VERSION 2022.²

GENERAL PROVISIONS:

TYPICAL DETAILS AND GENERAL NOTES APPLY TO ALL PARTS OF THE WORK EXCEPT WHERE SPECIFICALLY DETAILED OR UNLESS OTHERWISE NOTED.

DRAWINGS ARE NOT TO BE SCALED.

FOR DIMENSIONS NOT SHOWN, COORDINATE WITH ARCHITECTURAL DRAWINGS.

THE CONTRACTOR SHALL CAREFULLY REVIEW THE DRAWINGS TO IDENTIFY THE SCOPE OF WORK REQUIRED, VISIT THE SITE TO RELATE THE SCOPE OF WORK TO EXISTING CONDITIONS, AND DETERMINE THE EXTENT OF WHICH THOSE CONDITIONS AND PHYSICAL SURROUNDINGS WILL IMPACT THE WORK.

THE CONTRACTOR SHALL ASSUME THE MOST STRINGENT REQUIREMENTS APPLY IN CASE OF CONFLICT AMONG SPECIFICATIONS, STANDARDS, CODES AND DRAWINGS. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER IMMEDIATELY TO RESOLVE THE CONFLICT.

ANY DEVIATION, MODIFICATION, OR SUBSTITUTION FROM THE BID SET OF STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER FOR REVIEW/APPROVAL PRIOR TO ITS USE OR INCLUSION ON THE SHOP DRAWINGS. WITHOUT SUCH PRIOR APPROVAL, DEVIATIONS, MODIFICATIONS, OR SUBSTITUTIONS WILL BE REJECTED. COSTS FOR DEMOLITION AND REWORK OF SUCH ITEMS WILL BE BORNE BY THE CONTRACTOR.

THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED FOR IN-SERVICE LOADS ONLY. THE MEANS, METHODS, PROCEDURES, AND SEQUENCES OF CONSTRUCTION ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL PROVIDE AND MAINTAIN ALL NECESSARY TEMPORARY SYSTEMS (SHORING, BRACING, GUYS, FALSEWORK, FORMWORK, SHEETING ETC.) TO ENSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION. ALL WORK SHALL BE PERFORMED WITHOUT DAMAGE TO ADJACENT EXISTING WORK. SHORING SYSTEMS SHALL BE DESIGNED, SIGNED, AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE JURISDICTION WHERE THE PROJECT IS LOCATED.

THE CONTRACTOR IS RESPONSIBLE FOR SITE SAFETY. THE CONTRACTOR AND ALL SUBCONTRACTORS SHALL REVIEW THE STRUCTURAL CONTRACT DOCUMENTS AND SHALL NOTIFY THE STRUCTURAL ENGINEER OF ANY CONFLICTS BETWEEN THOSE DOCUMENTS AND ANY SAFETY REGULATIONS. SUCH REVIEW AND NOTIFICATION SHALL OCCUR PRIOR TO PRODUCTION OF SHOP DRAWINGS.

THE CONTRACTOR SHALL PROTECT ALL WORK, MATERIALS, AND EQUIPMENT FROM DAMAGE AND SHALL PROVIDE PROPER STORAGE FACILITIES FOR MATERIALS AND EQUIPMENT DURING CONSTRUCTION.

SITE VISITS PERFORMED BY THE ARCHITECT/ENGINEER DO NOT INCLUDE INSPECTIONS OF MEANS AND METHODS OF CONSTRUCTION PERFORMED BY THE CONTRACTOR.

STRUCTURAL OBSERVATIONS PERFORMED BY THE ARCHITECT/ENGINEER DURING CONSTRUCTION ARE NOT THE CONTINUOUS AND SPECIAL INSPECTION SERVICES AND DO NOT WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS REQUIRED OF THE BUILDING DEPARTMENT INSPECTOR OR THE TESTING AGENCY. ALSO, OBSERVATIONS DO NOT GUARANTEE THE CONTRACTOR'S PERFORMANCE AND SHALL NOT BE CONSIDERED AS SUPERVISION OF CONSTRUCTION.

THE ROOF DECK HAS BEEN DESIGNED ONLY FOR THE DESIGN LOADING CRITERIA AS INDICATED IN THE CONSTRUCTION DOCUMENTS. THE WEIGHT OF CONSTRUCTION MATERIALS AND EQUIPMENT ON THE STRUCTURE SHALL BE LIMITED TO THE DESIGN LOADING CRITERIA UNLESS APPROVED BY THE ENGINEER OF RECORD. ANY EQUIPMENT OR MATERIALS THAT EXCEED THE DESIGN LOADING WILL NOT BE PERMITTED WITHOUT AN ANALYSIS OF THE STRUCTURE BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF THE PROJECT. THE RESPONSIBILITY FOR THIS ANALYSIS IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

SHOP DRAWINGS:

REFER TO THE PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

REPRODUCTION OF THE STRUCTURAL DRAWINGS FOR USE IN PREPARATION OF SHOP DRAWINGS IS STRICTLY PROHIBITED WITHOUT THE WRITTEN CONSENT OF THE ENGINEER OF RECORD. SHOP DRAWINGS SUBMITTED WITH REPRODUCED STRUCTURAL DRAWINGS AND/OR DETAILS WITHOUT CONSENT WILL BE REJECTED.

SUBMIT SHOP DRAWINGS 15 BUSINESS DAYS (MINIMUM) PRIOR TO DATE THAT RETURNED SHOP DRAWINGS ARE REQUIRED.

SHOP DRAWINGS SHALL BEAR THE CONTRACTOR'S STAMP OF APPROVAL, WHICH SHALL CONSTITUTE CERTIFICATION THAT ALL FIELD MEASUREMENTS, CONSTRUCTION CRITERIA, AND MATERIALS SPECIFIED IN THE CONTRACT DOCUMENTS HAVE BEEN VERIFIED AND EACH DRAWING HAS BEEN CHECKED FOR COMPLETENESS, COORDINATION, AND COMPLIANCE WITH THE CONTRACT DOCUMENTS.

CONTRACTOR SHALL REFER TO ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL AND FOOD SERVICE DRAWINGS FOR SIZE AND LOCATIONS OF OPENINGS, SLEEVES, CONCRETE HOUSEKEEPING PADS, <u>GENERAL</u> INSERTS, AND DEPRESSIONS DURING SHOP DRAWING PREPARATION.

WHERE A DELEGATED DESIGN IS INDICATED ON THE DRAWINGS. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR EACH ITEM, COMPONENT, AND CONNECTION NOT SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS, SHOP DRAWINGS AND DESIGN CALCULATIONS SHALL BE SIGNED AND SEALED BY THE CONTRACTOR'S LICENSED ENGINEER (IN THE PROJECT'S JURISDICTION). DRAWINGS AND CALCULATIONS SHALL SHOW LOCATIONS AND MAGNITUDES OF LOADS IMPOSED ON THE STRUCTURE. THE ENGINEER OF RECORD RESERVES THE RIGHT TO MODIFY LOAD PATH SUGGESTED BY THE DELEGATED DESIGN ENGINEER.

DELEGATED DESIGN:

CONTRACTOR IS RESPONSIBLE FOR DESIGN OF THE FOLLOWING ITEMS INCLUDING DESIGN OF THE CONNECTIONS OF EACH ITEM TO THE SUPPORTING STRUCTURAL FRAMING:

OPEN-WEBBED STEEL JOISTS

STRUCTURAL STEEL CONNECTIONS COLD-FORMED STEEL FRAMING:

FACADE PANELS AND FACADE COMPONENTS

SHORING MONUMENT SIGN AND MONUMENT SIGN FOUNDATION

THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR EACH ITEM LISTED ABOVE. REFER TO THE "SHOP DRAWING" SECTION UNDER THE GENERAL NOTES FOR ADDITIONAL INFORMATION.

INFORMATION SHOWN IN THE CONTRACT DOCUMENTS (E.G., DEPTHS, GAUGES, SPACING, PLYS, ETC.) ARE CONSIDERED MINIMUMS AND ARE SCHEMATIC IN NATURE. INCREASED GAUGE/PLYS AND/OR DECREASED SPACINGS MAY BE REQUIRED AND SHALL BE COMPLETED AT NO CHARGE TO THE OWNER.

DESIGN LOAD	DINGS:			
GOVERNING BUILDING CO	DE: 2020 FLORIDA BUILD	ING CODE		
GRAVITY I OADS				
ROOF DEAD LOAD		20	PSF (SUPERIMPOSED +
			(ACTUAL MAT'L WEIGHTS)
ROOF LIVE LOAD		20	PSF	,
GROUND SNOW LOAD (Pg)	0	PSF	
	τ			
	NA. SCE 7 16):			
	SUE 7-10).	125	МОЦ	
		135		(ULTIWATE)
	GORY	II C		
DESIGN PRESSUE	RES (I II TIMATE)	0		
COMPONEN	NTS AND CLADDING (h =	16'-0" a = 6'-0")		
ROOF	= ZONE 1'			
	TRIB. AREA 0-10 SF	+16.6/-36.9	PSF	5. <u>0.6h</u>
	TRIB. AREA 20 SF	+16.0/-36.9	PSF	
	TRIB. AREA 50 SF	+16.0/-36.9	PSF	0.2h
	TRIB. AREA 100 SF	+16.0/-36.9	PSF	0
ROOF	ZONE 1			00 0 00
	TRIB. AREA 0-10 SF	+16.4/-64.2	PSF	0.6h
	TRIB. AREA 20 SF	+16.0/-59.9	PSF	
	TRIB. AREA 50 SF	+16.0/-54.3	PSF	
	TRIB. AREA 100 SF	+16.0/-50.1	PSF	
ROOF	ZONE 2			ROUF ZONES
	TRIB. AREA 0-10 SF	+36.9/-84.6	PSF	
	TRIB. AREA 20 SF	+35.2/-79.2	PSF	
	TRIB. AREA 50 SF	+33.1/-72.0	PSF	
	TRIB. AREA 100 SF	+31.4/-66.6	PSF	
ROOF	ZONE 3			
	TRIB. AREA 0-10 SF	+36.9/-84.6	PSF	
	TRIB. AREA 20 SF	+35.2/-79.2	PSF	
	TRIB. AREA 50 SF	+33.1/-72.0	PSF	
	TRIB. AREA 100 SF	+31.4/-66.6	PSF	

TRIB. AREA 20 SF +35.2/-38.3 PSF +33.1/-36.1 PSF TRIB. AREA 50 SF TRIB. AREA 100 SF +31.4/-34.5 PSF WALL ZONE 5 +36.9/-49.1 PSF TRIB. AREA 0-10 SF TRIB. AREA 20 SF +35.2/-45.9 PSF WALL ZONES +33.1/-41.6 PSF TRIB. AREA 50 SF TRIB. AREA 100 SF +31.4/-38.3 PSF +/- INDICATE PRESSURES ACTING TOWARD AND AWAY FROM THE SURFACE RESPECTIVELY SEISMIC DESIGN DATA (ASCE 7-16): CEICMIC IMPORTANCE EACTOR (I

