

STRUCTURAL NOTES

GENERAL NOTES:

- THESE NOTES SHALL APPLY EXCEPT WHERE OTHERWISE INDICATED BY DRAWINGS OR SPECIFICATIONS.
- CONTRACT DOCUMENTS INCLUDE THE STRUCTURAL DRAWINGS AND SPECIFICATIONS BUT DO NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS OR OTHER SUBMITTALS BY THE CONTRACTOR.
- REFERENCE TO DESIGN STANDARDS AND BUILDING CODES SHALL MEAN THE REFERENCE NOTED ON THE DRAWINGS UNLESS SPECIFICALLY STATED OTHERWISE.
- THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS AND NOTIFY ARCHITECT OF ANY DISCREPANCIES PRIOR TO PROCEEDING WITH WORK. FOR DIMENSIONS NOT SHOWN ON STRUCTURAL DRAWINGS, SEE ARCHITECTURAL DRAWINGS.
- ALL MATERIALS AND WORKMANSHIP ARE SUBJECT TO THE REVIEW OF THE ARCHITECT AND ENGINEER OF RECORD.
- STRUCTURAL DRAWINGS INDICATE TYPICAL AND CERTAIN SPECIFIC CONDITIONS ONLY. SHOP DRAWINGS SHALL DETAIL ALL CONDITIONS IN ACCORDANCE WITH SPECIFIED STANDARDS AND THE SPECIFIC REQUIREMENTS OF THIS PROJECT AS INDICATED ON THE DRAWINGS.
- COORDINATE WITH OTHER DISCIPLINE DRAWINGS FOR DRIPS, CHAMFERS, REGLETS, RUSTICATIONS, SLOTS, SLEEVES, ANCHORS AND INSERTS.
- THE GENERAL CONTRACTOR SHALL COORDINATE ALL SIZES AND LOCATIONS OF ROOF PENETRATIONS WITH MECHANICAL AND ARCHITECTURAL DRAWINGS. OPENINGS IN METAL ROOF DECK GREATER THAN 10" SHALL BE FRAMED AS SHOWN IN ROOF OPENING FRAMING DETAIL.
- UNLESS SHOWN ON STRUCTURAL DRAWINGS, NO OPENINGS LARGER THAN 12" x 12" SHALL BE PLACED IN SLABS OR WALLS. FOR OPENINGS NOT SHOWN ON STRUCTURAL DRAWINGS, APPROVAL MUST BE OBTAINED FROM THE ARCHITECT PRIOR TO CONSTRUCTION OF OPENING.
- THE CONTRACTOR HAS SOLE RESPONSIBILITY FOR THE MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES OF CONSTRUCTION.
- THE GENERAL CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH REQUIRED OSHA REGULATIONS.
- THE STRUCTURE DESCRIBED BY THESE DRAWINGS IS SELF SUPPORTING ONLY IN ITS COMPLETED FORM. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF TEMPORARY BRACING AND SHORING OF ALL WORK.
- THE GENERAL CONTRACTOR SHALL COORDINATE WITH THE BUILDING OFFICIAL REGARDING SUBMITTAL OF INSPECTION REPORTS TO THE BUILDING DEPARTMENT.
- REVIEW OF SUBMITTALS AND SHOP DRAWINGS BY THE ARCHITECT AND STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL ENGINEER. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.
- THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR THE DESIGN OF STEEL STAIRS, HANDRAILS, LADDERS, CURTAIN OR WINDOW WALL SYSTEMS, RACK STORAGE SYSTEMS, COLD-FORMED METAL FRAMING, OR OTHER SYSTEMS NOT SHOWN IN THE STRUCTURAL DOCUMENTS. SUCH SYSTEMS SHALL BE DESIGNED, FURNISHED AND INSTALLED AS REQUIRED BY OTHER PORTIONS OF THE CONTRACT DOCUMENTS.
- DO NOT HANG OR ATTACH DUCTWORK, PIPING, LIGHTING, CONDUIT, EQUIPMENT, CEILINGS, ETC. FROM METAL DECKING.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR SUPPORTING AND MAINTAINING THE EXCAVATIONS REQUIRED FOR THE CONSTRUCTION SHOWN.
- JOHNSTON BURKHOLDER ASSOCIATES, LLC, ITS EMPLOYEES AND REPRESENTATIVES SHALL NOT BE RESPONSIBLE FOR, AND WILL NOT HAVE CONTROL OF, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, OR FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE CONSTRUCTION WORK; NOR WILL THEY BE RESPONSIBLE FOR ANY FAILURE BY THE CONTRACTOR TO PERFORM OR COMPLETE CONSTRUCTION IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. JOBSITE VISITS BY THE STRUCTURAL ENGINEER OF RECORD SHALL NOT CONSTITUTE APPROVAL, AWARENESS OR LIABILITY FOR ANY HAZARDOUS CONDITIONS.

DESIGN CRITERIA:

- STRUCTURAL DRAWINGS HAVE BEEN PREPARED IN ACCORDANCE WITH THE 2020 FLORIDA BUILDING CODE.
- LIVE LOADS:

ROOF LOAD	20 PSF
ACCESS PLATFORMS	40 PSF
FLOOR SLAB-ON-GRADE	100 PSF
- DEAD LOADS:

ROOF LOAD	25 PSF
ACCESS PLATFORMS	10 PSF
- WIND LOADS:

ULTIMATE WIND SPEED, Vult (3 sec. GUST)	154 MPH
NOMINAL WIND SPEED, Vasd (3 sec. GUST)	119 MPH
RISK CATEGORY	II
WIND EXPOSURE	C
INTERNAL PRESSURE COEFFICIENT (ENCLOSED)	± 0.18
- SEISMIC LOADS:

SEISMIC IMPORTANCE FACTOR	1.0
RISK CATEGORY	II
MAPPED ACCELERATIONS:	
Ss	0.052g
S1	0.029g
SITE CLASS	D
SPECTRAL RESPONSE COEF:	
Sds	0.055g
Sd1	0.047g
SEISMIC DESIGN CATEGORY	A
RESPONSE MODIFICATION FACTOR (R)	3.0
SEISMIC FORCE RESISTING SYSTEM	STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
ANALYSIS PROCEDURE:	EQUIVALENT LATERAL FORCE
- SNOW LOADS:

GROUND SNOW LOAD (Pg)	0 PSF
SNOW EXPOSURE FACTOR (Ce)	1.0
THERMAL FACTOR (Ct)	1.0
SNOW IMPORTANCE FACTOR	1.0
FLAT ROOF SNOW LOAD (Pt), DRIFTING ADDITIONAL	0 PSF

FOUNDATION NOTES:

- A SITE INVESTIGATION AND GEOTECHNICAL REPORT WAS PREPARED BY BECHTOL ENGINEERING AND TESTING, INC., DATED 09/19/19. THE GENERAL CONTRACTOR SHALL READ AND BECOME FAMILIAR WITH THE REPORT PRIOR TO BIDDING. ALL SITE WORK SHALL CONFORM TO SOILS REPORT AND/OR SPECIFICATIONS.
- ALL FOUNDATIONS ARE DESIGNED TO BEAR ON NATURALLY OCCURRING SOILS OR ENGINEERED FILL CAPABLE OF SAFELY SUSTAINING 2000 PSF MINIMUM NET ALLOWABLE BEARING PRESSURE. IF SUITABLE BEARING CAPACITY AS DETERMINED BY A QUALIFIED GEOTECHNICAL ENGINEER IS NOT ENCOUNTERED AT THE ELEVATION INDICATED ON THE DRAWINGS, THE GENERAL CONTRACTOR MAY, UPON THE RECOMMENDATION OF THE GEOTECHNICAL ENGINEER, OVER-EXCAVATE UNTIL SUITABLE BEARING MATERIAL IS ENCOUNTERED. OVER-EXCAVATIONS MAY BE BACKFILLED WITH LEAN CONCRETE.
- CONVENTIONAL EXTERIOR FOUNDATIONS SHALL HAVE A MINIMUM BEARING DEPTH OF 1'-8" BELOW EXTERIOR GRADE ELEVATION. CONTINUOUS OR TIEKED SLAB EXTERIOR FOUNDATIONS SHALL HAVE A MINIMUM BEARING DEPTH OF 1'-0" BELOW EXTERIOR GRADE ELEVATION.
- PRIOR TO PLACING CONCRETE, ALL DEBRIS, WATER, AND LOOSE EARTH SHALL BE REMOVED FROM THE FOUNDATION BED.
- COLUMN FOOTINGS AND WALL FOOTINGS SHALL BE PLACED MONOLITHICALLY WITH ADJACENT FOOTINGS AT THE SAME ELEVATION.
- FOUNDATION WALLS THAT RETAIN EARTH SHALL BE BRACED AGAINST BACKFILLING PRESSURES UNTIL THE FLOOR SLABS AT THE TOP AND BOTTOM OF WALL ARE IN PLACE AND CURED.
- BACKFILL AGAINST WALLS SHALL NOT BE PLACED UNTIL CONCRETE HAS CURED FOR 7 DAYS. BACKFILL SHALL BE DEPOSITED EVENLY AGAINST BOTH SIDES OF WALLS UNTIL THE LOWER FINAL GRADE IS REACHED. COMPACTION OF BACKFILL WITHIN 10 FEET OF WALLS SHOULD BE PERFORMED WITH HAND OPERATED EQUIPMENT.
- PLACEMENT AND COMPACTION OF STRUCTURAL FILL SHALL BE MONITORED BY THE GEOTECHNICAL ENGINEER. MINIMUM COMPACTION REQUIREMENTS STATED IN GEOTECHNICAL REPORT SHALL BE ACHIEVED.
- ALL DRAIN TILE SHALL BE 4" DIA. (MIN) PERFORATED PLASTIC PIPE WITH FABRIC SOCK AND 12" MINIMUM WASHED RIVER ROCK SURROUNDING. DO NOT USE CRUSHED OR FINE GRAVEL.
- WHERE A UTILITY LINE PASSES UNDER A FOOTING, PROVIDE A STEEL OR CAST-IRON SLEEVE WITH MINIMUM 2" CLEAR ON ALL SIDES OF PIPE.

CONCRETE NOTES:

- ALL CONCRETE AND REINFORCEMENT HAS BEEN DESIGNED IN ACCORDANCE WITH THE AMERICAN CONCRETE INSTITUTE (ACI) STANDARD BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318). ALL CONCRETE WORK SHALL CONFORM TO THE ACI SPECIFICATIONS FOR STRUCTURAL CONCRETE (ACI 301) AND THE RECOMMENDATIONS OF THE ACI MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE (ACI 315).
- CONCRETE PROPORTIONS SHALL CONSIST OF THE FOLLOWING MATERIALS TO ACHIEVE A MIX THAT WILL YIELD A FINISHED PRODUCT THAT WILL PRODUCE THE REQUIRED COMPRESSIVE STRENGTHS LISTED IN NOTE #3 BELOW.
 - CEMENTITIOUS MATERIALS:
 - PORTLAND CEMENT, ASTM C150, TYPE I OR II.
 - FLY ASH AND SLAG CEMENT ARE NOT ALLOWED.
 - AGGREGATES: NOMINAL WEIGHT, ASTM C33; LIGHT WEIGHT, ASTM C330. PROVIDE ALL AGGREGATE FROM A SINGLE SOURCE.
 - WATER: ASTM C94, POTABLE.
 - AIR ENTRAINMENT ADMIXTURE: ASTM C260.
 - WATER REDUCING ADMIXTURE: ASTM C494, TYPE F OR G.
 - CALCIUM CHLORIDE: ANY ADMIXTURE CONTAINING MORE THAN 0.1% CHLORIDE IONS, CONTENT BY WEIGHT, ARE NOT PERMITTED.
- CONCRETE MIX DESIGNS MUST BE SUBMITTED TO THE E.O.R. FOR APPROVAL BEFORE CONSTRUCTION BEGINS. MIX DESIGNS MUST BE SUBMITTED WITH COMPRESSIVE STRENGTH INFORMATION. FOR THAT SPECIFIC MIX, SUBMISSIONS WITHOUT THIS INFORMATION WILL BE STAMPED "REJECTED" & RETURNED TO THE CONTRACTOR WITHOUT E.O.R. REVIEW. TEST DATA SHALL BE NO OLDER THAN 12 MONTHS. ALL CONCRETE COMPRESSIVE STRENGTH TESTS SHALL BE AVAILABLE ON THE JOB SITE FOR REVIEW BY THE INSPECTOR.
- ALL CONCRETE USED IN THE WORK SHALL HAVE THE FOLLOWING PROPERTIES:
 - FOOTINGS
 - 3000 PSI STRENGTH (fc) AT 28 DAYS
 - 0.50 MAXIMUM WATER/CEMENT RATIO
 - LESS THAN 3% AIR CONTENT
 - 4" ± 1" SLUMP AT POINT OF PLACEMENT
 - FOUNDATION WALLS
 - 4000 PSI STRENGTH (fc) AT 28 DAYS
 - 0.50 MAXIMUM WATER/CEMENT RATIO
 - LESS THAN 3% AIR CONTENT
 - 4" ± 1" SLUMP AT POINT OF PLACEMENT
 - INTERIOR SLABS-ON-GRADE
 - 4000 PSI STRENGTH (fc) AT 28 DAYS
 - 0.53 MAXIMUM WATER/CEMENT RATIO
 - LESS THAN 3% AIR CONTENT
 - 4" ± 1" SLUMP AT POINT OF PLACEMENT
 - EXTERIOR SLABS-ON-GRADE
 - 4000 PSI STRENGTH (fc) AT 28 DAYS
 - 0.50 MAXIMUM WATER/CEMENT RATIO
 - 4" ± 1" SLUMP AT POINT OF PLACEMENT
- SLAB ON GRADE CONTROL OR CONSTRUCTION JOINTS SHALL BE LOCATED AT COLUMN LINES AND INTERMEDIATELY SO THAT NO JOINT SPACING SHALL EXCEED 12'-0". NOIR SHALL THE LENGTH OF ANY PANEL EXCEED ONE AND A HALF (1.5) TIMES THE WIDTH OF THAT PANEL. REFER TO TYPICAL DETAILS FOR SLAB JOINT CONSTRUCTION.
- FLOOR SLAB CONSTRUCTION JOINTS MUST BE DOWELED.
- EXTERIOR SLABS SHALL DRAIN FREELY AWAY FROM THE BUILDING. COORDINATE ELEVATIONS WITH CIVIL ENGINEER AND ARCHITECT.
- REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SLAB FINISHES, SLOPES AND DEPRESSIONS OF INTERIOR SLABS.

REINFORCING STEEL NOTES:

- REINFORCING STEEL SHALL BE DETAILED, FABRICATED AND PLACED ACCORDING TO THE STANDARDS OF THE AMERICAN CONCRETE INSTITUTE (ACI) AND THE CONCRETE REINFORCING STEEL INSTITUTE (CRSI).
- MATERIALS:
 - REINFORCING BARS SHALL COMPLY WITH ASTM A615 GRADE 60.
 - WELDED WIRE FABRIC SHALL COMPLY WITH ASTM A82 AND A1064, FLAT SHEETS ONLY. REINFORCING BARS FOR WELDING SHALL COMPLY WITH ASTM A706.
- CLEAR MINIMUM COVER OF CONCRETE OVER REINFORCING BARS SHALL BE AS FOLLOWS:

CONCRETE PLACED AGAINST EARTH	3"
FORMED SURFACES EXPOSED TO EARTH	2"
TOP OF SLABS	3/4"
- ALL BARS DENOTED CONTINUOUS ON PLANS, SECTIONS AND DETAILS SHALL HAVE CLASS "B" TENSION SPlice LAPS AND CORNER BARS AND HOOKS AT DISCONTINUOUS ENDS. SPliced BARS SHALL BE SECURELY WIDED TOGETHER. SPICES OF ADJACENT REINFORCING BARS SHALL BE STAGGERED 24" WHEREVER POSSIBLE.
- WELDED WIRE FABRIC, WHERE REQUIRED, SHALL BE PLACED IN THE CENTER OF THE SLAB UNLESS NOTED OTHERWISE. LAP JOINTS ONE WIRE SPACING PLUS 2" OR A MINIMUM OF 6". EXTEND FABRIC TO WITHIN 1" OF EDGES OF SLABS ON GRADE.
- PROVIDE PRECAST DOBIES, WIRE OR PLASTIC CHAIRS, SUPPORT BARS, ETC. TO MAINTAIN SPECIFIED CLEARANCES FOR THE ENTIRE LENGTH OF ALL REINFORCING BARS AND WELDED WIRE FABRIC. DO NOT USE REBAR STAKES TO SUPPORT REINFORCING. ALL REINFORCEMENT SHALL BE FASTENED AND SECURED TO PREVENT DISPLACEMENT BY THE PLACING OF CONCRETE.
- ALL CONCRETE WALLS TO BE DETAILED IN ELEVATION ON SHOP DRAWINGS. NO MORE THAN 50% OF HORIZONTAL WALL REINFORCING SHALL LAP IN A SINGLE VERTICAL PLANE.
- PROVIDE (1) #4 REINFORCING BARS, 3'-0" LONG, IN CONCRETE SLABS AT ALL RE-ENTRANT CORNERS. PLACE BARS AT ONE-THIRD OF SLAB DEPTH FROM THE TOP WITH 2" MINIMUM HORIZONTAL CLEARANCE FROM SLAB CORNER.
- REBAR FOR WELDED CONNECTIONS MUST MEET ASTM A706. STANDARD ASTM A615 GRADE 60 REBARS ARE NOT ACCEPTABLE FOR WELDING. WELDING PROCEDURE SHALL CONFORM TO ANS/AWS D1.4 "STRUCTURAL WELDING CODE - REINFORCING STEEL". DO NOT TACK WELD.
- REINFORCING DOWELS, WHERE REQUIRED, SHALL MATCH THE SIZE, SPACING AND QUANTITY OF MAIN REINFORCING, UNLESS NOTED OTHERWISE.

STRUCTURAL STEEL NOTES:

- STRUCTURAL STEEL MATERIALS:

STRUCTURAL STEEL	ASTM A992, Fy = 50 KSI
OTHER SHAPES & PLATES:	ASTM A36, Fy = 36 KSI, OR ASTM A572, GRADE 50, Fy = 50 KSI
STRUCTURAL TUBING (HSS):	ASTM A1085, GRADE A, Fy = 50 KSI, OR ASTM A501, GRADE C, Fy = 50 KSI
STRUCTURAL PIPE:	ASTM A53, GRADE B, Fy = 35 KSI
CONNECTION BOLTS:	ASTM F3125, GRADE A325-N, U.N. O.
ANCHOR BOLTS:	ASTM F1554, A307 OR A36, Fy = 36 KSI.
WELDING ELECTRODES:	E70-XX, UNLESS NOTED OTHERWISE.
- STRUCTURAL STEEL DETAILING, FABRICATION, AND ERECTION SHALL BE IN ACCORDANCE WITH AISC 360-16, "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".
- BOLTED CONNECTIONS SHALL BE MADE USING 3/4" DIAMETER BOLTS AND WASHERS CONFORMING TO ASTM A325 UNLESS OTHERWISE NOTED. THEY SHALL BE ASSEMBLED, INSTALLED AND INSPECTED IN ACCORDANCE WITH "RCSC-2014, SPECIFICATIONS FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS". BOLTS SHALL BE TIGHTENED TO THE SNUG TIGHT CONDITION UNLESS OTHERWISE NOTED ON THE DRAWINGS. ALL HOLES TO BE DRILLED OR PUNCHED; FLAME CUT HOLES ARE ONLY PERMITTED WITH THE APPROVAL OF THE ENGINEER OF RECORD.
- SHOP OR FIELD SPICES BETWEEN SUPPORTS THAT ARE NOT REQUIRED BY DESIGN WILL NOT BE ALLOWED. ANY MEMBERS CONTAINING SUCH SPICES SHALL BE REMOVED AND REPLACED WITH UNSPLICED MEMBERS AT THE FABRICATOR'S EXPENSE.
- PROVIDE BOLTS AND PUNCHED HOLES IN STRUCTURAL AND MISCELLANEOUS STEEL FOR ATTACHMENTS OF WOOD NAILERS AS REQUIRED ON THE ARCHITECTURAL, MECHANICAL AND STRUCTURAL DRAWINGS.
- MINIMUM SIZE OF WELD IS 3/16" UNLESS NOTED OTHERWISE. ALL WELDING SHALL CONFORM TO THE PROVISIONS OF AWS D1.1. STRUCTURAL WELDING CODE BY THE AMERICAN WELDING SOCIETY. ALL WORK SHALL BE PERFORMED BY CERTIFIED WELDERS EXPERIENCED IN THE TYPE OF CONSTRUCTION INVOLVED. PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE.
- SHOP DRAWINGS SHALL SHOW COMPLETE WELDING INFORMATION, BOTH SHOP AND FIELD, USING AMERICAN WELDING SOCIETY SYMBOLS UNLESS OTHERWISE INDICATED.
- PROVIDE 1 1/2" NON-METALLIC, SHRINKAGE RESISTANT GROUT CONFORMING TO ASTM C1107 UNDER ALL COLUMN BASE PLATES.
- ALL STEEL SHALL HAVE A PRIME COAT OF PAINT EXCEPT AREAS TO BE FIELD WELDED.
- ALL PORTIONS OF STEEL ENCASED IN CONCRETE, GROUT, OR IN CONTACT WITH EARTH SHALL BE PAINTED WITH BITUMINOUS PAINT.
- PRIME AND PAINT ALL FIELD WELDS AFTER INSPECTION.
- STEEL FABRICATOR TO BE AN AISC CERTIFIED FABRICATOR.
- ALL PLAN DIMENSIONS ARE TO CENTERLINE OF STEEL MEMBERS, EXCEPT FOR STEEL CHANNELS, WHICH ARE DIMENSIONED TO THE BACK FACE OF THE WEB.
- REFER TO ARCHITECTURAL DRAWINGS FOR MISCELLANEOUS STEEL BRACKETS, BRACES, SUPPORTS, ETC. NOT INDICATED ON THE STRUCTURAL DRAWINGS.
- RECTANGULAR STRUCTURAL TUBING (HSS) SHALL BE ORIENTATED IN THE STRONG DIRECTION (LDV) UNLESS OTHERWISE NOTED - REFER TO PLANS.
- ALL UNPAINTED EXTERIOR STEEL SHALL BE GALVANIZED.

CASTELLATED STEEL BEAM NOTES:

- SEE STRUCTURAL STEEL NOTES FOR ADDITIONAL REQUIREMENTS.
- THE MATERIALS USED TO FABRICATE CASTELLATED BEAMS SHALL CONFORM TO ASTM A992, Fy = 50 KSI, OR ASTM A572, Fy = 50 KSI.
- ALL WELDING SHALL BE PERFORMED IN ACCORDANCE WITH AWS D1.1.
- SUBSTITUTIONS OF SECTIONS OR MODIFICATIONS OF DETAILS, OR BOTH, SHALL BE CLEARLY NOTED AND IDENTIFIED ON SHOP DRAWING SUBMITTAL.
- BRIDGING TERMINATING AT WIDE FLANGE BEAMS SHALL BE ANCHORED TO BEAM. BRIDGING SHALL BE WIDENED AT ENDS PRIOR TO APPLYING ROOF LOADS.
- CASTELLATION OPENINGS IN ADJACENT CASTELLATED BEAMS OF SIMILAR DESIGNATION AND LENGTH SHALL BE ALIGNED TO ALLOW FOR PROPER INSTALLATION OF BRIDGING AND ROUTING OF OTHER TRADE MATERIALS.
- FILL CASTELLATION HOLES WHERE REQUIRED FOR POINT LOADS OR BEAM CONNECTION LOCATIONS AS REQUIRED IN THE SHOP.

STEEL DECK NOTES:

- ROOF AND CANOPY DECK (Fy = 50 KSI - MIN.):
 - 1 1/2" DEEP, TYPE "B" (WIDE RIB), 20 GAUGE, GALVANIZED SHEET WIDTH = 36"
 - PREPARE AND REPAIR DAMAGED GALVANIZED COATING ON BOTH SURFACES OF DECK WITH GALVANIZING REPAIR PAINT ACCORDING TO ASTM 780 AND MANUFACTURER'S INSTRUCTIONS.
- STEEL DECK MANUFACTURER SHALL BE A MEMBER OF THE STEEL DECK INSTITUTE (SDI). ALL PRODUCTS SHALL BE FACTORY MUTUAL APPROVED.
- STEEL DECK SHALL BE CONTINUOUS OVER A MINIMUM OF 4 SUPPORTS, UNLESS OTHERWISE NOTED.
- DECK ATTACHMENT PATTERN SHALL BE AS INDICATED ON THE DRAWINGS.
- PROVIDE 1 1/2" MINIMUM DECK BEARING AT ALL SUPPORTS. DECK SHALL BE PLACED AT THE PERIMETER WITH A COMPLETE RIB BEARING ON THE STEEL SUPPORT.
- ALL STEEL DECK WELDING IS ONLY TO BE PERFORMED BY AN AWS D1.3 CERTIFIED WELDER. CERTIFICATIONS ARE TO BE AVAILABLE ON SITE FOR VERIFICATION.

SHOP DRAWING NOTES:

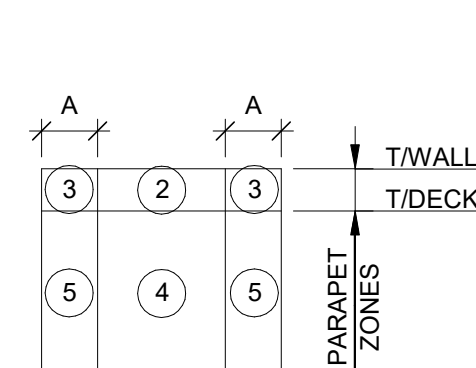
- CONTRACTOR SHALL FURNISH COMPLETE AND DETAILED SHOP DRAWINGS FOR THE FOLLOWING STRUCTURAL MATERIALS:
 - ANCHOR BOLT LAYOUT
 - STRUCTURAL STEEL AND STEEL DECK REINFORCING STEEL
 - CONCRETE MIX DESIGN
 - MASONRY UNITS, REINFORCING, MORTAR AND GROUT
 - LIGHT GAUGE METAL FRAMING AND CONNECTIONS (SEE NOTE #5)
- UNLESS NOTED, SUBMIT SHOP DRAWINGS FOR ALL FABRICATED MATERIALS. ALL SHOP DRAWINGS MUST BEAR THE REVIEW STAMP OF THE GENERAL CONTRACTOR. UNSTAMPED SHOP DRAWINGS WILL BE REJECTED WITHOUT REVIEW.
- ELECTRONIC COPIES OF SHOP DRAWINGS SHALL BE SUBMITTED THROUGH PROJECTMATES. ELECTRONIC COPIES OF SHOP DRAWINGS WILL ONLY BE ACCEPTED WITH THE APPROVAL OF THE ENGINEER OF RECORD. REPRODUCTION OF ELECTRONIC COPIES WILL BE BILLED TO GENERAL CONTRACTOR FOR REIMBURSEMENT OF PRINTING COSTS. DESIGN DRAWINGS SHALL NOT BE REPRODUCED FOR USE AS SHOP DRAWINGS. TWO SETS OF REVIEWED SHOP DRAWINGS WILL BE RETURNED TO THE GENERAL CONTRACTOR.
- ALLOW MINIMUM 10 WORKING DAYS FOR ARCHITECT/ENGINEER REVIEW OF EACH SHOP DRAWING SUBMITTAL. ALLOW AN ADDITIONAL WORKING DAY FOR EVERY 5 DRAWINGS OVER 30.
- SHOP DRAWINGS FOR CONSTRUCTION OF EACH BUILDING COMPONENT NOT DESIGNED BY THE DESIGN ENGINEER OF RECORD AND NOT SPECIFIED IN THE CONSTRUCTION DOCUMENTS SHALL BE SIGNED & SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT. COMPONENTS INCLUDE BUT ARE NOT LIMITED TO THE FOLLOWING: PRE ENGINEERED TRUSSES, RAILING, LADDERS, LIGHT GAUGE STUD FRAMING, RACK SYSTEMS, GLAZING SYSTEMS, AWNINGS AND ANY OTHER ITEM THAT IS DESIGNATED AS "DESIGNED BY OTHERS" OR "PRE ENGINEERED". THESE SHOP DRAWINGS SHALL BEAR CLEAR INDICATION THAT THEY HAVE BEEN REVIEWED AND APPROVED BY THE PROJECT ARCHITECT OR ENGINEER OF RECORD.
- SHOP DRAWINGS SUBMITTED TO THE ENGINEER OF RECORD SHALL ADHERE TO THE FOLLOWING GUIDELINES OR THEY WILL BE REJECTED AND RETURNED TO SENDER UNCHECKED:
 - THE LATEST ISSUE DATE, IN THE DRAWING REVISION BOX, OF ANY SHEET IN THE STRUCTURAL DRAWING SET, SHALL BE CLEARLY SHOWN ON THE TOP SHEET OF THE SHOP DRAWINGS.
 - NO PARTIAL SETS WILL BE ACCEPTED, UNLESS PREVIOUSLY APPROVED BY THE ENGINEER OF RECORD.
 - CASTELLATED STEEL BEAM SUBMITTALS SHALL HAVE JOB-SPECIFIC DETAILS INCLUDING COMPLETE DETAILS AND SCHEDULES FOR FABRICATION AND SHOP ASSEMBLY OF MEMBERS, INCLUDE DETAILS OF CUTS, CONNECTIONS, CAMBER, HOLES, FILLED HOLES, AND OTHER PERTINENT DATA. INDICATE WELDS BY STANDARD AWS SYMBOLS AND SHOW SIZE, LENGTH, AND TYPE OF EACH WELD.
- DETAILER SHALL COORDINATE WITH ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ALL ATTACHMENTS, CLIPS, OPENINGS OR DUCTWORK AFFECTING STRUCTURAL MEMBERS. ALL ITEMS SHALL BE SHOWN ON SHOP DRAWINGS.
- DIMENSIONAL COORDINATION SHALL BE PERFORMED BY THE GENERAL CONTRACTOR AND/OR THEIR FABRICATOR.
- PROPOSED CHANGES TO THE CONSTRUCTION DOCUMENTS SHALL BE CLEARLY MARKED IN THE SHOP DRAWINGS AND SHALL INCLUDE SIGNED & SEALED DRAWINGS AND CALCULATIONS BY AN ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED. THE ENGINEER OF RECORD WILL REVIEW THE PROPOSED CHANGE FOR ACCEPTANCE.
- THE CONTRACTOR SHALL HAVE PROOF OF WELDER CERTIFICATION AT THE JOBSITE AT ALL TIMES.
- APPROVED SHOP DRAWINGS SHALL BE AVAILABLE AT THE JOBSITE AT ALL TIMES.

MASONRY NOTES:

- MATERIALS, TESTING AND STORAGE OF MATERIALS SHALL CONFORM TO TMS-602 "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" AND TMS-602 "SPECIFICATION FOR MASONRY STRUCTURES."
- CONCRETE MASONRY UNITS:
 - MASONRY UNITS SHALL CONFORM TO ASTM C90, LIGHTWEIGHT UNITS
 - UNIT MINIMUM NET AREA COMPRESSIVE STRENGTH = 2000 PSI
 - MORTAR SHALL CONFORM TO ASTM C270 CEMENT-LIME, TYPE S
 - MINIMUM MASONRY MORTAR SYSTEM COMPRESSIVE STRENGTH (fm) = 2000 PSI BASED ON NET AREA
 - GROUT SHALL CONFORM TO ASTM C476 WITH A MAXIMUM AGGREGATE SIZE OF 3/8", A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI, AND A MINIMUM SLUMP OF 9 INCHES AND A MAXIMUM SLUMP OF 11 INCHES
- MASONRY UNITS AND MORTAR SHALL INCLUDE INTEGRAL WATERPROOFING AGENT "DRYBLOCK" BY W.R. GRACE.
- MASONRY SHALL BE LAID IN RUNNING BOND, UNLESS OTHERWISE NOTED.
- ALL REINFORCING BARS DENOTED AS CONTINUOUS SHALL HAVE LAP SPICES, CORNER BARS AND HOOKS AT DISCONTINUOUS ENDS. SEE MASONRY SPlice LAP SCHEDULE FOR LAP SPICE LENGTHS.
- HORIZONTAL JOINT REINFORCING SHALL BE LADDER TYPE, HOT DIPPED GALVANIZED, AND SPACED AT 18" ON CENTER. PROVIDE 1" MINIMUM WIRE SIZE #17, UNLESS OTHERWISE NOTED. MINIMUM STRAIGHT LAP = 16" AND MINIMUM CORNER OR TEE LAP = 30". JOINT REINFORCING SHALL BE FULLY EMBEDDED IN MORTAR WITH A MINIMUM COVER OF 5/8" FROM FACE OF MORTAR. DISCONTINUE HORIZONTAL JOINT REINFORCING AT CONTROL JOINTS.
- AT FIRST COURSE OF MASONRY, PROVIDE FULL MORTAR BED EQUAL TO WALL THICKNESS EXCEPT AT CELLS TO BE GROUTED SOLID.
- UNITS TO RECEIVE VERTICAL REINFORCING SHALL HAVE CELLS ALIGNED VERTICALLY FOR FULL HEIGHT REINFORCEMENT.
- FILL ALL MASONRY CELLS BELOW FINISHED FLOOR WITH GROUT. WHERE FINISHED FLOOR IS BELOW GRADE, FILL ALL CELLS BELOW GRADE.
- ALL ANCHOR BOLTS INTO MASONRY SHALL BE PLACED IN FULLY GROUTED CELLS, MINIMUM EMBEDMENT TO BE 4 1/4", UNLESS OTHERWISE NOTED.
- PIPES OR CONDUITS MAY PENETRATE HORIZONTALLY THROUGH MASONRY WALLS BY MEANS OF A SCHEDULE 40 GALVANIZED STEEL SLEEVE SOLIDLY GROUTED IN PLACE. CENTER TO CENTER SLEEVE SPACING SHALL NOT BE LESS THAN (3) SLEEVE DIAMETERS.
- AFTER MORTAR IS THOROUGHLY SET AND CURED, CLEAN MASONRY COMPLETELY USING THE LEAST HARSH METHOD POSSIBLE.
- THE MASONRY CONTRACTOR SHALL BE RESPONSIBLE FOR THE TEMPORARY BRACING OF MASONRY WALLS DURING CONSTRUCTION, AS REQUIRED.

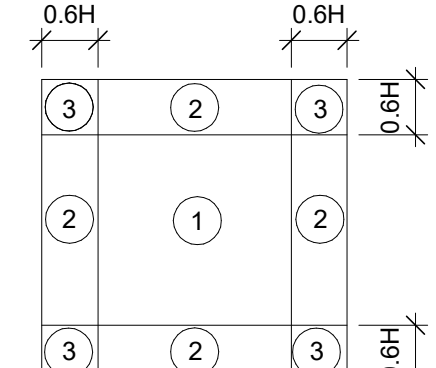
WIND PRESSURES - COMPONENT AND CLADDING 160 MPH - EXPOSURE C - ASCE 7-16				
	Zone	10 SF	50 SF	100 SF
Roof	1	22.7 psf	19.4 psf	18.0 psf
		-88.9 psf	-75.3 psf	-69.4 psf
		51.1 psf	45.9 psf	43.5 psf
	2	-117.3 psf	-99.8 psf	-92.2 psf
		51.1 psf	45.9 psf	43.5 psf
		-117.3 psf	-99.8 psf	-92.2 psf
Canopy Soffit	Typical	37.8 psf	33.3 psf	28.4 psf
		-37.8 psf	-36.9 psf	-30.7 psf
Walls	Zone	10 SF	50 SF	100 SF
		51.1 psf	45.8 psf	43.6 psf
		-55.3 psf	-50.1 psf	-47.8 psf
	5	51.1 psf	45.8 psf	43.6 psf
		-68.1 psf	-57.6 psf	-53.1 psf
		Zone	10 SF	20 SF
Parapets	2	165.6 psf	154.8 psf	140.7 psf
		-97.8 psf	-92.8 psf	-86.3 psf
		165.6 psf	154.8 psf	140.7 psf
3	-111.8 psf	-104.3 psf	-94.5 psf	
	Notes	1. TRIBUTARY PRESSURES ARE FOR THE SQUARE FOOT (SF) TRIBUTARY AREA SHOWN. FOR OTHER TRIBUTARY AREAS, LOGARITHMICALLY INTERPOLATE BETWEEN VALUES SHOWN ABOVE. 2. POSITIVE PRESSURES ACT TOWARD THE BUILDING. NEGATIVE PRESSURES ACT AWAY FROM THE BUILDING. 3. ALL WIND PRESSURES IN THE TABLE ARE BASED ON ULTIMATE WIND SPEEDS. TO CONVERT TO NOMINAL WIND PRESSURES MULTIPLE VALUES BY 0.6. 4. PARAPET PRESSURES ARE FOR SOLID SURFACES, WHERE BOTH SURFACES ARE CONNECTED TOGETHER (NO NET INTERNAL PRESSURE). 5. TO OBTAIN ROOF JOIST NET UPLIFT PRESSURES, REDUCE NOMINAL WIND PRESSURES, SEE NOTE #3, BY 6 PSF RELIABLE DEAD LOAD.		

