DESIGN CRITERIA

- D1 ALL WORK SHALL CONFORM TO AT LEAST THE MINIMUM STANDARDS OF THE FLORIDA BUILDING CODE (FBC), SEVENTH EDITION (2020) AND THE LIFE SAFETY CODES. WIND DESIGN FOR COMPONENTS AND CLADDING SHALL BE BASED ON THE ASCE 7-16 CODE WITH A WIND RISK CATEGORY FOR EDUCATIONAL FACILITIES OF III. THE BUILDING SHELL HAS BEEN DESIGNED FOR ASCE 7-16 CODE WITH A WIND RISK CATEGORY FOR EDUCATIONAL FACILITIES OF III, THIS APPLIES TO ALL STRUCTURAL COMPONENTS INCLUDING ALUMINUM WALKWAY CANOPIES, FLAG POLES, SITE LIGHTING POLES, BLEACHERS AND THE
- D2 DESIGN LOAD VALUES ARE INDICATED ON THE APPROPRIATE SHEETS AS FOLLOWS:
 - FOUNDATION DESIGN SEE FOUNDATION NOTE F2 ON THIS SHEET FLOOR FRAMING DESIGN - SHEET S102.1B ROOF FRAMING DESIGN - SHEET S103.1A WIND DESIGN - SHEET 5603
 - RAIN DESIGN: 15 MINUTE RAINFALL INTENSITY: 8.64 in/hr 60 MINUTE RAINFALL INTENSITY: 4.29 in/hr
- TO THE BEST OF THE ENGINEER'S KNOWLEDGE, THE STRUCTURAL PLANS AND SPECIFICATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE BUILDING CODE.
- D4 EVERY REASONABLE EFFORT HAS BEEN MADE TO ENSURE COORDINATION BETWEEN THESE DRAWINGS AND THE BOUND STRUCTURAL SPECIFICATIONS. SHOULD THERE BE ANY DISCREPANCIES, THE CONTRACTOR SHALL THEN REQUEST A CLARIFICATION IN WRITING.

SPECIALTY ENGINEERING REQUIREMENTS

- STEEL PAN STAIRS, PRECAST CONCRETE STAIRS, SHIPS LADDERS AND OTHER ROOF ACCESS LADDERS SHALL BE DESIGNED BY THE FABRICATOR'S SPECIALTY ENGINEER AND SHALL INCLUDE STRINGERS, TREADS, HAND RAILINGS, PLATFORMS (AS REQUIRED), PAN INSERTS AND MISCELLANEOUS SUPPORTS AND CONNECTIONS. SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. MINIMUM DESIGN LIVE LOAD SHALL BE 100 PSF FOR STAIRS. SUBMITTALS SHALL INCLUDE THE JOINTING IN THE CONCRETE FILL AS REQUIRED TO MITIGATE PLASTIC SHRINKAGE
- GUARDRAILS, HANDRAILS, POSTS AND SUPPORT CONNECTIONS SHALL BE DESIGNED BY THE FABRICATOR'S SPECIALTY ENGINEER. SHOP DRAMINGS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED DATED AND SEALED BY A ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. DUE CONSIDERATION SHALL BE GIVEN TO EXPANSION & CONTRACTION BY PROVIDING SLIP JOINTS AS REQUIRED. DESIGN LOADING(S) SHALL CONFORM TO ALL REQUIREMENTS OF THE BUILDING CODE (SEE DESIGN CRITERIA FOR THE APPLICABLE BUILDING CODE).
- EXTERIOR CURTAIN WALLS & STOREFRONT SYSTEMS SHALL BE DESIGNED BY THE VENDOR'S SPECIALTY ENGINEER AND SHALL INCLUDE FRAME, GLASS, GLAZING AND CONNECTIONS. SHOP DRAMINGS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. DESIGN LOADING(S) SHALL CONFORM TO ALL REQUIREMENTS OF THE BUILDING CODE (SEE DESIGN CRITERIA FOR APPLICABLE BUILDING CODE). THE VENDOR SHALL PROVIDE WINDOW WALL REACTIONS TO THE ARCHITECT.
- FLAGPOLES AND SITE LIGHTING POLES SHALL BE DESIGNED BY THE POLE VENDOR'S SPECIALTY ENGINEER AND SHALL INCLUDE POLES, FOUNDATIONS AND CONNECTIONS. SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. MINIMUM DESIGN LOADS SHALL CONFORM TO ANSI/NAAM FP100 "SPECIFICATIONS FOR DESIGN LOADS OF METAL
- ALUMINUM AMNINGS, MALKWAY CANOPIES AND THEIR FOUNDATIONS SHALL BE DESIGNED BY THE FABRICATOR'S SPECIALTY ENGINEER AND SHALL INCLUDE FRAME, COVERING AND CONNECTIONS. SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED. DATED AND SEALED BY A STRUCTURAL ENGINEER IN THE SAME STATE AS THE PROJECT LOCATION. DESIGN LOADING(S) SHALL CONFORM TO ALL REQUIREMENTS OF THE BUILDING CODE. (SEE DESIGN CRITERIA FOR THE APPLICABLE BUILDING CODE). ALUMINUM WALKWAY CANOPY MANUFACTURER SHALL PROVIDE CONCRETE SUB-CONTRACTOR BIDDERS WITH FOUNDATION AND REINFORCING REQUIREMENTS FOR THEIR CANOPY SYSTEM PRIOR TO BID.
- EXTERIOR LIGHT STEEL FRAMING, INCLUDING BUT NOT LIMITED TO: WALLS, EXTERIOR CEILINGS, FASCIAS AND SOFFITS SHALL BE DESIGNED BY A SPECIALTY ENGINEER. STRUCTURAL ELEMENTS HAVE BEEN PROVIDED FOR THE ATTACHMENT OF THE LIGHT STEEL FRAMING. THE LIGHT STEEL SYSTEM SUPPLIER SHALL DESIGN AND DETAIL ALL CONNECTIONS TO THESE ELEMENTS. ANY FURTHER ELEMENTS REQUIRED FOR THE SUPPORT SHALL BE DESIGNED AND SUPPLIED AS PART OF THE LIGHT STEEL SYSTEM. SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY AN ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. DESIGN LOADING(S) SHALL CONFORM TO ALL REQUIREMENTS OF THE BUILDING CODE (SEE DESIGN CRITERIA FOR THE APPLICABLE BUILDING CODE) AND MAXIMUM DEFLECTIONS SHALL BE AS FOLLOWS: L/600 (BRICK VENEER), L/360 (STUCCO) & L/240 (FLEXIBLE FINISHES). A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
- CONTRACTOR SHALL SUBMIT FOR ENGINEER'S APPROVAL A SHORING/RE-SHORING PLAN WITH SECTIONS AND CALCULATIONS, SIGNED, DATED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE SAME STATE AS PROJECT LOCATION.
- SES REFER TO STRUCTURAL STEEL NOTES FOR CONNECTION ENGINEERING.
- PROVIDE ENGINEERED SUBMITTALS, SIGNED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION, FOR ALL MECHANICAL, ELECTRICAL AND PLUMBING SUPPORTS OR ATTACHMENTS NOT INCLUDED WITHIN THE CONSTRUCTION DOCUMENTS FOR ALL ITEMS THAT REQUIRE ANYTHING OTHER THAN THE MANUFACTURER'S STANDARD HARDWARE OR ARE EXPOSED TO WIND LOADS. AN EXAMPLE OF POSSIBLE ITEMS THAT THIS WOULD APPLY TO ARE, INCLUDING BUT NOT LIMITED TO, ROOFTOP AND/OR WALL MOUNTED DUCTS, PIPES AND TRANSFORMERS.
- SE10 STRUT TYPE METAL SLOTTED CHANNEL FRAMING SYSTEM (STRUT SYSTEM) SHALL BE DESIGNED BY A SPECIALTY ENGINEER. STRUCTURAL ELEMENTS HAVE BEEN PROVIDED FOR THE ATTACHMENT OF THE STRUT SYSTEM. THE STRUT SYSTEM SUPPLIER SHALL DESIGN AND DETAIL ALL CONNECTIONS TO THESE ELEMENTS. ANY ADDITIONAL ELEMENTS REQUIRED FOR THE SUPPORT SHALL BE DESIGNED AND SUPPLIED AS PART OF THE STRUT SYSTEM. SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED, DATED AND SEALED BY AN ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. DESIGN LOADING(S) SHALL CONFORM TO ALL REQUIREMENTS OF THE BUILDING CODE (SEE DESIGN CRITERIA FOR THE APPLICABLE BUILDING CODE).

FLORIDA PRODUCT APPROVAL / NOTICE OF ACCEPTANCE

- FPA1 THE STRUCTURAL ROOF DECK(S) FOR THIS PROJECT HAVE BEEN DESIGNED TO BE COMPLIANT WITH THE CODES AND PRESSURES LISTED HEREIN. ANY PRODUCTS INSTALLED OVER THE STRUCTURAL DECK(S) SHALL HAVE BEEN TESTED AS AN ASSEMBLY, INCLUSIVE OF THE STRUCTURAL DECK, AND SHALL BE SUBMITTED FOR APPROVAL WITH THE FPA/NOA CLEARLY IDENTIFIED. FAILURE TO PROVIDE THIS INFORMATION SHALL BE CAUSE FOR REJECTION BY THE ARCHITECT AND BBM STRUCTURAL ENGINEERS, INC. ALL WARRANTIES MUST BE PART OF THE ROOFING SUBMITTAL. ANY WARRANTY CONTAINING LANGUAGE WHICH "EXCLUDES GALE AND/OR HURRICANE FORCE WIND DAMAGE", OR "EXCLUDES DAMAGE FROM GALE AND/OR HURRICANE FORCE WINDS", OR ANY OTHER SIMILAR VERBIAGE, SHALL BE REJECTED AND THE ENTIRE PRODUCT ASSEMBLY SHALL NOT BE PERMITTED FOR USAGE ON THE PROJECT.
- FPA2 RATIONAL ANALYSIS OF THE ROOF SYSTEM SHALL NOT BE PERMITTED WITHOUT THE WRITTEN CONSENT OF THE OWNER. ARCHITECT AND ENGINEER, AND THEN, ONLY WHERE PERMITTED BY THE LOCAL JURISDICTION HAVING DIRECT AUTHORITY.

GENERAL

- THE GENERAL CONTRACTOR SHALL REVIEW AND DETERMINE THAT DIMENSIONS ARE COORDINATED BETWEEN ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO FABRICATION OR START OF CONSTRUCTION.
- G2 THE GENERAL CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, THE WORK PERSONS AND OTHER PEOPLE DURING CONSTRUCTION. HE SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION.
- 63 NO STRUCTURAL MEMBER SHALL BE CUT, NOTCHED OR OTHERWISE REDUCED IN STRENGTH.
- THE GENERAL CONTRACTOR SHALL COORDINATE ARCHITECTURAL MECHANICAL AND ELECTRICAL DRAWINGS FOR ANCHORED, EMBEDDED AND SUPPORTED ITEMS WHICH AFFECT THE STRUCTURAL DRAWINGS AND NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES.
- ALL SHOP DRAWING SUBMITTALS SHALL BE SUBMITTED VIA ELECTRONIC MEDIA (i.e. PDF OR DWF FORMAT). HARD COPY SUBMITTALS WILL NOT BE ACCEPTED.
 - 1. SUBMITTALS SHALL NOT BE SECURED IN ANY FORMAT THAT WILL PREVENT COMMENTS FROM BEING ADDED
 - 2. SUBMITTALS THAT ARE REQUIRED TO BE SIGNED AND SEALED SHALL BE SUBMITTED WITH A VISIBLE INK SEAL OR SHADED RAISED SEAL AT TIME OF FIRST SUBMITTAL, AND SHALL BE DIGITALLY SIGNED THAT MEETS THE REQUIREMENTS OF THE STATE BOARD WHERE THE PROJECT IS LOCATED.
- G6 ANY SUBMITTALS RECEIVED BY ARCH/ENG THAT HAVE NOT BEEN CHECKED BY THE GC AND HIS SUB-CONTRACTOR SHALL BE RETURNED

SIMILAR UNLESS ANOTHER SECTION OR DETAIL IS NOTED.

- MITHOUT REVIEW. G7 ALL SECTIONS AND DETAILS SHALL BE CONSTRUED TO BE TYPICAL OR
- 68 ANY CONFLICTS NOTICED, OR OBSERVED, BETWEEN THE WRITTEN SPECIFICATIONS AND THE CONSTRUCTION DOCUMENTS DURING PROJECT BIDDING OR PROJECT CONSTRUCTION SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE STRUCTURAL ENGINEER-OF-RECORD. I SUCH DISCREPANCY IS NOT NOTICED OR BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER-OF-RECORD FOR WRITTEN CLARIFICATION, THE CONTRACTOR/SUB-CONTRACTOR SHALL PROVIDE, AT PROJECT BID OR DURING PROJECT CONSTRUCTION, THE MORE STRINGENT AND/OR MORE COSTLY OF THE TWO ITEMS IN THE BID AND/OR FINAL INSTALLATION.
- "BBM STUCTURAL ENGINEERS" ASSUMES NO RISK OR LIABILITY FOR THE SITE SAFETY OR WELL-BEING OF ANY CONTRACTOR, SUB-CONTRACTOR NOR THEIR EMPLOYEES DURING THE CONSTRUCTION OF THE PROJECT CONTAINED IN THESE DOCUMENTS.
- G10 GENERAL CONTRACTOR/CONSTRUCTION MANAGER SHALL SUPPLY ALL SUB-CONTRACTORS WITH THE STRUCTURAL GENERAL NOTE SHEETS AS WELL AS THE STRUCTURAL DRAWINGS.
- G11 THE STRUCTURAL STEEL AND OPEN MEB STEEL JOISTS SHALL BE FABRICATED AND ERECTED IN FULL CONFORMANCE WITH THE "OSHA STEEL ERECTION STANDARD". IF THE CONSTRUCTION DRAWINGS DEVIATE FROM THE OSHA STANDARD THEN THE FABRICATOR SHALL PROVIDE SUBMITTALS THAT CLEARLY INDICATE THE DEVIATION WITH A REVISION CLOUD AND REQUEST APPROVAL FROM "BBM" TO MAKE THE CHANGE SO THAT CONFORMANCE WITH THE OSHA STANDARD IS ASSURED.
- G12 THE CONTRACTOR'S MEANS AND METHODS SHALL FULLY CONFORM TO THE REQUIREMENTS OF SEI/ASCE 37 (DESIGN LOADS ON STRUCTURES DURING CONSTRUCTION) UNTIL ALL OF THE STRUCTURAL ELEMENTS ARE IN PLACE AND HAVE RECEIVED THE INSPECTOR'S APPROVAL.
- G13 REFER TO ARCHITECTURAL DRAWINGS FOR ROOF COVERINGS. ROOF COVERINGS FOR ENHANCED HURRICANE PROTECTION AREA (EHPA) FACILITIES SHALL BE PROVIDED IN ACCORDANCE WITH THE LATEST ASTM AND FACTORY MUTUAL STANDARDS FOR MATERIALS AND WIND UPLIFT FORCES. ROOFS SHALL BE INSPECTED BY A LICENSED ENGINEER/ARCHITECT AND A REPRESENTATIVE OF THE ROOFING MANUFACTURER AND REPORTS SHALL BE SUBMITTED TO THE OWNER AND ARCHITECT.

- THE FOUNDATION AND SLAB-ON-GRADE DESIGN CONTAINED HEREIN S BASED SOLELY UPON THE PROJECT'S GEOTECHNICAL REPORT (PROJECT NUMBER HD225074, DATED FEBRUARY 22, 2023, AS PREPARED BY TERRACON). BBM HAS RELIED EXCLUSIVELY ON THE CONTENTS AND RECOMMENDATIONS WITHIN THIS REPORT, BUT " ACCEPTS NO RESPONSIBILITY WHATSOEVER FOR ITS CONTENTS OR ACCURACY. THE GENERAL CONTRACTOR/CONSTRUCTION MANAGER SHALL MAKE THEMSELVES FAMILIAR WITH THE REPORT BEFORE THE COMMENCEMENT OF CONSTRUCTION.
- F2 > FOUNDATION DESIGN IS BASED ON AN ASSUMED NET ALLOWABLE SOIL BEARING PRESSURE OF 2,500 PSF. COMPACTION UNDER ALL FOUNDATIONS SHALL ALSO BE AS STATED IN NOTE SOG1.
- F3 A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY-STANDARD SOIL DENSITY TESTS TO ENSURE CONFORMANCE WITH GEOTECHNICAL SOILS REPORT. SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
- F4 CONTRACTOR, IN CONJUNCTION WITH GEOTECHNICAL FIELD REPRESENTATIVE, SHALL DETERMINE IF ANY UNSUITABLE CONDITIONS ARE DISCOVERED DURING EXCAVATION WHICH MOULD PREVENT ATTAINMENT OF THE DESIGN SOIL PRESSURE RECOMMENDED BY THE SOILS REPORT.
- FOR FOUNDATION DESIGN VALUES, SEE FOUNDATION SCHEDULE
- FOOTINGS SHALL BE CAST TO THE SCHEDULED SIZE AND SHALL NOT BE OVERSIZED BY MORE THAN 6" ON ANY SIDE FOR FOOTING MIDTH OF AT LEAST 6'-O". FOR FOOTINGS LESS THAN 6'-O" IN WIDTH THE MAXIMUM OVERSIZING SHALL BE 3".
- F7 CONTRACTOR SHALL BE PREPARED FOR AND SHALL INCLUDE COST OF FORMING FOUNDATIONS SHOULD THE EARTH NOT PROVIDE ADEQUATE BANK STABILITY.

SLAB ON GRADE

- UNLESS NOTED OTHERWISE IN THE GEOTECHNICAL REPORT, COMPACT INTERIOR FILL TO (95%) OF MODIFIED PROCTOR MAXIMUM DRY DENSITY (ASTM D1557). SOIL COMPACTION SHALL BE FIELD-CONTROLLED BY A REPRESENTATIVE TECHNICIAN OF A QUALIFIED LABORATORY. EACH LAYER OF FILL SHALL NOT EXCEED 12" THICK AND SHALL BE COMPACTED PRIOR TO PLACEMENT OF NEXT LAYER.
- 50G2 MAXIMUM SPACING OF CONTROL JOINTS (i.e. SAWCUT JOINT OR CONSTRUCTION JOINT) SHALL BE AS SET IN THE TABLE BELOW, OR AS NOTED ON PLANS. THE MORE STRINGENT SHALL APPLY. PATTERNS SHALL BE APPROXIMATELY SQUARE WITH A RATIO OF LONG SIDE TO SHORT SIDE NOT EXCEEDING 1.5 TO 1. SEE SLAB-ON-GRADE DETAILS FOR ADDITIONAL

SLAB THICKNESS (IN)	* 3/4" OR LARGER AGGREGATE SPACING (FT)	* MIX DESIGNS CONTAINING AGGREGATE LESS THAN 3/4" ARE NOT ACCEPTABLE.
4	12	
5	13	
6	14	
7 AND GREATER	15	

- 50G3 GENERAL CONTRACTOR SHALL COORDINATE EXACT LOCATION OF SJ'S AND CJ'S WITH ARCHITECTURAL FLOOR FINISHES TO ENSURE SLAB JOINTS DO NOT READ THROUGH.
- SOG4 SLAB THICKNESS SHALL BE INCREASED AS REQUIRED TO PROVIDE ADEQUATE SUPPORT FOR CRANE LOADS WITHOUT CRACKING SLAB.

POST-INSTALLED ANCHORS

- POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER-OF-RECORD (SEOR) PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REBAR. HOLES SHALL BE DRILLED AND CLEANED AND PRODUCT SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII). SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE SPECIFIED BELOW, SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER-OF-RECORD ALONG WITH CALCULATIONS THAT ARE PREPARED & SEALED BY A REGISTERED PROFESSIONAL ENGINEER IN THE SAME STATE AS PROJECT LOCATION. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING EQUIVALENT OR BETTER PERFORMANCE VALUES OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S) AS REQUIRED BY THE BUILDING CODE.
- THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS. ADDITIONALLY, INSTALLATION OF ADHESIVE ANCHORS IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION SHALL BE DONE BY CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS CERTIFIED THROUGH ACI AND IN ACCORDANCE WITH ACI 318-14 (SECTION 17.8.2.2). PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED PRIOR TO COMMENCEMENT OF INSTALLATION.

PIA3 CONCRETE ANCHORS:

- MECHANICAL ANCHORS FOR USE IN CRACKED AND UNCRACKED CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193. PRE-APPROVED MECHANICAL ANCHORS
- 1. DEWALT "POWER-STUD+ SD1" (ICC-ES ESR-2818) 2. DEMALT "POWER-STUD+ SD2" (ICC-ES ESR-2502)
- 3. DEWALT "POWER-STUD+ SD6" (ICC-ES ESR-3471)
- (316 STAINLESS STEEL) 4. DEWALT "SCREW-BOLT+" (ICC-ES ESR-3889)
- 5. DEWALT "SNAKE+" (ICC-ES ESR-2272) 6. DEWALT "MINI-UNDERCUT+" (ICC-ES ESR-3912,
- 7. HILTI KWIK BOLT-TZ EXPANSION ANCHOR SAFE SET SYSTEM WITH SI-AT-A22 TOOL WITH ADAPTIVE
- TORQUE FOR APPLICABLE SIZES (ICC-ES ESR-1917) 8. HILTI HUS EZ SCREW ANCHOR (ICC-ES ESR-3056) 9. SIMPSON STRONG-TIE "STRONG-BOLT 2" (ICC-ES ESR-3037)

10. SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR-2713)

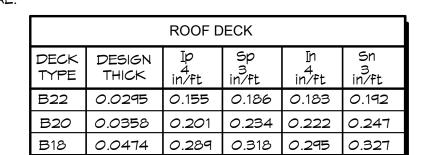
- ADHESIVE ANCHORS FOR USE IN CRACKED AND UNCRACKED CONCRETE SHALL BE INSPECTED AS FOLLOWS. AT THE ONSET OF EACH APPLICATION, A MANUFACTURER'S REPRESENTATIVE MUST BE PRESENT TO WITNESS AT LEAST FIVE COMPLETE INSTALLATIONS. INSTALLERS MUST BE TRAINED BY THE MANUFACTURER AND MANUFACTURER SHALL SUBMIT DOCUMENTATION TO THE CONTRACTOR INDICATING TRAINING HAS TAKEN PLACE. TRAINED INSTALLERS SHALL PROVIDE WRITTEN DOCUMENTATION TO THE CONTRACTOR THAT ALL ANCHORS HAVE BEEN INSTALLED PER THE MPII. CONTRACTOR SHALL SUBMIT THIS DOCUMENTATION TO THE SEOR. ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE HAVING MINIMUM AGE OF 21 DAYS. HOLES SHALL BE DRY AT THE TIME OF INSTALLATION. ADHESIVES SHALL HAVE MAX IN-SERVICE SHORT-TERM
- TEMPERATURE OF 150°F, AND MAX IN-SERVICE LONG-TERM TEMPERATURE OF 110°F. PRIOR TO INSTALLATION OF ADHESIVE ANCHORS IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATIONS RESISTING SUSTAINED TENSION LOADS. INSTALLERS ARE REQUIRED TO BE CERTIFIED IN ACCORDANCE WITH THE ACI. CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM AND MUST BE CONTINUOUSLY INSPECTED. ANCHORS SHALL ALSO HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC308. PRE-APPROVED ADHESIVE ANCHORS INCLUDE THE FOLLOWING PRODUCTS:
- . DEWALT "PURE 110+" (ICC-ES ESR-3298) 2. DEWALT AC200+ GOLD (ICC-ES ESR-4027)
- 3. HILTI a. HILTI HIT-HY 200 SAFE SET SYSTEM WITH THE HILTI HIT-Z ROD PER ICC ESR-3187.
- b. HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT (TE-CD OR TE-YD) WITH HAS-E THREADED ROD PER ICC ESR-3187. C. HILTI HY 200 ADHESIVE ANCHOR USING TRADITIONAL CLEANING METHOD, 2 BLOWS OF COMPRESSED AIR (90 PSI MINIMUM, OIL FREE COMPRESSOR), 2 BRUSHES WITH SAME SIZE AS HOLE WIRE BRUSH
- AND 2 MORE BLOWS, (2x2x2 CLEANING METHOD) PER ICC ESR-3187. d. HILTI HIT-RE 500-V3 EPOXY ADHESIVE ANCHORING SYSTEM WITH HAS-E THREADED ROD PER ICC ESR-2322 FOR SLOW CURE APPLICATIONS.
- 4. SIMPSON STRONG-TIE "SET-XP" (ICC-ES ESR-2508) 5. SIMPSON STRONG-TIE "AT-XP" (IAPMO UES ER-263) 6. SIMPSON STRONG-TIE "ET-HP" (ICC-ES ESR-3372)

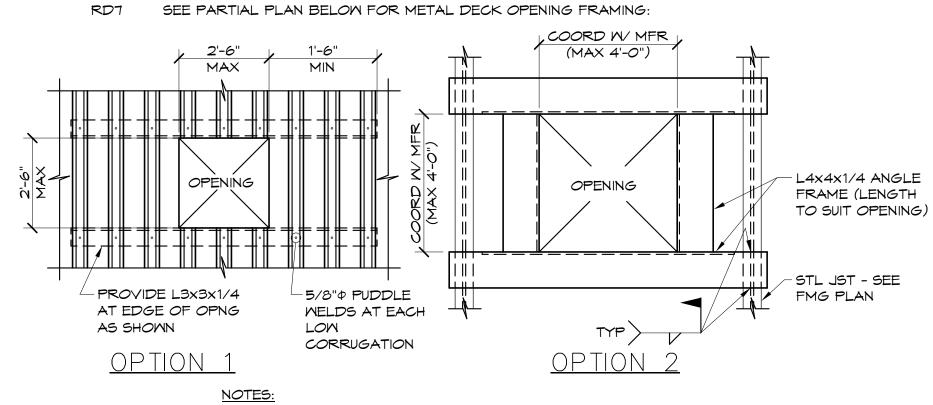
PIA4 MASONRY ANCHORS:

- A. ANCHORAGE TO SOLID-GROUTED CONCRETE MASONRY
 - MECHANICAL AND CONCRETE SCREW ANCHORS FOR USE IN SOLID-GROUTED CONCRETE MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES ACO1 OR AC106, RESPECTIVELY. PRE-APPROVED MECHANICAL AND CONCRETE SCREW ANCHORS INCLUDE:
 - a. DEWALT "POWER-STUD+ SD1" (ICC-ES ESR-2966)
 - b. DEWALT "SCREW-BOLT+" (ICC-ES ESR-4042) . HILTI KWIK BOLT 3 (ICC-ES ESR-1385) d. SIMPSON STRONG-TIE "STRONG-BOLT 2" (IAPMO UES ER-240)
 - e. SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR-1056)
 - 2. ADHESIVE ANCHORS FOR USE IN SOLID-GROUTED CONCRETE MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC58. PRE-APPROVED ADHESIVE ANCHORS INCLUDE:
 - a. DEWALT AC100+ GOLD (ICC-ES ISR-3200) . HILTI HY 270 ADHESIVE ANCHOR (ICC-ES ESR-4143)
 - SIMPSON STRONG-TIE "SET-XP" (IAPMO UES ER-265) d. SIMPSON STRONG-TIE "AT-XP" (IAPMO UES ER-281) e. SIMPSON STRONG-TIE "ET-HP" (IAPMO UES ER-241)
 - ANCHORAGE TO HOLLOW CONCRETE MASONRY / UNREINFORCED CLAY BRICK MASONRY SCREW ANCHORS FOR USE IN HOLLOW CONCRETE MASONRY
 - SHALL HAVE BEEN TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC106. PRE-APPROVED SCREW ANCHORS INCLUDE:
 - a. SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR-1056)
 - 2. ADHESIVE ANCHORS WITH SCREEN TUBES FOR USE IN HOLLOW CONCRETE MASONRY / UNREINFORCED CLAY BRICK MASONRY SHALL BE TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC58 OR AC60, AS APPROPRIATE. THE APPROPRIATE SCREEN TUBE SHALL BE USED AS RECOMMENDED BY THE ADHESIVE MANUFACTURER. PRE-APPROVED ADHESIVE ANCHORS WITH
 - SCREEN TUBES INCLUDE: a. DEWALT AC100+ GOLD (ICC-ES ISR-3200)-HOLLOW CONCRETE MASONRY b. DEWALT AC100+ GOLD (ICC-ES ISR-4105)-CLAY BRICK MASONRY . HILTI HY 270 ADHESIVE ANCHOR (ICC-ES ESR-4143) d. SIMPSON STRONG-TIE "SET" (ICC-ES ESR-1772)
- e. SIMPSON STRONG-TIE "ACRYLIC-TIE" (ICC-ES ER-5791) POMDER-ACTUATED FASTENERS (PAF) SHALL BE BY SIMPSON STRONG-TIE (ICC-ES ESR-2138), HILTI (ICC-ES ESR-1663), DEWALT "POWER DRIVEN
- FASTENERS" (ICC-ES ESR-1995) OR ENGINEER-APPROVED EQUAL GAS-ACTUATED FASTENERS (GAF) SHALL BE BY SIMPSON STRONG-TIE (ICC-ES ESR-2811), HILTI (ICC-ES ESR-1752), DEMALT "TRAK-IT C5 FASTENERS" (ICC-ES ESR-3275) OR ENGINEER-APPROVED EQUAL

STEEL ROOF DECK

- SEE ROOF FRAMING PLAN(S) FOR STEEL DECK ATTACHMENT TO STRUCTURE.
- STEEL ROOF DECK UNITS SHALL BE FABRICATED FROM STEEL CONFORMING TO SECTION AS OF THE LATEST EDITION OF THE AMERICAN IRON AND STEEL INSTITUTE, SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS. THE STEEL USED SHALL BE GRADE 80.
- IF OPTED BY GC, ALL FIELD WELDING OF DECK SHALL BE IN STRICT CONFORMANCE WITH ANSI/AMS D1.3 STRUCTURAL MELDING CODE.
- ALL SCREMS SHALL COMPLY WITH ASTM 1513, ICC AC43, AND ICC AC118. FASTENERS SHALL BE INSTALLED PERPENDICULAR TO ELEMENT SO AS TO PROPERLY SEAT FASTENER HEAD, AND TORQUED PER MANUFACTURES SPECIFICATIONS. NOT TO EXCEED MAXIMUM RECOMMENDED TORQUE.
- GALVANIZING SHALL CONFORM TO ASTM-A653, STRUCTURAL QUALITY AND FEDERAL SPEC. QQ-S-775.
- SEE CHART BELOW FOR MINIMUM SECTION PROPERTIES REQUIRED FOR STEEL DECK. PROPERTIES SHOWN ARE REPRODUCED FROM THE VULCRAFT





- FOR OPENINGS WITH A MAXIMUM DIMENSION OF 6" TO 1'-0", REINFORCE OPENING WITH A 20ga GALV FLAT PLATE 1'-O" LARGER THAN THE OPENING.
- ATTACH MITH 1" MELDS AT EACH RIB ALL AROUND B. FOR OPENINGS WITH A MAX DIMENSION OF 1'-O"
- TO 2'-6", SEE DETAIL OPTION 1 ABOVE. FOR OPENINGS WITH A MAX DIMENSION OF 4'-O" SEE DETAIL OPTION 2 ABOVE.
- NO ITEMS SHALL BE HUNG DIRECTLY FROM THE ROOF DECK UNLESS INDICATED OTHERWISE IN THE DRAWINGS.
- PROVIDE METAL CLOSURE STRIPS AT OPEN UNCOVERED ENDS AND EDGES OF ROOF DECKING AND IN VOIDS BETWEEN DECKING AND OTHER CONSTRUCTION. WELD INTO POSITION TO PROVIDE A COMPLETE ENGLOSED DECKING INSTALLATION. PROVIDE FLEXIBLE CLOSURE STRIPS INSTEAD OF METAL CLOSURES, AT CONTRACTOR'S OPTION, WHEREVER THEIR USE WILL ENSURE COMPLETE CLOSURE. INSTALL WITH ADHESIVE IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.

TILT-UP PANELS

- TUP1 ALL PANELS ARE VIEWED FROM THE INSIDE.
- TUP2 PANEL THICKNESS SHALL BE AS INDICATED ON PLANS. SPECIAL ATTENTION MUST BE GIVEN TO THE LOCATION AND PLACEMENT OF THE
- TUP3 REFER TO THE ARCHITECTURAL DRAWINGS FOR FINISH REQUIREMENTS CHAMFERS, ETC.
- TUP4 PANELS SHALL NOT BE LIFTED UNTIL CONCRETE HAS ATTAINED THE MINIMUM MODULUS OF RUPTURE AND COMPRESSIVE STRENGTH AS REQUIRED BY LIFTING ENGINEER.
- TUP5 THE CONTRACTOR SHALL HIRE SPECIALTY ENGINEER(S) TO PROVIDE DESIGN OF ALL ASPECTS OF THE LIFTING, BRACING AND TEMPORARY ERECTION STAGES FOR TILT-UP PANELS. THE ENGINEERING SHALL INCLUDE LIFTING INSERTS, BRACING DESIGN, BRACE CONNECTIONS VERIFICATION OF ADJOINING SURFACES WHICH BRACES ATTACH. TEMPORARY CONNECTIONS AT THE BASE OF PANEL WHILE PANEL IS TEMPORARILY BRACED & ANY ADDITIONAL REINFORCING STEEL REQUIRED FOR LIFTING OR BRACING OPERATION. PROVIDE SHOP DRAWINGS SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE SAME STATE AS PROJECT LOCATION. HOWEVER, NO ADDITIONAL REINFORCING SHALL BE ADDED WITHOUT THE EXPRESSED APPROVAL OF THE ENGINEER. THE DESIGNERS OF THE LIFTING INSERTS MUST CONSIDER THE REINFORCING ALREADY PRESENT IN THE PANELS AS INDICATED IN THIS SET OF CONSTRUCTION DRAWINGS. IN NO INSTANCE DOES ENGINEER OF RECORD IMPLY OR ACCEPT ANY RESPONSIBILITY FOR ANY MEANS, METHODS OR ADDITIONAL REQUIREMENTS AS RELATED TO TECHNIQUE, OR SAFETY OF THE PANEL LIFTING OPERATION. THE SLAB THICKNESS SHOWN HEREIN DOES NOT TAKE INTO CONSIDERATION SPECIFIC LOADS THAT MAY NEED TO BE RESISTED DURING THE BRACING OPERATION FOR THE TILT-UP PANELS. THE LIFTING / BRACING SPECIALTY ENGINEER FOR THIS PROJECT SHALL BE RESPONSIBLE FOR DESIGNING AND NOTIFYING ARCHITECT, ENGINEER AND GENERAL CONTRACTOR OF ANY AND ALL CONDITIONS RELATED TO THE LIFTING AND BRACING WHICH MAY REQUIRE ADDITIONAL SLAB THICKNESS. THIS INFORMATION IS TO BE CLEARLY TRANSMITTED TO ALL PARTIES IN A TIMELY MANNER TO ALLOW
- THE CONTRACTOR SHALL PROVIDE PANEL SHOP DRAWINGS THAT ACCURATELY SHOW THE LOCATION OF ALL EMBEDDED ITEMS, (I.E. PLATES, ANGLES, ETC.) THE LOCATION OF ALL OPENINGS AND THE CORRESPONDING DIMENSIONS. NO PANEL WORK SHALL BE PERFORMED WITHOUT APPROVED SHOP DRAWINGS.

FOR ANY AND ALL ADDITIONS / MODIFICATIONS TO BE INSTALLED.

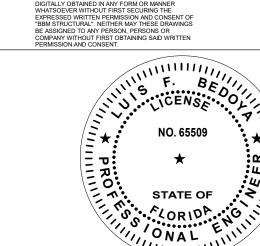
- TUP7 MISCELLANEOUS OPENINGS MAY BE REQUIRED FOR FIRE LINES, PLUMBING, SANITARY LINES, ELECTRICAL CONDUITS, ETC. CORE DRILLING AFTER ERECTION OF THE PANELS MUST HAVE THE APPROVAL OF THE ARCHITECT AND ENGINEER PRIOR TO PERFORMANCE OF THE WORK.
- THE REINFORCING STEEL SUPPLIER SHALL PROVIDE SHOP DRAWINGS INDICATING ALL THE NECESSARY INFORMATION REQUIRED TO ACCURATELY POSITION THE REBAR AS INDICATED, INCLUDING ANY ADDITIONAL REINFORCING AS REQUIRED BY SPECIALTY LIFTING/BRACING ENGINEERING AS NOTED IN TUP5 ABOVE. ENSURE CHAIRS. BOLSTERS OR OTHER MEANS OF SUPPORTING REBARS ARE PROVIDED AND ACCURATELY DETAILED. ALL REINFORCING BARS SHALL HAVE 48 BAR DIAMETER LAP.
- TUP9 ALL REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE ACCURATELY POSITIONED AND ADEQUATELY SECURED PRIOR TO PLACEMENT OF CONCRETE DO NOT WET-SET ITEMS AS THE CONCRETE IS PLACED
- TUP10 THE TILT-UP CONTRACTOR/SUBCONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE TEMPORARY PANEL BRACING AND THE SAFETY OF THE WORKERS DURING BRACING INSTALLATION AND REMOVAL. BRACING REMOVAL SHALL ONLY OCCUR AFTER ALL STRUCTURAL MEMBERS ARE INSTALLED, FULLY SECURED AND INSPECTED.
- TUP11 TILT-UP PANELS SHALL BE CONNECTED TOGETHER AT CORNERS AND AT INTERSECTIONS PER DETAILS SHOWN ON SHEET S501.

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ISSUE DATE: 12/16/2022 COMM. NO.: 2022107

DRAWN BY: NDM CHECKED BY: LB / JMP

STRUCTURAL GENERAL

CONCRETE AND REINFORCING

- A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD TESTING INCLUDING SLUMP TESTS AND CYLINDER BREAKS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
- CONCRETE WORK SHALL CONFORM TO ACI 318-14 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE.
- ALL CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES:

LOCATION	28 DAY STRENGTH	SLUMP	COARSE AGGREGATE(S)	COMMENTS	CA CL AC	ASSE 1318	ORIES ES (PE	ĒR
					F	5	M	C
FOUNDATIONS	3000 PSI	4" +/- 1"	1"		FO	50	MO	co
SLAB-ON-GRADE	3000 PSI	4" +/- 1"	1"		FO	50	MO	co
TIE BEAMS AND TIE COLUMNS (MAX 50' BETWEEN COLD JOINTS)	3000 PSI	6" +/- 1"	3/8"		F0	50	NO	co
BEAMS, COLUMNS & ELEVATED STAIRS	4000 PSI	4" +/- 1"	3/8" \$ 1"		F0	50	MO	C1
TILT-UP PANELS	4000 PSI	4" +/- 1"	3/8" \$ 1"		FO	50	MO	C 1
GROUT UNDER TILT-UP PANELS	6000 PSI	8" TO 11"	NONE		FO	50	MO	CO
ELEVATED SLABS (FORMED)	4000 PSI	4" +/- 1"	3/8" \$ 1"		FO	50	MO	CO
ELEVATED SLABS (MTL DECK)	3000 PSI	4" +/- 1"	3/8" \$ 1"		F0	50	MO	CO
TOPPINGS	4000 PSI	7" +/- 1"	3/8"	ADD 1 1/2 gal/CY OF TETRAGUARD OR ECLIPSE SHRINKAGE CONTROL ADDITIVE	F0	50	MO	C 1
FILLED CELL, PRECAST LINTELS & BOND BEAM GROUT (ASTM C476) - SEE NOTE 7	2500 PSI	8" TO 11"	COARSE GROUT: 3/8" FINE GROUT: NONE		FO	50	MO	co

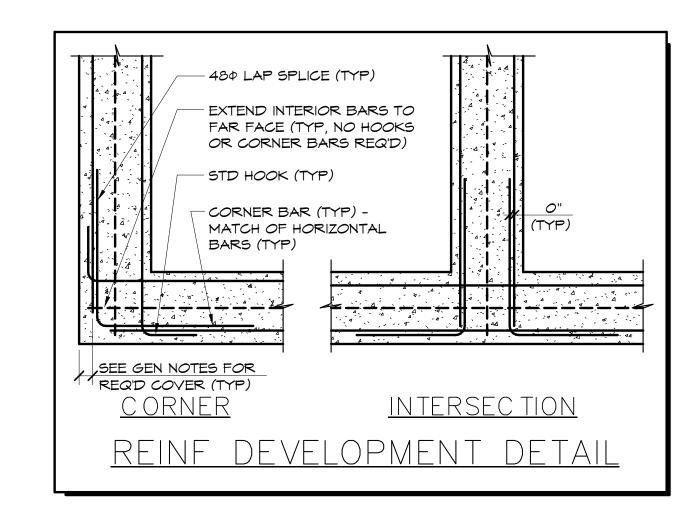
SLUMP FOR RAMPS AND SLOPING SURFACES SHALL NOT EXCEED 4".

- 2. ALL CONC MIXES SHALL HAVE A MAXIMUM SAND TO TOTAL AGGREGATE RATIO OF 0.50.
- 3. A 2" OR 3" PUMP SHALL BE ACCEPTABLE FOR COLUMNS, CELL FILL AND TIE BEAMS BUT WILL NOT BE ALLOWED FOR FOUNDATIONS, SLABS, TILT-UP PANELS AND CONCRETE BEAMS.
- . READY MIX SUPPLIER SHALL DESIGN THE MIXES THAT CONTAIN MULTIPLE AGGREGATES TO BE WELL GRADATED.
- 5. SLABS SHALL NOT BE AIR ENTRAINED.
- 6. FOR SLABS THAT SHALL RECEIVE MOISTURE SENSITIVE FLOORING:
- a. CONTRACTOR SHALL WORK WITH THE READY MIX SUPPLIER TO PROVIDE A MIX DESIGN THAT WILL BE AT OR BELOW 75% RELATIVE HUMIDITY AT THE TIME THE FLOORING IS SCHEDULED TO BE INSTALLED.
- b. DO NOT USE LIGHTWEIGHT AGGREGATES.
- C. PROVIDE A MIX WITH GOOD SELF-DESICCATING PROPERTIES. CONSIDER ADDING 2%-4% SILICA
- DO NOT HARD TROWEL THE SURFACE BUT INSTEAD PROVIDE A LIGHTLY TROWELLED SURFACE. SEE MASONRY NOTE M20 FOR TESTING REQUIREMENTS OF GROUT TO BE USED TO FILL CORES

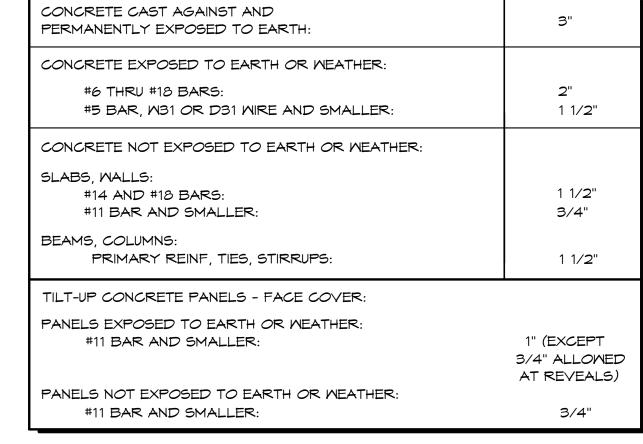
CONCRETE MIX DESIGN SUBMITTALS:

OF CMU.

