B. DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE

WHERE DRAWINGS AND SPECIFICATIONS ARE IN CONFLICT, THE MORE STRINGENT RESTRICTIONS AND REQUIREMENTS SHALL GOVERN.

D. PLAN NOTES, DETAILS AND SECTIONS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES. TYPICAL DETAILS AND SECTIONS NOT CUT ON PLANS SHALL APPLY UNLESS NOTED OTHERWISE.

E. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO INSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIE DOWNS. CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT CONSTRUCTION COMPLIES WITH OSHA REGULATION INCLUDING DESIGN OF CONNECTIONS OF MEMBERS THAT WILL NOT BE FULLY COMPLETED AT THE TIME OF INSTALLATION.

1011 CONTRACTOR PROPOSED CHANGES AND SUBSTITUTIONS: A. PROPOSED CHANGES OR SUBSTITUTIONS TO STRUCTURAL DETAILS OR PLANS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD (EOR) FOR REVIEW AND APPROVAL. SUBMITTALS SHALL CONTAIN FULL DOCUMENTATION OF CHANGES OR SUBSTITUTIONS WITH SUPPORTING, SEALED CALCULATIONS (WHERE APPLICABLE). THE REVIEW OF CHANGES AND SUBSTITUTIONS. RE-ANALYSIS AND/OR RE-DRAFTING TO INCORPORATE CHANGES OR SUBSTITUTIONS INTO CONTRACT DOCUMENTS ARE ADDITIONAL SERVICES FOR EOR. EOR IS NOT RESPONSIBLE FOR DETERMINING THE COST EFFECTIVENESS OF PROPOSED CHANGES.

1012 CONTRACTOR REQUIRED REMEDIAL WORK: A. DESIGN OF REMEDIAL WORK RELATED TO CONSTRUCTION ERRORS. INSTALLATIONS NOT IN CONFORMANCE WITH CONTRACT DOCUMENTS, OR IN ANY WAY BROUGHT ABOUT BY ACTIVITIES OF THE CONTRACTOR, IS NOT WITHIN THE SCOPE OF CA SERVICES PROVIDED BY ENGINEER OF RECORD. THE CONTRACTOR SHALL CARRY IN HIS BASE BID THE COSTS FOR ENGINEERING WORK ASSOCIATED WITH THE

1061 DESIGN LOADS:

A. THE STRUCTURAL SYSTEM FOR THIS BUILDING HAS BEEN DESIGNED IN ACCORDANCE WITH THE FBC 2020 BUILDING CODE.

B. LIVE LOADS ARE REDUCED FROM THE BASE LIVE LOADS SHOWN AS ALLOWED BY THE REFERENCED BUILDING CODE. THE FOLLOWING SUPERIMPOSED DEAD LOADS AND LIVE LOADS HAVE BEEN UTILIZED:

LIVE LOAD 20 PSF. FOR SNOW LOADS SEE BELOW SUPERIMPOSED DEAD LOAD 15 PSF DEAD LOAD TO RESIST UPLIFT 5 PSF LIVE LOAD 100 PSF. SUPERIMPOSED DEAD LOAD 10 PSF. OFFICE AREAS: LIVE LOAD - 50 PSF SUPERIMPOSED DEAD LOAD - 20 PSF LIVE LOAD - 125 PSF SUPERIMPOSED DEAD LOAD - 10 PSF THE DESIGN LIVE LOAD FOR THE SLAB ON GRADE IS 125 PSF.

C. WIND DESIGN LOADS: ASCE 7-16 V(ult) = 140 MPH (3 SECOND GUST)V(asd)= 110 MPH (3 SECOND GUST)

> KZ = 1.00KZT = 1.0KD = 0.85  $GCPI = \pm 0.18$ RISK CATEGORY: II BUILDING IS CONSIDERED TO BE ENCLOSED.

1120 SHOP DRAWING REVIEW:

EXPOSURE B

A. SHOP DRAWING SUBMITTALS ARE REQUIRED FOR ALL COMPONENTS SHOWN ON THESE STRUCTURAL CONTRACT DOCUMENTS INCLUDING, BUT NOT LIMITED TO: CONCRETE MIXES, CONCRETE AND MASONRY REINFORCING, STRUCTURAL STEEL AND CONNECTIONS, STEEL JOIST, STEEL DECK, LIGHT GAGE FRAMING, & WOOD ROOF TRUSS FRAMING.

B. SHOP DRAWINGS SHALL PROVIDE ACCURATE, DETAILED DIMENSIONAL INFORMATION AS WELL AS COMPLETE SHOP AND FIELD ERECTION DETAILS NOT SHOWN ON CONTRACT DOCUMENTS NECESSARY FOR FABRICATION AND INSTALLATION OF COMPONENT.

C. SHOP DRAWINGS SHALL BE REVIEWED AND APPROVED BY THE CONTRACTOR'S FIELD ENGINEER PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. DRAWINGS SUBMITTED WITHOUT REVIEW WILL BE RETURNED UNCHECKED.

). SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC.

. ELECTRONIC VERSIONS OF STRUCTURAL CONTRACT DOCUMENTS ARE THE SOLE, COPYRIGHTED PROPERTY OF KEG INC. ELECTRONIC VERSIONS OF DRAWINGS ARE NOT TO BE USED OR TRANSFERRED WITHOUT THE EXPRESS, WRITTEN PERMISSION OF KEG INC. USERS WILL SIGN A RELEASE AND REIMBURSE KEG ON KARINS. FOR EXPENSES INCURRED IN PREPARING AND TRANSMITTING ELECTRONIC DRAWINGS AT THE RATE TO BE DETERMINED UPON REQUEST.

F. CHANGES AND ADDITIONS MADE ON RE-SUBMITTALS SHALL BE CLEARLY FLAGGED AND NOTED. THE PURPOSE OF THE RE-SUBMITTALS SHALL BE CLEARLY NOTED ON THE LETTER OF TRANSMITTAL. ENGINEER REVIEW WILL BE LIMITED TO THOSE ITEMS CAUSING THE RE-SUBMITTAL.

1121 SHOP DRAWINGS FOR SPECIALTY ENGINEERED PRODUCTS: A. THE FOLLOWING SYSTEMS AND COMPONENTS AS A MINIMUM REQUIRE FABRICATION AND ERECTION DRAWINGS PREPARED BY A DELEGATED ENGINEER. DELEGATED ENGINEER SHALL POSSESS CURRENT PROFESSIONAL LICENSURE IN THE JURISDICTION OF THE PROJECT.

LIGHT GAGE STEEL EXTERIOR WALL SYSTEMS, ALUMINUM WALL SYSTEMS, GLAZED CURTAIN WALLS, PREFABRICATED STEEL STAIRS & RAILINGS. ARCHITECTURAL PRECAST CONCRETE ELEMENTS. STRUCTURAL PRECAST SYSTEMS, GLASS FIBER REINFORCED CONCRETE PANEL SYSTEMS, OPEN WEB STEEL JOISTS, STRUCTURAL STEEL CONNECTIONS REQUIRING ENGINEERING, & TILT—WALL ERECTION DRAWINGS.

B. SUBMITTALS SHALL CLEARLY IDENTIFY THE SPECIFIC PROJECT AND APPLICABLE CODES. LIST THE DESIGN CRITERIA, AND SHOW ALL DETAILS AND PLANS NECESSARY FOR PROPER FABRICATION AND INSTALLATION. CALCULATIONS AND SHOP DRAWINGS SHALL IDENTIFY SPECIFIC PRODUCT UTILIZED. GENERIC PRODUCTS WILL NOT BE ACCEPTED.

C. SHOP DRAWINGS AND CALCULATIONS SHALL BE PREPARED UNDER THE DIRECT SUPERVISION AND CONTROL OF THE DELEGATED ENGINEER. SHOP DRAWINGS AND CALCULATIONS REQUIRE THE IMPRESSED SEAL, DATE AND SIGNATURE OF THE DELEGATED ENGINEER.

REVIEW BY THE STRUCTURAL ENGINEER OF RECORD OF SUBMITTALS IS LIMITED TO VERIFYING

THAT THE SPECIFIED STRUCTURAL SUBMITTALS HAVE BEEN FURNISHED. THAT THE STRUCTURAL SUBMITTALS HAVE BEEN SIGNED AND SEALED BY THE DELEGATED 3. THAT THE DELEGATED ENGINEER HAS UNDERSTOOD THE DESIGN INTENT AND HAS USED THE SPECIFIED STRUCTURAL CRITERIA. (NO DETAILED CHECK OF CALCULATIONS WILL BE MADE). 4. THAT THE CONFIGURATION SET FORTH IN THE STRUCTURAL SUBMITTALS IS CONSISTENT WITH THE CONTRACT DOCUMENTS. (NO DETAILED CHECK OF DIMENSIONS OR QUANTITIES WILL BE MADE).

E. SUBMITTALS NOT MEETING THE ABOVE CRITERIA WILL NOT BE REVIEWED.

A. ALL CONTRACTOR RFI SHALL STATE CONTRACTOR'S SUGGESTION(S) FOR RESOLUTION AND COST IMPLICATIONS FOR SUGGESTION(S). KEG INC, IS NOT RESPONSIBLE FOR DETERMINING COST OR COST EFFECTIVENESS OF RFI RESPONSES.

A. SEE THE FOLLOWING REPORT FOR COMPLETE GEOTECHNICAL RECOMMENDATIONS AND INSTALLATION PROCEDURES:

> GEOTECHNICAL ENGINEERING REPORT BELTWAY COMMERCE SELF-STORAGE WOOD PINE DRIVE ORLANDO, ORANGE COUNTY, FLORIDA 32929 ESC PROJECT #24:6660

PREPARED FOR: CONDEV PROPERTIES LLC 921 N PENNSYLVANIA AVENUE WINTER PARK, FL, 32789 PREPARED BY: ESC FLORIDA, LLC GEOTECHNICAL ENGINEERING REPORT BELTWAY COMMERCE SELF-STORAGE WOOD PINE DRIVE ORLANDO, ORANGE COUNTY, FLORIDA 32829 PROJECT NUMBER: ESCPROJECT NO 24:6660 JULY 17, 2020

THIS REPORT SHALL BE CONSIDERED PART OF THE CONTRACT DOCUMENTS.

FOUNDATIONS ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING VALUE OF 2,000 PSF AT CMU WALLS, 3,000 PSF AT COLUMNS & POSTS AS NOTED WITHIN THE PRELIMINARY GEOTECHNICAL REPORT. FINAL ALLOWABLE BEARING VALUE SHALL BE DETERMINED BY GEOTECH PRIOR TO ANY

B. NO APPROVAL OR VERIFICATION OF RECOMMENDATIONS MADE WITHIN THE ABOVE NOTED GEOTECHNICAL REPORT IS IMPLIED THROUGH REFERENCE OR USE BY KEG INC. C. A GEOTECHNICAL ENGINEER, LICENSED WITHIN THE JURISDICTION OF THE PROJECT. SHALL VERIFY IN THE FIELD THAT ALL SITE PREPARATION FILL OPERATIONS, BEARING

D. SINKHOLE ACTIVITY: GEOTECH REPORT DOES NOT INDICATE POTENTAIL OF GROUND COVER COLLAPSE (SINKHOLE). NO MECHANISM TO PRECLUDE DAMAGE FROM POTENTIAL SINK HOLE ACTIVITY IS THUS INCLUDED IN THE FOUNDATION DESIGN. SUBGRADE PREPARATION AND VAPOR BARRIER INSTALLATION FOR SLAB-ON-GRADE SHALL BE PERFORMED IN ACCORDANCE WITH PROJECT GEOTECHNICAL REPORT.

CONDITIONS, FOUNDATION TESTING AND INSTALLATION COMPLY WITH THE SOILS REPORT.

F. CONCRETE FOR FOOTINGS SHALL BE PLACED IMMEDIATELY AFTER FINAL INSPECTION AND ACCEPTANCE BY THE GEOTECHNICAL ENGINEER. IN NO CASE SHALL FOOTING EXCAVATIONS BE ALLOWED TO STAND OPEN OVERNIGHT OR DURING RAIN.

G. FOUNDATION WALLS WITHOUT CANTILEVERED FOOTINGS SHALL NOT BE BACKFILLED UNTIL SHORED OR PERMANENTLY SUPPORTED AT THE TOP OF WALL.

H. BACKFILLING OF WALLS AND PIERS SHALL BE PLACED SUCH THAT SYMMETRICAL LOADING SHALL BE MAINTAINED ON BOTH SIDES. WHERE DESIGN CONDITIONS REQUIRE BACKFILLING EACH SIDE TO UNEQUAL HEIGHTS, WALLS OR PIERS SHALL BE FIRMLY SHORED ON POSITION. AND SHORES SHALL REMAIN UNTIL FLOORS OR OTHER PERMANENT BRACING ELEMENTS ARE PLACED AND PROPERLY SET TO PROVIDE FULL SUPPORT.

I. GRADE SHALL BE SUCH THAT THICKNESS OF FOUNDATION, SLAB ON GRADE, ETC. IS NOT REDUCED BY MORE THAN 5% OF THAT SHOWN ON DRAWINGS.

J. FOUNDATION DESIGN ASSUMES INSTALLATION OF EARTHQUAKE DRAINS AS OUTLINED IN GEO-TEC REPORT.

3101 FORMWORK AND SHORING (CONCRETE SLABS AND BEAMS): A. NO STRUCTURAL CONCRETE SHALL BE STRIPPED UNTIL IT HAS REACHED AT LEAST TWO-THIRDS OF THE 28 DAY DESIGN STRENGTH (& ALL TENDONS STRESSED FOR PT SLABS). A MINIMUM OF 3 STORIES OF SHORING AND (/OR) RESHORING SHALL BE USED

VERTICAL SHORES THAT COMPRISE AT LEAST 50% OF A COMPLETE SET. B. DRAWINGS FOR SHORING AND RESHORING SHALL BE PREPARED BY AN ENGINEER

WHICH SHALL CONSIST OF ONE COMPLETE SET OF VERTICAL SHORES AND TWO SETS OF

C. DESIGN, ERECTION AND REMOVAL OF ALL FORMWORK, SHORES AND RESHORES SHALL MEET REQUIREMENTS SET FORTH IN ACI STANDARDS 347 AND 301.

D. SUBMIT SIGNED & SEALED SHORING DRAWINGS INCLUDING POUR SEQUENCE AND CALCULATIONS, WHERE NECESSARY, TO DEMONSTRATE THAT THE POUR SEQUENCE AND SHORING/RE-SHORING METHODS DO NOT OVERSTRESS THE STRUCTURE. THIS ANALYSIS SHALL INCLUDE STRESSES CAUSED BY SHRINKAGE OF STRUCTURAL SLAB. PROVIDE LOCATION AND DETAILS OF POUR STRIPS IF REQUIRED TO REDUCE SHRINKAGE AND

E. SHORING INSPECTIONS SHALL BE PERFORMED BY THE SHORING ENGINEER.

F. UNLESS ARCHITECT SPECIFIES OTHERWISE, CONSTRUCT FORMWORK SO CONCRETE SURFACES CONFORM TO THE TOLERANCE LIMITS OF ACI 117 STANDARD SPECIFICATIONS FOR TOLERANCES FOR CONCRETE CONSTRUCTION MATERIALS]. THE CLASS OF SERVICE FOR OFFSET BETWEEN ADJACENT PIECES OF FORMWORK FACING MATERIAL SHALL BE CLASS B FOR SURFACES PERMANENTLY EXPOSED TO PUBLIC VIEW AND CLASS D FOR SURFACES THAT WILL BE PERMANENTLY CONCEALED.

