IF PLOTTED AT 11" X 17", SCALE IS ONE HALF AS NOTED.

STRUCTURAL NOTES

I. GENERAL NOTES:

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

ALL DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE SEOR BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK. DO NOT SCALE DRAWINGS.

HE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS. ALL RETAINING WALLS TO BE TIED INTO ELEVATED FLOOR FRAMING SHOULD BE ASSUMED TO NEED TO BE BRACED DURING CONSTRUCTION UNLESS SPECIFICALLY NOTED OTHERWISE ON THE PLANS.

ELECTRONIC VERSIONS OF THE STRUCTURAL DRAWINGS ARE THE SOLE, COPYRIGHTED PROPERTY OF SNELL ENGINEERING AND ARE NOT TO BE USED OR TRANSFERRED WITHOUT THE EXPRESS, WRITTEN PERMISSION OF SNELL ENGINEERING.

II. DESIGN LOADS:

THE STRUCTURAL SYSTEM FOR THIS BUILDING HAS BEEN DESIGNED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE – BUILDING 7TH EDITION (2020). THE FOLLOWING SUPERIMPOSED LOADINGS HAVE BEEN UTILIZED:

ROOF:
- LIVE LOAD - 20 PSF.
- LIVE LOAD (CONCENTRATED-FLAT ROOF ONLY) - 300 LBS.
- DEAD LOAD - 20 PSF.

STAIRS:
- LIVE LOAD - 100 PSF.
- LIVE LOAD (CONCENTRATED) - 300 LBS. OVER 2"X2" AREA

EQUIPMENT AND OBSERVATION AREA
- LIVE LOAD - 100 PSF.
- DEAD LOAD - 25 PSF.

FLOORS AND OTHER SIMILAR SURFACES SHALL BE DESIGNED TO SUPPORT THE UNIFORMLY DISTRIBUTED LIVE LOADS OR CONCENTRATED LIVE LOADS SHOWN ABOVE, WHICHEVER PRODUCES THE GREATER LOAD EFFECT. DISTRIBUTED LOADS AND CONCENTRATED LOADS ARE NON-CONCURRENT UNLESS NOTED OTHERWISE.

WIND:
- ASCE 7-16
- ULTIMATE WIND SPEED - 140 MPH
- ALLOWABLE WIND SPEED - 108 MPH
- EXPOSURE C
- ENCLOSED STRUCTURE
- INTERNAL PRESSURE COEFFICIENT - +/- 0.18
- RISK FACTOR II
- SEE WIND SCHEDULE FOR PRESSURES

SEISMIC:
- RISK CATEGORY - II
- SEISMIC IMPORTANCE FACTOR Ie - 1.0
- SITE CLASS D
- SEISMIC DESIGN CATEGORY A
- Ss - 0.047 g   S1 - 0.024 g
- Sds - 0.051 g   Sd1 - 0.038 g

IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO CONFIRM ALL FINAL WEIGHTS OF EQUIPMENT (MECHANICAL, CHILLED WATER, KITCHEN, SPA/POOL, (ETC.), CLADDING, FINISHES, ETC. PRIOR TO ERECTING STRUCTURE SUPPORTING THESE ITEMS. ALL WEIGHTS ACCOUNTED FOR IN THE STRUCTURAL DESIGN ARE LISTED ABOVE OR ON THE PLANS.

III. CONSTRUCTION ADMINISTRATION (SHOP DRAWINGS, RIFS):

SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC.

ANY COMPONENT NOTED AS "DELEGATED" SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF FLORIDA AND NOT BY THE SEOR. SIGNED AND SEALED DRAWINGS SHALL BE PROVIDED TO THE ARCHITECT AND SEOR FOR REVIEW AS A SHOP DRAWING; CALCULATIONS WILL BE PROVIDED IF REQUESTED.

ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/SEOR. DRAWINGS SUBMITTED WITHOUT REVIEW NOTATION WILL BE RETURNED UNCHECKED. EVERY EFFORT WILL BE MADE TO RETURN THE SHOP DRAWINGS WITHIN TEN BUSINESS DAYS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO SUBMIT THE SHOP DRAWINGS ALLOWING FOR AN ADEQUATE REVIEW PERIOD. ALL SHOP DRAWINGS SHALL BE SUBMITTED IN DIGITAL FORMAT.

IN ALL INSTANCES THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS AND ALL OTHER FORMS OF COMMUNICATION, UNLESS OTHERWISE SPECIFIED IN A REQUEST FOR INFORMATION (RFI) BY THE SEOR. EVERY EFFORT WILL BE MADE TO RETURN THE RFIS WITHIN TWO BUSINESS DAYS. SUBMITTALS SHALL CLEARLY IDENTIFY THE SPECIFIC PROJECT, APPLICABLE CODES AND DESIGN CRITERIA, AND DETAILING OF ALL COMPONENTS NECESSARY TO ENSURE PROPER INSTALLATION OF THE COMPONENTS AND SYSTEM.

SHOP DRAWINGS SHOULD BE SUBMITTED FOR ALL COMPONENTS OF THE STRUCTURAL FRAMING SYSTEM, AS REQUIRED BY THE ARCHITECT, AND AS NOTED ELSEWHERE IN THESE NOTES, INCLUDING, BUT NOT LIMITED TO:

a. CONCRETE MIX DESIGNS
b. MASONRY BLOCK
c. MASONRY BLOCK ACCESSORIES
d. MASONRY REINFORCING
e. CONCRETE REINFORCEMENT
f. PRE-ENGINEERED STEEL STAIRS
g. PRECAST CONCRETE COMPONENTS
h. ANY PROPOSED MANUFACTURER CHANGE FROM THE BASIS OF DESIGN

IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE ALL TRADES AND CONSULTANTS, CROSS REFERENCE THEIR DRAWINGS WITH THE OVERALL DESIGN, AND PROVIDE TO EACH A COMPLETE SET OF DRAWINGS AND SUBMITTALS TO ENSURE COMPATIBILITY OF CONSTRUCTION PER DESIGN INTENT.

IV. FOUNDATIONS:

ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2,000 PSF ON COMPACTED FILL OR COMPACTED NATIVE SOIL. BEFORE CONSTRUCTION COMMENCES AND IF REQUIRED BY THE JURISDICTION, SOIL BEARING CAPACITY SHALL BE VERIFIED BY A SUBSURFACE INVESTIGATION, AS WELL AS FIELD AND LABORATORY TESTS PERFORMED BY A CERTIFIED TESTING LABORATORY, WHOSE REPORT SHALL INCLUDE ANALYSIS AND RECOMMENDATIONS FOR SITE PREPARATION IN ORDER TO BEAR THE FOUNDATION LOADS. THE ABOVE REPORT SHALL BE SUBMITTED TO THE SEOR FOR REVIEW BEFORE THE FOUNDATION CONSTRUCTION BEGINS.

V. SOIL COMPACTION:

FOUNDATIONS ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE AS SPECIFIED ABOVE. SOIL COMPACTION SHALL MEET THE MORE STRINGENT OF THE CRITERIA LISTED BELOW OR AS SPECIFIED IN THE GEOTECHNICAL REPORT. REMOVE TOP-SOIL TO A MINIMUM DEPTH OF ONE FOOT OVER THE ENTIRE BUILDING AREA AND FIVE FEET BEYOND THE BUILDING LINES. COMPACT SUB-GRADE USING A VIBRATORY COMPACTER SUCH AS "VIBRATOW II" OR EQUIVALENT WITH A MINIMUM OF FOUR PASSES. PLACE AND COMPACT FILL MATERIAL TO FINISHED GRADE LEVEL IN LIFTS NOT EXCEEDING 12" THICK. SUB-GRADE AND EACH LIFT SHALL BE COMPACTED TO MINIMUM 95% MODIFIED PROCTOR DENSITY DETERMINED IN ACCORDANCE WITH ASTM. D-1557. VERIFICATION THAT THE COMPACTION REQUIREMENTS HAVE BEEN MET SHALL BE MADE BY AN INDEPENDENT GEOTECHNICAL CONSULTANT EMPLOYED BY THE OWNER AND APPROVED BY THE SEOR. LOCATIONS FAILING TO MEET THE REQUIREMENTS SHALL BE RECOMPACTED AND RETESTED AT THE CONTRACTORS' EXPENSE AND AS DIRECTED BY THE INDEPENDENT GEOTECHNICAL CONSULTANT.

VI. FORMWORK AND SHORING:

NO STRUCTURAL CONCRETE SHALL BE STRIPPED UNTIL IT HAS REACHED AT LEAST TWO-THIRDS OF THE 28 DAY DESIGN STRENGTH. DESIGN, ERECTION AND REMOVAL OF ALL FORMWORK, SHORES AND RESHORES SHALL MEET THE REQUIREMENTS SET FORTH IN ACI STANDARDS 347 AND 301.

VII. PENETRATIONS:

NO PENETRATIONS SHALL BE MADE IN ANY STRUCTURAL MEMBERS WITHOUT PREVIOUS APPROVAL OF THE EOR, EXCEPT THOSE PENETRATIONS SHOWN ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHOULD SUBMIT DRAWINGS SHOWING: ANY LOCATION WHERE OPENINGS, PENETRATIONS, OR ANY PLACE MORE THAN 3 PIPES OR CONDUITS ARE LOCATED IN A SLAB SYSTEM; ANY LOCATION IN A BEAM OR COLUMN WHERE MORE THAN 4% OF THE MEMBER SECTION IS REMOVED IN ANY ORIENTATION.

VIII. PLUMBING SLEEVES:

MINIMUM SLEEVE SPACING SHALL BE THREE DIAMETERS CENTER TO CENTER OF THE LARGER SLEEVE OR 6" CLEAR BETWEEN SLEEVES, WHICHEVER IS GREATER. PRIOR TO CONSTRUCTION SLEEVE LOCATIONS AND SIZES SHALL BE APPROVED BY THE SEOR. CONDUITS, PIPES AND SLEEVES SHALL BE PLACED AND SPACED IN ACCORDANCE WITH ACI 318 6.3.

IX. REINFORCING STEEL:

SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. SECURE APPROVAL OF SHOP DRAWINGS PRIOR TO COMMENCING FABRICATION.

REINFORCING BARS SHOWN IN SECTIONS DEPICT TYPICAL CONFIGURATION AND ARE NOT SPECIFIC TO THE CONCRETE MEMBER CUT ON PLAN; SEE PLAN AND SCHEDULES FOR ALL BAR SIZE AND QUANTITIES. TAKE-OFFS AND QUANTITIES SHALL BE OBTAINED FROM THE SCHEDULES AND PLANS, NOT FROM SECTIONS. WHERE HOOKS, LAP LENGTHS, ETC. ARE SHOWN IN SECTIONS, ALL LENGTHS AND DETAILS SHALL MEET ACI REQUIREMENTS FOR REINFORCING DETAILS. HOOKS SHALL BE ORIENTED AS REQUIRED TO FIT WITHIN THE CONCRETE MEMBER.

X. WELDED WIRE FABRIC:

A. TO CONFORM TO ASTM A-185, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. THE MINIMUM LAP SHALL BE ONE SPACE PLUS TWO INCHES.

XI. CONCRETE:

ALL CONCRETE SHALL MEET ACI 318 'BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE' AND ACI 301 'SPECIFICATIONS FOR STRUCTURAL CONCRETE', INCLUDING, BUT NOT LIMITED TO, MIX DESIGN, STRENGTH, COVER, PLACEMENT, CURING, TESTING, FORMS, FLATNESS, ETC. SEE THE ARCHITECTURAL SPECIFICATIONS AND PLANS FOR ANY ADDITIONAL REQUIREMENTS.

ALL CONCRETE SHALL BE AN APPROVED MIX DESIGN PROPORTIONED TO ACHIEVE A STRENGTH AT 28 DAYS AS LISTED BELOW WITH A PLASTIC AND WORKABLE MIX:

3000 PSI FOR FOUNDATIONS AND SLABS ON GRADE.
4000 PSI FOR ALL OTHER STRUCTURAL CONCRETE

SUBMIT PROPOSED MIX DESIGN FOR EACH PORTION OF WORK PRIOR TO USE. ALL MIX DESIGNS SHALL BE ACCOMPANIED BY A MINIMUM OF 15 FIELD STRENGTH TEST RECORDS, AS NOTED IN ACI 301 4.2.3.2(A); NO OTHER DOCUMENTATION OF AVERAGE COMPRESSIVE STRENGTH WILL BE ACCEPTED WITHOUT PRIOR WRITTEN APPROVAL BY THE SEOR. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. MIX SHALL MEET THE REQUIREMENTS OF ASTM C33 FOR COARSE AGGREGATE. FOR ALL FLATWORK, AT LEAST 75% OF LARGE AGGREGATE SHALL CONSIST OF #57 STONE. CONCRETE SHALL COMPLY WITH ALL THE REQUIREMENTS OF ASTM STANDARD C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME STAMPED WHEN CONCRETE IS BATCHED.

ALL CONCRETE MIX DESIGNS SHALL INCLUDE A WRITTEN DESCRIPTION INDICATING WHERE EACH PARTICULAR MIX IS TO BE PLACED WITHIN THE STRUCTURE.

CONCRETE SHALL BE PLACED AND CURED ACCORDING TO ALL STANDARDS AND SPECIFICATIONS, INCLUDING ALL ACI REQUIREMENTS.

THE MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE HALF (1-1/2) HOURS. IF FOR ANY REASON THERE IS A LONGER DELAY THAN THAT STATED ABOVE, THE CONCRETE SHALL BE DISCARDED. IT SHALL BE THE RESPONSIBILITY OF THE TESTING LAB TO NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NONCOMPLIANCE WITH THE ABOVE. ALL SLABS SHALL BE CURED USING A DISSIPATING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE 1-D AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE. ALL SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAILY. CALCIUM CHLORIDES SHALL NOT BE UTILIZED; OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE SEOR.

THE GENERAL CONTRACTOR SHALL NOTIFY THE SEOR A MINIMUM OF 48 HOURS PRIOR PLACEMENT OF ANY STRUCTURAL CONCRETE.

SEE PLAN NOTES FOR SPECIFIC COVER REQUIREMENTS. UNLESS NOTED OTHERWISE ON PLANS, THE FOLLOWING CONCRETE CLEAR COVER SHALL BE PROVIDED FOR ALL NON-PRESTRESSED CONCRETE REINFORCEMENT PER ACI 318:

CONCRETE CAST AGAINST EARTH:	ALL BARS	-	3"
CONCRETE EXPOSED TO EARTH (FORMED FACE):	ALL BARS	-	2"
CONCRETE EXPOSED TO WEATHER:	#6 BARS AND GREATER	-	2"
	#5 BARS AND SMALLER	-	1 1/2"
WHERE NOT EXPOSED TO EARTH OR WEATHER:			
SLABS, WALLS, AND JOISTS:	#14 & #18 BARS	-	1 1/2"
	#11 BARS AND SMALLER	-	1"
BEAMS AND COLUMNS:	ALL BARS	-	1 1/2"

CONCRETE TESTING: AN INDEPENDENT TESTING LABORATORY SHALL PERFORM THE FOLLOWING TESTS ON CAST IN PLACE CONCRETE:

A) ASTM C143 - "STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE." THE MAXIMUM SLUMP SHALL BE 4-6 INCHES, PRIOR TO ADDING A SUPER PLASTICIZER.

B) ASTM C39 - "STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS." A SEPARATE TEST SHALL BE CONDUCTED FOR EACH CLASS, FOR EVERY 50 CUBIC YARDS (OR FRACTION THEREOF), PLACED PER DAY. REQUIRED CYLINDER(S) QUANTITIES AND TEST AGE AS FOLLOWS:
1 AT 7 DAYS
2 AT 28 DAYS

ONE ADDITIONAL RESERVE CYLINDER TO BE TESTED UNDER THE DIRECTION OF THE SEOR, IF REQUIRED. IF 28 DAY STRENGTH IS ACHIEVED, THE ADDITIONAL CYLINDER(S) MAY BE DISCARDED. UPON RECEIPT, IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT ALL COMPRESSIVE TEST RESULTS BE SENT TO THE SEOR, ARCHITECT OF RECORD, CONCRETE SUBCONTRACTOR, AND CONCRETE SUPPLIER.

POUR STRUCTURAL CONCRETE WITHIN THE FOLLOWING TOLERANCES:

VARIATION FROM PLUMB:	1/4" IN 10'-0"
VARIATION FROM LEVEL IN TOPS OF PILASTERS:	1/8" IN 10'-0"
VARIATION FOOTINGS:	
PLAN DIMENSIONS:	+2", - 1/2"
THICKNESS:	- 0"

XII. NON-SHRINK GROUT:

NON-SHRINK GROUT SHALL BE A HIGH-STRENGTH MORTAR OR GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI AT 28 DAYS. THE GROUT IS TO BE NON-METALLIC, NON-CORROSIVE, CEMENT-BASED AND SHALL CONFORM TO THE REQUIREMENTS OF ASTM C1107. IT SHALL BOND PERMANENTLY TO A CLEAN METAL BASEPLATE AND CONCRETE SUBSTRATE AND WILL NOT SHRINK IN ITS PLASTIC STATE, AS TESTED IN ACCORDANCE WITH ASTM C827.

XIII. HOLLOW-CORE SLABS:

FLOORS SHALL BE PRESTRESSED CONCRETE HOLLOW-CORE SLABS DESIGNED IN ACCORDANCE WITH ACI 318-14 FOR THE DESIGN LOADS LISTED ABOVE.

SHOP DRAWINGS AND CALCULATIONS SHALL BE PREPARED FOR ALL WORK AND SUBMITTED FOR REVIEW. SHOP DRAWINGS SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER.

NO ALTERNATE PRODUCT SUBSTITUTION FOR HOLLOW-CORE SLABS WILL BE ACCEPTED.

XIV. CHEMICAL ANCHORS:

SHALL BE AN EQUAL TWO-PART EPOXY POLYMER INJECTION SYSTEM, SUCH AS SIMPSON SET-XP "STRUCTURAL ANCHORING ADHESIVE," HILTI HIT-HY 200, OR SEOR APPROVED SUBSTITUTION, INSTALLED IN ACCORDANCE WITH MANUFACTURERS INSTRUCTIONS. INSTALLERS SHALL BE TRAINED BY THE MANUFACTURER'S REPRESENTATIVE. BRUSH AND BLOW OUT ALL HOLES.

XV. MASONRY WALLS:

MASONRY UNITS SHALL MEET ASTM C-90 FOR HOLLOW LOAD BEARING TYPE MASONRY WITH UNIT STRENGTH OF 2000 PSI ON THE NET AREA (F'M = 2000 PSI). MORTAR SHALL BE TYPE "M" OR "S" AND MEET ASTM C-270. GROUT SHALL BE 3000 PSI MINIMUM COMPRESSIVE STRENGTH AND MEET ASTM C-476. PROVIDE HOOKED DOWELS IN FOOTINGS FOR ALL VERTICAL REINFORCING ABOVE. LAP SPLICES 48 BAR DIAMETERS.

MASONRY CONSTRUCTION SHALL BE LAID IN RUNNING BOND CONFIGURATION UNLESS OTHERWISE NOTED. AT ALL WALL INTERSECTIONS, AT LEAST FIFTY PERCENT OF THE MASONRY UNITS AT THE INTERFACE SHALL INTERLOCK.

BLOCK CELLS AS SHOWN ON PLANS SHALL BE GROUT FILLED WITH VERTICAL REINFORCING BARS. SEE PLAN NOTES FOR BAR SIZE AND SPACING. DOWELS SHALL BE USED TO PROVIDE CONTINUITY INTO THE STRUCTURE ABOVE AND/OR BELOW, UNLESS NOTED OTHERWISE. USE METAL LATH, MORTAR, OR SPECIAL UNITS TO CONFINE CONCRETE AND GROUT TO AREA REQUIRED. CELLS TO BE GROUT FILLED SHALL HAVE VERTICAL ALIGNMENT SUFFICIENT TO MAINTAIN A CLEAR, UNOBSTRUCTED, CONTINUOUS VERTICAL GROUT SPACE.

PROVIDE 9 GAUGE GALVANIZED HORIZONTAL JOINT REINFORCING (DUR-O-WALL OR SEOR APPROVED SUBSTITUTION) AT ALTERNATE BLOCK COURSES, BEGINNING 8" ABOVE FOOTINGS AND FLOOR LEVELS. MASONRY WALLS ABOVE OPENINGS SHALL BE REINFORCED AT THE SAME SPACING AS THE WALL WITH DOWELS HOOKED INTO THE BEAM OR LINTEL ABOVE THE OPENING.

GROUT LIFT: AN INCREMENT OF GROUT HEIGHT WITHIN A TOTAL GROUT POUR.
GROUT POUR: THE TOTAL HEIGHT OF MASONRY TO BE GROUTED PRIOR TO ERECTION OF ADDITIONAL MASONRY. A GROUT POUR CONSISTS OF ONE OR
MORE GROUT LIFTS. GROUT POURS SHALL SET FOR A MINIMUM OF 4
HOURS BEFORE ANY ADDITIONAL GROUT PLACEMENT.

GROUT SHALL HAVE A SLUMP BETWEEN 8 AND 11 INCHES, EXCEPT SELF-CONSOLIDATING GROUT. JOB-SITE PROPORTIONING OF SELF-CONSOLIDATING GROUT IS NOT PERMITTED.

MASONRY GROUTING REQUIREMENTS:

A. FIELD-MIXED GROUT SHALL BE PLACED WITHIN 1-1/2 HOURS FROM INTRODUCING WATER INTO THE MIXTURE AND BEFORE THE INITIAL SET.
B. GROUT SLUMP REQUIREMENTS:
   A. FOR GROUT SLUMP BETWEEN 8 AND 10 INCHES, THE MAXIMUM GROUT LIFT HEIGHT IS 5 FEET.
   B. FOR GROUT SLUMP BETWEEN 10 AND 11 INCHES, THE MAXIMUM GROUT LIFT HEIGHT IS 12.67 FEET.
   C. FOR SELF-CONSOLIDATING GROUT, THE GROUT LIFT HEIGHT SHALL NOT EXCEED THE GROUT POUR HEIGHT (24 FEET MAX.).
C. GROUT LIFT HEIGHTS EXCEEDING 5 FEET SHALL MEET THE FOLLOWING REQUIREMENTS:
   A. MASONRY MORTAR HAS CURED FOR AT LEAST 4 HOURS.
   B. GROUT SLUMP IS BETWEEN 10 AND 11 INCHES.
   C. NO INTERMEDIATE BOND BEAMS ARE PLACED BETWEEN THE TOP AND BOTTOM OF THE GROUT LIFT HEIGHT.
D. EACH GROUT LIFT SHALL BE CONSOLIDATED BY MECHANICAL VIBRATION AT THE TIME OF PLACEMENT. CONSOLIDATION IS NOT REQUIRED FOR SELF-CONSOLIDATING GROUT.
E. EACH GROUT LIFT SHALL BE RECONSOLIDATED BY MECHANICAL VIBRATION AFTER INITIAL WATER LOSS AND SETTLEMENT HAS OCCURRED, AND BEFORE ADDING THE SUBSEQUENT GROUT LIFT. RECONSOLIDATION IS NOT REQUIRED FOR SELF-CONSOLIDATING GROUT.
F. THE TIME BETWEEN PLACING GROUT LIFTS SHALL NOT EXCEED 1 HOUR.
G. THE MAXIMUM POUR HEIGHT IS 24 FEET.
H. A GROUT KEY SHALL BE PROVIDED AT THE TOP OF EACH GROUT LIFT AND GROUT POUR. GROUT KEYS SHOULD BE FORMED BY TERMINATING THE GROUT 1-1/2 INCHES BELOW A MORTAR JOINT.

MASONRY WALLS MARKED AS "LOAD-BEARING" ARE DESIGNED TO CARRY GRAVITY FLOOR LOADS AND MUST BE CONSTRUCTED TO SUPPORT THE FLOOR SYSTEM, ON CURRENTLY WITH ALL OTHER LOAD-BEARING CONCRETE AND STEEL COLUMNS.

MASONRY WALLS MARKED AS "INFILL" ARE NOT DESIGNED TO CARRY GRAVITY FLOOR LOADS AND MUST BE CONSTRUCTED AFTER THE FLOOR SYSTEM, WITH ALL LOAD-BEARING COMPONENTS, HAVE BEEN INSTALLED AND THE FLOOR SYSTEM UNSHORED.