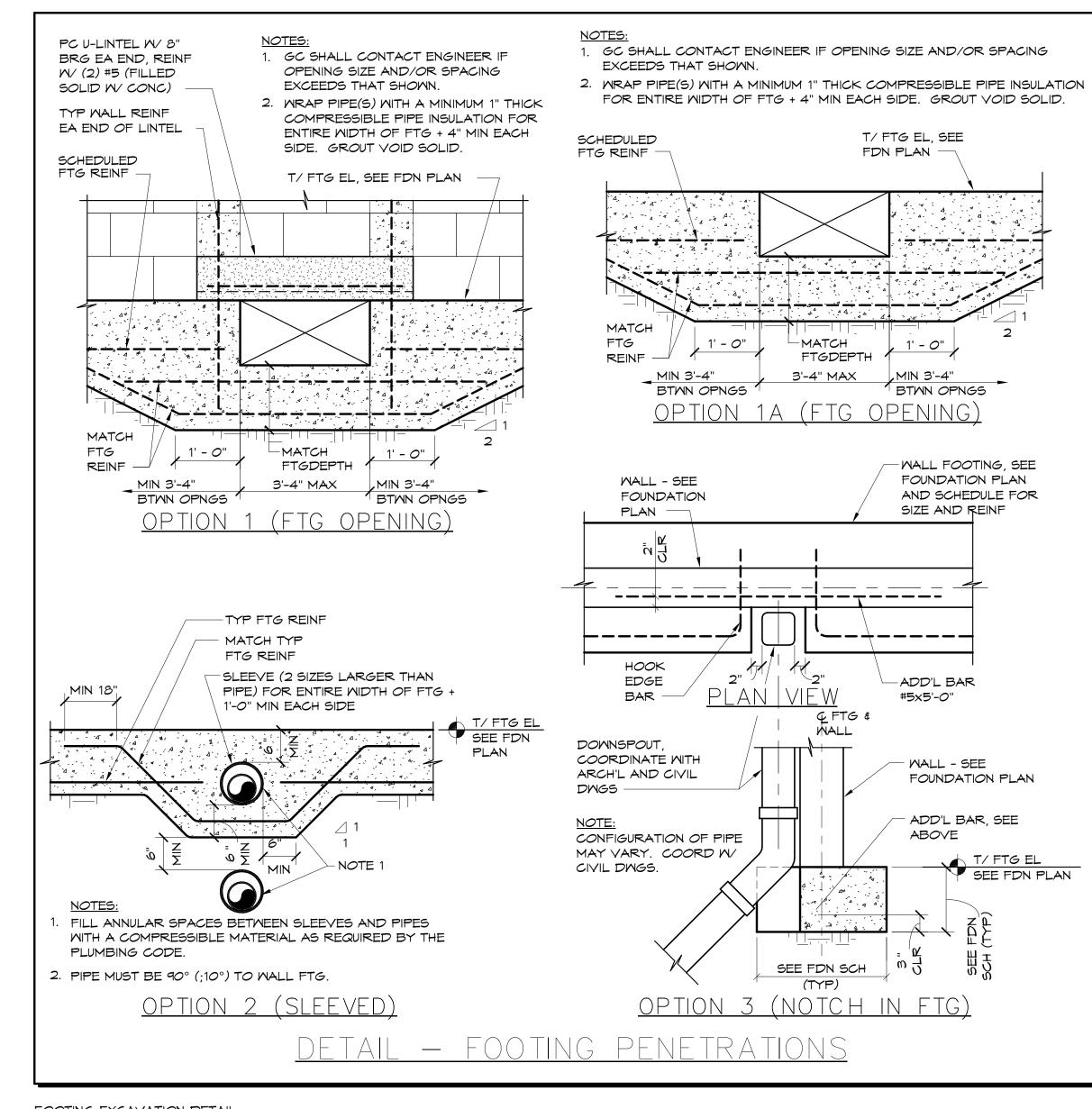
- 1. EACH MIX DESIGN SHALL BE LABELED TO INDICATE THE AREA IN WHICH THE CONCRETE IS TO BE PLACED (I.E. FOUNDATIONS, SLAB- ON-GRADE, COLUMNS, ETC.). FAILURE TO DO SO WILL CAUSE DELAY AND/OR REJECTION OF SUBMITTALS.
- 2. PROPOSED MIX DESIGN SHALL BE IN ACCORDANCE WITH METHOD 1 OR METHOD 2 OF ACI 301. PROVIDE SUPPORTING DATA IN TABULAR FORM FOR EACH SEPARATE PROPOSED MIX.
- 3. SUBMIT CONCRETE MIX DESIGN FOR EACH PROPOSED CLASS OF CONCRETE.
- REBAR SHALL CONFORM TO ASTM A615 GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064 AND SHALL BE LAPPED MINIMUM ONE MESH + 2" WHERE SPLICED. ALL REINFORCING SHALL BE DOMESTICALLY PRODUCED. ALL REBAR THAT IS TO BE WELDED SHALL BE LOW ALLOY ASTM A706 GRADE 60.
- SPLICES AND ANCHORAGE OF REINFORCING SHALL BE AS FOLLOWS (UNLESS OTHERWISE NOTED):
 - MELDED WIRE FABRIC: 8" ALL OTHER:
 - #6 BAR & SMALLER: 48 DIA (12" MIN) #7 BAR & LARGER: 60 DIA
- REINFORCEMENT IN WALLS, FOOTINGS AND BEAMS SHALL BE CONTINUOUS AND LAPPED 48 BAR DIA AT SPLICE UNLESS OTHERWISE NOTED. HOOK AND LAP ALL CORNER AND INTERSECTING BARS. (SEE REINF DEVELOPMENT DETAIL).



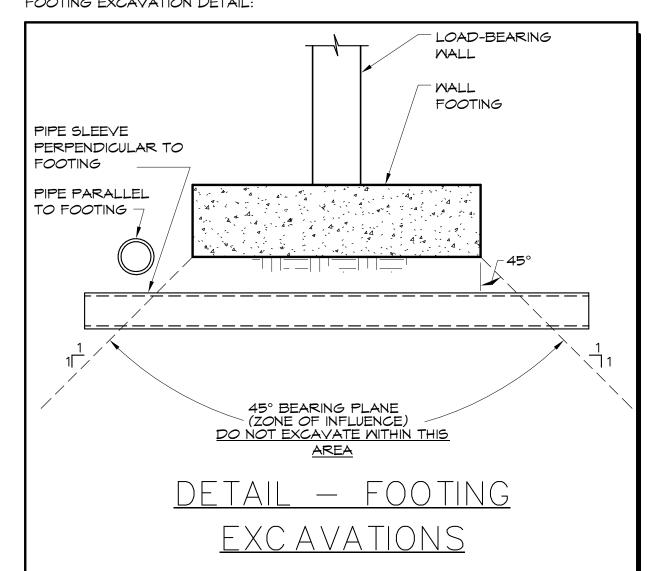
COVER FOR REINFORCING SHALL BE AS FOLLOWS:



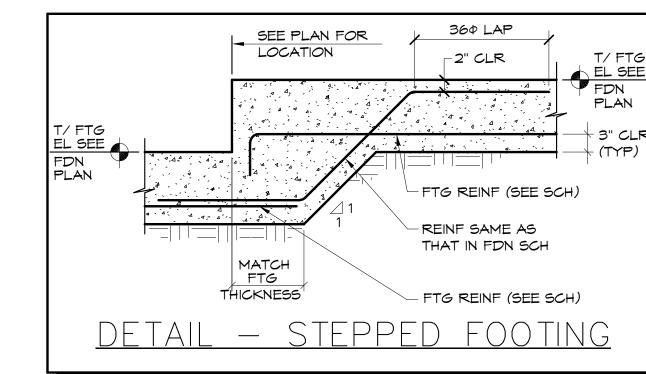
FOOTING PENETRATION DETAILS:



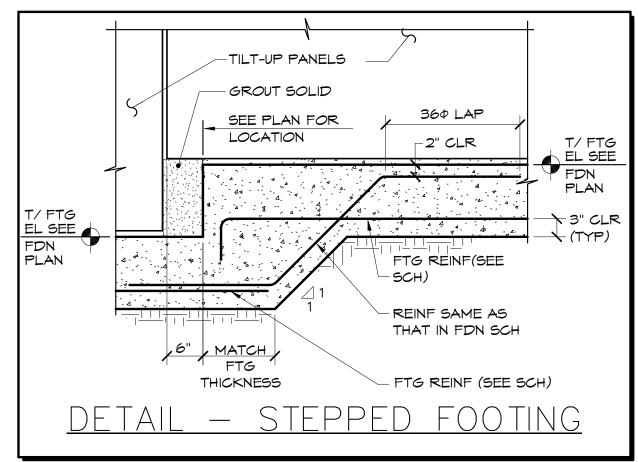
C10 FOOTING EXCAVATION DETAIL:



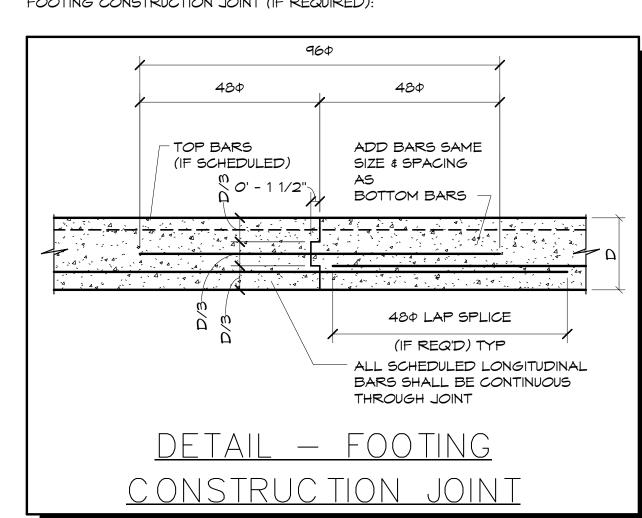
STEPPED FOOTING DETAIL (IF REQUIRED):



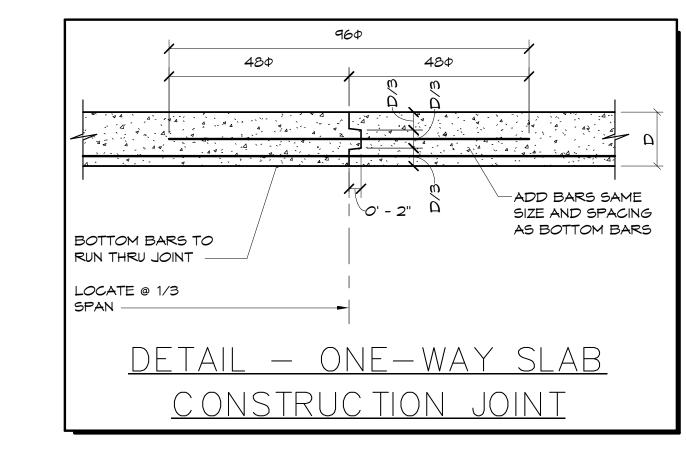
C11A STEPPED FOOTING DETAIL (IF REQUIRED):



- TERMINATE ALL DISCONTINUED ELEVATED SLAB TOP BARS WITH A 180 DEGREE STANDARD HOOK UNLESS OTHERWISE NOTED.
- CONTINUOUS TOP BARS SHALL BE SPLICED AT MID-SPAN. CONTINUOUS BOTTOM BARS SHALL BE SPLICED AT CENTER-LINE OF SUPPORTS (OR AS SHOWN ON TYPICAL DETAILS).
- C14 FOOTING CONSTRUCTION JOINT (IF REQUIRED):



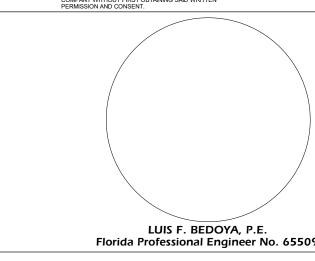
C15 ONE-WAY SLAB CONSTRUCTION JOINT (IF REQUIRED):



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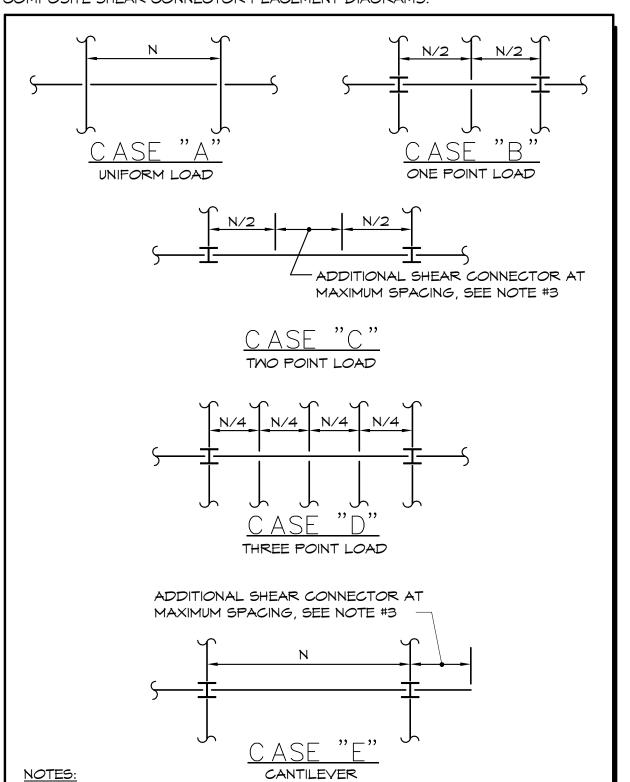
STRUCTURAL GENERAL NOTES

COMPOSITE STEEL FLOOR SYSTEM

COMPOSITE BEAM CONNECTIONS SHALL BE PROVIDED AS DETAILED

C52 SHEAR CONNECTOR SIZE AND QUANTITIES ARE SHOWN IN THE FLOOR FRAMING PLAN NOTES. THE CONTRACTOR SHALL SUBMIT STUD PLACEMENT SHOP DRAWINGS. THE TOP OF THE STUDS MUST BE A MINIMUM OF 1 1/2" ABOVE THE FLUTES OF THE METAL DECK BUT NOT SO HIGH AS TO EXTEND ABOVE THE SLAB SURFACE AFTER THE SLAB HAS BEEN CAST AND PRE-COMPOSITE DEFLECTIONS HAVE OCCURRED. THE LENGTH OF THE STUD INDICATED IN THE FLOOR FRAMING PLAN NOTES IS THE IN-PLACE LENGTH AFTER WELDING.

COMPOSITE SHEAR CONNECTOR PLACEMENT DIAGRAMS:



. N = SPECIFIED NUMBER OF SHEAR CONNECTORS. REFER TO FRAMING PLAN(S). 2. UNLESS NOTED OTHERWISE ON PLANS OR IN THE COMPOSITE

BEAM SCHEDULE, SHEAR CONNECTORS SHALL BE DISTRIBUTED ALONG THE LENGTH OF THE BEAM AS SHOWN ON DETAILS ABOVE. MAXIMUM SPACING OF SHEAR CONNECTOR SHALL BE AS FOLLOWS:

A. BEAMS PERPENDICULAR TO DECK SPAN = 36"

B. BEAMS PARALLEL TO DECK SPAN = (8 x TOTAL SLAB THICKNESS)

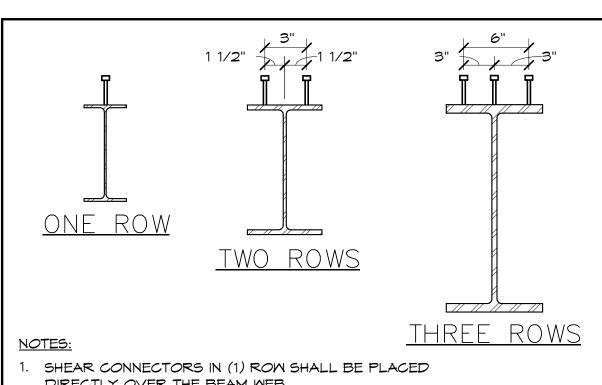
MINIMUM SPACING OF SHEAR CONNECTOR SHALL BE AS FOLLOMS:

A. BEAMS PERPENDICULAR TO DECK SPAN = 3"

B. BEAMS PARALLEL TO DECK SPAN = 4 1/2" WHERE STEEL DECK CORRUGATIONS DO NOT ALLOW FOR AN EVEN SPACING OF SHEAR CONNECTORS WITH (1) STUD IN EACH FLUTE, ADDITIONAL STUDS IN A SECOND ROW (AND THIRD ROW WHERE REQUIRED) SHALL BE PLACED SUCH THAT THE HIGHEST DENSITY OF SHEAR CONNECTORS OCCURS NEAR THE BEAM SUPPORT. WHERE THE SPECIFIED NUMBER OF SHEAR CONNECTOR IS LESS THAN THE BEAM SPAN LENGTH DIVIDED BY THE MAXIMUM SPACING (SEE NOTE 3) ADDITIONAL SHEAR CONNECTORS SHALL BE PROVIDED SUCH THAT THE MAXIMUM SPACING IS NOT EXCEEDED AT ANY LOCATION IN THE SPAN.

SUBMIT SHOP DRAWINGS SHOWING PLACEMENT OF SHEAR CONNECTORS TO ARCHITECT FOR ENGINEER'S APPROVAL.

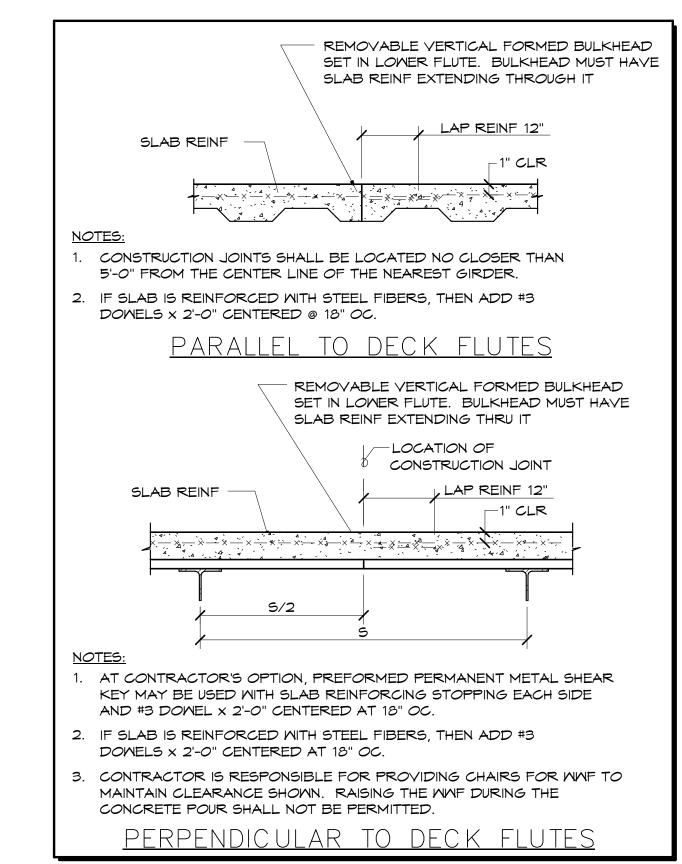
COMPOSITE SHEAR CONNECTOR SPACING DETAIL



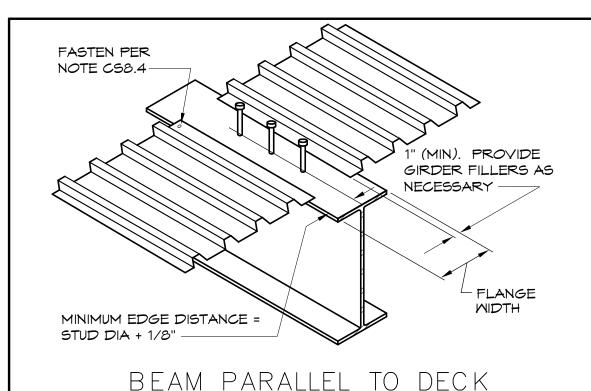
DIRECTLY OVER THE BEAM MEB.

PLACE STUD IN A SINGLE ROW WHERE SPACING REQUIREMENTS PERMIT STUDS SHALL BE PLACED IN (2) OR (3) ROMS ONLY WHERE REQUIRED IN ORDER TO PLACE THE TOTAL NUMBER OF STUDS. SEE TYPICAL DETAIL, SHEAR CONNECTOR PLACEMENT DIAGRAMS.

COMPOSITE SLAB CONSTRUCTION DETAIL DIAGRAMS:



COMPOSITE FLOOR DECK PLACEMENT DETAILS:



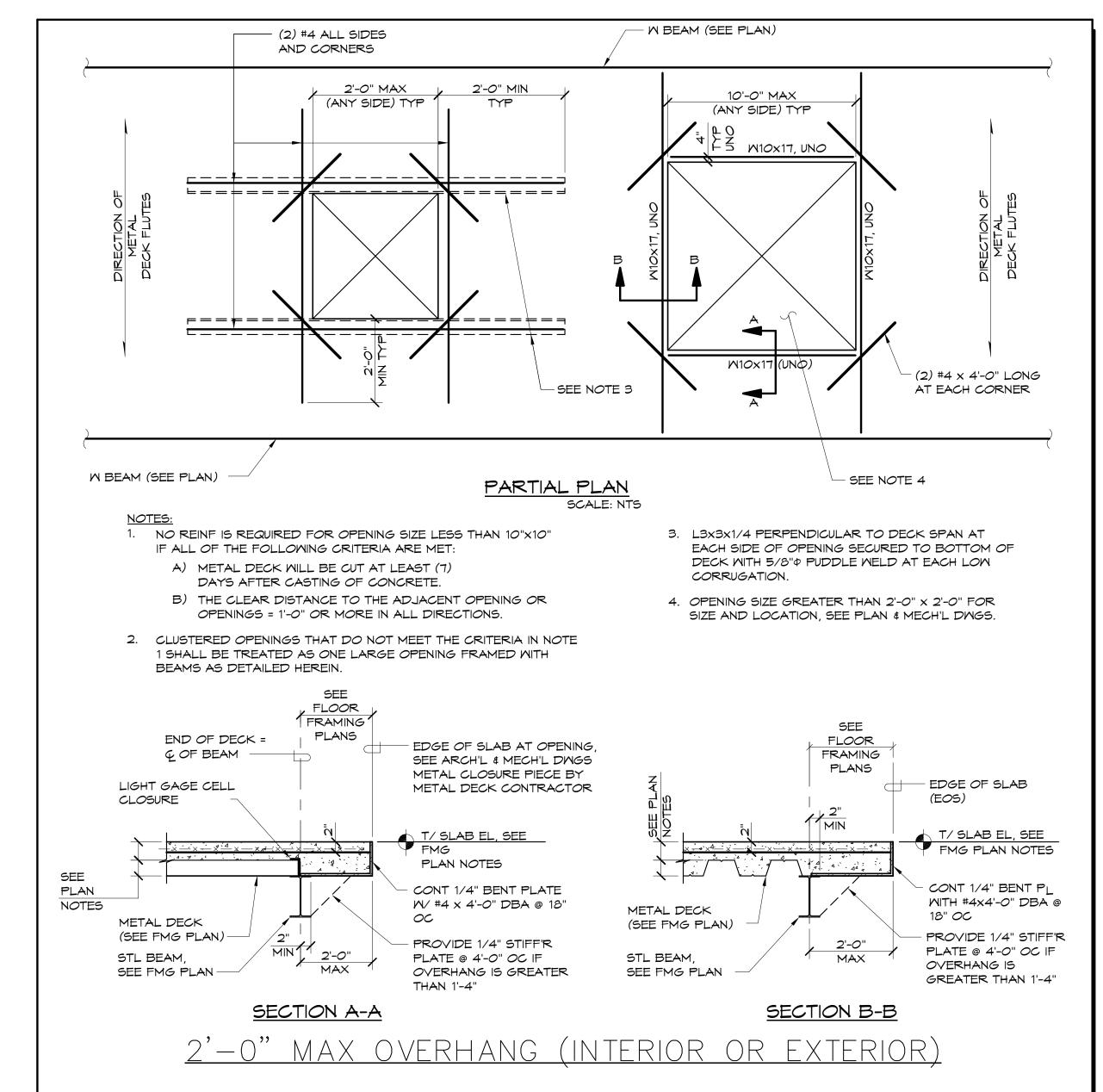
FLUTE (GIRDER BEAM) CLOSURES NOT REQUIRED FOR GAP OF 1/2" OR LESS 0" TO (FLG MIDTH - 3") BEAM PERPENDICULAR TO DECK FLUTE (FILLER BEAM)

NOTES: DECK SHALL BE FURNISHED IN MODULAR PANEL WIDTHS. ANY CUTTING THAT IS REQUIRED SHALL BE DONE BY THE DECK ERECTOR.

LATERAL AND LONGITUDINAL SPACING IS CONTROLLED BY SECTION 1.11.4 OF THE A.I.S.C.E. SPECIFICATIONS. FOR THE LOCATION OF THE STUD WITH RESPECT TO THE EDGE OF THE FLANGE, SEE THE STRUCTURAL MELDING CODE A.M.S., 428.8. (DETAIL 4).

WHEN STUDS ARE USED, DECK MUST NOT BE LAPPED IN ORDER TO PERMIT WELDING OF STUDS THROUGH SINGLE THICKNESS OF DECK. WELDING OF STUDS TO BEAM THROUGH (2) LAYERS OF DECK SHALL NOT BE PERMITTED.

PARTIAL TYPICAL FRAMING PLAN AT OPENINGS IN COMPOSITE SLAB:



COMPOSITE FLOOR DECK INSTALLATION:

1. INSTALL TEMPORARY SHORING, IF REQUIRED, BEFORE PLACING DECK PANELS.

2. PLACE DECK PANELS ON STRUCTURAL SUPPORTS AND ADJUST TO FINAL POSITION WITH ENDS ALIGNED. ATTACH FIRMLY TO THE SUPPORTS IMMEDIATELY AFTER PLACEMENT IN ORDER TO FORM A SAFE WORKING PLATFORM.

3. CUT AND NEATLY FIT DECK UNITS AND ACCESSORIES AROUND OPENINGS AND OTHER WORK PROJECTING THROUGH OR ADJACENT TO THE DECKING.

4. ANCHOR FLOOR DECK UNITS TO STEEL SUPPORTING MEMBERS BY HILTI X-ENP-19 L15 POWDER ACTUATED FASTENERS, WITH A FASTENER PLACED IN EACH DECK FLUTE CORRUGATION, AND AT 12" ON CENTER AT DECK EDGES. GC OPTION TO SUBSTITUTE ARC SPOT PUDDLE WELDS OF THE FOLLOWING DIAMETER AND SPACING (OR FILLET WELDS OF EQUAL STRENGTH) IN LIEU OF HILTI FASTENERS:

> A. MINIMUM VISIBLE WELD DIAMETER = 5/8" AT EACH CORRUGATION.

B. WELD EDGE RIBS OF PANELS AT EACH SUPPORT. SPACE ADDITIONAL WELDS AN AVERAGE OF 12" OC BUT NOT MORE THAN 18" OC IN ANY ONE LOCATION.

C. FASTEN SIDE LAPS AND PERIMETER EDGE OF UNITS BETWEEN SUPPORTS AT INTERVALS NOT EXCEEDING 24" OC. USING ONE OF THE FOLLOWING METHODS:

1. #10 SELF-DRILLING SCREMS

2. CRIMP OR BUTTON PUNCH

3. FOR DECKS THAT ARE 20ga AND HEAVIER: ARC PUDDLE WELDS 5/8" MINIMUM VISIBLE DIAMETER OR 1" LONG FILLET WELDS.

5. INSTALL DECK ENDS OVER SUPPORTS WITH A MINIMUM END BEARING OF 1 1/2".

6. FASTEN POUR STOPS AND GIRDER FILLERS TO SUPPORTING STRUCTURE ACCORDING TO THE MFR'S RECOMMENDATIONS.

7. FASTEN COLUMN CLOSURES, CELL CLOSURES AND Z CLOSURES TO DECK TO PROVIDE TIGHT FITTING CLOSURES AT OPEN ENDS OF RIBS AND SIDES OF DECKING. FASTEN CELL CLOSURES AT CHANGES OF DIRECTION OF FLOOR DECK UNITS.

COMPOSITE STEEL FLOOR UNITS SHALL BE FABRICATED FROM STEEL CONFORMING TO SECTION A3 OF THE LATEST EDITION OF THE AMERICAN IRON AND STEEL INSTITUTE, SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS. THE STEEL USED SHALL HAVE A MINIMUM YIELD STRENGTH OF 50 KSI (345 MPa). SEE CHART BELOW FOR MINIMUM SECTION PROPERTIES REQUIRED FOR STEEL DECK. PROPERTIES SHOWN ARE REPRODUCED FROM THE YULCRAFT MANUAL.

		COMPOSI	TE FLOOR DECK				
			'.	ا خ		Sn 3 in/f	
20/1120 00358 0409 0341 0406	2.0VL 22	0.0295	0.324	0.263	0.321	0.26	
2.0 4 [120 0.0000 0.404 0.041 0.400	2.0VL120	0.0358	0.409	0.341	0.406	0.34	
2.0VLI18 0.0474 0.559 0.495 0.558	2.0VL 18	0.0474	0.559	0.495	0.558	0.50	

ALL FIELD WELDING OF DECK SHALL BE IN STRICT CONFORMANCE WITH ANSI/AMSD1.3 STRUCTURAL MELDING

GALVANIZING SHALL CONFORM TO ASTM-A653, STRUCTURAL QUALITY AND FEDERAL SPEC. QQ-S-775 TO PROVIDE A MINIMUM COATING PROTECTION OF 690.

COMPOSITE BEAMS SHALL BE CAMBERED AS INDICATED ON THE FLOOR FRAMING PLAN(S). IF NO CAMBER IS SPECIFIED, THE FABRICATOR SHALL ENSURE THAT THE "NATURAL" CAMBER IN ALL ERECTED BEAMS OCCURS IN AN UPWARD DIRECTION.

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2300 Maitland Center Parkway

LUIS F. BEDOYA, P.E.

DATE

Florida Professional Engineer No. 6550

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Port St. Lucie, FL

PUBLIC SCHOOLS

St. Lucie Public Schools 9461 Brandywine Ln Port St. Lucie, FL 34986

ISSUE DATE: 12/16/2022 COMM. NO.: 2022107

CHECKED BY: LB / JMP DRAWN BY: NDM

STRUCTURAL GENERAL

STRUCTURAL PRECAST CONCRETE

GENERAL CONTRACTOR SHALL ENGAGE A CERTIFIED TESTING AGENCY TO PERFORM INDUSTRY STANDARD TESTING INCLUDING SLUMP TESTS AND CYLINDER BREAKS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.

SUBMIT SIGNED & SEALED SHOP DRAWINGS AND CALCULATIONS PREPARED BY OR UNDER THE DIRECT SUPERVISION OF A QUALIFIED PROFESSIONAL ENGINEER DETAILING FABRICATION AND INSTALLATION OF PRECAST CONCRETE UNITS. INDICATE MEMBER DIMENSIONS AND CROSS-SECTIONS, LOCATIONS, SIZES AND TYPES OF REINFORCEMENT, INCLUDING SPECIAL REINFORCEMENT AND LIFTING DEVICES NECESSARY FOR HANDLING AND ERECTION, AND INDICATE REACTIONS AT ALL PERMANENT SUPPORT LOCATIONS.

INDICATE LAYOUT AND DIMENSIONS, AND IDENTIFY EACH PRECAST CONCRETE UNIT CORRESPONDING TO SEQUENCE AND PROCEDURE OF INSTALLATION. INDICATE WELDED CONNECTIONS BY AWS STANDARD SYMBOLS. DETAIL LOOSE, CAST-IN, FIELD HARDWARE, INSERTS, CONNECTIONS AND JOINTS, INCLUDING ACCESSORIES AND CONSTRUCTION AT OPENINGS IN PRECAST UNITS. FURNISH TEMPLATES, IF REQUIRED, FOR ACCURATE PLACEMENT. DELIVER ANCHORAGE ITEMS THAT ARE TO BE EMBEDDED IN OTHER CONSTRUCTION BEFORE STARTING SUCH WORK. PROVIDE SETTING DIAGRAMS, TEMPLATES, INSTRUCTIONS AND

DIRECTIONS, AS REQUIRED, FOR INSTALLATION. FOR PRECAST CONCRETE UNITS INDICATED TO COMPLY WITH DESIGN LOADINGS OR CALCULATED FIRE-RESISTANCE REQUIREMENTS, INCLUDE STRUCTURAL ANALYSIS DATA SEALED AND SIGNED BY A QUALIFIED PROFESSIONAL ENGINEER LEGALLY AUTHORIZED TO PRACTICE IN THE JURISDICTION WHERE PROJECT IS LOCATED AND EXPERIENCED IN

PCI DESIGN STANDARD: COMPLY WITH RECOMMENDATIONS OF PCI MNL-120 "PCI DESIGN HANDBOOK - PRECAST AND PRESTRESSED

PROVIDING ENGINEERING SERVICES OF THE KIND INDICATED.

CONCRETE' PCI QUALITY-CONTROL STANDARD: COMPLY WITH REQUIREMENTS OF PCI MNL-116 "MANUAL FOR QUALITY CONTROL FOR PLANTS AND PRODUCTION OF PRECAST AND PRESTRESSED CONCRETE PRODUCTS," INCLUDING MANUFACTURING AND TESTING PROCEDURES, QUALITY- CONTROL RECOMMENDATIONS, CAMBER AND DIMENSIONAL TOLERANCES FOR TYPES OF

COMPLY WITH THE FOLLOWING ACI PUBLICATIONS APPLICABLE TO TYPES

OF STRUCTURAL PRECAST CONCRETE UNITS INDICATED: A. ACI 301 "SPECIFICATIONS FOR STRUCTURAL

CONCRETE FOR BUILDINGS" B ACI 318 (ACI 318M) "BUILDING CODE REQUIREMENTS

FOR REINFORCED CONCRETE" PCP8 COMPLY WITH APPLICABLE PROVISIONS OF AWS D1.1 "STRUCTURAL WELDING

STEEL". USE QUALIFIED AND CERTIFIED WELDERS. PCP9 WHEN FIRE-RESISTANCE-RATED ASSEMBLIES ARE INDICATED, PROVIDE STRUCTURAL PRECAST CONCRETE UNITS WHOSE CALCULATED FIRE RESISTANCE HAS BEEN DETERMINED ACCORDING TO ASTM E 119 AND PCI MNL-124 "DESIGN FOR FIRE RESISTANCE OF PRECAST PRESTRESSED CONCRETE" AND IS

CODE - STEEL" AND AMS D1.4 "STRUCTURAL MELDING CODE - REINFORCING

ACCEPTABLE TO AUTHORITIES HAVING JURISDICTION. PRODUCTS SHALL BE INDENTIFIED WITH APPROPRIATE MARKINGS OF APPLICABLE TESTING AND INSPECTING AGENCY.

PCP10 LIFT AND SUPPORT UNITS ONLY AT DESIGNATED LIFTING OR SUPPORTING POINTS AS SHOWN ON FINAL SHOP DRAWINGS.

PCP11 MATERIALS:

REINFORCING BARS: ASTM A615, GRADE 60 (ASTM A615M, GRADE 400),

STEEL-WELDED WIRE FABRIC: ASTM A185, PLAIN, COLD-DRAWN. STEEL SHAPES AND PLATES: ASTM A36 (ASTM A36M).

BOLTS AND STUDS: ASTM A307, GRADE A (ASTM F 568, PROPERTY CLASS 4.6), CARBON-STEEL, HEX-HEAD BOLTS AND STUDS, CARBON-STEEL NUTS AND FLAT, UNHARDENED STEEL WASHERS.

HIGH-STRENGTH BOLTS AND NUTS: ASTM A325 (ASTM A325M), TYPE 1, HEAVY HEX STEEL STRUCTURAL BOLTS, HEAVY HEX CARBON-STEEL NUTS AND HARDENED CARBON-STEEL WASHERS.

WELDED HEADED STUDS: AMS D1.1, TYPE B HEADED STUDS, COLD-FINISHED CARBON-STEEL BARS.

WELDING ELECTRODES: COMPLY WITH AMS STANDARDS. CEMENT GROUT: PORTLAND CEMENT, ASTM C 150, TYPE I, AND CLEAN, NATURAL SAND, ASTM C 404. MIX AT RATIO OF 1 PART CEMENT TO 2-1/2 PARTS SAND, BY VOLUME, WITH MINIMUM WATER REQUIRED FOR PLACEMENT AND HYDRATION.

NON-METALLIC, NON-SHRINK GROUT: PRE-MIXED, NON-METALLIC, NON-CORROSIVE, NON-STAINING GROUT CONTAINING SELECTED SILICA SANDS, PORTLAND CEMENT, SHRINKAGE-COMPENSATING AGENTS, PLASTICIZING AND MATER-REDUCING AGENTS, COMPLYING WITH ASTM C1107, WITH FLUID CONSISTENCY AND A 30-MINUTE WORKING TIME. EPOXY GROUT: ASTM C 881, 2-COMPONENT EPOXY RESIN, OF

TYPE, GRADE, AND CLASS TO SUIT REQUIREMENTS.

NORMAL-WEIGHT CONCRETE: PROPORTION MIXES BY EITHER LABORATORY TRIAL BATCH OR FIELD TEST DATA METHODS ACCORDING TO ACI 211.1 AND ACI 301, USING MATERIALS TO BE USED ON THE PROJECT, TO PROVIDE NORMAL-WEIGHT CONCRETE WITH THE FOLLOWING PROPERTIES: CONCRETE SHALL HAVE COMPRESSIVE STRENGTH (28-DAY) OF 4000 PSI FOR CAST-IN OPENINGS LARGER THAN 10 INCHES IN DIAMETER OR 10 INCHES SQUARE ACCORDING TO FINAL SHOP DRAWINGS. OTHER SMALLER HOLES MAY BE FIELD CUT BY TRADES REQUIRING THEM, AS ACCEPTABLE TO ARCHITECT.

PCP12 REPAIR DAMAGED GALVANIZED METAL SURFACES BY CLEANING AND APPLYING A COAT OF GALVANIZING REPAIR PAINT TO GALVANIZED

PCP13 REPAIR DAMAGED PAINTED METAL SURFACES BY CLEANING AND RE-PRIMING DAMAGED PAINTED SURFACES.

