

STRUCTURAL DESIGN CRITERIA

BUILDING CODES:
2020 FLORIDA BUILDING CODE (FBC)

LOADING CRITERIA:
A. APPLIES TO: OCCUPIED GROUND AND RAISED FLOOR.

DEAD LOADS:

MEZZANINES:	25 PSF	(INCLUDES MPE/CEILING ALLOWANCE)
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LIVE LOADS:

A. REDUCIBLE PER IBC: 2020

<u>ROOFS AND CANOPIES:</u>		
<u>FLOORS:</u>	20	PSF
FIRST FLOOR GYMNASIUM:	100	PSF
STAIRS:	100	PSF
HANDRAILS:	50	PLF
MEZZANINES:	125	PSF

WIND LOADS:

A. PER ASCE: 7-16

BASIC WIND SPEED (VULT):	160	MPH
BASIC WIND SPEED (VASD):	124	MPH
RISK CATEGORY:	II	
WIND EXPOSURE CATEGORY:	C	
MAIN WIND FORCE RESISTING SYSTEM (MWFRS):	PER PEMB MANUFACTURER	
COMPONENTS AND CLADDING PRESSURES:	REFER TO PEMB MANUFACTURER	

SEISMIC DESIGN CRITERIA:

IMPORTANCE FACTOR (Is):	1.0
RISK CATEGORY:	II
SITE CLASSIFICATION:	D
SEISMIC DESIGN CATEGORY:	A

MAPPED SPECTRAL RESPONSE ACCELERATION:

Ss:	0.051
S1:	0.029

MAPPED SPECTRAL DESIGN ACCELERATION:

SDS:	0.054
SD1:	0.047

BASIC SEISMIC FORCE RESISTING SYSTEM: PER PEMB MANUFACTURER
RESPONSE MODIFICATION COEFFICIENT (R): PER PEMB MANUFACTURER
DEFLECTION AMPLIFICATION FACTOR (Cd): PER PEMB MANUFACTURER
OVER STRENGTH (W): PER PEMB MANUFACTURER
SEISMIC RESPONSE COEFFICIENT (Cs): PER PEMB MANUFACTURER
DESIGN BASE SHEAR (V): PER PEMB MANUFACTURER
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

MATERIAL STRENGTHS:

CONCRETE:

A. DESIGN PER CURRENT EDITION OF: ACI 318

SLAB-ON-GRADE:	F'c = 4,000	PSI (MIN)
FOOTINGS/FOUNDATION WALLS:	F'c = 4,000	PSI
ELEVATED SLABS:	F'c = 4,000	PSI (LW)
ALL OTHER CONCRETE:	F'c = 3,000	PSI
REINFORCING STEEL:	ASTM A615, GRADE 60	
WELDED WIRE FABRIC:	ASTM A1064	

B. UNLESS SPECIFICALLY STATED OTHERWISE, ALL CONCRETE SHALL CONFORM TO AMERICAN CONCRETE INSTITUTE (ACI) AND NATIONAL READY MIX PROCEDURES ASSOCIATION (NRMPA) STANDARD CONSTRUCTION SPECIFICATIONS.

CONCRETE

GENERAL NOTES:

- PROVIDE 3/4" CHAMFER AT ALL EXPOSED CORNERS OF BEAMS, WALLS, SLABS, ETC.
- ALL CONCRETE SHALL BE MECHANICALLY VIBRATED IN ACCORDANCE WITH: ACI 304 AND ACI 309
- ALL EXTERIOR CONC. PERMANENTLY EXPOSED TO WEATHER SHALL CONTAIN AN AIR ENTRAINING ADMIXTURE.
- CONTRACTOR SHALL REFER TO AND COORDINATE WITH OTHER DISCIPLINES DRAWINGS AND OR VENDOR DRAWINGS FOR EMBEDDED ITEMS AND OR RECESSES NOT SHOWN IN THE STRUCTURAL DRAWINGS.

REINFORCING:

A. UNLESS NOTED OTHERWISE (U.N.O.) ON THE DWGS, THE MIN. COVER FOR REINFORCING SHALL BE AS FOLLOWS:

SLABS, WALLS, AND JOISTS:		
EXPOSED TO EARTH LIQUID OR WEATHER:	2.00	INCHES
NOT EXPOSED TO EARTH LIQUID OR WEATHER:	0.75	INCHES
FOOTINGS:	3.00	INCHES
COLUMNS/BEAMS:	1.50	INCHES
ELEVATED SLABS:	1.00	INCH (FROM TOP)
SLABS ON GRADE:	2.00	INCHES (FROM TOP)

B. ALL REINFORCING SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES IN CONFORMANCE WITH THE FOLLOWING DURING THE PLACING OF CONCRETE:

CRSI MANUAL OF STANDARD PRACTICE
ACI 315

C. ALL REINFORCING SHALL BE DETAILED IN ACCORDANCE WITH THE FOLLOWING:
ACI DETAILING MANUAL, SP-86
THE CRSI MANUAL OF CONCRETE PRACTICE
ACI 318

D. PROVIDE BAR SUPPORTS AND SPACERS IN ACCORDANCE WITH THE FOLLOWING:
CRSI MANUAL OF STANDARD PRACTICE
ACI 315

E. ALL BAR SUPPORTS IN AREA WHERE CONCRETE WILL BE EXPOSED SHALL HAVE PLASTIC TIPPED FEET. THE CONTRACTOR IS CAUTIONED THAT CARE MUST BE EXERCISED TO PREVENT EXPOSURE OF THE TIE WIRE OR OTHER MATERIAL WHICH MAY CAUSE STAINING OF EXPOSED CONCRETE. PROPER COVER AS INDICATED ABOVE SHALL BE MAINTAINED ON ALL REINFORCEMENT.

F. ALL HOOKS IN REINFORCING BARS SHALL BE ACI STANDARD HOOKS, U.N.O.

G. DOWELS FROM FOUND. OR SLABS TO WALLS SHALL MATCH WALL REINFORCING, UNLESS NOTED OTHERWISE. DOWELS SHALL BE PLACED BEFORE CONC. IS POURED. DOWELS SHALL NOT BE PUSHED INTO THE CONCRETE.

H. WHERE GRADE BEAMS OR STRIP FOOTINGS INTERSECT COLUMNS FOUNDATIONS, EXTEND GRADE BEAM OR STRIP FOOTING REINFORCEMENT CONTINUOUSLY THROUGH THE COLUMN FOUNDATION.

I. WELDED WIRE FABRIC SHALL BE LAPPED A MINIMUM OF WIRE SPACING PLUS 6" AND TIED.

J. WELDED WIRE FABRIC SHALL BE FABRICATED IN FLAT SHEETS. ROLLS ARE NOT ALLOWED.

K. UNLESS NOTED OTHERWISE, TENSION SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE: CLASS B

REINFORCING BARS:

F'c	#6 AND SMALLER				#7 AND LARGER			
	OTHER BARS		TOP BARS		OTHER BARS		TOP BARS	
	CLASS A	CLASS B	Class A	Class B	Class A	Class B	Class A	Class B
3,000 PSI	44 db	57 db	57 db	74 db	55 db	72 db	72 db	93 db
4,000 PSI	38 db	50 db	50 db	65 db	48 db	62 db	62 db	81 db
5,000 PSI	34 db	45 db	45 db	58 db	43 db	56 db	56 db	72 db

NOTES:

- ALL LAPS SHALL BE CLASS B UNLESS NOTED OTHERWISE (U.N.O.)
- BEAMS AND COLUMNS: INCREASE LAPS SHOWN BY 50% IF CLEAR SPACING OF BARS IS LESS THAN 2 db, OR IF CLEAR COVER OF BARS IS LESS THAN 1 db.
- WALLS, SLABS, AND FOOTINGS: INCREASE LAPS SHOWN BY 50% IF CLEAR SPACING OF BARS IS LESS THAN 2 db, OR IF CLEAR COVER OF BARS IS LESS THAN 2 db.
- INCREASE LAPS BY 25% FOR GRADE 75 REINFORCEMENT.
- INCREASE LAPS BY 33% FOR LIGHTWEIGHT CONCRETE.

CONTROL JOINTS:

A. SAWN CONTROL JOINTS IN SLAB ON GRADE SHALL BE CUT IN ACCORDANCE WITH: ACI 302.1R

B. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT.

C. CONTROL JOINTS ARE DIAGRAMMATICALLY SHOWN ON THE PLANS. THE CONTRACTOR MAY ADJUST THE SPACING OF THE JOINTS AND SUBMIT A REVISED SLAB CONTROL JOINT PLAN TO THE ENGINEER FOR APPROVAL. THE LENGTH TO WIDTH RATIO BETWEEN JOINTS SHALL NOT EXCEED 1.5 AND THE AREA BOUNDED BY THE JOINTS SHALL NOT EXCEED 200SF FOR 4' SLABS AND 400SF FOR 6' SLABS.

CONCRETE SLABS:

A. ALL CONCRETE SLABS-ON-GRADE SHALL BE CURED USING A LIQUID MEMBRANE FORMING CURING COMPOUND WHERE PRACTICAL. REFER TO THE SPECIFICATIONS FOR FURTHER INFORMATION.

B. SLAB-ON-GRADE VAPOR BARRIERS SHALL BE A MINIMUM OF 15 MILS THICK. OVERLAP SEAMS 6" AND TAPE.

C. PROVIDE TWO (2) #4 x 3'-0" LONG DIAGONAL BARS, SPACED 6" O.C AT 2' BELOW FINISHED FLOOR AT ALL RE-ENTRANT CORNERS IN SLABS. EXTEND REINFORCEMENT PAST RE-ENTRANT CORNERS A MINIMUM OF 12".

D. PLUMBING, MECHANICAL, AND ELECTRICAL CONTRACTORS SHALL SUBMIT SIZES AND LOCATIONS OF ALL PENETRATIONS THROUGH ELEVATED STRUCTURAL SLABS FOR THE STRUCTURAL ENGINEERS APPROVAL PRIOR TO PLACEMENT OF THE SLAB. NO OPENINGS OR PENETRATIONS SHALL BE ADJACENT TO A COLUMN OR WITHIN A DISTANCE EQUAL TO THE THICKNESS OF THE SLAB FROM THE FACE OF THE COLUMN UNLESS APPROVED BY THE STRUCTURAL ENGINEER.

E. ALL PIPE PENETRATIONS THROUGH ELEVATED CONCRETE SLABS SHALL BE SLEEVED PER: ACI 318

F. ANY CONDUIT AND/OR PIPE RUNNING IN A SLAB OR WALL SHALL BE SPACED NOT LESS THAN 3 DIAMETERS AND SHALL NOT BE LARGER THAN 1/3 THE SLAB THICKNESS.

G. PROVIDE 1/2" PRE-MOLDED EXPANSION JOINT MATERIAL WITH FLEXIBLE JOINT SEALANT WHERE SLAB ON GRADE IS POURED AROUND COLUMNS AND AGAINST GRADE BEAMS OR WALLS, UNLESS OTHERWISE SHOWN OR NOTED.

H. FOR FLATNESS AND LEVELNESS, CONCRETE SLABS SHALL CONFORM TO: ACI 117-90

ACCORDING TO: ASTM E 1155

	COMPOSITE FLATNESS (F)	COMPOSITE LEVELNESS (F)
SPECIFIED OVERALL VALUE:	25	20
MINIMUM LOCAL VALUE:	17	15

METAL DECKING

GENERAL NOTES:

- ALL METAL DECKING SHALL BE INSTALLED WITH A MINIMUM OF THREE SPANS.
- EACH DECKING PANEL SHALL BE CONNECTED TO SUPPORTING MEMBERS.
- ADJACENT PANELS SHALL BE INTERCONNECTED WITH SIDE LAP FASTENERS AT EACH SPAN.
- REFER TO PEMB MANUFACTURER FOR CONNECTION DETAILS

DESIGN CRITERIA:

A. DESIGN PER CURRENT EDITION OF: SDI

METAL FLOOR DECKING BY CONTRACTOR

Sheet Number	Sheet Name
SO.0	GENERAL NOTES
SO.1	SPECIAL INSPECTIONS
S1.0	FOUNDATION & MEZZANINE PLANS
S2.0	FOUNDATION SECTIONS AND DETAILS
S2.1	MASONRY & MEZZANINE SECTIONS AND DETAILS

REINFORCED MASONRY

DESIGN CRITERIA:

- DESIGN PER CURRENT EDITION OF: ACI 530
- REFER TO THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR ANY EMBEDDED ITEMS THAT SHALL BE CAST INTO MASONRY WALLS OR ANY OPENINGS REQUIRED THROUGH MASONRY WALLS.
- WALL REINFORCING LAID OUT ON PLAN DETAILS ARE DESIGNED FOR FINAL CONSTRUCTION USE PURPOSES ONLY. GC TO ENSURE TEMPORARY SHORING DURING CONSTRUCTION UNTIL FINAL BUILDING CONDITIONS ARE COMPLETE.