+36.9/-39.9 PSF

SEISMIC IMPORTANCE FACTOR (I)	1.0
RISK CATEGORY	II
MAPPED SPECTRAL RESPONSE	
SHORT PERIODS (Ss)	0.062
1 SEC. PERIODS (S1)	0.033
SPECTRAL RESPONSE COEFF.	
SHORT PERIODS (SDS)	0.066
1 SEC. PERIODS (SD1)	0.053
SITE CLASS	D
SEISMIC DESIGN CATEGORY	А

WALL ZONE 4

TRIB. AREA 0-10 SF

EARTHWORK/SUBSURFACE INVESTIGATION:

THE CONTRACTOR SHALL REFER TO THE THE GEOTECHNICAL SUBSURFACE INVESTIGATION REPORT AND SPECIFICATIONS FOR ALL REQUIREMENTS RELATED TO EXCAVATION, PREPARATION OF THE SUBGRADE, COMPACTION PROCEDURES, AND FOR ANY OTHER GEOTECHNICAL REQUIREMENTS. WHERE CONFLICTING REQUIREMENTS BETWEEN THE DRAWINGS AND GEOTECHNICAL SUBSURFACE INVESTIGATION REPORT ARE PRESENT, THE MOST STRINGENT REQUIREMENT SHALL BE BID UNLESS OTHERWISE ADDRESSED BY THE ENGINEER OF RECORD IN A FORMAL REQUEST FOR INFORMATION.

THE RECOMMENDATIONS PRESENTED HEREIN ARE IN ACCORDANCE WITH THE SUBSURFACE INVESTIGATION REPORT PREPARED BY:

TERRACON, PROJECT NUMBER H1235150, DATED JULY 17, 2023.

PROOFROLLING:

PRIOR TO EXCAVATION FOR STRUCTURES, PROOFROLL BUILDING AND PAVEMENT AREAS USING A HEAVILY LOADED DUMP TRUCK OR SIMILARLY HEAVILY LOADED VEHICLE. ALL SOFT, LOOSE OR UNSTABLE AREAS ARE TO BE STABILIZED WITH ADDITIONAL COMPACTION OR UNDERCUT AND REPLACED WITH ENGINEERED

ENGINEERED FILI

ENGINEERED FILL SHALL BE WELL-GRADED MATERIAL FREE FROM DEBRIS, ORGANIC MATERIAL, FROZEN MATERIALS, BRICK, LIME, CONCRETE AND OTHER MATERIALS THAT WOULD PREVENT ADEQUATE PERFORMANCE. FILL SHALL CONFORM TO ASTM D2487 SOIL CLASSIFICATION GROUPS GW, GP, GM, SW, SP OR SM

UNLESS OTHERWISE NOTED, THE PROPOSED ENGINEERED FILL MATERIALS ARE TO BE PLACED IN LIFTS NOT EXCEEDING EIGHT (8) INCHES IN LOOSE MEASURED THICKNESS. EACH LIFT IS TO BE COMPACTED A MINIMUM OF 95% MAXIMUM DENSITY BY ASTM D1557.

THE EARTHWORK PROGRAM SHALL BE CONDUCTED UNDER THE SUPERVISION OF A SOILS LABORATORY.

THE IN-PLACE DENSITIES ACHIEVED ARE TO BE VERIFIED BY TEST.

BACKFILL:

BACKFILL OPERATIONS SHALL BE PERFORMED IN ACCORDANCE WITH THE SPECIFICATIONS AND GEOTECHNICAL SUBSURFACE INVESTIGATION REPORT.

BACKFILL MATERIAL SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO INSTALLATION. WHERE FINAL GRADES ARE APPROXIMATELY EQUAL ON BOTH SIDES OF A WALL, BACKFILL EQUALLY ON BOTH SIDES OF THE WALL IN LIFTS TO MAINTAIN LEVEL ELEVATIONS TO WITHIN 1'-0" AT ANY GIVEN TIME.

FOUNDATION SYSTEMS:

GENERAL:

THE CONTRACTOR SHALL STUDY THE GEOTECHNICAL INVESTIGATION REPORT (REFER TO THE "EARTHWORK/SUBSURFACE INVESTIGATION" SECTION UNDER THE GENERAL NOTES") AND VISIT THE SITE PRIOR TO THE START OF ANY WORK. THE CONTRACTOR SHALL VERIFY ANY EXISTING FIELD CONDITION THAT MAY AFFECT THE INSTALLATION OF THE FOUNDATION SYSTEM.

THE CONTRACTOR SHALL EXERCISE GREAT CARE DURING EXCAVATION. UNDERGROUND UTILITY LOCATIONS, IF SHOWN, ARE APPROXIMATE. THE CONTRACTOR SHALL PREDETERMINE UTILITY LOCATIONS AND NOTIFY THE ENGINEER IMMEDIATELY IF DEVIATION FROM PLANS EXIST. THE CONTRACTOR IS RESPONSIBLE FOR THE SAFE SUPPORT OF UTILITIES ACROSS EXCAVATIONS.

SHEETING, SHORING, AND DEWATERING IS THE RESPONSIBILITY OF THE CONTRACTOR.

A SOILS TESTING LABORATORY SHALL BE RETAINED BY THE OWNER TO PROVIDE CONSTRUCTION REVIEW TO ENSURE CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS DURING THE EXCAVATIONS, BACKFILL, AND FOUNDATION PHASES OF THE PROJECT.

BOTTOM OF ALL FOOTINGS SHALL BEAR A MINIMUM OF 1'-6" BELOW ADJACENT FINAL GRADE.

SPREAD/TRENCH FOOTINGS:

BEARING ELEVATIONS ARE ESTIMATED FROM SOIL BORING DATA INDICATED IN THE GEOTECHNICAL SUBSURFACE INVESTIGATION REPORT. DETERMINATION OF FINAL BEARING ELEVATIONS, TOPSOIL AND EXCAVATION STRIPPING DEPTH, INSPECTION OF ALL SUBSOIL EXPOSED DURING STRIPPING, SITE GRADING, EXCAVATION OPERATIONS, APPROVAL OF FILL MATERIALS, DENSITY TESTING OF FILLS TO ENSURE PLACEMENT PER SPECIFICATION REQUIREMENTS, INSPECT FOUNDATION BEARING SURFACES, AND VERIFY ALLOWABLE BEARING PRESSURES ARE THE TESTING LABORATORY'S RESPONSIBILITY.

ALL FOUNDATIONS ARE TO REST ON FIRM UNDISTURBED SOIL OR COMPACTED FILL FREE FROM ORGANIC MATTER. IF POOR SOIL CONDITIONS ARE ENCOUNTERED AT FOUNDATION DEPTHS SHOWN, NOTIFY OWNER'S REPRESENTATIVE BEFORE PROCEEDING WITH CONSTRUCTION.

FOUNDATIONS HAVE BEEN DESIGNED BASED ON AN ALLOWABLE SOIL BEARING CAPACITY OF 2500 PSF INUNDATION AND LONG TERM EXPOSURE OF BEARING SURFACES, WHICH WILL RESULT IN DETERIORATION OF BEARING FORMATIONS, SHALL BE PREVENTED. FOOTINGS SHALL BE PLACED IMMEDIATELY FOLLOWING FOOTING EXCAVATIONS AND BEARING SURFACE INSPECTION.

UTILITY LINES SHALL NOT BE PLACED THROUGH OR BELOW FOUNDATIONS WITHOUT THE APPROVAL OF THE ENGINEER OF RECORD.

CONCRETE

GENERAL:

ALL CONCRETE CONSTRUCTION SHALL CONFORM TO ACI 301-10, "STANDARD SPECIFICATION FOR STRUCTURAL CONCRETE" AND ACI 302, 305 AND 306 UNLESS NOTED OTHERWISE.

ALL DETAILING, FABRICATION AND PLACING OF CONCRETE SHALL CONFORM TO ACI 318-14, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" AND THE ACI "MANUAL OF STANDARD PRACTICE FOR DETAIL REINFORCED CONCRETE STRUCTURES" UNLESS NOTED OTHERWISE.

SAFETY AND PERFORMANCE OF THE STRUCTURE ARE THE RESPONSIBILITY OF THE CONTRACTOR INSOFAR AS THEY ARE AFFECTED BY THE LOCATION AND DETAILS OF CONSTRUCTION JOINTS. SHOP DRAWINGS OF THE PROPOSED CONSTRUCTION JOINT LOCATIONS AND DETAILS ARE TO BE SUBMITTED TO THE ARCHITECT FOR APPROVAL.

ALL CONCRETE SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI IN 28 DAYS. REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60.

WELDED WIRE FABRIC REINFORCING SHALL CONFORM TO ASTM A1064 AND BE FURNISHED IN FLAT SHEETS AND INSTALLED ON CHAIRS OR PRECAST CONCRETE BLOCKS.

NO TACK WELDING OF REINFORCING IN THE FIELD IS PERMITTED.

PROVIDE CORNER BARS AT ALL LOCATIONS WHERE REINFORCEMENT CHANGES DIRECTION

PROVIDE STRAIGHT AND DIAGONAL BARS AT EDGES OF ALL OPENINGS.

REINFORCING EMBEDMENT AND LAP SPLICES (INCHES) FOR 4000 PSI CONCRETE

	OTHER		TOP*	
BAR SIZE	ANCHORAGE	SPLICE	ANCHORAGE	SPLICE
#3	15	19	19	24
#4	19	25	25	33
#5	24	31	31	41
#6	29	37	37	49
#7	42	54	54	71
#8	48	62	62	81
#9	54	70	70	91
#10	60	78	78	101
#11	66	85	85	111

* HORIZONTAL BARS WITH MORE THAN 12" OF CONCRETE BELOW BAR

CLEAR MINIMUM COVER OF CONCRETE OVER REINFORCING BARS SHALL BE AS FOLLOWS:

CONCRETE ΡΙ ΔCED ΔGΔΙΝΙST ΕΔΡΤΗ	3"
CONCRETE EXPOSED TO EARTH OR WEATHER	0
#6 TO #18 BARS	2"
#5 BAR OR SMALLER	1 1/2"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER	
SLABS & WALLS #11 BAR AND SMALLER	3/4"
CONCRETE BEAMS, COLUMNS, & PIERS	1 1/2"

STEEL JOISTS:

STEEL JOIST DESIGN, FABRICATION AND ERECTION SHALL CONFORM TO THE SPECIFICATIONS AND CODE OF STANDARD PRACTICE OF THE STEEL JOIST INSTITUTE OR AS SHOWN ON THE DRAWINGS. ALL 'K' JOISTS TO CONFORM TO SJI K-10 EDITION OF AISC AND SJI SPECIFICATIONS.

