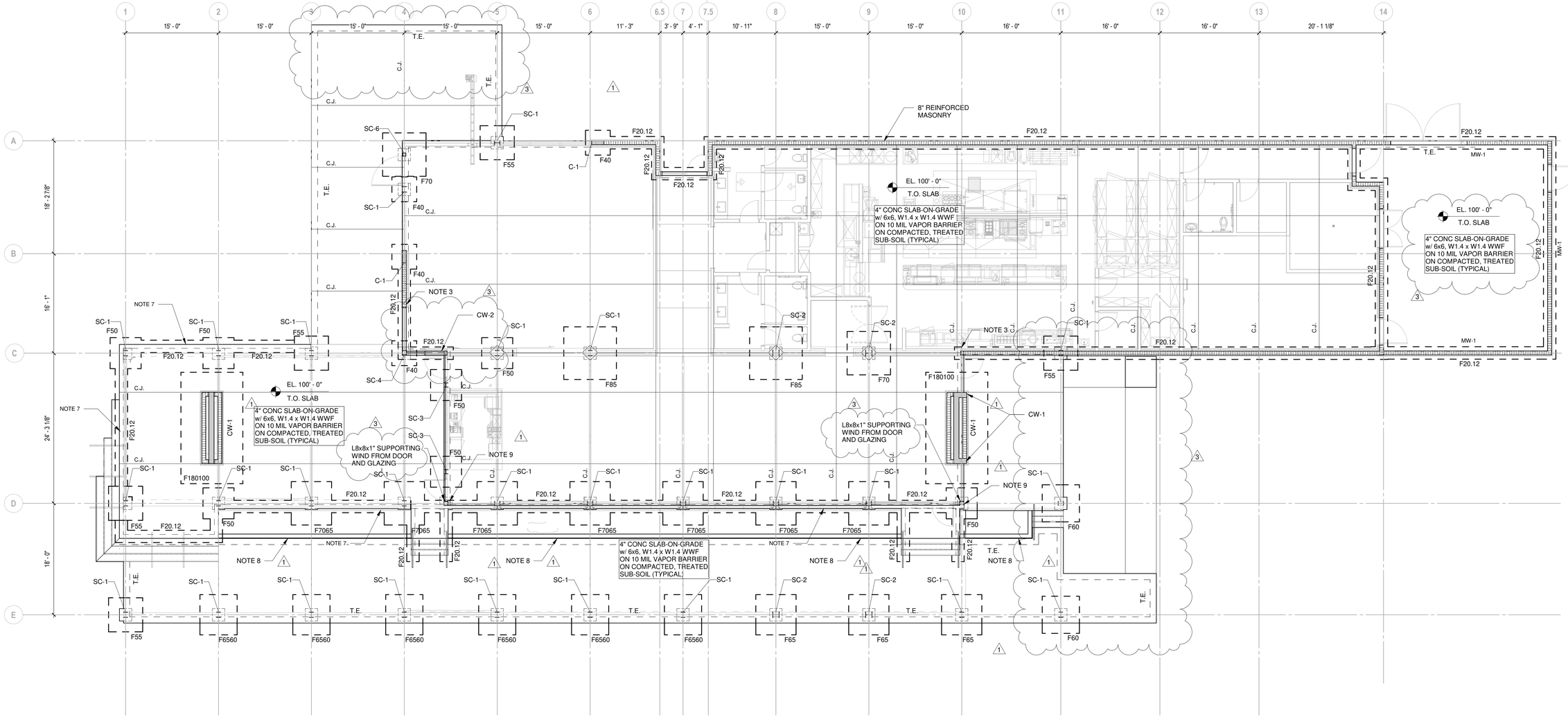

1 Foundation Plan
1/8" = 1'-0"

NOTE:

3 1. ALL CMU TO BE 8" MASONRY WITH #6 @ 40" O.C.. PROVIDE (2) FILLED CELLS AT EACH SIDE OF EACH OPENING GREATER THAN 3'-0" O.C. U.N.O.

2. PROVIDE FILLED CELL WITH REINFORCEMENT MATCHING WALL TYPE AT ENDS, CORNERS, AND INTERSECTIONS OF ALL CMU WALLS.

3. PROVIDE (2) FILLED CELLS AT END OF WALL WITH REINFORCEMENT MATCHING WALL TYPE.

4. PROVIDE CONTINUOUS BOND BEAM WITH (2) #5 x CONTINUOUS AT 8'-0" AT ALL CMU WALLS.

3 5. PROVIDE BB-1 AT TOP OF PARAPET WALLS U.N.O.

6. CP-1 INDICATES 24"X24" CONCRETE PIER W/ 8 #8 VERT. & #3 @ 12" o.c. 2 TIES @ 4 o.c. AT TOP.

3 7. CW-1 INDICATES 10" CONCRETE WALL W/ #5 @ 10" o.c. E.W. E.F.
CW-2 INDICATES 8" CONCRETE WALL W/ #5 @ 10" o.c. E.W. E.F.

8. RETAINING WALL AT SLAB. PROVIDE F20.12 FOUNDATION W/ 6" CMU WALL W/ #5 @ 48" o.c. WITH BB-1 AT TOP. MAX. RETAINED HEIGHT -3'-8".

9. PROVIDE SLIP CONNECTION FOR POST AT ROOF. 1

NOTE TO G.C.:

THERE ARE ADDITIOINAL SLAB ON GRADES THAT ARE FOR THE TREATMENT AND WELL AREAS THAT ARE REQUIRED (APPROXIMATELY 2 LOCATIONS). PROVIDE A 6" CONC. SLAB ON GRADE W/ 6x6 - W2.9xW2.9 WWF ON COMPACTED, TREATED SUB-SOIL. REFER TO ARCHITECTURAL, CIVIL, AND ALL OTHER DRAWINGS FOR SIZE AND LOCATION OF THESE SLABS. PROVIDE CONTROL JOINTS AT A MAX. OF 5'-0" o.c. IN EACH DIRECTION. 3

DRAWING LIST

Sheet Number	Sheet Name
S1.01	Foundation Plan
S1.02	Roof Framing Plan
S2.01	Structural Notes & Schedules
S2.02	Wind Loads
S2.03	Typical Details
S2.04	Typical PT Details
S3.01	Sections
S3.02	Sections

PHILIP J. RIZZO LICENSE No. 60998 STATE OF FLORIDA PROFESSIONAL ENGINEER

This item has been digitally signed and sealed by Philip J. Rizzo using a Digital Signature and date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

03/22/2024

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ATLANTIC FIELDS
GOLF & BEACH CLUB

ATLANTIC FIELDS - GOLF HOUSE

PERMIT SET

2645 SE BRIDGE ROAD, HOBE SOUND, FL 33455

Seal:

Foundation Plan

Revisions:

No.	Description	Date
1	Bldg. Dept. Comments	12.15.2023
3	Addendum #1	03.22.2024

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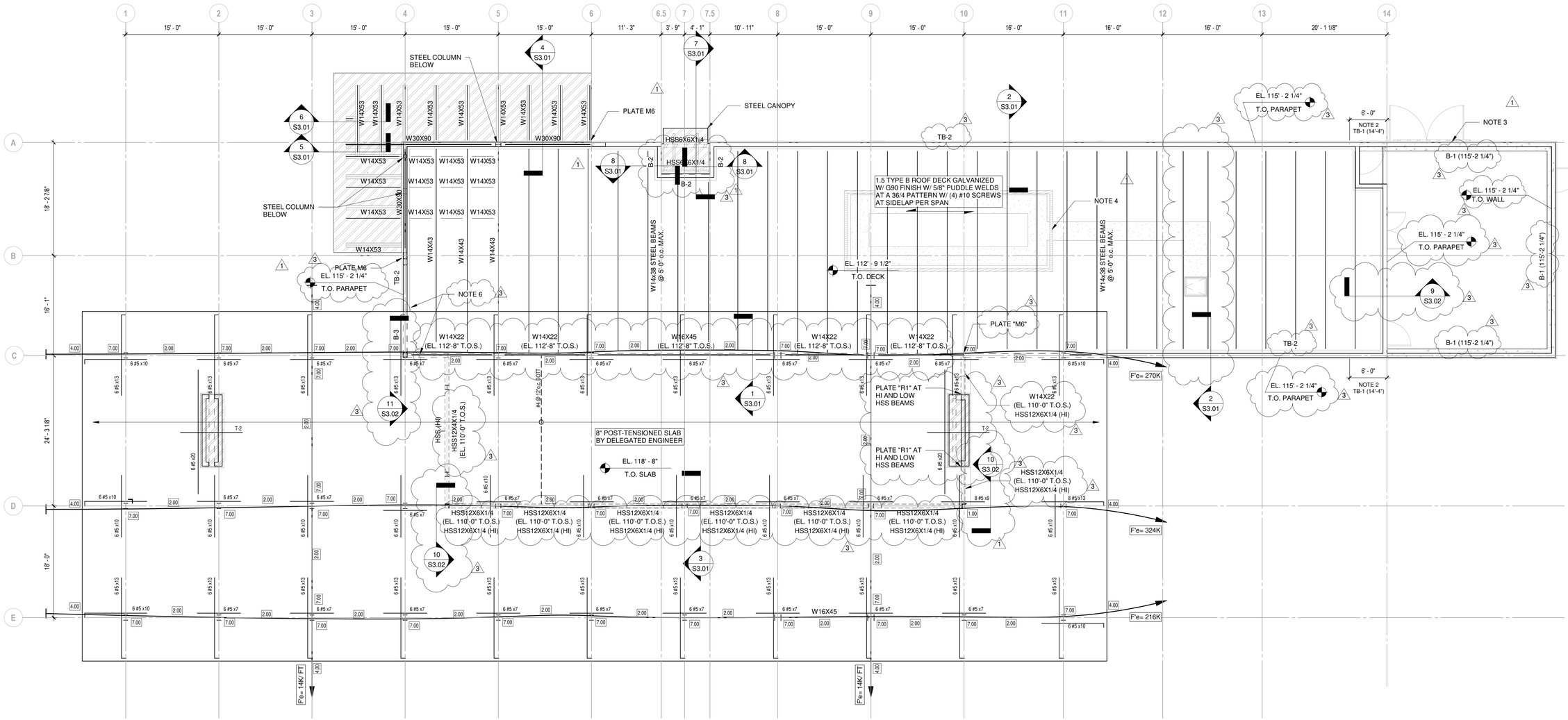
Sheet:

S1.01



1 Roof Framing Plan
1/8" = 1'-0"

NOTE:

1. PROVIDE BOTTOM MAT OF #4@ 18" O.C. E.W. IN POST TENSIONED CONCRETE SLAB. SEE PLAN FOR ADDITIONAL REINFORCEMENT.
2. EXTEND HORIZONTAL TIE BEAM REINFORCEMENT INTO B-1 BEAM 4'-0".
3. PROVIDE PRECAST LINTEL W/ (2) COURSE BOND BEAM W/ (2) #5 AT DOOR HEADER BELOW B-1.
4. ROOF SCREEN BY SPECIALTY SUPPLIER DESIGNED FOR LOCAL CODES. DRAWINGS AND DETAILS BY DELEGATED ENGINEER PER SHEET S2.01/DELEGATED ENGINEERED PRODUCTS.
5. PROVIDE TB-1 AT STEEL BEAM BEARING ELEVATION U.N.O.
6. PROVIDE CONT. 4x4x1/4" DECK EDGE ANGLE AND 5/8" EXPANSION ANCHOR INTO CONCRETE TIE BEAM @ 24" o.c.

POST-TENSIONED SLAB AND PENETRATIONS:

- ALL SLAB ANCHORS AND PENETRATIONS MUST BE INSTALLED IN THE SLAB PRIOR TO POURING CONCRETE AND TENSIONING CABLES. ENGINEER OF RECORD (E.O.R.) BEARS NO RESPONSIBILITY FOR CABLES DAMAGED AS A RESULT OF THIS OMISSION.
- IF DRILLING OR OTHER DESTRUCTIVE SLAB WORK IS REQUIRED, GENERAL CONTRACTOR MUST FIRST OBTAIN WRITTEN APPROVAL FROM E.O.R., THEN COORDINATE WORK WITH POST-TENSION SPECIALTY ENGINEER. (PRIOR TO SLAB POUR, GENERAL CONTRACTOR MAY WISH TO SPRAY PAINT FORM WORK UNDER POST-TENSION CABLES TO AID IN FUTURE CABLE LOCATING).

GENERAL CONTRACTOR NOTE :

- DO NOT SCALE DRAWINGS ! COORDINATE DIMENSIONS BETWEEN STRUCTURAL AND ARCHITECTURAL DRAWINGS - NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES BEFORE PROCEEDING WITH WORK.
- INSTALL ALL EMBED PLATES/ ANGLES/ FASTENERS IN SLAB, COLUMNS, BEAMS & CMU WALLS PRIOR TO CONCRETE POURS

NOTE :

MASONRY WALLS ARE NOT TO BE COMPLETED UNTIL THE FLOOR ABOVE HAS BEEN POURED, CABLES TENSIONED AND ALL SHORES AND RESHORES REMOVED. INTERIOR MASONRY WALLS ARE TO STOP 1/2" BELOW CONCRETE SLAB ABOVE.

ALL ELEVATOR HOIST BEAMS, SEPERATOR BEAMS AND CONNECTIONS BY SUPPLIER'S DELEGATED ENGINEER.

NOTE:

A MINIMUM OF TWO TENDONS SHALL BE PROVIDED IN EACH DIRECTION THROUGH SUPPORT COLUMNS (PER ACI 18.12.6)

LEGEND:

- INDICATES #4 HAIRPIN
- INDICATES #5 TOP BARS
- T-1 INDICATES #5 TOP BARS @ 12"o.c. (5' LONG W/ 10" HOOK)
- T-2 INDICATES #5 TOP BARS @ 12"o.c. (12' LONG W/ 10" HOOK)
- T-3 INDICATES #5 TOP BARS @ 12"o.c. (10' LONG W/ 10" HOOK)
- T-4 INDICATES (1) #5 DIAGONAL TOP & BOTTOM BARS (5' LONG)
- T-5 INDICATES #5 TOP BARS @ 12"o.c. (8' LONG W/ 10" HOOK)
- INDICATES #5 TOP BARS U.N.O.
- INDICATES #4 BOTTOM BARS U.N.O.
- SR-X INDICATES STUD RAIL PUNCHING SHEAR REINFORCING (FOR PRICING PUPOSE ONLY). REFER TO POST TENSION DELEGATED ENGINEER DRAWINGS FOR STUD RAIL DESIGN. SEE TYPICAL DETAIL ON SHEET SX.XX FOR MORE INFORMATION.

TYPICAL FRAMING NOTES:

- MASONRY WALLS ARE NOT TO BE COMPLETED UNTIL THE FLOOR ABOVE HAS BEEN POURED, CABLES TENSIONED AND ALL SHORES AND RESHORES REMOVED.
- ALL MASONRY WALLS TO HAVE A REINFORCED, GROUT FILLED CELL AT EVERY CORNER, BOTH EDGES OF EVERY OPENING. FOR EXTERIOR WALLS ONLY PROVIDE FILLED CELLS ON EACH SIDE OF OPENING W/ MATCHING VERTICAL REINFORCING PER THE MASONRY WALL SCHEDULE (U.N.O.).
- INTERIOR MASONRY WALLS ARE TO STOP 1/2" BELOW CONCRETE SLAB ABOVE.
- PROVIDE CONTROL JOINTS BETWEEN MASONRY WALLS AND INTERIOR COLUMNS AND SHEAR WALLS, EXCEPT THE AREA OVER THE SPREAD FOOTINGS.
- ALL REINF. BARS ON PLAN ARE #5 U.N.O. AMOUNT OF BARS ARE INDICATED ON PLAN.

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Roof Framing Plan

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S1.02

STRUCTURAL NOTES

CONTRACTOR NOTE:

THE CONTRACTOR IS SOLELY RESPONSIBLE FOR INITIATING, MAINTAINING AND SUPERVISING ALL SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK. O'DONNELL, NACCARATO, MIGNOGNA & JACKSON, INC. IS NOT RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION OR FOR RELATED SAFETY PRECAUTIONS AND PROGRAMS.

010 CODES AND STANDARDS

1. WIND LOADS AS PER:

A. SECTION 1609 OF THE FLORIDA BUILDING CODE 7TH EDITION (2020) WITH AN ULTIMATE WIND SPEED VULT = 170 MPH (NOMINAL WIND SPEED VASD = 132 MPH), FOR RISK CATEGORY II, EXPOSURE C AND INTERNAL PRESSURE COEFFICIENT +/- 0.18.

B. THIS BUILDING IS DESIGNED AS AN ENCLOSED BUILDING.

2. DESIGN LOADS:

A. LIVE LOADS - COMMERCIAL:

1.1 ROOF 20 PSF

B. ROOF SUPERIMPOSED DEAD LOADS:

2.1 MEP / MISC 5 PSF
2.2 ARCH'L FINISHES 10 PSF
2.3 CEILING 5 PSF
2.4 ROOFING 5 PSF
2.5 INSULATION 5 PSF

3. THE PROJECT WAS DESIGNED IN ACCORDANCE WITH THE:

A. FLORIDA BUILDING CODE 7TH EDITION (2020).

B. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318/LATEST EDITION).

C. MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES (ACI 315/ LATEST EDITION).

D. MANUAL OF STANDARD PRACTICE FOR WELDING REINFORCING STEEL, INSERTS & CONNECTIONS IN REINFORCED CONCRETE CONSTRUCTION. AWS. D1.4/LATEST EDITION.

E. SPECIFICATION FOR THE DESIGN, FABRICATION & ERECTION OF STRUCTURAL STEEL FOR BUILDINGS. (AMERICAN INSTITUTE OF STEEL CONSTRUCTION) AISC STEEL CONSTRUCTION MANUAL.

F. SPECIFICATION FOR STRUCTURAL CONCRETE FOR BUILDINGS, ACI 301/LATEST EDITION.

G. NATIONAL DESIGN SPECIFICATION, WOOD CONSTRUCTION NDS/LATEST EDITION.

H. BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530, 530.1/ASCE 5, 6/TMS 402, 602/LATEST EDITIONS).

4. ARCHITECTURAL AND MECHANICAL DRAWINGS:

A. THE STRUCTURAL DRAWINGS ARE PART OF THE CONTRACT DOCUMENTS AND DO NOT BY THEMSELVES PROVIDE ALL THE INFORMATION REQUIRED TO PROPERLY COMPLETE THE PROJECT STRUCTURE. THE GENERAL CONTRACTOR SHALL CONSULT THE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND EVERY OTHER DRAWING CREATED TO CONSTRUCT THIS PROJECT AND COORDINATE THE INFORMATION CONTAINED IN THESE DRAWINGS WITH THE STRUCTURAL DRAWINGS TO PROPERLY CONSTRUCT THE PROJECT.

B. REFER TO ARCHITECTURAL, MECHANICAL OR ELECTRICAL DRAWINGS FOR ADDITIONAL OPENINGS, DEPRESSIONS, FINISHES, INSERTS, BOLTS SETTINGS, DRAINS, REGLETS, ETC.

C. BEFORE ORDERING ANY MATERIALS OR DOING ANY WORK, THE CONTRACTOR SHALL VERIFY ALL MEASUREMENTS TO PROPERLY SIZE OR FIT THE WORK. NO EXTRA CHARGE OR COMPENSATION WILL BE ALLOWED BY THE OWNER RESULTING FROM THE CONTRACTOR'S FAILURE TO COMPLY WITH THIS REQUIREMENT.

D. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH ANY WORK.

E. ALL STRUCTURES HAVE BEEN DESIGNED TO RESIST THE DESIGN LOADS LISTED ONLY AS COMPLETED STRUCTURES. THE GENERAL CONTRACTOR SHALL FULLY BRACE AND OTHERWISE PROTECT WORK IN PROGRESS UNTIL THE STRUCTURES ARE COMPLETED. THE GENERAL CONTRACTOR SHALL ALSO ENSURE THAT ITS OPERATIONS AND PROCEDURES PROVIDE NO LOADING GREATER THAN THE DESIGN LOADS LISTED ON ANY MEMBER.

5. SECTIONS AND DETAILS:

ALL DETAILS, SECTIONS AND NOTES SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE UNLESS OTHERWISE SHOWN.