COMPONENT AND CLADDING WALL PRESSURE ZONES



A = 5.8 FEET
ELEVATION

COMPONENT AND CLADDING ROOF PRESSURE ZONES



0.6H = 8.6 FEET
PLAN

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PROJECT MANAGER:

SDH

CHECKED BY:

BWM

DRAWN BY:

JBA

ISSUE/REVISION RECORD

DATE	DESCRIPTION
02/19/22	PERMIT SET
05/21/20	REV1-SITE PLAN PKG
10/07/20	REV2-PHOTOMETRIC
10/20/20	REV3-AHJ COMMENTS
01/11/21	REFRESH 2022_0107
02/26/21	REV4-P&Z COMMENTS
11/05/21	REFRESH 2021_0401
02/07/22	REFRESH 2021_0507
08/12/22	REFRESH 2022_0202

RaceTrac

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**VERO BEACH
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RACETRAC STORE NUMBER

#1422

PROTOTYPE SERIES TC

2022 LH SV 0202

PLAN MODIFICATION NOTICE

SPB NO. 0202 DATE 07/08/22

STANDARD PLAN BULLETINS (SPB) MODIFY THE PROTOTYPE SERIES SET NOTED ABOVE. THE LISTED SPB REPRESENTS THE LATEST MODIFICATION INCORPORATED TO THIS PROTOTYPE SERIES SET AT ORIGINAL RELEASE. THE ISSUE/REVISION RECORD COLUMN ABOVE

PRE-ENGINEERED LIGHT GAUGE STRUCTURAL FRAMING:

GENERAL:

- LIGHT GAUGE FRAMING AND CONNECTIONS INCLUDING EXTERIOR WALL STUDS, INTERIOR WALLS AND SOFFIT STUDS, FLOOR JOISTS, AND ROOF JOISTS SHALL BE DESIGNED BY A STRUCTURAL ENGINEER REGISTERED IN THE PROJECT STATE.
- PRIOR TO FABRICATION, THE CONTRACTOR SHALL SUBMIT SEALED SHOP DRAWINGS INDICATING DESIGN DATA AND SHALL INCLUDE COMPLETE PLANS AND DETAILS.
- INFORMATION INCLUDED IN THESE PLANS ARE THE MINIMUM REQUIREMENTS. EXTERIOR WALL STUDS SHALL BE A MINIMUM OF 600 S 162-43 AT 16" O.C.
- METAL STUD STRENGTH CRITERIA:
 - GAUGES 20 THROUGH 18 - 33 KSI MIN. YIELD STRESS
 - GAUGES 16 THROUGH 12 - 50 KSI MIN. YIELD STRESS
 - TRACK RUNNER - 33 KSI MIN. YIELD STRESS
- METAL STUD MEMBER DESIGNATION:

600 S 162 - 33	MILS GAUGE
33	20
43	18
54	16
68	14
97	12

WEB DEPTH: 600 = 6"
MEMBER TYPE: S = STUD
FLANGE WIDTH: 162 = 1.625" (1 5/8")
MINIMUM THICKNESS IN MILS

- FASTENERS:
 - SCREW CONNECTIONS

FASTENING STUD TO STUD
USE #10-16 TEK5/3 x 5/8" LONG BY ITW BUILDEX UNLESS OTHERWISE NOTED.
MIN. EDGE DISTANCE = 3/4"
MIN. SPACING BETWEEN FASTENERS = 3/4"
 - POWDER ACTUATED FASTENERS (P.A.F.):

FASTENING TO CONCRETE:
USE 0.145" DIA. DOME HEAD NAIL BY HILTI, UNLESS NOTED OTHERWISE.
MIN. EMBEDMENT = 1 1/4" MIN. EDGE DISTANCE = 2"
MIN. SPACING BETWEEN FASTENERS = 3"

FASTENING TO STEEL:
USE 0.145" DIA. DOME HEAD KNURLED SHANK FASTENER BY HILTI, UNLESS NOTED OTHERWISE.
MIN. EMBEDMENT = FULL PENETRATION MIN. EDGE DISTANCE = 1/2"
MIN. SPACING BETWEEN FASTENERS = 1"

FASTENING TO CONCRETE:
USE 0.145" DIA. DOME HEAD NAIL BY HILTI, UNLESS NOTED OTHERWISE.
MIN. EMBEDMENT = 1 1/4" MIN. EDGE DISTANCE = 2"
MIN. SPACING BETWEEN FASTENERS = 3"

FASTENING TO STEEL:
USE 0.145" DIA. DOME HEAD KNURLED SHANK FASTENER BY HILTI, UNLESS NOTED OTHERWISE.
MIN. EMBEDMENT = FULL PENETRATION MIN. EDGE DISTANCE = 1/2"
MIN. SPACING BETWEEN FASTENERS = 1"

FASTENING:
MINIMUM FASTENING REQUIREMENTS, UNLESS NOTED OTHERWISE.

TRACK TO CONCRETE:
(2) FASTENERS AT 16" O.C. TYP w/ADD'L FASTENER 3" FROM END OF TRACK AND (2) FASTENERS MINIMUM AT JAMBS.

TRACK TO STRUCTURAL STEEL:
12" O.C. TYP. w/ADD'L FASTENER 3" FROM END OF TRACK AND (2) FASTENERS MINIMUM AT JAMBS.

STUD TO TRACK:
(1) SCREW MINIMUM AT EACH FLANGE

STUD TO STUD:
(3) SCREWS MINIMUM

STUD TO STEEL OR CONCRETE:
USE POWDER ACTUATED FASTENERS REFERENCED ABOVE. SEE DETAILS FOR CONNECTIONS, MINIMUM 2 FASTENERS.

CURTAIN WALL AND NON LOAD BEARING PARTITIONS:
MECHANICAL BRACING OF STUDS IS NOT NECESSARY WHERE WALL SHEATHING IS ATTACHED ON BOTH SIDES OF THE STUDS.

WHEN ONLY ONE FACE OF THE STUDS RECEIVE SHEATHING, PROVIDE BRACING BY ONE OF THE FOLLOWING METHODS:
A. COLD ROLLED CHANNEL, RUN HORIZONTALLY THROUGH STUD PUNCH OUTS AND ATTACHED AT EACH STUD.
B. 2" WIDE, 18 GAUGE, STEEL STRAPS RUN HORIZONTALLY, ON BOTH SIDES OF THE STUDS, AND ATTACHED AT EACH STUD.

VERTICAL SPACING OF THE BRACING IS LIMITED TO A MAXIMUM OF 4'-0" THROUGHOUT THE HEIGHT OF THE WALL.

PROVIDE ALL ACCESSORIES AS REQUIRED BY THE METAL STUD MANUFACTURER.

JOISTS SHALL BE FABRICATED TO PROVIDE 12" OF UNPUNCHED WEB AT BEARING ENDS.

PROVIDE JOIST WEB STIFFENERS PER METAL STUD MANUFACTURERS' RECOMMENDATIONS AT BEARING POINTS.

PROVIDE END BLOCKING WHERE JOISTS ARE NOT RESTRAINED AGAINST ROTATION.

NO LIGHT GAUGE MEMBER THAT HAS BEEN TRIMMED OR CUT SHALL BE INSTALLED SO THAT AN EDGE OF A WEB PUNCH OUT OCCURS WITHIN 10" MINIMUM OF THE TRIMMED OR CUT MEMBER END.

ALL MEMBER CUTTING MUST BE PERFORMED USING A SAW OR SHEAR. NO TORCH CUTTING IS ALLOWED AT ANY TIME. ALSO CUTTING OF ADDITIONAL HOLES, OTHER THAN THOSE PROVIDED BY THE MANUFACTURER, IN THE MEMBER WEB IS NOT PERMITTED AT ANY TIME.

SP LICING OF AXIALLY LOADED MEMBERS IS NOT ALLOWED AT ANY TIME.

BUILT UP LIGHT GAUGE HEADERS, CONSTRUCTED FOR EXTERIOR WALL CONDITIONS, SHALL HAVE INSULATION PLACED WITHIN THEM PRIOR TO THEIR INSTALLATION IN WALL SYSTEM.

PERFORMANCE REQUIREMENTS:

- STRUCTURAL PERFORMANCE: PROVIDE COLD FORMED METAL FRAMING AND CONNECTIONS CAPABLE OF WITHSTANDING DESIGN LOADS WITHIN LIMITS AND UNDER CONDITIONS INDICATED.
 - DESIGN LOADS: AS INDICATED IN STRUCTURAL DESIGN CRITERIA.
 - DEFLECTION LIMITS: DESIGN FRAMING SYSTEMS AND CONNECTIONS TO WITHSTAND DESIGN LOADS WITHOUT DEFLECTIONS GREATER THAN THE FOLLOWING:
 - EXTERIOR WALL FRAMING: HORIZONTAL DEFLECTION OF L/360 OF THE WALL HEIGHT.
 - INTERIOR WALL FRAMING: HORIZONTAL DEFLECTION OF L/240 OF THE WALL HEIGHT.
 - WALL FRAMING SUPPORTING BRICK VENEER: DEFLECTION OF L/600 OF THE SPAN LENGTH.
 - DESIGN FRAMING SYSTEMS AND CONNECTIONS TO PROVIDE FOR MOVEMENT OF FRAMING MEMBERS WITHOUT DAMAGE OR OVERSTRESSING, SHEATHING FAILURE, CONNECTION FAILURE, UNDUE STRAIN ON FASTENERS AND ANCHORS, OR OTHER DETRIMENTAL EFFECTS WHEN SUBJECT TO A MAXIMUM AMBIENT TEMPERATURE CHANGE OF 120 DEG. F.
 - DESIGN FRAMING SYSTEMS AND CONNECTIONS TO MAINTAIN CLEARANCES AT OPENINGS, TO ALLOW FOR CONSTRUCTION TOLERANCES, AND TO ACCOMMODATE LIVE LOAD DEFLECTION OF PRIMARY BUILDING STRUCTURE.
 - METAL STUD MANUFACTURER SHALL BE A MEMBER OF THE STEEL STUD MANUFACTURERS ASSOCIATION (SMSA).
 - ALL METAL STUDS AND ACCESSORIES SHALL HAVE A G60 GALVANIZED COATING, UNLESS NOTED OTHERWISE.

SUBMITTALS:

- PRODUCT DATA: FOR EACH TYPE OF COLD FORMED METAL FRAMING PRODUCT AND ACCESSORY INDICATED.
- SHOP DRAWINGS: SHOW LAYOUT, SPACINGS, SIZES, THICKNESSES, AND TYPES OF COLD FORMED METAL FRAMING; FABRICATION; AND FASTENING AND ANCHORAGE DETAILS, INCLUDING MECHANICAL FASTENERS, SHOW REINFORCING CHANNELS, OPENING FRAMING, SUPPLEMENTAL FRAMING, STRAPPING, BRACING, BRIDGING, SPLICES, ACCESSORIES, CONNECTION DETAILS, AND ATTACHMENT TO ADJOINING WORK.
 - FOR COLD FORMED METAL FRAMING INDICATED TO COMPLY WITH DESIGN LOADS, INCLUDE STRUCTURAL ANALYSIS DATA SIGNED AND SEALED BY THE QUALIFIED PROFESSIONAL ENGINEER (LICENSED IN THE PROJECT STATE) RESPONSIBLE FOR THEIR PREPARATION.

QUALITY ASSURANCE:

- INSTALLER QUALIFICATIONS: AN EXPERIENCED INSTALLER WHO HAS COMPLETED COLD FORMED METAL FRAMING SIMILAR IN MATERIAL, DESIGN, AND EXTENT TO THAT INDICATED FOR THIS PROJECT AND WHOSE WORK HAS RESULTED IN CONSTRUCTION WITH A RECORD OF SUCCESSFUL IN SERVICE PERFORMANCE.
- ENGINEERING RESPONSIBILITY: ENGAGE A QUALIFIED PROFESSIONAL ENGINEER (LICENSED IN THE PROJECT STATE) TO PREPARE DESIGN CALCULATIONS, SHOP DRAWINGS, AND OTHER STRUCTURAL DATA.

REFER TO LIGHT GAUGE FRAMING DRAWINGS FOR LIGHT GAUGE METAL NOTES, PLANS AND DETAILS REQUIRED TO COMPLETE SYSTEM.
- ASIS SPECIFICATIONS: COMPLY WITH AISI'S "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" OR "LOAD AND RESISTANCE FACTOR DESIGN SPECIFICATION FOR COLD FORMED STEEL STRUCTURAL MEMBERS".

WOOD NOTES:

- ALL DESIGN AND CONSTRUCTION SHALL CONFORM TO THE APPLICABLE BUILDING CODE AND ANSI/APA NDS, THE NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION.
- MINIMUM NAILING SHALL CONFORM TO IBC TABLE 2304.10.1 UNLESS OTHERWISE NOTED ON PLANS OR DETAILS. COMMON NAILS SHALL BE USED UNLESS OTHERWISE NOTED
- ALL BOLTS, LAG BOLTS AND LAG SCREWS SHALL BE PRE-DRILLED. HOLES FOR BOLTS SHALL BE 1/16" LARGER THAN BOLT DIAMETER. HOLES FOR SMOOTH PORTIONS OF LAG SHANKS SHALL BE OF THE SAME DIAMETER. LAG BOLTS AND SCREWS SHALL NOT BE HAMMERED INTO WOOD MEMBERS.
- ROOF SHEATHING SHALL BE APA SPAN RATED SHEATHING, EXPOSURE 1. ATTACH WITH COMMON NAILS AS INDICATED ON THE DRAWINGS. STAGGER PANELS AS REQUIRED TO AVOID LINING UP END JOINTS. PROVIDE EDGE PANEL CLIPS OR BLOCKING WHERE ROOF FRAMING EXCEEDS 16" ON CENTER.
- NOTCHES AT THE ENDS OF SOLID SAWN JOISTS SHALL NOT EXCEED 1/4 OF THE RAFTER DEPTH. HOLES FORED IN SOLID SAWN JOISTS SHALL NOT BE MADE WITHIN 2" OF THE TOP OR BOTTOM OF THE JOIST, WITH THE DIAMETER NOT TO EXCEED 1/3 OF THE RAFTER DEPTH. NOTCHES AT THE BOTTOM OF SOLID SAWN JOISTS SHALL NOT EXCEED 1/6 OF THE RAFTER DEPTH AND SHALL NOT BE LOCATED WITHIN THE MIDDLE 1/3 OF THE SPAN.
- ALL NAILS SHALL BE "COMMON" HOT DIP GALVANIZED UNLESS OTHERWISE NOTED.
 - 8d = 0.131" DIA x 2 1/2"
 - 10d = 0.148" DIA x 3"
 - 12d = 0.148" DIA x 3 1/4"
 - 16d = 0.162" DIA x 3 1/2"

ABBREVIATIONS			
A.B.	ANCHOR BOLT	MAS.	MASONRY
AFF	ABOVE FINISHED FLOOR	MAX.	MAXIMUM
ARCH	ARCHITECT	MFR.	MANUFACTURER
B/	BOTTOM OF	MIN.	MINIMUM
BM.	BEAM	MISC.	MISCELLANEOUS
BOTT	BOTTOM	M.O.	MASONRY OPENING
BLKG.	BLOCKING	MTL.	METAL
BLDG.	BUILDING	N.A.	NOT APPLICABLE
CJ	CONTROL/CONSTRUCTION JOINT	N/A	NOT APPLICABLE
C.L.	CENTER LINE	N.D.T.	NON-DESTRUCTIVE TESTING
CLR	CLEAR	N.I.C.	NOT IN CONTRACT
CMU	CONCRETE MASONRY UNIT	N.T.S.	NOT TO SCALE
COL	COLUMN	O.C.	ON CENTER
CONC	CONCRETE	O.D.	OUTSIDE DIAMETER
COND	CONDENSING UNIT	O.H.	OPPOSITE HAND
CONN	CONNECTION	OPNG	OPENING
CONT	CONTINUOUS	OPP.	OPPOSITE
CONST	CONSTRUCTION	P.A.F.	POWDER ACTUATED FASTENER
CONTR.	CONTRACTOR	PJF	PREMOLDED JOINT FILLER
COORD	COORDINATE	PL	PLATE
CTR.	CENTER	PLYWD.	PLYWOOD
DET.	DETAIL	PREFAB.	PREFABRICATED
DIA	DIAMETER	P.T.	PRESSURE TREATED
DN	DOWN	PTD.	PAINTED
DIM.	DIMENSION	RAD.	RADIUS
DWG.	DRAWING	REINF	REINFORCING
EA.	EACH	REQ'D	REQUIRED
E.J.	EXPANSION JOINT	SECT.	SECTION
EL.	ELEVATION	SHT.	SHEET
ELEV	ELEVATION	SIM.	SIMILAR
E.O.R.	ENGINEER OF RECORD	SPCS	SPACES
E.W.	EACH WAY	SQ.	SQUARE
EXIST.	EXISTING	STD.	STANDARD
E.O.S.	EDGE OF SLAB	STL	STEEL
EQ	EQUAL	STRUC.	STRUCTURAL
F.F.	FINISH FLOOR	S.W.	SHORT WAY
FIN. FLR.	FINISH FLOOR	TJ	TOP OF
FOOT	FOOT	T.D.S.	TURN DOWN STEP
F.S.	FOOTING STEP	THK.	THICK
FTG.	FOOTING	THRU	THROUGH
F.V.	FIELD VERIFY	T.O.S.	TOP OF STEEL
GA.	GAUGE	TYP	TYPICAL
GALV.	GALVANIZED	T&B	TOP AND BOTTOM
G.C.	GENERAL CONTRACTOR	U.N.O.	UNLESS NOTED OTHERWISE
GDR	GIRDER	VERT.	VERTICAL
HT.	HEIGHT	WD	WOOD
HORIZ	HORIZONTAL	WF	WIDE FLANGE
JBE	JOIST BEARING ELEVATION	WWF	WELDED WIRE FABRIC
JST	JOIST	WWM	WELDED WIRE MESH
JT	JOINT	W/	WITH
KB	KNEE BRACE	WO	WITHOUT
LG	LONG	WP	WORK POINT
LDH	LONG DIMENSION HORIZONTAL		
LDV	LONG DIMENSION VERTICAL		
LLH	LONG LEG HORIZONTAL		
LLV	LONG LEG VERTICAL		
L.W.	LONG WAY		

INCORPORATED SPB:

RaceTrac.
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PROJECT MANAGER:

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CHECKED BY:

BWM

DRAWN BY:

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ISSUE/REVISION RECORD

DATE	DESCRIPTION
02/19/22	PERMIT SET
05/21/20	REV1-SITE PLAN PKG
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**VERO BEACH
FL 32966
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RACETRAC STORE NUMBER

#1422

PROTOTYPE SERIES TC

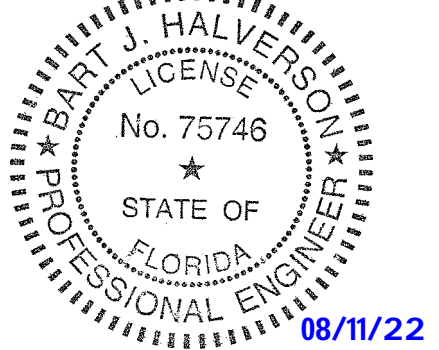
2022 LH SV 0202

PLAN MODIFICATION NOTICE

SPB NO. 0202 DATE 07/08/22

STANDARD PLAN BULLETINS (SPB) MODIFY THE PROTOTYPE SERIES SET NOTED ABOVE. THE LISTED SPB REPRESENTS THE LATEST MODIFICATION INCORPORATED TO THIS PROTOTYPE SERIES SET AT ORIGINAL RELEASE. THE ISSUE/REVISION RECORD COLUMN ABOVE LISTS ANY REVISIONS OR SPB INCORPORATED IN THIS SET AFTER THE ORIGINAL RELEASE. CONTACT RACETRAC ENGINEERING AND CONSTRUCTION FOR ANY SUBSEQUENT BULLETINS NOT INCORPORATED HEREIN.

PROFESSIONAL SEAL



PROJECT NUMBER

63200213

SHEET TITLE

**GENERAL
STRUCTURAL
NOTES**

SHEET NUMBER

S001

SPECIAL INSPECTION AND TESTING:

PER THE ENGINEER OF RECORD, THIS PROJECT REQUIRES SPECIAL INSPECTIONS AND TESTING BASED ON THE REQUIREMENTS OF CHAPTER 17 OF THE 2018 INTERNATIONAL BUILDING CODE IN THE ABSENCE OF FLORIDA BUILDING CODE PROVISIONS. THESE NOTES AND THE STATEMENT OF SPECIAL INSPECTIONS PREPARED FOR THE PROJECT OWNER ARE INTENDED TO INFORM THE CONTRACTOR OF THE QUALITY ASSURANCE PROGRAM AND THE EXTENT OF THE CONTRACTORS RESPONSIBILITIES.

THE SPECIAL INSPECTIONS & TESTING PROGRAM:

THE SPECIAL INSPECTION AND TESTING PROGRAM IS A QUALITY ASSURANCE PROGRAM INTENDED TO ENSURE THAT THE WORK IS PERFORMED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THESE INSPECTIONS ARE IN ADDITION TO THE INSPECTIONS SPECIFIED IN IBC SECTION 110. THE SPECIAL INSPECTION PROGRAM DOES NOT RELIEVE THE CONTRACTOR OF HIS OR HER RESPONSIBILITY TO COMPLY WITH THE OFFICIAL CONTRACT DOCUMENTS. FURTHER, IT IS NOT INTENDED THAT THE CONTRACTOR'S CONTRACTUAL AND STATUTORY OBLIGATIONS ARE ANYWAY RELIEVED OR FOREGONE BY THE PRESENCE OF THE SPECIAL INSPECTOR. THE CONTRACTOR HAS THE SOLE RESPONSIBILITY FOR ANY DEVIATIONS FROM THE OFFICIAL CONTRACT DOCUMENTS. THE SPECIAL INSPECTOR DOES NOT REPLACE THE DUTIES OF THE BUILDING OFFICIAL NOR THE QUALITY CONTROL RESPONSIBILITIES AND PERSONNEL OF THE CONTRACTOR. JOB SITE SAFETY AND MEANS AND METHODS OF CONSTRUCTION ARE SOLELY THE RESPONSIBILITY OF THE CONTRACTOR.

THE PROJECT OWNER IS RESPONSIBLE FOR EMPLOYING SPECIAL INSPECTION SERVICES. THE SPECIAL INSPECTOR/AGENCY SHALL NOT BE IN THE EMPLOY OF THE CONTRACTOR, SUBCONTRACTOR OR MATERIAL SUPPLIER. IN THE CASE OF AN OWNER/CONTRACTOR, THE SPECIAL INSPECTOR/AGENCY SHALL BE EMPLOYED AS SPECIFIED BY THE BUILDING OFFICIAL.

THE SPECIAL INSPECTOR IS OBLIGATED TO BOTH THE OWNER AND THE BUILDING OFFICIAL FOR OBSERVING THAT THE WORK IS EXECUTED IN SUBSTANTIVE ACCORDANCE WITH THE OFFICIAL CONTRACT DOCUMENTS. THE OFFICIAL CONTRACT DOCUMENTS ARE DEFINED AS THE PERMITTED PLANS AND SPECIFICATIONS, ADDENDA, CHANGE ORDERS, ISSUED SKETCHES AND REVISION DRAWINGS, AND ALL DIRECTIVES ISSUED BY ARCHITECT/ENGINEER.

THE INSPECTION AND TESTING AGENTS SHALL DISCLOSE ANY PAST OR PRESENT BUSINESS RELATIONSHIP OR POTENTIAL CONFLICT OF INTEREST WITH THE CONTRACTOR OR ANY OF THE SUBCONTRACTORS WHOSE WORK IS TO BE INSPECTED OR TESTED. THE SPECIAL INSPECTORS MAY HAVE AN INTEREST IN PRODUCTS OR SUBJECTS FOR WHICH THEY PROVIDE SPECIAL INSPECTION SERVICES.

SPECIAL INSPECTION REPORT REQUIREMENTS:

SPECIAL INSPECTION REPORTS AND A FINAL REPORT IN ACCORDANCE WITH SECTION 1704.2.4 SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF THE WORK IS APPROVED FOR OCCUPANCY.

REPORT REQUIREMENT: SPECIAL INSPECTORS SHALL KEEP RECORDS OF ALL INSPECTIONS. THE SPECIAL INSPECTOR SHALL FURNISH THE INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INDICATE THAT WORK OBSERVED OR TESTED WAS OR WAS NOT COMPLETED IN CONFORMANCE TO THE APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THEY ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO THE COMPLETION OF THAT PHASE OF THE WORK. A FINAL REPORT DOCUMENTING REQUIRED SPECIAL INSPECTIONS AND TESTS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS OR TESTS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF THE WORK IS APPROVED FOR OCCUPANCY.

CONTRACTOR RESPONSIBILITIES:

THE CONTRACTOR IS RESPONSIBLE FOR NOTIFYING THE SPECIAL INSPECTOR IN ADVANCE OF SCHEDULED AND PLANNED OPERATIONS IN ORDER TO ASSURE TIMELY AND APPROPRIATE INSPECTION FOR THE ITEMS LISTED IN THE SCHEDULE OF SPECIAL INSPECTIONS. THE CONTRACTOR SHALL PROVIDE ADEQUATE NOTICE TO THE SPECIAL INSPECTOR FOR ALL INSPECTIONS.

THE CONTRACTOR SHALL COOPERATE WITH AND ASSIST THE SPECIAL INSPECTOR IN PERFORMING HIS INSPECTION DUTIES. THE SPECIAL INSPECTOR SHALL HAVE FREE ACCESS TO THE PROJECT AT ALL TIMES. THE CONTRACTOR SHALL REVIEW THE SPECIAL INSPECTION PLAN AND COORDINATE THE SCHEDULE OF WORK TO ACCOMMODATE THE REQUIRED INSPECTIONS.

PROVIDE ACCESS TO APPROVED PLANS: THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING THE SPECIAL INSPECTOR ACCESS TO APPROVED PLANS. THE CONTRACTOR SHALL MAINTAIN A CURRENT SET OF THE CONTRACT DOCUMENTS AT THE JOB SITE.

CORRECT DISCREPANCIES AND DEVIATIONS: THE CONTRACTOR SHALL, UPON BEING INFORMED BY THE SPECIAL INSPECTOR, IMMEDIATELY CAUSE TO ELIMINATE SUCH DISCREPANCIES AND DEVIATIONS.

WORK COMPLETED WITHOUT INSPECTION: WORK REQUIRING INSPECTION WHICH IS COMPLETED WITHOUT INSPECTION WILL BE REJECTED SOLELY ON THAT BASIS.

RETAIN SPECIAL INSPECTION RECORDS: THE CONTRACTOR IS ALSO RESPONSIBLE FOR RETAINING AT THE JOB SITE ALL SPECIAL INSPECTION RECORDS COMPLETED BY THE SPECIAL INSPECTOR.

COORDINATE AND SUBMIT: THE CONTRACTOR IS RESPONSIBLE FOR PREPARING AND SUBMITTING TO THE BUILDING OFFICIAL AND THE OWNER A STATEMENT OF CONTRACTOR RESPONSIBILITY, IBC SECTION 1704.4, FOR THEMSELVES AND FOR SUBMITTING A STATEMENT OF CONTRACTOR RESPONSIBILITY FOR EACH STRUCTURAL COMPONENT SUBCONTRACTOR. THE STATEMENTS OF RESPONSIBILITY SHALL BE SUBMITTED PRIOR TO THE COMMENCEMENT OF WORK ON THE SYSTEM OR COMPONENT.

A. THE STATEMENT OF CONTRACTOR RESPONSIBILITY SHALL CONTAIN THE FOLLOWING:

- ACKNOWLEDGEMENT OF AWARENESS OF THE SPECIAL REQUIREMENTS CONTAINED IN THE QUALITY ASSURANCE PLAN.
- ACKNOWLEDGEMENT THAT CONTROL WILL BE EXERCISED TO OBTAIN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS APPROVED BY THE BUILDING OFFICIAL.
- PROCEDURES FOR EXERCISING CONTROL WITHIN THE CONTRACTOR'S ORGANIZATION, THE METHOD AND FREQUENCY OF REPORTING AND THE DISTRIBUTION OF THE REPORTS.
- IDENTIFICATION AND QUALIFICATIONS OF THE PERSONS EXERCISING SUCH CONTROL AND THEIR POSITIONS IN THE ORGANIZATION.

B. STRUCTURAL COMPONENT SUBCONTRACTORS INCLUDE BUT ARE NOT LIMITED TO STRUCTURAL STEEL FABRICATORS AND ERECTORS, COMPONENT FABRICATORS, SUCH AS STEEL JOISTS, CONCRETE AND MASONRY CONTRACTORS.

C. AT THE COMPLETION OF STRUCTURAL COMPONENT FABRICATION, THE FABRICATORS SHALL SUBMIT A CERTIFICATE OF COMPLIANCE STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR COSTS OF:

RETESTING AND REINSPECTION OF MATERIALS, WORK AND/OR PRODUCTS THAT DO NOT MEET THE REQUIREMENTS OF THE CONTRACT DOCUMENTS AND SHOP DRAWINGS/SUBMITTAL DATA.

REVIEW OF PROPOSED REPAIR AND/OR REPLACEMENT PROCEDURES BY THE REGISTERED DESIGN PROFESSIONAL, IN RESPONSIBLE CHARGE AND THE INSPECTORS AND TESTING AGENCIES.

REPAIR OR REPLACEMENT OF WORK THAT DOES NOT MEET THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.

THE CONTRACTOR IS RESPONSIBLE FOR THE TRAVEL COSTS OF THE SPECIAL INSPECTOR OR AGENTS WHEN SHOP INSPECTION IS REQUIRED OF AN UNAPPROVED STRUCTURAL COMPONENT FABRICATOR.

INSPECTION OF FABRICATION:

WHERE FABRICATION OF STRUCTURAL MEMBERS AND ASSEMBLIES ARE PERFORMED ON THE PREMISES OF THE FABRICATOR, THE SHOP FABRICATION REQUIRES SPECIAL INSPECTION DURING THE FABRICATION OF ITEMS FOR THIS PROJECT.

EXEMPTION:

FABRICATORS APPROVED BY THE BUILDING OFFICIAL ARE EXEMPT FROM THE ON PREMISE INSPECTION. THE APPROVAL BY THE BUILDING OFFICIAL OF ANY FABRICATOR SHOULD BE PROPERLY DOCUMENTED PRIOR TO THE COMMENCEMENT OF FABRICATION. EXEMPTION WILL BE PROVIDED TO FABRICATORS WHO PROVIDE PROOF OF CERTIFICATION BY A NATIONALLY RECOGNIZED GOVERNING ASSOCIATION WHICH PERFORMS PERIODIC INSPECTIONS AND MAINTAINS QUALITY ASSURANCE CRITERIA. EXAMPLES ARE: AISC CERTIFICATION FOR A STEEL FABRICATOR OR SJI CERTIFICATION FOR A STEEL JOIST MANUFACTURER.

AT THE COMPLETION OF FABRICATION, THE FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

CONCRETE TESTING NOTES:

CONCRETE TESTING AND INSPECTION SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF ACI 318 AND THE SCHEDULE OF SPECIAL INSPECTIONS. SAMPLES FOR STRENGTH TESTS OF EACH CLASS OF CONCRETE PLACED EACH DAY SHALL BE TAKEN NOT LESS THAN ONCE A DAY, NOR LESS THAN ONCE FOR EACH 75 CY. OF CONCRETE USED FOR FOOTINGS, NOR LESS THAN ONCE FOR EACH 5000 SQUARE FEET OF SURFACE AREA FOR SLABS. TEST REPORTS INDICATING NON-COMPLIANCE SHALL BE PROVIDED TO THE OWNER, ARCHITECT & CONTRACTOR. A COPY OF THE TEST REPORTS SHALL BE AVAILABLE AT THE JOBSITE.

MASONRY TESTING NOTES:

CONCRETE MASONRY TESTING AND INSPECTION SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF ACI 530 AND THE SCHEDULE OF SPECIAL INSPECTIONS. INSPECTION SHALL INCLUDE GENERAL INSPECTION OF WORK IN PROGRESS TO CONFIRM THAT MATERIALS, CONSTRUCTION AND WORKMANSHIP ARE IN COMPLIANCE WITH PLANS, SPECIFICATIONS AND GOOD CONSTRUCTION PRACTICES. ADDITIONALLY, MORTAR SHALL BE SAMPLED AND TESTED IN ACCORDANCE WITH ASTM C780 ANNEX A6. EACH TEST SHALL CONSIST OF THREE (3) SPECIMENS.

FREQUENCY OF TESTING: AT THE BEGINNING OF MASONRY CONSTRUCTION AND ONCE FOR EVERY 5000 SQUARE FEET OF MASONRY INSTALLED THEREAFTER.

COLD WEATHER = LESS THAN 40 DEGREES FAHRENHEIT
HOT WEATHER = GREATER THAN 90 DEGREES FAHRENHEIT

INSPECTOR TO BE CERTIFIED BY THE INTERNATIONAL CODE COUNCIL.

STEEL INSPECTION & TESTING NOTES:

STRUCTURAL STEEL TESTING AND INSPECTION SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF THE REFERENCED STANDARDS AND THE SCHEDULE OF SPECIAL INSPECTIONS.

FIELD BOLTED CONNECTIONS WILL BE TESTED AND INSPECTED ACCORDING TO RCSC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS."

FIELD WELDS SHALL BE INSPECTED AND TESTED ACCORDING TO AWS D1.1. IN ADDITION TO VISUAL INSPECTION, WELDED MOMENT CONNECTIONS WILL BE TESTED BY ULTRASONIC, ASTM E164, OR OTHER AWS APPROVED METHOD.

OTHER REQUIRED INSPECTIONS:

THE REQUIREMENTS OF SPECIAL INSPECTIONS AND TESTING IN ACCORDANCE OF THE INTERNATIONAL BUILDING CODE ARE MINIMUM REQUIREMENTS AND DO NOT LIMIT THE REQUIREMENTS FOR THE CONTRACTOR TO PROVIDE OTHER QUALITY CONTROL INSPECTIONS AND TESTING REQUIRED BY THE OWNER, CONTRACT DOCUMENTS, OR OTHER GOVERNING AUTHORITIES HAVING JURISDICTION.

SCHEDULE OF SPECIAL INSPECTIONS

1704.2.5 INSPECTION OF FABRICATORS

MATERIAL/ACTIVITY	SERVICE	APPLICABLE TO PROJECT	
		Y/N	EXTENT
VERIFY FABRICATION/QUALITY CONTROL PROCEDURES	IN-PLANT REVIEW (3) DURING FABRICATION	Y	SPECIAL INSPECTIONS ARE NOT REQUIRED WHERE THE FABRICATOR IS REGISTERED AND APPROVED IN ACCORDANCE WITH SECTION 1704.2.5.1

1705.2 STEEL CONSTRUCTION

MATERIAL/ACTIVITY	SERVICE	APPLICABLE TO PROJECT	
		Y/N	EXTENT
1. FABRICATOR AND ERECTOR DOCUMENTS VERIFY REPORTS AND CERTIFICATES AS LISTED IN AISC 300, CHAPTER N, SECTION N3, PARAGRAPH 2 FOR COMPLIANCE WITH CONSTRUCTION DOCUMENTS	SUBMITTAL REVIEW	Y	EACH SUBMITTAL
2. MATERIAL VERIFICATION OF STRUCTURAL STEEL, HIGH-STRENGTH BOLTS, NUTS, & WASHERS, & WELD FILLER MATERIALS	SHOP (3) AND FIELD INSPECTION	Y	PERIODIC
3. VERIFY ANCHOR DIAMETER, GRADE, TYPE, LENGTH & EMBEDMENT (SEE 1705.3)	FIELD INSPECTION	Y	PERIODIC
4. VERIFY MEMBERS LOCATIONS, BRACES, STIFFENERS, AND APPLICATION OF JOINT DETAILS AT EACH CONNECTION COMPLY WITH CONSTRUCTION DOCUMENTS	FIELD INSPECTION	Y	PERIODIC
5. STRUCTURAL STEEL WELDING	---	---	---
a. INSPECTION TASKS PRIOR TO WELDING (OBSERVE OR PERFORM FOR EACH WELDED JOINT OR MEMBER, THE QA TASKS LISTED IN AISC 300 TABLE N6.4-1)	SHOP (3) AND FIELD INSPECTION	Y	OBSERVE OR PERFORM AS NOTED (4)
b. INSPECTION TASKS DURING WELDING (OBSERVE OR PERFORM FOR EACH WELDED JOINT OR MEMBER, THE QA TASKS LISTED IN AISC 300 TABLE N6.4-2)	SHOP (3) AND FIELD INSPECTION	Y	OBSERVE (4)
c. INSPECTION TASKS AFTER WELDING (OBSERVE OR PERFORM FOR EACH WELDED JOINT OR MEMBER, THE QA TASKS LISTED IN AISC 300 TABLE N6.4-3)	SHOP (3) AND FIELD INSPECTION	Y	OBSERVE OR PERFORM AS NOTED (4)
d. NONDESTRUCTIVE (NDT) TESTING OF WELDED JOINTS	---	---	---
1) NOT USED	---	---	---
2) COMPLETE PENETRATION GROOVE WELDS 5/16" OR GREATER IN RISK CATEGORY II	SHOP (3) OR FIELD ULTRASONIC TESTING - 10% OF WELDS, MINIMUM	Y	PERIODIC
7) FABRICATORS NOT REPORTS WHEN FABRICATORS PERFORMS WDT	VERIFY REPORTS	Y	EACH SUBMITTAL (5)
6. STRUCTURAL STEEL BOLTING	SHOP (3) AND FIELD INSPECTION	---	---
a. INSPECTION TASKS PRIOR TO BOLTING (OBSERVE OR PERFORM TASKS FOR EACH BOLTED CONNECTION IN ACCORDANCE WITH QA TASKS LISTED IN AISC 300 TABLE N6.6-1)	---	---	OBSERVE OR PERFORM AS NOTED (4)
b. INSPECTION TASKS DURING BOLTING (OBSERVE THE QA TASKS LISTED IN AISC 300 TABLE N6.6-2)	---	---	OBSERVE (4)
1) PRE-TENSIONED AND SLIP-CRITICAL JOINTS	---	---	---
a) NOT USED	---	---	---
b) DIRECT TENSION INDICATOR	---	Y	PERIODIC
c) TWIST-OFF TYPE TENSION CONTROL BOLT	---	Y	PERIODIC
2) SNUG-TIGHT JOINTS	---	Y	PERIODIC
c. INSPECTION TASKS AFTER BOLTING (PERFORM TASKS FOR EACH BOLTED CONNECTION IN ACCORDANCE WITH QA TASKS LISTED IN AISC 300, TABLE N6.6-3)	SHOP (3) AND FIELD INSPECTION AND TESTING	Y	PERFORM (4)

1705.2.2 COLD FORMED STEEL DECK

TABLE 1.1 INSPECTION OR EXECUTION TASKS PRIOR TO DECK PLACEMENT

TASK	Y/N	EXTENTS			
			Y	R	R
A. VERIFY COMPLIANCE OF MATERIAL (DECK AND ALL DECK ACCESSORIES) WITH CONSTRUCTION DOCUMENTS, INCLUDING PROFILE, MATERIAL PROPERTIES, AND BASE METAL THICKNESS	Y	EACH SUBMITTAL			
B. DOCUMENT ACCEPTANCE OR REJECTION OF DECK AND DECK ACCESSORIES	Y	EACH SUBMITTAL			

TABLE 1.2 INSPECTION OR EXECUTION TASKS AFTER DECK PLACEMENT

TASK	Y/N	EXTENTS			
			Y	R	R
A. VERIFY COMPLIANCE OF DECK AND ALL DECK ACCESSORIES INSTALLATION WITH CONSTRUCTION DOCUMENTS	Y	PERIODIC			
B. VERIFY DECK MATERIALS ARE REPRESENTED BY THE MLL CERTIFICATIONS THAT COMPLY WITH THE CONSTRUCTION DOCUMENTS	Y	PERIODIC			
C. DOCUMENT ACCEPTANCE OR REJECTION OF INSTALLATION OF DECK AND DECK ACCESSORIES	Y	PERIODIC			

TABLE 1.3 INSPECTION OR EXECUTION TASKS PRIOR TO WELDING

TASK	Y/N	EXTENTS			
			Y	R	C
A. WELDING PROCEDURE SPECIFICATIONS (WPS) AVAILABLE	Y	PERIODIC			
B. MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	Y	PERIODIC			
C. MATERIAL IDENTIFICATION (TYPE/GRADE)	Y	PERIODIC			
D. CHECK WELDING EQUIPMENT	Y	PERIODIC			

TABLE 1.4 INSPECTION OR EXECUTION TASKS DURING WELDING

TASK	Y/N	EXTENTS			
			Y	R	C
A. USE OF QUALIFIED WELDERS	Y	PERIODIC			
B. CONTROL AND HANDLING OF WELDING CONSUMABLES	Y	PERIODIC			
C. ENVIRONMENTAL CONDITIONS, (WIND SPEED, MOISTURE, TEMPERATURE)	Y	PERIODIC			
D. WPS FOLLOWED	Y	PERIODIC			

TABLE 1.5 INSPECTION OR EXECUTION TASKS AFTER WELDING

TASK	Y/N	EXTENTS			
			Y	R	C
A. VERIFY SIZE AND LOCATION OF WELDS, INCLUDING SUPPORT, SIDE LAP, AND PERIMETER WELDS.	Y	PERIODIC			
B. WELDS MEET VISUAL ACCEPTANCE CRITERIA	Y	PERIODIC			
C. VERIFY REPAIR ACTIVITIES	Y	PERIODIC			
D. DOCUMENT ACCEPTANCE OR REJECTION OF WELDS	Y	PERIODIC			

1705.3 CONCRETE CONSTRUCTION

MATERIAL/ACTIVITY	APPLICABLE TO PROJECT		REFERENCED STANDARD	IBC REFERENCE
	Y/N	EXTENT		
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	Y	PERIODIC	ACI 318 CH 20, 25.2, 25.3, 28.6.1-28.6.3	1908.4
2. REINFORCING BAR WELDING: a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A660 b. INSPECT SINGLE-PASS FILLET WELDS, MAX. 5/16" c. INSPECT ALL OTHER WELDS.	Y Y Y	PERIODIC PERIODIC CONTINUOUS	AWIS D1.4 ACI 318: 28.6.4	-
3. INSPECT ANCHORS CAST IN CONCRETE.	Y	PERIODIC	ACI 318: 17.8.2	-
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS a. ADHESIVE ANCHORS INSTALLED IN HORIZONTAL OR UNFAVORABLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS. b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.4. 5. VERIFY USE OF REQUIRED DESIGN MIX.	Y Y Y	CONTINUOUS PERIODIC PERIODIC	ACI 318: 17.8.2.4 ACI 318: 17.8.2	-
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	Y	CONTINUOUS	ACI 318, CH. 19, 28.5.3, 28.5.4, ASTM C172, ASTM C31	1904.1, 1904.2, 1908.1, 1908.3, 1908.4
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	Y	CONTINUOUS	ACI 318: 28.5, 28.12	1908.6, 1908.7, 1908.8
8. VERIFY MAINTENANCE OF SPECIFIED DURING TEMPERATURE AND TECHNIQUES.	Y	PERIODIC	ACI 318: 28.5.3, 28.5.5	1908.9
9. INSPECT PRESTRESSED CONCRETE FOR: a. APPLICATION OF PRESTRESSING FORCES b. GROUTING OF BONDED PRESTRESSING TENDONS. 10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.	N N N	CONTINUOUS CONTINUOUS PERIODIC	ACI 318: 28.10	-
11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	N	PERIODIC	ACI 318: 26.11.2	-
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS FOR PREPARED OR PREBLENDED MORTAR, PRESTRESSING GROUT, AND GROUT OTHER THAN SELF-CONSOLIDATING GROUT.	Y	PERIODIC	ACI 318: 26.11.1, 26b	-
13. CONCRETE STRENGTH TESTING AND DIMENSIONAL COMPLIANCE WITH CONSTRUCTION DOCUMENTS.	Y	PERIODIC	-	-

(a) WHEN APPLICABLE, SEE ALSO SECTION 1705.12, SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE.

