

# STRUCTURAL ABBREVIATIONS

Table of structural abbreviations with columns for letter, description, and full name. Includes items like ANCHOR BOLT, AMERICAN CONCRETE INSTITUTE, and various steel and concrete terms.

# SHOP DRAWINGS AND SUBMITTALS

- 1. THE GENERAL CONTRACTOR SHALL FOLLOW THE ARCHITECTS INSTRUCTIONS FOR DISTRIBUTION OF SHOP DRAWINGS.
2. SHOP DRAWING REVIEW IS FOR GENERAL CONFORMANCE WITH THE DESIGN INTENT. CORRECTIONS OR COMMENTS MADE ON THIS REVIEW DO NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR ERRORS AND/OR OMISSIONS.
3. APPROVAL OF SHOP DRAWINGS DOES NOT INDICATE AN ACCEPTANCE OF DEVIATIONS FROM THE CONTRACT DOCUMENTS OR PREVIOUS SHOP DRAWING REVIEW, UNLESS SPECIFICALLY NOTED THEREIN BY ENGINEER OF RECORD.

# CAST-IN-PLACE CONCRETE NOTES

- 1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE PUBLICATIONS 'BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE' - ACI 318 LATEST EDITION, AND 'SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS' - ACI 301 LATEST EDITION.
2. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS IN ACCORDANCE WITH THE FOLLOWING 'SCHEDULE OF CAST-IN-PLACE CONCRETE CONSTRUCTION MATERIALS':

SCHEDULE OF CAST-IN-PLACE CONCRETE CONSTRUCTION MATERIALS table with columns for LOCATION, 28-DAY COMPRESSIVE STRENGTH, fc, and MAX w/c RATIO. Includes rows for FOOTINGS, FOUNDATIONS, SLABS ON GROUND, etc.

PROTECTION FOR REINFORCEMENT IN CAST-IN-PLACE CONCRETE table with columns for APPLICATION and CLEAR COVER. Includes rows for CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH, etc.

# CONCRETE SLAB ON GROUND NOTES

- 1. THE CONCRETE SLAB ON GROUND FOR THIS PROJECT IS PRESCRIPTIVE. NO STRUCTURAL DESIGN HAS BEEN PROVIDED.
2. THE CONCRETE SLAB ON GROUND HAS BEEN SPECIFIED BASED ON THE FOLLOWING ASSUMPTIONS:
A. MINIMUM SOIL BEARING PRESSURE OF 2500 PSF.
B. SOIL CONSTANT 'K' VALUE OF 100 PSI/IN.

# STRUCTURAL STEEL NOTES

- 1. ALL STRUCTURAL STEEL DETAILING, FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH 'SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS' - AISC 360, LATEST EDITION AS ADOPTED BY NOTED GOVERNING BUILDING CODES.
2. WELDED CONNECTIONS SHALL CONFORM TO THE LATEST EDITION CODE OF THE AMERICAN WELDING SOCIETY, AWS D1.1.
3. ALL FABRICATION AND ERECTION WORK SHALL BE PERFORMED BY AISC CERTIFIED FABRICATORS AND ERECTORS.

# GOVERNING CODES AND STANDARDS

- 1. THE STRUCTURAL DESIGN AND ALL WORK REFERENCED HEREIN SHALL CONFORM TO THE FOLLOWING CODES AND STANDARDS. USE THE LATEST EDITION UNLESS NOTED OTHERWISE.
1. \*FLORIDA BUILDING CODE - FBC 2020, 7TH EDITION.
2. \*MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES\* - ASCE 7-16.

# FOUNDATION AND GEOTECHNICAL NOTES

- 1. REFERENCE THE GEOTECHNICAL REPORT COMPLETED FOR THIS SITE FOR FURTHER INFORMATION RELATING TO THE EXISTING SUBSURFACE SOIL CONDITIONS AND REQUIRED SITE PREPARATION PROCEDURES.
2. THE ALLOWABLE NET SOIL BEARING PRESSURE IS 2500 PSF. THIS DESIGN SOIL BEARING PRESSURE IS BASED ON THE ACCEPTED COMPLETION OF ALL RECOMMENDATIONS AND REQUIREMENTS IN THE REFERENCED GEOTECHNICAL REPORT.
3. ALL REQUIREMENTS FOR SITE PREPARATION AND SOIL COMPACTION SPECIFIED IN THE GEOTECHNICAL REPORT SHALL BE FOLLOWED UNLESS ADDITIONAL MORE STRINGENT REQUIREMENTS ARE SPECIFIED. A CERTIFIED TESTING AGENCY SHALL PERFORM SOIL DENSITY AND COMPACTION TESTS TO ENSURE CONFORMANCE WITH THE GEOTECHNICAL REPORT. SUBMIT ALL TESTS RESULTS TO THE PROJECT ARCHITECT AND ENGINEER, TEST PER THE FOLLOWING:

# MASONRY / CMU NOTES

- 1. ALL MASONRY WORK SHALL BE IN ACCORDANCE WITH THE PUBLICATIONS 'BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES' - TMS 402 LATEST EDITION, AND 'SPECIFICATION FOR MASONRY STRUCTURES' - TMS 602, LATEST EDITION, OR EDITIONS AS ADOPTED BY THE GOVERNING BUILDING CODES REFERENCED IN THE GENERAL NOTES.
2. HOLLOW LOAD BEARING UNITS SHALL BE NORMAL WEIGHT CONFORMING TO ASTM C90, WITH A MINIMUM NET COMPRESSIVE STRENGTH OF 2000 PSI (fm = 2000 PSI).
3. UNITS SHALL BE MANUFACTURERS STANDARD UNITS WITH NOMINAL FACE DIMENSION OF 16" LONG.

# OPEN WEB STEEL JOIST NOTES

- 1. DESIGN, DETAILING, FABRICATION AND ERECTION OF STEEL JOISTS AND JOIST GIRDERS SHALL BE IN ACCORDANCE WITH 'SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS' - AISC 360, LATEST EDITION AND WITH THE LATEST CODES AND STANDARDS OF THE STEEL JOIST INSTITUTE, SJI.
2. JOISTS SHALL BE DESIGNED FOR THE COMBINED DEAD, LIVE, AND WIND LOADS AS NOTED IN THE LOAD TABLES AND AS NOTED ON PLAN. IN ALL CASES, JOISTS SHALL NOT BE DESIGNED FOR LESS LOAD THAN PRESCRIBED IN THE STANDARD JOIST LOADING TABLES PER THE STEEL JOIST INSTITUTE.
3. PROVIDE UPLIFT BRIDGING AND STANDARD JOIST BRIDGING IN ACCORDANCE WITH THE LATEST SJI SPECIFICATIONS.

# GENERAL CONDITIONS

- 1. THE GENERAL CONTRACTOR SHALL REVIEW AND VERIFY THAT ALL DIMENSIONS ARE COORDINATED BETWEEN THE ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO FABRICATION OR START OF CONSTRUCTION.
2. THE GENERAL CONTRACTOR SHALL VERIFY AND BE RESPONSIBLE FOR ALL CONDITIONS AT THE PROJECT SITE AND SHALL NOTIFY ARCHITECT/ENGINEER OF DISCREPANCIES BETWEEN THE ACTUAL CONDITIONS AND INFORMATION SHOWN ON THE DRAWINGS BEFORE PROCEEDING WITH ANY WORK.
3. THESE STRUCTURAL DRAWINGS ARE TO BE USED IN COMBINATION WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND CIVIL DRAWINGS, AND ANY OTHER PROJECT CONTRACT DOCUMENTS NOT REFERRED TO THESE DRAWINGS FOR DETAILS AND INFORMATION THAT MAY RELATE TO STRUCTURAL COMPONENTS.

# FOUNDATION AND GEOTECHNICAL NOTES

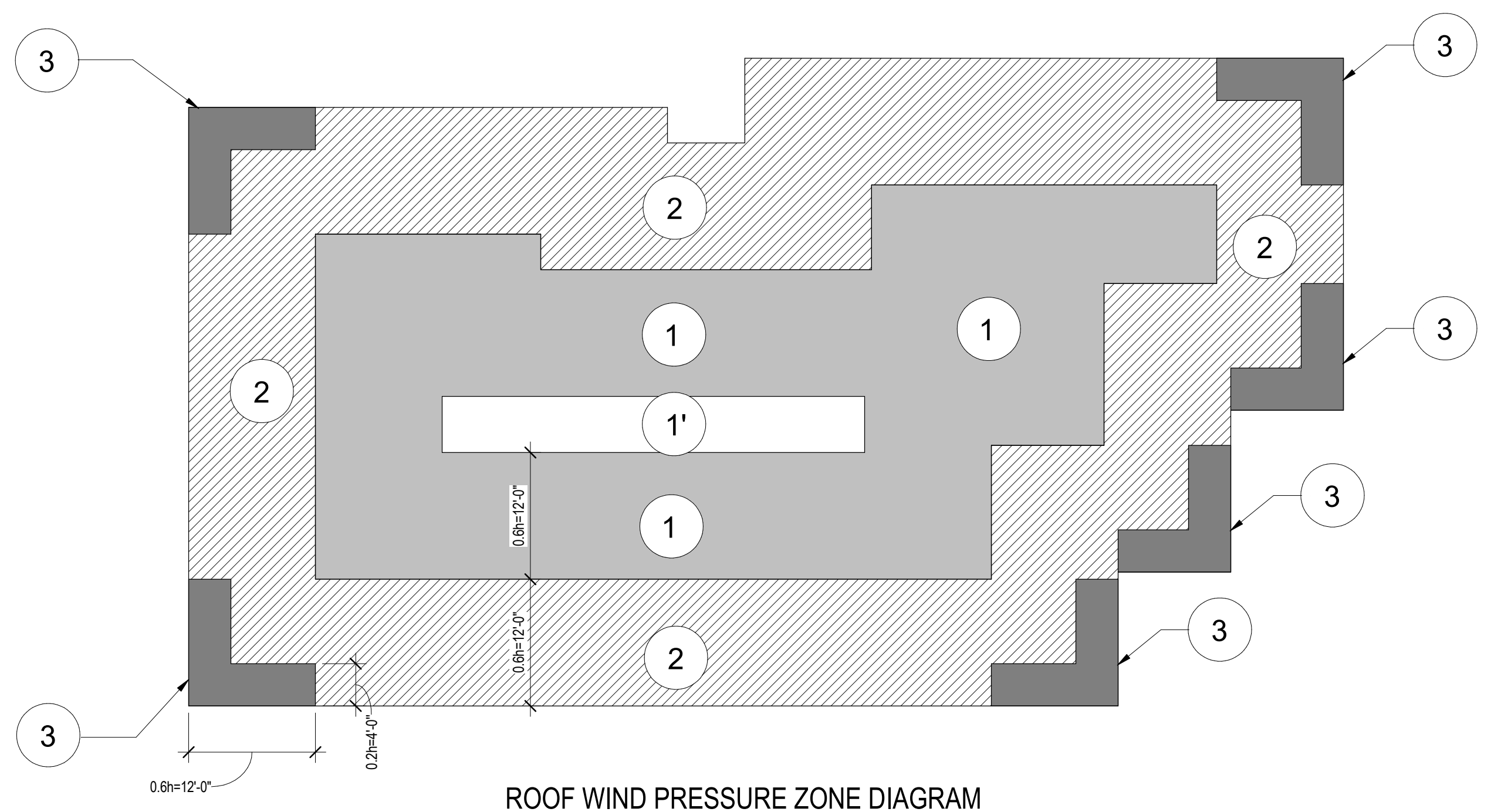
- 1. REFERENCE THE GEOTECHNICAL REPORT COMPLETED FOR THIS SITE FOR FURTHER INFORMATION RELATING TO THE EXISTING SUBSURFACE SOIL CONDITIONS AND REQUIRED SITE PREPARATION PROCEDURES.
2. THE ALLOWABLE NET SOIL BEARING PRESSURE IS 2500 PSF. THIS DESIGN SOIL BEARING PRESSURE IS BASED ON THE ACCEPTED COMPLETION OF ALL RECOMMENDATIONS AND REQUIREMENTS IN THE REFERENCED GEOTECHNICAL REPORT.
3. ALL REQUIREMENTS FOR SITE PREPARATION AND SOIL COMPACTION SPECIFIED IN THE GEOTECHNICAL REPORT SHALL BE FOLLOWED UNLESS ADDITIONAL MORE STRINGENT REQUIREMENTS ARE SPECIFIED. A CERTIFIED TESTING AGENCY SHALL PERFORM SOIL DENSITY AND COMPACTION TESTS TO ENSURE CONFORMANCE WITH THE GEOTECHNICAL REPORT. SUBMIT ALL TESTS RESULTS TO THE PROJECT ARCHITECT AND ENGINEER, TEST PER THE FOLLOWING:

# METAL ROOF DECK NOTES

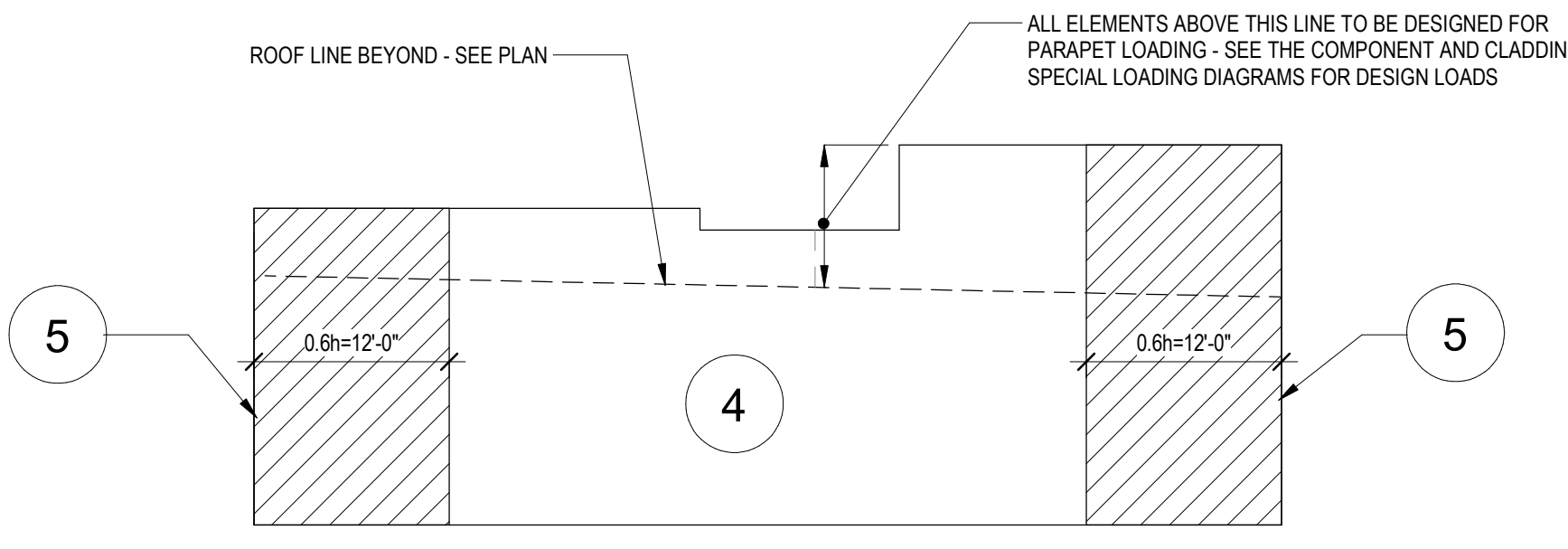
- 1. DETAILING, FABRICATION, AND ERECTION OF STEEL DECK SHALL BE IN ACCORDANCE WITH THE LATEST STEEL DECK INSTITUTE SPECIFICATIONS, AWS, AND CONTRACT DOCUMENTS. DECK SHALL CONFORM TO 'BASIC DESIGN SPECIFICATIONS' AS ADOPTED BY THE STEEL DECK INSTITUTE, SDI.
2. STEEL DECK PROFILE SHALL CONFORM TO FACTORY MANUFACTURED REQUIREMENTS.
3. METAL ROOF DECK SHALL BE MINIMUM 1/4" DEEP 22 GA. TYPE B AS IDENTIFIED BY SDI. PAINTED WHITE UNDERSIDE AND GRAY TOP SIDE STEEL DECK CONFORMING TO ASTM A1028 OR ASTM A1038 WITH MINIMUM YIELD STRESS OF 50 KSI. REFERENCE DRAWINGS FOR REQUIRED DECK STRENGTH. DECK FINISH SHALL BE SHOP PRIMED WITH BAKED-ON, LEAD- AND CHROMATE-FREE RUST-INHIBITIVE PRIMER COMPLYING WITH PERFORMANCE REQUIREMENTS OF SSPC-PAINT 25.

Professional Engineer seal for Jonathan D. Collins, License No. 74693, State of Florida. Includes contact information for WEST MARKET LOT 10 and GENERAL NOTES.

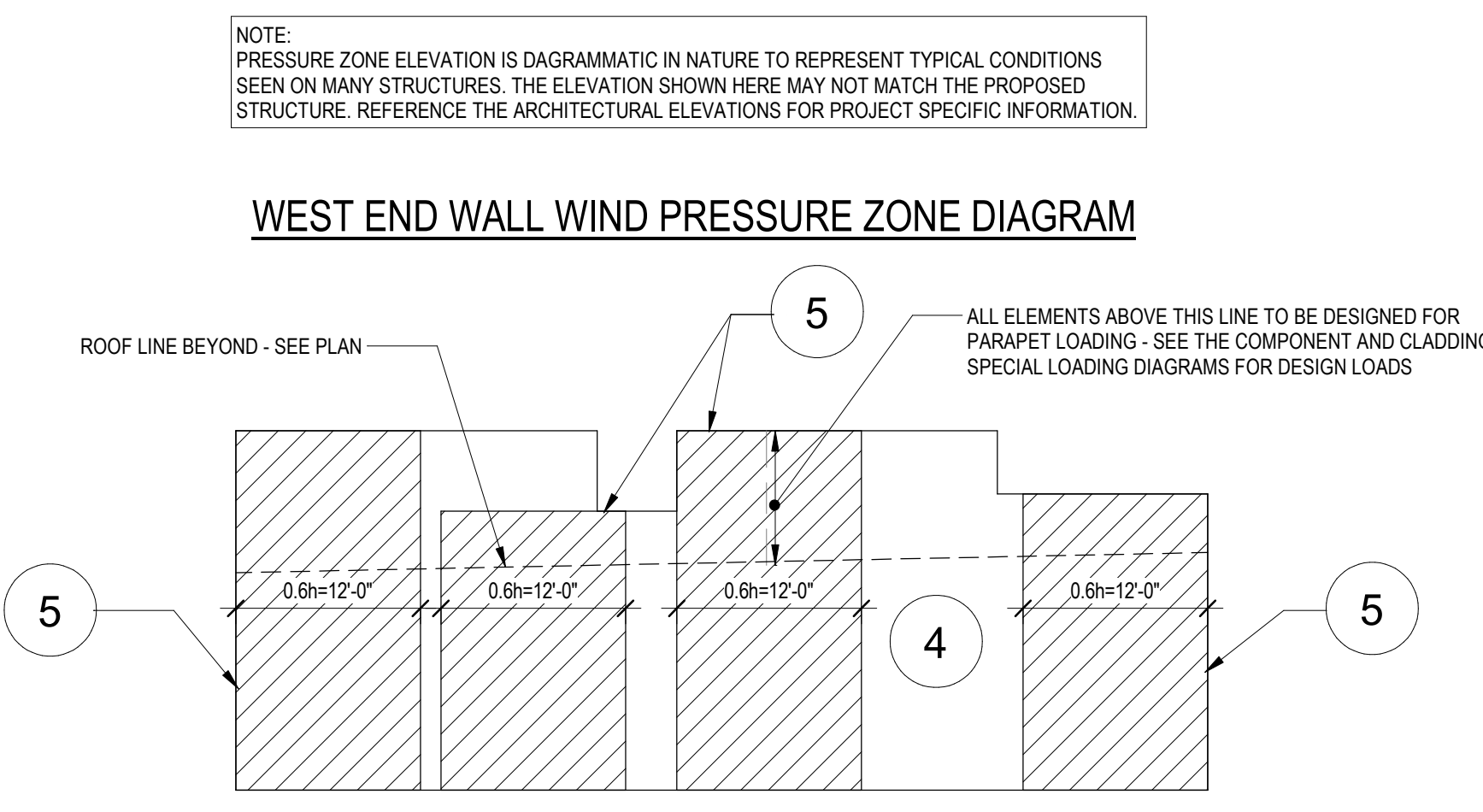




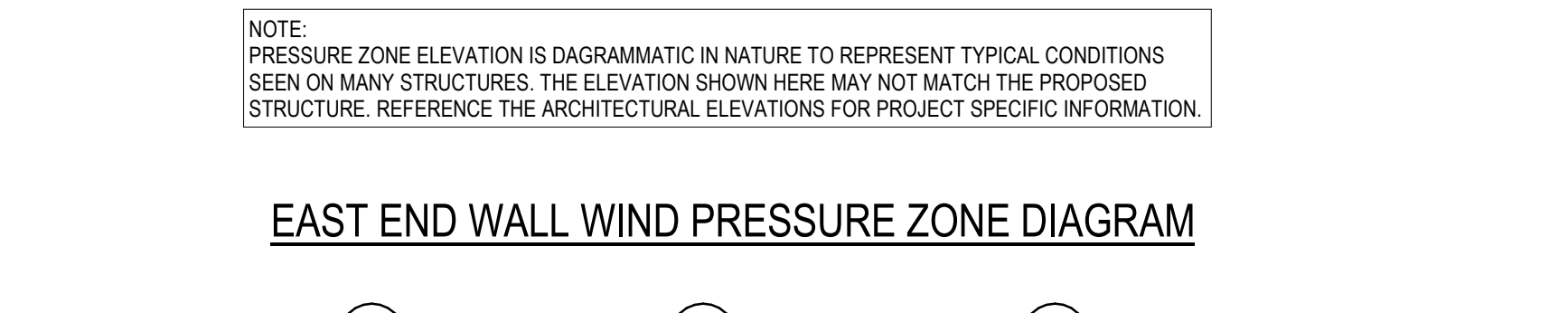
ROOF WIND PRESSURE ZONE DIAGRAM



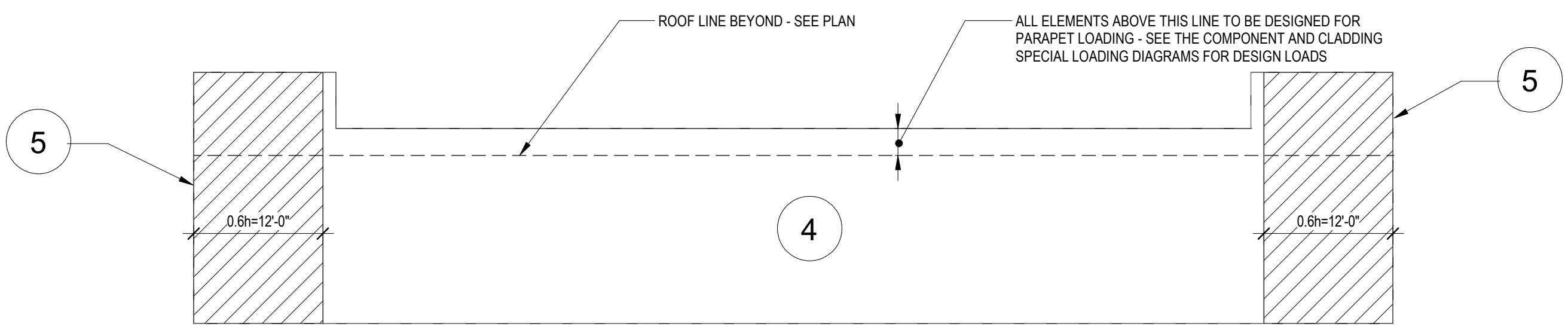
NORTH SIDE WALL WIND PRESSURE ZONE DIAGRAM



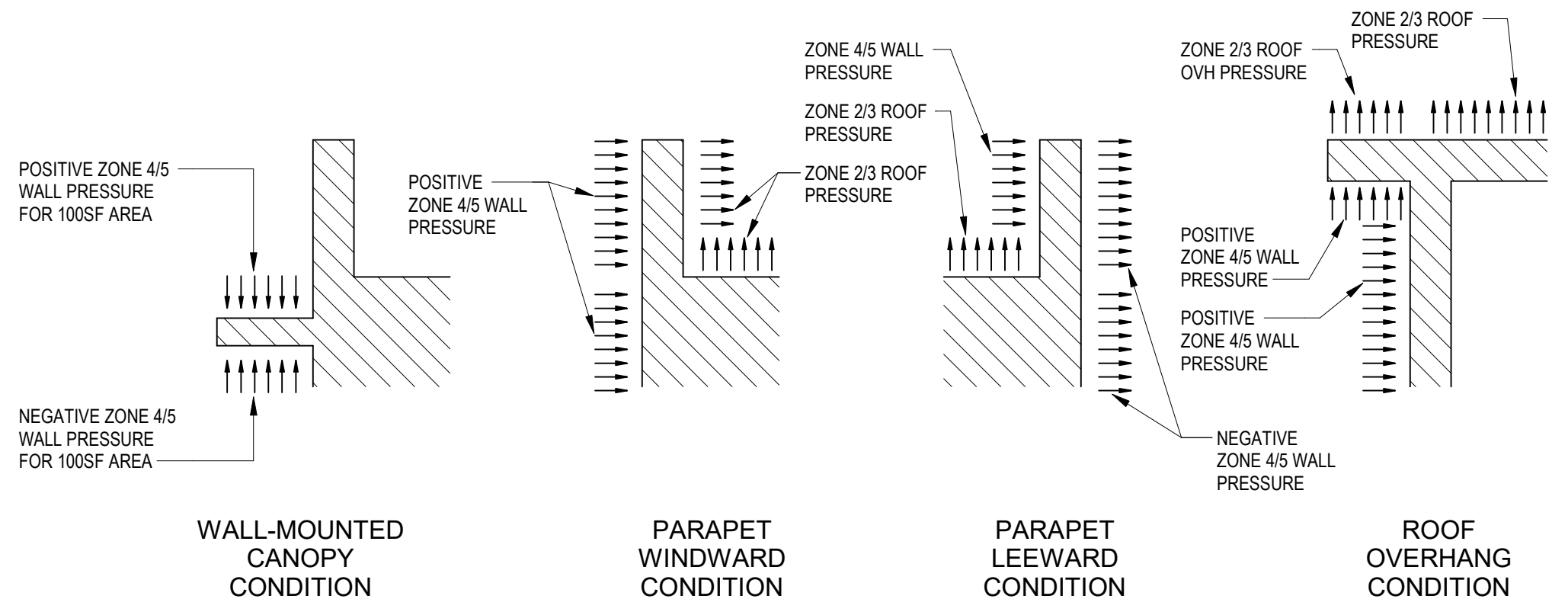
WEST END WALL WIND PRESSURE ZONE DIAGRAM



EAST END WALL WIND PRESSURE ZONE DIAGRAM



SOUTH SIDE WALL WIND PRESSURE ZONE DIAGRAM



COMPONENTS AND CLADDING SPECIAL LOADING DIAGRAMS

TABLE 1: DESIGN LOADS & DESIGN CRITERIA			
DEAD LOADS		LIVE LOADS	
BUILDING COMPONENT	WEIGHT	LOAD CONDITION	LOAD
TPO ROOFING	2.0 PSF	ROOF LIVE LOAD	20.0 PSF
INSULATION	2.0 PSF	ROOF CONCENTRATED	150 LBS
METAL DECK	2.0 PSF		
STEEL JOISTS	3.0 PSF		
FIRE SPRINKLERS	2.0 PSF		
MECHANICAL	8.0 PSF		
MISCELLANEOUS	6.0 PSF		
<b>TOTAL DEAD LOAD</b>	<b>25.0 PSF</b>		

WIND LOAD CRITERIA			
BASIC WIND SPEED (ULT)	139 MPH	BUILDING RISK CATEGORY	II
BASIC WIND SPEED (ASD)	108 MPH	EXPOSURE CATEGORY	C
VELOCITY PRESSURE, q <sub>h</sub> (ULT)	37.9	ENCLOSURE CLASSIFICATION	ENCLOSED
VELOCITY PRESSURE, q <sub>h</sub> (ASD)	22.8	INTERNAL PRESSURE COEFFICIENT	+/- 0.18

NOTES:  
 1. ROOF LIVE LOADS MAY BE REDUCED, WHERE APPLICABLE, PER FLORIDA BUILDING CODE SEC 1607.12.2.1. REDUCED UNIFORM LIVE LOAD SHALL NOT BE LESS THAN 12.0 PSF.  
 2. CONCENTRATED ROOF LOADS OVER STEEL JOIST ROOFS SHALL BE APPLIED AS BEND CHECK LOADS TO THE ROOF JOISTS AT THE TOP AND BOTTOM CHORDS (NOT SIMULTANEOUSLY).

TABLE 2: STRENGTH DESIGN (ULTIMATE) WIND PRESSURES

POSITIVE PRESSURES ON WALLS & WALL OPENINGS					
WIND PRESSURE ZONE	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA
	≤ 10 SF	20 SF	50 SF	100 SF	≥ 200 SF
WALL ZONE ④	41.0 PSF	39.1 PSF	36.7 PSF	34.9 PSF	33.1 PSF
WALL ZONE ⑤	41.0 PSF	39.1 PSF	36.7 PSF	34.9 PSF	33.1 PSF

NEGATIVE WIND PRESSURES ON WALLS & WALL OPENINGS					
WIND PRESSURE ZONE	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA
	≤ 10 SF	20 SF	50 SF	100 SF	≥ 200 SF
WALL ZONE ④	-44.4 PSF	-42.6 PSF	-40.2 PSF	-38.3 PSF	-36.5 PSF
WALL ZONE ⑤	-54.6 PSF	-51.0 PSF	-46.2 PSF	-42.6 PSF	-38.9 PSF

POSITIVE WIND PRESSURES ON ROOFING & ROOF FRAMING					
WIND PRESSURE ZONE	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA
	≤ 10 SF	20 SF	50 SF	100 SF	≥ 200 SF
ROOF ZONE ①	18.2 PSF	17.1 PSF	16.0 PSF	16.0 PSF	16.0 PSF
ROOF ZONE ②	18.2 PSF	17.1 PSF	16.0 PSF	16.0 PSF	16.0 PSF
ROOF ZONE ③	18.2 PSF	17.1 PSF	16.0 PSF	16.0 PSF	16.0 PSF

NEGATIVE WIND PRESSURES ON ROOFING & ROOF FRAMING					
WIND PRESSURE ZONE	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA
	≤ 10 SF	20 SF	50 SF	100 SF	≥ 200 SF
ROOF ZONE ①	-41.0 PSF	-41.0 PSF	-41.0 PSF	-41.0 PSF	-35.2 PSF
ROOF ZONE ②	-71.3 PSF	-66.6 PSF	-60.4 PSF	-55.7 PSF	-51.0 PSF
ROOF ZONE ③	-94.0 PSF	-88.0 PSF	-80.0 PSF	-74.0 PSF	-67.9 PSF
ROOF ZONE ④	-128.2 PSF	-116.1 PSF	-100.1 PSF	-88.0 PSF	-75.9 PSF

NOTES:  
 1. WIND PRESSURES IN THE TABLES ABOVE ARE BASED ON CALCULATIONS FROM ASCE 7-16.  
 2. OVERHANG PRESSURES IN THE TABLES ABOVE SHALL APPLY TO ALL ROOFS OVER BALCONIES, BREEZEWAYS, AND COVERED ENTRIES. COORDINATE WITH ARCHITECTURAL DRAWINGS.  
 3. WHEN THE BASIC WIND SPEED (ULT) IN TABLE 1, DESIGN LOADS & DESIGN CRITERIA IS 140 MPH OR HIGHER, PROVIDE IMPACT RESISTANT GLAZING AS REQUIRED FOR WIND BORNE DEBRIS PER FLORIDA BUILDING CODE.  
 4. MAXIMUM ALLOWABLE DEAD LOADS TO BE USED TO RESIST UPLIFT SHALL BE AS FOLLOWS:  
 A. NET UPLIFT = ULTIMATE UPLIFT - 10 PSF DEAD LOAD  
 B. NET UPLIFT = ALLOWABLE UPLIFT - 6 PSF DEAD LOAD

TABLE 3: ALLOWABLE STRESS (ASD) WIND PRESSURES

POSITIVE WIND PRESSURES ON WALLS & WALL OPENINGS					
WIND PRESSURE ZONE	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA
	≤ 10 SF	20 SF	50 SF	100 SF	≥ 200 SF
WALL ZONE ④	24.6 PSF	23.5 PSF	22.0 PSF	21.0 PSF	19.9 PSF
WALL ZONE ⑤	24.6 PSF	23.5 PSF	22.0 PSF	21.0 PSF	19.9 PSF

NEGATIVE WIND PRESSURES ON WALLS & WALL OPENINGS					
WIND PRESSURE ZONE	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA
	≤ 10 SF	20 SF	50 SF	100 SF	≥ 200 SF
WALL ZONE ④	-26.6 PSF	-25.5 PSF	-24.1 PSF	-23.0 PSF	-21.9 PSF
WALL ZONE ⑤	-32.8 PSF	-30.6 PSF	-27.7 PSF	-25.5 PSF	-23.4 PSF

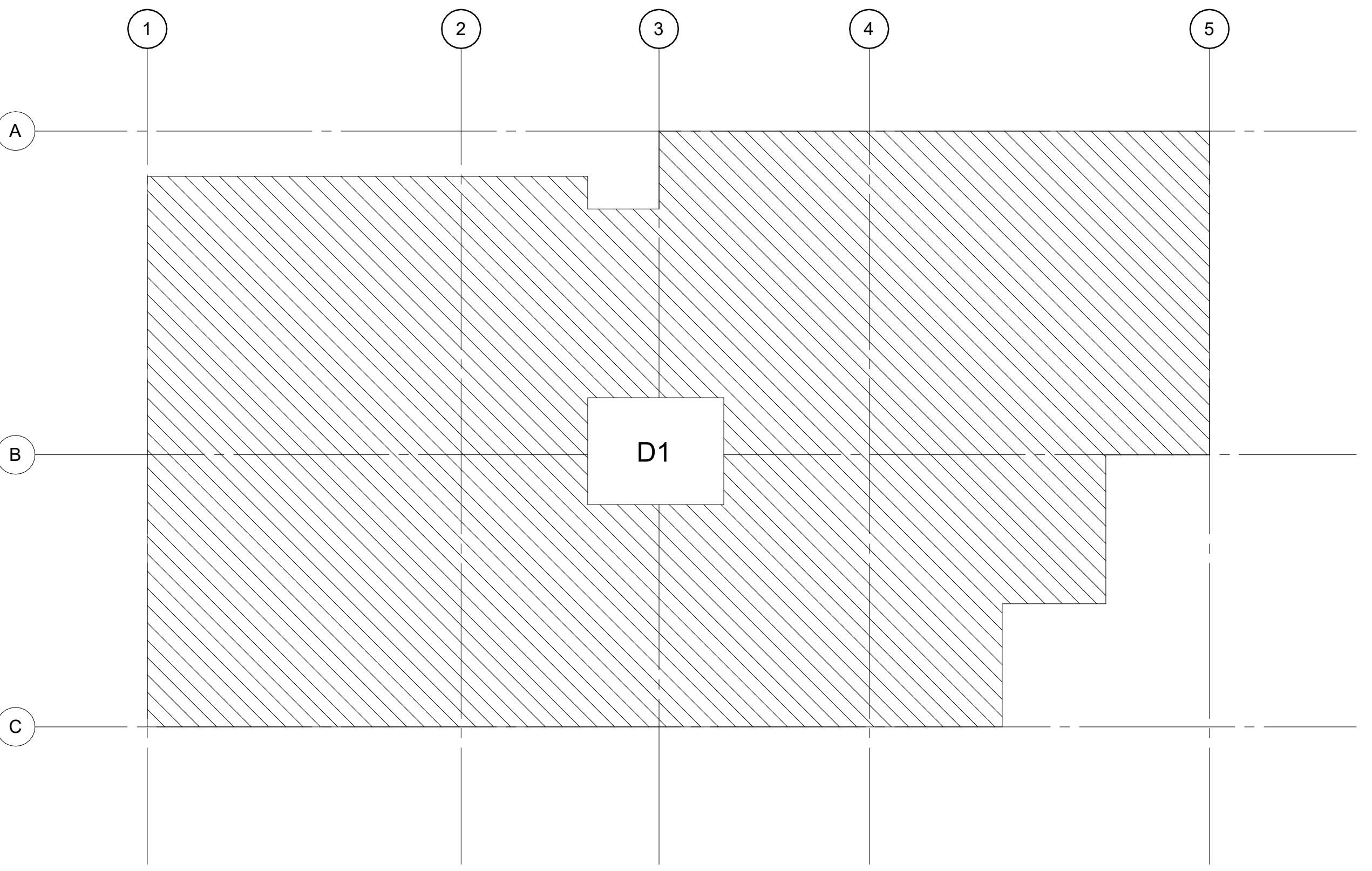
  

POSITIVE WIND PRESSURES ON ROOFING & ROOF FRAMING					
WIND PRESSURE ZONE	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA
	≤ 10 SF	20 SF	50 SF	100 SF	≥ 200 SF
ROOF ZONE ①	10.9 PSF	10.2 PSF	9.6 PSF	9.6 PSF	9.6 PSF
ROOF ZONE ②	10.9 PSF	10.2 PSF	9.6 PSF	9.6 PSF	9.6 PSF
ROOF ZONE ③	10.9 PSF	10.2 PSF	9.6 PSF	9.6 PSF	9.6 PSF

NEGATIVE WIND PRESSURES ON ROOFING & ROOF FRAMING					
WIND PRESSURE ZONE	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA
	≤ 10 SF	20 SF	50 SF	100 SF	≥ 200 SF
ROOF ZONE ①	-24.6 PSF	-24.6 PSF	-24.6 PSF	-24.6 PSF	-21.1 PSF
ROOF ZONE ②	-42.8 PSF	-39.9 PSF	-36.2 PSF	-33.4 PSF	-30.6 PSF
ROOF ZONE ③	-56.4 PSF	-52.8 PSF	-48.0 PSF	-44.4 PSF	-40.7 PSF
ROOF ZONE ④	-76.9 PSF	-69.6 PSF	-60.0 PSF	-52.8 PSF	-45.5 PSF

NOTES:  
 1. WIND PRESSURES IN THE TABLES ABOVE ARE BASED ON CALCULATIONS FROM ASCE 7-16.  
 2. OVERHANG PRESSURES IN THE TABLES ABOVE SHALL APPLY TO ALL ROOFS OVER BALCONIES, BREEZEWAYS, AND COVERED ENTRIES. COORDINATE WITH ARCHITECTURAL DRAWINGS.  
 3. WHEN THE BASIC WIND SPEED (ULT) IN TABLE 1, DESIGN LOADS & DESIGN CRITERIA IS 140 MPH OR HIGHER, PROVIDE IMPACT RESISTANT GLAZING AS REQUIRED FOR WIND BORNE DEBRIS PER FLORIDA BUILDING CODE.  
 4. MAXIMUM ALLOWABLE DEAD LOADS TO BE USED TO RESIST UPLIFT SHALL BE AS FOLLOWS:  
 A. NET UPLIFT = ULTIMATE UPLIFT - 10 PSF DEAD LOAD  
 B. NET UPLIFT = ALLOWABLE UPLIFT - 6 PSF DEAD LOAD



ROOF DECKING LAYOUT PLAN

ROOF DECK SCHEDULE				
ZONE	DECK TYPE	SUPPORT FASTENERS	FASTENER PATTERN	FASTENER PER SPAN
D1	1-1/2" - 20 GA - TYPE 'B'	5/8" DIA. PUDDLE WELDS	36/5	#12-14 TEK SCREWS

NOTES:  
 1. FASTEN DECK EDGE SIDELAP AT PERIMETER WITH 5/8" DIA. PUDDLE WELDS AT 6" O.C.  
 2. FASTEN DECK EDGE SUPPORT AT PERIMETER WITH 5/8" DIA. PUDDLE WELDS TO MATCH FASTENER PATTERN AS NOTED IN SCHEDULE.  
 3. REFERENCE FASTENER PATTERNS AND SIDELAP FASTENER SPACING DETAILS SHOWN IN DETAILS BELOW.

