#### **FOUNDATIONS**

- 1. FOUNDATIONS ARE DESIGNED FOR AN ASSUMED SOIL BEARING PRESSURE
- 2. A GEOTECHNICAL ENGINEER RETAINED BY THE OWNER SHALL INSPECT ALL FOUNDATION WORK TO CONFIRM THAT REQUIRED SOIL BEARING CAPACITIES ARE MET. THE GEOTECHNICAL ENGINEER SHALL FURNISH A SIGNED AND SEALED SUMMARY REPORT STATING THAT THE FOUNDATION PREPARATION PROVIDES THE REQUIRED BEARING CAPACITIES FOR
- 3. THE PROJECT SITE SHALL BE IMPROVED WITH VIBRATORY ROLLING COMPACTION FROM THE STRIPPED GRADE PRIOR TO CONSTRUCTION OF THE FOUNDATION PAD. THE TOP 2 FEET BELOW STRIPPED GRADE SHALL BE COMPACTED TO A MINIMUM OF 95% DENSITY PRIOR TO PLACING FILL TO ACHIEVE FINAL GRADE.
- AREAS UNDER SLABS AND FOOTINGS TO BE BACK FILLED SHALL BE COMPACTED IN LIFTS OF NOT MORE THAN TWELVE INCHES UNCOMPACTED DEPTH (SIX INCHES UNCOMPACTED DEPTH IF HAND OPERATED EQUIPMENT IS USED) TO 95% OF THE MAXIMUM DRY DENSITY AS DEFINED BY ASTM 0698 OR ASTM D1557.

#### SPEC. ENGINEERED SYSTEMS

1. THE DESIGN WIND PRESSURES FOR EXTERIOR WINDOWS AND DOORS ARE SUPPLIED ON THE ELEVATION DRAWINGS. THE CONTRACTOR SHALL SUBMIT PRODUCT SUPPLIED ON THE ELEVATION DRAWINGS. THE CONTRACTOR SHALL SUBMIT PRODUCT CONTROL APPROVALS TO THE ENGINEER SHOWING THAT THE DOORS, WINDOWS AND OTHER CLADDING SYSTEMS MEET THE DESIGN PRESSURES AND DEBRIS IMPACT CRITERIA FOR THE CONSTRUCTION PROJECT PER FBC REQUIREMENTS.

#### INDIAN RIVER COUNTY BUILDING DEPARTMENT CODE COMPLIANCE DIVISION DESIGN CERTIFICATION FOR WIND LOAD COMPLIANCE

THIS CERTIFICATION MUST BE COMPLETED BY THE PROJECT DESIGNER, ARCHITECT, OR ENGINEER. THIS CERTIFICATION MUST BE SUBMITTED IN DUPLICATE WITH ALL APPLICATIONS FOR BUILDING PERMITS INVOLVING THE CONSTRUCTION OF NEW RESIDENCE (SINGLE OR MULTI-FAMILY), RESIDENTIAL ADDITION, ANY ACCESSORY STRUCTURE REQUIRING A BUILDING PERMIT AND ANY NON-RESIDENTIAL STRUCTURE. THIS CERTIFICATION SHALL NOT APPLY TO INTERIOR RENOVATIONS (PROVIDE THAT NO EXTERIOR STRUCTURAL WALLS, COLUMNS OR OTHER COMPONENTS ARE BEING AFFECTED AND CERTAIN OTHER MINOR BUILDING PERMITS FOR FURTHER ASSISTANCE PLEASE CONTACT THE BUILDING INSPECTION

OJECT NAME:	NEW THREE-STORY STORAGE FACILTY
OJECT ADDRESS:	1801 U.S. HIGHWAY ONE & 857 18TH PL, VERO BEACH, FL, 32960

S1-STORAGE OCCUPANCY TYPE: CONSTRUCTION: CMU/PEMB

PFRMIT #

I CERTIFY THAT, TO THE BEST OF MY KNOWLEDGE AND BELIEF, THESE PLANS AND SPECIFICATIONS HAVE BEEN DESIGNED TO COMPLY WITH THE APPLICABLE STRUCTURAL PORTION OF THE BUILDING CODES CURRENTLY ADOPTED AND ENFORCED BY OKEECHOBEE COUNTY. I ALSO CERTIFY THAT STRUCTURAL ELEMENTS DEPICTED ON THESE PLANS PROVIDE ADEQUATE RESISTANCE TO THE WIND LOAD AND FORCES SPECIFIED BY CURRENT CODE PROVISIONS.

#### DESIGN PARAMETERS AND ASSUMPTIONS USED

2. BUILDING DESIGN IS ENCLOSED:	BUILDING CODE X	PARTIALLY ENCLO	OSED:	(	OPEN BUILDING:	
3. MEAN ROOF HEIGHT: 42 <b>'-4" A.F.F</b>	<del></del>	ROOF PITCH:	1/8:12			RE COEFFICIENT: +/- 0.18
4. WIDTH OF END ZONES: 14'-6"		WIND SPEED:	150 (3 SEC. 0	GUSTS)		
5. BUILDING CLASSIFICATION TABLE 1-1 ASCE 7	7: <b>II-B</b>	FBC TABLE:	1604.5			
6. WIND EXPOSURE CLASSIFICATION:	В	ADJUSTMENT FA	CTOR FOR EXPOS	URE & HEI	IGHT: <u> </u>	.12
7. COMPONENTS & CLADDING WIND PRESSU	ĪRE ON ROOF ZOI	VE: 1	-26.6 PSF	2	-44.6 PSF 3	-67.2 PSF
8. COMPONENTS & CLADDING WIND PRESSU	IRE ON WALL ZON	IE: 4	N/A	5	N/A	SEE ELEVATIONS ON A-200 PAGES
9. COMPONENTS & CLADDING WIND PRESSU	IRE ON OVERHEA	D GARAGE DOOR.	N/A PSF	_		
10. LOADS: FLOOR: <u>125 PSF</u>	ROOF/DEAD:	60 PSF	ROOF/LIVE:	20 PSF		
11. SHEAR WALLS CONSIDERED FOR STRUC	TURE?	YES	(IF NO, ATTACH EX	XPLANATIO	ON)	
12. CONTINUOUS LOAD PATH PROVIDED?	·	YES	(IF NO, ATTACH EX	XPLANATIO	ON)	
13. ARE COMPONENT AND CLADDING DETAIL	S PROVIDED?	YES	(IF NO, ATTACH EX	XPLANATIO	ON)	SEE ELEVATIONS ON A-200 PAGES
14. MINIMUM SOIL BEARING PRESSURE:	2500	PRESUMPTIVE:	N/A PSF	BY TEST:	2500	
15. HURRICANE SHUTTERS AS INDICATED OF	N PRODUCT APPR	OVAL: NONE, AL	OPENINGS SHAL	L BE IMPA	CT RATED.	

- ALL CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C-90. MORTAR SHALL BE TYPE M OR S COARSE MASONRY. GROUT SHALL CONFORM TO ASTM C476 WITH A MINIMUM COMPRESSIVE STRENGTH OF 1.800 PSI AND A SLUMP OF: ±10". MINIMUM 28 DAY COMPRESSIVE STRENGTH OF THE MASONRY UNIT SHALL BE 1,500 PSI.

AND BUILDING CODE REQUIREMENTS (ACI 530/530.1-13).

**MASONRY** 

MATERIALS, TESTING OF MATERIALS AND STORAGE OF MATERIALS SHALL

CONFORM TO ACI STANDARD SPECIFICATIONS FOR MASONRY STRUCTURES

- 3. ALL MASONRY CONSTRUCTION SHALL BE RUNNING BOND. MORTAR JOINTS SHALL NOT EXCEED 5/8" THICKNESS IN CONCEALED AREAS AND WITHIN SPECIFIED TOLERANCES IN AREAS EXPOSED TO FOUNDATIONS WHERE VERTICAL AND HORIZONTAL ALIGNMENT WOULD CAUSE MORTAR JOINTS TO BE LESS THAN 1/4" OR MORE THAN 1/2" THICK SHALL BE CORRECTED PRIOR TO COMMENCING MASONRY CONSTRUCTION.
- 4. PIPES OR CONDUITS MAY PENETRATE HORIZONTALLY THROUGH MASONRY WALLS BY MEANS OF A GALVANIZED STEEL SLEEVE NO THINNER THAN STANDARD WEIGHT (SCHEDULE 40), ASTM A53, SOLIDLY GROUTED OR MORTARED IN PLACE. PLACE SLEEVES NOT CLOSER THAN THREE DIAMETERS ON CENTER. MAXIMUM SIZE OF SLEEVE SHALL BE 12".
- MAXIMUM SIZE OF EMBEDDED VERTICAL PIPE OR CONDUIT IN A BEARING WALL SHALL NOT EXCEED 1/3 OF WALL THICKNESS. PIPES SHALL NOT BE

PLACED CLOSER THAN THREE DIAMETERS ON CENTER.

- HORIZONTAL JOINT REINFORCEMENT IN ALL CMU WALLS SHALL BE 9 GA. DEFORMED GALVANIZED TRUSS TYPE. JOINT REINFORCEMENT SHALL BE FULLY EMBEDDED IN MORTAR WITH A MINIMUM COVER OF 5/8". HORIZONTAL MASONRY REINFORCEMENT SHALL BE PLACED IN EVERY OTHER COURSE UNLESS OTHERWISE NOTED.
- SEE PLANS FOR VERTICAL REINFORCEMENT IN WALLS. PROVIDE MINIMUM LAP OF 48 BAR DIAMETERS. FILL ALL REINFORCED CELLS WITH GROUT FOR FULL HEIGHT. ALL CELLS LOCATED BELOW GRADE SHALL BE FILLED WITH GROUT REINFORCE CELLS ADJACENT TO DOOR, WINDOW AND OPENINGS IN ACCORDANCE WITH CMU OPENING REINFORCING DETAIL. GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI.
- ALIGN VERTICAL CORES EVENLY FOR THE FULL HEIGHT WHERE CELLS ARE TO BE GROUTED. GROUT IN VERTICAL CORES SHALL NOT BE PLACED IN LIFTS GREATER THAN 5 FEET. PROVIDE CLEAN OUTS WHEN GROUTING
- MASONRY WALLS MUST BE BRACED DURING ERECTION FOR WIND AND CONSTRUCTION LOADS. BRACES MUST BE DESIGNED, CONSTRUCTED AND ERECTED IN A FASHION AS TO PREVENT PERMANENT SCARRING OF EXPOSED MASONRY SURFACES.
- 10. ALL MASONRY WALLS SHOWN ON STRUCTURAL DRAWINGS SHALL BE IN PLACE PRIOR TO FORMING OR PLACING CONCRETE IN COLUMNS AND BEAMS OVER THE WALLS, EXCEPT AS OTHERWISE SPECIFIED. HORIZONTAL JOINT REINFORCING SHALL EXTEND A MINIMUM OF 6" INTO ALL BUTTING

### **STRUCTURAL NOTES**

1. REFER TO A-PAGES FOR WALL DETAILS, AND S-PAGES FOR STRUCTURAL NOTES

2. TRUSS MANUFACTURER TO PROVIDE TRUSS ENGINEERING AND LAYOUT FOR APPROVAL PRIOR TO INSTALLATION.

#### REINFORCED CONCRETE

- 1. ALL CONCRETE CONSTRUCTION SHALL CONFORM TO ACI 318, LATEST EDITION, SPECIFICATIONS FOR STRUCTURAL CONCRETE.
- 2. UNLESS NOTED OTHERWISE, DETAILS OF CONCRETE REINFORCEMENT AND ACCESSORIES SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF ACI 315, MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES, AND CRSI MSP-1, MANUAL OF STANDARD PRACTICE, LATEST EDITIONS.
- 3. SHOP DRAWINGS SHOWING ALL FABRICATION DIMENSIONS AND LOCATIONS FOR PLACING OF THE REINFORCING STEEL AND ACCESSORIES SHALL BE SUBMITTED FOR REVIEW TO THE ARCHITECT/ENGINEER. NO FABRICATION SHALL BEGIN UNTIL SHOP DRAWINGS ARE COMPLETED AND
- 4. SUBMIT CONCRETE MATERIALS, MIX DESIGN AND METHODS OF CURING FOR REVIEW BY THE ENGINEER.
- 5. TESTING LABORATORY EMPLOYED BY THE OWNER SHALL SUBMIT ONE COPY OF ALL CONCRETE TEST REPORTS DIRECTLY TO THE ENGINEER. SEE SPECIAL INSPECTION SCHEDULE FOR REQUIRED TESTS.
- 6. UNLESS NOTED OTHERWISE, 28 DAY COMPRESSIVE STRENGTH OF CAST-IN-PLACE CONCRETE FOR EACH PORTION OF THE STRUCTURE SHALL BE AS

FOOTINGS, SLAB ON GRADE BEAMS COLUMNS AND SLABS ABOVE GRADE = 4,000 PSI PRE-STRESSED ELEMENTS AND PILES = 5,000 PSI

UNLESS NOTED OTHERWISE, MINIMUM CONCRETE COVER FOR REINFORCEMENT SHALL BE AS FOLLOWS:

CONCRETE CAST AGAINST EARTH	3"
FORMED CONCRETE EXPOSED TO	
EARTH OR WEATHER:	
#6 THROUGH #18 BARS	2"
#5 BARS AND SMALLER	1 1/2"
INTERIOR SLABS AND WALLS W/	
#11 BAR AND SMALLER	3/4"
INTERIOR BEAMS, AND COLUMNS:	1 1/2"

8. CONCRETE SLUMP SHALL BE 3" MIN. TO 5" MAX.

ENGINEER OR HIS REP.

- 9. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60.
- 10. ALL SPLICES SHALL CONFORM TO ACI 318, CLASS "B" SPLICES UNLESS NOTED OTHERWISE. PROVIDE CORNER BARS OF 30"X30" FOR TOP AND BOTTOM BARS AT ALL CORNERS OF TIE BEAMS AND FOUNDATIONS.
- 11. CONSTRUCTION JOINTS SHALL FALL AT CONTROL JOINTS AND SHALL BE
- 12. WELDING OR TACK WELDING OF REINFORCING STEEL SHALL NOT BE PERMITTED EXCEPT AS AUTHORIZED OR DIRECTED BY THE STRUCTURAL
- 13. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064 AND SHALL BE SUPPLIED IN SHEETS. LAP SHEETS AT LEAST 8" TO CONTAIN AT LEAST ONE CROSS WIRE WITHIN 8" OF CENTRAL JOINT.
- 14. PROVIDE 2 #4 (X3'-0 LONG) BARS AT ALL RE-ENTRANT CORNERS OF ALL SLABS ON GRADE AND UPPER FLOOR SLABS.
- 15. PROVIDE CORNER BARS MATCHING THE MAIN REINFORCEMENT AT ALL FOOTING AND TIE BEAM INTERSECTIONS. THE LENGTH OF CORNER BARS IN EACH DIRECTION SHALL BE A MINIMUM OF 30 BAR DIAMETERS.
- 16. IF REQUIRED. HOUSEKEEPING PADS SHALL BE 4" THICK CONCRETE CAST OVER THE FLOOR SLAB. REINFORCE WITH 6X6 W2.1XW2.1 PLACED AT CENTER OF SLAB. COORDINATE SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

#### STRUCTURAL STEEL

- 1. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED ACCORDING TO THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES, LATEST EDITIONS.
- 2. SHOP DRAWINGS PREPARED IN ACCORDANCE WITH THE LATEST "STRUCTURAL STEEL DETAILING MANUAL OF THE AISC" SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN FLORIDA, AND SUBMITTED TO THE ARCHITECT/ENGINEER FOR REVIEW NO FABRICATION SHALL BEGIN UNTIL SHOP DRAWINGS ARE COMPLETED AND REVIEWED.
- 3. UNLESS NOTED OTHERWISE, STRUCTURAL STEEL SECTIONS SHALL CONFORM TO THE FOLLOWING:

ASTM A501 OR ASTM A53, GRADE B.

- A. W-SHAPES SHALL CONFORM TO ASTM A992, GRADE 50. (ASTM A 572, GRADE 50 MAY BE SUBSTITUTED FOR ASTM A992.)
- B. SQUARE/RECTANGULAR HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO ASTM A500, GRADE B
- C. ROUND HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO
- D. OTHER STEEL SHAPES (CHANNELS. ANGLES, AND PLATES) MAY CONFORM TO ASTM A36.
- 4. BEAMS WITHOUT SPECIFIED CAMBER ARE TO BE FABRICATED SUCH THAT AFTER ERECTION ANY CAMBER DUE TO ROLLING OR SHOP FABRICATION IS UPWARD AT MID SPAN.
- 5. BOLTS, WELDS, ANCHORS, AND HEADED STUDS:
  - A. WELDS SHALL CONFORM TO "STRUCTURAL WELDING CODE" OF THE AMERICAN WELDING SOCIETY, AWS DI.I. USE E70XX ELECTRODES. PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE. MINIMUM WELD SIZE IS ACCORDING TO AISC, LATEST
- B. UNLESS NOTED OTHERWISE BOLTS SHALL CONFORM TO ASTM A325, AND SHALL BE MINIMUM 3/4" DIAMETER.
- C. UNLESS NOTED OTHERWISE STUDS SHALL BE 3/4" DIA. 5-3/16" LONG HEADED STUDS, BY NELSON, OR APPROVED EQUAL.
- D. UNLESS NOTED OTHERWISE ALL EXPANSION ANCHORS ARE TO BE 3/4" DIA. HILTI KWIK - BOLTS WITH 43/4" MIN. EMBEDMENT, OR AN APPROVED EQUAL.
- E. ANCHOR BOLTS SHALL CONFORM TO ASTM A307 AND SHALL BE GALV. IF EXPOSED IN THE FINISHED STRUCTURE.
- 6. THE FABRICATOR SHALL BE RESPONSIBLE FOR THE DESIGN AND DETAILING OF ALL CONNECTIONS NOT SPECIFICALLY DETAILED ON THE CONTRACT DRAWINGS. THIS DESIGN SERVICE SHALL BE INCLUDED IN THE CONTRACTOR'S SCOPE OF SERVICES. SHOP DRAWINGS AND CALCULATIONS OF SUCH CONNECTIONS SHALL BE SEALED BY AN ENGINEER LICENSED IN FLORIDA. REVIEW BY THE E.O.R. DOES NOT RELIEVE THE CONTRACTOR OF THE FULL RESPONSIBILITY FOR THE DESIGN AND ADEQUACY OF SUCH.
- UNGALVANIZED PIECES EXCEPT AT AREAS TO BE FIELD WELDED. TOUCH UP FIELD WELDS AND ANY DAMAGED AREAS OF PAINT IN FIELD AFTER WELDING (USE GALVANIZED PAINT FOR TOUCH UP OF GALVANIZED STEEL). GALVANIZED PAINT SHALL BE IN ACCORDANCE WITH THE FDOT QUALIFIED PRODUCTS LIST.