(b) SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION SHALL BE INCLUDED IN THE RESEARCH REPORT FOR THE ANCHOR ISSUED BY THE APPROVED SOURCE IN ACCORDANCE WITH 17.8.2 AND OTHER QUALIFICATION PROCEDURES, WHERE SPECIFIC REQUIREMENTS ARE NOT PROVIDED. SPECIAL INSPECTION REQUIREMENTS SHALL BE SPECIFIED BY THE REGISTERED DESIGN PROFESSIONAL AND SHALL BE APPROVED BY THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF THE WORK.

1705.4 MASONRY CONSTRUCTION - LEVEL 2 VERIFICATION AND INSPECTION

MATERIAL/ACTIVITY	APPLICABLE TO PROJECT			
	Y/N	LEVEL 1	LEVEL 2	LEVEL 3
MINIMUM VERIFICATION REQUIREMENTS				
1. PRIOR TO CONSTRUCTION, VERIFICATION OF COMPLIANCE OF SUBMITTALS.	Y	R	R	R
2. PRIOR TO CONSTRUCTION, VERIFICATION OF FM AND FMAC, EXCEPT WHERE SPECIFICALLY EXEMPT BY THE CODE.	Y	NR	R	R
3. DURING CONSTRUCTION, VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) WHEN SELF-CONSOLIDATING GROUT IS DELIVERED TO THE PROJECT SITE.	Y	NR	R	R
4. DURING CONSTRUCTION, VERIFICATION OF FM AND FMAC FOR EVERY 5000 SQUARE FEET.	N	NR	NR	R
5. DURING CONSTRUCTION, VERIFICATION OF PROPORTIONS OF MATERIALS AS DELIVERED TO THE PROJECT SITE FOR PREPARED OR PREBLENDED MORTAR, PRESTRESSING GROUT, AND GROUT OTHER THAN SELF-CONSOLIDATING GROUT.	N	NR	NR	R
MINIMUM INSPECTION REQUIREMENTS				
1. AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE: a. PROPORTIONS OF SITE-PREPARED MORTAR b. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGE c. GRADE, TYPE AND SIZE OF REINFORCEMENT, CONNECTORS, ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGE d. PRESTRESSING TECHNIQUE e. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY f. SAMPLE PANEL CONSTRUCTION	Y	NR	P	P
2. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE: a. GROUT SPACE b. PLACEMENT OF PRESTRESSING TENDONS AND ANCHORAGES c. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND ANCHOR BOLTS d. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS	Y	NR	P	C
3. VERIFY COMPLIANCE OF THE FOLLOWING DURING CONSTRUCTION: a. MATERIALS AND PROCEDURES WITH THE APPROVED SUBMITTALS b. PLACEMENT OF MASONRY UNITS AND MORTAR JOINT CONSTRUCTION c. SIZE AND LOCATION OF STRUCTURAL MEMBERS d. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION e. WELDING OF REINFORCEMENT f. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40 DEG F) OR HOT WEATHER (TEMPERATURE ABOVE 90 DEG F) g. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE h. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE	Y	NR	P	P
1. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	N	NR	CH / PH	C
4. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS	Y	NR	P	C

(a) FREQUENCY REFERS TO THE FREQUENCY OF INSPECTION, WHICH MAY BE CONTINUOUS DURING THE LISTED TASK OR PERIODICALLY DURING THE LISTED TASK, AS DEFINED IN THE TABLE.

NR = NOT REQUIRED, R = REQUIRED, P = PERIODIC, C = CONTINUOUS

(b) REQUIRED FOR THE FIRST 5000 SQUARE FEET OF AAC MASONRY

(c) REQUIRED AFTER THE FIRST 5000 SQUARE FEET OF AAC MASONRY.

1705.6 SOILS

MATERIAL/ACTIVITY	SERVICE	APPLICABLE TO PROJECT	
		Y/N	EXTENT
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	FIELD INSPECTION	Y	PERIODIC
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	FIELD INSPECTION	Y	PERIODIC
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	FIELD INSPECTION	Y	PERIODIC
4. VERIFY USE OF PROPER MATERIALS, DENSITIES, AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	FIELD INSPECTION	Y	CONTINUOUS
5. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	FIELD INSPECTION	Y	PERIODIC

1705.16 EXTERIOR INSULATION AND FINISH SYSTEM (EIFS)

MATERIAL/ACTIVITY	SERVICE	APPLICABLE TO PROJECT	
		Y/N	EXTENT
1. VERIFY MATERIALS, DETAILS AND INSTALLATIONS ARE PER THE APPROVED CONSTRUCTION DOCUMENTS. A. MANDATORY EXCEPT FOR APPLICATIONS INSTALLED OVER MASONRY OR CONCRETE WALLS, OR WHERE INSTALLED OVER A WATER RESISTIVE BARRIER WITH A MEANS OF DRAINING MOISTURE TO THE EXTERIOR.	FIELD INSPECTION	Y	PERIODIC
2. INSPECTION OF WATER-RESISTIVE BARRIER OVER SHEATHING SUBSTRATE. A. MANDATORY WHERE WATER-RESISTIVE BARRIER COATING IS INSTALLED OVER SHEATHING SUBSTRATE.	FIELD INSPECTION	Y	PERIODIC (PER REQUIREMENTS OF ASTM E2570)

NOTES:

1. THE INSPECTION AND TESTING AGENT OR AGENTS, SHALL BE ENGAGED BY THE OWNER OR THE OWNER'S AGENT, AND NOT BY THE CONTRACTOR OR SUBCONTRACTOR WHOSE WORK IS TO BE INSPECTED OR TESTED. ANY CONFLICT OF INTEREST MUST BE DISCLOSED TO THE BUILDING OFFICIAL PRIOR TO COMMENCING WORK. THE QUALIFICATIONS OF THE SPECIAL INSPECTOR AND/OR TESTING AGENCIES MAY BE SUBJECT TO THE APPROVAL OF THE BUILDING OFFICIAL AND/OR THE DESIGN PROFESSIONAL.

2. SUBMIT A LIST OF THE SPECIAL INSPECTORS ON A SEPARATE DOCUMENT TO THE BUILDING OFFICIAL AND THE DESIGN PROFESSIONAL.

3. SPECIAL INSPECTIONS AS REQUIRED BY SECTION IBC 1704.2.5 ARE NOT REQUIRED WHERE THE FABRICATOR IS APPROVED IN ACCORDANCE WITH IBC SECTION 1704.2.5.1.

4. OBSERVE ON A RANDOM BASIS, OPERATIONS NEED NOT BE DELAYED BY THESE INSPECTIONS. PERFORM THESE TASKS FOR EACH WELDED JOINT, BOLTED CONNECTION, OR STEEL ELEMENT.

5. NON-DESTRUCTIVE TESTING (NDT) OF WELDS COMPLETED IN AN APPROVED FABRICATORS SHOP MAY BE PERFORMED BY THAT FABRICATOR WHEN APPROVED BY THE A/E. REFER TO AISC 300, 7.

DEFINITIONS:

- SPECIAL INSPECTION: INSPECTION OF CONSTRUCTION REQUIRING THE EXPERTISE OF AN APPROVED SPECIAL INSPECTOR IN ORDER TO ENSURE COMPLIANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.
- SPECIAL INSPECTOR: QUALIFIED FIRM OR INDIVIDUAL RESPONSIBLE FOR PERFORMING SPECIFIC TESTS OR INSPECTIONS AS PART OF THE SPECIAL INSPECTION PROGRAM.
- PERIODIC SPECIAL INSPECTION: THE PART TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK, MAY BE ALLOWED WHEN COMPLIANCE OF THE WORK OR PRODUCT CAN BE DETERMINED AFTER BEING INCORPORATED INTO THE STRUCTURE.
- CONTINUOUS SPECIAL INSPECTION: THE FULL TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED.

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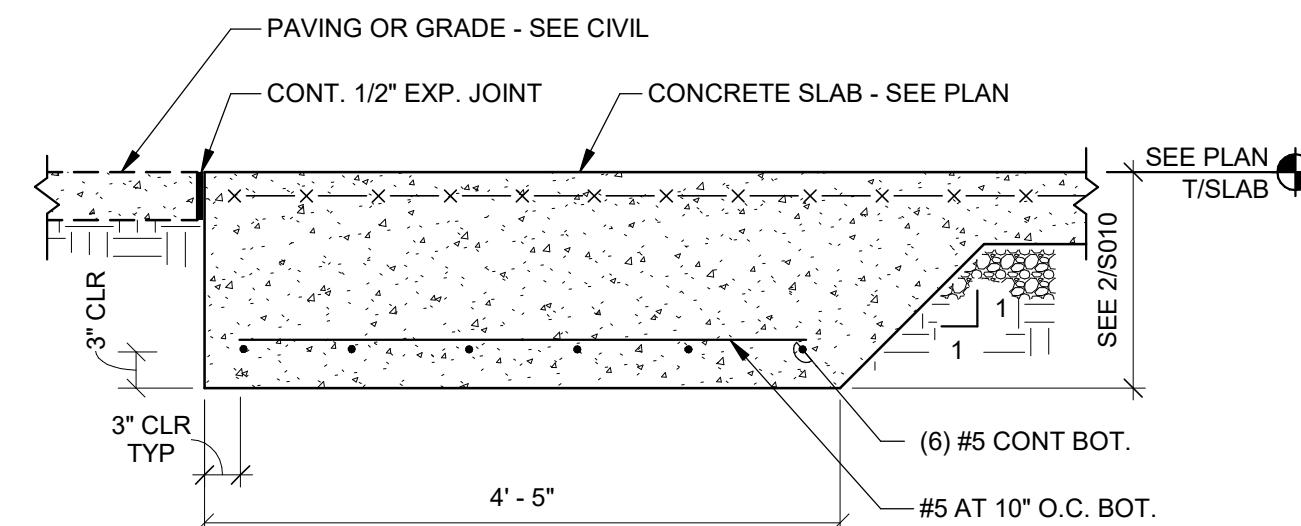
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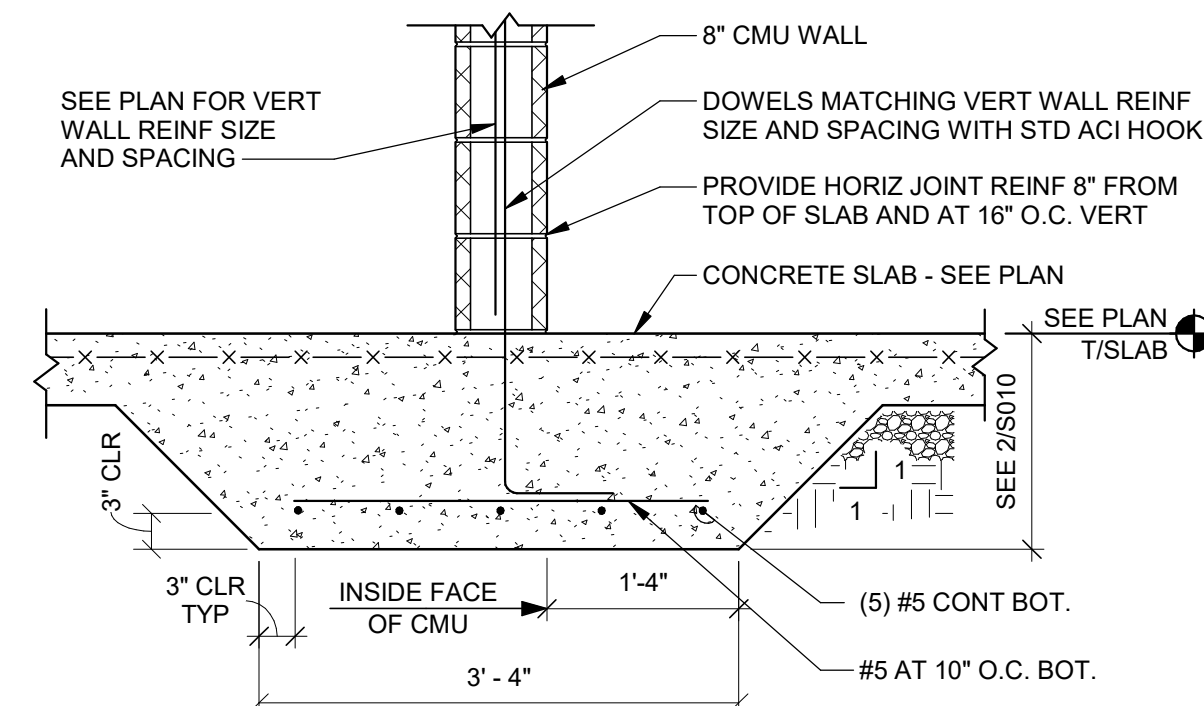
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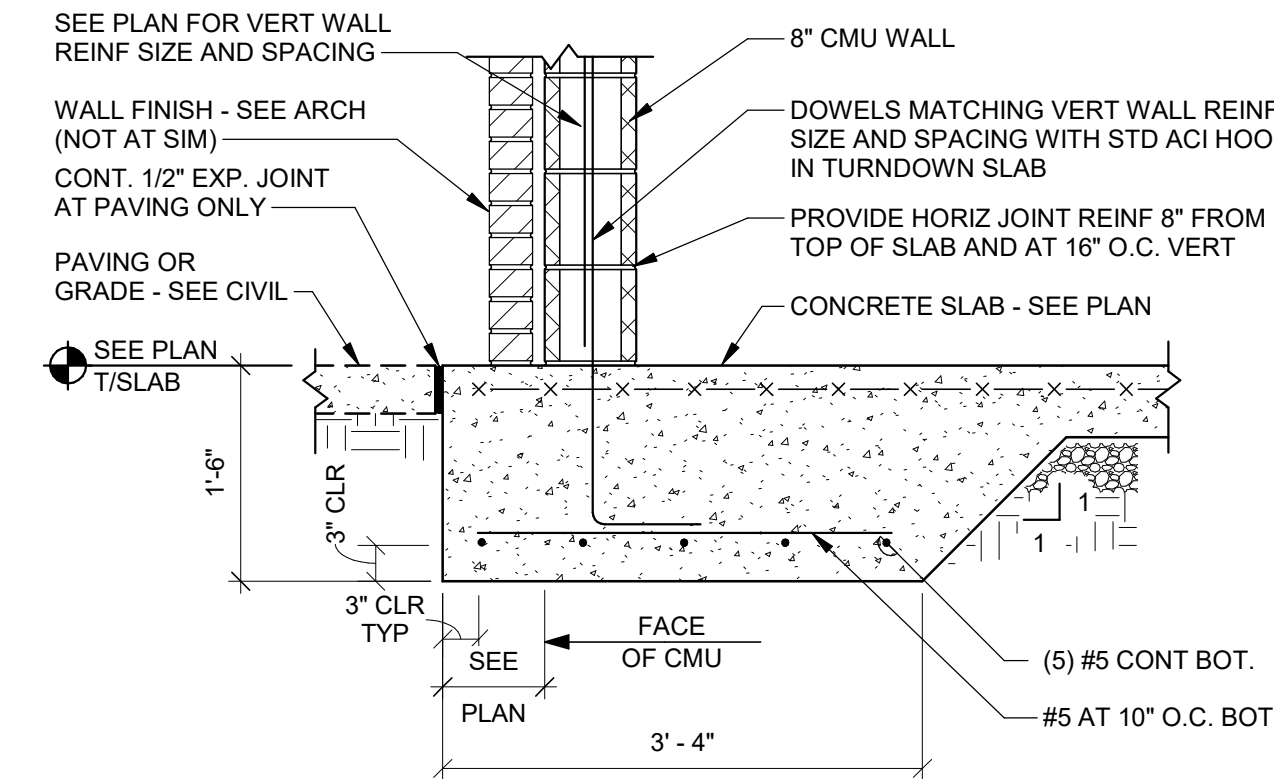
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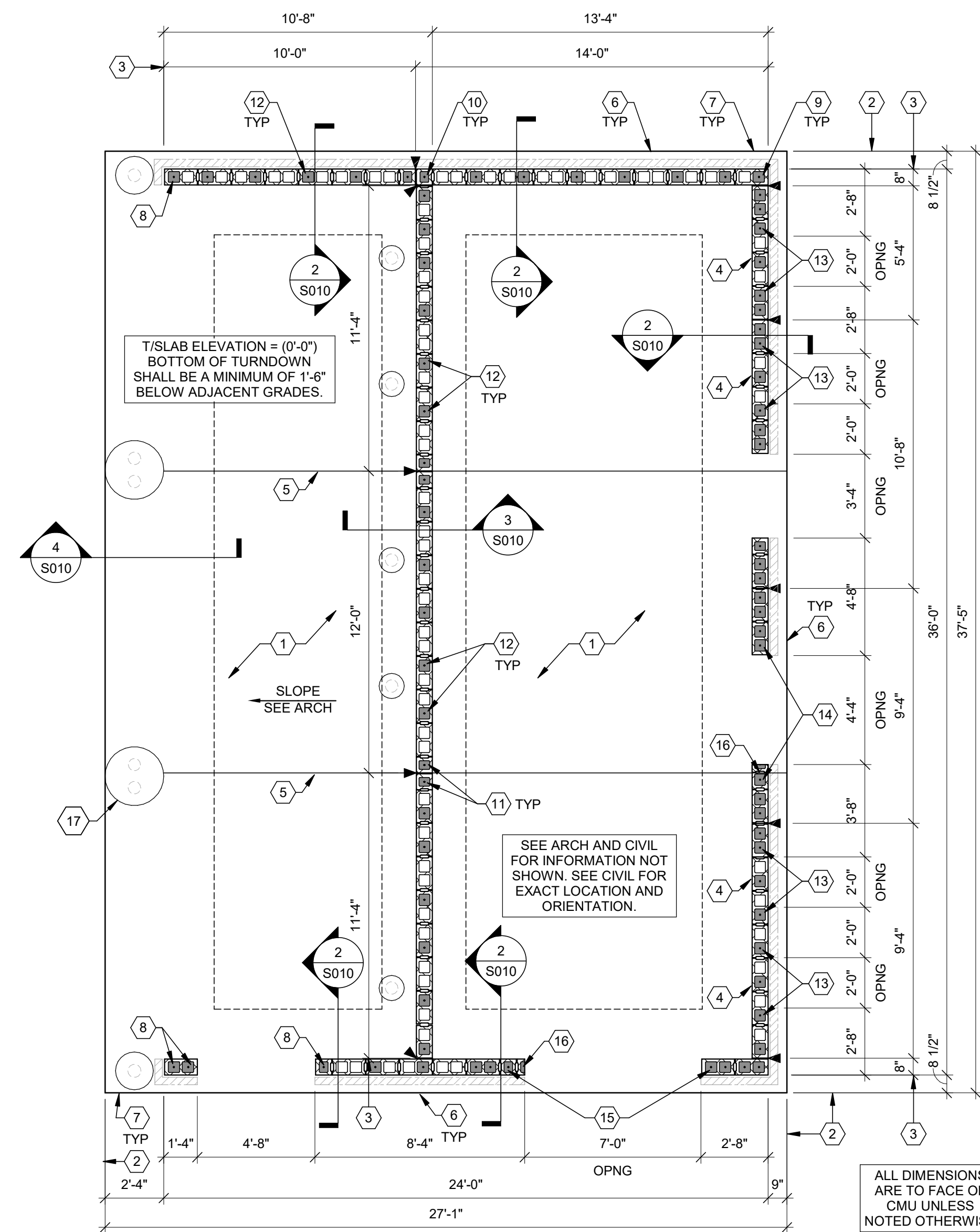
4 DUMPSTER SLAB TURNDOWN
3/4" = 1'-0"



3 ENCLOSURE SLAB TURNDOWN
3/4" = 1'-0"



2 TYPICAL ENCLOSURE SLAB TURNDOWN
3/4" = 1'-0"



1 DUMPSTER ENCLOSURE FOUNDATION PLAN
1/4" = 1'-0"
C_LH_V2

GENERAL NOTES

1. PROVIDE STEEL OR CAST IRON PIPE SLEEVE FOR PIPES PASSING UNDER FOOTINGS.
2. REFER TO ARCHITECTURAL AND CIVIL DRAWINGS FOR ANY SIDEWALK, PLANTER, OR PAVER LOCATIONS AND DETAILS.
3. CONDUITS AND PIPES EMBEDDED IN SLABS SHALL NOT BE LARGER IN OUTSIDE DIMENSION THAN ONE THIRD THE OVERALL THICKNESS OF THE SLAB.
4. CONDUITS AND PIPES EMBEDDED IN SLABS SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS OR WIDTHS ON CENTER.
5. A MINIMUM SLAB THICKNESS OF 2 1/2" MUST BE MAINTAINED OVER ITEMS EMBEDDED IN THE SLAB.
6. COORDINATE SLOPED SLABS AND DRAIN LOCATIONS WITH ARCHITECTURAL AND CIVIL DRAWINGS.

LEGEND

INDICATES CMU CONTROL JOINT - SEE ARCH FOR DETAILS AND ADDITIONAL INFORMATION.

SHEET KEYNOTES

1. 6" CONCRETE SLAB REINFORCED WITH #6-W2.9XW2.9 WWF (IN SHEETS) LOCATED 2" FROM TOP OF SLAB. SEE GEOTECHNICAL REPORT FOR BASE MATERIAL.
2. EDGE OF SLAB.
3. MASONRY EXPANSION JOINT - SEE ARCH FOR DETAILS.
4. OPENING ABOVE FINISHED FLOOR ELEVATION - SEE ARCH FOR HEIGHT AND LOCATION.
5. SLAB CONTROL JOINT - SEE 5/S430. SIMILAR
6. PROVIDE 3/4" CHAMFER AT EDGE OF SLAB TURNDOWN WHERE NOT ADJACENT TO PAVING - SEE ARCH AND CIVIL. TYP.
7. PROVIDE CORNER BARS IN SLAB TURNDOWN TO MATCH SIZE OF LONGITUDINAL REINFORCEMENT - TYP.
8. (1) VERTICALLY REINFORCED CELL AT END OF WALL - TYP.
9. (1) VERTICALLY REINFORCED CELL AT CORNERS - TYP.
10. (1) VERTICALLY REINFORCED CELL AT CMU WALL INTERSECTION - TYP.
11. (1) VERTICALLY REINFORCED CELL EACH SIDE OF CMU EXPANSION JOINT - TYP.
12. TYPICAL CMU WALL VERTICAL REINFORCEMENT U.N.O.: (1) #6 AT 24" O.C. - FULLY GROUT ALL REINFORCED CELLS.
13. (1) VERTICALLY REINFORCED CELL EACH END OF MASONRY OPENING UP TO 2'-0".
14. (3) VERTICALLY REINFORCED CELLS EACH END OF MASONRY OPENING UP TO 4'-4".
15. (3) VERTICALLY REINFORCED CELLS EACH END OF MASONRY OPENING UP TO 7'-0".
16. FULLY GROUT PARTIAL CELLS AT MASONRY OPENINGS.
17. GATE POST AND BOLLARD FOUNDATIONS, TYP. SEE ARCH/

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FLORIDA LICENSE #0000000000
JBA PROJECT #: 20131

PROJECT MANAGER:

SDH

CHECKED BY:

BWM

DRAWN BY:

JBA

ISSUE/REVISION RECORD

DATE	DESCRIPTION
02/19/22	PERMIT SET
05/21/20	REV1-SITE PLAN PKG
10/07/20	REV2-PHOTOMETRIC
10/20/20	REV3-AHJ COMMENTS
01/11/21	REFRESH 2022_0107
02/26/21	REV4-P&Z COMMENTS
11/05/21	REFRESH 2021_0401
02/07/22	REFRESH 2021_0507
08/12/22	REFRESH 2022_0202

RaceTrac

RACETRAC INC.
200 GALLERIA PARKWAY SOUTHEAST
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ATLANTA, GEORGIA 30339
(770) 431-7600

PROJECT NAME

VERO BEACH, FL

VERO BEACH FL 32966
8990 20TH STREET

RACETRAC STORE NUMBER

#1422

PROTOTYPE SERIES TC

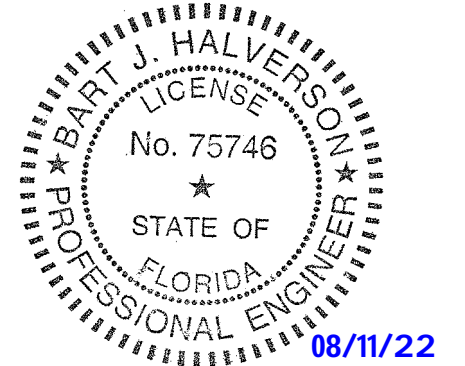
2022 LH SV 0202

PLAN MODIFICATION NOTICE

SPB NO. 0202 DATE 07/08/22

STANDARD PLAN BULLETINS (SPB) MODIFY THE PROTOTYPE SERIES SET NOTED ABOVE. THE LISTED SPB REPRESENTS THE LATEST MODIFICATION INCORPORATED TO THIS PROTOTYPE SERIES SET AT ORIGINAL RELEASE. THE ISSUE/REVISION RECORD COLUMN ABOVE LISTS ANY REVISIONS OR SPB INCORPORATED IN THIS SET AFTER THE ORIGINAL RELEASE. CONTACT RACETRAC ENGINEERING AND CONSTRUCTION FOR ANY SUBSEQUENT BULLETINS NOT INCORPORATED HEREIN.

PROFESSIONAL SEAL



PROJECT NUMBER

63200213

SHEET TITLE

ENCLOSURE FOUNDATION PLANS AND DETAILS

SHEET NUMBER

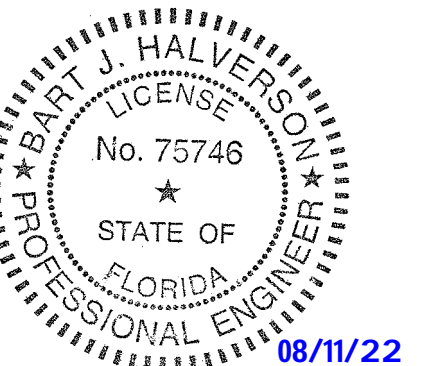
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INCORPORATED SPB:

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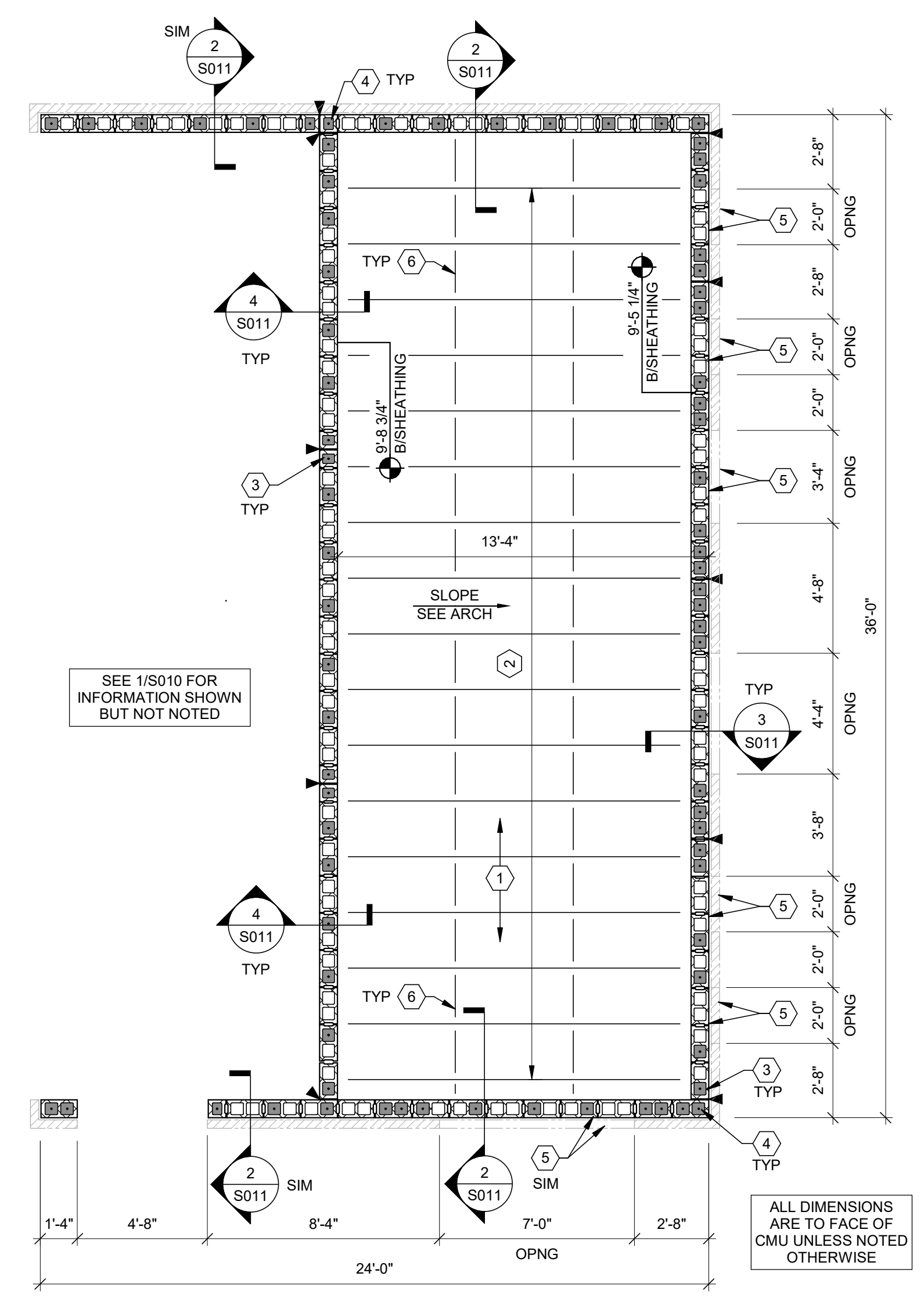
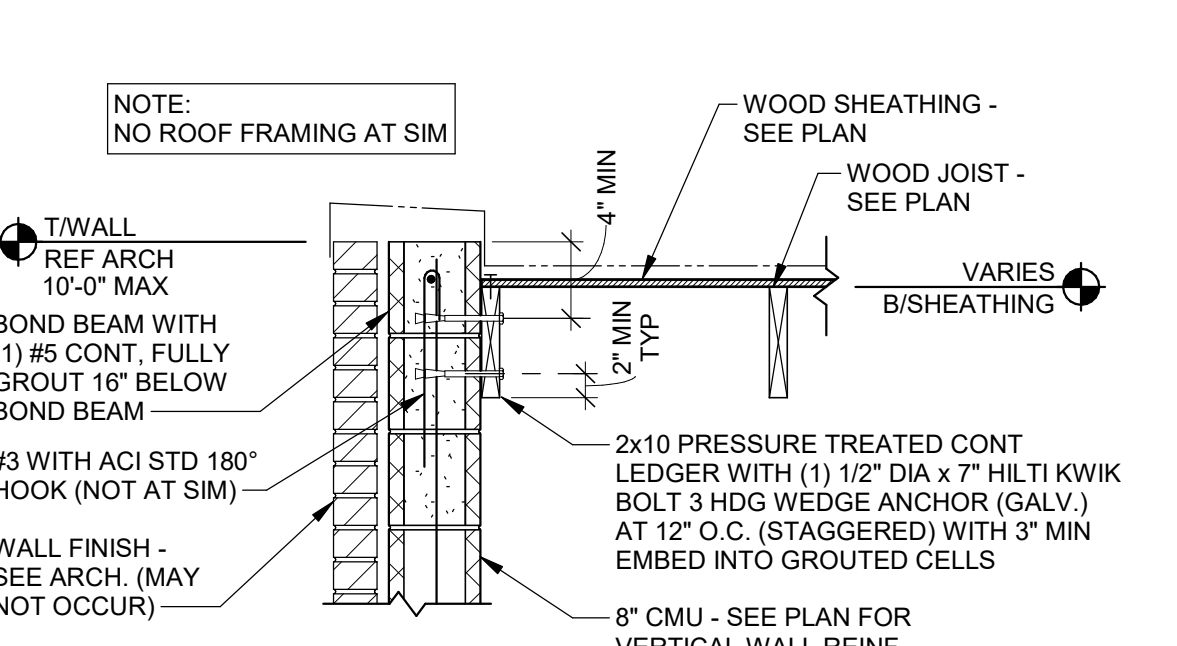
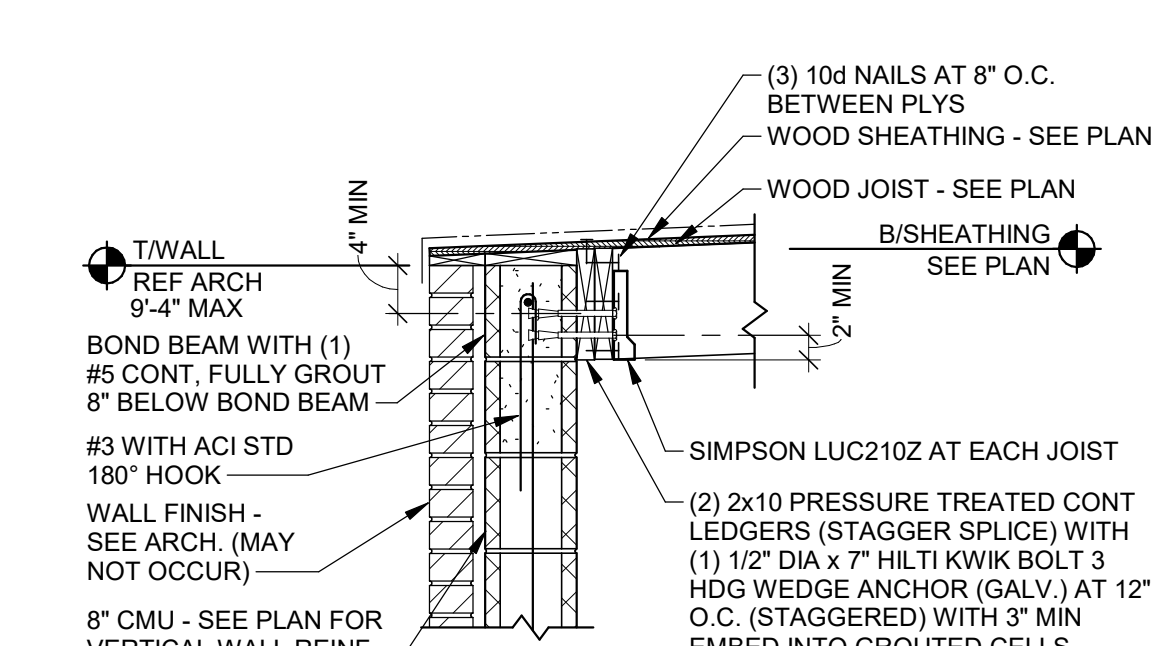
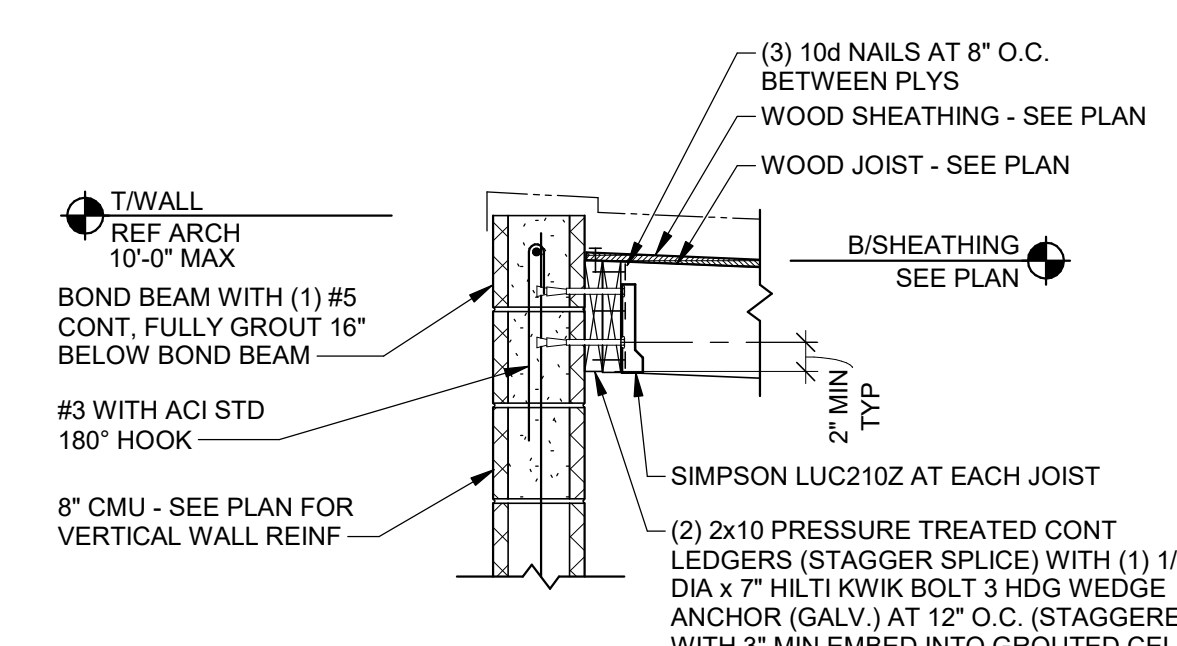
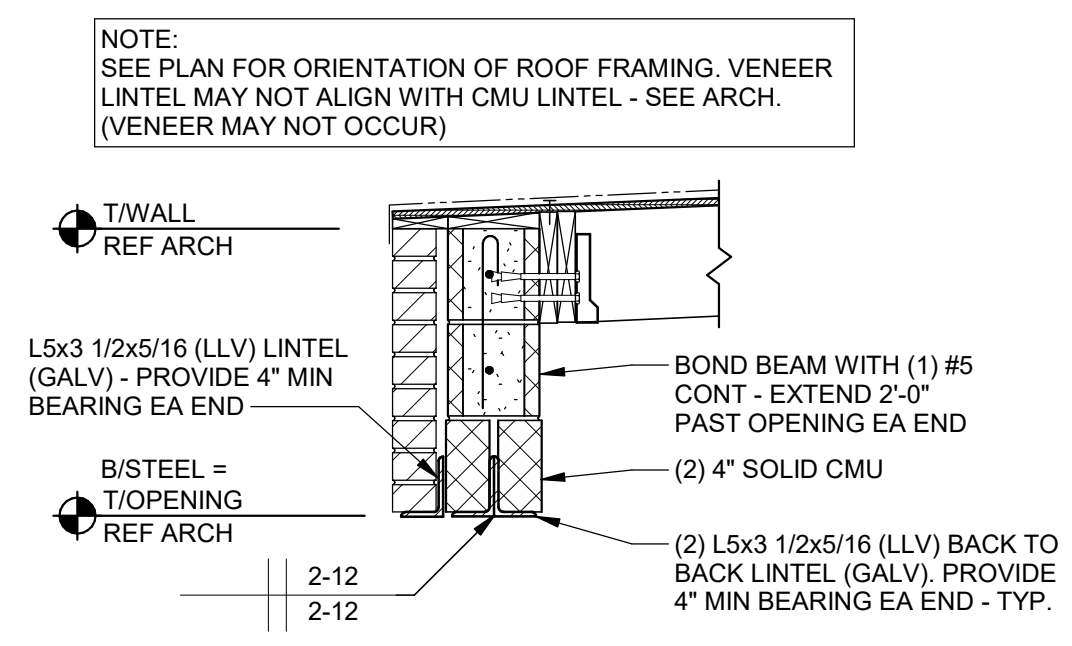
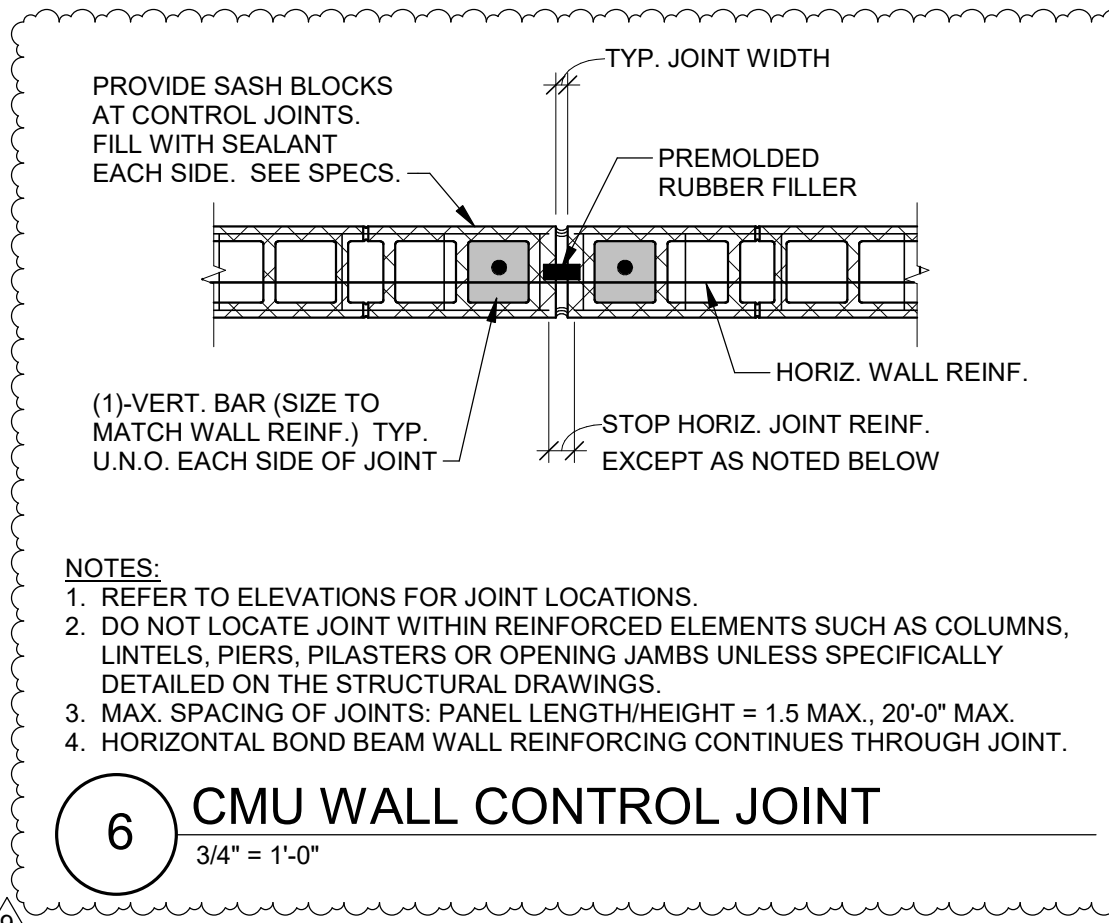


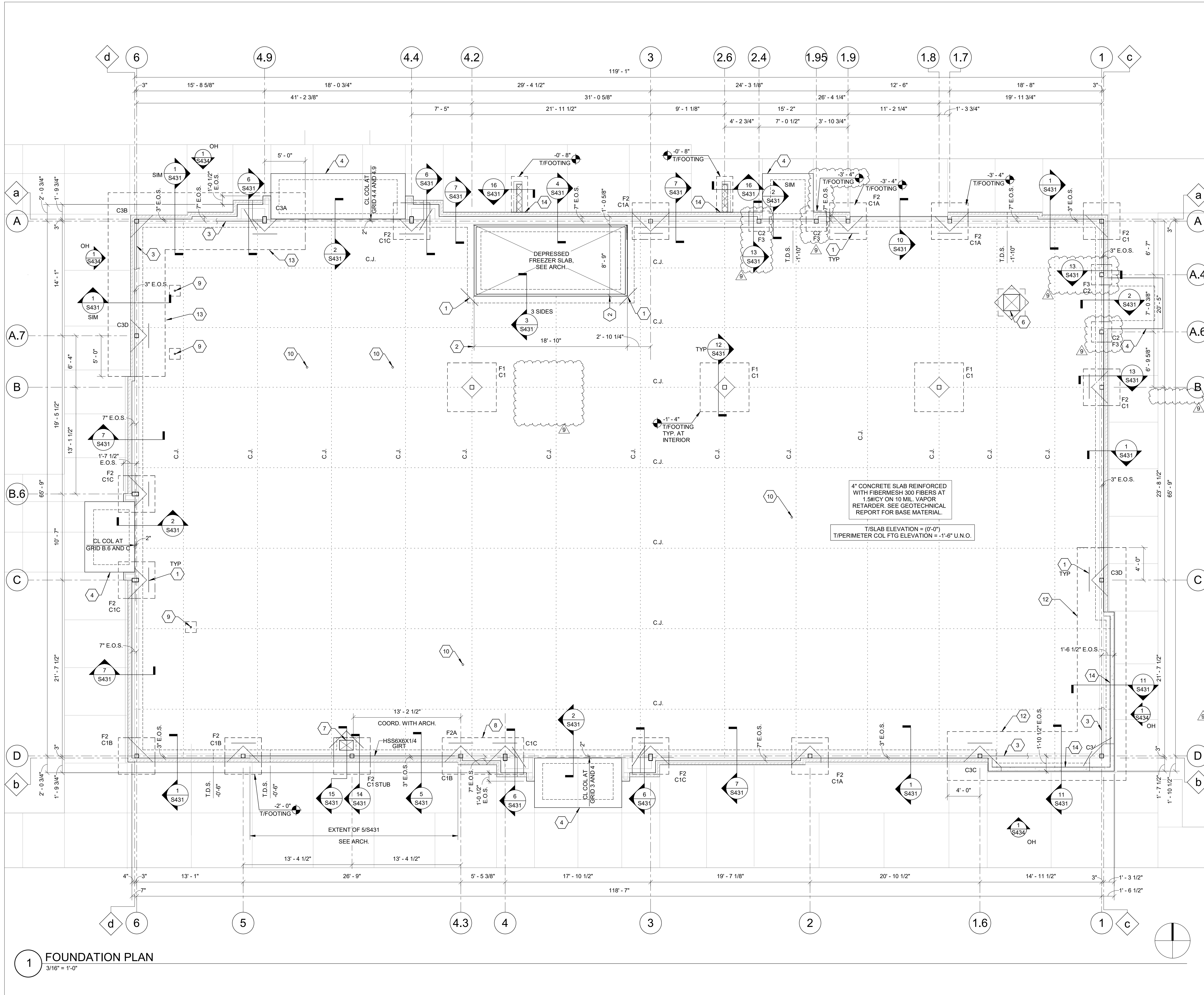
LEGEND

INDICATES CMU CONTROL JOINT - SEE ARCH FOR DETAILS AND ADDITIONAL INFORMATION.

SHEET KEYNOTES

- 19/32" MIN THICKNESS, 40/20 APA SPAN RATED CDX WOOD SHEATHING, EXPOSURE 1, UNBLOCKED, ATTACH WITH 10d NAILS AT 6" OC AT PANEL EDGES AND AT 12" OC AT INTERMEDIATE SUPPORTS (FIELD). RUN PANELS PERPENDICULAR TO FRAMING AND STAGGER END JOINTS.
- 2x10 PRESSURE TREATED (NO. 2) WOOD JOISTS AT 24" O.C. MAX.
- TOP OF WALL BOND BEAM REINF SHALL BE CONTINUOUS THROUGH CONTROL JOINTS.
- PROVIDE 2'-0"x2'-0" CORNER BARS IN BOND BEAM AT TOP OF WALL AT CORNERS AND WALL INTERSECTIONS.
- SEE DETAIL S/S011 FOR STEEL LINTEL AT CMU AND BRICK VENEER AT OPENING. BRICK VENEER MAY NOT OCCUR - SEE ARCH. AND PLAN.
- 2x10 BLOCKING AT (3) EQUAL SPACES - ATTACH EA END WITH (2) 10d TOENAILS.





GENERAL NOTES

- STEP FOOTINGS BELOW MECHANICAL, ELECTRICAL OR PLUMBING LINES AS REQUIRED TO AVOID INTERFERENCE. SEE TYPICAL FOOTING STEP DETAIL. COORDINATE WITH OTHER TRADES. PROVIDE STEEL OR CAST IRON PIPE SLEEVE TWO PIPE SIZES GREATER THAN THE PIPE PASSING THROUGH WALLS.
- PROVIDE STEEL OR CAST IRON PIPE SLEEVE FOR PIPES PASSING UNDER FOOTINGS.
- BOTTOM OF FOUNDATION TURNDOWN SHALL BE A MINIMUM OF 1'-6" BELOW ADJACENT GRADES.
- PROVIDE LEVELING NUTS ON ANCHOR RODS AT ALL COLUMN LOCATIONS.
- SIDEWALK SLAB SHALL BE 4" THICK CONCRETE REINFORCED WITH FIBERMESH 300 FIBERS AT 1-1/2 #/CY. SEE GEOTECHNICAL REPORT FOR BASE MATERIAL.
- REFER TO ARCHITECTURAL AND CIVIL DRAWINGS FOR ANY SIDEWALK, PLANTER, OR PAVEMENT LOCATIONS AND DETAILS.
- C.J. = FLOOR SLAB CONTROL OR CONSTRUCTION JOINTS. SEE DETAILS ON SHEET S430. PROVIDE CONCRETE SLAB CONTROL JOINTS PER SHEET S000. PROVIDE FIRST JOINT NO MORE THAN 6'-0" FROM TYPICAL EDGE OF SLAB.
- CONDUITS AND PIPES EMBEDDED IN SLABS SHALL NOT BE LARGER IN OUTSIDE DIMENSION THAN ONE THIRD THE OVERALL THICKNESS OF THE SLAB.
- CONDUITS AND PIPES EMBEDDED IN SLABS SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS OR WIDTHS ON CENTER.
- A MINIMUM SLAB THICKNESS OF 2 1/2" MUST BE MAINTAINED OVER ITEMS EMBEDDED IN THE SLAB.
- COORDINATE DEPRESSED FREEZER SLAB SIZE WITH ARCHITECTURAL AND APPROVED VENDOR DRAWINGS.

SOIL TREATMENT NOTES

- FURNISH AND INSTALL COMPLETE SOILS TREATMENT WITH TERMITICIDE UNDER AND ADJACENT TO BUILDING TO PROVIDE UNIFORM TOXIC CONTINUOUS TREATED ZONE IN ALL ROUTES OF TERMITE ENTRY. COORDINATE SOIL TREATMENT APPLICATION WITH EXCAVATION, FILLING, GRADING AND CONCRETE OPERATIONS. TREAT SOIL UNDER FOOTINGS, GRADE BEAMS, AND GROUND-SUPPORTED SLABS BEFORE CONSTRUCTION. FOR INTERIOR SLAB-ON-GRADE CONCRETE, COORDINATE WORK SO VAPOR RETARDER CAN BE INSTALLED AS SOON AS POSSIBLE AFTER APPLICATION OF TERMITICIDE. PROTECTION ON TOP OF SOIL BASE OR AGGREGATE BASE. GENERAL CONTRACTOR/APPLICATOR SHALL PROVIDE CERTIFICATES REQUIRED BY ANY AUTHORITIES HAVING JURISDICTION.
- APPLICATOR SHALL BE LICENSED PEST PROFESSIONAL ACCORDING TO REGULATIONS OF AUTHORITIES HAVING JURISDICTION WITH MANUFACTURER'S CERTIFICATION TRAINING IN CORRECT APPLICATION METHODS TO APPLY TERMITICIDE CONTROL TREATMENT AND PRODUCTS IN JURISDICTION WHERE PROJECT IS LOCATED.

SHEET KEYNOTES

- RE-ENTRANT CORNER BAR, SEE DETAIL 6/S430.
- COORDINATE FINAL DIMENSIONS WITH APPROVED VENDOR DRAWINGS.
- BRACED FRAME. SEE DETAIL 3/S434 FOR BOTTOM OF BRACED FRAME.
- TURNDOWN SLAB, SEE 9/S430.
- NOT USED.
- MOP SINK DEPRESSION SLOPED TO DRAIN. SEE 13/S430 AND ARCH DRAWINGS FOR DETAILS.
- SAFE PIT. SEE 9/S431 AND 15/S431 FOR SAFE PIT DETAIL.
- CENTER FOOTING ON COLUMN GROUP.
- 1'-4"x1'-4"x0'-8" THICKENED SLAB - SEE ARCH. FOR LOCATIONS. PROVIDE (2) #4 x 4'-0" AT 2' FROM TOP OF SLAB.
- RECESSED EQUIPMENT OUTLET, COORDINATE WITH ELECTRICAL.
- 6'-0" WIDE x 1'-8" THK. MAT FOUNDATION - REINFORCE WITH (7) #6 LONGITUDINAL T&B. PROVIDE STD ACI 90 DEG. HOOK AT TOP BARS EACH END, AND WITH #6 TRANSVERSE AT 18" OC T&B. OVERLAP LONGITUDINAL REINFORCEMENT AT INTERSECTION AT COLUMN. EXTEND MAT FOUNDATION 4'-0" BEYOND COLUMN GRID, AS SHOWN.
- 7'-0" WIDE x 3'-0" THK. MAT FOUNDATION - REINFORCE WITH (7) #6 LONGITUDINAL T&B. PROVIDE STD ACI 90 DEG. HOOKS AT TOP BARS EA END, AND WITH #6 TRANSVERSE AT 12" OC T&B. OVERLAP LONGITUDINAL REINFORCEMENT AT INTERSECTION AT COLUMN. EXTEND MAT FOUNDATION 5'-0" BEYOND COLUMN GRID, AS SHOWN.
- CONCRETE CURB
EXTEND LONGITUDINAL SCREEN WALL FOOTING REINFORCEMENT INTO TURNDOWN SLAB. PROVIDE CORNER BARS PER 4/S430.
- REINFORCEMENT INTO TURNDOWN SLAB. PROVIDE CORNER BARS PER 4/S430.

FOOTING SCHEDULE

MARK	WIDTH	LENGTH	THICKNESS	REINFORCING
F1	6'-0"	6'-0"	1'-4"	(6) #5 E.W. T&B
F2	4'-6"	4'-6"	1'-4"	(5) #5 E.W. T&B
F2A	4'-6"	10'-0"	1'-4"	(5) #5 L.W. T&B AND (10) #5 S.W. T&B
F3	2'-6"	2'-6"	1'-4"	(4) #5 E.W. T&B

COLUMN SCHEDULE

MARK	SIZE	BASE PL		ANCHOR BOLTS
		SIZE	TYPE	
C1	HSS6X6X1/4	3/4"x12"x1'-0"	TYPE A	(4) 3/4" DIA x 0'-9" EMBED
C1A	HSS6X6X5/16	3/4"x12"x1'-0"	TYPE A	(4) 3/4" DIA x 0'-9" EMBED
C1B	HSS6X6X3/8	3/4"x12"x1'-0"	TYPE A	(4) 3/4" DIA x 0'-9" EMBED
C1C	HSS10X6X1/2	3/4"x12"x1'-4"	TYPE A	(4) 3/4" DIA x 0'-9" EMBED
C2	HSS6X6X1/4	3/4"x12"x1'-0"	TYPE A	(4) 3/4" DIA x 0'-9" EMBED
C3	HSS6X6X5/16	1 1/4"x16"x1'-8"	TYPE D	(6) 1 1/8" DIA x 1'-4" EMBED
C3A	HSS10X8X1/2	1"x14"x1'-6"	TYPE E	(4) 1 1/8" DIA x 1'-4" EMBED
C3B	HSS6X6X5/16	1 1/4"x16"x1'-8"	TYPE D	(6) 1 1/4" DIA x 1'-8" EMBED
C3C	HSS6X6X3/8	1"x14"x1'-2"	TYPE C	(4) 1 1/8" DIA x 1'-4" EMBED
C3D	HSS6X6X3/8	1 1/4"x14"x1'-2"	TYPE A	(4) 1 1/8" DIA x 1'-4" EMBED
CX	HSS6X6X1/4		SIM	

NOTE: SEE 8/S430 AND 3/S434 FOR BASE PLATE TYPES

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PROJECT NAME
VERO BEACH, FL

VERO BEACH FL 32966
 8990 20TH STREET

RACETRAC STORE NUMBER
#1422

PROTOTYPE SERIES TC
2022 LH SV 0202

PLAN MODIFICATION NOTICE
 SPB NO. 0202 DATE 07/08/22

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PROFESSIONAL SEAL

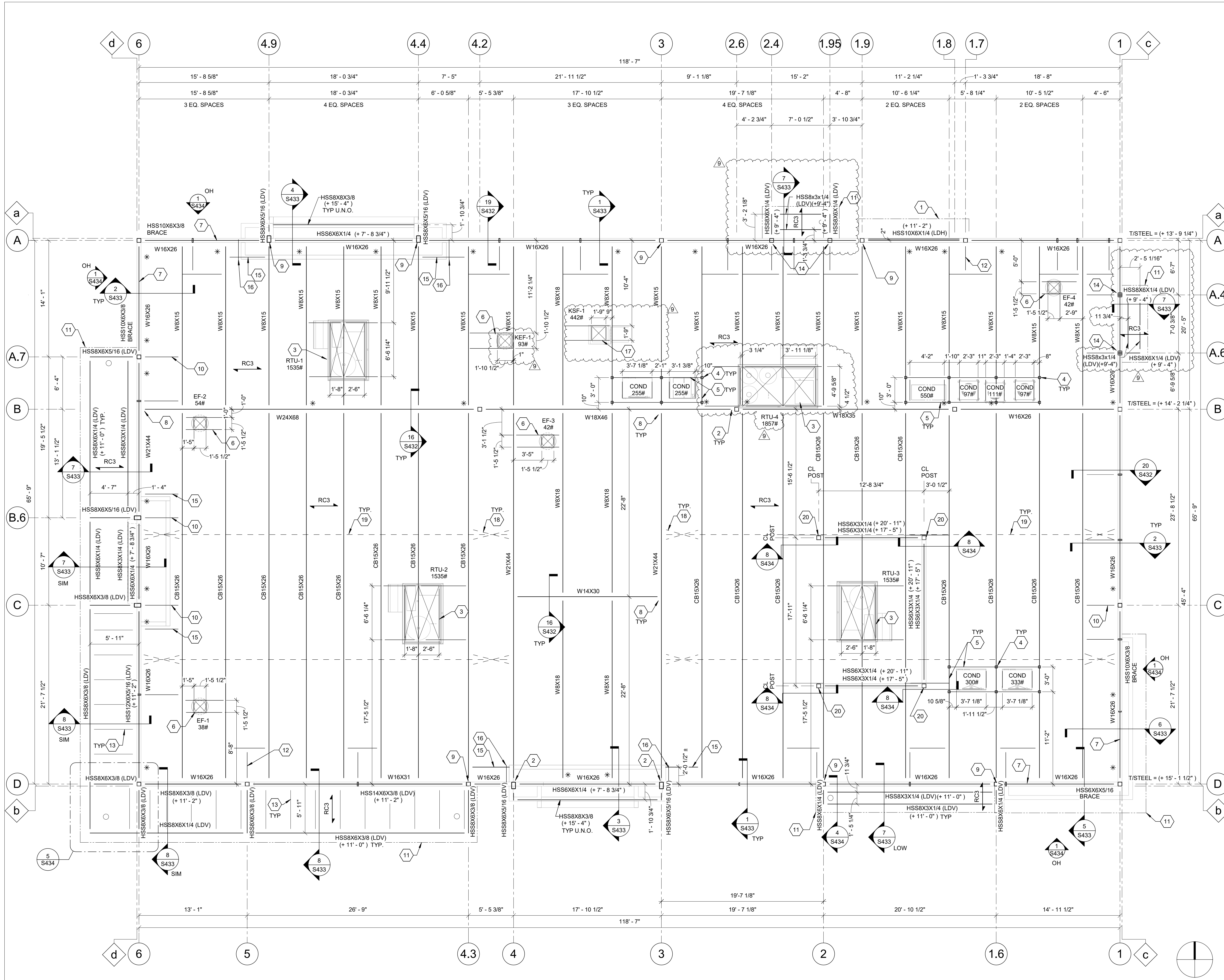
BARBARA J. HALVERSON
 LICENSE No. 75746
 STATE OF FLORIDA
 PROFESSIONAL ENGINEER
 08/11/22

PROJECT NUMBER
63200213

SHEET TITLE
FOUNDATION PLAN

SHEET NUMBER
S120

1 FOUNDATION PLAN
 3/16" = 1'-0"



1 ROOF FRAMING PLAN
3/16" = 1'-0"

GENERAL NOTES

- STRUCTURAL STEEL ERECTION SHALL BE IN ACCORDANCE WITH AISC 360, "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".
- UPLIFT BRIDGING IS REQUIRED AT ROOF BEAMS. OFFSET BRIDGING WHERE REQUIRED AT ROOF TOP MECHANICAL UNITS. REFER TO PLAN AND DETAILS FOR TYPICAL BEAM BRIDGING INFORMATION.
- ROOF TOP UNIT CURB SHALL ATTACH TO STEEL SUPPORT FRAMING WITH #12 TEK SCREWS AT 6" O.C. OR BY 5/8" DIA. PUDDLE WELDS AT 6" O.C. ALL ROOF TOP UNIT DIMENSIONS SHALL BE COORDINATED BY THE G.C. THE CURB MANUFACTURER IS TO PROVIDE HVAC AND ROOF TOP EQUIPMENT UPLIFT RESTRAINT BRACKET FOR APPLICABLE WIND SPEED.
- ROOF DECK WELD PATTERN TO SUPPORTING MEMBERS SHALL BE A 3/7 PATTERN OF 5/8" DIA. PUDDLE WELDS. DECK SIDELAP FASTENERS SHALL BE (9) #10 TEK SCREWS PER SPAN. ATTACH DECK TO PERIMETER SUPPORTS WITH 5/8" DIA. PUDDLE WELDS AT 6" O.C. FASTEN DECK AT OPENINGS FRAMES WITH #12 TEK SCREWS AT 6" O.C. REFER TO TYPICAL ROOF DECK ATTACHMENT DETAIL.
- (X-X) INDICATES TOP OF COLUMN ELEVATION ABOVE FINISHED FLOOR.
- (U.T.BM.) INDICATES TOP OF COLUMN IS BOTTOM OF BEAM.
- (X-X') INDICATES TOP OF STEEL BEAM ELEVATION ABOVE FINISHED FLOOR.
- ALL ROOF PENETRATIONS, DIMENSIONED AND NON-DIMENSIONED, ARE SUBJECT TO CHANGE DUE TO FIELD CONDITIONS. COORDINATE LOCATIONS WITH GENERAL CONTRACTOR AND OTHER TRADES PRIOR TO PENETRATING ROOF.
- REFER TO ARCHITECTURAL DRAWINGS FOR MISCELLANEOUS STEEL BRACKETS, BRACES, SUPPORTS, ECT. NOT INDICATED ON THE STRUCTURAL DRAWINGS.
- REFER TO 4/S201 FOR STEEL FRAMING SHOWN IN EXTERIOR WALLS BUT NOT NOTED.
- DIMENSIONS AT EPS AND RTUS ARE TO OUTSIDE FACE OF CURB (BY OTHERS).
- DIMENSIONS AT CONDENSERS ARE TO FACE OF EQUIPMENT HOUSING.

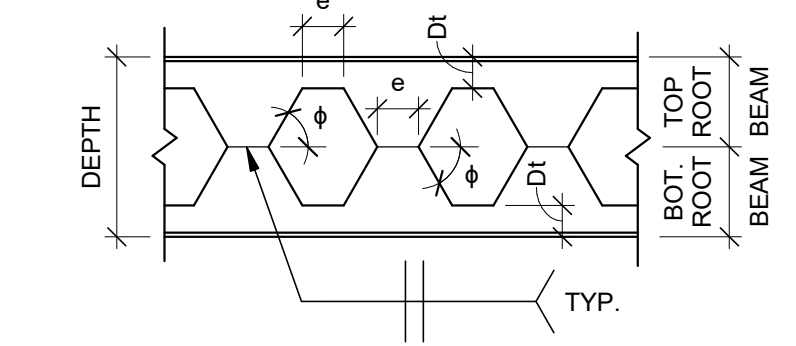
LEGEND

- RC3 INDICATES METAL ROOF DECK DIRECTION - SEE GENERAL NOTES FOR DETAILED INFORMATION.
- INDICATES BOTTOM FLANGE BRACE - SEE DETAIL 1/S433, 2/S433 OR 16/S432. WHERE NOTED AT PERIMETER WALL BEAMS ALONG NUMBERED GRIDS, PROVIDE WITHIN 2'-0" OF END OF BEAM AND AT MID-SPAN, WHERE NOTED.

SHEET KEYNOTES

- SEE ARCHITECTURAL FOR EXTERIOR OVERHEAD DOOR.
- SEE 2/S432 FOR TYP. BEAM-TO-COL. CONNECTION.
- SEE 7/S432 FOR TYP. RTU SUPPORT FRAMING.
- SEE 10/S432 FOR HSS STUB COLUMN AT EQUIPMENT.
- SEE 10/S432 FOR TYP. CONDENSER SUPPORT FRAMING.
- SEE 6/S432 FOR TYP. SUPPORT FRAMING AT ROOF PENETRATIONS.
- BRACED FRAME. SEE 2/S434 FOR TOP OF BRACED FRAME.
- SEE 3/S432 FOR TYP. BEAM-TO-BEAM CONNECTION.
- SEE 2/S432 FOR BEAM CONNECTION AT COLUMN.
- SEE 18/S432 FOR PLATE STRUT AT FACE OF COLUMN.
- FACE OF FINISH MATERIAL - SEE ARCHITECTURAL.
- SEE 6/S434 FOR STRUT AT FACE OF COLUMN.
- SEE 8/S433 FOR BRICK VENEER SUPPORT.
- SEE 1/S432 FOR WIND COLUMN CONN TO BOTTOM OF BEAM.
- C12x20.7 BELOW BRICK VENEER - ALIGN IN DECK FLUTES AND WELD TO BEAM EACH END.
- GALV. L4x4x1/4 BELOW BRICK VENEER - FIELD WELD TO CHANNEL EACH SIDE AND TO VERT LEG OF CONT EDGE ANGLE.
- SEE 7/S432 FOR MAKE-UP AIR UNIT SUPPORT FRAMING.
- L2x2x1/4 DIAGONAL X-BRIDGING AT TERMINATION OF CONTINUOUS UPLIFT BRIDGING - SEE 11/S432.
- L2x2x1/4 CONTINUOUS HORIZONTAL UPLIFT BRIDGING LOCATED AT THIRD-POINTS OF BEAM SPAN - SEE 11/S432.
- HSS6x6x1/4 POST AT EQUIPMENT SCREEN - SEE 8/S434 FOR ADDITIONAL INFORMATION.