3103 PLUMBING SLEEVES AND EMBEDDED CONDUITS:

CONFORM TO SECTION 6.3 OF ACI 318 AND THE FOLLOWING:

LICENSED WITHIN THE JURISDICTION OF THE PROJECT.

A. LOCATION DRAWINGS FOR ALL SLEEVES AND BLOCKOUTS IN THE CONCRETE SHALL BE SUBMITTED FOR APPROVAL BY THE STRUCTURAL ENGINEER PRIOR TO PLACEMENT. B. All CONDUIT, SLEEVES, AND PIPES EMBEDDED IN OR PASSING THRU CONCRETE SHALL

I. SLEEVES AND PIPES SHALL BE PLACED SO THAT REINFORCING STEEL CAN BE PLACED WITH THE SPECIFIED COVER AND CLEAR DISTANCE BETWEEN BARS.

2. MINIMUM SLEEVE SPACING SHALL BE THREE DIAMETERS CENTER TO CENTER OF THE LARGER SLEEVE OR 6" CLEAR BETWEEN SLEEVES, WHICHEVER IS GREATER.

3. SLEEVES OR GROUPS OF SLEEVES 16 INCH IN DIAMETER AND LARGER SHALL BE TREATED AS A SLAB OPENING AND REINFORCED PER TYPICAL OPENING REINFORCING DETAILS. 4. CONDUIT AND PIPES PLACED WITHIN SLABS, BEAMS, WALLS AND TOPPING OVER

SLABS SHALL OCCUPY ONLY THE MIDDLE ONE THIRD OF THE MEMBER DEPTH OR THICKNESS. MAXIMUM CONDUIT O.D. FOR SINGLE CONDUITS OR SUM OF O.D.'S FOR MULTIPLE CONDUITS THAT CROSS SHALL BE NO LARGER THAN ONE THIRD THE MEMBER DEPTH. PARALLEL CONDUITS SHALL BE SPACED WITH A MINIMUM OF 3 DIAMETERS CLEAR CONDUITS SHALL BE A MINIMUM OF ONE DIAMETER AWAY FROM AND SHALL NOT INTERFERE WITH OR DISPLACE ANY TENDONS OR REINFORCING. CONDUIT SHALL NOT BE TIED TO REINFORCING OR TENDONS. CONDUITS SHALL NOT OCCUR WITHIN TRANSFER GIRDERS OR COLUMN ZONES OF SLABS.

5. CONDUITS AND PIPES PLACED IN COLUMNS SHALL NOT DISPLACE MORE THAN 4% OF THE CROSS SECTIONAL AREA OF COLUMN AND SHALL BE LOCATED ON THE CENTER LINE OF COLUMN. OUTLET BOXES IN COLUMNS SHALL BE APPROVED BY THE ENGINEER, SHALL NOT DISPLACE REINFORCING AND SHALL NOT BE DEEPER THAN REQUIRED CLEARANCE FOR

3104 CONSTRUCTION JOINTS AND CONTROL JOINTS: A. CONSTRUCTION JOINTS AND CONTROL JOINTS SHALL BE LOCATED AS SHOWN IN PLAN

B. UNLESS NOTED OTHERWISE, CONTROL JOINTS IN SLABS ON GRADE SHALL BE PROVIDED SO THAT THE MAXIMUM DISTANCE BETWEEN JOINTS SHALL BE NO MORE THAN 3 TIMES THE SLAB THICKNESS IN FEET (OR AS SHOWN ON PLANS). SAWCUT CONTROL JOINTS SHALL BE MADE AS SOON AS SLAB WILL SAFELY SUPPORT MEN AND EQUIPMENT AND THE SLAB WILL NOT BE DAMAGED BY EQUIPMENT. ASPECT RATIO (LONGSIDE TO SHORTSIDE OF CONCRETE AREA) SHALL NOT EXCEED 1.5.

C. DEVIATION FROM OR ADDITION TO CONSTRUCTION OR CONTROL JOINT LOCATIONS SHOWN SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL AND ARE ACCEPTABLE ONLY AS A CHANGE ORDER THAT WILL INCLUDE ENGINEERING CHARGES BY THE ENGINEER OF RECORD FOR REDESIGN OF THE STRUCTURE AS REQUIRED. SHORING REQUIREMENTS TO IMPLEMENT REVISED CONSTRUCTION JOINTS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

3201 REINFORCING STEEL: A. SHALL BE ASTM A615 GRADE 60 DEFORMED BARS (WELDABLE REINFORCING "DBA" SHALL CONFORM TO ASTM A-706 GRADE 60), FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. SECURE APPROVAL OF SHOP DRAWINGS PRIOR TO COMMENCING FABRICATION. REINFORCING BAR DETAILING SHALL COMPLY WITH ACI 315 "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" AND

B. CLEAR COVER TO REINFORCING SHALL BE AS INDICATED BELOW. WHERE A SPECIFIC CONDITION IS NOT NOTED, REFER TO ACI REQUIREMENTS FOR COVER:

CRSI MANUAL OF STANDARD PRACTICE.

STRUCTURAL ENGINEER.

TOP BOTTOM SIDES/EDGES FRAMED SLABS ON GRADE 1" 3 2" EXTERIOR FOUNDATION WALL/PILASTER NA NA 1 1/2"INTERIOR SPREAD FOOTING COLUMNS & WALLS 2" AGAINST N NA 1 1/2" TYPICAL ELEVATED FRAMED SLAB, 3/4 3/4" INTERIOR PT INTERIOR NON-PT 3/4" 3/4" 1" 3/4" 1" EXTERIOR PT EXTERIOR NON-PT - #5 AND SMALLER 1 1 1/2" #6 AND LARGER 2" 2" 1 1/2" 1 1/2" 1 1/2" (EXTERIOR EXPOSURE) 2" 1 1/2" 1 1/2"

NOTE: MAXIMUM DEVIATION IN BAR PLACEMENT SHALL BE AS DICTATED BY ACI.

C. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR DRIPS, CHAMFERS, REGLETS, SLOTS, SLEEVES, ANCHORS, AND INSERTS. UNLESS SHOWN ON STRUCTURAL DRAWINGS NO OPENINGS LARGER THAN 12"x12" SHALL BE PLACED IN SLABS OR WALLS. FOR OPENINGS NOT SHOWN ON STRUCTURAL DRAWINGS, APPROVALS MUST BE OBTAINED FROM THE ENGINEER PRIOR TO FABRICATION OF STEEL AND PLACEMENT OF CONCRETE. SEE NOTES ON EMBEDDED ITEMS FOR ADDITIONAL LIMITATIONS.

PROVIDE CONTINUOUS REINFORCING WHERE POSSIBLE, SPLICE ONLY AS SHOWN ON DRAWINGS OR AS APPROVED BY STRUCTURAL ENGINEER. PROVIDE CORNER BARS AT ALL WALL, GRADE BEAM AND STRIP FOOTING CORNERS. BARS SHALL BE THE SAME SIZE AND SPACING AS THE HORIZONTAL REINFORCING. INTERSECTING WALLS, GRADE BEAMS AND STRIP FOOTINGS SHALL BE DOWELED TOGETHER IN THE SAME MANNER. PROVIDE 2 NO. 4 TOP DIAGONAL BARS 4'-0" LONG AT ALL REENTRANT CORNERS IN ALL SLABS ON GRADE AND ELEVATED SLABS.

. SHOP DRAWINGS SHALL ADEQUATELY DEPICT THE REINFORCING BAR SIZES AND PLACEMENT. SHOP DRAWINGS SHALL INCLUDE ADEQUATE SECTIONS, ELEVATIONS AND DETAILS. WRITTEN DESCRIPTIONS ARE NOT ACCEPTABLE. ALL CONCRETE WALLS SHALL BE DETAILED IN ELEVATION. H. SPLICING OF REINFORCING SHALL BE AS SHOWN OR AS INDICATED IN SCHEDULE. MECHANICAL SPLICING DEVICES SHALL DEVELOP 125% OF THE SPECIFIED YIELD STRENGTH (FY) OF THE BAR. STAGGER MECHANICAL SPLICES WHERE POSSIBLE.? DO NOT WELD OR TACK WELD REINFORCING STEEL UNLESS APPROVED OR DIRECTED BY THE H. TIE ALL REINFORCING AND EMBEDS SECURELY IN PLACE PRIOR TO PLACING CONCRETE. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN THE POSITION OF REINFORCEMENT AND EMBEDS WITHIN SPECIFIED TOLERANCES DURING ALL CONSTRUCTION ACTIVITIES.

I. THE SHOP DRAWINGS FOR REINFORCING STEEL SHALL INCLUDE SCALE ELEVATIONS OF ALL CONCRETE WALLS.

OPENINGS THROUGH CONCRETE WALLS, SLABS OR OTHER STRUCTURAL ELEMENTS NOT DETAILED ON THE STRUCTURAL DRAWINGS MUST BE LOCATED AND SHOWN ON THE APPLICABLE REINFORCING STEEL SHOP DRAWINGS. THE FINAL LOCATION OF ALL OPENINGS MUST BE REVIEWED BY THE A/E BEFORE THE CONCRETE IS POURED.

A. WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A 185, FREE FROM OIL, SCALE, AND RUST, AND PLACED IN ACCORDANCE WITH THE TYPICAL PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. MINIMUM LAP SHALL BE ONE SPACE PLUS TWO INCHES. USE OF FLAT MANUFACTURED SHEETS IS RECOMMENDED. B. THE WELDED WIRE FABRIC IN THE COMPOSITE ELEVATED SLAB SHALL BE SUPPORTED BY PLACING CONTINUOUS HEAVY BOLSTERS AT 2'-6" O.C. MAXIMUM OVER THE COMPOSITE METAL C. THE WELDED WIRE FABRIC IN THE CONCRETE SLAB-ON-GRADE SHALL BE SUPPORTED BY CONTINUOUS #4 SUPPORT BARS AT 2'-6" O.C. MAXIMUM. THE #4 BARS SHALL BE TIED AND SUPPORTED BY CONCRETE BRICK SPACERS (2 1/4" HIGH) AT 2'-6" O.C. MAXIMUM.

3202 WELDED WIRE FABRIC:

3000 PSI FOR FOUNDATIONS

ENGINEER FOR USE. MIX DESIGN SHALL INCLUDE THE FOLLOWING:

USE OF WATER REDUCERS WHERE REINFORCING CONGESTION WARRANTS.

3203 FIBER REINFORCING: MICRO FIBER SHALL CONFORM TO ASTM D7508 "FRC CSD-2500" AS SUPPLIED BY FRC INDUSTRIES - SYNTHETIC MACRO FIBER SHALL BE "TUF-STRAND SF" AS SUPPLIED BY EUCLID CHEMICAL CO. OR APPROVED EQUAL, REFERENCE PLAN NOTES FOR DOSAGES.

3301 CAST-IN-PLACE CONCRETE: A. ALL CAST-IN-PLACE CONCRETE SHALL BE PER AN APPROVED MIX DESIGN PROPORTIONED TO ACHIEVE A STRENGTH AT 28 DAYS AS LISTED BELOW WITH A PLASTIC AND WORKABLE MIX:

4000 PSI FOR SLABS ON GRADE, SLABS ON DECK, BEAMS & COLUMNS. NOTE THAT SLAB ON GRADE MIX FOR INTERIOR SLABS SHALL CONTAIN HRWR.

B. CONCRETE SHALL BE PLACED AND CURED ACCORDING TO ACI 301(-05) STANDARDS AND C. PRIOR TO CONCRETE PLACEMENT, MIX DESIGN SHALL BE SUBMITTED AND ACCEPTED BY

1. MIX DESIGN WHICH SHALL INCLUDE TESTED, STATISTICAL BACK-UP DATA AS PER CHAPTER 5 OF ACL 318 2. ONLY TYPE II CEMENT SHALL BE USED FOR SLAB-ON GRADE CONCRETE. 3. CONCRETE MIX DESIGNS SHALL INCLUDE A WRITTEN DESCRIPTION INDICATING WHERE EACH PARTICULAR MIX IS TO BE PLACED WITHIN THE STRUCTURE. FAILURE TO COMPLY MAY RESULT IN REJECTION OF THE MIX. IF ACCEPTED, PEA ROCK PUMP MIX USE IS LIMITED TO VERTICAL ELEMENT POURS AND BEAM POURS LESS THAN 60 LINEAL FEET PER POUR 4. MIX DESIGN SHALL MEET THE REQUIREMENTS OF ASTM C33 FOR COARSE AGGREGATE. 5. CALCIUM CHLORIDES SHALL NOT BE UTILIZED 6. OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE ENGINEER. 7. THE CONTRACTOR IS RESPONSIBLE FOR REVIEWING STRUCTURAL DRAWINGS AND SPECIFYING THE

D. CONCRETE SHALL COMPLY WITH THE REQUIREMENTS OF ASTM STANDARD C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME STAMPED WHEN CONCRETE IS BATCHED. THE MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE HALF (1-1/2) HOURS. IF FOR ANY REASON THERE IS A LONGER DELAY THAN THAT STATED ABOVE, THE CONCRETE SHALL BE DISCARDED. IT SHALL BE THE RESPONSIBILITY OF THE TESTING LAB TO NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NONCOMPLIANCE WITH THE ABOVE.

E. SLABS SHALL BE CURED USING A DISSIPATING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE 1-D AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE. SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED ILL CONCRETE EXPOSED TO THE WEATHER SHALL BE AIR—ENTRAINED. FOR SURFACE FINISHES AND OTHER REQUIREMENTS, REFER TO THE CONCRETE SPECIFICATIONS.

D. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR DRIPS, CHAMFERS, REGLETS, SLOTS, SLEEVES, ANCHORS, AND INSERTS. UNLESS SHOWN ON STRUCTURAL DRAWINGS NO OPENINGS LARGER THAN 12"x12" SHALL BE PLACED IN SLABS OR WALLS. FOR OPENINGS NOT SHOWN ON STRUCTURAL DRAWINGS, APPROVALS MUST BE OBTAINED FROM THE ENGINEER PRIOR TO FABRICATION OF STEEL AND PLACEMENT OF CONCRETE. LOCATION DRAWINGS FOR ALL SLEEVES AND BLOCKOUTS IN THE CONCRETE SHALL BE SUBMITTED FOR APPROVAL BY THE STRUCTURAL ENGINEER PRIOR TO PLACEMENT.

