### **DESIGN CRITERIA**

THE STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 7TH EDITION. REFERENCE TO OTHER STANDARD SPECIFICATIONS OR CODES SHALL MEAN THE LATEST PUBLICATION.

FOUNDATIONS ARE DESIGNED FOR AN ALLOWABLE BEARING PRESSURE OF 3,000 PSF.

SUPERIMPOSED LIVE LOADS:	
ROOFS AND CANOPIES: (REDUCIBLE)	20 PSF
WIND LOADS:	
ULTIMATE WIND SPEED	
NOMINAL WIND SPEED	127 MPH
MEAN ROOF HEIGHT	18 FT
RISK CATEGORY	II
WIND EXPOSURE	C
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	± 0.18
DIRECTIONALITY FACTOR (Kd)	0.85
SHAPE FACTORS	PER CODE

THIS BUILDING IS LOCATED IN THE WIND BORNE DEBRIS REGION. IMPACT RESISTANT GLAZING IS REQUIRED.

### CONCRETE

3,000

ALL CONCRETE PROPORTIONING, MIXING, TRANSPORTATION, PLACING, AND CURING SHALL CONFORM TO ACI 301.

ALL CONCRETE SHALL BE LABORATORY DESIGNED AND CONTROLLED TO MEET THE REQUIREMENTS OF ACI 318 AND THE PROJECT'S DESIGN BUILDING CODE.

USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS IS CONCRETE IS PROHIBITED

THE AIR CONTENT IN ALL CONCRETE EXPOSED TO WEATHER SHALL BE BETWEEN 1% & 4%.

CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE AS NOTED BELOW, OR PER LATEST

ALL OTHER CONCRETE

CHAMFER ALL EXPOSED CORNER 3/4" MINIMUM.

NORMAL WEIGHT

ACI 318 FOR CONDITIONS OTHER THAN THOSE LISTED:

ALL HOOKS CALLED FOR IN STRUCTURAL DRAWINGS SHALL BE ACI STANDARD HOOKS, UNO.

REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60 UNO.

ALL WELDED WIRE MESH SHALL CONFORM TO ASTM A185. LAP TWO SQUARES AT SPLICES. DO NOT WELD REINFORCING STEEL UNLESS APPROVED IN WRITING BY THE STRUCTURAL ENGINEER.

TIE ALL REINFORCING STEEL AND EMBEDS SECURELY IN PLACE PRIOR TO PLACING CONCRETE. THE CONTRACTOR SHALL PROVIDE SUPPORTS TO MAINTAIN THE REQUIRED REINFORCING POSITION. "WET STICKING" DOWELS INTO CONCRETE IS NOT PERMITTED.

THE CONTRACTOR SHALL COMPARE THE STRUCTURAL PLANS AND DETAILS WITH THE ARCHITECTURAL PLANS AND DETAILS AND REPORT ANY DISCREPANCIES TO THE ARCHITECT PRIOR TO THE COMMENCEMENT OF SHOP DRAWINGS.

THE CONTRACTOR SHALL PROVIDE AN ALLOWANCE OF 1% OF TOTAL REINFORCING STEEL FOR THE PROJECT TO BE FABRICATED AND PLACED DURING CONSTRUCTION AT THE DIRECTION OF THE STRUCTURAL ENGINEER, IN ADDITION TO THE REINFORCING STEEL REQUIRED BY THE STRUCTURAL DRAWINGS. THE OWNER SHALL RECEIVE CREDIT FOR ANY UNUSED QUANTITY AT THE END OF THE PROJECT.

ALL REINFORCING SHALL HAVE AN ACI CLASS B SPLICE AT BAR LAPS.

### STEEL JOIST

ALL OPEN WEB JOIST AND JOIST GIRDERS SHALL BE FABRICATED, FURNISHED, AND ERECTED IN ACCORDANCE WITH THE STEEL JOIST INSTITUTE'S STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOISTS, LATEST EDITION.

THE DESIGN OF ALL STEEL JOIST AND JOIST GIRDERS SHALL BE BY THE MANUFACTURER'S ENGINEER. ALL JOIST AND JOIST GIRDERS ALONG WITH THE BRIDGING SHALL BE DESIGNED FOR THE GRAVITY LOADS OF THE PROJECT IN ADDITION TO THE WIND LOADS SHOWN ON THE STRUCTURAL DRAWINGS AND THE MECHANICAL LOADS SHOWN ON THE MECHANICAL DRAWINGS.

K SERIES JOISTS SHALL BEAR A MINIMUM OF 2 1/2" OVER STEEL SUPPORTS. LH SERIES JOISTS AND JOIST GIRDERS SHALL BEAR A MINIMUM OF 4" OVER STEEL SUPPORTS. BEARING PLATES AND ANGLES SHALL BE ADJUSTED TO PROVIDE LEVEL BEARING ON SUPPORT SURFACE.

## EXTEND BOTTOM CHORDS OF JOISTS AND JOIST GIRDERS AT COLUMNS.

SHOP DRAWINGS SHALL BE SUBMITTED COMPLETELY DETAILING THE JOISTS FOR ERECTION. INCLUDE ALL BRIDGING AND CONNECTIONS. CALCULATIONS AND SHOP DRAWINGS SHALL BE SIGNED, SEALED, AND DATED BY A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT.

PROVIDE A SHOP COAT OF STANDARD PRIMER PAINT. COLOR TO BE SPECIFIED BY THE ARCHITECT OR OWNER. PRIMER TO BE COMPATIBLE WITH FINISH COAT WHERE APPLICABLE. TOUCH UP DAMAGED AREAS WITH SAME PAINT USED FOR SHOP COAT.