AT SILLS OF MASONRY OPENINGS IN LOAD-BEARING MASONRY WALLS, PROVIDE AN 8" KNOCKOUT COURSE, GROUTED SOLID AND REINFORCED WITH 1 #5 CONTINUOUS HORIZONTAL BAR, TYPICAL UNLESS NOTED OTHERWISE ON PLAN OR DETAILS.

ALL WALLS EXCEEDING 16 FEET TALL SHALL HAVE A MID-HEIGHT DOUBLE-KNOCK OUT BLOCK, GROUTED SOLID WITH (2)#5 BARS CONT. IN EACH COURSE, UNLESS A CONCRETE TIE-BEAM OR BOND BEAM IS NOTED.

XVI: TIE BEAMS:

BEAMS WITH THE PREFIX "TB" SHALL BE OF CONCRETE POURED AFTER THE MASONRY WALLS BELOW ARE IN PLACE AND ARE INTENDED TO BE FULLY SUPPORTED BY THE MASONRY WALL. REINFORCING SHALL BE CONTINUOUS THROUGH TIE BEAMS FOR THE FULL LENGTH OF THE MASONRY WALL WITH MINIMUM LAP SPLICES OF 48 BAR DIAMETERS AND BENT BARS AT CORNERS. USE METAL LATH, MORTAR, OR SPECIAL UNITS TO CONFINE CONCRETE TO AREA REQUIRED, IN ACCORDANCE WITH ACI 530.1, SECTION 4.3.3.3 (SOLID METAL OR FELT CAVITY CAPS ARE PROHIBITED). WHERE TIE BEAMS ARE NOTED AS KNOCK-OUT (K.O.) BLOCKS, KNOCK-OUT BLOCKS SHALL BE GROUT-FILLED AS SPECIFIED IN THE MASONRY SECTION ABOVE.

UNLESS NOTED ON PLAN, DETAILED, OR SCHEDULED, AN 8"X16" CONCRETE TIE BEAM SHALL BE PROVIDED AT THE TOP OF ALL LOAD-BEARING MASONRY WALLS. THE TOP OF TIE BEAM SHALL BE AT THE TOP OF WALL ELEVATION AND REINFORCED WITH (2)#5 BARS TOP AND BOTTOM, WITH #3 STIRRUPS AT 24" O.C. MAX. WHERE A SLAB WITH A THICKNESS OF 10" OR GREATER BEARS ON THE MASONRY WALL, THE TIE BEAM MAY BE OMITTED, PROVIDED THE VERTICAL MASONRY REINFORCING IS HOOKED INTO THE SLAB.

XVII: MASONRY LINTELS/WALL OPENINGS:

ALL OPENINGS IN MASONRY WALLS SHALL BE SPANNED WITH A CONCRETE BEAM OR PRECAST CONCRETE LINTEL. THE BEAM OR LINTEL SHALL SPAN THE FULL WIDTH OF THE OPENING AS A CONTINUOUS WIDTH AND DEPTH UNLESS NOTED OTHERWISE ON THE PLANS.

SEE PLANS AND SCHEDULE FOR LINTEL SIZE AND REINFORCING.

XVIII: STEEL STAIRS:

ENGINEERED STEEL STAIR SYSTEM AND ALL CONNECTIONS OF THE SAME TO THIS STRUCTURE SHALL BE DESIGNED BY AN ENGINEER REGISTERED IN THE STATE OF FLORIDA. SUBMIT SHOP DRAWINGS BEARING THE EMBOSSED SEAL AND THE SIGNATURE OF THE DELEGATED ENGINEER FOR REVIEW PRIOR TO FABRICATION. THE CONFIGURATION OF THE STEEL STAIR SYSTEM SHALL BE AS SHOWN ON THE ARCHITECTURAL DRAWINGS. STEEL STAIR SYSTEM AND ALL CONNECTIONS SHALL BE DESIGNED FOR ALL APPLICABLE LOADS AS INDICATED ON THE PLANS AND IN THE BUILDING CODE. THE LOADS SHALL BE CLEARLY INDICATED ON ALL SHOP DRAWINGS. SHOP DRAWINGS SHALL SHOW AND SPECIFY ALL CONNECTIONS UTILIZED WITHIN THE STEEL STAIR SYSTEM AS WELL AS CONNECTIONS TO AND LOADS IMPOSED UPON THE STRUCTURAL SYSTEM SHOWN ON THESE PLANS.

MINIMUM STAIR DESIGN LOADS:
- UNIFORM LIVE LOAD = 100 PSF
- CONCENTRATED LIVE LOAD = 300 LBS

SHOP DRAWINGS SHALL SHOW AND SPECIFY ALL CONNECTIONS (INCLUDING THE HANDRAIL) UTILIZED WITHIN THE STEEL STAIR SYSTEM AS WELL AS CONNECTIONS TO AND LOADS IMPOSED UPON THE STRUCTURAL SYSTEM SHOWN ON THESE PLANS. FAILURE TO PROVIDE ALL CONNECTIONS OF THE STEEL STAIR SYSTEM TO THE STRUCTURE SHALL BE CAUSE TO REJECT THE ENTIRE STAIR SUBMITTAL AND INSTALLATION.

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Issue Dates

Revisions

STRUCTURAL NOTES

Scale : 1/4" = 1'-0"

Plot Date
2/13/2024 5:25:00 PM

Project No.
21049

Drawn By
DC / CT

S-1.00

STATE OF FLORIDA
THOMAS R WALTER
7305
REGISTERED ARCHITECT

[illegible]

Date of Issue
07/21/2023

IF PLOTTED AT 11" X 17", SCALE IS ONE HALF AS NOTED.

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IF PLOTTED AT 11" X 17", SCALE IS ONE HALF AS NOTED.

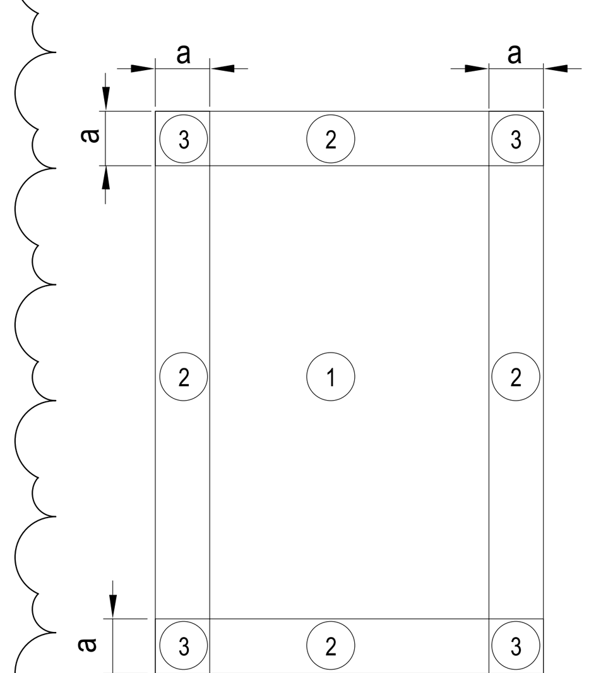
WIND LOAD SCHEDULE

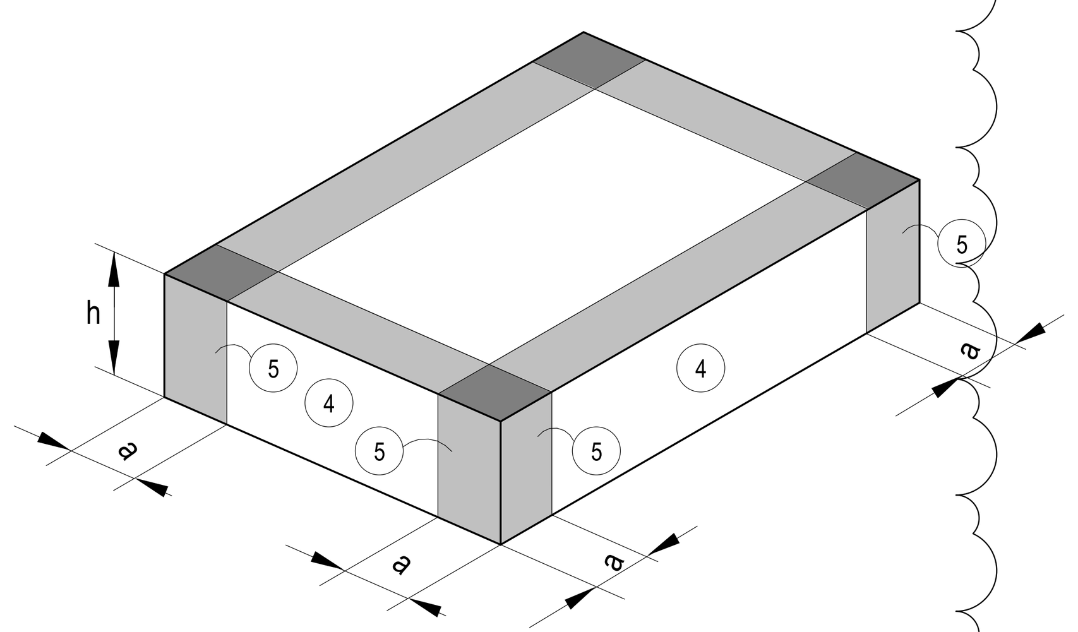
SCHEDULE OF COMPONENTS AND CLADDING LOADS

ZONE	ZONE DESCRIPTION	TRIBUTARY AREA (SF)	IN (PRESSURE) (+ PSF)	OUT (PRESSURE) (- PSF)
1	ROOF INTERIOR ZONE	LESS THAN 20	10.0	22.9
		20 - 100	10.0	22.9
		MORE THAN 100	10.0	22.9
2	ROOF EDGE ZONE	LESS THAN 20	10.0	26.5
		20 - 100	10.0	26.0
		MORE THAN 100	10.0	24.7
3	ROOF CORNER ZONE	LESS THAN 20	10.0	35.5
		20 - 100	10.0	32.4
		MORE THAN 100	10.0	24.7
4	WALL INTERIOR ZONE	LESS THAN 20	21.1	22.9
		20 - 50	20.2	22.0
		20 - 100	19.0	20.8
		MORE THAN 100	17.9	20.2
5	WALL EDGE ZONE	LESS THAN 20	21.1	28.3
		20 - 50	20.2	26.5
		20 - 100	19.0	23.8
		MORE THAN 100	17.9	22.0

NOTE: WIND PRESSURES SHOWN ARE BASED ON Vasd

CODE =	ASCE 7-16
ULTIMATE WIND SPEED Vult =	140 MPH
ALLOWABLE WIND SPEED Vasd =	108 MPH
RISK CATEGORY =	II
EXPOSURE =	B
ENCLOSURE CLASSIFICATION =	ENCLOSED
INTERNAL PRESSURE COEFFICIENT (GCpi) =	±0.18
a =	10.0 FT
2a =	20.0 FT





FLAT ROOF (ϴ < 1.5°)

SCALE: N.T.S.

⬜ Interior Zones	▨ End Zones	■ Corner Zones
ZONE 1 - ROOF ZONE 4 - WALL	ZONE 2 - ROOF ZONE 5 - WALL	ZONE 3 - ROOF

1

MASONRY WALL REINFORCING NOTES:

1. USE (1) #5 VERT BAR IN FILLED CELLS @ 32" O.C.
2. USE (1) #5 VERT BAR AT EACH SIDE OF OPENING UP TO 6'-0", (2) #5 VERT BAR AT EACH SIDE OF OPENING GREATER THAN 6'-0". S PER DETAIL 6/S-4.00.
3. USE (3) #5 VERT BARS @ CORNERS PER DETAIL 6/S-4.00.
4. AT THE CONTROL JOINTS IN MASONRY, REFER TO DETAIL 5/S-4.00. SEE STRUCTURAL DWGS FOR LOCATIONS.
5. FOR HORIZONTAL REINFORCING, USE 9 GA LADDER TYPE DUR-O-WALL @ 16" O.C. VERTICALLY.
6. FOR BOND BEAM CORNER REINFORCING, SEE DETAIL 4/S-4.00.
7. FOR WALLS WITH CLEAR HEIGHT GREATER THAN 16'-0", PROVIDE (2) #5 VERT BARS IN FILLED CELLS AT 16" O.C. PROVIDE (2) 8" K.O. BLOCKS WITH (1) #5 BAR IN EACH COURSE PER STRUCTURAL NOTES.

FOUNDATION PLAN NOTES:

1. SLAB ON GRADE SHALL BE A MINIMUM OF 6" CONCRETE SLAB ON GRADE REINFORCED WITH ONE LAYER OF 6x6 - W2.9 x W2.9 WELDED WIRE FABRIC IN UPPER HALF OF SLAB (UNLESS INDICATED OTHERWISE). REFER TO STRUCTURAL NOTES FOR SOIL COMPACTION REQUIREMENTS. REINFORCING "CHAIRS" ARE REQUIRED PER FLORIDA BUILDING CODE.
2. THE CONCRETE SLAB SHALL BE CAST ON A VAPOR BARRIER (6 MIL MIN. THICKNESS) ON WELL COMPACTED, CLEAN, TERMITE TREATED FILL. SLAB CURING IS REQUIRED.
3. ELEVATIONS SHOWN ON THESE PLANS REFER TO 0'-0" REFERENCE ELEVATION (TOP OF GROUND FLOOR SLAB).
4. REFER TO SHEET S-1.00 FOR STRUCTURAL NOTES.
5. VERIFY ALL DIMENSIONS, ELEVATIONS, SLAB STEPS AND SLAB FINISHES WITH BUILDING DRAWINGS PRIOR TO COMMENCING CONSTRUCTION. FOR ADDITIONAL DIMENSIONAL INFORMATION NOT GIVEN HERE, REFER TO ARCH. DRAWINGS.
6. SLAB ON GRADE CONTRACTION JOINTS (CJ) SHALL BE TOOLED OR SAWCUT WITHIN 6 HOURS OF POURING. SEE DETAIL 1/S-4.00. JOINT PATTERN SHALL BE 8 - 12 FEET O.C. MAX EACH WAY.
7. SIDE FORMS SHALL BE USED FOR ALL FOOTING UNLESS IT CAN BE DEMONSTRATED THAT SOILS ARE STABLE ENOUGH TO MAINTAIN VERTICAL "FORM-CUT" DURING CONSTRUCTION. INSPECTOR TO VERIFY AND ADVISE.
8. PROVIDE 8" W x 12" D THICKENED CONCRETE EDGE REINFORCED WITH (1) #5 BAR CONTINUOUS AT ALL DOOR OPENINGS.
9. COORDINATE FINAL GRADE ELEVATIONS WITH TOP / FOOTING ELEVATIONS AND CIVIL PLANS PRIOR TO COMMENCING CONSTRUCTION. VERIFY 6" MINIMUM FROM TOP OF FOOTING TO FINISHED GRADE.
10. SEE DETAIL 1/S-4.00 FOR CONSTRUCTION JOINT DETAILS (SLAB COLD JOINTS, IF NEEDED).

BUILDING & ACCESSORY STRUCTURES FOOTING SCHEDULE

MARK	SIZE			REINFORCING			REMARKS
	LENGTH	WIDTH	DEPTH	TOP	CENTER	BOTTOM	
TE10	CONT.	1'-0"	1'-0"	-	-	(1) #5 BARS CONT.	THICKENED SLAB EDGE
SF20	CONT.	2'-0"	1'-0"	-	-	(2) #5 BARS CONT.	
SF25	CONT.	2'-6"	1'-0"	-	-	(3) #5 BARS CONT.	
SF30	CONT.	3'-0"	1'-0"	-	-	(3) #5 BARS CONT.	
FP10	10'-0"	4'-0"	2'-0"	-	-	#5 BARS AT 12" O.C. EACH WAY	PYLON SIGN
FP20	2'-0"	2'-0"	1'-0"	-	-	(2) #5 BARS CONT. EACH WAY	COLUMN AT CONTROL ROOM
FP40	3'-6"	3'-6"	1'-0"	-	-	(4) #5 BARS CONT. EACH WAY	COLUMN AT TUNNEL ENTRY

6

NOTES:

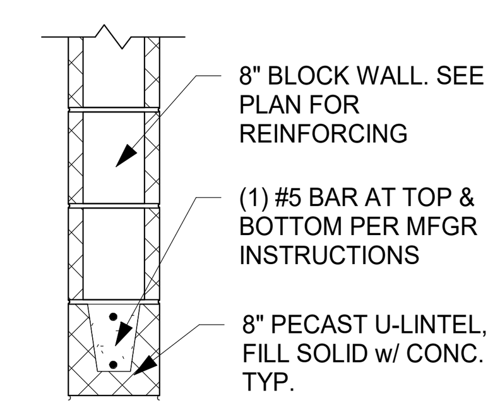
1. THE BOTTOM OF ALL FOOTINGS SHALL BE (1'-0") MIN. BELOW FINISHED EARTH GRADE.
2. TOP OF ALL INTERIOR FOOTINGS SHALL BE (8") MIN. BELOW FINISHED FLOOR UNLESS NOTED OTHERWISE.
3. REFER TO STRUCTURAL FOUNDATION SITE PLAN FOR FOOTING SIZES FOR CANOPIES, PYLON SIGN & SITE SIGNAGE.

MASONRY LINTEL SCHEDULE

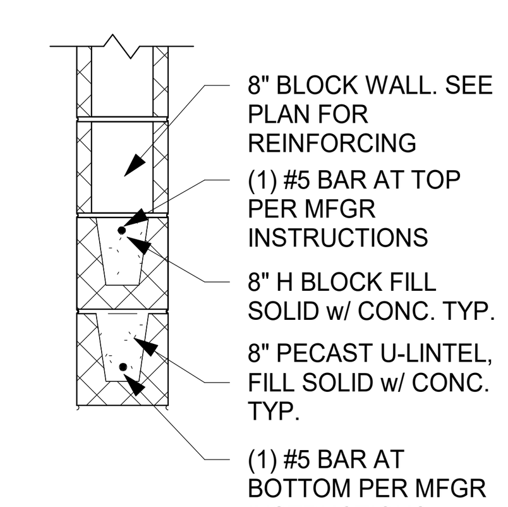
MARK	LOCATION	TYPE	REMARKS
L-1	DOOR & WINDOWS	8 - 8"	8" DEEP FILLED IN MASONRY LINTEL w/ #5 TOP & BOTTOM REINFORCING
L-2	DOOR & WINDOWS	8 - 16"	16" DEEP FILLED IN MASONRY LINTEL w/ #5 TOP & BOTTOM REINFORCING
L-3	DOOR & WINDOWS	8 - 24"	24" DEEP FILLED IN MASONRY LINTEL w/ #5 TOP & BOTTOM REINFORCING
L-4	DOOR & WINDOWS	8 - 48"	48" DEEP FILLED IN MASONRY LINTEL w/ #5 TOP & BOTTOM REINFORCING

NOTES:

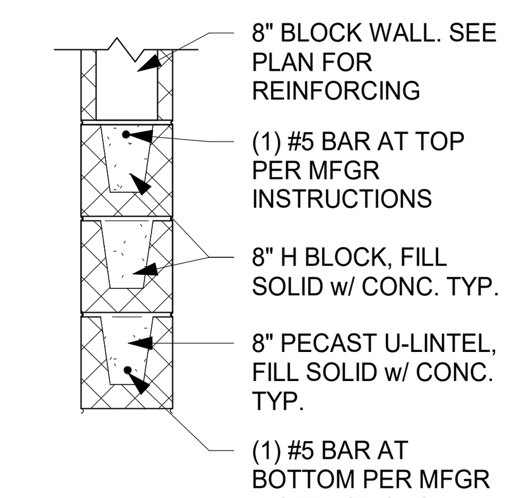
1. PER MFGR'S INSTUCTIONS, PROVIDE MINIMIUM 3" +/- 1/2" BEARING AT EACH END.
2. LINTEL MAY ACT AS COMPOSITE WITH BOND BEAMS AT THE FLOOR AND/OR ROOF LEVELS.



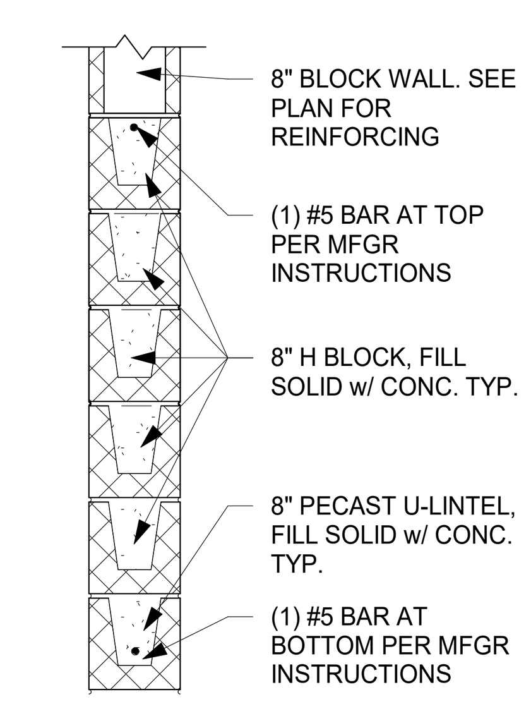
1 L1 8-8
Scale: 3/4" = 1'-0"



2 L2 8-16
Scale: 3/4" = 1'-0"



3 L3 8-24
Scale: 3/4" = 1'-0"



4 L4 8-48
Scale: 3/4" = 1'-0"

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ZOOM EXPRESS CAR WASH

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Maitland, FL 32751

Issue Dates

Revisions

1	05/17/23	Review Comments
6	02/13/24	Field Clarification

SCHEDULES & WIND PRESSURES

Scale : As indicated

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Project No.
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Date of Issue
07/21/2023

IF PLOTTED AT 11" X 17", SCALE IS ONE HALF AS NOTED.