CASTELLATED BEAM DIAGRAM AND CRITERIA



BEAM MARK	CASTELLATED BEAM CONSTRUCTION		ROOT BEAM	
	MAX	MIN	MAX	BOTTOM
CB15X26	2.625	7.75	3	62

BEAM MARK	CASTELLATED BEAM FABRICATION				DEPTH (IN)
	D1 (IN)	MAX	MIN	MAX	
CB15X26	2.625	7.75	3	62	58

- CASTELLATED BEAM NOTATIONS:
• CB(DEPTH)(ROOT BEAM WEIGHT)
• CB(DEPTH)(TOP RB WEIGHT)(BOT. RB WEIGHT)
- FINAL FABRICATED BEAM DEPTH AND WEIGHT IS APPROXIMATE

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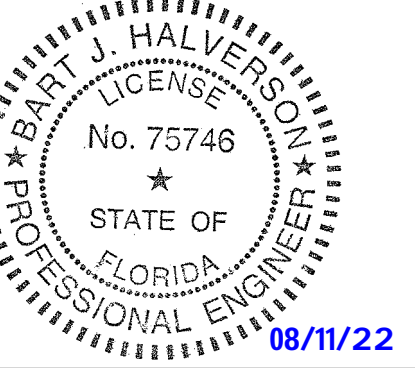
RACETRAC STORE NUMBER
#1422

PROTOTYPE SERIES TC
2022 LH SV 0202

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 SPB NO. 0202 DATE 07/08/22

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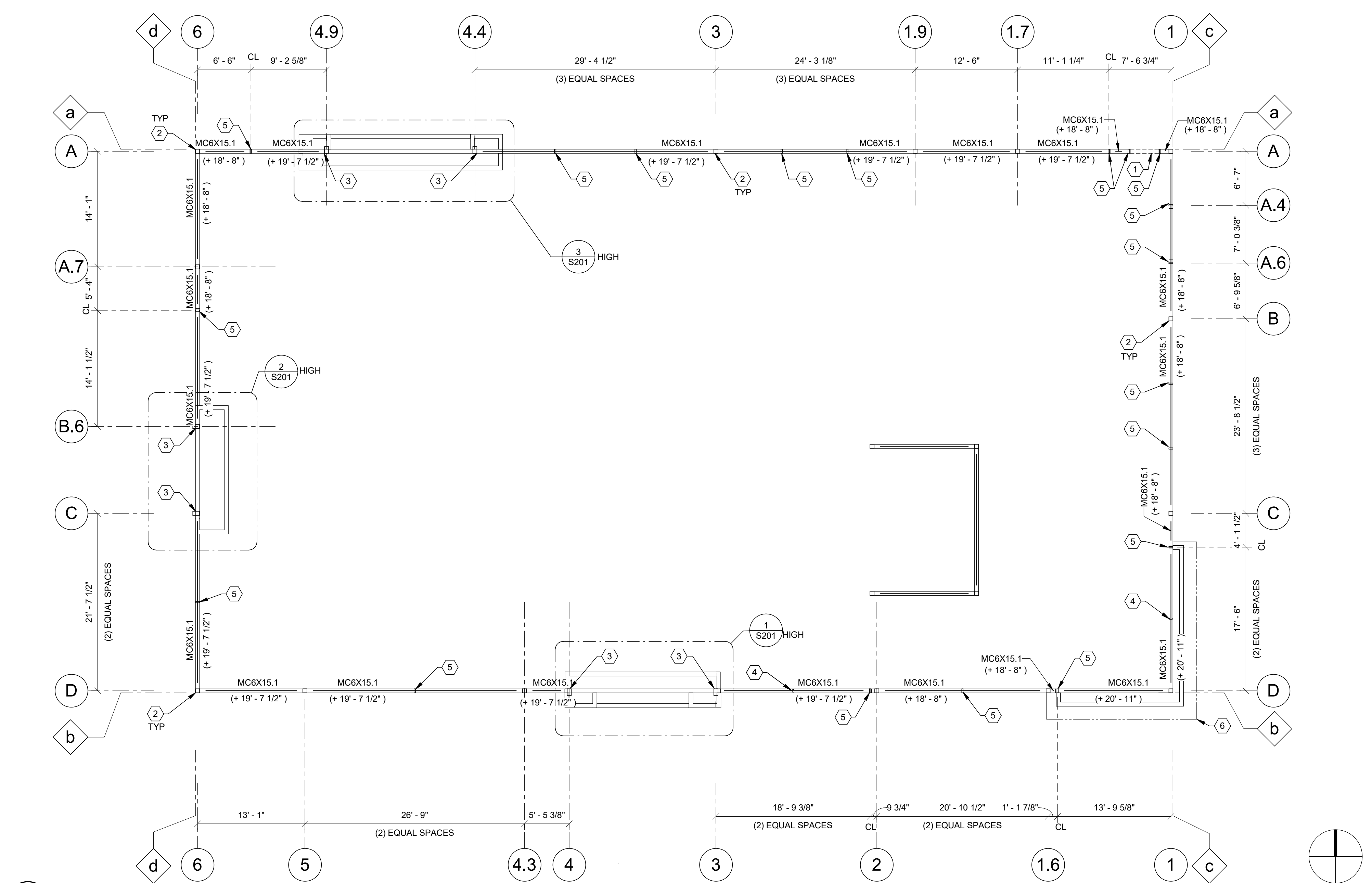


PROJECT NUMBER
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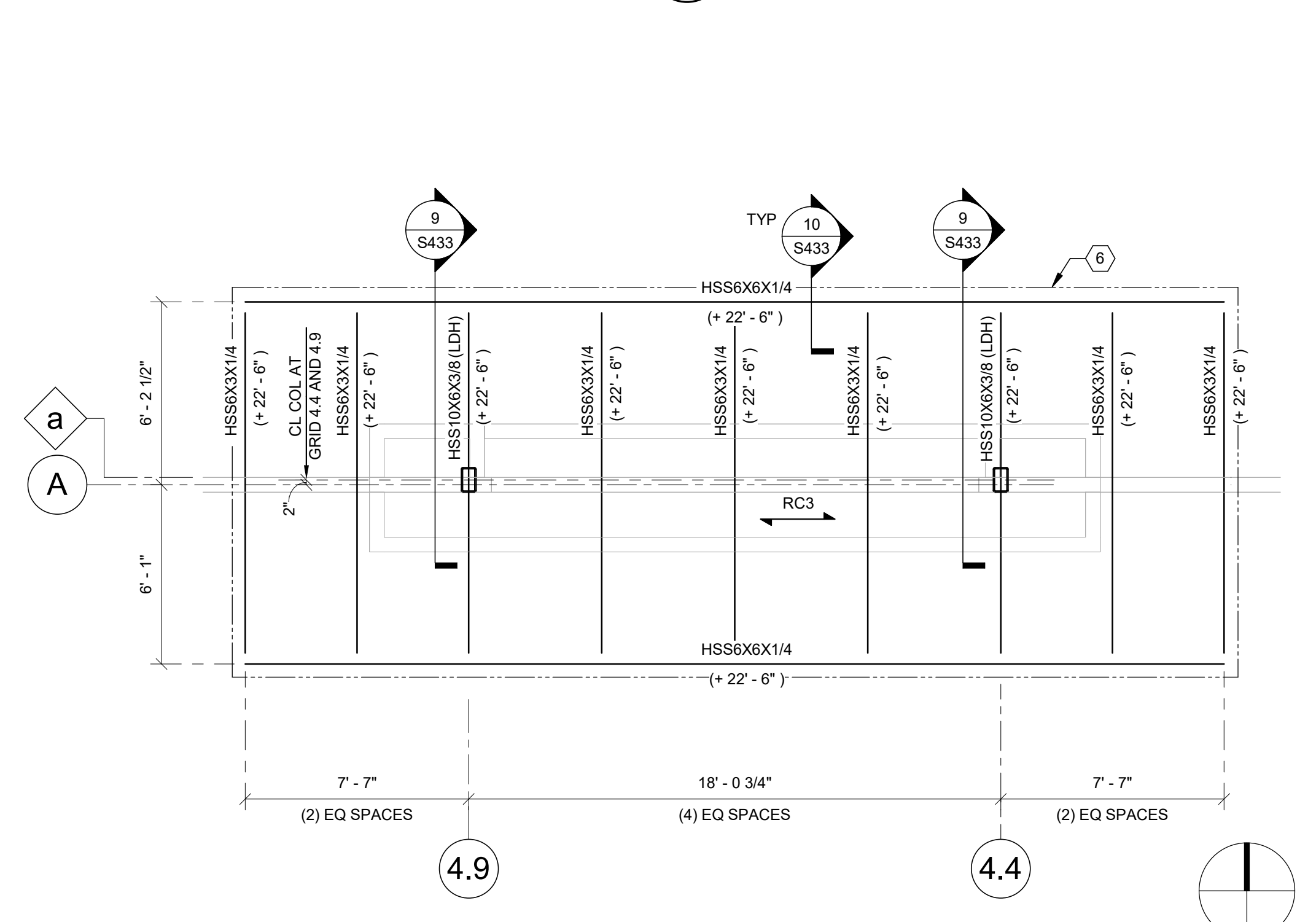
SHEET TITLE
ROOF FRAMING PLAN

SHEET NUMBER
S200

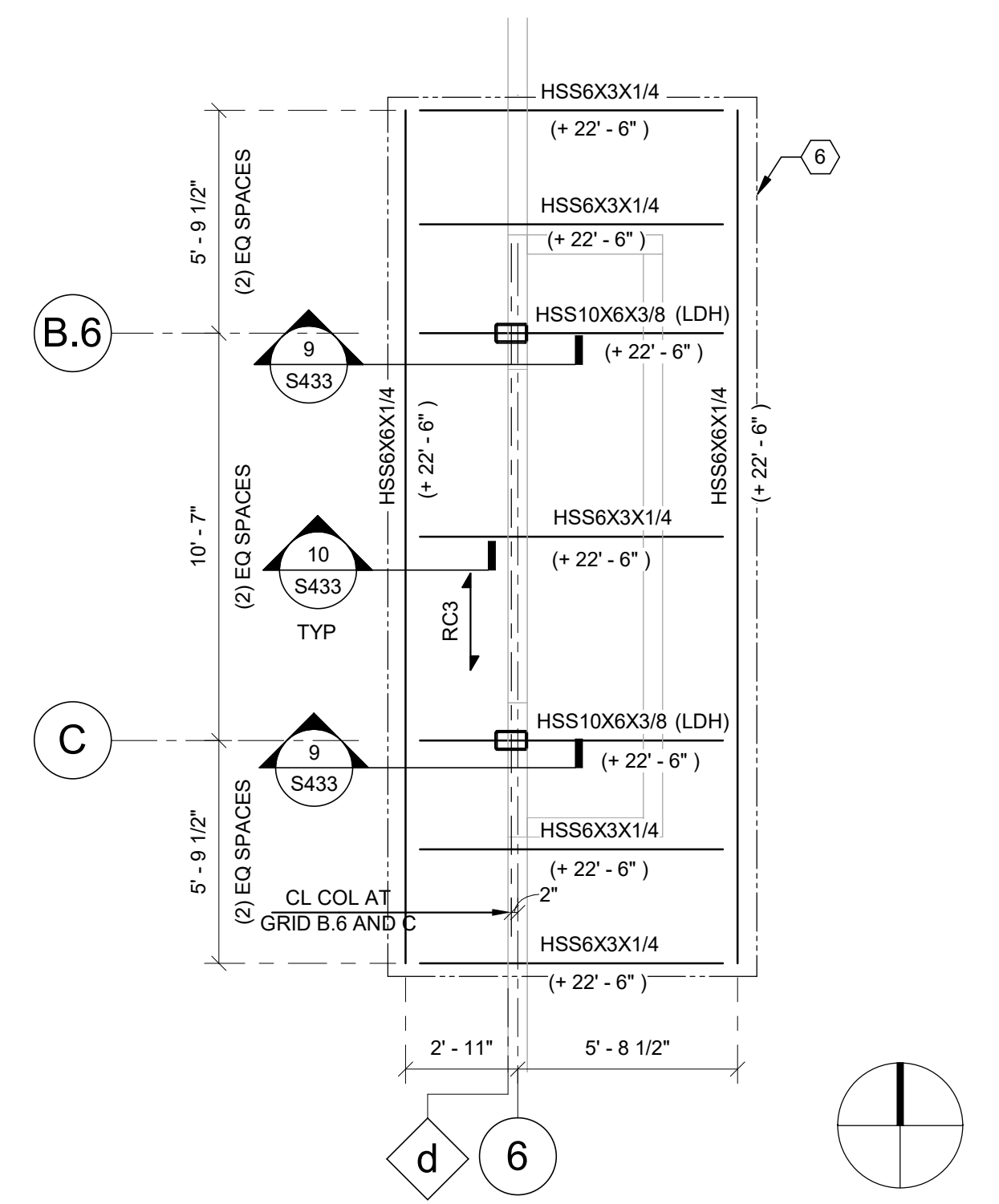
INCORPORATED SPB:



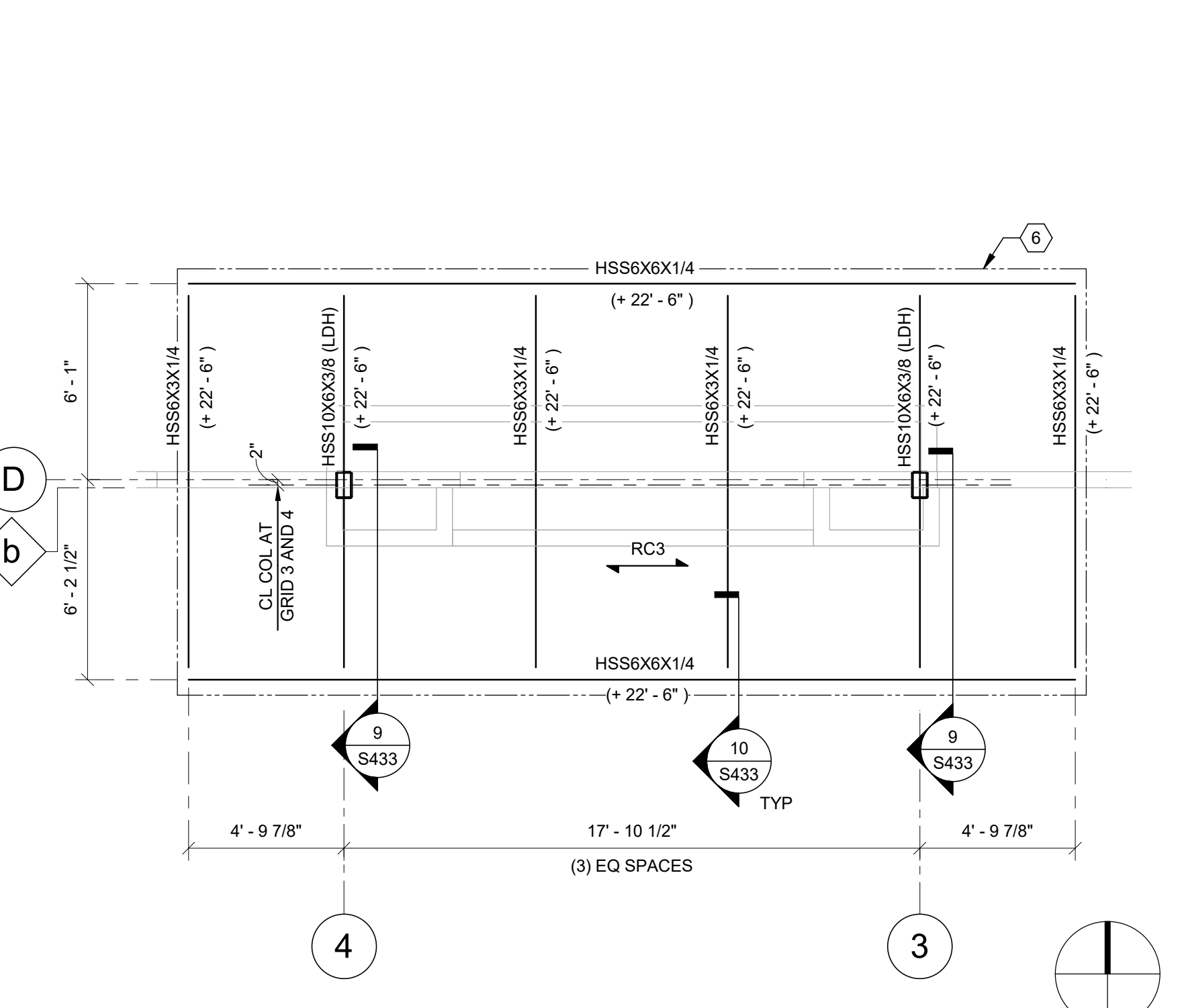
4 PARAPET FRAMING PLAN
1/8" = 1'-0"



3 HIGH ROOF FRAMING PLAN ALONG GRID A
1/4" = 1'-0"



2 HIGH ROOF FRAMING PLAN ALONG GRID 6
1/4" = 1'-0"



1 HIGH ROOF FRAMING PLAN ALONG GRID D
1/4" = 1'-0"

GENERAL NOTES

1. (X'-X") INDICATES TOP OF COLUMN ELEVATION ABOVE FINISHED FLOOR.
2. (X'-X") INDICATES TOP OF STEEL BEAM ELEVATION ABOVE FINISHED FLOOR.
3. SEE SHEETS S000 THRU S002 FOR GENERAL NOTES.
4. VERIFY ALL PARAPET STEP LOCATIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO FABRICATION.

LEGEND

RC3 INDICATES METAL ROOF DECK DIRECTION - SEE GENERAL NOTES FOR SIZE, TYPICAL.

SHEET KEYNOTES

1. NOTCH PARAPET FOR LADDER - SEE 12/S432. SEE ARCH FOR LOCATION AND DIMENSIONS.
2. TOP OF COLUMN ELEVATION SHALL BE EQUAL TO TOP OF THE HIGHEST ADJACENT PARAPET CHANNEL, U.N.O.
3. COLUMN CONT. UP TO HIGH ROOF.
4. PROVIDE VERTICAL C4X5.4 FOR PARAPET CHANNEL SUPPORT - SEE 14/S432.
5. HSS6X2X1/4 STUB COLUMN (CANTILEVERED) - SEE 17/S432 FOR BASE ATTACHMENT.
6. FACE OF FINISH MATERIAL - SEE ARCHITECTURAL.

RaceTrac

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DESIGN PROFESSIONALS

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816.421.4200 - WWW.JBAENGINEER.COM
FORMERLY CVA & PARTNERS
JBA PROJECT #: 20131

PROJECT MANAGER:
SDH
CHECKED BY:
BWM
DRAWN BY:
JBA

ISSUE/REVISION RECORD

DATE	DESCRIPTION
02/19/22	PERMIT SET
05/21/20	REV1-SITE PLAN PKG
10/07/20	REV2-PHOTOMETRIC
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11/05/21	REFRESH 2021_0401
02/07/22	REFRESH 2021_0507
08/12/22	REFRESH 2022_0202

RaceTrac

RACETRAC INC.
200 GALLERIA PARKWAY SOUTHEAST
SUITE 900
ATLANTA, GEORGIA 30339
(770) 431-7600

PROJECT NAME

VERO BEACH, FL

VERO BEACH FL 32966
8990 20TH STREET

RACETRAC STORE NUMBER

#1422

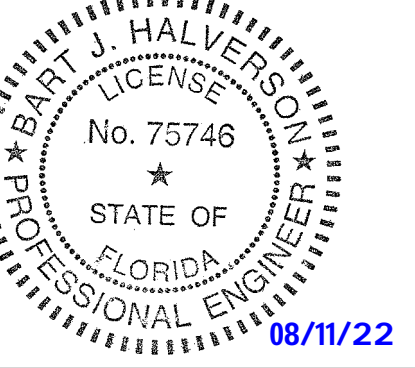
PROTOTYPE SERIES TC
2022 LH SV 0202

PLAN MODIFICATION NOTICE

SPB NO. 0202 DATE 07/08/22

STANDARD PLAN BULLETINS (SPB) MODIFY THE PROTOTYPE SERIES SET NOTED ABOVE. THE LISTED SPB REPRESENTS THE LATEST MODIFICATION INCORPORATED TO THIS PROTOTYPE SERIES SET AT ORIGINAL RELEASE. THE ISSUE/REVISION RECORD ABOVE LISTS ANY REVISIONS OR SPB INCORPORATED IN THIS SET AFTER THE ORIGINAL RELEASE. CONTACT RACETRAC ENGINEERING AND CONSTRUCTION FOR ANY SUBSEQUENT BULLETINS NOT INCORPORATED HEREIN.