## SUPPLEMENTARY NOTES

THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE DESIGN, ADEQUACY, AND SAFETY OF ERECTION, BRACING, SHORING, TEMPORARY SUPPORTS, ETC. THE STRUCTURAL ELEMENTS ARE NOT CONSIDERED STABLE UNTIL THE STRUCTURE IS COMPLETE.

REVIEW OF THE SUBMITTAL INFORMATION SHALL BE FOR GENERAL REQUIREMENTS OF THE PROJECT, AND SHALL NOT INCLUDE CHECKING OF DETAILED DIMENSIONS OR DETAILED QUANTITIES, NOR REVIEW OF THE CONTRACTOR'S SAFETY MEASURES ON OF OFF THE WORKSITE OR THE MEANS AND METHODS OF DOING ANY WORK.

THE CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS AND ARCHITECTURAL AND STRUCTURAL PLAN DIMENSIONS AND ELEVATIONS PRIOR TO THE COMMENCEMENT OF CONSTRUCTION AND NOTIFY THE STRUCTURAL ENGINEER OF ANY CONFLICTS.

PLUMBING EQUIPMENT SHALL BE VERIFIED WITH EQUIPMENT PURCHASED BEFORE PROCEEDING WITH STRUCTURAL WORK AFFECTED.

ALL STRUCTURAL OPENINGS AROUND OR AFFECTED BY MECHANICAL, ELECTRICAL AND

ANY ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL BEAR THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF THE PROJECT.

GENERAL CONTRACTOR MUST REVIEW AND APPROVE SHOP DRAWINGS PRIOR TO SUBMITTAL TO ARCHITECT/ENGINEER. SUBMITTALS WHICH DO NOT CONTAIN THE CONTRACTOR'S SHOP DRAWING STAMP OR HAVE BEEN MERELY "RUBBER STAMPED" SHALL BE RETURNED WITHOUT REVIEW. CONTRACTOR SHOULD ALLOW TWO WEEKS FOR SUBMITTAL REVIEW.

CONTRACTOR SHALL NOT ORDER MATERIAL OR COMMENCE WITH CONSTRUCTION UNTIL SHOP DRAWINGS HAVE BEEN REVIEWED AND APPROVED. WORK DONE OR ORDERS PLACED BEFORE SHOP DRAWING APPROVAL IS AT THE CONTRACTOR'S RISK.

THE PROJECT'S STRUCTURAL DRAWINGS AND ELECTRONIC FILES ARE THE PROPERTY OF LOWE STRUCTURES, INC. THE CONTRACTOR AND SUBCONTRACTOR SHALL COMPENSATE LOWE STRUCTURES, INC. FOR THE USE OF THE PROJECT'S CAD FILES FOR ANY PURPOSE INCLUDING SHOP DRAWING PREPARATION.

THE CONTRACTOR SHALL NOT SCALE DRAWINGS. DIMENSIONS SHOWN ON ARCHITECTURAL

# TILT-UP WALL PANEL NOTES

DESIGN AND DETAILING OF ALL TILT-UP WALL PANELS SHALL BE PROVIDED BY

DELEGATED SPECIALITY ENGINEER.

SIGNED AND SEALED SHOP DRAWINGS OF ALL TILT-UP SHALL BE SUBMITTED TO EOR FOR REVIEW. SHOP DRAWINGS SHALL SPECIFY ALL PANEL THICKNESS, DIMENSIONS, REINFORCING, EMBEDS, HOLDOWNS, DOWELS, AND JOINT LOCATIONS.

### **FOUNDATIONS**

A GEOTECHNICAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT SHALL INSPECT AND ASSURE THE ADEQUACY OF ALL SUBGRADES, FILLS, AND BACKFILLS BEFORE PLACEMENT OF FOUNDATIONS, FOOTINGS, SLABS, ETC. WRITTEN ACCEPTANCE OF THE WORK INSPECTED AND VERIFICATION OF ASSUMED SOIL BEARING PRESSURE SHALL BE SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER.

WHERE FOOTING STEPS ARE REQUIRED, THE STEPS SHALL BE NO STEEPER THAN ONE VERTICAL TO TWO HORIZONTAL, UNO ON PLAN. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING THE REQUIRED FOOTING STEP LOCATIONS.

FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE PROJECT'S GEOTECHNICAL REPORT PREPARED BY TSF DATED AUGUST 27, 2021.

### <u>STEEL</u>

ALL STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING:

ASTM A992 FOR W-SHAPES ASTM A36 FOR PLATES, ANGLES, & MISC FRAMING ASTM A588, GRADE 50, WHERE NOTED ASTM A501 OR A53 FOR PIPE ASTM A500, GRADE B FOR TUBES

WELDED PER MANUFACTURER'S RECOMMENDATIONS.

OR BOLTED WITH VERTICAL BOLT SPACING OF 3".

HEADED STUDS SHALL COMPLY WITH ASTM A108 AND SHALL BE AUTOMATICALLY END

ANCHOR RODS SHALL CONFORM TO ASTM F1554. GRADE 36, UNO.

ALL CONNECTION BOLTS SHALL CONFORM TO ASTM A325.

WELDING OF STRUCTURAL STEEL SHALL BE PER AWS D1.1. LATEST EDITION.

WHERE FILLET WELD SIZE ISN'T CALLED-OUT ON WELD SYMBOL, FILLET SIZE SHALL BE 1/16" SMALLER THAN THE THICKNESS OF THE THINNER OF THE ELEMENTS TO BE WELDED.