LOAD BEARING AND EXTERIOR WALLS:

A. COMPOSED OF HOLLOW CONCRETE MASONRY UNITS (CMU)

B. CMU SHALL BE LAID IN A "FULL BEDDING" OF MORTAR.

PATTERN:	RUNNING BOND
CMU PER:	ASTM C90
TESTED IN ACCORDANCE WITH:	ASTM C140
CMU TYPE:	NORMAL WEIGHT
DENSITY:	135 LBS/CU. FT.
NET COMPRESSIVE STRENGTH	1,900 PSI

MORTAR:

A. FOR CMU WALLS:

TYPE:	S
PROPORTIONED PER:	ASTM C270
TESTED IN ACCORDANCE WITH:	ASTM C780

GROUP:

A. FOR CMU WALLS:

COMPRESSIVE STRENGTH	2,000 PSI
GROUT MIX DESIGNED & TESTED IN ACCORDANCE WITH:	ASTM C476

REINFORCEMENT:

A. ALL REINFORCING STEEL SHALL BE: ASTM A615, GRADE 60

B. ALL REINF. STEEL SHALL BE SECURED IN PLACE AND INSPECTED BY THE TESTING AGENCY PRIOR TO GROUTING.

C. REINFORCE CMU WALLS PER THE REINFORCING SCHEDULE OR AS INDICATED ON PLANS.

D. REFER TO THE REINFORCEMENT SCHEDULE ON SHEET: S2.1

E. REINFORCE EA. CORNER, WALL END, WALL INTERSECTION, EACH SIDE OF CONTROL JOINTS AND EXPANSION JOINTS, AND EA. SIDE OF OPENINGS (DOORS AND WINDOWS) WITH ONE (1) #5 VERTICAL, FULL HEIGHT OF WALL.

F. ALL REINFORCED CELLS SHALL BE GROUDED SOLID.

G. REINFORCE ALL CORNERS WITH CONTINUOUS CORNER BARS IN BOND BEAMS.

H. FOR CORNER REINFORCEMENT, REFER TO SHEET: S2.1

HORIZONTAL JOINT REINFORCEMENT:

TYPE:	LADDER (HORIZONTAL JOINT REINFORCEMENT)
SPACING:	16 INCHES ON CENTER, VERTICALLY, U.N.O.

A. INSTALL JOINT REINFORCING IN THE FIRST TWO MORTAR JOINTS ABOVE AND BELOW OPENINGS, EXTENDING AT LEAST 24" BEYOND THE OPENING, TYPICAL.

B. PROVIDE HORIZONTAL JOINT REINFORCEMENT IN PARAPETS AND FREE STANDING WALLS AT 8" O.C. VERTICALLY, LAP JOINT REINFORCEMENT 6" MIN. AND USE PREFABRICATED UNITS AT CORNERS AND T INTERSECTIONS.

VERTICAL REINFORCEMENT:

A. VERTICAL BAR POSITIONERS SHALL BE UTILIZED TO SECURELY HOLD VERTICAL BARS IN POSITION IN THE CENTER OF THE CORES. GROUT SHALL BE PLACED IN ALL REINFORCED CELLS. GROUT SHALL BE CONSOLIDATED, (VIBRATED), DURING PLACEMENT. MAXIMUM GROUT LIFT SHALL BE 4'-0". VERTICAL REINFORCING SHALL BE DETAILED AND LAPPED ACCORDINGLY.

B. LAP VERTICAL BARS PER THE MASONRY LAP SCHEDULE. ALL LAPS SHALL BE SECURED WITH WIRE TIES.

C. REFER TO THE MASONRY LAP SCHEDULE ON SHEET: S2.1

EXPANSION AND CONTROL JOINTS:

A. COORDINATE EXPANSION OR CONTROL JOINTS IN MASONRY WALLS WITH ARCHITECTURAL DRAWINGS. EXPANSION OR CONTROL JOINTS SHALL NOT OCCUR WITHIN MASONRY "SHEAR WALLS" AS INDICATED ON THE PLANS.

SUPPLEMENTARY NOTES

A. PROVIDE ALL TEMPORARY BRACING, GUYING OR OTHER MEANS TO AVOID EXCESSIVE STRESSES AND TO HOLD STRUCTURAL ELEMENTS IN PLACE DURING CONSTRUCTION. THE STRUCTURE SHOULD NOT BE CONSIDERED STABLE UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN CONSTRUCTED.

B. THE STRUCTURAL ENGINEER SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES OR SEQUENCES. FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR, OR ANY OTHER PERSONS PERFORMING THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

C. VERIFY ALL DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS.

D. SEE THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR EMBEDS, OPENINGS, SLEEVES, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS.

E. ALL STRUCTURAL OPENINGS AROUND OR AFFECTED BY MECHANICAL, ELECTRICAL, AND PLUMBING EQUIPMENT SHALL BE VERIFIED WITH EQUIPMENT PURCHASED BEFORE PROCEEDING WITH STRUCTURAL WORK AFFECTED.

F. STRUCTURAL ENGINEER OF RECORD FOR THIS PROJECT IS NOT RESPONSIBLE FOR THE DESIGN OF STEEL STAIRS, HANDRAILS, COLD FORMED METAL FRAMING, OR OTHER SYSTEMS NOT INDICATED ON THE STRUCTURAL DOCUMENTS. REFER TO SPECIFICATIONS FOR THESE ITEMS FOR DEFERRED DESIGN SUBMITTAL REQUIREMENTS.

G. ANY ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL BEAR THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF THE PROJECT.

H. GENERAL CONTRACTOR MUST REVIEW AND APPROVE SHOP DRAWINGS PRIOR TO SUBMITTAL TO ARCHITECT/ENGINEER SUBMITTALS WHICH DO NOT CONTAIN THE CONTRACTORS SHOP DRAWING OR STAMP OR HAVE BEEN MERELY "RUBBER STAMPED" SHALL BE RETURNED WITHOUT REVIEW.

I. DO NOT REPRODUCE STRUCTURAL ENGINEERS' DRAWINGS. ERECTION AND SHOP DRAWINGS WILL NOT BE REVIEWED IF ANY PORTION CONTAINS REPRODUCTIONS OF STRUCTURAL ENGINEERS' DRAWINGS.

J. DO NOT SCALE DRAWINGS. IF DIMENSIONS ARE IN QUESTION, THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING CLARIFICATION FROM THE ARCHITECT BEFORE CONTINUING WITH CONSTRUCTION.

STRUCTURAL SPECIFICATIONS

MASONRY:

A. MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO THE REQUIREMENTS OF "SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530.1/ASCE 6)", LATEST EDITION, EXCEPT AS MODIFIED BY REQUIREMENTS OF THE CONTRACT DOCUMENTS.

SITE PREPARATION:

A. FOOTING AND SLAB SUBGRADE PREPARATION SHALL BE IN ACCORDANCE WITH RECOMMENDATIONS OF THE GEOTECHNICAL REPORT FOR THE PROJECT AND SHALL BE IN COMPLIANCE WITH APPLICABLE REQUIREMENTS OF GOVERNING AUTHORITIES HAVING JURISDICTION. SPECIAL ATTENTION SHALL BE GIVEN TO RECOMMENDED UNDERCUTTING OF MATERIAL, CONTAINING ORGANIC MATERIAL AND PLACEMENT OF SUBGRADE FILL REQUIREMENTS OF PERCENT PASSING AND COMPACTION.

GEOTECHNICAL:

A. A GEOTECHNICAL TESTING AND INSPECTION FIRM SHALL BE EMPLOYED TO PERFORM A SOIL SURVEY FOR SATISFACTORY SOIL MATERIALS, SAMPLING AND TESTING FOR QUALITY CONTROL AS PER THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT FOR THIS PROJECT. ALL EARTHWORK OPERATIONS SHALL BE PERFORMED TO THE SATISFACTION OF THE GEOTECHNICAL TESTIN...

CONCRETE:

A. CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS", (ACI 301).

B. CONCRETE SHALL BE PROPORTIONED, BATCHED, MIXED, PLACED, CONSOLIDATED, AND CURED IN ACCORDANCE WITH ACI 301.304,308,309 AND 318.

C. ALL CONCRETE MIXES SHALL BE PROPORTIONED BY THE FIELD EXPERIENCE METHOD OR THE LABORATORY TRIAL METHOD IN ACCORDANCE W/ ACI 318, MAX WATER TO CEMENT RATIO SHALL BE 0.55.

D. PROVIDE COMPRESSIVE STRENGTH TESTS CONFORMING TO ASTM C31 AND ASTM C39. ONE SET OF FOUR CYLINDERS FOR EACH 150 CUBIC YARDS OR FRACTION THEREOF. OF EACH STRENGTH OF CONCRETE PLACED IN ANY ONE DAY. TEST ONE SPECIMEN AT SEVEN DAYS, TEST TWO SPECIMENS AT 28 DAYS AND HOLD ONE IN RESERVE. PERFORM ONE SLUMP TEST FOR EACH SET OF COMPRESSIVE STRENGTH TEST SPECIMENS. SUBMIT RESULTS DIRECTLY TO ENGINEER.

JACKIE ROBINSON TRAINING COMPLEX EXECUTIVE BUILDING

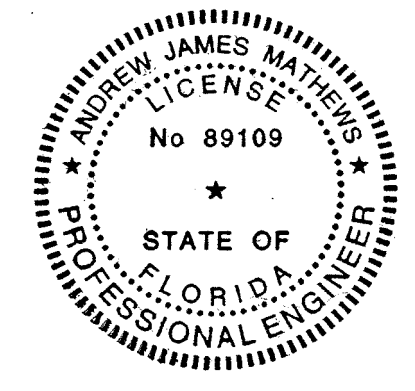
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Project No.	21081.1
Drawn By	LEB
Checked By	GRG
Date	02.10.2023

Revisions:

ANDREW J. MATHEWS
LICENSE NO. 89109



"To the best of the Architect's or Engineer's knowledge, the plans and specifications comply with the applicable minimum building codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 663 of Florida Statutes."