6. MATERIALS AND ASSEMBLY TEST AS FOLLOWS:

A. EXTERIOR WINDOWS, SLIDING AND PATIO GLASS DOORS GLASS BLOCK, AND ANY OTHER PRODUCT USED IN THE EXTERIOR OF THE BUILDING. SHALL BE TESTED BY AN APPROVED INDEPENDENT TESTING LABORATORY, AND SHALL BE LABELED WITH AN APPROVED LABEL IDENTIFYING THE MANUFACTURER, PERFORMANCE CHARACTERISTICS AND APPROVED PRODUCT CERTIFICATION AGENCY, TESTING LABORATORY, EVALUATION ENTITY OR FLORIDA STATE WIDE PRODUCT APPROVAL NUMBER TO INDICATE COMPLIANCE WITH THE REQUIREMENTS OF ONE OF THE FOLLOWING SPECIFICATIONS:

AAMA/WDMA/CSA 101/I.S.2/A440 OR TAS202 (HVHZ SHALL COMPLY WITH TAS202 AND ATSM E1300 OR FBC SECTION 2404)

B. EXTERIOR DOOR ASSEMBLIES SHALL BE TESTED FOR STRUCTURAL INTEGRITY IN ACCORDANCE WITH ASTM E330 OR TAS202 (HVHZ SHALL COMPLY WITH TAS202) AT A LOAD OF 1.5 TIMES THE REQUIRED DESIGN PRESSURE LOAD. THE LOAD SHALL BE SUSTAINED FOR 10 SECONDS WITH NO PERMANENT DEFORMATION OF ANY MAIN FRAME OR PANEL MEMBER IN EXCESS OF 0.4 PERCENT OF ITS SPAN AFTER THE LOAD IS REMOVED. HVHZ SHALL COMPLY WITH TAS 202. AFTER EACH SPECIFIED LOADING, THERE SHALL BE NO GLASS BREAKAGE, PERMANENT DAMAGE TO FASTENERS, HARDWARE PARTS, OR ANY OTHER DAMAGE, WHICH CAUSES THE DOOR TO BE INOPERABLE.

C. SECTIONAL GARAGE DOORS SHALL BE TESTED FOR DETERMINATION OF STRUCTURAL PERFORMANCE UNDER UNIFORM STATIC AIR PRESSURE DIFFERENCE IN ACCORDANCE WITH ANSI/DASMA 108, ASTM E330 PROCEDURE A OR TAS 202 (HVHZ SHALL COMPLY WITH TAS 202).

D. CUSTOM (ONE OF A KIND) EXTERIOR DOOR ASSEMBLIES SHALL BE TESTED BY AN APPROVED TESTING LABORATORY OR BE ENGINEERED IN ACCORDANCE WITH ACCEPTED ENGINEERING PRACTICES.

E. WINDOW AND DOOR ASSEMBLIES SHALL BE ANCHORED IN ACCORDANCE WITH THE PUBLISHED MANUFACTURER'S RECOMMENDATIONS TO ACHIEVE THE DESIGN PRESSURE SPECIFIED. SUBSTITUTE ANCHORING SYSTEM USED FOR SUBSTRATES NOT SPECIFIED BY THE FENESTRATION MANUFACTURER SHALL PROVIDE EQUAL OR GREATER ANCHORING PERFORMANCE AS DEMONSTRATED BY ACCEPTED ENGINEERING PRACTICE.

F. EXTERIOR GLAZED OPENINGS IN BUILDINGS SHALL COMPLY WITH FLORIDA BUILDING CODE 7TH EDITION (2020) BY EITHER BEING DESIGNED FOR IMPACT RESISTANCE OR BEING PROTECTED BY IMPACT PROTECTIVE SYSTEMS.

7. ALL FASTENERS DESIGNATED, AS STAINLESS STEEL SHALL CONFORM TO AISI 316 STAINLESS STEEL.

011 DELEGATED ENGINEERED PRODUCTS

1. THE GENERAL CONTRACTOR IS RESPONSIBLE TO COORDINATE THE PROPER SUBMISSION OF SPECIALTY ENGINEERED/DELEGATED DRAWINGS WHICH SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE OF FLORIDA. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ASSURE THAT THE DELEGATED ENGINEERED DRAWINGS ARE SUBMITTED IN A TIMELY MANNER SO AS TO ALLOW REVIEWS AND RESUBMISSIONS AS REQUIRED. ALL ITEMS NOTED BELOW AND SIMILAR ITEMS SHALL BE SUPPLIED AND INCLUDED IN THE CONTRACTORS BID. ALL DELEGATED ENGINEERED PRODUCTS SHALL BE DESIGNED FOR THE APPROPRIATE GRAVITY LOADS AND WIND LOADS INCLUDING UPLIFT AND LATERAL LOADS. DELEGATED ENGINEERED PRODUCTS SHALL BE, BUT ARE NOT LIMITED TO, THE FOLLOWING:

A. LIGHT GAUGE METAL, INCLUDING BUT NOT LIMITED TO, SOFFITS, CLADDING, CEILINGS, ETC.

B. MISCELLANEOUS METALS INCLUDING STEEL STAIRS, HANDRAILS AND SAFETY RAILS, MECHANICAL EQUIPMENT SUPPORTS, FRAMES THAT SUPPORT MACHINES, HANGERS, PIPES OR OTHER STRUCTURAL METAL USED FOR SUPPORT OF MECHANICAL SYSTEMS.

C. TRELLIS, ARBORS, CHANDELIERS, CABINETS, ARTWORK SUPPORTS, VIDEO OR SOUND EQUIPMENT SUPPORT, METAL FRAMES, LADDERS, RIGGING, HANGING WALLS, RAILINGS, GLAZING FRAMES, CLADDING SUCH AS STONE, PRECAST CONCRETE, ALUMINUM, METAL PANELS, CABLE BARRIER SYSTEMS, ETC. OR ANY OTHER MISCELLANEOUS PRODUCT REQUIRED BY ANY OF THE CONSTRUCTION DOCUMENTS.

D. IN ADDITION TO THE LOADS SHOWN IN THE DESIGN LOAD SCHEDULE, THE DELEGATED ENGINEER SHALL DESIGN FOR THE WEIGHT OF ALL MECHANICAL, PLUMBING AND ELECTRICAL EQUIPMENT AND FIXTURES, AS WELL AS CHANDELIER FIXTURES, BAR CABINETS, AND ARTWORK / MOBILES.

GENERAL CONTRACTOR TO INCLUDE IN THEIR BID THE COST OF THE ABOVE NOTED DELEGATED ENGINEERING.

013 SHOP DRAWINGS

1. THE SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DRAWINGS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUALITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC.

2. ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. DRAWINGS SUBMITTED WITHOUT REVIEW WILL BE RETURNED UNCHECKED.

3. IN ALL INSTANCES, THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED BY THE ENGINEER.

4. PRE-ENGINEERED ITEMS SHALL BE SUBMITTED SIGNED AND SEALED BY A SPECIALTY ENGINEER REGISTERED IN THE STATE OF FLORIDA.

5. ALL SHOP DRAWINGS SHALL BE SUBMITTED VIA ELECTRONIC FORMAT, UNLESS OTHERWISE REQUIRED FOR A SPECIFIC COMPONENT OR SYSTEM.

6. DETAILER SHALL BE RESPONSIBLE FOR CHECKING ALL ARCHITECTURAL, ELECTRICAL AND MECHANICAL DRAWINGS FOR OPENINGS AND EMBEDS, EFFECTING STRUCTURAL MEMBERS.

016 SHORING AND RESHORING

1. THE CONTRACTOR SHALL PROVIDE AND MAINTAIN SHORING, BRACING AND STRUCTURAL SUPPORTS AS REQUIRED TO PRESERVE THE STABILITY OF THE STRUCTURE DURING CONSTRUCTION. SHORING DRAWINGS ARE TO BE PREPARED AND SIGNED AND SEALED BY AN ENGINEER EXPERIENCED IN SUCH WORK AND LICENSED IN THE STATE OF FLORIDA. SUBMIT DRAWINGS TO THE SPECIAL INSPECTOR AND BUILDING OFFICIAL AS REQUIRED. SHORING AND RESHORING DESIGN AND CONSTRUCTION IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND THE ENGINEER IN HIS EMPLOY.

THE ENGINEER WHO PREPARES THE SHORING AND RESHORING DRAWINGS SHALL INSPECT THE SHORING AND RESHORING AND PROVIDE FIELD REPORTS OF EACH INSPECTION.

2. THE BRACING DETAILS OF THE EXTERIOR WALLS OF WHICH IN SOME CASES, THE ROOF DECK DIAPHRAGM AND ROOFING MEMBERS WILL BE REMOVED LEAVING THE EXTERIOR WALLS UNBRACED. IT IS THE CONTRACTOR'S RESPONSIBILITY TO HIRE A SPECIALTY SHORING AND BRACING ENGINEER TO PROVIDE THE REQUIRED DOCUMENTS FOR THIS EFFORT.

020 FOUNDATION

1. ALL SITE PREPARATION AND EXCAVATION WORK IS TO BE PERFORMED IN STRICT ACCORDANCE WITH THE:

A. RECOMMENDATIONS ON SOILS AND FOUNDATIONS INVESTIGATION PREPARED BY A FLORIDA LICENSED GEOTECHNICAL ENGINEER PRIOR TO COMMENCING ANY WORK OR ORDERING OF MATERIALS. THE GENERAL CONTRACTOR SHALL INCLUDE THE COST OF THE GEOTECHNICAL ENGINEERING AND ALL OTHER SURVEYS, TESTS, AND OTHER REQUIREMENTS OF THE GOVERNING AUTHORITIES IN THEIR BID OR ARRANGE WITH AND NOTIFY THE OWNER OF ALL SUCH COSTS.

THE GENERAL CONTRACTOR SHALL SUBMIT THE FINAL GEOTECHNICAL REPORT SIGNED AND SEALED TO THE BUILDING OFFICIAL WHEN SUBMITTING FOR PERMIT.

2. THE BUILDING SITE SHOULD BE EXCAVATED TO THE DEPTH AND EXTENT INDICATED IN THE SOILS REPORT. ALL SUBGRADES SHALL BE APPROVED IN WRITING BY THE SOILS ENGINEER PRIOR TO BACKFILLING. GENERAL CONTRACTOR SHALL SUPPLY SURVEYOR'S FLOOD PLAIN CERTIFICATION THAT THE FLOOR SLAB ELEVATION IS ABOVE THE GOVERNING AUTHORITY'S REQUIRED FLOOR ELEVATION BEFORE COMMENCING ANY WORK OR ORDERING ANY MATERIALS.

3. BOTTOM OF FOOTINGS ASSUMED TO BEAR ON SOIL CAPABLE OF SAFELY SUPPORTING 2500 PSF.

4. SOILS SUPPORTING ALL FOOTINGS MUST BE INSPECTED AND APPROVED BY A REGISTERED SOILS ENGINEER BEFORE COMMENCING WORK, ORDERING MATERIALS, OR MOVING FORWARD IN ANY WAY. APPROVAL IN WRITING MUST INDICATE THE SOIL IS ADEQUATE TO SAFELY SUSTAIN SPECIFIED SOIL BEARING PRESSURE.

5. EXCAVATION & BACKFILL:

A. ALL EXCAVATION SHALL BE KEPT DRY. DE-WATERING WILL BE REQUIRED AND SHALL BE PROVIDED BY THE CONTRACTOR. THE DE-WATERING SHALL BE PROVIDED SO ALL EXCAVATIONS ARE DRY AND THE TESTING AGENCY CAN TAKE THE APPROPRIATE DENSITY TESTS AND ALL OTHER REQUIREMENTS OF THE GEOTECHNICAL REPORT AND PROJECT CONSTRUCTION DOCUMENTS ARE MET. EXCAVATE TO DEPTHS AND DIMENSIONS INDICATED. TAKE EVERY PRECAUTION TO GUARD AGAINST ANY MOVEMENT OR SETTLEMENT OF ADJACENT STRUCTURES, UTILITIES, PIPING, ETC.

B. PROVIDE ANY BRACING OR SHORING NECESSARY TO AVOID SETTLEMENT OR DISPLACEMENT OF EXISTING FOUNDATION OR STRUCTURES.

6. CENTERLINE OF FOOTINGS: SHALL COINCIDE WITH CENTERLINE OF COLUMNS UNLESS OTHERWISE NOTED ON DRAWINGS.

7. DIMENSIONS: ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE STRUCTURAL DRAWINGS MUST BE VERIFIED AND COORDINATED WITH THE ARCHITECTURAL DRAWINGS BY THE CONTRACTOR BEFORE PROCEEDING WITH THE CONSTRUCTION. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT OR ENGINEER IN WRITING BEFORE PROCEEDING WITH ANY WORK.

030 CONCRETE

1. CONCRETE ELEMENTS TO HAVE THE FOLLOWING STRENGTHS:

A. FOUNDATIONS 3000 PSI
B. SLAB-ON-GRADE 3000 PSI
C. COLUMNS 4000 PSI
D. BEAMS 4000 PSI
E. STRUCTURAL SLABS 5000 PSI
F. MASONRY GROUT 3000 PSI

ALL OTHER CONCRETE TO BE 4000 PSI UNLESS NOTED OTHERWISE.

2. ALL CONCRETE SHALL BE READY MIX AND MEET THE FOLLOWING REQUIREMENTS:

A. A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI @ 28 DAYS

B. A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI @ 28 DAYS

C. A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI @ 28 DAYS

D. SLUMPS SHALL BE 3" MINIMUM AND 5" MAXIMUM.

E. ALL CONCRETE TO HAVE MAXIMUM WATER/CEMENT RATIO OF 0.55.

F. JOBSITE WATER SHALL NOT BE ADDED.

G. CEMENT SHALL CONFORM WITH ASTM C150 TYPE 1. SLAG, ASTM C989 SHALL BE LIMITED TO 50% (BY WEIGHT OF CEMENTITIOUS MATERIAL) AND FLY ASH, ASTM C618, CLASS F, SHALL BE LIMITED TO 25% (BY WEIGHT) OF CEMENTITIOUS MATERIAL.

3. ALL CONCRETE WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE ACI BUILDING CODE (ACI 318/ LATEST EDITION), THE ACI DETAILING MANUAL (ACI 315/ 1994 EDITION), AND THE SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301/ LATEST EDITION).

CONTRACTOR TO PROVIDE CONCRETE AND ALL OTHER CONCRETE BASED PRODUCTS THAT COMPLY WITH LOCAL, STATE AND FEDERAL REQUIREMENTS REGARDING RADON OFF GASSING.

4. CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS REQUIRED BY ACI SPECIFICATIONS. ALL EXTERIOR CONCRETE SLABS SHALL BE SLOPED TO PROVIDE ADEQUATE DRAINAGE.

5. WELDED WIRE FABRIC SHALL COMPLY WITH ASTM A1064/A1064M, UNLESS OTHERWISE SPECIFIED. PLACE FABRIC 2" CLEAR FROM TOP OF THE SLAB IN SLAB ON GRADE AND SUPPORT ON SLAB BOLSTERS SPACED AT 3'-0" O.C.

6. REQUIREMENTS:

A. REINFORCING STEEL SHALL CONFORM TO ASTM DESIGNATION A 615 GRADE 60.

B. WWF SHALL COMPLY WITH ASTM A1064/A1064M.

C. EPOXY COATED REBAR SHALL CONFORM TO ASTM A775. INCREASE SPLICE AND DEVELOPMENT LENGTHS OF EPOXY COATED BARS BY 40%.

D. GALVANIZED REBAR SHALL CONFORM TO ASTM A767.

E. PROVIDE 10 MIL VAPOR BARRIER COMPLIANT WITH ASTM E1745 BELOW ALL SLAB ON GRADE.

7. LAP ALL BARS WITH CLASS B TENSION LAP SPLICE UNLESS OTHERWISE NOTED ON DRAWINGS. LAP ALL WWF A MINIMUM OF 12 INCHES (UNLESS OTHERWISE NOTED).

8. REINFORCING BARS:

A. AT CORNERS OF CONCRETE WALLS, BEAMS AND CONTINUOUS WALL FOOTINGS, PROVIDE MATCHING HORIZONTAL BENT BARS FOR EACH HORIZONTAL BAR SCHEDULED AT EACH FACE. SEE WALL CORNER DETAIL (5'-0" MINIMAL LENGTH).

B. WHERE COLUMNS ARE AN INTEGRAL PART OF CONCRETE WALLS, WALL REINFORCEMENT SHALL BE CONTINUOUS THRU THE COLUMNS.

C. ALL HOOKS SHOWN IN REINFORCEMENT SHALL BE ACI RECOMMENDED HOOKS UNLESS OTHERWISE NOTED.

D. FOR BALCONIES, SLABS AND WALKWAYS EXPOSED TO WEATHER ALL REINFORCING STEEL (TOP AND BOTTOM) AS WELL AS SPACERS AND OTHER DEVICES FOR SPACING, SUPPORTING AND FASTENING REINFORCING SHALL BE GALVANIZED CONFORMING TO ASTM A767. BOLSTERS AND CHAIRS TO BE PLASTIC. CONCRETE PLACED IN THESE AREAS TO HAVE .40 W/C RATIO MAXIMUM AND CONTAIN 2.5 GALLONS OF CALCIUM NITRATE PER CUBIC YARD. REBAR COVER TO BE 1.5" MINIMUM.

E. ALL REBARS THAT ARE TO BE DRILLED AND FASTENED WITH ADHESIVE ANCHORS (ONLY IN AN OVERHEAD, INCLINED UPWARD OR HORIZONTAL POSITION) INTO CONCRETE, REQUIRE THE INSTALLER BE ACI CERTIFIED PER ACI 318 (LATEST EDITION). THE ALTERNATIVE IS TO PERFORM A PULL TEST ON EVERY REBAR.

F. CONTRACTOR SHALL INCLUDE IN HIS BASE BID THE COST OF 2 TONS OF ADDITIONAL REINFORCING STEEL, INCLUDING DETAILING, FABRICATION, BENDING, FURNISHING, AND PLACING. THIS EXTRA STOCK SHALL BE FURNISHED AND USED FOR SPECIAL CONDITIONS AS DIRECTED BY THE ARCHITECT, THE ARCHITECT'S AGENT OR BY THE OWNER'S CONSTRUCTION SUPERVISOR. THE PRICE OF THE UNUSED EXTRA STOCK SHALL BE CREDITED TO THE OWNER'S ACCOUNT.

9. CONCRETE LINTELS:

A. DROP BOTTOM OF BEAM OR SLAB AT WINDOWS, DOORS, ANY OVERHEAD OPENINGS, AND MASONRY OPENINGS AS REQUIRED BY ANY DRAWINGS, INCLUDING LOUVERS AND ALL OTHER WALL OPENINGS SHOWN IN THE ARCHITECTURAL OR MECHANICAL DRAWINGS. PROVIDE A CONCRETE CLOSURE BETWEEN THE BOTTOM OF THE BEAM AND REQUIRED OPENINGS OR PROVIDE A PRECAST CONCRETE LINTEL, 8F8 – 1B, BY CASTCRETE IF NOT NEXT TO A POURED CONCRETE COLUMN.

B. MAXIMUM DROP SHALL BE 24". PROVIDE 2 #5 EACH FACE @12" O.C. AND AT BOTTOM OF DROP INCLUDING #3 TIES @ 24" O.C. EXTENDING TO TOP OF BEAM REINFORCING. IF THE LINTEL EXCEEDS THE ABOVE LIMIT OF DROP, A SEPARATED LINTEL SHALL BE PROVIDED AS FOLLOWS:

L1. OPENING LESS THAN 6'0" WIDE 8" X 8" W/2 #5 BOTTOM BARS.
L2. OPENING BETWEEN 6'0" AND 12'0" WIDE 8" X 16" W/2 #6 TOP AND BOTTOM BARS AND #3 @ 6"0 O.C.

C. LINTELS TO HAVE 8" MINIMUM BEARING AT EACH END.

D. IF THE MASONRY OPENING HAS AN END ADJACENT TO A CONCRETE COLUMN PROVIDE (2) #5 OR #6 DOWELS, AS THE CASE MAY BE, IN THE CONCRETE COLUMN WITH SHEAR KEY 1-1/2 INCH DEEP BY LINTEL'S DEPTH AND WIDTH FOR ITS SUBSEQUENT CONSTRUCTION.