UNLESS NOTED OTHERWISE PROVIDE THE GREATER OF THE FOLLOWING BEAM END

CONNECTIONS:

1) MINIMUM 5/16" DOUBLE ANGLE SHEAR CONNECTION, FULL DEPTH OF THE BEAM, WELDED

2) WHERE BEAM REACTIONS ARE SHOWN, CONNECTIONS SHALL DEVELOP THE REACTION

3) WHEN BEAM REACTIONS ARE NOT SHOWN, CONNECTIONS SHALL BE PROPORTIONED TO SUPPORT 60% OF THE TOTAL UNIFORM LOAD CAPACITY SHOWN IN THE UNIFORM LOAD TABLES OF THE AISC MANUAL FOR THE GIVEN BEAM, SPAN, AND GRADE OF STEEL USED. FOR COMPOSITE BEAMS THE PROPORTION CONNECTIONS FOR 90% OF THE UNIFORM LOAD CAPACITY.

4) CONNECTIONS SHALL BE PROPORTIONED FOR THE ECCENTRICITY BETWEEN CENTROIDS OF THE CONNECTION AND THE SUPPORTING MEMBER.

STRUCTURAL STEEL SHALL BE SHOP PRIMED PER SSPC PAINT SYSTEM NO. 7.00. PRIMER SHALL BE SSPC PAINT WITH A MINIMUM DRY FILM THICKNESS OF 2.0 MILS. OMIT PAINT AT SURFACES TO BE FIREPROOFED. TOUCH-UP BOLT HEADS, NUTS, FIELD WELDS, AND ABRASIONS IN SHOP PRIMER WITH SAME PAINT.

ALL STEEL EXPOSED TO WEATHER SHALL BE HOT-DIPPED GALVANIZED.

IF ANY DISCREPANCIES BETWEEN SPECIFICATIONS, NOTES, AND DRAWINGS ARE NOTED, THE MORE STRINGENT REQUIREMENT GOVERNS.

COLUMN ANCHOR BOLTS ARE DESIGNED FOR COMPLETE CONDITION ONLY. CONTRACTOR SHALL PROVIDE ALL TEMPORARY BRACING REQUIRED TO MAINTAIN STABILITY UNTIL ALL BRACING ELEMENTS REQUIRED FOR STRUCTURAL STABILITY ARE IN PLACE.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF MISC STEEL SHOWN ON THE STRUCTURAL DRAWINGS SUCH AS SHELF ANGLES, LINTELS, SUPPORT MEMBERS FOR CURTAIN WALL OR MASONRY AND EDGE ANGLES FOR OPENINGS AND PERIMETER CONDITIONS. IT IS THE INTENT OF THESE DRAWINGS THAT THESE ITEMS ARE FIELD ATTACHED BY FIELD WELDING OR FIELD BOLTING TO MEET TOLERANCES REQUIRED BY THE OTHER TRADES, THESE TOLERANCES MAY BE MORE STRINGENT THAN AISC TOLERANCES.

THE CONTRACTOR SHALL COMPARE THE STRUCTURAL PLANS AND DETAILS WITH THE ARCHITECTURAL PLANS AND DETAILS AND REPORT ANY DISCREPANCIES TO THE ARCHITECT PRIOR TO THE COMMENCEMENT OF SHOP DRAWINGS.

THE CONTRACTOR SHALL PROVIDE AN ALLOWANCE OF 1% OF TOTAL STRUCTURAL STEEL FOR THE PROJECT TO BE FABRICATED AND INSTALLED DURING CONSTRUCTION AS DIRECTED BY THE STRUCTURAL ENGINEER IN ADDITION TO THE STRUCTURAL STEEL REQUIRED BY THE CONSTRUCTION DRAWINGS. THE OWNER SHALL RECEIVE CREDIT FOR ANY UNUSED QUANTITY AT THE COMPLETION OF THE PROJECT.

## POST INSTALLED ANCHORS

POST- INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE EOR PRIOR TO INSTALLING POST- INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. HOLES SHALL BE DRILLED AND CLEANED IN ACCORDANCE WITH THE MANUFACTURERS WRITTEN INSTRUCTIONS. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE SPECIFIED BELOW SHALL BE SUBMITTED BY THE CONTRACTOR TO THE EOR ALONG WITH CALCULATIONS THAT ARE PREPARED & SEALED BY A REGISTERED PROFESSIONAL ENGINEER. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERTINENT EQUIVALENT PERFORMANCE VALUES (MINIMUM) OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S) AS REQUIRED BY THE BUILDING CODE. PROVIDE CONTINUOUS SPECIAL INSPECTION FOR ALL MECHANICAL AND ADHESIVE ANCHORS PER THE APPLICABLE EVALUATION REPORT.