PROFESSIONAL SEAL



PROJECT NUMBER
63200213

SHEET TITLE

HIGH ROOF FRAMING PLANS

SHEET NUMBER

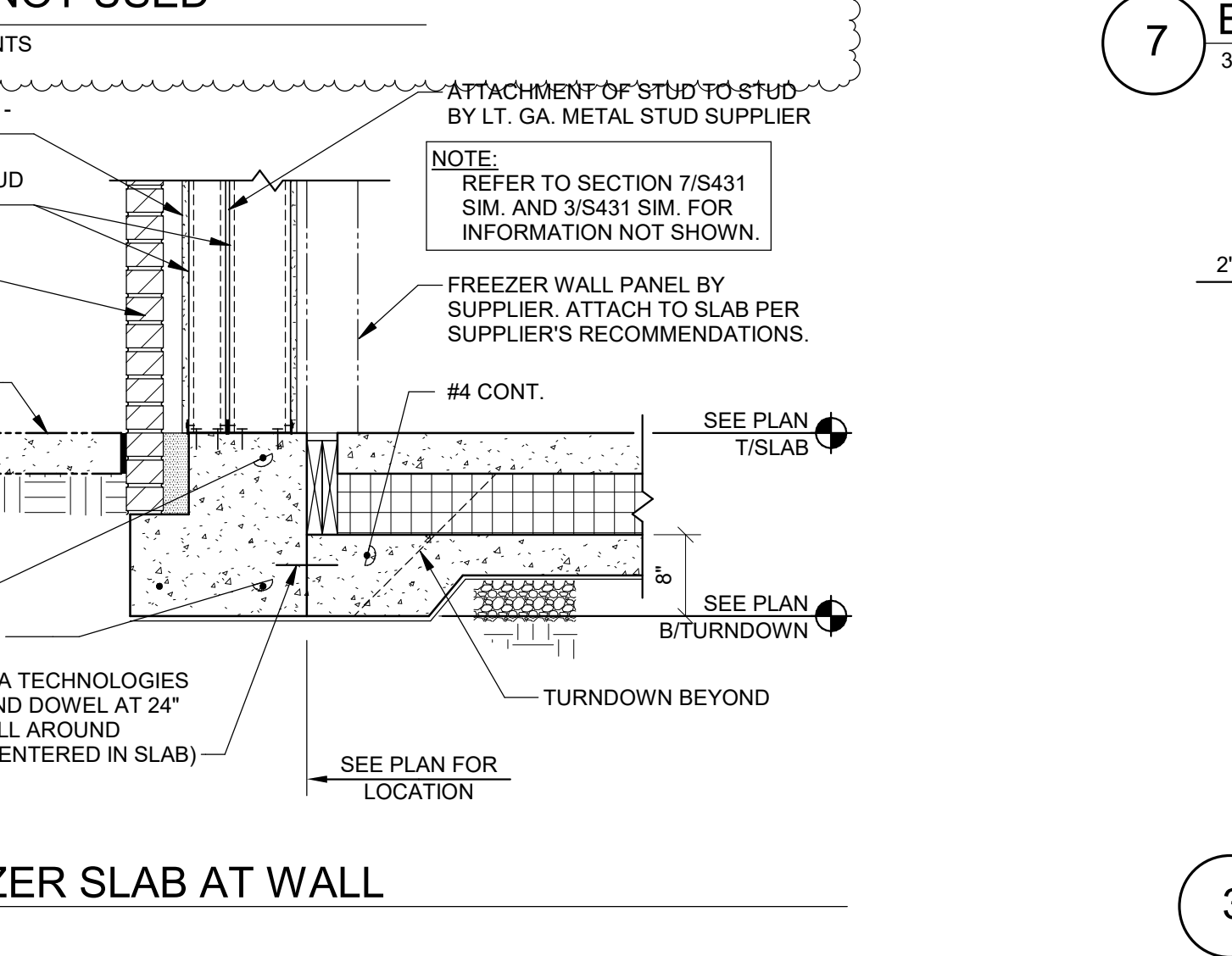
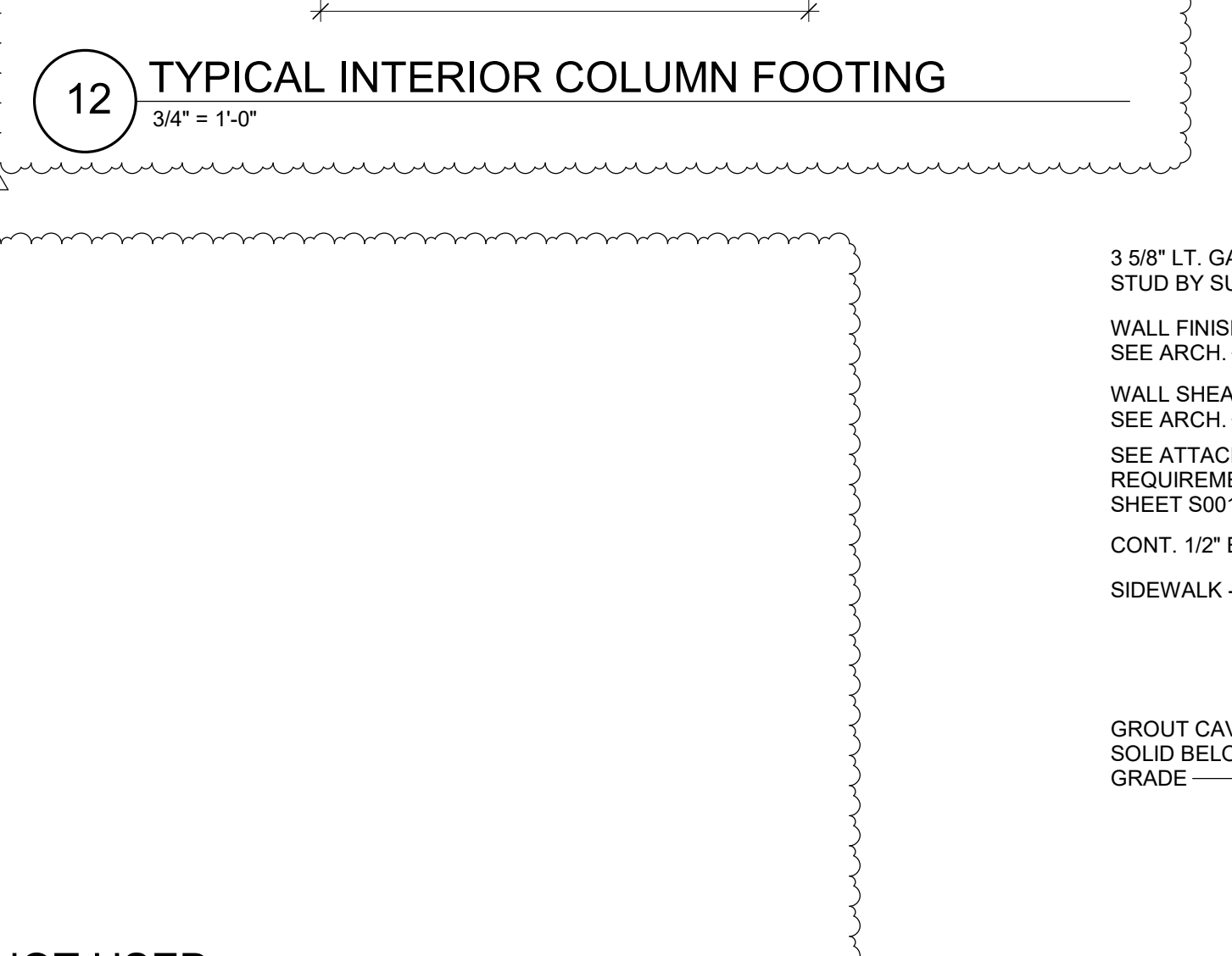
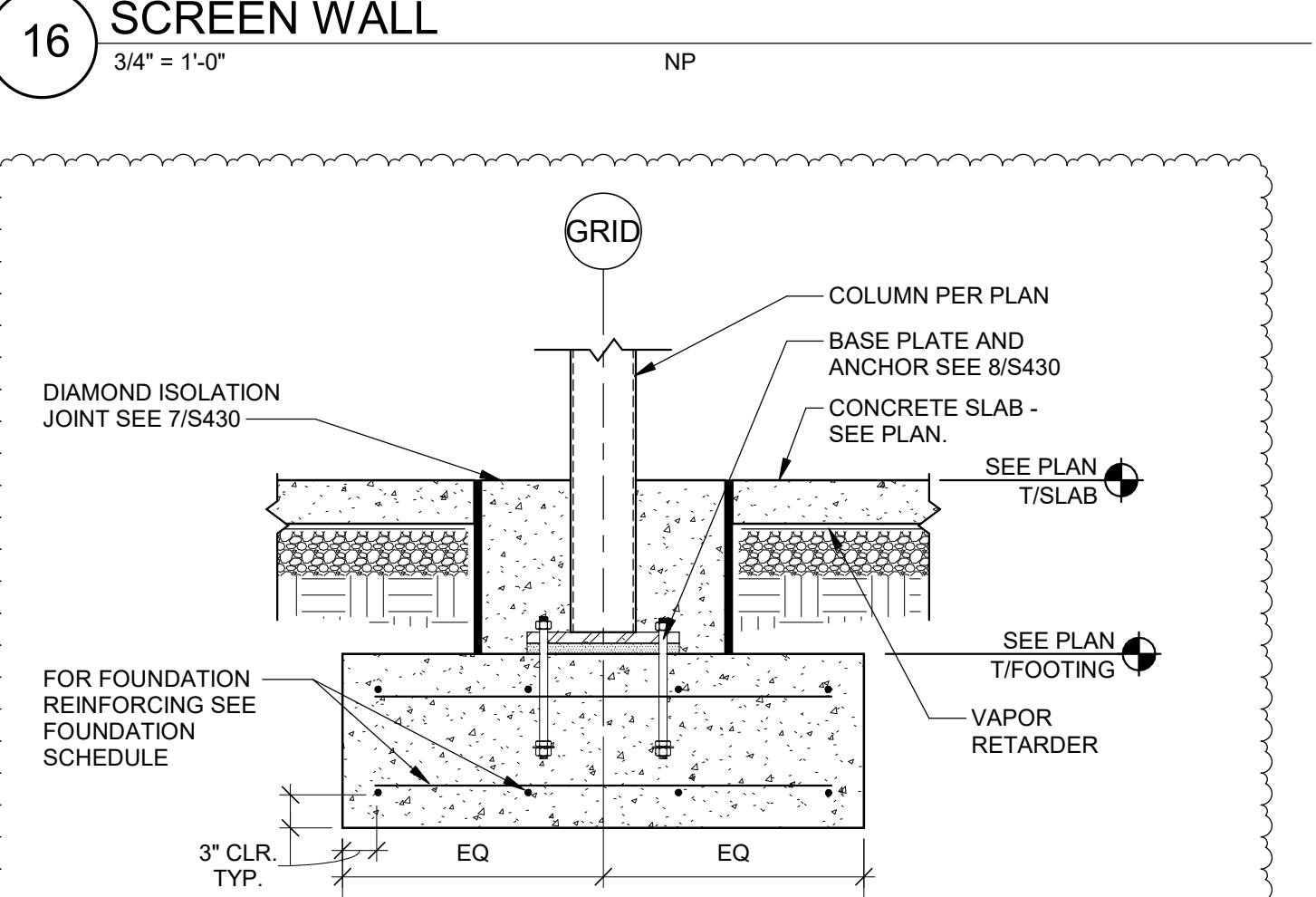
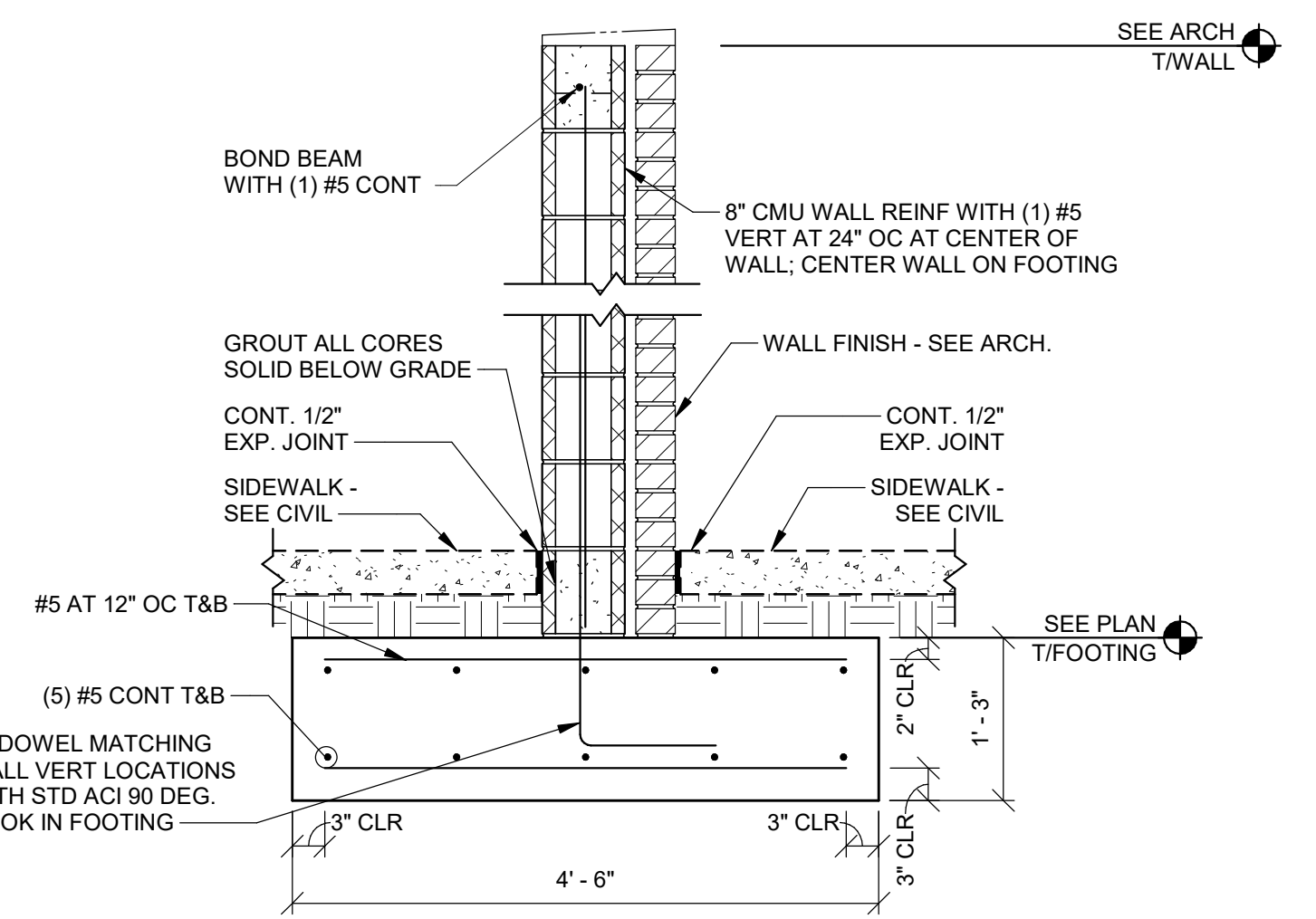
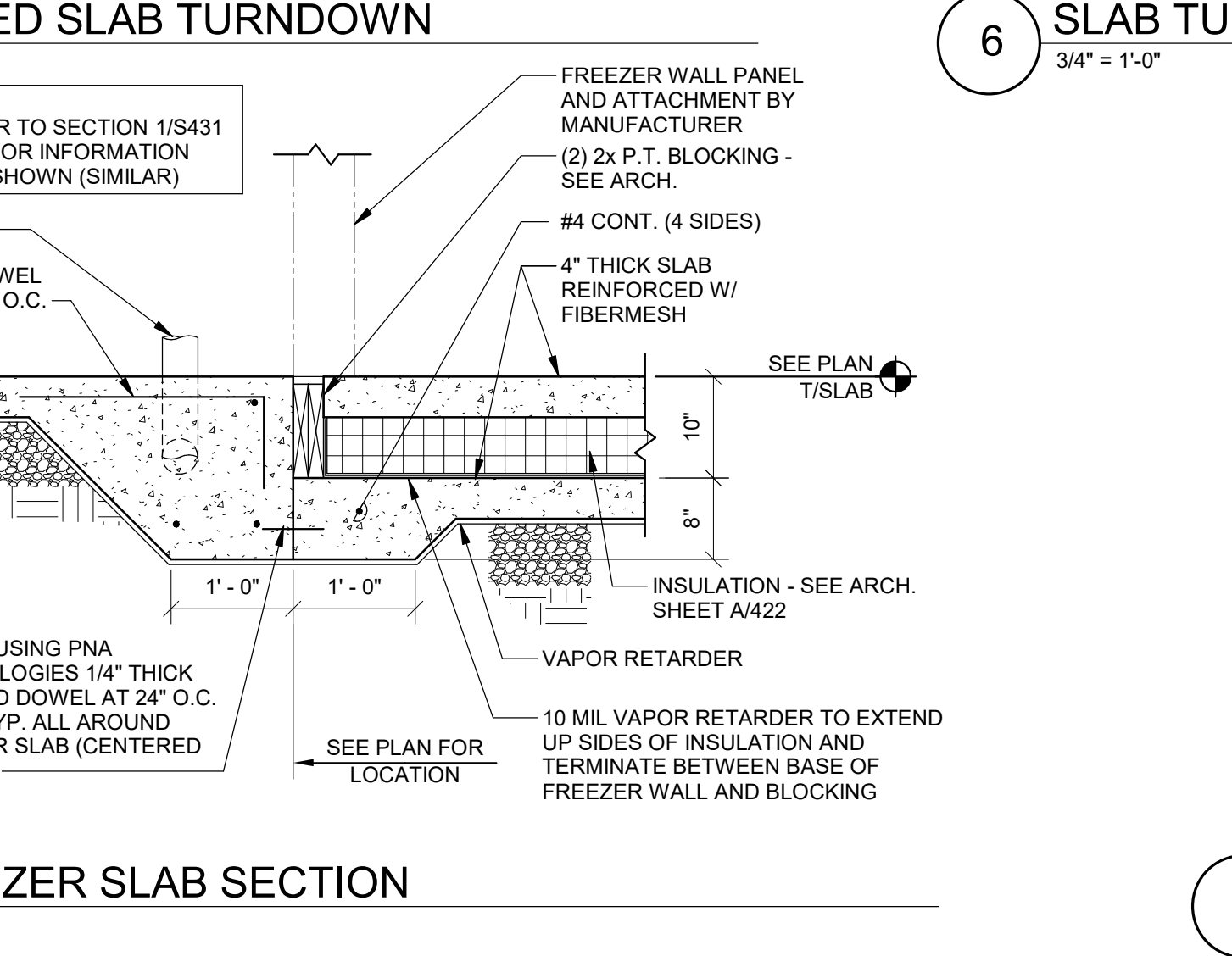
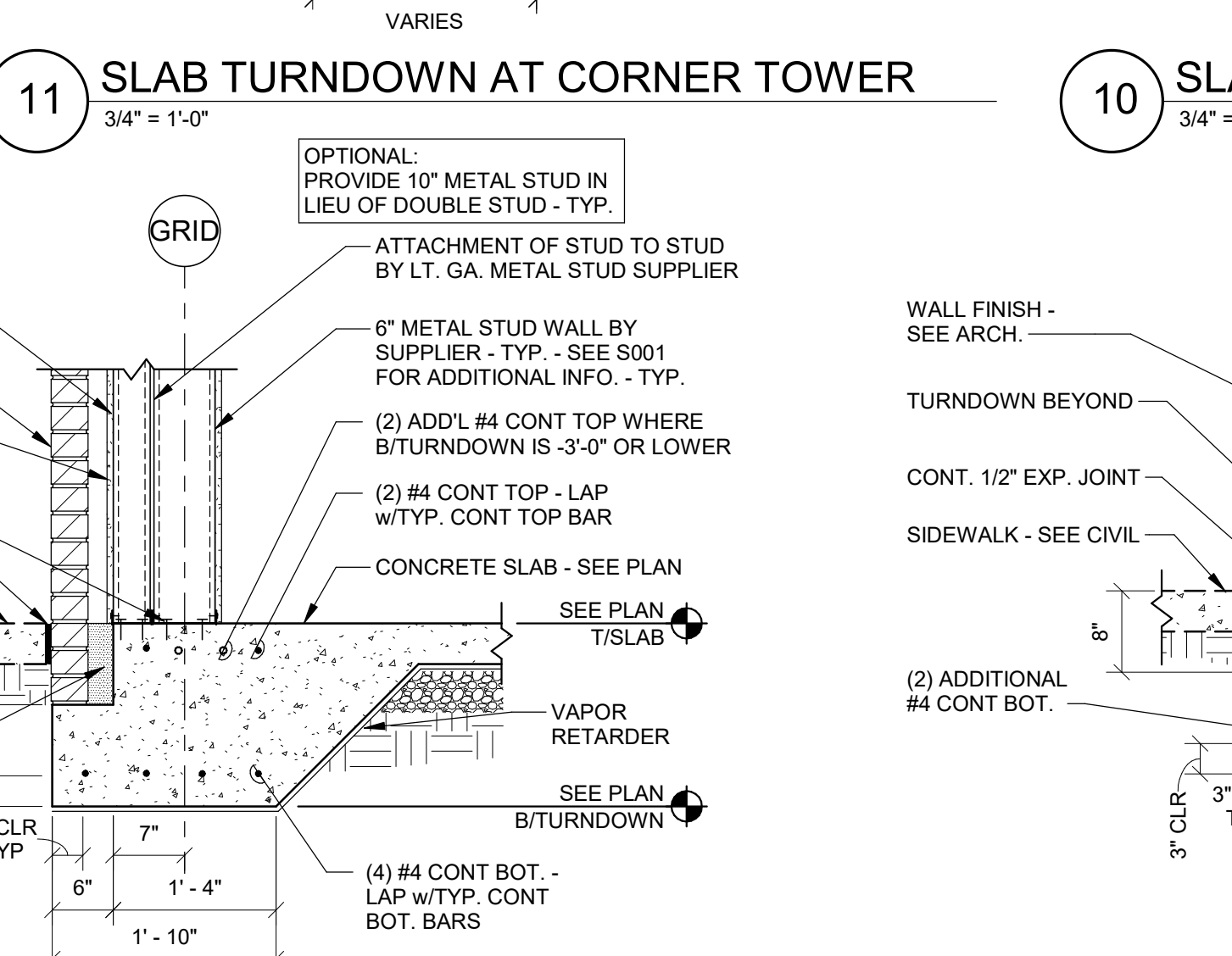
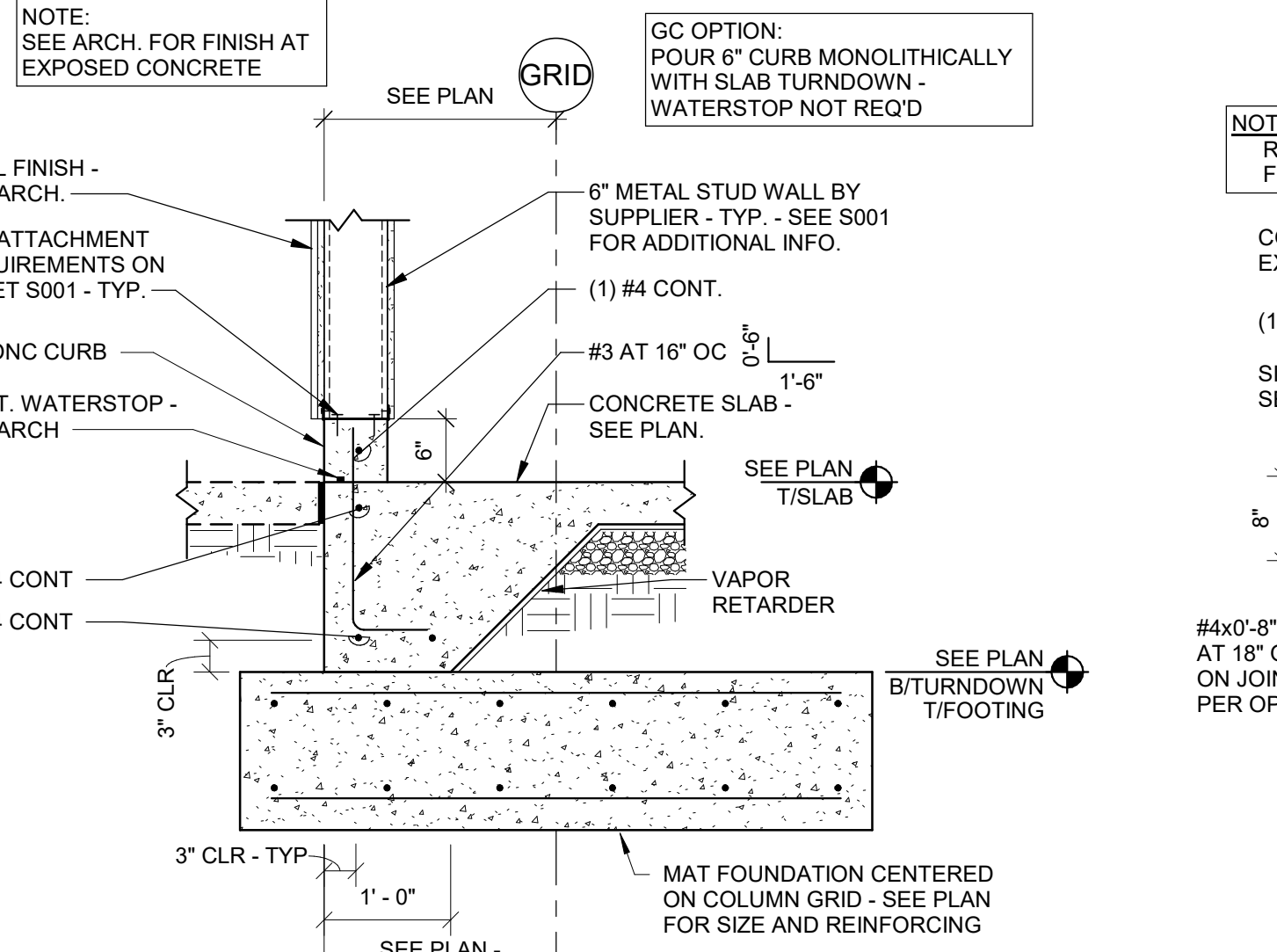
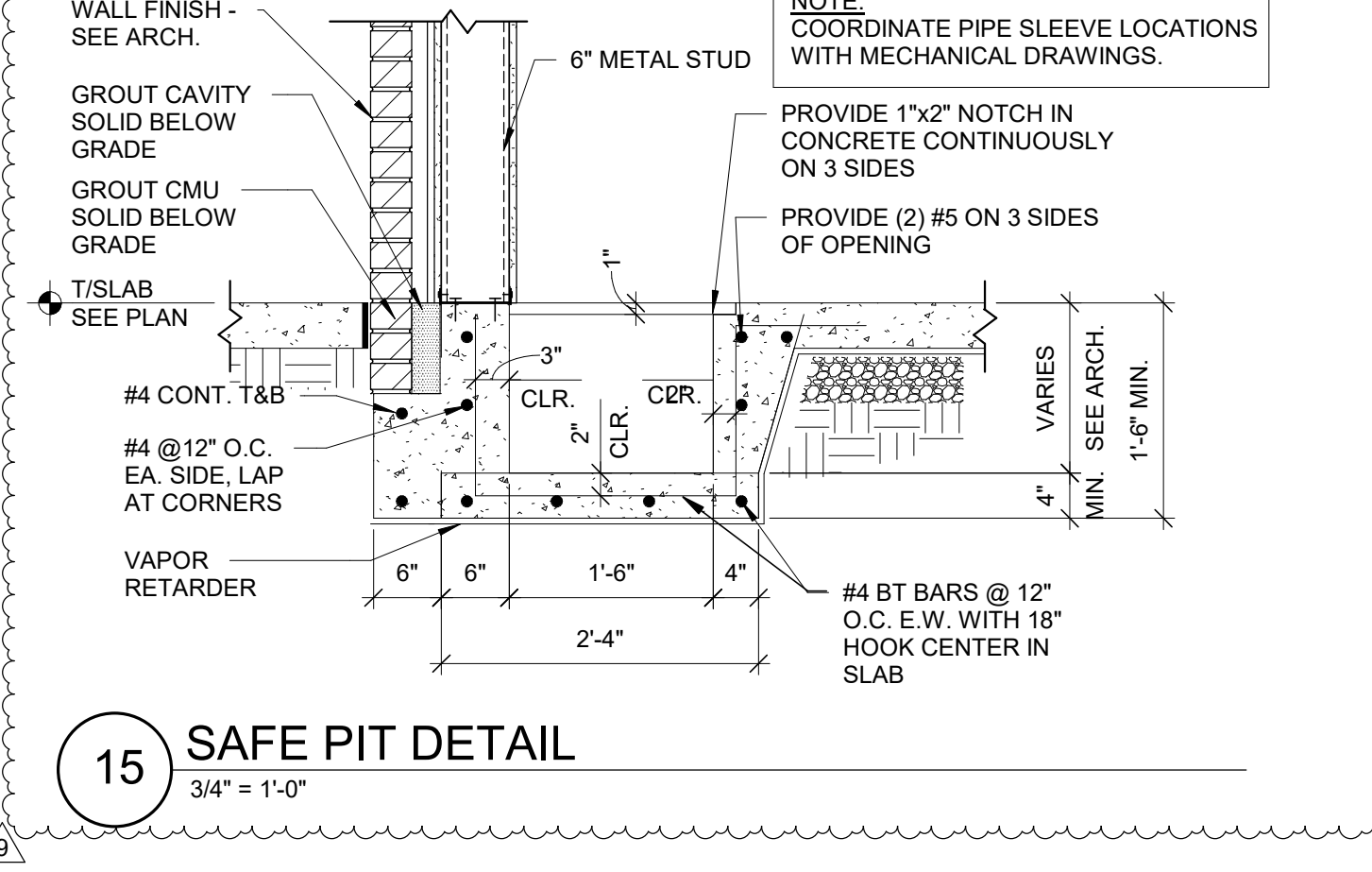
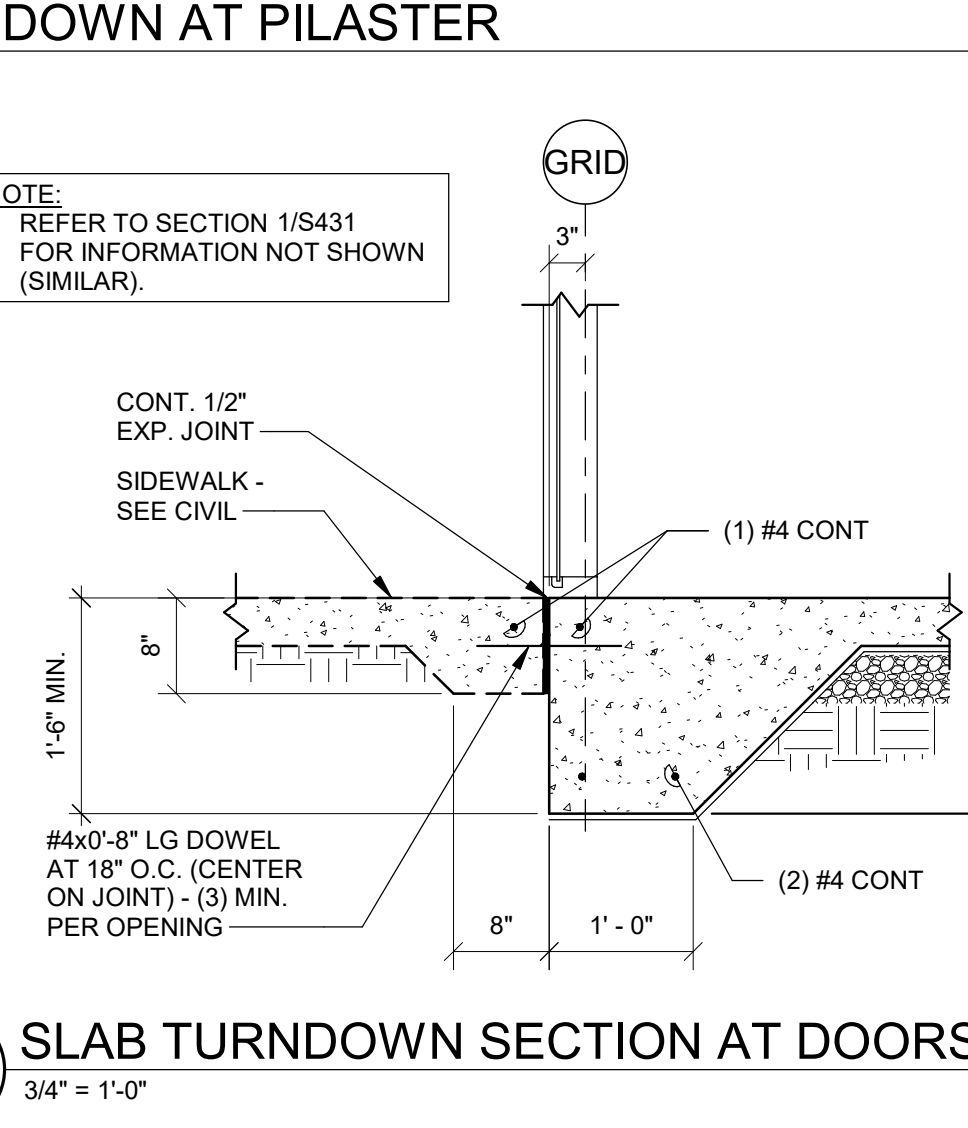
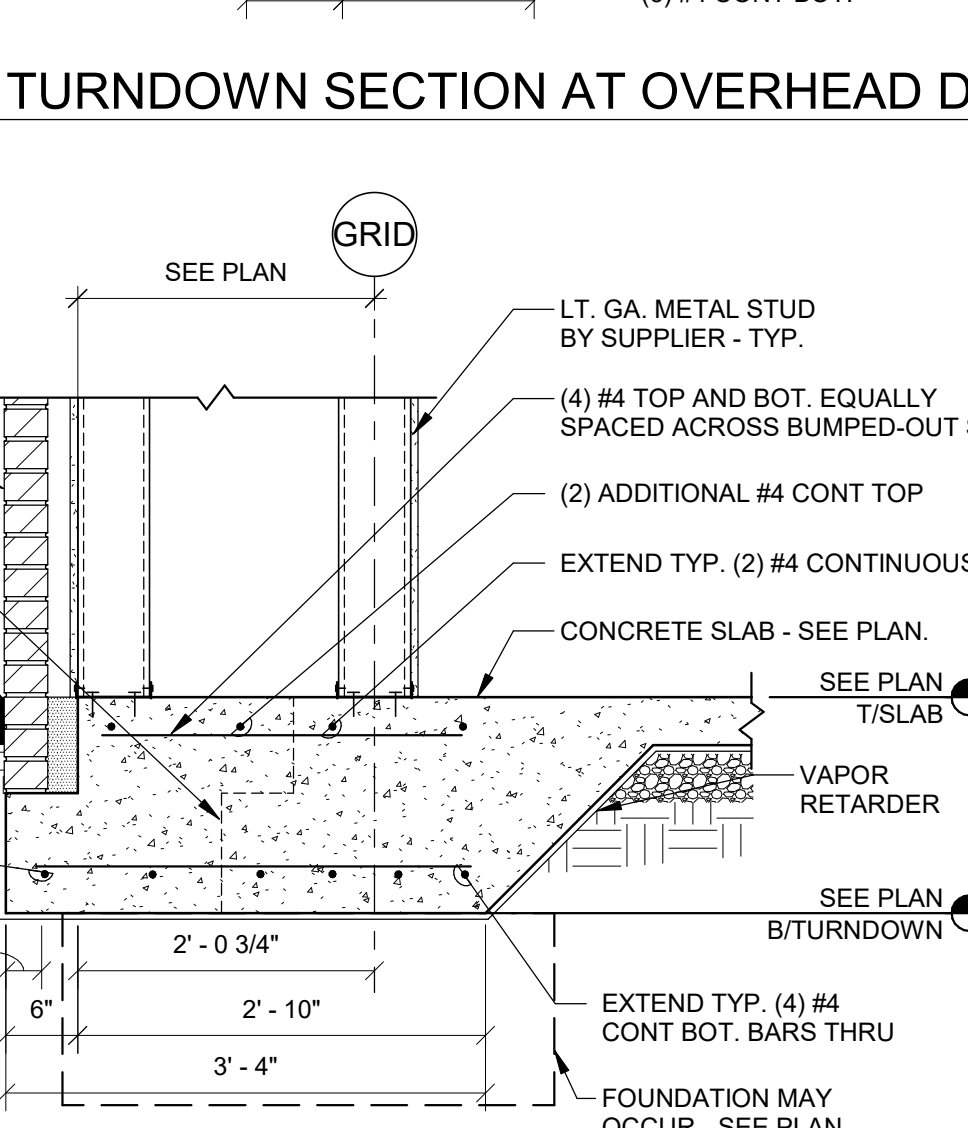
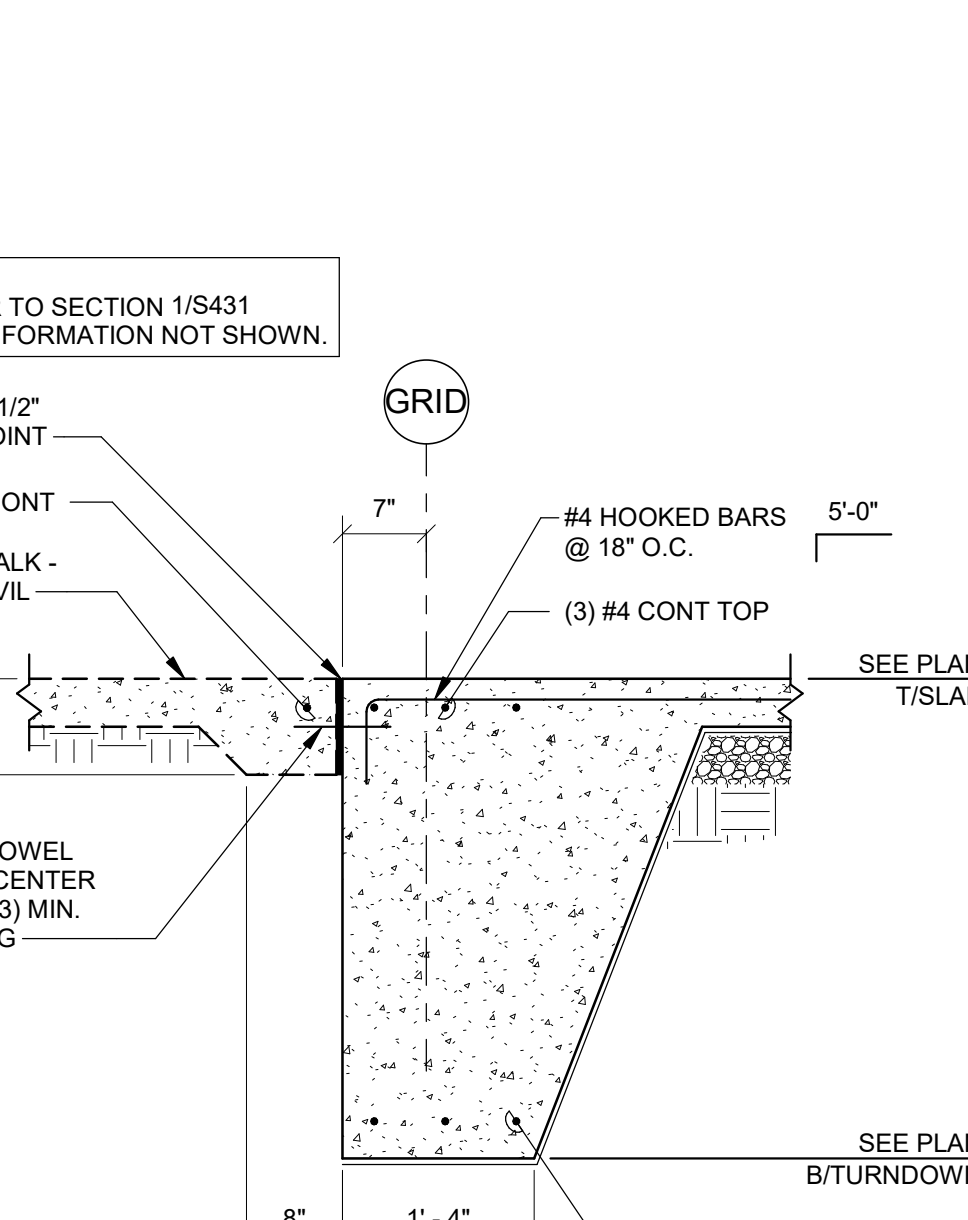
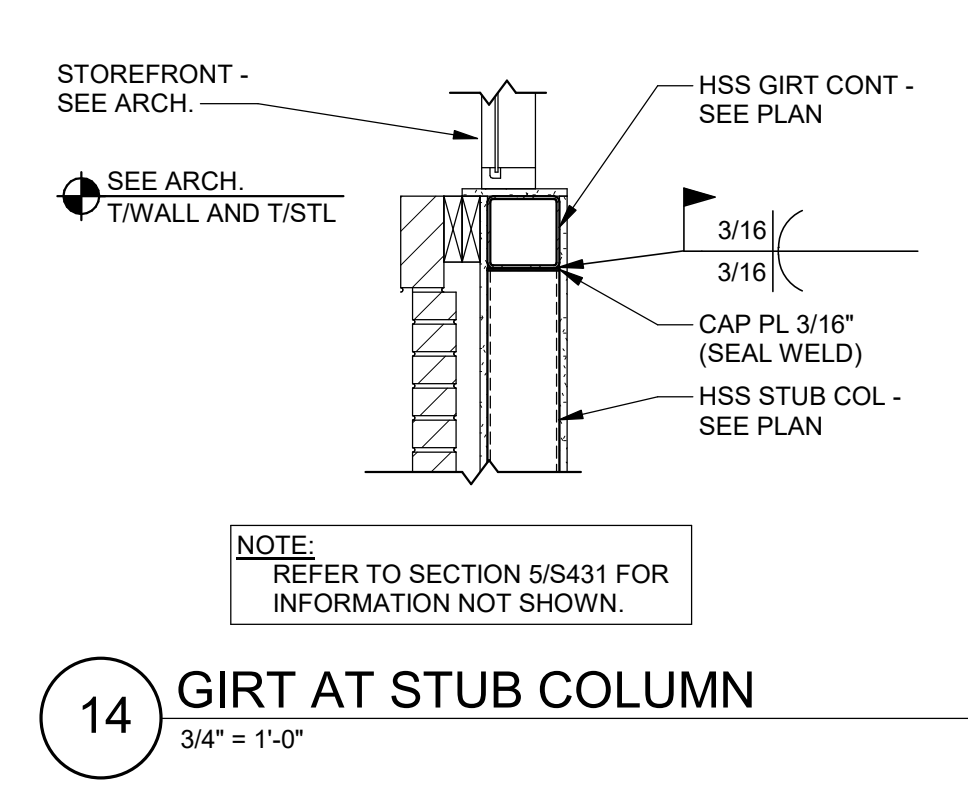
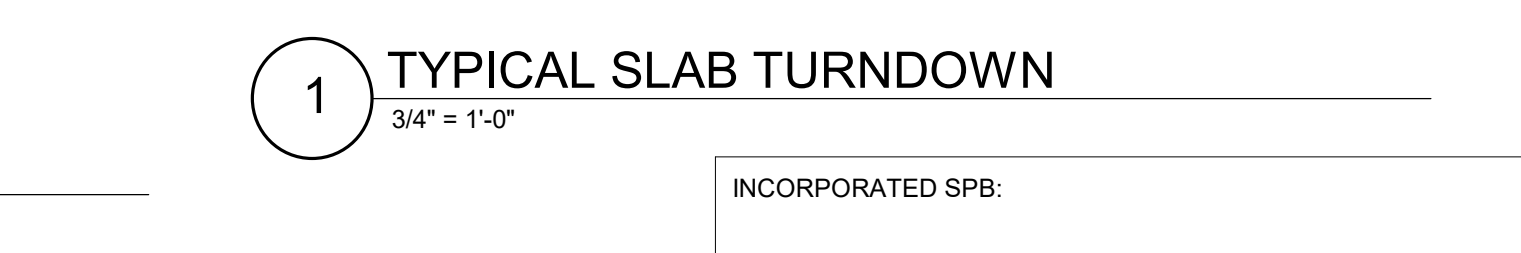
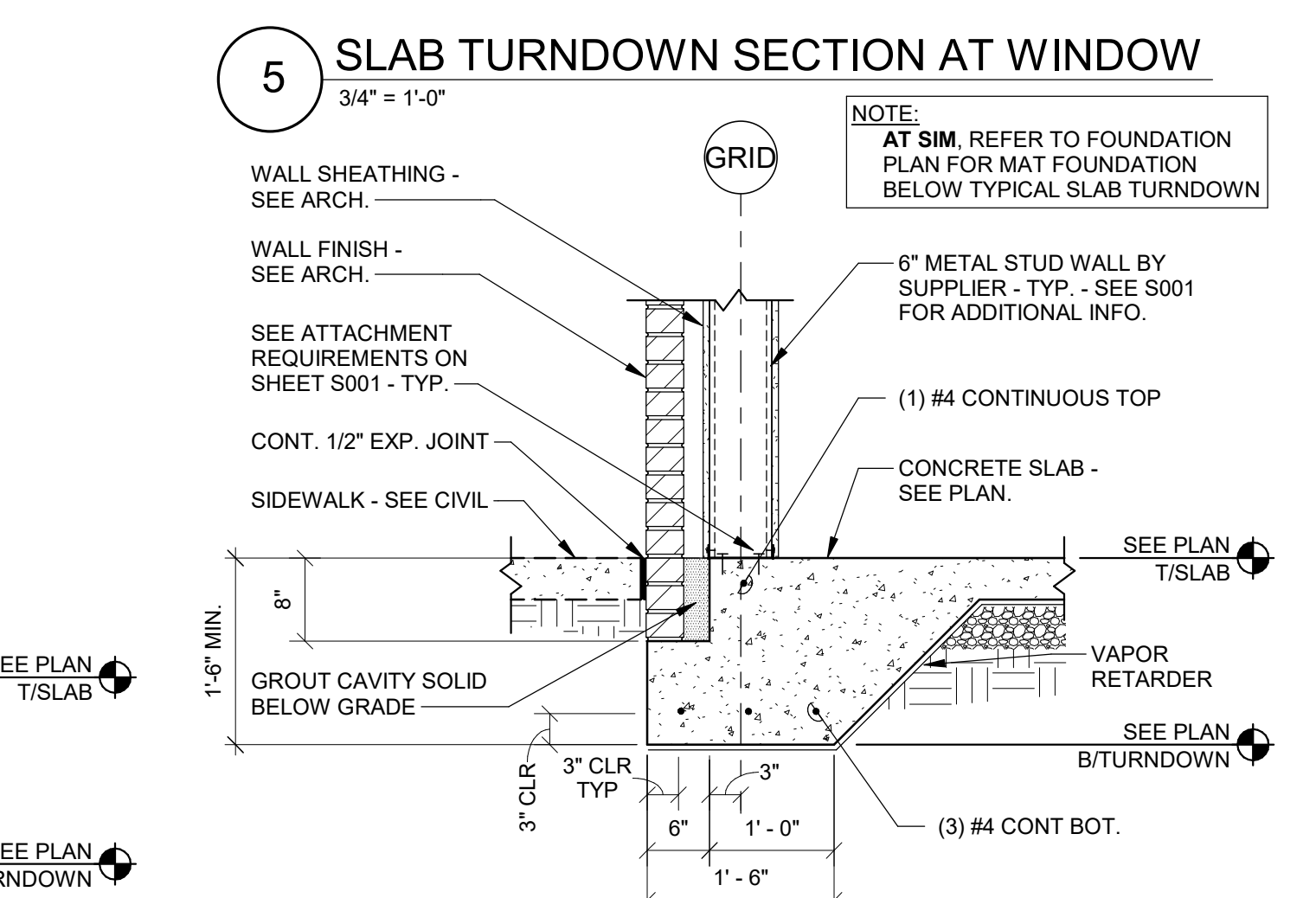
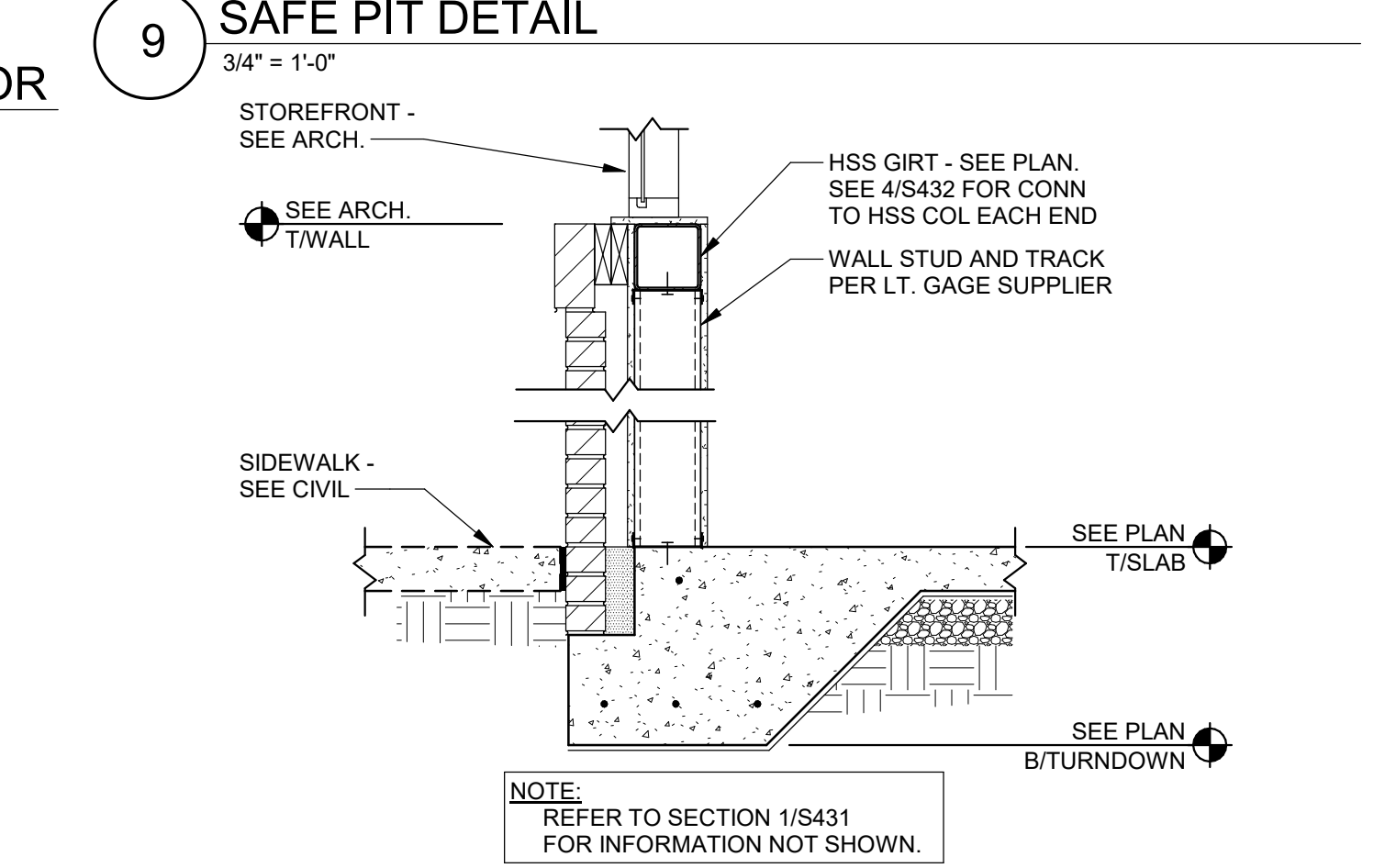
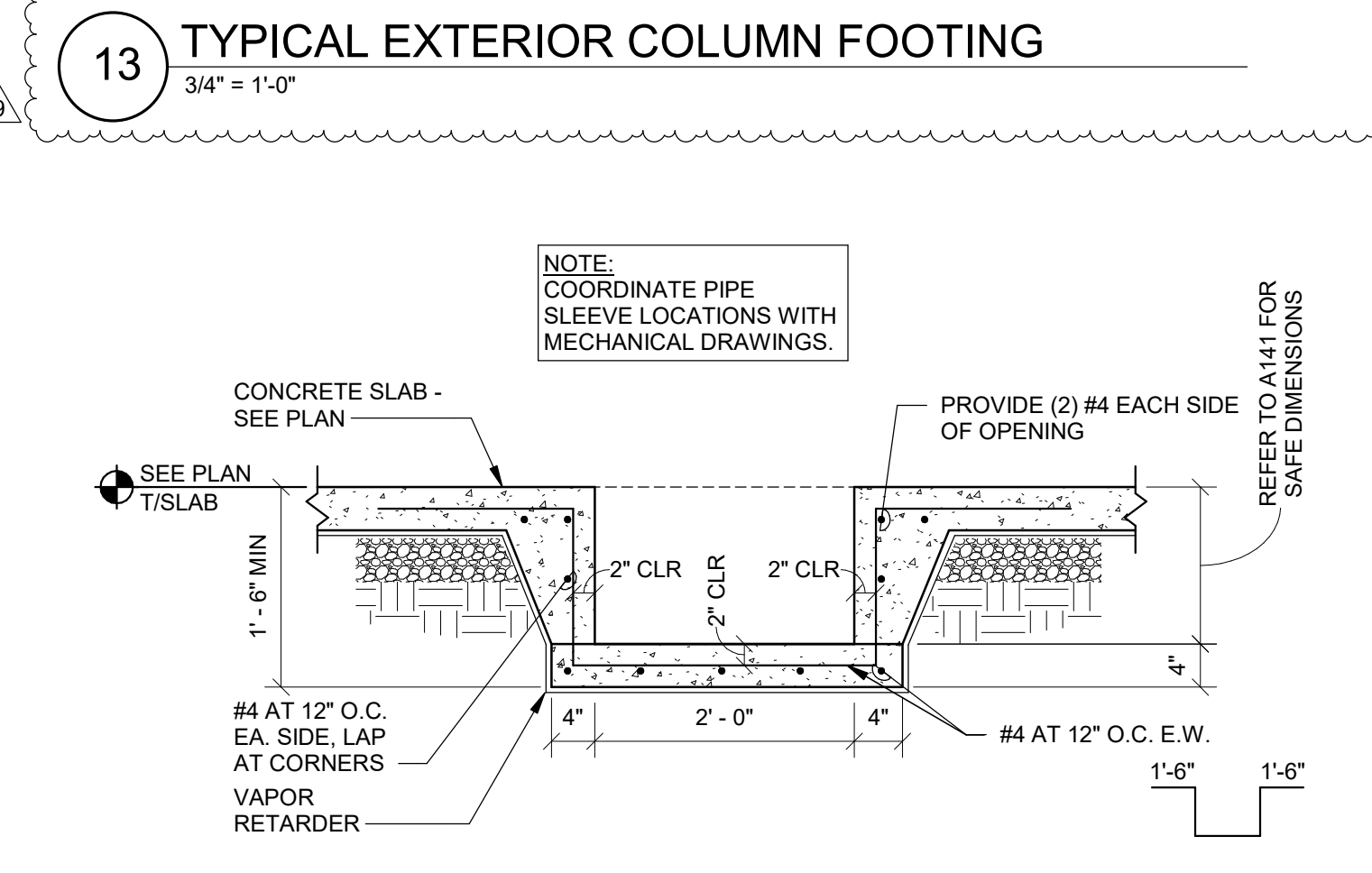
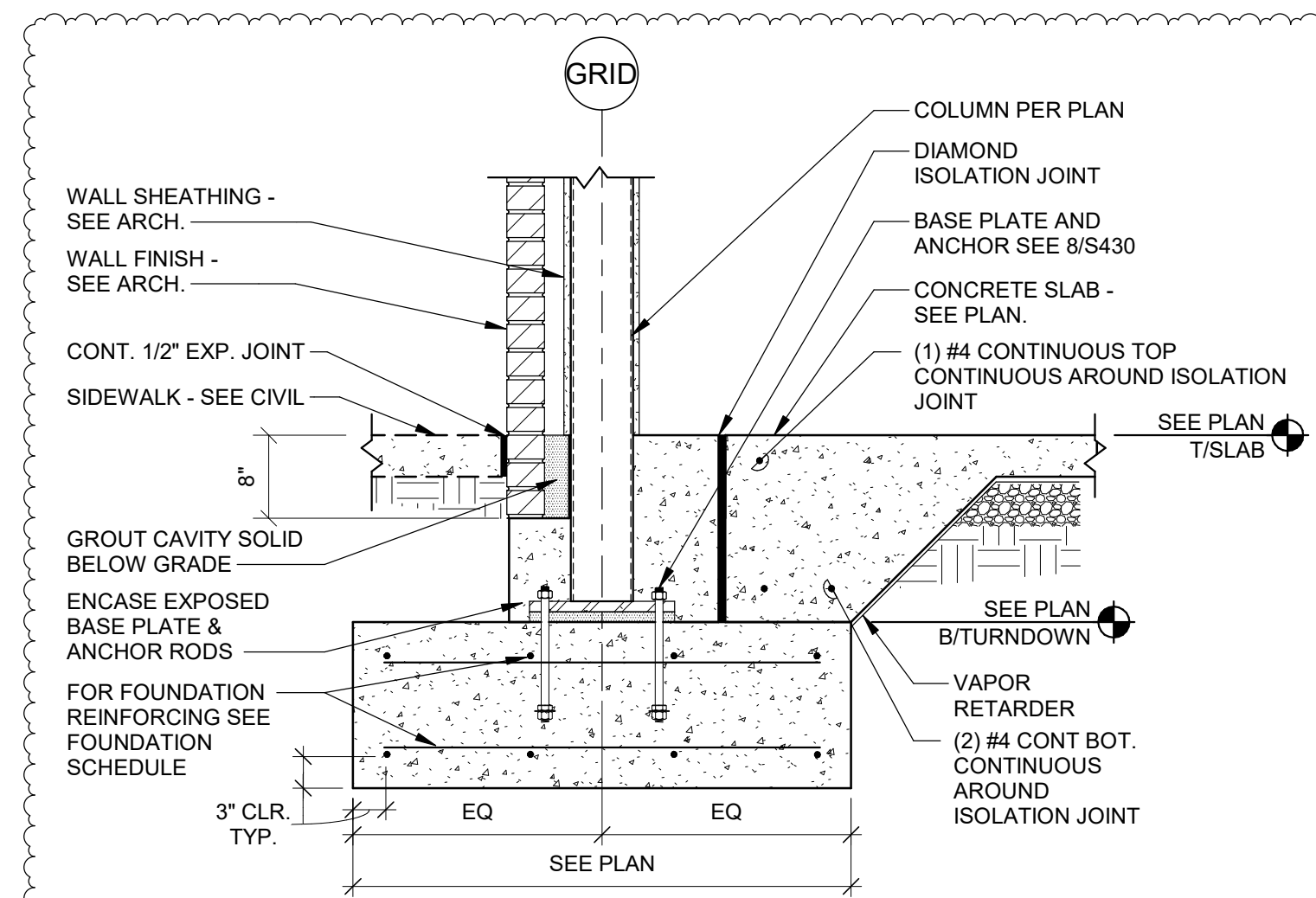
S201

INCORPORATED SPB:

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05/21/20	REV1-SITE PLAN PKG
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11/05/21	REFRESH 2021_0401
02/07/22	REFRESH 2021_0507
08/12/22	REFRESH 2022_0202