032 POST-TENSION CONCRETE

1. ENGAGE THE SERVICES OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA TO PREPARE AND SUBMIT COMPLETE SHOP DRAWINGS AND COMPLETE STRUCTURAL DESIGN COMPUTATIONS, INCLUDING STRESS LOSS CALCULATIONS, FOR THE WORK OF THIS SECTION. DRAWINGS SHALL BE SIGNED AND BEAR THE ENGINEER'S PROFESSIONAL SEAL. POST-TENSION SHOP DRAWINGS SHALL INCLUDE REBAR LAYOUT PLAN FROM POST-TENSION SPECIALTY ENGINEER RESULTING FROM SPECIALTY ENGINEERS CALCULATIONS.

DESIGN COMPUTATIONS SHALL INCLUDE LOADS SHOWN IN THE DESIGN LOAD SCHEDULE AND THE WEIGHT OF ALL MECHANICAL, PLUMBING, AND ELECTRICAL EQUIPMENT AND FIXTURES, AS WELL AS CHANDELIER FIXTURES, BAR CABINETS AND ART WORK.

2. PRE-STRESSING SHALL CONFORM TO THE STANDARDS OF THE ACI BUILDING CODE 318/LATEST EDITION.

3. POST TENSIONING CABLES SHALL HAVE A CORROSION PROTECTION SYSTEM, WHICH IS IN ACCORDANCE WITH THE RECOMMENDATION OF PRESTRESSED CONCRETE INSTITUTE FOR CABLES USED IN A CORROSIVE ENVIRONMENT.

4. TENDONS FOR PRE-STRESSING SHALL CONFORM TO THE ASTM SPECIFICATION A416 AND SHALL BE 270K GRADE WITH A MINIMUM ULTIMATE STRENGTH OF 270,000 PSI AND A YIELD STRENGTH OF 240,000 PSI.

5. ALL TENDONS SHALL BE UNBONDED.

6. FORCES SHOWN ON STRUCTURAL PLANS ARE EFFECTIVE AFTER ALL LOSSES.

7. ALL TENDONS SHALL HAVE PARABOLIC DRAPE (U.N.O.).

8. C.G.S. IS THE CENTER OF GRAVITY OF THE TENDONS MEASURED FROM THE MEMBER SOFFIT, SEE TYPICAL TENDONS PROFILE.

9. TENDONS REQUIRED TO PROVIDE THE EXTRA FORCE SCHEDULED FOR END SPANS ARE TO BE ANCHORED AT THE QUARTER POINT OF ADJACENT SPAN.

10. THE STRENGTH OF CONCRETE AT TRANSFER, F`CI, SHALL BE ADEQUATE FOR THE REQUIREMENT OF THE ANCHORAGES. THE MINIMUM STRENGTH AT TRANSFER SHALL BE 3000 PSI. THERE MUST BE A MINIMUM PERIOD OF 48 HOURS BETWEEN CONCRETE POURING AND CABLE TENSIONING.

11. AFTER STRESSING IS COMPLETED AND ELONGATIONS VERIFIED IN WRITING BY THE DELEGATED ENGINEER, TENDONS SHALL BE SHEARED OFF (NO BURNING ALLOWED), AND ENCAPSULATION SYSTEM INSTALLED IN ACCORDANCE WITH THE DELEGATED ENGINEERING DOCUMENTS. STRAND LENGTH PROTRUDING FROM THE WEDGES AFTER CUTTING SHALL BE AS SPECIFIED BY THE ENCAPSULATION SYSTEM MANUFACTRURER. CLEAR COVER FROM THE FACE OF THE SLAB TO THE ENCAPSULATION CAP COVERING THE TENDON END SHALL BE PER THEE MANUFACTURER'S REQUIREMENTS.

12. PROVIDE #4 HAIRPINS AS REQUIRED TO MAINTAIN HORIZONTAL CURVE IN CABLES.

13. SLAB BURSTING REINFORCING SHALL BE MINIMUM #4 TOP AND BOTTOM CONTINUOUS ALONG SLAB EDGE AS SHOWN OR LARGER IF REQUIRED BY POST-TENSIONING SUPPLIER'S SYSTEM.

14. PROVIDE #4 TOP AND BOTTOM CONTINUOUS, EXTERIOR CRACK CONTROL BARS AT ALL SLAB EDGES.

15. CONSTRUCTION JOINTS IN POST-TENSIONED MEMBERS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER, PRIOR TO CONSTRUCTION.

16. REBAR SHOP DRAWINGS, SHEAR HEAD AND SHEAR RAIL SHOP DRAWINGS SHALL BE SUBMITTED CONCURRENTLY WITH POST-TENSION SHOP DRAWINGS AND CALCULATIONS.

17. POST-TENSIONED CABLE ENDS TO BE MECHANICALLY CUT. TORCHING NOT PERMITTED. POST-TENSIONED STRAND LENGTHS PROTRUDING FROM THE WEDGES AFTER CUTTING SHALL BE AS SPECIFIED BY THE ENCAPSULATION SYSTEM MANUFACTRER. CLEAR COVER FROM THE FACE OF THE SLAB TO THE ENCAPSULATION CAP COVERING THE TENDON END SHALL BE PER THE MANUFACTURER'S REQUIREMENTS.

18 ALSO, GC TO COORDINATE ALL SLEEVE OPENINGS IN SLABS WITH THE MEP AND ARCHITECTURAL DRAWINGS, SLEEVES TO BE INDICATED ON THE POST-TENSIONED DELEGATED ENGINEER DRAWINGS.

040 MASONRY

1. MASONRY UNITS SHALL BE

A. LOAD BEARING ASTM C90
B. TYPE II NON-MOISTURE CONTROLLED
C. NORMAL WEIGHT
D. ALL CMU SHALL BE LAID IN A FULL BED OF MORTAR IN RUNNING BOND (U.N.O.).

2. THE COMPRESSIVE STRENGTH OF MASONRY (F'M) SHALL BE 2,000 PSI AS CALCULATED IN ACCORDANCE WITH ASTM C1314 WITH TYPE M OR S MORTAR AS REQUIRED.

3. ALL MORTAR SHALL BE IN ACCORDANCE WITH ASTM SPECIFICATION C270.

4. GROUT SHALL BE A HIGH SLUMP MIX

A. IN ACCORDANCE WITH ASTM SPECIFICATION C476

B. HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI

5. ALL CONCRETE MASONRY BEARING AND SHEAR WALLS SHALL BE

A. CONSTRUCTED

IN ACCORDANCE WITH THE "BUILDING CODE REQUIREMENT FOR MASONRY STRUCTURES" (ACI 530/ASCE 5/TSM 402) AND "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TSM 602)/ LATEST EDITIONS.

6. PROVIDE 8" X 8" MASONRY BEAM WITH 2 #5 CONT. AT EVERY WINDOW SILL. EXTEND BEAM 8" BEYOND EDGE OF OPENING.

7. ALL BRICK MASONRY UNITS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 8,000 PSI AND BONDED TOGETHER WITH TYPE S MORTAR.

8. PROVIDE HOT DIPPED GALVANIZED LADDER TYPE HORIZONTAL JOINT REINFORCEMENT (9 GA.) AT 16" ON CENTER VERTICAL IN ALL MASONRY WALLS. PROVIDE DOVE TAIL SLOT ANCHORS AT CONCRETE COLUMNS.

FOR JOINT REINFORCEMENT, WALL TIES, ANCHORS AND INSERTS, APPLY A MINIMUM COAT OF 1.5 OUNCES PER SQUARE FOOT (PSF) (458/G/M2) COMPLY WITH THE REQUIREMENTS OF ASTM A153, CLASS B.

9. PROVIDE CONTROL JOINTS IN MASONRY WALLS AT A SPACING OF 25' +/- O.C. AND ALIGN WITH ARCHITECTURAL CONTROL JOINTS.

10. EPOXY GROUT SHALL BE NON-SHRINK HIGH CREEP RESISTANT, AND SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE PROPERTIES:
TENSILE STRENGTH, ASTM C 307: 1,500 PSI
FLEXURAL STRENGTH, ASTM C 580: 4,000 PSI
COMPRESSIVE STRENGTH, ASTM C 579: 1,600 PSI/7 DAYS.

11. MINIMUM LAP SPLICES FOR REINFORCED CMU AS FOLLOWS:

BAR SIZE (48DB)				
#4	#5	#6	#7	#8
24"	30"	36"	42"	48"

A. LAP SPLICES SHALL OCCUR DIRECTLY ABOVE FOOTINGS AND SLABS. NO SPLICES ARE ALLOWED AT MID-HEIGHT OF WALL.

B. LAP SPLICE THAT OCCUR AT MID HEIGHT OF WALL SHALL BE INCREASED BY 50% TO 72 BAR DIAMETERS.

C. LAP SPLICES THAT OCCUR AT CANTILEVERED WALLS SUCH AS: PARAPETS, RETAINING WALLS, ETC. SHALL HAVE LAP SPLICE LENGTHS INCREASED BY 50% TO 72 BAR DIAMETERS.

12. MASONRY LINTELS:

A. A PRECAST CONCRETE LINTEL, 8F8 – 1B BY CASTCRETE OR EQUAL MAY BE PROVIDED OVER MASONRY WALL OPENINGS UNLESS A CAST-IN-PLACE LINTEL IS REQUIRED IN THE CONCRETE LINTEL NOTES. THE LINTEL SHALL BE FULLY GROUTED.

B. LINTELS TO HAVE 4" MINIMUM BEARING AT EACH END.

C. SHORE PRECAST LINTEL PER MANUFACTURER'S INSTRUCTIONS.

13. MASONRY JAMBS

A. ADJACENT TO ANY EXTERIOR/INTERIOR WALL OPENING, PLACE (1) MATCHING VERTICAL BARS IN CELLS GROUTED SOLID FULL HEIGHT UNLESS NOTED OTHERWISE PER THE DRAWINGS.

B. AT ENDS, CORNERS, AND INTERSECTION OF WALLS PLACE (1) MATCHING VERTICAL IN CELL GROUTED SOLID FULL HEIGHT. U.N.O.

050 STEEL

1. ALL STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST AISC CODE. STRUCTURAL STEEL SHALL CONFORM TO:

A. ASTM SPECIFICATION A 992 GRADE 50 FOR ALL WIDE FLANGE BEAMS.

B. ASTM SPECIFICATION A 36 FOR MISCELLANEOUS STEEL SHAPES (ANGLES, PLATES, ETC.).

C. SQUARE OR RECTANGULAR HSS SHALL CONFORM TO ASTM SPECIFICATION A 500 GRADE B (FY=46 KSI).

D. ALL STEEL TO HAVE A SHOP COAT OF RUST INHIBITIVE PAINT.

E. DELETE PAINT ON ALL STEEL TO RECEIVE SPRAYED ON FIREPROOFING OR CONCRETE ENCASEMENT.

F. ALL MILL CAMBER TO BE ORIENTED UPWARD DURING FABRICATION AND ERECTION.

G. STEEL BEAMS INSTALLED IN PARALLEL WITH STEEL BAR JOISTS MUST HAVE CAMBER EQUAL TO BAR JOISTS.

H. ALL EXTERIOR STRUCTURAL STEEL SHALL BE HOT DIPPED GALVANIZED.

2. ALL SHOP AND FIELD WELDING SHALL BE PERFORMED BY WELDERS QUALIFIED, AS DESCRIBED IN "AMERICAN WELDING SOCIETY'S STANDARD QUALIFICATION PROCEDURE" (AWS D1.1), TO PERFORM THE TYPE OF WORK REQUIRED.

3. ALL CONNECTIONS SHALL BE BOLTED WITH 3/4" DIAMETER, A-325 HIGH STRENGTH BOLTS OR WELDED (UNLESS SHOWN OTHERWISE ON THE DRAWINGS).

A. FULL DEPTH DOUBLE CLIP ANGLE CONNECTIONS ARE TO BE USED ON ALL GIRDER AND BEAM CONNECTIONS TO COLUMNS. BOLTS TO BE AT 3-INCH O/C VERTICAL.

B. ALL CONNECTIONS TO HOLLOW STRUCTURAL SECTION (HSS) COLUMNS ARE TO BE THRU-PLATE UNLESS NOTED OTHERWISE.

C. ALL CONNECTIONS SHALL BE DOUBLE ANGLES UNLESS NOTED OTHERWISE.

4. ALL ALUMINUM AND STEEL MEMBERS TO BE TREATED OR PROPERLY SEPARATED TO PREVENT GALVANIC AND CORROSIVE EFFECTS.

5. ALL STEEL WELDING RODS SHALL BE E70XX ELECTRODES.

6. SUBMIT ALL STEEL SHOP DRAWINGS FOR APPROVAL PRIOR TO ANY FABRICATION.

7. FULL DEPTH DOUBLE ANGLE END CONNECTIONS ARE TO BE USED ON ALL COMPOSITE BEAMS AND GIRDERS WITH SHEAR STUDS. CONNECTIONS ARE TO BE DESIGNED FOR 150% OF AISC TABLE VALUES.

8. SHEAR CONNECTORS TO BE HEADED STUD TYPE, AWS D1.1-TYPE B FU=65KSI, COLD FINISHED CARBON STEEL, WITH DIMENSIONS COMPLYING WITH AISC SPECIFICATIONS.

9. STAINLESS STEEL SHALL CONFORM TO THE LATEST AISC CODE. STAINLESS STEEL ROD MATERIAL SHALL MEET THE REQUIREMENTS OF ASTM F593 (AISI 304). STAINLESS STEEL WASHERS SHALL MEET ANSI B18.22.1/LATEST EDITION, TYPE A PLAIN, REQUIREMENTS. STAINLESS STEEL NUTS SHALL MEET ASTM F594 REQUIREMENTS.

10. EQUIPMENT SUPPORTS:

PROVIDE ALL SUPPORTING STEEL NOT INDICATED ON PLAN AS REQUIRED FOR THE INSTALLATION OF MECHANICAL EQUIPMENT AND MATERIALS, INCLUDING ANGLES, CHANNELS, BEAMS, HANGERS, ETC. DO NOT SUPPORT EQUIPMENT OR PIPING FROM METAL DECKING.

11. DECK SUPPORTS:

PROVIDE 1/4" BENT PLATES AT ALL HIPS, VALLEYS, SKEWED BEAMS AND OTHER AREAS FOR DECK SUPPORT.

12. CONTRACTOR SHALL INCLUDE IN HIS BASE BID THE COST OF 2000 POUNDS OF ADDITIONAL STRUCTURAL STEEL (MEMBERS OVER 10 LBS. PER FOOT) AND 2000 POUNDS OF MISCELLANEOUS STEEL (MEMBERS 10 LBS. PER FOOT OR LESS) INCLUDING FURNISHING, DETAILING, FABRICATION, CLEANING, PAINTING, DELIVERY AND ERECTION. THIS EXTRA STOCK SHALL BE FURNISHED AND USED FOR SPECIAL CONDITIONS AS DIRECTED BY THE ARCHITECT, THE ARCHITECT'S AGENT OR BY THE OWNER'S CONSTRUCTION SUPERVISOR. THE PRICE OF THE UNUSED EXTRA STOCK SHALL BE CREDITED TO THE OWNER'S ACCOUNT.

054 JOISTS

1. ALL JOISTS SHALL HAVE A SHOP COAT OF RUST INHIBITIVE NON BITUMINOUS PAINT.

2. JOIST FABRICATOR SHALL HAVE A SPECIALTY ENGINEER REGISTERED IN THE STATE OF FLORIDA SIGN AND SEAL ALL STEEL JOIST SHOP DRAWINGS. THESE SHOP DRAWINGS SHALL CONTAIN A STATEMENT CERTIFYING THAT THE STEEL JOISTS CAN SAFELY RESIST THE WIND UPLIFT FORCES AS NOTED.

IN ADDITION TO THE LOADS SHOWN IN THE DESIGN LOAD SCHEDULE, THE SPECIALTY ENGINEER SHALL DESIGN FOR THE WEIGHT OF ALL MECHANICAL, PLUMBING AND ELECTRICAL EQUIPMENT AND FIXTURES, AS WELL AS CHANDELIER FIXTURES, BAR CABINETS, AND ART WORK / MOBILES.

3. STEEL JOISTS SHALL BE DESIGNED, FABRICATED AND ERECTED TO THE REQUIREMENTS OF THE SPECIFICATIONS OF THE STEEL JOIST INSTITUTE FOR:

A. SERIES K JOISTS

4. JOIST MANUFACTURER MUST DESIGN BRIDGING ASSUMING NO BRACING IS GIVEN BY THE ROOF DECK.

055 DECK

1. STEEL ROOF DECK SHALL BE:

A. 1-1/2", 20 GA. TYPE B METAL DECK, GALVANIZED, WITH MINIMAL COATING CLASS OF G90 AS SHOWN ON ROOF PLAN AS MANUFACTURED BY VULCRAFT/NUCOR OR APPROVED EQUAL. MANUFACTURER SHALL BE A MEMBER OF THE STEEL DECK INSTITUTE. ROOF DECK MUST COMPLY WITH STEEL DECK INSTITUTE STANDARDS. ALL ROOF DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS. ROOF DECK WITH LIGHTWEIGHT INSULATING CONCRETE SHALL BE VENTED.

2. ALL ROOF DECK TO BE DESIGNED, MANUFACTURED, AND INSTALLED IN ACCORDANCE WITH LATEST FACTORY MUTUAL STANDARDS.

3. WELDING WASHERS ARE TO BE USED ON ALL CONNECTIONS OF STEEL DECK WITH METAL THICKNESS LESS THAN 22 GA. TO STRUCTURAL STEEL SUPPORTS.

4. IN AREAS OF WARPED ROOF DECK, SELF DRILLING SCREWS ARE TO BE USED ON CONNECTIONS OF STEEL ROOF DECK TO STRUCTURAL STEEL SUPPORTS. SCREW SIZES TO COMPLY WITH MANUFACTURER'S AND FACTORY MUTUAL REQUIREMENTS. ATTACH DECK TO ALL SUPPORTING ROOF JOISTS.

5. STEEL ROOF AND FLOOR DECK 20 GA OR THINNER SHALL BE GALVANIZED (G90) PER ASTM A653.

BEAM SCHEDULE

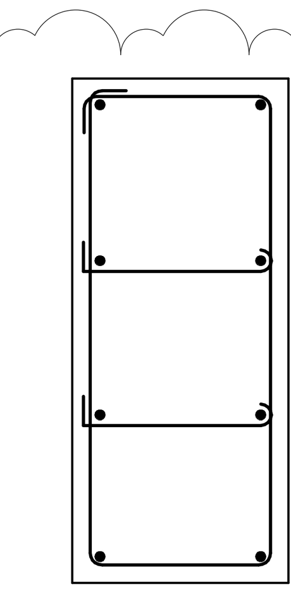
MARK	SIZE BxH (inches)	REINFORCING			STIRRUPS	
		BOTT	TOP	INT	TIES	SPACING
B-1	16 x 16	2 #7	2 #7	2 #7	#3	@ 9" o.c.
B-2	8 x 68	4 #5*	2 #5	6 #5**	#3	@ 12" o.c.
B-3	8 x 33 1/2	2 #6	2 #5	2 #5	#3	@ 12" o.c.
BB-1	8 x 8			2 #5		
TB-1	8 x 16	2 #6	2 #5	-	#3	@ 5" o.c. OVER OPNGS. & @ 12" o.c. ELSEWHERE
TB-2	8 x 24	2 #6	2 #5	2 #5	#3	@ 10" o.c. OVER OPNGS. & @ 12" o.c. ELSEWHERE

3

NOTES:
1. * DENOTES PLACE REINFORCEMENT IN (2) LAYERS.
2. ** DENOTES PLACE REINFORCEMENT IN (3) LAYERS.

CONCRETE COLUMN SCHEDULE

MARK	SIZE (inches)	VERT. REINF.	TIES	SPACING	TYPE
C-1	8 x 24	8 #5	#3	@ 8" o.c.	A



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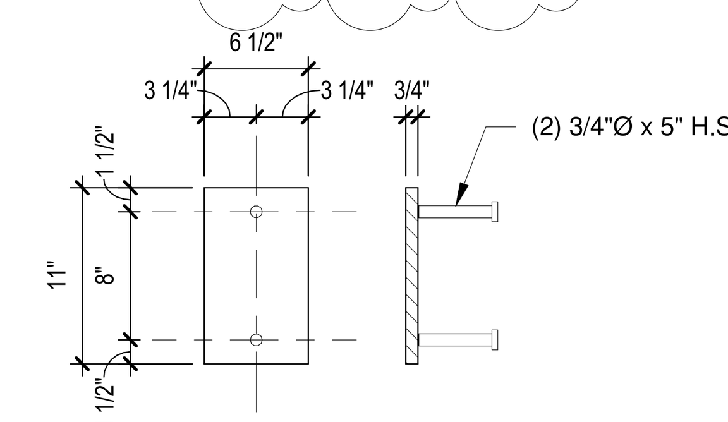

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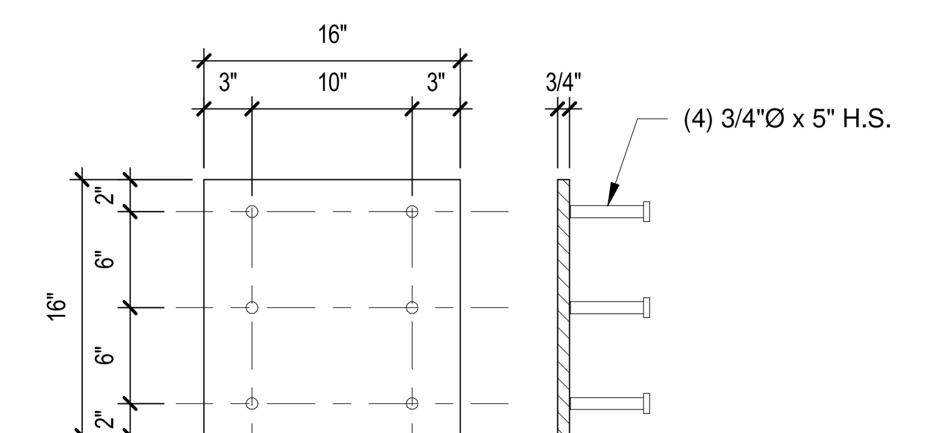

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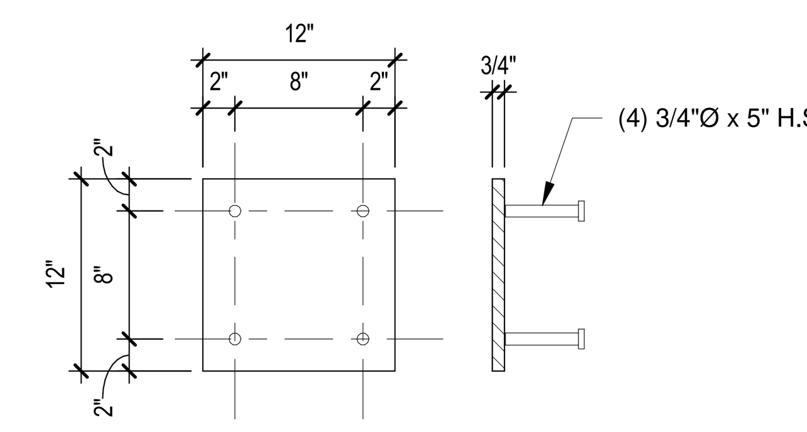

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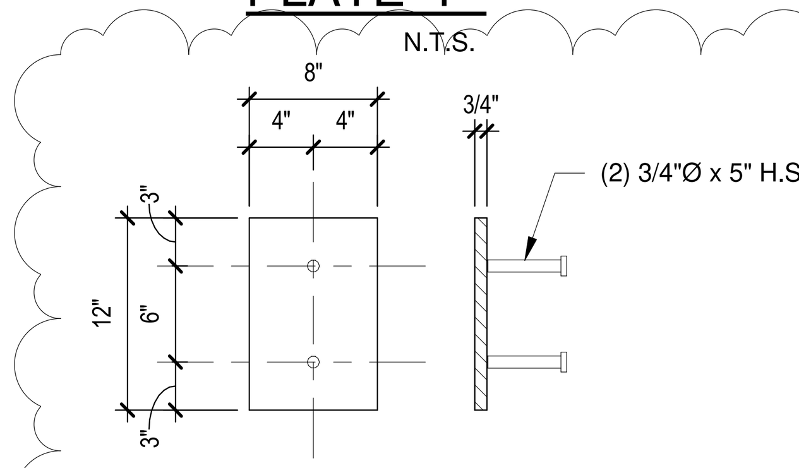


PLATE "J"
N.T.S.

3

FOOTING SCHEDULE

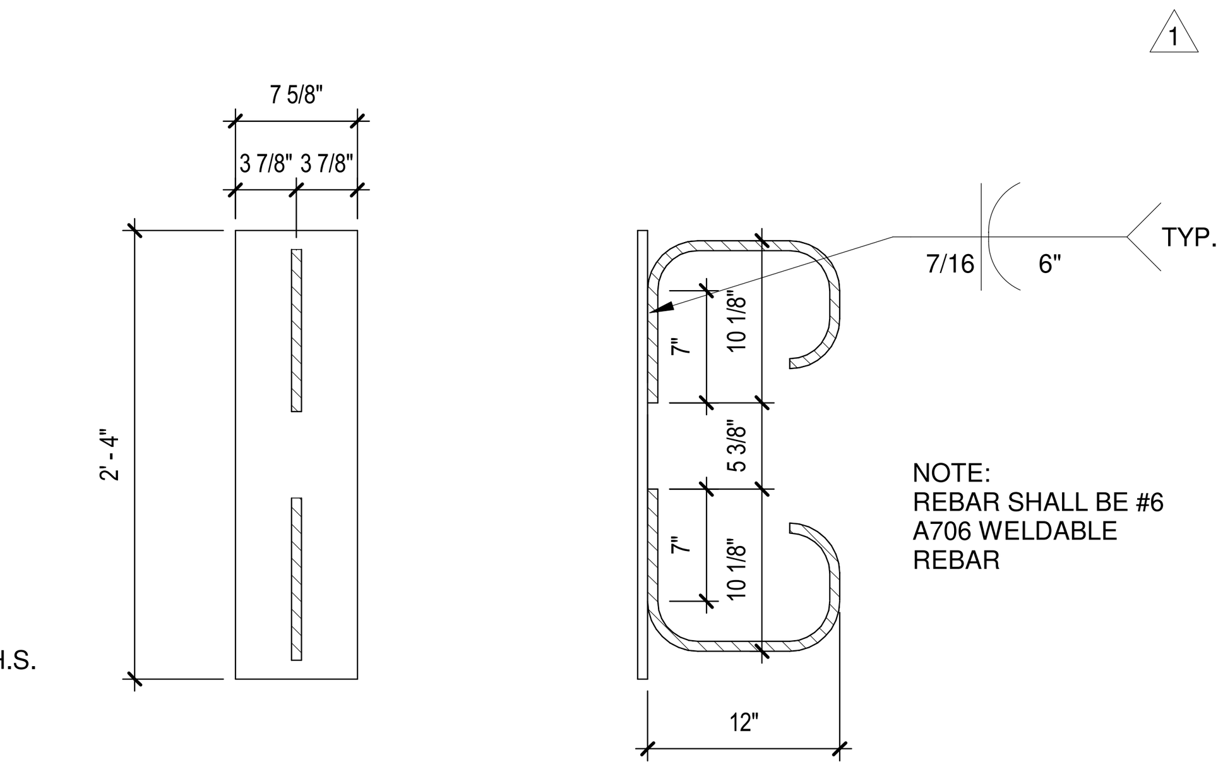
MARK	SIZE	REINFORCING
F20.12	2'-0" x CONT. x 12"	(3) #5 X CONT. & #5 @ 14" o.c. TRANSV.
F40	4'-0" x 4'-0" x 12"	4 #5 E.W. BOTT.
F50	5'-0" x 5'-0" x 12"	5 #5 E.W. BOTT.
F55	5'-6" x 5'-6" x 12"	6 #5 E.W. BOTT.
F60	6'-0" x 6'-0" x 12"	6 #5 E.W. BOTT.
F65	6'-6" x 6'-6" x 12"	7 #5 E.W. BOTT.
F70	7'-0" x 7'-0" x 12"	7 #5 E.W. BOTT.
F85	8'-6" x 8'-6" x 12"	9 #5 E.W. BOTT.
F6560	6'-6" x 6'-0" x 12"	7 #5 E.W. BOTT.
F7065	7'-0" x 6'-6" x 12"	7 #5 E.W. BOTT.
F180100	18'-0" x 10'-0" x 20"	#7 @ 12" o.c. E.W. BOTT.

1

STEEL COLUMN SCHEDULE

MARK	SIZE	BASE PL (Inches)	A.B.	REMARK
SC-1	W10X45	15x15x3/4"	(4) 3/4"Ø x 7"	PLATE F
SC-2	W12X72	18x16x3/4"	(4) 3/4"Ø x 7"	PLATE R
SC-3	W8X31	12x14x3/4"	(4) 3/4"Ø x 7"	PLATE F
SC-4	HSS8x8x3/16"	14x14x3/4"	(4) 3/4"Ø x 7"	PLATE F
SC-6	HSS6x6x1/2"	12x12x3/4"	(4) 3/4"Ø x 7"	PLATE F

1



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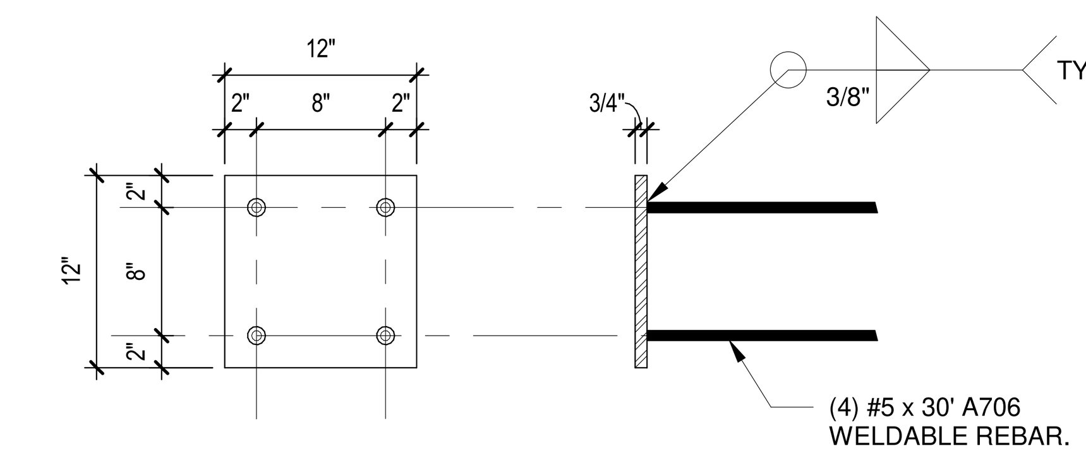

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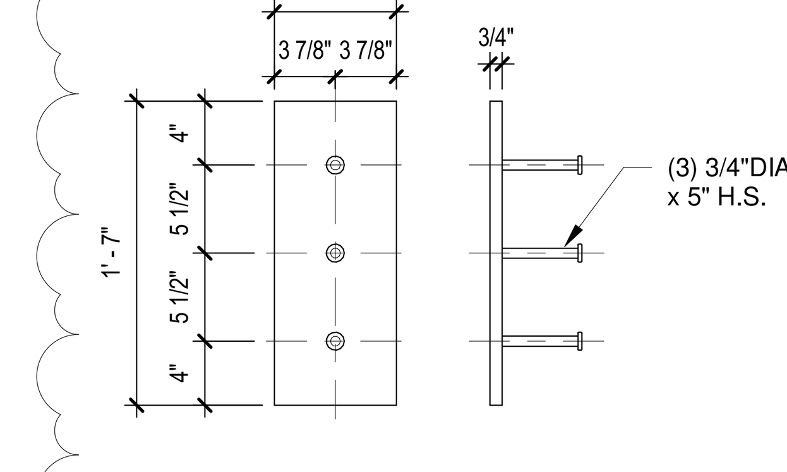

PLATE "R1"
N.T.S.

3

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ATLANTIC FIELDS - GOLF HOUSE

PERMIT SET

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Seal:

Structural Notes & Schedules

Revisions:

No.	Description	Date
1	Bldg. Dept. Comments	12.15.2023
3	Addendum #1	03.22.2024

Comm:	Date: 11.01.2023
Drawn By: CJB	Checked: PJR

Sheet:
S2.01

PHILIP J. RIZZO LICENSE No. 60998 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/22/2024

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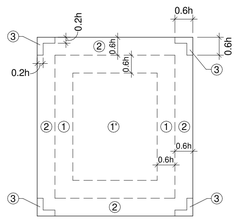
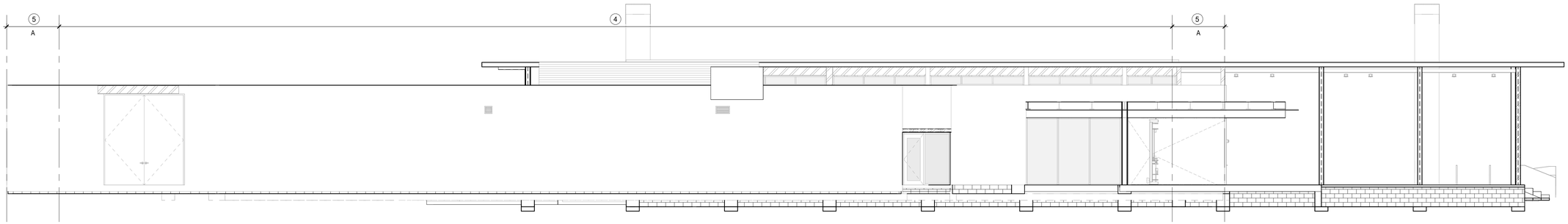
ALLOWABLE					
COMPONENT & CLADDING WIND DESIGN PRESSURES					
PRESSURES BASED ON V_{ASD}	ROOF WIND LOADS				WALL WIND LOADS (SEE NOTE 1)	
	ROOF AREA (10 SF)				WALL AREA (10 SF)	
Kd IS INCLUDED	1	1'	2	3	4	5
PRESSURE (PSF)	17	17	17	17	37	37
SUCTION (PSF)	-64	-37	-85	-116	-40	-50

1. EXTERIOR GLAZED OPENINGS IN BUILDINGS SHALL COMPLY WITH FLORIDA BUILDING CODE 7TH EDITION (2020) BY EITHER BEING DESIGNED FOR IMPACT RESISTANCE OR BEING PROTECTED BY IMPACT PROTECTIVE SYSTEMS.
2. REFER TO STRUCTURAL NOTES FOR ALL WIND LOAD PARAMETERS.
3. CORNER DISTANCE, A = 8 FEET
   .6h = 12 FEET
   .2h = 4 FEET

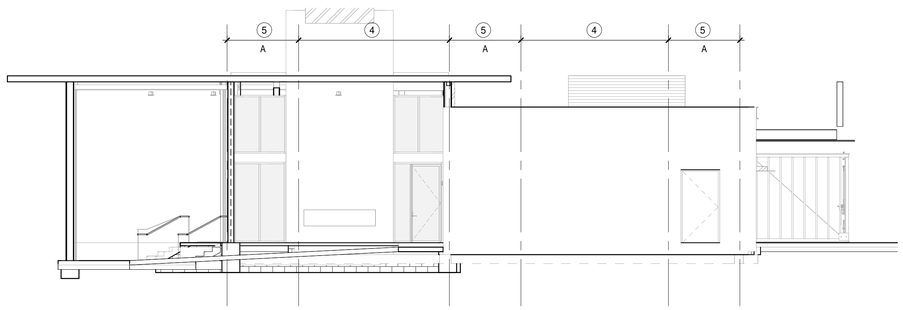


PLAN

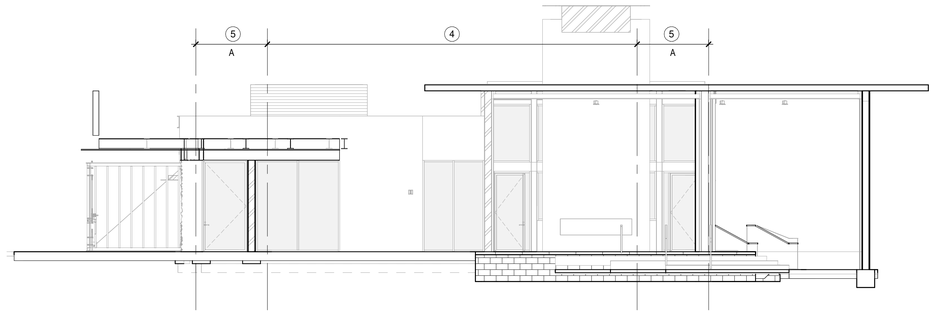
3 South Elevation
1/8" = 1'-0"



2 North
1/8" = 1'-0"



1 East Elevation
1/8" = 1'-0"



4 West Elevation
1/8" = 1'-0"

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ATLANTIC FIELDS
GOLF & BEACH CLUB

ATLANTIC FIELDS - GOLF HOUSE

PERMIT SET

2645 SE BRIDGE ROAD, HOBE SOUND, FL 33455

Seal:

Wind Loads

Revisions:

No.	Description	Date

Comm:	Date: 11.01.2023
Drawn By: CJB	Checked: PJR

Sheet:

S2.02

PHILIP J. RIZZO — No. 60998 — STATE OF FLORIDA — PROFESSIONAL ENGINEER

03/22/2024

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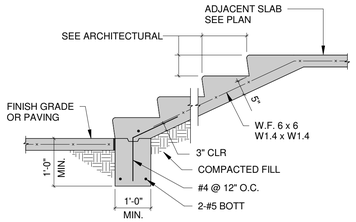
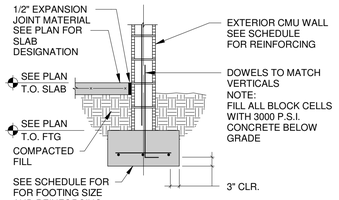


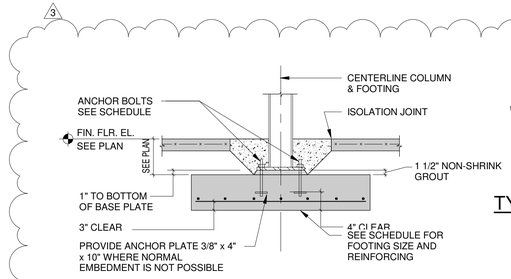
TYPICAL CONCRETE STAIR ON GRADE

N.T.S.



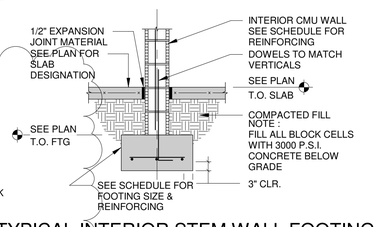
TYPICAL EXTERIOR WALL FOOTING

N.T.S.



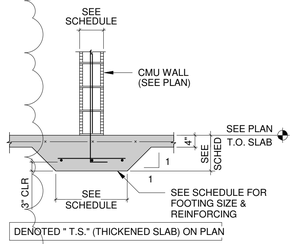
TYPICAL STEEL COLUMN FOOTING

N.T.S.



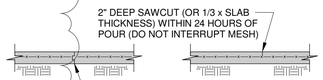
TYPICAL INTERIOR STEM WALL FOOTING

N.T.S.



TYPICAL INTERIOR MONOLITHIC WALL FOOTING
(NON-BEARING)

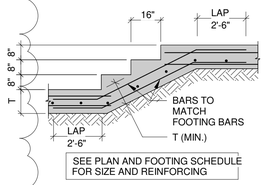
N.T.S.



TYPICAL SLAB-ON-GRADE

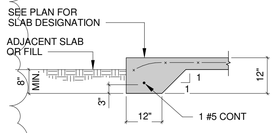
N.T.S.

NOTE:
CONTROL JOINTS/CONSTRUCTION JOINTS SHALL CREATE PANELS OF 144 SQ. FEET (MAXIMUM) AND THE LENGTH TO WIDTH RATIO OF THE JOINT SPACING SHALL NOT EXCEED 1.25.



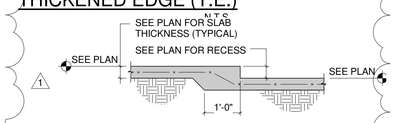
TYPICAL STEP FOOTING DETAIL

N.T.S.



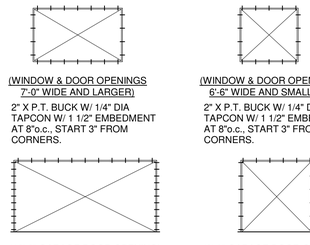
THICKENED EDGE (T.E.)

N.T.S.



TYPICAL SLAB RECESS

N.T.S.

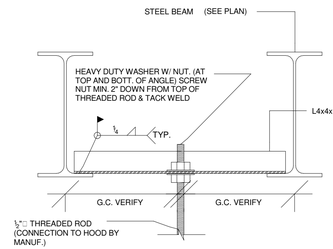


BUCKS TO BE FASTENED HORIZONTALLY AND VERTICALLY TO CONCRETE BEAMS AND COLUMNS OR CONCRETE FILLED MASONRY.

G.C. TO COORDINATE OPENING DIMENSIONS.

TYPICAL WOOD BUCK TO CONCRETE CONNECTION DETAIL

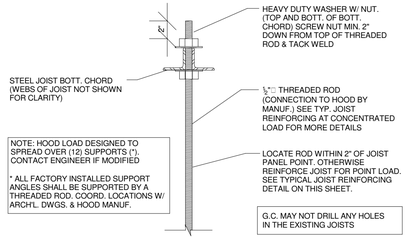
1. EXTERIOR GLAZED OPENINGS IN BUILDINGS SHALL COMPLY WITH 2020 FLORIDA BUILDING CODE BY EITHER BEING DESIGNED FOR IMPACT RESISTANCE OR BEING PROTECTED BY IMPACT PROTECTIVE SYSTEMS.



SUPPORT LOCATED BETWEEN STEEL BEAMS

TYPICAL HANGING HOOD DETAILS

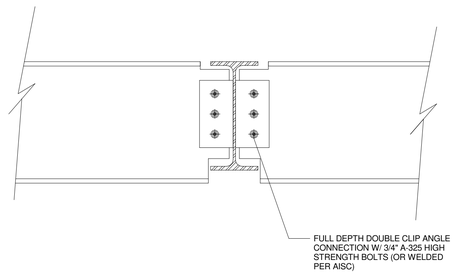
N.T.S.

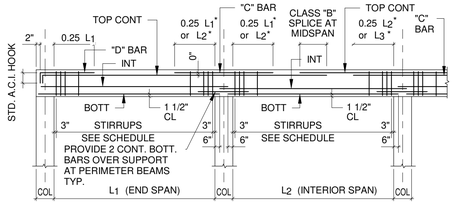


SUPPORT LOCATED AT STEEL JOIST

TYPICAL HANGING HOOD DETAILS

N.T.S.

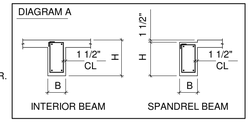




"C" BAR : (IF NOTED IN SCHED.)	TOP BAR AT INTERIOR SUPPORT (IN ADDITION TO TOP CONT BARS) PLACE IN SAME LAYER AS TOP CONT BARS (U.O.N.). LOCATE AT RIGHT SUPPORT OF SPAN INDICATED IN SCHEDULE.
"D" BAR : (IF NOTED IN SCHED.)	TOP BAR AT EXTERIOR SUPPORT (IN ADDITION TO TOP CONT BARS) PLACE IN SAME LAYER AS TOP CONT BARS (U.O.N.).
"INT" BARS :	INTERMEDIATE BARS LOCATED AT A SPACING EQUAL TO THE WIDTH OF THE BEAM BUT NOT GREATER THAN 12" ABOVE BOTT BARS. IF MORE THAN ONE PAIR, PLACE IN LAYERS OF TWO.

CLASS "B" TENSION SPLICE (3000 PSI)	#4 38"	#7 82"
	#5 47"	#8 93"
	#6 56" 105"	#9
(*)	WHICHEVER IS GREATER.	

DIAGRAM A



NOTES :

WHEN ADJACENT BEAMS OR TIE BEAMS HAVE TOP CONT BARS OF DIFFERENT SIZE, THE TRANSITION SHOULD BE MADE AT MIDSPAN OF THE BEAM WITH SMALLER SCHEDULED BARS. USE LAP SPLICE LENGTH OF SMALLER SIZE BAR.

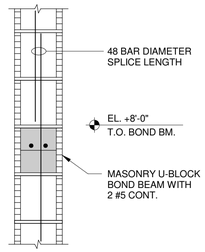
(2L) - INDICATES BARS PLACE IN TWO LAYERS. WHERE BARS ARE PLACED IN TWO LAYERS, THE SECOND LAYER BARS MUST BE PLACED DIRECTLY UNDER BARS IN THE FIRST LAYER (IF TOP BAR) OR DIRECTLY OVER BAR IN THE FIRST LAYER (IF BOTT BAR). PROVIDE 1" CLEAR DISTANCE BETWEEN LAYERS OR ONE BAR DIAMETER, WHICHEVER IS THE GREATER DISTANCE.

SCHEDULED BEAM SIZES : [SEE DIAGRAM A]

"B" INDICATES BEAM WIDTH DIMENSION. WHEN BEAM IS OVER A BLOCK WALL, USE ACTUAL BLOCK WIDTH (7 5/8" or 11 5/8").
"H" INDICATES BEAM DEPTH DIMENSION. LESS 1 1/2" FOR RECESS FOR BLOCK WALL DEDUCTED WHERE APPLICABLE, OR MINIMUM DEPTH IN A VARIABLE DEPTH BEAM. COORDINATE BEAM CONFIGURATION WITH ARCHITECTURAL DRAWINGS.

TYPICAL BEAM BAR PLACEMENT DIAGRAM

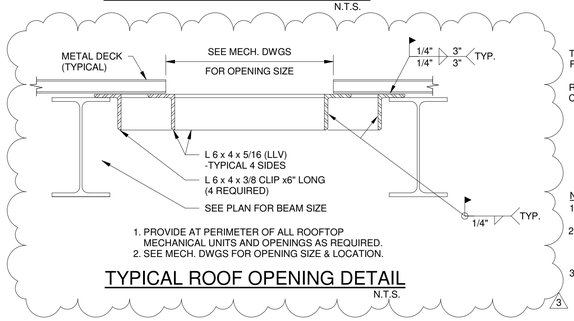
N.T.S.



NOTES:
1. WHERE HEIGHT OF MASONRY WALL SEGMENT EXCEEDS 12'-0", INTERMEDIATE BOND BEAM IS REQUIRED.
2. BOND BEAM REINFORCEMENT MUST BE CONT. HOOK BARS INTO REINFORCED JAMB AT WALL OPENINGS.
3. OMIT BOND BEAMS WHERE IT COINCIDES WITH OPENING HEADERS (LINTELS/BEAMS). REINFORCEMENT MUST BE CONTINUOUS AT SUCH LOCATIONS.

TYPICAL BOND BEAM DETAIL

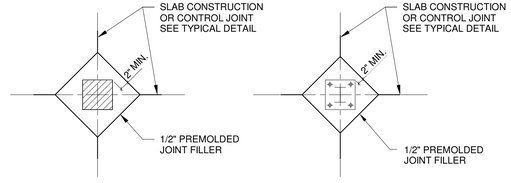
N.T.S.



1. PROVIDE AT PERIMETER OF ALL ROOFTOP MECHANICAL UNITS AND OPENINGS AS REQUIRED.
2. SEE MECH. DWGS FOR OPENING SIZE & LOCATION.

TYPICAL ROOF OPENING DETAIL

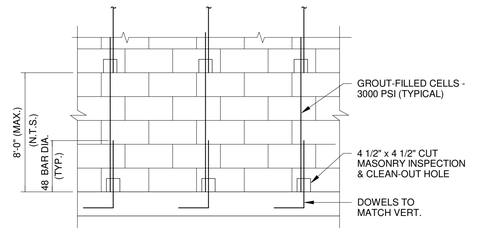
N.T.S.



AT CONCRETE COLUMN AT STEEL COLUMN

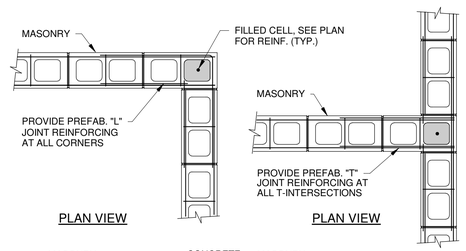
TYPICAL ISOLATION JOINTS AT COLUMNS

N.T.S.



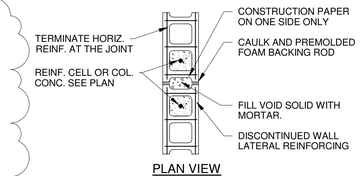
TYPICAL MASONRY FILLED CELL DETAIL

N.T.S.



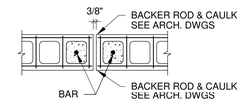
TYPICAL MASONRY DETAILS

N.T.S.



PLAN VIEW

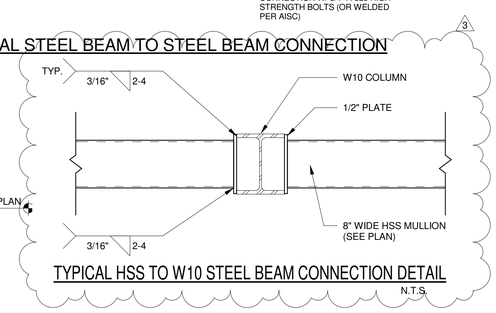
NOTES :
1.- SAW CUT BOND BEAMS, TIE BEAMS 1" DEEP TO CONTINUE WALL CONTROL JOINT TO TOP OF WALL.
2.- CONTROL JOINT SPACING IS NOT TO EXCEED 25'-0"o.c. IN WALLS WITH MORE THAN 25'-0" OF UNINTERRUPTED MASONRY. REFER TO DWG'S. FOR ADDITIONAL
3.- SPECIFIED LOCATIONS AS NOTED THUS (WCJ).. THROUGH THE JOINT.



ALTERNATE METHOD

CMU WALL CONTROL JOINT (WCJ) DETAIL

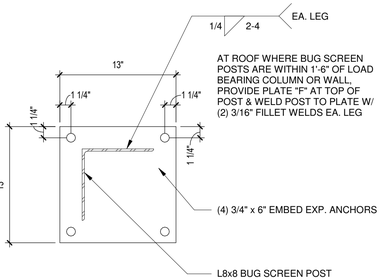
N.T.S.



TYPICAL STEEL BEAM TO STEEL BEAM CONNECTION

TYPICAL HSS TO W10 STEEL BEAM CONNECTION DETAIL

N.T.S.



2 8x8 L COLUMN BASE PLATE DETAIL
1 1/2" = 1'-0"

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03/22/2024

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2645 SE BRIDGE ROAD, HOBE SOUND, FL 33455

Seal:

Typical Details

Revisions:

No.	Description	Date
1	Bldg. Dept. Comments	12.15.2023
3	Addendum #1	03.22.2024

Comm:	Date: 11.01.2023
Drawn By: CJB	Checked: PJR

Sheet:

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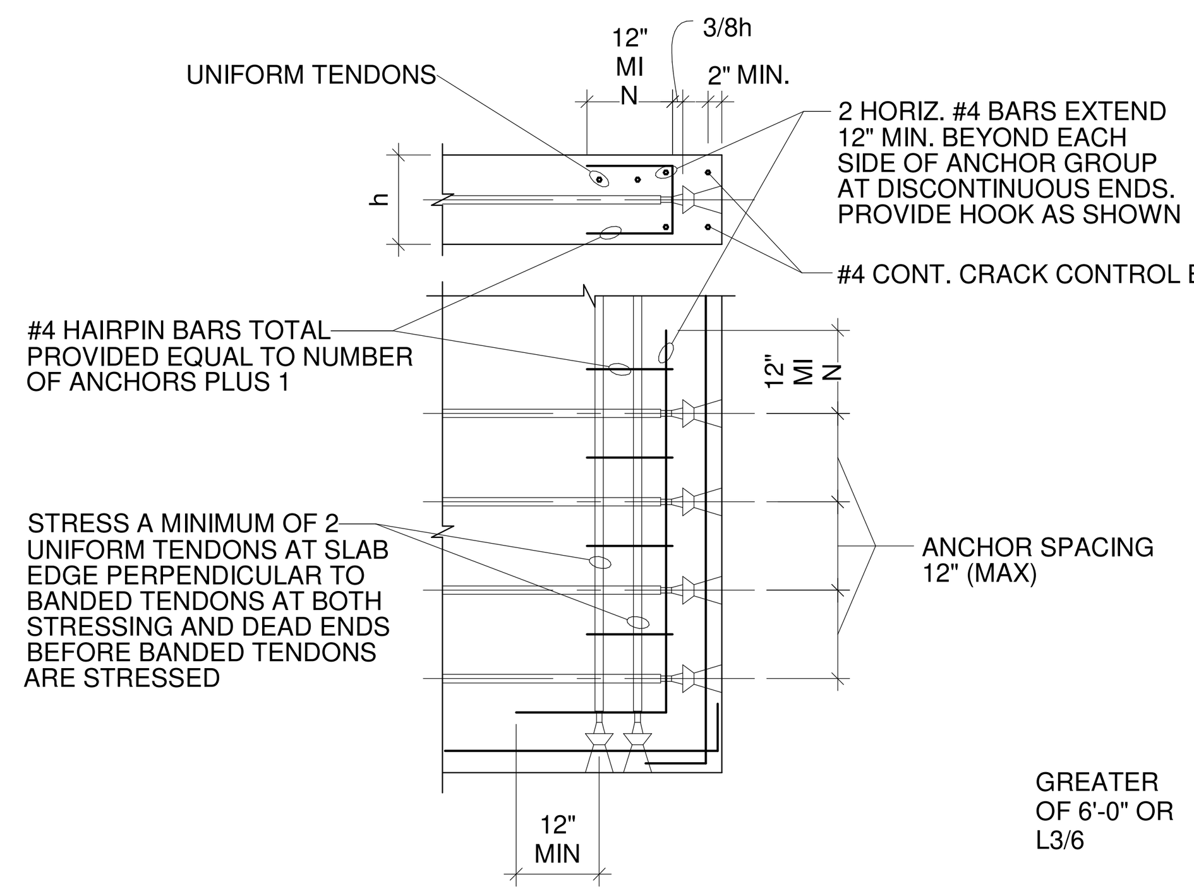
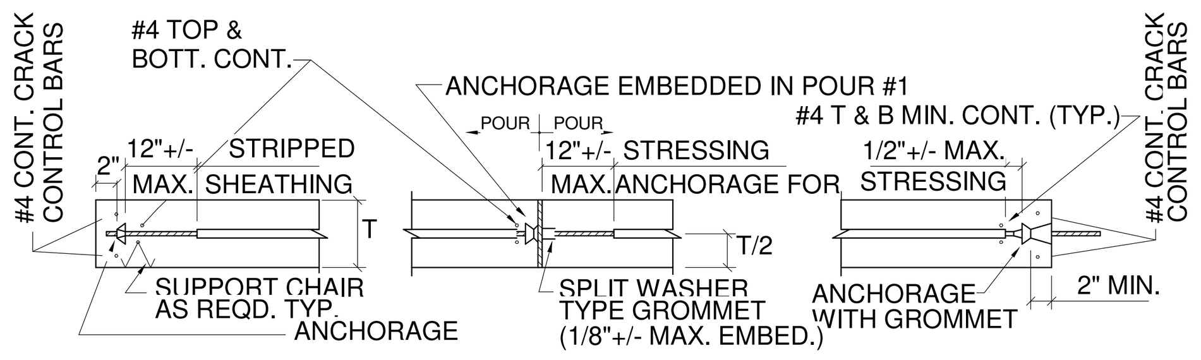


TYPICAL ANCHOR ZONE REINFORCEMENT FOR SIX OR MORE TENDONS ANCHORAGES IN SLABS

N.T.S.

NOTE:
1) FOR SLAB LESS THAN 7" PLACE ANCHORS HORIZONTAL WITH MINIMUM CENTER TO CENTER SPACING OF 6"
2) FOR SLABS 7" OR GREATER ANCHORS MAY BE PLACED VERTICAL WITH A MINIMUM CENTER TO CENTER SPACING OF 3"

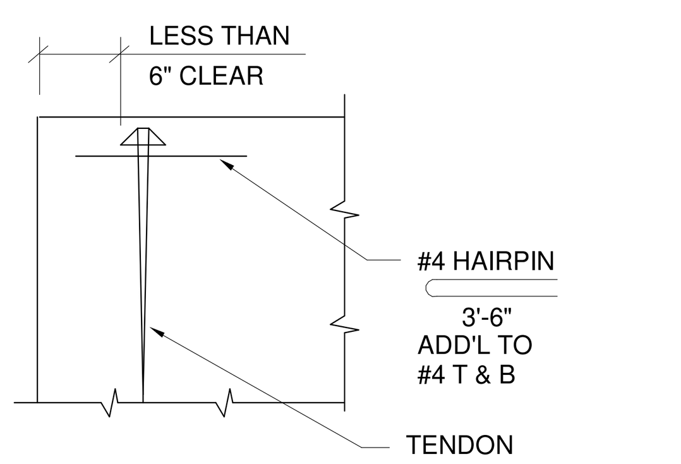


DEAD END ANCHORAGE
N.T.S.

INTERMEDIATE STRESSING ANCHORAGE
N.T.S.

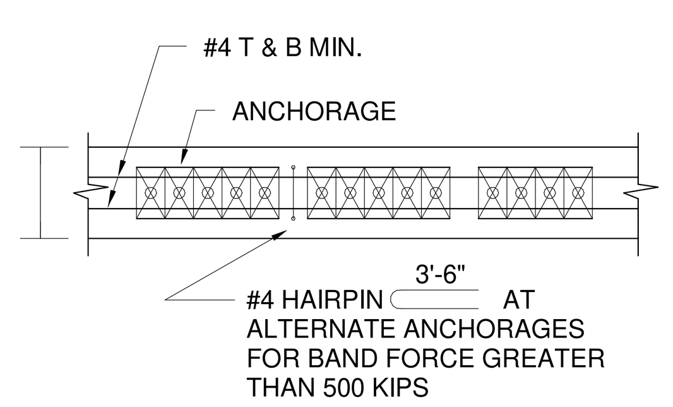
STRESSING END ANCHORAGE
N.T.S.

NOTE:
1) FOR SLAB LESS THAN 7" PLACE ANCHORS HORIZONTAL WITH MINIMUM CENTER TO CENTER SPACING OF 6"
2) FOR SLABS 7" OR GREATER ANCHORS MAY BE PLACED VERTICAL WITH A MINIMUM CENTER TO CENTER SPACING OF 3"
3) PLACE TENDONS IN GROUPS OF 5 OR LESS
4) AT EXTERIOR SLAB CONDITIONS #4 CRACK BARS SHALL BE GALVANIZED.



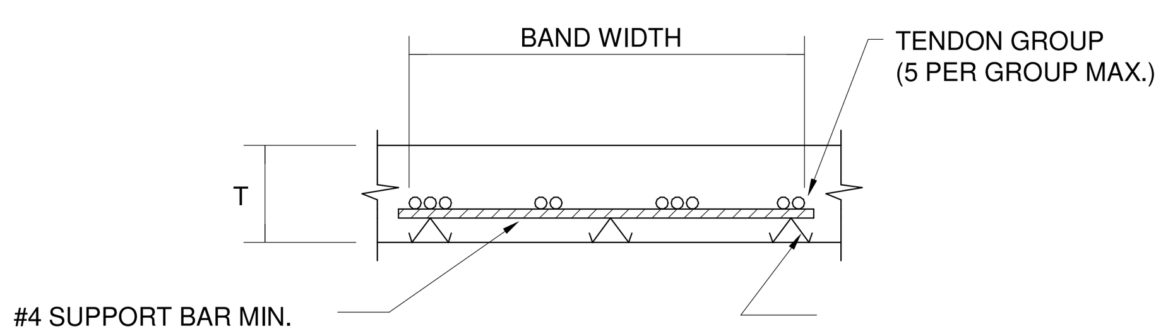
SLAB CORNER W/ TENDON WITHIN 6" OF SLAB EDGE

N.T.S.



TENDON ELEVATION WITH 5 OR LESS TENDON GROUPS

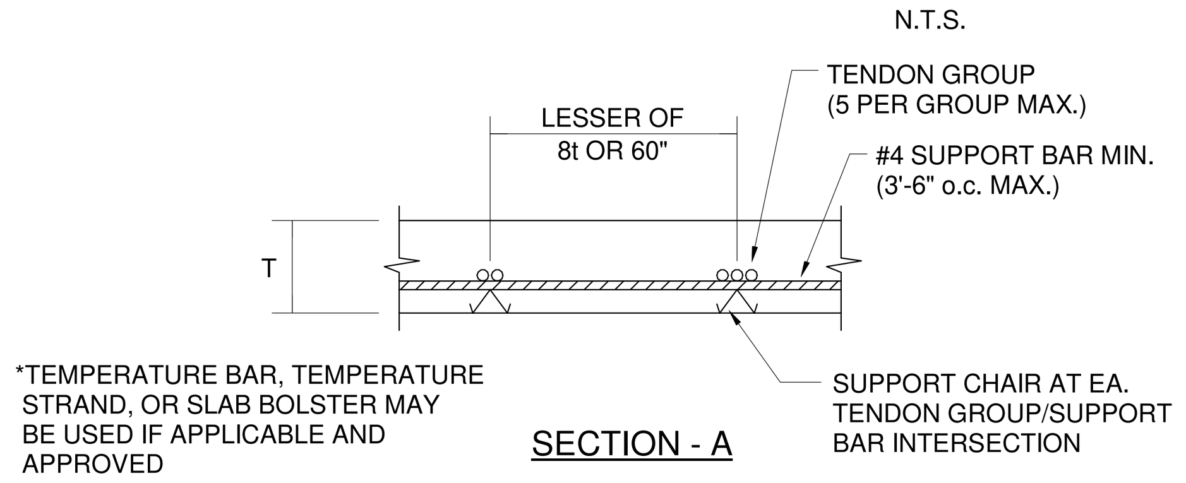
N.T.S.



SECTION - A

TYPICAL BANDED SLAB TENDON SUPPORT

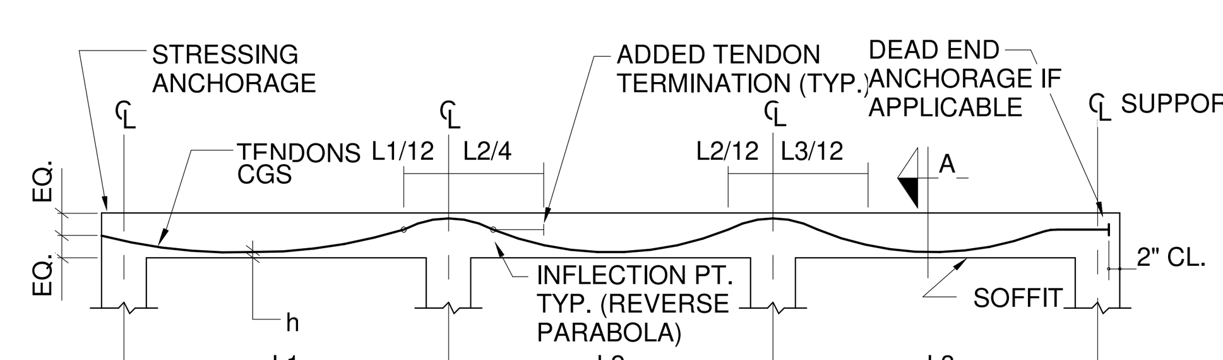
N.T.S.



SECTION - A

TYPICAL UNIFORM SLAB TENDON SUPPORT

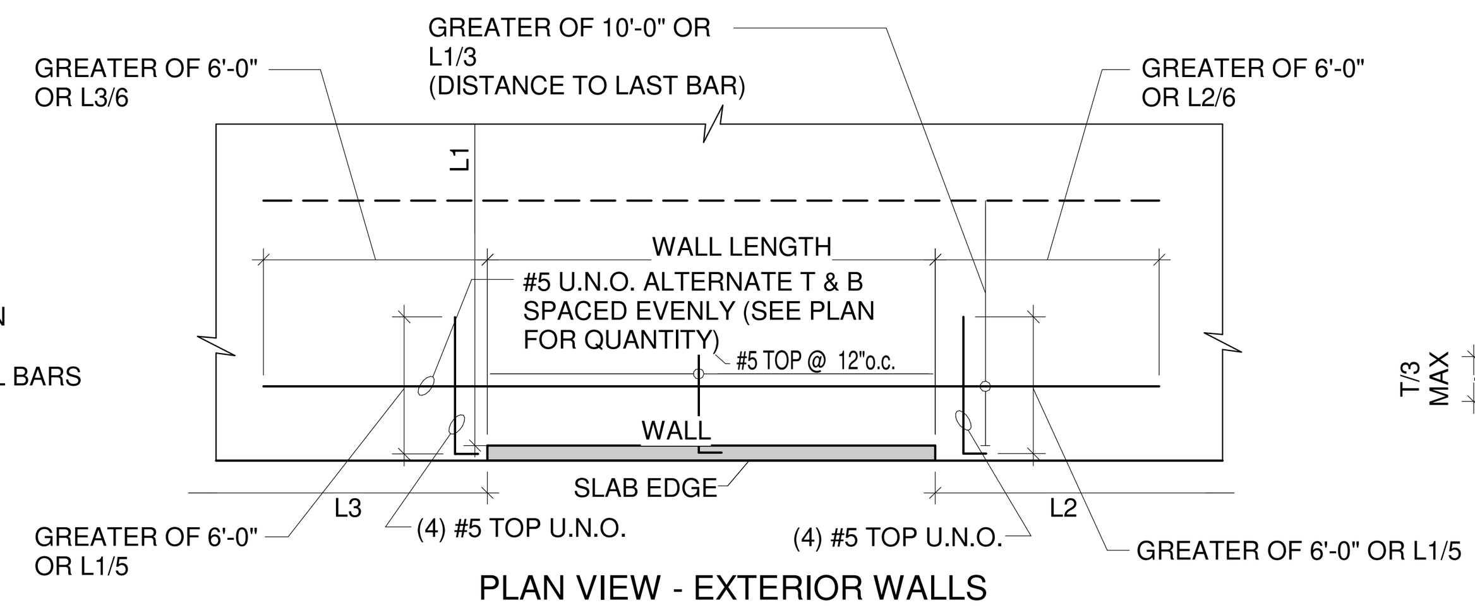
N.T.S.



NOTE- h: SEE PLAN FOR DRAPE (TO CABLE CENTERLINE) OF TENDON AT SUPPORT POINTS AND MID-SPAN.

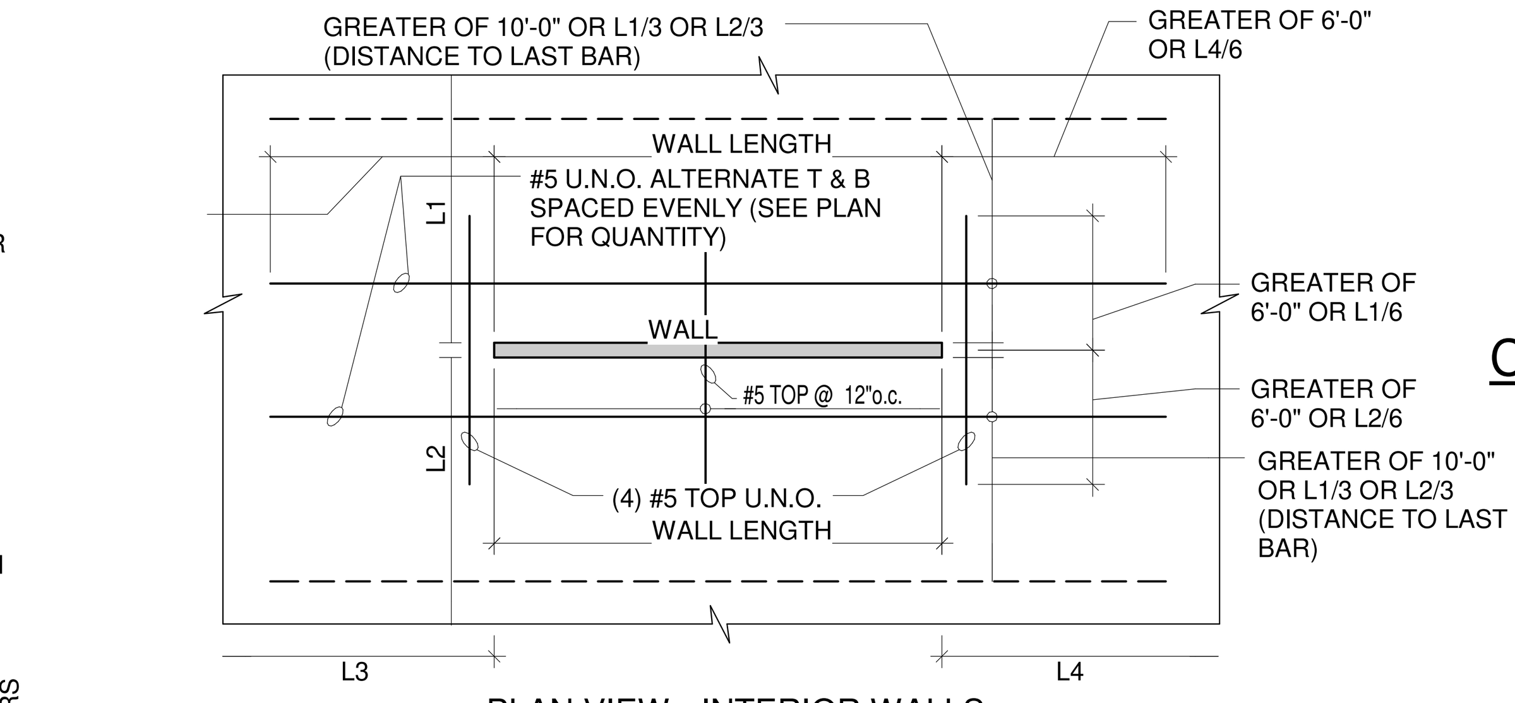
TYPICAL SLAB TENDON FORCE PROFILE

N.T.S.



PLAN VIEW - EXTERIOR WALLS

NOTE: L1, L2, L3 INDICATE CLEAR LENGTH BETWEEN SUPPORT MEMBERS



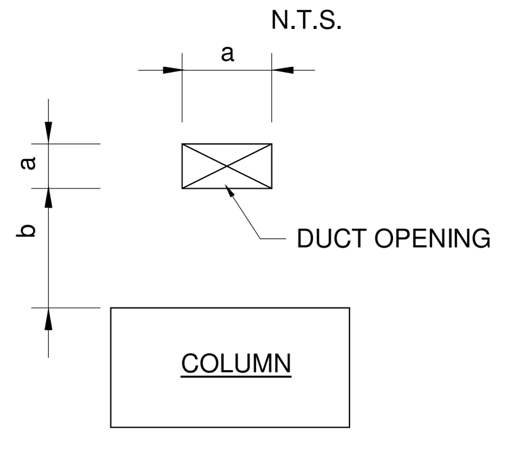
PLAN VIEW - INTERIOR WALLS

NOTE: L1, L2, L3, L4 INDICATE CLEAR LENGTH BETWEEN SUPPORT MEMBERS

TYPICAL REINFORCEMENT AT BEARING AND SHEAR WALLS

N.T.S.

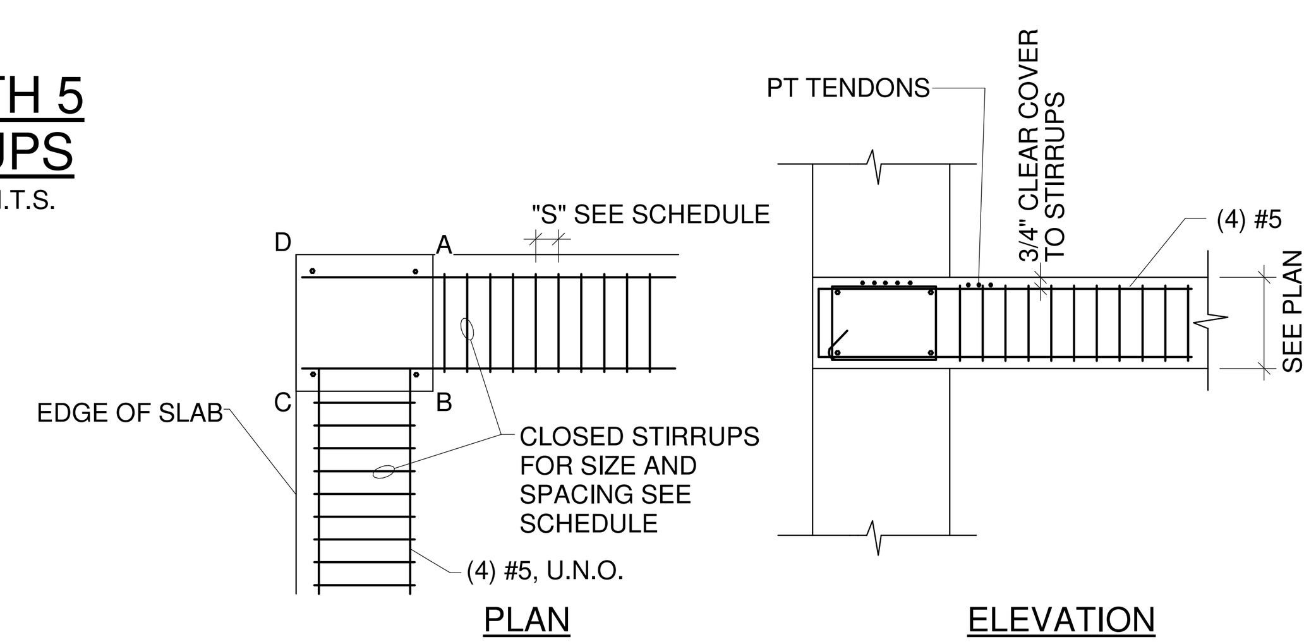
a DIM.	b DIM.
12"	3'-0"
14"	3'-4"
16"	3'-8"
18"	4'-2"
20"	4'-6"
22"	4'-10"
24" OR LARGER	5'-2"



DUCT OPENINGS AT COLUMNS TYP. DETAIL

N.T.S.

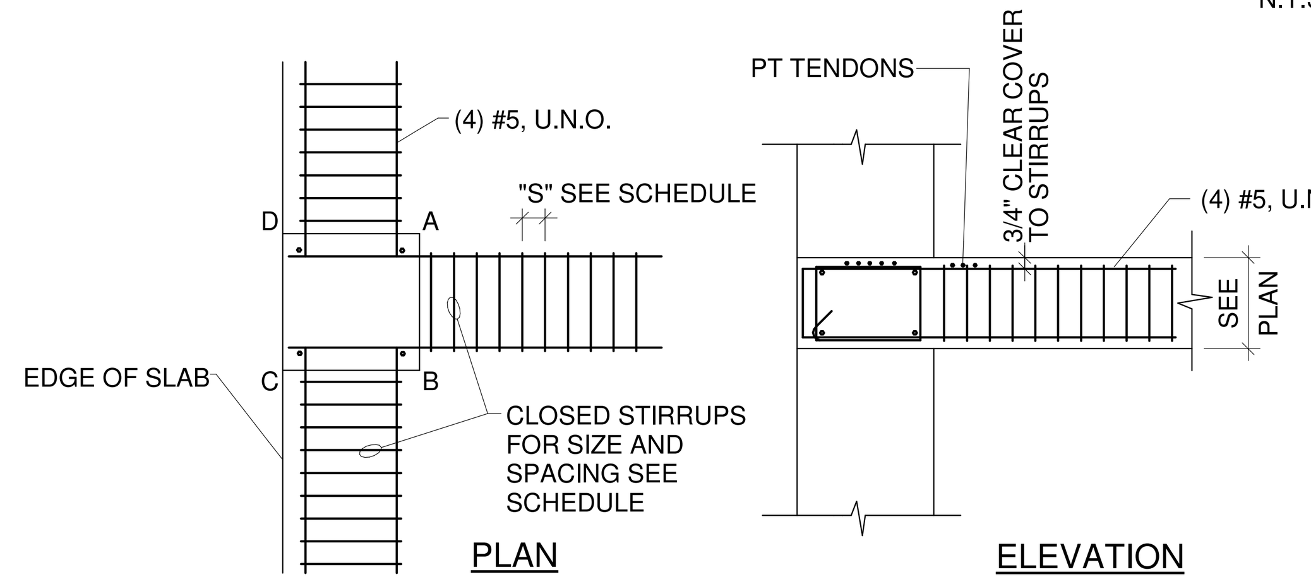
NOTE:
1) DUCT OPENINGS MAY OCCUR ONLY ON ONE FACE OF A COLUMN
2) DUCT OPENINGS MAY NOT OCCUR NEAR COLUMNS WITH PIPE SLEEVES OR VISE VERSA.
3) PUNCHING SHEAR REINFORCEMENT MAY BE REQUIRED OR MAY BE INCREASED AS A RESULT OF FINAL DUCT OPENING DIMENSION AND LAYOUT. G.C. AND MEP ENGINEER SHALL SUBMIT FINAL COORDINATED DUCT OPENING LAYOUT TO ENGINEER OF RECORD AND POST-TENSION SPECIALTY ENGINEER FOR FINAL REVIEW.
4) RELATIONSHIP OF a TO b DIMENSIONS MAY BE REVISED ONLY WITH APPROVAL FROM STRUCTURAL ENGINEER.



NOTE: STIRRUP CONCRETE COVER PER ACI CODE. STIRRUPS AT AREAS EXPOSED TO WEATHER SHALL BE GALVANIZED.

STIRRUP SLAB SHEAR REINFORCEMENT CORNER COLUMN.

N.T.S.

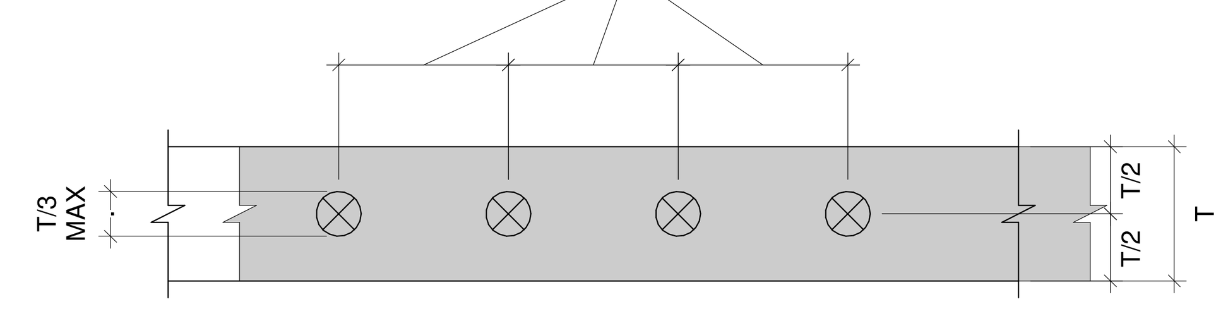


NOTE: STIRRUP CONCRETE COVER PER ACI CODE. STIRRUPS AT AREAS EXPOSED TO WEATHER SHALL BE GALVANIZED.

STIRRUP SLAB SHEAR REINFORCEMENT EDGE COLUMN.

N.T.S.