• MECHANICAL ANCHORS: SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193 FOR CRACKED AND UNCRACKED CONCRETE RECOGNITION. PRE-APPROVED MECHANICAL ANCHORS INCLUDE:

-SIMPSON STRONG TIE "TITEN-HD"

-SIMPSON STRONG TIE "TITEN"

CONCRETE RECOGNITION. PRE-APPROVED ADHESIVE ANCHORS INCLUDE:

-SIMPSON STRONG TIE "POWER DRIVEN FASTENERS"

-HILTI "KWIK BOLT 3"
-DEWALT "SCREW-BOLT +"

• ADHESIVE ANCHORS: SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308 FOR CRACKED AND UNCRACKED

-SIMPSON STRONG TIE "TORQ-CUT"

-SIMPSON "SET - XP" -SIMPSON "SET-3G" -DEWALT "PURE 110+"

-HILTI "HY 200-A"

• POWDER AND GAS ACTUATED FASTENERS: SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC70. PRE-APPROVED MECHANICAL ANCHORS INCLUDE:

-SIMPSON STRONG TIE "GAS ACTUATED FASTENERS"

MASONRY ANCHORS
 MECHANICAL ANCHORS: SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC01 OR AC106. PRE-APPROVED MECHANICAL ANCHORS INCLUDE:

DE : -SIMPSON STRONG TIE "TITEN-HD" -SIMPSON STRONG TIE "TITEN"

-SIMPSON STRONG TIE "TTT -BUILDEX "TAPCON" -HILTI "KWIK BOLT 3"

-DEWALT "SCREW-BOLT +"

• ADHESIVE ANCHORS: SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC58. PRE-APPROVED ADHESIVE ANCHORS INCLUDE

-HILTI "HY 270" -SIMPSON "SET-XP" -SIMPSON "200-A" -DEWALT "AC100+ GOLD"

ANCHORS INSTALLED IN CONCRETE MASONRY SHALL BE INSTALLED INTO SOLID GROUTED

ADHESIVE ANCHORS SHALL BE INSTALLED WITH 12 BAR DIAMETER EMBEDMENT MINIMUM

### METAL DECK

UNLESS NOTED OTHERWISE ON PLANS, THE METAL DECK SHALL BE GALVANIZED (G60 MINIMUM) CONFORMING TO THE STEEL DECK INSTITUTE'S SPECIFICATIONS, LATEST

SEE STRUCTURAL DRAWINGS FOR METAL DECK TYPE AND GAGE.

THE HANGING OF ANY LOADS INCLUDING MECHANICAL EQUIPMENT FROM THE ROOF DECK IS PROHIBITED.

ALL METAL ROOF DECK SHALL BE FASTENED TO THE STEEL SUPPORTS AT END OF PANEL AND AT ALL INTERMEDIATE SUPPORTS WITH 5/8"  $\varnothing$  PUDDLE WELDS IN A 36/7 PATTERN, SEE 2/S0.1.

DECKING SHALL BE CONTINUOUS OVER 3 SPANS MINIMUM WHERE ALLOWABLE WITH SUPPORTING STRUCTURE.

TOUCH UP ALL AREAS DAMAGED DURING INSTALLATION WITH REPAIR PAINT MATCHING THE SHOP COAT.

#### LIGHT GAGE METAL FRAMING

DESIGN OF LIGHT GAGE STEEL FRAMING IS BASED ON SECTION PROPERTIES AND STANDARD NOMENCLATURE AS DEFINED IN "STEEL STUD MANUFACTURERS ASSOCIATION (SSMA)-PRODUCT TECHNICAL INFORMATION, ICBO ER-4943P." ALTERNATE MANUFACTURER'S FRAMING SIZE SHALL MEET THE MINIMUM SECTION PROPERTIES OF THE MEMBERS INDICATED ON THE DESIGN DRAWINGS.

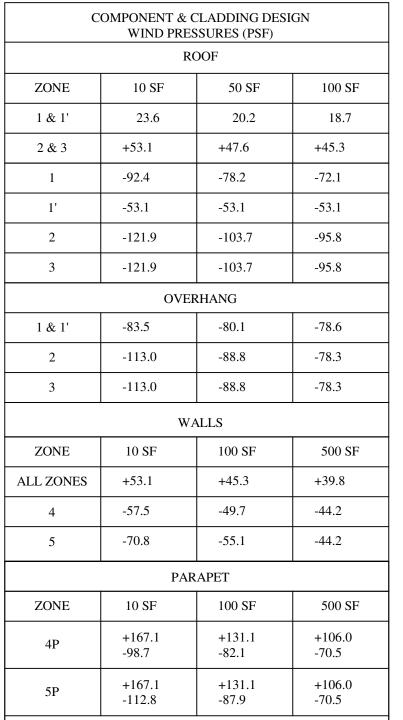
THE LIGHT GAGE STEEL FRAMING SHOWN ON THE DRAWINGS IS DIAGRAMMATIC ONLY. THE STRUCTURAL DESIGN OF LIGHT GAGE METAL FRAMING SHALL BE PERFORMED BY A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT. SHOP DRAWINGS AND CALCULATIONS BEARING THE SEAL OF THE REGISTERED ENGINEER SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR APPROVAL. SUBMITTED SHOP DRAWINGS SHALL INDICATE THE MEMBER GAGES, SPACING, AND SIZES IN ADDITION TO ALL CONNECTION DETAILS. NO FABRICATION SHALL BEGIN UNTIL THE SHOP DRAWINGS HAVE BEEN APPROVED.

ALL LIGHT GAGE STEEL FRAMING SHALL BE DESIGNED IN ACCORDANCE WITH AMERICAN IRON AND STEEL INSTITUTE (AISI) "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS.

ALL FRAMING MEMBERS SHALL BE FORMED FROM STEEL WITH A MINIMUM YIELD STRENGTH OF 33 KSI FOR 33 AND 43 MILL AND 50KSI FOR 54, 68, AND 97 MILL MATERIAL.

WALLS COMPOSED OF LIGHT GAGE STEEL STUDS SHALL HAVE LATERAL BRACING INSTALLED AT A MAXIMUM SPACING OF 48".

ALL FRAMING SHALL BE GALVANIZED, ASTM924 COATING (MIN).



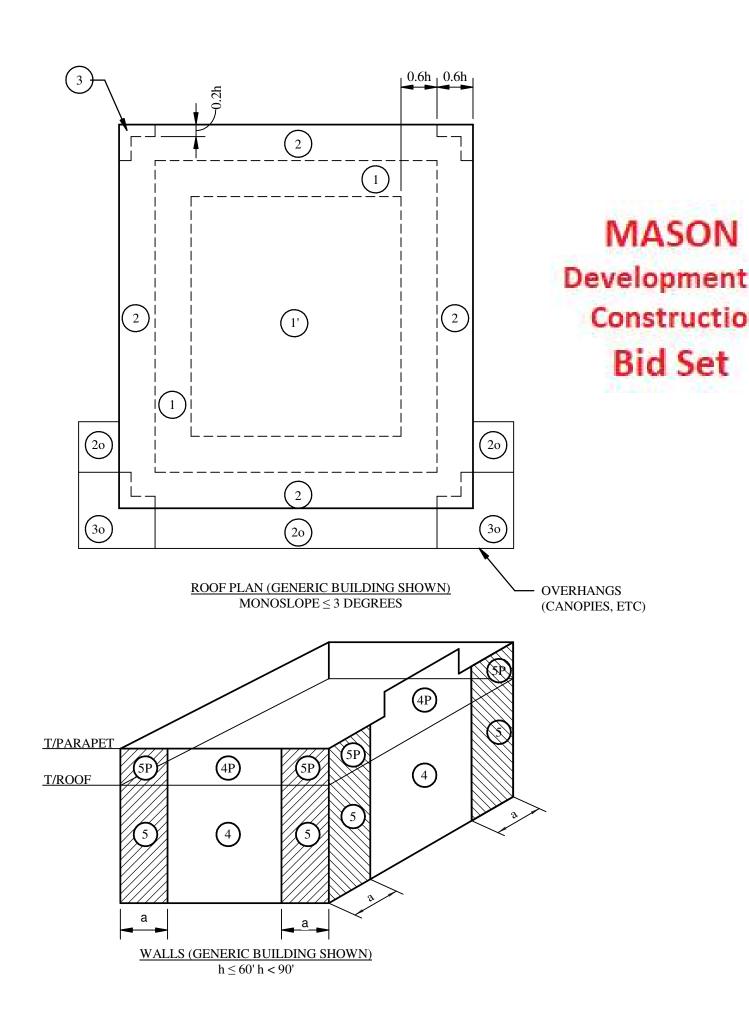
NOTES:

1. TABLE PRESSURES ARE FOR THE SQUARE FOOT (SF)
TRIBUTARY AREA SHOWN. FOR OTHER TRIBUTARY
AREAS, LINEARLY INTERPOLATE BETWEEN VALUES

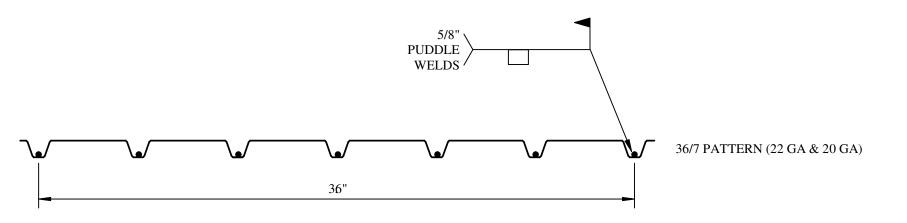
a = 6'-7''

2. POSITIVE PRESSURES ACT TOWARD THE BUILDING.
NEGATIVE PRESSURES ACT AWAY FROM THE
BUILDING.

SEE DIAGRAMS FOR ZONE LOCATIONS.
 ALL PRESSURES SHOWN ARE ULTIMATE PRESSURES.







## TYP WELD PATTERN FOR ROOF DECK

ROOF DECK ATTACHMENT NOTES:

1. ATTACHMENT AT SUPPORTS SHALL BE PUDDLE WELDS.

2. ATTACHMENT AT SIDE LAPS SHALL BE #10 TEKS SCREWS @ 22 GA

 ATTACHMENT AT SIDE LAPS SHALL BE #10 TEKS SCREWS @ 22 GA DECK AND WELDS AT 20 GA DECK.
 ATTACHMENT AROUND OPENINGS SHALL BE WELDS @ 6" OC.
 ATTACHMENT AT SIDE SUPPORTS ALONG PERIMETER SHALL BE

5. MINIMUM SHEET LAP SHALL BE 3" TYP.6. PROVIDE THE FOLLOWING SIDELAPS PER SPAN:

WELDS @ 6" OC.



This item has been electronically sealed by J. Matthew Lowe, P.E. using a digital signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.



