

CONCRETE AND REINFORCING

- C1 A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD TESTING INCLUDING SLUMP TESTS AND CYLINDER BREAKS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
- C2 CONCRETE WORK SHALL CONFORM TO ACI 318-14 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE.
- C3 ALL CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES:

LOCATION	28 DAY STRENGTH	SLUMP	COARSE AGGREGATE(S)	COMMENTS	EXPOSURE CATEGORIES & CLASSES (PER ACI 318-14 SECTION 19.3.1)			
					F	S	A	C
FOUNDATIONS	3000 PSI	4" +/- 1"	1"		FO	SO	NO	CO
SLAB-ON-GRADE	3000 PSI	4" +/- 1"	1"		FO	SO	NO	CO
TIE BEAMS AND TIE COLUMNS (MAX 50' BETWEEN COLD JOINTS)	3000 PSI	6" +/- 1"	3/8"		FO	SO	NO	CO
BEAMS, COLUMNS & ELEVATED STAIRS	4000 PSI	4" +/- 1"	3/8" & 1"		FO	SO	NO	C1
TILT-UP PANELS	4000 PSI	4" +/- 1"	3/8" & 1"		FO	SO	NO	C1
GROUT UNDER TILT-UP PANELS	6000 PSI	8" TO 11"	NONE		FO	SO	NO	CO
ELEVATED SLABS (FORMED)	4000 PSI	4" +/- 1"	3/8" & 1"		FO	SO	NO	CO
ELEVATED SLABS (MTL DECK)	3000 PSI	4" +/- 1"	3/8" & 1"		FO	SO	NO	CO
TOPPING	4000 PSI	7" +/- 1"	3/8"		FO	SO	NO	C1
FILLED CELL, PRECAST LITELS & BOND BEAM GROUT (ASTM C416) - SEE NOTE 7	2500 PSI	8" TO 11"	COARSE GROUT: 3/8" FINE GROUT: NONE		FO	SO	NO	CO

NOTES:

- SLUMP FOR RAMPS AND SLOPING SURFACES SHALL NOT EXCEED 4".
- ALL CONC MIXES SHALL HAVE A MAXIMUM SAND TO TOTAL AGGREGATE RATIO OF 0.50.
- A 2" OR 3" PUMP SHALL BE ACCEPTABLE FOR COLUMNS, CELL FILL AND TIE BEAMS BUT WILL NOT BE ALLOWED FOR FOUNDATIONS, SLABS, TILT-UP PANELS AND CONCRETE BEAMS.
- READY MIX SUPPLIER SHALL DESIGN THE MIXES THAT CONTAIN MULTIPLE AGGREGATES TO BE WELL GRADATED.
- SLABS SHALL NOT BE AIR ENTRAINED.
- FOR SLABS THAT SHALL RECEIVE MOISTURE SENSITIVE FLOORING:
 - CONTRACTOR SHALL WORK WITH THE READY MIX SUPPLIER TO PROVIDE A MIX DESIGN THAT WILL BE AT OR BELOW 75% RELATIVE HUMIDITY AT THE TIME THE FLOORING IS SCHEDULED TO BE INSTALLED.
 - DO NOT USE LIGHTWEIGHT AGGREGATES.
 - PROVIDE A MIX WITH GOOD SELF-DESICCATING PROPERTIES. CONSIDER ADDING 2%-4% SILICA FUME.
 - DO NOT HARD TROWEL THE SURFACE BUT INSTEAD PROVIDE A LIGHTLY TROWELLED SURFACE.
- SEE MASONRY NOTE M20 FOR TESTING REQUIREMENTS OF GROUT TO BE USED TO FILL CORES OF CMU.

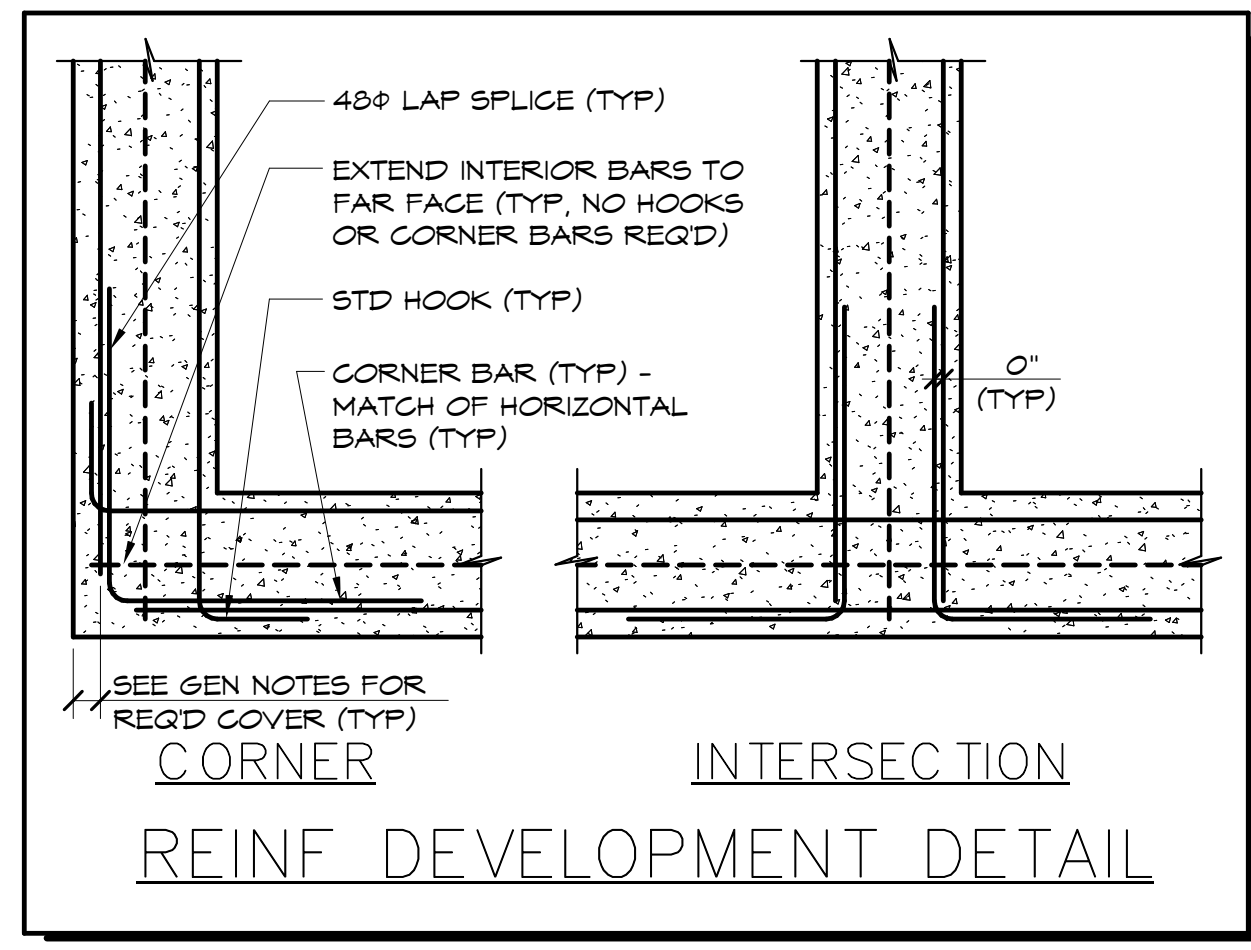
CONCRETE MIX DESIGN SUBMITTALS:

- EACH MIX DESIGN SHALL BE LABELED TO INDICATE THE AREA IN WHICH THE CONCRETE IS TO BE PLACED (I.E. FOUNDATIONS, SLAB-ON-GRADE, COLUMNS, ETC.). FAILURE TO DO SO WILL CAUSE DELAY AND/OR REJECTION OF SUBMITTALS.
- PROPOSED MIX DESIGN SHALL BE IN ACCORDANCE WITH METHOD 1 OR METHOD 2 OF ACI 301. PROVIDE SUPPORTING DATA IN TABULAR FORM FOR EACH SEPARATE PROPOSED MIX.
- SUBMIT CONCRETE MIX DESIGN FOR EACH PROPOSED CLASS OF CONCRETE.

REBAR SHALL CONFORM TO ASTM A615 GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064 AND SHALL BE LAPPED MINIMUM ONE MESH + 2" WHERE SPLICED. ALL REINFORCING SHALL BE DOMESTICALLY PRODUCED. ALL REBAR THAT IS TO BE WELDED SHALL BE LOW ALLOY ASTM A106 GRADE 60.

SPLICES AND ANCHORAGE OF REINFORCING SHALL BE AS FOLLOWS (UNLESS OTHERWISE NOTED):
 WELDED WIRE FABRIC: 8"
 ALL OTHER: #6 BAR & SMALLER: 48 DIA (12" MIN)
 #7 BAR & LARGER: 60 DIA

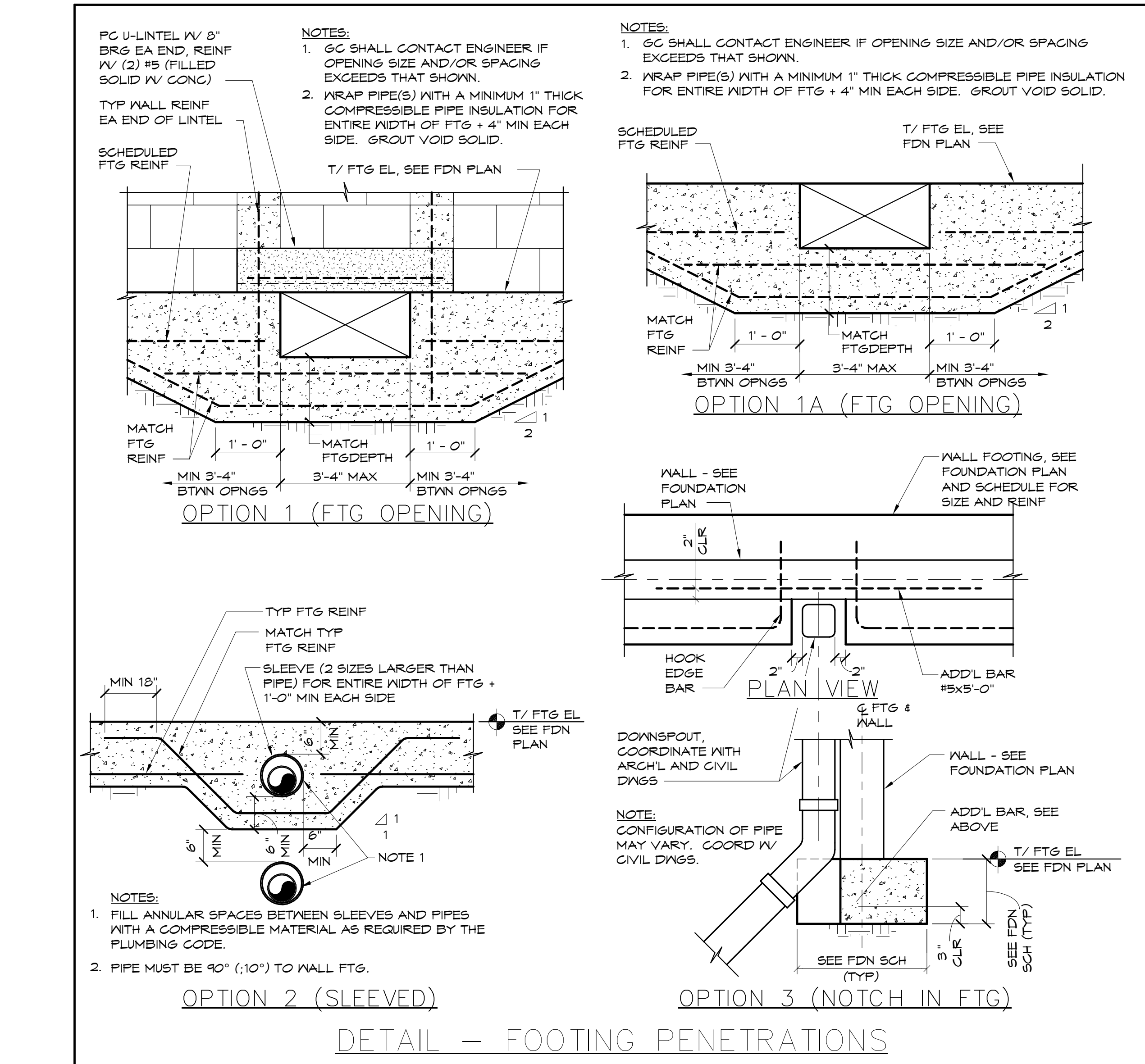
REINFORCEMENT IN WALLS, FOOTINGS AND BEAMS SHALL BE CONTINUOUS AND LAPPED 48 BAR DIA AT SPLICE UNLESS OTHERWISE NOTED. HOOK AND LAP ALL CORNER AND INTERSECTING BARS. (SEE REINF DEVELOPMENT DETAIL).



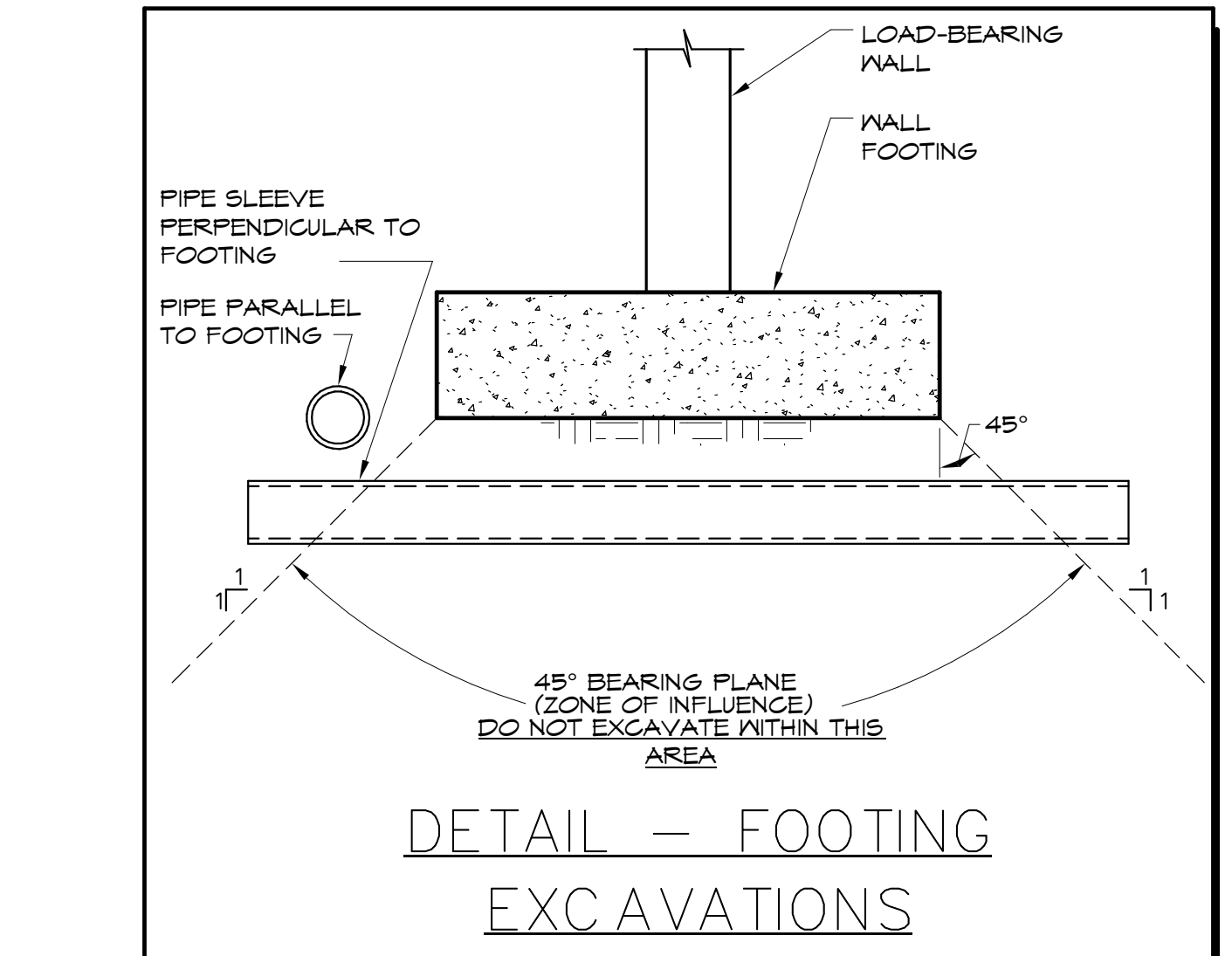
COVER FOR REINFORCING SHALL BE AS FOLLOWS:

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	3"
CONCRETE EXPOSED TO EARTH OR WEATHER: #6 THRU #16 BARS: #5 BAR, #31 OR #31 WIRE AND SMALLER:	2" 1 1/2"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER: SLABS, WALLS: #14 AND #16 BARS: #11 BAR AND SMALLER:	1 1/2" 3/4"
BEAMS, COLUMNS: PRIMARY REINF, TIES, STIRRUPS:	1 1/2"
TILT-UP CONCRETE PANELS - FACE COVER: PANELS EXPOSED TO EARTH OR WEATHER: #11 BAR AND SMALLER:	1" (EXCEPT 3/4" ALLOWED AT REVEALS)
PANELS NOT EXPOSED TO EARTH OR WEATHER: #11 BAR AND SMALLER:	3/4"

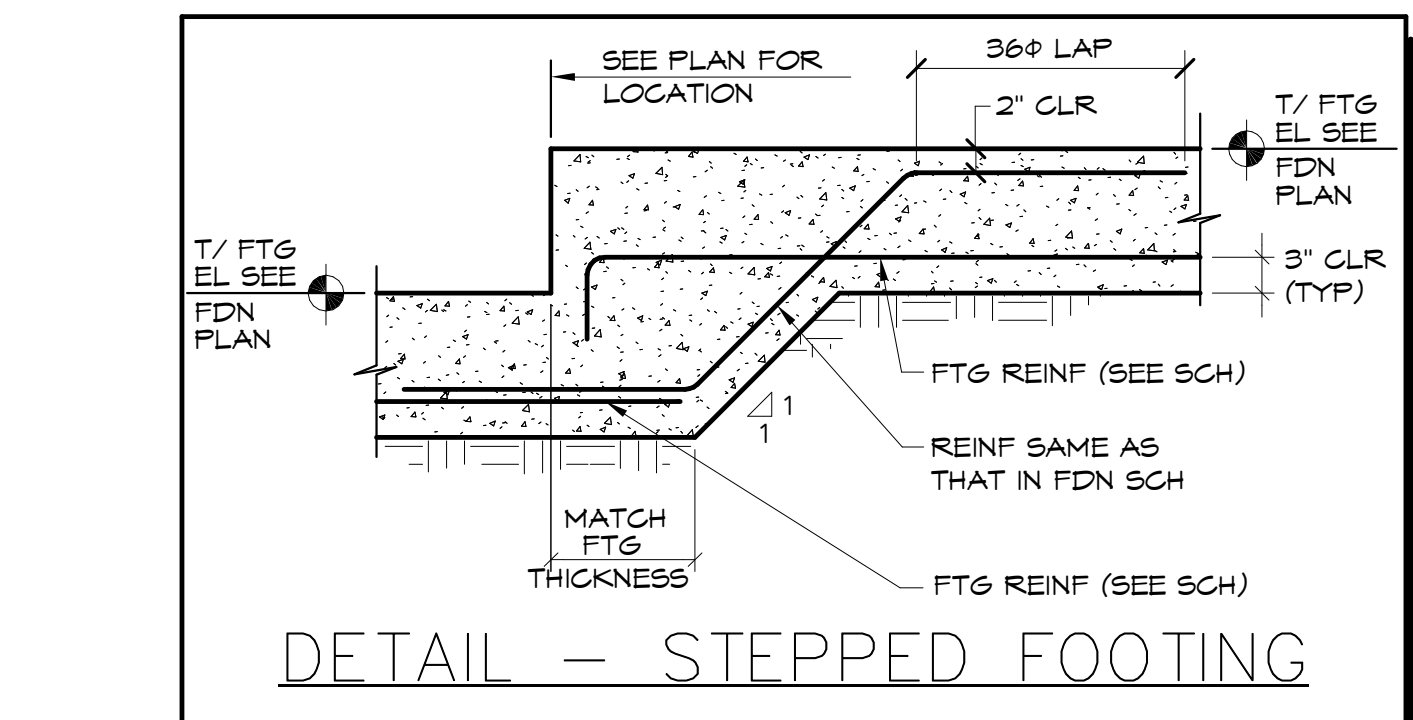
FOOTING PENETRATION DETAILS:



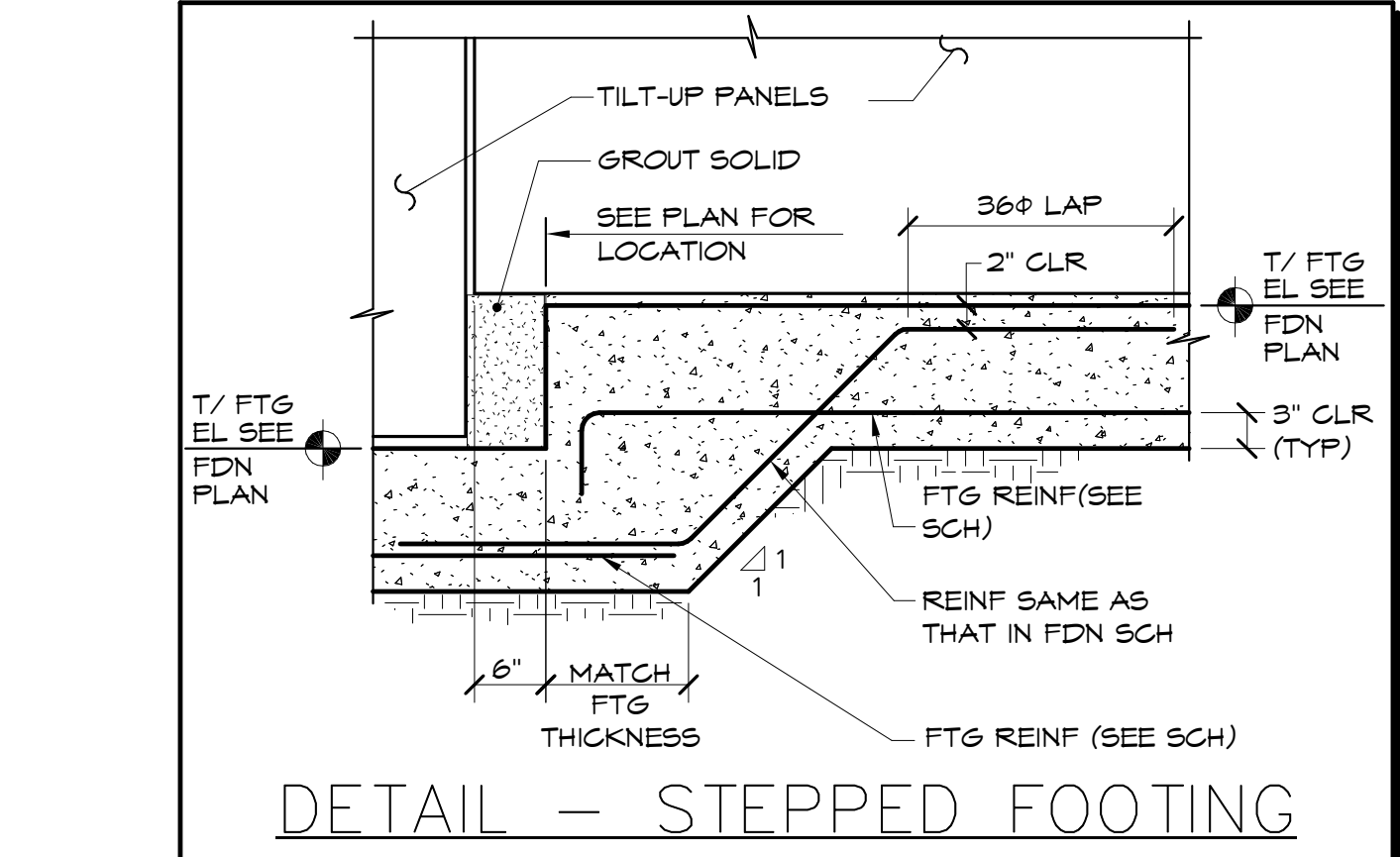
FOOTING EXCAVATION DETAIL:



STEPPED FOOTING DETAIL (IF REQUIRED):



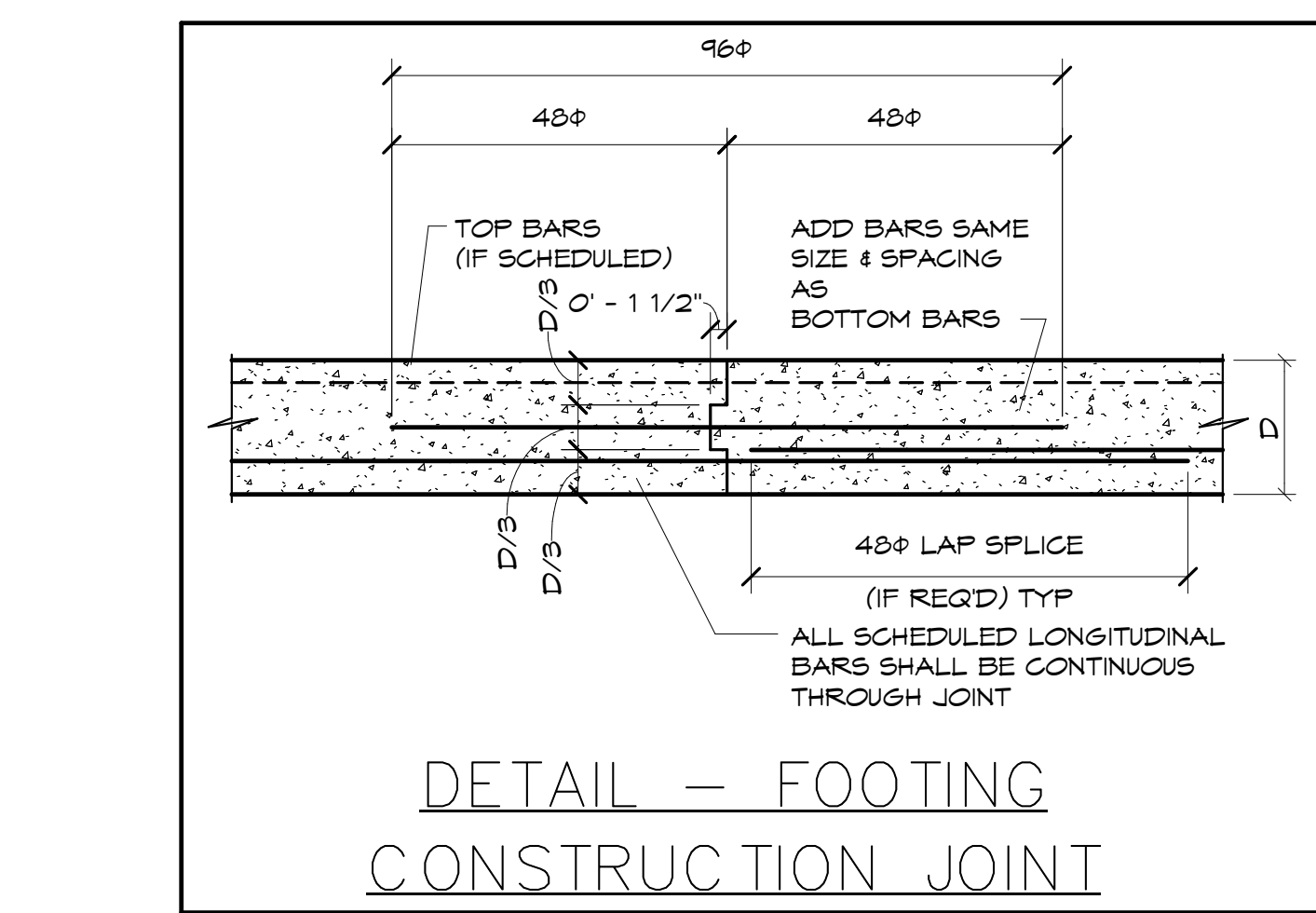
STEPPED FOOTING DETAIL (IF REQUIRED):



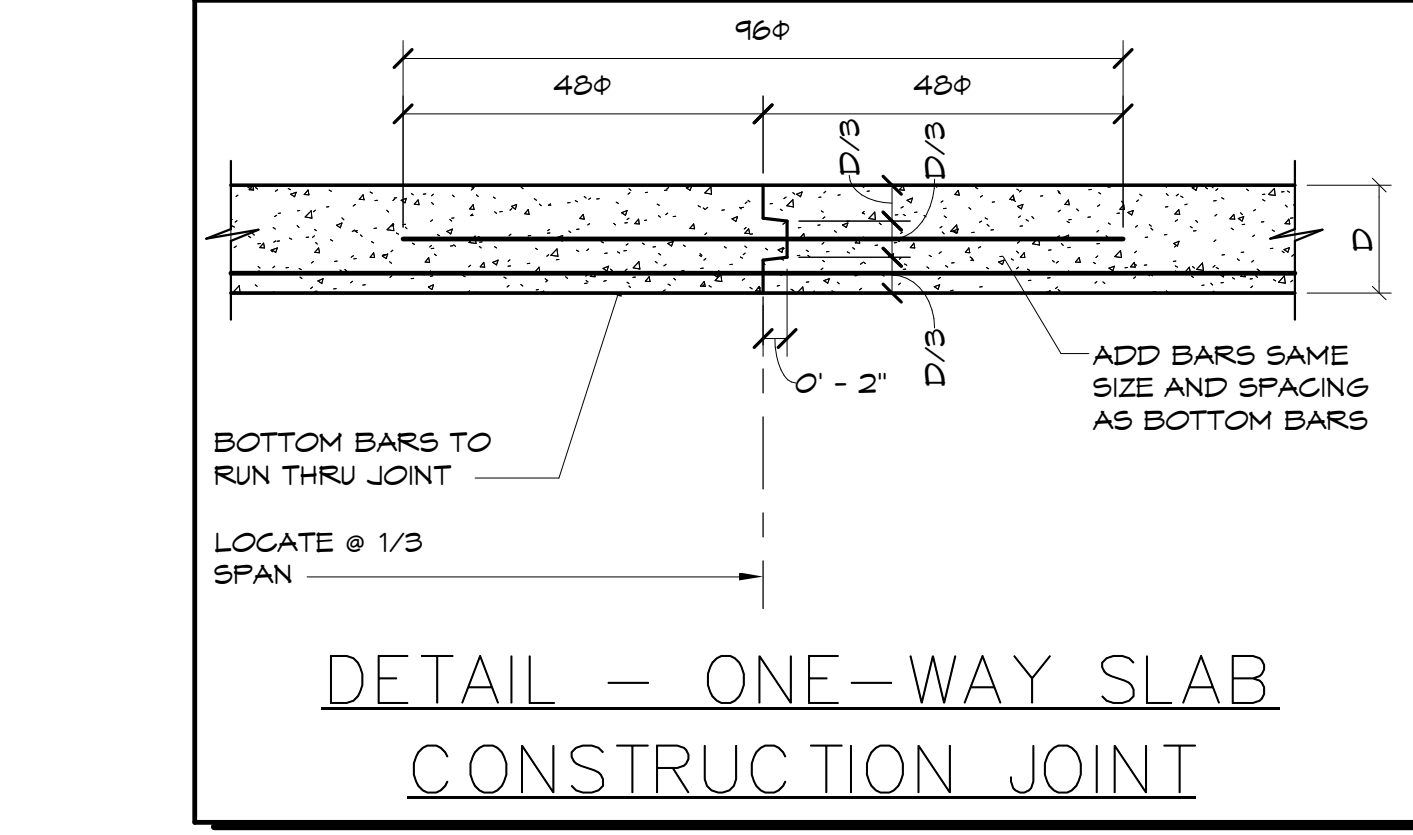
TERMINATE ALL DISCONTINUED ELEVATED SLAB TOP BARS WITH A 180 DEGREE STANDARD HOOK UNLESS OTHERWISE NOTED.

CONTINUOUS TOP BARS SHALL BE SPLICED AT MID-SPAN. CONTINUOUS BOTTOM BARS SHALL BE SPLICED AT CENTER-LINE OF SUPPORTS (OR AS SHOWN ON TYPICAL DETAILS).

FOOTING CONSTRUCTION JOINT (IF REQUIRED):



ONE-WAY SLAB CONSTRUCTION JOINT (IF REQUIRED):



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LUIS F. BEDOYA, P.E.
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MARK	DESCRIPTION	DATE

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St. Lucie Public Schools
 9461 Brandywine Ln
 Port St. Lucie, FL 34986

ISSUE DATE: 12/16/2022
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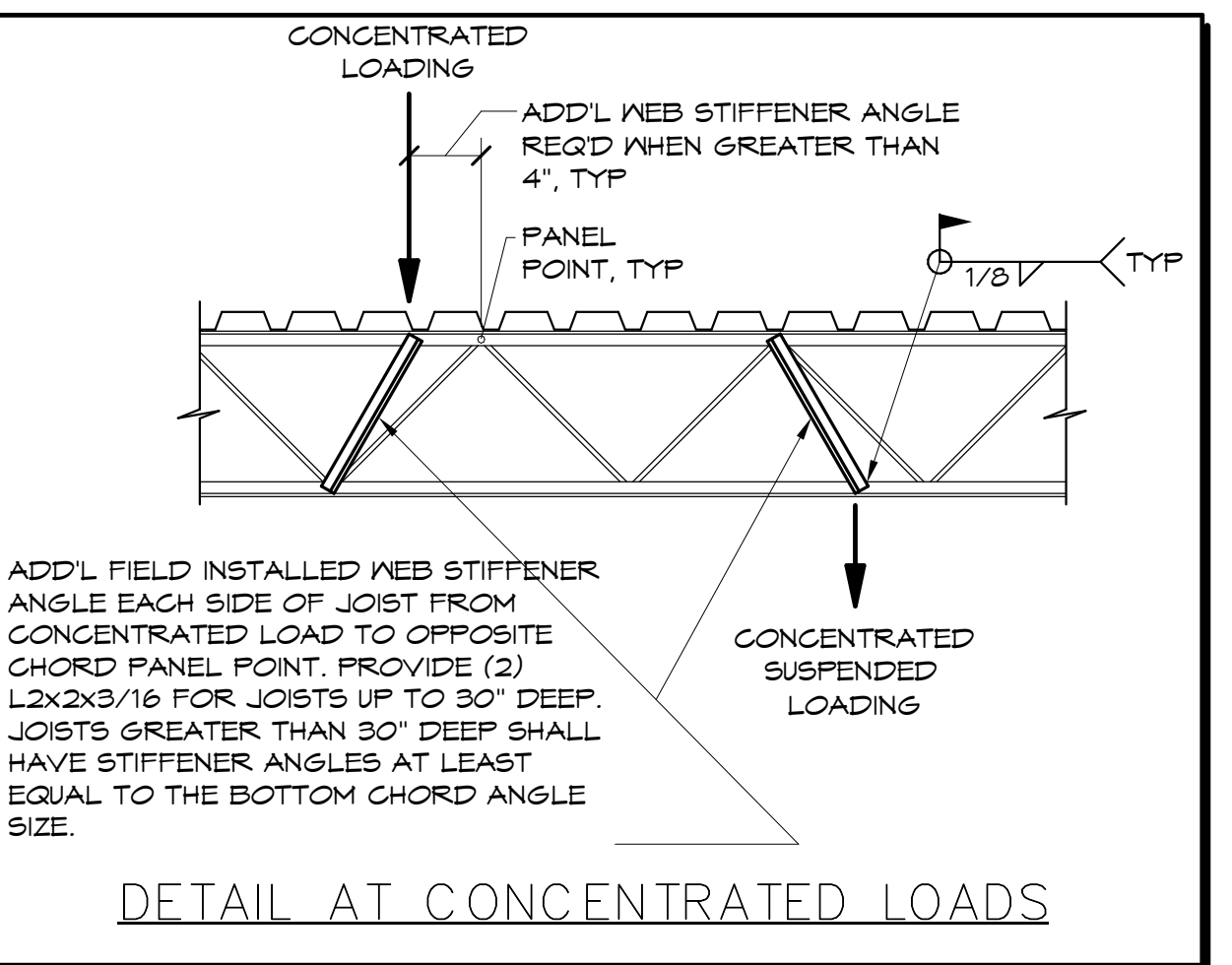
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STRUCTURAL GENERAL NOTES

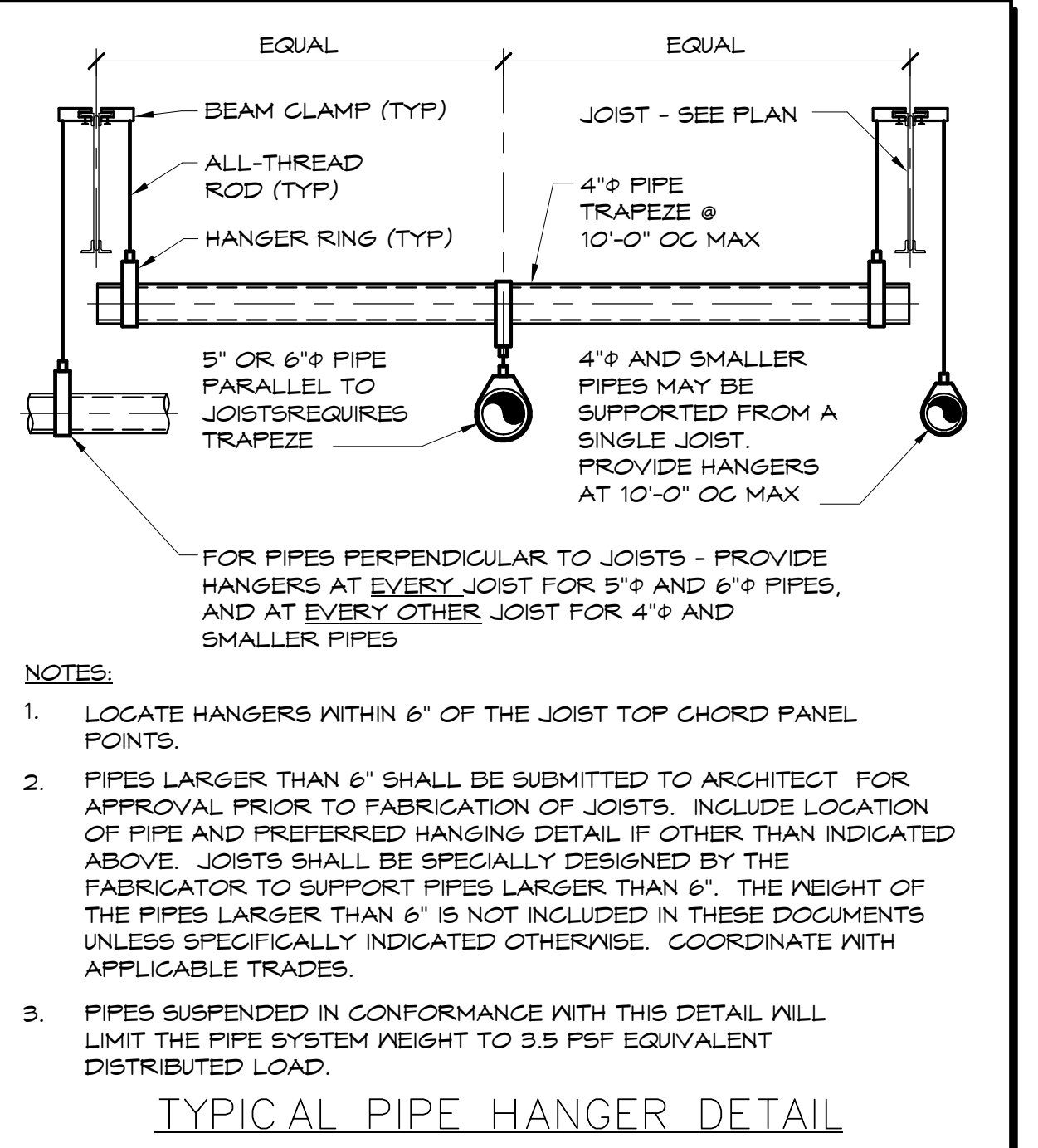
S002

STEEL JOISTS

- SJ1 A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
- SJ2 ALL DESIGN, FABRICATION AND ERECTION OF STEEL JOISTS AND BRIDGING SHALL BE IN STRICT ACCORDANCE WITH THE SPECIFICATIONS OF STEEL JOIST INSTITUTE (OPEN WEB STEEL JOIST SJI PUBLICATION SJI 100-2015, AND JOIST GIRDERS SJI 200-2015) AND RECOMMENDED CODE OF STANDARD PRACTICE.
- SJ3 THE ENDS OF ALL BRIDGING LINES TERMINATING AT WALLS OR BEAMS SHALL BE ANCHORED TO THE WALL OR BEAM.
- SJ4 ALL STEEL JOISTS ARE TO BE CAMBERED AS SPECIFIED BY STEEL JOIST INSTITUTE.
- SJ5 PROVIDE BOTTOM AND/OR TOP CHORD EXTENSIONS AS SHOWN ON DRAWINGS.
- SJ6 UNLESS NOTED OTHERWISE, MINIMUM JOIST BEARING SHALL BE 2 1/2" FOR K-SERIES JOISTS, 4" FOR LH, DLH AND SLH 15-18, AND 6" FOR SLH 19-25 ON A STEEL MEMBER OR EMBED PLATE.
- SJ7 BRIDGING SHALL BE FURNISHED AND INSTALLED TO MEET THE SIZE AND SPACING REQUIREMENTS OF THE SJI STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOISTS. ALL BRIDGING AND BRIDGING ANCHORS SHALL BE COMPLETELY INSTALLED BEFORE CONSTRUCTION LOADS ARE PLACED ON THE JOISTS.
- SJ8 ALL HANGERS, CURBS AND/OR ROOFTOP FRAMES TO SUPPORT MECHANICAL EQUIPMENT, ETC., TO BE SUPPORTED BY THE JOISTS SHALL BE LOCATED AT THE PANEL POINTS OF THE JOISTS IF POSSIBLE. HOWEVER, IF THE CONCENTRATED LOAD MUST BE LOCATED FURTHER THAN 4" FROM A PANEL POINT, PROVIDE WEB STIFFENER ANGLES. WEB STIFFENERS MUST BE INSTALLED EACH SIDE OF JOIST FROM CONCENTRATED LOAD TO OPPOSITE CHORD PANEL POINT BEFORE LOAD IS APPLIED. SEE DETAIL BELOW.



- SJ9 CONTRACTOR TO FURNISH BAR JOIST CERTIFICATIONS SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. THE SPECIALTY ENGINEER FOR THE STEEL JOIST SUPPLIER SHALL ALSO CERTIFY THAT THE STEEL JOIST BOTTOM CHORDS WILL SAFELY RESIST THE WIND UPLIFTS, CONSIDERING THE SPACING OF BRIDGING.
- SJ10 PROVIDE UPLIFT BRIDGING PER TABULATED PRESSURES ON SHEET S603.
- SJ11 ALL ITEMS SUSPENDED FROM JOISTS (I.E. CATWALKS, BALCONIES, OPERABLE PARTITIONS, ETC.) SHALL BE INSTALLED AFTER DEAD LOAD HAS BEEN APPLIED.
- SJ12 BOLTED TIE JOISTS (BTJ) ARE USED IN STEEL FRAMES WHERE COLUMNS ARE NOT FRAMED IN AT LEAST TWO DIRECTIONS WITH STRUCTURAL STEEL MEMBERS. JOIST(S) AT COLUMN LINES SHALL BE FIELD BOLTED AT THE COLUMNS WITH TWO 1/2" BOLTS TO PROVIDE LATERAL STABILITY DURING CONSTRUCTION.
- SJ13 STEEL JOISTS SHALL RECEIVE SHOP COAT OF PRIMER (COLOR AS DIRECTED BY ARCHITECT) EXCEPT THOSE AREAS WHICH WILL RECEIVE SPRAY-ON FIRE PROTECTION.
- SJ14 ANY STEEL JOIST WITHIN A 4'-0" DISTANCE FROM A PARALLEL SUPPORT SHALL BE FABRICATED IN SUCH A WAY THAT CAMBER OF THE JOIST WILL NOT CAUSE A PROBLEM INSTALLING THE METAL DECK.
- SJ15 IN THE EVENT THAT FIRE SPRINKLERS ARE REQUIRED FOR THIS PROJECT, THE STEEL FABRICATOR SHALL PROVIDE A DIMENSIONED JOIST BRIDGING AND JOIST GIRDER BOTTOM CHORD BRACE PLAN ALONG WITH DETAILS TO THE SPRINKLER CONTRACTOR. THE FABRICATOR AND SPRINKLER CONTRACTOR SHALL COORDINATE WITH EACH OTHER TO ENSURE THAT ANY CONFLICTS ARE RESOLVED BEFORE ANY FABRICATION BEGINS.
- SJ16 ALL PIPES MUST BE SUPPORTED AS SHOWN BELOW:



- NOTES:**
- LOCATE HANGERS WITHIN 6" OF THE JOIST TOP CHORD PANEL POINTS.
 - PIPES LARGER THAN 6" SHALL BE SUBMITTED TO ARCHITECT FOR APPROVAL PRIOR TO FABRICATION OF JOISTS. INCLUDE LOCATION OF PIPE AND PREFERRED HANGING DETAIL, IF OTHER THAN INDICATED ABOVE. JOISTS SHALL BE SPECIALLY DESIGNED BY THE FABRICATOR TO SUPPORT PIPES LARGER THAN 6". THE WEIGHT OF THE PIPES LARGER THAN 6" IS NOT INCLUDED IN THESE DOCUMENTS UNLESS SPECIFICALLY INDICATED OTHERWISE. COORDINATE WITH APPLICABLE TRADES.
 - PIPES SUSPENDED IN CONFORMANCE WITH THIS DETAIL WILL LIMIT THE PIPE SYSTEM WEIGHT TO 3.5 PSF EQUIVALENT DISTRIBUTED LOAD.

STEEL JOISTS (cont.)

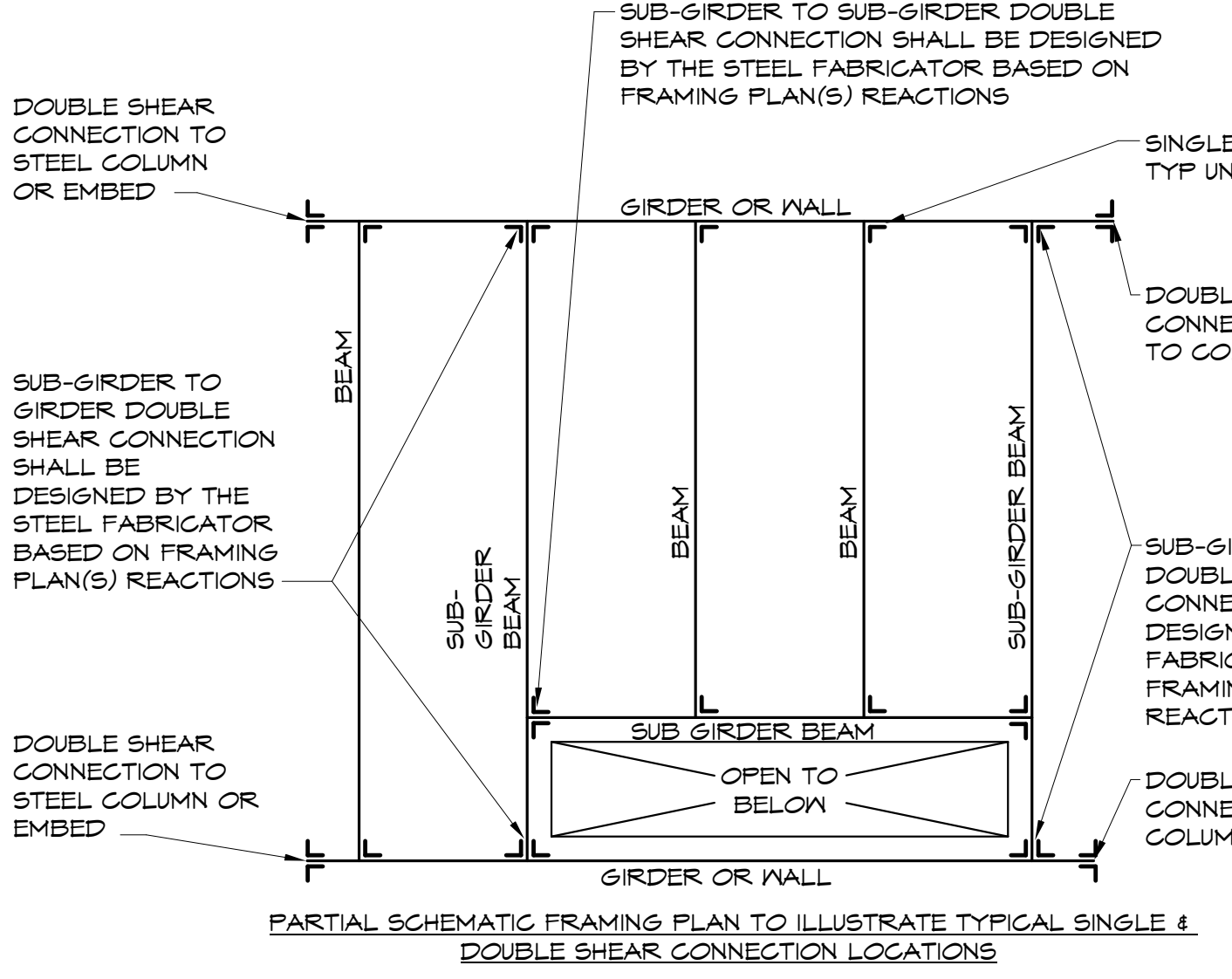
- SJ17 FABRICATOR SHALL ENSURE THAT ALL OSHA REQUIREMENTS ARE MET. PARTICULAR ATTENTION SHALL BE PAID TO THE ERECTION PROCESS. BOLTED CONNECTIONS MAY BE REQUIRED. SUBMIT DETAILS FOR APPROVAL.
- SJ18 THE JOIST MANUFACTURER MAY NOT INCREASE ALLOWABLE STRESSES.
- SJ19 THE OPEN WEB STEEL JOISTS SHALL BE FABRICATED AND ERECTED IN FULL CONFORMANCE WITH THE "OSHA STEEL ERECTION STANDARD". IF THE CONSTRUCTION DRAWINGS DEVIATE FROM THE OSHA STANDARD THEN THE FABRICATOR SHALL PROVIDE SUBMITTALS THAT CLEARLY INDICATE THE DEVIATION WITH A REVISION CLOUD AND REQUEST APPROVAL FROM BEM TO MAKE THE CHANGE SO THAT CONFORMANCE WITH THE OSHA STANDARD IS ASSURED.
- SJ20 THE SUPPORTS FOR SCISSOR, ARCHED CHORDS OR ANY OTHER SIMILAR TYPE JOIST, UNLESS NOTED OTHERWISE, SHALL BE DESIGNED WITH A PINNED CONNECTION AT ONE END AND A HORIZONTAL ROLLER AT THE OTHER END. THE SUPPORTING STRUCTURE, UNLESS NOTED OTHERWISE, HAS NOT BEEN DESIGNED FOR ANY GRAVITY LOAD HORIZONTAL FORCE DUE TO DEFLECTION. DO NOT DESIGN THESE TYPE OF JOISTS WITH PINNED SUPPORTS EACH END.
- SJ21 K-SERIES STEEL JOISTS WITH SPANS 40'-0" AND LONGER SHALL BE ERECTED IN PANELS SO THAT BOLTED CONNECTIONS ARE NOT REQUIRED EXCEPT AT THE COLUMN LINES). THE GC SHALL INSURE THAT ALL RELATED JOIST FRAMING COMPONENTS ARE COORDINATED TO MEET THIS REQUIREMENT.
- SJ22 ALL ROOFS THAT EXCEED 1/4"/FT SLOPE SHALL HAVE THE JOIST BEARING SEATS SLOPED AS REQUIRED PER STEEL JOIST INSTITUTE.

PRECAST HOLLOW-CORE PLANKS

- HC1 PRECAST PLANKS SHALL BE 8" PRESTRESSED HOLLOW CONCRETE PLANKS, MACHINE-EXTRUDED WITH ZERO SLUMP. THE PRESTRESSED CONCRETE MANUFACTURER'S PLANT SHALL CONFORM TO THE REQUIREMENTS DEFINED IN PRESTRESSED CONCRETE INSTITUTE'S "116-MANUAL FOR QUALITY CONTROL FOR PLANTS AND PRODUCTION OF PRECAST, PRESTRESSED CONCRETE PRODUCTS".
- HC2 CONCRETE COMPRESSIVE STRENGTH: MINIMUM 4000 PSI AT TRANSFER, AND 5000 PSI AT 28 DAYS.
- HC3 PRESTRESSED STRAND: PER MANUFACTURER.
- HC4 DESIGN IN ACCORDANCE WITH ACI 318. LIVE-LOAD DEFLECTION SHALL NOT EXCEED THE SPAN/360.
- HC5 REFER TO THE ARCHITECTURAL DRAWINGS FOR ANY FIRE RATING REQUIREMENTS.
- HC6 SUBMIT SHOP DRAWINGS SHOWING SETTING PLAN, UNIT SIZE, STEEL CONTENT AND ALL NECESSARY DETAILS. ALL SHOP DRAWINGS SHALL BE CHECKED AND INITIALED BY CONTRACTOR. SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION.
- HC7 PRECAST PLANKS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. PROVISIONS SHALL BE MADE FOR ANCHORAGE TO BEAMS, WALLS AND OTHER STRUCTURAL ELEMENTS TO PROVIDE LATERAL SUPPORT OF SUCH MEMBERS.
- HC8 INSTALL PLANKS IN A TIGHT, LEVEL POSITION ON TRUE LEVEL BEARING SURFACE. PRECAST SLABS SHALL BEAR ON 1/8" BEARING PADS TO DISTRIBUTE THE BEARING STRESSES. MINIMUM BEARING SHALL BE 3" ON MASONRY AND 2 1/2" ON STRUCTURAL STEEL AND CONCRETE FOR STRESSES IN THE SUPPORT MEMBERS. THE MINIMUM BEARING FOR STRESSES IN THE PLANKS SHALL BE DETERMINED BY THE PRE-CAST MANUFACTURER. THE GREATER OF THE MINIMUM BEARINGS SHALL BE PROVIDED OR THE PLANKS SHALL BE IMMEDIATELY SHORED AND THE ARCHITECT NOTIFIED.
- HC9 SUPPLY AND INSTALL STEEL HANGERS WHERE REQUIRED FOR LARGE OPENINGS.
- HC10 DO NOT CUT ANY REINFORCING WITHOUT APPROVAL OF PRECAST PLANK MANUFACTURER AND ENGINEER.
- HC11 GROUT PRECAST UNITS WITH 1:3 CEMENT-SAND GROUT MIX PRIOR TO PLACING TOPPING OR LOADS ON THE UNITS, PROPERLY FILLING SHEAR KEYWAYS BETWEEN UNITS.
- HC12 THE PRE-CAST MANUFACTURER SHALL TAKE INTO ACCOUNT THE REDUCTION OF THE TOPPING THICKNESS CAUSED BY CAMBER IN THE PLANK DESIGN. THE TOPPING THICKNESS CALLED FOR IN THE DRAWINGS REFERS TO THE THICKNESS AS MEASURED AT THE BEARING LOCATIONS. THE GENERAL CONTRACTOR SHALL NOTIFY THE ARCHITECT AND ENGINEER DURING PLANK DESIGN PHASE AND PRIOR TO FABRICATION OF ANY "PROBLEM" PLANKS SHOULD THE REDUCTION IN TOPPING THICKNESS AT MID-SPAN DECREASE THE LOADING CARRYING CAPACITY BELOW DESIGN LOADS SPECIFIED ON THE FRAMING PLAN(S).
- HC13 THE CONCRETE TOPPING SHALL BE NORMAL-WEIGHT CONCRETE OF MINIMUM 3000 PSI COMPRESSIVE STRENGTH WITH A MINIMUM THICKNESS OF 2", REINFORCED WITH 6x6-11.4X11.4 WAF. ADEQUATE BOND TO BE ACCOMPLISHED BY CLEANING AND WETTING PLANKS BEFORE POURING THE TOPPING.
- HC14 ALL EXTERIOR TOPPING AND INTERIOR TOPPING EXPOSED TO VIEW SHALL HAVE A 4" WIDE RUBBER BOND BREAKER STRIP PLACED OVER, CENTERED ON AND AFFIXED TO THE PLANKS (BUT NOT THE GROUT JOINT ITSELF) AT EVERY PLANK-TO-PLANK JOINT PRIOR TO PLACING THE CONCRETE TOPPING. THE CONCRETE TOPPING SHALL BE TOoled AT EVERY PLANK-TO-PLANK JOINT WITH A "DEEP AND THIN GROOVER" AS MANUFACTURED BY GOLDBLATT TOOLS (PART #606211) OR APPROVED EQUAL WHICH WILL CREATE AN 1/8" WIDE X 1" DEEP GROOVE.

STRUCTURAL STEEL

- S51 A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.
- S52 FABRICATE AND ERECT STRUCTURAL STEEL IN CONFORMANCE WITH THE LATEST VERSION OF AISC 360-16.
- S53 MATERIAL SPECIFICATIONS:
ALL STEEL SHALL BE PRODUCED DOMESTICALLY.
ROLLED SHAPES, PLATES AND BARS: ASTM A36, EXCEPT WIDE-FLANGE 4 FT SECTIONS, WHICH SHALL BE ASTM A992.
HOLLOW STRUCTURAL SECTION (HSS): ASTM A500, GRADE B.
ANCHOR BOLTS, RODS, NUTS AND WASHERS: FER BASE PLATE SCHEDULE.
HEADED STUDS: ASTM A108, GRADE 1015 THROUGH 1020, GOLD-FINISHED CARBON STEEL, AYS D1.1, TYPE B.
BOLTED STRUCTURAL CONNECTIONS, UNLESS NOTED OTHERWISE, ALL BOLTS SHALL BE 3/4" ASTM A325, TYPE N. BOLTS INDICATED LESS THAN 3/8" SHALL BE ASTM A307.
WELDED CONNECTIONS: ELECTRODES - E70XX UNO (LOW HYDROGEN). FILLET WELDS SHALL BE 3/16" UNO.
HIGH-STRENGTH FIELD-BOLTED CONNECTIONS SHALL BE INSTALLED, TIGHTENED, TESTED AND INSPECTED ACCORDING TO "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS" BY THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (RCSC). ALL BOLTS IN STEEL TO STEEL CONNECTIONS SHALL BE BROUGHT TO A "SNUG-TIGHT" CONDITION, AS DEFINED IN THE SPECIFICATION. ALL BOLTS IN STEEL TO EMBED CONNECTIONS SHALL BE FINGER-TIGHT WITH FINED THREADS. SLIP-CRITICAL (SC) BOLTS MUST BE FULLY TENSIONED PER SPECIFICATION.
- S55 STANDARD NON-SLOPED AND NON-SKEWED SHEAR CONNECTIONS HAVE BEEN DESIGNED AND THE NECESSARY INFORMATION MAY BE FOUND IN THE SCHEDULES. THE ULTIMATE (I.E. FACTORED) REACTIONS HAVE BEEN PROVIDED AT EACH END OF EACH MEMBER SHOULD THE FABRICATOR WISH TO RE-ENGINEER THE CONNECTIONS TO THEIR PREFERENCES. SHOULD THE FABRICATOR WISH TO RE-ENGINEER THE CONNECTIONS, THEY MUST PROVIDE SUBMITTALS THAT HAVE BEEN PREPARED AND SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE SAME STATE AS THE PROJECT LOCATION.
- S55A NON-STANDARD SLOPED AND/OR SKEWED SHEAR CONNECTIONS SHALL BE DESIGNED & DETAILED BY THE FABRICATOR'S ENGINEER. PROVIDE SIGNED & SEALED CONNECTION SUBMITTAL FOR REVIEW.
- S56 ALL WIDE FLANGE FLOOR MEMBERS SHALL BE CONNECTED TO THE SUPPORTING STRUCTURE AS DETAILED IN CONNECTION SCHEDULES ON SHEET S602. UNLESS SPECIFICALLY NOTED OTHERWISE ON PLAN(S), ANY FLOOR MEMBER SUPPORTING ANOTHER FLOOR MEMBER SHALL BE CONNECTED AS DETAILED IN DOUBLE SHEAR SCHEDULES N1/S602 AND N9/S602. SINGLE SHEAR CONNECTIONS AS DETAILED IN SCHEDULES J1/S602 AND J12/S602 SHALL ONLY BE USED FOR FLOOR MEMBERS SUPPORTING DECK/SLAB ONLY (I.E. FILLER BEAMS) OR AS SPECIFICALLY IDENTIFIED ON PLAN OR SECTION. THE USE OF A DOUBLE SHEAR CONNECTION MAY BE REQUIRED FOR A "TYPICAL" FLOOR BEAM DUE TO BEAM REACTION. REFER TO REACTION NOTED ON PLAN(S) & COORDINATE WITH SCHEDULED MAXIMUM VALUES TO DETERMINE CONNECTION TYPE REQUIRED. SEE PARTIAL SCHEMATIC FRAMING PLAN BELOW THAT ILLUSTRATES WHERE TYPICAL SINGLE AND DOUBLE SHEAR CONNECTIONS ARE REQUIRED.