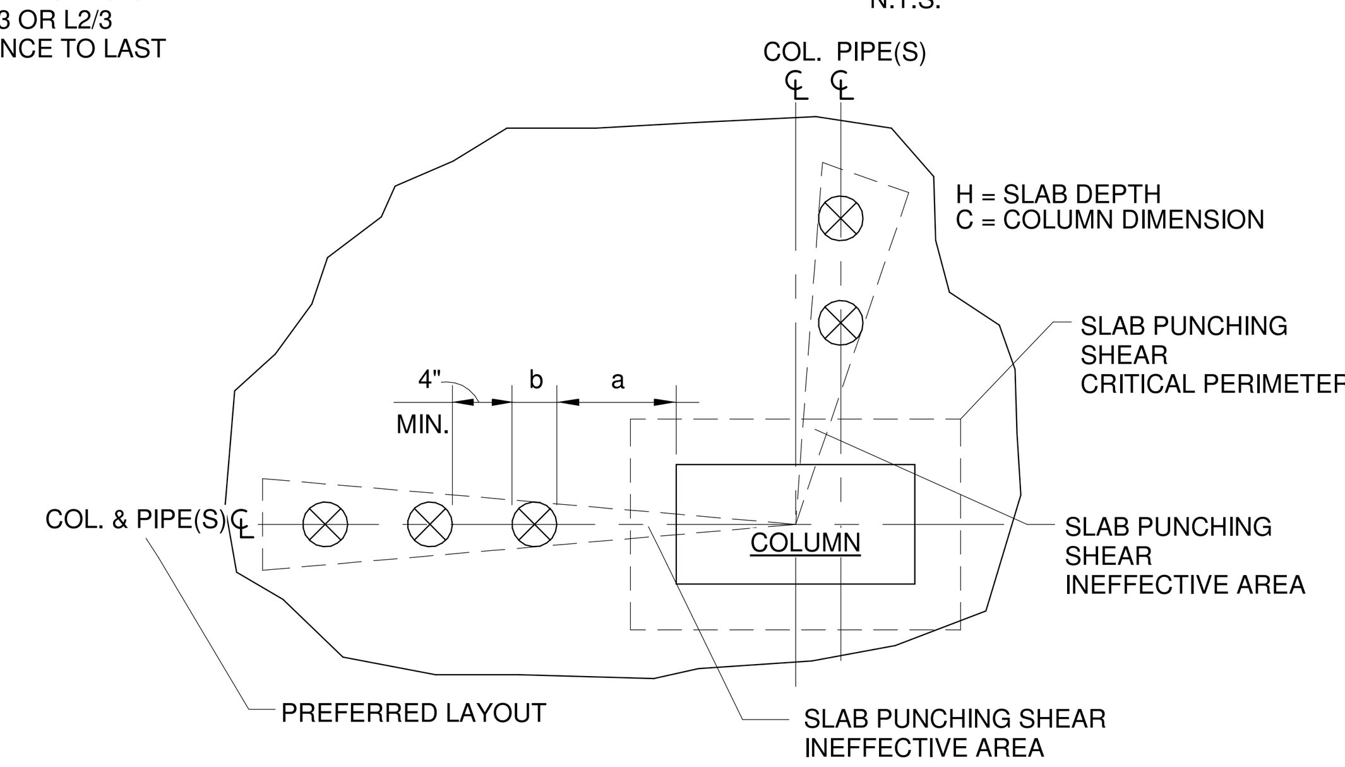
3 CONDUIT DIAMETERS OR 6" WHICHEVER IS GREATER



NOTES:
1. CONDUITS SHALL NOT BE LARGER IN OUTSIDE DIAMETER THAN 1/3 THE OVERALL THICKNESS OF THE SLAB IN WHICH THEY ARE EMBEDDED.
2. CONDUITS SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS OR WIDTHS ON CENTER. NO MORE THAN 3 CONDUITS SHALL BE PLACED IN A 6' STRIP OF SLAB WITHOUT ENGINEERS APPROVAL. UM SHALL NOT BE EMBEDDED IN STRUCTURAL SLABS
4. CONDUITS SHALL BE PLACED BETWEEN TOP AND BOTTOM SLAB REINFORCEMENT AND SHALL BE LOCATED AS CLOSE AS POSSIBLE TO THE MID-DEPTH OF THE SLAB.
5. MINIMUM CONCRETE COVER FOR CONDUITS SHALL BE THE SAME AS THAT OF THE SLAB MAIN REINFORCEMENT.
6. CONDUIT SHALL NEVER BE PLACED IN SUCH A MANNER THAT IT INTERFERES WITH THE POSITION OF THE POST-TENSION REINFORCEMENT. POST TENSIONING SHALL TAKE PRECEDENCE OVER CONDUIT IN ALL CASES. CONSULT ENGINEER OF RECORD WITH ALL INTERFERENCES.
7. CONDUIT SHALL BE PLACED AT 90° OR PARALLEL TO THE DIRECTION OF POST-TENSION STEEL.
8. CONDUIT SHALL NOT PASS THROUGH COLUMN PUNCHING SHEAR REINFORCEMENT WITHOUT ENGINEERS APPROVAL

CONDUITS EMBEDDED IN POST-TENSIONED STRUCTURAL SLABS

N.T.S.

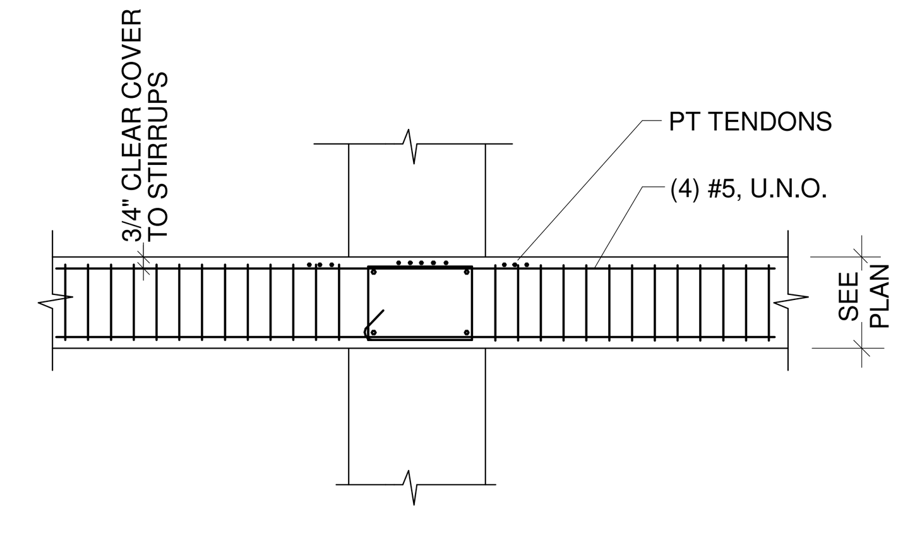


1.- DIMENSION a = 2 x b (MIN.) BUT NOT LESS THAN H OR I2" WHICHEVER IS LARGER. UTSIDE DIMENSION OF PIPE SLEEVE.
3.- MIN. SPACING OF PIPE SLEEVES = 4"
4.- PIPE SLEEVES MUST NOT BE GROUPED OR PLACED IN A STRING ALONG THE FACE OF COLUMN.
5.- PIPE SLEEVES MAY BE STRUNG OUT PERPENDICULAR TO ANY OF TWO FACES OF INTERIOR COLUMNS AS SHOWN.
6.- PIPE SLEEVES MAY BE STRUNG OUT PERPENDICULAR TO ANY OF ONE FACE OF EXTERIOR COLUMNS (NOT SHOWN).

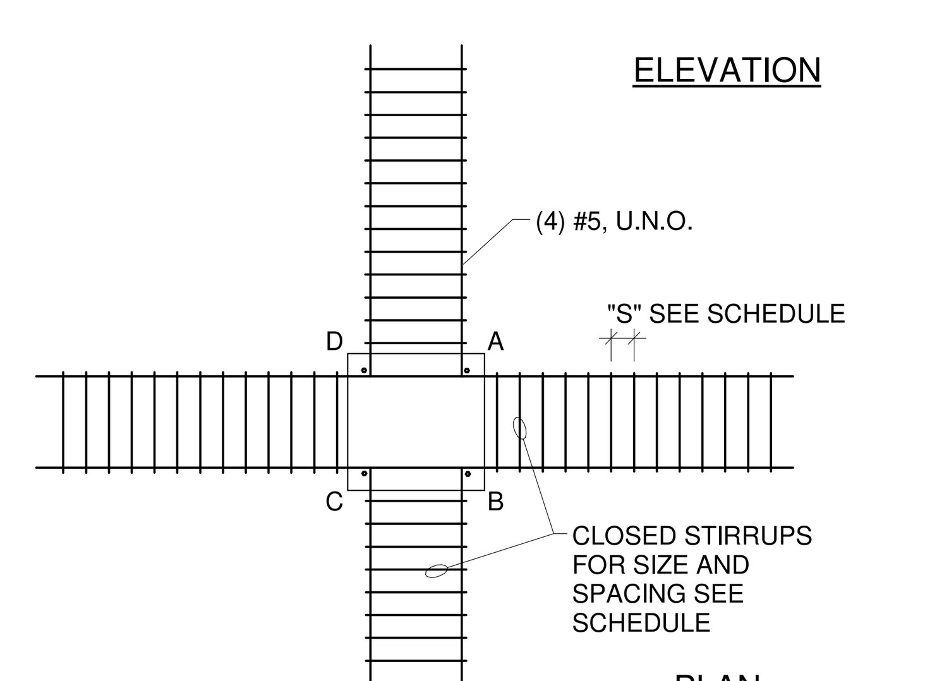
PIPE SLEEVES AT COLUMNS TYPICAL DETAIL

N.T.S.

NOTE:
1) PUNCHING SHEAR REINFORCEMENT MAY BE REQUIRED OR MAYBE INCREASED AS A RESULT OF FINAL SLEEVE DIMENSION AND LAYOUT. G.C. AND MECHANICAL/ELECTRICAL ENGINEER SHALL SUBMIT FINAL SLEEVE COORDINATED LAYOUT TO ENGINEER OF RECORD AND POST TENSION SPECIALTY ENGINEER FOR FINAL REVIEW.
2) G.C. VERIFY PIPE SLEEVE BACKGROUND DATE USED ON STRUCTURAL PLAN AND CONTACT ARCHITECT OR ENGINEER IMMEDIATELY WITH ANY DISCREPANCIES WITH BACKGROUND DATE OR LAYOUT ON STRUCTURAL AND MECHANICAL-ELECTRICAL-PLUMBING PLANS. DO NOT ADD PIPE SLEEVE NOT SHOWN ON STRUCTURAL PLANS WITHOUT WRITTEN AUTHORIZATION FROM STRUCTURAL ENGINEER
3) THIS DETAIL PROVIDES GUIDELINES TO MINIMIZE THE IMPACT OF THE SLEEVE DIMENSION AND LAYOUT ON THE SLAB REINFORCEMENT AT THE COLUMNS. IGNORING THIS GUIDELINE WILL MAXIMIZE THE SLAB SHEAR INTEGRITY.



ELEVATION



PLAN

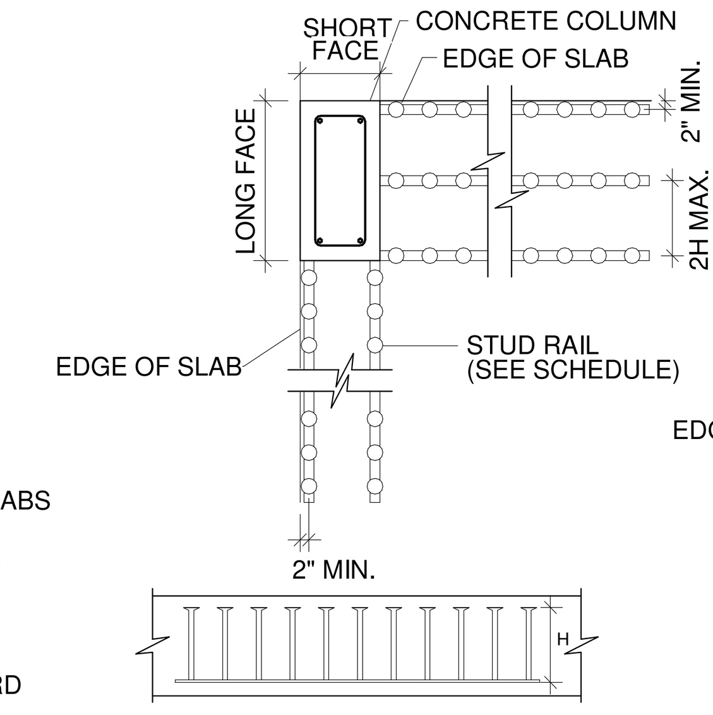
NOTE: STIRRUP CONCRETE COVER PER ACI CODE. STIRRUPS AT AREAS EXPOSED TO WEATHER SHALL BE GALVANIZED.

STIRRUP SLAB SHEAR REINFORCEMENT INTERIOR COLUMN.

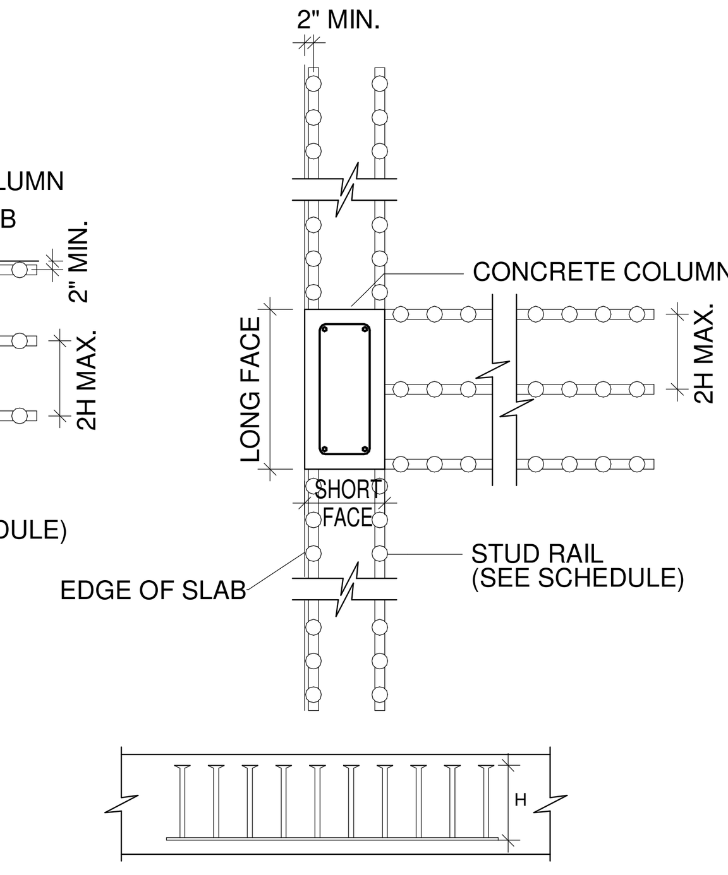
N.T.S.

STIRRUP SLAB SHEAR REINFORCING DETAILS

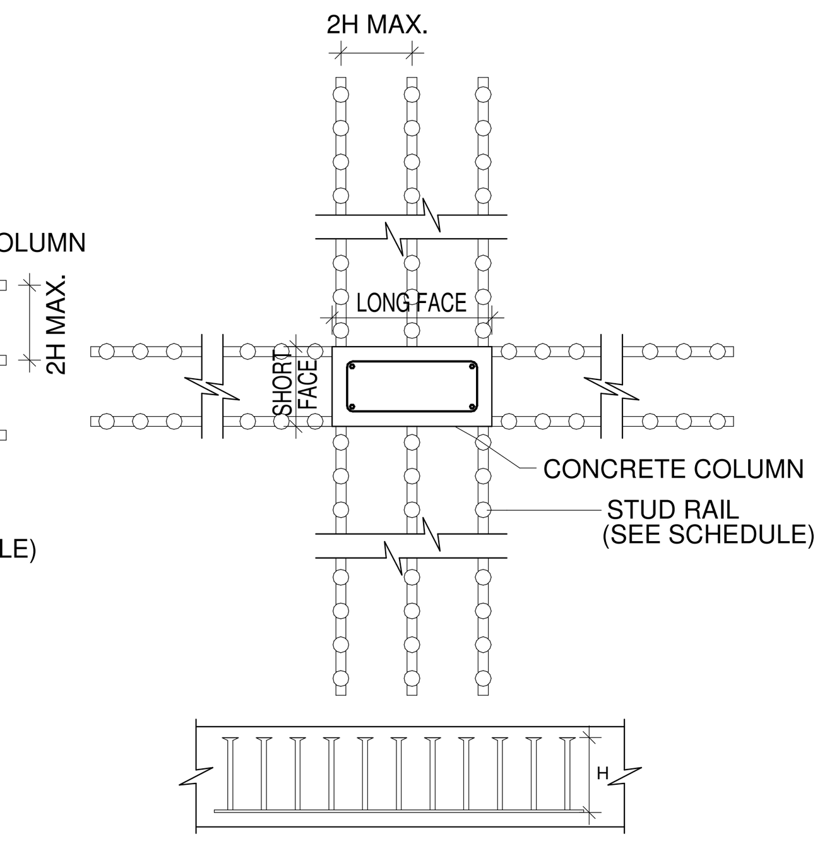
NOTE:
NO SLAB PENETRATIONS IN STIRRUP CONFIGURATION WITHOUT APPROVAL FROM ENGINEER OF RECORD



TYPICAL STUDRAILS DETAIL AT CORNER COLUMN
N.T.S.



TYPICAL STUDRAILS DETAIL AT EXTERIOR COLUMN
N.T.S.

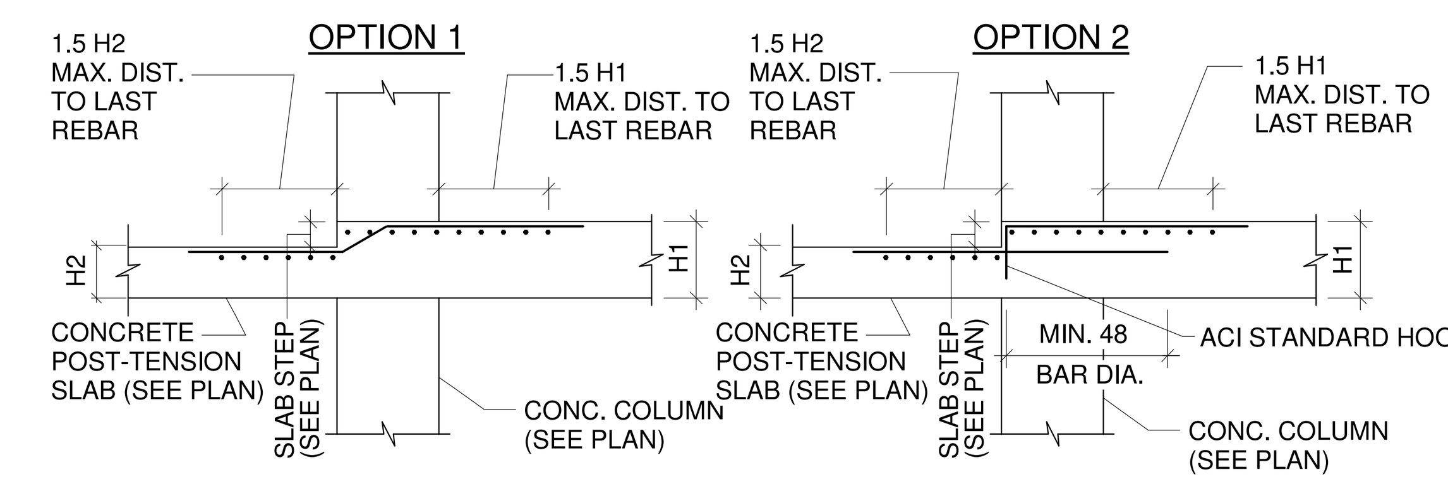


TYPICAL STUDRAILS DETAIL AT INTERIOR COLUMN
N.T.S.

STUDRAIL SLAB SHEAR REINFORCING DETAILS

N.T.S.

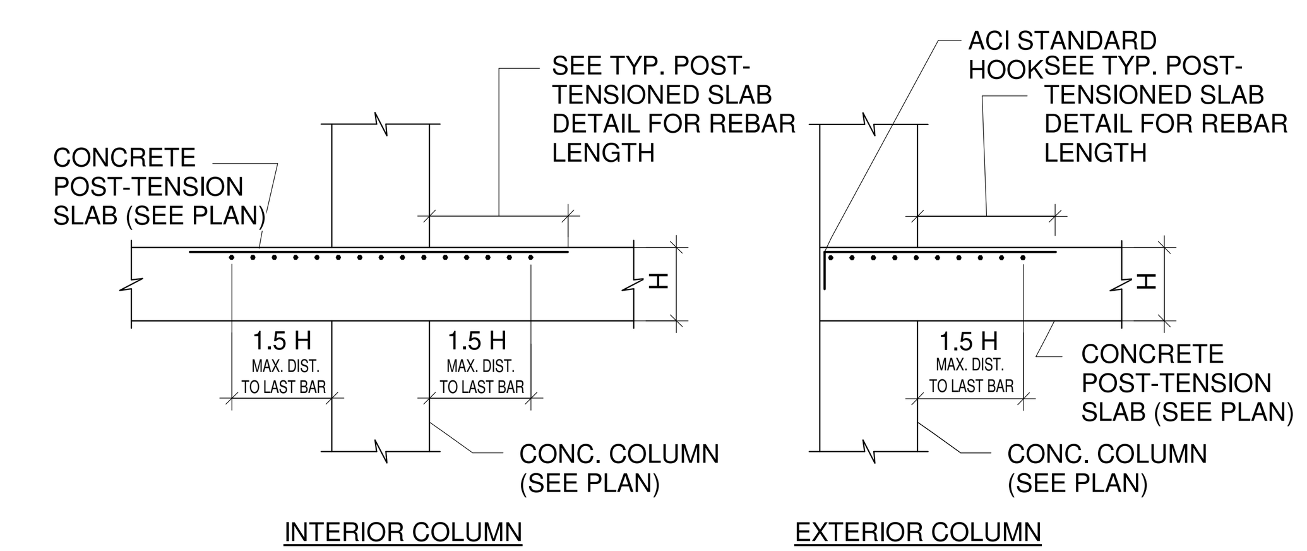
NOTE:
1) NO SLAB PENETRATIONS IN STUDRAIL CONFIGURATION WITHOUT APPROVAL FROM ENGINEER OF RECORD
2) STUDRAILS AT AREAS EXPOSED TO WEATHER SHALL BE GALVANIZED



TYPICAL 2-WAY PT SLABS TOP REINFORCEMENT DETAIL @ SLAB STEPS

N.T.S.

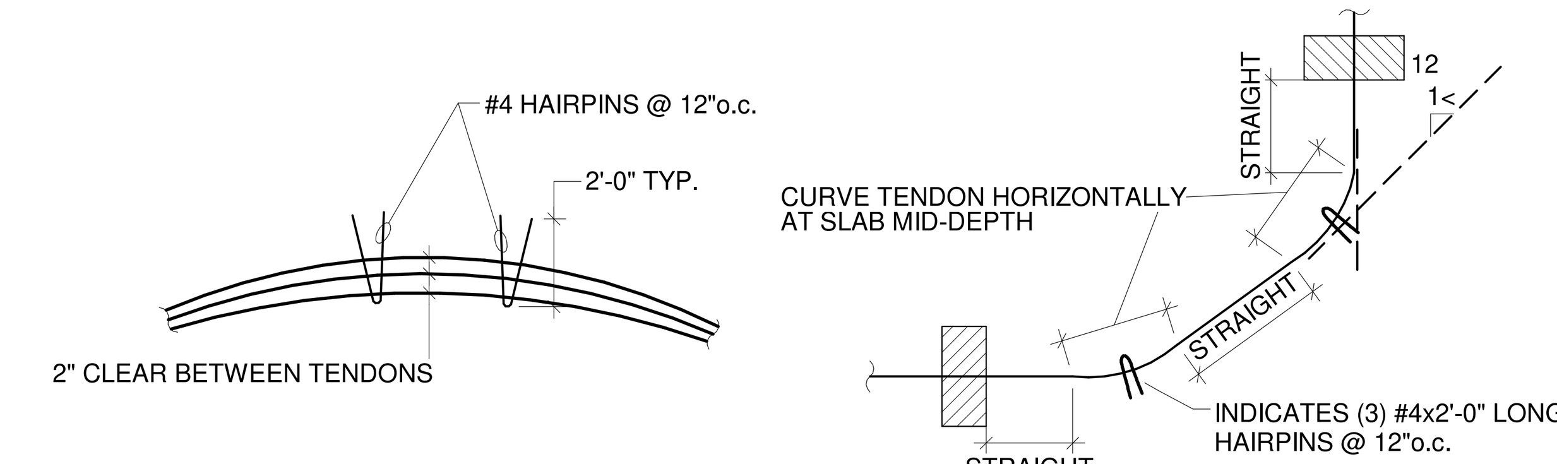
NOTE:
1) PT CABLE & SHEAR REINFORCING NOT SHOWN FOR CLARITY.
2) CONCRETE COVER PER ACI CODE.



TYPICAL TOP BAR PLACING AT COLUMN FOR POST-TENSIONED SLABS

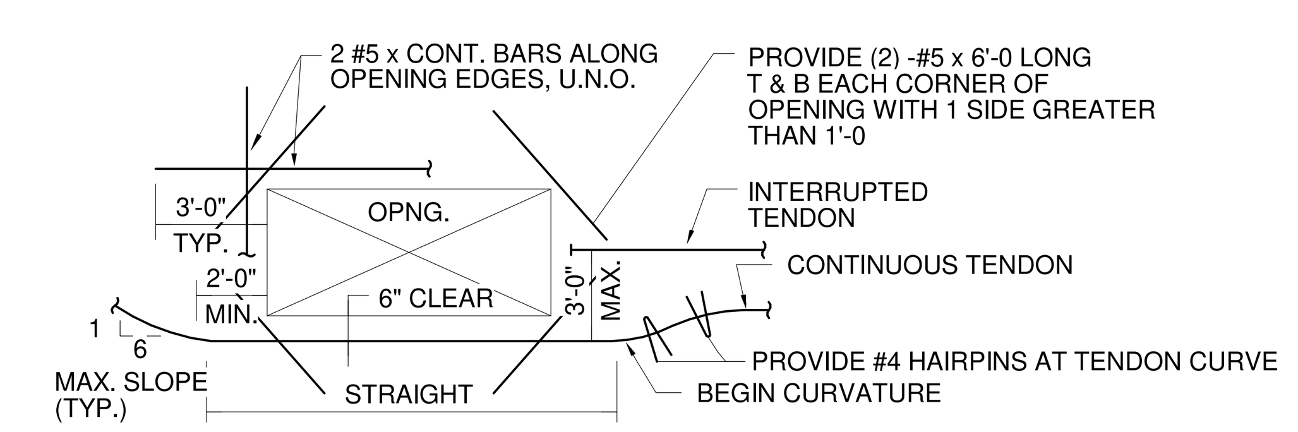
N.T.S.

NOTE:
1) START BY PLACING 2 BARS EVENLY SPACED AND CENTERED ON COLUMN, THEN EVENLY SPACE REMAINING BARS MAX. SPACE= 12".
2) REINFORCEMENT SHALL BE EVENLY SPACED & CENTERED ON COLUMN.
3) CORNER COLUMN IS SIMILAR.
4) H = SLAB DEPTH, C = COLUMN DIMENSION.
5) PT CABLE & SHEAR REINFORCING NOT SHOWN FOR CLARITY.
6) FOR CANTILEVERED EDGE AND CORNER CONDITIONS EXTEND TOP BARS TO EDGE OF CANTILEVER
7) CONCRETE COVER PER ACI CODE.



TYPICAL HAIRPIN PLACEMENT DETAIL

N.T.S.



TYPICAL POST TENSIONED SLAB OPENING

N.T.S.

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03/22/2024

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Seal:

Typical PT Details

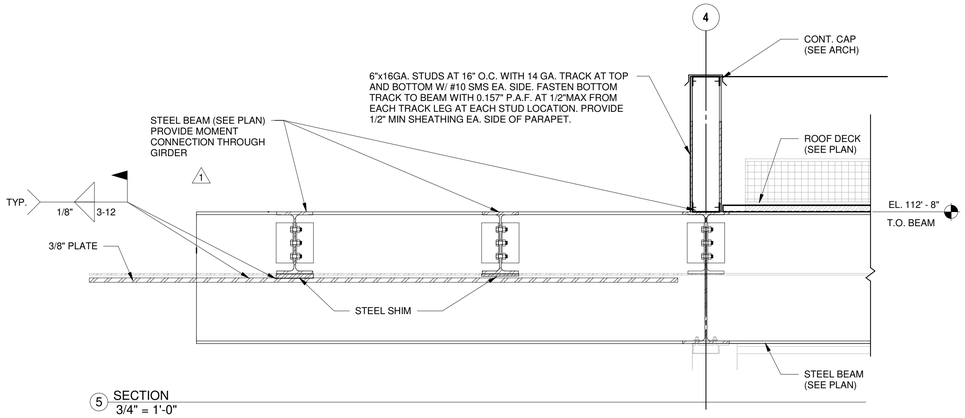
Revisions:

No.	Description	Date

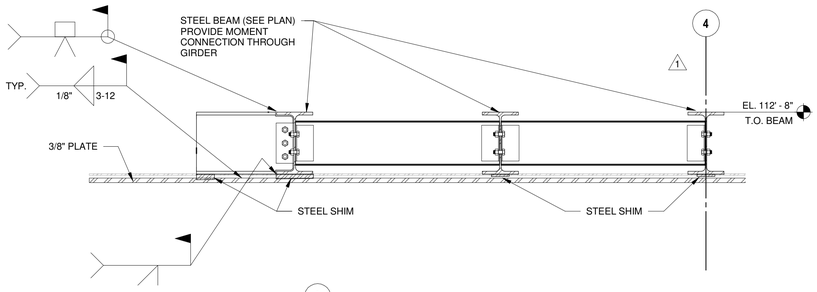
Comm:	Date: 11.01.2023
Drawn By: CJB	Checked: PJR

Sheet:

S2.04



5 SECTION
3/4" = 1'-0"



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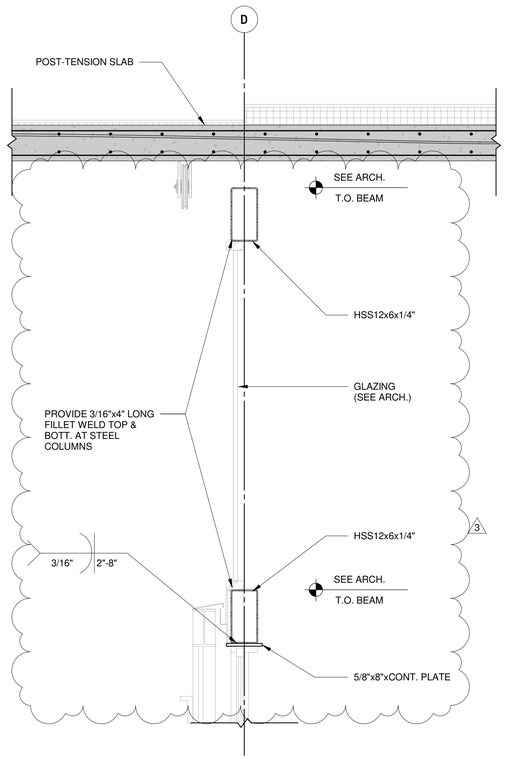

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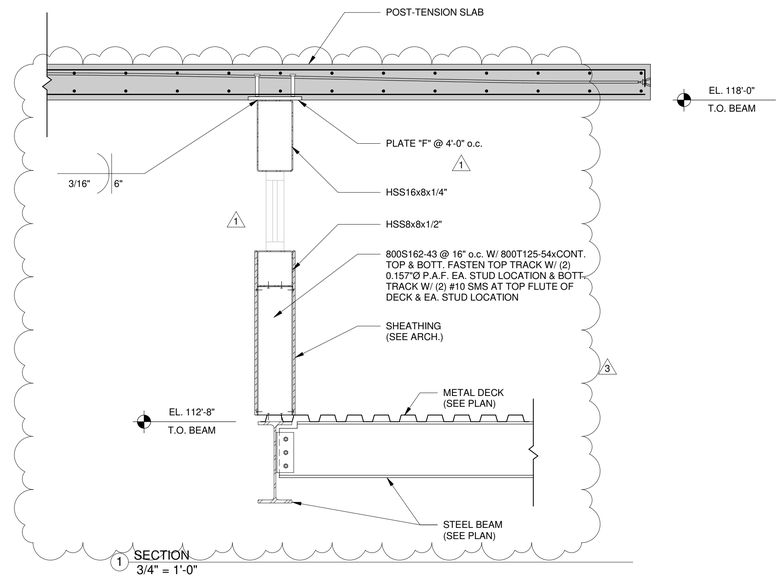
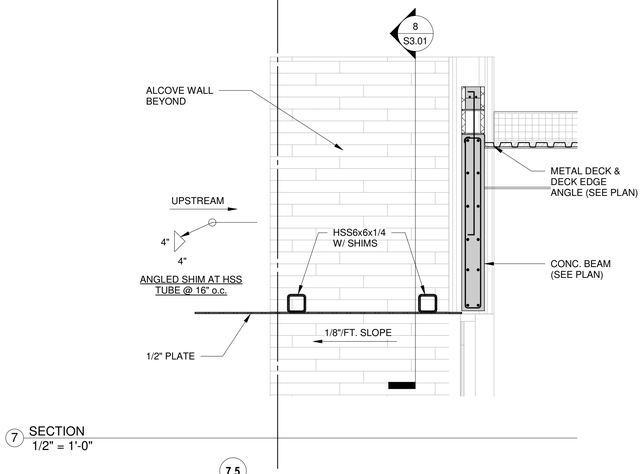


1 SECTION
3/4" = 1'-0"

2 SECTION
3/4" = 1'-0"



7 SECTION
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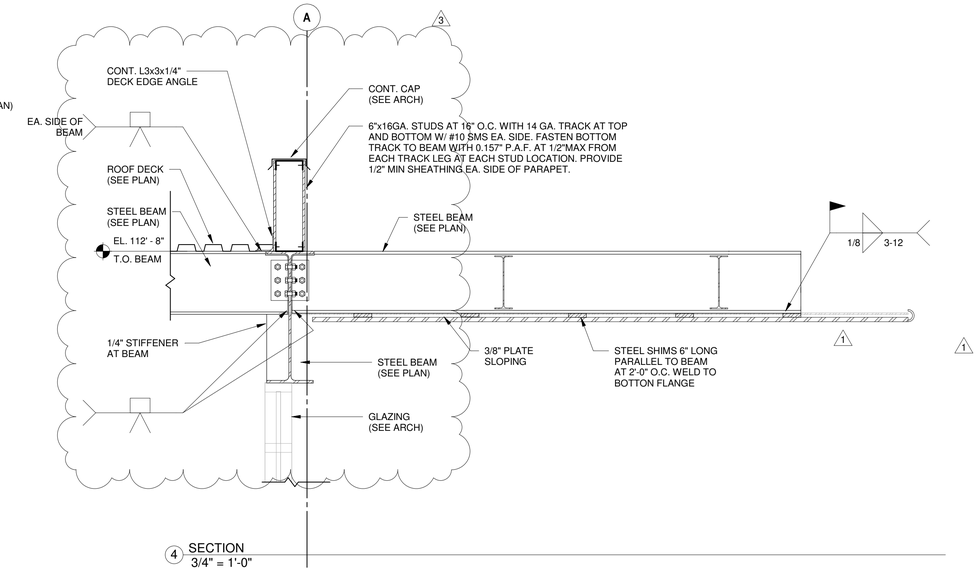

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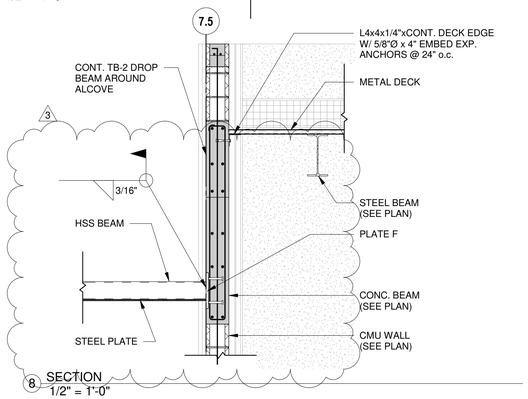

8 SECTION
1/2" = 1'-0"

1

03/22/2024

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Seal:

Sections

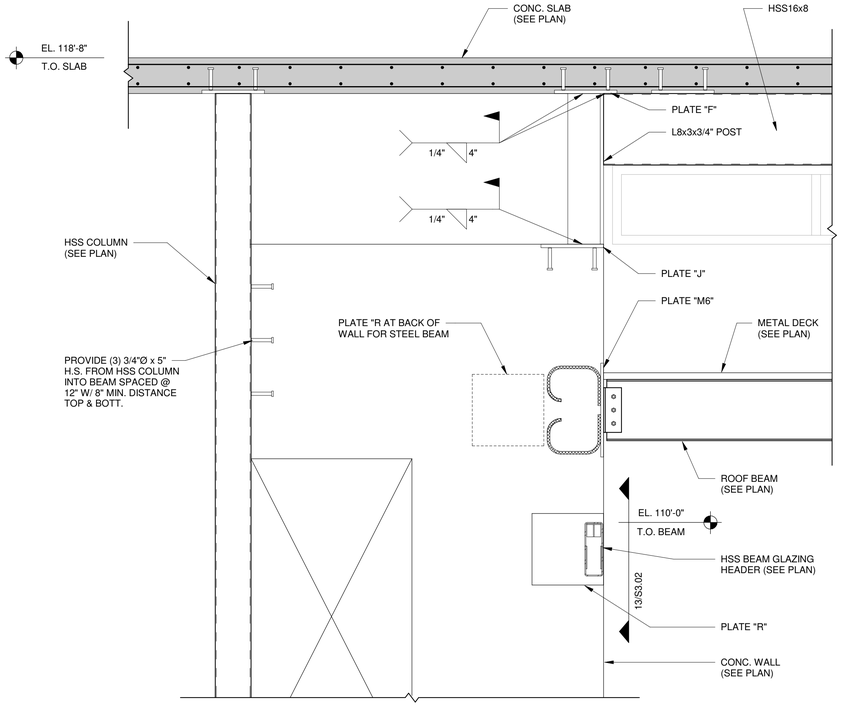
Revisions:

No.	Description	Date
1	Bldg. Dept. Comments	12.15. 2023
3	Addendum #1	03.22. 2024

Comm:	Date: 11.01.2023
Drawn By: CJB	Checked: PJR

Sheet:

S3.01



12 SECTION
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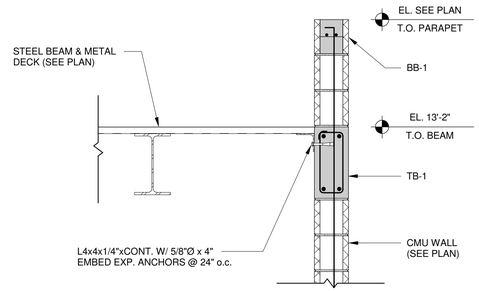

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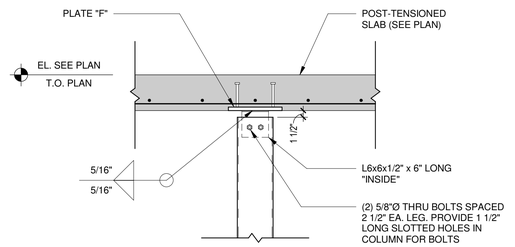

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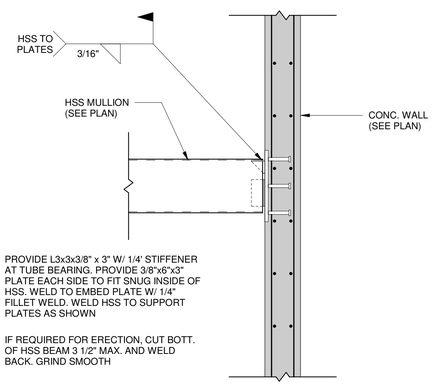

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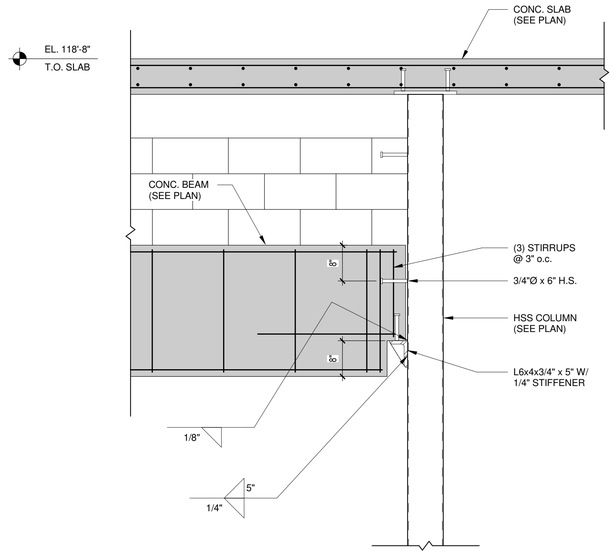

11 SECTION
3/4" = 1'-0"

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No.	Description	Date
3	Addendum #1	03.22.2024

Comm:	Date: 11.01.2023
Drawn By: CJB	Checked: PJR

Sheet:
3
S3.02