METHODS:
- A. COLD ROLLED CHANNEL, RUN HORIZONTALLY THROUGH STUD PUNCH OUTS AND ATTACHED AT EACH STUD.
- B. 2" WIDE, 18 GAUGE, STEEL STRAPS RUN HORIZONTALLY, ON BOTH SIDES OF THE STUDS, AND ATTACHED AT EACH STUD.
- VERTICAL SPACING OF THE BRACING IS LIMITED TO A MAXIMUM OF 4'-0" THROUGHOUT THE HEIGHT OF THE WALL.
10. PROVIDE ALL ACCESSORIES AS REQUIRED BY THE METAL STUD MANUFACTURER.
11. JOISTS SHALL BE FABRICATED TO PROVIDE 12" OF UNPUNCHED WEB AT BEARING ENDS.
12. PROVIDE JOIST WEB STIFFENERS PER METAL STUD MANUFACTURERS' RECOMMENDATIONS AT BEARING POINTS.
13. PROVIDE END BLOCKING WHERE JOISTS ARE NOT RESTRAINED AGAINST ROTATION.
14. NO LIGHT GAUGE MEMBER, THAT HAS BEEN TRIMMED OR CUT, SHALL BE INSTALLED SO THAT AN EDGE OF A WEB PUNCH OUT OCCURS WITHIN 10" MINIMUM OF THE TRIMMED OR CUT MEMBER END.
15. ALL MEMBER CUTTING MUST BE PERFORMED USING A SAW OR SHEAR. NO TORCH CUTTING IS ALLOWED AT ANY TIME. ALSO CUTTING OF ADDITIONAL HOLES, OTHER THAN THOSE PROVIDED BY THE MANUFACTURER, IN THE MEMBER WEB IS NOT PERMITTED AT ANY TIME.
16. SPLICING OF AXIALLY LOADED MEMBERS IS NOT ALLOWED AT ANY TIME.
17. BUILD UP LIGHT GAUGE HEADERS, CONSTRUCTED FOR EXTERIOR WALL CONDITIONS, SHALL HAVE INSULATION PLACED WITHIN THEM PRIOR TO THEIR INSTALLATION IN WALL SYSTEM.

DELEGATED DESIGN:

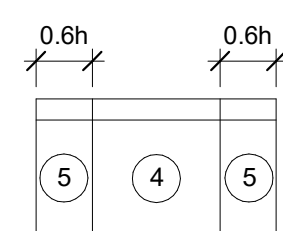
1. CONTRACTOR IS RESPONSIBLE FOR DESIGN OF THE FOLLOWING ITEMS INCLUDING DESIGN OF THE CONNECTIONS OF EACH ITEM TO THE SUPPORTING STRUCTURAL FRAMING:
- OPEN-WEB STEEL BAR JOIST
- CANOPIES
- PREFABRICATED ACCESS LADDER & FALL PROTECTION
- SHORING
2. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR EACH ITEM LISTED ABOVE. REFER TO THE "SHOP DRAWING" SECTION UNDER THE GENERAL NOTES FOR ADDITIONAL INFORMATION.
3. INFORMATION SHOWN IN THE CONTRACT DOCUMENTS (E.G., DEPTHS, GAUGES, SPACING, PLYS, ETC.) ARE CONSIDERED MINIMUMS AND ARE SCHEMATIC IN NATURE. INCREASED GAUGEPLYS AND/OR DECREASED SPACINGS MAY BE REQUIRED AND SHALL BE COMPLETED AT NO CHARGE TO THE OWNER.
4. WHERE A DELEGATED DESIGN IS INDICATED ON THE DRAWINGS, THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR EACH ITEM, COMPONENT, AND CONNECTION NOT SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS. SHOP DRAWINGS AND DESIGN CALCULATIONS SHALL BE SIGNED AND SEALED BY THE CONTRACTOR'S LICENSED ENGINEER (IN THE PROJECT'S JURISDICTION). DRAWINGS AND CALCULATIONS SHALL SHOW LOCATIONS AND MAGNITUDES OF LOADS IMPOSED ON THE STRUCTURAL FRAMING. THE ENGINEER OF RECORD RESERVES THE RIGHT TO MODIFY LOAD PATH SUGGESTED BY THE DELEGATED DESIGN ENGINEER.

SHOP DRAWING NOTES:

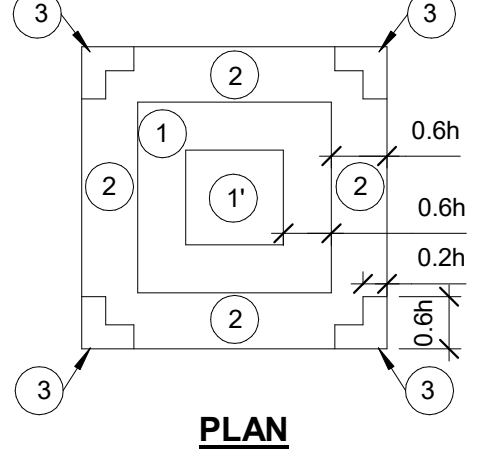
1. CONTRACTOR SHALL FURNISH COMPLETE AND DETAILED SHOP DRAWINGS FOR THE FOLLOWING STRUCTURAL MATERIALS:
- ANCHOR BOLT LAYOUT
- REINFORCING STEEL
- CONCRETE MIX DESIGN
- MASONRY UNITS, REINFORCING, MORTAR AND GROUT
- LIGHT GAUGE METAL FRAMING
- STRUCTURAL STEEL
- OPEN-WEB STEEL BAR JOIST
- METAL DECK
2. UNLESS NOTED, SUBMIT SHOP DRAWINGS FOR ALL FABRICATED MATERIALS. ALL SHOP DRAWINGS MUST BEAR THE REVIEW STAMP OF THE GENERAL CONTRACTOR. UNSTAMPED SHOP DRAWINGS WILL BE REJECTED WITHOUT REVIEW.
3. ELECTRONIC COPIES OF SHOP DRAWINGS SHALL BE SUBMITTED THROUGH PROJECTMATES. ELECTRONIC COPIES OF SHOP DRAWINGS WILL ONLY BE ACCEPTED WHEN REVIEWED SHOP DRAWINGS NOTED AS "NO EXCEPTIONS NOTED" OR "EXCEPTIONS NOTED" BY THE ENGINEER OF RECORD. REPRODUCTION OF ELECTRONIC COPIES WILL BE BELLED TO GENERAL CONTRACTOR FOR REIMBURSEMENT OF PRINTING COSTS. DESIGN DRAWINGS SHALL NOT BE REPRODUCED FOR USE AS SHOP DRAWINGS. TWO SETS OF REVIEWED SHOP DRAWINGS WILL BE RETURNED TO THE GENERAL CONTRACTOR.
4. ALLOW MINIMUM 10 WORKING DAYS FOR ARCHITECT/ENGINEER REVIEW OF EACH SHOP DRAWING SUBMITTAL. ALLOW AN ADDITIONAL WORKING DAY FOR EVERY 5 DRAWINGS OVER 30.
5. SHOP DRAWINGS FOR CONSTRUCTION OF EACH BUILDING COMPONENT NOT DESIGNED BY THE DESIGN ENGINEER OF RECORD AND NOT SPECIFIED IN THE CONSTRUCTION DOCUMENTS SHALL BE SIGNED & SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT. COMPONENTS INCLUDE BUT ARE NOT LIMITED TO THE FOLLOWING: PRE-ENGINEERED TRUSSES, RAILINGS, LADDERS, LIGHT GAUGE STUD FRAMING, RACK SYSTEMS, GLAZING SYSTEMS, AWNINGS AND ANY OTHER ITEM THAT IS DESIGNATED AS "DESIGNED BY OTHER" OR "PRE-ENGINEERED". THESE SHOP DRAWINGS SHALL BEAR CLEAR INDICATION THAT THEY HAVE BEEN REVIEWED AND NOTED AS "NO EXCEPTIONS NOTED" OR "EXCEPTIONS NOTED" BY THE PROJECT ARCHITECT OR ENGINEER OF RECORD.
6. SHOP DRAWINGS SUBMITTED TO THE ENGINEER OF RECORD SHALL ADHERE TO THE FOLLOWING GUIDELINES OR THEY WILL BE REJECTED AND RETURNED TO SENDER UNCHECKED:
- THE LATEST ISSUE DATE, IN THE DRAWING REVISION BOX, OF ANY SHEET IN THE STRUCTURAL DRAWING SET, SHALL BE CLEARLY SHOWN ON THE TOP SHEET OF THE SHOP DRAWINGS.
- NO PARTIAL SETS WILL BE ACCEPTED, UNLESS PREVIOUSLY APPROVED BY THE ENGINEER OF RECORD.
7. DETAILER SHALL COORDINATE WITH ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ALL ATTACHMENTS, CLIPS, OPENINGS OR DUCTWORK AFFECTING STRUCTURAL MEMBERS. ALL ITEMS SHALL BE SHOWN ON SHOP DRAWINGS.
8. DIMENSIONAL COORDINATION SHALL BE PERFORMED BY THE GENERAL CONTRACTOR AND/OR HIS FABRICATOR.
9. PROPOSED CHANGES TO THE CONSTRUCTION DOCUMENTS SHALL BE CLEARLY MARKED IN THE SHOP DRAWINGS AND SHALL INCLUDE SIGNED & SEALED DRAWINGS AND CALCULATIONS BY AN ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED. THE ENGINEER OF RECORD WILL REVIEW THE PROPOSED CHANGE FOR ACCEPTANCE.
10. THE CONTRACTOR SHALL HAVE PROOF OF WELDER CERTIFICATION AT THE JOBSITE AT ALL TIMES.
11. REVIEWED SHOP DRAWINGS NOTED AS "NO EXCEPTIONS NOTED" OR "EXCEPTIONS NOTED" SHOP DRAWINGS SHALL BE AVAILABLE AT THE JOBSITE AT ALL TIMES.
- MASONRY:**
1. ALL BRICK MASONRY SHALL COMPLY WITH THE RECOMMENDATIONS OF BRICK INSTITUTE OF AMERICA (BIA) AND LOCAL BUILDING CODE REQUIREMENTS.)
2. ALL CONCRETE MASONRY SHALL CONFORM TO "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" (TMS 402-16) AND SPECIFICATION FOR MASONRY STRUCTURES" (TMS 602-16) AND LOCAL BUILDING CODE REQUIREMENTS.
3. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, TYPE I OR II.
4. THE MINIMUM PRISM COMPRESSIVE STRENGTH (fm) SHALL BE 2500 PSI.
5. ASTM C270, TYPE "S" MORTAR WITH A MINIMUM COMPRESSIVE STRENGTH OF 3250 PSI SHALL BE USED FOR ALL MASONRY WALLS.
6. GROUT TO FILL CORES SHALL BE ASTM C476, COARSE GROUT (3/8" MAXIMUM AGGREGATE) WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI IN 28 DAYS.
7. REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60.
8. LAY MASONRY UNITS WITH FULL MORTAR COVERAGE ON HORIZONTAL AND VERTICAL FACE SHELLS. BED WEBS IN MORTAR IN STARTING COURSE OF FOOTINGS AND IN ALL COURSES OF COLUMNS AND PILASTERS, AND WHERE ADJACENT TO CELLS OR CAVITIES TO BE REINFORCED OR FILLED WITH CONCRETE OR GROUT.
9. MASONRY SHALL BE LAID IN RUNNING BOND, UNLESS NOTED OTHERWISE.
10. VERTICAL AND HORIZONTAL REINFORCING LAP SPICES SHALL BE AS FOLLOWS:
- | BAR SIZE | TENSION LAP SPICE CLASS "A" | TENSION LAP SPICE CLASS "B" | COMPRESSION SPICE |
|----------|-----------------------------|-----------------------------|-------------------|
| #4 | 22" | 20" | 11" |
| #5 | 28" | 36" | 14" |
| #6 | 33" | 43" | 17" |
11. PROVIDE HORIZONTAL LADDER STYLE JOINT REINFORCING WITH 9 GAUGE SIDE AND CROSS RODS (GALVANIZED) SPACED AT 16" ON CENTER VERTICALLY. HORIZONTAL JOINT REINFORCING SHALL BE LAPPED A MINIMUM OF (2) CROSS BARS OR 6", WHICHEVER IS GREATER.
12. MAXIMUM GROUT POUR SHALL BE 5 FEET. CONSOLIDATE BY MECHANICAL VIBRATION.
13. MORTAR PROTRUSIONS, EXTENDING INTO CELLS OR CAVITIES TO BE REINFORCED AND FILLED, SHALL BE REMOVED.
14. GROUT A MINIMUM OF 16 INCHES x 24 INCHES WIDE CENTERED UNDER ALL BEAM BEARINGS AND 8 INCHES x 16 INCHES WIDE CENTERED UNDER ALL LINTEL BEARINGS.
15. GROUT A MINIMUM OF 8 INCHES x 24 INCHES WIDE CENTERED UNDER ALL JOIST BEARINGS.
16. GROUT CORES SOLID A MINIMUM OF ONE COURSE BELOW ANY CHANGE IN WALL THICKNESS.
17. THE COLLAR-JOINT IN MULTI-WYTHE WALLS BELOW GRADE SHALL BE FULLY GROUTED AS THE WALL IS CONSTRUCTED.
18. FILL ALL BEARING POCKETS WITH SOLID MASONRY AFTER INSTALLING BEAMS.
19. WHERE THERE IS A CHANGE IN BOND BEAM ELEVATION, PROVIDE LAP BETWEEN BONDS BEAMS THROUGH 12 BARS OF VERTICAL REINFORCING OR 4 FEET, WHICHEVER IS GREATER.
20. ALL CORNERS ARE TO BE TIED BY MASONRY BOND.
21. CMU WALLS SHALL HAVE VERTICAL CONTROL JOINTS LOCATED APPROXIMATELY 20'-0" O.C. REFER TO TYPICAL CONTROL JOINT DETAILS ON STRUCTURAL DRAWINGS FOR CONTROL JOINT DETAILS AND RESTRICTIONS. LOCATIONS OF CMU CONTROL JOINTS DO NOT HAVE TO ALIGN WITH VENEER CONTROL JOINTS. REFER TO ARCHITECTURAL DRAWINGS FOR VENEER CONTROL JOINT LOCATIONS.
22. PROVIDE MATERIAL MEANS TO DEBOND MORTAR FROM DISSIMILAR MATERIALS IN ALL VENEERS (I.E., CAST-STONE AND CLAY BRICK, CONCRETE BLOCK AND CLAY BRICK, ETC.)
23. PROVIDE 8" DEEP BOND BEAM w/ (2) #5 CONTINUOUS BARS ABOVE ALL WALL OPENINGS OF 2'-8" WIDE AND LESS. EXTEND BOND BEAM 8" PAST OPENING EACH END OF BEARING.

C&C WIND PRESSURES				
Zone:	10 SF	50 SF	100 SF	
Roof:	1	24.5 psf	20.9 psf	19.4 psf
	1'	-96.0 psf	-81.3 psf	-74.9 psf
	1'	24.5 psf	20.9 psf	19.4 psf
Overhang Zone 2	2	-55.1 psf	-55.1 psf	-55.1 psf
	2	55.1 psf	49.5 psf	47.0 psf
	3	-126.6 psf	-107.7 psf	-99.6 psf
Overhang Zone 3	2	-181.7 psf	-157.2 psf	-146.6 psf
	3	55.1 psf	49.5 psf	47.0 psf
	5	-181.7 psf	-157.2 psf	-146.6 psf
Zone:	10 SF	50 SF	100 SF	
Walls:	4	55.1 psf	49.5 psf	47.0 psf
	4	59.7 psf	-54.1 psf	-51.6 psf
	5	55.1 psf	49.5 psf	47.0 psf
Notes:	1	-73.5 psf	-62.2 psf	-57.3 psf
	1	1	1	1
	1	1	1	1

COMPONENT AND CLADDING WALL PRESSURE ZONES



COMPONENT AND CLADDING ROOF PRESSURE ZONES



PLYWOOD DECKING AND SHEATHING:

1. ALL PLYWOOD SHEATHING AT WALLS SHALL BE:
- A. AT INTERIOR NOT EXPOSED TO LONG TERM WEATHER
- a. 24/16 APA RATED 1/2" STRUCTURAL I WALL SHEATHING EXPOSURE 1.
2. ALL PLYWOOD WALLS SHALL BE NAILED TO SUPPORTING MEMBERS ALONG THE EDGES WITH 10d NAILS SPACED AT 6" O.C. AND AT INTERMEDIATE SUPPORTS WITH 10d NAILS SPACED AT 12" O.C. UNLESS NOTED OTHERWISE AT SHEARWALL SCHEDULE

SPECIAL INSPECTION AND TESTING:

THIS PROJECT REQUIRES SPECIAL INSPECTION AND TESTING IN ACCORDANCE WITH CHAPTER 17 OF THE FLORIDA BUILDING CODE. THESE NOTES AND THE STATEMENT OF SPECIAL INSPECTIONS PREPARED FOR THE PROJECT OWNER ARE INTENDED TO INFORM THE CONTRACTOR OF THE QUALITY ASSURANCE PROGRAM AND THE EXTENT OF THE CONTRACTOR'S RESPONSIBILITIES.

THE SPECIAL INSPECTIONS AND TESTING PROGRAM:

THE SPECIAL INSPECTION AND TESTING PROGRAM IS A QUALITY ASSURANCE PROGRAM INTENDED TO ENSURE THAT THE WORK IS PERFORMED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THESE INSPECTIONS ARE IN ADDITION TO THE INSPECTIONS SPECIFIED IN IBC SECTION 110. THE SPECIAL INSPECTION PROGRAM DOES NOT RELIEVE THE CONTRACTOR OF HIS OR HER RESPONSIBILITY TO COMPLY WITH THE OFFICIAL CONTRACT DOCUMENTS. FURTHER, IT IS NOT INTENDED THAT THE CONTRACTOR'S CONTRACTUAL AND STATUTORY OBLIGATIONS ARE ANYWAY RELIEVED OR FOREGONE BY THE PRESENCE OF THE SPECIAL INSPECTOR. THE CONTRACTOR HAS THE SOLE RESPONSIBILITY FOR ANY DEVIATIONS FROM THE OFFICIAL CONTRACT DOCUMENTS. THE SPECIAL INSPECTOR DOES NOT HARDEN THE DUTIES OF THE BUILDING OFFICIAL NOR THE QUALITY CONTROL RESPONSIBILITIES AND PERSONNEL OF THE CONTRACTOR. JOB SITE SAFETY AND MEANS AND METHODS OF CONSTRUCTION ARE SOLELY THE RESPONSIBILITY OF THE CONTRACTOR.

THE PROJECT OWNER IS RESPONSIBLE FOR EMPLOYING SPECIAL INSPECTION SERVICES. THE SPECIAL INSPECTOR/AGENCY SHALL NOT BE IN THE EMPLOY OF THE CONTRACTOR, SUBCONTRACTOR OR MATERIAL SUPPLIER. IN THE CASE OF AN OWNER/CONTRACTOR, THE SPECIAL INSPECTOR/AGENCY SHALL BE EMPLOYED AS SPECIFIED BY THE BUILDING OFFICIAL.

THE SPECIAL INSPECTOR IS OBLIGATED TO BOTH THE OWNER AND THE BUILDING OFFICIAL FOR OBSERVING THAT THE WORK IS EXECUTED IN SUBSTANTIVE ACCORDANCE WITH THE OFFICIAL CONTRACT DOCUMENTS. THE OFFICIAL CONTRACT DOCUMENTS ARE DEFINED AS THE PERMITTED PLANS AND SPECIFICATIONS, ADDENDA, CHANGE ORDERS, ISSUED SKETCHES AND REVISION DRAWINGS, AND ALL DIRECTIVES ISSUED BY ARCHITECT/ENGINEER.

THE INSPECTION AND TESTING AGENTS SHALL DISCLOSE ANY PAST OR PRESENT BUSINESS RELATIONSHIP OR POTENTIAL CONFLICT OF INTEREST WITH THE CONTRACTOR OR ANY OF THE SUBCONTRACTORS WHOSE WORK IS TO BE INSPECTED OR TESTED. THE SPECIAL INSPECTORS MAY HAVE NO FINANCIAL INTEREST IN PROJECTS FOR WHICH THEY PROVIDE SPECIAL INSPECTION SERVICES.

SPECIAL INSPECTION REPORT REQUIREMENTS:

SPECIAL INSPECTION REPORTS AND A FINAL REPORT SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF THE WORK IS APPROVED FOR OCCUPANCY.

REPORT REQUIREMENT: SPECIAL INSPECTORS SHALL KEEP RECORDS OF ALL INSPECTIONS AND TESTS. THE SPECIAL INSPECTOR SHALL FURNISH THE INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INDICATE THAT THE WORK INSPECTED WAS OR WAS NOT COMPLETED IN CONFORMANCE TO THE APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THEY ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO THE COMPLETION OF THAT PHASE OF THE WORK. A FINAL REPORT DOCUMENTING THE REQUIRED SPECIAL INSPECTIONS, TESTS, AND CORRECTION OF ANY OF THE DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF THE WORK IS APPROVED FOR OCCUPANCY.

CONTRACTOR RESPONSIBILITIES:

THE CONTRACTOR IS RESPONSIBLE FOR NOTIFYING THE SPECIAL INSPECTOR IN ADVANCE OF CONSTRUCTION SCHEDULES AND PLANNED OPERATIONS IN ORDER TO ASSURE TIMELY AND APPROPRIATE INSPECTION FOR THE ITEMS LISTED IN THE SCHEDULE OF SPECIAL INSPECTIONS. THE CONTRACTOR SHALL PROVIDE ADEQUATE NOTICE TO THE SPECIAL INSPECTOR FOR ALL INSPECTIONS.

THE CONTRACTOR SHALL COOPERATE WITH AND ASSIST THE SPECIAL INSPECTOR IN PERFORMING HIS INSPECTION DUTIES. THE SPECIAL INSPECTOR SHALL HAVE FREE ACCESS TO THE PROJECT AT ALL TIMES. THE CONTRACTOR SHALL REVIEW THE SPECIAL INSPECTION PLAN AND COORDINATE THE SCHEDULE OF WORK TO ACCOMMODATE THE REQUIRED INSPECTIONS.

PROVIDE ACCESS TO APPROVED PLANS: THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING THE SPECIAL INSPECTOR ACCESS TO APPROVED PLANS. THE CONTRACTOR SHALL MAINTAIN A CURRENT SET OF THE CONTRACT DOCUMENTS AT THE JOB SITE.

CORRECT DISCREPANCIES AND DEVIATIONS: THE CONTRACTOR SHALL, UPON BEING INFORMED BY THE SPECIAL INSPECTOR, IMMEDIATELY CAUSE TO ELIMINATE SUCH DISCREPANCIES AND DEVIATIONS.

WORK COMPLETED WITHOUT INSPECTION: WORK REQUIRING INSPECTION WHICH IS COMPLETED WITHOUT INSPECTION WILL BE REJECTED SOLELY ON THAT BASIS.

RETAIN SPECIAL INSPECTION RECORDS: THE CONTRACTOR IS ALSO RESPONSIBLE FOR RETAINING AT THE JOB SITE ALL SPECIAL INSPECTION RECORDS COMPLETED BY THE SPECIAL INSPECTOR.

COORDINATE AND SUBMIT: THE CONTRACTOR IS RESPONSIBLE FOR PREPARING AND SUBMITTING TO THE BUILDING OFFICIAL AND THE OWNER A STATEMENT OF CONTRACTOR RESPONSIBILITY (IBC SECTION 1704.4) FOR THEMSELVES AND FOR SUBMITTING A STATEMENT OF CONTRACTOR RESPONSIBILITY FOR EACH STRUCTURAL COMPONENT SUBCONTRACTOR. THE STATEMENTS OF RESPONSIBILITY SHALL BE SUBMITTED PRIOR TO THE COMMENCEMENT OF WORK ON THE SYSTEM OR COMPONENT.

A. THE STATEMENT OF CONTRACTOR RESPONSIBILITY SHALL CONTAIN THE FOLLOWING:

1. ACKNOWLEDGEMENT OF AWARENESS OF THE SPECIAL INSPECTION REQUIREMENTS CONTAINED IN THE QUALITY ASSURANCE PLAN.
2. ACKNOWLEDGEMENT THAT CONTROL WILL BE EXERCISED TO OBTAIN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS APPROVED BY THE BUILDING OFFICIAL.
3. PROCEDURES FOR EXERCISING CONTROL WITHIN THE CONTRACTOR'S ORGANIZATION, THE METHOD AND FREQUENCY OF REPORTING, AND THE DISTRIBUTION OF THE REPORTS.
4. IDENTIFICATION AND QUALIFICATIONS OF THE PERSONS EXERCISING SUCH CONTROL AND THEIR POSITIONS IN THE ORGANIZATION.

- B. STRUCTURAL COMPONENT SUBCONTRACTORS INCLUDE BUT ARE NOT LIMITED TO STRUCTURAL STEEL FABRICATORS AND ERECTORS, COMPONENT FABRICATORS SUCH AS STEEL JOISTS, METAL OR WOOD TRUSSES, CONCRETE, AND MASONRY CONTRACTORS.
- C. AT THE COMPLETION OF STRUCTURAL COMPONENT FABRICATION, THE FABRICATORS SHALL SUBMIT A CERTIFICATE OF COMPLIANCE STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR COSTS OF:

RETESTING AND REINSPECTION OF MATERIALS, WORK, AND/OR PRODUCTS THAT DO NOT MEET THE REQUIREMENTS OF THE CONTRACT DOCUMENTS AND SHOP DRAWINGS/SUBMITTAL DATA.

REVIEW OF PROPOSED REPAIR AND/OR REPLACEMENT PROCEDURES BY THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE AND THE INSPECTORS AND TESTING AGENCIES.

REPAIR OR REPLACEMENT OF WORK THAT DOES NOT MEET THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.

THE CONTRACTOR IS RESPONSIBLE FOR THE TRAVEL COSTS OF THE SPECIAL INSPECTOR OR AGENTS WHEN SHOP INSPECTION IS REQUIRED OF A NON-APPROVED STRUCTURAL COMPONENT FABRICATOR.

INSPECTION OF FABRICATION:

WHERE FABRICATION OF STRUCTURAL LOAD BEARING, OR LATERAL LOAD RESISTING MEMBERS OR ASSEMBLIES ARE PERFORMED ON THE PREMISES OF THE FABRICATOR, THE SHOP FABRICATOR REQUIRES SPECIAL INSPECTION DURING THE FABRICATION OF ITEMS FOR THIS PROJECT.