- S57 ALL WIDE FLANGE ROOF MEMBERS SHALL BE CONNECTED TO THE SUPPORTING STRUCTURE AS DETAILED IN CONNECTION SCHEDULES ON SHEET S602. UNLESS SPECIFICALLY NOTED OTHERWISE ON PLAN, ALL ROOF MEMBERS SHALL BE CONNECTED AS DETAILED IN THE SINGLE SHEAR SCHEDULES J1/S602 AND J12/S602.
- S58 BRACE AND MAINTAIN ALL STEEL IN ALIGNMENT UNTIL OTHER PARTS OF CONSTRUCTION NECESSARY FOR PERMANENT SUPPORT ARE COMPLETED. CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLING TEMPORARY SHORING AS REQUIRED FOR THE STABILITY OF THE STEEL FRAME UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN COMPLETED AND BUILDING IS ENCLOSED.
- S59 ALL WELDING SHALL CONFORM TO THE REQUIREMENTS OF "THE STANDARD CODE FOR WELDING IN BUILDING CONSTRUCTION" OF THE AMERICAN WELDING SOCIETY.
- S60 GROUT FOR COLUMN BASE PLATES AND PRESET BEARING PLATES SHALL BE NON-SHRINK, NON-METALLIC GROUT (5000 PSI MIN).
- S61 SUBMIT SHOP DRAWINGS INDICATING ALL SHOP AND ERECTION DETAILS INCLUDING PROFILES, SIZES, SPACING AND LOCATIONS OF STRUCTURAL MEMBERS, CONNECTION ATTACHMENTS, FASTENERS, LOADS AND TOLERANCES.
- S62 ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIP GALVANIZED IN ACCORDANCE WITH ASTM A123 FOR MEMBERS AND ASTM A193 FOR CONNECTION ELEMENTS, EXCEPT THAT ALL ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS) SHALL BE BLAST CLEANED AND COATED IN ACCORDANCE WITH THE STRUCTURAL STEEL AND PAINT SPECIFICATIONS.
- S63 STRUCTURAL STEEL SHALL RECEIVE A SHOP COAT OF PRIMER (COLOR AS DIRECTED BY ARCHITECT) EXCEPT THOSE AREAS WHICH WILL RECEIVE SPRAY-ON FIRE PROTECTION, OR WHERE HEADED STUDS ARE TO BE WELDED.
- S64 PROVIDE ALLOWANCE FOR 300 FEET OF L3x3x1/4 AND LABOR TO INSTALL SAME AT ARCHITECT OR ENGINEER'S DIRECTION. CREDIT OWNER WITH ALL MATERIAL AND LABOR NOT USED.
- S65 STEEL BEAMS SHALL BE CAMBERED AS INDICATED ON THE FRAMING PLAN(S). IF NO CAMBER IS SPECIFIED, THE FABRICATOR SHALL ENSURE THAT THE "NATURAL" CAMBER IN ALL ERECTED BEAMS OCCURS IN AN UPWARD DIRECTION.
- S66 ALL STEEL BEAMS THAT ARE PARALLEL TO BAR JOISTS SHALL BE CAMBERED TO MATCH THE JOIST CAMBER AND THE TOP OF THE BEAM SHALL MATCH THE TOP OF THE JOIST, TYP UNO.
- S67 THE STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN FULL CONFORMANCE WITH THE "OSHA STEEL ERECTION STANDARD". IF THE CONSTRUCTION DRAWINGS DEVIATE FROM THE OSHA STANDARD THEN THE FABRICATOR SHALL PROVIDE SUBMITTALS THAT CLEARLY INDICATE THE DEVIATION WITH A REVISION CLOUD AND REQUEST APPROVAL FROM BEM TO MAKE THE CHANGE SO THAT CONFORMANCE WITH THE OSHA STANDARD IS ASSURED.
- S68 REFER TO SPECIALTY ENGINEERING (SE) NOTES FOR DELEGATED ENGINEERING REQUIREMENTS.
- S69 CONTRACTOR SHALL INCLUDE IN THE BID THE COST TO FABRICATE, SUPPLY AND INSTALL FOUR TONS OF MISCELLANEOUS STEEL IN ADDITION TO WHAT IS SHOWN IN THE CONSTRUCTION DOCUMENTS. LABOR FOR THIS SHALL BE A MINIMUM OF 150 MAN-HOURS.

ELEVATION SYMBOL

GRID ID

REVISION MARK

TYPICAL PLAN KEYNOTE SYMBOL

CHANGE IN SLAB ELEVATION

SECTION CUTS

NUMBER FOR SECTION LETTER FOR DETAIL

SHEET ON WHICH SECTION OR DETAIL OCCURS

SECTION & DETAIL ID

H10 SECTION

SCALE

TYPICAL LINE TYPES

- GRID LINE
- EXISTING
- MATCH LINE
- HIDDEN
- DASHED
- CONTINUOUS

MATERIALS

- GROUT
- STEEL
- PRECAST CONCRETE
- CONCRETE
- EARTH

SECTION HATCHES

- 5" LOAD-BRG CMU WALLS
- NON-LOAD-BRG CMU WALLS
- CONCRETE TILT-UP WALL PANELS

PLAN HATCHES

SYMBOL LEGEND

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MARK	DESCRIPTION	DATE
ADDRESS NO. 2		06/06/2022


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	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18
R																		
A.	GENERAL:																	
1.	THE THRESHOLD INSPECTION SHALL VISUALLY INSPECT THE STRUCTURAL COMPONENTS AT THE MAIN JOB SITE AFTER FABRICATION. THE THRESHOLD INSPECTOR MAY EMPLOY DULY AUTHORIZED REPRESENTATIVE IN THE FIELD. HOWEVER, SUCH REPRESENTATIVES MUST:									SLAB ON GRADE								
2.	THE SCOPE OF INSPECTION SHALL INCLUDE, BUT SHALL NOT NECESSARILY BE LIMITED TO, THE ITEMS LISTED IN THIS PLAN. ALL STRUCTURAL COMPONENTS, WHICH ARE RELATED TO THE PUBLIC HEALTH, SAFETY, OR WELFARE, ARE TO BE INSPECTED BY A SPECIAL INSPECTOR.									1. SLAB SUBGRADE SHALL BE INSPECTED FOR COMPACTION AND LEVELNESS. CHECK ALL UTILITY TRENCHES FOR PROPER BACK-FILL AND COMPACTION. ENSURE THAT NO DEPRESSIONS OR RUTS EXIST IN SLAB AREA OTHER THAN THOSE REQUIRED BY DESIGN. SPECIAL INSPECTOR SHALL CHECK STRUCTURAL LOAD-BEARING (NON-STRUCTURAL FLOATING SLABS NEED NOT BE CHECKED) SLAB THICKNESS.								
2A.	THE SPECIAL INSPECTOR SHALL BE A REGISTERED ARCHITECT OR PROFESSIONAL ENGINEER IN FLORIDA. THE SPECIAL INSPECTOR SHALL BE A SINGULAR PERSON, NOT A FIRM OR COMPANY, NOR TWO OR MORE PERSONS TAKEN COLLECTIVELY. THE SPECIAL INSPECTOR SHALL MEET THE REQUIREMENTS OF SECTION 61615-35.003, F.A.C.									2. CHECK VAPOR BARRIER FOR PROPER THICKNESS AND PLACEMENT, ENSURE THAT THERE ARE NO PUNCTURES OR TEARS AND THAT THE EDGES HAVE BEEN LAPPED AND TAPED AS REQUIRED BY CONSTRUCTION DOCUMENTS.								
	A. PROOF OF CURRENT LICENSE IN GOOD STANDING AS A LICENSED PROFESSIONAL ENGINEER IN THE STATE OF FLORIDA WHOSE PRINCIPAL PRACTICE IS STRUCTURAL ENGINEERING OR WHOSE PRINCIPAL PRACTICE IS IN PERFORMING STRUCTURAL FIELD INSPECTIONS ON THRESHOLD BUILDINGS.									3. INSPECT PLACEMENT OF SLAB REINFORCEMENT (I.E. REBAR OR WELDED WIRE FABRIC). ENSURE THAT REINFORCEMENT HAS BEEN PROPERLY CHAIRED OR SUPPORTED.								
	B. LICENSED PROFESSIONAL ENGINEERS WHOSE PRINCIPAL PRACTICE IS STRUCTURAL ENGINEERING SHALL ALSO HAVE THREE (3) YEARS OF EXPERIENCE IN PERFORMING STRUCTURAL FIELD INSPECTIONS ON THRESHOLD BUILDINGS OR EQUIVALENT PURSUANT TO A THRESHOLD / SPECIAL INSPECTION PLAN RELEVANT TO THE WORK PERFORMED AND TWO (2) YEARS OF EXPERIENCE IN THE STRUCTURAL DESIGN OF THRESHOLD BUILDINGS FOR THE PURPOSE OF THESE CRITERIA. STRUCTURAL DESIGN SHALL MEAN THE DESIGN OF ALL STRUCTURAL COMPONENTS OF THE BUILDING AND SHALL NOT BE LIMITED TO SPECIFIC STRUCTURAL COMPONENTS ONLY, SUCH AS FOUNDATIONS, PRESTRESSED OR POST-TENSIONED CONCRETE, ETC.									4. ENSURE THAT REINFORCING REMAINS AS PLACED AND IS NOT WALKED DOWN DURING CONCRETE PLACEMENT.								
	C. LICENSED PROFESSIONAL ENGINEERS WHOSE PRINCIPAL PRACTICE IS STRUCTURAL FIELD INSPECTIONS SHALL HAVE FIVE (5) YEARS OF EXPERIENCE IN PERFORMING STRUCTURAL FIELD INSPECTIONS ON THRESHOLD BUILDINGS OR EQUIVALENT PURSUANT TO A THRESHOLD / SPECIAL INSPECTION PLAN RELEVANT TO THE WORK PERFORMED AND POSSESS EACH OF THE CERTIFICATIONS IDENTIFIED IN PARAGRAPH 61615-35.004(2)(F), F.A.C., AT THE TIME OF APPLICATION.									5. AFTER CONCRETE PLACEMENT, CHECK THAT PROVISION FOR SAWCUT AND CONSTRUCTION JOINTS HAVE BEEN MET. NOTE: SAW CUTTING OF SLABS MUST BE PERFORMED WITHIN SAME DAY AS PLACEMENT.								
3.	THE THRESHOLD INSPECTOR'S PRINCIPAL RESPONSIBILITY IS TO ASSURE THAT THE STRUCTURAL COMPONENTS ARE CONSTRUCTED IN ACCORDANCE WITH THE APPROVED PLANS AND SPECIFICATIONS. THE SPECIAL INSPECTOR OR HIS/HER DULY AUTHORIZED REPRESENTATIVE SHALL PERFORM INSPECTIONS ACCORDING TO THE SPECIAL INSPECTION PLAN.									COLUMNS								
4.	NOTIFY THE CONTRACTOR OF ANY VARIATION FROM CONTRACT DOCUMENTS, DEVIATIONS, WHICH ARE NOT CORRECTED, OR OTHERWISE ADDRESSED, SHALL BE REPORTED TO THE CONTRACTOR IN WRITING AND TO THE OWNER, ARCHITECT AND BUILDING OFFICIAL HAVING JURISDICTION.									1. DONNELS FOR COLUMNS SHOULD BE CHECKED FOR ADEQUATE EMBEDMENT AND PROJECTION.								
5.	KEEP A DAILY RECORD IDENTIFYING TYPE AND LOCATION OF WORK BEING DONE, INCLUDING WEATHER, TEMPERATURE, TIME OF DAY AND OTHER INFORMATION AS REQUIRED BY THE BUILDING OFFICIAL. A WEEKLY PROGRESS REPORT OF DAILY INSPECTIONS SHALL BE SUBMITTED TO THE OWNER, ARCHITECT AND BUILDING OFFICIAL AND SHALL BE SIGNED, SEALED, AND DATED BY THE THRESHOLD INSPECTOR.									2. REINFORCING FOR COLUMNS SHOULD BE CHECKED FOR PROPER SIZES, ASSEMBLY AND ORIENTATION OF ANY OFFSETS OR BENDS. SPECIAL INSPECTOR SHALL CHECK FOR THE GRADE OF REINFORCING STEEL.								
6.	THE SPECIAL INSPECTOR, UPON COMPLETION OF THE BUILDING AND PRIOR TO THE ISSUANCE OF A DETAIL CERTIFICATE, SHALL FILE A SIGNED AND SEALED STATEMENT WITH THE ENFORCEMENT AGENCY IN SUBSTANTIALLY THE FOLLOWING FORM: TO THE BEST OF MY KNOWLEDGE AND BELIEF, THE CONSTRUCTION OF ALL STRUCTURAL LOAD-BEARING COMPONENTS DESCRIBED IN THE THRESHOLD INSPECTION PLAN COMPLIES WITH THE PERMITTED DOCUMENTS AND THE SPECIALTY SHORING DESIGN PROFESSIONAL ENGINEER HAS ASCERTAINED THAT THE SHORING AND RESHORING CONFORMS WITH THE SHORING AND RESHORING PLANS SUBMITTED TO THE DISTRICT OFFICE, PURSUANT TO THE FLORIDA STATUTES, 559.79, (7) (a), "THE SPECIAL INSPECTOR SHALL DETERMINE THAT A PROFESSIONAL ENGINEER WHO SPECIALIZES IN SHORING DESIGN HAS INSPECTED THE SHORING AND RE-SHORING FOR CONFORMANCE WITH THE SHORING AND RE-SHORING PLANS SUBMITTED TO THE ENFORCING AGENCY."									3. REINFORCING CAGES SHOULD BE CHECKED AFTER ERECTION FOR PROPER ANCHORAGE TO THE DONNELS AND FOR PROPER SIZE AND QUANTITY. DOUBLE-CHECK LAPS FOR DONNELS AND FOR SIZE AND QUANTITY.								
7.	THE SPECIAL INSPECTOR SHALL READ THE THRESHOLD INSPECTION PLAN IN ADVANCE AND STATE TO THE OWNER, IN WRITING, THAT HE INTENDS TO COMPLY.									4. AFTER FORM PLACEMENT, THE REINFORCING CAGE SHOULD BE CHECKED FOR PROPER CLEARANCE AND ADEQUATE BRACING TO KEEP IT IN PLACE DURING POURING. COLUMN FORMS SHOULD BE BRACED ADEQUATELY.								
8.	THE SPECIAL INSPECTOR SHALL VERIFY THAT ALL SUBMITTALS THAT ARE REQUIRED TO BE SIGNED AND SEALED SHALL MEET THE ELECTRONIC DOCUMENTS REQUIREMENT OF THE FLORIDA BOARD OF PROFESSIONAL ENGINEERS.									5. THE SPECIAL INSPECTOR SHALL CHECK SIZES (WIDTH AND DEPTH) OF REINFORCED CONCRETE COLUMNS.								
B.	SOIL COMPACTION:																	
1.	CONFIRM PROOF-ROLLING HAS BEEN PERFORMED.																	
C.	MUCK (IF PRESENT AT THE SITE):																	
1.	CONFIRM THAT ALL MUCK HAS BEEN REMOVED.																	
D.	CONCRETE:																	
	GENERAL																	
1.	A QUALIFIED REPRESENTATIVE OF THE TESTING LABORATORY SHOULD CHECK ALL CONCRETE FOR PROPER SLUMP AND PROPER PREPARATION OF TEST CYLINDERS. NO WATER SHALL BE ADDED AFTER TEST CYLINDERS ARE MADE. COPIES OF THE TEST REPORTS SHOULD BE PROMPTLY FURNISHED TO THE CONTRACTOR, ARCHITECT, ENGINEER-OF-RECORD AND BUILDING OFFICIAL.																	
2.	USE THE STRUCTURAL DRAWINGS IN CONJUNCTION WITH APPROVED SHOP DRAWINGS FOR CHECKING REINFORCING PLACEMENT. NOTIFY ENGINEER OF ANY DISCREPANCIES.																	
3.	FORMING, SHORING AND RE-SHORING LAYOUT SHALL BE PREPARED BY THE FORMING SUBCONTRACTOR AND HIS QUALIFIED PROFESSIONAL ENGINEER. THE APPROVED SUBMITTALS SHALL BE AVAILABLE AT THE JOB SITE TO ENSURE THAT THE WORK IS DONE CORRECTLY.																	
4.	ALL SHORING AND RE-SHORING DRAWINGS SHOULD GIVE ADEQUATE INFORMATION ON THE SIZES AND CAPACITIES OF THE SHORES, INCLUDING DIMENSIONS, SO THAT THE SHORES ON THE JOB CAN BE VERIFIED AS THE PROPER UNITS. REQUIRED BRACING SHOULD ALSO BE INDICATED.																	
5.	SHORING AND RE-SHORING SHOULD BE INSPECTED FOR PROPER SPACING, PLUMB, PHYSICAL CONDITION, PROPER BEARING AT SUPPORTS AND OF FORMING MEMBERS ON THE SHORES. BRACING WHEN REQUIRED SHOULD ALSO BE CHECKED FOR PROPER CONNECTION TO THE SHORES. DEFLECTION OF THE FORMS AND SETTLEMENT OF THE SHORES SHALL BE PERFORMED DURING THE PLACEMENT OF CONCRETE IF DEEMED NECESSARY BY THE THRESHOLD ENGINEER.																	
6.	THE CONCRETE COMPRESSIVE CYLINDER TESTS SHALL BE CHECKED AND ANY FAILURES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD.																	
7.	OBSERVE PLACEMENT OF ALL CONCRETE. VERIFY SLUMPS AND DETERMINE IF ANY WATER IS ADDED TO CONCRETE AT JOB SITE AND WHO AUTHORIZED IT. RECORD METHODS USED TO CONSOLIDATE CONCRETE.																	
	FOUNDATIONS																	
1.	OBTAIN A COPY OF THE GEOTECHNICAL ENGINEER'S REPORT, ALONG WITH ANY ADDENDUMS, AND CONFIRM THAT THE GEOTECHNICAL ENGINEER IS SATISFIED WITH THE BUILDING PAD PREPARATION.																	
1A.	THE SPECIAL INSPECTOR SHALL RECEIVE FROM THE GEOTECHNICAL ENGINEER A FINAL SUMMARIZING STATEMENT STATING THAT ALL SOIL CONSOLIDATION AND/OR DEEP FOUNDATION WORK CONFORMS TO THE CONTRACTUAL DOCUMENTS AND THE GEOTECHNICAL ENGINEERING REPORT.																	
2.	A QUALIFIED TESTING LABORATORY WILL BE EMPLOYED TO TAKE DENSITY TESTS OF SOIL BENEATH FOOTINGS AND SLABS PRIOR TO PLACING REINFORCING IN FOOTINGS. VERIFY TESTS ARE COMPLETED BY SOILS TESTING LABORATORY AND THAT SATISFACTORY COMPACTION IS ACHIEVED PRIOR TO COMMENCEMENT OF CONSTRUCTION ON SUBGRADE.																	
3.	ALL EXCAVATIONS, BACKFILLING, AND COMPACTION PROCEDURES SHALL BE CARRIED OUT IN ACCORDANCE WITH THE GEOTECHNICAL ENGINEER'S RECOMMENDATIONS.																	
4.	VERIFY FOOTING DIMENSIONS.																	
5.	VERIFY TOP OF FOOTING ELEVATIONS.																	
6.	REINFORCING STEEL IN FOUNDATIONS SHOULD BE CHECKED FOR PROPER SIZE, GRADE, QUANTITY AND REQUIRED CLEARANCE FROM SOIL AND FORMS.																	
6A.	REINFORCING STEEL IN FOOTINGS SHALL BE CHECKED FOR CORRECT GRADE OF STEEL (GRADE 60).																	
7.	DONNELS FOR WALLS AND COLUMNS SHOULD BE FIRMLY SUPPORTED AND ACCURATELY LOCATED. PLACING DONNELS AS CONCRETE IS POURED SHALL NOT BE PERMITTED. THE INSPECTOR SHOULD REQUEST THE CONTRACTOR TO SPOT CHECK DONNEL POSITIONS IN HIS PRESENCE. THE INSPECTOR SHALL VERIFY THE UPWARD EXTENSION OF ALL DONNELS FOR PURPOSE OF SPLICES.																	
8.	REINFORCING STEEL IN FOOTINGS SHALL BE CHECKED FOR CORRECT GRADE OF STEEL (GRADE 60).																	
	SUPPORTED SLABS/ BEAMS/ JOISTS																	
1.	REINFORCING IN SLABS SHOULD BE CHECKED FOR QUANTITY, SIZE, SPACING, PROPER HEIGHTS AND PROPER BAR SUPPORTS. REINFORCING MATS SHOULD BE TIED AND SUPPORTED TO ENSURE IT WILL STAY IN POSITION UNDER TRAFFIC AND POURING. SPECIAL INSPECTOR SHALL CHECK FOR THE GRADE OF REINFORCING STEEL.																	
2.	CONCRETE BEAMS/JOISTS SHOULD BE CHECKED FOR SIZE, QUANTITY AND PROPER PLACEMENT OF REINFORCING. PROPER CLEARANCES BETWEEN STEEL AND FORMS SHOULD BE MAINTAINED. SPECIAL INSPECTOR SHALL CHECK FOR THE GRADE OF REINFORCING STEEL.																	
3.	STIRRUPS AND TIES SHOULD BE CHECKED FOR SIZE, QUANTITY, PROPER BENDS AND PROPER PLACEMENT. STIRRUP SPACING WITH RESPECT TO SUPPORT LOCATIONS IS EXTREMELY IMPORTANT.																	
4.	THE SPECIAL INSPECTOR SHALL CHECK SIZES (WIDTH AND DEPTH) OF REINFORCED CONCRETE SLABS, BEAMS, JOISTS AND GRADE OF REINFORCING STEEL.																	
	TILT-UP PANELS																	
1.	OVERALL DIMENSIONS OF FORMS FOR PANEL CONFIGURATIONS SHOULD BE CHECKED FOR GENERAL CONFORMITY. PAY PARTICULAR ATTENTION TO THE PANEL THICKNESS(ES).																	
2.	REINFORCING STEEL SHOULD BE CHECKED FOR PROPER GRADE, SIZE, QUANTITY, SPACING, LOCATION AND CLEARANCE FROM FORMS. ENSURE THAT REINFORCING DESIGNATED "EACH FACE" IN THE DRAWINGS ARE NOT SIMPLY TIED TO THE CENTER MAT. THEY SHOULD BE CHAIRED SEPARATELY.																	
3.	ADDITIONAL REINFORCING FOR OPENINGS SUCH AS DOORS, WINDOWS AND MECHANICAL CHASES SHOULD BE CHECKED FOR PROPER SIZE, QUANTITY AND LOCATION.																	
4.	CORNER BARS SHOULD BE CHECKED FOR PROPER SIZE, QUANTITY AND LENGTH. CORNER BARS ARE TO BE PLACED IN THE CENTER OF PANEL WITHIN THE VERTICAL/HORIZONTAL CAGES. DO NOT PLACE DIAGONAL CORNER BARS ON THE OUTSIDE OF A GAGE OF STEEL.																	
5.	EMBEDS SHOULD BE CHECKED FOR SIZE, SPACING AND LOCATIONS AGAINST APPROVED STRUCTURAL STEEL AND JOIST SHOP DRAWINGS AND SHOULD BE SECURED IN PLACE PRIOR TO PLACEMENT OF CONCRETE.																	
6.	ADDITIONAL REINFORCING FOR LIFTING AND LIFTING INSERTS SHOULD BE CHECKED FOR PROPER SIZE, QUANTITY, SPACING AND SHOULD BE SECURED IN PLACE PRIOR TO PLACEMENT OF CONCRETE.																	
7.	REINFORCING CAGES FOR GIRDERS SHOULD BE CHECKED FOR PROPER SIZE, QUANTITY, SPACING OF BOTH VERTICALS AND STIRRUPS, AND CLEARANCE FROM FORMS.																	
8.	ALL REINFORCING SHOULD BE CHECKED FOR PROPER CHAIRING OR SUPPORTS. PROPER CHAIRING SHOULD RESULT IN REINFORCING BEING SUPPORTED ENOUGH THAT THE REINFORCING CAN NOT BE DEFLECTED BY THE WEIGHT OF CONCRETE, PERSONNEL, OR ANY OTHER LOADS IMPOSED DURING PLACEMENT PROCEDURES.																	
	E. MASONRY:																	
	GENERAL																	
1.	VERIFY TYPE OF MASONRY BLOCKS PROVIDED IS AS SPECIFIED.																	
2.	VERIFY COMPLIANCE OF MORTAR MIX WITH CONTRACT DOCUMENTS.																	
3.	VERIFY CONCRETE TIE BEAM AND/OR MASONRY BOND BEAM SIZE, LOCATION, VERTICAL SPACING, REINFORCING STEEL, CORNER BARS AND END CONDITION.																	
4.	CHECK TIE COLUMNS FOR SPACING, SIZE, AND SPECIFIED REINFORCING STEEL.																	
5.	VERIFY THAT CONCRETE FOR TIE COLUMNS AND TIE BEAMS IS TO BE PLACED AFTER BLOCK MASONRY IS IN PLACE. OTHERWISE, VERIFY THAT MECHANICAL CONNECTORS SUCH AS DOVETAILS ARE PROVIDED.																	
6.	CHECK FOR HORIZONTAL REINFORCING, GROUTED CELLS, DONNELS AND INSERTS. CHECK FOR ANY CRACKED BLOCKS.																	
7.	VERIFY LOCATION OF CONTROL JOINTS AND PROPER INSTALLATION OF JOINT ACCESSORIES.																	
8.	VERIFY CELLS ARE FILLED WITH INSULATION, SAND, GROUT OR OTHER MATERIALS AS SPECIFIED.																	
	REINFORCED MASONRY																	
1.	VERIFY REINFORCING STEEL FOR GRADE, SIZE AND FLAKING RUST.																	
2.	CHECK THAT INSPECTION HOLES ARE CUT AT THE BOTTOM OF FILLED CELLS IF REQUIRED BY THE GROUTING DETAIL IN THE DRAWINGS.																	
3.	VERIFY SPLICE LENGTH OF THE DONNELS.																	
4.	VERIFY THAT VERTICAL STEEL IS PLACED IN THE SAME CELL AS DONNELS.																	
5.	CHECK GROUT FOR SPECIFIED STRENGTH, SLUMP AND AGGREGATE SIZE.																	
6.	VERIFY TEST PRISMS FOR GROUT ARE MADE.																	
	7. VERIFY THE GROUT IS PLACED IN STAGES AS PER GROUTING DETAIL IN THE DRAWINGS.																	
	8. VERIFY BLOCKS ARE DRY BEFORE PLACING GROUT.																	
	9. ENSURE END CELLS, CELLS ADJACENT TO OPENINGS, CORNERS AND AS OTHERWISE INDICATED ON THE DRAWINGS HAVE VERTICAL REINFORCING AND ARE FULLY GROUTED.																	
	10. ENSURE SPACING OF GROUTED CELLS IS CORRECT.																	
	11. VERIFY EMBEDMENT OF VERTICAL STEEL INTO TIE BEAMS, BOND BEAMS OR SLABS ABOVE.																	
	12. CHECK FOR VOIDS AT GROUTED CELLS.																	
	13. INSPECT FOR CRACKS IN THE WALL AFTER A FEW DAYS.																	
	14. DO NOT ALLOW TOOTHING OF WALL(S) AT END OF DAY'S WORK. WALL(S) SHALL BE STEPPED BACK AT END OF DAY.																	
	F. STRUCTURAL STEEL:																	
	GENERAL																	
1.	REVIEW THE CONSTRUCTION PROCEDURE IN PRE-CONSTRUCTION MEETING AND MAKE SURE THAT IT ACCOMMODATES THE DESIGN ASSUMPTIONS.																	
2.	REVIEW THE CONSTRUCTION DOCUMENTS AND MAKE SURE ALL THE DRAWINGS, SPECIFICATIONS, SHOP DRAWINGS, ADDENDUMS, SKETCHES ARE AVAILABLE AND APPROVED BY DESIGN ENGINEERS.																	
3.	VERIFY PREVIOUS INSPECTIONS FOR CORRECTIONS INDICATED ON THE CORRECTION CHECKLIST.																	
4.	VERIFY ANCHOR BOLTS FOR SIZE, LENGTH, PLUMB, EMBEDMENT AND PROTRUSION OF THREADED END FOR NUT ENGAGEMENT.																	
5.	VERIFY THAT STEEL ON THE JOBSITE IS AS SHOWN ON THE PLANS AND SPECIFICATIONS INCLUDING SIZES AND SHAPES OF ALL MEMBERS.																	
6.	VERIFY MILL CERTIFICATIONS AND TEST REPORTS, IF REQUIRED.																	
7.	VERIFY GRADE OF STEEL MEMBERS, PIPE, TUBING AND BOLTS FOR CONFORMANCE WITH SPECIFICATIONS.																	
8.	INSPECT STEEL MEMBERS FOR DISTORTION, EXCESSIVE RUST, FLAKS AND BURNED HOLES.																	
9.	CHECK GAMBER IN BEAMS PRIOR TO THEM BEING ERECTED.																	
10.	INSPECT FOR SURFACE FINISH GALVANIZED OR SHOP PAINT COAT.																	
11.	CHECK THE COLUMNS FOR BEARING SURFACES, ALIGNMENT, SIZE, ADEQUATE BASE PLATES, SPLICE PLATES, BEARING AND EMBEDMENT IN MASONRY OR CONCRETE.																	
12.	CHECK SPLICING FOR CONFORMANCE TO PLANS.																	
13.	ENSURE THAT ENDS OF BEAMS BEARING ON MASONRY OR CONCRETE CONFORM TO THE DETAILS ON THE PLAN. CONNECTIONS TO EMBED PLATES ARE GENERALLY DETAILED TO ALLOW FOR HORIZONTAL SLIP. REVIEW CONSTRUCTION DOCUMENTS FOR SPECIFIC REQUIREMENTS.																	
14.	VERIFY FIRE PROTECTION REQUIREMENTS ARE IN ACCORDANCE WITH CONTRACT DOCUMENTS.																	
15.	CHECK APPROVED SHOP DRAWINGS AGAINST ACTUAL FIELD CONDITIONS.																	
	WELDED CONNECTIONS																	
1.	VERIFY CERTIFICATION OF WELDERS, THEIR NAMES AND CERTIFICATE NUMBERS.																	
2.	VERIFY TYPE OF ELECTRODES USED.																	
3.	VERIFY TESTING HAS BEEN COMPLETED ACCORDING TO CONTRACT DOCUMENTS.																	
	BOLTED CONNECTIONS																	
1.	CHECK BOLTS FOR PROPER SIZE, LENGTH, WASHERS, TYPE & GRADE OF BOLTS AND NUTS.																	
2.	NOTE ANY OMISSION OF REQUIRED BOLTS.																	
3.	CHECK THAT THE JOISTS NEAREST THE COLUMNS ARE FIELD BOLTED TO PROVIDE LATERAL STABILITY DURING ERECTION.																	
4.	VERIFY THAT TESTING HAS BEEN COMPLETED ACCORDING TO CONTRACT DOCUMENTS.																	
	K. GLAZING:																	
1.	REVIEW THE CONSTRUCTION DOCUMENTS AND MAKE SURE ALL THE DRAWINGS, SPECIFICATIONS, ADDENDUMS AND SKETCHES ARE AVAILABLE AND UP TO DATE																	
2.	VERIFY THAT SIGNED AND SEALED ENGINEERING SUBMITTALS FOR ALL SPECIALTY-ENGINEERED ITEMS HAVE BEEN REVIEWED BY THE ARCHITECT, BDM AND THE GENERAL CONTRACTOR/CM AND THAT THE "FOR CONSTRUCTION" SET IS BEING USED IN THE FIELD.																	
3.	INSPECT GLAZING MEMBERS FOR PROPER PROFILE, MATERIAL, LOCATION OF SPLICES, REINFORCEMENT AND CONNECTIONS TO SUPPORTING MEMBERS.																	
4.	VERIFY PROPER FASTENER SIZE, QUANTITY AND LOCATIONS.																	
5.	VERIFY THAT BRACING CONFORMS TO CONTRACT DOCUMENTS AND SHOP DRAWINGS. BRING ANY DISCREPANCIES TO THE ARCHITECT AND BDM'S ATTENTION.																	
	WELDED CONNECTIONS																	
1.	VERIFY CERTIFICATION OF WELDERS, THEIR NAMES AND CERTIFICATE NUMBERS.																	
2.	VERIFY TYPE OF ELECTRODES USED.																	
3.	VERIFY TESTING HAS BEEN COMPLETED ACCORDING TO CONTRACT DOCUMENTS.																	
	BOLTED CONNECTIONS																	
1.	INSPECT BOLT HOLES TO VERIFY THE DIAMETER OF HOLE, PROPER HOLE ALIGNMENT, AND QUANTITY, LOCATION, SPACING, EDGE AND END DISTANCES																	
2.	CHECK BOLTS FOR PROPER SIZE, LENGTH, WASHERS, TYPE & GRADE OF BOLTS AND NUTS.																	
3.	NOTE ANY OMISSION OF REQUIRED BOLTS.																	
4.	VERIFY THAT METHODS USED IN APPLYING THE REQUIRED MINIMUM TORQUE, CALIBRATION OF THE TORQUE WRENCHES, OR LOAD INDICATORS CONFORM TO CONTRACT DOCUMENTS.																	
5.	VERIFY THAT FINISHES ON CONTACT SURFACES BETWEEN CONNECTION PLATES ARE IN ACCORDANCE WITH SPECIFICATIONS.																	
6.	CHECK FOR LOCK NUTS OR UPSET THREADS, IF SPECIFIED.																	
7.	CHECK FOR HARDENED WASHERS WHEN REQUIRED BY SPECIFICATIONS AND BEVELED WASHERS WHERE THE SURFACE OF NUT AND BOLT HEAD CONTACTS AND NOT SUFFICIENTLY PARALLEL.																	
8.	VERIFY GROUTING OF BASE PLATE.																	
9.	VERIFY THAT TESTING HAS BEEN COMPLETED ACCORDING TO CONTRACT DOCUMENTS.																	
	STEEL PAN STAIRS																	
1.	CHECK THAT STAIRS HAVE BEEN FABRICATED IN CONFORMANCE WITH THE REVIEWED STRUCTURAL STEEL SUBMITTALS.																	
2.	CHECK STAIR CONNECTIONS TO SUPPORTING MEMBERS.																	
3.	ENSURE THAT MID-LANDINGS ARE PROPERLY CONNECTED. MASONRY MUST BE GROUTED SOLID MINIMUM 4" ALL-AROUND BOLTS THAT SECURE THE MID-LANDINGS TO MASONRY WALLS.																	
	G. METAL DECK:																	
1.	VERIFY METAL DECK TYPE, SIZE AND SURFACE FINISH.																	
2.	VERIFY METAL DECK WELDING ELECTRODES.																	
3.	VERIFY USE OF WELD WASHERS IF REQUIRED.																	
4.	VERIFY THAT REQUIRED WELDS ARE PROPERLY INSTALLED.																	
5.	VERIFY THAT REQUIRED SIDELAP STITCH SCREENS ARE PROPERLY INSTALLED.																	
6.	CHECK FOR ANY DAMAGE DURING TRANSPORTATION OR ERECTION.																	
	H. COLD-FORMED (LIGHT GAUGE) METAL FRAMING:																	
1.	REVIEW THE CONSTRUCTION DOCUMENTS AND MAKE SURE ALL THE DRAWINGS, SPECIFICATIONS, ADDENDUMS AND SKETCHES ARE AVAILABLE AND UP TO DATE.																	
2.	VERIFY THAT SIGNED AND SEALED ENGINEERING SUBMITTALS FOR ALL SPECIALTY-ENGINEERED ITEMS HAVE BEEN REVIEWED BY THE ARCHITECT, BDM AND THE GENERAL CONTRACTOR/CM AND THAT THE "FOR CONSTRUCTION" SET IS BEING USED IN THE FIELD.																	
3.	INSPECT LIGHT-GAUGE METAL STRUCTURAL MEMBERS FOR PROPER PROFILE AND MATERIAL GAUGE, LOCATION OF SPLICES, REINFORCEMENT WHEN STUDS AND PLATES ARE CUT AND ADEQUATE BEARING ON AND CONNECTION TO SUPPORTING MEMBERS.																	
4.	VERIFY PROPER FASTENER SIZE, QUANTITY AND LOCATIONS.																	
5.	VERIFY THAT BRACING OF STUDS AND JOISTS CONFORMS TO CONTRACT DOCUMENTS AND SHOP DRAWINGS. BRING ANY DISCREPANCIES TO THE ARCHITECT AND BDM'S ATTENTION.																	

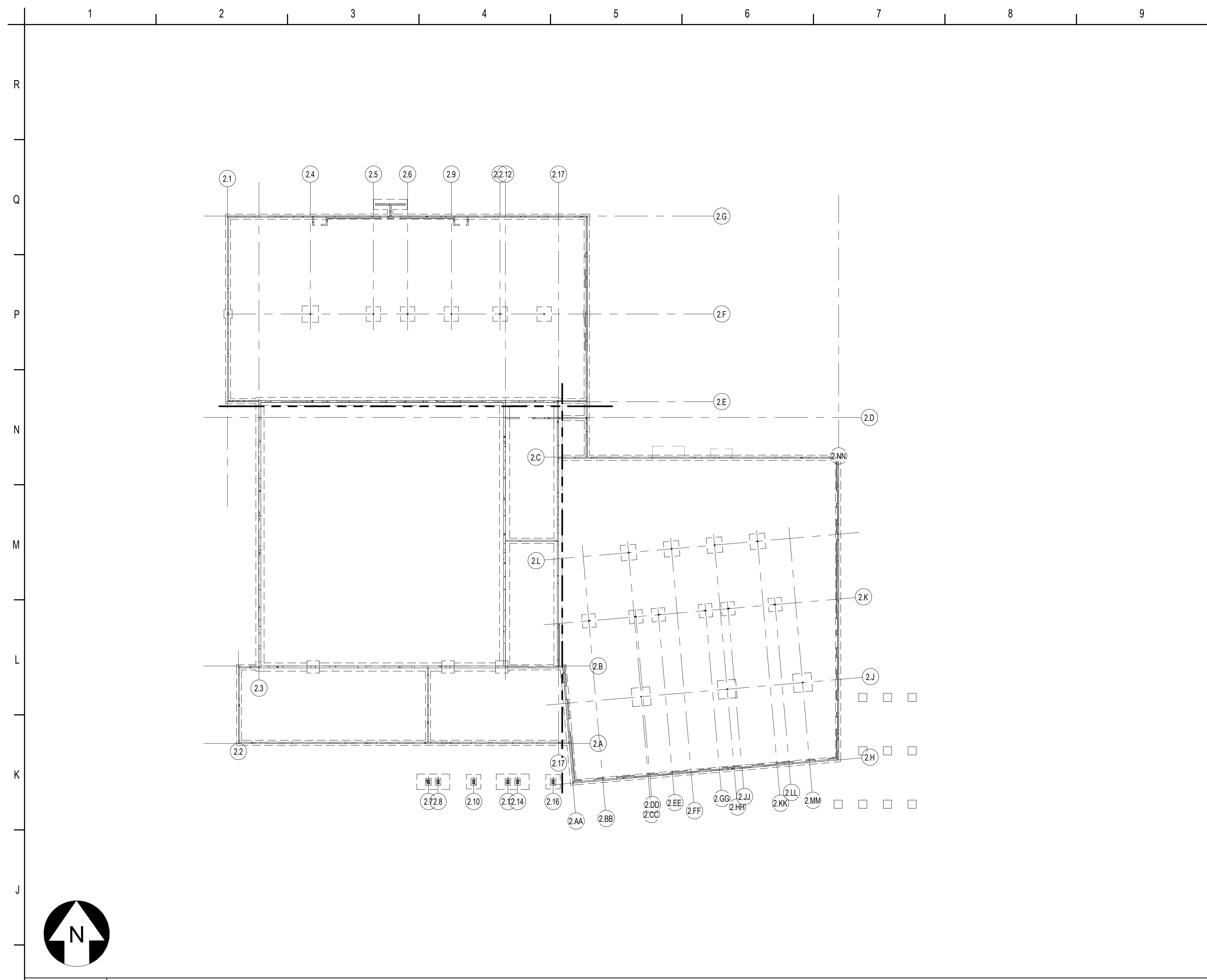
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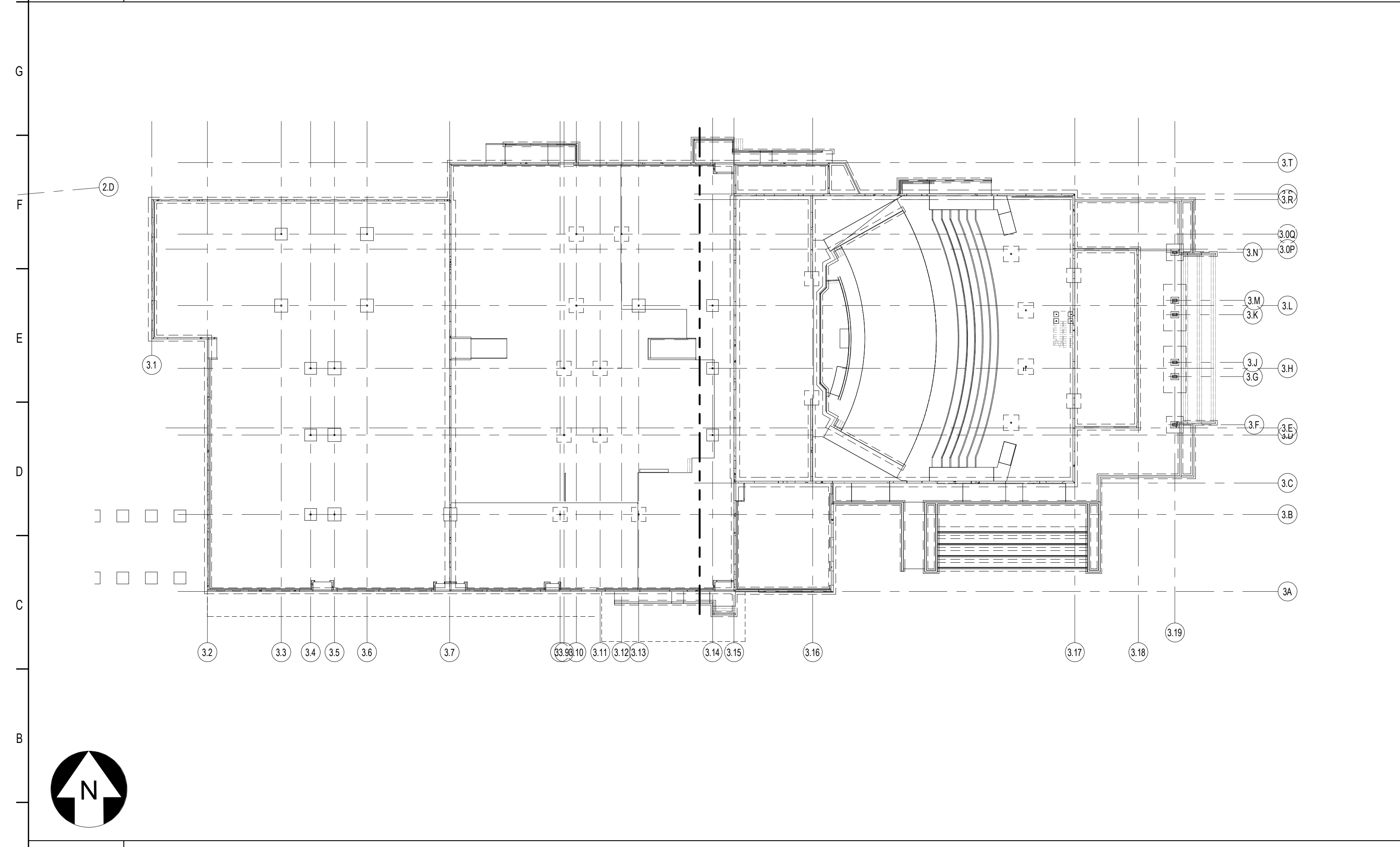


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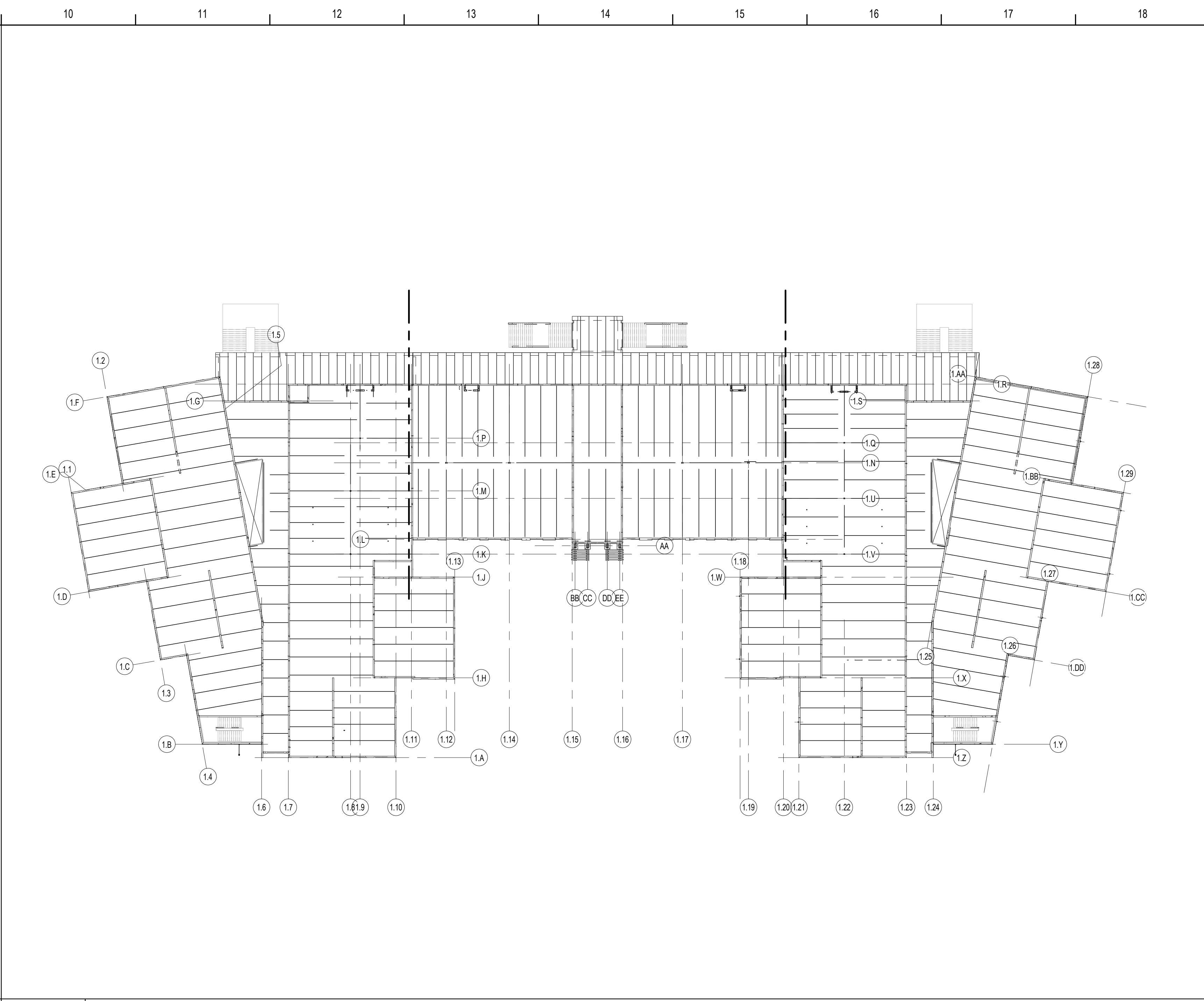
H1 BLDG 2 - OVERALL PLAN - FOUNDATION

1/32" = 1'-0"



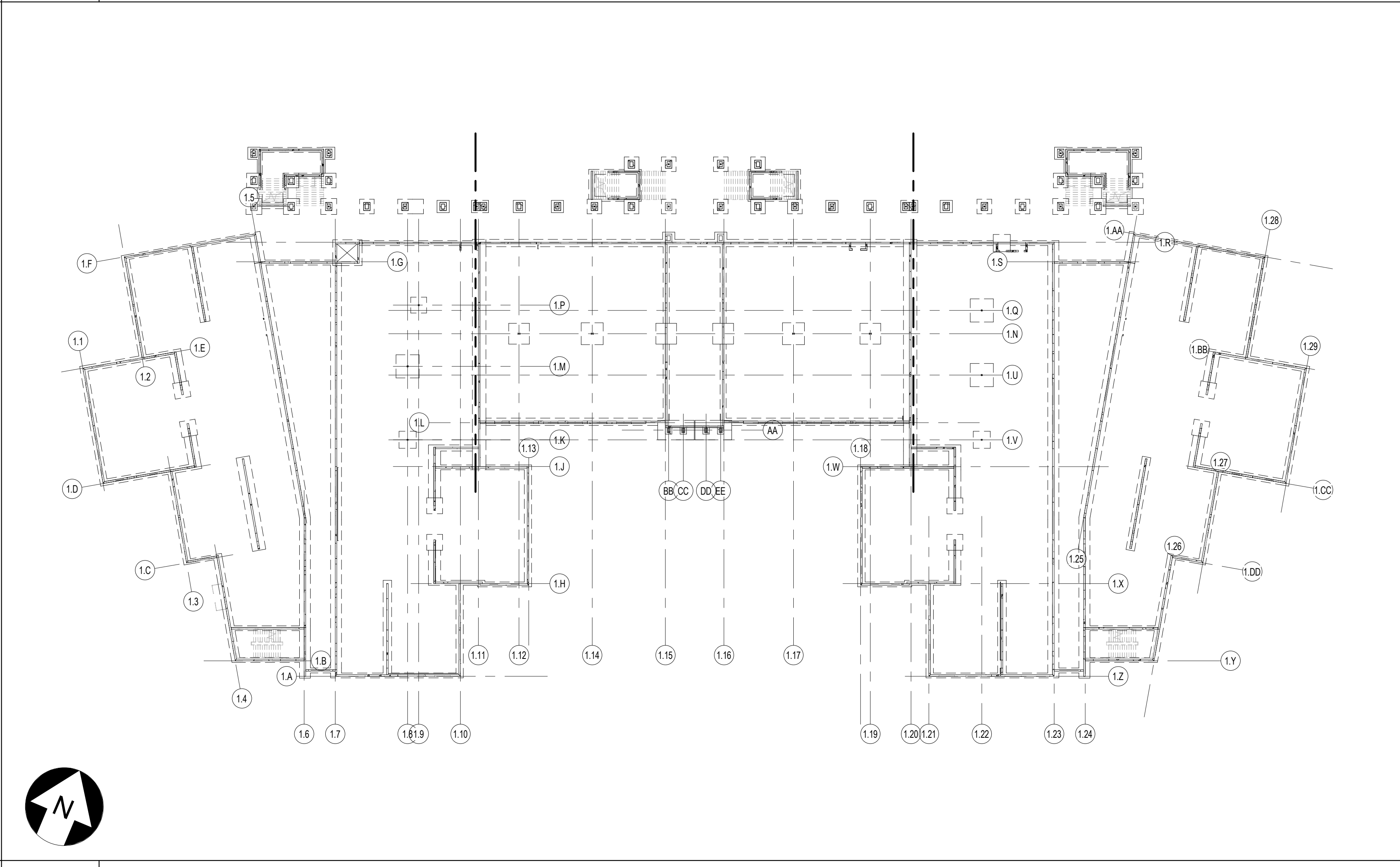
A1 BLDG 3 - OVERALL PLAN - FOUNDATION

1/32" = 1'-0"



H10 BLDG 1 - OVERALL PLAN - SECOND FLOOR FRAMING

1/32" = 1'-0"



A10 BLDG 1 - OVERALL PLAN - FOUNDATION

1/32" = 1'-0"

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LUIS F. BEDOYA, P.E.
Florida Professional Engineer No. 65509

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ST. LUCIE HIGH SCHOOL DDD

Port St. Lucie, FL

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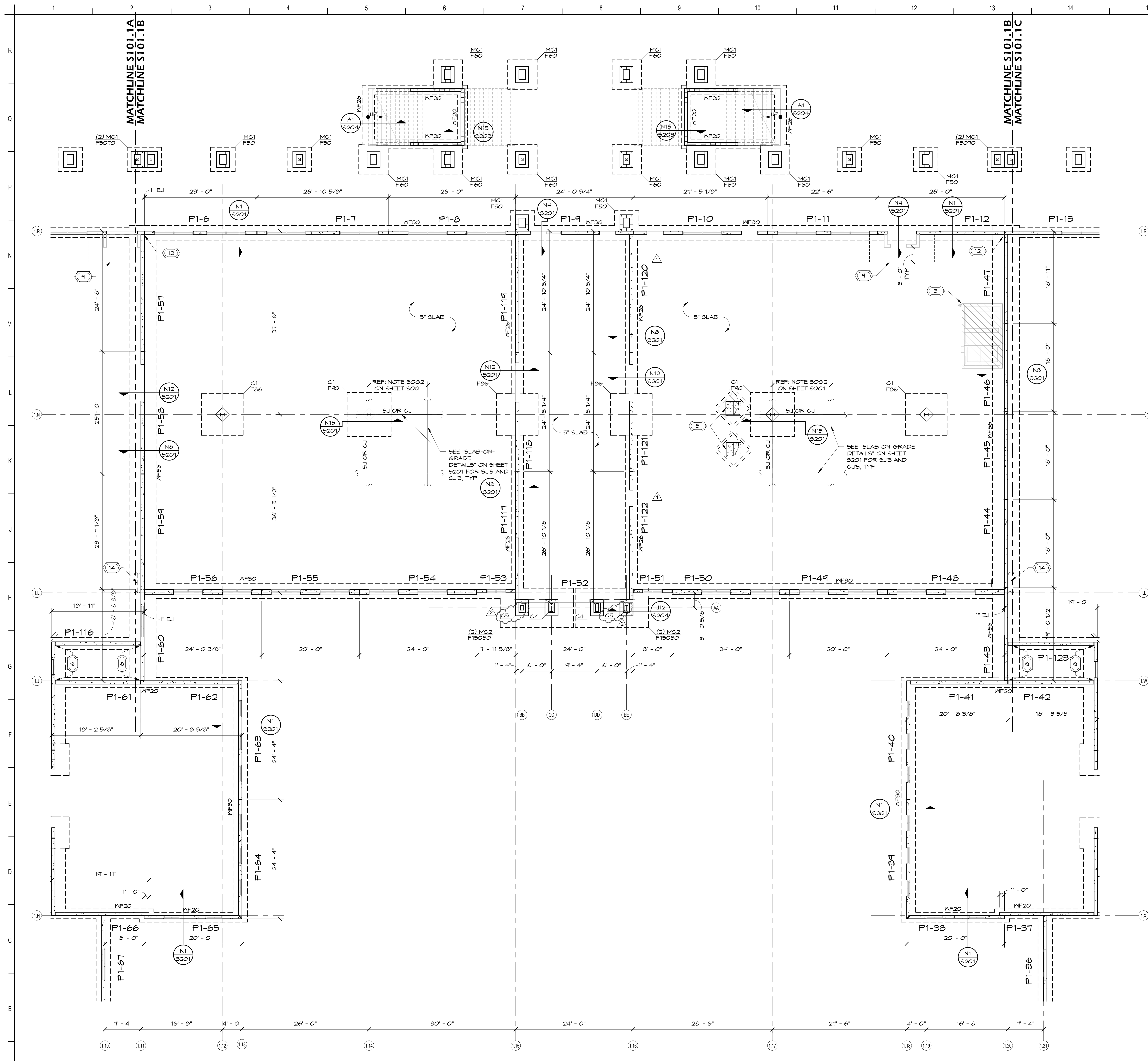
St. Lucie Public Schools
9451 Brandywine Ln
Port St. Lucie, FL 34986

ISSUE DATE: 12/16/2022
COMM. NO.: 2022107

DRAWN BY: NDM CHECKED BY: LB / JMP

BUILDINGS 1, 2 & 3 - OVERALL PLAN

S050



- FOUNDATION PLAN NOTES:**
- FLOOR SLAB SHALL BE 6" THICK CONCRETE REINF WITH 6x6x12-2 #4x2 #4 W/F UNO IN ARCHITECTURAL SPECIFICATIONS. PROVIDE 10 MIL (MIN) VAPOR RETARDER (ASTM E 1745-11) ON COMPACTED SUBGRADE. SEE "SLAB-ON-GRADE DETAILS" ON SHEET S201 FOR PLACEMENT OF REINF.
 - T/ SLAB EL = 100'-0" (TYP, UNO). REFERENCE ONLY - SEE CIVIL DWGS FOR ACTUAL ELEVATION.
 - T/ WALL FTG EL = 98'-8" (TYP, UNO).
 - T/ COL FTG EL = 98'-8" (TYP, UNO).
 - SEE S500 SERIES SHEETS FOR TILT-UP PANEL THICKNESSES. PANEL DESIGN INCLUDES UP TO A 3/4" DEEP REVEAL. ALLOWANCE, TYP.
 - ALL TILT-UP PANELS ARE VIEWED FROM THE INSIDE OF THE BUILDING.
 - STEP AND/OR LOWER FOUNDATIONS WHERE SHOWN AND AS NECESSARY TO AVOID INTERFERENCE WITH OTHER TRADES. SEE CONCRETE GENERAL NOTES FOR DETAILS AND SECTIONS. PARTICULAR ATTENTION SHALL BE PAID TO DOWNSPROUTS ENSURING THAT PROPER ACTIONS HAVE BEEN TAKEN TO PREVENT PIPES FROM CONFLICTING WITH THE FOUNDATION SYSTEM.
 - ALL FTGS ARE CENTERED BENEATH THE BEARING WALLS AND COLUMNS (TYP, UNO).
 - SEE SHEET S601 FOR PANEL REINF, EMBEDDED ITEMS AND JOINT DETAILS.
 - ALL CMU BEARING WALLS ARE 8" (TYP, UNO).
 - REINF LOAD-BEARING CMU WALLS WITH #5 VERT BAR CENTERED IN GROUT-FILLED CELL AT ENDS, CORNERS AND AT MAX SPACING OF 32" OC. PROVIDE (3) REINF CELLS AT ALL BEARING LOCATIONS WHERE A TIE COLUMN IS NOT CALLED OUT. FOR REINF OF NON-LOAD-BEARING INTERIOR CMU WALLS, SEE SECTION J1/S202. ADDITIONAL FILLED CELLS REQUIRED AT JAMBS ARE SHOWN THUS "M" ON EACH STRUCTURAL PLAN. SEE "ILLUSTRATIVE PLAN OF VARIOUS CMU WALL CONDITIONS" ON SHEET S204 FOR ADDITIONAL REQUIRED FILLED CELLS AND OTHER INFORMATION.
 - TYP SPACING OF FILLED CELLS SHALL APPLY ABOVE AND BELOW OPENINGS ALSO.
 - SEE SHEETS S001 THR S005 FOR STRUCTURAL GENERAL NOTES.
 - MAINTAIN STRUCTURAL SLAB THICKNESS AT ALL FLOOR SLOPES AND DEPRESSIONS.
 - THE CONTRACTOR SHALL COORDINATE ALL UNDERGROUND UTILITIES, PIPES, ETC. WITH THE FOUNDATION PLAN AND FOUNDATION ELEVATIONS. FOOTING PENETRATION DETAILS MAY BE FOUND IN THE CONCRETE AND REINFORCING SECTION OF THE STRUCTURAL GENERAL NOTES.