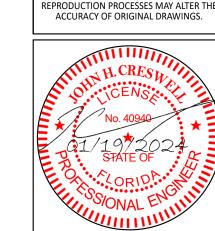
502 NW 2ND STREET, OKEECHOBEE, FLORIDA 34972 P: (863) 467-1111 F: (772) 872-5121 WWW.GDCFLORIDA.COM

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**ISSUED FOR** PERMIT

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REFERENCE ONLY AS NORMAL



JOHN H. CRESWELL PE #40940/ COA #30250 C: (863) 467-1111

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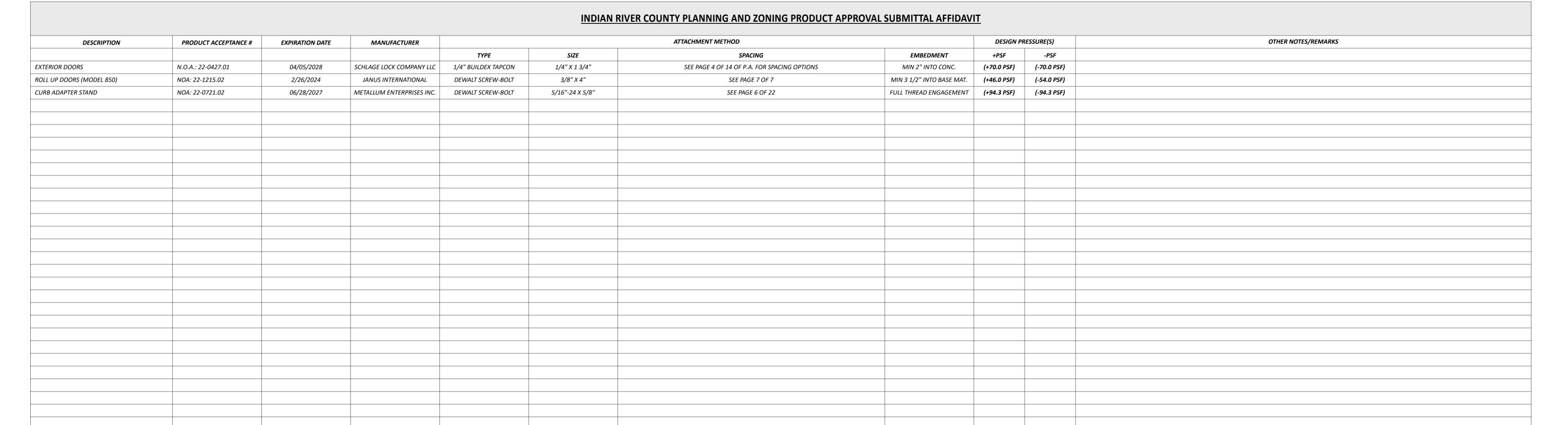
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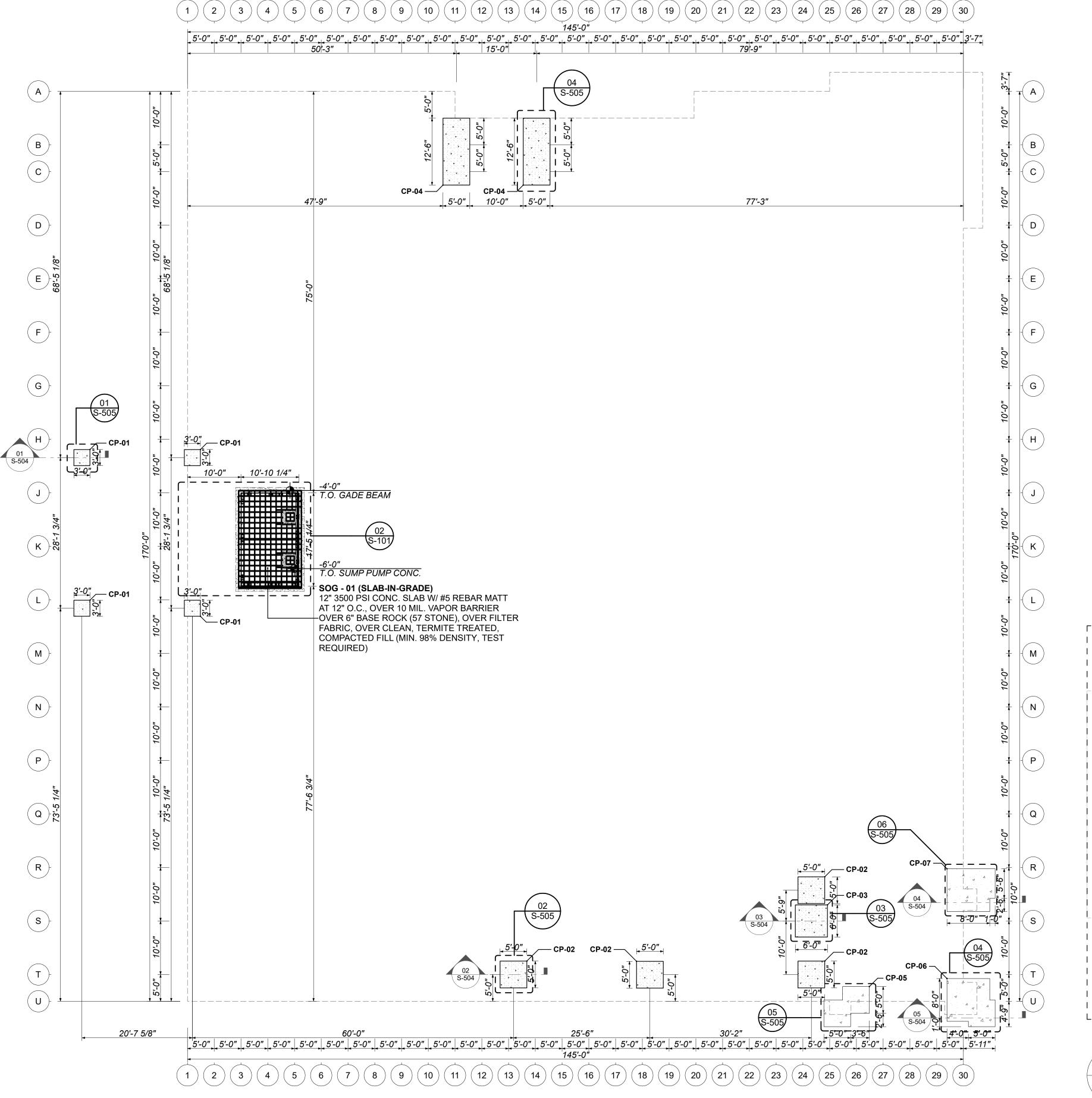
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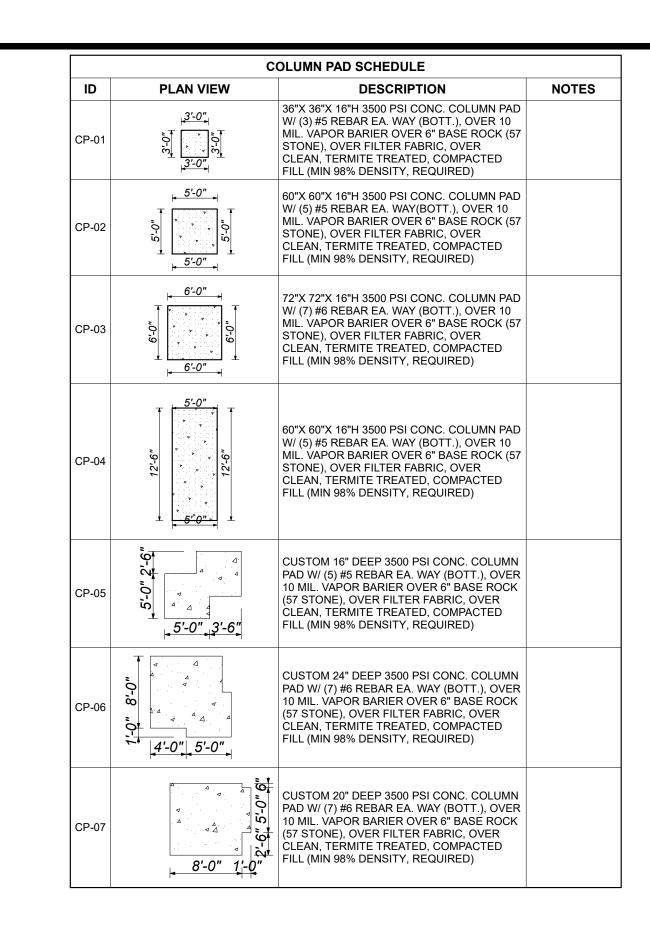
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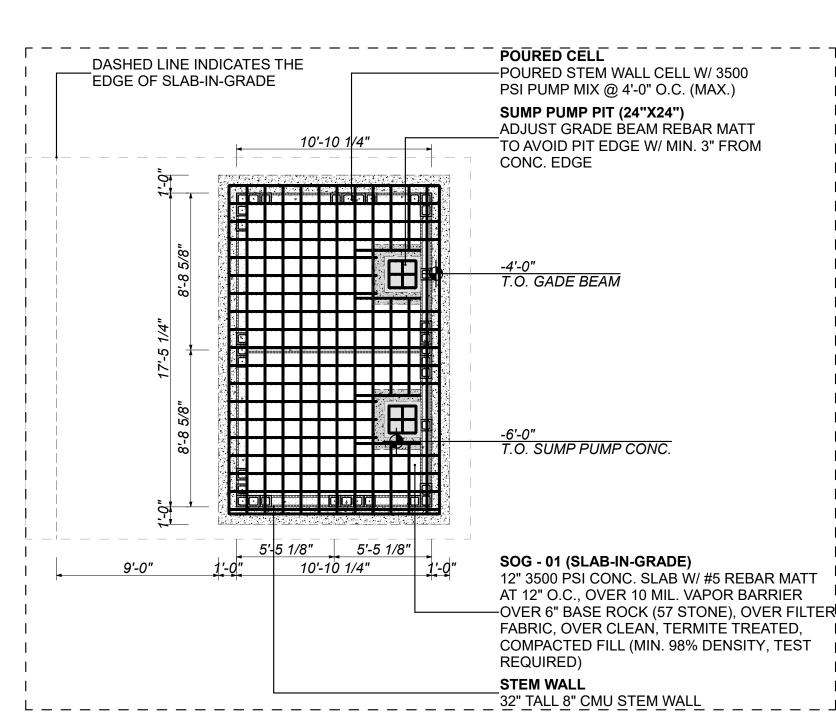
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> DATE 1/19/2024









# **GRADE BEAM, STEM WALL, & COLUMN PAD PLAN**

**ENLARGED GRADE BEAM/ SLAB PLAN AT ELEVATOR** SCALE: 3/16" = 1'-0"

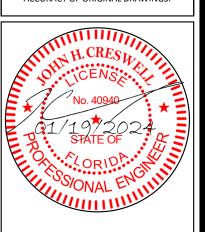
**ENGINEERS** COA#30250, AA#26003565 502 NW 2ND STREET, OKEECHOBEE, FLORIDA 34972

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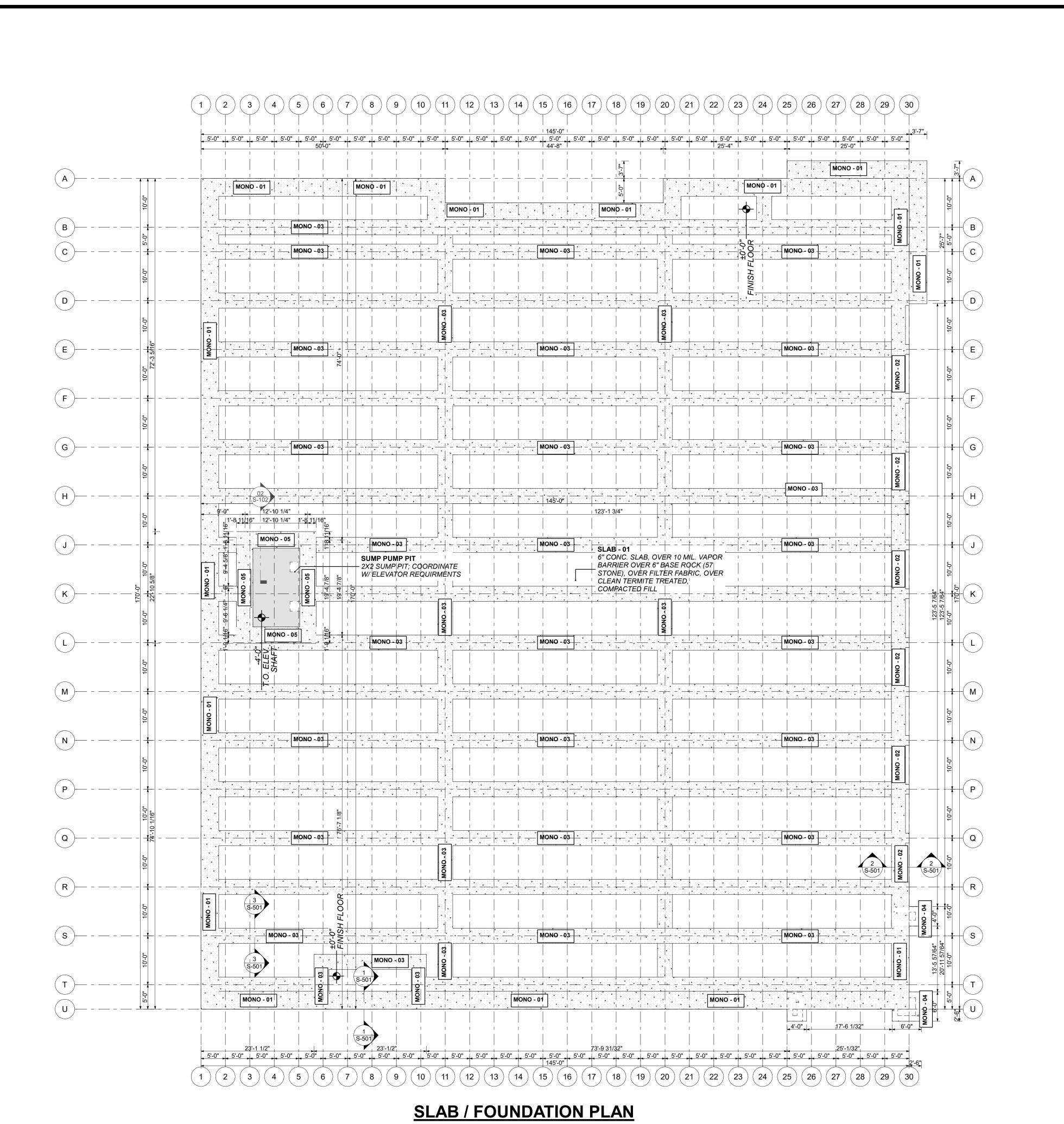
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SCALE: 3/32" = 1'-0"

**NOTES** 

1. SEE S-500 PAGES FOR TYPICAL FOOTING DETAILS, REBAR DETAILS, AND SAWCUT PROCEDURES.

2. PLUMBING DROPS INDICATED ARE FOR REFERENCE ONLY! IT IS THE PLUMBING CONTRACTORS RESPONSIBILITY TO FIELD VERIFY ALL DOMESTIC

> 38"W X 16"H 3500 PSI MONO - FOOTING W/ (2) #5 CONT. REBAR AT TOP AND (4) #5 CONT. REBAR

- NEW COMPATED EARTH

**GB - 01** 12" 3500 PSI CONC. SLAB OVER 10 MIL. VAPOR

BARRIER OVER 4" BASE - ROCK (57 STONE), OVER

DENSITY, REQUIRED)

FILTER FABRIC, OVER
CLEAN, TERMITE TREATED,
COMPACTED FILL (MIN. 98%

-8" C.M.U. STEMWALL/

AT BOTTOM

**ELEVATOR PIT** 

AND SANITARY DROPS PRIOR TO FINISH SLAB INSTALLATION. 3. TOP OF ALL EQUIPMENT PADS MUST BE AT OR ABOVE BASE FLOOD ENGINEERS COA#30250, AA#26003565

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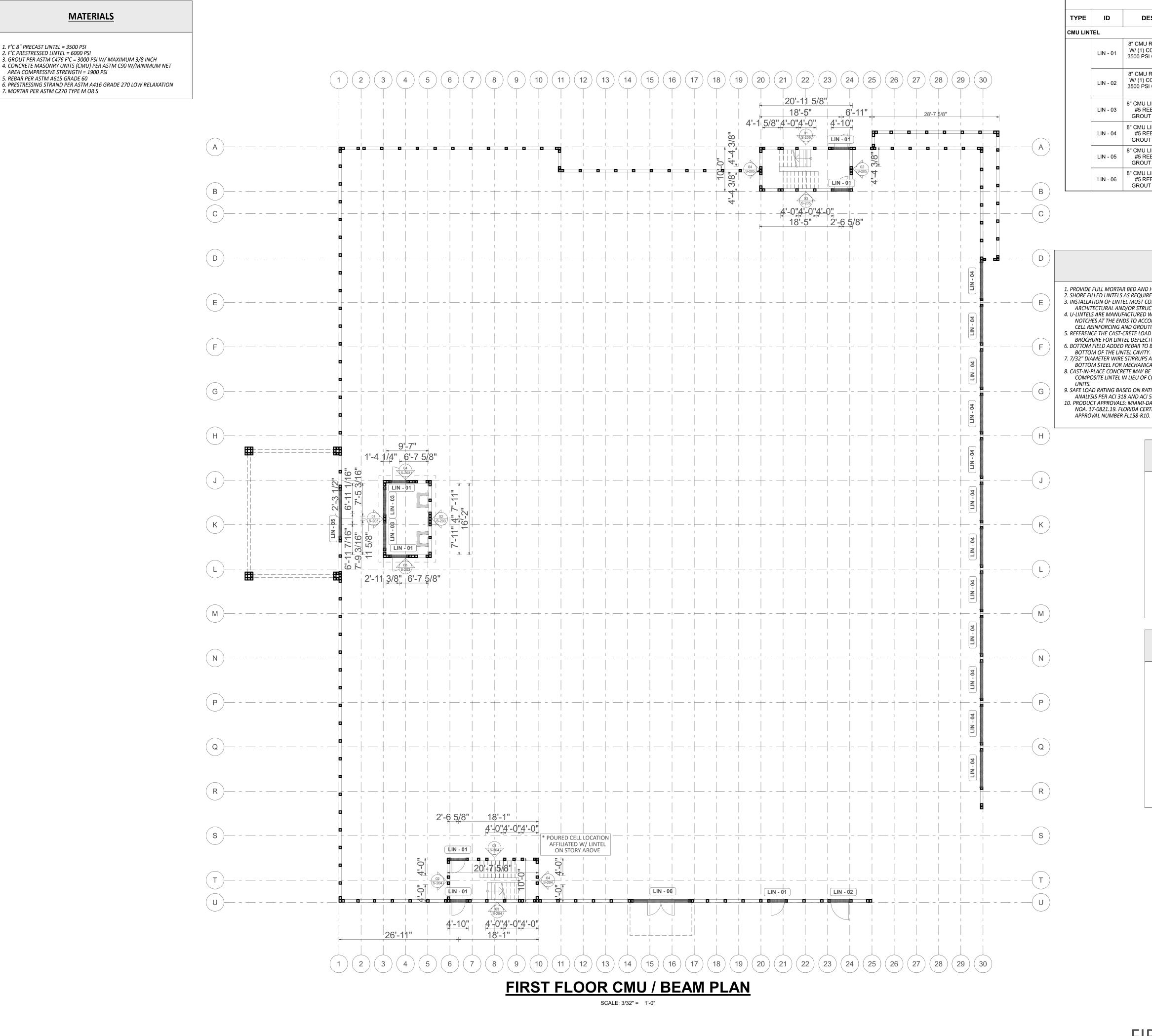
# **SECTION @ ELEVATOR PIT**

SCALE: 1" = 1'-0"

	М	ONO FOOTING SCHEDULE	
ID	SECTION PROFILE	DESCRIPTION	NOTES
MONO - 01	0-E	36"W X 36"H 3500 PSI MONO FOOTING W/ (6) #5 REBAR CONT. [(2) TOP, (4) BOTT.], OVER CLEAN TERMITE TREATED COMPACTED FILL TO MIN. 95% COMPACTION DENSITY	
MONO - 02	1 1/2"  1 1/2"  1 1/2"  3'-0"	36"W X 36"H 3500 PSI MONO FOOTING W/ (6) #5 REBAR CONT. [(2) TOP, (4) BOTT.] WITH 7- 1/4"W X 1-1/2"H ROLL-UP RECESS AT EDGE, OVER 6 MIL VAPOR BARRIER, OVER CLEAN TERMITE TREATED COMPACTED FILL TO MIN. 95% COMPACTION DENSITY	
MONO - 03	"0-12 12-0"	24"W X 24"H 3500 PSI MONO FOOTING W/ (3) #5 REBAR CONT. OVER 6 MIL VAPOR BARRIER, OVER CLEAN TERMITE TREATED COMPACTED FILL TO MIN. 95% COMPACTION DENSITY	
MONO - 04	50 <u></u> [10"₁	8"W X 10"H 3500 PSI CONC. THICKENED EDGE W/ (1) #5 REBAR CONTINUOUS	
MONO - 05	3'-2"	38"W X 16"H 3500 PSI MONO FOOTING W/ (2) #5 CONT. REBAR AT TOP AND (4) \$5 CONT. REBAR AT BOTT.	STEM WALL BELOW FOOTING

C: (863) 467-1111 E: john@gdcflorida.com REVISION WRITTEN DIMENSIONS ON THESE DOCUMENTS SHALL TAKE PRECEDENCE OVER ALL SCALED DIMENSIONS. DRAWN BY CHECKED BY S.M.K. DATE 1/19/2024