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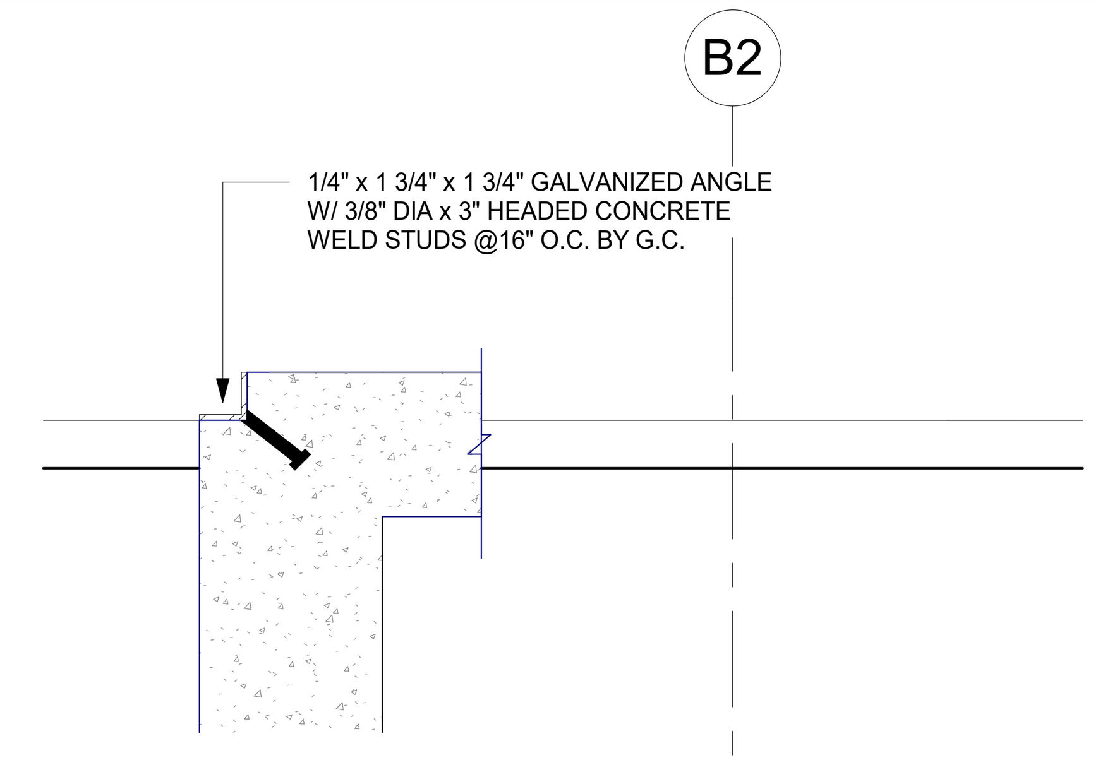
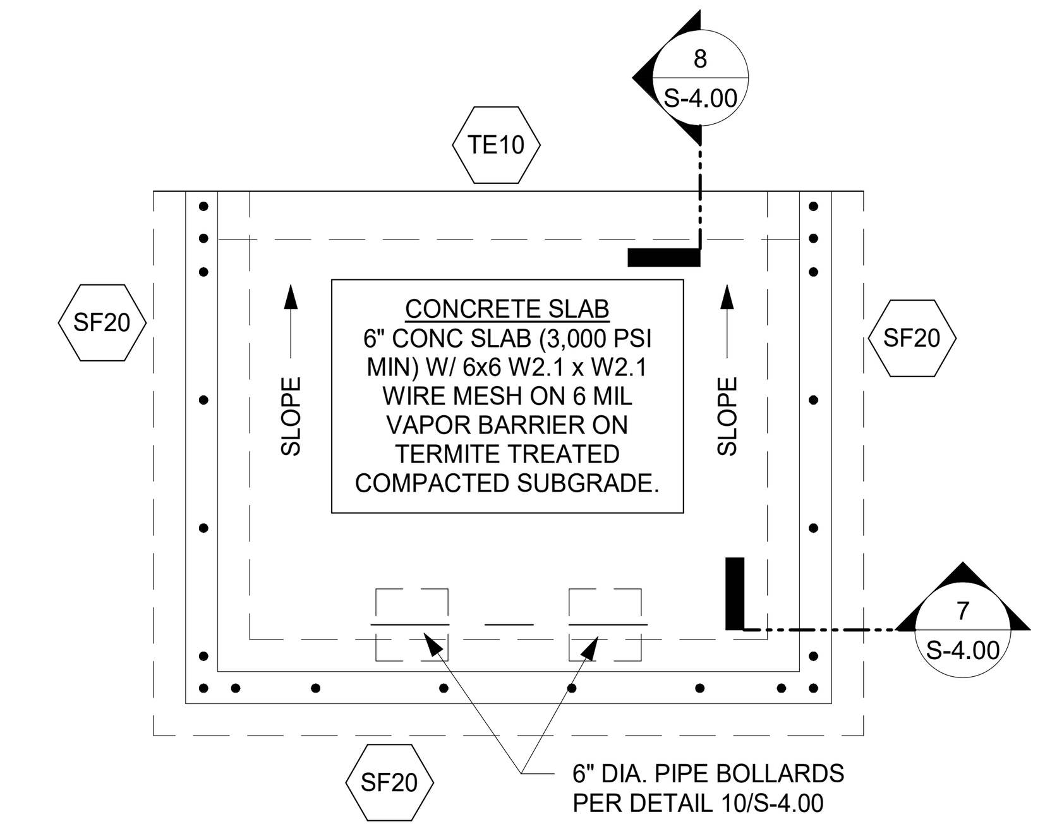
NOTE:

CANOPY STRUCTURE SUPPLIED & INSTALLED BY CANOPY VENDOR UNDER SEPARATE CONTRACT. CANOPY VENDOR TO SUPPLY ANCHOR BOLTS & TEMPLATE TO G.C. AT START OF CONSTRUCTION

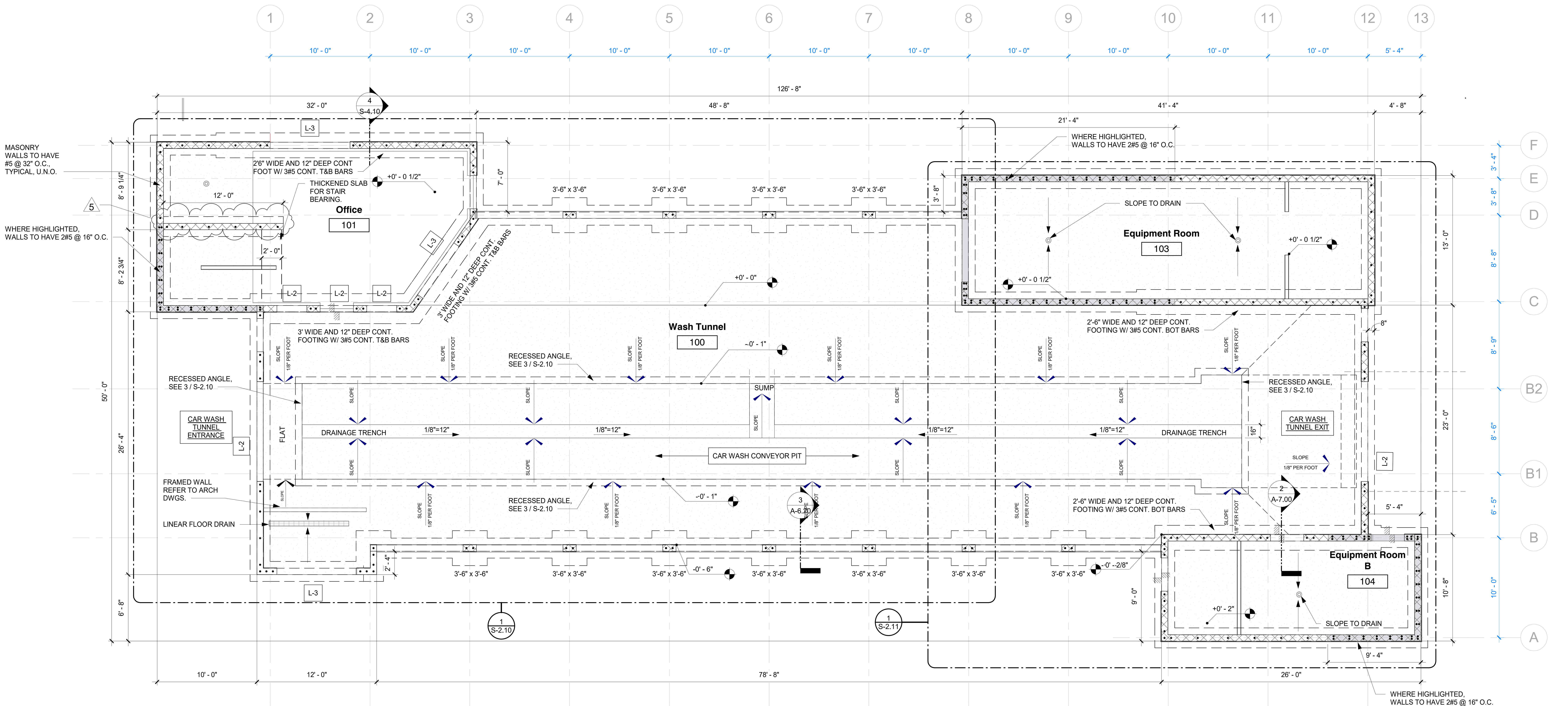
CANOPY FOUNDATIONS TO BE PROVIDED & INSTALLED AS A PART OF THIS CONTRACT.
COORDINATE FOOTING SIZES WITH CANOPY ENGINEERED SHOP DRAWINGS.
G.C. TO SET ANCHOR BOLTS.



3 PIT EDGE ANGLE DETAIL
Scale: 1 1/2" = 1'-0"



2 FOUNDATION - DUMPSTER ENCLOSURE
Scale: 1/4" = 1'-0"



1 OVERALL FOUNDATION PLAN
Scale: 3/16" = 1'-0"

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Issue Dates		
Revisions		
2	12/07/23	VE Changes
5	01/31/24	Design Coordination Revision

OVERALL FOUNDATION PLAN

Scale : As indicated

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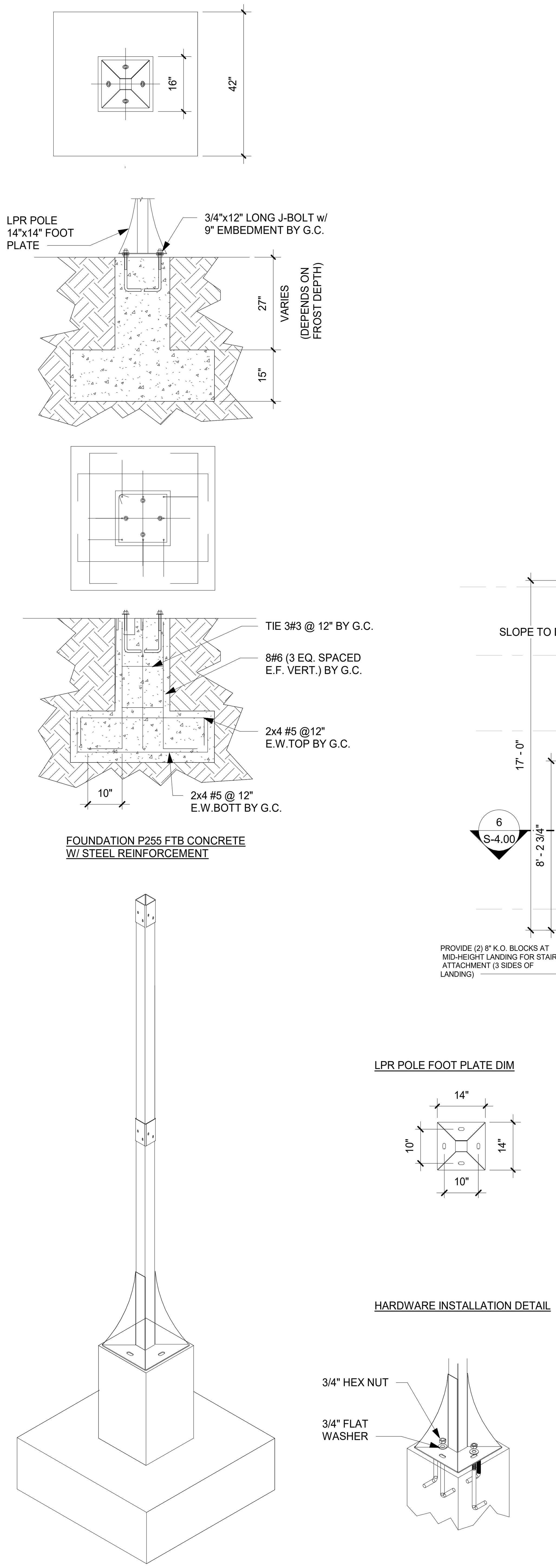
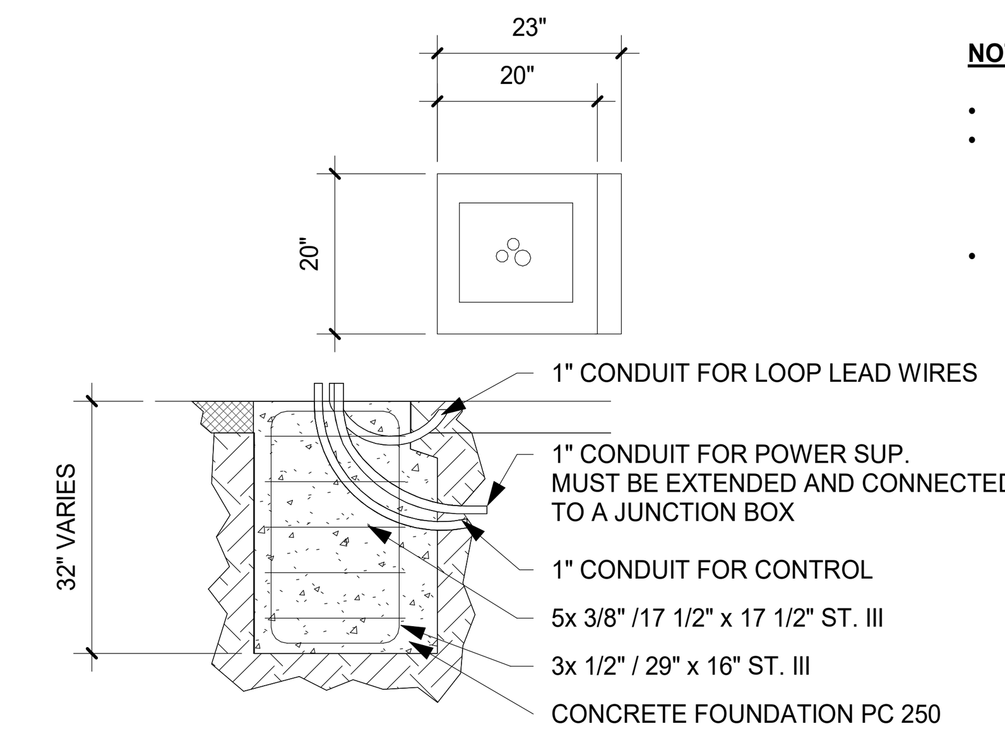

16"
42"
LPR POLE 14"x14" FOOT PLATE
3/4"x12" LONG J-BOLT w/ 9" EMBEDMENT BY G.C.
27"
15"
VARIES (DEPENDS ON FROST DEPTH)
TIE 3#3 @ 12" BY G.C.
8#6 (3 EQ. SPACED E.F. VERT.) BY G.C.
2x4 #5 @12" E.W.TOP BY G.C.
10"
2x4 #5 @ 12" E.W.BOTT BY G.C.
FOUNDATION P255 FTB CONCRETE W/ STEEL REINFORCEMENT
LPR POLE FOOT PLATE DIM
14"
10"
14"
10"
HARDWARE INSTALLATION DETAIL
3/4" HEX NUT
3/4" FLAT WASHER

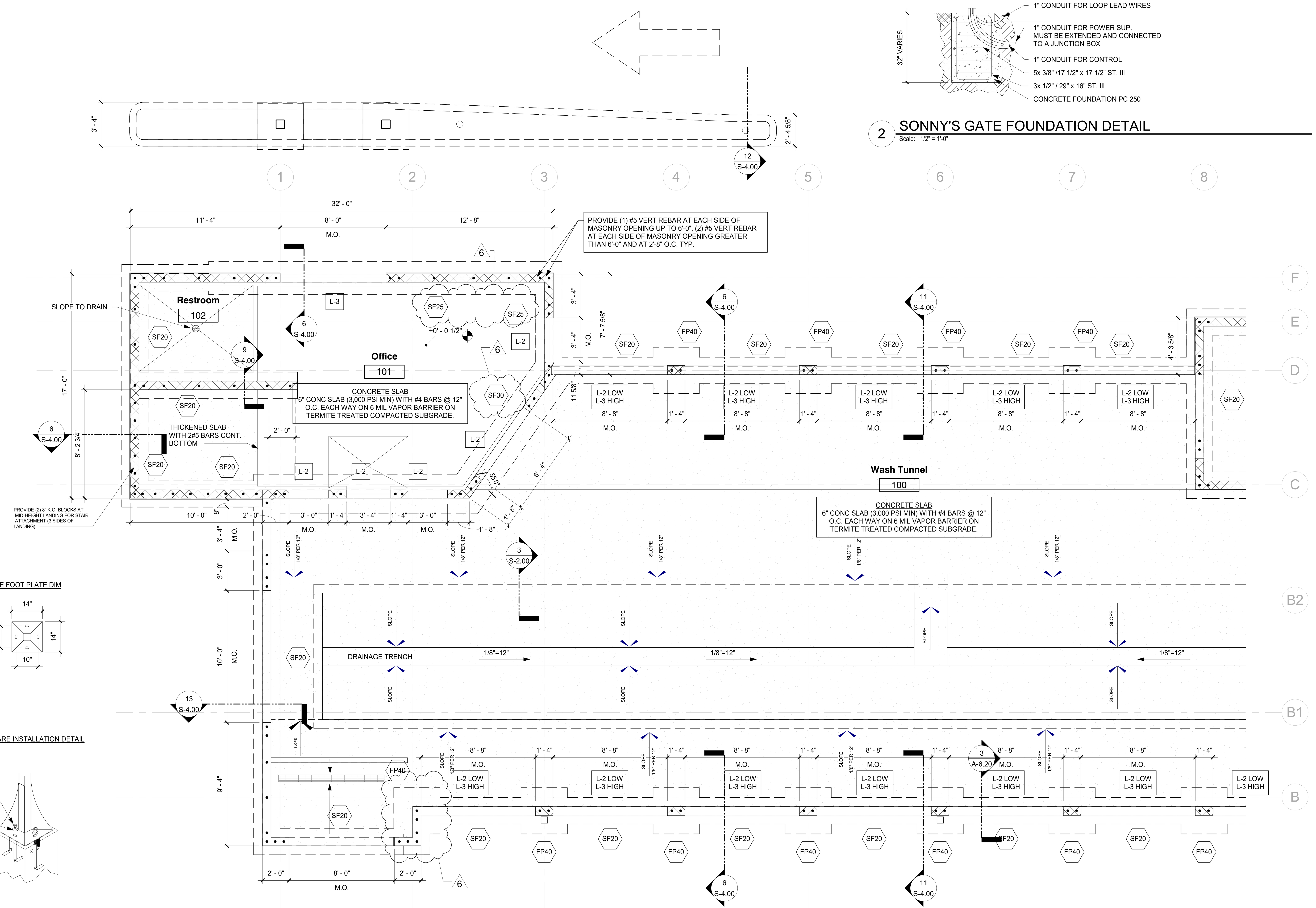
3 SONNY'S LPR POLE INSTALLATION DETAIL
Scale: 1/2" = 1'-0"



NOTES:

- FOUNDATION MUST BE LEVEL AND FLAT
- INSTALL BARRIER BOTTOM FLANGE USING HILTI HEA M 12x110 GLUE CARTIDGE AND M 12x160 TREADED ANCHOR ROD
- CUT OFF INSTALLATION TUBES, MAKING SURE TUBE ENDS ARE FLUSH W/ UPPER EDGE OF BOTTOMFLANGE

2 SONNY'S GATE FOUNDATION DETAIL
Scale: 1/2" = 1'-0"


3' - 4"
2' - 4 5/8"
12 S-4.00
1 2 3 4 5 6 7 8
F E D C B2 B1 B
32' - 0"
11' - 4"
8' - 0"
12' - 8"
M.O.
PROVIDE (1) #5 VERT REBAR AT EACH SIDE OF MASONRY OPENING UP TO 6'-0", (2) #5 VERT REBAR AT EACH SIDE OF MASONRY OPENING GREATER THAN 6'-0" AND AT 2'-8" O.C. TYP.
SLOPE TO DRAIN
Restroom
102
Office
101
+0' - 0 1/2"
CONCRETE SLAB
6" CONC SLAB (3,000 PSI MIN) WITH #4 BARS @ 12" O.C. EACH WAY ON 6 MIL VAPOR BARRIER ON TERMITE TREATED COMPACTED SUBGRADE.
THICKENED SLAB WITH 2#5 BARS CONT. BOTTOM
PROVIDE (2) 8" K.O. BLOCKS AT MID-HEIGHT LANDING FOR STAIR ATTACHMENT (3 SIDES OF LANDING)
17' - 0"
8' - 2 3/4"
10' - 0"
2' - 0"
3' - 0"
1' - 4"
3' - 4"
1' - 8"
6' - 4"
55.0°
11 5/8"
7' - 7 5/8"
4' - 3 5/8"
SF20
SF25
SF30
FP40
L-2
L-3
L-2 LOW L-3 HIGH
8' - 8"
Wash Tunnel
100
CONCRETE SLAB
6" CONC SLAB (3,000 PSI MIN) WITH #4 BARS @ 12" O.C. EACH WAY ON 6 MIL VAPOR BARRIER ON TERMITE TREATED COMPACTED SUBGRADE.
SLOPE
1/8" PER 12"
DRAINAGE TRENCH
1/8"=12"
9' - 4"
6 S-4.00
9 S-4.00
11 S-4.00
13 S-4.00
3 S-2.00
3 A-6.20
6

1 ENLARGED FOUNDATION PLAN - A
Scale: 1/4" = 1'-0"

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Issue Dates		
Revisions		
2	12/07/23	VE Changes
6	02/13/24	Field Clarification

ENLARGED FOUNDATION PLAN

Scale : As indicated

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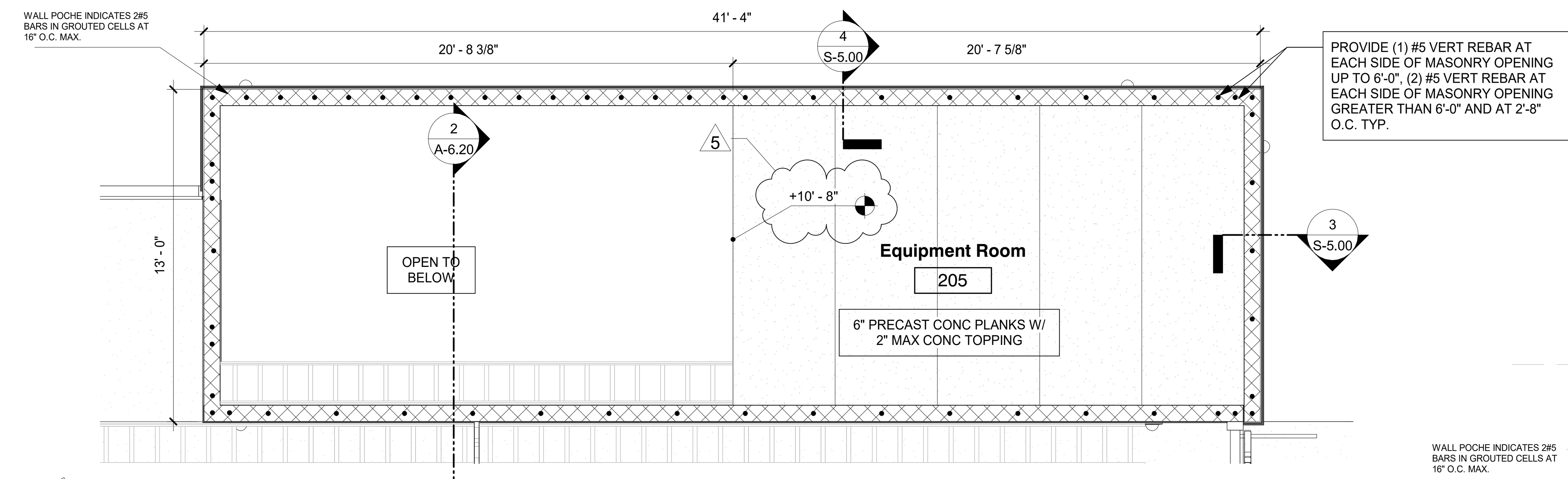
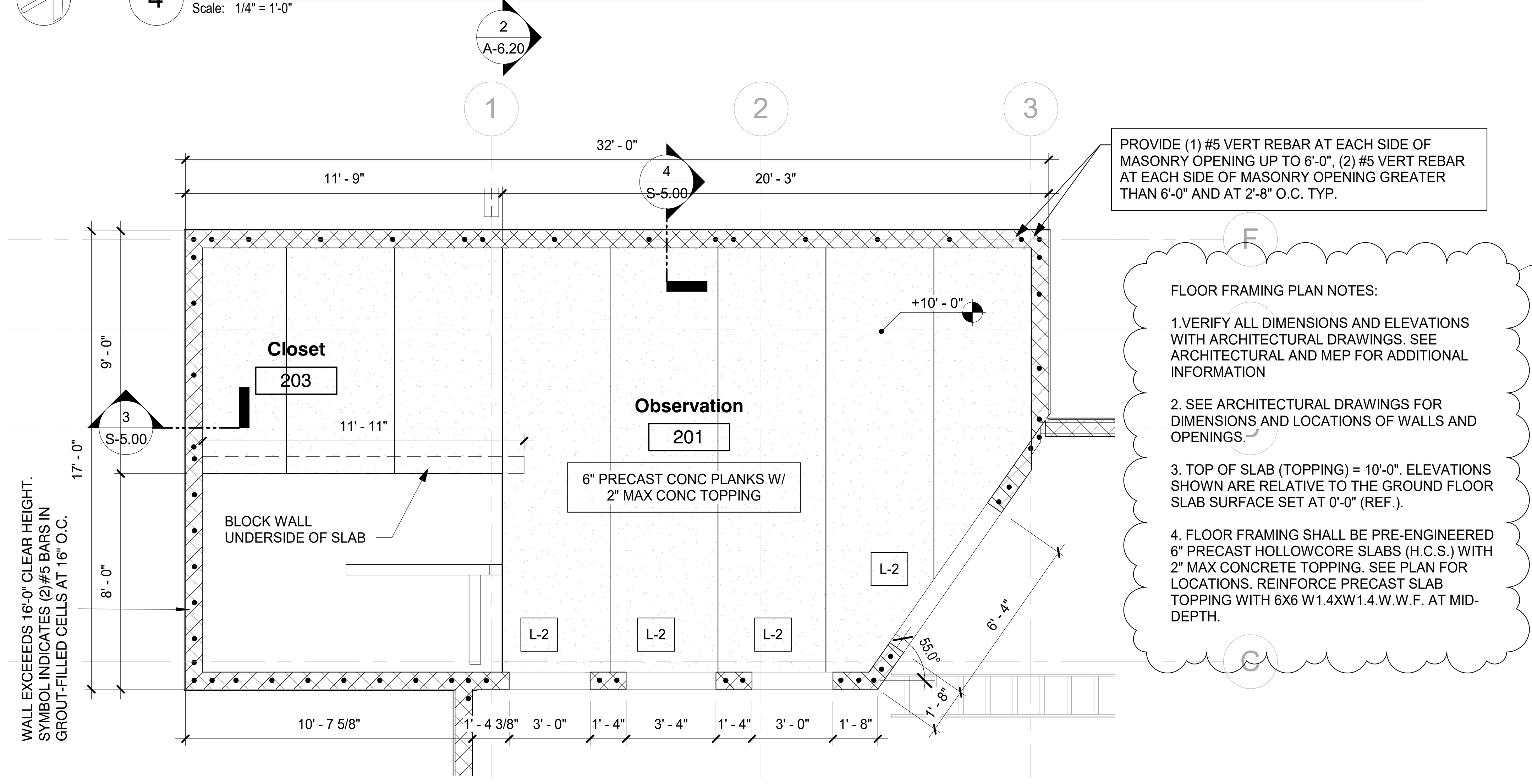


