DESIGN CRITERIA

1. GOVERNING BUILDING CODE: ... **2020 FLORIDA BUILDING CODE**
2. RISK CATEGORY ... **II**
3. SUPERIMPOSED DEAD
   ROOF:
   - SPRINKLERS ... **3 PSF**
   - MECHANICAL & LIGHTING ... **3 PSF**
   - ROOFING AND MISC ... **3 PSF**
   - STRUCTURE ... **6 PSF**
   - TOTAL ... **15 PSF**
   - POINT LOAD ON AT COLUMN FOR HVAC UNIT ... **3,5000 LB**
4. LIVE
   ROOF ... **20 PSF**
   *LIVE LOAD REDUCTION CONSIDERED FOR JOIST GIRDERS AND COLUMNS DESIGN
5. SNOW
   GROUND SNOW (Pg) ... **0 PSF**
6. RAIN
   DESIGN RAINFALL INTENSITY (i) ... **8.77 IN/HR**
7. WIND
   ULTIMATE DESIGN WIND SPEED (3 SECOND GUST), Vult ... **150 MPH**
   NOMINAL DESIGN WIND WIND SPEED (3 SECOND GUST), Vasd ... **117 MPH**
   WIND EXPOSURE CATEGORY ... **C**
   INTERNAL PRESSURE COEFFICIENT, GCpi ... **+/-0.18**
   DESIGN WIND PRESSURE ON COMPONENTS AND CLADDING

GROSS ROOF PRESSURE (0.6W) - C & C	EFFECTIVE TRIBUTARY AREA	
	≤10 SQ. FT.	≥200 SQ. FT.
CORNER ZONE (3)	-98.3 PSF	-58.2 PSF
END ZONE (2)	-72.1 PSF	-52.1 PSF
INTERIOR ZONE (1)	-54.7 PSF	-39.1 PSF
INTERIOR ZONE (1')	-31.4 PSF	-27.0 PSF
POSITIVE (ALL ZONES)	14.0 PSF	11.1 PSF

WALL PRESSURES (0.6W) - C & C	EFFECTIVE TRIBUTARY AREA	
	≤10 SQ. FT.	≥200 SQ. FT.
ZONE 4	-34.0 PSF	-28.0 PSF
ZONE 5	-41.9 PSF	-29.9 PSF
ZONE 4 & 5	31.4 PSF	25.4 PSF

ROOF ALL OTHER CONDITIIONS (0.6W) - MWFRS	EFFECTIVE TRIBUTARY AREA
	>700 SQ. FT.
FOR 0 TO H/2 = 0 FT TO 16 FT	-27.5 PSF
H/2 TO H = 16 FT TO 32 FT	-27.5 PSF
H TO 2H = 32 FT TO 64 FT	-17.6 PSF
>2H = >64 FT	-12.7 PSF

NOTES:
1. RE: ASCE 7-16 FIGURES 30.3-1 AND 30.3-2A.
2. REFER TO CODE FOR EFFECTIVE TRIBUTARY AREAS NOT LISTED.
3. POSITIVE VALUES SIGNIFY PRESSURES ACTING TOWARD THE NOTED SURFACE AND NEGATIVE VALUES SIGNIFY PRESSURES ACTING AWAY FOR THE NOTED SURFACE.

8. SEISMIC
   0.2 SEC. SPECTRAL RESPONSE ACCELERATION (Ss) ... **0.056**
   1.0 SEC. SPECTRAL RESPONSE ACCELERATION (S1) ... **0.032**
   DESIGN SPECTRAL ACCELERATION (SDS) ... **0.060**
   DESIGN SPECTRAL ACCELERATION (SD1) ... **0.051**
   SITE CLASSIFICATION ... **D (ASSUMED)**
   SEISMIC DESIGN CATEGORY ... **A**
   IMPORTANCE FACTOR ... **1.0**

GENERAL

1. THE PROJECT SPECIFICATIONS, DRAWINGS, STANDARD DETAILS, DETAILS IN THE DRAWINGS, AND THE STRUCTURAL NOTES ARE TO BE COMPLEMENTARY. IN THE CASE OF AN INCONSISTENCE NOT CLARIFIED BY THE DESIGNER OF RECORD THE MOST STRINGENT, HIGHEST QUALITY AND BEST QUALITY PROVISIONS SHALL BE PROVIDED.
2. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS BEFORE CONSTRUCTION. THE ARCHITECT SHALL BE NOTIFIED OF ANY DISCREPANCIES OR INCONSISTENCIES.DO NOT SCALE DRAWINGS; COORDINATE DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
3. ALL WORK SHALL CONFORM TO THE MINIMUM STANDARDS OF THE GOVERNING BUILDING CODE
4. RE: ARCHITECTURAL DRAWINGS INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
   A. SIZE AND LOCATION OF ALL OPENINGS, EXCEPT AS NOTED.
   B. SIZE AND LOCATION OF ALL INTERIOR AND EXTERIOR NONBEARING WALLS
   C. SIZE AND LOCATION OF ALL CONCRETE CURBS, FLOOR DRAINS, SLOPES, DEPRESSED AREAS, CHANGES IN LEVEL, CHAMFERS, GROOVES, INSERTS, ETC.
   D. SIZE AND LOCATION OF ALL FLOOR AND ROOF OPENINGS, EXCEPT AS SHOWN.
   E. FLOOR AND ROOF FINISHES.
   F. STAIR FRAMING AND DETAILS, EXCEPT AS SHOWN.
   G. DIMENSIONS NOT SHOWN ON STRUCTURAL DRAWINGS.
5. RE: MEP CONSTRUCTION DOCUMENTS FOR THE FOLLOWING, INCLUDING BUT NOT LIMITED TO:
   A. PIPE RUNS, SLEEVES, HANGERS, TRENCHES, WALL AND SLAB OPENINGS, ETC., EXCEPT AS SHOWN OR NOTED.
   B. ELECTRICAL CONDUIT RUNS, BOXES, OUTLETS IN WALLS AND SLABS.
   C. CONCRETE INSERTS FOR FIXTURES.
   D. SIZE AND LOCATION OF MACHINE OR EQUIPMENT BASES, ANCHOR BOLTS FOR MOTOR MOUNTS.
   E. SEISMIC BRACING REQUIREMENTS.
6. METHODS, PROCEDURES, AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN AND ENSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION.
7. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, WORKERS AND VISITORS DURING CONSTRUCTION. SUCH MEASURE SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, SHORING FOR CONSTRUCTION LOADS, ETC. VISITS TO THE SITE BY THE STRUCTURAL ENGINEER SHALL NOT INCLUDE REVIEW OF THE ABOVE ITEMS.
8. OPENINGS, POCKETS, ETC. SHALL NOT BE PLACED IN STRUCTURAL ELEMENTS UNLESS SPECIFICALLY DETAILED OR APPROVED BY THE STRUCTURAL ENGINEER.
9. CONSTRUCTION LOAD (MATERIAL AND EQUIPMENT) SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT. PROVIDE ADEQUATE SHORING AND/ OR BRACING WHERE STRUCTURES HAVE NOT ATTAINED DESIGN STRENGTH.
10. WHEN A DETAIL IS IDENTIFIED, THE CONTRACTOR SHALL APPLY THIS DETAIL IN ESTIMATING AND CONSTRUCTION TO EVERY LIKE CONDITION WHETHER OR NOT THE REFERENCE IS MADE IN EVERY INSTANCE.
11. ANY REFERENCES TO THE RECOMMENDATIONS, GUIDELINES, OR REQUIREMENTS IN NATIONAL PUBLICATIONS, SUCH AS BUT NOT LIMITED TO ASCE, ASTM, IBC, ACI, AISC, NDS, OR AWS, IN THE CONSTRUCTION DOCUMENTS SHALL BE FOLLOWED AS IF THEY ARE SPECIFICALLY MANDATED.

FOUNDATION

1. THE SUBSURFACE INFORMATION AND FOUNDATION DESIGN ARE BASED ON THE FOLLOWING GEOTECHNICAL REPORT:
   REPORT PREPARED BY ... **GEO-TECHNOLOGY ASSOCIATES, INC.**
   DATED ... **10/03/2023**
2. FOUNDATIONS FOR THE STRUCTURE HAVE BEEN DESIGNED USING THE FOLLOWING VALUES:
   ALLOWABLE SOIL BEARING ... **2,500 PSF**
   FROST DEPTH ... **18"**
3. THE CONTRACTOR SHALL PERFORM EXCAVATIONS, FOOTING CONSTRUCTION AND PREPARATION OF THE SUB GRADE UNDER THE SLAB ON GRADE IN ACCORDANCE WITH THE RECOMMENDATIONS CONTAINED IN THE GEOTECHNICAL REPORT AND THE PROJECT SPECIFICATIONS.
4. FOUNDATION CONDITIONS NOTED DURING CONSTRUCTION, WHICH DIFFER FROM THOSE DESCRIBED IN THE GEOTECHNICAL REPORT SHALL BE REPORTED TO THE STRUCTURAL ENGINEER AND/OR GEOTECHNICAL ENGINEER BEFORE FURTHER CONSTRUCTION IS ATTEMPTED.
5. CONTRACTOR WILL PROVIDE FOR DE-WATERING OF EXCAVATIONS FROM EITHER SURFACE, GROUND, OR SEEPAGE WATER.
6. ALL ABANDONED FOOTINGS, UTILITIES, ETC., THAT INTERFERE WITH NEW CONSTRUCTION SHALL BE REMOVED.
7. SITE PREPARATION, OVER-EXCAVATION / RECOMPACTION OF SOILS, AND THE INSTALLATION OF FOUNDATION AND WALL DRAINS AS REQ'D SHALL BE PERFORMED IN ACCORDANCE WITH RECOMMENDATIONS PRESENTED IN THE SOILS REPORT REFERENCED ABOVE.
8. CONTRACTOR SHALL PROVIDE FOR DESIGN AND INSTALLATION OF ALL CRIBBING, SHEATHING, AND SHORING REQUIRED TO SAFELY RETAIN THE EARTH BANKS.

CONCRETE

1. AGGREGATE FOR NORMAL WEIGHT CONCRETE SHALL CONFORM TO ASTM C33. AGGREGATE FOR LIGHTWEIGHT CONCRETE SHALL CONFORM TO ASTM C330. PORTLAND CEMENT SHALL BE TYPE I OR TYPE II AND SHALL CONFORM TO ASTM C150. MINIMUM COARSE AGGREGATE SIZE IS 1/2 INCH. USE AGGREGATES WITH A NOMINAL MAXIMUM SIZE OF 1 1/2" FOR SLABS ON GRADE.
2. ADMIXTURES MAY BE USED WITH PRIOR APPROVAL OF THE ENGINEER. ADMIXTURES USED TO INCREASE THE WORKABILITY OF THE CONCRETE SHALL NOT BE CONSIDERED TO REDUCE THE SPECIFIED MINIMUM CEMENT CONTENT. CALCIUM CHLORIDE SHALL NOT BE USED.
3. COMPRESSIVE STRENGTHS OF CONCRETE AT 28 DAYS SHALL BE AS FOLLOWS:
   - FOOTINGS & MECHANICAL PADS ... **3000 PSI**
   - PEDESTALS AND GRADE BEAMS ... **4500 PSI**
   - SLAB ON GRADE ... **4000 PSI**
   - CONCRETE TILT-WALL ... **4000 PSI**
   - CONCRETE FOUNDATION AND RETAINING WALLS ... **4000 PSI**
4. CONCRETE SLUMP SHALL BE 4 INCHES +/- 1 INCH. EXCEPTION: MIX DESIGNED WITH PLASTICISER OR WATER REDUCER.
5. MAXIMUM WEIGHT OF NORMAL-WEIGHT CONCRETE SHALL BE 150 PCF
6. REFER TO SPECIFICATIONS FOR ADDITIONAL CONCRETE MIX REQUIREMENTS.
7. REFER TO SPECIFICATIONS FOR LOW-CARBON CONCRETE BID ALTERNATE.
8. MIXING, TRANSPORTING, AND PLACING OF CONCRETE SHALL CONFORM TO THE LATEST EDITION OF ACI 304R AND PROJECT SPECIFICATIONS. ALL CONCRETE SURFACES AGAINST WHICH CONCRETE IS TO BE PLACED SHALL BE THOROUGHLY CLEANED. LAITANCE AND STANDING WATER SHALL BE REMOVED.
9. ALL REINFORCING BARS, WELDED WIRE FABRIC, ANCHOR BOLTS, EMBEDDED PLATES AND OTHER CONCRETE INSERTS SHALL BE WELL SECURED IN POSITION PRIOR TO PLACING CONCRETE. PROVIDE STANDARD BAR CHAIRS AND SPACERS AS REQUIRED TO MAINTAIN CONCRETE PROTECTION SPECIFIED. "PULLING-UP" WELDED WIRE FABRIC WITH HOOKS DURING CONCRETE PLACEMENT IS NOT PERMITTED.
10. CONCRETE COVER PROTECTION FOR REINFORCEMENT BARS SHALL BE AS FOLLOWS:
    (RE: ACI 318 SECTION 7.7 FOR CONDITIONS NOT NOTED.)
    A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH ... **3"**
    B. CONCRETE EXPOSED TO EARTH OR WEATHER:
       BARS #6 AND LARGER ... **2"**
       BARS #5 AND SMALLER ... **1 1/2"**
    C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
       SLABS, WALLS, JOISTS - #11 BARS AND SMALLER ... **3/4"**
       BEAMS, COLUMNS - TIES, STIRRUPS, SPIRALS ... **1 1/2"**
11. REINFORCING STEEL FOR CONCRETE SHALL BE GRADE 60 OR GRADE 75 AS SPECIFIED AND SHALL CONFORM TO ASTM A615 OR A706 (GRADE 60 ONLY) FOR WELD TYPE REINFORCING STEEL. REINFORCING BARS SHALL NOT BE TACK WELDED, WELDED, HEATED, OR CUT UNLESS INDICATED ON THE CONTRACT DOCUMENTS OR APPROVED BY THE STRUCTURAL ENGINEER.
12. WELDING REINFORCEMENT BARS, WHEN APPROVED BY THE STRUCTURAL ENGINEER, SHALL CONFORM TO THE AMERICAN WELDING SOCIETY STANDARD D1.4, LATEST EDITION.E70XX ELECTRODES SHALL BE USED IN WELDING A706 REINFORCING BARS TO STRUCTURAL STEEL.
13. DETAILING OF CONCRETE REINFORCEMENT BARS AND ACCESSORIES SHALL CONFORM TO THE RECOMMENDATIONS OF THE LATEST EDITION OF THE ACI 315 DETAILING MANUAL.
14. GROUT SHALL BE NON-SHRINK GROUT CONFORMING TO ASTM C1107 AND SHALL HAVE A SPECIFIED COMPRESSIVE STRENGTH AT 28 DAYS OF 5000 PSI. PRE GROUTING OF BASE PLATES WILL NOT BE PERMITTED.
15. FORMS FOR CONCRETE SHALL BE LAID OUT AND CONSTRUCTED TO PROVIDE FOR THE REQUIRED CAMBERS/SLOPES. DO NOT REMOVE FORMS OR BRACING UNTIL CONCRETE HAS GAINED THE SPECIFIED 28 DAY STRENGTH OR SUFFICIENT STRENGTH TO CARRY ITS OWN WEIGHT AND SUPERIMPOSED LOADS PER THE APPLICABLE PROVISIONS OF ACI 347.
16. CONDUIT OR PIPE SIZE (OD) SHALL NOT EXCEED 30 PERCENT OF SLAB THICKNESS AND SHALL BE PLACED BETWEEN TOP AND BOTTOM REINFORCING, UNLESS SPECIFICALLY DETAILED OTHERWISE. CONCENTRATION OF CONDUITS OR PIPES SHALL BE AVOIDED EXCEPT WHERE DETAILED OPENINGS ARE PROVIDED.
17. PROVIDE SLEEVES FOR PLUMBING AND ELECTRICAL OPENINGS IN CONCRETE BEFORE PLACING. CORING THROUGH CONCRETE IS NOT PERMITTED EXCEPT WHERE SHOWN. NOTIFY THE STRUCTURAL ENGINEER IN ADVANCE OF CONDITIONS NOT SHOWN ON THE DRAWINGS.
18. CURE AND PROTECT CONCRETE IMMEDIATELY AFTER PLACEMENT IN ACCORDANCE WITH ACI 308, ACI 305, AND ACI 306. CURING COMPOUNDS USED ON CONCRETE THAT IS TO RECEIVE A RESILIENT TILE FINISH SHALL BE APPROVED BY THE TILE MANUFACTURER BEFORE USE.
19. PROVIDE CONSTRUCTION OR CONTROL JOINTS IN SLABS-ON-GRADE AS SHOWN IN TYPICAL DETAILS SO AS TO DIVIDE SLABS INTO APPROXIMATELY RECTANGULAR AREAS NOT OVER 225 SQUARE FEET WITH A RATIO OF LONG TO SHORT SIDES NOT OVER 1.5 AND SPACING NOT EXCEEDING 15'-0" ON CENTER. IN ADDITION, PROVIDE CONTROL JOINTS OFF OF ALL REENTRANT CORNERS TO INTERSECTION OF CONTROL JOINTS BEYOND. PROVIDE CONTROL JOINTS TO CONNECT OFFSET COLUMNS, PITS AND OTHER INTERRUPTIONS TO THE SLAB.
20. AN INDEPENDENT TESTING AGENCY TO PERFORM FIELD QUALITY CONTROL TEST. PROVIDE FREE ACCESS TO CONCRETE OPERATIONS AT PROJECT SITE AND COOPERATE WITH APPOINTED FIRM. SUBMIT PROPOSED MIX DESIGN OF EACH CLASS OF CONCRETE TO INSPECTION AND TESTING FIRM FOR REVIEW PRIOR TO COMMENCEMENT OF CONCRETE OPERATIONS. COMPRESSIVE STRENGTH TESTS: ASTM C39/C39M. FOR EACH TEST, MOLD, AND CURE THREE CONCRETE TEST CYLINDERS. OBTAIN TEST SAMPLES FOR EVERY 100 CU YD OR LESS OF EACH CLASS OF CONCRETE PLACED. TAKE ONE ADDITIONAL THREE TEST CYLINDERS DURING COLD & HOT WEATHER CONCRETING AS DEFINED BY ACI 305 AND ACI 306, CURED ON JOB SITE UNDER SAME CONDITIONS AS CONCRETE IT REPRESENTS. PERFORM ONE SLUMP TEST FOR EACH SET OF TEST CYLINDERS TAKEN, FOLLOWING PROCEDURES OF ASTM C143/C143M. PERFORM ONE AIR CONTENT TEST FOR EACH SET OF COMPRESSIVE STRENGTH SPECIMENS, COMPLYING ASTM C231.
21. WHERE INDICATED ON THE DRAWINGS, INTENTIONALLY ROUGHENED CONCRETE SHALL BE CLEAN AND FREE OF LAITANCE AND ROUGHENED TO A FULL AMPLITUDE OF 1/4".

PRECAST WALL PANELS

1. THE PRECAST CONCRETE WALL SUPPLIER SHALL BE CERTIFIED BY THE NATIONAL PRECAST CONCRETE ASSOCIATION. VERIFY PLAN CERTIFICATION PRIOR TO THE BEGINNING OF WALL PRODUCTION.
2. THE PRECAST WALL SUPPLIER SHALL PRODUCE A COMPLETE SET OF SHOP DRAWINGS AND CALCULATIONS SIGNED AND SEALED BY A REGISTERED ENGINEER LICENSED IN THE PROJECT STATE. THE SHOP DRAWINGS SHALL INCLUDE ALL INFORMATION REQUIRED TO PROPERLY INSTALL PRECAST PANELS AND ANY DETAILS REQUIRED FOR INTERFACING WITH THE BUILDING FRAME. ALL CONNECTIONS TO THE PRECAST PANELS SHALL BE DESIGNED AND DETAILED BY THE PRECAST SUPPLIER. ALL LIFTING AND ERECTION REQUIREMENTS SHALL BE INCLUDED DESIGNED AND DETAILS BY PRECAST PANEL SUPPLIER.
3. PRECAST PANEL ERECTION SHALL BE THE RESPONSIBILITY OF THE PRECAST PANEL SUPPLIER.
4. FOUNDATIONS SHALL ACHIEVE 75 PERCENT OF THE SPECIFIED MINIMUM CONCRETE COMPRESSIVE STRENGTH (F'c) PRIOR TO THE ERECTION OF PRECAST PANELS. GENERAL CONTRACTOR SHALL VERIFY CONCRETE COMPRESSIVE STRENGTH.
5. THE PRECAST PANELS SHALL BE BRACED DURING INSTALLATION TO WITHSTAND THE DESIGN WIND AND SEISMIC LOADS. SPECIAL CONDITIONS SHALL BE CLEARLY NOTED AND REVIEWED WITH THE GENERAL CONTRACTOR. THE PRECAST PANEL SUPPLIER SHALL REVIEW THE INSTALLATION PROCEDURES WITH THE GENERAL CONTRACTOR TO ENSURE PROPER INSTALLATION.
6. ROOF AND FLOOR DECK SHALL BE INSTALLED AND ALL CONNECTIONS BETWEEN THE STEEL AND PRECAST PANELS MADE BEFORE THE TEMPORARY BRACING IS REMOVED.
7. DAMAGED PRECAST PANELS SHALL BE REPORTED IMMEDIATELY AND SHALL NOT BE INSTALLED WITHOUT APPROVAL FROM THE ARCHITECT.
8. PRECAST WALLS SHALL BE TRANSPORTED HANDLED, LIFTED AND STORED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
9. PRECAST MANUFACTURER SHALL BE FABCON.
10. BEARING PADS SHALL BE ELASTOMERIC PLAIN, VULCANIZED, 100% POLYCHLORPROPENE (NEOPRENE) ELASTOMER MEETING AASH TO M251, MOLDED TO SIZE OR CUT FROM A MOLDED SHEET, WITH A MINIMUM TENSILE STRENGTH OF 2250 PSI.
11. PROVIDE SUITABLE 40'-0" WIDE ALL-WEATHER ACCESS TO AND AROUND BUILDING WITH PROPER DRAINAGE AND FIRM LEVEL BEARING FOR HAULING AND ERECTION EQUIPMENT, COORDINATE WITH PRECAST SUPPLIER
12. PROVIDE TRUE, LEVEL BEARING SURFACES FOR WALLS. ENSURE BEARING SURFACES ARE CLEAN AND READY FOR PRECAST PANEL INSTALLATION.
13. LIFT PANELS BY MEANS OF SUITABLE LIFTING DEVICES AT POINTS PROVIDED BY THE MANUFACTURER. PROVIDE TEMPORARY SHORING AND BRACING IN COMPLIANCE WITH THE MANUFACTURER'S RECOMMENDATIONS, APPLICABLE BUILDING CODES AND PCI OR NPCA DESIGN REQUIREMENTS. TEMPORARY BRACING DESIGN SHALL BE SIGNED AND SEALED BY A STRUCTURAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED.

METAL DECK

1. METAL DECK SHALL CONFORM TO THE SPECIFICATIONS OF THE STEEL DECK INSTITUTE.
2. ROOF NON-COMPOSITE DECK SHALL BE 1 1/2" TYPE B -36 (GRADE 50) WIDE RIB RE: ROOF DIAPHRAGM DIAGRAM FOR GAUGE & CONNECTIONS.
3. UNLESS NOTED OTHERWISE ON PLAN, CONNECT DECKING TO SUPPORTING MEMBERS WITH MECHANICAL FASTENERS SPACED AT 12" ON CENTER.
4. IF WELDING IS BASIS OF DESIGN, WELDERS SHALL BE AWS-CERTIFIED FOR LIGHT-GAGE WELDING.
5. LAYOUT DECK SPANS TO PROVIDE A MINIMUM OF (3) SPANS PER LENGTH OF DECK.
6. PROVIDE L5x3x1/4 (LLV) FIELD-FABRICATED FRAME BETWEEN JOISTS AT OPENINGS LARGER THAN 10"x10", U.N.O., (INCLUDING EXHAUST FAN OPENINGS REGARDLESS OF OPENING SIZE). RE: 1/S0.11

LIGHT GAGE STEEL FRAMING

1. LIGHT-GAGE STEEL SHALL CONFORM TO:
   A. ASTM A 653 SS GRADE 50, CLASS 1 OR CLASS 3 (Fy = 50 KSI) FOR 54 MILS THROUGH 68 MILS THICKNESS.
   B. ASTM A 653 SS GRADE 33 (Fy = 33 KSI) FOR 18 MILS THROUGH 43 MILS THICKNESS.
2. ALL FABRICATION, ERECTION, AND IDENTIFICATION OF LIGHT-GAGE STEEL FRAMING SHALL CONFORM TO IBC SECTIONS 2209 AND 2210 AND AISI SPECIFICATIONS.
3. PROVIDE ALL ACCESSORIES INCLUDING, BUT NOT LIMITED TO, TRACKS, CLIPS, WEB STIFFENERS, ANCHORS, FASTENING DEVICES AND OTHER ACCESSORIES REQUIRED FOR A COMPLETE AND PROPER INSTALLATION.
4. INSTALL BRIDGING/BLOCKING IN LIGHT-GAGE STEEL STUD WALLS IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND AS SHOWN IN THE DRAWINGS.
5. WELD LIGHT-GAGE STEEL FRAMING CONNECTIONS, EXCEPT WHERE SELF- DRILLING SCREWS ARE SPECIFIED.
6. WELDS SHALL CONFORM TO AWS SPECIFICATIONS. WELDERS SHALL BE CERTIFIED FOR LIGHT-GAGE STEEL UNDER AWS SPECIFICATIONS.
7. DESIGNATIONS OF COLD-FORMED, LIGHT-GAGE STEEL SHAPES REFER TO THOSE DESCRIBED IN ICC-ES EVALUATION REPORT ESR-3064P OF THE METAL STUD MANUFACTURERS' ASSOCIATION.
8. SHEET METAL SCREWS SHALL BE OF THE MAKE SPECIFIED IN THE DRAWINGS OR, WHERE NO SPECIFIC MAKE IS GIVEN, SHALL BE RATED BY THEIR MANUFACTURER AS POSSESSING DESIGN LOAD CAPACITIES IN SHEAR AND TENSION AT LEAST EQUAL TO THOSE PUBLISHED IN ICBO EVALUATION REPORT NO. 4943 OF THE METAL STUD MANUFACTURERS' ASSOCIATION FOR THE SCREW SIZE SPECIFIED.
9. FOR EXTERIOR STUDS RE: DETAILS AT PERIMETER. FOR NON-BEARING INTERIOR STUDS RE: ARCH.

STRUCTURAL STEEL

1. ALL W-SECTION SHAPES SHALL CONFORM TO ASTM A992. CHANNEL SHAPES AND PLATES SHALL CONFORM TO ASTM A36. (UNLESS OTHERWISE NOTED ON THE DWG).
2. STRUCTURAL STEEL PIPE SHALL CONFORM TO ASTM A53, GRADE B (Fy = 35 KSI). MILL TEST REPORTS FOR STEEL PIPE SHALL BE SUBMITTED FOR APPROVAL.
3. HOLLOW STRUCTURAL SECTIONS SHALL CONFORM TO ASTM A500, GRADE C (Fy = 50 KSI RECTANGULAR, Fy = 46 KSI ROUND).
4. ANCHOR BOLTS SHALL CONFORM TO ASTM F1554 GR 36, UNLESS NOTED OTHERWISE.
5. STRUCTURAL STEEL DETAILING, FABRICATION AND ERECTION SHALL CONFORM TO THE LATEST EDITION OF AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" WITH AMENDMENTS, AND THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES," WITH AMENDMENTS.
6. BOLTS FOR STEEL BEAM AND COLUMN CONNECTIONS SHALL BE 3/4-INCH DIAMETER (MIN.) ASTM F3125, GRADE A325-N HIGH-STRENGTH BOLTS UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS.
7. ALL BOLTED JOINTS SHALL BE SNUG TIGHT UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS . FOR MARKING, TWIST-OFF-TYPE TENSION CONTROL BOLT ASSEMBLIES (ASTM F3125, GRADE F1852), OR DIRECT TENSION PRETENSIONED OR SLIP-CRITICAL JOINTS, THE METHOD OF INSTALLATION SHALL BE TURN-OF-NUT WITH MATCH INDICATORS (ASTM F959).
8. SPLICING OF STRUCTURAL STEEL MEMBERS WHERE NOT DETAILED ON THE CONTRACT DOCUMENTS IS PROHIBITED WITHOUT THE PRIOR APPROVAL OF THE STRUCTURAL ENGINEER AS TO LOCATION, TYPE OF SPLICE AND CONNECTION TO BE MADE.
9. HEADED CONCRETE ANCHORS SHALL BE NELSON HEADED CONCRETE ANCHORS (OR APPROVED EQUAL), AND SHALL CONFORM TO ASTM A108. ANCHORS SHALL BE AUTOMATICALLY END WELDED WITH SUITABLE STUD WELDING EQUIPMENT IN THE SHOP OR IN THE FIELD. WELDING SHALL BE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE NELSON STUD WELDING COMPANY.
10. DEFORMED BAR ANCHORS (DBA) SHALL BE NELSON DEFORMED BAR ANCHORS (OR APPROVED EQUAL), AND SHALL BE MADE FROM LOW CARBON STEEL CONFORMING TO ASTM A496. ANCHORS SHALL BE AUTOMATICALLY END-WELDED WITH SUITABLE WELDING EQUIPMENT IN THE SHOP OR IN THE FIELD. WELDING SHALL BE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE NELSON STUD WELDING COMPANY.
11. WELDS USED IN MEMBERS & CONNECTIONS DESIGNATED IN THE DRAWINGS AS SEISMIC FORCE REISSTING SYSTEM (SFRS) SHALL BE MADE WITH FILLER METALS MEETING THE REQUIREMENTS IN AWS D1.8 SECTION 6.3 (AISC341-10 SECTIONS A3.4a&b). WELDS USED IN MEMBERS & CONNECTIONS DESIGNATED IN THE DRAWINGS AS DEMAND CRITICAL (DC) SHALL BE MADE WITH FILLER METALS MEETING THE REQUIREMENTS IN AWS D1.8 SECTION 6.3, INCLUDING SUB-CLAUSES 6.3.5, 6.3.6, 6.3.7, & 6.3.8
12. SUBMIT A WELDING PROCEDURE IN ACCORDANCE WITH LATEST EDITION OF AWS D1.1. WHERE WELDS ARE FOR MEMBERS DESIGNATED PART OF THE SFRS OR LABELED DEMAND CRITICAL, WELDING PROCEDURES SHALL CONFORM TO AWS D1.8 AND MANUFACTURER'S RECOMMENDATIONS (WHERE APPLICABLE). APPROVED PROCEDURES TO BE SUBMITTED TO SPECIAL INSPECTOR FOR REVIEW AND APPROVAL THEN TO THE ENGINEER FOR REVIEW.
13. WELDS SHALL CONFORM TO AWS SPECIFICATIONS. WELDERS SHALL BE CERTIFIED UNDER AWS SPECIFICATIONS. E70xx ELECTRODES SHALL BE USED FOR ALL WELDS.
14. RE: FRAME ELEVATIONS FOR LOCATION OF PROTECTED ZONES FOR LATERAL RESISTIVE FRAMES. NO CONNECTIONS OR ATTACHMENTS ARE PERMITTED WITHIN PROTECTED ZONES.
15. LOWEST ANTICIPATED SERVICE TEMPERATURE (LAST) SHALL BE 50° F FOR INDOOR CONDITIONED STRUCTURES & 0° F FOR OUTDOOR/UNCONDITIONED STRUCTURES
16. ALL EXTERIOR STEEL TO BE GALVANIZED. PLUG GALV HOLES w/ ALUMINUM PLUGS.

POST-INSTALLED ANCHORS

1. POST-INSTALLED ANCHOR SYSTEMS SHALL COMPLY WITH THE LATEST REVISION OF ICC-ES ACCEPTANCE CRITERIA AND HAVE A VALID ICC-ES REPORT (OR APPROVED EQUIVALENT) IN ACCORDANCE WITH THE APPLICABLE BUILDING CODE.
2. UNLESS OTHERWISE NOTED ON THE DRAWINGS USE ANCHORS LISTED BELOW:
   - EXPANSION ANCHORS IN CONCRETE SHALL BE ONE OF THE FOLLOWING:
     - HILTI HSL-3 CARBON STEEL HEAVY DUTY EXPANSION ANCHOR (ICC-ES REPORT SR-1545)
     - HILTI HDA CARBON AND STAINLESS STEEL UNDERCUT ANCHOR (ICC-ES REPORT ESR-1546)
     - HILTI KWIK BOLT TZ CARBON AND STAINLESS STEEL ANCHORS (ICC-ES REPORT ESR-1917)
     - DeWALT POWER-STUD+SD2 ANCHOR (ICC-ES REPORT ESR-2502)
     - SIMPSON STRONG-TIE STRONG-BOLT 2 ANCHOR (ICC-ES REPORT ESR-3037)
   - ADHESIVE ANCHORS IN CONCRETE SHALL BE ONE OF THE FOLLOWING:
     - HILTI HIT-RE 500 V3 ADHESIVE ANCHOR (ICC-ES REPORT ESR-3814)
     - HILTI HIT-HY 200 ADHESIVE ANCHOR (ICC-ES REPORT ESR-3187)
     - DeWALT PURE 110+ EPOXY ADHESIVE ANCHOR (ICC-ES REPORT ESR-3298)
     - DeWALT AC200+ ADHESIVE ANCHOR (ICC-ES REPORT ESR-4027)
     - SIMPSON STRONG-TIE SET-XP EPOXY ADHESIVE ANCHOR (ICC-ES REPORT ESR-2508)
     - SIMPSON STRONG-TIE AT-XP EPOXY ADHESIVE ANCHOR (IAPMO UES ER-263)
   - SCREW ANCHORS IN CONCRETE SHALL BE ONE OF THE FOLLOWING:
     - DeWALT SCREW-BOLT+ SCREW ANCHOR (ICC-ES REPORT ESR-3889)
     - HILTI KWIK HUS-EZ SCREW ANCHOR (ICC-ES REPORT ESR-3027)
     - SIMPSON STRONG-TIE TITEN HD SCREW ANCHOR (ICC-ES REPORT ESR-2713)
   - ANCHORS IN CONCRETE OVER STEEL DECK SHALL BE ONE OF THE FOLLOWING:
     - HILTI KWIK BOLT TZ CARBON AND STAINLESS STEEL ANCHORS (ICC-ES REPORT ESR-1917)
     - HILTI HIT-RE 500 V3 ADHESIVE ANCHORS (ICC-ES REPORT ESR-3814)
     - DeWALT POWER-STUD+SD2 EXPANSION ANCHOR (ICC-ES REPORT ESR-2502)
     - DeWALT POWER-STUD+SD1 EXPANSION ANCHOR (ISS-ES REPORT ESR-2818)
     - DeWALT SCREW-BOLT+ SCREW ANCHOR (ICC-ES REPORT ESR-3889)
     - SIMPSON STRONG-TIE STRONG-BOLT 2 WEDGE ANCHOR (ICC-ES REPORT ESR-3037)
     - SIMPSON STRONG-TIE TITEN HD SCREW ANCHOR (ICC-ES REPORT ESR-2713)
   - EXPANSION ANCHORS IN MASONRY SHALL BE ONE OF THE FOLLOWING:
     - HILTI KWIK BOLT 3 (KB3) ANCHORS (ICC-ES ESR-1385)
     - DeWALT POWER-STUD+SD1 (ICC-ES ESR-2818)
     - SIMPSON STRONG-TIE WEDGE-ALL ANCHOR (ICC-ES REPORT ESR-1396)
     - SIMPSON STRONG-TIE STRONG-BOLT 2 WEDGE ANCHOR (IAPMO UES ER-240)
   - ADHESIVE ANCHORS IN MASONRY SHALL BE ONE OF THE FOLLOWING:
     - HILTI HIT-HY 270 ADHESIVE ANCHOR (ICC-ES REPORT ESR 4143 &4144)
     - DeWALT AC100+ GOLD ADHESIVE ANCHOR (ICC-ES REPORT ESR-3200 FOR CMU & ICC-ES REPORT ESR-4105 FOR UNREINFORCED MASONRY)
     - SIMPSON STRONG-TIE SET EPOXY ADHESIVE ANCHOR (ICC-ES REPORT ESR-1772)
     - SIMPSON STRONG-TIE SET-XP EPOXY ADHESIVE ANCHOR (IAPMO UES ER-265)
     - SIMPSON STRONG-TIE AT-XP EPOXY ADHESIVE ANCHOR (IAPMO UES ER-281)
   - SCREW ANCHORS IN MASONRY SHALL BE ONE OF THE FOLLOWING:
     - HILTI KWIK HUS-EZ SCREW ANCHOR (ICC-ES REPORT ESR-3056)
     - DeWALT SCREW-BOLT+ SCREW ANCHOR (ICC-ES REPORT ESR-4042)
     - SIMPSON STRONG-TIE TITEN HD SCREW ANCHOR (ICC-ES REPORT ESR-1056)
3. ANCHORS INSTALLED IN THE BOTTOM OF CONCRETE OVER STEEL DECK SHALL BE INSTALLED IN THE BOTTOM FLUTE ONLY.
4. ANCHORS ARE NOT TO BE INSTALLED UNTIL CONCRETE HAS REACHED ITS DESIGN STRENGTH.
5. FOR ANCHOR EMBEDMENT, RE: DRAWINGS OR TYPICAL DETAIL. USE EMBEDMENT RECOMMENDED BY MANUFACTURER WHERE NO EMBEDMENT IS SHOWN.
6. MANUFACTURER'S INSTALLATION TRAINING AND CERTIFICATION IS REQUIRED ON ALL POST-INSTALLED ANCHORS FOR ANCHOR INSTALLER.
7. CONTRACTOR COORDINATE ANCHOR AND REINFORCING LOCATION. IT IS UNACCEPTABLE TO CUT REBAR FOR POST INSTALLED ANCHORS WITHOUT PRIOR APPROVAL FROM A&E.
8. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON DRAWINGS.

OPEN WEB STEEL JOISTS AND JOIST GIRDERS

1. GENERAL CONTRACTOR TO COORDINATE MECHANICAL, ELECTRICAL, PLUMBING, AND SPRINKLER LOADS WITH JOIST DESIGNER.
2. ALL LOADS UNLESS OTHERWISE NOTED ARE ALLOWABLE LOADS (ASD).
3. OPEN WEB STEEL JOISTS & JOIST GIRDERS WITH THEIR BRIDGING, BRACING, END SUPPORTS AND ANCHORAGE, AND ERECTION STABILITY AND HANDLING REQUIREMENTS SHALL CONFORM TO THE APPLICABLE STEEL JOIST INSTITUTE STANDARD SPECIFICATIONS FOR STEEL JOISTS AND JOIST GIRDERS, LATEST EDITION. TOP CHORDS OF JOISTS AND JOIST GIRDERS SHALL CONSIST OF ANGLES OR TEES.
4. SUBMIT ERECTION DRAWINGS AND CALCULATIONS (BEARING THE SEAL OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT) FOR THE DESIGN OF THE STEEL JOISTS AND JOIST GIRDERS, PER SECTION 2207 OF THE IBC. PROVIDE A CERTIFICATE OF COMPLIANCE FROM THE MANUFACTURER PER SECTION 2207 OF THE IBC. APPROVED ERECTION DRAWINGS AND CALCULATIONS ARE TO BE SUBMITTED TO JURISDICTION FOR REVIEW AND PERMITTING. CONTRACTOR TO COORDINATE ALL MECHANICAL, ELECTRICAL, PLUMBING, AND SPRINKLER LOADS WITH THE JOIST DESIGNER.
5. JOISTS THAT SUPPORT CONCENTRATED LOADS SHALL HAVE THEIR CHORDS DESIGNED TO WITHSTAND ALL BENDING STRESSES OR LOADS SHALL OCCUR WITHIN 3 INCHES OF JOIST PANEL POINTS. JOISTS WITH LOADS OUTSIDE OF PANEL POINTS SHALL BE REINFORCED PER THE "TYPICAL JOIST REINFORCING DETAIL", 1/S5.51. CONCENTRATED LOADS SHALL BE CENTERED ON JOISTS AND NOT ATTACHED TO THE EDGE OF CHORD ANGLES
6. JOISTS AND JOIST GIRDERS SHALL RESIST THE UPLIFT PRESSURE AS INDICATED IN THE DESIGN CRITERIA SECTION. AN ALLOWABLE STRESS INCREASE IS NOT PERMITTED.
7. FOR ALL MEMBERS THAT REQUIRE SPECIFIC ORIENTATION, PROVIDE TAG AT ONE END AND DEFINE LOCATION OF ERECTION DRAWINGS.
8. JOISTS AND JOIST GIRDERS SHALL BE ATTACHED TO SUPPORTING MEMBER PER SJI SPECIFICATIONS. BOLT JOIST TO SUPPORTING MEMBER IN CONFORMANCE WITH THE OCCUPATIONAL SAFETY AND HAZARD ADMINISTRATION (OSHA) AND SJI REQUIMENTS. BOLTS SHALL REMAIN IN PLACE AFTER INSTALLATION.
9. JOIST BRIDGING AND ERECTION STABILITY SHALL BE PROVIDED IN ACCORDANCE WITH OSHA AND SJI.
10. JOIST MANUFACTURER SHALL DESIGN THE COMPRESSION CHORD OF ALL JOISTS SUPPORTING ROOF TOP UNITS FOR AN UNBRACED LENGTH APPLICABLE TO THE CONDITIONS.

SPECIAL INSPECTIONS

1. THE OWNER WILL EMPLOY THE SERVICES OF ONE OR MORE SPECIAL INSPECTORS TO PROVIDE SPECIAL INSPECTIONS DURING CONSTRUCTION FOR THE REQUIRED SPECIAL INSPECTION ITEMS.
2. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN OF THE STRUCTURE, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.
3. DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:
   A. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATIONS. THE INSPECTOR MAY NOT ALTER, MODIFY, ENLARGE OR WAVE ANY OF THE REQUIREMENTS OF THE DOCUMENTS.
   B. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, THE PROFESSIONAL-OF-RECORD, AND THE CONTRACTOR. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF UNCORRECTED, SUBMIT A COMPLETE LIST OF ALL OUTSTANDING DISCREPANCIES ON A WEEKLY BASIS TO THE OWNER, THE BUILDING OFFICIAL, AND THE PROFESSIONAL-OF-RECORD, UNTIL ALL CORRECTIONS HAVE BEEN COMPLETED.
   C. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE INSPECTOR'S KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE BUILDING CODE.
4. SPECIAL INSPECTIONS SHALL BE REQUIRED FOR THE FOLLOWING GENERAL AREAS. REFERENCE THE FOLLOWING TABLE FOR MORE DETAILED INSPECTION REQUIREMENTS IN EACH AREA.
   A. INSPECTION OF FABRICATORS: PER IBC SECTION 1704.2.
   B. STEEL CONSTRUCTION: PER IBC SECTION 1704.3 AND IBC TABLE 1704.0.
   C. CONCRETE: PER IBC SECTION 1704.4 AND IBC TABLE 1704.4.PORT.
   D. MASONRY CONSTRUCTION: PER IBC SECTION 1704.5. AND IBC TABLE 1704.5.1.
   E. SOILS: PER IBC SECTION 1704.7 AND THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT.

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MELBOURNE, FL

Revisions / Submissions

ID	Description	Date

PRELIMINARY FOR COORDINATION AND STRUCTURAL STEEL BIDDING ONLY

Project Number:	761818-04
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Drawn By:	JRR
Checked By:	MG/CJJ
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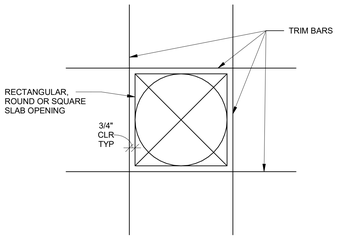
Issue:	BID

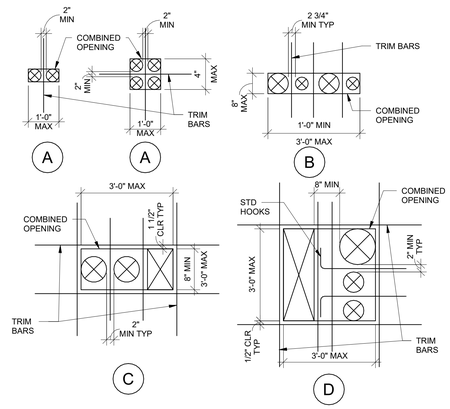
Sheet Title:
STRUCTURAL GENERAL NOTES

S0.00



NOTES:
1. THIS DIAGRAM IS FOR ISOLATED SLAB OPENINGS WHOSE LARGER DIMENSION IS BETWEEN 1'-0" AND 3'-0". NO TRIM BARS ARE REQUIRED FOR ISOLATED SLAB OPENINGS SMALLER THAN 1'-0". RE: PLANS FOR OPENINGS LARGER THAN 3'-0"
2. PROVIDE (1) #5 TRIM BAR TOP AND BOTTOM ON EACH SIDE OF OPENING. BARS SHALL EXTEND 1'-6" PAST EDGES OF OPENING.
3. DISPLACE SLAB PRINCIPAL REINFORCEMENT TO EACH SIDE OF OPENINGS. DO NOT CUT SLAB PRINCIPAL REINFORCEMENT.

1 S0.10 TRIM BARS FOR ISOLATED SLAB OPENINGS
1 1/2" = 1'-0"



NOTES:
1. SLAB OPENINGS WHICH ARE CLOSER TO ONE ANOTHER THAN THE DIAMETER OR SHORTER SIDE OF THE LARGER OF THE TWO ARE CONSIDERED TO FORM A COMBINED OPENING.
2. THESE DIAGRAMS ARE FOR COMBINED OPENINGS WHOSE LARGER DIMENSIONS DOES NOT EXCEED 3'-0". RE: PLANS FOR OPENINGS LARGER THAN 3'-0"
3. TRIM BAR EXTENSION PAST EDGES OF COMBINED OPENINGS SHALL BE 1'-0" FOR #4 BARS AND 1'-6" FOR #5 BARS.
4. DISPLACED SLAB PRINCIPAL REINFORCEMENT TO EACH SIDE OF COMBINED OPENINGS OR PLACE BETWEEN INDIVIDUAL OPENINGS. DO NOT CUT SLAB PRINCIPAL REINFORCEMENT.
5. TENDONS MAY PASS BETWEEN SLAB OPENINGS WHEN THE DISTANCE BETWEEN OPENINGS IS GREATER THAN OR EQUAL TO THE SLAB THICKNESS PLUS (N-1) TIMES 3 INCHES, WHERE 'N' IS THE NUMBER OF TENDONS IN A BUNDLE OR DUCT. TENDONS MUST FOLLOW A STRAIGHT ALIGNMENT (IN PLAN) THROUGH THE COMBINED OPENING AND FOR AT LEAST 24" ON EACH SIDE.
6. SUBMIT SPECIAL SITUATIONS TO ENGINEER FOR REVIEW.
7. RE: 1 / S0.10 FOR TRIM BARS AT ISOLATED OPENINGS.

TRIM BAR REQUIREMENTS:

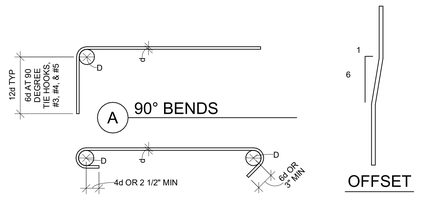
A. IF THE COMBINED OPENING IS SMALLER THAN 1'-0", PROVIDE (1)#4 TOP AND BOTTOM BETWEEN OPENINGS.

B. IF THE LARGER DIMENSION OF A COMBINED OPENING EXCEEDS 1'-0" BUT THE SMALLER DIMENSION IS LESS THAN OR EQUAL TO 8", AND PROVIDED THE COMBINED OPENING IS ALIGNED WITH THE PRINCIPAL REINFORCEMENT, PROVIDE (1)#4 TOP AND BOTTOM BETWEEN OPENINGS.

C. IN ALL OTHER CASES WHERE OPENINGS ARE ARRANGED IN A SINGLE LINE PROVIDE (1)#4 TOP AND BOTTOM BETWEEN OPENINGS AND (1)#5 TOP AND BOTTOM AROUND PERIMETER OF COMBINED OPENING.

D. WHERE INDIVIDUAL OPENINGS OF A COMBINED OPENING FORM TWO OR MORE ROWS, THE ROWS SHALL BE SEPARATED BY AT LEAST 8" OF CONCRETE. PROVIDE (2)#4 TOP AND BOTTOM BETWEEN ROWS OF OPENINGS, (1)#4 TOP AND BOTTOM BETWEEN OPENINGS IN THE PERPENDICULAR DIRECTION, AND (1)#5 TOP AND BOTTOM AROUND PERIMETER OF COMBINED OPENING. PROVIDE STANDARD HOOKS WHERE BARS TERMINATE WITHIN THE COMBINED OPENING.

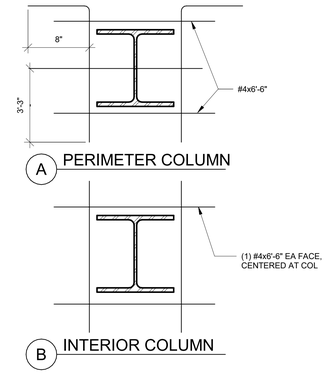
2 S0.10 TRIM BARS FOR MULTIPLE SLAB OPENINGS
1 1/2" = 1'-0"



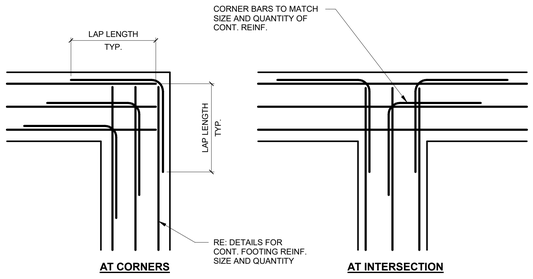
REBAR SIZE	D
#3 THRU #8	6d
#9 THRU #11	8d
#14 & #18	10d
#3 THRU #5 TIES	4d

NOTE:
1. TO REDUCE CONGESTION, IT IS GENERALLY ACCEPTABLE TO REPLACE A BEAM/COLUMN LONGITUDINAL BAR HOOK (90° OR 180°) WITH A TERMINATOR, PRIOR EOR APPROVAL REQ'D

3 S0.10 TYPICAL REINFORCING BAR BENDS
1 1/2" = 1'-0"



4 S0.10 ADDITIONAL REINFORCING
1 1/2" = 1'-0"



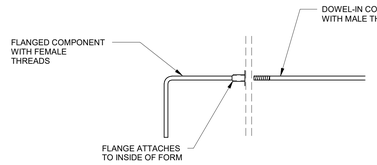
5 S0.10 TYPICAL CORNER BAR DETAIL
3/4" = 1'-0"

CONCRETE REINFORCING LAP LENGTH SCHEDULE

BAR SIZE	3000psi		4000psi		4500psi		5000psi		7000psi	
	STRUCTURAL ELEMENT MINIMUM COMPRESSIVE STRENGTH (f'c)									
	TOP BARS	OTHER	TOP BARS	OTHER	TOP BARS	OTHER	TOP BARS	OTHER	TOP BARS	OTHER
#3	28"	22"	25"	19"	23"	18"	22"	17"	19"	14"
#4	38"	29"	33"	25"	31"	24"	29"	23"	25"	19"
#5	47"	36"	41"	31"	38"	30"	36"	28"	31"	24"
#6	56"	43"	49"	37"	46"	35"	44"	34"	37"	28"
#7	81"	63"	71"	54"	67"	51"	63"	49"	54"	41"
#8	93"	72"	81"	62"	76"	59"	72"	56"	61"	47"
#9	105"	81"	91"	70"	86"	66"	81"	63"	69"	53"
#10	118"	91"	102"	79"	96"	74"	92"	71"	77"	60"

NOTES:
1. LAP LENGTH FOR TOP BARS SHALL BE USED WHEN MORE THAN 12 INCHES OF FRESH CONCRETE IS PLACED BELOW HORIZONTAL REINFORCEMENT.

6 S0.10 CONCRETE REINFORCING LAP SCHEDULE
3/4" = 1'-0"

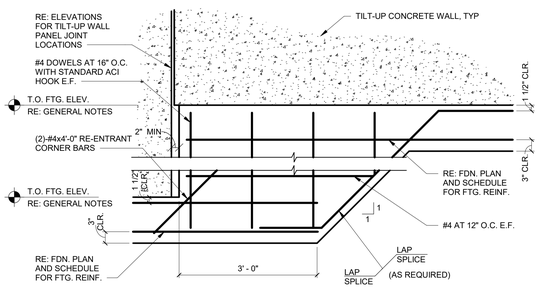


NOTES:
1. AT THE CONTRACTORS OPTION THE TWO-PART DOWEL PRODUCTS LISTED AT RIGHT MAY BE USED TO REPLACE ORDINARY DOWELS SPECIFIED IN THE DRAWINGS
2. FOLLOW MANUFACTURERS DIRECTIONS AND APPLICABLE ICBO REPORTS. SPECIAL INSPECTION IS REQUIRED
3. TWO-PART DOWEL COMPONENTS MUST BE OF SPECIFIED LENGTH AFTER ASSEMBLY

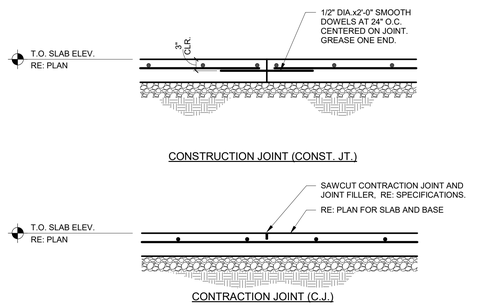
REQUIRED SIZE OF FABRICATED TWO-PART DOWEL COMPONENTS

MANUFACTURER & BRAND NAME	SPECIFIED SIZE FOR REBAR					
	#4		#5		#6	
	DOWEL-IN COMPONENT	T	DOWEL-IN COMPONENT	T	DOWEL-IN COMPONENT	T
RICHMOND DOWEL BAR SUBSTITUTION	#4	#4	#5	#5	#6	#6
LENTON FORM SAVER	#4	#4	#5	#5	#6	#6
DAYTON-SUPERIOR D-50 DOWEL BAR REPLACEMENT	#4	#4	#5	#5	#6	#6

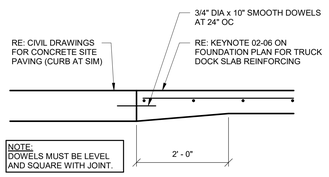
7 S0.10 TWO PART DOWELS SYSTEM
1 1/2" = 1'-0"



8 S0.10 EXTERIOR WALL FOOTING STEP
3/4" = 1'-0"



9 S0.10 TYPICAL SLAB JOINT DETAILS
3/4" = 1'-0"



NOTE:
DOWELS MUST BE LEVEL AND SQUARE WITH JOINT.

10 S0.10 CONSTRUCTION JOINT AT TRUCK DOCK RAMP
3/4" = 1'-0"

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MELBOURNE, FL

Revisions / Submissions

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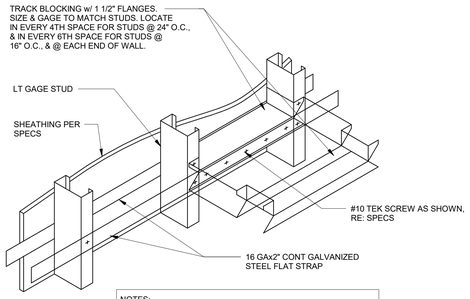
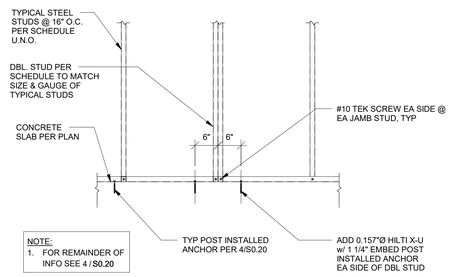
Sheet Title:
TYPICAL DETAILS

S0.10



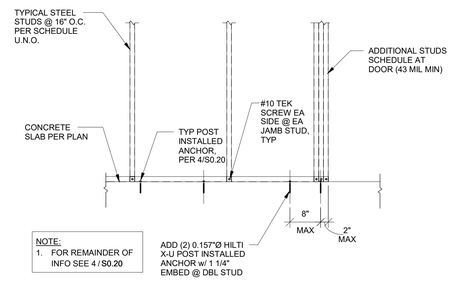
NOTES:
1. INSTALL BRACING ROWS @ 4'-0" O.C. VERT SP, U.O.N..
2. U.O.N., INSTALL STRAP ON SIDE w/ NO SHTG AT LOCATIONS WHERE SHTG IS ON ONE SIDE ONLY. INSTALL STRAP ON BOTH SIDES WHERE NO SHTG IS PRESENT.

1 S0.20 STUD WALL LATERAL BRACING LOCATIONS
1 1/2" = 1'-0"



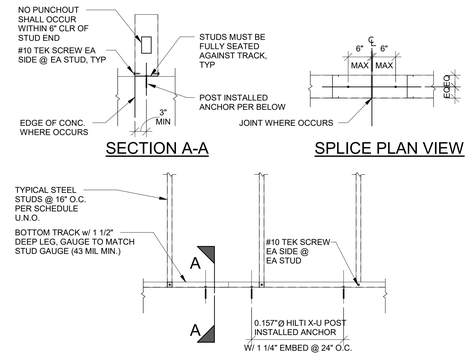
NOTE:
1. FOR REMAINDER OF INFO SEE 4 / S0.20

2 S0.20 TYP BOTTOM TRACK ANCHORAGE @ WINDOW
3/4" = 1'-0"



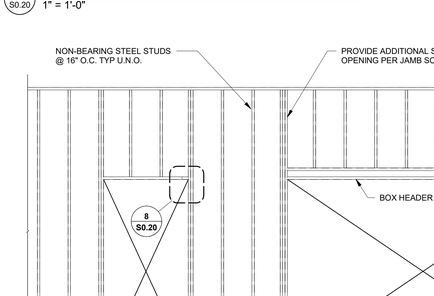
NOTE:
1. FOR REMAINDER OF INFO SEE 4 / S0.20

3 S0.20 TYP @ DOOR BOTTOM TRACK ANCHORAGE
3/4" = 1'-0"



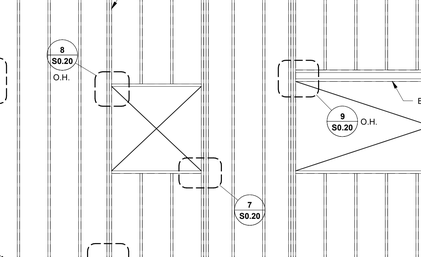
4 S0.20 TYPICAL BOTTOM TRACK SPLICE & ANCHORAGE
3/4" = 1'-0"

5 S0.20 TYPICAL TOP TRACK SPLICE
3" = 1'-0"

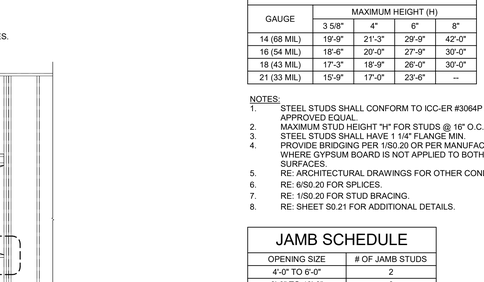


NOTES:
1. SPLICE TO OCCUR AS REQ'D.
2. LOCATE SPLICE AT THIRD POINTS ONLY

6 S0.20 STUD SPLICE DETAIL
3/4" = 1'-0"



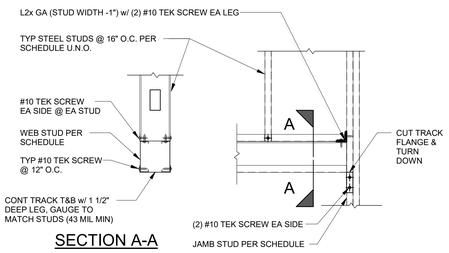
7 S0.20 TYPICAL SILL CONNECTION
1 1/2" = 1'-0"



NOTE:
APPLIES TO INTERIOR NON-BEARING WALL OPENINGS UP TO 4'-0" WIDE, MAX.

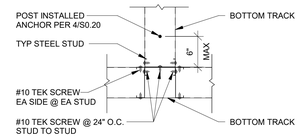
8 S0.20 TYPICAL TRACK HEADER CONNECTION
1 1/2" = 1'-0"

HEADER SCHEDULE	
OPENING SIZE (L)	WEB STUD
L < 6'-0"	(2) 362 S162-43
6'-0" < L ≤ 10'-0"	(2) 800 S162-43
10'-0" < L ≤ 12'-0"	(2) 1000 S162-54

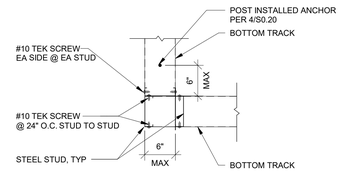


SECTION A-A

9 S0.20 TYPICAL BOX HEADER
1" = 1'-0"



PLAN VIEW, INTERSECTION OF STUDS



PLAN VIEW, CORNER CONDITION

10 S0.20 TYP. STUD FRAMING @ INTERSECTIONS
1" = 1'-0"

WALKABLE CEILING JOIST SIZE					
	MAXIMUM LENGTH (L)				
GAUGE	6"	8"	10"	12"	14"
20 (33 MIL)	8'-0"	9'-0"	--	--	
18 (43 MIL)	10'-0"	11'-6"	13'-0"	--	
16 (54 MIL)	12'-4"	15'-6"	18'-4"	19'-0"	20'-0"
14 (68 MIL)	13'-3"	16'-8"	20'-0"	23'-1"	24'-11"

NOTES:
1. STEEL STUDS SHALL CONFORM TO ICC-ER #3064P OR APPROVED EQUAL.
2. MAXIMUM STUD LENGTH "L" SINGLE SPAN FOR STUDS @ 16" O.C.
3. STEEL STUDS SHALL HAVE 1 5/8" FLANGE, MIN.
4. PROVIDE BRIDGING & BRACING AS REQUIRED.
5. JOIST SIZES IN TABLE ONLY APPLICABLE FOR INTERIOR CONDITIONS WITH TWO LAYERS OF 5/8" GYPSUM BOARD ON THE BOTTOM OF THE JOIST WITH ONE LAYER OF 3/4" PLYWOOD ON TOP OF THE JOIST. DESIGN LL = 20psf
6. RE: ARCHITECTURAL DRAWINGS FOR OTHER CONDITIONS.

CEILING JOIST SIZE						
	MAXIMUM LENGTH (L)					
	3 5/8"	3 5/8"	4"	4"	6"	6"
GAUGE	NO BRACING	MIDSPAN BRACING	NO BRACING	MIDSPAN BRACING	NO BRACING	MIDSPAN BRACING
20 (33 MIL)	9'-5"	13'-2"	9'-8"	13'-8"	11'-0"	15'-10"
18 (43 MIL)	10'-5"	14'-4"	10'-8"	15'-0"	11'-11"	17'-0"

NOTES:
1. STEEL STUDS SHALL CONFORM TO ICC-ER #3064P OR APPROVED EQUAL.
2. MAXIMUM STUD LENGTH "L" FOR STUDS @ 16" O.C.
3. STEEL STUDS SHALL HAVE 1 1/4" FLANGE, MIN.
4. PROVIDE LATERAL BRACING AS REQUIRED.
5. JOIST SIZES IN TABLE ONLY APPLICABLE FOR INTERIOR CONDITIONS WITH ONE LAYER OF 5/8" GYPSUM BOARD.
6. RE: ARCHITECTURAL DRAWINGS FOR OTHER CONDITIONS.

11 S0.20 TYP NON-BEARING STUD WALL FRAMING
3/8" = 1'-0"

NOTES:
1. RE: 4/S02.0 AND 5/S0.20 FOR TYPICAL TRACK SPLICES.
2. RE: 10/S0.20 FOR FRAMING AT INTERSECTIONS.

STEEL STUD SIZE				
	MAXIMUM HEIGHT (H)			
GAUGE	3 5/8"	4"	6"	8"
14 (68 MIL)	19'-9"	21'-3"	29'-9"	42'-0"
16 (54 MIL)	18'-6"	20'-0"	27'-9"	30'-0"
18 (43 MIL)	17'-3"	18'-9"	26'-0"	30'-0"
21 (33 MIL)	15'-9"	17'-0"	23'-6"	--

NOTES:
1. STEEL STUDS SHALL CONFORM TO ICC-ER #3064P OR APPROVED EQUAL.
2. MAXIMUM STUD HEIGHT "H" FOR STUDS @ 16" O.C.
3. STEEL STUDS SHALL HAVE 1 1/4" FLANGE MIN.
4. PROVIDE BRIDGING PER 1/S0.20 OR PER MANUFACTURER WHERE GYPSUM BOARD IS NOT APPLIED TO BOTH SURFACES.
5. RE: ARCHITECTURAL DRAWINGS FOR OTHER CONDITIONS.
6. RE: 6/S0.20 FOR SPLICES.
7. RE: 1/S0.20 FOR STUD BRACING.
8. RE: SHEET S0.21 FOR ADDITIONAL DETAILS.

JAMB SCHEDULE	
OPENING SIZE	# OF JAMB STUDS
4'-0" TO 6'-0"	2
6'-0" TO 10'-0"	2
10'-0" TO 12'-0"	3

NOTES:
1. JAMB STUDS TO MATCH SIZE & GAGE OF TYP STUDS
2. RE: 3/S0.20 FOR ADDITIONAL JAMB DETAILS

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Revisions / Submissions

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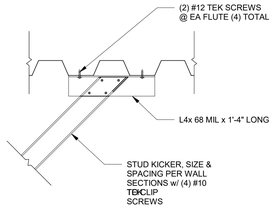
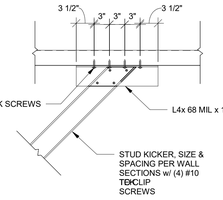
Sheet Title:
TYPICAL INTERIOR LIGHT GAGE STEEL DETAILS

S0.20

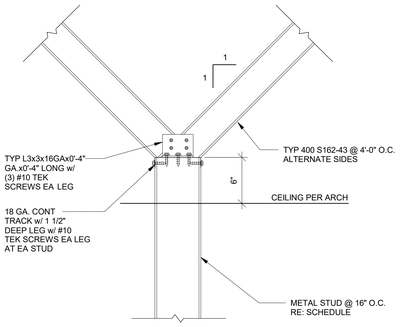


PERPENDICULAR TO FLUTES

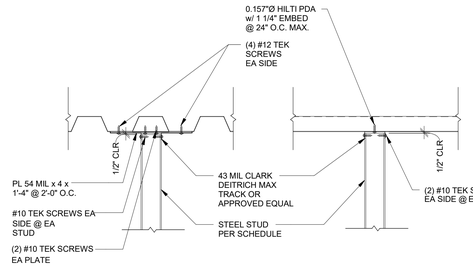


PARALLEL TO FLUTES

1 S0.21 TYPICAL TRACK KICKER TO DECK
1" = 1'-0"



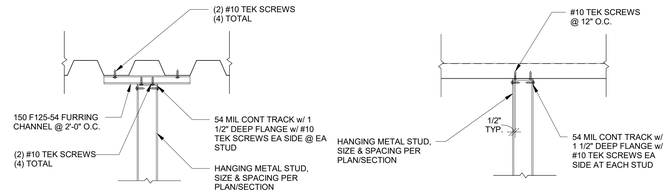
2 S0.21 TYPICAL TRACK PARTITION WALL TOP
1 1/2" = 1'-0"



PARALLEL TO FLUTES

PERPENDICULAR TO FLUTES

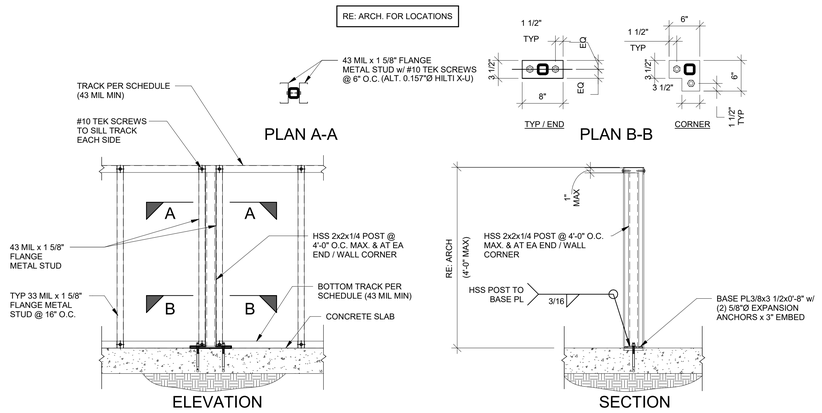
3 S0.21 TYPICAL NON-BEARING WALL TOP
1" = 1'-0"



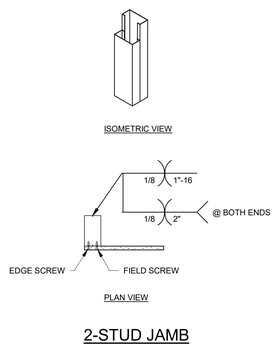
PARALLEL TO FLUTES

PERPENDICULAR TO FLUTES

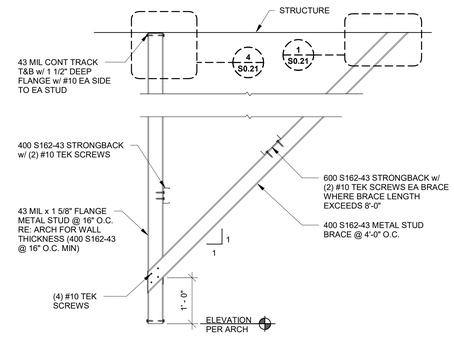
4 S0.21 HANGING METAL STUD RIGID CONNECTION
1" = 1'-0"



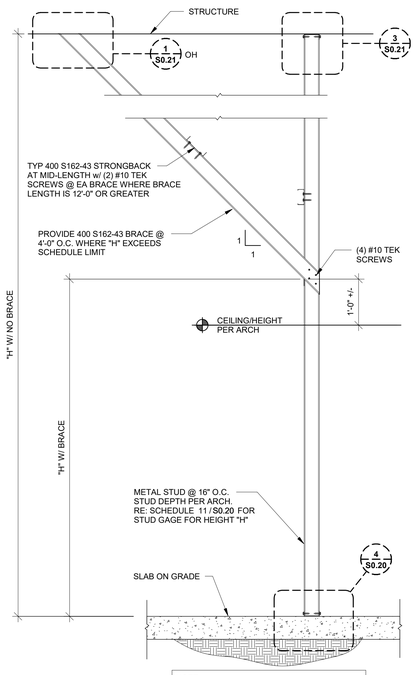
5 S0.21 TYPICAL INTERIOR LOW WALL
1" = 1'-0"



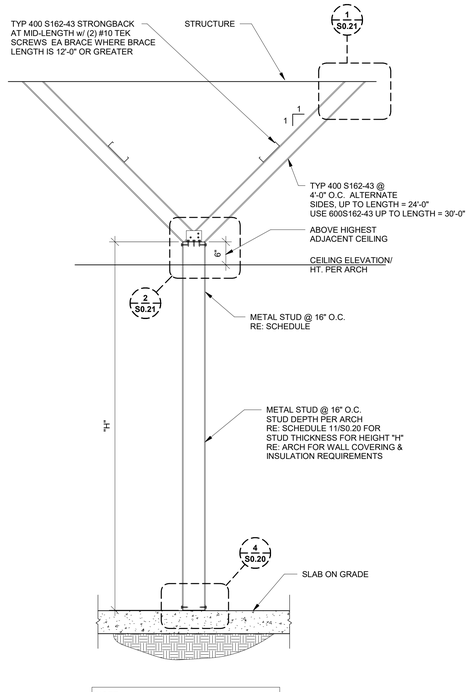
6 S0.21 TYPICAL TRACK HEADER CONNECTION
1" = 1'-0"



7 S0.21 TYPICAL HANGING WALL
3/4" = 1'-0"



8 S0.21 TYPICAL FULL-HEIGHT PARTITION WALL
3/4" = 1'-0"



9 S0.21 TYPICAL PARTIAL HEIGHT PARTITION WALL
3/4" = 1'-0"

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Sheet Title:
TYPICAL INTERIOR LIGHT GAGE STEEL DETAILS

S0.21

GENERAL NOTES

A. FOR GENERAL STRUCTURAL NOTES RE: S0.00
B. FOR TYPICAL STRUCTURAL DETAILS RE: S0.10
C. FOR SLAB-ON-GRADE AND FOUNDATION SUB-BASE, VAPOR-RETARDING MEMBRANE, GEOTEXTILE AND DRAINAGE REFER TO GEOTECHNICAL REPORT
D. LOCATE CL OF FOOTINGS AT CL OF COLUMNS AND/OR WALLS, U.N.O.
E. RE: GENERAL STRUCTURAL NOTES FOR CONTROL/CONSTRUCTION JOINT REQUIREMENTS FOR SLAB ON GRADE
F. RE: TYPICAL DETAILS FOR REINFORCEMENT AT SLAB PENETRATION AND BLOCKOUTS.
G. RE: TYPICAL DETAILS FOR TYPICAL REINFORCEMENT AT WALL AND FOOTING CORNERS AND INTERSECTIONS.
H. RE: TYPICAL DETAILS FOR REINFORCEMENT LAP SPLICE LENGTH.
I. CHAIR SLAB REINFORCING AS REQ'D. LIFTING OF BARS WHILE PLACING OF CONC NOT ALLOWED.
J. FOOTING ELEVATION CRITERIA PER STRUCTURAL DETAILS. RE: 8/S0.10 COORDINATE w/ GRADING, UTILITIES & MEP. STEP FOOTING AS REQUIRED PER TYPICAL DETAILS. COORDINATE LOCATION OF FOOTING STEPS WITH PRECAST MANUFACTURER.
K. RE: 7/S4.00 FOR PIPE PENETRATIONS AT FOUNDATION WALLS.

LEGEND

FA — FOOTING PER SCHEDULE

HSS — COLUMN PER PLAN

KEYNOTES

02-01 7" CONCRETE SLAB-ON-GRADE W/#3 @ 18" O.C. EA WAY. FOR SUB-BASE PREPARATION RE: GEOTECH REPORT.

02-02 CONTROL/CONSTRUCTION JOINTS PER 9/S0.10.

02-03 DOCK LEVELER. RE: DETAIL 6/S4.00.

02-04 COLUMN BLOCK OUT PER DETAIL 1/S3.00.

02-05 CANOPY FOOTING PER S2.00.

02-06 8" AIR ENTRAINED CONCRETE SLAB-ON-GRADE W/#6 @ 16" O.C. EA WAY. FOR SUB-BASE PREPARATION RE: GEOTECH REPORT.

02-07 PREFAB METAL STAIR. DESIGN BY STAIR FABRICATOR.

02-08 TRASH ENCLOSURE, RE: ARCH./CIVIL FOR EXACT LOCATION AND EXTENTS.

02-09 POUR STRIP WITH REINF PER KEYNOTE S2-01, CONTINUE REINF THROUGH POUR STRIP.

02-10 INSTALL STRUCTURAL FILL PRIOR TO PURING CONCRETE FOR RAMP SLAB (SEE CIVIL FOR RAMP SLAB DETAIL.)

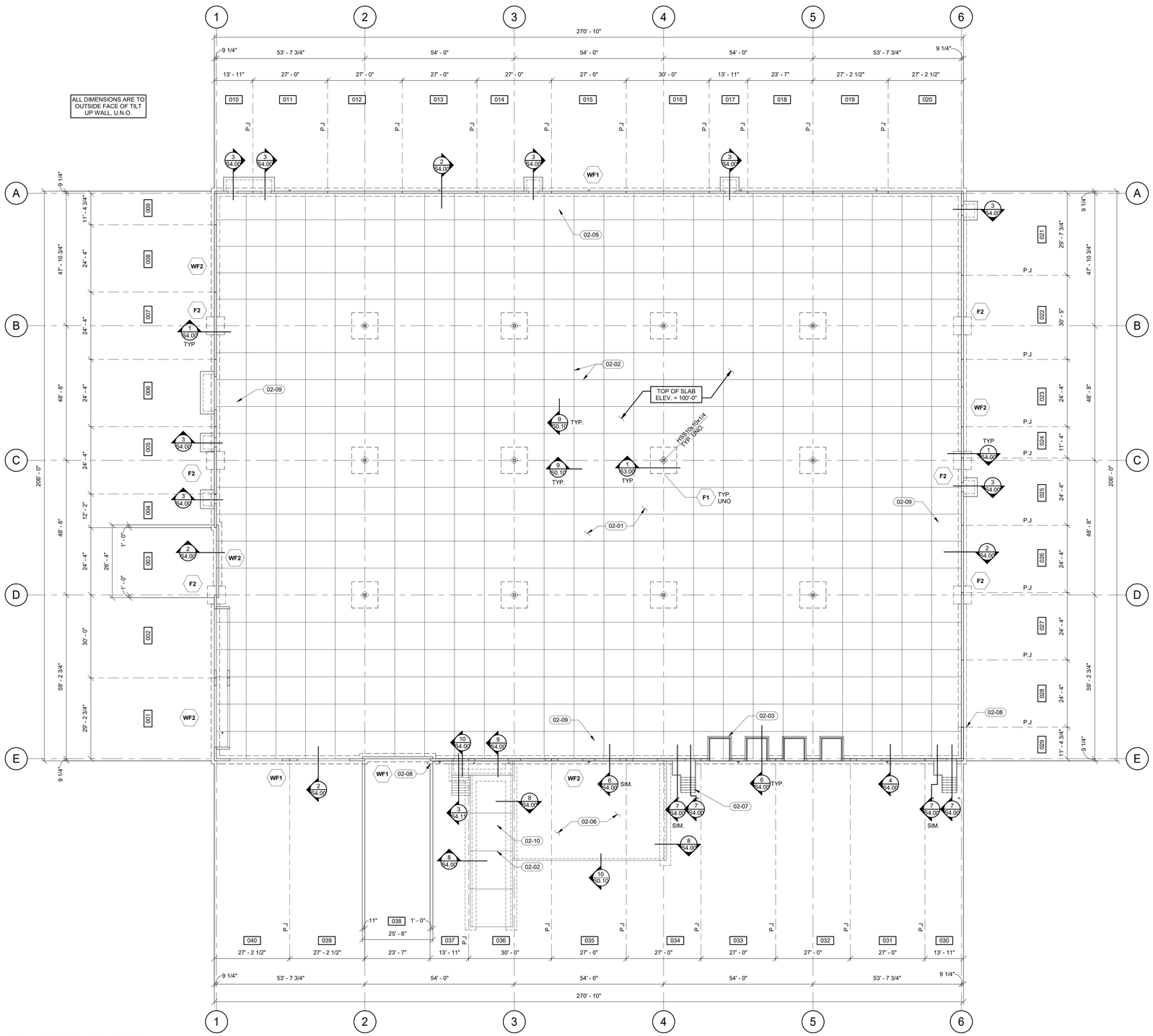
FOOTING SCHEDULE

MARK	SIZE			BOTTOM REINFORCING		TOP REINFORCING		NOTES	T.O.F. ELEV.
	LENGTH	WIDTH	THICKNESS	LONGITUDINAL	TRANSVERSE	LONGITUDINAL	TRANSVERSE		
FOUNDATION DESIGN PENDING									

WALL FOOTING SCHEDULE

MARK	SIZE		BOTTOM REINFORCING		TOP REINFORCING		NOTES	T.O.F. ELEV.
	WIDTH	THICKNESS	CONTINUOUS	TRANSVERSE	CONTINUOUS	TRANSVERSE		
FOUNDATION DESIGN PENDING								

NOTES:
1. TOP OF FOOTING ELEVATION (T.O.F. ELEV.) IS REFERENCED TO FINISHED FLOOR.
2. STEP FOOTING PER 8/S0.10 AS REQUIRED.



1 S1.00 FOUNDATION PLAN
1/16" = 1'-0"

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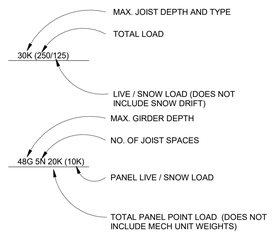
Sheet Title:
FOUNDATION PLAN

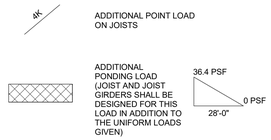
S1.00

GENERAL NOTES

A. FOR GENERAL STRUCTURAL NOTES RE: S0.00
B. FOR TYPICAL STRUCTURAL DETAILS RE: S0.10
C. RE: ARCHITECTURAL DRAWINGS FOR ELEVATIONS.
D. COORDINATE PERIMETER CONDITIONS WITH ARCHITECTURAL
E. RE: DETAILS FOR ADDITIONAL AXIAL LOADS REQ'D FOR GIRDERS, JOISTS, AND JOIST SEATS
F. CONTRACTOR TO COORDINATE AND CONFIRM MECH UNIT SIZE, LOCATION, & WEIGHTS. COORDINATE AND SUPPLY ALL REQUIRED LOADING TO THE JOIST SUPPLIER
G. CONTRACTOR TO COORDINATE w/ JOIST SUPPLIER ALL PIPING 4" DIAMETER AND LARGER
H. RE: SHEET TYPICAL DETAILS FOR ADDITIONAL FRAMING REQUIRED FOR MECH EQUIPMENT AND OPENINGS. RE: ARCH FOR LOCATIONS OF SKYLIGHTS, SMOKE VENTS, ETC.
I. RE: 1/S5.10 FOR ADDITIONAL SUPPORT AT LOADS OVER 500 LBS.

LEGEND





KEYNOTES

03-01 CANOPY FRAMING PER S2.00.

03-02 HVLS FAN

03-03 MECHANICAL UNIT PER MEP. COORDINATE LOCATION AND FRAMING WITH MEP, DEVELOPER AND TENANT. RE: 3/S5.10 FOR ATTACHMENT.

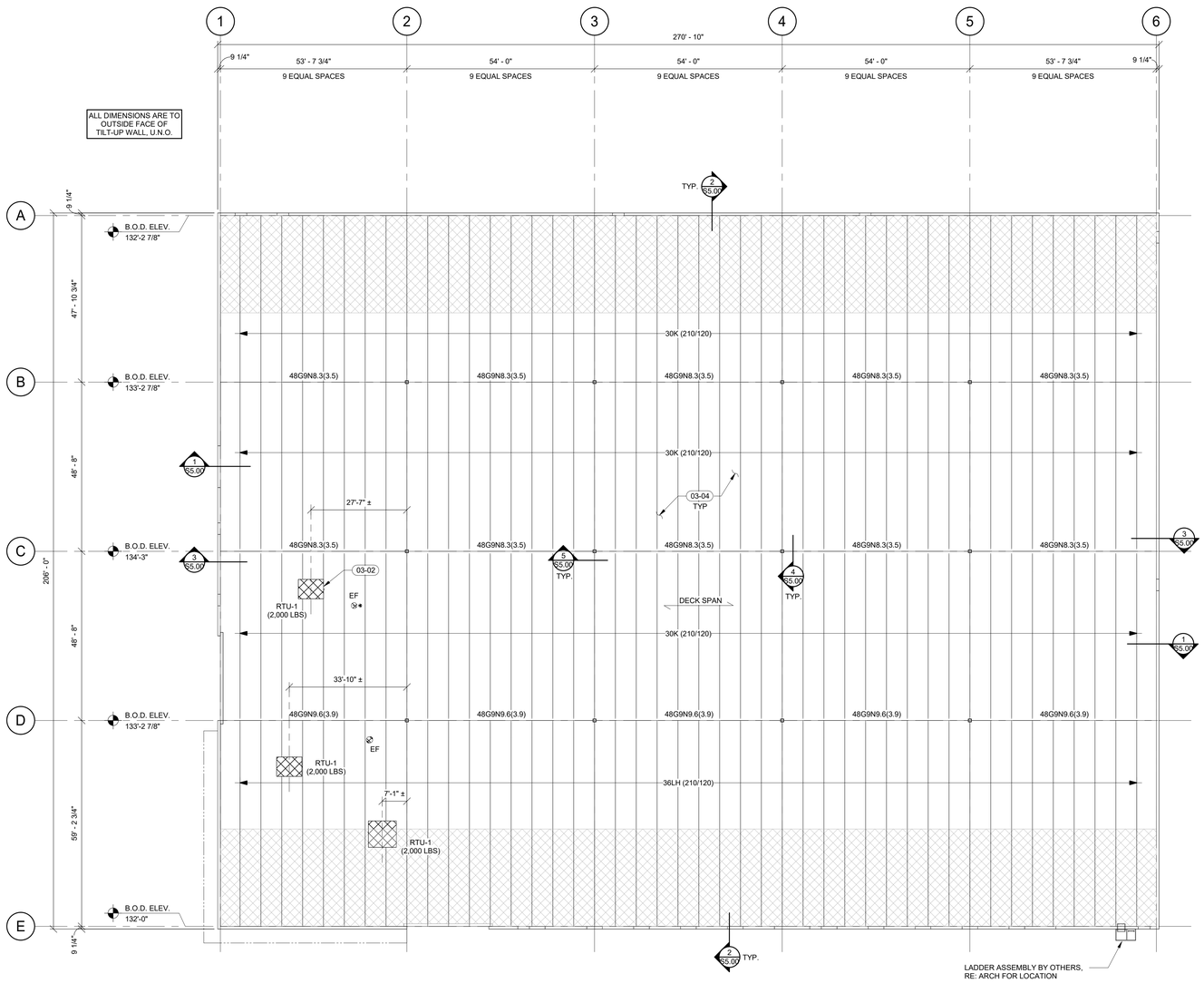
03-04 1 1/2" TYPE B WIDE RIB DECK. RE: 3/S3.00 FOR DECK GAGE AND ATTACHMENT TO STEEL SUPPORTS.

03-05 WALL MOUNTED DAVIT CRANE, REF ARCH AND SPECS. REF 6/S5.00 FOR WALL LOADING.

JOISTS AND JOIST GIRDERS

1. JOISTS AND JOIST GIRDERS SHALL BE DESIGNED FOR THE UNIFORM LOAD CAPACITY (AS SPECIFIED IN THE SJI STANDARD LOAD TABLES) IN ADDITION TO CONCENTRATED LOADS SHOWN.
2. UNIFORM GRAVITY LOADS ARE SHOWN FOR EACH JOIST (RE: SHEET S 0.00).
   - DEAD LOAD (DL)
   - LIVE LOAD OR SNOW LOAD (LL/SL)
   - PONDING LOAD (PL)

   IN ADDITION TO THE GRAVITY LOADS, DESIGN JOISTS TO ACCOMODATE SNOW DRIFT, RTU LOADS, FAN LOADS, OTHER HANGING LOADS, DRAINLINES, SOFFIT LOADS, AND OTHER CONCENTRATED LOADS.
3. ALL LOADS SHOWN ARE WORKING STRESS LOADS (ASD). APPLY LOAD FACTORS PER CODE.
4. JOISTS SHALL BE DESIGNED FOR ANY STRESS REVERSAL DUE TO UNEVENLY DISTRIBUTED.
5. ALL JOIST SHALL HAVE 2 1/2" DEEP JOIST SEATS AND JOIST GIRDERS SHALL HAVE 7 1/2" DEEP SEAT, UNO.
6. ALL JOISTS SHALL BE DESIGNED FOR A 500 LB FLOATING POINT LOAD.



1 S1.10 ROOF FRAMING PLAN
1/16" = 1'-0"

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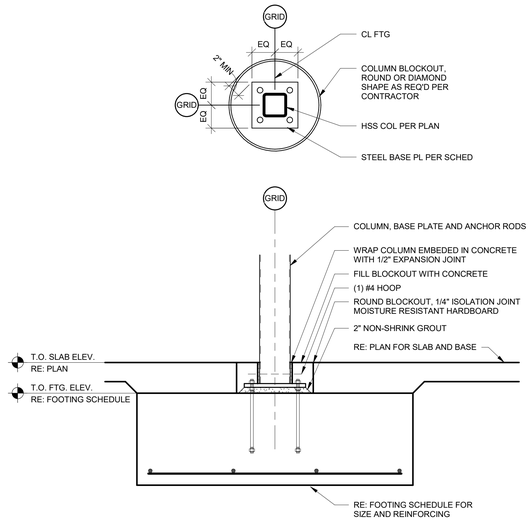
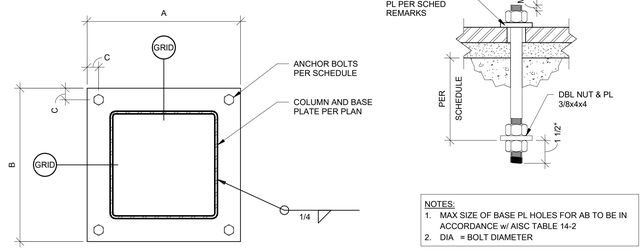
ROOF FRAMING PLAN

S1.10



1 S3.00 HSS COLUMN TO FOOTING
NOT TO SCALE

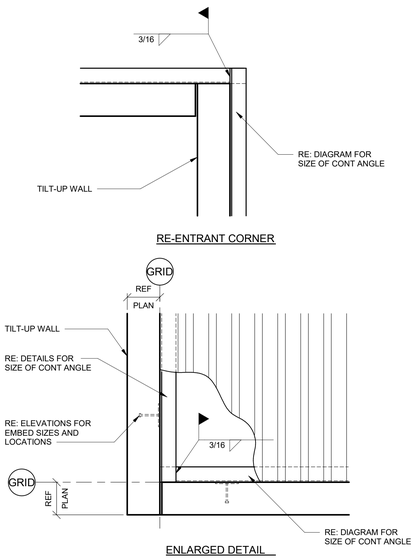


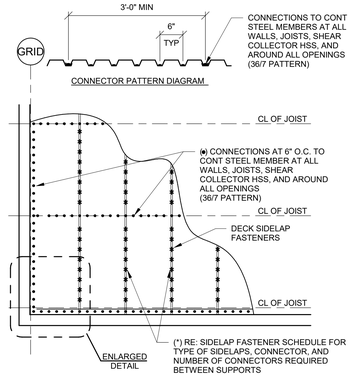
NOTES:
1. MAX SIZE OF BASE PL HOLES FOR AB TO BE IN ACCORDANCE w/ AISC TABLE 14-2
2. DIA = BOLT DIAMETER

STRUCT - BASE PLATE (BP) SCHEDULE

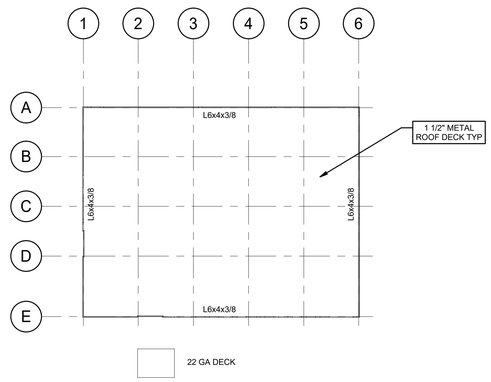
MARK	BASE PLATE				ANCHOR BOLTS			REMARKS
	THICKNESS	"A"	"B"	"C"	NO.	DIA.	EMBED	
HSS10x10x1/4	3/4"	16"	16"	1 1/2"	4	3/4"	1'-0"	BUILDING
HSS8x8x1/4	1"	16"	16"	2"	4	1"	1'-6"	CANOPY

2 S3.00 BASE PLATE AND ANCHOR BOLT SCHEDULE
12" = 1'-0"





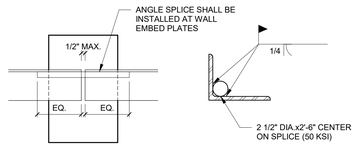
3 S3.00 ROOF DIAPHRAGM CONNECTION DIAGRAM AND SCHEDULE
NTS



ROOF DECK DIAGRAM

ROOF DECK FASTENER SCHEDULE		
(•) DECK TO CONTINUOUS STEEL MEMBER CONNECTOR TYPE (36/7 PATTERN)	(*) SIDE LAP CONNECTOR SPACING	SUITABLE BASE MATERIAL THICKNESS RANGE (IN)
5/8" VISIBLE DIAMETER PUDDLE WELDS	(8) AT (9) EQUAL SPACES	ALL THICKNESSES
POWDER ACTUATED HILTI X-HSN-24	(12) AT (13) EQUAL SPACES	1/8 TO 3/8 INCLUSIVE

NOTES:
1. ALL SIDE LAP SCREWS SHALL BE #10-16 SELF-DRILLING SCREWS AT EQUAL SPACES.
2. DIAPHRAGM CHORD ANGLES DENOTE ARE ALL LLH AND CONTINOUS, RE: SPLICE DETAIL.



TYPICAL CONTINUOUS ANGLE SPLICE

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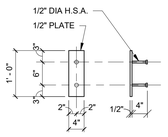
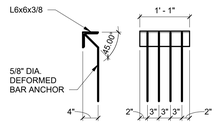
GENERAL SCHEDULES AND DETAILS

S3.00



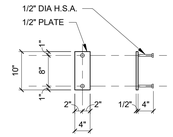
EMBED "A"

PL. 3/8"x4"x1'-0" WITH
(2) 1/2" DIA x4" H.S.A.



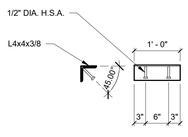
EMBED "B"

L6x6x3/8x1'-1" WITH (4) 5/8" DIA x24" D.B.A.
FOR 10" TALL x7" DP. x1'-1" WIDE JOIST
GIRDER POCKET.



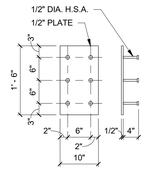
EMBED "C"

PL. 3/8"x4"x0'-10" WITH
(2) 1/2" DIA x4" H.S.A.



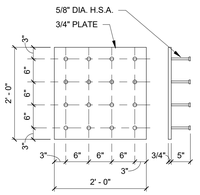
EMBED "D"

L4x4x3/8x1'-0" LONG WITH
(2) 1/2" DIA. x 4" H.S.A.



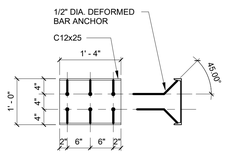
EMBED "E"

PL. 3/8"x10"x1'-6" WITH
(6) 1/2" DIA x4" H.S.A.



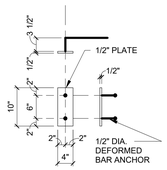
EMBED "F"

PL. 3/4"x2'-0"x2'-0" WITH
(16) 5/8" DIA x6" H.S.A.



EMBED "G"

11 1/4" THICK PANEL
C12x25x1'-4" WITH (6)
1/2" DIA x 18" D.B.A.



EMBED "H"

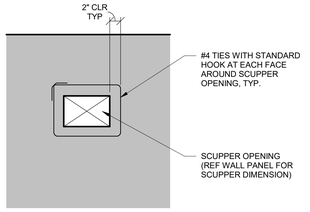
PL. 3/8"x4"x0'-10" WITH
(2) 1/2 DIA x18" D.B.A.

SITE CAST TILT-WALL NOTES:

1. CONCRETE FOR WALL PANELS SHALL BE MINIMUM COMPRESSIVE STRENGTH OF 5,000 PSI AT 28 DAYS. RE: AND THE PROJECT SPECIFICATION FOR ADDITIONAL REQUIREMENTS.
2. ALL PANELS ARE VIEWED FROM INSIDE (UNLESS NOTED OTHERWISE) AND SHALL BE CAST WITH EXTERIOR FACE DOWN.
3. RE: PANEL ELEVATIONS FOR PANEL THICKNESS.
4. RE: ARCHITECTURAL DRAWINGS FOR EXTERIOR FINISH.
5. RE: STRUCTURAL SECTIONS FOR DOWELS TO SLAB AND EMBEDDED POCKET AND PLATES FOR SUPPORT OF JOISTS, BEAMS, AND GIRDERS TO PANELS.
6. ALL PLANS AND SECTIONS SHALL BE REVIEWED FOR REINFORCING AND EMBEDDED ITEMS NOT SHOWN ON THE PANEL ELEVATIONS.
7. RE: ARCHITECTURAL DRAWINGS FOR CONNECTIONS OF WOOD, ETC. TO PANELS.
8. RE: ARCH. AND MECHANICAL DRAWINGS FOR ADDITIONAL OPENINGS IN PANELS.
9. PANELS SHALL NOT BE LIFTED INTIL CONCRETE HAS ATTAINED A MINIMUM COMPRESSICE STRENGTH OF 3,000 PSI. CONTRACTOR SHALL NOT SUPPORT CRANE LOADS TO SLAB ON GRADE UNLESS AUTHORIZED BY THE OWNER I WRITING.
10. THE CONTRACTOR SHALL PROVIDE DESIGN FOR THE LIFTING INSERTS AND ANY ADDITIONAL REINFORCING STEEL REQUIRED FOR LIFTING OPERATION. THE PANELS HAVE BEEN DESIGNED FOR GRAVITY, WIND AND SEISMIC CONDITIONS ONLY.
11. THE CONTRACTOR SHALL EMPLOY THE SERVICE OF A LIFTING ACCESSORY SUPPLIER TO DESIGN AND SUPPLY ALL THE HARDWARE FOR LIFTING AND BRACING THE WALL PANELS.
12. THE LIFTING ACCESSORIES SUPPLIER SHALL COMPLETE SHOP DRAWINGS COVERING ERECTION PROCEDURES, ADDITIONAL REINFORCEMENT AND TEMPORARY BRACING. THE SHOP DRAWINGS SHALL BE PREPARED AND SEALED BY A REGISTERED STRUCTURAL ENGINEER IN THE STATE OF WHERE THE PROJECT IS LOCATED.
13. THE CONTRACTOR SHALL VERIFY ALL WALL PANEL ELEVATIONS FOR DIMENSIONS, OPENINGS, BEAM AND JOIST GIRDER POCKET LOCATIONS AND WELD PLATE LOCATIONS AND REPORT AND DISCREPANCIES TO THE STRUCTURAL ENGINEER BEFORE POURING ANY PANELS.
14. EMBEDDED PLATES SHOWN ON TILT-WALL ELEVATIONS ARE FOR CONTRACTORS CONVENIENCE ONLY. CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL EMBEDDED PLATES, ETC. INDICATED ON THE STRUCTURAL PLANS AND/OR SECTIONS WHETHER SHOWN OR NOT ON PANEL ELEVATIONS.
15. THE TILT-WALL ELEVATINOS BAY BE REPRODUCED FOR SHOP DRAWINGS USE AS LONG AS SEALS, COMPANY TITLE BLOCKS, ETC. ARE REMOVED. IN USING THESE DRAWINGS THE CONTRACTOR/FABRICATOR SHALL TAKE FULL RESPONSIBILITY FOR CORRECTNESS OF ALL DIMENSIONS, EMBEDDED ITEAMS AND OPENINGS SHOWN.
16. DO NOT SCALE TILT-WALL PANEL ELEVATIONS.
17. CONTRACTOR SHALL FURNISH AND SUBMIT SHOP DRAWINGS FOR REVIEW BY THE ENGINEER ON THE FOLLOWING ITEMS:
    - A. REINFORCING STEEL
    - B. EMBEDDED ITEMS FOR PLACEMENT
    - C. PANEL LIFTING INSERTS
    - D. DETAILS FOR TILT-WALL PANELS
    - E. PROPOSED EMBEDDED CONDUIT PLACEMENT
18. SYMBOLS AND ABBREVIATIONS:
    - N.S.------- NEAR SIDE
    - F.S.------- FAR SIDE
    - H.S.A.--- HEADED STUD ANCHOR
    - D.B.A.--- DEFORMED BAR ANCHOR
19. TEMPORARY BRACING OF PANELS SHALL NOT BE REMOVED UNTIL AFTER THE PANEL POUR STRIP HAS BEEN POURED AND ROOF DIAPHRAGM CONSTRUCTION AND ALL CONNECTIONS HAVE BEEN COMPLETED.
20. LIFTING INSERTS VISIBLE AFTER FINAL CONSTRUCTION SHALL BE PATCHED AND FINISHED TO MEET ARCHITECTS APPROVAL.
21. ALL PANEL LEGS THAT ARE REMAINING AROUND THE SIDES OF OPENINGS ARE TO BE REINFORCED AS SHOWN BELOW. VERTICAL X PANEL HEIGHT AT EACH FACE OF PANEL WITH #4 TIES ARE 12" O.C. VERTICAL

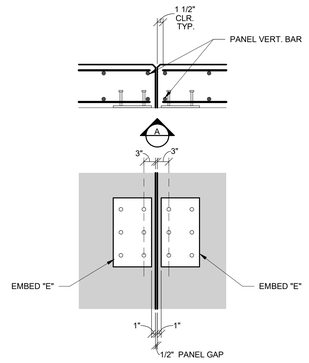
LEG WIDTH	VERT. REINF. (9 1/4")
2'-6" TO 2'-11"	(5) #6 (EA. FACE)
3'-0" TO 4'-0"	(6) #6 (EA. FACE)
4'-1" TO 5'-0"	(10) #6 (EA. FACE)

22. UNLESS OTHERWISE NOTED ON PANEL ELEVATIONS, PROVIDE (3) #6 CONTINUOUS AT BOTTOM OF PANEL, (2) #6 VERTICAL X PANEL HEIGHT. ON EACH END OF PANEL, (2) #6 CONTINUOUS AT TOP OF PANEL, AND (2) #6 x6'-0" (36" LEGS) AT ALL CORNERS OF PANEL. PROVIDE ONE BAR EA. FACE AT DOUBLE MAT REINF.
23. MAT STEEL REINFORCING IS TO BE AS FOLLOWS:
    - A. 9 1/4" THICK PANELS ---------- #6 AT 12" O.C. VERTICAL BAR (EA. FACE)
    - B. 9 1/4" THICK PANELS ---------- #4 AT 12" O.C. HORIZONTAL BAR (EA. FACE)
    - C. MAT STEEL REINFORCING TO BE EACH FACE FOR 11 1/4" TILT WALL PANEL THICKNESS. ALL VERTICAL BARS TO BE CONTINUOUS. (UNLESS NOTED OTHERWISE).
24. UNLESS OTHERWISE NOTED ON PANEL ELEVATIONS, PROVIDE (4) #6 VERTICAL AT 6" O.C. CENTERED IN PANEL X PANEL HIGHT AT ALL BEAMS/JOIST GIRDER LOCATIONS IN ADDITION TO TYPICAL PANEL REINFORCING.
25. AT ALL PANEL OPENINGS OR FUTURE KNOCKOUT OPENINGS PROVIDE (2) #6 VERTICAL X PANEL HEIGHT AT EACH SIDE OF OPENING, (2) #6 x PANING WIDTH AT TOP AND BOTTOM OF OPENING AND (2) #6 x5'-0" DIAG. AT EA. CORNER OF OPENING. PROVIDE ONE BAR EACH FACE AT DOUBLE MAT REINF.
26. RE: GENERAL NOTES SECTION: "CONCRETE" "REINFORCING STEEL", AND ANCHORS FOR ADDITIONAL SPECIFICATIONS
27. EACH WALL PANEL SHALL BE SET ON MINIMUM OF (2) SETTING PADS, (1) PAD AT EACH END WHEN WALL PANEL ENDS RESTS ON TWO SPOT FOOTIGS. AT CONTINUOUS FOOTING LOCAITONS, EACH WALL PANEL SHALL BE SET ON MINIMUM OF (5) SETTING PADS, (1) PAD AT EACH END AND QUARTER POINTS OF PANEL.
28. GROUT SPACE BENEATH WALL PANELS FOR FULL BEARING THE SAME DAY WALL PANELS ARE SET. NON-SHRINK GROUT SHALL BE A NON-METALLIC, SHRINKAGE RESISTANT, PREMIXED, NON-CORROSIVE, NON-STAINING PRODUCT CONTAINING PORTLAND CEMENT, SILICA SAND, SHRINKAGE COMPENSATING AGENTS, AND FLUIDITY IMPROVING COMPOUNDS. NON-SHRINK GROUT SHALL CONFORM TO CORPS OF ENGINEERS SPECIFICATION FOR NON-SHRINK GROUT, C3-CRD-C621. TWENTY EIGHT DAY COMPRESSICE STRENGTH AS DETERMINED BY GROUT CUBE TESTS SHALL BE 5,000 PSI PACK SPACES UNDERNEATH LOAD-BEARING ELEMENTS WITH STIFF GROUT MATERIAL, TAMPING UNTIL VOIDS ARE COMPLETLY FILLED. PLACE GROUT TO FINISH SMOOTH, PLUMB, AND LEVEL WITH ADJACENT CONCRETE SURFACES ENSURING THAT ALL BEARING PLATES AND STEEL SHIMS ARE ENCASED.
29. TEMPORARY ERECTION BRACING OF PANELS SHALL BE LOCATED SUCH THAT NO INTERFERENCE OR INTERRUPTION WITH CONTINUOUS ROOF EDGE ANGLES IS REQUIRED.
30. PANELS DAMAGED DURING ERECTION SHALL BE REPAIRED AS DIRECTED BY THE STRUCTURAL ENGINEER OF RECORD.



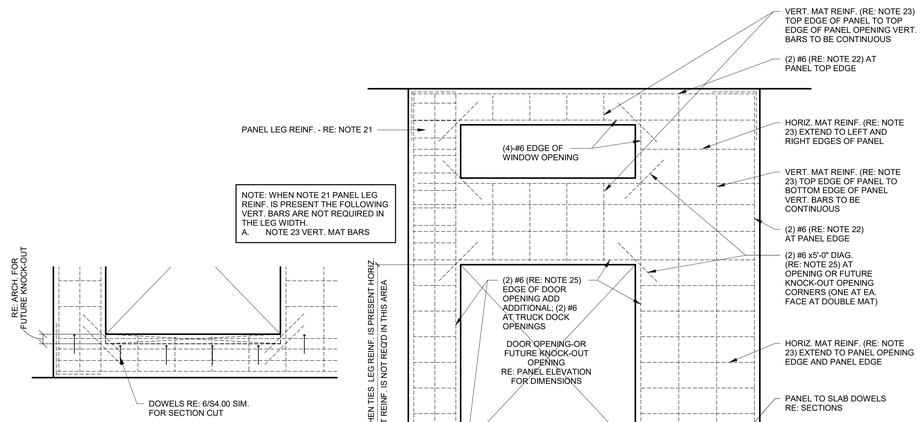
REINFORCEMENT AROUND SCUPPER OPENING

2 / S3.10 — 3/4" = 1'-0"



EMBED DETAIL AT PANEL JOINT / EDGE

3 / S3.10 — 3/4" = 1'-0"



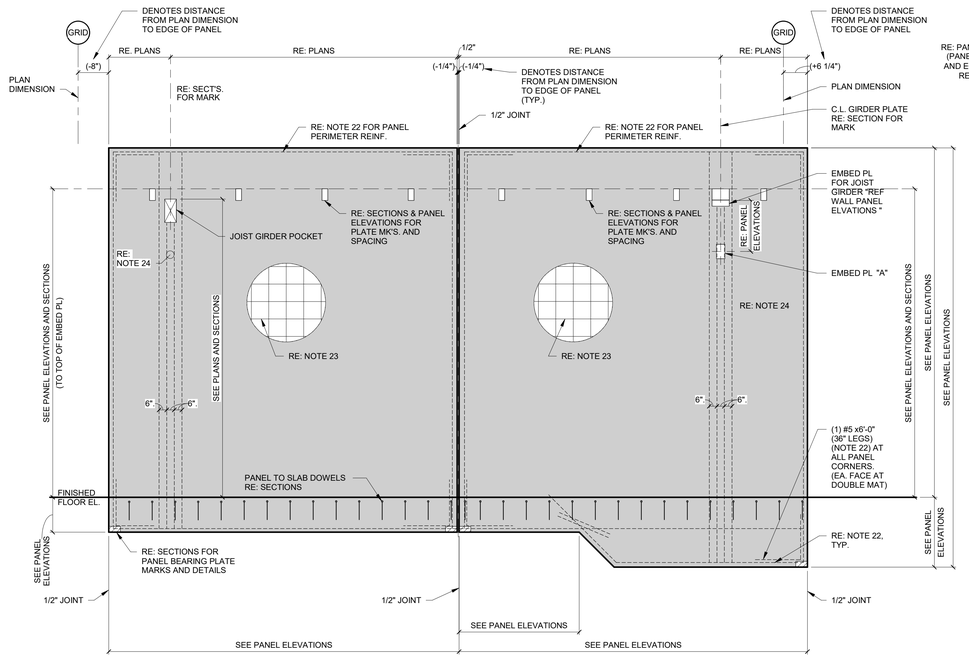
PANEL MK.

PANEL THK.

TYPICAL PANEL REINF. INFORMATION
AT PANELS WITH OPENINGS

NOTE: RE: PANEL ELEVATIONS FOR ANY ADDITIONAL OR REPLACEMENT REINFORCING

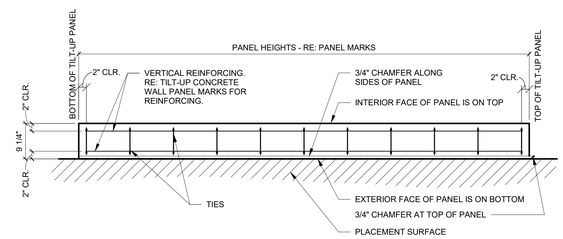
NOTE: PANEL LEG REFERS TO SMALLEST LEG SECTION FROM EDGE OF WALL PANEL / OPENINGS WHEN VERTICALLY SPACED OPENINGS ARE OF UNEQUAL WIDTH.



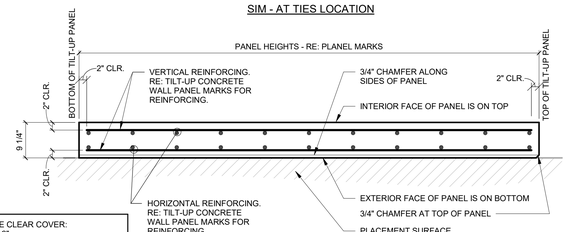
PANEL MK. **PANEL MK.**

TYPICAL PANEL REINF. INFORMATION

NOTE: RE: PANEL ELEVATIONS FOR ANY ADDITIONAL OR REPLACEMENT REINFORCING



SIM - AT TIES LOCATION



CONCRETE CLEAR COVER:
TOP = 2"
BOTTOM = 2"
END = 2"
SIDE:
1 1/2" (VERTICAL REINFORCING BAR)
1" (TIES)

NOTE:
PLACE VERTICAL BARS NEAR EA. FACE OF PANEL AS SHOWN.

DOUBLE LAYER REINFORCING

TYP. TILT-UP PANEL REINFORCEMENT DETAIL

1 / S3.10 — 3/4" = 1'-0"

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MELBOURNE, FL

Revisions / Submissions

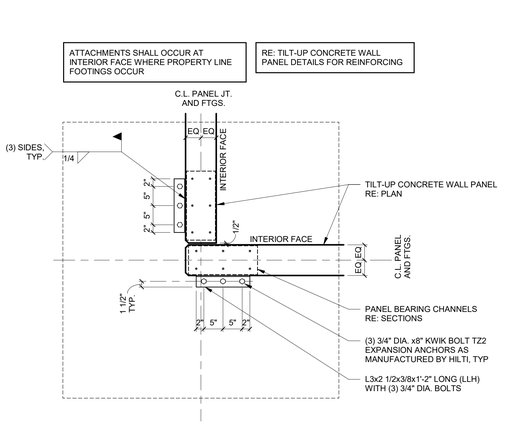
ID	Description	Date

PRELIMINARY FOR COORDINATION AND STRUCTURAL STEEL BIDDING ONLY

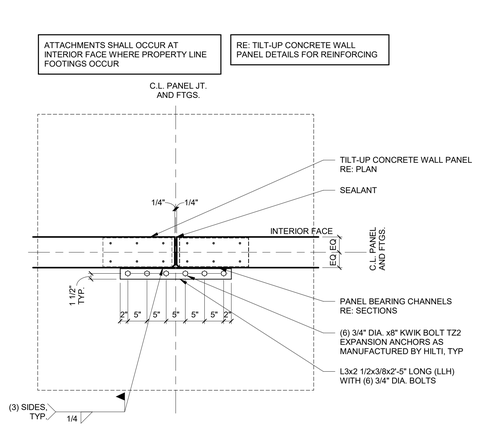
Project Number:	761818-04
Scale:	3/4" = 1'-0"
Drawn By:	JRR
Checked By:	MG/CJJ/US
Date:	2023.11.11
Issue:	BID

Sheet Title:
PANEL NOTES AND EMBEDS

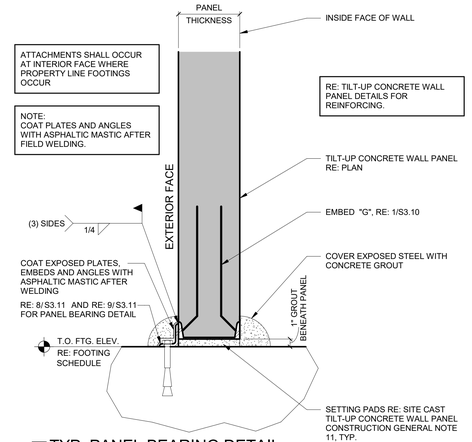
S3.10



9 PANEL BEARING DETAIL
3/4" = 1'-0"

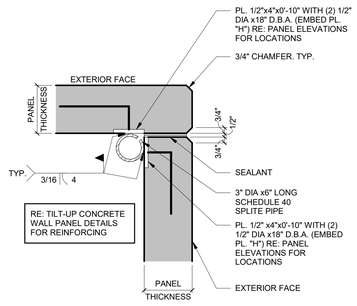


8 PANEL BEARING DETAIL
3/4" = 1'-0"

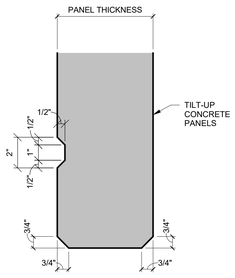


7 TYP. PANEL BEARING DETAIL
1 1/2" = 1'-0"

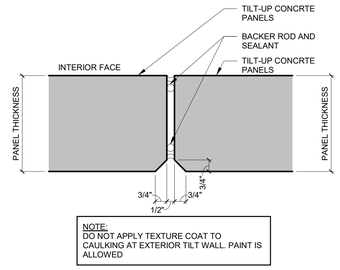
6 DETAIL B
1 1/2" = 1'-0"



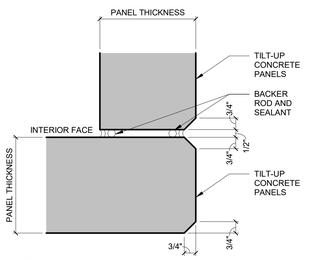
5 CORNER STITCH DETAIL
1 1/2" = 1'-0"



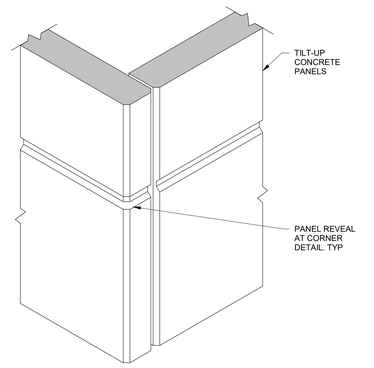
4 REVEAL DETAIL
3" = 1'-0"



3 TYPICAL PANEL JOINT DETAIL
3" = 1'-0"



2 CORNER DETAIL
3" = 1'-0"



1 REVEAL AT CORNER DETAIL
1" = 1'-0"

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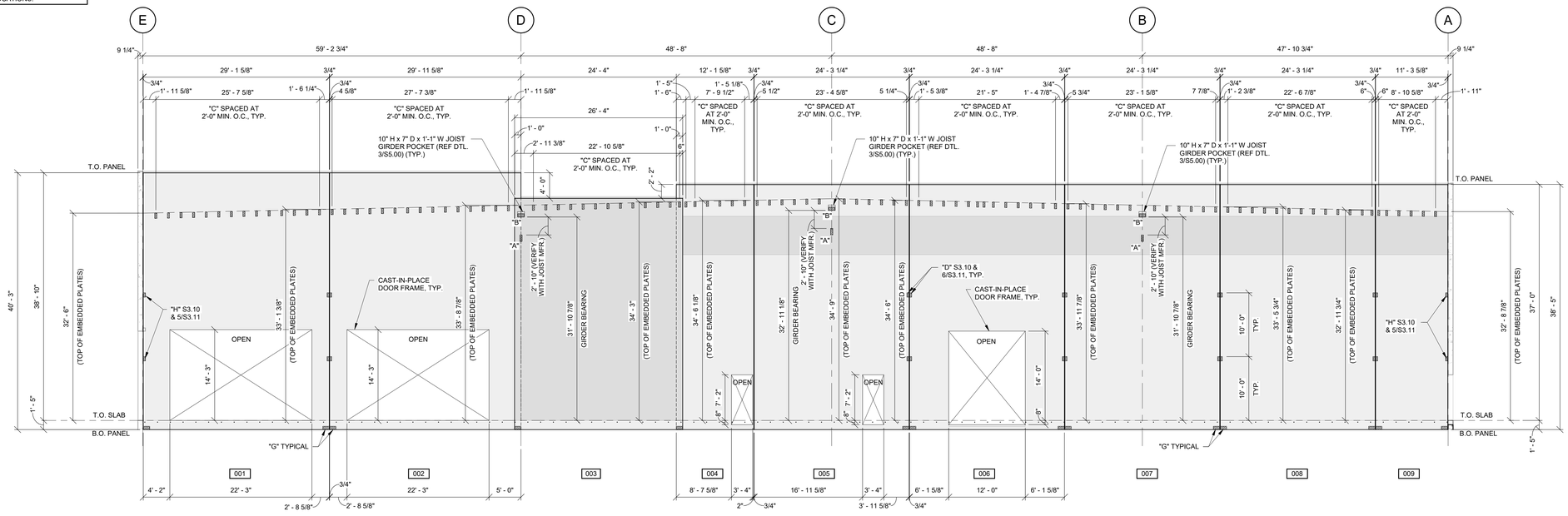
Drawn By:	JRR
Checked By:	MG/CJJ
Date:	2023.11.11
Issue:	BID

Sheet Title:
WALL PANEL DETAILS

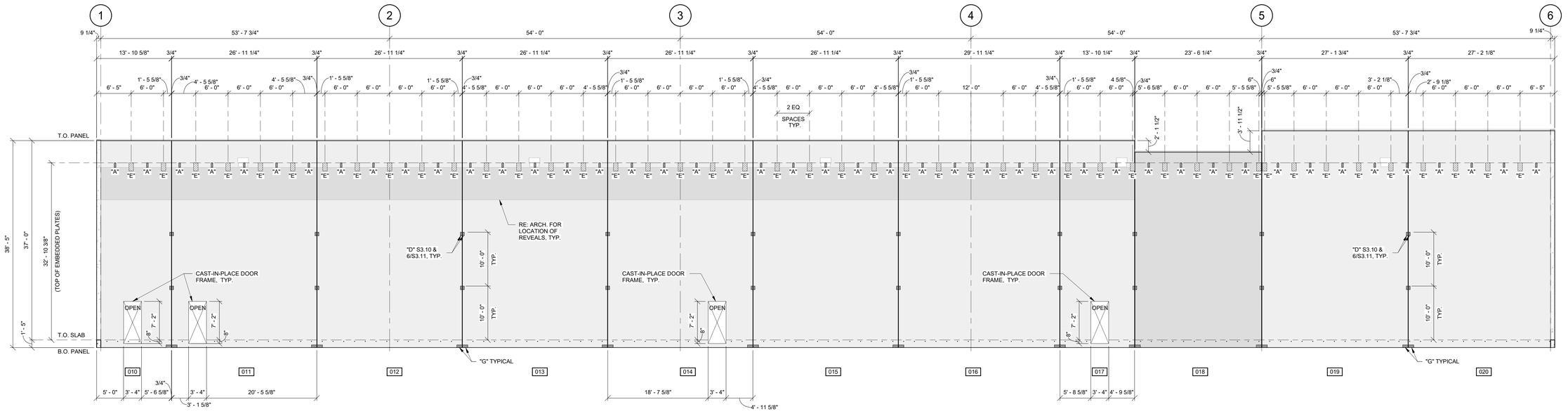
S3.11

NOTES:
1. TRUCK DOCK LEVELER DEPTH (VERIFY WITH MANUFACTURER) + 1'-0" FOR FLOOR SLAB, TYP. U.N.O.
2. LOWER EMBED "A" BY 2" TO FLUSH WITH PRIMARY SCUPPER. WELD BOTTOM OF DIAPHRAGM CHORD ANGLE TO EMBED AT THESE LOCATIONS.



1 S3.20 GRID 1 ELEVATION - PANELS 1 THRU 9
1/8" = 1'-0"

NOTES:
1. TRUCK DOCK LEVELER DEPTH (VERIFY WITH MANUFACTURER) + 1'-0" FOR FLOOR SLAB, TYP. U.N.O.
2. LOWER EMBED "A" BY 2" TO FLUSH WITH PRIMARY SCUPPER. WELD BOTTOM OF DIAPHRAGM CHORD ANGLE TO EMBED AT THESE LOCATIONS.



2 S3.20 GRID A ELEVATION - PANELS 10 THRU 20
1/8" = 1'-0"

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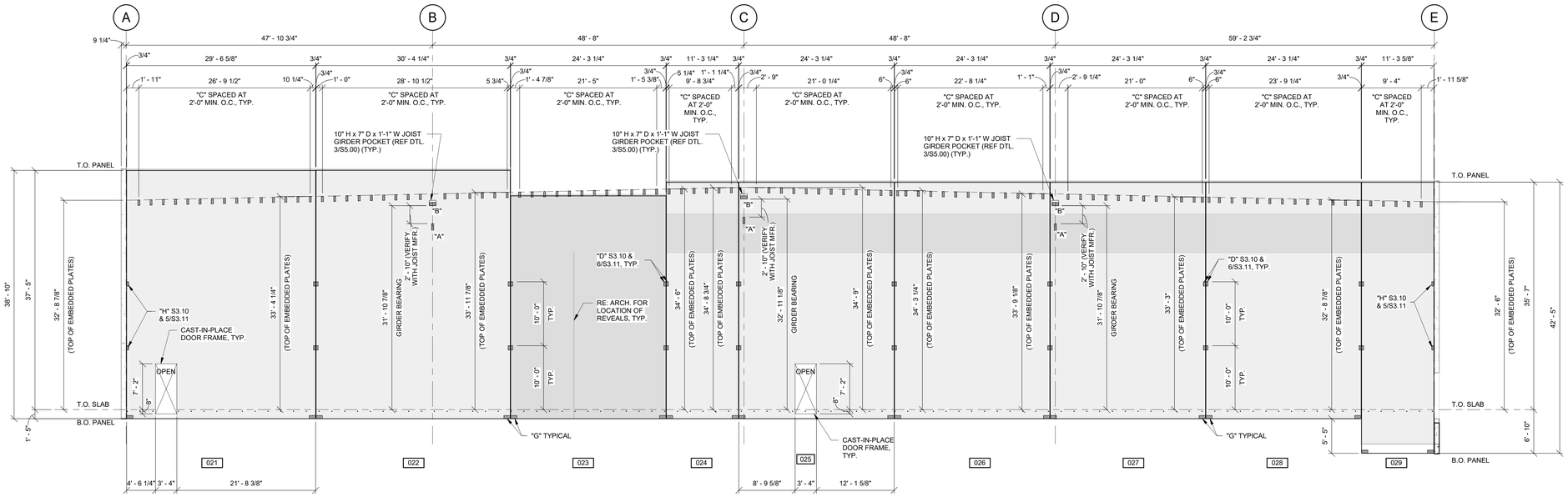
Issue:	BID

Sheet Title:
EXTERIOR WALL ELEVATIONS

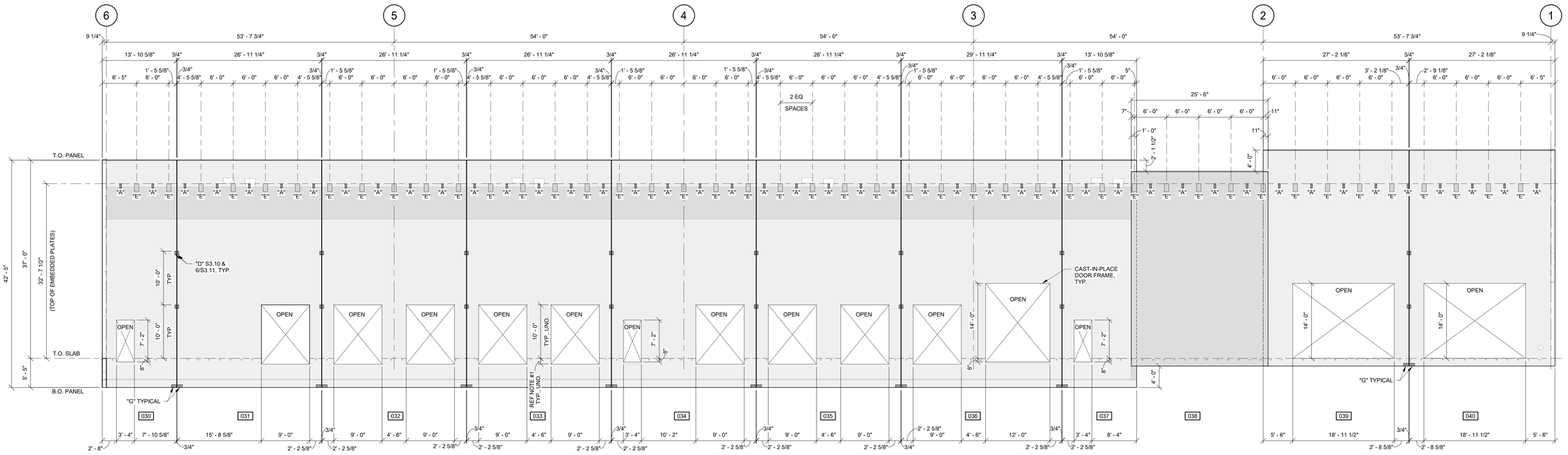
S3.20

NOTES:
1. TRUCK DOCK LEVELER DEPTH (VERIFY WITH MANUFACTURER) + 1'-0" FOR FLOOR SLAB, TYP. U.N.O.
2. LOWER EMBED "A" BY 2" TO FLUSH WITH PRIMARY SCUPPER. WELD BOTTOM OF DIAPHRAGM CHORD ANGLE TO EMBED AT THESE LOCATIONS.



1 S3.21 GRID 6 ELEVATION - PANELS 21 THRU 29
1/8" = 1'-0"

NOTES:
1. TRUCK DOCK LEVELER DEPTH (VERIFY WITH MANUFACTURER) + 1'-0" FOR FLOOR SLAB, TYP. U.N.O.
2. LOWER EMBED "A" BY 2" TO FLUSH WITH PRIMARY SCUPPER. WELD BOTTOM OF DIAPHRAGM CHORD ANGLE TO EMBED AT THESE LOCATIONS.



2 S3.21 GRID E ELEVATION - PANELS 30 THRU 40
1/8" = 1'-0"

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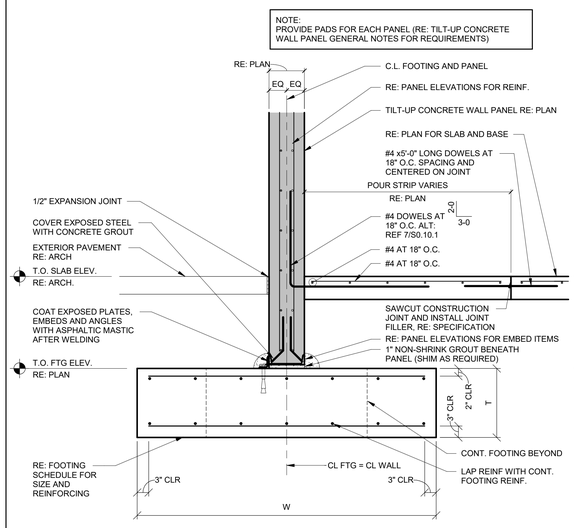
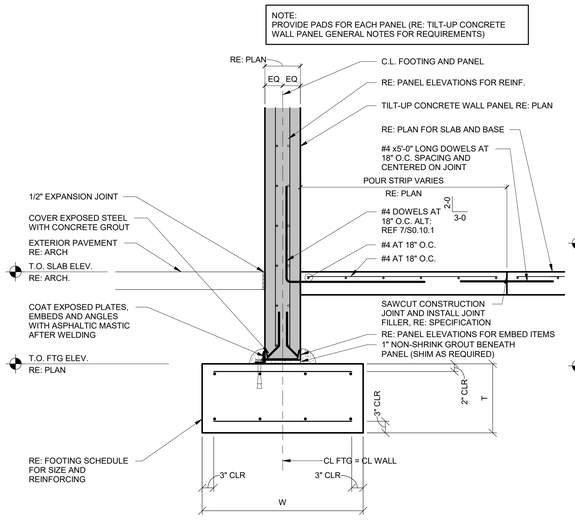
Sheet Title:
EXTERIOR WALL ELEVATIONS

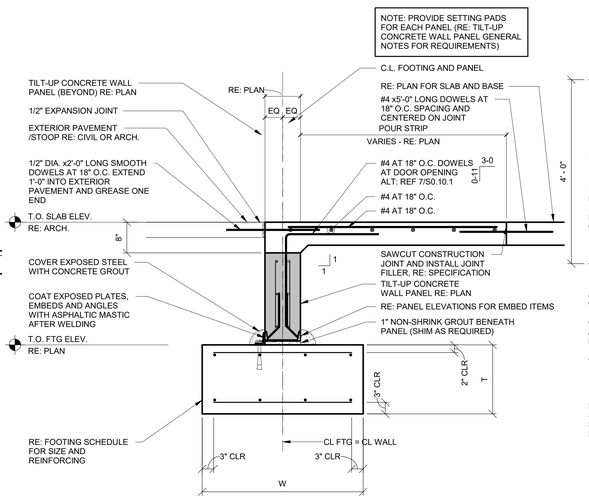
S3.21



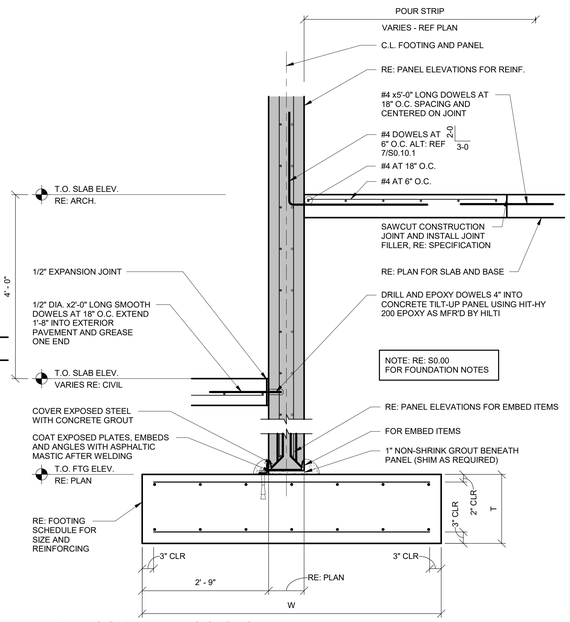
1 S4.00 TILT-UP EXTERIOR WALL SPOT FOOTING
3/4" = 1'-0"



2 S4.00 SECTION AT WALL BASE AT CONTINUOUS WALL FOOTING
3/4" = 1'-0"

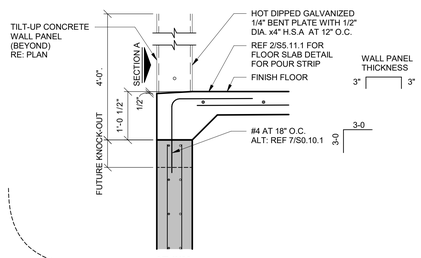


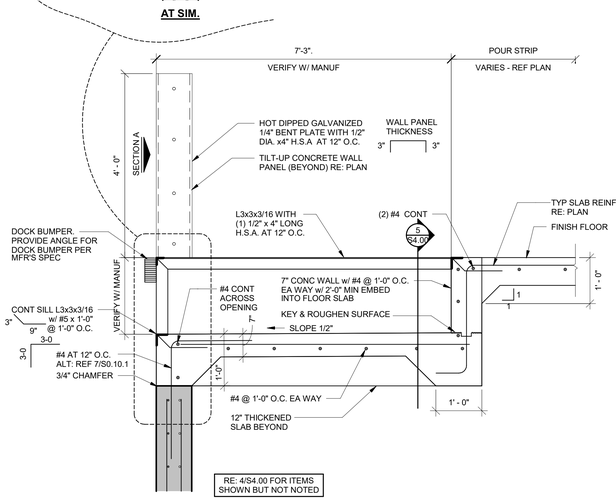
3 S4.00 SLAB AT OPENING
3/4" = 1'-0"



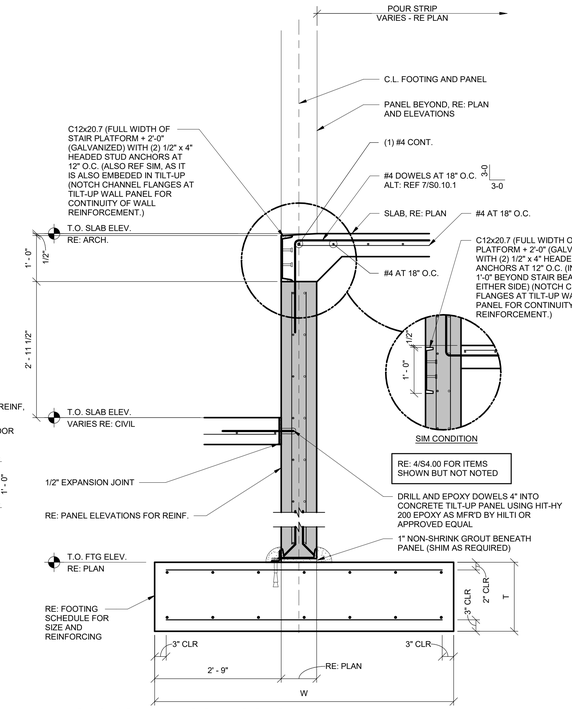
4 S4.00 DOCK WALL FOOTING
3/4" = 1'-0"

5 S4.00 SIDE OF DOCK LEVELER
3/4" = 1'-0"

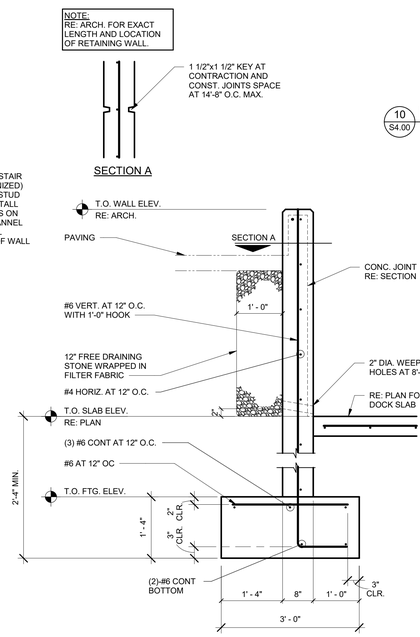




6 S4.00 DOCK LEVELER SECTION
3/4" = 1'-0"

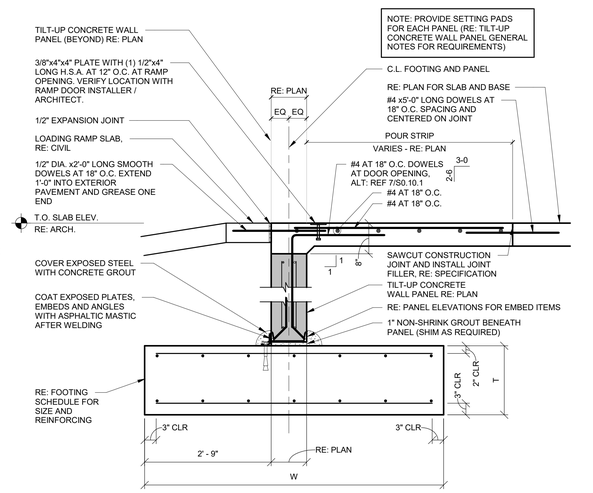


7 S4.00 SECTION AT DOCK DOORS
3/4" = 1'-0"



8 S4.00 DOCK RETAINING WALL
3/4" = 1'-0"

10 S4.00 RAMP DETAIL
3/4" = 1'-0"



9 S4.00 RAMP OPENING DETAIL
3/4" = 1'-0"

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MELBOURNE, FL

Revisions / Submissions

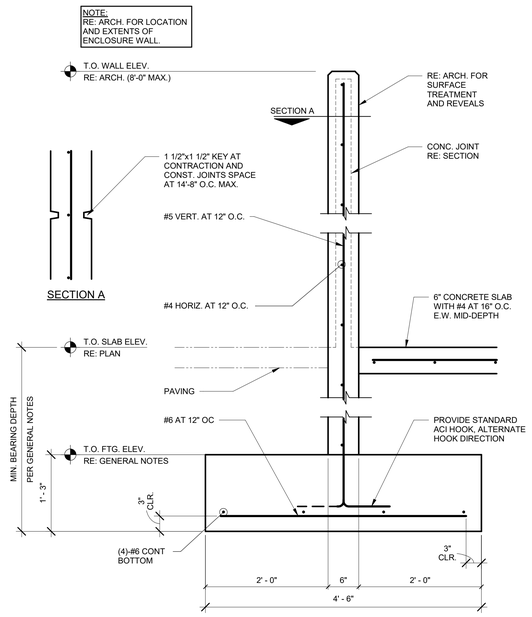
ID	Description	Date

PRELIMINARY FOR COORDINATION AND STRUCTURAL STEEL BIDDING ONLY

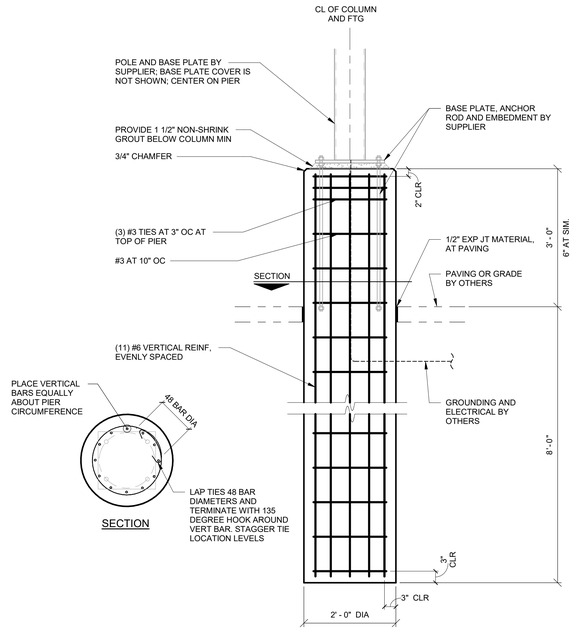
Project Number:	761818-04
Scale:	3/4" = 1'-0"
Drawn By:	JRR
Checked By:	MG/CJJ
Date:	2023.11.11
Issue:	BID

Sheet Title:
FOUNDATION DETAILS

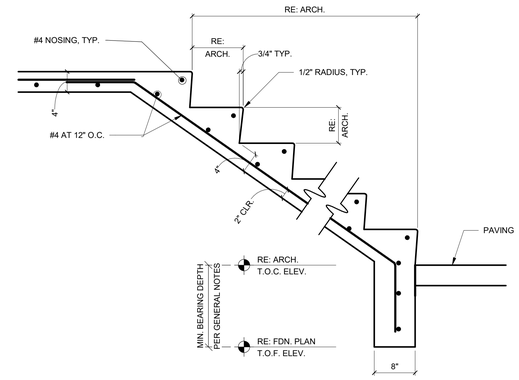
S4.00



1 S4.11 TRASH ENCLOSURE WALL SECTION
1" = 1'-0"



2 S4.11 LIGHT POLE FOUNDATION
3/4" = 1'-0"



3 S4.11 SECTION AT STAIRS
1" = 1'-0"

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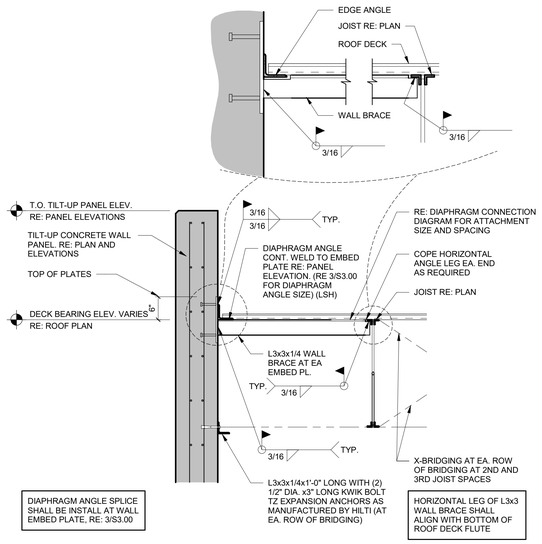
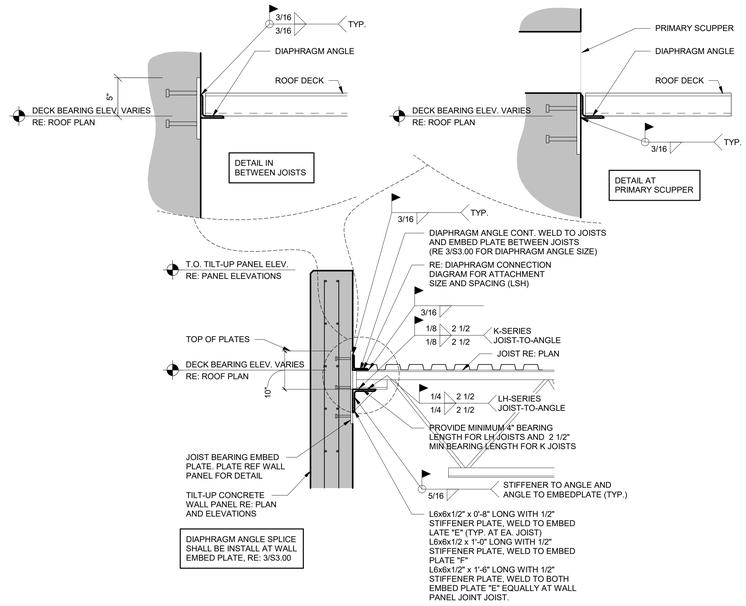
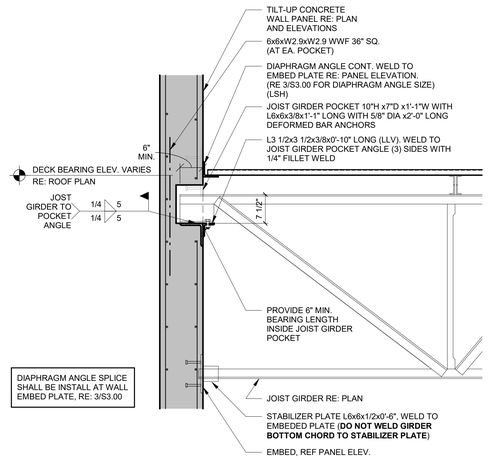
S4.11



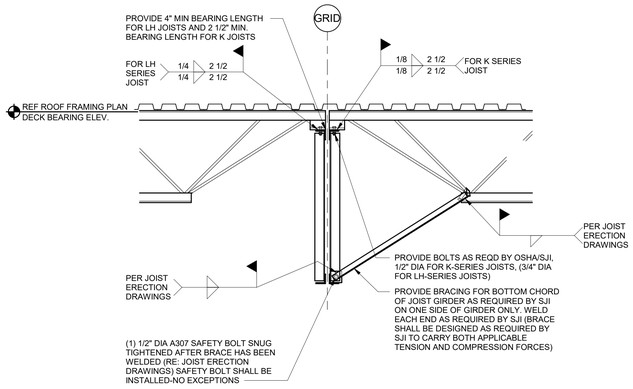
1 S5.00 WALL SECTION PARALLEL TO JOISTS
3/4" = 1'-0"



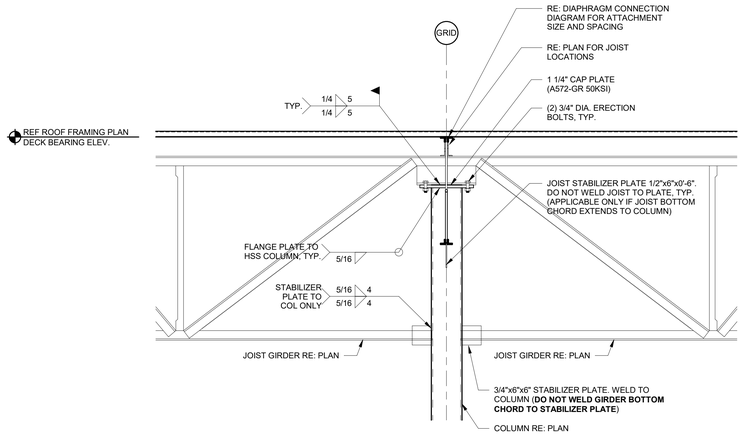
2 S5.00 WALL SECTION PERPENDICULAR TO JOISTS
3/4" = 1'-0"



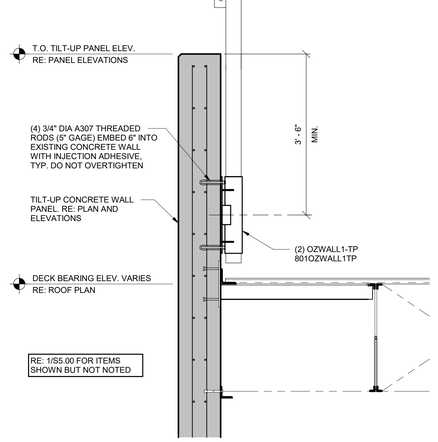
3 S5.00 GIRDER BEARING AT TILT-UP WALL
3/4" = 1'-0"



4 S5.00 JOIST TO JOIST GIRDER
3/4" = 1'-0"



5 S5.00 JOIST GIRDER AT COLUMN
3/4" = 1'-0"



6 S5.00 CRANE CONNECTON DETAIL
3/4" = 1'-0"

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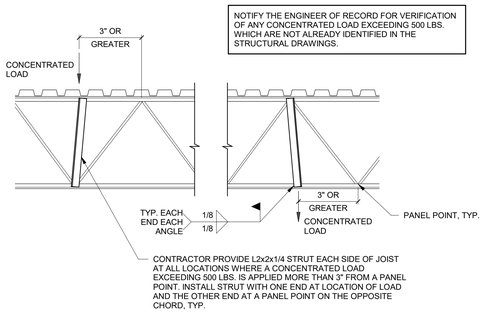
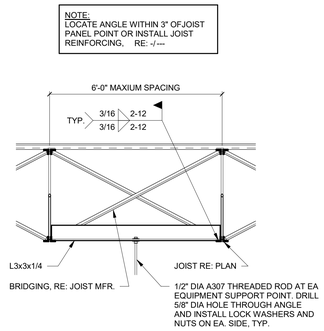
Sheet Title:
ROOF FRAMING DETAILS

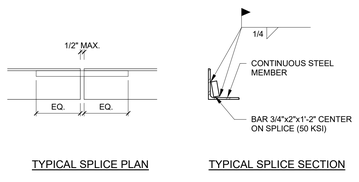
S5.00



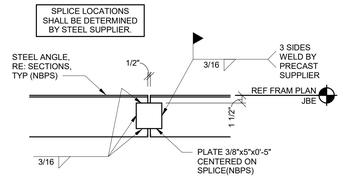
1 S5.10 JOIST REINFORCING DETAIL
3/4" = 1'-0"



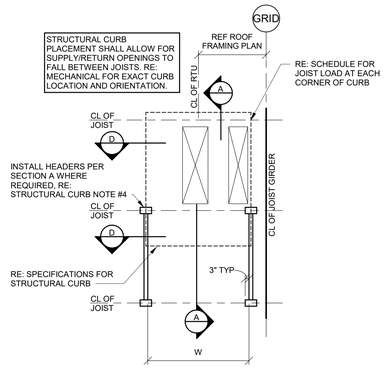
2 S5.10 TYP SUSPENDED EQUIPMENT SUPPORT DETAIL
NOT TO SCALE



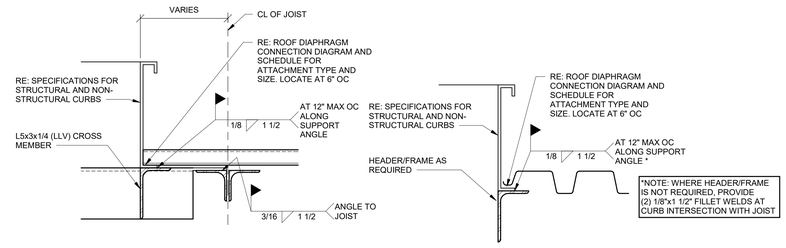
3 S5.10 DECK ANGLE SPLICE DETAIL
1 1/2" = 1'-0"



4 S5.10 PERIMETER BEARING ANGLE SPLICE
1" = 1'-0"

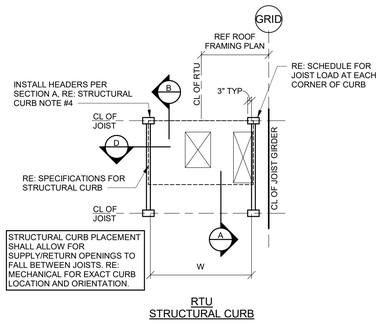


20 TON RTU
STRUCTURAL CURB

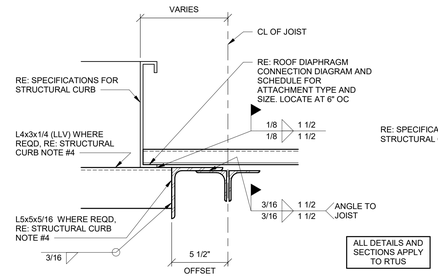


SECTION C

SECTION D

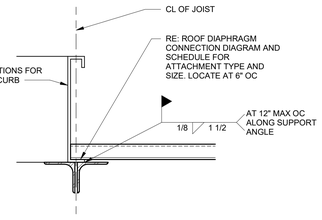


RTU
STRUCTURAL CURB

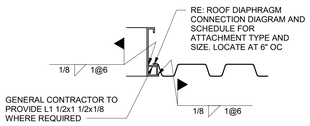


SECTION A

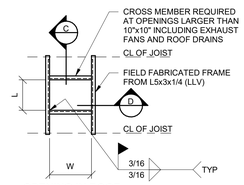
ALL DETAILS AND SECTIONS APPLY TO RTUS



SECTION B



DETAIL 1



OPENING IN ROOF
LARGER THAN 10"x10"

NOTES:

1. INSTALL STRUCTURAL CURBS, HEADERS, AND FRAMES AND WELD TO SUPPORT STEEL BEFORE DECK IS PLACED.
2. ADJUST LOCATION OF RTU SO CURB FALLS WITHIN 3 INCHES OF JOIST PANEL POINTS. IF CURB IS MORE THAN 3 INCHES FROM PANEL POINT ADD REINFORCING PER JOIST REINFORCING DETAIL. (1/S5.10).
3. GENERAL CONTRACTOR SHALL COORDINATE RTU DIMENSIONS AND FRAMING LOCATIONS WITH THE STEEL FABRICATOR, MECHANICAL AND ERECTION SUBCONTRACTORS.
4. HEADERS ARE REQUIRED FOR STRUCTURAL CURBS WHEN THE CURB DOES NOT SPAN BETWEEN TWO JOIST OR THE CURB CANTILEVERS MORE THAN TWO FEET OVER THE JOIST. HEADERS ARE REQUIRED AS SHOWN FOR AHU'S AND HYBRID HOUSES.
5. RE: DETAIL 1 FOR CONNECTION OF DECK PARALLEL TO CURB, WHERE REQUIRED.
6. DO NOT INSTALL STRUCTURAL CURBS ON TOP OF DECK.
7. STEEL SUPPLIER TO FURNISH STOCK ANGLE FOR FIELD FABRICATED FRAMES.
8. RE: SPECIFICATIONS AND MECHANICAL ROOFTOP HVAC UNIT SCHEDULES FOR RTU ANCHORAGE.
9. CURB WIDTHS SHOWN IN THE STRUCTURAL CURB SCHEDULE ARE FOR ROOF FRAMING DESIGN PURPOSES ONLY AND SHALL NOT BE USED FOR STRUCTURAL CURB FABRICATION.

5 S5.10 RTU FRAMING SUPPORT PLAN
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S5.10