CONSTRUCTION DOCUMENT SET

S.O.0

GENERAL NOTES

Originals printed at 24" x 36" scale as required
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FBC 2020 SPECIAL INSPECTIONS

STATEMENT OF SPECIAL INSPECTIONS	
THIS STATEMENT OF SPECIAL INSPECTIONS IS SUBMITTED AS REQUIRED FOR BUILDING PERMIT ISSUANCE IN ACCORDANCE WITH THE SPECIAL INSPECTIONS AND STRUCTURAL TESTING REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE. THIS STATEMENT OF SPECIAL INSPECTIONS IS ONLY FOR THE STRUCTURAL PORTION OF THE WORK. REFER TO OTHER DISCIPLINES FOR OTHER SPECIAL INSPECTION REQUIREMENTS FOR THIS PROJECT.	
THE OWNER OR REGISTERED DESIGN PROFESSIONAL IN CHARGE (ARCHITECT) ACTING AS THE OWNERS AGENT SHALL EMPLOY ONE OR MORE AGENCIES APPROVED BY THE BUILDING OFFICIAL TO PERFORM INSPECTIONS DURING CONSTRUCTION. THESE INSPECTIONS ARE IN ADDITION TO SECTION 110 OF THE IBC. CONTRACTOR IS RESPONSIBLE TO ENSURE THE INSPECTOR IS PRESENT WHERE WORK REQUIRES PERIODIC OR CONTINUOUS INSPECTION.	
RESPONSIBILITIES OF THE SPECIAL INSPECTOR	
THE INSPECTOR SHALL KEEP RECORDS OF ALL INSPECTIONS AND SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. DISCOVERED DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF SUCH DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. A FINAL REPORT DOCUMENTING ALL THE REQUIRED SPECIAL INSPECTIONS AND TESTING, AND CORRECTION OF ANY DISCREPANCIES NOTED PREVIOUSLY SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO ISSUANCE OF A CERTIFICATE OF USE AND OCCUPANCY.	
FABRICATIONS	
SPECIAL INSPECTIONS OF THE FABRICATION PROCESS SHALL NOT BE REQUIRED WHERE FABRICATION OF STRUCTURAL LOAD BEARING MEMBERS AND ASSEMBLIES IS BEING PERFORMED ON THE PREMISES OF A FABRICATOR IS REGISTERED AND APPROVED TO PERFORM THE WORK WITHOUT SPECIAL INSPECTIONS. AT THE COMPLETION OF THE FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL.	
THE SPECIAL INSPECTIONS ARE IN ADDITION TO THE MATERIAL TESTING AND INSPECTIONS LISTED IN THE CONTRACT SPECIFICATIONS. CONTRACTOR IS TO COORDINATE SPECIAL INSPECTIONS, MATERIAL SPECIFIC TESTING AND INSPECTIONS WITH THE OWNER FURNISHED SPECIAL INSPECTOR MATERIAL TESTING LABS.	
THE SPECIAL INSPECTIONS INDICATED HEREIN DO NOT RELIEVE THE CONTRACTOR FROM THEIR RESPONSIBILITIES. THE CONTRACTOR SHALL PAY FOR ANY ADDITIONAL TESTING OR INSPECTION REQUIRED FROM WORK OR MATERIALS NOT IN CONFORMANCE WITH THE CONTRACT DOCUMENTS.	
THE STATEMENT OF SPECIAL INSPECTIONS INCLUDES REQUIRED VERIFICATION AND INSPECTION OF THE FOLLOWING SECTIONS:	
1. CONCRETE INSPECTIONS	
2. SOILS INSPECTIONS	
3. MASONRY INSPECTIONS	
5. STRUCTURAL STEEL INSPECTIONS	
6. SPECIAL ADDITIONAL INSPECTIONS FOR MAIN WIND FORCE RESISTING SYSTEM	
COMPONENTS PART OF THE MAIN WIND FORCE RESISTING SYSTEM AND SUBJECTED TO SPECIAL INSPECTIONS FOR WIND RESISTANCE:	
COMPONENTS PART OF THE MAIN SEISMIC FORCE RESISTING SYSTEM AND SUBJECTED TO SPECIAL INSPECTION FOR SEISMIC RESISTANCE:	

CONCRETE INSPECTIONS

FBC 2020 TABLE 1705.3: REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION				
TYPE	REFERENCED STANDARD	CONTINUOUS	PERIODIC	
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	ACI 318: Ch. 20, 25.2, 25.3, 26.6.1-26.6.3 IBC SECTION 1908.4	---	X	
2. REINFORCING BAR WELDING:		---	---	
2A. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706	AWS D14 ACI 318: CH. 26.6.4	---	X	
2B. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"	AWS D14 ACI 318: CH. 26.6.4	---	X	
2C. INSPECT ALL OTHER WELDS	AWS D14 ACI 318: CH. 26.6.4	X	---	
3. INSPECT ANCHORS CAST IN CONCRETE	ACI 318: CH. 17.8.2	---	X	
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS.		---	---	
4A. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.	ACI 318: CH. 17.8.2.4	X	---	
4B. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4A.	ACI 318: CH. 17.8.2	---	X	
5. VERIFY USE OF REQUIRED DESIGN MIX.	ACI 318: CH. 19, 26.4.3, 26.4.4 IBC SECTION 1904.1, 1904.2, 1908.2, 1908.3	---	X	
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	ASTM C172 ASTM C31 ACI 318: CH. 20.4, 26.12 IBC SECTION 1908.10	---	X	
7. INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	ACI 318: CH. 5.9, 5.10 IBC SECTION 1910.6 - 1910.8	---	X	
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	ACI 318: 26.5.3-26.5.5 IBC SECTION 1908.9	---	X	
9. INSPECTION OF PRESTRESSED CONCRETE FOR:				
9A. APPLICATION OF PRESTRESSING FORCES.	ACI 318: CH. 26.10	X	---	
9B. GROUTING OF BONDED PRESTRESSING TENDONS	ACI 318: CH. 26.10	X	---	
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.	ACI 318: CH. 26.8	---	X	
11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	ACI 318: CH. 26.11.2	---	X	
12. INSPECT FRAMEWORK FOR SHAPE, LOCATION, AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	ACI 318: CH. 21.11.1.2(b)	---	X	

SOILS INSPECTIONS

FBC 2020 TABLE 1705.6: REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS				
TYPE	REFERENCED STANDARD	CONTINUOUS	PERIODIC	
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.		---	X	
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.		---	X	
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.		---	X	
4. VERIFY USE OF PROPER MATERIALS, DENSITIES, AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.		X	---	
5. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.		---	X	

MASONRY INSPECTIONS

FOR RISK CATEGORY I, II, III BUILDINGS				
TMS 402-16 TABLE 3.1 & TMS 602-16 TABLE 3 & 4: LEVEL 2 QUALITY ASSURANCE				
MINIMUM TESTS				
PRIOR TO CONSTRUCTION, VERIFICATION OF COMPLIANCE OF SUBMITTALS.				
PRIOR TO CONSTRUCTION, VERIFICATION OF Fm AND FAAC IN ACCORDANCE WITH TMS 602 ARTICLE 1.4 B PRIOR TO CONSTRUCTION, EXCEPT WHERE SPECIFICALLY EXEMPTED BY THE CODE.				
DURING CONSTRUCTION, VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) WHEN SELF-CONSOLIDATING GROUT IS DELIVERED TO THE PROJECT SITE IN ACCORDANCE WITH TMS 602 ART. 1.5 & 1.6.3.				
	INSPECTION TASK	REFERENCED STANDARD	CONTINUOUS	PERIODIC
1.	AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:			
1A.	PROPORTIONS OF SITE-PREPARED MORTAR.	TMS 602: ART. 2.1, 2.6 A & 2.6 C	---	X
1B.	GRADE, TYPE, AND SIZE OF REINFORCEMENT AND ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES.	TMS 602: ART. 3.4 & 3.6 A	---	X
2.	PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:			
2A.	GROUT SPACE.	TMS 602: ART. 3.2 D & 3.2 F	---	X
2B.	PLACEMENT OF REINFORCEMENT, CONNECTORS, AND ANCHOR BOLTS.	TMS 402: SECT. 6.1, 6.3.1, 6.3.6, & 6.3.7 TMS 602: ART. 3.2 E & 3.4	---	X
2C.	PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS	TMS 602: ART. 2.6 B & 2.4 G.1.b	---	X
3.	VERIFY COMPLIANCE OF THE FOLLOWING DURING CONSTRUCTION:			
3A.	MATERIALS AND PROCEDURES WITH THE APPROVED SUBMITTALS.	TMS 602: ART. 1.5	---	X
3B.	PLACEMENT OF MASONRY UNITS AND MORTAR JOINT CONSTRUCTION.	TMS 602: ART. 3.3 B	---	X
3C.	SIZE AND LOCATION OF STRUCTURAL MEMBERS.	TMS 602: ART. 3.3 F	---	X
3D.	TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION.	TMS 402: SECT. 1.2.1(e), 6.2.1, & 6.3.1	---	X
3E.	WELDING OF REINFORCEMENT.	TMS 402: SECT. 6.1.6.1.2	X	---
3F.	PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F (4.4°C)) OR HOT WEATHER (TEMPERATURE ABOVE 90°F (32.2°C))	TMS 602: ART. 1.8 C & 1.8 D	---	X
3G.	PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE	TMS 602: ART. 3.5 & 3.6 C	X	---
4.	OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS.	TMS 602: ART. 1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3, & 1.4 B.4	---	X

WIND RESISTANCE

REQUIRED VERIFICATION AND INSPECTION OF WIND RESISTANCE				
VERIFICATION AND INSPECTION	REFERENCED STANDARD	CONTINUOUS	PERIODIC	
1. WOOD CONSTRUCTION: INSPECTION OF THE MAIN WIND FORCES RESISTING ELEMENTS OF THE BUILDING INCLUDING:				
1A. FIELD GLUING OPERATIONS OF ELEMENTS OF THE MWFRS	IBC SECTION 1705.11.1	X	---	
1B. DIAPHRAGM	IBC SECTION 1705.11.1	X	---	
1C. SHEAR WALLS	IBC SECTION 1705.11.1	X	---	
1D. HARD DOWNS	IBC SECTION 1705.11.1	X	---	
1E. ANCHORAGE	IBC SECTION 1705.11.1	X	---	
1F. NAILING	IBC SECTION 1705.11.1	X	---	
1G. BOLTING	IBC SECTION 1705.11.1	X	---	
1H. DRAG STRUTS	IBC SECTION 1705.11.1	X	---	
1I. INSPECTION IS NOT REQUIRED FOR WOOD SHEAR WALLS, SHEAR PANELS, DIAPHRAGMS, INCLUDING NAILING, BOLTING, FASTENING WHERE SHEATHING FASTENING IS MORE THAN 4" O.C.	---	---	---	
2. COLD-FORMED STEEL LIGHT-FRAME CONSTRUCTION				
2A. WELDING OPERATIONS OF MAIN WIND FORCE RESISTING SYSTEM	IBC SECTION 1705.110.2	---	X	
2B. SCREW ATTACHMENT, BOLTING, ANCHORING AND OTHER FASTENING WITHIN THE MAIN WIND FORCE RESISTING SYSTEM	IBC SECTION 1705.11.2	---	X	
2C. SPECIAL INSPECTIONS NOT REQUIRED IN LIGHT FRAMED SHEAR WALLS, BRACES, DIAPHRAGMS, COLLECTORS, HOLD DOWNS WHERE EITHER OF THE FOLLOWING APPLIES				
2C.1. SHEATHING IS GYPSUM BOARD OR FIBER BOARD	---	---	---	
2C.2. SHEATHING IS WOOD STRUCTURAL PANEL OR STEEL SHEETS ON ONE SIDE ONLY AND FASTENER SPACING IS MORE THAN 4" O.C.	---	---	---	
3. WIND RESISTING COMPONENTS:				
3A. ROOF CLADDING	IBC SECTION 1705.11.3	---	X	
3B. WALL CLADDING	---	---	X	

SHEET NOTES:
1) CONTRACTOR MAY ADHERE TO LOCAL AHJ REQUIREMENTS FOR SPECIAL INSPECTIONS PROVIDED THAT NONE OF THE REQUIREMENTS HEREIN ARE NEGLECTED.