H. CONCRETE WALLS SHALL BE CAST MONOLITHIC WITH ADJOINING COLUMNS UNLESS SPECIFICALLY NOTED OTHERWISE. CONCRETE FOR SUCH WALLS SHALL BE THE SAME TYPE AND STRENGTH AS SPECIFIED COLUMNS.

CONFORM TO ACI 306R FOR COLD WEATHER CONCRETING AND ACI 305R FOR HOT WEATHER CONCRETING WHEN ANY COMBINATION OF HIGH TEMPERATURE. LOW RELATIVE HUMIDITY AND WIND VELOCITY TEND TO IMPAIR THE QUALITY OF THE CONCRETE. CONCRETE IS TO BE REJECTED IF ITS TEMPERATURE AT TIME OF PLACEMENT IS 90°F OR ABOVE. PROTECT SURFACES OF EXPOSED CONCRETE FROM PRECIPITATION DAMAGE UNTIL ADEQUATE STRENGTH IS GAINED TO PREVENT DAMAGE.

J. A ROUND BAR GRATE WITH A WORKING VIBRATOR MOUNTED ON THE GRATE IS RECOMMENDED WHEN ADDING MACRO FIBER TO A PUMPABLE MIX AND REDUCING HANG UPS OR DRAPED FIBERS ON THE GRATE.

AN INDEPENDENT TESTING LABORATORY SHALL PERFORM THE FOLLOWING TESTS ON CAST-IN-PLACE CONCRETE:

A. ASTM C143 "STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE." MAXIMUM SLUMP SHALL BE XX INCHES. B. ASTM C39 "STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS." A SEPARATE TEST SHALL BE CONDUCTED FOR EACH CLASS, FOR EVERY 50 CUBIC YARDS (OR FRACTION THEREOF), PLACED PER DAY. REQUIRED CYLINDER(S) QUANTITIES AND TEST AGE AS FOLLOWS:

> 1 AT 7 DAYS 2 AT 28 DAYS

D. N/U.

ONE ADDITIONAL RESERVE CYLINDER TO BE TESTED UNDER THE DIRECTION OF THE ENGINEER, IF REQUIRED. IF 28 DAY STRENGTH IS ACHIEVED, THE ADDITIONAL CYLINDER(S) MAY BE

3601 POST-INSTALLED ANCHORS: A. POST INSTALLED ANCHORS SHALL BE OF THE TYPE SPECIFIED ON THE DRAWINGS AND ONLY USED WHERE SPECIFIED ON THE DRAWINGS. B. CONTRACTOR SHALL OBTAIN APPROVAL FROM ENGINEER OF RECORD PRIOR TO USING

C. CARE SHALL BE GIVEN TO AVOID DAMAGING EXISTING REBAR WHEN DRILLING HOLES. HOLES SHALL BE DRILLED AND CLEANED PER MANUFACTURER'S INSTRUCTIONS.

POST-INSTALLED ANCHORS FOR CONDITIONS NOT SHOWN ON THE DRAWINGS.

E. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE LISTED SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL WITH CALCULATIONS PREPARED, SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE JURISDICTION OF THE PROJECT SHOWING THAT THE SUBSTITUTED PRODUCT WILL ACHIEVE AN EQUIVALENT CAPACITY USING THE APPROPRIATE DESIGN PROCEDURE.

4203 MASONRY WALLS: A. MASONRY UNITS SHALL MEET ASTM C90 FOR HOLLOW LOAD BEARING TYPE MASONRY WITH UNIT STRENGTH OF 1900 PSI ON THE NET AREA (f'm = 1500 PSI). MORTAR SHALL BE TYPE "M" WHERE IN CONTACT WITH EARTH AND TYPE "M" OR "S" ELSEWHERE AND MEET ASTM C270. GROUT SHALL BE 2000 PSI MINIMUM COMPRESSIVE STRENGTH AND MEET ASTM C476. GROUT SLUMP SHALL BE BETWEEN 9" AND 11". WHERE MINIMUM DIMENSION OF ANY VERTICAL, CONTINUOUS CELL IS 3 INCHES OR LESS, USE FINE GROUT, OTHERWISE USE COURSE (PEA GRAVEL) GROUT.

B. BLOCK CELLS SHALL BE GROUT FILLED WITH VERTICAL REINFORCING BARS, SAME SIZE BARS AS REINFORCING, AT CORNERS, INTERSECTIONS, EACH SIDE OF OPENINGS OVER 4 FEET WIDE, AND AS SHOWN ON THE PLANS. DOWELS SHALL BE USED TO PROVIDE CONTINUITY INTO THE STRUCTURE ABOVE AND/OR BELOW. UNLESS NOTED OTHERWISE. AT VERTICAL BAR TERMINATION. HOOK INTO FOOTING AND BOND BEAM. TIE BEAM, OR SLAB AT TOP OF WALL. USE METAL LATH, MORTAR, OR SPECIAL UNITS TO CONFINE CONCRETE AND GROUT TO AREA REQUIRED. MASONRY SHALL BE LAID IN RUNNING BOND PATTERN UNLESS NOTED OTHERWISE. ALL CELLS BELOW GRADE ARE TO BE GROUTED SOLID. SEE CMU SCHEDULE FOR VERTICAL REINFORCING, SIZE AND SPACING.

C. PROVIDE 9 GAUGE GALVANIZED HORIZONTAL JOINT REINFORCING (DUR-O-WALL OR ENGINEER APPROVED SUBSTITUTION) AT ALTERNATE BLOCK COURSES 16" O.C. VERTICALLY.

D. CONTROL JOINTS SHALL BE PROVIDED IN CONCRETE MASONRY CONSTRUCTION AT LOCATIONS INDICATED ON DRAWINGS. HORIZONTAL WALL REINFORCING SHALL BE STOPPED EACH SIDE OF CONTROL JOINTS. SEE ARCHITECTURAL DRAWINGS FOR SEALANT REQUIREMENTS AT CONTROL JOINTS.

11 INCHES. USE OF SUPERPLASTICIZER IS PROHIBITED. F. CELLS TO BE GROUT FILLED SHALL HAVE VERTICAL ALIGNMENT SUFFICIENT TO MAINTAIN A CLEAR, UNOBSTRUCTED, CONTINUOUS VERTICAL GROUT SPACE, CLEANOUT OPENINGS SHALL BE PROVIDED AT THE BOTTOM OF CELLS TO BE GROUT FILLED IN EACH POUR IN EXCESS OF 5 FEET IN HEIGHT. ANY OVERHANGING MORTAR OR OTHER OBSTRUCTION OR DEBRIS SHALL BE REMOVED FROM THE INSIDES OF

IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. GROUT SLUMP SHALL BE BETWEEN 8 AND

SUBMIT PROPOSED GROUT MIX DESIGN FOR REVIEW PRIOR TO USE. MIX SHALL BE UNIQUELY

G. VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 192 BAR DIAMETERS. CELLS CONTAINING REINFORCEMENT SHALL BE FILLED SOLIDLY WITH GROUT. GROUT SHALL BE CONSOLIDATED AT TIME OF PLACING BY VIBRATING AND RECONSOLIDATED LATER BY VIBRATING BEFORE PLASTICITY IS LOST.

SUCH CELL WALLS. THE CLEANOUTS SHALL BE SEALED BEFORE GROUTING, AFTER INSPECTION.

H. PLACE GROUT IN LIFTS NOT EXCEEDING 5 FT.. MINIMUM CELL DIMENSION SHALL BE IN ACCORDANCE WITH TABLE 5 OF ACI 530.1 (3"  $\times$  3" FOR COARSE GROUT). MAXIMUM POUR HEIGHT SHALL BE LIMITED TO

I. WHEN THE GROUTING IS STOPPED FOR ONE HOUR OR LONGER, HORIZONTAL CONSTRUCTION JOINTS SHALL BE MADE BY STOPPING THE POUR OF GROUT NOT LESS THAN 1-1/2 INCH BELOW THE TOP OF THE UPPERMOST UNIT GROUTED.

J. PROVIDE BRACING FOR TOP OF ALL INTERIOR CMU WALLS. WHERE CONNECTION DETAILS ARE NOT SHOWN, PROVIDE A 1" GAP BETWEEN WALL AND BOTTOM OF STRUCTURE, BRACE TOP OF WALL WITH 14 GA BENT PLATE 4x4x0'-6" LONG AT 4'-0" OC. PROVIDE (3) 1/4" HILTI KWIK CON II SCREWS TO WALL AND (4) #10 SCREWS TO BOTTOM OF STRUCTURE (OR EQUIVALENT CAPACITY CONNECTION). CONNECTION TO ALLOW FOR 1" DEFLECTION OF STRUCTURE THROUGH USE OF SLOTTED HOLES.

K. CONCRETE MASONRY QUALITY CONTROL:

1. WORK IN PROGRESS SHALL BE INSPECTED FOR CONFORMANCE WITH SPECIFIED MATERIALS AND THAT WORKMANSHIP AND CONSTRUCTION IS IN COMPLIANCE WITH PLANS, SPECIFICATIONS, AND INDUSTRY

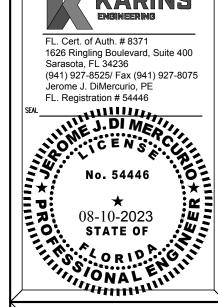
2. STRENGTH DETERMINATION SHALL BE MADE USING UNIT STRENGTH OR PRISM TESTS IN ACCORDANCE WITH ACI 530.

		DRAWING INDEX
	DWG NO.	DESCRIPTION
1	S0.1	GENERAL STRUCTURAL NOTES
2	S0.2	WIND PRESSURES, ABBREVIATIONS & BAR LAP SCHEDULES
3	S1.1	TYPICAL FOUNDATION DETAILS
4	S1.2	TYPICAL MASONRY DETAILS
5	S1.3	TYPICAL FRAMING DETAILS
6	S2.0	SCHEDULES AND DETAILS
7	S3.0	1ST FLOOR / FOUNDATION PLAN
8	S3.1	2ND FLOOR FRAMING PLAN
9	S3.2	3RD FLOOR FRAMING PLAN
	S3.3	ROOF FRAMING PLAN

TO THE BEST OF MY KNOWLEDGE AND ABILITY. THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH APPLICABLE MINIMUM BUILDING CODES

 $\overline{\mathbf{C}}$ Ш S Ш Z Z

RHR JDM 21DS-0007 AS NOTED 2022-03-21



BAR DEVELOPMENT SCHEDULE								
	DEVELOPMENT LENGTH SHALL BE PER FOLLOWING TABLE MODIFIED PER NOTES BELOW							
GRADE 60 STEEL								
	NORMAL WEIGHT CONCRETE STRENGTH							
BAR	3000 PSI	4000 PSI	5000 PSI	6000 PSI	7000 PSI	8000 PSI	9000 PSI	10000 PSI AND HIGHER
#3	1'-5"	1'-3"	1'-1"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"
#4	1'-10"	1'-7"	1'-5"	1'-4"	1'-3"	1'-2"	1'-1"	1'-1"
<b>#</b> 5	2'-3"	2'-0"	1'-10"	1'-8"	1'-6"	1'-5"	1'-4"	1'-4"
#6	2'-9"	2'-5"	2'-2"	2'-0"	1'-10"	1'-10"	1'-8"	1'-8"
#7	4'-0"	3'-6"	3'-2"	2'-10'	2'-8"	2'-6"	2'-4"	2'-3"
#8	4'-7"	4'-0"	3'-7"	3'-3"	3'-0"	2'-10"	2'-8"	2'-7"
#9	5'-2"	4'-6"	4'-0"	3'-8"	3'-5"	3'-2"	3'-1"	3'-0"
#10	5'-10"	5'-1"	4'-6"	4'-2"	3'-10"	3'-7"	3'-5"	3'-4"
#11	6'-6"	5'-7"	5'-0"	4'-7"	4'-3"	4'-0"	3'-10"	3'-8"

## <u>NOTES:</u>

- 1. FOR CLEAR SPACING BETWEEN BARS <db AND/OR CLEAR COVER <db, MULTIPLY BY 1.5.
  2. FOR TOP BARS MULTIPLY BY 1.3.
- 3. FOR EPOXY COATED BARS, IF SPECIFIED OR APPROVED AS AN ALTERMNATE, MULTIPLY BY 1.3.
- 4. FOR MMFX BARS, IF SPECIFIED OR USED, USE GRADE 75 KSI VALUES. 5. WHERE MORE THAN ONE FACTOR APPLIES, PRODUCT OF ALL APPLICABLE FACTORS SHALL BE APPLIED.
- 6. IF DETAILER IS TO USE A DIFFERENT SCHEDULE, HE/SHE MUST SUBMIT A SEALED LETTER INDICATING THAT HIS/HER VALUES CORRESPOND TO CURRENT ACI 318 CODE.

		B	AR LAF	SPLIC	E SCH	EDULE		
	LAP SPLICE	LENGTH SH	HALL BE PE	R FOLLOWIN	G TABLE MC	DIFIED PER	NOTES BEL	.OW
				GRADE 60	STEEL			
			NORMAL	WEIGHT CO	NCRETE STR	ENGTH		
BAR	3000 PSI	4000 PSI	5000 PSI	6000 PSI	7000 PSI	8000 PSI	9000 PSI	10000 PSI AND HIGHER
#3	1'-9"	1'-6"	1'-5"	1'-3"	1'-2"	1'-1"	1'-0"	1'-0"
#4	2'-4"	2'-1"	1'-10"	1'-8"	1'-7"	1'-5"	1'-4"	1'-4"
<b>#</b> 5	3'-0"	2'-7"	2'-4"	2'-1"	2'-0"	1'-10"	1'-9"	1'-8"
#6	3'-7"	3'-1"	2'-9"	2'-6"	2'-4"	2'-2"	2'-1"	2'-0"
#7	5'-2"	4'-6"	4'-0"	3'-8"	3'-5"	3'-2"	3'-0"	2'-10"
#8	6'-0"	5'-2"	4'-7"	4'-2"	4'-0"	3'-8"	3'-5"	3'-3"
#9	6'-8"	5'-10"	5'-2"	4'-9"	4'-5"	4'-1"	3'-10"	3'-8"
#10	7'-6"	6'-6"	5'-10"	5'-4"	5'-0"	4'-7"	4'-4"	4'-2"
#11	8'-4"	7'-3"	6'-6"	6'-0"	5'-6"	5'-1"	4'-10"	4'-7"

## NOTES:

- 1. FOR CLEAR SPACING BETWEEN BARS <db AND/OR CLEAR COVER <db, MULTIPLY BY 1.5.
  2. FOR TOP BARS MULTIPLY BY 1.3.
- FOR TOP BARS MOLITELT BT 1.3.
   FOR EPOXY COATED BARS, IF SPECIFIED OR APPROVED AS AN ALTERMNATE, MULTIPLY BY 1.3.
   FOR MMFX BARS, IF SPECIFIED OR USED, USE GRADE 75 KSI VALUES.
   WHERE MORE THAN ONE FACTOR APPLIES, PRODUCT OF ALL APPLICABLE FACTORS SHALL BE APPLIED.
- 5. WHERE MORE THAN ONE FACTOR APPLIES, PRODUCT OF ALL APPLICABLE FACTORS SHALL BE APPLIED.
  6. IF DETAILER IS TO USE A DIFFERENT SCHEDULE, HE/SHE MUST SUBMIT A SEALED LETTER INDICATING THAT HIS/HER VALUES CORRESPOND TO CURRENT ACI 318 CODE.