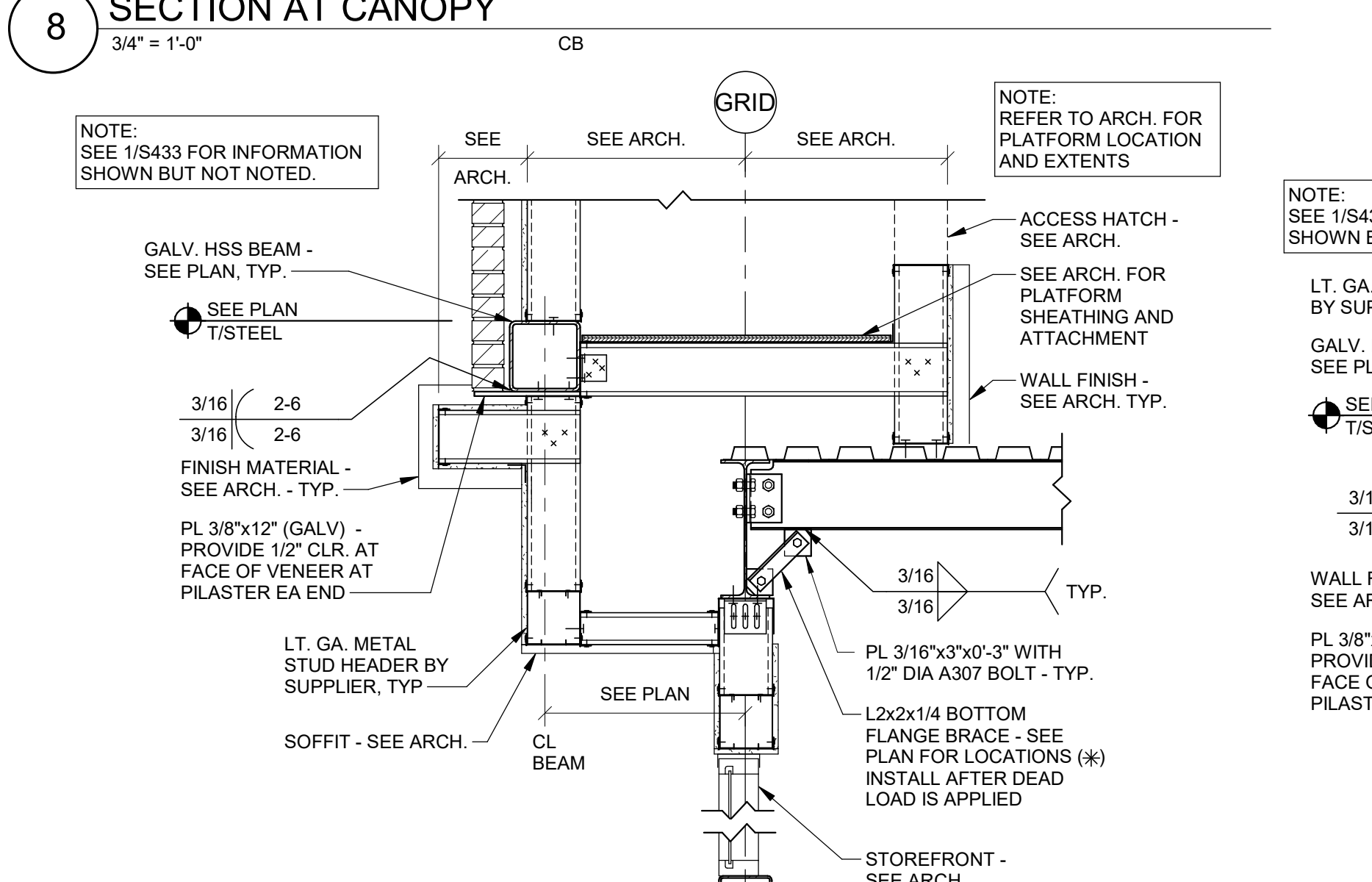
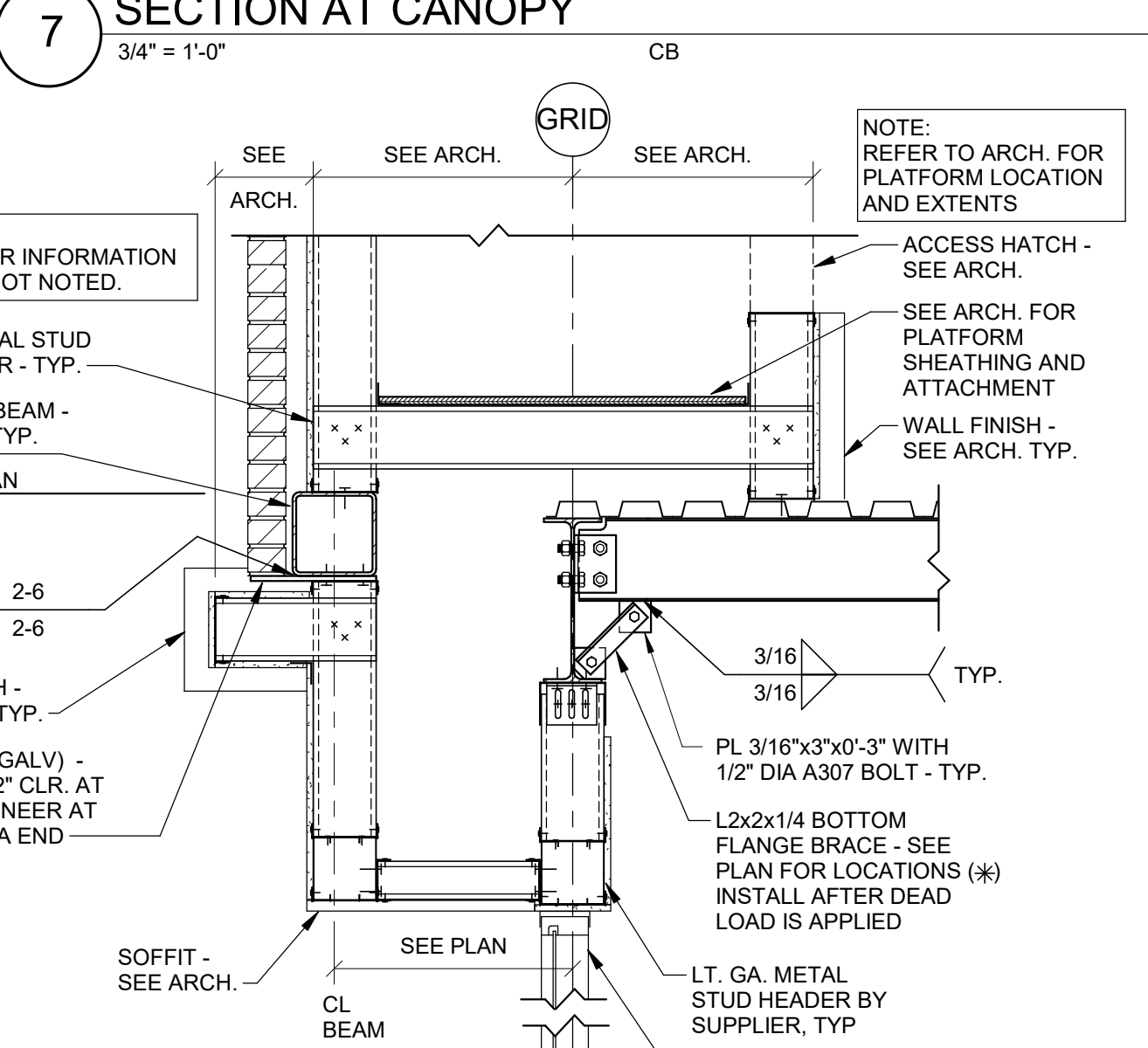
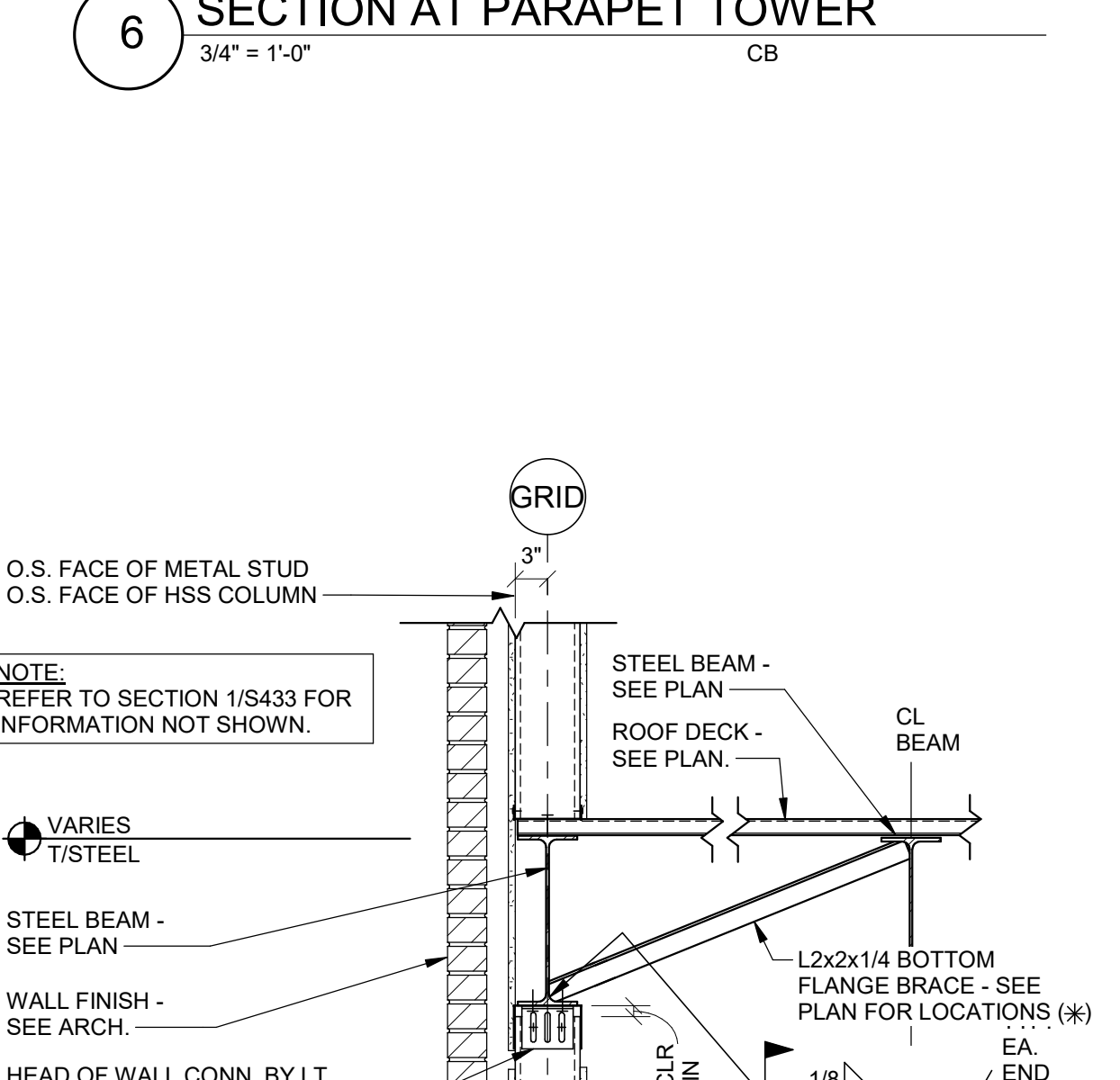
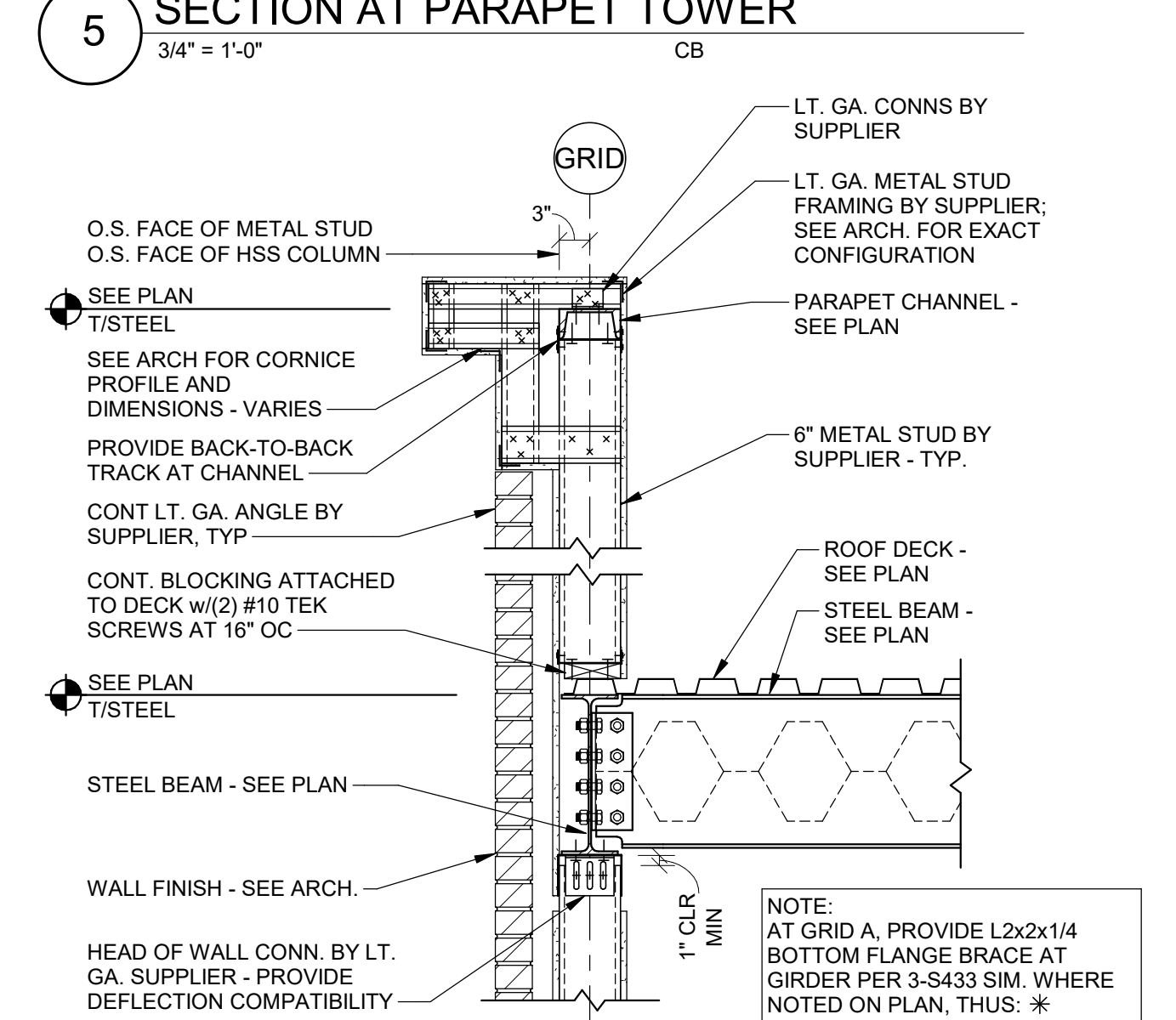
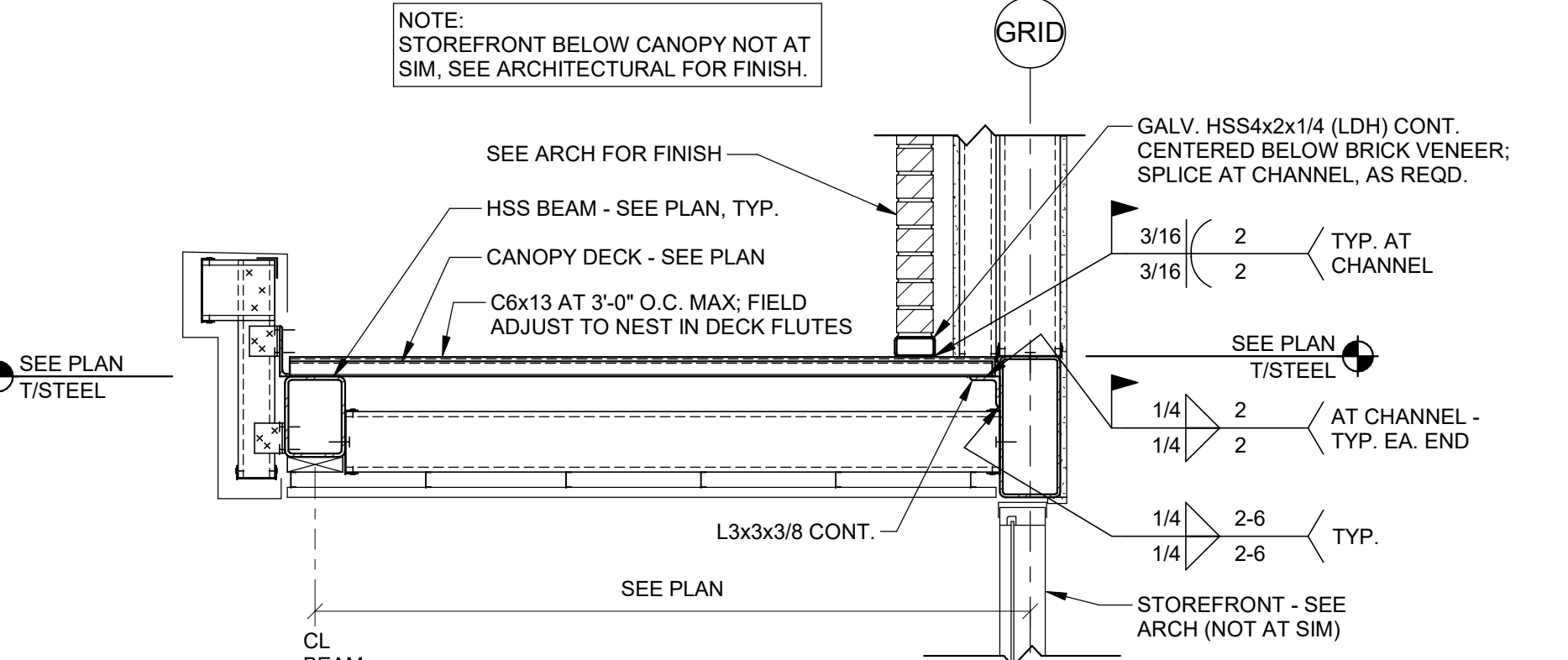
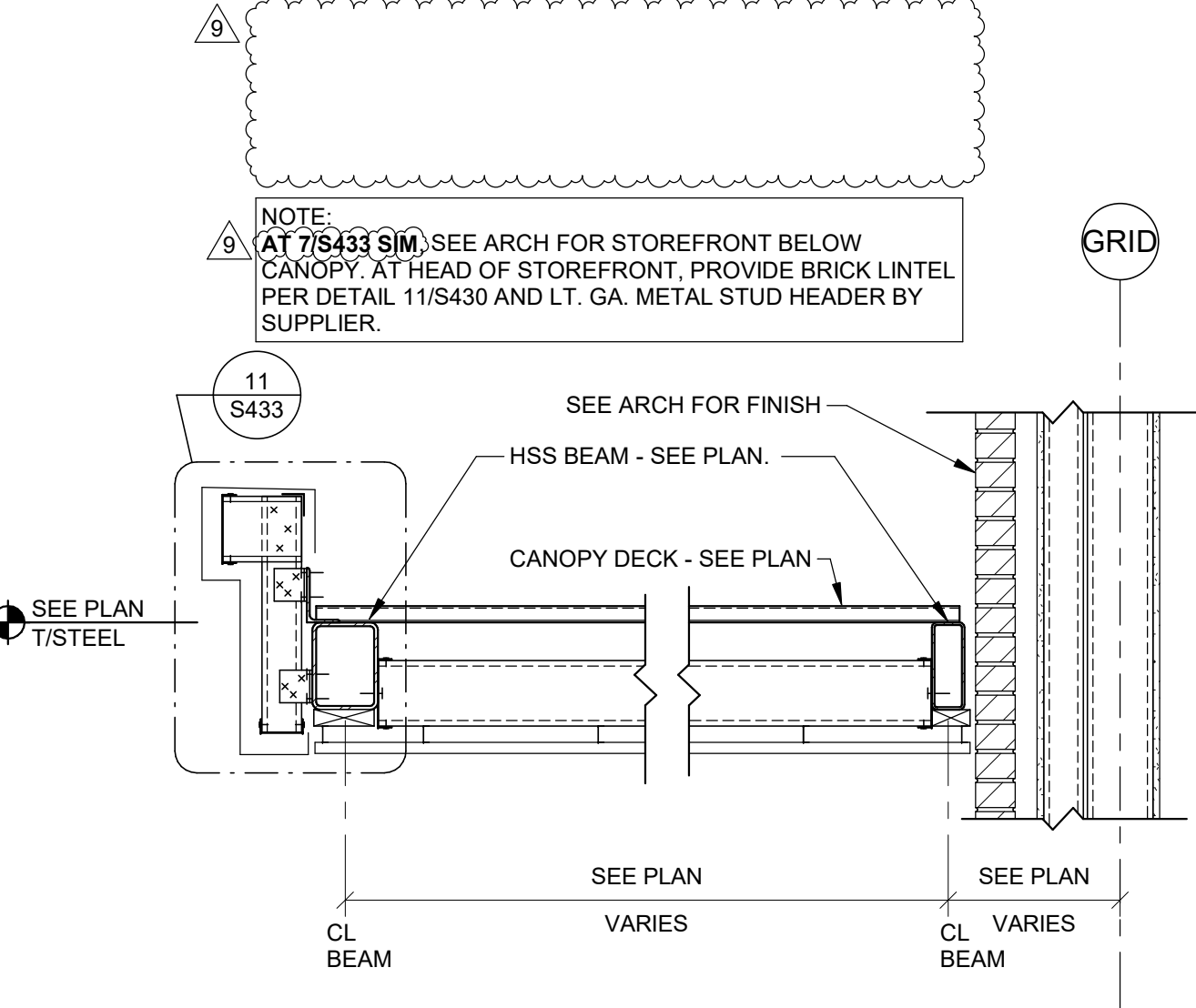
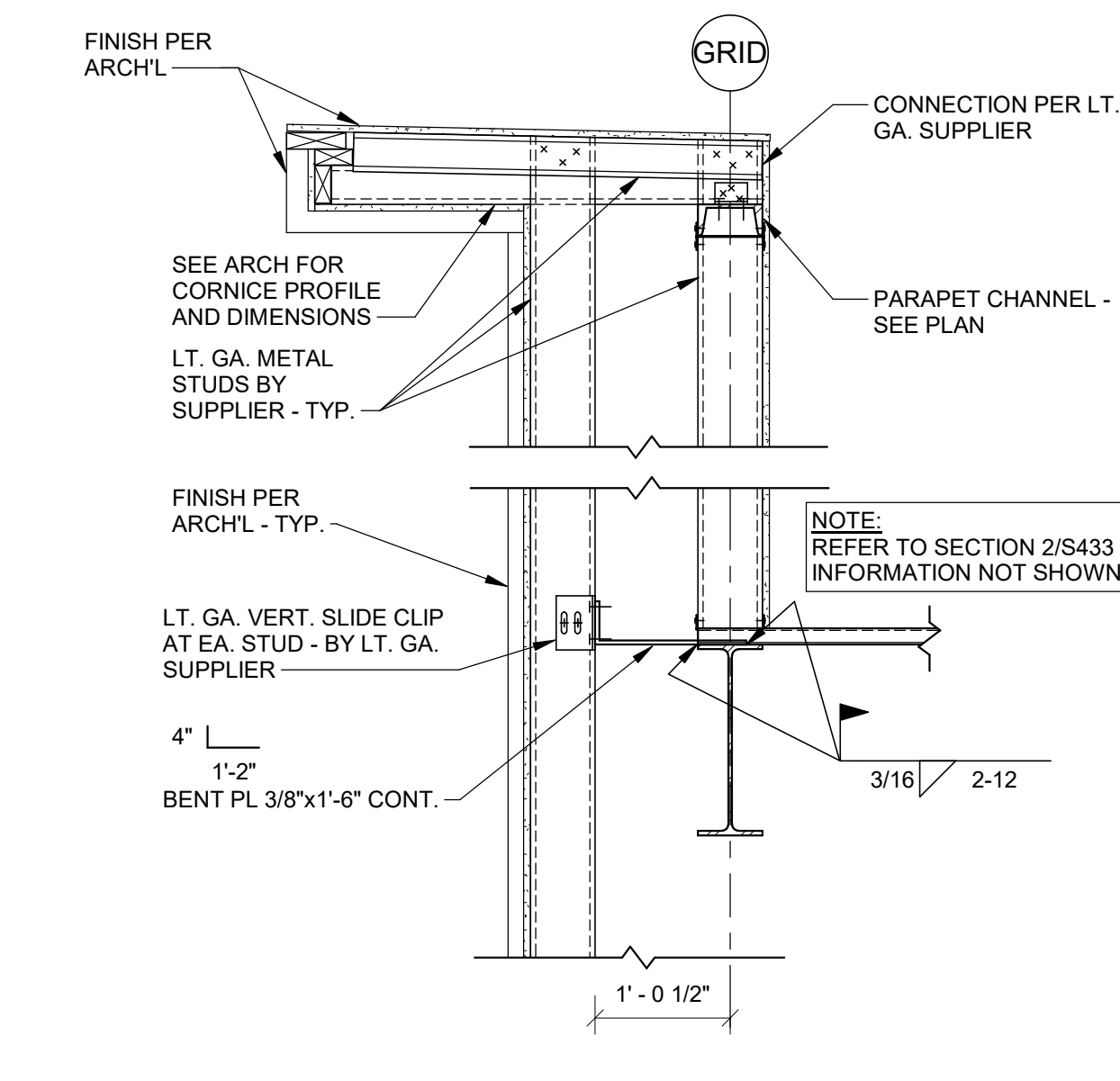
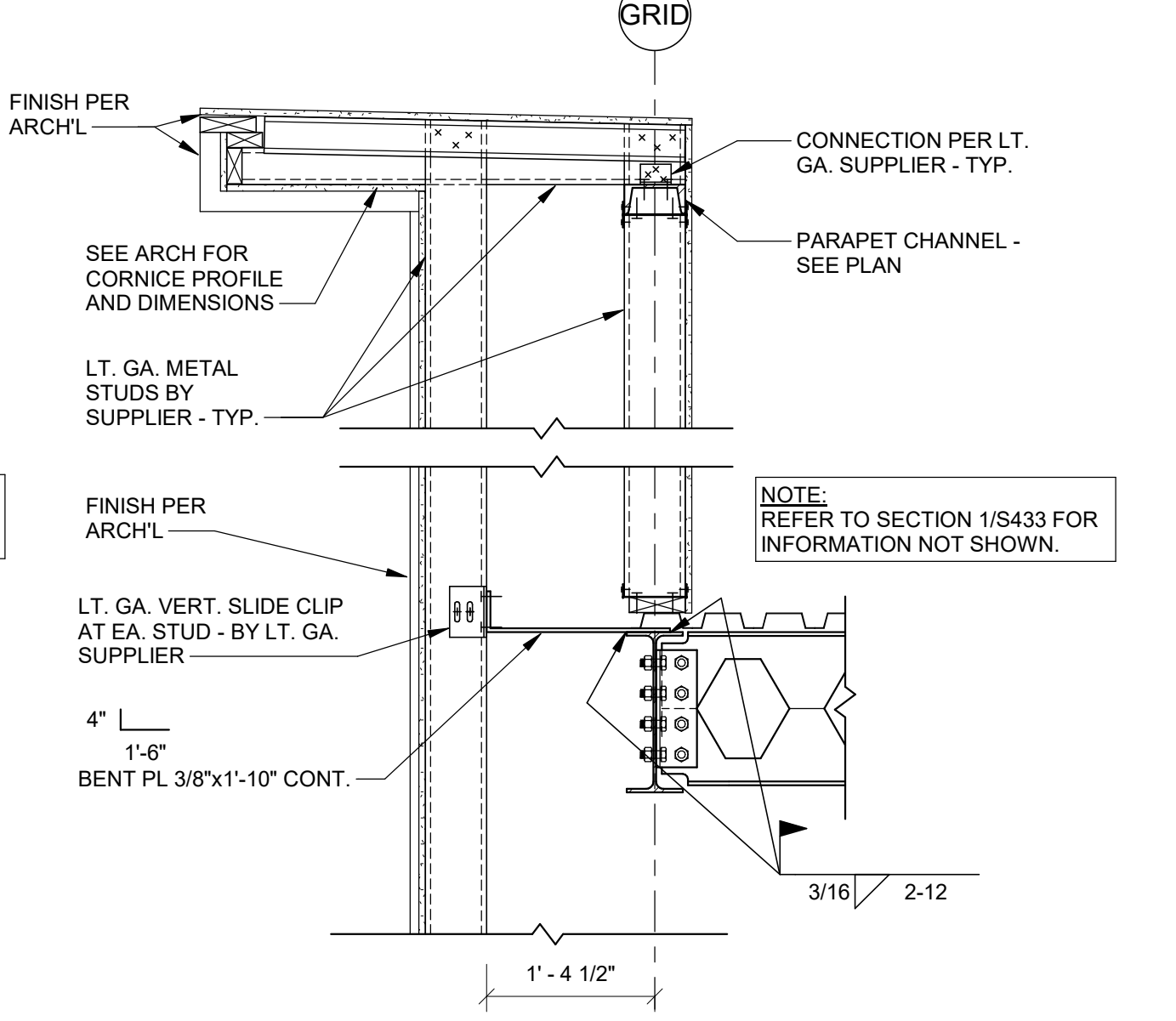
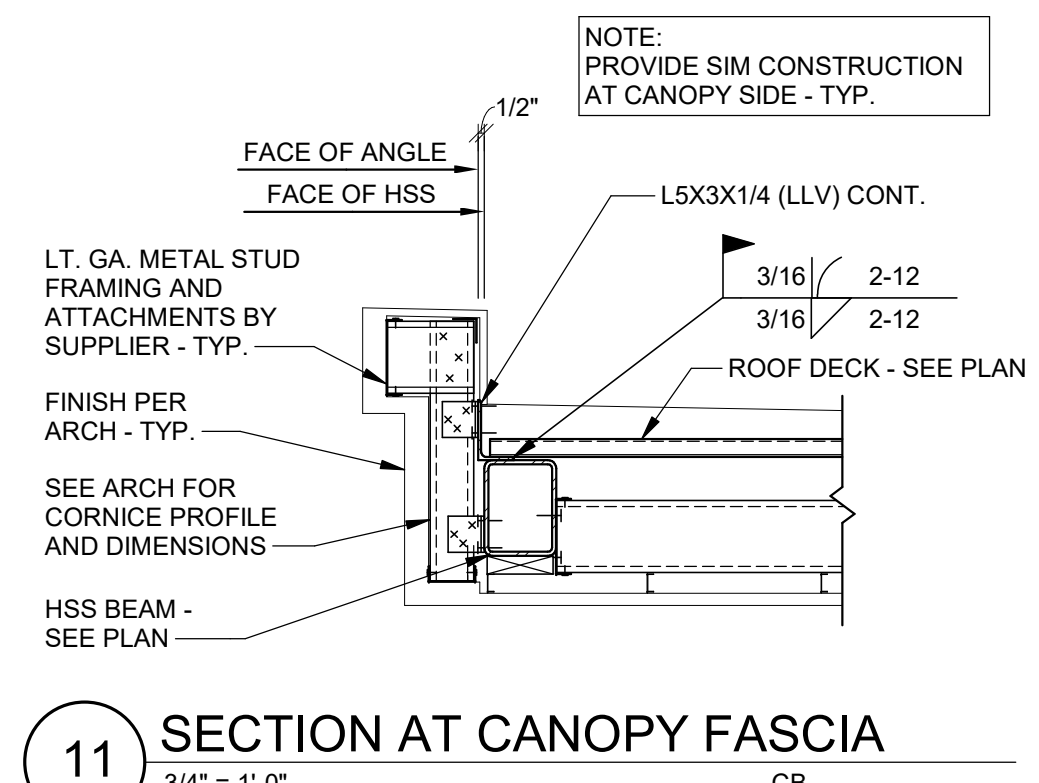
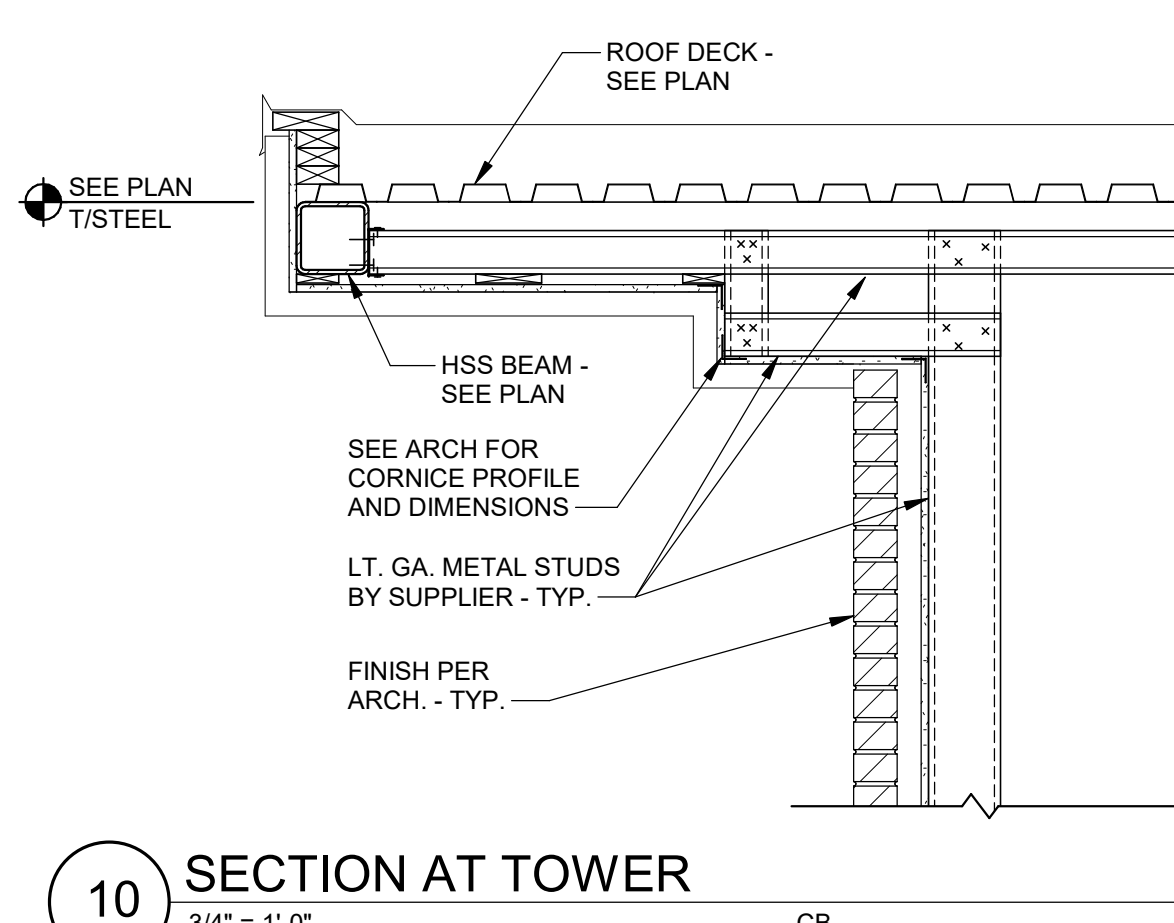
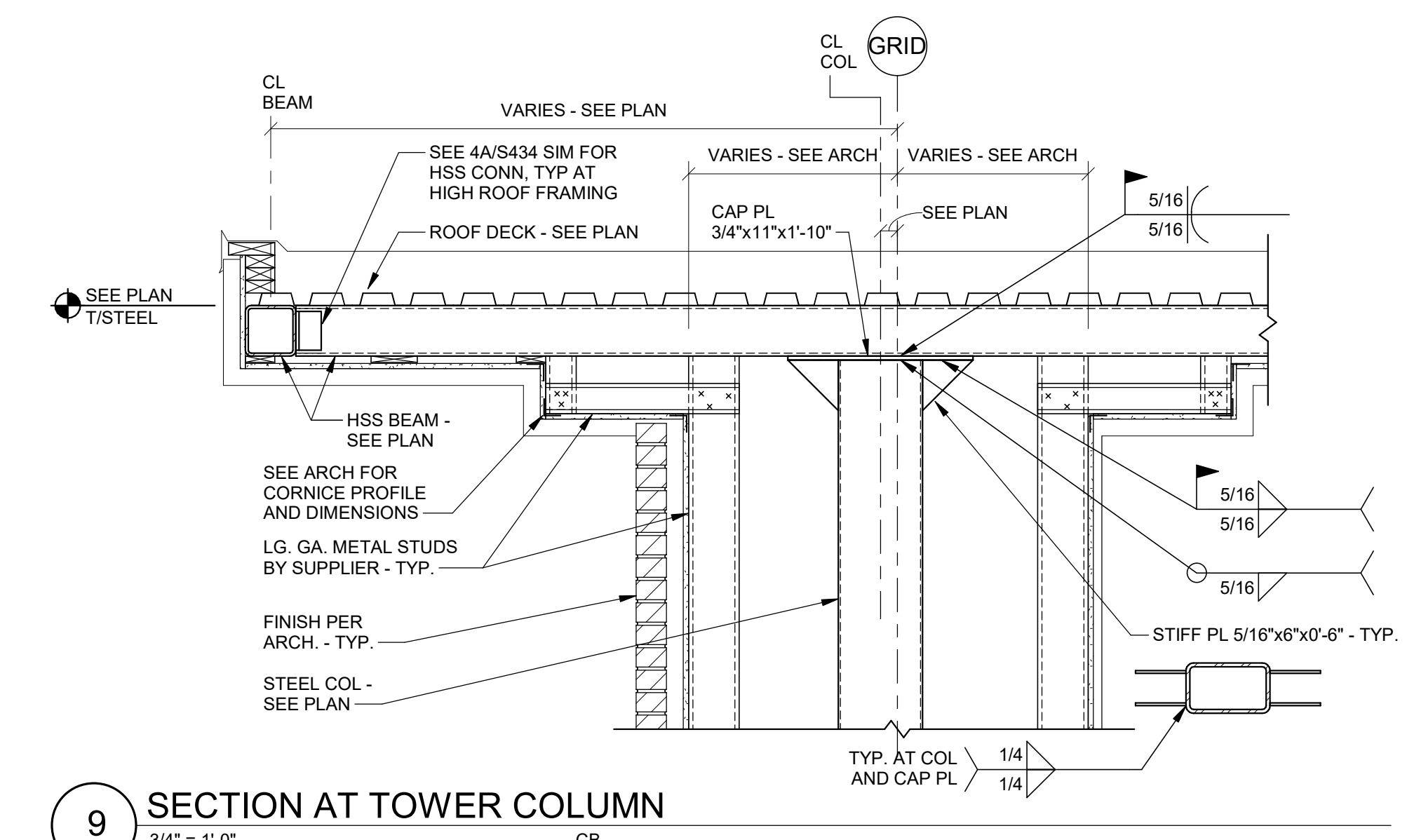
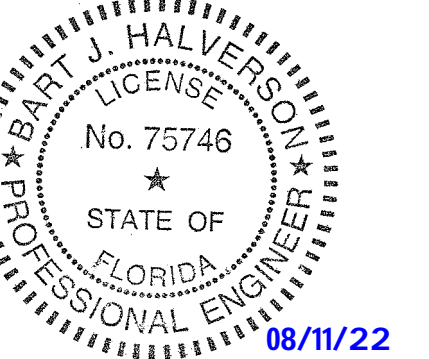
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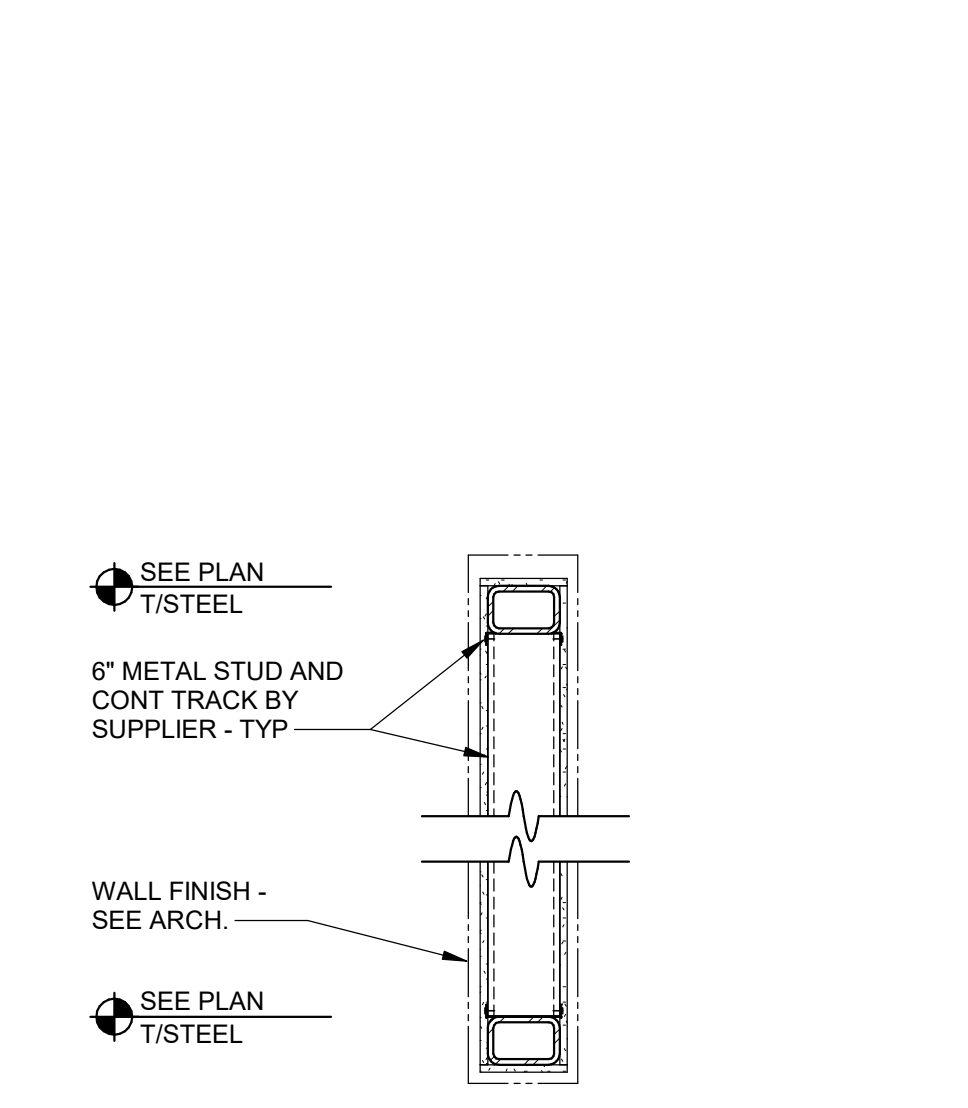
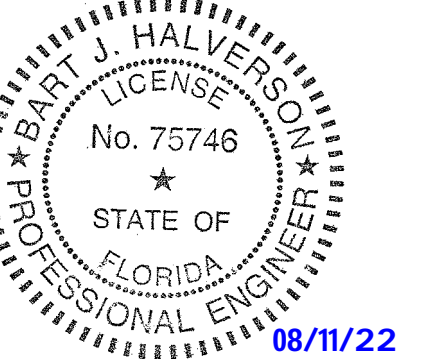