Owner: \_\_\_\_\_

Revised: \_\_\_\_\_

Scale: AS NOTED

Date: 12/15/23

Drawn By: SGZ

Checked By: JDC

**WEST MARKET LOT 10**

WEST MARKET LOT 10  
14230 WEST COLONIAL DRIVE  
WINTER GARDEN, FL

**DESIGN LOAD DATA**

Certified by **JOHNATHAN D. COLLINS**  
Professional Engineer  
No 74693

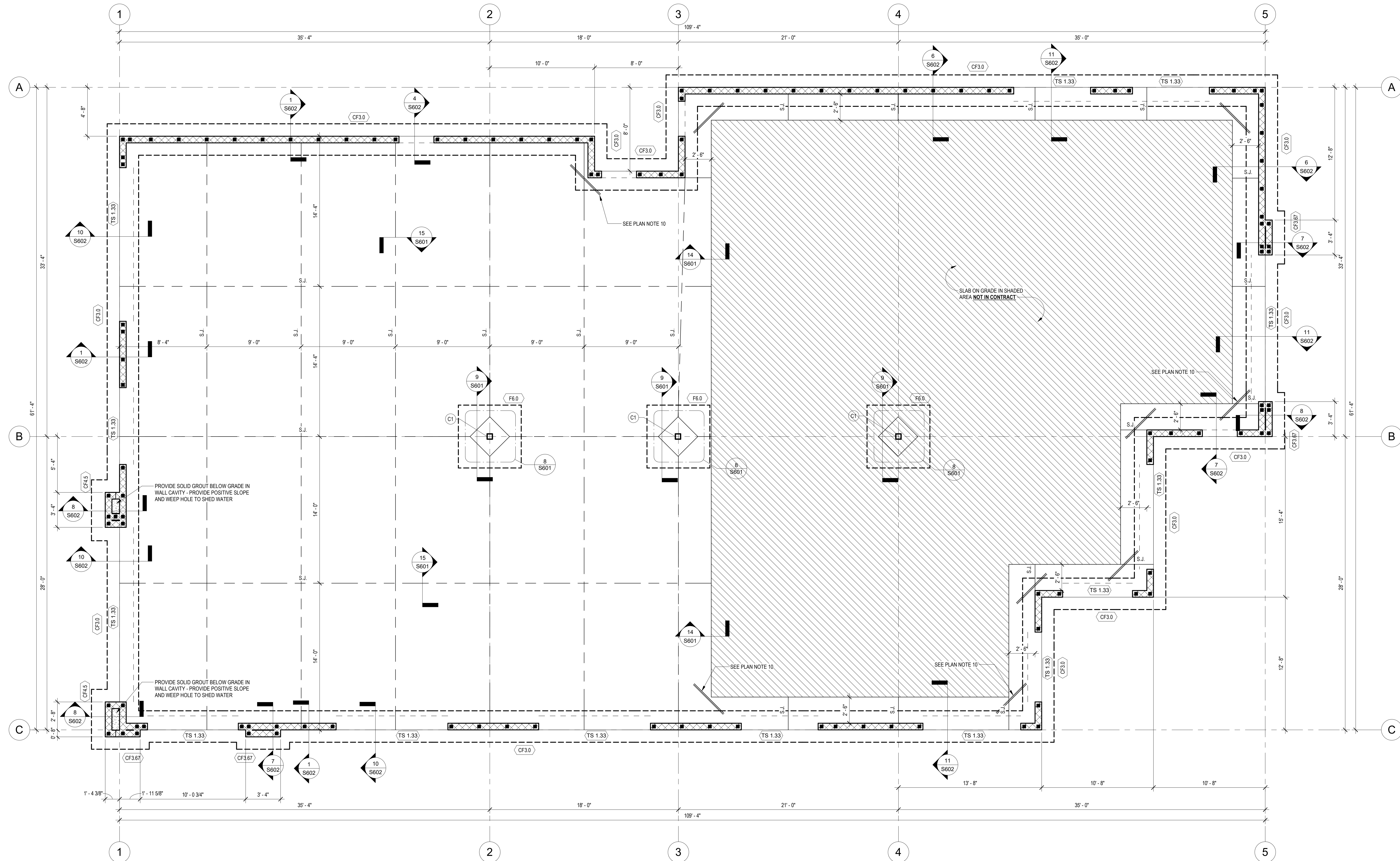
Electronic Signature:  
 THIS DRAWING HAS BEEN ELECTRONICALLY SIGNED AND SEALED BY JOHNATHAN D. COLLINS, P.E. (LICENSE NO. 74693 ON 12/15/2023). PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED, AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPY.

The drawing has been electronically signed and sealed on the date shown in the seal, using a digital signature. Printed copies of this document are not considered signed and sealed, and the signature must be verified on any electronic copy.

Drawing Number: **S002**

Job Number: \_\_\_\_\_

A/E Job Number: **23432**



**1 FOUNDATION PLAN**  
S101 1/4" = 1'-0"

**LEGEND**

- EL - #'-##" INDICATES TOP OF CONCRETE FOOTING ELEVATION.
- T.O. FTG INDICATES TOP OF FOOTING.
- FR# INDICATES SPREAD FOOTING TYPE. SEE FOUNDATION SCHEDULE FOR SIZE AND REINFORCEMENT.
- CF# INDICATES CONTINUOUS FOOTING TYPE. SEE FOUNDATION SCHEDULE FOR SIZE AND REINFORCEMENT.
- TS# INDICATES THICKENED SLAB EDGE TYPE. SEE FOUNDATION SCHEDULE FOR SIZE AND REINFORCEMENT.
- CF INDICATES STEEL COLUMN TYPE. SEE STEEL COLUMN SCHEDULE FOR COLUMN SIZE AND BASEPLATE DETAILS.
- S.F. INDICATES STEPPED FOOTING PER DETAILS 2/S601 AND 3/S601.
- S.J. INDICATES SLAB SAWCUT JOINT PER DETAIL 1/S602.
- C.S.J. INDICATES CONSTRUCTION JOINT PER TYPICAL DETAIL 2/S602 OR AS REQUIRED PER CONSTRUCTION SEQUENCING.
- INDICATES 8" REINFORCED MASONRY WALL - PROVIDE VERT #5 AT 32" O.C. (MAX) U.N.O. GROUT WALLS SOLID BELOW SLAB.
- INDICATES VERTICAL REINFORCEMENT AT CENTER OF CMU WALL GROUDED SOLID. BAR SIZE MUST MATCH WALL REINF. NOTED.
- SLAB-ON-GRADE NOT IN CONTRACT. SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATION.

**FOUNDATION PLAN NOTES:**

1. REFERENCE THE STRUCTURAL GENERAL NOTES ON DRAWINGS S001 & S002. GENERAL NOTES INCLUDE CODES AND STANDARDS, DESIGN LOADS AND OTHER REQUIREMENTS.
2. CONTRACTOR TO VERIFY ALL ELEVATIONS AND DIMENSIONS SHOWN WITH ARCHITECTURAL DRAWINGS AND EQUIPMENT SUPPLIERS SHOP DRAWINGS PRIOR TO FABRICATION AND/OR START OF CONSTRUCTION.
3. COORDINATE EXISTING / INSTALLED UNDERGROUND UTILITIES AND OTHER BURIED PIPES AND CONDUITS PRIOR TO PLACEMENT OF FOOTINGS. DO NOT PLACE BUILDING FOUNDATIONS OVER EXISTING / INSTALLED PIPES AND CONDUITS UNLESS APPROVED OTHERWISE.
4. T.O. FOOTING ELEVATION IS AT -1'-4" (U.N.O.) THIS IS A REFERENCE ELEVATION ONLY. SEE FOUNDATION DETAIL SHEETS AND SCHEDULES FOR FOUNDATION SIZE AND REINFORCEMENT.
5. EXTEND ALL CONTINUOUS FOOTING REINFORCEMENT INTO ADJACENT SPREAD FOOTINGS A MINIMUM DISTANCE OF 4'-0".
6. ALL WALLS AND COLUMNS ARE TO BE CENTERED ON FOUNDATIONS UNLESS NOTED OTHERWISE. SEE PLAN DIMENSIONS FOR OFFSETS.
7. PREPARE THE SLAB SUB-BASE AND COMPACT THE SOIL PER THE PROJECT GEOTECHNICAL REPORT, THE CIVIL DRAWINGS, AND THE STRUCTURAL GENERAL NOTES. IF ANY OF THESE DRAWINGS OR NOTES ARE IN CONFLICT, THE CONTRACTOR MUST ALERT THE ENGINEER FOR CLARIFICATION PRIOR TO START OF CONSTRUCTION.
8. T.O. SLAB ELEVATION IS AT 0'-0" (U.N.O.) THIS IS A REFERENCE ELEVATION ONLY. SEE CIVIL ELEVATIONS FOR SITE FINISHED FLOOR ELEVATION PER NAVD83. SEE FOUNDATION AND SLAB ON GRADE DETAIL SHEETS.
9. SLAB ON GROUND SHALL BE A 4" THICK MINIMUM CONCRETE. UNLESS NOTED OTHERWISE REINFORCE SLAB WITH 3.0 LBS / CUBIC YARD OF FIBER MESH. (ALTERNATE REINFORCEMENT TO FIBER MESH SHALL BE 6#6 - W1.4W1.4 WELDED WIRE MESH; PROVIDE A 15 MIL VAPOR BARRIER ON TERMITE TREATED COMPACTED SUBGRADE. SEE PLAN FOR SAW-CUT JOINT SPACING.
10. PROVIDE (2) #4 x 4'-0" LG BARS AT TOP OF SLAB AT ALL RE-ENTRANT CORNERS AND DISCONTINUOUS ENDS OF SLAB SAW-CUT JOINTS. SEE DETAIL 6/S602.
11. REFERENCE THE ARCHITECTURAL DRAWINGS FOR SLAB EDGES, FLOOR SLOPES, WALL OPENINGS, AND OTHER DIMENSIONS NOT GIVEN. CONTRACTOR MUST COORDINATE AND VERIFY ALL DIMENSIONS WITH PROJECT ARCHITECT PRIOR TO FABRICATION.

Owner:  
  
**G4 ARCHITECTURE**  
 135 W. Central Blvd., Suite 400  
 Orlando, Florida 32801  
 TEL: 407.363.6136  
 © Copyright 2023

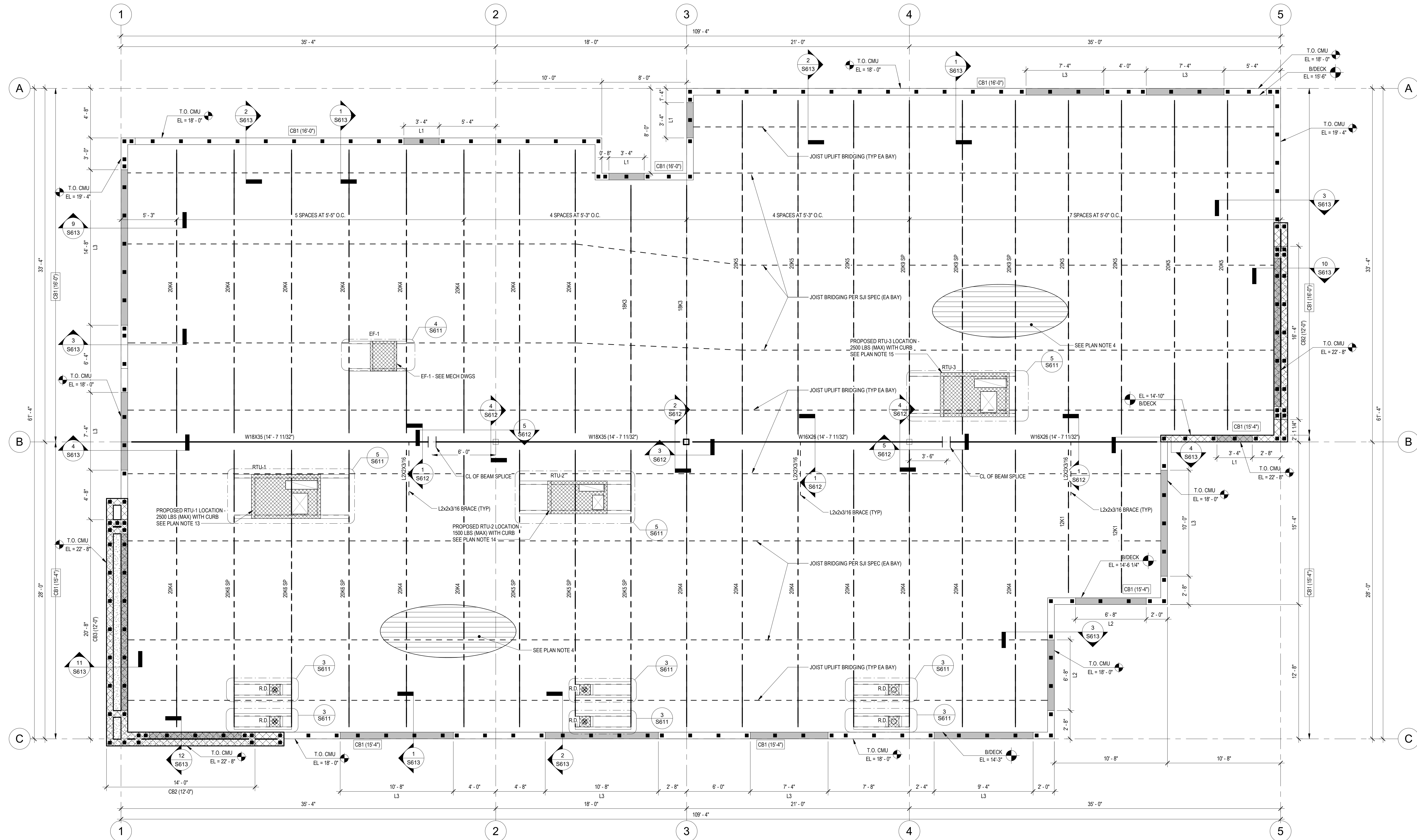
Revisions:

Scale:	AS NOTED
Date:	12/15/23
Drawn By:	SGZ
Checked By:	JDC

**WEST MARKET LOT 10**  
 WEST MARKET LOT 10  
 14230 WEST COLONIAL DRIVE  
 WINTER GARDEN, FL  
**FOUNDATION PLAN**

Certified Professional Engineer  
  
**JONATHAN D. COLLINS**  
 No. 74693  
 STATE OF FLORIDA  
 PROFESSIONAL ENGINEER

Electronic Signature:  
  
 This drawing has been electronically signed and sealed by Jonathan D. Collins, P.E. License No. 74693 on 12/15/2023. Printed copies of this document are not considered signed and sealed, and the signature must be verified on any electronic copies.  
 Drawing Number:  
**S101**  
 Job Number:  
 A/E Job Number:  
**23432**



**1 ROOF FRAMING PLAN**  
 1/4" = 1'-0"

**LEGEND**

- (#'-##") INDICATES BOTTOM OF DECK ELEVATION. SEE PLAN.
- c = #" INDICATES REQUIRED POSITIVE CAMBER IN W-BEAM.
- CB INDICATES STEEL COLUMN TYPE. SEE STEEL COLUMN SCHEDULE FOR COLUMN SIZE AND BASEPLATE DETAILS.
- CCB INDICATES CONCRETE COLUMN TYPE. SEE CONCRETE COLUMN SCHEDULE FOR COLUMN SIZE AND REINFORCEMENT DETAILS.
- R.D. ⊗ INDICATES ROOF DRAIN LOCATIONS. REFER TO ARCHITECTURAL AND MEP DRAWINGS FOR COORDINATION.
- |— INDICATED BEAM SPICE CENTERLINE

**ROOF FRAMING PLAN NOTES:**

1. REFERENCE THE STRUCTURAL GENERAL NOTES ON DRAWINGS S001 & S002. GENERAL NOTES INCLUDE CODES AND STANDARDS, DESIGN LOADS AND OTHER REQUIREMENTS.
2. TOP OF STEEL ELEVATIONS MAY VARY. SEE THE STRUCTURAL AND ARCHITECTURAL PLANS AND SECTIONS FOR REQUIRED ELEVATIONS. (#'##") INDICATES TOP OF STEEL ELEVATION.
3. CONTRACTOR SHALL VERIFY ALL ELEVATIONS AND DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS WITH THE ARCHITECTURAL DRAWINGS AND EQUIPMENT SUPPLIER'S SHOP DRAWINGS PRIOR TO FABRICATION AND/OR START OF CONSTRUCTION.
4. ROOF DECK SHALL BE GALV. 1-1/2" - 20GA WIDE RIB METAL DECK, (L.I.N.O.) SPANNING OVER OPEN WEB STEEL JOISTS SPACED AT 6'-0" O.C. (MAX) OR AS INDICATED ON ROOF FRAMING PLAN.  
 DECK SECTION PROPERTIES:  
 Ip = 0.201 in<sup>4</sup>/ft  
 Ix = 0.222 in<sup>4</sup>/ft  
 Sy = 0.234 in<sup>3</sup>/ft  
 Sx = 0.247 in<sup>3</sup>/ft  
 Fy = 50 KSI
5. FASTEN ROOF DECK TO ALL SUPPORTS w/ 5/8" DIA. PUDDLE WELDS AND ATTACH DECK SIDELAPS w/ #12 TEK SCREWS. REFERENCE THE ROOF DECK FASTENING SCHEDULE AND ROOF DECKING PLAN PROVIDED ON SHEET S002.
6. PROVIDE 1/4"x1/4" EDGE ANGLE AROUND ALL OPENINGS AND AROUND PERIMETER OF ROOF. COORDINATE WITH ARCHITECTURAL DRAWINGS FOR EXACT LOCATION OF EDGE ANGLES. SEE DECK ANGLE SPURCE DETAILS ON SHEET S611.
7. (5' L) (5' R) OR (5' LR) INDICATE 5' JOIST SEAT AT END OF STEEL JOIST AS NOTED IN LIEU OF STANDARD 2-1/2" SEAT. STEEL DETAILER SHALL COORDINATE TO STEEL ELEVATIONS WITH JOIST MANUFACTURER.
8. OPEN WEB STEEL JOISTS SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE SPECIFICATION OF THE STEEL JOIST INSTITUTE. PROVIDE JOIST REINFORCEMENT PER TYPICAL DETAIL 15S611 AT CONCENTRATED LOADS > 150 LBS.
9. PROVIDE L2x2x3/16 BRACE FROM JOIST TO BOTTOM FLANGE OF STEEL BEAM AT BEAM MID-SPAN OR AS NOTED ON PLAN. SEE DETAIL 15S62.
10. PROVIDE STANDARD JOIST BRIDGING AND UPLIFT BRIDGING PER LATEST SJI SPECIFICATIONS AND THE STEEL JOIST SHOP DRAWINGS (TYPICAL).
11. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS, ELEVATIONS AND DETAILS NOT SHOWN. RESOLVE ALL DISCREPANCIES PRIOR TO FABRICATION.
12. COORDINATE ALL ROOF OPENINGS AND MISCELLANEOUS ROOF DECK PENETRATIONS AND EQUIPMENT WITH THE ARCHITECTURAL AND MEP DRAWINGS. REFERENCE THE TYPICAL ROOF OPENING DETAILS ON SHEET S611.
13. JOIST NOTED AS "20K6 SP" ON PLAN SHALL BE DESIGNED FOR PROJECT TYPICAL UNIFORM LOADING AS WELL AS SERVICE CONCENTRATED LOADS AS NOTED AT THE CURB FRAMES.  
 DEAD LOAD: 600LBS  
 WIND LOAD: +/-360LBS
14. JOIST NOTED AS "20K9 SP" ON PLAN SHALL BE DESIGNED FOR PROJECT TYPICAL UNIFORM LOADING AS WELL AS SERVICE CONCENTRATED LOADS AS NOTED AT THE CURB FRAMES.  
 DEAD LOAD: 450LBS  
 WIND LOAD: +/-450LBS
15. JOIST NOTED AS "20K6 SP" ON PLAN SHALL BE DESIGNED FOR PROJECT TYPICAL UNIFORM LOADING AS WELL AS SERVICE CONCENTRATED LOADS AS NOTED AT THE CURB FRAMES.  
 DEAD LOAD: 600LBS  
 WIND LOAD: +/-360LBS

Revised:

Scale:	AS NOTED
Date:	12/15/23
Drawn By:	SGZ
Checked By:	JDC

**WEST MARKET LOT 10**  
 WEST MARKET LOT 10  
 14230 WEST COLONIAL DRIVE  
 WINTER GARDEN, FL  
**ROOF FRAMING PLAN**

Certified Professional Engineer  
**JONATHAN D. COLLINS**  
 No 74693  
 STATE OF FLORIDA  
 PROFESSIONAL ENGINEER

Electronic Signature:  
 THIS DRAWING HAS BEEN ELECTRONICALLY SIGNED AND SEALED BY JONATHAN D. COLLINS, P.E. (FL LICENSE NO. 74693 ON 12/15/2023). PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED, AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPY.

*JDCollins*

Drawing Number:  
**S121**  
 Job Number:  
**23432**  
 A/E Job Number:  
**23432**

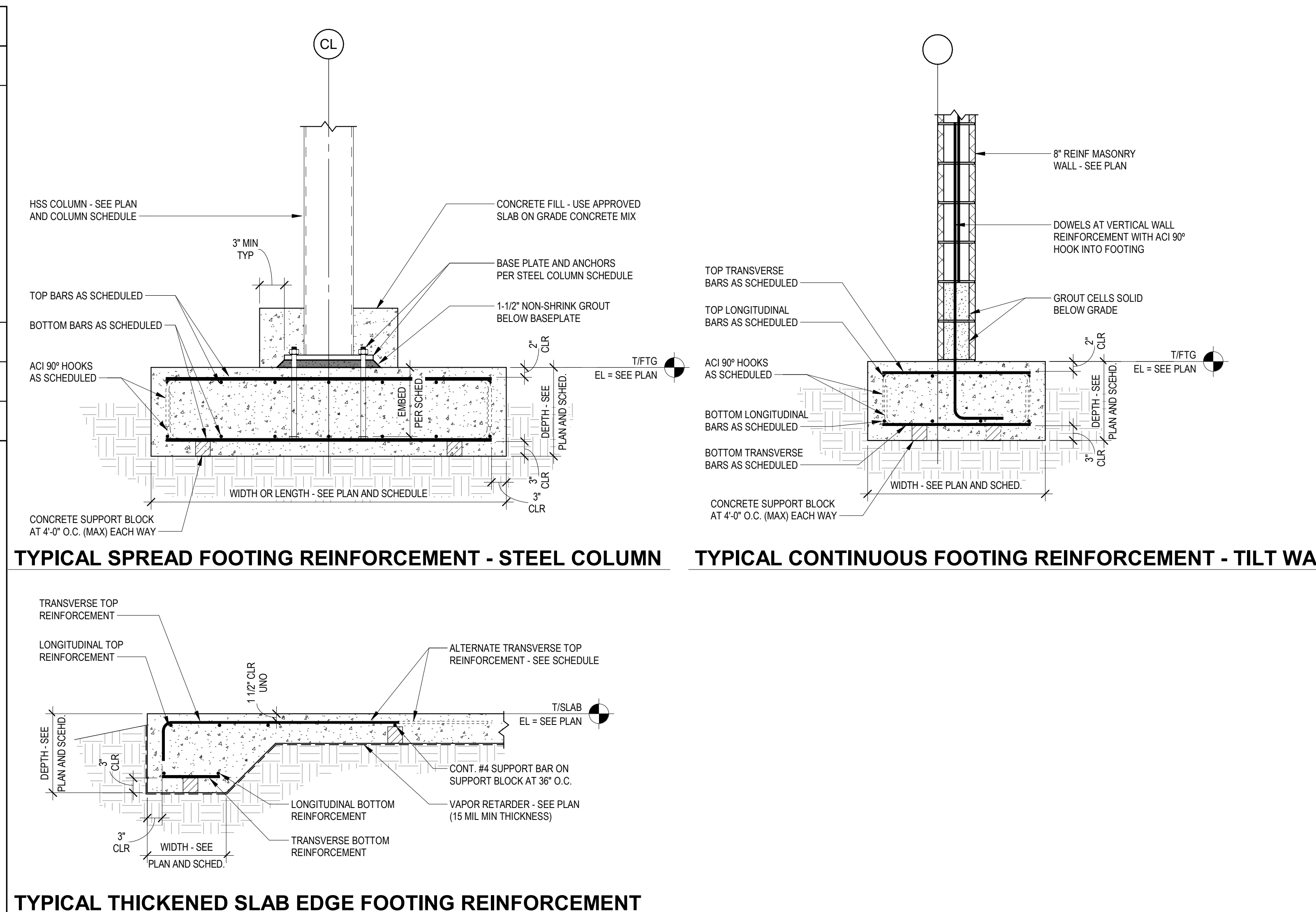


STEEL COLUMN AND BASE PLATE SCHEDULE	
TYPE	HSS 6x6x1/16
BASE PLATE PLAN	
BASE PLATE	PL 1" x 12" x 12"
ANCHOR BOLTS	(4) 3/4" DIA. ANCHORS 12" EMBEDMENT
COLUMN MARK	C1

**COLUMN AND BASE PLATE NOTES:**

- PROVIDE 1/4" CAP PLACES ON ALL TUBE AND PIPE COLUMNS THAT ARE NOT LOADED THROUGH THE CAP PLATE.
- ALL BASE PLATE AND SHEAR LUG MATERIAL TO BE ASTM A572 GRADE 50, UNLESS NOTED OTHERWISE.
- ALL ANCHORS TO BE ASTM F1554, 55 KSI, S1 WELDABLE MATERIAL UNLESS NOTED OTHERWISE.
- SET ALL BASE PLATES ON WEDGES, SHIMS, OR LEVELING NUTS AS REQUIRED. FULL BED OF NON-METALLIC, NON-SHRINK, HIGH STRENGTH GROUT MUST BE INSTALLED BELOW BASE PLATE. GROUT MUST BE INSTALLED IMMEDIATELY AFTER ERECTION OF MAIN JOIST GIRDERS AND JOISTS AT COLUMN LINES.
- ALL WELDS ARE CONTINUOUS AROUND COLUMN BASE.
- UNLESS OTHERWISE NOTED, PROVIDE 5/16" FILLET WELDS AT COLUMN MATERIAL GREATER THAN 1/2" WALL THICKNESS. PROVIDE 1/4" FILLET WELD OTHERWISE.

**TYPICAL COLUMN ANCHOR BOLT DETAIL**

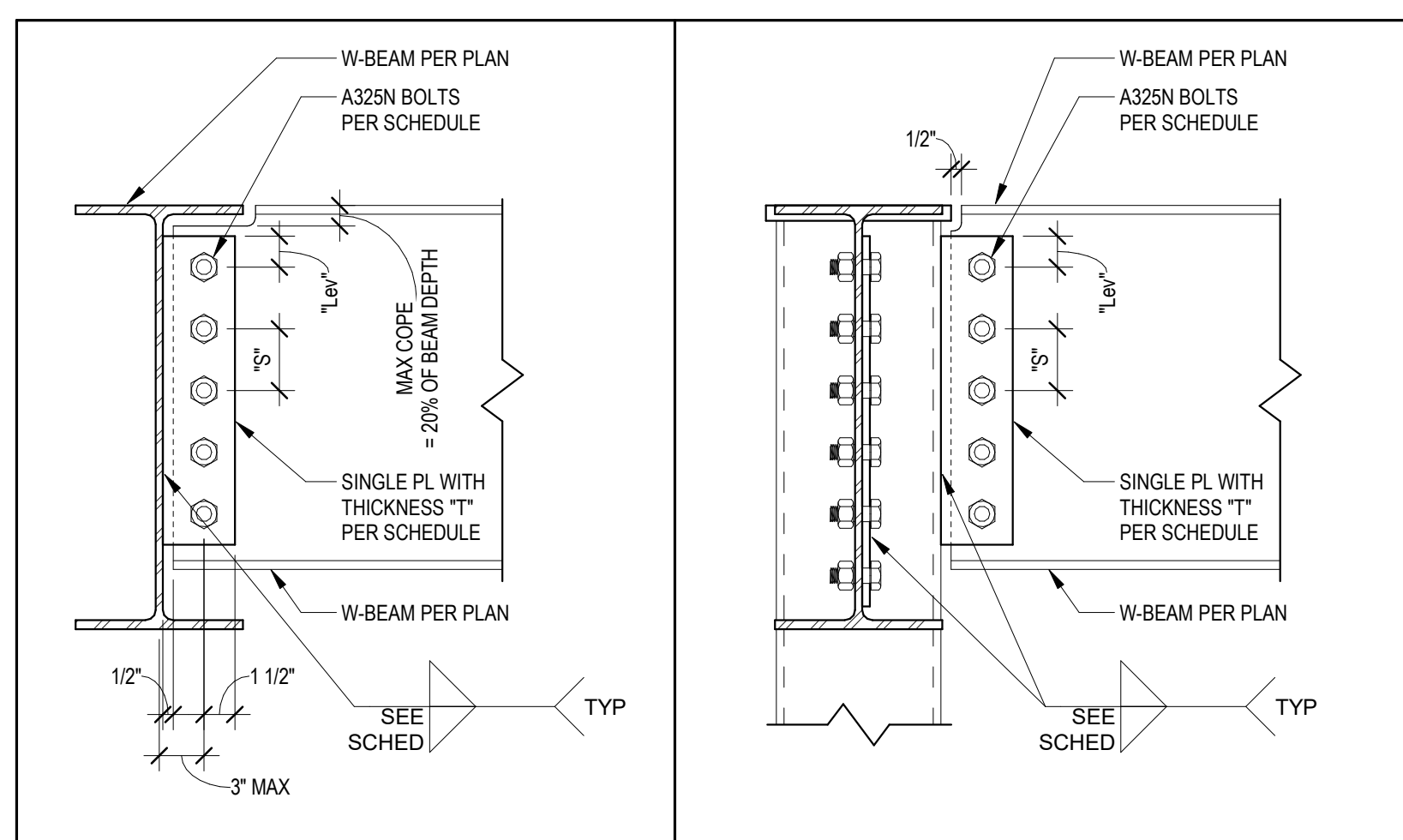


CONCRETE FOOTING SCHEDULE					
TYPE	SIZE			REINFORCING	REMARKS
	WIDTH	LENGTH	DEPTH		
F6.0	6'-0"	6'-0"	2'-6"	(7) #7 x 5'-6" LG. EA WAY TOP (7) #7 x 5'-6" LG. EA WAY BOTTOM	SPREAD FOOTING
CF 3.0	3'-0"	CONT.	1'-0"	LONG. (4) #5 x CONT. BOTTOM TRANS. #4 x 2'-6" LG. AT 16" O.C. BOTTOM	CONTINUOUS FOOTING
CF 3.67	3'-8"	CONT.	1'-0"	LONG. (4) #5 x CONT. BOTTOM TRANS. #4 x 3'-2" LG. AT 16" O.C. BOTTOM	CONTINUOUS FOOTING
CF 4.3	4'-6"	CONT.	1'-0"	LONG. (5) #5 x CONT. BOTTOM TRANS. #4 x 4'-0" LG. AT 16" O.C. BOTTOM	CONTINUOUS FOOTING
TS 1.33	1'-4"	CONT.	1'-4"	LONG. (2) #5 x CONT. TOP LONG. (2) #5 x CONT. BOTTOM ALT. TRANS. #4 x 4'-0" & 5'-6" LG. w/ ACI 90° HOOK AT 18" O.C. TOP #4 x 1'-0" LG. SKEWED SUPPORT BAR AT 36" O.C. BOT	THICKENED EDGE SLAB

TENSION DEVELOPMENT LENGTHS OF ACI STANDARD HOOKS FOR UNCOATED BARS							
BAR SIZE	3000 PSI	3500 PSI	4000 PSI	5000 PSI	6000 PSI	7000 PSI	≥8000 PSI
#3	10	9	9	8	7	7	6
#4	13	12	12	10	10	9	8
#5	17	16	15	13	12	11	10
#6	20	19	17	16	14	13	12
#7	23	22	20	18	17	15	14
#8	27	25	23	21	19	18	16
#9	30	28	26	23	21	20	18
#10	34	31	29	26	24	22	21
#11	37	35	32	29	27	25	23
#14	45	42	39	35	32	29	28
#18	60	55	52	46	45	39	37

**NOTES:**

- TABULATED VALUES ARE BASED ON GRADE 60 REINFORCING BARS AND NORMAL WEIGHT CONCRETE. ALL VALUES ARE LENGTHS IN INCHES.
- TENSION DEVELOPMENT LENGTHS OF STANDARD HOOKS ARE CALCULATED PER ACI 318-14, SECTION 25.4.3.
- FOR BAR SIZES #3 THROUGH #11 ONLY, THE FOLLOWING FURTHER REDUCTIONS IN LENGTH CAN BE APPLIED:
  - IF CONCRETE COVER CONFORMS TO ACI 318-14, SECTION 25.4.3.2
    - A MODIFICATION FACTOR OF 0.7 MAY BE APPLIED, HOWEVER:
      - THE FINAL CALCULATED LENGTH OF THE HOOK SHALL NOT BE LESS THAN EITHER 8.0 (in) **NOR** 6 INCHES.
    - IF HOOK IS ENCLOSED IN TIES OR STIRRUPS PER ACI 318-14, SECTION 25.4.3.2:
      - A MODIFICATION FACTOR OF 0.8 MAY BE APPLIED, HOWEVER:
        - THE FINAL CALCULATED LENGTH OF THE HOOK SHALL NOT BE LESS THAN EITHER 8.0 (in) **NOR** 6 INCHES.
  - FOR LIGHTWEIGHT AGGREGATE CONCRETE, ALL TABULATED VALUES SHALL BE MULTIPLIED BY A FACTOR OF 1.3.