	COMPONENT & CL					WIN	D PRE	SSURE	ES
III TIMA T	E WIND SPEED Vult =		PER ASC	/		100 mpl	HEIGHT, <b>h</b> =		38 ft
	ULTIMATE WIND SPEED, Vult =  EXPOSURE CATEGORY:		NOMINAL WIND SPEED, Vasd = RISK CATEGORY:			II PARAPET HEIGHT=		EIGHT=	4 ft
ENCLOSED BUILDING, GCpi=		H-/- 0.18	8 DIRECTIONALITY FACTOR, Kd			1.00	ROOF SLOPE =		0.25/12
	COMPONENT LOCATION		POSITIVE PRESSURES (psf)			NEGATIVE PRESSURES (1			
	EFFECTIVE AREA, Ae	10 ft²	20 ft²	50 ft²	100 ft²	10 ft²	20 ft²	50 ft²	100 ft²
	ZONE 1: FIRST INTERIOR FROM EDGES	18.1	16.9	16.0	16.0	-70.7	-66.0	-59.9	-55.2
	ZONE 1': SECOND INTERIOR FROM EDGES	18.1	16.9	16.0	16.0	-40.6	-40.6	-40.6	-40.6
F.S.	ZONE 2: WITHIN "0.6h" DISTANCE FROM EDGES	40.6	38.8	36.4	34.6	-93.3	-87.3	-79.3	-73.3
ROOFS	ZONE 3: WITHIN " <b>0.2h</b> " DISTANCE FROM EDGES AT CORNERS	40.6	38.8	36.4	34.6	-93.3	-87.3	-79.3	-73.3
	ZONE 2 AT OVERHANGS	N/A	N/A	N/A	N/A	-86.5	-78.5	-67.9	-59.9
	ZONE 3 AT OVERHANGS	N/A	N/A	N/A	N/A	-86.5	-78.5	-67.9	-59.9
WALLS	ZONE 4: INTERIOR	40.6	38.8	36.4	34.6	-44.0	-42.2	-39.8	-38.0
WA	ZONE 5: CORNER	40.6	38.8	36.4	34.6	-54.2	-50.6	-45.8	-42.2
	CASE A   CASE B		CAS	SE A			CAS	SEB	
PETS	ZONE 2: EDGE ZONE 4: INTERIO	123.8	115.8	105.2	97.2	-73.1	-69.4	-64.5	-60.8
PARAPETS	ZONE 3: CORNER   ZONE 5: CORNE	123.8	115.8	105.2	97.2	-83.6	-78.0	-70.7	-65.1

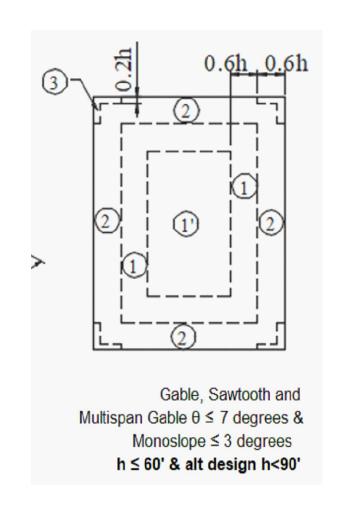
NOTES:

1) "Ae" INDICATES EFFECTIVE AREA AS DEFINED BY SECTION 26.2 OF ASCE 7.

2) PRESSURE VALUES IN ABOVE TABLE ARE BASED ON THE PARAMETERS LISTED AT THE TOP OF THE TABLE.
3) GLAZED OPENINGS SHALL BE PROTECTED IN ACCORDANCE WITH SECTION 26.10.3 OF ASCE 7.

4) PRESSURES IN ABOVE TABLE ARE ULTIMATE VALUES. FOR ALLOWABLE STRESS DESIGN, OR FOR TESTING BASED ON NOMINAL PRESSURES,

TABLE VALUES ARE PERMITTED TO BE MULTIPLIED BY **0.60.**5) WHERE WIND LOADS ARE COMBINED WITH OTHER LOADS PER ASCE-7 LOAD COMBINATIONS, PRESSURES IN ABOVE TABLE ARE PERMITTED TO BE MULTIPLIED BY **0.85**.



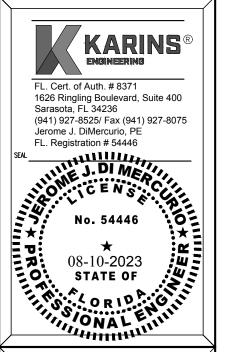
	ABBREV	IOITAI	NS
AB	- ANCHOR BOLT	K	- KIP(s)
ALT APPROX	<ul><li>ALTERNATE</li><li>APPROXIMATELY</li></ul>	KIP(s) KLF KJ	<ul><li>1000 POUNDS</li><li>KIPS PER LINEAR FOOT</li></ul>
ARCH	- APPROXIMATELY - ARCHITECT	KLF K.I	- KIPS PER LINEAR FOOT - CONSTRUCTION JOINT
ARCH'L	- ARCHITECTURAL	L	- ANGLE
B/	- BOTTOM OF	LG	- LONG
BC	- BOTTOM CHORD	LLH	
BLDG	- BUILDING	LLV	- LONG LEG VERTICAL
BM BOTT	<ul><li>BEAM</li><li>BOTTOM</li></ul>	LP LW	<ul><li>LOW POINT</li><li>LONG WAY</li></ul>
BRG	- BEARING	MFR	
c/c	<ul> <li>CENTER TO CENTER</li> </ul>	MAS	- MASONRY
CIP	- CAST IN PLACE	MO	
CJ CL	<ul><li>CONTRACTION JOINT</li><li>CENTERLINE</li></ul>	MAT'L MAX	<ul><li>MATERIAL</li><li>MAXIMUM</li></ul>
CLR	- CLEAR	MECH'L	- MECHANICAL
CMU	- CONCRETE MASONRY UNIT		- METAL
COL	- COLUMN	MIN	
CONC	- CONCRETE	MISC	
CONFIG CONT	<ul><li>CONFIGURATION</li><li>CONTINUOUS</li></ul>	NS NIC	
CONTR	- CONTRACTOR	NTS	
CTR	<ul><li>CENTER</li></ul>	OC	- ON CENTER
DBL	- DOUBLE	ОН	- OPPOSITE HAND
DET DIA	<ul><li>DETAIL</li><li>DIAMETER</li></ul>	OPNG PAF	<ul><li>OPENING</li><li>POWDER ACTUATED FASTENERS</li></ul>
DIM	- DIMENSION	PART	<ul><li>POWDER ACTUATED FASTENERS</li><li>PARTITION</li></ul>
DN	- DOWN	PART'L	- PARTIAL
DR	- DOOR/DRAIN	PCJ	- PRECAST CONCRETE JOIST
DWG EA	<ul><li>DRAWING</li><li>EACH</li></ul>	PL PLF	<ul><li>PLATE</li><li>POUNDS PER LINEAR FOOT</li></ul>
EE	- EACH END	PSF	- POUNDS PER SQUARE FOOT
EF	- EACH FACE	PSI	- POUNDS PER SQUARE INCH
EJ	- EXPANSION JOINT	PT	- POST TENSIONED/PRESSURE TREATED
LL FLEV	- ELEVATION - FLEVATION / FLEVATOR	R DEC	- RISER/RADIUS
ENGR	- ENGINEER	REINF	- REINFORCING
EOR	- ENGINEER OF RECORD	REM	- REMAINDER
EOS	- EDGE OF SLAB	REQ'D	- REQUIRED
EQ FW	- EQUAL - FACH WAY	REV DM	- REVISED/REVISION
EXIST	- EXISTING	RO	- ROUGH OPENING
EXP	- EXPANSION	RQMTS	- REQUIREMENTS
EXT	- EXTERIOR	SCHED	- SCHEDULE
FIN	- FINISH	SECT	- SECTION
FND	<ul><li>FOUNDATION</li></ul>	SI	- SLOPE
FOM	- FACE OF MASONRY	SOG	- SLAB-ON-GRADE
FS	- FAR SIDE	SP	- SPIRAL
FT	- FOOT	SQ	- SQUARE
GA	- GAGE	SS STD	- STANDARD
GALV	- GALVANIZED	STL	- STEEL
GC	<ul> <li>GENERAL CONTRACTOR</li> </ul>	STRUCT'L	- STRUCTURAL
GT	- GIRDER TRUSS	SW	- SHEARWALL/SHORT WAY
HCP	- HOLLOW CORE PLANK	I/ TR	- IUP UF - TIE REAM
HDG	- HOT DIPPED GALVANIZED	TC	- TIE COLUMN/TOP CHORD
HG	- HIP GIRDER	TEMP	- TEMPERATURE
HK	- HOOK	TJ	POUNDS PER SQUARE FOOT POUNDS PER SQUARE INCH POST TENSIONED/PRESSURE TREATED RISER/RADIUS REGULAR REINFORCING REMAINDER REQUIRED REVISED/REVISION ROOM ROUGH OPENING REQUIREMENTS SCHEDULE SECTION SIMILAR SLOPE SLAB—ON—GRADE SPIRAL SQUARE STAINLESS STEEL STANDARD STEEL STRUCTURAL SHEARWALL/SHORT WAY TOP OF TIE BEAM TIE COLUMN/TOP CHORD TEMPERATURE TIE JOIST THRU OUT TREAD/TRUSS TYPICAL UNLESS NOTED OTHERWISE VERTICAL WITH WITHOUT WOOD WORK POINT
HOKIZ   HP	— HUKIZUNTAL — HIGH POINT	I/U TR	- IHKU UUI - TRFAD/TRUSS
HS	- HIGH STRENGTH	TYP	- TYPICAL
IJ	- ISOLATION JOINT	UNO	- UNLESS NOTED OTHERWISE
INFO	- INFORMATION	VERT	- VERTICAL
INT	- INTERIOR	W/	- WITH
JR	— INNEGULAR — JAMB RFINFORCING	w/ U WD	- WOOD
JT	- JOINT	WP	- WORK POINT
		WWF	- WELDED WIRE FABRIC
1			