- FOUNDATION PLAN KEY NOTES:**
- (2) #4x4'-0" LONG @ 3" OC PLACED 2" CLR FROM CORNER, CENTERED IN SLAB (TYP WHERE SHOWN).
 - SHADED AREA INDICATES CONCRETE CURB, REF A6/S204.
 - HATCHED AREA INDICATES CONCRETE HOUSE-KEEPING PAD, REF ARCH 4 MECH DWGS FOR EXACT SIZE & LOCATION. SEE A12/S202 FOR ADDITIONAL INFORMATION.
 - VALLEY BALL FLOOR INSERT SUPPORT, REF ARCH FOR EXACT QUANTITY & LOCATIONS. SEE E12/S201 FOR ADDITIONAL INFORMATION.
 - SHADED AREA INDICATES TURN DOWN WALL FOUNDATION ON TOP OF ELEVATOR MAT, REF A1/S201.
 - RECESS TILT-UP EMBED PLATES, FULL-HEIGHT OF PANEL. SEE A15/S201 FOR ADDITIONAL INFORMATION.
 - STAIR COLUMN & CONN TO SLAB BY STAIR MFR, REF S61/S201.
 - HATCHED AREA INDICATES SLAB DEPRESSION. REF ARCH DWGS FOR EXACT SIZE & LOCATION. SEE A15/S202 FOR ADDITIONAL INFORMATION.
 - 8" NON-LOAD-BEARING MASONRY WALL, REF J1/S202. PROVIDE 6" SLAB UNDER WALL.
 - SLOPE SLAB TO DRAIN, REF ARCH FOR EXACT LOCATION. MAINTAIN STRUCTURAL SLAB THICKNESS THROUGHOUT ENTIRE LENGTH OF SLOPE.
 - COORD PIPE IV PLUMBING, REF C4/S202.
 - FOOTING PENETRATION FOR ROOF DRAIN, COORD WITH PLUMBING DRAWINGS. REF OPTION 1A ON C4/S202.
 - #55 3x3x1/4" SUPPORT FOR ROLL-UP DOORS. SEE DETAIL E3/S401 FOR ANCHORAGE AND SUPPORT. SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS AND DETAILS.
 - COORDINATE 8" OVERFLOW ROOF DRAIN SO THAT IT DOES NOT AFFECT TILT-UP COLUMN GAGE REINFORCING.
 - 8" WIDE C/P CONC STEMWALL R/W #5 @ 12" OC EA WAY. COORD OPENINGS IN WALL IV EQUIP DWGS.

- FOUNDATION LEGEND:**
- MC - INDICATES MASONRY COLUMN. SEE DETAIL ON SHEET S203 FOR INFO.
 - C - INDICATES STEEL COLUMN. SEE SCHEDULE ON SHEET S601 FOR INFO.
 - F - INDICATES PAD FOOTINGS. SEE FOUNDATION SCHEDULE ON SHEET S601 FOR INFO.
 - W - INDICATES WALL FOOTINGS. SEE FOUNDATION SCHEDULE ON SHEET S601 FOR INFO.
 - MNF - INDICATES MONOLITHIC WALL FOOTINGS. SEE FOUNDATION SCHEDULE ON SHEET S601 FOR INFO.

NOTE TO S6:
 LOCATION OF MASONRY CONTROL JOINTS (MCJ) SHALL BE COORDINATED WITH ARCHITECTURAL DRAWINGS AND SHALL NOT EXCEED REQUIREMENTS OUTLINED IN NOTE M19 ON SHT S204. FOR ADDITIONAL FILLED CELLS REQUIRED AT MASONRY CONTROL JOINTS AND NOT SHOWN ON FOUNDATION PLANS, SEE "ILLUSTRATIVE PLAN OF VARIOUS CMU WALL CONDITIONS" ON SHT S204. SUBMIT MCJ PLAN TO ARCHITECT FOR APPROVAL.

NOTE:
 ALUMINUM CANOPY COLUMNS ARE NOT SHOWN. SEE S65 ON SHEET S001 FOR SPECIALTY ENGINEERING REQUIREMENTS. REF ARCHITECTURAL & CIVIL DRAWINGS FOR LOCATIONS.

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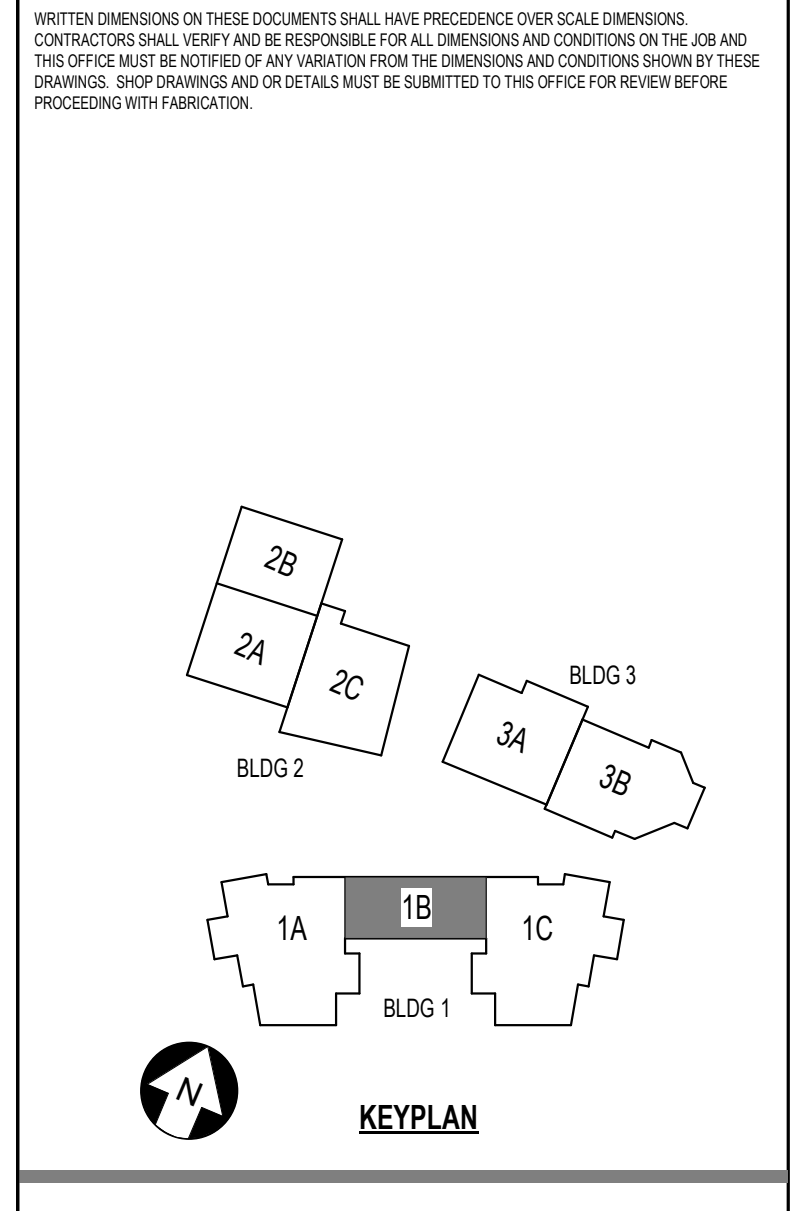
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 LICENSE NO. 65599
 STATE OF FLORIDA
 CIVIL ENGINEERING (10A)

LUIS F. BIDOYA, P.E.
 Florida Professional Engineer No. 65599

MARK	DESCRIPTION	DATE
1	ADDED MARK 12	10/06/2022
2	REV 2	10/06/2022

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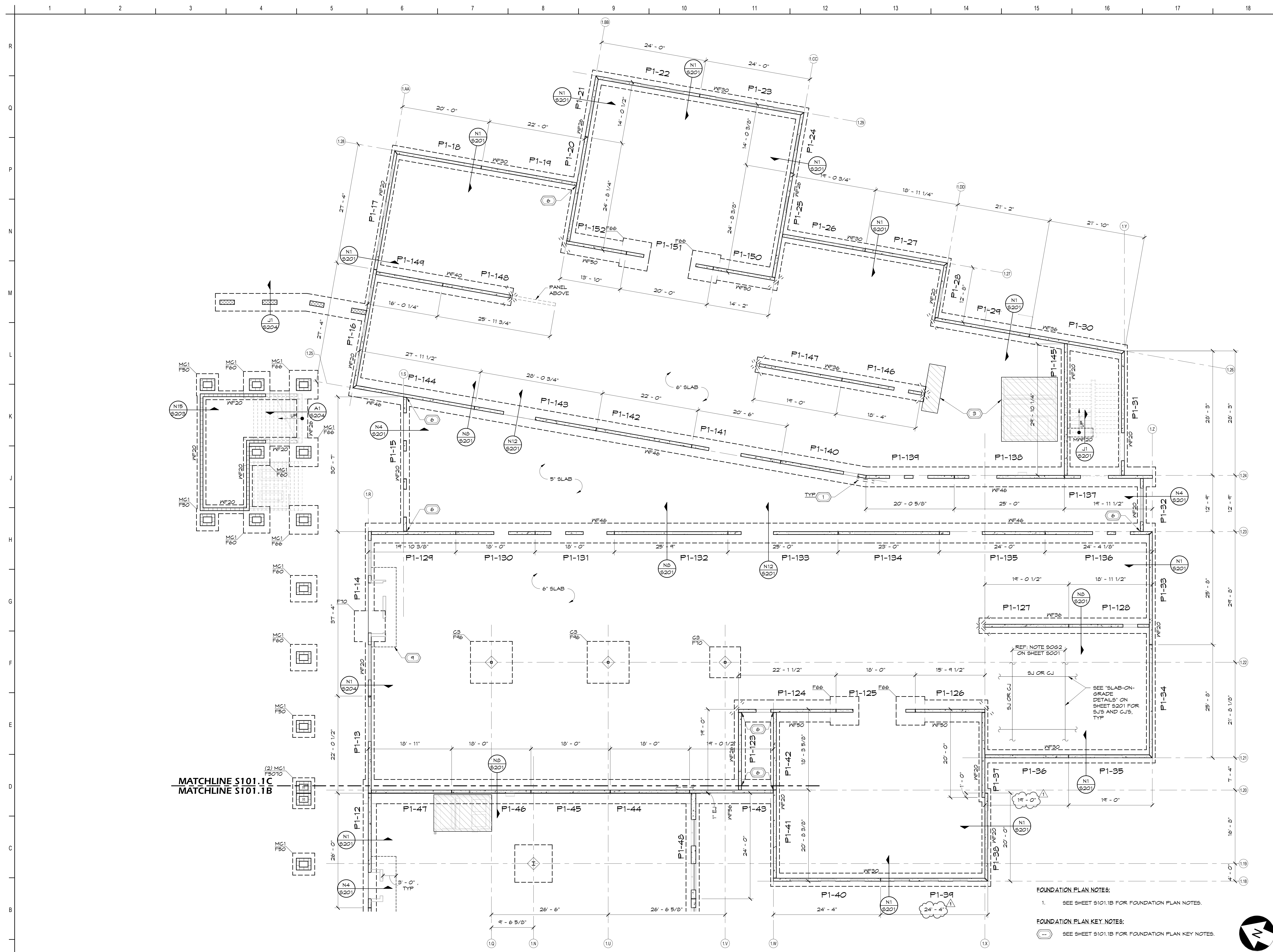
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BUILDING 1 AREA 1B - FOUNDATION PLAN

S101.1B

A1 BUILDING 1 AREA 1B - FOUNDATION PLAN

1/8" = 1'-0"



MATCHLINE S101.1C
MATCHLINE S101.1B

- FOUNDATION PLAN NOTES:**
- SEE SHEET S101.1B FOR FOUNDATION PLAN NOTES.
- FOUNDATION PLAN KEY NOTES:**
- SEE SHEET S101.1B FOR FOUNDATION PLAN KEY NOTES.

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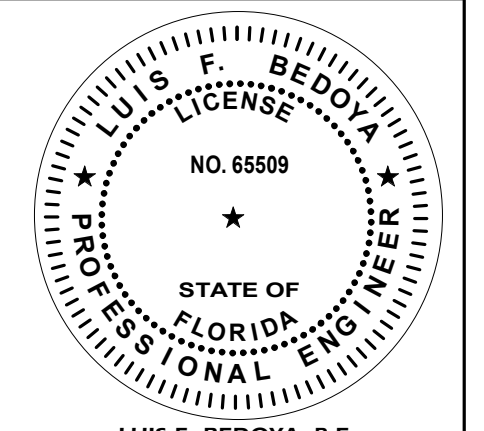
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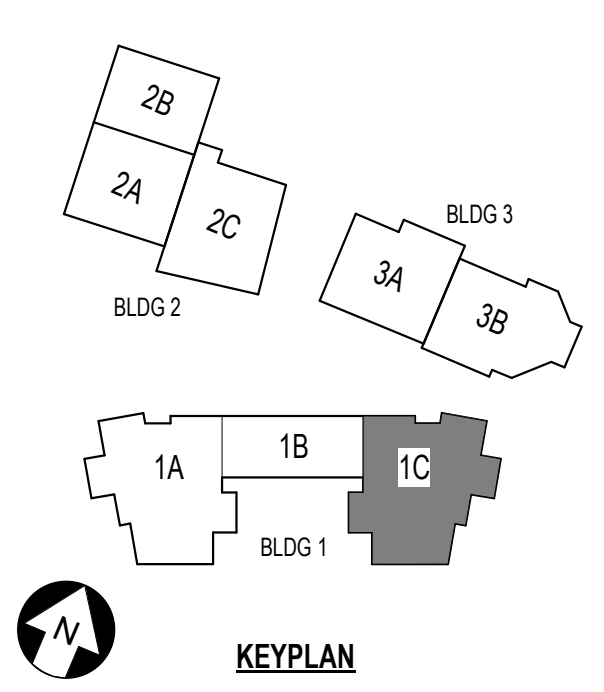
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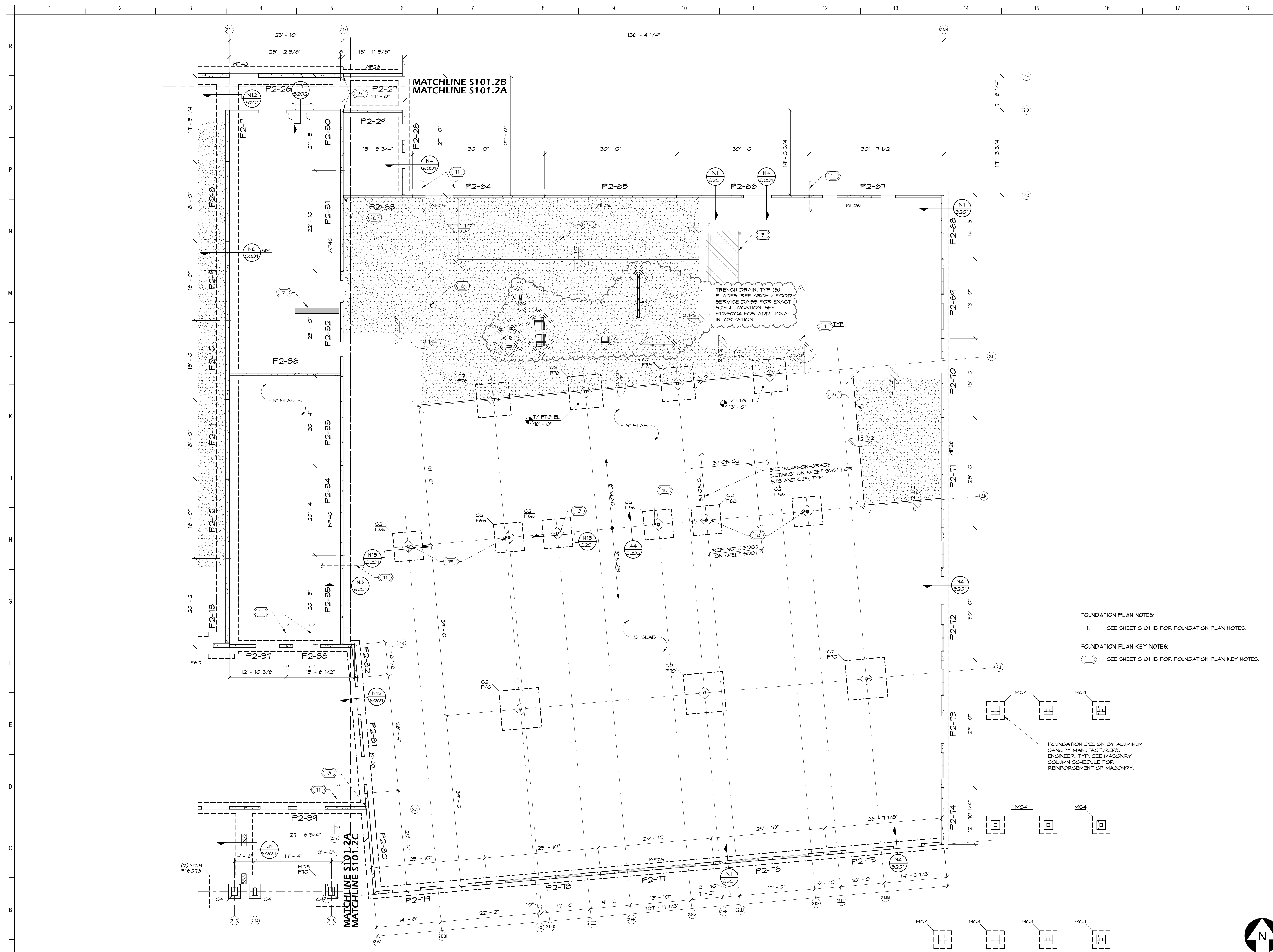
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BUILDING 1 AREA 1C - FOUNDATION PLAN

S101.1C



FOUNDATION PLAN NOTES:
 1. SEE SHEET S101.1B FOR FOUNDATION PLAN NOTES.

FOUNDATION PLAN KEY NOTES:
 (1) SEE SHEET S101.1B FOR FOUNDATION PLAN KEY NOTES.

FOUNDATION DESIGN BY ALUMINUM CANOPY MANUFACTURER'S ENGINEER, TYP. SEE MASONRY COLUMN SCHEDULE FOR REINFORCEMENT OF MASONRY.

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 Florida Professional Engineer No. 65509

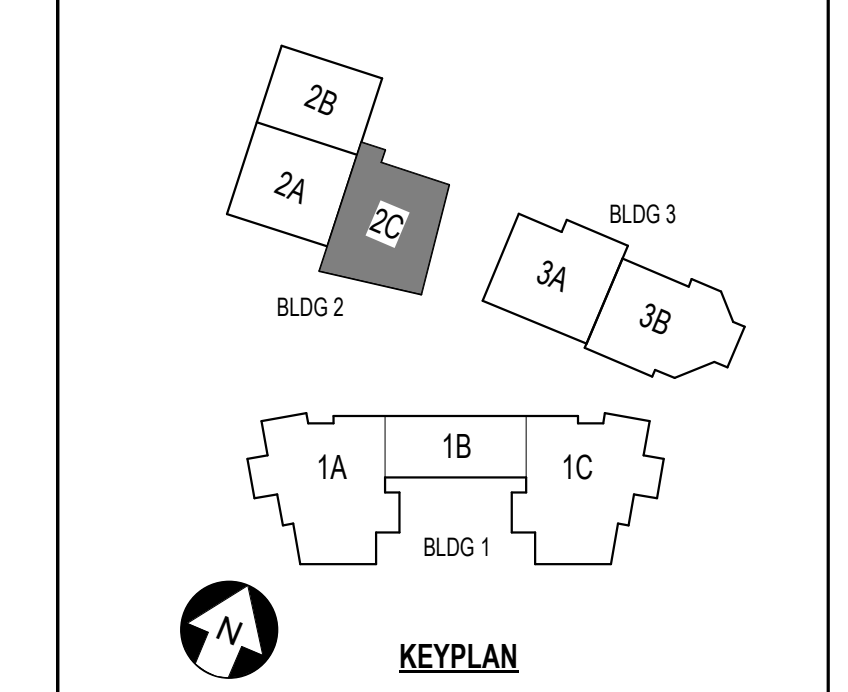
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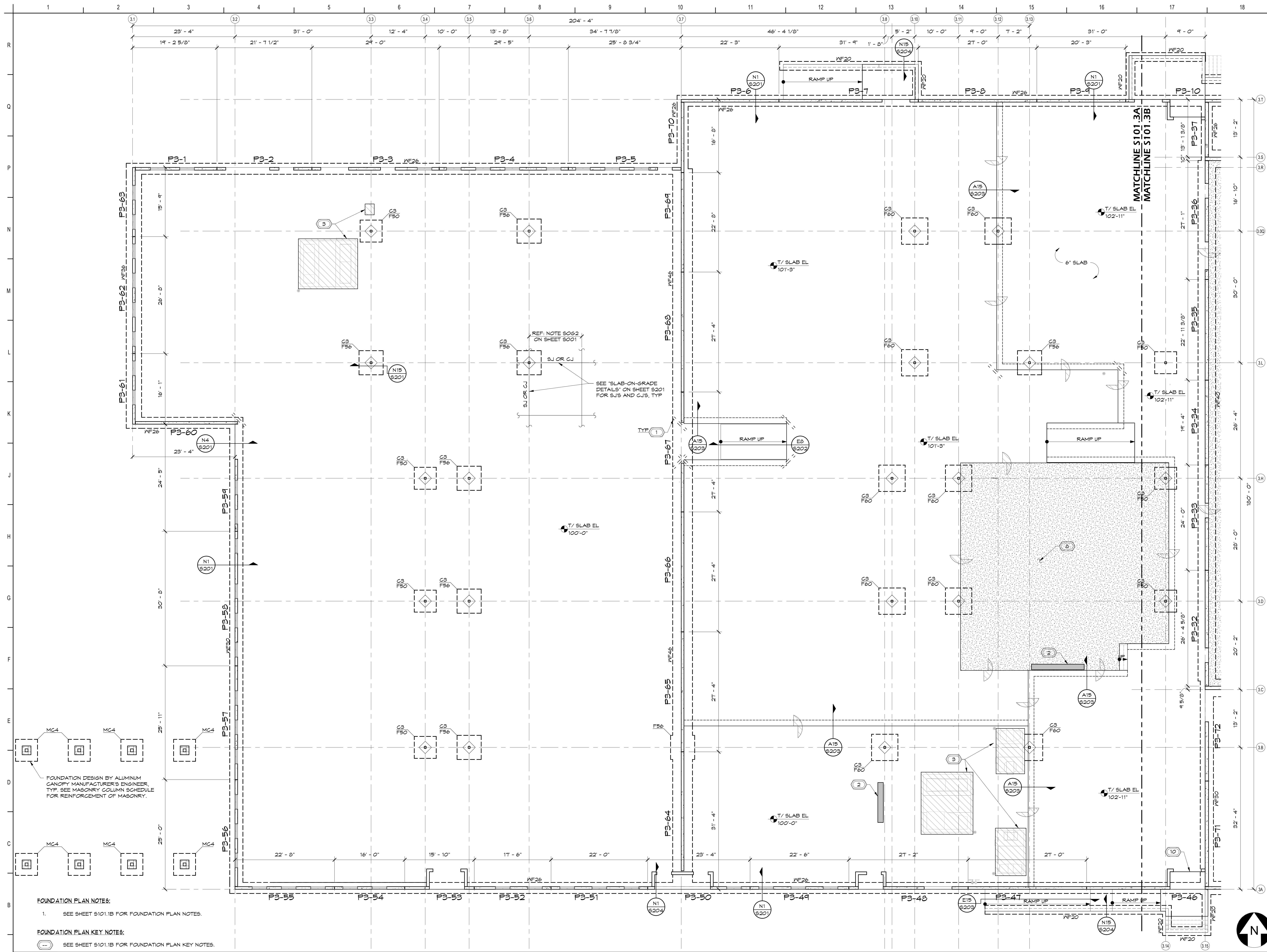
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BUILDING 2 AREA 2C - FOUNDATION PLAN

S101.2C



FOUNDATION PLAN NOTES:

- SEE SHEET S101.1B FOR FOUNDATION PLAN NOTES.

FOUNDATION PLAN KEY NOTES:

SEE SHEET S101.1B FOR FOUNDATION PLAN KEY NOTES.

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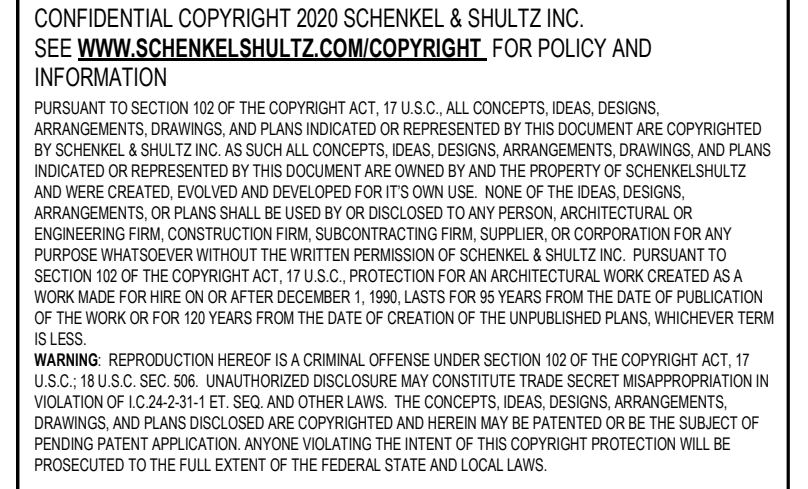
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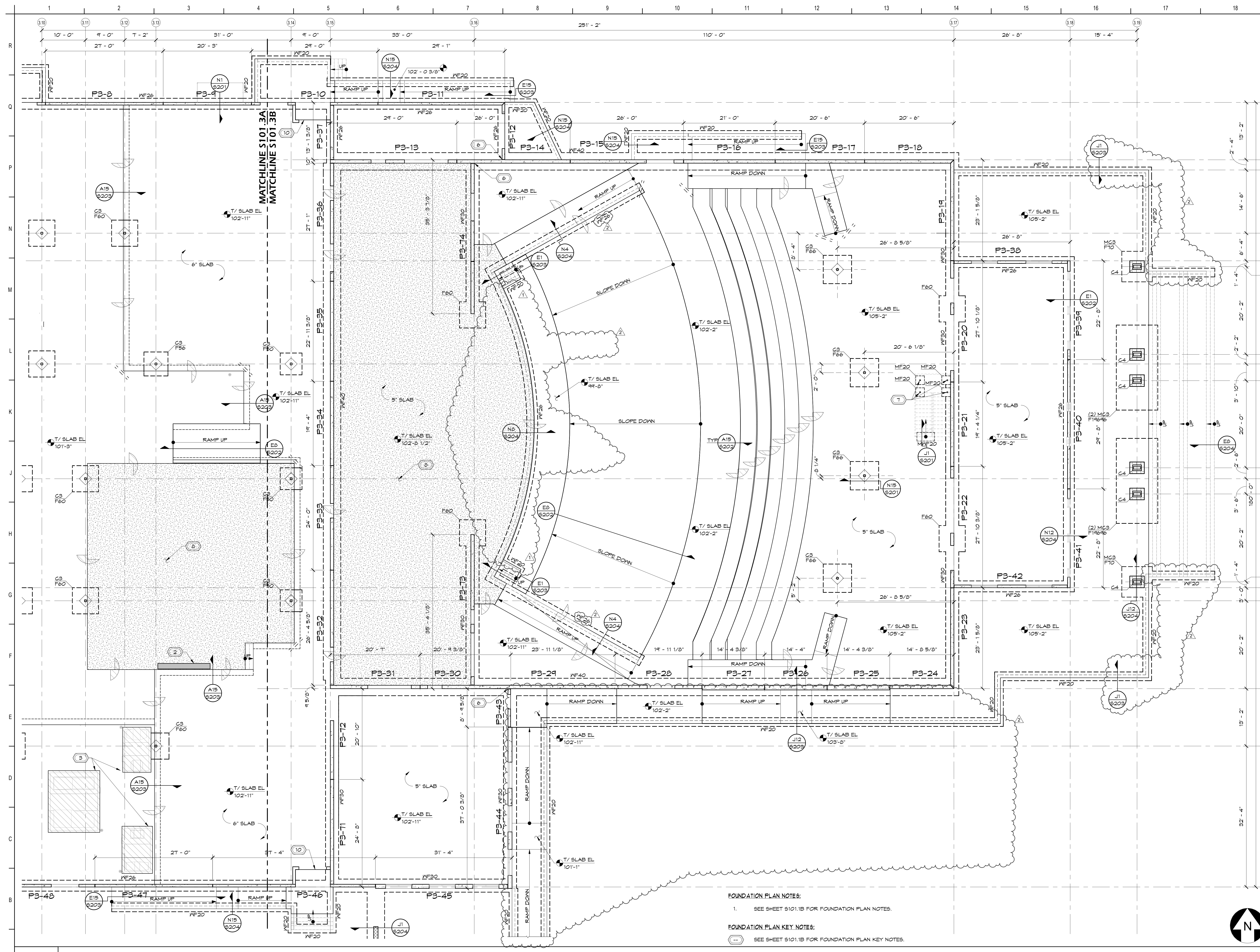
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BUILDING 3 AREA 3A - FOUNDATION PLAN

S101.3A

PHASE III - CONSTRUCTION DOCUMENTS



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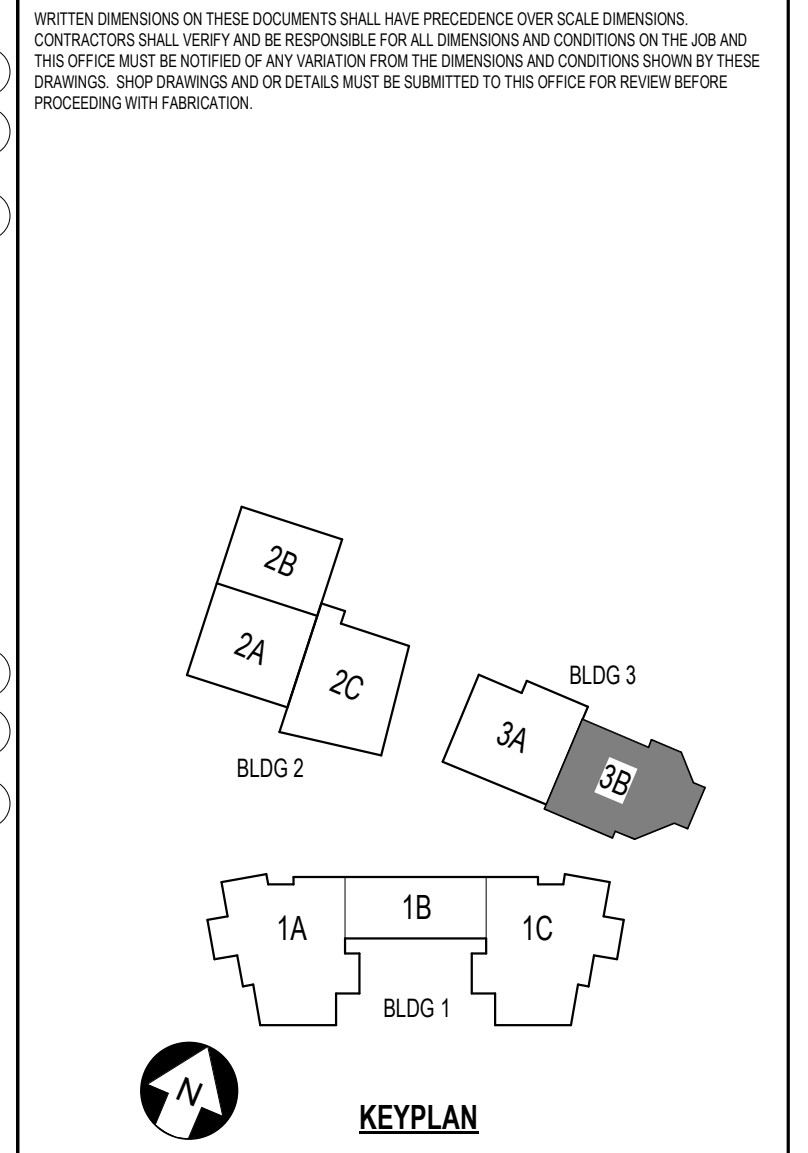
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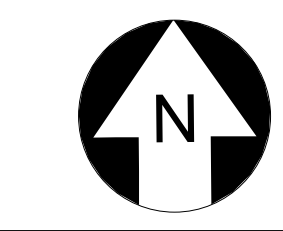
BUILDING 3 AREA 3B - FOUNDATION PLAN

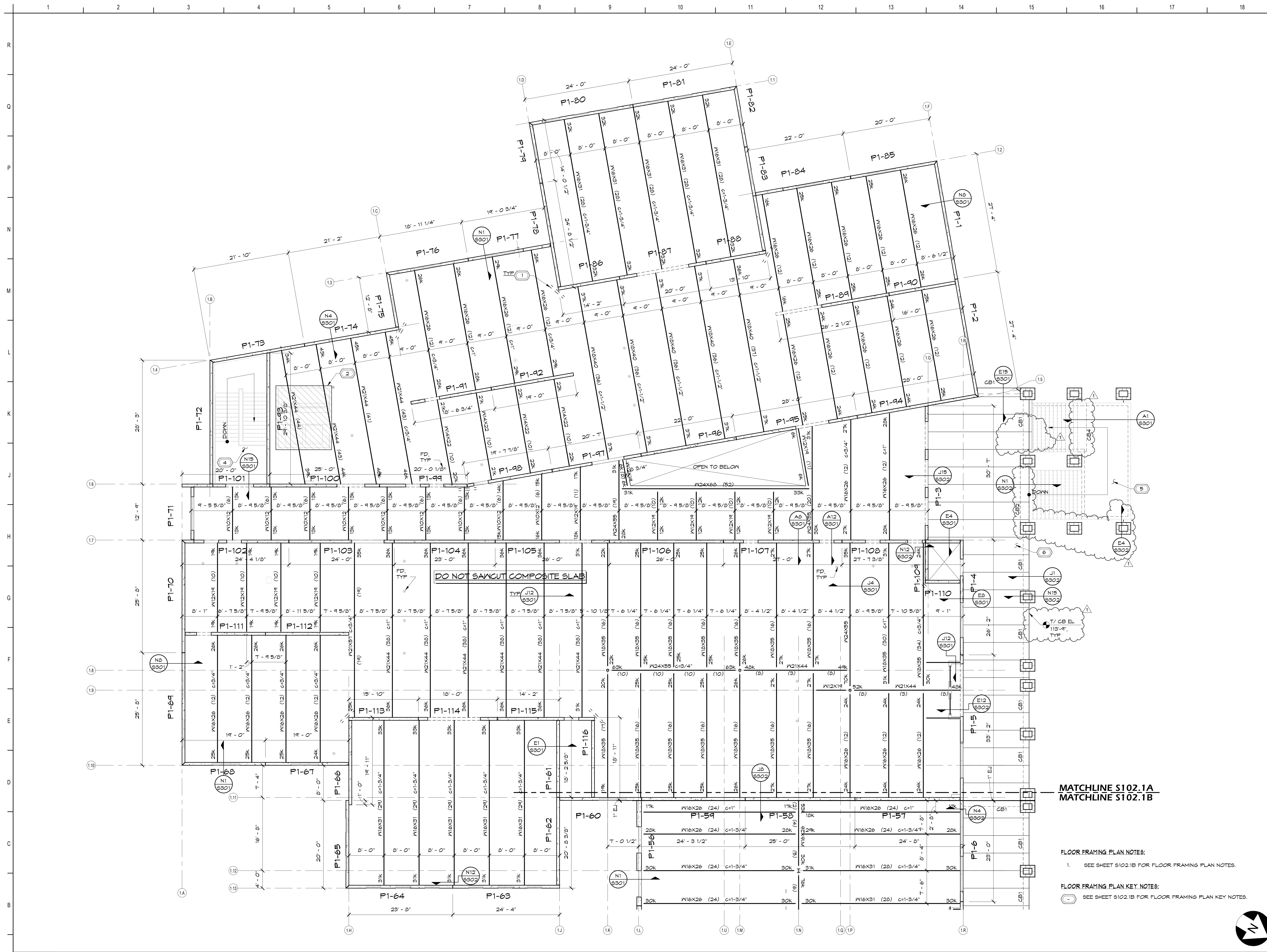
S101.3B

- FOUNDATION PLAN NOTES:
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- FOUNDATION PLAN KEY NOTES:
- SEE SHEET S101.1B FOR FOUNDATION PLAN KEY NOTES.

A1 BUILDING 3 AREA 3B - FOUNDATION PLAN

1/8" = 1'-0"





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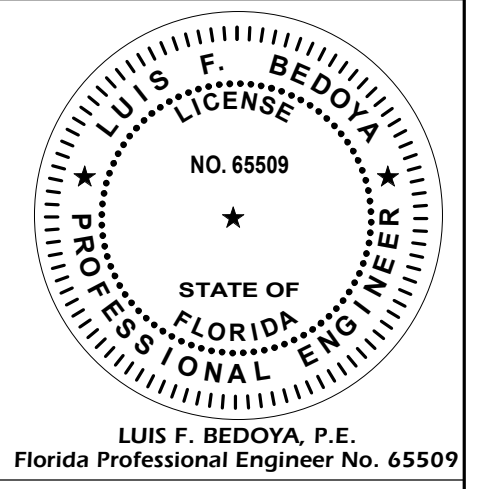
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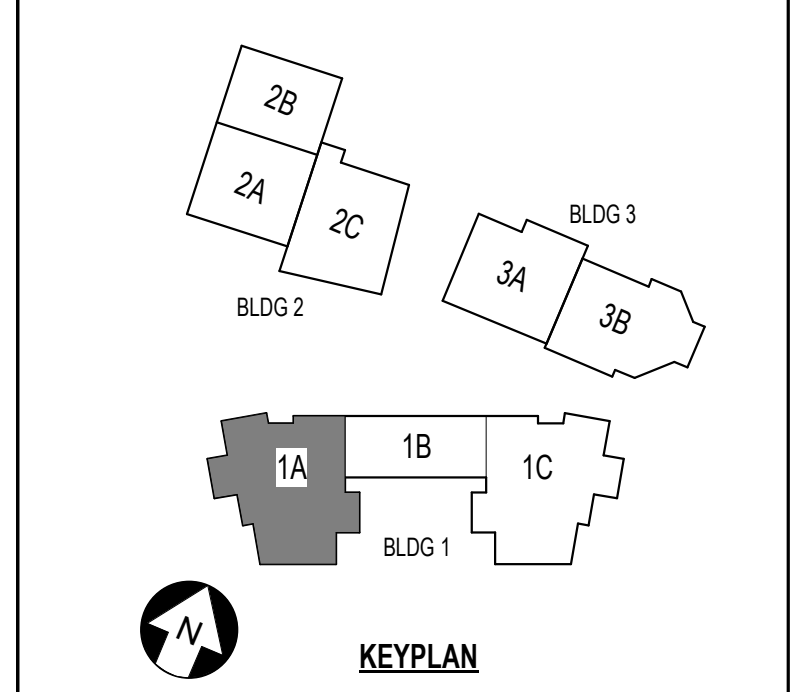


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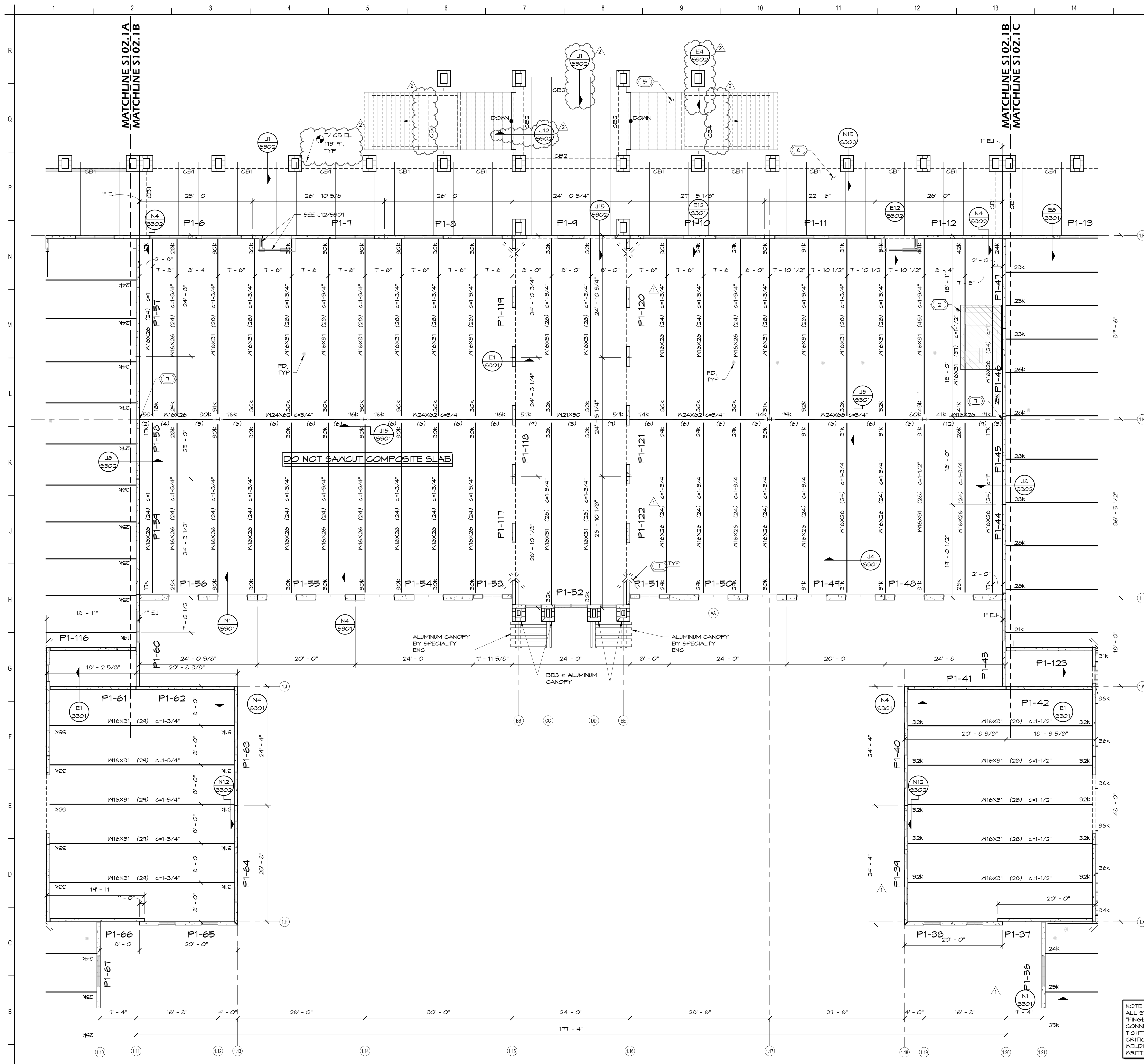
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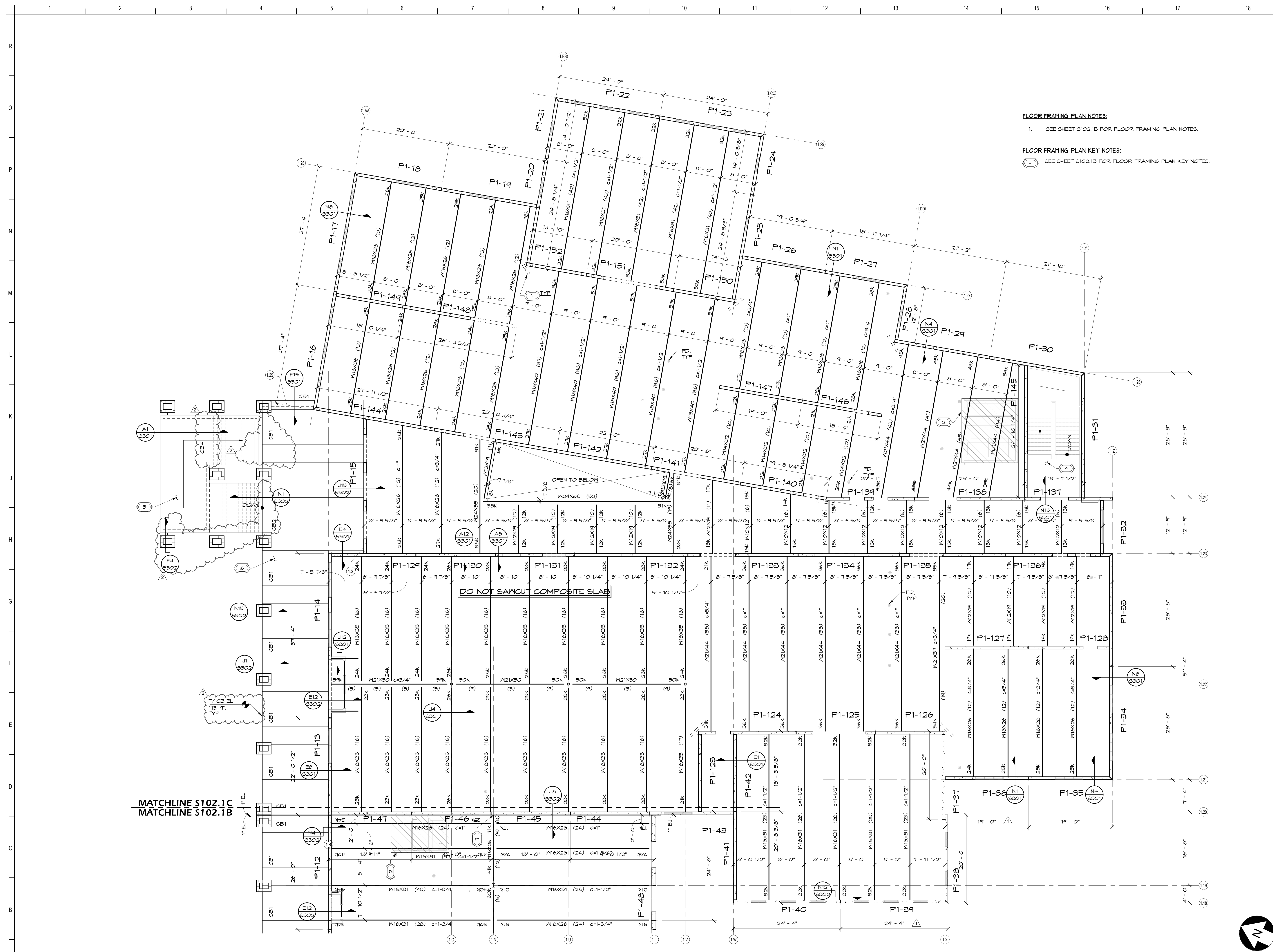
BUILDING 1 AREA 1A - SECOND FLOOR FRAMING PLAN

S102.1A

A1 BUILDING 1 AREA 1A - SECOND FLOOR FRAMING PLAN

1/8" = 1'-0"





FLOOR FRAMING PLAN NOTES:

- SEE SHEET S102.1B FOR FLOOR FRAMING PLAN NOTES.

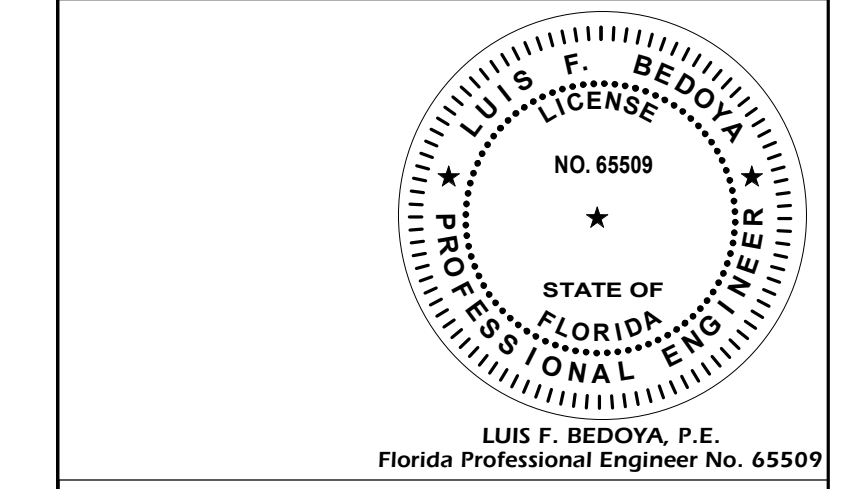
FLOOR FRAMING PLAN KEY NOTES:

- SEE SHEET S102.1B FOR FLOOR FRAMING PLAN KEY NOTES.

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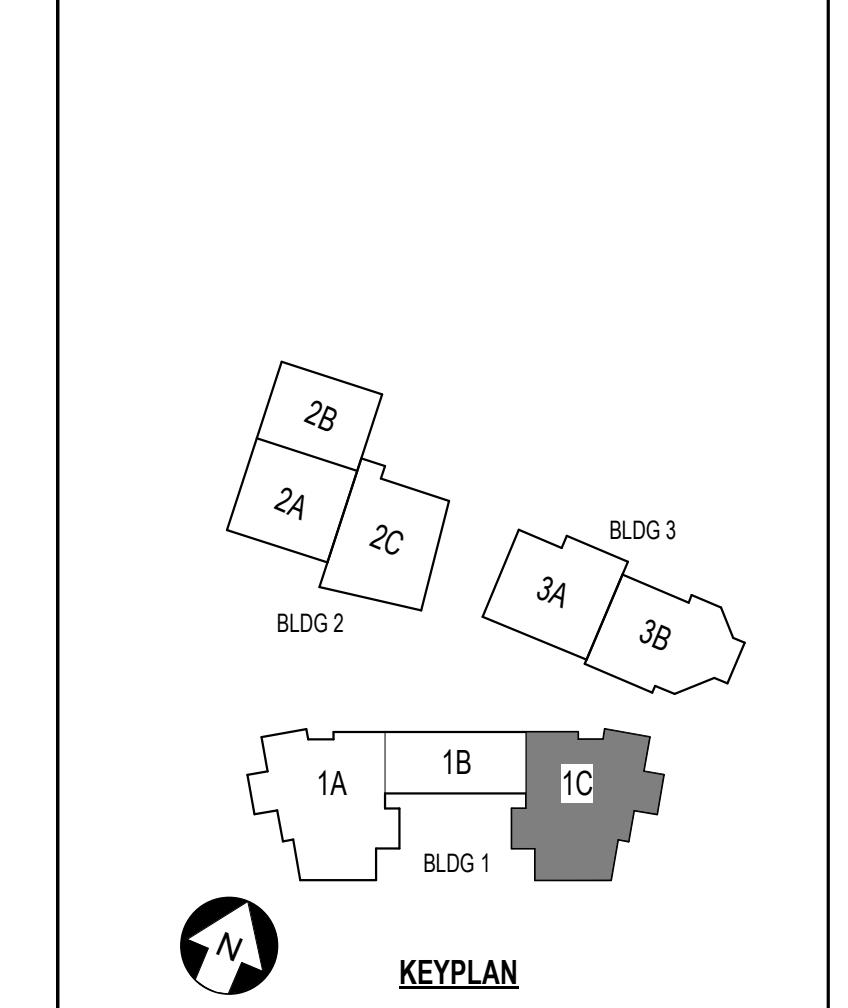
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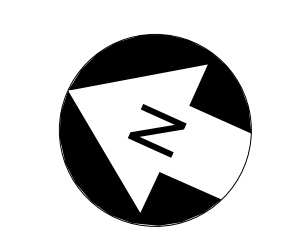
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BUILDING 1 AREA 1C - SECOND FLOOR FRAMING PLAN

S102.1C

A1 BUILDING 1 AREA 1C - SECOND FLOOR FRAMING PLAN

1/8" = 1'-0"





- ROOF FRAMING PLAN NOTES:**
- CELLULAR LIGHTWEIGHT INSULATING CONCRETE ROOFING ON 1 1/2" - 2 1/2" TYPE 'B' GALV G40 METAL DECK WITH VENTING HOLES IF REQUIRED BY THE INSULATING MFR/PROVIDER. PER FBC SECTIONS 1917.4.1 AND 1917.4.1.1 SECURED WITH HILTI PROVIDER ACTUATED FASTENERS AT SUPPORTS AND #10 TEK SCREWS AT SIDELAPS ON OPEN WEB BAR JOISTS AND/OR STRUCTURAL STEEL MEMBERS. USE X-HSN-24 FOR STEEL BAR JOISTS TOP CHORD OR FLANGE THICKNESS 1/8" TO 3/8" AND X-HSN-118 FOR STRUCTURAL STEEL AND STEEL BAR JOISTS WITH TOP CHORD OR FLANGE THICKNESS 1/4" OR THICKER. PROVIDE A 3/8" PATTERN AT SUPPORTS WITH SPACING AT EXTREME SIDES OF 8" OC. (7) SCREWS PER SPAN SHALL BE PROVIDED AT SIDELAPS. FLORIDA PRODUCT APPROVALS AND/OR NOTICE OF ACCEPTANCE TESTING DATA MAY REQUIRE A DIFFERENT ATTACHMENT PATTERN, DIFFERENT SIDELAP SPACING AND/OR DIFFERENT ATTACHMENT TYPE(S). PROVIDE THE MORE STRINGENT OF WHAT IS CALLED FOR HEREIN AND WHAT THE TESTED ASSEMBLY REQUIRES.
 - SEE STRUCTURAL GENERAL NOTES FOR BRIDGING REQUIREMENTS FOR OPEN WEB STEEL JOISTS.
 - JOISTS DESIGNATED 'KCS' (K-SERIES CONSTANT SHEAR PER 'VULCRAFT' TABLES) SHALL BE UTILIZED (IF REQ'D) AT CONCENTRATED LOADS (SEE FRAMING PLAN).
 - SEE FRAMING PLAN FOR JOIST BEARING ELEVATIONS.
 - REF: ARCH. DWS FOR INTERIOR DRAINS, SCUPPERS, CRICKETS AND SCUTTLES.
 - 'BTJ' INDICATES BOLTED TIE JOIST PER OSHA REQUIREMENTS (TYP). SEE GENERAL NOTE S112 FOR ADDITIONAL INFORMATION.
 - COORDINATE SIZE AND LOCATION OF ROOFTOP UNITS WITH MECHANICAL DWS.
 - BEAM END REACTIONS AS SHOWN ON PLAN ARE ULTIMATE REACTIONS (ALREADY FACTORED).
 - ALL ROOF DRAINS (WHERE REQUIRED) SHALL BE SUPPORTED BY A 1/2" X 3/4" ANGLE FRAME, TYP.
 - MASONRY BOND BEAMS AND TIE BEAMS THAT ARE INDICATED ON THE PLAN SHALL BE CONTINUOUS FOR THE ENTIRE LENGTH OF THE WALL, UNO, ELEVATIONS INDICATED ON PLAN ARE TO THE TOP OF BEAM, UNO.
 - ALL WIDE FLANGE MEMBERS SHALL BE CONNECTED TO THE SUPPORTING STRUCTURE AS DETAILED IN THE CONNECTION SCHEDULES ON SHEET S602. UNLESS SPECIFICALLY NOTED OTHERWISE ON PLAN, ALL ROOF MEMBERS SHALL BE CONNECTED AS DETAILED IN THE SINGLE SHEAR SCHEDULES J1/S602 AND J2/S602.
 - BOND BEAM BB1 SHALL BE INSTALLED AT TOP OF ALL LOAD-BEARING MASONRY WALLS, TYP (UNO).

- ROOF FRAMING PLAN KEY NOTES:**
- ROOF HATCH / MECHANICAL EQUIPMENT, REF A8/S402.
 - BASKETBALL BACK BOARD SUPPORT, REACTIONS TO BE COORDINATED W/ JOIST MFR.
 - PROVIDE 2" LONG HORIZ HOLES IN STEEL BEAM. SEE A8/S602 FOR ADDITIONAL INFORMATION. USE FIBER BEARING PAD UNDER STEEL BEAM BEARING ON ANGLE.
 - 3/8" X 20/20 GA CELLULAR DECK. ATTACH PER NOTE 1. USE TWO HILTI PINS FOR ATTACHMENT TO STRUCTURAL SUPPORTS.
 - SKEWED CONN BY SPECIALTY ENGINEER, REF S95A ON SHEET S005.
 - CONNECTION OF HSS TO WALL W/ EMBED PLATE PER J1/S602, 3/16 FILLET WELD ALL AROUND.
 - HSS 4x2x3/16 (LLV) PERIMETER DECK CLOSURE.
 - W8x18 HOIST BEAM SUPPORTED ON BOTTOM CHORD OF STEEL JOIST. STEEL JOIST DESIGN TO INCLUDE 23K POINT LOAD.
- ROOF LEGEND:**
- BB - INDICATES BOND BEAM. SEE SCHEDULE ON SHEET S601 FOR INFO.
 - CB - INDICATES CONCRETE BEAM. SEE SCHEDULE ON SHEET S601 FOR INFO.
 - TB - INDICATES TIE BEAM. SEE SCHEDULE ON SHEET S601 FOR INFO.
 - BTJ - INDICATES BOLTED TIE JOIST. SEE DETAIL ON SHEET S401 FOR INFO.

NOTE TO ERECTOR:
 ALL STRUCTURAL STEEL CONNECTIONS TO EMBED PLATES SHALL BE "FINGER TIGHT" WITH FINED THREADS. ALL STRUCTURAL STEEL CONNECTIONS TO OTHER STRUCTURAL STEEL MEMBERS SHALL BE "SNUG TIGHT". NO BOLTED CONNECTION SHALL BE FULLY TENSIONED UNLESS SLIP CRITICAL (SC) BOLTS ARE SPECIFICALLY INDICATED. ADDITIONALLY, FIELD WELDING OF ANY BOLTED CONNECTION IS STRICTLY PROHIBITED UNLESS WRITTEN ACCEPTANCE IS PROVIDED BY BSM.

ROOF DESIGN LOADS	
DEAD LOAD	LIVE LOAD
25 PSF	20 PSF (UNREDUCIBLE)

NOTES:

- SEE TABLE ON SHEET S603 FOR GROSS UPLIFT LOADS.
- SEE PLAN FOR CONCENTRATED LOADS.