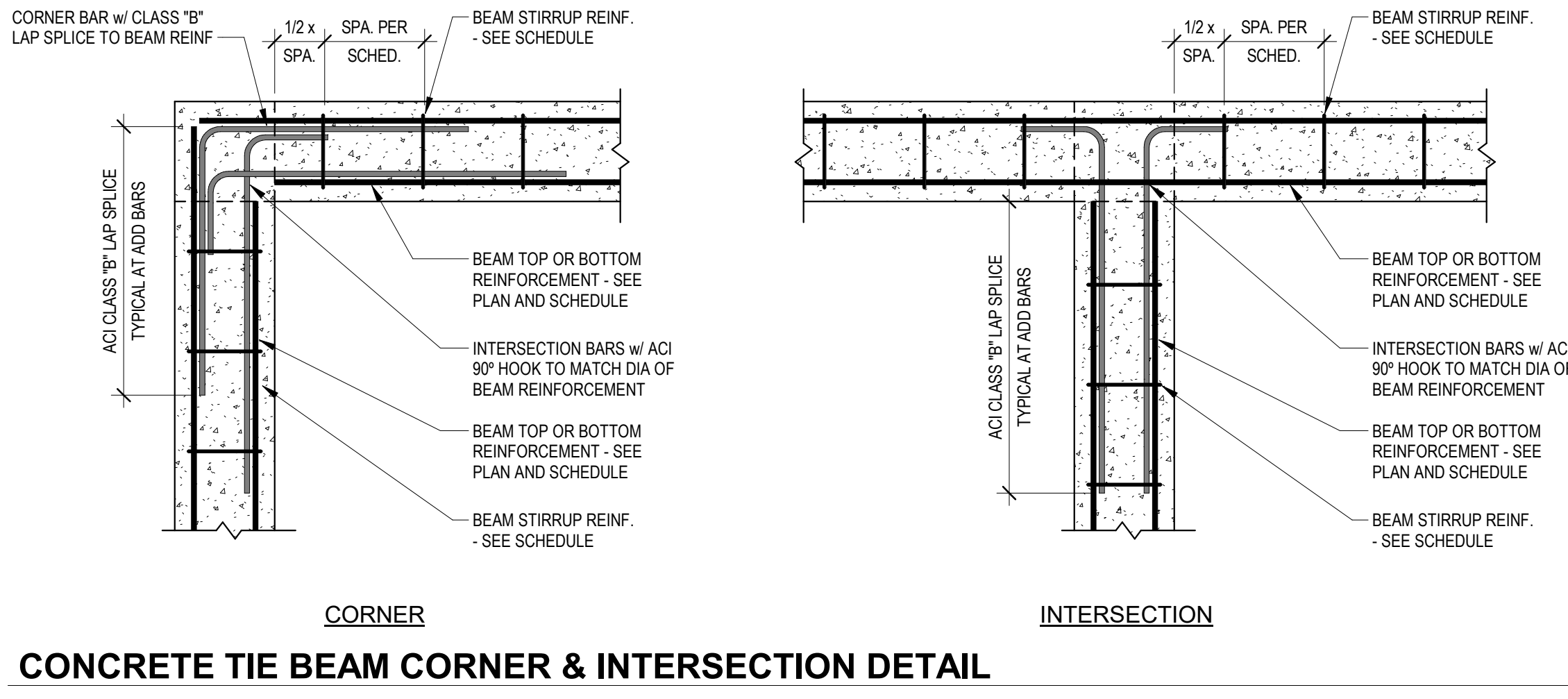


CAPACITY (KIPS) (FACTORED)	BEAM SIZE	NO. OF A325N BOLTS	HOLE TYPE	PLATE DIMENSIONS		BOLT SPACING (S)	MIN. EDGE DISTANCE (L <sub>min</sub> )	FILLET WELD SIZE (EA SIDE)
				T	L			
25	W8	(2) 3/4" DIA.	SSLT	3/8"	6"	3"	1 1/2"	1/4"
44	W10	(3) 3/4" DIA.	SSLT	3/8"	8"	2 3/4"	1 1/4"	1/4"
44	W12	(3) 3/4" DIA.	SSLT	3/8"	9"	3"	1 1/2"	1/4"
44	W14	(3) 3/4" DIA.	SSLT	3/8"	9"	3"	1 1/2"	1/4"
63	W16	(4) 3/4" DIA.	SSLT	3/8"	12"	3"	1 1/2"	1/4"
82	W18	(5) 3/4" DIA.	SSLT	3/8"	15"	3"	1 1/2"	1/4"
89	W21	(6) 3/4" DIA.	SSLT	5/16"	18"	3"	1 1/2"	1/4"
108	W24	(7) 3/4" DIA.	SSLT	5/16"	21"	3"	1 1/2"	1/4"
127	W27	(8) 3/4" DIA.	SSLT	5/16"	24"	3"	1 1/2"	1/4"
142	W30	(9) 3/4" DIA.	SSLT	5/16"	27"	3"	1 1/2"	1/4"
157	W33	(10) 3/4" DIA.	SSLT	5/16"	30"	3"	1 1/2"	1/4"
173	W36	(11) 3/4" DIA.	SSLT	5/16"	33"	3"	1 1/2"	1/4"

**STD** STANDARD HOLES  
**SSLT** SHORT-SLOTTED HOLES (TRANSVERSE TO DIRECTION OF LOAD)  
**N** THREADS INCLUDED IN THE SHEAR PLANE (ALLOWED)  
**X** THREADS EXCLUDED FROM THE SHEAR PLANE

**VALUES ARE BASED ON TABLE 10-10b OF THE AISC STEEL CONSTRUCTION MANUAL - 15TH ED. VALUES ARE BASED ON A MINIMUM WEB THICKNESS OF 1/4"**

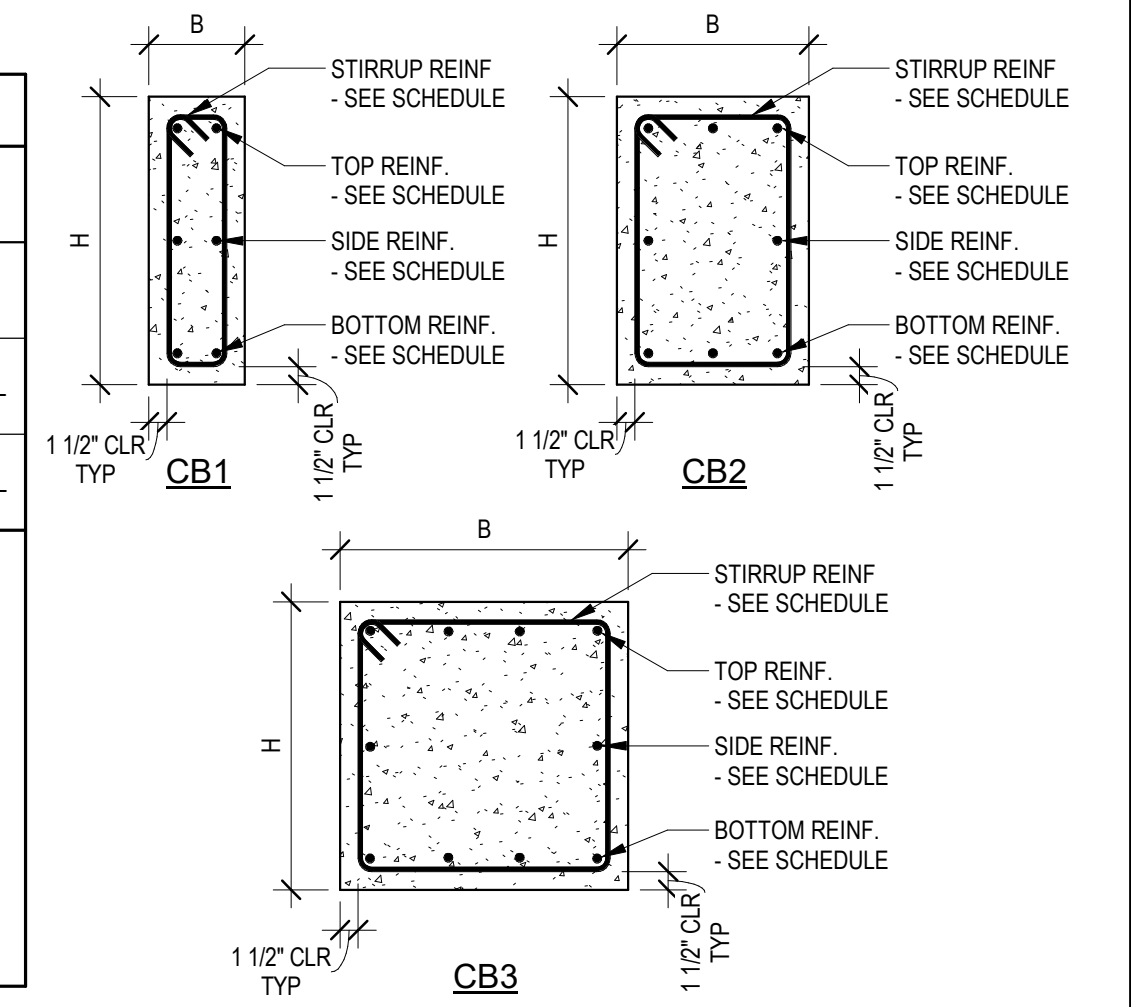
STEEL EMBED PLATE SCHEDULE				
EMBED MARK	PLATE / ANGLE SIZE	HEADED STUD QUANTITY AND LAYOUT	EMBED USE	COMMENTS
EP01	L5x3-1/2x3/8 x 0'-10" LG. (LLH)	(2) 5/8" DIA. x 6" LG. HSA - AT 45°	TYP JOIST TO THE BEAM CONNECTION	----
EP02	PL 1/2" x 12" x 1'-4" LG.	(6) 5/8" DIA. x 5" LG. HSA	STEEL BEAM TO PANEL CONNECTION	----
EP03	PL 1/2" x 12" x 1'-6" LG.	(6) 5/8" DIA. x 5" LG. HSA	STEEL BEAM TO PANEL CONNECTION	----



CONCRETE BEAM SCHEDULE						
MARK	SIZE		REINFORCING			REMARKS
	WIDTH	DEPTH	TOP	MIDDLE	BOTTOM	
CB1 - 8x24	8"	24"	(2) #5	(2) #5 EA FACE	(2) #5	#3 AT 12" OC 8" NOMINAL - 7.5/8" ACTUAL
CB2 - 16x24	16"	24"	(3) #5	(2) #5 EA FACE	(3) #5	#3 AT 12" OC 16" NOMINAL - 15.5/8" ACTUAL
CB3 - 24x24	24"	24"	(4) #6	(2) #5 EA FACE	(4) #6	#3 AT 12" OC 24" NOMINAL - 23.5/8" ACTUAL

**TYPICAL BEAM REINFORCING NOTES:**

- PROVIDE ACI 90 HOOK AT TOP AND BOTTOM REINFORCEMENT AT DISCONTINUOUS ENDS OF BEAMS (LNO).
- FIRST STIRRUP MUST START ONE-HALF OF THE INITIAL STIRRUP SPACING NOTED IN THE SCHEDULE AWAY FROM THE FACE OF SUPPORT. PROVIDE ACI 135 HOOK AT ALL STIRRUPS.
- WHERE REQUIRED, TOP REINFORCEMENT TO BE SPICED AT MID-SPAN OF BEAM AND BOTTOM REINFORCEMENT TO BE SPICED OVER TOP OF COLUMN SUPPORT. IN EACH CASE, PROVIDE ACI CLASS "B" LAP SPICE.
- SPICES IN SCHEDULED BARS LESS THAN 60'-0" IN LENGTH ARE NOT PERMITTED. SPICES IN SCHEDULED BARS MORE THAN 60'-0" IN LENGTHS SHALL BE SUBMITTED TO ENGINEER FOR APPROVAL.
- WHERE REQUIRED, EXTEND BARS THROUGH PERPENDICULAR GIRDER TO CANTILEVERED BEAM.



Owner:

Revisions:

Scale: AS NOTED  
 Date: 12/15/23  
 Drawn By: SGZ  
 Checked By: JDC

WEST MARKET LOT 10  
 WEST MARKET LOT 10  
 14230 WEST COLONIAL DRIVE  
 WINTER GARDEN, FL  
**SCHEDULES**

Certified by: **JOHNATHAN D. COLLINS**  
 LICENSE NO. 74693  
 STATE OF FLORIDA  
 PROFESSIONAL ENGINEER

Electronic Signature:  
 THIS DRAWING HAS BEEN DIGITALLY SIGNED AND SEALED BY JOHNATHAN D. COLLINS, P.E. LICENSE NO. 74693 ON 12/15/2023. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED. THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

Signature: *JDCollins*

This drawing has been electronically signed and sealed on the date shown in the seal, unless a digital signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Drawing Number:  
**S501**

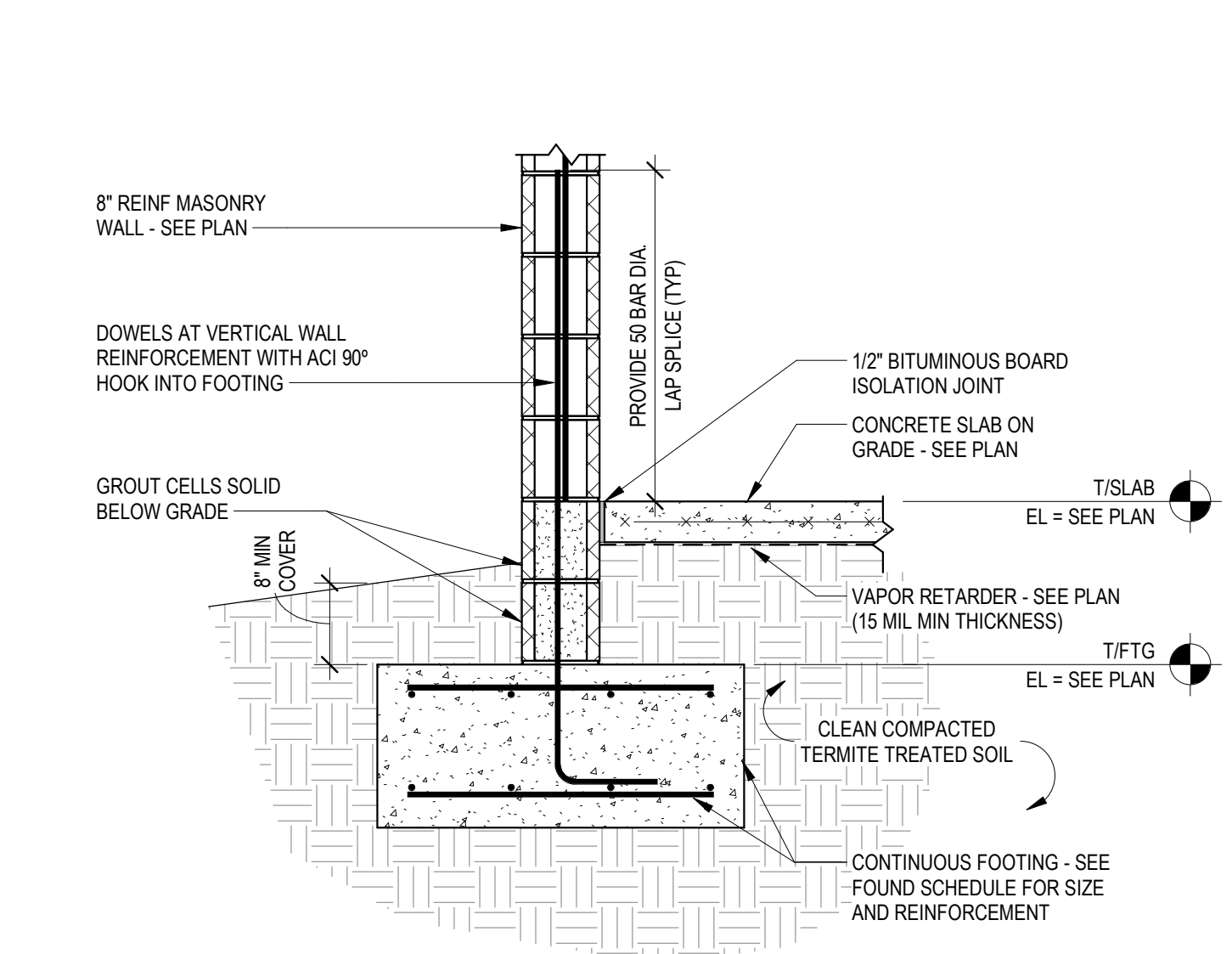
Job Number:  
 23432

A/E Job Number:  
 23432

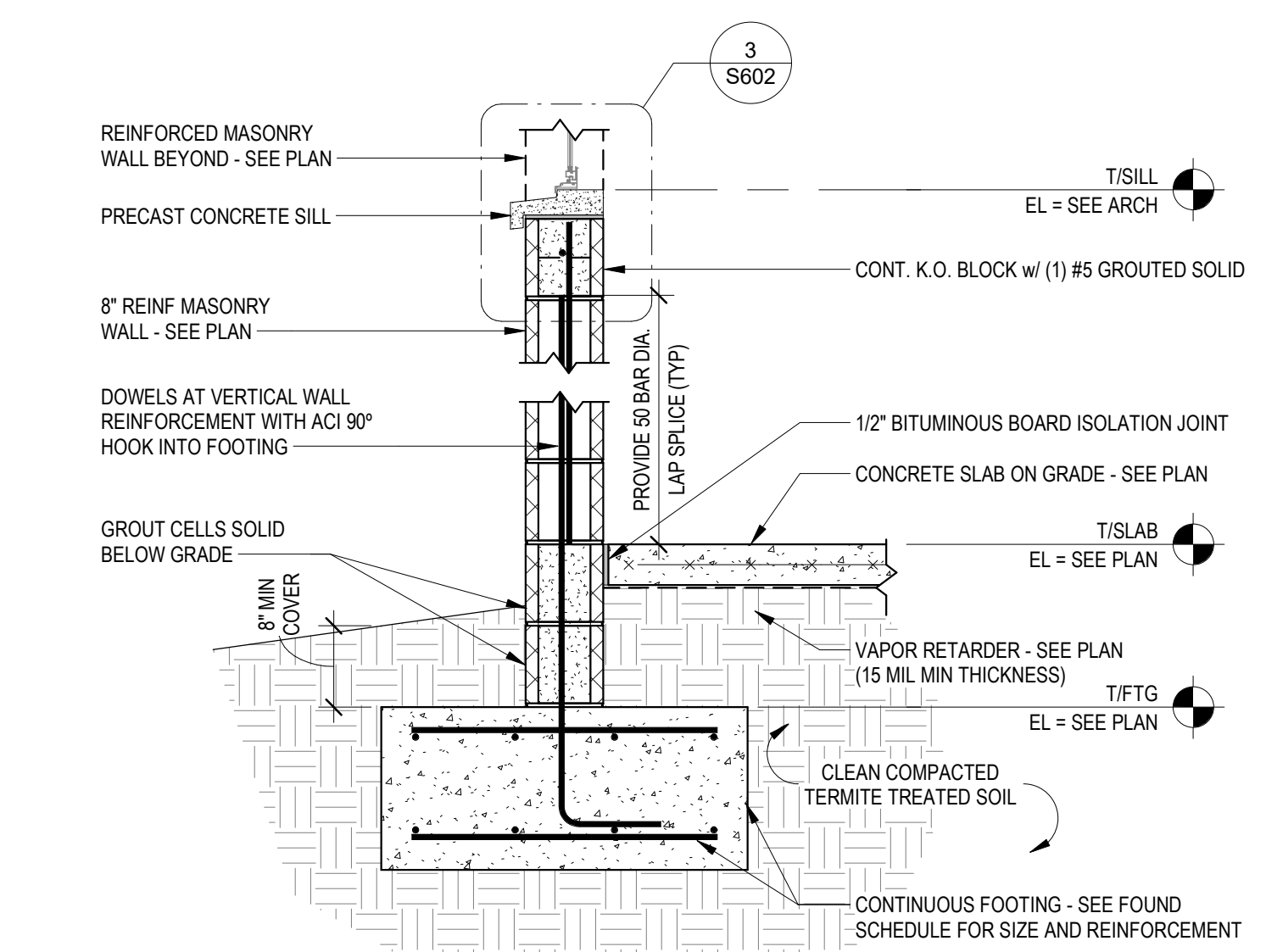




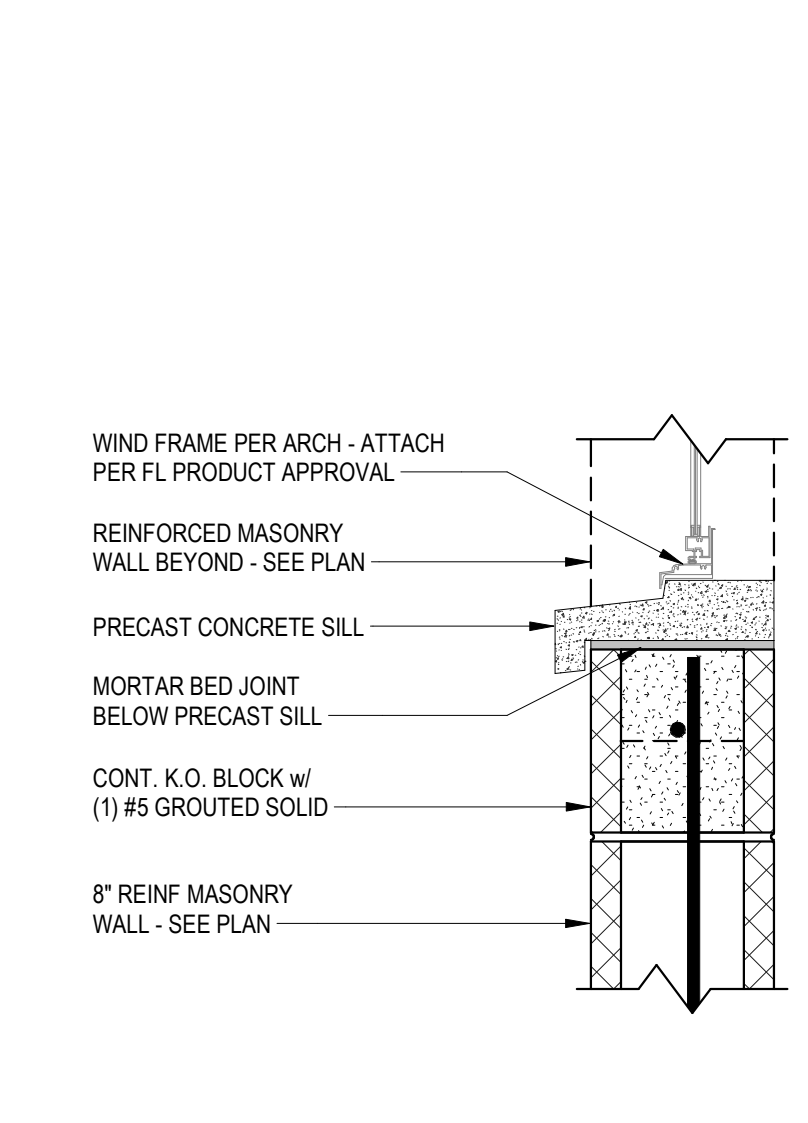




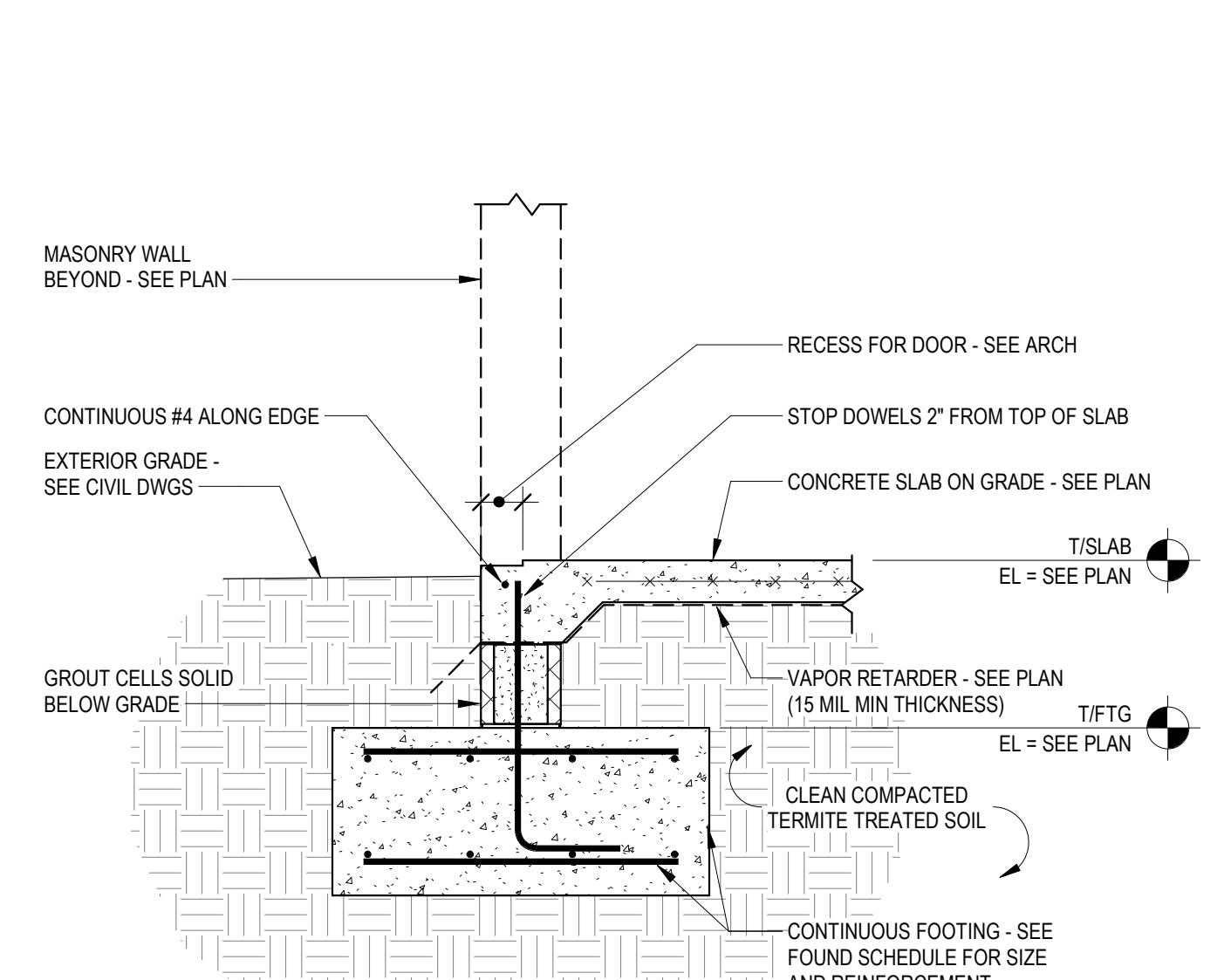
**1 TYPICAL PERIMETER SECTION**  
 S602 3/4" = 1'-0"



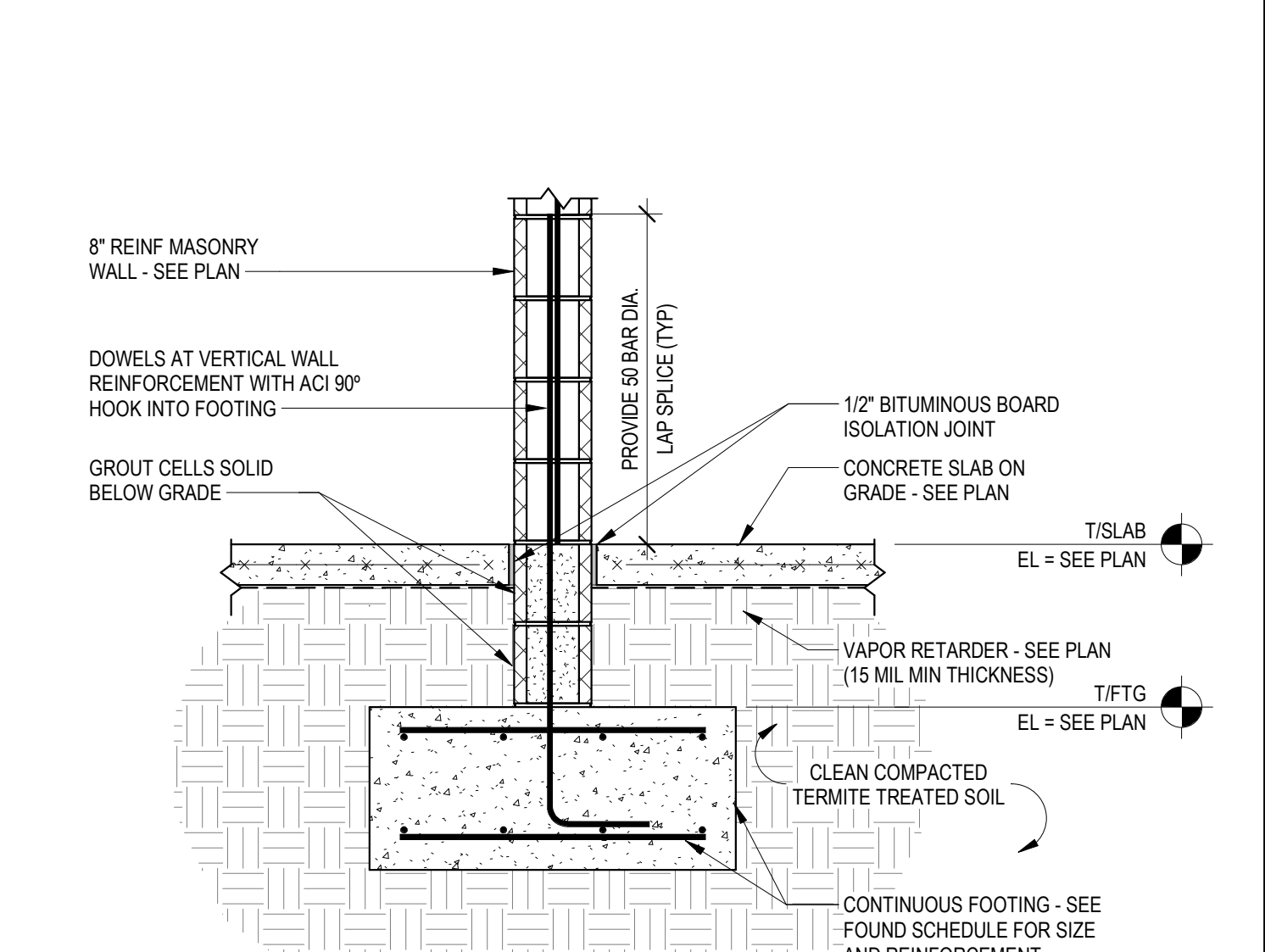
**2 TYPICAL WINDOW SILL SECTION**  
 S602 3/4" = 1'-0"



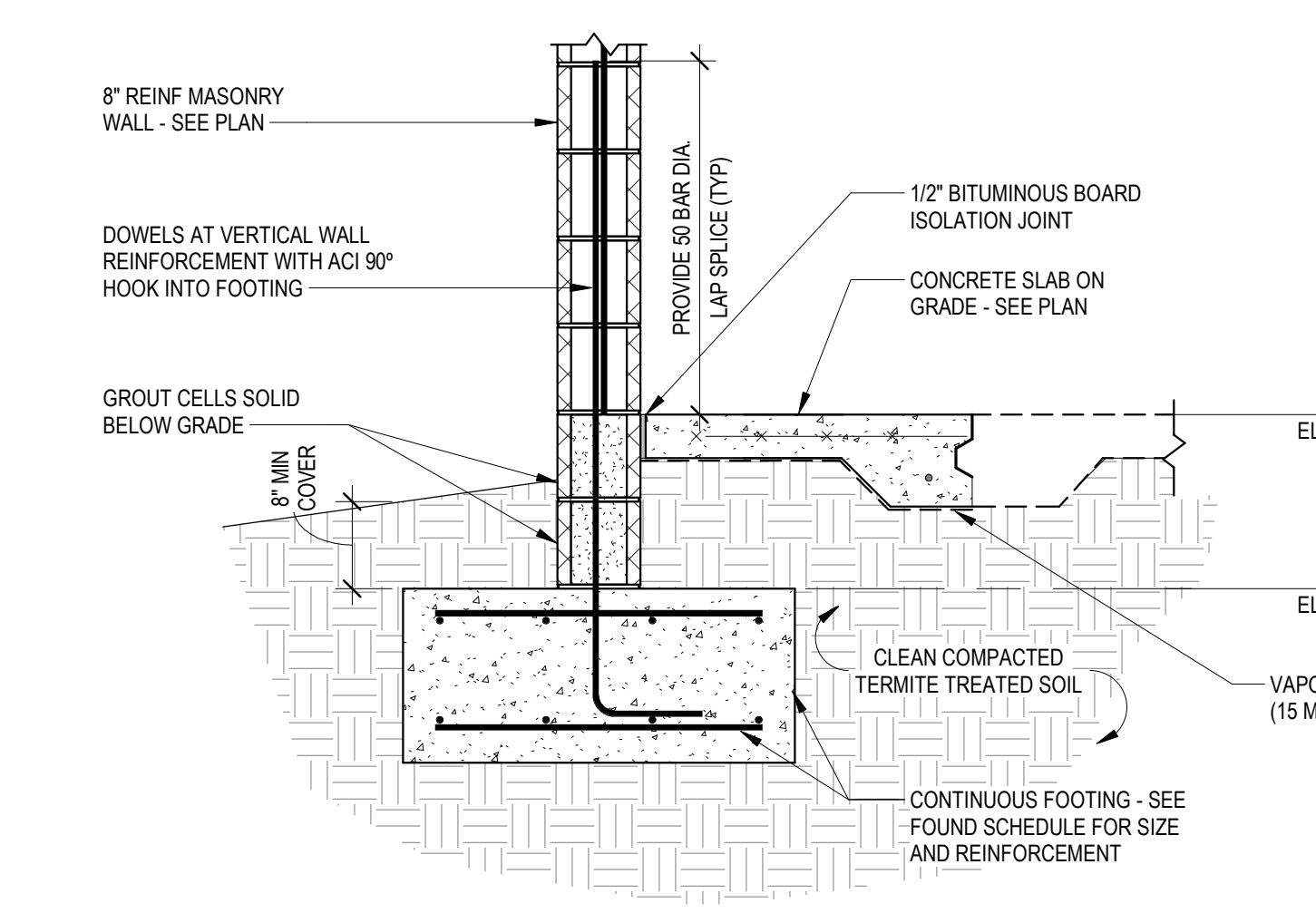
**3 WINDOW SILL DETAIL**  
 S602 1 1/2" = 1'-0"



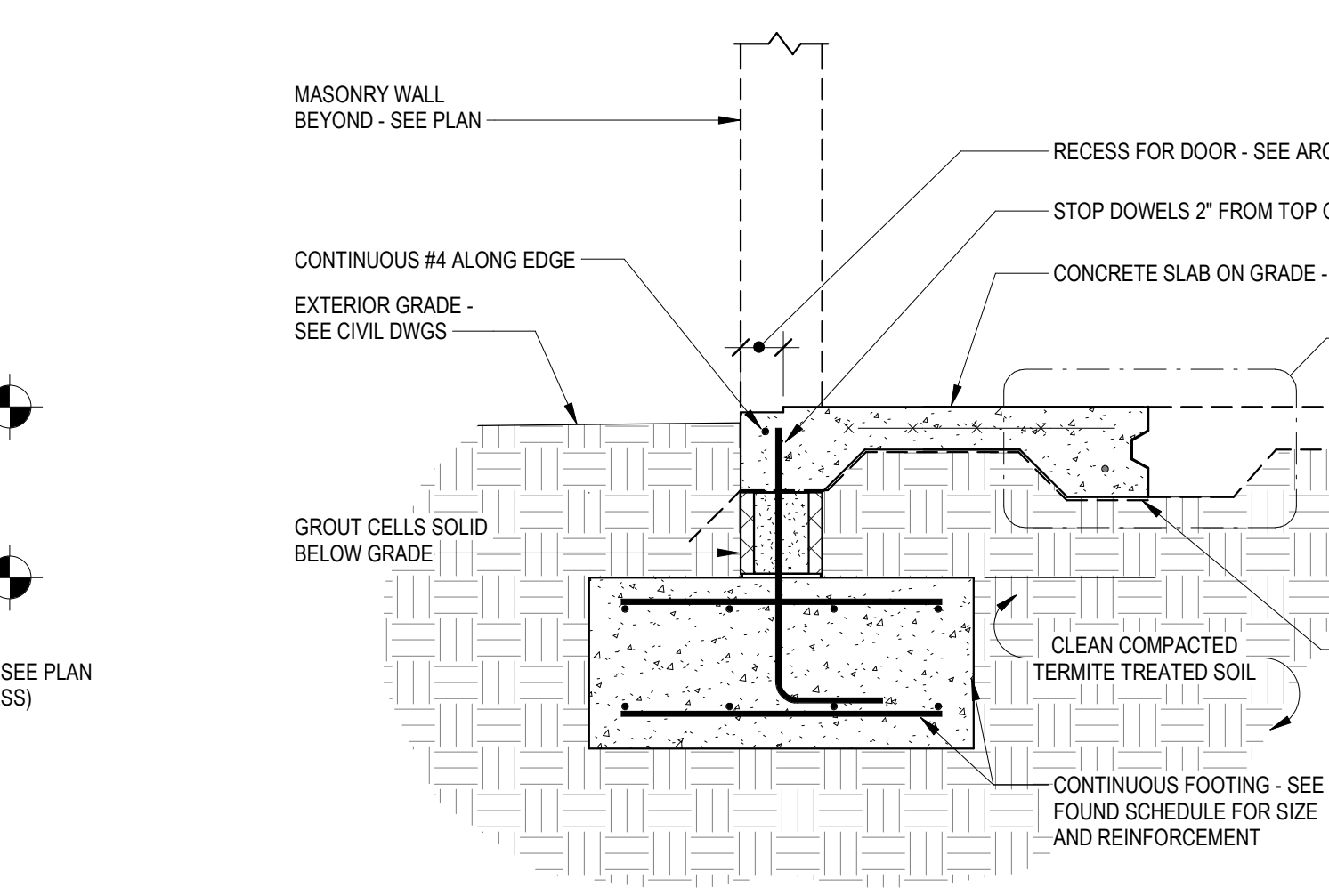
**4 TYPICAL SECTION AT DOOR**  
 S602 3/4" = 1'-0"