WIND PRESSURE ZONE DIAGRAM AND WIND LOAD TABLE

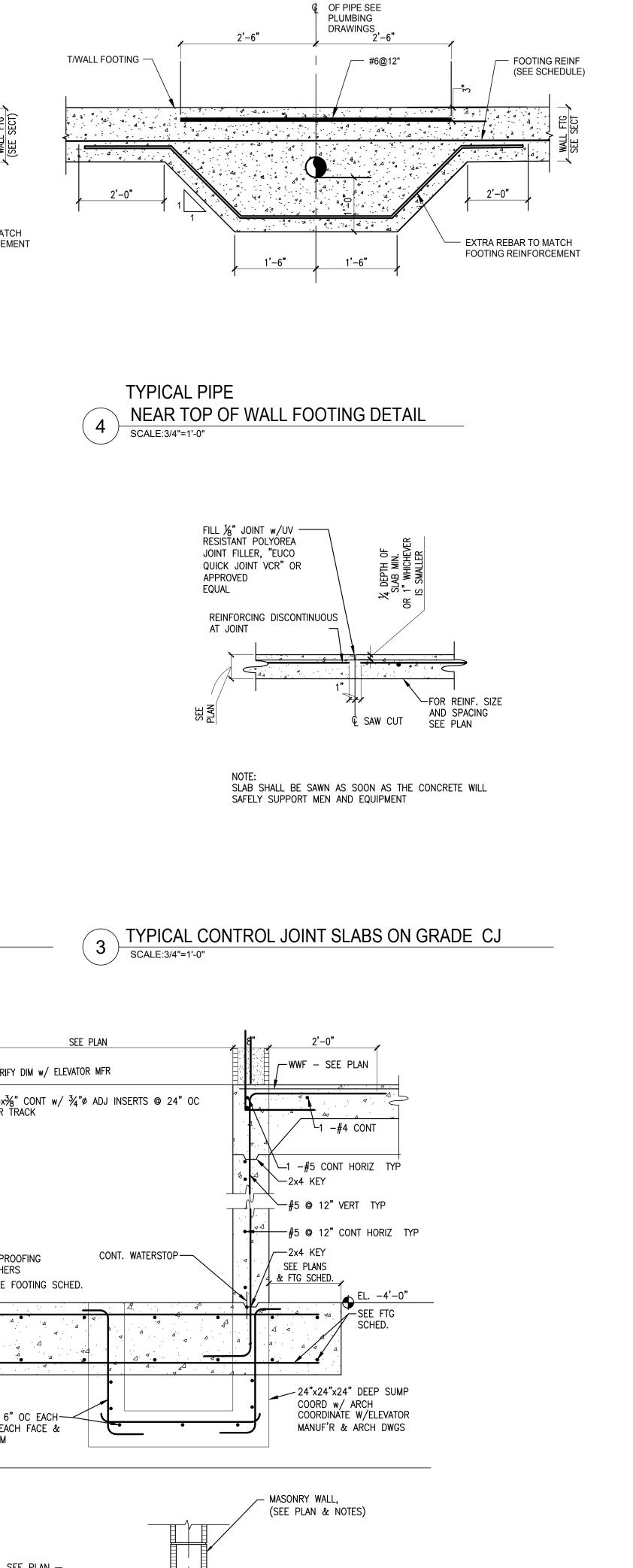
1 NTS

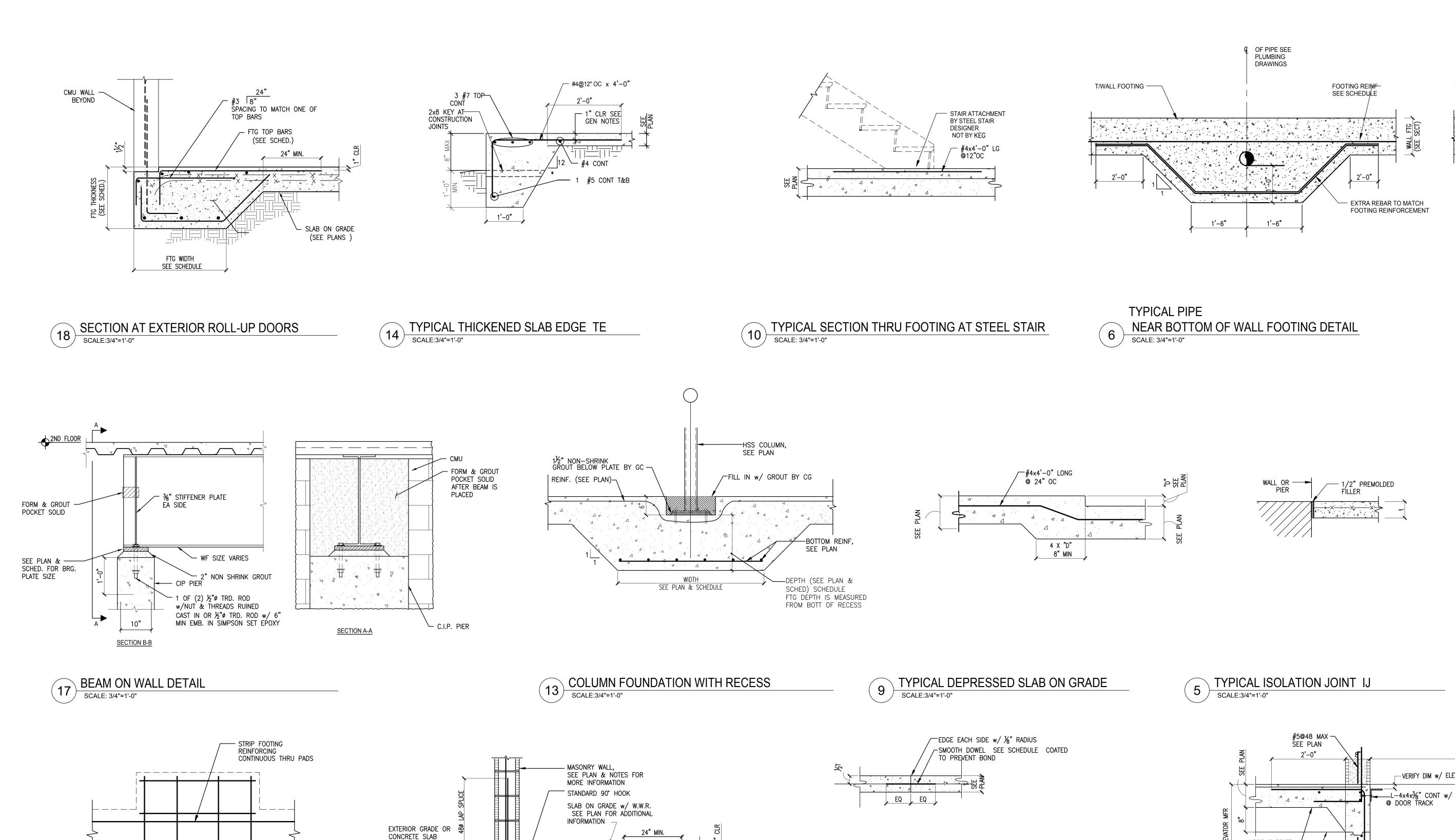
WIND PRESSURES, ABBREVIATIONS AND BAR L SCHEDULES

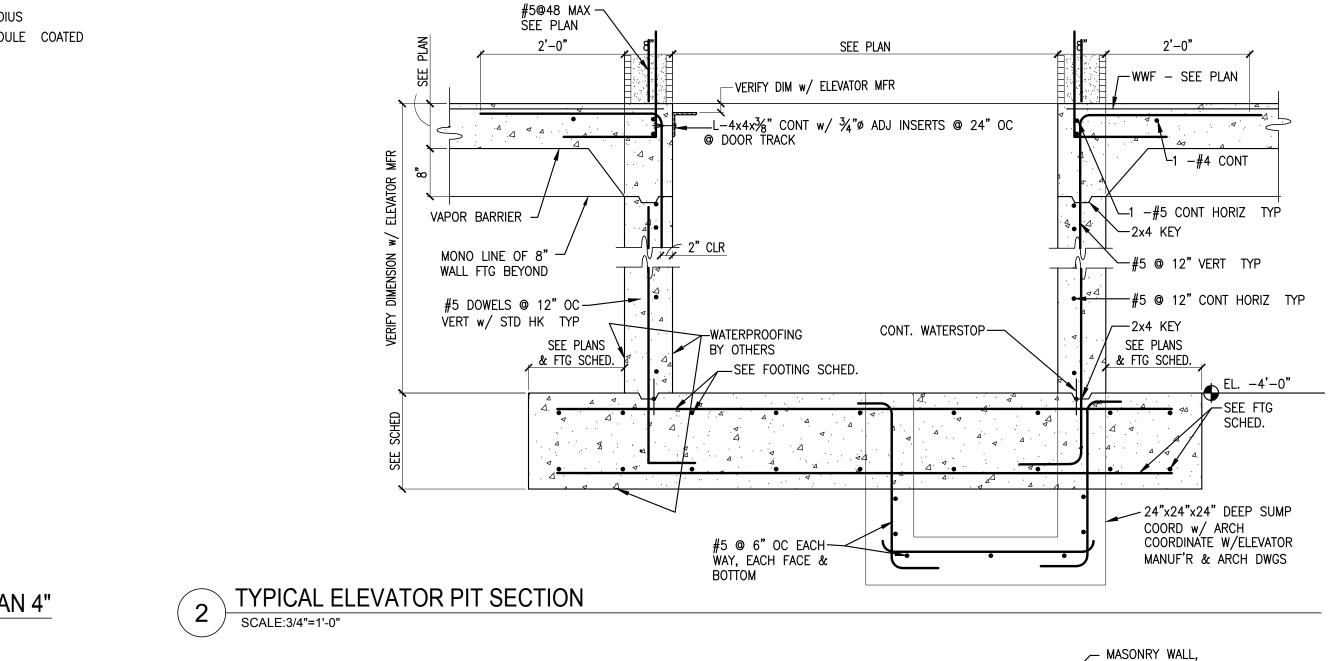
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Scale: AS NOTED
Date: 2022-03-21

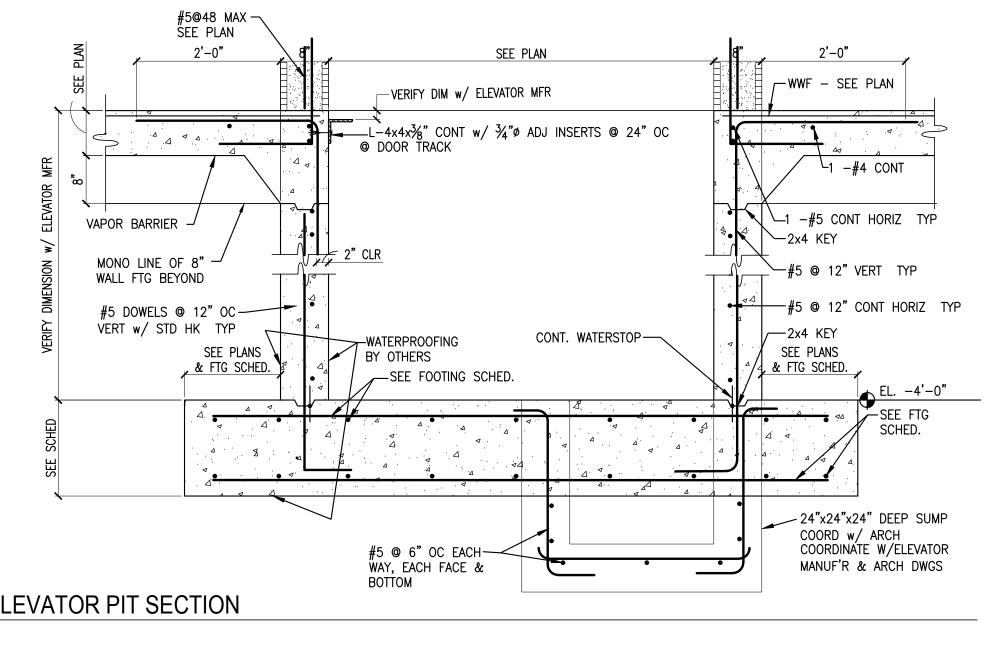


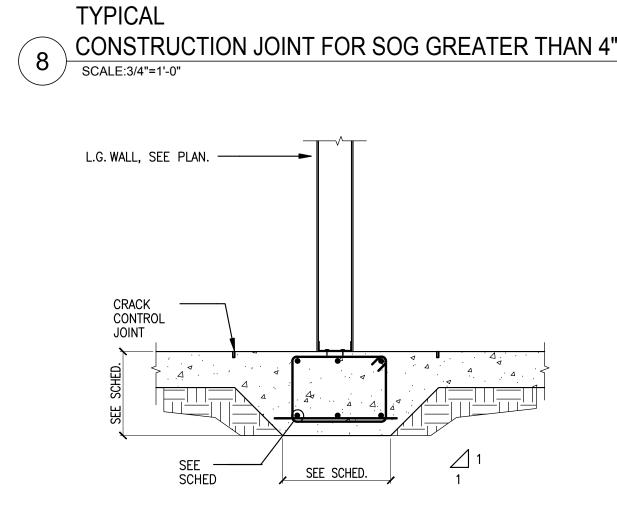
TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH APPLICABLE MINIMUM BUILDING CODES











SMOOTH DOWEL

SIZE AND SPACING

%"øx1'−0" @ 12" OC

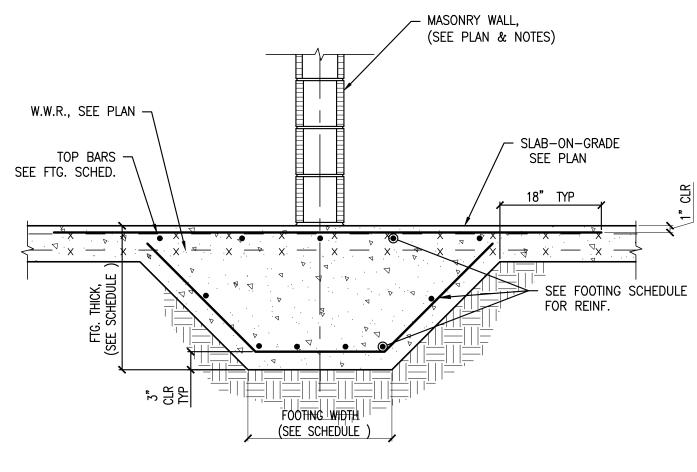
 $\frac{3}{4}$ "øx1'-2" @ 12" OC

½"øx1'−2" @ 12" OC

1"øx1'-2" @ 12" OC

1½"ø×1'−4" @ 12" OC

<sup>1</sup>¼"ø×1'−4" @ 12" OC



TYP. MONOLITHIC INTERIOR FOOTING AT CMU WALL SCALE: 3/4"=1'-0"

TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH APPLICABLE MINIMUM BUILDING CODES

MEASURED //<//// FROM BOTT. OF RECESS FTG WIDTH
(SEE PLAN & SCHEDULE) (SEE SCHEDULE) TYP. MONOLITHIC FTG AT RECESSED COLUMN

<u>PLAN AT PAD</u>

\_\_HSS COLUMN, (SEE PLAN )

COLUMN INSTALLATION

- RECESS TO EXTEND 2" BEYOND BASE

PLATE ALL AROUND. GROUT SOLID AFTER

SCHED SCHEDULE FTG. DEPTH IS

16 TYPICAL FOOTING INTERSECTION DETAIL

SCALE:3/4"=1'-0"

SEE BASE PLATE AND -

1½" NON-SHRINK GROUT BELOW PLATE BY GC

W.W.R. —\_\_\_\_

(SEE PLAN )

ANCHOR DETAILS

TYPICAL MONOLITHIC FOOTING STEPPED BOTTOM

SCALE:3/4"=1'-0"

4'-0" MIN.

FTG WIDTH

(SEE SCHEDULE )

4'-0" TYP.

4'-0" MIN.

FTG. SECTION AT EXTERIOR WALL

SCALE:3/4"=1'-0"

WHERE APPLICABLE

SCHEDULE FOR SIZE AND

7 TYPICAL MONOLITHIC FOOTING AT LG WALL SCALE:3/4"=1'-0"

RHR

JDM

21DS-0007

AS NOTED

2022-03-21

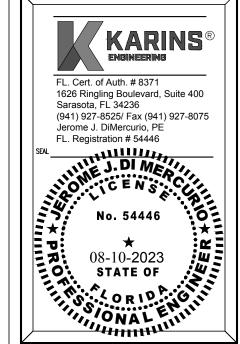
1626 Ringling Boulevard, Suite 400 Sarasota, FL 34236

(941) 927-8525/ Fax (941) 927-8075 Jerome J. DiMercurio, PE

SELF STORA
FLORIDA

V MINNEOL, MINNEOLA,

NDE

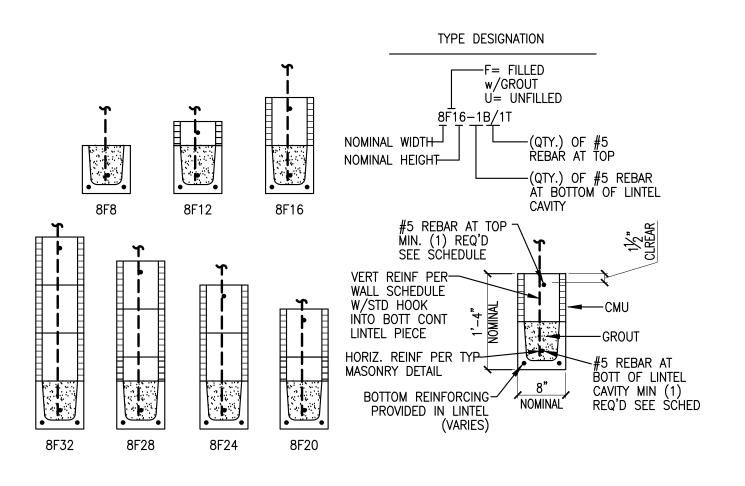


TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH APPLICABLE MINIMUM BUILDING CODES

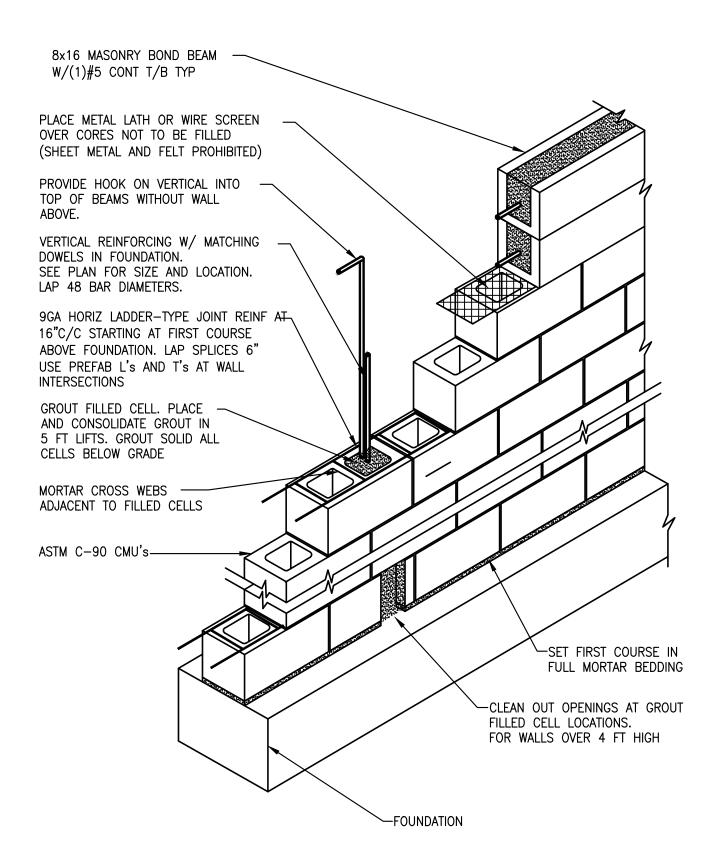
MARK	REINFORCING	SIZE	REMARK
MWA	#6 @ 32" O.C.	8"	1
MWB	#5 @ 32" O.C.	8"	1
MWC	#6 @ 32" O.C.	12"	1

	LINTEL SCHE	DULE
LINTEL SPAN	LINTEL DESIGNATION	REMARKS
UP TO 4'-0"	8 F8-1B/1T	
FROM 4'-1" TO 6'-0"	8 F16-1B/1T	
FROM 6'-1" TO 8'-0"	8 F20-1B/1T	
FROM 8'-1" TO 10'-0"	8 F24-1B/1T	
FROM 10'-1" TO 13'-0"	8 F28-1B/1T	
OVER 13'-0"	8 F32-1B/1T	PRE-STRESSED LINTEL REQD

NOTES:
1. LINTELS SHALL BE "CAST—CRETE" OR EQUIVALENT PRECAST CONCRETE UNO.
2. LINTELS SHALL BEAR 8" MIN ON CMU/CONC AND 3" MIN ON STEEL ANGLES.