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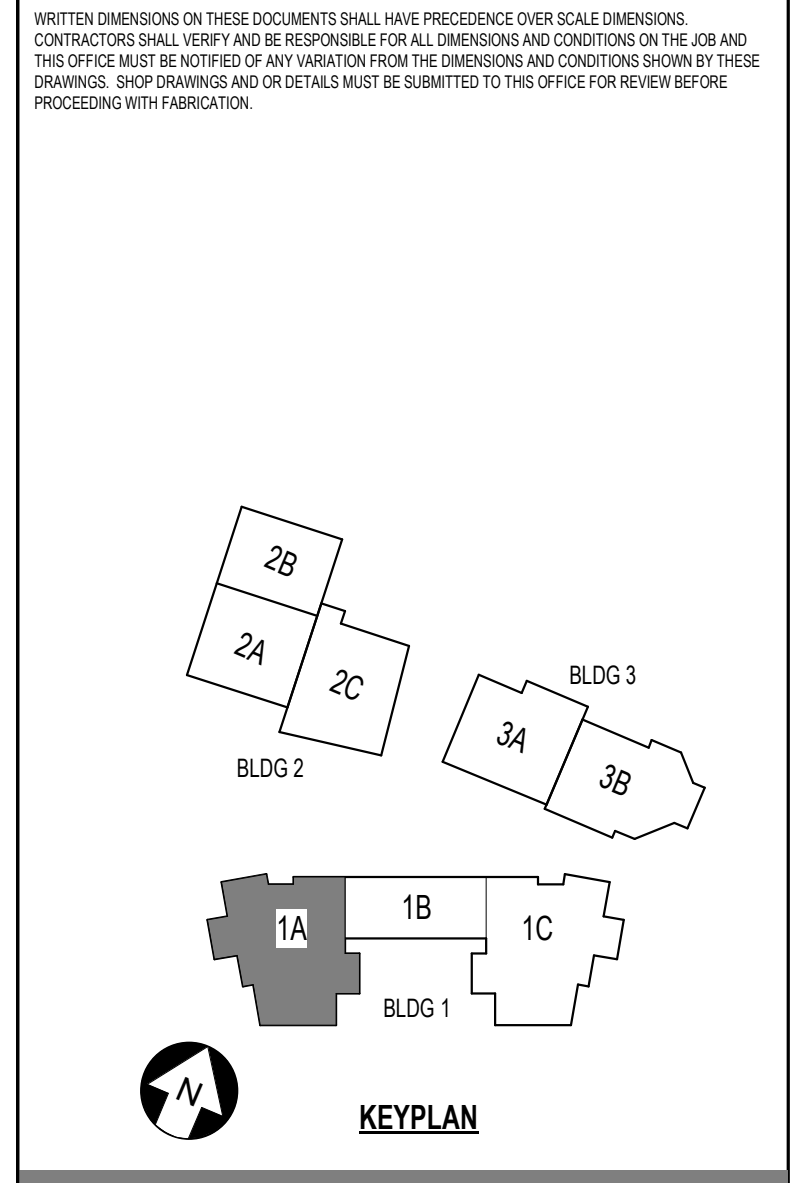
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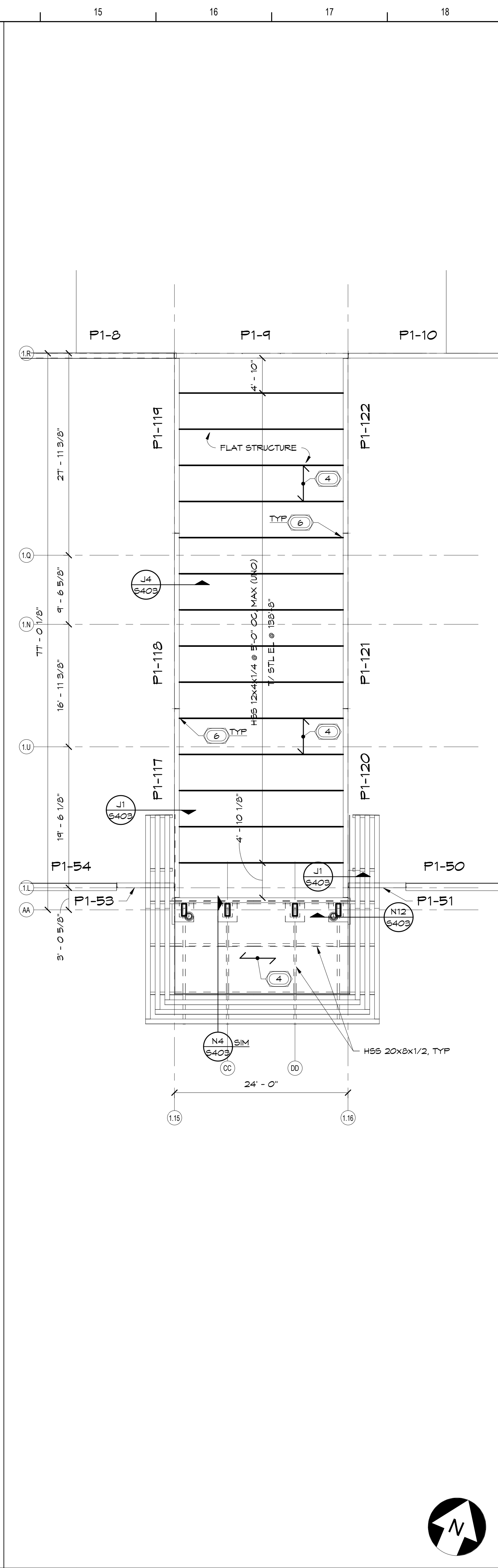
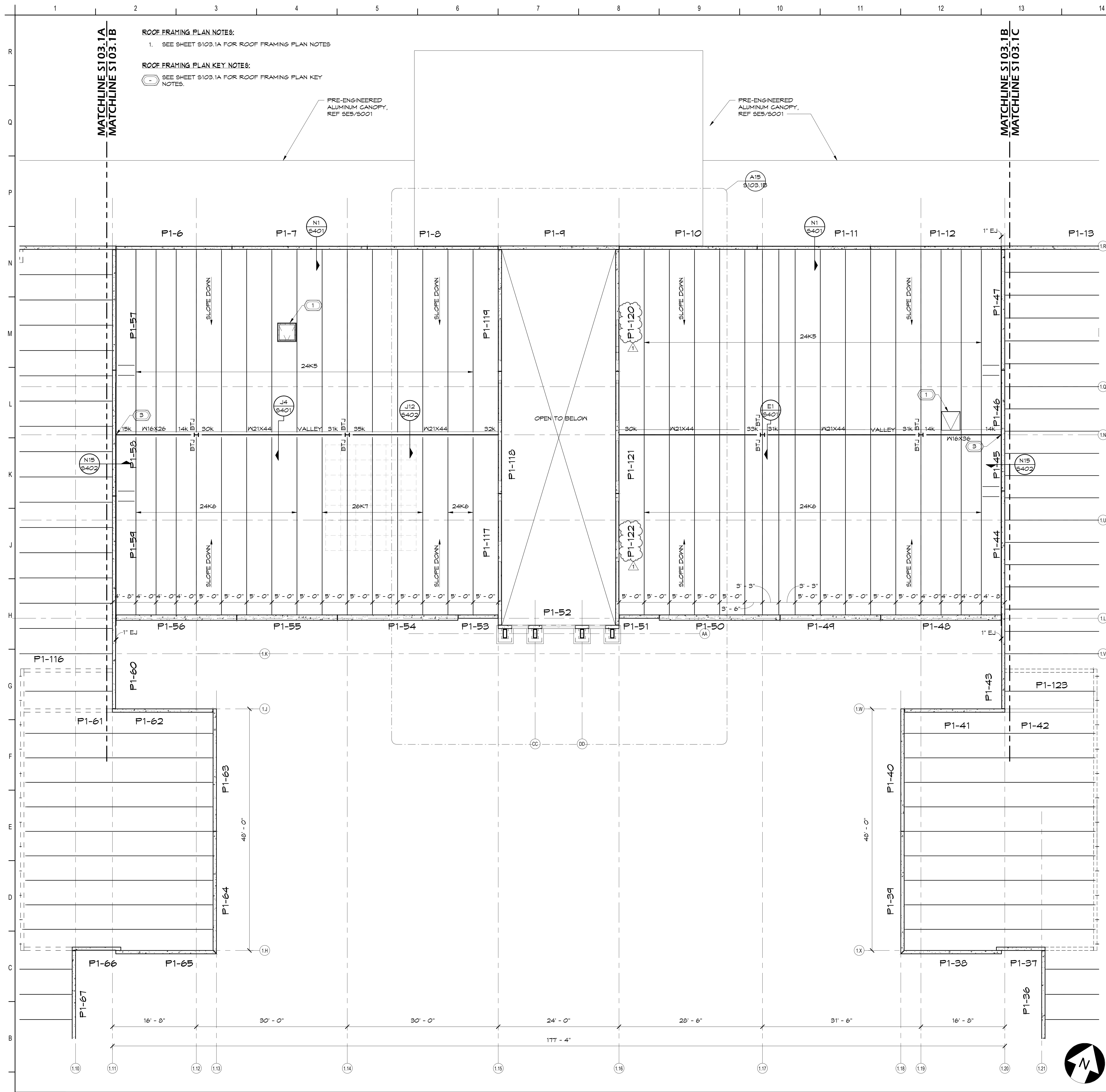
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BUILDING 1 AREA 1A - ROOF FRAMING PLAN

S103.1A



ROOF FRAMING PLAN NOTES:
 1. SEE SHEET S103.1A FOR ROOF FRAMING PLAN NOTES

ROOF FRAMING PLAN KEY NOTES:
 SEE SHEET S103.1A FOR ROOF FRAMING PLAN KEY NOTES.

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 STATE OF FLORIDA
 PROFESSIONAL ENGINEER

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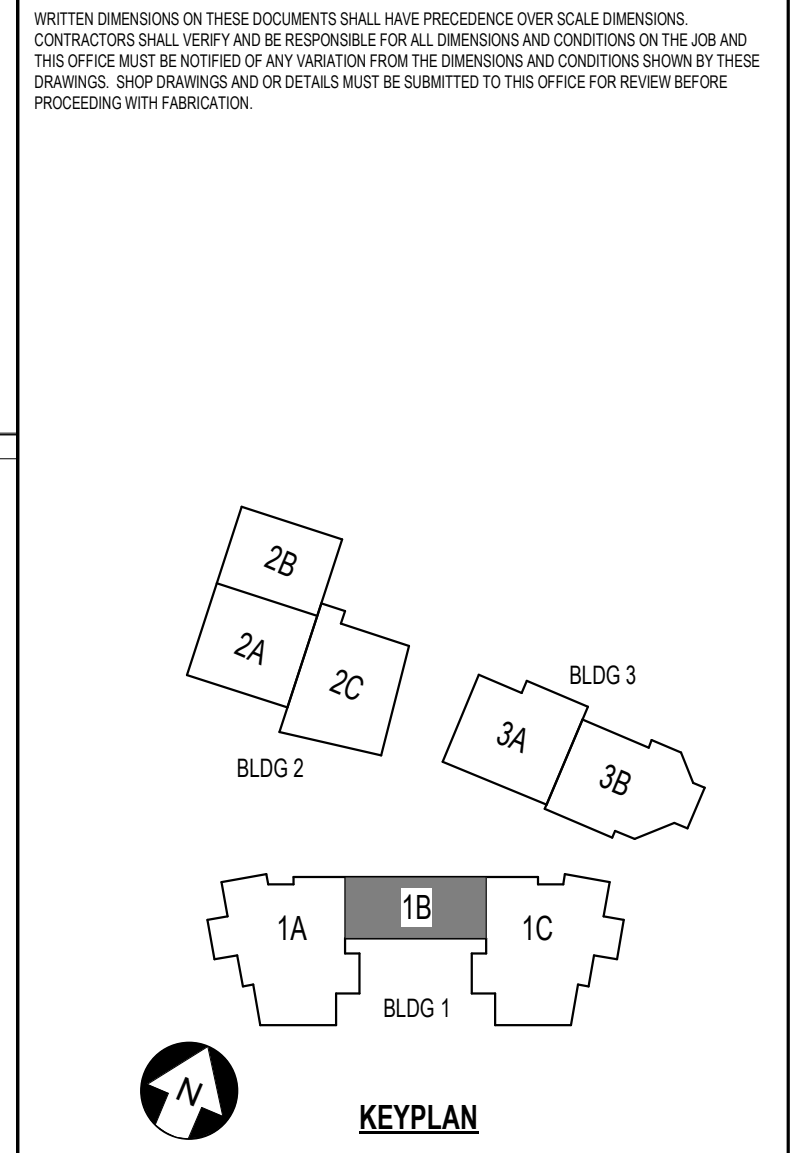
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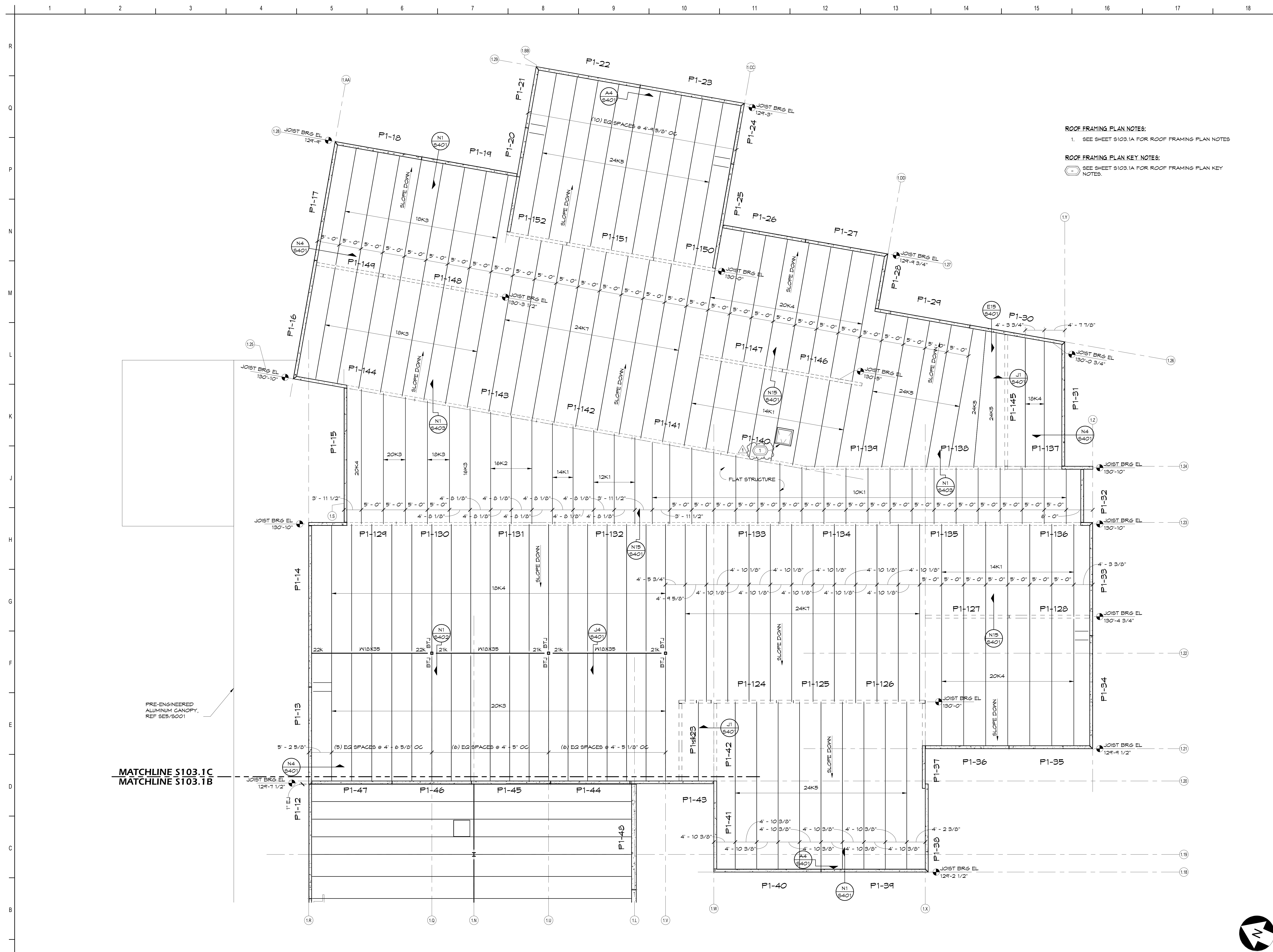
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BUILDING 1 AREA 1B - ROOF / HIGH ROOF FRAMING PLAN

S103.1B

A1 BUILDING 1 AREA 1B - LOW ROOF FRAMING PLAN 1/8" = 1'-0"

A15 BUILDING 1 AREA 1B - HIGH ROOF FMG PLAN 1/8" = 1'-0"



ROOF FRAMING PLAN NOTES:
 1. SEE SHEET S103.1A FOR ROOF FRAMING PLAN NOTES

ROOF FRAMING PLAN KEY NOTES:
 (N1) SEE SHEET S103.1A FOR ROOF FRAMING PLAN KEY NOTES.

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MATCHLINE S103.1C
 MATCHLINE S103.1B

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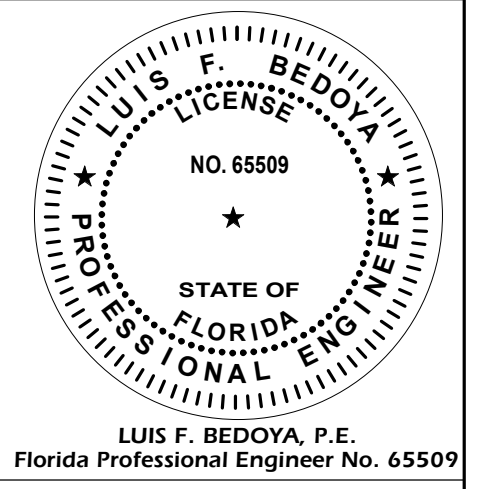
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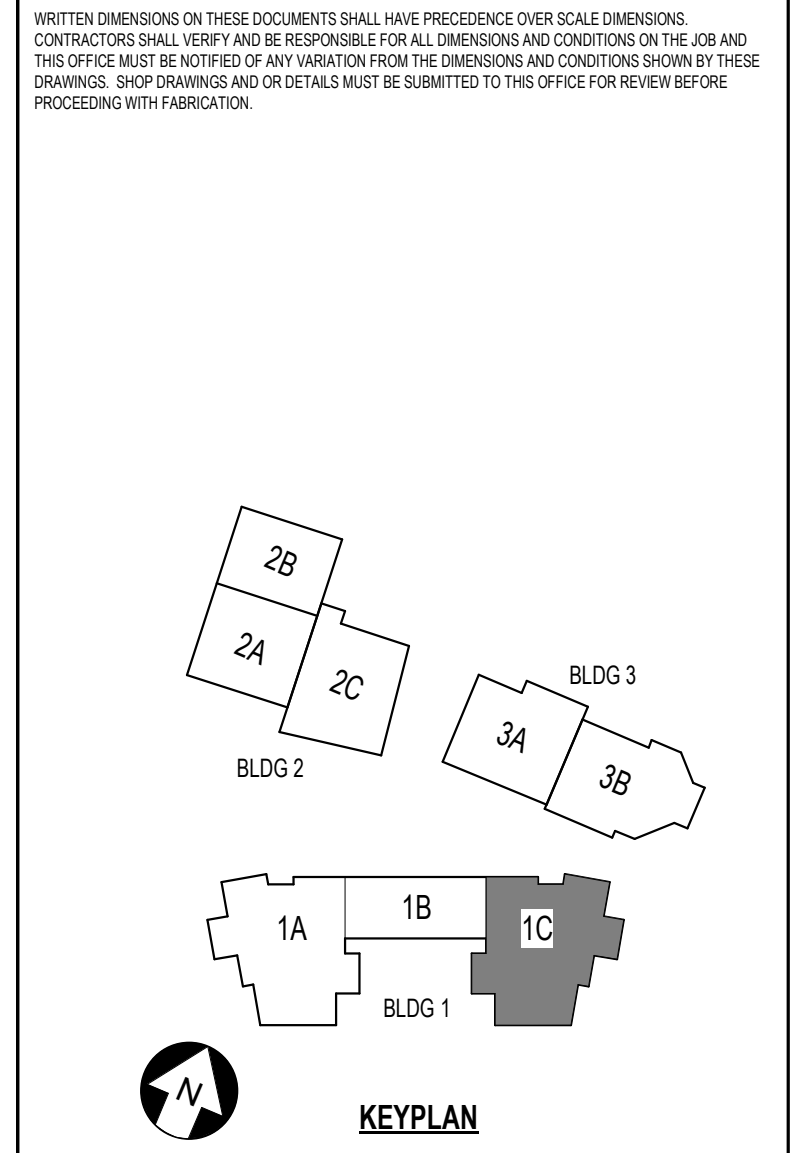
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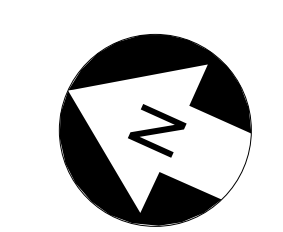
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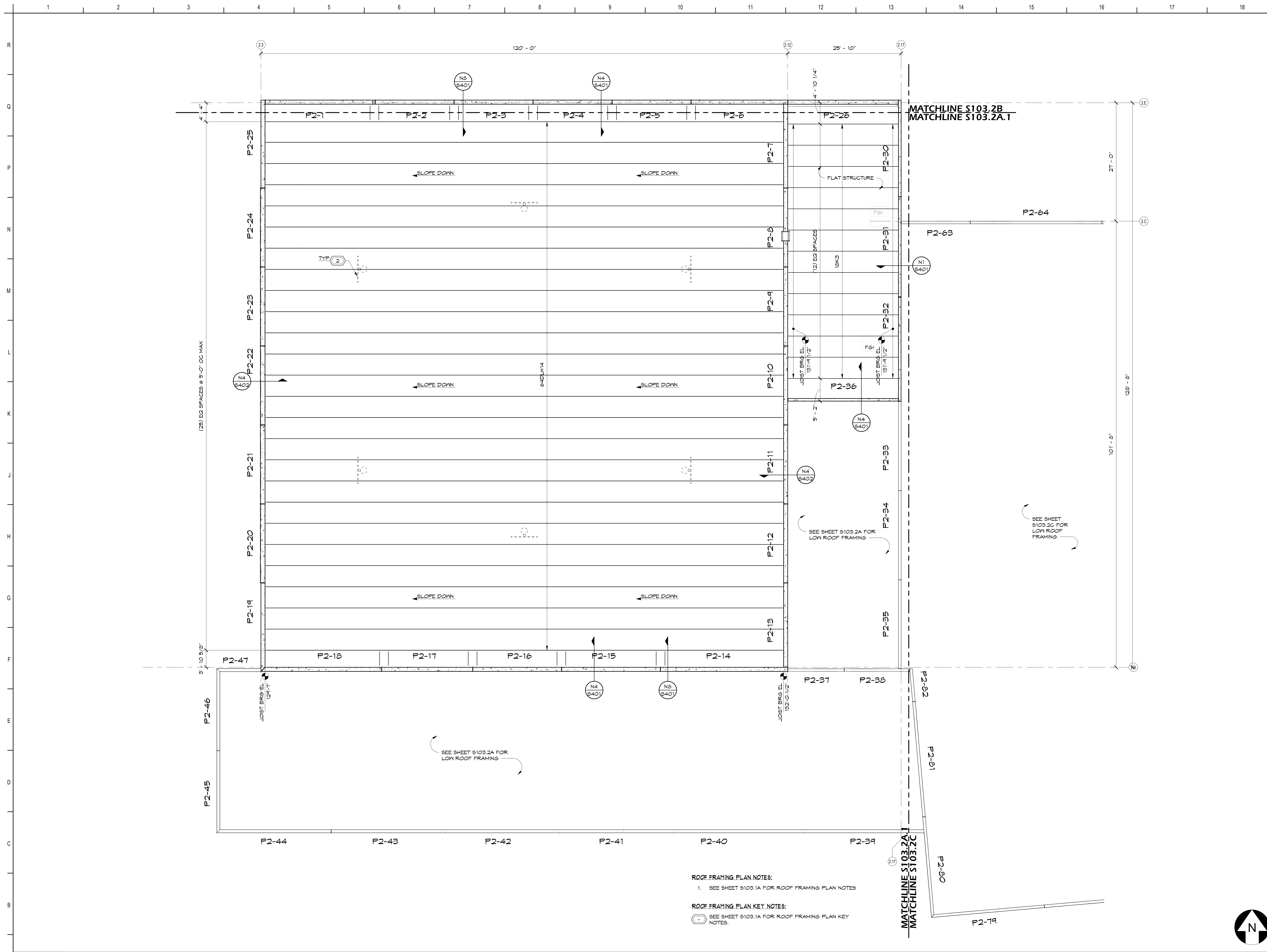
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BUILDING 1 AREA 1C - ROOF FRAMING PLAN

S103.1C





- ROOF FRAMING PLAN NOTES:**
- SEE SHEET S103.1A FOR ROOF FRAMING PLAN NOTES
- ROOF FRAMING PLAN KEY NOTES:**
- SEE SHEET S103.1A FOR ROOF FRAMING PLAN KEY NOTES.

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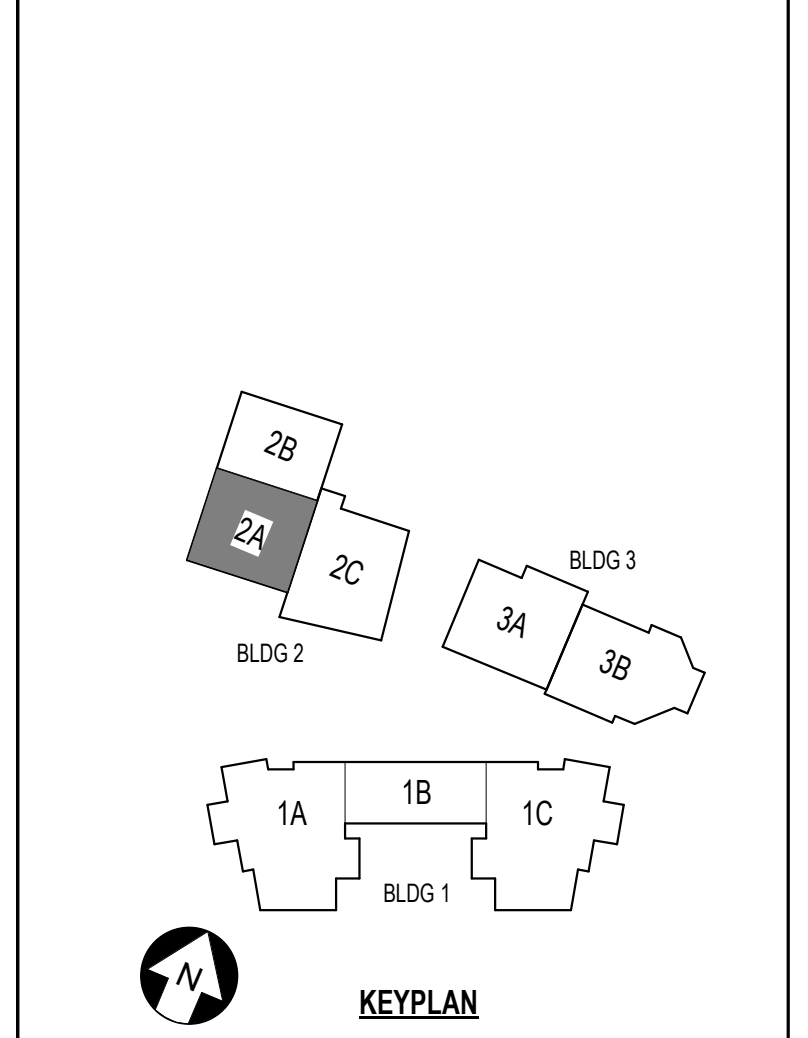
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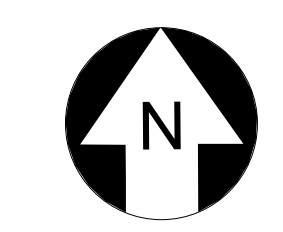
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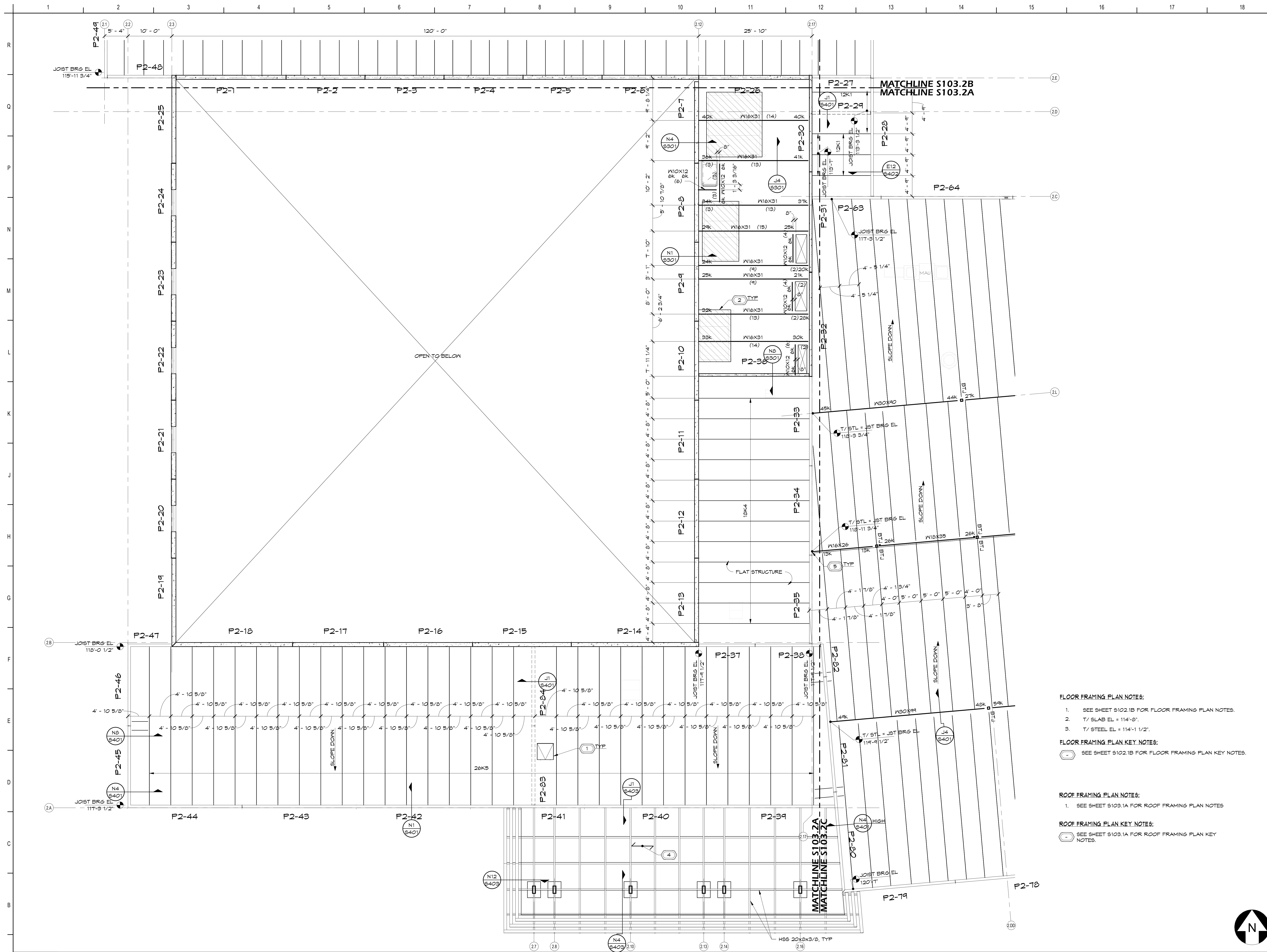
BUILDING 2 AREA 2A - HIGH ROOF FRAMING PLAN



S103.2A.1

A1 BUILDING 2 AREA 2A - HIGH ROOF FRAMING PLAN

1/8" = 1'-0"



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DESIGNED BY: LUIS F. BEDOYA, P.E.
CHECKED BY: LUIS F. BEDOYA, P.E.
APPROVED BY: LUIS F. BEDOYA, P.E.
DATE: 12/16/2022

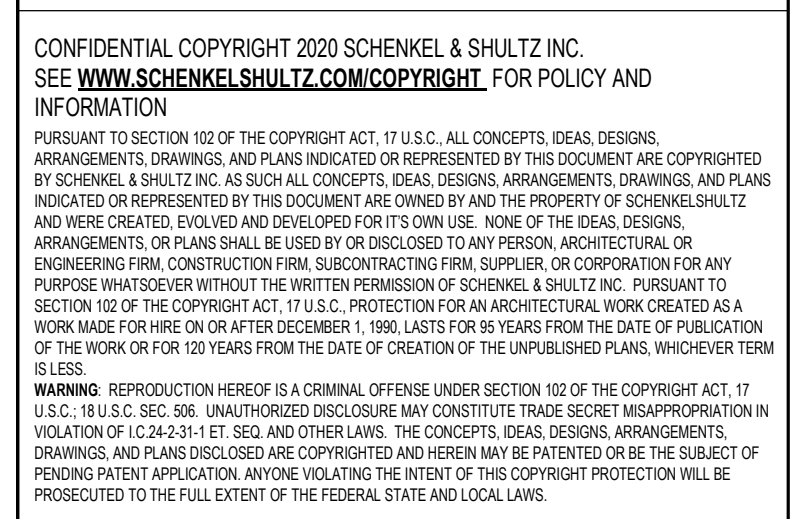
LUIS F. BEDOYA, P.E.
Florida Professional Engineer No. 65509

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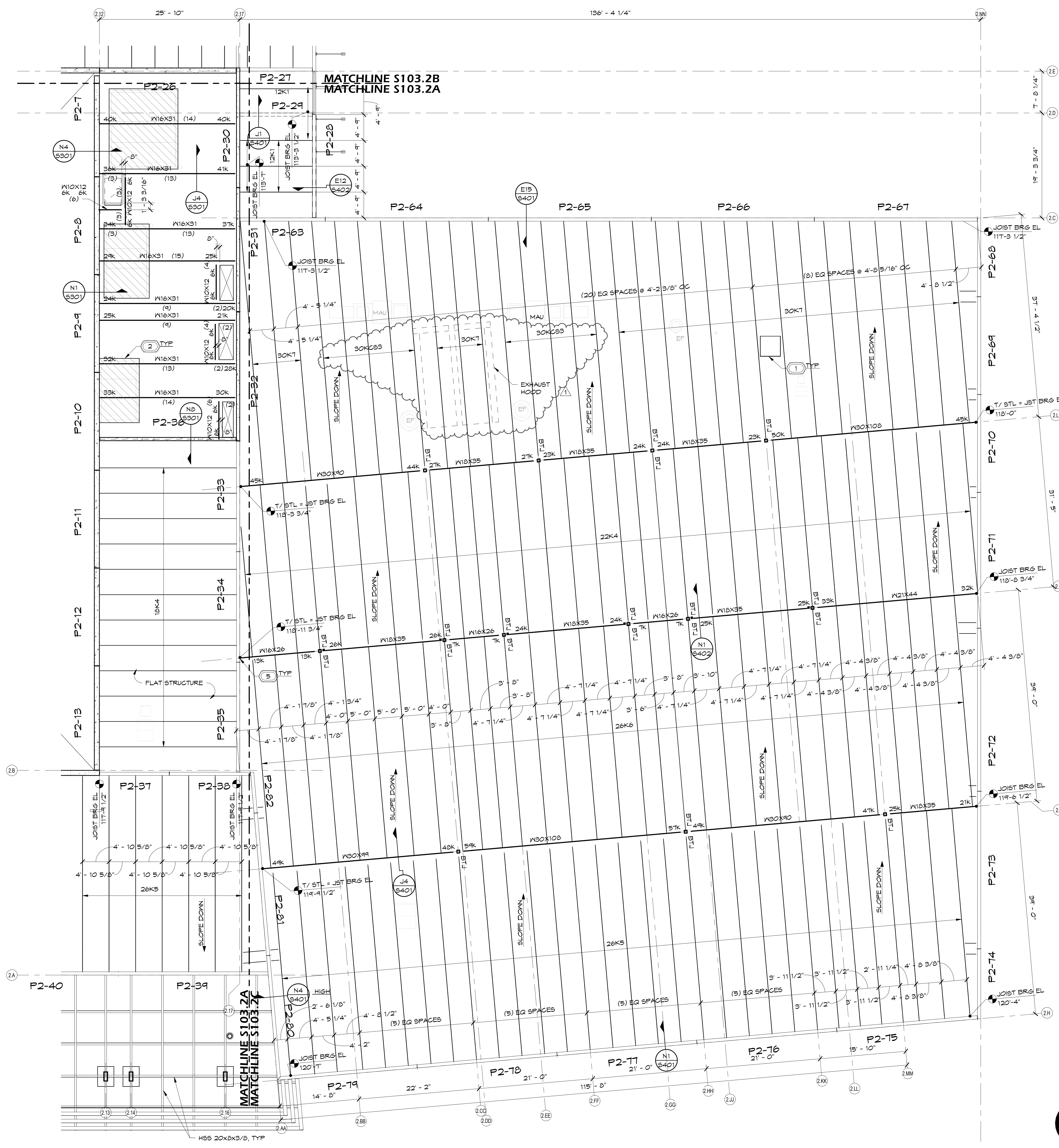
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- FLOOR FRAMING PLAN NOTES:**
- SEE SHEET S102.1B FOR FLOOR FRAMING PLAN NOTES.
 - T/ SLAB EL = 114'-0".
 - T/ STEEL EL = 114'-1 1/2".
- FLOOR FRAMING PLAN KEY NOTES:**
- SEE SHEET S102.1B FOR FLOOR FRAMING PLAN KEY NOTES.
- ROOF FRAMING PLAN NOTES:**
- SEE SHEET S103.1A FOR ROOF FRAMING PLAN NOTES.
- ROOF FRAMING PLAN KEY NOTES:**
- SEE SHEET S103.1A FOR ROOF FRAMING PLAN KEY NOTES.

A1 BUILDING 2 AREA 2A - ROOF FRAMING PLAN & MEZZANINE FLOOR FRAMING PLAN

1/8" = 1'-0"

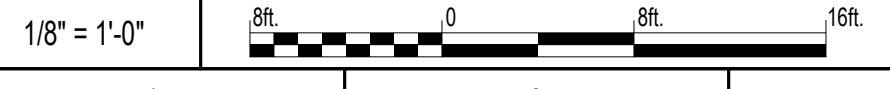
S103.2A



ROOF FRAMING PLAN NOTES:
 1. SEE SHEET S103.1A FOR ROOF FRAMING PLAN NOTES

ROOF FRAMING PLAN KEY NOTES:
 SEE SHEET S103.1A FOR ROOF FRAMING PLAN KEY NOTES.

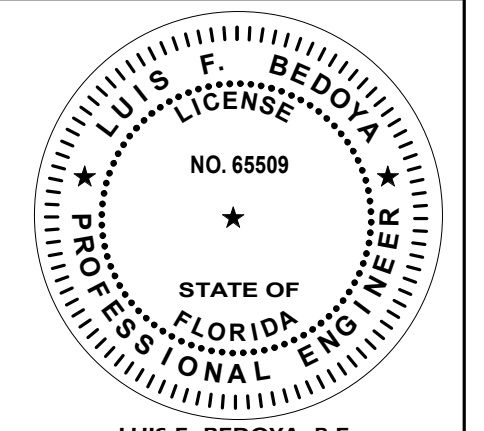
A1 BUILDING 2 AREA 2C - ROOF FRAMING PLAN



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REVISIONS

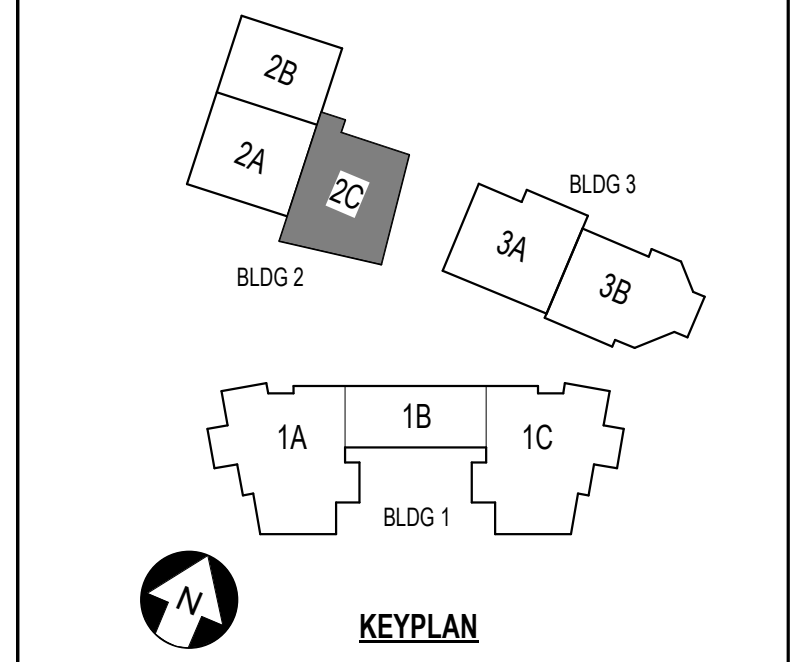
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ADDENDUM NO. 2		08/09/2022

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BUILDING 2 AREA 2C - ROOF FRAMING PLAN

S103.2C



ROOF FRAMING PLAN NOTES:
 1. SEE SHEET S103.1A FOR ROOF FRAMING PLAN NOTES

ROOF FRAMING PLAN KEY NOTES:
 (C) SEE SHEET S103.1A FOR ROOF FRAMING PLAN KEY NOTES.

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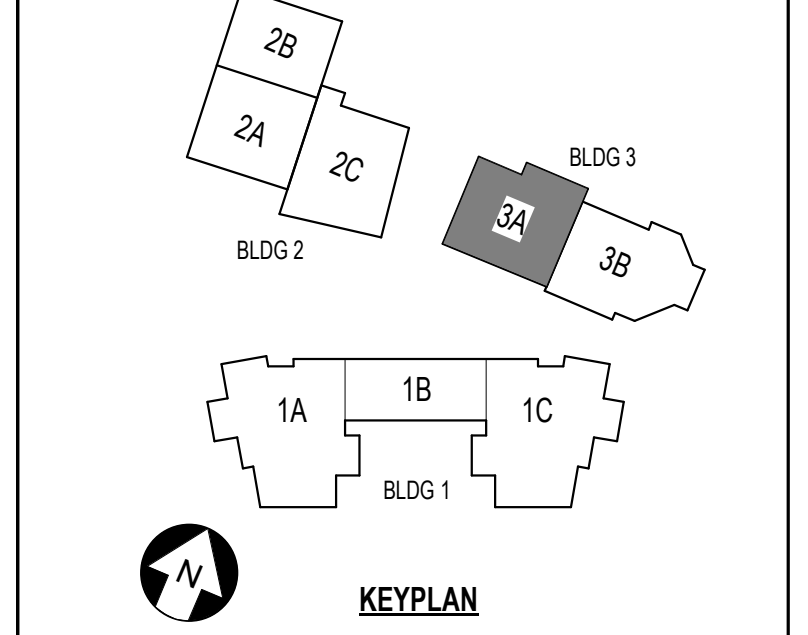
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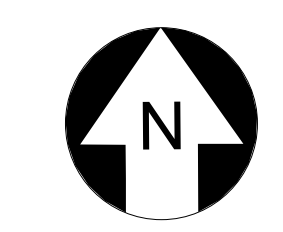
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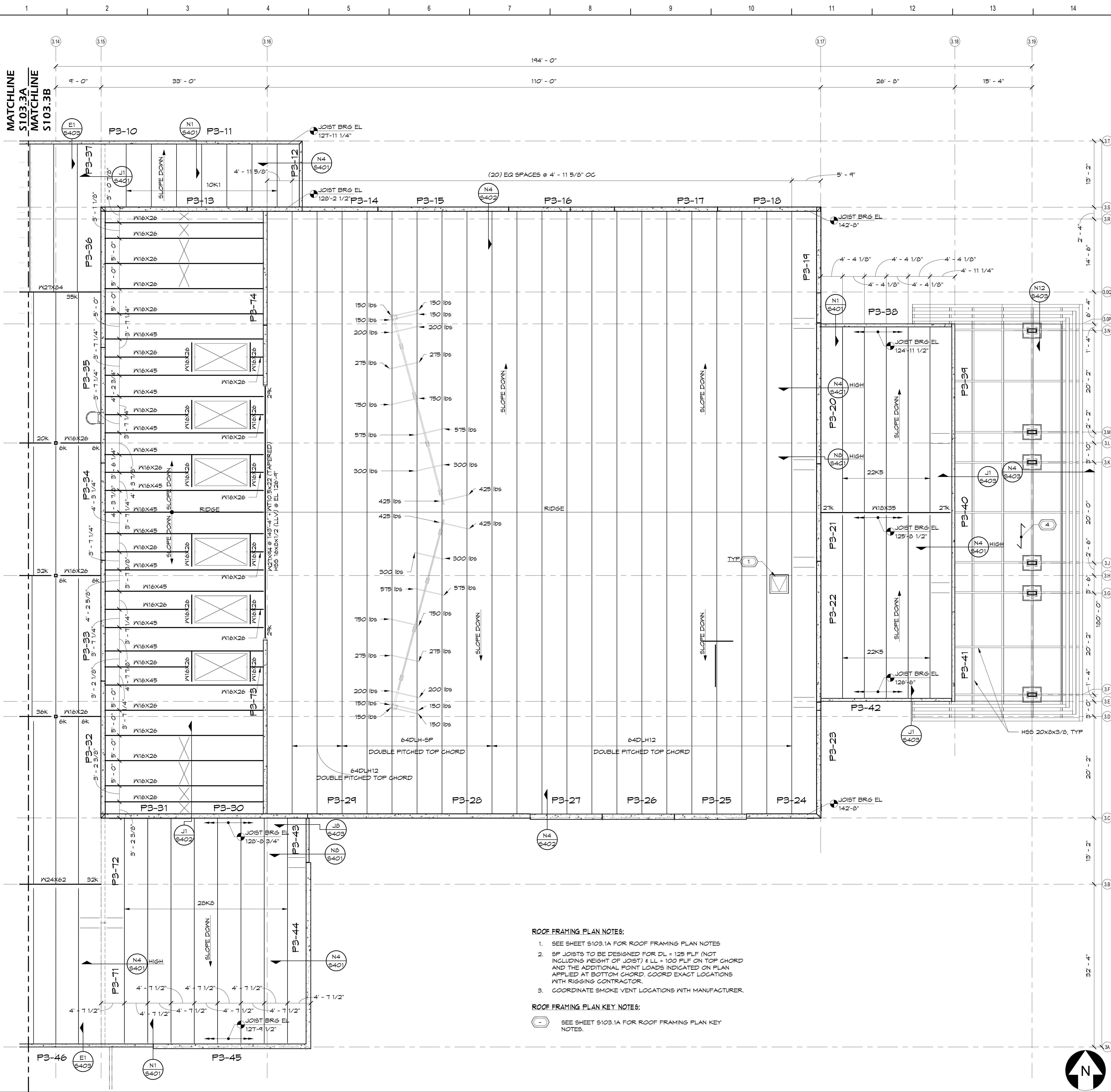
BUILDING 3 AREA 3A - ROOF FRAMING PLAN

S103.3A



A1 BUILDING 3 AREA 3A - ROOF FRAMING PLAN

1/8" = 1'-0"

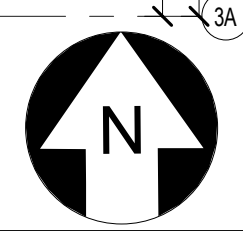


ROOF FRAMING PLAN NOTES:

- SEE SHEET S103.1A FOR ROOF FRAMING PLAN NOTES
- SP JOISTS TO BE DESIGNED FOR DL = 125 PLF (NOT INCLUDING WEIGHT OF JOIST) & LL = 100 PLF ON TOP CHORD AND THE ADDITIONAL POINT LOADS INDICATED ON PLAN APPLIED AT BOTTOM CHORD. COORD EXACT LOCATIONS WITH RISING CONTRACTOR.
- COORDINATE SMOKE VENT LOCATIONS WITH MANUFACTURER.

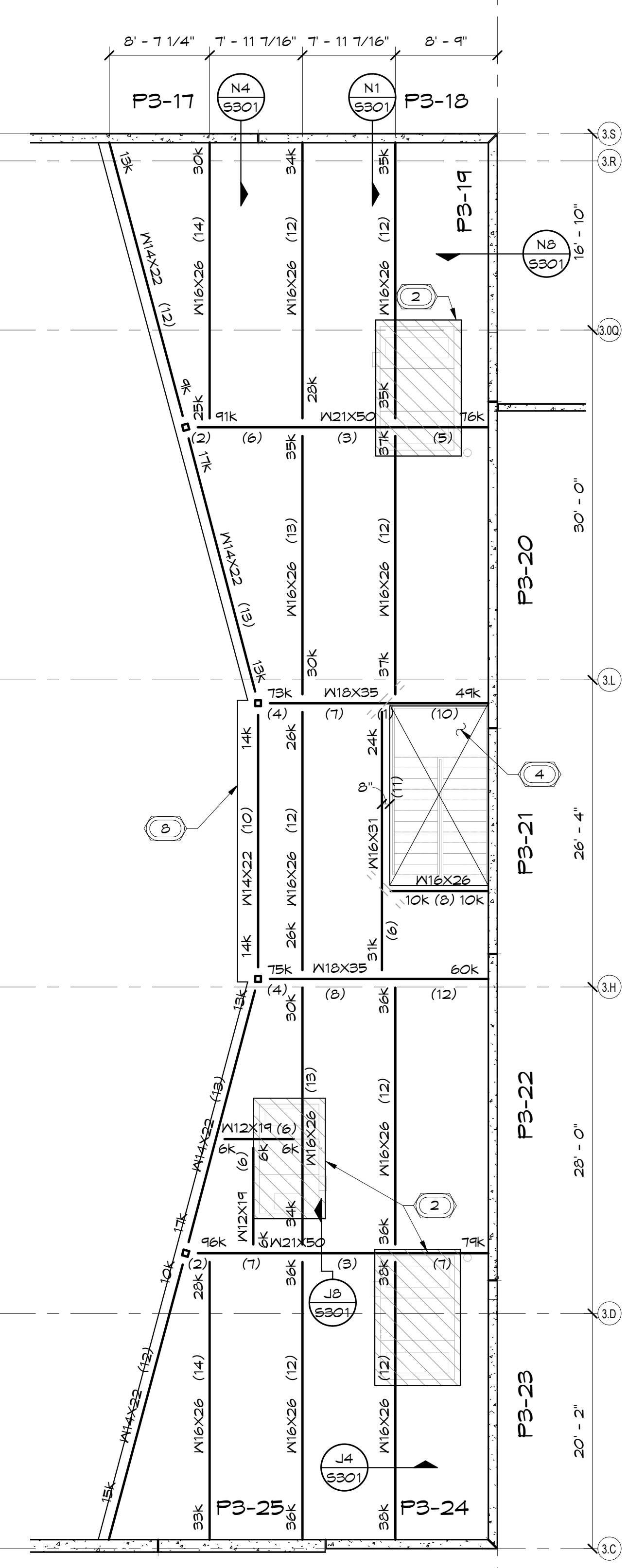
ROOF FRAMING PLAN KEY NOTES:

SEE SHEET S103.1A FOR ROOF FRAMING PLAN KEY NOTES.



A1 BUILDING 3 AREA 3B - HIGH ROOF FRAMING PLAN

1/8" = 1'-0"

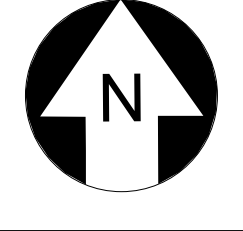


FLOOR FRAMING PLAN NOTES:

- SEE SHEET S102.1B FOR FLOOR FRAMING PLAN NOTES.
- T/ SLAB EL = 119'-0".
- T/ STEEL EL = 118'-5 1/2".

FLOOR FRAMING PLAN KEY NOTES:

SEE SHEET S102.1B FOR FLOOR FRAMING PLAN KEY NOTES.



A15 BUILDING 3 AREA 3B - MEZZANINE FMG PLAN

1/8" = 1'-0"

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Florida Professional Engineer No. 65509

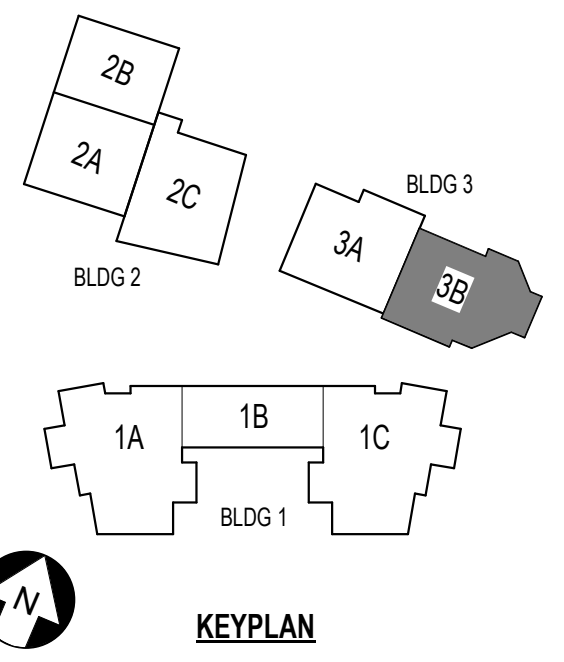
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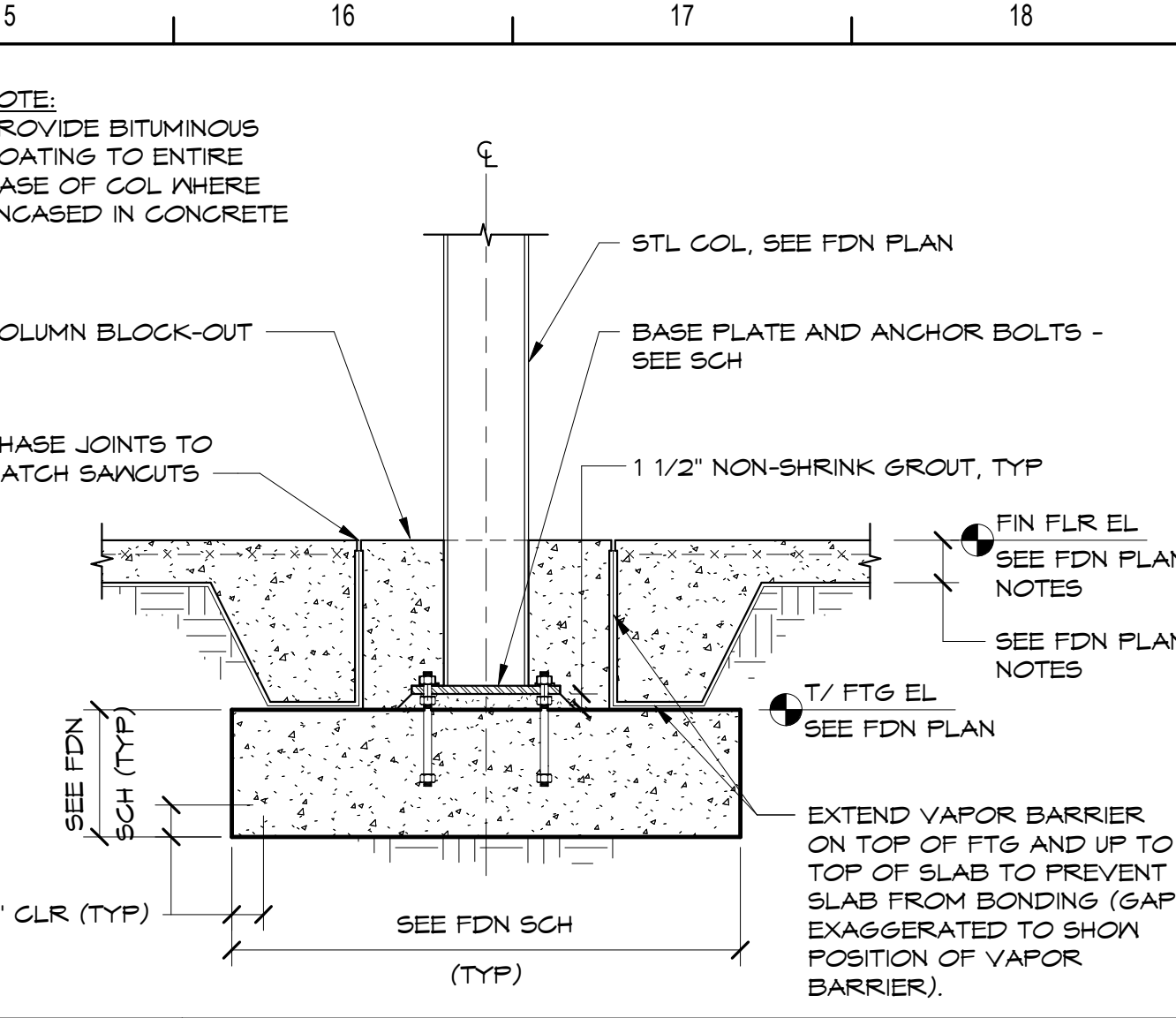
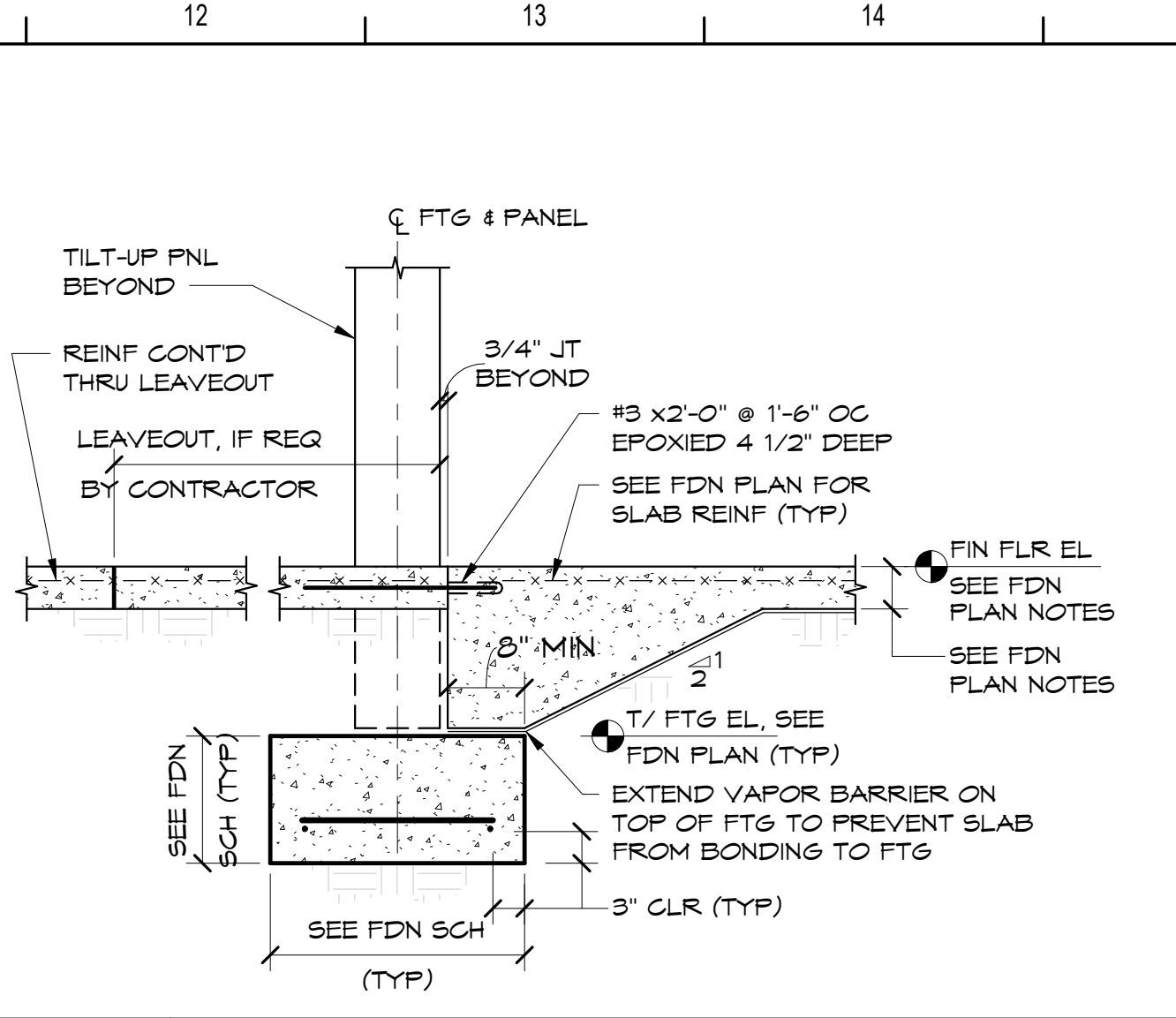
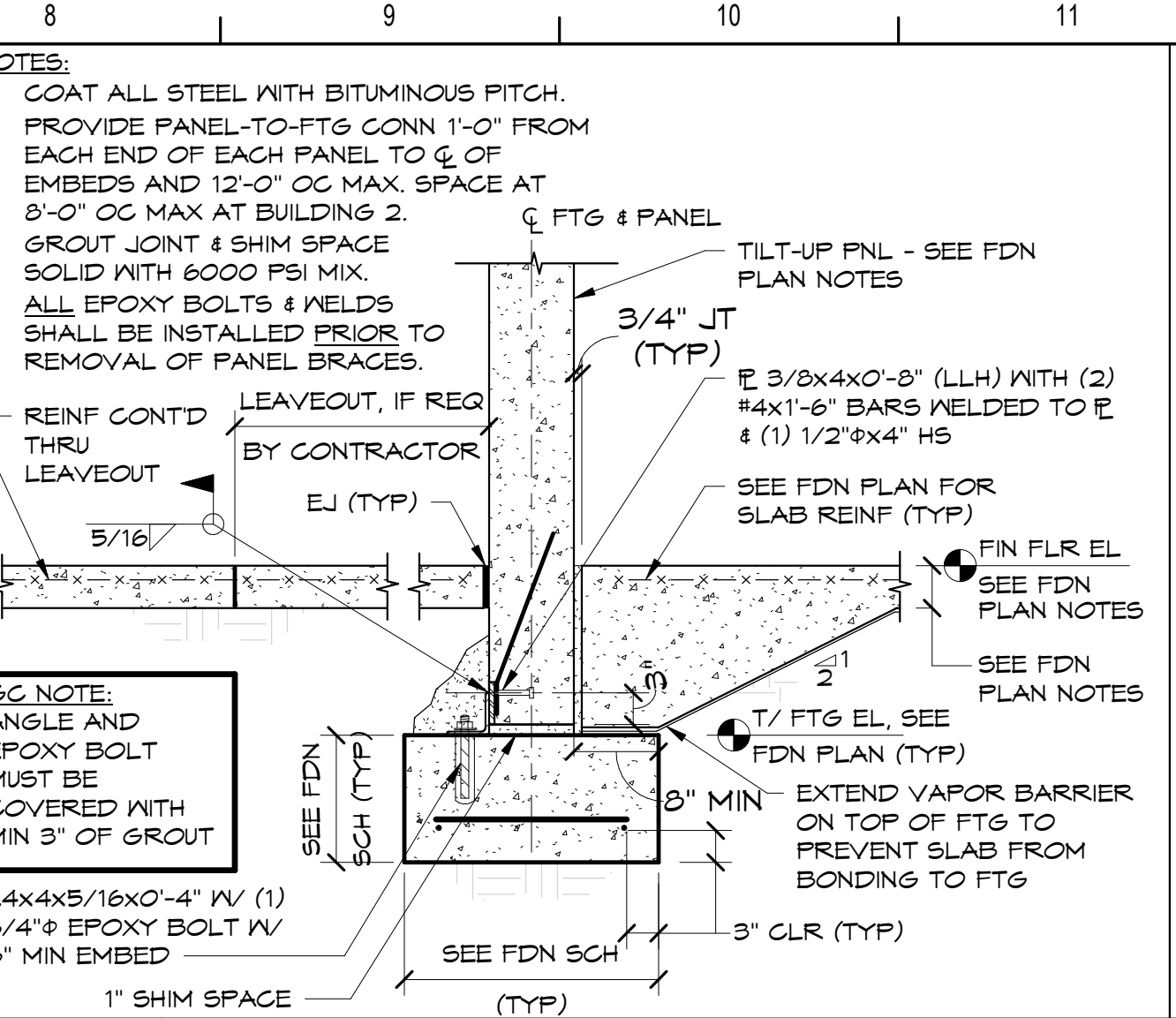
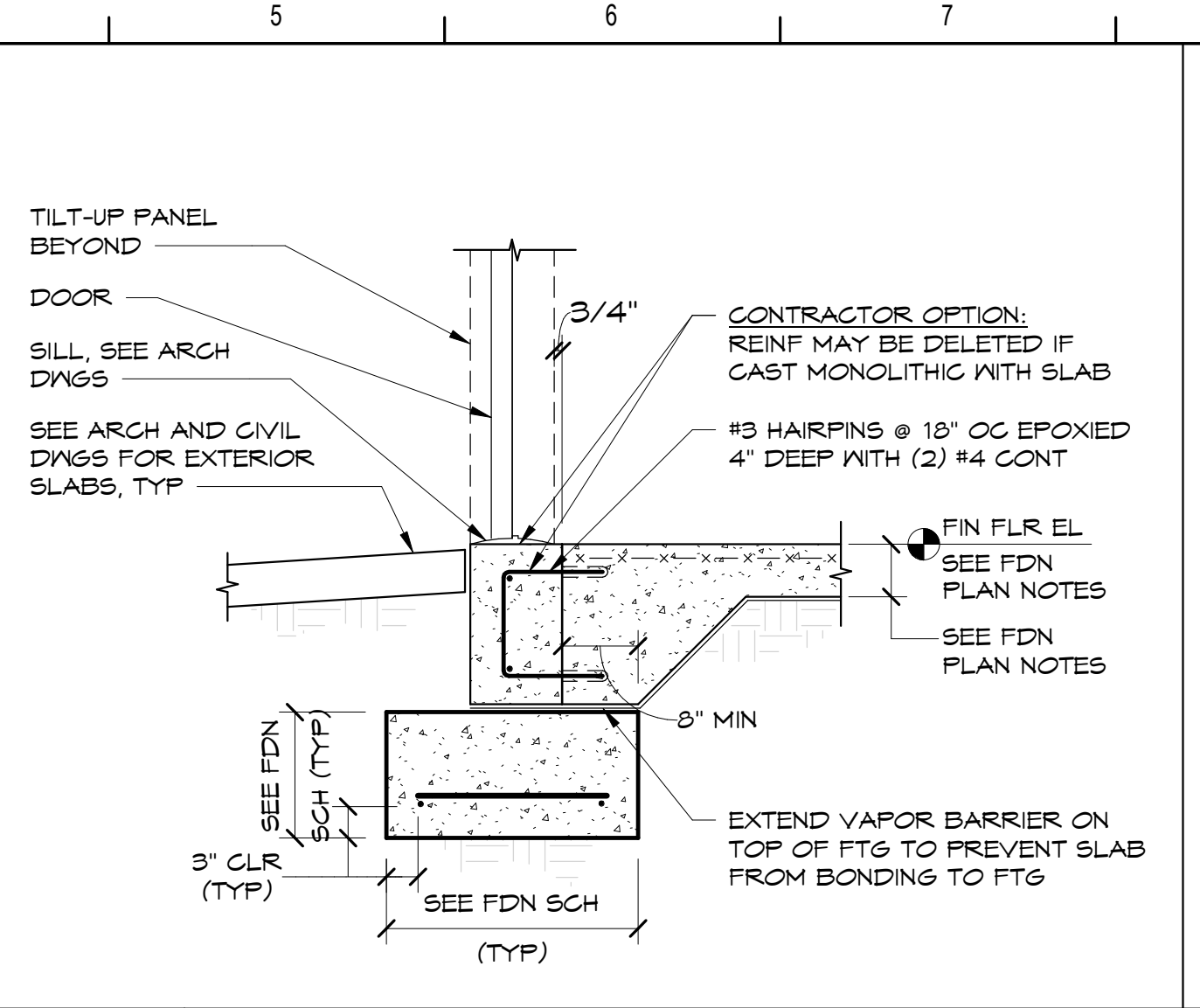
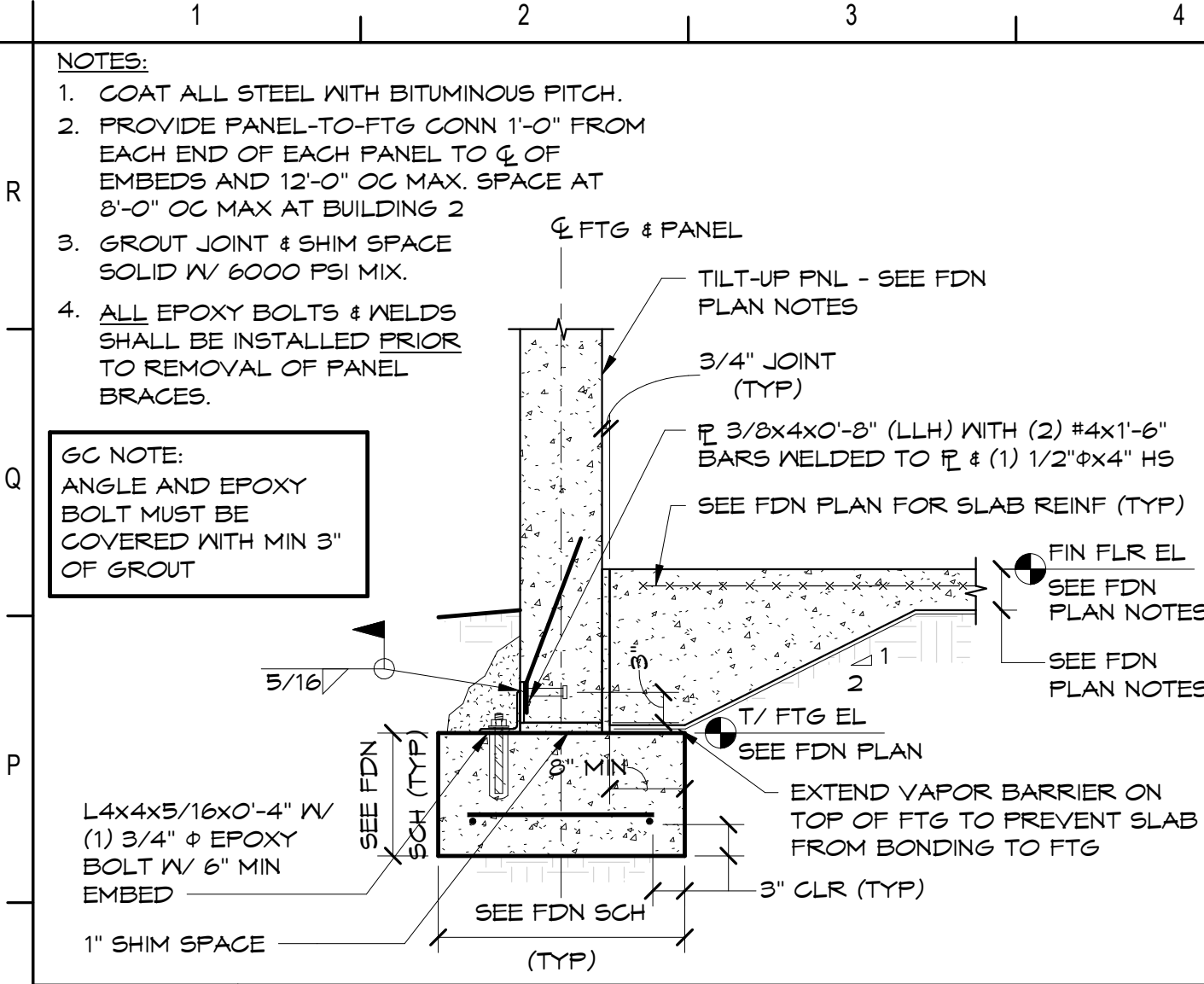
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BUILDING 3 AREA 3B - ROOF FRAMING PLAN & MEZZANINE FLOOR FRAMING PLAN