EXEMPTION:

FABRICATORS APPROVED BY THE BUILDING OFFICIAL ARE EXEMPT FROM THE ON PREMISE INSPECTION. THE APPROVAL BY THE BUILDING OFFICIAL OF ANY FABRICATOR SHOULD BE PROPERLY DOCUMENTED PRIOR TO THE COMMENCEMENT OF FABRICATION. EXEMPTION WILL BE PROVIDED TO FABRICATORS WHO PROVIDE PROOF OF CERTIFICATION BY A NATIONALLY RECOGNIZED GOVERNING ASSOCIATION WHICH PERFORMS PERIODIC INSPECTIONS AND MAINTAINS QUALITY ASSURANCE CRITERIA. EXAMPLES ARE: AISC CERTIFICATION FOR A STEEL FABRICATOR, SJI CERTIFICATION FOR A STEEL JOIST MANUFACTURER, WTC AND TPI CERTIFICATION FOR A PRE-ENGINEERED WOOD TRUSS MANUFACTURER.

AT THE COMPLETION OF FABRICATION, THE FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

CONCRETE TESTING NOTES:

CONCRETE TESTING AND INSPECTION SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF ACI 318 AND THE SCHEDULE OF SPECIAL INSPECTIONS. SAMPLES FOR STRENGTH TESTS OF EACH CLASS OF CONCRETE PLACED EACH DAY SHALL BE TAKEN NOT LESS THAN ONCE A DAY, NOR LESS THAN ONCE FOR EACH 75 CUBIC YARDS, OF CONCRETE USED FOR FOOTINGS, NOR LESS THAN ONCE FOR EACH 5000 SQUARE FEET OF SURFACE AREA FOR SLABS. TEST REPORTS INDICATING NON-COMPLIANCE SHALL BE PROVIDED TO THE OWNER, ARCHITECT AND CONTRACTOR. A COPY OF THE TEST REPORTS SHALL BE AVAILABLE AT THE JOBSITE.

STEEL INSPECTION AND TESTING NOTES:

STRUCTURAL STEEL TESTING AND INSPECTION SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF THE REFERENCED STANDARDS AND THE SCHEDULE OF SPECIAL INSPECTIONS.

FIELD BOLTED CONNECTIONS WILL BE TESTED AND INSPECTED ACCORDING TO RCSC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS."

FIELD WELDS SHALL BE TESTED AND TESTED ACCORDING TO AWS D1.1. IN ADDITION TO VISUAL INSPECTION, WELDED MOMENT CONNECTIONS WILL BE INSPECTED BY ULTRASONIC, ASTM E164, OR OTHER AWS APPROVED METHOD.

OTHER REQUIRED INSPECTIONS:

THE REQUIREMENTS OF SPECIAL INSPECTIONS AND TESTING IN ACCORDANCE OF THE INTERNATIONAL BUILDING CODE ARE MINIMUM REQUIREMENTS AND DO NOT LIMIT THE REQUIREMENTS FOR THE CONTRACTOR TO PROVIDE OTHER QUALITY CONTROL INSPECTIONS AND TESTING REQUIRED BY THE OWNER, CONTRACT DOCUMENTS, OR OTHER GOVERNING AUTHORITIES HAVING JURISDICTION.

SCHEDULE OF SPECIAL INSPECTIONS

INSPECTION OF FABRICATORS

MATERIAL/ACTIVITY	SERVICE	APPLICABLE TO PROJECT	
		Y/N	EXTENT
VERIFY FABRICATION/QUALITY CONTROL PROCEDURES	IN-PLANT REVIEW (3) DURING FABRICATION	Y	SPECIAL INSPECTIONS ARE NOT REQUIRED WHERE THE FABRICATOR IS REGISTERED AND APPROVED IN ACCORDANCE WITH FBC CHAPTER 2.

STEEL CONSTRUCTION

(AISC 360: CHAPTER N)

MATERIAL/ACTIVITY	SERVICE	APPLICABLE TO PROJECT	
		Y/N	EXTENT
1. FABRICATOR AND ERECTOR DOCUMENTS (VERIFY REPORTS AND CERTIFICATES AS LISTED IN AISC 360, CHAPTER N, SECTION N3, PARAGRAPH 2 FOR COMPLIANCE WITH CONSTRUCTION DOCUMENTS)	SUBMITTAL REVIEW	Y	EACH SUBMITTAL
2. MATERIAL VERIFICATION OF STRUCTURAL STEEL HIGH-STRENGTH BOLTS, NUTS, WASHERS AND WELD FILLER MATERIALS.	SHOP (3) AND FIELD INSPECTION	Y	PERIODIC
3. EMBEDMENTS, VERIFY DIAMETER, GRADE, TYPE, LENGTH AND EMBEDMENT. (SEE 1705.3 FOR ANCHORS)	FIELD INSPECTION	Y	PERIODIC
4. VERIFY MEMBERS LOCATIONS, BRACES, STIFFENERS AND APPLICATION OF JOINT DETAILS AT EACH CONNECTION COMPLY WITH CONSTRUCTION DOCUMENTS	FIELD INSPECTION	Y	PERIODIC
5. STRUCTURAL STEEL WELDING		—	—
a. INSPECTION TASKS PRIOR TO WELDING (OBSERVE, OR PERFORM FOR EACH WELDED JOINT OR MEMBER, THE QA TASKS LISTED IN AISC 360, TABLE N5.4-1)	SHOP (3) AND FIELD INSPECTION	Y	OBSERVE OR PERFORM AS NOTED (4)
b. INSPECTION TASKS DURING WELDING (OBSERVE, OR PERFORM FOR EACH WELDED JOINT OR MEMBER, THE QA TASKS LISTED IN AISC 360, TABLE N5.4-2)	SHOP (3) AND FIELD INSPECTION	Y	OBSERVE (4)
c. INSPECTION TASKS AFTER WELDING (OBSERVE, OR PERFORM FOR EACH WELDED JOINT OR MEMBER, THE QA TASKS LISTED IN AISC 360, TABLE N5.4-3)	SHOP (3) AND FIELD INSPECTION	Y	OBSERVE OR PERFORM AS NOTED (4)
d. NONDESTRUCTIVE (NDT) TESTING OF WELDED JOINTS (AISC 360: N5.5):		—	—
1) USE OF QUALIFIED NONDESTRUCTIVE TESTING PERSONNEL	PERFORMED	Y	—
2) COMPLETE PENETRATION GROOVE WELDS 5/16" OR GREATER IN RISK CATEGORY II	SHOP (3) OR FIELD ULTRASONIC TESTING - 20% OF WELDS MINIMUM	Y	PERFORM
3) WELDED JOINTS SUBJECT TO FATIGUE.	DT AND UT SHALL BE PERFORMED ON 100% OF WELDED JOINTS IDENTIFIED ON CONTRACT DRAWINGS AS BEING SUBJECT TO FATIGUE.	Y	PERFORM
4) WELDED TAB REMOVAL SITES.	AT THE END OF WELDS WHERE WELD TABS HAVE BEEN REMOVED, MAGNETIC PARTICLE TESTING SHALL BE PERFORMED ON THE SAME BEAM TO COLUMN JOINTS RECEIVING UT.	Y	PERFORM
5) FABRICATORS NDT REPORTS WHEN FABRICATORS PERFORMS NDT	VERIFY REPORTS	Y	EACH SUBMITTAL (5)
6. STRUCTURAL STEEL BOLTING:	SHOP (3) AND FIELD INSPECTION	—	—
a. INSPECTION TASKS PRIOR TO BOLTING (OBSERVE, OR PERFORM TASKS FOR EACH BOLTED CONNECTION, IN ACCORDANCE WITH QA TASKS LISTED IN AISC 360, TABLE N5.6-1).	—	—	OBSERVE (4)
b. INSPECTION TASKS DURING BOLTING (OBSERVE THE QA TASKS LISTED IN AISC 360, TABLE N5.6-2)	—	—	OBSERVE (4)
1) PRE-TENSIONED AND SLIP-CRITICAL JOINTS	—	—	—
a) TURN-OF-NUT METHOD (MATCHMARKING)	—	Y	PERIODIC
b) DIRECT TENSION INDICATOR	—	Y	PERIODIC
c) TWIST-OFF TYPE TENSION CONTROL BOLT	—	Y	PERIODIC
2) SNUG-TIGHT JOINTS	—	Y	PERIODIC
c. INSPECTION TASKS AFTER BOLTING (PERFORM TASKS FOR EACH BOLTED CONNECTION IN ACCORDANCE WITH QA TASKS LISTED IN AISC 360, TABLE N5.6-3)	SHOP (3) AND FIELD INSPECTION AND TESTING	Y	PERFORM (4)

OPEN-WEB STEEL JOIST AND JOIST GIRDERS

TYPE	APPLICABLE TO PROJECT		REFERENCED STANDARD
	Y/N	EXTENT	
1. INSTALLATION OF OPEN-WEB STEEL JOISTS AND JOIST GIRDERS			
a. END CONNECTIONS - WELDED OR BOLTED.	Y	PERIODIC	SJI SPECIFICATIONS LISTED IN SECTION 2207.1.
b. BRIDGING - HORIZONTAL OR DIAGONAL.	—	—	—
1) STANDARD BRIDGING.	Y	PERIODIC	SJI SPECIFICATIONS LISTED IN SECTION 2207.1.
2) BRIDGING THAT DIFFERS FROM THE SJI SPECIFICATIONS LISTED IN SECTION 2207.1.	Y	PERIODIC	—

SOILS

MATERIAL/ACTIVITY	SERVICE	APPLICABLE TO PROJECT	
		Y/N	EXTENT
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	FIELD INSPECTION	Y	PERIODIC
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	FIELD INSPECTION	Y	PERIODIC
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS	FIELD INSPECTION	Y	PERIODIC
4. VERIFY USE OF PROPER MATERIALS, DENSITIES, AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	FIELD INSPECTION	Y	CONTINUOUS
5. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY	FIELD INSPECTION	Y	PERIODIC

CONCRETE CONSTRUCTION

MATERIAL/ ACTIVITY	Y/N	APPLICABLE TO PROJECT		REFERENCED STANDARD	
		EXTENT			
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS AND VERIFY PLACEMENT.	Y	PERIODIC	ACI 318 CH.20, 25.2, 25.3, 26.6.1-26.6.3	1908.4	
2. REINFORCING BAR WELDING:					
a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706	Y	PERIODIC	AWS D14 ACI 318: 26.6.4	-	IBC REFERENCE
b. INSPECT SINGLE-PASS FILLET WELDS, MAX. 5/16"	Y	CONTINUOUS			
c. INSPECT ALL OTHER WELDS.	Y	PERIODIC	ACI 318: 17.8.2	-	
3. INSPECT ANCHORS CAST IN CONCRETE.	Y	PERIODIC	ACI 318: 17.8.2.4	-	
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS.	Y	CONTINUOUS	ACI 318: 17.8.2.4	-	
a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.	Y	PERIODIC	ACI 318: 17.8.2	-	
b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.4.	Y	PERIODIC	ACI 318: CH. 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3	
5. VERIFY USE OF REQUIRED DESIGN MIX.	Y	PERIODIC	ASTM C172 ASTM C31	1908.10	
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	Y	CONTINUOUS	ACI 318: 26.5, 26.12	1908.6, 1908.7, 1908.8	
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	Y	CONTINUOUS	ACI 318: 26.5	1908.6, 1908.7, 1908.8	
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE ARCHIVES.	Y	PERIODIC	ACI 318: 26.5.3-26.5.5	1908.9	
9. INSPECT PRESTRESSED CONCRETE FOR:					
a. APPLICATION OF PRESTRESSING FORCES	N	CONTINUOUS	ACI 318: 26.10	-	
b. GROUTING OF BONDED PRESTRESSING TENDONS.	N	CONTINUOUS		-	
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.	N	PERIODIC	ACI 318: CH 26.9	-	
11. VERIFY IN-SITU CONCRETE STRENGTH PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	N	PERIODIC	ACI 318: 26.11.2	-	
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	Y	PERIODIC	ACI 318: 26.11.1[2b]	-	
13. CONCRETE STRENGTH TESTING AND VERIFICATION OF COMPLIANCE WITH CONSTRUCTION DOCUMENTS.	Y	PERIODIC	-	-	

FOR SI: INCH = 25.4 mm.

(a) SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION SHALL BE INCLUDED IN THE RESEARCH REPORT FOR THE ANCHOR ISSUED BY AN APPROVED SOURCE IN ACCORDANCE WITH 17.8.2 IN ACI 318, OR OTHER QUALIFICATION PROCEDURES, WHERE SPECIFIC REQUIREMENTS ARE NOT PROVIDED. SPECIAL INSPECTION REQUIREMENTS SHALL BE SPECIFIED BY THE REGISTERED DESIGN PROFESSIONAL AND SHALL BE APPROVED BY THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF THE WORK.

COLD FORMED STEEL DECK

(SDI QAQC - 2017)

TABLE 1.1 INSPECTION OR EXECUTION TASKS PRIOR TO DECK PLACEMENT

TASK	Y/N	EXTENTS	
		Y	EXTENT
A. VERIFY COMPLIANCE OF MATERIAL, (DECK AND ALL DECK ACCESSORIES) WITH CONSTRUCTION DOCUMENTS, INCLUDING PROFILES, MATERIAL PROPERTIES, AND BASE METAL THICKNESS.	Y	Y	EACH SUBMITTALS
B. DOCUMENT ACCEPTANCE OR REJECTION OF DECK AND DECK ACCESSORIES	Y	Y	EACH SUBMITTALS

TABLE 1.2 INSPECTION OR EXECUTION TASKS AFTER DECK PLACEMENT

TASK	Y/N	EXTENTS	
		Y	EXTENT
A. VERIFY COMPLIANCE OF DECK AND ALL DECK ACCESSORIES INSTALLATION WITH CONSTRUCTION DOCUMENTS	Y	Y	PERIODIC
B. VERIFY DECK MATERIALS ARE REPRESENTED BY THE MILL CERTIFICATIONS THAT COMPLY WITH THE CONSTRUCTION DOCUMENTS	Y	Y	PERIODIC
C. DOCUMENT ACCEPTANCE OR REJECTION OF INSTALLATION OF DECK AND DECK ACCESSORIES	Y	Y	PERIODIC

TABLE 1.3 INSPECTION OR EXECUTION TASKS PRIOR TO WELDING

TASK	Y/N	EXTENTS	
		Y/N	EXTENTS
A. WELDING PROCEDURE SPECIFICATIONS, (WPS) AVAILABLE	Y	Y	PERIODIC
B. MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	Y	Y	PERIODIC
C. MATERIAL IDENTIFICATION, (TYP/GRADE)	Y	Y	PERIODIC
D. CHECK WELDING EQUIPMENT	Y	Y	PERIODIC

TABLE 1.4 INSPECTION OR EXECUTION TASKS DURING WELDING

TASK	Y/N	EXTENTS	
		Y/N	EXTENTS
A. USE OF QUALIFIED WELDERS	Y	Y	PERIODIC
B. CONTROL AND HANDLING OF WELDING CONSUMABLES	Y	Y	PERIODIC
C. ENVIRONMENTAL CONDITIONS, (WIND SPEED, MOISTURE, TEMPERATURE)	Y	Y	PERIODIC
D. WPS FOLLOWED	Y	Y	PERIODIC

TABLE 1.5 INSPECTION OR EXECUTION TASKS AFTER WELDING

TASK	Y/N	EXTENT	
		Y	EXTENT
A. VERIFY SIZE AND LOCATION OF WELDS, INCLUDING SUPPORT, SIDELAP, AND PERIMETER WELDS.	Y	Y	PERIODIC
B. WELDS MEET VISUAL ACCEPTANCE CRITERIA	Y	Y	PERIODIC
C. VERIFY REPAIR ACTIVITIES	Y	Y	PERIODIC
D. DOCUMENT ACCEPTANCE OR REJECTION OF WELDS	Y	Y	PERIODIC

TABLE 1.6 INSPECTION OR EXECUTION TASKS PRIOR TO MECHANICAL FASTENING

TASK	Y/N	EXTENTS	
		Y	EXTENTS
A. MANUFACTURER INSTALLATION INSTRUCTIONS AVAILABLE FOR MECHANICAL FASTENERS	Y	Y	PERIODIC
B. PROPER TOOLS AVAILABLE FOR FASTENER INSTALLATION	Y	Y	PERIODIC
C. PROPER STORAGE FOR MECHANICAL FASTENERS	Y	Y	PERIODIC

TABLE 1.7 INSPECTION OR EXECUTION TASKS DURING MECHANICAL FASTENING

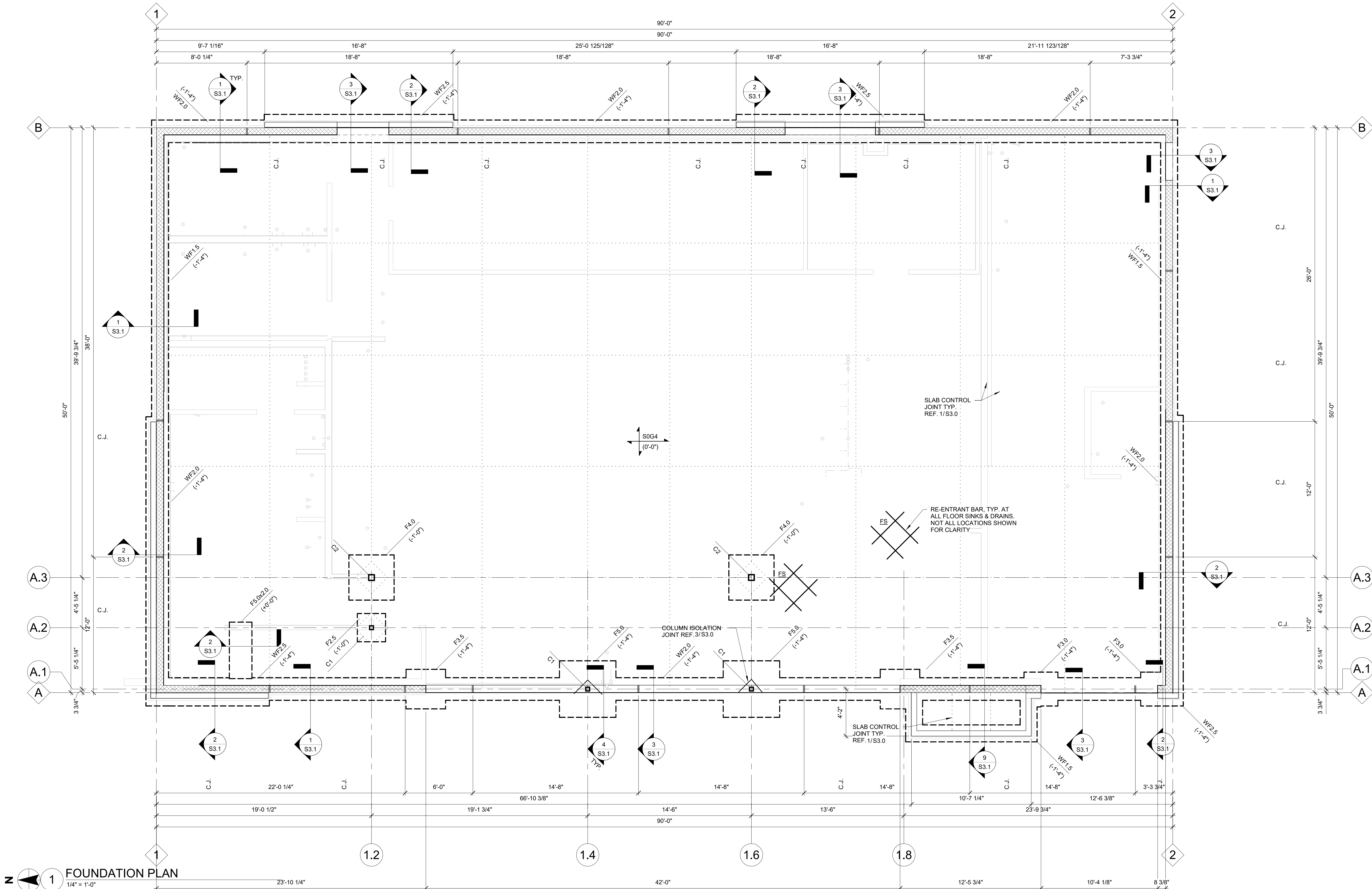
TASK	Y/N	EXTENTS	
		Y	EXTENTS
A. FASTENERS ARE POSITIONED AS REQUIRED	Y	Y	PERIODIC
B. FASTENERS ARE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.	Y	Y	PERIODIC

TABLE 1.8 INSPECTION OR EXECUTION TASKS AFTER MECHANICAL FASTENING

TASK	Y/N	EXTENTS	
		Y/N	EXTENTS
A. CHECK SPACING, TYPE, AND INSTALLATION OF SUPPORT FASTENERS	Y	Y	PERIODIC
B. CHECK SPACING, TYPE, AND INSTALLATION OF SIDELAP FASTENERS	Y	Y	PERIODIC
C. CHECK SPACING, TYPE, AND INSTALLATION OF PERIMETER FASTENERS	Y	Y	PERIODIC
D. VERIFY REPAIR ACTIVITIES	Y	Y	PERIODIC
E. DOCUMENT ACCEPTANCE OR REJECTION OF WELDS	Y	Y	PERIODIC

MASONRY CONSTRUCTION

MINIMUM VERIFICATION	REQUIRED FOR QUALITY ASSURANCE			REFERENCE FOR CRITERIA	
	LEVEL 1	LEVEL 2	LEVEL 3	TMS 402	TMS 602
1. PRIOR TO CONSTRUCTION, VERIFICATION OF COMPLIANCE OF SUBMITTALS.	R	R	R		ART. 1.5
2. PRIOR TO CONSTRUCTION, VERIFICATION OF <i>f_m</i> AND <i>f_{rac}</i> , EXCEPT WHERE SPECIFICALLY EXEMPTED BY THE CODE.	NR	R	R		ART. 1.4 B
3. DURING CONSTRUCTION, VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) WHEN SELF-CONSOLIDATING GROUT IS DELIVERED TO THE PROJECT SITE.	NR	R	R		ART. 1.5 AND 1.6.3
4. DURING CONSTRUCTION, VERIFICATION OF <i>f_m</i> AND <i>f_{rac}</i> , FOR EVERY 5,000 SQUARE FEET.	NR	NR	R		ART. 1.4 B
5. DURING CONSTRUCTION, VERIFICATION OF PROPORTIONS OF MATERIALS AS DELIVERED TO THE PROJECT SITE FOR PREMIXED OR PREBLENDED MORTAR, PRESTRESSING GROUT, AND GROUT OTHER THAN SELF-CONSOLIDATING GROUT.	NR	NR	R		ART. 1.4 B
MINIMUM SPECIAL INSPECTIONS	FREQUENCY			REFERENCE FOR CRITERIA	
	LEVEL 1	LEVEL 2	LEVEL 3	TMS 402	TMS 602
1. AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:					
A. PROPORTIONS OF SITE-PREPARED MORTAR.	NR	P	P		ART. 2.1, 2.6 A AND 2.6C
B. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES.	NR	P	P		ART. 2.4 B AND 2.4 H
C. GRADE, TYPE, AND SIZE OF REINFORCEMENT, CONNECTORS, ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES.	NR	P	P		ART. 3.4 AND 3.6 A
D. PRESTRESSING TECHNIQUE	NR	P	P		ART. 3.6 B
E. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	NR	C ^{(b)(PH)}	P		ART. 2.1 C.1
F. SAMPLE PANEL CONSTRUCTION	NR	P	C		ART. 1.6 D
2. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:					
A. GROUT SPACE	NR	P	C		ART. 3.2 D AND 3.2 F
B. PLACEMENT OF PRESTRESSING TENDONS AND ANCHORAGES.	NR	P	P	SEC. 10.8 AND 10.9	ART. 2.4 AND 3.6
C. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND ANCHOR BOLTS.	NR	P	C	SEC. 6.1, 6.3.1, 6.3.6 AND 6.3.7	ART. 3.2 E AND 3.4
D. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS.	NR	P	P		ART. 2.6 B AND 2.4 G.1 b
3. VERIFY COMPLIANCE OF THE FOLLOWING DURING CONSTRUCTION:					
A. MATERIALS AND PROCEDURES WITH THE APPROVED SUBMITTALS.	NR	P	P		ART. 1.5
B. PLACEMENT OF MASONRY UNITS AND MORTAR JOINT CONSTRUCTION	NR	P	P		ART. 3.3 B
C. SIZE AND LOCATION OF STRUCTURAL MEMBERS.	NR	P	P		ART. 3.3 F
D. TYPE, SIZE, LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OR MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION.	NR	P	C	SEC. 1.2.1 (e), 6.2.1, AND 6.3.1	
E. WELDING OF REINFORCEMENT.	NR	C	C	SEC. 6.1.6.1.2	
F. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURES BELOW 40° F) OR HOT WEATHER (TEMPERATURE ABOVE 90° F).	NR	P	P		ART. 1.8 C AND 1.8 D
G. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE	NR	C	C		ART. 3.6 B
H. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE.	NR	C	C		ART. 3.5 AND 3.6 C
I. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	NR	C ^{(b)(PH)}	C		ART. 3.3 B.9 AND 3.3 F.1 b
4. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS:	NR	P	C		ART. 1.4 B.2 a, 3.1.4 B.2 b, 3.1.4 B.2 c, 3.1.4 B.4



- A. DO NOT SCALE DRAWINGS.
- B. FOR DIMENSIONS NOT SHOWN, SEE ARCHITECTURAL DRAWINGS.
- C. REFER TO SHEETS S1.0 FOR GENERAL STRUCTURAL NOTES.
- D. COORDINATE WORK WITH OTHER TRADES, EQUIPMENT FURNISHED BY OTHERS, REQUIREMENTS OF THE TENANT AND LANDLORD/BUILDING OWNER, AND THE CONSTRAINTS OF THE EXISTING CONDITIONS OF THE PROJECT SITE. COORDINATE THE INSTALLATION WITH OTHER TRADES AS REQUIRED TO ENSURE A NEAT AND ORDERLY INSTALLATION. NOTIFY ARCHITECT/ENGINEER OF ANY DISCREPANCIES BEFORE STARTING WORK.
- E. GENERAL CONTRACTOR AND SUB-CONTRACTORS SHALL CAREFULLY REVIEW THE CONSTRUCTION DOCUMENTS. INFORMATION REGARDING THE COMPLETE WORK IS DISPERSED THROUGHOUT THE DOCUMENT SET AND CANNOT BE ACCURATELY DETERMINED WITHOUT REFERENCE TO THE COMPLETE DOCUMENT SET.
- F. ELEVATIONS ARE REFERENCED FROM TOP OF SLAB ELEVATION = (0'-0"). SEE CIVIL DRAWINGS FOR ACTUAL ELEVATION. ALL ELEVATIONS REFERENCED FROM BASE ELEVATION AND ARE SHOWN AS (X'-X.X").
- G. TOP OF FOUNDATIONS ARE (-1'-4") U.O.N.
- H. SEE ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS FOR SLAB PENETRATIONS AND DRAIN LOCATIONS.
- I. CONTROL JOINTS TO BE PROVIDED IN CONCRETE PER PLAN. REF. DETAIL 1/S3.0.
- J. PROVIDE (2)#4 x 3'-0" L.G. RE-ENTRANT BARS AT ALL RE-ENTRANT SLAB CORNERS. REF. TO DETAIL 2/S3.0.
- K. SEE SHEET S3.0 FOR TYPICAL FOUNDATION DETAILS.