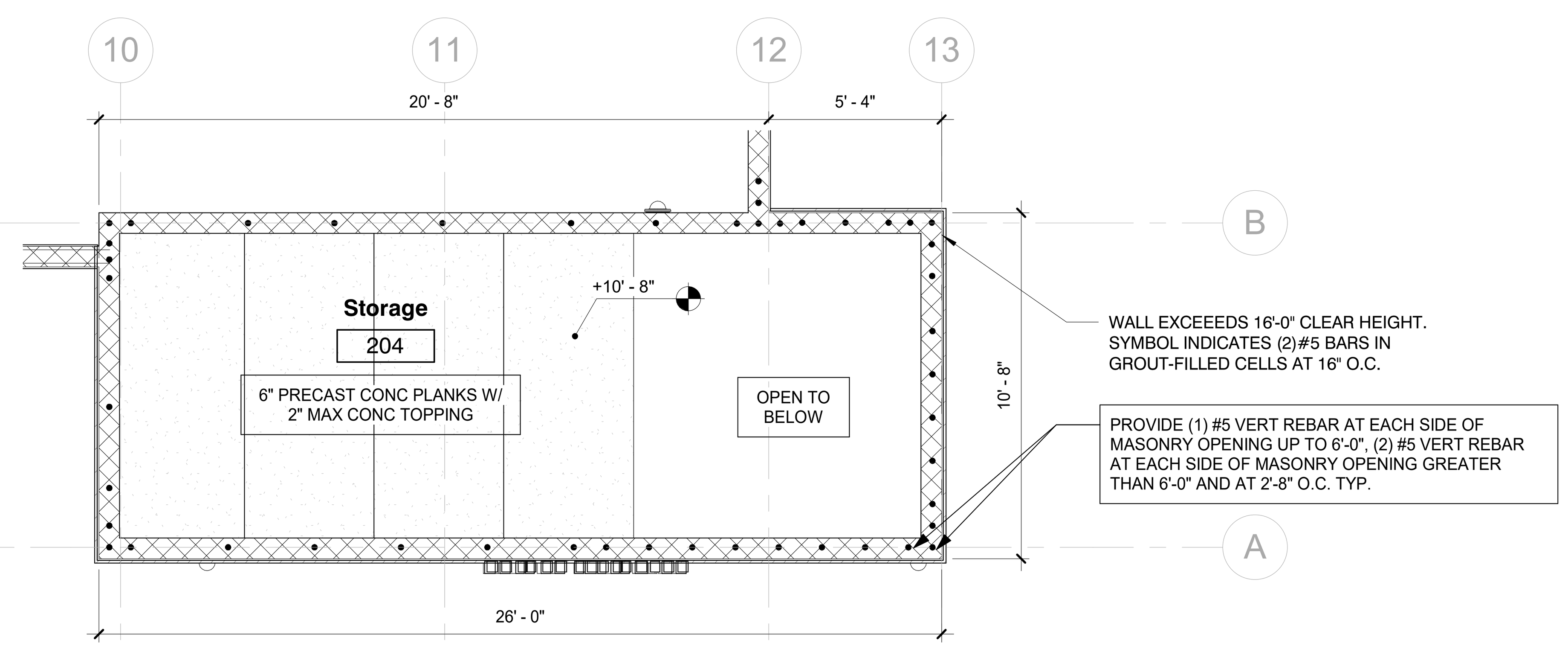
WALL POCHE INDICATES 2#5 BARS IN GROUTED CELLS AT 16" O.C. MAX.
41' - 4"
20' - 8 3/8"
20' - 7 5/8"
4
S-5.00
PROVIDE (1) #5 VERT REBAR AT EACH SIDE OF MASONRY OPENING UP TO 6'-0", (2) #5 VERT REBAR AT EACH SIDE OF MASONRY OPENING GREATER THAN 6'-0" AND AT 2'-8" O.C. TYP.
2
A-6.20
5
+10' - 8"
13' - 0"
OPEN TO BELOW
Equipment Room
205
6" PRECAST CONC PLANKS W/ 2" MAX CONC TOPPING
3
S-5.00

4 ENLARGED LEVEL 2 - TOWER C
Scale: 1/4" = 1'-0"



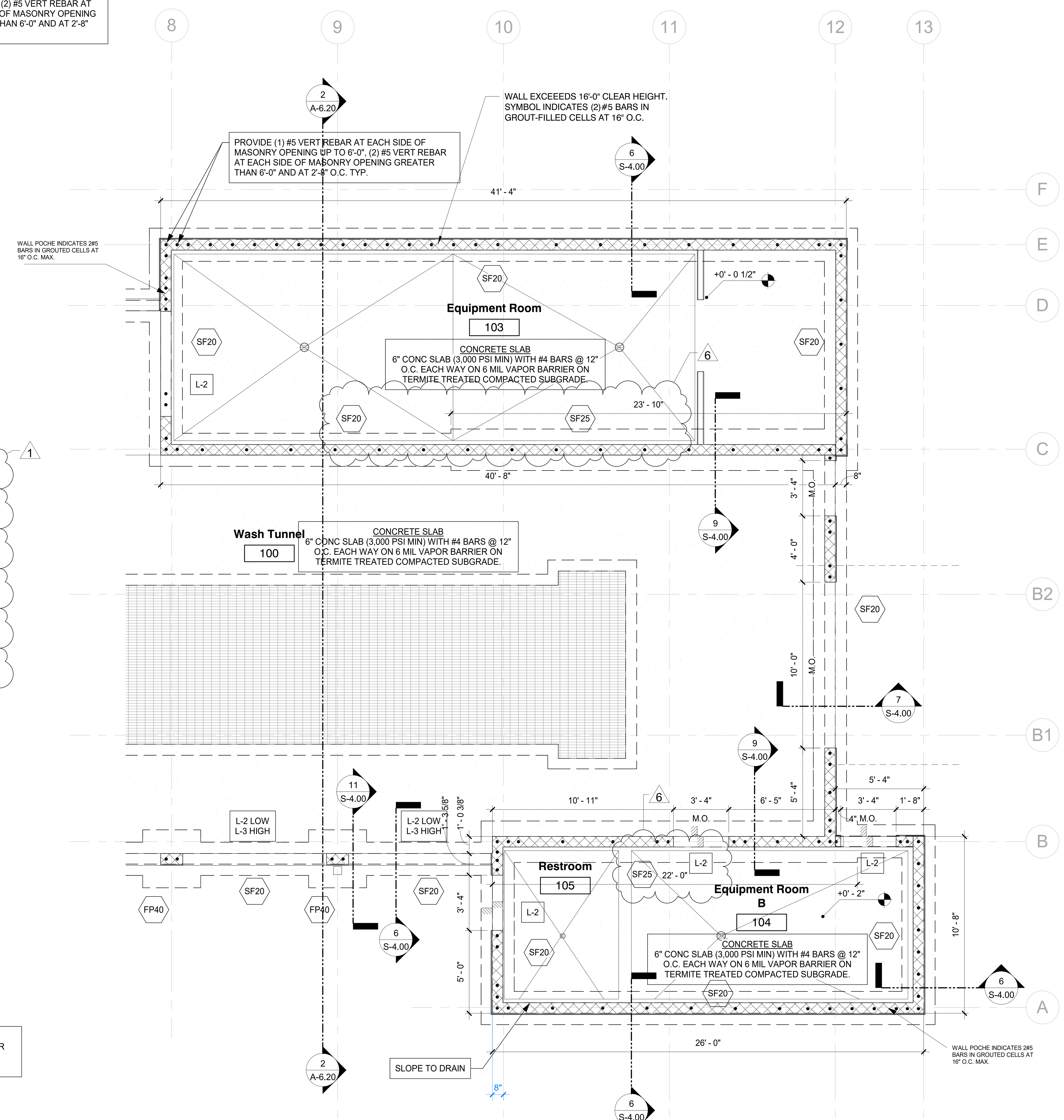
2
A-6.20
1
2
3
32' - 0"
11' - 9"
20' - 3"
4
S-5.00
PROVIDE (1) #5 VERT REBAR AT EACH SIDE OF MASONRY OPENING UP TO 6'-0", (2) #5 VERT REBAR AT EACH SIDE OF MASONRY OPENING GREATER THAN 6'-0" AND AT 2'-8" O.C. TYP.
+10' - 0"
Closet
203
3
S-5.00
11' - 11"
Observation
201
6" PRECAST CONC PLANKS W/ 2" MAX CONC TOPPING
9' - 0"
17' - 0"
8' - 0"
BLOCK WALL UNDERSIDE OF SLAB
WALL EXCEEEDS 16'-0" CLEAR HEIGHT. SYMBOL INDICATES (2)#5 BARS IN GROUT-FILLED CELLS AT 16" O.C.
L-2
L-2
L-2
L-2
6' - 4"
55.0°
1' - 8"
10' - 7 5/8"
1' - 4 3/8"
3' - 0"
1' - 4"
3' - 4"
1' - 4"
3' - 0"
1' - 8"
1
FLOOR FRAMING PLAN NOTES:
1.VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL AND MEP FOR ADDITIONAL INFORMATION
2. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND LOCATIONS OF WALLS AND OPENINGS.
3. TOP OF SLAB (TOPPING) = 10'-0". ELEVATIONS SHOWN ARE RELATIVE TO THE GROUND FLOOR SLAB SURFACE SET AT 0'-0" (REF.).
4. FLOOR FRAMING SHALL BE PRE-ENGINEERED 6" PRECAST HOLLOWCORE SLABS (H.C.S.) WITH 2" MAX CONCRETE TOPPING. SEE PLAN FOR LOCATIONS. REINFORCE PRECAST SLAB TOPPING WITH 6X6 W1.4XW1.4.W.W.F. AT MID-DEPTH.

3 ENLARGED LEVEL 2 - TOWER A
Scale: 1/4" = 1'-0"



10
11
12
13
20' - 8"
5' - 4"
B
+10' - 8"
Storage
204
6" PRECAST CONC PLANKS W/ 2" MAX CONC TOPPING
OPEN TO BELOW
10' - 8"
WALL EXCEEEDS 16'-0" CLEAR HEIGHT. SYMBOL INDICATES (2)#5 BARS IN GROUT-FILLED CELLS AT 16" O.C.
PROVIDE (1) #5 VERT REBAR AT EACH SIDE OF MASONRY OPENING UP TO 6'-0", (2) #5 VERT REBAR AT EACH SIDE OF MASONRY OPENING GREATER THAN 6'-0" AND AT 2'-8" O.C. TYP.
A
26' - 0"

2 ENLARGED LEVEL 2 - TOWER B
Scale: 1/4" = 1'-0"



8
9
10
11
12
13
WALL EXCEEEDS 16'-0" CLEAR HEIGHT. SYMBOL INDICATES (2)#5 BARS IN GROUT-FILLED CELLS AT 16" O.C.
2
A-6.20
PROVIDE (1) #5 VERT REBAR AT EACH SIDE OF MASONRY OPENING UP TO 6'-0", (2) #5 VERT REBAR AT EACH SIDE OF MASONRY OPENING GREATER THAN 6'-0" AND AT 2'-8" O.C. TYP.
6
S-4.00
F
41' - 4"
E
WALL POCHE INDICATES 2#5 BARS IN GROUTED CELLS AT 16" O.C. MAX.
SF20
+0' - 0 1/2"
D
Equipment Room
103
CONCRETE SLAB
6" CONC SLAB (3,000 PSI MIN) WITH #4 BARS @ 12" O.C. EACH WAY ON 6 MIL VAPOR BARRIER ON TERMITE TREATED COMPACTED SUBGRADE.
SF20
SF20
6
L-2
23' - 10"
SF20
SF25
C
40' - 8"
8"
3' - 4"
M.O.
9
S-4.00
4' - 0"
Wash Tunnel
100
CONCRETE SLAB
6" CONC SLAB (3,000 PSI MIN) WITH #4 BARS @ 12" O.C. EACH WAY ON 6 MIL VAPOR BARRIER ON TERMITE TREATED COMPACTED SUBGRADE.
B2
SF20
10' - 0"
M.O.
7
S-4.00
B1
9
S-4.00
11
S-4.00
5' - 4"
L-2 LOW
L-3 HIGH
L-2 LOW
L-3 HIGH
1' - 3 5/8"
1' - 0 3/8"
10' - 11"
6
3' - 4"
6' - 5"
5' - 4"
3' - 4"
1' - 8"
M.O.
4" M.O.
B
Restroom
105
SF25
22' - 0"
L-2
L-2
Equipment Room B
104
+0' - 2"
SF20
SF20
SF20
FP40
FP40
L-2
SF20
6
S-4.00
3' - 4"
5' - 0"
CONCRETE SLAB
6" CONC SLAB (3,000 PSI MIN) WITH #4 BARS @ 12" O.C. EACH WAY ON 6 MIL VAPOR BARRIER ON TERMITE TREATED COMPACTED SUBGRADE.
SF20
10' - 8"
6
S-4.00
A
26' - 0"
WALL POCHE INDICATES 2#5 BARS IN GROUTED CELLS AT 16" O.C. MAX.
SLOPE TO DRAIN
8"
2
A-6.20
6
S-4.00

1 ENLARGED FOUNDATION PLAN - B
Scale: 1/4" = 1'-0"

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ENLARGED FOUNDATION PLAN - CONT
Scale : 1/4" = 1'-0"

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7305
REGISTERED ARCHITECT

[illegible]

Date of Issue
07/21/2023

IF PLOTTED AT 11" X 17", SCALE IS ONE HALF AS NOTED.

ROOF FRAMING PLAN NOTES:

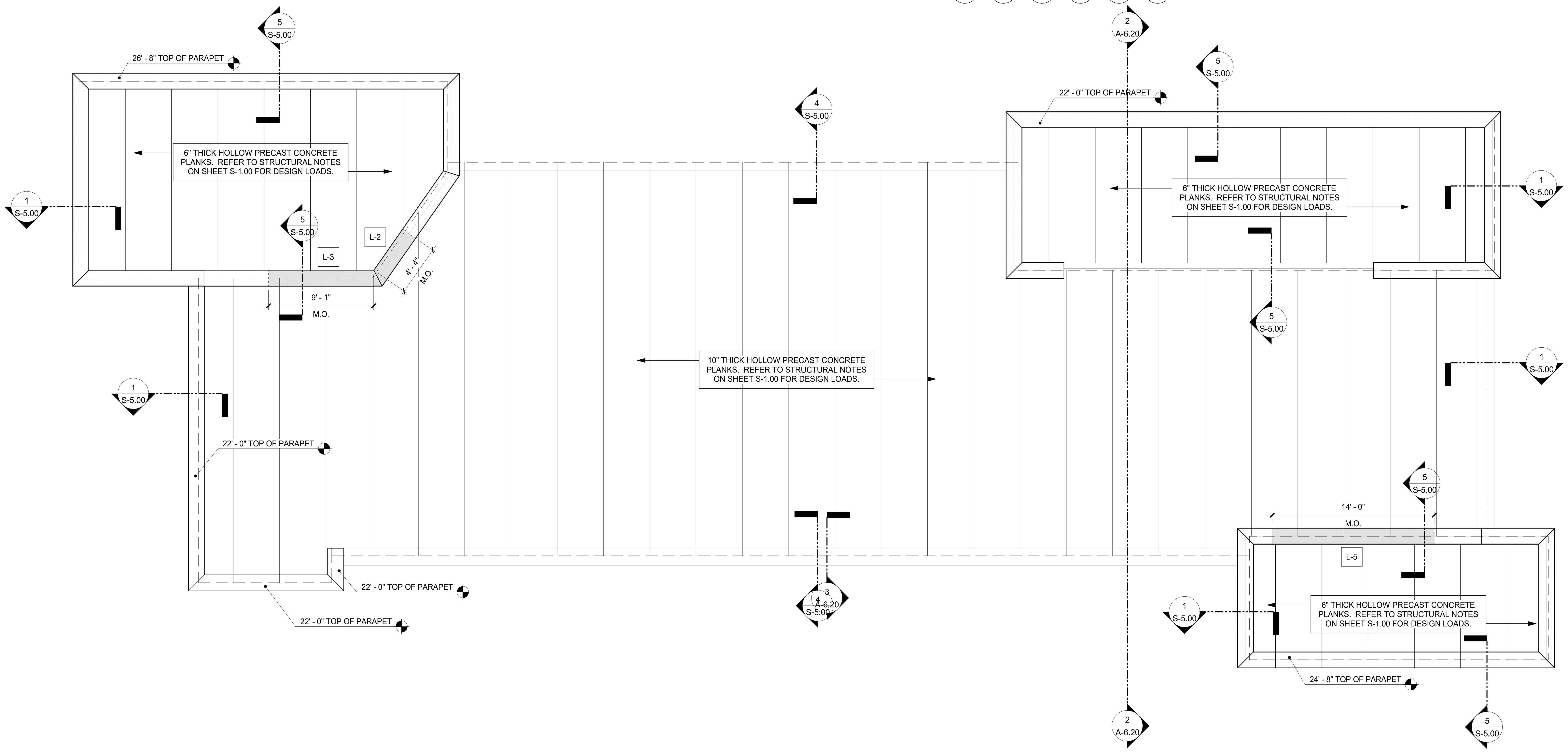
1. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL AND MEP FOR ADDITIONAL INFORMATION.

2.SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND LOCATIONS OF WALLS AND OPENINGS.

3.ROOF FRAMING SHALL BE PRE-ENGINEERED UNTOPPED 10" PRECAST HOLLOWCORE SLABS (H.C.S).

4.TOP OF HOLLOWCORE SLABS SLOPE SEE SECTIONS FOR ELEVATIONS.

2



26' - 8" TOP OF PARAPET
6" THICK HOLLOW PRECAST CONCRETE PLANKS. REFER TO STRUCTURAL NOTES ON SHEET S-1.00 FOR DESIGN LOADS.
L-2
L-3
9' - 1"
M.O.
4' - 4"
M.O.
22' - 0" TOP OF PARAPET
10" THICK HOLLOW PRECAST CONCRETE PLANKS. REFER TO STRUCTURAL NOTES ON SHEET S-1.00 FOR DESIGN LOADS.
22' - 0" TOP OF PARAPET
6" THICK HOLLOW PRECAST CONCRETE PLANKS. REFER TO STRUCTURAL NOTES ON SHEET S-1.00 FOR DESIGN LOADS.
14' - 0"
M.O.
L-5
6" THICK HOLLOW PRECAST CONCRETE PLANKS. REFER TO STRUCTURAL NOTES ON SHEET S-1.00 FOR DESIGN LOADS.
24' - 8" TOP OF PARAPET
22' - 0" TOP OF PARAPET
22' - 0" TOP OF PARAPET
5 S-5.00
4 S-5.00
1 S-5.00
2 A-6.20
3 A-6.20

1 ROOF FRAMING PLAN

Scale: 3/16" = 1'-0"

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Issue Dates		
Revisions		
2	12/07/23	VE Changes

ROOF FRAMING PLAN

Scale : 3/16" = 1'-0"

Plot Date
2/13/2024 5:26:18 PM

Project No.
21049

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S-3.00

STATE OF FLORIDA
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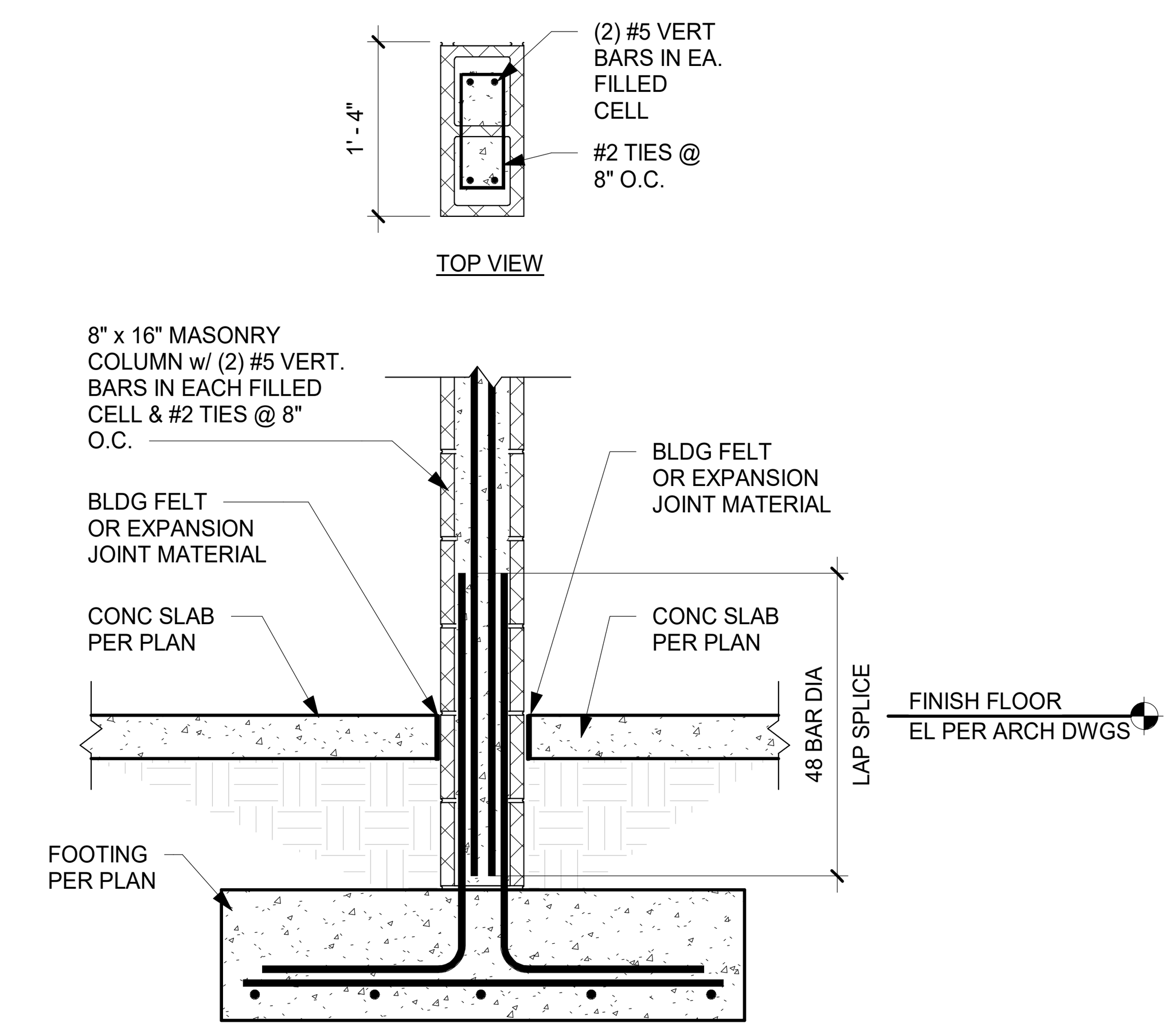
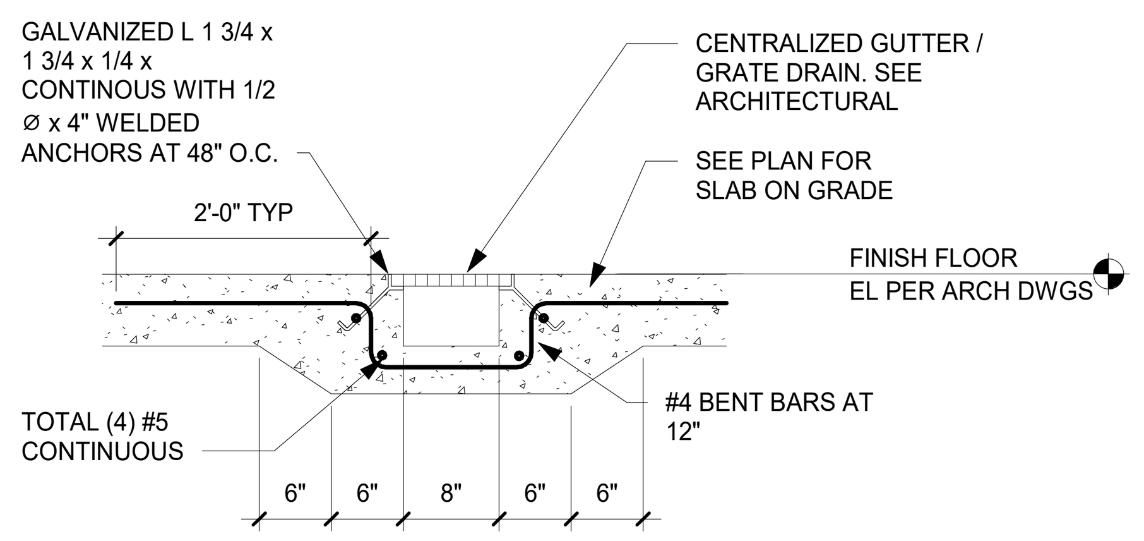
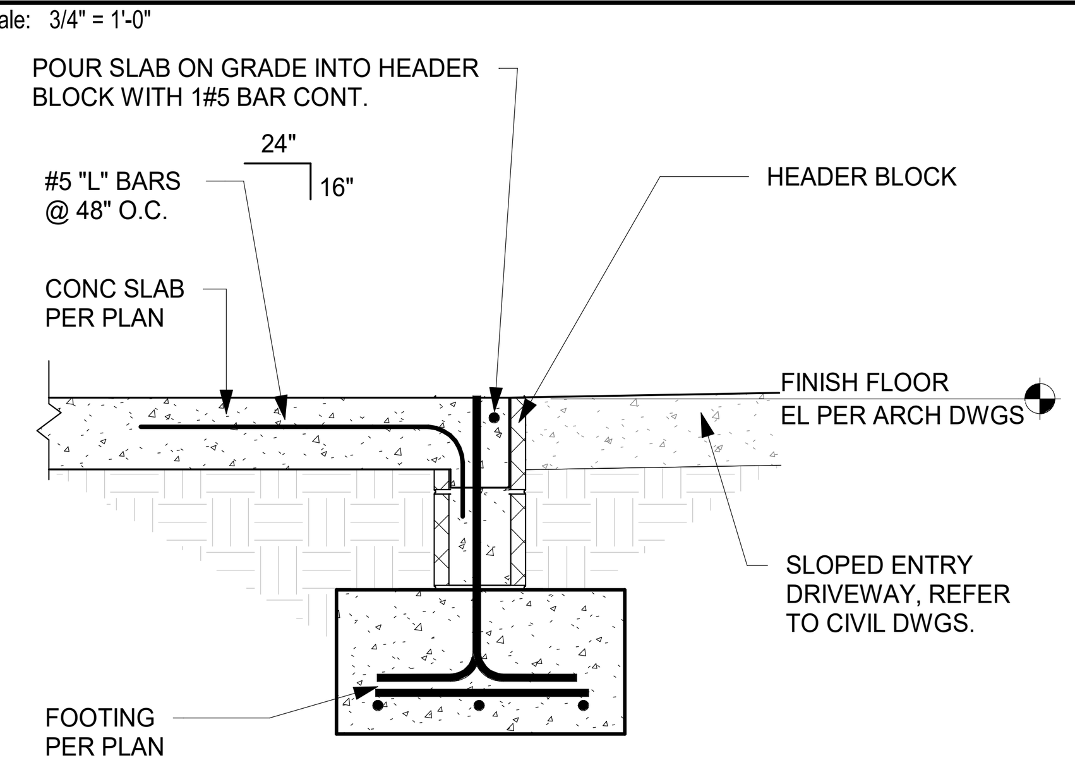