SLAB / FOUNDATION PLAN



				LINTEL SCHEDUL	E				
TYPE	ID	DESCRIPTION	LENGTH	REQUIRED ORDERING LEANGTH	PRE-STRESSED LINTEL	LINTEL QUANTITY	NOTES	QUANTITY	LINTE
CMU LINT	TEL								•
	LIN - 01	8" CMU RECESSED LINTEL W/ (1) CONT. #5 REBAR IN 3500 PSI GROUT PUMP MIX (TYP)		44'-0"		11	LENGTH - 3'-11 1/2"	11	44'-0'
	LIN - 02	8" CMU RECESSED LINTEL W/ (1) CONT. #5 REBAR IN 3500 PSI GROUT PUMP MIX (TYP)		5'-4"		1	LENGTH - 4'-11 1/2"	1	5'-4'
	LIN - 03	8" CMU LINTEL W/ (1) CONT. #5 REBAR IN 3500 PSI GROUT PUMP MIX (TYP)	6'-3 5/8"	39'-0"		6		6	39'-0
	LIN - 04	8" CMU LINTEL W/ (1) CONT. #5 REBAR IN 3500 PSI GROUT PUMP MIX (TYP)	8'-7 5/8"	112'-0"		12		12	112'-(
	LIN - 05	8" CMU LINTEL W/ (1) CONT. #5 REBAR IN 3500 PSI GROUT PUMP MIX (TYP)	10'-11 5/8"	11'-4"		1		1	11'-4
	LIN - 06	8" CMU LINTEL W/ (1) CONT. #5 REBAR IN 3500 PSI GROUT PUMP MIX (TYP)	13'-0"	13'-4"		1		1	13'-4

#### **LINTEL NOTES** 11. THE EXTERIOR SURFACE OF LINTELS INSTALLED IN EXTERIOR CONCRETE MASONRY WALLS SHALL HAVE A 1. PROVIDE FULL MORTAR BED AND HEAD JOINTS. 2. SHORE FILLED LINTELS AS REQUIRED. 3. INSTALLATION OF LINTEL MUST COMPLY WITH THE COATING OF STUCCO APPLIED IN ACCORDANCE WITH ARCHITECTURAL AND/OR STRUCTURAL DOCUMENTS. ASTM C926 OR OTHER APPROVED COATING. – F'm=1500psi 4. U-LINTELS ARE MANUFACTURED WITH 5 1/2" LONG 12. LINTELS LOADED SIMULTANEOUSLY WITH VERTICAL (GRAVITY OR UPLIFT) AND HORIZONTAL (LATERAL) NOTCHES AT THE ENDS TO ACCOMMODATE VERTICAL LOADS SHOULD BE CHECKED FOR THE COMBINED CELL REINFORCING AND GROUTING. SEE SAFE LATERAL LOAD 5. REFERENCE THE CAST-CRETE LOAD DEFLECTION GRAPH LOADING WITH THE FOLLOWING EQUATION: TABLES FOR LOAD RATING BROCHURE FOR LINTEL DEFLECTION INFORMATION. FOR EACH ADDITIONAL APPLIED HORIZONTAL APPLIED VERTICAL 6. BOTTOM FIELD ADDED REBAR TO BE LOCATED AT THE REINFORCED CMU FULL LOAD SAFE HORIZONTAL BOTTOM OF THE LINTEL CAVITY. COURSE 7. 7/32" DIAMETER WIRE STIRRUPS ARE WELDED TO THE SAFE VERTICAL ≤ 1.0 BOTTOM STEEL FOR MECHANICAL ANCHORAGE. LOAD 8. CAST-IN-PLACE CONCRETE MAY BE PROVIDED IN 13. ADDITIONAL LATERAL LOAD CAPACITY CAN BE OBTAINED BY THE DESIGNER BY PROVIDING ADDITIONAL REINFORCED CONCRETE MASONRY ABOVE THE LINTEL. — 1/2" CLR TYP COMPOSITE LINTEL IN LIEU OF CONCRETE MASONRY (1) #5 REBAR 9. SAFE LOAD RATING BASED ON RATIONAL DESIGN ANALYSIS PER ACI 318 AND ACI 530 10. PRODUCT APPROVALS: MIAMI-DADE COUNTY, FLORIDA NOA. 17-0821.19. FLORIDA CERTIFICATE OF PRODUCT

### **SAFE LOAD TABLE NOTES**

1. ALL VALUES BASED ON MINIMUM 4 INCH NOMINAL BEARING.

EXCEPTION: SAFE LOADS FOR UNFILLED LINTELS MUST BE REDUCED BY 20% IF BEARING LENGTH IS LESS THAN 6 1/2 INCHES.

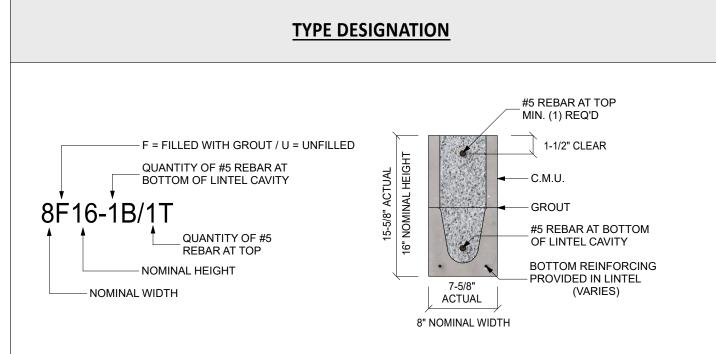
2. N.R. = NOT RATED

2. N.R. = NOT RAIED
3. SAFE LOADS ARE SUPERIMPOSED ALLOWABLE LOADS.
4. SAFE LOADS BASED ON GRADE 40 OR GRADE 60 FIELD REBAR.
5. ONE #7 REBAR MAY BE SUBSTITUTED FOR TWO #5 REBARS IN 8" LINTELS ONLY.
6. THE DESIGNER MAY EVALUATE CONCENTRATED LOADS FROM THE SAFE LOAD TABLES BY CALCULATING THE MAXIMUM RESISTING MOMENT AND SHEAR AT D-AWAY

7. FOR COMPOSITE LINTEL HEIGHTS NOT SHOWN, USE SAFE LOAD FROM NEXT LOWER HEIGHT SHOWN.

FROM FACE OF SUPPORT.

8. FOR LINTELS LENGTHS NOT SHOWN, USE SAFE LOAD FROM NEXT LONGEST LENGTH SHOWN.
9. ALL SAFE LOADS IN UNITS OF POUNDS PER LINEAR FOOT.
10. ALL SAFE LOADS BASED ON SIMPLY SUPPORTED SPAN.
11. THE NUMBER IN THE PARENTHESIS INDICATES THE PERCENT REDUCTION FOR GRADE 40 FIELD ADDED REBAR.
EXAMPLE 7'-6" LINTEL TYPE 8F32-1B SAFE GRAVITY LOAD = 6472 =>(15)W/ 15%
REDUCTION 6472 (.85) = 5501 PLF



ISSUED FOR PERMIT

ARCHITECTS ENGINEERS

COA#30250, AA#26003565

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F: (772) 872-5121

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JOHN H. CRESWELL PE #40940/ COA #30250 C: (863) 467-1111 E: john@gdcflorida.com

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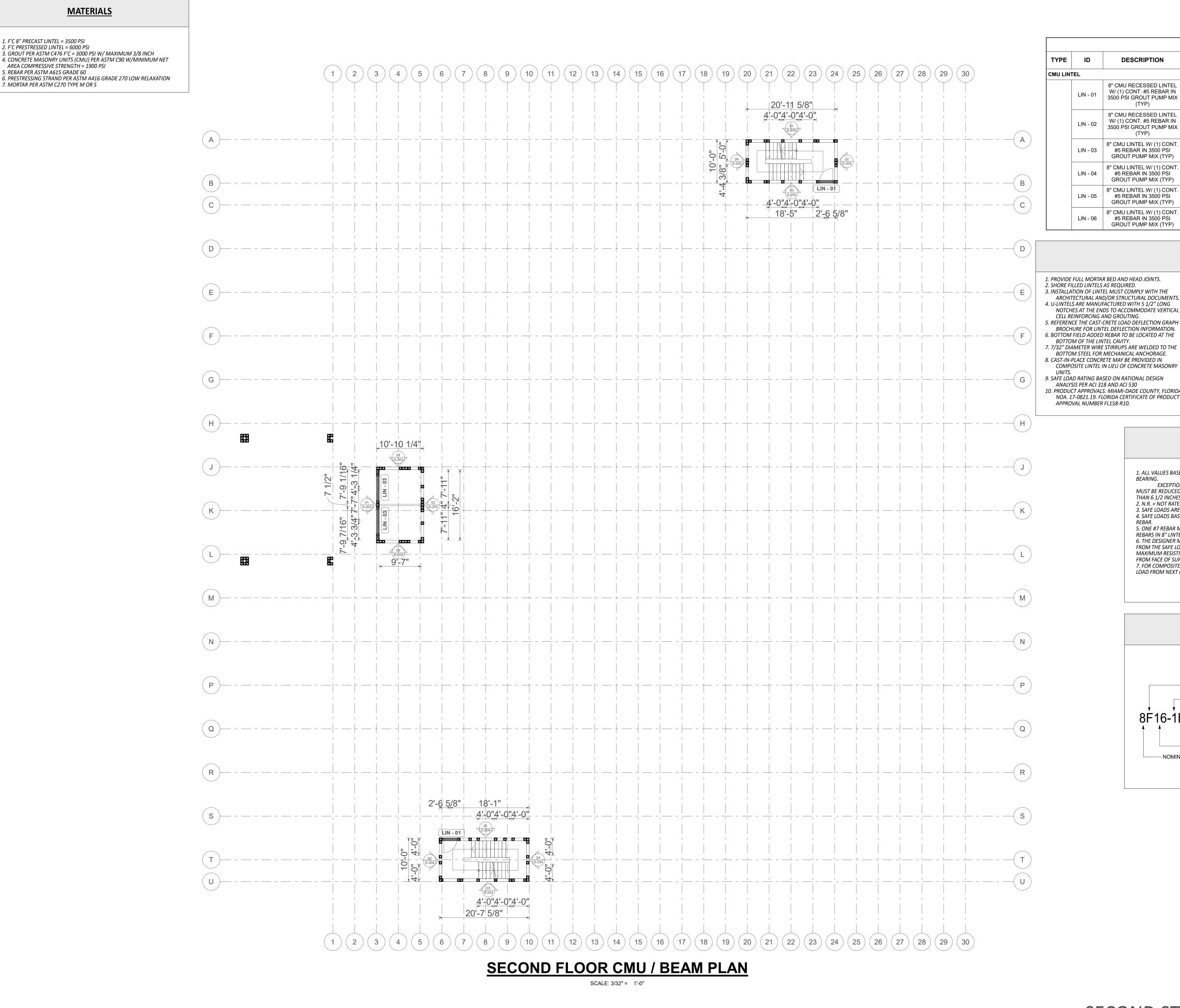
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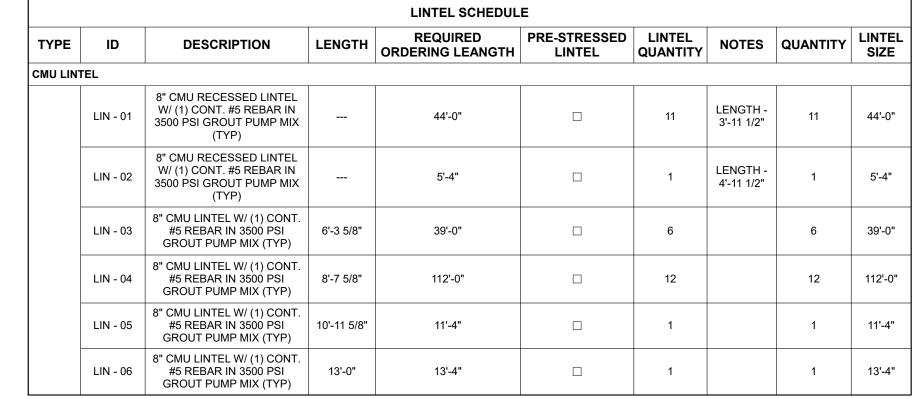
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**DATE**1/19/2024 **SHEET** 

FIRST STORY CMU / CONCRETE BEAM PLAN

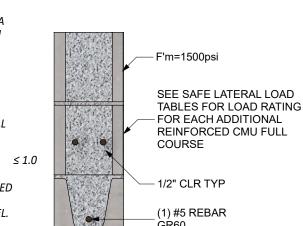
3





#### **LINTEL NOTES**

- 1. PROVIDE FULL MORTAR BED AND HEAD JOINTS. 2. SHORE FILLED LINTELS AS REQUIRED. 3. INSTALLATION OF LINTEL MUST COMPLY WITH THE
- ARCHITECTURAL AND/OR STRUCTURAL DOCUMENTS. 4. U-LINTELS ARE MANUFACTURED WITH 5 1/2" LONG NOTCHES AT THE ENDS TO ACCOMMODATE VERTICAL
- BROCHURE FOR LINTEL DEFLECTION INFORMATION. 6. BOTTOM FIELD ADDED REBAR TO BE LOCATED AT THE BOTTOM OF THE LINTEL CAVITY. 7. 7/32" DIAMETER WIRE STIRRUPS ARE WELDED TO THE BOTTOM STEEL FOR MECHANICAL ANCHORAGE.
- 8. CAST-IN-PLACE CONCRETE MAY BE PROVIDED IN COMPOSITE LINTEL IN LIEU OF CONCRETE MASONRY 9. SAFE LOAD RATING BASED ON RATIONAL DESIGN
- ANALYSIS PER ACI 318 AND ACI 530 10. PRODUCT APPROVALS: MIAMI-DADE COUNTY, FLORIDA NOA. 17-0821.19. FLORIDA CERTIFICATE OF PRODUCT APPROVAL NUMBER FL158-R10.
- 11. THE EXTERIOR SURFACE OF LINTELS INSTALLED IN EXTERIOR CONCRETE MASONRY WALLS SHALL HAVE A COATING OF STUCCO APPLIED IN ACCORDANCE WITH ASTM C926 OR OTHER APPROVED COATING. 12. LINTELS LOADED SIMULTANEOUSLY WITH VERTICAL (GRAVITY OR UPLIFT) AND HORIZONTAL (LATERAL) LOADS SHOULD BE CHECKED FOR THE COMBINED
- APPLIED HORIZONTAL APPLIED VERTICAL LOAD SAFE HORIZONTAL SAFE VERTICAL LOAD
- 13. ADDITIONAL LATERAL LOAD CAPACITY CAN BE OBTAINED BY THE DESIGNER BY PROVIDING ADDITIONAL REINFORCED CONCRETE MASONRY ABOVE THE LINTEL.



#5 REBAR AT BOTTOM

OF LINTEL CAVITY

#### SAFE LOAD TABLE NOTES

1. ALL VALUES BASED ON MINIMUM 4 INCH NOMINAL EXCEPTION: SAFE LOADS FOR UNFILLED LINTELS
MUST BE REDUCED BY 20% IF BEARING LENGTH IS LESS THAN 6 1/2 INCHES.

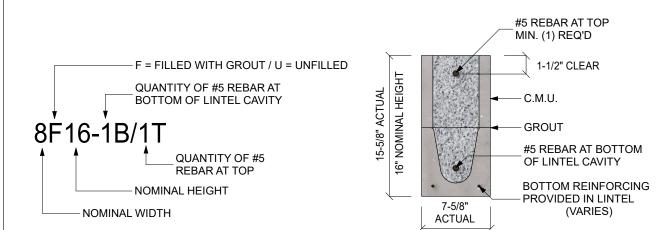
3. SAFE LOADS ARE SUPERIMPOSED ALLOWABLE LOADS. 4. SAFE LOADS BASED ON GRADE 40 OR GRADE 60 FIELD 5. ONE #7 REBAR MAY BE SUBSTITUTED FOR TWO #5 REBARS IN 8" LINTELS ONLY. 6. THE DESIGNER MAY EVALUATE CONCENTRATED LOADS

FROM THE SAFE LOAD TABLES BY CALCULATING THE MAXIMUM RESISTING MOMENT AND SHEAR AT D-AWAY 7. FOR COMPOSITE LINTEL HEIGHTS NOT SHOWN, USE SAFE LOAD FROM NEXT LOWER HEIGHT SHOWN.

8. FOR LINTELS LENGTHS NOT SHOWN, USE SAFE LOAD FROM NEXT LONGEST LENGTH SHOWN. 9. ALL SAFE LOADS IN UNITS OF POUNDS PER LINEAR FOOT. 10. ALL SAFE LOADS BASED ON SIMPLY SUPPORTED SPAN. 11. THE NUMBER IN THE PARENTHESIS INDICATES THE PERCENT REDUCTION FOR GRADE 40 FIELD ADDED REBAR. EXAMPLE 7'-6" LINTEL TYPE 8F32-1B SAFE GRAVITY LOAD = 6472 =><sup>(15)</sup>W/ 15% REDUCTION 6472 (.85) = 5501 PLF

8" NOMINAL WIDTH

# TYPE DESIGNATION



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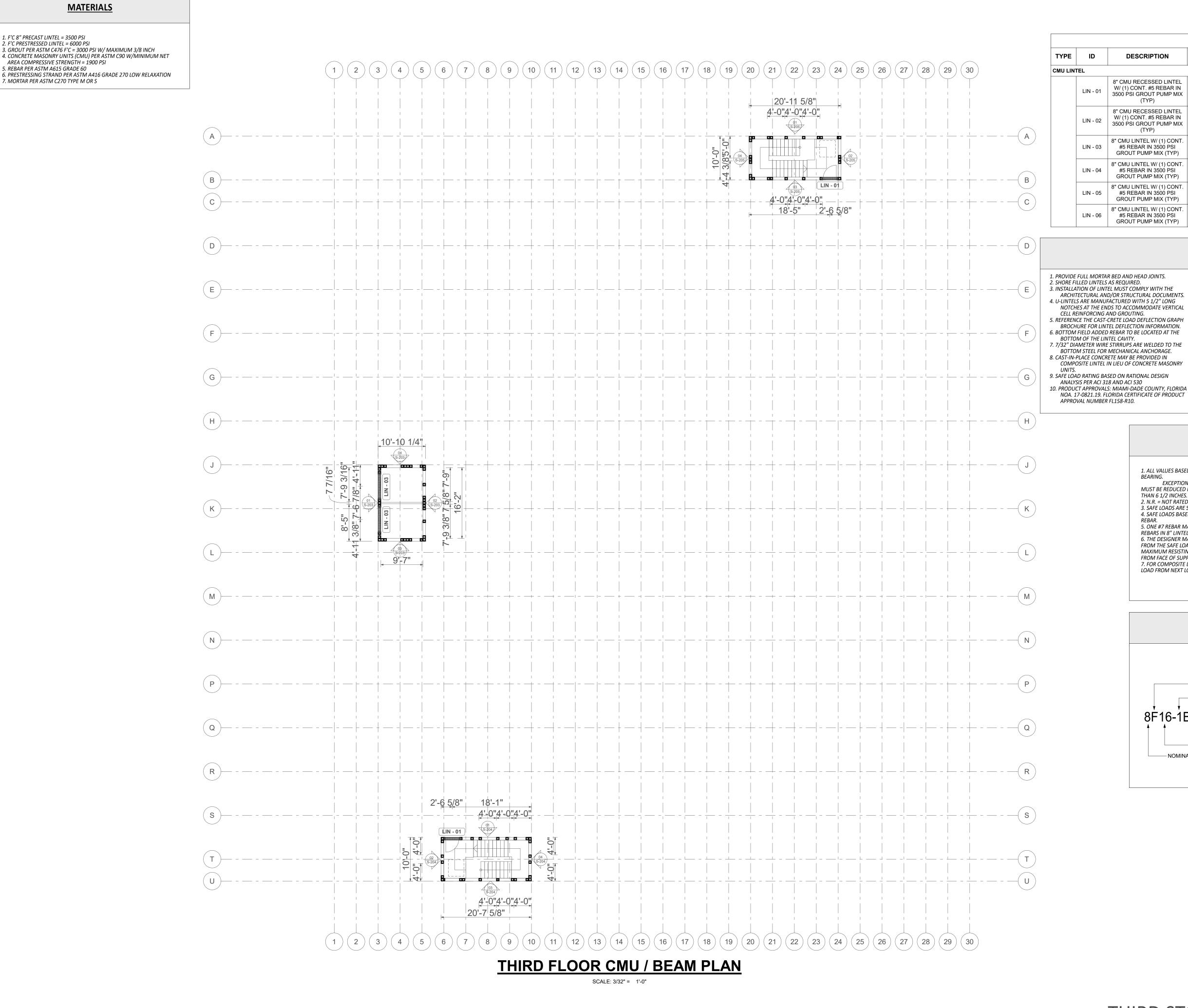
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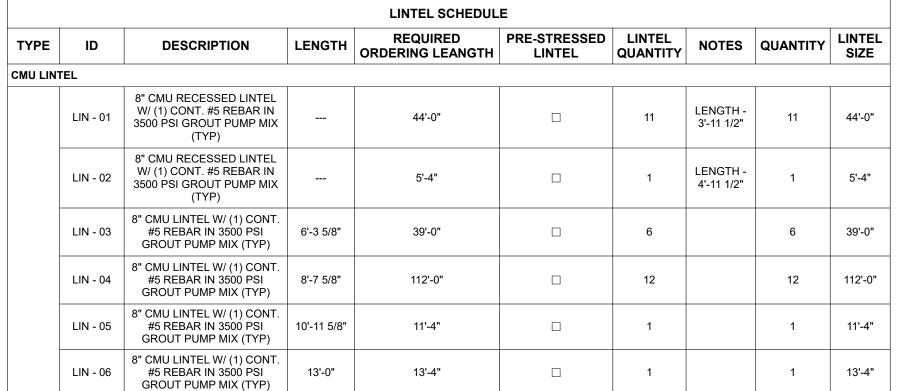
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SECOND STORY CMU / CONCRETE BEAM PLAN

