

STRUCTURAL NOTES

MISCELLANEOUS

- 1. THE STRUCTURAL SYSTEM IS UNSTABLE UNTIL ALL CONNECTIONS HAVE BEEN MADE AND ALL CONCRETE HAS REACHED ITS MINIMUM DESIGN STRENGTH...
2. CONTRACTOR IS RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION TO ENSURE THE SAFETY OF THE BUILDING UNTIL STRUCTURAL SYSTEM IS COMPLETED...
3. CONTRACTOR TO SUPPORT, BRACE AND SECURE EXISTING STRUCTURE AS REQUIRED...
4. APPLICABLE BUILDING CODE: FLORIDA BUILDING CODE 8th EDITION (2020) - BUILDING...
5. ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE APPLICABLE BUILDING CODE...
6. DESIGN LOADS (U.N.D.): AREA ROOF SUPERIMPOSED LIVE LOAD 20 PSF...
7. DESIGN WIND VELOCITY = 144 MPH (ULTIMATE) 112 MPH (EVALUATED)...
8. COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS...
9. CONTACT ENGINEER WITH ANY QUESTIONS OR DISCREPANCIES FOUND ON DRAWINGS...
10. SUBMIT SHOP DRAWINGS AS REQUIRED HEREIN...
11. CONTRACTOR SHALL NOT BE RELIEVED FROM RESPONSIBILITY FOR ERRORS OR OMISSIONS...
12. SUBMIT ONE PRINT OF ALL SHOP DRAWINGS...
13. CONTRACTOR SHALL NOTIFY THIS OFFICE WHEN THE STRUCTURAL SYSTEM IS SUBSTANTIALLY COMPLETED...
14. SITE WORK
15. A SUBSURFACE INVESTIGATION HAS BEEN COMPLETED AT THE PROJECT SITE...
16. ALL SITE WORK SHALL BE DONE IN STRICT ACCORDANCE WITH THE PROJECT SOILS REPORT...
17. DESIGN SOIL BEARING CAPACITY = 2500 PSF...
18. A QUALIFIED TESTING LABORATORY SHALL BE RETAINED TO PERFORM THE FOLLOWING TESTS...
19. THE SIDES OF FOOTINGS MAY BE EARTH-FRAMED IF THE EXCAVATION CAN BE KEPT VERTICAL...
20. FOUNDATION WALLS THAT RETAIN EARTH SHALL BE BRACED AGAINST BACKFILLING PRESSURES...
21. CAST IN PLACE CONCRETE
22. CONCRETE TO BE NORMAL WEIGHT WITH THE FOLLOWING MINIMUM COMPRESSIVE STRENGTHS AT 28 DAYS...
23. REINFORCING STEEL: ASTM A615 GRADE 60...
24. WELD WIRE FABRIC: ASTM A-1064...
25. VERIFY THE EXACT SIZE AND LOCATION OF ALL FLOOR AND ROOF OPENINGS...
26. SHOP PAINT - METAL ALKOYD-OIL PRIMER...
27. SURFACE PREPARATION - PREPARE STEEL SURFACE...
28. A QUALIFIED TESTING LABORATORY SHALL BE RETAINED TO PERFORM THE FOLLOWING TESTS...
29. CONTRACTOR SHALL VERIFY LOCATIONS OF ALL OPENINGS, SLEEVES, AND SLAB RECESSES...
30. ALL CONCRETE SHALL BE CURED IMMEDIATELY AFTER FINISHING OPERATIONS...
31. GENERAL CONTRACTOR IS RESPONSIBLE FOR THE PROPER DESIGN AND CONSTRUCTION OF ALL FORMWORK...
32. A QUALIFIED TESTING LABORATORY SHALL BE RETAINED TO PERFORM THE FOLLOWING CONCRETE TESTS...
33. RESTRICT THE ADDITION OF MIX WATER AT THE JOB SITE...
34. MAXIMUM WATER TO CEMENT RATIO WHEN NO BACK-UP DATA IS AVAILABLE...
35. REINFORCING BAR COVER (UNLESS NOTED OTHERWISE)...
36. CONCRETE SHALL BE PLACED WITHIN 90 MINUTES OF BATCH TIME...
37. WHERE BAR LENGTHS ARE GIVEN ON DRAWINGS, LENGTH OF HOOK, IF REQUIRED, IS NOT INCLUDED...
38. PROVIDE COMMERCIAL FORM COATING COMPOUNDS THAT WILL NOT BOND, STAIN, OR ADVERSELY AFFECT CONCRETE SURFACES...
39. ALL CONCRETE SHALL BE CONSOLIDATED IN PLACE USING INTERNAL VIBRATORS...
40. REPAIR AND PATCH DEFECTIVE AREAS WITH CEMENT MORTAR IMMEDIATELY AFTER REMOVAL OF FORMS...
41. PROVIDE 3/4" CHAMFERS ON ALL EXPOSED CORNERS OF COLUMNS, BEAMS, AND WALLS...
42. SUBMITTALS: PROVIDE CORNER BARS AT ALL BEAM AND WALL FOOTING CORNERS TO MATCH HORIZONTAL BARS...
43. SUBMITTALS:

- 20. ALL BUILDING AND SITE SLABS-ON-GRADE SHALL BE AT LEAST 5" THICK, REINFORCED WITH 6x6-40 9 x 42 9 x 4 W.F. ...
21. STEP AND SLOPE ALL WALKWAYS AWAY FROM THE BUILDING...
24. TILT-UP CONCRETE PANELS
25. REFER TO CAST-IN-PLACE CONCRETE FOR ALL ADDITIONAL REQUIRED INFORMATION...
26. CONCRETE TO BE NORMAL WEIGHT WITH A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS...
27. MINIMUM CONNECTION STRENGTH AT LIFTING TO BE 2500 PSI UNLESS SPECIFIED OTHERWISE...
28. SEE DRAWINGS FOR PANEL THICKNESS...
29. ERECTOR SHALL HAVE AT LEAST TWO (2) YEARS OF EXPERIENCE IN ERECTION OF TILT-UP WALL PANELS...
30. LOCATIONS OF LIFTING INSERTS AND ANY ADDITIONAL REINFORCEMENT OR STRONGBACKS...
31. COORDINATE INSTALLATION OF INSERTS AND ANCHORAGES...
32. CAST PANELS INDIVIDUALLY ON BUILDING FLOOR SLAB...
33. SHOP DRAWINGS: SUBMIT SHOP DRAWINGS INDICATING PANEL DIMENSIONS, OPENINGS...
34. PANELS SHALL BE CAST WITH INTERIOR FACE UP...
35. MINIMUM REBAR COVER = 1-1/2 INCHES...
36. SEE ARCHITECTURAL DRAWINGS FOR CHAMFERS, REVEALS, AND ANY EMBEDDED ITEMS...
37. PROVIDE 3/4" CHAMFERS ON ALL EXPOSED CORNERS OF COLUMNS, BEAMS, AND WALLS...
38. PANELS SHALL BE ERECTED AND BRACED BY A COMPETENT AND EXPERIENCED ERECTION CREW...
39. LIFT PANELS IN ACCORD WITH RECOMMENDATIONS OF LIFTING HARDWARE SUPPLIER...
40. PANELS SHALL BE ACCURATELY SET AND PLUMBED...
41. AFTER PLACING, PROVIDE TEMPORARY BRACES AND SUPPORTS TO SECURELY HOLD PANELS...
42. PATCH HOLES IN PANEL SURFACES CAUSED BY LIFTING AND BRACING DEVICES...
43. STRUCTURAL STEEL
44. STRUCTURAL STEEL SHALL CONFORM TO THE AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS"...
45. WELDED CONNECTIONS SHALL CONFORM TO THE LATEST REVISED CODE OF THE AMERICAN WELDING SOCIETY...
46. ALL CONNECTIONS TO BE DOUBLE ANGLE FRAMED BEAM CONNECTION PER AISC...
47. SIZE AND USE OF HOLES: SEE AISC TABLE J3.3 U.N.D...
48. ALL STEEL BEAMS SHALL BE FABRICATED WITH THE NATURAL CORNER WITHIN THE WALL TOLERANCE...
49. VERIFY THE EXACT SIZE AND LOCATION OF ALL FLOOR AND ROOF OPENINGS...
50. SHOP PAINT - METAL ALKOYD-OIL PRIMER...
51. MANUFACTURER DESIGNATION: PORTER NE 258, MERBLE NE 13F812, THINCKE NE 1009, AMERON NE 5102 AMERDANT...
52. SHOP PAINT ALL SURFACES OF STEEL DEEPER ANCHOR BOLTS AND SURFACES TO BE FIELD WELDED...
53. SURFACE PREPARATION - PREPARE STEEL SURFACE...
54. A QUALIFIED TESTING LABORATORY SHALL BE RETAINED TO PERFORM THE FOLLOWING TESTS...
55. CONTRACTOR SHALL VERIFY LOCATIONS OF ALL OPENINGS, SLEEVES, AND SLAB RECESSES...
56. ALL CONCRETE SHALL BE CURED IMMEDIATELY AFTER FINISHING OPERATIONS...
57. GENERAL CONTRACTOR IS RESPONSIBLE FOR THE PROPER DESIGN AND CONSTRUCTION OF ALL FORMWORK...
58. A QUALIFIED TESTING LABORATORY SHALL BE RETAINED TO PERFORM THE FOLLOWING CONCRETE TESTS...
59. RESTRICT THE ADDITION OF MIX WATER AT THE JOB SITE...
60. MAXIMUM WATER TO CEMENT RATIO WHEN NO BACK-UP DATA IS AVAILABLE...
61. REINFORCING BAR COVER (UNLESS NOTED OTHERWISE)...
62. CONCRETE SHALL BE PLACED WITHIN 90 MINUTES OF BATCH TIME...
63. WHERE BAR LENGTHS ARE GIVEN ON DRAWINGS, LENGTH OF HOOK, IF REQUIRED, IS NOT INCLUDED...
64. PROVIDE COMMERCIAL FORM COATING COMPOUNDS THAT WILL NOT BOND, STAIN, OR ADVERSELY AFFECT CONCRETE SURFACES...
65. ALL CONCRETE SHALL BE CONSOLIDATED IN PLACE USING INTERNAL VIBRATORS...
66. REPAIR AND PATCH DEFECTIVE AREAS WITH CEMENT MORTAR IMMEDIATELY AFTER REMOVAL OF FORMS...
67. PROVIDE 3/4" CHAMFERS ON ALL EXPOSED CORNERS OF COLUMNS, BEAMS, AND WALLS...
68. SUBMITTALS: PROVIDE CORNER BARS AT ALL BEAM AND WALL FOOTING CORNERS TO MATCH HORIZONTAL BARS...
69. SUBMITTALS:

- 3. SEE STANDARD JOIST SPECIFICATIONS FOR CAMBER REQUIREMENTS...
4. VERIFY THE EXACT LOCATION AND WEIGHT OF ALL MECHANICAL EQUIPMENT...
5. SUBMIT DETAILED SHOP DRAWINGS OF REINFORCING BARS SHOWING NUMBER, SIZE, AND LOCATION...
6. PROVIDE BOTTOM CHORD CEILING EXTENSIONS WHERE REQUIRED BY ARCHITECT...
7. JOIST MANUFACTURER SHALL DESIGN ROOF JOISTS AND BRIDGING FOR THE NET WIND LOAD...
8. WHERE COLUMNS ARE NOT FRAMED IN AT LEAST TWO DIRECTIONS WITH STRUCTURAL STEEL...
9. JOIST BRIDGING SHALL BE FURNISHED AND INSTALLED TO MEET THE DESIGN AND SPACING REQUIREMENTS...
10. RIGID X-BRIDGING AS SHOWN ON THE PLANS SHALL BE BOLTED OR WELDED AT THE INTERSECTION...
11. JOISTS, JOIST GIRDERS, AND ACCESSORIES SHALL HAVE ONE COAT OF PAINT MEETING THE MINIMUM PERFORMANCE REQUIREMENTS...
12. SEE PLAN FOR ANY CONCENTRATED LOADS OR UNUSUAL CONDITIONS...
13. SUBMITTALS: CONTRACTOR SHALL SUBMIT DETAILED SHOP DRAWINGS SHOWING JOISTS, BRIDGING, AND ALL CONNECTIONS...
14. JOIST DESIGN ENGINEER MUST BE PROVIDED WITH A COPY OF THESE DRAWINGS...
15. A QUALIFIED TESTING LABORATORY SHALL BE RETAINED TO VISUALLY INSPECT ALL JOIST JOIST ACCESSORIES, WELDS, AND CONNECTIONS...
30. CONTRACTOR TO SUBMIT THE FOLLOWING:
31. SUBMIT DETAILED SHOP DRAWINGS FOR STEEL FRAMING SHOWING THE TYPE AND SPACING OF ALL MEMBERS...
32. SUBMITTED SHOP DRAWINGS MUST BE CHECKED AND SIGNED BY THE GENERAL CONTRACTOR...
33. CARPENTRY
34. DIMENSIONED LUMBER SHALL BE DRESSED S4S, AND SHALL BEAR THE GRADE STAMP OF THE MANUFACTURER'S ASSOCIATION...
35. ALL LUMBER SHALL BE SOUND, SEASONED, AND FREE FROM WARP...
36. ALL LUMBER SHALL BE SOUTHERN PINE WITH 19% MAXIMUM MOISTURE CONTENT...
37. FOR STRUCTURAL LIGHT FRAMING (2 TO 4 INCHES THICK, 2 TO 4 INCHES WIDE) AND STRUCTURAL FRAMING (2 TO 4 INCHES THICK, 5 INCHES AND WIDER)...
38. ALL LUMBER IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESURE TREATED...
39. PRESSURE TREATED LUMBER SHALL BE IMPREGATED WITH AN ALC (ALKALINE COPPER QUATERNARY) TREATMENT...
40. PLYWOOD WALL AND ROOF SHEATHING SHALL BE APA RATED SHEATHING...
41. ALL NAILING AND BOLTING SHALL COMPLY WITH AMERICAN INSTITUTE OF TIMBER CONSTRUCTION REQUIREMENTS...

METAL DECKING

- 1. ALL METAL DECK SHALL CONFORM TO THE REQUIREMENTS OF THE STEEL DECK INSTITUTE...
2. WELDING CONNECTIONS SHALL CONFORM TO THE LATEST REVISED CODE OF THE AMERICAN WELDING SOCIETY...
3. METAL ROOF DECK SHALL BE 1 1/2" DEEP, 22 GA., WIDE RIB TYPE B AND GALVANIZED U.N.D...
4. MINIMUM FASTENING OF ROOF DECK WITH (7) 5/8" DIA. PIVOTED WELDS AT EA SUPPORT AND (3) #10 TEK SCREW SIDE LAP FASTENERS EQUALLY SPACED...
5. MINIMUM FASTENING AT BUILDING PERIMETER OF DECK SHALL BE 5/8" DIAMETER PIVOTED WELDS AT 4' O.C...
6. INSTALL ALL BRACING 3 SPAN CONTINUOUS...
7. DO NOT WARE OR ATTACH DUCTWORK, CONDUIT, PIPING, EQUIPMENT, CEILING, ETC. FROM METAL DECKING...
8. ALL ROOF DECK OPENINGS 12" DIAMETER OR LARGER ARE TO HAVE SUPPORT ANGLES PER TYPICAL DECK OPENING DETAIL...
9. SUBMITTALS: CONTRACTOR SHALL SUBMIT DETAILED SHOP DRAWINGS SHOWING LAYOUT OF DECK, TYPE OF DECK, ALL CONNECTIONS INCLUDING END WELDS, SEAM WELDS, INTERMEDIATE WELDS, AND ALL ACCESSORY MATERIAL...
10. A QUALIFIED TESTING LABORATORY SHALL BE RETAINED TO VISUALLY INSPECT ALL DECK WELDS AND FASTENERS...

COLD-FORM STEEL FRAMING

- 1. ALL STEEL FRAMING SHALL CONFORM TO "THE SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS"...
2. WELDED CONNECTIONS SHALL CONFORM TO THE LATEST REVISED CODE OF THE AMERICAN WELDING SOCIETY...
3. ASTM A-568 STANDARD SPECIFICATION FOR GENERAL REQUIREMENTS FOR STEEL, CARBON AND HIGH STRENGTH LOW-ALLOY HOT ROLLED SHEET AND COLD ROLLED SHEET...
4. ALL STEEL FRAMING SHALL BE INSTALLED BY PERSONNEL EXPERIENCED IN LIGHT GAUGE STEEL FRAMING INSTALLATION...
5. WHERE STEEL FRAMING MEMBERS ARE COMPONENTS OF ASSEMBLIES INDICATED FOR A FIRE-RESISTANT RATING...
6. PROTECT LIGHT GAUGE STEEL FRAMING MEMBERS FROM RUSTING AND DAMAGE...
7. WITH EACH TYPE OF STEEL FRAMING REQUIRED, PROVIDE MANUFACTURER'S STANDARD STEEL RUNNERS (TRACKS), BLOCKING, LINTELS, CLIP ANGLES, BRACING, REINFORCEMENTS, FASTENERS, AND ACCESSORIES AS RECOMMENDED BY MANUFACTURER...
8. FABRICATE METAL FRAMING COMPONENTS OF STRUCTURAL QUALITY SHEET STEEL WITH A MINIMUM YIELD POINT OF 50,000 PSI FOR STUDS...
9. SCREWS SHALL BE AS RECOMMENDED BY MANUFACTURER...
10. PROVIDE GALVANIZED FINISH TO METAL FRAMING COMPONENTS...
11. PROVIDE MANUFACTURER'S STANDARD STRUCTURAL "C" SHAPED STEEL STUDS...
12. ALL FRAMING MEMBERS SHALL BE DESIGNED BY THE MANUFACTURER TO SUPPORT ALL LIVE, DEAD, AND WIND LOADS...
13. THE EXTERIOR WALL SYSTEM SHALL BE DESIGNED TO WITHSTAND BOTH POSITIVE AND NEGATIVE WIND PRESSURE...
14. FRAMING COMPONENTS MAY BE PREFABRICATED INTO PANELS PRIOR TO ERECTION...
15. INSTALL METAL FRAMING SYSTEMS IN ACCORDANCE WITH MANUFACTURER'S PRINTED OR WRITTEN INSTRUCTIONS...
16. INSTALL CONTINUOUS TRACKS SIZED TO MATCH STUD DEPTH...
17. FRAME BOTH SIDES OF EXPANSION AND CONTROL JOINTS...
18. WHERE REQUIRED, TEMPORARY BRACING SHALL BE PROVIDED UNTIL ERECTION IS COMPLETED...
19. RESISTANCE TO BENDING AND RETENTION ABOUT THE MINOR AXIS SHALL BE PROVIDED BY MECHANICAL LATERAL BRACING...
20. ATTACHMENTS OF SIMILAR COMPONENTS SHALL BE DONE BY WELDING...
21. WELDING OF MEMBERS LIGHTER THAN 18 GAUGE SHALL NOT BE PERMITTED...
22. SPLICES SHALL NOT BE PERMITTED...
23. MINIMUM NUMBERS OF EQUALLY SPACED JOIST BRIDGING FOR THE SPANS SHOWN...
24. MINIMUM NUMBER OF EQUALLY SPACED HORIZONTAL WALL BRIDGING FOR THE HEIGHTS SHOWN...
25. FULLY INSTALL ALL BRIDGING BEFORE APPLYING LOADS...
26. JOIST SHALL BEAR DIRECTLY ON STUDS UNLESS HEADERS ARE USED...
27. PROVIDE JOIST WEB STIFFENERS WHERE JOIST BRACING IS LESS THAN 3-1/2"...
28. FOR WELDED CONNECTIONS, FUSION WELDING IS RECOMMENDED WITH A DIRECT CURRENT WELDER...
29. TRUSS ERECTOR IS RESPONSIBLE FOR ALL TEMPORARY BRACING OF THE TRUSS SYSTEM DURING CONSTRUCTION...

OPEN WEB STEEL JOISTS AND JOIST GIRDERS (NOTED "JOISTS" HEREIN)

- 1. STEEL JOIST MANUFACTURER SHALL BE A MEMBER OF THE STEEL JOIST INSTITUTE...
2. STEEL JOISTS SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE AISC STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOISTS K-SERIES...

GLAZING NOTES:

- FLORIDA BUILDING CODE: 1609.12
ALL EXTERIOR GLAZING SHALL BE IMPACT RESISTANT.
1. GLAZED OPENINGS LOCATED WITHIN 30 FEET OF GRADE SHALL MEET THE REQUIREMENTS OF THE LARGE MISSILE TEST.
2. GLAZED OPENINGS LOCATED MORE THAN 30 FEET ABOVE GRADE SHALL MEET THE PROVISIONS OF THE SMALL MISSILE TEST.
3. GLAZING IN OCCUPANCY II, III, OR IV BUILDINGS LOCATED OVER 60 FEET ABOVE GRADE AND OVER 30 FEET ABOVE AGGREGATE SURFACE ROOF LOCATED WITHIN 1500 FEET OF THE BUILDING SHALL BE PERMITTED TO BE UNPROTECTED...

STEEL CONSTRUCTION SPECIAL INSPECTIONS:

- 1. STRUCTURAL STEEL
- SPECIAL INSPECTIONS AND NON-DESTRUCTIVE TESTING OF STRUCTURAL STEEL ELEMENTS SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISI S 362...
2. COLD-FORMED STEEL DECK
- SPECIAL INSPECTION AND QUALIFICATION OF WELDING SPECIAL INSPECTORS FOR COLD-FORMED STEEL FLOOR AND ROOF DECK SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISI S 362...
3. OPEN-WEB STEEL JOIST AND JOIST GIRDERS
- SPECIAL INSPECTION OF OPEN-WEB STEEL JOISTS AND JOIST GIRDERS SHALL BE AS FOLLOWS:
- SPECIAL INSPECTION OF END CONNECTIONS (WELDED AND/OR BOLTED)
- SPECIAL INSPECTION OF HORIZONTAL AND/OR DIAGONAL BRIDGING (STANDARD AND/OR SPECIALTY BRIDGING)
4. REPORTS/CERTIFICATION
- SPECIAL INSPECTION REPORTS TO BE SENT DIRECTLY TO THE OWNER, ARCHITECT, ENGINEER, CONTRACTOR AND BUILDING DEPARTMENT...
5. SPECIAL INSPECTIONS TO BE DONE DIRECTLY TO THE OWNER, ARCHITECT, ENGINEER, CONTRACTOR AND BUILDING DEPARTMENT...
6. UPON COMPLETION OF STEEL CONSTRUCTION/SPECIAL INSPECTIONS, SPECIAL INSPECTORS TO SIGN AND DIRECTLY TO THE OWNER, ARCHITECT, ENGINEER, CONTRACTOR AND BUILDING DEPARTMENT...
7. UPON COMPLETION OF STEEL CONSTRUCTION IS IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS...

SHEET INDEX:

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S5.1 TILT-UP PANEL KEYPLAN
S5.2 TILT-UP PANEL DETAILS
S5.3 TILT-UP PANEL ELEVATIONS
S5.4 TILT-UP PANEL ELEVATIONS

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Project Name

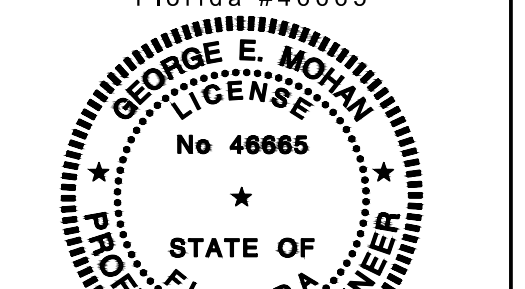
MIKE ERDMAN CADILLAC
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Sheet Title

STRUCTURAL NOTES

Revisions
3/7/2024 ADDENDUM NO. 1

State Registration
George E. Mohan, P.E.



Project Number 22026
Drawn By YB / LD
Checked By GEM
Scale AS NOTED
Issue Date 02.09.24
Sheet Number

S1.1

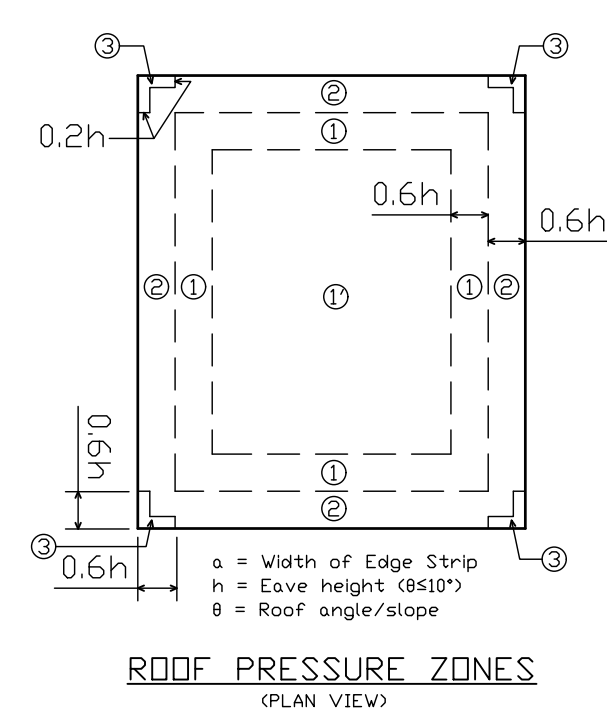
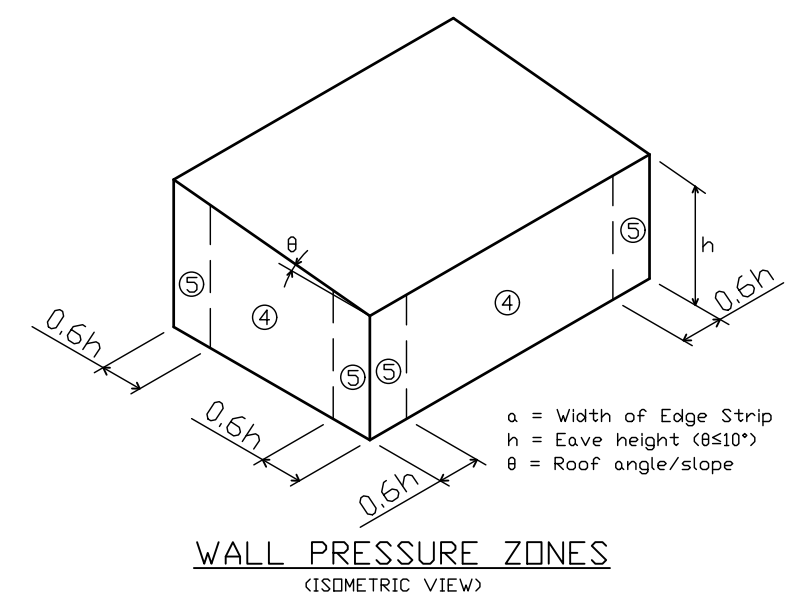
BUILDING DATA (FLAT ROOF)

Wind Velocity:	144 mph (Ultimate)	Internal Pressure Coefficient:	+/- 0.18
Basic Wind Spd. (3 sec.):	112 mph (ASD)	(Enclosed Building Per ASCE 7-22)	
Risk Category:	II	Mean Roof Height, h (ft):	24
Roof Angle, θ (degrees):	$0.0 < \theta < 7.0$	Width of Edge Strip, 0.6h (ft):	14.1
Exposure Category:	C	Width of Edge Strip, 0.2h (ft):	4.7
Roof DL Resisting Uplift (ASD):	12 psf	Height & Exposure Adjustment Factor:	0.93

WIND DESIGN LOADS - COMPONENTS & CLADDING (ASD)

ZONE	AREA (sf)	ROOF DESIGN PRESSURE (psf)			WALLS, DOORS, WINDOWS DESIGN PRESSURE (psf)			
		Pos.	Neg.	Net Uplift	Pos.	Neg.	Net Uplift	
1	10	12.1	-47.5	-35.5	4	10	27.3	-29.6
1	20	11.4	-44.4	-32.4	4	20	26.1	-28.4
1	50	10.4	-40.2	-28.2	4	50	24.5	-26.8
1	100	10.0	-37.1	-25.1	4	100	23.3	-25.6
1'	10	12.1	-27.3	-15.3	5	10	27.3	-36.4
1'	20	11.4	-27.3	-15.3	5	20	26.1	-34.0
1'	50	10.4	-27.3	-15.3	5	50	24.5	-30.8
1'	100	10.0	-27.3	-15.3	5	100	23.3	-28.4
2	10	12.1	-62.7	-50.7				
2	20	11.4	-58.6	-46.6				
2	50	10.4	-53.3	-41.3				
2	100	10.0	-49.3	-37.3				
3	10	12.1	-85.4	-73.4				
3	20	11.4	-77.4	-65.4				
3	50	10.4	-66.7	-54.7				
3	100	10.0	-58.6	-46.6				

- Notes:
- 1) For effective areas between those given above the load may be interpolated; otherwise use the load associated with the lower effective area.
 - 2) Plus and minus signs signify pressures acting low and end away from the building surfaces, respectively.
 - 3) See pressure zone diagrams for corresponding zones.
 - 4) Roof coverings, finishes, etc. shall be designed for the full negative design pressure.
 - 5) Roof framing members shall be designed to resist the net uplift design pressure specified.
 - 6) For ultimate wind pressure, multiply value in the table by 1.67.



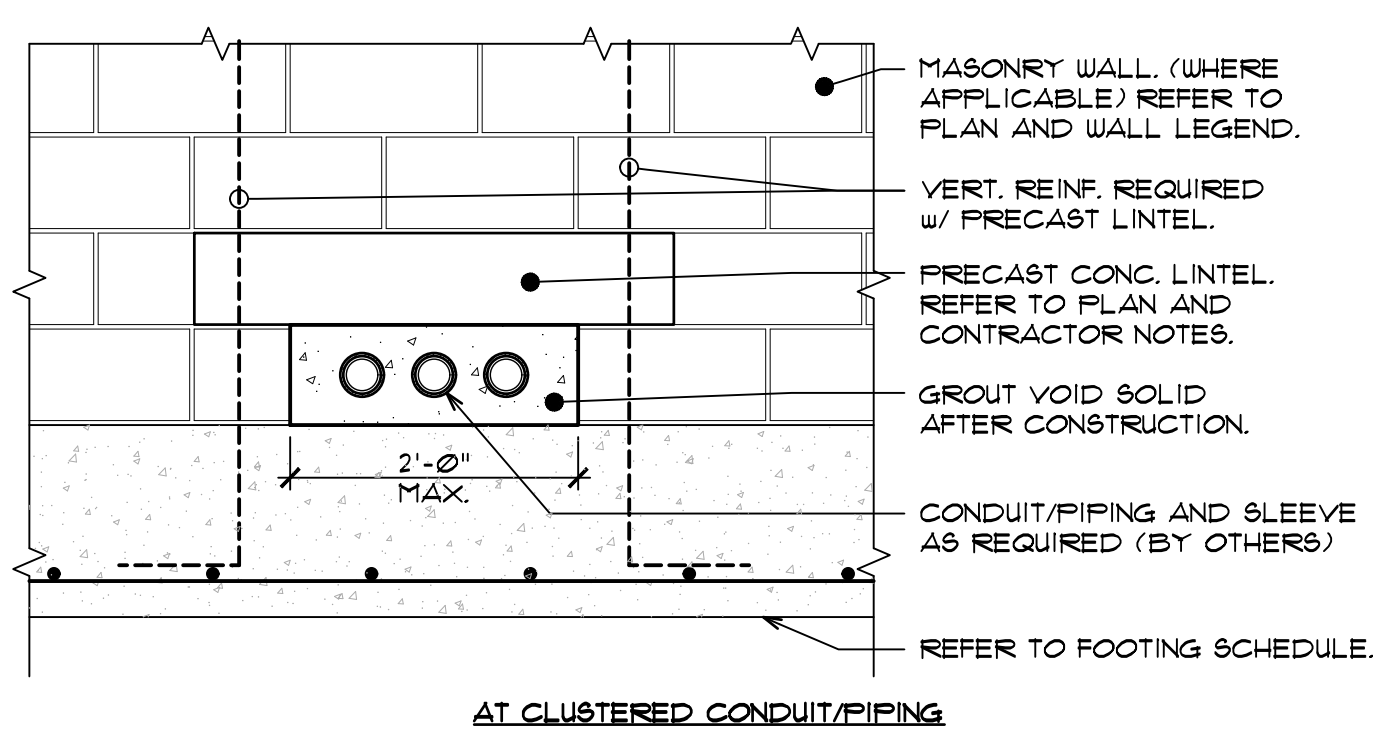
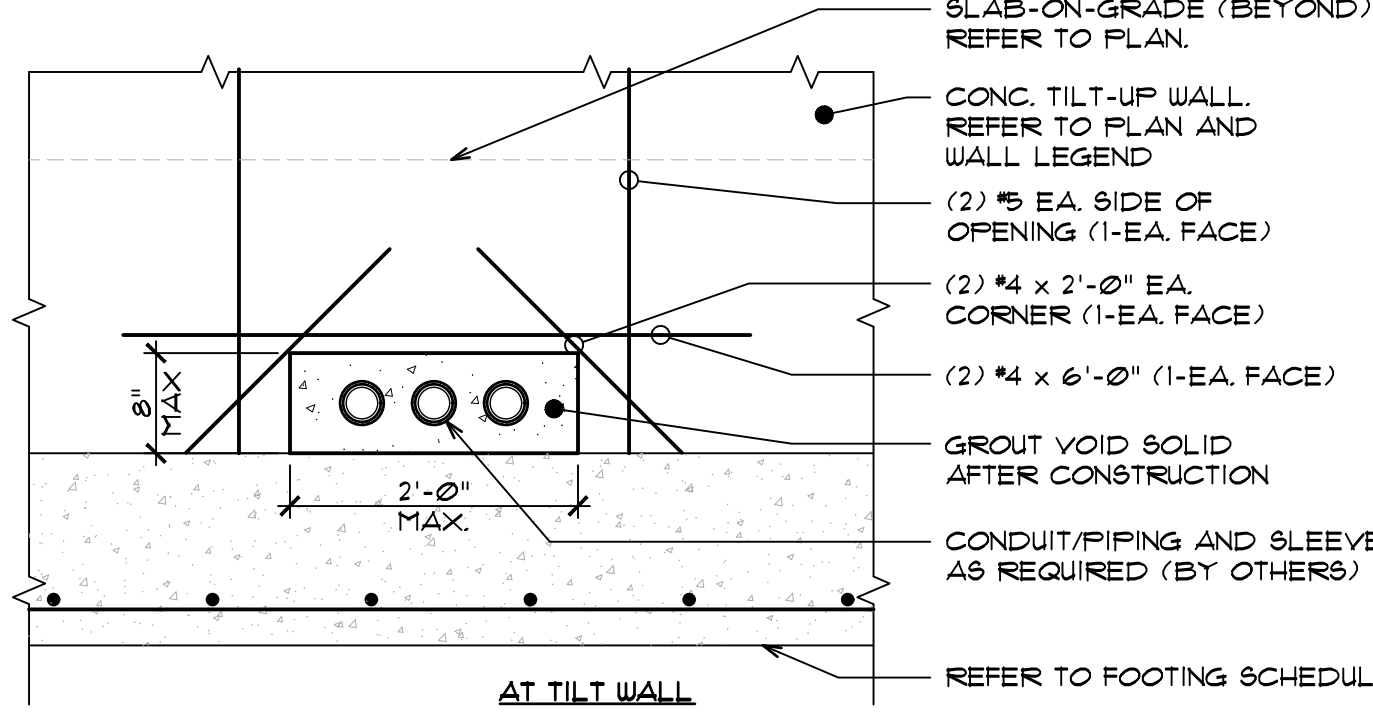
WIND DESIGN CRITERIA:
FLORIDA BLDG. CODE 8th EDITION (2024) - BUILDING

ROOFTOP STRUCTURES AND EQUIPMENT (PER ASCE 7-16)

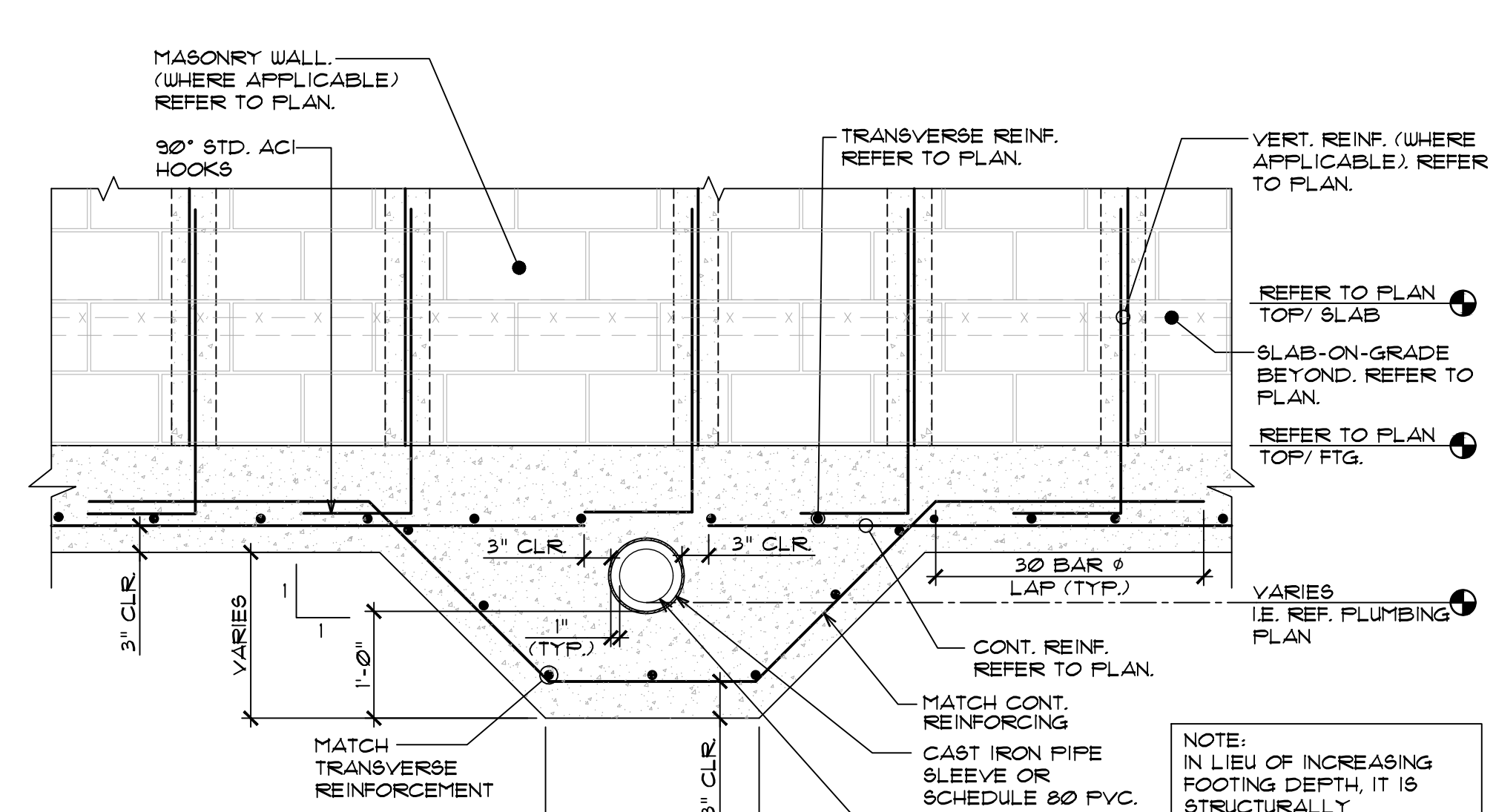
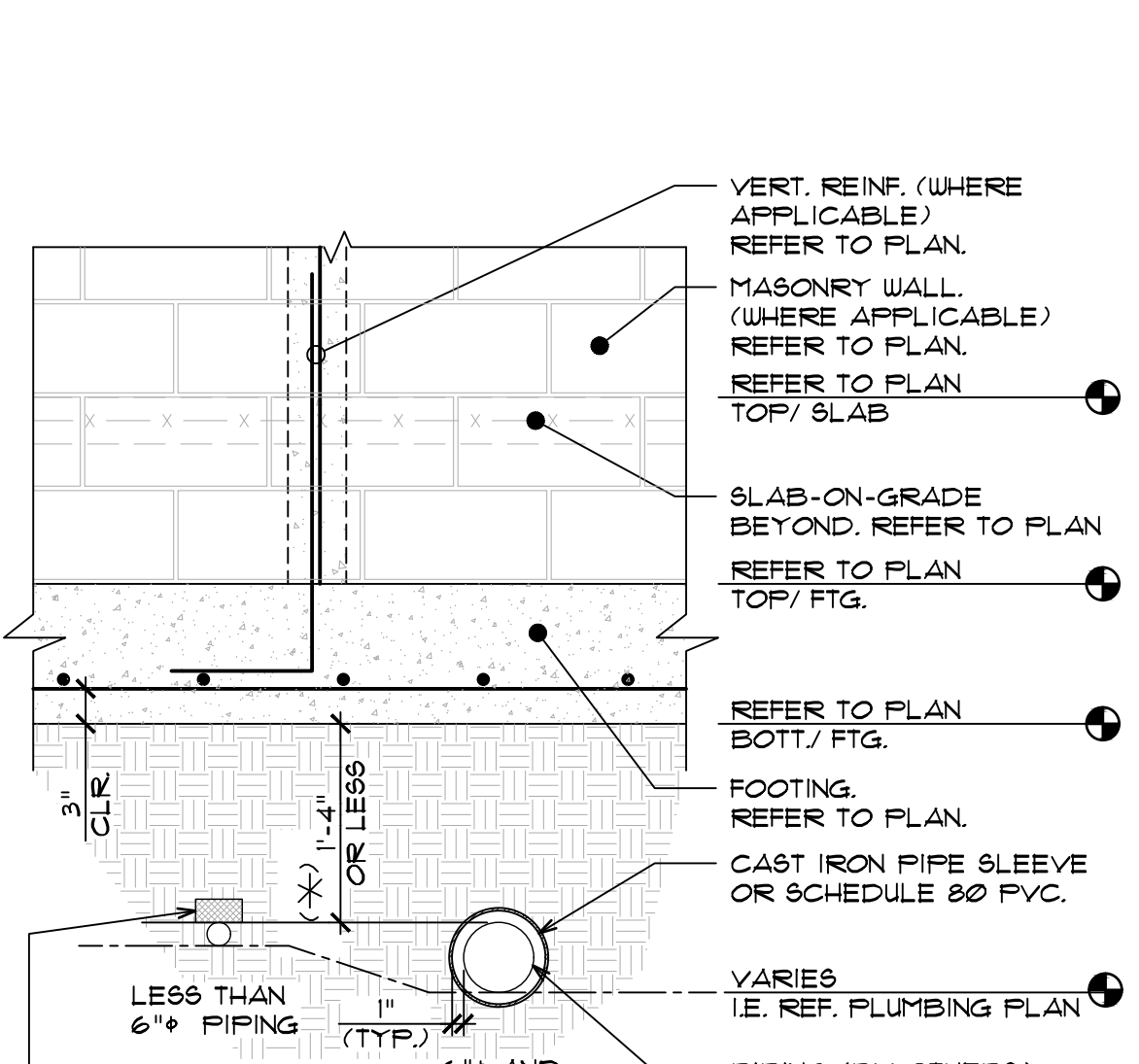
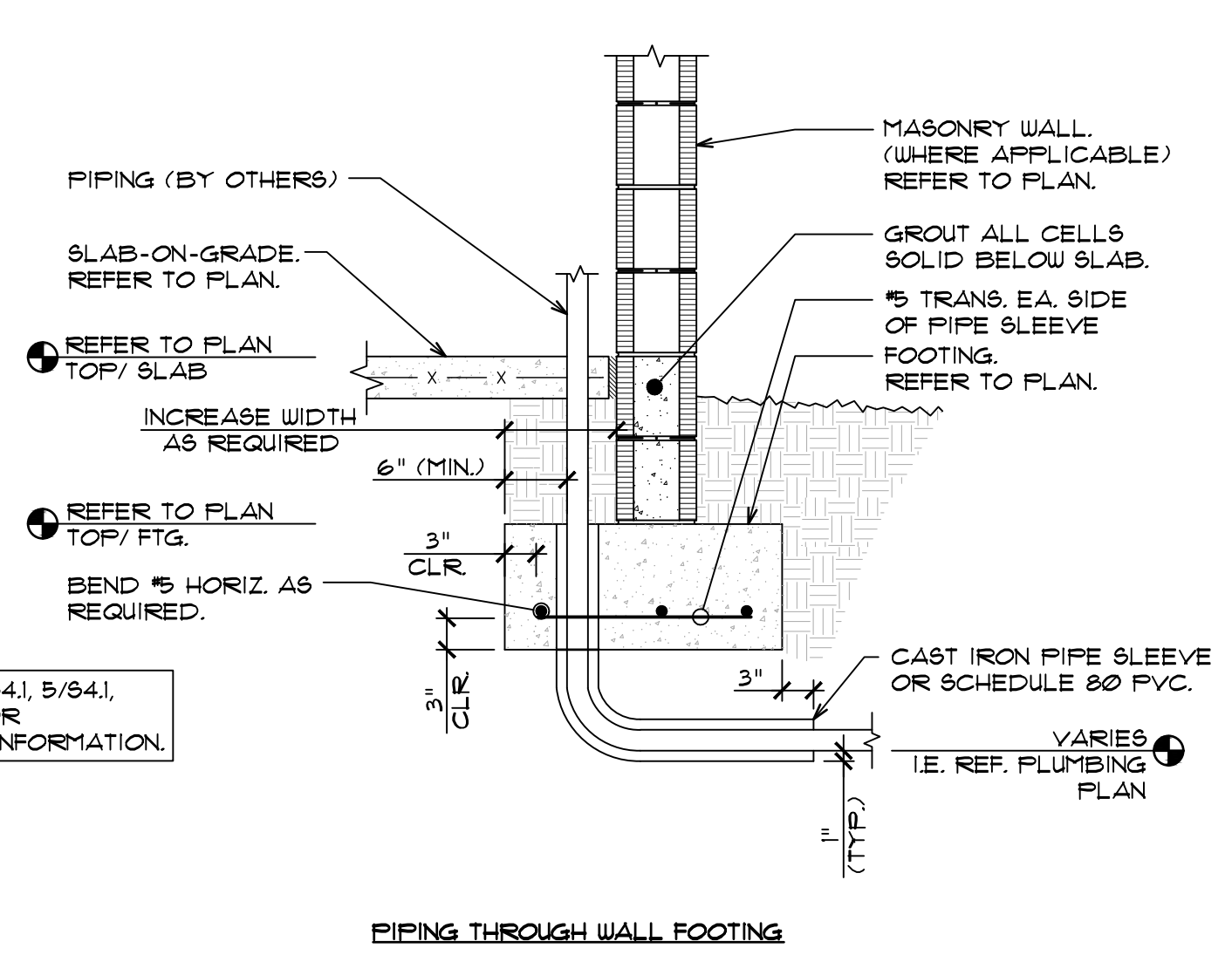
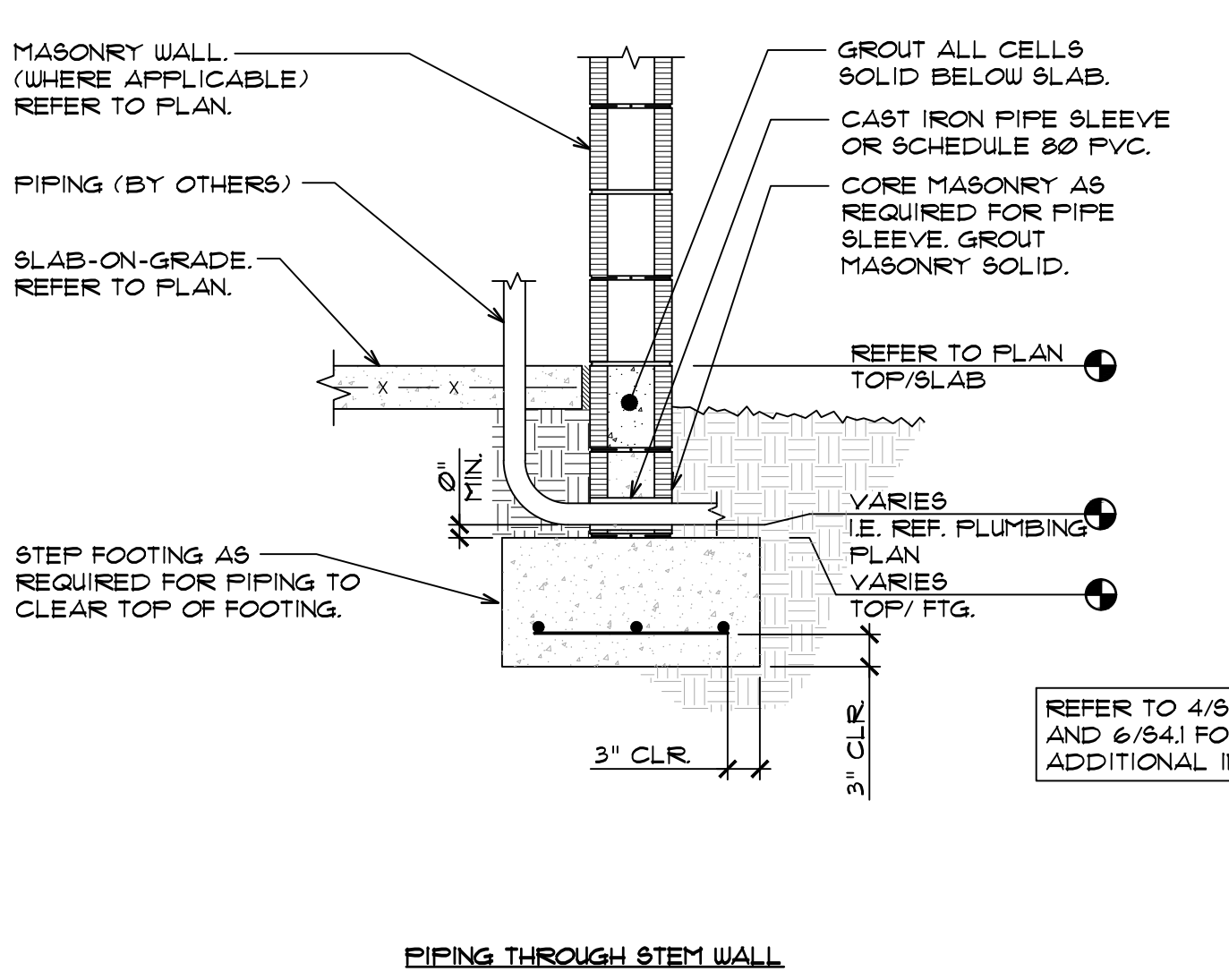
1. BASIC WIND SPEED = 144 MPH (ULTIMATE)
= 112 MPH (ASD)
2. RISK CATEGORY = II
3. WIND EXPOSURE = 'C'
4. NET PRESSURE COEFFICIENTS:
HORIZONTAL: 1.5
VERTICAL: 1.5
5. DESIGN PRESSURE:
HORIZONTAL = (+/-) 65.4 PSF (ULTIMATE)
= (+/-) 50.3 PSF (ASD)
VERTICAL = (+/-) 51.6 PSF (ULTIMATE)
= (+/-) 39.2 PSF (ASD)

1 WIND DESIGN CRITERIA
NOT TO SCALE

- CONTRACTOR NOTES:**
1. USE THIS DETAIL ONLY WHERE REQUIRED. DO NOT PENETRATE WALL LEGS OR CLUSTERED WALL REINF.
 2. PROVIDE 2'-0" MIN. BETWEEN ADJACENT OPENINGS.
 3. STEP FOOTING AS REQUIRED. REFER TO 1/84.0.
 4. NOTIFY ARCH/ENG. OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION.



- CONTRACTOR NOTES:**
1. USE THIS DETAIL ONLY WHERE REQUIRED. DO NOT PENETRATE CONC. COLUMNS OR CLUSTERED WALL REINFORCEMENT.
 2. PROVIDE 2'-0" MIN. BETWEEN ADJACENT OPENINGS.
 3. STEP FOOTING AS REQUIRED. REFER TO 2/84.1.
 4. FOR OPENING 2'-0" TO 3'-4" USE PRECAST CONCRETE LINTEL. (L1) NOTIFY ARCHITECT/ENGINEER OF ANY OTHER CONDITIONS. RECESSED PRECAST LINTELS ARE STRUCTURALLY ACCEPTABLE.
 5. NOTIFY ARCHITECT/ENGINEER OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION.



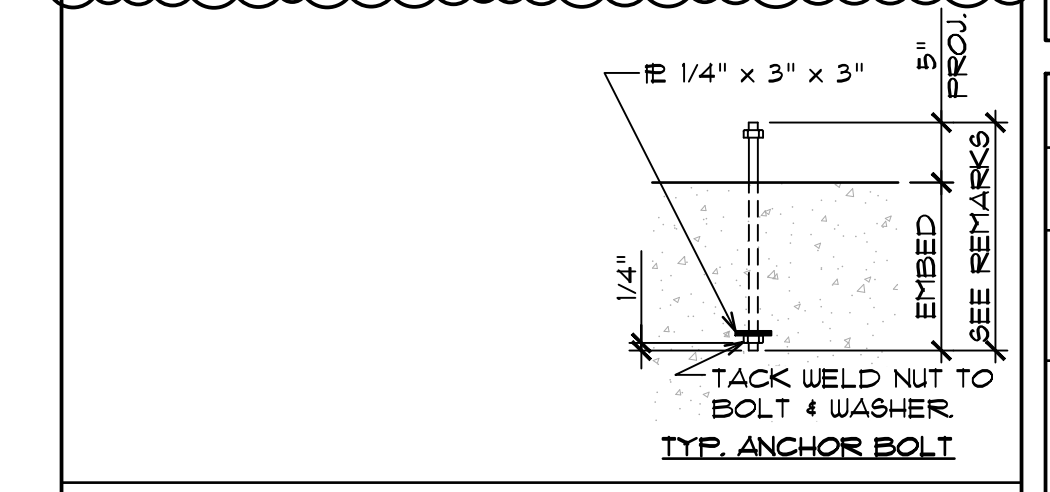
- CONTRACTOR CAUTION:**
1. DO NOT LOCATE CONDUIT/PIPE AT COLUMNS, CLUSTERED REINF. OR COLUMN FOOTING. NOTIFY ARCHITECT/ENGINEER OF ANY CONFLICTS.
 2. FOUNDATION AND REINFORCING SHOWN FOR REFERENCE ONLY. REFER TO FOOTING SCHEDULE, PLAN, AND DETAILS FOR SPECIFIC REINFORCING.

2 TYPICAL CONDUIT/PIPE DETAILS
SCALE: 3/4" = 1'-0"

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STEEL COLUMN SCHEDULE

MARK	COLUMN SIZE (H.S.S. UNO.)	BASE PLATE SIZE	BASE PLATE TYPE	ANCHOR BOLT QTY, DIA., EMBED.
BC1	6"x6"x1/2"	3/4"x12"x12"	"A"	4 3/4" @ 9"
BC2	6"x6"x5/8"	3/4"x12"x12"	"A"	4 3/4" @ 9"
BC3	8"x6"x5/8"	3/4"x14"x14"	"B"	4 3/4" @ 9"
PIF1	1/2 STD	3/4"x18"x18"	"C"	4 3/4" @ 9"



NOTES:
 1. REFER TO 4/941 AND 5/641 FOR H.S.S. STEEL COLUMN DETAILS.
 2. BASE PLATE TO BE CENTERED BELOW COLUMN (TYP.) UNO.

ADHESIVE ANCHOR NOTES:
 1. ALL ADHESIVE ANCHORS ARE TO BE INSPECTED IN ACCORDANCE W/ ACI 308, ACI 308.5, AND THE MANUFACTURER'S REQUIREMENTS.
 2. ALL ADHESIVE ANCHOR INSTALLERS ARE TO BE CERTIFIED IN ACCORDANCE W/ ACI 308 AND THE MANUFACTURER'S REQUIREMENTS.
 3. CONTRACTOR TO COORDINATE ADHESIVE ANCHOR INSPECTIONS AND INSTALLER CERTIFICATIONS AS REQUIRED.
 4. A COPY OF ALL INSPECTION REPORTS/INSTALLER CERTIFICATIONS SHALL BE SENT DIRECTLY TO THE OWNER, ARCHITECT, STRUCTURAL ENGINEER, AND THE GENERAL CONTRACTOR.

FOOTING SCHEDULE

MARK	SIZE	DEPTH	REINFORCING EACH WAY	REMARKS
F1	1'-0" x CONT.	1'-0" (MIN.)	(1) #5 CONT. TOP (2) #5 CONT. BOTT.	THICK SLAB EDGE
UF2	6" x CONT.	1'-0"	(3) #5 CONT. BOTT. #5 TRANSV. AT 48" O.C. MAX. BOTT.	WALL FTG.
F4	4'-0" x 4'-0"	1'-4"	(5) #5 EA. WAY T 4 B	COL. FTG.
F5	5'-0" x 5'-0"	1'-4"	(6) #5 EA. WAY T 4 B	COL. FTG.
F6	6'-0" x 6'-0"	1'-4"	(7) #5 EA. WAY T 4 B	COL. FTG.
F7	7'-0" x 7'-0"	1'-4"	(8) #5 EA. WAY T 4 B	COL. FTG.

NOTES:
 1. CENTER ALL FOOTINGS BELOW WALL OR COLUMN UNO.
 2. ALL FOOTING REINFORCING TO BE BOTTOM BARS UNO.
 3. () INDICATES TOP OF FOOTING.
 4. INCREASE DEPTH OF THICKENED SLAB EDGE AS REQUIRED TO MAINTAIN 1'-0" (MIN.) EMBEDMENT BELOW GRADE (TYP.) UNO.

WALL LEGEND

WALL TYPE HATCH	REMARKS
	INDICATES PRE-ENGINEERED CONCRETE TILT UP WALL. REFER TO PLAN, SHEET 951 AND SHEET 952 FOR MINIMUM REINFORCING.
	INDICATES 6", 16 GA. (6@2 5/62-54) METAL STUDS AT 16" O.C. MAX. W/ CONT. 6", 16 GA. (6@2 1/25-54) METAL TRACK TOP AND BOTT. REFER TO SHEET 931 FOR TYPICAL METAL STUD FRAMING NOTES.

NOTES:
 1. REFER TO PLAN FOR ADDNL./SPECIFIC REIN. REQUIREMENTS.

METAL STUD FRAMING NOTES:
 1. THE METAL STUD FRAMING (SIZES, CONFIGURATIONS, DETAILS AND CONNECTIONS) SHOWN ON THE DRAWINGS ARE MINIMUM SIZES/MAXIMUM SPACINGS AND QUANTITIES TO BE UTILIZED AND ARE FOR ESTIMATING PURPOSES. ALL METAL STUD FRAMING SHALL BE VERIFIED BY THE CONTRACTOR'S DELEGATE SPECIALTY ENGINEER AND INCREASED AS REQUIRED. REFER TO STRUCTURAL NOTES SHEET FOR ADDITIONAL INFORMATION.
 2. METAL STUD FRAMING CONNECTIONS FOR STUDS, TRACKS, CLIP ANGLES, ETC. ARE BY THE CONTRACTOR'S DELEGATE SPECIALTY ENGINEER. REFER TO STRUCTURAL DRAWINGS FOR MINIMUM SIZES, MINIMUM FASTENERS, ETC. AS REQUIRED.
 3. INCREASE METAL STUD/TRACK GAGE AS REQUIRED FOR WINDOW, DOOR, ETC. PRODUCT APPROVAL REQUIREMENTS. CONTRACTOR TO COORDINATE PRODUCT APPROVAL REQUIREMENTS W/ MANUFACTURER AS REQUIRED.
 4. AS REQUIRED BY THE CONTRACTOR'S DELEGATE SPECIALTY ENGINEER, THE CONTRACTOR SHALL PROVIDE CLIP ANGLES FOR ALL METAL STUDS SO PROPER ALIGNMENT CAN BE ATTAINED. CLIP ANGLES SHALL MEET OR EXCEED THE STUD GAGE.
 5. CONNECT WALL SHEATHING, 5/4" (MIN. NOM.) EXTERIOR PLYWOOD (COORD. W/ ARCH.) TO METAL STUDS WITH #2 TEK SCREWS AT 4" O.C. AT PANEL EDGES AND AT 12" O.C. FIELD. BLOCK ALL SHEATHING EDGES (48" O.C. MAX.). STAGGER SHEATHING EDGES 16" MIN. 1-STD SPACE.
 6. COORDINATE WALL SHEATHING REQUIREMENTS WITH ARCHITECTURAL DRAWINGS/MANUFACTURER'S REQUIREMENTS.
 7. COORDINATE ALL DIMENSIONS/ELEVATIONS WITH ARCHITECTURAL DRAWINGS/MANUFACTURER'S REQUIREMENTS.

METAL STUD FASTENING NOTES:

(UNLESS NOTED OTHERWISE)
 1. REFER TO PLAN DETAILS, AND WALL SECTIONS FOR MINIMUM STUD SIZES.
 2. MINIMUM CLIP ANGLE TO BE AS FOLLOWS:
 3 5/8" STUDS + L3" x 1 1/2" x 14 GA. x 0-3" (MIN.)
 6" STUDS + L3" x 1 1/2" x 14 GA. x 0-5" (MIN.)
 8" STUDS + L3" x 1 1/2" x 14 GA. x 0-7" (MIN.)
 3. METAL STUDS AND/OR CLIP ANGLES SHALL BE CONNECTED TO METAL STUDS WITH:
 (4) #0 TEK SCREWS AT 3 5/8" STUDS
 (5) #0 TEK SCREWS AT 6" STUDS
 (5) #0 TEK SCREWS AT 8" STUDS
 4. METAL STUDS AND/OR CLIP ANGLES SHALL BE CONNECTED TO STRUCTURAL STEEL WITH (3) #145" RAMSET PDF'S OR EQUAL.
 5. METAL STUDS AND/OR CLIP ANGLES SHALL BE CONNECTED TO CONCRETE AND/OR MASONRY W/ (2) 1/4" x 1 3/4" TAPCON SCREWS. (AT 6" AND 8" STUD USE (3) TAPCON SCREWS).
 6. CONTINUOUS METAL STUD TRACKS SHALL BE CONNECTED TO STRUCTURAL STEEL WITH (2) #145" RAMSET PDF'S OR EQUAL AT 16" O.C. MAXIMUM.
 7. CONTINUOUS METAL STUD TRACKS SHALL BE FASTENED TO CONCRETE AND/OR MASONRY WITH (1) 1/4" x 1 3/4" TAPCON SCREWS AT 16" O.C. MAXIMUM.
 8. FASTEN METAL TRACK TO EACH METAL STUD WITH (1) #0 TEK SCREW EA. LEG AT EA. STUD.
 9. AS REQUIRED, THE CONTRACTOR SHALL PROVIDE CLIP ANGLES FOR ALL METAL STUDS SO PROPER ALIGNMENT CAN BE ATTAINED. CLIP ANGLES SHALL MEET OR EXCEED THE STUD GAGE. SEE FASTENING REQUIREMENTS ABOVE.
 10. STUD/TRACK/CLIP SIZES AND GAGES AND FASTENER SIZES AND QUANTITIES SHALL BE VERIFIED BY DELEGATE SPECIALTY ENGINEER AND INCREASED AS REQUIRED.

GENERAL NOTES:

- PROVIDE CORNER BARS AT ALL CORNERS AND INTERSECTIONS OF FOOTINGS AND THICKENED SLAB EDGES. REFER TO 3/841 FOR ADDITIONAL INFORMATION.
- () INDICATES TOP OF BEAM.
() INDICATES BOTTOM OF BEAM.
() INDICATES BEARING ELEVATION.
- REFER TO ARCHITECTURAL DRAWINGS FOR ALL OPENINGS. CONTRACTOR TO VERIFY ALL ROUGH OPENINGS WITH MANUFACTURER'S REQUIREMENTS AND NOTIFY ARCHITECT/ENGINEER OF ANY DISCREPANCIES.
- REFER TO ARCHITECTURAL/PLUMBING DRAWINGS FOR FIXTURE DRAIN LOCATIONS AND REQUIREMENTS. STEP FOOTINGS AS REQUIRED (1/841) AND NOTIFY ARCHITECT/ENGINEER OF ANY DISCREPANCIES. REFER TO 2/812 FOR TYPICAL CONDUIT/PIPE DETAILS.
- CONTRACTOR TO COORDINATE ALL SLOPES, DEPRESSIONS, AND SLAB OPENINGS W/ ARCHITECTURAL/MEP DRAWINGS PRIOR TO CONSTRUCTION. REFER TO 1/841 FOR TYPICAL SLAB STEPS.
- REFER TO 1/912 FOR WIND DESIGN CRITERIA.

DIMENSION/ELEVATION NOTES:

- REFER TO ARCHITECTURAL SHEETS FOR DIMENSIONS AND ELEVATIONS.
- ALL DIMENSIONS AND ELEVATIONS SHOWN ARE BASED ON DIMENSIONS AND ELEVATIONS PER ARCHITECTURAL SHEETS.
- CONTRACTOR TO COORDINATE ALL DIMENSIONS AND ELEVATIONS SHOWN WITH ARCHITECTURAL SHEETS AND MANUFACTURER'S REQUIREMENTS AND NOTIFY ARCHITECT/ENGINEER OF ANY DISCREPANCIES.

JOIST MANUFACTURER NOTES:

- ALL JOIST BRIDGING TO BE IN ACCORDANCE WITH THE STEEL JOIST INSTITUTE (SJI) MINIMUM REQUIREMENTS UNO.
- MECHANICAL EQUIPMENT WEIGHTS AND LOCATIONS TO BE VERIFIED PRIOR TO JOIST/GIRDER FABRICATION. G.C. COORDINATE WEIGHTS/LOCATIONS W/ SPECIALTY TRADES AS REQUIRED.

architecture design planning

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Sheet Title

SCHEDULES

Revisions

3/7/2024 ADDENDUM NO. 1

Slate Registration

George E. Mohan, P.E.
Florida: 46665

Project Number

22026

Drawn By

YB / LD

Checked By

GEM

Scale

AS NOTED

Issue Date

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Sheet Number

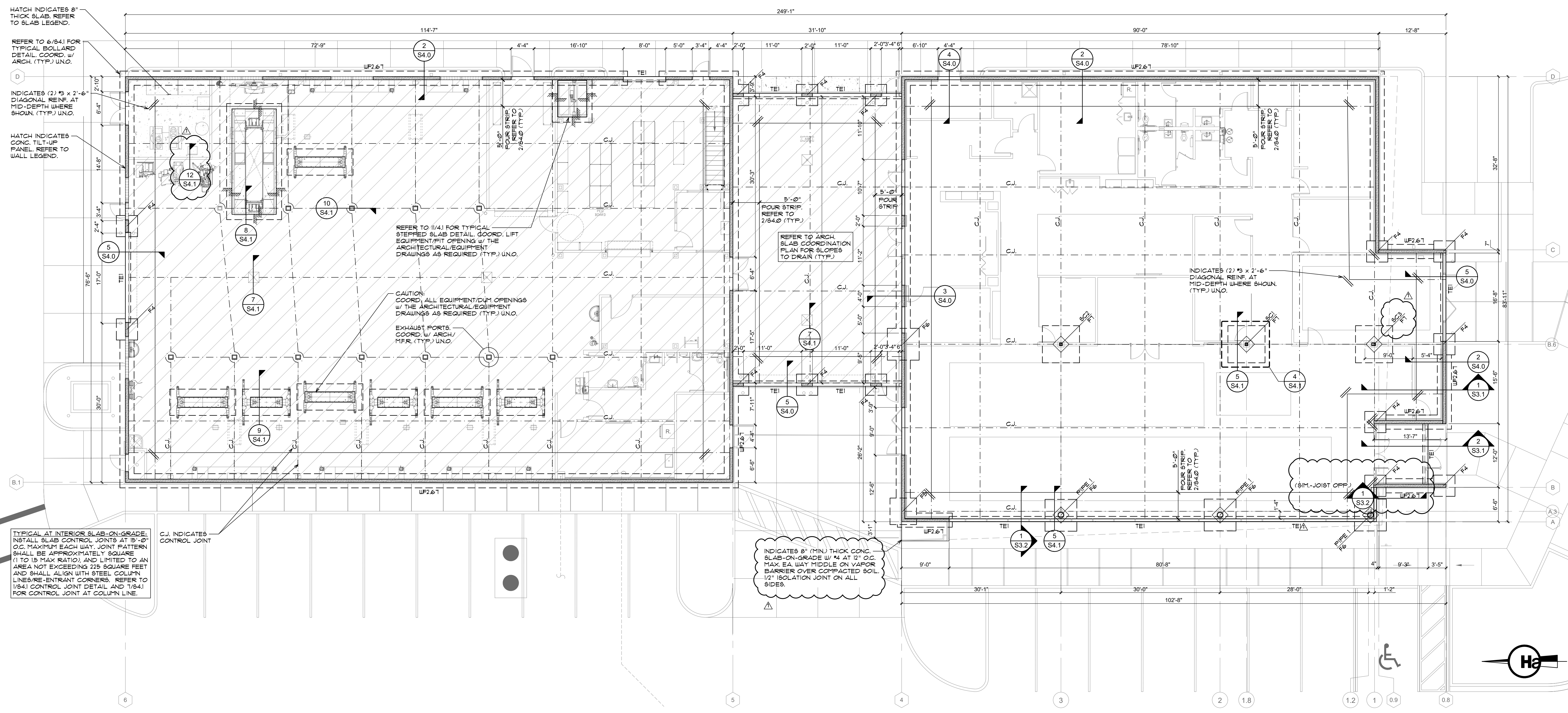
S1.3

NOTES:
1. REFER TO SHEET 013 FOR TYPICAL NOTES, SCHEDULES, AND LEGENDS.

FOUNDATION PLAN NOTES:
1. REFER TO GEOTECHNICAL REPORT FOR SOIL PREPARATION REQUIREMENTS AND/OR BASE COARSE REQUIREMENTS BELOW VAPOR BARRIER.
2. SLAB-ON-GRADE TO BE 5" THICK WITH 6X6 - W/3X12.9 W/FP ON VAPOR BARRIER OVER COMPACTED SOIL UNO. COORDINATE VAPOR BARRIER WITH ARCHITECT. SLAB SLOPE TOLERANCE = 1/8" PER 10'-0" PER ACI 111. REFER TO ARCHITECTURAL DRAWINGS FOR TERTITE TREATMENTS.
3. TOP OF SLAB ELEVATION = 0'-0" UNO. TOP OF ISOLATED COLUMN FTG. ELEVATION = (-) 1'-0" UNO. TOP OF WALL FTG ELEVATION = (-) 1'-4" UNO.
4. () INDICATES TOP OF FOOTING ELEVATION.
5. STEP FOOTINGS WITH GRADE AS REQUIRED. PROVIDE 10" MIN. SOIL COVER OVER ALL EXTERIOR FOOTINGS. CONTRACTOR TO FIELD-VERIFY FOOTING ELEVATIONS. NOTIFY ENGINEER OF ANY CONFLICTS OR DISCREPANCIES. REFER TO 104.0 FOR STEPPED FOOTING DETAIL.
6. WALL FOOTINGS TO BE TYPE "WF2.6" UNO.

SLAB LEGEND:

- HATCH INDICATES EXTENT OF 5" (MIN) THICK CONG. SLAB-ON-GRADE W/ 6X6 W/3X12.9 W/FP. COORD. W/ ARCHITECTURAL AND CIVIL DRAWINGS.
- HATCH INDICATES EXTENT OF 6" (MIN) THICK CONG. SLAB-ON-GRADE W/ 6X6 W/3X12.9 W/FP. COORD. W/ ARCHITECTURAL AND CIVIL DRAWINGS.
- HATCH INDICATES EXTENT OF 8" (MIN) THICK CONCRETE SLAB-ON-GRADE W/ #4 AT 12" O.C. MAX. EA. WAY TOP AND BOTTOM ON VAPOR BARRIER OVER COMPACTED SOIL.



HATCH INDICATES 8" THICK SLAB. REFER TO SLAB LEGEND.
REFER TO 618.4 FOR TYPICAL BOLLARD DETAIL. COORD. W/ ARCH. (TYP.) UNO.
INDICATES (2) #3 x 2'-6" DIAGONAL REINF. AT MID-DEPTH WHERE SHOWN. (TYP.) UNO.
HATCH INDICATES CONC. TILT-UP PANEL. REFER TO WALL LEGEND.

TYPICAL AT INTERIOR SLAB-ON-GRADE. INSTALL SLAB CONTROL JOINTS AT 15'-0" O.C. MAXIMUM EACH WAY. JOINT PATTERN SHALL BE APPROXIMATELY SQUARE (1 TO 1.5 MAX RATIO), AND LIMITED TO AN AREA NOT EXCEEDING 225 SQUARE FEET AND SHALL ALIGN WITH STEEL COLUMN LINES-RE-ENTRANT CORNERS. REFER TO 104.1 CONTROL JOINT DETAIL AND 104.1 FOR CONTROL JOINT AT COLUMN LINE.
C.J. INDICATES CONTROL JOINT

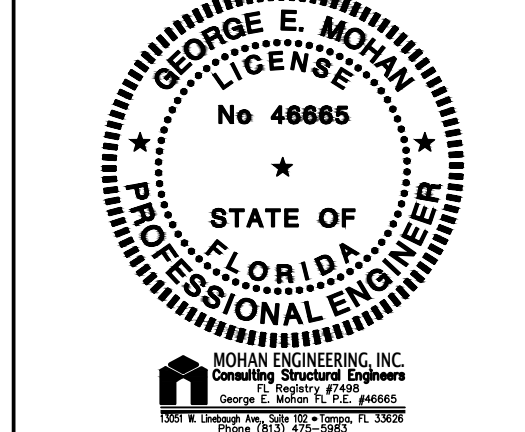
INDICATES 8" (MIN) THICK CONG. SLAB-ON-GRADE W/ #4 AT 12" O.C. MAX. EA. WAY MIDDLE ON VAPOR BARRIER OVER COMPACTED SOIL. 1/2" ISOLATION JOINT ON ALL SIDES.

Sheet Title

FOUNDATION PLAN

Revisions
3/7/2024 ADDENDUM NO. 1

State Registration
George E. Mohan, P.E.
Florida #46665



Project Number
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S2.1

FOUNDATION PLAN SCALE 1/8"=1'-0" 1

ROOF FRAMING PLAN NOTES:

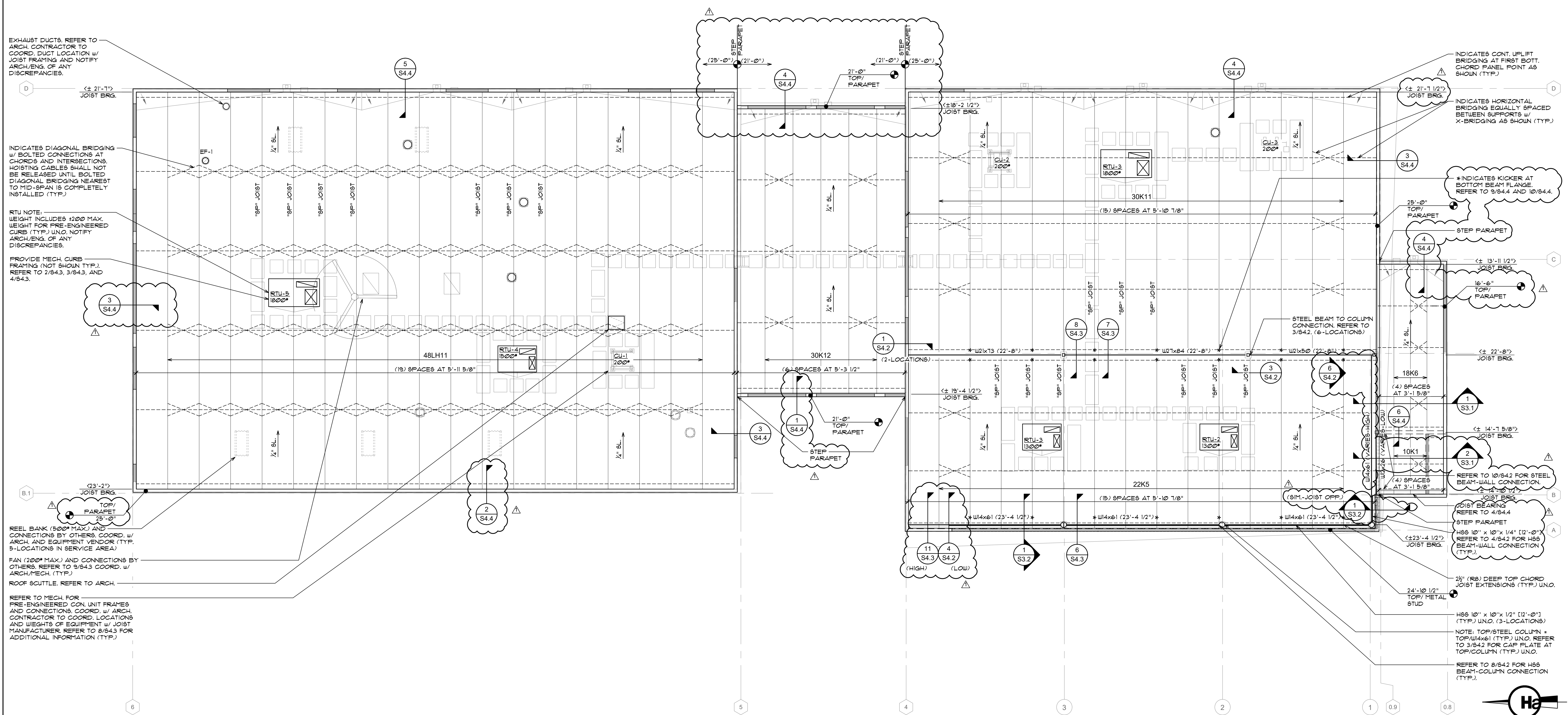
STEEL JOISTS

- ROOF FRAMING TO BE FIRE-ENGINEERED STEEL JOISTS. JOIST SPACING + SEE PLAN. JOISTS EQUALLY SPACED BETWEEN SUPPORTS UNO.
- CONTRACTOR/ERECTOR IS RESPONSIBLE FOR ALL TEMPORARY BRACING.
- METAL ROOF DECK TO BE 1 1/2" DEEP, 22 GA. GALVANIZED, TYPE 'B' FASTEN ROOF DECK w/ (7) 5/8" DIAMETER RUDDLE WELDS AT EACH SUPPORT AND (5) #10 TEK SCREW SIDELAP FASTENERS EQUALLY SPACED BETWEEN SUPPORTS (PER 36" WIDTH). REFER TO 1/84.3.
- ROOF SLOPE + REFER TO ARCH.
- JOIST BRG. ELEVATION + (VARIES) REFER TO PLAN.
- REFER TO STRUCTURAL NOTES/SPECIFICATIONS FOR 'SP' JOIST NOTES.
- REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR EXACT SIZE AND LOCATION OF REQUIRED MECHANICAL OPENINGS. CONTRACTOR TO COORDINATE SIZE, WEIGHT, AND LOCATION OF MECHANICAL EQUIPMENT AND PENETRATIONS WITH MECHANICAL EQUIPMENT SHOP DRAWINGS PRIOR TO JOIST FABRICATION. NOTIFY ARCHITECT/ENGINEER OF ANY DISCREPANCIES FROM THE STRUCTURAL DRAWINGS. REFER TO 2/84.3, 3/84.3, AND 4/84.3.

NOTE FOR ALL ARCHITECTURALLY EXPOSED STEEL:

THE EXPOSED STEEL AT THE CURTAIN WALL SYSTEM IS ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (A.E.S.S.) AND SHALL CONFORM TO SECTION 10 OF THE AISC CODE OF STANDARD PRACTICE.

- AT SURFACES EXPOSED TO VIEW ALL WELDS SHALL BE SURFACE GROUND SMOOTH AND MEET THE APPROVAL OF THE ARCHITECT PRIOR TO FRIMING.
- ALL A.E.S.S. SHALL BE SANDBLASTED (SSPC-SP6) PRIOR TO COATING.
- A.E.S.S. EXPOSED TO WEATHER SHALL BE GALVANIZED PRIOR TO FRIMING.
- PRIMER SHALL BE COMPATIBLE WITH THE FINAL PAINT COAT AND SHALL BE APPROVED BY THE FINISH PAINT CONTRACTOR. SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR PAINT SPECIFICATIONS.



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Sheet Title	ROOF FRAMING PLAN
Revisions	3/7/2024 ADDENDUM NO. 1
State Registration	George E. Mohan, P.E. Florida # 46665
Professional Engineer	FLORIDA PROFESSIONAL ENGINEER No. 46665 STATE OF FLORIDA
Project Number	22026
Drawn By	YB / LD
Checked By	GEM
Scale	AS NOTED
Issue Date	02.09.24
Sheet Number	1

ROOF FRAMING PLAN SCALE 1/8"=1'-0" 1

S2.2

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WALL SECTIONS

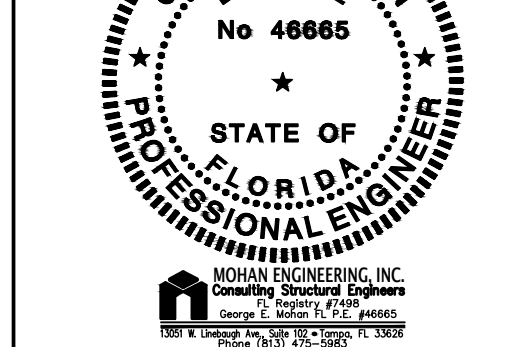
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Slate Registration

George E. Mohan, P.E.

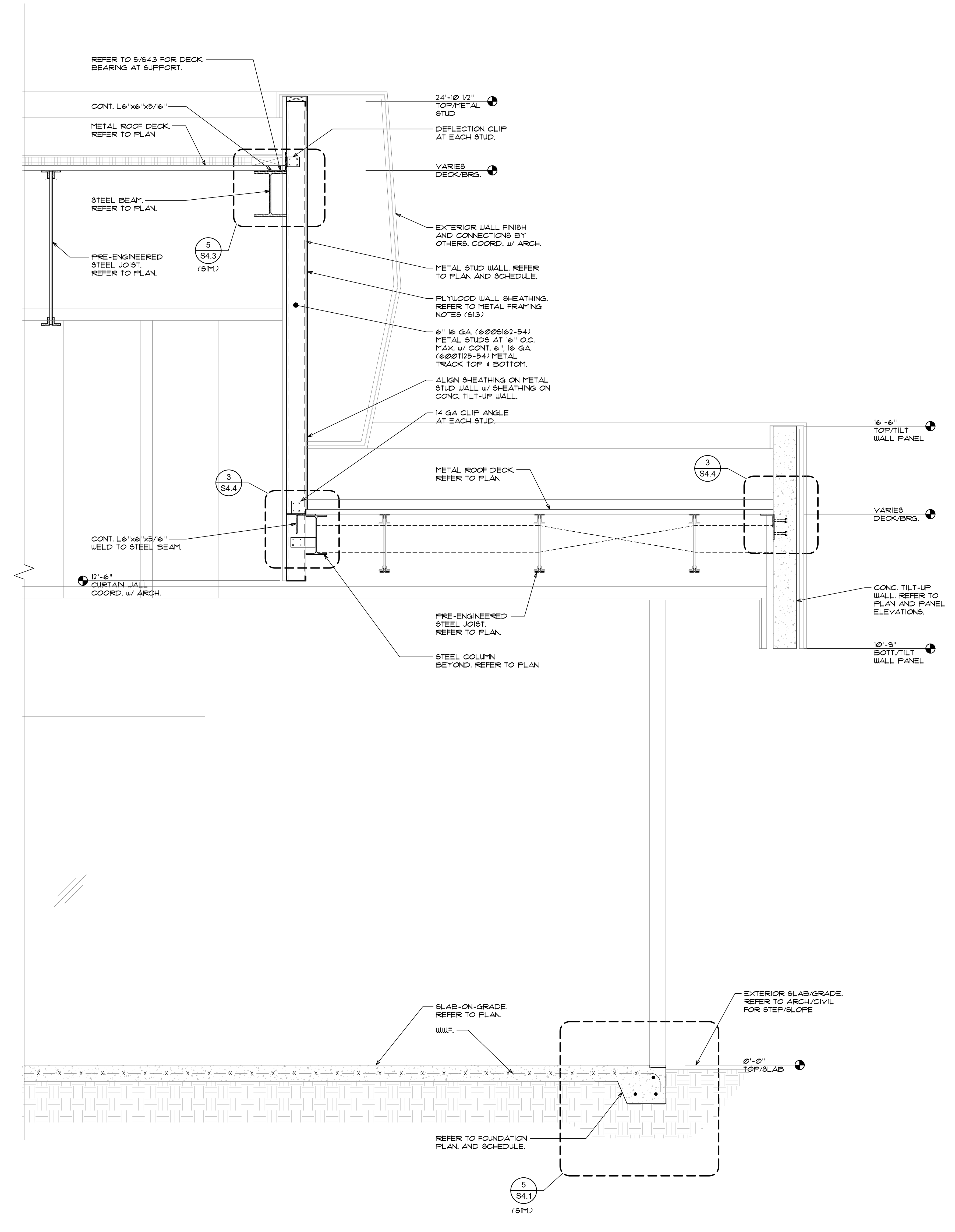
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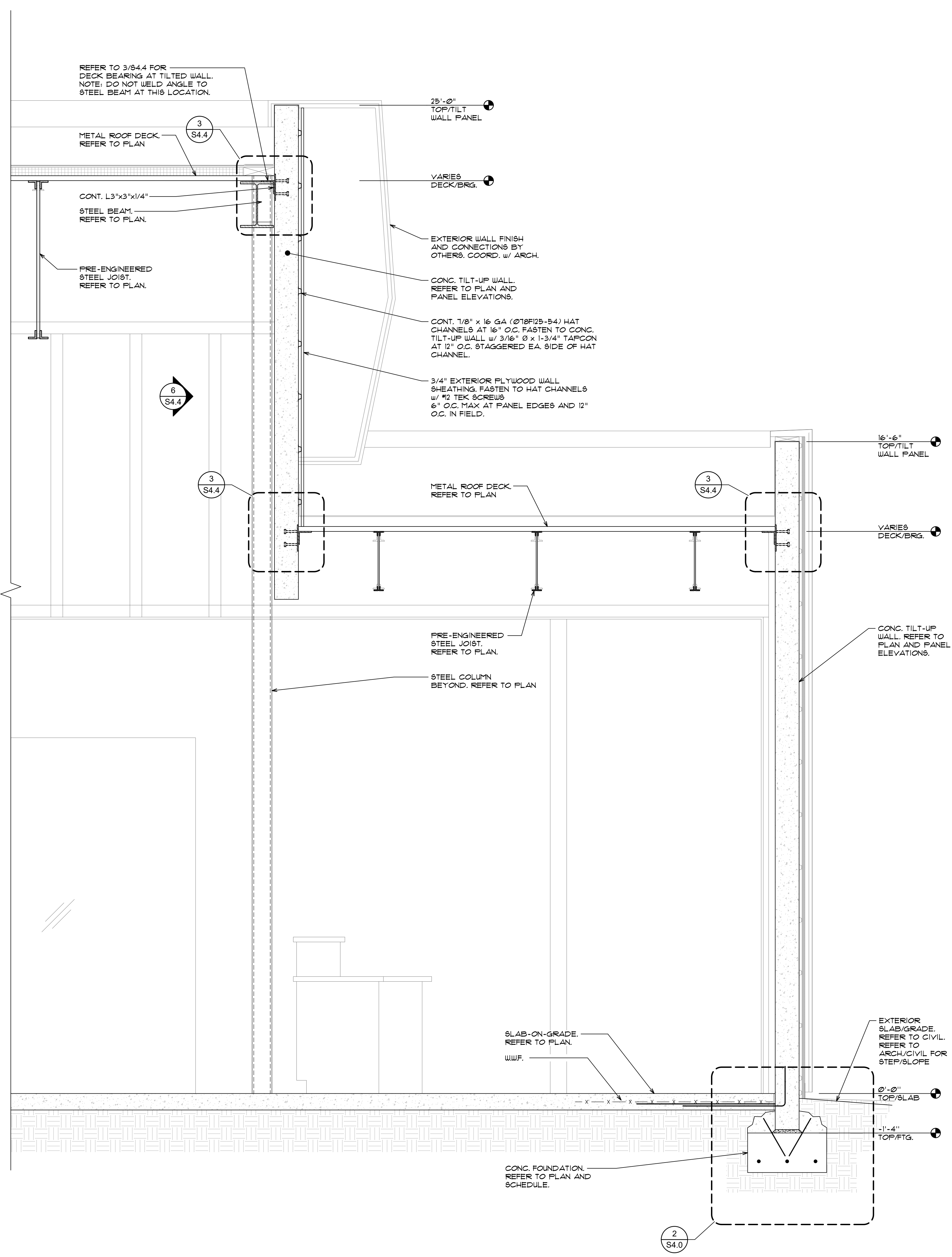
To the best of the knowledge of the architect & engineer, said plans and specifications comply with the applicable building codes and minimum fire safety standards.

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S3.1



SECTION SCALE 3/4"=1'-0" **2**



SECTION SCALE 3/4"=1'-0" **1**

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4750 Highway 520
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WALL SECTIONS

Revisions

Symbol	Description
△	3/7/2024 ADDENDUM NO. 1
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State Registration

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Florida #46665



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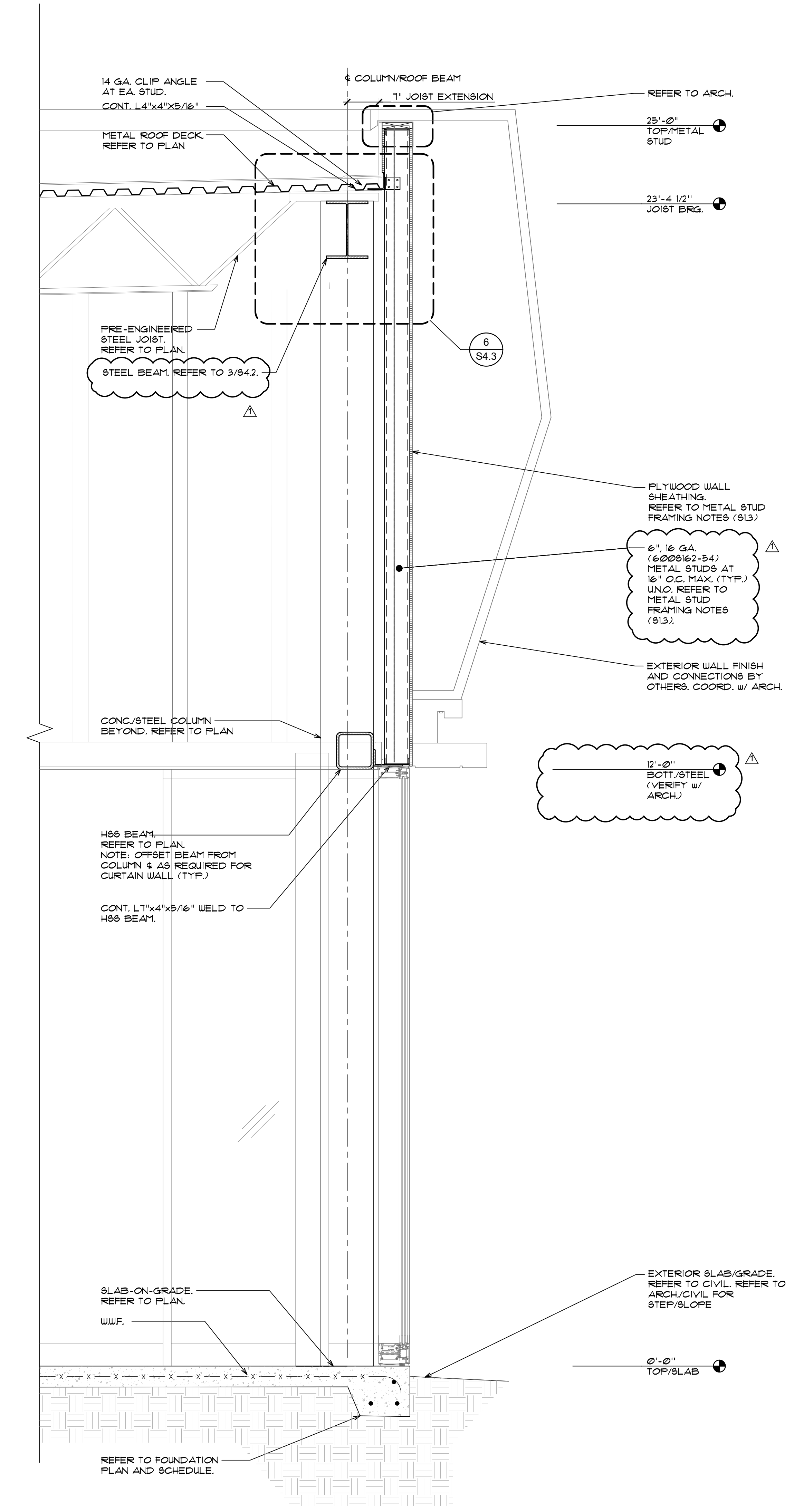
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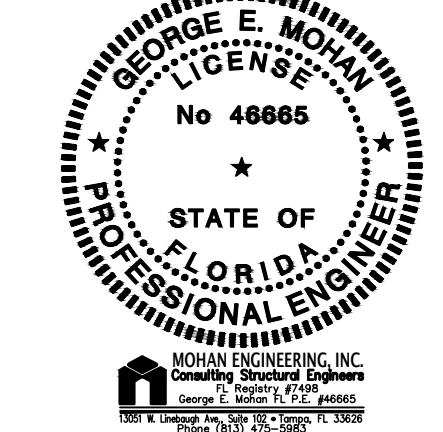
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SECTIONS & DETAILS

Revisions

Slate Registration

George E. Mohan, P.E.
Florida: #46665



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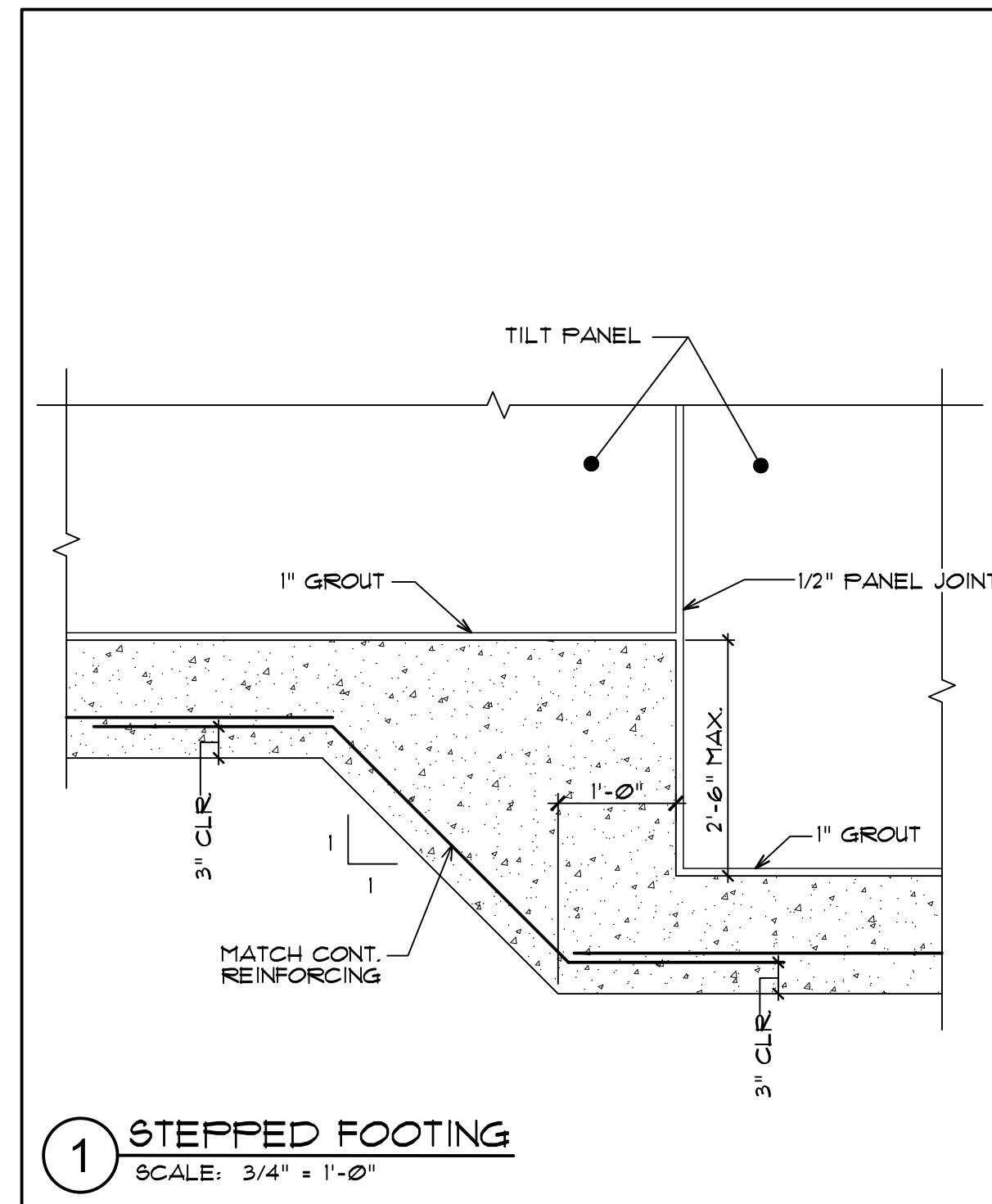
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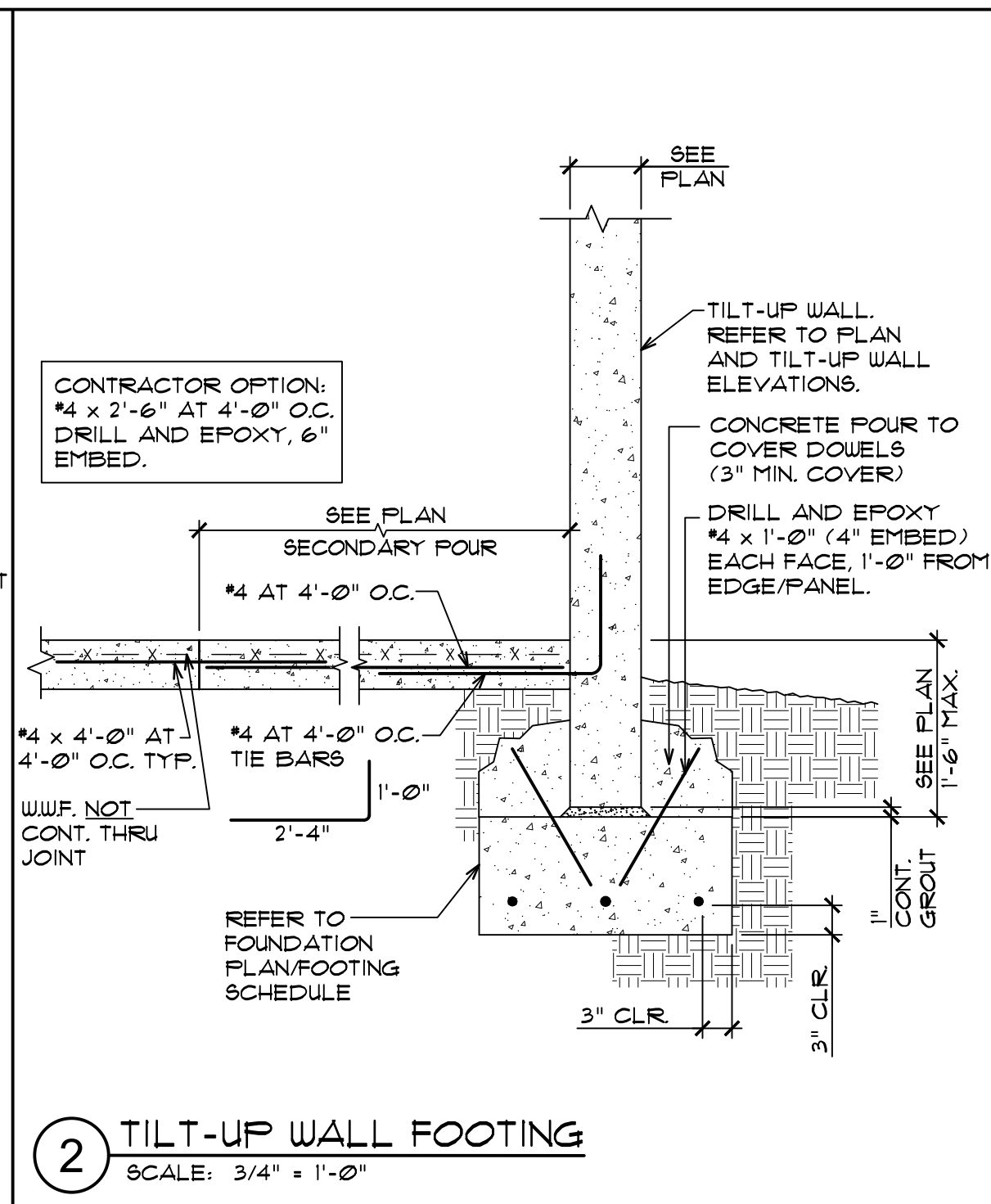
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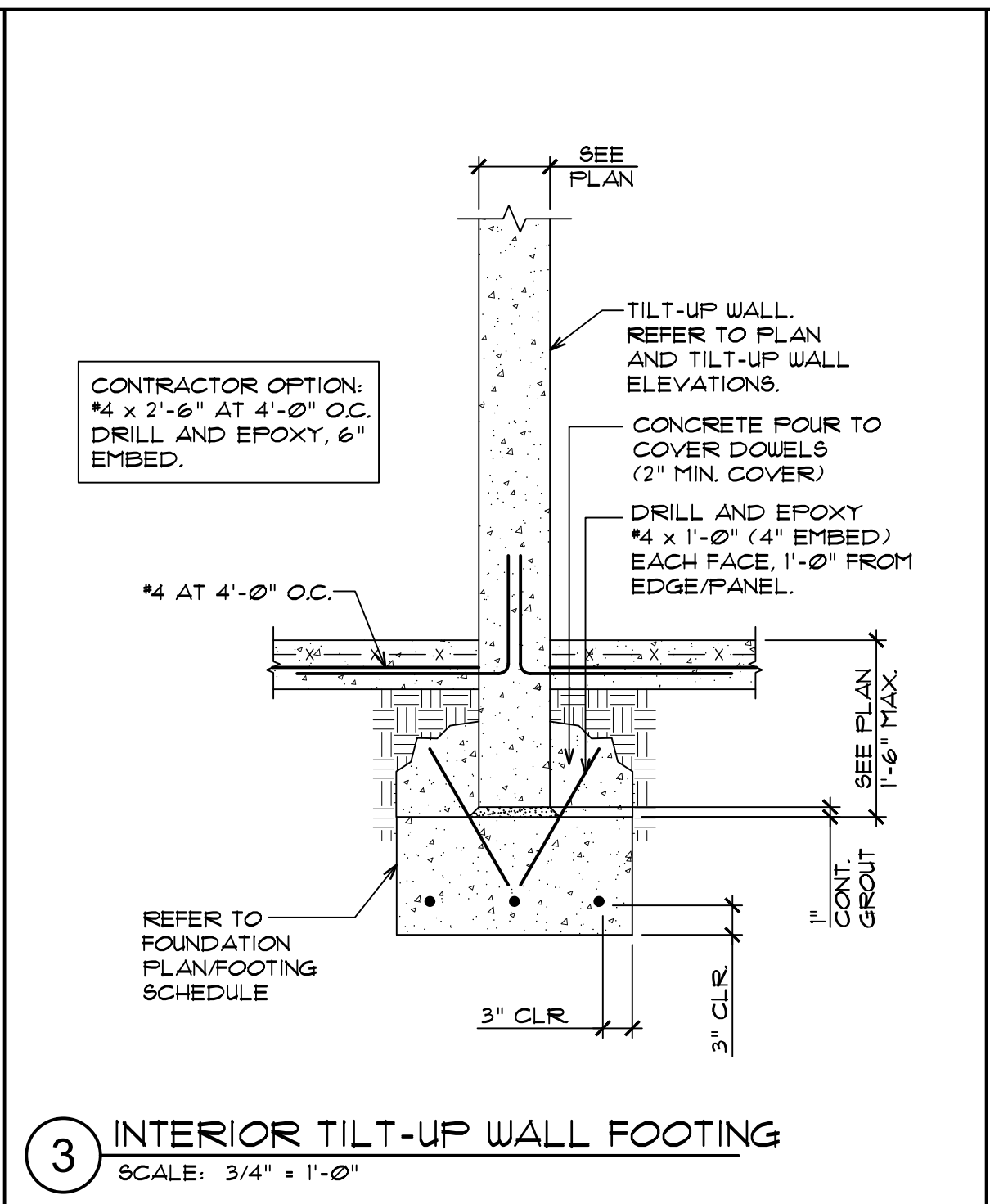
S4.0



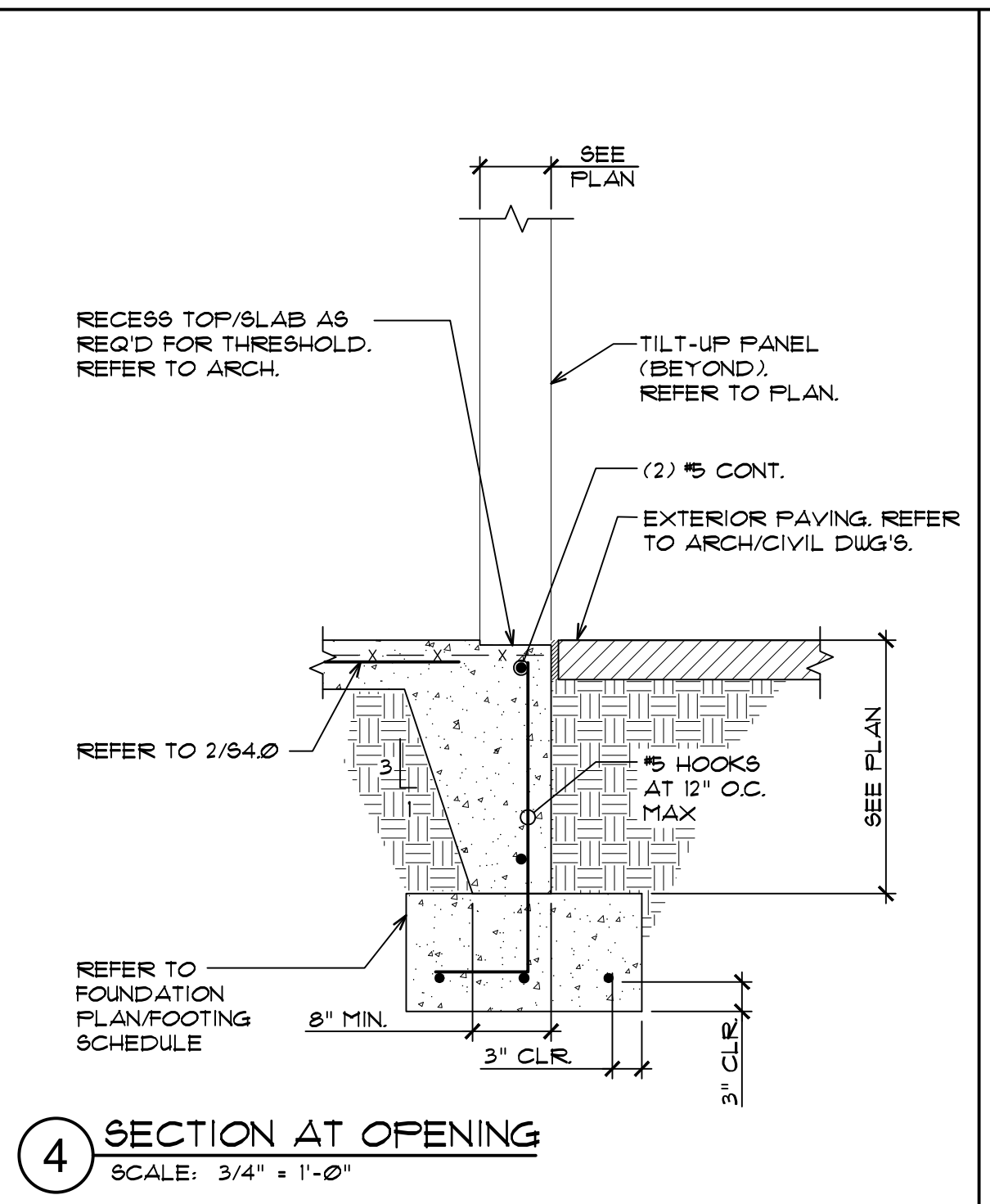
1 STEPPED FOOTING
SCALE: 3/4" = 1'-0"



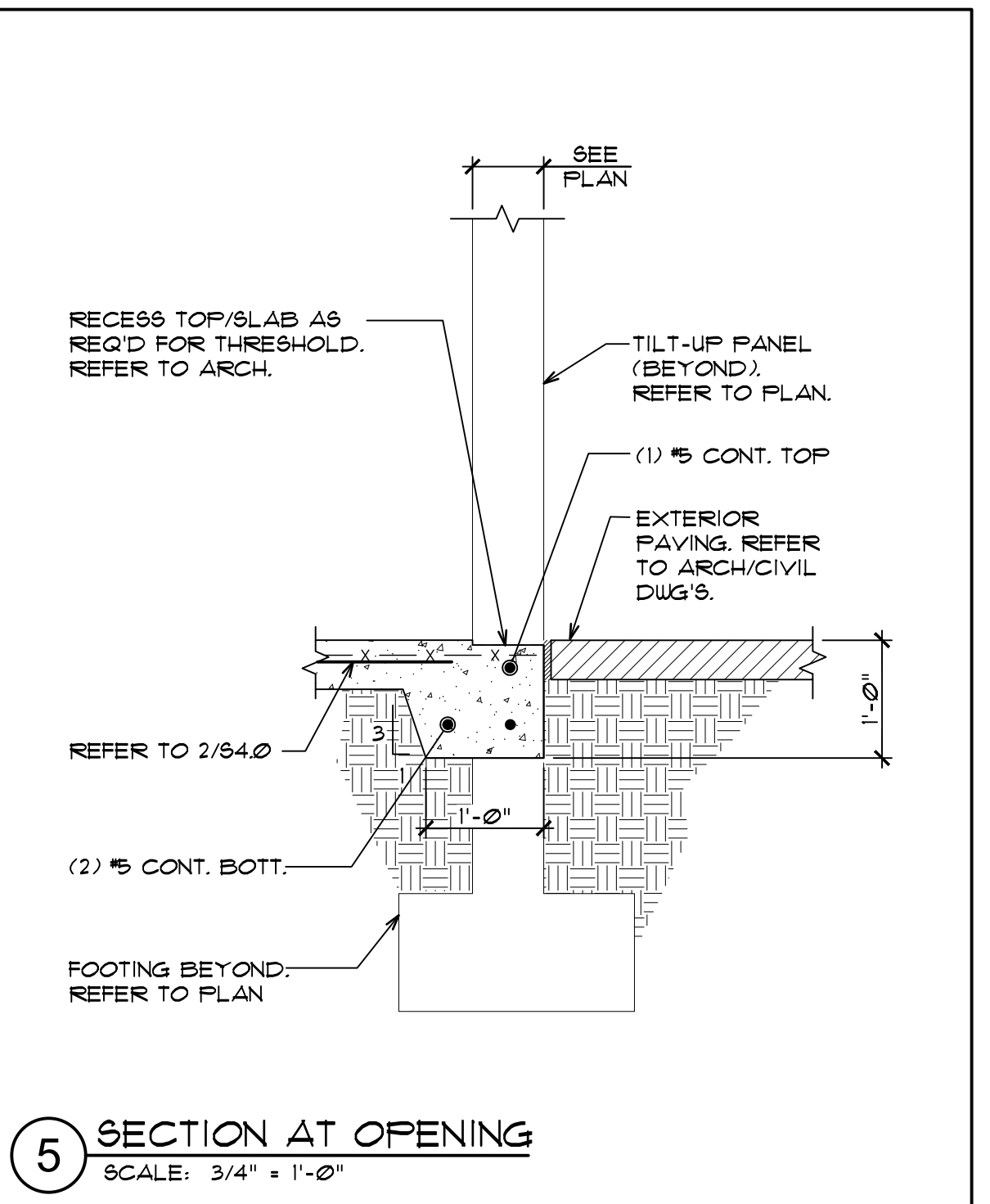
2 TILT-UP WALL FOOTING
SCALE: 3/4" = 1'-0"



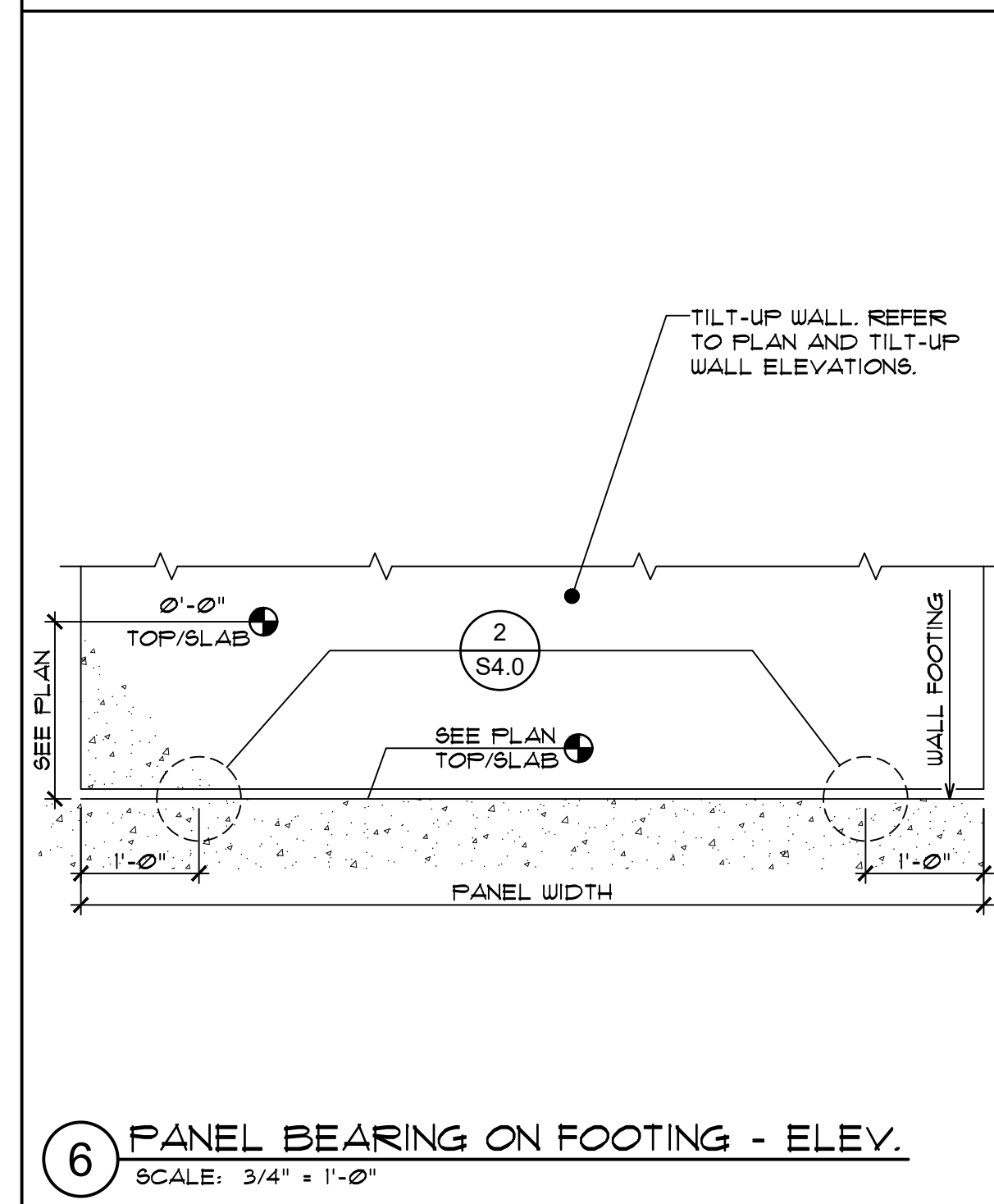
3 INTERIOR TILT-UP WALL FOOTING
SCALE: 3/4" = 1'-0"



4 SECTION AT OPENING
SCALE: 3/4" = 1'-0"



5 SECTION AT OPENING
SCALE: 3/4" = 1'-0"



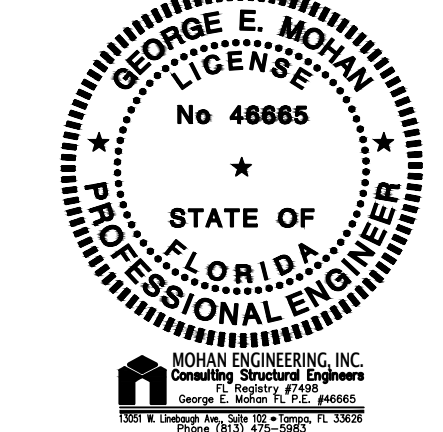
6 PANEL BEARING ON FOOTING - ELEV.
SCALE: 3/4" = 1'-0"

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SECTIONS & DETAILS

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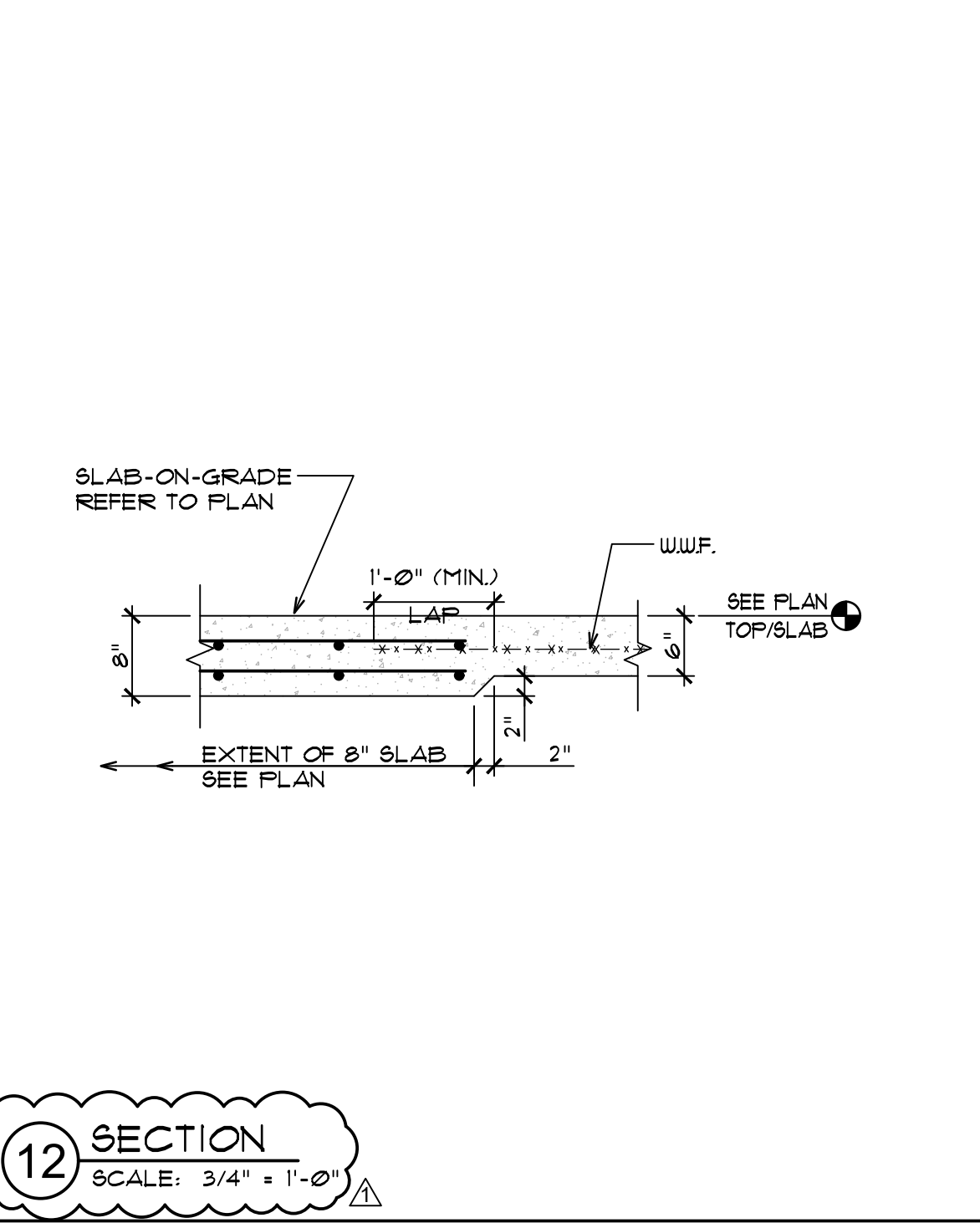
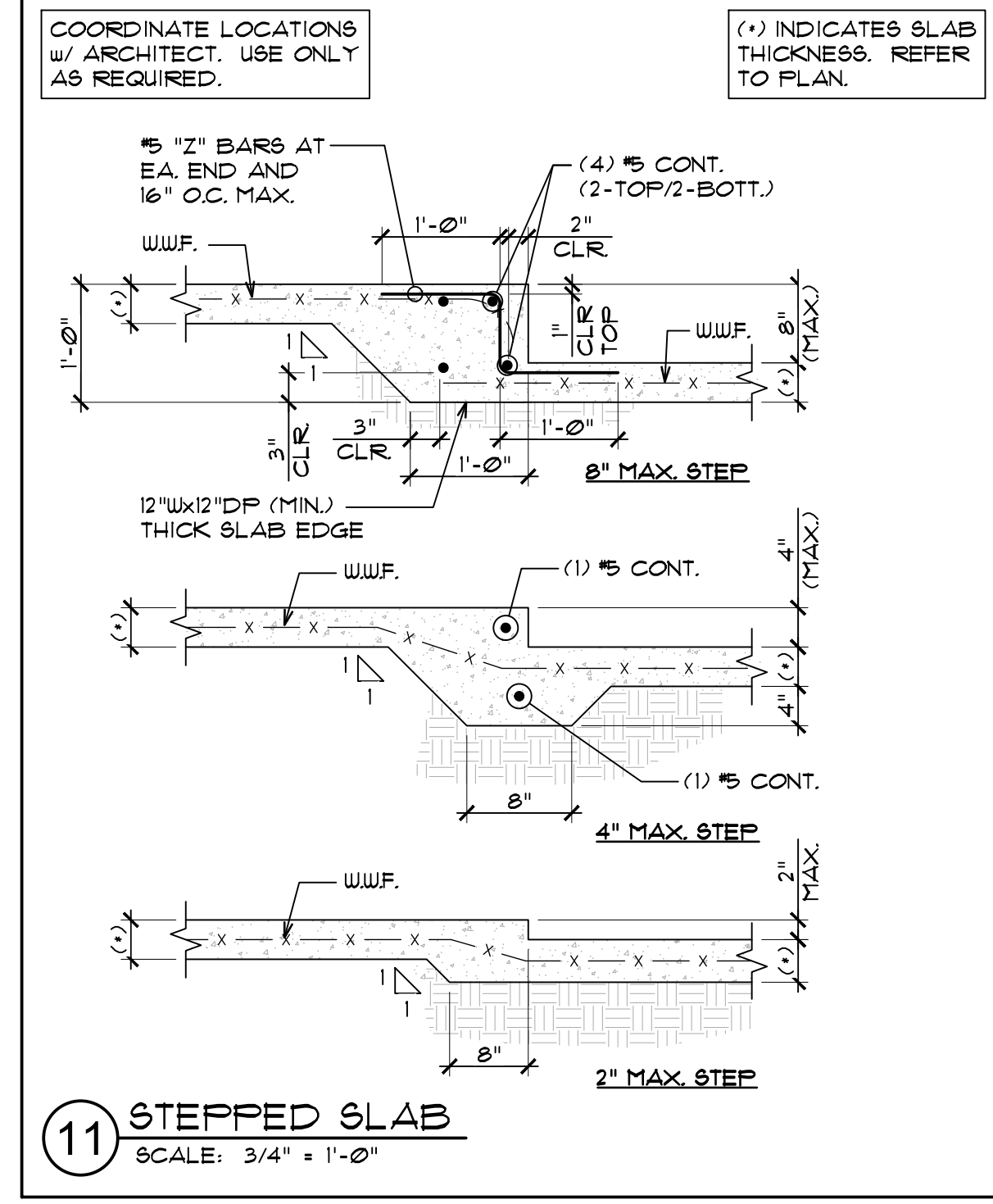
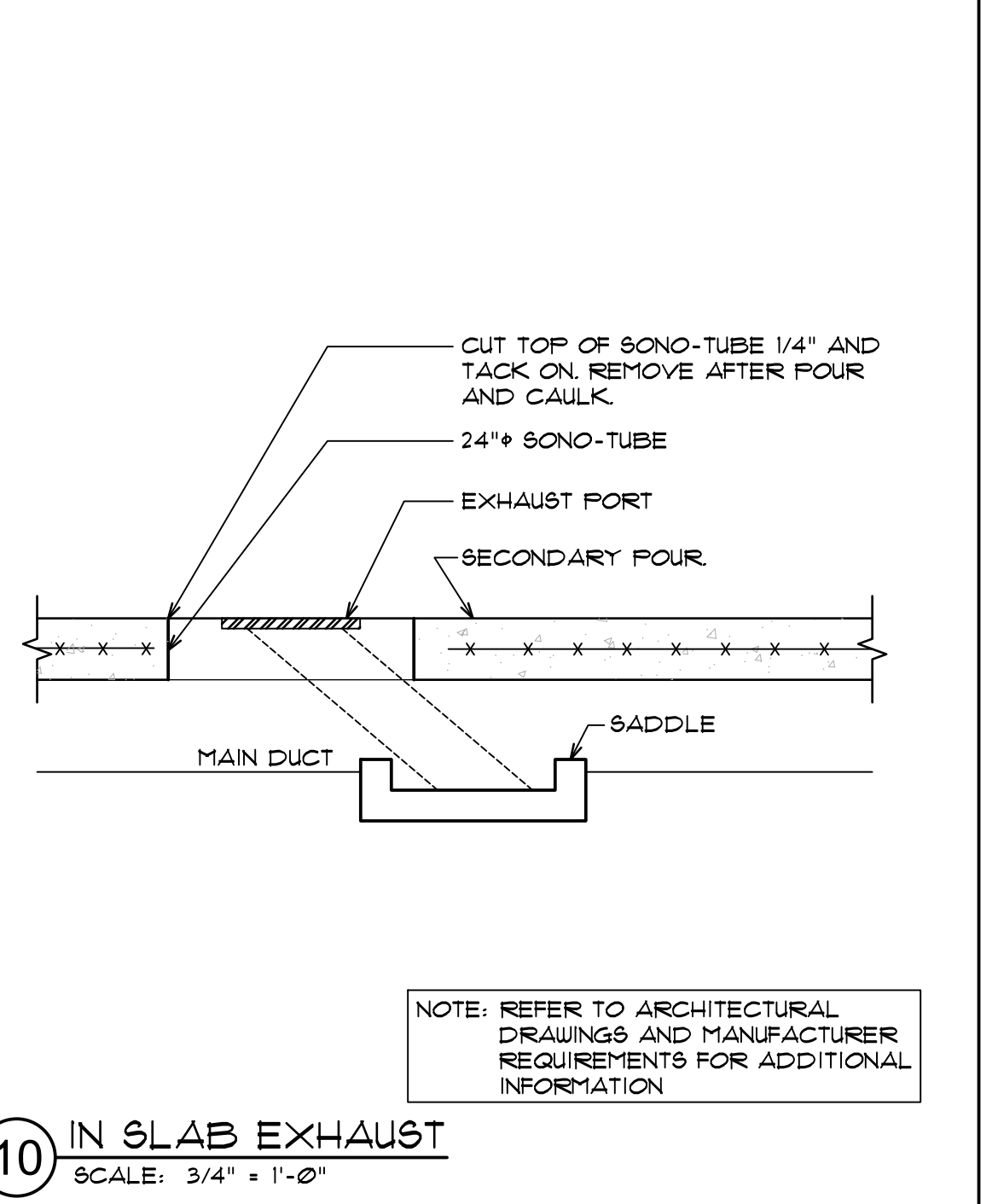
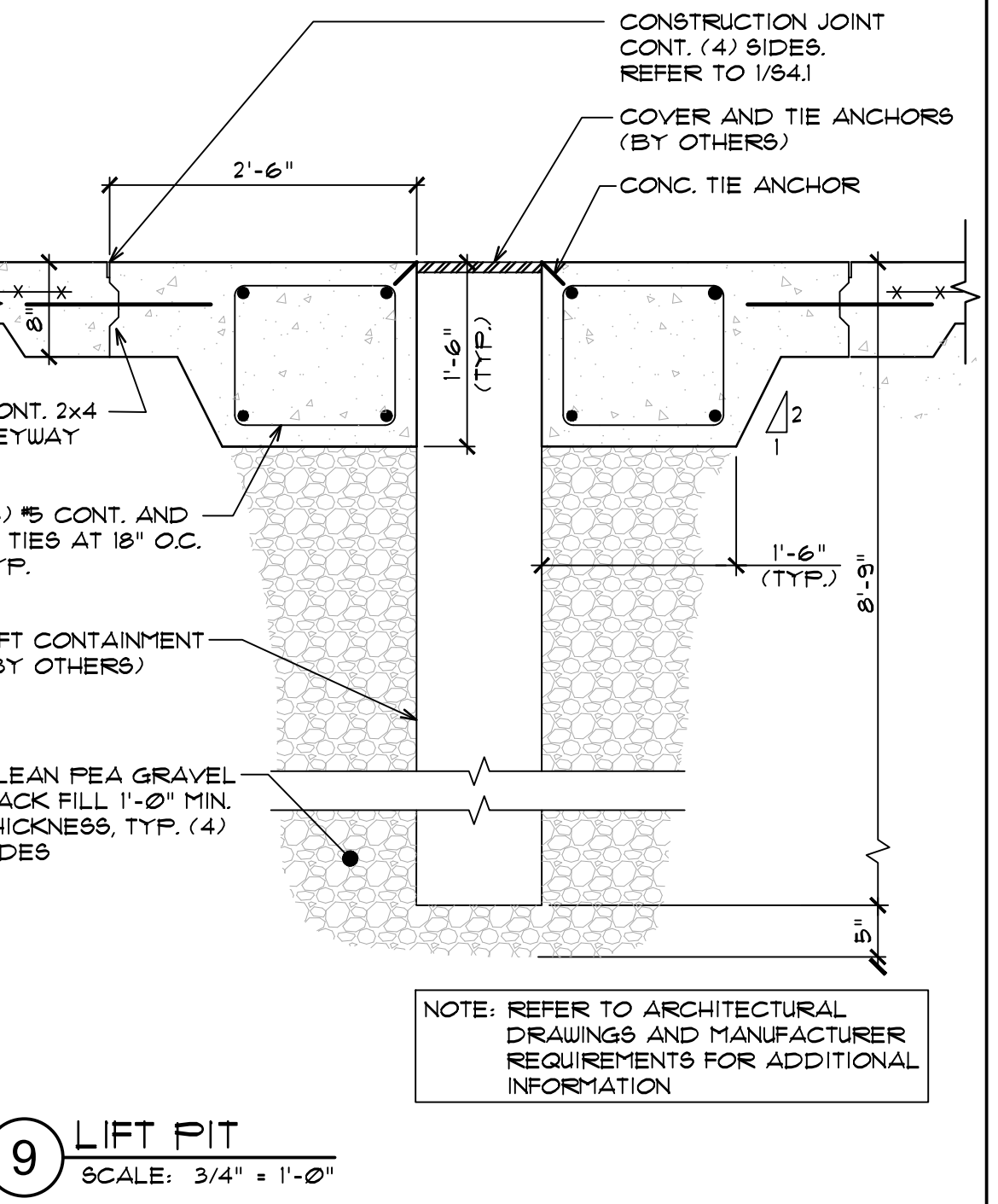
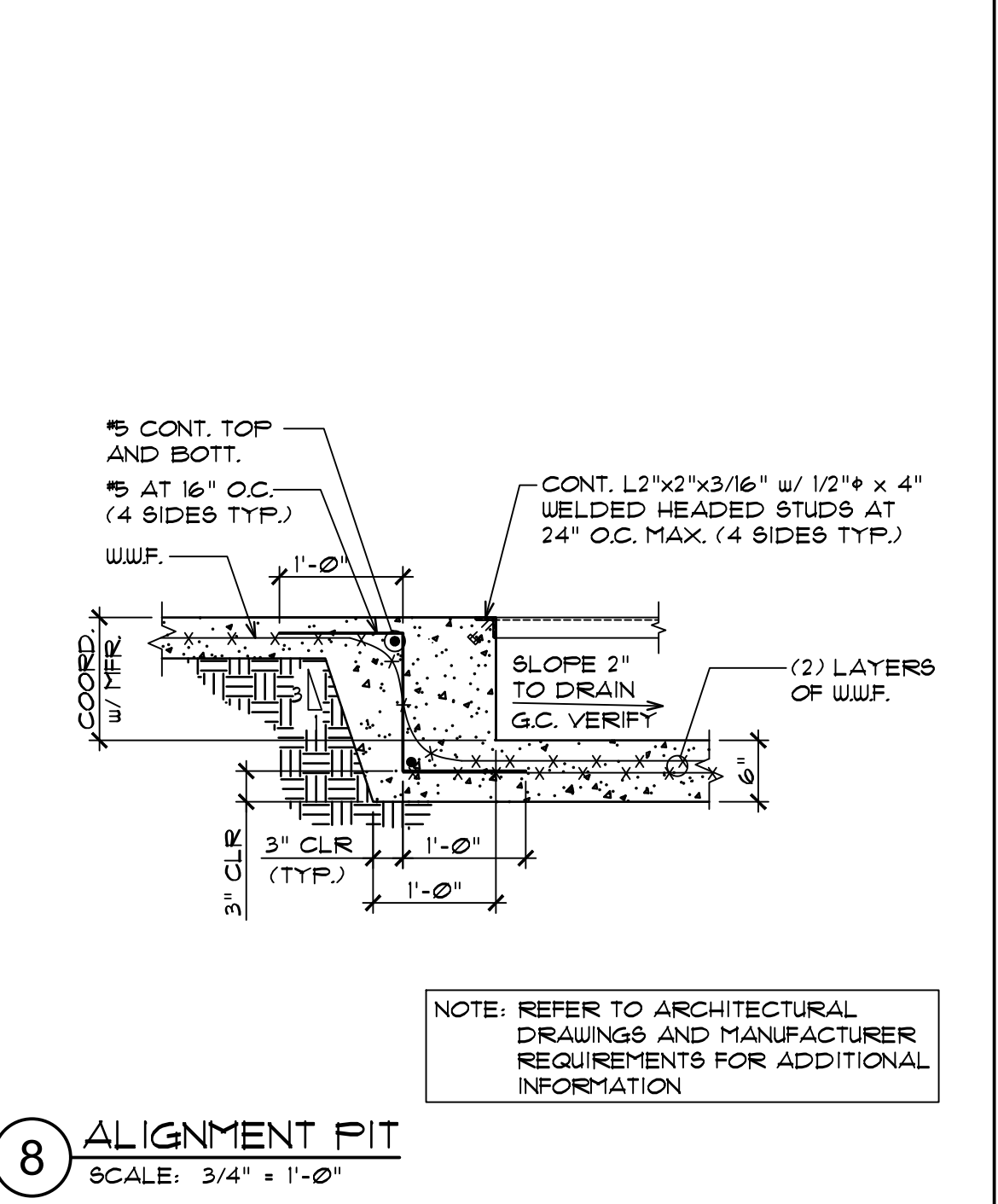
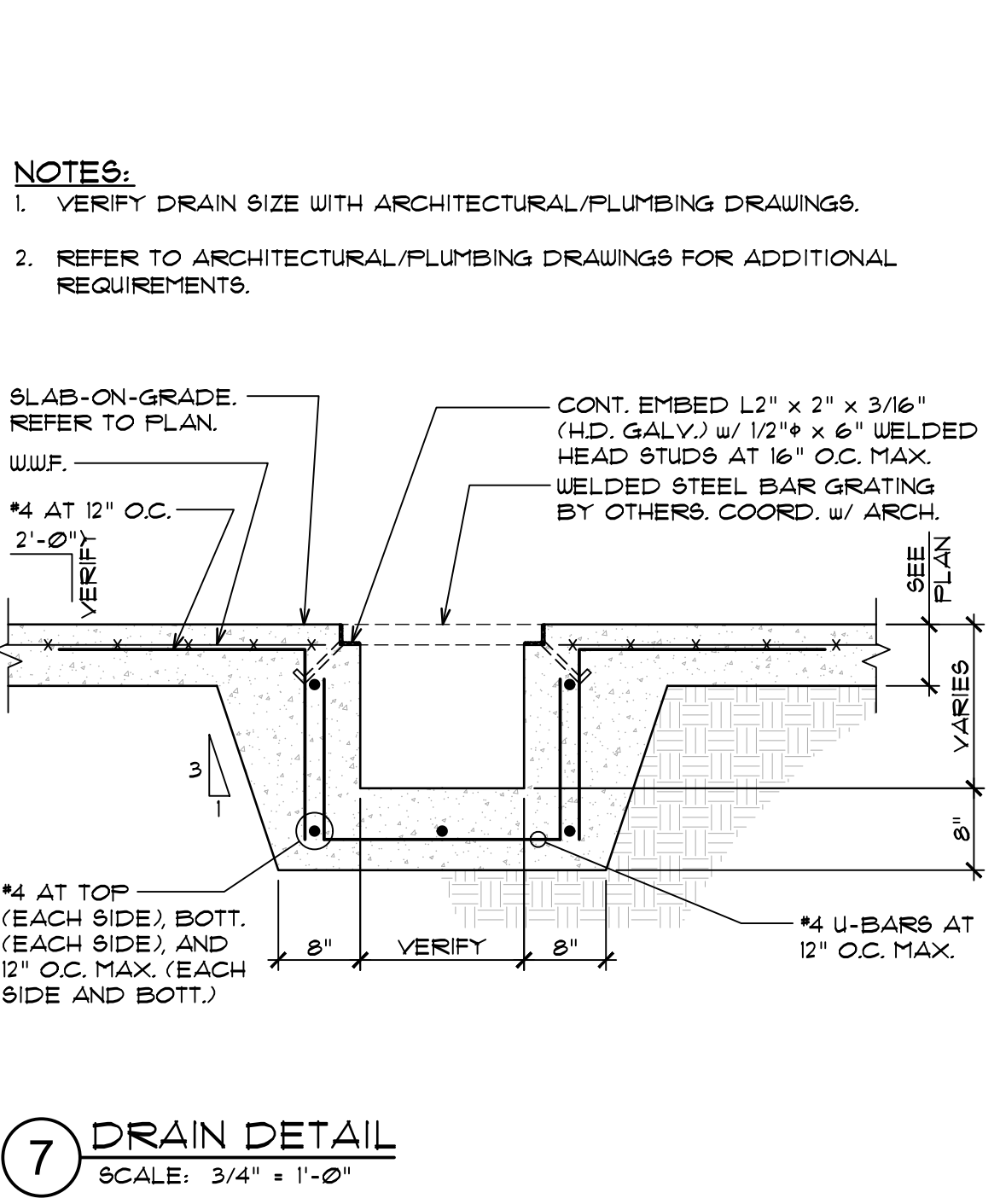
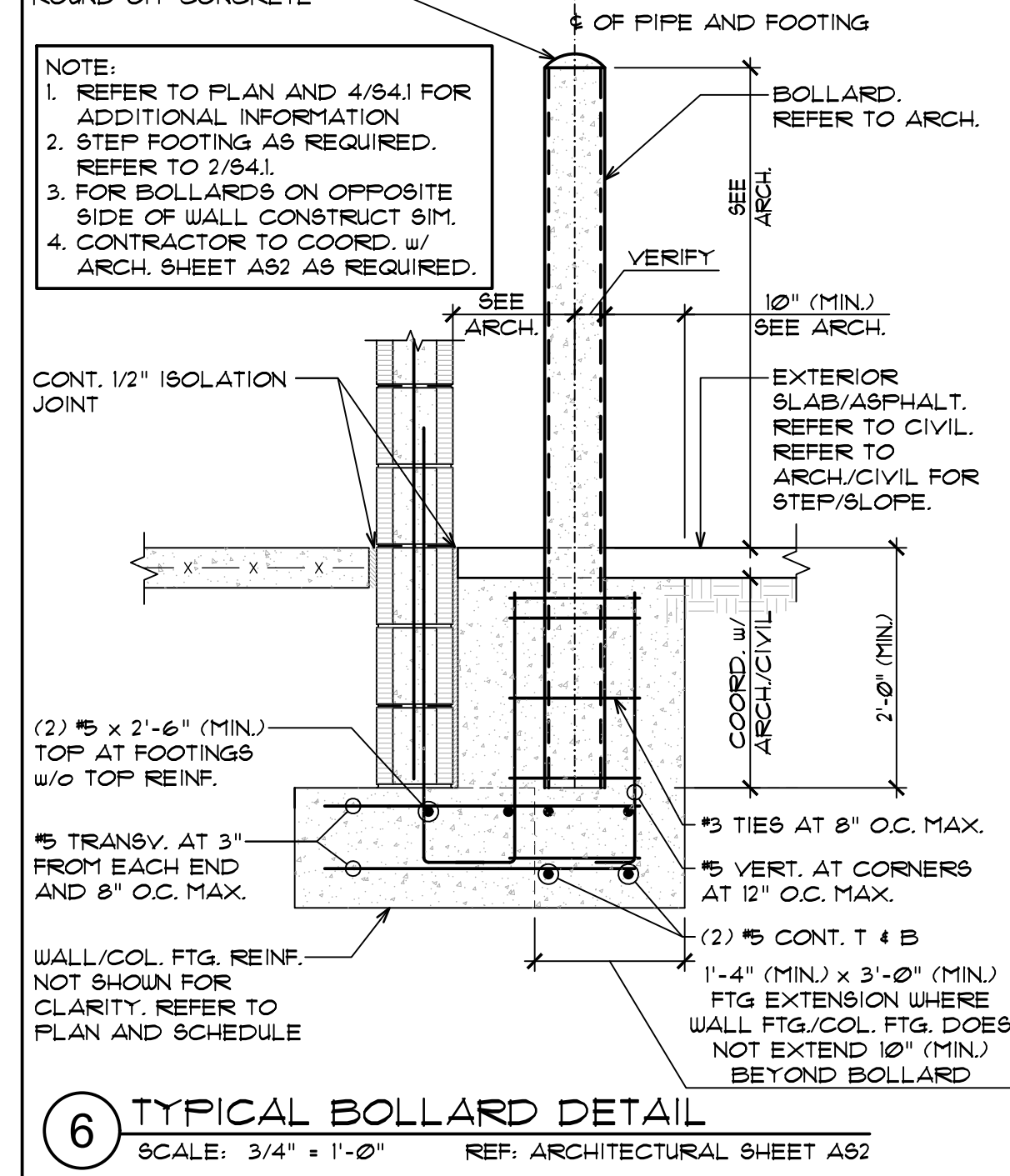
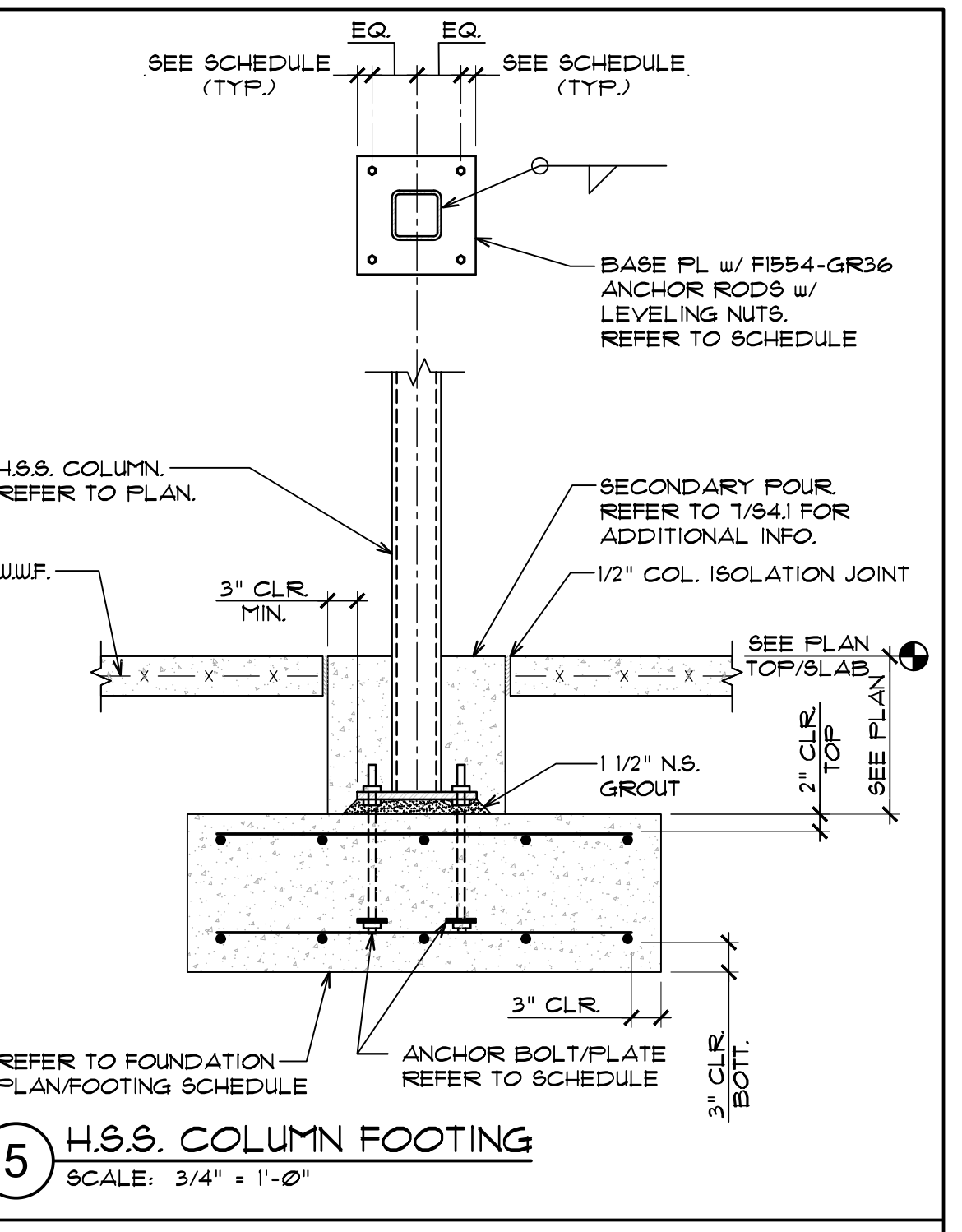
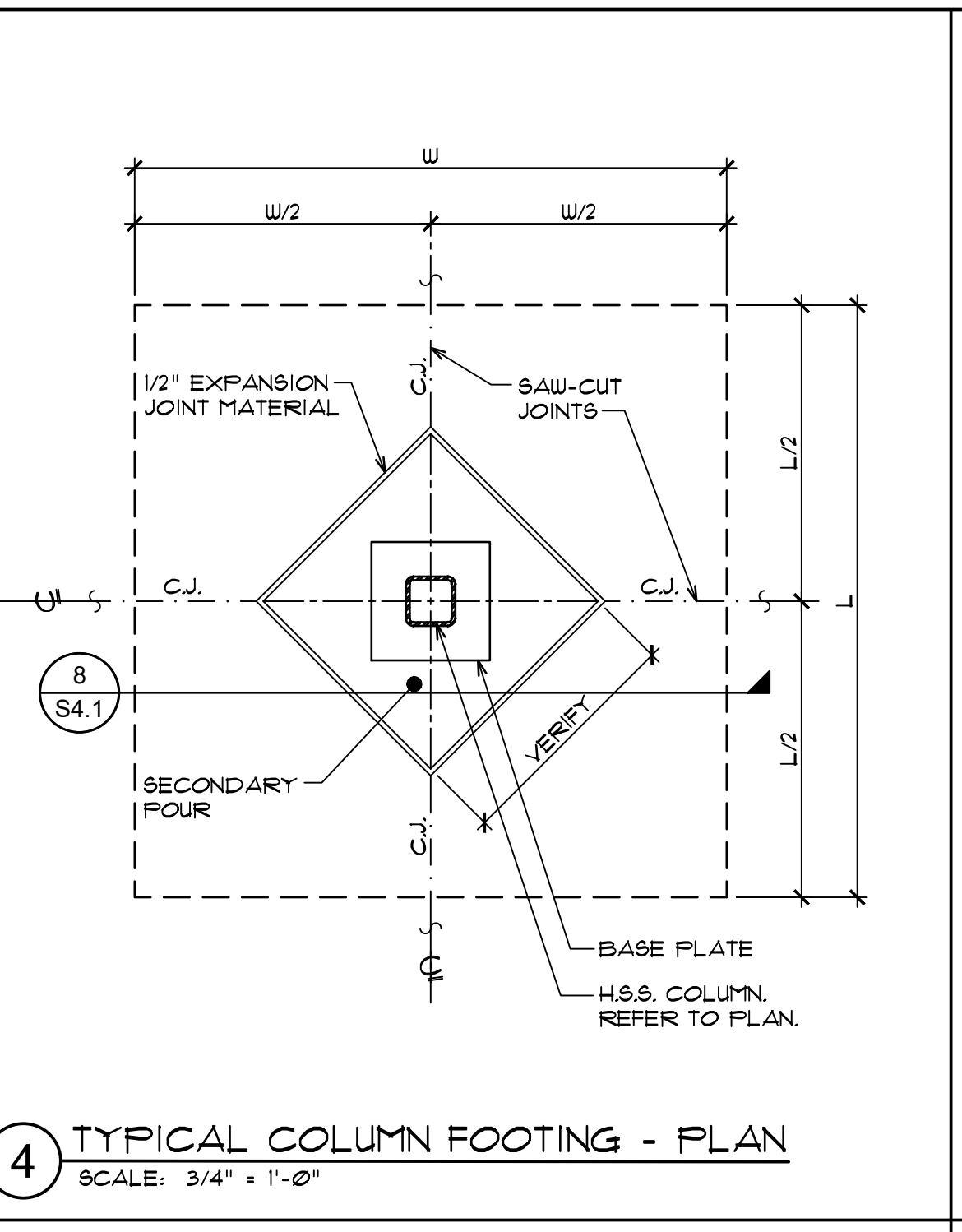
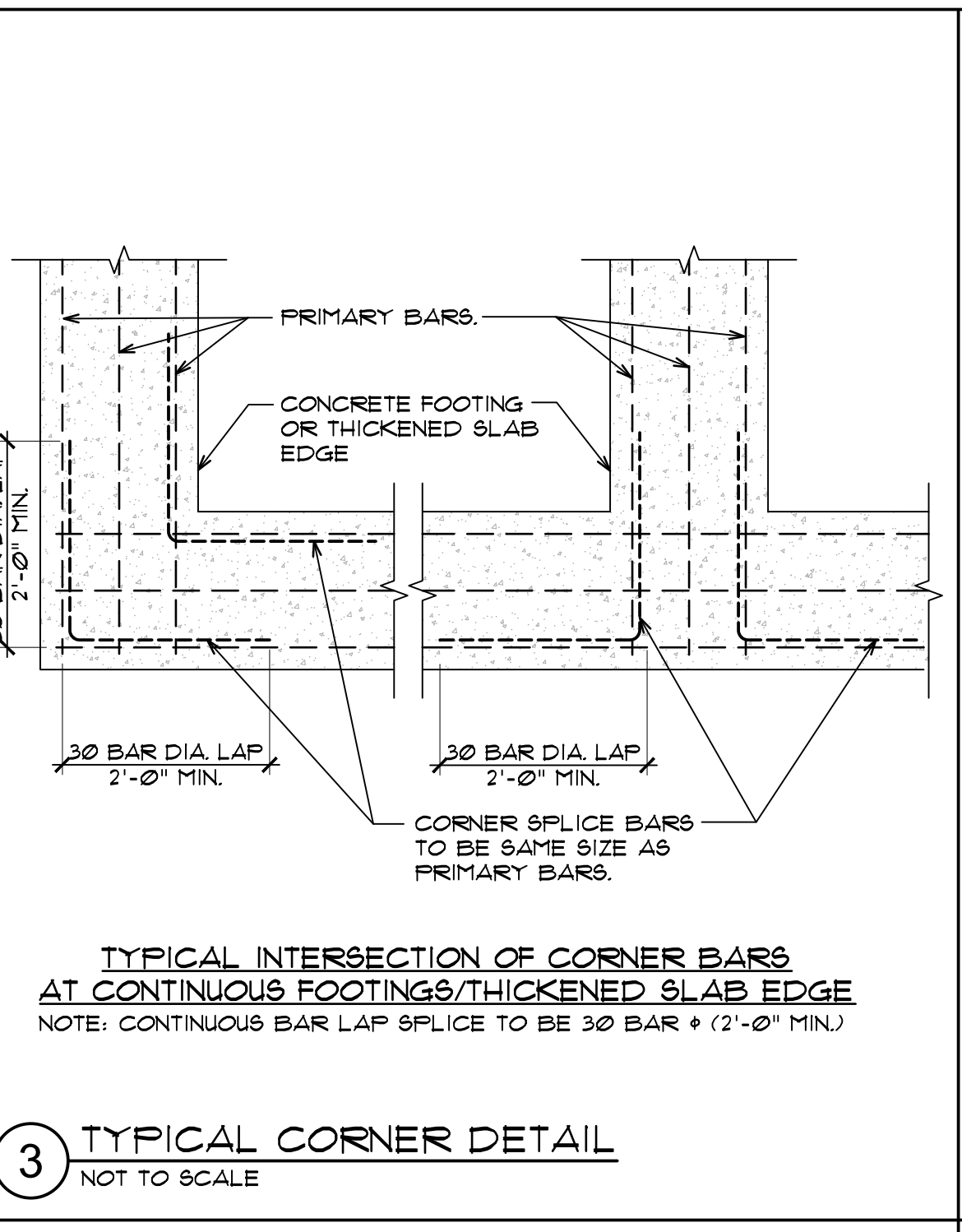
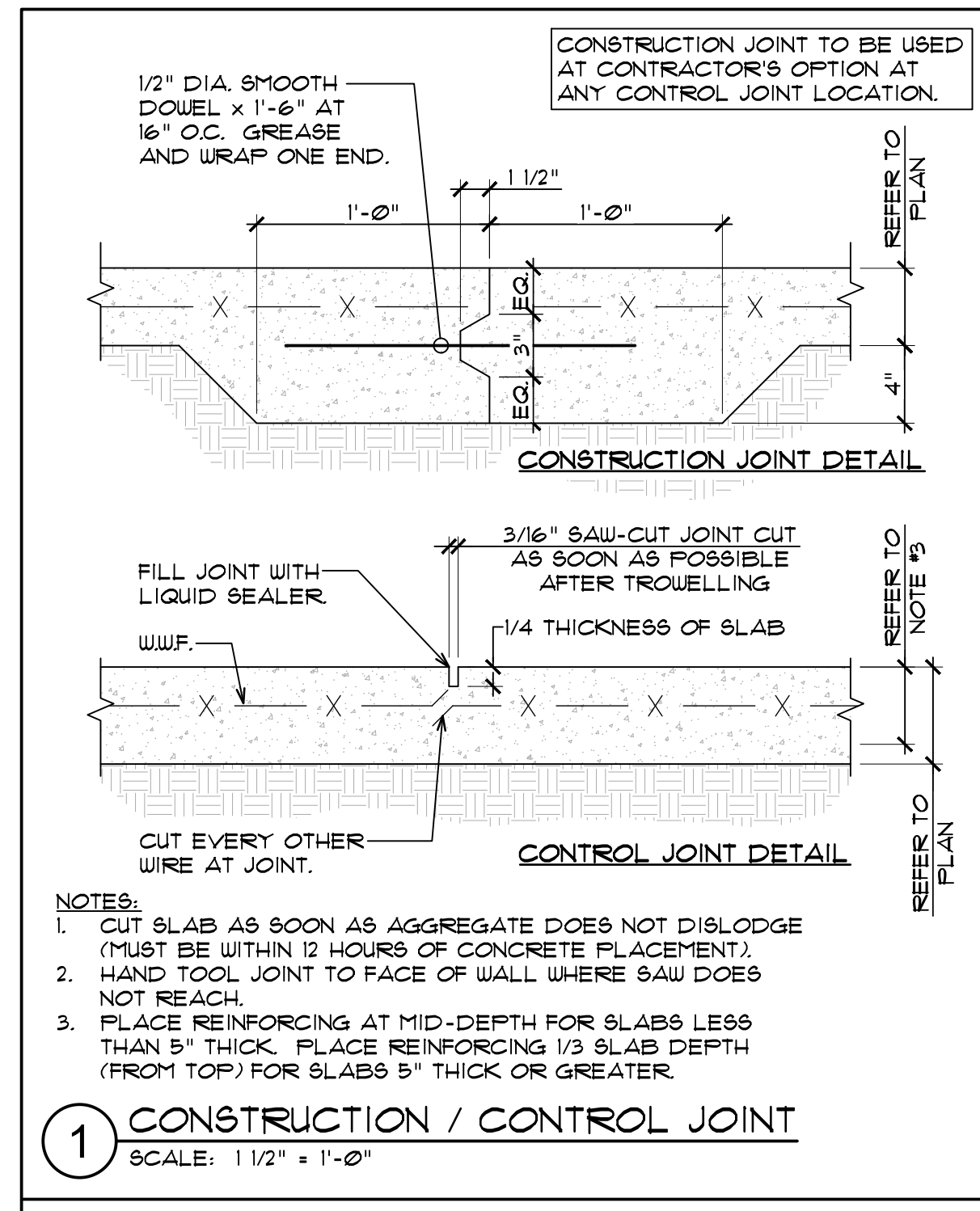
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S4.1

S4.1



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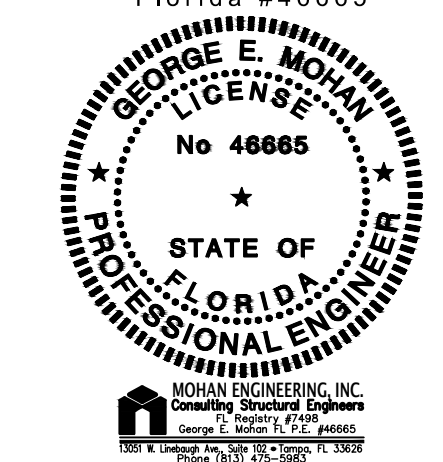
MIKE ERDMAN CADILLAC
4750 Highway 520
Cocoa, FL 32926

SECTIONS & DETAILS

Revisions

3/7/2024	ADDENDUM NO. 1

Slate Registration
George E. Mohan, P.E.
Florida: 44665

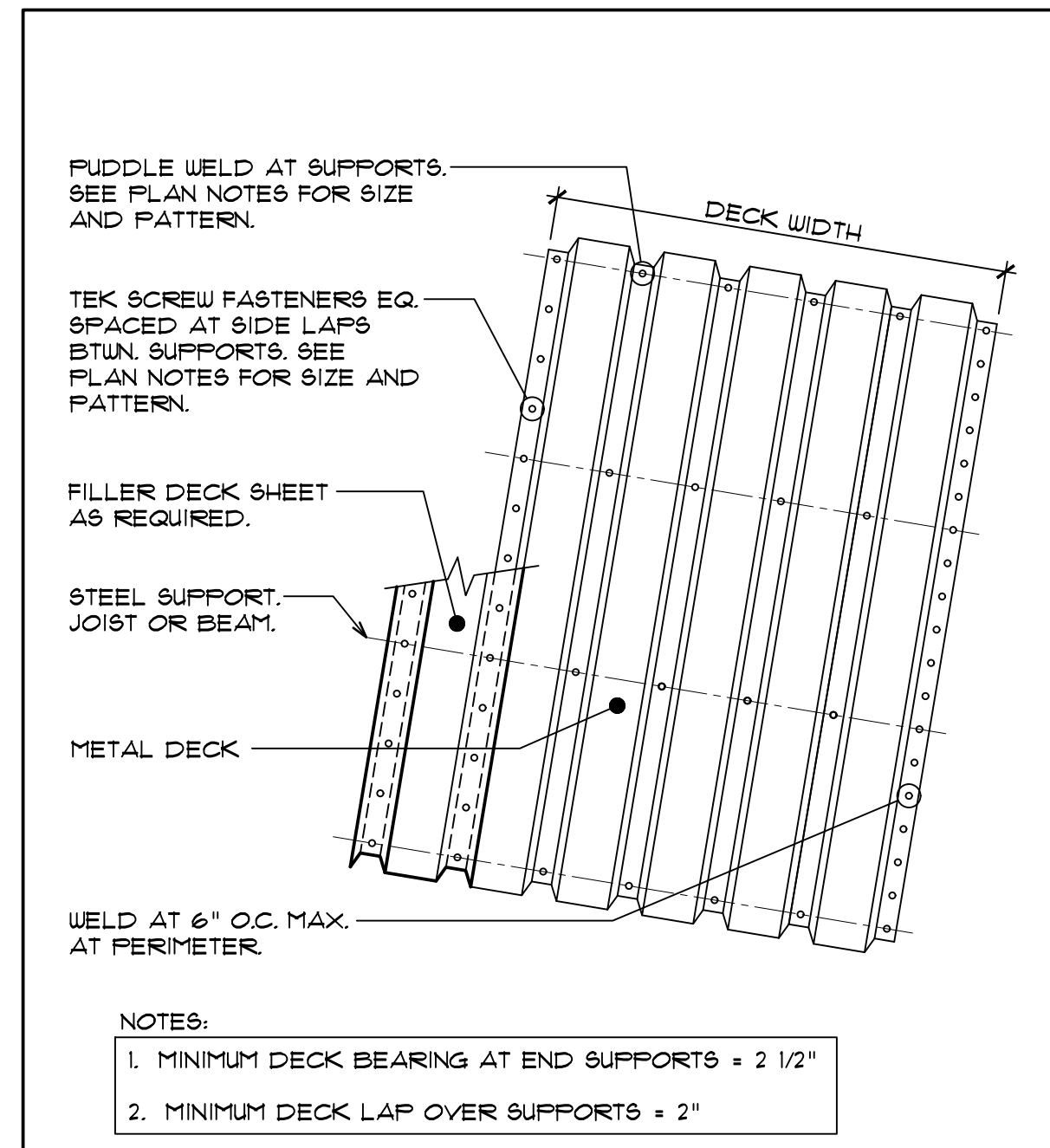


To the best of the knowledge of the architect & engineers, said plans and specifications comply with the applicable building codes and minimum fire safety standards.

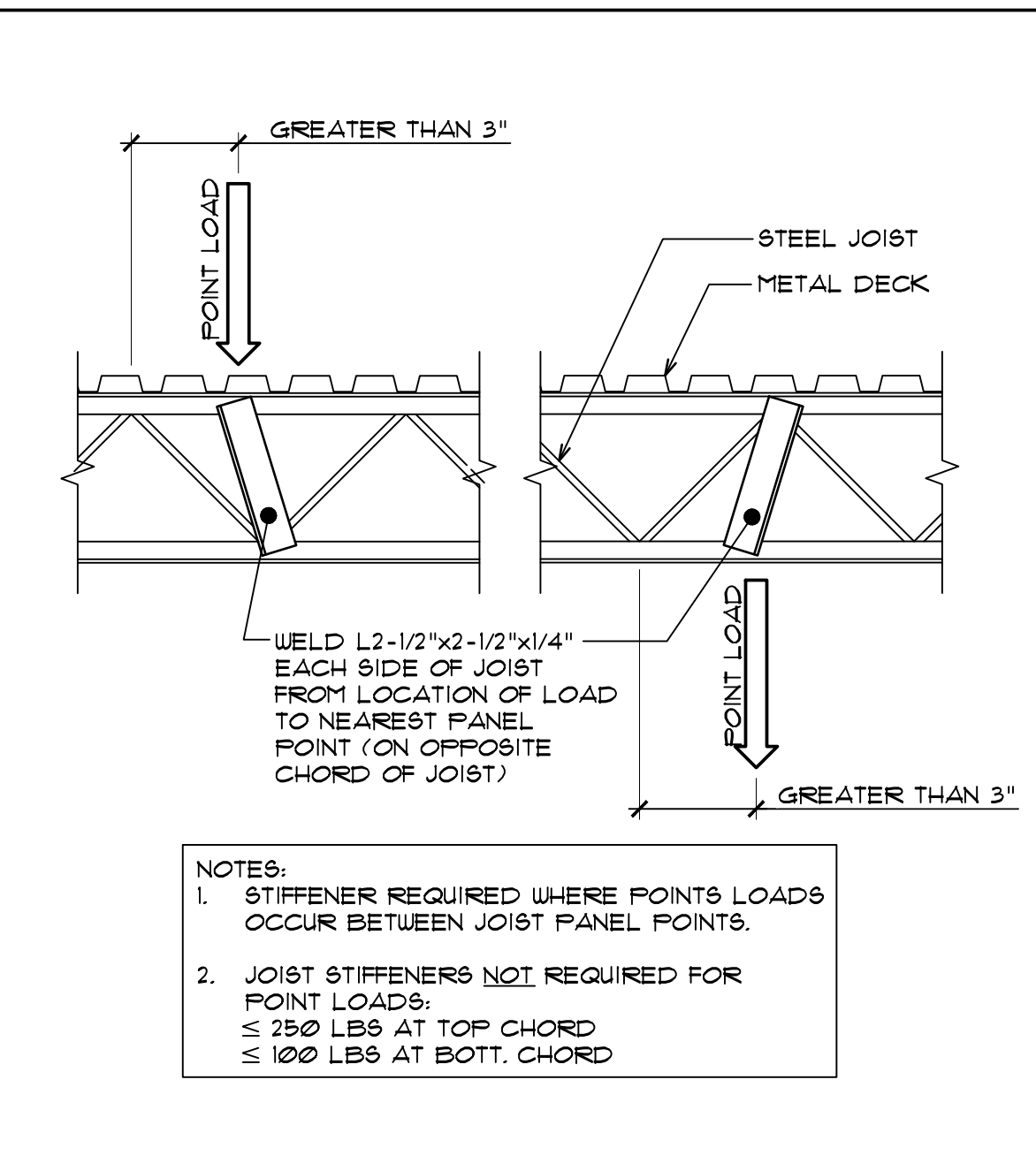
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Drawn By	YB / LD
Checked By	GEM
Scale	AS NOTED
Issue Date	02.09.24
Sheet Number	S4.2

S4.2

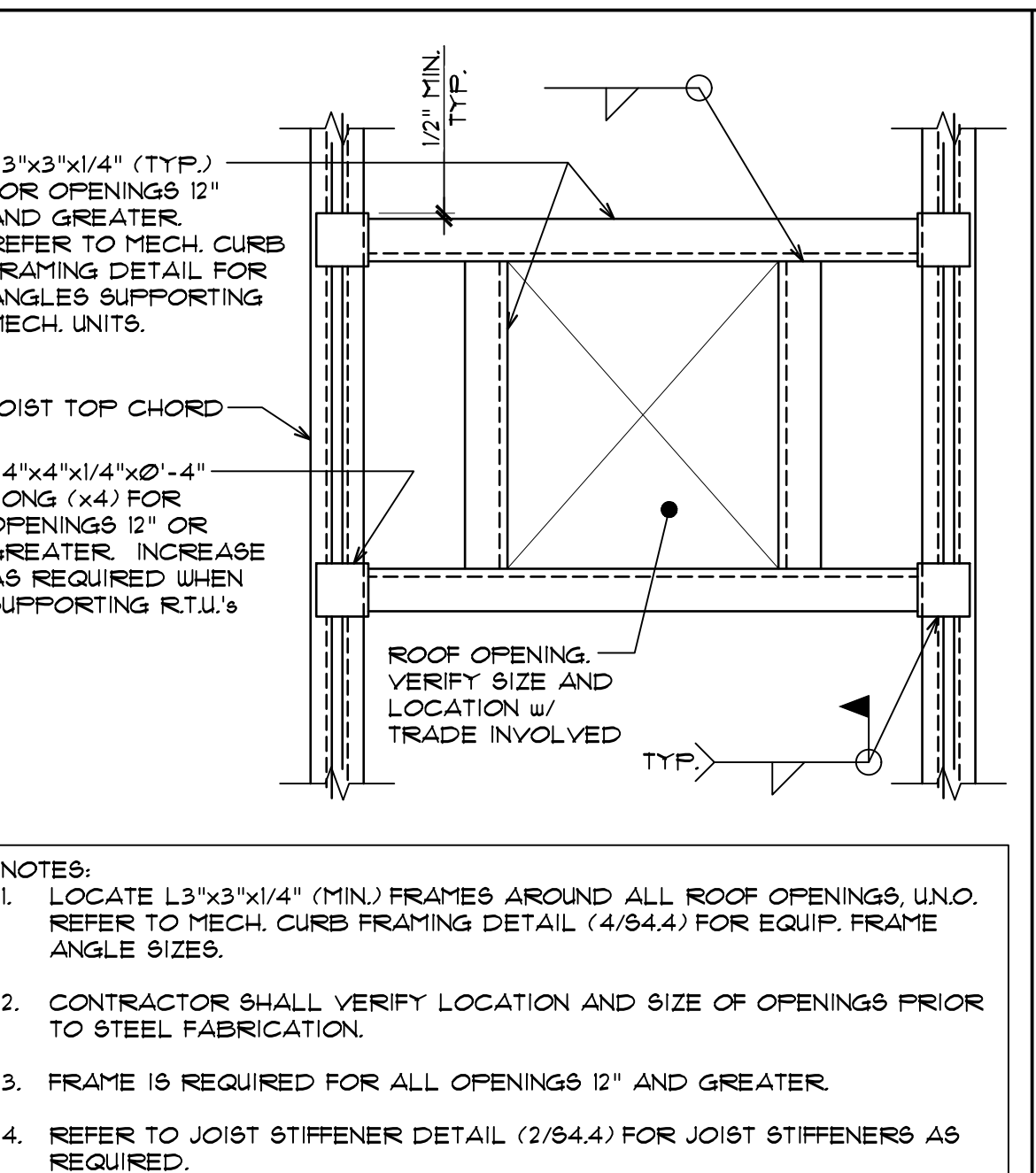
<table border="1"> <thead> <tr> <th>BEAM SIZE</th> <th>BOLTS</th> <th>PL LENGTH "H"</th> <th>NO. OF STUDS "G"</th> </tr> </thead> <tbody> <tr> <td>W8, C8, MC8</td> <td>2 ROWS</td> <td>6"</td> <td>4</td> </tr> <tr> <td>W10, C10, MC10</td> <td>2 ROWS</td> <td>6"</td> <td>4</td> </tr> <tr> <td>W12, C12, MC12</td> <td>3 ROWS</td> <td>9"</td> <td>6</td> </tr> <tr> <td>MC13</td> <td>3 ROWS</td> <td>9"</td> <td>6</td> </tr> <tr> <td>W14</td> <td>3 ROWS</td> <td>9"</td> <td>6</td> </tr> <tr> <td>C15</td> <td>4 ROWS</td> <td>12"</td> <td>8</td> </tr> <tr> <td>W16</td> <td>4 ROWS</td> <td>12"</td> <td>8</td> </tr> <tr> <td>W18, MC18</td> <td>4 ROWS</td> <td>12"</td> <td>8</td> </tr> <tr> <td>W21</td> <td>5 ROWS</td> <td>15"</td> <td>10</td> </tr> <tr> <td>W24</td> <td>6 ROWS</td> <td>18"</td> <td>12</td> </tr> <tr> <td>W27</td> <td>7 ROWS</td> <td>21"</td> <td>14</td> </tr> <tr> <td>W30</td> <td>8 ROWS</td> <td>24"</td> <td>16</td> </tr> </tbody> </table> <p>CONNECTION SHOWN FOR GRAPHICAL REPRESENTATION ONLY. REFER TO STRUCTURAL STEEL SECTION OF STRUCT. NOTES/SPEC. FOR ADDITIONAL INFORMATION.</p> <p>EMBED FL 1/2" w/ 3/4" DIA. HEADED STUDS AND 1/8" STEEL BARS (WELDED)</p> <p>WHERE STEEL BEAM DOES NOT FRAME INTO CONC. COLUMN OR BEAM, PROVIDE A 16" (MIN-DEEP) x 24" (MIN-LONG) CONC. TIE BEAM w/ (2) #5 TOP, BOT, AND MID w/ #3 TIES AT 8" O.C.</p> <p>NOTE: AT CONC. BEAM LOCATIONS, INSTALL ADDITIONAL MID. REINF./CLUSTERED TIES</p> <p>DOUBLE ANGLE BEAM CONNECTION w/ LONG SLOTTED HOLES. SHIP LOOSE AND TACK WELD NUT TO BOLT AFTER ERECTION. NOTE: SLOT BEAM WEB OR REFER TO STRUCT. NOTES/SPEC FOR COVER PLATE AT LONG-SLOTTED HOLES</p> <p>1/2" MIN. MAX. TOP AND BOTTOM</p> <p>1/2" MIN. TILT-UP WALL. REFER TO PLAN AND TILT-UP WALL ELEVATIONS.</p> <p>1 BOLT CLUSTER SCALE: 3/4" = 1'-0"</p>	BEAM SIZE	BOLTS	PL LENGTH "H"	NO. OF STUDS "G"	W8, C8, MC8	2 ROWS	6"	4	W10, C10, MC10	2 ROWS	6"	4	W12, C12, MC12	3 ROWS	9"	6	MC13	3 ROWS	9"	6	W14	3 ROWS	9"	6	C15	4 ROWS	12"	8	W16	4 ROWS	12"	8	W18, MC18	4 ROWS	12"	8	W21	5 ROWS	15"	10	W24	6 ROWS	18"	12	W27	7 ROWS	21"	14	W30	8 ROWS	24"	16	<p>LOCATE SHEAR PLATE AT FACE OF BEAM (OFFSET SHEAR PLATE/BOLT FROM WALL CENTERLINE AS REQ'D.)</p> <p>CONNECTION SHOWN FOR GRAPHICAL REPRESENTATION ONLY. REFER TO STRUCTURAL STEEL SECTION OF STRUCT. NOTES/SPEC. FOR ADDITIONAL INFORMATION.</p> <p>FACE OF WALL/COLUMN</p> <p>EMBED FL 1/2" x 2"-11" w/ 3/4" DIA. WELDED HEADED STUDS AND 1/8"x1/8" STEEL BARS (WELDED)</p> <p>WHERE STEEL BEAM DOES NOT FRAME INTO CONC. COL., BEAM, OR WALL, PROVIDE CONC. TIEBEAM (MIN. OF 16" DEEP x 48" LONG w/ (2) #5 TOP, MID, BOT, w/ #3 TIES AT 8" O.C. (AT CONC. BM LOCATIONS ADD MID. REINF. AND TIES TO MEET THESE REQUIREMENTS.)</p> <p>SINGLE SHEAR PLATE BEAM CONNECTION w/ 3/4" DIA. A325 BOLTS IN SHORT-SLOTTED HOLES. SHIP LOOSE AND TACK WELD NUT TO BOLT AFTER ERECTION. SHEAR PLATE THICKNESS TO MATCH BEAM WEB (MIN.). REFER TO 1/843 FOR ADDITIONAL INFORMATION.</p> <p>THIS CONNECTION ONLY TO BE USED WHERE DOUBLE ANGLE CONNECTION WILL NOT FIT (CORNER END OF WALL, ETC.)</p> <p>2 BOLT CLUSTER SCALE: 1" = 1'-0"</p>	<p>CONNECTION SHOWN FOR GRAPHICAL REPRESENTATION ONLY. REFER TO STRUCTURAL STEEL SECTION OF STRUCT. NOTES/SPEC FOR ADDITIONAL INFORMATION.</p> <p>WELDED CAP FL 1/2" AT TOP OF COLUMN. NOTE: REFER TO PLAN FOR LOCATIONS w/ BEAMS/JOIST PERPENDICULAR EXTEND COLUMN/CAP 2-1/2" AS REQUIRED.</p> <p>STEEL BEAM. REFER TO PLAN.</p> <p>SLOTTED SHEAR PLATE CONN. AT COLUMN w/ 3/4" DIA. A325 BOLTS IN 13/16" DIA. HOLES (NOTE: BOLTS IN SINGLE SHEAR). SHEAR PLATE THICKNESS TO MATCH w/ BEAM WEB (MIN.)</p> <table border="1"> <thead> <tr> <th>BEAM SIZE</th> <th>BOLTS</th> </tr> </thead> <tbody> <tr> <td>W8, C8, MC8</td> <td>2 ROWS</td> </tr> <tr> <td>W10, C10, MC10</td> <td>2 ROWS</td> </tr> <tr> <td>W12, C12, MC12</td> <td>3 ROWS</td> </tr> <tr> <td>MC13</td> <td>3 ROWS</td> </tr> <tr> <td>W14</td> <td>3 ROWS</td> </tr> <tr> <td>C15</td> <td>4 ROWS</td> </tr> <tr> <td>W16</td> <td>4 ROWS</td> </tr> <tr> <td>W18, MC18</td> <td>4 ROWS</td> </tr> <tr> <td>W21</td> <td>5 ROWS</td> </tr> <tr> <td>W24</td> <td>6 ROWS</td> </tr> <tr> <td>W27</td> <td>7 ROWS</td> </tr> <tr> <td>W30</td> <td>8 ROWS</td> </tr> </tbody> </table> <p>HSS COLUMN. REFER TO PLAN.</p> <p>* FILLET WELD = 1/16" LESS THAN SOLID WALL THICKNESS</p> <p>3 BEAM-COLUMN CONNECTION SCALE: 3/4" = 1'-0"</p>	BEAM SIZE	BOLTS	W8, C8, MC8	2 ROWS	W10, C10, MC10	2 ROWS	W12, C12, MC12	3 ROWS	MC13	3 ROWS	W14	3 ROWS	C15	4 ROWS	W16	4 ROWS	W18, MC18	4 ROWS	W21	5 ROWS	W24	6 ROWS	W27	7 ROWS	W30	8 ROWS	<p>CONNECTION SHOWN FOR GRAPHICAL REPRESENTATION ONLY. REFER TO STRUCTURAL STEEL SECTION OF STRUCT. NOTES/SPEC ON S11 FOR ADDITIONAL INFORMATION.</p> <p>L4"x4"x1/4" x 0'-4" TOP & BOT.</p> <p>HSS BEAM. REFER TO PLAN.</p> <p>EMBED 1/2"x21"x0'-11" w/ (6) PAIRS OF 3/4" WELDED HEADED STUDS (1-3/4" GAGE) AND 1/8"x1/4" STEEL BARS (WELDED)</p> <p>CONC. COLUMN. REFER TO PLAN.</p> <p>3-SIDES TYP.</p> <p>3" (TYP.)</p> <p>4 HSS BEAM CONNECTION SCALE: 1" = 1'-0"</p>	<p>CONNECTION SHOWN FOR GRAPHICAL REPRESENTATION ONLY. REFER TO STRUCTURAL STEEL SECTION OF STRUCT. NOTES/SPEC ON S11 FOR ADDITIONAL INFORMATION.</p> <p>EMBED 1/2"x21"x0'-11" w/ (5) PAIRS OF 3/4" WELDED HEADED STUDS (1-3/4" GAGE) AND 1/8"x1/4" STEEL BARS (WELDED)</p> <p>CONC. TILT-UP PANEL. SEE 11/85.4.</p> <p>HSS COLUMN. REFER TO PLAN.</p> <p>3-SIDES TYP.</p> <p>3" (MAX.)</p> <p>3" (MAX.)</p> <p>SEE 6/84.2.</p> <p>5 HSS COLUMN CONNECTION SCALE: 1" = 1'-0"</p>
BEAM SIZE	BOLTS	PL LENGTH "H"	NO. OF STUDS "G"																																																																															
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<p>EMBED 1/2"x8"x1'-6" w/ (2) 3/4"x4" WELDED HEADED STUDS.</p> <p>SEE PLAN TOP/STEEL (S4.2)</p> <p>3-SIDES TYP. 1/4"</p> <p>HSS COLUMN. REFER TO PLAN.</p> <p>L4"x4"x1/2" x 0'-6" WELDED (1-EA. SIDE OF COLUMN)</p> <p>METAL LOW ROOF DECK (FAR SIDE). REFER TO PLAN.</p> <p>TILT-UP WALL. REFER TO PLAN AND TILT-UP WALL ELEVATIONS.</p> <p>6 SECTION SCALE: 3/4" = 1'-0"</p>	<p>LOCATE SPLICE BTWN ANCHOR BOLT SPACING</p> <p>STEEL DECK ANGLE.</p> <p>PLATE 3/8" x 3" x 1'-0"</p> <p>REFER TO 3/84.5 FOR DECK ANGLE CONNECTION TO WALL.</p> <p>TYP. 1/4"</p> <p>7 DECK LEDGER SPLICE SCALE: 1 1/2" = 1'-0"</p>	<p>CONNECTION SHOWN FOR GRAPHICAL REPRESENTATION ONLY. REFER TO STRUCTURAL STEEL SECTION OF STRUCT. NOTES/SPEC FOR ADDITIONAL INFORMATION.</p> <p>WELDED CAP 1/2" AT TOP OF COLUMN. NOTE: REFER TO PLAN FOR LOCATIONS w/ BEAMS/JOIST PERPENDICULAR EXTEND COLUMN/CAP 2-1/2" AS REQUIRED.</p> <p>TYP. FLUSH</p> <p>TYP. EXTEN.</p> <p>HSS BEAM. REFER TO PLAN.</p> <p>HSS COLUMN. REFER TO PLAN.</p> <p>* FILLET WELD = 1/16" LESS THAN SOLID WALL THICKNESS</p> <p>8 BEAM-COLUMN CONNECTION SCALE: 3/4" = 1'-0"</p>	<p>TILT-UP WALL. REFER TO PLAN AND TILT-UP WALL ELEVATIONS.</p> <p>TILT-UP WALL. REFER TO PLAN AND TILT-UP WALL ELEVATIONS.</p> <p>TILT-UP WALL. REFER TO PLAN AND TILT-UP WALL ELEVATIONS.</p> <p>L6"x4"x5/16" (MIN) EA. SIDE OF BEAM</p> <p>#3 HAIRPINS. 24" LONG.</p> <p>STEEL BEAM. REFER TO PLAN</p> <p>EMBED 1/2" WELDED HEADED STUDS AND STEEL BARS. REFER TO 1/842</p> <p>9 BOLT CLUSTER AT TILT PANEL JOINT SCALE: 3/4" = 1'-0"</p>	<p>TILT-UP WALL. REFER TO PLAN AND TILT-UP WALL ELEVATIONS.</p> <p>L6"x4"x5/16" (MIN)</p> <p>STEEL BEAM. REFER TO PLAN</p> <p>EMBED 1/2" WELDED HEADED STUDS AND STEEL BARS. REFER TO 1/842</p> <p>10 BOLT CLUSTER AT TILT PANEL SCALE: 3/4" = 1'-0"</p>																																																																														



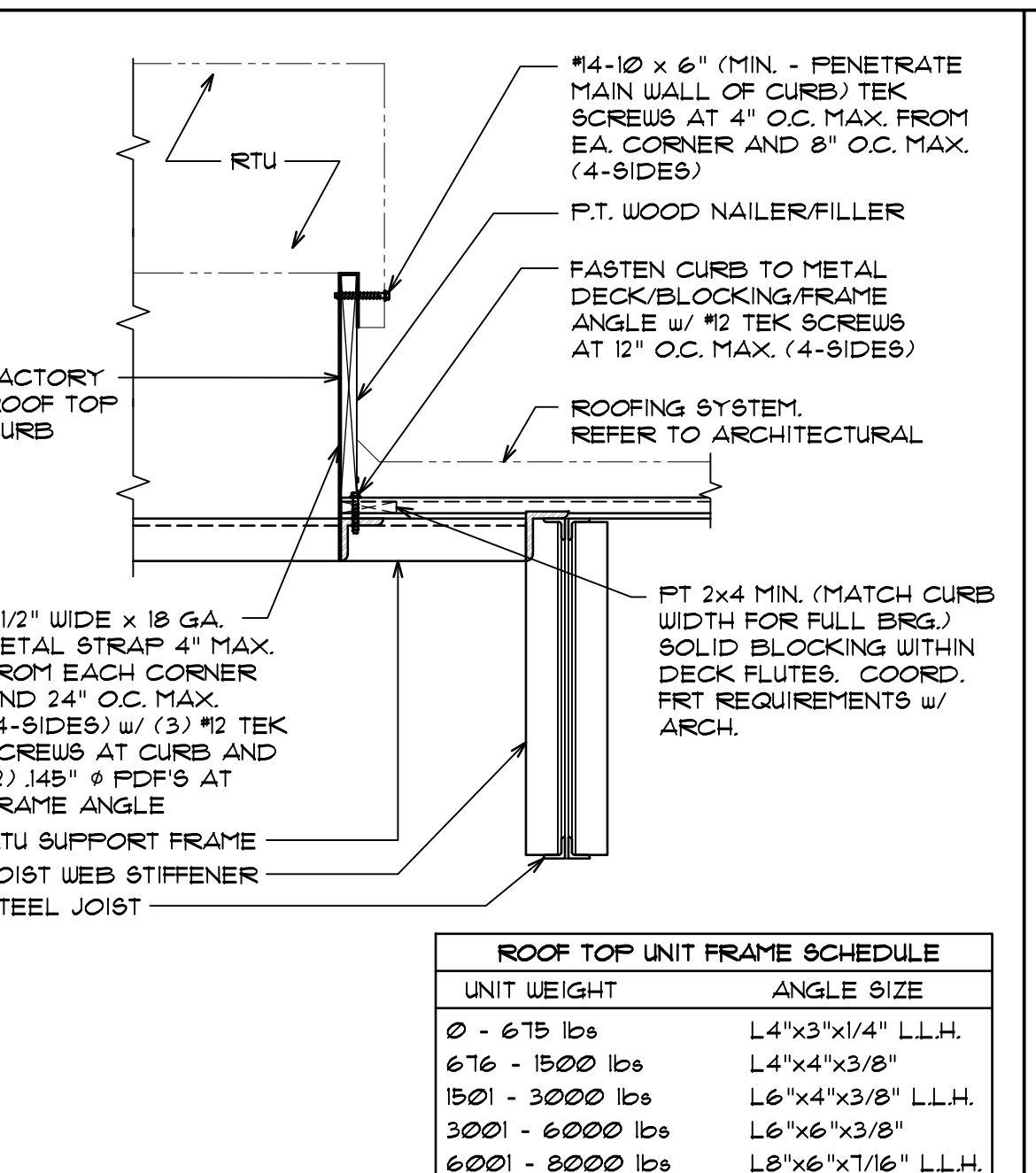
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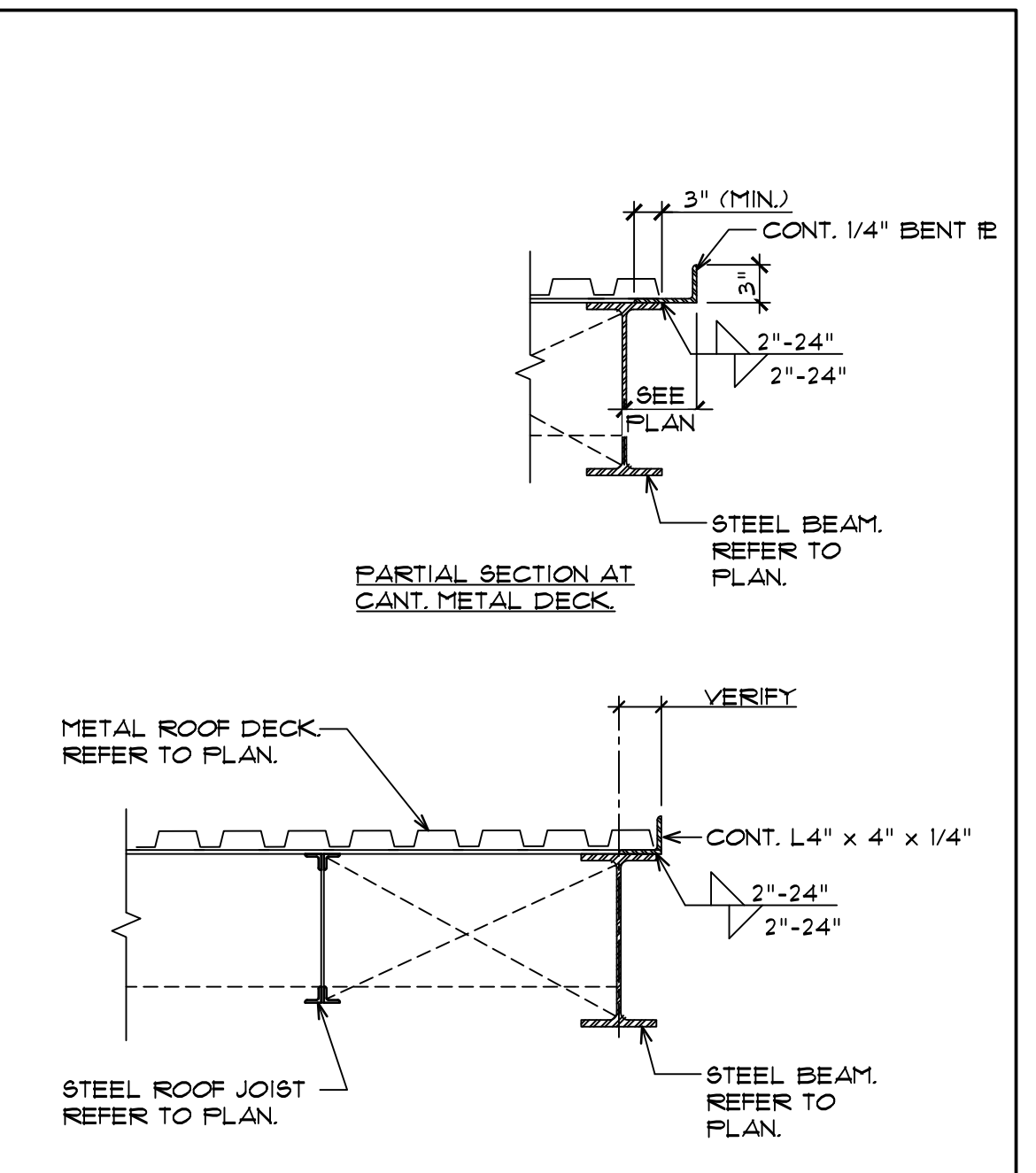
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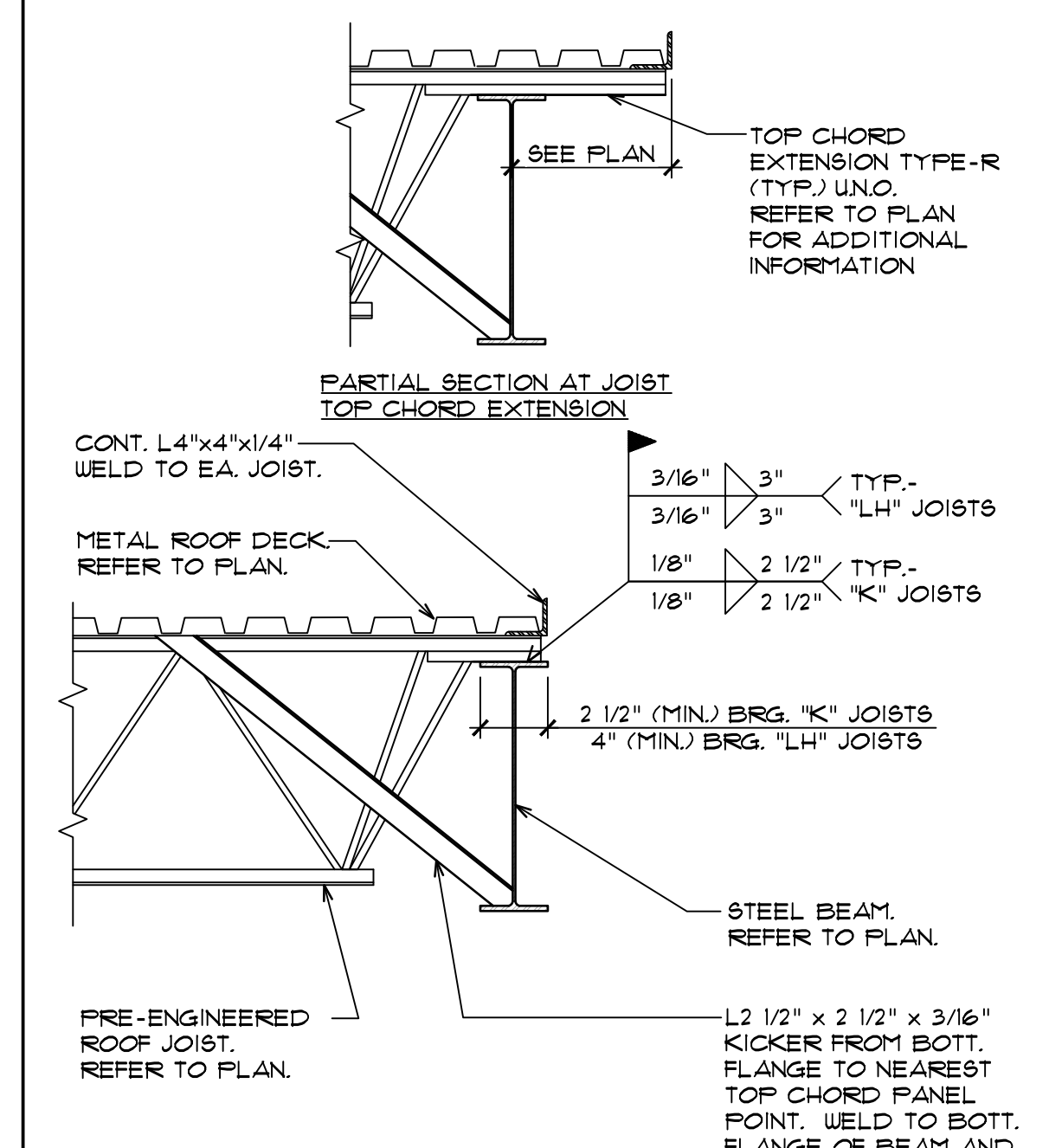
3 ROOF OPENING/EQUIP. FRAMING



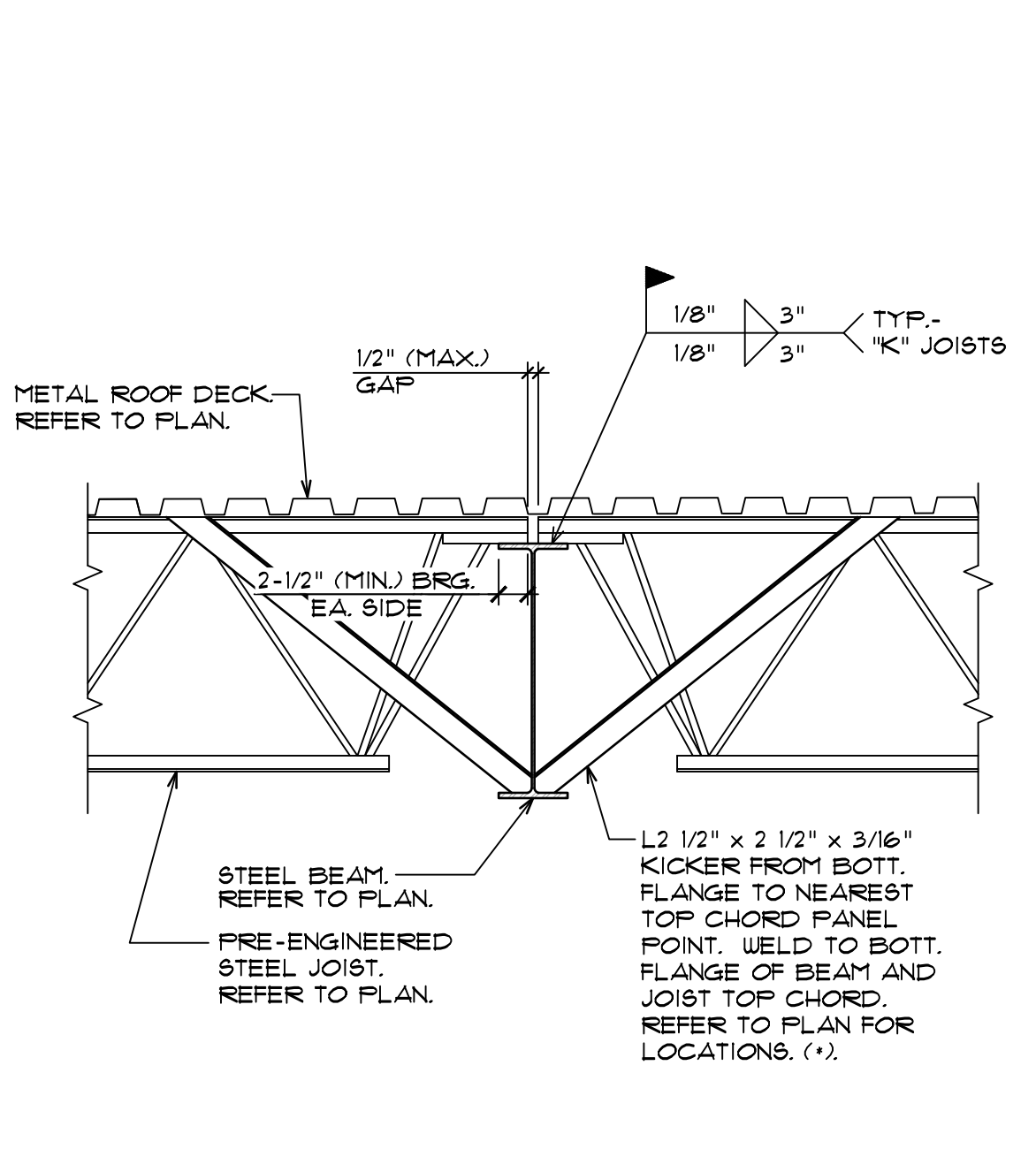
4 MECH. CURB FRAMING
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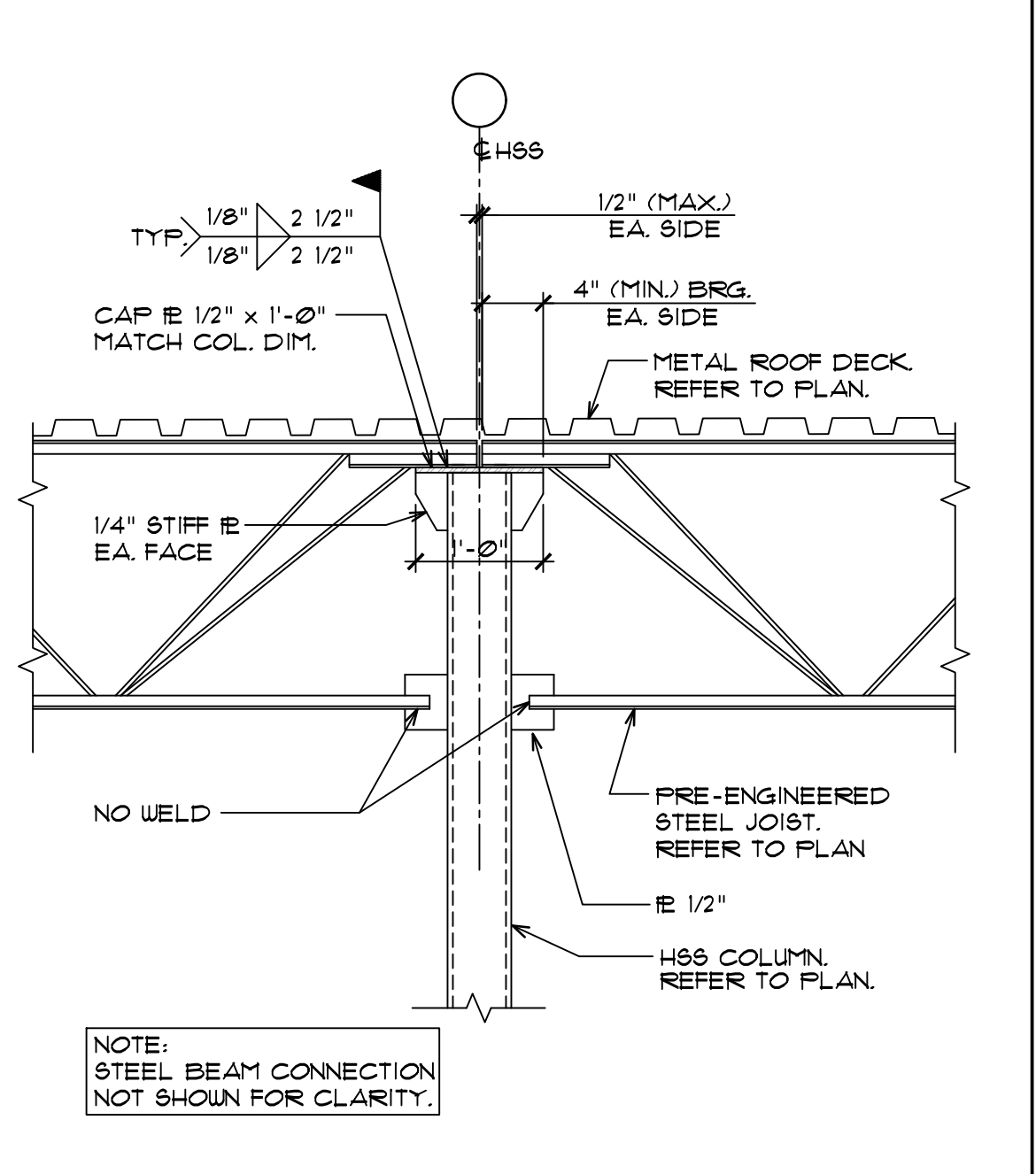
5 EDGE OF OPENING DETAILS
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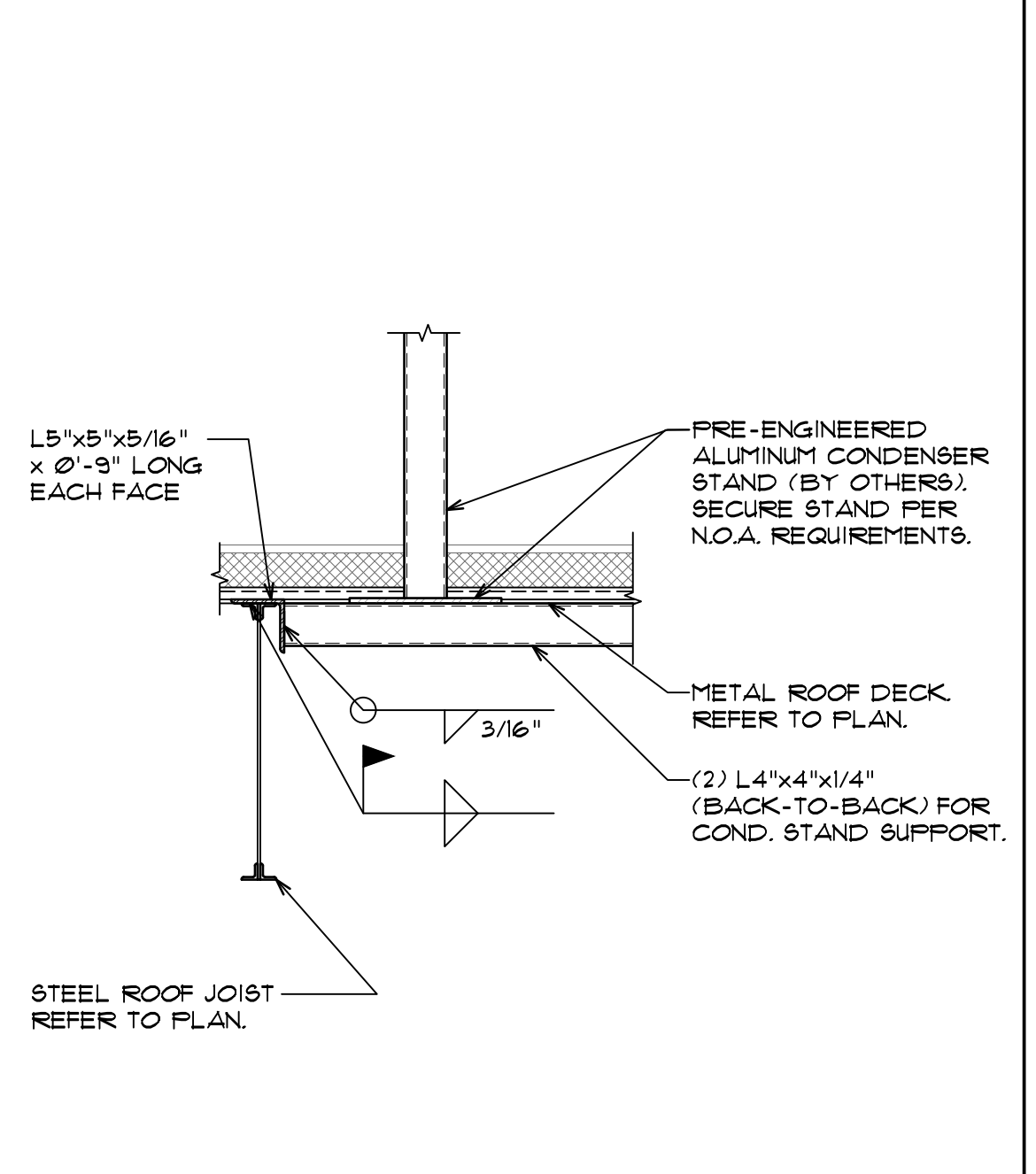
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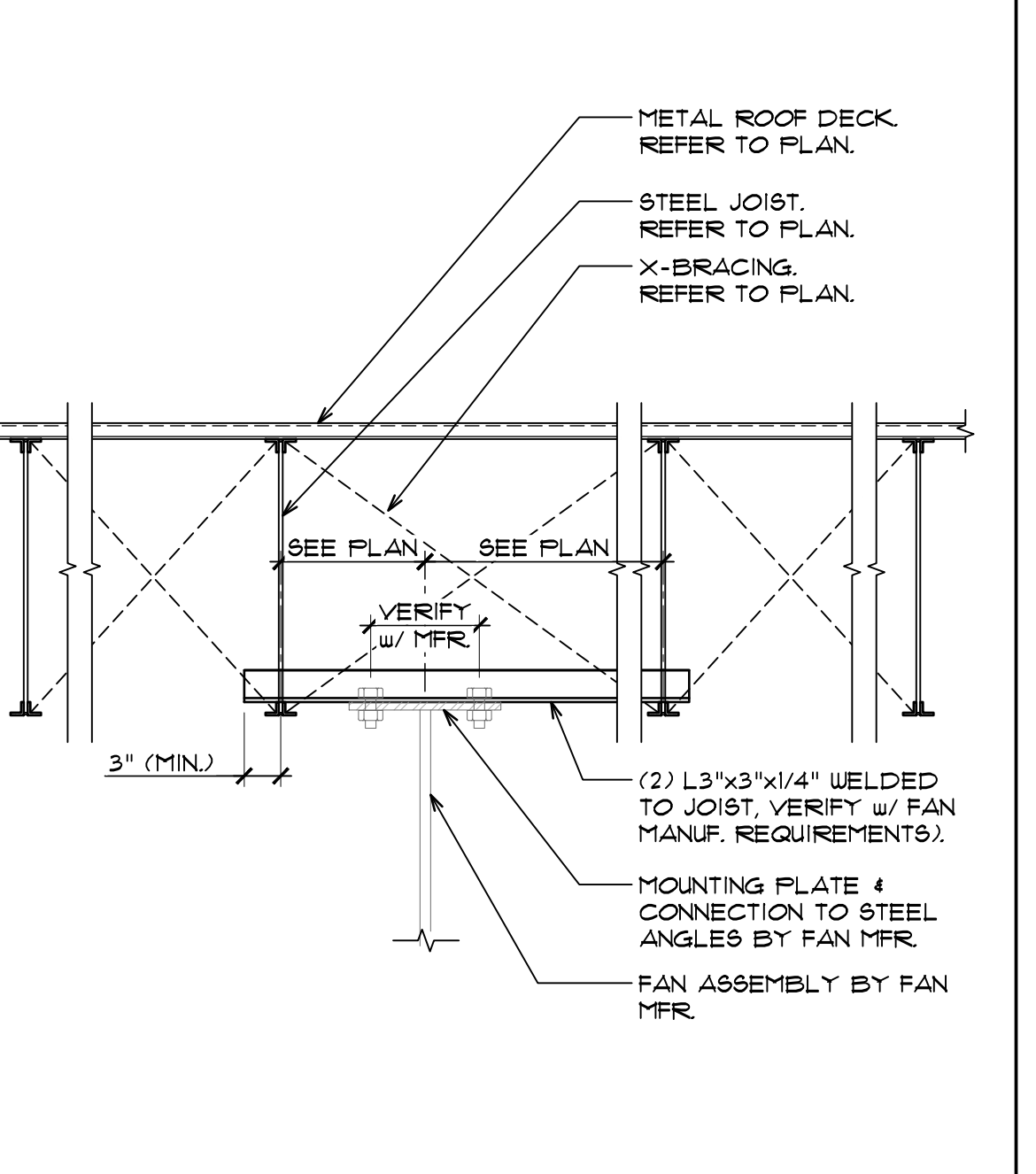
7 JOIST BEARING (2-1/2\"/>



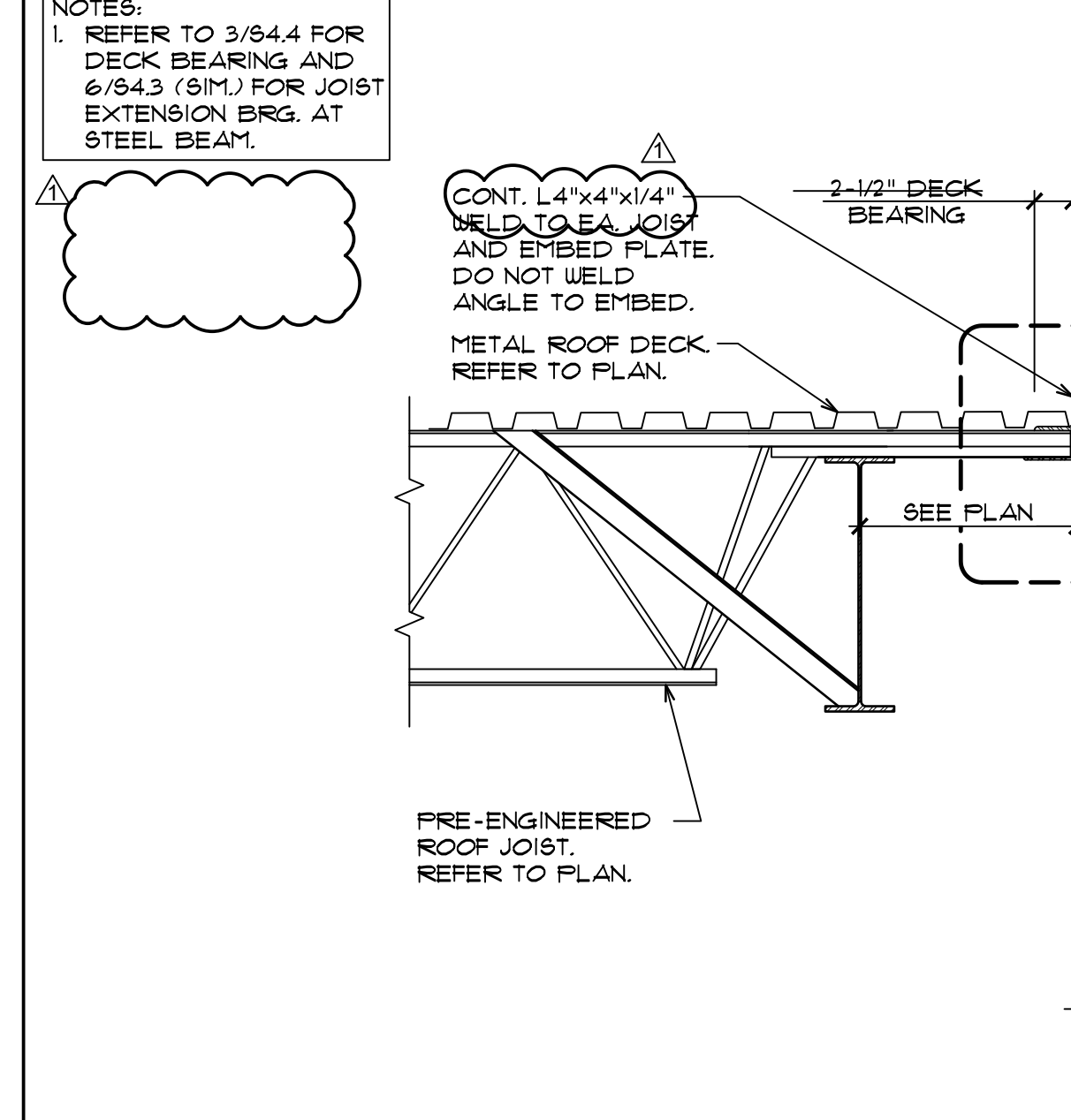
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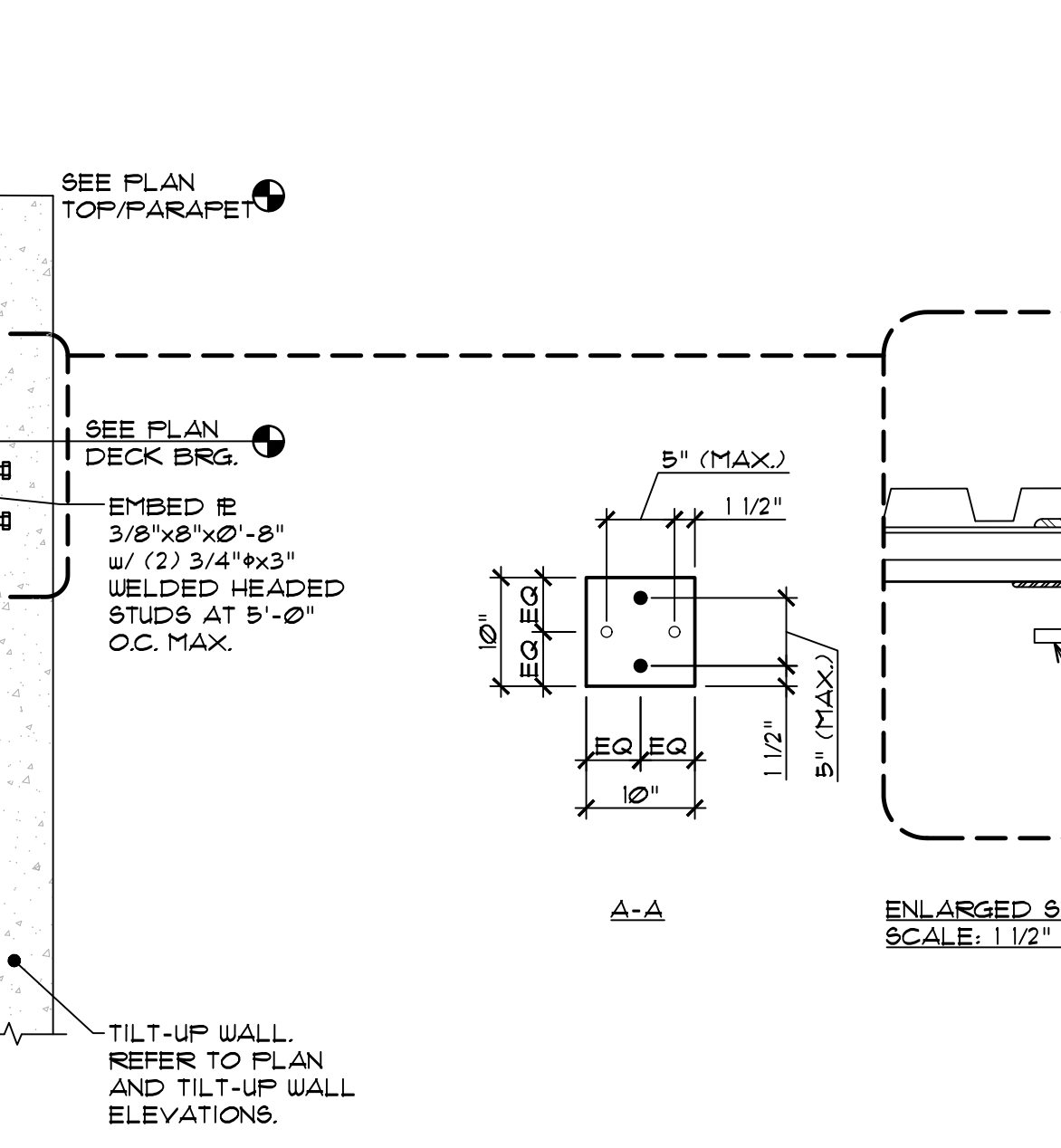
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10 SECTION
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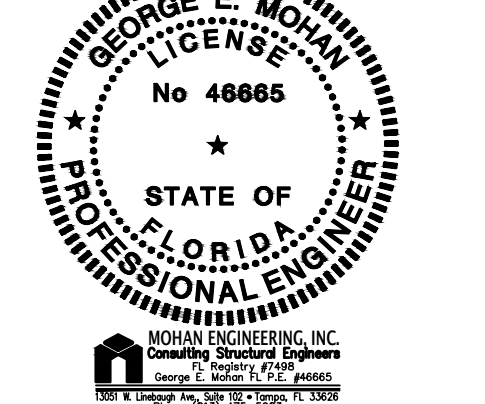


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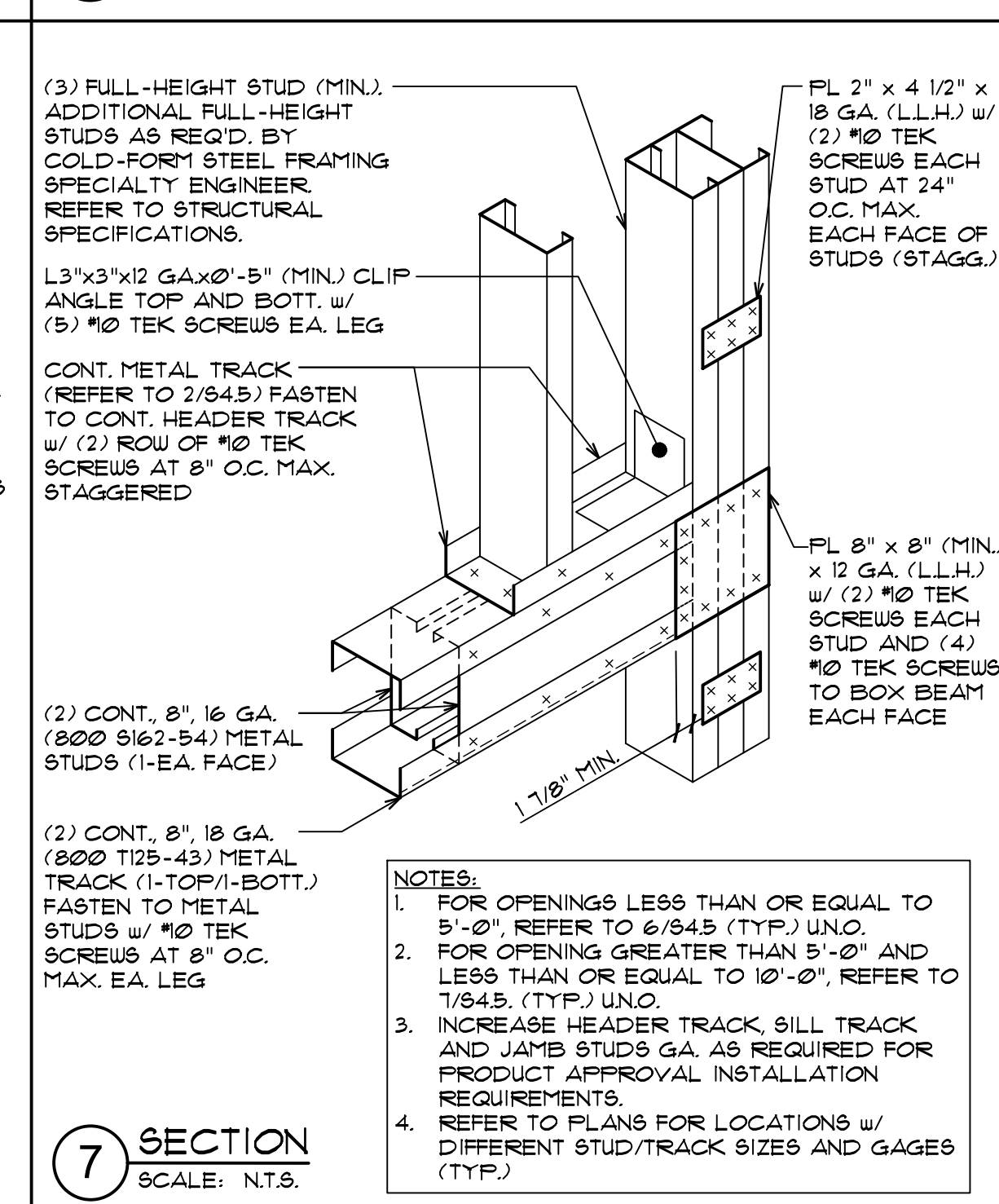
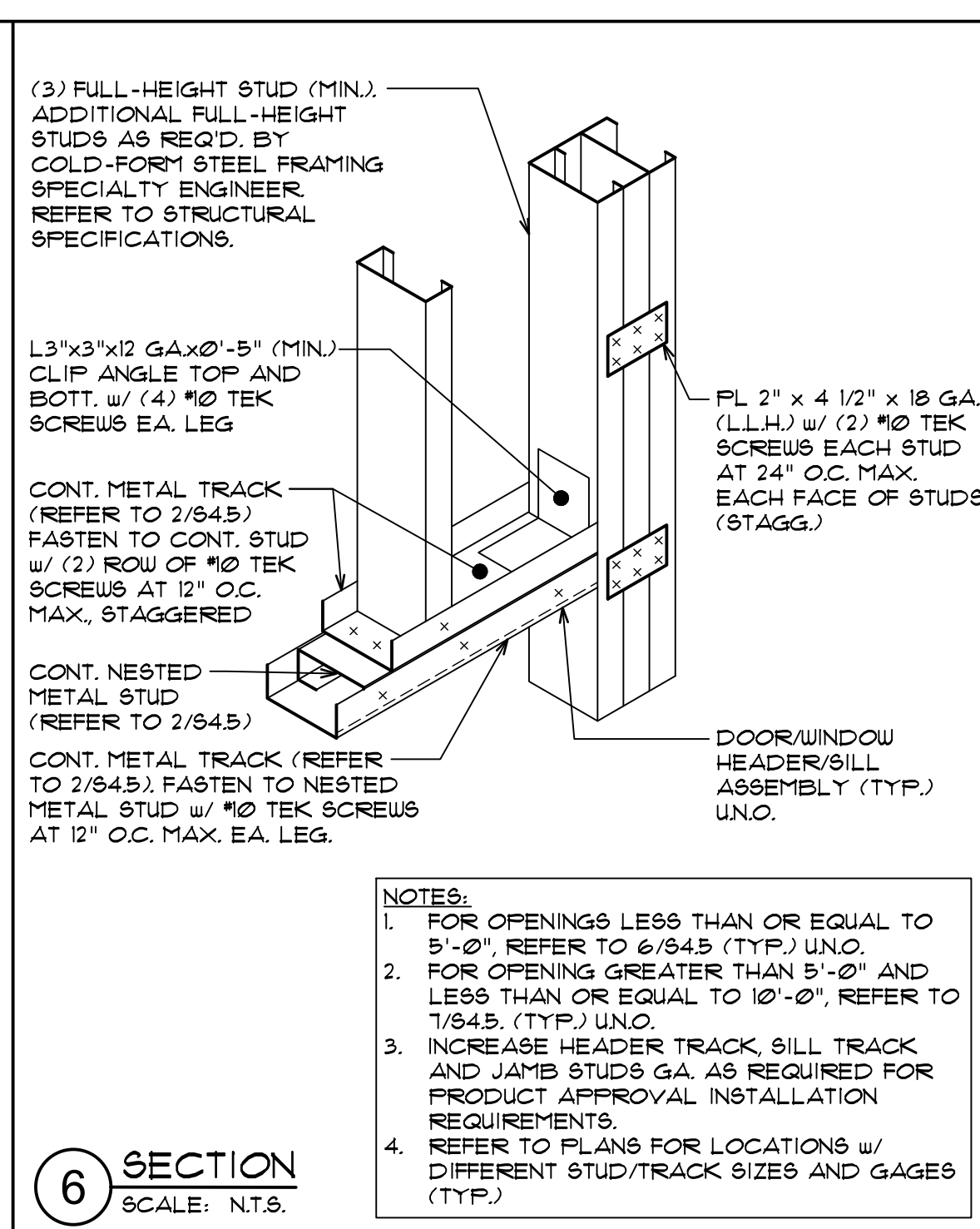
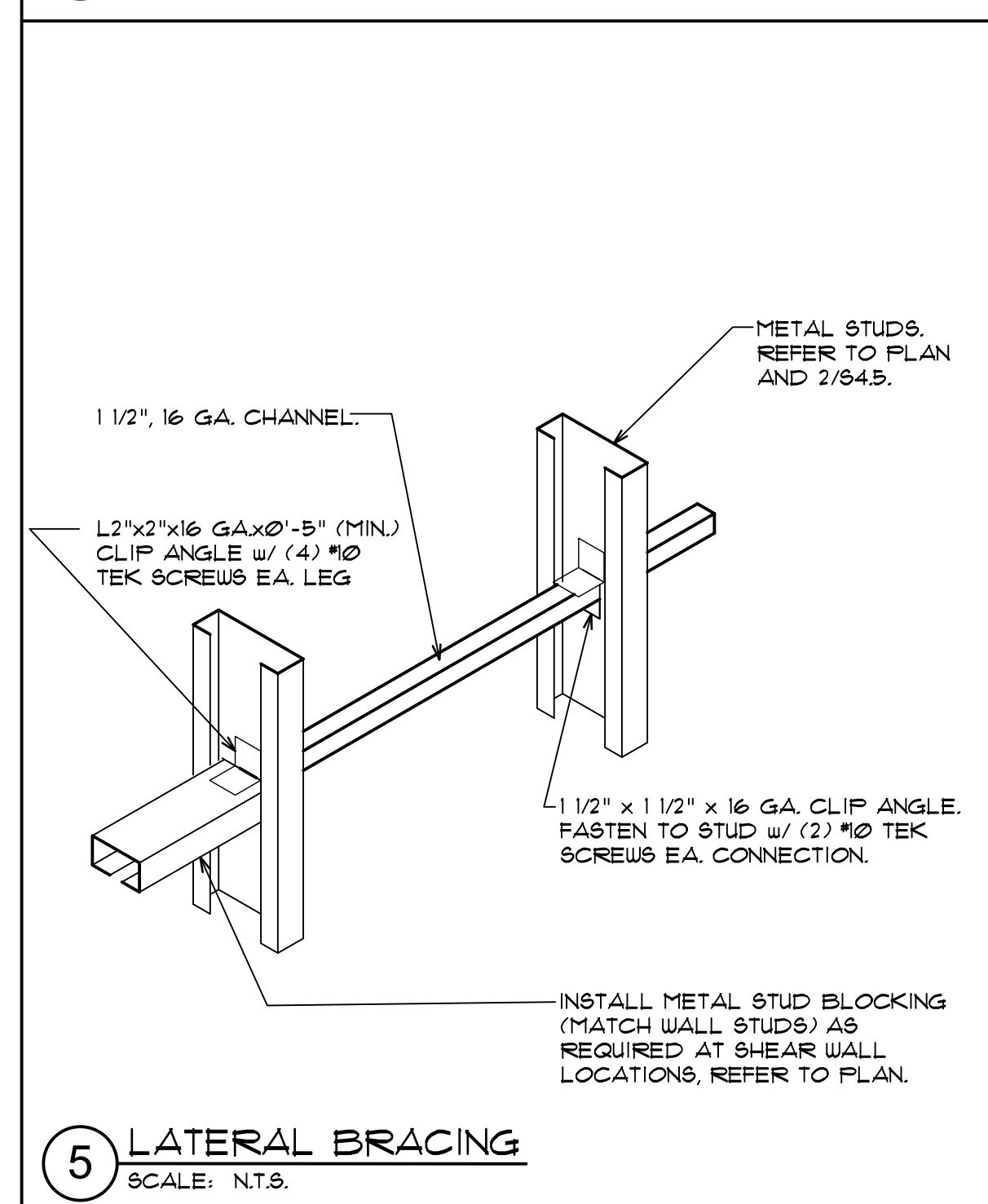
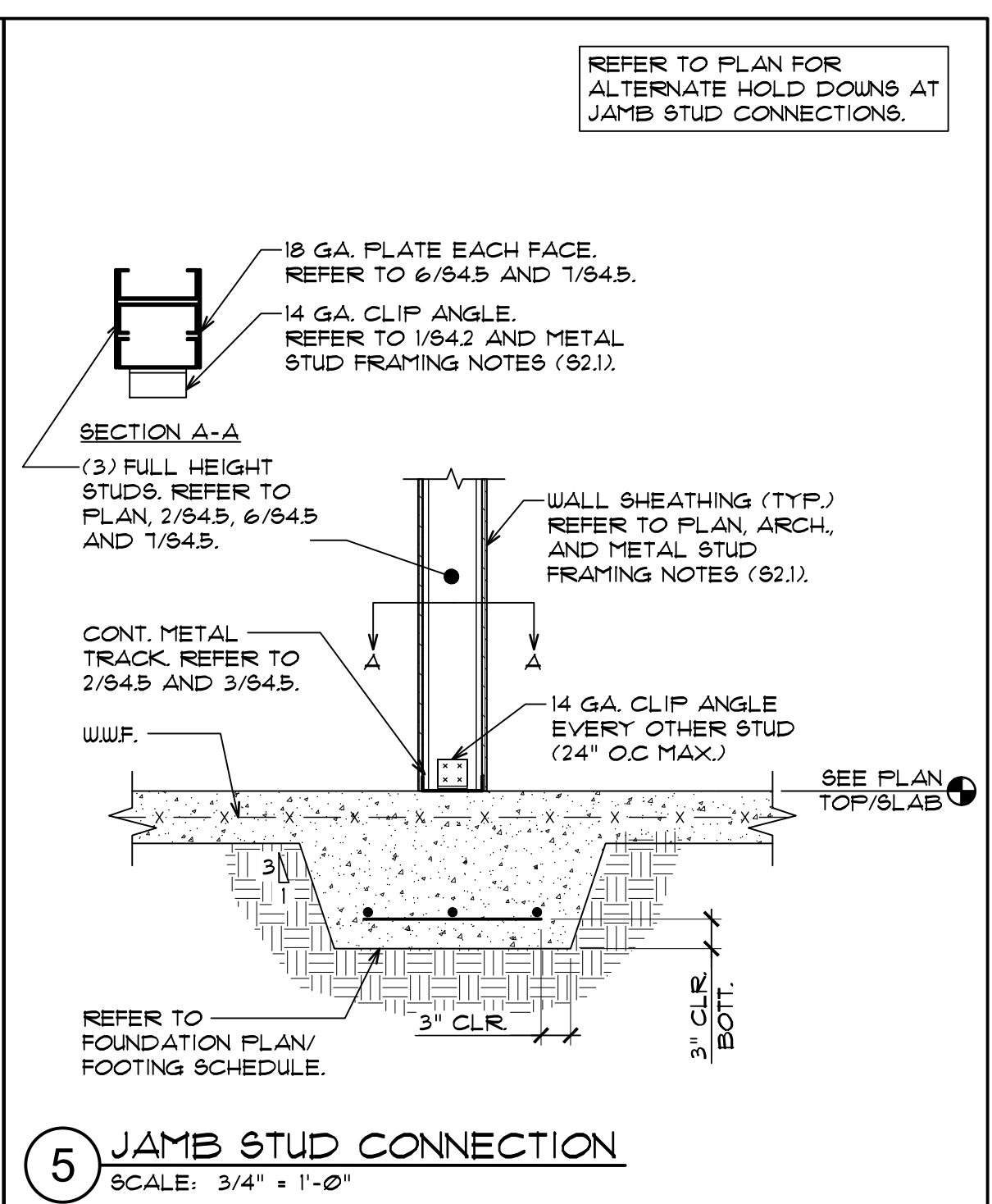
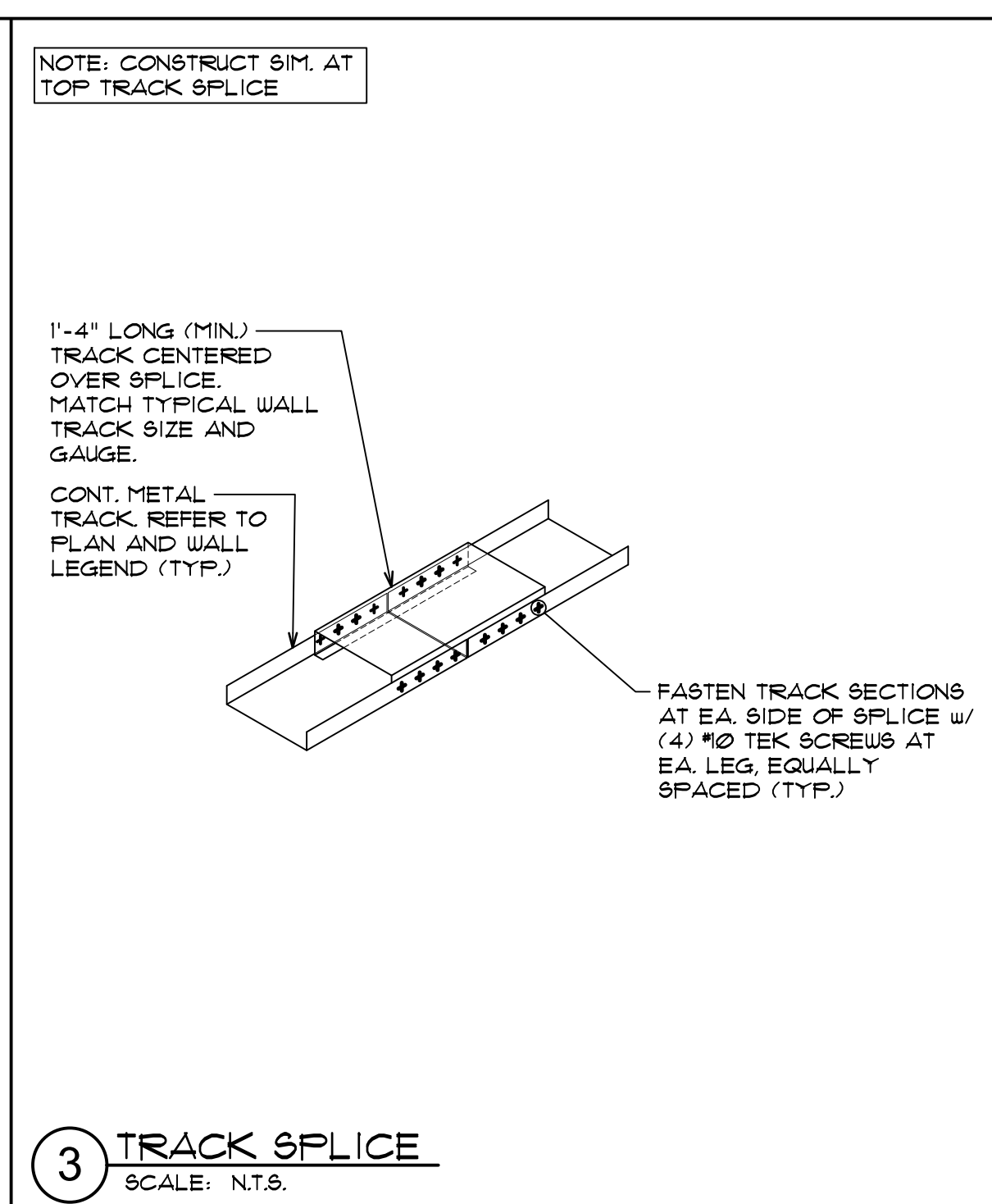
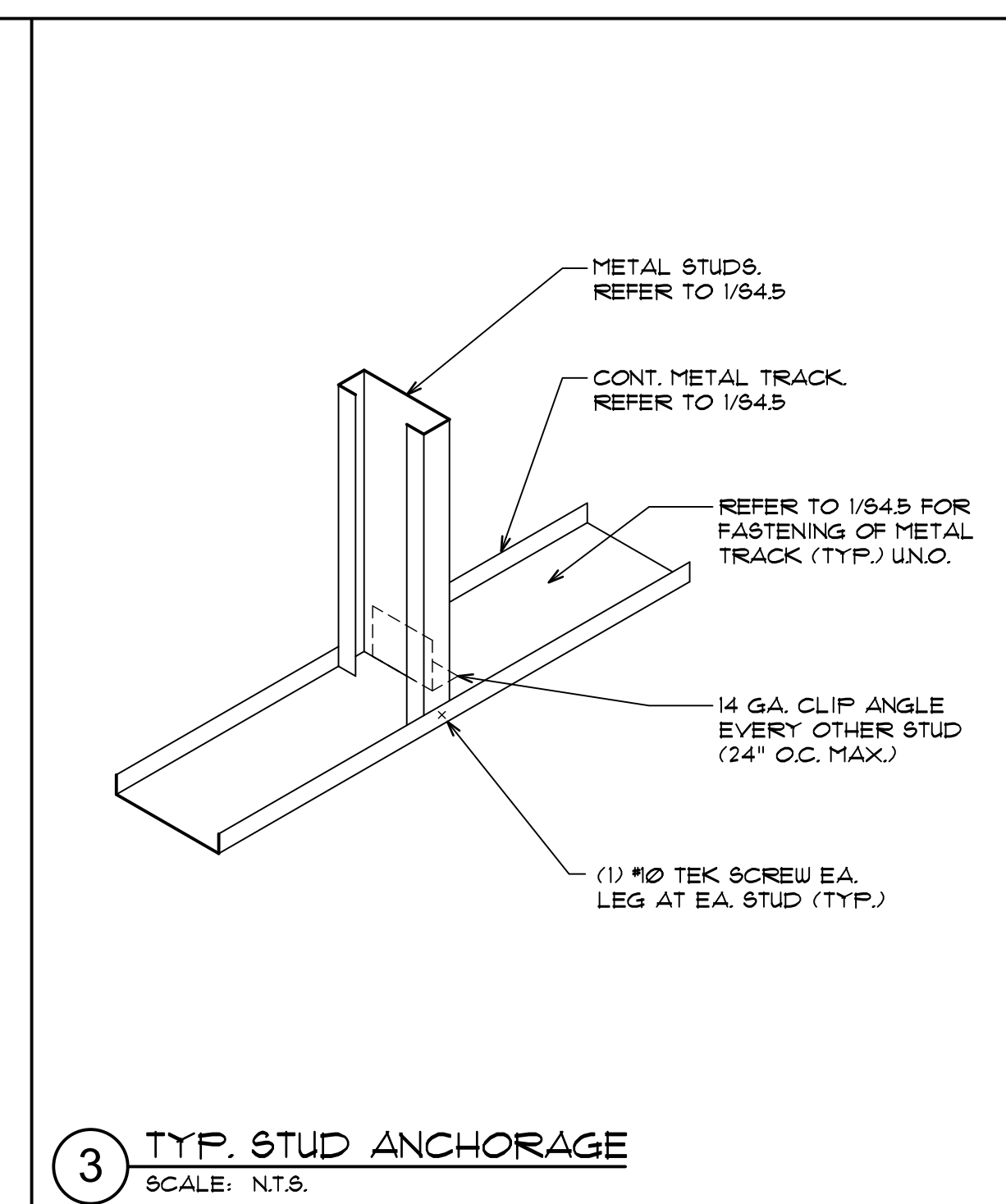
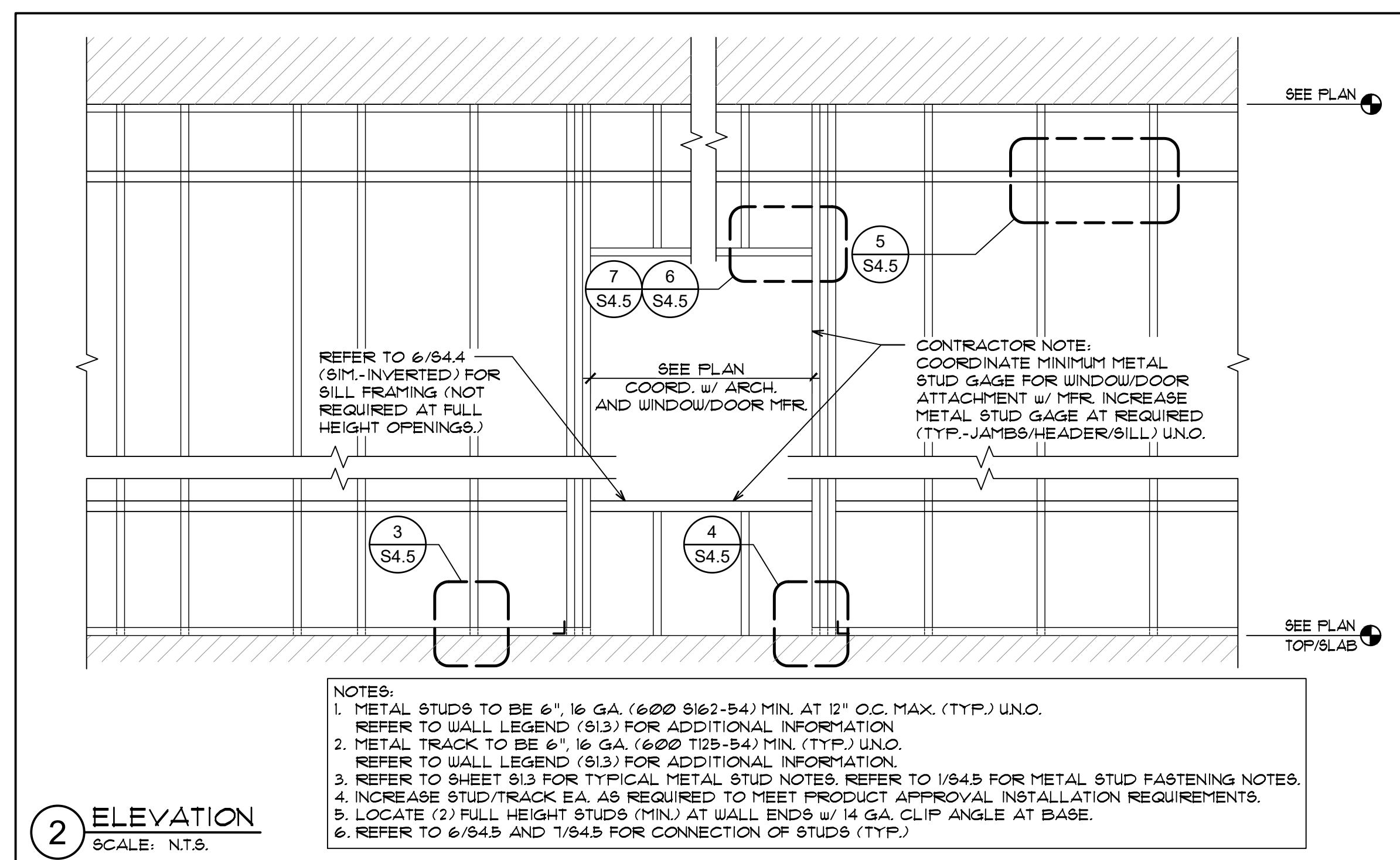


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To the best of the knowledge of the architect & engineer, said plans and specifications comply with the applicable building codes and minimum fire safety standards.



TILT-UP WALL PANEL REINF. SCHEDULE

PANEL MARK	PANEL THICKNESS	VERT. REINF.	HORIZ. REINF.	REMARKS
W1	1 1/4"	#5 AT 12" O.C.	#4 AT 12" O.C.	TYPICAL WALL
W2	1 1/4"	#5 AT 8" O.C.	#4 AT 12" O.C.	FEATURE WALL

- NOTES:**
- THE DETAILER SHALL REFER TO SHEET 95.2 ALONG WITH THE PANEL REINFORCING SCHEDULE AND PANEL ELEVATIONS IN THE PREPARATION OF SHOP DRAWINGS FOR PANELS AND THEIR REINFORCING.
 - THE REINFORCING IN DETAILS ON SHEET 95.2 IS ADDITIVE. ALL PANELS HAVE VERT. REINFORCING, HORIZONTAL REINFORCING, PERIMETER BARS AND SLAB TIE BARS. PANELS WITH OPENINGS HAVE ADDITIONAL REINFORCING AROUND OPENINGS, AS SHOWN.
 - CENTER VERTICAL REINFORCEMENT IN PANELS REINFORCED WITH ONE LAYER HORIZONTAL REINFORCEMENT SHALL BE PLACED TO THE INTERIOR SIDE.

PANEL EMBED PLATE NOTES:

- REFER TO 4/95.2 FOR TYPICAL PANEL TO PANEL CONNECTIONS AS REQUIRED.
- REFER TO SHEET 94.4 FOR TYPICAL EMBED PLATES/ANGLES AT ROOF FRAMING (JOIST BEARING AND DECK BEARING). CONTRACTOR TO COORDINATE EMBED PLATE/ANGLES LOCATIONS w/ JOIST MANUFACTURER AS REQUIRED.
- REFER TO 1/6.4.2 FOR EMBED PLATES AT STEEL BEAMS. CONTRACTOR TO COORDINATE EMBED PLATES AT STEEL BEAM LOCATIONS w/ JOIST MANUFACTURER AS REQUIRED.

PANEL ELEVATION NOTES:

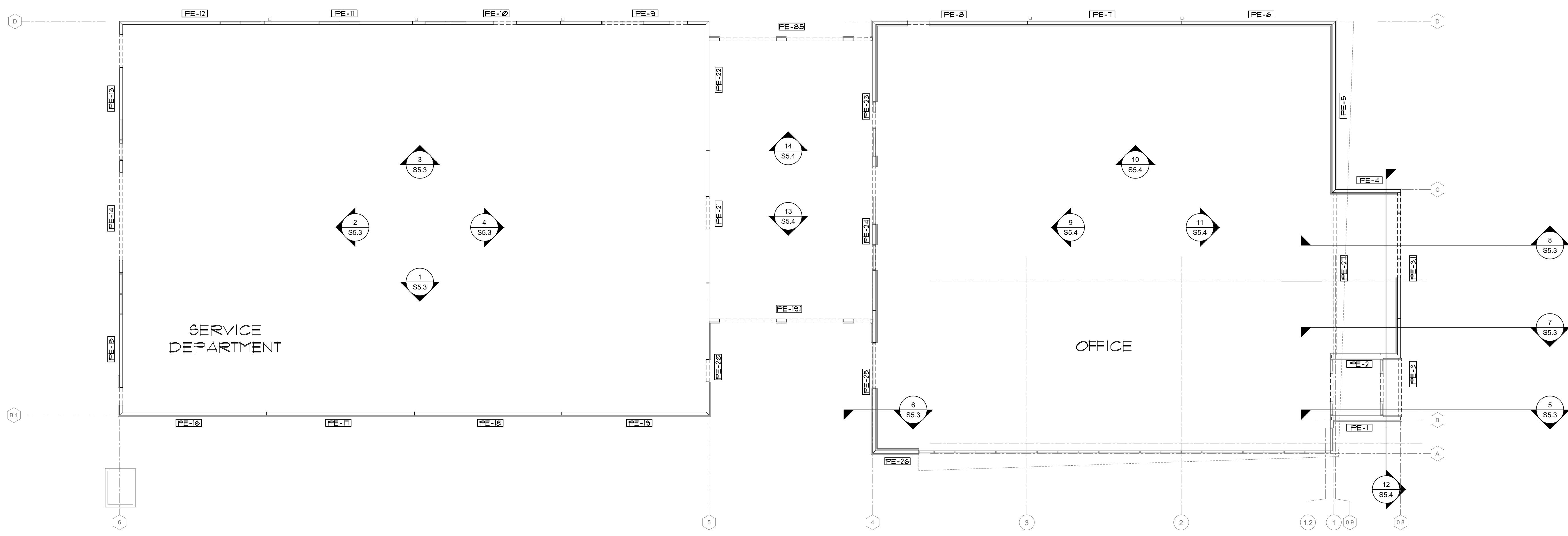
- ALL WALL PANEL ELEVATIONS ARE INTERIOR FACE. PANELS ARE TO BE CAST EXTERIOR FACE DOWN.
- THE GENERAL CONTRACTOR SHALL VERIFY AND APPROVE ALL WALL PANEL AND OPENING DIMENSIONS, BEARING POCKET AND WELD PLATE LOCATIONS, AND ALL OTHER EMBEDDED ITEM LOCATIONS WITH THE ARCHITECTURAL, ENGINEERING, AND MECHANICAL CONTRACT DOCUMENTS. ANY DISCREPANCIES SHALL BE REPORTED TO THE ENGINEER BEFORE THE CASTING OF ANY WALL PANELS.
- STRUCTURAL CONCRETE FOR ALL WALL PANELS SHALL BE NORMAL WEIGHT AGGREGATE AND SHALL DEVELOP A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 4000 PSI.
- ALL EXPOSED EDGES SHALL HAVE A 3/4" CHAMFER, UNLESS NOTED OTHERWISE. REFER TO ARCH.
- ALL EMBEDDED ITEMS SHOWN ARE TO BE CAST INTO THE INSIDE FACE UNLESS NOTED OTHERWISE.
- NO OPENINGS SHALL BE MADE IN THE TILT-UP WALL PANELS, EXCEPT AS SHOWN ON THE DRAWING WITHOUT PRIOR APPROVAL OF THE ENGINEER.
- WALLS ARE 1-1/4" THICK UNO. (SEE PANEL REINFORCING SCHEDULE ON 95.1 FOR MORE INFORMATION.)
- CENTER VERT. REINF. ON 1-1/4" DIMENSION TYP. UNO. INSTALL HORIZONTAL STEEL ON INSIDE FACE OF VERT. TYP. UNO. REFER TO SHEET 95.2 FOR PANEL REINFORCING.
- PROVIDE 1" CONCRETE COVER TO OUTERMOST STEEL EACH FACE (INCLUDING STIRRUPS) AT DOUBLE-LAYER/COLUMN REINFORCING. REFER TO 95.2 FOR DETAILS.
- BOTTOM OF WALL PANELS MUST BE FULLY GROUTED AND FOOTING DOUELS INSTALLED PER 2/8.4.0 PRIOR TO INSTALLATION OF ROOF JOISTS.
- SLAB-ON-GRADE SECONDARY POUR AND ROOF DECK TO BE FULLY INSTALLED PRIOR TO REMOVAL OF TILT-WALL BRACING.

architecture
design
planning

HARTMANN
architecture inc.

4427 W. Kennedy Blvd.
Suite 250
Tampa, Florida 33609
813.251.2190
AR09622

Project Name



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4750 Highway 520
Cocoa, FL 32926

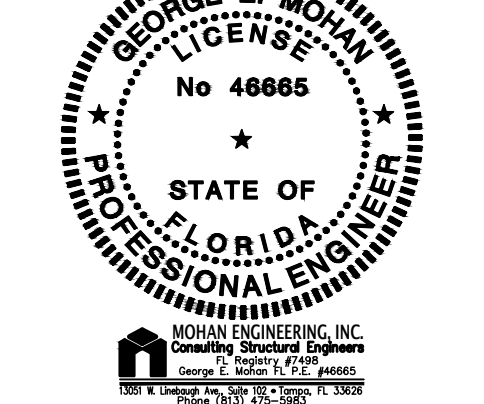
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TILT-UP PANEL KEYPLAN

Revisions

State Registration

George E. Mohan, P.E.



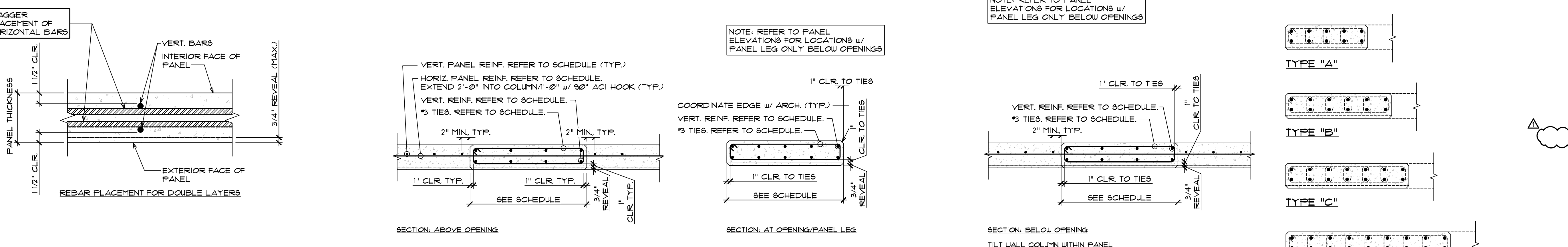
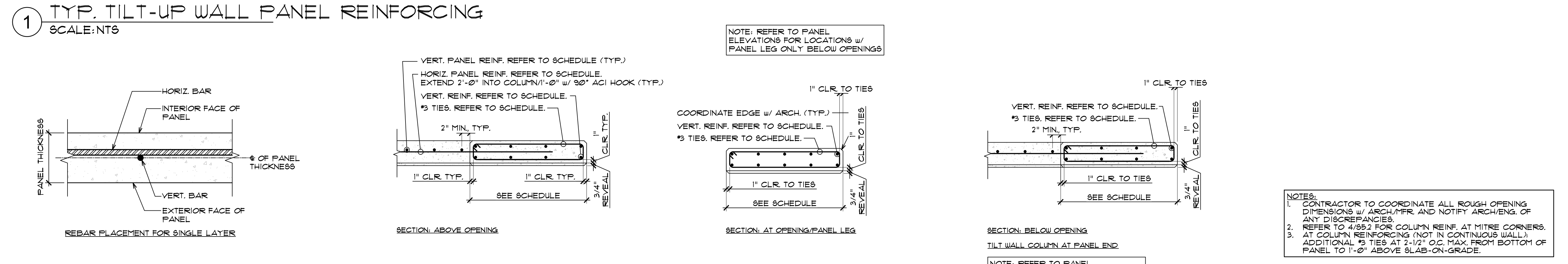
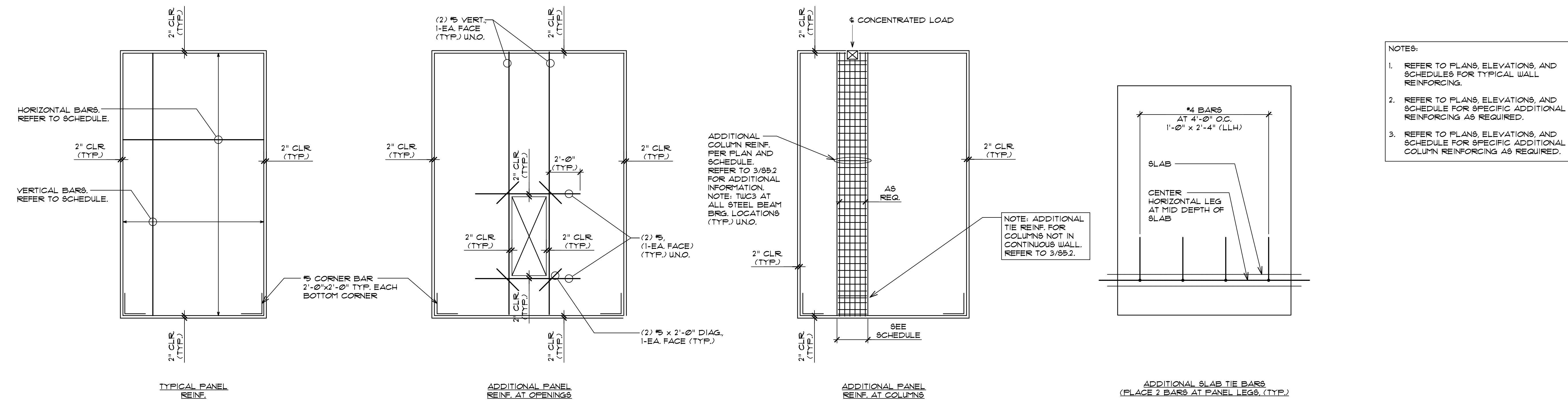
To the best of the knowledge of the architect, engineer, and other professionals, the design and specifications comply with the applicable building codes and minimum fire safety standards.

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KEY PLAN SCALE 3/32"=1'-0"

S5.1

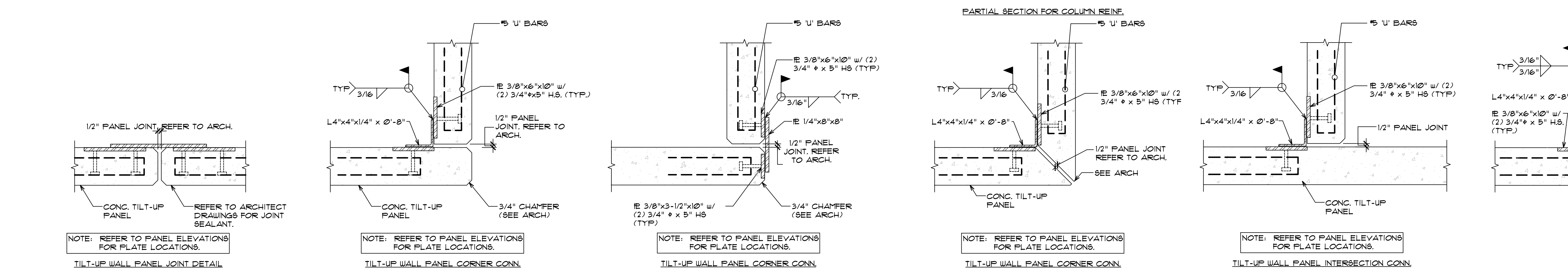
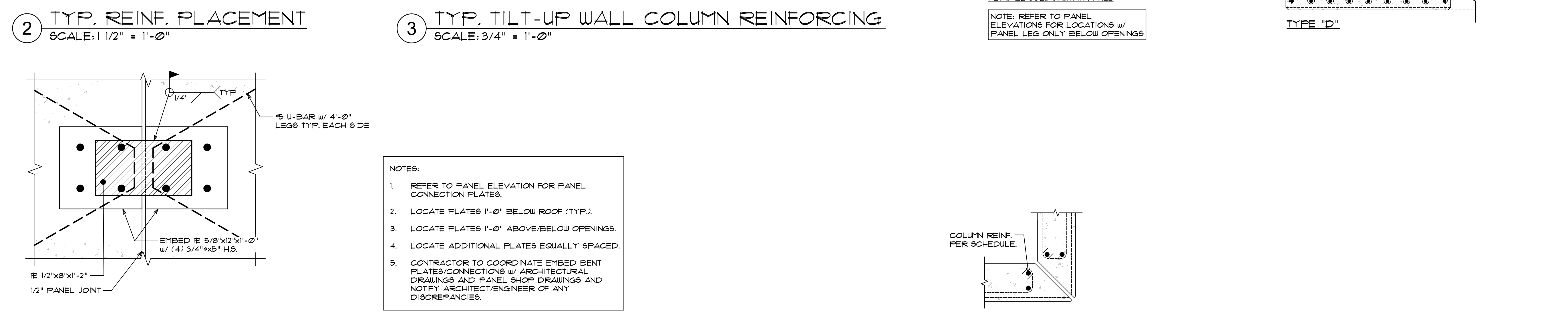


COLUMN SCHEDULE

MARK	SIZE	REINFORCING	TIES	TYPE
TUC1	1-1/4" x 24"	(10) #5	#3 AT 8" O.C.	TYPE "A"
TUC2	1-1/4" x 26"	(10) #5	#3 AT 8" O.C.	TYPE "A"
TUC3	1-1/4" x 28"	(12) #5	#3 AT 8" O.C.	TYPE "B"
TUC4	1-1/4" x 34"	(14) #5	#3 AT 8" O.C.	TYPE "C"
TUC5	1-1/4" x 36"	(14) #5	#3 AT 8" O.C.	TYPE "C"

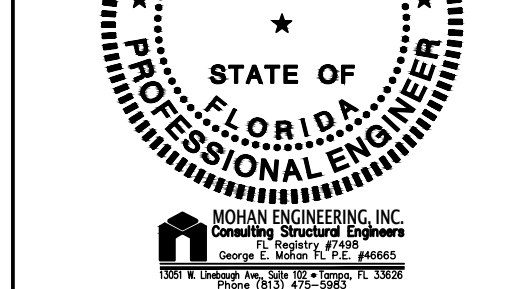
NOTES (TYP.) UNQ.:

- REBAR DETAILER TO COORDINATE w/ DETAIL 1/65.2 AND 3/65.2
- EXTEND HORIZONTAL WALL REINFORCING 2'-0" (MIN) INTO TIE COLUMNS (30" STD. ACI HOOK AT COLUMNS LESS THAN 2'-8")
NOTE: CONT. HORIZONTAL WALL REIN. THROUGH COLUMN REIN. FOR COLUMNS IN MIDDLE OF PANELS.
- ALL COLUMNS REINFORCING SHALL BE CONTINUOUS FROM FOUNDATION TO TOP OF WALL PANEL.
AT 2-STORY WALL PANEL: COLUMNS REQUIRED FOR 1-STORY WALL ONLY ARE REQUIRED TO HAVE REIN. EXTENDED 2'-0" (MIN) ABOVE SECOND FLOOR SLAB. HOOK COL. REIN. INTO WALL/BEAM.

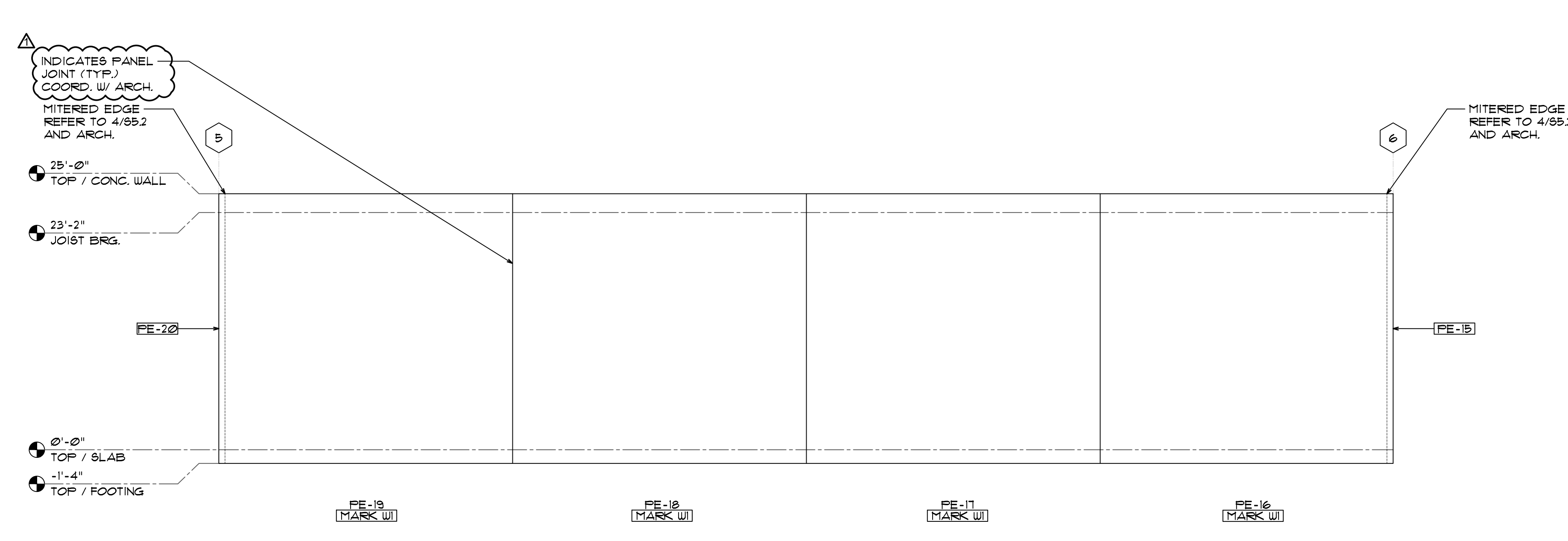


Revisions

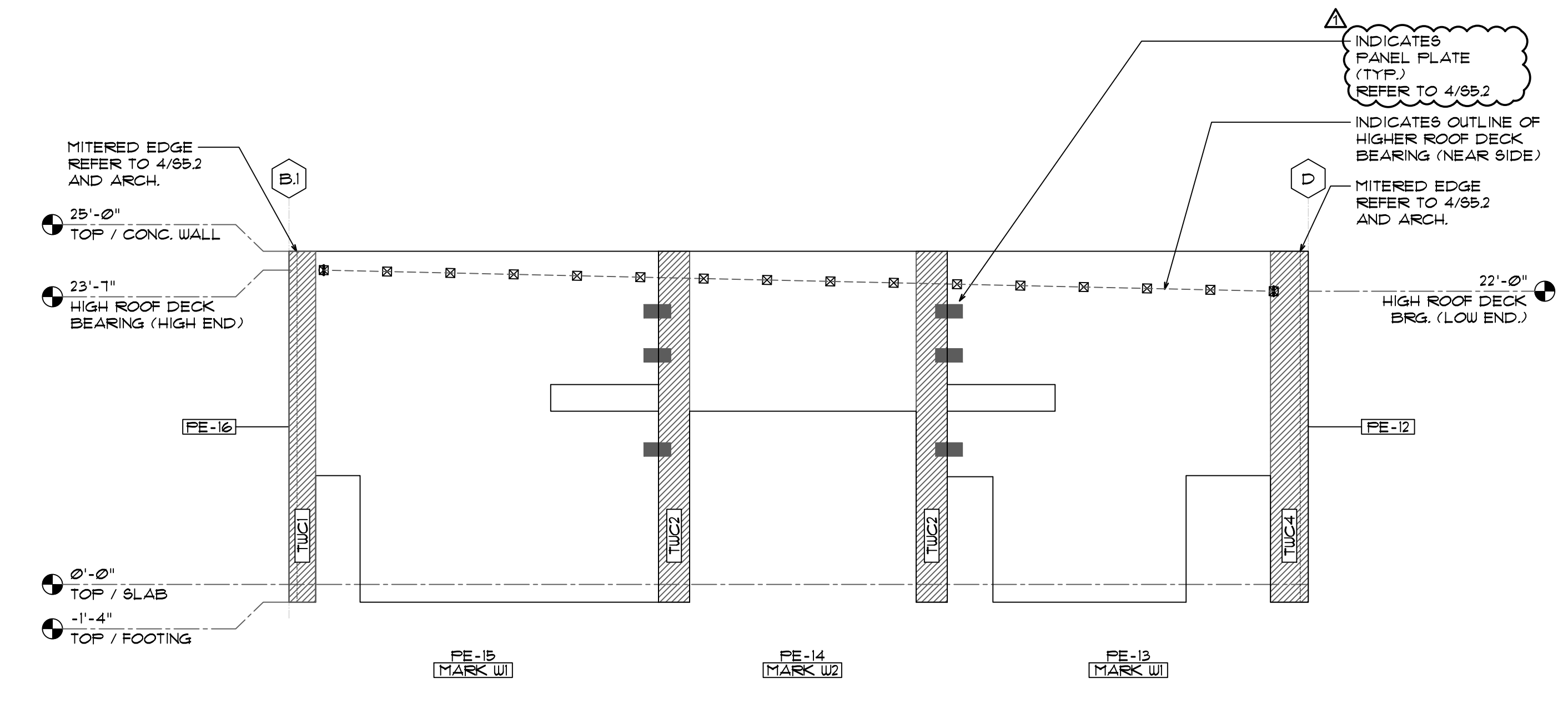
3/7/2024	ADDENDUM NO. 1



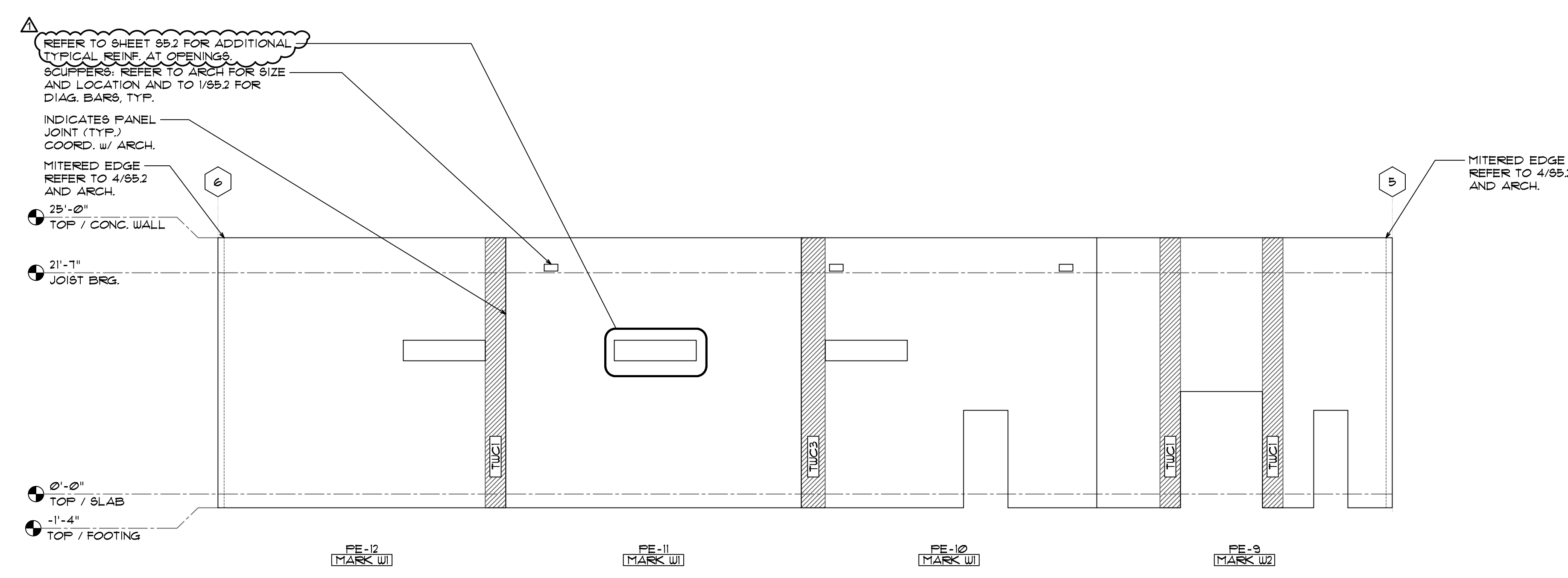
To the best of the knowledge of the architect & engineer, said plans and specifications comply with the applicable building codes and minimum fire safety standards.