S103.3B



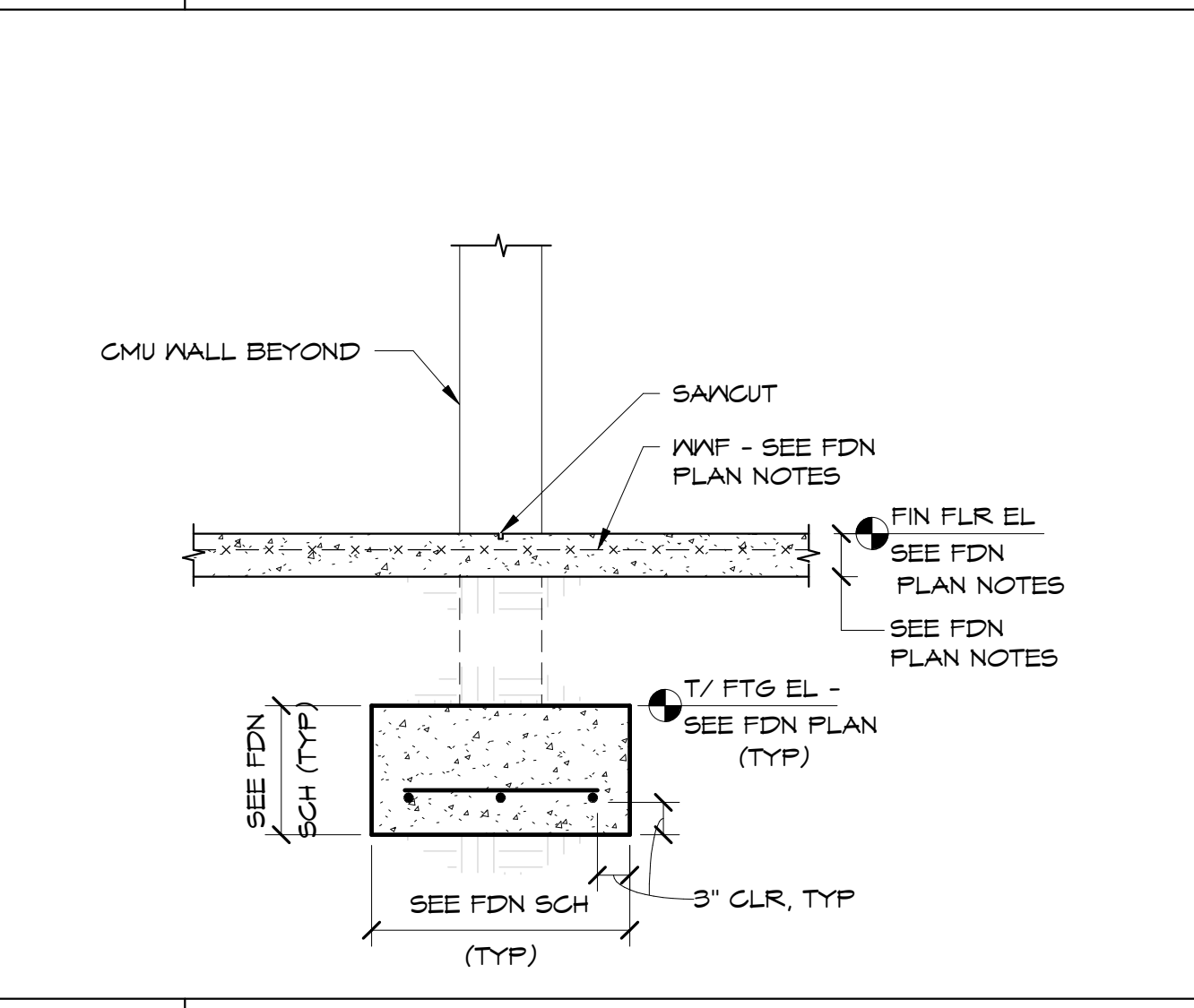
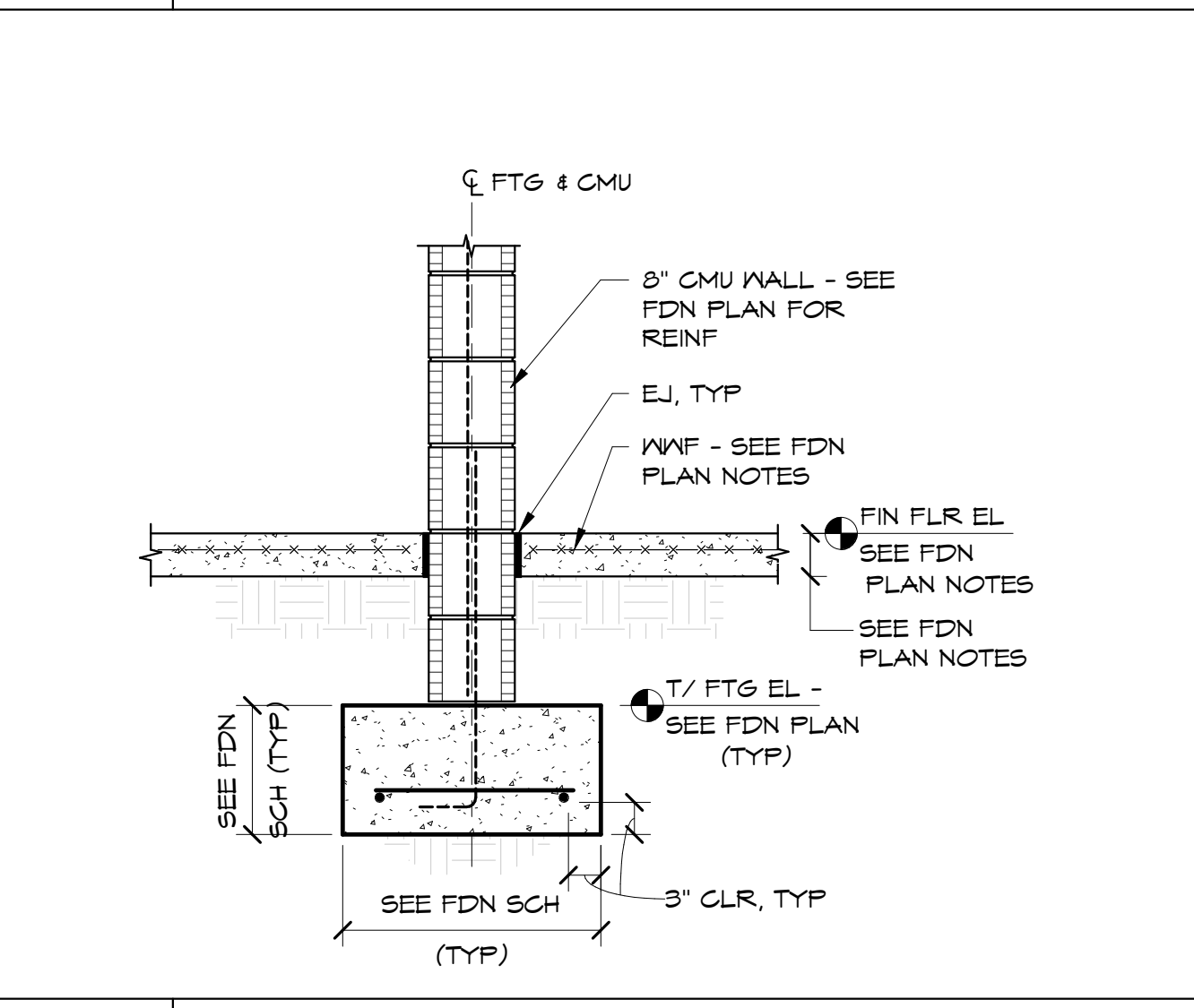
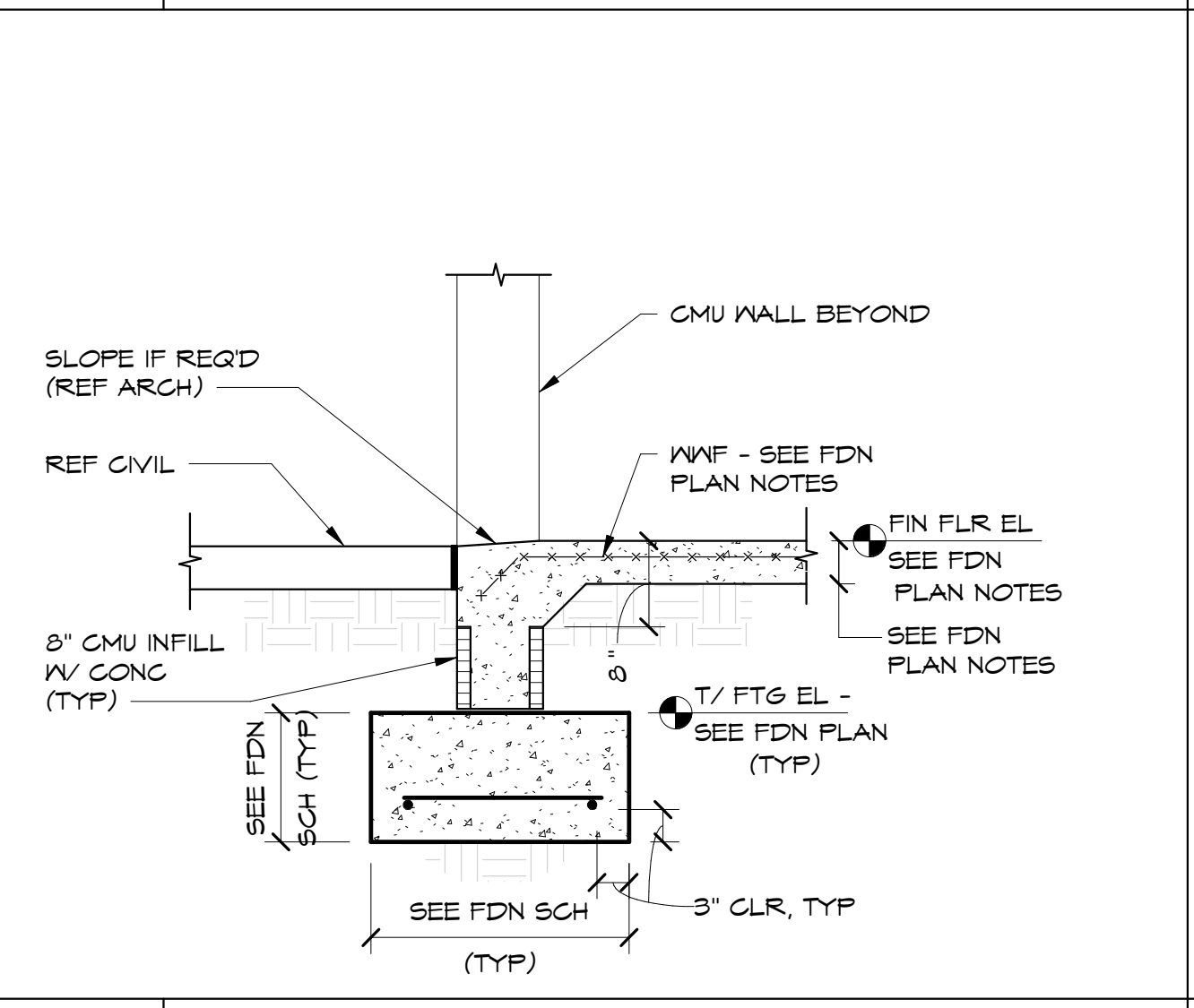
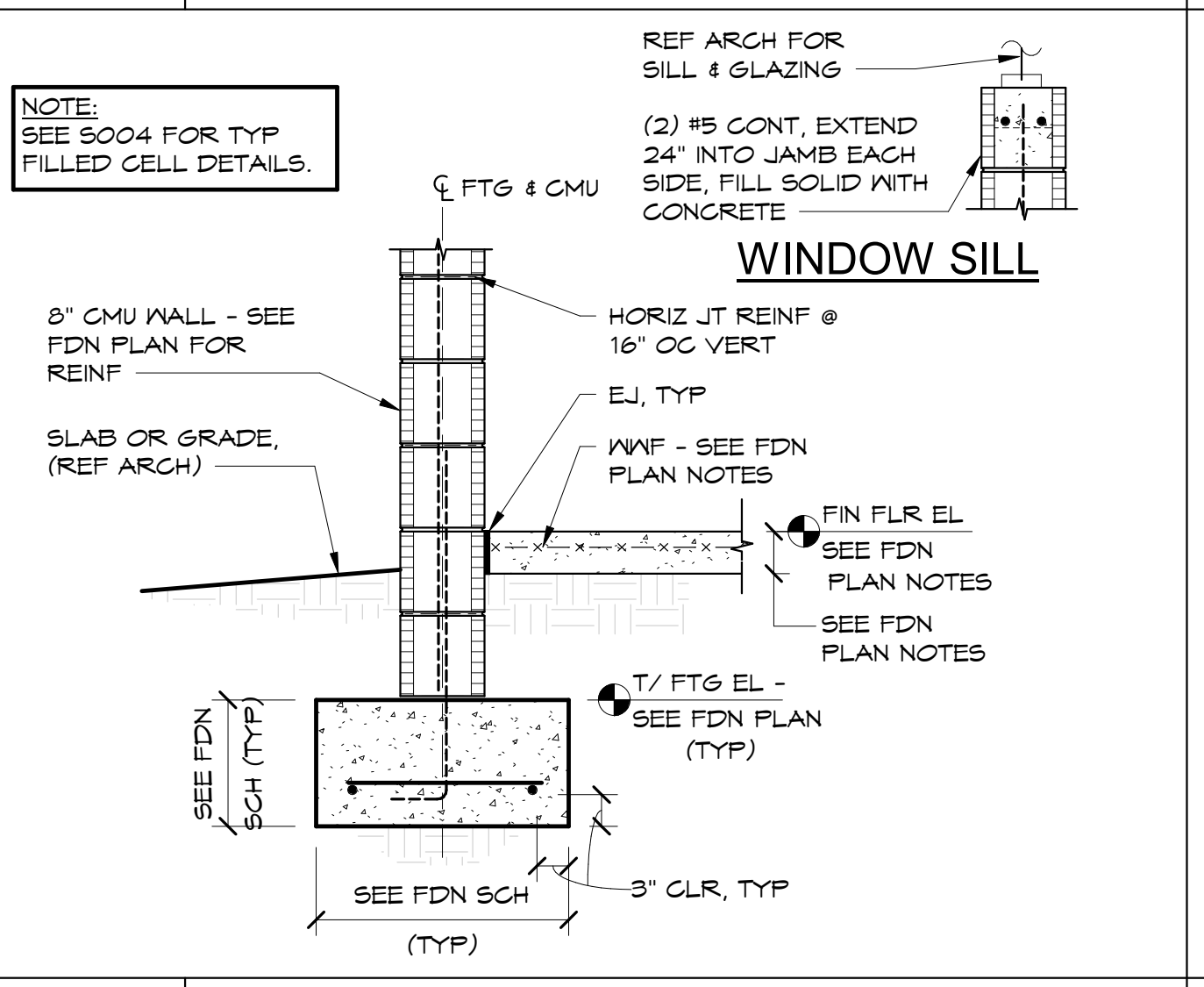
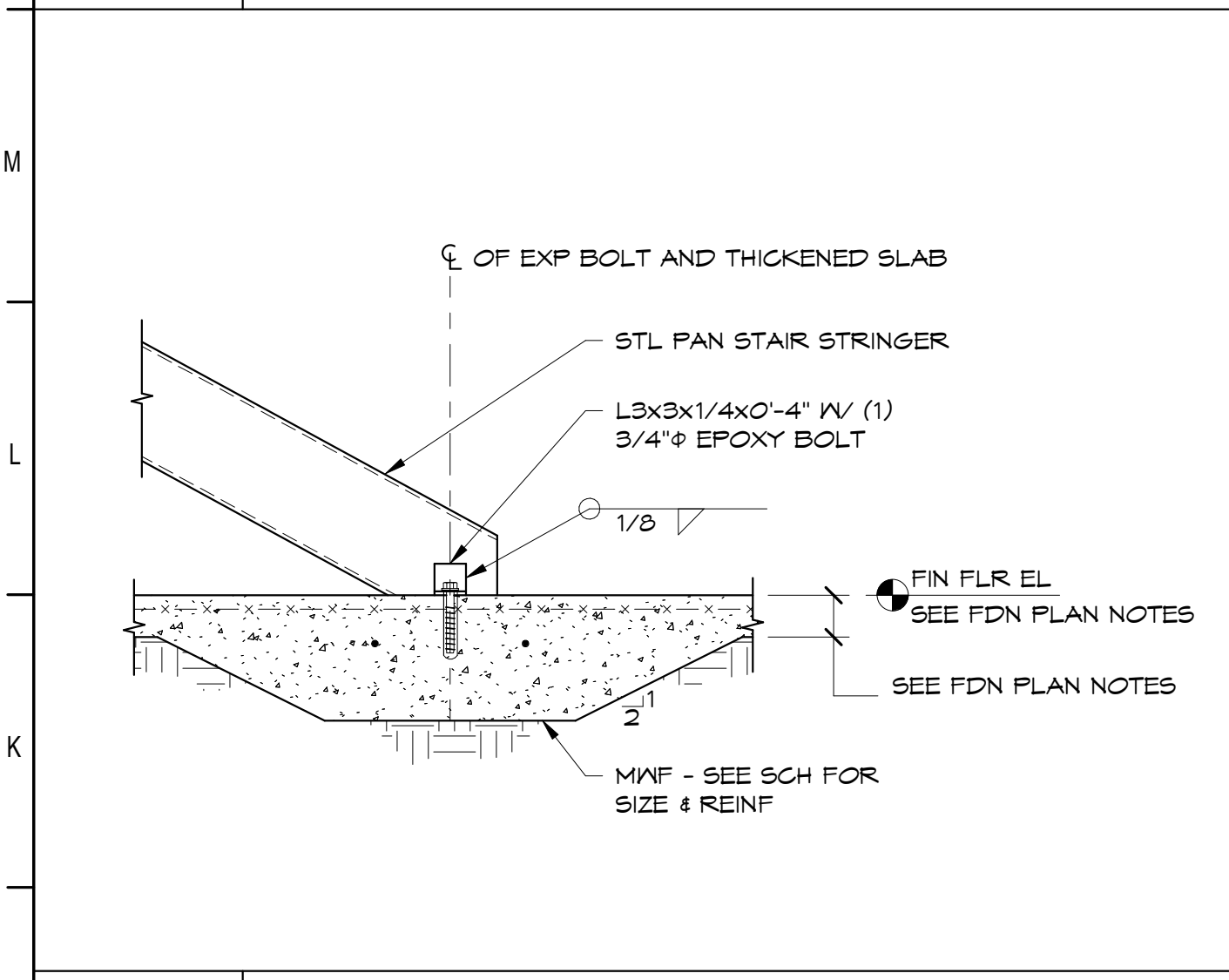
N1 SECTION @ TILT-UP PANEL FTG
3/4" = 1'-0"

N4 SECTION @ DOOR OPENING
3/4" = 1'-0"

N8 SECTION @ INT TILT-UP PANEL FTG
3/4" = 1'-0"

N12 SECTION @ INT TILT-UP PNL OPENING
3/4" = 1'-0"

N15 SECTION @ INT STL COL FTG
3/4" = 1'-0"



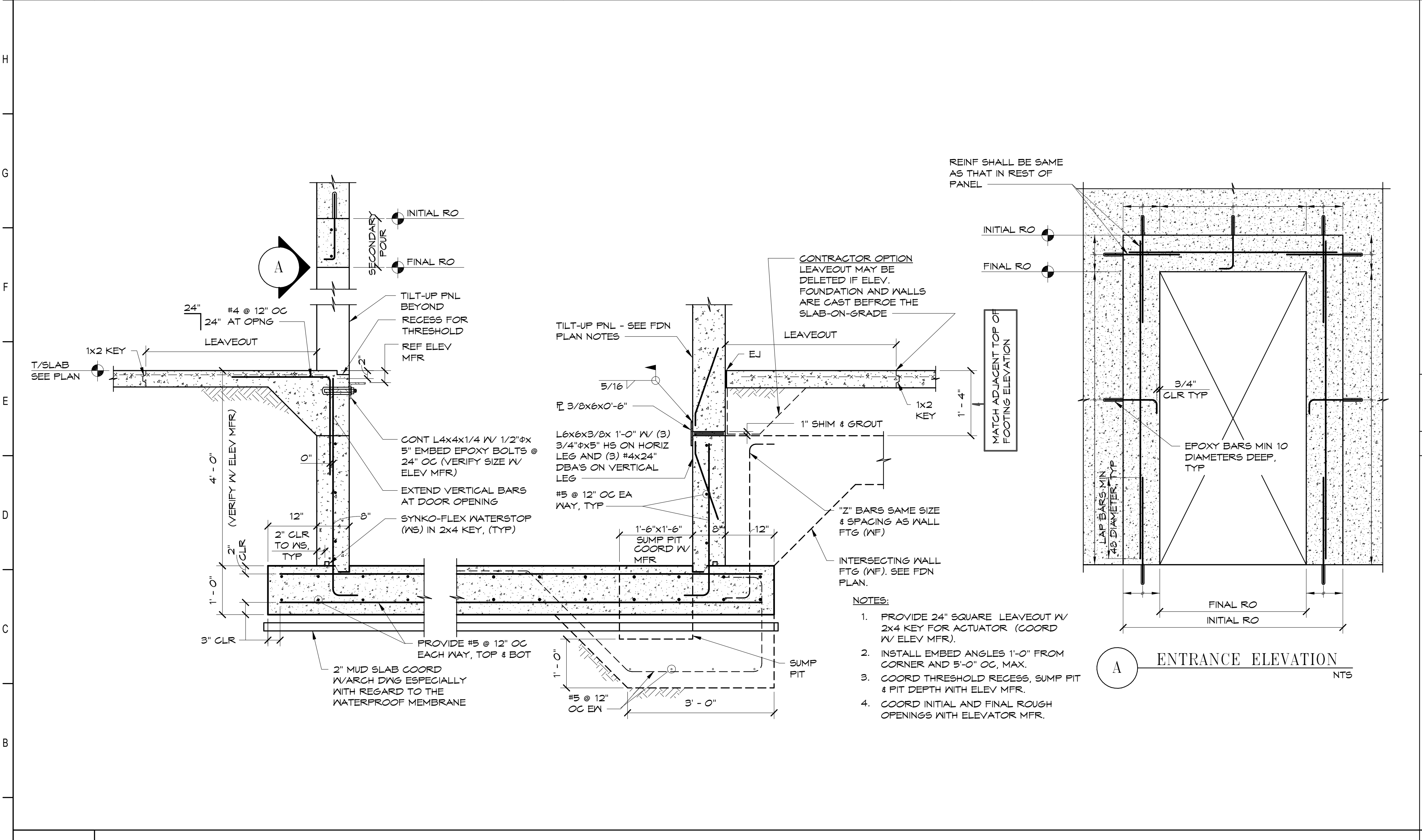
J1 SECTION @ STAIR STRINGER CONN TO SLAB
3/4" = 1'-0"

J4 SECTION @ EXTERIOR CMU WALL
3/4" = 1'-0"

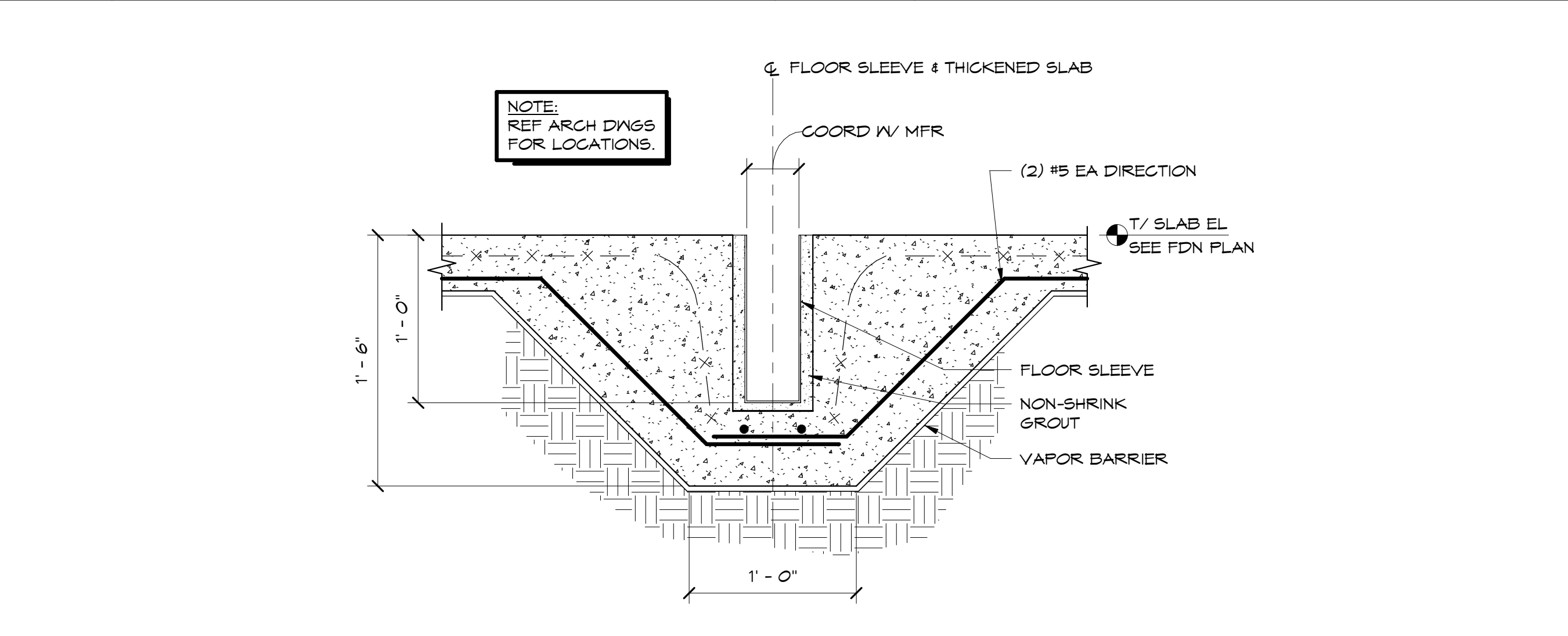
J8 SECTION @ EXTERIOR CMU WALL OPNG
3/4" = 1'-0"

J12 SECTION @ INTERIOR CMU WALL
3/4" = 1'-0"

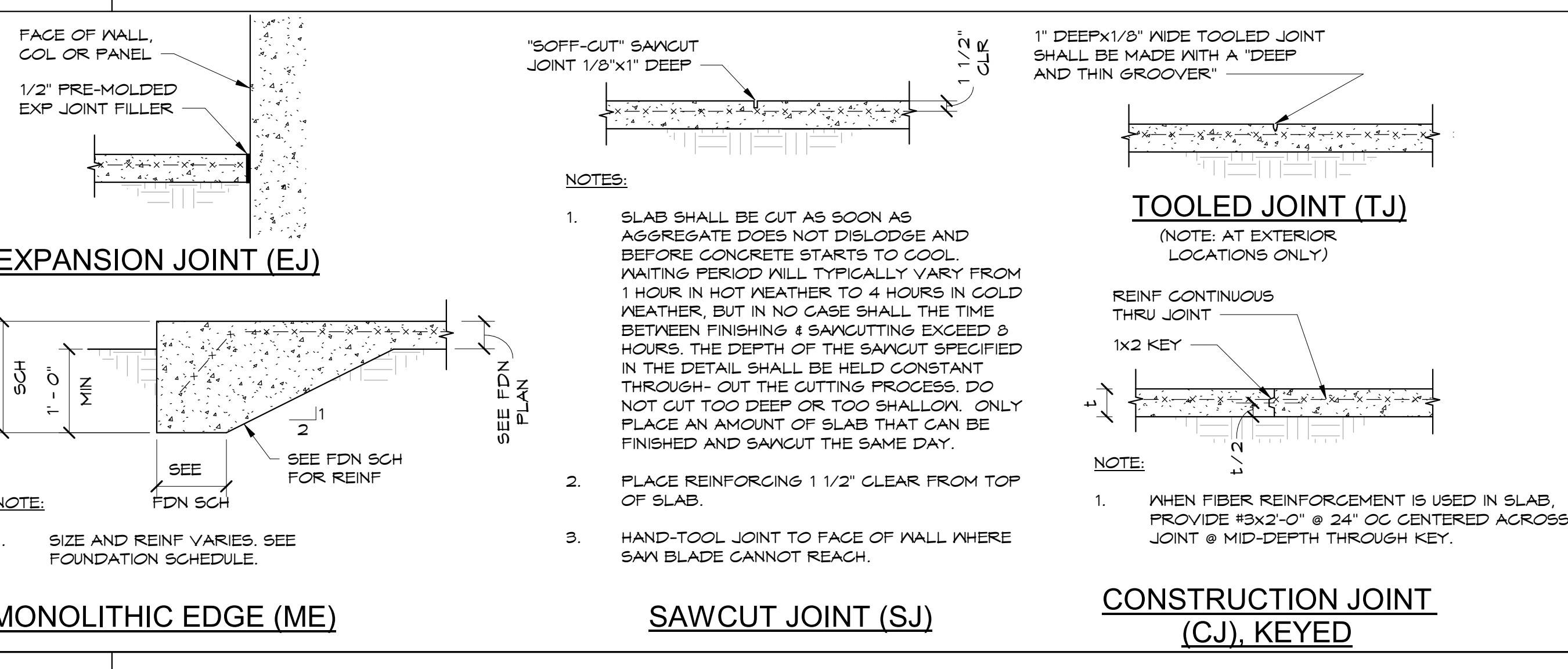
J15 SECTION @ INTERIOR CMU WALL OPNG
3/4" = 1'-0"



A1 SECTION @ ELEVATOR PIT
3/4" = 1'-0"



E12 DETAIL - VOLLEY BALL FLOOR INSERT @ GYM
1 1/2" = 1'-0"



A12 SLAB-ON-GRADE DETAILS (FOOT TRAFFIC)
3/4" = 1'-0"

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LUIS F. BEDOYA, P.E.
Florida Professional Engineer No. 65509

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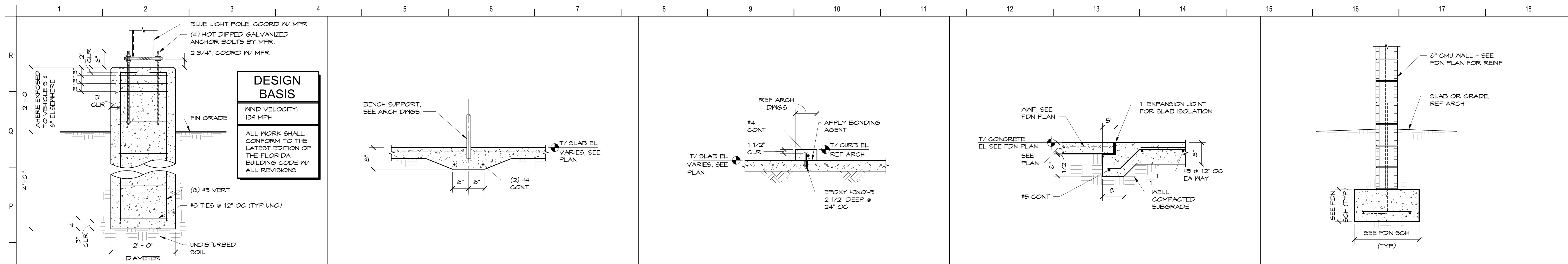
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FOUNDATION SECTIONS & DETAILS

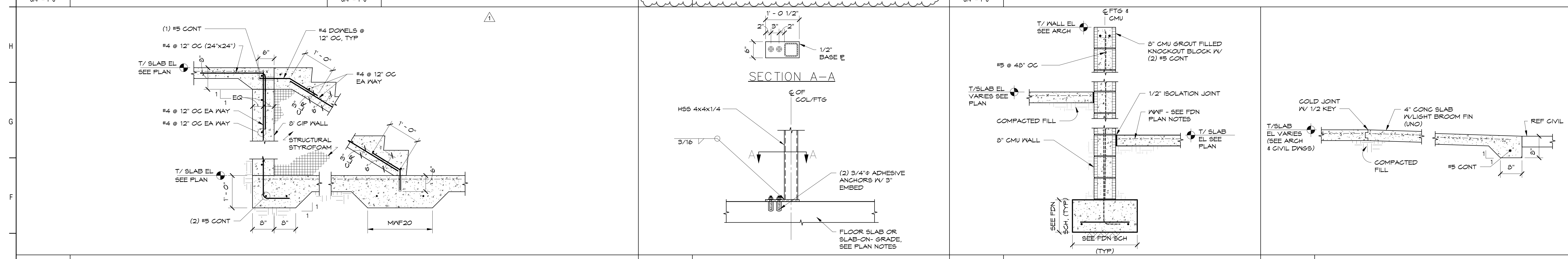
S201



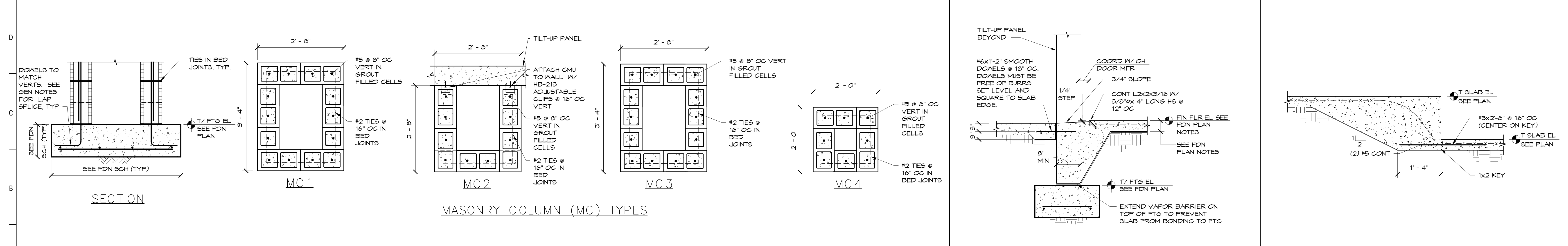
N1 SECTION @ LIGHT POLE FDN 3/4" = 1'-0"
N4 SECTION @ LOCKER ROOM BENCH SUPPORT 3/4" = 1'-0"
N8 SECTION @ CURB 3/4" = 1'-0"
N12 ISOLATED SLAB @ MUSIC BLDG MECH ROOM 3/4" = 1'-0"
N15 SECTION @ STAIR FOUNDATION 3/4" = 1'-0"



J1 SECTION @ EXTERIOR WALKWAY 3/4" = 1'-0"
J4 SECTION @ OVERHEAD DOOR OPENING 3/4" = 1'-0"
J12 SECTION @ RAMP 3/4" = 1'-0"



E1 SECTION @ STAIRS @ AUDITORIUM 3/4" = 1'-0"
E8 SECTION @ HSS COL FOR SHIPS LADDER 3/4" = 1'-0"
E12 SECTION @ WALL 3/4" = 1'-0"
E15 SECTION @ EXTERIOR RAMP 3/4" = 1'-0"



SECTION A-A
MASONRY COLUMN (MC) TYPES
A1 MASONRY COLUMNS 3/4" = 1'-0"
A12 SECTION @ OVERHEAD DOOR OPENING 3/4" = 1'-0"
A15 SECTION @ RAISED SLAB 3/4" = 1'-0"

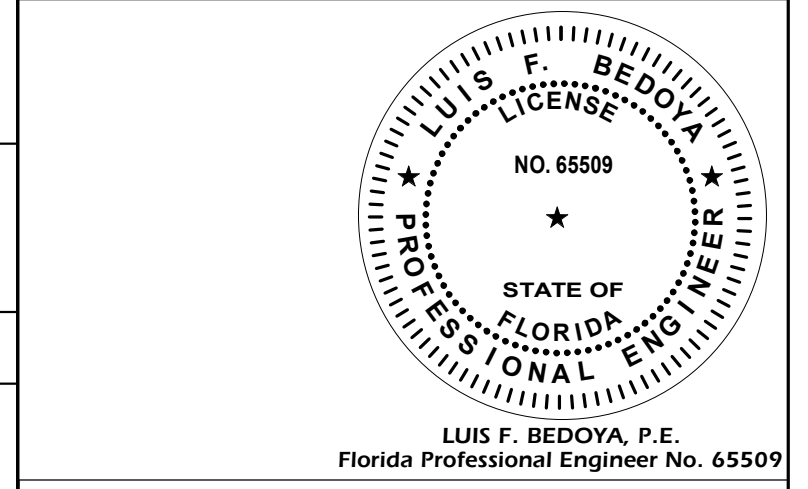
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MARK	DESCRIPTION	DATE
1	ISSUE FOR PERMIT	10/26/2022
2	REVISED	10/26/2022

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Port St. Lucie, FL

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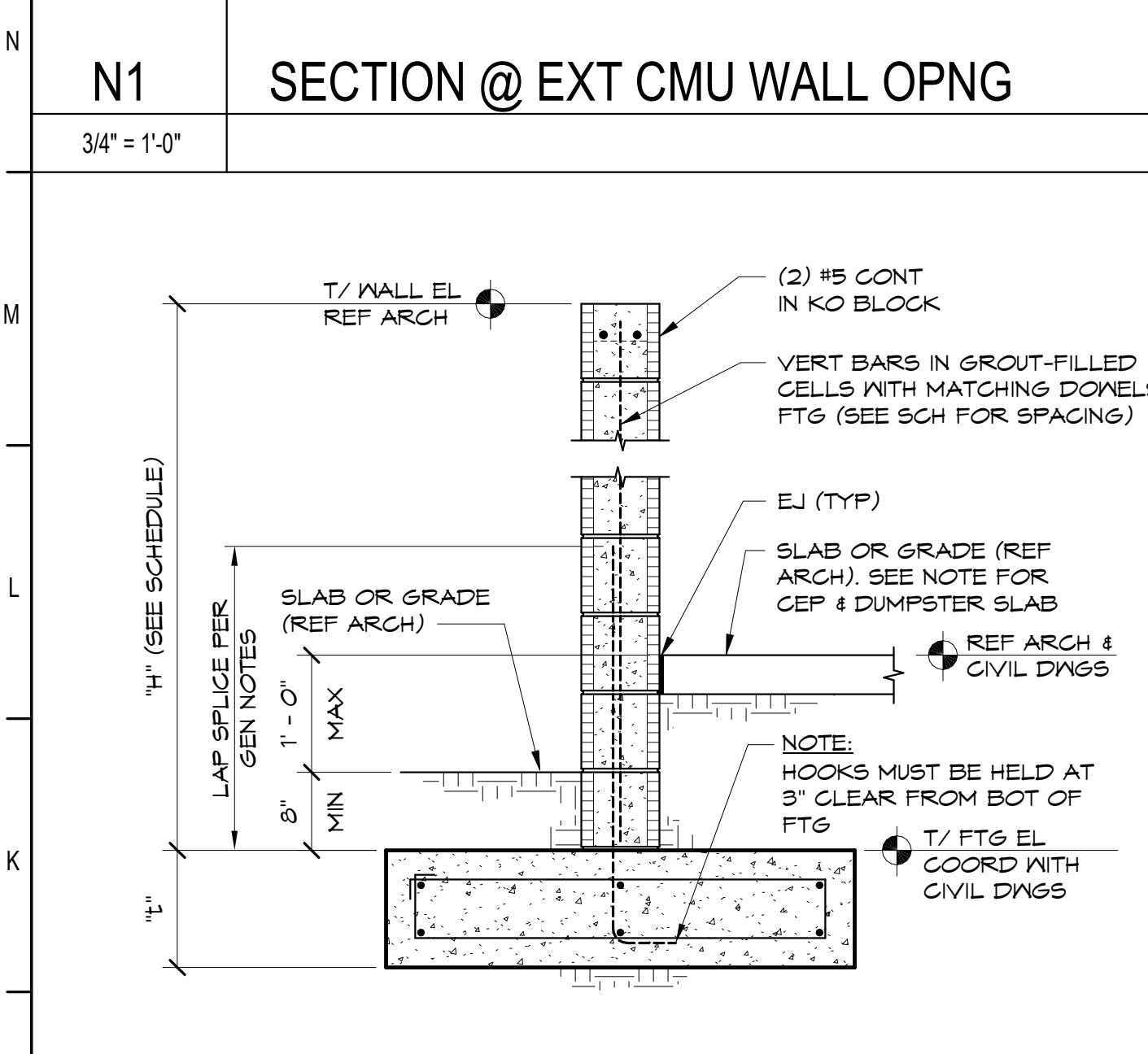
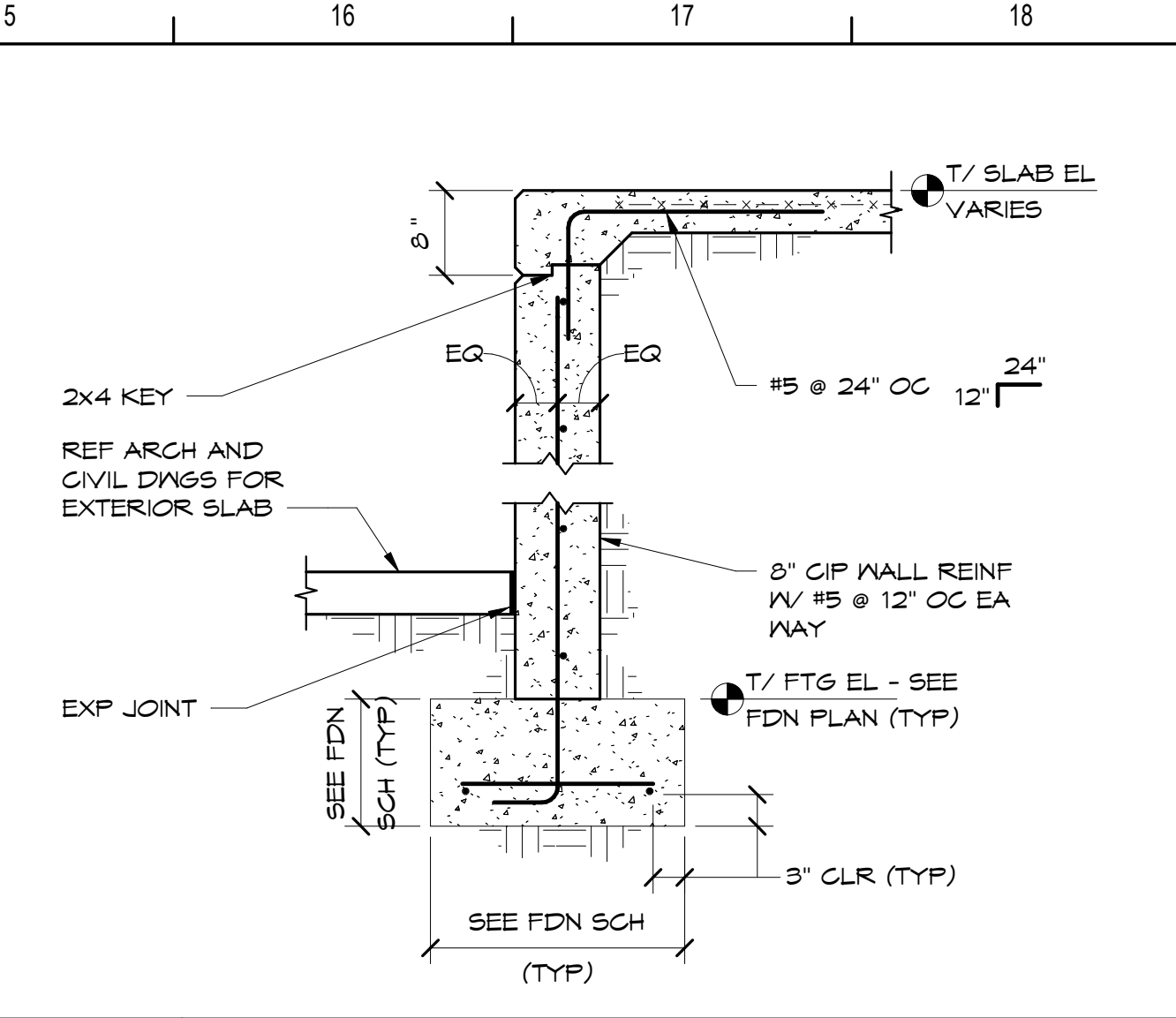
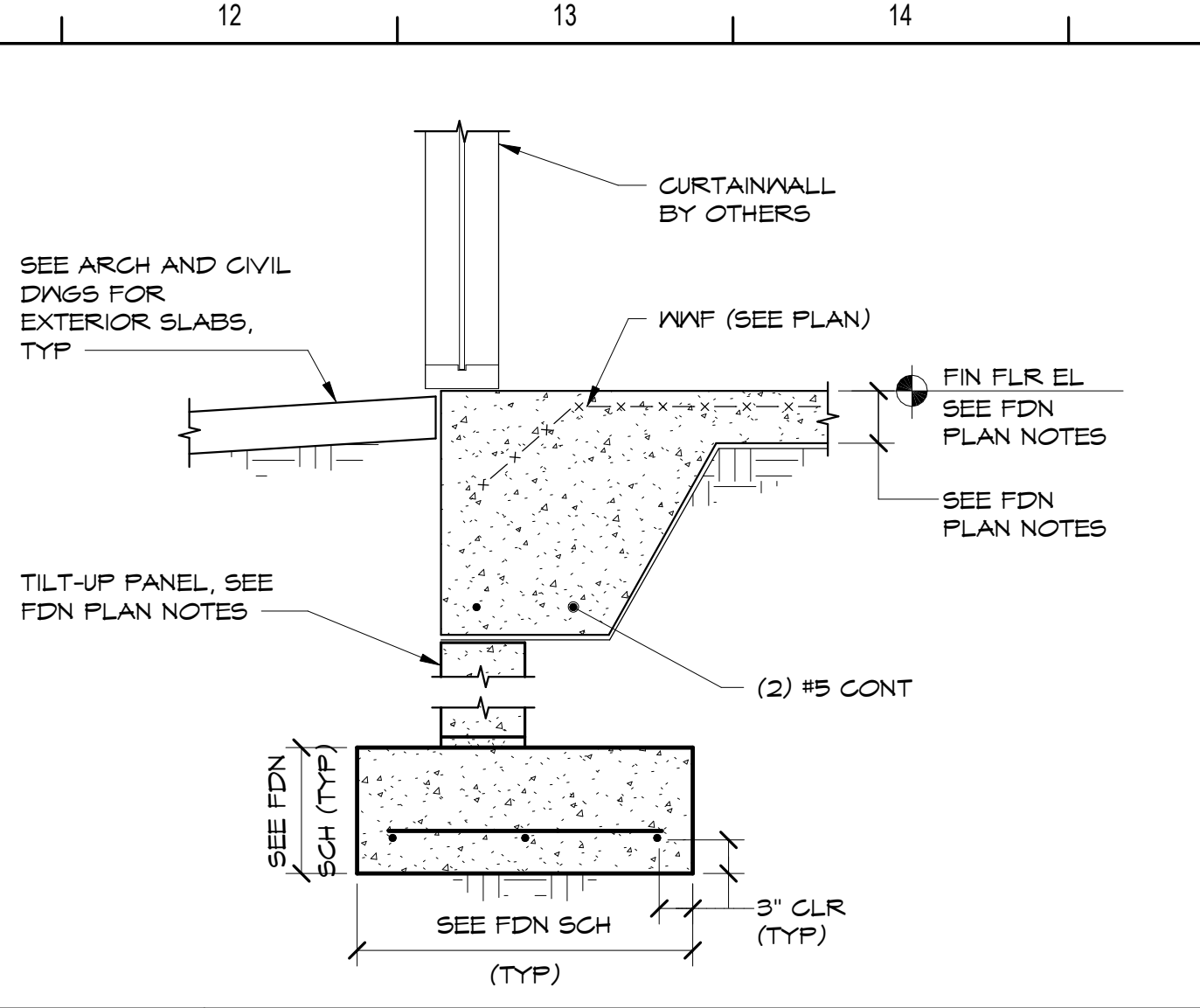
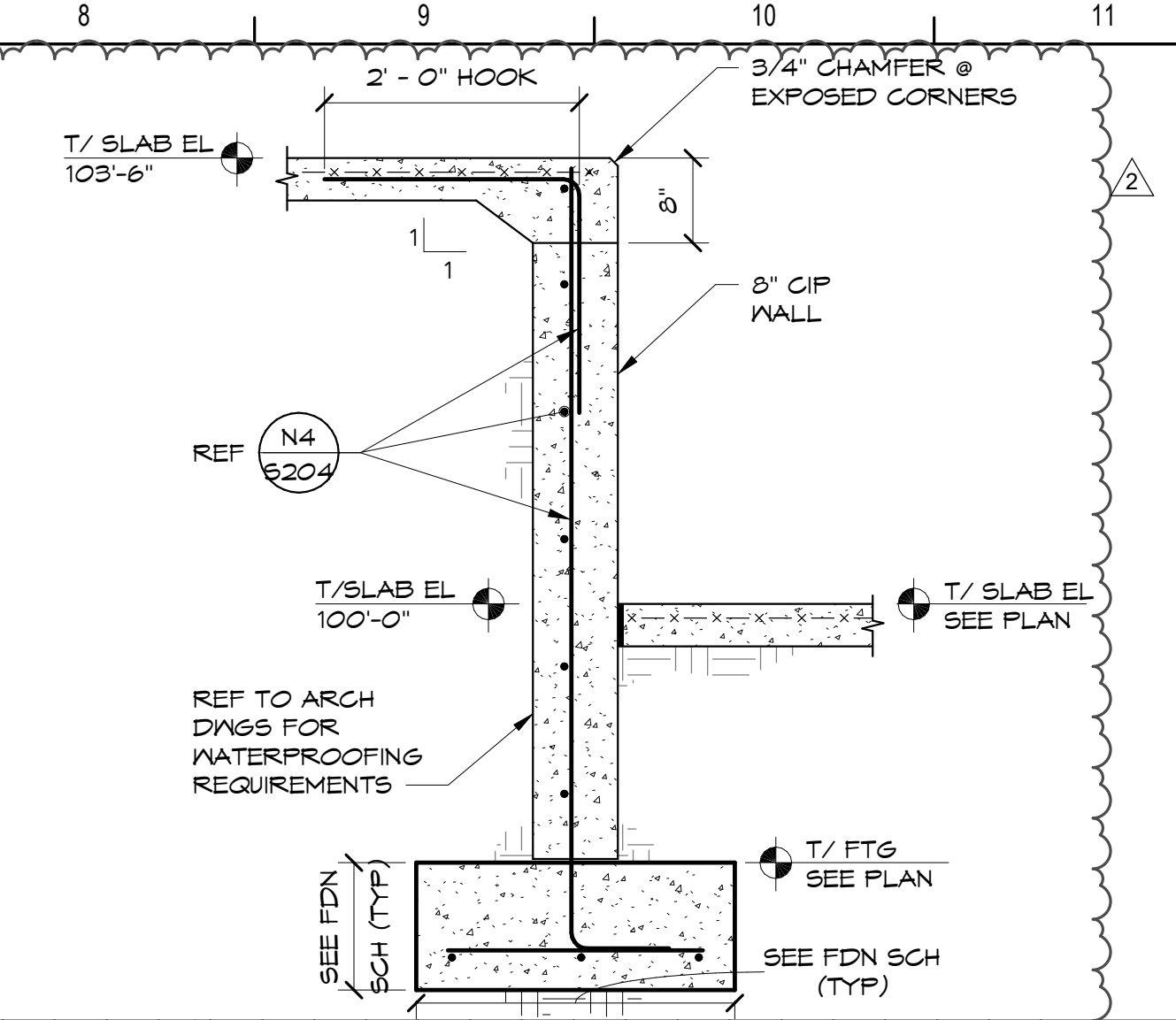
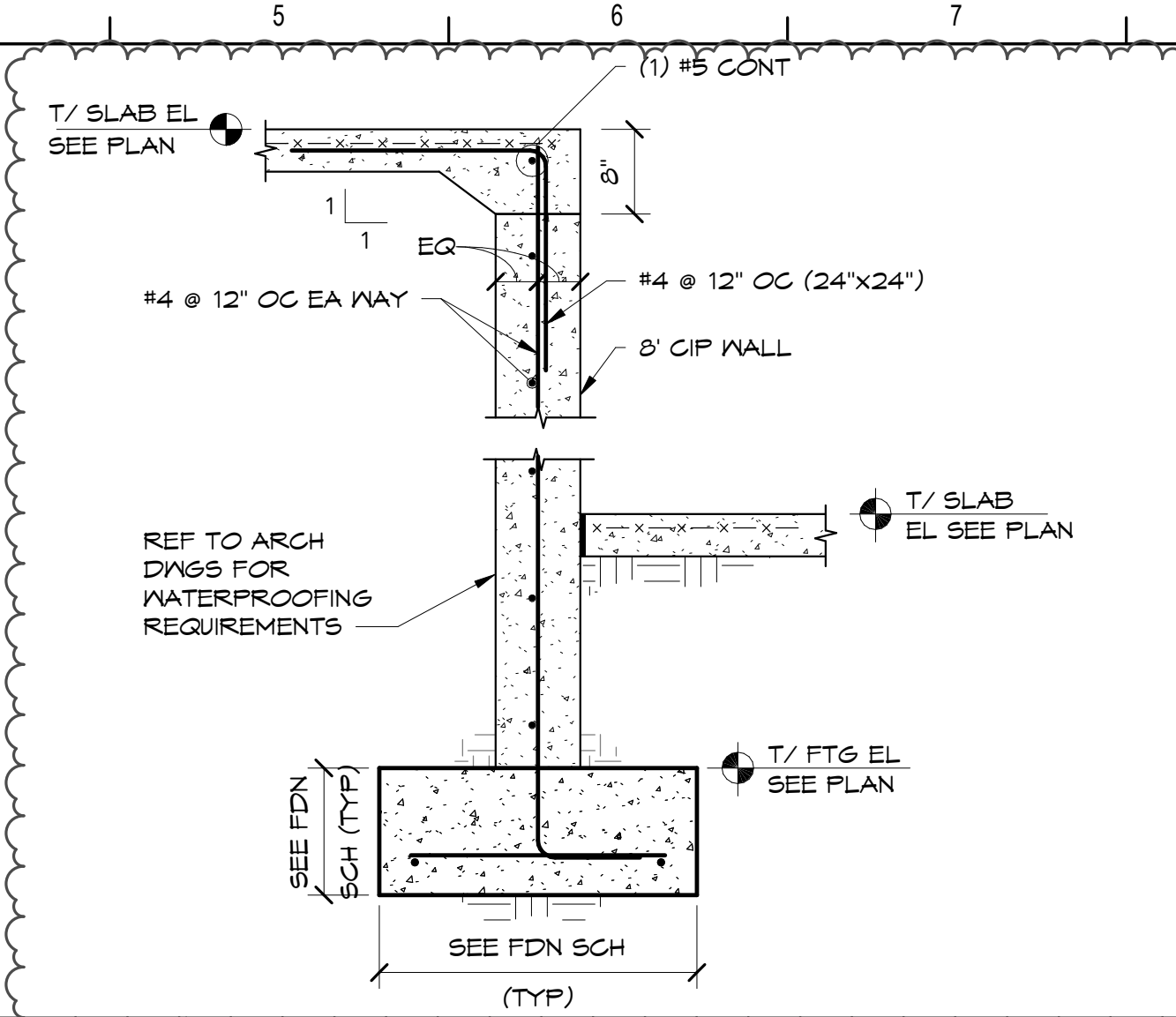
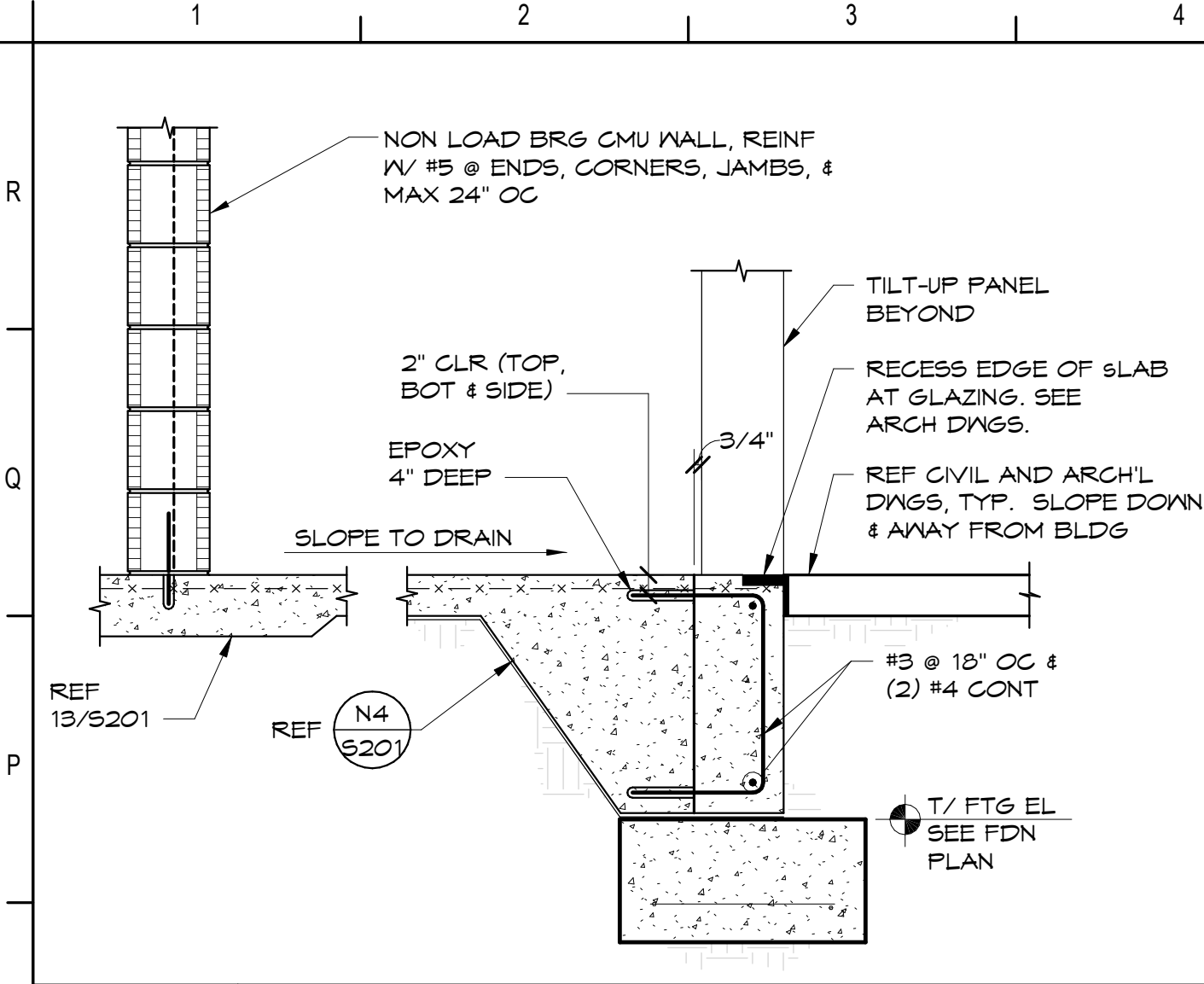
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ISSUE DATE: 12/16/2022
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FOUNDATION SECTIONS & DETAILS