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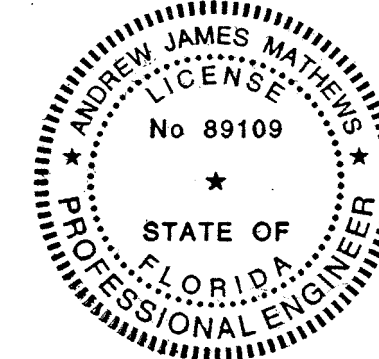
JACKIE ROBINSON TRAINING COMPLEX
EXECUTIVE BUILDING

3951 26TH STREET
VERO BEACH, FLORIDA 32960

Project No. 21081.1
Drawn By LEB
Checked By GRG
Date 02.10.2023

Revisions:

ANDREW J. MATHEWS
LICENSE NO. 89109

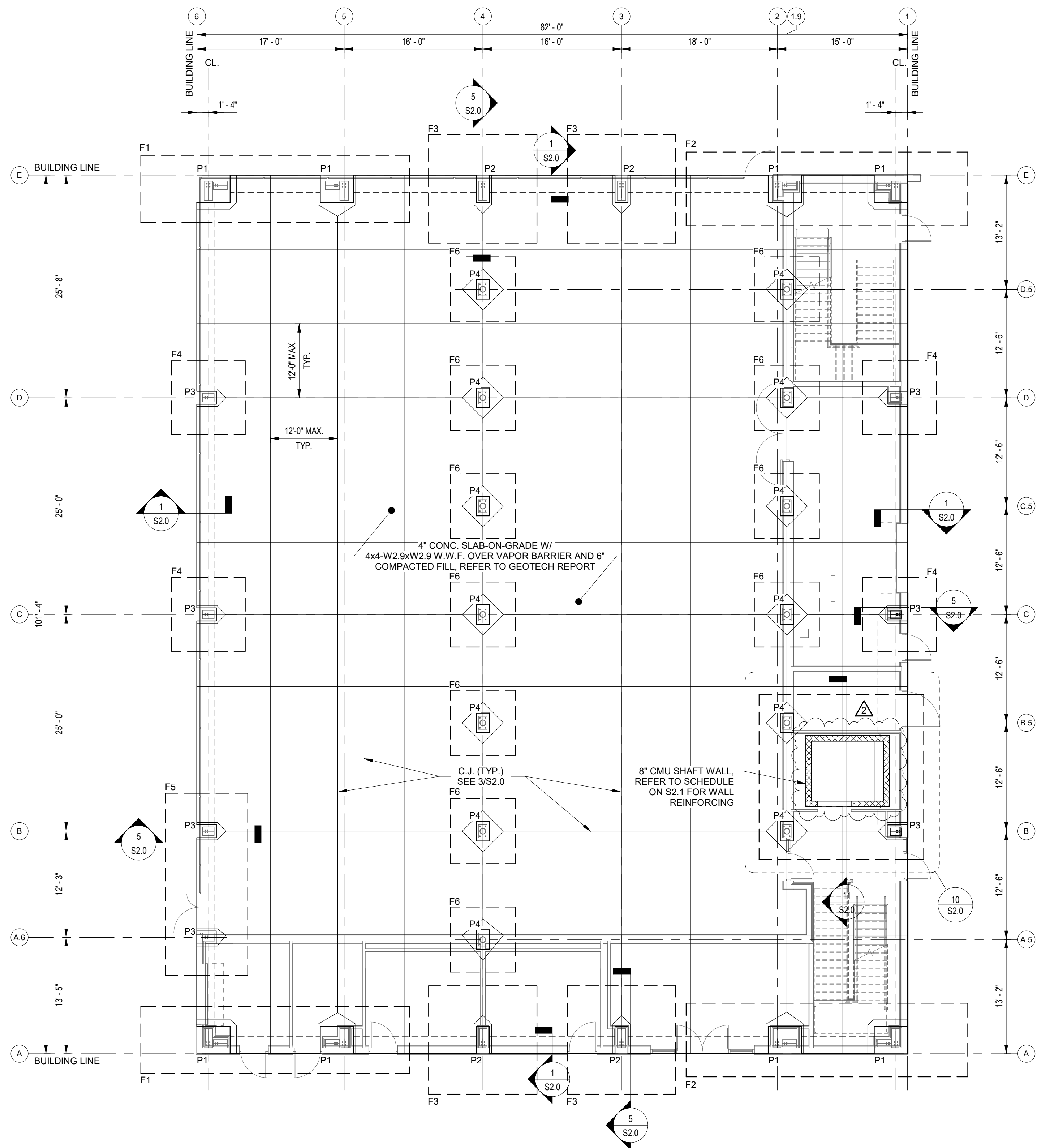


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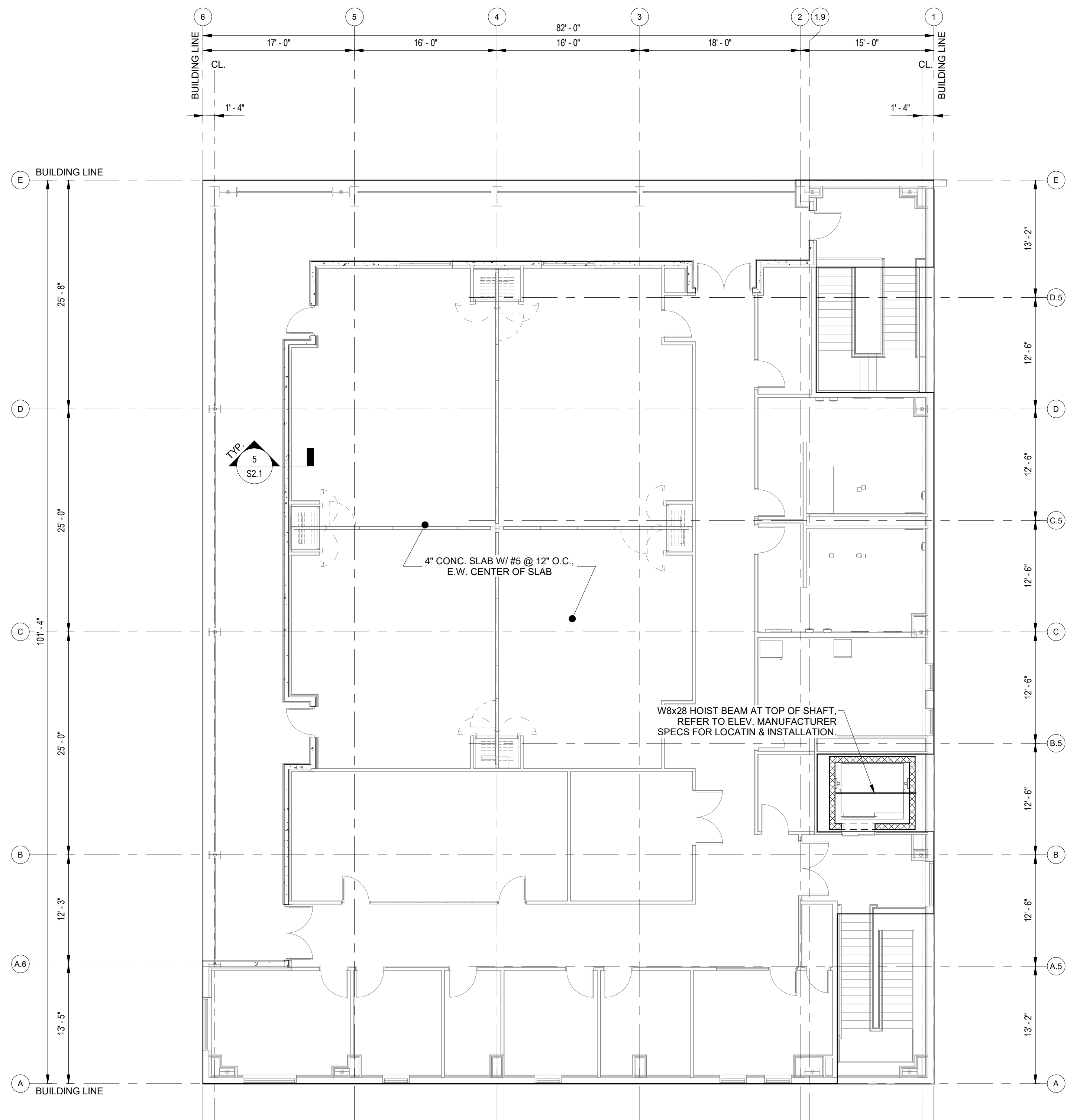
CONSTRUCTION DOCUMENT SET

S0.1
SPECIAL INSPECTIONS

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1 FOUNDATION PLAN
1/8" = 1'-0"



2 MEZZANINE PLAN
1/8" = 1'-0"

- NOTES:**
- SEE CIVIL DWG. FOR F.F.E.
 - SLAB CONTROL JOINTS NOT TO EXCEED AND AREA OF 200 SQ. FEET IN AS SQUARE A PATTERN AS POSSIBLE.
 - SEE GENERAL NOTES FOR INFO NOT SHOWN ON PLAN.
 - SEE ARCH. DWG. FOR DIMENSIONS NOT SHOWN.
 - REFER TO GEOTECH REPORT FOR SUBGRADE FILL REQUIREMENTS.

MARK	WIDTH	LENGTH	THK.	DEPTH	REINFORCEMENT	REMARKS
F1	7'-9"	31'-0"	30"	36"	(10) #7 TOP & BOT. LONG., (38) #7 TOP & BOT. TRANS.	A, C
F2	8'-6"	32'-6"	30"	36"	(11) #7 TOP & BOT. LONG., (44) #7 RANS. TOP & BOT.	A, C
F3	12'-6"	12'-6"	30"	36"	(8) #7 TOP & BOTTOM E.W.,	A
F4	8'-6"	8'-6"	18"	18"	(8) #7 TOP & BOTTOM E.W.	A
F5	9'-6"	21'-0"	18"	18"	(9) #7 TOP & BOT. LONG., (17) #7 OC TOP & BOT. TRANS.	A, D
F6	7'-6"	7'-6"	18"	18"	(6) #7 BOT. E.W.	A

- REMARKS:**
- TERMINATE ALL BARS WITH ACI STANDARD HOOKS.
 - TERMINATE TOP BARS ONLY WITH ACI STANDARD HOOKS.
 - TOP TRANS BARS ONLY NEEDED WITHIN 3'-10" OF PEDESTAL.
 - TOP TRANS BARS ONLY NEEDED WITHIN 2'-4" OF PEDESTAL.
 - DEPTH IS IN REFERENCE TO DIMENSION BELOW F.F.E. (±0'-0") TO TOP OF FOOTING

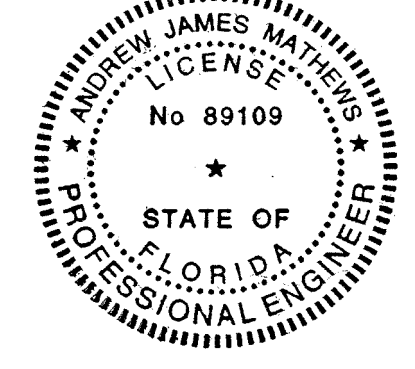
**JACKIE ROBINSON TRAINING COMPLEX
EXECUTIVE BUILDING**