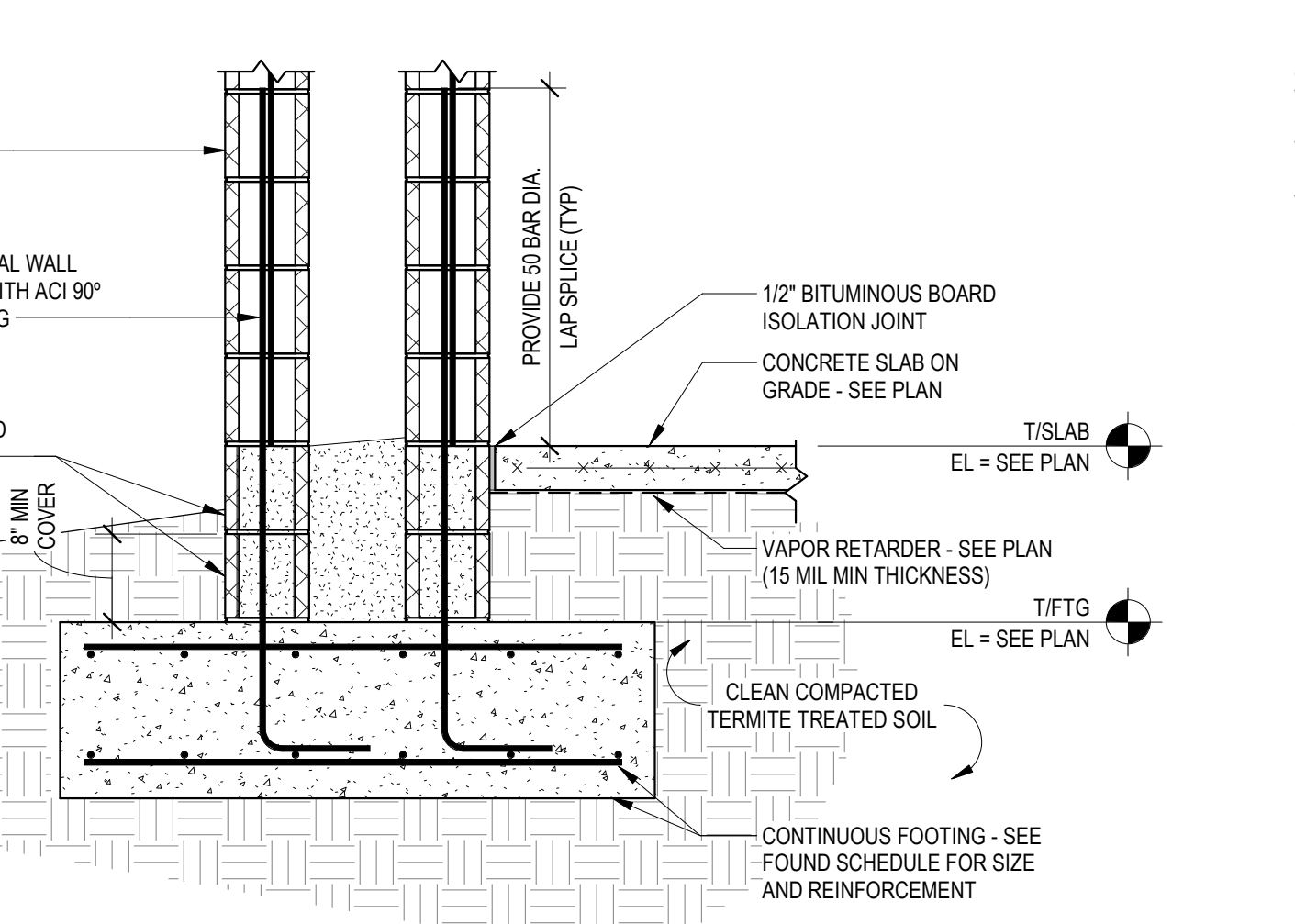
**5 TYPICAL INTERIOR SECTION**  
 S602 3/4" = 1'-0"



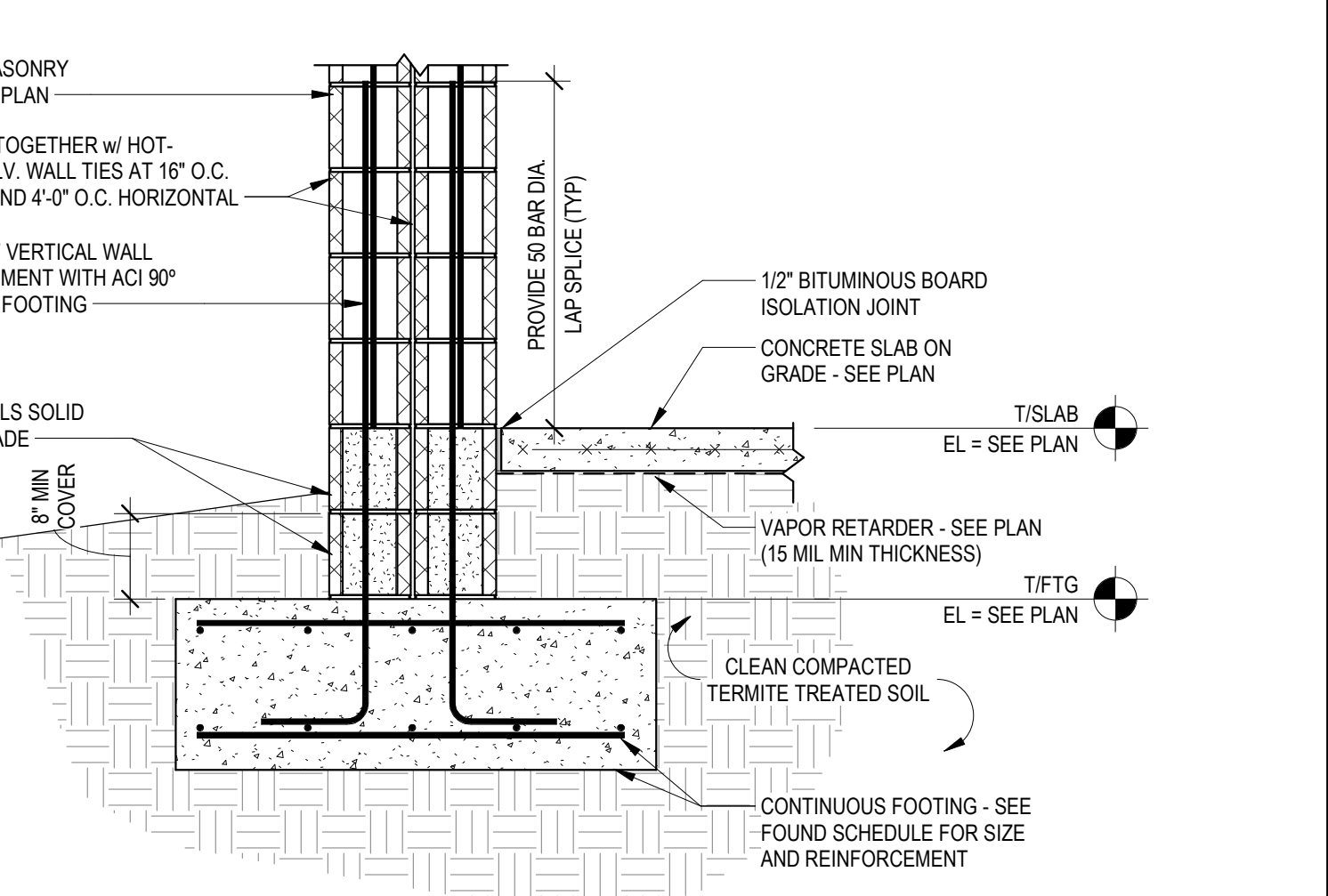
**6 TYPICAL PERIMETER SECTION**  
 S602 3/4" = 1'-0"



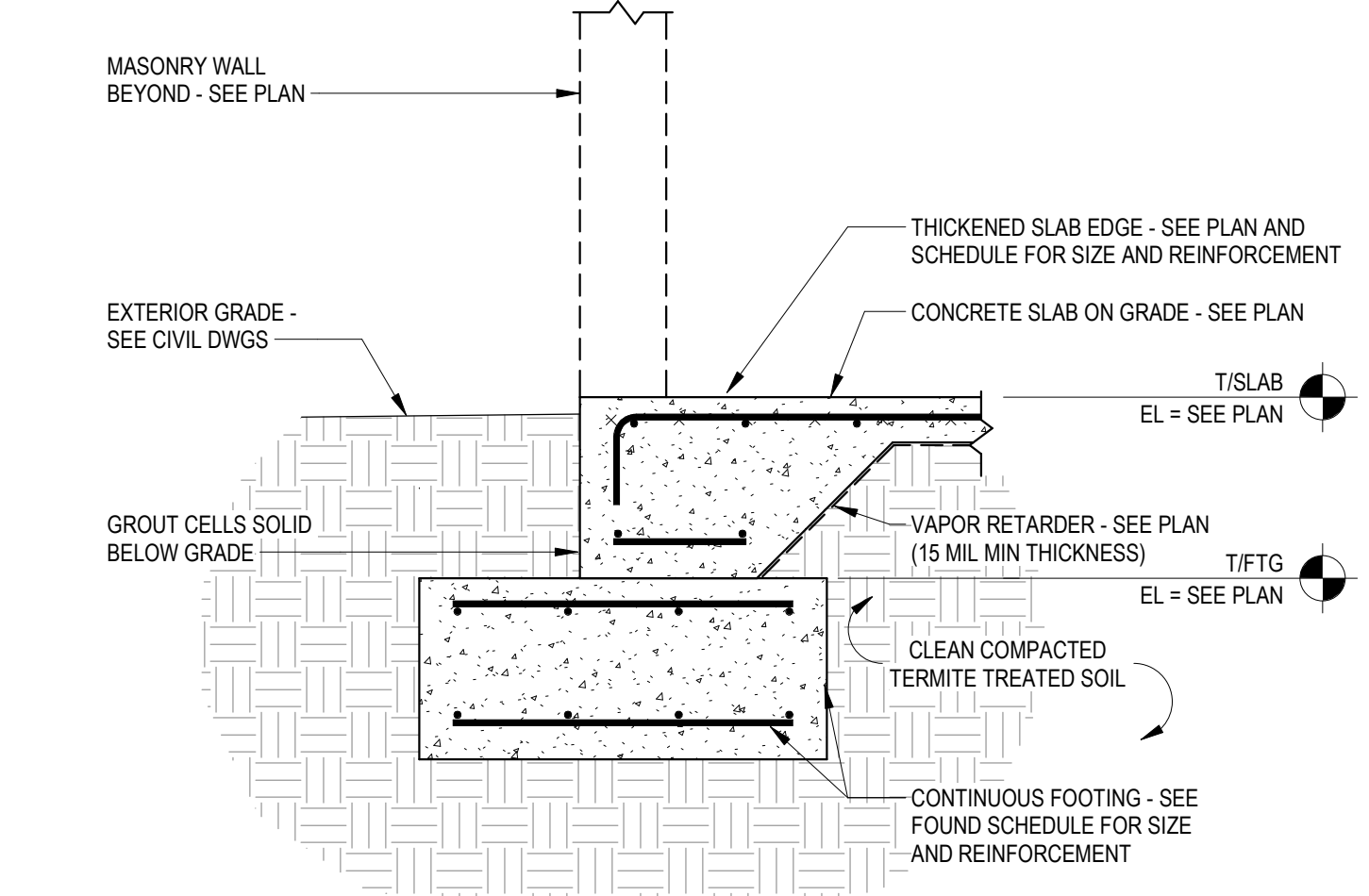
**7 TYPICAL SECTION AT DOOR**  
 S602 3/4" = 1'-0"



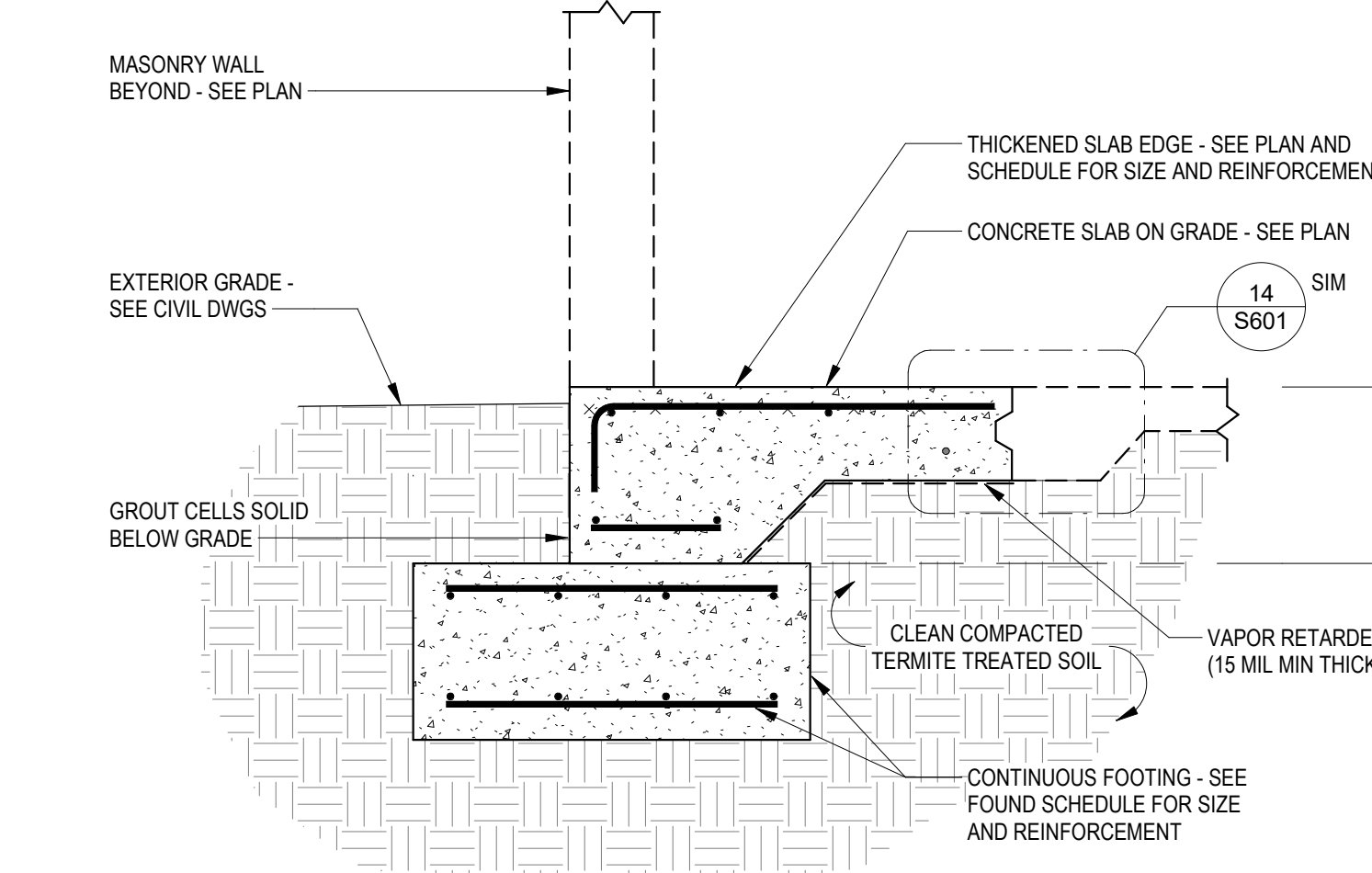
**8 TYPICAL PERIMETER SECTION**  
 S602 3/4" = 1'-0"



**9 TYPICAL PERIMETER SECTION**  
 S602 3/4" = 1'-0"



**10 TYPICAL SECTION AT LARGE OPENING**  
 S602 3/4" = 1'-0"



**11 TYPICAL SECTION AT LARGE OPENING**  
 S602 3/4" = 1'-0"

Owner:

**G4 ARCHITECTURE**  
 135 W. Central Blvd., Suite 400  
 Orlando, Florida 32801  
 TEL: 407.363.6136  
 © Copyright 2023

Revisions:

Scale:	AS NOTED
Date:	12/15/23
Drawn By:	SGZ
Checked By:	JDC

**WEST MARKET LOT 10**  
 WEST MARKET LOT 10  
 14230 WEST COLONIAL DRIVE  
 WINTER GARDEN, FL

**FOUNDATION SECTIONS AND DETAILS**

Certified **JONATHAN D. COLLINS**  
 LICENSE  
 No 74693  
 STATE OF FLORIDA  
 PROFESSIONAL ENGINEER

Electronic Signature:  
 THIS DRAWING HAS BEEN DIGITALLY SIGNED AND SEALED BY JONATHAN D. COLLINS, P.E. (A LICENSE NO. 74693 ON 12/15/2023). PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

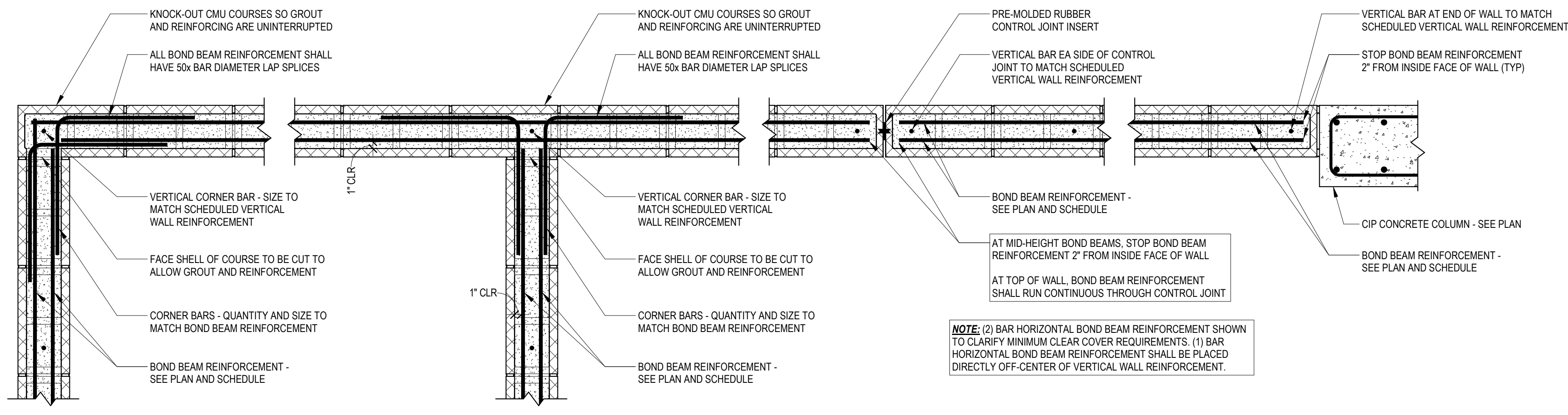
*JDCollins*

This drawing has been electronically signed and sealed on the date shown in the seal, using a digital signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

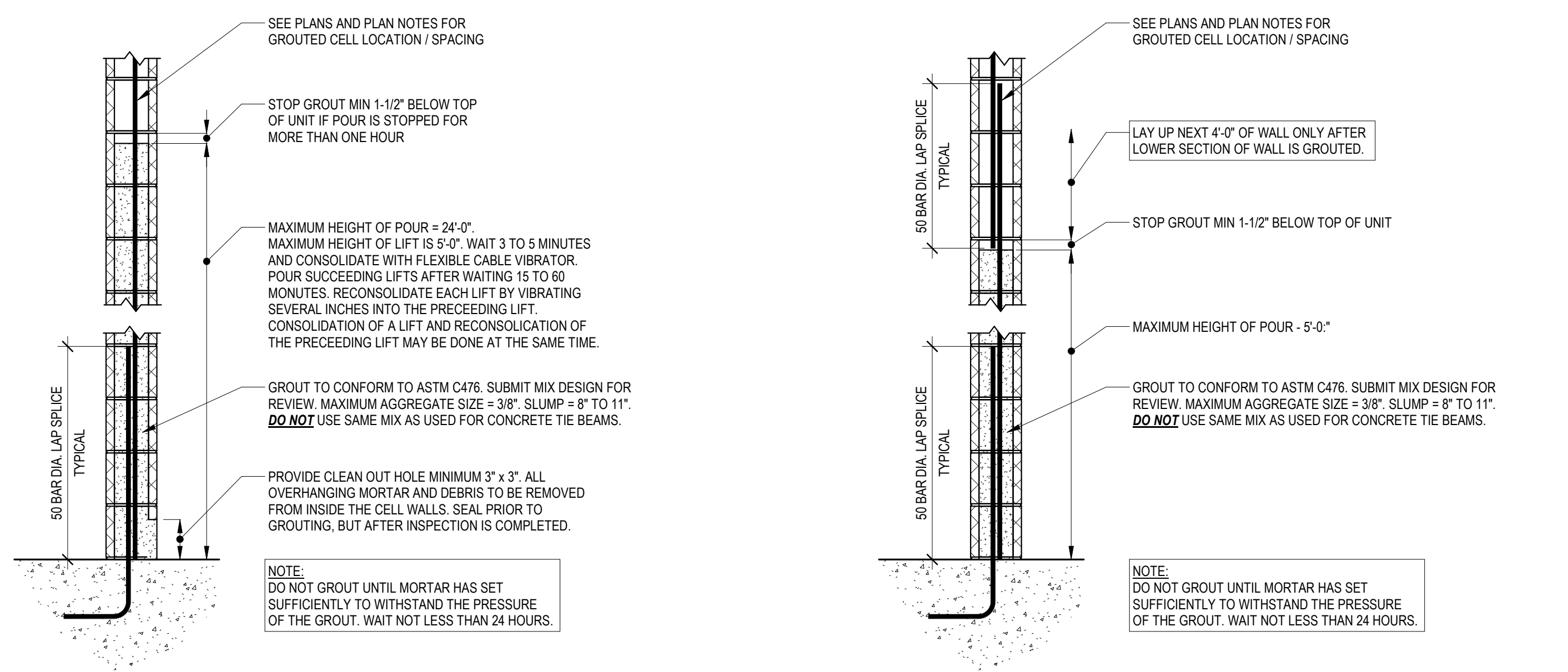
Drawing Number:  
**S602**

Job Number:  
 23432

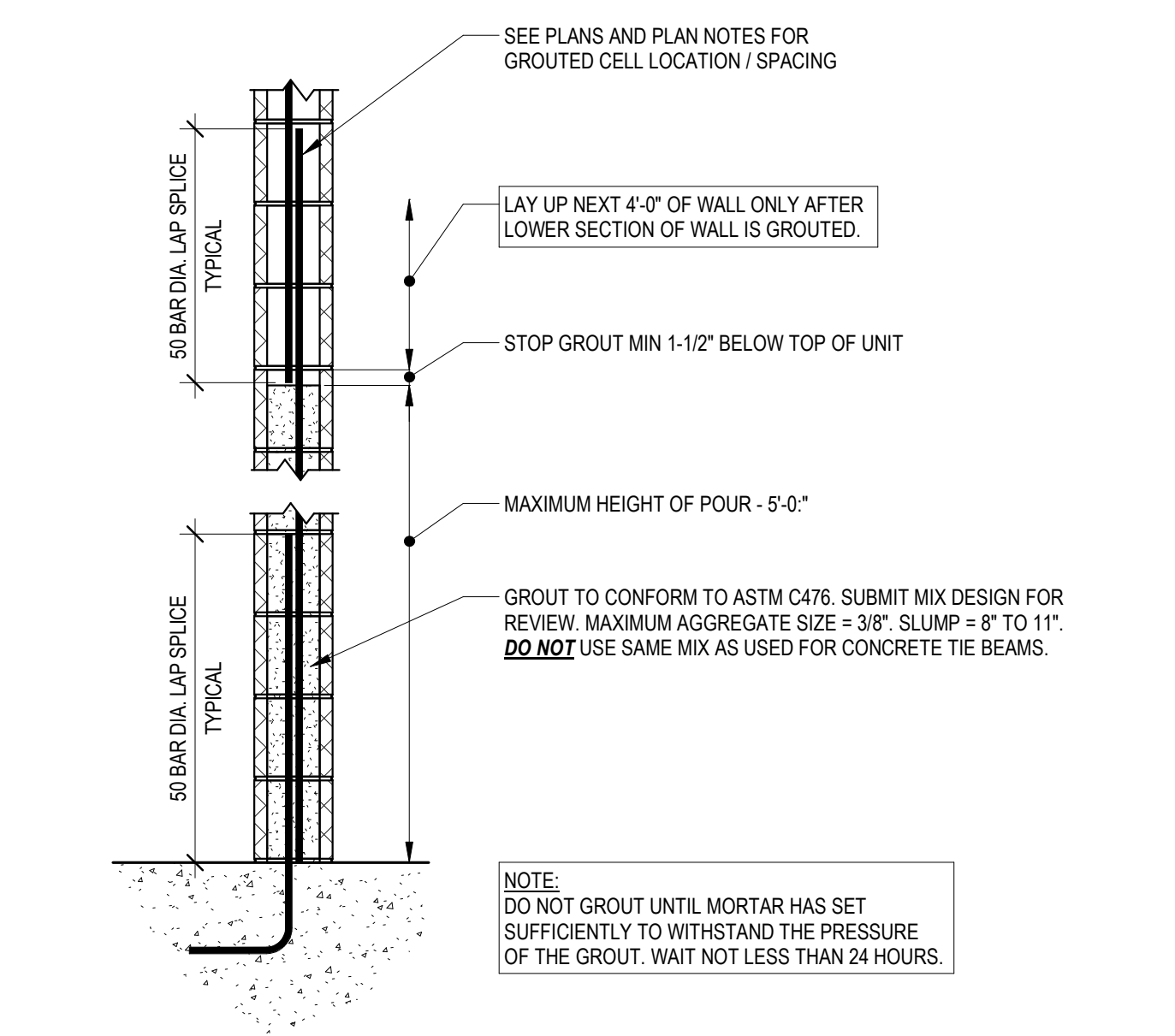
A/E Job Number:  
 23432



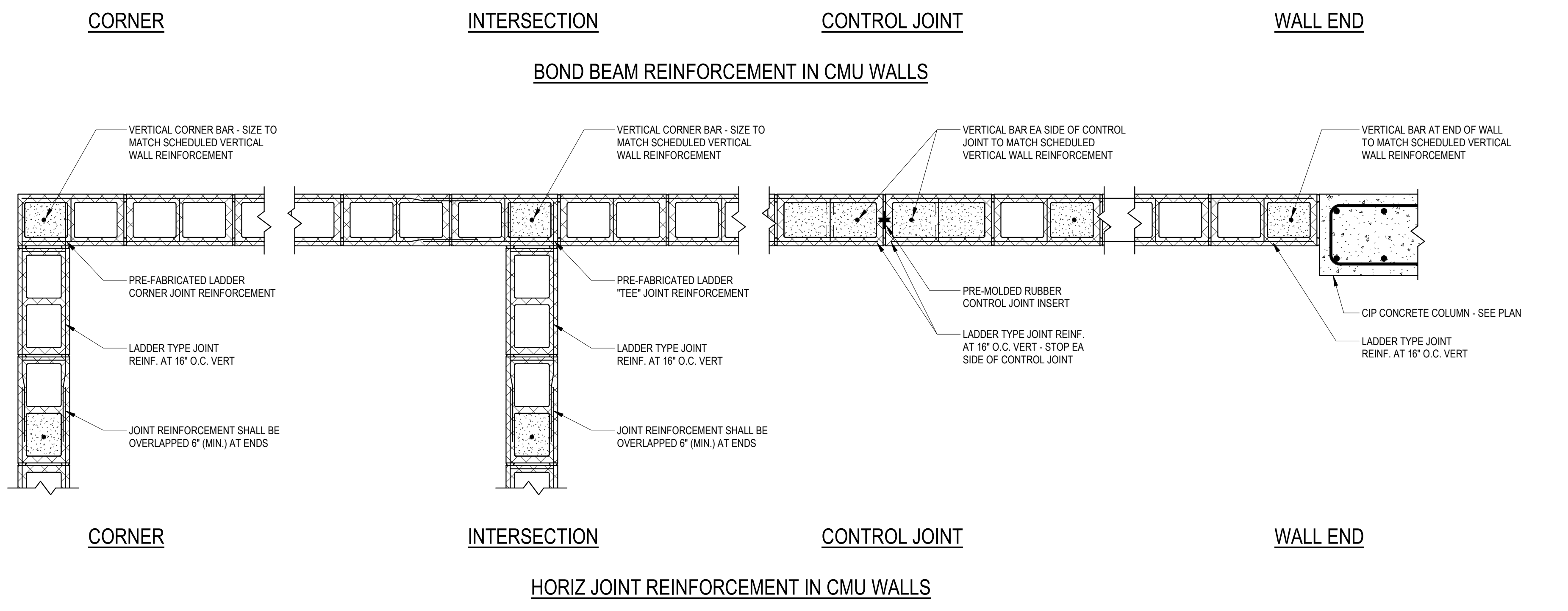
**2 TYPICAL MASONRY WALL CONTROL JOINT DETAILS**  
S603 1" = 1'-0"



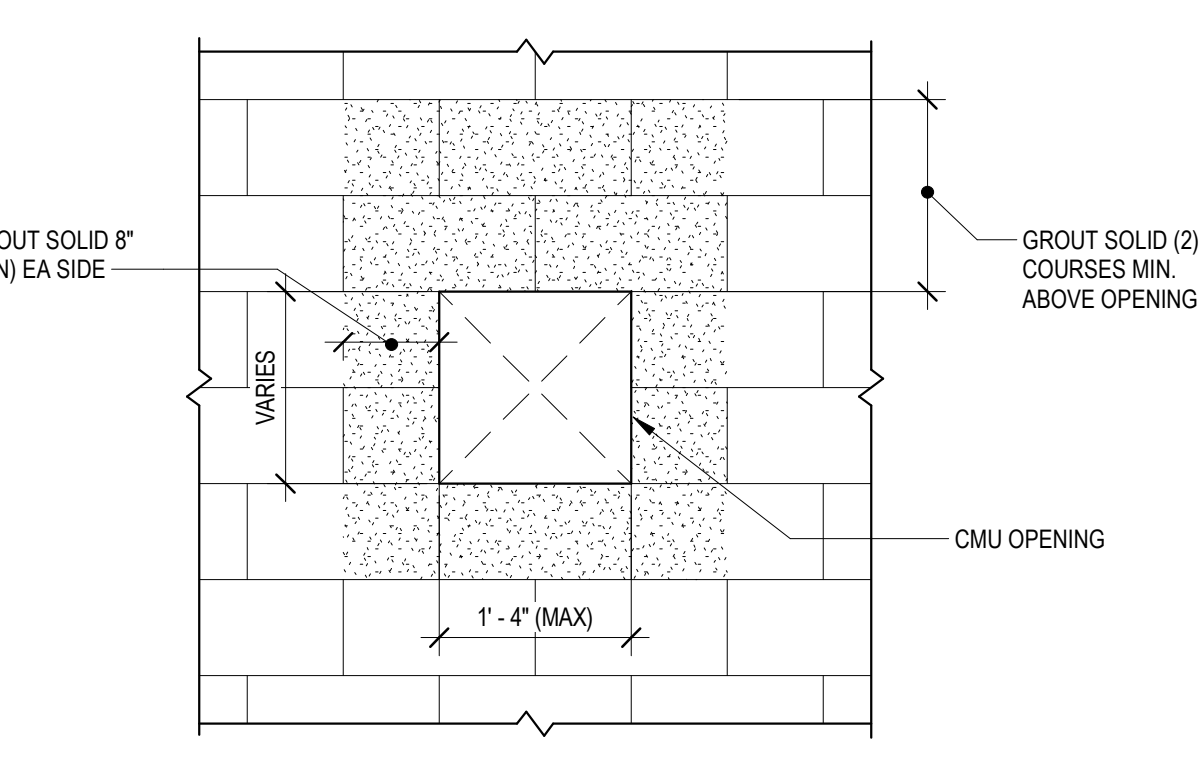
**3 TYPICAL HIGH LIFT GROUTING DETAIL**  
S603 N.T.S.



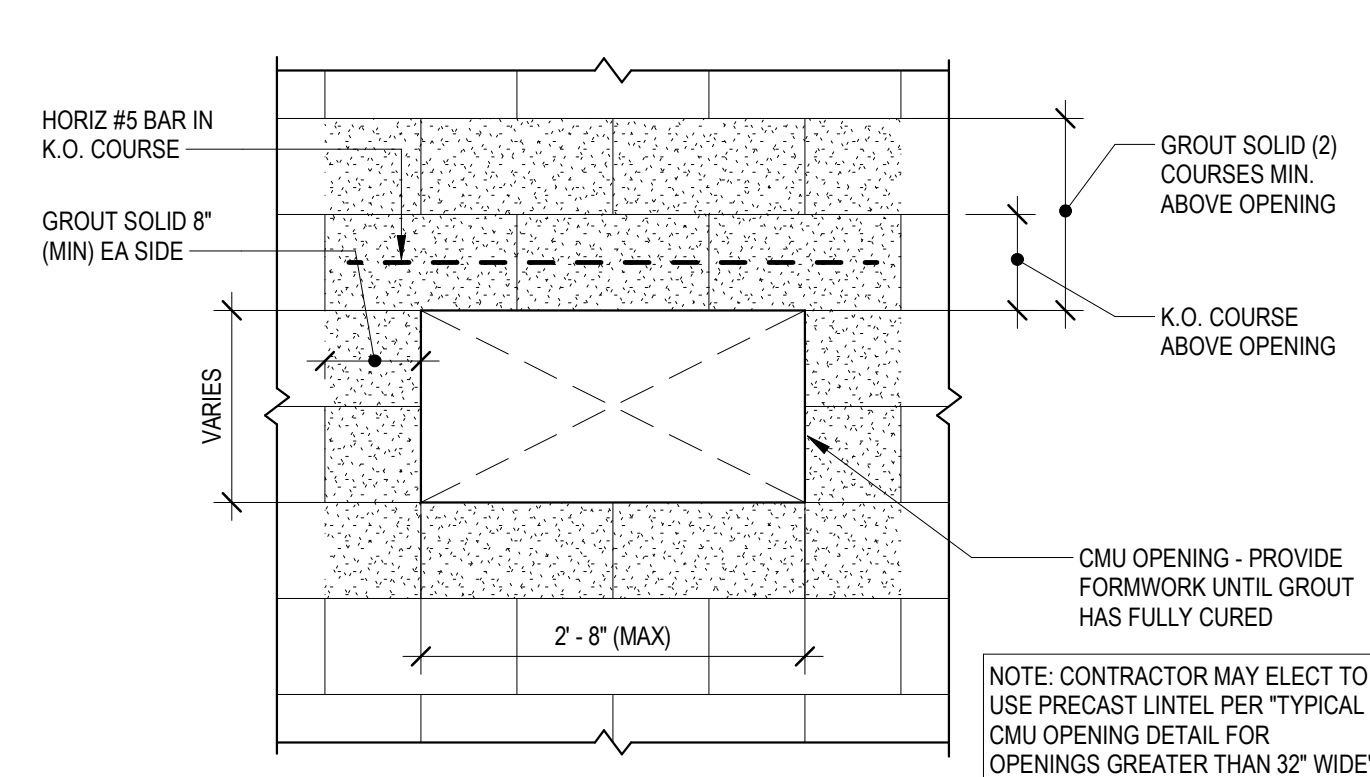
**4 TYPICAL LOW LIFT GROUTING DETAIL**  
S603 N.T.S.