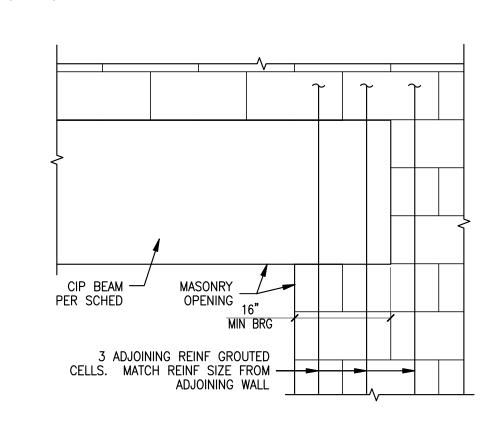


3 LINTEL DETAILS AND SCHEDULE
SCALE:3/4"=1'-0"

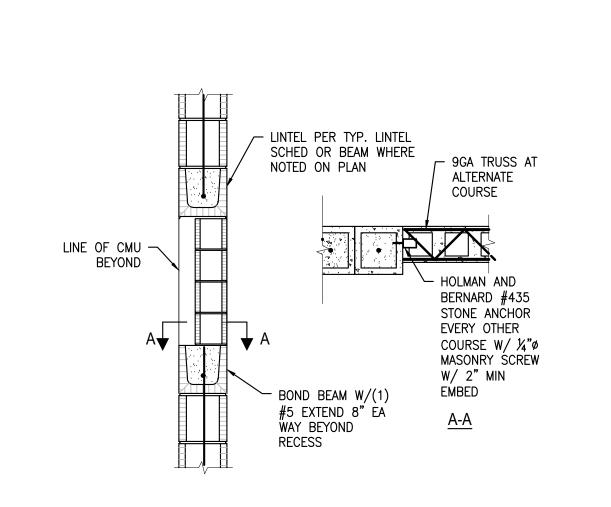


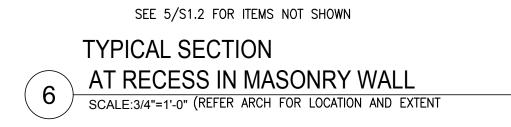
TYPICAL MASONRY WALL DETAIL FOR BOND BEAM

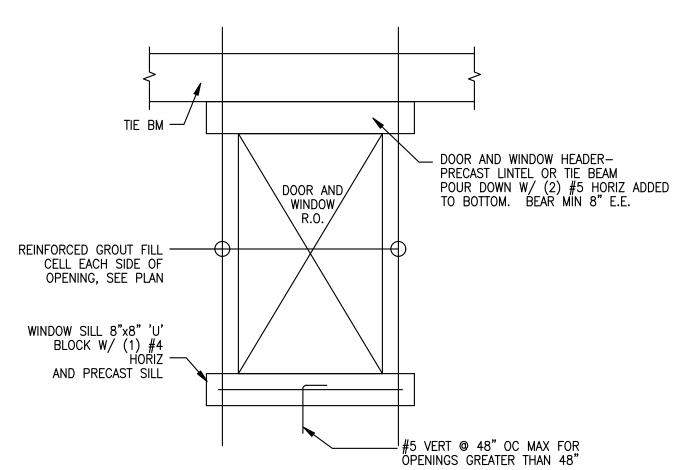
SCALE:3/4"=1'-0"



1 TYPICAL CIP BEAM BEARING ON CMU
SCALE:3/4"=1'-0"

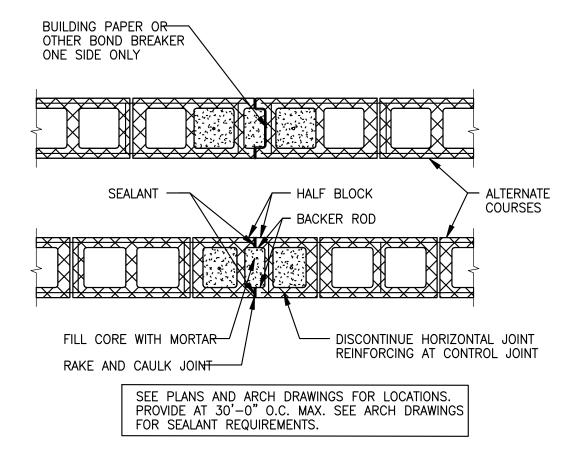






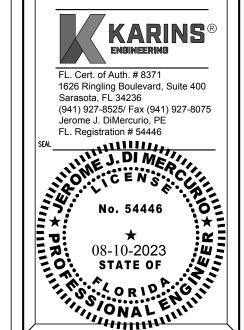
MASONRY OPENING FRAMING DETAIL

SCALE:3/4"=1'-0"



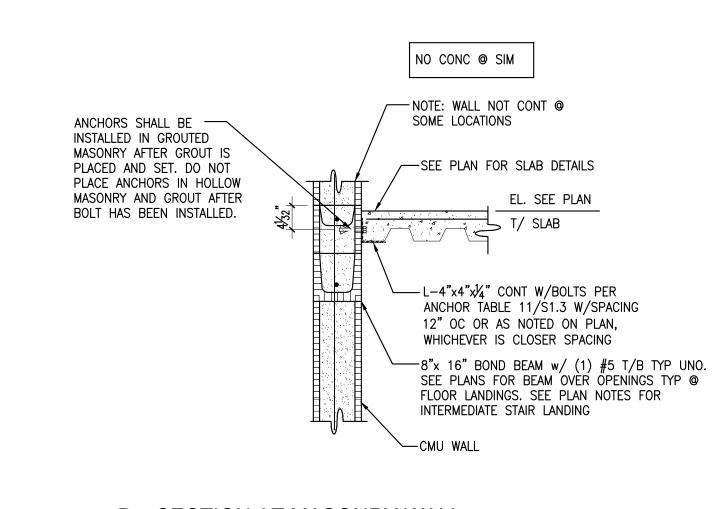
4 TYPICAL MASONRY CONTROL JOINT
SCALE:3/4"=1'-0"

CONDE

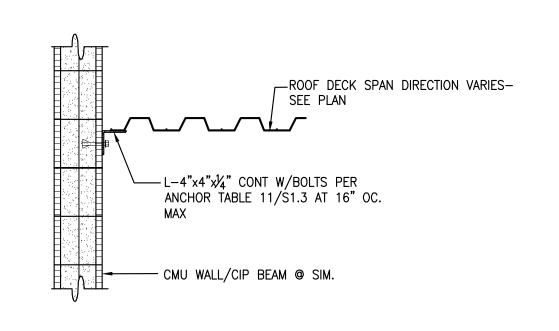


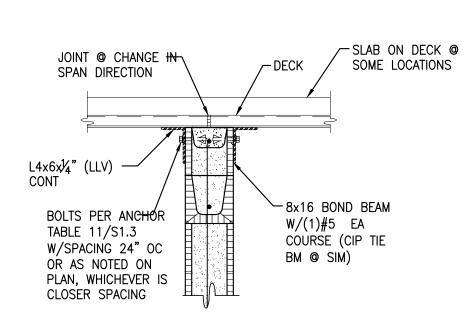
SECTION

SCALE:3/4"=1'-0" NOTE: SECTIONS 1,5 NOT USED. TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH APPLICABLE MINIMUM BUILDING CODES



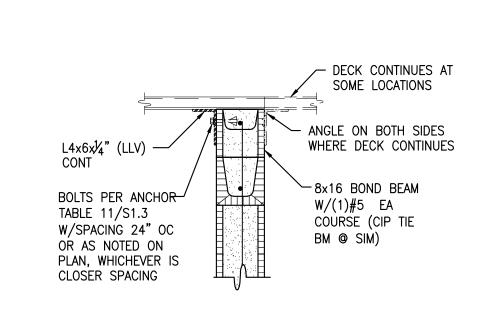


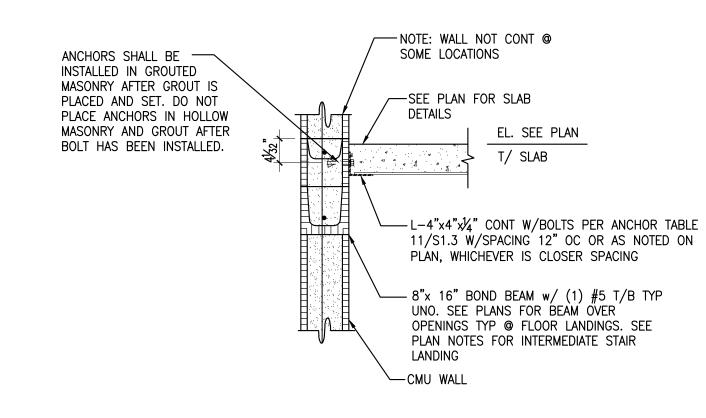






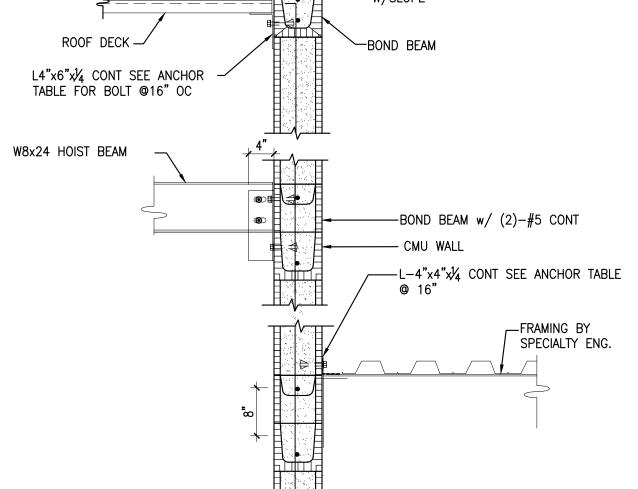


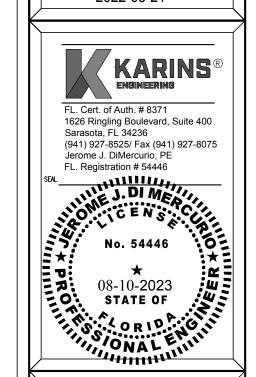










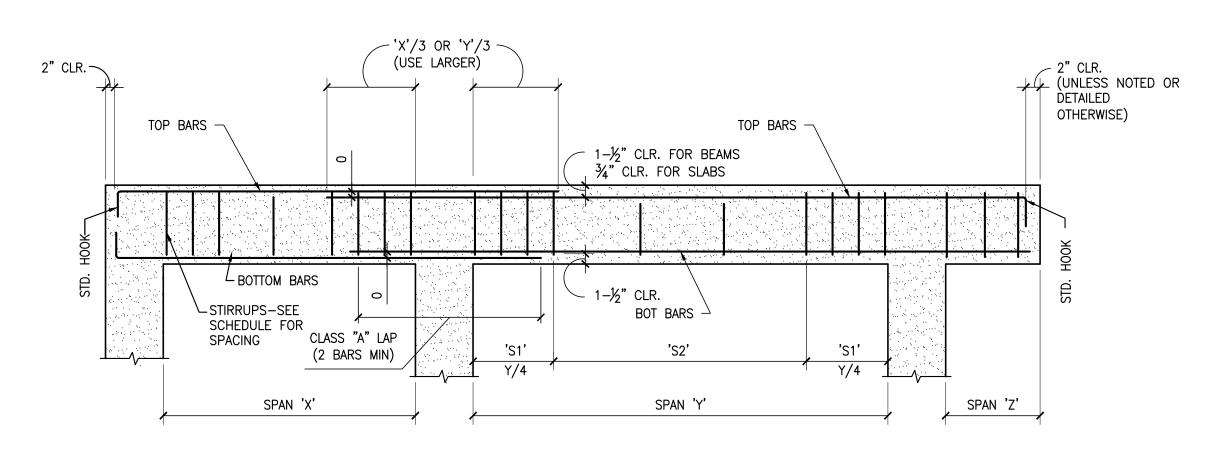


	CONCRETE BEAM SCHEDULE							
MADIZ	SIZE WxD:			REINFORCEMENT	NOTEO			
MARK (IN)	(IN)	(IN) TYPE	TOP	ВОТТОМ	STIRRUPS S1/S2	NOTES		
CB-1	8 x 16	CIP	2#5	2#5	#3@8"	NOTE 1		
CB-2	8 x 16	CIP	2#5	2 LAYER 2#6 EA	#3@6"	NOTE 1		
MB-1	8 x 24	MASONRY	1#5	1#5	#3@6"	-		
MB-2	8 x 32	MASONRY	1#5	1#5	#3@6"	-		
MB-3	8 x 48	MASONRY	1#5	1#5	#3@6"	NOTE 1 & 2		
TB-1	8 x 16	MASONRY	1#5	1#5	_	TIE BEAM		
CTB2	8 x 16	CIP	1#5	1#5	#3@12"	-		

NOTES:

1. MASONRY BEAMS ARE TO BE CONSTRUCTED USING PRECAST LINTEL. REBAR SPECIFIED SHALL BE CONTINUOUS AND NOT LAPPED WITHIN SPAN.