PROVIDE HORIZONTAL AND/OR DIAGONAL BRIDGING PER SJI REQUIREMENTS.

ALL JOISTS SHALL BE DESIGNED TO RESIST DESIGN PRESSURES AS SHOWN UNDER "DESIGN LOADINGS".

ENDS OF EVERY JOIST WHICH RESTS ON STEEL SUPPORTS SHALL BE WELDED PER SJI REQUIREMENTS.

NO LIGHT GAUGE FRAMING, MECHANICAL, ELECTRICAL, OR OTHER EQUIPMENT SHALL BE SUSPENDED FROM OR ATTACHED TO ANY INTERIOR BRIDGING.

AT ALL CONCENTRATED LOADS NOT LOCATED AT JOIST PANEL POINTS, HANGING FROM TOP OR BOTTOM JOIST CHORDS, FIELD WELD ADDITIONAL WEB ANGLE 2x2x1/4 FROM LOAD LOCATION TO ADJACENT PANEL POINT

PROVIDE SINGLE BOTTOM CHORD CEILING EXTENSIONS WHERE ACOUSTICAL CEILING IS INDICATED.

GENERAL CONTRACTOR SHALL VERIFY ALL STRUCTURAL STEEL JOIST LOCATIONS, MECHANICAL UNIT WEIGHTS AND OPENING SIZES AND LOCATIONS WITH MECHANICAL CONTRACTOR AND VENDOR'S DRAWINGS FOR ACTUAL MECHANICAL UNITS PURCHASED.

MASONRY

ALL BRICK MASONRY SHALL COMPLY WITH THE RECOMMENDATIONS OF BRICK INSTITUTE OF AMERIC AND LOCAL BUILDING CODE REQUIREMENTS.)

ALL CONCRETE MASONRY SHALL CONFORM TO "BUILDING CODE REQUIREMENTS AND SPECIFICATIO MASONRY STRUCTURES" (TMS 402-16/602-16) AND LOCAL BUILDING CODE REQUIREMENTS.

CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, TYPE I OR II.

THE MINIMUM PRISM COMPRESSIVE STRENGTH (fm) SHALL BE 1500 PSI.

ASTM C270, TYPE "S" MORTAR WITH A MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI SHALL BE USE ALL MASONRY WALLS.

GROUT TO FILL CORES SHALL BE ASTM C476, COARSE GROUT (3/8" MAXIMUM AGGREGATE) WITH A COMPRESSIVE STRENGTH OF 2500 PSI IN 28 DAYS.

LAY MASONRY UNITS WITH FULL MORTAR COVERAGE ON HORIZONTAL AND VERTICAL FACE SHELLS WEBS IN MORTAR IN STARTING COURSE OF FOOTINGS AND IN ALL COURSES OF COLUMNS AND PIL

AND WHERE ADJACENT TO CELLS OR CAVITIES TO BE REINFORCED OR FILLED WITH CONCRETE OR MASONRY SHALL BE LAID IN RUNNING BOND, UNLESS NOTED OTHERWISE.

VERTICAL REINFORCING LAP SPLICES SHALL BE 48 BAR DIAMETERS.

REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60.

MAXIMUM GROUT POUR SHALL BE 5 FEET. CONSOLIDATE BY MECHANICAL VIBRATION.

MORTAR PROTRUSIONS, EXTENDING INTO CELLS OR CAVITIES TO BE REINFORCED AND FILLED, SHA

REMOVED. GROUT A MINIMUM OF 16 INCHES x 24 INCHES WIDE CENTERED UNDER ALL BEAM BEARINGS AND 8

16 INCHES WIDE CENTERED UNDER ALL LINTEL BEARINGS.

GROUT A MINIMUM OF 8 INCHES x 24 INCHES WIDE CENTERED UNDER ALL JOIST BEARINGS.

GROUT CORES SOLID A MINIMUM OF ONE COURSE BELOW ANY CHANGE IN WALL THICKNESS. THE COLLAR-JOINT IN MULTI-WYTHE WALLS BELOW GRADE SHALL BE FULLY GROUTED AS THE WALL

FILL ALL BEARING POCKETS WITH SOLID MASONRY AFTER INSTALLING BEAMS.

ALL CORNERS ARE TO BE TIED BY MASONRY BOND.

CONSTRUCTED.

CMU WALLS SHALL HAVE VERTICAL CONTROL JOINTS LOCATED PER FRAMING PLAN (APPROXIMATEL O.C.), REFER TO TYPICAL CONTROL JOINT DETAILS ON STRUCTURAL DRAWINGS FOR CONTROL JOIN DETAILS AND RESTRICTIONS. LOCATIONS OF CMU CONTROL JOINTS DO NOT HAVE TO ALIGN WITH CONTROL JOINTS. REFER TO ARCHITECTURAL DRAWINGS FOR VENEER CONTROL JOINT LOCATIONS

PROVIDE MATERIAL/MEANS TO DEBOND MORTAR FROM DISSIMILAR MATERIALS IN ALL VENEERS (I.E STONE AND CLAY BRICK, CONCRETE BLOCK AND CLAY BRICK, ETC.)

EMBEDDED ELECTRICAL CONDUIT SHALL NOT BE LOCATED IN THE SAME CELL WHERE VERTICAL REINFORCEMENT IS LOCATED WITHOUT PERMISSION OF THE STRUCTURAL ENGINEER OF RECORD. CONFLICT AREAS TO ENGINEER FOR REVIEW PRIOR TO INSTALLING CONDUIT.

STRUCTURAL STEE

TUBE:ASTM A500 GRADE C (Fy = 50 KSI)ANCHOR RODS:ASTM F1554 GRADE 36 (GALVANIZED)	W SHAPES: M,S,C SHAPES: PLATE, ANGLES: TUBE: ANCHOR RODS:	ASTM A992 (Fy = 50 KSI) ASTM A36 UNO ASTM A36 UNO ASTM A500 GRADE C (Fy = 50 KSI) ASTM F1554 GRADE 36 (GALVANIZED)		
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DETAILING. FABRICATION. AND ERECTION SHALL CONFORM TO AISC 360-16 SPECIFICATIONS.

FIELD CONNECTIONS SHALL BE BOLTED. BEARING TYPE UNLESS NOTED OTHERWISE USING 3/4" HIGH STRENGTH BOLTS CONFORMING TO ASTM A325. ONE SIDED CONNECTIONS ARE NOT PERMITTED U DETAILED ON DRAWINGS

GENERAL CONTRACTOR SHALL VERIFY ALL MECHANICAL EQUIPMENT LOCATIONS, OPENING DIMENS AND WEIGHTS PRIOR TO STRUCTURAL STEEL FABRICATION. NOTIFY ENGINEER IF DIFFERENT FROM SHOWN ON DRAWINGS

SLIP CRITICAL BOLTS (FRICTION BOLTS) SHALL BE USED WERE INDICATED IN SECTIONS AND DETAIL: ALL CONNECTIONS TO TUBES SHALL USE THRU PLATES UNLESS NOTED OTHERWISE.

WHEN FORCES ARE NOT SHOWN. THE CONNECTION SHALL DEVELOP 1/2 OF THE ALLOWABLE UNIFOL AS SPECIFIED IN THE BEAM TABLES OF AISC (ASD). A MINIMUM OF 2 BOLTS SHALL BE USED.

ALL WELDING SHALL BE DONE USING E-70XX ELECTRODES IN ACCORDANCE WITH THE AWS SPECIFIC PRIME ALL STEEL NOT IN CONTACT WITH CONCRETE. DO NOT PRIME STEEL IN AREAS TO RECEIVE

ALL STRUCTURAL STEEL BEAMS AND COLUMNS ADJACENT TO MASONRY SHALL HAVE ADJUSTABLE MASONRY ANCHORS AT 2'-8" ON CENTER.

GALVANIZE ALL STEEL THAT IS EXPOSED TO WEATHER. GALVANIZED STEEL SHALL BE SHOP FABRIC AND CUT TO LENGTH PRIOR TO GALVANIZING. DO NOT FIELD CUT. DAMAGED GALVANIZING IS TO BE REPAIRED WITH A HIGH ZINC CONTENT PAINT MEETING MILITARY SPECIFICATION MIL-P-21035. GALVA STEEL PER ASTM A123.

THE DESIGN OF ALL STEEL CONNECTIONS, INCLUDING MOMENT CONNECTIONS, SHALL BE PERFORM UNDER THE DIRECT SUPERVISION OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHEF PROJECT IS LOCATED, AND EMPLOYED BY THE STEEL FABRICATOR. THE REGISTERED PROFESSION. ENGINEER SHALL SUBMIT COMPLETE DESIGN CALCULATIONS FOR EACH CONNECTION. SUCH CALCULATIONS SHALL SHOW DETAILS OF THE ASSEMBLED JOINT WITH ALL BOLTS AND WELDS REQUIRED. WHERE PREDESIGNED CONNECTIONS ARE TAKEN DIRECTLY FROM TABLES IN THE AISC

SPECIFICATION, CALCULATIONS NEED NOT BE SUBMITTED PROVIDED THE JOB DESIGN CONDITIONS PRECISELY MATCH THOSE ASSUMED IN THE AISC TABLES.

METAL ROOF DECK:

CRITICAL BOLTS (FRICTION BOLTS).

CONNECTIONS TO STRUCTURAL STEEL SUPPORTS SHALL BE FUSION TYPE WELDS PERFORMED BY COMPETENT WELDERS WHO HAVE QUALIFIED BY TESTS AS PRESCRIBED BY THE AMERICAN WELDIN SOCIETY TO PERFORM THE TYPE OF WORK REQUIRED. THE FIRST AND LAST RIBS OF EACH SHEET M WELDED TO ALL SUPPORTS. END WELDS AND THOSE OCCURRING AT LAPS SHALL BE WELDED THRO ALL THICKNESSES. SIDE JOINTS SHALL BE MECHANICALLY FASTENED AT MID-SPAN (UNO)

NO LIGHT GAUGE FRAMING, MECHANICAL, ELECTRICAL OR OTHER EQUIPMENT SHALL BE SUSPENDE OR ATTACHED TO ANY METAL ROOF DECK.

OPENINGS SMALLER THAN 12" SQUARE ARE TO BE CUT BY INDIVIDUAL TRADE AND FLASHED BY ROOFING/SIDING CONTRACTOR. ALL LARGER OPENINGS TO BE CUT AND FLASHED BY ROOFING/SIDI CONTRACTOR WITH OPEN EDGES SUPPORTED BY STRUCTURAL STEEL.