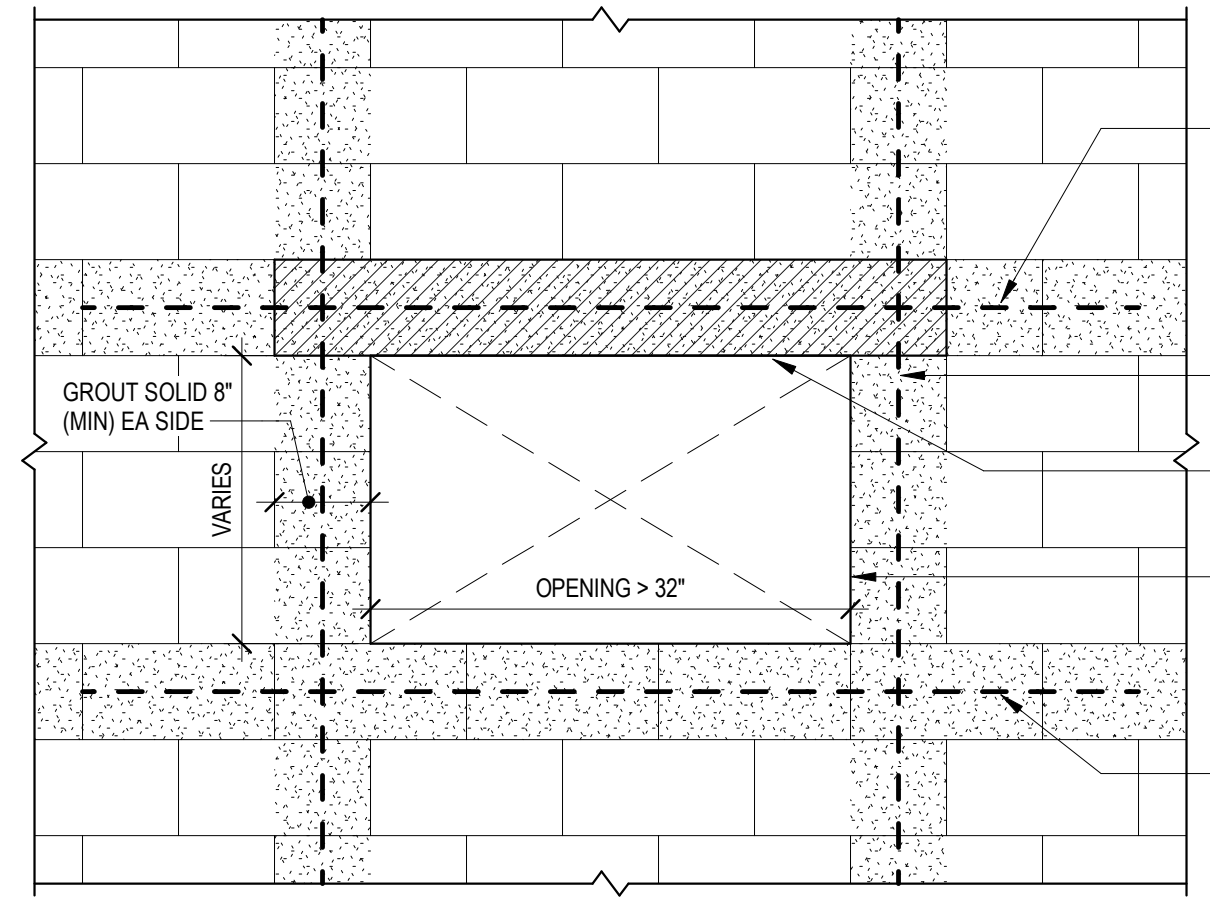
**1 TYPICAL MASONRY WALL REINFORCING**  
S603 1" = 1'-0"



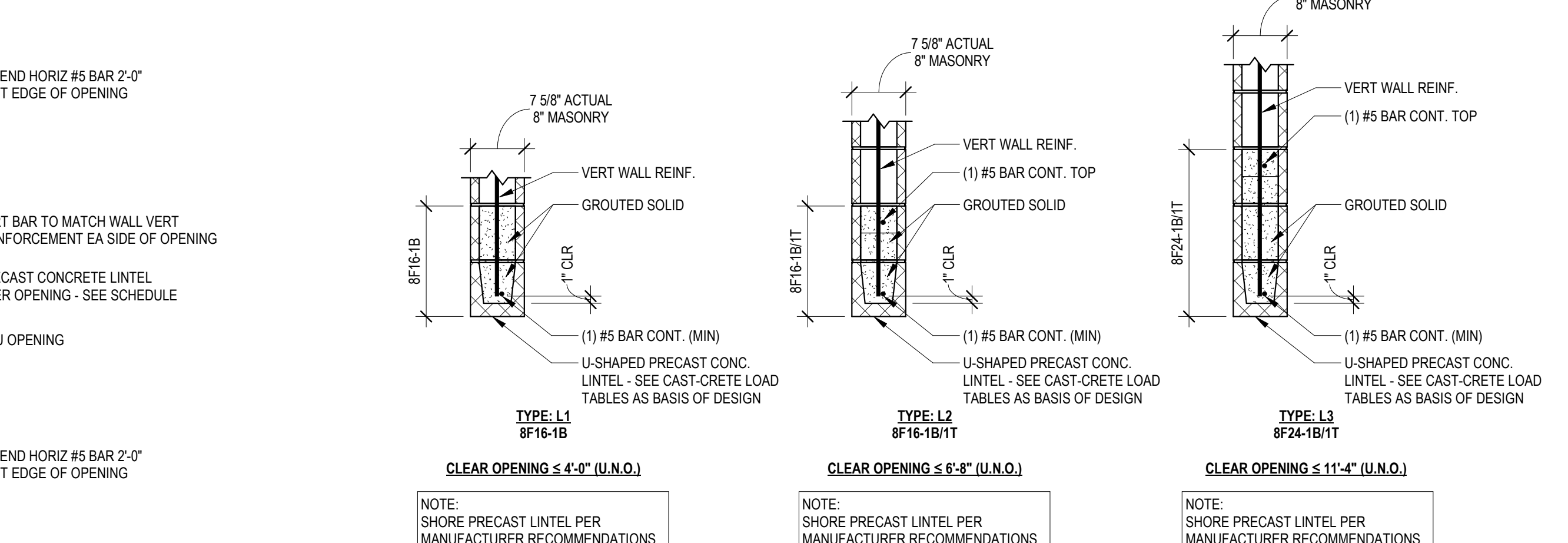
**5 TYPICAL CMU OPENING DETAIL FOR 16\"/>**



**6 TYPICAL CMU OPENING DETAIL FOR 32\"/>**



**7 TYPICAL CMU OPENING DETAIL FOR 32\"/>**



**8 TYPICAL PRECAST LINTEL SCHEDULE**  
S603 N.T.S.

Owner:

**4** ARCHITECTURE  
135 W. Central Blvd., Suite 400  
Orlando, Florida 32801  
TEL: 407.363.6136  
© Copyright 2023

Revisions:

Scale:	AS NOTED
Date:	12/15/23
Drawn By:	SGZ
Checked By:	JDC

**WEST MARKET LOT 10**  
WEST MARKET LOT 10  
14230 WEST COLONIAL DRIVE  
WINTER GARDEN, FL

**MASONRY SECTIONS AND DETAILS**

Certified Professional Engineer  
**JONATHAN D. COLLINS**  
No. 74693  
STATE OF FLORIDA  
PROFESSIONAL ENGINEER

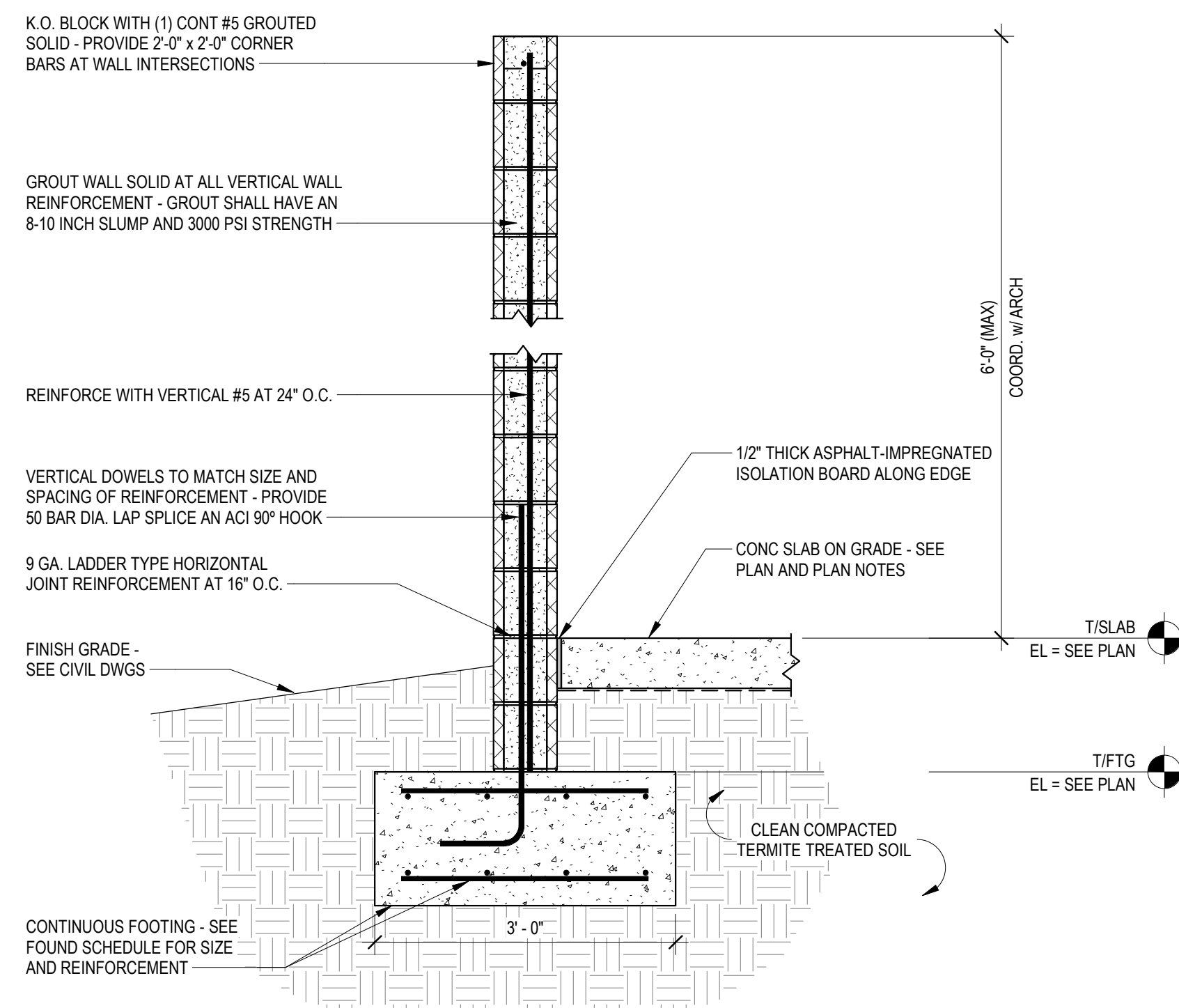
Electronic Signature:  
THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY JONATHAN D. COLLINS, P.E. (LICENSE NO. 74693 ON 12/18/2023). PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

This drawing has been electronically signed and sealed on the date shown in the seal, unless a digital signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

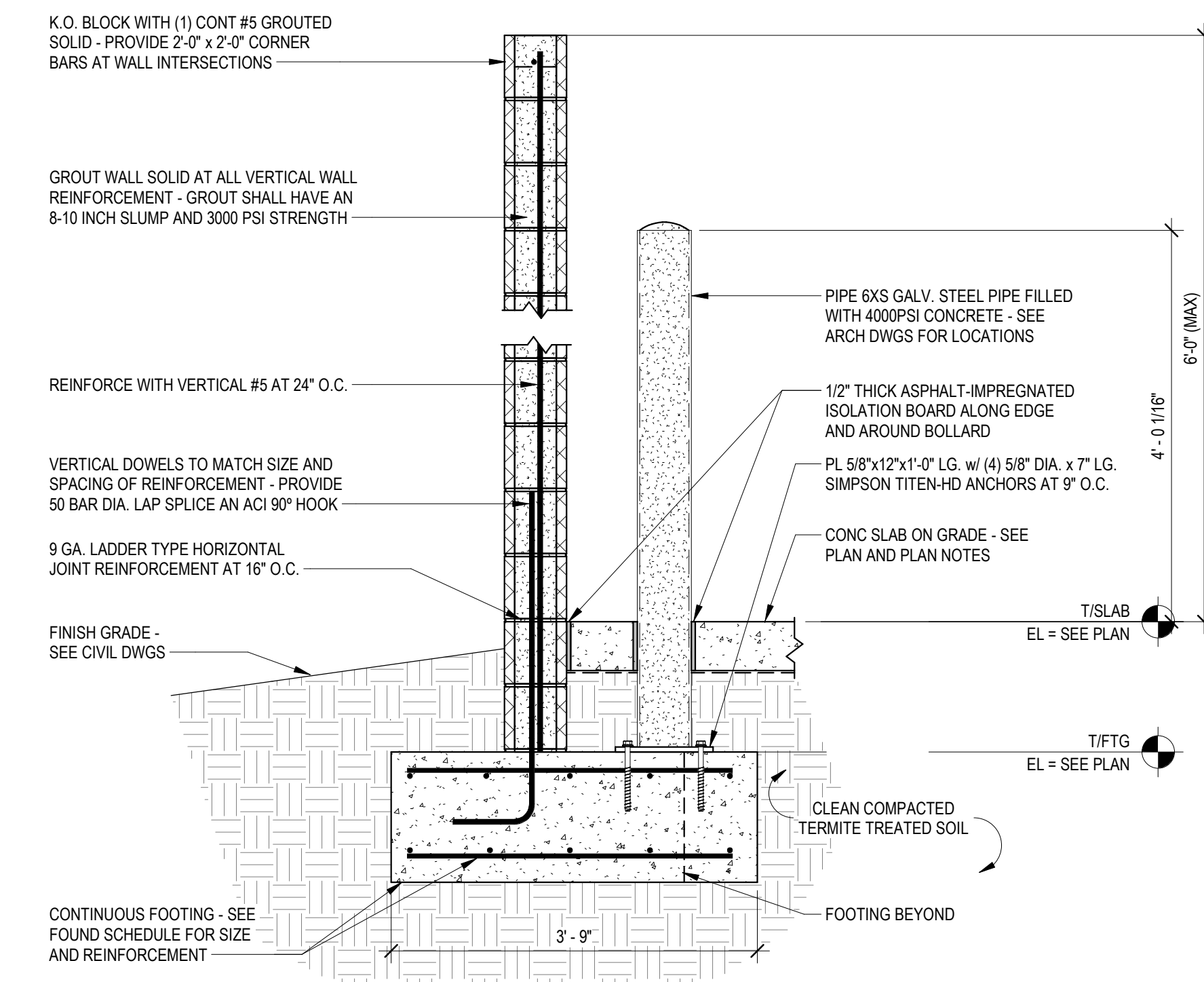
Drawing Number:  
**S603**

Job Number:  
A/E Job Number:  
**23432**

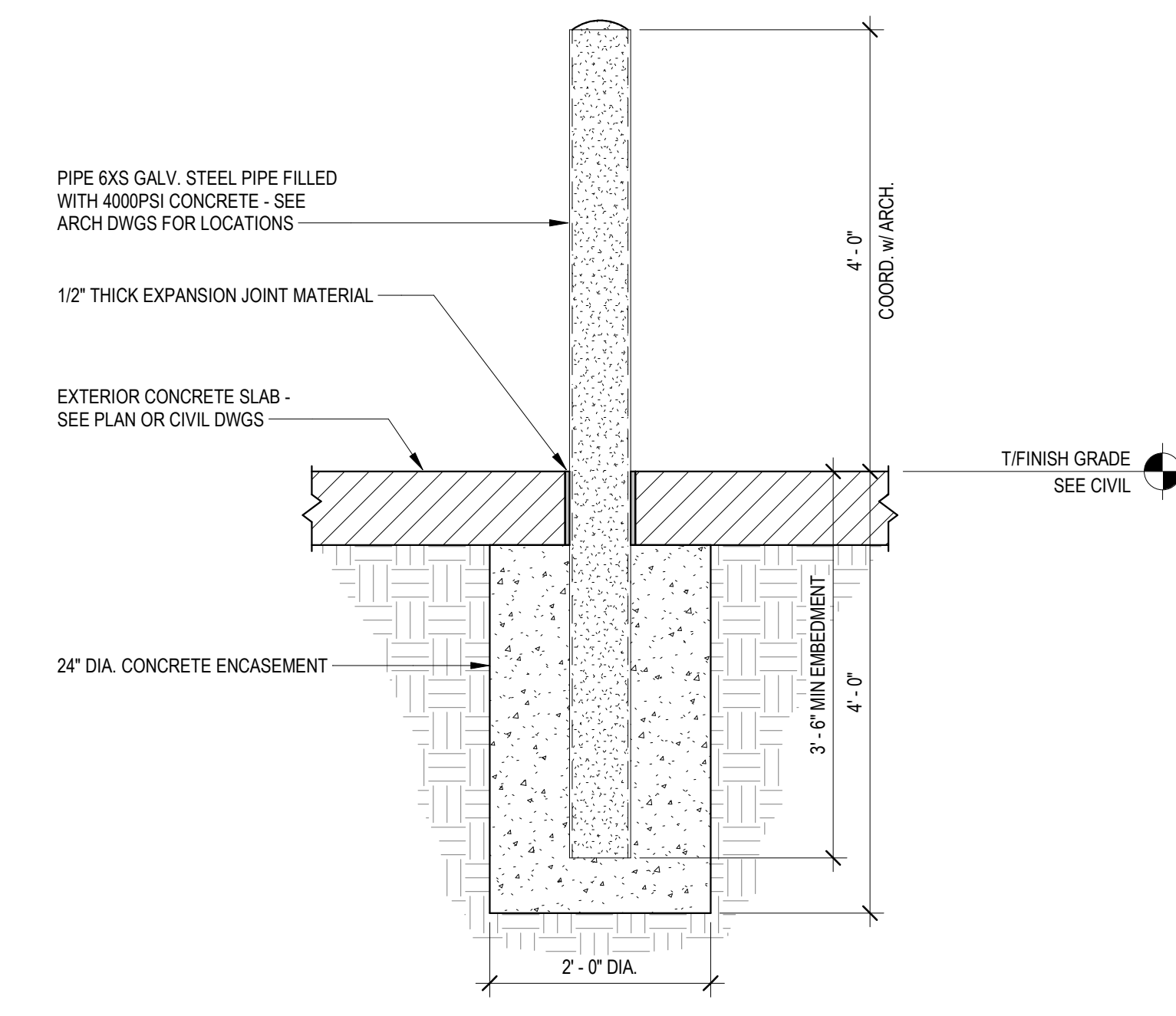




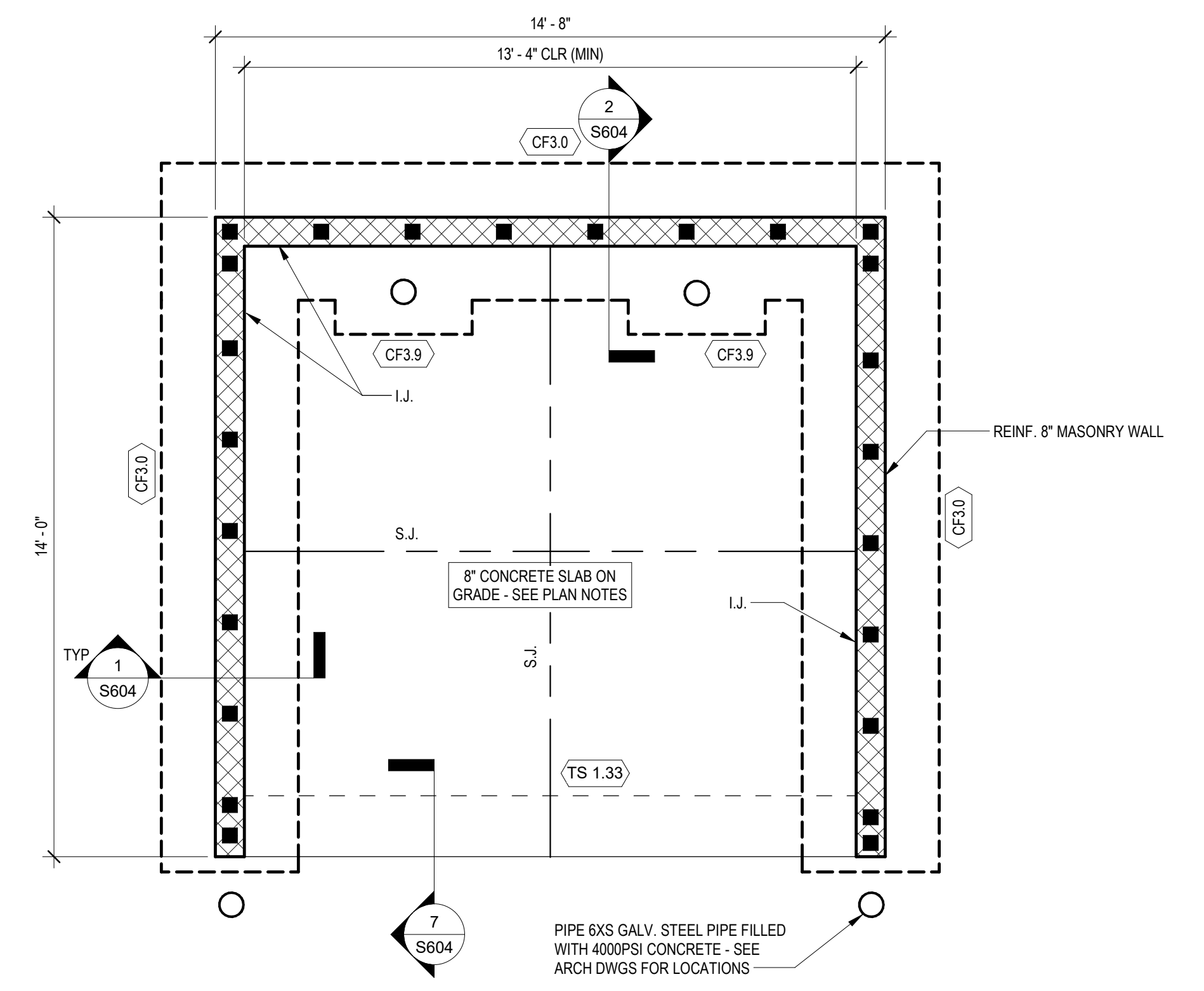
**1 TYPICAL CMU SCREEN WALL AT DUMPSTER**  
S604 3/4" = 1'-0"



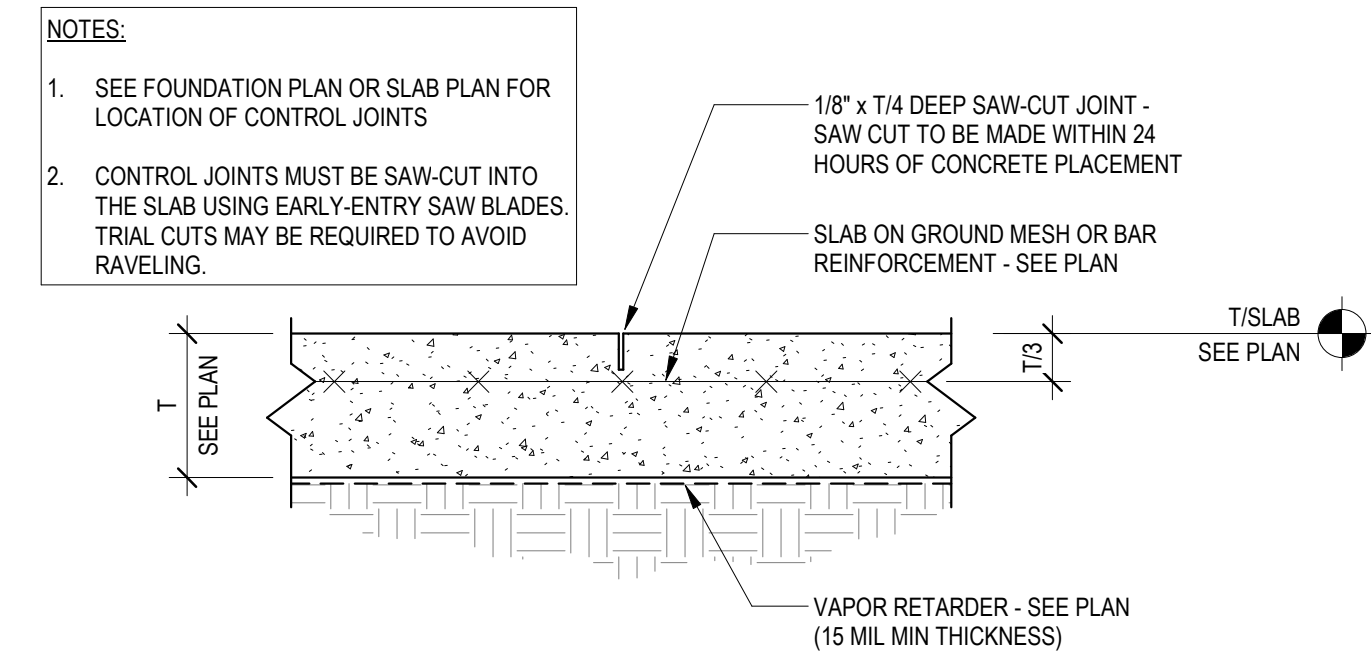
**2 TYPICAL CMU SCREEN WALL AT DUMPSTER WITH BOLLARD**  
S604 3/4" = 1'-0"



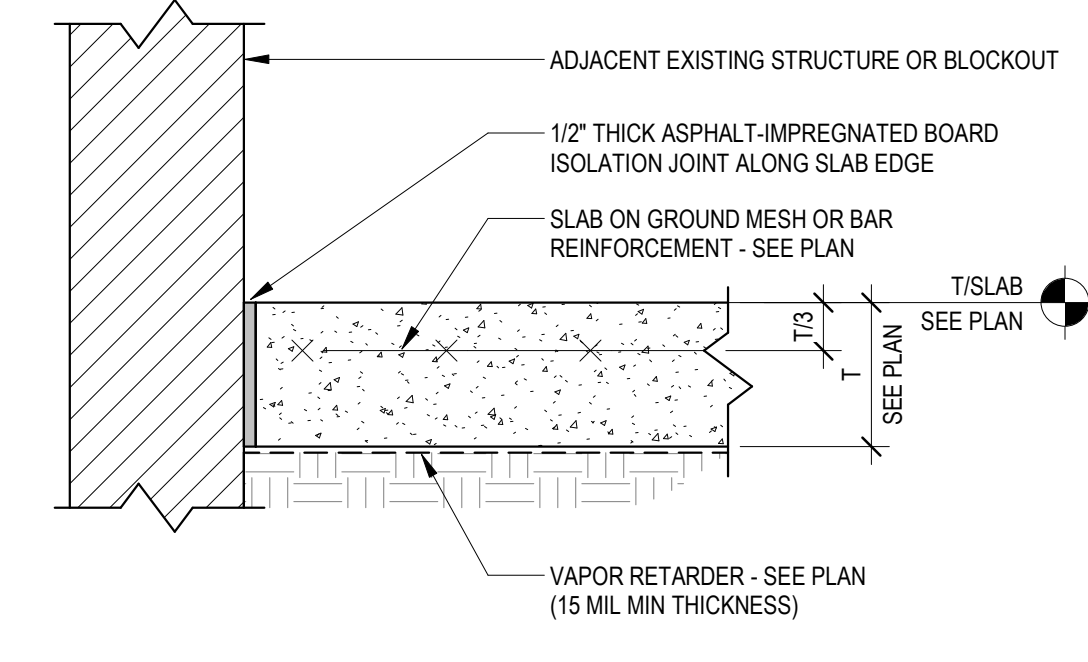
**3 TYP 6" PIPE BOLLARD DETAIL**  
S604 3/4" = 1'-0"



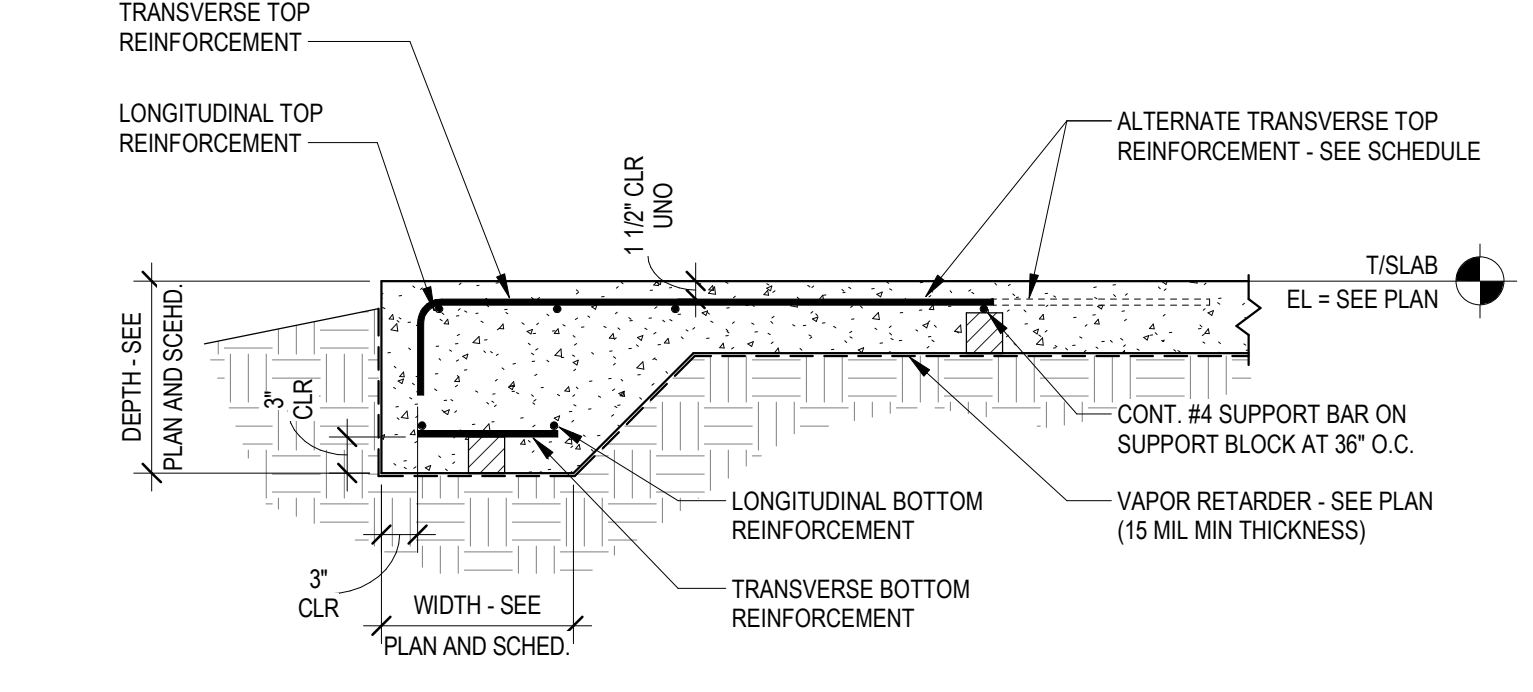
**4 FOUNDATION PLAN AT DUMPSTER AREA**  
S604 3/8" = 1'-0"



**5 TYPICAL SLAB ON GROUND SAW-CUT JOINT**  
S604 1 1/2" = 1'-0"



**6 TYPICAL SLAB ISOLATION JOINT**  
S604 1 1/2" = 1'-0"



**7 TYPICAL THICKENED SLAB EDGE FOOTING REINFORCEMENT**  
S604 3/4" = 1'-0"

**FOUNDATION PLAN NOTES:**

- REFERENCE THE STRUCTURAL GENERAL NOTES ON DRAWINGS S001 & S002. GENERAL NOTES INCLUDE CODES AND STANDARDS, DESIGN LOADS AND OTHER REQUIREMENTS.
- PREPARE THE SLAB SUB-BASE AND COMPACT THE SOIL PER THE PROJECT GEOTECHNICAL REPORT, THE CIVIL DRAWINGS, AND THE STRUCTURAL GENERAL NOTES. IF ANY OF THESE DRAWINGS OR NOTES ARE IN CONFLICT, THE CONTRACTOR MUST ALERT THE ENGINEER FOR CLARIFICATION PRIOR TO START OF CONSTRUCTION.
- T.O. FOOTING ELEVATION IS AT -1'-4" (U.N.O.) THIS IS A REFERENCE ELEVATION ONLY. SEE FOUNDATION DETAIL SHEETS AND SCHEDULES FOR FOUNDATION SIZE AND REINFORCEMENT.
- T.O. SLAB ELEVATION IS AT 0'-0" (U.N.O.) THIS IS A REFERENCE ELEVATION ONLY. SEE FOUNDATION AND SLAB ON GRADE DETAIL SHEETS.
- SLAB ON GROUND IS 8" MINIMUM THICK 4000 PSI CONCRETE SLAB U.N.O. REINFORCED WITH #4 AT 12" O.C. EACH WAY TOP & BOTTOM.
- REFERENCE THE ARCHITECTURAL DRAWINGS FOR SLAB EDGES, FLOOR SLOPES, WALL OPENINGS, AND OTHER DIMENSIONS NOT GIVEN. CONTRACTOR MUST COORDINATE AND VERIFY ALL DIMENSIONS WITH PROJECT ARCHITECT PRIOR TO FABRICATION.