1' - 4"
(2) #5 VERT BARS IN EA. FILLED CELL
#2 TIES @ 8" O.C.
TOP VIEW
8" x 16" MASONRY COLUMN w/ (2) #5 VERT. BARS IN EACH FILLED CELL & #2 TIES @ 8" O.C.
BLDG FELT OR EXPANSION JOINT MATERIAL
CONC SLAB PER PLAN
BLDG FELT OR EXPANSION JOINT MATERIAL
CONC SLAB PER PLAN
48 BAR DIA
LAP SPLICE
FINISH FLOOR
EL PER ARCH DWGS
FOOTING PER PLAN

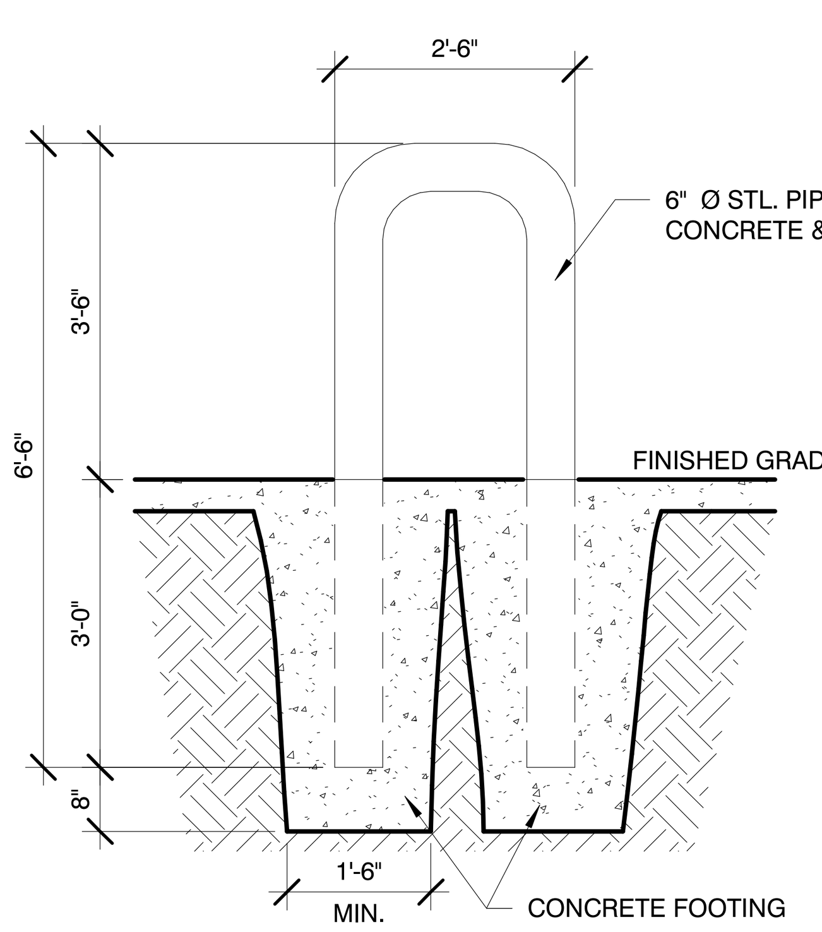
11 MASONRY COLUMN AT WASH TUNNEL
SCALE: 3/4" = 1'-0"



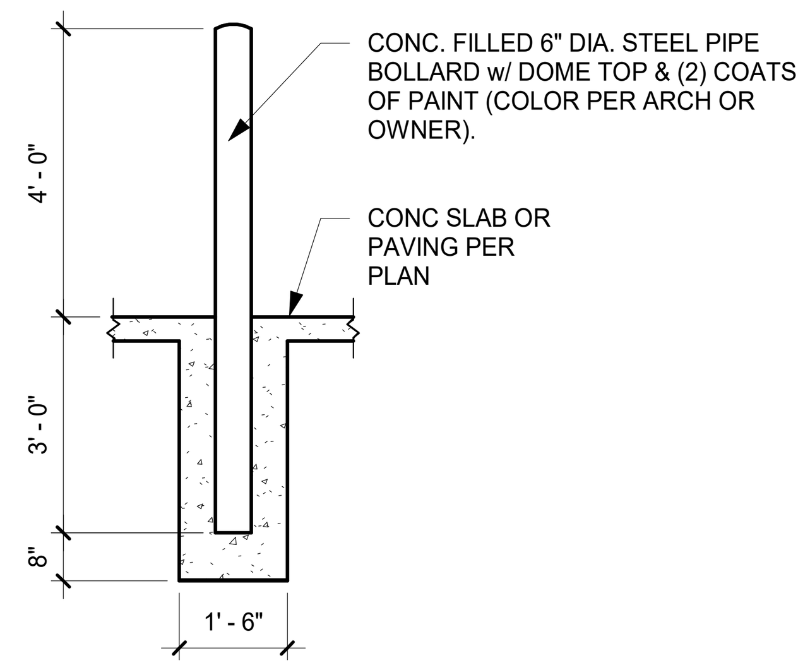
14 TRENCH DRAIN
Scale: 3/4" = 1'-0"



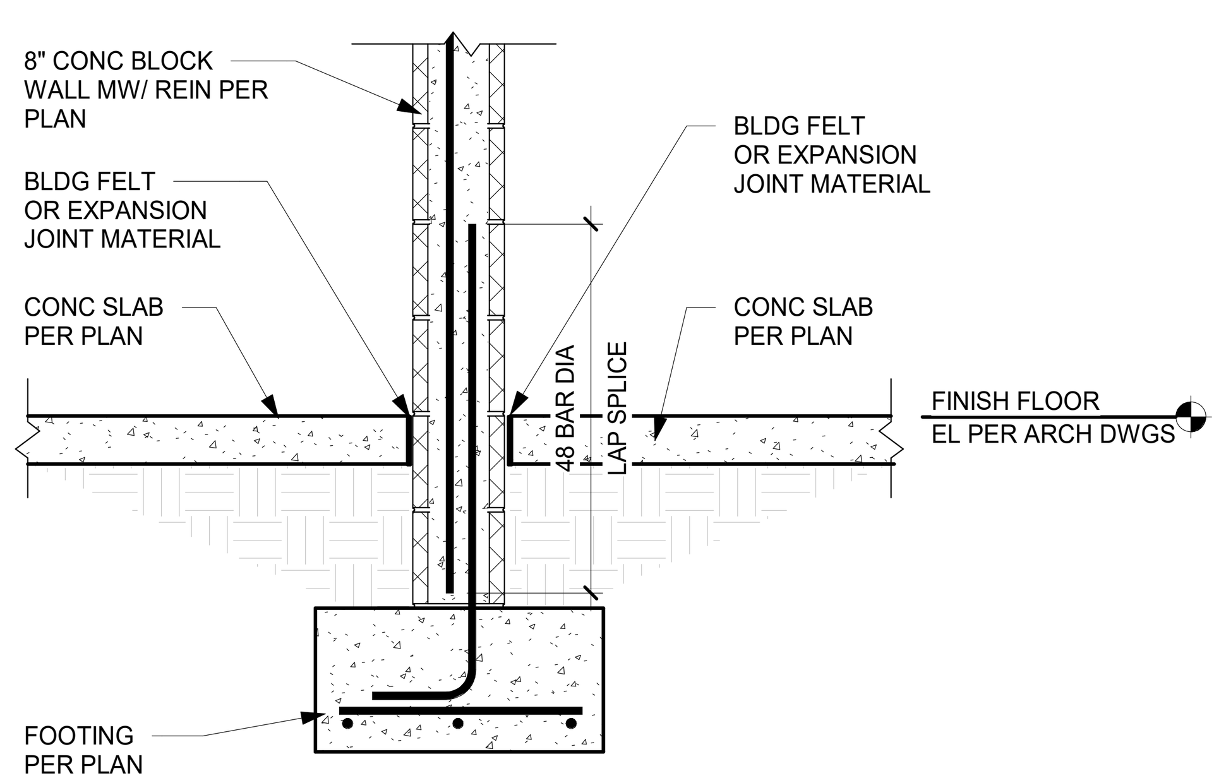
13 STEMWALL C AT WASH TUNNEL ENTRY
Scale: 3/4" = 1'-0"



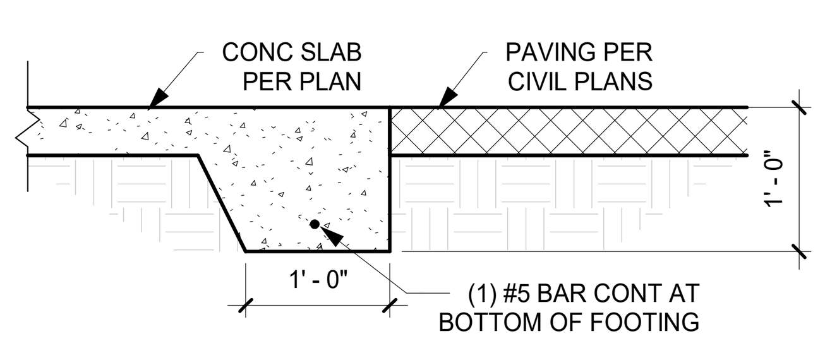
12 STRUCTURAL - ARCHED BOLLARD
Scale: 1/2" = 1'-0"



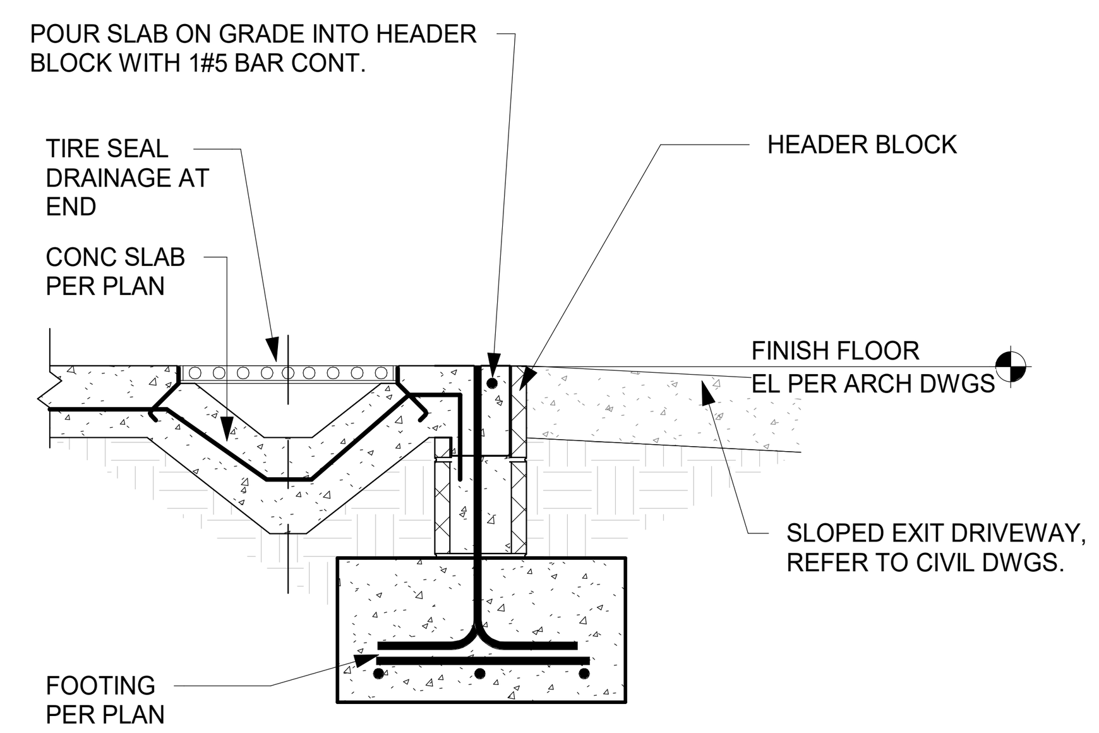
10 BOLLARD
SCALE: 3/8" = 1'-0"



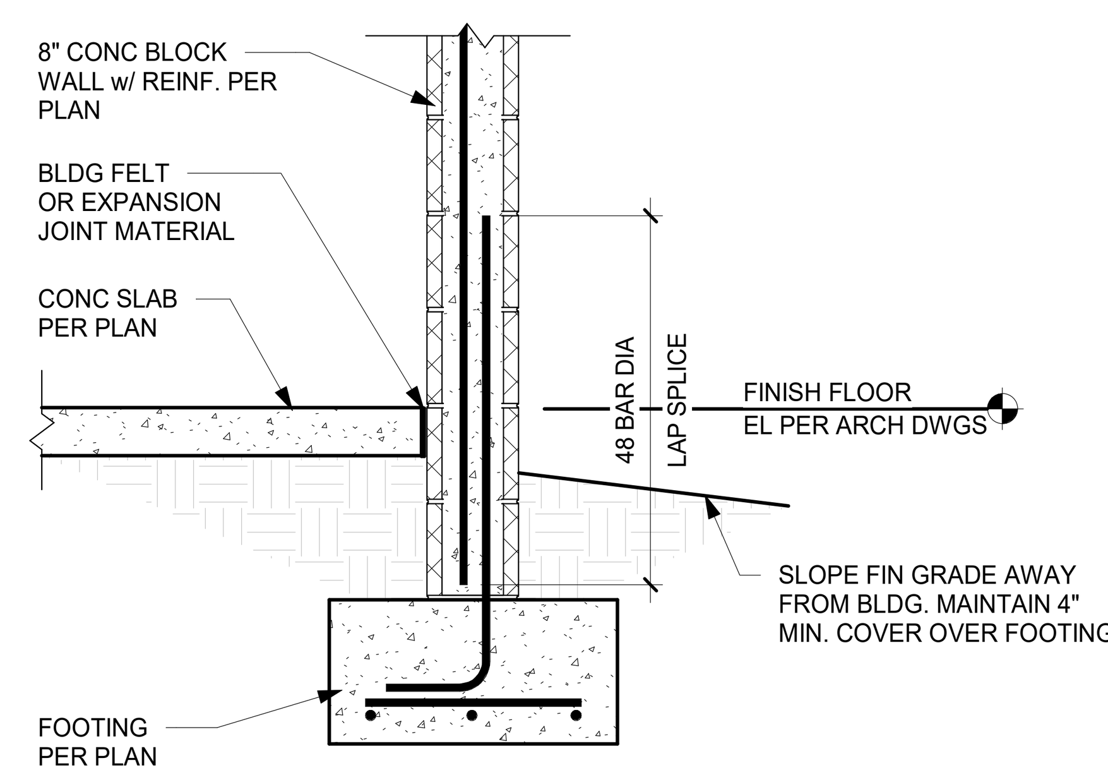
9 INTERIOR STEMWALL
SCALE: 3/4" = 1'-0"



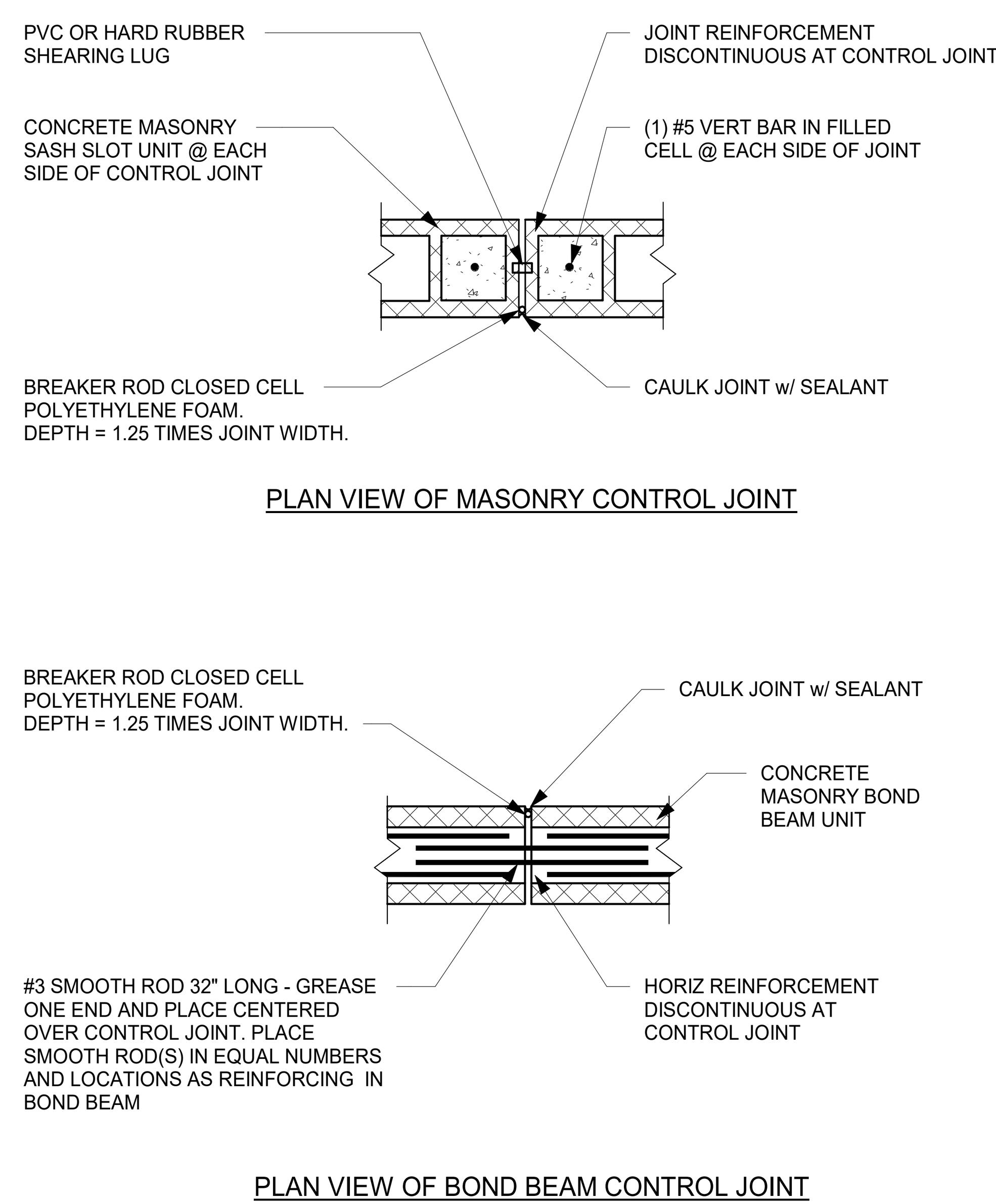
8 THICKENED EDGE
SCALE: 3/4" = 1'-0"