PLACE DECK UNITS ON SUPPORTING STEEL FRAMEWORK IN LENGTHS TO SPAN 4 OR MORE SUPPOR SPANS) UNO.

METAL ROOF DECK SHALL BE GALVANIZED HIGH STRENGTH STEEL. IF DECK IS TO BE FIELD PAINTED PROVIDE SHOP PRIMED FINISH OVER TOP OF GALVANIZATION.





COLD-FORMED STEEL FRAMING	
DESIGN OF METAL STUD FRAMING IS BASED ON CSJ TYPE (1 5/8" FLANGE) STUDS WITH CLARK STEEL FRAMING SYSTEMS SECTION PROPERTIES AND ALLOWABLE RESISTING MOMENT CAPACITY. ALTERNATE	
MANUFACTURER'S FRAMING SIZE SHALL MEET THE MINIMUM SECTION PROPERTIES AND ALLOWABLE RESISTING MOMENT CAPACITY OF THE MEMBERS INDICATED ON THE DESIGN DRAWINGS. ALL LIGHT GAUGE FRAMING SHALL BE DESIGNED IN ACCORDANCE WITH AMERICAN IRON AND STEEL	GPD GROUP, INC. LIC. # - 30920
INSTITUTE (AISI) "DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS." ALL FRAMING MEMBERS SHALL BE FORMED FROM STEEL WITH A MINIMUM YIELD STRENGTH OF 33 KSI.	520 South Main Street, Suite 2531 Akron, OH 44311 330.572.2100 Fax 330.572.2101
ALL FRAMING SHALL BE GALVANIZED.	
ALL CONNECTIONS SHALL BE SCREWED OR WELDED. POWDER DRIVEN FASTENERS ARE NOT ACCEPTABLE FOR ANY STRUCTURAL APPLICATIONS.	
ALL WELDS SHALL BE TOUCHED UP WITH A ZINC-RICH PAINT.	
REVIEW FOR ALL COLD FORMED METAL FRAMING COMPONENTS AND CONNECTIONS. FOR ALL FRAMING COMPONENTS AND CONNECTIONS NOT SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS INCLUDING TRUSSES, HEADERS, JAMBS, ETC. SUBMIT SHOP DRAWINGS AND CALCULATIONS STAMPED BY AN ENGINEER REGISTERED IN THE STATE THE PROJECT IS LOCATED.	
HEADED STUDS AND DEFORMED BAR ANCHOR:	
HEADED STUDS AND DEFORMED BAR ANCHORS SHALL BE ELECTRIC-ARC STUD WELDED PER MANUFACTURERS RECOMMENDATIONS AND THE AWS CODE. FILLET WELDING SHALL NOT BE ALLOWED WITHOUT APPROVAL OF THE STRUCTURAL ENGINEER. WELDMENT SHALL BE IN SUCH A MANNER AS TO	
PROVIDE COMPLETE FUSION BETWEEN THE END OF THE STUD AND THE PLATE. THERE SHOULD BE NO POROSITY OF EVIDENCE OF LACK OF FUSION BETWEEN THE WELDED END OF THE STUD AND THE PLATE. THE STUD WILL DECREASE IN LENGTH DURING WELDING APPROXIMATELY 1/8" FOR 5/8" DIAMETER AND SMALLER, 3/16" FOR 5/8" DIAMETER AND LARGER.	
HEADED STUDS SHALL BE TYPE B PER THE AWS CODE WITH A MINIMUM YIELD STRENGTH OF 51 KSI. NELSON GRANULAR FLUX-FILLED (OR APPROVED EQUAL). STUDS SHALL BE MANUFACTURED OF COLD DRAWN BAR STOCK CONFORMING TO ASTM A-108.	
DEFORMED BAR ANCHORS SHALL COMPLY WITH ASTM A-706 WITH A MINIMUM YIELD STRENGTH OF 70 KSI. UNLESS NOTED OTHERWISE, DEFORMED BAR ANCHOR LENGTH SHALL BE AS FOLLOWS:	
BAR DIAMETER EMBEDMENT LENGTH	FOR BIDDING
5/6 24 1/2" 24" 5/8" 30"	
ADHESIVE DOWELLED ANCHORS:	CONSTRUCTION.
REINFORCING, BAR DOWELS, REINFORCING BARS, THREADED RODS, BOLTS ETC. WHICH ARE INDICATED TO BE ADHESIVE DOWELLED INTO CONCRETE OR SOLID MASONRY SHALL BE ACCOMPLISHED USING HIT HY-200 V3 SAFESET ADHESIVE BY HILT FASTENING SYSTEMS OF THE SALOK (ICC REPORT NO. ESR 3013), OP FOUND	ġ.
DRILL, BRUSH, AND CLEAN ALL HOLES, AND INSTALL ALL ANCHORS IN COMPLETE ACCORDANCE WITH MANUFACTURERS PUBLISHED RECOMMENDATIONS, AS WELL AS ALL APPLICABLE BUILDING CODES OR ENGINEERING REPORTS.	MENT Intre Driv 101
PROVIDE THE FOLLOWING MINIMUM ANCHOR EMBEDMENT DEPTHS UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DETAILS:	Dental Sental IL 624
A. REINFORCING BARS BAR SIZE EMBEDMENT DEPTH	and E Vetwc
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	Learti: 200 h ffingt
#5 7" #6 9" #7 10"	
#8 12" #9 13" #10 16"	
#10 10 #11 18"	
B. BOLTS OR THREADED RODS DIAMETER EMBEDMENT DEPTH 3/8" 5"	
1/2" 7" 5/8" 8" 3/4" 10"	
5/4 10 7/8" 12" 1" 13"	nta 118
C. HILTI HIS INSERTS DIAMETER EMBEDMENT DEPTH	C C C C C C
3/8" 4 1/4" 1/2" 5" 5/8" 6 5/8"	, <mark>Радиника и Па</mark> 27527 27527
3/4" 8 1/4" WHEN INSTALLING DRILLED-IN-ANCHORS, LISE CARE AND CALITION TO AVOID CUTTING OR DAMAGING THE	DD CHAI
EXISTING REINFORCING BARS.	
WOOD SHEATHING:	
ALL EXTERIOR WALL SHEATHING TO BE 40/20 APA RATED 5/8" PLYWOOD SHEATHING EXPOSURE 1. ALL SHEATHING TO BE FASTENED TO SUPPORTING CFS STUDS WITH #10 SCREWS AT 6" ON CENTER AT PANEL EDGES AND 12" ON CENTER AT INTERMEDIATE SUPPORTS UNLESS NOTED OTHERWISE.	
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	11/15/2023 PERMIT SET mk date issue
	GENERAL NOTES
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SPECIAL INSPECTION AND TESTING:

THIS PROJECT REQUIRES SPECIAL INSPECTION AND TESTING IN ACCORDANCE WITH CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE. THESE NOTES AND THE STATEMENT OF SPECIAL INSPECTIONS PREPARED FOR THE PROJECT OWNER ARE INTENDED TO INFORM THE CONTRACTOR OF THE QUALITY ASSURANCE PROGRAM AND THE EXTENT OF THE CONTRACTOR'S RESPONSIBILITIES.

THE SPECIAL INSPECTIONS AND TESTING PROGRAM:

THE SPECIAL INSPECTION AND TESTING PROGRAM IS A QUALITY ASSURANCE PROGRAM INTENDED TO ENSURE THAT THE WORK IS PERFORMED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THESE INSPECTIONS ARE IN ADDITION TO THE INSPECTIONS SPECIFIED IN IBC SECTION 110. THE SPECIAL INSPECTION PROGRAM DOES NOT RELIEVE THE CONTRACTOR OF HIS OR HER RESPONSIBILITY TO COMPLY WITH THE OFFICIAL CONTRACT DOCUMENTS. FURTHER, IT IS NOT INTENDED THAT THE CONTRACTOR'S CONTRACTUAL AND STATUTORY OBLIGATIONS ARE ANYWAY RELIEVED OR FOREGONE BY THE PRESENCE OF THE SPECIAL INSPECTOR. THE CONTRACTOR HAS THE SOLE RESPONSIBILITY FOR ANY DEVIATIONS FROM THE OFFICIAL CONTRACT DOCUMENTS. THE SPECIAL INSPECTOR DOES NOT REPLACE THE DUTIES OF THE BUILDING OFFICIAL NOR THE QUALITY CONTROL RESPONSIBILITIES AND PERSONNEL OF THE CONTRACTOR. JOB

THE PROJECT OWNER IS RESPONSIBLE FOR EMPLOYING SPECIAL INSPECTION SERVICES. THE SPECIAL INSPECTOR/AGENCY SHALL NOT BE IN THE EMPLOY OF THE CONTRACTOR, SPECIFIED BY THE BUILDING OFFICIAL.

THE SPECIAL INSPECTOR IS OBLIGATED TO BOTH THE OWNER AND THE BUILDING OFFICIAL FOR OBSERVING THAT THE WORK IS EXECUTED IN SUBSTANTIVE ACCORDANCE WITH THE OFFICIAL CONTRACT DOCUMENTS. THE OFFICIAL CONTRACT DOCUMENTS ARE DEFINED AS THE PERMITTED PLANS AND SPECIFICATIONS, ADDENDA, CHANGE ORDERS, ISSUED SKETCHES AND REVISION DRAWINGS, AND ALL DIRECTIVES ISSUED BY ARCHITECT/ENGINEER.

THE INSPECTION AND TESTING AGENTS SHALL DISCLOSE ANY PAST OR PRESENT BUSINESS RELATIONSHIP OR POTENTIAL CONFLICT OF INTEREST WITH THE CONTRACTOR OR ANY OF THE SUBCONTRACTORS WHOSE WORK IS TO BE INSPECTED OR TESTED. THE SPECIAL INSPECTORS MAY HAVE NO FINANCIAL INTEREST IN PROJECTS FOR WHICH THEY PROVIDE SPECIAL INSPECTION SERVICES.

SPECIAL INSPECTION REPORT REQUIREMENTS:

SPECIAL INSPECTION REPORTS AND A FINAL REPORT IN ACCORDANCE WITH SECTION 1704.2.4 SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF THE WORK IS APPROVED FOR OCCUPANCY.

REPORT REQUIREMENT: SPECIAL INSPECTORS SHALL KEEP RECORDS OF ALL INSPECTIONS AND TESTS. THE SPECIAL INSPECTOR SHALL FURNISH THE INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INDICATE THAT THE WORK INSPECTED WAS OR WAS NOT COMPLETED IN CONFORMANCE TO THE APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THEY ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO THE COMPLETION OF THAT PHASE OF THE WORK. A FINAL REPORT DOCUMENTING THE REQUIRED SPECIAL INSPECTIONS, TESTS, AND CORRECTION OF ANY OF THE DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF THE WORK IS APPROVED FOR OCCUPANCY.