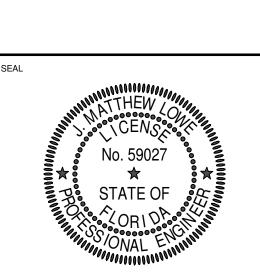
RETAIL 2 - SHELL

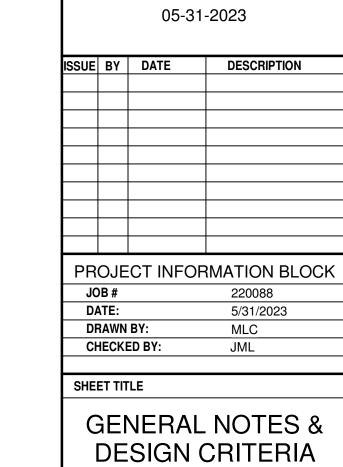
OO SW DISCOVERY WAY

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RFORMED BY SUBCONTRACTORS OR OTHER TRADES WITH THE DESIGN

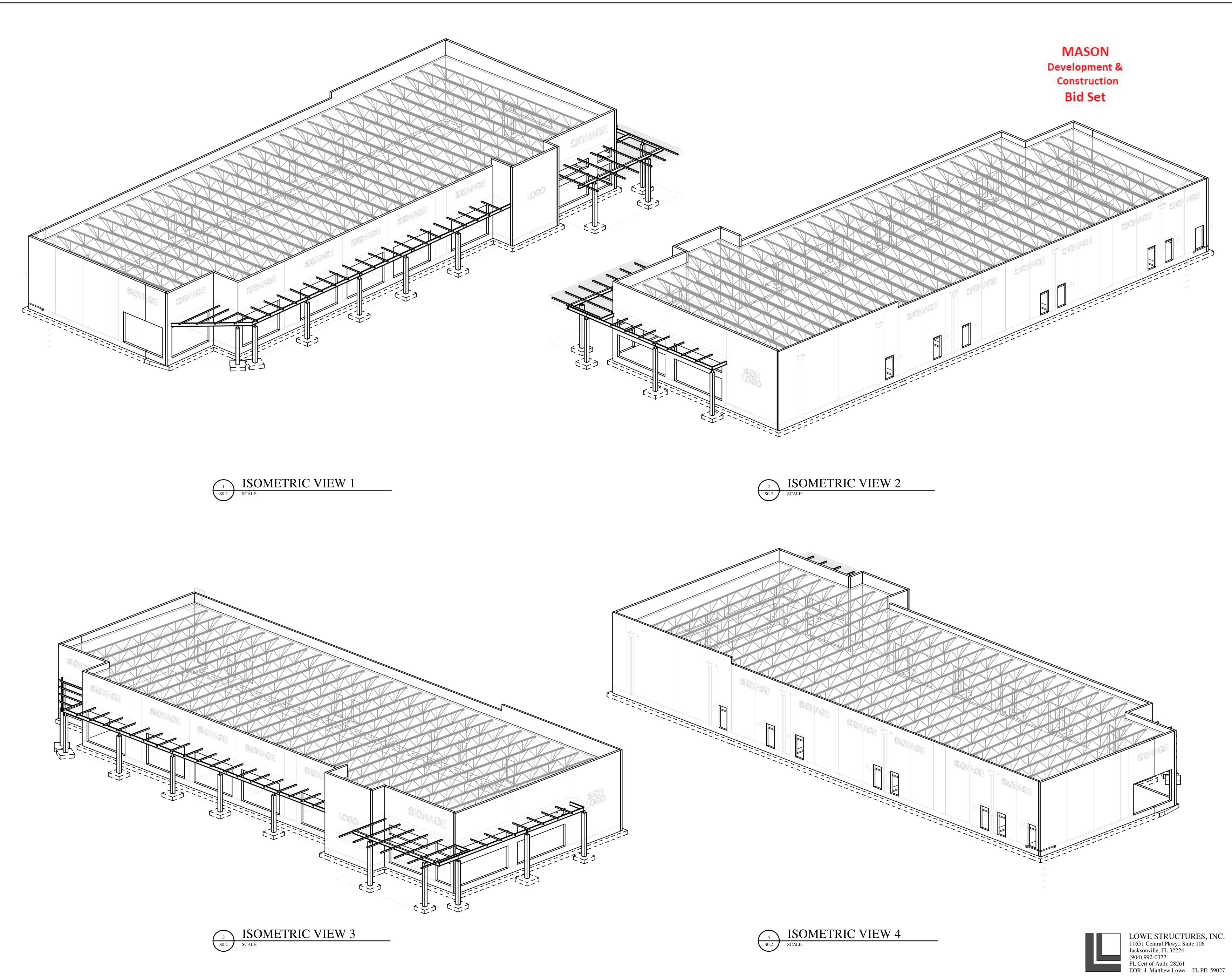
KEYPLAN





LOWE STRUCTURES, INC. 11651 Central Pkwy., Suite 106 Jacksonville, FL 32224 (904) 992-0377 FL Cert of Auth: 28261 EOR: J. Matthew Lowe FL PE: 59027

SHEET NUMBER

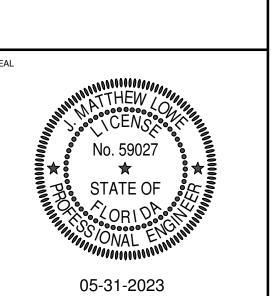




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PROJECT INFORMATION BLOCK

5/31/2023 DRAWN BY: CHECKED BY:

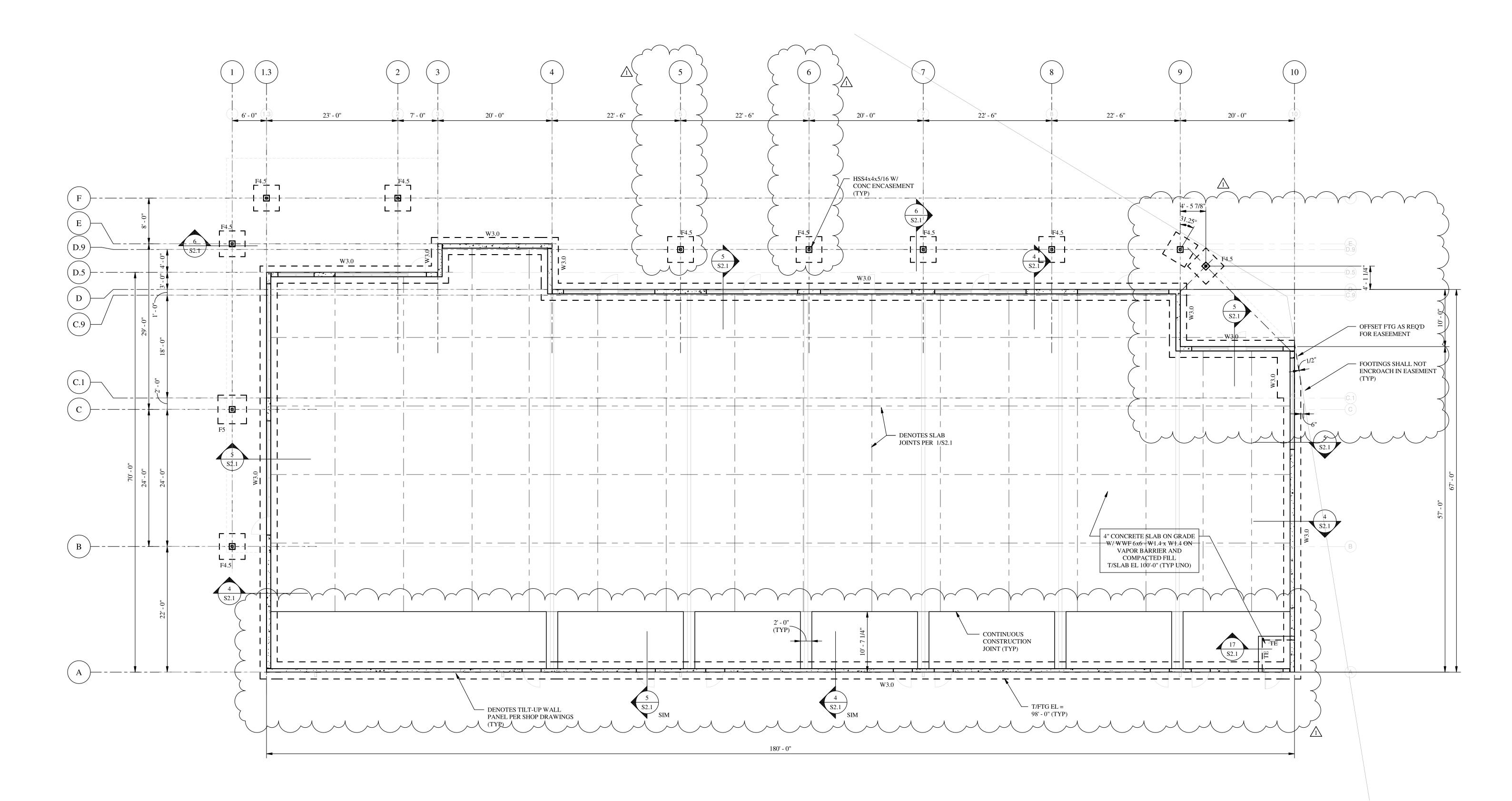
ISOMETRIC VIEWS

SHEET NUMBER

SHEET TITLE

S0.2

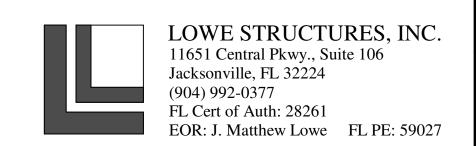
MASON **Development &** Construction **Bid Set** 





- <u>PLAN NOTES:</u>
  1. SEE S0.1 FOR GENERAL NOTES AND DESIGN CRITERIA. 2. CONTRACTOR TO COORD ALL DIMS & LAYOUT W/ ARCH PLAN PRIOR TO
- THE COMMENCEMENT OF CONSTRUCTION. ARCH DIMS AND ELEVATIONS WILL CONTROL.
  3. ALL ELEVATIONS ARE BASED ON A DATUM T/ SLAB ELEVATION OF 100'-0".
- COORD ELEVATION W/ CIVIL & ARCH. 4. W# & F# DENOTES FOOTING TYPE. SEE SCHEDULE ON THIS SHEET FOR
- FOOTING SIZE & REINF. 5. SEE 2/S2.1 WHERE PIPES PENETRATE BUILDING PERIMETER.
  6. SEE 3/S2.1 FOR REINF @ WALL FTG CORNERS.

				FOOTING SCH	<u>IEDULE</u>			
TYPE	LENGTH	WIDTH	DEPTH	BOTTOM REINF (LONG WAY)	BOTTOM REINF (SHORT WAY)	TOP REINF (LONG WAY)	TOP REINF (SHORT WAY)	REMARKS
F4.5	4' - 6"	4' - 6"	1' - 2"	5-#5	5-#5	-	-	
F5	5' - 0"	5' - 0"	1' - 2"	6-#5	6-#5	-	-	
W3.0	CONT	3' - 0"	1' - 2"	4-#5	#4 @ 18" OC	-	-	