PCP14 ERECTION TOLERANCES: INSTALL PRECAST UNITS LEVEL, PLUMB, SQUARE AND TRUE, WITHOUT EXCEEDING THE RECOMMENDED ERECTION TOLERANCES OF PCI MNL-127 "RECOMMENDED PRACTICE FOR ERECTION OF PRECAST

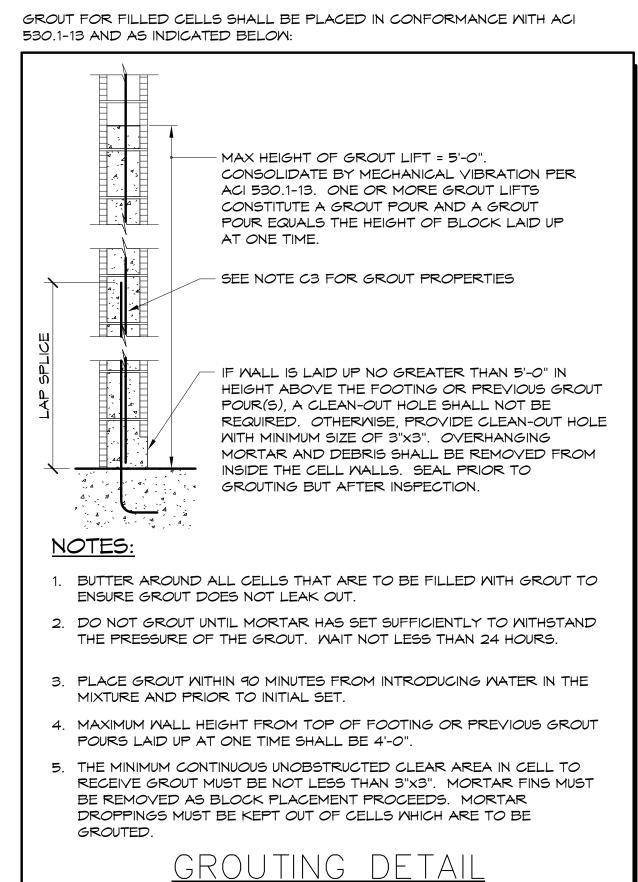
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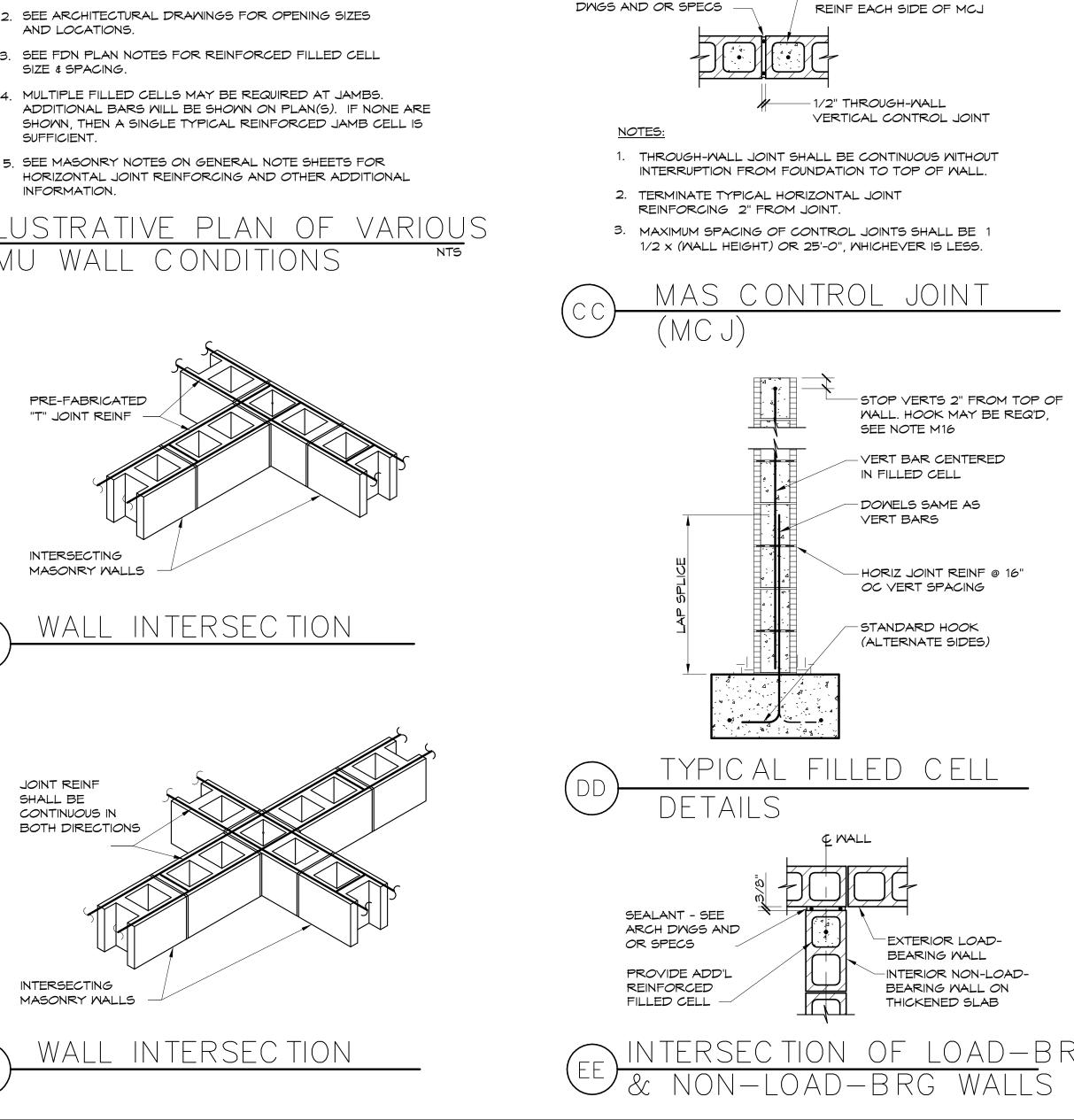
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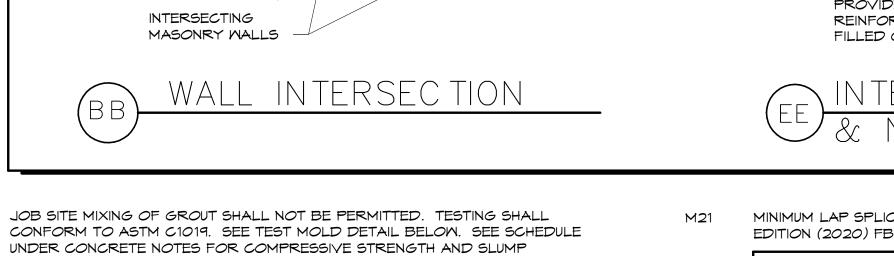
MASONRY

- MASONRY CONSTRUCTION MATERIALS AND INSPECTIONS SHALL CONFORM TO THE LATEST EDITION OF THE ACI BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES (ACI 530-13, ASCE 5-13, TMS 402-2016), SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530.1-13, ASCE 6-13, TMS 602-2016) ASTM C476-02, ASTM C1019-09 AND NCMA TEK 107.
- CONCRETE BLOCKS SHALL CONFORM TO ASTM C-90. (F'm = 1500 PSI) (1900 PSI ON THE NET AREA).
- MORTAR SHALL COMPLY WITH ASTM C270, TYPE 'M' FOR RETAINING WALLS AND WALLS BELOW GRADE, TYPE 'S' FOR TYPICAL WALLS. (COMPRESSIVE STRENGTH = 2500 PSI AND 1800 PSI, RESPECTIVELY. SITE TESTED MORTAR CUBES SHALL ACHIEVE A MINIMUM OF 80% OF THE DESIGN COMPRESSIVE STRENGTH)
- BLOCK SHALL NOT BE MOISTENED BEFORE GROUTING.
- ALL MASONRY CROSS WEBS SHALL BE FULLY BEDDED IN MORTAR AROUND CELLS TO BE GROUTED.
- REINFORCE WALLS WITH LADDER TYPE (ASTM A-153, #9 GAGE WIRE) DEFORMED REINFORCEMENT EQUAL TO DUR-O-WAL IN BED JOINTS AT 16" OC UNO, MEASURED VERTICALLY. PLACE PER MFR'S INSTRUCTIONS. LAP ALL HORIZONTAL JOINT REINFORCING 6" MIN.
- VERTICAL REINFORCING MUST HAVE A MINIMUM CLEARANCE OF 1/2" TO INSIDE FACE. VERTICAL REINFORCEMENT IN WALLS SHALL BE SECURED AND LATERALLY SUPPORTED AGAINST DISPLACEMENT AT INTERVALS NOT EXCEEDING 192 x (BAR DIAMETER) OR 10 FT (WHICHEVER IS LESS) WHENEVER A CLEANOUT IS REQUIRED. SEE GROUTING DETAIL NOTE FOR CLEANOUT REQUIREMENTS.
- GROUT PLACEMENT STOPPED FOR (1) HOUR OR MORE SHOULD BE STOPPED 1 1/2" BELOW THE TOP OF THE MASONRY UNIT TO PROVIDE A KEY FOR SUBSEQUENT GROUTING.
- TYPICAL VERTICAL REINFORCING SIZE AND SPACING SHALL BE ABOVE AND BELOW ALL WALL OPENINGS.
- TEMPORARY BRACING AND SHORING OF WALLS TO PROVIDE STABILITY DURING CONSTRUCTION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- REINFORCE MASONRY OPENINGS LESS THAN 2'-O" WIDE, WITH HORIZ JOINT REINF PLACED IN (2) HORIZ JOINTS APPROXIMATELY 8" APART, IMMEDIATELY ABOVE THE GROUT FILLED LINTEL AND IMMEDIATELY BELOW THE GROUT FILLED SILL. EXTEND REINFORCING A MINIMUM OF 2'-O" BEYOND JAMBS OF THE OPENING EXCEPT AT CONTROL JOINTS.
- PROVIDE FILLED PRECAST U-LINTELS AS MANUFACTURED BY CAST- CRETE OR APPROVED EQUAL WITH (2) #5 CONT AT ALL OPENINGS WHERE BEAMS ARE NOT SHOWN, SCHEDULED OR NOTED GREATER THAN 2'-O" MIDE. LINTELS SHALL HAVE MINIMUM UNFILLED CAPACITY OF 400 Ib/LF AND BEAR NOMINAL 6" (MIN 6") EACH END ON A GROUT FILLED CELL. PROVIDE PRE-CAST LINTEL MFR'S STANDARD TABULATED LOAD TABLES AS EVIDENCE THAT THE MINIMUM CAPACITIES AS LISTED IN THE BEAM SCHEDULE ARE SATISFIED. REFER TO MASONRY WALL BEAM SCHEDULE FOR TYPICAL PRECAST LINTEL SPANS AND DETAILS.
- STOPPING AND RESUMING WORK: RACK BACK 1/2-UNIT LENGTH IN EACH COURSE. DO NOT TOOTH. CLEAN EXPOSED SURFACES OF SET MASONRY. REMOVE LOOSE MASONRY UNITS AND MORTAR PRIOR TO LAYING FRESH MASONRY.
- DO NOT APPLY UNIFORM LOADS TO MASONRY WALLS FOR (3) DAYS.
- DO NOT APPLY CONCENTRATED LOADS TO MASONRY WALLS FOR (7)
- EXTEND ALL VERTICAL WALL REINFORCEMENT TO WITHIN 2" OF TOP OF WALL OR BEAM UNLESS NOTED OTHERWISE. TERMINATE REINFORGING WITH STANDARD ACI 90 DEGREE HOOK IF ROOF JOISTS AND/OR TRUSSES BEAR ON TOP OF WALL AND THERE IS NO PARAPET. IF PARAPET EXISTS, HOOK IS NOT REQUIRED.
- REFER TO ARCHITECTURAL DRAWINGS FOR WATERPROOFING DETAILS AT MASONRY CONTROL JOINTS.
- GROUT FOR FILLED CELLS SHALL BE PLACED IN CONFORMANCE WITH ACI



TYPICAL MASONRY DETAILS: DOOR OPENING MINDOW OPNG SPACING (SEE NOTE 4) (3) REINF CELLS (SEE NT 4) AT CORNERS, TYP SEE PLANS FOR ADD'L TYP EXTERIOR FILLED CELLS REQ'D LOAD-BEARING CMU AT OPENINGS (VARIES) WALL ON FOOTING TYP INTERIOR LOAD-BEARING CMU WALL ON FOOTING SPACING TYP INTERIOR NON-LOAD-BEARING CMU MALL ON THICKENED SLAB 1. SEE DETAIL "CC" FOR LOCATING MASONRY CONTROL JOINTS. CONTRACTOR SHALL SUBMIT MCJ PLAN TO ARCHITECT FOR APPROVAL. 2. SEE ARCHITECTURAL DRAWINGS FOR OPENING SIZES AND LOCATIONS. 3. SEE FDN PLAN NOTES FOR REINFORCED FILLED CELL SIZE & SPACING. 4. MULTIPLE FILLED CELLS MAY BE REQUIRED AT JAMBS. ADDITIONAL BARS WILL BE SHOWN ON PLAN(S). IF NONE ARE SHOWN, THEN A SINGLE TYPICAL REINFORCED JAMB CELL IS SUFFICIENT. 5. SEE MASONRY NOTES ON GENERAL NOTE SHEETS FOR HORIZONTAL JOINT REINFORGING AND OTHER ADDITIONAL INFORMATION.







3/8"M x 1"D VERT

OF TIE BEAM

BY MIN 2"

(DUMMY) JOINT EACH SIDE

ADJUST STIRRUP LOCATION

SLIGHTLY TO CLEAR JOINT

SEALANT - SEE ARCH

DWGS AND OR SPECS

BOND BEAM REINF

SEALANT - SEE ARCH

CONTINUOUS AT MCJ

MCJ @ TIE BEAM

MCJ @ BOND BEAM

STIRRUPS - SEE

REINF IS CONTINUOUS

CONC TIE BEAM

- FILLED CELL W/ TYP WALL

REINF EACH SIDE OF MCJ

YERTICAL CONTROL JOINT

1/2" THROUGH-WALL

— FILLED CELL WITH TYP WALL

AT CONTROL JOINT IN

SCH FOR SIZE

AND SPACING -

Cl	MU WALL	CMU WALLS WITH <u>CENTERED</u> VERTICAL REINFORCING								
#4	#5	#6	#7	#8	#9 (NOTE 1)	#10 (NOTE 2)				
15"	23"	35"	48"	72"	82"					
CN	ИU WALLS	ICAL REINFOI	RCING							

CMU WALLS WITH <u>EACH FACE</u> VERTICAL REINFORCING										
#4	#5	#6	#7	#8	#9 (NOTE 1)	#10 (NOTE 2)				
23"	36"	54"	63"	72"	82"					

- 1. #9 BARS ARE NOT ALLOWED IN 8" CMU BUT ACCEPTABLE FOR 10" AND 12" CMU. MAXIMUM BAR DIAMETER SHALL NOT EXCEED ONE-EIGHTH OF THE NOMINAL WALL THICKNESS.
- 2. #10 BARS SHALL BE SPLICED USING MECHANICAL CONNECTORS AND SHALL ONLY BE ALLOWED IN 12" CMU.
- 3. EPOXY COATED BARS SHALL NOT BE USED.

ABBREVIATIONS AUTOCLAVED AERATED CONCRETE POUND(S) ANCHOR BOLT LG LONG ABOVE LIVE LOAD LONG LEG BACK-TO-BACK AMERICAN CONCRETE INSTITUTE LONG LEG HORIZONTAL ADDITIONAL ADDL LONG LEG VERTICAL ABOVE FINISH FLOOR LNTL LINTEL AGGREGATE AMERICAN INSTITUTE OF STEEL LONG SLOTTED CONSTRUCTION LOC LOCATION AMERICAN IRON AND STEEL LONG LONGITUDINAL LOW POINT LOAD & RESISTANCE FACTOR LRFD **ALUMINUM** DESIGN ALTERNATE LONG MAY ANCHOR ROD ARCHITECT(URAL) MASONRY ALLOWABLE STRESS DESIGN MATERIAL AMERICAN SOCIETY OF TESTING MAXIMUM MAX MATERIALS MACHINE BOLT AMERICAN WELDING SOCIETY METAL BUILDING MFR BOTTOM OF MASONRY COLUMN BOND BEAM MASONRY CONTROL JT BUILDING MOMENT CONNECTION BLOCKING MONOLITHIC EDGE BLKG BELOW BLM MECHANICAL MEZZ MEZZANINE BOTTOM MONOLITHIC FOOTING BASE PLATE MFR MANUFACTURE(ER) BRIDGING MINIMIM BEARING MISCELLANEOUS BRICK BRK MASONRY OPENING BOTH SIDES MANUFACTURERS PRODUCT BTJ BOLTED TIE JOIST INSTALLATION INSTRUCTIONS BTMN BETWEEN METAL STUD CENTER TO CENTER MONOLITHIC WALL FOOTING CANTILEVER NOT IN CONTRACT CONCRETE BEAM NEAR SIDE CONCRETE COLUMN NOT TO SCALE NTS

OVERALL

ON CENTER

OVERHEAD

OPENING

OPPOSITE

BUILDING

PRECAST

PLATE

PLYMD PLYMOOD

PLACES

PARTITION

REFERENCE

RADIUS

REQUIRE

ROOF

ROOM

RETURN

SCHEDULE

SECTION

SHEET

SIMILAR

SLOPE

SPACE(S)

STANDARD

STIFFENER

STRENGTH

STRUCTURAL

SHORT WAY

TIE BEAM

THICK

TOPPING

TYPICAL

TOP OF

VERTICAL

TIE COLUMN

TEMPERATURE

TRANSVERSE

VERIFY IN FIELD

WALL FOOTING

MORKING POINT

MELDED WIRE FABRIC

WATERSTOP

MITH

SYMMETRICAL

TOP AND BOTTOM

SOUTHERN YELLOW PINE

UNLESS NOTED OTHERWISE

MINDOM OPENING (MASONRY)

SQUARE

SHEAR MALL

SAMOUT JOINT

SLAB ON GRADE

SPECIFICATIONS

STAINLESS STEEL

SHORT SLOTTED

REQUIRED

RADIUS POINT

REINFORCED WITH

STEPPED FOOTING

STEEL DECK INSTITUTE

STEEL JOIST INSTITUTE

SHORT LEG BACK-TO-BACK

OHD

OPNG

REINF

REQ

RF

RTN

SCH

SDI

SECT

SHM

50G

STD

STL

STR

STRL

SYMM

T&B

THK

TYP

VIF

MD

MS

MMF

VERT

TOPG

TB

STIFFR

SPECS

REQ'D

OUTSIDE FACE

FASTENER(S)

PRE-CAST BEAM

PERPENDICULAR

POUNDS PER LINEAR FOOT

POUNDS PER SQUARE FOOT

POUNDS PER SQUARE INCH

REINFORCED CONCRETE

REINFORCE(D) (ING)

OPPOSITE HAND

COLD FORMED METAL FRAMING COLD-FORMED STEEL CFS CAST-IN-PLACE CONSTRUCTION JOINT (OR CONTROL JOINT) CENTERLINE CLEAR(ANCE) CONSTRUCTION MANAGER CONCRETE MASONRY UNIT COLUMN CONCRETE CONN CONNECTION

CONC CONTINUOUS CONT CONTR CONTRACTOR COORD COORDINATE COUNTER SINK CENTER CENTERED CUBIC YARD

DEFORMED BAR ANCHOR DIAMETER DIAG DIAGONAL DEAD LOAD DNDOMN

EA

DTL DETAIL DMG DRAMING EACH END EACH FACE EXPANSION JOINT ENGINEER

EDGE OF SLAB ELEVATION EQ EQUAL EQUAL SPACE(S) (ING) ES EACH SIDE EACH MAY **EXPANSION** EXT EXTERIOR **EXIST** EXISTING

FLORIDA BUILDING CODE FD FLOOR DRAIN FOUNDATION FDN FINISHED FLOOR FLOOR FLANGE FRAMING FS FAR SIDE

FTG FOOTING GALVANIZE(D) GRADE BEAM GENERAL CONTRACTOR GROUT FILLED CELL(S) / COURSE

FOOT

GLU-LAM BEAM GRADE HOLLOM-CORE HDG HOT DIPPED GALVANIZED HOOK HORIZONTAL HORIZ HIGH POINT HEADED STUD HOLLOW STRUCTURAL SECTION

INSIDE DIAMETER INSIDE FACE INT INTERIOR

TMIOL KIP KO KNOCK-OUT

HK

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BBM PROJECT #22006

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STRUCTURAL

2300 Maitland Center Parkway

LUIS F. BEDOYA, P.E.

DATE

Florida Professional Engineer No. 65509

WRITTEN DIMENSIONS ON THESE DOCUMENTS SHALL HAVE PRECEDENCE OVER SCALE DIMENSIONS CONTRACTORS SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS ON THE JOB AND HIS OFFICE MUST BE NOTIFIED OF ANY VARIATION FROM THE DIMENSIONS AND CONDITIONS SHOWN BY THE DRAWINGS. SHOP DRAWINGS AND OR DETAILS MUST BE SUBMITTED TO THIS OFFICE FOR REVIEW BEFORE

ST. LUCIE HIGH SCHOOL DDD

Port St. Lucie, FL



St. Lucie Public Schools 9461 Brandywine Ln Port St. Lucie, FL 34986

ISSUE DATE: 12/16/2022 COMM. NO.: 2022107

DRAWN BY: NDM CHECKED BY: LB / JMP

STRUCTURAL GENERAL NOTES & ABBREVIATIONS

STRUCTURAL GENERAL NOTES & ABBREVIATIONS

3/4" = 1'-0"

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PHASE III - CONSTRUCTION DOCUMENTS

Autodesk Docs://2022107 St. Lucie HS DDD/SLPS_HS DDD_2022 - S.rvt

REQUIREMENTS.

GROUT TEST PRISM

COYER UNITS USING ABSORPTIVE PAPER

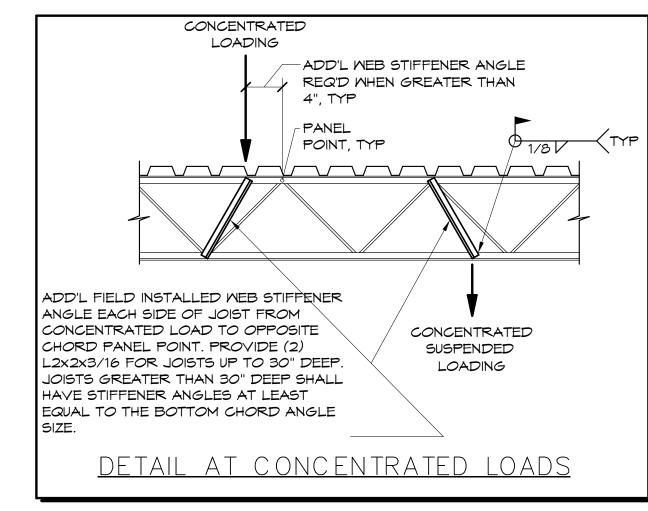
TOWELLING W/ TAPE

3"x3"x1 5/8" MOOD BLOCK

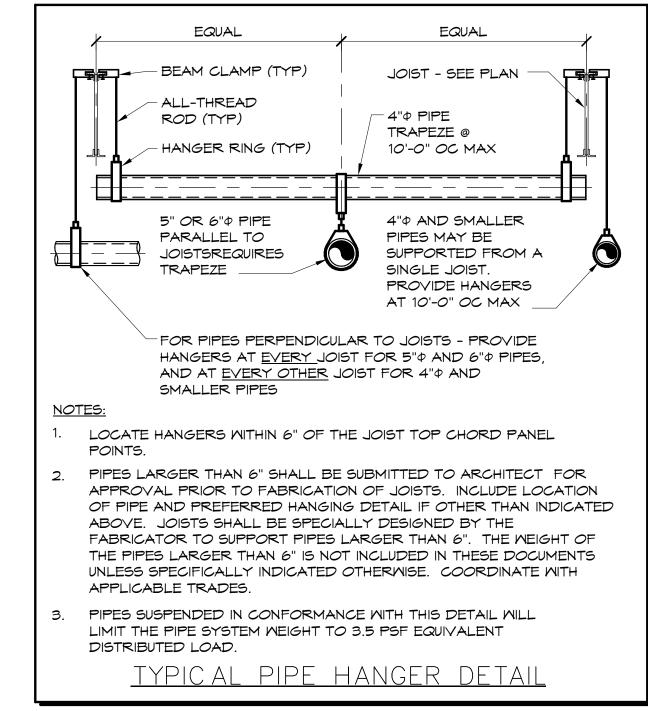
(4) 8x8x16 CMU BLOCKS REQUIRED (FRONT BLOCK

REMOVED FOR CLARITY)

- A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
- ALL DESIGN, FABRICATION AND ERECTION OF STEEL JOISTS AND BRIDGING SHALL BE IN STRICT ACCORDANCE WITH THE SPECIFICATIONS OF STEEL JOIST INSTITUTE (OPEN WEB STEEL JOIST SJI PUBLICATION SJI 100-2015, AND JOIST GIRDERS SJI 200-2015) AND RECOMMENDED CODE OF STANDARD PRACTICE.
- THE ENDS OF ALL BRIDGING LINES TERMINATING AT WALLS OR BEAMS SHALL BE ANCHORED TO THE WALL OR BEAM.
- ALL STEEL JOISTS ARE TO BE CAMBERED AS SPECIFIED BY STEEL
- PROVIDE BOTTOM AND/OR TOP CHORD EXTENSIONS AS SHOWN ON DRAWINGS.
- UNLESS NOTED OTHERWISE, MINIMUM JOIST BEARING SHALL BE 2 1/2" FOR K-SERIES JOISTS, 4" FOR LH, DLH AND SLH 15-18, AND 6" FOR SLH 19-25 ON A STEEL MEMBER OR EMBED PLATE.
- BRIDGING SHALL BE FURNISHED AND INSTALLED TO MEET THE SIZE AND SPACING REQUIREMENTS OF THE SJI STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOISTS. ALL BRIDGING AND BRIDGING ANCHORS SHALL BE COMPLETELY INSTALLED BEFORE CONSTRUCTION LOADS ARE PLACED ON THE JOISTS.
- ALL HANGERS, CURBS AND/OR ROOFTOP FRAMES TO SUPPORT MECHANICAL EQUIPMENT, ETC., TO BE SUPPORTED BY THE JOISTS SHALL BE LOCATED AT THE PANEL POINTS OF THE JOISTS IF POSSIBLE. HOMEVER, IF THE CONCENTRATED LOAD MUST BE LOCATED FURTHER THAN 4" FROM A PANEL POINT, PROVIDE WEB STIFFENER ANGLES. WEB STIFFENERS MUST BE INSTALLED EACH SIDE OF JOIST FROM CONCENTRATED LOAD TO OPPOSITE CHORD PANEL POINT BEFORE LOAD IS APPLIED. SEE DETAIL BELOW:



- CONTRACTOR TO FURNISH BAR JOIST CERTIFICATIONS SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. THE SPECIALTY ENGINEER FOR THE STEEL JOIST SUPPLIER SHALL ALSO CERTIFY THAT THE STEEL JOIST BOTTOM CHORDS WILL SAFELY RESIST THE WIND UPLIFTS, CONSIDERING THE SPACING OF BRIDGING.
- PROVIDE UPLIFT BRIDGING PER TABULATED PRESSURES ON SHEET S603. ALL ITEMS SUSPENDED FROM JOISTS (I.E. CATWALKS, BALCONIES, OPERABLE PARTITIONS, ETC.) SHALL BE INSTALLED AFTER DEAD LOAD HAS BEEN
- BOLTED TIE JOISTS (BTJ) ARE USED IN STEEL FRAMES WHERE COLUMNS ARE NOT FRAMED IN AT LEAST TWO DIRECTIONS WITH STRUCTURAL STEEL MEMBERS. JOIST(S) AT COLUMN LINES SHALL BE FIELD BOLTED AT THE COLUMNS WITH TWO 1/2" \$\rightarrow\$ BOLTS TO PROVIDE LATERAL STABILITY DURING CONSTRUCTION.
- STEEL JOISTS SHALL RECEIVE SHOP COAT OF PRIMER (COLOR AS DIRECTED BY ARCHITECT) EXCEPT THOSE AREAS WHICH WILL RECEIVE SPRAY-ON FIRE PROTECTION.
- ANY STEEL JOIST WITHIN A 4'-0" DISTANCE FROM A PARALLEL SUPPORT SHALL BE FABRICATED IN SUCH A WAY THAT CAMBER OF THE JOIST WILL NOT CAUSE A PROBLEM INSTALLING THE METAL DECK.
- IN THE EVENT THAT FIRE SPRINKLERS ARE REQUIRED FOR THIS PROJECT, THE STEEL FABRICATOR SHALL PROVIDE A DIMENSIONED JOIST BRIDGING AND JOIST GIRDER BOTTOM CHORD BRACE PLAN ALONG WITH DETAILS TO THE SPRINKLER CONTRACTOR. THE FABRICATOR AND SPRINKLER CONTRACTOR SHALL COORDINATE WITH EACH OTHER TO ENSURE THAT ANY CONFLICTS ARE RESOLVED BEFORE ANY FABRICATION BEGINS.
- ALL PIPES MUST BE SUPPORTED AS SHOWN BELOW:



STEEL JOISTS (cont.)

FABRICATOR SHALL ENSURE THAT ALL OSHA REQUIREMENTS ARE MET. PARTICULAR ATTENTION SHALL BE PAID TO THE ERECTION PROCESS, BOLTED CONNECTIONS MAY BE REQUIRED. SUBMIT DETAILS FOR APPROVAL.

THE JOIST MANUFACTURER MAY NOT INCREASE ALLOWABLE STRESSES.

- THE OPEN WEB STEEL JOISTS SHALL BE FABRICATED AND ERECTED IN FULL CONFORMANCE WITH THE "OSHA STEEL ERECTION STANDARD". IF THE CONSTRUCTION DRAWINGS DEVIATE FROM THE OSHA STANDARD THEN THE FABRICATOR SHALL PROVIDE SUBMITTALS THAT CLEARLY INDICATE THE DEVIATION WITH A REVISION CLOUD AND REQUEST APPROVAL FROM BBM TO MAKE THE CHANGE SO THAT CONFORMANCE WITH THE OSHA STANDARD IS
- THE SUPPORTS FOR SCISSOR, ARCHED CHORDS OR ANY OTHER SIMILAR TYPE JOIST, UNLESS NOTED OTHERWISE, SHALL BE DESIGNED WITH A PINNED CONNECTION AT ONE END AND A HORIZONTAL ROLLER AT THE OTHER END. THE SUPPORTING STRUCTURE, UNLESS NOTED OTHERWISE, HAS NOT BEEN DESIGNED FOR ANY GRAVITY LOAD HORIZONTAL FORCE DUE TO DEFLECTION. DO NOT DESIGN THESE TYPE OF JOISTS WITH PINNED SUPPORTS
- K-SERIES STEEL JOISTS WITH SPANS 40'-O" AND LONGER SHALL BE ERECTED IN PANELS SO THAT BOLTED CONNECTIONS ARE NOT REQUIRED (EXCEPT AT THE COLUMN LINES). THE GC SHALL INSURE THAT ALL RELATED JOIST FRAMING COMPONENTS ARE COORDINATED TO MEET THIS REQUIREMENT.
- ALL ROOFS THAT EXCEED 1/4"/FT SLOPE SHALL HAVE THE JOIST BEARING SEATS SLOPED AS REQUIRED PER STEEL JOIST INSTITUTE.

PRECAST HOLLOW-CORE PLANKS

EACH END.

- PRECAST PLANKS SHALL BE 8" PRESTRESSED HOLLOW CONCRETE PLANKS, MACHINE-EXTRUDED WITH ZERO SLUMP. THE PRESTRESSED CONCRETE MANUFACTURER'S PLANT SHALL CONFORM TO THE REQUIREMENTS DEFINED IN PRESTRESSED CONCRETE INSTITUTE'S "116-MANUAL FOR QUALITY CONTROL FOR PLANTS AND PRODUCTION OF PRECAST, PRESTRESSED CONCRETE PRODUCTS".
- CONCRETE COMPRESSIVE STRENGTH: MINIMUM 4000 PSI AT TRANSFER, AND 5000 PSI AT 28 DAYS.
- PRESTRESSED STRAND: PER MANUFACTURER.
- DESIGN IN ACCORDANCE WITH ACI 318. LIVE-LOAD DEFLECTION SHALL NOT EXCEED THE SPAN/360.
- REFER TO THE ARCHITECTURAL DRAWINGS FOR ANY FIRE RATING REQUIREMENTS.
- SUBMIT SHOP DRAWINGS SHOWING SETTING PLAN, UNIT SIZE, STEEL CONTENT AND ALL NECESSARY DETAILS. ALL SHOP DRAWINGS SHALL BE CHECKED AND INITIALED BY CONTRACTOR. SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION.
- PRECAST PLANKS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. PROVISIONS SHALL BE MADE FOR ANCHORAGE TO BEAMS, WALLS AND OTHER STRUCTURAL ELEMENTS TO PROVIDE LATERAL SUPPORT OF SUCH MEMBERS.
- INSTALL PLANKS IN A TIGHT, LEVEL POSITION ON TRUE LEVEL BEARING SURFACE. PRECAST SLABS SHALL BEAR ON 1/8" BEARING PADS TO DISTRIBUTE THE BEARING STRESSES. MINIMUM BEARING SHALL BE 3" ON MASONRY AND 2 1/2" ON STRUCTURAL STEEL AND CONCRETE FOR STRESSES IN THE SUPPORT MEMBERS. THE MINIMUM BEARING FOR STRESSES IN THE PLANKS SHALL BE DETERMINED BY THE PRE-CAST MANUFACTURER. THE GREATER OF THE MINIMUM BEARINGS SHALL BE PROVIDED OR THE PLANKS SHALL BE IMMEDIATELY SHORED AND THE ARCHITECT NOTIFIED ..
- SUPPLY AND INSTALL STEEL HANGERS WHERE REQUIRED FOR LARGE HC9 OPENINGS.
- DO NOT CUT ANY REINFORCING WITHOUT APPROVAL OF PRECAST PLANK MANUFACTURER AND ENGINEER.
- GROUT PRECAST UNITS WITH 1:3 CEMENT-SAND GROUT MIX PRIOR TO PLACING TOPPING OR LOADS ON THE UNITS, PROPERLY FILLING SHEAR KEYMAYS BETWEEN UNITS.
- THE PRE-CAST MANUFACTURER SHALL TAKE INTO ACCOUNT THE REDUCTION OF THE TOPPING THICKNESS CAUSED BY CAMBER IN THE PLANK DESIGN. THE TOPPING THICKNESS CALLED FOR IN THE DRAWINGS REFERS TO THE THICKNESS AS MEASURED AT THE BEARING LOCATIONS. THE GENERAL CONTRACTOR SHALL NOTIFY THE ARCHITECT AND ENGINEER DURING PLANK DESIGN PHASE AND PRIOR TO FABRICATION OF ANY "PROBLEM" PLANKS SHOULD THE REDUCTION IN TOPPING THICKNESS AT MID-SPAN DECREASE THE LOADING CARRYING CAPACITY BELOW DESIGN LOADS SPECIFIED ON THE FRAMING PLAN(S).
- THE CONCRETE TOPPING SHALL BE NORMAL-WEIGHT CONCRETE OF MINIMUM 3000 PSI COMPRESSIVE STRENGTH WITH A MINIMUM THICKNESS OF 2", REINFORCED WITH 6x6-W1.4xW1.4 MWF. ADEQUATE BOND TO BE ACCOMPLISHED BY CLEANING AND WETTING PLANKS BEFORE POURING
- THE TOPPING. ALL EXTERIOR TOPPINGS AND INTERIOR TOPPINGS EXPOSED TO VIEW SHALL HAVE A 4" WIDE RUBBER BOND BREAKER STRIP PLACED OVER, CENTERED ON AND AFFIXED TO THE PLANKS (BUT NOT THE GROUT JOINT ITSELF) AT EVERY PLANK-TO-PLANK JOINT PRIOR TO PLACING THE CONCRETE TOPPING. THE CONCRETE TOPPING SHALL BE TOOLED AT EVERY PLANK-TO-PLANK JOINT WITH A "DEEP AND THIN GROOVER" AS MANUFACTURED BY GOLDBLATT TOOLS (PART #G06211) OR APPROVED EQUAL WHICH WILL CREATE AN 1/8" WIDE X 1" DEEP GROOVE.

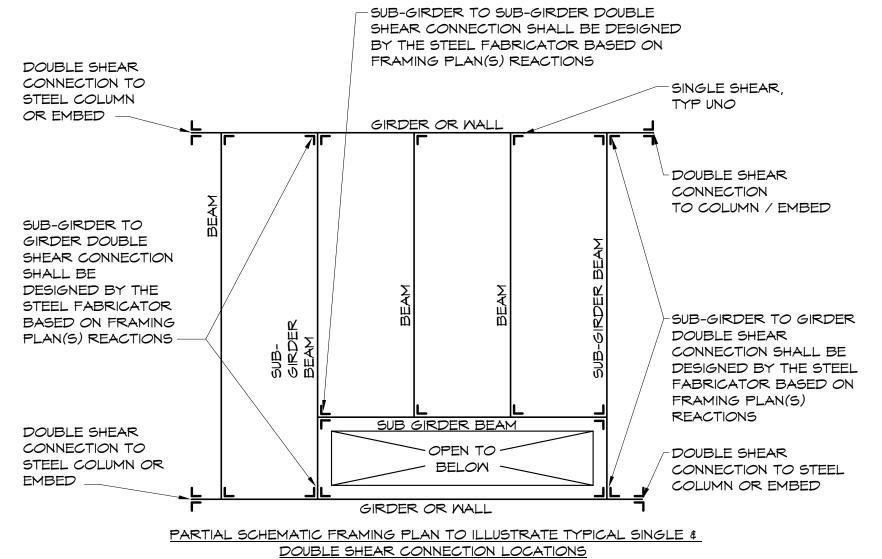
STRUCTURAL STEEL

- A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
- FABRICATE AND ERECT STRUCTURAL STEEL IN CONFORMANCE WITH THE LATEST VERSION OF AISC 360-16.
- MATERIAL SPECIFICATIONS:
 - ALL STEEL SHALL BE PRODUCED DOMESTICALLY.
 - ROLLED SHAPES, PLATES AND BARS: ASTM A36, EXCEPT WIDE-FLANGE & MT SECTIONS, WHICH SHALL BE ASTM A992.
 - HOLLOW STRUCTURAL SECTION (HSS): ASTM A500, GRADE B. ANCHOR BOLTS, RODS, NUTS AND WASHERS:
 - PER BASE PLATE SCHEDULE.
 - HEADED STUDS: ASTM A108, GRADE 1015 THROUGH 1020, COLD-FINISHED CARBON STEEL, AWS D1.1, TYPE B. BOLTED STRUCTURAL CONNECTIONS: UNLESS NOTED OTHERWISE,
 - ALL BOLTS SHALL BE 3/4" PASTM A325, TYPE N. BOLTS INDICATED LESS THAN 5/8" PSHALL BE ASTM A307. WELDED CONNECTIONS: ELECTRODES - ETOXX UNO (LOW
- HYDROGEN). FILLET WELDS SHALL BE 3/16" UNO. HIGH-STRENGTH FIELD-BOLTED CONNECTIONS SHALL BE INSTALLED, TIGHTENED, TESTED AND INSPECTED ACCORDING TO "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS" BY THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (RCSC). ALL BOLTS IN STEEL
- SHALL BE FINGER-TIGHT WITH PEENED THREADS. SLIP-CRITICAL (SC) BOLTS MUST BE FULLY TENSIONED PER SPECIFICATION. STANDARD NON-SLOPED AND NON-SKEWED SHEAR CONNECTIONS HAVE BEEN DESIGNED AND THE NECESSARY INFORMATION MAY BE FOUND IN THE

TO STEEL CONNECTIONS SHALL BE BROUGHT TO A "SNUG-TIGHT" CONDITION, AS

DEFINED IN THE SPECIFICATION. ALL BOLTS IN STEEL TO EMBED CONNECTIONS

- SCHEDULES. THE ULTIMATE (i.e. FACTORED) REACTIONS HAVE BEEN PROVIDED AT EACH END OF EACH MEMBER SHOULD THE FABRICATOR WISH TO RE-ENGINEER THE CONNECTIONS TO THEIR PREFERENCES. SHOULD THE FABRICATOR WISH TO RE-ENGINEER THE CONNECTIONS, THEY MUST PROVIDE SUBMITTALS THAT HAVE BEEN PREPARED AND SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE SAME STATE AS THE PROJECT
- NON-STANDARD SLOPED AND/OR SKEMED SHEAR CONNECTIONS SHALL BE DESIGNED & DETAILED BY THE FABRICATOR'S ENGINEER. PROVIDE SIGNED & SEALED CONNECTION SUBMITTAL FOR REVIEW.
- ALL WIDE FLANGE FLOOR MEMBERS SHALL BE CONNECTED TO THE SUPPORTING STRUCTURE AS DETAILED IN CONNECTION SCHEDULES ON SHEET S602. UNLESS SPECIFICALLY NOTED OTHERWISE ON PLAN(S), ANY FLOOR MEMBER SUPPORTING ANOTHER FLOOR MEMBER SHALL BE CONNECTED AS DETAILED IN DOUBLE SHEAR SCHEDULES N1/S602 AND N8/S602. SINGLE SHEAR CONNECTIONS AS DETAILED IN SCHEDULES J1/S602 AND J12/S602 SHALL ONLY BE USED FOR FLOOR MEMBERS SUPPORTING DECK/SLAB ONLY (I.E. FILLER BEAMS) OR AS SPECIFICALLY IDENTIFIED ON PLAN OR SECTION. THE USE OF A DOUBLE SHEAR CONNECTION MAY BE REQUIRED FOR A "TYPICAL" FLOOR BEAM DUE TO BEAM REACTION. REFER TO REACTION NOTED ON PLAN(S) & COORDINATE WITH SCHEDULED MAXIMUM VALUES TO DETERMINE CONNECTION TYPE REQUIRED. SEE PARTIAL SCHEMATIC FRAMING PLAN BELOW THAT ILLUSTRATES WHERE TYPICAL SINGLE AND DOUBLE SHEAR CONNECTIONS ARE REQUIRED.



ALL WIDE FLANGE ROOF MEMBERS SHALL BE CONNECTED TO THE SUPPORTING STRUCTURE AS DETAILED IN THE CONNECTION SCHEDULES ON SHEET S602. UNLESS SPECIFICALLY NOTED OTHERWISE ON PLAN, ALL ROOF MEMBERS SHALL BE CONNECTED AS DETAILED IN THE SINGLE SHEAR SCHEDULES J1/S602 AND J12/5602.

BRACE AND MAINTAIN ALL STEEL IN ALIGNMENT UNTIL OTHER PARTS OF CONSTRUCTION NECESSARY FOR PERMANENT SUPPORT ARE COMPLETED. CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLING TEMPORARY SHORING AS REQUIRED FOR THE STABILITY OF THE STEEL FRAME UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN COMPLETED AND BUILDING IS ENCLOSED.

ALL MELDING SHALL CONFORM TO THE REQUIREMENTS OF "THE STANDARD CODE FOR WELDING IN BUILDING CONSTRUCTION" OF THE AMERICAN WELDING

SOCIETY. GROUT FOR COLUMN BASE PLATES AND PRESET BEARING PLATES SHALL

BE NON-SHRINK, NON-METALLIC GROUT (5000 PSI MIN). SUBMIT SHOP DRAWINGS INDICATING ALL SHOP AND ERECTION DETAILS INCLUDING PROFILES, SIZES, SPACING AND LOCATIONS OF STRUCTURAL MEMBERS, CONNECTION ATTACHMENTS, FASTENERS, LOADS AND TOLERANCES

ALL STEEL EXPOSED TO MEATHER SHALL BE HOT DIP GALVANIZED IN ACCORDANCE WITH ASTM A123 FOR MEMBERS AND ASTM A153 FOR CONNECTION ELEMENTS, EXCEPT THAT ALL ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS) SHALL BE BLAST CLEANED AND COATED IN ACCORDANCE WITH THE STRUCTURAL STEEL AND PAINT SPECIFICATIONS.

STRUCTURAL STEEL SHALL RECEIVE A SHOP COAT OF PRIMER (COLOR AS DIRECTED BY ARCHITECT) EXCEPT THOSE AREAS WHICH WILL RECEIVE SPRAY-ON FIRE PROTECTION, OR WHERE HEADED STUDS ARE TO BE WELDED.

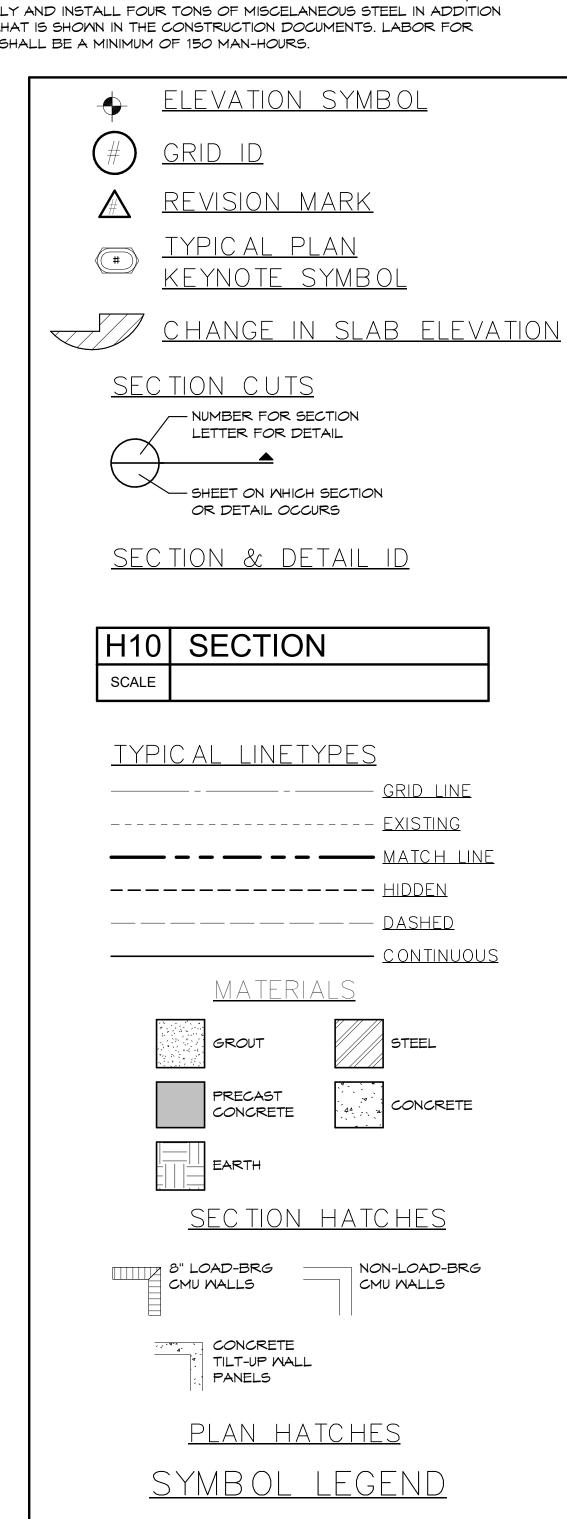
PROVIDE ALLOMANCE FOR 300 FEET OF L3x3x1/4 AND LABOR TO INSTALL SAME AT ARCHITECT OR ENGINEER'S DIRECTION. CREDIT OWNER WITH ALL MATERIAL AND LABOR NOT USED. STEEL BEAMS SHALL BE CAMBERED AS INDICATED ON THE FRAMING PLAN(S).

IF NO CAMBER IS SPECIFIED, THE FABRICATOR SHALL ENSURE THAT THE "NATURAL" CAMBER IN ALL ERECTED BEAMS OCCURS IN AN UPWARD DIRECTION. ALL STEEL BEAMS THAT ARE PARALLEL TO BAR JOISTS SHALL BE CAMBERED TO MATCH THE JOIST CAMBER AND THE TOP OF THE BEAM SHALL MATCH THE TOP OF THE JOIST, TYP UNO.

THE STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN FULL CONFORMANCE WITH THE "OSHA STEEL ERECTION STANDARD". IF THE CONSTRUCTION DRAWINGS DEVIATE FROM THE OSHA STANDARD THEN THE FABRICATOR SHALL PROVIDE SUBMITTALS THAT CLEARLY INDICATE THE DEVIATION WITH A REVISION CLOUD AND REQUEST APPROVAL FROM BBM TO MAKE THE CHANGE SO THAT CONFORMANCE WITH THE OSHA STANDARD IS

REFER TO SPECIALTY ENGINEERING (SE) NOTES FOR DELEGATED ENGINEERING REQUIREMENTS.

CONTRACTOR SHALL INCLUDE IN THE BID THE COST TO FABRICATE SUPPLY AND INSTALL FOUR TONS OF MISCELANEOUS STEEL IN ADDITION TO WHAT IS SHOWN IN THE CONSTRUCTION DOCUMENTS. LABOR FOR THIS SHALL BE A MINIMUM OF 150 MAN-HOURS.