CONTRACTOR RESPONSIBILITIES:

THE CONTRACTOR IS RESPONSIBLE FOR NOTIFYING THE SPECIAL INSPECTOR IN ADVANCE OF CONSTRUCTION SCHEDULES AND PLANNED OPERATIONS IN ORDER TO ASSURE TIMELY AND APPROPRIATE INSPECTION FOR THE ITEMS LISTED IN THE SCHEDULE OF SPECIAL INSPECTIONS. THE CONTRACTOR SHALL PROVIDE ADEQUATE NOTICE TO THE SPECIAL INSPECTOR FOR ALL INSPECTIONS.

THE CONTRACTOR SHALL COOPERATE WITH AND ASSIST THE SPECIAL INSPECTOR IN PERFORMING HIS INSPECTION DUTIES. THE SPECIAL INSPECTOR SHALL HAVE FREE ACCESS TO THE PROJECT AT ALL TIMES. THE CONTRACTOR SHALL REVIEW THE SPECIAL INSPECTION PLAN AND COORDINATE THE SCHEDULE OF WORK TO ACCOMMODATE THE REQUIRED INSPECTIONS.

PROVIDE ACCESS TO APPROVED PLANS: THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING THE SPECIAL INSPECTOR ACCESS TO APPROVED PLANS. THE CONTRACTOR SHALL MAINTAIN A CURRENT SET OF THE CONTRACT DOCUMENTS AT THE JOB SITE.

CORRECT DISCREPANCIES AND DEVIATIONS: THE CONTRACTOR SHALL, UPON BEING INFORMED BY THE SPECIAL INSPECTOR, IMMEDIATELY CAUSE TO ELIMINATE SUCH DISCREPANCIES AND DEVIATIONS.

WORK COMPLETED WITHOUT INSPECTION: WORK REQUIRING INSPECTION WHICH IS COMPLETED WITHOUT INSPECTION WILL BE REJECTED SOLELY ON THAT BASIS.

RETAIN SPECIAL INSPECTION RECORDS: THE CONTRACTOR IS ALSO RESPONSIBLE FOR RETAINING AT THE JOB SITE ALL SPECIAL INSPECTION RECORDS COMPLETED BY THE SPECIAL INSPECTOR.

COORDINATE AND SUBMIT: THE CONTRACTOR IS RESPONSIBLE FOR PREPARING AND SUBMITTING TO THE BUILDING OFFICIAL AND THE OWNER A STATEMENT OF CONTRACTOR RESPONSIBILITY, IBC SECTION 1704.4, FOR THEMSELVES AND FOR SUBMITTING A STATEMENT OF CONTRACTOR RESPONSIBILITY FOR EACH STRUCTURAL COMPONENT SUBCONTRACTOR. THE STATEMENTS OF RESPONSIBILITY SHALL BE SUBMITTED PRIOR TO THE COMMENCEMENT OF WORK ON THE SYSTEM OR COMPONENT.

- A. THE STATEMENT OF CONTRACTOR RESPONSIBILITY SHALL CONTAIN THE FOLLOWING: 1. ACKNOWLEDGEMENT OF AWARENESS OF THE SPECIAL INSPECTION REQUIREMENTS
- CONTAINED IN THE QUALITY ASSURANCE PLAN.
- 2. ACKNOWLEDGEMENT THAT CONTROL WILL BE EXERCISED TO OBTAIN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS APPROVED BY THE BUILDING OFFICIAL.
- 3. PROCEDURES FOR EXERCISING CONTROL WITHIN THE CONTRACTOR'S ORGANIZATION, THE METHOD AND FREQUENCY OF REPORTING, AND THE DISTRIBUTION OF THE REPORTS.
- 4. IDENTIFICATION AND QUALIFICATIONS OF THE PERSONS EXERCISING SUCH CONTROL AND THEIR POSITIONS IN THE ORGANIZATION.

B. STRUCTURAL COMPONENT SUBCONTRACTORS INCLUDE BUT ARE NOT LIMITED TO STRUCTURAL STEEL FABRICATORS AND ERECTORS, COMPONENT FABRICATORS SUCH AS STEEL JOISTS, METAL OR WOOD TRUSSES, CONCRETE, AND MASONRY CONTRACTORS.

C. AT THE COMPLETION OF STRUCTURAL COMPONENT FABRICATION, THE FABRICATORS SHALL SUBMIT A CERTIFICATE OF COMPLIANCE STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR COSTS OF:

RETESTING AND REINSPECTION OF MATERIALS, WORK, AND/OR PRODUCTS THAT DO NOT MEET THE REQUIREMENTS OF THE CONTRACT DOCUMENTS AND SHOP DRAWINGS/SUBMITTAL DATA.

REVIEW OF PROPOSED REPAIR AND/OR REPLACEMENT PROCEDURES BY THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE AND THE INSPECTORS AND TESTING AGENCIES.

REPAIR OR REPLACEMENT OF WORK THAT DOES NOT MEET THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.

THE CONTRACTOR IS RESPONSIBLE FOR THE TRAVEL COSTS OF THE SPECIAL INSPECTOR OR AGENTS WHEN SHOP INSPECTION IS REQUIRED OF A NON APPROVED STRUCTURAL COMPONENT FABRICATOR.

INSPECTION OF FABRICATION:

WHERE FABRICATION OF STRUCTURAL, LOAD BEARING, OR LATERAL LOAD RESISTING MEMBERS OR ASSEMBLIES ARE PERFORMED ON THE PREMISES OF THE FABRICATOR, THE SHOP FABRICATION REQUIRES SPECIAL INSPECTION DURING THE FABRICATION OF ITEMS FOR THIS PROJECT.

EXEMPTION:

FABRICATORS APPROVED BY THE BUILDING OFFICIAL ARE EXEMPT FROM THE ON PREMISE INSPECTION. THE APPROVAL BY THE BUILDING OFFICIAL OF ANY FABRICATOR SHOULD BE PROPERLY DOCUMENTED PRIOR TO THE COMMENCEMENT OF FABRICATION. EXEMPTION WILL BE PROVIDED TO FABRICATORS WHO PROVIDE PROOF OF CERTIFICATION BY A NATIONALLY RECOGNIZED GOVERNING ASSOCIATION WHICH PERFORMS PERIODIC INSPECTIONS AND MAINTAINS QUALITY ASSURANCE CRITERIA.

EXAMPLES ARE: AISC CERTIFICATION FOR A STEEL FABRICATOR, SJI CERTIFICATION FOR A STEEL JOIST MANUFACTURER, WTC AND TPI CERTIFICATION FOR A PRE-ENGINEERED WOOD TRUSS MANUFACTURER.

AT THE COMPLETION OF FABRICATION, THE FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

CONCRETE TESTING NOTES:

CONCRETE TESTING AND INSPECTION SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF ACI 318 AND THE SCHEDULE OF SPECIAL INSPECTIONS. SAMPLES FOR STRENGTH TESTS OF EACH CLASS OF CONCRETE PLACED EACH DAY SHALL BE TAKEN NOT LESS THAN ONCE A DAY, NOR LESS THAN ONCE FOR EACH 75 CUBIC YARDS. OF CONCRETE USED FOR FOOTINGS, NOR LESS THAN ONCE FOR EACH 5000 SQUARE FEET OF SURFACE AREA FOR SLABS. TEST REPORTS INDICATING NON-COMPLIANCE SHALL BE PROVIDED TO THE OWNER, ARCHITECT AND CONTRACTOR. A COPY OF THE TEST REPORTS SHALL BE AVAILABLE AT THE JOBSITE.

STEEL INSPECTION AND TESTING NOTES:

STRUCTURAL STEEL TESTING AND INSPECTION SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF THE REFERENCED STANDARDS AND THE SCHEDULE OF SPECIAL INSPECTIONS.

FIELD BOLTED CONNECTIONS WILL BE TESTED AND INSPECTED ACCORDING TO RCSC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS."

FIELD WELDS SHALL BE INSPECTED AND TESTED ACCORDING TO AWS D1.1. IN ADDITION TO VISUAL INSPECTION. WELDED MOMENT CONNECTIONS WILL BE TESTED BY ULTRASONIC, ASTM E164, OR OTHER AWS APPROVED METHOD.

MASONRY INSPECTION AND TESTING NOTES:

CONCRETE MASONRY TESTING AND INSPECTION SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF TMS 402 / TMS 602 AND THE SCHEDULE OF SPECIAL INSPECTIONS. INSPECTION SHALL INCLUDE GENERAL INSPECTION OF WORK IN PROGRESS TO CONFIRM THAT MATERIALS, CONSTRUCTION, AND WORKMANSHIP ARE IN COMPLIANCE WITH PLANS. SPECIFICATIONS AND GOOD CONSTRUCTION PRACTICES. ADDITIONALLY, MORTAR SHALL BE SAMPLED AND TESTED IN ACCORDANCE WITH ASTM C780 ANNEX A6. EACH TEST SHALL CONSIST OF THREE SPECIMENS.

FREQUENCY OF TESTING: AT THE BEGINNING OF MASONRY CONSTRUCTION AND FOR EVERY 5000 SQUARE FEET OF MASONRY INSTALLED THEREAFTER.

COLD WEATHER = LESS THAN 40 DEGREES FAHRENHEIT HOT WEATHER = GREATER THAN 90 DEGREES FAHRENHEIT

INSPECTOR TO BE CERTIFIED BY THE INTERNATIONAL CODE COUNCIL.

OTHER REQUIRED INSPECTIONS:

THE REQUIREMENTS OF SPECIAL INSPECTIONS AND TESTING IN ACCORDANCE OF THE INTERNATIONAL BUILDING CODE ARE MINIMUM REQUIREMENTS AND DO NOT LIMIT THE REQUIREMENTS FOR THE CONTRACTOR TO PROVIDE OTHER QUALITY CONTROL INSPECTIONS AND TESTING REQUIRED BY THE OWNER, CONTRACT DOCUMENTS, OR OTHER GOVERNING AUTHORITIES HAVING JURISDICTION.