S.M.K. SHEET





#### **LINTEL NOTES**

1. PROVIDE FULL MORTAR BED AND HEAD JOINTS. 2. SHORE FILLED LINTELS AS REQUIRED. 3. INSTALLATION OF LINTEL MUST COMPLY WITH THE

ARCHITECTURAL AND/OR STRUCTURAL DOCUMENTS. 4. U-LINTELS ARE MANUFACTURED WITH 5 1/2" LONG NOTCHES AT THE ENDS TO ACCOMMODATE VERTICAL 5. REFERENCE THE CAST-CRETE LOAD DEFLECTION GRAPH

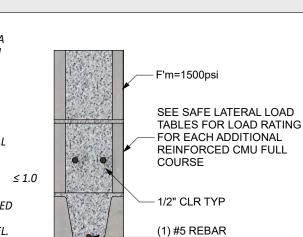
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#### SAFE LOAD TABLE NOTES

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REBAR AT TOP

--- NOMINAL HEIGHT

— NOMINAL WIDTH

8. FOR LINTELS LENGTHS NOT SHOWN, USE SAFE LOAD FROM NEXT LONGEST LENGTH SHOWN. 9. ALL SAFE LOADS IN UNITS OF POUNDS PER LINEAR FOOT. 10. ALL SAFE LOADS BASED ON SIMPLY SUPPORTED SPAN. 11. THE NUMBER IN THE PARENTHESIS INDICATES THE PERCENT REDUCTION FOR GRADE 40 FIELD ADDED REBAR. EXAMPLE 7'-6" LINTEL TYPE 8F32-1B SAFE GRAVITY LOAD = 6472 =><sup>(15)</sup>W/ 15%

REDUCTION 6472 (.85) = 5501 PLF

\_\_#5 REBAR AT TOP MIN. (1) REQ'D F = FILLED WITH GROUT / U = UNFILLED QUANTITY OF #5 REBAR AT BOTTOM OF LINTEL CAVITY #5 REBAR AT BOTTOM QUANTITY OF #5

## TYPE DESIGNATION

OF LINTEL CAVITY BOTTOM REINFORCING — PROVIDED IN LINTEL (VARIES) 7-5/8" ACTUAL 8" NOMINAL WIDTH

ENGINEERS

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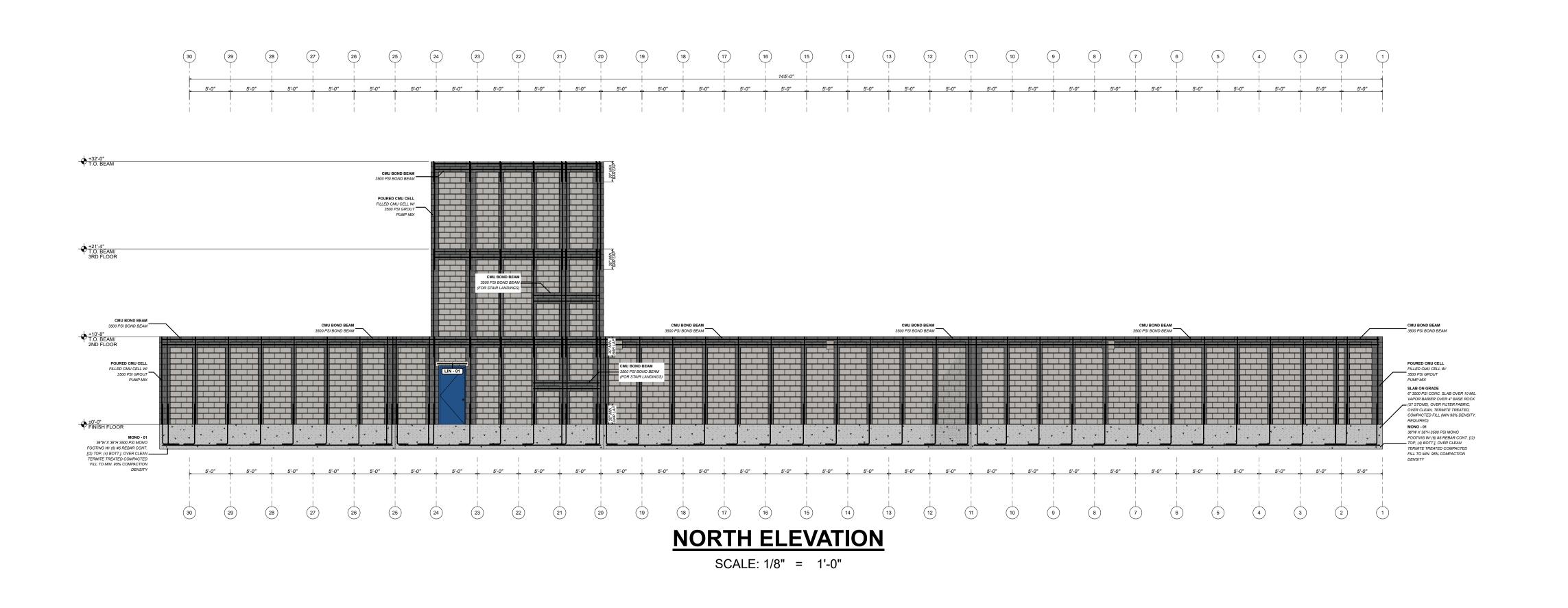
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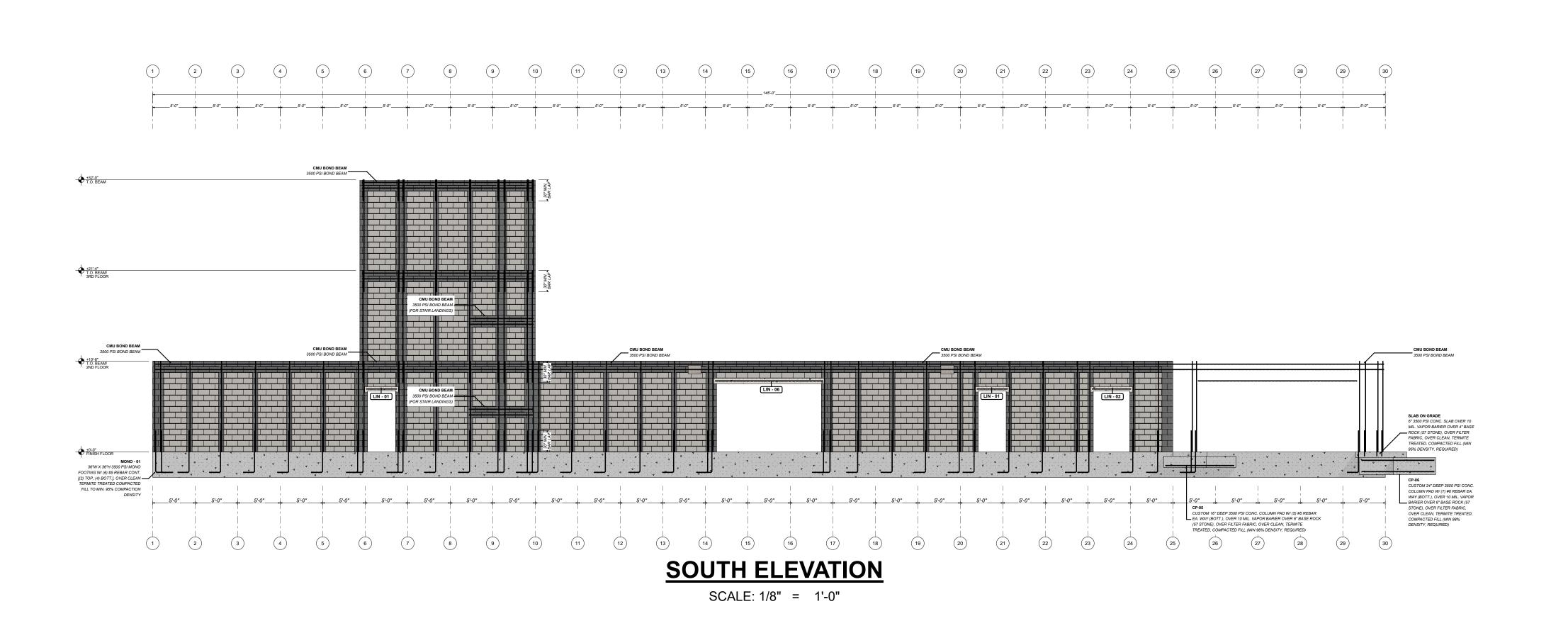
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THIRD STORY CMU / CONCRETE BEAM PLAN



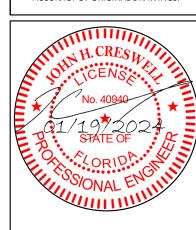


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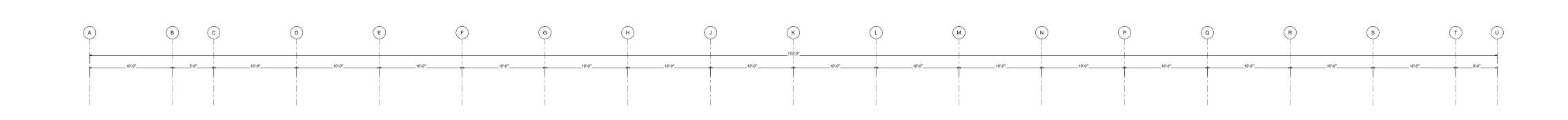
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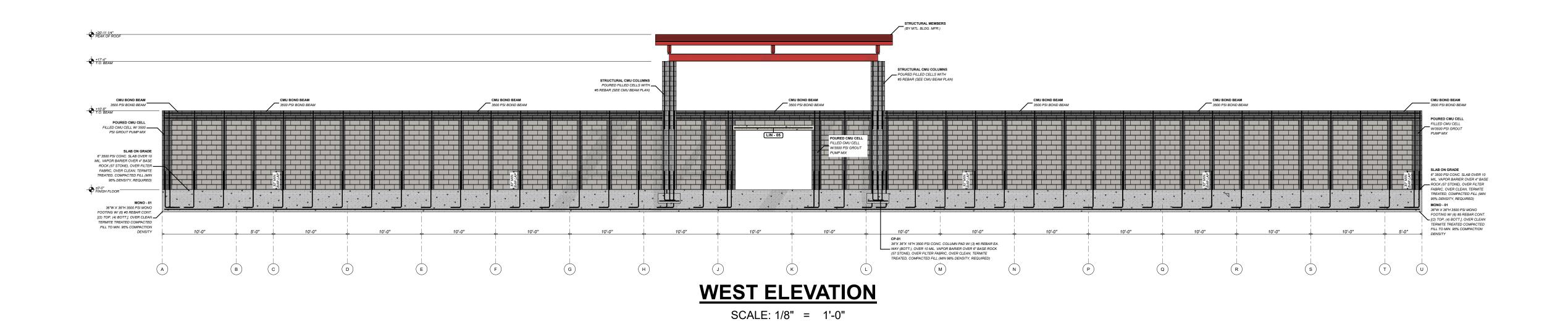
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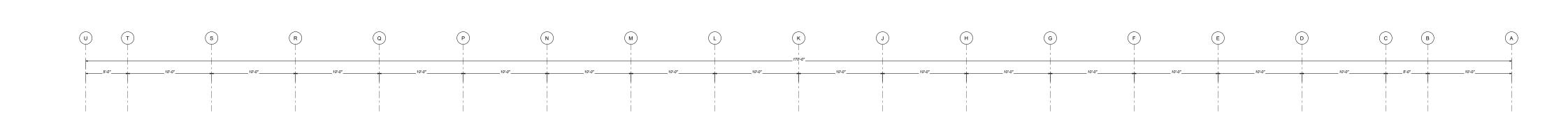
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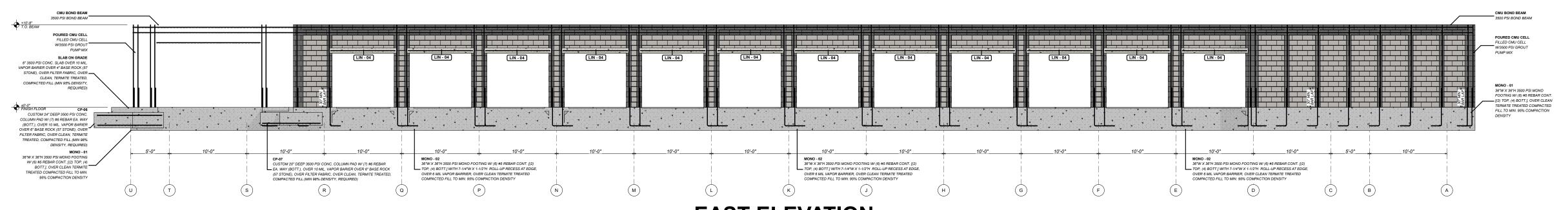
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STRUCTURAL ELEVATIONS









**EAST ELEVATION** SCALE: 1/8" = 1'-0"

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THREE-STORY STORAGE
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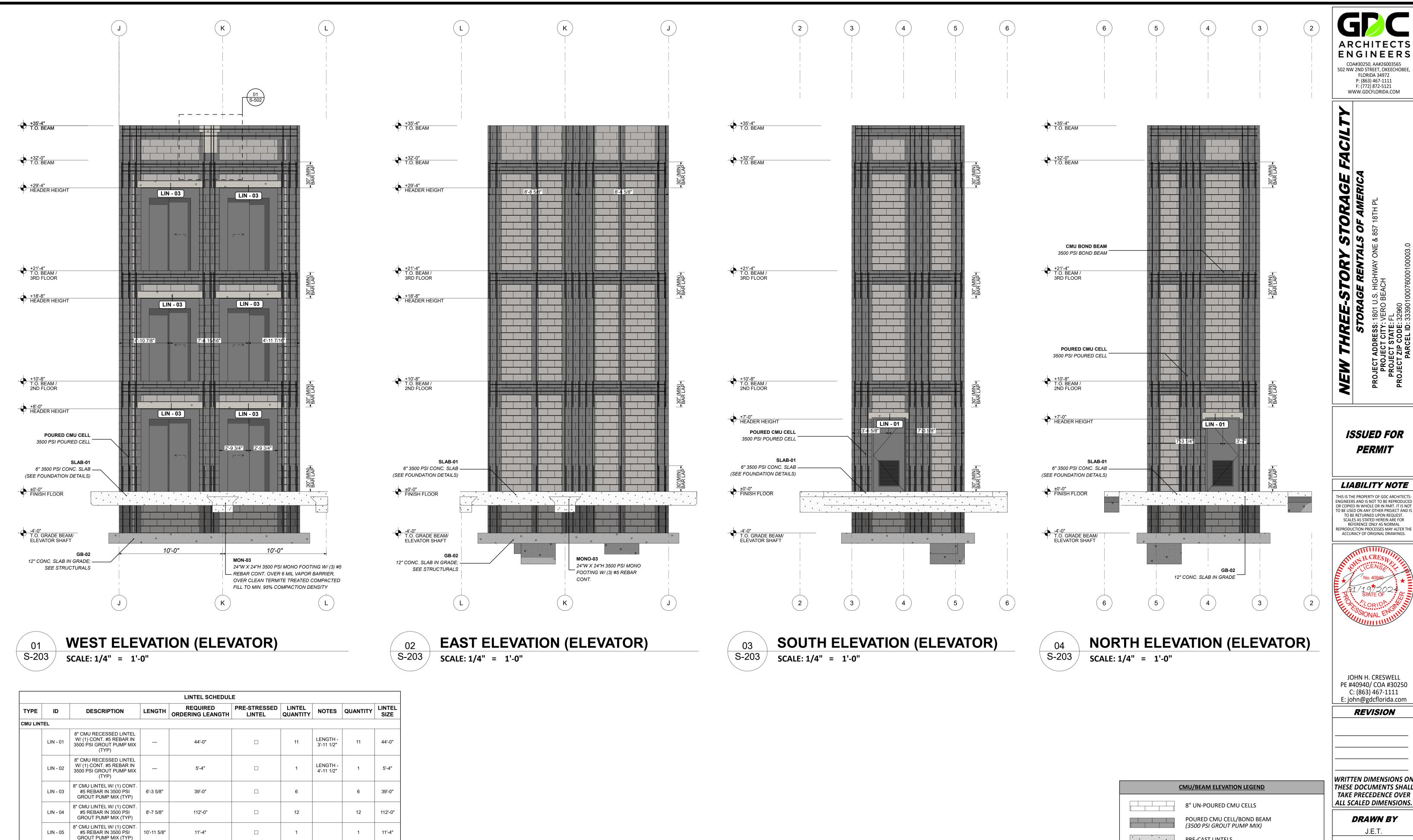
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8" CMU LINTEL W/ (1) CONT

#5 REBAR IN 3500 PSI

GROUT PUMP MIX (TYP)

13'-4"

13'-0"

13'-4"

STRUCTURAL ELEVATIONS

CONCRETE SECTION CUTS

CONCRETE IN ELEVATION

CONTINUOUS #5 REBAR IN 3500 PSI CONC.