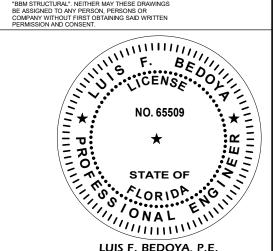


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Florida Professional Engineer No. 6550

ADDENDUM NO 2

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WRITTEN DIMENSIONS ON THESE DOCUMENTS SHALL HAVE PRECEDENCE OVER SCALE DIMENSIONS ONTRACTORS SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS ON THE JOB AND HIS OFFICE MUST BE NOTIFIED OF ANY VARIATION FROM THE DIMENSIONS AND CONDITIONS SHOWN BY THES RAWINGS. SHOP DRAWINGS AND OR DETAILS MUST BE SUBMITTED TO THIS OFFICE FOR REVIEW BEFORE

ST. LUCIE HIGH SCHOOL DDD

Port St. Lucie, FL



St. Lucie Public Schools 9461 Brandywine Ln Port St. Lucie, FL 34986

ISSUE DATE: 12/16/2022 COMM. NO.: 2022107

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STRUCTURAL GENERAL NOTES & SYMBOL LEGEND

STRUCTURAL GENERAL NOTES & SYMBOL LEGEND

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3/4" = 1'-0"

- THE THRESHOLD INSPECTION SHALL VISUALLY INSPECT THE STRUCTURAL COMPONENTS AT THE MANUFACTURER'S PLACE OF BUSINESS THAT CANNOT BE INSPECTED AT THE JOB SITE AFTER FABRICATION. THE THRESHOLD INSPECTOR MAY EMPLOY DULY AUTHORIZED REPRESENTATIVE IN THE FIELD. HOWEVER, SUCH REPRESENTATIVES MUST:
- A. BE A LICENSED ARCHITECT OR PROFESSIONAL ENGINEER, OR B. GRADUATE FROM AN ENGINEERING EDUCATION PROGRAM IN CIVIL OR STRUCTURAL ENGINEERING, OR C. GRADUATE FROM AN ARCHITECTURAL EDUCATION PROGRAM, OR
- D. SUCCESSFULLY COMPLETE THE NCEES FUNDAMENTALS EXAMINATION, OR
- E. BE LICENSED AS EITHER A BUILDING INSPECTOR OR A GENERAL CONTRACTOR.
- 2. THE SCOPE OF INSPECTION SHALL INCLUDE, BUT SHALL NOT NECESSARILY BE LIMITED TO, THE ITEMS LISTED IN THIS PLAN. ALL STRUCTURAL COMPONENTS, WHICH ARE RELATED TO THE PUBLIC HEALTH, SAFETY, OR WELFARE, ARE TO BE INSPECTED BY A SPECIAL INSPECTOR.
 - THE SPECIAL INSPECTOR SHALL BE A REGISTERED ARCHITECT OR PROFESSIONAL ENGINEER IN FLORIDA. THE SPECIAL INSPECTOR SHALL BE A SINGULAR PERSON, NOT A FIRM OR COMPANY, NOR TWO OR MORE PERSONS TAKEN COLLECTIVELY. THE SPECIAL INSPECTOR SHALL MEET THE REQUIREMENTS OF SECTION 61G15-35.003, F.A.C.
 - A. PROOF OF CURRENT LICENSURE IN GOOD STANDING AS A LICENSED PROFESSIONAL ENGINEER IN THE STATE OF FLORIDA WHOSE PRINCIPAL PRACTICE IS STRUCTURAL ENGINEERING OR WHOSE PRINCIPAL PRACTICE IS IN PERFORMING STRUCTURAL FIELD INSPECTIONS ON THRESHOLD BUILDINGS.
 - B. LICENSED PROFESSIONAL ENGINEERS WHOSE PRINCIPAL PRACTICE IS STRUCTURAL ENGINEERING SHALL ALSO HAVE THREE (3) YEARS OF EXPERIENCE IN PERFORMING STRUCTURAL FIELD INSPECTIONS ON THRESHOLD BUILDINGS OR EQUIVALENT PURSUANT TO A THRESHOLD / SPECIAL INSPECTION PLAN RELEVANT TO THE WORK PERFORMED AND TWO (2) YEARS OF EXPERIENCE IN THE STRUCTURAL DESIGN OF THRESHOLD BUILDINGS. FOR THE PURPOSE OF THESE CRITERIA, STRUCTURAL DESIGN SHALL MEAN THE DESIGN OF ALL STRUCTURAL COMPONENTS OF THE BUILDING AND SHALL NOT BE LIMITED TO SPECIFIC STRUCTURAL COMPONENTS ONLY, SUCH AS FOUNDATIONS, PRESTRESSED OR POST-TENSIONED CONCRETE, ETC.
 - C. LICENSED PROFESSIONAL ENGINEERS WHOSE PRINCIPAL PRACTICE IS STRUCTURAL FIELD INSPECTIONS SHALL HAVE FIVE (5) YEARS OF EXPERIENCE IN PERFORMING STRUCTURAL FIELD INSPECTIONS ON THRESHOLD BUILDINGS OR EQUIVALENT PURSUANT TO A THRESHOLD / SPECIAL INSPECTION PLAN RELEVANT TO THE WORK PERFORMED AND POSSESS EACH OF THE CERTIFICATIONS IDENTIFIED IN PARAGRAPH 61G15-35.004(2)(f), F.A.C., AT THE TIME OF APPLICATION.
- THE THRESHOLD INSPECTOR'S PRINCIPAL RESPONSIBILITY IS TO ASSURE THAT THE STRUCTURAL COMPONENTS ARE CONSTRUCTED IN ACCORDANCE WITH THE APPROVED PLANS AND SPECIFICATIONS. THE SPECIAL INSPECTOR OR HIS/HER DULY AUTHORIZED REPRESENTATIVE SHALL PERFORM INSPECTIONS ACCORDING TO THE SPECIAL INSPECTION PLAN.
- NOTIFY THE CONTRACTOR OF ANY VARIATION FROM CONTRACT DOCUMENTS. DEVIATIONS, WHICH ARE NOT CORRECTED, OR OTHERWISE ADDRESSED, SHALL BE REPORTED TO THE CONTRACTOR IN WRITING AND TO THE OWNER, ARCHITECT AND BUILDING OFFICIAL HAVING JURISDICTION.
- KEEP A DAILY RECORD IDENTIFYING TYPE AND LOCATION OF WORK BEING DONE, INCLUDING WEATHER, TEMPERATURE, TIME OF DAY AND OTHER INFORMATION AS REQUIRED BY THE BUILDING OFFICIAL. A WEEKLY PROGRESS REPORT OF DAILY INSPECTIONS SHALL BE SUBMITTED TO THE OWNER, ARCHITECT AND BUILDING OFFICIAL AND SHALL BE SIGNED, SEALED, AND DATED BY THE THRESHOLD INSPECTOR.
- THE SPECIAL INSPECTOR, UPON COMPLETION OF THE BUILDING AND PRIOR TO THE ISSUANCE OF A CERTIFICATE OF OCCUPANCY, SHALL FILE A SIGNED AND SEALED STATEMENT WITH THE ENFORCEMENT AGENCY IN SUBSTANTIALLY THE FOLLOWING FORM: TO THE BEST OF MY KNOWLEDGE AND BELIEF, THE CONSTRUCTION OF ALL STRUCTURAL LOAD-BEARING COMPONENTS DESCRIBED IN THE THRESHOLD INSPECTION PLAN COMPLIES WITH THE PERMITTED DOCUMENTS AND THE SPECIALTY SHORING DESIGN PROFESSIONAL ENGINEER HAS ASCERTAINED THAT THE SHORING AND RESHORING CONFORMS WITH THE SHORING AND RESHORING PLANS SUBMITTED TO THE DISTRICT OFFICE. PURSUANT TO THE FLORIDA STATUTES, 553.79, (7) (a), "THE SPECIAL INSPECTOR SHALL DETERMINE THAT A PROFESSIONAL ENGINEER WHO SPECIALIZES IN SHORING DESIGN HAS INSPECTED THE SHORING AND RE-SHORING FOR CONFORMANCE WITH THE SHORING AND RE-SHORING PLANS SUBMITTED TO THE ENFORCING AGENCY. THE SPECIAL INSPECTOR SHALL READ THE THRESHOLD INSPECTION PLAN IN ADVANCE AND STATE TO
- THE OWNER, IN WRITING, THAT HE/SHE INTENDS TO COMPLY. THE SPECIAL INSPECTOR SHALL VERIFY THAT ALL SUBMITTALS THAT ARE REQUIRED TO BE SIGNED AND SEALED SHALL MEET THE ELECTRONIC DOCUMENTS REQUIREMENT OF THE FLORIDA BOARD OF
- B. <u>SOIL COMPACTION:</u>
- CONFIRM PROOF-ROLLING HAS BEEN PERFORMED.
- MUCK (IF PRESENT AT THE SITE):

PROFESSIONAL ENGINEERS.

CONFIRM THAT ALL MUCK HAS BEEN REMOVED.

CONCRETE GENERAL

- A QUALIFIED REPRESENTATIVE OF THE TESTING LABORATORY SHOULD CHECK ALL CONCRETE FOR PROPER SLUMP AND PROPER PREPARATION OF TEST CYLINDERS. NO WATER SHALL BE ADDED AFTER TEST CYLINDERS ARE MADE. COPIES OF THE TEST REPORTS SHOULD BE PROMPTLY FURNISHED TO THE CONTRACTOR, ARCHITECT, ENGINEER-OF-RECORD AND BUILDING OFFICIAL.
- USE THE STRUCTURAL DRAWINGS IN CONJUNCTION WITH APPROVED SHOP DRAWINGS FOR CHECKING REINFORCING PLACEMENT. NOTIFY ENGINEER OF ANY DISCREPANCIES.
- FORMING, SHORING AND RE-SHORING LAYOUT SHALL BE PREPARED BY THE FORMING SUBCONTRACTOR AND HIS QUALIFIED PROFESSIONAL ENGINEER. THE APPROVED SUBMITTALS SHALL BE AVAILABLE AT THE JOB SITE TO ENSURE THAT THE WORK IS DONE CORRECTLY.
- ALL SHORING AND RE-SHORING DRAWINGS SHOULD GIVE ADEQUATE INFORMATION ON THE SIZES AND CAPACITIES OF THE SHORES, INCLUDING DIMENSIONS, SO THAT THE SHORES ON THE JOB CAN BE VERIFIED AS THE PROPER UNITS. REQUIRED BRACING SHOULD ALSO BE INDICATED.
- SHORING AND RE-SHORING SHOULD BE INSPECTED FOR PROPER SPACING, PLUMB, PHYSICAL CONDITION, PROPER BEARING AT SUPPORTS AND OF FORMING MEMBERS ON THE SHORES. BRACING WHEN REQUIRED SHOULD ALSO BE CHECKED FOR PROPER CONNECTION TO THE SHORES. DEFLECTION OF THE FORMS AND SETTLEMENT OF THE SHORES SHALL BE PERFORMED DURING THE PLACEMENT OF CONCRETE IF DEEMED NECESSARY BY THE THRESHOLD ENGINEER
- THE CONCRETE COMPRESSIVE CYLINDER TESTS SHALL BE CHECKED AND ANY FAILURES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD.
- OBSERVE PLACEMENT OF ALL CONCRETE. VERIFY SLUMPS AND DETERMINE IF ANY WATER IS ADDED TO CONCRETE AT JOB SITE AND WHO AUTHORIZED IT. RECORD METHODS USED TO CONSOLIDATE CONCRETE. FOUNDATIONS
- OBTAIN A COPY OF THE GEOTECHNICAL ENGINEER'S REPORT, ALONG WITH ANY ADDENDUMS, AND CONFIRM
- THAT THE GEOTECHNICAL ENGINEER IS SATISFIED WITH THE BUILDING PAD PREPARATION. THE SPECIAL INSPECTOR SHALL RECEIVE FROM THE GEOTECHNICAL ENGINEER A FINAL SUMMARIZING STATEMENT STATING THAT ALL SOIL CONSOLIDATION AND/OR DEEP FOUNDATION WORK CONFORMS TO
- THE CONTRACTUAL DOCUMENTS AND THE GEOTECHNICAL ENGINEERING REPORT 2. A QUALIFIED TESTING LABORATORY WILL BE EMPLOYED TO TAKE DENSITY TESTS OF SOIL BENEATH FOOTINGS AND SLABS PRIOR TO PLACING REINFORCING IN FOOTINGS. VERIFY TESTS ARE COMPLETED BY SOILS TESTING LABORATORY AND THAT SATISFACTORY COMPACTION IS ACHIEVED PRIOR TO COMMENCEMENT OF
- ALL EXCAVATIONS, BACKFILLING, AND COMPACTION PROCEDURES SHALL BE CARRIED OUT IN ACCORDANCE WITH THE GEOTECHNICAL ENGINEER'S RECOMMENDATIONS
- 4. VERIFY FOOTING DIMENSIONS
- VERIFY TOP OF FOOTING ELEVATIONS.

CONSTRUCTION ON SUBGRADE.

- REINFORCING STEEL IN FOUNDATIONS SHOULD BE CHECKED FOR PROPER SIZE, GRADE, QUANTITY AND REQUIRED CLEARANCE FROM SOIL AND FORMS.
- 6A. REINFORCING STEEL IN FOOTING SHALL BE CHECKED FOR CORRECT GRADE OF STEEL (GRADE 60). DOWELS FOR WALLS AND COLUMNS SHOULD BE FIRMLY SUPPORTED AND ACCURATELY LOCATED. PLACING DOWELS AS CONCRETE IS POURED SHALL NOT BE PERMITTED. THE INSPECTOR SHOULD REQUEST THE CONTRACTOR TO SPOT CHECK DOWEL POSITIONS IN HIS PRESENCE. THE INSPECTOR SHALL VERIFY THE UPWARD EXTENSION OF ALL DOWELS FOR PURPOSE OF SPLICES.
- REINFORCING STEEL IN FOOTINGS SHALL BE CHECKED FOR CORRECT GRADE OF STEEL (GRADE 60).

SLAB ON GRADE

- 1. SLAB SUBGRADE SHALL BE INSPECTED FOR COMPACTION AND LEVELNESS. CHECK ALL UTILITY TRENCHES FOR PROPER BACK FILL AND COMPACTION. ENSURE THAT NO DEPRESSIONS OR RUTS EXIST IN SLAB AREA OTHER THAN THOSE REQUIRED BY DESIGN. SPECIAL INSPECTOR SHALL CHECK STRUCTURAL LOAD-BEARING (NON-STRUCTURAL FLOATING SLABS NEED NOT BE CHECKED) SLAB
- CHECK VAPOR BARRIER FOR PROPER THICKNESS AND PLACEMENT, ENSURE THAT THERE ARE NO PUNCTURES OR TEARS AND THAT THE EDGES HAVE BEEN LAPPED AND TAPED AS REQUIRED BY CONSTRUCTION DOCUMENTS.
- 3. INSPECT PLACEMENT OF SLAB REINFORCEMENT (I.E. REBAR OR WELDED WIRE FABRIC). ENSURE THAT REINFORCEMENT HAS BEEN PROPERLY CHAIRED OR SUPPORTED.
- 4. ENSURE THAT REINFORCING REMAINS AS PLACED AND IS NOT WALKED DOWN DURING CONCRETE
- 5. AFTER CONCRETE PLACEMENT, CHECK THAT PROVISION FOR SAWCUT AND CONSTRUCTION JOINTS HAVE BEEN MET. NOTE: SAM CUTTING OF SLAB MUST BE PERFORMED MITHIN SAME DAY AS PLACEMENT.

DOWELS FOR COLUMNS SHOULD BE CHECKED FOR ADEQUATE EMBEDMENT AND PROJECTION REINFORCING FOR COLUMNS SHOULD BE CHECKED FOR PROPER SIZES, ASSEMBLY AND ORIENTATION

OF ANY OFFSETS OR BENDS. SPECIAL INSPECTOR SHALL CHECK FOR THE GRADE OF REINFORCING

- REINFORCING CAGES SHOULD BE CHECKED AFTER ERECTION FOR PROPER ANCHORAGE TO THE DOMELS AND FOR PROPER SIZE AND QUANTITY. DOUBLE- CHECK LAPS FOR DOMELS AND FOR SIZE
- AFTER FORM PLACEMENT, THE REINFORCING CAGE SHOULD BE CHECKED FOR PROPER

CLEARANCE AND ADEQUATE BRACING TO KEEP IT IN PLACE DURING POURING. COLUMN FORMS

5. THE SPECIAL INSPECTOR SHALL CHECK SIZES (WIDTH AND DEPTH) OF REINFORCED CONCRETE

SUPPORTED SLABS/ BEAMS/ JOISTS

SHOULD BE BRACED ADEQUATELY.

- REINFORCING IN SLABS SHOULD BE CHECKED FOR QUANTITY, SIZE, SPACING, PROPER HEIGHTS AND PROPER BAR SUPPORTS. REINFORCING MATS SHOULD BE TIED AND SUPPORTED TO ENSURE IT WILL STAY IN POSITION UNDER TRAFFIC AND POURING. SPECIAL INSPECTOR SHALL CHECK FOR THE GRADE OF REINFORCING STEEL
- 2. CONCRETE BEAMS/JOISTS SHOULD BE CHECKED FOR SIZE, QUANTITY AND PROPER PLACEMENT OF REINFORCING. PROPER CLEARANCES BETWEEN STEEL AND FORMS SHOULD BE MAINTAINED. SPECIAL INSPECTOR SHALL CHECK FOR THE GRADE OF REINFORCING STEEL.
- 3 STIRRUPS AND TIES SHOULD BE CHECKED FOR SIZE, QUANTITY, PROPER BENDS AND PROPER PLACEMENT. STIRRUP SPACING WITH RESPECT TO SUPPORT LOCATIONS IS EXTREMELY IMPORTANT.
- 4. THE SPECIAL INSPECTOR SHALL CHECK SIZES (WIDTH AND DEPTH) OF REINFORCED CONCRETE SLABS, BEAMS, JOISTS AND GRADE OF REINFORCING STEEL.

- 1. OVERALL DIMENSIONS OF FORMS FOR PANEL CONFIGURATIONS SHOULD BE CHECKED FOR GENERAL CONFORMITY. PAY PARTICULAR ATTENTION TO THE PANEL THICKNESS(ES).
- REINFORCING STEEL SHOULD BE CHECKED FOR PROPER GRADE, SIZE, QUANTITY, SPACING, LOCATION AND CLEARANCE FROM FORMS. ENSURE THAT REINFORCING DESIGNATED "EACH FACE" IN THE DRAMINGS ARE NOT SIMPLY TIED TO THE CENTER MAT. THEY SHOULD BE CHAIRED SEPARATELY
- 3. ADDITIONAL REINFORCING FOR OPENINGS SUCH AS DOORS, WINDOWS AND MECHANICAL CHASES SHOULD BE CHECKED FOR PROPER SIZE, QUANTITY AND LOCATION.
- 4. CORNER BARS SHOULD BE CHECKED FOR PROPER SIZE, QUANTITY AND LENGTH. CORNER BARS ARE TO BE PLACED IN THE CENTER OF PANEL/WITHIN THE VERTICAL/HORIZONTAL CAGES. DO NOT PLACE DIAGONAL CORNER BARS ON THE OUTSIDE OF A CAGE OF STEEL
- 5. EMBEDS SHOULD BE CHECKED FOR SIZE, SPACING AND LOCATIONS AGAINST APPROVED STRUCTURAL STEEL AND JOIST SHOP DRAWINGS AND SHOULD BE SECURED IN PLACE PRIOR TO PLACEMENT OF CONCRETE.
- 6. ADDITIONAL REINFORCING FOR LIFTING AND LIFTING INSERTS SHOULD BE CHECKED FOR PROPER SIZE, QUANTITY, SPACING AND SHOULD BE SECURED IN PLACE PRIOR TO PLACEMENT OF CONCRETE.
- 7. REINFORCING CAGES FOR GIRDERS SHOULD BE CHECKED FOR PROPER SIZE, QUANTITY, SPACING OF BOTH VERTICALS AND STIRRUPS, AND CLEARANCE FROM FORMS.
- 8. ALL REINFORCING SHOULD BE CHECKED FOR PROPER CHAIRING OR SUPPORTS. PROPER CHAIRING SHOULD RESULT IN REINFORCING BEING SUPPORTED ENOUGH THAT THE REINFORCING CAN NOT BE DEFLECTED BY THE WEIGHT OF CONCRETE, PERSONNEL, OR ANY OTHER LOADS IMPOSED DURING PLACEMENT PROCEDURES.

MASONRY:

GENERAL

- 1. VERIFY TYPE OF MASONRY BLOCKS PROVIDED IS AS SPECIFIED.
- VERIFY COMPLIANCE OF MORTAR MIX WITH CONTRACT DOCUMENTS.
- VERIFY CONCRETE TIE BEAM AND/OR MASONRY BOND BEAM SIZE, LOCATION, VERTICAL SPACING, REINFORCING STEEL, CORNER BARS AND END CONDITION.

IS IN PLACE. OTHERMISE, VERIFY THAT MECHANICAL CONNECTORS SUCH AS DOVETAILS ARE PROVIDED.

- 4. CHECK TIE COLUMNS FOR SPACING, SIZE, AND SPECIFIED REINFORCING STEEL
- 5. VERIFY THAT CONCRETE FOR TIE COLUMNS AND TIE BEAMS IS TO BE PLACED AFTER BLOCK MASONRY
- 6. CHECK FOR HORIZONTAL REINFORCING, GROUTED CELLS, DOWELS AND INSERTS. CHECK FOR ANY CRACKED BLOCKS.
- 7. VERIFY LOCATION OF CONTROL JOINTS AND PROPER INSTALLATION OF JOINT ACCESSORIES.
- 8. VERIFY CELLS ARE FILLED WITH INSULATION, SAND, GROUT OR OTHER MATERIALS AS SPECIFIED.
- VERIFY REINFORCING STEEL FOR GRADE, SIZE AND FLAKING RUST.
- 2. CHECK THAT INSPECTION HOLES ARE CUT AT THE BOTTOM OF FILLED CELLS IF REQUIRED BY THE GROUTING DETAIL IN THE DRAWINGS.
- 3. VERIFY SPLICE LENGTH OF THE DOWELS.

REINFORCED MASONRY

- 4. VERIFY THAT VERTICAL STEEL IS PLACED IN THE SAME CELL AS DOWELS
- 5. CHECK GROUT FOR SPECIFIED STRENGTH, SLUMP AND AGGREGATE SIZE.
- 6. VERIFY TEST PRISMS FOR GROUT ARE MADE.

7. VERIFY THE GROUT IS PLACED IN STAGES AS PER GROUTING DETAIL IN THE DRAWINGS.

- 8. VERIFY BLOCKS ARE DRY BEFORE PLACING GROUT.
- 9. ENSURE END CELLS, CELLS ADJACENT TO OPENINGS, CORNERS AND AS OTHERWISE INDICATED ON THE DRAWINGS HAVE VERTICAL REINFORCING AND ARE FULLY GROUTED.
- 10. ENSURE SPACING OF GROUTED CELLS IS CORRECT
- 11. VERIFY EMBEDMENT OF VERTICAL STEEL INTO TIE BEAMS, BOND BEAMS OR SLABS ABOVE.
- 12. CHECK FOR YOIDS AT GROUTED CELLS.
- 13. INSPECT FOR CRACKS IN THE WALL AFTER A FEW DAYS.
- 14. DO NOT ALLOW TOOTHING OF WALL(S) AT END OF DAY'S WORK. WALL(S) SHALL BE STEPPED BACK AT END OF DAY.

STRUCTURAL STEEL:

- REVIEW THE CONSTRUCTION PROCEDURE IN PRE-CONSTRUCTION MEETING AND MAKE SURE THAT IT ACCOMMODATES THE DESIGN ASSUMPTIONS.
- REVIEM THE CONSTRUCTION DOCUMENTS AND MAKE SURE ALL THE DRAWINGS, SPECIFICATIONS, SHOP DRAWINGS, ADDENDUMS, SKETCHES ARE AVAILABLE AND
- APPROVED BY DESIGN ENGINEERS. VERIFY PREVIOUS INSPECTIONS FOR CORRECTIONS INDICATED ON THE CORRECTION
- CHECKLIST. 4. VERIFY ANCHOR BOLTS FOR SIZE, LENGTH, PLUMB, EMBEDMENT AND PROTRUSION OF
- THREADED END FOR NUT ENGAGEMENT. VERIFY THAT STEEL ON THE JOBSITE IS AS SHOWN ON THE PLANS AND SPECIFICATIONS
- NCLUDING SIZES AND SHAPES OF ALL MEMBERS. 6. VERIFY MILL CERTIFICATIONS AND TEST REPORTS, IF REQUIRED.
- VERIFY GRADE OF STEEL MEMBERS, PIPE, TUBING AND BOLTS FOR CONFORMANCE WITH SPECIFICATIONS.
- 8. INSPECT STEEL MEMBERS FOR DISTORTION, EXCESSIVE RUST, FLAMS AND BURNED
- 9. CHECK CAMBER IN BEAMS PRIOR TO THEM BEING ERECTED.
- 10. INSPECT FOR SURFACE FINISH GALVANIZED OR SHOP PAINT COAT. 11. CHECK THE COLUMNS FOR BEARING SURFACES, ALIGNMENT, SIZE, ADEQUATE BASE
- PLATES, SPLICE PLATES, BEARING AND EMBEDMENT IN MASONRY OR CONCRETE.
- 12. CHECK SPLICING FOR CONFORMANCE TO PLANS.

13. ENSURE THAT ENDS OF BEAMS BEARING ON MASONRY OR CONCRETE CONFORM TO THE

- DETAILS ON THE PLAN. CONNECTIONS TO EMBED PLATES ARE GENERALLY DETAILED TO ALLOW FOR HORIZONTAL SLIP. REVIEW CONSTRUCTION DOCUMENTS FOR SPECIFIC REQUIREMENTS.
- 14. VERIFY FIRE PROTECTION REQUIREMENTS ARE IN ACCORDANCE WITH CONTRACT
- DOCUMENTS. 15. CHECK APPROVED SHOP DRAWINGS AGAINST ACTUAL FIELD CONDITIONS.

MELDED CONNECTIONS

- VERIFY CERTIFICATION OF MELDERS, THEIR NAMES AND CERTIFICATE NUMBERS.
- 2. VERIFY TYPE OF ELECTRODES USED. 3. VERIFY TESTING HAS BEEN COMPLETED ACCORDING TO CONTRACT DOCUMENTS.

- BOLTED CONNECTIONS INSPECT BOLT HOLES TO VERIFY THE DIAMETER OF HOLE, PROPER HOLE ALIGNMENT, AND QUANTITY,
- LOCATION, SPACING, EDGE AND END DISTANCES.
- 2. CHECK BOLTS FOR PROPER SIZE, LENGTH, WASHERS, TYPE & GRADE OF BOLTS AND NUTS.
- 3. NOTE ANY OMISSION OF REQUIRED BOLTS.
- 4. VERIFY THAT METHODS USED IN APPLYING THE REQUIRED MINIMUM TORQUE. CALIBRATION OF THE TORQUE WRENCHES, OR LOAD INDICATORS CONFORM TO CONTRACT DOCUMENTS VERIFY THAT FINISHES ON CONTACT SURFACES BETWEEN CONNECTION PLATES ARE IN
- 6. CHECK FOR LOCK NUTS OR UPSET THREADS, IF SPECIFIED.
- CHECK FOR HARDENED WASHERS WHEN REQUIRED BY SPECIFICATIONS AND BEVELED MASHERS WHERE THE SURFACE OF NUT AND BOLT HEAD CONTACTS AND NOT SUFFICIENTLY
- 8. VERIFY GROUTING OF BASE PLATE.

ACCORDANCE WITH SPECIFICATIONS.

- 9. VERIFY THAT TESTING HAS BEEN COMPLETED ACCORDING TO CONTRACT DOCUMENTS. STEEL PAN STAIRS
- CHECK THAT STAIRS HAVE BEEN FABRICATED IN CONFORMANCE WITH THE REVIEWED STRUCTURAL STEEL SUBMITTALS.

2. CHECK STAIR CONNECTIONS TO SUPPORTING MEMBERS

ENSURE THAT MID-LANDINGS ARE PROPERLY CONNECTED. MASONRY MUST BE GROUTED SOLID

MINIMUM 4" ALL-AROUND BOLTS THAT SECURE THE MID-LANDINGS TO MASONRY WALLS

- 1. VERIFY METAL DECK TYPE, SIZE AND SURFACE FINISH.
- 2. VERIFY METAL DECK WELDING ELECTRODES
- 3. VERIFY USE OF WELD WASHERS IF REQUIRED.
- 4. VERIFY THAT REQUIRED WELDS ARE PROPERLY INSTALLED.
- 5. VERIFY THAT REQUIRED SIDELAP STITCH SCREWS ARE PROPERLY INSTALLED. 6. CHECK FOR ANY DAMAGE DURING TRANSPORTATION OR ERECTION.

COLD-FORMED (LIGHT GAUGE) METAL FRAMING:

- REVIEW THE CONSTRUCTION DOCUMENTS AND MAKE SURE ALL THE DRAWINGS
- SPECIFICATIONS, ADDENDUMS AND SKETCHES ARE AVAILABLE AND UP TO DATE. 2. VERIFY THAT SIGNED AND SEALED ENGINEERING SUBMITTALS FOR ALL SPECIALTY- ENGINEERED ITEMS HAVE BEEN REVIEWED BY THE ARCHITECT, BBM AND THE GENERAL CONTRACTOR/CM AND THAT THE "FOR CONSTRUCTION" SET IS BEING USED IN THE FIELD.
- 3. INSPECT LIGHT-GAUGE METAL STRUCTURAL MEMBERS FOR PROPER PROFILE AND MATERIAL GAUGE, LOCATION OF SPLICES, REINFORCEMENT WHEN STUDS AND PLATES ARE CUT AND ADEQUATE BEARING ON AND CONNECTION TO SUPPORTING MEMBERS.
- 4. VERIFY PROPER FASTENER SIZE, QUANTITY AND LOCATIONS
- VERIFY THAT BRACING OF STUDS AND JOISTS CONFORMS TO CONTRACT DOCUMENTS AND SHOP DRAWINGS. BRING ANY DISCREPANCIES TO THE ARCHITECT AND BBM'S ATTENTION

OPEN-WEB JOISTS AND JOIST GIRDERS:

- 1. VERIFY THAT MEMBERS ON THE JOBSITE ARE AS SHOWN ON THE PLANS AND SPECIFICATIONS INCLUDING SIZES AND SHAPES OF ALL MEMBERS.
- 2. INSPECT MEMBERS FOR DISTORTION, EXCESSIVE RUST, FLAMS, SMEEP AND TORCH CUT HOLES OR ANY OTHER ANOMALY THAT WILL DECREASE THE MEMBER'S STRENGTH.
- 3. INSPECT SURFACE FINISH FOR THE PROPER COATING (IF ANY).
- 4. CHECK THAT HORIZONTAL EMBED PLATES, IN MASONRY OR CONCRETE WALLS, THAT JOISTS AND JOIST GIRDERS WILL BEAR ON ARE SET FLUSH TO OR A MAXIMUM OF 1/2" FROM THE FACE OF THE WALL THAT THE MEMBER SPANS TO.
- 5. CHECK SPLICING FOR CONFORMANCE TO SUBMITTALS.
- 6. ENSURE THAT BEARING OF THE JOISTS AND JOIST GIRDERS ARE IN CONFORMANCE WITH THE STEEL JOIST INSTITUTE (SJI) SPECIFICATIONS.
- VERIFY FIRE PROTECTION REQUIREMENTS ARE IN ACCORDANCE WITH CONTRACT DOCUMENTS.
- 8. CHECK APPROVED SHOP DRAWINGS AGAINST ACTUAL FIELD CONDITIONS.
- AND THE CONSTRUCTION DRAWINGS. 10. CHECK THAT ALL ITEMS ATTACHED TO, SUSPENDED FROM OR BEARING ON THE JOISTS OR

9. CHECK BRIDGING FOR CONFORMANCE WITH THE STEEL JOIST INSTITUTE (SJI), THE SUBMITTALS

JOIST GIRDERS OCCUR AS SHOWN IN THE CONSTRUCTION DRAWINGS. LOCAL BENDING OF THE CHORD MAY OCCUR IF LOADS ARE NOT AT PANEL POINTS OR IF A FIELD WELDED WEB STIFFENER IS NOT ADDED.

MELDED CONNECTIONS

- 1. VERIFY CERTIFICATION OF WELDERS, THEIR NAMES AND CERTIFICATE NUMBERS
- 2. VERIFY TYPE OF ELECTRODES USED.
- 3. VERIFY TESTING HAS BEEN COMPLETED ACCORDING TO CONTRACT DOCUMENTS
- 1. CHECK BOLTS FOR PROPER SIZE, LENGTH, WASHERS, TYPE & GRADE OF BOLTS AND NUTS.
- 2. NOTE ANY OMISSION OF REQUIRED BOLTS.
- 3. CHECK THAT THE JOISTS NEAREST THE COLUMNS ARE FIELD BOLTED TO PROVIDE LATERAL STABILITY DURING ERECTION.
- 4. VERIFY THAT TESTING HAS BEEN COMPLETED ACCORDING TO CONTRACT DOCUMENTS.

- REVIEW THE CONSTRUCTION DOCUMENTS AND MAKE SURE ALL THE DRAWINGS, SPECIFICATIONS, ADDENDUMS AND SKETCHES ARE AVAILABLE AND UP TO DATE
- VERIFY THAT SIGNED AND SEALED ENGINEERING SUBMITTALS FOR ALL SPECIALTY- ENGINEERED ITEMS HAVE BEEN REVIEWED BY THE ARCHITECT, BBM AND THE GENERAL CONTRACTOR/CM AND THAT THE "FOR CONSTRUCTION" SET IS BEING USED IN THE FIELD.
- INSPECT GLAZING MEMBERS FOR PROPER PROFILE, MATERIAL, LOCATION OF SPLICES, REINFORCEMENT AND CONNECTIONS TO SUPPORTING MEMBERS.
- 4. VERIFY PROPER FASTENER SIZE, QUANTITY AND LOCATIONS
- VERIFY THAT BRACING CONFORMS TO CONTRACT DOCUMENTS AND SHOP DRAWINGS. BRING ANY DISCREPANCIES TO THE ARCHITECT AND BBM'S ATTENTION.

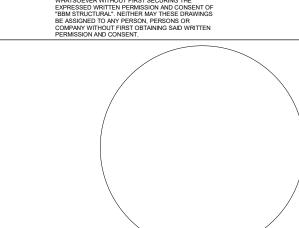
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Port St. Lucie, FL



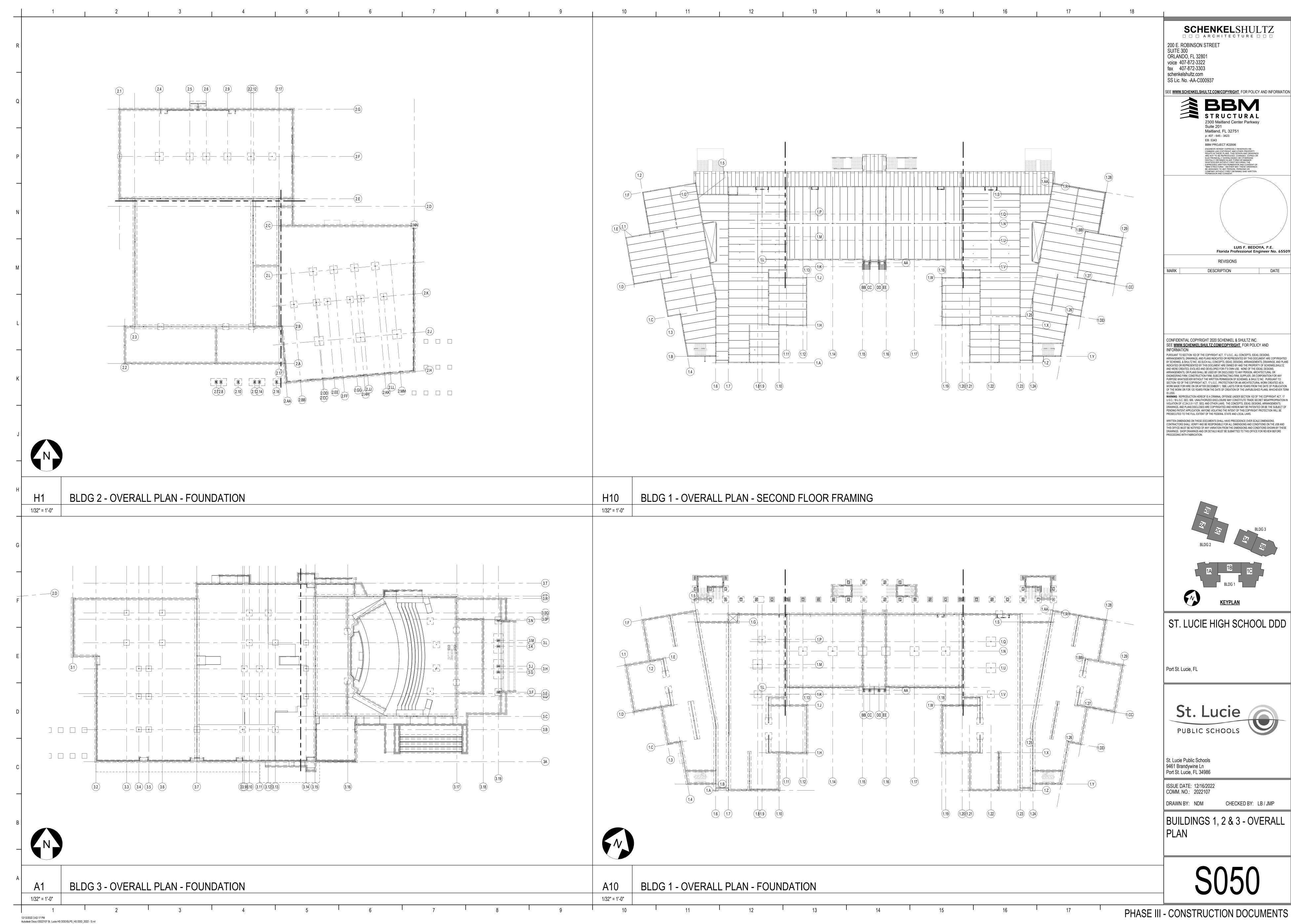
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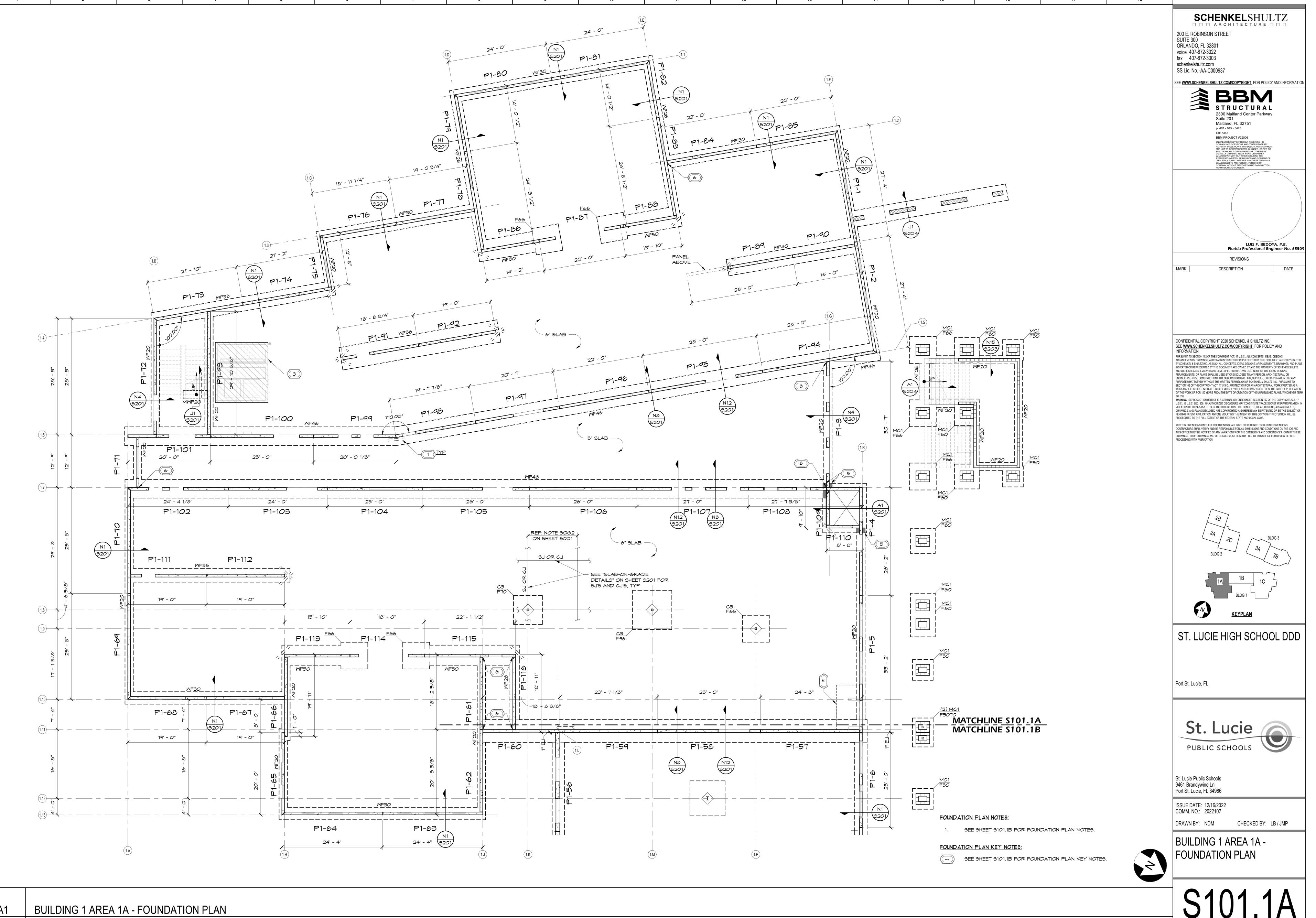
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THRESHOLD INSPECTION

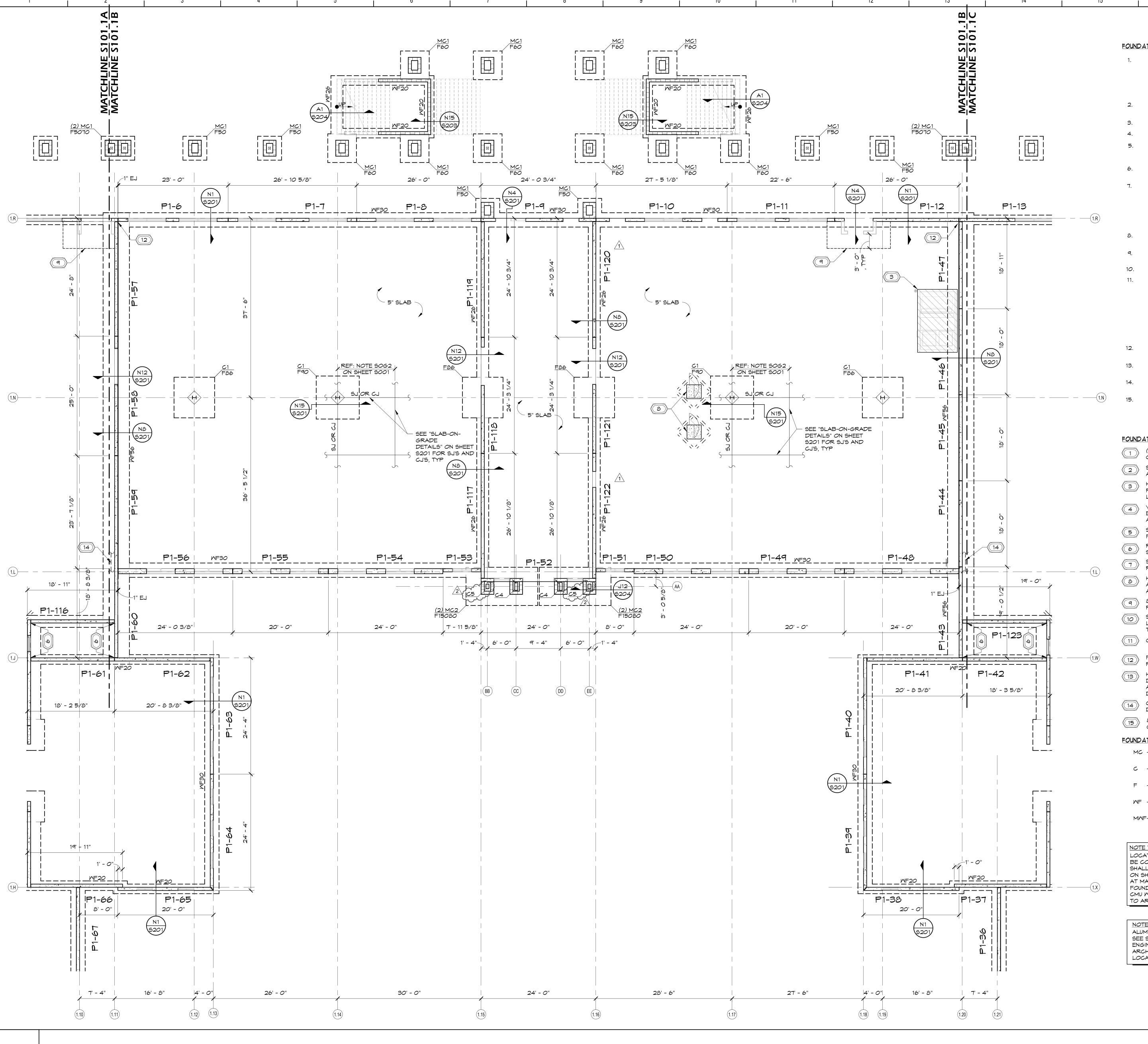
12" = 1'-0"





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FOUNDATION PLAN NOTES:

1. FLOOR SLAB SHALL BE 6" THICK CONCRETE REINF WITH 6x6-M2.9xM2.9 MMF. UNO IN ARCHITECTURAL SPECIFICATIONS, PROVIDE 10 MIL (MIN) VAPOR RETARDER (ASTM E 1745-11) ON COMPACTED SUBGRADE. SEE "SLAB-ON-GRADE DETAILS" ON SHEET S201 FOR PLACEMENT OF REINF.