4. PROTE 5. H-PILE 1705. TYPE 1. INSTALL GIRDERS. a. END	
5. H-PILE 1705. TYPE 1. INSTALL GIRDERS. a. END	4. PROTE
1705. TYPE 1. INSTALL GIRDERS. a. ENI	5. H-PILE
1705. TYPE 1. INSTALL GIRDERS. a. ENI	
TYPE 1. INSTALL GIRDERS. a. ENI	1705.
1. INSTALL GIRDERS. a. ENI	TYPE
a. ENI	1. INSTALL GIRDERS.
	a. ENI

VERIFY PROCED

REPORTS CHAPTER COMPLIA 2 MATER HIGH-STE FILLER M/

LENGTH A 4. VERIFY STIFFENER EACH CON DOCUMEN

3. EMBED

5. STRUCT a. INSPEC OR PERF MEMBER N5.4-1)

b. INSPEC MEMBER N5.4-2)

c INSPEC

MEMBER N5.4-3) d. NONDE

JOINTS (AI 1) LISE OF PERSONN

> 2) COMPL OR GREA 3) WELDEI

4) WELDED

5) FABRIC PERFORM

6. STRUCT a INSPE

OR PERF CONNECT LISTED IN b. INSPEC THE QA TA

1) PRE-TEN a) TURN-0 b) DIRECT

c) TWIST-2) SNUG-1 INSPE(

ACCORDANCE WITH QA TASKS LISTED IN AISC 360, TABLE N5.6-3) (AISC 34 MATERI 1 ANCHO

STRUCTUR 2. FABRICA

3. REDUCE

b. BRI

SITE SAFETY AND MEANS AND METHODS OF CONSTRUCTION ARE SOLELY THE RESPONSIBILITY OF THE CONTRACTOR.

SUBCONTRACTOR OR MATERIAL SUPPLIER, IBC SEC. 1704.2. IN THE CASE OF AN OWNER/CONTRACTOR, THE SPECIAL INSPECTOR/AGENCY SHALL BE EMPLOYED AS

SCHEDULE OF SPECIAL INSPECT	IONS		
1704.2.5 INSPECTION OF FABRICATORS			
MATERIAL/ACTIVITY	SERVICE	APPLIC	CABLE TO PROJECT
VERIFY FABRICATION/QUALITY CONTROL PROCEDURES	IN-PLANT REVIEW (3) DURING FABRICATION	Y/N Y	EXTENT SPECIAL INSPECTIONS ARE NOT REQUIRED WHERE THE FABRICATOR IS REGISTERED AND APPROVED IN ACCORDANCE WITH SECTION 1704.2.5.1.
		L	
1705.2 STEEL CONSTRUCTION (AIS	SC 360: CHAPTER N)		
MATERIAL/ACTIVITY	SERVICE	APPLIC	CABLE TO PROJECT
1. FABRICATOR AND ERECTOR DOCUMENTS (VERIFY REPORTS AND CERTIFICATES AS LISTED IN AISC 360, CHAPTER N, SECTION N3, PARAGRAPH 2 FOR COMPLIANCE WITH CONSTRUCTION DOCUMENTS)	SUBMITTAL REVIEW	Y	EACH SUBMITTAL
2. MATERIAL VERIFICATION OF STRUCTURAL STEEL HIGH-STENGTH BOLTS, NUTS, WASHERS AND WELD FILLER MATERIALS.	SHOP (3) AND FIELD INSPECTION	Y	PERIODIC
3. EMBEDMENTS, VERIFY DIAMETER, GRADE, TYPE, LENGTH AND EMBEDMENT. (SEE 1705.3 FOR ANCHORS)	FIELD INSPECTION	Y	PERIODIC
4. VERIFY MEMBERS LOCATIONS, BRACES, STIFFENERS AND APPLICATION OF JOINT DETAILS AT EACH CONNECTION COMPLY WITH CONSTRUCTION DOCUMENTS	FIELD INSPECTION	Y	PERIODIC
5. STRUCTURAL STEEL WELDING			
a. INSPECTION TASKS PRIOR TO WELDING (OBSERVE, OR PERFORM FOR EACH WELDED JOINT OR MEMBER, THE QA TASKS LISTED IN AISC 360, TABLE N5.4-1)	SHOP (3) AND FIELD INSPECTION	Y	OBSERVE OR PERFORM AS NOTED (4)
b. INSPECTION TASKS DURING WELDING (OBSERVE, OR PERFORM FOR EACH WELDED JOINT OR MEMBER, THE QA TASKS LISTED IN AISC 360, TABLE N5.4-2)	SHOP (3) AND FIELD INSPECTION	Y	OBSERVE (4)
c. INSPECTION TASKS AFTER WELDING (OBSERVE, OR PERFORM FOR EACH WELDED JOINT OR MEMBER, THE QA TASKS LISTED IN AISC 360, TABLE N5.4-3)	SHOP (3) AND FIELD INSPECTION	Y	OBSERVE OR PERFORM AS NOTED (4)
d. NONDESTRUCTIVE (NDT) TESTING OF WELDED JOINTS (AISC 360: N5.5):			
1) USE OF QUALIFIED NONDESTRUCTIVE TESTING PERSONNEL.	PERFORMED	N	
2) COMPLETE PENETRATION GROOVE WELDS 5/16" OR GREATER IN RISK CATEGORY II	SHOP (3) OR FIELD ULTRASONIC TESTING - 20% OF WELDS MINIMUM	- N	PERFORM
3) WELDED JOINTS SUBJECT TO FATIGUE.	DT AND UT SHALL BE PERFOMED ON 100% OF WELDED JOINTS IDENTIFIED ON CONTRACT DRAWINGS AS BEING SUBJECT TO FATIGUE.	N	PERFORM
4) WELDED TAB REMOVAL SITES.	AT THE END OF WELDS WHERE WELD TABS HAVE BEEN REMOVED, MAGNETIC PARTICLE TESTING SHALL BE PERFORMED ON THE SAME BEAM TO COLUMN JOINTS RECEIVING UT.	N	PERFORM
5) FABRICATORS NDT REPORTS WHEN FABRICATORS PERFORMS NDT	VERIFY REPORTS	N	EACH SUBMITTAL (5)
6. STRUCTURAL STEEL BOLTING:	SHOP (3) AND FIELD INSPECTION		
a. INSPECTION TASKS PRIOR TO BOLTING (OBSERVE, OR PERFORM TASKS FOR EACH BOLTED CONNECTION, IN ACCORDANCE WITH QA TASKS LISTED IN AISC 360, TABLE N5.6-1)			OBSERVE OR PERFORM AS NOTED (4)
b. INSPECTION TASKS DURING BOLTING (OBSERVE THE QA TASKS LISTED IN AISC 360, TABLE N5.6-2)			OBSERVE (4)
1) PRE-TENSIONED AND SLIP-CRITICAL JOINTS			
a) TURN-OF-NUT METHOD (MATCHMARKING)			
b) DIRECT TENSION INDICATOR		N	PERIODIC
c) TWIST-OFF TYPE TENSION CONTROL BOLT		N	PERIODIC
2) SNUG-TIGHT JOINTS		Y	PERIODIC
C. INSPECTION TASKS AFTER BOLTING (PERFORM TASKS FOR EACH BOLTED CONNECTION IN	SHOP (3) AND FIELD INSPECTION AND TESTING	Y	PERFORM (4)

1705.2.1 STEEL CONSTRUCTION - OTHER INSPECTIONS

AL/ACTIVITY	SERVICE	APPLICA	APPLICABLE TO PROJECT		
	GERVICE	Y/N	EXTENT		
R RODS AND OTHER EMBEDMENTS SUPPORTING RAL STEEL.		Y	PERFORM		
TED STEEL OR ERECTED STEEL FRAME.		Y	OBSERVE		
D BEAM SECTIONS (RBS) WHERE/IF OCCURS.		N	DOCUMENT		
TED ZONES.		N	DOCUMENT		
WHERE/IF OCCURS.		Ν	DOCUMENT		

.2.3 OPEN-WEB STEEL JOIST AND JOIST GIRDERS

	APPLIC	ABLE TO PROJECT	REFERENCED STANDARD		
	Y/N	EXTENT			
ATION OF OPEN-WEB STEEL JOISTS AND JOIST					
O CONNECTIONS - WELDED OR BOLTED.	Y	PERIODIC	SJI SPECIFICATIONS LISTED IN SECTION 2207.1.		
DGING - HORIZONTAL OR DIAGONAL.	-				
1) STANDARD BRIDGING.	Y	PERIODIC	SJI SPECIFICATIONS LISTED IN SECTION 2207.1.		
2) BRIDGING THAT DIFFERS FROM THE SJI SPECIFICATIONS LISTED IN SECTION 2207.1.	N	PERIODIC			

1705.	1705.2.2 COLD FORMED STEEL DECK (SDI QAQC - 2017)					
TABL	E 1.1 INSPECTION OR EXECUTION TASKS PRIOR TO DECK PLACEMENT					
	TASK	Y/N	EXTENTS			
A	VERIFY COMPLIANCE OF MATERIAL, (DECK AND ALL DECK ACCESSORIES) WITH CONSTRUCTION DOCUMENTS, INCLUDING PROFILES, MATERIAL PROPERTIES AND BASE METAL THICKNESS	Y	EACH SUBMITTALS			
В	DOCUMENT ACCEPTANCE OR REJECTION OF DECK AND DECK ACCESSORIES.	Y	EACH SUBMITTALS			
TABL	E 1.2 INSPECTION OR EXECUTION TASKS AFTER DECK PLACEMENT					
	TASK	Y/N	EXTENTS			
A	VERIFY COMPLIANCE OF DECK AND ALL DECK ACCESSORIES INSTALLATION WITH CONSTRUCTION DOCUMENTS.	Y	PERIODIC			
В	VERIFY DECK MATERIALS ARE REPRESENTED BY THE MILL CERTIFICATIONS THAT COMPLY WITH THE CONSTRUCTION DOCUMENTS.	Y	PERIODIC			
С	DOCUMENT ACCEPTANCE OR REJECTION OF INSTALLATION OF DECK AND DECK ACCESSORIES.	Y	PERIODIC			
TABL	E 1.3 INSPECTION OR EXECUTION TASKS PRIOR TO WELDING					
	TASK	Y/N	EXTENTS			
А	WELDING PROCEDURE SPECIFICATIONS, (WPS) AVAILABLE.	Y	PERIODIC			
В	MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE.	Y	PERIODIC			
С	MATERIAL IDENTIFICATION, (TYPE/GRADE).	Y	PERIODIC			
D	CHECK WELDING EQUIPMENT.	Y	PERIODIC			
TABL	E 1.4 INSPECTION OR EXECUTION TASKS DURING WELDING					
	TASK	Y/N	EXTENTS			
А	USE OF QUALIFIED WELDERS.	Y	PERIODIC			
В	CONTROL AND HANDLING OF WELDING CONSUMABLES.	Y	PERIODIC			
С	ENVIRONMENTAL CONDITIONS, (WIND SPEED, MOISTURE, TEMPERATURE).	Y	PERIODIC			
D	WPS FOLLOWED.	Y	PERIODIC			
TABL	E 1.5 INSPECTION OR EXECUTION TASKS AFTER WELDING					
	TASK	Y/N	EXTENTS			
A	VERIFY SIZE AND LOCATION OF WELDS, INCLUDING SUPPORT, SIDELAP AND PERIMETER	Y	PERIODIC			
В	WELDS. WELDS MEET VISUAL ACCEPTANCE CRITERIA.	Y	PERIODIC			
С	VERIFY REPAIR ACTIVITIES.	Y	PERIODIC			
D	DOCUMENT ACCEPTANCE OR REJECTION OF WELDS.	Y	PERIODIC			
ταρι						
TADL						
•		Y/N	EXTENIS			
A	MANUFACTURER INSTALLATION INSTRUCTIONS AVAILABLE FOR MECHANICAL FASTENERS.	ř				
B	PROPER TOOLS AVAILABLE FOR FASTENER INSTALLATION.	Y	PERIODIC			
TARI			PERIODIC			
IADL			EVEENTO			
•		Y/N	EXTENIS			
A		Y				
D	PASTEMERS ARE INSTALLED IN ACCORDANCE WITH MANUFACTURERS INSTRUCTIONS.	T	PERIODIC			
TABL	E 1.8 INSPECTION OR EXECUTION TASKS AFTER MECHANICAL FASTENING	1	1			
	TASK	Y/N	EXTENTS			
A	CHECK SPACING, TYPE AND INSTALLATION OF SUPPORT FASTENERS.	Y	PERIODIC			
В	CHECK SPACING, TYPE AND INSTALLATION OF SIDELAP FASTENERS.	Y	PERIODIC			
С	CHECK SPACING, TYPE AND INSTALLATION OF PERIMETER FASTENERS.	Y	PERIODIC			
D	VERIFY REPAIR ACTIVITIES.	Y	PERIODIC			
E	DOCUMENT ACCEPTANCE OR REJECTION OF MECHANICAL FASTENERS.	Y	PERIODIC			