PRE-CAST LINTELS

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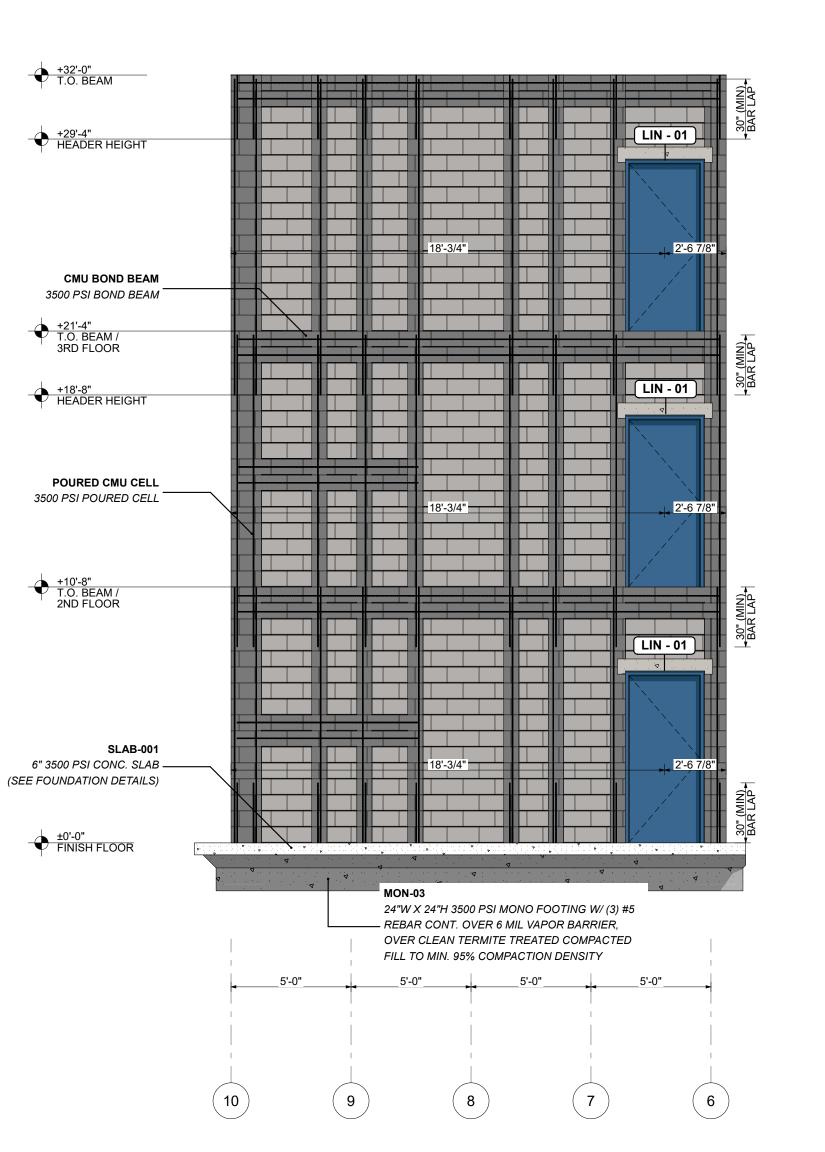
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S-204

SCALE: 1/4" = 1'-0"

02 S-204



SCALE: 1/4" = 1'-0"

03 S-204

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# **SOUTH ELEVATION AT STAIR #1**

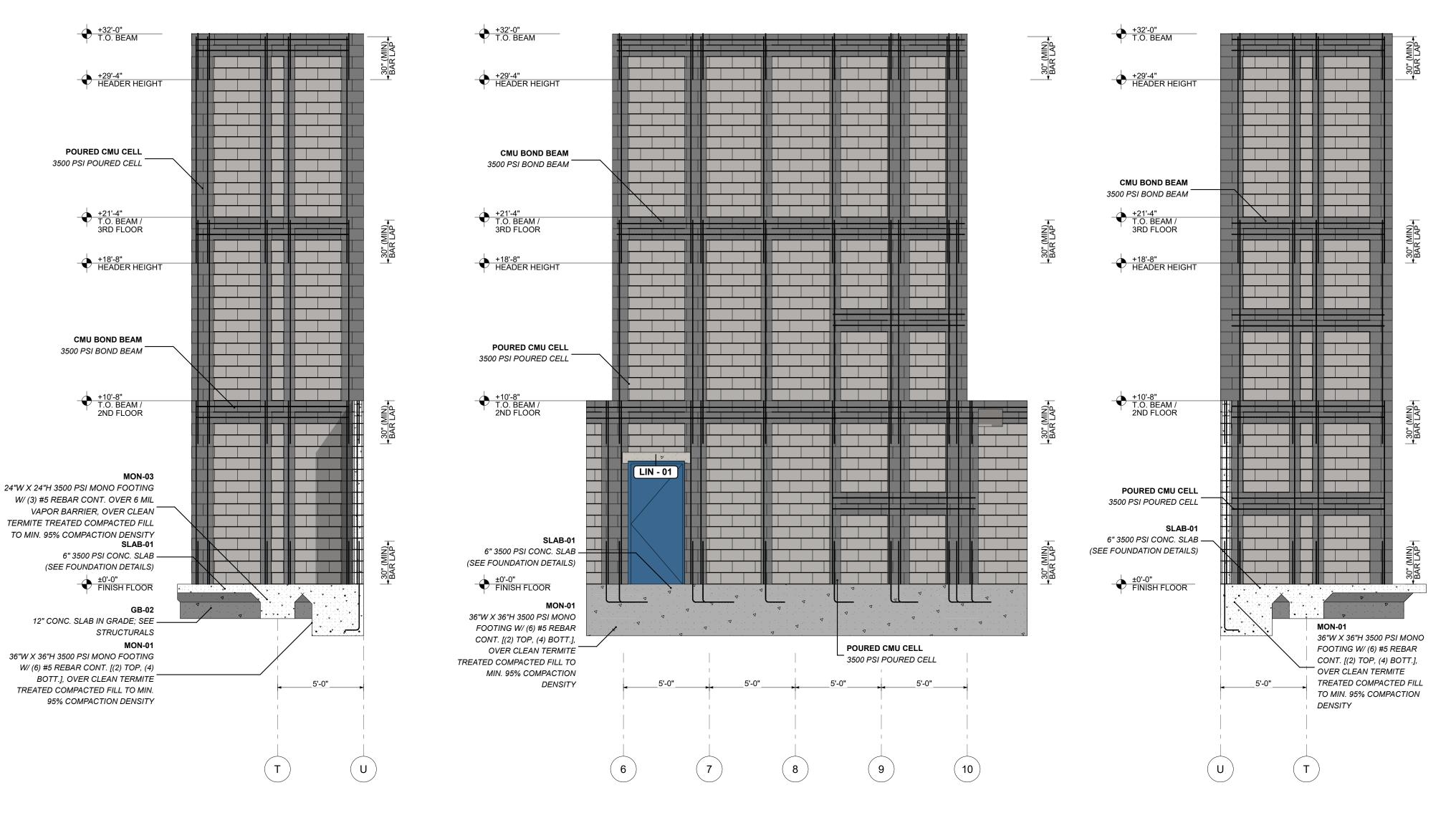
SCALE: 1/4" = 1'-0"

- (04 S-204)

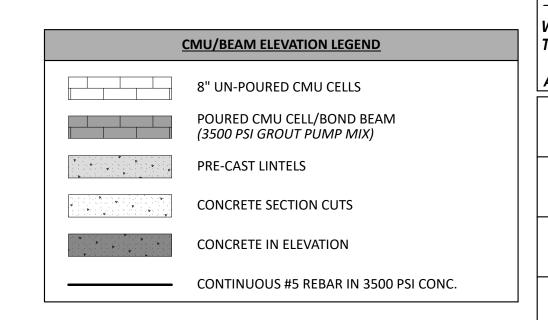
## **WEST ELEVATION AT STAIR #1**

SCALE: 1/4" = 1'-0"

LINTEL SCHEDULE									
TYPE	ID	DESCRIPTION	LENGTH	REQUIRED ORDERING LEANGTH	PRE-STRESSED LINTEL	LINTEL QUANTITY	NOTES	QUANTITY	LINTEL
CMU LIN	TEL		•		•				
	LIN - 01	8" CMU RECESSED LINTEL W/ (1) CONT. #5 REBAR IN 3500 PSI GROUT PUMP MIX (TYP)		44'-0"		11	LENGTH - 3'-11 1/2"	11	44'-0"
	LIN - 02	8" CMU RECESSED LINTEL W/ (1) CONT. #5 REBAR IN 3500 PSI GROUT PUMP MIX (TYP)		5'-4"		1	LENGTH - 4'-11 1/2"	1	5'-4"
	LIN - 03	8" CMU LINTEL W/ (1) CONT. #5 REBAR IN 3500 PSI GROUT PUMP MIX (TYP)	6'-3 5/8"	39'-0"		6		6	39'-0"
	LIN - 04	8" CMU LINTEL W/ (1) CONT. #5 REBAR IN 3500 PSI GROUT PUMP MIX (TYP)	8'-7 5/8"	112'-0"		12		12	112'-0"
	LIN - 05	8" CMU LINTEL W/ (1) CONT. #5 REBAR IN 3500 PSI GROUT PUMP MIX (TYP)	10'-11 5/8"	11'-4"		1		1	11'-4"
	LIN - 06	8" CMU LINTEL W/ (1) CONT. #5 REBAR IN 3500 PSI GROUT PUMP MIX (TYP)	13'-0"	13'-4"		1		1	13'-4"







STRUCTURAL ELEVATIONS

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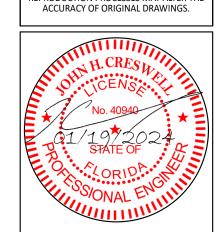
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-STORY STORAGE FACIL 4GE RENTALS OF AMERICA
U.S. HIGHWAY ONE & 857 18TH PL

PROJECT ADDRESS: 1801 U.S. HIGHY PROJECT CITY: VERO BEACH PROJECT STATE: FL PROJECT ZIP CODE: 32960

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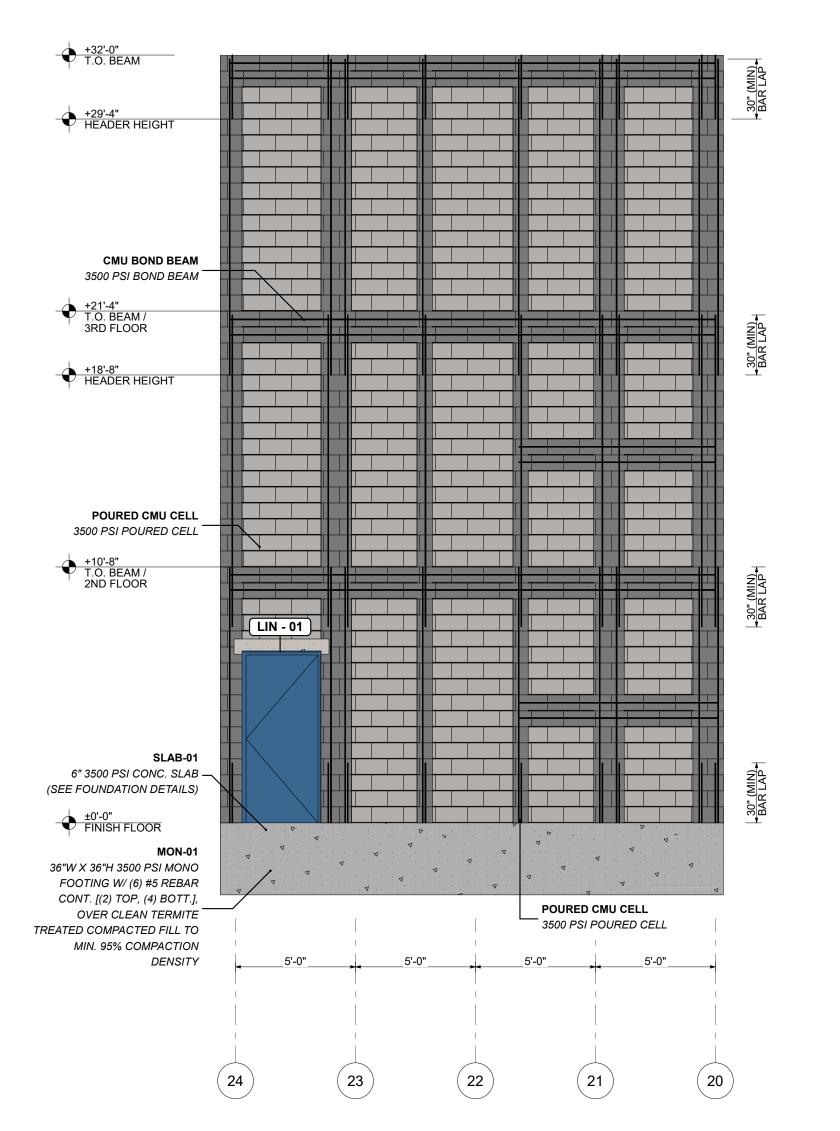
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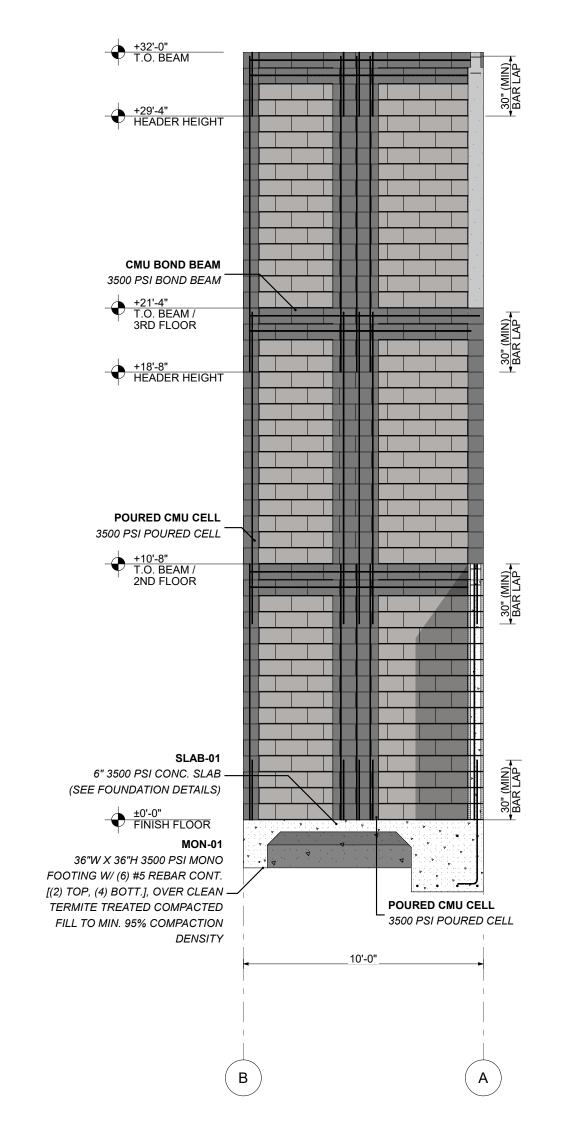
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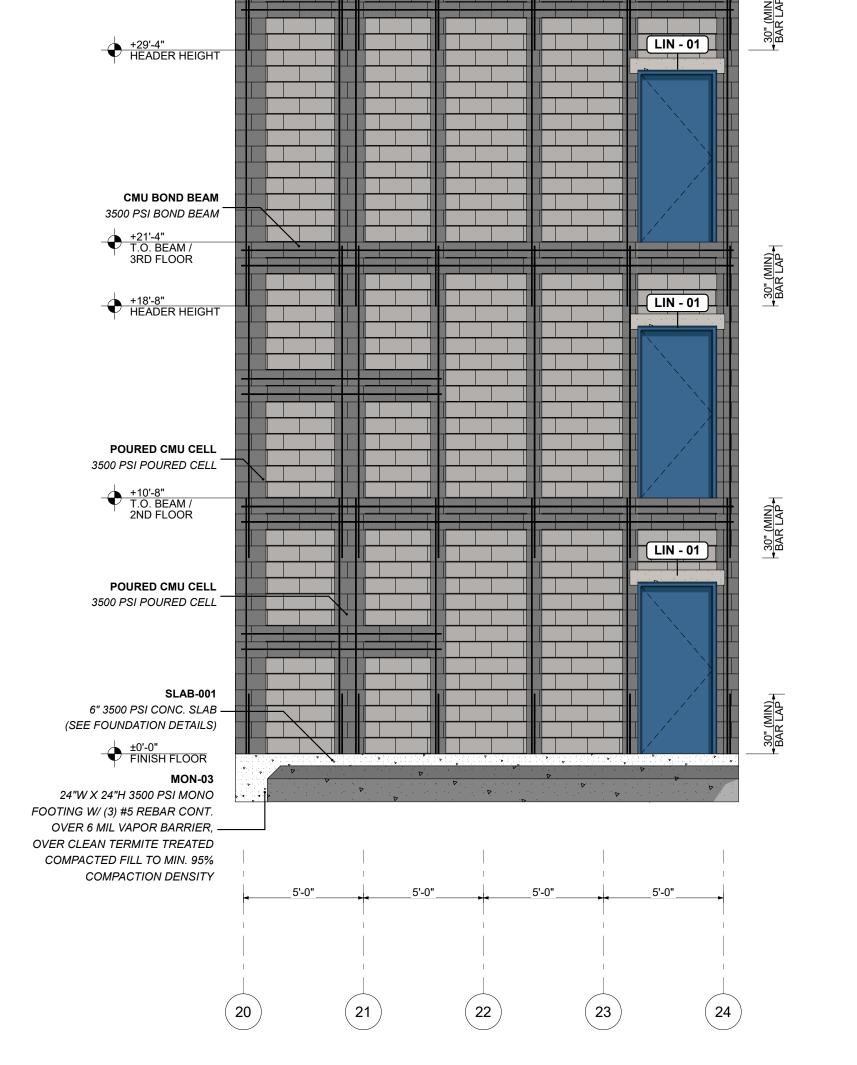
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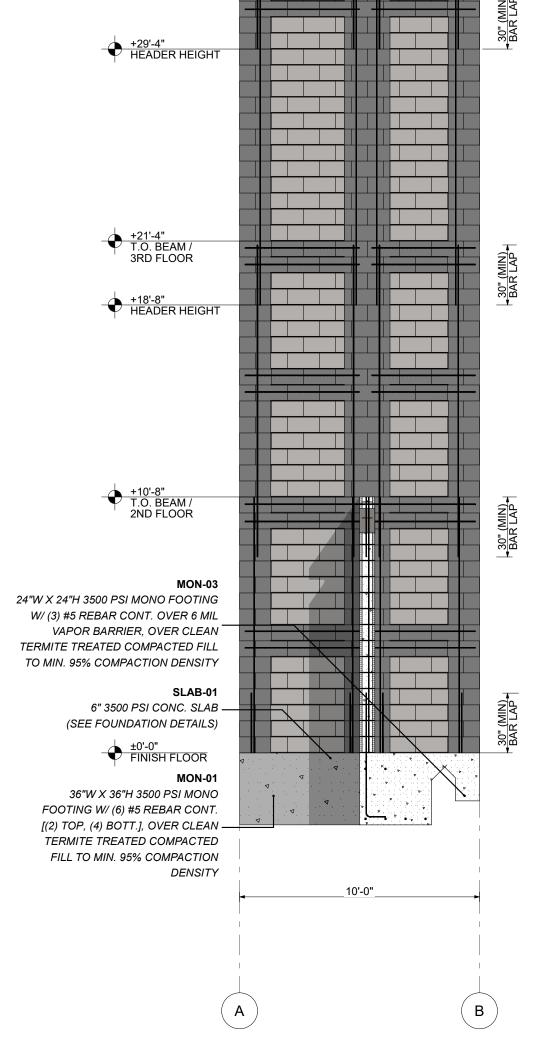
**DATE** 1/19/2024

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01 S-205

NORTH ELEVATION AT STAIR #2

SCALE: 1/4" = 1'-0"

02 S-205

20

**EAST ELEVATION AT STAIR #2** 

SCALE: 1/4" = 1'-0"

03 S-205 **SOUTH ELEVATION AT STAIR #2** 

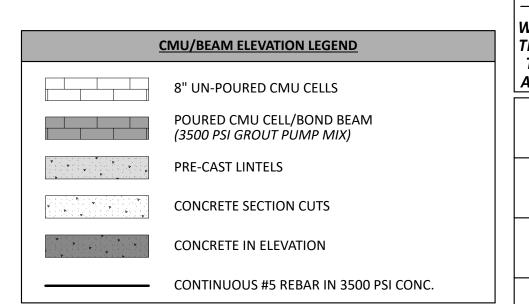
SCALE: 1/4" = 1'-0"

04 S-205

**WEST ELEVATION AT STAIR #2** 

SCALE: 1/4" = 1'-0"

LINTEL SCHEDULE REQUIRED PRE-STRESSED LINTEL TYPE LENGTH NOTES QUANTITY DESCRIPTION ORDERING LEANGTH LINTEL QUANTITY CMU LINTEL W/ (1) CONT. #5 REBAR IN LENGTH -44'-0" 44'-0" 3500 PSI GROUT PUMP MIX 3'-11 1/2" 8" CMU RECESSED LINTEL W/ (1) CONT. #5 REBAR IN LENGTH -5'-4" 5'-4" 4'-11 1/2" 3500 PSI GROUT PUMP MIX 8" CMU LINTEL W/ (1) CONT. #5 REBAR IN 3500 PSI 6'-3 5/8" 39'-0" 39'-0" GROUT PUMP MIX (TYP) 8" CMU LINTEL W/ (1) CONT. 12 112'-0" #5 REBAR IN 3500 PSI 8'-7 5/8" 112'-0" 12 GROUT PUMP MIX (TYP) 8" CMU LINTEL W/ (1) CONT 11'-4" 11'-4" #5 REBAR IN 3500 PSI 10'-11 5/8" GROUT PUMP MIX (TYP) 8" CMU LINTEL W/ (1) CONT 13'-4" 13'-0" 13'-4" #5 REBAR IN 3500 PSI GROUT PUMP MIX (TYP)



STRUCTURAL ELEVATIONS

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ENGINEERS

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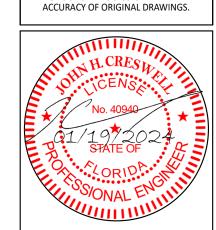
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STORY STORAGE FAC SE RENTALS OF AMERICA HIGHWAY ONE & 857 18TH PL