S203



SITE WALL REINFORCING SCHEDULE

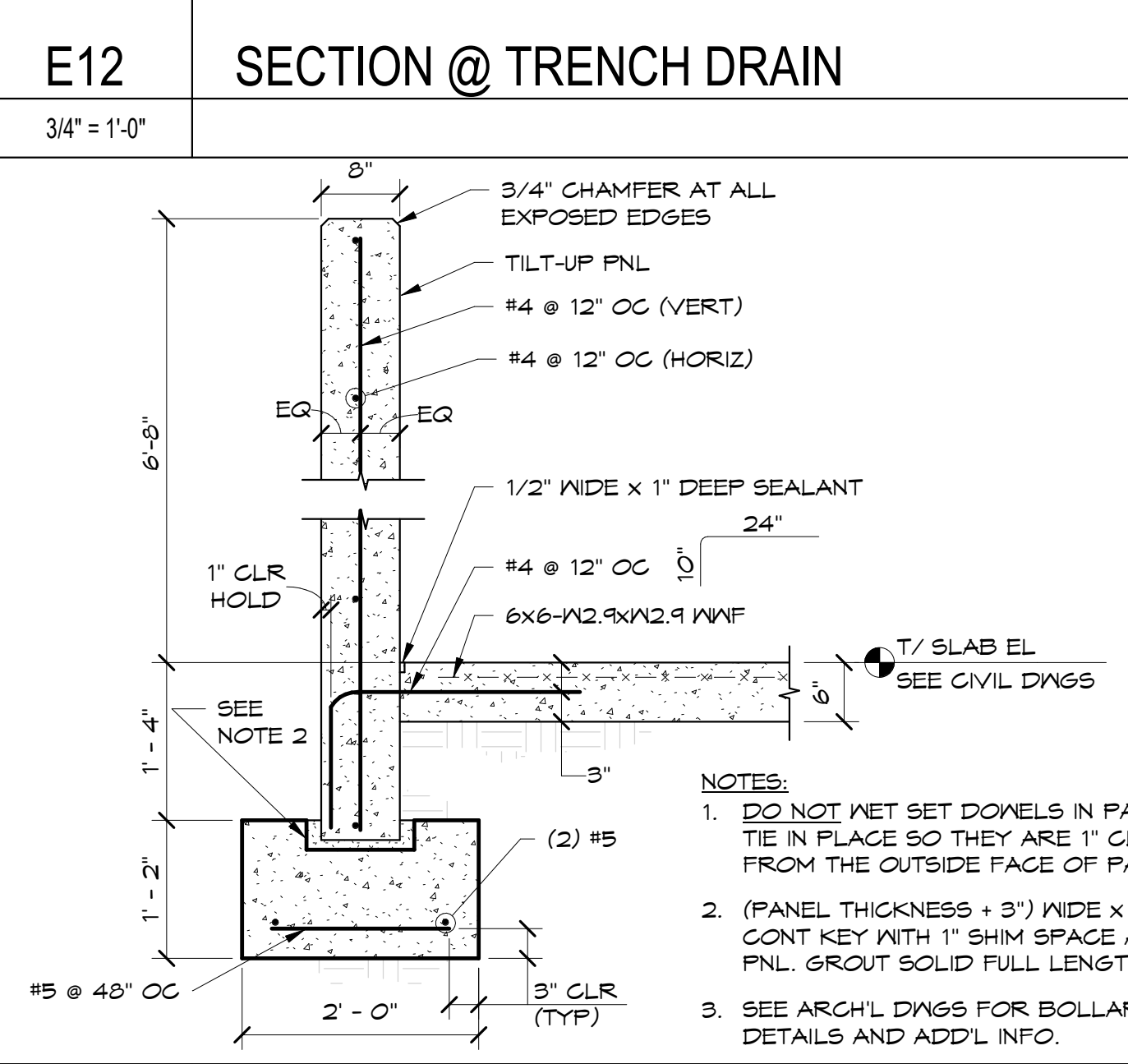
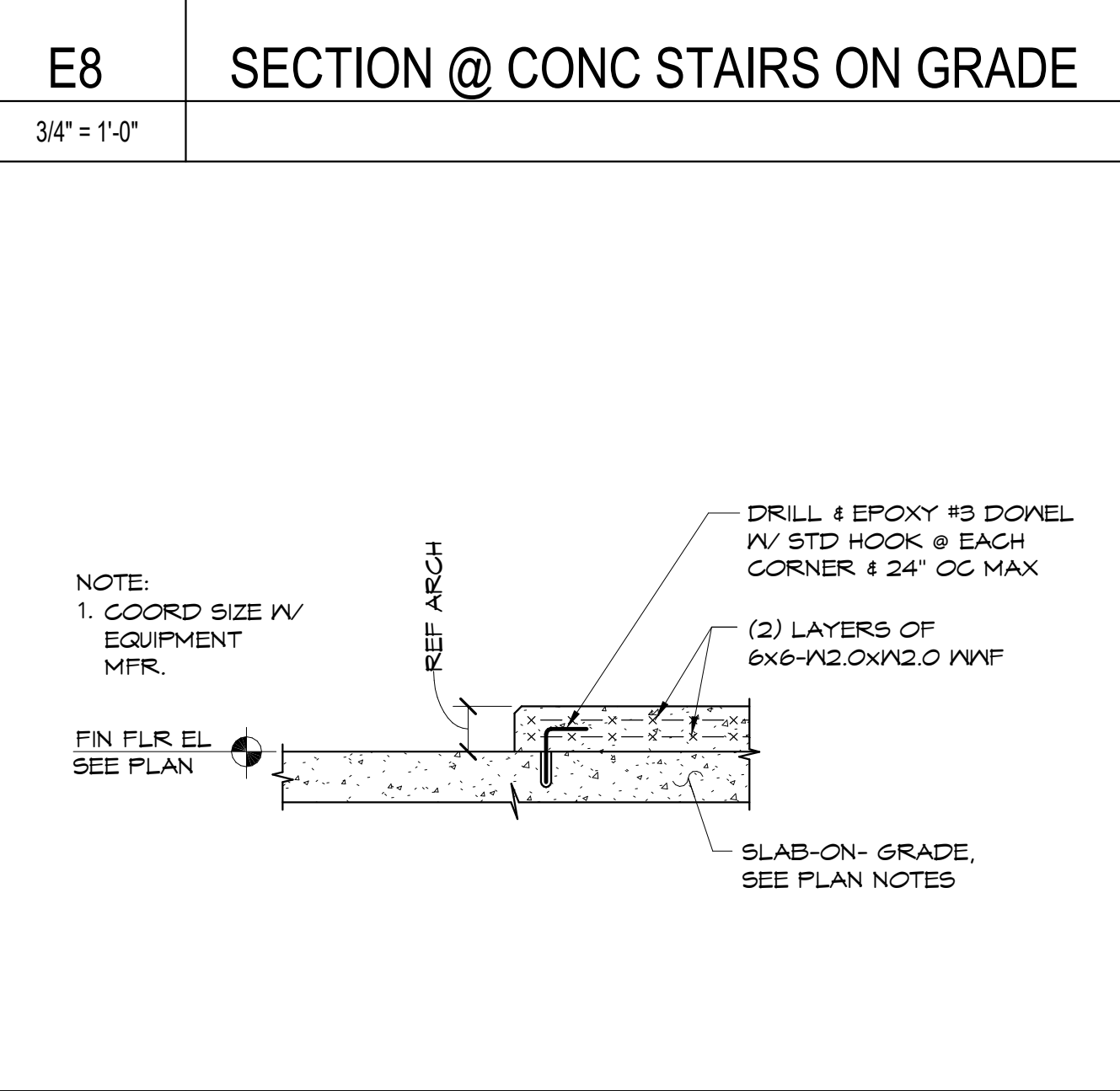
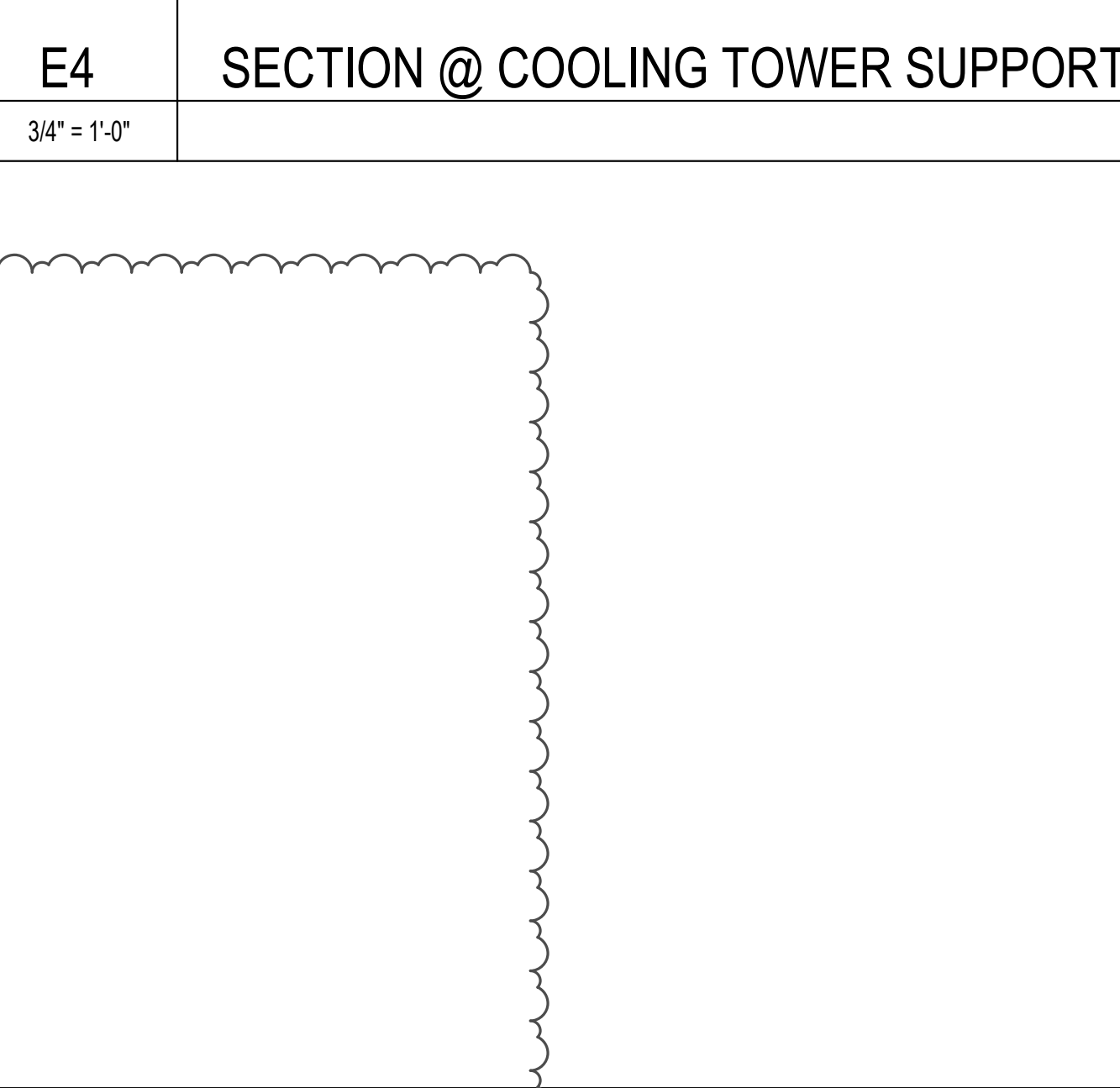
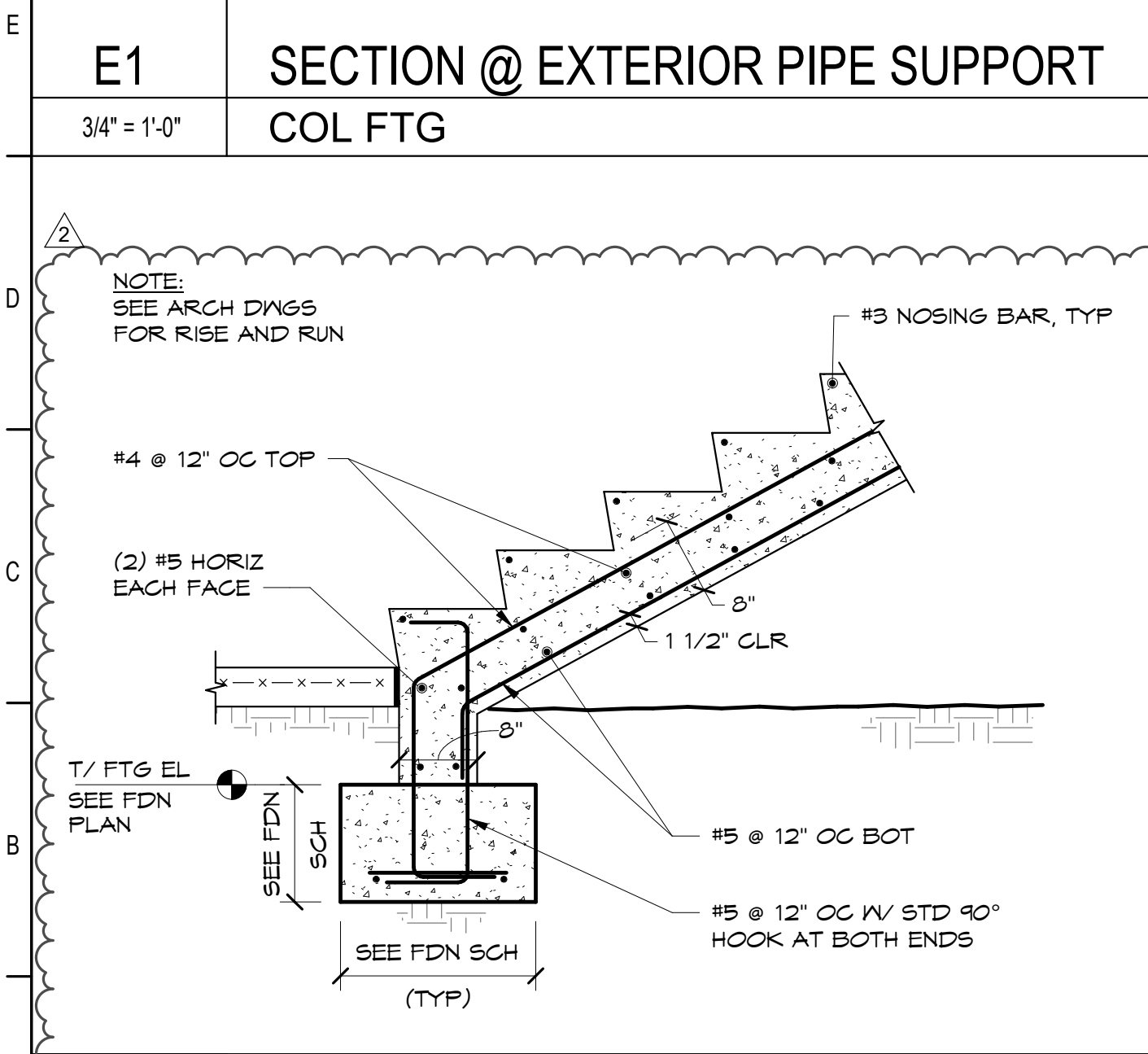
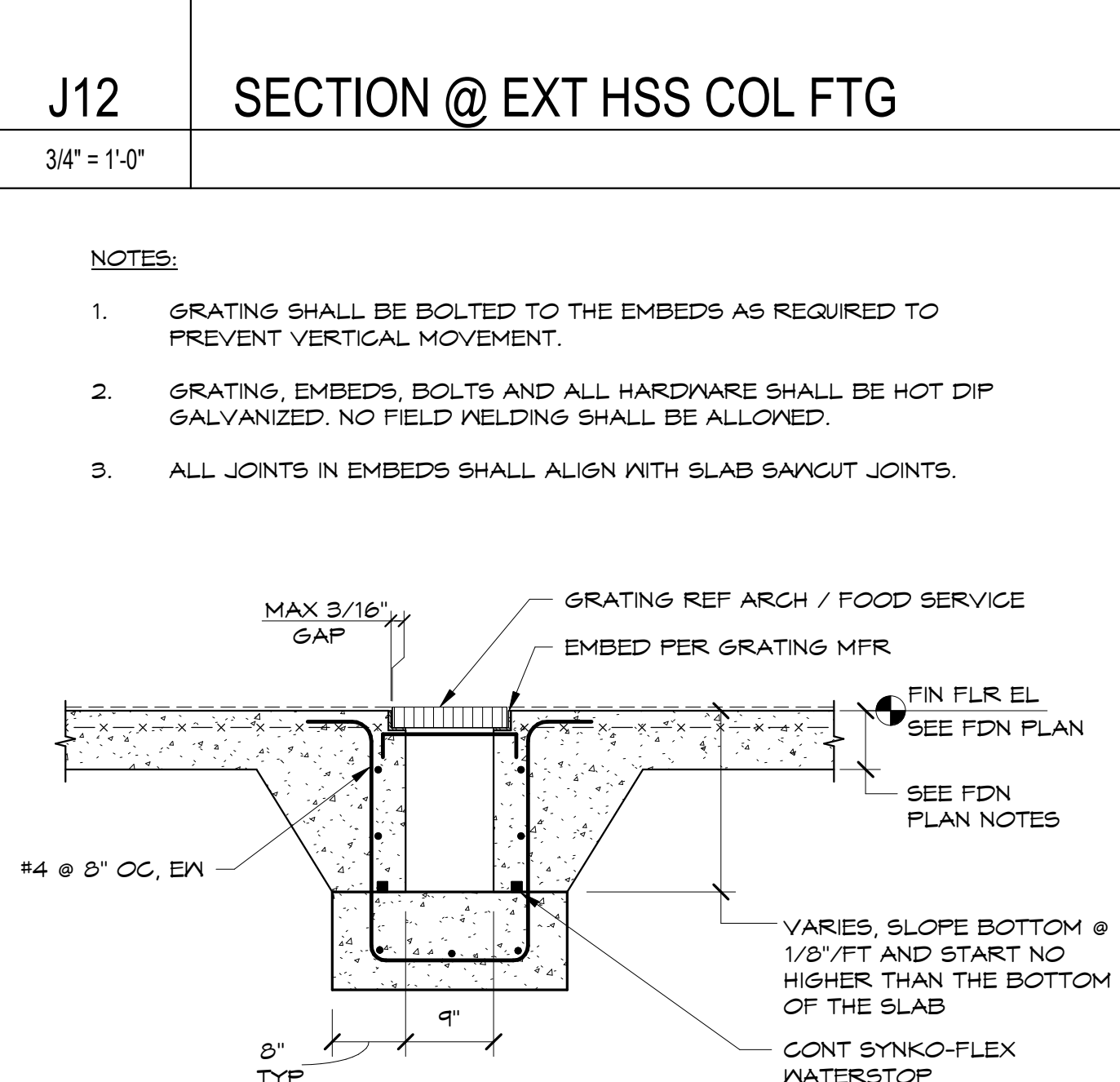
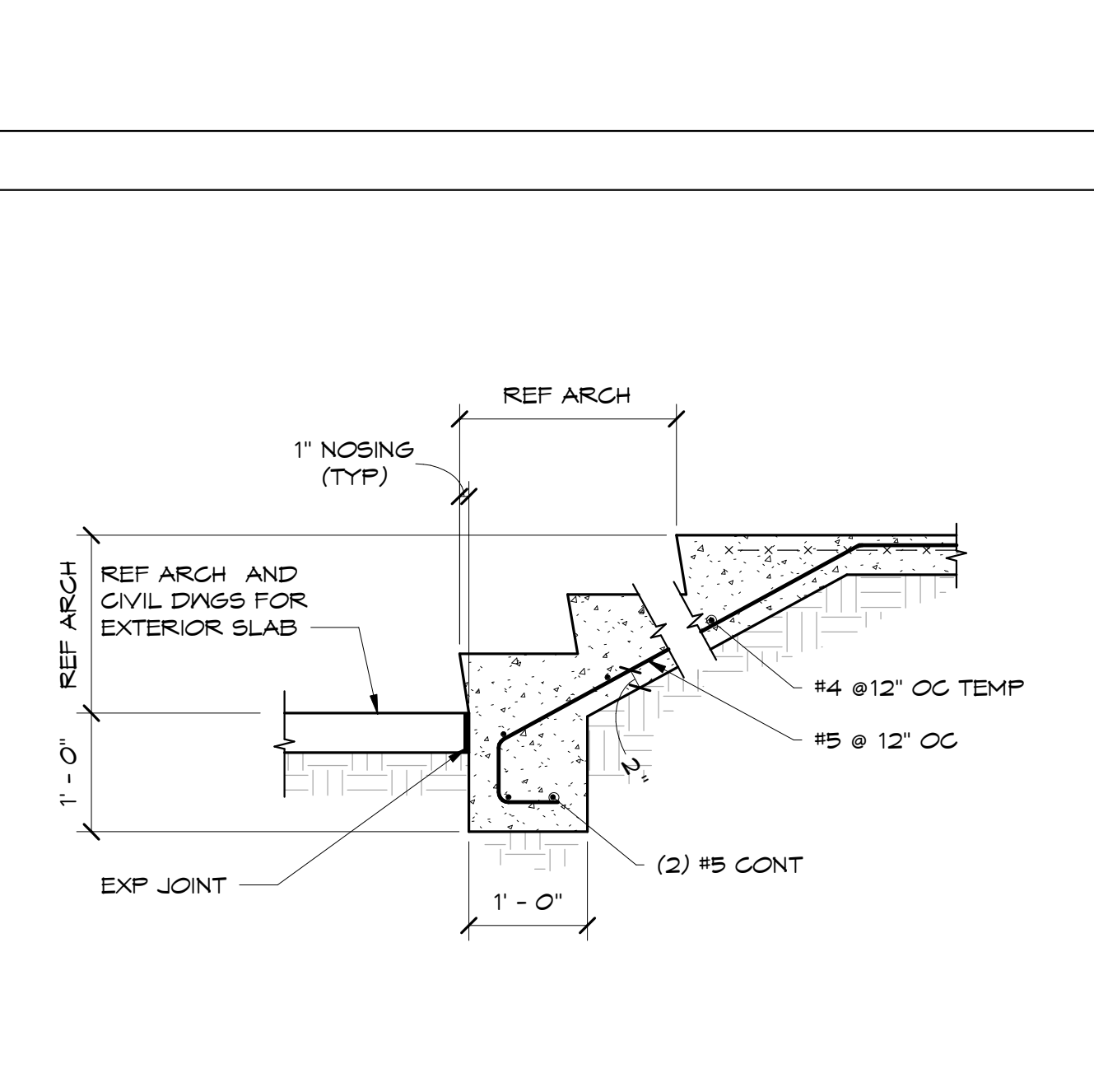
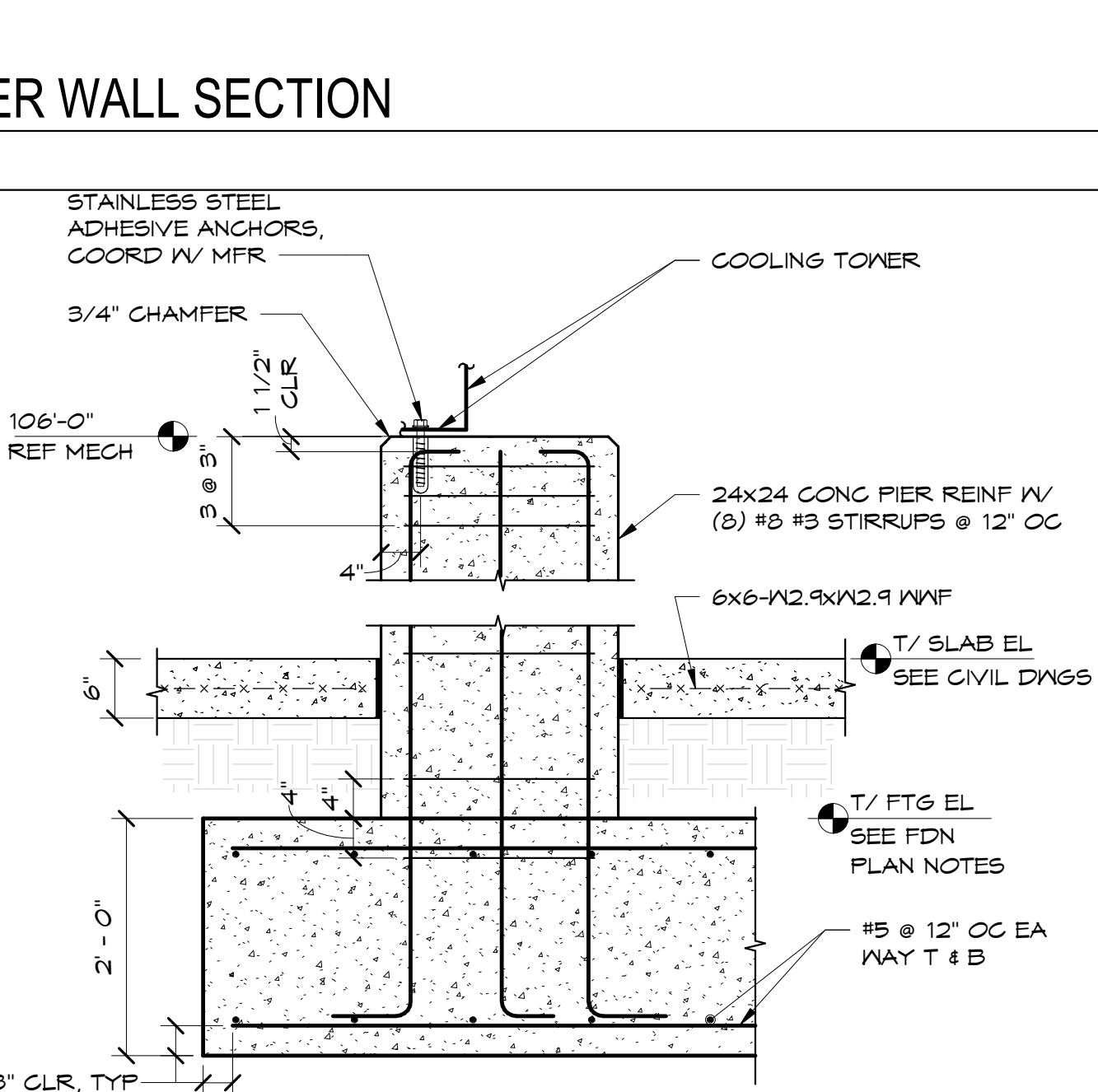
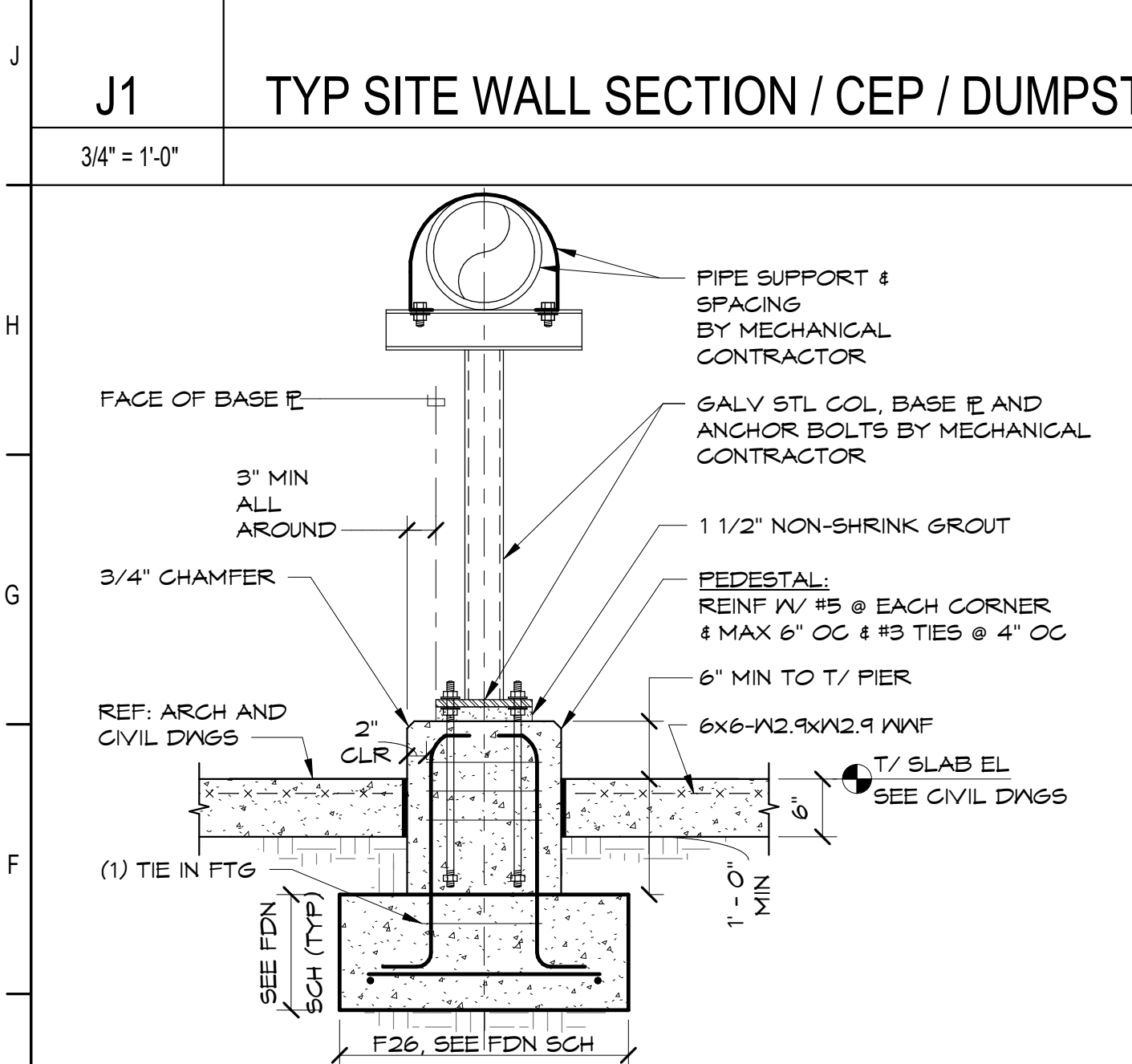
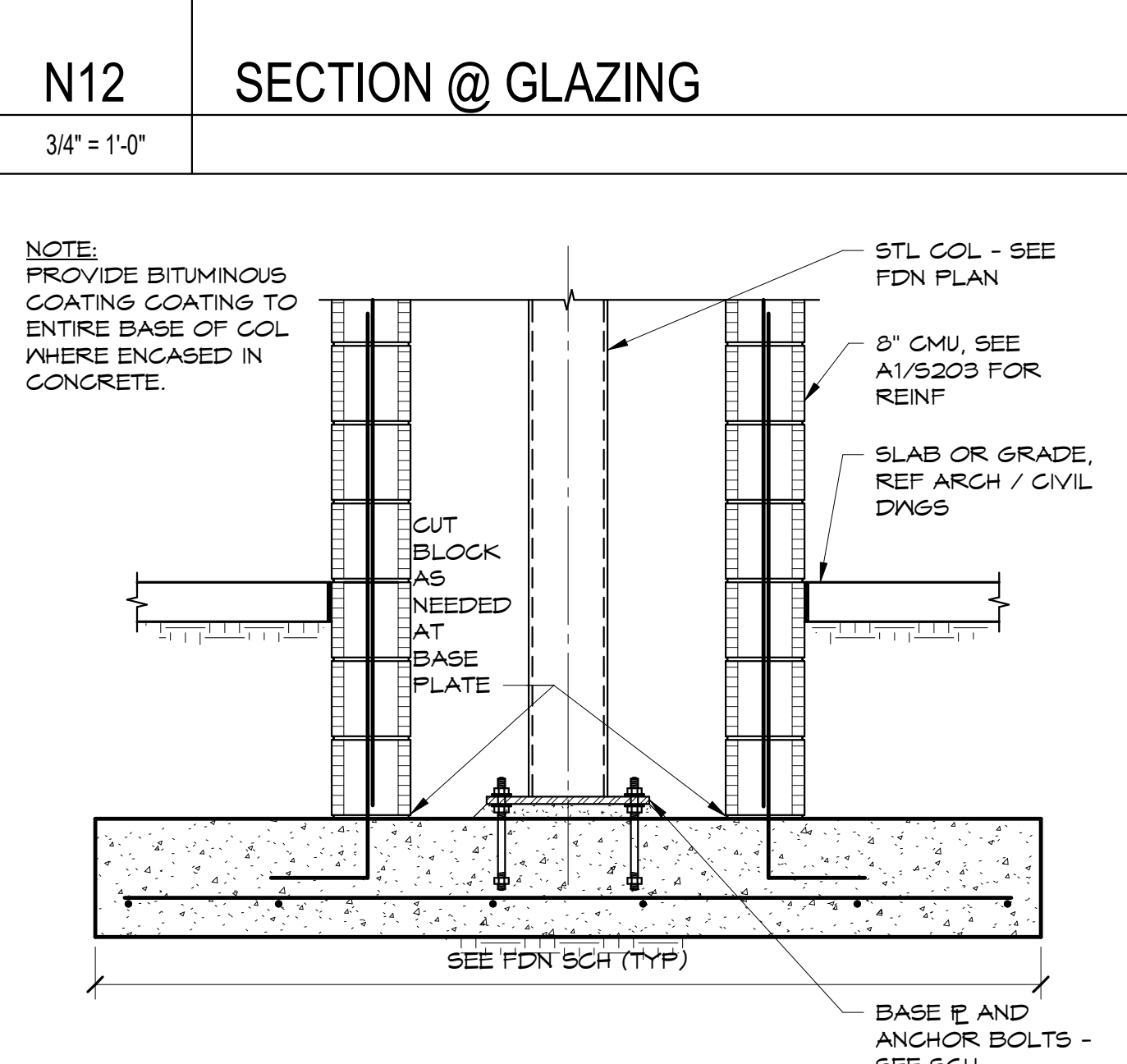
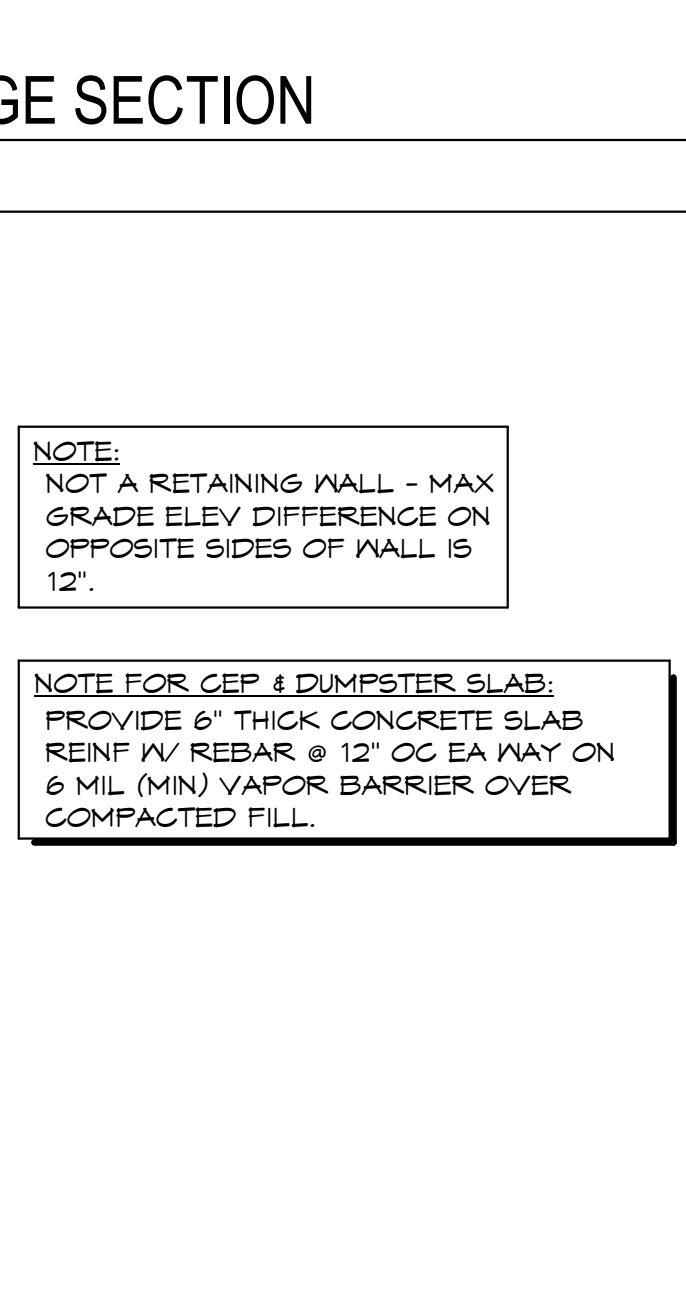
WALL H	FOOTING SIZE (W x L)	CMU WIDTH (NOMINAL)	FOOTING REINFORCING	WALL REINFORCING	T
MAX 5'-0"	3'-0" x CONT	8	(3) #5 CONT T&B, #4 STIRRUPS @ 24" OC	#5 @ 48" OC, SEE NOTE 2	1'-1"
MAX 7'-0"	4'-0" x CONT	8	(4) #5 CONT T&B, #4 STIRRUPS @ 18" OC	#5 @ 40" OC, SEE NOTE 2	1'-1"
MAX 9'-0"	5'-0" x CONT	8	(4) #5 CONT T&B, #4 STIRRUPS @ 18" OC	#5 @ 24" OC, SEE NOTE 2	1'-1"
MAX 11'-0"	5'-0" x CONT	12	(4) #5 CONT T&B, #4 STIRRUPS @ 18" OC	#6 @ 32" OC, SEE NOTE 2	1'-3"
MAX 13'-0"	6'-0" x CONT	12	(5) #5 CONT T&B, #4 STIRRUPS @ 12" OC	#6 @ 24" OC, SEE NOTE 2	1'-3"
MAX 15'-0"	7'-0" x CONT	12	(6) #5 CONT T&B, #4 STIRRUPS @ 12" OC	#6 @ 16" OC, SEE NOTE 2	1'-3"

NOTE:

- PLACE THRU-WALL CONTROL JOINTS AT 30'-0" OC MAX AND 15'-0" FROM CORNERS. DO NOT CONTINUE HORIZONTAL REINF ACROSS CONTROL JOINTS. SEE MASONRY CONTROL JOINT DETAIL IN GENERAL NOTES.
- PROVIDE (2) BARS IN EACH CELL (ONE AT EACH FACE) AT ENTRANCE SECURITY WALLS BETWEEN BUILDINGS.

NOTE: NOT A RETAINING WALL - MAX GRADE ELEV DIFFERENCE ON OPPOSITE SIDES OF WALL IS 12".

NOTE FOR CEP / DUMPSTER SLAB: PROVIDE 6" THICK CONCRETE SLAB REINF IV REBAR @ 12" OC EA WAY ON 6 MIL (MIN) VAPOR BARRIER OVER COMPACTED FILL.



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E. 5343
BBM PROJECT #22000

FLORIDA PROFESSIONAL ENGINEER NO. 65509

LUIS F. BEDOYA, P.E.
Florida Professional Engineer No. 65509

REVISIONS

MARK	DESCRIPTION	DATE
1	ADDITIONAL NO. 2	08/06/2022
2	REV NO. 3	08/06/2022

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Port St. Lucie, FL

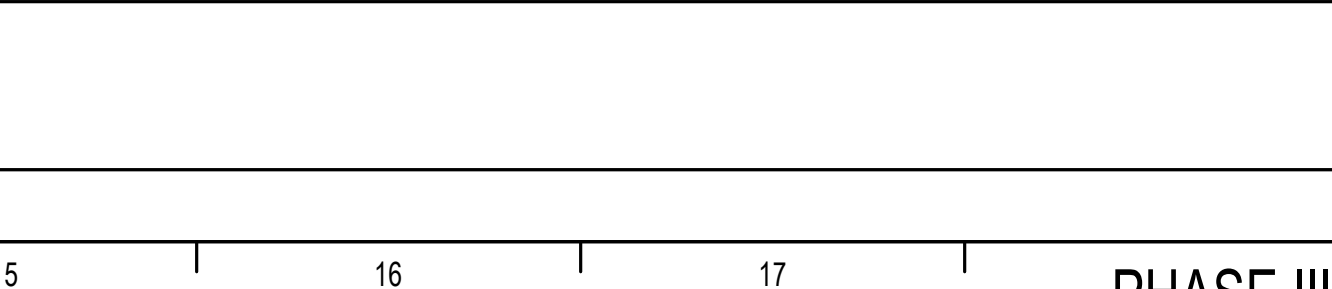
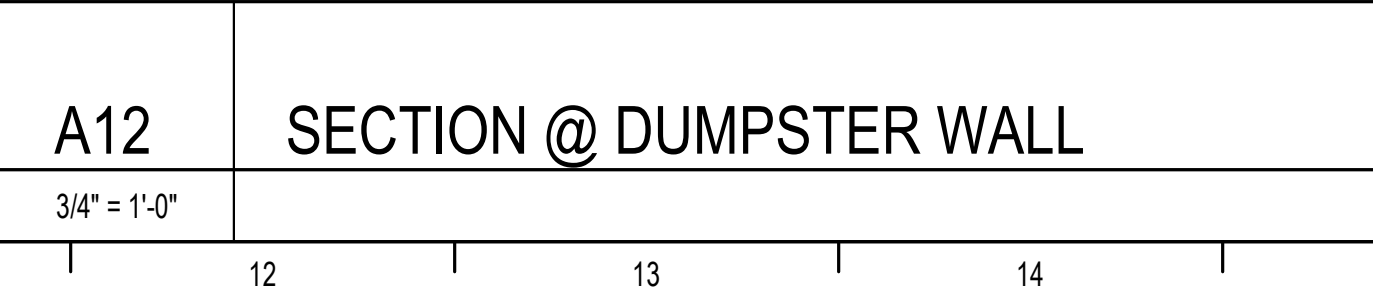
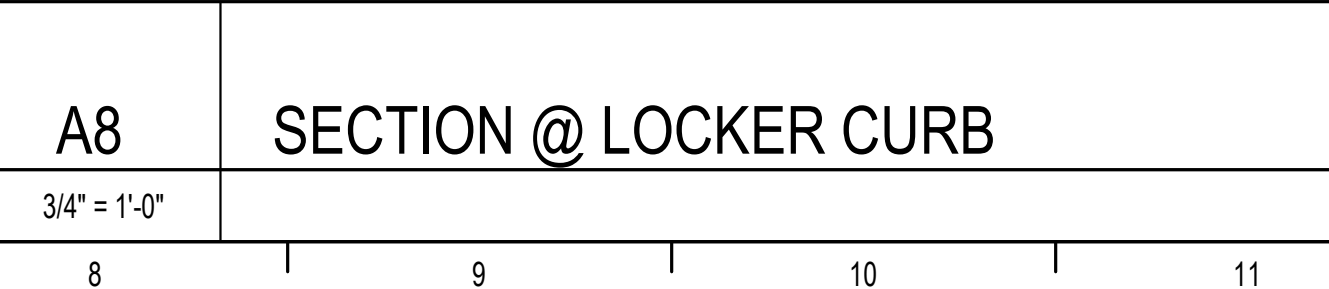
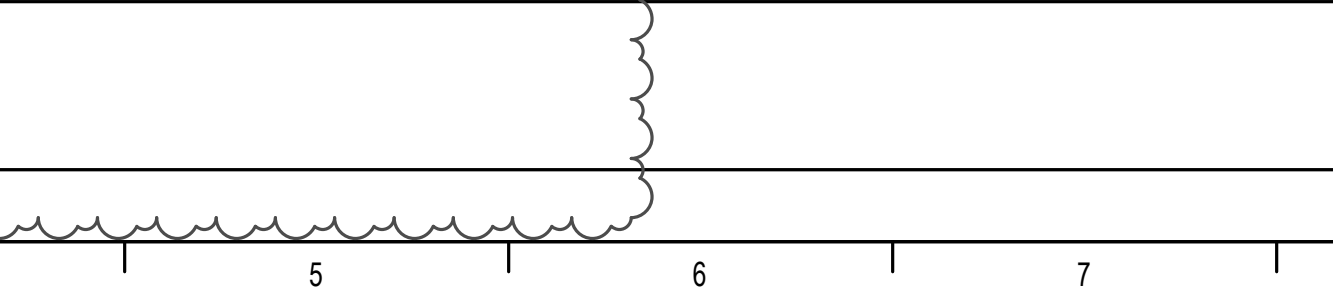
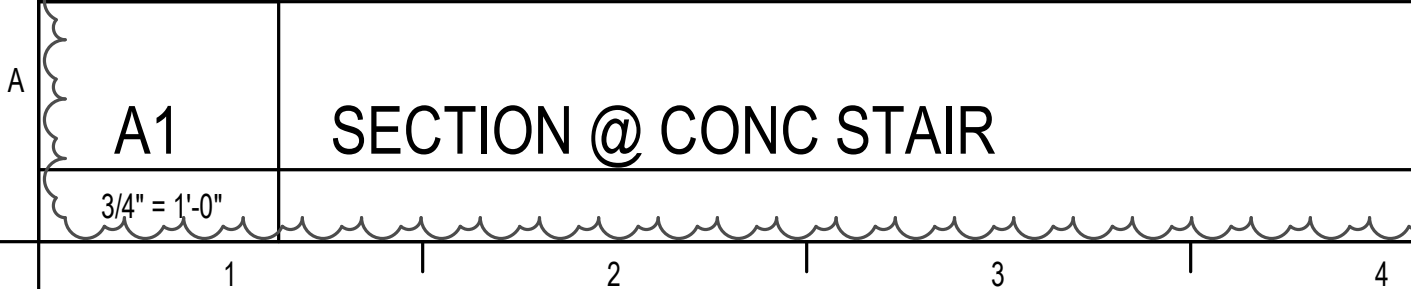
St. Lucie
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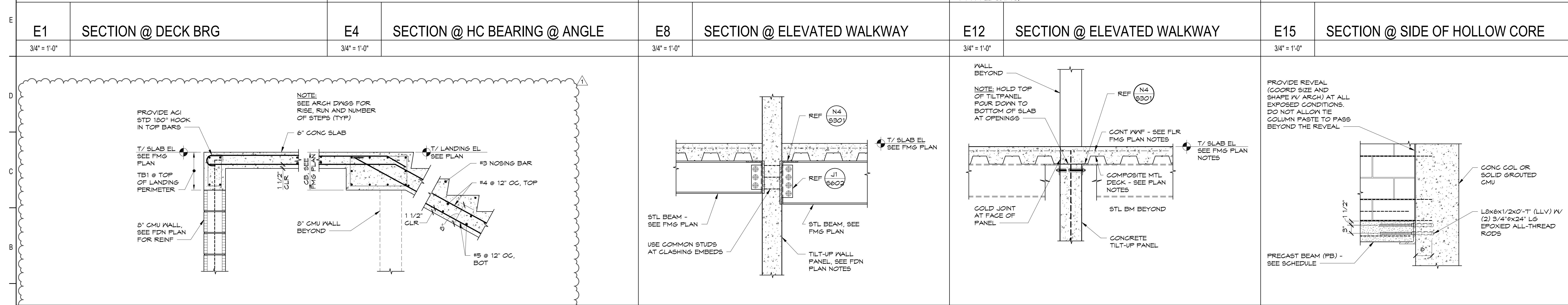
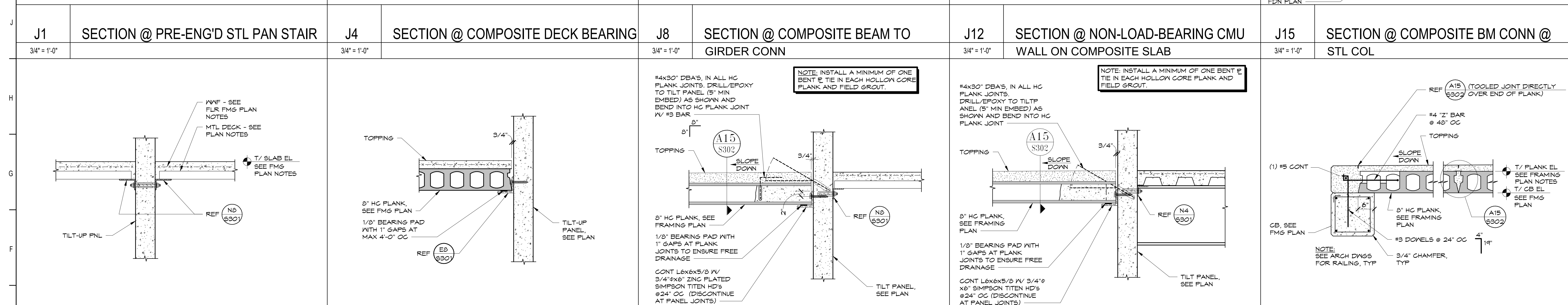
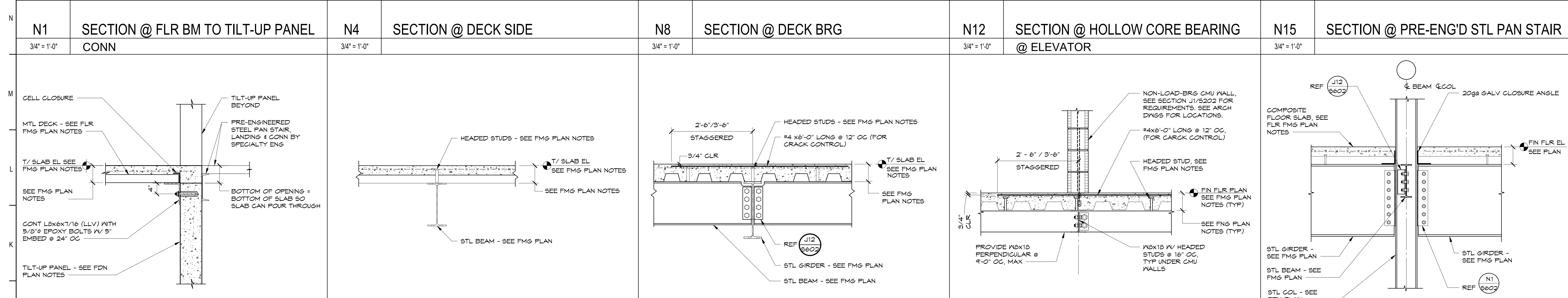
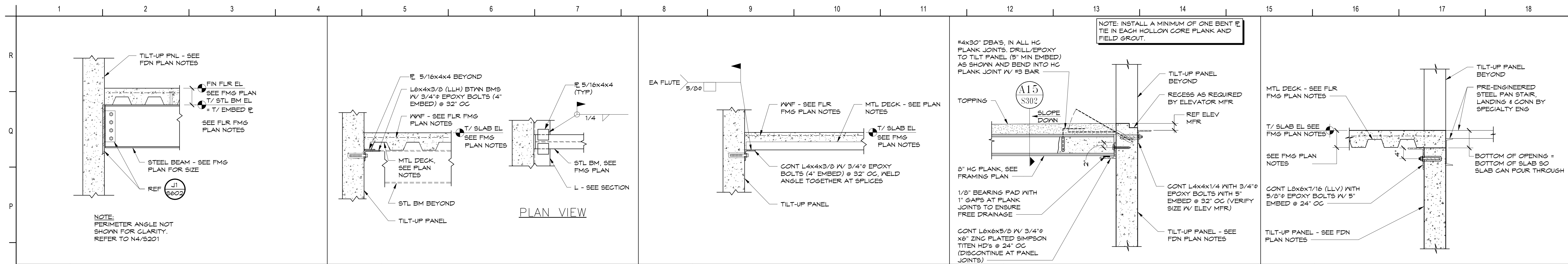
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FOUNDATION SECTIONS & DETAILS



S204



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LUIS F. BEDOYA, P.E.
 Florida Professional Engineer No. 65509