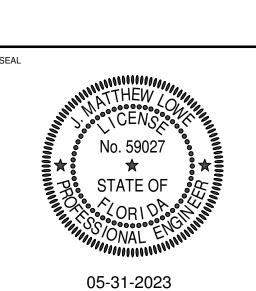


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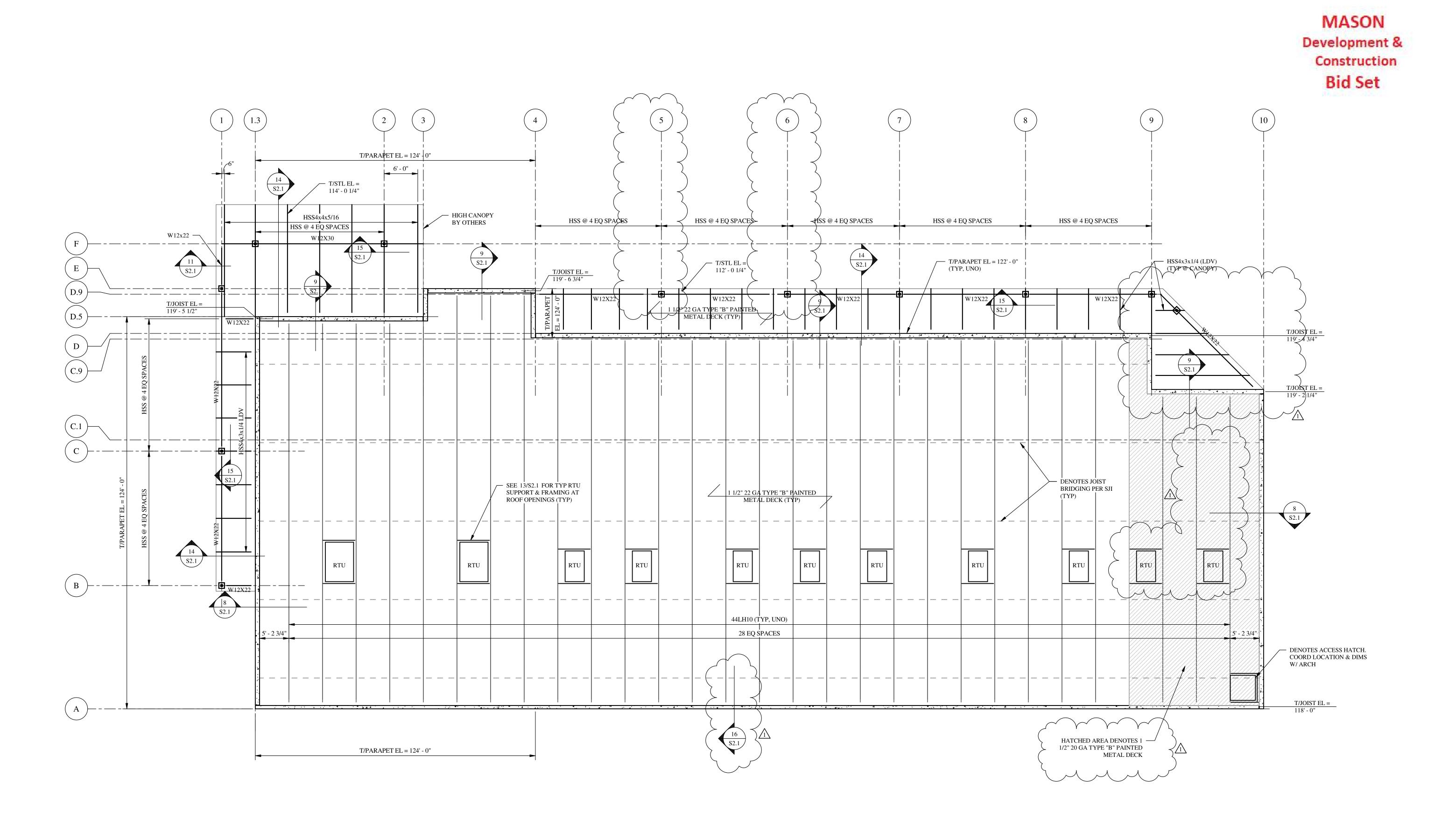
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THESE PLANS FOR ANY REASON, BY ANY PERSON OTHER THAN BDG
ARCHITECTS SHALL AUTOMATICALLY VOID ANY DESIGN-RELATED OBLIGATIONS
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COMPLETE RELEASE OF BDG ARCHITECTS FROM ANY LIABILITY, CLAIMS, OR
DAMAGES INCLUDING ERRORS AND OMISSIONS ARISING OUT OF OR RELATED
TO ERFORMED BY SUBCONTRACTORS OR OTHER TRADES WITH THE DESIGN



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ISSUE	ВҮ	DATE	DESCRIPTION
1		5/19/23	Revision 1
PR	OJE	CT INFOR	RMATION BLOCK
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FOUNDATION PLAN

SHEET NUMBER





NOOF FRAMING PLAN

PLAN NOTES:

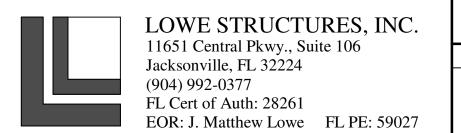
1. ELEVATIONS ARE BASED ON TOP OF SLAB ON GRADE DATUM 100'-0".

2. FOR DESIGN CRITERIA AND GENERAL NOTES, SEE SHEET S0.1.

3. SEE 2/S0.1 FOR ROOF DECK ATTACHMENT REQUIREMENTS.

4. SEE 12/S2.1 FOR JOIST REINFORCING AT CONCENTRATED LOADS.

SEE 2/S0.1 FOR ROOF DECK ATTACHMENT REQUIREMENTS.
 SEE 12/S2.1 FOR JOIST REINFORCING AT CONCENTRATED LOADS.
 SEE 13/S2.1 FOR FRAMING AT ROOF OPENINGS.
 COORD ALL RTU WEIGHTS AND LOCATIONS W/ MECH PLANS.





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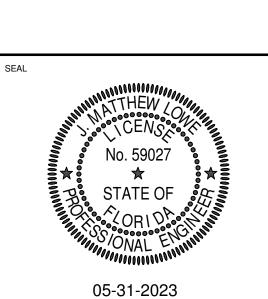
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