PROJECT ADDRESS: 1801 U.S. HIGHWAY O PROJECT CITY: VERO BEACH PROJECT STATE: FL PROJECT ZIP CODE: 32960

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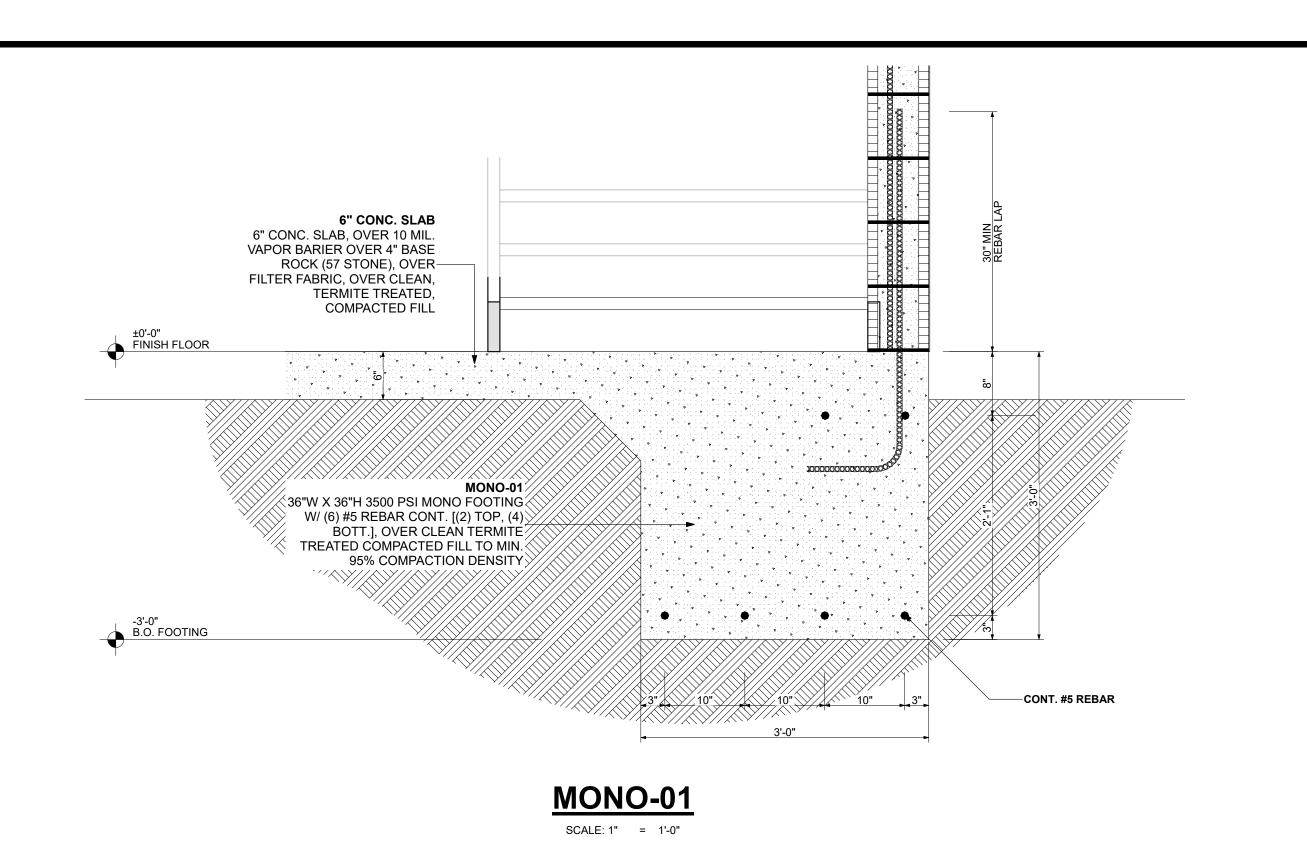
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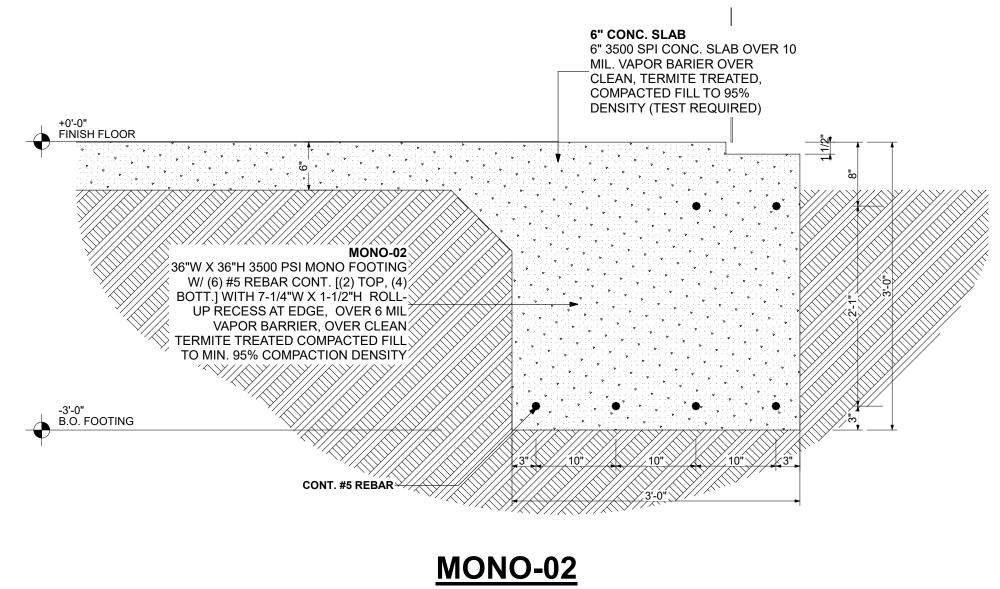
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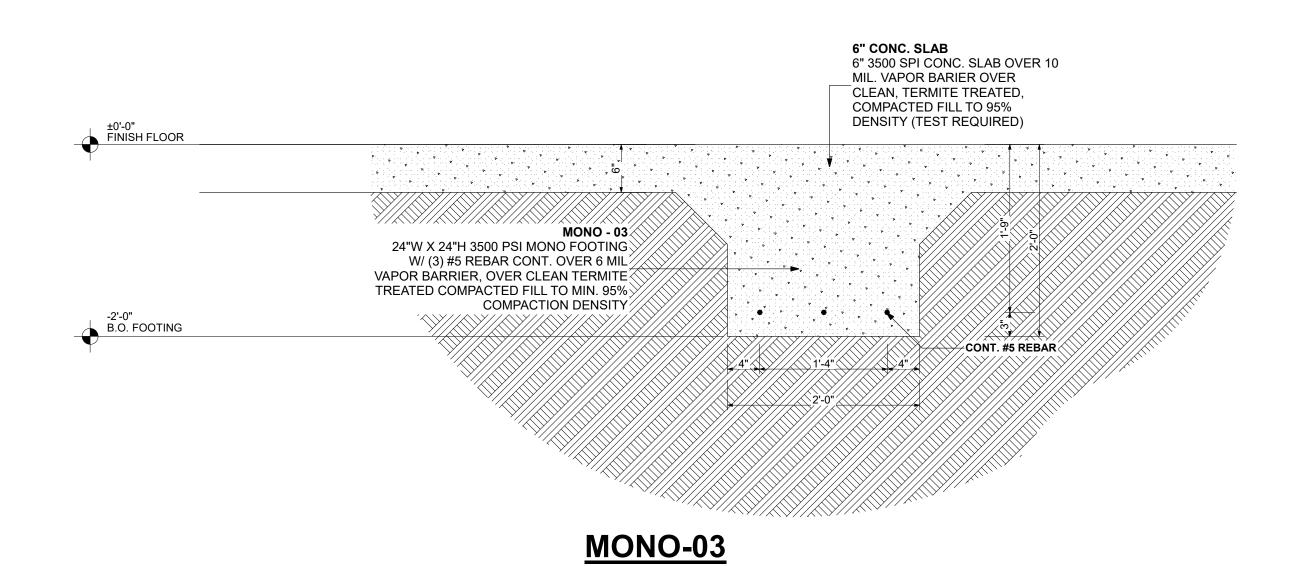
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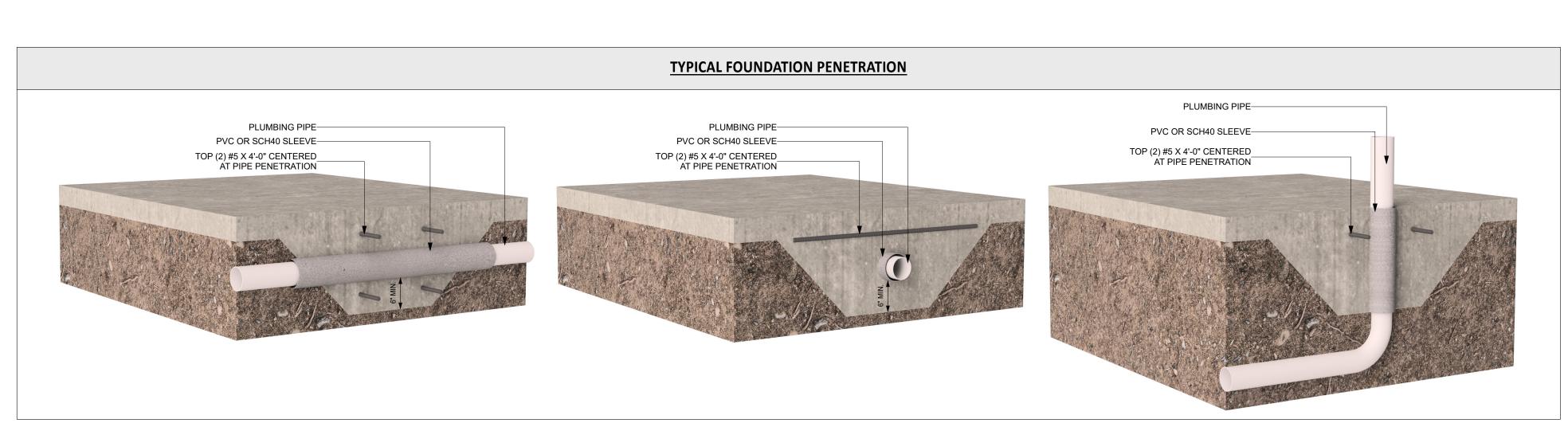
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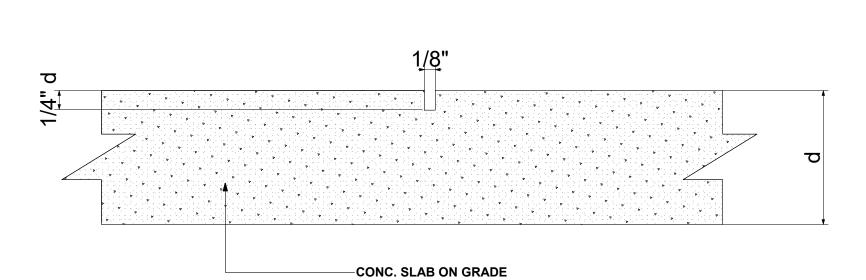
1/19/2024 **SHEET** 











**CONCRETE JOINT DETAIL** 

SCALE: 3" = 1'-0"

THE TIMING OF THE INSTALLATION OF CONTROL JOINTS IS VERY IMPORTANT, THEREFORE THE CUTTING OF THE CONCRETE SHOULD BEGIN AFTER THE CONCRETE HAS ATTAINED SUFFICIENT STRENGTH TO SUPPORT THE WEIGHT OF THE SAW AND ALL THE BLADE TO CUT THROUGH THE AGGREGATE WITHOUT RAVELING.

FOUNDATION DETAILS

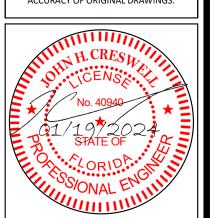
SCALE: 1" = 1'-0"

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**REVISION** 

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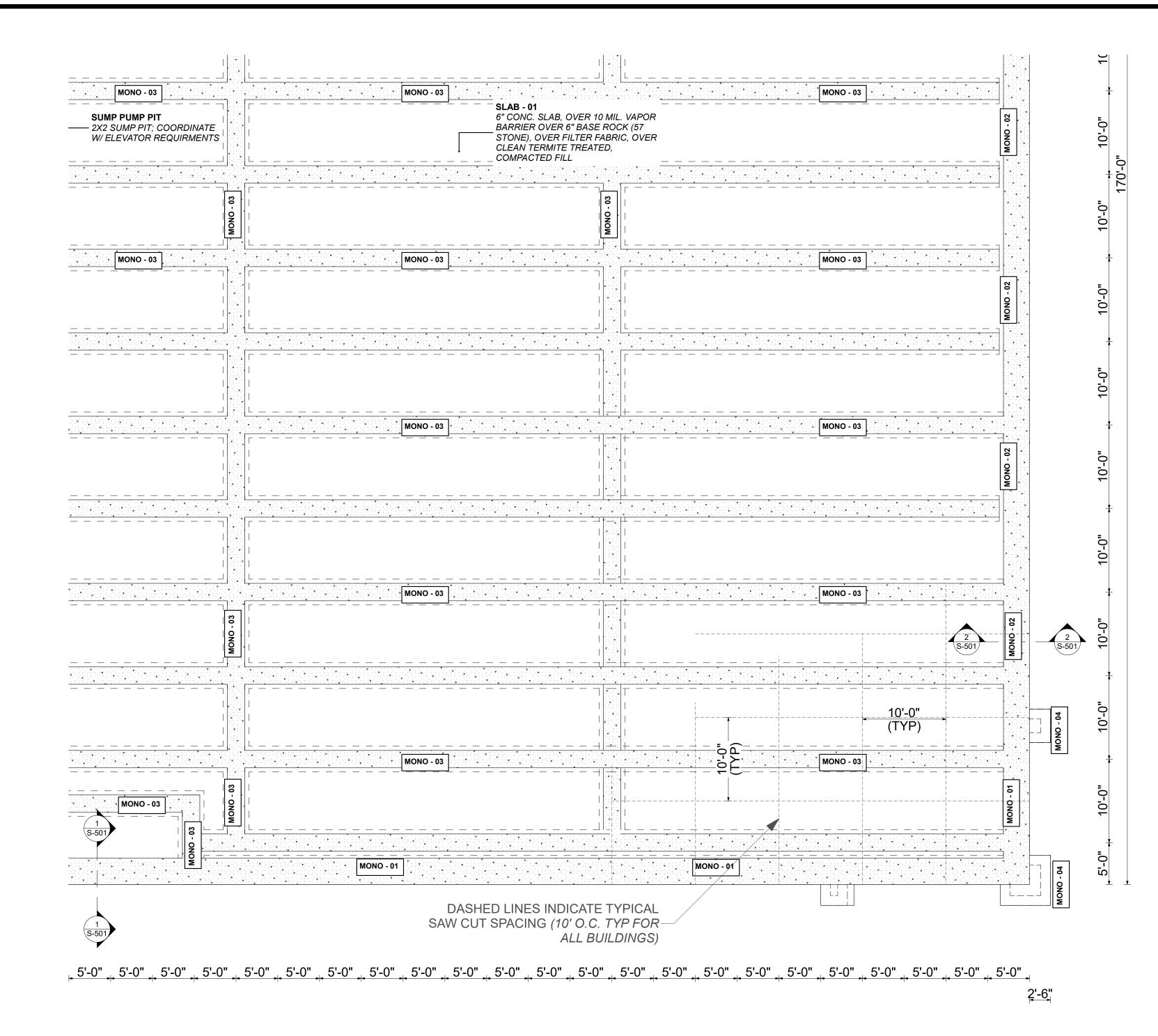
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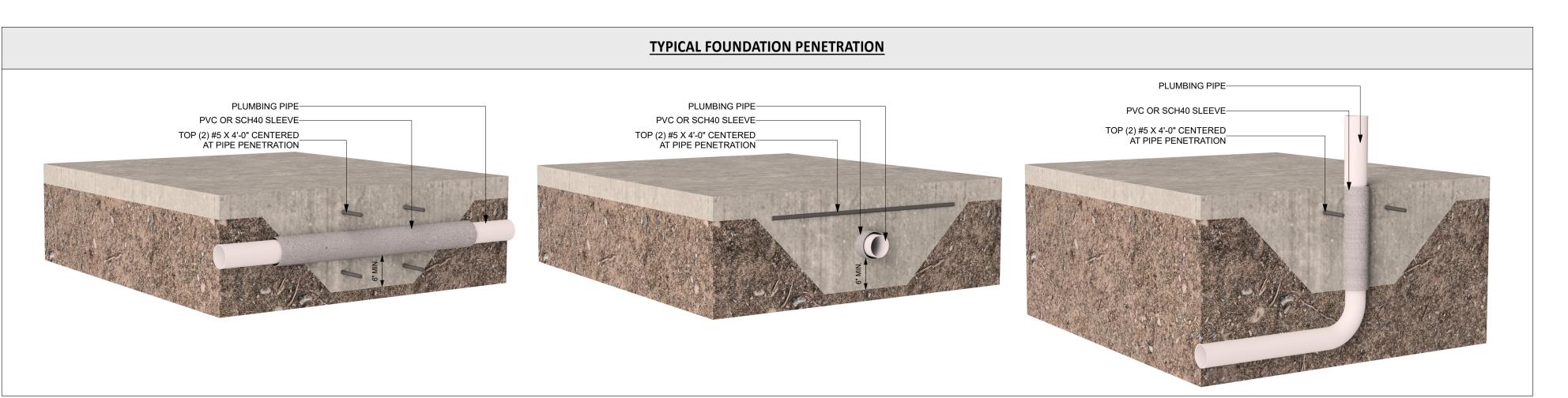
> DATE 1/19/2024

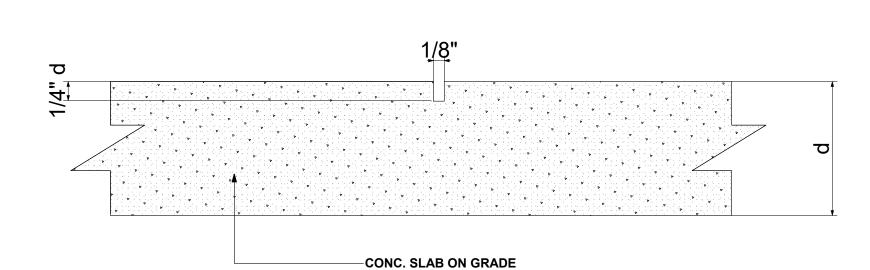
SHEET



# **FOUNDATION (SAW CUT)**

SCALE: 1/8" = 1'-0"





**CONCRETE JOINT DETAIL** 

SCALE: 3" = 1'-0"

THE TIMING OF THE INSTALLATION OF CONTROL
JOINTS IS VERY IMPORTANT, THEREFORE THE
CUTTING OF THE CONCRETE SHOULD BEGIN AFTER
THE CONCRETE HAS ATTAINED SUFFICIENT
STRENGTH TO SUPPORT THE WEIGHT OF THE SAW
AND ALL THE BLADE TO CUT THROUGH THE
AGGREGATE WITHOUT RAVELING.

FOUNDATION DETAILS



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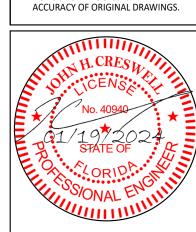
V I TKEE-S I OKY S I OKAG.