REVISIONS

MARK	DESCRIPTION	DATE
A12 NO. 3		06/02/23

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Port St. Lucie, FL

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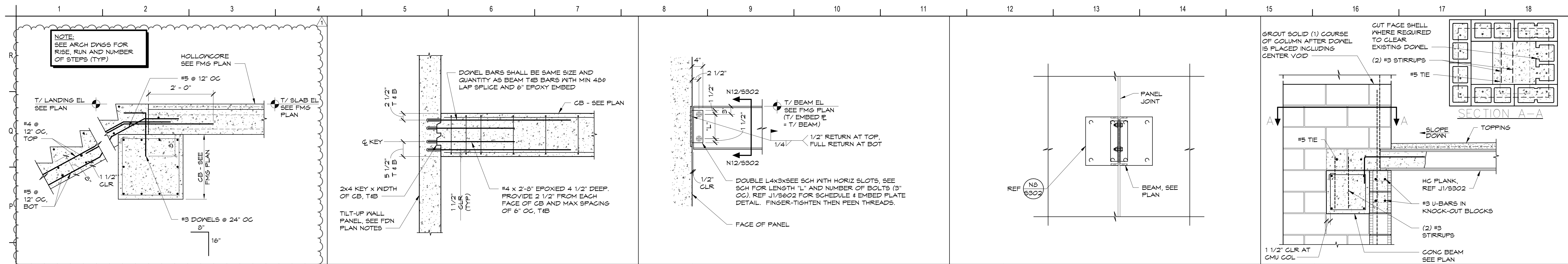
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FLOOR FRAMING SECTIONS & DETAILS

S301



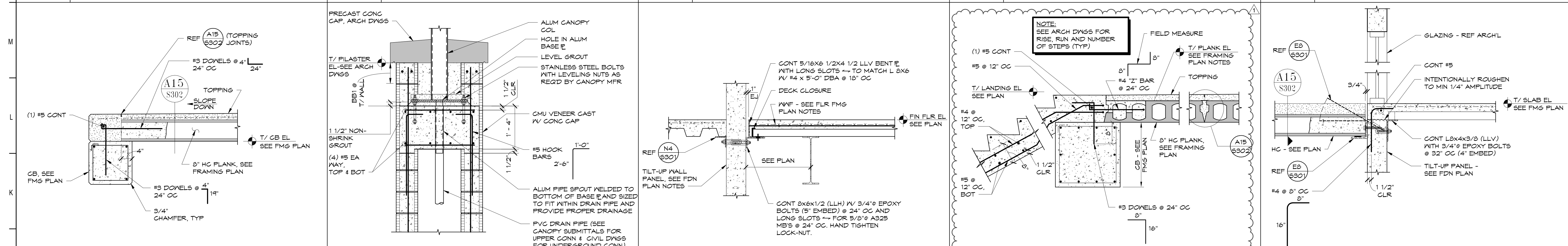
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3/4" = 1'-0"

N4 SECTION @ CB TO TILT PANEL CONN
3/4" = 1'-0"

N8 SECTION @ PANEL JOINT BEAM BRG
3/4" = 1'-0"

N12 CONNECTION @ JOINT
3/4" = 1'-0"

N15 MC DETAIL @ CB
3/4" = 1'-0"



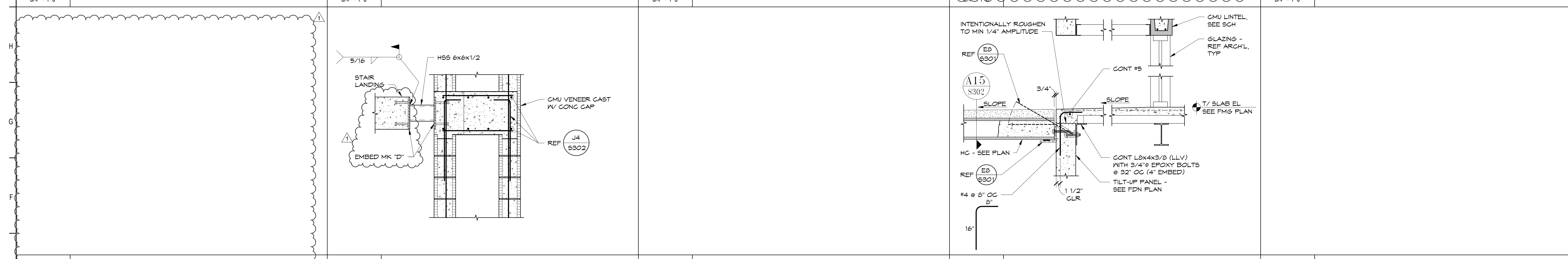
J1 SECTION @ HOLLOW CORE BRG @ BM
3/4" = 1'-0"

J4 SECTION @ AL COL CONN TO CMU COL
3/4" = 1'-0"

J8 SECTION @ FLOOR EXP JOINT
3/4" = 1'-0"

J12 SECTION @ STAIR WALKWAY
3/4" = 1'-0"

J15 SECTION @ ENTRY
3/4" = 1'-0"



E4 SECTION @ PRECAST STAIR LANDING
3/4" = 1'-0"

E12 SECTION @ ENTRY
3/4" = 1'-0"



A15 DETAIL @ JOINT IN EXPOSED TOPPING
3/4" = 1'-0"

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REGISTERED PROFESSIONAL ENGINEER
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REGISTERED IN THE STATE OF ALABAMA
REGISTERED IN THE STATE OF ARIZONA
REGISTERED IN THE STATE OF CALIFORNIA
REGISTERED IN THE STATE OF COLORADO
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Florida Professional Engineer No. 65509

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Port St. Lucie, FL

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PUBLIC SCHOOLS

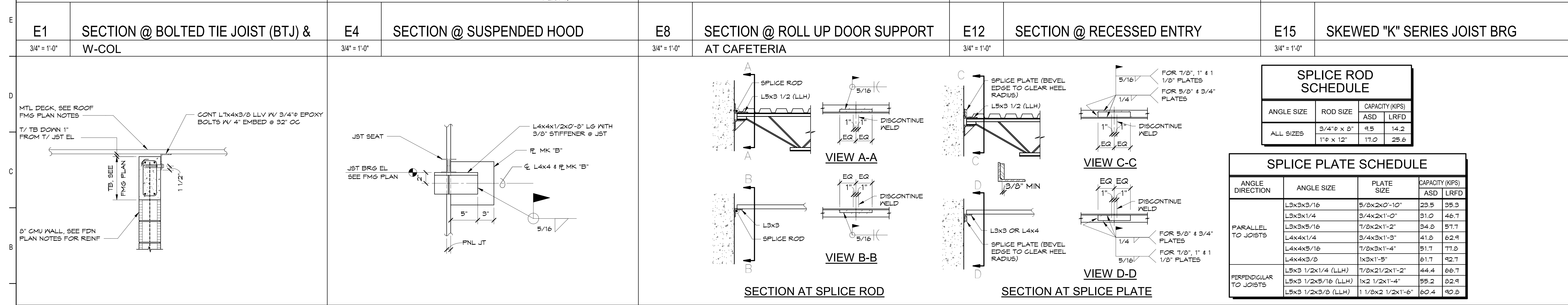
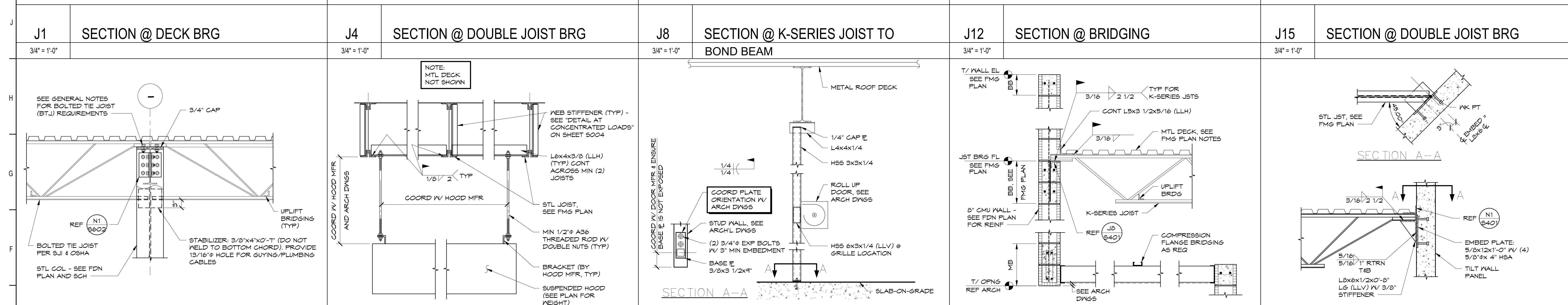
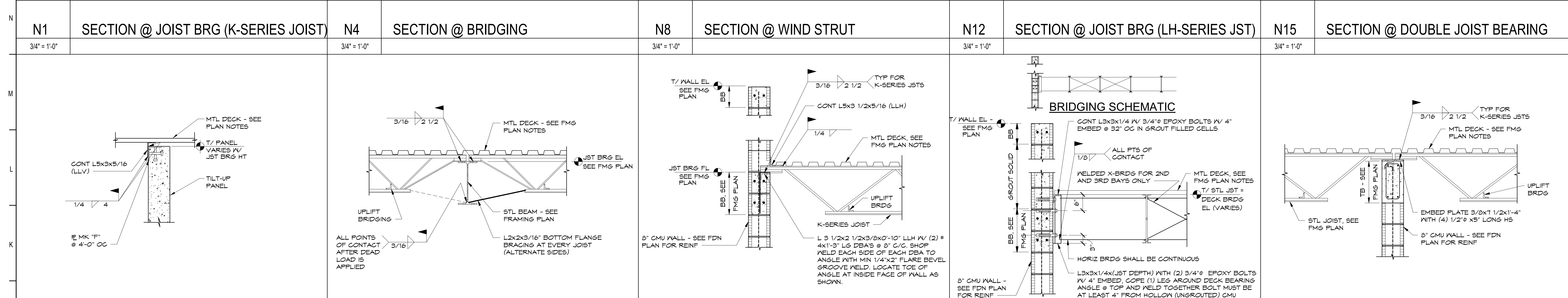
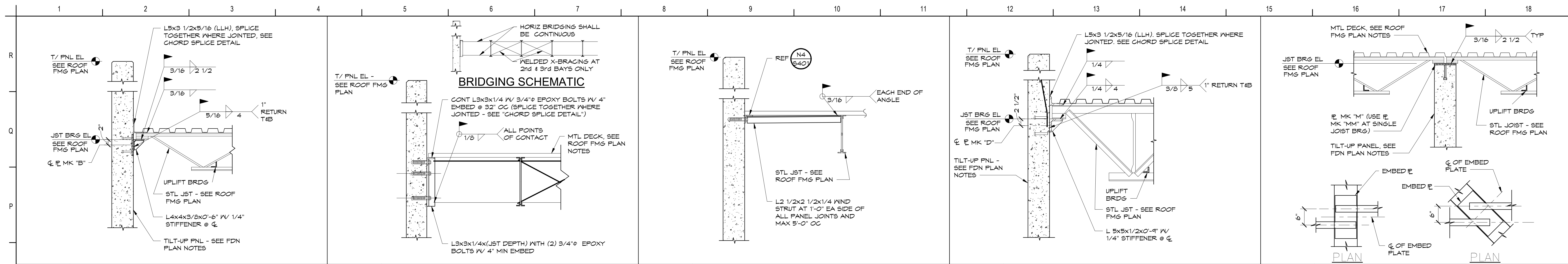
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ISSUE DATE: 12/16/2022
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FLOOR FRAMING SECTIONS & DETAILS

S302



SPLICE ROD SCHEDULE

ANGLE SIZE	ROD SIZE	CAPACITY (KIPS)	
		ASD	LRFD
ALL SIZES	3/4" x 8"	9.5	14.2
	1" x 12"	17.0	25.6

SPLICE PLATE SCHEDULE

ANGLE DIRECTION	ANGLE SIZE	PLATE SIZE	CAPACITY (KIPS)		
			ASD	LRFD	
PARALLEL TO JOISTS	L3x3x3/16	5/8x2x0'-10"	23.5	35.3	
	L3x3x1/4	3/4x2x1'-0"	31.0	46.7	
	L3x3x5/16	1/2x2x1'-2"	34.8	51.7	
	L4x4x1/4	3/4x3x1'-3"	41.8	62.9	
PERPENDICULAR TO JOISTS	L4x4x5/16	1/2x3x1'-4"	51.7	77.8	
	L4x4x3/8	1x3x1'-5"	61.7	92.7	
	L5x3 1/2x1/4 (LLH)	1/2x2x1'-2"	44.4	66.7	
	L5x3 1/2x5/16 (LLH)	1x2 1/2x1'-4"	55.2	82.9	
		L5x3 1/2x3/8 (LLH)	1 1/8x2 1/2x1'-6"	80.4	90.8

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Florida Professional Engineer No. 65509

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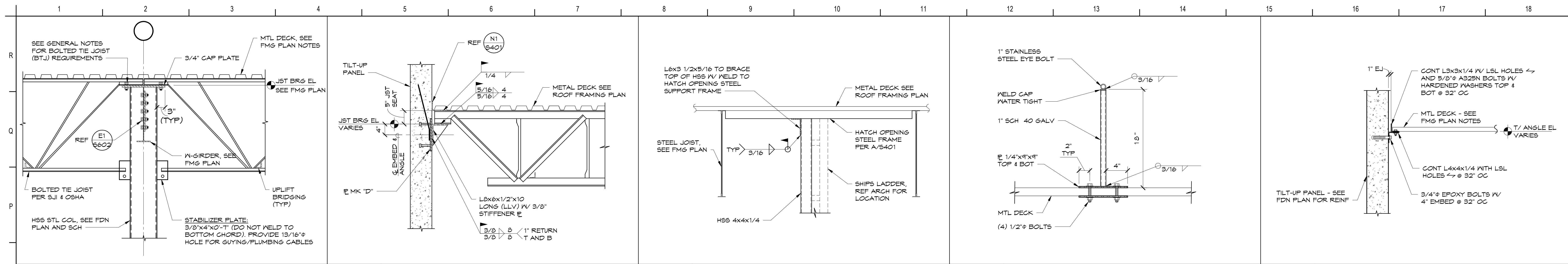
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ISSUE DATE: 12/16/2022
COMM. NO.: 2022107

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ROOF FRAMING SECTIONS & DETAILS

S401



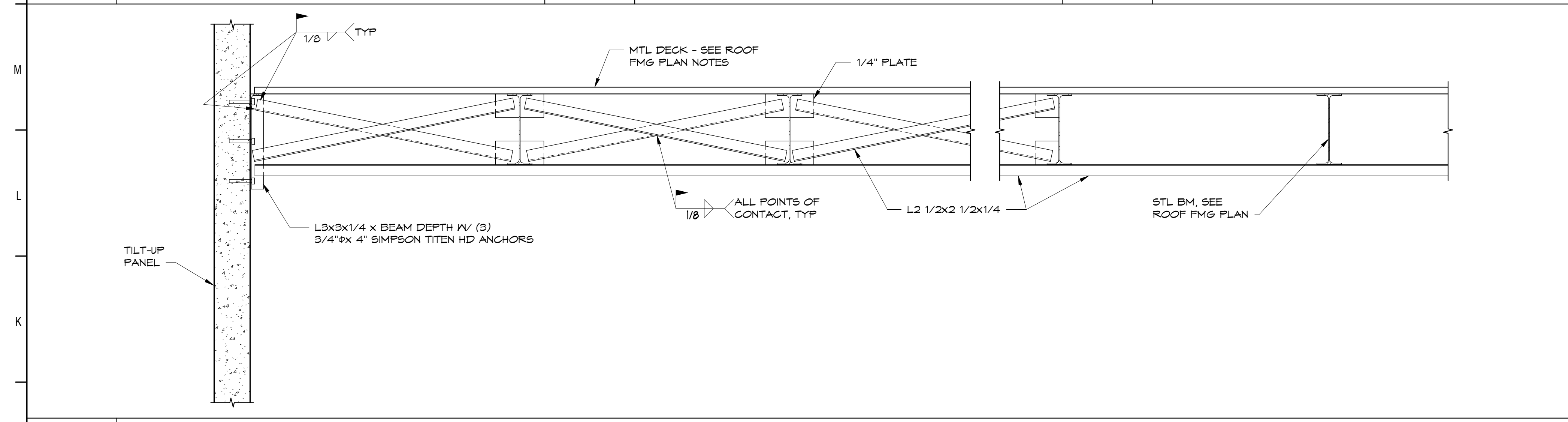
N1 SECTION @ BOLTED TIE JOIST (BTJ) & HSS COL 3/4" = 1'-0"

N4 TYPICAL "DLH" SERIES JOIST BRG SECTION 3/4" = 1'-0"

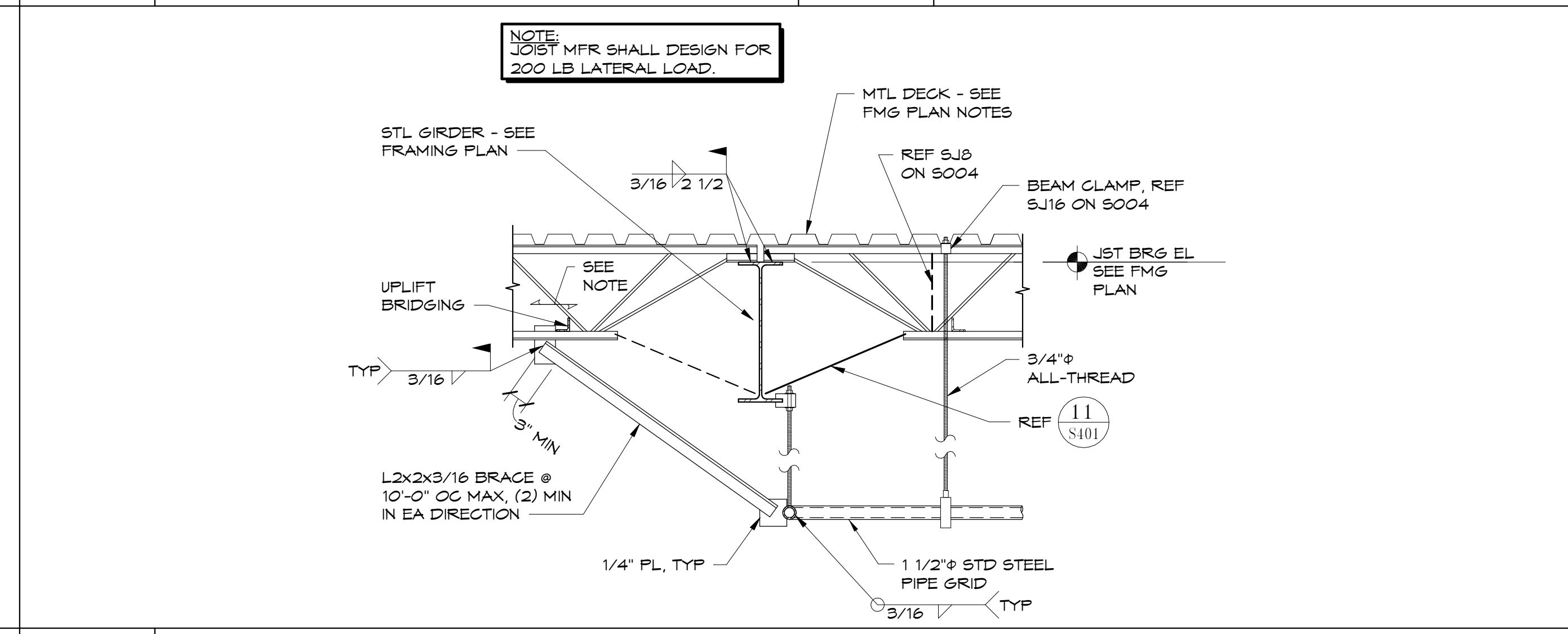
N8 SECTION @ SHIPS LADDER SUPPORT 3/4" = 1'-0"

N12 EYE BOLT ROOF MECHANICAL TIE DOWN 3/4" = 1'-0"

N15 SECTION @ EXP JOINT 3/4" = 1'-0"



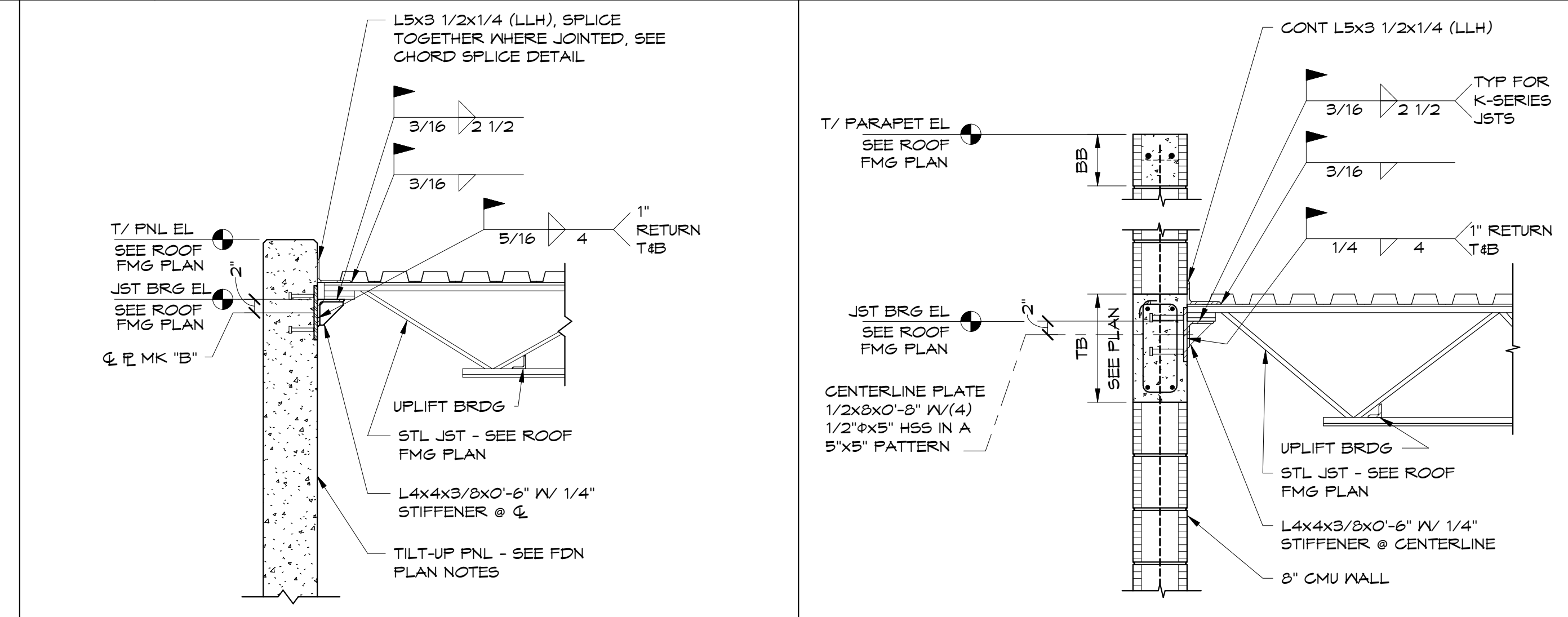
J1 SECTION @ BRACING FOR THEATRE RIGGING 3/4" = 1'-0"



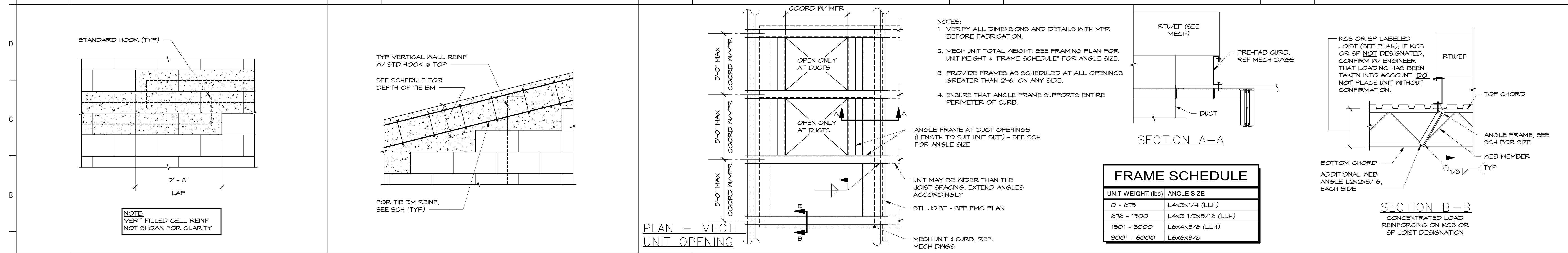
J12 SECTION @ PIPE GRID SWAY BRACE 3/4" = 1'-0"



E12 SECTION @ JOIST BRG (K-SERIES JST) 3/4" = 1'-0"



E15 SECTION @ JOIST BRG (K-SERIES JST) 3/4" = 1'-0"



A1 DETAIL - STEPPED BOND BEAM 3/4" = 1'-0"

A4 DETAIL - RAKED TIE BEAM 3/4" = 1'-0"

A8 DETAIL - ROOF TOP MECHANICAL UNIT SUPPORT AND ROOF OPENINGS 3/4" = 1'-0"

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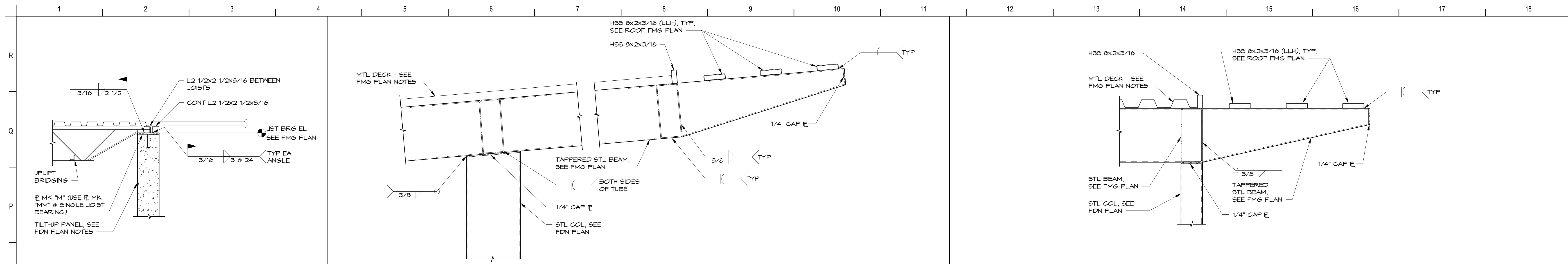
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ROOF FRAMING SECTIONS & DETAILS

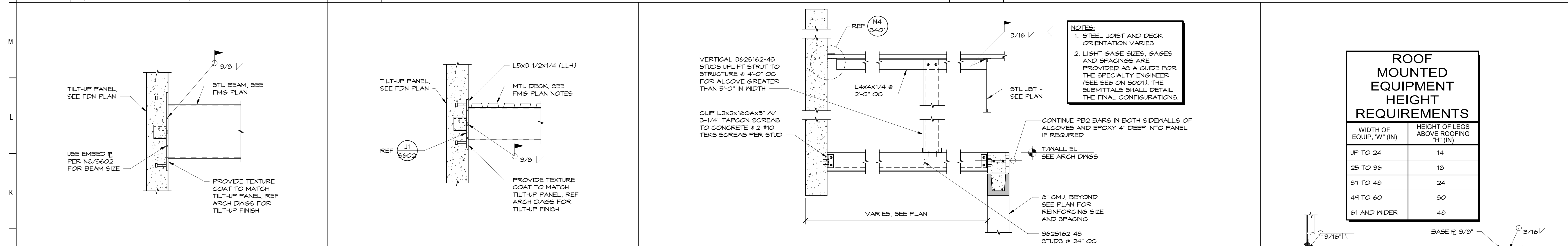
S402



N1 SECTION @ ROOF DECK TRANSITION
3/4" = 1'-0"
("K" SERIES JOISTS)

N4 SECTION @ CANOPY ROOF
3/4" = 1'-0"

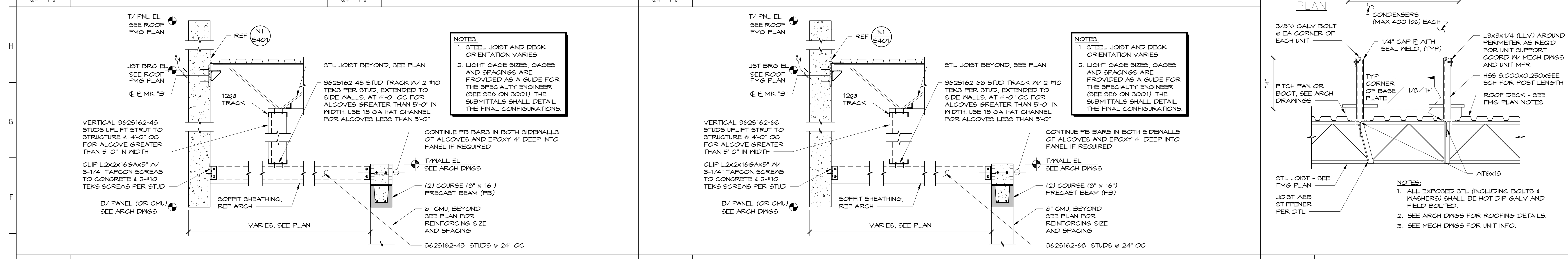
N12 SECTION @ CANT CANOPY BEAM
3/4" = 1'-0"



J1 SECTION @ CANOPY CONN
3/4" = 1'-0"

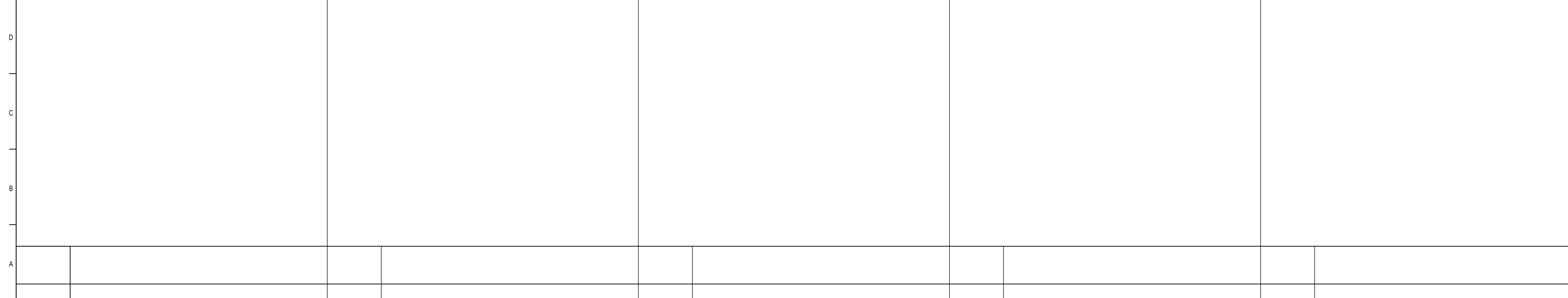
J4 SECTION @ HIGH ROOF
3/4" = 1'-0"

J8 SECTION @ ALCOVE ENTRY (FOR BLDG 1 & BLDG 3)
3/4" = 1'-0"



E1 TYPICAL SECTION @ ENTRY ALCOVE (FOR BLDG 1 & BLDG 3)
3/4" = 1'-0"

E8 TYPICAL SECTION @ ENTRY ALCOVE (FOR BLDG 2)
3/4" = 1'-0"



E15 SECTION @ CONDENSER SUPPORT FRAME
3/4" = 1'-0"

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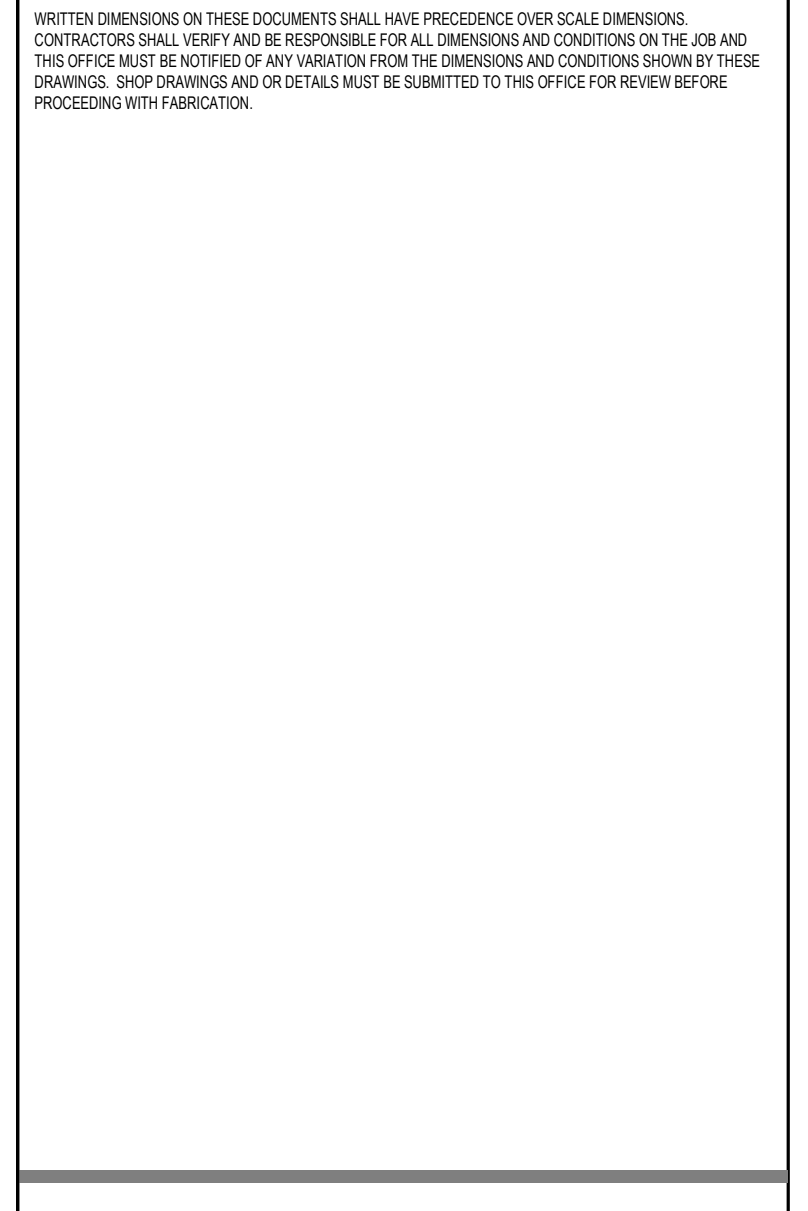
MARK	DESCRIPTION	DATE

ROOF MOUNTED EQUIPMENT HEIGHT REQUIREMENTS

WIDTH OF EQUIP. "W" (IN)	HEIGHT OF LEGS ABOVE ROOFING "H" (IN)
UP TO 24	14
25 TO 36	18
37 TO 48	24
49 TO 60	30
61 AND WIDER	48

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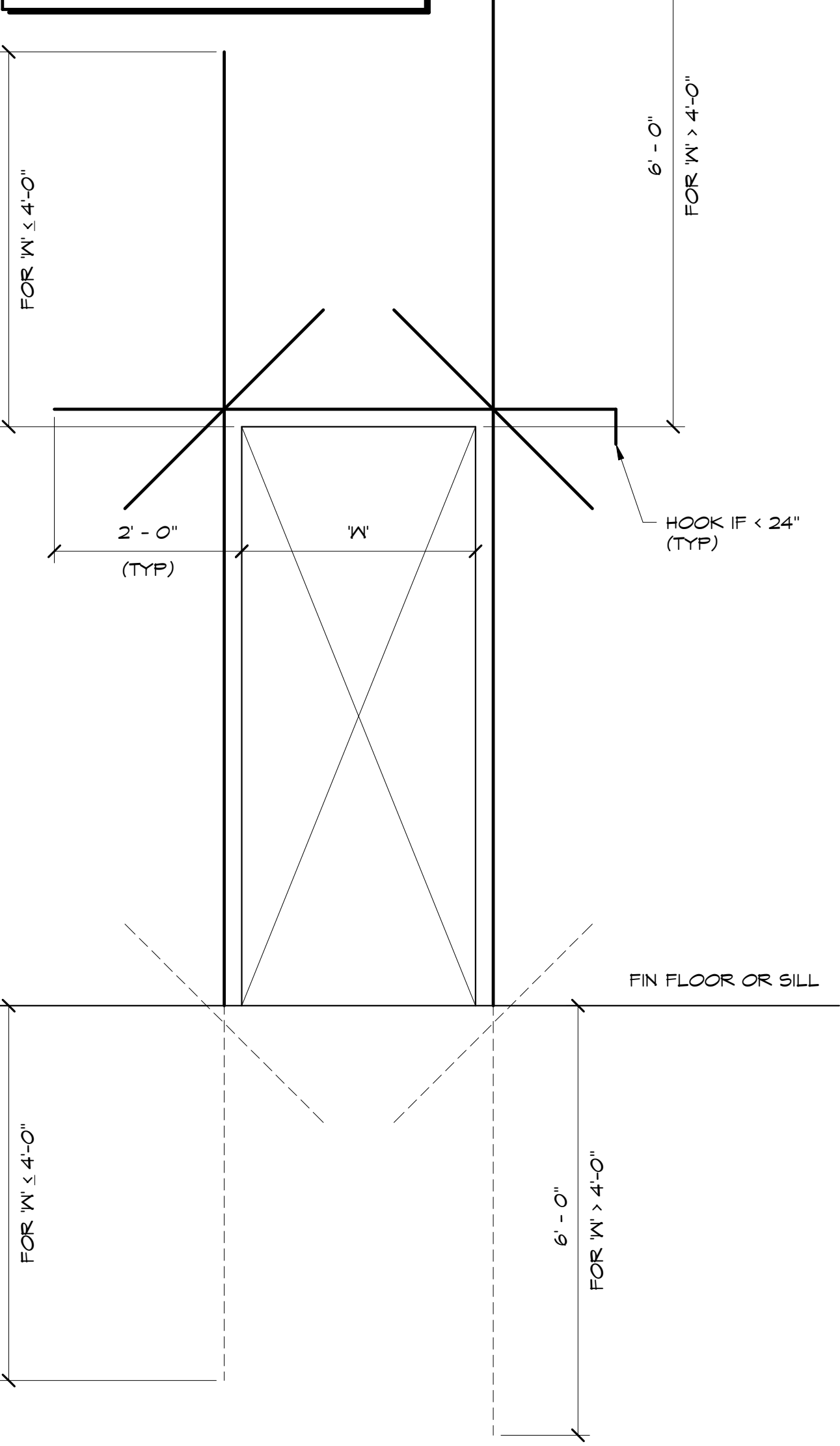
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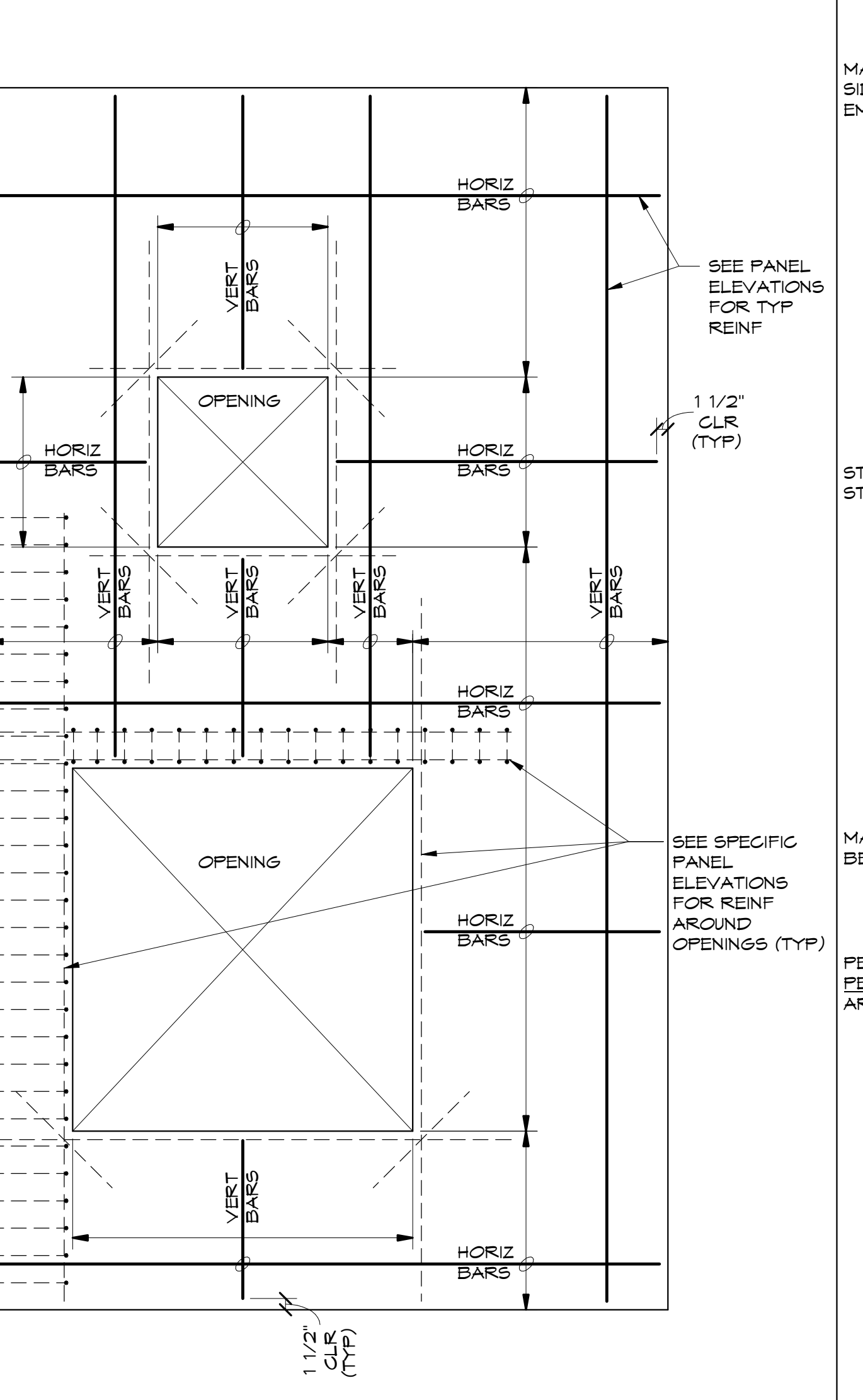
ROOF FRAMING SECTIONS & DETAILS

NOTE:
WHERE BAR EXTENSIONS ABOVE OR BELOW THE OPENING CANNOT BE ACHIEVED, EXTEND BAR 1 1/2" CLR OF PANEL EDGE.



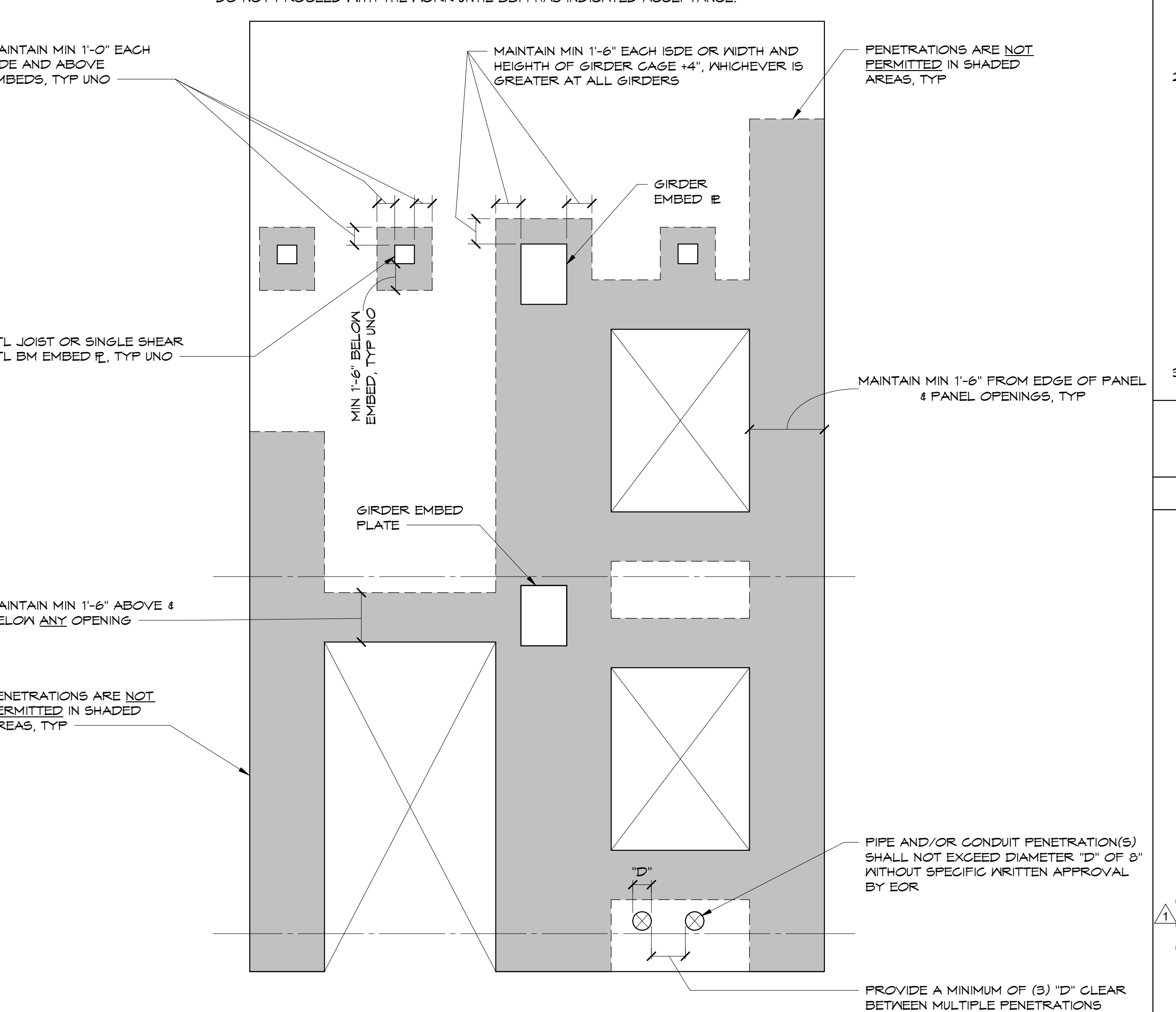
J1 TYP BAR LENGTH (ABOVE & BELOW OPENING)
3/4" = 1'-0"

NOTE:
COORD W/ SPECIFIC PANEL ELEVATIONS FOR GIRDER STL REINF



J4 ILLUSTRATIVE BAR PLACEMENT FOR TILT-UP PANELS
3/4" = 1'-0"

NOTE:
1. DO NOT PENETRATE PANEL AT ANY 'EACH FACE' REINFORCING CASE OR AT ANY SHADED AREAS.
2. PENETRATIONS NOT SPECIFICALLY INDICATED ON THE STRUCTURAL DRAWINGS MAY BE ALLOWED IN THE UNSHADED AREAS. SUBMIT RFI REQUESTING BBM TO REVIEW THE SPECIFIC SITUATION AND DO NOT PROCEED WITH THE WORK UNTIL BBM HAS INDICATED ACCEPTANCE.



J8 TYP GUIDE FOR TILT-UP PANEL PENETRATIONS
3/4" = 1'-0"

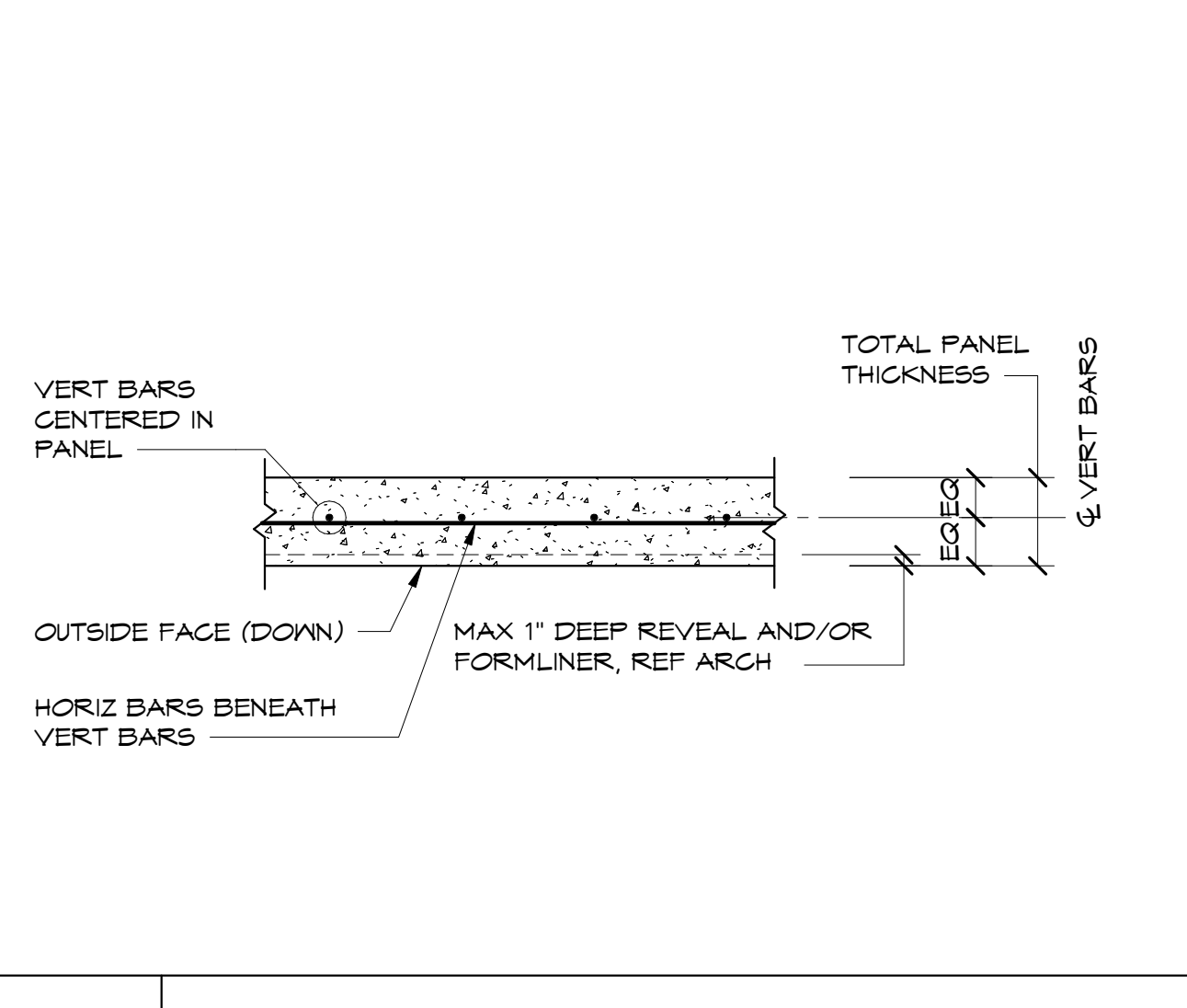
TILT-UP PANEL NOTES:

- DUCT PENETRATIONS HAVE BEEN GRAPHICALLY REPRESENTED AND COORDINATED WITH THE MECHANICAL CONTRACTOR AND SUBMITTED TO THE ARCHITECT IN THE FORM OF TILT-UP PANEL REINFORCING SHOP DRAWINGS. OTHER MISCELLANEOUS MINOR PENETRATIONS FOR CONDUIT, PIPING, ETC., SHALL BE COORDINATED WITH ALL TRADES AND SLEEVES PROVIDED IN THE TILT-UP PANELS. SLEEVES SHALL BE SPACED SO THAT THE CENTER TO CENTER DIMENSION IS AT LEAST 4 TIMES THE SLEEVE DIAMETER. THESE SLEEVED PENETRATIONS DO NOT REQUIRE ANY ADDITIONAL REINFORCING AND THEREFORE DO NOT NEED TO BE SUBMITTED TO THE ARCHITECT.
- OPENINGS IN TILT-UP PANELS, IN ADDITION TO THOSE INDICATED HEREIN, MAY BE REQUIRED. COORDINATE WITH ALL TRADES SO THAT THE OPENINGS THAT ARE REQUIRED HAVE BEEN FORMED OR SLEEVED INTO THE PANELS. CORE DRILLING HOLES INTO THE PANELS MAY BE DONE ONLY AFTER WRITTEN APPROVAL FROM BBM STRUCTURAL ENGINEERS. OPENINGS NOT INDICATED HEREIN SHALL BE REINFORCED AS FOLLOWS:
 - OPENINGS UP TO 2'-0" IN ANY DIRECTION SHALL BE REINFORCED WITH (2) #5 BARS ALL AROUND. THE BARS SHALL BE EXTENDED A MINIMUM OF 2'-0" BEYOND THE OPENING. IF THE OPENING IS NOT ROUND, ALSO PROVIDE (1) #5 x 4'-0" LG DIAGONAL CORNER BAR AT EACH CORNER. OPENINGS SHALL BE PLACED AT LEAST 12" AWAY FROM PANEL JOINTS AND SHALL NOT INTERFERE WITH ANY OTHER JAMB, HEAD OR SILL REINFORCING.
 - OPENINGS GREATER THAN 2'-0" IN ANY DIRECTION SHALL BE MADE ONLY AFTER WRITTEN DIRECTION FROM BBM STRUCTURAL ENGINEERS IS OBTAINED. FOR PRICING PURPOSE ONLY, PROVIDE (2) #5 BARS ALL AROUND THE OPENING. THE BARS SHALL BE EXTENDED A MINIMUM OF 4'-0" BEYOND THE OPENING. IF THE OPENING IS NOT ROUND, ALSO PROVIDE (1) #5 x 4'-0" LG DIAGONAL CORNER BAR AT EACH CORNER. OPENINGS SHALL BE PLACED AT LEAST 12" AWAY FROM PANEL JOINTS AND SHALL NOT INTERFERE WITH ANY OTHER JAMB, HEAD OR SILL REINFORCING.
- IF JAMB STEEL DOES NOT EXTEND FROM EDGE OF JAMB TO EDGE OF PANEL, FILL IN LEFT-OVER AREA WITH TYPICAL VERTICAL STEEL.

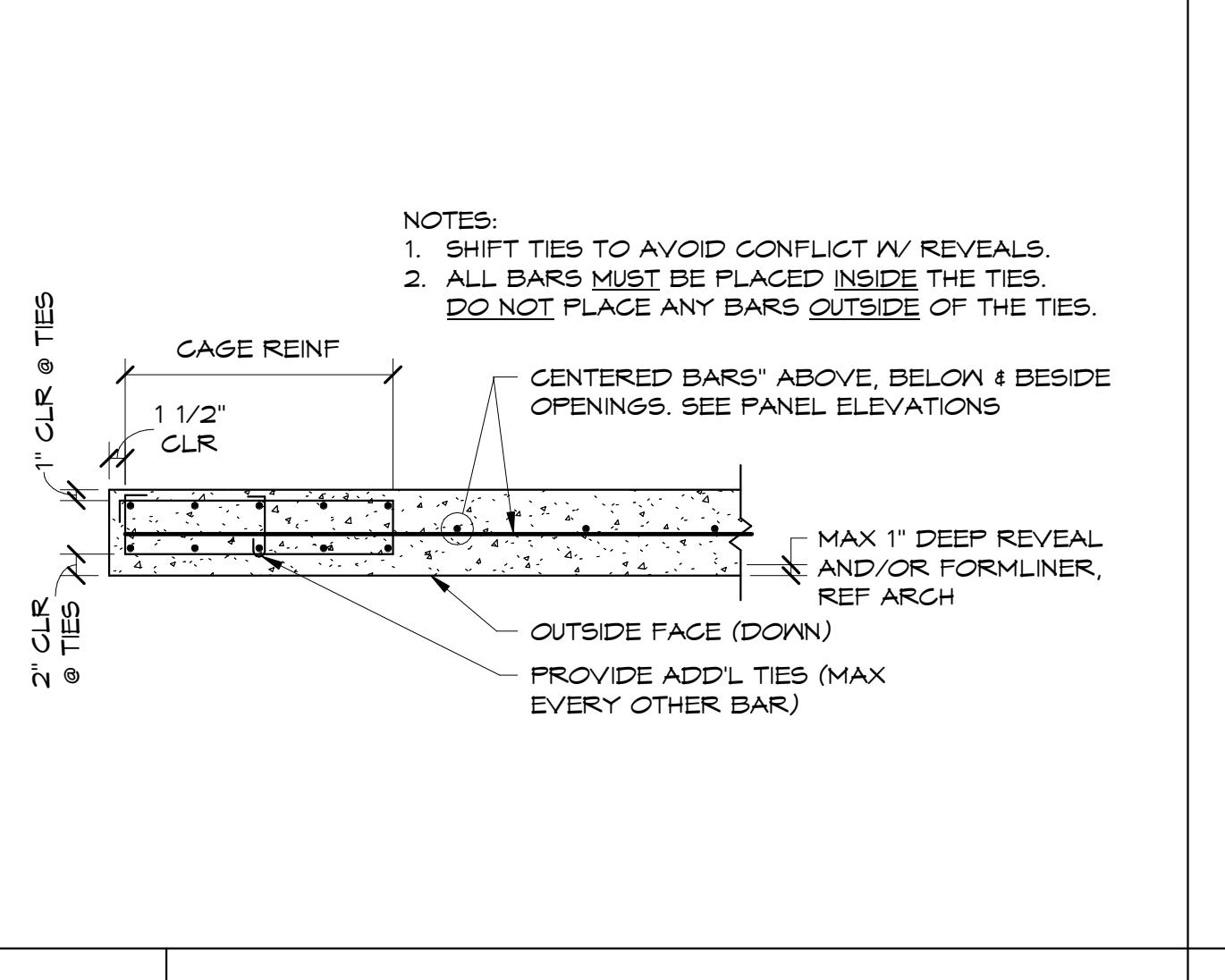
N15 TILT-UP PANEL NOTES
3/4" = 1'-0"

MARK	DESCRIPTION	COMMENT
A	(3) #5 EF, EQ SPACED IN 1'-0" CASE W/ #3 TIES @ 8" OC.	
B	(3) #5 EF, EQ SPACED IN PANEL LEG W/ #3 TIES @ 8" OC.	
C	(4) #5 EF, EQ SPACED IN PANEL LEG W/ #3 TIES @ 8" OC.	
D	(4) #5 EF, EQ SPACED IN 2'-0" CASE W/ #3 TIES @ 8" OC.	
E	(6) #5 EF, EQ SPACED IN PANEL LEG W/ #3 TIES @ 8" OC.	
F	(8) #5 EF, EQ SPACED IN PANEL LEG W/ #3 TIES @ 8" OC.	
G	(6) #5 EF, EQ SPACED IN HEADER W/ #3 STIRRUPS @ 12" OC.	
H	(6) #5 EF, EQ SPACED IN 2'-0" HEADER CASE W/ #3 STIRRUPS @ 12" OC.	
J	(6) #5 EF, EQ SPACED IN HEADER W/ #3 STIRRUPS @ 12" OC.	
K	(6) #5 EF, EQ SPACED IN 2'-0" CASE W/ #3 TIES @ 8" OC.	
L	NOT USED.	
M	(3) #5 EF, EQ SPACED IN HEADER CASE W/ #3 STIRRUPS @ 8" OC.	
N	(4) #5 EF, EQ SPACED IN 1'-0" HEADER CASE W/ #3 STIRRUPS @ 8" OC.	