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MATERIAL/ACTIVITY	SERVICE	APPLICABLE TO PROJECT		
	0202	Y/N	EXTENT	
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	FIELD INSPECTION	Y	PERIODIC	
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	FIELD INSPECTION	Y	PERIODIC	
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS	FIELD INSPECTION	Y	PERIODIC	
4. VERIFY USE OF PROPER MATERIALS, DENSITIES, AND LIFT THICKNESSESS DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	FIELD INSPECTION	Y	CONTINUOUS	
5. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY	FIELD INSPECTION	Y	PERIODIC	

1. THE INSPECTION AND TESTING AGENT OR AGENTS, SHALL BE ENGAGED BY THE OWNER OR THE OWNER'S AGENT, AND NOT BY THE CONTRACTOR OR SUBCONTRACTOR WHOSE WORK IS TO BE INSPECTED OR TESTED. ANY CONFLICT OF INTEREST MUST BE DISCLOSED TO THE BUILDING OFFICIAL PRIOR TO COMMENCING WORK. THE QUALIFICATIONS OF THE SPECIAL INSPECTOR AND/OR TESTING AGENCIES MAY BE SUBJECT TO THE APPROVAL OF THE BUILDING OFFICIAL AND/OR THE DESIGN PROFESSIONAL.

2. SUBMIT A LIST OF THE SPECIAL INSPECTORS ON A SEPARATE DOCUMENT TO THE BUILDING OFFICIAL AND THE DESIGN PROFESSIONAL.

SPECIAL INSPECTIONS AS REQUIRED BY SECTION 1704.2.5 ARE NOT REQUIRED WHERE THE FABRICATOR IS APPROVED IN ACCORDANCE WITH IBC SECTION 1704.2.5

4. OBSERVE ON A RANDOM BASIS, OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS. PERFORM THESE TASKS FOR EACH WELDED JOINT, BOLTED CONNECTION, OR STEEL ELEMENT

5. NDT OF WELDS COMPLETED IN AN APPROVED FABRICATOR'S SHOP MAY BE PERFORMED BY THAT FABRICATOR WHEN APPROVED BY THE AHJ. REFER TO AISC 360, N7.

. SPECIAL INSPECTION: INSPECTION OF CONSTRUCTION REQUIRING THE EXPERTISE OF AN APPROVED SPECIAL INSPECTOR IN ORDER TO ENSURE OMPLIANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS

2. SPECIAL INSPECTOR: QUALIFIED FIRM OR INDIVIDUAL RESPONSIBLE FOR PERFORMING SPECIFIC TESTS OR INSPECTIONS AS PART OF THE SPECIAL INSPECTION PROGRAM.

PERIODIC SPECIAL INSPECTION: THE PART TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK. MAY BE ALLOWED WHEN COMPLIANCE OF THE WORK OR PRODUCT CAN BE DETERMINED AFTER BEING INCORPORATED INTO THE STRUCTURE

4. CONTINUOUS SPECIAL INSPECTION: THE FULL TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED

1705.3 CONCRETE CONSTRUCTION				
MATERIAL/ ACTIVITY	APPLIC/	ABLE TO PROJECT	REFERENCED	IBC REFERENCE
	Y/N	EXTENT	STANDARD	
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS AND VERIFY PLACEMENT.	Y	PERIODIC	ACI 318 CH.20, 25.2, 25.3, 26.6.1-26.6.3	1908.4
2. REINFORCING BAR WELDING: a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706	N	PERIODIC	AWS D1.4 ACI 318: 26.6.4	-
b. INSPECT SINGLE-PASS FILLET WELDS, MAX. 5/16" c. INSPECT ALL OTHER WELDS.	N N	PERIODIC CONTINUOUS		
3. INSPECT ANCHORS CAST IN CONCRETE.	Y	PERIODIC	ACI 318: 17.8.2	-
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS.	N	CONTINUOUS	ACI 318: 17 8 2 4	
UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.		CONTINUCUUS	AGI 310. 17.0.2.4	-
b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.a.	N	PERIODIC	ACI 318: 17.8.2	
5. VERIFY USE OF REQUIRED DESIGN MIX.	Y	PERIODIC	ACI 318: CH. 19. 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	Y	CONTINUOUS	ASTM C172 ASTM C31 ACI 318: 26.5, 26.12	1908.10
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	Y	CONTINUOUS	ACI 318: 26.5	1908.6, 1908.7, 1908.8
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	Y	PERIODIC	ACI 318: 26.5.3-26.5.5	1908.9
9. INSPECT PRESTRESSED CONCRETE FOR: a. APPLICATION OF PRESTRESSING FORCES b. GROUTING OF BONDED PRESTRESSING TENDONS.	N N	CONTINUOUS CONTINUOUS	ACI 318: 26.10	-
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.	N	PERIODIC	ACI 318: CH 26.9	-
11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	N	PERIODIC	ACI 318: 26.11.2	-
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	Y	PERIODIC	ACI 318: 26.11.1.2[b]	-
13. CONRETE STRENGTH TESTING AND VERIFICATION OF COMPLIANCE WITH CONSTRUCTION DOCUMENTS.	Y	PERIODIC	-	-

FOR SI: INCH = 25.4 mm.

{a} WHERE APPLICABLE, SEE ALSO SECTION 1705.12. SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE. (b) SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION SHALL BE INCLUDED IN THE RESEARCH REPORT FOR THE ANCHOR ISSUED BY AN PPROVED SOURCE IN ACCORDANCE WITH 17.8.2 IN ACI 318, OR OTHER QUALIFICATION PROCEDURES. WHERE SPECIFIC REQUIREMENTS ARE NOT PROVIDED, SPECIAL INSPECTION REQUIREMENTS SHALL BE SPECIFIED BY THE REGISTERED DESIGN PROFFESIONAL AND SHALL BE APPROVED BY THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF THE WORK

1705.4 MASONRY CONSTRUCTION						
MINIMUM VERIFICATION	REQ	REQUIRED FOR QUALITY ASSURANCE		REFERENCE FOR CRITERIA		
	LEVEL 1	LEVEL 2	LEVEL 3	TMS 402	TMS 602	
1. PRIOR TO CONSTRUCTION, VERIFICATION OF COMPLIANCE OF SUBMITTALS.	R	R	R		ART. 1.5	
2. PRIOR TO CONSTRUCTION, VERIFICATION OF fm AND faac, EXCEPT WHERE SPECIFICALLY EXEMPTED BY THE CODE.	NR	R	R		ART. 1.4 B	
3. DURING CONSTRUCTION, VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) WHEN SELF CONSOLIDATING GROUT IS DELIVERED TO THE PROJECT SITE.	NR	R	R		ART. 1.5 AND 1.6.3	
 DURING CONSTRUCTION, VERIFICATION OF fm AND faac, FOR EVERY 5,000 SQUARE FEET. 	NR	NR	R		ART. 1.4 B	
5. DURING CONSTRUCTION, VERIFICATION OF PROPORTIONS OF MATERIALS AS DELIVERED TO THE PROJECT SITE FOR PREMIXED OR PREBLENDED MORTAR, PRESTRESSING GROUT, AND GROUT OTHER THAN SELF-CONSOLIDATING GROUT.	NR	NR	R		ART. 1.4 B	
MINIMUM SPECIAL INSPECTIONS		FREQUENCY		REFERENCE FOR CI	ITERIA	
	LEVEL 1	LEVEL 2	LEVEL 3	TMS 402	TMS 602	
1. AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:						
A. PROPORTIONS OF SITE-PREPARED MORTAR.	NR	Р	Р		ART. 2.1, 2.6 A, AND 2.6C	
B. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES.	NR	Р	Р		ART. 2.4 B AND 2.4 H	
C. GRADE, TYPE, AND SIZE OF REINFORCEMENT, CONNECTORS, ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES.	NR	Р	Р		ART. 3.4 AND 3.6 A	
D. PRESTRESSING TECHNIQUE	NR	Р	Р		ART. 3.6 B	
E. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	NR	C(b)/P(c)	Р		ART. 2.1 C.1	
F. SAMPLE PANEL CONSTRUCTION	NR	Р	С		ART. 1.6 D	
2. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:						
A. GROUT SPACE	NR	Р	С		ART. 3.2 D AND 3.2	
B. PLACEMENT OF PRESTRESSING TENDONS AND ANCHORAGES	NR	Р	Р	SEC. 10.8 AND 10.9	ART. 2.4 AND 3.6	
C. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND ANCHOR BOLTS.	NR	Р	С	SEC. 6.1, 6.3.1, 6.3.6 AND 6.3.7	ART. 3.2 E AND 3.4	
D. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS.	NR	Р	Р		ART. 2.6 B AND 2.4 G.1.b	
3. VERIFY COMPLIANCE OF THE FOLLOWING DURING CONSTRUCTION:		·				
A. MATERIALS AND PROCEDURES WITH THE APPROVED SUBMITTALS.	NR	Р	P		ART. 1.5	
B. PLACEMENT OF MASONRY UNITS AND MORTAR JOINT CONSTRUCTION.	NR	Р	Р		ART. 3.3 B	
C. SIZE AND LOCATION OF STRUCTURAL MEMBERS.	NR	Р	Р		ART. 3.3 F	
D. TYPE, SIZE, LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OR MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION.	NR	Р	С	SEC. 1.2.1 (e), 6.2.1, AND 6.3.1		
E. WELDING OF REINFORCEMENT.	NR	С	С	SEC. 6.1.6.1.2		
F. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURES BELOW 40° F) OR HOT WEATHER (TEMPERATURE ABOVE 90° F).	NR	P	P		ART. 1.8 C AND 1.8	
G. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE.	NR	С	С		ART. 3.6 B	
H. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE.	NR	С	С		ART. 3.5 AND 3.6 C	
I. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	NR	C(p)/b(c)	С		ART. 3.3 B.9 AND 3. F.1.b	
4. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS:	NR	Р	С		ART. 1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3, AND 1.4 B.4	