2. T/ SLAB EL = 100'-0" (TYP, UNO). REFERENCE ONLY - SEE CIVIL DWGS FOR ACTUAL ELEVATION.

T/ WALL FTG EL = 98'-8" (TYP, UNO).

T/ COL FTG EL = 98'-8" (TYP, UNO).

SEE 5500 SERIES SHEETS FOR TILT-UP PANEL THICKNESSES. PANEL DESIGN INCLUDES UP TO A 3/4" DEEP REVEAL ALLOWANCE, TYP.

6. ALL TILT-UP PANELS ARE VIEWED FROM THE INSIDE OF THE BUILDING.

STEP AND/OR LOWER FOUNDATIONS WHERE SHOWN AND AS NECESSARY TO AVOID INTERFERENCE WITH OTHER TRADES. SEE CONCRETE GENERAL NOTES FOR DETAILS AND SECTIONS. PARTICULAR ATTENTION SHALL BE PAID TO DOWNSPOUTS ENSURING THAT PROPER ACTIONS HAVE BEEN TAKEN TO PREVENT PIPES FROM CONFLICTING WITH THE FOUNDATION

ALL FTGS ARE CENTERED BENEATH THE BEARING MALLS AND COLUMNS (TYP, UNO).

SEE SHEET S501 FOR PANEL REINF, EMBEDDED ITEMS AND JOINT DETAILS.

ALL CMU BEARING WALLS ARE 8" (TYP, UNO).

REINF LOAD-BEARING CMU WALLS WITH #5 VERT BAR CENTERED IN GROUT-FILLED CELL AT ENDS, CORNERS AND AT MAX SPACING OF 32" OC. PROVIDE (3) REINF CELLS AT ALL GIRDER BEARING LOCATIONS WHERE A TIE COLUMN IS NOT CALLED OUT. FOR REINF OF NON- LOAD-BEARING INTERIOR CMU WALLS, SEE SECTION J1/S202. ADDITIONAL FILLED CELLS REQUIRED AT JAMBS ARE SHOWN THUS "■" ON EACH STRUCTURAL PLAN. SEE "ILLUSTRATIVE PLAN OF VARIOUS CMU WALL CONDITIONS" ON SHEET SOO4 FOR ADDITIONAL REQUIRED FILLED CELLS AND OTHER INFORMATION.

TYP SPACING OF FILLED CELLS SHALL APPLY ABOVE AND BELOW OPENINGS ALSO.

SEE SHEETS SOO1 THR SOO5 FOR STRUCTURAL GENERAL NOTES.

MAINTAIN STRUCTURAL SLAB THICKNESS AT ALL FLOOR SLOPES AND DEPRESSIONS.

THE CONTRACTOR SHALL COORDINATE ALL UNDERGROUND UTILITIES, PIPES, ETC...WITH THE FOUNDATION PLAN AND FOUNDATION ELEVATIONS. FOOTING PENETRATION DETAILS MAY BE FOUND IN THE CONCRETE AND REINFORCING SECTION OF THE STRUCTURAL GENERAL NOTES.

FOUNDATION PLAN KEY NOTES:

(2) #4x4'-0" LONG @ 3" OC PLACED 2" CLR FROM CORNER, CENTERED IN SLAB (TYP WHERE SHOWN).

SHADED AREA INDICATES CONCRETE CURB, REF A8/S204.

HATCHED AREA INDICATES CONCRETE HOUSE-KEEPING PAD. REF ARCH & MECH DWGS FOR EXACT SIZE &

LOCATION. SEE A12/5202 FOR ADDITIONAL INFORMATION. VOLLEY BALL FLOOR INSERT SUPPORT. REF ARCH FOR EXACT QUANTITY & LOCATIONS. SEE E12/S201 FOR

ADDITIONAL INFORMATION. SHADED AREA INDICATES TURN DOWN WALL

FOUNDATION ON TOP OF ELEVATOR MAT, REF A1/S201 RECESS TILT-UP EMBED PLATES, FULL-HEIGHT OF PANEL,

SEE A15/S501 FOR ADDITIONAL INFORMATION. STAIR COLUMN & CONN TO SLAB BY STAIR MFR,

REF SE1/SO01.

HATCHED AREA INDICATES SLAB DEPRESSION. REF ARCH DWGS FOR EXACT SIZE & LOCATION. SEE A15/S202 FOR ADDITIONAL INFORMATION.

8" NON-LOAD-BEARING MASONRY WALL, REF J1/S202. PROVIDE 6" SLAB UNDER WALL.

SLOPE SLAB TO DRAIN, REF ARCH FOR EXACT LOCATION. MAINTAIN STRUCTURAL SLAB THICKNESS

THROUGHOUT ENTIRE LENGTH OF SLOPE.

 $\langle (11) \rangle$ COORD PIPE W/ PLUMBING, REF C9/5002.

FOOTING PENETRATION FOR ROOF DRAIN, COORD WITH PLUMBING DRAWINGS. REF OPTION 1A ON C9/5002.

HSS 3x3x1/4 SUPPORT FOR ROLL-UP DOORS. SEE DETAIL E8/5401 FOR ANCHORAGE AND SUPPORT. SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS AND

COORDINATE 8" POVERFLOW ROOF DRAIN SO THAT IT DOES NOT AFFECT TILT-UP COLUMN CAGE REINFORCING.

8" WIDE CIP CONC STEMWALL R/W #5 @ 12" OC EA WAY,

COORD OPENINGS IN WALL W/ EQUIP DWGS. FOUNDATION LEGEND:

MC - INDICATES MASONRY COLUMN. SEE DETAIL ON

SHEET S203 FOR INFO. C - INDICATES STEEL COLUMN. SEE SCHEDULE ON

SHEET S601 FOR INFO. F - INDICATES PAD FOOTING. SEE FOUNDATION SCHEDULE ON SHEET S601 FOR INFO.

WF - INDICATES WALL FOOTING. SEE FOUNDATION SCHEDULE ON SHEET S601 FOR INFO.

MWF- INDICATES MONOLITHIC WALL FOOTING. SEE FOUNDATION SCHEDULE ON SHEET S601 FOR INFO.

NOTE TO GC:

LOCATION OF MASONRY CONTROL JOINTS (MCJ) SHALL BE COORDINATED WITH ARCHITECTURAL DRAWINGS AND SHALL NOT EXCEED REQUIREMENTS OUTLINED IN NOTE M19 ON SHT SOO4. FOR ADDITIONAL FILLED CELLS REQUIRED AT MASONRY CONTROL JOINTS AND NOT SHOWN ON FOUNDATION PLANS, SEE "ILLUSTRATIVE PLAN OF VARIOUS CMU WALL CONDITIONS" ON SHT SOO4. SUBMIT MCJ PLAN TO ARCHITECT FOR APPROVAL.

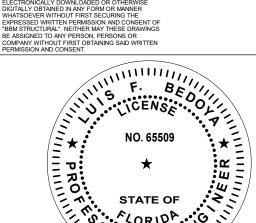
ALUMINUM CANOPY COLUMNS ARE NOT SHOWN. SEE SE5 ON SHEET SOO1 FOR SPECIALTY ENGINEERING REQUIREMENTS. REF: ARCHITECTURAL & CIVIL DRAWINGS FOR



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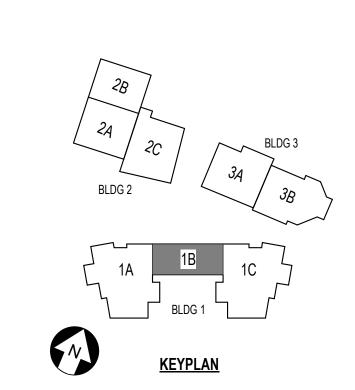


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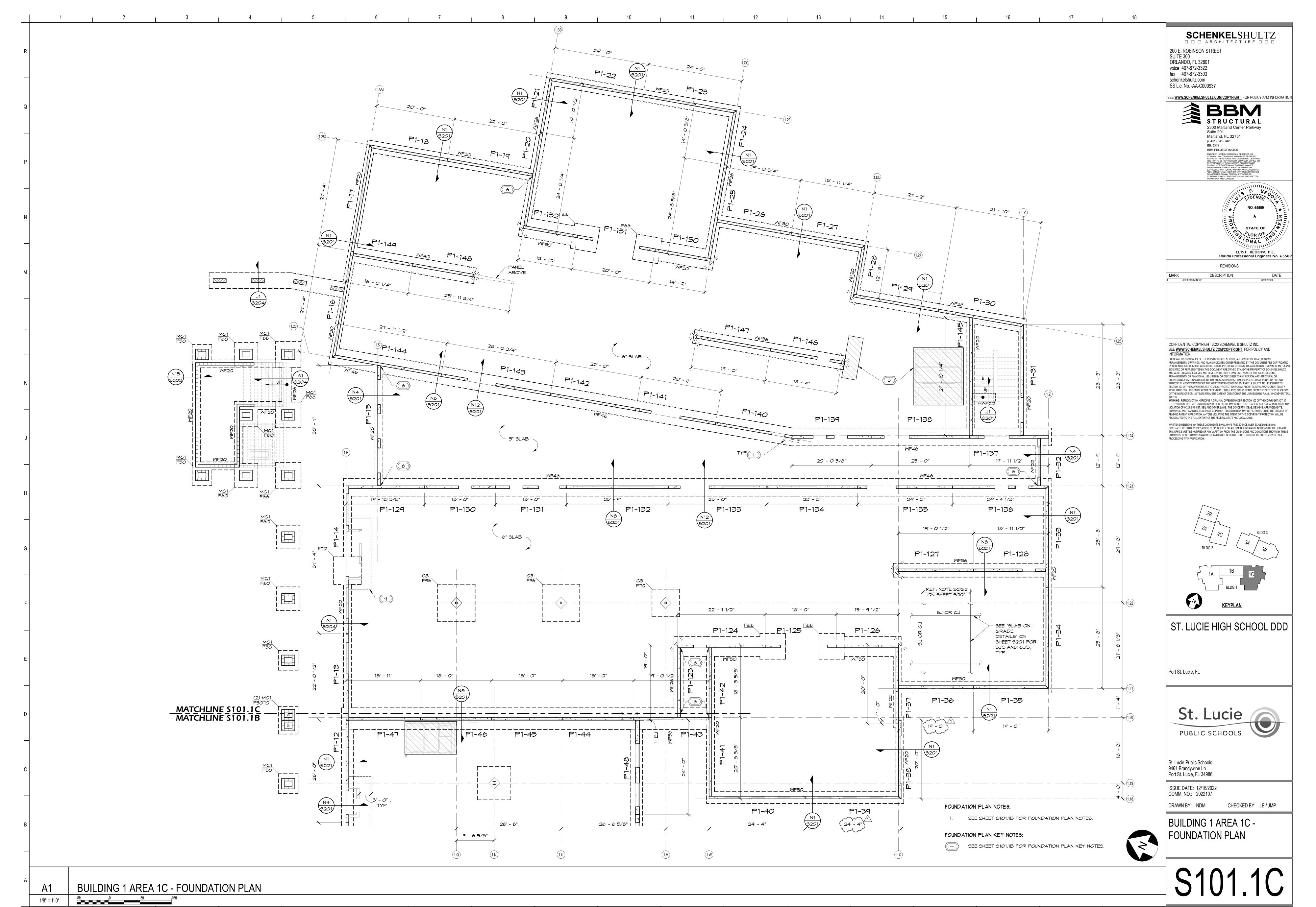


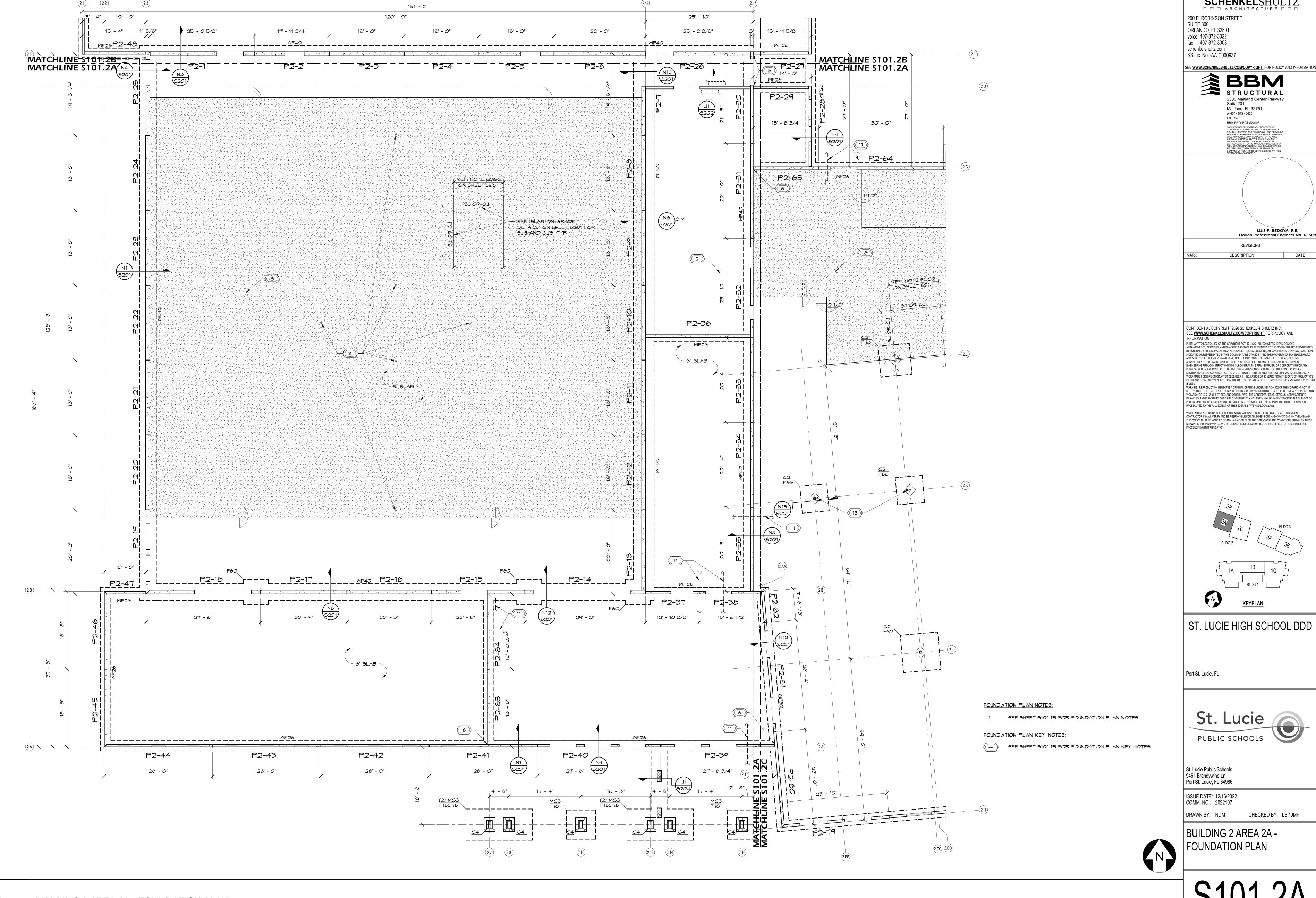
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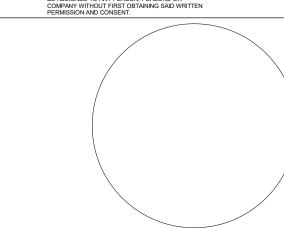
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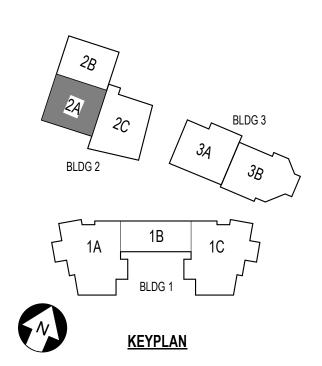


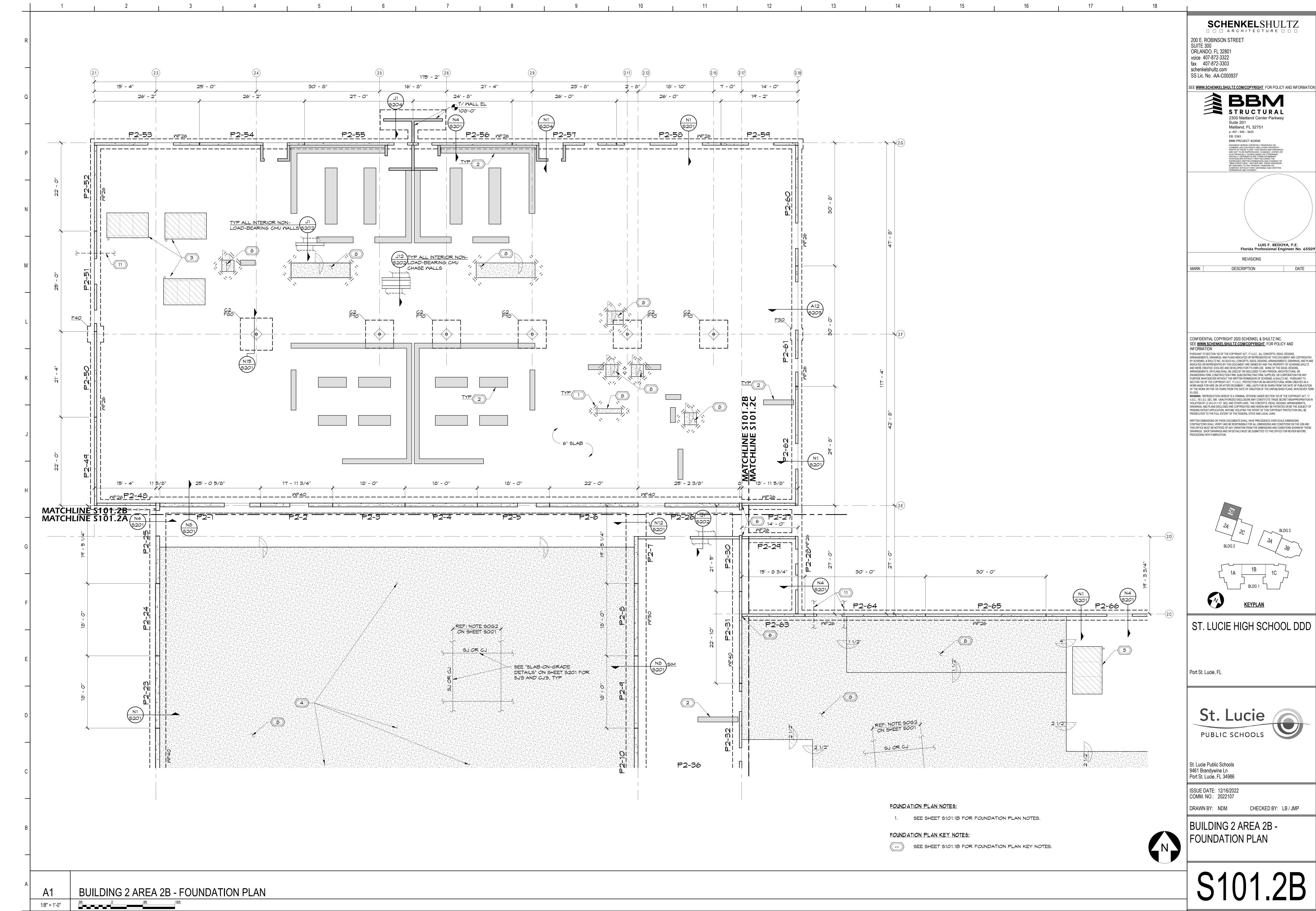
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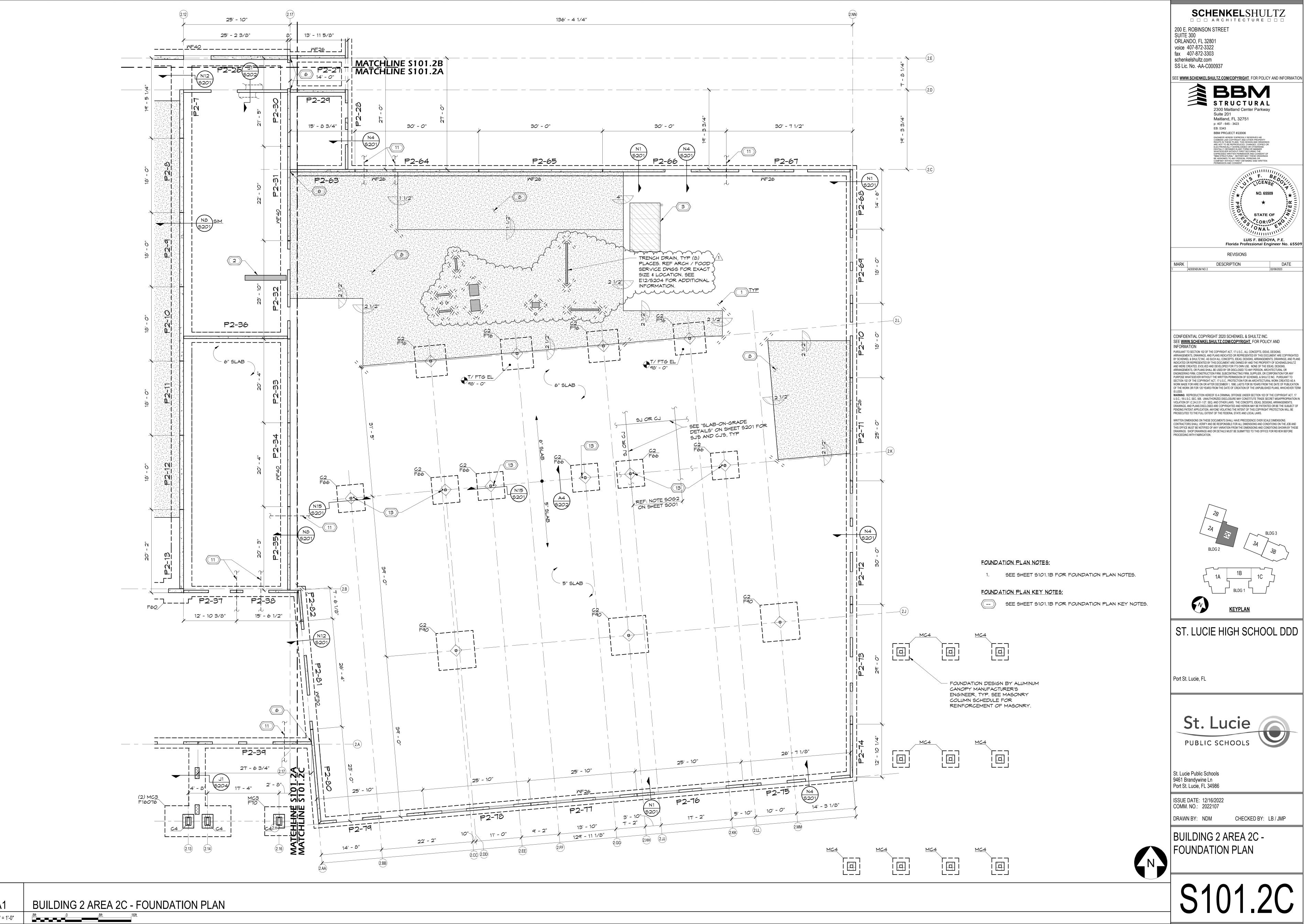
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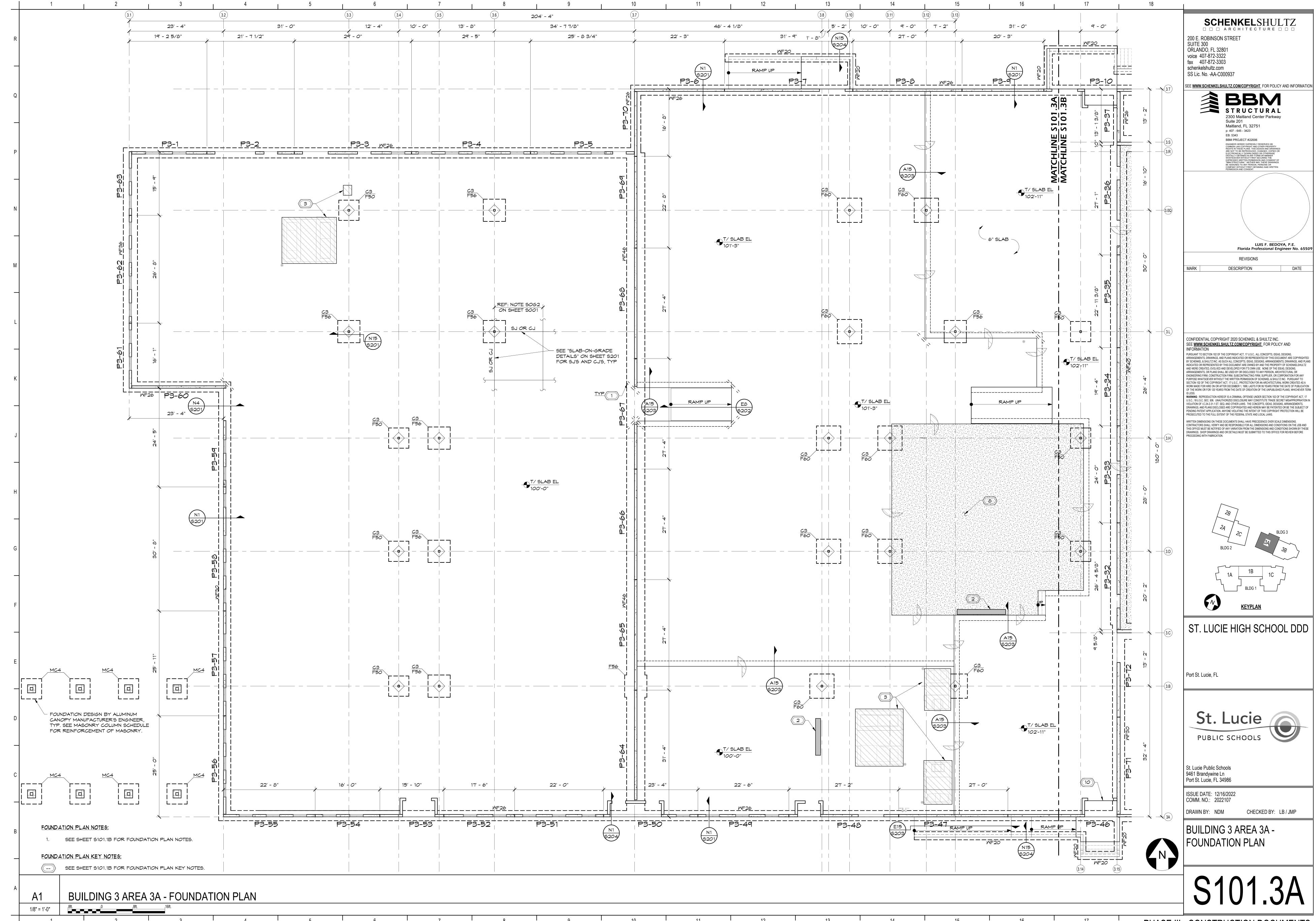


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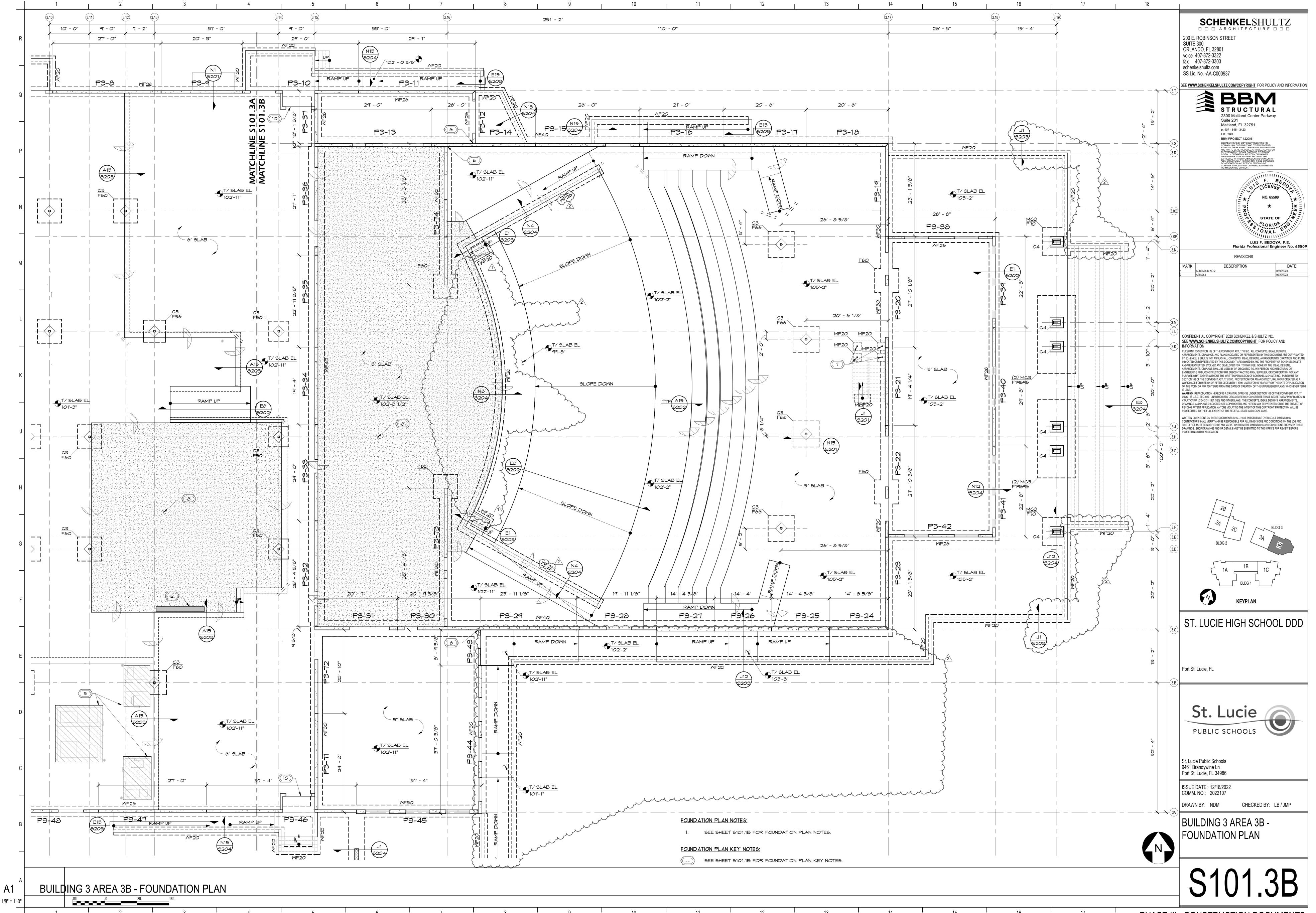






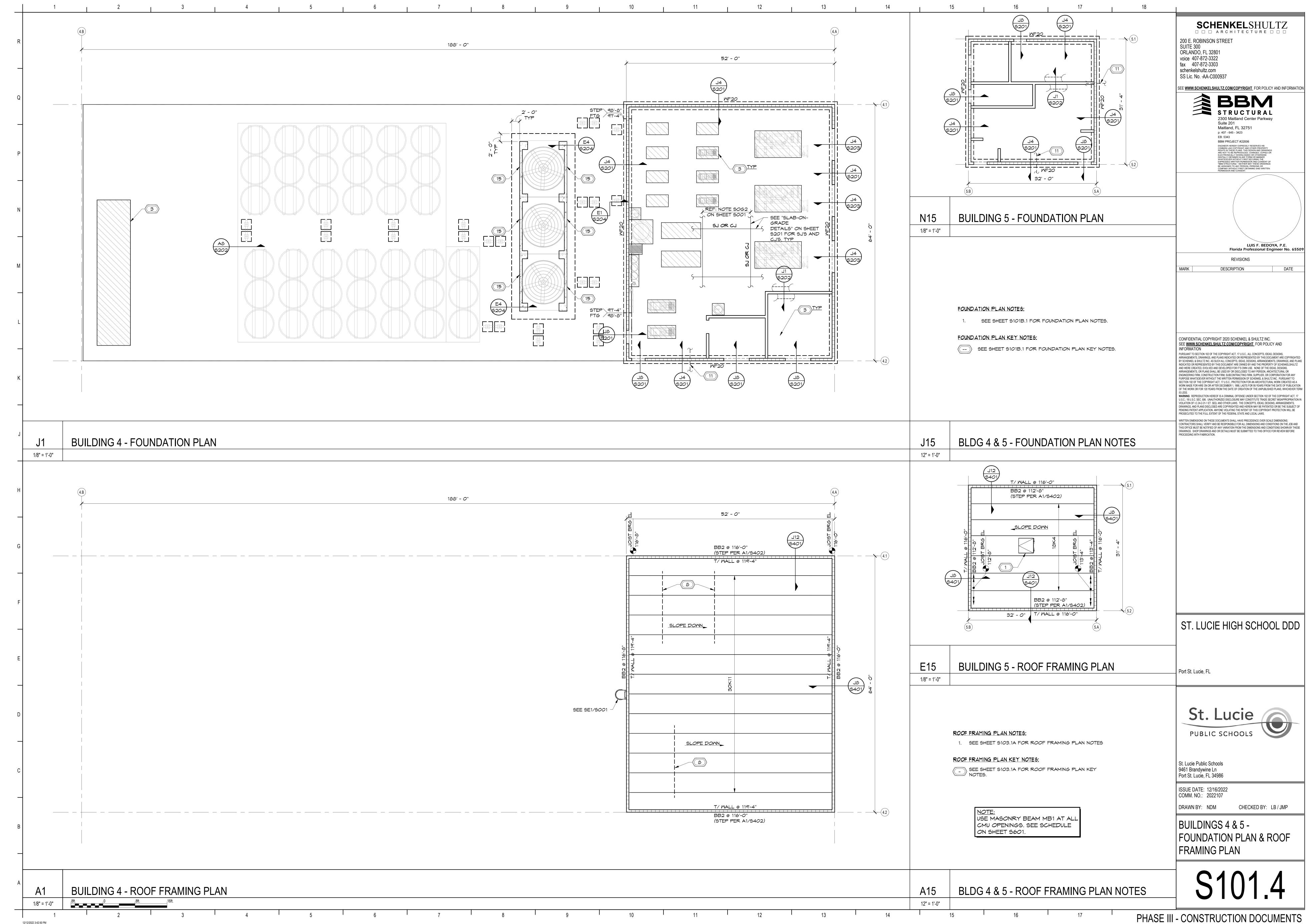
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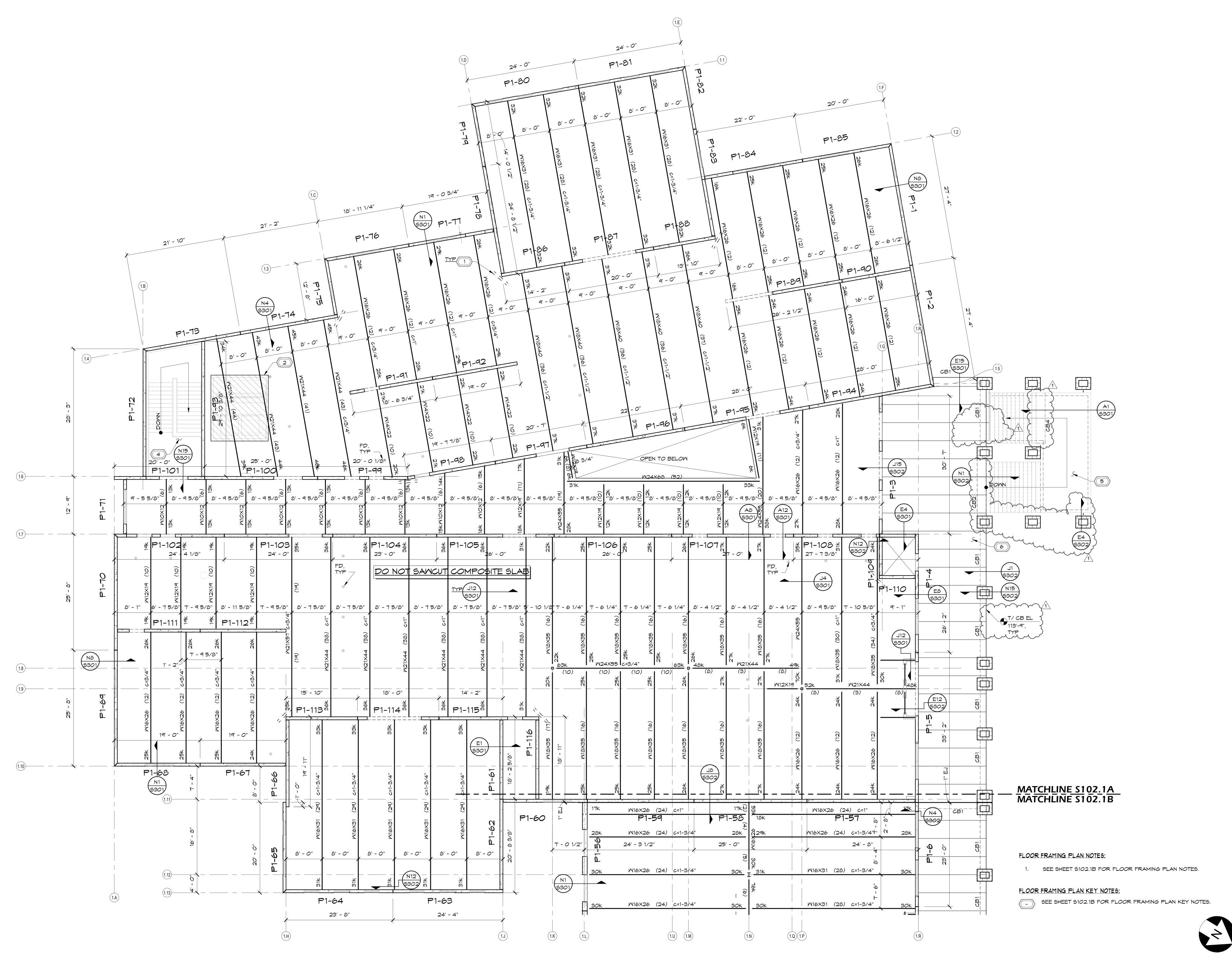
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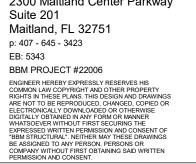
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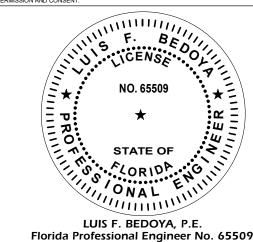




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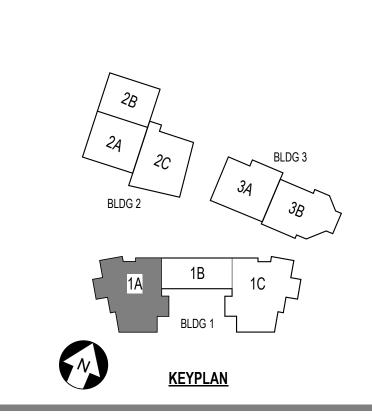


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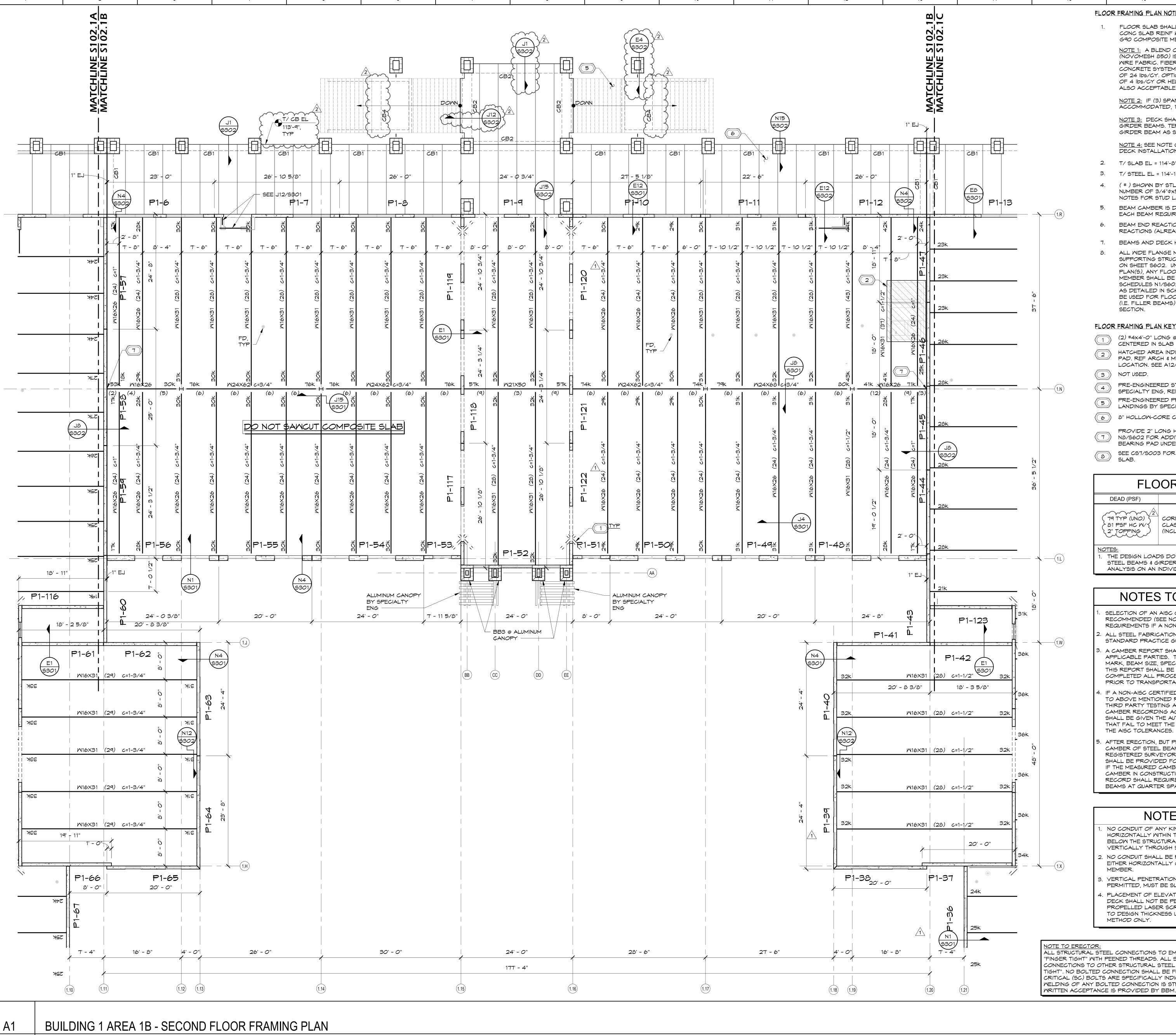
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BUILDING 1 AREA 1A -SECOND FLOOR FRAMING



FLOOR FRAMING PLAN NOTES:

1. FLOOR SLAB SHALL BE AN UNSHORED 6 1/2" (TOTAL DEPTH) CONC SLAB REINF W/ 6x6-W2.1xW2.1 WWF ON 2"-20ga GALV G90 COMPOSITE METAL DECK, (3) SPAN MIN.