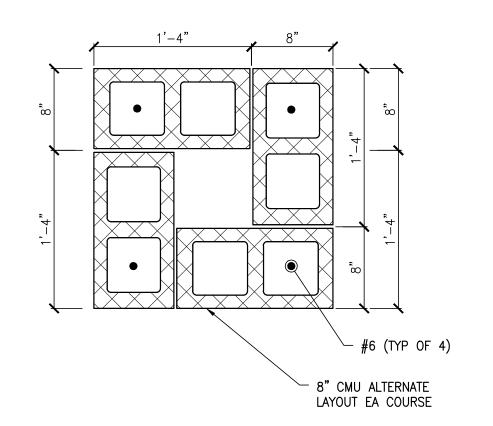
2. PROVIDE 1 #5 MID DEPTH IN ADDITION TO TOP AND BOTTOM BARS.

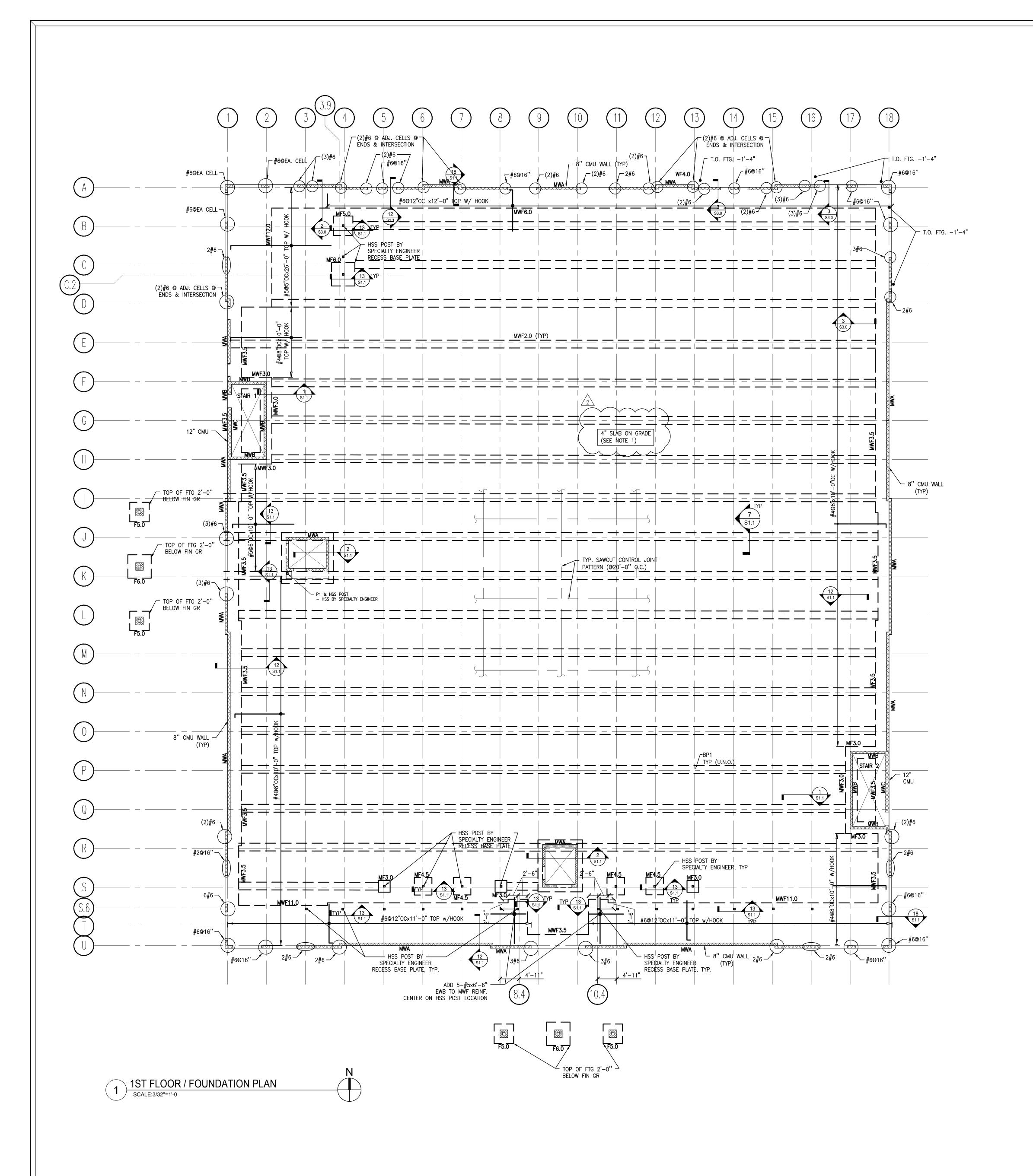


3 TYPICAL REINF. DIAGRAM FOR BEAMS & ONE WAY SLABS

	FOOTI	NG SCHEDULE	
MARK	SIZE (WxDxL)	REINFORCING	REMARKS
MF4.0	4'-0"x4'-0"x1'-6"	6-#5 EWB	
MF4.5	4'-6"x4'-6"x1'-6"	7-#5 EWB	
MF5.0	5'-0"x5'-0"x1'-6"	7-#5 EWB	
MF6.0	6'-0"x6'-0"x2'-0"	9-#5 EWB	
		$\frac{2}{\sqrt{2}}$	
MWF2.0	2'-0"x1'-0"xCONT	3-#5 LWB, #5@12" SWB	
MWF3.0	3'-0"x1'-0"xCONT	3-#5 LWB, #5@12" SWB	
MWF3.0A	3'-0"x2'-0"xCONT	4-#5 LWB, #5@12" SWB	
MWF3.5	3'-6"x2'-0"xCONT	4-#5 LWB, #5@12" SWB	
MWF4.0	4'-0"x2'-0"xCONT	5-#5 LWB, #5@12" SWB	
MWF6.0	6'-0"x2'-0"xCONT	7-#6 LWB, #6@12" SWB	
MWF8.0	8'-0"x2'-0"xCONT	8-#6 LWB, #6@12" SWB	
MWF11.0	11'-0"x2'-0"xCONT	11-#8 LWB, #7@12" SWB	
MWF12.0	12'-0"x1'-6"xCONT	12-#8 LWB, #6@12" SWB	
			$\hat{\Lambda}$
F6.0	6'-0"x6'-0"x2'-0"	10-#5 EWB, 10-#5 EWT	
F5.0	5'-0"x5'-0"x2'-0"	9-#5 EWB, 9-#5 EWT	
TE2	1'-0"x1'-0"x2'-0"	2-#5 LWB, 1-#5 LWT	

NOTES:
"F" INDICATES A COLUMN SPREAD FOOTING.
"MF" INDICATES A MONOLITHIC INTERIOR COLUMN FOOTING.
"WF" INDICATES A STEM WALL CONTINUOUS FOOTING.
"MWF" INDICATES A MONOLITHIC WALL FOOTING
"TE" INDICATES THICKENED EDGE
"CMF" INDICATES CONTINUOUS INTERIOR MONOLITHIC FOOTING





FOUNDATION NOTES:

1. INTERIOR SLAB—ON—GRADE SHALL BE 4" THICK, F'C=4,000 PSI, REINFORCED W/65 LB/YD3, STEEL FIBERS, OR W/9.2 LB/YD3 SYNTHETIC MACRO FIBERS, OR 6X6—W1.4XW1.4 WWF MID.) (SEE REINFORCING STEEL NOTE #3202 ON SHEET SO.1 FOR MORE INFORMATION). SAWCUT CONTROL JOINTS (CJ) SHALL BE AS SHOWN ON PLAN. CURE INTERIOR SLAB W/25% SOLID CURE & SEAL, EUCO SUPER AQUA—CURE VOX, SUPER DIAMOND CLEAR OR APPROVED FOLIAL LISE AN 10 MIL VAPOR BARRIER PELOW THE SLAP.

CURE & SEAL, EUCO SUPER AQUA—CURE VOX, SUPER DIAMOND CLEAR OR APPROVED EQUAL. USE AN 10 MIL VAPOR BARRIER BELOW THE SLAB.

2. DROP FOOTINGS AS REQUIRED TO AVOID INTERFERENCE WITH PLUMBING AND/OR OTHER

UTILITIES.

3. SEE ARCHITECTURAL AND SITE DRAWINGS FOR BUILDING ORIENTATION AND LOCATION.

SEE ARCHITECTURAL AND SITE DRAWINGS FOR BUILDING ORIENTATION AND LOCATION.
 SEE ARCHITECTURAL DRAWINGS FOR SIDEWALK LOCATIONS.
 SEE ARCHITECTURAL DRAWINGS FOR DOOR SIZES AND LOCATIONS.

SEE SHEET SO.1 FOR CONTROL JOINT AND CONSTRUCTION JOINT REQUIREMENTS. DO NOT SAW CUT SLAB AT POST LOCATIONS.
 FOUNDATION SUB-GRADE MUST BE PREPARED IN ACCORDANCE WITH SOILS REPORT REFERENCED ON SHEET SO.1 COORD. VAPOR BARRIER REQUIREMENTS W/ ARCH.

EXTERIOR FOOTINGS MUST BEAR A MINIMUM OF 1'-0" BELOW FINISHED GRADE.

9. "F" INDICATES A POST SPREAD FOOTING.
"MF" INDICATES A MONOLITHIC INTERIOR POST FOOTING.
"WF" INDICATES A STEM WALL CONTINUOUS FOOTING.
"CMF" INDICATES CONTINUOUS INTERIOR MONOLITHIC FOOTING.

"TE" INDICATES THICKENED EDGE

MWX DENOTES 8" CMU WALL w/ REINFORCING SEE SCHEDULE & DETAILS ON SHEET S1.2, AND ON PLAN PROVIDE (1) #5 GROUTED SOLID IN EACH FIRST CELL, AT END OF WALLS, CORNERS, TEES, EACH END OF WALL CONTROL JOINTS AND EACH SIDE OF OPENINGS UNO. REINFORCED CELL SHALL BE FULL HEIGHT OF WALL. TYP. UNO. UNLESS NOTED OTHERWISE CMU WALLS SHALL BE MWA.

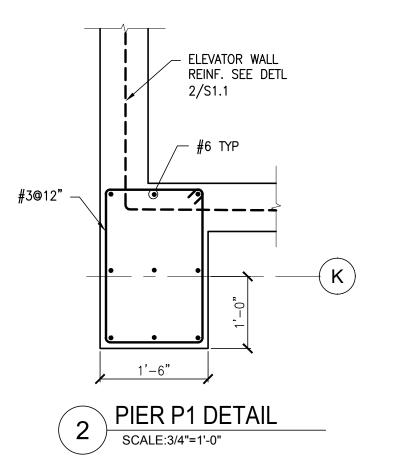
3. ELEVATIONS SHOWN ON THESE PLANS ARE RELATIVE ELEVATIONS WITHIN THE STRUCTURAL DRAWING SET ONLY WITH TOP OF SLAB AT GROUND LEVEL SET ARBITRARILY AT 0'-0". THE ELEVATIONS CONTAINED ON THESE PLANS DO NOT REPRESENT ACTUAL NGVD ELEVATIONS. REF ARCH AND CIVIL DRAWINGS FOR ACTUAL TOP OF SLAB, TOP OF STEEL AND TOP OF ROOF ELEVATIONS.

14. BP-X INDICATES A BASE PLATE TYPE & ITS ORIENTATION, SEE S2.0.

15. INDICATES LIGHT GAGE STUD "LG" LOAD BEARING WALL.

16. COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS BEFORE COMMENCING CONSTRUCTION. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND STRUCTURAL ENGINEER.

17. CM1 (1)#6 EACH CELL W/ 90° HOOK TOP OF COLUMN



NOTE:

DO NOT USE STRUCTURAL DRAWINGS ALONE FOR BUILDING LAYOUT.

DO NOT SCALE THESE DRAWINGS MANUALLY OR ELECTRONICALLY.

COORDINATE LOCATIONS OF ALL STRUCTURAL ELEMENTS, INCLUDING

COLUMNS, WALLS, SLAB EDGES, DEPRESSIONS AND OPENINGS WITH

ARCHITECTURAL DRAWINGS AND RESOLVE ANY CONFLICTS PRIOR TO

BUILDING LAYOUT. A REGISTERED SURVEYOR SHALL PERFORM BUILDING

LAYOUT AND LOCATION OF ALL STRUCTURAL ELEMENTS AT ALL LEVELS.

POST SYMBOLS

(XXX) POST NO. (SEE SCHEDULE)

INDICATES POST ABOVE

INDICATES POST THROUGH

INDICATES POST BELOW

**S3.0** 

STATE OF

JDM

21DS-0007 AS NOTED

FL. Cert. of Auth. # 8371

Sarasota, FL 34236

1626 Ringling Boulevard, Suite 400

(941) 927-8525/ Fax (941) 927-8075 Jerome J. DiMercurio, PE FL. Registration # 54446

SELF ST FLORIDA

V MINNEO

TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THESE PLANS COMPLIES WITH APPLICABLE MINIMUM BUILDING CODES



PLAN NOTES:

- 1. COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS BEFORE COMMENCING CONSTRUCTION. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND STRUCTURAL ENGINEER.
- 2. 2" 20ga. COMPOSITE METAL DECK (VULCRAFT 2VLI OR EQ.) w/2½" NORMAL WEIGHT CONCRETE (TOTAL = 4½"), REINFORCE w/ 6x6-W1.4xW1.4 WWF. OR 20 lb/yd3 OF 2" STEEL FIBERS OR 4 lb/yd³ OF MACRO SYNTHETIC FIBERS (SEE STEEL REINF. NOTE ON SHEET SO.1 FOR DETAILS). INSTALL DECK IN 3 SPAN LENGTHS MINIMUM.
- 3. SEE TYPICAL DETAILS FOR DECK ATTACHMENT.
- 4. ELEVATIONS SHOWN ON THESE PLANS ARE RELATIVE ELEVATIONS WITHIN THE STRUCTURAL DRAWING SET ONLY WITH TOP OF SLAB AT GROUND LEVEL SET ARBITRARILY AT 0'-0". THE ELEVATIONS CONTAINED ON THESE PLANS DO NOT REPRESENT ACTUAL NGVD ELEVATIONS. REF ARCH AND CIVIL DRAWINGS.
- 5. \_\_\_\_ INDICATES A BRACE FRAME TYPE, SEE S4.1 & S4.2.
- 6. (#) INDICATES POST TYPE. SEE SCHEDULE ON S2.0.
  7. SC INDICATES SLIP CRITICAL BOLTED CONNECTION.
- 8. EXXXXXX MWX DENOTES 8" CMU WALL w/ REINFORCING SEE SCHEDULE & DETAILS ON SHEET S1.2 AND ON PLAN PROVIDE (1) #5 GROUTED SOLID IN EACH FIRST CELL, AT END OF WALLS, CORNERS, TEES, EACH END OF WALL CONTROL JOINTS AND EACH SIDE OF OPENINGS UNO. REINFORCED CELL SHALL BE FULL HEIGHT OF WALL. TYP. UNO. UNLESS NOTED OTHERWISE CMU WALLS SHALL BE MWA.
- 9. DESIGN CONNECTION FOR D=19K L=29K (ASD)
- 10. DESIGN CONNECTION FOR D=23K L=38K (ASD)
- 11. LOCATE BOTTOM OF BEAM AT HEAD OF OPENING

NOTE:

DO NOT USE STRUCTURAL DRAWINGS ALONE FOR BUILDING LAYOUT.