STORAGE RENTALS OF AMERIC
SCT ADDRESS: 1801 U.S. HIGHWAY ONE & 857 18TH PL
ROJECT CITY: VERO BEACH

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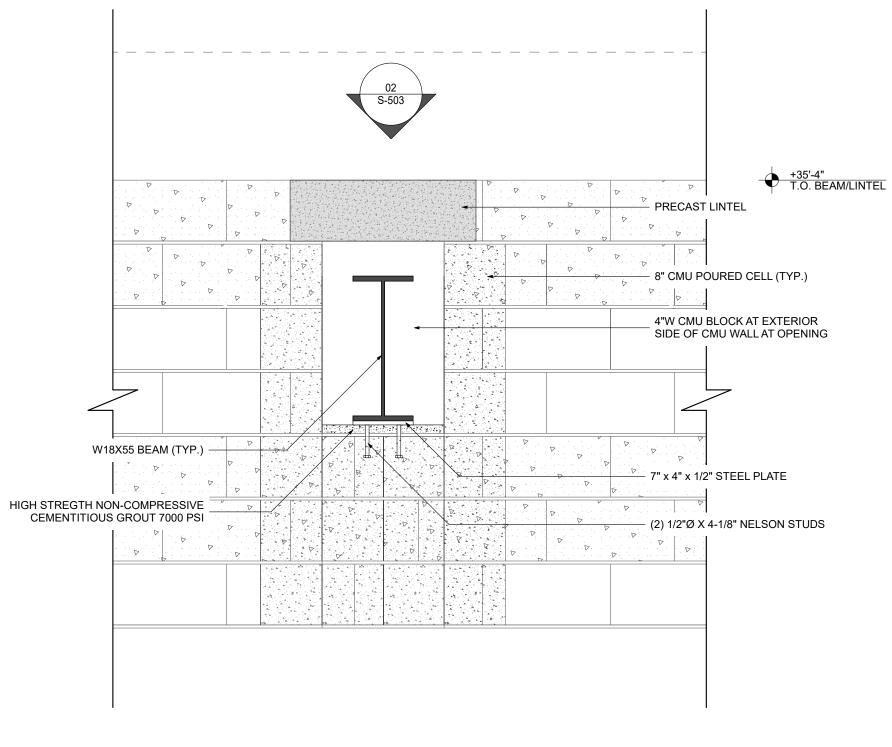
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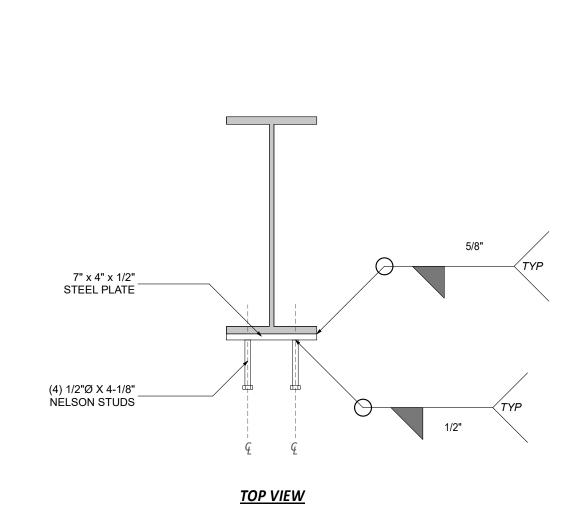


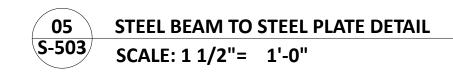


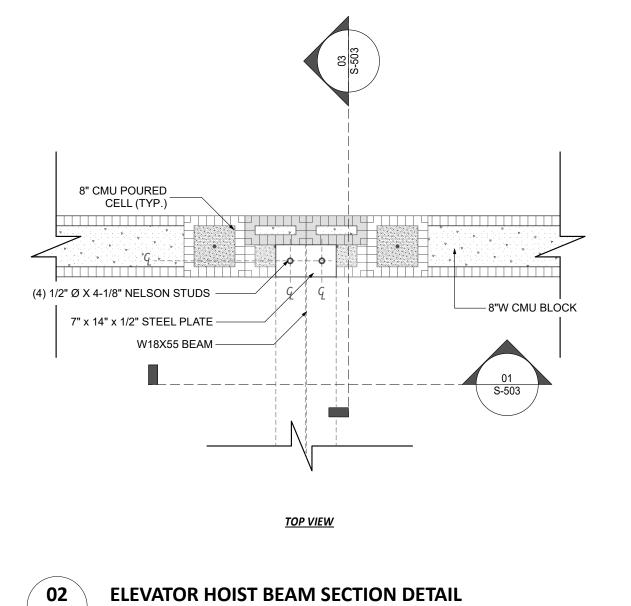
7" x 4" x 1/2" STEEL PLATE

(2) 1/2"Ø X 4-1/8" NELSON STUDS

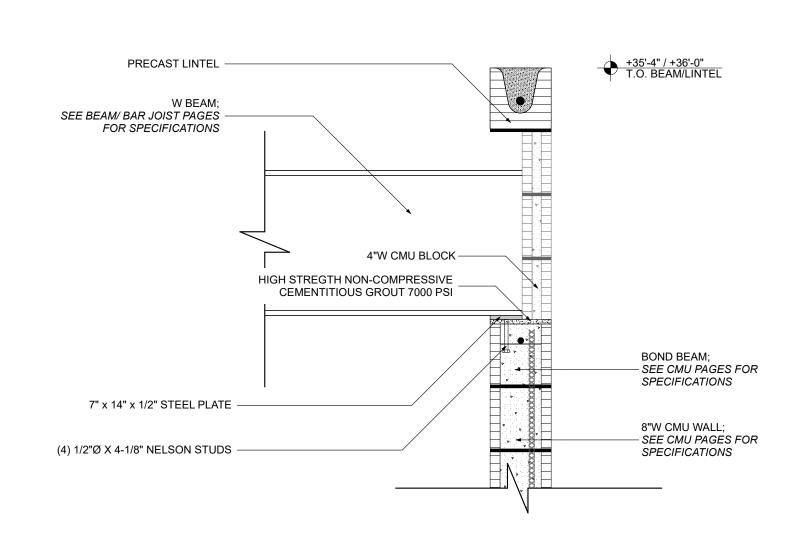
STEEL PLATE DETAIL











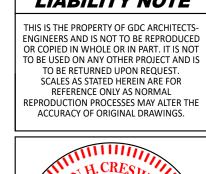


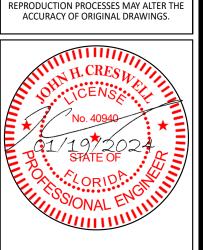
ARCHITECTS ENGINEERS

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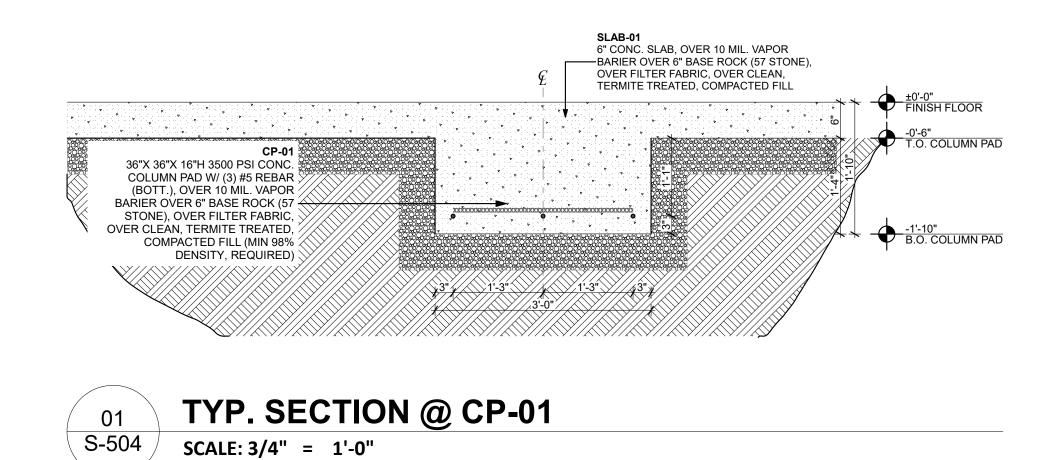
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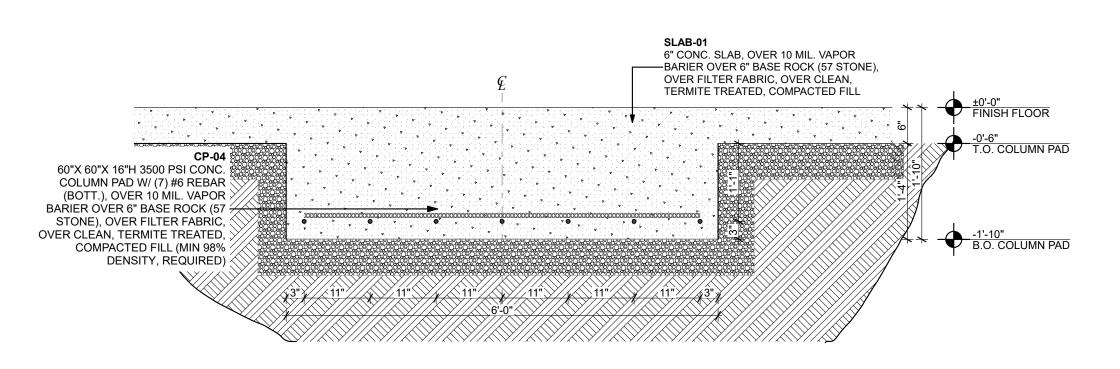
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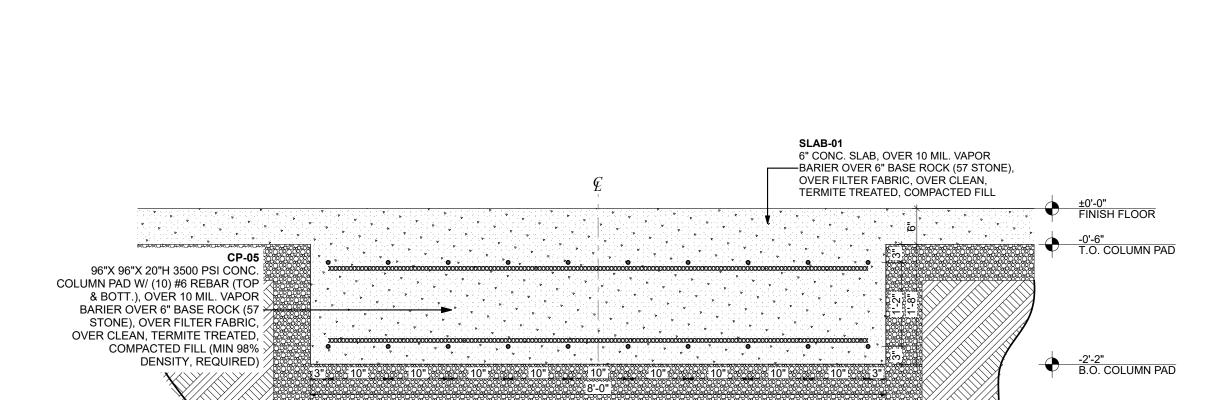
DATE

1/19/2024 **SHEET** 





03 TYP. SECTION @ CP-04 S-504 SCALE: 3/4" = 1'-0"



SLAB-01
6" CONC. SLAB, OVER 10 MIL. VAPOR
—BARIER OVER 6" BASE ROCK (57 STONE),
OVER FILTER FABRIC, OVER CLEAN,
TERMITE TREATED, COMPACTED FILL

±0'-0" FINISH FLOOR

-0'-6" T.O. COLUMN PAD

-1'-10" B.O. COLUMN PAD

04 TYP. SECTION @ CP-05 S-504 SCALE: 3/4" = 1'-0"

60"X 60"X 16"H 3500 PSI CONC. COLUMN PAD W/ (5) #5 REBAR (BOTT.), OVER 10 MIL. VAPOR BARIER OVER 6" BASE ROCK (57

STONE), OVER FILTER FABRIC,

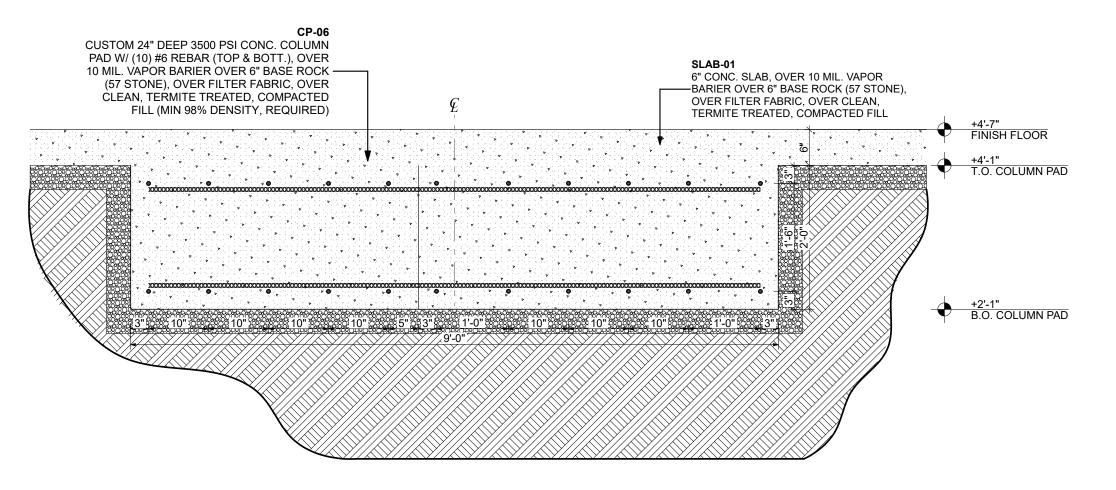
COMPACTED FILL (MIN 98% DENSITY, REQUIRED)

TYP. SECTION @ CP-02

SCALE: 3/4" = 1'-0"

OVER CLEAN, TERMITE TREATED,

S-504



05 TYP. SECTION @ CP-06 S-504 SCALE: 3/4" = 1'-0"



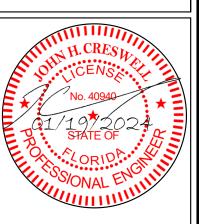
OF AMERICA
857 18TH PL

EW THREE-STORY STORAGE

STORAGE RENTALS OF AMERICA
ROJECT ADDRESS: 1801 U.S. HIGHWAY ONE & 857 18TH PL
PROJECT CITY: VERO BEACH

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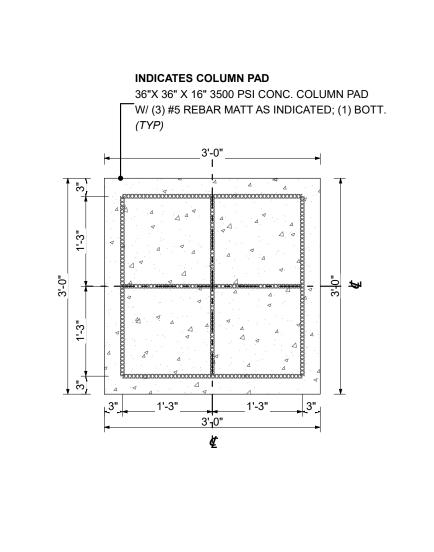
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**ENLARGED DETAIL @ CP-01** 

SCALE: 3/4" = 1'-0"

INDICATES COLUMN PAD 60"X 60" X 16" 3500 PSI CONC. COLUMN PAD  $\overline{\phantom{a}}$ W/ (5) #5 REBAR MATT AS INDICATED; (1) BOTT 3" 1'-1 1/2" 1'-1 1/2" 1'-1 1/2" 3" 5'-0" 72"X 72" X 16" 3500 PSI CONC. COLUMN PAD W/ (7) #6 REBAR MATT AS INDICATED; (1) BOTT

INDICATES COLUMN PAD

3" 1'-1 1/2" 1'-1 1/2" 1'-1 1/2" 1'-1 1/2" 3"

ENLARGED DETAIL @

**CUSTOM CP-03** 

INDICATES COLUMN PAD

W/ #5 REBAR MATT AS INDICATED;

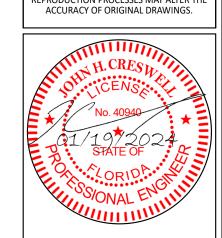
CUSTOM 16" DEEP, 3500 PSI CONC. COLUMN PAD

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S-505

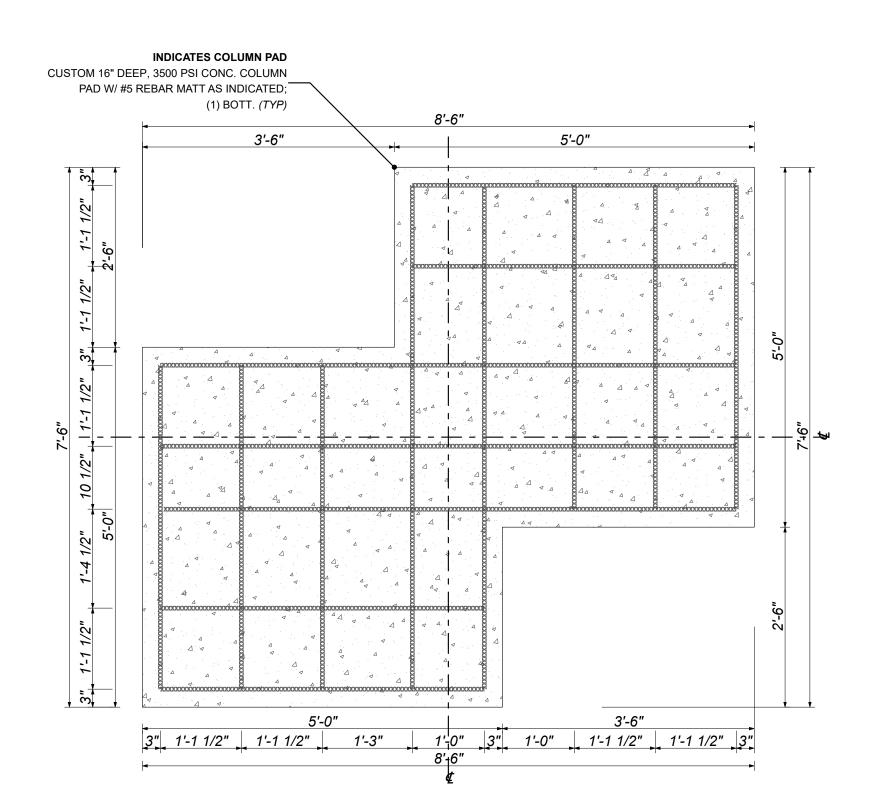
**ENLARGED DETAIL @ CP-02** 

SCALE: 3/4" = 1'-0"

S-505

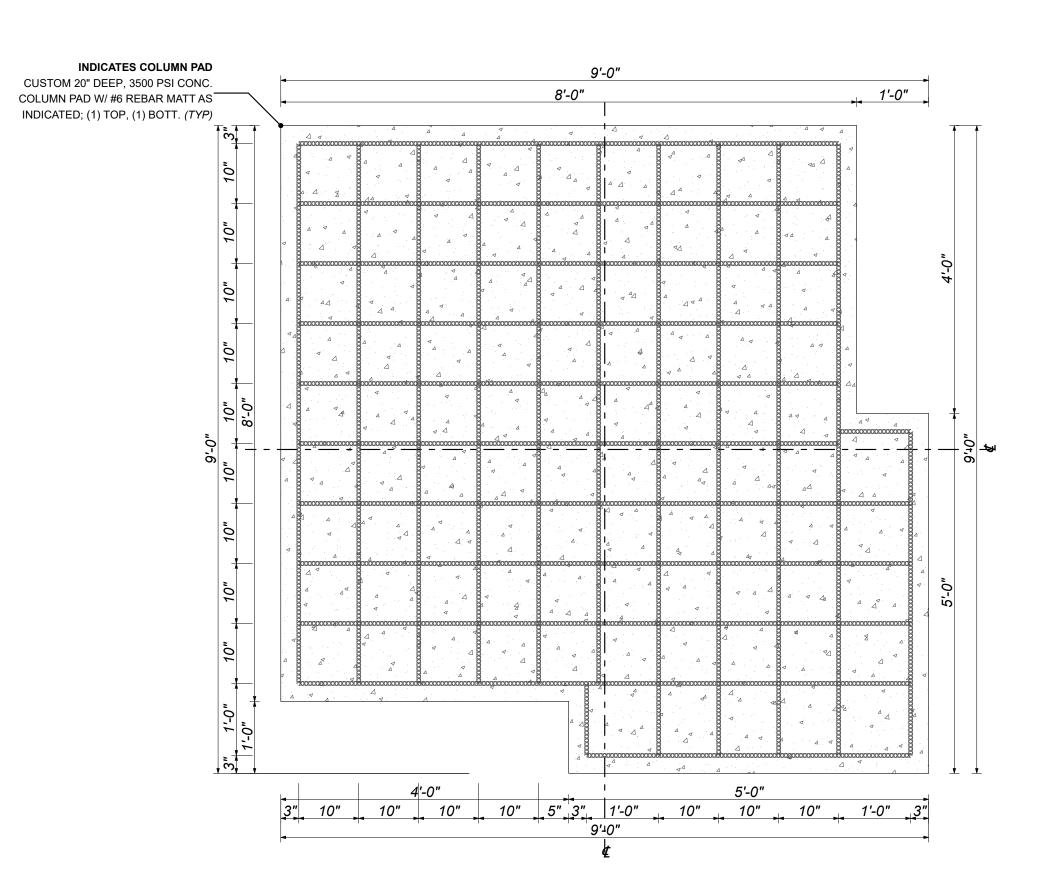
ENLARGED DETAIL @ CP-04 SCALE: 3/4" = 1'-0"

S-505

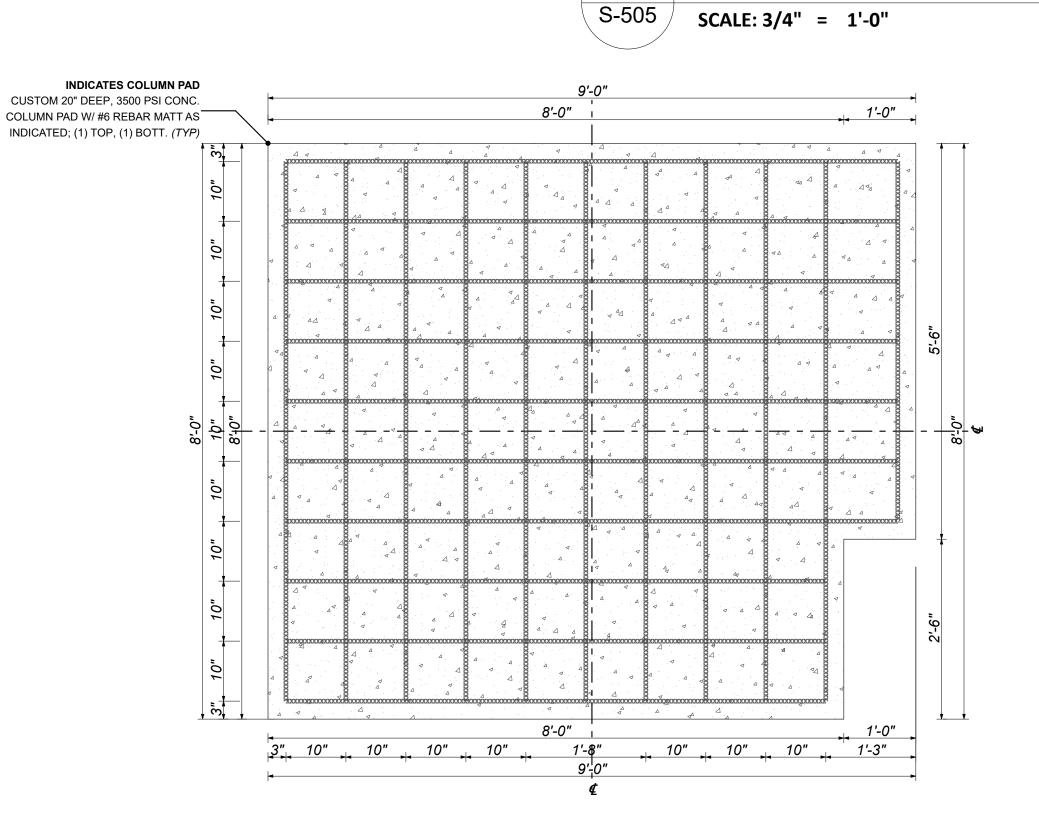


ENLARGED DETAIL @ CUSTOM CP-05 SCALE: 3/4" = 1'-0"

S-505

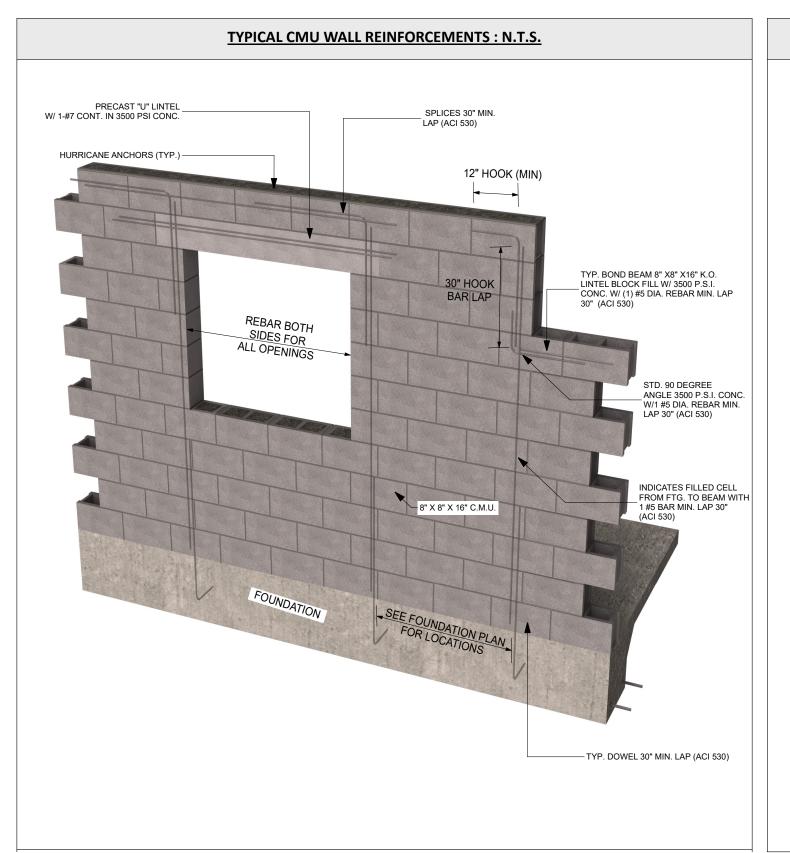


**ENLARGED DETAIL @ CUSTOM CP-06** S-505 SCALE: 3/4" = 1'-0"

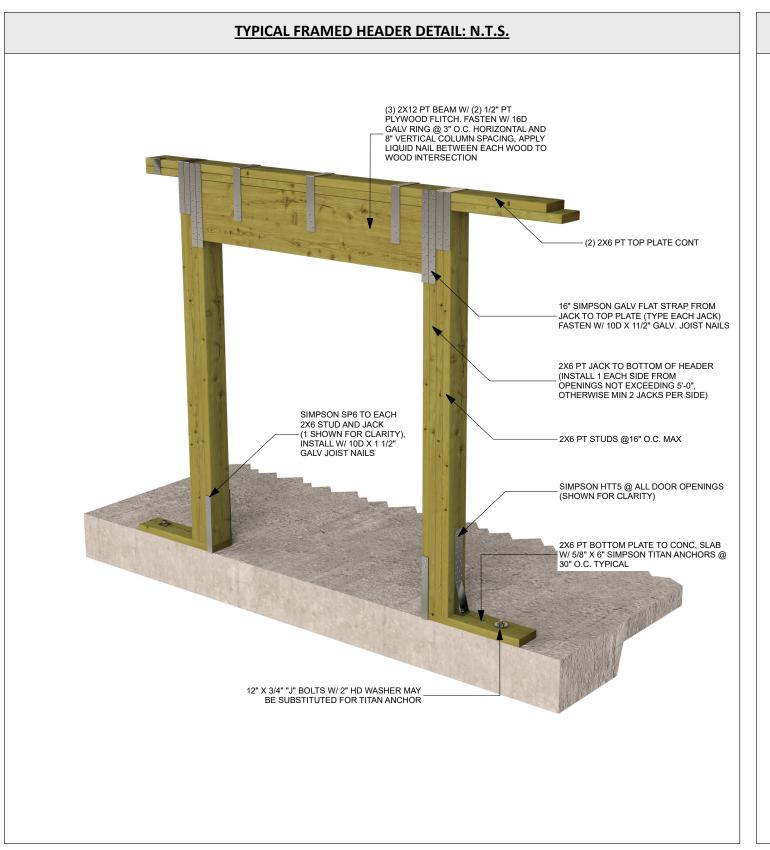


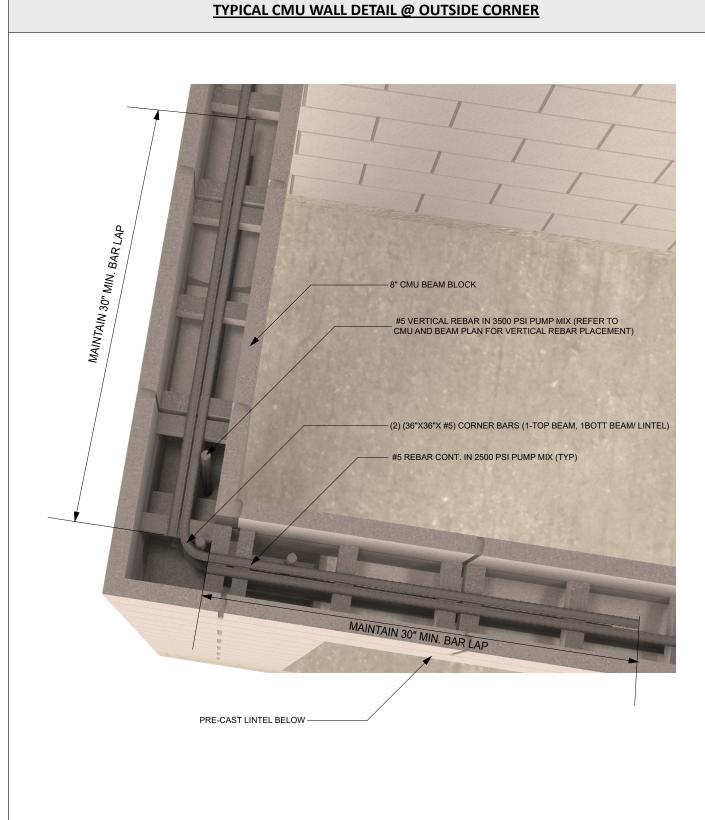
**ENLARGED DETAIL @ CUSTOM CP-07** SCALE: 3/4" = 1'-0"

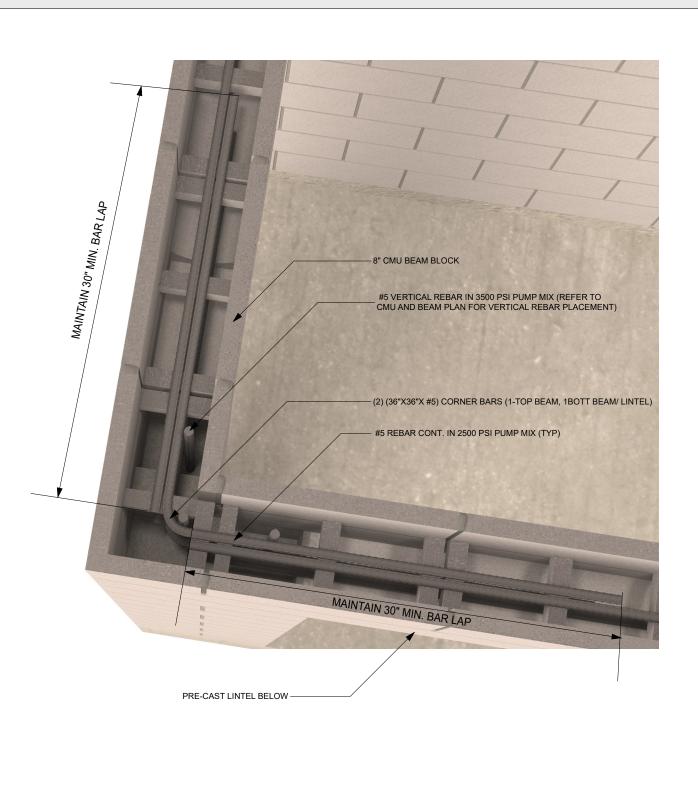
COLUMN PAD DETAILS

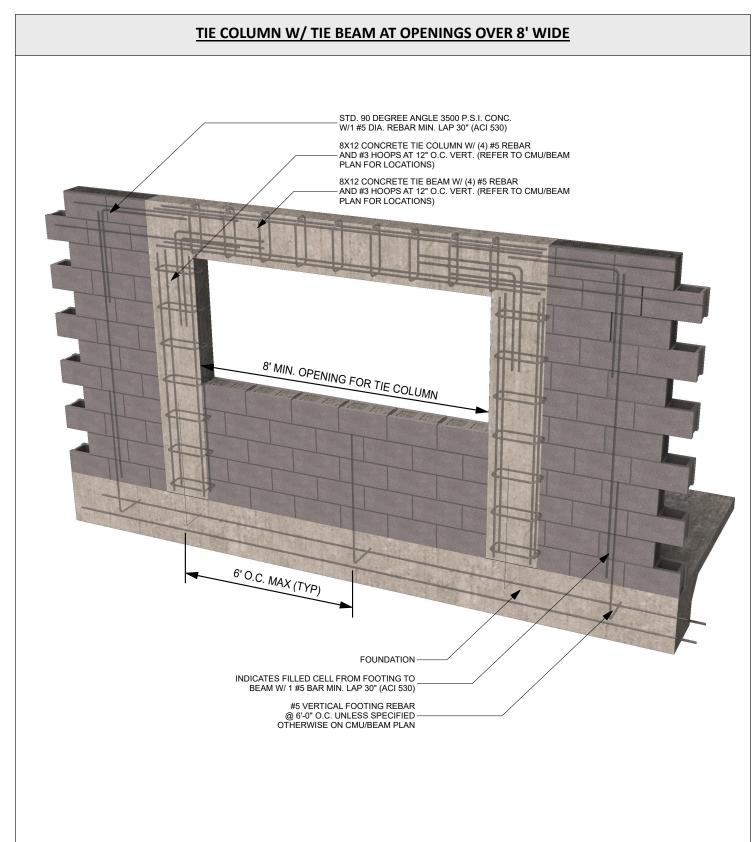


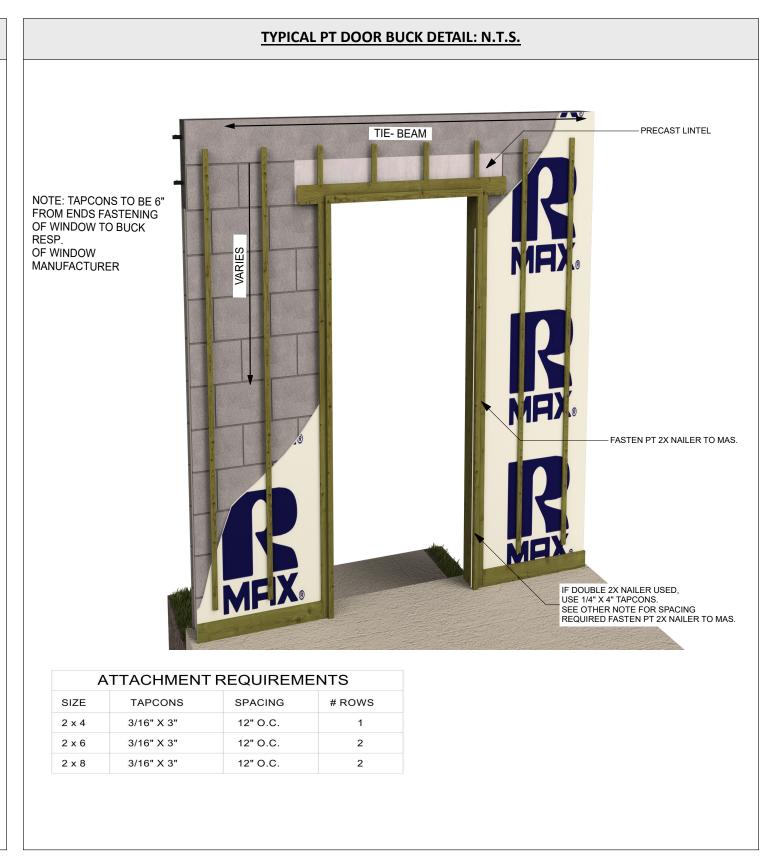


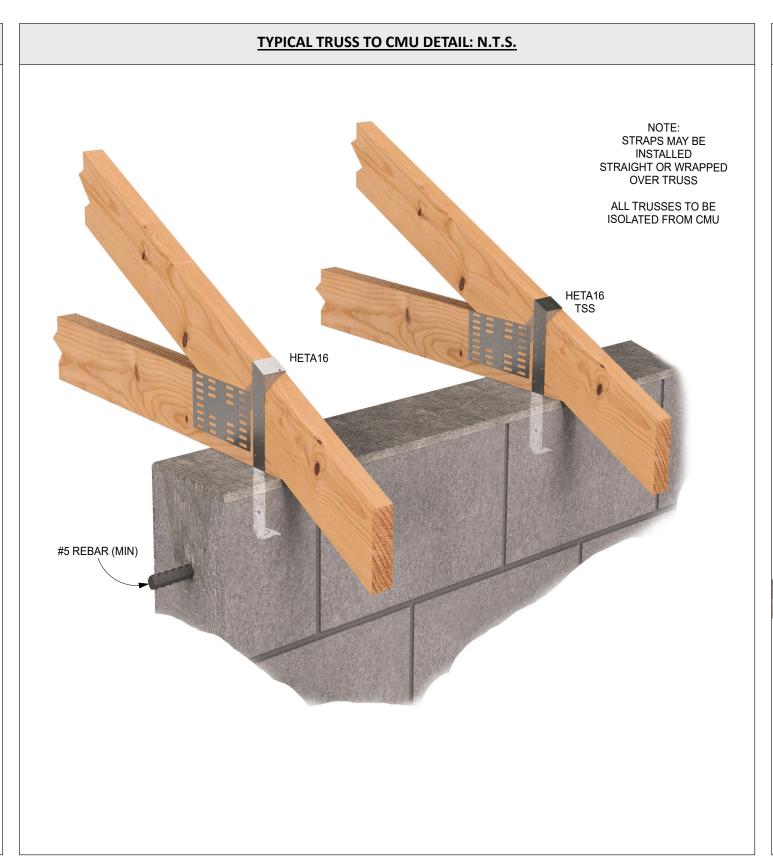


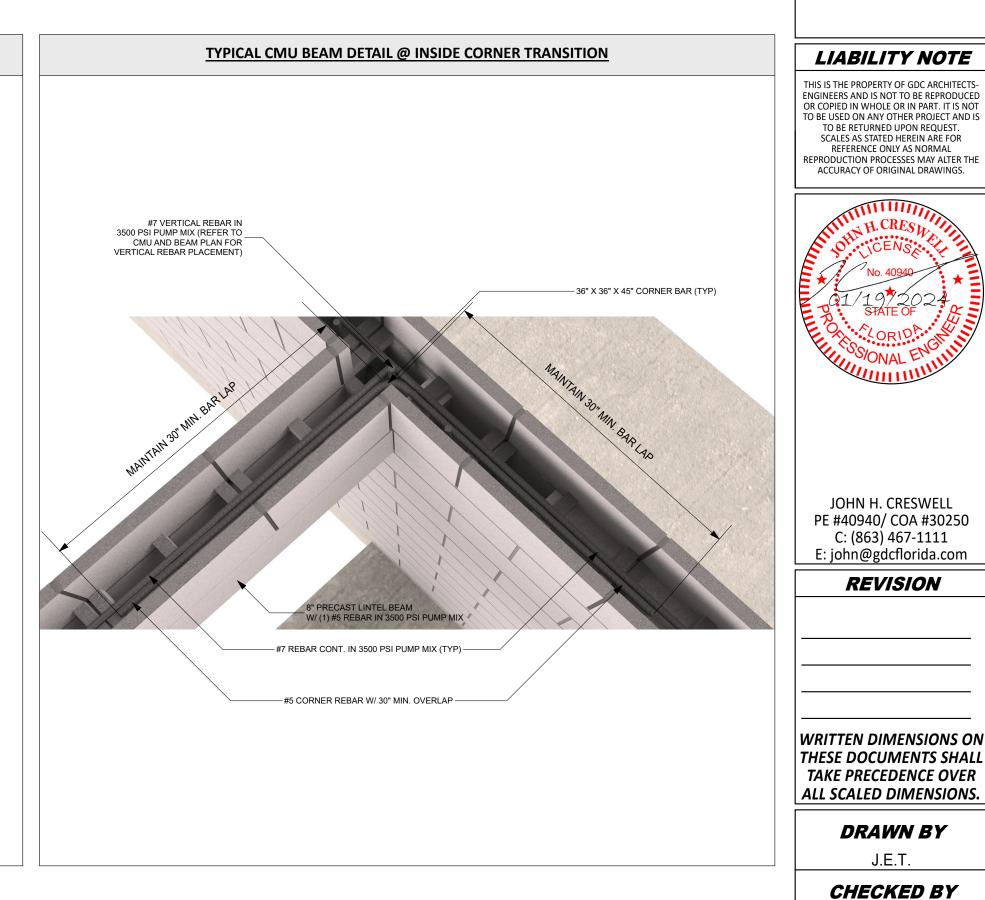












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