NOTE 1: A BLEND OF STEEL AND POLYPROPYLENE FIBERS (NOVOMESH 850) IS AN ACCEPTABLE ATERNATIVE TO WELDED-WIRE FABRIC. FIBERS SHALL BE AS MANUFACTURED BY PROPEX CONCRETE SYSTEMS (OR APPROVED EQUAL) APPLIED AT A RATE OF 24 lbs/CY. OPTIONALLY FIBERMESH 650 APPLIED AT A RATE OF 4 lbs/CY OR HELIX 5-25 APPLIED AT A RATE OF 7 lbs/CY ARE ALSO ACCEPTABLE.

NOTE 2: IF (3) SPAN CONFIGURATION CANNOT BE ACCOMMODATED, 18ga GALV G90 MTL DECK MUST BE USED.

NOTE 3: DECK SHALL NOT BE PLACED CONTINUOUS OVER THE GIRDER BEAMS. TERMINATE AND START DECK ON EACH SIDE OF GIRDER BEAM AS SHOWN ON NOTE CS6 ON SHT SO03.

NOTE 4: SEE NOTE CS8 ON SHEET SO03 FOR COMPOSITE FLOOR DECK INSTALLATION REQUIREMENTS.

2. T/ SLAB EL = 114'-8"

T/ STEEL EL = 114'-1 1/2"

(#) SHOWN BY STL BEAM CALLOUT ON PLAN INDICATES NUMBER OF 3/4"\$x5" LONG HEADED STUDS (SEE GENERAL NOTES FOR STUD LAYOUT ON BEAMS).

BEAM CAMBER IS DESIGNATED AS "C=" FOR EACH BEAM REQUIRING CAMBER.

BEAM END REACTIONS AS SHOWN ON PLAN ARE ULTIMATE REACTIONS (ALREADY FACTORED).

BEAMS AND DECK HAVE BEEN DESIGNED TO BE UNSHORED.

ALL WIDE FLANGE MEMBERS SHALL BE CONNECTED TO THE SUPPORTING STRUCTURE AS DETAILED IN CONNECTION SCHEDULES ON SHEET 5602. UNLESS SPECIFICALLY NOTED OTHERWISE ON PLAN(5), ANY FLOOR MEMBER SUPPORTING ANOTHER FLOOR MEMBER SHALL BE CONNECTED AS DETAILED IN DOUBLE SHEAR SCHEDULES N1/5602 AND N8/5602. SINGLE SHEAR CONNECTIONS AS DETAILED IN SCHEDULES J1/S602 AND J12/S602 SHALL ONLY BE USED FOR FLOOR MEMBERS SUPPORTING DECK/SLAB ONLY (I.E. FILLER BEAMS) OR AS SPECIFICALLY IDENTIFIED ON PLAN OR

FLOOR FRAMING PLAN KEY NOTES:

(2) #4x4'-0" LONG @ 3" OC PLACED 2" CLR FROM CORNER,

CENTERED IN SLAB (TYP WHERE SHOWN). HATCHED AREA INDICATES CONCRETE HOUSE-KEEPING

PAD. REF ARCH & MECH DWGS FOR EXACT SIZE & LOCATION. SEE A12/S202 FOR ADDITIONAL INFORMATION.

PRE-ENGINEERED STEEL PAN STAIRS & LANDINGS BY SPECIALTY ENG, REF SE1/SOO1.

PRE-ENGINEERED PRECAST CONCRETE STAIRS &

LANDINGS BY SPECIALTY ENG, REF SE1/SOO1.

8" HOLLOW-CORE CONCRETE PLANKS.

PROVIDE 2" LONG HORIZ HOLES IN STEEL BEAM. SEE N8/S602 FOR ADDITIONAL INFORMATION. USE PTFE BEARING PAD UNDER STEEL BEAM BEARING ON ANGLE

SEE CS7/S003 FOR ADDITIONAL PLATES AT EDGE OF

FLOOR DESIGN LOADS

DEAD (PSF) LIVE (PSF) OTHER (PSF) 79 TYP (UNO) CORRIDORS = 80 STAIRS & EXITMAYS > 81 PSF HC M/ \(\) CLASSROOMS = 65 MALKMAYS = 100 (INCLUDES 15 PARTITIONS) | MECHANICAL = 150

THE DESIGN LOADS DO NOT INCLUDE THE SELF WEIGHT OF THE STEEL BEAMS & GIRDERS. THEY ARE ACCOUNTED FOR IN THE ANALYSIS ON AN INDIVIDUAL BASIS.

NOTES TO GC& OWNER:

SELECTION OF AN AISC CERTIFIED FABRICATOR IS HIGHLY RECOMMENDED (SEE NOTE 4 BELOW FOR ADDITIONAL REQUIREMENTS IF A NON-AISC FABRICATOR IS SELECTED). ALL STEEL FABRICATION SHALL MEET AISC TOLERANCES AND

STANDARD PRACTICE GUIDELINES. A CAMBER REPORT SHALL BE PROVIDED FOR REVIEW BY ALL APPLICABLE PARTIES. THIS REPORT SHALL TABULATE BEAM

MARK, BEAM SIZE, SPECIFIED CAMBER AND INDUCED CAMBER. THIS REPORT SHALL BE PREPARED AFTER THE STEEL HAS COMPLETED ALL PROCESSES OF FABRICATION AND IMMEDIATELY PRIOR TO TRANSPORTATION. IF A NON-AISC CERTIFIED FABRICATOR IS SELECTED, IN ADDITION

TO ABOVE MENTIONED REQUIREMENTS THE OWNER SHALL HIRE A THIRD PARTY TESTING AND INSPECTION AGENCY TO WITNESS THE CAMBER RECORDING ACTIVITY AS MENTIONED ABOVE AND SHALL BE GIVEN THE AUTHORITY TO STOP SHIPMENT OF BEAMS THAT FAIL TO MEET THE SPECIFIED CAMBER PLUS OR MINUS (+/-) THE AISC TOLERANCES.

AFTER ERECTION, BUT PRIOR TO PLACEMENT OF THE CONCRETE CAMBER OF STEEL BEAMS SHALL BE MEASURED BY A REGISTERED SURVEYOR HIRED BY THE GC/CM AND REPORT SHALL BE PROVIDED FOR REVIEW BY ALL APPLICABLE PARTIES. IF THE MEASURED CAMBER IS LESS THAN 75% OF SPECIFIED CAMBER IN CONSTRUCTION DOCUMENTS, THE ENGINEER OF RECORD SHALL REQUIRE THE GC/CM TO PLACE SHORES UNDER BEAMS AT QUARTER SPAN POINTS.

NOTES TO GC:

NO CONDUIT OF ANY KIND SHALL BE PERMITTED TO RUN HORIZONTALLY WITHIN THE SLAB. CONDUIT MUST BE RUN BELOW THE STRUCTURAL MEMBERS AND COME UP VERTICALLY THROUGH SLAB WHERE NECESSARY. NO CONDUIT SHALL BE PERMITTED TO RUN THROUGH,

EITHER HORIZONTALLY OR VERTICALLY, ANY STRUCTURAL

VERTICAL PENETRATIONS THROUGH THE SLAB, WHERE PERMITTED, MUST BE SLEEVED.

4. PLACEMENT OF ELEVATED CONCRETE SLABS ON METAL DECK SHALL NOT BE PERFORMED USING ANY SELF PROPELLED LASER SCREED EQUIPMENT. PLACE CONCRETE TO DESIGN THICKNESS USING "DIPSTICK/STORY-POLE" METHOD ONLY.

ALL STRUCTURAL STEEL CONNECTIONS TO EMBED PLATES SHALL BE FINGER TIGHT" WITH PEENED THREADS. ALL STRUCTURAL STEEL CONNECTIONS TO OTHER STRUCTURAL STEEL MEMBERS SHALL BE "SNUG FIGHT". NO BOLTED CONNECTION SHALL BE FULLY TENSIONED UNLESS SLIP CRITICAL (SC) BOLTS ARE SPECIFICALLY INDICATED. ADDITIONALLY, FIELD WELDING OF ANY BOLTED CONNECTION IS STRICTLY PROHIBITED UNLESS



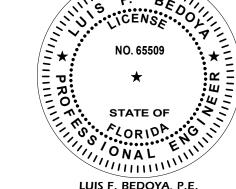


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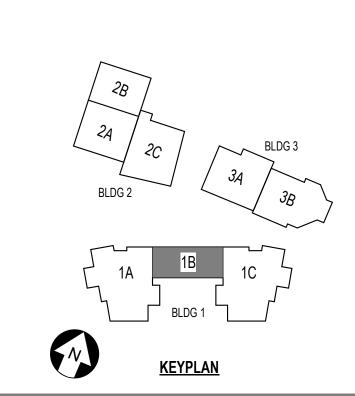
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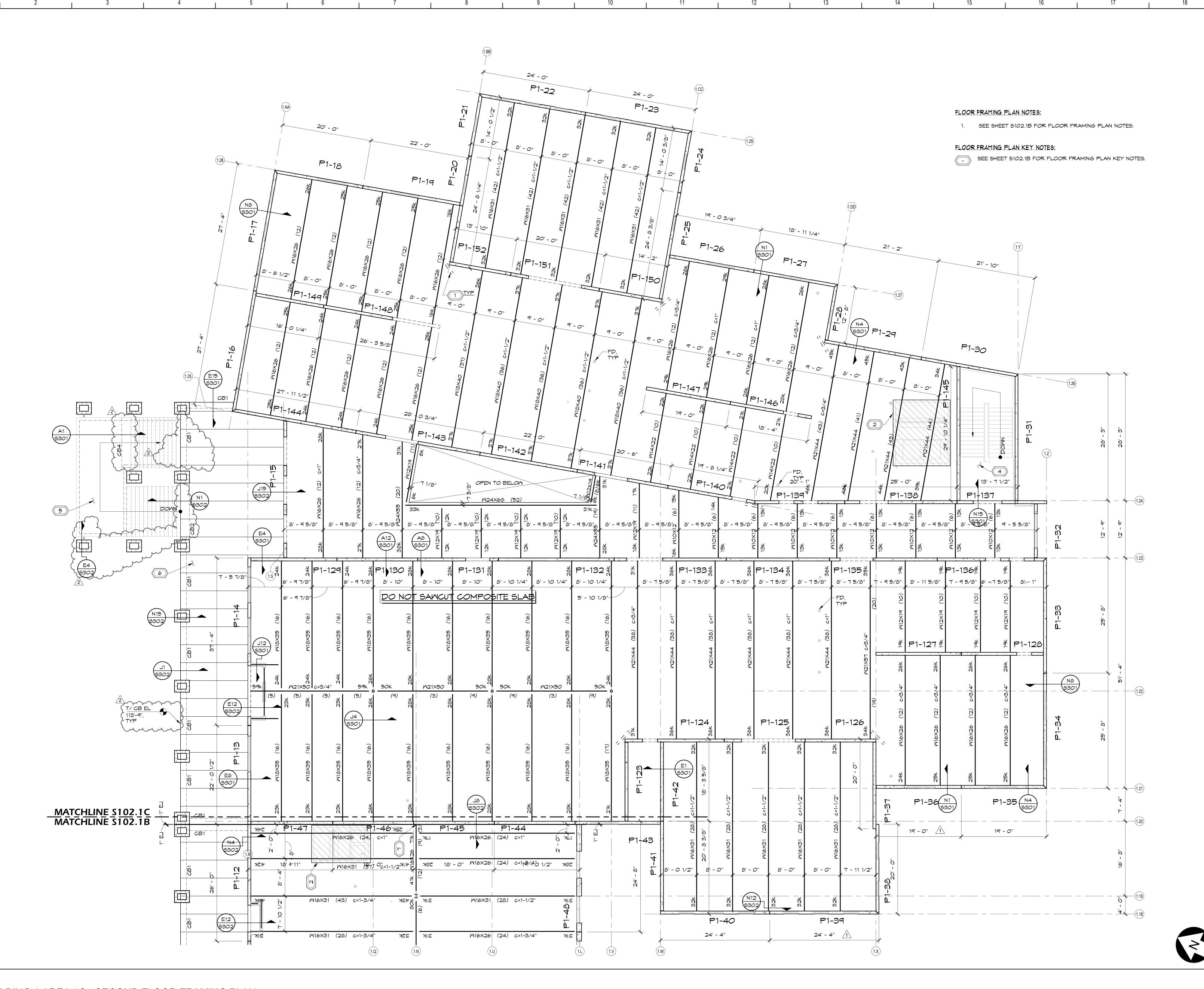
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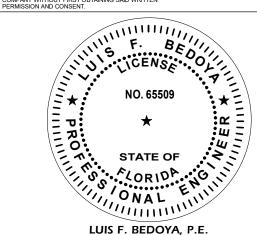
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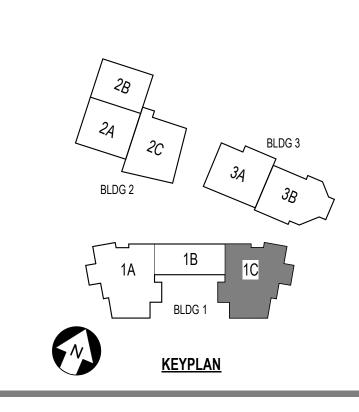
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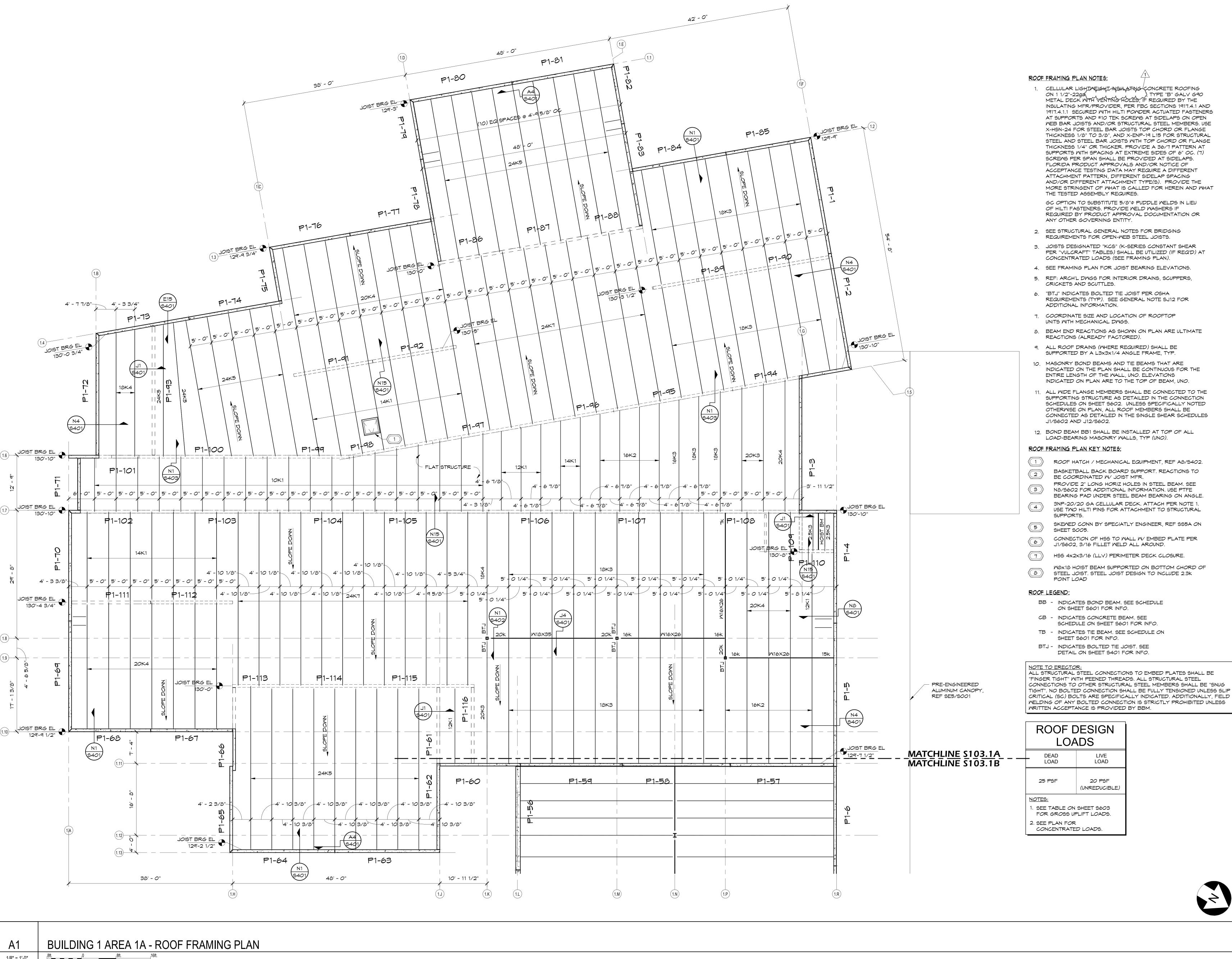
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BUILDING 1 AREA 1C -SECOND FLOOR FRAMING PLAN

S102.1C

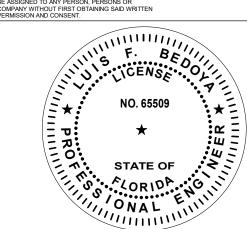


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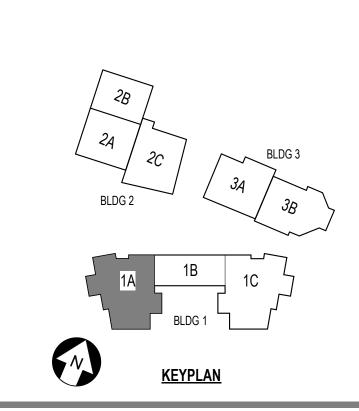
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ST. LUCIE HIGH SCHOOL DDD

Port St. Lucie, Fl



St. Lucie Public Schools 9461 Brandywine Ln Port St. Lucie, FL 34986

Port St. Lucie, FL 34986

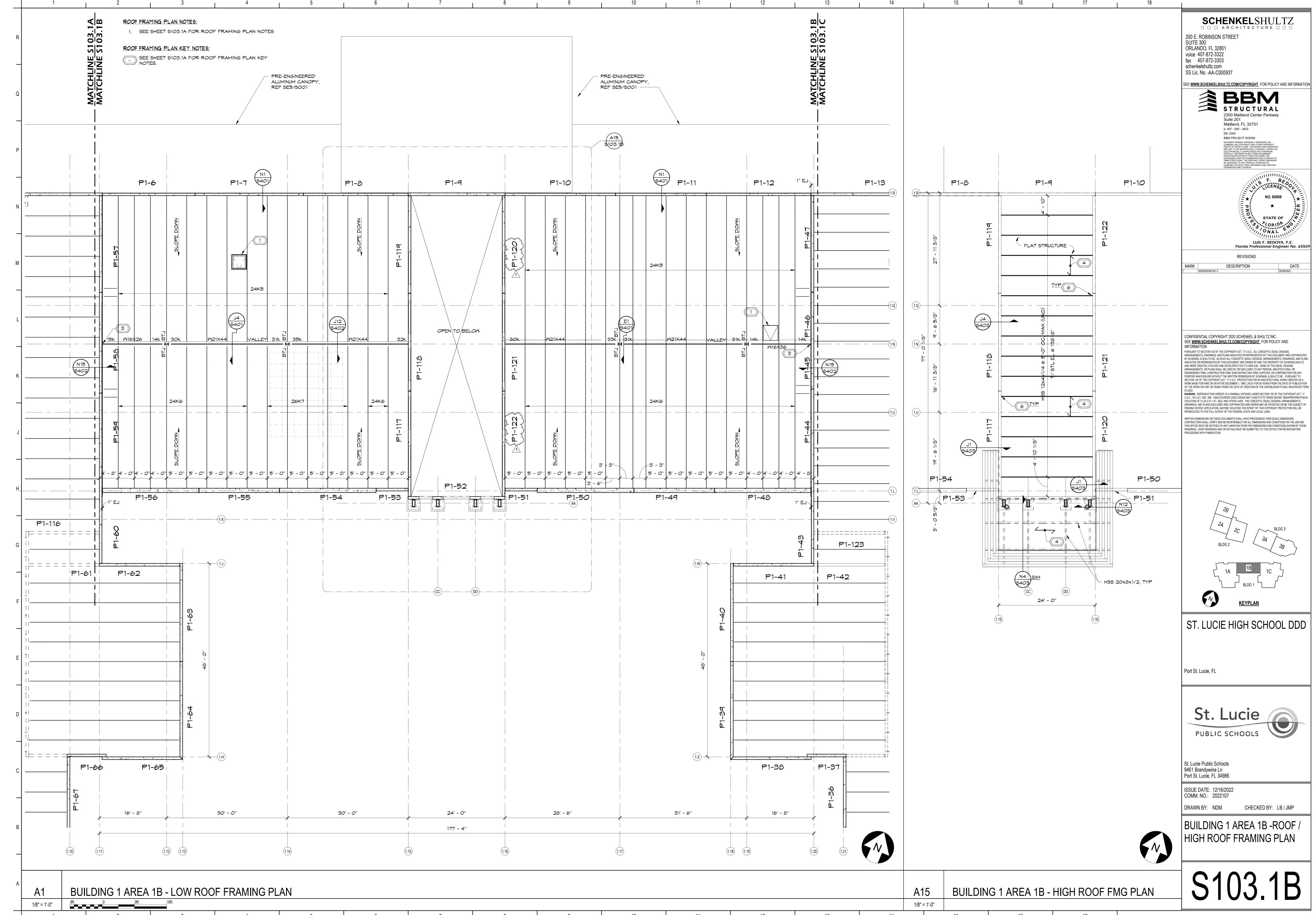
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BUILDING 1 AREA 1A - ROOF FRAMING PLAN

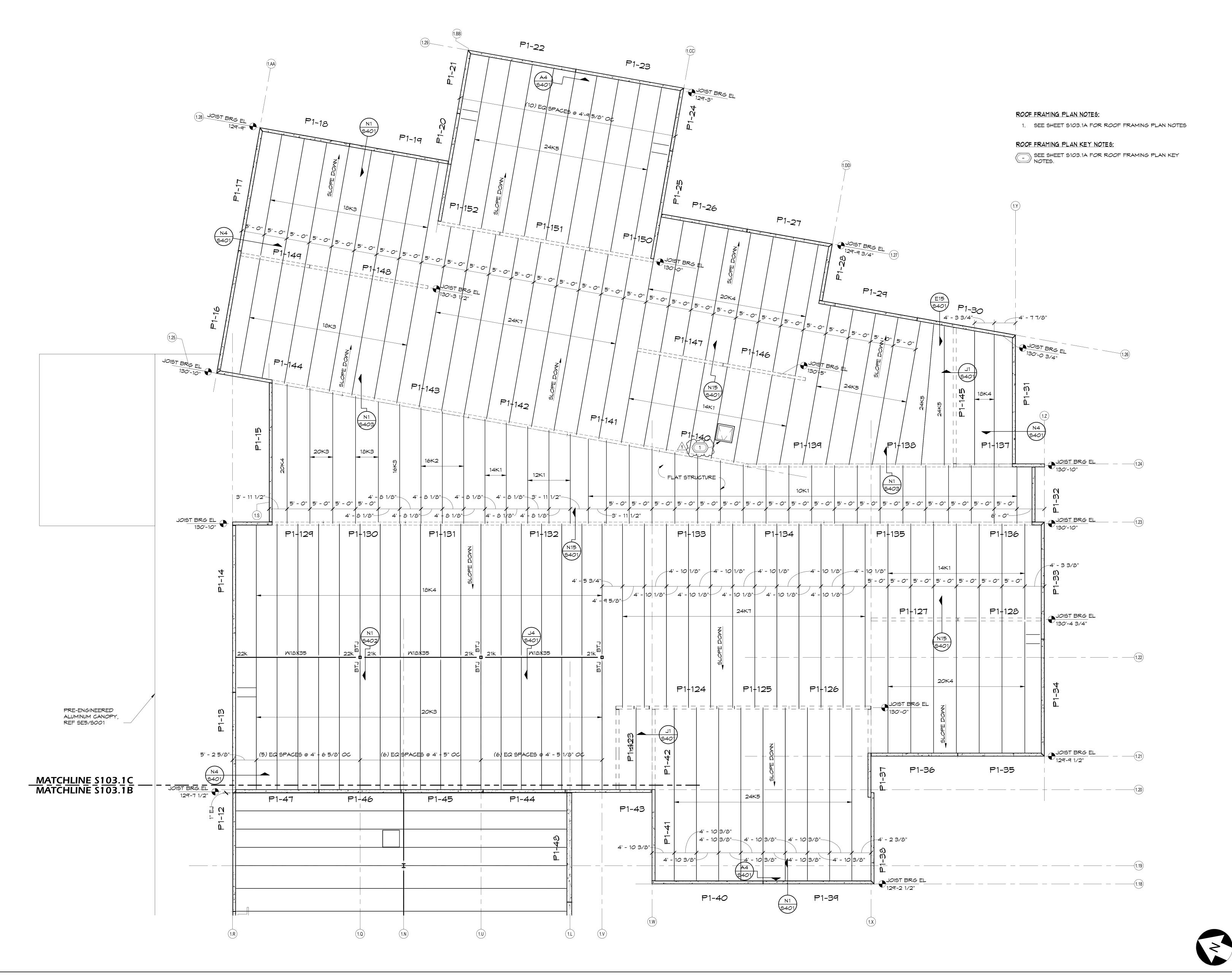
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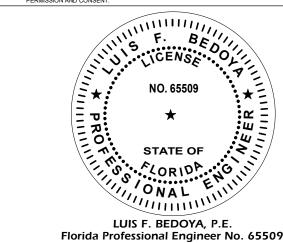
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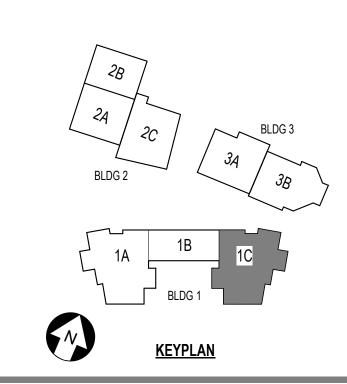
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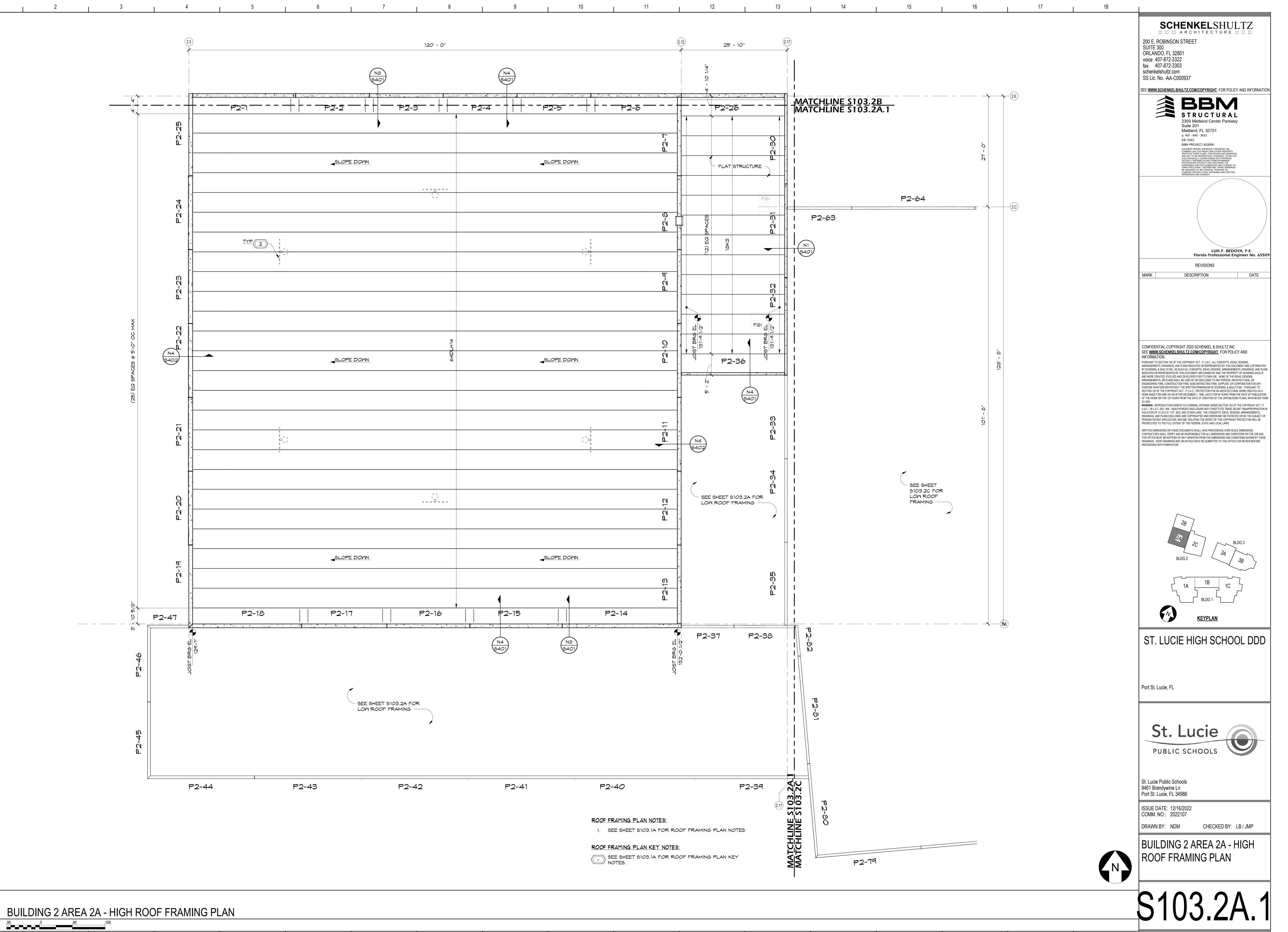


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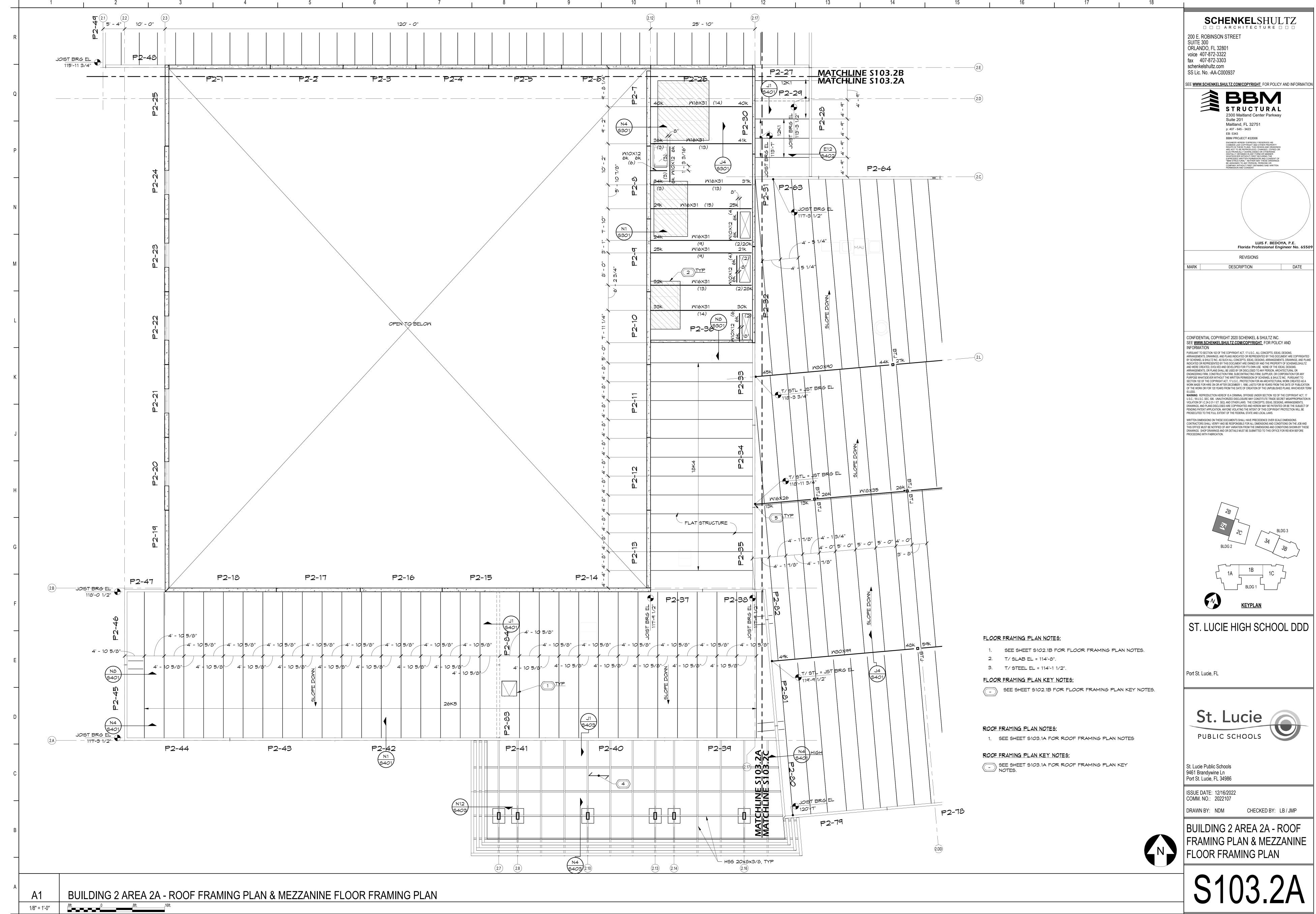
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BUILDING 1 AREA 1C - ROOF FRAMING PLAN



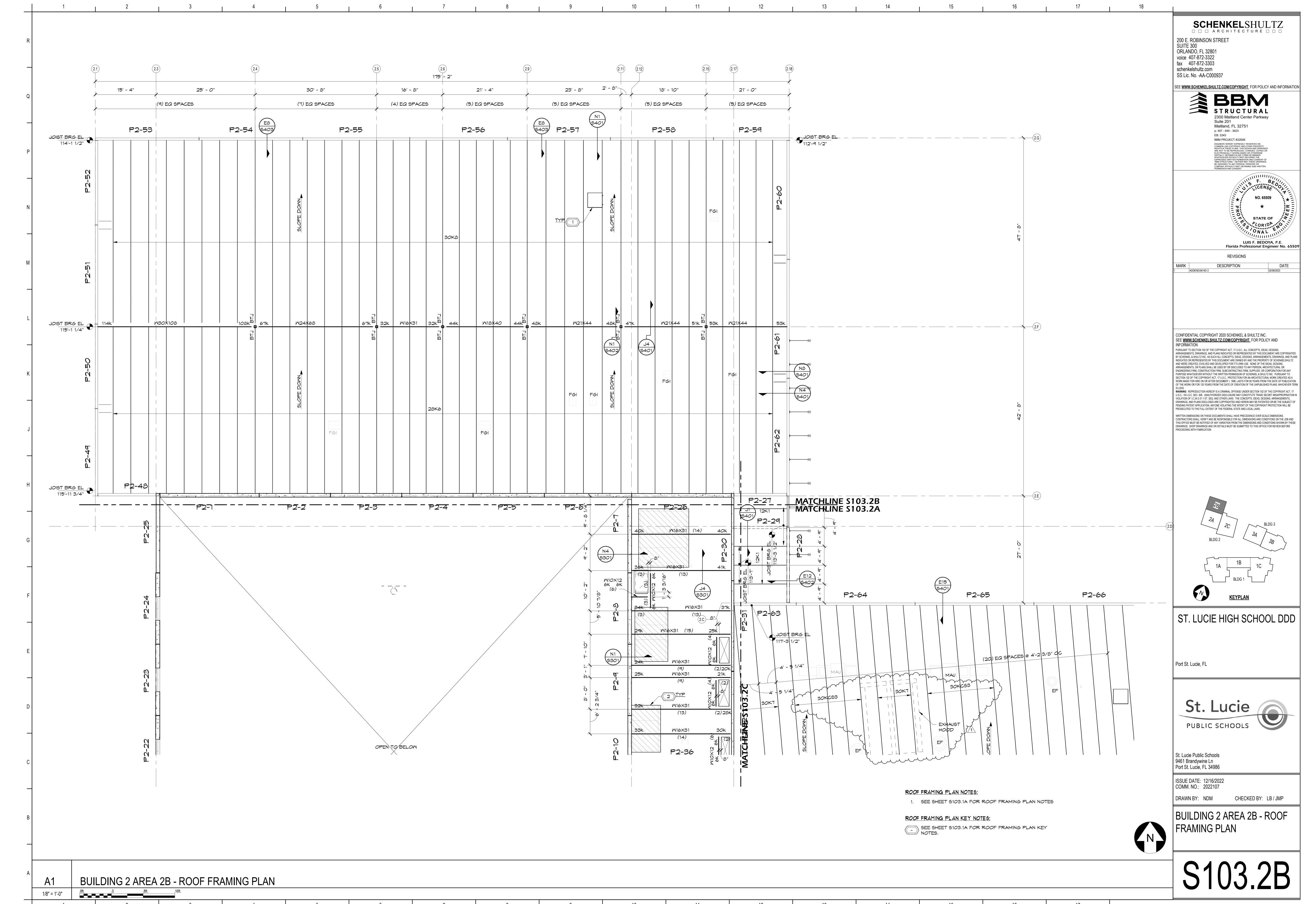
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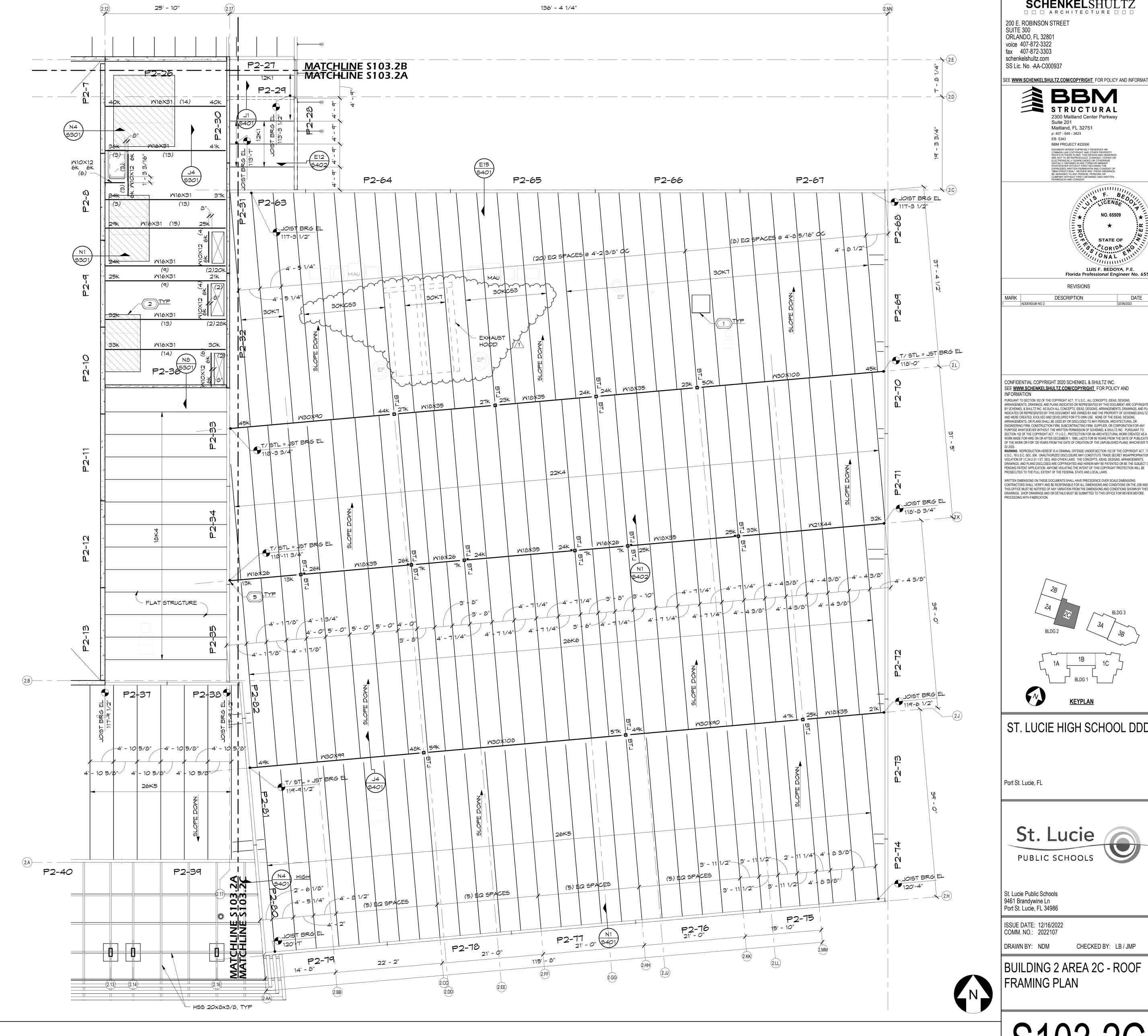
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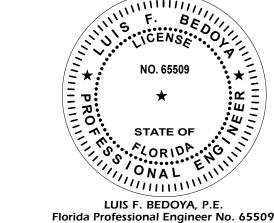
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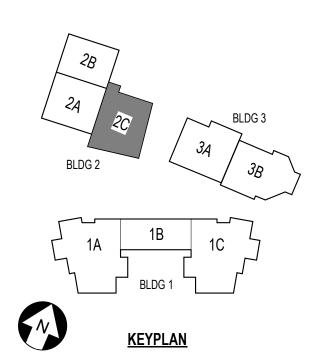


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CHECKED BY: LB / JMP BUILDING 2 AREA 2C - ROOF

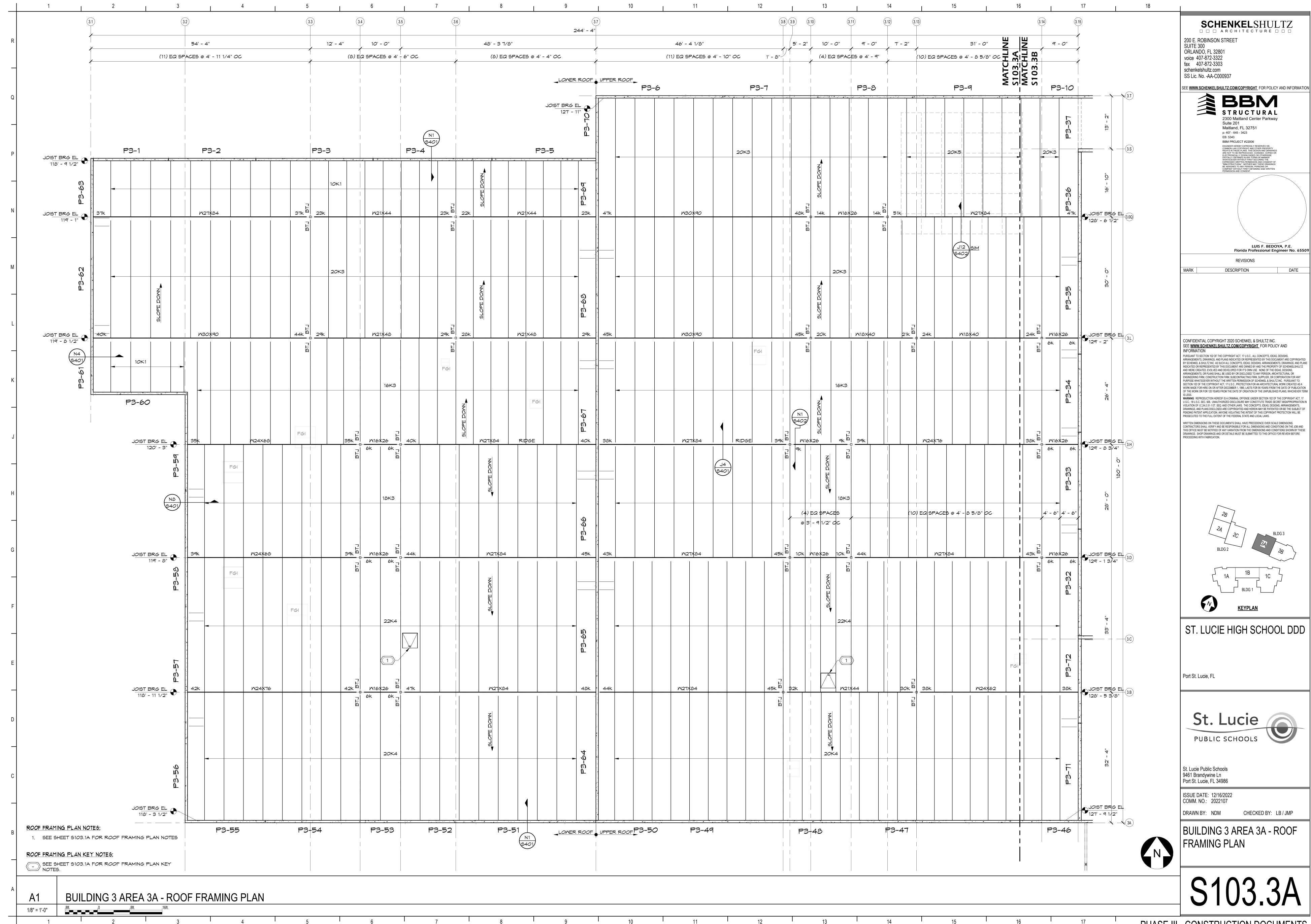
BUILDING 2 AREA 2C - ROOF FRAMING PLAN off.

1. SEE SHEET S103.1A FOR ROOF FRAMING PLAN NOTES

SEE SHEET S103.1A FOR ROOF FRAMING PLAN KEY NOTES.

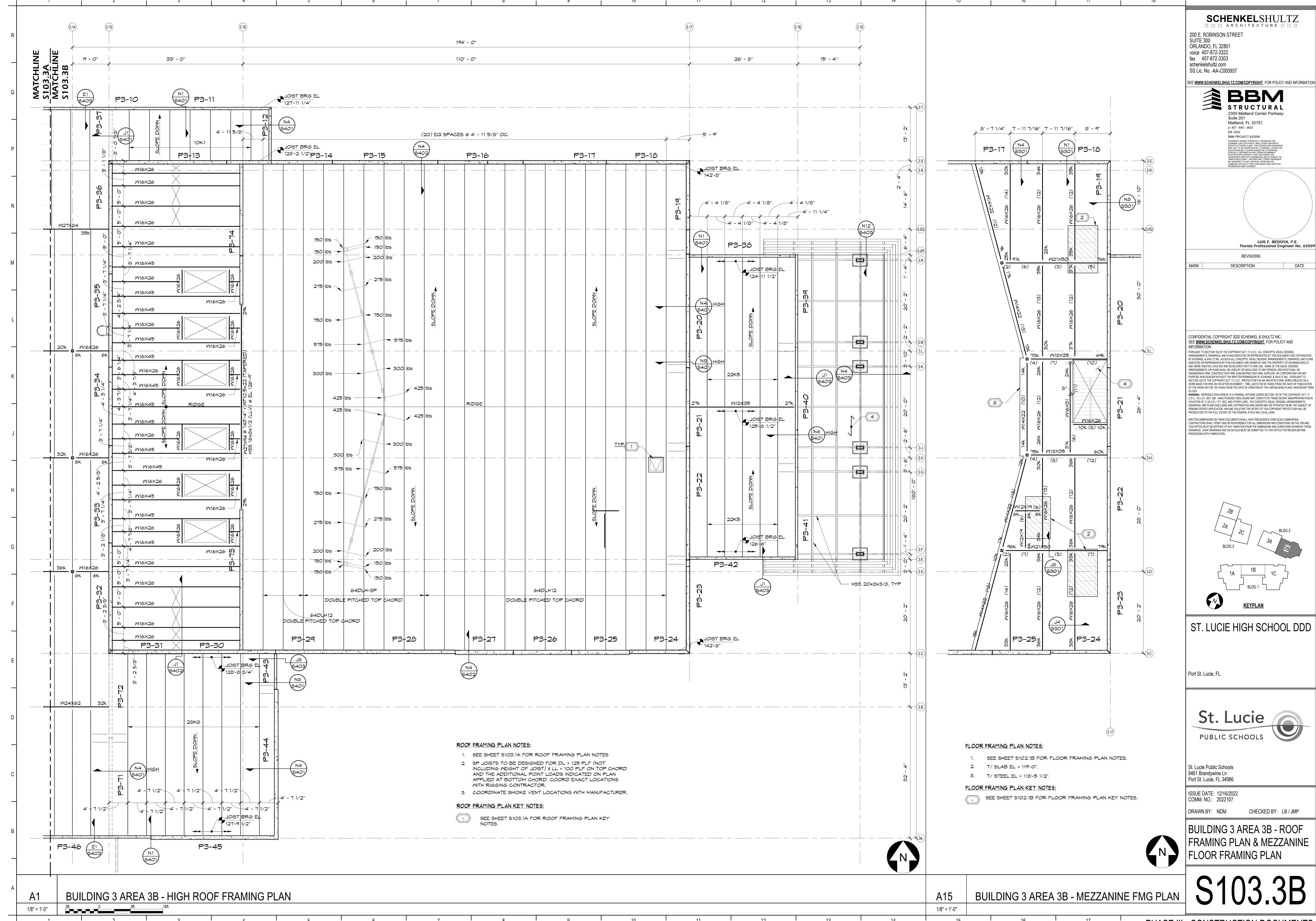
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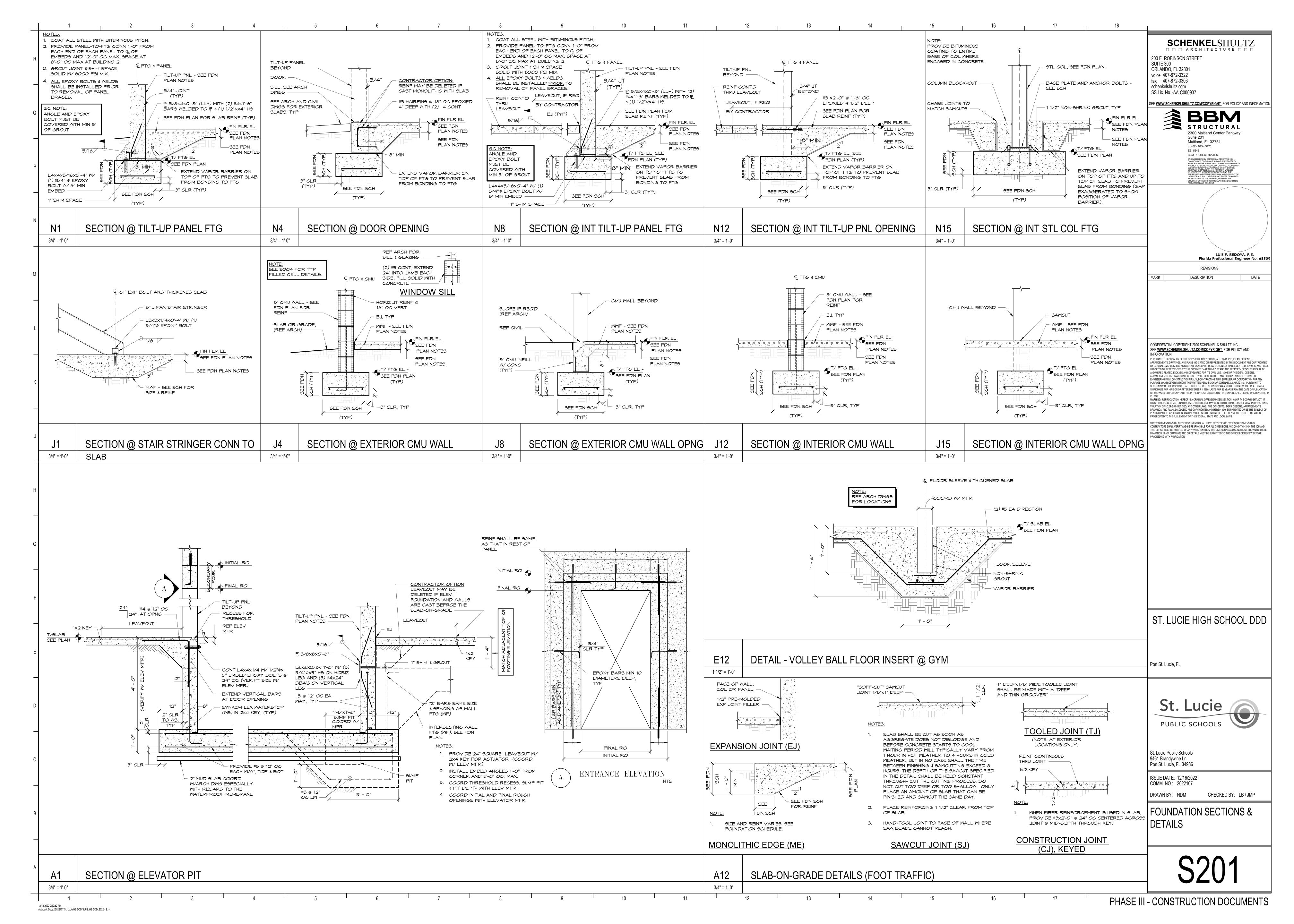
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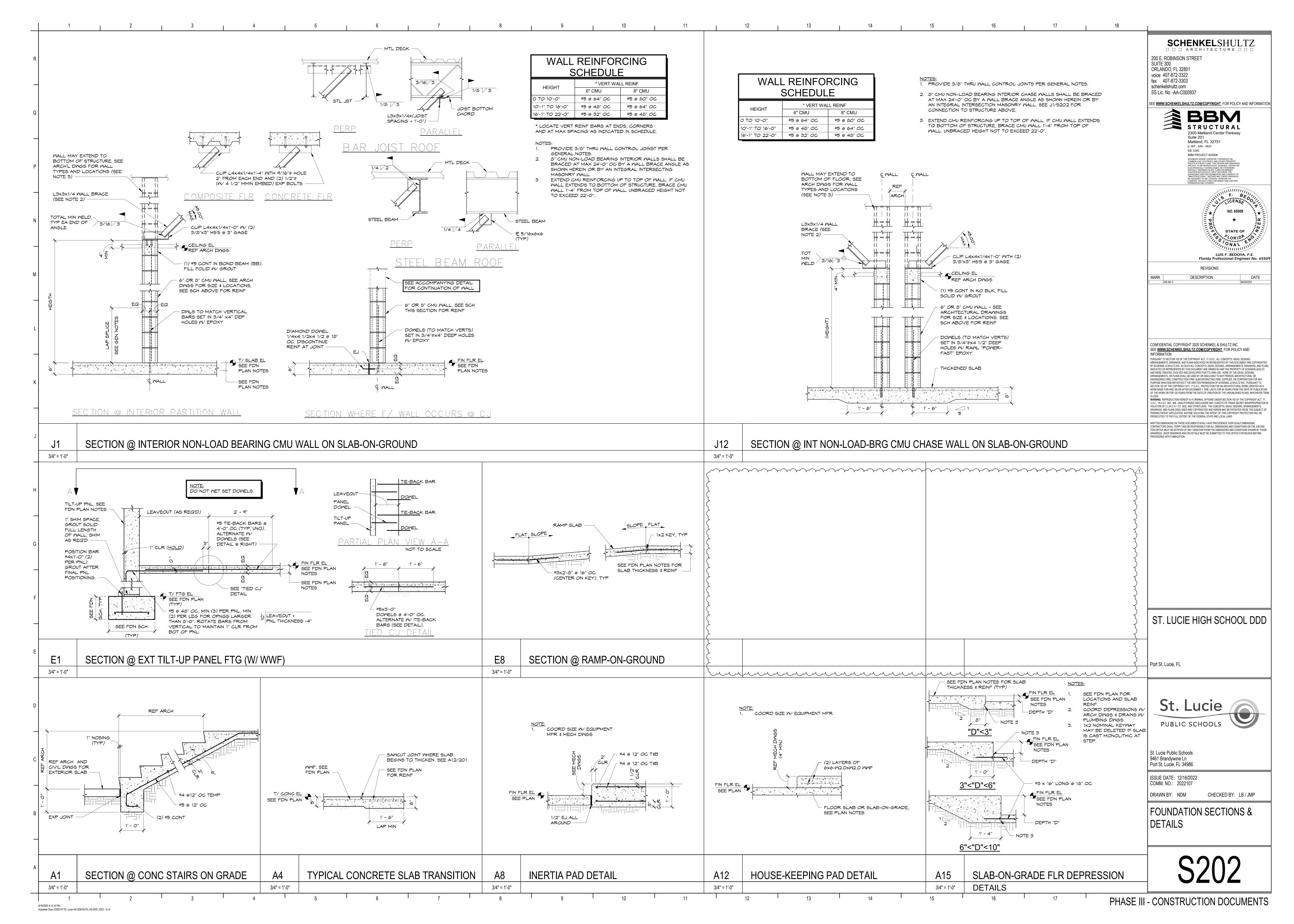


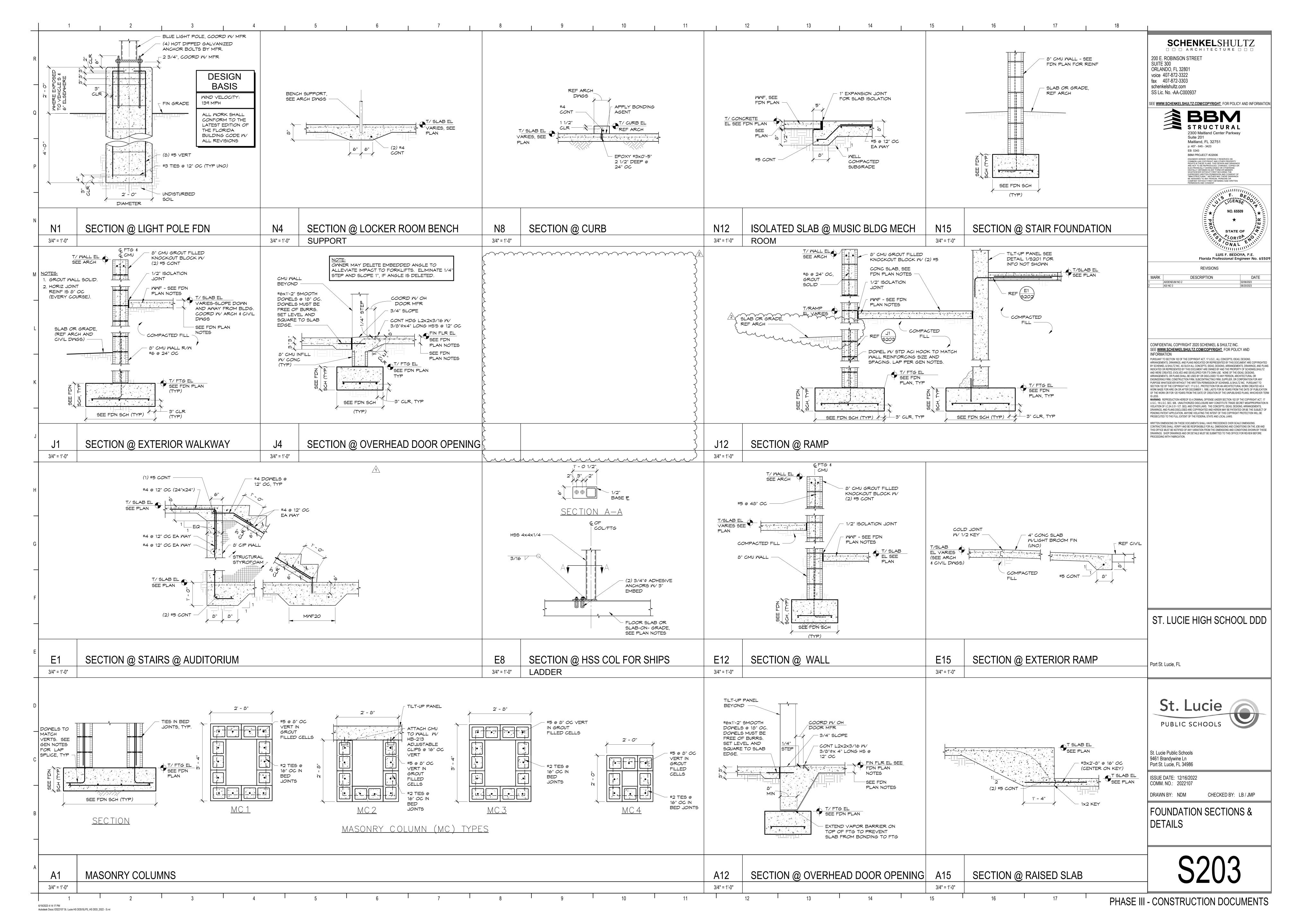
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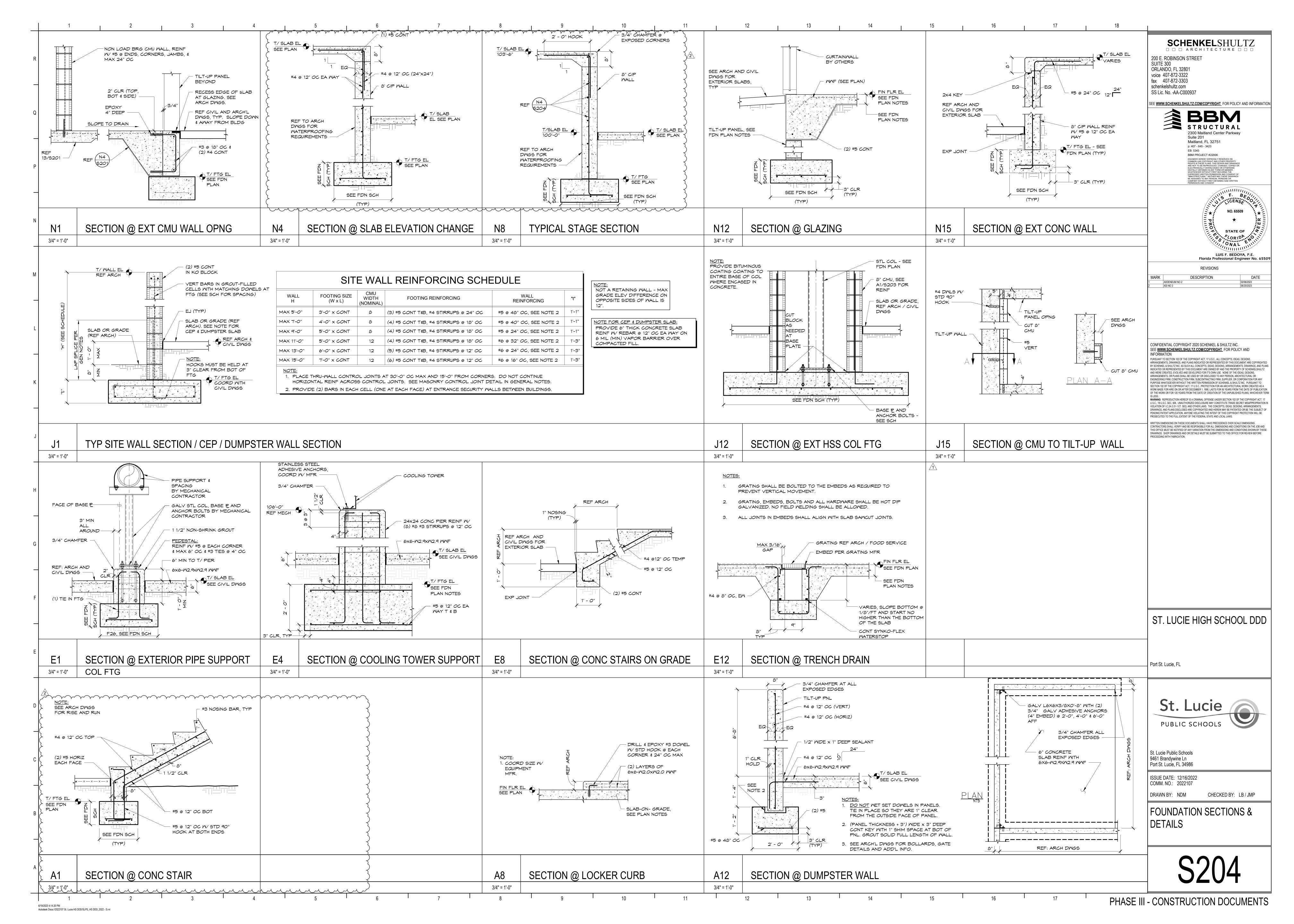
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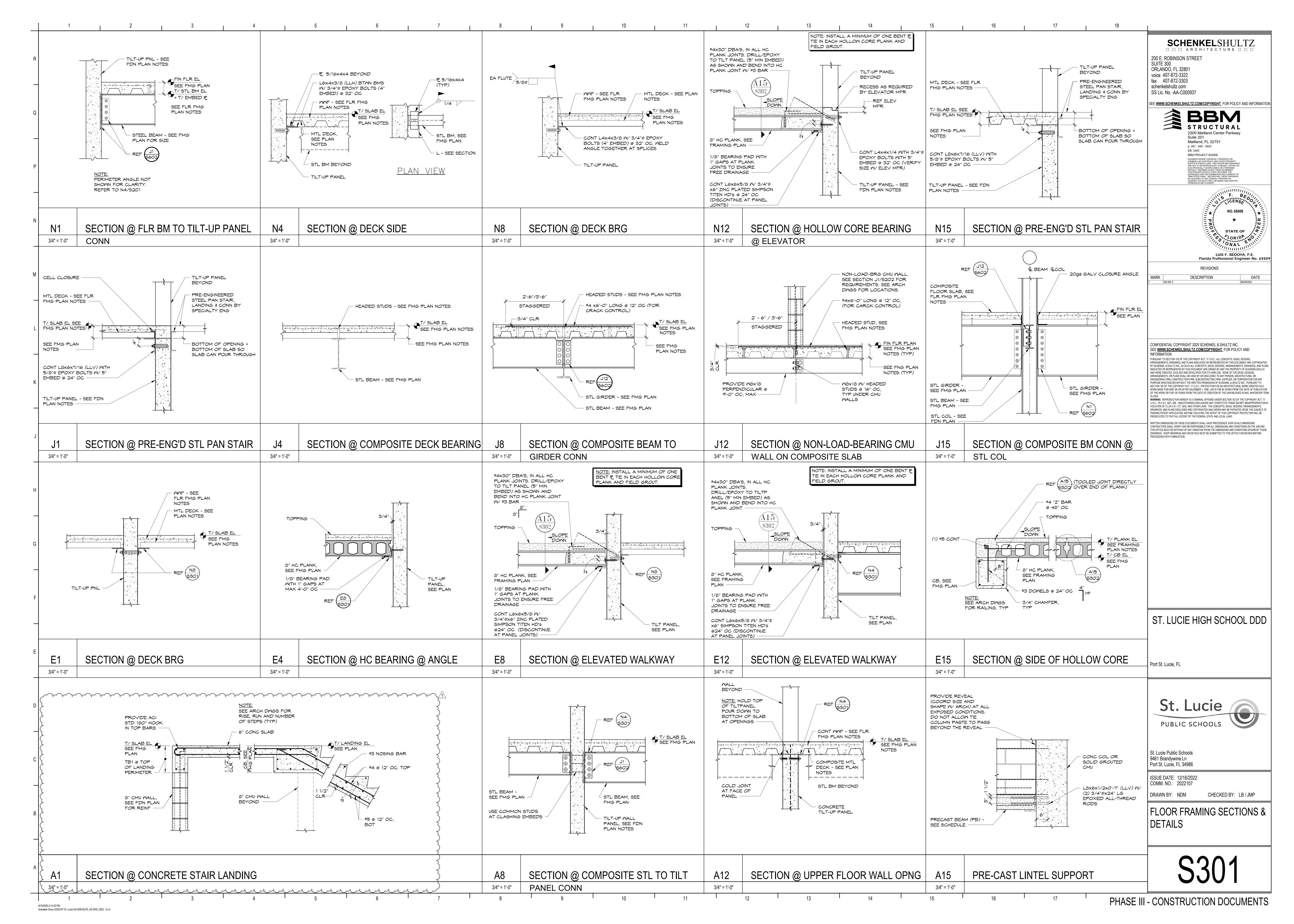