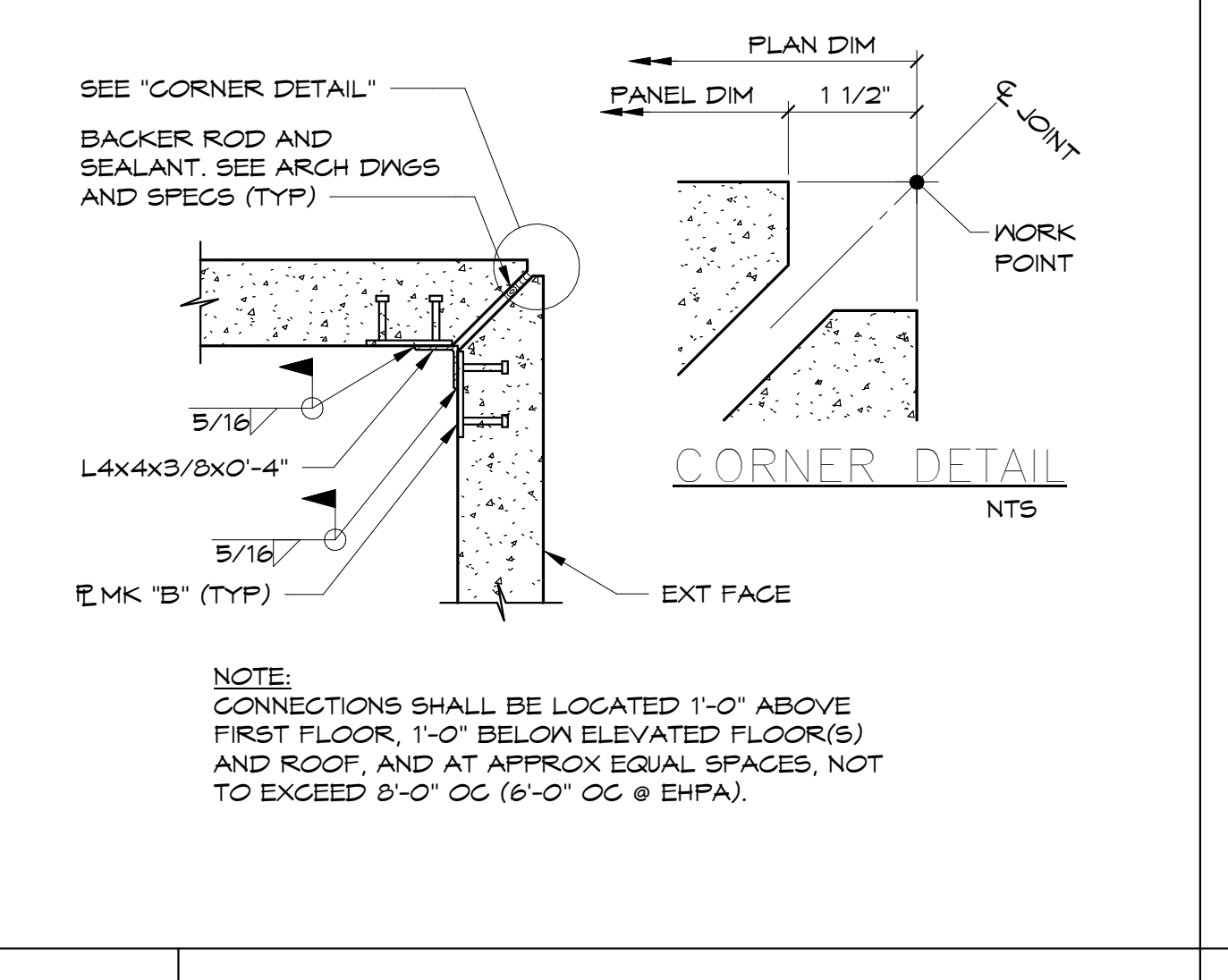
J15 CAGED PANEL REINFORCING SCHEDULE
3/4" = 1'-0"



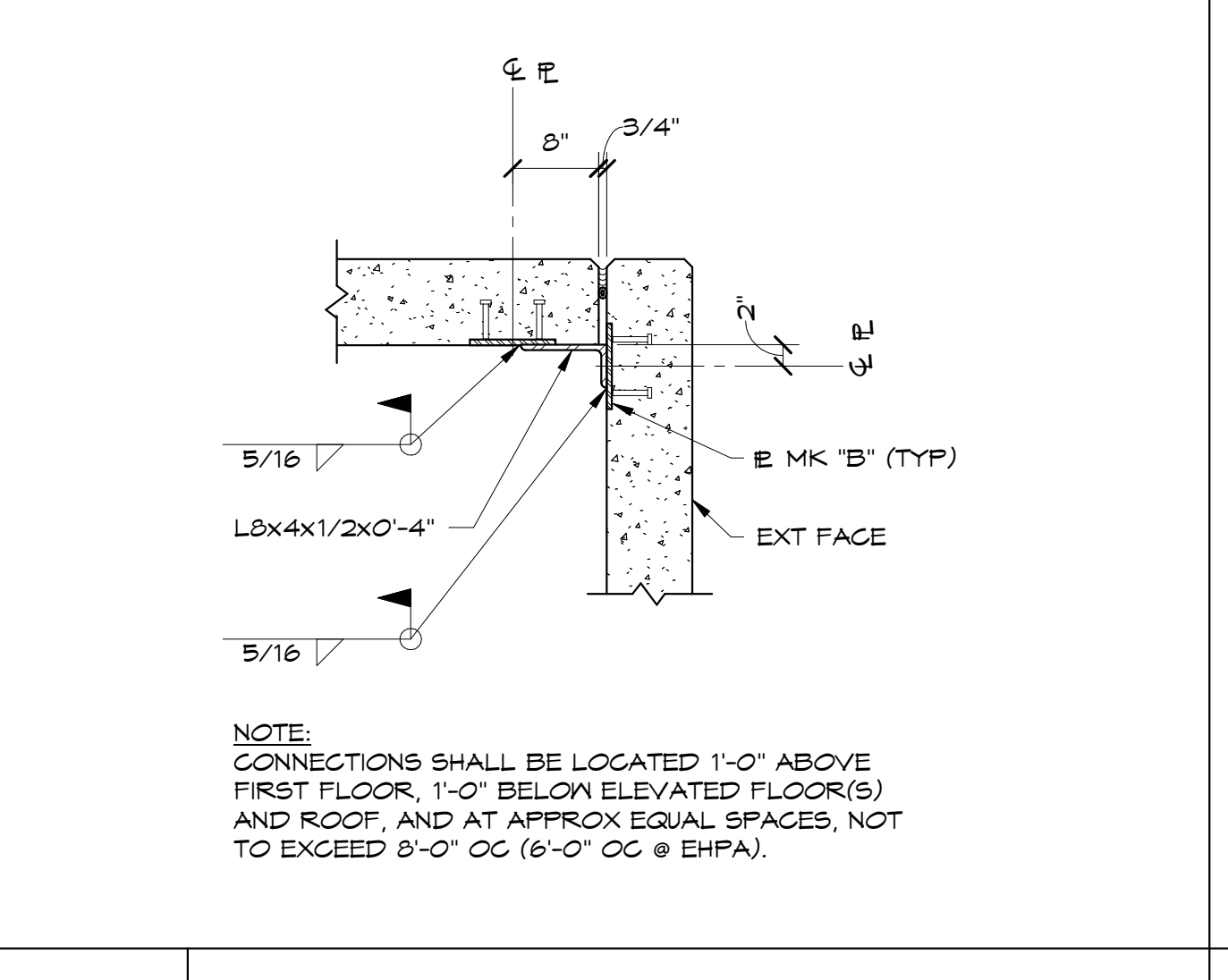
E1 TYPICAL CENTERED REINF PLACEMENT
3/4" = 1'-0"



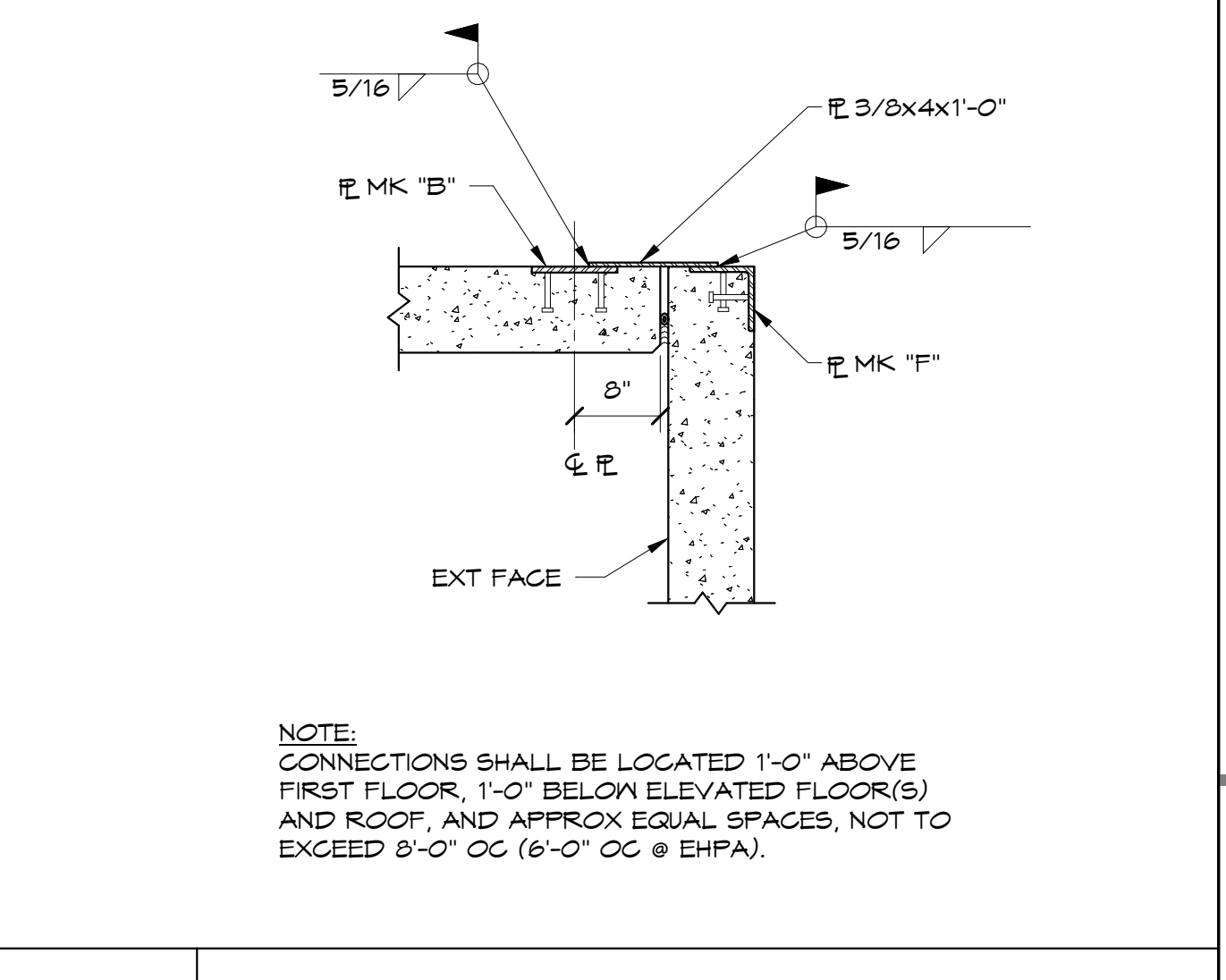
E4 TYP CAGE & CENTERED REINF TRANSITION
3/4" = 1'-0"



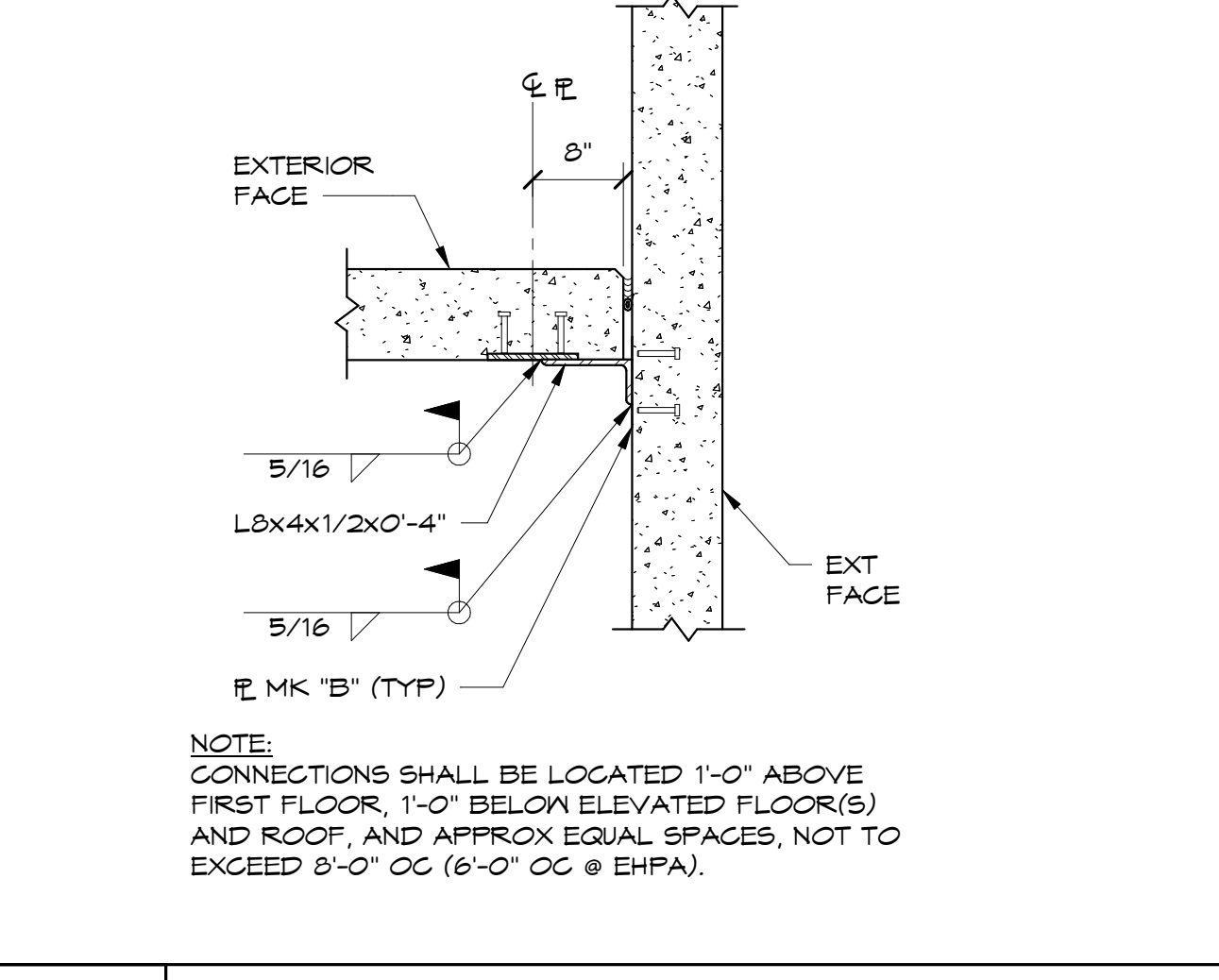
E8 DETAIL - MITER CONN
3/4" = 1'-0"



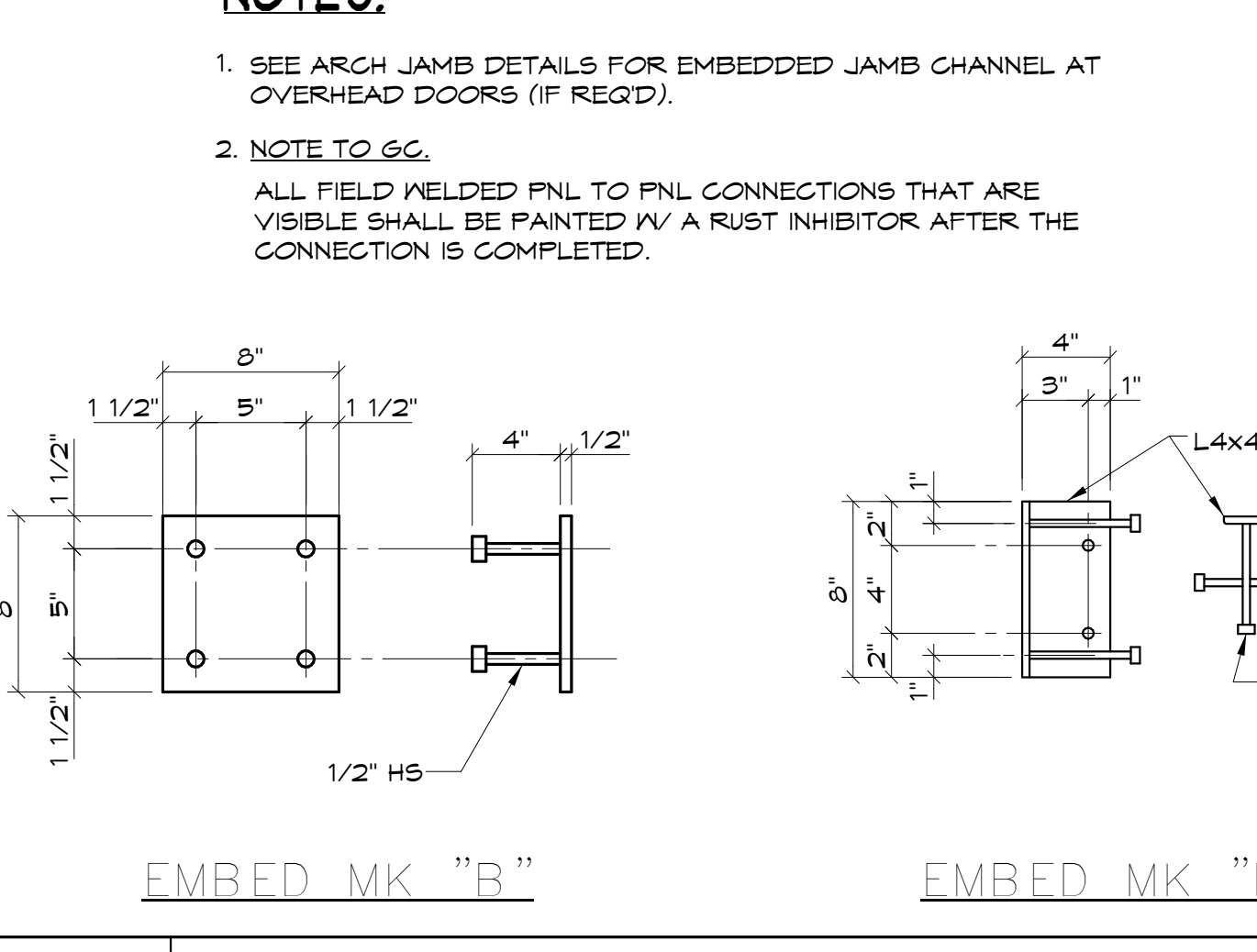
E12 DETAIL - BUTT CONN OUTSIDE CORNER
3/4" = 1'-0"



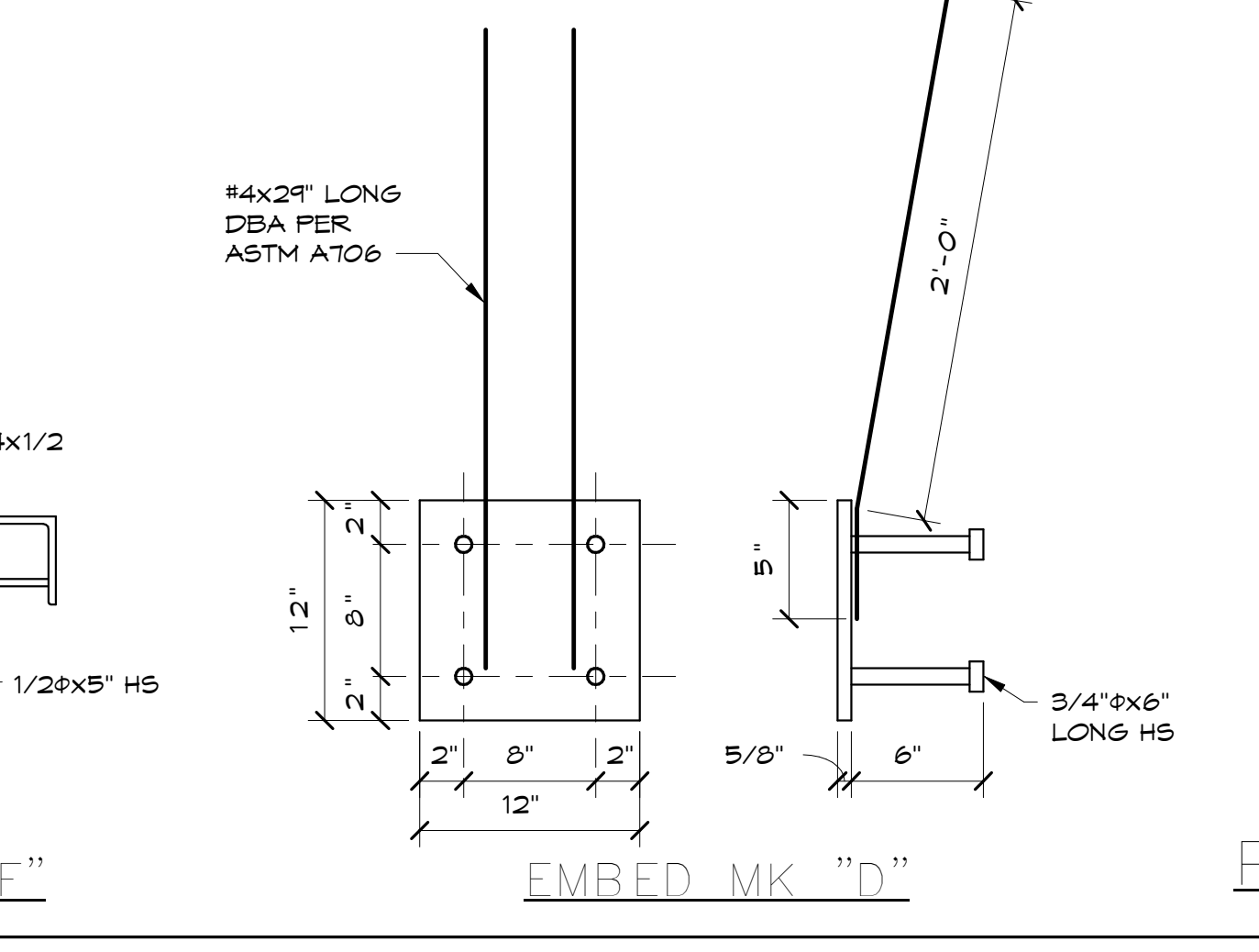
E15 DETAIL - BUTT CONN INSIDE CORNER
3/4" = 1'-0"



A1 DETAIL - MID-PANEL CONN
3/4" = 1'-0"



A4 EMBED PLATE DETAILS
3/4" = 1'-0"



A15 TYP EXPOSED CORNER CONNECTION
1 1/2" = 1'-0"

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RENEWAL DATE: 12/31/2023
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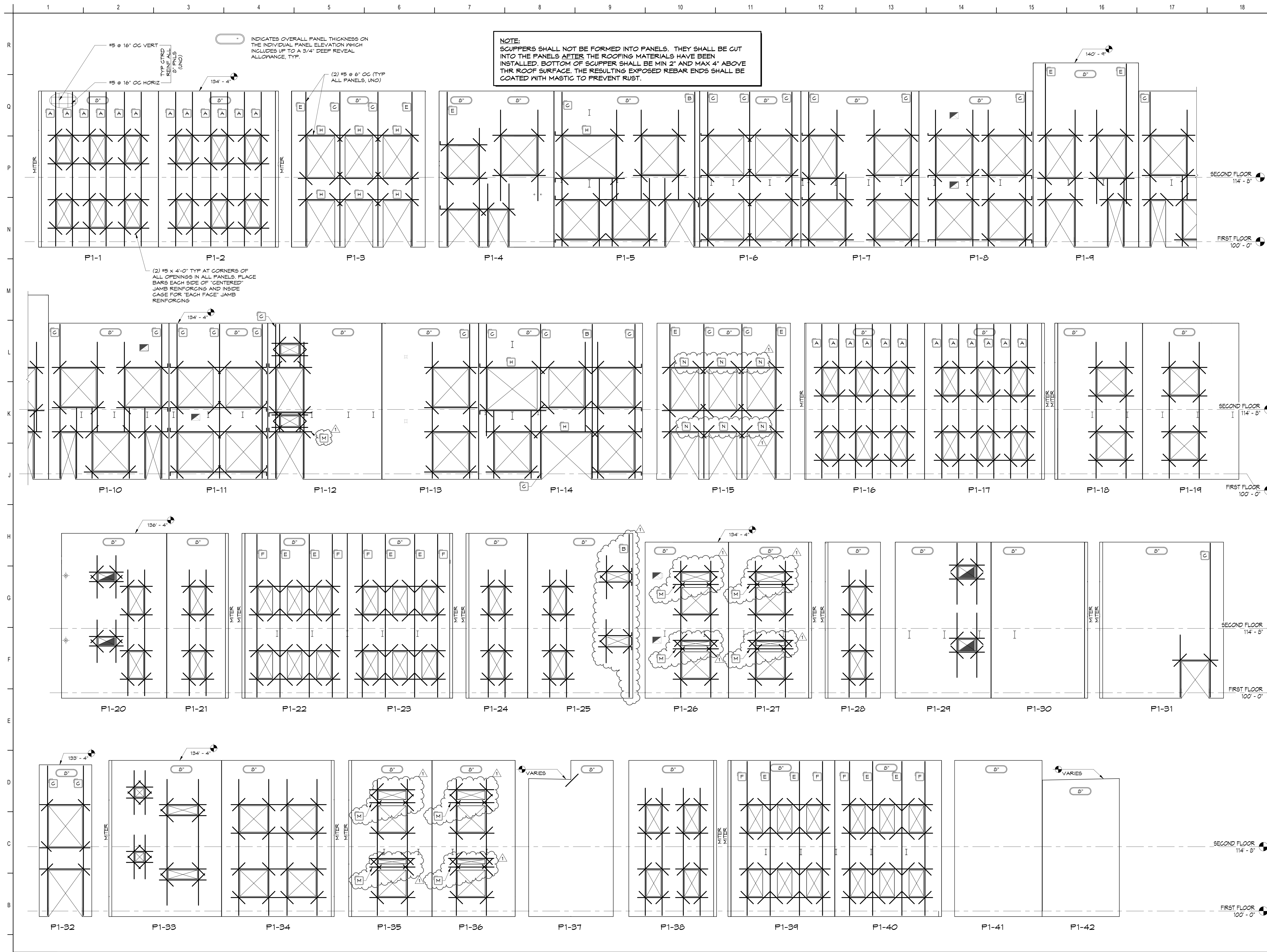
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TILT-UP REINFORCING, NOTES & DETAILS

S501

PHASE III - CONSTRUCTION DOCUMENTS



NOTE:
SCUPPERS SHALL NOT BE FORMED INTO PANELS. THEY SHALL BE CUT INTO THE PANELS AFTER THE ROOFING MATERIALS HAVE BEEN INSTALLED. BOTTOM OF SCUPPER SHALL BE MIN 2" AND MAX 4" ABOVE THR ROOF SURFACE. THE RESULTING EXPOSED REBAR ENDS SHALL BE COATED WITH MASTIC TO PREVENT RUST.

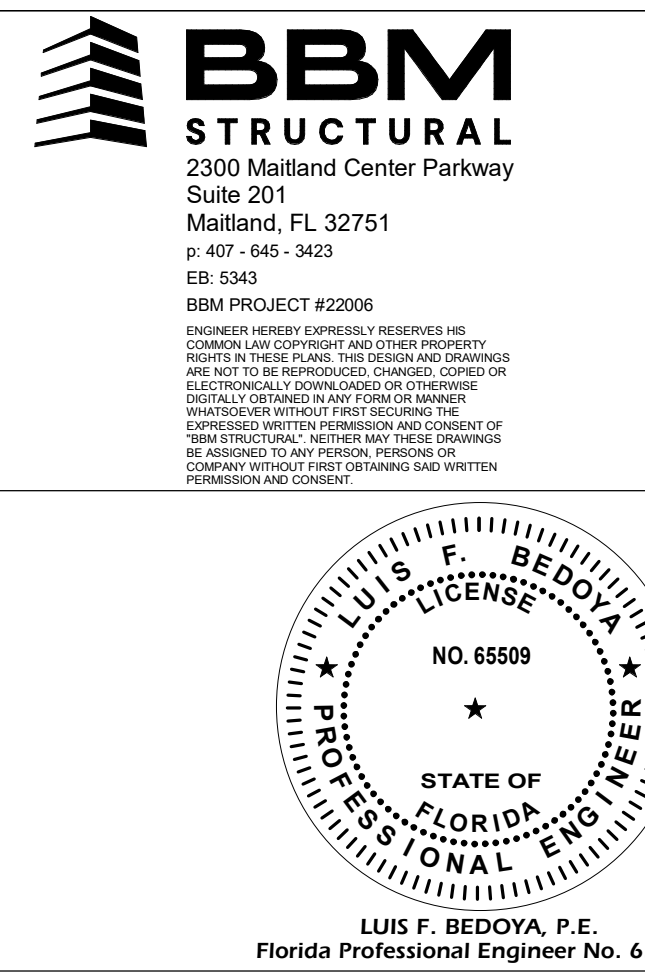
INDICATES OVERALL PANEL THICKNESS ON THE INDIVIDUAL PANEL ELEVATION WHICH INCLUDES UP TO A 3/4" DEEP REVEAL ALLOWANCE, TYP.

(2) #5 x 4'-0" TYP AT CORNERS OF ALL OPENINGS IN ALL PANELS. PLACE BARS EACH SIDE OF "CENTERED" JAMB REINFORCING AND INSIDE CASE FOR "EACH FACE" JAMB REINFORCING

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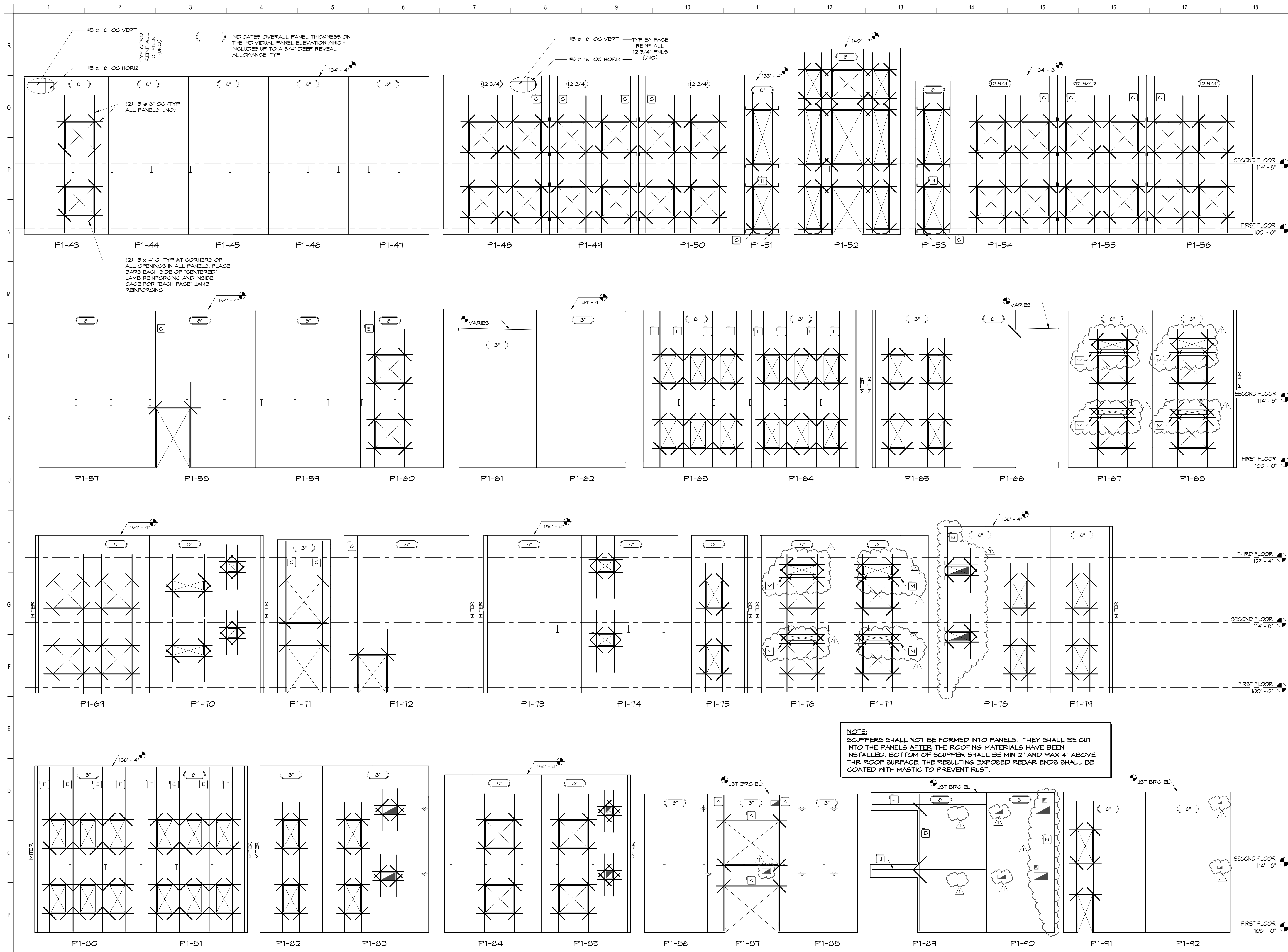
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TILT-UP PANEL ELEVATIONS - BUILDING 1

S502

A1 TILT-UP PANEL ELEVATIONS - BUILDING 1

1/8" = 1'-0"



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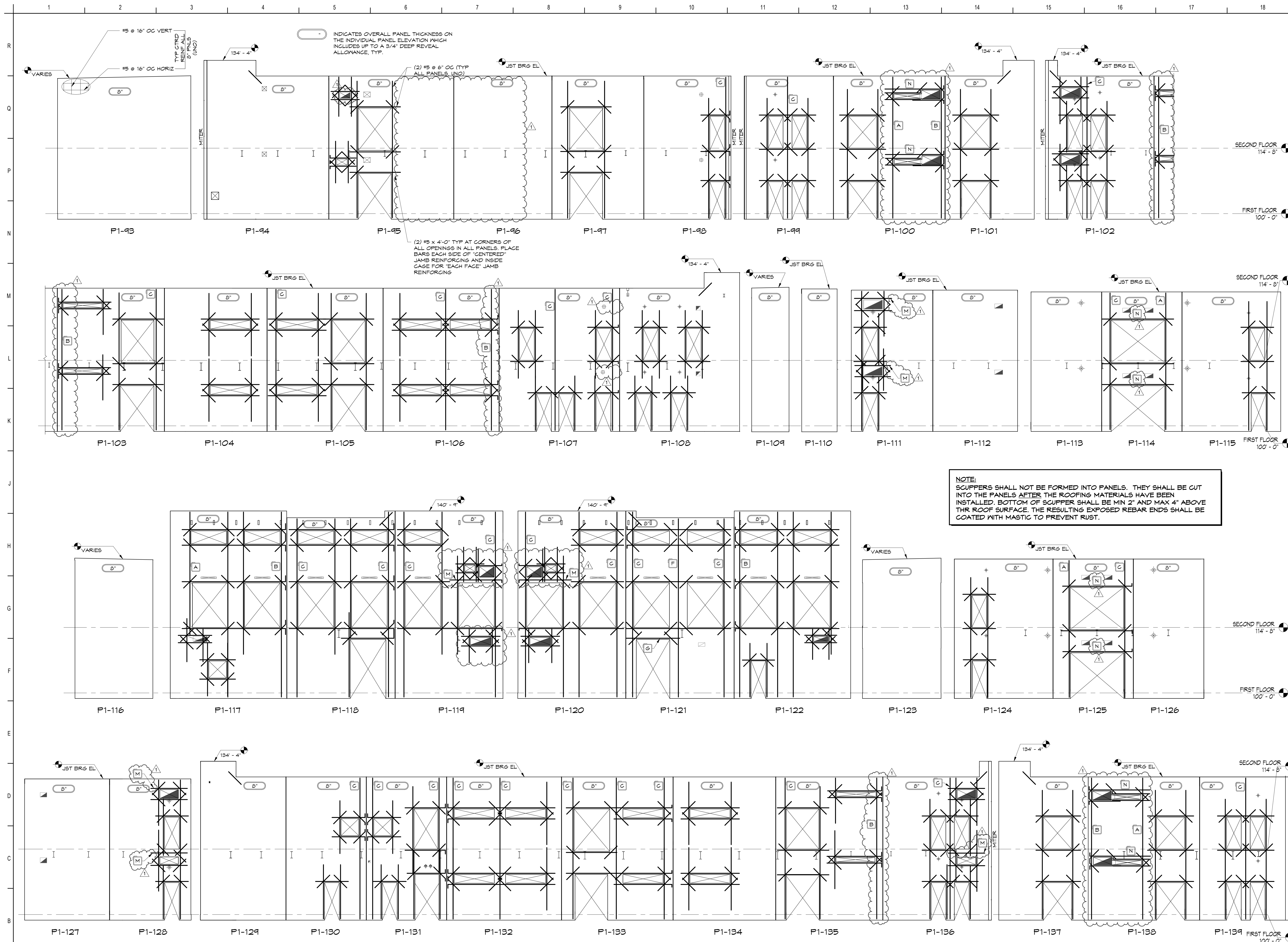
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S503



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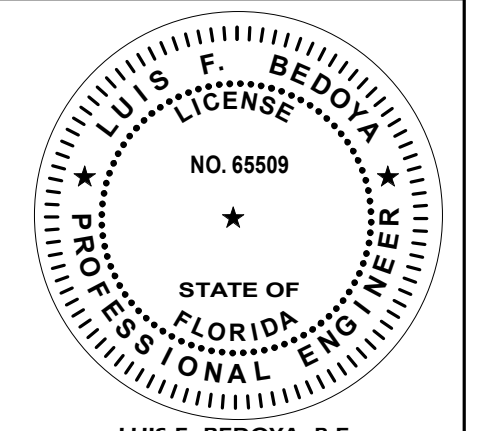
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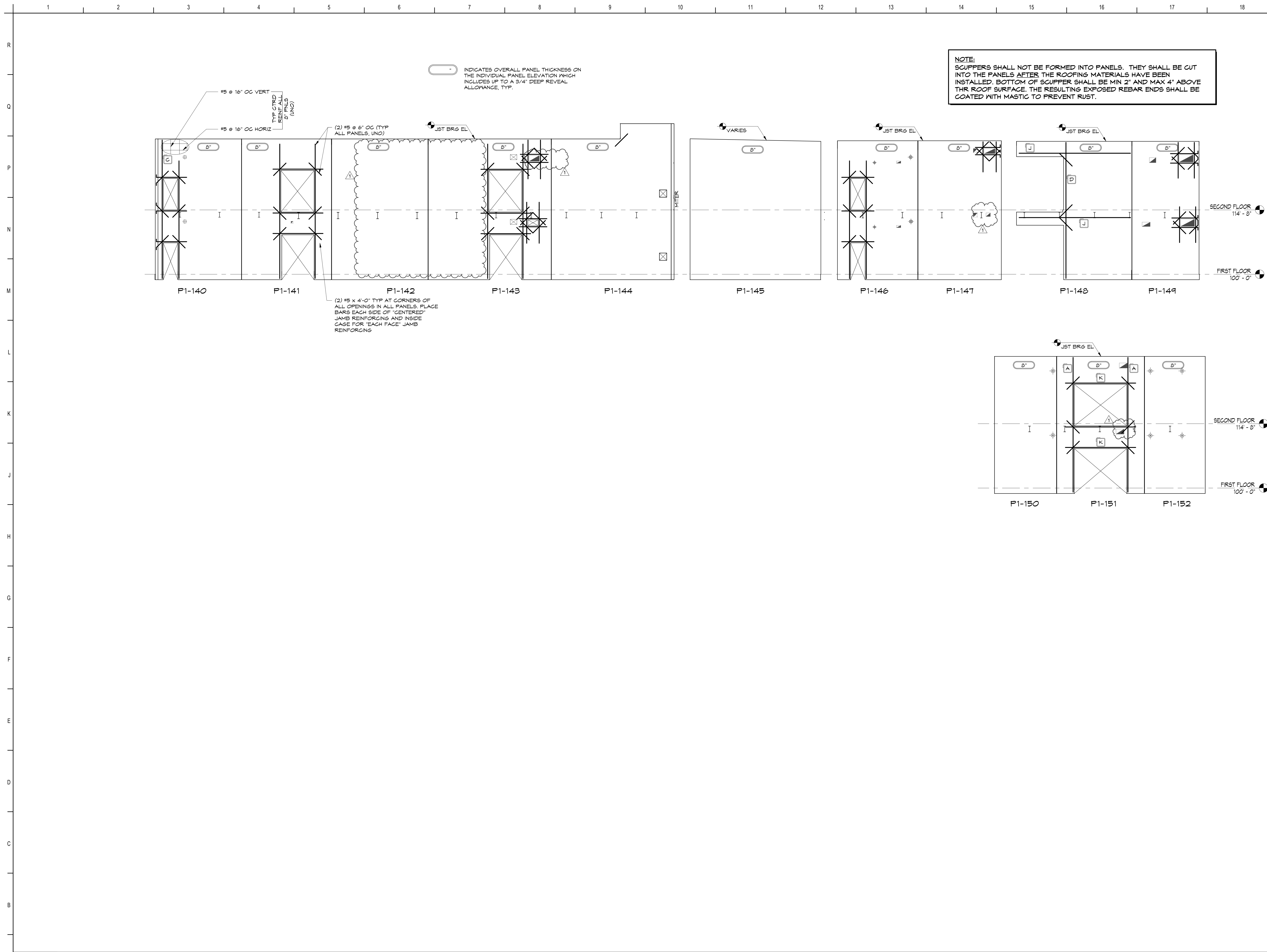
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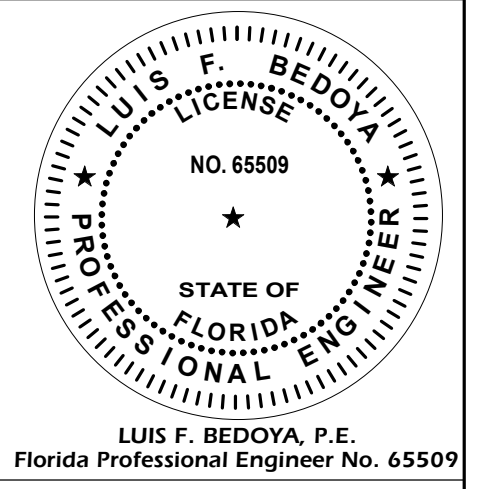
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NO. 65509 IN THE STATE OF FLORIDA
DATE: 12/16/2022
BY: LUIS F. BEDOYA, P.E.
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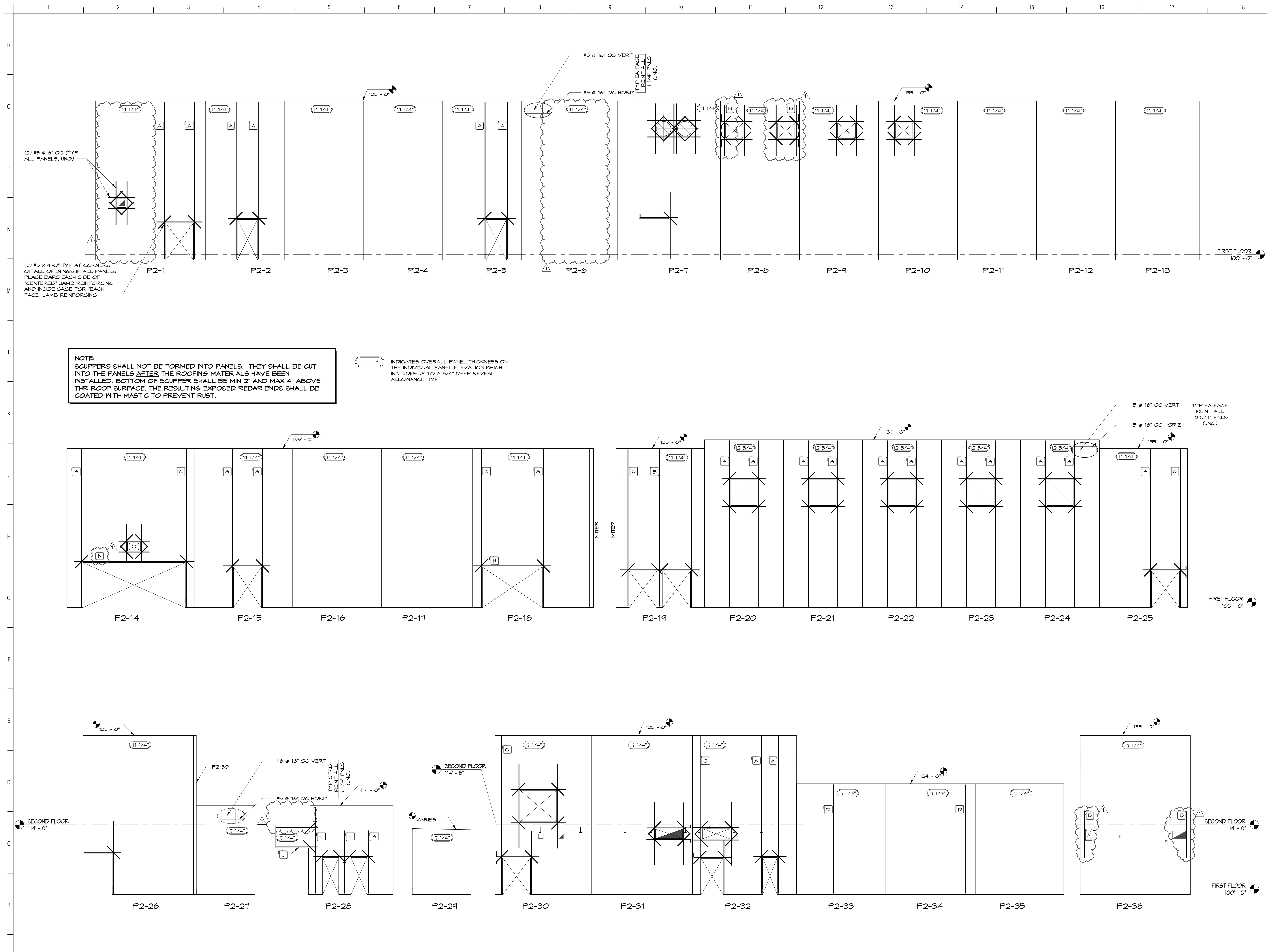
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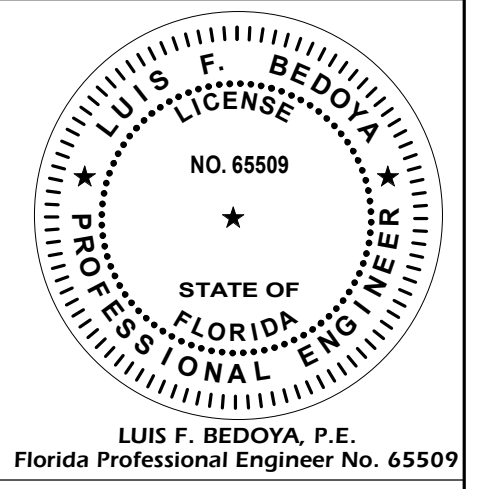
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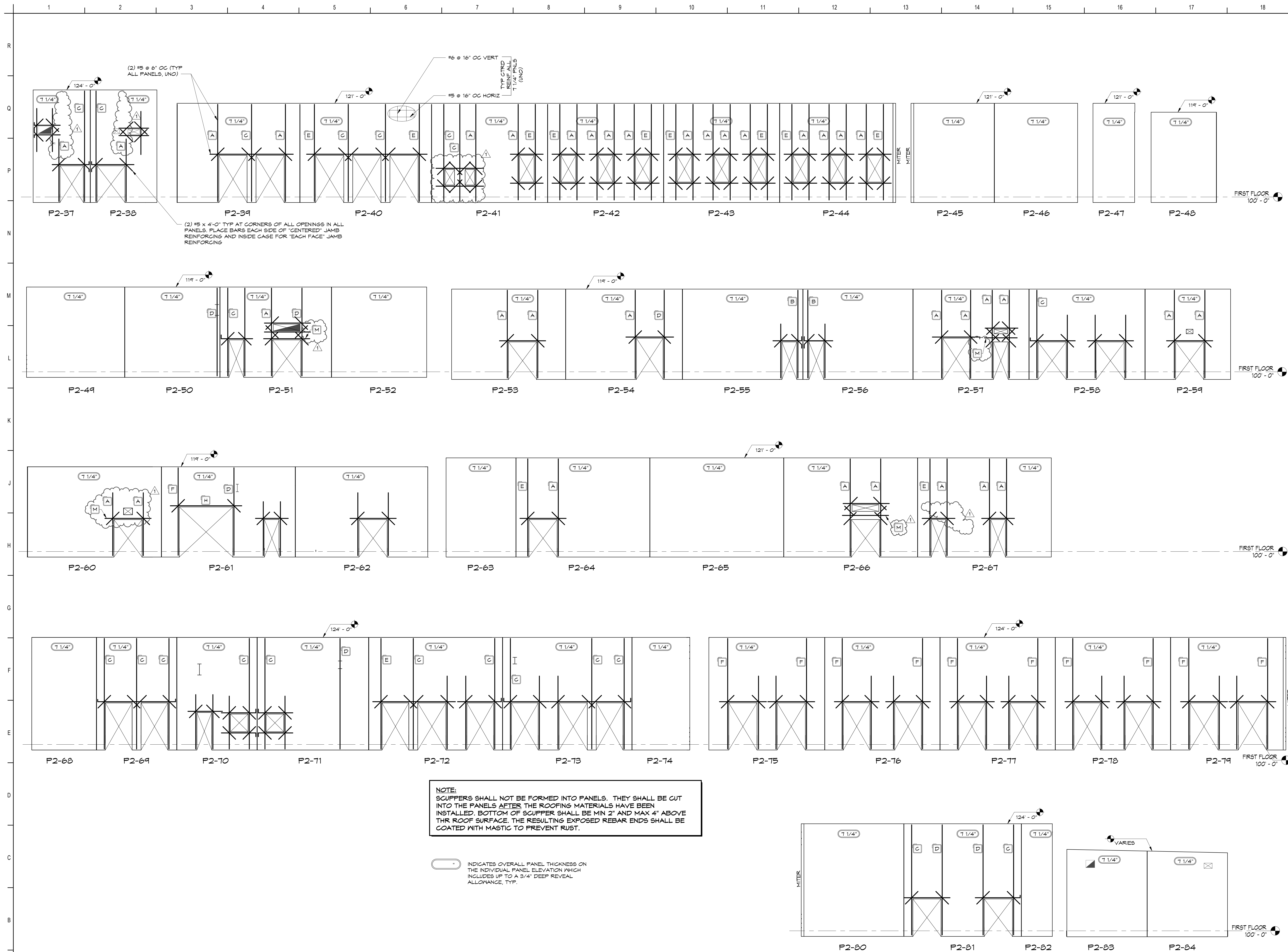
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ISSUE DATE: 12/16/2022
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TILT-UP PANEL ELEVATIONS - BUILDING 2

S506



NOTE:
SCUPPERS SHALL NOT BE FORMED INTO PANELS. THEY SHALL BE CUT INTO THE PANELS AFTER THE ROOFING MATERIALS HAVE BEEN INSTALLED. BOTTOM OF SCUPPER SHALL BE MIN 2" AND MAX 4" ABOVE THE ROOF SURFACE. THE RESULTING EXPOSED REBAR ENDS SHALL BE COATED WITH MASTIC TO PREVENT RUST.

○ INDICATES OVERALL PANEL THICKNESS ON THE INDIVIDUAL PANEL ELEVATION WHICH INCLUDES UP TO A 3/4" DEEP REVEAL ALLOWANCE, TYP.

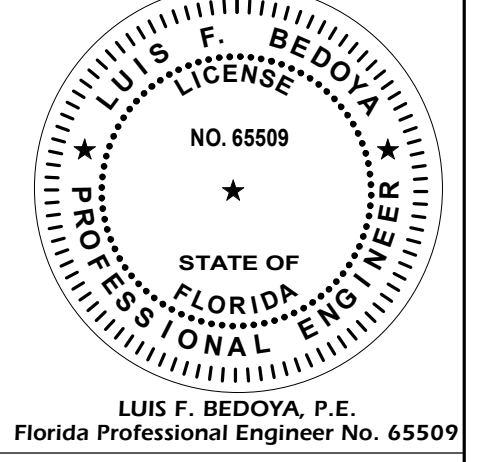
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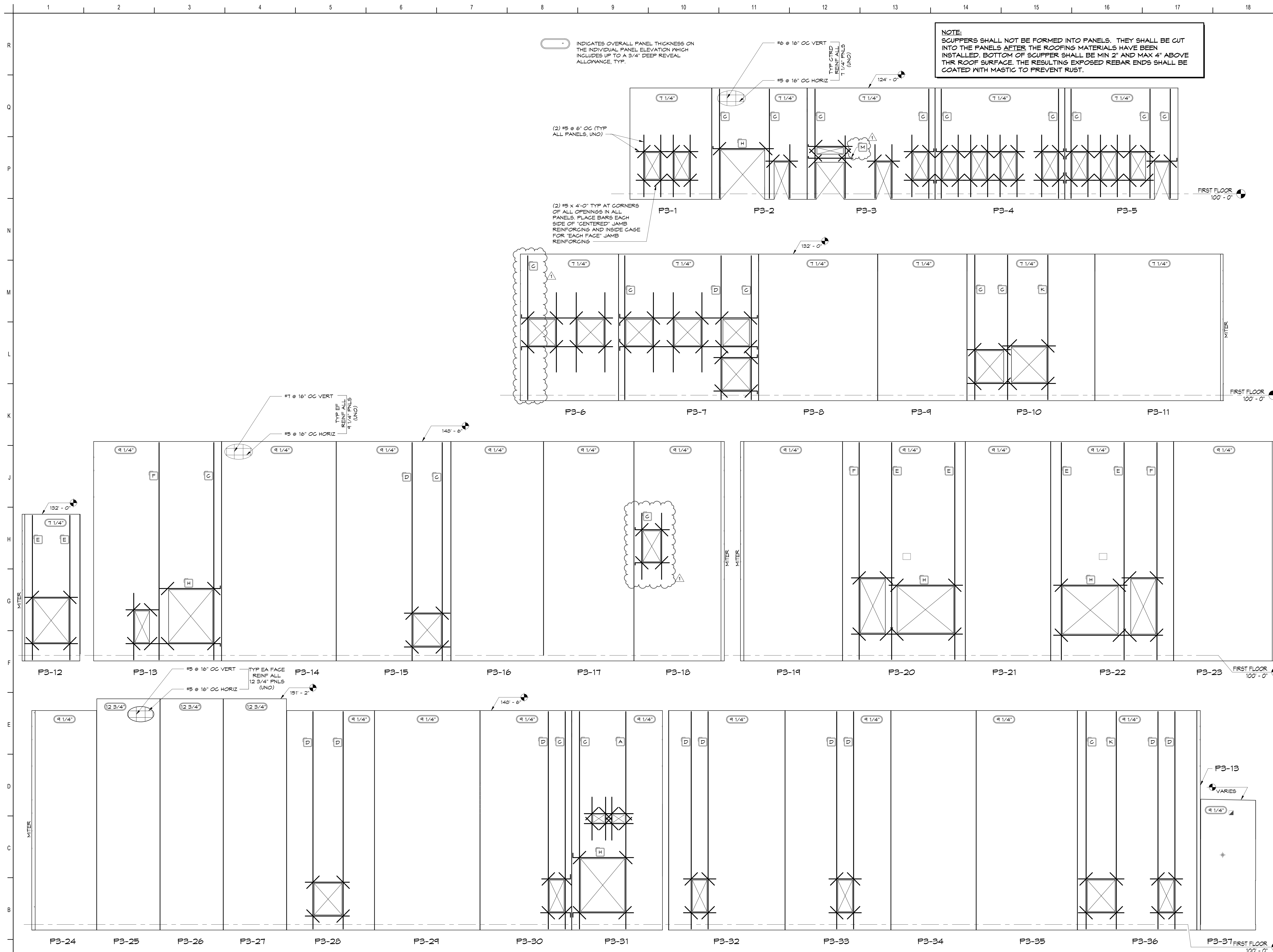
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TILT-UP PANEL ELEVATIONS - BUILDING 2

S507



NOTE:
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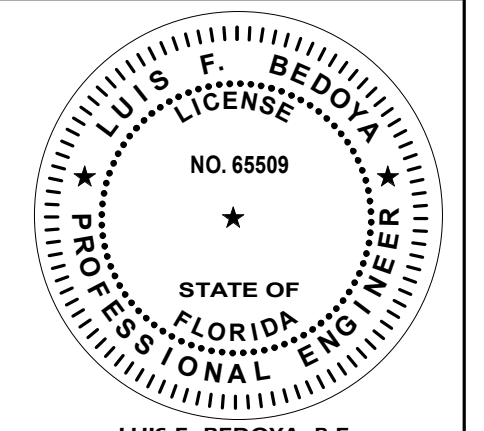
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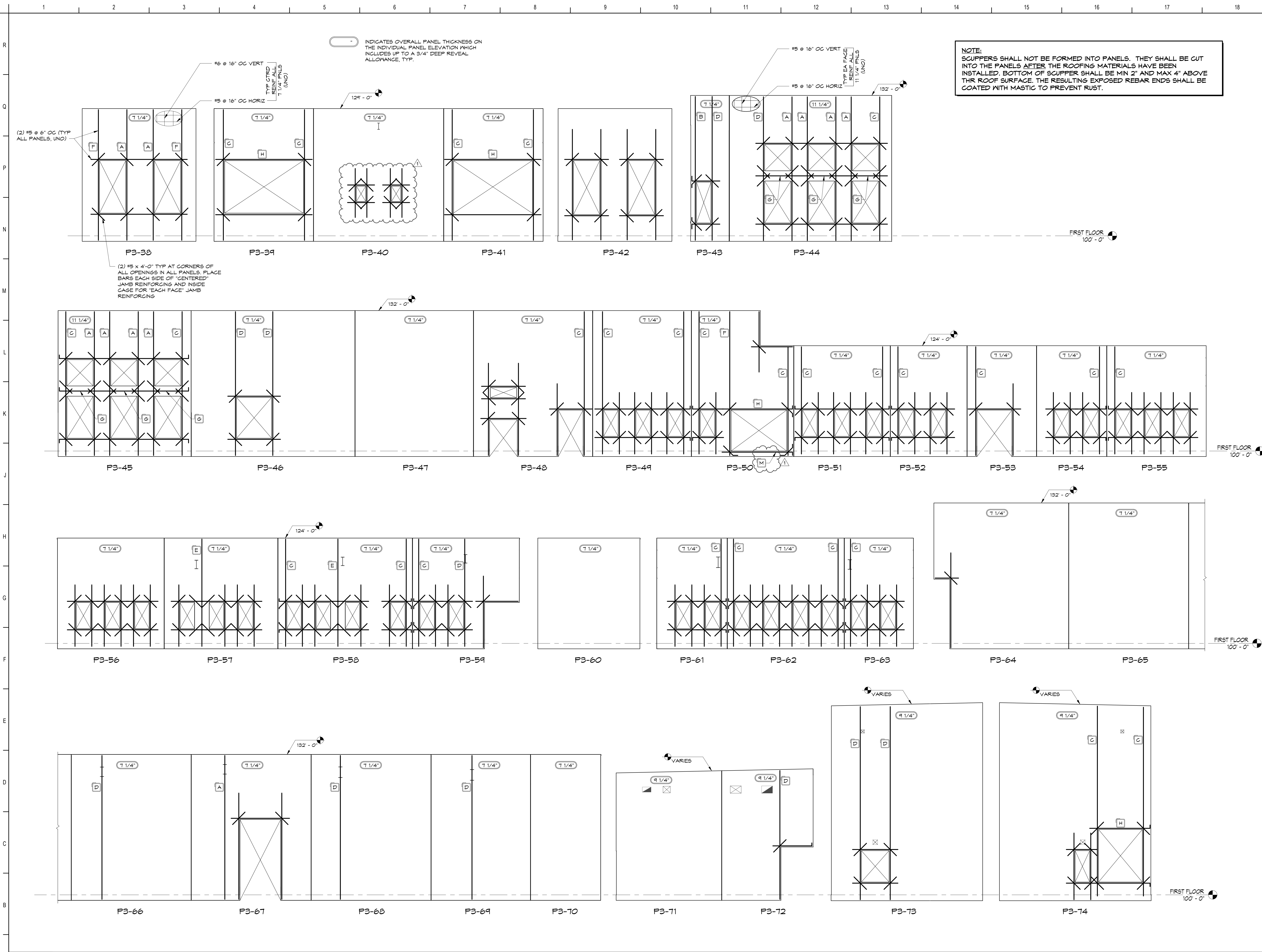
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TILT-UP PANEL ELEVATIONS - BUILDING 3

S508



NOTE:
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LUIS F. BEDOYA, P.E.
Florida Professional Engineer No. 65509

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TILT-UP PANEL ELEVATIONS - BUILDING 3

S509

A1 TILT-UP PANEL ELEVATIONS - BUILDING 3

1/8" = 1'-0"