DO NOT SCALE THESE DRAWINGS MANUALLY OR ELECTRONICALLY.

COORDINATE LOCATIONS OF ALL STRUCTURAL ELEMENTS, INCLUDING

COLUMNS, WALLS, SLAB EDGES, DEPRESSIONS AND OPENINGS WITH

ARCHITECTURAL DRAWINGS AND RESOLVE ANY CONFLICTS PRIOR TO

BUILDING LAYOUT. A REGISTERED SURVEYOR SHALL PERFORM BUILDING

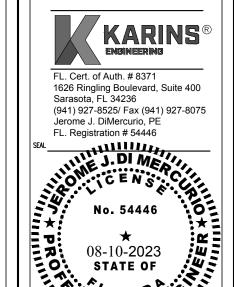
LAYOUT AND LOCATION OF ALL STRUCTURAL ELEMENTS AT ALL LEVELS.

C	OLUMN SYMBOLS
(XX)	COLUMN NO. (SEE SCHEDULE
$\bigcirc$	INDICATES COLUMN ABOVE
$\bigcirc$	INDICATES COLUMN THROUGH
	INDICATES COLUMN BELOW

CONDEV MINNEOLA SELF S MINNEOLA, FLORID

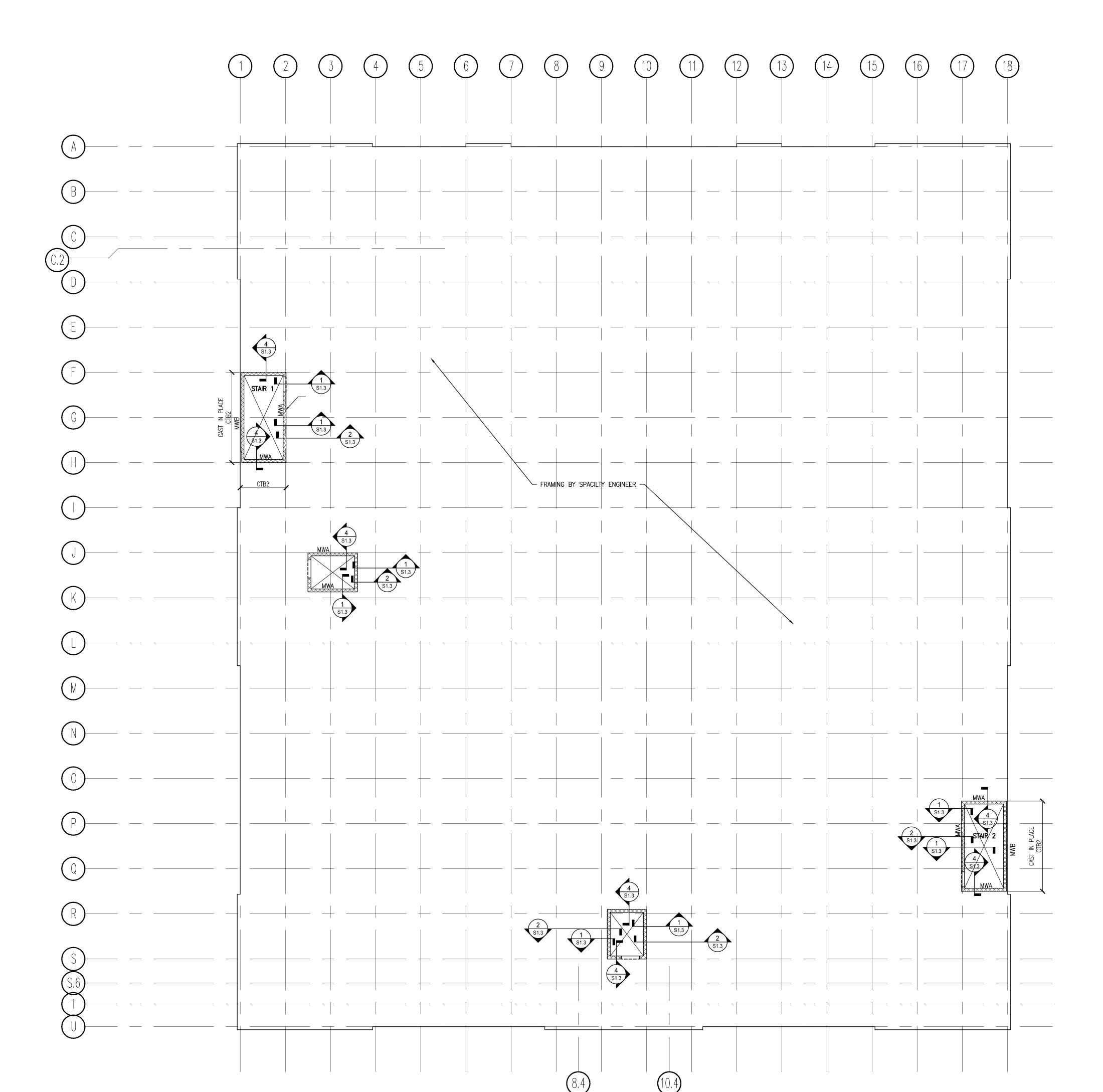
ND FLOOR FRAMING PLAN

Drawn: RHR
Checked: JDM
KEG File#: 21DS-0007
Scale: AS NOTED





**COLUMN SYMBOLS** (XX) COLUMN NO. (SEE SCHEDULE INDICATES COLUMN ABOVE INDICATES COLUMN THROUGH INDICATES COLUMN BELOW



3RD FLOOR FRAMING PLAN
SCALE:3/32"=1'-0

PLAN NOTES:

1. COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS BEFORE COMMENCING CONSTRUCTION. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND STRUCTURAL ENGINEER.

2. 2" 20ga. COMPOSITE METAL DECK (VULCRAFT 2VLI OR EQ.)  $w/2\frac{1}{2}$ " NORMAL WEIGHT CONCRETE (TOTAL =  $4\frac{1}{2}$ "), REINFORCE w/ 6x6-W1.4xW1.4 WWF. OR 20 lb/yd3 OF 2" STEEL FIBERS OR 4 lb/yd3 OF MACRO SYNTHETIC FIBERS (SEE STEEL REINF. NOTE ON SHEET SO.1 FOR DETAILS). INSTALL DECK IN 3 SPAN LENGTHS

3. SEE TYPICAL DETAILS FOR DECK ATTACHMENT.

4. ELEVATIONS SHOWN ON THESE PLANS ARE RELATIVE ELEVATIONS WITHIN THE STRUCTURAL DRAWING SET ONLY WITH TOP OF SLAB AT GROUND LEVEL SET ARBITRARILY AT 0'-0". THE ELEVATIONS CONTAINED ON THESE PLANS DO NOT REPRESENT ACTUAL NGVD ELEVATIONS. REF ARCH AND CIVIL DRAWINGS

5.  $\longrightarrow$   $\longrightarrow$  INDICATES A BRACE FRAME TYPE, SEE S4.1 & S4.2.

6. (C#) INDICATES POST TYPE. SEE SCHEDULE ON S2.0.

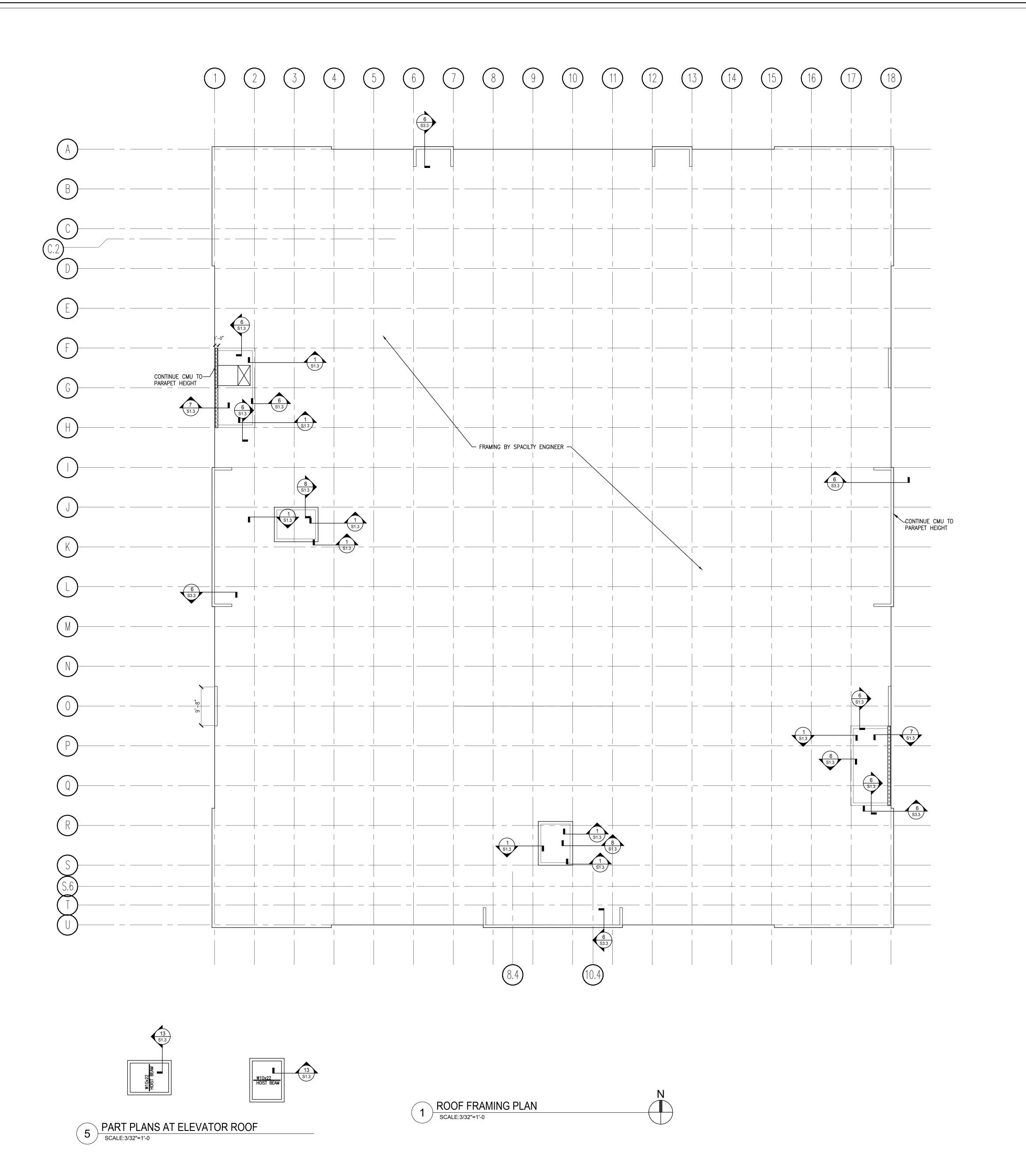
7. SC INDICATES SLIP CRITICAL BOLTED CONNECTION.

8. 

MWX DENOTES 8" CMU WALL W/ REINFORCING SEE SCHEDULE & DETAILS ON SHEET S1.2 AND ON PLAN PROVIDE (1) #5 GROUTED SOLID IN EACH FIRST CELL, AT END OF WALLS, CORNERS, TEES, EACH END OF

WALL CONTROL JOINTS AND EACH SIDE OF OPENINGS UNO. REINFORCED CELL SHALL BE FULL HEIGHT OF

NOTE:
DO NOT USE STRUCTURAL DRAWINGS ALONE FOR BUILDING LAYOUT. DO NOT SCALE THESE DRAWINGS MANUALLY OR ELECTRONICALLY. COORDINATE LOCATIONS OF ALL STRUCTURAL ELEMENTS, INCLUDING COLUMNS, WALLS, SLAB EDGES, DEPRESSIONS AND OPENINGS WITH ARCHITECTURAL DRAWINGS AND RESOLVE ANY CONFLICTS PRIOR TO BUILDING LAYOUT. A REGISTERED SURVEYOR SHALL PERFORM BUILDING LAYOUT AND LOCATION OF ALL STRUCTURAL ELEMENTS AT ALL LEVELS.



5. ELEVATIONS SHOWN ON THESE PLANS ARE RELATIVE ELEVATIONS WITHIN THE STRUCTURAL DRAWING SET ONLY WITH TOP OF SLAB AT GROUND LEVEL SET ARBITRARILY AT 0'-0". THE ELEVATIONS CONTAINED ON THESE PLANS DO NOT REPRESENT ACTUAL NGVD ELEVATIONS. REF ARCH AND CIVIL DRAWINGS FOR ACTUAL TOP OF SLAB, TOP OF STEEL AND TOP OF ROOF ELEVATIONS.

6. ROOF TOP MECHANICAL UNIT FRAMING SHOWN IS DIAGRAMMATIC ONLY, THE ACTUAL PLAN LOCATIONS SHALL BE COORDINATED WITH MECHANICAL ENGINEER AND MECHANICAL UNIT DETAILS. DO NOT SCALE RTU SUPPORT FRAMING LOCATIONS FROM THIS PLAN.

7. @ INDICATES A COLUMN TYPE SEE COLUMN SCHEDULE S2.0.

8. SC INDICATES SLIP CRITICAL BOLTED CONNECTION.

9. F# INDICATES A BRACED FRAME TYPE, SEE S4.1 10. WXXXXXX MWX DENOTES 8" CMU WALL W/ REINFORCING SEE SCHEDULE

& DETAILS ON SHEET S1.2, S2.0 AND ON PLAN PROVIDE (1) #5 GROUTED SOLID IN EACH FIRST CELL, AT END OF WALLS, CORNERS, TEES, EACH END OF WALL CONTROL JOINTS AND EACH SIDE OF OPENINGS UNO. REINFORCED CELL SHALL BE FULL HEIGHT OF WALL. TYP. UNO. UNLESS NOTED OTHERWISE CMU WALLS SHALL BE MWA.

NOTE:
DO NOT USE STRUCTURAL DRAWINGS ALONE FOR BUILDING LAYOUT. DO NOT SCALE THESE DRAWINGS MANUALLY OR ELECTRONICALLY. COORDINATE LOCATIONS OF ALL STRUCTURAL ELEMENTS, INCLUDING COLUMNS, WALLS, SLAB EDGES, DEPRESSIONS AND OPENINGS WITH ARCHITECTURAL DRAWINGS AND RESOLVE ANY CONFLICTS PRIOR TO BUILDING LAYOUT. A REGISTERED SURVEYOR SHALL PERFORM BUILDING LAYOUT AND LOCATION OF ALL STRUCTURAL ELEMENTS AT ALL LEVELS.

**COLUMN SYMBOLS** (xx) COLUMN NO. (SEE SCHEDULE) INDICATES COLUMN ABOVE INDICATES COLUMN THROUGH

INDICATES COLUMN BELOW

AS NOTED

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V MINNEOLA MINNEOLA