- LEGEND**
- "Fx.x" DENOTES COLUMN FOOTING. REFER TO FOOTING SCHEDULE.
- "WFx.x" DENOTES CONTINUOUS WALL FOOTING. REFER TO FOOTING SCHEDULE.
- "C1" DENOTES STEEL COLUMN, SEE COLUMN SCHEDULE ON THIS SHEET.
- DENOTES SLAB DEPRESSION. REFER TO ARCHITECTURAL PLANS FOR ADDITIONAL INFORMATION.
- 4" NORMAL WEIGHT CONCRETE SLAB ON GRADE REINFORCED WITH #6x12 SWW 2 WWF (CENTER IN SLAB). BEAR SLAB ON 4" THICK WELL-GRADED COMPACTED GRANULAR FILL MATERIAL. PROVIDE 6 MIL. MIN. POLYVINYL CHLORIDE OR POLYETHYLENE SHEET MEMBRANE BETWEEN BOTTOM OF SLAB AND GRANULAR BASE.
- DENOTES VERTICAL REINFORCING AT END OF SHEAR WALLS. REF. TO DETAIL 9/S3.2 FOR ADDITIONAL INFORMATION.
- "FS" DENOTES FLOOR SINK. PROVIDE RE-ENTRANT BARS AT ALL FLOOR SINKS. REFER TO PLUMBING PLAN FOR ADDITIONAL INFORMATION.
- "FD" DENOTES FLOOR DRAIN. SLOPE SLAB TO DRAIN PER ARCHITECTURAL PLANS. REFER TO ARCHITECTURAL AND PLUMBING PLANS FOR ADDITIONAL INFORMATION.
- "HD" DENOTES HUB DRAIN. REFER TO PLUMBING PLAN FOR ADDITIONAL INFORMATION.

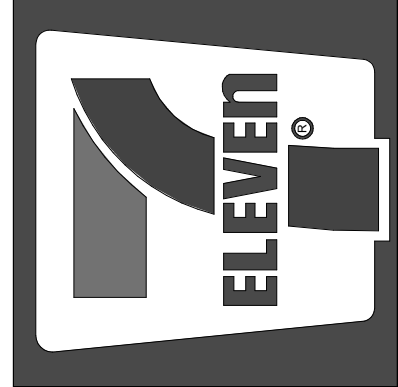
SPREAD FOOTING SCHEDULE				
MARK	WIDTH	LENGTH	THICKNESS	REINFORCING
F2.5	2'-0"	2'-6"	1'-0"	(3) #5 BARS CONT. BOT.
F3.0	3'-0"	3'-0"	1'-0"	(3) #5 BARS EA. WAY BOT.
F3.5	3'-6"	3'-6"	1'-0"	(4) #5 BAR EA. WAY BOT.
F4.0	4'-0"	4'-0"	1'-0"	(4) #5 BARS EA. WAY BOT.
F5.0	5'-0"	5'-0"	1'-4"	(5) #5 BAR EA. WAY BOT.
F5.0x2.0	2'-0"	5'-0"	1'-0"	(3) #5 BARS LONG AND (5) #5 BARS SHORT, BOT.

WALL FOOTING SCHEDULE			REINFORCING
MARK	WIDTH	THICKNESS	
WF1.5	1'-6"	1'-0"	(2) #5 BARS CONT. BOT. AND #5s AT 12" O.C.
WF2.0	2'-0"	1'-0"	(3) #5 BARS CONT. BOT. AND #5s AT 12" O.C.
WF2.5	2'-6"	1'-0"	(3) #5 BARS CONT. BOT. AND #5s AT 12" O.C.

COLUMN SCHEDULE				
NOTE: REFERENCE DETAIL 4/S3.0 FOR TYPICAL BASE PLATE DETAILS. NOTE: TOP OF COLUMN SHALL MATCH BOTTOM OF DECKING = (+16'-6 1/4") U.O.N.				
MARK	SIZE	BASE PLATE WALKTH	ANCHOR ROD	EMBEDMENT
C1	HSS4X4X3/8	10"x10"x3/4"	3/4" DIA.	0'-9"
C2	HSS6X6X1/2	12"x12"x3/4"	3/4" DIA.	0'-9"

REV #	DATE	DESCRIPTION

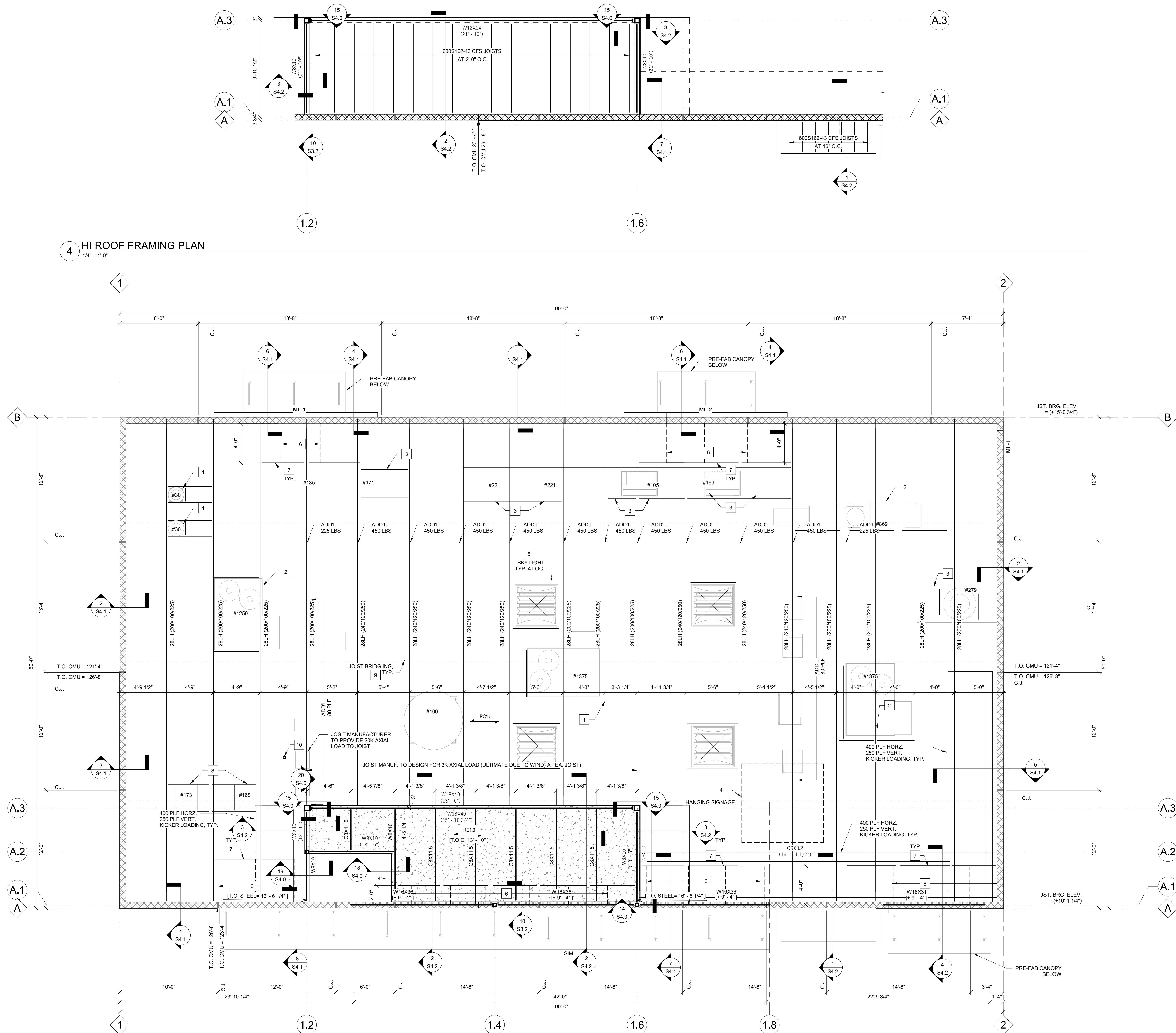
7-ELEVEN INC
3200 HACKBERRY ROAD, IRVING TEXAS 75063
7-ELEVEN #1055406
NORTHLAKE & COCONUT (COCONUT CROSSING)
PALM BEACH GARDENS, FL
FOUNDATION PLAN



Drawings and Specifications as instruments of service are and shall remain the property of the Architect. They are not to be used on other projects or extensions to this project except by agreement in writing and with appropriate compensation to the Architect.

The seal and signature apply only to the document to which they are affixed and we expressly disclaim any responsibility for all other plans, specifications, estimates, reports or other documents or instruments relating to or intended to be used for any part or parts of the project.

Florida Certificate of Authority #02000001916



4 HI ROOF FRAMING PLAN
1/4" = 1'-0"

1 ROOF FRAMING PLAN
1/4" = 1'-0"

GENERAL NOTES

A. DO NOT SCALE DRAWINGS.

B. FOR DIMENSIONS NOT SHOWN, SEE ARCHITECTURAL DRAWINGS.

C. REFER TO SHEETS S1.0 FOR GENERAL STRUCTURAL NOTES.

D. COORDINATE WORK WITH OTHER TRADES. EQUIPMENT FURNISHED BY OTHERS. REQUIREMENTS FOR THE CONTRACTOR AND LAND/OVERBUILDING OWNER, AND THE CONSTRAINTS OF THE EXISTING CONDITIONS OF THE PROJECT SITE. COORDINATE THE INSTALLATION WITH OTHER TRADES AS REQUIRED TO ENSURE A NEAT AND ORDERLY INSTALLATION. NOTIFY ARCHITECT/ENGINEER OF ANY DISCREPANCIES BEFORE STARTING WORK.

E. GENERAL CONTRACTOR AND SUB-CONTRACTORS SHALL CAREFULLY REVIEW THE CONSTRUCTION DOCUMENTS. INFORMATION REGARDING ANY OF THE COMPLETELY WAS DISPERSED THROUGHOUT THE DOCUMENT SET AND CANNOT BE ACCURATELY DETERMINED WITHOUT REFERENCE TO THE COMPLETE DOCUMENT SET.

F. ELEVATIONS ARE REFERENCED FROM TOP OF SLAB ELEVATION = +0'-0"

G. PROVIDE ROOF DECK SUPPORT ANGLE AT JOIST BEARING EDGES OF ROOF DECK PERIMETER. LOCATE JOINTS IN CONTINUOUS ANGLE AT JOIST LOCATIONS. WELD ANGLE TO EACH JOIST TOP CHORD TRY. U.N.O.

H. ALL ROOF TOP UNIT DIMENSIONS SHALL BE COORDINATED BY THE G.C. THE CURB MANUFACTURER IS TO PROVIDE HVAC AND ROOF TOP EQUIPMENT UP/UT RESTRAINT BRACKET FOR APPLICABLE WIND SPEED.

I. THE TRUSS MANUFACTURER SHALL DESIGN THE TRUSSES FOR ADDITIONAL LOADING PER PLAN. RTU WEIGHTS SHALL BE APPLIED TO THE TOP CHORD, AND ADDITIONAL POINT LOADS AND LINE LOADS ARE TO BE APPLIED TO THE ARCHITECTURE COMPOUND (U.N.O.) AND SHALL BE APPLIED TO THE BOTTOM CHORD.

J. THE LOCATION OF HANGING ARCHITECTURAL COMPONENTS, AND THE RESULTANT LOADS, SHALL BE COORDINATED BY THE G.C.

K. ALL ROOF PENETRATIONS, DIMENSIONED AND NON-DIMENSIONED, ARE SUBJECT TO CHANGE DUE TO FIELD CONDITIONS. COORDINATE SIZE & LOCATIONS WITH GENERAL CONTRACTOR AND OTHER TRADES PRIOR TO PENETRATING ROOF.

L. FOR MASONRY WALL REINFORCING REFER TO SECTION 353.2.

M. REFER TO DETAIL 353.2 FOR TYPICAL MASONRY WALL CONTROL JOINT DETAIL.

N. REFER TO SHEET 353.2 FOR TYPICAL MASONRY DETAILS.

LEGEND	
RC1.5	1.5 TYPE B 20 GA. METAL ROOF DECK DIRECTION - SEE GENERAL NOTES FOR FASTENINGS SCHEDULE.
RC1.0	21/2" NORMAL WEIGHT CONCRETE OVER 10 TYPE C 22 GA. METAL DECK DIRECTION - SEE GENERAL NOTES FOR FASTENING SCHEDULE.
[X<X']	INDICATES BOTTOM OF STEEL BEAM ELEVATION ABOVE FINISHED FLOOR.
[X<X'']	INDICATES TOP OF STEEL BEAM ELEVATION ABOVE FINISHED FLOOR.
<+>X'>	INDICATES ELEVATION OF TOP OF BEAM ELEVATION ABOVE J.B.E. TOP OF BEAM TO MATCH TOP OF BAR JOIST AND SLOPE OF ROOF.
C.J.	INDICATES MASONRY CONTROL JOINT. SEE 3/33.2 FOR CONTROL JOINT DETAILS
_LH (*****)	LH(*****) INDICATES LH-SERIES JOINT WITH (DL+RL LOAD / RL LOAD/DOWNWARD WIND LOAD). ALL ADDITIONAL POINT LOADS AND SNOW DRIFT SHOWN ON THE PLANS SHALL BE DESIGNER TO THE LOAD SHOWN BY THE JOIST
	ADDED/SUPPLIER.

KEYNOTES 

- 1 EXHAUST FAN. REFERENCE DETAIL 6/S4.0 FOR TYPICAL EXHAUST FAN SUPPORTS AND ROOF PENETRATION.
- 2 ROOF TOP EQUIPMENT. REFERENCE DETAIL/S4.0 FOR TYPICAL RTU SUPPORT FRAMING DETAIL.
- 3 CONDENSER TO BE SUPPORTED P/ 2. 6x6 CURBS ON TOP OF ROOF DECK SPANNING BETWEEN ROOF JOIST. REFERENCE DETAIL 6/S4.0 FOR TYPICAL CONDENSER SUPPORT FRAMING DETAIL.
- 4 HANGING SIGNAGE BELOW TO BE SUPPORTED FROM ROOF STRUCTURE. APPROXIMATE WEIGHT = 500 LBS. VERIFY WEIGHT WITH DECOR SUBMITTAL. ADVISE ARCHITECT PRIOR TO COMMENCEMENT OF DESIGN OF HANGING SYSTEM FOR GRAVITY AND LATERAL SUPPORT TO BE DESIGNED AND SUPPLIED BY SIGN VENDOR. REFER TO DELEGATED DESIGN NOTES FOR ADDITIONAL INFORMATION. REF. DETAIL 10/S4.0 FOR TYPICAL JOIST WEB REINFORCING.
- 5 REFER TO TYPICAL ROOF DECK OPENING DETAIL, 6/S4.0. REFER TO ARCHT. PLANS FOR ADDITIONAL INFORMATION.
- 6 L3x3x1/16 HORIZONTAL & DIAGONAL, WALL BRACING AT 4'-0" O.C. REFER TO SECTION 5 & 6 ON SHEET S4.0 FOR ADDITIONAL INFORMATION.
- 7 L6x6x7/16 FRAMING. REF. DETAIL 5/S4.0 FOR ADDITIONAL INFORMATION.
- 8 HS54x3x1/16 HORIZONTAL BRACE AT TOP OF COLUMN. REF. 4/S4.0 FOR TYPICAL BRACE TO COLUMN DETAIL.
- 9 PROVIDE TYPICAL JOIST AND UPLIFT BRIDGING PER STEEL JOIST INSTITUTE REQUIREMENTS. OFFSET BRIDGING WHERE REQUIRED AT ROOF TOP MECHANICAL UNITS. SEE SHEET S1.0 FOR UPLIFT LOADS. REFER TO DETAILS FOR TYPICAL JOIST BRIDGING INFORMATION.
- 10 ROOF ANTENNA LOCATION SHOWN IS APPROXIMATE. REF. 13/S4.0 FOR TYPICAL ANTENNA SUPPORT. REFER TO ARCHT. PLANS FOR ADDITIONAL INFORMATION.

[illegible]

7-ELEVEN, INC.
3200 HACKBERRY ROAD, IRVING TEXAS 75063

7-ELEVEN #1055406
NORTHLAKE & COCONUT (COCONUT CROSSING)
PALM BEACH GARDENS, FL

ROOF FRAMING PLAN



Drawings and Specifications as Instruments of service are and shall remain the property of the Architect. They are not to be used on other projects or extensions to this project except by agreement in writing and with appropriate compensation to the Architect.

Contractor is responsible for confirming and correlating dimensions at job site; the Architect will not be responsible for construction means, methods, techniques, sequences or procedures, or for safety precautions and programs in connection with the project.

The seal and signature apply only to the document to which they are affixed and we expressly disclaim any responsibility for all other plans, specifications, estimates, reports or other documents or instruments relating to or intended to be used for any part or parts of the project.

Florida Certificate of Authority #FC0200001956

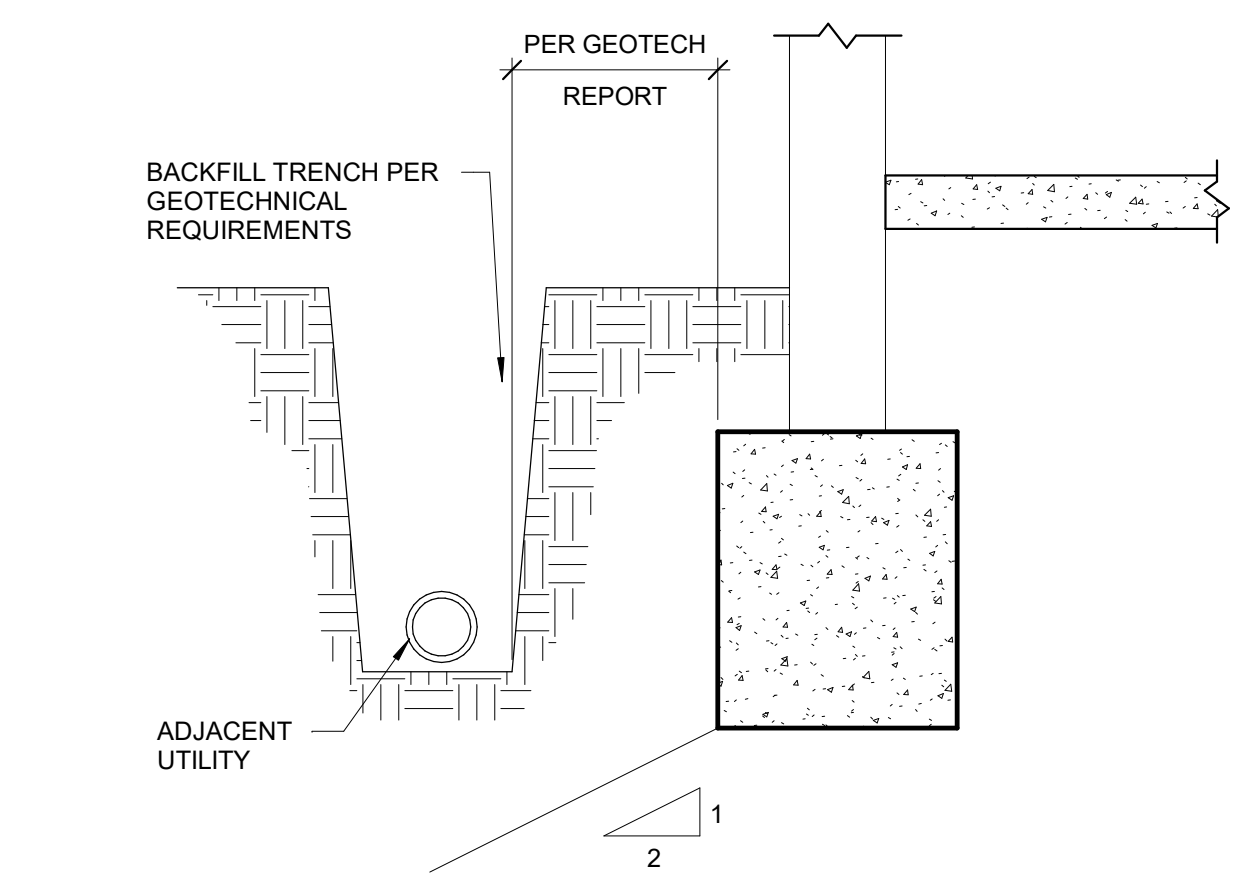
Florida Certificate of Authority #F02000001956

JOB#:	25023
DATE:	08/14/2025

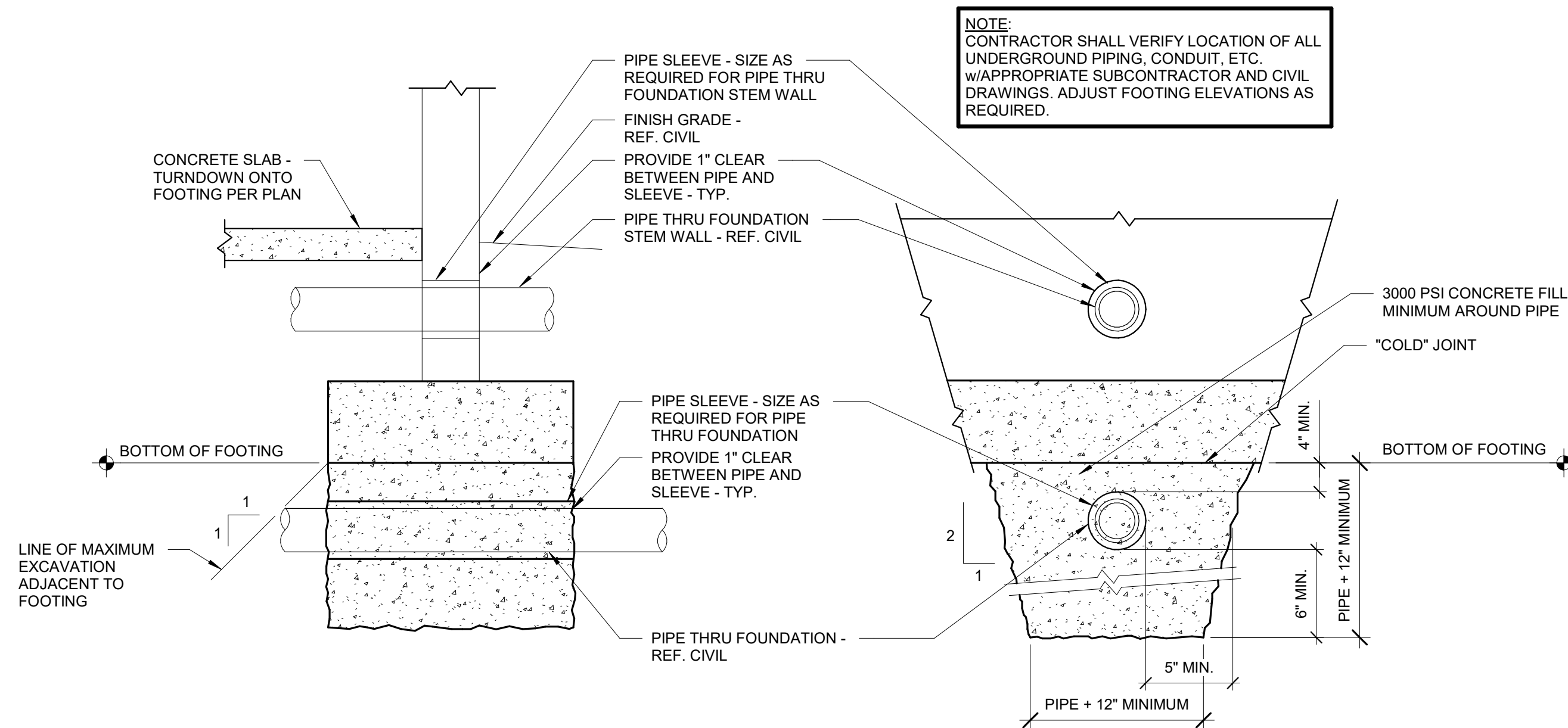
SHEET: **S2.1**



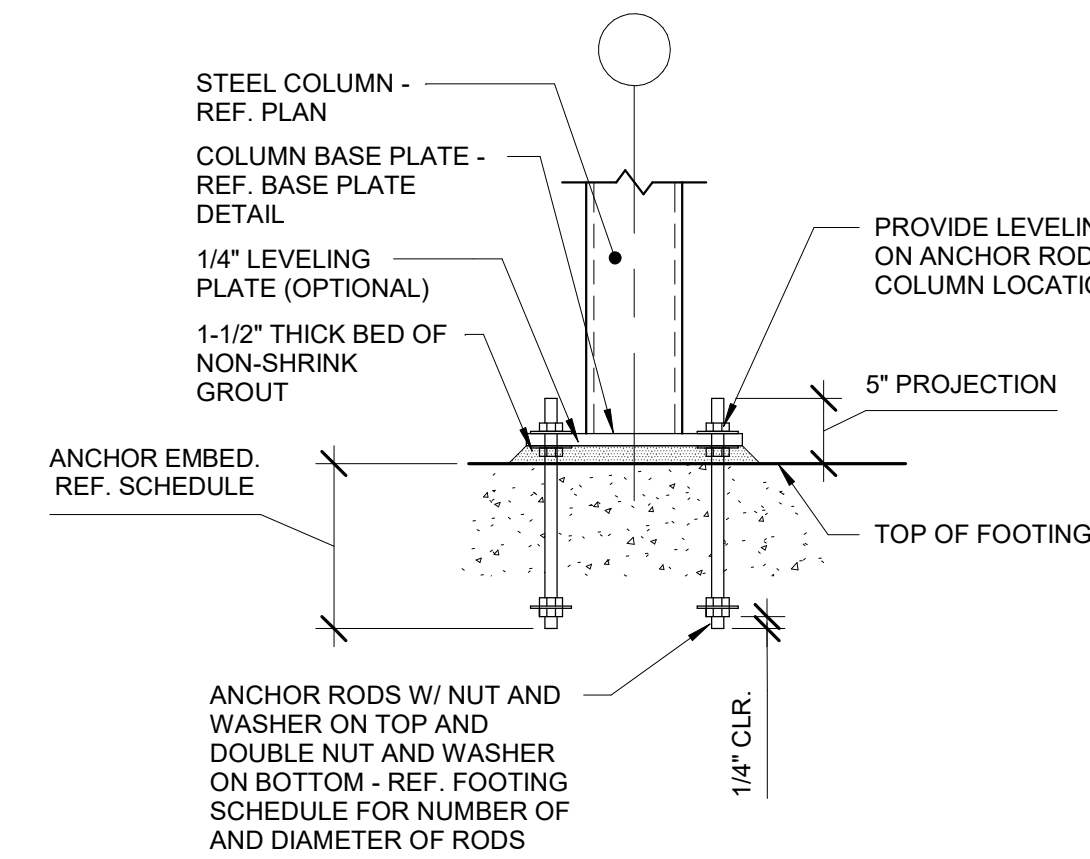
JCAA
4100 Wadsworth Blvd.
Wheat Ridge, CO 80033
p 303.985.3260
FL Firm No.29586



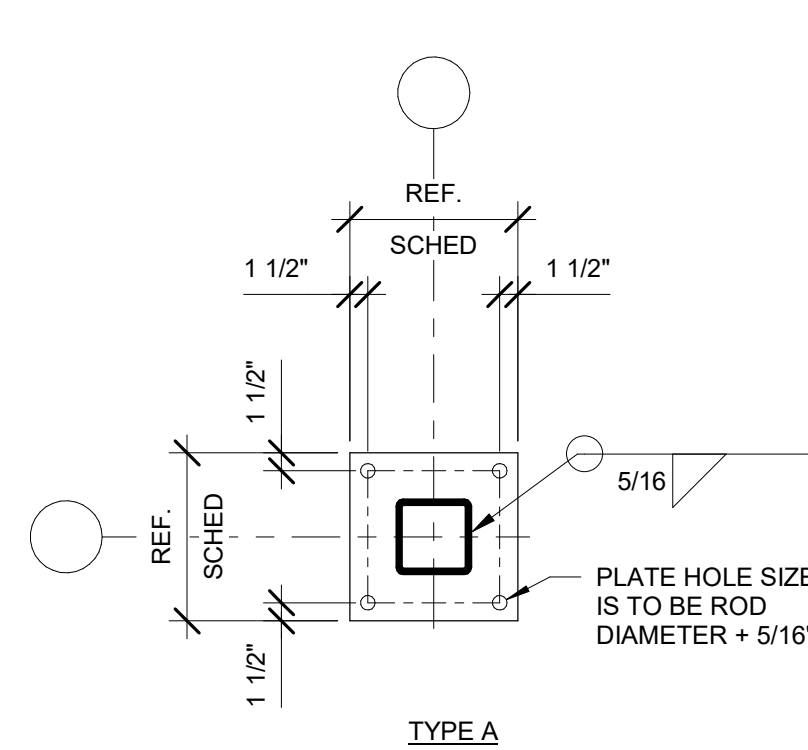
8 TYPICAL FOOTING ADJACENT TO UTILITY
3/4" = 1'-0"



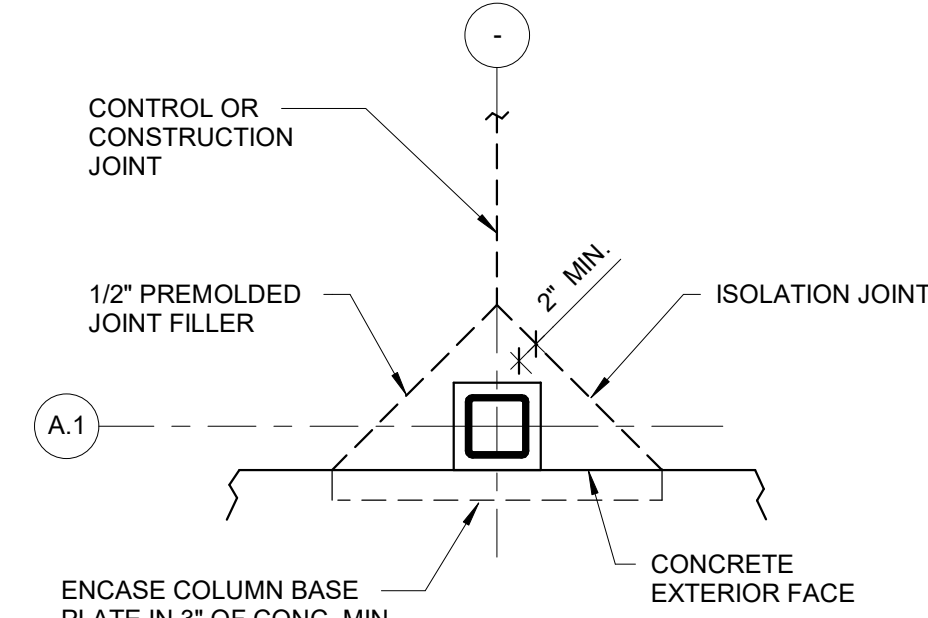
7 TYPICAL PIPE AT FOUNDATION / WALL DETAIL
3/4" = 1'-0"



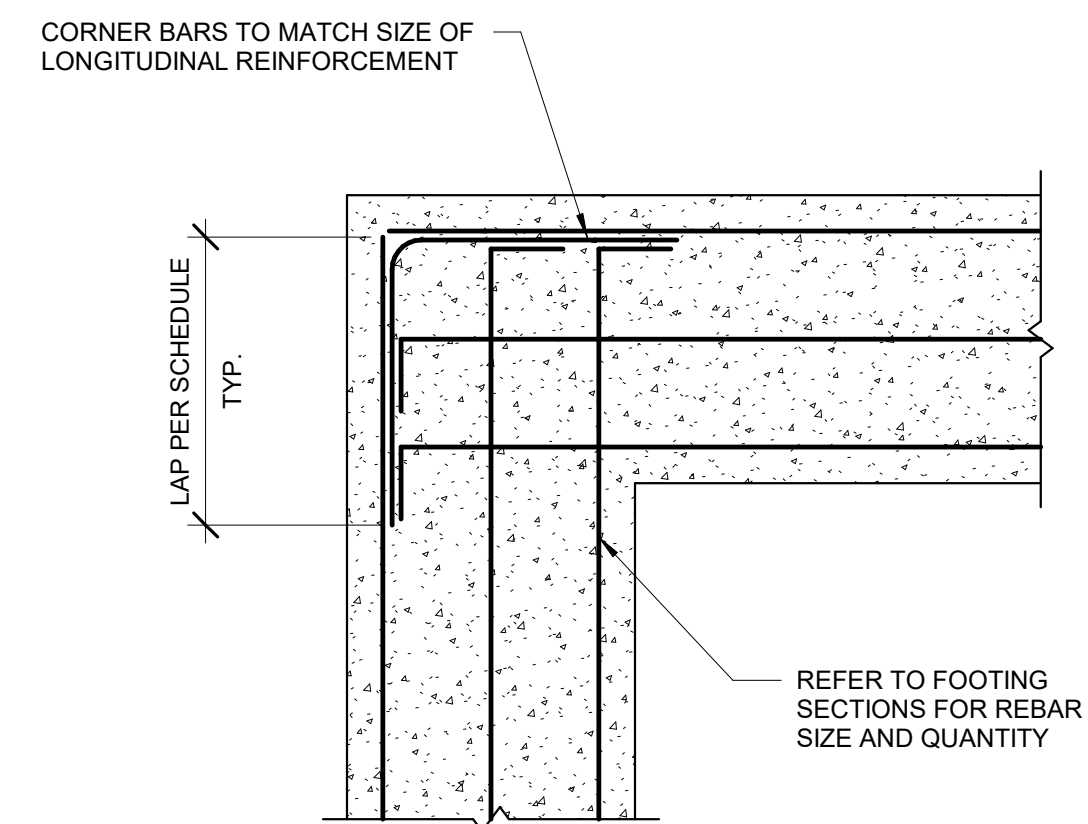
4 TYPICAL BASE PLATE DETAILS



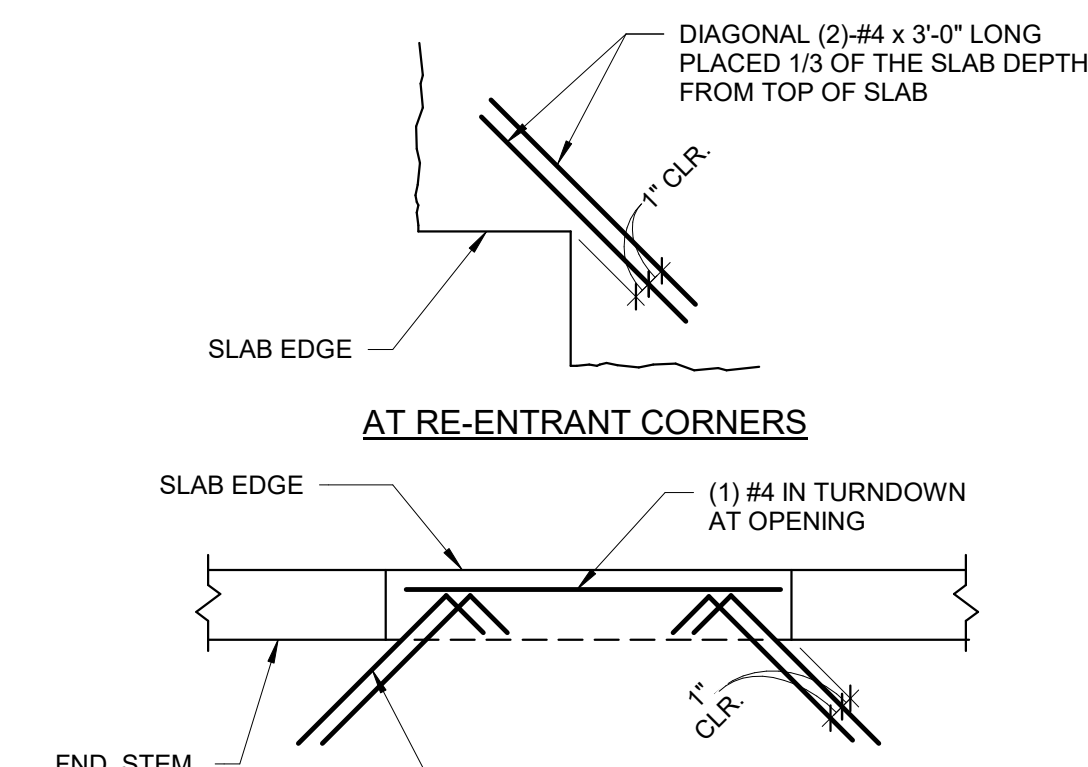
TYPE A



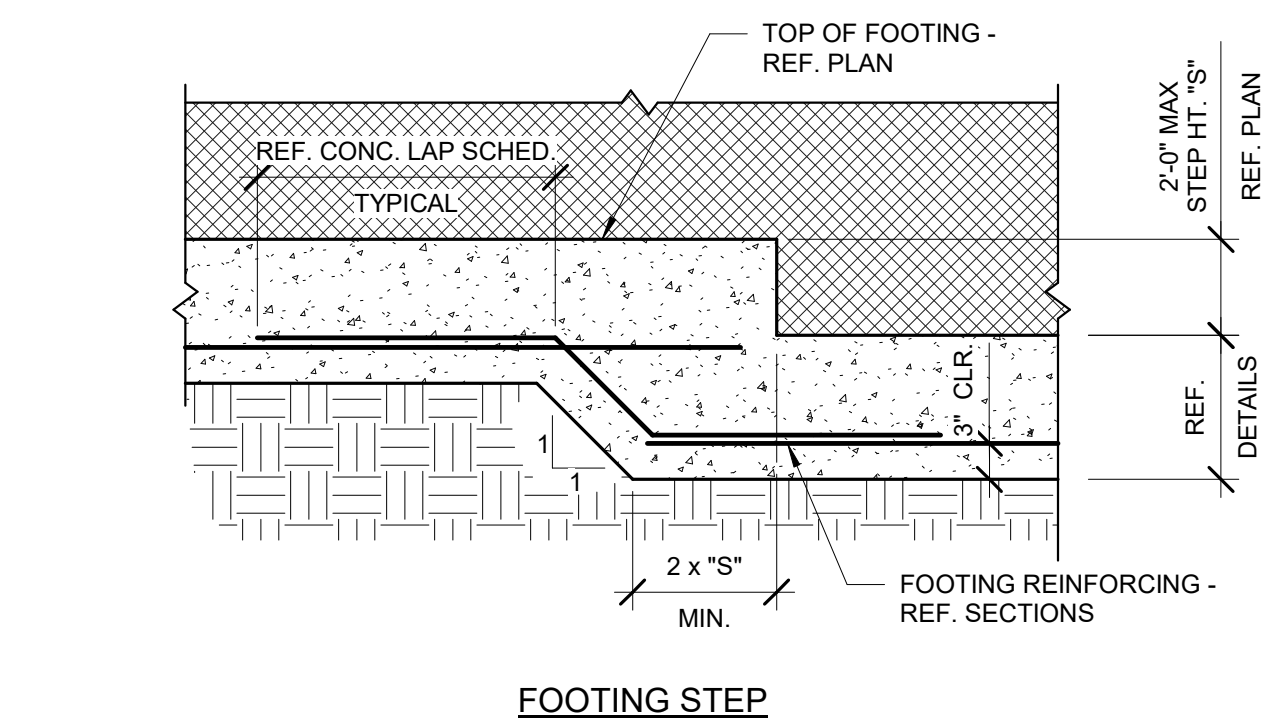
3 TYP. STEEL COLUMN ISOLATION JOINT
3/4" = 1'-0"



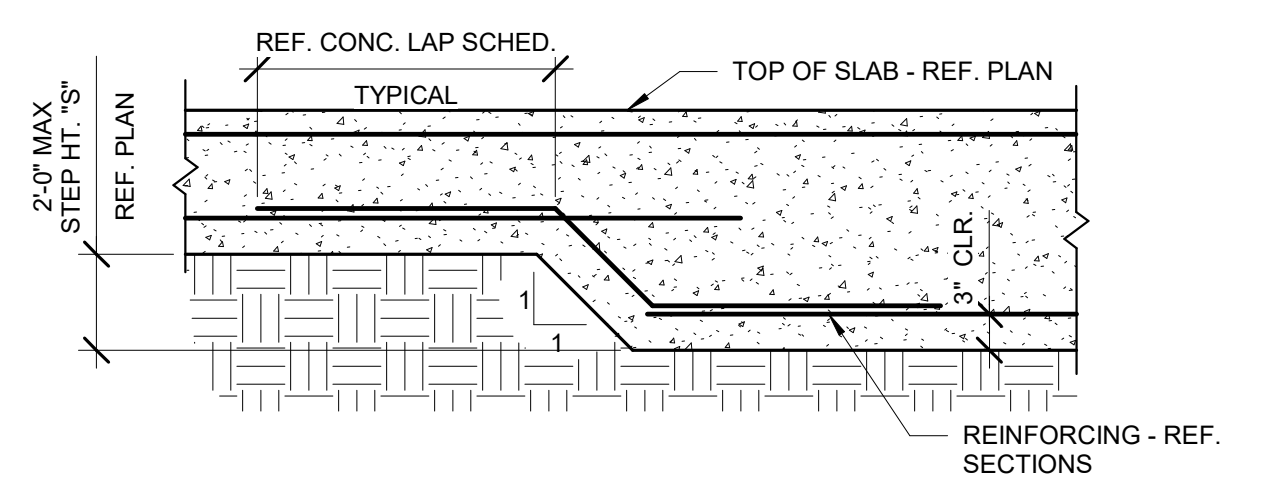
6 TYPICAL FOOTING CORNER DETAIL
3/4" = 1'-0"



2 TYP. SLAB CORNER REINFORCING
3/4" = 1'-0"

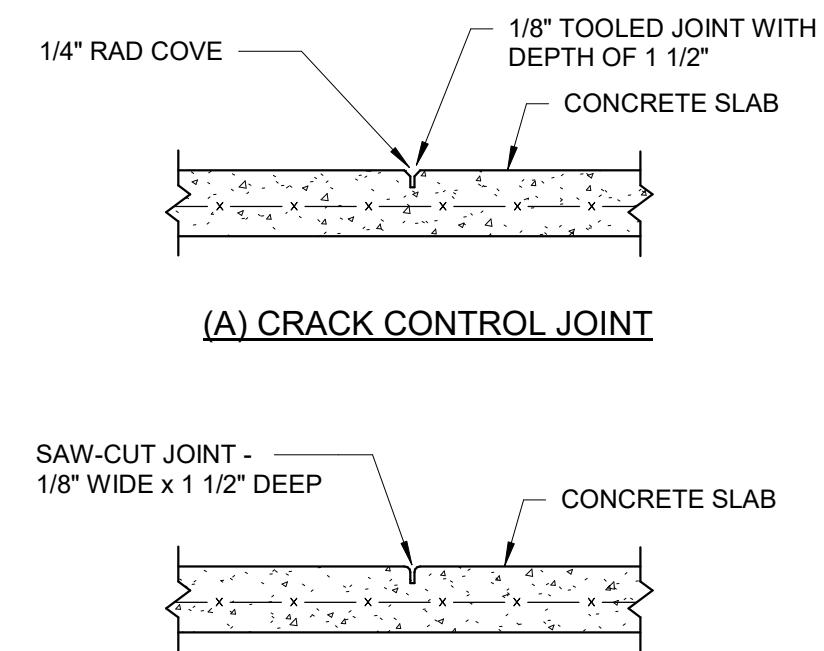


FOOTING STEP



TURN DOWN STEP

5 TYPICAL FOOTING AND TURN DOWN STEP DETAIL
3/4" = 1'-0"



(B) CRACK CONTROL JOINT

NOTE:
THE SLAB SHALL BE SAW-CUT AS SOON AS THE CONCRETE HAS
HARDENED SUFFICIENTLY TO WITHSTAND CHIPPING, SPALLING,
OR TEARING (WITHIN 8 HOURS)

1 CONTROL JOINT DETAILS

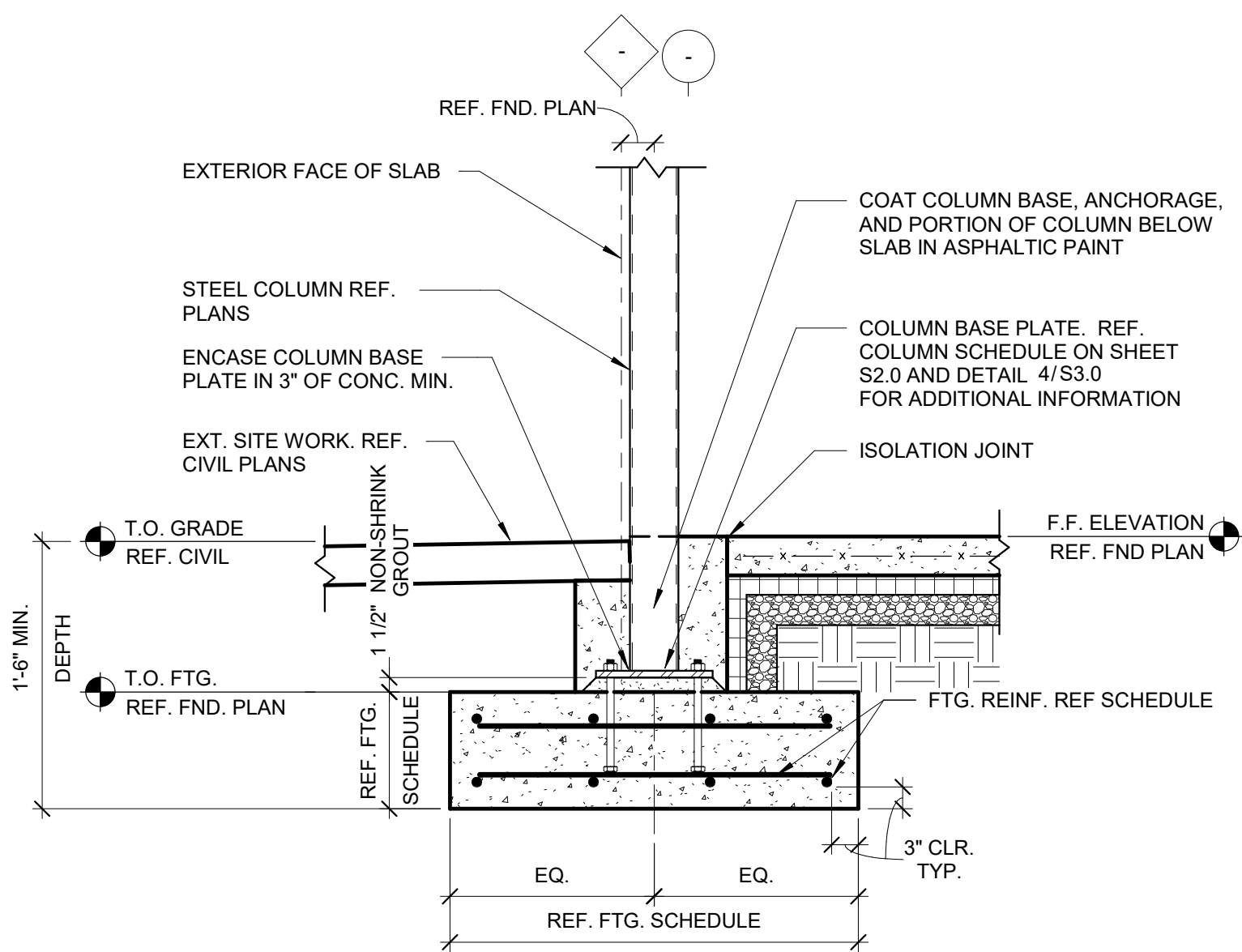


JCAA
4100 Wadsworth Blvd.
Wheat Ridge, CO 80033
p 303.985.3260
FL Firm No.29586

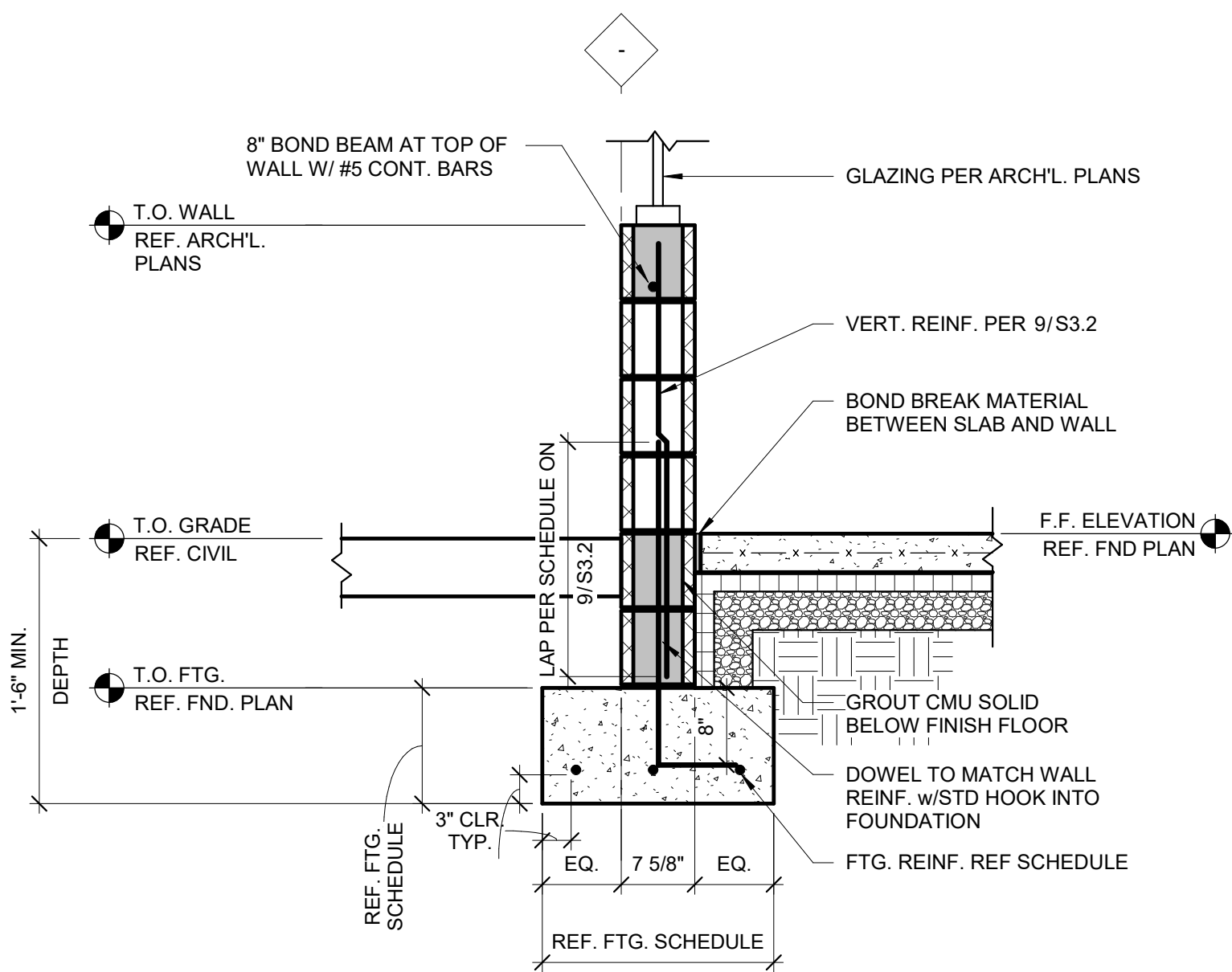
JOB#:	25023
DATE:	08/14/2025

S3.0

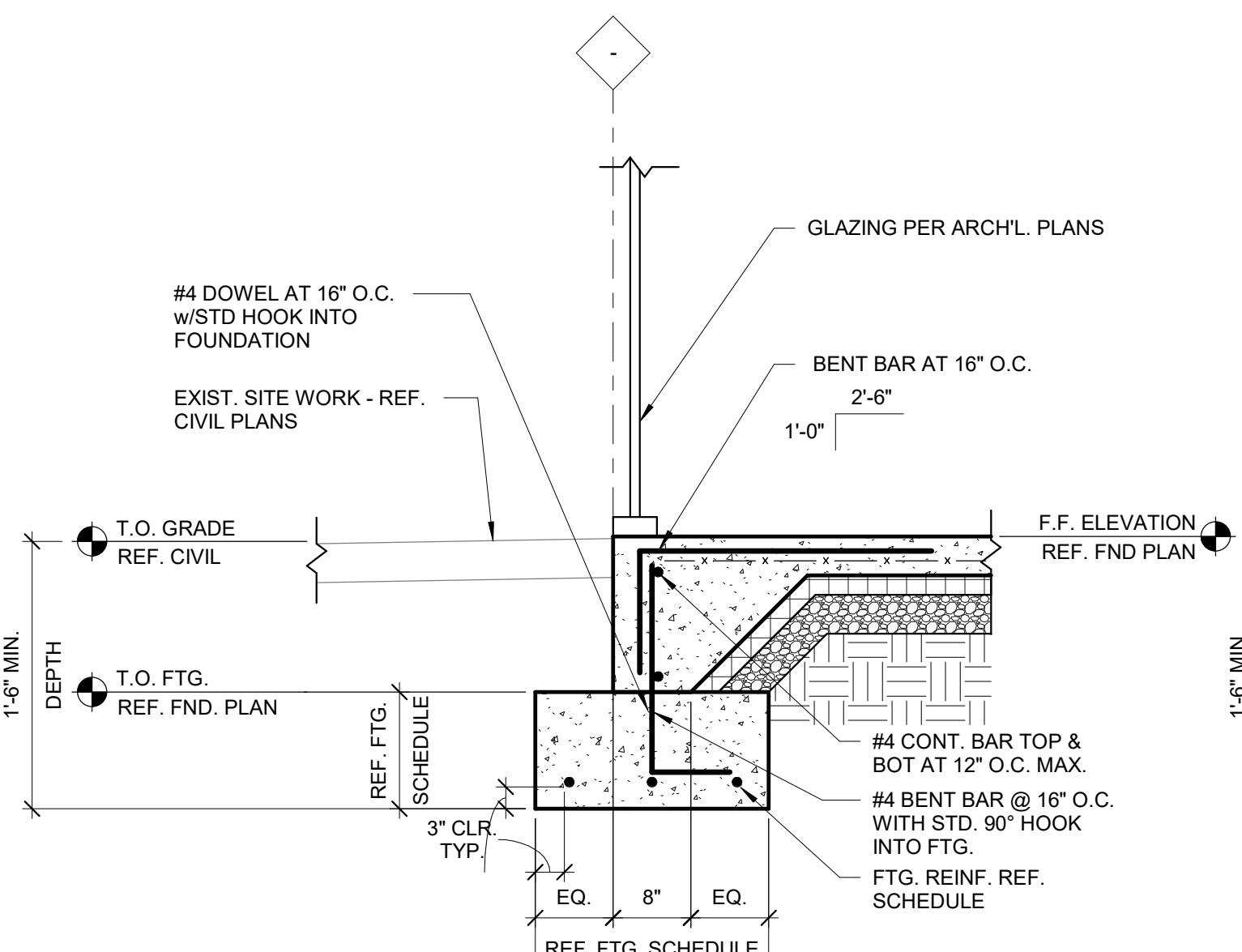
4 TYP. FOUNDATION SECTION AT COLUMN
3/4" = 1'-0"



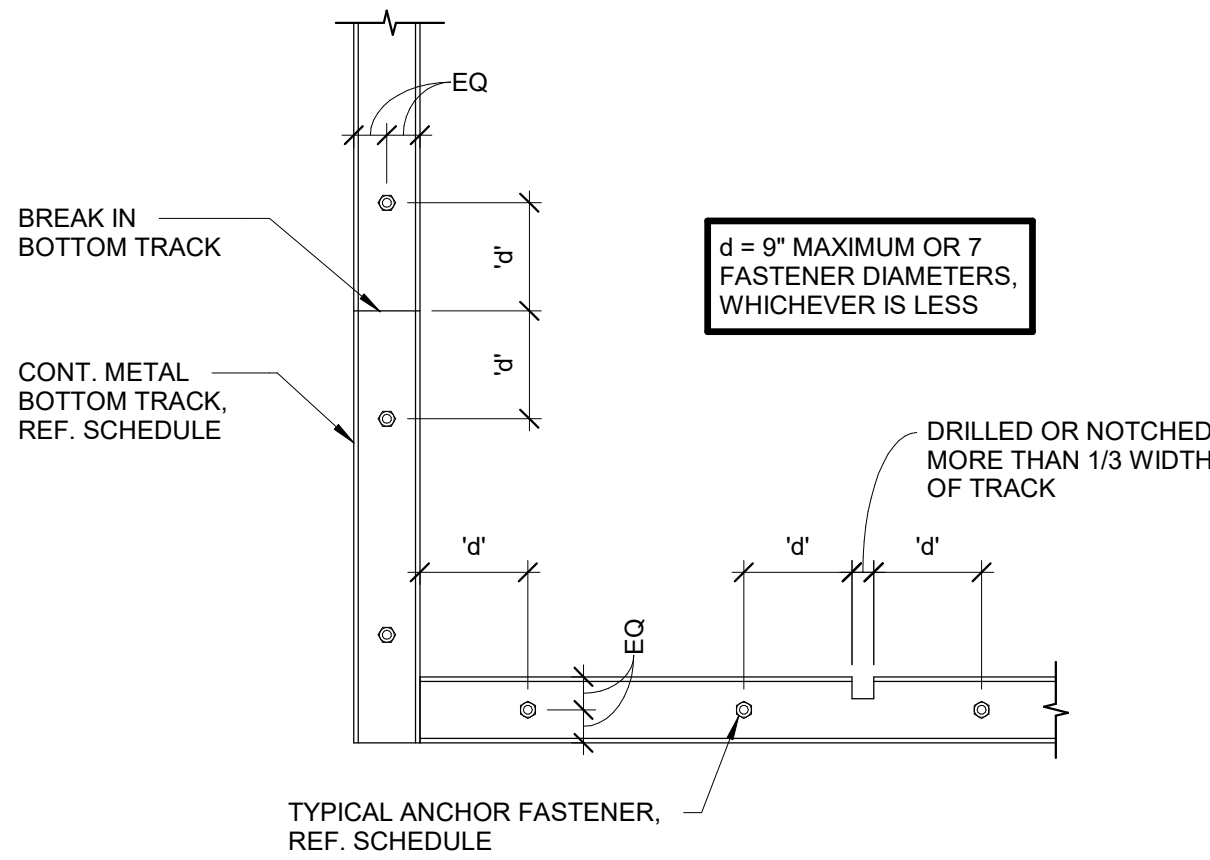
8 TYP. FOUNDATION SECTION
3/4" = 1'-0"



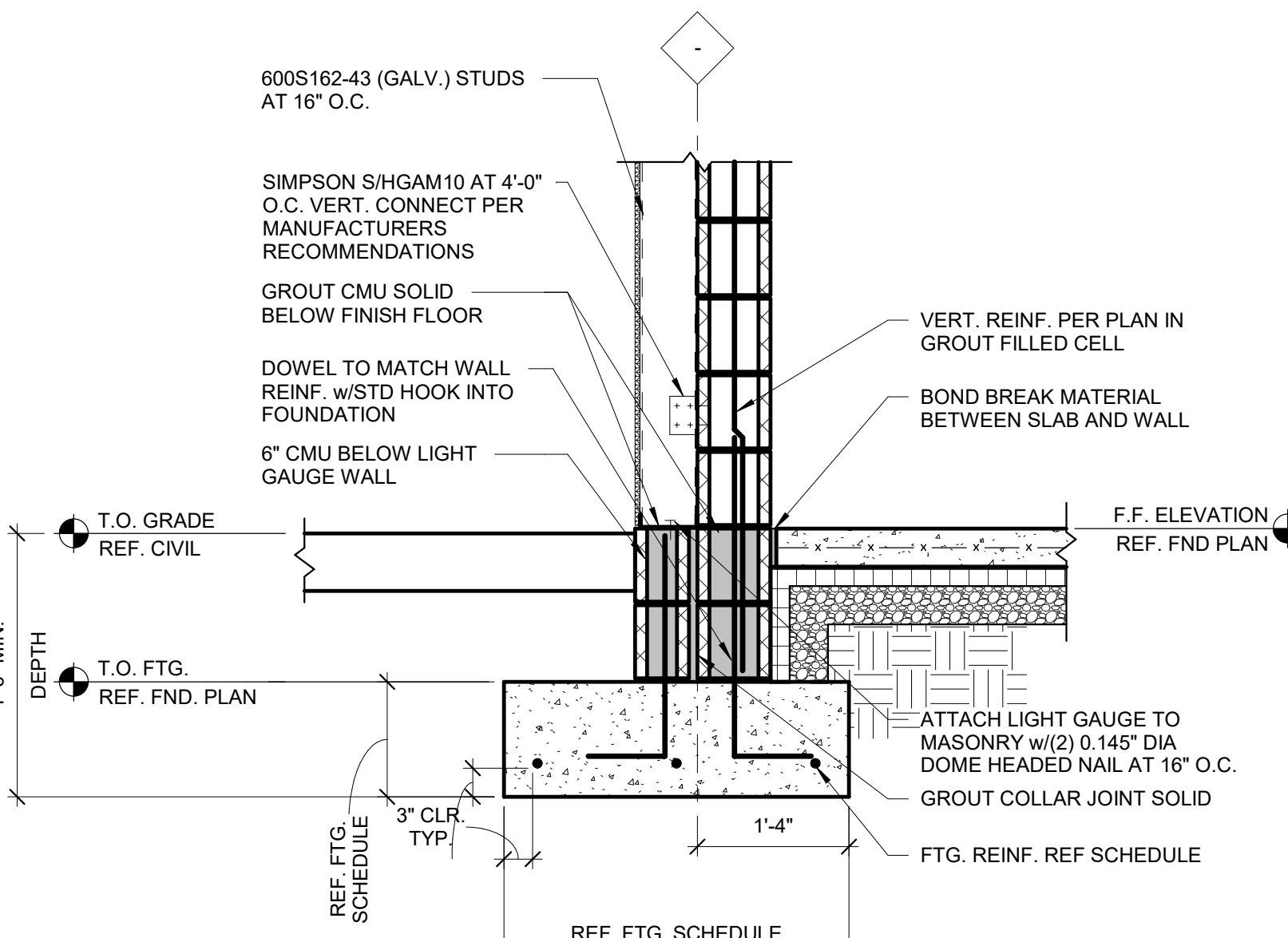
3 TYP. FND SECTION AT ENTRY/STORE FRONT
3/4" = 1'-0"



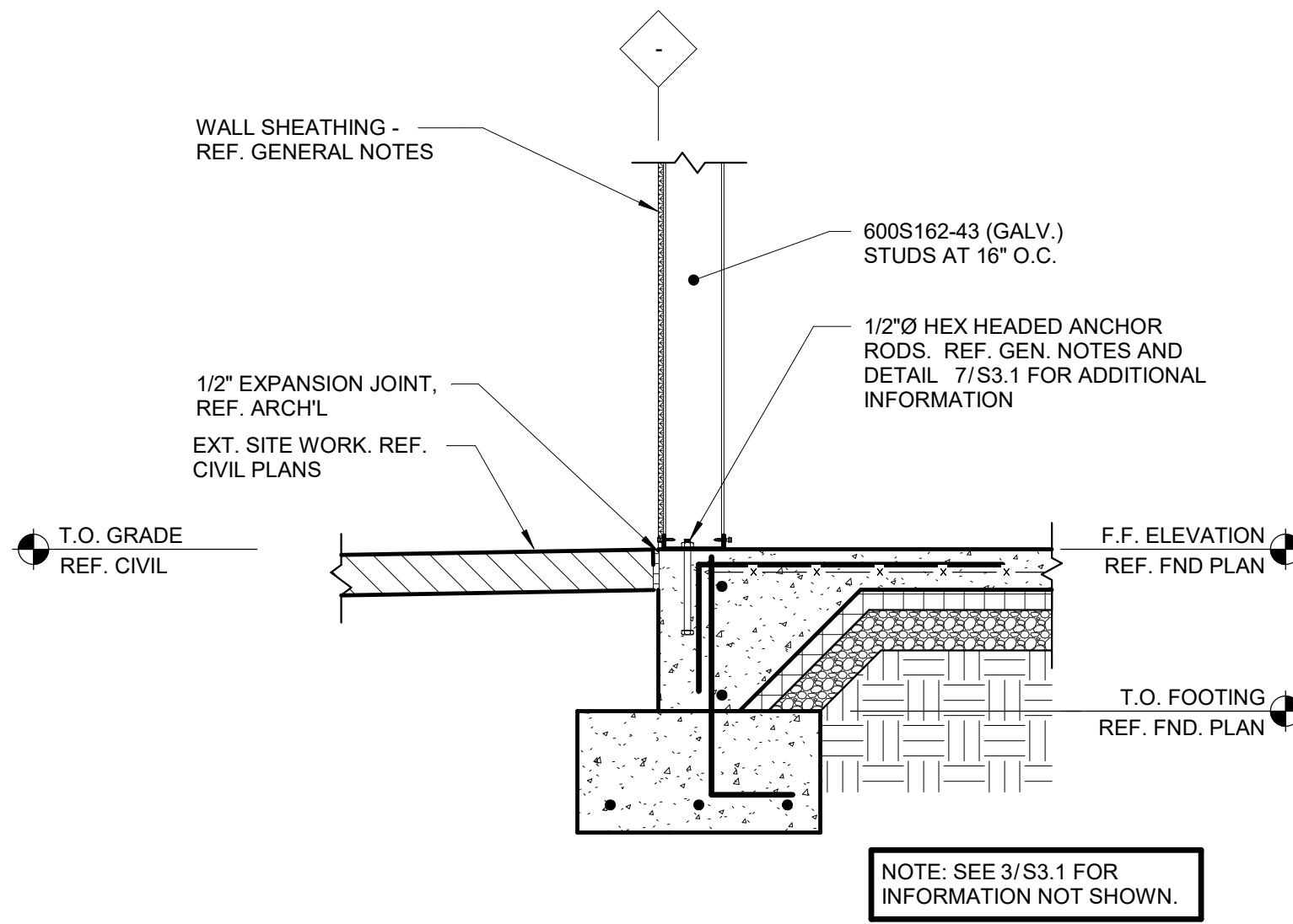
7 BOTTOM TRACK FASTENER DETAIL
3/4" = 1'-0"



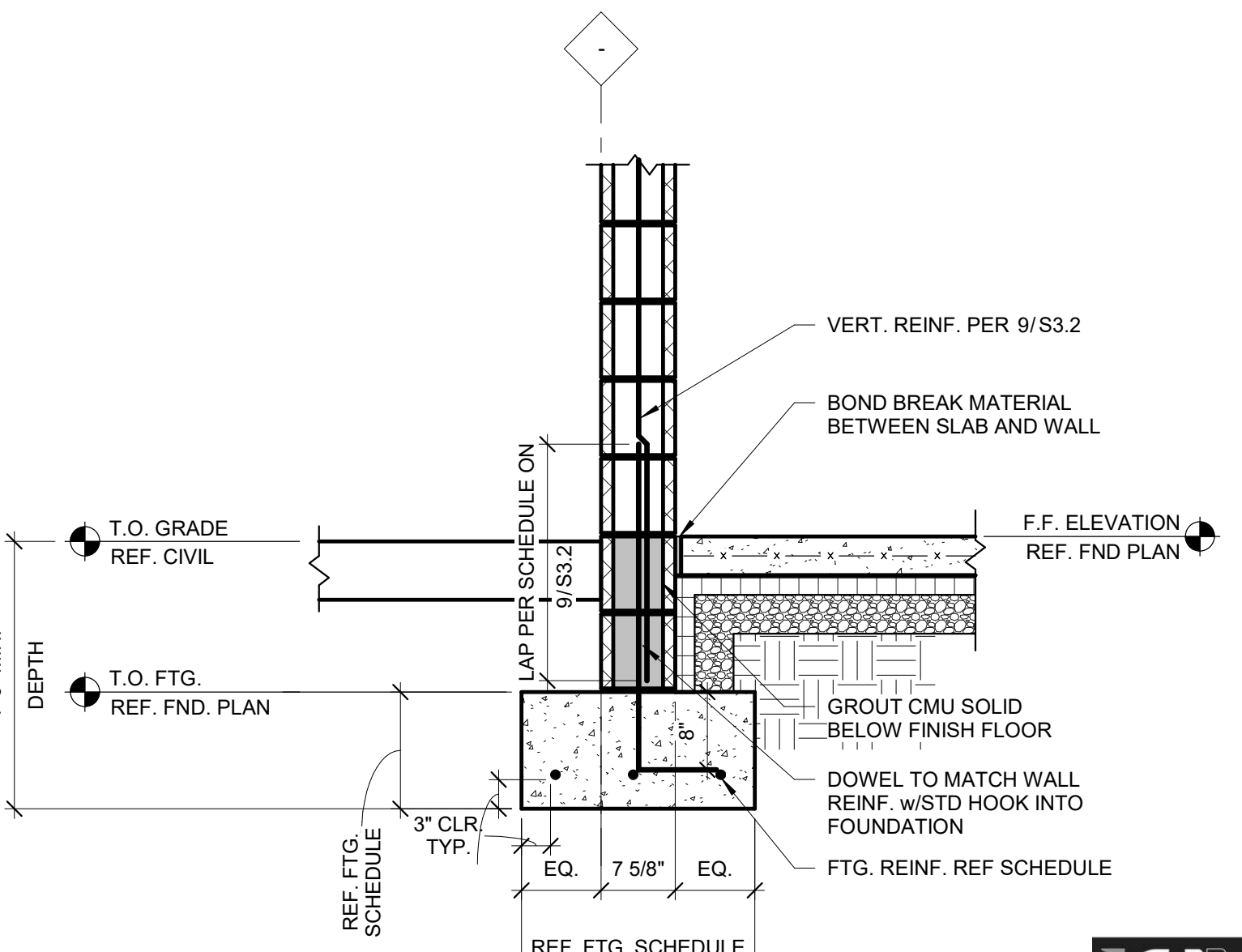
2 FOUNDATION SECTION AT BUMPOUT
3/4" = 1'-0"



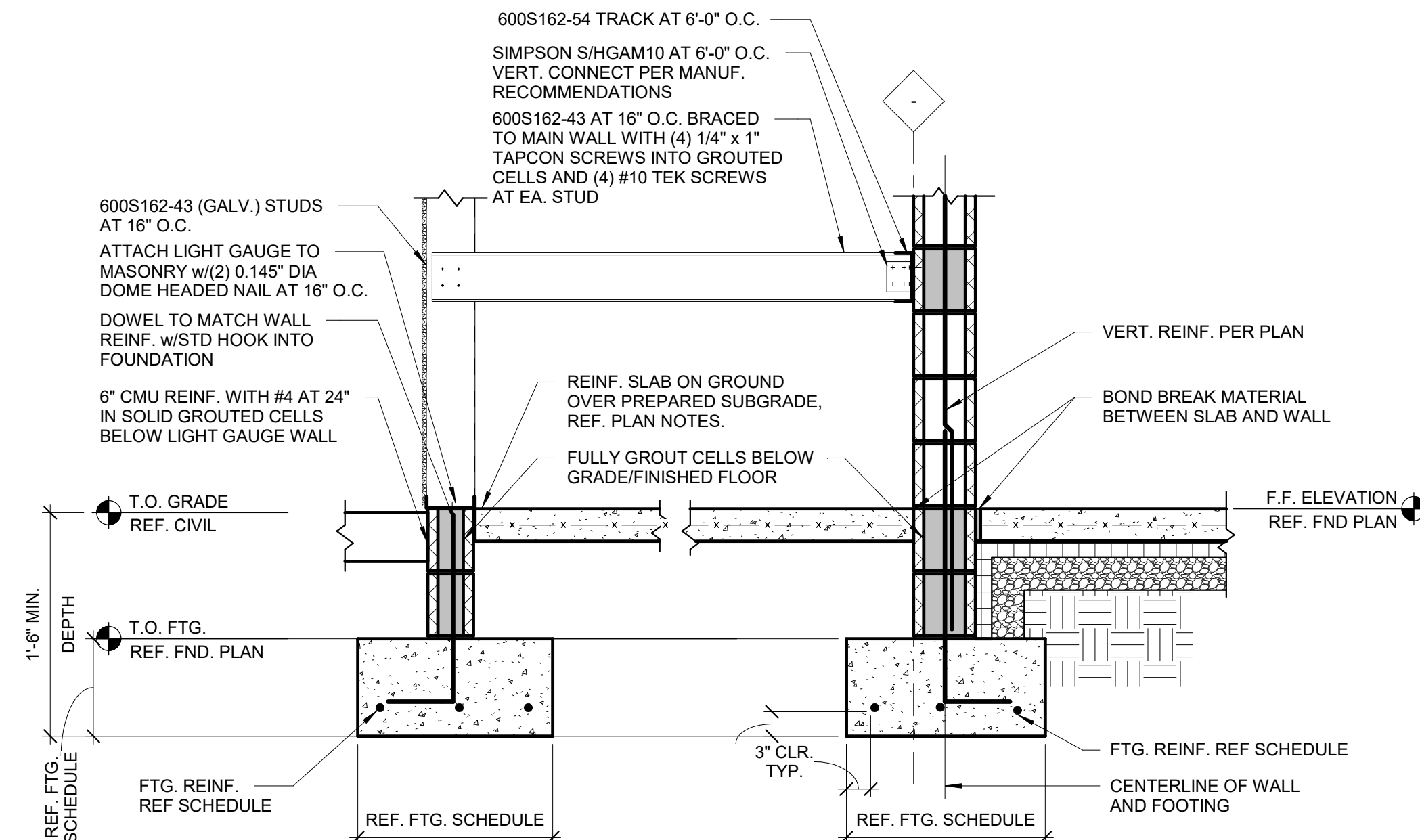
6 FOUNDATION SECTION AT WALL
3/4" = 1'-0"



5 TYP. FOUNDATION SECTION AT ENTRY
3/4" = 1'-0"



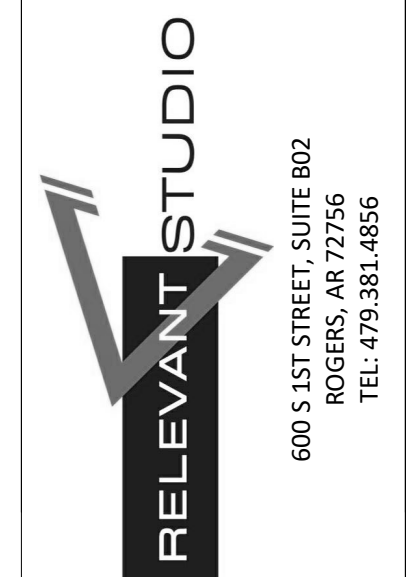
9 FOUNDATION SECTION AT BUMPOUT
3/4" = 1'-0"



REV #	DATE	DESCRIPTION

7-ELEVEN INC
3200 HACKBERRY ROAD, IRVING TEXAS 75063
7-ELEVEN #1055406
NORHLAKE & COCONUT (COCONUT CROSSING)
PALM BEACH GARDENS, FL

FOUNDATION SECTIONS AND DETAILS



Drawings and Specifications as instruments of service are and shall remain the property of the Architect. They are not to be used on other projects or extensions to this project except by agreement in writing and with appropriate compensation to the Architect.

The seal and signature apply only to the document to which they are affixed and we agree to disclaim any responsibility for all other plans, specifications, estimates, reports or other documents or instruments relating to or intended to be used for any part or parts of the project.

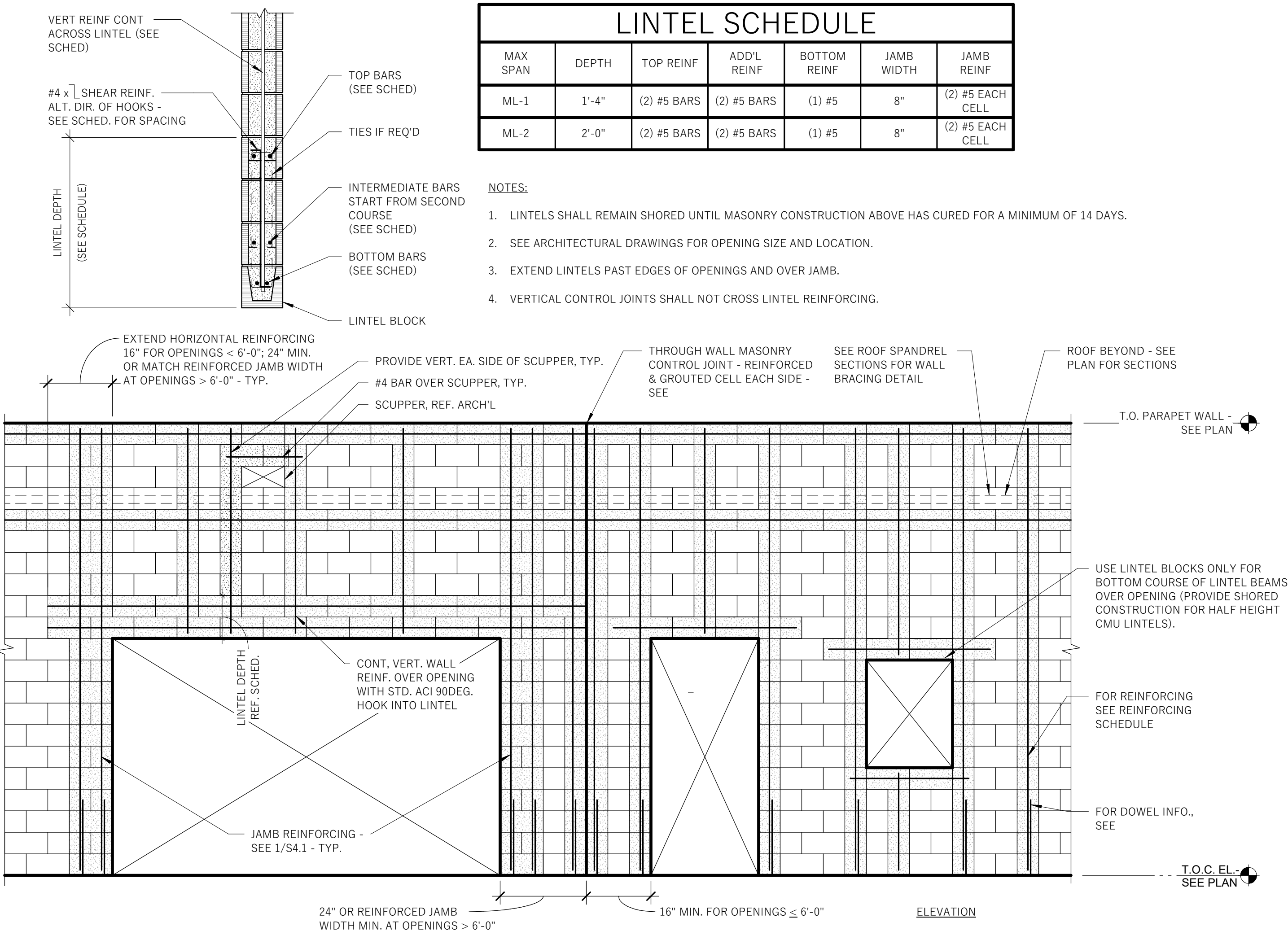
Florida Certificate of Authority #FC0200001956

JOB#: 25023
DATE: 08/14/2025
SHEET: 425 OF 425



JCAA
4100 Wadsworth Blvd.
Wheat Ridge, CO 80033
p 303.985.3260
FL Firm No. 29586

S3.1

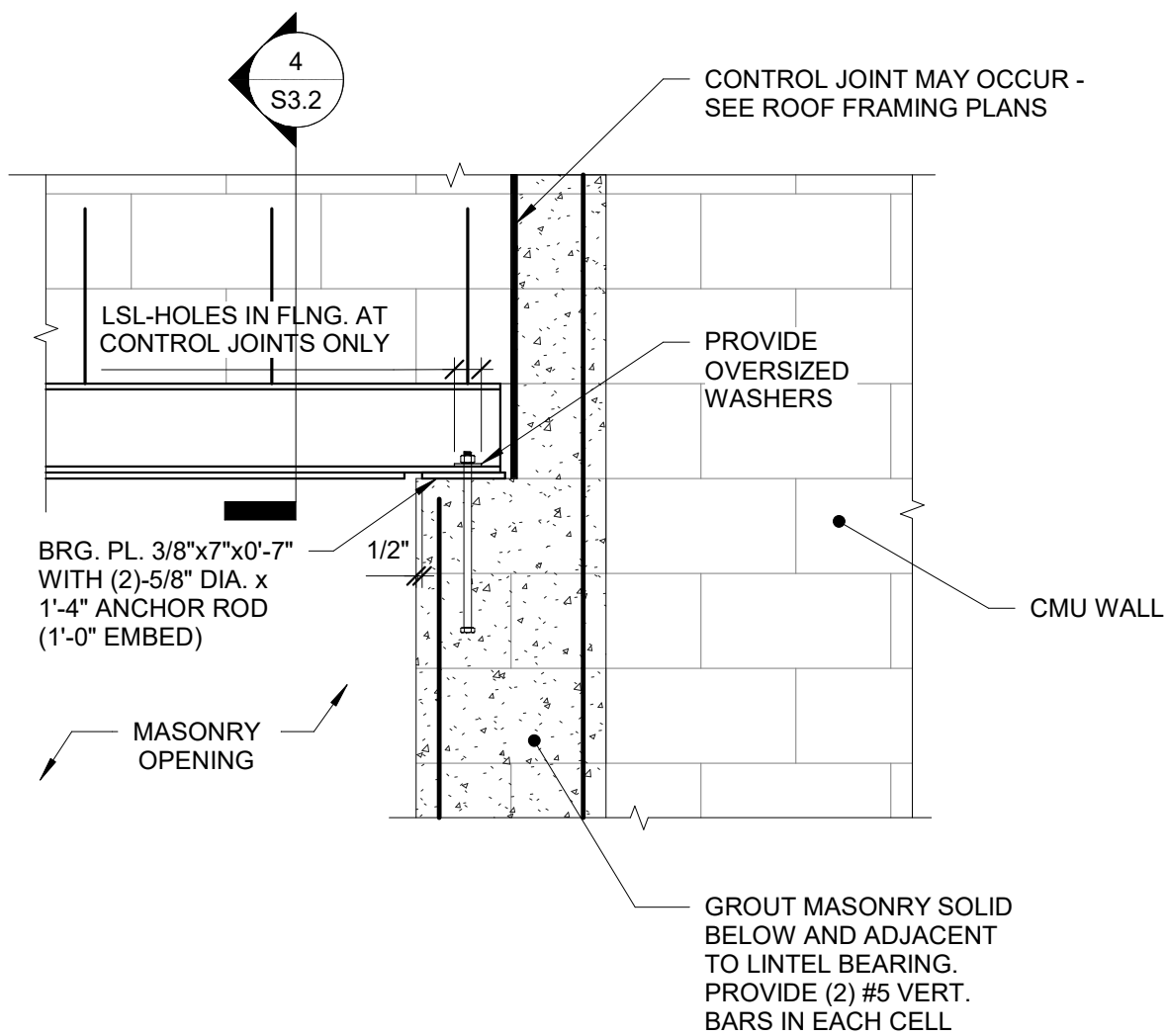


11 TYPICAL EXTERIOR CMU WALL REINFORCING DETAIL
3/4" = 1'-0"

- MASONRY WALL REINFORCING, TYP.**
- ALL MASONRY WALLS ARE USED AS SPECIAL REINFORCED MASONRY SHEAR WALLS AND SHALL BE DETAILED AS SUCH.
 - REINFORCING SHOWN IS THE MINIMUM REQUIRED. SEE PLANS, DETAILS, AND NOTES FOR ADDITIONAL WALL REINFORCING REQUIRED.
- 8" CMU PERIMETER WALLS VERT. REINFORCING:
TOP OF WALL ELEV. ≤ 21'-4": (1) #5 VERT. AT 24" O.C.
TOP OF WALL ELEV. ≤ 26'-8": (2) #6 VERT. AT 16" O.C.
- SEE PLANS, DETAILS, AND NOTES FOR ADDITIONAL REINFORCING REQUIRED.
- REINFORCING SHALL BE CENTERED IN CMU WALL U.N.O. WHERE MULTIPLE ROWS OF REINFORCING IS REQUIRED, PROVIDE 2-1/2" COVER DISTANCE FROM EACH FACE OF CMU. PROVIDE (2) VERTICAL BARS AT ALL CORNERS, INTERSECTIONS AND EACH SIDE OF ALL MASONRY OPENINGS & CONTROL JOINTS. REINFORCING SIZE TO MATCH ADJACENT REINFORCING. SEE DETAIL 1/S3.1. PROVIDE DOWEL INTO FOOTING OF SAME SIZE, SPACING AND LOCATION AS VERTICAL REINFORCING. OMIT DOWELS AT DOORS.
 - WHERE ROOF SLOPES, STEP BOND BEAM REINFORCING TO FOLLOW ROOF LINE. PROVIDE CORNER BARS IN ALL BOND BEAMS AT ALL CORNERS AND INTERSECTIONS. LAP SPlice BOND BEAM REINFORCING PER SCHEDULE ABOVE. REFER TO DETAIL 6/S3.2 & 7/S3.2 FOR TYPICAL WALL REINFORCING AT WALL CORNER & INTERSECTIONS.
 - WALLS SHALL BE GROUTED AT ALL CELLS CONTAINING HORIZONTAL AND VERTICAL REINFORCING. ALL BOND BEAMS, ALL CELLS BELOW FINISHED FLOOR OR GRADE. ALL CELLS AT PARAPET ABOVE ROOF LINE. ALL CELLS TO RECEIVE ANCHOR BOLTS OR EMBEDDED PLATES, AND ALL OTHER LOCATIONS NOTED ON THE DRAWINGS TO BE GROUTED SOLID.
 - BOND BEAM REINFORCING AT ROOF LINE AS NOTED BELOW SHALL BE CONTINUOUS THROUGH CONTROL JOINTS. SEE DETAIL 3/S3.2 FOR BOND BEAM
- BOND BEAM LOCATION AND REINFORCING SHALL BE AS FOLLOWS:
- A. (2) #5 IN 16" DEEP BOND BEAM AT ROOF LINE.
B. (2) #5 IN 8" DEEP BOND BEAM AT TOP OF PARAPET WALL & TOP OF WALL.
C. ADDITIONAL BOND BEAMS AS SHOWN IN WALL DETAILS AND AS NOTED IN NOTE 4.
- PROVIDE VERTICAL CONTROL JOINTS IN WALL PER PLANS. SEE DETAIL 3/S3.2 FOR WALL CONTROL JOINT DETAIL.

8" CMU LAP SCHEDULE		
	(1)-CURTAIN REINF. CENTERED	(2)-CURTAINS OF REINF.
#4	21"	34"
#5	26"	53"
#6	40"	99"

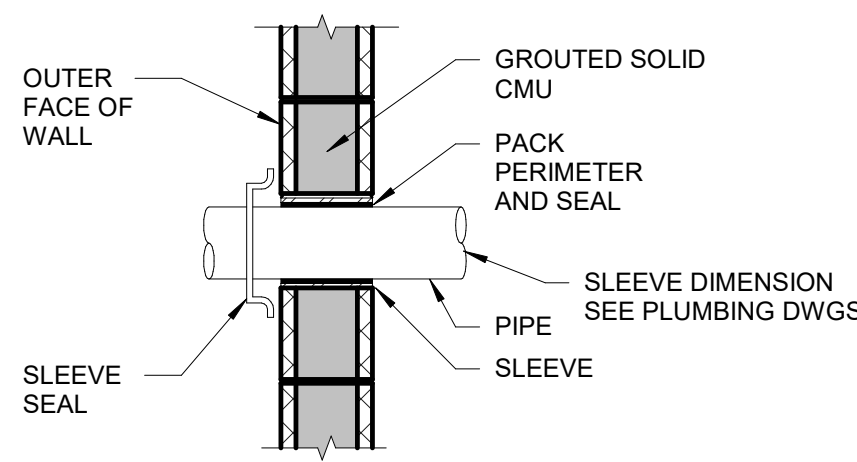
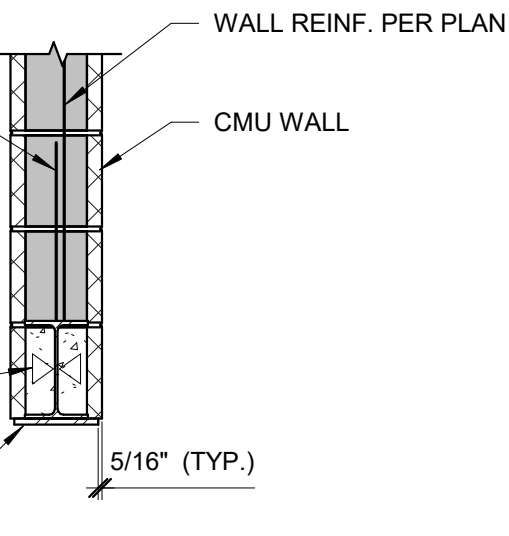
9 TYP. MASONRY WALL REINF. SECTION & NOTES
1/2" = 1'-0"



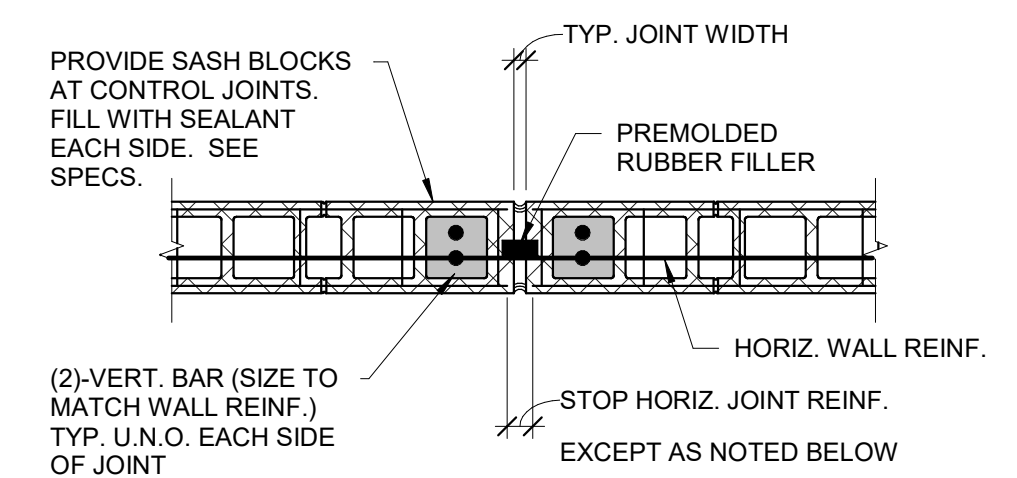
5 TYP. STEEL BEAM BEARING LINTEL
3/4" = 1'-0"



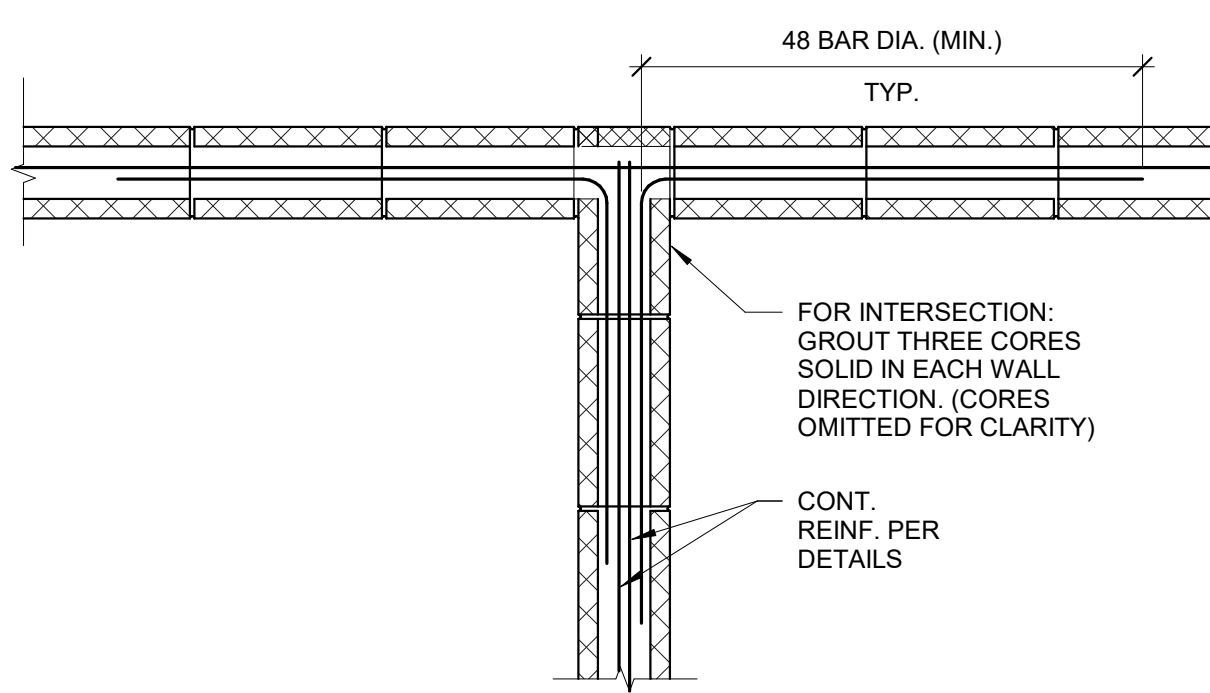
4 SECTION
3/4" = 1'-0"



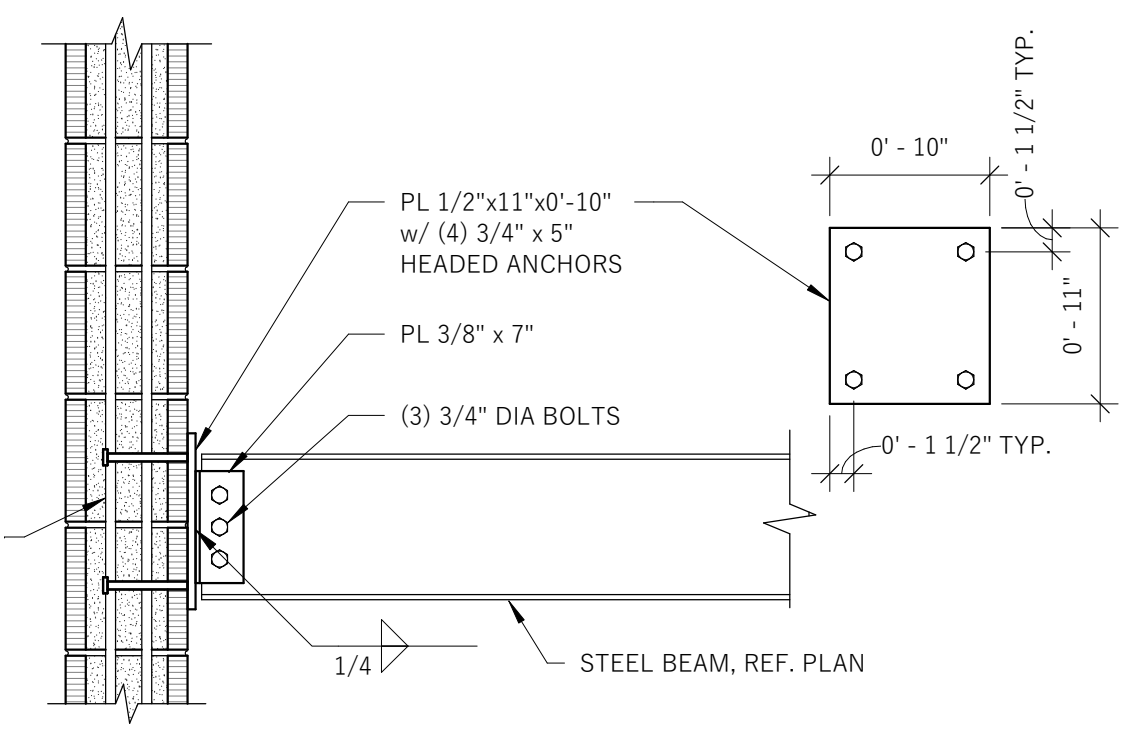
8 TYP. SLEEVE DETAIL
3/4" = 1'-0"



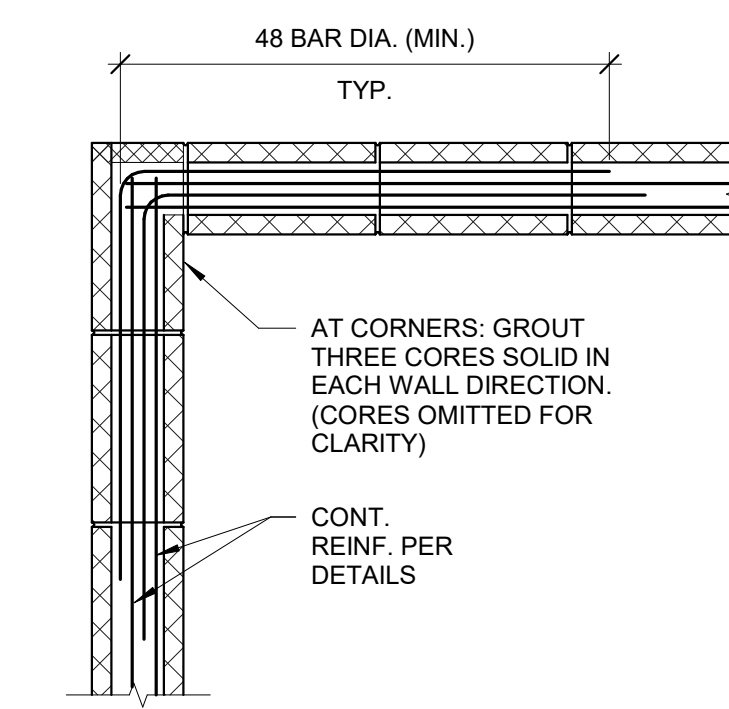
3 TYP. MASONRY CONTROL JOINT DETAIL
3/4" = 1'-0"



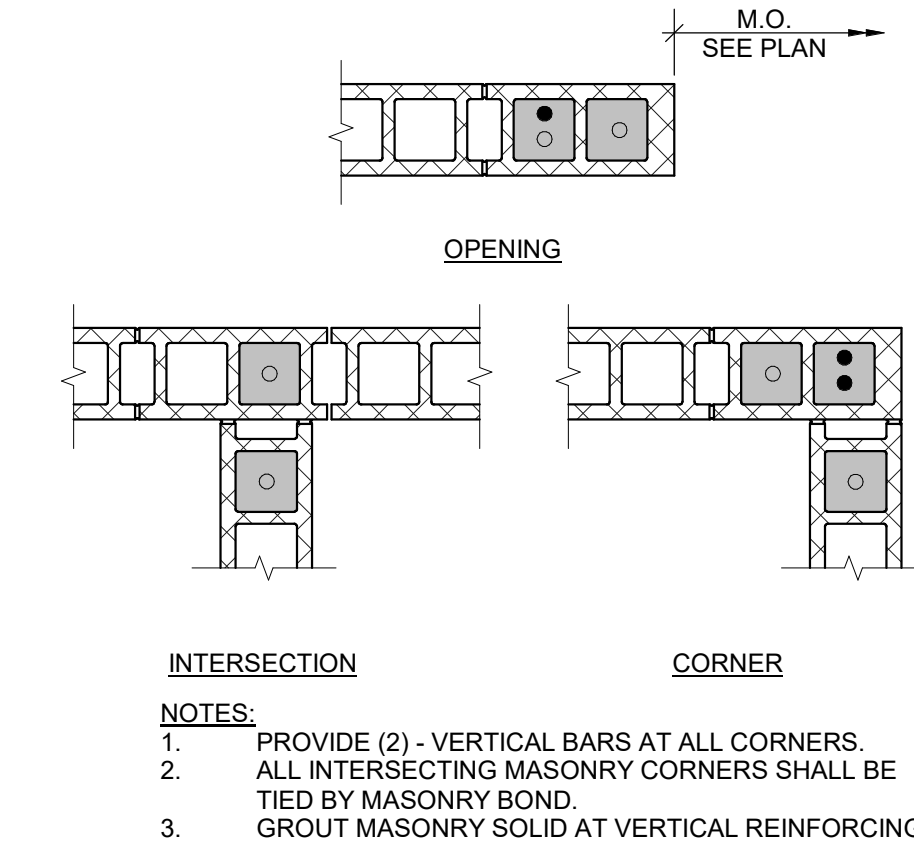
7 TYP. MASONRY BOND BEAM AT INTERSECTION
3/4" = 1'-0"



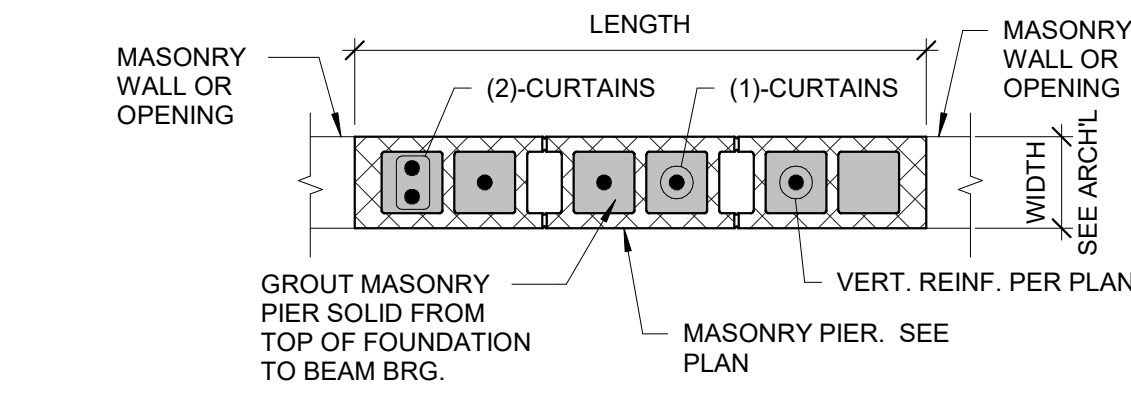
10 W BEAM TO CMU CONNECTION
1" = 1'-0"



6 TYP. MASONRY BOND BEAM AT CORNERS
3/4" = 1'-0"



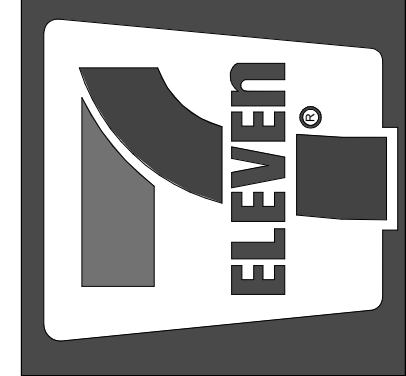
1 TYP. MASONRY REINFORCEMENT DETAILS
3/4" = 1'-0"



2 TYP. MASONRY PIER DETAIL
3/4" = 1'-0"

REV #	DATE	DESCRIPTION

7-ELEVEN INC
3200 HACKBERRY ROAD, IRVING TEXAS 75063
7-ELEVEN #1055406
NORTHLAKE & COCONUT (COCONUT CROSSING)
PALM BEACH GARDENS, FL
TYP. CMU DETAILS



RELEVANT STUDIO
600 11ST STREET, SUITE B02
BOCA RATON, FL 33433
TEL: 475-393-4055

Drawings and Specifications as instruments of service are not, shall remain the property of the Architect. They are not to be used on other projects or extensions to this project except by agreement in writing and with appropriate compensation to the Architect.

Contractor is responsible for confirming and correlating dimensions at job site; the Architect will not be responsible for construction means, methods, techniques, sequences or procedures, or for safety precautions and programs in connection with the project.

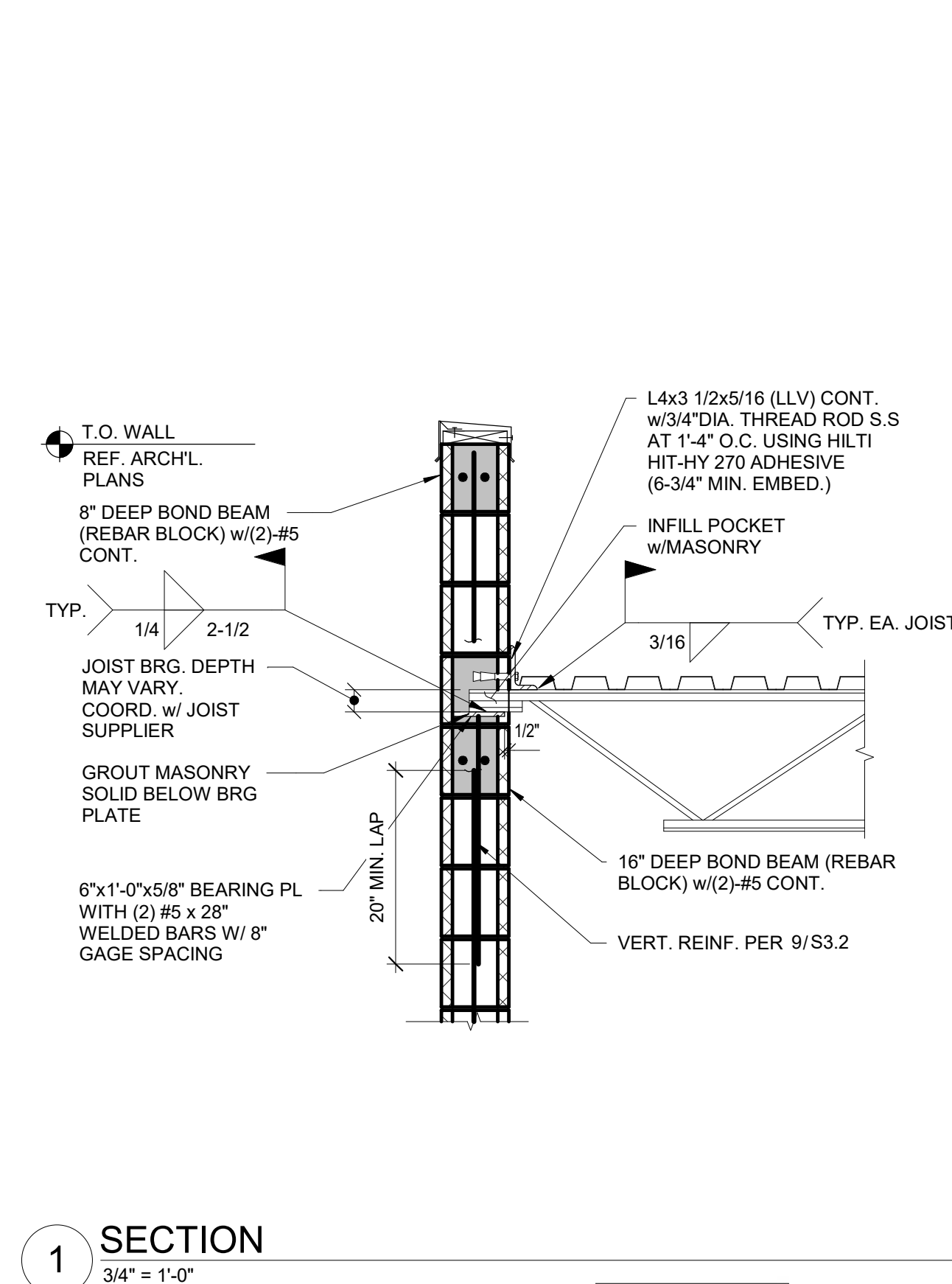
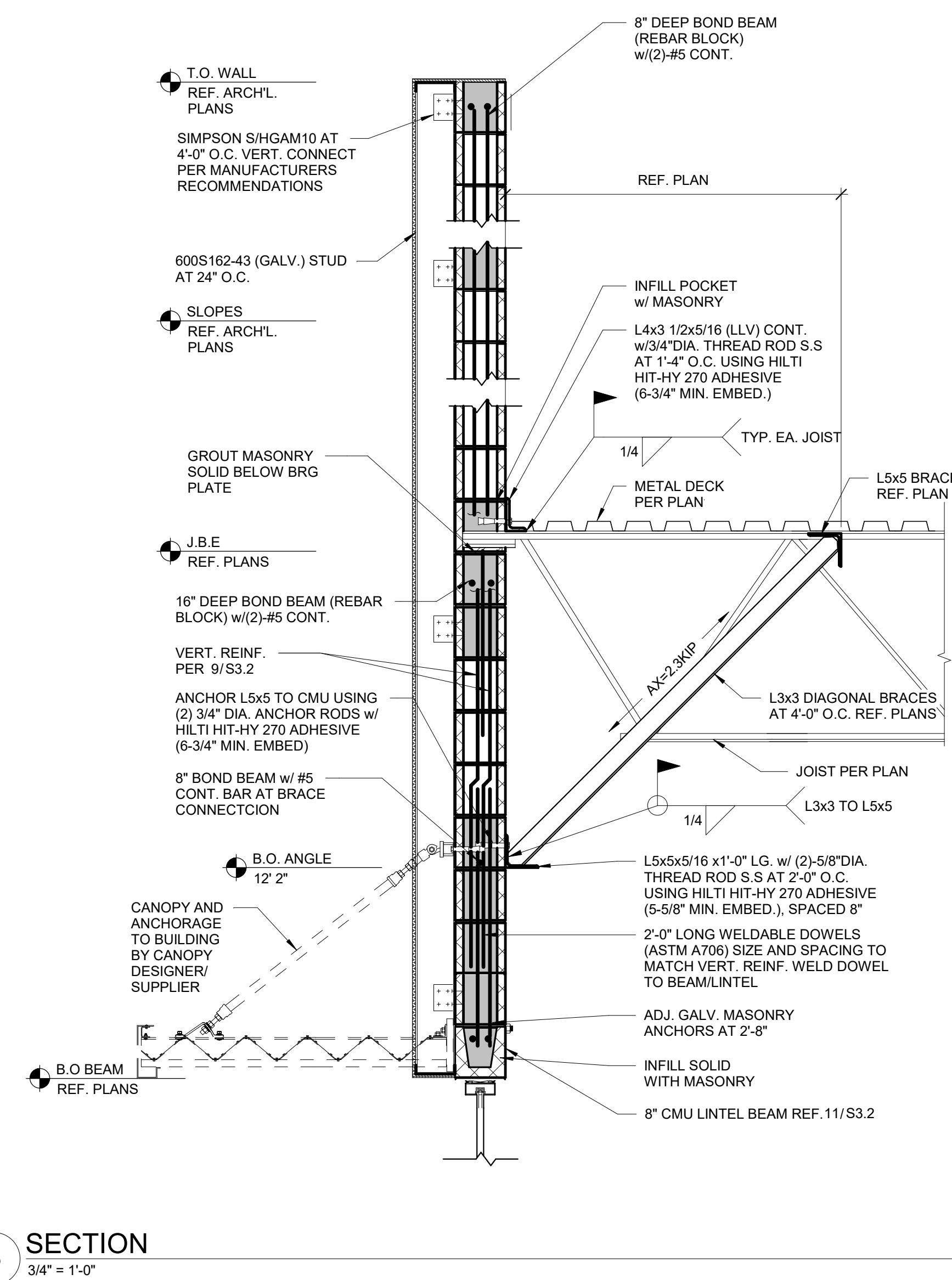
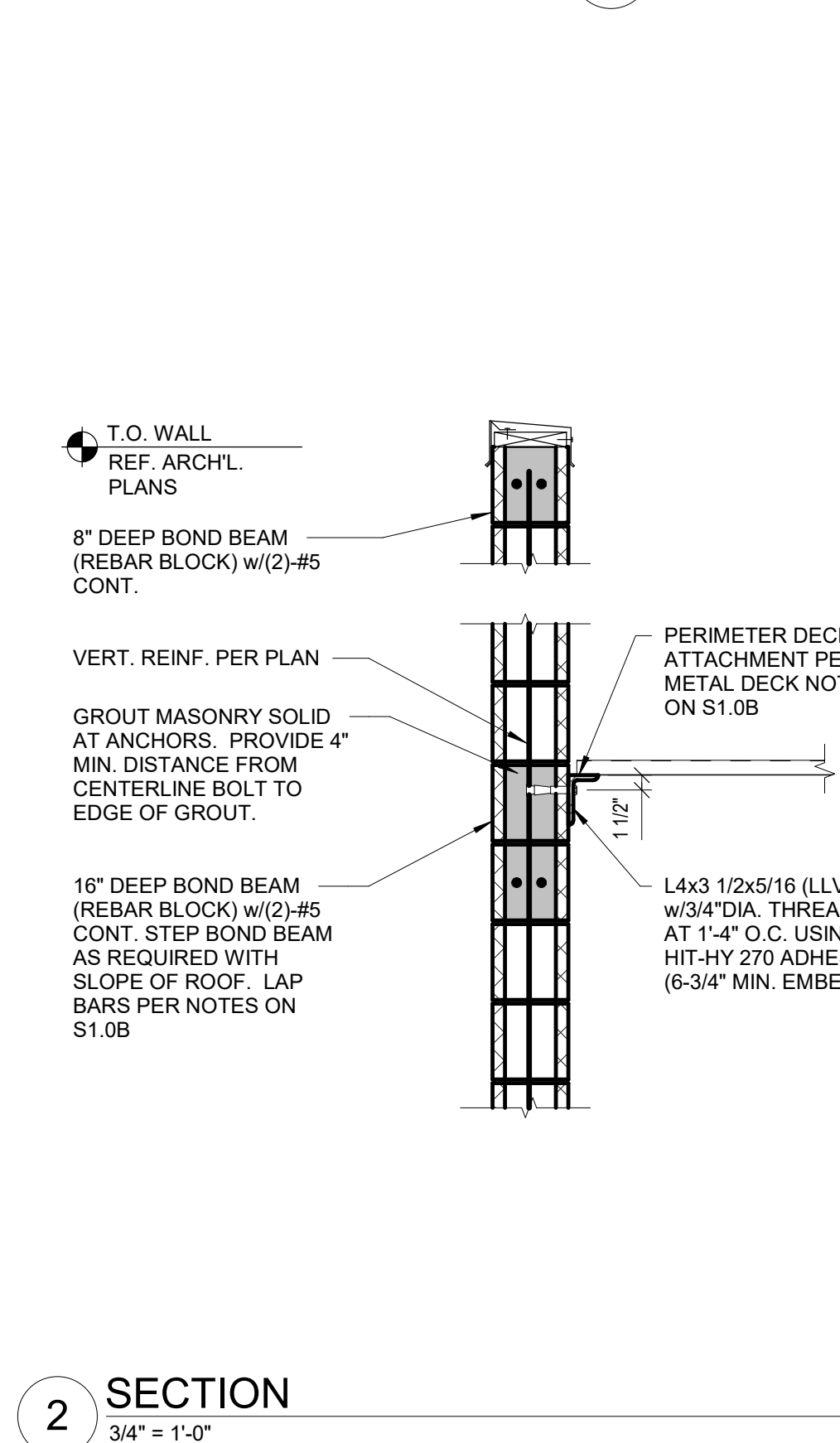
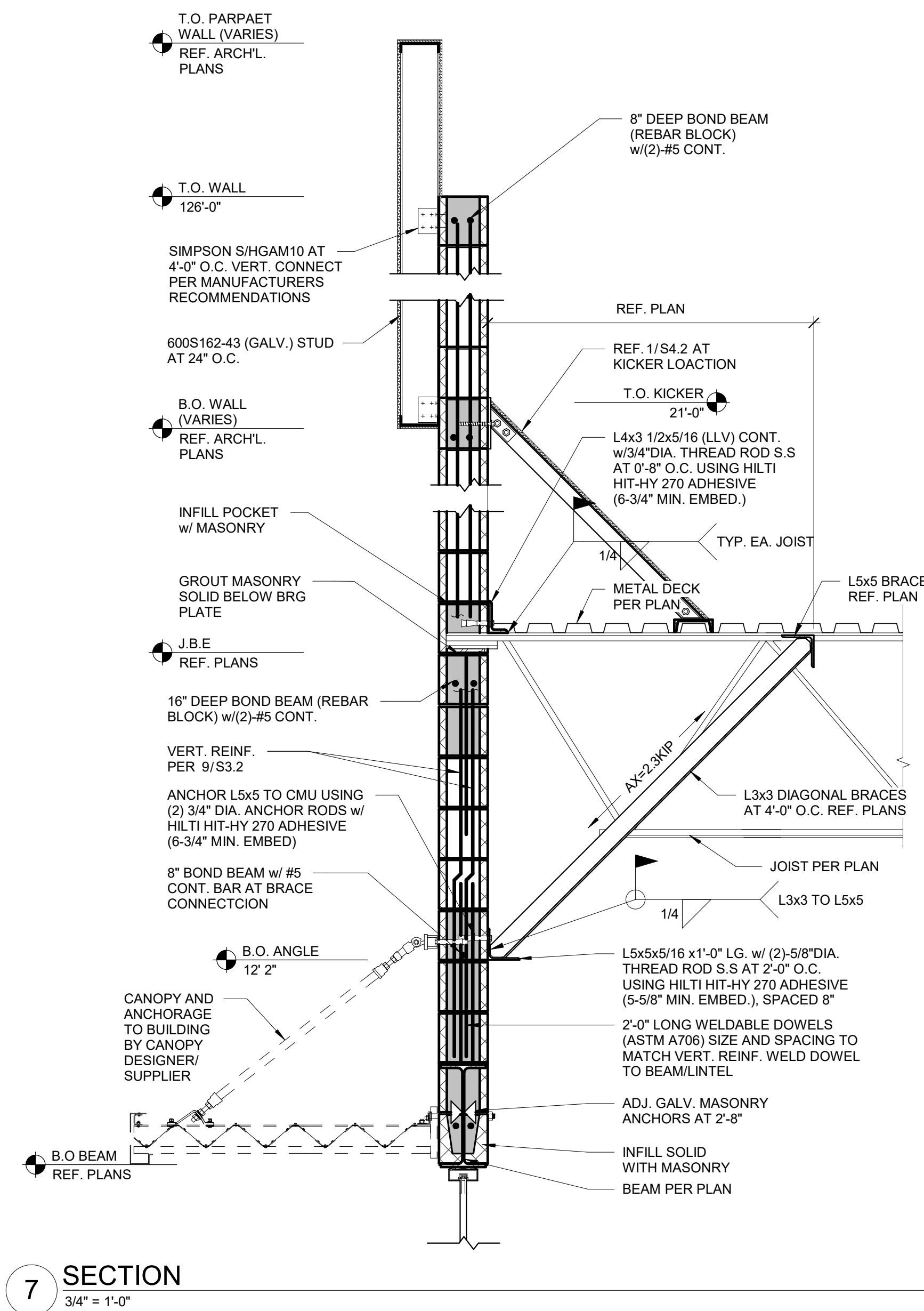
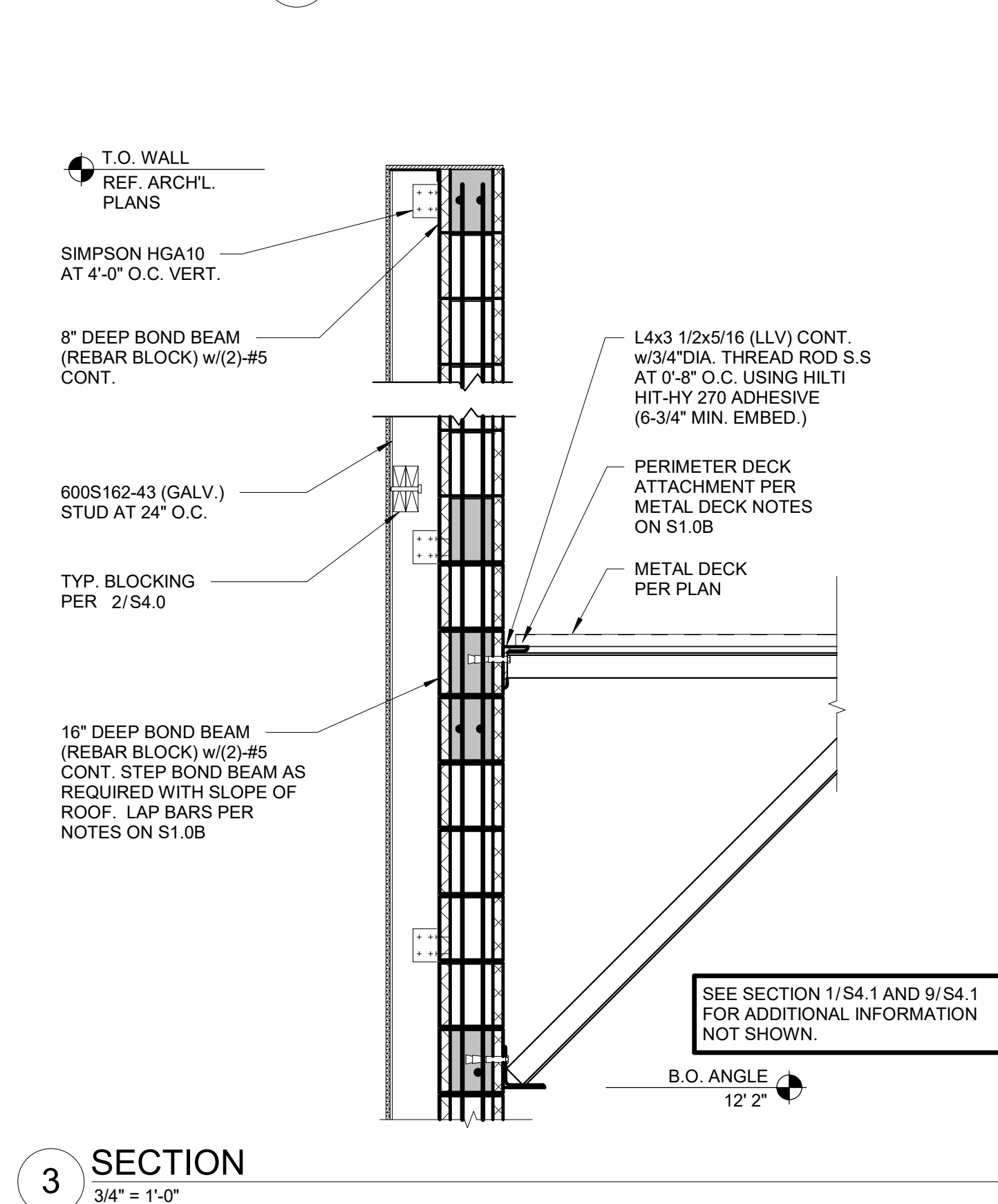
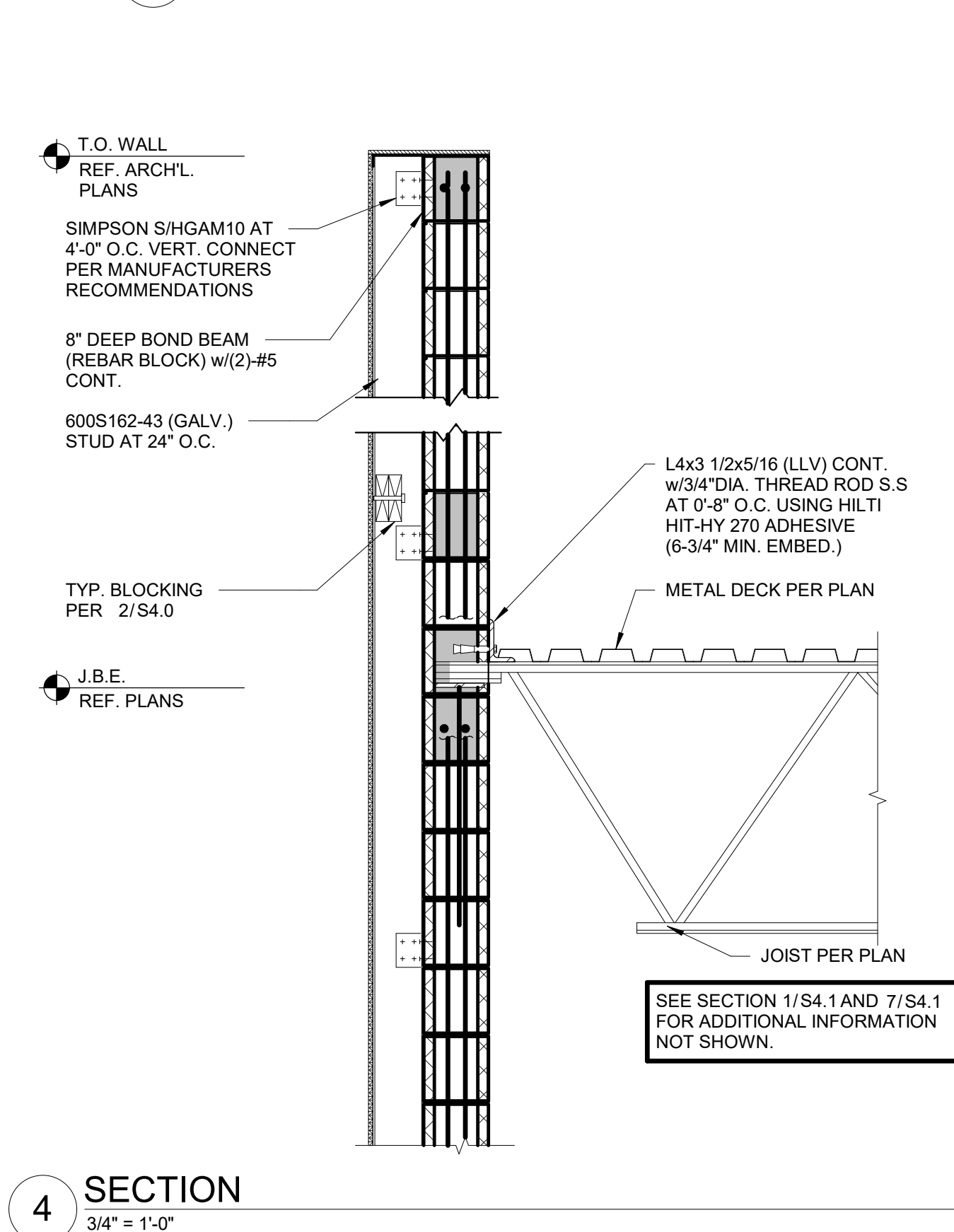
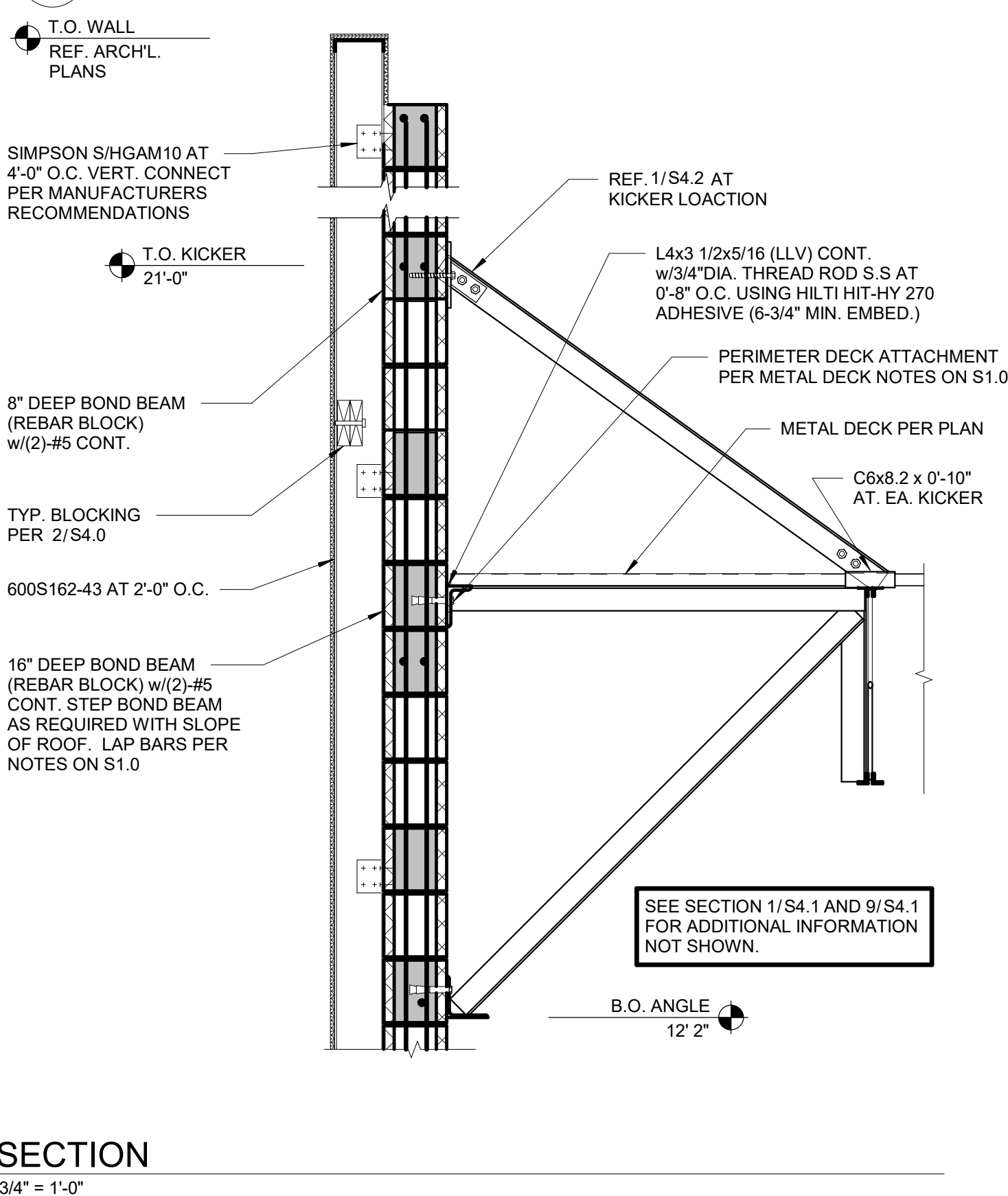
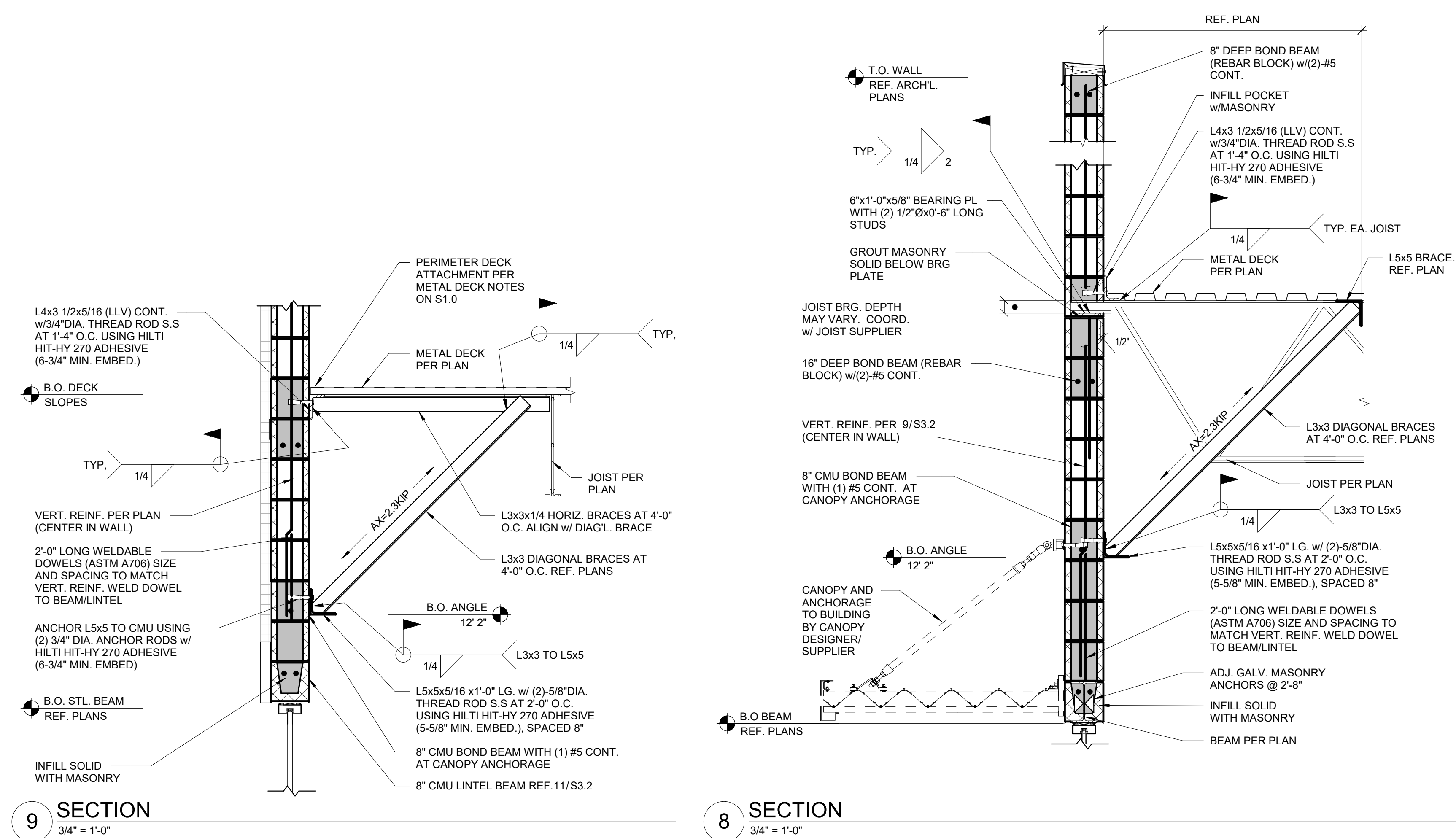
The seal and signature apply only to the document to which they are affixed and we expressly disclaim any responsibility for all other plans, specifications, estimates, reports or other documents or instruments relating to or intended to be used for any part of the project.