**LEGEND**

- EL = #'-##" INDICATES TOP OF CONCRETE FOOTING ELEVATION
- T.O. FTG INDICATES TOP OF CONCRETE FOOTING ELEVATION
- CF#.# INDICATES CONTINUOUS FOOTING TYPE. SEE FOUNDATION SCHEDULE FOR SIZE AND REINFORCEMENT.
- TS#.# INDICATES THICKENED SLAB EDGE TYPE. SEE FOUNDATION SCHEDULE FOR SIZE AND REINFORCEMENT.
- S.J. INDICATES SLAB SAWCUT JOINT PER DETAIL 5/S604.
- I.J. INDICATES ISOLATION JOINT WITH 1/2" THICK ASPHALT-IMPREGNATED BOARD PER TYPICAL DETAIL 6/S604.
- INDICATES 8" REINFORCED MASONRY WALL - PROVIDE VERT #5 AT 24" O.C. (MAX) U.N.O. GROUT WALLS SOLID BELOW SLAB.
- INDICATES VERTICAL REINFORCEMENT AT CENTER OF CMU WALL GROUTED SOLID. BAR SIZE MUST MATCH WALL REINF. NOTED

**CONCRETE FOOTING SCHEDULE**

TYPE	SIZE			REINFORCING	REMARKS
	WIDTH	LENGTH	DEPTH		
CF 3.0	3'-0"	3'-0"	1'-6"	LONG. (4) #5 x CONT. TOP & BOTTOM TRANS. #4 x 4'-0" LG. AT 12" O.C. TOP & BOTTOM	CONTINUOUS FOOTING
CF 3.3	3'-0"	3'-9"	1'-6"	LONG. (5) #5 x CONT. TOP & BOTTOM TRANS. #4 x 4'-0" LG. AT 12" O.C. TOP & BOTTOM	CONTINUOUS FOOTING
TS 1.33	1'-4"	CONT.	1'-4"	LONG. (2) #5 x CONT. TOP LONG. (2) #5 x CONT. BOTTOM ALT. TRANS. #4 x 4'-0" & 5'-0" LG. w/ ACI 90° HOOK AT 18" O.C. TOP #4 x 1'-0" LG. SKEWED SUPPORT BAR AT 36" O.C. BOT	THICKENED SLAB EDGE

Owner:

**4** ARCHITECTURE  
135 W. Central Blvd., Suite 400  
Orlando, Florida 32801  
TEL: 407.363.6136  
© Copyright 2023

Revised:

Scale: AS NOTED  
Date: 12/08/23  
Drawn By: SGZ  
Checked By: JDC

**WEST MARKET LOT 10**  
WEST MARKET LOT 10  
14230 WEST COLONIAL DRIVE  
WINTER GARDEN, FL

**DUMPSTER ENCLOSURE**

Certified **JONATHAN D. COLLINS**  
LICENSE  
No 74693  
STATE OF FLORIDA  
PROFESSIONAL ENGINEER

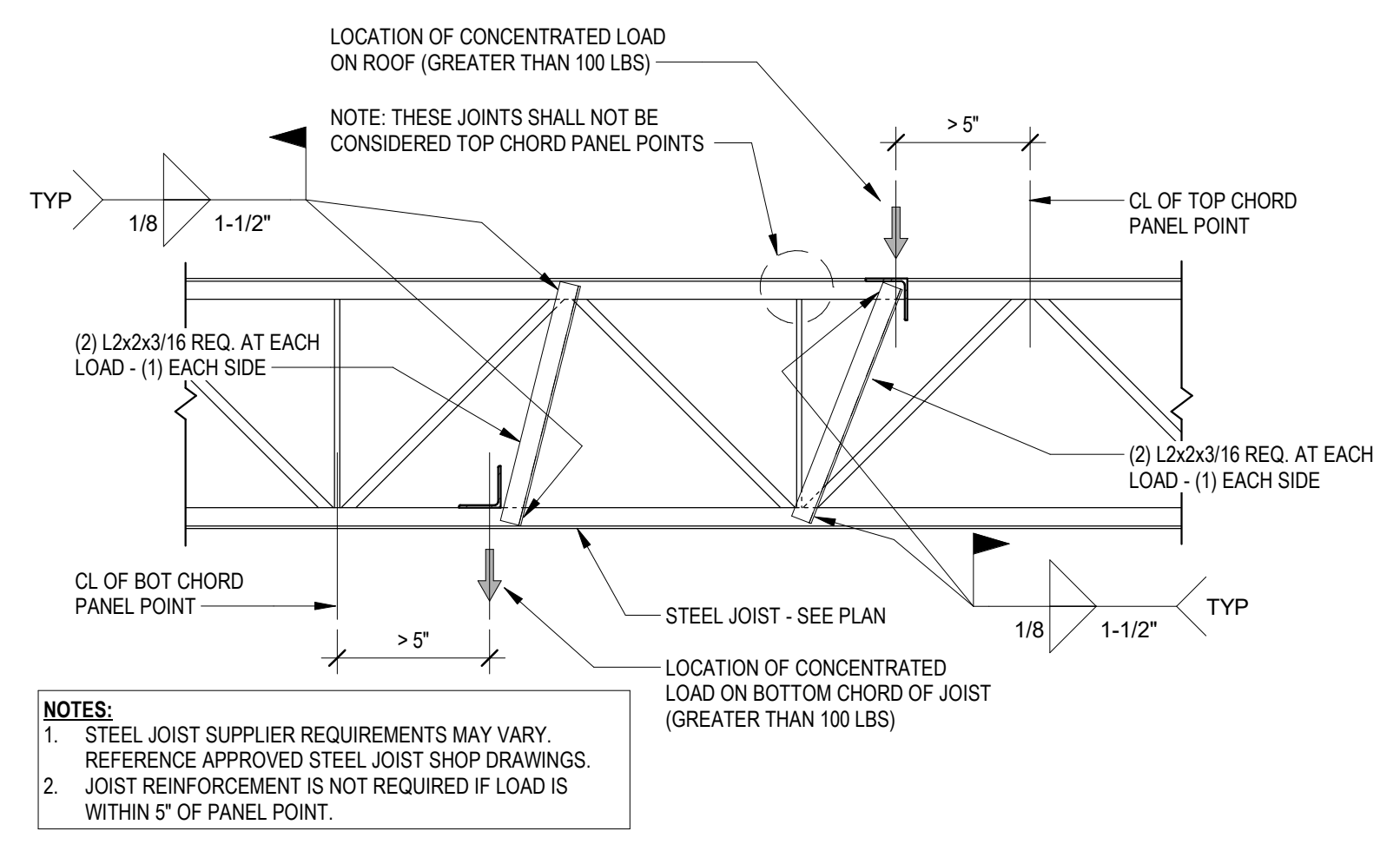
Electronic Signature:  
THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY JONATHAN D. COLLINS, P.E. (LICENSE NO. 74693 ON 12/14/2023). PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED, AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

This drawing has been electronically signed and sealed on the date shown in the seal, unless a digital signature. Printed copies of this document are not considered signed and sealed, and the signature must be verified on any electronic copies.

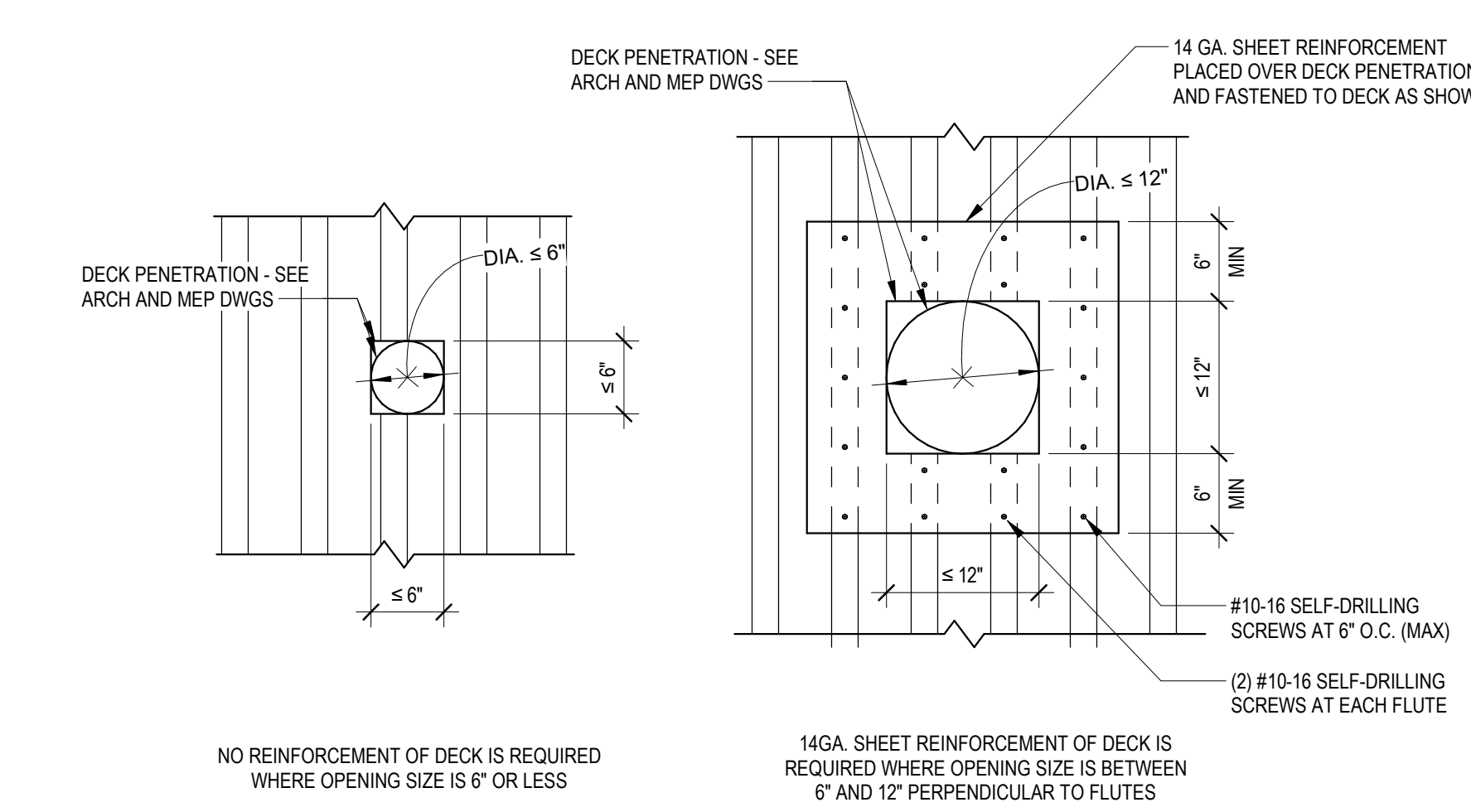
Drawing Number:  
**S604**

Job Number:  
23432

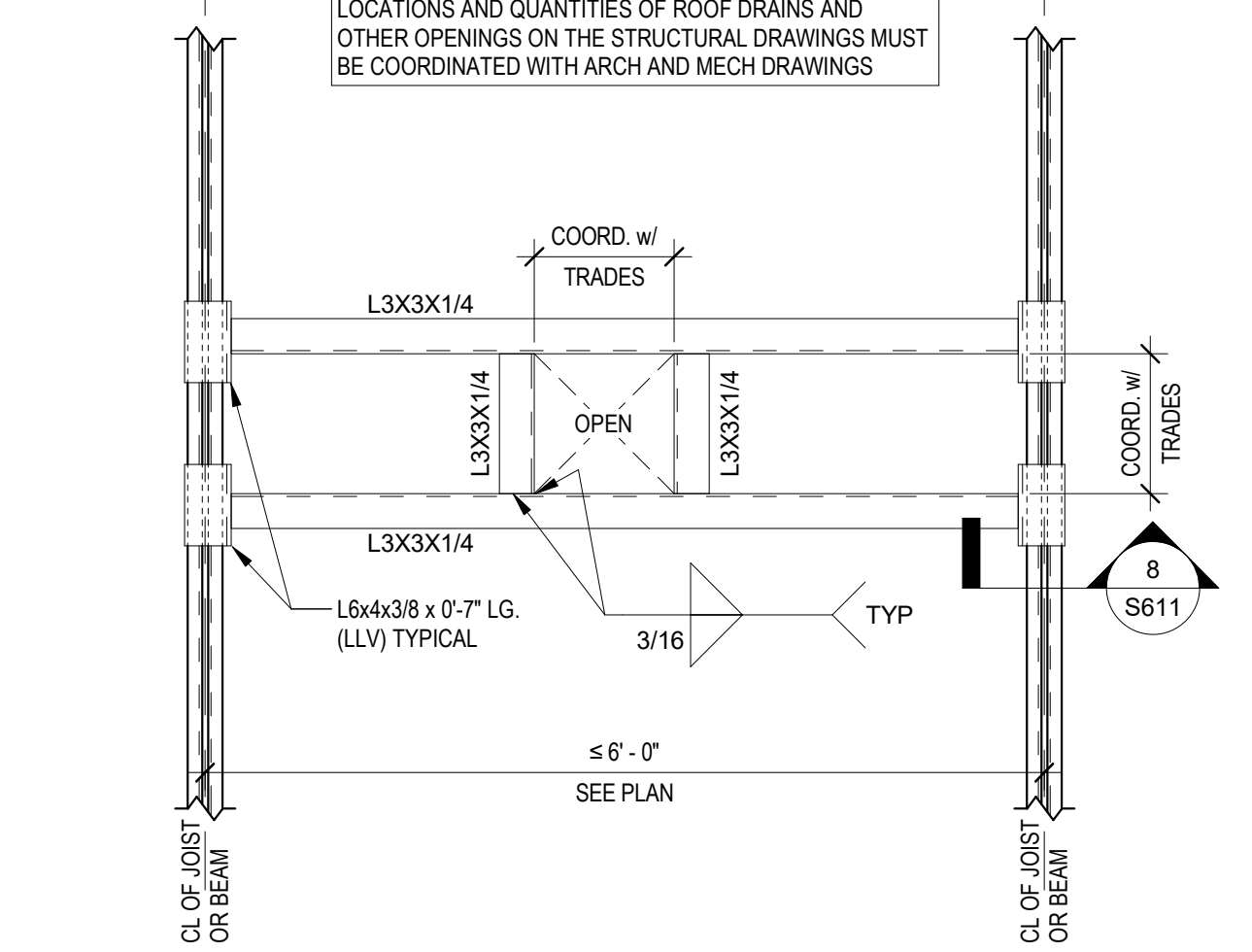
A/E Job Number:  
23432



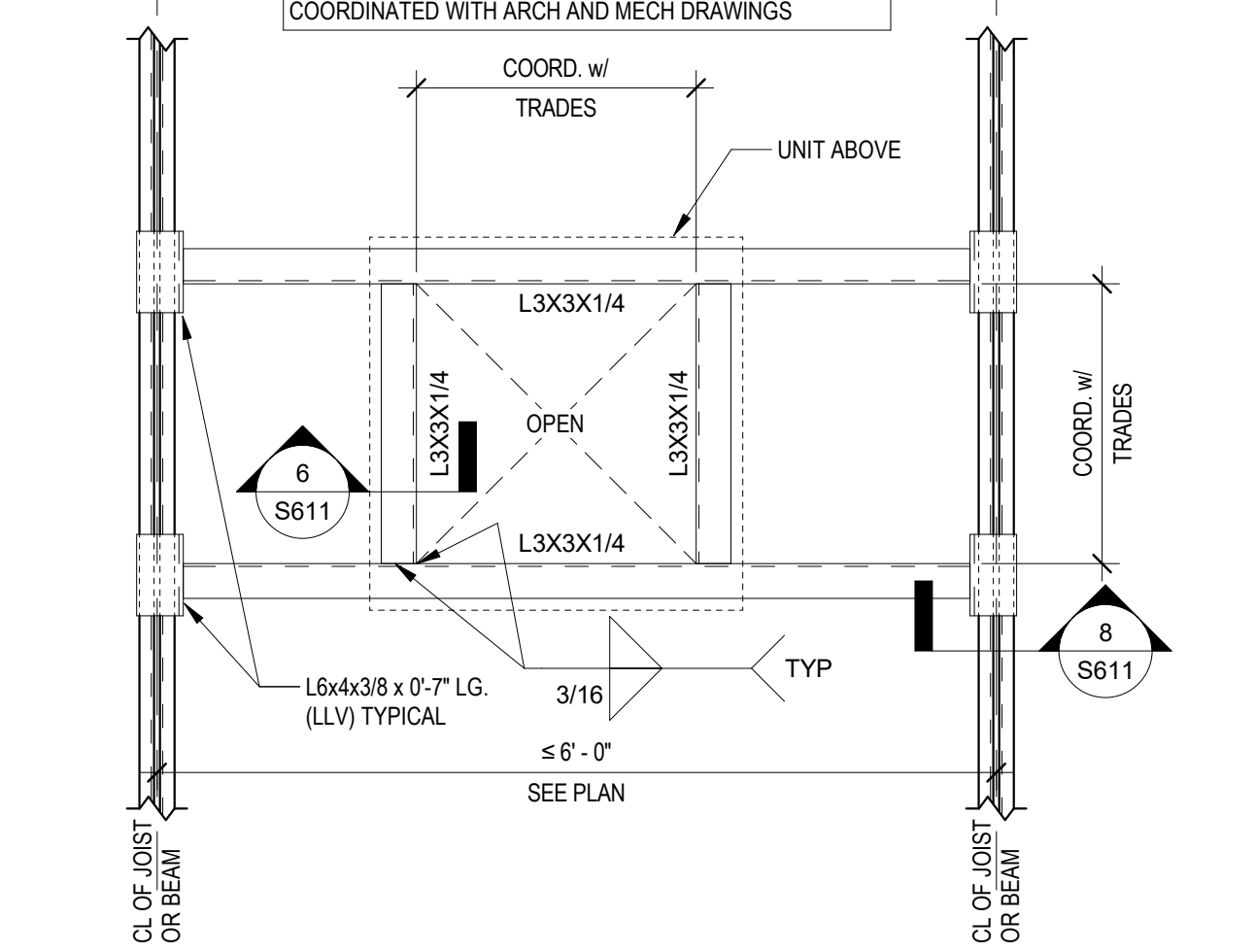
1 TYPICAL STEEL JOIST REINF AT CONCENTRATED LOADS  
S611 3/4" = 1'-0"



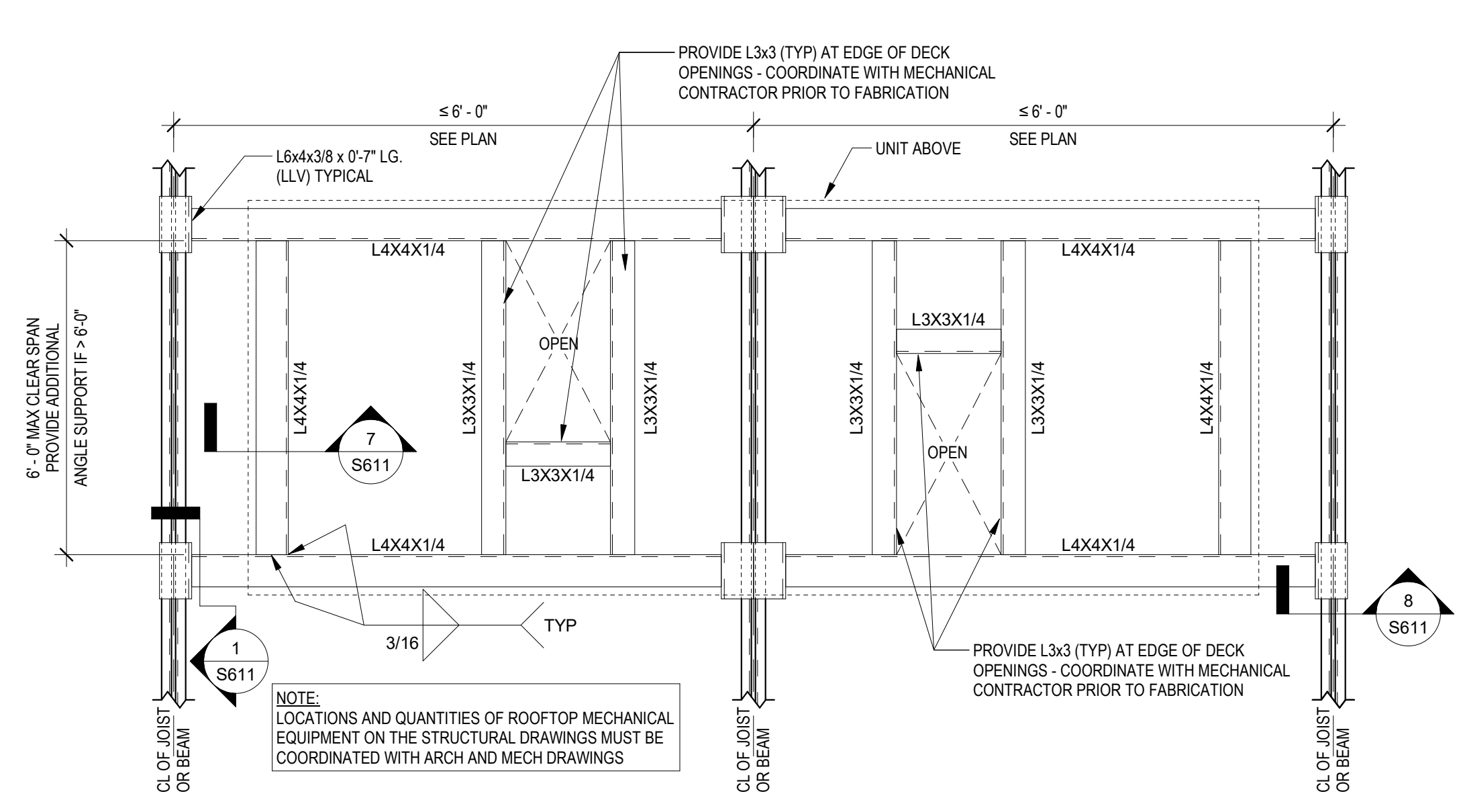
2 ROOF DECK REINFORCEMENT AT SMALL PENETRATIONS  
S611 1" = 1'-0"



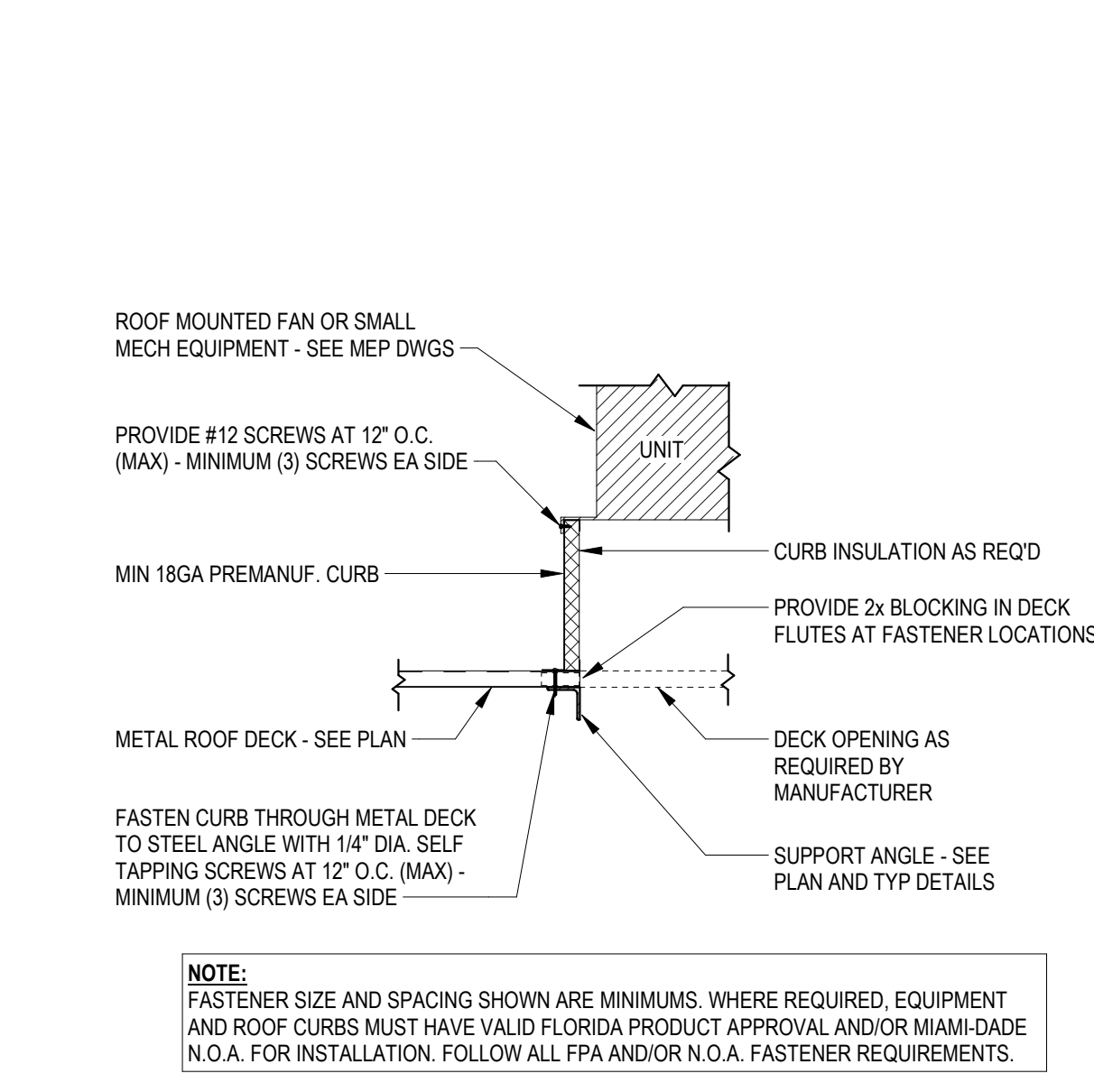
3 ROOF DRAIN AND OPENINGS > 12"  
S611 3/4" = 1'-0"



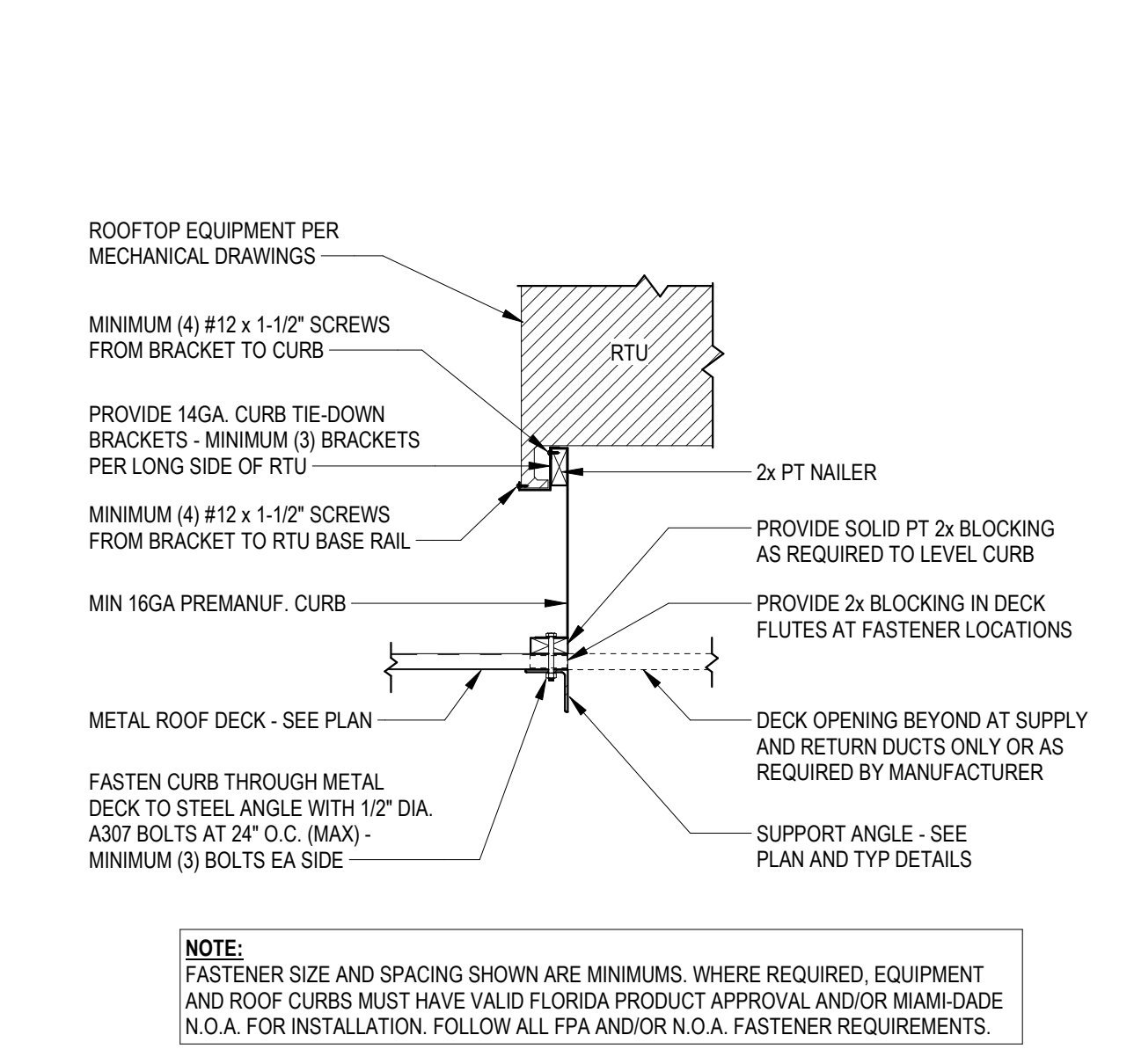
4 TYPICAL SMALL FAN / ROOF OPENING DETAIL  
S611 3/4" = 1'-0"



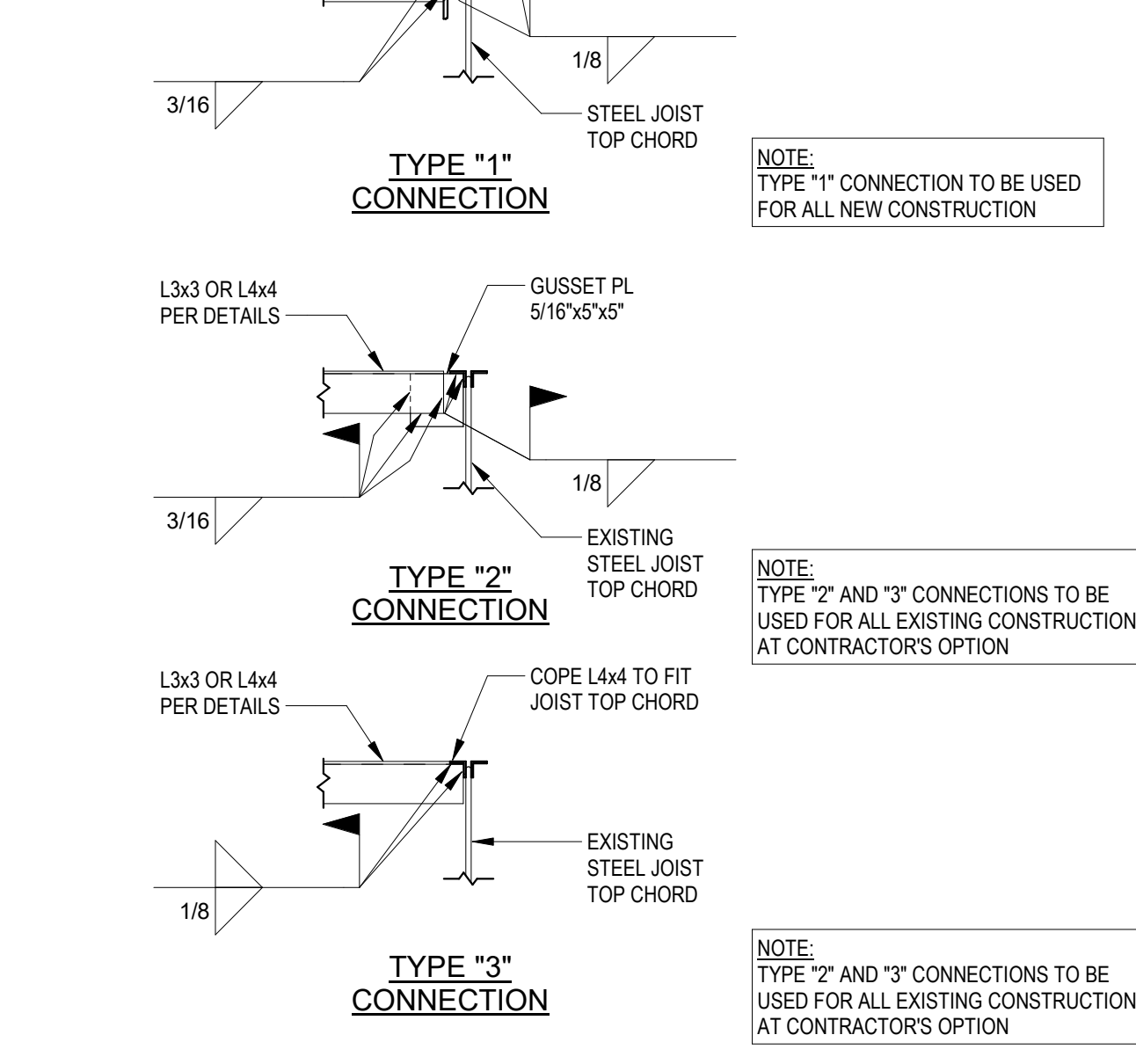
5 TYPICAL RTU/DECK OPENING DETAIL - 3 OR MORE SUPPORT JOISTS  
S611 3/4" = 1'-0"



6 TYPICAL SMALL FAN CONNECTION DETAIL  
S611 3/4" = 1'-0"



7 TYPICAL RTU / CURB CONNECTION DETAIL  
S611 3/4" = 1'-0"

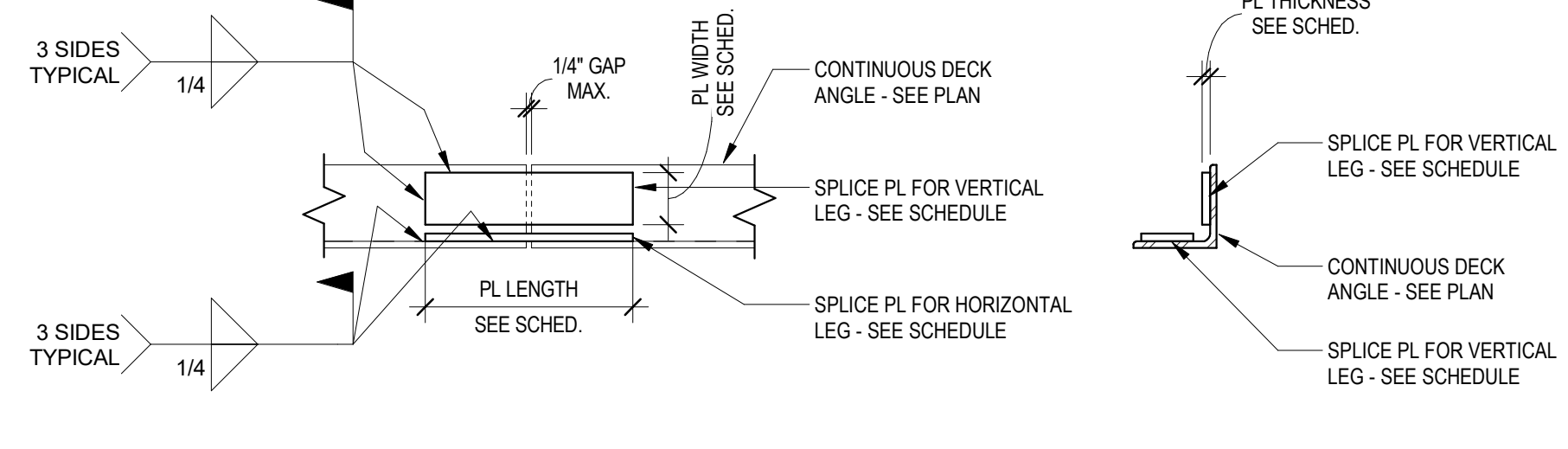


8 TYP SUPPORT ANGLE CONNECTION TO JOIST  
S611 3/4" = 1'-0"

DECK ANGLE SPLICE PLATE SCHEDULE

ANGLE LEG SIZE	PL THICKNESS (IN)	PL WIDTH (IN)	PL LENGTH (IN)
3" x 1/4"	3/8"	2"	8"
3" x 5/16"	1/2"	2"	8"
3" x 3/8"	5/8"	2"	8"
4" x 1/4"	3/8"	2-1/2"	8"
4" x 5/16"	1/2"	2-1/2"	8"
4" x 3/8"	1/2"	2-1/2"	8"
5" x 1/4"	3/8"	3"	10"
5" x 5/16"	1/2"	3"	10"
5" x 3/8"	1/2"	3"	10"
5" x 1/2"	3/4"	3"	14"
6" x 1/4"	3/8"	4"	12"
6" x 5/16"	3/8"	4"	12"
6" x 3/8"	1/2"	4"	12"
6" x 1/2"	3/4"	4"	16"

SCHEDULE EXAMPLE:  
SPECIFIED DECK SUPPORT ANGLE PER PLAN IS L5x3x5/16 (LLH).  
VERTICAL SPLICE PLATE TO BE PL 1/2" x 2" x 0-8" LG.  
HORIZONTAL SPLICE PLATE TO BE PL 1/2" x 3" x 0-10" LG.  
CONTRACTOR MAY ELECT TO USE EQUAL SIZE PLATES PROVIDED THAT THE MINIMUM PLATE SIZE IN THE SCHEDULE IS MET.