7 STEMWALL B AT WASH TUNNEL EXIT
SCALE: 3/4" = 1'-0"

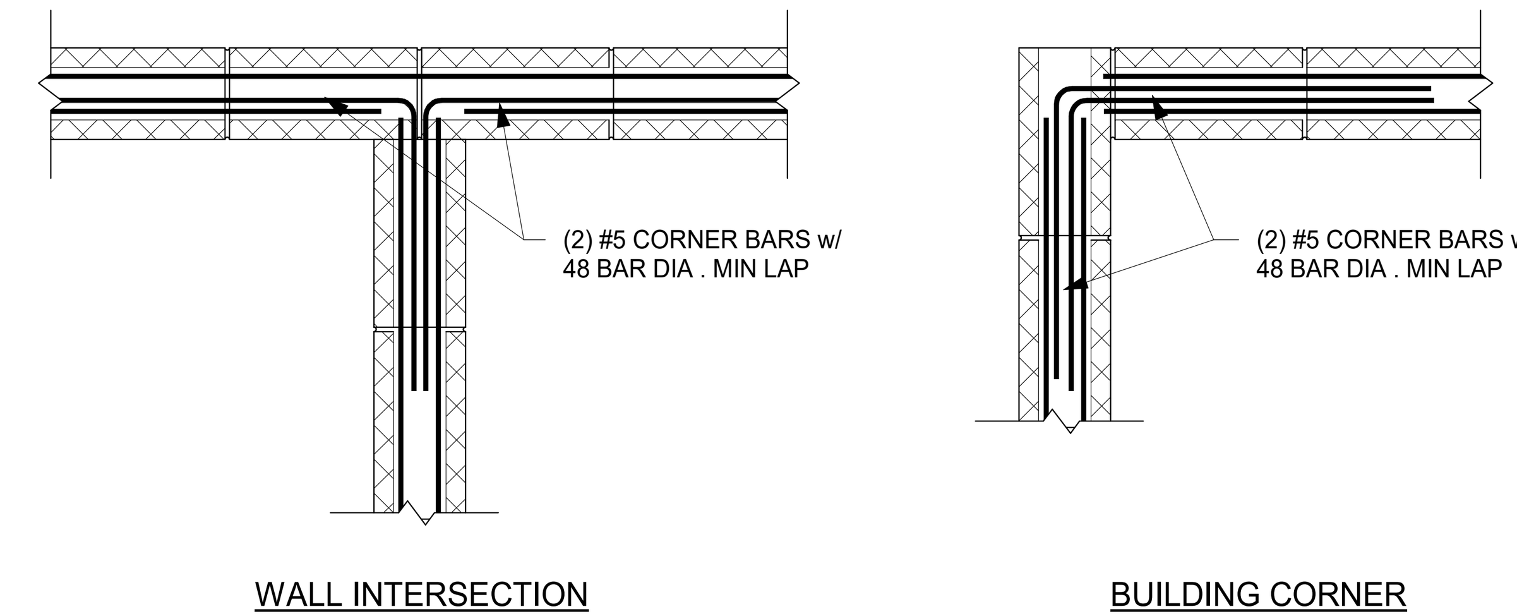


6 STEMWALL A
SCALE: 3/4" = 1'-0"

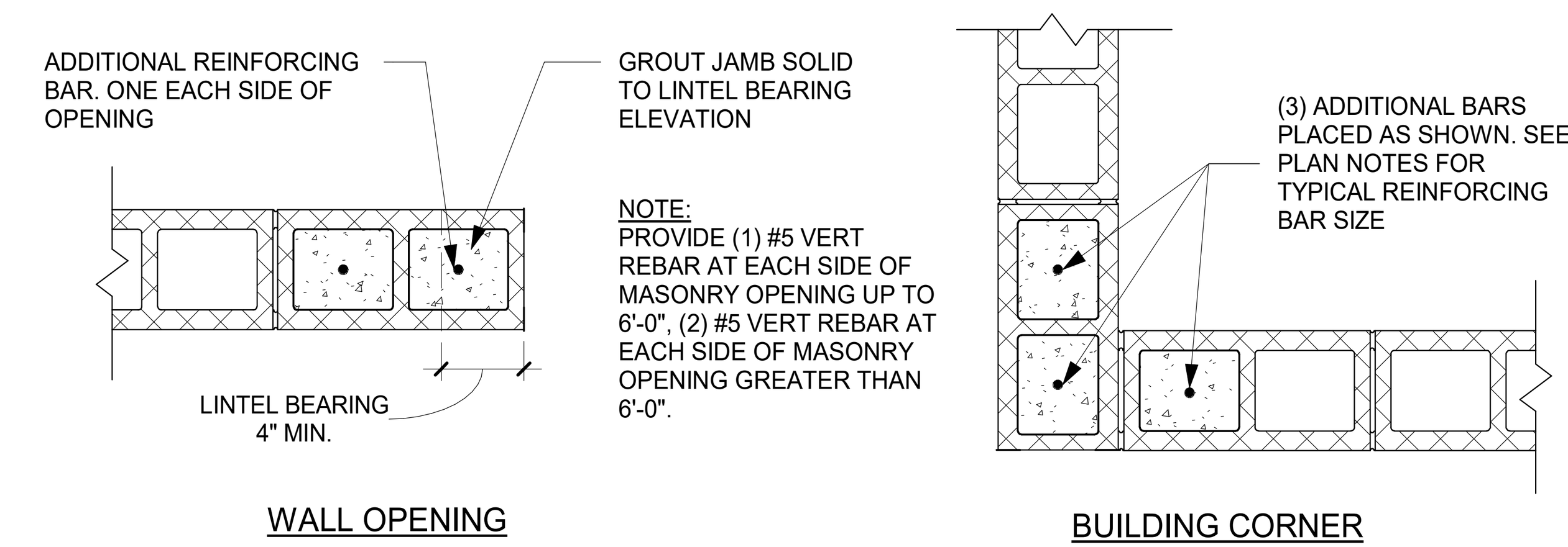


PVC OR HARD RUBBER SHEARING LUG
JOINT REINFORCEMENT DISCONTINUOUS AT CONTROL JOINT
CONCRETE MASONRY SASH SLOT UNIT @ EACH SIDE OF CONTROL JOINT
(1) #5 VERT BAR IN FILLED CELL @ EACH SIDE OF JOINT
BREAKER ROD CLOSED CELL POLYETHYLENE FOAM. DEPTH = 1.25 TIMES JOINT WIDTH.
CAULK JOINT w/ SEALANT
PLAN VIEW OF MASONRY CONTROL JOINT
BREAKER ROD CLOSED CELL POLYETHYLENE FOAM. DEPTH = 1.25 TIMES JOINT WIDTH.
CAULK JOINT w/ SEALANT
CONCRETE MASONRY BOND BEAM UNIT
#3 SMOOTH ROD 32" LONG - GREASE ONE END AND PLACE CENTERED OVER CONTROL JOINT. PLACE SMOOTH ROD(S) IN EQUAL NUMBERS AND LOCATIONS AS REINFORCING IN BOND BEAM
HORIZ REINFORCEMENT DISCONTINUOUS AT CONTROL JOINT
PLAN VIEW OF BOND BEAM CONTROL JOINT

5 MASONRY CONTROL JOINT
SCALE: 1" = 1'-0"

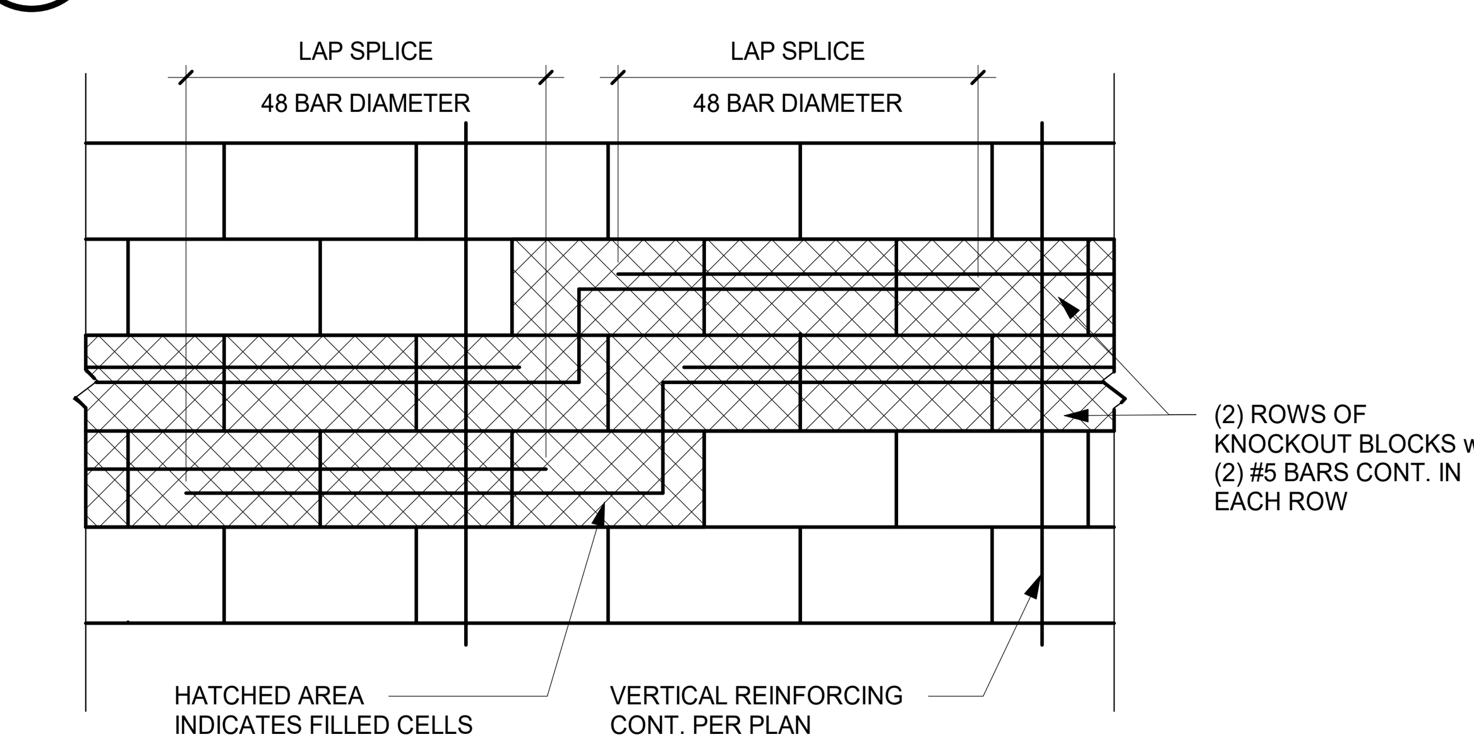


4 BOND BEAM CORNER CONTINUITY
SCALE: 3/4" = 1'-0"

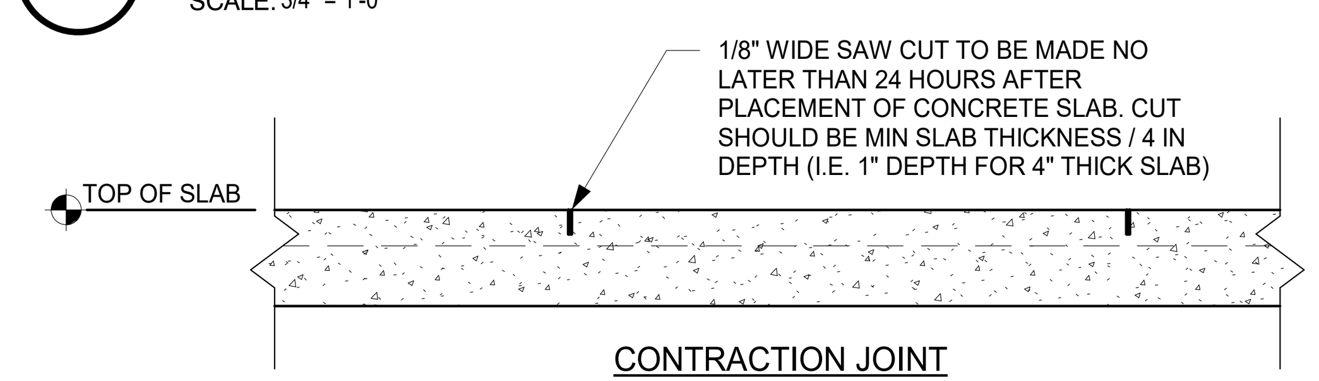


ADDITIONAL REINFORCING BAR. ONE EACH SIDE OF OPENING
GROUT JAMB SOLID TO LINTEL BEARING ELEVATION
NOTE:
PROVIDE (1) #5 VERT REBAR AT EACH SIDE OF MASONRY OPENING UP TO 6'-0", (2) #5 VERT REBAR AT EACH SIDE OF MASONRY OPENING GREATER THAN 6'-0".
LINTEL BEARING 4" MIN.
WALL OPENING
(3) ADDITIONAL BARS PLACED AS SHOWN. SEE PLAN NOTES FOR TYPICAL REINFORCING BAR SIZE
BUILDING CORNER

3 TYPICAL MASONRY DETAILS
SCALE: 1" = 1'-0"

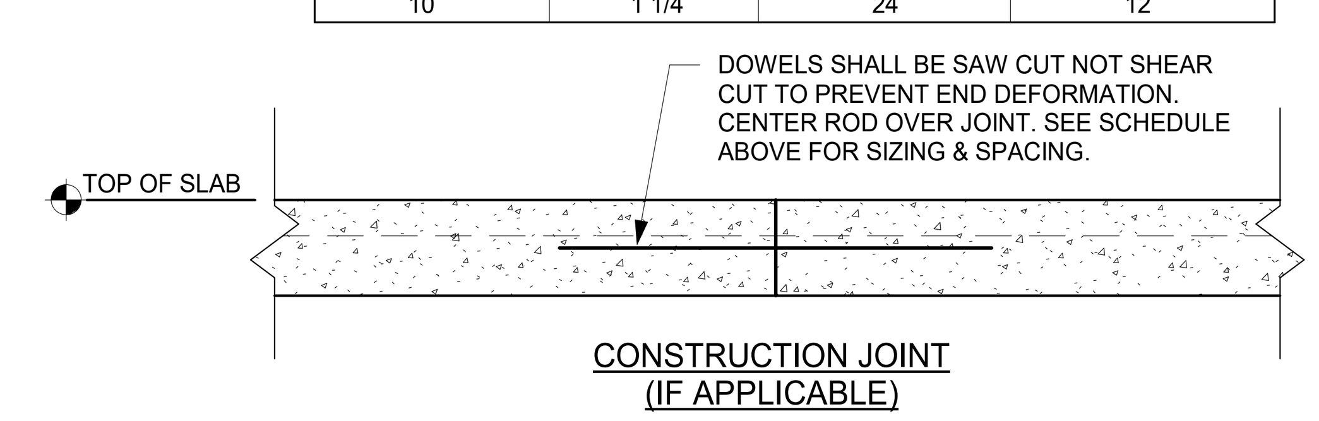


2 MASONRY BOND BEAM STEP
SCALE: 3/4" = 1'-0"



SMOOTH DOWEL SCHEDULE

SLAB DEPTH (IN)	DIAMETER (IN)	TOTAL LENGTH (IN)	SPACING C TO C (IN)
4	1/2	18	12
5	5/8	18	12
6	3/4	18	12
7	7/8	24	12
8	1	24	12
9	1 1/8	24	12
10	1 1/4	24	12



1 CONTRACTION & CONSTRUCTION JOINTS
SCALE: 1 1/2" = 1'-0"

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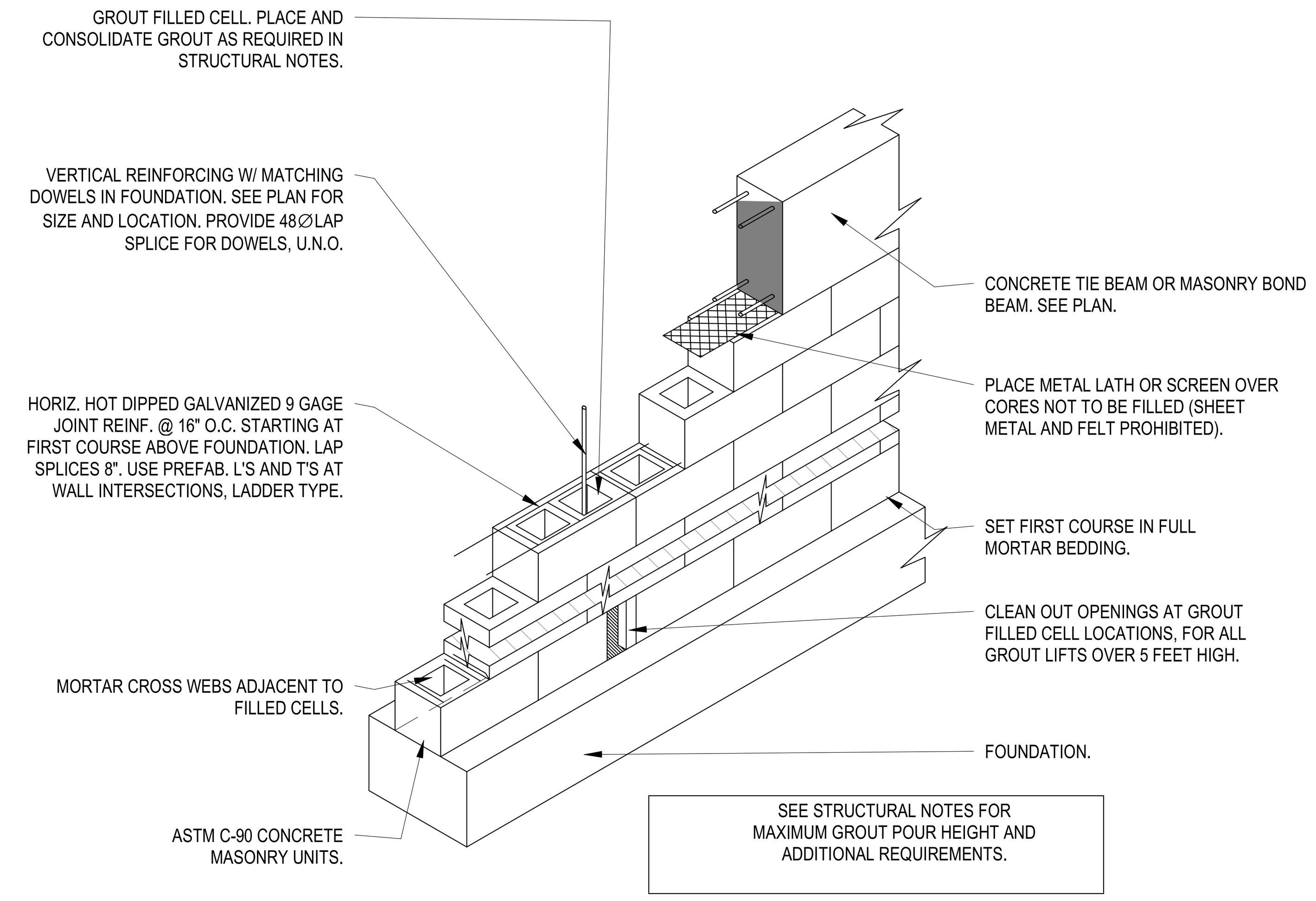
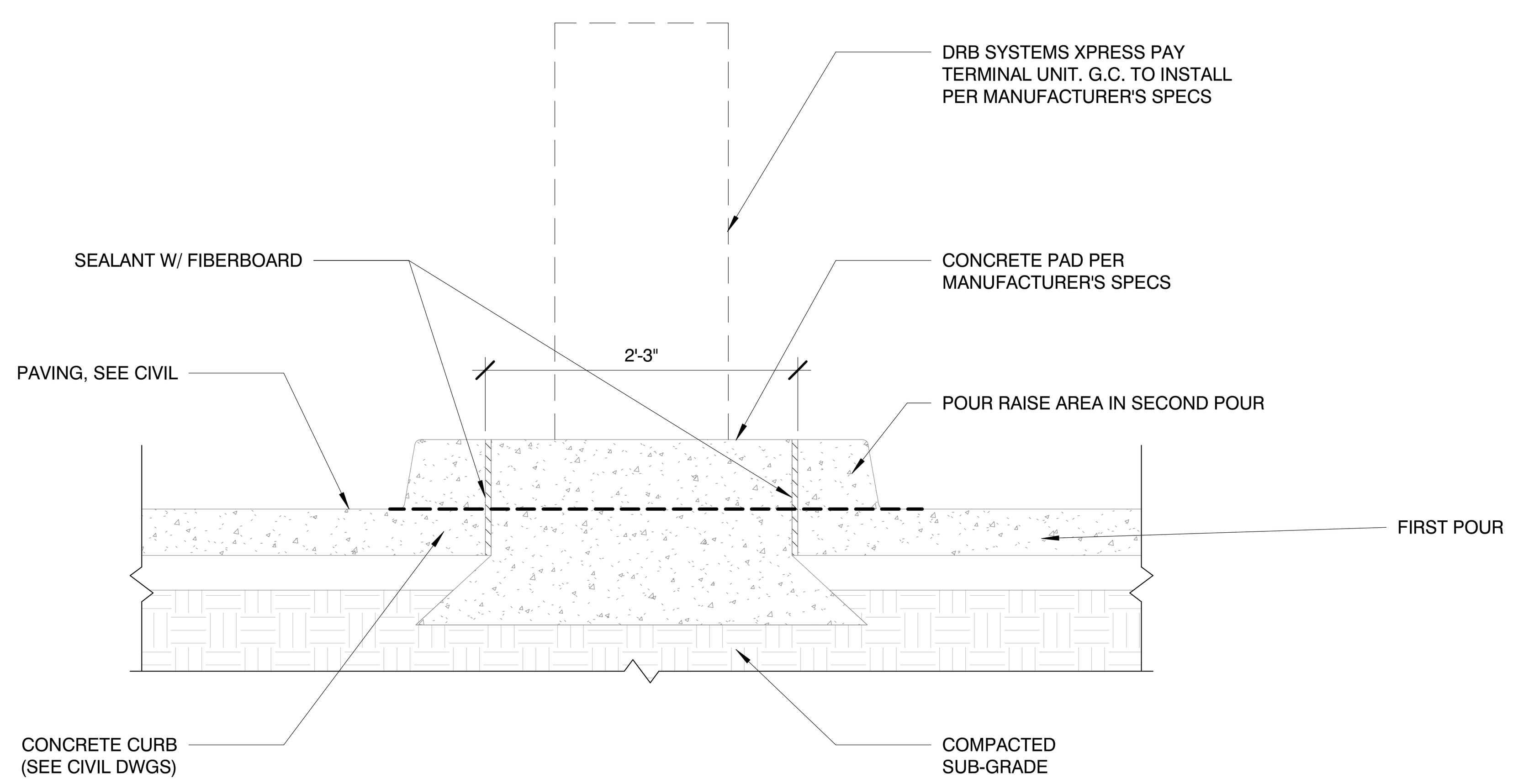


GROUT FILLED CELL. PLACE AND CONSOLIDATE GROUT AS REQUIRED IN STRUCTURAL NOTES.
VERTICAL REINFORCING W/ MATCHING DOWELS IN FOUNDATION. SEE PLAN FOR SIZE AND LOCATION. PROVIDE 48∅LAP SPLICE FOR DOWELS, U.N.O.
HORIZ. HOT DIPPED GALVANIZED 9 GAGE JOINT REINF. @ 16" O.C. STARTING AT FIRST COURSE ABOVE FOUNDATION. LAP SPLICES 8". USE PREFAB. L'S AND T'S AT WALL INTERSECTIONS, LADDER TYPE.
MORTAR CROSS WEBS ADJACENT TO FILLED CELLS.
ASTM C-90 CONCRETE MASONRY UNITS.
CONCRETE TIE BEAM OR MASONRY BOND BEAM. SEE PLAN.
PLACE METAL LATH OR SCREEN OVER CORES NOT TO BE FILLED (SHEET METAL AND FELT PROHIBITED).
SET FIRST COURSE IN FULL MORTAR BEDDING.
CLEAN OUT OPENINGS AT GROUT FILLED CELL LOCATIONS, FOR ALL GROUT LIFTS OVER 5 FEET HIGH.
FOUNDATION.
SEE STRUCTURAL NOTES FOR MAXIMUM GROUT POUR HEIGHT AND ADDITIONAL REQUIREMENTS.