FOR SI: INCH = 25.4mm. A) FREQUENCY REFERS TO THE FREQUENCY OF INSPECITON, WHICH MAY BE CONTINUOUS DURING THE LISTED TASK OR PERIODICALLY DURING THE LISTED TASK AS DEFINED IN THE TABLE. NR=NOT REQUIRED, P=PERIODIC, C=CONTINUOUS. (b) REQUIRED FOR THE FIRST 5000 SQUARE FEET OF AAC MASONRY

(c.) REQUIRED AFTER THE FIRST 5000 SQUARE FEET OF AAC MASONRY



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GENERAL SHEET NOTES

- SEE SHEET S001 FOR STRUCTURAL GENERAL NOTES.
- SEE SHEET S301 FOR TYPICAL FRAMING DETAILS.
- BASE REFERENCE ELEVATION IS FROM FINISH FLOOR ELEVATION 157.25 = (0'-0"). ALL ELEVATIONS ARE REFERENCED FROM BASE ELEVATION AND ARE SHOWN AS (±x'-x").
- FOR DIMENSIONS NOT SHOWN, SEE ARCHITECTURAL DRAWINGS.
- COORDINATE ALL OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS.
- COORDINATE ALL EQUIPMENT SIZES, LOCATIONS AND WEIGHTS WITH THE MECHANICAL CONTRACTOR. NOTIFY THE ENGINEER OF ANY DISCREPANCIES OR DEVIATIONS FROM WHAT IS SHOWN.
- SEE SHEET S201 FOR TYPICAL MASONRY WALL REINFORCING AND DETAILS.
- TOP OF STEEL FOR WIDE FLANGE BEAMS RUNNING PARALLEL TO JOIST FRAMING SHALL MATCH TOP OF ADJACENT JOISTS.

<u>LEGEND</u>

- (±x'-x") DENOTES TOP OF STEEL ELEVATION ABOVE FINISH FLOOR ELEVATION OF (0'-0").
- [±x'-x"] DENOTES BOTTOM OF STEEL ELEVATION ABOVE FINISH FLOOR ELEVATION OF (0'-0").
- "MP" DENOTES MASONRY PIER. SEE FOUNDATION PLAN FOR PIER MARKS.
- "L-x" DENOTES REINFORCED MASONRY LINTEL. SEE LINTEL DETAILS ON SHEET \$301.
- "C-x" DENOTES STEEL COLUMN. SEE COLUMN SCHEDULE THIS SHEET.
- "C.J." DENOTES MASONRY CONTROL JOINT IN CMU WALL. SEE DETAIL C3/S201.

DENOTES KICKER EXTENTS AT PARAPET RETURN WALLS

RC1.5 1 1/2"x22 GAUGE GALVANIZED WIDE RIB METAL ROOF DECK (TYPE B). ATTACH TO SUPPORT STEEL PER D1/S301

SHEET KEYNOTES

- 1. L2x2 BOTTOM CHORD BRACE. SEE DETAIL B4/S301
- 2. MASONRY CONTROL JOINT TO ALGN WITH STEP IN MASONRY PARAPET ELEVATION. SEE ARCHITECTURAL DRAWINGS FOR LOCATION.
- 3. ANGLE FRAMING AT ROOF OPENING. SEE DETAIL A4/S301
- 4. PROVIDE CHANNEL UNDER ALL SIDES OF MECHANICAL UNIT CURB. SEE DETAIL A2/S301
- PRE-ENGINEERED METAL CANOPY, SEE ARCH. SEE DETAIL B1/S301 FOR ATTACHMENT TO CMU.
- 6. AWNING AND ATTACHMENT TO CMU BY AWNING SUPPLIER. SEE ARCH. WHERE CFS OVERBUILD OCCURS, PROVIDE SOLID BLOCKING IN STUD SPACE.
- 7. PROVIDE TYPE L-1 MASONRY LINTEL ABOVE SCUPPER OPENINGS.
- 8. CHANNEL LAID IN DECK FLUTES UNDER PARAPET RETURN WALL FRAMING AND KICKERS. SEE SECTIONS.
- 9. BEAR CHANNEL ON PERIMETER ANGLE AT CMU WALL AND WELD THROUGH DECK FLUTES SIMILAR TO C3/S302.
- 10. L5x3-1/2x3/8 (LLV, GALV.) LOOSE ANGLE LINTEL w/ VERTICAL LEG INSTALLED TIGHT TO BACK FACE OF BRICK. BEAR 8" IN MASONRY EACH END.
- 11. DEMISING WALL OCCURS BELOW BEAM, SEE ARCH. PROVIDE SLIP TRACK CONNECTION TO BOTTOM FLANGE.
- 12. FUTURE EXHAUST HOOD BELOW (1300LBS MAX). ATTACHMENT TO ROOF STRUCTURE BY TENANT.
- 13. FUTURE MECHANICAL UNIT, SUPPORT, AND ROOF OPENINGS BY TENANT. UNIT WEIGHT = 300LBS MAX U.N.O. JOIST DESIGN INCLUDES FUTURE WEIGHTS NOTED.



GPD GROUP, INC. LIC. # - 30920 520 South Main Street, Suite 2531 Akron, OH 44311 330.572.2100 Fax 330.572.2101 FOR BIDDING ONLY NOT FOR CONSTRUCTION. \Box ork I Heartland Dental Clayton, NC Ò WMG PROJECT # NC22-0 2165 PRITCHARD ROAD CLAYTON, NC 27527 1/24/2024 BID SET 11/15/2023 PERMIT SET mk date issue ROOF FRAMING PLAN - AREA A CHECKED BY DRAWN BY AC JS 2023064.33

VERSION 2022.1



22/2024 10:33:12 AI

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- 7. PROVIDE TYPE L-1 MASONRY LINTEL ABOVE SCUPPER OPENINGS.
- 8. CHANNEL LAID IN DECK FLUTES UNDER PARAPET RETURN WALL FRAMING AND KICKERS. SEE SECTIONS.
- 9. BEAR CHANNEL ON PERIMETER ANGLE AT CMU WALL AND WELD THROUGH DECK FLUTES SIMILAR TO C3/S302.
- PROVIDE CHANNELS FOR SUPPORT OF FUTURE CONDENSING UNIT SIMILAR TO DETAIL A2/S301
 PROVIDE MP3.4 ABOVE LINTEL. SEE MASONRY PIER DETAILS ON SHEET S201.
- 12. W8x18 + BOT PL 3/8 (GALV) LINTEL. BOTTOM OF STEEL = (+10'-0"). SEE WF LINTEL BEARING DETAILS ON S301.

13. ROOF ACCESS LADDER, SEE ARCH.

COLUMN SCHEDULE BASE PLATE

TYPE MARK	TYPE	BxNxTH	ANCHOR ROD	EMBEDMENT
C-1	HSS6X6X1/4	12"x12"x3/4"	(4)-3/4" DIA.	12"
C-2	HSS8X8X3/8	14"x14"x3/4"	(4)-3/4" DIA.	12"
		•		

NOTE: - SEE DETAIL A1/S301 FOR BASE PLATE DETAILS. - SEE DETAIL A5/S201 FOR ANCHOR ROD DETAIL.



<u>KEYPLAN</u>

GPD GRC Lic. # 330.572	DUP, INC. - 30920 Main Street, Suite 2531 Akron, OH 44311 2100 Fax 330.572.2101
FOR	Heartland Dental 1200 Network Centre Drive Effingham, IL 62401
Heartland Dental - Clayton, NC	WMG PROJECT # NC22-0118 2165 PRITCHARD ROAD CLAYTON, NC 27527
1/24/2024 11/15/2023 mk date ROOF FI PLAN - A	BID SET BID SE

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CFS STUD PER
 SECTION B5/S302

CONT 43 MIL TRACK w/ (2) 1/4"
 DIA. x 1-3/4" HILTI KWIK CON II
 SCREWS @ 16" O.C.

GROUT COLLAR JOINT SOLID

- 6" CMU

1/2" EXP. JOINT FILLER

SIDEWALK, SEE CIVIL

(m













NOTES:

RUB CONCRETE TO REMOVE SONOTUBE FORM LINES AND FILL ALL HOLES FOR SMOOTH FINISH. WEATHERSEAL ALL EXPOSED CONCRETE.

COORDINATE ANCHOR ROD LAYOUT AND EMBEDMENT w/ POLE SUPPLIER

COORDINATE WITH CIVIL AND PHOTOMETRIC DRAWINGS FOR EXACT LOCATIONS AND QUANTITY

TYPICAL LIGHT POLE BASE DETAIL



C4



- HSS COL. SEE PLAN

- PAINT COL. w/ INTUMESCENT PAINT BELOW GRADE PROVIDE 3" MIN CONCRETE COVER ON ANCHOR RODS AND BASE PL.

REINFORCING, SEE
 SCHEDULE







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PUDDLE WELD DECK TO ANGLE @ 6" O.C.

AT SIM. SECTION, DECK

L6x6x5/16 CONT. w/ 3/4"DIA. THREADED RODS @ 1'-4"

O.C. w/HILTI HIT HY-270

L4x4x5/16 CONT. w/ 3/4"DIA.

THREADED RODS @ 1'-4"

ADHESIVE (6 3/4" EMBED.)

ATTACH DECK TO ANGLE

PER D1/S301

O.C. w/HILTI HIT HY-270

ADHESIVE (6 3/4" EMBED.)

ORIENTATION VARIES



T/CMU = 25'-4" MAX. SEE ARCH.

BRICK, SEE ARCH.

8" CMU, SEE S201

FOR REINF.

T/CMU = (+23'-4") MAX. SEE ARCH.

GROUT MASONRY SOLID @

ANCHORS.

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