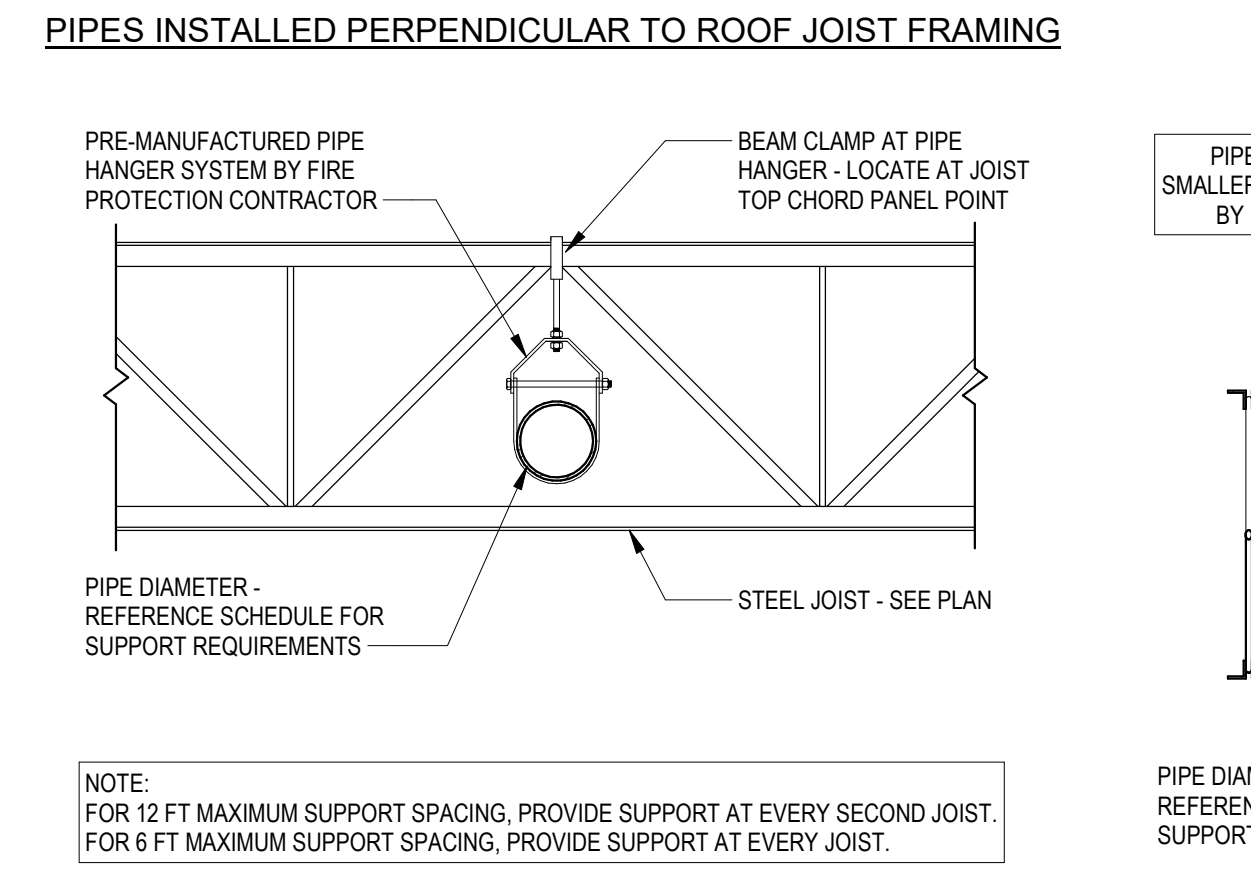


9 TYPICAL DECK ANGLE SPLICE DETAIL  
S611 1 1/2" = 1'-0"

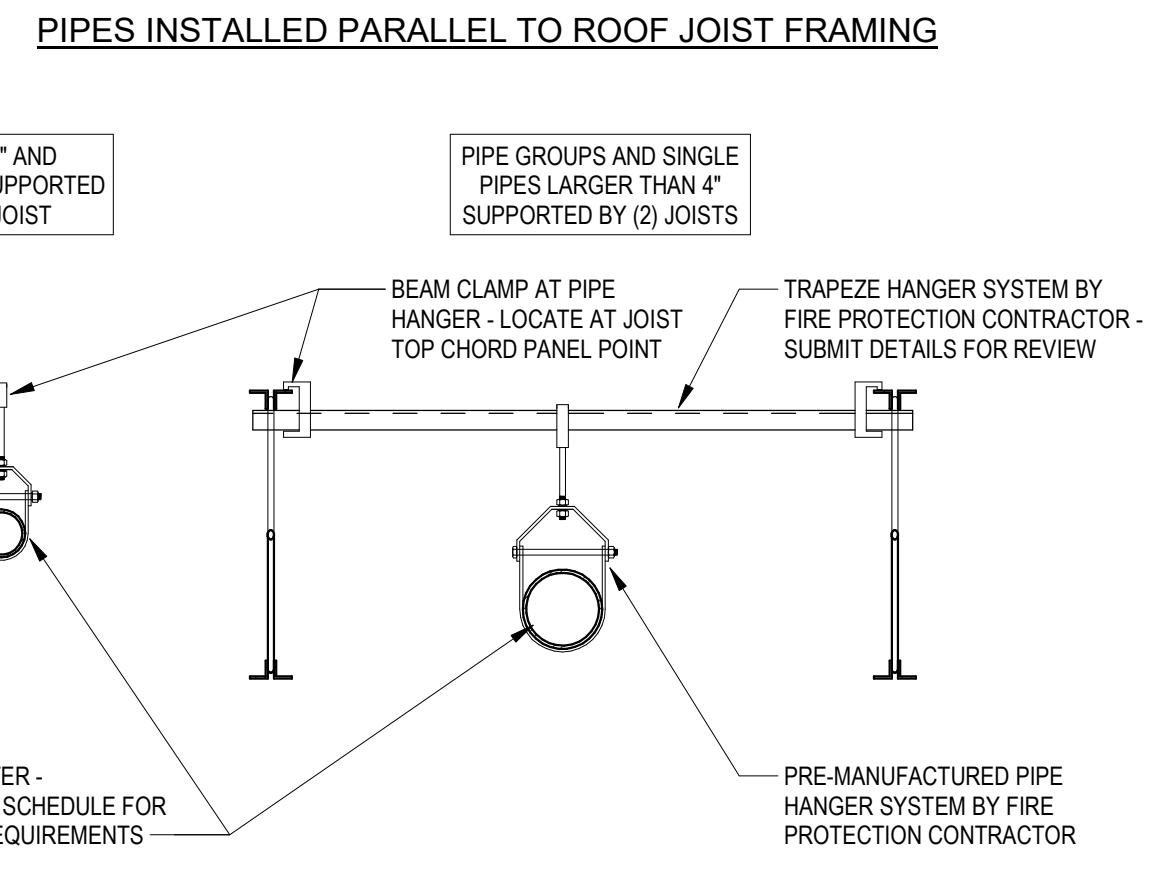
WATER PIPE SUPPORT SCHEDULE

PIPE NOMINAL DIA. (IN)	PIPE WEIGHT (LB/FT)	PARALLEL PIPE MAX SUPPORT SPACING (FT)	CONCENTRATED LOAD ON SUPPORT (LB)	PERPENDICULAR PIPE MAX SUPPORT SPACING (FT)	CONCENTRATED LOAD ON SUPPORT (LB)
2 1/2"	8.0	12	96	12	96
3"	11.0	12	132	12	132
4"	17.0	8	136	12	204
6"	32.0	12*	192*	6	192
8"	51.0	12*	306*	6	306

NOTES:  
1. PIPES IN TABLE ARE ASSUMED TO BE SCHEDULE 40 (OR STANDARD) ASTM A53 GRADE B STEEL PIPE.  
2. PIPE WEIGHT PROVIDED INCLUDES WEIGHT OF PIPE + WATER.  
3. EXACT PIPING LAYOUT IS TO BE PROVIDED BY FIRE PROTECTION SPRINKLER SHOP DRAWINGS. SHOP DRAWINGS SHALL INCLUDE PROPOSED LOCATION OF ALL PIPE RUNS AND INTENDED PIPE SUPPORT DETAILS AND ACCESSORIES.  
4. (\*) IN SCHEDULE INDICATES PIPES RUNNING PARALLEL WITH JOISTS WHERE PIPE SUPPORTS MUST BE DISTRIBUTED ACROSS (2) JOISTS. LOAD PROVIDED IS THE EXPECTED CONCENTRATED LOAD ON EACH JOIST.  
5. ANY PROPOSED PIPING LARGER THAN 8" NOMINAL DIAMETER MUST BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER OF RECORD WITH PROPOSED PIPE SUPPORT DETAILS AND ROUTING.  
6. ALL PIPING SHALL BE INSTALLED SO THAT ALL PIPES, PIPE HANGERS, ETC. ARE LOCATED ABOVE THE BOTTOM OF THE ROOF FRAMING MEMBERS.

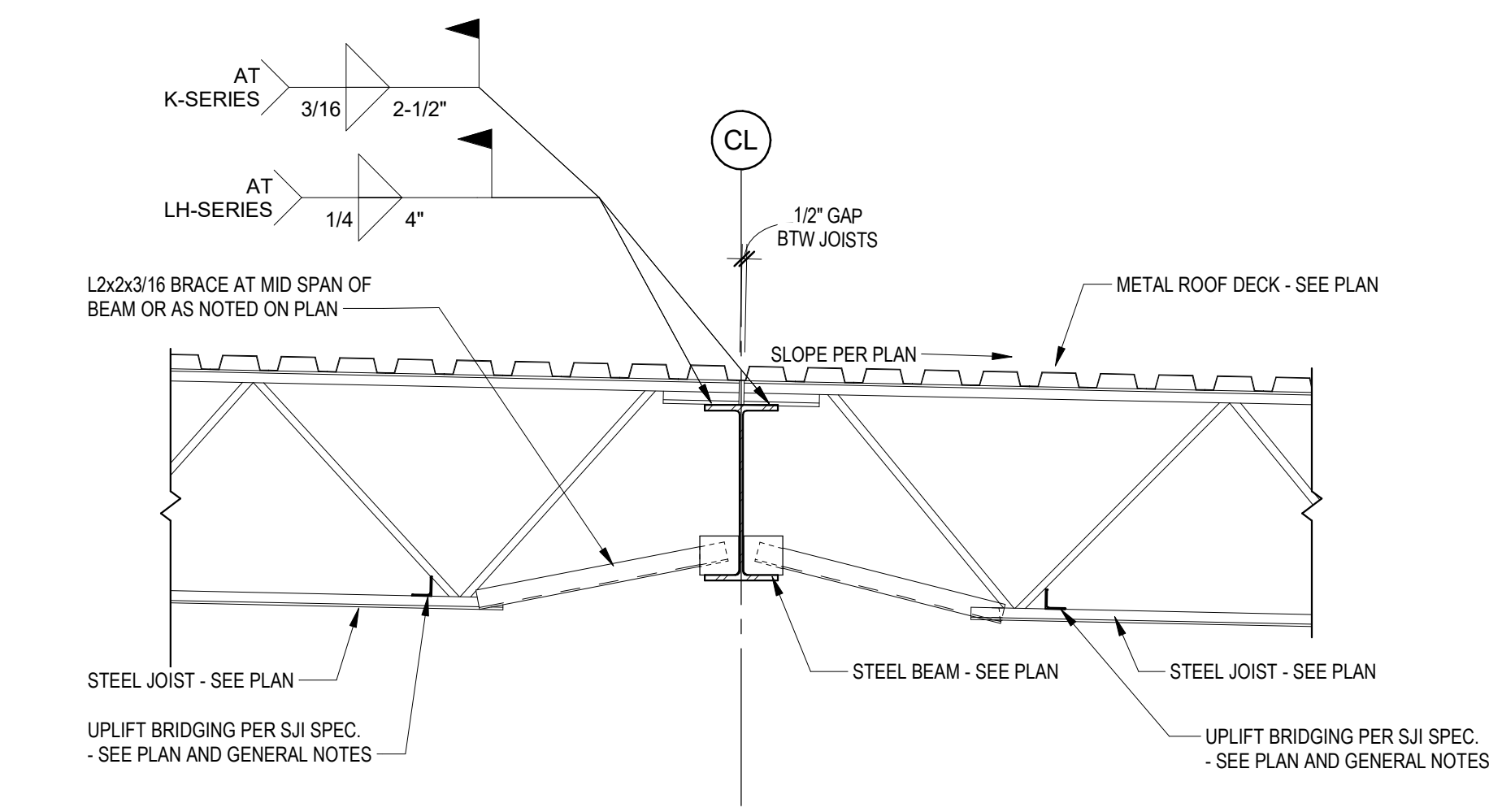


10 WATER PIPE SUPPORT SCHEDULE AND DETAILS  
S611 3/4" = 1'-0"



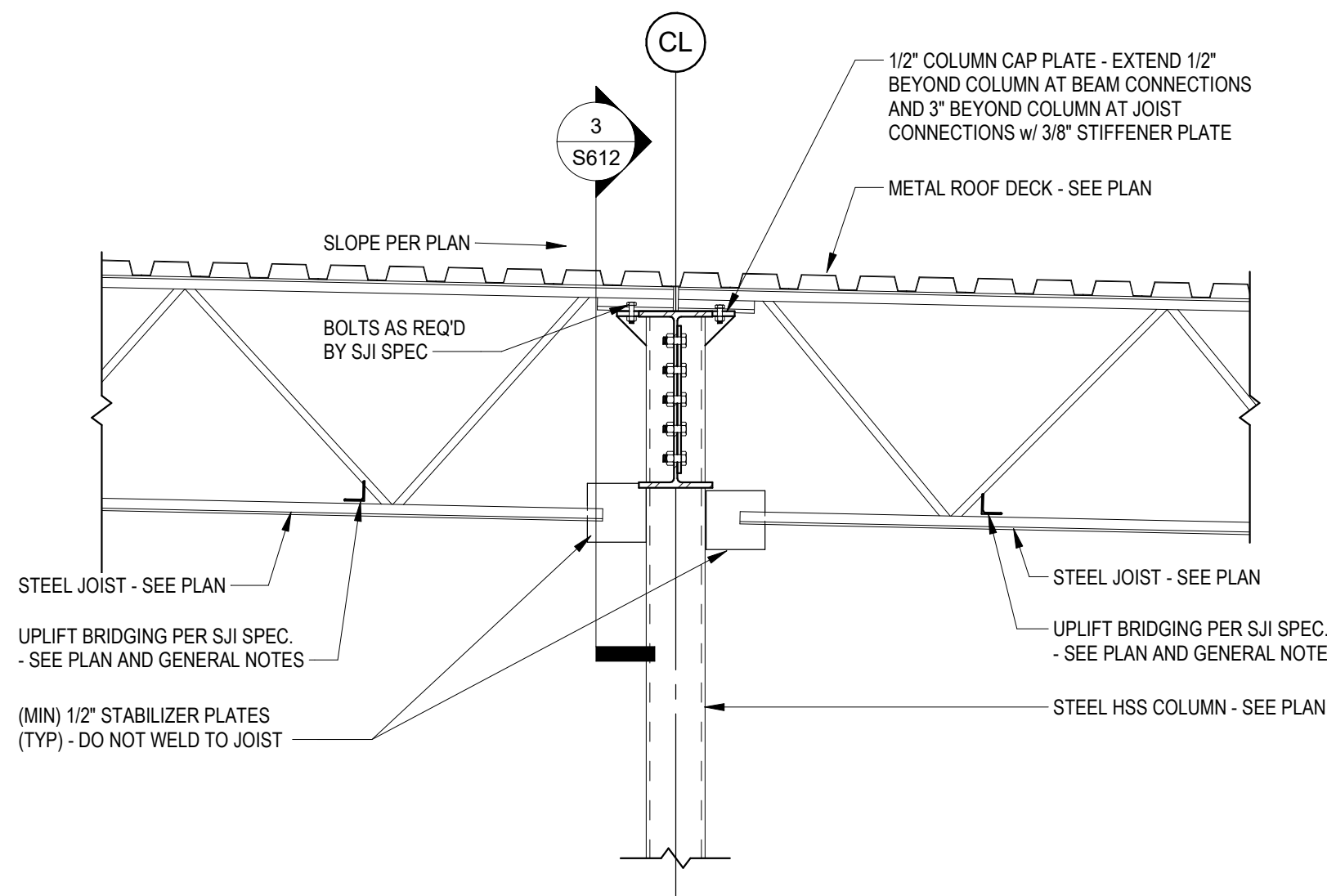
10 WATER PIPE SUPPORT SCHEDULE AND DETAILS  
S611 3/4" = 1'-0"





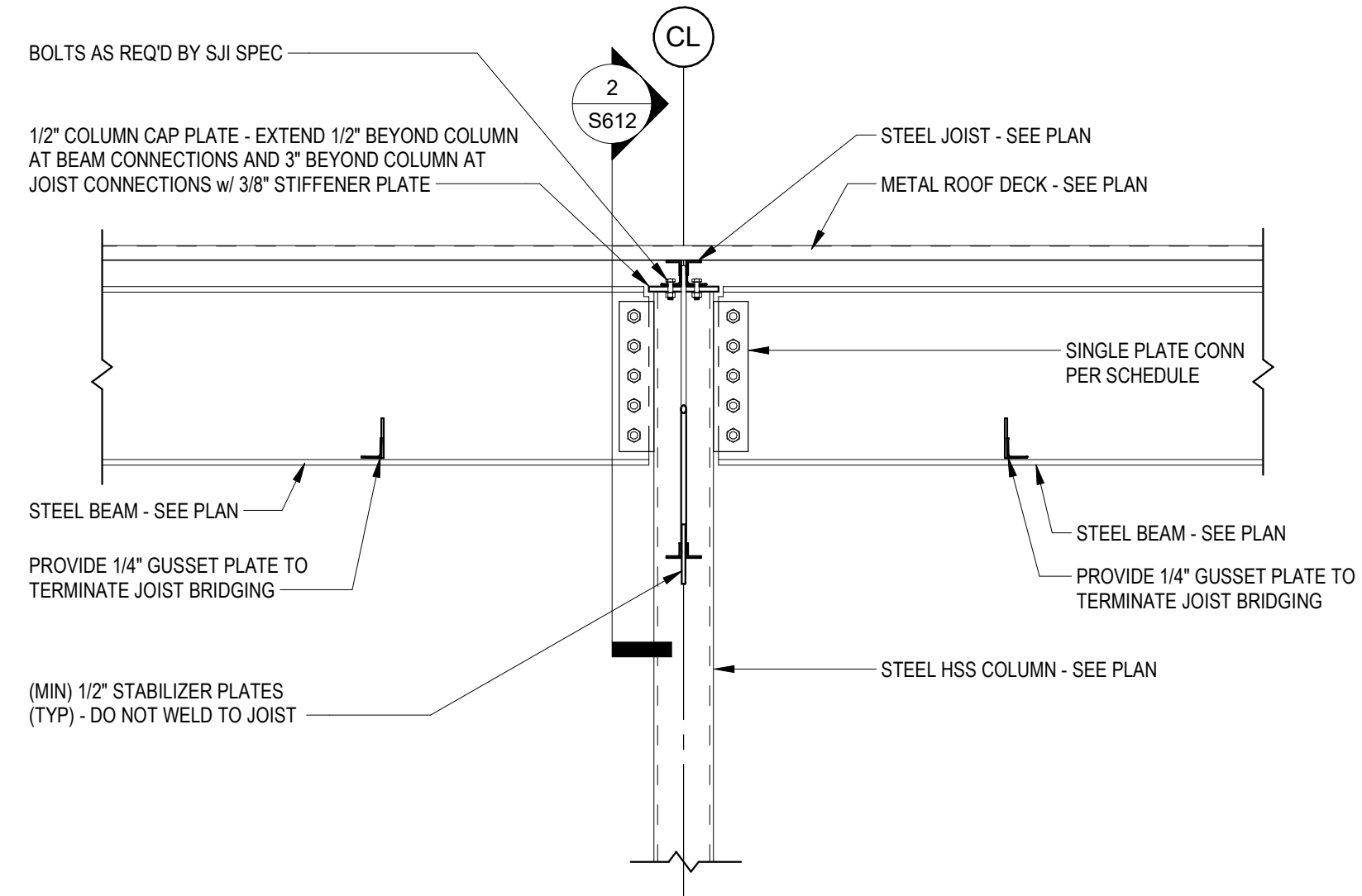
1 TYPICAL STEEL JOIST BRACE TO WIDE FLANGE GIRDER

S612 3/4" = 1'-0"



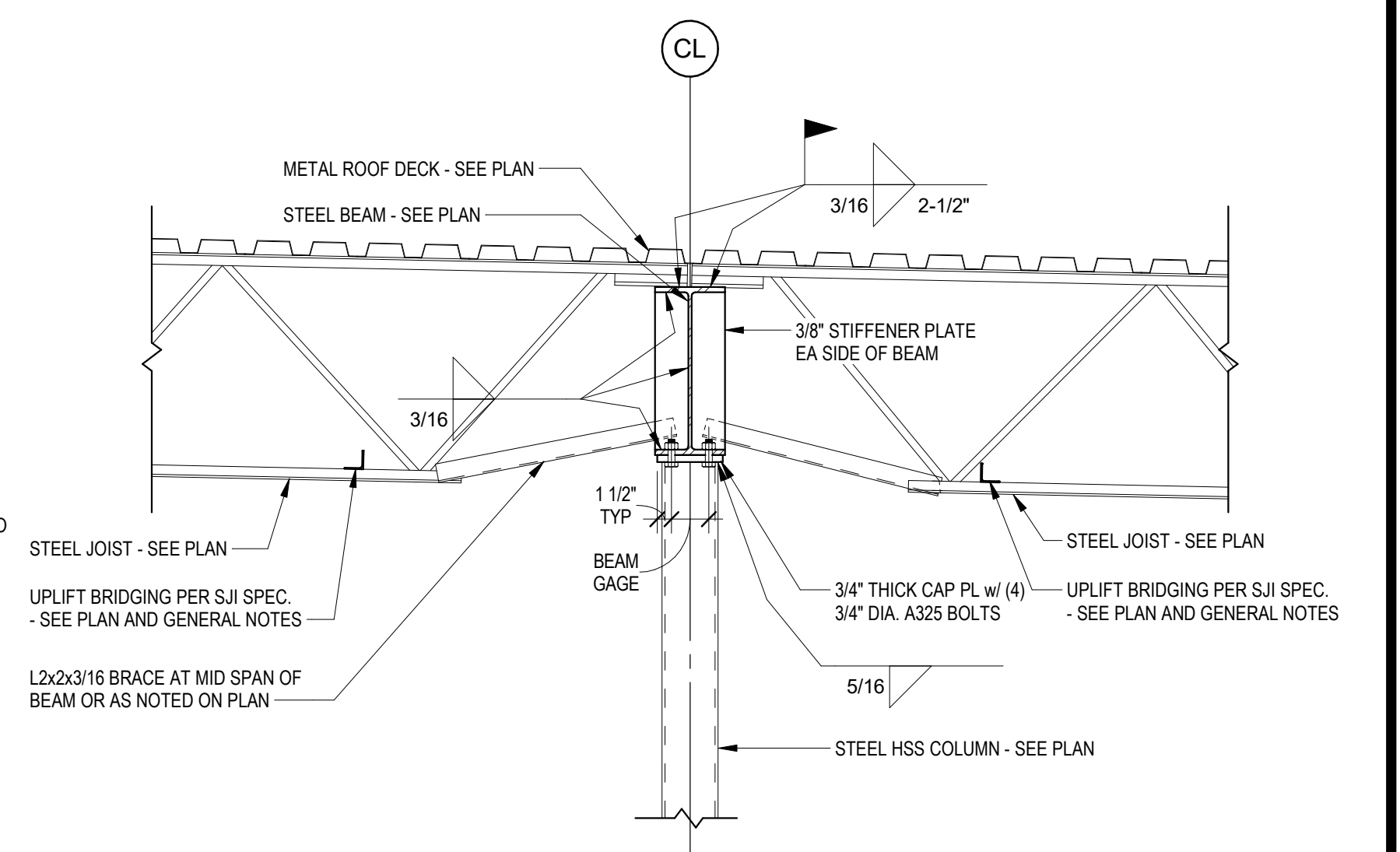
2 TYPICAL BEAM AND JOIST SECTION AT COLUMN

S612 3/4" = 1'-0"



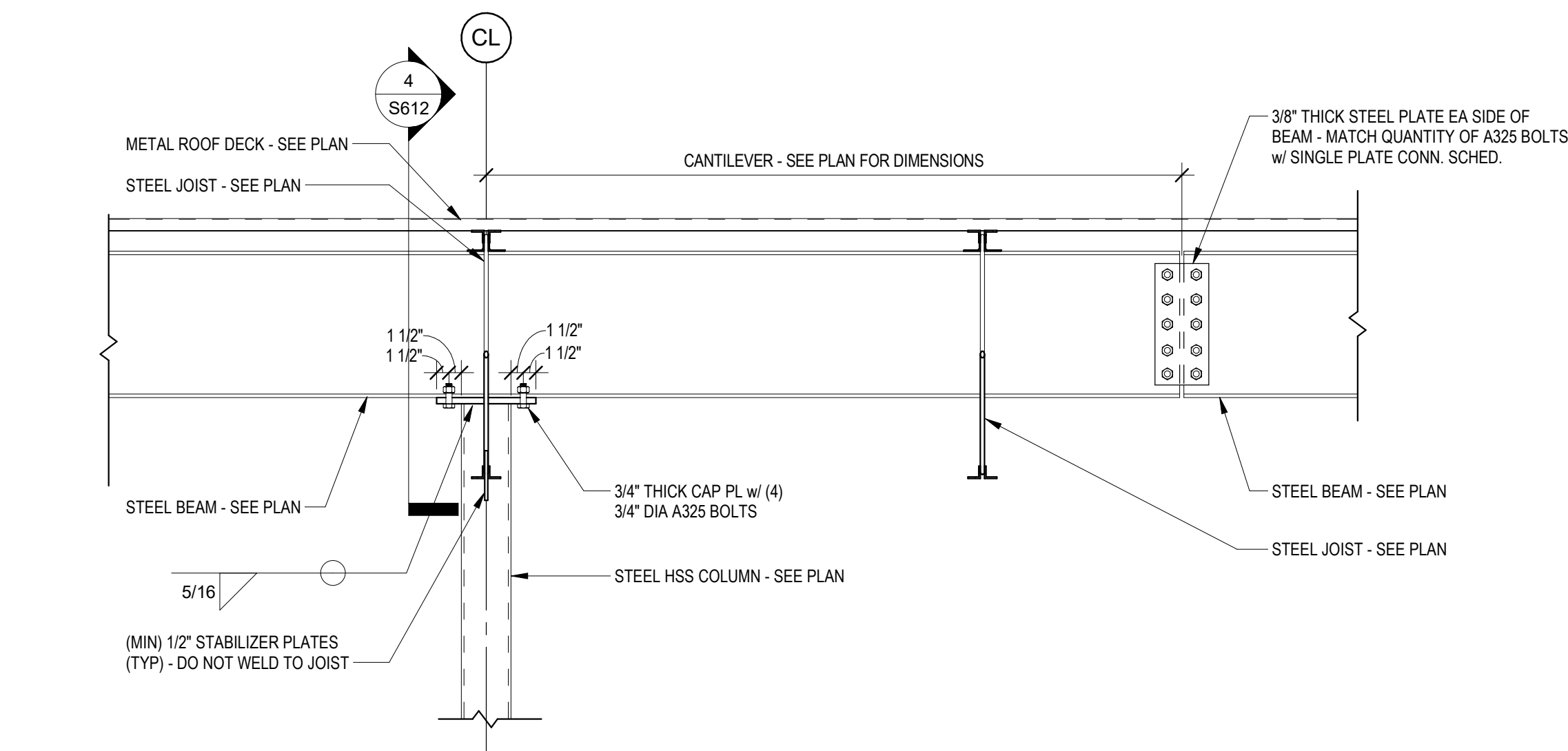
3 TYPICAL JOIST AND BEAM SECTION AT COLUMN

S612 3/4" = 1'-0"



4 TYPICAL CANTILEVER BEAM SECTION AT COLUMN

S612 3/4" = 1'-0"



5 TYPICAL CANTILEVER BEAM SECTION AT COLUMN

S612 3/4" = 1'-0"

Owner:



ARCHITECTURE  
135 W. Central Blvd., Suite 400  
Orlando, Florida 32801  
TEL: 407.363.6136  
© Copyright 2023

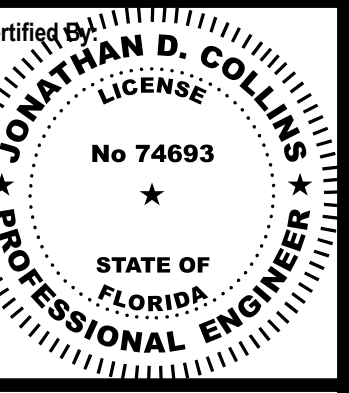
Revisions:

Scale:	AS NOTED
Date:	12/15/23
Drawn By:	SGZ
Checked By:	JDC

WEST MARKET LOT 10

WEST MARKET LOT 10  
14230 WEST COLONIAL DRIVE  
WINTER GARDEN, FL

FRAMING SECTIONS AND DETAILS



Electronic Signature:  
THIS DRAWING HAS BEEN DIGITALLY SIGNED AND SEALED BY JONATHAN D. COLLINS, P.E. LICENSE NO. 74693 ON 12/15/2023. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

*JDC*

This drawing has been electronically signed and sealed on the date shown in the seal, which is my signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