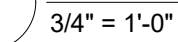
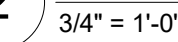
Florida Certificate of Authority #FC2000001956



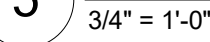
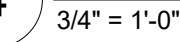
JCAA
4100 Wadsworth Blvd.
Wheat Ridge, CO 80033
p 303.985.3260
FL Firm No.29586

JOB#: 25023
DATE: 08/14/2025
SHEET: S3.2

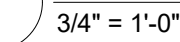


$$\frac{3}{4}'' = 1'-0''$$

$$3/4" = 1'-0"$$


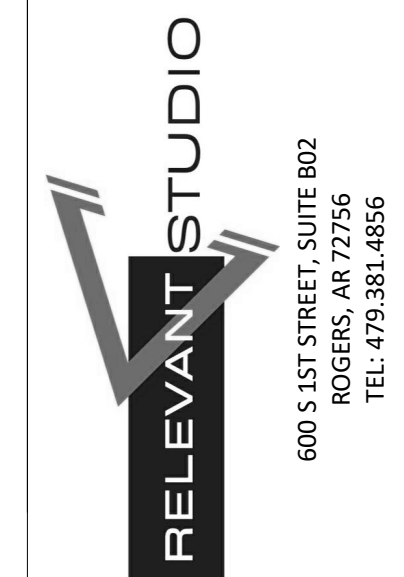
3/4" = 1'-0"


$$\frac{3}{4}'' = 1'-0''$$


$3/4" = 1'-0"$



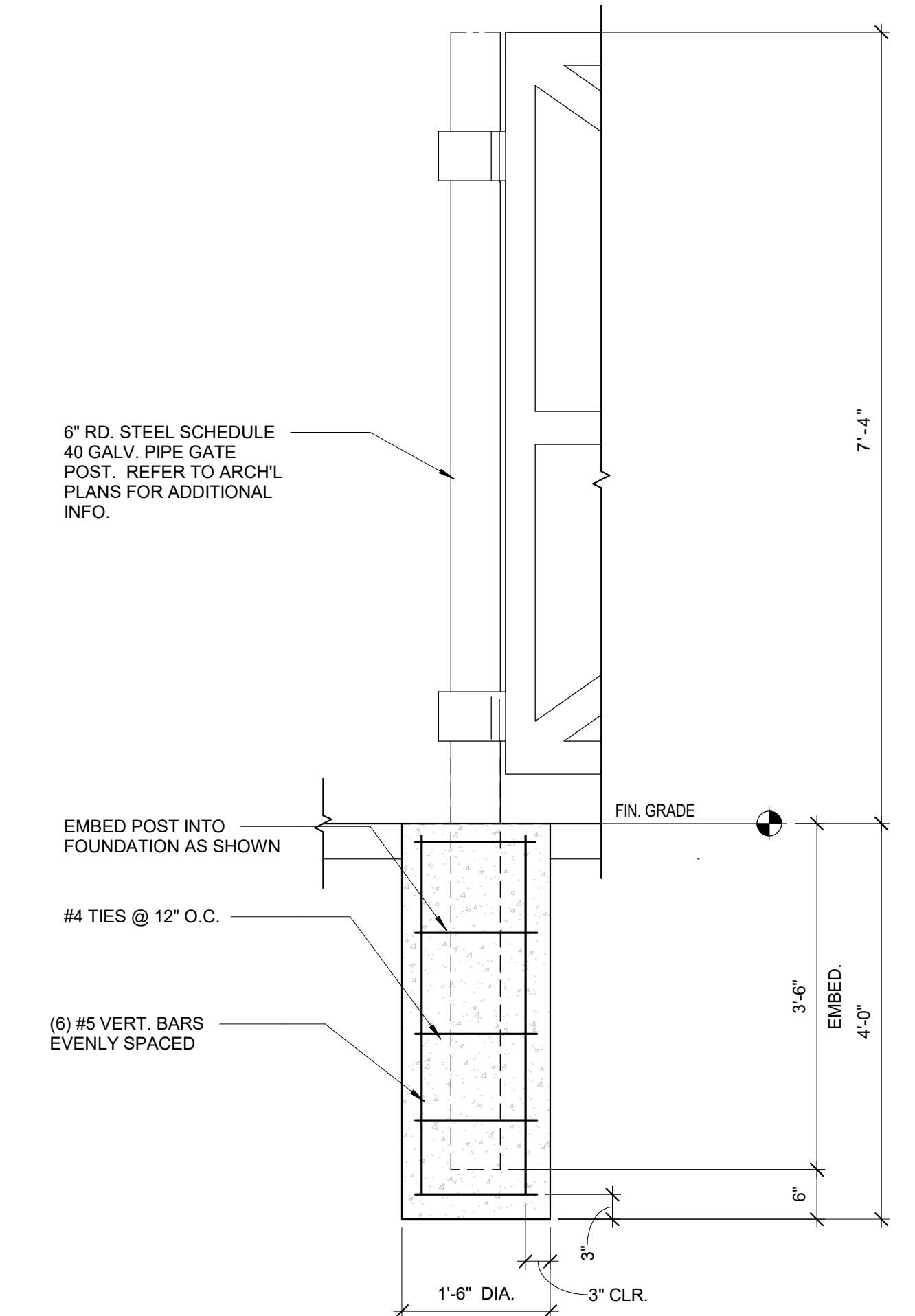
7-ELEVEN, INC
3200 HACKBERRY ROAD, IRVING TEXAS 75063
7-ELEVEN #1055406
NORTHLAKE & COCONUT (COCONUT CROSSING)
PALM BEACH GARDENS, FL
TYPICAL FRAMING DETAILS



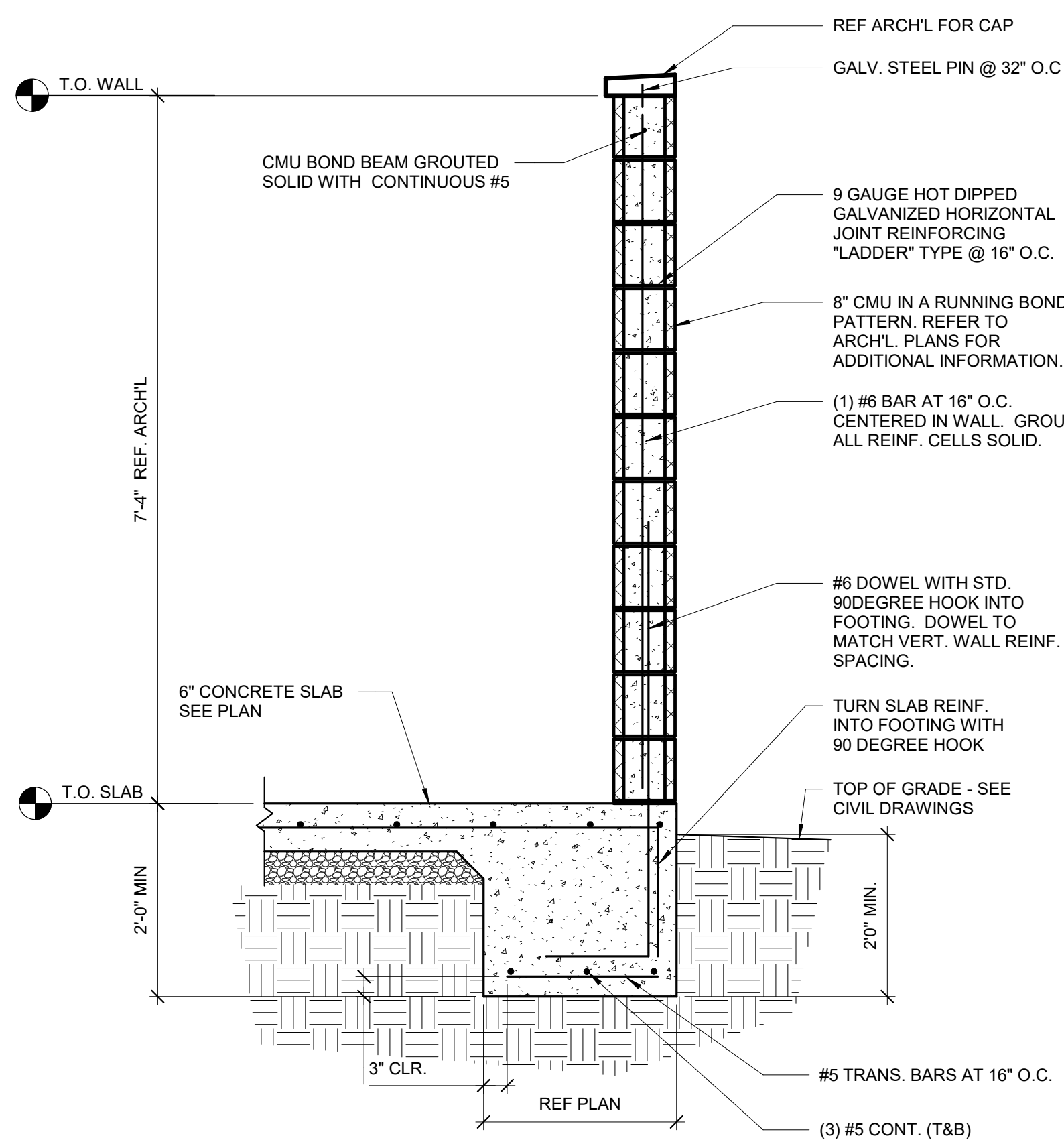
The seal and signature apply only to the document to which they are affixed and we expressly disclaim any responsibility for all other plans, specifications, estimates, reports or other documents or instruments relating to or intended to be used for any part or parts of the project.

Florida Certificate of Authority #F02000001956

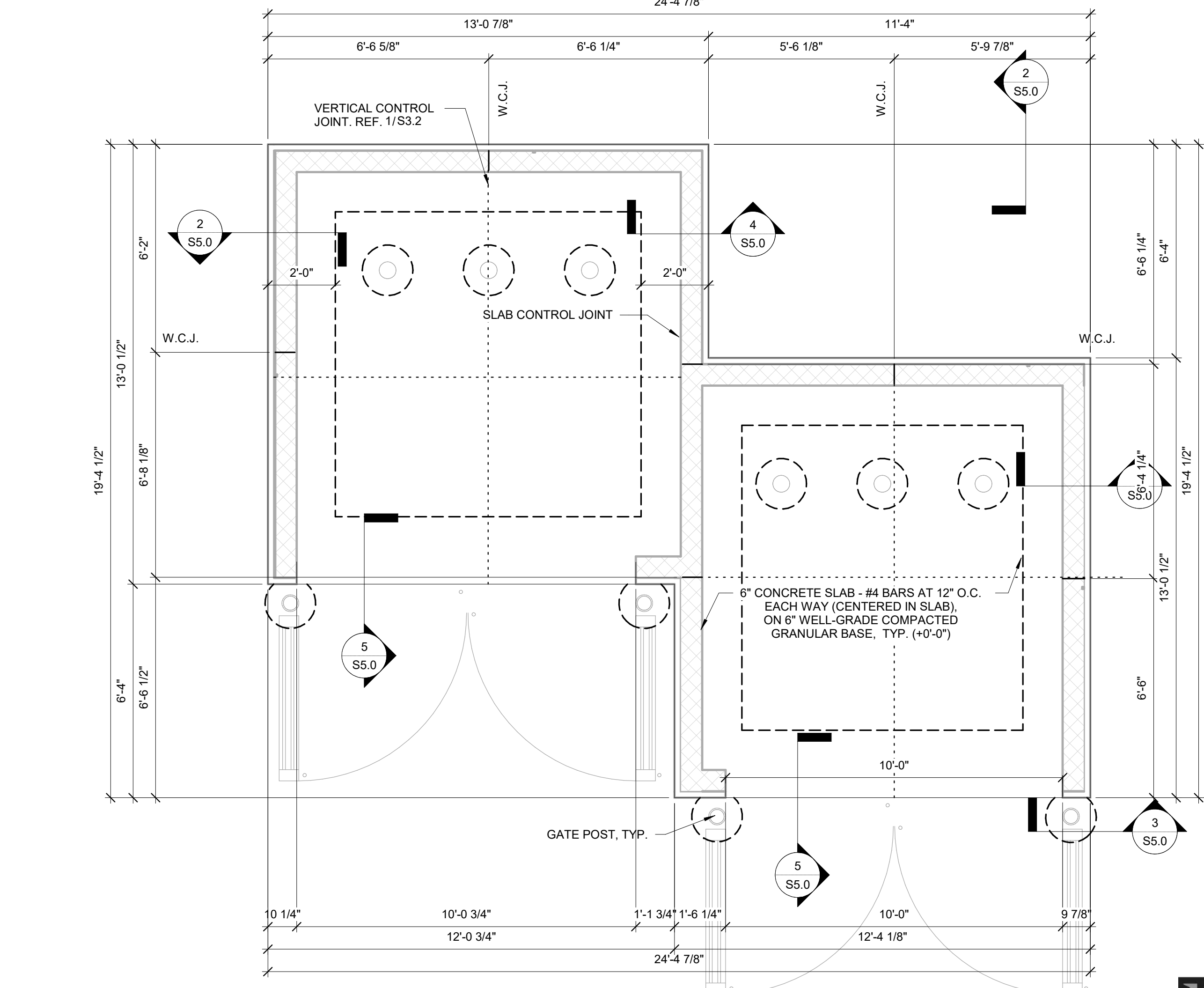
S4.2



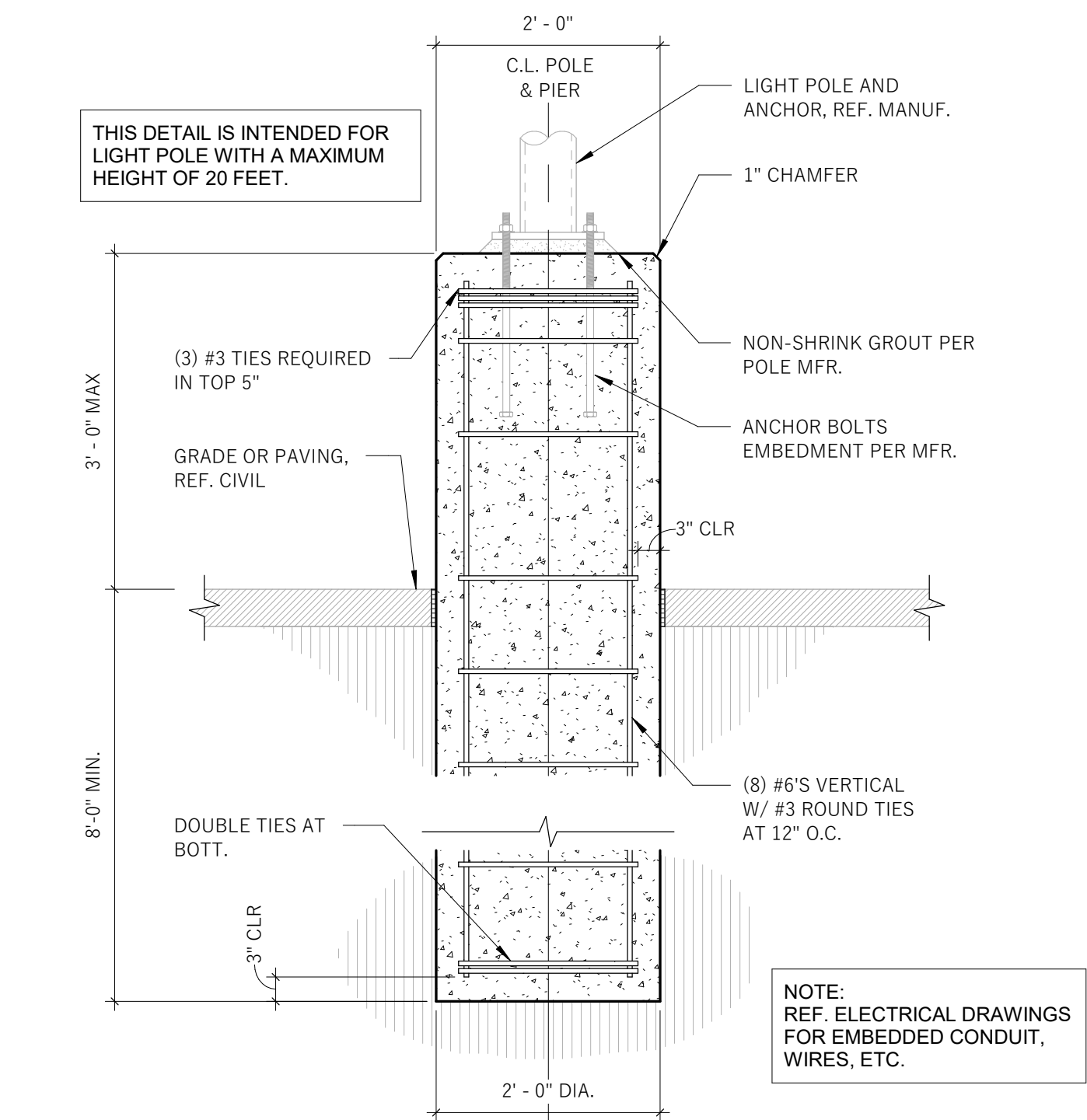
3 ENCLOSURE GATE POST FOUNDATION
3/4" = 1'-0"



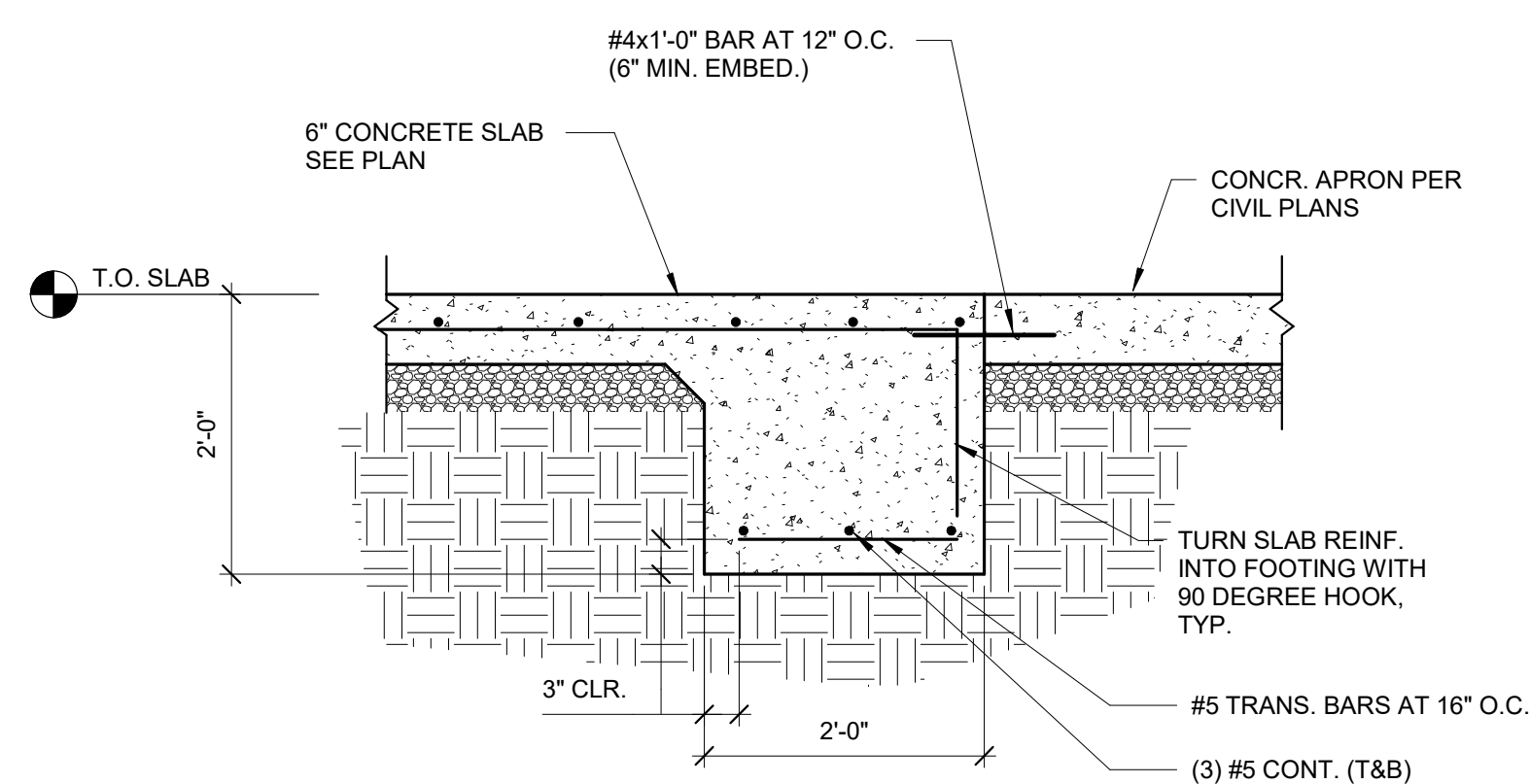
2 DUMPSTER WALL SECTION
3/4" = 1'-0"



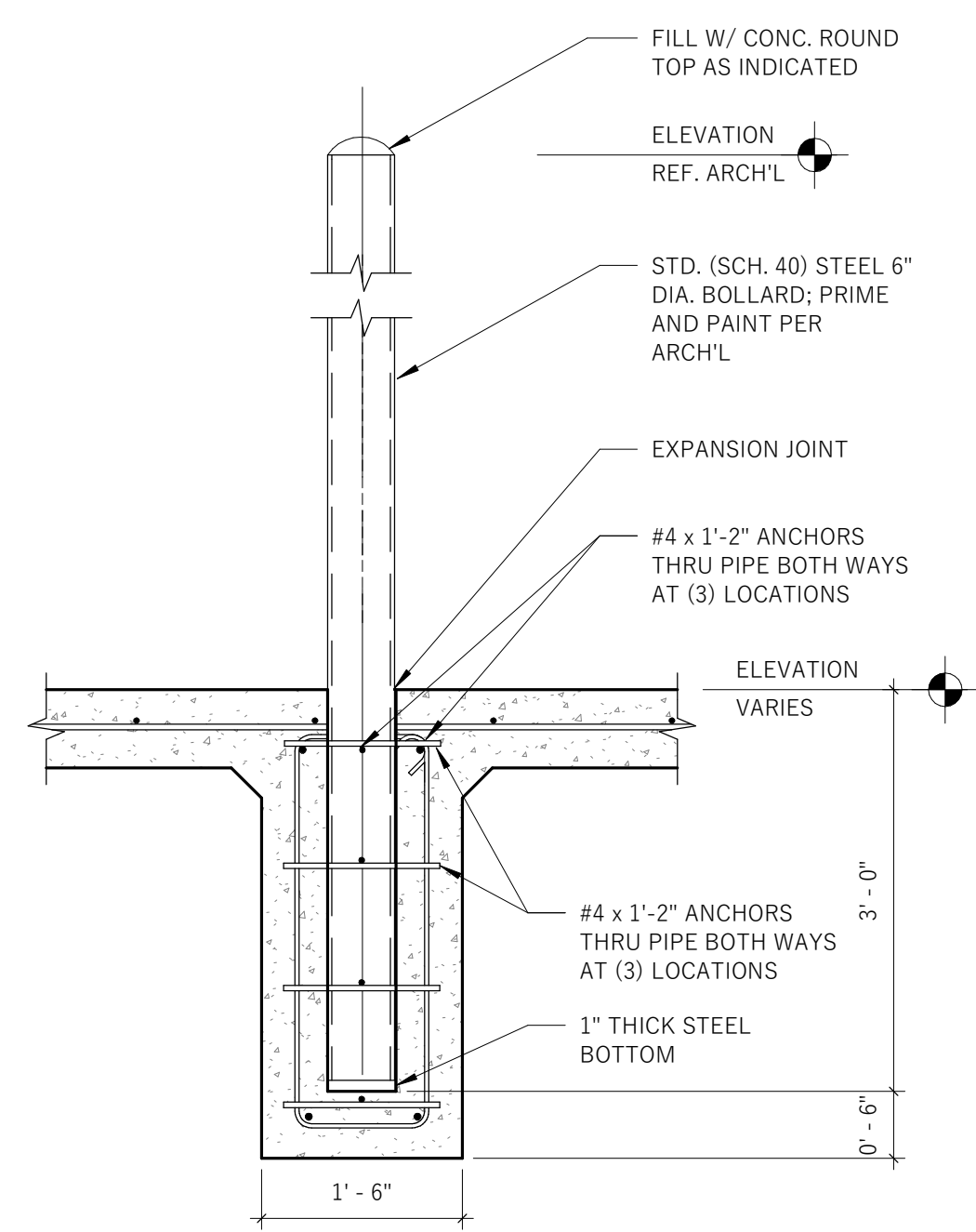
1 DUMPSTER ENCLOSURE PLAN
3/8" = 1'-0"



6 TYP. LIGHT STANDARD DETAIL
3/4" = 1'-0"



5 DUMPSTER FOUNDATION SECTION
3/4" = 1'-0"



4 BOLLARDS IN SLAB
3/4" = 1'-0"

REV #	DATE	DESCRIPTION

7-ELEVEN, INC
3200 HACKBERRY ROAD, IRVING TEXAS 75063
7-ELEVEN #1055406
NORTHLAKE & COCONUT (COCONUT CROSSING)
PALM BEACH GARDENS, FL



RELEVANT STUDIO
600 S 1ST STREET, SUITE B02
ROCKFORD, IL 61107
TEL: 415-381-4656

Drawings and Specifications as instruments of service are and shall remain the property of the Architect. They are not to be used on other projects or extensions to this project except by agreement in writing and with appropriate compensation to the Architect.

The seal and signature apply only to the document to which they are affixed and we expressly disclaim any responsibility for all other plans, specifications, estimates, reports or other documents or instruments relating to or intended to be used for any part or parts of the project.

Florida Certificate of Authority #02000001956



JCAA
4100 Wadsworth Blvd.
Wheat Ridge, CO 80033
p 303.985.3260
FL Firm No. 29586

JOB#: 25023
DATE: 08/14/2025
SHEET: S5.0