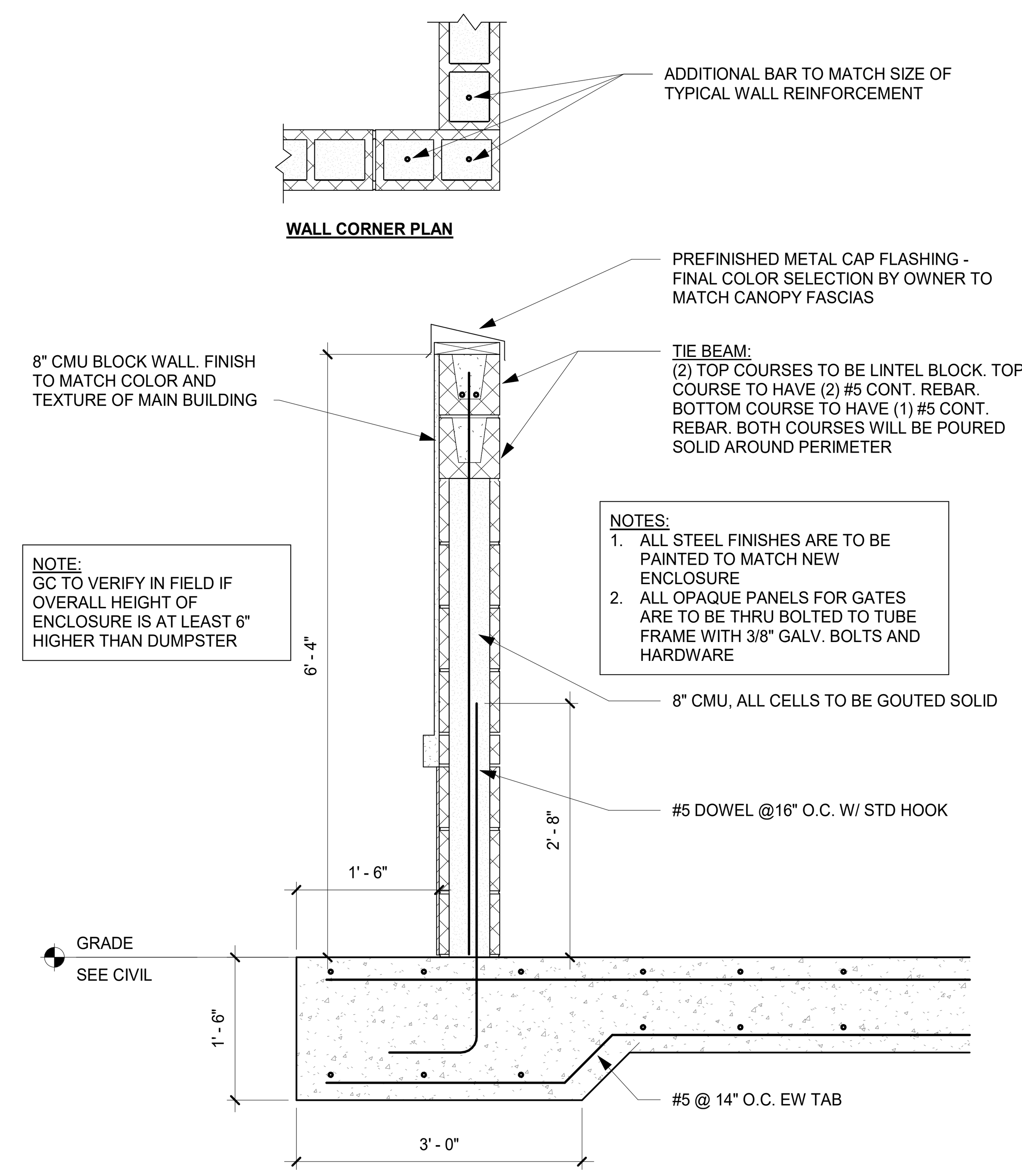
TYPICAL MASONRY WALL CONSTRUCTION

SCALE: NTS



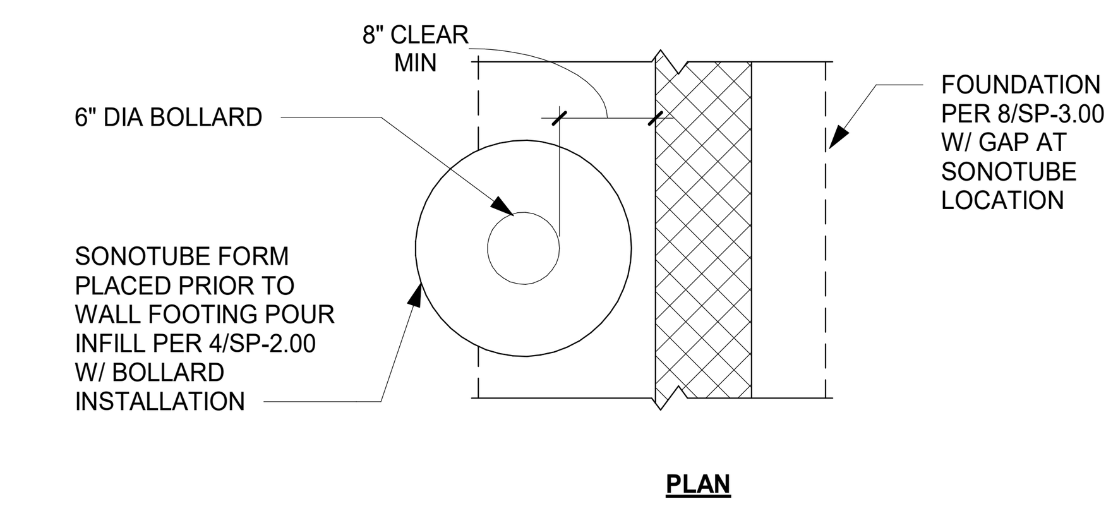
DRB SYSTEMS XPRESS PAY TERMINAL UNIT. G.C. TO INSTALL PER MANUFACTURER'S SPECS
SEALANT W/ FIBERBOARD
CONCRETE PAD PER MANUFACTURER'S SPECS
PAVING, SEE CIVIL
2'-3"
POUR RAISE AREA IN SECOND POUR
FIRST POUR
CONCRETE CURB (SEE CIVIL DWGS)
COMPACTED SUB-GRADE

8 P.O.S. PEDESTAL DETAIL
Scale: 1" = 1'-0"

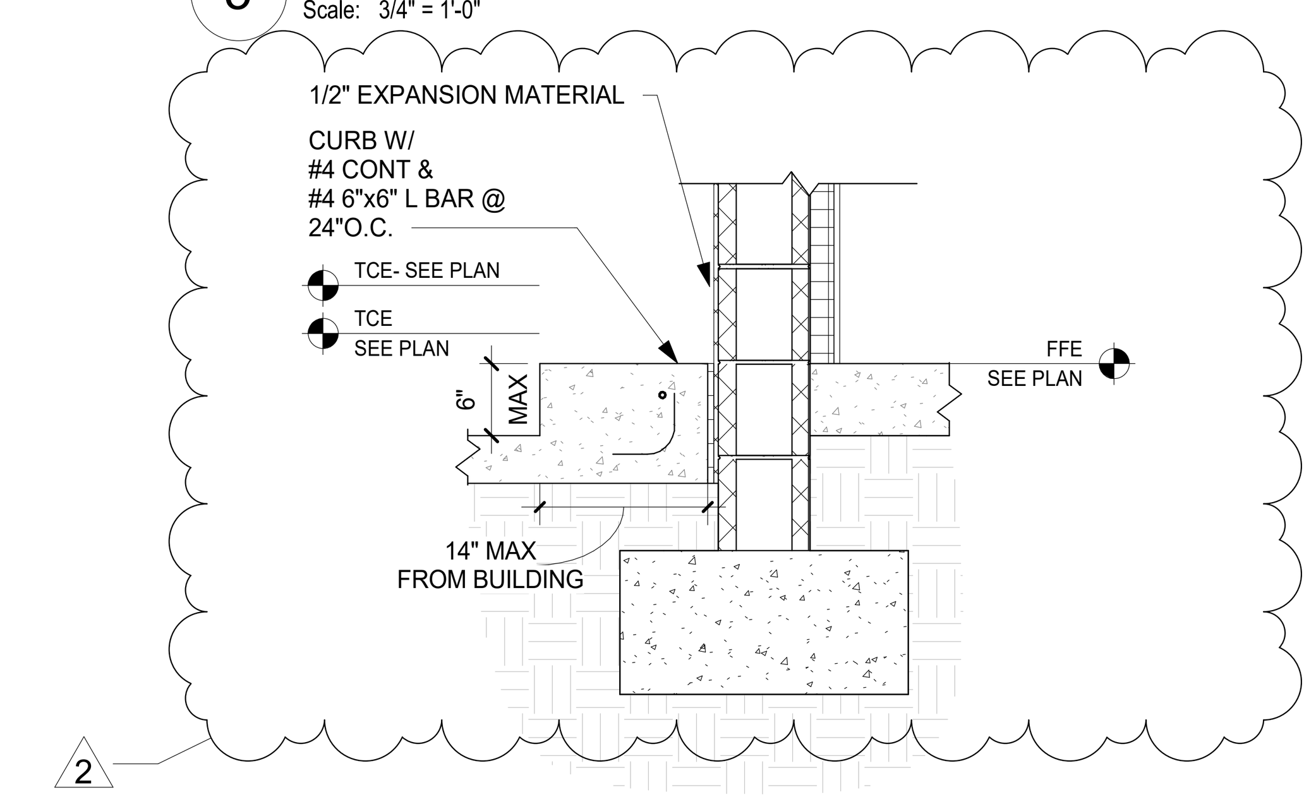


ADDITIONAL BAR TO MATCH SIZE OF TYPICAL WALL REINFORCEMENT
WALL CORNER PLAN
PREFINISHED METAL CAP FLASHING - FINAL COLOR SELECTION BY OWNER TO MATCH CANOPY FASCIAS
8" CMU BLOCK WALL. FINISH TO MATCH COLOR AND TEXTURE OF MAIN BUILDING
TIE BEAM:
(2) TOP COURSES TO BE LINTEL BLOCK. TOP COURSE TO HAVE (2) #5 CONT. REBAR. BOTTOM COURSE TO HAVE (1) #5 CONT. REBAR. BOTH COURSES WILL BE POURED SOLID AROUND PERIMETER
NOTES:
1. ALL STEEL FINISHES ARE TO BE PAINTED TO MATCH NEW ENCLOSURE
2. ALL OPAQUE PANELS FOR GATES ARE TO BE THRU BOLTED TO TUBE FRAME WITH 3/8" GALV. BOLTS AND HARDWARE
NOTE:
GC TO VERIFY IN FIELD IF OVERALL HEIGHT OF ENCLOSURE IS AT LEAST 6" HIGHER THAN DUMPSTER
6' - 4"
8" CMU, ALL CELLS TO BE GOUTED SOLID
2' - 8"
#5 DOWEL @16" O.C. W/ STD HOOK
1' - 6"
GRADE
SEE CIVIL
1' - 6"
#5 @ 14" O.C. EW TAB
3' - 0"

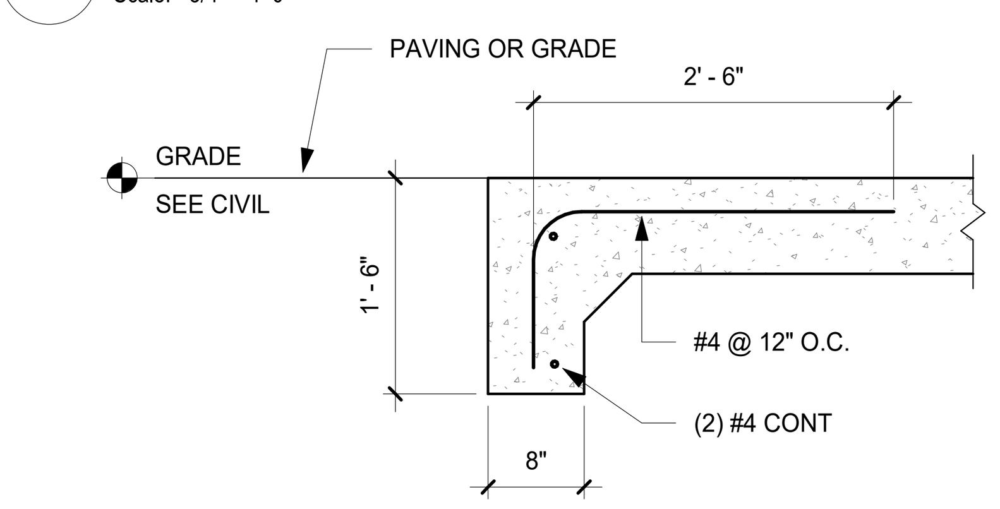
7 TRASH ENCLOUSURE WALL SECTION DETAIL
Scale: 3/4" = 1'-0"



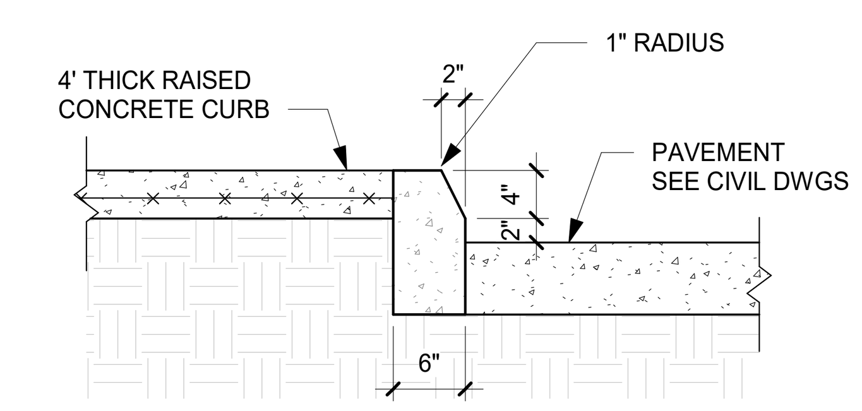
5 DUMPSTER BOLLARD DETAIL
Scale: 3/4" = 1'-0"



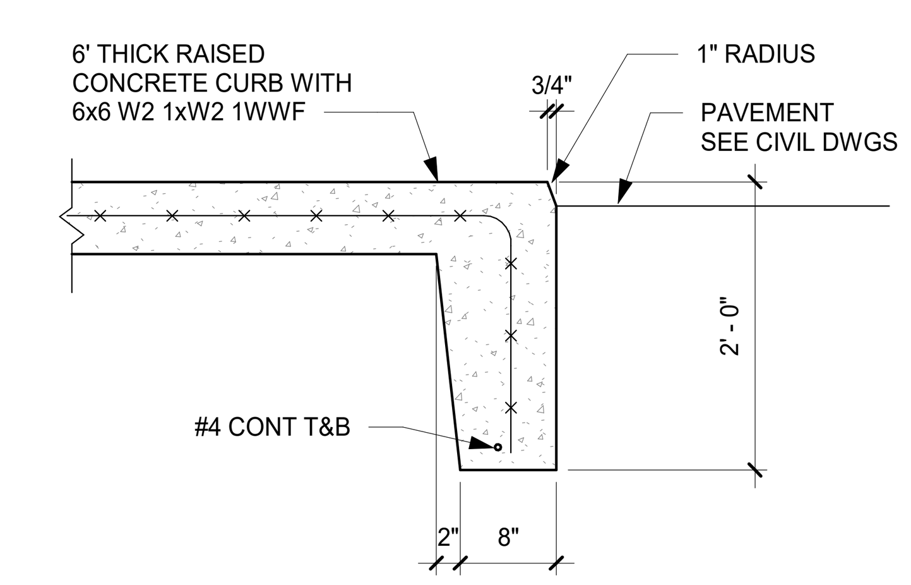
4 TOWER A CURB DETAIL
Scale: 3/4" = 1'-0"



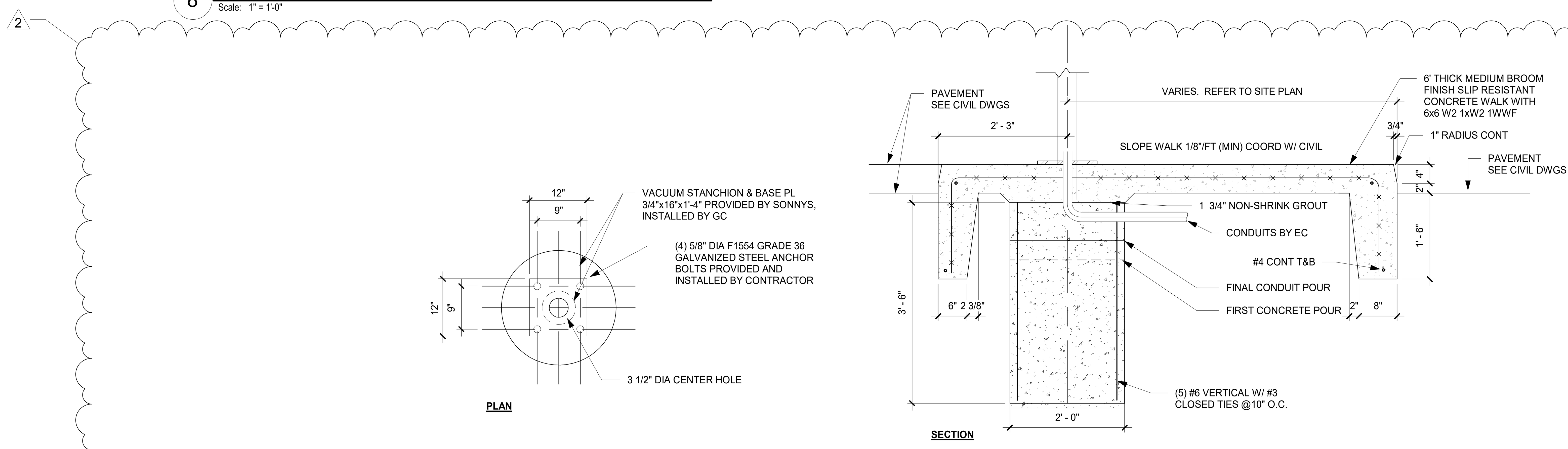
3 TYPICAL STOOP / TURNED DOWN
Scale: 3/4" = 1'-0"



2 RAISED D CURB
Scale: 3/4" = 1'-0"



1 RAISED CURB
Scale: 3/4" = 1'-0"



2
12"
9"
VACUUM STANCHION & BASE PL 3/4"x16"x1'-4" PROVIDED BY SONNYS, INSTALLED BY GC
(4) 5/8" DIA F1554 GRADE 36 GALVANIZED STEEL ANCHOR BOLTS PROVIDED AND INSTALLED BY CONTRACTOR
3 1/2" DIA CENTER HOLE
PLAN
PAVEMENT SEE CIVIL DWGS
VARIES. REFER TO SITE PLAN
6' THICK MEDIUM BROOM FINISH SLIP RESISTANT CONCRETE WALK WITH 6x6 W2 1xW2 1WWF
2' - 3"
3/4"
1" RADIUS CONT
SLOPE WALK 1/8"/FT (MIN) COORD W/ CIVIL
PAVEMENT SEE CIVIL DWGS
4"
2"
1 3/4" NON-SHRINK GROUT
CONDUITS BY EC
1' - 6"
#4 CONT T&B
FINAL CONDUIT POUR
FIRST CONCRETE POUR
3' - 6"
6" 2 3/8"
2"
8"
(5) #6 VERTICAL W/ #3 CLOSED TIES @10" O.C.
2' - 0"
SECTION

6 VACUUM BASE & CURB DETAIL
Scale: 3/4" = 1'-0"

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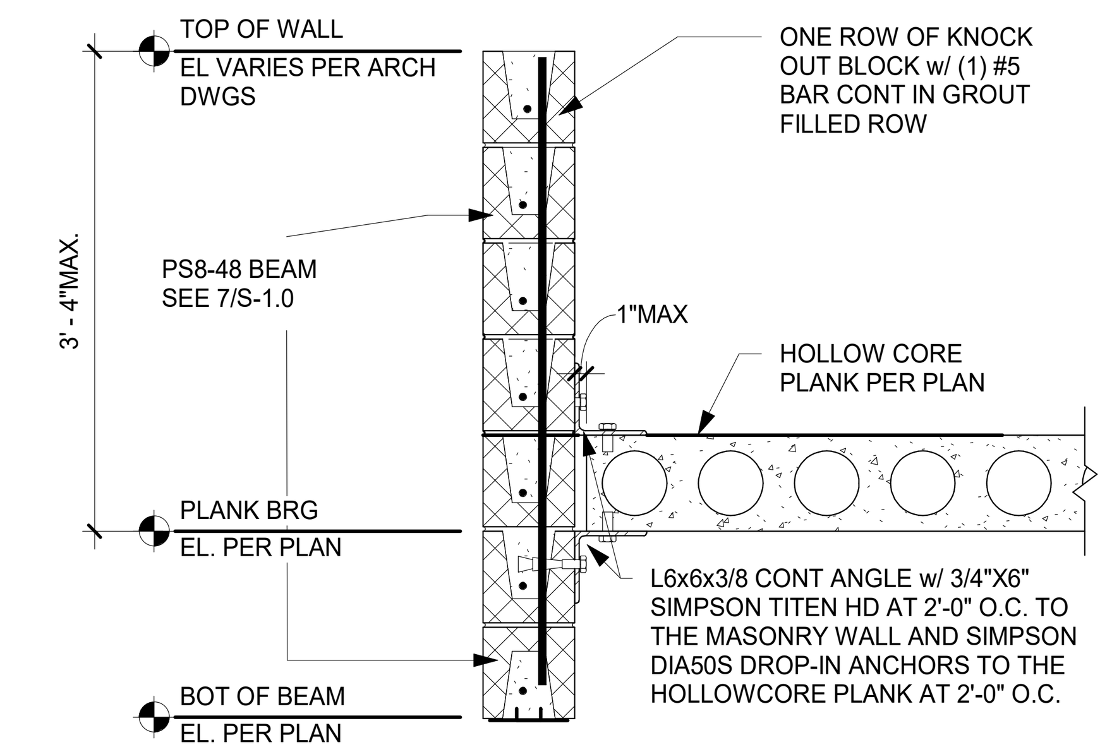
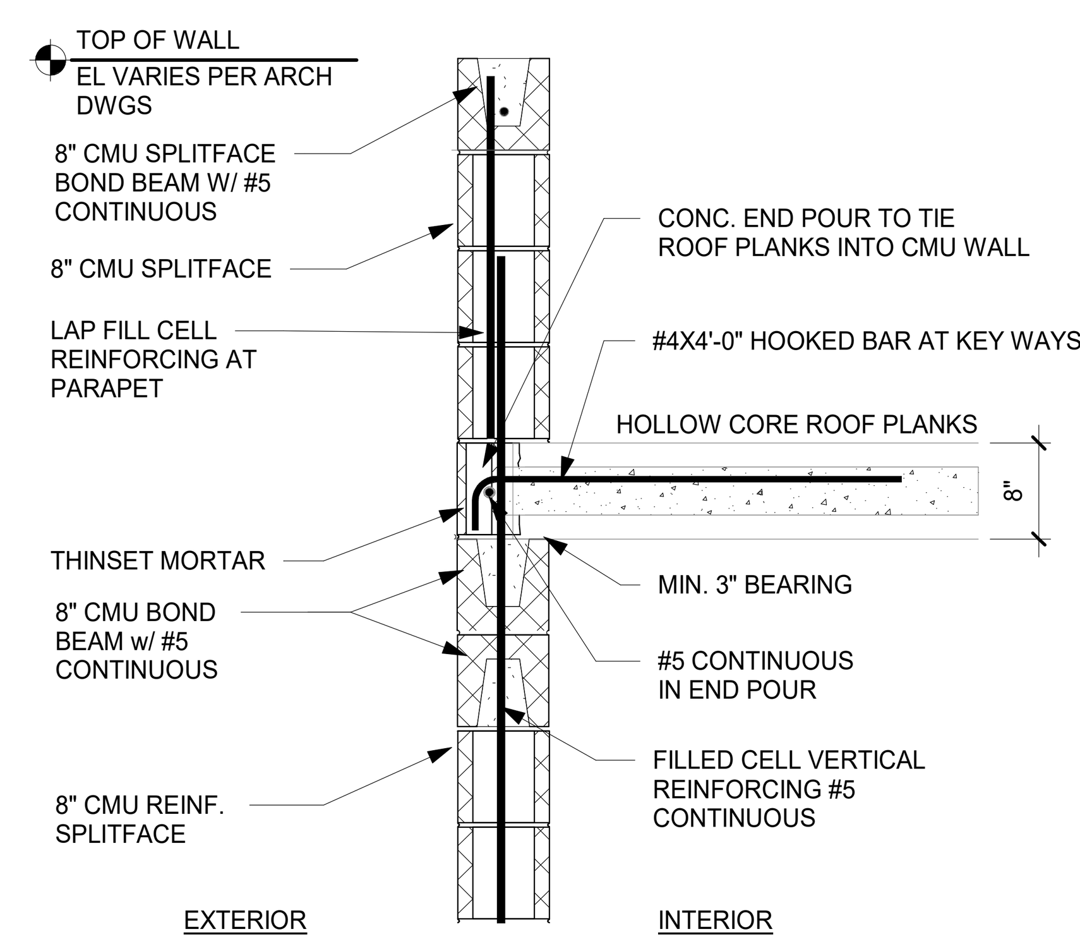


TOP OF WALL
EL VARIES PER ARCH DWGS
ONE ROW OF KNOCK OUT BLOCK w/ (1) #5 BAR CONT IN GROUT FILLED ROW
3' - 4"MAX.
PS8-48 BEAM SEE 7/S-1.0
1"MAX
HOLLOW CORE PLANK PER PLAN
PLANK BRG
EL. PER PLAN
L6x6x3/8 CONT ANGLE w/ 3/4"X6" SIMPSON TITEN HD AT 2'-0" O.C. TO THE MASONRY WALL AND SIMPSON DIA50S DROP-IN ANCHORS TO THE HOLLOWCORE PLANK AT 2'-0" O.C.
BOT OF BEAM
EL. PER PLAN

3 PLANK EDGE BEARING ON BEAM

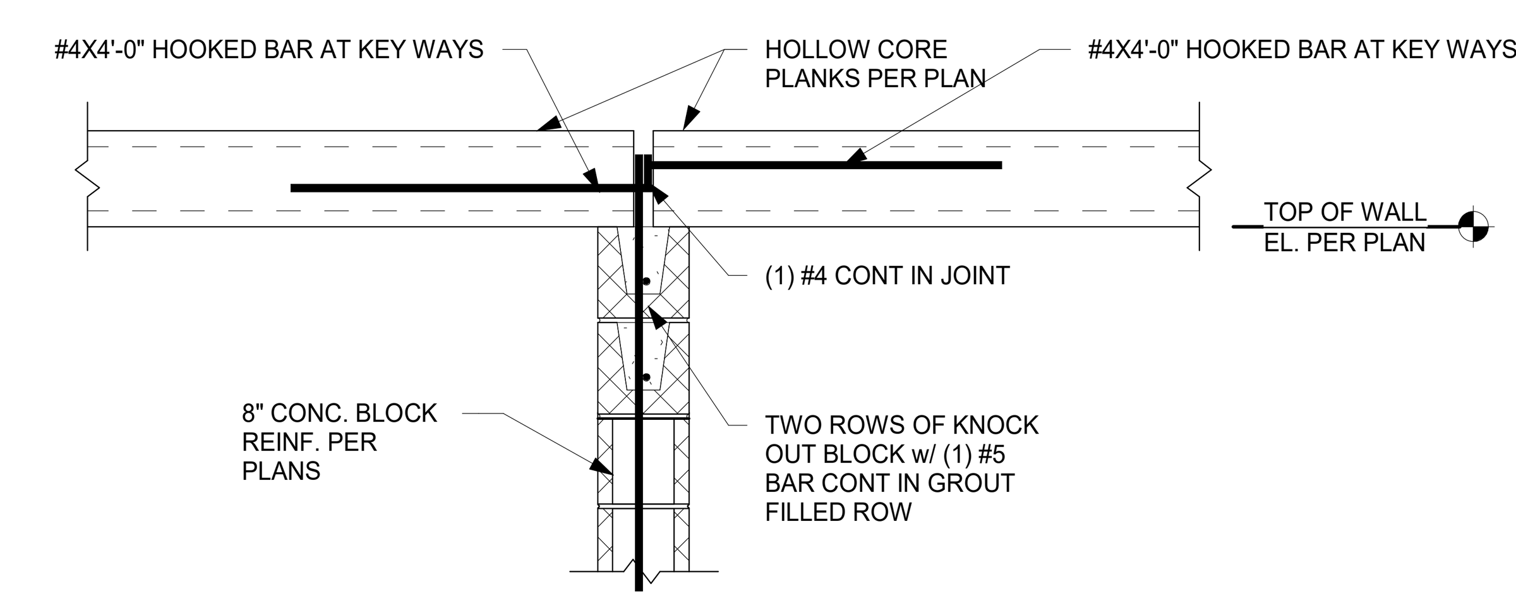
SCALE: 3/4" = 1'-0"



TOP OF WALL
EL VARIES PER ARCH DWGS
8" CMU SPLITFACE BOND BEAM W/ #5 CONTINUOUS
8" CMU SPLITFACE
LAP FILL CELL REINFORCING AT PARAPET
CONC. END POUR TO TIE ROOF PLANKS INTO CMU WALL
#4X4'-0" HOOKED BAR AT KEY WAYS
HOLLOW CORE ROOF PLANKS
8"
THINSET MORTAR
MIN. 3" BEARING
8" CMU BOND BEAM w/ #5 CONTINUOUS
#5 CONTINUOUS IN END POUR
FILLED CELL VERTICAL REINFORCING #5 CONTINUOUS
8" CMU REINF. SPLITFACE
EXTERIOR
INTERIOR

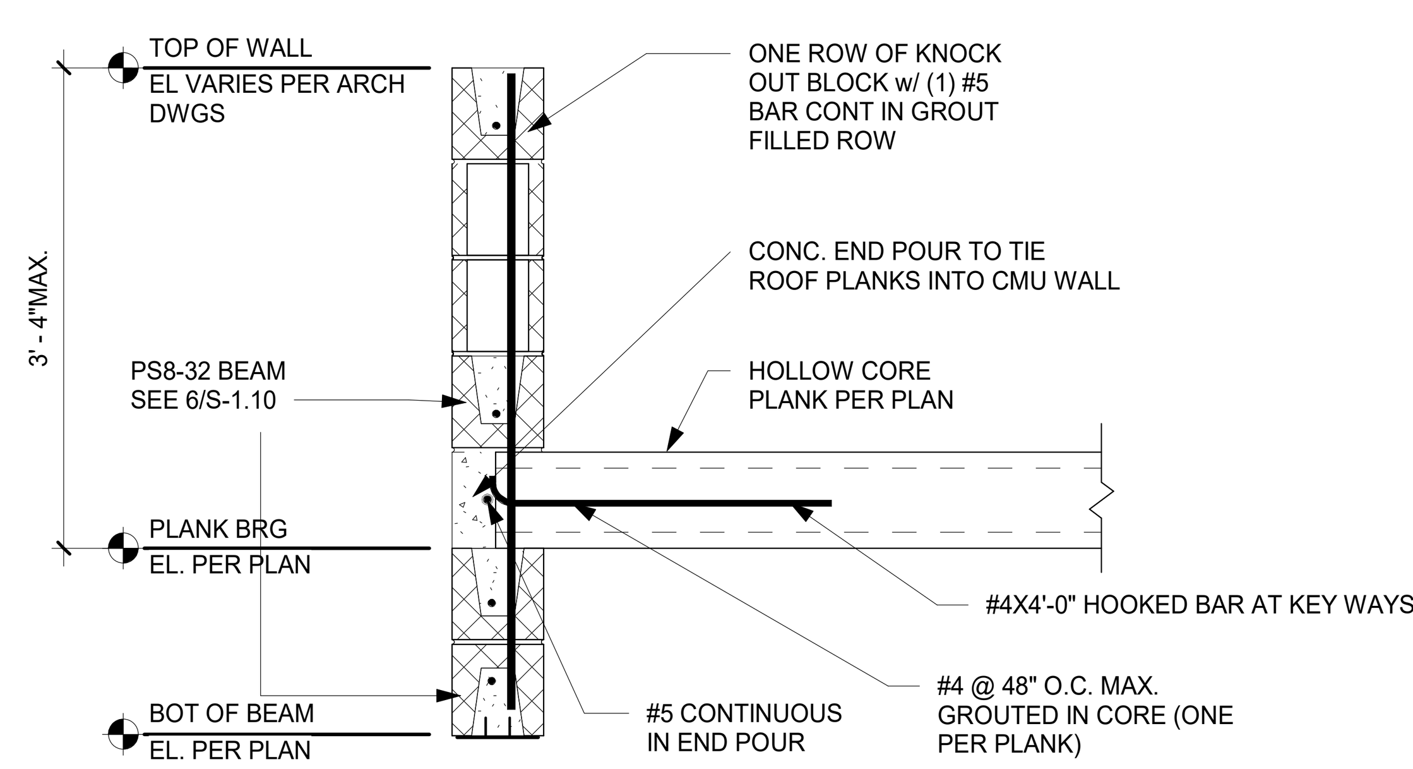
5 PLANK BEARING ON WALL

SCALE: 3/4" = 1'-0"



2 PLANK BEARING ON INTERIOR WALL

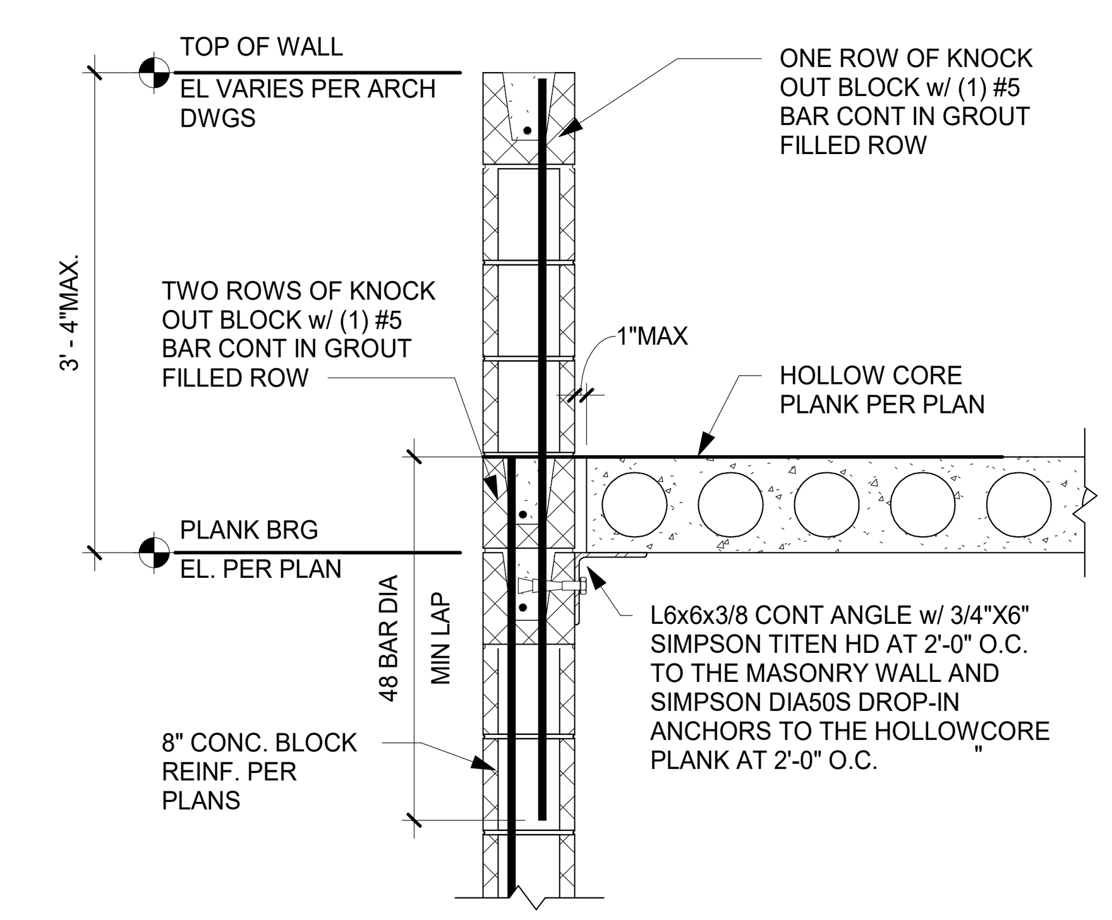
SCALE: 3/4" = 1'-0"



TOP OF WALL
EL VARIES PER ARCH DWGS
ONE ROW OF KNOCK OUT BLOCK w/ (1) #5 BAR CONT IN GROUT FILLED ROW
CONC. END POUR TO TIE ROOF PLANKS INTO CMU WALL
3' - 4"MAX.
PS8-32 BEAM SEE 6/S-1.10
HOLLOW CORE PLANK PER PLAN
PLANK BRG
EL. PER PLAN
#4X4'-0" HOOKED BAR AT KEY WAYS
#4 @ 48" O.C. MAX. GROUTED IN CORE (ONE PER PLANK)
BOT OF BEAM
EL. PER PLAN
#5 CONTINUOUS IN END POUR

4 PLANK BEARING ON BEAM

SCALE: 3/4" = 1'-0"



TOP OF WALL
EL VARIES PER ARCH DWGS
ONE ROW OF KNOCK OUT BLOCK w/ (1) #5 BAR CONT IN GROUT FILLED ROW
3' - 4"MAX.
TWO ROWS OF KNOCK OUT BLOCK w/ (1) #5 BAR CONT IN GROUT FILLED ROW
1"MAX
HOLLOW CORE PLANK PER PLAN
PLANK BRG
EL. PER PLAN
48 BAR DIA
MIN LAP
L6x6x3/8 CONT ANGLE w/ 3/4"X6" SIMPSON TITEN HD AT 2'-0" O.C. TO THE MASONRY WALL AND SIMPSON DIA50S DROP-IN ANCHORS TO THE HOLLOWCORE PLANK AT 2'-0" O.C.
8" CONC. BLOCK REINF. PER PLANS

1 PLANK EDGE BEARING ON WALL

SCALE: 3/4" = 1'-0"

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Issue Dates

Revisions

ROOF FRAMING DETAILS

Scale : 3/4" = 1'-0"

Plot Date
2/13/2024 5:25:21 PM

Project No.
21049

Drawn By
DC / CT

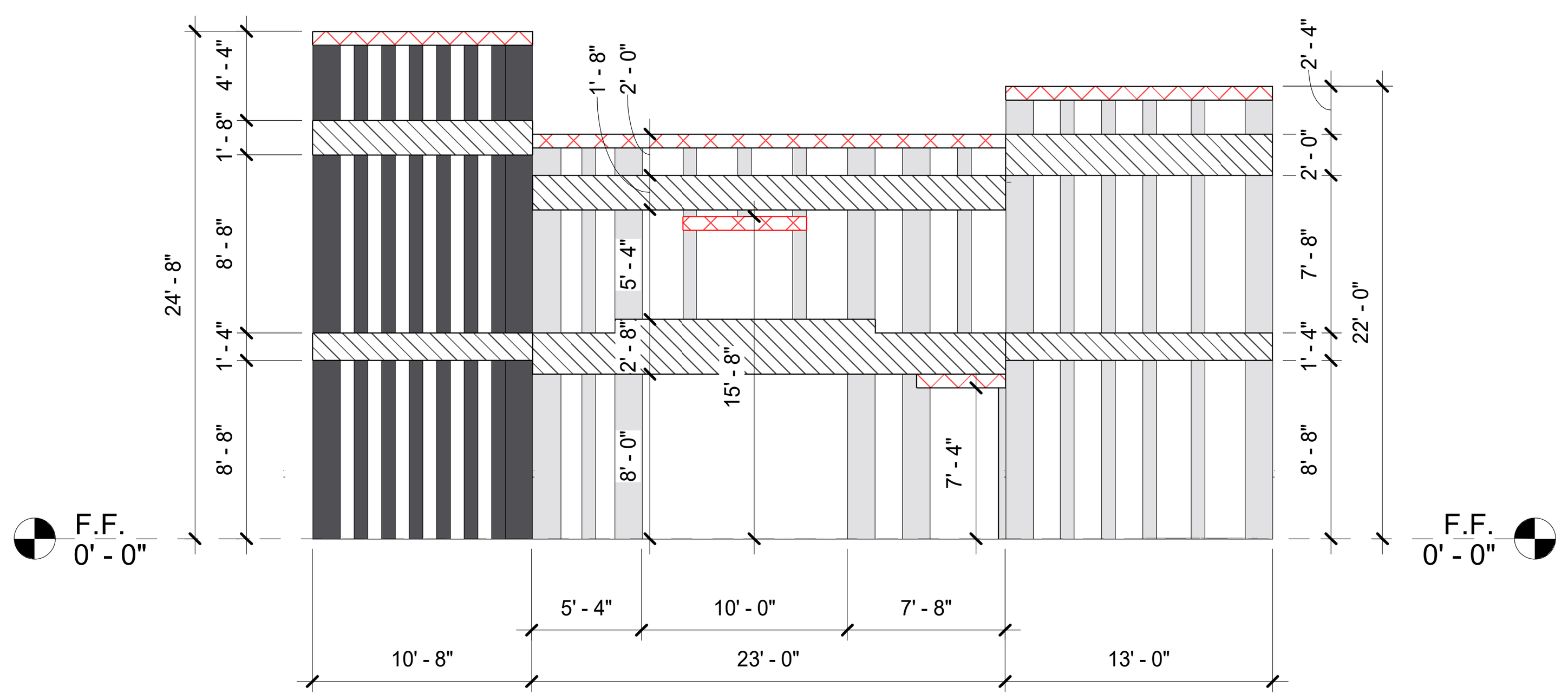
S-5.00

STATE OF FLORIDA
THOMAS R WALTER
7305
REGISTERED ARCHITECT

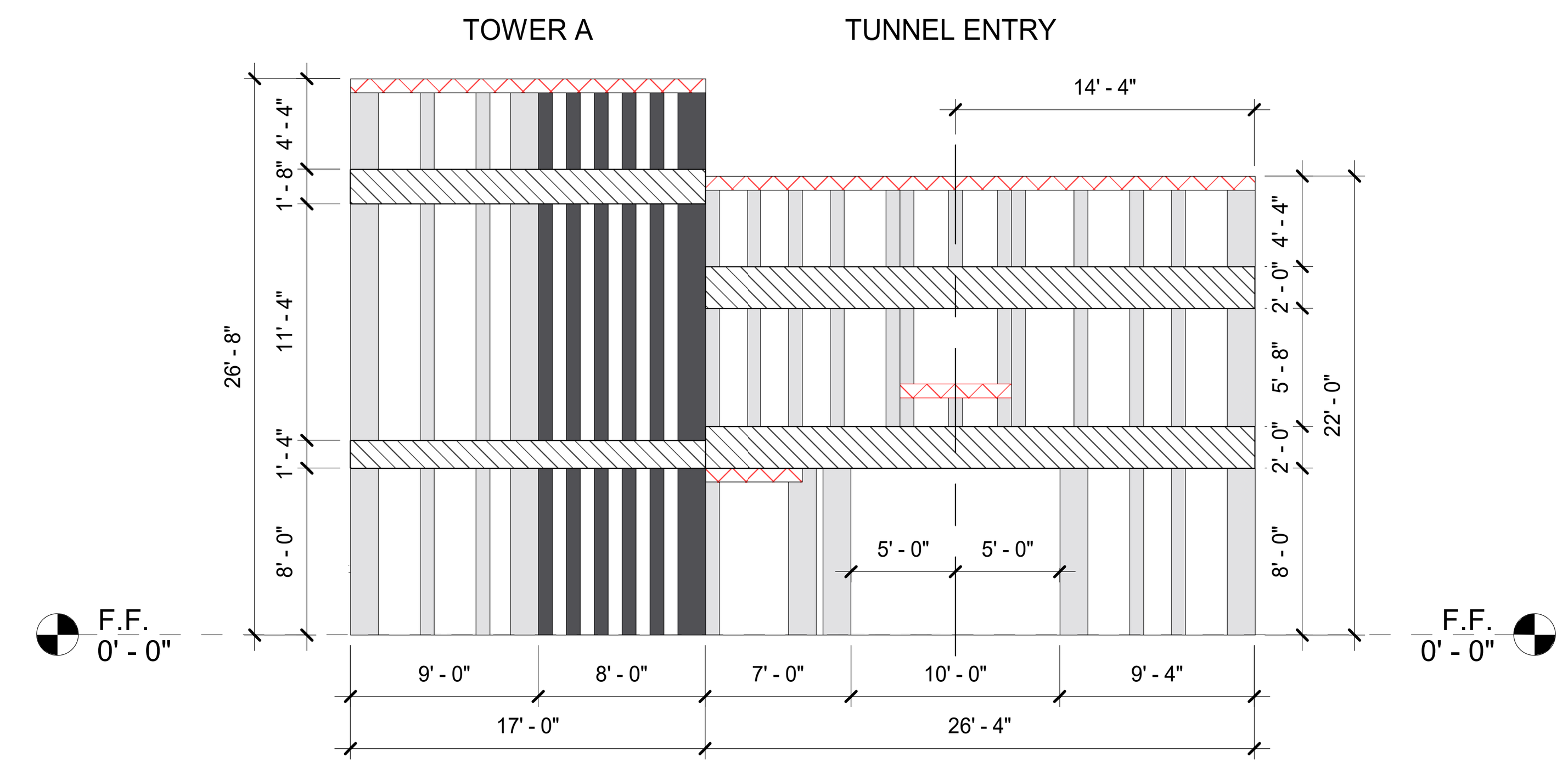
[illegible]

Date of Issue
07/21/2023

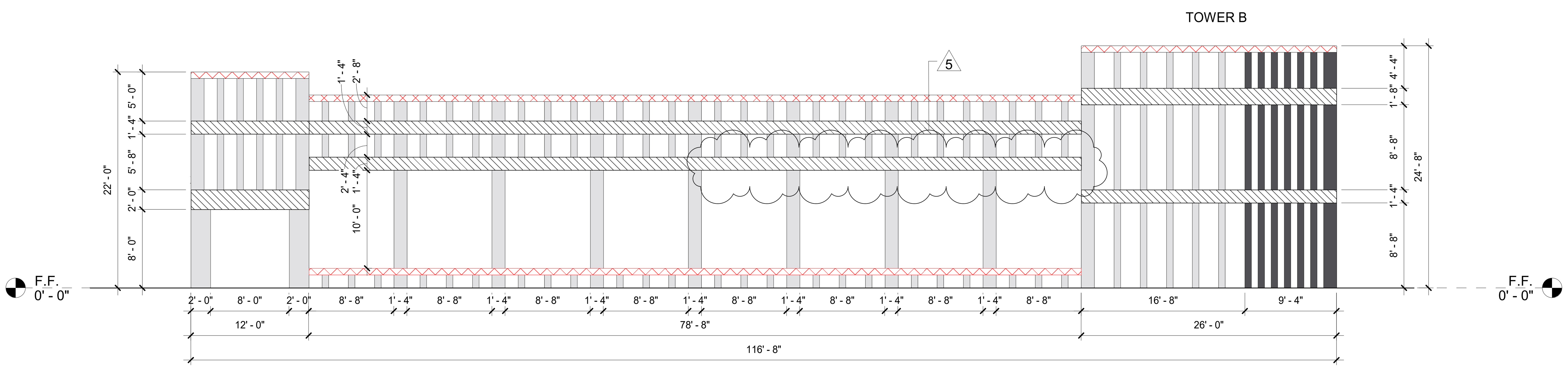
IF PLOTTED AT 11" X 17", SCALE IS ONE HALF AS NOTED.



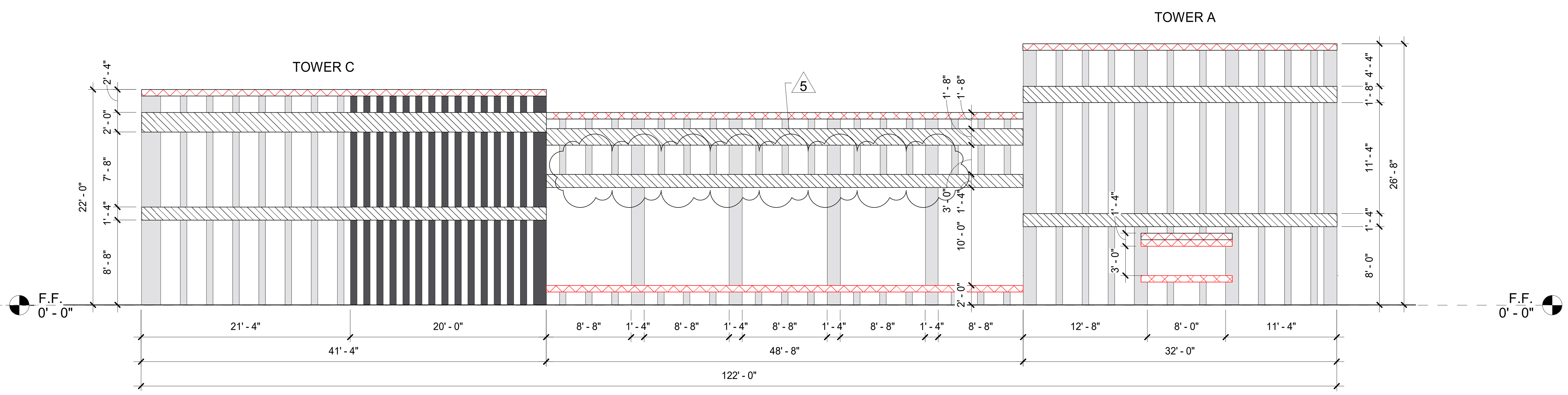
4 STRUCTURAL ELEVATION - SOUTH
Scale: 1/8" = 1'-0"



3 STRUCTURAL ELEVATION - NORTH
Scale: 1/8" = 1'-0"



2 STRUCTURAL ELEVATION - WEST
Scale: 1/8" = 1'-0"



1 STRUCTURAL ELEVATION - EAST
Scale: 1/8" = 1'-0"

▨	LINTEL
▨	BOND BEAM
⬜	FILLED IN CELLS WITH REBAR
⬛	FILLED IN CELLS WITH 2#5 16" O.C.

C:\Users\John.sikora\Documents\RAINTREE ECOWASH - 22013 - 160 Candace Dr - R23_John.S-pwg.net.rvt

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ZOOM EXPRESS CAR WASH

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Maitland, FL 32751

Issue Dates		
Revisions		
2	12/07/23	VE Changes
5	01/31/24	Design Coordination Revision

ELEVATIONS

Scale : As indicated

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Author

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