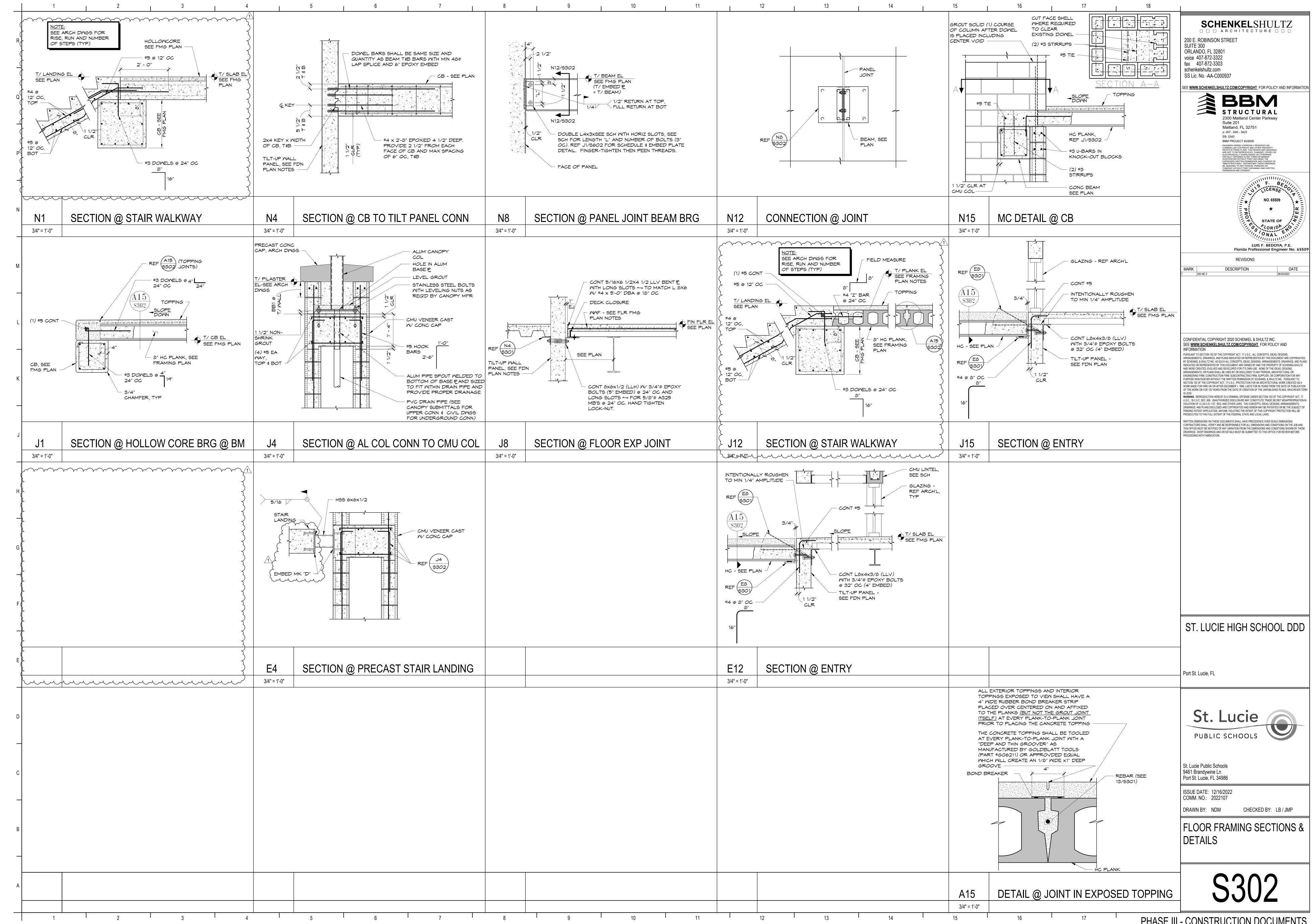


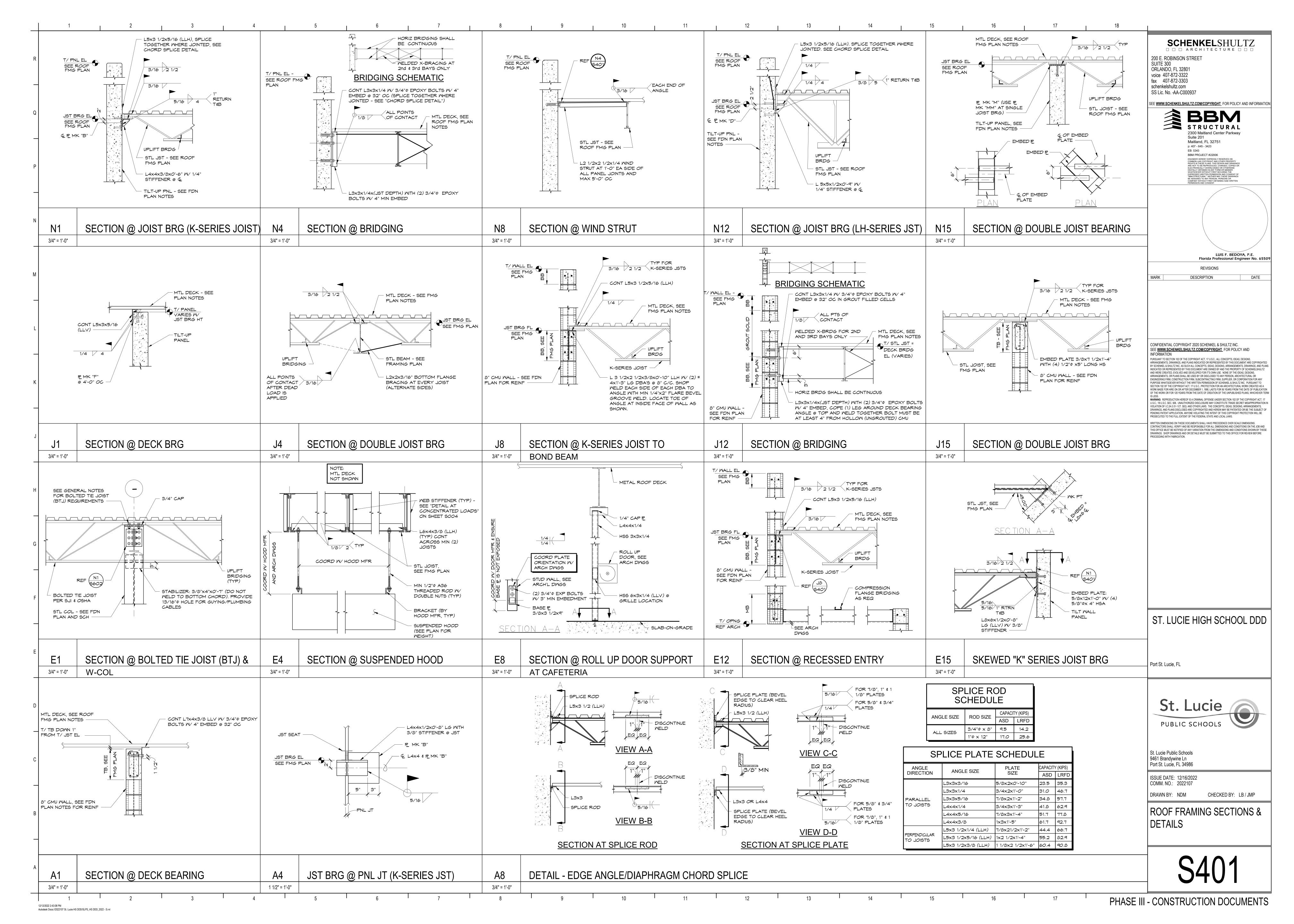


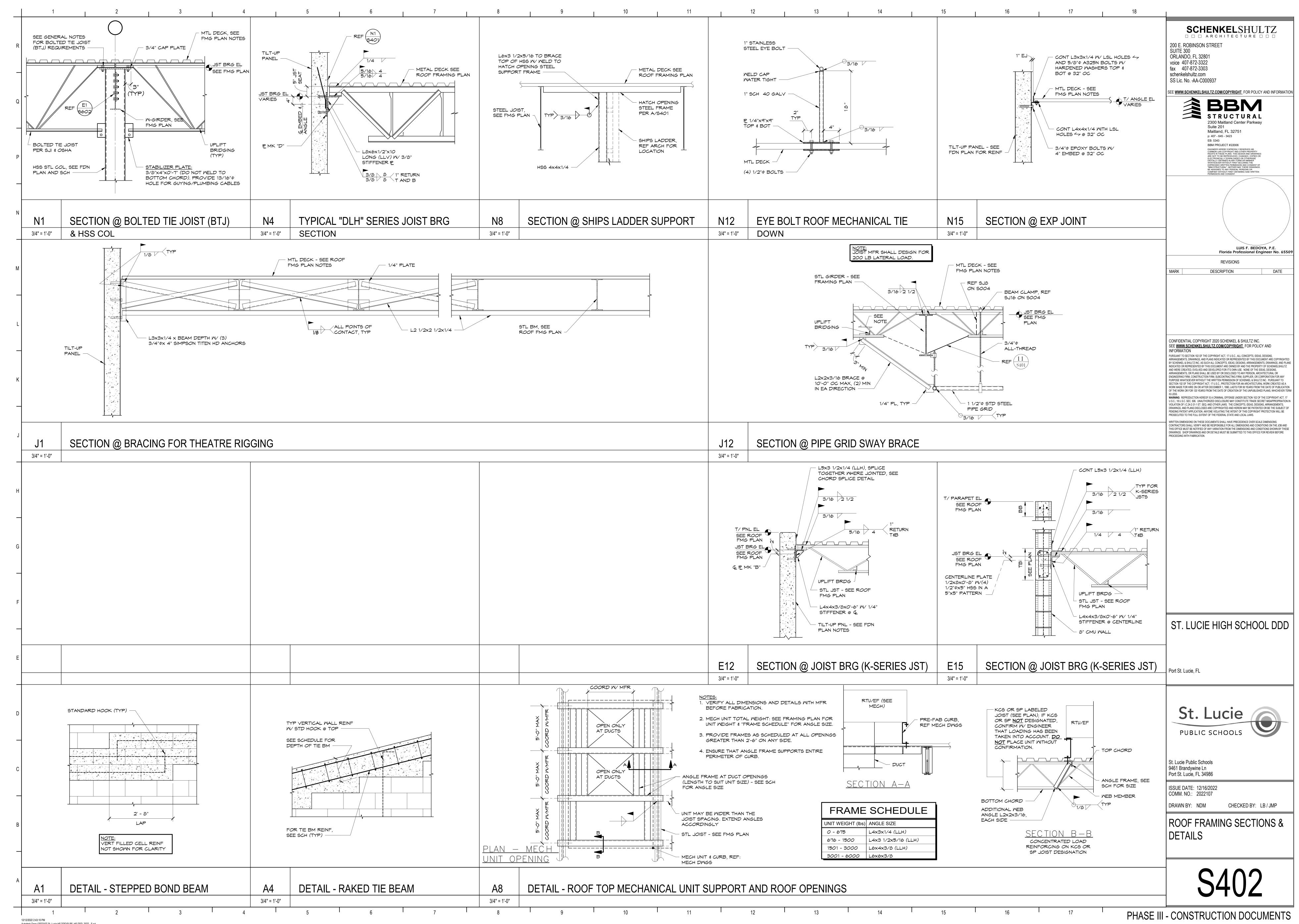


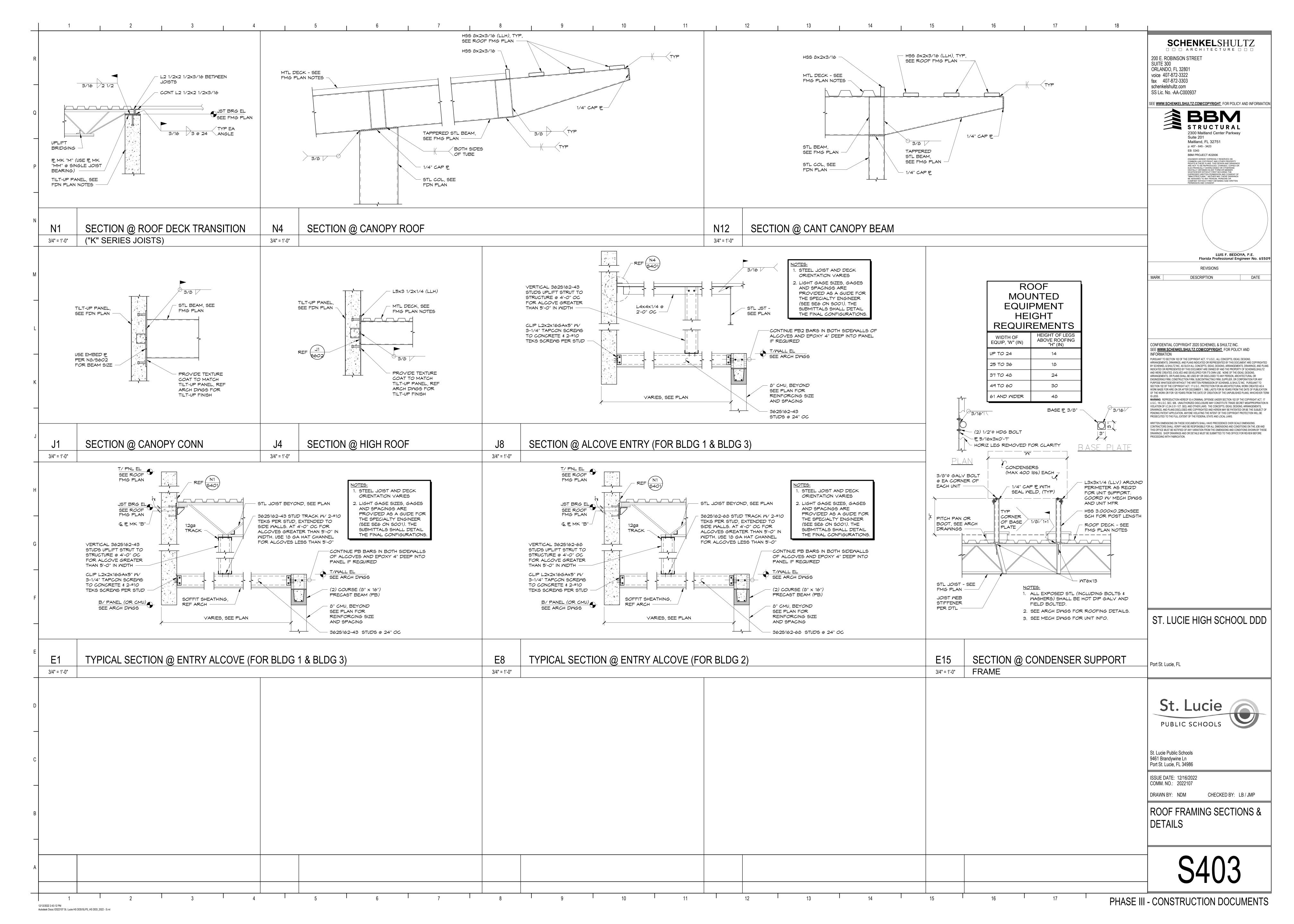




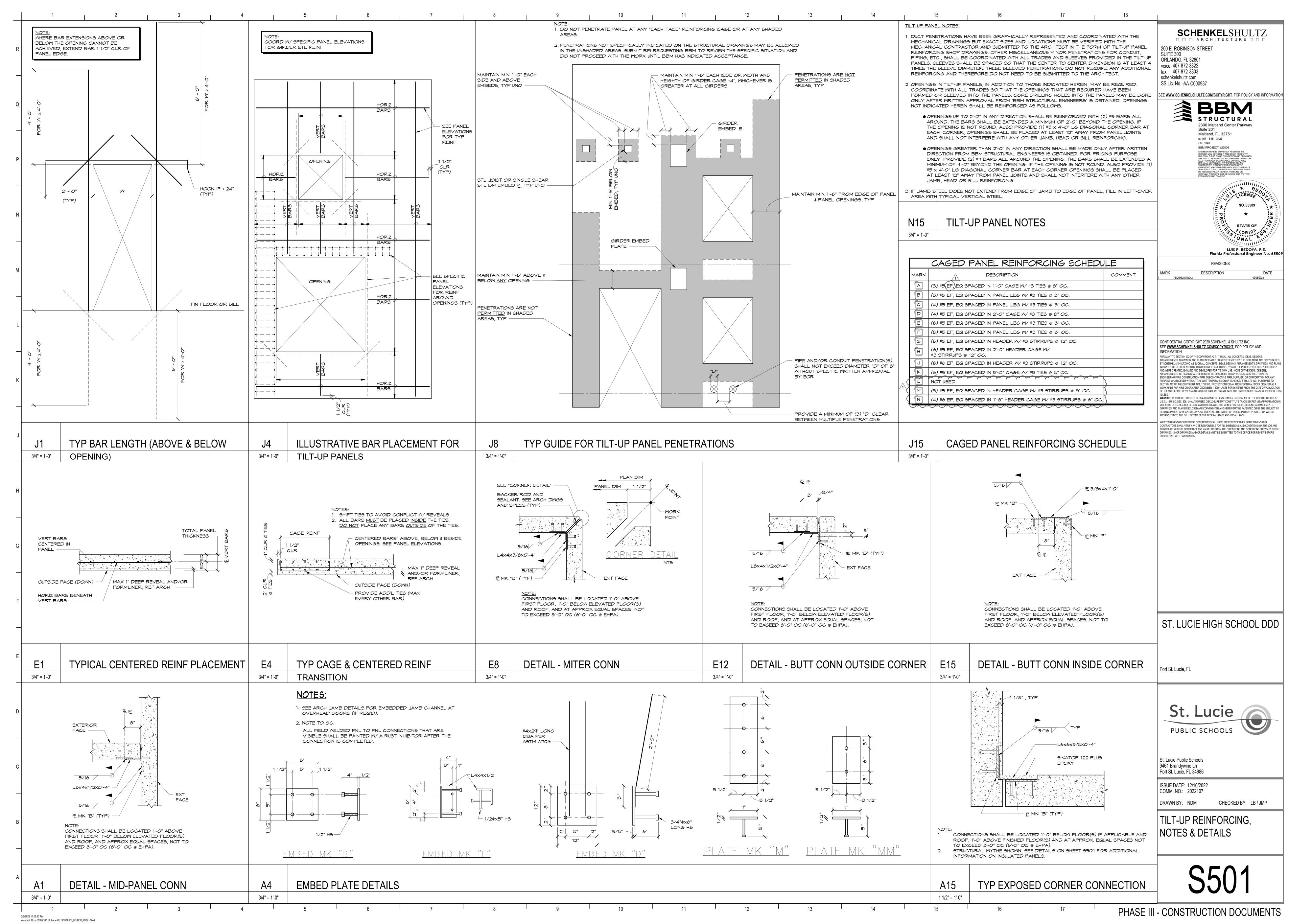


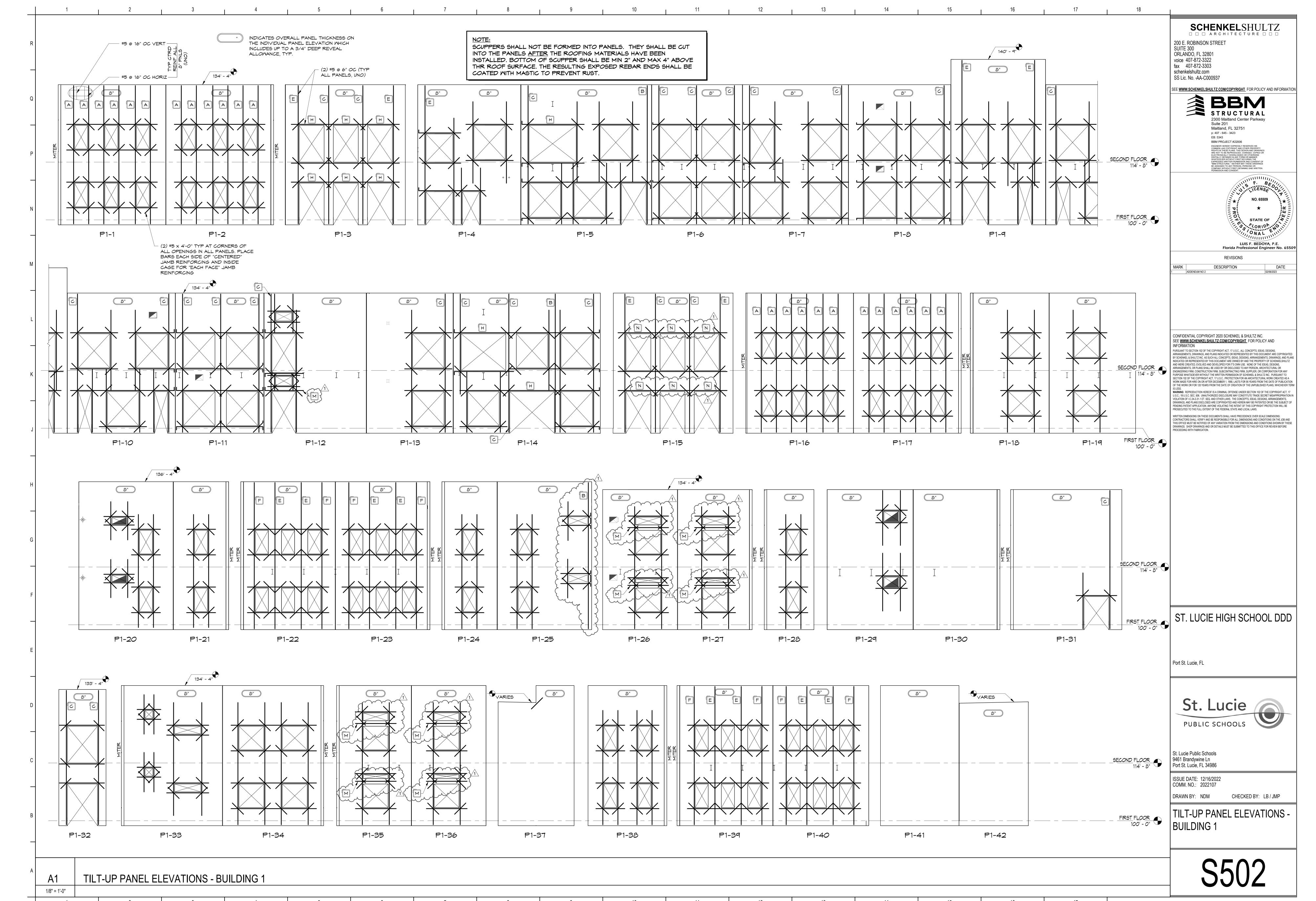




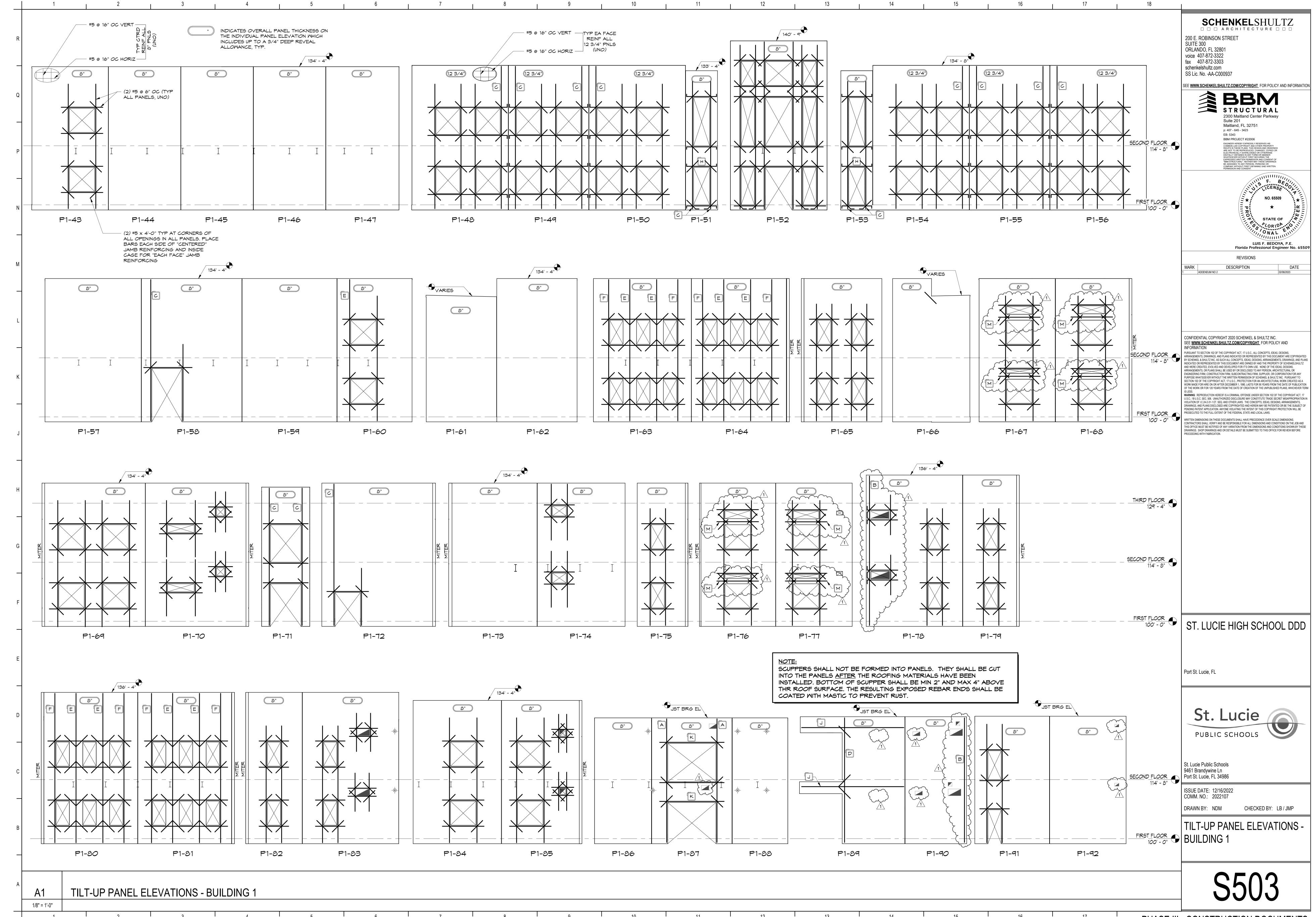


		SCHENKELSHULTZ
LE LEVATIONS CAGE REINF CAGE REINF EA FACE BARS: ABOVE, BELOW & BESIDE OPNGS. SEE PANEL ELEVATIONS (2) LAYERS OF VERT BARS 1" CLR 1" CLR	SEE PLAN 5/16/ E MK "B" (TYP)	200 E. ROBINSON STREET SUITE 300 ORLANDO, FL 32801 voice 407-872-3322 fax 407-872-3303 schenkelshultz.com SS Lic. NoAA-C000937 SEE WWW.SCHENKELSHULTZ.COM/COPYRIGHT FOR POLICY AND INFORMATION AND INFORMA
MAX 3/4" DEEP REVEAL AND/OR FORMLINER OUTSIDE FACE (DOWN) MAX 3/4" DEEP REVEAL AND/OR FORMLINER OUTSIDE FACE (DOWN) MAX 3/4" DEEP REVEAL AND/OR FORMLINER HORIZ BARS MAX 3/4" DEEP REVEAL AND/OR FORMLINER NOTES: 1. SHIFT TIES TO AVOID CONFLICT W/ REVEALS.	BENT P. 6X6X1/2X0'-4" LG BACKER ROD AND SEALANT. SEE ARCH DWGS AND SPECS, (TYP) NOTE:	STRUCTURAL 2300 Maitland Center Parkway Suite 201 Maitland, FL 32751 p: 407 - 645 - 3423 EB: 5343 BBM PROJECT #22006 ENGINEER HEREBY EXPRESSLY RESERVES HIS COMMON LAW COPYRIGHT AND OTHER PROPERTY RIGHTS IN THESE PLANS. THIS DESIGN AND DRAWINGS ARE NOT TO BE REPRODUCED, CHANGED. COPIED OR ELECTRONICALLY DOWNANCE AND THE AWAY OF
2. ALL BARS <u>MUST</u> BE PLACED INSIDE THE TIES. <u>DO</u> <u>NOT</u> PLACE ANY BARS <u>OUTSIDE</u> OF THE TIES.	CONNECTIONS SHALL BE LOCATED 1'-0" ABOVE FIRST FLOOR, 1'-0" BELOM ELEVATED FLOOR(S) AND ROOF, AND AT APPROX. EQUAL SPACES, NOT TO EXCEED 8'-0" OC (6'-0" OC @ EHPA).	PERMISSION AND CONSENT.
N1 TYP CAGE & EA FACE REINF 3/4" = 1'-0" TRANSITION (PANEL THICKNESS > 10") N4 TYP EACH FACE REINF PLACEMENT 3/4" = 1'-0" (PANEL THICKNESS > 10")	N8 DETAIL - PANEL JOINT 3/4" = 1'-0"	
		REVISIONS MARK DESCRIPTION DATE
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		St. Lucie
		St. Lucie PUBLIC SCHOOLS
		St. Lucie Public Schools 9461 Brandywine Ln Port St. Lucie, FL 34986 ISSUE DATE: 12/16/2022 COMM. NO.: 2022107
		TILT-UP PANEL CONNECTION
		SEN11

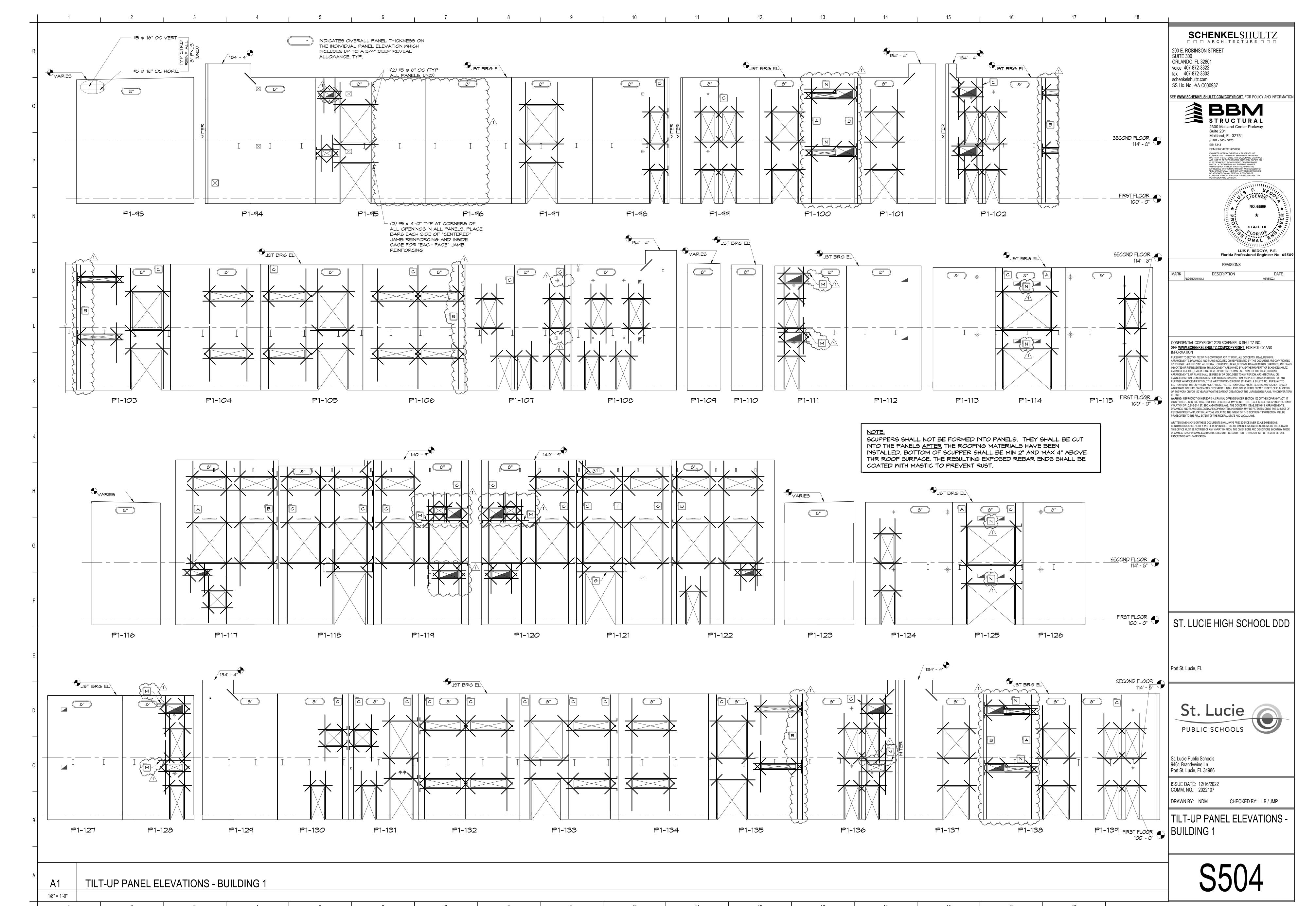




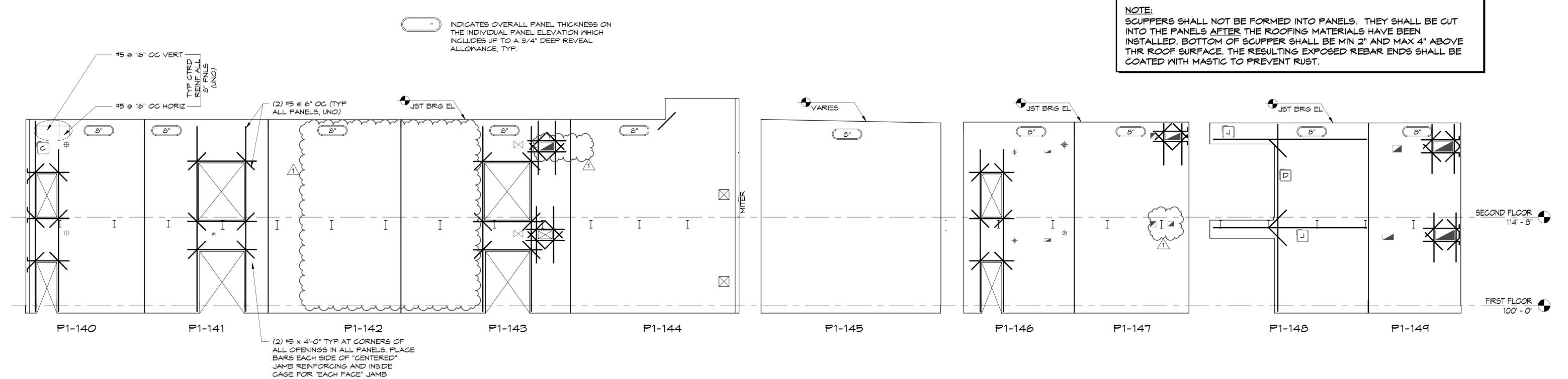
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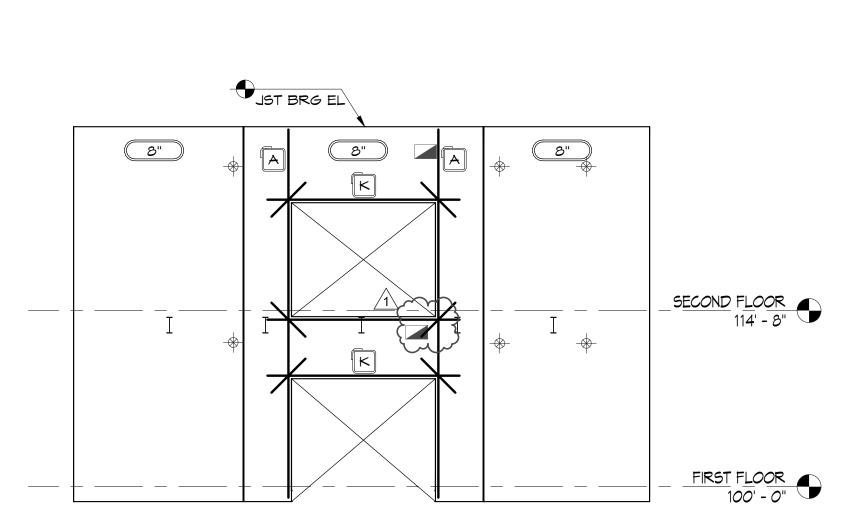


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P1-152

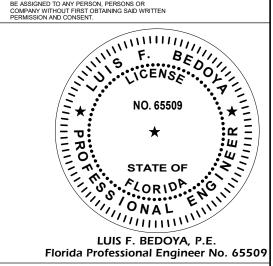
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114' - 8"

SECOND FLOOR
114' - 8"

SECOND FLOOR
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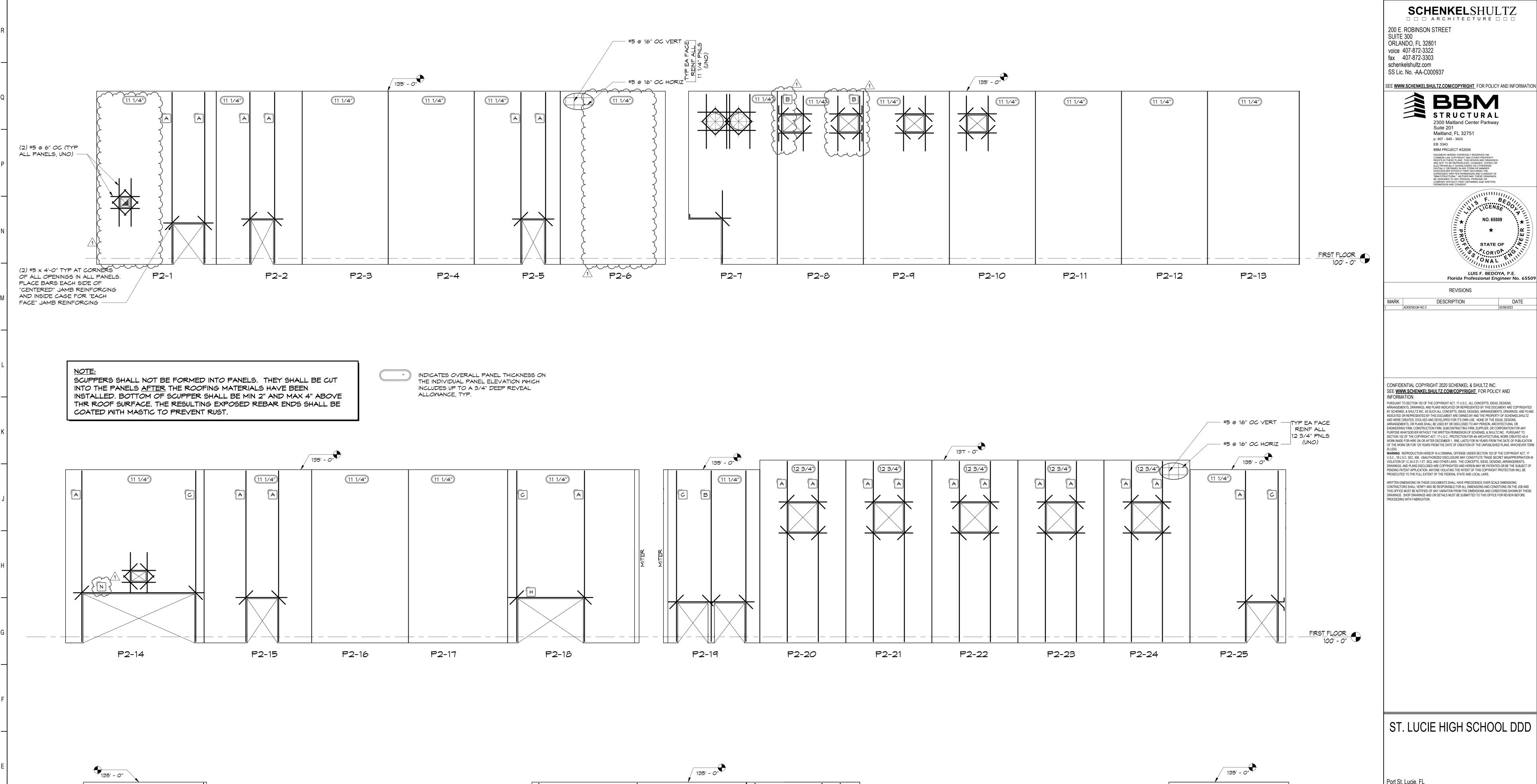
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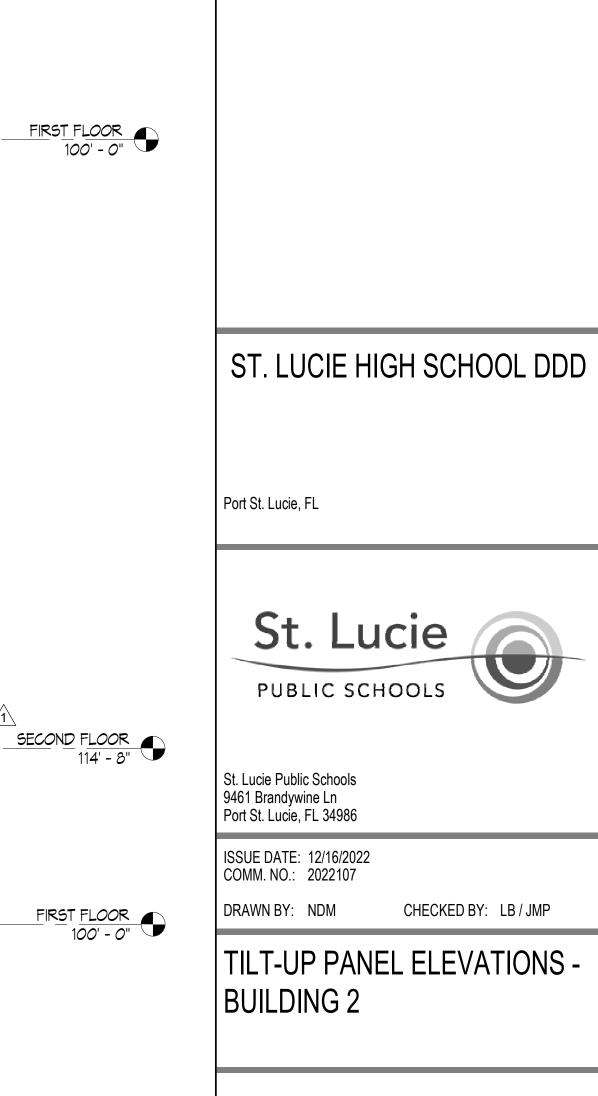
TILT-UP PANEL ELEVATIONS -

BUILDING 1

REINFORCING

1/8" = 1'-0"





7 1/4"

P2-36

124' - 0"

P2-34

7 1/4"

P2-33

7 1/4"

7 1/4"

P2-35

DATE

TILT-UP PANEL ELEVATIONS - BUILDING 2

P2-26

(11 1/4")

SECOND FLOOR 114' - 8"

1/8" = 1'-0"

_ P2-30

7 1/4"

P2-27

- #5 @ 16" OC HORIZ -

P2-28

PHASE III - CONSTRUCTION DOCUMENTS

P2-31

7 1/4"

7 1/4"

P2-32

7 1/4"

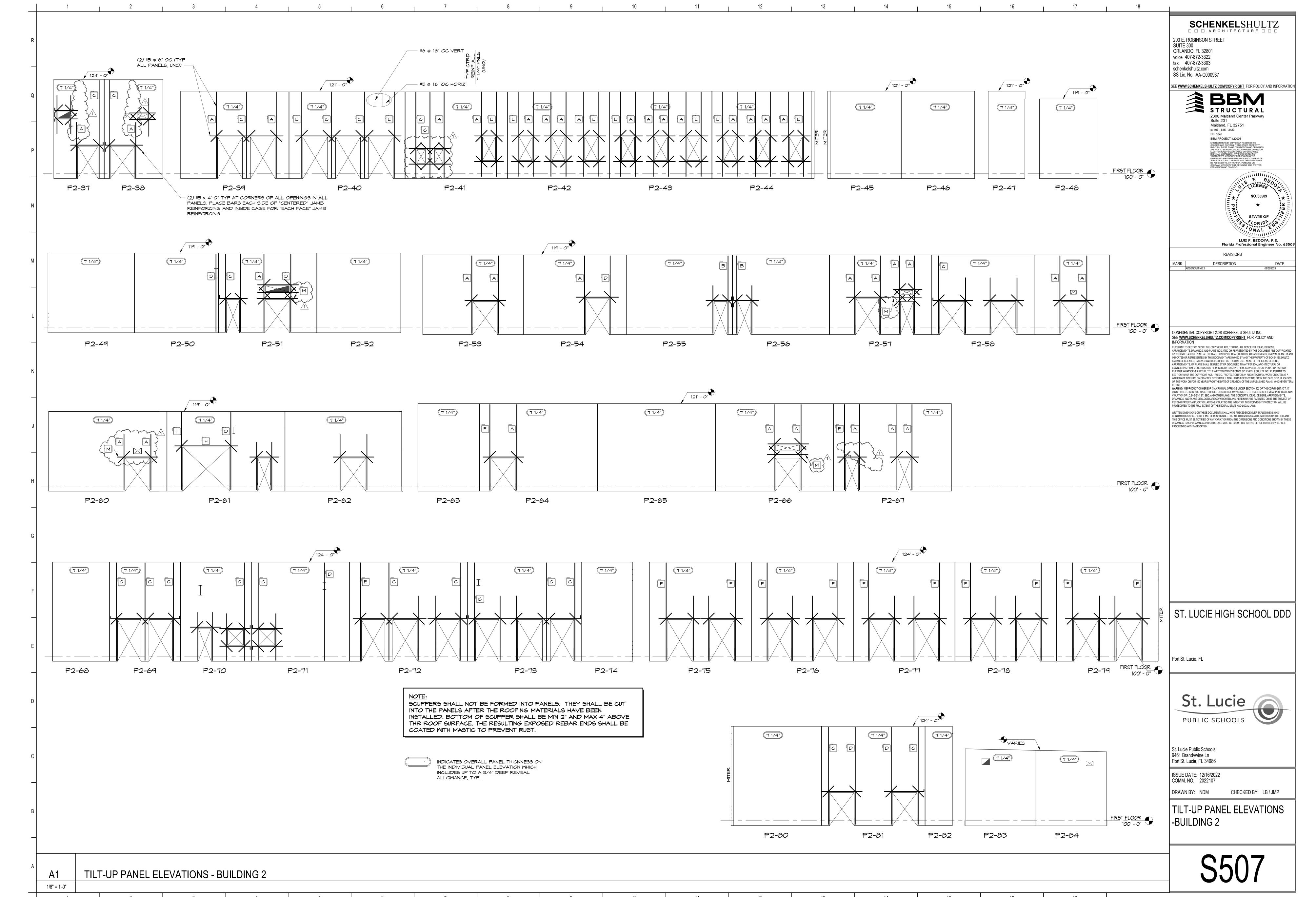
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SECOND FLOOR 114' - 8"

VARIES

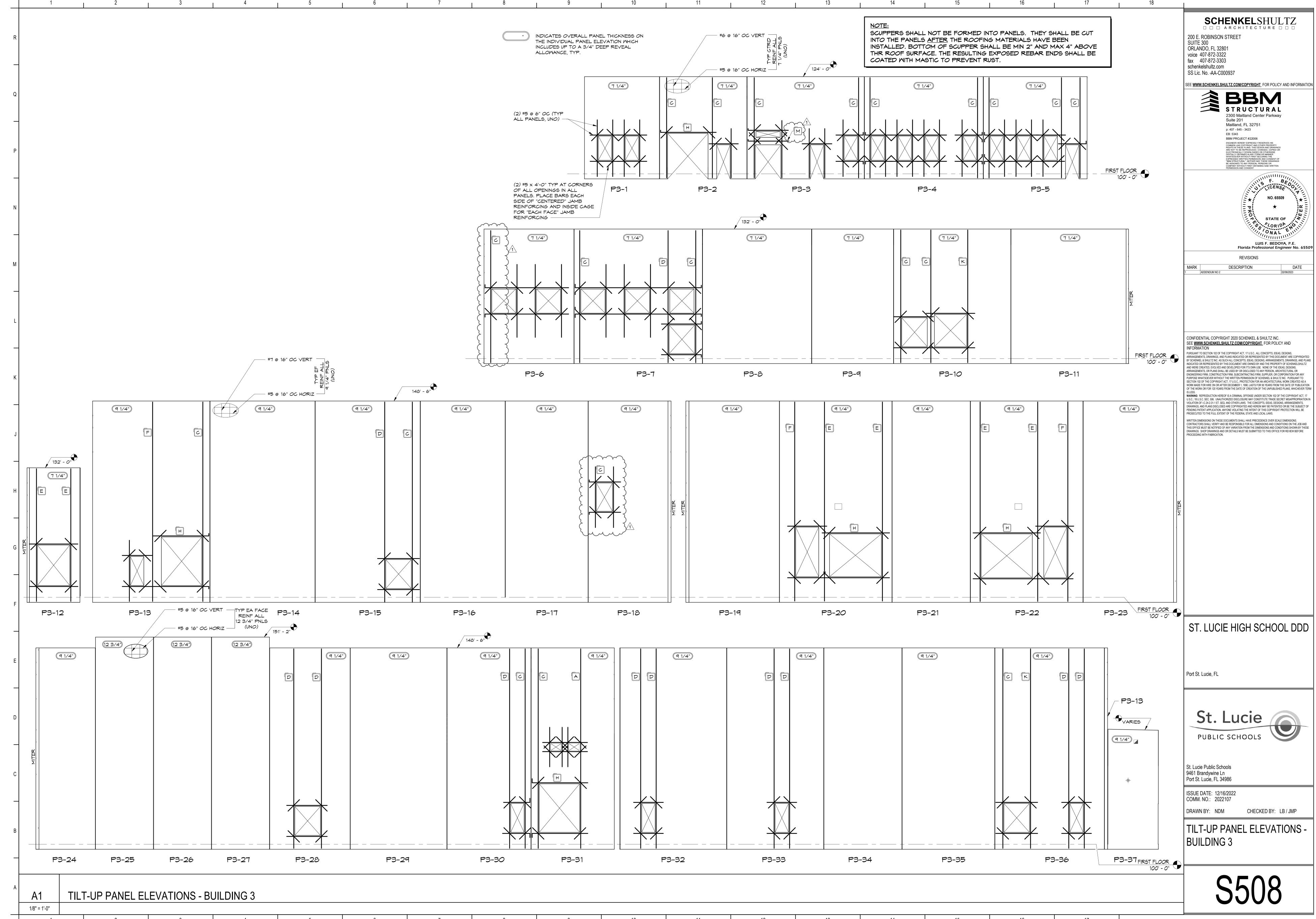
7 1/4"

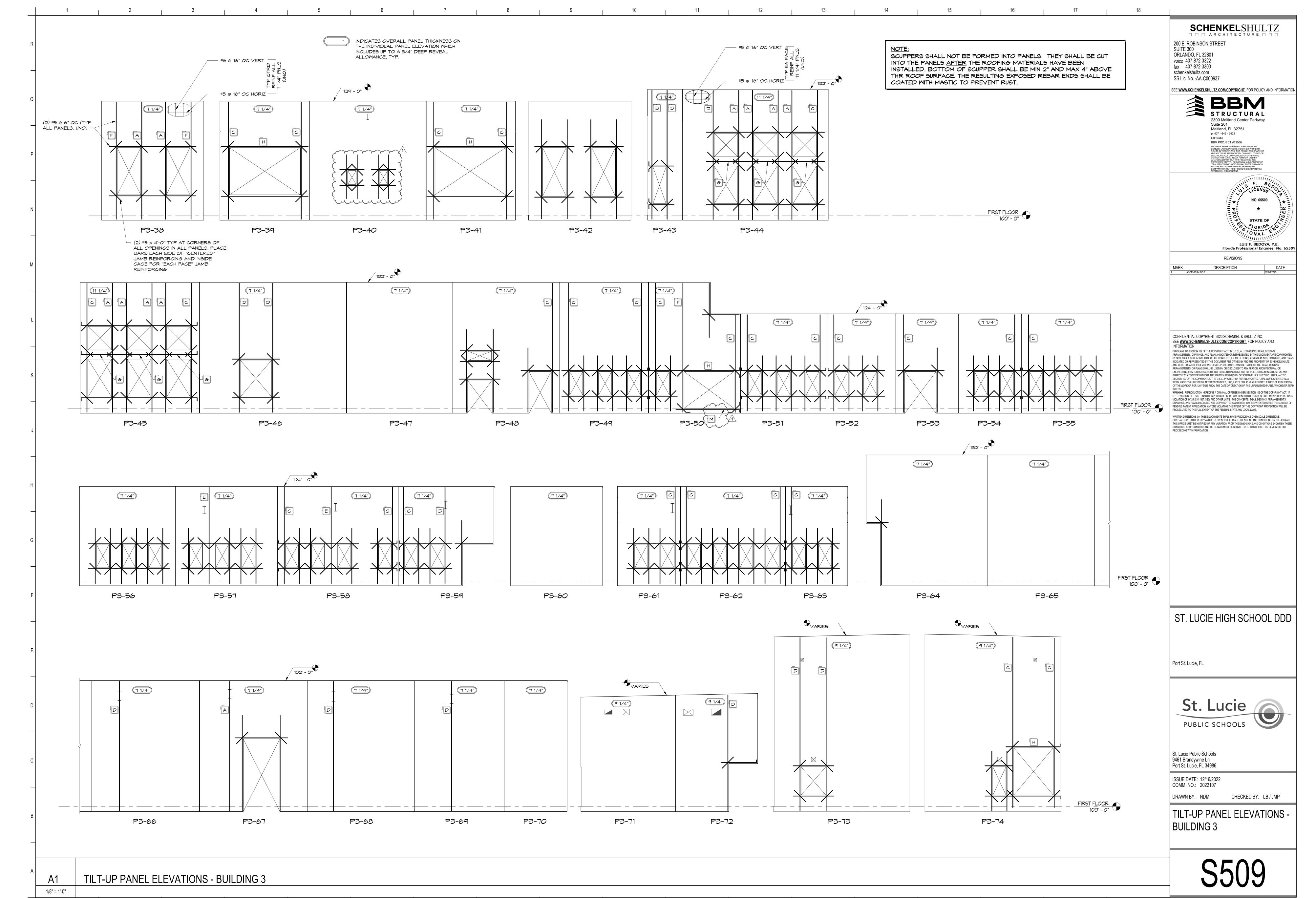
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FOUNDATION SCHEDULE

12" = 1'-0"

		COLUMN & BASE PLATE SCHEDULE							
	MARK	SIZE	BASE PLATE SIZE (INCHES)		ANCHOR ROD (INCHES)		COMMENTS		
			Α	В	Т	E	d	COMMINICIATS	
	C 1	W10x39	16	16	1 1/4	9	3/4		
<u>^1</u> -{	C2	HSS 6x6x5/16	12	12	1	11	7/8		
	<i>C</i> 3	HSS 6x6x5/8	12	12	1	11	7/8		
	C4	HSS 20x8x1/2	20	26	1	12	1	SEE DETAIL "AA" BELOW	
	65	HSS 12x8x1/2	20	20	1	12	1	SEE DETAIL "AA" BELOW	
\				3	\ \	~	\		
	NOTES: 1 ANCHOR RODS SHALL BE ASTM F1554 (GRADE 36) UNO THREADED EACH END WITH NUT AT BOTTOM								

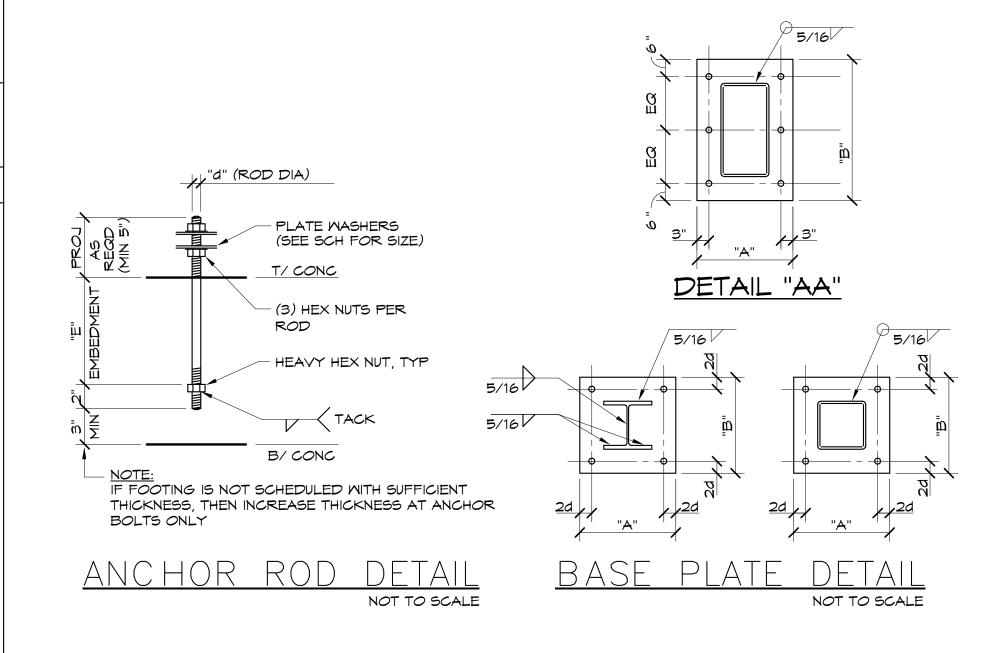
ANCHOR RODS SHALL BE ASTM F1554 (GRADE 36) UNO, THREADED EACH END WITH NUT AT BOTTOM. PLATE WASHER AT BOTTOM NUT IS NOT REQ'D. INCREASE FOOTING THICKNESS AT ANCHOR RODS TO PROVIDE MIN 3" COVER. SEE NOTE BELOW IN ANCHOR ROD DETAIL.

2. PLATE WASHERS SHALL HAVE HOLE WITH DIAMETER EQUAL TO ROD DIAMETER + 1/16". CIRCULAR OR SQUARE WASHERS ARE ACCEPTABLE.

3. IT SHALL BE ACCEPTABLE TO PROVIDE A HEAVY HEX HEAD BOLT IN LIEU OF THE THREADED ROD WITH HEAVY HEX NUT AT BOTTOM.

OVERSIZE HOLES AND PLATE WASHERS FOR BASE PLATES

ROD DIAMETER	HOLE DIAMETER	PLATE WASHER	ROD DIAMETER	HOLE DIAMETER	PLATE WASHER
3/4"	1 5/16"	1/4"x2"x2"	1 1/2"	2 5/16"	1/2"x3 1/2"
7/8"	1 9/16"	5/16"x2 1/2"x2 1/2"	1 3/4"	2 3/4"	5/8"x4"
1"	1 13/16"	3/8"x3"x3"	2"	3 1/4"	3/4"x5"
1 1/4"	2 1/16"	1/2"x3"x3"	2 1/2"	3 3/4"	7/8"x5 1/2"



COLUMN & BASE PLATE SCHEDULE

12" = 1'-0"

(W"xD") | BOT | MID | TOP | SIZE | SPACING BB1A 12x8 BB2 8x16 (2) #5 --BB2A 12×16 (2) #5 BB3 8x24 (2) #5 (2) #5 (2) #5 --CB1 16×16 (3) #6 -- |(3)#6|#3| 24x24 (4) #9 (2) #6 (4) #9 #3 24×24 (8) #9 (2) #6 (4) #9 #3 6" 24×16 (4) #9 (2) #6 (4) #9 #3 6" TWO LAYERS OF BOTTOM REINFORCING TB1 8×16 (MIN) (2) #5 |(2) #5 | #3 | 24" TB1A 12×16 (2) #5 (2) #5 | #3 TB2 8x12 (MIN) (2) #5 (2) #5 | #3 SLOPED TOP & STEPPED BOTTOM 3A |12x12 (MIN)| (2) #5 | TB2A SLOPED TOP & STEPPED BOTTOM PB1 MIN 1200 PLF CAP FOR CLEAR SPAN UP TO 6'-0" (NOTE 1) 8x8 (2) #5 PB1A 12x8 (2) #5 MIN 1900 PLF CAP FOR CLEAR SPAN UP TO 6'-0" (NOTE 1) PB2 8×16 (2) #5 MIN 1150 PLF CAP FOR CLEAR SPAN UP TO 12'-0" (NOTE 1) PB2A 12×16 (2) #5 MIN 1350 PLF CAP FOR CLEAR SPAN UP TO 12'-0" (NOTE 1) 8×24 | (2) #5 | (2) #5 | (2) #5 | PB3 MIN 1700 PLF CAP FOR CLEAR SPAN UP TO 16'-0" (NOTE 1) PB2A 12×24 | (2) #5 | (2) #5 | (2) #5 | MIN 1500 PLF CAP FOR CLEAR SPAN UP TO 16'-0" (NOTE 1 8x16 (2) #5 -- (2) #5 #3 8" 2B 74 8A NOTES: MIN PB CAPACITY FOR THE INDICATED SPAN MUST BE VERIFIED BY THE PRECAST MFR AND DOCUMENTATION PROVIDED IN THE SUBMITTALS. "BB" = BOND BEAM (COMPOSED OF KNOCK-OUT BLOCK). "CB" = CONCRETE BEAM (FORMED, SHORED AND POURED). "TB" = TIE BEAM (FORMED AND POURED). "PB" = PRE-CAST BEAM (COMPOSED OF PRE-CAST U-LINTEL ATT BOTTOM AND KNOCK-OUT BLOCK AT TOP, ALL GROUTED SOLID TO FORM INTEGRAL BEAM). "MB" = MASONRY BEAM (COMPOSED OF MASONRY LINTEL BLOCK AT BOTTOM AND KNOCK-OUT BLOCK AT ALL REINFORCED COURSES). TYP FILLED CELL REINF - FILL CELL SOLID SHALL APPLY TO THE MITH GROUT AREA ABOVE AND BELOW THE OPENING ALSO SEE BEAM SCHEDULE FOR SIZE & REINF 2' - 0" NEATLY SAMOUT COORD SILL SLOT TO ALLOW DETAILS W/ YERTICAL JAMB ARCH DWGS REINF TO PASS UNINTERRUPTED (IF MASONRY OPENING SLOT IS NOT REF ARCH DWGS FOR ALREADY PROVIDED SIZE AND LOCATION Α BY MFR) VERT WALL REINF (CENTER BARS IN CELLS) - SEE FDN (IF REQ'D) PLAN NOTES 2' - 0" SLAB, CONCRETE BEAM, FOUNDATION, OR WALL The state of the s PRECAST BEAM FLEVATION 90° STD ACI HOOK LAP TOP BARS AT MID-SPAN (IF NECESSARY) (TYP) BOTTOM BARS - BEAR CB FULLY ON TC CONCRETE COLUMN TIE COLUMN PLACEMENT OF REINF PER ACI 318. FOR BOTTOM BARS PROVIDE A CLASS "A" TENSION LAP CENTERED OVER SUPPORT OR TERMINATE BARS AS CONDITION WARRANTS. SEE COLUMN SCHEDULES AND DETAILS FOR SIZE AND REINFORCING. CONCRETE BEAM FIEVATION

MASONRY WALL BEAM SCHEDULE

REINFORCING STIRRUPS

MARK

SIZE

MASONRY WALL BEAM SCHEDULE

SCHENKELSHULTZ

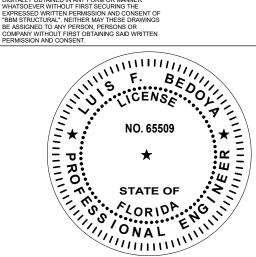
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LUIS F. BEDOYA, P.E.
Florida Professional Engineer No. 65509

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