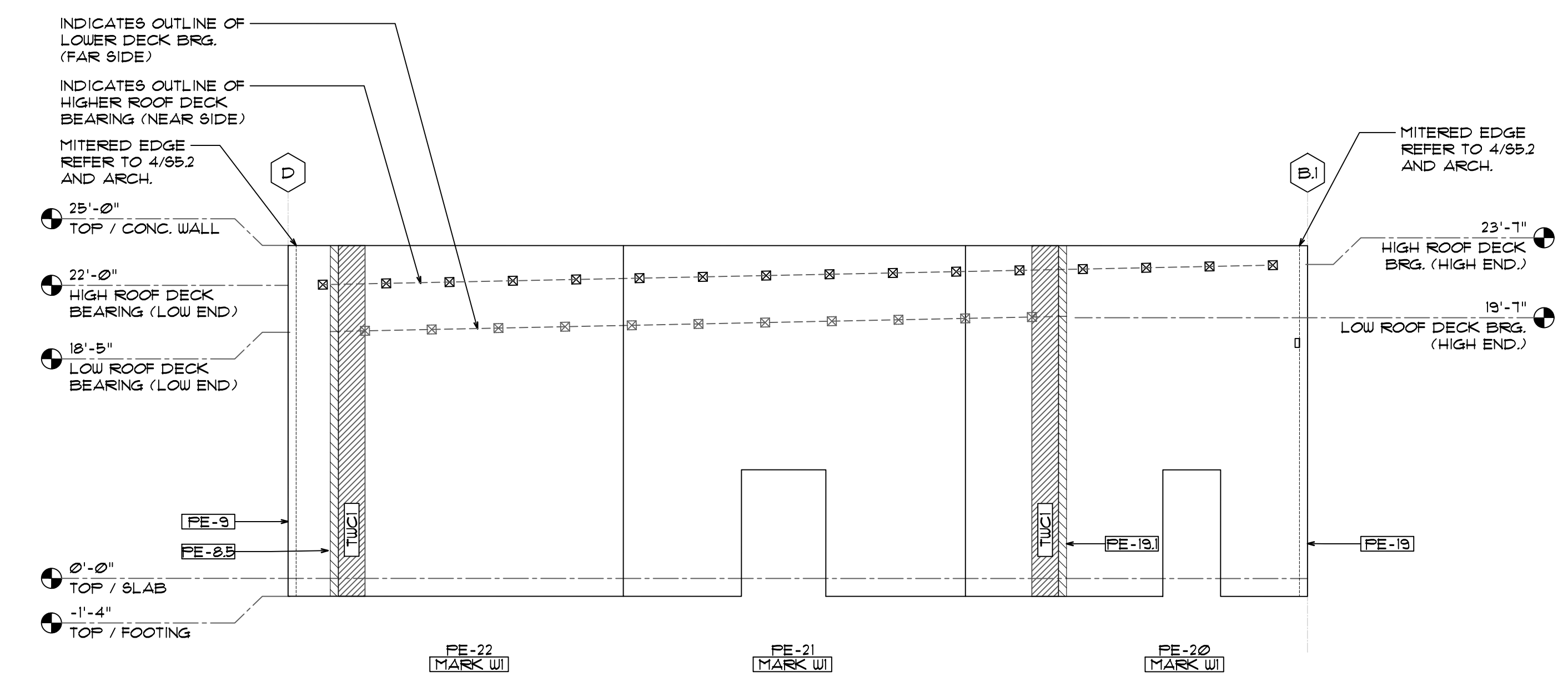
1 EAST FACE ELEVATION
SCALE: 1/8" = 1'-0"



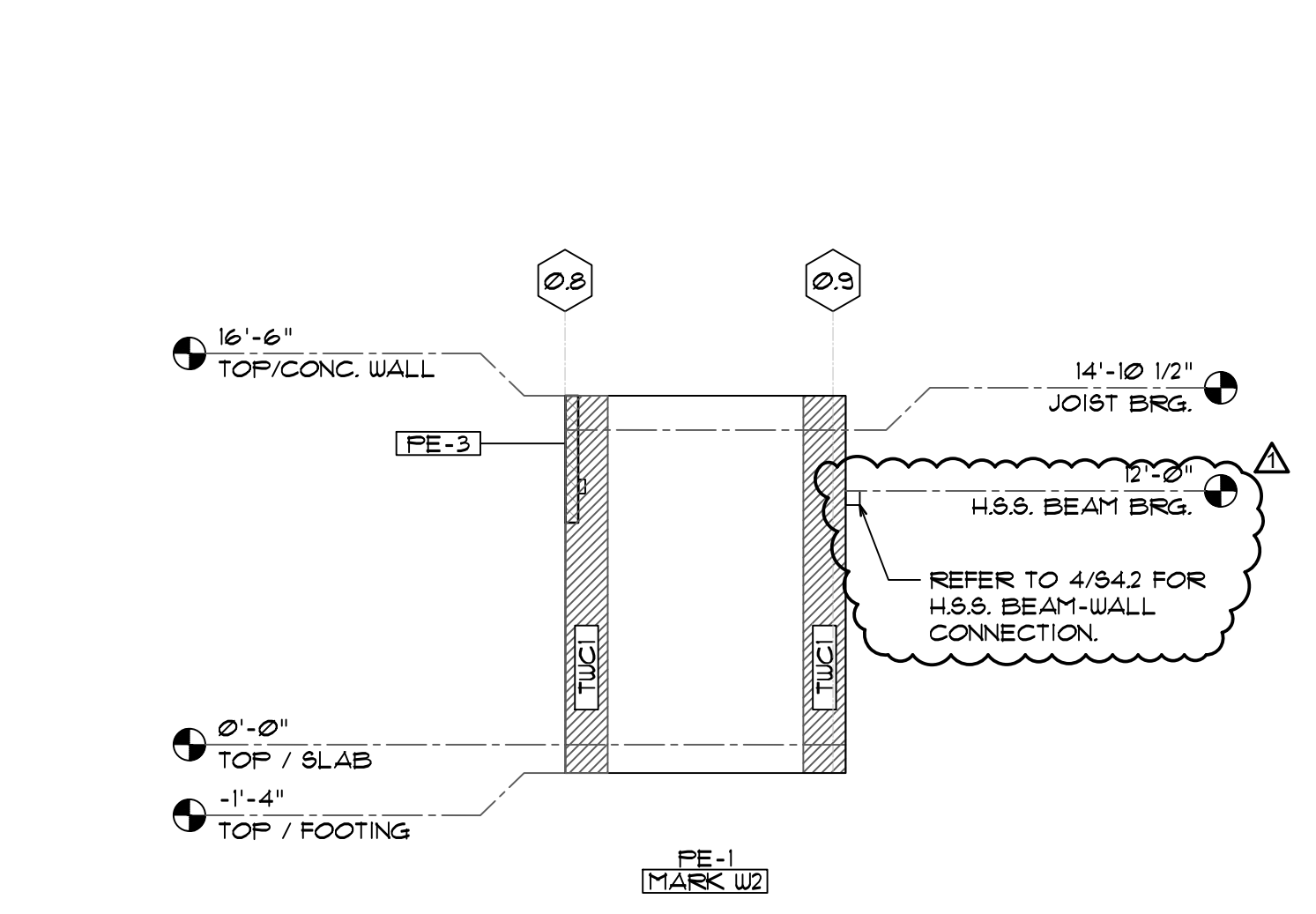
2 SOUTH FACE ELEVATION
SCALE: 1/8" = 1'-0"



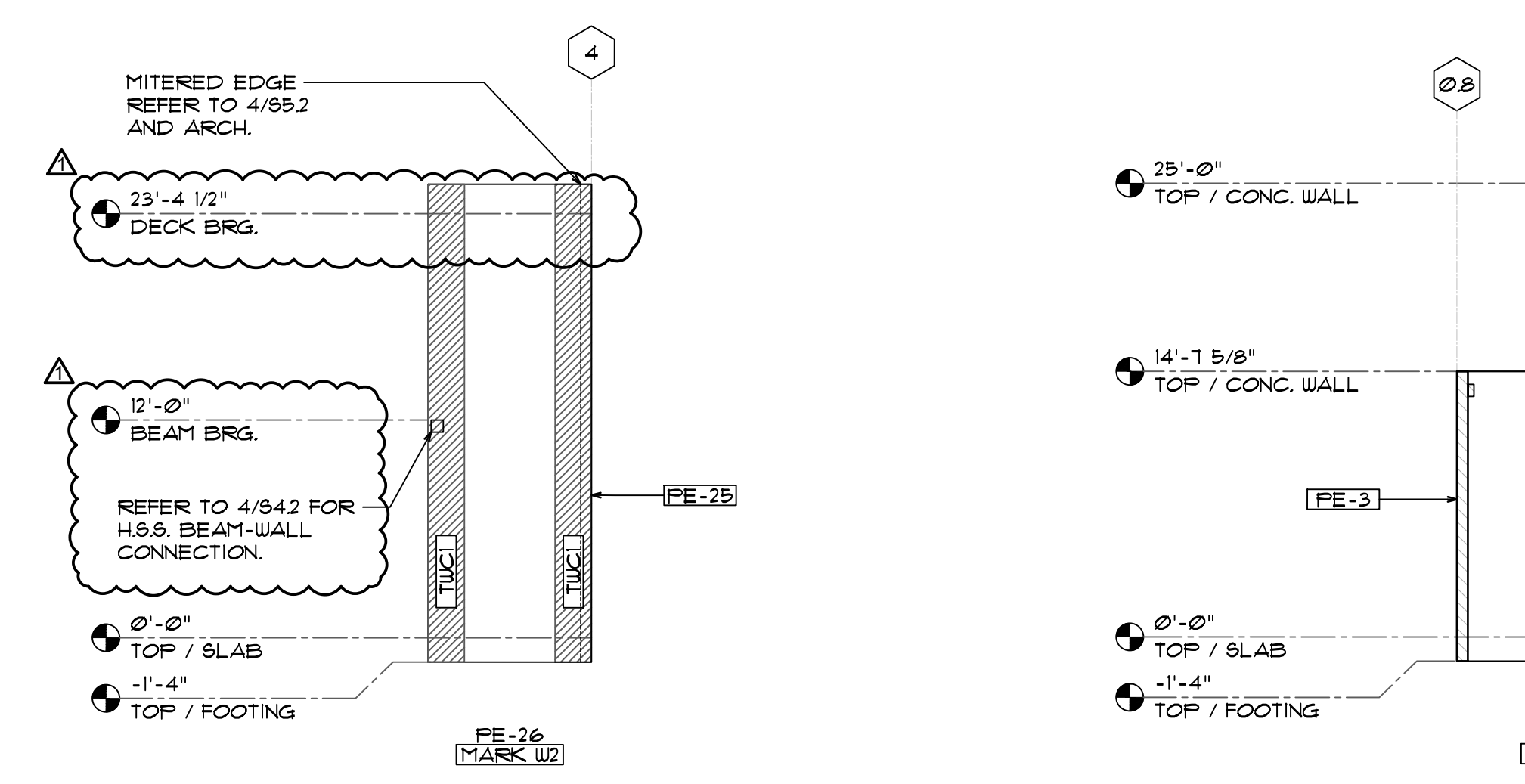
3 WEST FACE ELEVATION
SCALE: 1/8" = 1'-0"



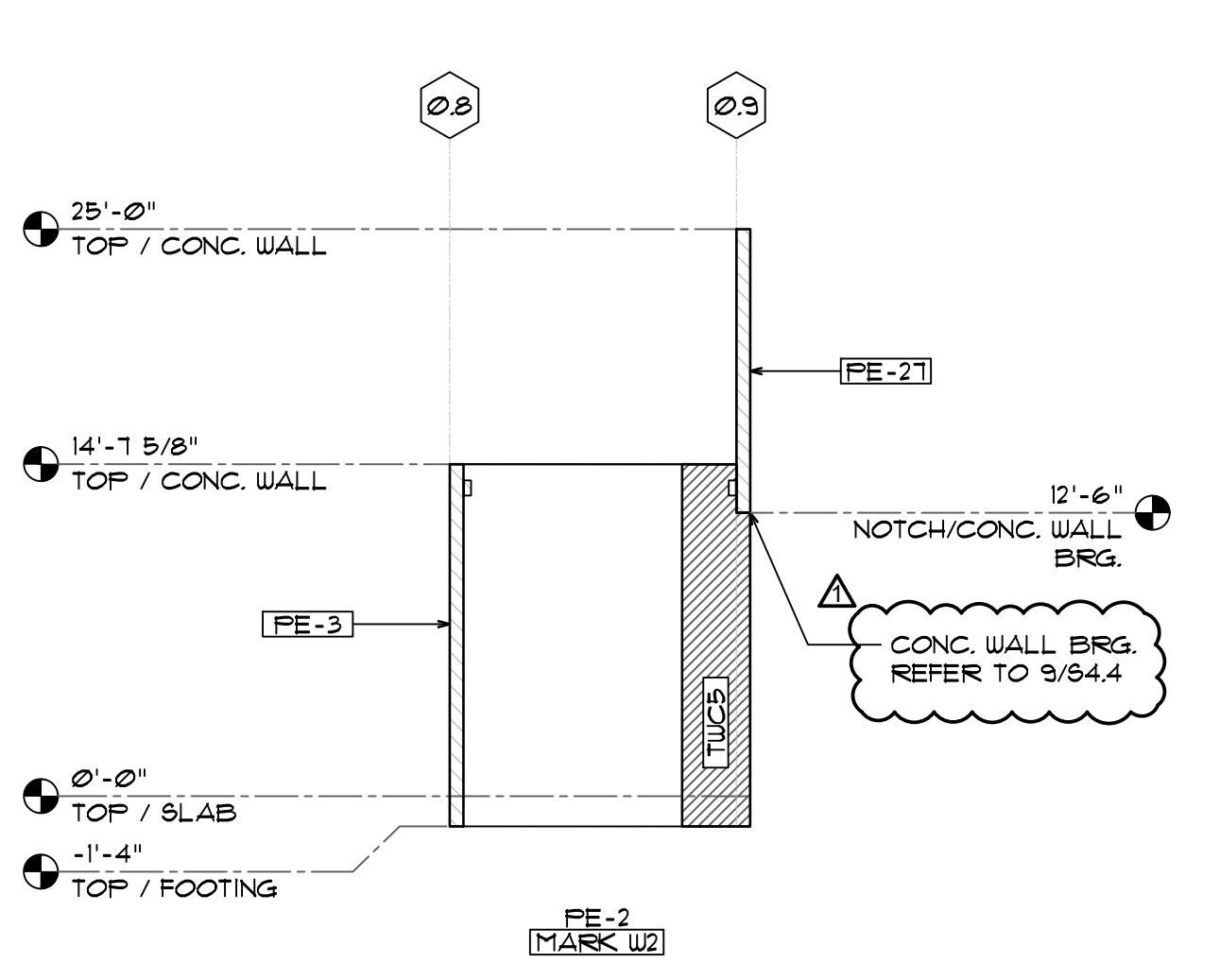
4 NORTH FACE ELEVATION
SCALE: 1/8" = 1'-0"



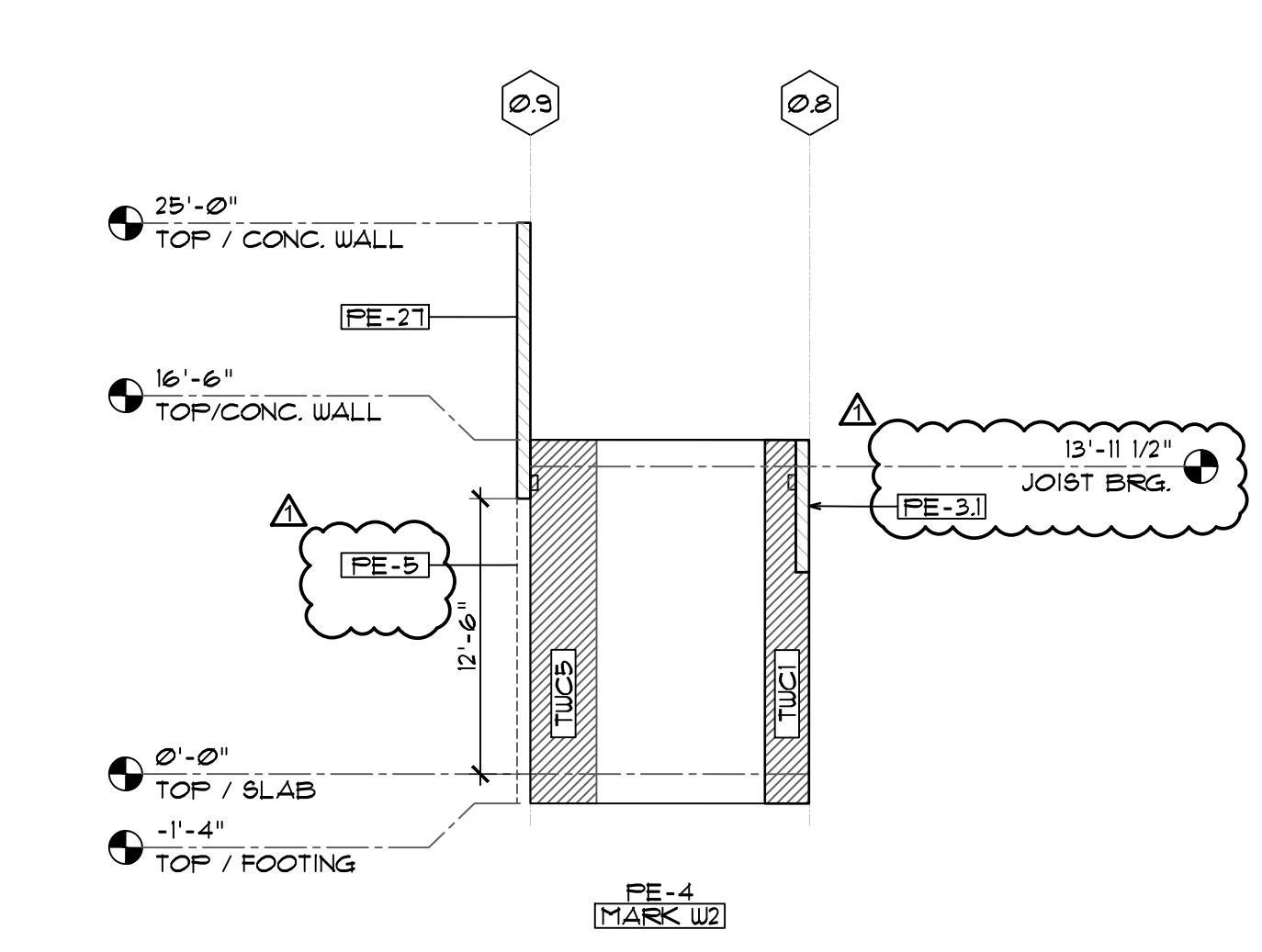
5 EAST FACE ELEVATION
SCALE: 1/8" = 1'-0"



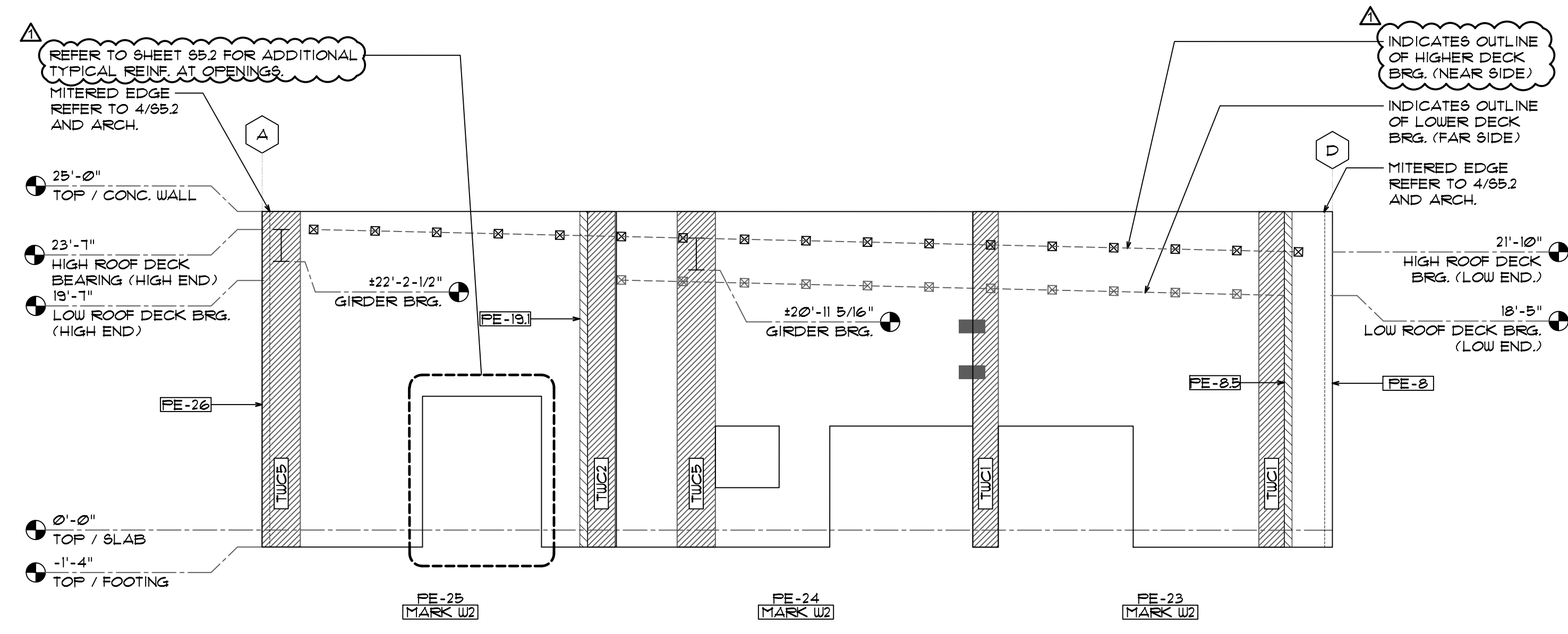
6 EAST FACE ELEVATION
SCALE: 1/8" = 1'-0"



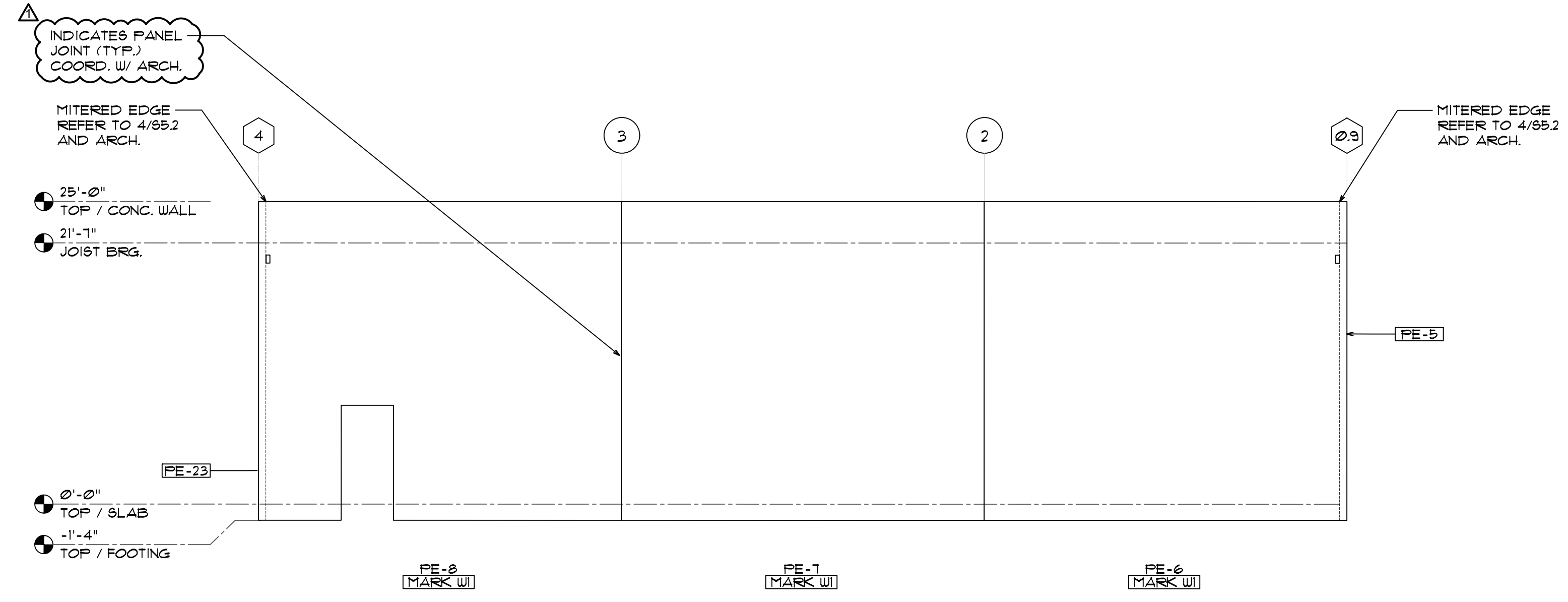
7 EAST FACE ELEVATION
SCALE: 1/8" = 1'-0"



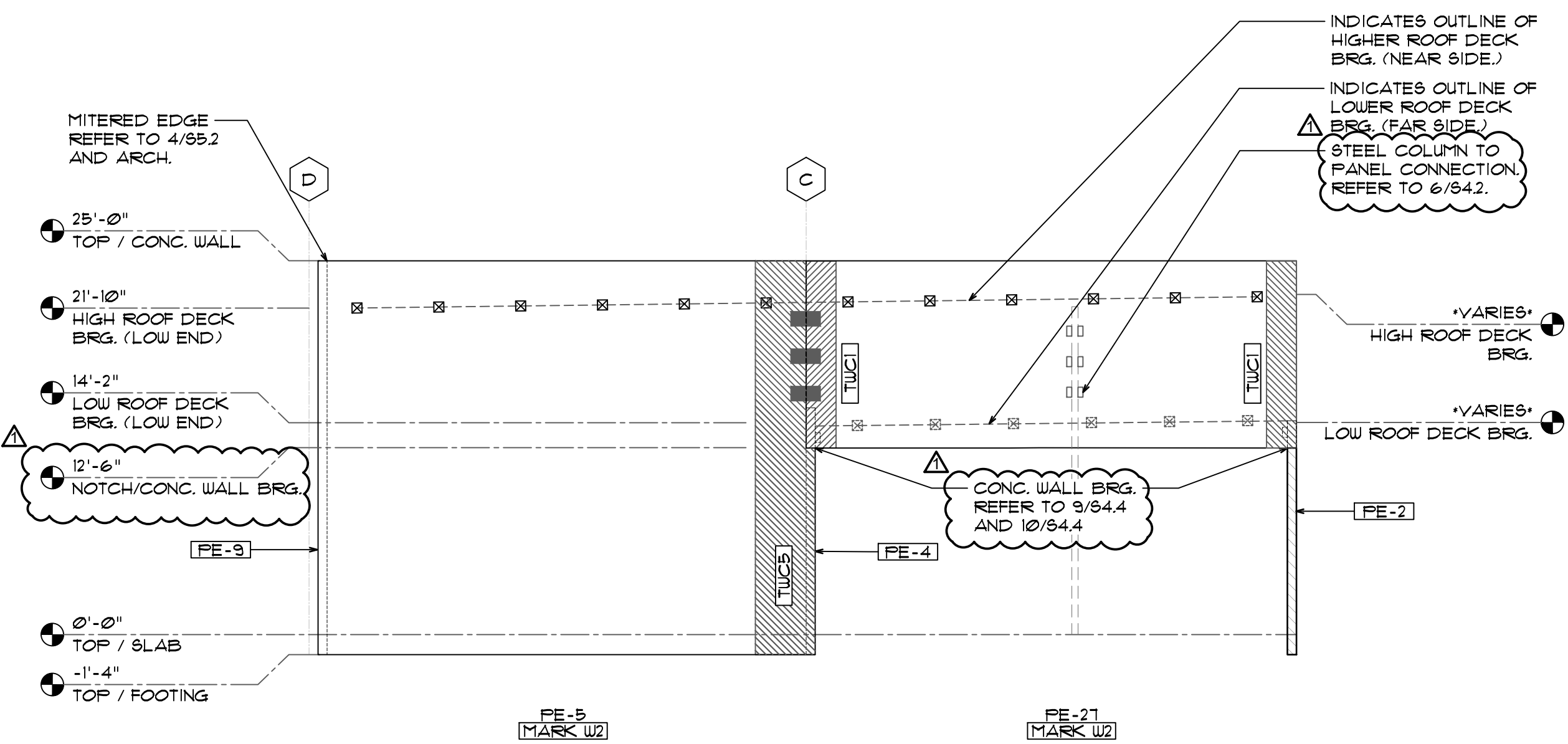
8 WEST FACE ELEVATION
SCALE: 1/8" = 1'-0"



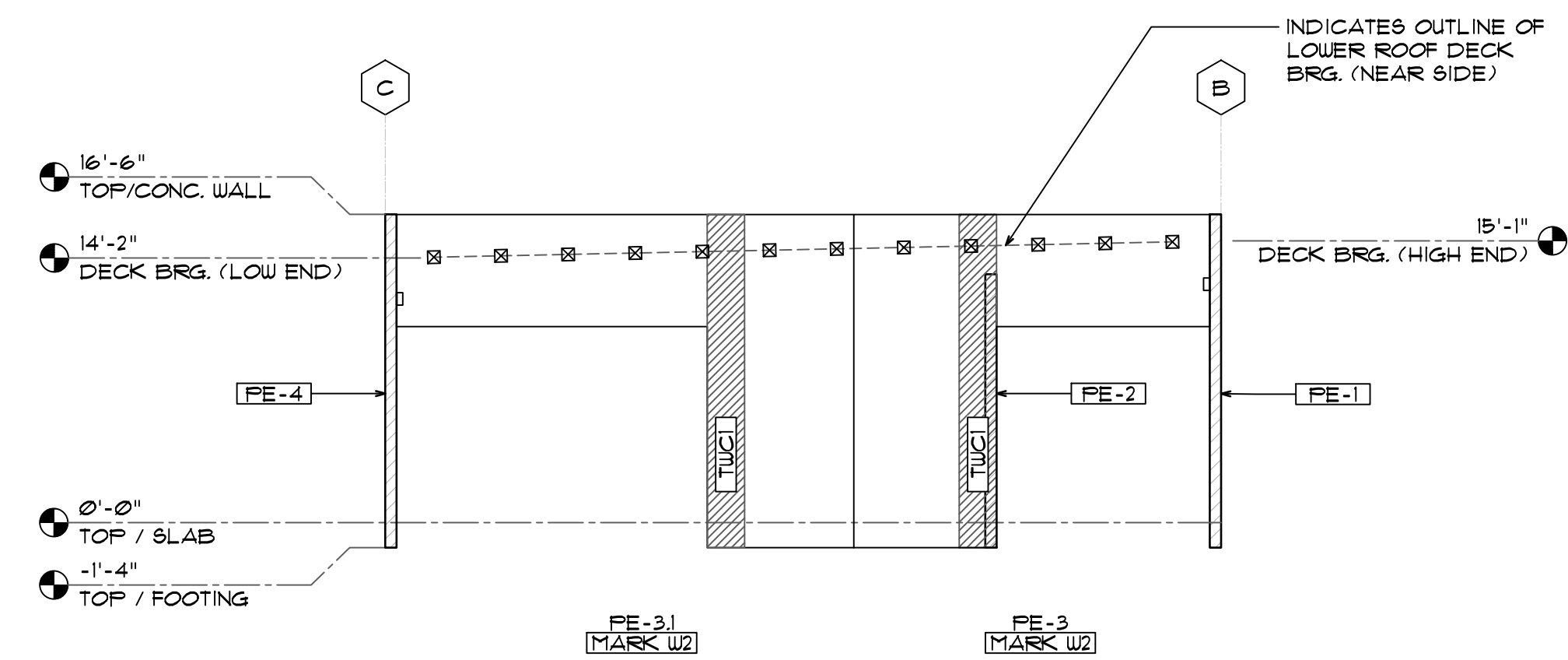
9 SOUTH FACE ELEVATION
SCALE: 1/8" = 1'-0"



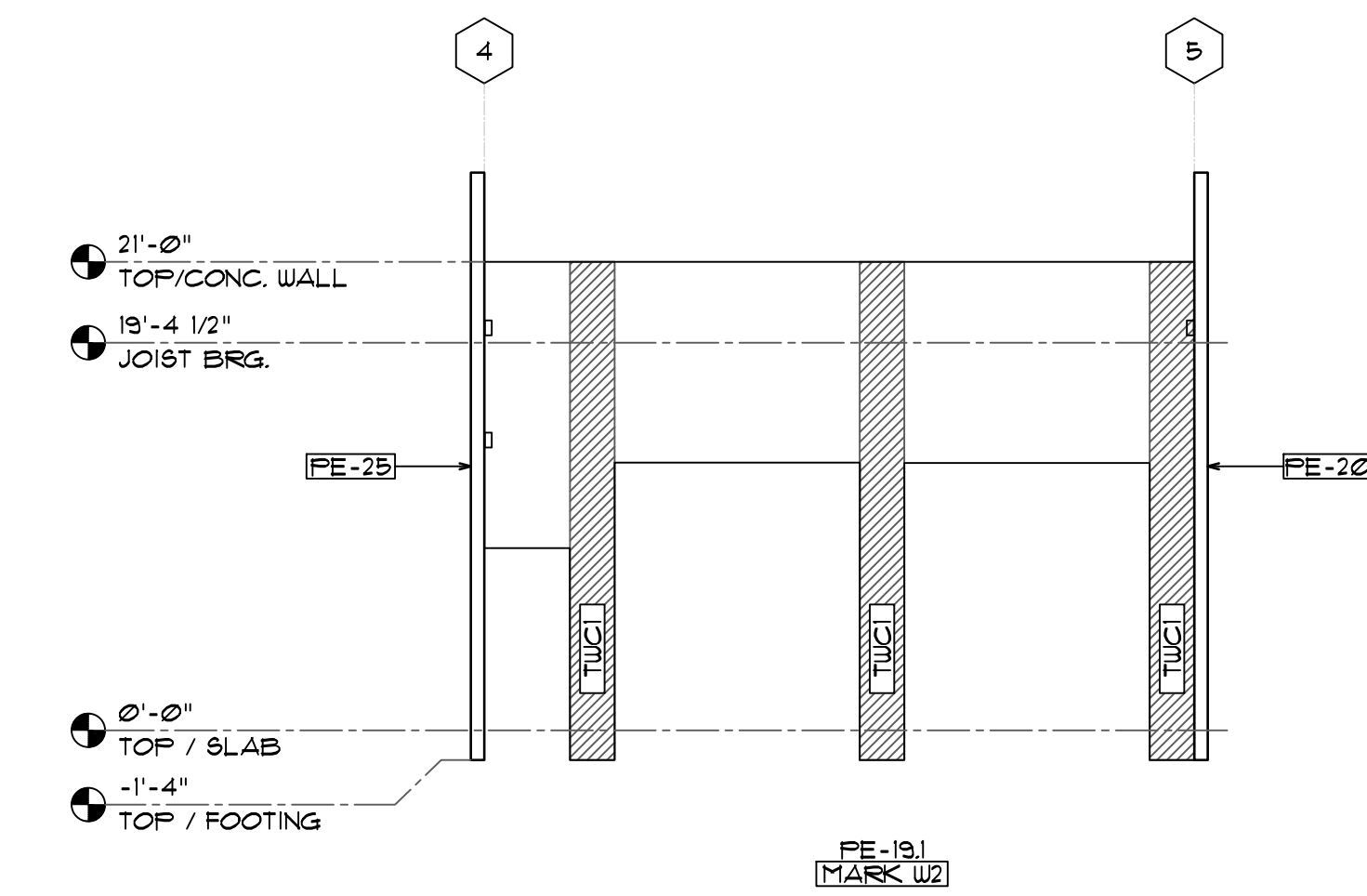
10 WEST FACE ELEVATION
SCALE: 1/8" = 1'-0"



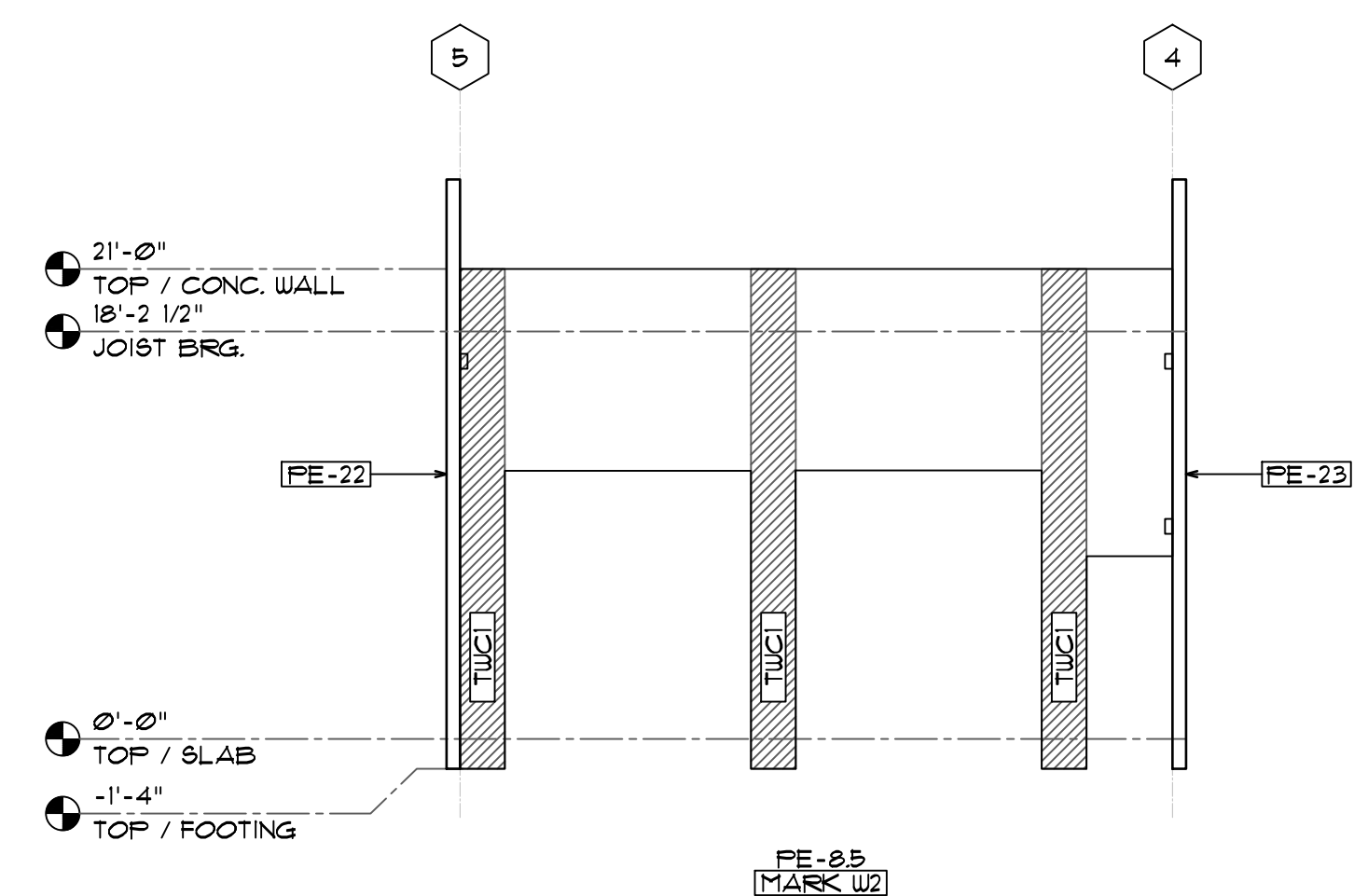
11 NORTH FACE ELEVATION
SCALE: 1/8" = 1'-0"



12 NORTH FACE ELEVATION
SCALE: 1/8" = 1'-0"

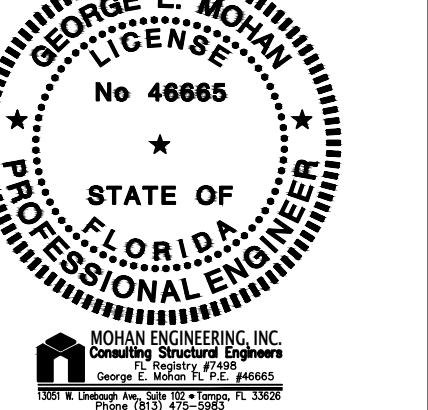


13 EAST FACE ELEVATION
SCALE: 1/8" = 1'-0"



14 WEST FACE ELEVATION
SCALE: 1/8" = 1'-0"

△	3/7/2024 ADDENDUM NO. 1
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△	



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