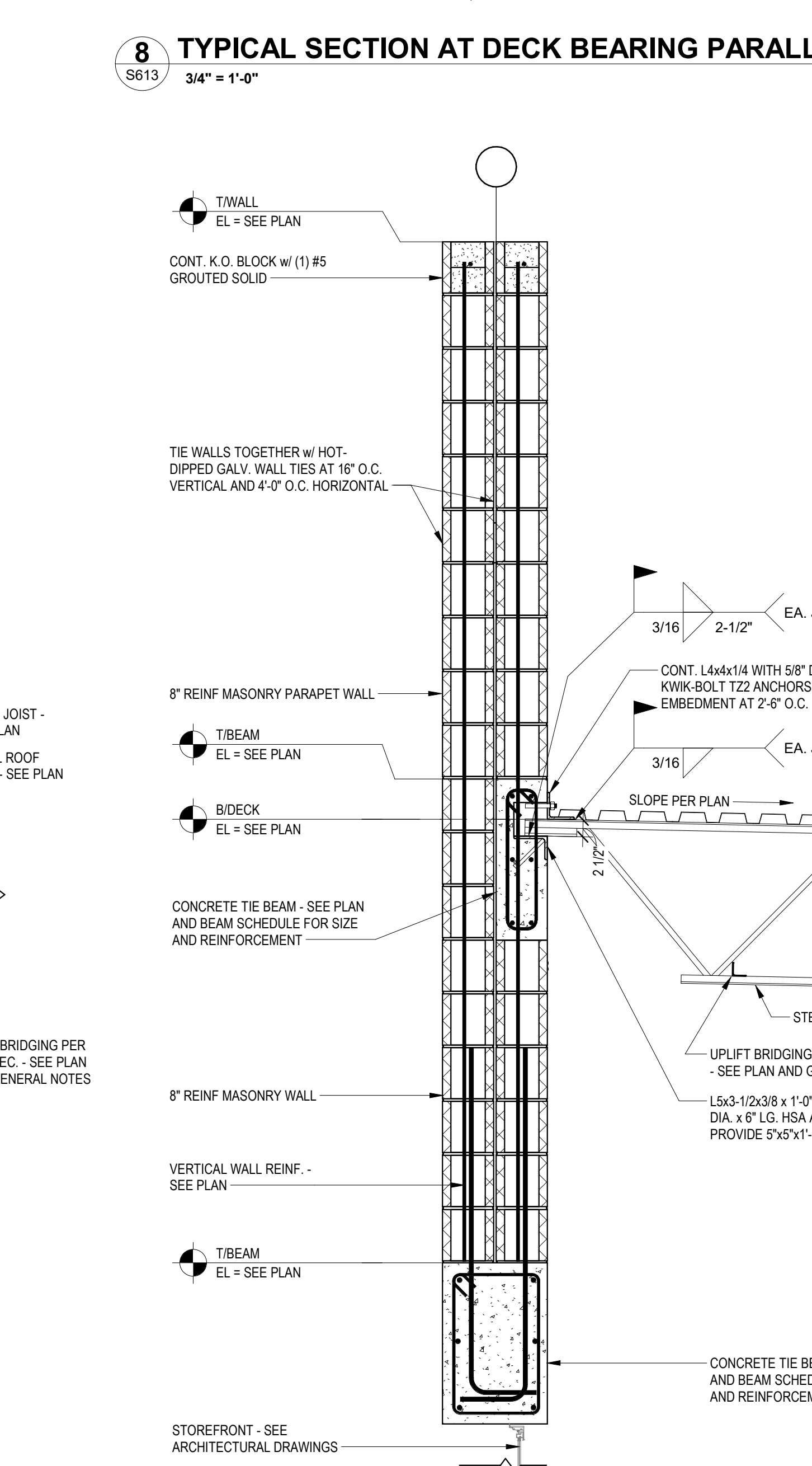
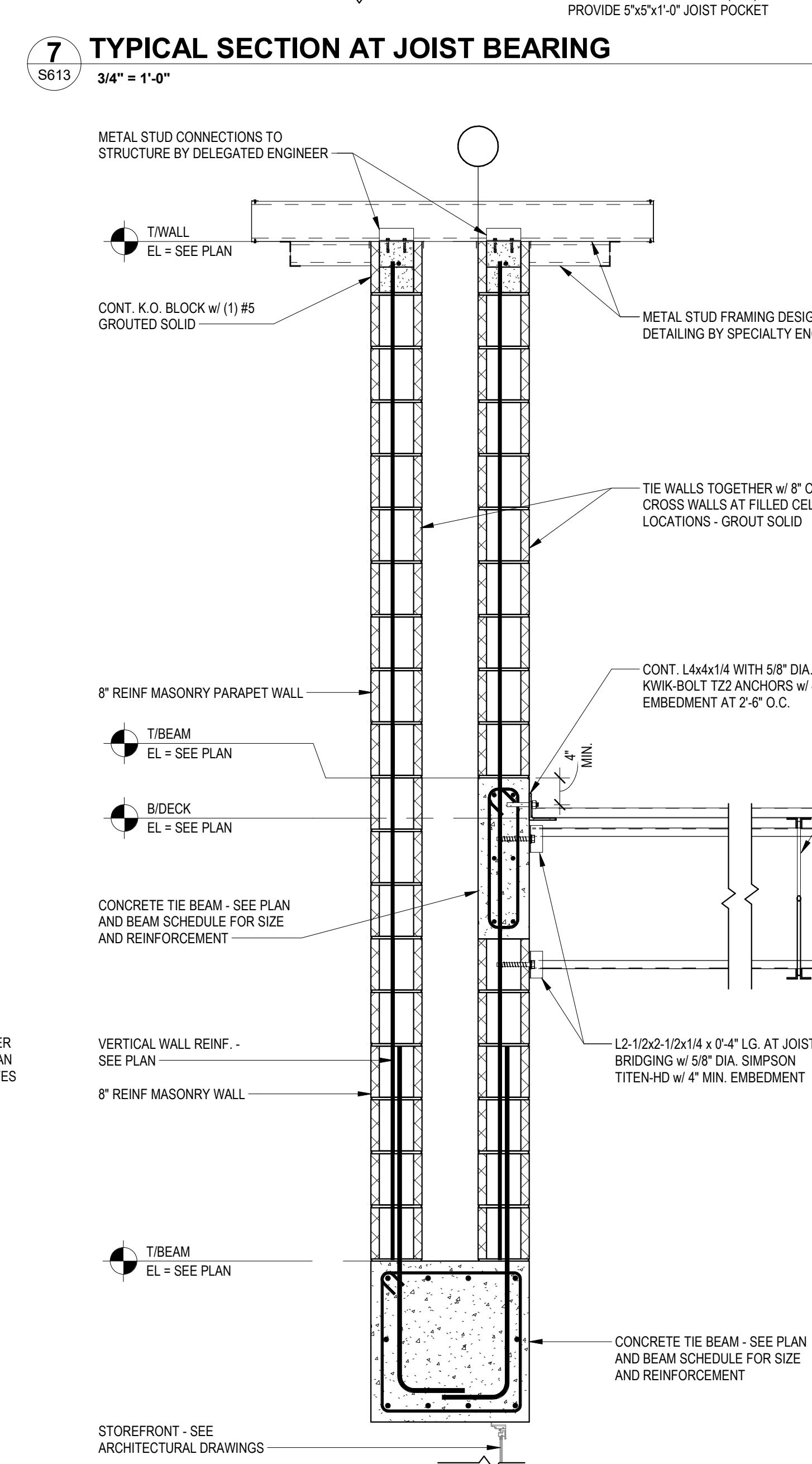
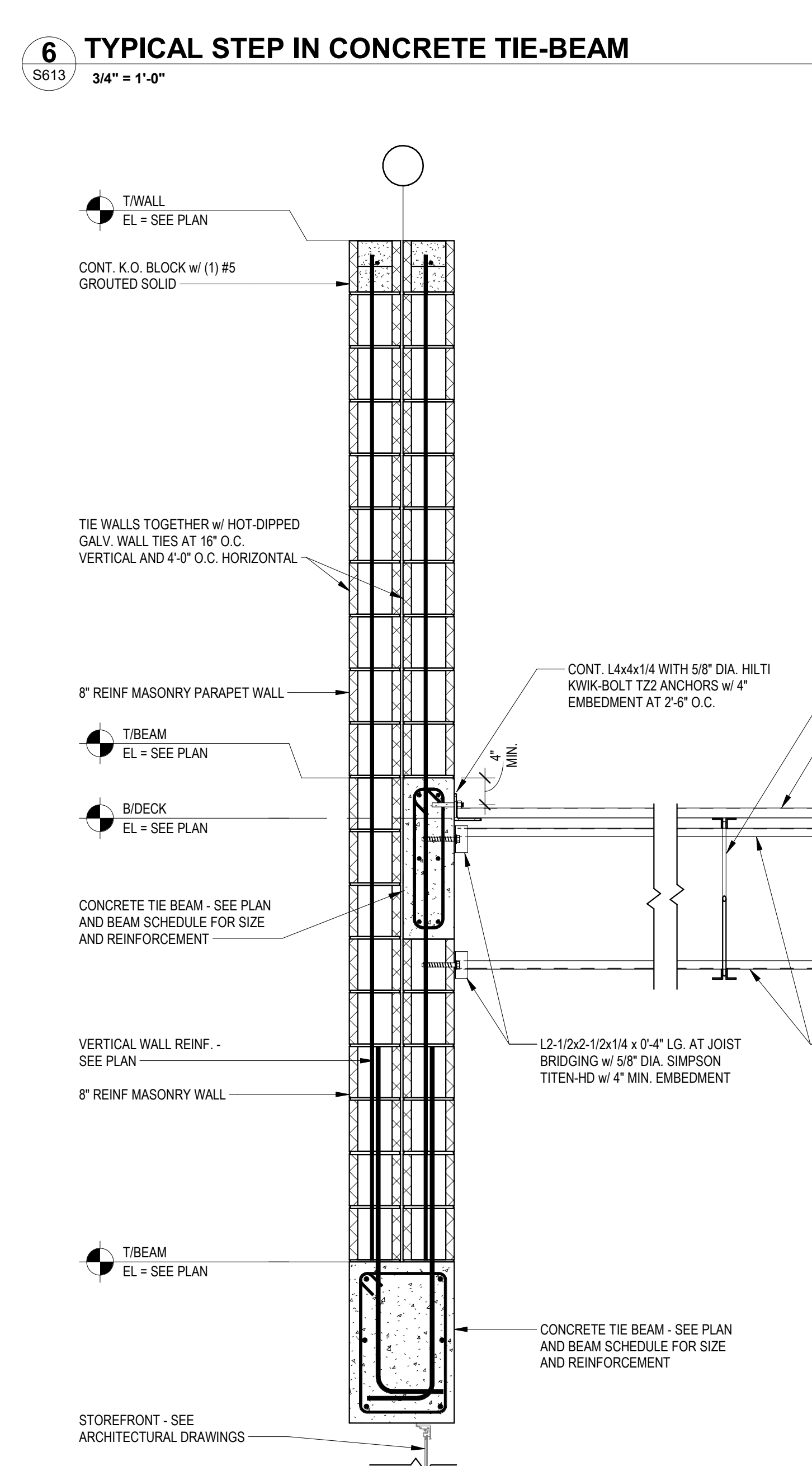
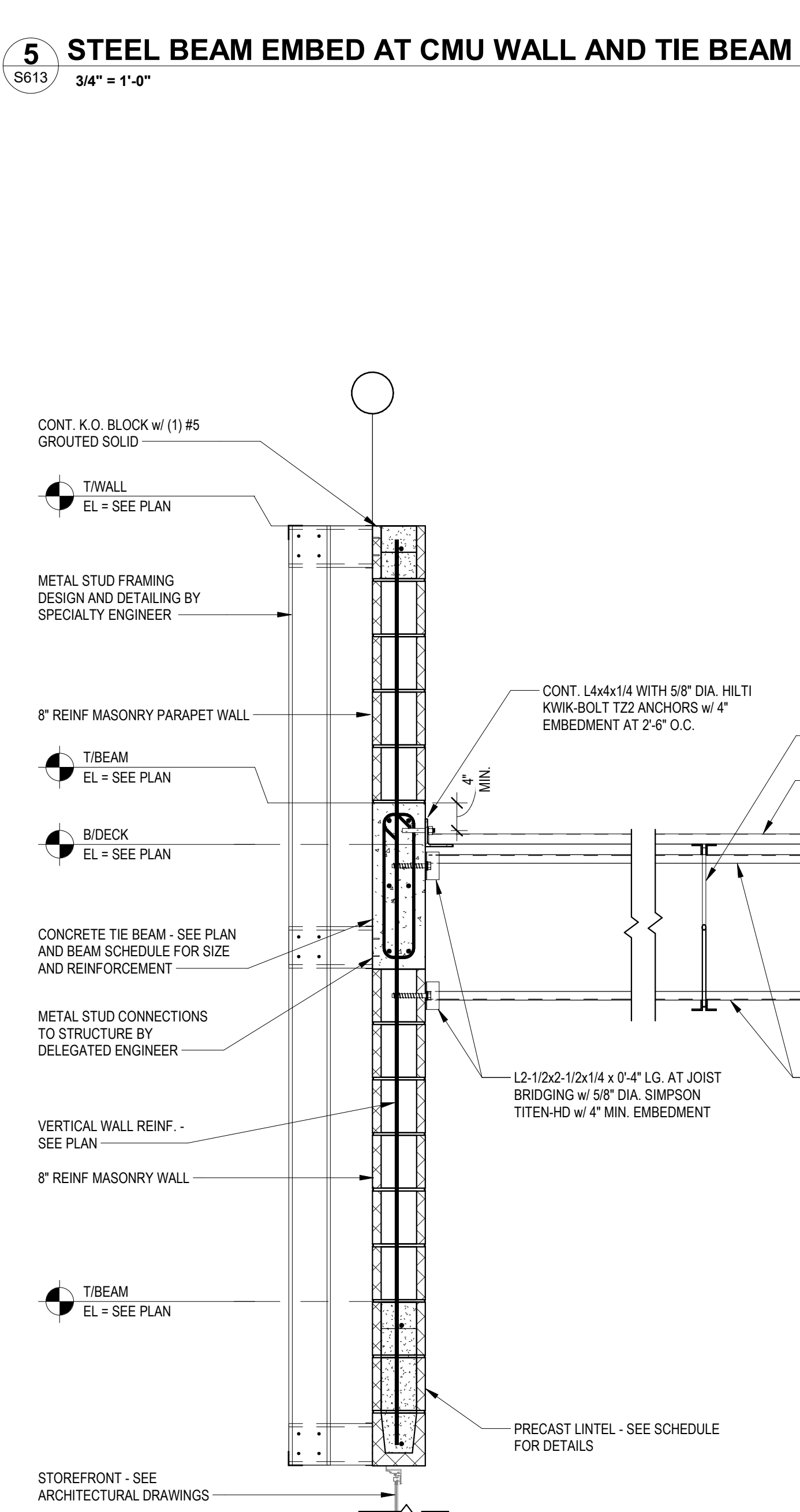
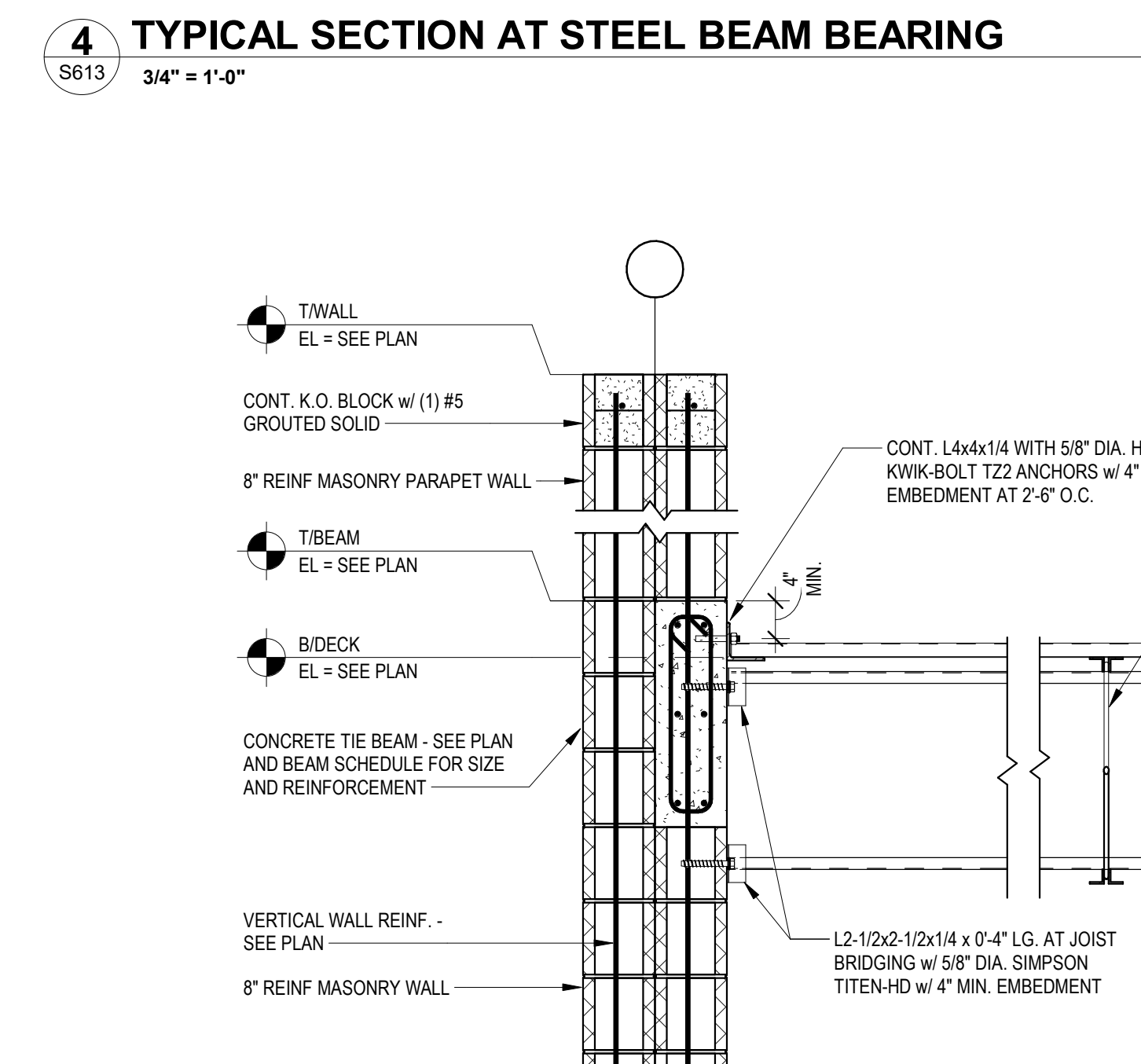
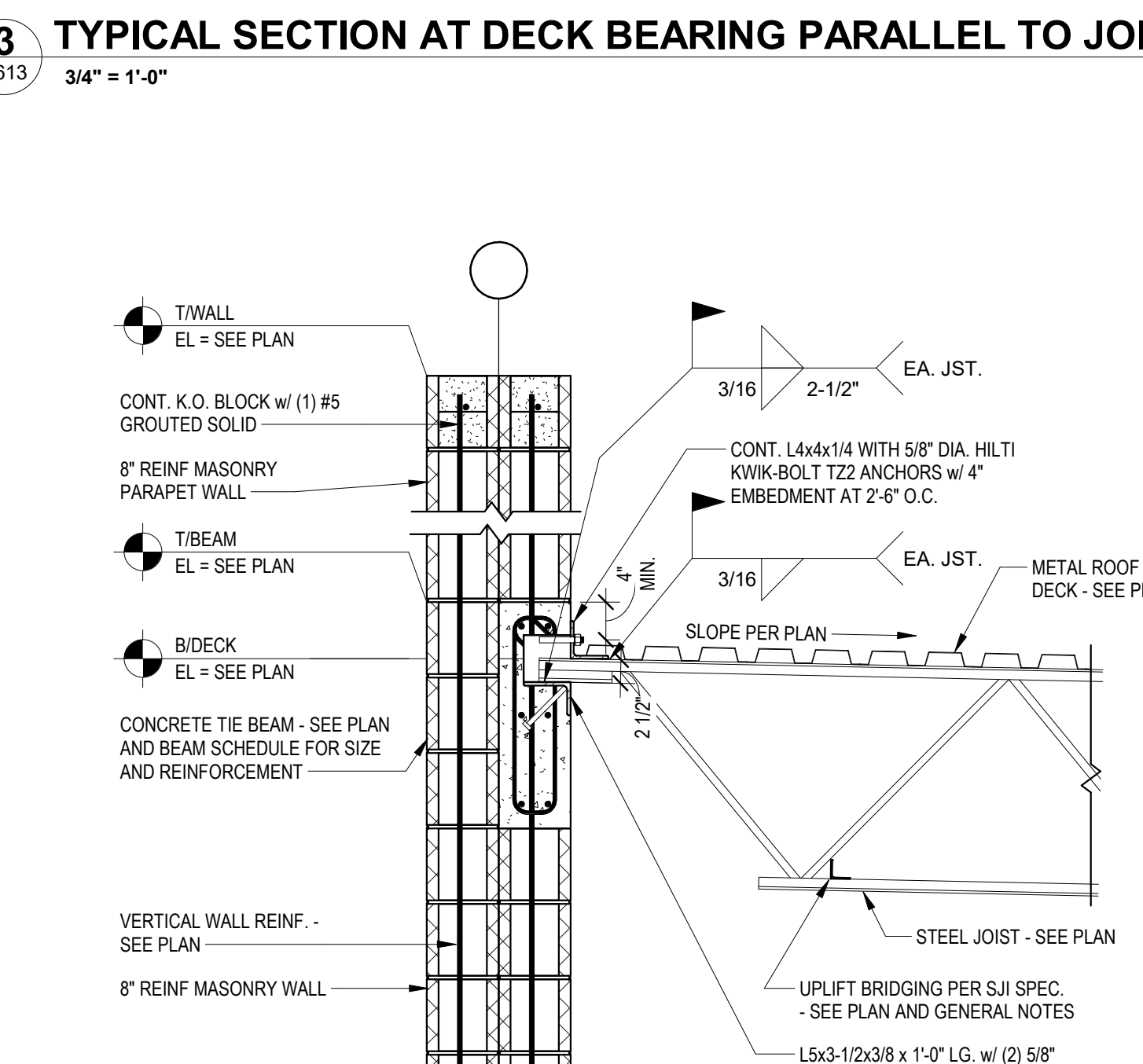
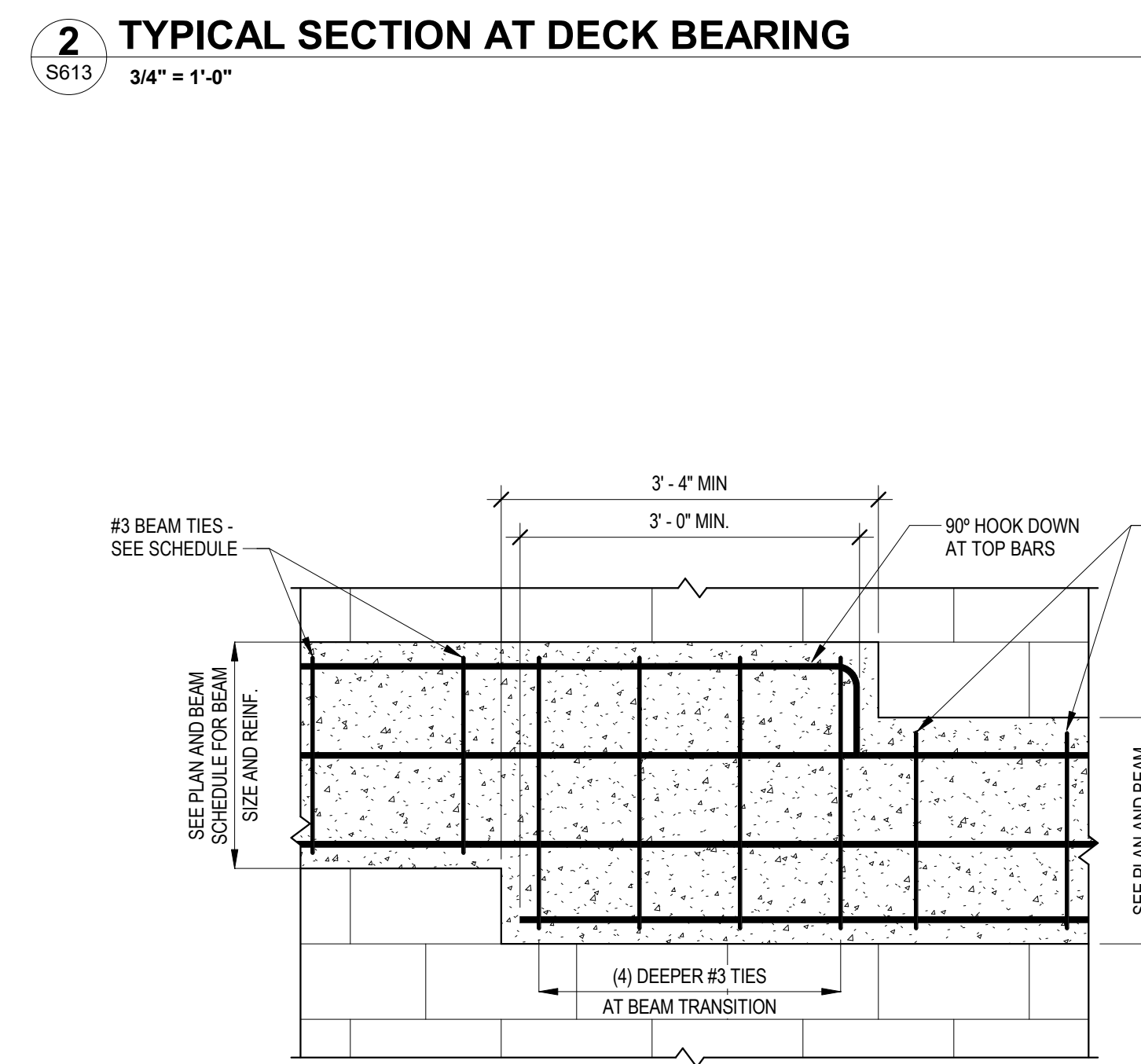
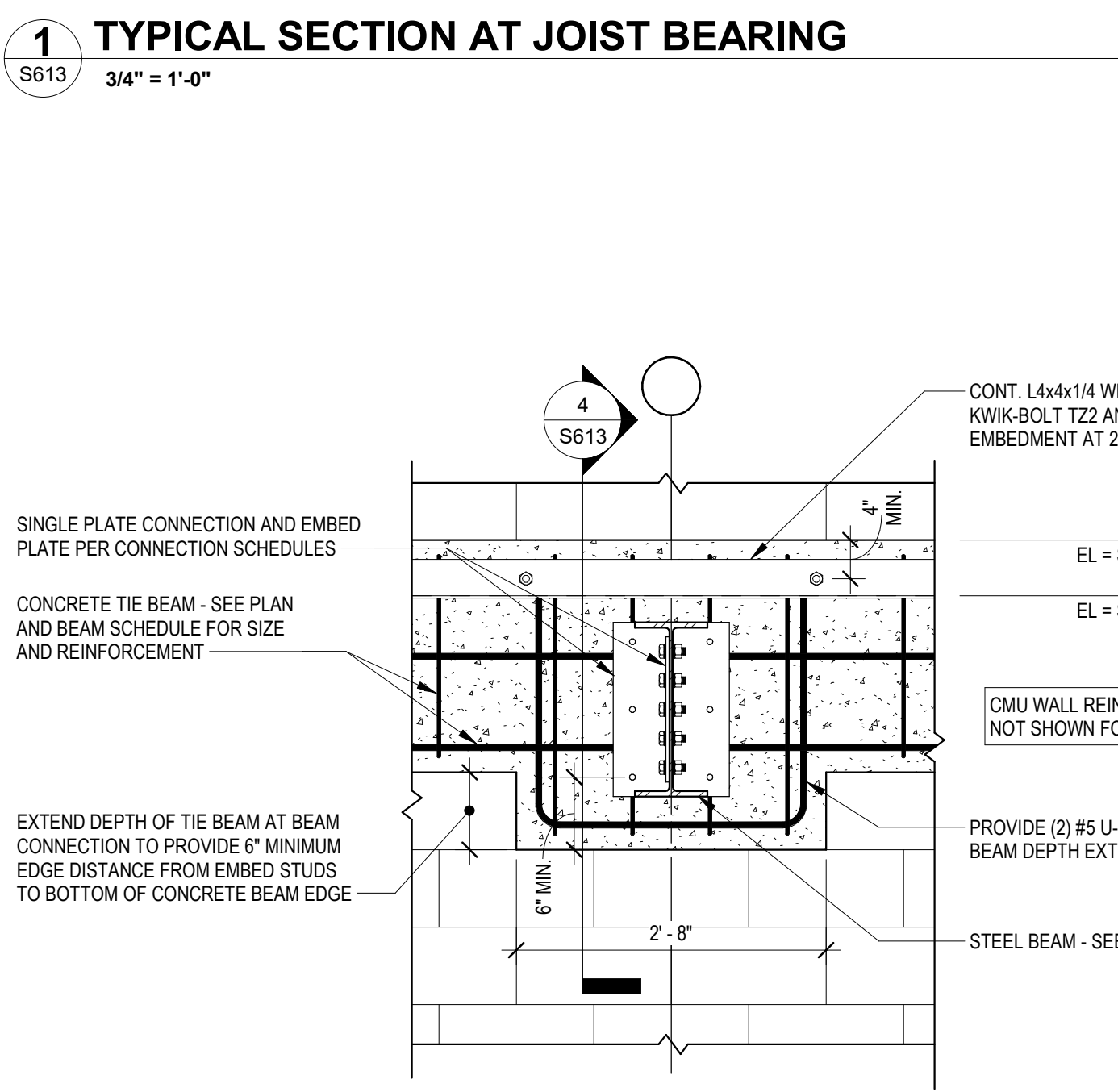
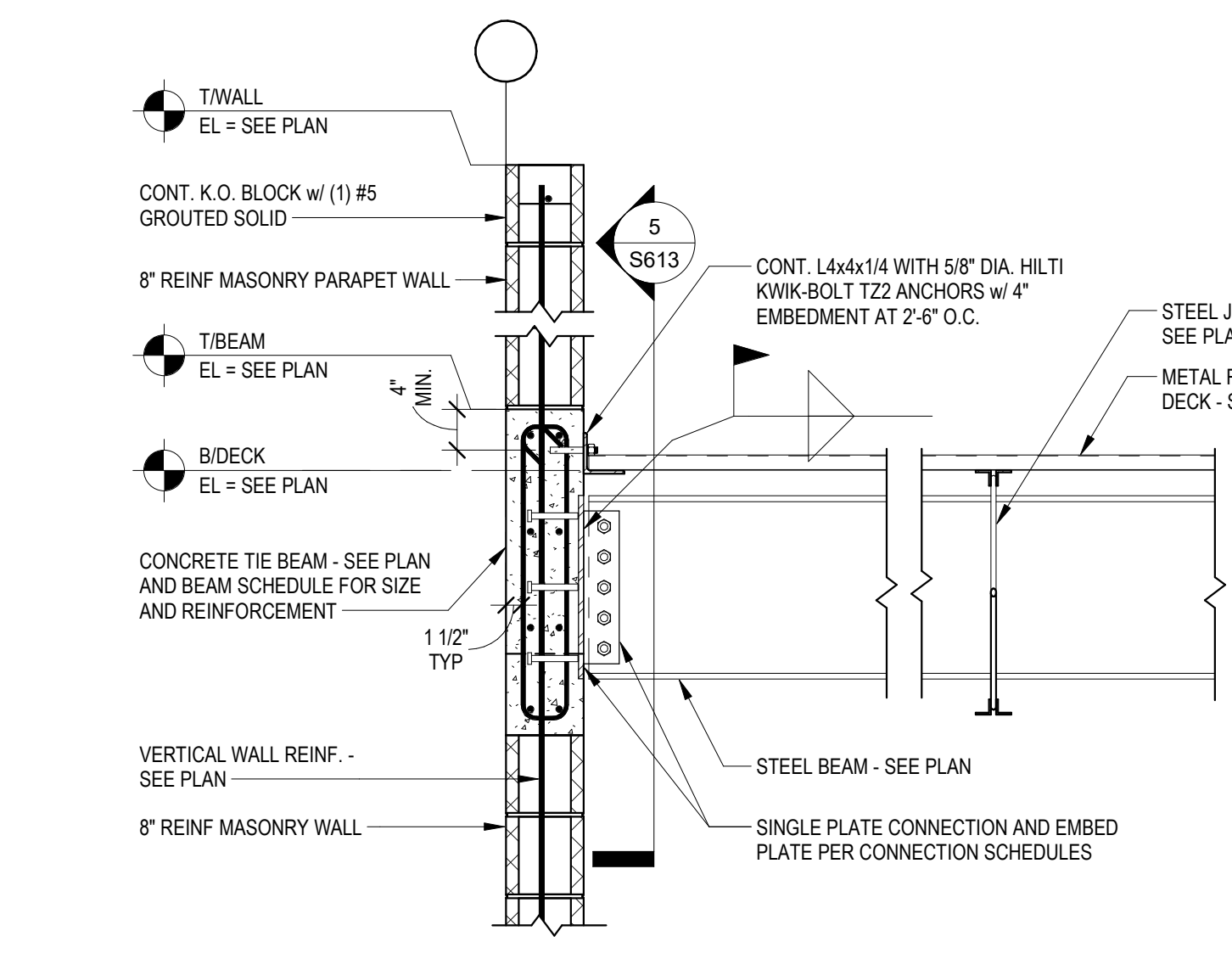
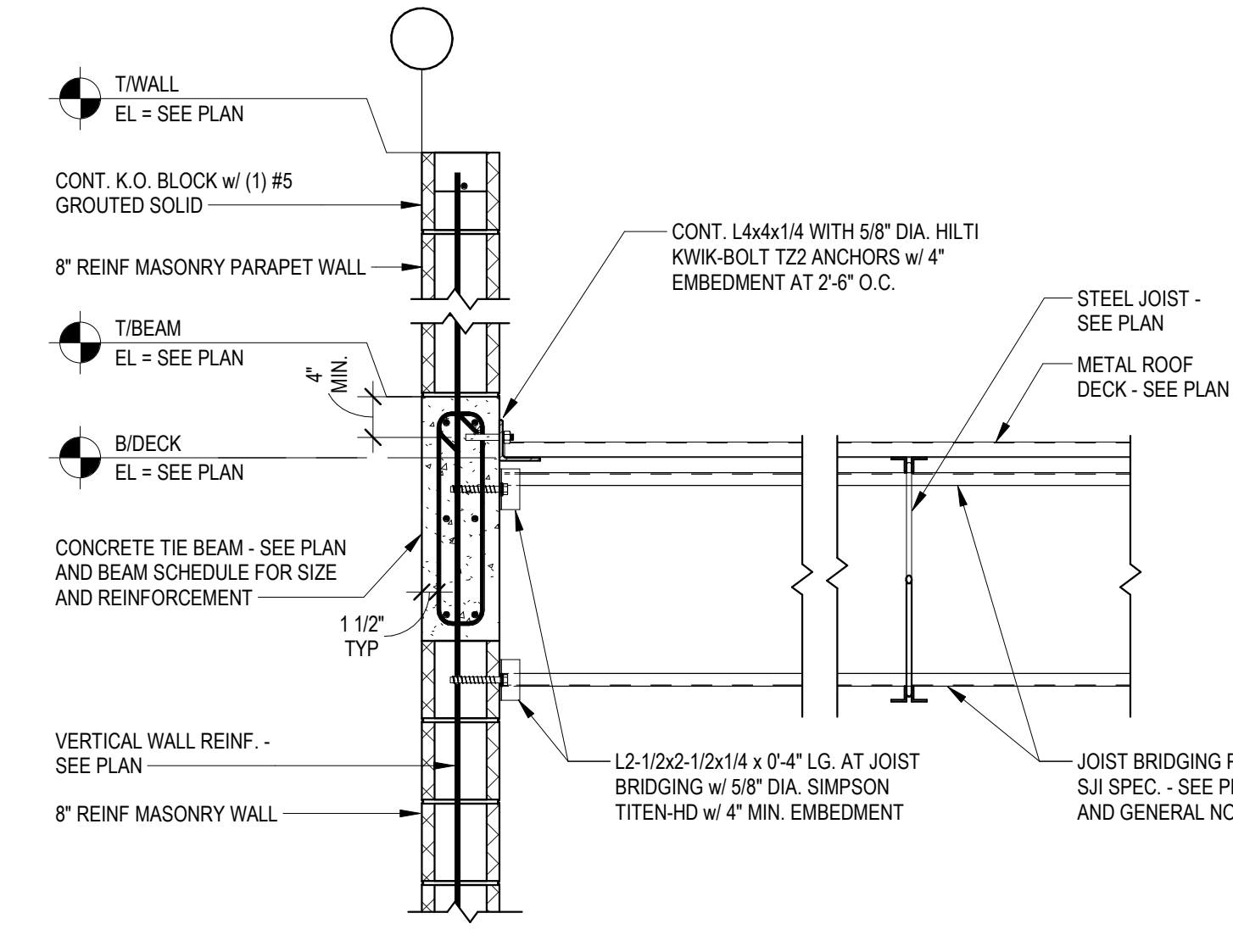
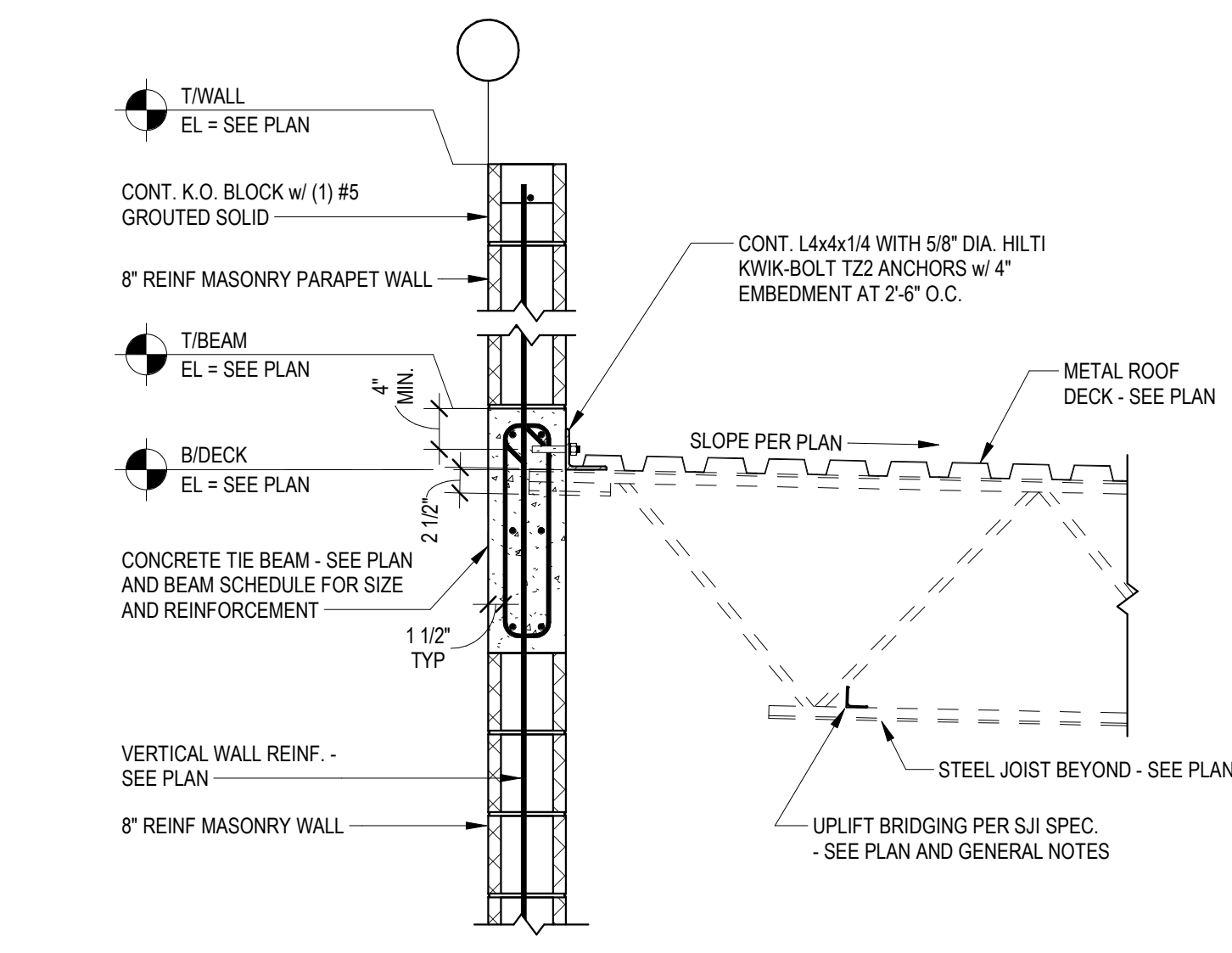
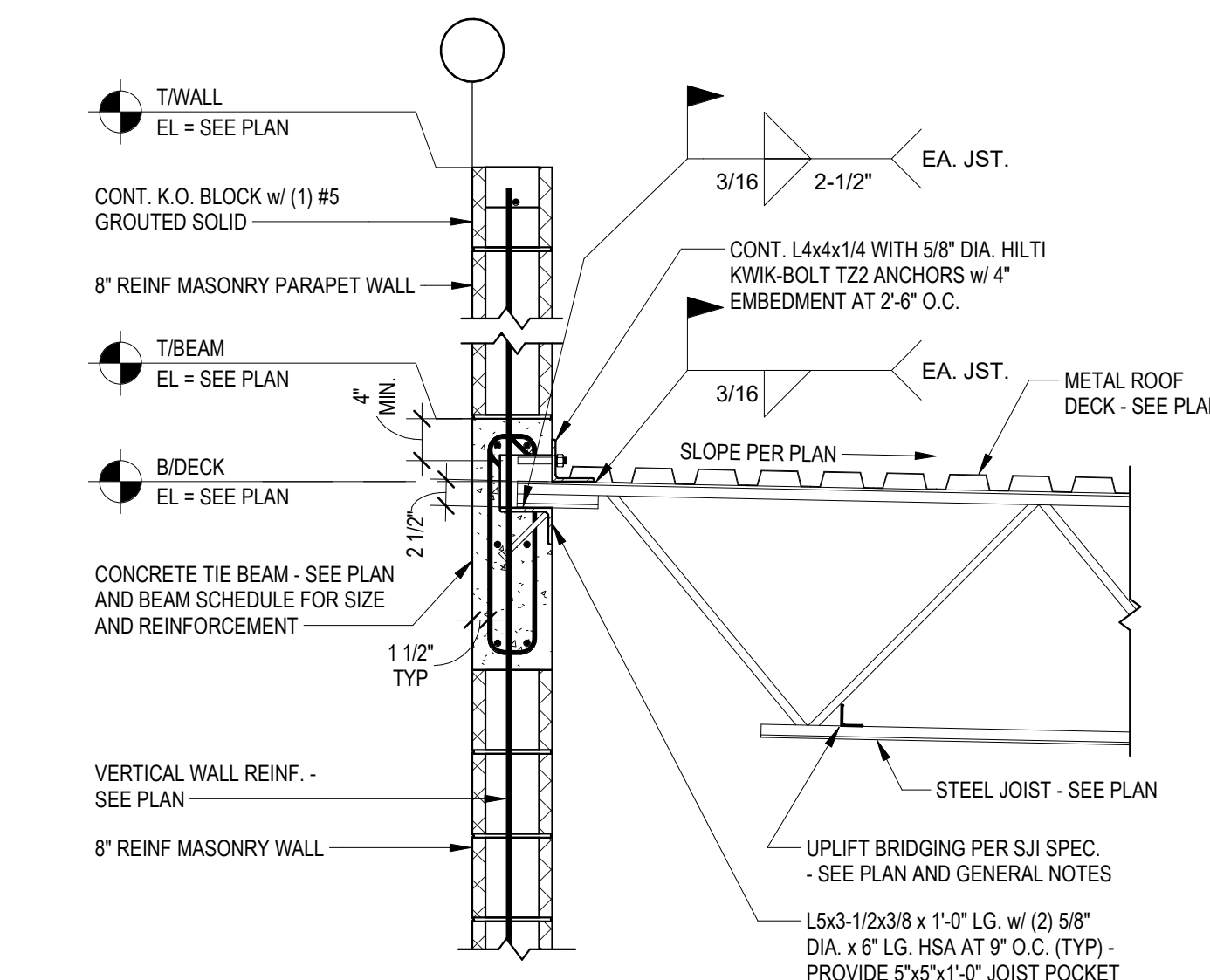
Drawing Number:

S612

Job Number:

A/E Job Number:

23432



Owner:

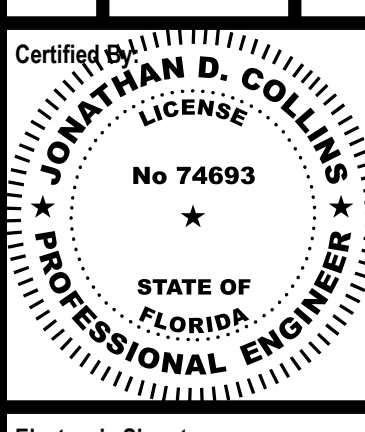


Revisions:

Scale:	AS NOTED
Date:	12/15/23
Drawn By:	SGZ
Checked By:	JDC

WEST MARKET LOT 10  
WEST MARKET LOT 10  
14230 WEST COLONIAL DRIVE  
WINTER GARDEN, FL

FRAMING SECTIONS AND DETAILS



Electronic Signature:  
This drawing has been electronically signed and sealed by JONATHAN D. COLLINS, P.E. LICENSE NO. 74693 ON 12/15/2023. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

This drawing has been electronically signed and sealed by JONATHAN D. COLLINS, P.E. LICENSE NO. 74693 ON 12/15/2023. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

Drawing Number:  
**S613**

Job Number:  
23432