3951 26TH STREET
VERO BEACH, FLORIDA 32960

Project No. 21081.1
Drawn By LEB
Checked By GRG
Date 02-10-2023

Revisions:
2 ADDENDUM 2 04-20-23

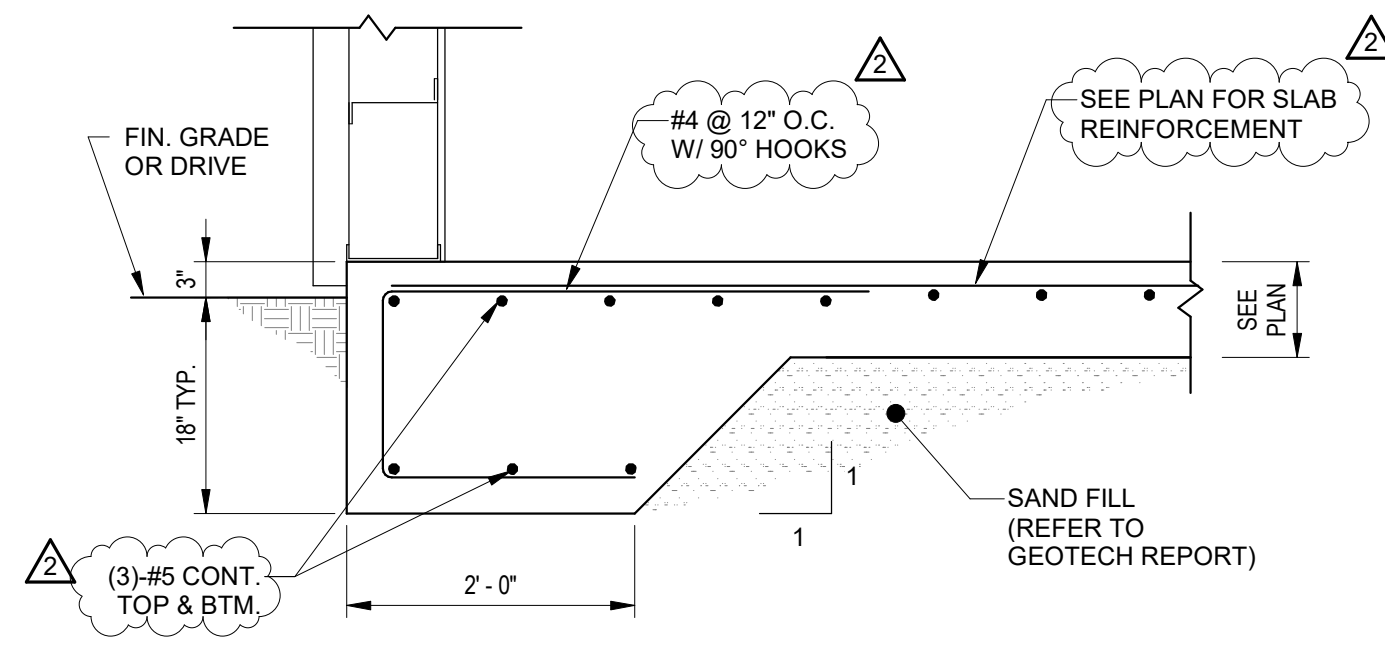
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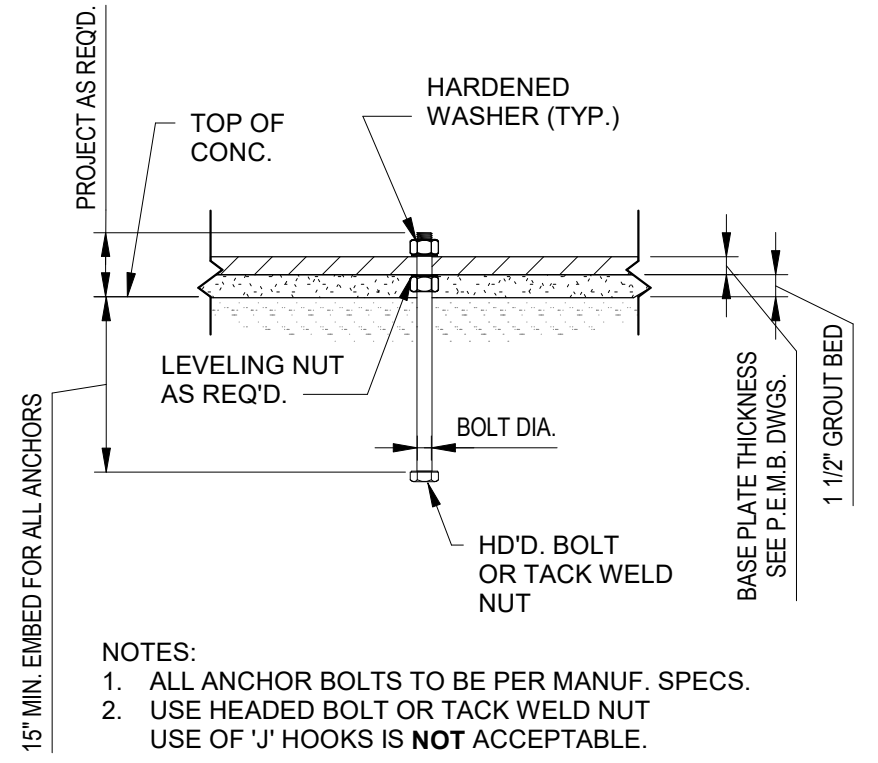
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CONSTRUCTION DOCUMENT SET

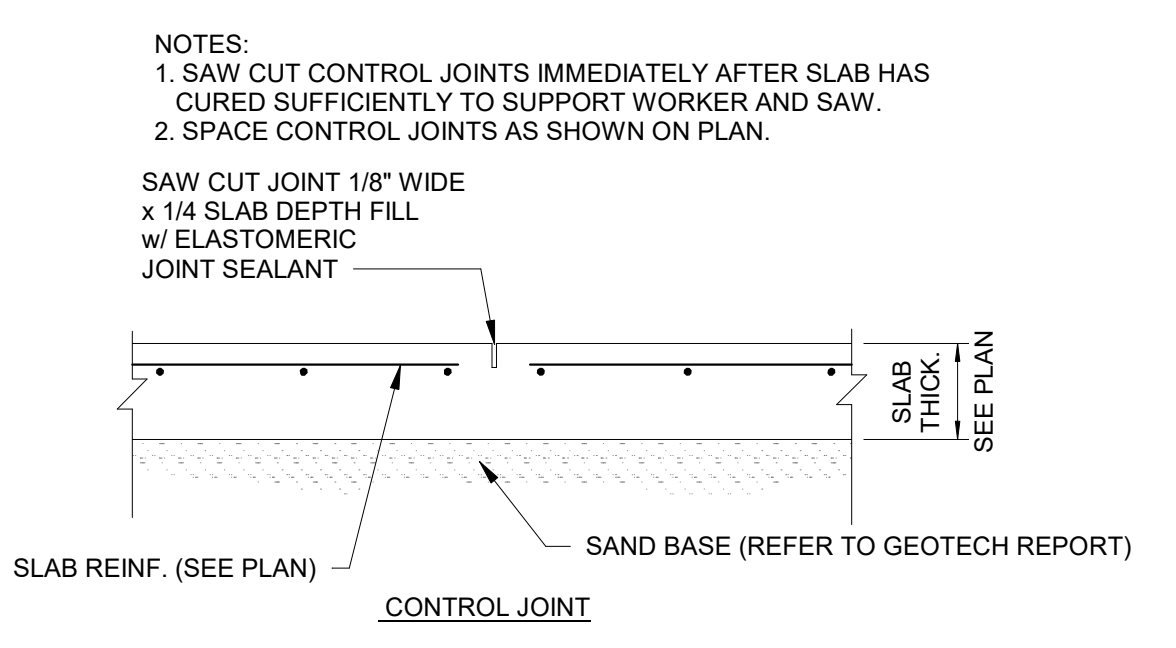
S1.0
FOUNDATION & MEZZANINE PLANS



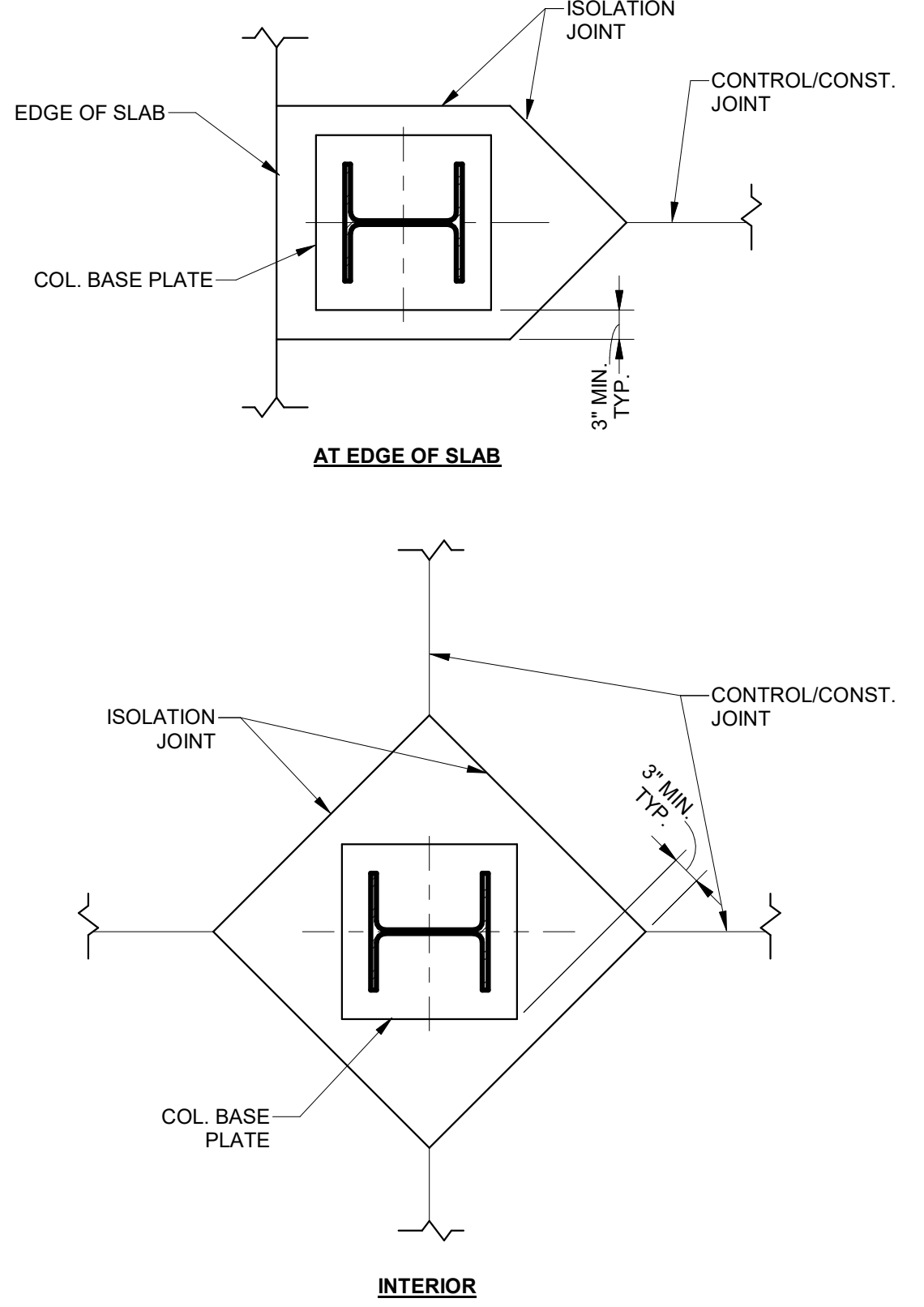
1 TYP. TURNED DOWN SLAB
3/4" = 1'-0"



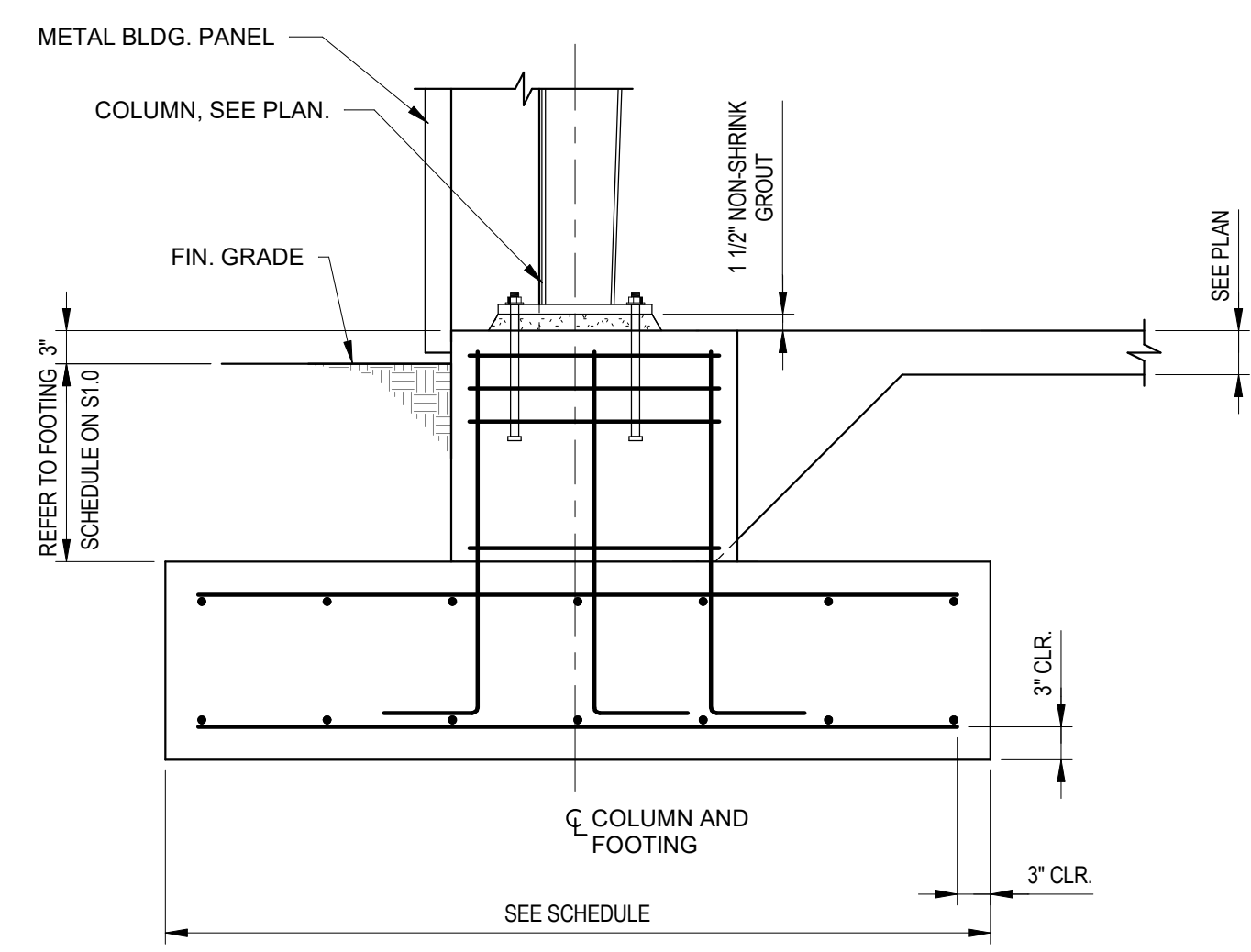
2 METAL BLDG. ANCHOR BOLT DETAIL
3/4" = 1'-0"



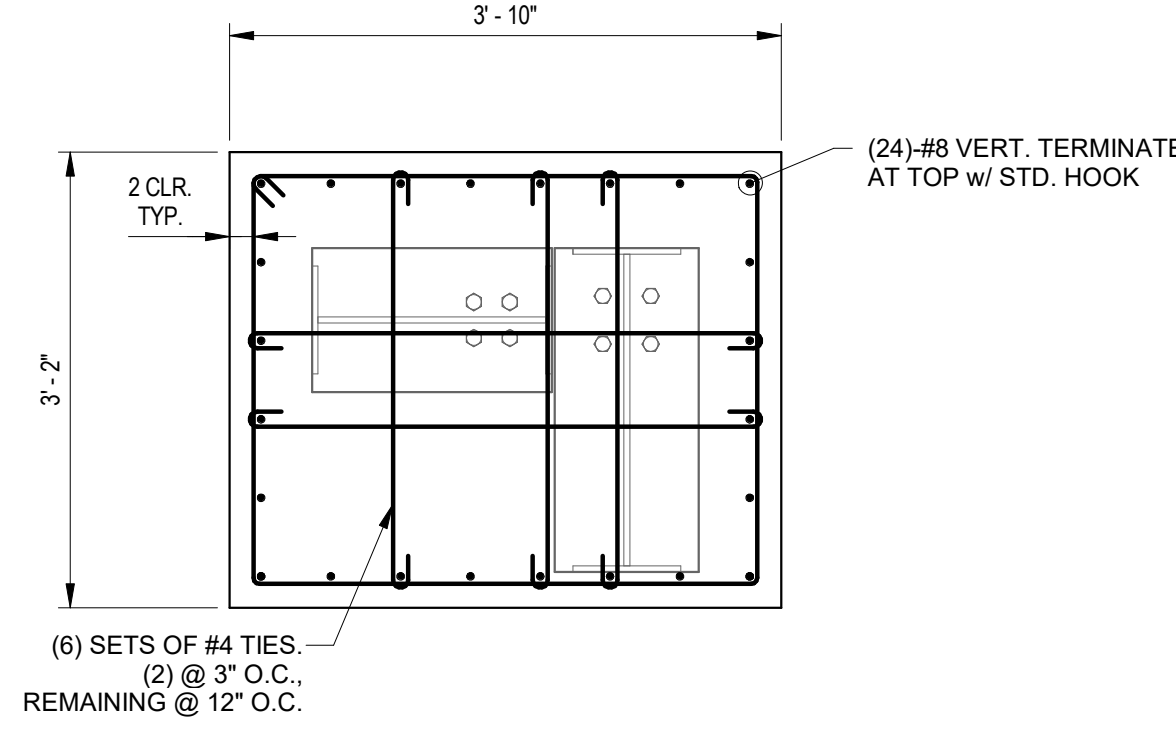
3 TYP. SLAB ON GRADE JOINTS
3/4" = 1'-0"



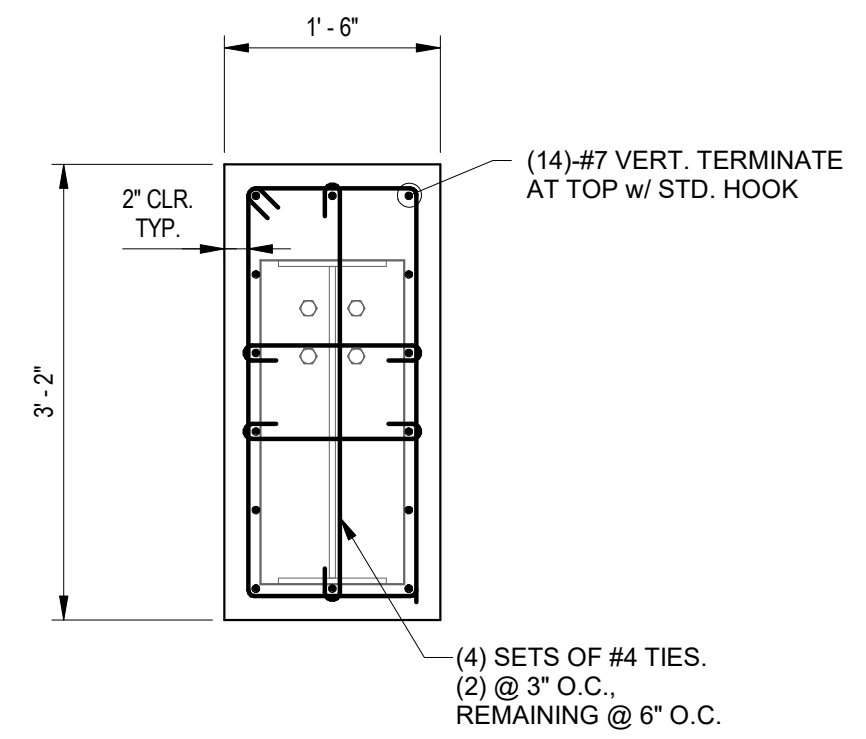
4 TYPICAL ISOLATION JOINTS
3/4" = 1'-0"



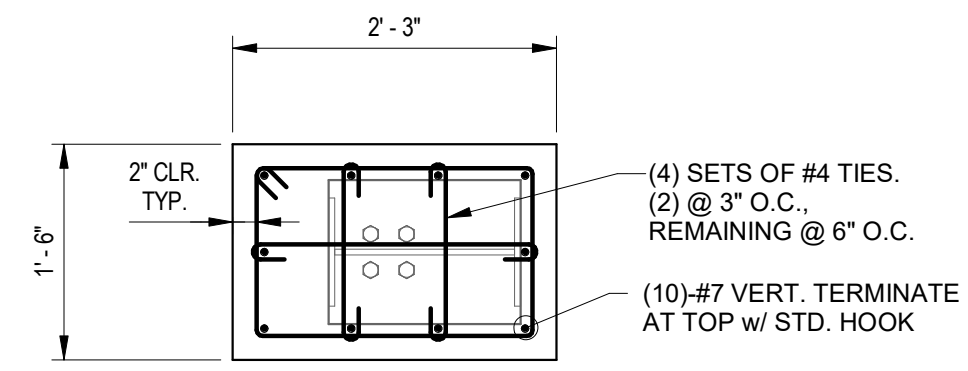
5 TYPICAL COLUMN FOOTING
3/4" = 1'-0"



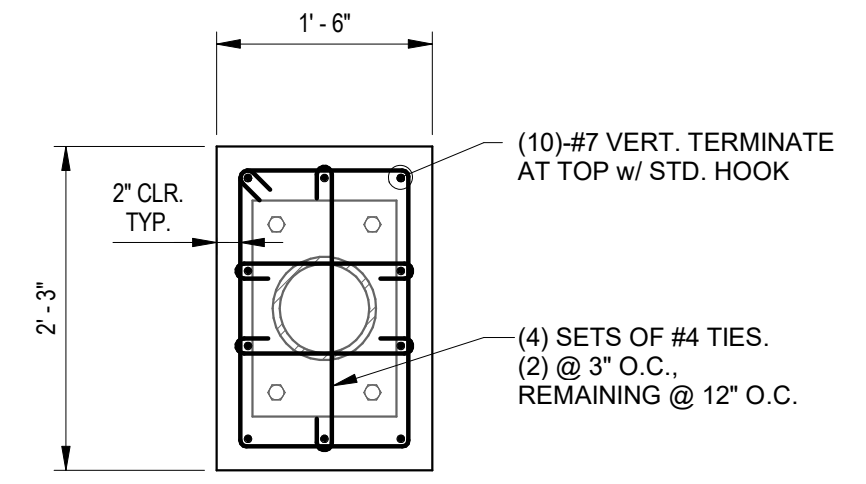
6 PEDESTAL P1
3/4" = 1'-0"



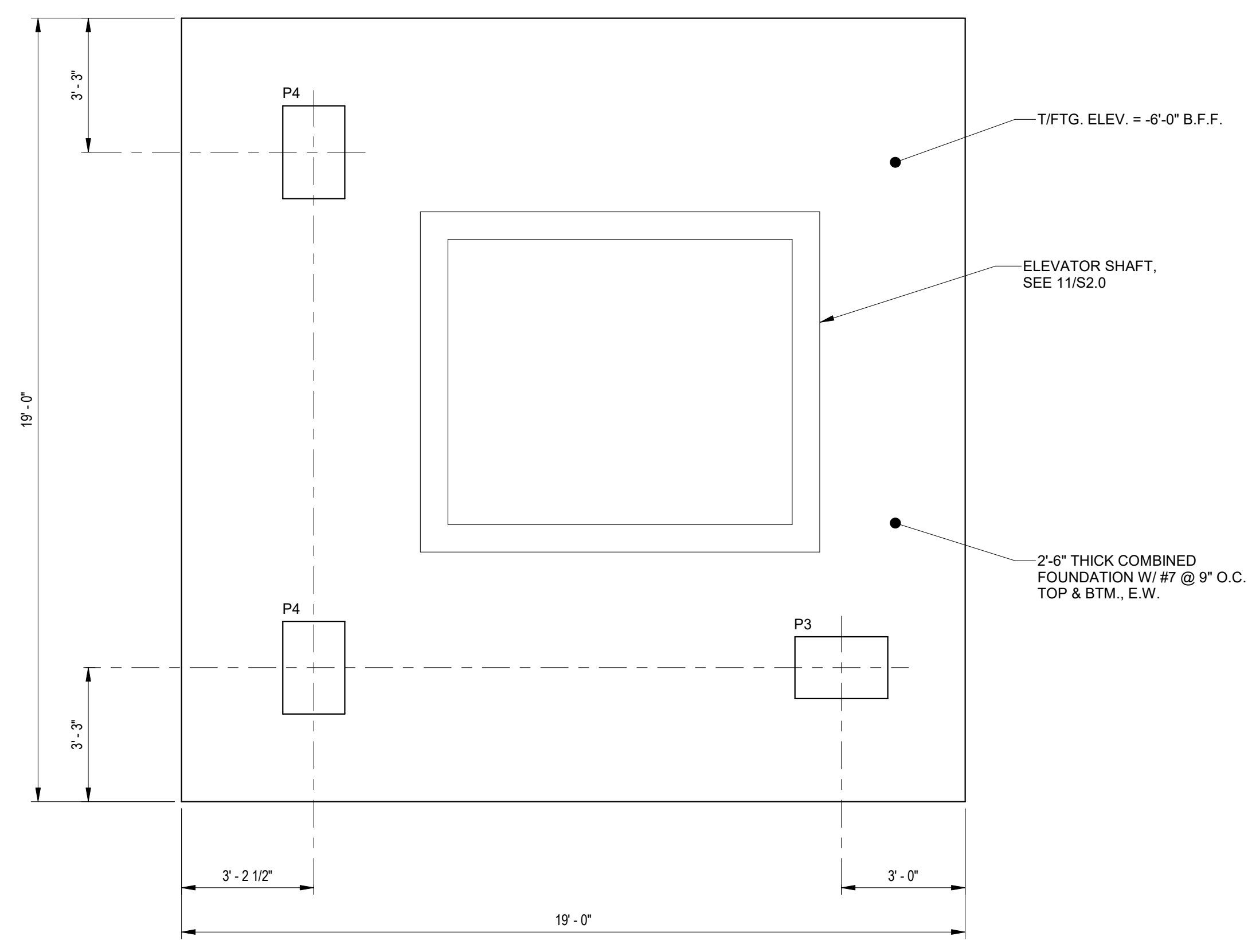
7 PEDESTAL P2
3/4" = 1'-0"



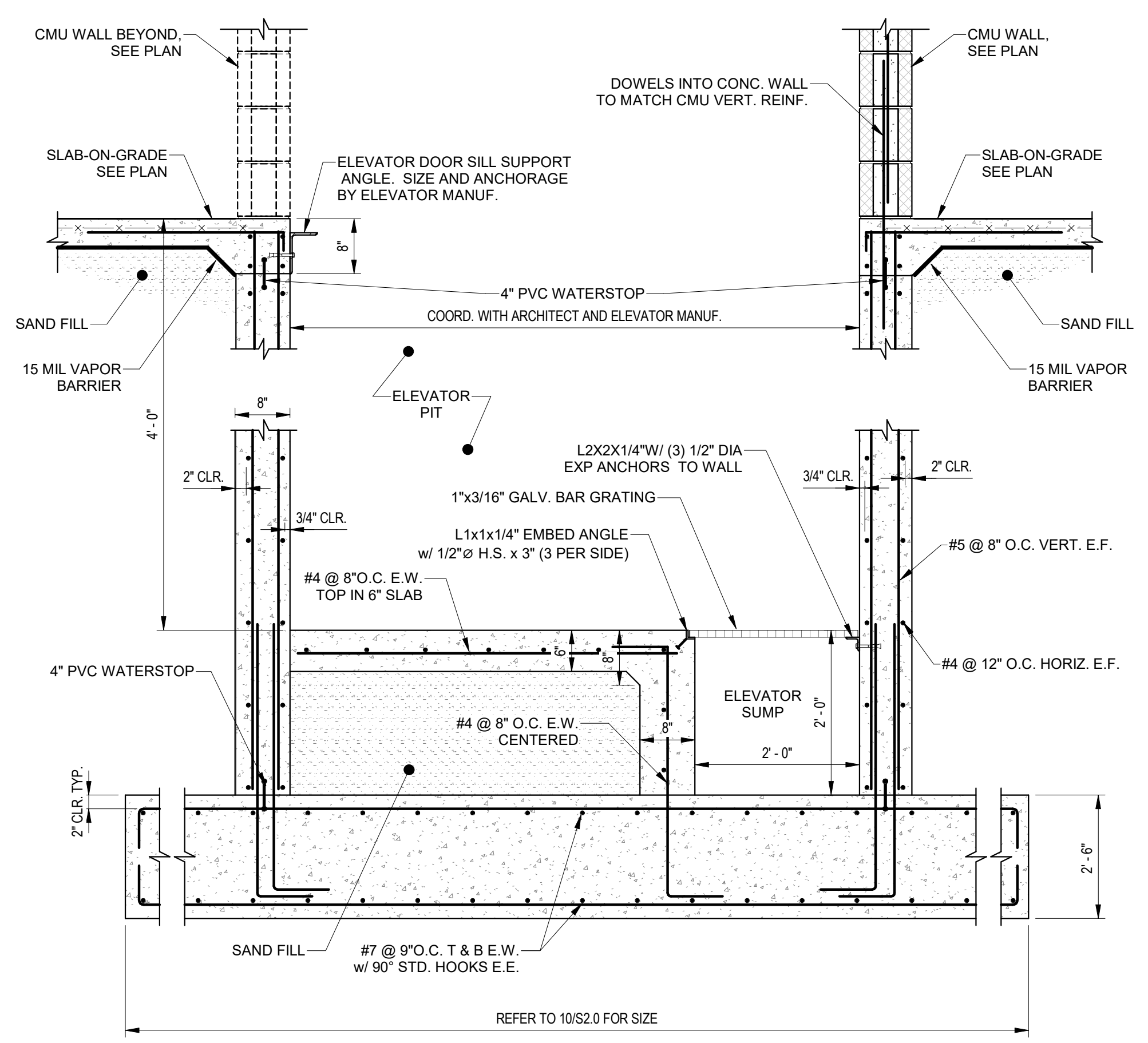
8 PEDESTAL P3
3/4" = 1'-0"



9 PEDESTAL P4
3/4" = 1'-0"



10 ELEVATOR MAT FOUNDATION
3/8" = 1'-0"

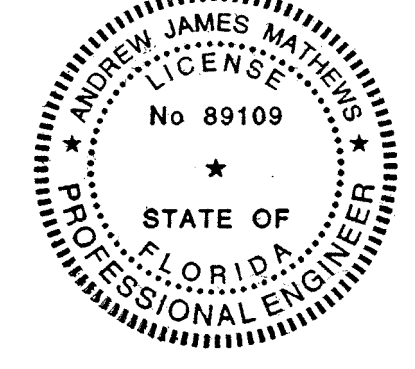


11 ELEVATOR SECTION
3/4" = 1'-0"

Project No.	21081.1
Drawn By	LEB
Checked By	GRG
Date	02.10.2023

Revisions:	
2	ADDENDUM 2 04.20.23

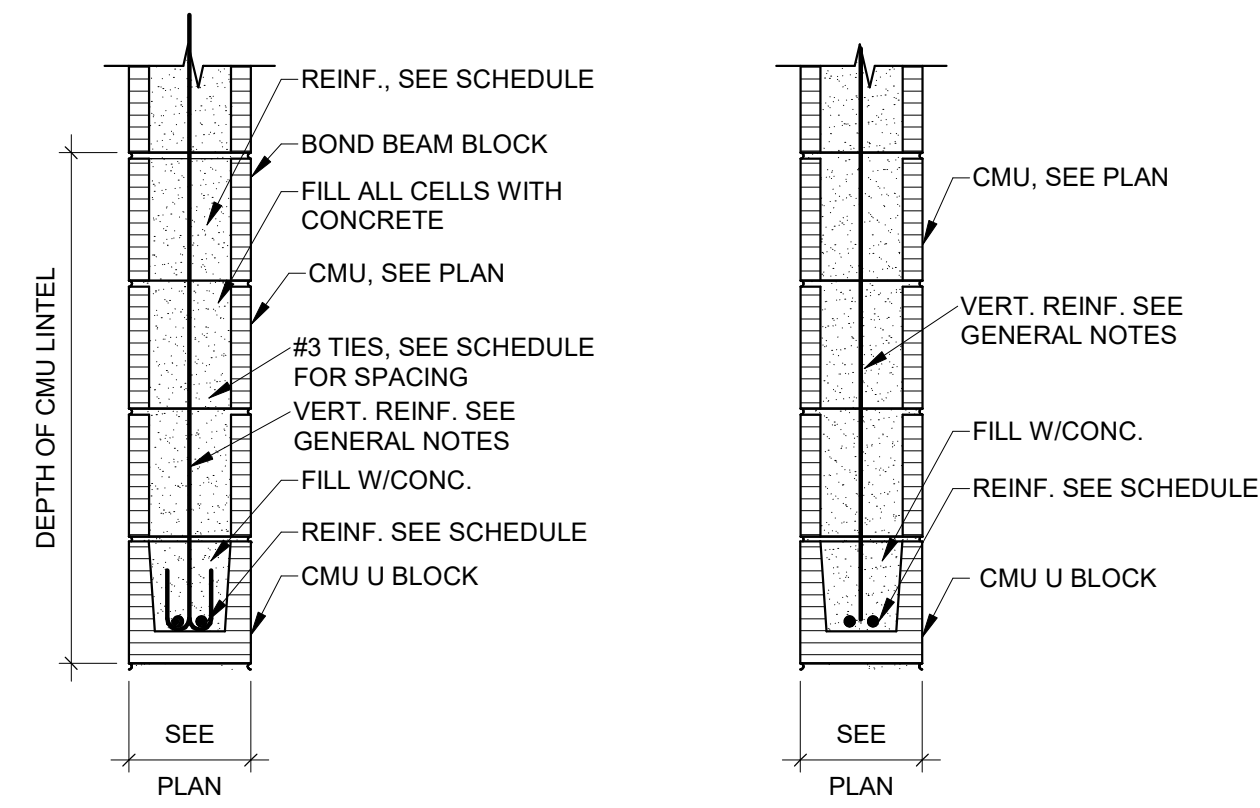
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CONSTRUCTION DOCUMENT SET

S2.0
FOUNDATION SECTIONS AND DETAILS

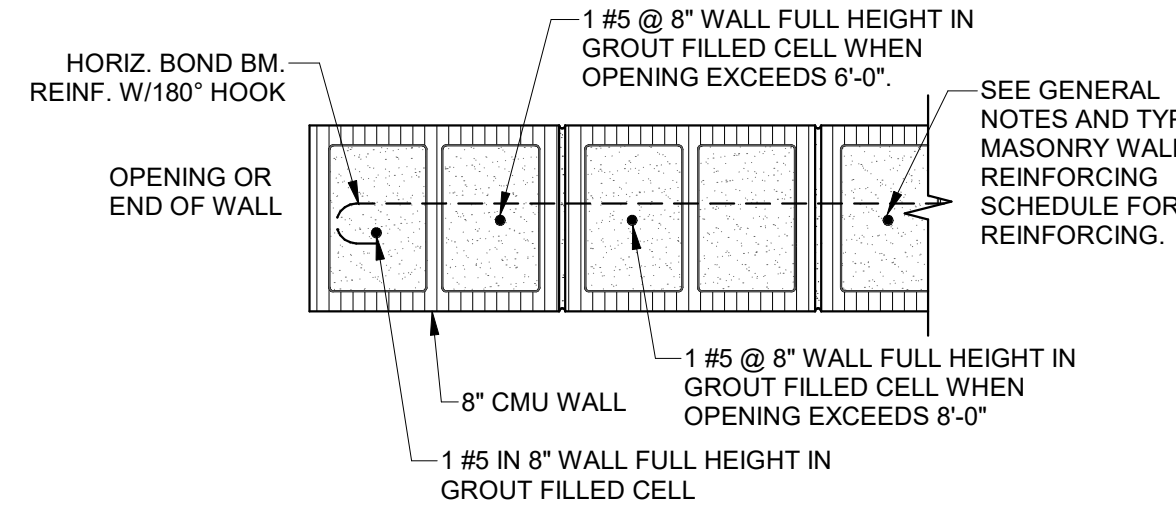


MASONRY OPENING MORE THAN 5'-0" (SEE SCHEDULE FOR SIZE AND REINFORCING)
MASONRY OPENING 5'-0" OR LESS.

MASONRY LINTEL SCHEDULE					
CLEAR SPAN	DEPTH	BOTTOM REINFORCING	TOP REINFORCING	#3 TIE SPACING	JAMB REINF. FULL HEIGHT (1) BAR PER CELL
< 5'-0"	8"	2 - #5	-	-	1 - #5

- NOTES:**
1. FOR OPENING LOCATIONS SEE ARCHITECTURAL DRAWINGS.
 2. PROVIDE 8" BEARING EACH SIDE OF OPENING.
 3. EXTEND MASONRY REINFORCING IN LINTEL 24" PAST OPENING, MIN.

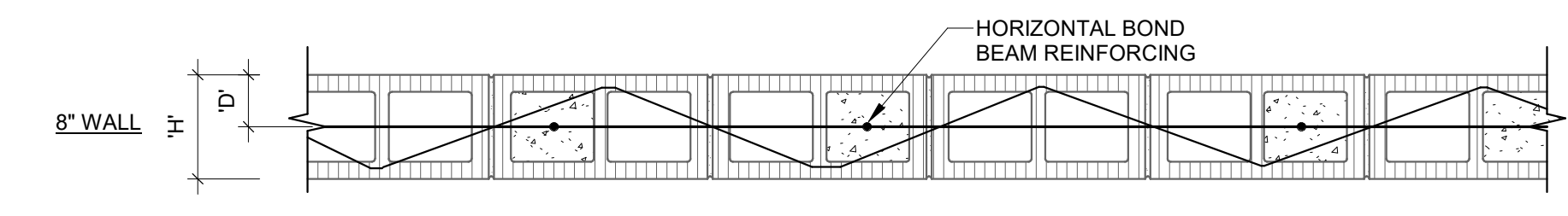
1 MASONRY LINTEL SCHEDULE
1" = 1'-0"



MASONRY LAP SPLICE LENGTHS	
BAR SIZE	MINIMUM LAP SPLICE LENGTH, in (mm)
#3 (#10)	12 (305)
#4 (#13)	15 (381)
#5 (#16)	23 (584)
#6 (#19)	43 (1092)

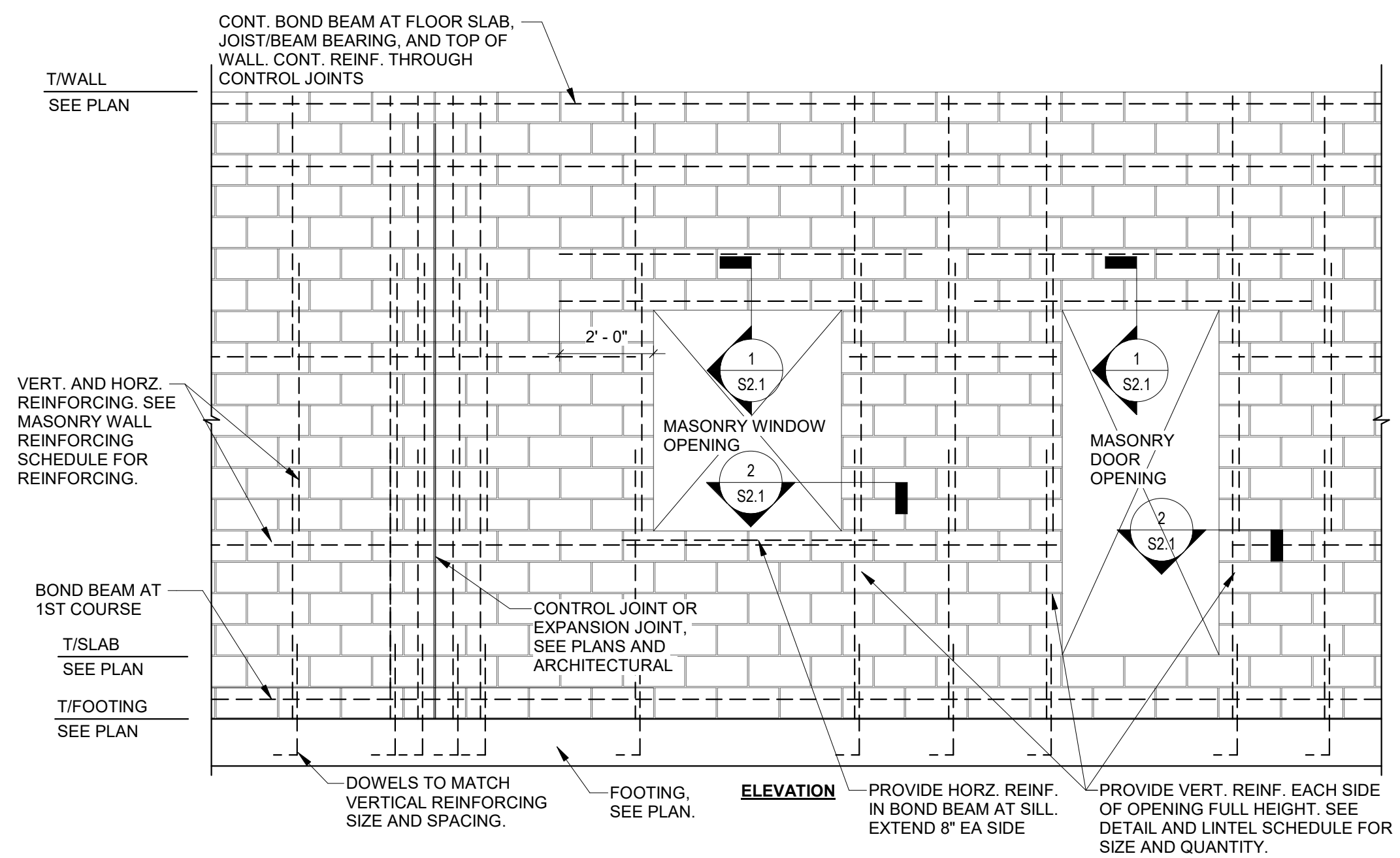
2 MASONRY LAP SPLICE SCHEDULE
1" = 1'-0"

MASONRY WALL REINFORCING SCHEDULE					
WALL TYPE	'H'	'D'	VERTICAL STEEL	HORIZONTAL STEEL	GROUT
8" EXT. CMU WALLS	8"	4"	#5 @ 24" O.C. VERTICAL	1-#5 @ 4'-0" O.C. & STD. CONT. LADDER REINFORCING @ 16" O.C.	@ REINFORCING
8" INT. CMU WALLS	8"	4"	#5 @ 24" O.C. VERTICAL	1-#5 @ 4'-0" O.C. & STD. CONT. LADDER REINFORCING @ 16" O.C.	@ REINFORCING



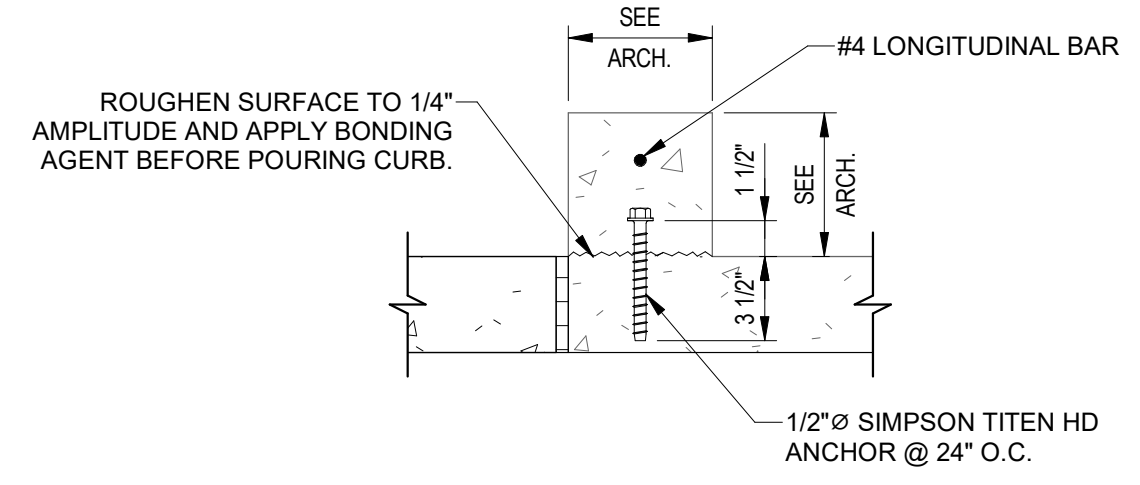
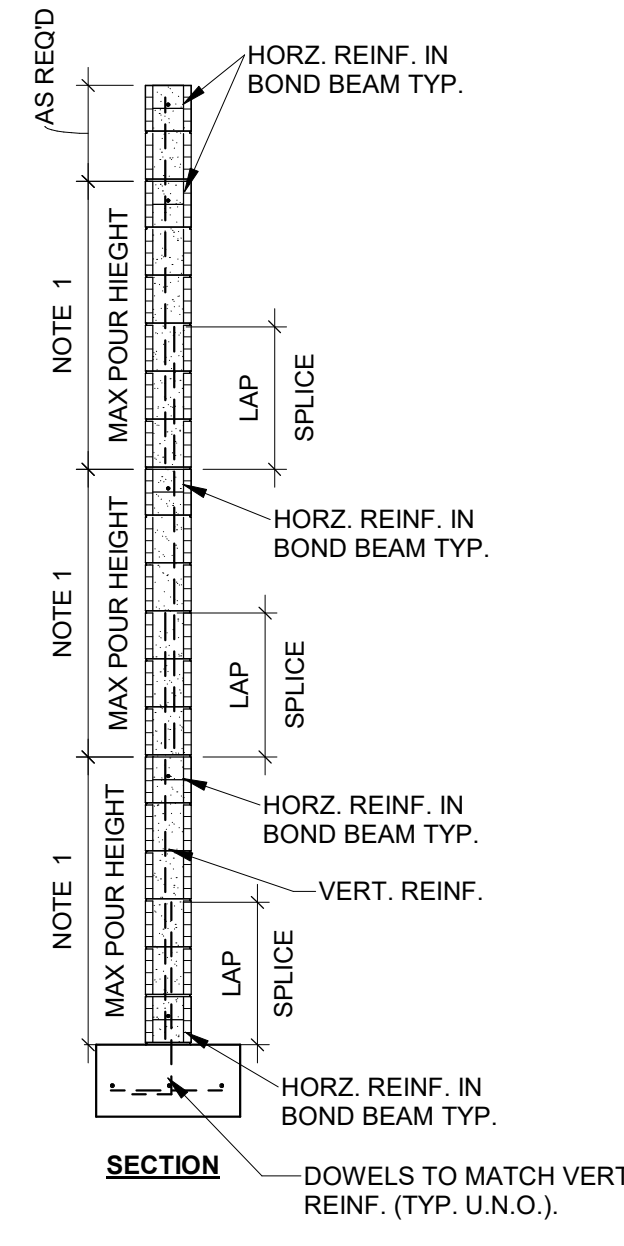
- NOTES:**
1. PROVIDE VERTICAL JOINTS @ MAXIMUM 20'-0" OR AS SHOWN ON PLAN.
 2. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS AND TYPES OF CMU WALLS.
 3. 'H' & 'D' ARE NOMINAL DIMENSIONS.
 4. STANDARD CONTINUOUS REINFORCING IS #9 DEFORMED SIDE & TRUSS WIRE.
 5. PROVIDE #5 VERTICAL BARS (FULL HEIGHT & FULLY GROUTED) ON EACH SIDE OF OPENINGS.
 6. HORIZONTAL STEEL REINFORCING SHALL HAVE STD. 180° HOOKS AT EDGE OF OPENINGS, END OF WALLS, AT EXP. JOINTS, & CONTROL JOINTS IN WALLS.

3 MASONRY WALL REINFORCING SCHEDULE
1" = 1'-0"



- NOTES:**
1. 4'-0" MAXIMUM POUR HEIGHT FOR GROUTING OF CMU WALLS.
 2. DETAIL VERTICAL REINFORCING WITH LAP SPLICES AT MAX. GROUT POUR HEIGHT.
 3. VERTICAL LAP SPLICE LENGTH SHALL BE AS SHOWN IN THE MASONRY LAP SPLICE SCHEDULE 2/S2.1.

4 TYPICAL MASONRY WALL REINFORCING DETAILS
3/8" = 1'-0"



- NOTE:**
1. CONTRACTOR SHALL NOT CUT ANY EXISTING REINFORCING.

5 TYPICAL CURB
1 1/2" = 1'-0"

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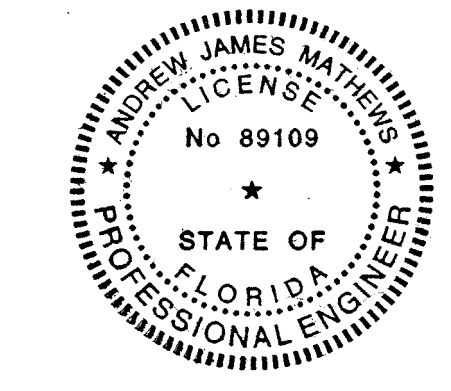
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CONSTRUCTION DOCUMENT SET

S2.1
MASONRY & MEZZANINE SECTIONS AND DETAILS

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