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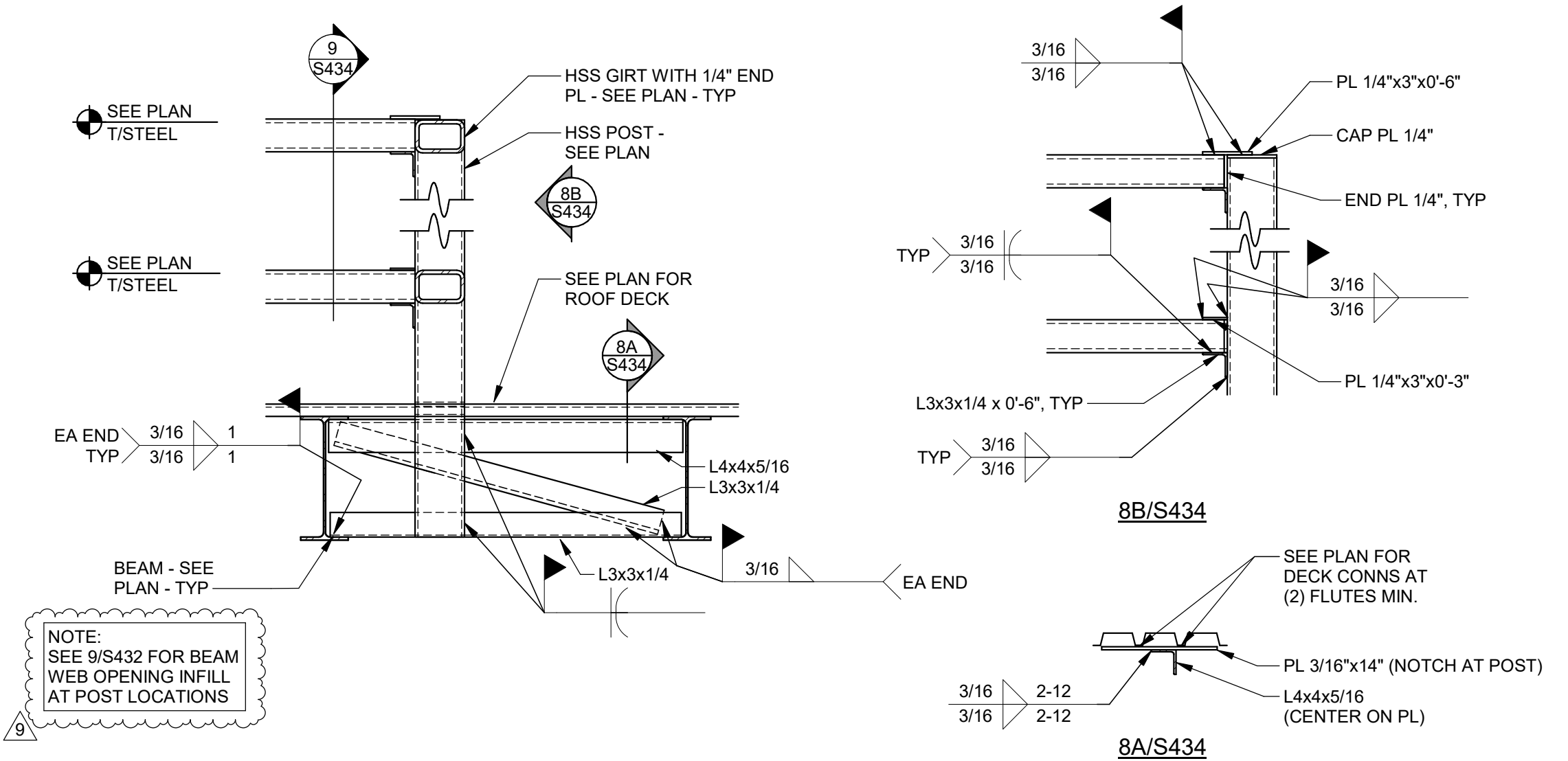
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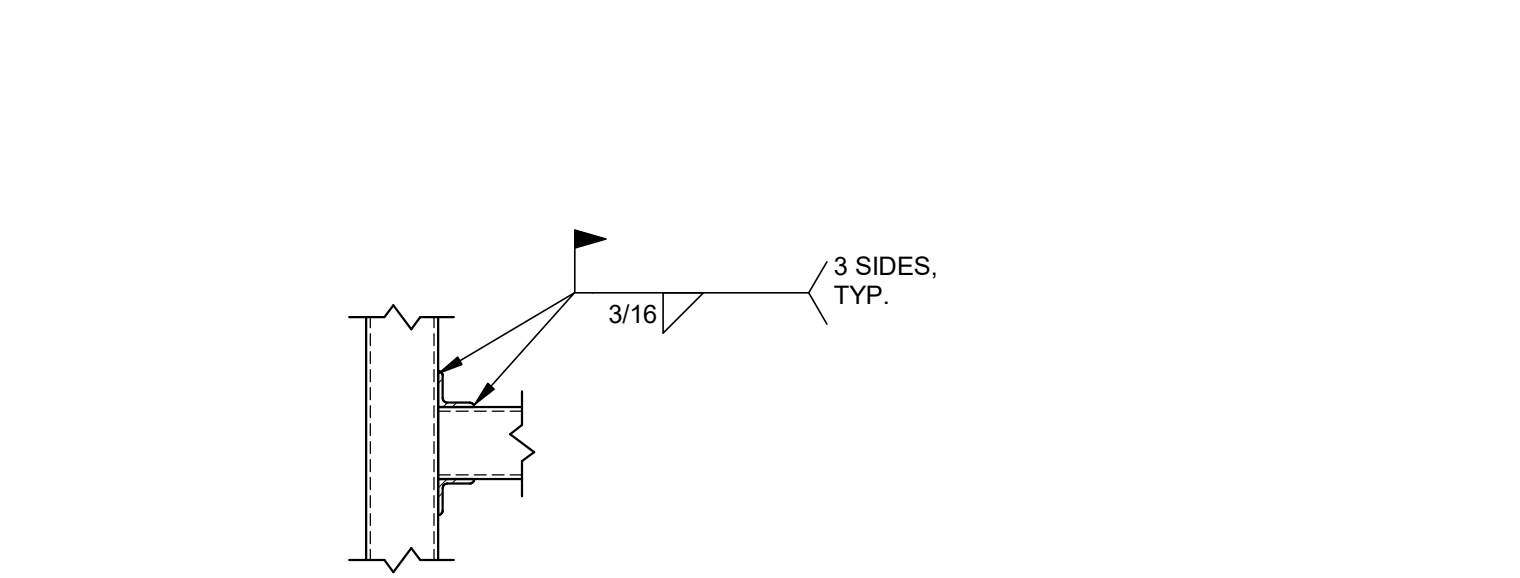
9 SCREEN WALL FRAMING
3/4" = 1'-0" NP



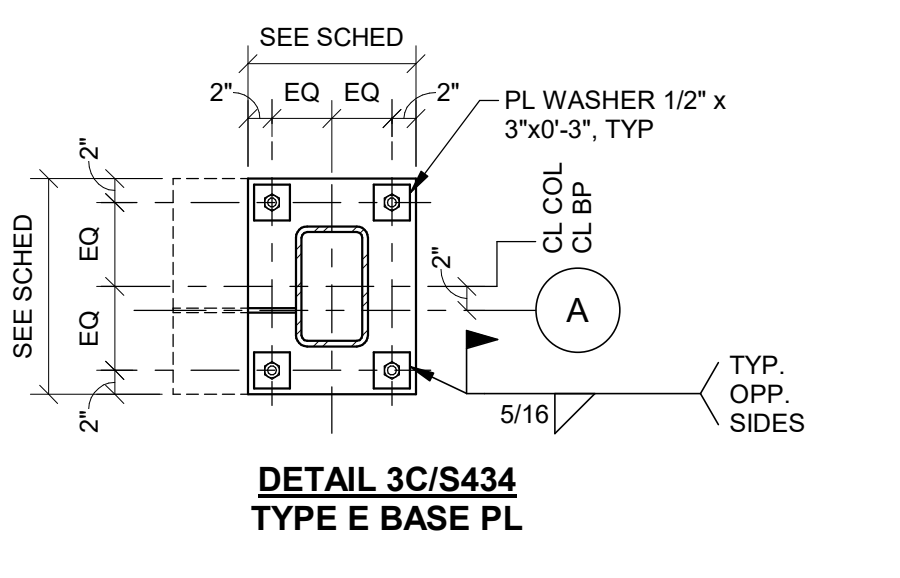
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3/4" = 1'-0" NP



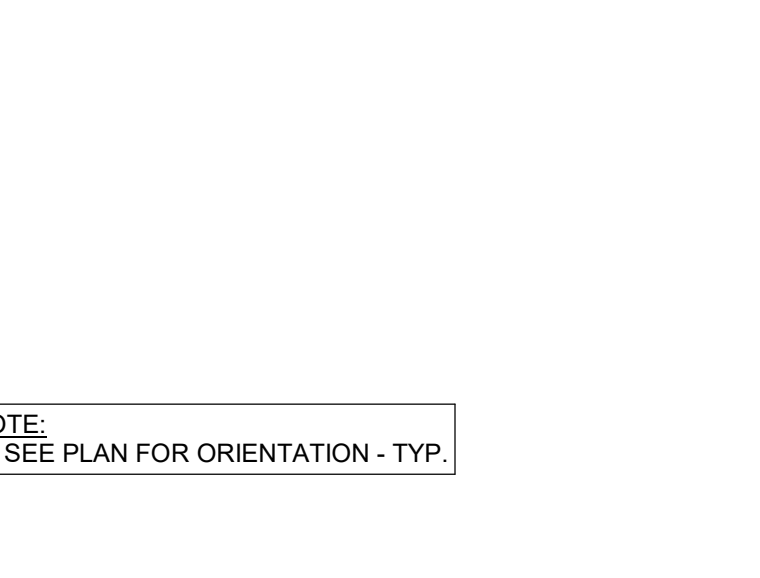
7 NOT USED
NTS



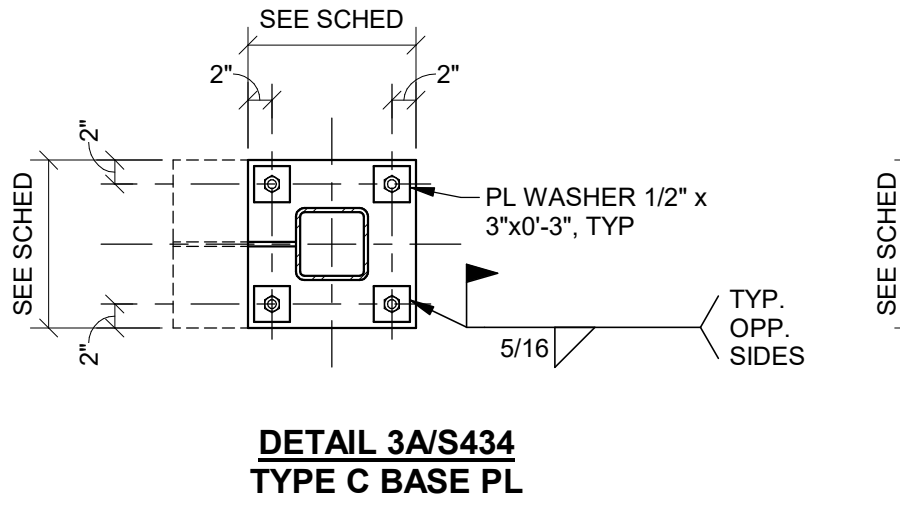
4 SECTION AT CANOPY ARM
3/4" = 1'-0" CB



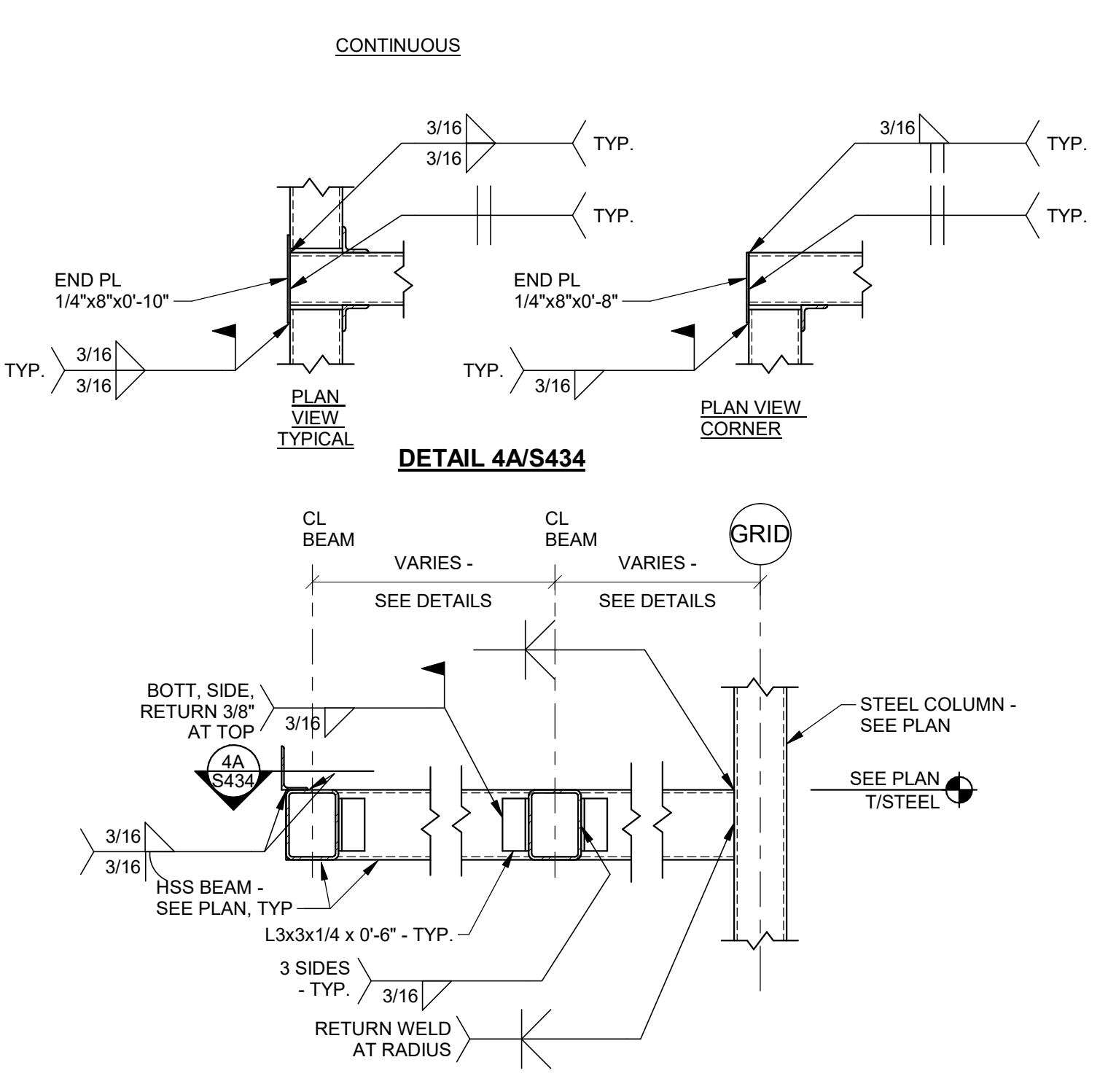
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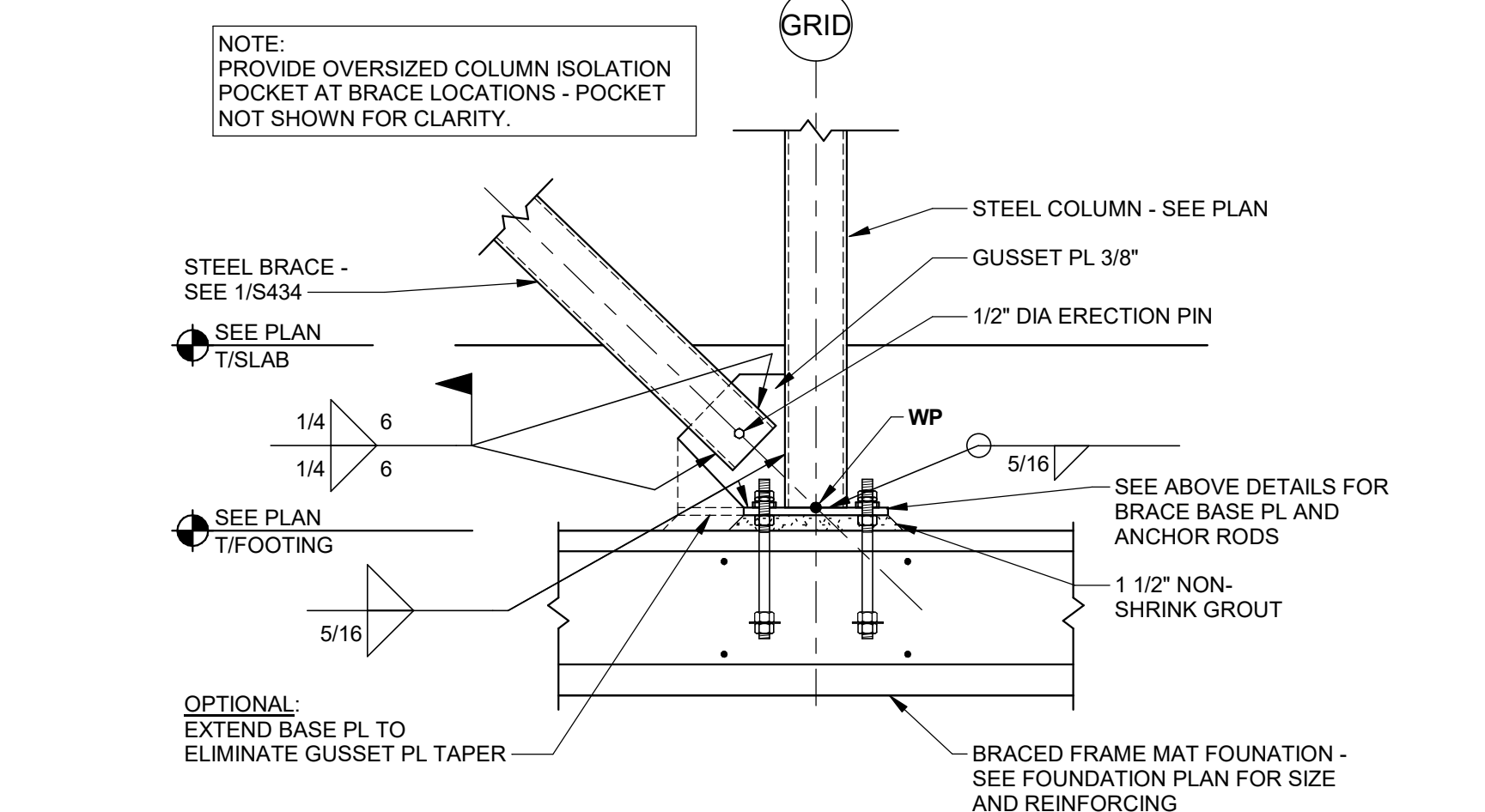
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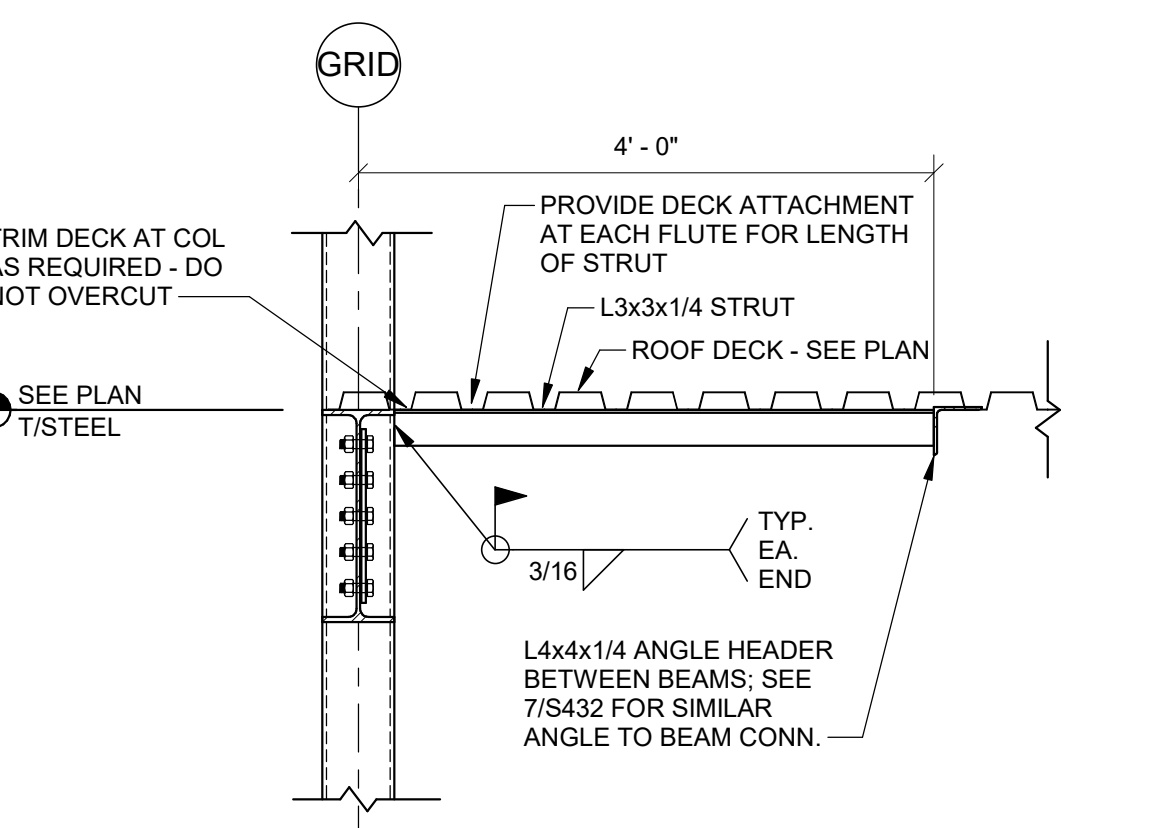
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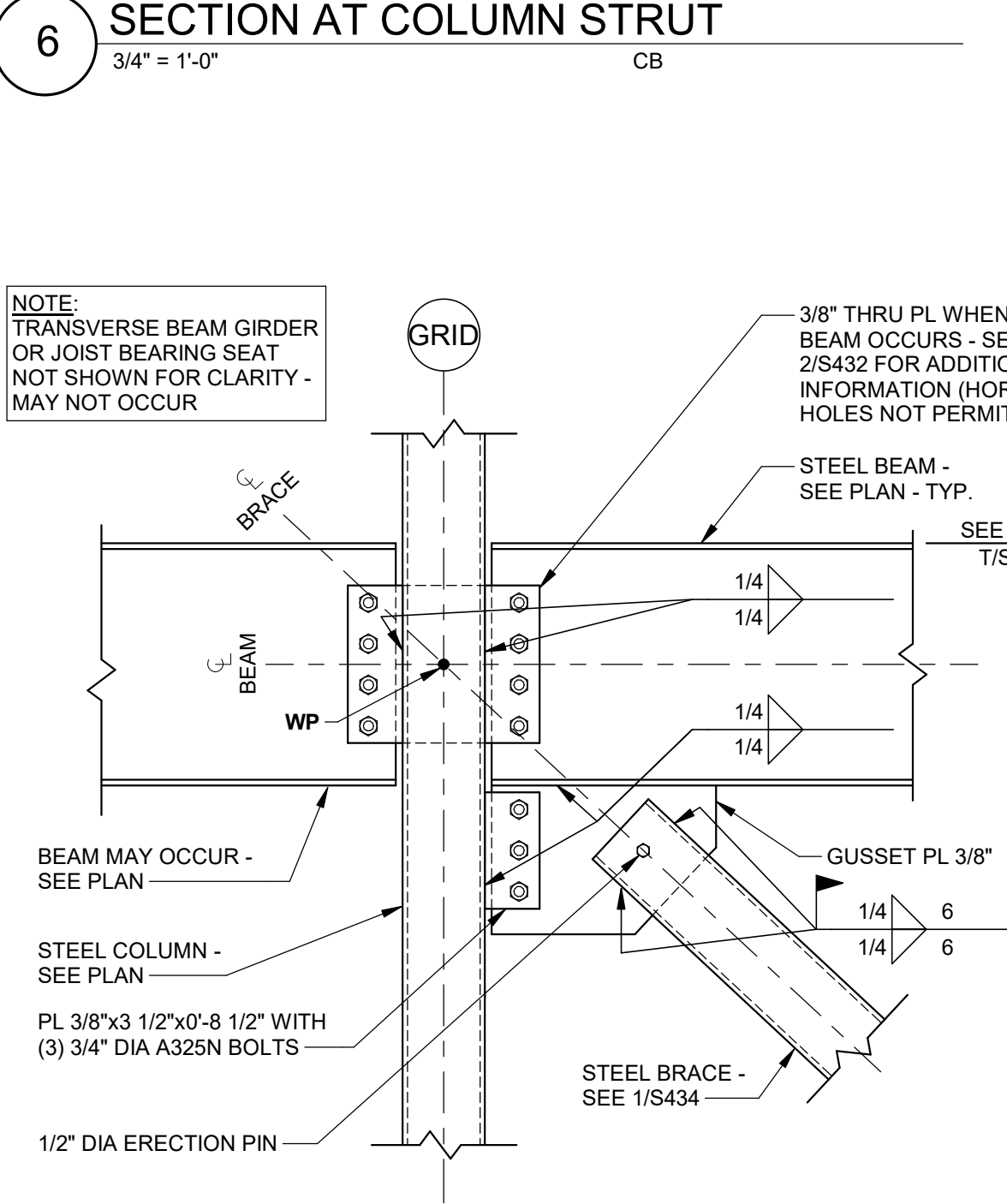
4 SECTION AT CANOPY ARM
3/4" = 1'-0" CB



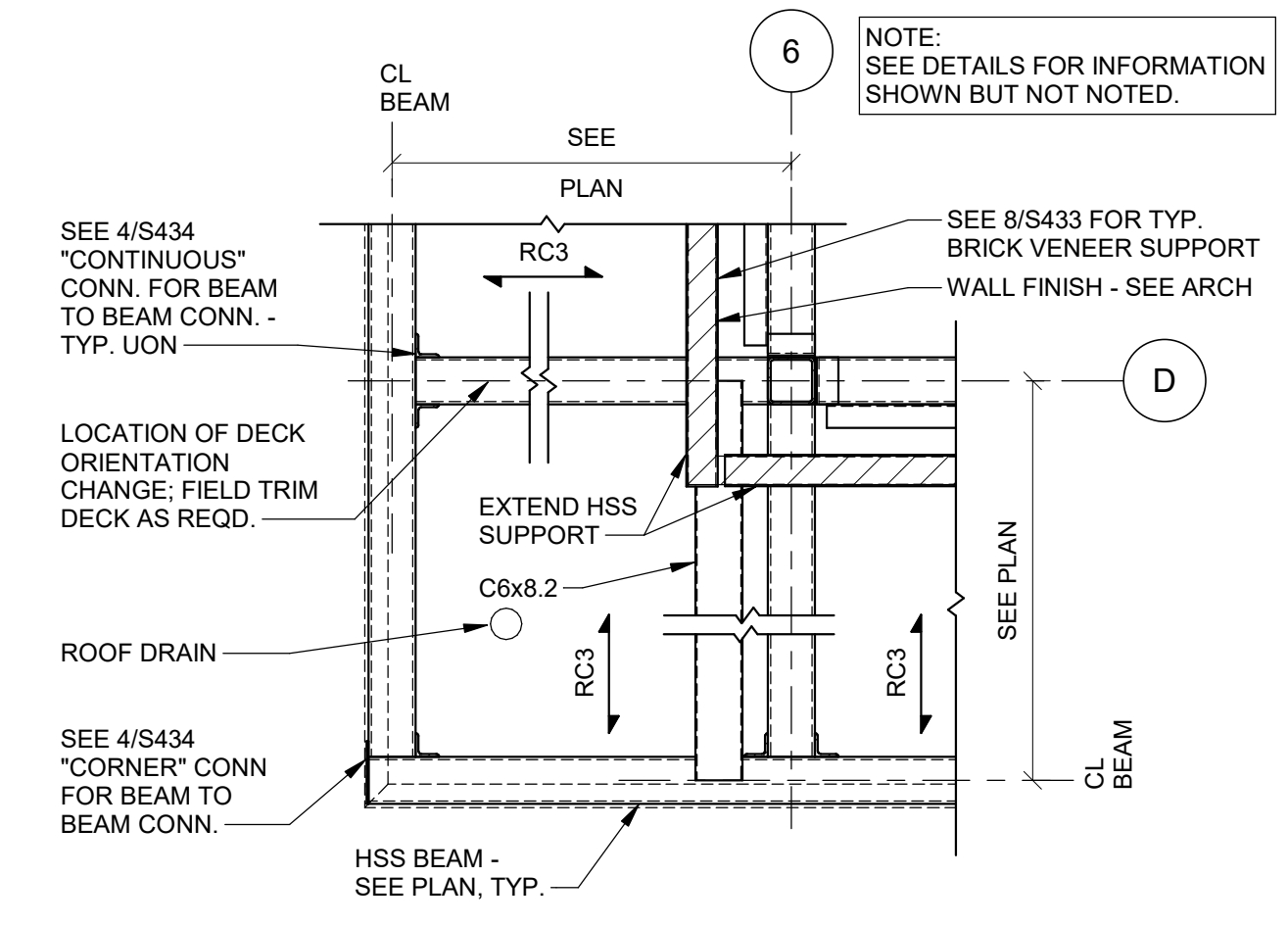
3 BRACE BASE CONNECTION
3/4" = 1'-0" CB



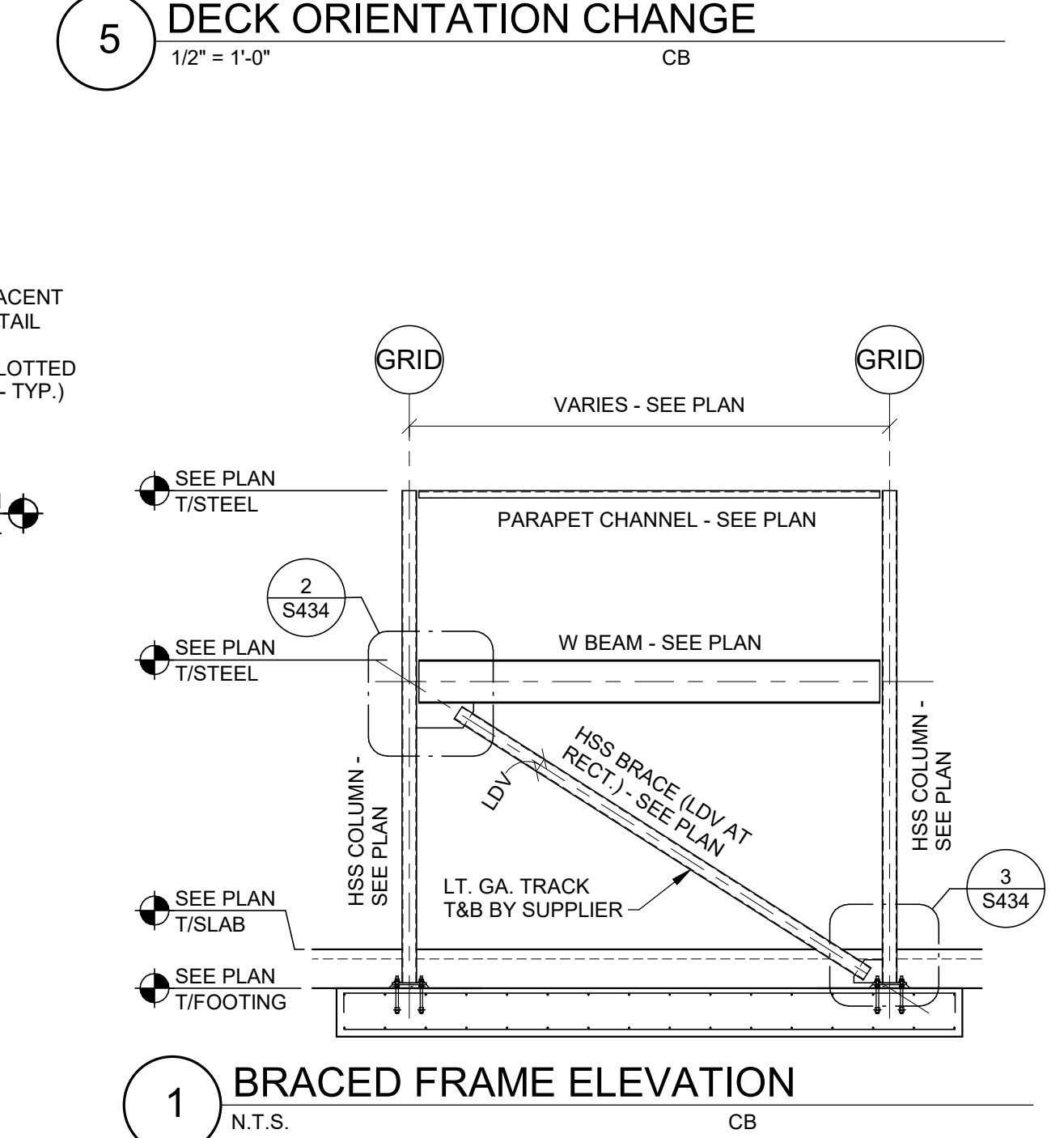
6 SECTION AT COLUMN STRUT
3/4" = 1'-0" CB



2 DIAGONAL HSS BRACE CONNECTION DETAIL
1" = 1'-0" CB



5 DECK ORIENTATION CHANGE
1/2" = 1'-0" CB



1 BRACED FRAME ELEVATION
N.T.S. CB

INCORPORATED SPB: