 FLORIDA BUILDING CODE 7TH EDITION [2020] - BUILDING FLORIDA BUILDING CODE 7TH EDITION [2020] - ACCESSIBILITY FLORIDA BUILDING CODE 7TH EDITION [2020] - ENERGY CONSERVATION FLORIDA FIRE PREVENTION CODE, 7TH EDITION [2020] FLORIDA BUILDING CODE 7TH EDITION [2020] - LIFE SAFETY FLORIDA BUILDING CODE 7TH EDITION [2020] - FUEL GAS FLORIDA BUILDING CODE 7TH EDITION [2020] - MECHANICAL FLORIDA BUILDING CODE 7TH EDITION [2020] - PLUMBING NATIONAL ELECTRIC CODE [N.E.C.] NFPA 70 2014 (FBC ADOPTED DEC. 31, 2017) 	DESIGN CODES USED:	
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FLG

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AB	ANCHOR BOLT
ABV	ABOVE
ACI	AMERICAN CONCRETE
	INSTITUTE
ADD'L	ADDITIONAL
AFF	ABOVE FINISHED FLR.
AGGR	AGGREGATE
AISC	AMERICAN IRON AND
	STEEL CONSTRUCTION
	AMERICAN IRON AND
	STEEL INSTITUTE
AL	ALUMINUM
ALT	ALTERNATE
ARCH	ARCHITECT(URAL)
ASTM	AMERICAN SOCIETY OF
	TESTING MATERIALS
AWS	AMERICAN WELDING
	SOCIETY
3/	BOTTOM OF
-, 3B	BOND BEAM
BIDG	BUILDING
31 K	BLOCKING
3M	BFAM
30T	BOTTOM
3P	BASE PLATE
BRDG	BRIDGING
BRG	BFARING
3RK	BRICK
39	BOTH SIDES
3T	BOTTOM TRACK
	CENTER TO CENTER
CANT	CANTILEV/FR
CB	CONCRETE BEAM
CFS	COLD FORMED STEEL
	CONSTRUCTION /
	CONTROL JOINT
CL	CONTROL JOINT CENTER LINE
CL	CONTROL JOINT CENTER LINE CLEAR(ANCE)
CL CLR CMU	CONTROL JOINT CENTER LINE CLEAR(ANCE) CONCRETE MASONRY
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FOUNDATION

FDN

BE	BREVIATIONS
	FINISHED FLOOR
	FLOOR
	FLANGE
	FRAMING
	FOOT
	FOOTING
	GAUGE
	GALVANIZE(D)
	GRADE BEAM
	GENERAL CONTRACTOR
	GROUT FILLED CELL(S)
	GIRT
	HORIZONTAL
	HIGH POINT
	HEADED STUD
	HOLLOW STRUCTURAL
	SECTION
	INSIDE FACE
	INTERIOR
	JOIST
	JOINT
	KIP
	LONG
	LONG POINT
	MASONRY
	MAXIMUM
	MACHINE BOLT
	METAL BUILDING
	MANUFACIURER
	MASONRY CONTROL
	JOINT
	MECHANICAL
	MEZZANINE
	MISCELLANEOUS
	MASONRY OPENING
	METAL STUD
	METAL
	NOT TO SCALE
	OUTSIDE DIAMETER
	OUTSIDE FACE
	OVER HEAD
	OPENING
	OPPOSITE
	POWER-ACTUATED
	PRE-ENGINEERED
	METAL BUILDING
	PERPENDICULAR
	PRECAST
	PLATE
	YOUNDS PER
	PANEL
	POUNDS PER
	SQUARE FOOT
	POUNDS PER
	DRUAKE INCH
	RADIUS

1	
RC	REINFORCED CONC.
REF	REFERENCE
REINF	REINFORCE(D) (ING)
	REQUIRE
RF	ROOF
RL	RAIN LIP
RO	ROUGH OPENING
RP	RADIUS POINT
RTN	RETURN
RW	RETAINING WALL
SC	SLIP CRITICAL
SCH	SCHEDULE
SECT	SECTION
SG	SUBGIRT
50	
51	SAWCUT JUINT
501	SHEET LEDGE
SP	SPECIAL
SPECS	SPECIFICATIONS
SQ	SQUARE
SSL	SHORT SLOTTED
STD	STANDARD
STIFF'R	STIFFENER
STL	STEEL
STR	STRENGTH
STRL	STRUCTURAL
SW	SHORT WAY OR
	SHEAR WALL
STMM	SYMMETRICAL
TR	TIF BEAM
TC	
TDS	TURNED-DOWN SLAB
TEMP	TEMPERATURE
ТНК	THICK
THNS	THICKENED SLAB
TOP'G	TOPPING
TYP	TYPICAL
T/	TOP OF
TT	TOP TRACK
UNO	UNLESS NOTED
	ULIKA DEK
WF	WALL FOOTING
WO	WINDOW OPENING
WWF	WELDED WIRE FABRIC
W/	WITH

	Delray Beach Golf Club	
TYPICAL GRID BOBB TYPICAL GRID BOBB COR COR COR COR 29G 26G COR 29G 10D	GF GF	RID NU
Image: Constant of the second seco	TYPICAL GRID E	
COR G" R 29G 26G C" R 29G C" R 29G C" R 29G C" R 29G PBR PAN CRC 5' B HOR 29G HALL		
G" R 29G 26G 26G 29G PBR PAN CRC 5' B HOR 29G HALL	p	CORI
G" R 29G PBR PAN CRC 5' B HOR 29G HALL		6" R- 29G/ 26G/
PBR PAN CRC 5' B HOR 29G HALL		6" R- 29G,
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HOR 29G		CRO: 5' BA
HALI		HOR 29G/
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LIBERTY DELRAY DELRAY, FL

IOB	SITE			SHFF
JOD				JIILL
			(k, k)	
	Puttin Around		C-1	COVER SHEET
Lake Ida Rd			C-2	BUILDING LAYOUT
			C-3	BUILDING LAYOUT
	Hampton Inn Deltay Be		5-0	GENERAL NOTES
			S-0.0	CODE ANALYSIS
			S-0.1	CODE ANALYSIS
Arts Carage		A1A Decel Grand Decert	S-0.2	ROOF ANALYSIS
			5-0.3	INSULATION SPECS
ere Delinay Beach			S-0.4	SPECIAL INSPECTION
			S-0.5	SPECIAL INSPECTION
	Courtyard by		S-0.6	TYPICAL SCHEDULE
	Maniott Delrayib		S-0.7	TYPICAL SCHEDULE
			5-1.0	BUILDING C PARTITION PLAN
			S-1.1	BUILDING C PARTITION PLAN
			5-1.2	BUILDING C COLUMN FRAMIN
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			5-1.4	BUILDING C ROOF FRAMING
			S-1.5	BUILDING C ROOF FRAMING
			5-1.6	BUILDING CINSULATION & RC
			5-17	BUILDING C FLEVATIONS
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TYPICAL NO	MINCLATURE		0-1.0	
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			5-2.0	BUILDING D PARTITION PLAN
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	– PAGE NUM.		5-2.4	BUILDING D ROOF FRAMING
			5-2.5	BUILDING D ROOF FRAMING
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DUBBLE ITPICAL DETA	IL FLAG	I TIFICAL SECTION CUT		
AL LINE KEY	TYP	ICAL PANELS	5-3.0	BUILDING E PARITION PLAN
			5-3.1	BUILDING E COLUMN FRAMIN
	L L	3' COVERAGE	5-3.2	BUILDING E ROOF FRAMING F
CORNER PIER, HEADER, PIER			5-3.3	BUILDING E INSULATION & RC
			5-3.4	BUILDING E ELEVATIONS
6" R-19 INSULATION IN PERIMETER WALL		PDR PANEL STEEL LINE	S-3.5	BUILDING E SECTION
29GA LINER PANEL INTERIOR AND				
26GA VERTICAL PANEL EXTERIOR			S-4.0	BUILDING F PARTITION PLAN
			5-4.1	BUILDING F COLUMN FRAMIN
29GA LINER PANEL BOTH SIDES	ł – – – – – – – – – – – – – – – – – – –	3' COVERAGE	S-4.2	BUILDING F ROOF FRAMING F
			5-4.3	BUILDING FINSULATION & RC
		PBU PANEL REV ROLL STEEL LINE	5-4.4	BUILDING F ELEVATIONS
PBR, 26GA VERTICAL PERIMETER WALL			S-4.5	BUILDING F SECTION
FANLL				
			SD-I	STRUCTURAL DETAILS
CROSS BEAMS NEED MORE THAN	r	2'-8" COVERAGE	SD-2	STRUCTURAL DETAILS
5' BAY			SD-3	STRUCTURAL DETAILS
		PBD PANEL STEEL LINE	SD-4	STRUCTURAL DETAILS
HORIZONTAL PARTITION PANEL				
29GA GALVALUME TYPE "U"			SB-1.0	BUILDING C BLACK IRON LAY
			SB-2.0	BUILDING D BLACK IRON LAYO
HALL WAY SYSTEM	<u>/</u>	<u> </u>	SB-3.0	BUILDING E BLACK IRON LAYO
			SBD-1	BLACK IRON DETAILS
		7.2 PANEL STEEL LINE		
COLUMNS COLUMNS			SF-1.0	BUILDING C FOUNDATION PLA
WEB			SF-2.0	BUILDING D FOUNDATION PLA
TOP FLANGE		3' COVERAGE	SF-3.0	BUILDING E FOUNDATION PLA
ZEE BEAMS			SF-4.0	BUILDING F FOUNDATION PLA
BOT FLANGE				
		2.0 VL 18GA GGO STEEL LINE	SFD-1.0	FOUNDATION DETAILS
TWO HOUR RATED CMU WALL				
@ STAIR UL # U905				
TWO HOUR RATED CMU WALL	<u>r</u>	3' COVERAGE		
@ ELEVATOR UL # U905				
		I .5" 20GA B-DECK STEEL LINE		
THREE HOUR RATED SEPERATION				
WALL UL #U419				

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	EAST COAST OFFICE: 1041 CROWN PARK CIRCLE WINTER GARDEN IL 34767 PHONE: (800)-989-0220	CONFIDENTIAL: THIS DOCUMENT AND THE INFORMATION CONTAINED HEREIN ARE THE PROPERTY OF MAKORABGO UNAUTHORIZED COPYING, DISCLOSURE OR OTHER UNAUTHORIZED USES ARE PROHIBITED.
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RBE CONSULTING SERVICES, M	2875 SFUT OAK COURT - OVEDO, FL. 32766 WEB/TE: WWW.REPECS.COM - ENANL: REPCSLIC@GMAIL.COM MAIN PHONE LINE: 407-796-3820 - FX LINE: 407-971-6718 MECHANICAL DEPT: 407-596-3776 - EMAIL: NEMATH@REPCS.COM ELECTRICAL DEPT: 407-5173-113 - EMAIL: REMATH@REPCS.COM STRUCTURAL DEPT: 407-5659 - EMAIL: REMATH@REPCS.COM STRUCTURAL DEPT: 407-5659 - EMAIL: REMATH@REPCS.COM	PLOREDA CERCIFICATE OF AUTHORIZATION #27269 REF CONSULTING SERVICES, ILC'HEREEY REF CONSULTING SERVICES, ILC'HEREEY EPRERSA'REFERVES ITS COMMON LAW COPPRETATIAND OTHER PROFENS INGHTS TO THESE PLANES AND THE RESOLUCION RANNOS PLESCALLANES AND THE DESIGN AND CONTRAINED WITHIN THESE PLANES AND THE REVISION AND CONSENT OF REMISSION OR MANNER WATTON THE REVESS WETTEN PERMISSION AND CONSENT OF "REE CONSULTING PERMISSION AND CONSENT OF "REE CONSULTING SERVICES, ILC'.
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FOUNDATION NOTES

GENERAL

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- 3. OWNER / CONTRACTOR IS RESPONSIBLE FOR ALL NECESSARY AND REQUIRED PERMITS, FEES, DEPOSITS, ECT.
- THE OWNER AND/OR CONTRACTOR SHALL REVIEW AND DETERMINE THAT ALL DIMENSIONS ARE COORDINATED AS REQUIRED WITH ALL OTHER DESIGN PROFESSIONALS DRAWINGS AND SHOP DRAWINGS FOR PROJECT PRIOR TO FABRICATION OF MATERIALS OR THE START OF CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO THE METAL BUILDING COMPANY AND ENGINEER OF RECORD.
- 5. SHOP DRAWING ARE CRITICAL TO ENSURE THE DIMENSIONS AND DESIGN OUTLINED IN THESE PLANS MEET THE MINIMUM REQUIREMENTS REQUIRED BY THESE SCOPES OF WORK IF UNDER CONTRACT BY OTHERS. IN THE EVENT THE CONTRACTOR'S OR OWNER'S FAILING TO PROVIDE, HE SHALL BE RESPONSIBLE FOR THE RESULTS OR ANY SUCH ERRORS OR OMISSIONS AND THE COST OF RECTIFYING THE SAME (EXAMPLES: ELEVATOR, STAIRWELL, DOORS, ETC....).
- 6. IF ANY ERRORS OR OMISSIONS APPEAR IN THE DRAWINGS. SPECIFICATIONS OR OTHER DOCUMENT, THE OWNER / CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD AND METAL BUILDING COMPANY IN WRITING OF SUCH OMISSIONS OR ERRORS PRIOR TO PROCEEDING WITH ANY WORK WHICH APPEARS IN QUESTION. EVERY REASONABLE EFFORT HAS BEEN MADE TO ENSURE COORDINATION BETWEEN THESE DRAWINGS AND THE STRUCTURAL PLANS. IN THE EVENT THE CONTRACTOR'S OR OWNER'S FAILING TO GIVE SUCH NOTICE, THEY SHALL BE RESPONSIBLE FOR THE RESULTS OR ANY SUCH ERRORS OR OMISSIONS AND THE COST OF RECTIFYING THE SAME.
- 7. ANY OMISSIONS AND/OR CONFLICTS WITH PLANS SHALL BE REPORTED TO METAL BUILDING COMPANY SO THAT THEY CAN BE RESOLVED PRIOR TO PROCEEDING WITH WORK.
- 8. DO NOT SCALE DRAWINGS IF A REQUIRED DIMENSION IS MISSING PLEASE CONTACT THE METAL BUILDING COMPANY AND / OR ENGINEER OF RECORD.
- 9. NO MODIFICATIONS TO PLANS SHALL BE MADE WITHOUT THE PERMISSION OF METAL BUILDING COMPANY AND ENGINEER OF RECORD. MODIFICATIONS REQUIRED DUE TO FIELD CONDITIONS OR OTHER CONTRACTORS OR ITEMS THAT WHICH MAY ADVERSELY AFFECT THE STRUCTURE REQUIRES WRITTEN PERMISSION (NO MODIFICATIONS TO STRUCTURAL MEMBERS IS ALLOWED).
- IO. ALL SECTIONS AND DETAILS SHALL BE CONSIDERED TO BE TYPICAL OR SIMILAR UNLESS ANOTHER SECTION OR DETAIL IS REFERENCED ON THE PLANS.
- II. SCOPE OF WORK OF METAL BUILDING COMPANY IS INDICATED IN THE CONTRACT. THE DRAWINGS REFLECT SCOPES OF WORK AS REQUIRED FOR PERMITTING OR AT THE DIRECTION OF OWNER / CONTRACTOR.
- 12. SUBMITTALS TO THE ENGINEER OF RECORD FOR REVIEW MUST CONTAIN THE CONTRACTOR'S OR OWNER'S STAMP SIGNIFYING THEIR REVIEW / ACCEPTANCE. SUBMITTALS SENT WITHOUT WILL BE RETURNED AT THEIR EXPENSE WITHOUT REVIEW. A MAX. OF THREE SETS ADDITIONAL SETS WILL BE DISCARDED
- 13. THE CONTRACTOR OR OWNER SHALL TAKE ALL NECESSARY STEPS TO PROTECT THE STRUCTURE, THE WORK PERSONS AND OTHER PEOPLE DURING CONSTRUCTION. HE SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION.

SLAB ON GRADE

UNLESS SPECIFICALLY STATED OTHERWISE IN THE GEOTECHNICAL SOILS REPORT, THE FOLLOWING MINIMUM CRITERIA SHALL BE ADHERED TO.

- a.) INTERIOR FILL SHALL BE COMPACTED TO A MINIMUM OF 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY TEST (ASTM D | 557).
- b.) COMPACTION OF THE SOIL IN THE FIELD SHALL BE MONITORED/CONTROLLED BY A REPRESENTATIVE OF A QUALIFIED LABORATORY.
- c.) EACH LAYER OF FILL SHALL NOT EXCEED 12" THICK AND SHALL BE COMPACTED PRIOR TO PLACEMENT OF THE NEXT LAYER.

MAXIMUM SPACING OF CONTROL JOINTS SHALL BE AS SHOWN IN THE TABLE BELOW. PATTERNS SHALL BE APPROXIMATELY SQUARE W/ RATIO OF LONG SIDE TO SHORT SIDE NOT TO EXCEED 1.5 TO 1.0.

SLAB THICKNESS (IN) -	4	5	6	7	8	9	10
SPACING (FT)	12	13	15	18	20	23	25

MIX DESIGNS CONTAINING AGGREGATE LESS THAN 3/4" ARE NOT ACCEPTABLE CUT SLAB AS SOON AS AGGREGATE DOES NOT DISLODGE (MUST BE WITHIN THE SAME DAY AS THE CONC. WAS PLACED)

CARE SHALL BE TAKEN BY THE GENERAL CONTRACTOR WHEN DETERMINING THE LOCATION OF SJ'S AND CJ'S TO ENSURE SLAB JOINTS DOES NOT READ THROUGH THE ARCHITECTURAL FINISHES.

WAREHOUSE SLABS SHALL BE POWER-TROWELLED TO A HARD, SMOOTH BURNISHED FINISH. THE FINAL TROWEL PASS SHALL BE DONE BY MACHINE - NOT BY HAND. WITHIN 30 MINUTES OF THE FINAL TROWEL PASS, THE FLOOR SHALL BE CURED WITH EUCLID'S SUPER REZ-SEAL OR APPROVED EQUAL, WHICH MAY BE WAIVED AT THE OWNER'S OPTION.

SLAB THICKNESS SHALL BE INCREASED AS REQUIRED TO PROVIDE ADEQUATE SUPPORT FOR CRANE LOADS WITHOUT CRACKING SLAB.

ALL CONCRETE SLABS ON GRADE SHALL BE A MINIMUM OF 4" THICK AND BE REINFORCED WITH 6 X6 W I.4 X W I.4 EXCEPT WERE SPECIFICALLY NOTED ON PLANS. FIBERMESH CONCRETE INSTEAD OF WIRE MESH IS AN ACCEPTABLE ALTERNATE ON SINGLE STORY BUILDING AND MULTISTORY BUILDING WITHOUT LOAD BEARING FLOOR PADS. FIBERMESH SHALL BE IN COMPLIANCE WITH ASTM C-1116 TYPE III AND ASTM CIIG LEVEL I AND SHALL BE PLANT BATCH MIX WITH PROPORTIONS OF I.5 POUNDS OF FIBERMESH PER CUBIC YARD OF CONCRETE.

PLACE A MINIMUM 6 MIL POLYETHYLENE VAPOR BARRIER (LAPPED A MIN. OF 6") OVER COMPACTED SOIL BETWEEN FOUNDATION AND SLAB UNLESS NOTED OTHERWISE IN GEOTHECHNICAL ENGINEERS REPORT FOR THE PROJECT.

DESIGN CRITERIA

CONSTRUCTION DOCUMENTS WERE DESIGNED AND MEET THE REQUIREMENTS OF THE OF THE LOCAL BUILDING CODE DESIGNATED UNDER STRUCTURAL DESIGN CRITERIA. (SEE STRUCTURAL PLANS FOR DESIGN LOADS).

FOUNDATIONS

- CONFORMANCE WITH THE REQUIREMENTS OUTLINED IN THE GEOTECHNICAL SOILS REPORT SHALL BE DETERMINED BY PERFORMING INDUSTRY STANDARD SOIL DENSITY TESTS BY A CERTIFIED TESTING AGENCY.
- 2. IF SUBSURFACE INVESTIGATION FOR FOUNDATIONS HAS NOT BEEN PERFORMED AND GEOTECHNICAL ENGINEERING REPORT WAS NOT AVAILABLE FOR FOUNDATION DESIGN. PRIOR TO CONSTRUCTION, THE OWNER SHALL RETAIN THE SERVICES OF A QUALIFIED GEOTECHNICAL ENGINEER TO PERFORM SOILS BORINGS. PROVIDE RECOMMENDATIONS FOR FOUNDATION DESIGN (INCLUDING NET ALLOWABLE SOIL BEARING PRESSURE) PROVIDE EARTHWORK CONSTRUCTION CRITERIA AND PERFORM SOIL TESTING DURING CONSTRUCTION. THE OWNER / CONTRACTOR IS RESPONSIBLE FOR PERFORMING ALL EARTHWORK OPERATIONS IN STRICT ACCORDANCE WITH THIS REPORT. IF THE FOUNDATION RECOMMENDATIONS AND NET ALLOWABLE SOIL BEARING CAPACITY OR ANY OTHER ASSUMPTIONS (SEE BELOW) DIFFER FROM THE ASSUMED VALUE, THEN MODIFICATIONS TO THE STRUCTURAL DRAWINGS SHALL BE REQUIRED. SHOULD THIS OCCUR, THE OWNER / CONTRACTOR SHALL STOP CONSTRUCTION AND NOTIFY METAL BUILDING COMPANY AND THE ENGINEER OF RECORD IMMEDIATELY.
- FOUNDATION PLANS HAVE BEEN DESIGNED WITH THE FOLLOWING ASSUMPTIONS IN THE ABSENCE OF A SUBSURFACE INVESTIGATION BY A GEOTECHNICAL ENGINEER. NET ALLOW. SOIL BEARING PRESSURE OF 2,000 PSF.
- CONDITIONS DISCOVERED BY THE CONTRACTOR AND/OR GEOTECHNICAL FIELD REPRESENTATIVE DURING EXCAVATION WHICH MAY PREVENT THE ATTAINEMENT OF THE ALLOWABLE BEARING PRESSURE STATED IN THE GEOTECHNICAL SOILS REPORT, SHALL BE REPORTED TO THE ENGINEER.
- 5. THE SHEETLEDGES, RAINLIPS AND MASONRY LEDGES ARE VITAL TO THE PROPER FIT OF THE STEEL CONSTRUCTION. OWNER / CONTRACTOR SHALL FIELD VERIFY ALL TO BE AS SHOWN ON PLANS. IF THEY ARE NOT TO THE DRAWINGS CONTACT METAL BUILDING COMPANY PRIOR TO FABRICATION OR BEFORE STEEL ERECTION STARTS.
- 6. ALL ISOLATED PAD FOOTING ARE TO BEAR A MINIMUM OF 18" BELOW THE TOP OF CONCRETE SLAB / PAVEMENT OR A MINIMUM OF 12" BELOW FINISHED GRADE U.N.O. ON PLANS.
- 7. THE MINIMUM BEARING WIDTH OF CONTINUOUS FOOTINGS SHALL NOT BE LESS THAN I 2" UNLESS SPECIFICALLY NOTED IN GEOTECHNICAL ENGINEERING REPORT.
- 8. PLANS AND DETAILS REFLECT A DESIGN TO ACCOMMODATE A MAXIMUM FROST PROTECTION OF 12" UNLESS NOTED OTHERWISE IN THE CONSTRUCTION DOCUMENTS. IF THE REQUIRED FROST DEPTH EXCEEDS 12" THAN ENGINEER OF RECORD SHALL BE NOTIFIED PRIOR TO EXCAVATION OR FOUNDATIONS.



AND RE	PRESENTS THE	DLE SHOWN BELOW RECOMMENDED WE	IS FROM THE VUL	lkafi Metal DE(anual
	DECK TYPE	TOTAL SLAB DEPTH <= 4 3/4"	RECOMMEN	DED WELDED WIR	E FABRIC	
-	1.5VL,VLIorR	> 4 3/4"	6X6-W	/2. X W2.		
	2VLI	<= 5 1/4"	6 X 6 - W	/I.4 X WI.4		
F	2VLI	> 5 /4"	6X6-W	72.1 X W2.1		
F	3VLI	> 6 /4"	6 X 6 - W	/2. X W2.		
	ABLE ALTERNAT	TIVES:		IC TYPE L CONT	TAININIA STEE	FIRFPC
M Di 25 Co M Di 4	EETING THE CRI ETERMINED BY 1 5LB/CU YD IS AI ONCRETE SPECI ACROSYNTHETIN ETERMINED BY 1 B/CU YD IS AN	TERIA OF ASTM A82 THE FIBER MANUFAC N ACCEPTABLE ALTER IFIED IN ACCORDANC C FIBERS MEETING T THE FIBER MANUFAC	20, TYPE I, TYPE II, TURER FOR THE AF RNATIVE TO THE W CE WITH ASTM CI I THE CRITERIA OF AS TURER FOR THE AF	OR TYPE V, AT A PPLICATION, BUT WF SPECIFIED AB I G, TYPE III, COI 6TM D7508 AT A PPLICATION, BUT	DOSAGE RATE NOT LESS THAI OVE. JTAINING DOSAGE RATE NOT LESS THAI	N N N
RETE: NCRET	TE SHALL HAVE	THE FOLLOWING MIN	N. PROPERTIES:	28 DAY STRENGTH	SLUMP	MAX AGGR.
	FOUNDATIC	DN		3,000 psi	4" <u>+</u> <u>#</u> "	/2"
	SLAB-ON-G SLAB-ON-G	GRADE (UP TO 4" THIC GRADE (OVER 4" THIC	CK) CK)	3,000 psi 4,000 psi	4" + 1"	/2"
	TIE BEAMS			4,000 psi	4" + "	3/4"
	CAST-IN-PI	ACE BEAMS		3,000 psi 4,000 psi	4" <u>+</u> " 4" + "	3/4" "
	CAST-IN-PL	ACE COLUMNS		4,000 psi	4" + "	"
	EQUIPMENT	NELS		4,000 psi 4,000 psi	4" + "	"
	GROUT UNE	DER TILT-UP PANELS		5,000 psi	8" + "	3/8"
	ELEVATED S	BLABS FORMED AND	POURED	4,000 psi	4" + "	"
	GROUT FOR	R FILLED CELLS		2,500 psi	8" <u>+</u> "	3/8"
	3. COLD JO SHOULD CONCRETE PRO PROCEDURES E AND CYLINDER ARCH./ENG. FO CONCRETE WOI STRUCTURAL C	DINTS ARE NOT RECO DE PLACED A MINIP DPERTIES SHALL BE BY A CERTIFIED TEST BEAKS FOR COMPR R REVIEW. RK SHALL CONFORM ONCRETE FOR BUILD	OMMENDED - ALTH MUM OF 2'-O" OFF /ERIFIED THROUGH ING AGENCY. MIN ESSIVE STRENGTH I TO LATEST EDITIC DINGS, ACI 3 1 8 BL	IOUGHT IF REQUI CENTERLINE OF C I INDUSTRY STAN TEST REQUIRED FINDINGS SHAL ONS OF ACI 30 I JILDING CODE REC	RED THEY COLUMNS. DARD TESTING SHALL INCLUDI L BE SUBMITTE SPECIFICATION: QUIREMENTS F	E SLUMP ED TO THE S FOR OR
~		ONCRETE AND ACI 3	15 DETAILS AND D	PETAILING OF CON	ICRETE REINFO	RCEMENT
C	UNCRETE MIX [DESIGN SHALL MEET	THE FULLOWING C	KIIEKIA:	FTHOD I OP	
	2. METHOD 2	יוא ערטאטע אווא טעדע אווא 2 .	. DL AUCUNDANCE	WITT ACLOUT M	LINUV I UK	
	3. ENTRAPPE 4. ADMIVTUR	D AIR CONTENT SHA	LL NOT EXCEED 39	Ceptable διι C	ONCRETE TO F	BE
	NORMAL V	WIGHT WITH A DESIG	GN STRENGTH AT 2	8 DAYS.	_	
SI	TE ADDED WAT	ER IS NOT ACCEPTAL	BLE. ADDING WATI	ER TO THE MIX WI	LL RESULT IN R	EJECTION
C	ONTRACTOR IS	RESPONSIBLE FOR	THE ADEQUACY OF	THE FORMS AND	SHORING AND) FOR
5	AFE PRACTICE I	N THEIR USE AND RE	EMOVAL.			
F E T FI N	XCEEDING 7 1/2 UBULAR SHUTE EET. ALTERNATI OT BE USED UN	2 FEET IN HEIGHT. C S LOCATED SUCH TH E PLACEMENT METHONICS APPROVED BY	ONCRETE SHALL B IAT THE FREE AIR I OD OF CONCRETE ENGINEER OF REC	E PLACED THROU DROP OF THE MIX WITH OR WITHOU CORD.	GH "ELEPHANT DOES NOT EX ADMIXTURES	TRUNK CEED G SHALL
	MONOLITHIC S	BLAB FINISHES				
	THE FOLLOWIN / FLOOR LEVEL THESE REQUIR	IG REQUIREMENTS A NESS (FL) VALUES/M EMENTS AND ENFOR	RE BASED ON THE ETHODS. BIDS FO CEMENT THEREOF	LATEST FLOOR F OR THIS WORK SH CAN BE EXPECTE	LATNESS (FF) IALL REFLECT D.	
	NON-CRITICAL I. FLOAT FII 2. SPECIFIE 3. MINIMUM 4 ∧PPI∨FI	FLOOR TOLERANCE NISH (FLT-FN) D OVERALL VALUE: 1 LOCAL VALUE: FF2	FF25/FL20 20/FL17	PFACES THAT ADE		
	MUD SET TO BE CO	TILE AND OTHER TH	NORMAL STEEL	TO SLAB SURFA	CES WHICH AR	2E
	TROWEL FINIS	H (TR-FN)	, NUINIVIAL JIZEU K			
	 SPECIFIE MINIMUM APPLY TR WOOD FI 	D OVERALL VALUE: F 1 LOCAL VALUE: FF2 20WEL FINISH TO MC LOORING, CARPET, F	F30/FL23 25/FL20 DNOLITHIC SLAB SI PAINT, OR OTHER T	JRFACES THAT AI HIN FILM FINISH (RE TO RECEIVE COATING SYSTI	EM.
	LARGE ROOMS			16.		
	INVEL FINIS	0 (GUT DI AND UVLI	R) AND PUBLIC ARE	AJ:		
	I. SPECIFIE 2. MINIMUM	H 2 (TR-FN2) D OVERALL VALUE: A LOCAL VALUE: FF3	R) AND PUBLIC ARE FF3G/FL25 30/FL22	IPFACES TUAT A		
	 SPECIFIE MINIMUN APPLY TR RESILIEN 	H 2 (TR-FN2) D OVERALL VALUE: A LOCAL VALUE: FF3 ROWEL FINISH TO MO T FLOORING, CARPE	R) AND PUBLIC ARE FF3G/FL25 30/FL22 DNOLITHIC SLAB S T, PAINT, OR OTHE	URFACES THAT A R THIN FILM FINIS	RE TO RECEIVE 5H COATING SY	′STEM.
	 SPECIFIE MINIMUM APPLY TR RESILIEN ELEVATED SLAN MINIMUM LOCA 	B (GOT ST AND OVER H 2 (TR-FN2) D OVERALL VALUE: A LOCAL VALUE: FF3 ROWEL FINISH TO MO T FLOORING, CARPE BS SHALL HAVE A ST AL OF FF20 WITH NO	R) AND PUBLIC ARE FF3G/FL25 30/FL22 DNOLITHIC SLAB S T, PAINT, OR OTHE PECIFIED OVERALL D FL NUMBER DEFI	URFACES THAT AI R THIN FILM FINIS VALUE OF FF22 ⁻ NED.	RE TO RECEIVE 5H COATING SY TO FF27 AND A	'STEM.
	 SPECIFIE MINIMUM APPLY TR RESILIEN ELEVATED SLAI MINIMUM LOCA FLOOR TOLERA FLOOR SLABS IN ASTM E I 15 MAY BE WAIVE MIN = A/10. If TESTING LABO OPERATION, A SHALL BE TAKE THE EDWARD N TESTS. INCLUS 	B (GOT ST AND OVER H 2 (TR-FN2) D OVERALL VALUE: A LOCAL VALUE: FF3 ROWEL FINISH TO MO T FLOORING, CARPE BS SHALL HAVE A ST AL OF FF20 WITH NO SHALL BE CONDUCT STALL BE CONDUCT STALL BE CONDUCT STALL BE CONDUCT STALL BE CONDUCT STATORY WITH THE EXO FLOOR TOLERANCE N PRATORY WITHIN 24 ND BEFORE FORMS EN USING A DIPSTICI W. FACE COMPANY, DING A FORMAL NOT	R) AND PUBLIC ARE FF3G/FL25 30/FL22 DNOLITHIC SLAB S T, PAINT, OR OTHE PECIFIED OVERALL D FL NUMBER DEFIN S: FLOOR FLATNE TED IN ACCORDAN CEPTION OF SUBPA OPTION, ALSO ZI C MEASUREMENTS SI HOURS AFTER CON AND SHORES HAV K AUTO-READ FLOC INC. OF NORFOLK, ICE OF ACCEPTANC	URFACES THAT AI R THIN FILM FINIS VALUE OF FF22 VED. SS AND LEVELNE CE WITH THE PRO RAGRAPHS 7.2.3 ALCULATION THE HALL BE MADE BY MPLETION OF THE E BEEN REMOVED OR PROFILER AS I VA. RESULTS O CE OR REJECTION	RE TO RECEIVE 5H COATING SY TO FF27 AND A ESS TESTS ON 2VISIONS SET F 3 AND 7.3.2 W 3E USED SHALI 7 THE INDEPENE FINAL TROWEL 0. MEASUREME MANUFACTURE F FLOOR TOLER 0F THE WORK	STEM.

HANDRAILS, POSTS AND SUPPORT CONNECTIONS SHALL BE DESIGNED BY A SPECIALTY ENGINEER, HIRED BY THE STEEL FABRICATOR. SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED & SEALED BY A ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. DESIGN LOADING(S) SHALL CONFORM TO AT LEAST THE MINIMUM REQUIREMENTS OF THE APPLICABLE BUILDING CODE. (SEE DESIGN CRITERIA).

LOCATION. MINIMUM DESIGN LIVE LOAD SHALL BE 100 PSF.

MASONRY

SPECIFIED IN THESE CONTRACT DOCUMENTS.

f'm=1,500psi (1,900psi ON THE NET AREA)

COMPRESSIVE STRENGTH)

BLOCK SHALL NOT BE MOISTENED BEFORE GROUTING.

GROUT FOR FILLED CELLS SHALL BE PLACED AS INDICATED BELOW



GROUTATINGS DEFTALL HOUR OR MORE SHOULD BE

COURSE. DO NOT TOOTH. CLEAN EXPOSED SURFACES OF SET UNITS AND MORTAR PRIOR TO LAYING FRESH MASONRY.

TRUSS BEARING LOCATIONS - (TYP. UNO)



ISSUE

30' APART NOR 15' OFF ANY CORNER OR RETURN.



8'-5" LL TOP BARS ARE DEFINED AS HORIZ. BARS WITH MORE THAN 12" OF CONCRETE BELOW BARS

7'-7"

|'-|0" 2'-5" 3'-0" 3'-7" 5'-3" 6'-0" 6'-9"

| |/2" 3/4"

| |/2"

INTERSECTION

LENGTHS

OTHERS

BE SIGNED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT



- II. MAINTAIN STRUCTURAL SLAB THICKNESS AT ALL FLOOR SLOPES AND DEPRESSIONS.



GC SHALL COORDINATE ALL T/CMU WALL HEIGHTS WITH RABCO ENTERPRISES, LLC PRIOR TO CONSTRUCTION TO ENSURE PROPER FIT OF THE METAL BUILDING COMPONENTS. FAILURE TO GET WRITTEN APPROVAL PRIOR TO ERECTING THE CMU WALLS MAY RESULT IN NEEDED MODIFICATIONS TO EITHER THE CMU WALL AND/OR THE METAL BUILDING COMPONENTS AT THE G.C.'S EXPENSE.

SCALE

SHEET

AS NOTED

SF-2.0

ł	21'-3" TO MATCHLINE	*	23'	/			70'	
MATCHLINE		- +	5 SFD-J					ভা
		<u> </u>						
	=							
	47'-22	2 5FD-1			23' 5 6FD-1		47'-2 ¹ "	
50'		N	ł		N	ł		

SINGLE STORY FOUNDATION PLAN NOTES:

- I. FLOOR SLAB SHALL BE 4" THICK, FC'=3,000ps1 CONCRETE REINF W/ 6x6 W I .4xW I .4 WWF OVER I O MIL VAPOR BARRIER (LAPPED A MIN. OF 6") ON COMPACTED SUBGRADE. (SEE SLAB ON GRADE DETAILS FOR PLACEMENT OF REINF)
- I a. FIBER REINFORCED CONCRETE IS AN ACCEPTABLE ALTERNATIVE TO WELDED WIRE FABRIC. REINFORCED CONCRETE FIBERS SHALL BE 100% VIRGIN POLYPROPYLENE, FIBRILLATED FIBERS AS MANUFACTURED BY FIBER MESH CO, OR APPROVED EQUAL APPLIED AT A RATE OF 1 1/21bs/CY.
- 2. T/ SLAB EL = 0'-0", UNO (REFERENCE ONLY) SEE CIVIL DWGS FOR ACTUAL ELEVATION.
- 3. T/INT. CMU WALL FTG EL = -1'-4", TYP UNO. T/EXT. CMU WALL FTG. EL = -1'-4", TYP. UNO (VERIFY W/ FROST DEPTH REQUIREMENTS)
- 4. T/INT. STL COL FTG EL = -1'-0", TYP UNO. T/EXT. STL COL FTG EL = - I '-4", TYP UNO (VERIFY W/ FROST DEPTH REQUIREMENTS)
- 5. ALL CMU BRG WALLS ARE 8", TYP UNO.
- 6. STEP FTG WHERE SHOWN AND AS REQUIRED TO AVOID INTERFERENCE W/ OTHER TRADES. SEE TYP STEPPED FTG DETAIL
- 7. ALL FTGS ARE CENTERED BENEATH BEARING WALLS AND COLUMNS, TYP UNO.
- 8. REINF CMU WALLS WITH #5 VERT BAR CENTERED IN GROUT FILLED CELL AT ENDS, CORNERS AND AT MAX SPACING OF 48"o.c. SEE "TYP FILLED CELL PARTIAL PLAN" FOR ADDITIONAL INFORMATION.
- 9. TYPICAL SPACING OF FILLED CELLS SHALL APPLY ABOVE AND BELOW OPENINGS ALSO. SEE GENERAL NOTES SHEETS FOR ADDITIONAL INFORMATION.
- 10. (2) #4 x 4'-0" LONG @ 3" C/C PLACED 2" CLEAR FROM CORNER, CENTERED IN SLAB, TYP.
- II. MAINTAIN STRUCTURAL SLAB THICKNESS AT ALL FLOOR SLOPES AND DEPRESSIONS.









NOTES:

- WITHOUT EXCEPTION, ALL RAINLIP, SHEETLEDGE AND BLOCKLEDGE SHALL BE 1-1/2" DOWN FROM THE T/FINISHED FLOOR ELEVATION. 2. WITHOUT EXCEPTION, ALL DOOR THRESHOLD SHALL BE $\frac{3}{4}$ " DOWN FROM THE T/FINISHED FLOOR ELEVATION.
- 3. G.C. SHALL VERIFY WIDTH OF RAINLIP, SHEETLEDGE, BLOCKLEDGE AND DOOR THRESHOLD WITH
- CONTRACT DOCUMENTS PRIOR TO FORMING SLAB EDGE. 4. IN THE ABSENCE OF RAINLIP, SHEETLEDGE, BLOCKLEDGE OR DOOR THRESHOLD BEING SPECIFIED ON THE PLANS, THE G.C. SHALL VERIFY IN WRITING WITH THE STEEL CONTRACTOR THE INTENT PRIOR TO FORMING
- SLAB EDGE. RAINLIP, SHEETLEDGE, BLOCKLEDGE AND DOOR THRESHOLD ARE VITAL TO THE PROPER FIT OF THE STEEL
- CONSTRUCTION. G.C. SHALL FIELD VERIFY THE RAINLIP, SHEETLEDGE, AND BLOCKLEDGE HAVE BEEN CONSTRUCTED IN
- ACCORDANCE WITH CONTRACT DOCUMENTS BEFORE STEEL CONSTRUCTION BEGINS.

NOTE:

GC SHALL COORDINATE ALL T/CMU WALL HEIGHTS WITH RABCO ENTERPRISES, LLC PRIOR TO CONSTRUCTION TO ENSURE PROPER FIT OF THE METAL BUILDING COMPONENTS. FAILURE TO GET WRITTEN APPROVAL PRIOR TO ERECTING THE CMU WALLS MAY RESULT IN NEEDED MODIFICATIONS TO EITHER THE CMU WALL AND/OR THE METAL BUILDING COMPONENTS AT THE G.C.'S EXPENSE.

			EAST COAST OFFICE: 1041 CROWN PARK CIRCLE WINTER GARDEN FL 34767 PHONE: (800)-989-0220		CONFIDENTIAL: THIS DOCUMENT AND THE INFORMATION CONTAINED HEREIN ARE THE PROPERTY OF MAKORABCO UNAUTHORIZED COPYING, DISCLOSURE OR OTHER UNAUTHORIZED USES ARE PROHIBITED.
		LIBERTY DELRAY.		UELKAY FL	
ENGIN This i signed Beatti and da docum and se verifie 08/0	EER S	STATION STATIO	IP M. L EN 554 ★ TE R IAL cop t cor te sij lectri	SEA SEA SEA SEA SEA SEA SEA SEA SEA SEA	ically t M. Signature this d signed e must be opties.
RBE CONSULTING SERVICES, LLC	2075 SPLIT OAK COURT - OVIEDO, FL. 32766 WEBDITE: WWW.RBECS.COM - EMALE: REECSLE@GMAIL.COM MAIN PHONE LINE: 407-796-3820 - FAX LINE: 407-971 -671 8	VECHANICAL DEFT: 407-796-3776 - EMAIL: NSHAH@RBECS.COM LIECTRICAL DEFT: 407-617-3113 - EMAIL: FAMILIBA@RBECS.COM TRUCTURAL DEFT: 407-568-7568 - EMAIL: RBEATTIE@RBECS.COM	ROBERT M. BEATTIE, PE; FLORIDA PE #55428 FLORIDA CERTIFICATE OF AUTHORIZATION #27269	REE CONSULTING SERVICES, LLC" HEREDY EXPRESSIVE RESERVED 15 COMMON LAW COPRIGHT AND OTHER PROFENT RIGHTS TO THEREE PAYAR AND THE PRECISE COMPLYING WATHIN	THE DESIGN AND DRAWINGS SHALL NOT TO BE ALTER, COFFED, REFRESOUNCE IN ANY TORM OR MANNER MITHOUT THE EXPERSION WATTEN FERMISSION AND CONENT OF YEE CONSULTING FERMISSION AND CONENT OF YEE CONSULTING
SHEET FC DATE DRAW CHECH JOB N SCALE SHEET				5. N PL 00	C ANS G-12-23 AW/MNG XXX E 2705 5 NOTED











Received 7.17.23

- IXI 2X26ga GALVALUME PARTITION ANGLE USED TO EXTEND PARTITION PANEL TO UNDER SIDE OF ROOF PURLIN[SEE PLAN].

PIFR -MULLION-∽CSI W/O CLIP ∽CSI W/O CLIP CSI W/O CLIP - CSI W/O CLIP CEE CEE COLUMN CEE COLUMN COLUMN CSI W/ CLIP CS-15 THIS SIDE OF COLUMN SAME HEIGHT OF CEE COLUMN CSI W/ CLIP-CSI W/O CLIP-CSI W/ CLIP - CSI W/O CLIP U-PANEL _ CEE CEE COLUMN U-PANEL CEE COLUMN COLUMN CSI W/ CLIP NOTE: CS-1'S TO BE THE SAME HEIGHT AS COLUMNS IN THE SAME ROW - CSI W/ CLIP CEE COLUMN CEE COLUMN See CEE COLUMN _CSIW/O CSI W/O CLIP CSI W/O CLIP CSI W/O CLIP CLIP MULLION -PIER-PIER — PIER — 10' 10'

´2 `

GENERIC FRAMING FOR PARTITIONS WITH CROSS PARTITIONS (N.T.S.) $\$ SD-3 / SCALE: 3/4" = 1'



5 \ SD-3 / SCALE: 3/4" = 1'

INSULATED WALL FRAMING





BERTY DELRA m Ë







ΧХ



ASCE 7 Hazards Report

Standard: ASCE/SEI 7-16 Latitude: 26.4649 Risk Category: II Section 11.4.3)

Longitude: -80.07419 Soil Class: D - Default (see Elevation: 19.6092138164712 ft (NAVD 88)





Wind

Results: Wind Speed 10-year MRI 25-year MRI

50-year MRI

100-year MRI

Data Source:

Date Accessed:

168 Vmph 90 Vmph 112 Vmph 127 Vmph 138 Vmph

ASCE/SEI 7-16, Fig. 26.5-1B and Figs. CC.2-1–CC.2-4, and Section 26.5.2 Tue Jun 27 2023

NOAA National Weather Service, Precipitation Frequency Data Server, Atlas 14

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-16 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

Site is in a hurricane-prone region as defined in ASCE/SEI 7-16 Section 26.2. Glazed openings shall be protected against wind-borne debris as specified in Section 26.12.3.

https://asce7hazardtool.online/

Page 1 of 6

Tue Jun 27 2023



Results:

15-minute Precipitation Intensity: 9.58 in./h

60-minute Precipitation Intensity: 5.23 in./h

Data Source:

Date Accessed:

https://asce7hazardtool.online/

(https://www.nws.noaa.gov/oh/hdsc/) Tue Jun 27 2023

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Tue Jun 27 2023

Received 8.9.23



Values provided are ground snow loads. In areas designated "case study required," extreme local variations in ground snow loads preclude mapping at

Snow load values are mapped to a 0.5 mile resolution. This resolution can create a mismatch between the mapped elevation and the site-specific elevation in topographically complex areas. Engineers should consult the local authority having jurisdiction in locations where the reported 'elevation' and

Tue Jun 27 2023

Tue Jun 27 2023

0 DELRAY AY FL ERTY | DELR/ m NGINEER STAMP BERT M. BEY No. 55428 * * STATE OF TON YORIDA This item has been electronically signed and sealed by Robert M. Beattie, PE using a Digital Signature and date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies. 08/04/2023 Digitally signed by Robert Beattie Date: 2023.08.04 14:46:44-04'00 NGINEER SHEET TITLE CODE ANALYSIS 06-12-23 DATE DRAWN BY AW/MNG CHECKED BY X JOB NO. E 2705 SCALE AS NOTED **SHEET**

S-0-0

PRELIMINARY NOT FOR PERMITTING ANALYSIS WILL BE PROVIDED

Received 8.2.23

WIDTH	HEIGHT	Vi
(FT)	(FT)	Vno
8	8	+
10	10	+
4	14	+
9	7	+
16	7	+

NOMINAL (ASD) GARAGE DOOR & ROLLING DOOR WIND LOADS FOR BUILDINGS IN EXPOSURE "B" W/ MEAN ROOF HEIGHT OF 30FT (PSF)

/ult	I OO MPH	I I O MPH	I 20 MPH	130 MPH	140 MPH	I 50 MPH	I 60 MPH	170 MPH	180 MPH	190 MPH	200 MPH
1011	78 MPH	85 MPH	93 MPH	IOI MPH	108 MPH	IIG MPH	124 MPH	132 MPH	139 MPH	147 MPH	155 MPH
		ROOF	ANGL	E 0 <u><</u>	I O DE	EGREE	5				
_	8.7	10.5	12.5	14.7	-19.3	19.6	22.3	25.1	28.2	31.4	34.8
	8.4	10.2	12.1	14.2	16.5	18.9	21.5	24.3	27.3	30.4	33.7
_	8.0	9.7	-12.8	13.5	-17.4	-20.0	20.5	23.1	25.9 -28.8	28.9	32.0
	/	ROO	F ANG	LE >	O DEC	GREES	/	/		2	/
-	9.6	-12.9	13.7 -15.5	16.1 -18.2	18.5	21.3	24.3	27.6	30.6 -34.6	34.2 -38.6	38.0
-	9.2	10.9	13.1	15.5 -17.2	17.7 -19.7	20.4	23.3 -26.0	26.4 -29.4	29.3 -32.6	32.7 -36.5	36.4 -40.6

NOTES: I. FOR EFFECTIVE AREAS OR WIND SPEEDS BETWEEN THOSE GIVEN ABOVE, THE LOAD MAY BE INTERPOLATED, OTHERWISE USE THE LOAD ASSOCIATED WITH THE LOWER EFFECTIVE AREA. 2. VALUES SHOWN ABOVE SHALL BE ADJUSTED FOR HEIGHT AND

EXPOSURE BY MULTIPLYING BY THE RELAVENT COEFFICIENT SHOWN IN THE CHART TO THE RIGHT. 3. PLUS (+) AND MINUS (-) SIGNS SIGNFY PRESSURES ACTING TOWARDS

AND AWAY FROM BUILDING SURFACES. 4. VALUES SHOWN IN TABLE ABOVE ALREADY INCLUDE THE LOAD REDUCTION FACTOR OF 0.6.

ADJUSTMENT FACTOR FOR BUILDING HEIGHT \$ EXPOSURE

MEAN ROOF HEIGHT	EXPO	OSURE CATEGO	DRY
(FT)	В	С	D
15	1.00	1.21	1.47
20	1.00	1.29	١.55
25	1.00	I.35	1.61
30	1.00	1.40	1.66
35	1.05	1.45	1.70
40	1.09	1.49	1.74
45	1.12	1.53	1.78
50	1.16	1.56	1.81
55	1.19	1.59	1.84
60	1.22	1.62	1.87

			EAST COAST OFFICE: 1041 CROWN PARK CIRCLE WINTER GARDEN FL 34787 FHONE: (800)-989-0220			CONFIDENTIAL: THIS DOCUMENT AND THE INFORMATION CONTAINED HEREIN ARE THE PROPERTY OF	
		LIBERTY DELRAY.					
ENGIN	IEERS	STAN	1P				
RBE CONSULTING SERVICES, A	2675 SPUT OAK COURT - OVEDO, FL. 32766 WEBSITE: WWW.RECS.COM - EMAIL: RECSILC@GMAIL.COM MAIN PHONE UNE: 407-796-3820 - FAX UNE: 407-971-5718	M MECHANICAL DEPT: 407-796.3776 - EMALI: NSHAH@REECS.COM ELECTRCAL DEPT: 407-517.3113 - EMALI: FAMILENA@REECS.COM 5TRUCTURAL DEPT: 407-568-7569 - EMALI: REEATLE@REECS.COM	ROBERT M. BEATTIE, PE; FLORIDA PE #55428 FLORIDA CERTIFICATE OF AUTHORIZATION #27269	"REE CONSULTING SERVICES, ILC" HEREBY EXPRESSLY RESERVICES, ILC" HEREBY COPYRIGHT AND OTHER REOPERTY RIGHTS TO	THESE PLANS AND THE DESIGN CONTAINED WITHIN. THIS DESIGN AND DRAWINGS SHALL NOT TO BE	ALTERE, COPTED, OR ERFENDUCTOT IN ANY FORM OR MANUER WITHOUT THE REPRESS WRITTEN FERMISSION AND CONSENT OF REPRESS ONSULTING REPRISES ON AND CONSENT OF REPRESSION	(1), 1), (1), (1), (1), (1), (1), (1), (

DATE DRAWN BY CHECKED BY JOB NO. SCALE **SHEET**

06-12-23 AW/MNG XXX E 2705 AS NOTED

S-0- |

						Edge Condi	ition -a- Ft.	8.00	Edg	e Condition	n - 0.6h-Ft.	14.70	Edge Conditio	on - 0.2h-Ft.	4.90									
								5 FT SPAN	N											2.5 FT SP	AN			
ROOFING SYSTEM: MBCI	SELF WT.	CAPACITY -	CAPACITY -				NET DES	IGN WIND P	RESSURE				ROOFING SYSTEM: MBCI	SELF WT.	CAPACITY	CAPACITY	l			NET DESIGN WIND	PRESSURE			
	PSF	PSF	PSF	ZONE 1'	ZONE 1	ZONE 2	ZONE 2e	ZONE 2n	ZONE 2r	ZONE 3	ZONE 3e	ZONE 3r		PSF	PSF	PSF	ZONE 1'	ZONE 1	ZONE 2	ZONE 2e ZONE 2	n ZONE 2r	ZONE 3	ZONE 3e	ZONE 3r
Final Comparison Values			Positive	16.00	16.00	16.89				16.89			Final Comparison Values			Positive	16.00	16.00	16.89			16.89		
		1	Negative	36.87	54.88	86.25				117.95				1	_	Negative	36.87	54.88	86.25			117.95		
24" 26GA UD	1.02	17.65	55.40	No	No	No	NA	NA	NA	No	NA	NA	24" 26GA UD	1.02	30.10	146.90	No	No	No	NA NA	NA	No	NA	NA
24" 26GA UD CLIPS	1.02	36.86	55.40	No	No	No	NA	NA	NA	No	NA	NA	24" 26GA UD CLIPS	1.02	86.67	146.90	Yes	Yes	Yes	NA NA	NA	No	NA	NA
24" 24GA UD	1.23	23.87	75.90	No	No	No	NA	NA	NA	No	NA	NA	24" 24GA UD	1.23	54.14	204.00	Yes	No	No	NA NA	NA	No	NA	NA
24" 24GA UD CLIPS	1.23	42.83	75.90	Yes	NO	NO	N/A	NA	NA	NO	NA NA	N/A N/A	24" 24GA UD CLIPS	1.23	76.04	204.00	Yes	Yes	NO	NA NA	NA NA	NO No	IVA	NA
	1.50	20.73	106.30	INO Voc	No	NO	NA NA	NA NA	INA MA	NO	NA NA	MA	24 22GA UD	1.50	74.95	290.90	Yes	Yes	NO		NA NA	NO No	N/A M/A	NA NA
24 22GA UD OLIPS	1.30	40.00	100.30	No	No	No	N/A M/A	N/A M/A	N/A M/A	No	NA MA	N/A N/A	24 22GA UD CLIPS	1.30	55 10	290.90	Voc	Vos	No	NA NA	NA NA	No	NA NA	
	1.32	4.95	101.00	Vos	No	No	MΔ	N/A	NA NA	No	NA NA	NA NA	18" 24GA UD CLIPS	1.32	113.00	272.00	Vos	Vos	Vos	NA NA	NA	No	NA NA	N/A N/A
18" 22GA UD	1.66	40.48	142 10	Yes	No	No	N/A	ΝΔ	NΔ	No	NΔ	NΔ	18" 22GA UD	1.66	77.67	395.90	Yes	Yes	No	NA NA	NA	No	NA NA	NΔ
18" 22GA UD CLIPS	1.66	66 13	142.10	Yes	Yes	No	NA	NA	NA	No	NA	NA	18" 22GA UD CLIPS	1.66	150.29	395.90	Yes	Yes	Yes	NA NA	NA	Yes	NA	NA
12" 24GA UD	1.00	52 00	123 00	Yes	No	No	NA	NA	NA	No	NA	NA	12" 24GA UD	1.60	95 33	408.00	Yes	Yes	Yes	NA NA	NA	No	NA	NA
12" 22GA UD	1.86	78.00	178.60	Yes	Yes	No	NA	NA	NA	No	NA	NA	12" 22GA UD	1.86	104.00	593.90	Yes	Yes	Yes	NA NA	NA	No	NA	NA
24" 24GA DL	1.23	40.84	79.00	Yes	No	No	NA	NA	NA	No	NA	NA	24" 24GA DL	1.23	63,75	204.00	Yes	Yes	No	NA NA	NA	No	NA	NA
24" 24GA DL CLIPS	1.23	61.12	79.00	Yes	Yes	No	NA	NA	NA	No	NA	NA	24" 24GA DL CLIPS	1.23	126.24	204.00	Yes	Yes	Yes	NA NA	NA	Yes	NA	NA
18" 22GA LOKSEAM	1.60		47.50	No	No	No	NA	NA	NA	No	NA	NA	18" 22GA LOKSEAM	1.60	55.00	200.00	Yes	Yes	No	NA NA	NA	No	NA	NA
16" 24GA LOKSEAM	1.34	-	36.20	No	No	No	NA	NA	NA	No	NA	NA	16" 24GA LOKSEAM	1.34	44.11	166.10	Yes	No	No	NA NA	NA	No	NA	NA
16" 22GA LOKSEAM	1.71		53.20	No	No	No	NA	NA	NA	No	NA	NA	16" 22GA LOKSEAM	1.71	65.39	200.00	Yes	Yes	No	NA NA	NA	No	NA	NA
12" 24GA LOKSEAM	1.41		47.70	No	No	No	NA	NA	NA	No	NA	NA	12" 24GA LOKSEAM	1.41	59.48	200.00	Yes	Yes	No	NA NA	NA	No	NA	NA
12" 22GA LOKSEAM	1.81		70.10	No	No	No	NA	NA	NA	No	NA	NA	12" 22GA LOKSEAM	1.81	82.55	200.00	Yes	Yes	No	NA NA	NA	No	NA	NA
26GA PBR SD	0.94	43.33	46.37	Yes	No	No	NA	NA	NA	No	NA	NA	26GA PBR SD	0.94	133.48	119.08	Yes	Yes	Yes	NA NA	NA	Yes	NA	NA
24GA PBR SD	1.14	44.56	44.80	Yes	No	No	NA	NA	NA	No	NA	NA	24GA PBR SD	1.14	126.37	125.69	Yes	Yes	Yes	NA NA	NA	Yes	NA	NA
								4 FT SPAN	N											3 FT SP	١N			
ROOFING SYSTEM MBCI	SELF WT.	CAPACITY -	CAPACITY -						DECOUDE				ROOFING SYSTEM: MBCI	SELF WT.	CAPACITY	CAPACITY					DDESCUDE			
NOOT ING OTOTEM. INDOI	PSF	PSF	PSF	ZONE 1'	ZONE 1	ZONE 2	ZONE 20		ZONE 2r	ZONE 3	ZONE 30	ZONE 3r		PSF	PSF	PSF	ZONE 1'	ZONE 1	ZONE 2		70NE 2r	ZONE 3	ZONE 30	ZONE 3r
Final Comparison Values			Positive	16.00	16.00	16.89	ZONE Ze	ZONE ZI	ZONE ZI	16.89	ZONE Je	ZONE SI	Final Comparison Values			Positive	16.00	16.00	16.89	ZONE ZE ZONE Z		16.89	ZONE Je	ZONE JI
			Negative	36.87	54.88	86.25				117.95						Negative	36.87	54.88	86.25			117.95		
24" 26GA UD	1.02	22.63	91.80	No	No	No	NA	NA	NA	No	NA	NA	24" 26GA UD	1.02	27.61	122.40	No	No	No	NA NA	NA	No	NA	NA
24" 26GA UD CLIPS	1.02	56.78	91.80	Yes	Yes	No	NA	NA	NA	No	NA	NA	24" 26GA UD CLIPS	1.02	76.70	122.40	Yes	Yes	No	NA NA	NA	No	NA	NA
24" 24GA UD	1.23	35.98	127.50	No	No	No	NA	NA	NA	No	NA	NA	24" 24GA UD	1.23	48.09	170.00	Yes	No	No	NA NA	NA	No	NA	NA
24" 24GA UD CLIPS	1.23	56.12	127.50	Yes	Yes	No	NA	NA	NA	No	NA	NA	24" 24GA UD CLIPS	1.23	69.40	170.00	Yes	Yes	No	NA NA	NA	No	NA	NA
24" 22GA UD	1.56	47.22	185.60	Yes	No	No	NA	NA	NA	No	NA	NA	24" 22GA UD	1.56	65.70	247.50	Yes	Yes	No	NA NA	NA	No	NA	NA
24" 22GA UD CLIPS	1.56	63.59	185.60	Yes	Yes	No	NA	NA	NA	No	NA	NA	24" 22GA UD CLIPS	1.56	78.64	247.50	Yes	Yes	No	NA NA	NA	No	NA	NA
18" 24GA UD	1.32	37.02	170.00	Yes	No	No	NA	NA	NA	No	NA	NA	18" 24GA UD	1.32	49.13	226.70	Yes	No	No	NA NA	NA	No	NA	NA
18" 24GA UD CLIPS	1.32	74.09	170.00	Yes	Yes	No	NA	NA	NA	No	NA	NA	18" 24GA UD CLIPS	1.32	100.63	226.70	Yes	Yes	Yes	NA NA	NA	No	NA	NA
18" 22GA UD	1.66	55.36	239.30	Yes	Yes	No	NA	NA	NA	No	NA	NA	18" 22GA UD	1.66	70.23	329.90	Yes	Yes	No	NA NA	NA	No	NA	NA
18" 22GA UD CLIPS	1.66	99.79	239.30	Yes	Yes	Yes	NA	NA	NA	No	NA	NA	18" 22GA UD CLIPS	1.66	133.46	329.90	Yes	Yes	Yes	NA NA	NA	Yes	NA	NA
12" 24GA UD	1.48	69.33	255.00	Yes	Yes	No	NA	NA	NA	No	NA	NA	12" 24GA UD	1.48	86.66	340.00	Yes	Yes	Yes	NA NA	NA	No	NA	NA
12" 22GA UD	1.86	88.40	335.10	Yes	Yes	Yes	NA	NA	NA	No	NA	NA	12" 22GA UD	1.86	98.80	494.70	Yes	Yes	Yes	NA NA	NA	No	NA	NA
24" 24GA DL	1.23	50.01	127.50	Yes	No	No	NA	NA	NA	No	NA	NA	24" 24GA DL	1.23	59.17	170.00	Yes	Yes	No	NA NA	NA	No	NA	NA
24° 24GA DE CLIPS	1.23	87.17	127.50	Yes	Yes	Yes	NA	NA	NA	NO	NA	NA NA	24" Z4GA DE CLIPS	1.23	113.22	170.00	Yes	Yes	Yes	NA NA	NA	NO	NA	NA NA
18" 22GA LOKSEAM	1.60	41.60	81.80	Yes	No	No	NA	NA	NA	No	NA	NA NA	18" 22GA LOKSEAM	1.60	46.80	145.30	Yes	No	No	NA NA	NA	No	NA	NA NA
10 Z4GA LUKSEAM	1.34	53.22 E6.0E	04.90	NO Voc	NO Voc	NO No	N/A N/A	NA NA	INA NA	NO	N/A M/A	IVA NIA		1.34	30.33	115.30	NO Voc	N0	INO No	NA NA	IVA NA	NO	NA MA	NA NA
10 ZZGA LUKSEAM	1./1	50.05	91.30	res	res	110	IVA	INA	INA I	NO	IVA	INA I	10 ZZGA LUKSEAM	1.71	01.00	102.40	res	res	INO		IVA	NO	N/A M/A	N/A
	1 / 1	/ 6 211	QE 10	Vaa	No	No	NI A	N/A	NI A	No	A/A	AIA I	12" 24CALOKSEAM	1 / 1	52 00	151 20	Vaa	NIC	No				A second s	1 IVA
12" 24GA LOKSEAM	1.41	46.80	85.10	Yes	No	No	NA	NA NA	NA NA	No	NA NA	NA NA	12" 24GA LOKSEAM	1.41	52.00 78.00	151.30	Yes	No	No	NA NA NA NA	NA NA	No	NA	MΔ
12" 24GA LOKSEAM 12" 22GA LOKSEAM 26GA PBR SD	1.41 1.81 0.94	46.80 72.80 75.08	85.10 119.10 52.22	Yes Yes	No Yes Yes	No No	NA NA	NA NA	NA NA	No No	NA NA	NA NA NA	12" 24GA LOKSEAM 12" 22GA LOKSEAM 26GA PBR SD	1.41 1.81	52.00 78.00	151.30 200.00 119.08	Yes Yes	Yes Yes	No No	NA NA NA NA	NA NA NA	No	NA	NA
12" 24GA LOKSEAM 12" 22GA LOKSEAM 26GA PBR SD 24GA PBR SD	1.41 1.81 0.94 1.14	46.80 72.80 75.08 71.08	85.10 119.10 52.22 70.70	Yes Yes Yes	No Yes Yes Yes	No No No	NA NA NA	NA NA NA NA	NA NA NA NA	No No No	NA NA NA	NA NA NA NA	12" 24GA LOKSEAM 12" 22GA LOKSEAM 26GA PBR SD 24GA PBR SD	1.41 1.81 0.94 1.14	52.00 78.00 133.48 126.37	151.30 200.00 119.08 125.69	Yes Yes Yes Yes	No Yes Yes Yes	No No Yes Yes	NA NA NA NA NA NA NA NA	NA NA NA NA	No No Yes Yes	NA NA NA NA	NA NA NA

Roof Span Comparison

Received 8.9.23

NULL FILL	
RISE(in/ft)	DEG
0.250	1.19
0.375	1.79
0.500	239
0.625	298
0.750	3.58
0.875	417
1.000	4.76
1.125	5636
1.250	595
1375	654
1500	713
1625	7.71
1.320	830
1.875	8.89
200	9/6
2125	10.40
2250	10.07
220	11.20
25/5	11.20
200	10.24
2020	12.04
2/30	12.91
20/0	14.04
3.00	14.04
3.120	14.00
3.20	45.74
3.3/0	10.71
3.500	16.20
3.625	16.81
3.750	17.30
3.8/5	17.90
400	18.43
4.125	18.97
4:250	19.50
4.3/5	20.03
4.500	2055
4.625	21.08
4.750	21.60
4.875	2211
5.000	2262
5.125	23.13
5.250	2363
5.375	24.13
5.500	24.62
5.625	25.11
5.750	25.6
5.875	26.09
e m	2657

Diagrams

V	ot	ation	
ļ	=	10% of l	(
		3 ft (0.9)	1

3 ft (0.9 m).

Notes

TYPICAL ULTRA DEK ROOFING SYSTEM		
(SEE ELEVATIONS FOR ROOFING SYSTEM BY STRUCTURE)	LOAD BEAR	RING
	18	
24" 26GA UD	2	
24" 26GA UD WITH EXTERNAL WIND CLAMPS	N/A	
24" 24GA UD	2	
24" 24GA UD WITH EXTERNAL WIND CLAMPS	N/A	
24" 22GA UD	2	
24" 22GA UD WITH EXTERNAL WIND CLAMPS	N/A	
18" 24GA UD	2	
18" 24GA UD WITH EXTERNAL WIND CLAMPS	N/A	
18" 22GA UD	2	
18" 22GA UD WITH EXTERNAL WIND CLAMPS	N/A	
12" 24GA UD	2	
I 2" 22GA UD	N/A	
	•	



sustainable *k* insulation.

Specification Sheet

Metal Building Insulation 202-96

Composition and Materials: The product is composed of tan, uniformly textured, inorganic fibrous

pre-engineered metal buildings. It should be laminated on a first-in, first-out basis and should be kept.

dry at all times during processing and end use. After lamination, packaging should not exceed a 5.5:1

compression ratio. For additional information, please refer to the appendix of the NAIMA 202-96 (Rev.

Sizes: Standard available sizes as noted in table below. Contact CertainTeed for non-standard sizes.

Limitations: This product is designed for use in interior (weather protected) walls and roofs of

PRODUCT DESCRIPTION

2000) standard.

Basic Use: CertainTeed Fiber Glass Metal Building Insulation 202-96 is a flexible blanket insulation furnished in rolls and intended to be laminated on one side with a suitable vapor netarder. It is used as a thermal and acoustical insulation in the roofs and sidewalls of pre-engineered metal buildings and post frame construction.

Benefits: Metal Building Insulation 202-96 reduces transmission of exterior sound to the interior of the building and absorbs reverberating sounds within the building.

glass and formed with a formaldehyde-free binding agent.



CertainTeed Metal Building Product Name Insulation 202-96 CertainTeed Corporation Manufacturer P.O. Box 860 Address Valley Forge, PA 19482-0105 Phone 610-341-7000 + 800-233-8990 Fax 610-341-7571

www.certainteed.com/insulation

TECHNICAL DATA

Website

Applicable Standards

 Model Building Codes: -100

- Material Standards:
- AST M C991, Type I - NAIMA 202-96 (Rev. 2000) Fire Resistance

Fire Hazard Classification:

- UL 723, AST M E84, NFPA 255 Max. Flame Spread Index: 25 Max. Smoke Developed Index: 50 - CANAULC-S102-M88
- Non-combustible:

4.40

- ASTM E136 / Meets requirements sical/Chemical Properties

- Thermal Resistance: - ASTM C518 and/or ASTM C177 at 75°F (24°C)
- mean temperature: see table at left Acoustical Performance: see tables on other side
- Water Vapor Sorption: - ASTM C1104 / No greater than 5.0% by weight
- Corrosiveness: - ASTM C665 / Meets requirements for steel,
- copper and aluminum Odor Emission:
- ASTM C1304 / Pass
- Fungi Resistance:
- AST M C1338 / Pass Test Quality Assurance

CertainTeed's commitment to quality and environmental management has ensured the registration of the Athens, Chowchilla and Kansas City plants to ISO 9001:2000 and ISO 14001: 2004 standards.



R-Value	Nominal	Thickness		Width	ler	rgth
R	in.	mm	in.	mm	ft.	m
10	33%	762	36,48,60,72	914, 1219, 1524, 1829	100	90.5
11	334	889	36,48,60,72	914, 1219, 1524, 1829	100	30.5
13	43%	101.6	36,48,60,72	914, 1219, 1524, 1829	75	229
16	5%	133.35	36,48,60,72	914, 1219, 1524, 1829	50	152
19	6%	127	36,48,60,72	914, 1219, 1624, 1829	50	152
21*	634	1524	36,48,60,72	914, 1219, 1524, 1829	45	13.7
25	8	203.2	36,48,60,72	914, 1219, 1524, 1829	30	9.1
30°	9%	203.2	36,48,60,72	914, 1219, 1624, 1829	25	76

Non-standard widths are available and subject to an upchange on an individual basis determined by manufacturer's capability, quantity, lead times and packaging availability. "R-21, R-25 and R-30 are made to order.

Received 8.9.23

www.certainteed.com/insulation/hvac-mechanical

AVAILABILITY AND COST

Manufactured and sold throughout the United State and Canada. For availability and cost, contact your local distributor or call CertainTeed Sales Support Group in Valley Forge, PA at 800-233-8990.

WARRANTY

In as much as CertainTeed has no control over installation design, installation workmanship, accessory materials or conditions of application, CertainTeed does not warrant the performance or results of any installation containing its products.

MAINTENANCE

An inspection and preventative maintenance program for the insulation and vapor retarder system is recommended to ensure optimum performance.

TECHNICAL SERVICES

Technical assistance can be obtained either from the local CertainTeed sales representative, or by calling CertainTeed Sales Support Group in Valley Forge, PA at 800-233-8990.

FILING SYSTEMS CertainTeed Pub. No. 30-25-056.

 Additional product information available upon request.

			SOUM	ID ABSORP	TION - UNF	ACED			~
R-Value	Nom. Ti	nickness	Abs	orption Coef	ficients @ 0	ctave Band F	requencies	(Hz)	NDO
R	in.	mm	125	250	500	1000	2000	4000	NRC
10	3%	86	0.29	0.82	1.02	0.94	0.96	0.98	0.95
11	3¾	95	0.39	0.91	1.01	0.92	0.93	0.98	0.95
13	43%	111	0.53	0.97	1.04	0.90	0.95	0.98	0.95
16	5¼	133	0.67	1.05	1.02	0.92	0.98	0.99	1.00
19	6%	162	0.89	1.22	1.02	0.98	1.01	1.00	1.05

SOUND TRANSMISSION

	Transmission Loss in dB at the Octave Frequencies								
Construction type	125	250	500	1000	2000	4000	Rating		
		RO	0FS						
No Insulation	12	13	19	24	30	32	24		
R-10 Faced 202-96 Insulation Over the Purlins	12	16	26	37	45	49	29		
R-19 Faced 202-96 Insulation Over the Purlins	13	20	30	41	49	51	32		
202-96 Insulation Over & Between the Purlins to Fill the Cavity (R-25 Combined)	14	24	34	44	53	56	36		
		WA	LLS						
No Insulation	12	14	19	19	20	27	21		
R-10 Faced 202-96 Insulation Over the Girts	13	16	25	32	37	46	28		
R-13 Faced 202-96 Insulation Over the Girts	13	17	26	33	38	47	29		
R-13 Faced 202-96 Insulation Over the Girts 3-5/8" Steel Studs on 24" Centers with 1/2" Gyp. Board on Interior	26	40	51	60	64	65	50		
R-13 Faced 202-96 Insulation Over the Girts 3-5/8 " Steel Studs on 24" Centers with R-11 Batts & 1/2 " Gyp. Poord on Interior	31	43	55	68	73	75	54		

Sound Transmission Class (STC) in accordance with ASTM E90. - Roof construction is 24ga, standing seam roof with 8" Z purlins on 5' centers. Wall construction is 2.9a, wall panels screwed to 8" Z girts placed on 7" centers.
 Interior metal furring wall studs were 3-5/8" by 25ga. on 24' centers.







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Insurance Bulletin 19

Fire Testing of Interior Separation Wall Joint

This test program was undertaken by the Metal Building Manufacturers Association and tested at Omega Point Laboratories to evaluate the fire performance of the junction between a one-hour fire rated wall assembly and the non-combustible metal roof panels. (Ref. Omega Point Project No. 16343-108145).

The fire exposure used to evaluate the joint was the timetemperature curve in ASTM E 119 for fire rated walls. There is no test method for evaluating the fire integrity of the joint where a fire rated wall meets the roof insulation under non-combustible metal roof panels. Also, building codes do not require this joint to be fire rated. This bulletin serves to answer questions raised by building and fire code officials concerning fire spread over the top of the fire rated wall via the vinyl vapor retarder.

The test was performed in the worst-case orientation, with purlins framed perpendicular to the wall framing of the one-hour wall. Therefore, the same performance can be expected with the wall framed in any orientation with respect to the purlins. The test also showed there is no need to cut the vapor retarder, so it can be continuous over the top of the wall.



Interior Separation Wall Joint



Note: Gypsum Wallboard not shown. Fire caulking shall fill all openings where gypsum wallboard fits around purlin penetrations.

The fire test was terminated at 1 hour and forty-one minutes, during which time no flaming occurred on the unexposed surface of the wall or roof at the joint. The construction details that successfully prevented flaming over the top of the wall in the test are shown below.

- 1. Metal Roof Panels No. 26 MSG min. galvanized, Galvalume®, or painted steel. 2. Batts or Blankets – Vinyl faced compressible glass fiber
- insulation weighing between 0.6 and 0.7 pcf. Installed at the bottom side of roof deck panels over top of purlins. Note: Fiberglass insulation with alternate facing materials can be used, if flame spread is less than or equal to 25, and has a smoke developed rating of less than or equal to 50 per ASTM E84. 3. Mineral Wool Batts – 2 inches thick, 8-9 pcf density, fire
- stop across top of wall. 4. Steel Roof Purlin - C or Z-shaped, minimum 8 inches deep,
- No. 16 MSG min. galvanized or painted steel. 5. Wallboard, gypsum - 5/8 inch thick, Type X gypsum wallboard.
- 6. Steel Studs Channel shaped min. 2 ¹/₂ inch wide, 1 ¹/₄ inch flanges, and 1/4 inch return steel studs of No. 25 MSG min. steel, spaced 24 inches O.C. max. (Ref. UL Design No. U425).
- 7. Floor and Ceiling Runners Channel shaped of 25 MSG min. steel with min. 1-inch long legs to accommodate main wall studs. Runner at the top of the wall attached to the bottom of the purlins. Inverted runner attached to the top of the wall runner between purlins in order to support the short length of steel studs capped with a runner to compress the mineral wool batts and the glass fiber insulation under the roof panels.
- 8. Fire Caulking Bead of fire caulk along the joint formed by the gypsum wallboard and the glass fiber insulation applied along the top of the wall and on both sides of the wall in accordance with the manufacturer's specifications. Fire caulking of all openings between the gypsum wallboard and the purlins on both sides of the wall.

ALPHA	ASSOCIATES, INC. • 145 LEHIGH AVE. •	LAKEWOOD, NJ 08701		
	T: 732.634.5700 • 800.631.5399 • F: VR-R PLUS	732.634.1430		
DESCRIPTION Rein	forced Polypropylene Film – Metalized P	olyester Film Laminate		
APPLICATIONS Met	al Building Insulation Facing	3		
CERTIFICATIONS Mee	ts all criteria for ASIM C1136 Types II, IV COMPOSITION	/ UL 723/ASTM E84, ULC-:	>102M fire testing	
	comrosilion	VALUES*		
COMPONENT	DESCRIPTION	English	Metric	
White Film	Polypropylene	0.0011 inch	28.0 micron	
Adhesive	Flame Resistant			
Reinforcement	Tri-directional Fiberglass	4/inch (MD)	20/100 mm (MD	
T:L.S.	Deliverter	4/inch (CD)	8/100 mm (MD)	
FIIFN	Polyester	0.0005 Inch	12.7 micron	
	TYPICAL PHYSICAL PROI	PERTIES		
		VALUES*		
CHARACTERISTIC	METHOD	<u>ENGLISH</u>	METRIC	
BASIS WEIGHT	Scale	13.2 lbs/1000 SF	64.1 g/SM	
THICKNESS	ASTM-D-1777	0.007"	178 microns	
PERMEABILITY (WVTR)	ASTM-E-96	0.02 Perms	1.15 ng/N's	
BURST STRENGTH	ASTM-D-774	100 psi	7.0 kg/cm ²	
PUNCTURE RESISTANCE	ASTM-C-1136	300 units (79 in-lbs)	9.0 Joules	
TENSILE STRENGTH	ASTM-C-1136	30 lbs/inch MD 30 lbs/inch CD	5.25 kN /m 5.25 kN /m	
ACCELERATED AGING	30 days @ 95% RH, 120 °F (49°C)	No Corrosion No Delamination	No Corrosion No Delamination	
TEMPERATURE RESISTANC	E	-40 °F to 220 °F	-40 °C to 104 °C	
WATER IMMERSION	24 hours @ 73°F (23°C)	No Delamination	No Delamination	
MOLD RESISTANCE	ASTM-C-665	No Growth	No Growth	
LIGHT REFLECTIVITY	ASTM-C-523	85%	85%	
	Physical properties are based upon nominal values.	Weight / Thickness +/- 10%		
FIRE TESTING	UL723/AS	TM E84/CAN ULC-5102M		
	FLAME SPREAD	<u>Composite</u> 25		

intense exposure to UV sources such as direct or indirect sunlight and/or high-output UV lighting is not recommended as these will degrade the plastic film facing.

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RBE CONSULTING SERVICES,	LLC 2875 SPUT OAK COURT - OVIEDO, FL. 32766 WEBSITE:	WWW.RBECS.COM - EMAIL: RBECSLIC@GMAIL.COM MAIN PHONE LINE: 407-796-3820 - FAX LINE: 407-971-6718	MICCANICAL DETT: 407-796-3776 - EMALI VISHAPREECS.COM ELECTRICAL DETT: 407-5167-3113 - EMALI. FAMILEIAGREECS.COM STEUCTURAL DETT: 407-568-7595 - EMALI. REATTEGEBECS.COM	ROBERT M. BEATTIE, PE, FLORIDA PE #55428 FLORIDA CERTIFICATE OF AUTHORIZATION #27269	REE CONSULTING SERVICES, LLC" HEREBY EXPRESSLY RESERVES ITS COMMON LAW COPYRIGHT AND OTHER PROPERTY RIGHTS TO	THIS TEAKS AND THE DESAK CAN AND WITHIN. THIS DESKI AND RAWINGS SHALL NOT TO BE ALTERED. COFFID, OR ERPRODUCTION RAYLYORM OR MANURE WITHOUT THE EXPRESS WATTEN PREMISSION AND CONSENT OF "REE CONSULTING SERVICES, LLC"
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	STATEMENT OF S	PEC	IAL INSPECTIO	N						
ALL REQUIRED SPECIAL INSPECTIONS SHALL BE AS INDICATED BELOW. NO ADDITIONAL REQUIREMENTS FOR SPECIAL INSPECTION OR TESTING FOR SEISMIC OR WIND RESISTANCE SHALL BE REQUIRED.										
SCHEDULE OF SPECIAL INSPECTION SERVICES										
PROJECT										
	SERVICE	V/N	AS APPLICABLE TO	O THIS PROJECT						
1704.2.5 Inspection of Fabricators	SERVICE	1710		AGENT	DATE COMPLETED					
Verify fabrication/quality control procedures	In-plant review (3)	N	Periodic							
1705.1.1 Special Cases (work unusual in nature, including but not limited to alternative materials and systems, unusual design applications, materials and systems with special manufacturer's requirements)	Submittal review, shop (3) and/or field inspection	N								
1705.2 Steel Construction										
1. Fabricator and erector documents (Verify reports and certificates as listed in AISC 360, chapter N, paragraph 3.2 for compliance with construction documents)	Submittal Review	N	Each submittal							
2. Material verification of structural steel	Shop (3) and field inspection	Y	Periodic							
3. Embedments (Verify diameter, grade, type, length, embedment. See 1705.3 for anchors)	Field inspection	Y	Periodic							
 Verify member locations, braces, stiffeners, and application of joint details at each connection comply with construction documents 	Field inspection	Y	Periodic							
5. Structural steel welding:										
 a. Inspection tasks Prior to Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-1) 	Shop (3) and field inspection	Y	Observe or Perform as noted (4)							
b. Inspection tasks During Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-2)	Shop (3) and field inspection	Y	Observe (4)							
c. Inspection tasks After Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-3)	Shop (3) and field inspection	Y	Observe or Perform as noted (4)							
d. Nondestructive testing (NDT) of welded joints: see Commentary		Y								
1) Complete penetration groove welds 5/16" or greater in <i>risk</i> <i>category</i> III or IV	Shop (3) or field ultrasonic testing - 100%	Y	Periodic							
2) Complete penetration groove welds 5/16" or greater in <i>risk</i> <i>category</i> II	Shop (3) or field ultrasonic testing - 10% of welds minimum	Y	Periodic							
3) Thermally cut surfaces of access holes when material t > 2"	Shop (3) or field magnetic Partical or Penetrant testing	Y	Periodic							
 Welded joints subject to fatigue when required by AISC 360, Appendix 3, Table A-3.1 	Shop (3) or field radiographic or Ultrasonic testing	Y	Periodic							
5) Fabricator's NDT reports when fabricator performs NDT	Verify reports	Y	Each submittal (5)							
6. Structural steel bolting: a. Inspection tasks Prior to Bolting (Observe, or perform tasks for each bolted connection, in accordance	Shop (3) and field inspection	Y	Observe or Perform as noted (4)							

	CONEDULE OF SPEC		ISPECTION SEP		
PROJECT		2			
		MAL		O THIS PROJECT	
MATERIAL / ACTIVITY	SERVICE	Y/N	EXIENI	AGEN I*	
3. Inspection of anchors cast in concrete where allowable loads have been increased per section 1908.5 or where strength design is used	Shop (3) and field inspection	Y	Periodic		
4. Inspection of anchors and reinforcing steel post-installed in hardened concrete: Per research reports including verification of anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, concrete minimum thickness, anchor embedment and tightening torque	Field inspection	Y	Periodic or as required by the research report issued by an approved source		
5. Verify use of approved design mix	Shop (3) and field inspection	Y	Periodic		
6. Fresh concrete sampling, perform slump and air content tests and determine temperature of concrete	Shop (3) and field inspection	Y	Continuous		
7. Inspection of concrete and shotcrete placement for proper application techniques	Shop (3) and field inspection	Y	Continuous		
 Inspection for maintenance of specified curing temperature and techniques 	Shop (3) and field inspection	Y	Periodic		
9. Inspection of prestressed concrete:	Shop (3) and field inspection	NA		4.	
a. Application of prestressing force		NA	Continuous		
 B. Grouting of bonded prestressing tendons in the seismic-force-resisting system 		NA	Continuous		
10. Erection of precast concrete		NA		-	
a. Inspect in accordance with construction documents	Field inspection	NA	In accordance with construction documents		
 b. Perform inspections of welding and bolting in accordance with Section 1705.2 	Field inspection	NA	In accordance with Section 1705.2		
11. Verification of in-situ concrete strength, prior to stressing of tendons in post tensioned concrete and prior to removal of shores and forms from beams and structural slabs	Review field testing and laboratory reports	NA	Periodic		
12. Inspection of formwork for shape, lines, location and dimensions	Field inspection	Y	Periodic		
13. Concrete strength testing and verification of compliance with construction documents	Field testing and review of laboratory reports	Y	Periodic		
1705.4 Masonry Construction					
(A) Level A, B and C Quality Assurance:					
1. Verify compliance with approved submittals	Field Inspection	Y	Periodic		
(B) Level B Quality Assurance:					
1. Verification of fm and f _{AAC} prior to construction	Testing by unit strength method or prism test method	Y	Periodic		

SCHEDULE OF SPECIAL INSPECTION SERVICES						
PROJECT						
			AS APPLICABLE TO THIS PROJECT			
MATERIAL / ACTIVITY	SERVICE	Y/N	EXTENT	AGENT*	DATE COMPLETED	
b.Inspection tasks During Bolting (Observe the QA tasks listed in AISC 360. Table N5.6-2)		Y	Observe (4)			
1) Pre-tensioned and slip-critical		Y				
a) Turn-of-nut with matching		Y	Periodic			
markings		1	Basara			
 b) Direct tension indicator c) Twist-off type tension control 		Y	Periodic			
bolt		Y	Periodic			
d) Turn-of-nut without matching markings		Y	Continuous			
e) Calibrated wrench		Y	Continuous			
2) Snug-tight joints		Y	Periodic			
c. Inspection tasks After Bolting (Perform tasks for each bolted connection in accordance with QA tasks listed in AISC 360, Table N5.6- 3)		Y	Perform (4)			
7. Inspection of steel elements of composite construction prior to concrete placement in accordance with QA tasks listed in AISC 360, Table N6.1	Shop (3) and field inspection and testing	N	Observe or Perform as noted (4)			
1705.2.2 Steel Construction Other Than Structural Steel						
1. Material verification of cold-formed steel deck:				н -		
a. Identification markings	Field inspection	Y	Periodic	6 F		
 Manufacturer's certified test reports 	Submittal Review	Y	Each submittal			
2. Connection of cold-formed steel deck to supporting structure:	Shop (3) and field inspection					
a. Welding		NA	Periodic			
b. Other fasteners (in accordance with AISC 360 Section N6)		Y				
1) Verify fasteners are in conformance with approved		Y	Periodic			
2) Verify fastener installation is in conformance with approved submittal and manufacturer's recommendations		Y	Periodic			
3. Reinforcing steel	Shop (3) and field inspection					
a. Verification of weldability of steel other than ASTM A706		Ν	Periodic			
b. Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames, boundary elements of special concrete structural walls and shear reinforcement		N	Continuous			
c. Shear reinforcement	-	Y	Continuous			
d. Other reinforcing steel		N	Periodic			
4. Cold-formed steel trusses spanning		N				
a. Verify temporary and permanent restraint/bracing are installed in accordance with the approved truss submittal package	Field inspection	N	Periodic			
1705.3 Concrete Construction						
1. Inspection of reinforcing steel installation (see 1705.2.2 for welding)	Shop (3) and field inspection	Y	Periodic			
2. Inspection of prestressing steel installation	Shop (3) and field inspection	NA	Periodic			

SCHEDULE OF SPECIAL INSPECTION SERVICES								
PROJECT								
			AS APPLICABLE TO	THIS PROJECT				
MATERIAL / ACTIVITY	SERVICE	Y/N	EXTENT	AGENT*	DATE COMPLETED			
(C) Level C Quality Assurance:								
1. Verification of fm and f_{AAC} prior to construction and for every 5,000 SF during construction	Testing by unit strength method or prism test method	N	Periodic					
2. Verification of proportions of materials in premixed or preblended mortar, prestressing grout, and grout other than self- consolidating grout, as delivered to the project site	Field inspection	N	Continuous					
3. Verify placement of masonry units	Field Inspection	N	Periodic					
(D) Levels B and C Quality Assurance:								
1. Verification of Slump Flow and Visual Stability Index (VSI) of self- consolidating grout as delivered to the project	Field testing	Y	Continuous					
Verify compliance with approved submittals	Field inspection	Y	Periodic					
3. Verify proportions of site-mixed mortar, grout and prestressing grout for bonded tendons	Field Inspection	Y	Periodic					
 Verify grade, type, and size of reinforcement and anchor bolts, and prestressing tendons and anchorages 	Field Inspection	Y	Periodic					
5. Verify construction of mortar joints	Field Inspection	Y	Periodic					
 Verify placement of reinforcement, connectors, and prestressing tendons and anchorages 	Field Inspection	Y	Level B - Periodic					
		N	Level C - Continuous					
7. Verify grout space prior to	Field Inspection	Y	Level B - Periodic					
grouting	~	NA	Level C - Continuous					
8. Verify placement of grout and prestressing grout for bonded tendons	Field Inspection	N	Continuous					
9. Verify size and location of structural masonry elements	Field Inspection	Y	Periodic					
10. Verify type, size, and location of anchors, including details of anchorage of masonry to structural members, frames, or other construction.	Field inspection	Y	Level B - Periodic					
		Ν	Level C - Continuous					
11. Verify welding of reinforcement (see 1705.2.2)	Field inspection	N	Continuous					
12. Verify preparation, construction, and protestion of masonry during cold weather (temperature below 40° F) or hot weather (temperature above 90° F)	Field inspection	Y	Periodic					
13. Verify application and measurement of prestressing force	Field Inspection	N	Continuous					

	SCHEDULE OF SPE		SPECTION SEF	RVICES	
PROJECT				THEDROF	
MATERIAL / ACTIVITY	SERVICE	Y/N	EXTENT	AGENT*	DATE COMPLETED
14. Verify placement of AAC masonry units and construction of thin-bed mortar joints (first 5000 SF of AAC masonry)	Field inspection	N	Continuous		
15. Verify placement of AAC masonry units and construction of thin-bed mortar joints (after the first 5000 SF of AAC masonry)	Field inspection	N	Level B - Periodic		
		N	Level C - Continuous		
16. Verify properties of thin-bed mortar for AAC masonry (first 5000 SF of AAC masonry)	Field inspection	N	Continuous		
17. Verify properties of thin-bed mortar forAAC masonry (after the first 5000 SF of AAC masonry)	Field inspection	N	Level B - Periodic		
		N	Level C - Continuous		
18. Prepare grout and mortar specimens	Field testing	Y	Level B - Periodic		
speaniens		N	Level C - Continuous		
19. Observe preparation of prisms	Field inspection	Y	Level B - Periodic		
1.9 0. 11		N	Level C - Continuous	L'	
1705.5 Wood Construction				-	
1. Inspection of the fabrication process of wood structural elements and assemblies in accordance with Section 1704.2.5	In-plant review (3)	N	Periodic		
 For high-load diaphragms, verify grade and thickness of structural panel sheathing agree with approved building plans 	Field inspection	N	Periodic		
3. For high-load diaphragms, verify nominal size of framing members at adjoining panel edges, nail or staple diameter and length, number of fastener lines, and that spacing between fasteners in each line and at edge margins agree with approved building plans	Field inspection	N	Periodic		
4. Metal-plate-connected wood trusses spanning 60 feet or greater: verify temporary and permanent restraint/bracing are installed in accordance with the approved truss submittal package	Field inspection	N	Periodic		
1705.6 Soils					
 Verify materials below shallow foundations are adequate to achieve the design bearing capacity. 	Field inspection	Y	Periodic		
 Verify excavations are extended to proper depth and have reached proper material. 	Field inspection	Y	Periodic	-	
 Perform classification and testing of controlled fill materials. 	Field inspection	Y	Periodic		
 Verify use of proper materials, densities, and lift thicknesses during placement and compaction of controlled fill 	Field inspection	Y	Continuous		
5. Prior to placement of controlled fill, observe subgrade and verify that site has been prepared properly	Field inspection	Y	Periodic		



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RBE CONSULTING SERVICES,	2875 SPLIT OAK COURT - OVEDO, FL 32766 WEBSTE: WARDES CONT - RAML: RESECLIGGEAULCOM ANAIN FIONULINE: 407-796-3820 - FX NIE: 407-971-6718 MECHANICAL DEFT: 407-796-3776 - EMAIL: NIEHAH@REECS COM	ELECTRICAL DETT: 407-617-3113- EMAIL: FAMILA@REECS.COM STRUCTURD LEFT: 407-568-7866 - EMAIL: REETTIE@REECS.COM ROBERTM. PE. TLORIDA F# #55428 FLORIDA CERTIFICATE OF AUTHORIZATION #27269	PRESEVENTING SERVICES, LLC" HEREDY EXPRESEVENTING SERVICES, LLC" HEREDY EXPRESEVENTING DIFFLER REPERTY RENATION TAW LAGE DA MAG, AND THE REPERTY CANTANED MITHUM	THIS DESIGN AND REWINES SHALL NOT TO BE ALTRED, COFFIC OR REFROUCED IN ANT PORA OR MANIER WITHOUT THE DRYRESS WRITTING PERMISSION AND CONSENT OF "REE CONSULTING PERMISSION AND CONSENT OF "REE CONSULTING
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PROJECT		r				
MATERIAL / ACTIVITY	SERVICE	Y/N	EXTENT	AGENI		
1705.7 Driven Deep Foundations						
 Verify element materials, sizes and engths comply with requirements 	Field inspection	N	Continuous			
 Determine capacities of test elements and conduct additional load tests, as required 	Field inspection	N	Continuous			
 Observe driving operations and maintain complete and accurate records for each element 	Field inspection	N	Continuous			
4. Verify placement locations and plumbness, confirm type and size of hammer, record number of blows per foot of penetration, determine required penetrations to achieve design capacity, record tip and butt elevations and document any damage to foundation element	Field inspection	N	Continuous			
5. For steel elements, perform additional inspections per Section 1705.2	See Section 1705.2	N	See Section 1705.2			
6. For concrete elements and concrete- filled elements, perform additional inspections per Section 1705.3	See Section 1705.3	N	See Section 1705.3			
7. For specialty elements, perform additional inspections as determined by the registered design professional in responsible charge	Field inspection	N	In accordance with construction documents			
8. Perform additional inspections and tests in accordance with the construction documents	Field Inspection and testing	N	In accordance with construction documents			
1705.8 Cast-in-Place Deep Foundations						
1.Observe drilling operations and maintain complete and accurate records for each element	Field inspection	N	Continuous			
2. Verify placement locations and olumbness, confirm element diameters, bell diameters (if applicable), lengths, embedment into bedrock (if applicable) and adequate end-bearing strata capacity. Record concrete or grout volumes	Field inspection	N	Continuous			
3. For concrete elements, perform additional inspections in accordance with Section 1705.3	See Section 1705.3	N	See Section 1705.3			
4. Perform additional inspections and tests in accordance with the construction documents	Field Inspection and testing	N	In accordance with construction documents			
1705.9 Helical Pile Foundations						
I. Verify installation equipment, pile dimensions, tip elevations, final depth, inal installation torque and other data as required.	Field inspection	N	Continuous			
2. Perform additional inspections and ests in accordance with the construction documents	Field Inspection and testing	N	In accordance with construction documents			

SCHEDULE OF SPECIAL INSPECTION SERVICES							
PROJECT							
			AS APPLICABLE TO	O THIS PROJECT			
	SERVICE	Y/N	EXTENT	AGENT*	DATE COMPLETED		
1705.11.5 Architectural Components Special Inspections for Seismic Resistance							
1. Inspection during the erection and fastening of exterior cladding and interior cladding and	Field inspection	N	Periodic				
2. Inspection during the erection and fastening of interior and exterior nonbearing walls	Field inspection	N	Periodic				
3. Inspection during anchorage of access floors	Field inspection	N	Periodic				
1705.11.6 Mechanical and Electrical Components Special Inspections for Seismic Resistance							
 Inspection during the anchorage of electrical equipment for emergency or standby power systems 	Field inspection	N	Periodic				
2. Inspection during the anchorage of other electrical equipment	Field inspection	N	Periodic				
 Inspection during installation and anchorage of piping systems designed to carry hazardous materials, and their associated mechanical units 	Field inspection	N	Periodic				
4. Inspection during the installation and anchorage of HVAC ductwork that will contain hazardous materials	Field inspection	N	Periodic				
 Inspection during the installation and anchorage of vibration isolation systems 	Field inspection	N	Periodic				
1705.11.7 Storage Racks Special Inspections for Seismic Resistance							
Inspection during the anchorage of storage racks 8 feet or greater in height	Field inspection	N	Periodic				
1705.11.8 Seismic Isolation Systems							
Inspection during the fabrication and installation of isolator units and energy dissipation devices used as part of the seismic isolation system	Shop and field inspection	N	Periodic				
1705.12.1 Concrete Reinforcement Testing and Qualification for Seismic Resistance							
1. Review certified mill test reports for each shipment of reinforcement used to resist earthquake-induced flexural and axial forces in reinforced concrete special moment frames, special structural walls, and coupling beams connecting special structural walls	Review certified mill test reports	N	Each shipment				

SCHEDULE OF SPECIAL INSPECTION SERVICES							
PROJECT		0.00					
			AS APPLICABLE TO	O THIS PROJECT			
MATERIAL / ACTIVITY	SERVICE	Y/N	EXTENT	AGENT*	DATE COMPLETED		
1705.10.1 Structural Wood Special Inspections For Wind Resistance							
1. Inspection of field gluing operations of elements of the main windforce- resisting system	Field inspection	N	Continuous				
2. Inspection of nailing, bolting, anchoring and other fastening of components within the main windforce- resisting system	Shop (3) and field inspection	N	Periodic				
1705.10.2 Cold-formed Steel Special Inspections For Wind Resistance							
1.Inspection during welding operations of elements of the main windforce- resisting system	Shop (3) and field inspection	N	Periodic				
2.Inspections for screw attachment, bolting, anchoring and other fastening of components within the main windforce-resisting system	Shop (3) and field inspection	Y	Periodic				
1705.10.3 Wind-resisting Components				р П			
1. Roof cladding	Shop (3) and field inspection	N	Periodic				
2. Wall cladding	Shop (3) and field inspection	N	Periodic				
1/05.11.1 Structural Steel Special Inspections for Seismic Resistance							
Inspection of structural steel in accordance with AISC 341	Shop (3) and field inspection	N	In accordance with AISC 341				
1705.11.2 Structural Wood Special Inspections for Seismic Resistance							
1. Inspection of field gluing operations of elements of the seismic-force resisting system	Field inspection	N	Continuous				
 Inspection of nailing, bolting, anchoring and other fastening of components within the seismic-force- resisting system 	Shop (3) and field inspection	N	Periodic				
1705.11.3 Cold-formed Steel Light-Frame Construction Special Inspections for Seismic Resistance							
 Inspection during welding operations of elements of the seismic-force- resisting system 	Shop (3) and field inspection	N	Periodic				
 Inspections for screw attachment, bolting, anchoring and other fastening of components within the seismic-force- resisting system 	Shop (3) and field inspection	Y	Periodic				
1705.11.4 Designated Seismic Systems Verification							
Inspect and verify that that the component label, anchorage or mounting conforms to the certificate of compliance in accordance with Section 1705.12.3	Field inspection	N	Periodic				

SCHEDULE OF SPECIAL INSPECTION SERVICES								
PROJECT								
			AS APPLICABLE TO	O THIS PROJECT				
MATERIAL / ACTIVITY	SERVICE	Y/N	EXTENT	AGENT*	DATE COMPLETED			
2. Verify reinforcement weldability of ASTM A615 reinforcement used to resist earthquake-induced flexural and axial forces in reinforced concrete special moment frames, special structural walls, and coupling beams connecting special structural walls	Review test reports	N	Each shipment					
1705.12.2 Structural Steel Testing and Qualification for Seismic Resistance								
Test in accordance with the quality assurance requirements of AISC 341	Shop (3) and field testing	N	Per AISC 341					
1705.12.3 Seismic Certification of Nonstructural Components		1. · ·						
Review certificate of compliance for designated seismic system components.	Certificate of compliance review	N	Each submittal					
1705.12.4 Seismic Isolation Systems								
Test seismic isolation system in accordance with ASCE 7 Section 17.8	Prototype testing	N	Per ASCE 7					
1705.13 Sprayed Fire-resistant Materials								
 Verify surface condition preparation of structural members 	Field inspection	N	Periodic					
 Verify application of sprayed fire- resistant materials 	Field inspection	N	Periodic					
 Verify average thickness of sprayed fire-resistant materials applied to structural members 	Field inspection	N	Periodic					
 Verify density of the sprayed fire- resistant material complies with approved fire-resistant design 	Field inspection and testing	N	Per IBC Section 1705.13.5					
 Verify the cohesive/adhesive bond strength of the cured sprayed fire- resistant material 	Field inspection and testing	N	Per IBC Section 1705.13.6					
1705.14 Mastic and Inturnescent Fire-Resistant Coatings								
Inspect mastic and intumescent fire- resistant coatings applied to structural elements and decks	Field inspection	N	Periodic					
1705.15 Exterior Insulation and Finish Systems (EIFS)								
 Verify materials, details and installations are per the approved construction documents 	Field inspection	N	Periodic					
Inspection of water-resistive barrier over sheathing substrate	Field inspection	N	Periodic					

CHEDULE OF SP	ECIAL IN	ISPECTION SEF	RVICES	
		AS APPLICABLE TO	O THIS PROJECT	
SERVICE	Y/N	EXTENT	AGENT*	DATE COMPLETED
Field testing	Y	Per ASTM E2174		
Field testing	Y	Per ASTM E2393		
Field testing	N	Periodic		
Field testing	N	Periodic		
		ADDRESS		TELEPHONE NO.
e engaged by the Owner or the sclosed to the Building Official p proval of the Building Official an ilited as a separate document, i 1704.2.5 are not required when ed not be delayed pending thes bricator's shop may be perform uded in the Statement of Speci ed in the Statement of Speci	Owner's Agent, rior to commenc d/or the Design if noted so abov re the fabricator te inspections. P ed by that fabric ecial Inspections al Inspections	and not by the Contractor or ing work. The qualifications of Professional. e. is approved in accordance w lerform these tasks for each w ator when approved by the Au ator when approved by the Au	Subcontractor whose w f the Special Inspector(s with IBC Section 1704.2.3 velded joint, bolted conn HJ. Refer to AISC 360, 1 Yes No Yes No	ork is to be inspected or s) and/or 5.2 ection, or steel element. N7.
	CHEDULE OF SP SERVICE Field testing Field te	CHEDULE OF SPECIAL IN SERVICE Y/N Field testing Y Field testing Y Field testing Y Field testing N Field testing	CHEDULE OF SPECIAL INSPECTION SEF AS APPLICABLE TO SERVICE Y/N EXTENT Field testing Y Per ASTM E2174 Field testing Y Per ASTM E2393 Field testing N Periodic <	CHEDULE OF SPECIAL INSPECTION SERVICES AS APPLICABLE TO THIS PROJECT SERVICE Y/N EXTENT AGENT* SERVICE Y/N EXTENT AGENT* Field testing Y Per ASTM E2174 Field testing Y Per ASTM E2393 Field testing N Periodic <



 \mathbf{O} MAKOR LIBERTY DELRAY, DELRAY FL ENGINEER STAMP ATTILL OBELICENSE No. 55428 * * :* STATE OF TORIDA STATE OF (1)This item has been electronically signed and sealed by Robert M. Beattie, PE using a Digital Signature and date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies. 08/04/2023 Digitally signed by Robert Beattie Date: 2023.08.04 14:46:47-04'00' ENGINEER N BE N WWW.KUL PHONE LI CAL DEPT URAL DEPT ROBERT THESE PL EXPR EXPR EXPR EXPR THESE PL ALTERED OR M SHEET TITLE SPECIAL INSPECTIONS 06-12-23 DATE DRAWN BY AW/MNG CHECKED BY JOB NO. XXX E 2705 AS NOTED SCALE SHEET S-0-5

	POWERS LINTEL SCHEDULE						
MARK	W (")	H (")	REINFORCING	MARK	W (")	H (")	REINFORCING
PL88	8	8	(I) #5 MID BAR	PLI 28	12	8	(1) #5 MID BAR
PL812	8	12	(I) #5 BOT	PL1212	12	12	(I) #5 BOT
PL816	8	16	(I) #5 TOP ¢ BOT BAR	PL1216	12	16	(I) #5 TOP ∉ BOT BAR
PL816-1	8	16	(2) #5 TOP ¢ BOT BAR	PL1216-1	12	16	(2) #5 TOP ¢ BOT BAR
PL820	8	20	(I) #5 TOP ¢ BOT BAR	PL1220	12	20	(I) #5 TOP ∉ BOT BAR
PL824	8	24	(I) #5 TOP ≰ BOT BAR	PL1224	12	24	(I) #5 TOP ∉ BOT BAR
PL824-1	8	24	(2) #5 TOP ¢ BOT BAR	PL1224-1	12	24	(2) #5 TOP ∉ BOT BAR
PL828	8	28	(I) #5 TOP ¢ BOT BAR	PL1228	12	28	(I) #5 TOP ∉ BOT BAR
PL832	8	32	(2) #5 TOP ∉ BOT BAR	PL1232	12	32	(2) #5 TOP
PL836	8	36	(2) #5 TOP ∉ BOT BAR	PLI 236	12	36	(2) #5 TOP
PL840	8	40	(2) #5 TOP, MID, ≰ BOT BAR	PLI 240	12	40	(2) #5 TOP, MID, ¢ BOT BAR
PL840-1	8	40	(2) #G TOP & BOT BAR, (2) #5 MID BARS	PL1240-1	12	40	(2) #6 TOP & BOT BAR, (2) #5 MID BARS
PL844	8	44	(2) #5 TOP, MID, ≰ BOT BAR	PL1244	12	44	(2) #5 TOP, MID, ¢ BOT BAR
PL848	8	48	(2) #5 TOP, MID, ≰ BOT BAR	PL1248	12	48	(2) #5 TOP, MID, ¢ BOT BAR
PL848-1	8	48	(2) #G TOP & BOT BAR, (2) #5 MID BARS	PL1248-1	12	48	(2) #6 TOP & BOT BAR, (2) #5 MID BARS

FOOTNOTES: I. ALL HEIGHTS SHOWN ARE MINIMUM DIMENSIONS. INCREASE IN DEPTH IS ACCEPTABLE AS FIELD CONDITIONS DICTATE. WHERE POWERS LINTEL EXTENDS INTO BOND BEAM, CONTINUE THE BOND BEAM REINFORCING THROUGH THE POWERS LINTEL AND ELIMINATE THE TOP BAR SPECIFIED IN THE SCHEDULE ABOVE. HEIGHT OF _____ POWERS STEEL L-2 LINTEL (PATENT NO. 5465538) COMPOSITE LINTEL PREFORMED STEEL LINTEL: GALVANIZED I G GA. THICKNESS (0.0596 INCHES) AND I 8 GAGE THICKNESS (0.0478 INCHES) COIL STEEL AS MANUFACTURED BY POWERS STEEL & WIRE PRODUCTS, INC. STEEL GRADE IS ASTM AS70 GRADE C (PY=40 KS)). GALVANIZTION SHAIL COMPLY WITH ASTM AS25 THE POWERS GALVANIZED STEEL VARIES - SEE PLAN UNTEL COMPLIES WITH ASTM ASZS THE POWERS GALVANIZED STEEL LINTEL COMPLIES WITH ASTM AS70 WHICH IS THE SPECIFICATION FOR STRUCTURAL QUALITY STEEL. 2 SHORE LINTELS AS REQUIRED TO COMPENSATE FOR DEAD LOAD DEFLECTION ON NON-CURED MASONRY GROUT. ALL LINTELS GREATER THAN 18-0" ARE BUILT WITH 1/2" CHAMBER. LINTEL TO BE USED WITH CONCRETE MASONRY UNITS HAVING MINIMUM Pm AS SHOWN. 4. STEEL SURFACES IN CONTACT WITH GROUT AND/OR MORTAR SHALL BE UNPAINTED AND FREE OF MATERIAL THAT MIGHT INHIBIT BOND. BEARING EACH END SHALL BE 4" <u>+</u> 1". BEARING SHALL BE ON A MINIMUM 8" DEEP GROUTED CELL EXCEPT THAT L2 LINTELS MAY BEAR ON 1 1/4" THICK MINIMUM FACE SHELL. MATCH WALL WIDTH (NOM) 6. fm = 1500PSI. MASONRY UNITS SHALL CONFORM TO ASTM C90, GRADE N. TYPICAL POWER /____ 2 1/2" LINTEL SECTION GROUT = 3000PSI. SLUMP RANGE: 8" TO II". ROD OR VIBRATE GROUT **→/** ADEQUATELY TO ENSURE CONSOLIDATION OF GROUT (NO AIR POCKETS), GROUT SHALL COMPLY WITH ASTM C476-83 AND BE EITHER COARSE OR FINE GROUT. ALL LINTELS SHALL BE GROUT FILLED. 8. MORTAR: TYPE "S" OR TYPE "M" 1800PSI. ⊟∙∙●∵⊢ —INVERTED KO, TOP REINFORCING OR TOP OF WALL REINFORCING, IS REQUIRED BY CODES TO PROVIDE A CONTINUOUS TIE AROUND A STRUCTURE AND TO PROVIDE FOR UPLIFT RESISTANCE AT LINTELS. TYP 10. ATTACHMENTS TO TOP OF WALL PER ARCHITECTURAL AND/OR ENGINEERING DRAWINGS. II. NOT USED. 12. NOT USED. || • | 13. MASONRY WALL MAY OCCUR ABOVE COMPOSITE LINTEL HEIGHT. IN THE 1.10 4 CASE THAT THE MASONRY WALL IS TWICE THE HEIGHT OF A 24" AND GREATER LINTEL, THE DEAD LOAD WEIGHT OF THE LINTEL MAY BE CLUDED FROM THE LOAD CALCULATIONS DUE TO THE COMPOSITE

POWERS STEEL LINTEL INSTALLATION REQ.

L-2 STEEL LINTEL — (2)

- L-2 STEEL LINTEL NATURE OF THE LINTEL ASSEMBLY.

MARK	W (")	H (")	REINFORCING	
BL88	8	8	(1) #5 MID BAR	E
BL8 2	8	12	(I) #5 BOT	E
BL8 G	8	16	(I) #5 TOP ¢ BOT BAR	E
BL816-1	8	16	(2) #5 TOP ¢ BOT BAR	E
BL820	8	20	(I) #5 TOP ¢ BOT BAR	E
BL824	8	24	(I) #5 TOP ¢ BOT BAR	E
BL824-1	8	24	(2) #5 TOP \$ BOT BAR	E
BL828	8	28	(I) #5 TOP ≰ BOT BAR	E
BL832	8	32	(2) #5 TOP \$ BOT BAR	E
BL836	8	36	(2) #5 TOP ¢ BOT BAR	E
BL840	8	40	(2) #5 TOP, MID, ∉ BOT BAR	E
BL840-1	8	40	(2) #6 TOP \$ BOT BAR, (2) #5 MID BARS	E
BL844	8	44	(2) #5 TOP, MID, ∉ BOT BAR	E
BL848	8	48	(2) #5 TOP, MID, ≰ BOT BAR	E
BL848-1	8	48	(2) #6 TOP ¢ BOT BAR, (2) #5 MID BARS	E

- IN DEPTH IS ACCEPTABLE AS FIELD CONDITIONS DICTATE.
- CONTINUE THE BOND BEAM REINFORCING THROUGH THE
- THE SCHEDULE ABOVE.





Received 8.9.23



FLC 3RD

IST FLOO

JAMB

W/ ARCH DWGS

SILL

(IF REQ'D)

(2) #5

HEADER/JAMB SCHEDULE FOR GROUND SNOW LOAD < 30PSF

FLOOR LEVEL	ROUGH OPENING	HEADER SIZE	HEADER TOP TRACK	HEADER BOT TRACK	JAMB STUD	KING STUD
3RD FLOOR	UP TO 8'-0"	DBL 8C16	CONT.x Ggax2"DEEP LEG	CONT.x Ggax2"DEEP LEG	(I) 6CI4	(1)6C14
to t/wall	UP TO 10'-0"	DBL IOCI6	CONT.x 6gax2"DEEP LEG	CONT.x Ggax2"DEEP LEG	(I) 6CI4	(1)6C14
	UP TO 12'-0"	DBL 12C16	CONT.x 6gax2"DEEP LEG	CONT.x 6gax2"DEEP LEG	(2) 6C 4	(2) 6C 4
2ND TO	UP TO 8'-0"	DBL 8C12	CONT.x Ggax2"DEEP LEG	CONT.x 6gax2"DEEP LEG	(1)6012	()6C 2
3RD FLOOR	UP TO 10'-0"	DBL IOCI2	CONT.x 6gax2"DEEP LEG	CONT.x 6gax2"DEEP LEG	(1)6012	() 6C 2
	UP TO 12'-0"	DBL 12C12	CONT.x 2gax2"DEEP LEG	CONT.x 2gax2"DEEP LEG	() 6x4x 2ga	(2) 6C 2
IST TO 2ND	UP TO 8'-0"	DBL 8C12	CONT.x 2gax2"DEEP LEG	CONT.x 2gax2"DEEP LEG	(I) 6CI2	(I) 6CI2
FLOOR	UP TO 10'-0"	DBL IOCI2	CONT.x 2gax2"DEEP LEG	CONT.x 2gax2"DEEP LEG	() 6x4x 2ga	(2) 6C 2
	UP TO 12'-0"	DBL 2x3x 2ga	CONT.x 2gax2"DEEP LEG	CONT.x 2gax2"DEEP LEG	(2) 6x4x 2ga	(2) 6x4x 2ga

HEADER/JAMB SCHEDULE FOR GROUND SL > 30PSF < 60PSF

FLOOR LEVEL	ROUGH OPENING	HEADER SIZE	HEADER TOP TRACK	HEADER BOT TRACK	JAMB STUD	KING STUD
3RD FLOOR	UP TO 6'-0"	DBL 8C16	CONT.x Ggax2"DEEP LEG	CONT.x 6gax2"DEEP LEG	(I) 6CI4	(1)6C14
TO T/WALL	UP TO 8'-0"	DBL IOCI6	CONT.x 6gax2"DEEP LEG	CONT.x 6gax2"DEEP LEG	(I) 6CI4	(1)6C14
	UP TO 10'-0"	DBL 12C16	CONT.x 6gax2"DEEP LEG	CONT.x 6gax2"DEEP LEG	(2) 6C 4	(2) 6C 4
2ND TO	UP TO 6'-0"	DBL 8C12	CONT.x 6gax2"DEEP LEG	CONT.x 6gax2"DEEP LEG	(1) 6012	(1)6C12
3RD FLOOR	UP TO 8'-0"	DBL IOCI2	CONT.x 6gax2"DEEP LEG	CONT.x 6gax2"DEEP LEG	(1) 6012	(1)6C12
	UP TO 10'-0"	DBL 12C12	CONT.x 2gax2"DEEP LEG	CONT.x 2gax2"DEEP LEG	() 6x4x 2ga	(2) 6C 2
IST TO 2ND	UP TO 6'-0"	DBL 8C12	CONT.x 2gax2"DEEP LEG	CONT.x 2gax2"DEEP LEG	(1) 6012	(1)6C12
FLOOR	UP TO 8'-0"	DBL IOCI2	CONT.x 2gax2"DEEP LEG	CONT.x 2gax2"DEEP LEG	() 6x4x 2ga	(2) 6C 2
	UP TO 10'-0"	DBL 2x3x 2ga	CONT.x 2gax2"DEEP LEG	CONT.x 2gax2"DEEP LEG	(2) 6x4x 2ga	(2) 6x4x 2ga

TYPICAL SECTION EXTERIOR LIGHT-GAGE LOAD BEARING WALL



GENERAL NOTES

- THESE PLANS AND THE INFORMATION CONTAINED HEREIN ARE THE PROPERTY OF SELLER/ SUBCONTRACTOR AND ARE TO BE USED SOLELY IN CONNECTION WITH ERECTION OF BUILDING SYSTEMS AND MATERIALS SOLD TO OWNER/ CONTRACTOR BY SELLER/ SUBCONTRACTOR. UNAUTHORIZED COPYING, DISCLOSURE OR OTHER UNAUTHORIZED USES ARE PROHIBITED.
- OWNER/ CONTRACTOR IS RESPONSIBLE TO PROVIDE SELLER/ SUBCONTRACTOR WITH APPROVED PLANS PRIOR TO FABRICATION. OWNER/ CONTRACTOR IS RESPONSIBLE FOR ALL NECESSARY AND
- REQUIRED PERMITS FEES DEPOSITS ETC. THE OWNER AND/ OR CONTRACTOR SHALL REVIEW AND DETERMINE THAT ALL DIMENSIONS ARE COORDINATED AS REQUIRED WITH ALL OTHER DESIGN PROFESSIONALS' DRAWINGS AND SHOP DRAWINGS FOR PROJECT PRIOR TO FABRICATION OF MATERIALS OR THE START OF CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO THE SELLER/ SUBCONTRACTOR
- AND ENGINEER OF RECORD PRIOR TO FABRICATION. PRODUCTS SHIPPED TO OWNER/ CONTRACTOR SHALL BE INSPECTED
- IMMEDIATELY UPON ARRIVAL. THE SHEET LEDGES, RAINLIPS, AND MASONRY LEDGES ARE VITAL TO THE PROPER FIT OF THE STEEL CONSTRUCTION. OWNER/ CONTRACTOR SHALL FIELD VERIFY ALL TO BE AS SHOWN ON DRAWINGS. IF THEY ARE NOT IN COMPLIANCE WITH THE DRAWINGS, OWNER/ CONTRACTOR SHALL CONTACT SELLER/ SUBCONTRACTOR PRIOR TO START OF STEEL ERECTION.
- 7. ANY OMISSIONS AND/OR CONFLICTS WITH PLANS SHALL BE REPORTED TO METAL BUILDING COMPANY SO THAT THEY CAN BE RESOLVED PRIOR TO PROCEEDING WITH WORK. DO NOT SCALE DRAWINGS - IF A REQUIRED DIMENSION IS MISSING PLEASE
- CONTACT THE METAL BUILDING COMPANY AND / OR ENGINEER OF RECORD. 6. NO MODIFICATIONS TO PLANS SHALL BE MADE WITHOUT THE PERMISSION OF METAL BUILDING COMPANY AND ENGINEER OF RECORD MODIFICATIONS REQUIRED DUE TO FIELD CONDITIONS OR OTHER CONTRACTORS OR ITEMS THAT WHICH MAY ADVERSELY AFFECT THE
- STRUCTURE REQUIRES WRITTEN PERMISSION (NO MODIFICATIONS TO STRUCTURAL MEMBERS IS ALLOWED). 10. ALL SECTIONS AND DETAILS SHALL BE CONSIDERED TO BE TYPICAL OR SIMILAR UNLESS ANOTHER SECTION OR DETAIL IS REFERENCED ON THE
- PLANS II. SCOPE OF WORK OF METAL BUILDING COMPANY IS INDICATED IN THE
- CONTRACT. THE DRAWINGS REFLECT SCOPES OF WORK AS REQUIRED FOR PERMITTING OR AT THE DIRECTION OF OWNER / CONTRACTOR. 12. METAL BUILDING COMPANY DOES NOT PROVIDE A RATED CEILING IN SHAFT
- ENCLOSURES UNLESS SPECIFIC NOTED ON PLANS. 13. CONSTRUCTION DOCUMENTS WERE DESIGNED AND MEET THE REQUIREMENTS OF THE OF THE LOCAL BUILDING CODE DESIGNATED
- UNDER STRUCTURAL DESIGN CRITERIA. 14. ALL COMPONENTS AND CLADDING DESIGNED AND/OR FURNISHED BY OTHERS NOT SHALL MEET THE MINIMUM REQUIREMENTS OUTLINED IN THESE CONSTRUCTION DOCUMENTS AND SHOP DRAWINGS SHALL BE SUBMITTED REVIEW/APPROVAL BY EOR AND GOVERNING AUTHORITIES PRIOR TO FABRICATION AND/OR ERECTION
- 15. PONDING LOADS ARE NOT APPLICABLE ON SELF STORAGE BUILDINGS WHERE THE PRIMARY ROOF DRAINAGE IS ACCOMPLISHED BY GUTTERS AND DOWNSPOUTS. SECONDARY DRAINAGE OCCURS WITH GUTTER OVERFLOW WHICH IS AT OR BELOW EAVE HEIGHT OF BLDG(S).
- 16. THE CONTRACTOR OR OWNER SHALL TAKE ALL NECESSARY STEPS TO PROTECT THE STRUCTURE, THE WORK PERSONS AND OTHER PEOPLE DURING CONSTRUCTION. HE SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION.
- 17. THE CONTRACTOR/ OWNER SHALL TAKE ALL NECESSARY STEPS TO PROTECT THE STRUCTURE. THE WORK. AND OTHER PEOPLE DURING CONSTRUCTION. IF OWNER/ CONTRACTOR PERFORMS THE ERECTION. CONTRACTOR/ OWNER SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION.
- 18. IF ANYTHING IN THESE DRAWINGS CONFLICTS WITH THE CONTRACT, THE TERMS OF THE CONTRACT SHALL GOVERN. 19. TEMPORARY BRACING AND SHORING OF WALLS TO PROVIDE STABILITY
- DURING CONSTRUCTION IS THE RESPONSIBILITY OF THE OWNER AND/ OR CONTRACTOR U.N.O. IN CONSTRUCTION DOCUMENTS.
- THE OWNER AND/ OR CONTRACTOR SHALL TEMPORARILY BRACE AND/ OR SHORE EXISTING AND NEW CONSTRUCTION AS REQUIRED TO ENSURE THE STRUCTURAL STABILITY IS NOT COMPROMISED IF DEMOLITION IS INVOLVED. BRACING AND/ OR SHALL REMAIN IN PLACE UNTIL THE STRUCTURAL WORK IS COMPLETE AND HAS BEEN INSPECTED BY A TESTING AGENCY AND IS CERTIFIED TO BE IN SUBSTANTIAL COMPLIANCE WITH PLANS AND SPECIFICATIONS.
- 21. EVERY REASONABLE EFFORT HAS BEEN MADE TO ENSURE COORDINATION BETWEEN THESE DRAWINGS AND THE BOUND STRUCTURAL SPECIFICATIONS. IF A DISCREPANCY IS DISCOVERED, THE OWNER/ CONTRACTOR SHALL, IN WRITING, REQUEST A CLARIFICATION IN THE ABSENCE OF SAID REQUEST, THE MORE STRINGENT REQUIREMENT SHALL GOVERN
- 22. ALL CELLS OF CMU WALLS IN CLIMATE CONTROLLED BUILDING WHICH ARE NOT GROUTED SHALL BE FILLED WITH PERLITE, VERMICULITE, OR POLYURETHANE FOAMED-IN-PLACE AS SPECIFIED BY THE LATEST ADOPTED EDITION ON THE INTERNATIONAL ENERGY CONSERVATION CODE.
- 23. AREAS WITHIN HURRICANE-PRONE REGIONS LOCATED WITHIN I MILE OF THE COASTAL MEAN HIGH WATER LINE WHERE THE BASIC DESIGN WIND SPEED. V. IS I 30MPH OR GREATER: OR IN ALL AREAS WHERE THE BASIC DESIGN WIND SPEED IS I 40MPH OR GREATER SHALL MEET THE REQUIREMENTS FOR WIND BORNE DEBRIS REGION.
- 24. GLAZING IN BUILDINGS IN AREAS THAT MEET THE CRITERIA FOR WIND BORNE DEBRIS REGION SHALL BE IMPACT RESISTANT OR PROTECTED WITH AN IMPACT RESISTANT COVERING MEETING THE LARGE MISSILE TEST (ASTM E 1996) FOR OPENINGS LOCATED WITHIN 30FT OF GRADE OR SHALL MEET THE REQUIREMENTS FOR SMALL MISSILE TEST (ASTM E1996) FOR OPENINGS LOCATED MORE THAN 30FT ABOVE GRADE.
- THE GENERAL CONTRACTOR SHALL TAKE ALL NECESSARY STEPS TO PROTECT THE STRUCTURE, THE WORK PERSONS, AND OTHER PEOPLE DURING CONSTRUCTION. HE SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION.
- NO STRUCTURAL MEMBER SHALL BE CUT, NOTCHED OR OTHERWISE REDUCED IN STRENGTH
- THE GENERAL CONTRACTOR SHALL COORDINATE ALL DESIGN PROFESSIONALS DRAWINGS AND SPECIFICATIONS, FOR ITEMS WHICH MAY ADVERSELY AFFECT THE STRUCTURE AND NOTIFY THE ARCHITECT/ENGINEER 4. LOAD-BEARING STUD TO TRACK CONNECTIONS: THE ENDS OF THE OF ANY DISCREPANCIES AND/OR OMISSIONS.
- SUBMITTAL SETS SHALL CONSIST OF A MAXIMUM OF 1 SEPIA AND 2 BLUE LINES, ADDITIONAL BLUE LINES WILL BE DISCARDED. • SUBMITTALS TO ENGINEER OF RECORD FOR REVIEW MUST CONTAIN THE
- GENERAL CONTRACTOR'S STAMP SIGNIFYING HIS REVIEW/ACCEPTANCE. SUBMITTALS SENT WITHOUT THE GC'S REVIEW STAMP WILL BE RETURNED AT HIS EXPENSE AND WITHOUT REVIEW.

STRUCTURAL NOTES

- TEMPORARY BRACING SHOULD BE PROVIDED TO RESIST WIND LOADING ON STRUCTURAL COMPONENTS AND STRUCTURAL ASSEMBLIES DURING
- ERECTION AND CONSTRUCTION PHASE. 2. NEVER ALLOW YOUR ROOF TO COME IN CONTACT WITH, OR WATER RUNOFF FROM, ANY DISSIMILAR METAL, INCLUDING, BUT NOT LIMITED TO, COPPER, LEAD, OR GRAPHITE, THIS INCLUDES COPPER AND ARSENIC SALTS. USED IN TREATED LUMBER, AND CALCIUM USED IN CONCRETE, MORTAR, AND GROUT.
- 3. SCOPES OF WORK BY OTHERS WHOSE LATERAL LOADS WILL BE TRANSFERRED INTO STEEL MEMBER PROVIDED BY METAL BUILDING COMPANY SHALL BE TEMPORARILY BRACED BY OTHERS IN A METHOD THAT DOES NOT INTERFERE WITH ERECTION OF STEEL, UNTIL STEEL ERECTION COMPLETE.
- THE UNCOATED MINIMUM STEEL THICKNESS OF THE COLD-FORMED PRODUCTS AS DELIVERED SHALL NOT BE LESS THAN 95% PERCENT OF THE DESIGN THICKNESS. THICKNESS MEASUREMENTS MAY BE MADE ANYWHERE ACROSS THE WIDTH OF THE SHEET, BUT NOT CLOSER TO THE EDGES THAN 3. THE MINIMUM DISTANCES SPECIFIED IN THE RELEVANT ASTM SPECIFICATIONS. THICKNESS AT BENDS, SUCH AS CORNERS, MAY BE LESS THAN 95% OF DESIGN THICKNESS, DUE TO COLD-FORMING EFFECTS, AND STILL BE ACCEPTABLE
- RECESSED ENTRIES AND BREEZEWAYS MUST BE RECESSED BELOW FINISHED FLOOR TO AVOID POTENTIAL WATER PROBLEMS, ROLL-UP DOORS LOCATED IN BREEZE WAY NEED TO BE INSTALLED IN RECESSED AREA. IF A CHANGE IS MADE BY OWNER/ CONTRACTOR, SELLER/ SUBCONTRACTOR MUST BE NOTIFIED IMMEDIATELY.
- ALL ERECTION. FABRICATION. WORKMANSHIP AND INSTALLATION SHALL BE IN ACCORDANCE WITH INSTALLATION PROCEDURES MANUAL AND/ OR INDUSTRY STANDARDS APPROVED BY SELLER/ SUBCONTRACTOR AND THE ENGINEER OF RECORD.

ROOF SYSTEMS: MBCI OR EQUAL

- ROOFING SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS 7 AND SPECS, ALONG WITH SELLER/ SUBCONTRACTOR INSTALLATION PROCEDURES MANUAL ROOF SHEETS SHOULD BE INSTALLED FROM THE LOWEST STEP-DOWN TO
- HIGHEST ELEVATION. INSULATION: BAY INSULATION SUPPLY

CO. OR EQUAL

- I. VINYL-BACKED INSULATION- BY BAY INSULATION CO. OR EQUAL. R-VALUES FOR WINTER CONDITIONS (MEAN 40 DEGREES F.): 3.0"-0.6 LB DENSITY FIBERGLASS 3.0" = 103.5"- 0.6 LB DENSITY FIBERGLASS 3.0" = 1 1
- 4.0"- 0.6 LB DENSITY FIBERGLASS 3.0" = 13 6.0"- 0.6 LB DENSITY FIBERGLASS 3.0" = 19
- POLYFOIL FIRE-RATED R-FOIL BY TVM OR EQUAL
- SINGLE BUBBLE R-VALUES FOR ROOF = 10DOUBLE BUBBLE R-VALUES FOR ROOF = 10
- INSULATION SHALL BE INSTALLED PER THE MANUFACTURER'S PUBLISHED INSTRUCTIONS. INSULATION MAY BE INSTALLED IN ONE OR MULTIPLE LAYERS TO MEET THE REQUIRED R-VALUE.

FASTENERS AND ANCHORS

- I. THE FOLLOWING OUTLINES THE MECHANICAL ANCHORS APPROVED FOR USE ON THIS PROJECT.
- A. EXPANSION ANCHORS- "KWICK BOLT 3" BY HILTI OR EQUAL DRILL HOLE IN CONCRETE OR GROUT-FILLED CMU AND REMOVE DUST. THE MIN. HOLE DEPTH MUST EXCEED THE ANCHOR EMBEDMENT PRIOR TO TORQUING BY ONE HOLE DIAMETER. CRIVE THE ANCHOR INTO THE HOLE USING A HAMMER. A MINIMUM OF SIX THREADS MUST BE BELOW THE SURFACE OF THE FIXTURE. TIGHTEN THE NUT TO THE
- RECOMMENDED INSTALLATION TORQUE $(\frac{1}{2})^{"} = 40 \text{ lbs./ft.}$. ADHESIVE ANCHORS IN CONCRETE- "HIT HY 150" BY HILTI OR EQUAL C. ADHESIVE ANCHORS IN GROUT-FILLED BLOCK- "HIT HY 20" BY HILTI OR EQUAL.
- D. ADHESIVE ANCHORS IN HOLLOW BLOCK- "HIT HY 20" WITH SCREEN TUBES BY HILTI OR EQUAL.
- CONCRETE MASONRY SCREWS- "KWIK CON II" BY HILTI OR EQUAL. POWDER-ACTUATED FASTENERS (PAF)- "DX" BY HILTI OR EQUAL. 2. ALL FASTENERS SHALL BE INSTALLED PER MANUFACTURER'S
- RECOMMENDATIONS AND SPECIFICATIONS.

COLD FORMED STEEL: LGSI SECTIONS OR EQUAL

- COLD FORM STEEL SECTIONS SHALL CONFORM TO APPLICABLE PROVISIONS OF ASTM A572, ASTM AG07 AND/OR ASTM AG11
- MIN. DELIVERED THICKNESS OF COLD FORMED STEEL C'S \notin Z'S

GAGE	DESIGN THICKNESS	FINISH	Fy
12	0.105	RED-OXIDE UNO ON PLANS	55ksı
14	0.07	RED-OXIDE UNO ON PLANS	55ksı
16	0.059	RED-OXIDE UNO ON PLANS	55ksı
18	0.0468	GALVANIZED	33ksı
20	0.0352	GALVANIZED	33ksı

3. MIN. DELIVERIED THICKNESS OF COLD FORMED STEEL PIERS & HEADERS

GAGE	DESIGN THICKNESS	FINISH	Fy			
4	0.07	TEX-COTE FINISH APPLIED IN FIELD	55ksı			
16	0.059	TEX-COTE FINISH APPLIED IN FIELD	55ksı			
16	0.055	PRE-FINISHED	55ksı			
18	0.0468	TEX-COTE FINISH APPLIED IN FIELD	33ksı			
NOTES: INSIDE RETURN ON PIERS IS 3 1/2" TO 4", VERIFY REQUIRED						

DIMENSIONS FOR EXTERIOR ROLL UP DOORS LOAD-BEARING STUDS MUST BE INSTALLED INTO THE TOP AND BOTTOM TRACKS SO THAT THE GAP BETWEEN THE ENDS OF THE STUD AND THE WEB OF THE TRACK IS AS SMALL AS PRACTICABLE AND IN NO CASE GREATER THAN $\frac{3}{16}$ " AT THE TIME OF INSTALLATION. THE GAP MUST BE LESS THAN $\frac{1}{16}$ " AFTER THE DEAD LOAD OF THE STRUCTURE IS IN PLACE. ALL BOLTED CONNECTIONS OF COLD FORMED STEEL BEAMS

SUPPORTING ROOF LOADING ONLY SHALL BE MADE USING 1/2"DIA. A307 BOLTS OR HIGHER GRADE (U.N.O.).

- ALL BOLTED CONNECTIONS OF COLD FORMED STEEL BEAMS SUPPORTING FLOOR LOADING SHALL BE MADE USING A325 BOLTS OR HIGHER GRADE. (U.N.O.)
- 7. IN THE EVENT THAT THE BOLT HOLES IN THE COLD FORMED STEEL BEAMS DO NOT ALIGN WITH THE SUPPORTING CONNECTION MEMBER IT IS ACCEPTABLE TO SUBSTITUTING #12 TEK SCREWS FOR THE FOLLOWING SIZE/GRADE BOLTS.
- BOLT * (6) #12 TEK SCREWS PER EACH MIS-ALIGNED 1/2" DIAMETER A325

HOT ROLLED STEEL



ENCLOSED. WELDING SOCIETY

TOLERANCES. 9. FIRE PROTECTION.

FNGINFFR

24"x26qa 24"x24qa

24"x22ga COMPOSITE STEEL FLOOR DECK SHALL BE ATTACHED TO THE SUPPORTING STRUCTURE AS RECOMMENDED BY THE MANUFACTURER, WITH MAXIMUM SPACING NOT TO EXCEED 24" oc.

7. THE VALUES LISTED IN THE TABLE SHOWN BELOW IS FROM THE VULCRAFT METAL DECK PRODUCT MANUAL, AND REPRESENTS THE MINIMUM COMPOSITE FLOOR DECK SECTION PROPERTIES THAT ARE REQUIRED BY DESIGN

TYPE 1.5VL22 1.5VL2C 1.5VL18 2.0VL22

DECK

2.0VL2C 2.0VL18 2.0VL16 3.0VL22

3.0VL20 3.0VL18

SUPPORTING STRUCTURE AS RECOMMENDED BY THE MANUFACTURER, WITH MAXIMUM SPACING NOT TO EXCEED 24" O.C. STEEL USED TO MANUFACTURE THE NON-COMPOSITE METAL FLOOR DECKING SHALL CONFORM TO THE REQUIREMENTS OF ASTM-A1008 GRADES C, D, OR E, AND/OR A653 STRUCTURAL QUALITY. 10. THE VALUES LISTED IN THE TABLE SHOWN BELOW IS FROM THE VULCRAFT METAL DECK PRODUCT MANUAL, AND REPRESENTS THE MINIMUM



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DESIGN OF STRUCTURAL STEEL ELEMENTS WAS COMPLETED UNDER THE REQUIREMENTS SET FORTH IN THE "MANUAL OF STEEL CONSTRUCTION-ALLOWABLE STRESS DESIGN (LATEST EDITION)"

ALL STEEL SHALL BE DOMESTICALLY PRODUCED. ASTM A36- ROLLED SHAPES, PLATES, AND BARS. Fy=36ksı ASTM A992- WIDE FLANGE SECTIONS. Fy=50ksı ASTM A53, TYPE E, GRADE B - PIPE. Fy=35ksı ASTM A500 GRADE B - TUBES. Fv=46ksi ASTM F1554 (A36) - ANCHOR BOLTS, RODS, NUTS ¢ WASHERS.

ASTM A 108 GRADE 1015 THROUGH 1020, COLD FINISHED CARBON STEEL, AWS DI.I, TYPE B - HEADED STUDS. ASTM A325, TYPE N - BOLTED STRUCTURAL CONNECTIONS.

ASTM A307 - FOR BOLTED CONN. OF LESS THEN 5/8" DIA. E70XX ELECTRODE (LOW HYDROGEN) - WELDED CONNECTIONS (U.N.O.). WELDED CONNECTIONS SHALL BE A MIN. OF 3/16" FILLET WELD ALL AROUND FOR CONNECTING MEMBERS UP TO 1/4" THICK. USE 1/4" FILLET WELD FOR ALL OTHER MEMBER THICKNESSES (U.N.O.). ALL BOLTED CONNECTIONS SHALL BE "SNUG-TIGHT" AS DEFINED IN THE SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS" (LATEST ADOPTED EDITION) BY RESEARCH COUNCIL ON STRUCTURAL

BOLTED CONNECTIONS INDICATED TO BE "SLIP-CRITICAL" (SC) SHALL BE INSTALLED, TIGHTENED, TESTED, AND INSPECTED AS OUTLINED IN THE "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS" (LATEST ADOPTED EDITION) BY RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (ROSC). (U.N.O.)

BRACE AND MAINTAIN ALL STEEL IN ALIGNMENT UNTIL OTHER PARTS OF CONSTRUCTION NECESSARY FOR PERMANENT SUPPORT ARE COMPLETED. CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLING TEMPORARY SHORING AS REQUIRED FOR THE STABILITY OF THE STEEL FRAME UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN COMPLETED AND BUILDING IS

ALL WELDING SHALL CONFORM TO THE REQUIREMENTS OF "THE STANDARD CODE FOR WELDING IN BUILDING CONSTRUCTION" OF THE AMERICAN

GROUT FOR COLUMN BASE PLATES AND PRESET BEARING PLATES SHALL BE NON-SHRINK, NON-METALLIC GROUT. (5000psi MIN) SUBMIT SHOP DRAWINGS INDICATING ALL SHOP AND ERECTION DETAILS INCLUDING PROFILES, SIZES, SPACING AND LOCATIONS OF STRUCTURAL MEMBERS, CONNECTION ATTACHMENTS, FASTENERS, LOADS AND

ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED IN ACCORDANCE WITH THE MINIMUM REQUIREMENTS OF ASTM A 1 23. 10. STRUCTURAL STEEL SHALL RECEIVE SHOP COAT OF PRIMER (COLOR AS DIRECTED BY ARCHITECT) EXCEPT AREAS THAT WILL RECEIVE SPRAY-ON

II. BEAM TO BEAM AND BEAM TO COLUMN CONNECTIONS SHALL BE DESIGNED BY THE STEEL FABRICATOR FOR THE REACTIONS SHOWN ON THE FRAMING PLANS. SIGNED AND SEALED SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW, WHICH CLEARLY INDICATES THE ALLOWABLE LOAD CAPACITY OF EACH UNIQUE CONNECTION. WHERE REACTION IS NOT INDICATED ON THE PLANS, THE CONNECTION SHALL BE DESIGNED FOR THE MAXIMUM SHEAR CAPACITY OF THE BEAM, FOR THE GIVEN SPAN.

STRUCTURAL STEEL

I. A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND

FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL BE IN STRICT CONFORMANCE WITH THE LATEST ADOPTED EDITION OF THE AISC MANUAL. DESIGN OF STRUCTURAL STEEL ELEMENTS WAS COMPLETED UNDER THE REQUIREMENTS SET FORTH IN THE "MANUAL OF STEEL CONSTRUCTION -ALLOWABLE STRESS DESIGN (LATEST EDITION)".

STEEL DECKING

THE REQUIREMENTS OF THE LATEST ADOPTED EDITION OF THE AISI SECTION A3, SHALL GOVERN FABRICATION OF THE SPECIFIED STEEL DECK. THE MINIMUM YIELD STRENGTH OF STEEL USED, SHALL BE, 33KSI (230

ALL FIELD WELDING OF DECK SHALL BE IN STRICT CONFORMANCE WITH ANSI/AWSD I .3 STRUCTURAL WELDING CODE. 4. GALVANIZING SHALL CONFORM TO ASTM-A653, STRUCTURAL QUALITY,

AND FEDERAL SPEC. QQ-S-775. 5. THE VALUES LISTED IN THE TABLE SHOWN BELOW IS FROM THE MBCI ULTRA-DEK TECHNICAL-INSTALLATION INFORMATION MANUAL. AND REPRESENTS THE MINIMUM ROOF DECK SECTION PROPERTIES THAT ARE

REQUIRED BY DESIGN.								
	ROOF DECK							
DECK TYPE	DESIGN THICK (in)	lxe ın^4/ft	Sxe ın^3/ft	lxe ın^4/ft	Sxe ın^3/ft	Fy (ksı)		
24"x26ga ULTRA-DEK	0.0179	0.1025	0.0694	0.2202	0.0901	50		
24"x24ga ULTRA-DEK	0.0239	0.1355	0.0951	0.2803	0.1156	50		
24"x22ga ULTRA-DEK	0.0299	0.1837	0.1332	0.3640	0.1504	50		

COMPOSITE FLOOR DECK

	DESIGN THICK (1n)	lp 4 ın /ft	In 4 ın /ft	Sp 3 ın /ft	Sn 3 ın /ft	STL. STRENGTH
-	0.0295	0.150	0.182	0.178	0.186	Fy=50ksı
2	0.0358	0.195	0.222	0.231	0.240	Fy=50ksı
ò	0.0474	0.282	0.295	0.315	0.327	Fy=50ksı
-	0.0295	0.322	0.329	0.274	0.277	Fy=50ksı
С	0.0358	0.418	0.415	0.355	0.360	Fy=50ksı
6	0.0474	0.559	0.558	0.495	0.504	Fy=50ksı
	0.0598	0.704	0.704	0.653	0.653	Fy=40ksı
-	0.0295	0.746	0.745	0.429	0.442	Fy=50ksı
2	0.0358	0.938	0.937	0.553	0.572	Fy=50ksı
6	0.0474	1.251	1.251	0.795	0.803	Fy=50ksı

8. NON-COMPOSITE STEEL FLOOR DECK SHALL BE ATTACHED TO THE

NON-COMPOSITE FLOOR DECK SECTION PROPERTIES THAT ARE REQUIRED BY DESIGN

NON COMPOSITE FLOOR DECK

DECK TYPE	DESIGN THICK (11)	lp 4 ın /ft	In 4 ın /ft	Sp 3 ın /ft	Sn 3 ın /ft
0.6C26	0.0179	0.015	0.015	0.043	0.043
0.6C24	0.0239	0.019	0.019	0.057	0.057
0.6C22	0.0298	0.024	0.024	0070	0.070

MINIMUM STANDARD FOR ELEVATED FLOOR SLABS

ALI	ALL CONCRETE SHALL HAVE THE FOLLOWING MIN. PROPERTIES:					
	LOCATION	28 DAY STRENGTH	SLUMP	MAX AGGR.		
ELEVA	TED SLABS FORMED AND POURED	4,000 ры	4" <u>+</u> "	1		
ELEVA	TED SLABS FORMED W/ MTL DECK	4,000 ры	4" <u>+</u> "	1		
А. В.	A. SLUMP FOR RAMPS AND SLOPING SURFACES SHALL NOT EXCEED 4". B. COLD JOINTS ARE NOT RECOMMENDED - ALTHOUGH IF REQUIRED THEY SHOULD BE PLACED A MINIMUM OF 2'-0" OFF CENTERLINE OF COLUMNIS					

- CONCRETE PROPERTIES SHALL BE VERIFIED THROUGH INDUSTRY STANDARD TESTING PROCEDURES BY A CERTIFIED TESTING AGENCY. MIN. TEST REQUIRED SHALL INCLUDE SLUMP AND CYLINDER BEAKS FOR COMPRESSIVE STRENGTH. FINDINGS SHALL BE SUBMITTED TO THE ARCH./ENG. FOR REVIEW.
- 3. CONCRETE WORK SHALL CONFORM TO LATEST EDITIONS OF ACI 301 SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS, ACI 318 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE AND ACI 3 | 5 DETAILS AND DETAILING OF CONCRETE REINFORCEMENT.
- 4. CONCRETE MIX DESIGN SHALL MEET THE FOLLOWING CRITERIA
- A. PROPOSED MIX DESIGN SHALL BE ACCORDANCE WITH ACI 301 METHOD 1 OR METHOD 2 .
- ENTRAPPED AIR CONTENT SHALL NOT EXCEED 3%. ADMIXTURES USED TO ENTRAIN AIR ARE NOT ACCEPTABLE ALL CONCRETE TO BE NORMAL WEIGHT WITH A DESIGN STRENGTH AT 28 D.
- SITE ADDED WATER IS NOT ACCEPTABLE. HOLD BACK WATER CLEARLY IDENTIFIED TO BE ADDED AT THE SITE AS SPECIFIED IN EITHER THE MIX DESIGN AND/OR THE BATCH PLANT LOG SHALL BE CONSIDERED ACCEPTABLE. IN THE ABSENCE SPECIFIED HOLD BACK WATER. THE MIX WILL RESULT IN A REJECTION OF THE RESULTS BY THE ENGINEER OF RECORD.
- 6. THE USE OF MIX DESIGNS WITH EITHER SLAG OR FLY ASH ARE NOT ACCEPTABLE FOR ALL EXPOSED SURFACES. SLABS ON GRADE. OR ELEVATED SLABS, AND WHERE THE SURFACE IS INTENDED TO BE EXPOSED.
- THE USE OF RIDE ON POWER TROWELS IS STRICTLY PROHIBITED ON ELEVATED SLABS. THE USE OF WALK BEHIND POWER TROWELS ARE RECOMMENDED.
- CONTRACTOR IS RESPONSIBLE FOR THE ADEQUACY OF THE FORMS AND SHORING AND FOR SAFE PRACTICE IN THEIR USE AND REMOVAL.
- 9. THE VALUES IN THE TABLE SHOWN BELOW IS FROM THE VULCRAFT METAL DECK PRODUCT MANUAL AND REPRESENTS THE RECOMMENDED WELDED WIRE FABRIC.

DECK TYPE	TOTAL SLAB DEPTH	RECOMMENDED WELDED WIRE FABRIC
1.5 VL, VLI or R	<= 4 3/4"	6 x 6 - W2.1 x W2.1
1.5 VL, VLI or R	> 4 3/4"	6 x 6 - W2.1 x W2.1
2VLI	<= 5 /4"	6 x 6 - W2.1 x W2.1
2VLI	> 5 /4"	6 x 6 - W2.1 x W2.1
3VLI	<= 6 /4"	6 x 6 - W2.1 x W2.1
3VLI	> 6 /4"	6 x 6 - W2. x W2.

THE FOLLOWING METHODS OF REINFORCEMENT SHALL BE ACCEPTABLE ALTERNATES TO THE WELDED WIRE FABRIC SPECIFIED FOR THE ELEVATED SLABS ABOVE

- I. CONCRETE IN ACCORDANCE WITH ASTM CIIIG, TYPE I, CONTAINING STEEL FIBERS MEETING THE CRITERIA OF ASTM A820, TYPE I. TYPE II OR TYPE V. AT A DOSAGE RATE DETERMINED BY THE FIBER MANUFACTURER FOR THIS APPLICATION, BUT NOT LESS THAN 25 LB/CU YD.
- 2. CONCRETE IN ACCORDANCE WITH ASTM CI IG, TYPE III CONTAINING MACROSYNTHETIC FIBERS MEETING THE CRITERIA OF ASTM D7508 AT A DOSAGE RATE DETERMINED BY THE FIBER MANUFACTURER FOR THE APPLICATION, BUT NOT LESS THAN 4 LB/CU YD.

CONCRETE WORK

- MONOLITHIC SLAB FINISHES: THE FOLLOWING REQUIREMENTS ARE BASED ON THE LATEST FLOOR FLATNESS (FF)/ FLOOR LEVELNESS (FL) VALUES/ METHODS. BIDS FOR THIS WORK SHALL REFLECT THESE REQUIREMENTS AND ENFORCEMENT THEREOF CAN BE EXPECTED.
- A. NON-CRITICAL FLOOR TOLERANCE
- a. FLOAT-FINISH (FLT-FN) b
- SPECIFIED OVERALL VALUE: FF25/FL20 MINIMUM LOCAL VALUE: FF25/FL20
- d. APPLY FLOAT FINISH TO MONOLITHIC SLAB SURFACES THAT ARE TO RECEIVE MUD SET TILE AND OTHER THICK FINISHES, AND TO
- SLAB SURFACES WHICH ARE TO BE COVERED WITH WATERPROOFING MEMBRANE. B. TYPICAL CORRIDOR OR NORMAL SIZED ROOMS (100-600 SF)
- a. TROWEL FINISH | (TR-FN |) SPECIFIED OVERALL VALUE: FF30/FL23
- MINIMUM LOCAL VALUE: FF25/FL20
- APPLY TROWEL FINISH TO MONOLITHIC SLAB SURFACES THAT ARE TO RECEIVE RESILIENT FLOORING, CARPET, PAINT, OR OTHER THIN FILM FINISH COATING SYSTEMS
- ELEVATED SLABS SHALL HAVE A SPECIFIED OVERALL VALUE OF FF22 AND FF27 AND A MINIMUM LOCAL OF FF20 WITH NO FL NUMBER DEFINED.
- MINIMUM THICKNESS OF SLAB ON GRADE IS THE GREATER OF 3" OR 0.8 TIMES ANCHOR EMBEDMENT SPECIFIED IN CONSTRUCTION DOCUMENTS (ASSUMES USE OF HILTI KWIK BOLT 3).
- CONTROL JOINTS SHOULD BE PLACED A MINIMUM 2'-O" OFF THE CENTERLINE OF COLUMNS. IF THE DISTANCE BETWEEN COLUMNS IS LESS THAN 4'-O" BUT GREATER THAN 2'-G" THEN PLACE AT MID-POINT OTHERWISE CONTACT ENGINEER OF RECORD.

SITE REQUIREMENT NOTES

- I. OWNER/ CONTRACTOR SHALL ENSURE THAT SITE IS STABILIZED AND MAINTAINED CURING HEAVY PRECIPITATION.
- 2. OWNER/ CONTRACTOR TO PROVIDE MATERIAL STORAGE AREA ON SITE
- OTHER THAN BUILDING BEING ERECTED. 3. OWNER/ CONTRACTOR SHALL PROVIDE A CONSTRUCTION DUMPSTER UNIT ON THE JOBSITE AT NO COST TO SELLER/ CONTRACTOR.
- ERECTED.
- 5. TEMPORARY POWER MUST MEET ALL APPLICABLE CODES AND SAFETY REQUIREMENTS
- BEGINNING INSTALLATION SEQUENCE AS AGREED.
- OBSTRUCTIONS TO DELIVERY OR ERECTION.
- AND STRUCTURAL MATERIALS, IS NOT THE RESPONSIBILITY OF SELLER/ SUBCONTRACTOR.
- PUBLIC SAFETY AND AGAINST ACCIDENTS, WEATHER OR ANY OTHER (INCLUDES FALL PROTECTION ON MULTI-STORY BUILDINGS).

MAINTENANCE NOTES

- I. ROOF MAINTENANCE GUIDELINES
- INSPECT ROOF FOR DAMAGE AFTER HEAVY STORM.
- PENETRATIONS WITH URETHANE SEALANT. F. ALWAYS GET MANUFACTURER APPROVAL BEFORE MAKING ANY
- MODIFICATION TO THE ROOF. WHEN PERFORMING ROOF MAINTENANCE ALWAYS TAKE THE FOLLOWING PRECAUTIONS
- A. USE FALL PROTECTION AND OTHER SAFETY EQUIPMENT AS REQUIRED.
- OR RIDGE FLASHING.
- C. DO NOT WALK ON LIGHT TRANSMITTING PANELS (LTP). THEY DO NOT SUPPORT A PERSON'S WEIGHT. GUARD ALL LTP'S AND ROOF OPENINGS
- STEP ONLY IN THE PANEL FLAT DIRECTLY ON OR IN CLOSE PROXIMITY TO A SUPPORTING ROOF STRUCTURAL.
- **KEY INSPECTION TIMES:**
- ROOF ARFA
- HAIL OR ABNORMALLY HEAVY RAINS OR ICE AND SNOW. AFTER OTHER TRADES HAVE BEEN ON THE ROOF FOR ANY REASON
- MUST BE PROTECTED DESIGNED AND INSTALLED ROOF WALKWAY SYS
- CORROSION POST-ERECTION ROOF CLEANING: AFTER THE ROOF INSTALLATION IS
- ANY RESULTING CORROSION.
- DEBRIS REMOVAL: ANY FOREIGN DEBRIS SUCH AS SAWDUST, DIRT, TIME. THE ROOF SHOULD BE PERIODICALLY INSPECTED FOR SUCH CONDITIONS AND IF FOUND. THEY SHOULD BE RECTIFIED IN A MANNER CONSISTENT WITH THESE ROOF MAINTENANCE GUIDELINES. CONTACT WITH ROOF PANELS, ESPECIALLY GALVALUME, FOR EXTENDED PERIOD OF TIME.

CONTINGENCIES

- THE CONTINGENCIES SPECIFIED IN THIS SECTION. LIMITS TO, THE FOLLOWING: CONSTRUCTION MANAGEMENT FEES.
- MATERIAL, DETAILING, FABRICATION, SHIPPING, INSTALLATION, AND INSPECTION COSTS. CONCRETE: 5CYREINFORCING STEEL: STRUCTURAL STEEL: 3.0 TONS C.F.M.F.:
- ANCHORS: THE SER.

4. OWNER/ CONTRACTOR TO PROVIDE TEMPORARY POWER TO ALLOW FOR A MAXIMUM POWER LEAD RUN OF 200 FEET TO EACH STRUCTURE BEING

6. OWNER/ CONTRACTOR MUST ENSURE THAT BUILDING PADS ARE BROOM CLEAN AND FREE OF DEBRIS PRIOR TO SELLER/ SUBCONTRACTOR CREW 7. OWNER/ CONTRACTOR SHALL ENSURE THAT THE SITE AND ALL SIDES OF

BUILDING ARE ACCESSIBLE WITH EQUIPMENT AND FREE FROM ANY 8. CLEANING OF MATERIALS, INCLUDING BUT NOT LIMITED TO WALL PANELS

9. THE OWNER/ CONTRACTOR SHALL PROPERLY PROTECT THE WORK FOR

HAZARDS WITH LIGHTS, GUARDRAILS OR BARRICADES AS APPLICABLE

A. WALK IN THE FLAT OF THE PANEL NEAR THE STRUCTURAL SUPPORTS. KEEP ROOF, GUTTERS AND DOWNSPOUTS FREE OF DEBRIS.

REMOVE EXCESS ICE AND SNOW ACCUMULATIONS AS NECESSARY. INSPECT AND RESEAL AS NECESSARY ALL ROOF CURBS AND OTHER

DO NOT WALK ON THE ROOF FLASHING SUCH AS GUTTER, RAKE, HIP,

A. AFTER A FIRE, VANDALISM, OR KNOWN DAMAGE TO AN ADJACENT

EXPOSURE TO SEVERE WEATHER CONDITIONS, INCLUDING HIGH WINDS,

INSPECT THE ROOF FOR DAMAGE CAUSED BY WORKERS INCLUDING CHEMICAL OR SOLVENT SPILLS, SCRATCHES IN THE FINISH, EXCESSIVE ROOF TRAFFIC, AND PUNCTURES. MAKE SURE THAT ANY DEBRIS OR SCRAP LEFT BEHIND BY THE WORKERS IS REMOVED FROM THE ROOF IMMEDIATELY. AVOID USING CUTOFF SAWS AND WELDING EQUIPMENT OVER THE ROOF IN CASES WHERE THIS IS NOT POSSIBLE, THE ROOF

FOOT TRAFFIC: KEEP FOOT TRAFFIC TO A MINIMUM, HEAVY FOOT TRAFFIC CAN CAUSE PONDING ON LOW PITCHED ROOFS. THIS IS A PARTICULARLY TRUE JUST ABOVE THE EAVE AND AT ENDLAPS. ALWAYS WALK IN A FLAT OF PANEL NEAR A SUPPORTING ROOF STRUCTURAL. DO NOT WALK ON TRIM OR IN GUTTERS. ON BARE GALVALUME ROOFS, EXCESSIVE FOOT TRAFFIC IS PLANNED FOR A ROOF, PROVISIONS SHOULD BE MADE FOR A PROPERLY

DRAINAGE: KEEP ROOF FREE OF DEBRIS AND KEEP DEBRIS OUT OF GUTTER TO ALLOW WATER TO QUICKLY DRAIN FROM ROOF. DO NOT USE WOOD BLOCKING TO HOLD UP EQUIPMENT OFF PANEL SEAMS. THIS BLOCKS THE FLOW OF WATER AND HOLDS MOISTURE. DO NOT ALLOW ROOFTOP A/C UNITS OR EVAPORATIVE COOLERS TO DRAIN ONTO THE ROOF. ANYTHING THAT TRAPS OF HOLDS MOISTURE ON A ROOF WILL CAUSE PREMATURE

COMPLETE, ALL FITTINGS, SHAVINGS, ETC. FROM FASTENER INSTALLATION, ETC. SHALL BE SWEPT COMPLETELY CLEAR OF THE ROOF PANELS. IF THIS DOES NOT HAPPEN, THE MANUFACTURER WILL NOT BE RESPONSIBLE FOR

LEAVES, ANIMAL DROPPINGS, ETC. WILL CAUSE CORROSION OF THE ROOF, GUTTERS, TRIM, ETC. IF LEFT ON BUILDING SURFACE FOR A LONG ENOUGH

NEVER ALLOW TREATED LUMBER OR CONCRETE/ MORTAR/ GROUT TO COME

THE CONTRACTOR IS TO PROVIDE AN ALLOWANCE IN THE BASE BID FOR

THE CONTINGENCIES ARE TO INCLUDE ALL COSTS ASSOCIATED WITH THE INSTALLATION OF THE STRUCTURAL COMPONENT. THIS INCLUDES BUT NOT

100 FEET, #G BAR

(30) 600-5-168-68 10'-0" LG. STUDS, 100 FT. OF 600-T-125-54. (20) ³/₄"Ø x 1'-0" LG. ALL CONTINGENCIES TO BE FABRICATED AND INSTALLED AS DIRECTED BY

MAINTENANCE NOTES

TERMITE PROTECTION

- ALL BUILDING PADS ARE TO BE PROTECTED FOR TERMITES AS REQUIRED BY GOVERNING CODE AND LOCAL JURISDICTION.
- FIRE RESISTANCE RATING REQUIREMENTS FOR BUILDING ELEMENTS CONSTRUCTION TYPES II-B ∉ V-B ALL BUILDING ELEMENTS RATINGS ARE ZERO (O).
- FIRE RESISTANCE RATING REQUIREMENTS FOR EXTERIOR WALLS

CONSTRUCTION TYPES II-B ∉ V-B (ALL USE GROUPS EXCEPT H) REQUIRE A MINIMUM FIRE SEPARATION DISTANCE OF TEN (10) FEET FOR A ZERO (0) RATING.

ERCENTAGE OF OPENINGS IN EXTERIOR WALLS

BUILDINGS WHOSE EXTERIOR WALL AND STRUCTURAL FRAME ARE NOT REQUIRED TO BE FIRE-RESISTANCE RATED SHALL BE PERMITTED TO HAVE UNLIMITED UNPROTECTED OPENINGS.

FIRE BARRIERS/ FIRE WALLS/ STRUCTURE DESIGN W/O SPRINKLERS

STORAGE OCCUPANCIES DESIGNED WITHOUT SPRINKLER SYSTEM CAN'T BE MORE THAN 3 STORIES CONTAIN FIRE AREA GREATER THAN 12,000 SF, OF HAVE COMBINED FIRE AREAS GREATER THAN 24,000 SF. OUR TYPICAL DESIGN STANDARD IS TO USE 3 HOUR FIRE BARRIERS TO DIVIDE BUILDING INTO FIRE AREAS AND FIRE WALLS TO SEPARATE LARGE STRUCTURES INTO SEPARATE BUILDING TO AVOID EXCEEDING MAXIMUM COMBINED FIRE AREAS.

CORRIDOR FIRE RESISTANCE RATING

IT IS THE INTENT OF DESIGN TO KEEP OCCUPANCY LOAD SERVED BY A CORRIDOR LESS THAN OR EQUAL TO COMPLY WITH ZERO RATING IN A BUILDING WITHOUT SPRINKLERS.

FIRE RATED DOOR AND HARDWARE

FIRE DOORS AND FRAME SHALL BE LABELED SHOWING THE NAME OF MANUFACTURER, THE NAME OF THE THIRD-PARTY INSPECTOR AGENCY, AND THE FIRE PROTECTION RATING. ALL RATED DOORS ARE TO BE INSTALLED WITH A POSITIVE STRIKE DEVICE, AND BE SELF-CLOSING.

PROTE RATI	CTION NGS	LIMI W	TING SIZES IRED GLAS	5 OF 55
TYPE	MINIMUM RATING (HOURS)	MAX. AREA (SQ. INCHES)	MAX. HEIGHT (INCHES)	MAX. WIDTH (INCHES)
FIRE BARR	IERS			
3 HOUR	3	0	0	0
2 HOUR	/2	100	33	10
I HOUR	I	100	33	10
RATED EXT	ERIOR WALLS			
3 HOUR	/2	0	0	0
2 HOUR	/2	0	0	0
I HOUR	3/4	1296	54	54
FIRE PARTI	TIONS			
I HOUR	1/3	1296	54	54

HARDWARE REQUIREMENTS FOR RATED DOORS

DOOR SHALL BE SELF OR AUTOMATIC CLOSING. DOORS SHALL BE PROVIDED WITH AN ACTIVE LATCH BOLT THAT WILL SECURE THE DOOR WHEN IT IS CLOSED

- 3. DOOR AND FRAME SHALL BE LABELED SHOWING NAME OF MANUFACTURER AND THE THIRD-PARTY INSPECTION AGENCY.

COMMON PATH OF TRAVEL

IF A BUILDING'S OCCUPANCY LOAD IS LESS THAN 50 THEN PERSONNEL DOORS ARE ALLOWED TO SWING BOTH IN THE DIRECTION OF EGRESS AND AGAINST THE DIRECTION OF EGRESS THUS INSTALLING EXIT LIGHTS ON BOTH SIDE OF PERSONAL DOORS IN CORRIDORS WOULD MAKE THE CORRIDOR A TWO-WAY PATH VERSUS A SINGLE-DIRECTION PATH.

ACCESSIBLE AREA REQUIREMENTS

HANDLES, PULLS, LATCHES, LOCKS, AND OTHER OPENING DEVICES SHALL HAVE A LEVER OPERATED MECHANISM. IF THE DOOR HAS A CLOSER, THEN THE SWEEP PERIOD OF THE CLOSER SHALL BE ADJUSTED SO THAT FROM AN OPEN POSITION OF 70 DEGREES, THE DOOR WILL TAKE AT LEAST 3 SECONDS TO MOVE TO A POINT 3 INCHES FROM THE LATCH MEASURED TO THE LEADING EDGE OF THE DOOR. THE MAXIMUM FORCE FOR PUSHING OR PULLING OPEN A DOOR SHALL BE AS FOLLOWS: EXTERIOR DOOR ≤ 8.5 LBF.

INTERIOR DOOR < 5 LBF.

REFERENCE THE DOOR DETAIL PAGE FOR DETAILS WITH REGARDS TO PROPER INSTALLATION.

			EAST COAST OFFICE: 1041 CROWN PARK CIRCLE WINTER GARDEN FL 34787 PHONE	CONFIDENTIAL: THIS DOCUMENT AND THE INFORMATION CONTAINED HI MAKORABCO UNAUTHORIZED COPYING, DISCLOSURE OR OTHER UNAUTH
		LIBERTY DELRAY.		DELKAY FL
ENGIN This it signed Beatti and da docum and see verifie 08/0	EER S	STAN No. 4 STA STA SON as bee sealed using rinted re not and th any e D23	AP M. A EN, 554 TE PRW NAV en e 1 by g a I cop t con te si lectr	OF OF OF OF OF OF OF OF OF OF OF OF OF O
RBE CONSULTING SERVICES, Z	2875 5PUT OAK COURT - OVIEDO, FL. 32766 WEBGITE: WWW.RBECS.COM - EMAIL: REECSLLOGGMAIL.COM MAIN PHONE LINE: 407-796-3820 - FAX LINE: 407-971-6716	Incentation DEPT: 407-796-3776 - EMAIL: NSHAH@REECS.COM ELECTROM. DEPT: 407-5617-3113 - EMAIL: FAMILIBIA@REECS.COM STRUCTURAL DEPT: 407-598-7658 - EMAIL: REEATLE@REECS.COM	ROBERT M. BEATTIE, PE; FLORIDA PE #55428 FLORIDA CERTIFICATE OF AUTHORIZATION #27269	REE CONSULTING SERVICES, LLC" HEREPY EXPRESSIV RESERVES ITS COMMON LAW COPRESENT AND THE PESICA CONTAINED WITHIN THESP PLANS AND THE DESICA CONTAINED WITHIN THESP PLANS AND THE DESICA CONTAINED WITHIN THE DESICA AND DRAWINGS SHALL NOT TO BE ALTERED, COPIED, OR ERPRODUCED MANY PORM OR MANURE WITHOUT THE EXPRESS WRITEN PERMISSION AND CONSENT OF "REE CONSULTING SERVICES, LLC".
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ISSUE



LINE KEY
 HORIZONTAL PARTITION PANEL 29GA GALVALUME TYPE "U"
 HALL WAY SYSTEM
 6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND 26GA VERT. OR HORIZ. PANEL EXTERIOR
 6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND WOOD VENEER BY OTHERS
 6" R-19 INSULATION IN WALL WITH 29GA PANEL BOTH SIDES

DOOR SCHEDULE

- 2 9'-0" x 7'-0" SLIDING DOOR (BY CUSTOMER)
- 3 3'-0" x 7'-0" PERSONNEL DOOR (BY CUSTOMER)
- (4) 6'-0" x 7'-0" DOUBLE GLASS DOOR (BY CUSTOMER)

WINDOW SCHEDULE

- A 14'-0" x 8'-0" WINDOW
- () | 4'-0" X 8'-0" WINDOW
- () | 4'-0" X 8'-0" WINDOW () | 4'-0" X 8'-0" WINDOW

NOTE: GC/CUSTOMER TO PROVIDE DISTANCE TO HOLD DOWN PARTITIONS FROM BOTTOM OF ROOF DECK BEFORE ANY MATERIALS CAN BE ORDERED.

PIER & HEADER FINISHES:

PREPAINTED PIERS & HEADERS

- THERE IS NO ADDITIONAL APPLICATIONS REQ'D.
 REMOVE THE PROTECTIVE FILM FROM AREAS OF BOTH PIERS & HEADERS WHERE THEY CONTACT EACH OTHER.
- LEAVE THE FILM ON TO PROTECT THE PREPAINTED SURFACE UNTIL CONSTRUCTION IS NEAR COMPLETION.
- GALVANEAL PIERS & HEADERS REQUIRING FIELD PAINTING.
- VERIFY PAINT COLOR WITH COLOR REQ'D IN LABOR CONTRACT.
 CAULK PIERS TO HEADERS AND HEADERS TO HEADERS IF STACKED.
- PRIME ALL EXPOSED SURFACES OF PIERS & HEADERS.
 PAINT ALL EXPOSED SURFACES OF PIERS & HEADERS WITH PAINT SPECIFIED IN
- LABOR CONTRACT EVENLY SO PRIMER CAN NOT BE SEEN THROUGH TOP COAT.
 IF TEXTURED, ROLL OUT TO ACHIEVE AND SAND FINISH STUCCO LIKE LOOK.



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|/8" = |'





	I	47'-2 <u>1</u> "
LINE KEY	,	
	- PARTITION PANEL	
29GA GALVA	ALUME TYPE "U"	
 HALL WAY S	YSTEM	
 6" R-19 INSI	ULATION IN PERIMETER WALL	
26GA VERT.	OR HORIZ. PANEL EXTERIOR	
 6" R-19 INSI 29GA PANEL	ULATION IN PERIMETER WALL . INTERIOR AND	

LINE KEY
HORIZONTAL PARTITION PANEL 29GA GALVALUME TYPE "U"
 HALL WAY SYSTEM
 6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND 26GA VERT. OR HORIZ. PANEL EXTERIOR
 6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND WOOD VENEER BY OTHERS

6" R-19 INSULATION IN WALL WITH 29GA PANEL BOTH SIDES

DOOR SCHEDULE

- (2) 9'-0" x 7'-0" SLIDING DOOR (BY CUSTOMER)
- 3 3'-0" x 7'-0" PERSONNEL DOOR (BY CUSTOMER)
- (4) 6'-0" x 7'-0" DOUBLE GLASS DOOR (BY CUSTOMER)
- WINDOW SCHEDULE
- (A) 14'-0" x 8'-0" WINDOW () | 4'-0" X 8'-0" WINDOW

- () | 4'-0" X 8'-0" WINDOW
- () | 4'-0" X 8'-0" WINDOW

NOTE: GC/CUSTOMER TO PROVIDE DISTANCE TO HOLD DOWN PARTITIONS FROM BOTTOM OF ROOF DECK BEFORE ANY MATERIALS CAN BE ORDERED.

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- PRIME ALL EXPOSED SURFACES OF PIERS & HEADERS.
- PAINT ALL EXPOSED SURFACES OF PIERS & HEADERS WITH PAINT SPECIFIED IN LABOR CONTRACT EVENLY SO PRIMER CAN NOT BE SEEN THROUGH TOP COAT.IF TEXTURED, ROLL OUT TO ACHIEVE AND SAND FINISH STUCCO LIKE LOOK.

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 		47'-2 <u>1</u> "				23'			47'-2 <u>1</u> "			'-4" 9'- "	l'-4"	35	5'-4"		'-6"	





|/8" = |'







LINE KEY									
BOX HDR	FRAMED COLUMN OPENING W/ BOX HEADER								
	6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND 26GA VERT. OR HORIZ. PANEL EXTERIOR								
	6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND WOOD VENEER BY OTHERS								
	6" R-19 INSULATION IN WALL WITH 29GA PANEL BOTH SIDES								
	CROSS BEAMS NEED MORE THAN 5' BAY								
	COLUMNS								
	ZEE BEAMS								
×××××	ROOF STRAPPING								
[
TYPICAL COLUMNS &	BEAMS UNLESS NOTED OTHERWISE								
COLUMN : MSI = 6CI	G - G"X2 1/2" X 16GA CEE PRIME PAINTED								
COLUMN : M52 = 6CI	4 - 6"X2 1/2" X 14GA CEE PRIME PAINTED								
PURLIN : PI = 6ZI6 -	6"X2 1/2" X 16GA ZEE PRIME PAINTED								

ANCHOR SCH	IEDULE		FASTENER ALL FASTE	<u>RS AND ANCHORS</u> ENERS SHALL BE INSTALLE	ED PER MANUFACTURER	S RECOMME	ENDATIONS AN	ID SPEC	CIFICATI	IONS
BRAND: DeWA	LT (OR EQUAL)									
ANCHOR TYPE	ANCHOR NAME	ICC-ES #	ANCHOR DIA.(IN)	MIN BASE MATERIAL THICKNESS (IN)	(MIN. OR NOM) / MAX. OR REF.) EMBEDMENT (IN.)	MIN ANCHOR SPACING (IN)	MIN EDGE / END DISTANCE (IN)	ALLOWA	BLE BAS LW CONC.	E MATERIAL CONC. OVER STEEL DECK
EPOXY	AC100+GOLD	ESR-2582	1/2"dia.	4" [CONC]	2-3/4"(MIN) / 6" MAX	2-1/2"	2-1/2"	Х	х	
		[CONC]	3/4"dia.	5-3/4" [CONC]	3-1/2"(MIN) / 9" MAX	3-3/4"	3-3/4"	1		
		ESR-3200	1/2"dia.	8" GF CMU	4" (MIN)	(I) PER CELL	4"	1		
		[CMU]	3/4"dia.	8" GF CMU	G" (MIN)	(I) PER CELL	4"	1		
MECHANICAL	POWER STUD	ESR-2818	1/2"dia.	4" [CONC]	2-1/2"(NOM) / 2" (EFF)	4-1/2"	6"	Х	Х	Х
	+SD1	[CONC]	3/4"dia.	6" [CONC]	4"(NOM) / 2" (EFF)	6"	6"	1		
		ESR-2966	3/8"dia.	8" GF CMU	2-1/2"(MIN)	(1) PER CELL	4"	1		
		[CONC]	1/2"dia.	8" GF CMU	4-3/4"(MIN)	(1) PER CELL	12"	1		
	POWER STUD	ESR-2502	1/2"dia.	4-1/2" [CONC]	2-1/2"(NOM) / 2" (EFF)	6"	8"	x	x	x
	+SD2	[CONC]	3/4"dia.	7" [CONC]	4-1/2"(NOM) / 3-3/4" (EFF)	6"	12"			
			3/8"dia.	$2\frac{1}{2}$ [CONC OVER STL DECK]	2-3/8"(NOM) / 2" (EFF)	- /2"	4"	1		
			1/2"dia.	2 ¹ / ₂ " [CONC OVER STL DECK]	2-1/2"(NOM) / 2" (EFF)	8"	4"	1		
	SCREW-	ESR-3889	1/4"dia.	8" GF [CONC]	2-1/2" (MIN.)	4"	2"			
	BOLT+	[CONC]	1/2"dia.	8" GF [CONC]	3-1/4" (MIN.)	6"	3"	1		
			1/2"dia.	3" [CONC]	I -5/8"(NOM) / I .2" (EFF)	- /2"	- /2"	l _x	x	×
			3/4"dia.	3" [CONC]	2"(NOM) / 1-1/3" (EFF)	2"	- /2"			
			3/8"dia.	3" [CONC]	2-1/2"(NOM) / 1-3/4" (EFF)	2-3/4"	I -3/4"			
			1/2"dia.	3-3/4" [CONC]	4-1/4"(NOM) / 3.08" (EFF)	3"	I -3/4"	1		
			1/4"dia.	2 ¹ / ₂ " [CONC OVER STL DECK]	1-5/8"(NOM) / 1.2" (EFF)	- /2"	- /2"			
			3/8"dıa.	2 ¹ / ₂ " [CONC OVER STL DECK]	2"(NOM) / 1.33" (EFF)	2"	2"	1		
			1/2"dia.	2 ¹ / ₂ [CONC OVER STL DECK]	2-1/2"(NOM) / 1-3/4" (EFF)	2-1/2"	2-1/2"	1		
		ESR-4042	1/2"dia.	8" GF CMU	4-1/4"(MIN)	(1) PER CELL	4"	1		
		[CMU]	3/4"dia.	8" GF CMU	6-1/4"(MIN)	(I) PER CELL	4"]		
DIRECT FASTEN	CSI SPIRAL DRIVE PINS	ESR-2024	0157.dıa	3/16" (STEEL)	FASTENER POINT MUST PENETRATE STEEL MEMBER	- /2"	1/2"	x	х	x
			0157.dia	2-1/4" [CONC]	3/4"(MIN)	4"	3-1/2"			
			0157.dia	3" [CONC]	I "(MIN)	4"	3-1/2"]		
			0157.dia	3-3/4" [CONC]	I - I /4"(MIN)	4"	3-1/2"	1		
			0157.dia	8" GF CMU	I "(MIN)	4"	3-3/4"	1		
			0157.dia 8" HOLLOW CMU I "(MIN) 8"	8"	3-3/4"	1				



ANCHOR SCHEDULE UNLESS NOTED OTHERWISE
BOTTOM FLOOR (MULTI)
ASE TRACK INSTALLATION /2"X3" DeWALT SCREW BOLT+ (OR EQUAL) SPACING 3' O.C. (1 3/4" EFFECTIVE EMBEDMENT)
ER INSTALLATION /2"X3" DeWALT SCREW BOLT+ (OR EQUAL) @ EA. PIER (1 3/4" EFFECTIVE EMBEDMENT)
EAVY STEEL DECK SUPPORT ANGLE (1/4" THICK OR GREATER) (TO CMU) '4"XG" DeWALT SREW BOLT+ (OR EQUAL) 2' O.C. (5" MIN. EMBEDMENT) (4" MIN. FROM EDGE OF CMU)
/4" MIN. EXPANSION ANGLE INSTALLATION (TO CMU)) 3/4"XG" DeWALT SREW BOLT+ (OR EQUAL) (5" MIN. EMBEDMENT) (4" MIN. FROM EDGE OF CMU)
SS STEEL COLUMN BASE PLATE INSTALLATION SE DeWALT AC200+ EPOXY TO INSTALL. DIAMETER TO MATCH ANCHOR IN COLUMN SCHEDULE X 12" DNG THREADED ROD WITH LEVELING NUTS (7" MIN. EMBEDMENT)



9 10 11 12 10' 10'-2" 9'-10"		(15) (16) (17)											
	450' OVERALL		<u>/</u>										
23'	70'		23'										
			L										
	MSI II'-6 I/8"	=	, 										
	" MSI II'-I 5/8"		I										
	MSI II'-0 3/8"		, 										
	GMS110'-111/8"		·										
	MSI 10'-9 7/8"												
	MSII0'-8 5/8"	p											
	MSI 10'-7 3/8"												
	MSI 10'-6 1/8"												
		 											
			 										
	MS1 10'-1 1/8"												
p p	MSI 9'-II 7/8"	 	, 										
	MSI 9'-10 5/8"												
	M51 9'-9 3/8"												
	-M52 -M51 9'-10 1/4" -M52	M52	-M52										
	1 5D-1 42'-2 ¹ / ₂ "	23'											





S-2.2



	LINE KEY
BOX HDR	FRAMED COLUMN OPENING W/ BOX HEADER
	6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND 2GGA VERT. OR HORIZ. PANEL EXTERIOR
	6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND WOOD VENEER BY OTHERS
	6" R-19 INSULATION IN WALL WITH 29GA PANEL BOTH SIDES
	CROSS BEAMS NEED MORE THAN 5' BAY
	COLUMNS
	ZEE BEAMS
×××××	ROOF STRAPPING
TYPICAL COLUMNS &	BEAMS UNLESS NOTED OTHERWISE
COLUMN : MSI = GCI	G - G"X2 1/2" X 16GA CEE PRIME PAINTED
COLUMN : MS2 = 6CI	4 - 6"X2 1/2" X 14GA CEE PRIME PAINTED
PURLIN : PI = 6ZI6 -	G"X2 1/2" X 16GA ZEE PRIME PAINTED

ANCHOR SCH	IEDULE	1	FASTENER ALL FASTE	<u>S AND ANCHORS</u> ENERS SHALL BE INSTALLE	ED PER MANUFACTURE	R'S RECOMME	ENDATIONS AN	D SPEC	CIFICATI	IONS
BRAND: DeWA	lt (or equal)									
ANCHOR TYPE	ANCHOR NAME	FAS1 ALL 1 IQUAL) ANC DIA COUAL) ICC-ES # ICONC] ANC DIA ICC-ES # ICONC] II2 ICONC] II2 ICONC] ICC-ES # ICONC] II2 ICONC] II2 ICONC] ICONC] II2 ICONC] II2 II2 ICONC] IT+ ICSR-3889 ICONC] II4 II2 ICONC] II2 ICONC] II2 II2 II2 ICONC] II2 II2 II2 II2 II2 II2 II2 II2 II2 II2	ANCHOR DIA.(IN)	MIN BASE MATERIAL THICKNESS (IN)	(MIN. OR NOM) / MAX. OR REF.) EMBEDMENT (IN.)	MIN ANCHOR SPACING (IN)	MIN EDGE / END DISTANCE (IN)	ALLOWA	BLE BAS LW CONC.	E MATERIAL CONC. OVER STEEL DECK
EPOXY	AC100+GOLD	ESR-2582	1/2"dia.	4" [CONC]	2-3/4"(MIN) / 6" MAX	2-1/2"	2-1/2"	х	х	
		[CONC]	3/4"dia.	5-3/4" [CONC]	3-1/2"(MIN) / 9" MAX	3-3/4"	3-3/4"			
		ESR-3200	1/2"dia.	8" GF CMU	4" (MIN)	(I) PER CELL	L 4"			
		[CMU]	3/4"dia.	8" GF CMU	G" (MIN)	(I) PER CELL	4"			
MECHANICAL	POWER STUD	ESR-2818	1/2"dia.	4" [CONC]	2-1/2"(NOM) / 2" (EFF)	4-1/2"	6"	Х	Х	х
	+SD1	[CONC]	3/4"dıa.	6" [CONC]	4"(NOM) / 2" (EFF)	6"	6"			
		ESR-2966	3/8"dıa.	8" GF CMU	2-1/2"(MIN)	(I) PER CELL	4"	1		
		[CONC]	1/2"dia.	8" GF CMU	4-3/4"(MIN)	(I) PER CELL	12"			
	POWER STUD	ESR-2502	1/2"dia.	4-1/2" [CONC]	2-1/2"(NOM) / 2" (EFF)	6"	8"	v	v	v
	+SD2	[CONC]	3/4"dıa.	7" [CONC]	4-1/2"(NOM) / 3-3/4" (EFF)	6"	12"			~
			3/8"dıa.	2 ¹ / ₂ [CONC OVER STL DECK]	2-3/8"(NOM) / 2" (EFF)	- /2"	4"			
			1/2"dia.	2 ¹ / ₂ [CONC OVER STL DECK]	2-1/2"(NOM) / 2" (EFF)	8"	4"	İ		
	SCREW-	ESR-3889	1/4"dia.	8" GF [CONC]	2-1/2" (MIN.)	4"	2"			
	BOLT+	[CONC]	1/2"dia.	8" GF [CONC]	3-1/4" (MIN.)	6"	3"			
			1/2"dia.	3" [CONC]	-5/8"(NOM) / .2" (EFF)	- /2"	I-I/2"	v	v	~
			3/4"dıa.	3" [CONC]	2"(NOM) / 1-1/3" (EFF)	2"	I-I/2"		~	~
			3/8"dıa.	3" [CONC]	2-1/2"(NOM) / 1-3/4" (EFF)	2-3/4"	I -3/4"	1		
			1/2"dia.	3-3/4" [CONC]	4-1/4"(NOM) / 3.08" (EFF)	3"	-3/4"			
			1/4"dia.	2 ¹ / ₂ [CONC OVER STL DECK]	-5/8"(NOM) / .2" (EFF)	- /2"	- /2"	1		
			3/8"dıa.	2 ¹ / ₂ " [CONC OVER STL DECK]	2"(NOM) / 1.33" (EFF)	2"	2"			
			1/2"dia.	2 ¹ / ₂ [CONC OVER STL DECK]	2-1/2"(NOM) / 1-3/4" (EFF)	2-1/2"	2-1/2"			
		ESR-4042	1/2"dia.	8" GF CMU	4-1/4"(MIN)	(I) PER CELL	4"	1		
		[CMU]	3/4"dıa.	8" GF CMU	6-1/4"(MIN)	(I) PER CELL	4"			
DIRECT FASTEN	CSI SPIRAL DRIVE PINS	ESR-2024	0157.dia	3/I G" (STEEL)	FASTENER POINT MUST PENETRATE STEEL MEMBER	- /2"	1/2"	х	х	х
			0157.dia	2-1/4" [CONC]	3/4"(MIN)	4"	3-1/2"			
			0157.dia	3" [CONC]	L"(MIN)	4"	3-1/2"	1		
			0157.dia	3-3/4" [CONC]	- /4"(MIN)	4"	3-1/2"	1		
			0157.dia	8" GF CMU	L"(MIN)	4"	3-3/4"	1		
			0157.dia	8" HOLLOW CMU	I "(MIN)	8"	3-3/4"	1		



ANCHOR SCHEDULE UNLESS NOTED OTHERWISE
BOTTOM FLOOR (MULTI)
ASE TRACK INSTALLATION /2"X3" DeWALT SCREW BOLT+ (OR EQUAL) SPACING 3' O.C. (1 3/4" EFFECTIVE EMBEDMENT)
IER INSTALLATION /2"X3" DeWALT SCREW BOLT+ (OR EQUAL) @ EA. PIER (1 3/4" EFFECTIVE EMBEDMENT)
EAVY STEEL DECK SUPPORT ANGLE (1/4" THICK OR GREATER) (TO CMU) /4"X6" DeWALT SREW BOLT+ (OR EQUAL) 2' O.C. (5" MIN. EMBEDMENT) (4" MIN. FROM EDGE OF CMU)
/4" MIN. EXPANSION ANGLE INSTALLATION (TO CMU) 3) 3/4"XG" DeWALT SREW BOLT+ (OR EQUAL) (5" MIN. EMBEDMENT) (4" MIN. FROM EDGE OF CMU)
SS STEEL COLUMN BASE PLATE INSTALLATION SE DeWALT AC200+ EPOXY TO INSTALL. DIAMETER TO MATCH ANCHOR IN COLUMN SCHEDULE X I 2" DNG THREADED ROD WITH LEVELING NUTS (7" MIN. EMBEDMENT)



NORTH BLDG. COLUMN FRAMING PLAN

|/8" = |'

	32)	33		35) (3	9'-1 0"	37) (3	38	39 (40) (41)
			450'	OVERALL						1
70	1			/	23'	ł				73'-
	2 5D-1			8 2						(SD-
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		MS2	Г-M52			MS2	Г _М52		LMS2	L _{MS2}
				F		-M62	-M62			
	r								M52	
	47'-2 <u>1</u> "		- 	9'-11"	-4	35'	4"	/	11'-6"	







(I 2 9'-IO" IO'-2	(3) (4) (4) (0)	5 6 (10' 10' 10'	7 (8) I O' I O'	9 10 (10' 10'-2" 10'-2"	9'-10"		4 (15) (1 10' 10' 10'	6 (17) (1 10' 10' 10'	8 (19) (20 (21) (22) 10' 10' (22)	(23) 10'-2" 9'-10" (24)
						450' OVERALL						
		65'-3"		23'			70'		2.	3'	48'-9" TO MATCHLINE	
IN E Q				#					#			MATCHLINE
			YI				P	l ł	·1			
	PI	PI	PI	PI	PI	PI	P	IP	PIF	1	PI PI	P
õ	P1	P1	P1	P1	P1	PI	P	IP	°IF	1	PI PI	PI
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50 50					PI		F	I F				
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							+					
									I			
.N A		51'-9 <u>2</u> "	11'-6"	25'-6"	'-4" 9'- "	"	42'-2 ¹ /2"		23'	42'-2 ^j	'-4'	9'- " -4"

	LINE KEY
BOX HDR	FRAMED COLUMN OPENING W/ BOX HEADER
	6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND 26GA VERT. OR HORIZ. PANEL EXTERIOR
	6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND WOOD VENEER BY OTHERS
	6" R-19 INSULATION IN WALL WITH 29GA PANEL BOTH SIDES
	CROSS BEAMS NEED MORE THAN 5' BAY
U	COLUMNS
	ZEE BEAMS
×××××	ROOF STRAPPING
TYPICAL COLUMNS 4	BEAMS UNLESS NOTED OTHERWISE
COLUMN : MSI = 6CI	G - 6"X2 1/2" X 16GA CEE PRIME PAINTED
COLUMN : MS2 = 6CI	4 - 6"X2 1/2" X 14GA CEE PRIME PAINTED
PURLIN: PI = GZIG -	6"X2 1/2" X 16GA ZEE PRIME PAINTED



NORTH BLDG. ROOF FRAMING PLAN

|/8" = |'



ISSUE

24	25 (26) 10' 10' 10	27 28 10' 10'	29 (30) 0' 0'		33) (34) (10' 10'	35 0'-2" 9'- 0" 36 37 9'- 0"	7) (38) O' O'	(39) (40) (41) 10' 10' 10'
		23'		70'	450'	OVERALL 23'		73
		PI	PI	PI	PI	 PI	PI	PI
 P	P1	PI	PI	PI	P1	P1	P1	PI
P P	P1	P1	P1	PI	P1	P1	P1	PI
PI	PI	P1	P1	PI	P1	PI	P1	P1
P	PI	PI	PI	PI	PI	P1	P1	P1
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 	P1	P1	P1	PI	PI	PI	PI	P1
 	P1	PI	PI	PI	PI	P1	PI	PI
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P	PI	PI	PI	PI	P1	PI	P1	P I
P	PI	PI	PI	PI	P1	PI	PI	PI
P				PI	PI		PI	PI
	P1	PI	PI	PI	P1	PI		PI
	PI	PI	PI	PI	P1	PI	PI	PI
	47'-2 <u>1</u> "		23'	47'-2 <u>1</u> "		<u>-</u> 	35'-4"	<u>∎</u> <u>∎</u> '-6"
			۲́		1			<i>* * *</i>

LINE KEY									
BOX HDR	FRAMED COLUMN OPENING W/ BOX HEADER								
	6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND 26GA VERT. OR HORIZ. PANEL EXTERIOR								
	6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND WOOD VENEER BY OTHERS								
	6" R-19 INSULATION IN WALL WITH 29GA PANEL BOTH SIDES								
	CROSS BEAMS NEED MORE THAN 5' BAY								
U	COLUMNS								
	ZEE BEAMS								
\times	ROOF STRAPPING								
r									
TYPICAL COLUMNS ∉	BEAMS UNLESS NOTED OTHERWISE								
COLUMN : MSI = GCI	G - G"X2 1/2" X 16GA CEE PRIME PAINTED								
COLUMN : M52 = 6CI	4 - 6"X2 1/2" X 14GA CEE PRIME PAINTED								
PURLIN : PI = GZIG -	6"X2 1/2" X 16GA ZEE PRIME PAINTED								





NORTH BLDG. ROOF FRAMING PLAN

|/8" = |'



DRAWN BY AW/MNG CHECKED BY JOB NO. E 2705 SCALE SHEET AS NOTED

BLDG. D

ROOF FRAMING PLANS

06-12-23

XXX

SHEET TITLE

DATE

:*



ISSUE

S-2.7

(L) (K) (.	4'-10"	ı) (5'	H 5'	G	(F) (E	4'-10"	5'-4")
		- 3 1/2" (R-11) UNFAC WITH ZEE BEAMS) G (RUNS PERPENDICUL (3" WIDE X 5/8" HIGF	CED INSULATION(RL " (R- I 9 VINYLBACK AR TO ZEE BEAMS 1 (R-5)	JNS PARALLEL ED ROOF INSL.) THERMAL BLOCK			— 24" X 2	24ga STANDING SEA	AM		
	10'-9 <u>7</u> "	- 0- 80 - 0-	- 7 <u>3</u> "			- 0-3 <u>8</u>		-0- -0- 	= 	0 	9105 <u>-</u> 0
			° SECTIO								
		_	SCALE: 1/	$4^{"} = 1^{'}$			NOTES I. UN HE	ILESS NOTED OTHE EIGHTS USED FOR D	RWISE ON THE PLA DESIGN PURPOSES	NS. THE FOLLOWING	

2. NON-CLIMATE CONTROLLED SINGLE STORY BUILDINGS HAVE BEEN DESIGNED BASED ON THE TOP OF ALL INTERIOR PARTITION PANELS BEING HELD TO WITHIN 8" OF THE BOTTOM OF THE ROOF DECK.

3. CLIMATE CONTROLLED SINGLE STORY BUILDING HAVE BEEN DESIGNED BASED ON THE TOP OF ALL

INTERIOR PARTITION PANELS BEING HELD TO WITHIN 18" OF THE BOTTOM OF THE ROOF DECK. 4. CLIMATE CONTROLLED MULTI-STORY BUILDING HAVE BEEN DESIGNED BASED ON THE TOP OF ALL

INTERIOR PARTITION PANELS BEING HELD TO WITHIN 18" OF THE BOTTOM OF THE FLOOR/ROOF DECK.

5. SHOULD THE PARTITION PANELS NEED TO BE HELD DOWN FURTHER THAN 18" FROM THE BOTTOM OF THE FLOOR/ROOF DECK, THE REQUEST SHALL BE SUBMITTED IN WRITING AND A FORMAL WRITTEN RESPONSE PROVIDED PRIOR TO FABRICATION AND/OR INSTALLATION.

NOTES

ALL MAN DOORS, ROLL-UP DOORS, SLIDING GLASS DOORS, WINDOWS AND TRANSOMS SHALL BE FIELD VERIFIED PRIOR TO FABRICATION TO ENSURE PROPER FIT. G.C./SUBCONTRACTOR SHALL NOT FABRICATE FROM THE CONSTRUCTION DOCUMENTS. SHOULD THE G.C./ SUBCONTRACTOR CHOOSE TO FABRICATE THE PREVIOUSLY REFERENCED ITEMS FROM THE CONTRACT/PROPOSAL DOCUMENTS AND NOT FROM THE FIELD THEY DO SO AT THEIR OWN RISK AND ANY AND ALL MODIFICATIONS REQUIRED AS A RESULT SHALL BE AT THEIR OWN EXPENSE.

NOTE

GC SHALL COORDINATE ALL T/CMU WALL HEIGHTS WITH RABCO ENTERPRISES, LLC PRIOR TO CONSTRUCTION TO ENSURE PROPER FIT OF THE METAL BUILDING COMPONENTS. FAILURE TO GET WRITTEN APPROVAL PRIOR TO ERECTING THE CMU WALLS MAY RESULT IN NEEDED MODIFICATIONS TO EITHER THE CMU WALL AND/OR THE METAL BUILDING COMPONENTS AT THE G.C.'S EXPENSE.

NES STANDARD PARTITION

14ga TOP TRACK

- 6" R-19 INSULATION IN PERIMETER WALL 29ga PANEL INTERIOR AND 26ga VERTICAL PANEL EXTERIOR

INSET CEE GIRT W/ CLIPS @ 5'-0" O.C. MAX - 6CI6 AT 10'-0" O.C. I Gga BOTTOM TRACK

ISSUE

S-2.8

	(32)		(33)		(34)		(35)		36		(37)		(38)		39		(40)
0'	/	O'		10'		10'	50' OVERAL	10'-2"		9'- 0"		10'		O'		0'	
	70'							k		23'		L					
			,														
					+												
					+		+										
					+												
		47'-2 <u>1</u> "					'-4"	9'- "	'-4"				35'-4"				11'-6

|/8" = |'

W-BEAM SCHEDULE								
BI	W18X35	R= I OK						
B2	W24X68	R=20K						
B3	W12X22	R=5K						

TYP. BEAM HEIGHTS(TO BE VERIFIED BY G.C./STEEL FABRICATOR PRIOR TO CONSTRUCTION / FABRICATION.)BOTTOM & MIDDLE FLOORSTOP OF BEAMS = BOTTOM OF METAL DECK (4.5" BELOW ELEVATED SLAB, U.N.O.)INFILL FRAMING BELOW BEAMS MAY BE REQUIRED AT ANY OPENINGS, U.N.O.TOP FLOORBOTTOM OF BEAMS = 2.5" ABOVE OPENING OF WINDOW, TO ALLOW FOR BLOCKINGFOR PROPER WINDOW ATTACHMENT. INFILL FRAMING EITHER SIDE OF WINDOW MAY BEREQUIRED TO CREATE ROUGH OPENING.

COLUMNS (SEE COL. SCHEDULE ON SHEET SBD-1 FOR ADDITIONAL INFO.)							
COLUMN MARK	COLUMN SIZE						
C44-4	HSS4x4x1/4						
C44-6	HSS4x4x3/8						
C44-8	HSS4x4x1/2						
C66-4	HSS6x6x1/4						
C66-6	HSS6x6x3/8						
C66-8	HSS6x6x1/2						
C88-4	HSS8x8x1/4						
C88-6	HSS8x8x3/8						
C88-8	HSS8x8x1/2						
C88-10	HSS8x8x5/8						
C 0 0-4	HSSIOxIOxI/4						
C1010-6	HSS 0x 0x3/8						
C1010-8	HSSIOxIOxI/2						
C 0 0- 0	HSS 0x 0x5/8						
CB-I	8x16 CONC. BM.						
TC-I	8x24 CONC. COL.						
TC-2	I GX I GX8 CONC. COL.						
TC-3							

0 LIBERTY DELRAY DELRAY FL ENGINEER STAMP BERT M. BEA ATTILL No. 55428 * × : * STATE OF STATE OF STATE OF STATE OF STATE OF STATE OF STORIDA mm This item has been electronically signed and sealed by Robert M. Beattie, PE using a Digital Signature and date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies. 08/04/2023 Digitally signed by Robert Beattie Date: 2023.08.04 14:46:53-04'00' ENGINEER DRM DRM N COPY THESE F THIS C ALTEREI SHEET TITLE BLDG. D BLACK IRON PLANS DATE 06-12-23 DATE DRAWN BY CHECKED BY JOB NO. SCALE SHEET AW/MNG XXX E 2705 AS NOTED SB-1.0

	(32)	(33	(34)	(35)	(36	37)	(3	8	(39)		(40)
10					450' OVERA	10-2"	9-10		10.	, 10		10	
	70'					1	23'		1				
								·					
								·					
												<u> </u>	
	4	7'-2 <u>1</u> "			'-4"	9'- "	'-4"		35'-4				11'-6

Received 7.17.23

|/8" = |'

$\frac{\text{TOP FLOOR}}{\text{BOTTOM OF BEAMS}} = 2.5" \text{ ABO'}$ FOR PROPER WINDOW ATTACHME REQUIRED TO CREATE ROUGH OF	VE OPENING OF WINDOW, TO ALLOW FOR BLOCKING ENT. INFILL FRAMING EITHER SIDE OF WINDOW MAY E PENING.
COLUMNS (S SHEET SBD-1	GEE COL. SCHEDULE ON FOR ADDITIONAL INFO.)
COLUMN MARK	COLUMN SIZE
C44-4	HSS4x4x1/4
C44-6	HS54x4x3/8
C44-8	HSS4x4x1/2
C66-4	HSS6x6x1/4
C66-6	HSS6x6x3/8
C66-8	HSS6x6x1/2
C88-4	HS58x8x1/4
C88-6	HS58x8x3/8
C88-8	HS58x8x1/2
C88-10	HSS8x8x5/8
C1010-4	HSS 0x 0x /4
C1010-6	HSS 0x 0x3/8
C1010-8	HSS 0x 0x /2
C1010-10	HSS 0x 0x5/8
CB-1	8x16 CONC. BM.
TC-I	8x24 CONC. COL.
TC-2	I 6x I 6x8 CONC. COL.

W-BEAM SCHEDULE

TYP. BEAM HEIGHTS (TO BE VERIFIED BY G.C./STEEL FABRICATOR PRIOR TO CONSTRUCTION / FABRICATION.) BOTTOM & MIDDLE FLOORS TOP OF BEAMS = BOTTOM OF METAL DECK. (4.5" BELOW ELEVATED SLAB, U.N.O.) INFILL FRAMING BELOW BEAMS MAY BE REQUIRED AT ANY OPENINGS, U.N.O. TOP FLOOP

R=_K

R=_K

W_X_

W__X__

B2

TC-3

PRELIMINARY

			EAST COAST OFFICE: 1041 CROWN PARK CIRCLE WINTER GARDEN FL 34787 PHONE		CONFIDENTIAL: THIS DOCUMENT AND THE INFORMATION CONTAINED HE MAKORABCO UNAUTHORIZED COPYING, DISCLOSURE OR OTHER UNAUTH	
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RBE CONSULTING SERVICES, D	2875 5PLT DAK COURT - OVIEDO, FL. 32766 WEBSITE: WWW.RBECS.COM - EMAIL: RBECSLC@GMAIL.COM MANN PHONE LINE: 407-776-3820 - FXX LINE: 407-971-6718	MECHANICAL DEFT: 407-796-3776 - EMAL: NSHAH@RBECS.COM ELECTRICAL DEFT: 407-617-3113 - EMALI: NAMILBNA@RBECS.COM 5TRUCTURAL DEFT: 407-588-7858 - EMALI: RBEATIE@RBECS.COM	ROBERT M. BEATTIE, PE; FLORIDA PE #55428 FLORIDA CERTIFICATE OF AUTHORIZATION #27269	REE CONSULTING SERVICES, LLC" HEREBY EXPRESSLY RESERVED ITS COMMON LAW COPPRIGHT AND OTHER REOPERTY REIHTS TO THESE PLANS AND THE DESIGN CONTAINED WITHIN. THIS DESIGN AND DRAWINGS SHALL NOT TO BE	ALTERE, OPPED, OR REMOUNDED IN ANY PRIM OR MANUER WITHOUT THE EXPRESS WRITTIN PERMISSION AND CONSENT OF "REE CONSULTING SERVICES, LLC"	
SHEET TITLE BLACK IRON PLANS DATE OG-12-23 DRAWN BY AW/MNG CHECKED BY XXX JOB NO. E 2705 SCALE AS NOTED SHEET SB-2.0						
	S	SВ	-2	2.0		

ISSUE

	CC	DLI	JM	Ν	ŧ [BAS	5E						
	PLA	$\Delta T I$	ΞS	Ct	1E	DU	LE						
COLUMN			BASE PLATE (")		ANC BOI	CHOR LT (")	CAP PLATE	E		ADY			<u> /2"</u>
MARK	SIZE	A	В	T	E (NT 2≰3)	d			REMARKS				
C44-4	HSS4x4x1/4	10	10	1/2	7	3/4							
C44-6	HSS4x4x3/8	10	10	3/4	7	3/4							
C44-8	HSS4x4x1/2	10	10		7	3/4						↓ □	
C66-4	HSS6x6x1/4	12	12	1/2	7	3/4							"A"
C66-6	HSS6x6x3/8	12	12	3/4	7	3/4							BP-A
C66-8	HSS6x6x1/2	12	12		7	3/4						NOTE: BY MIN.	INCREASE . 1/4" FRO
C66-10	HSS6x6x5/8	12	12	- /4	7	3/4						SH	IOWN IN S
C84-6	HSS8x4x3/8	16	12		7	3/4							
C88-4	HSS8x8x1/4	14	4	1/2	7	3/4							
C88-6	HSS8x8x3/8	14	14	3/4	7	3/4							
C88-8	HSS8x8x1/2	14	14		7	3/4							
C88-10	HSS8x8x5/8	14	14	- /4	7	3/4							
CI0I0-4	HSS 0x 0x /4	16	16	1/2	7	3/4							
CI0I0-6	HSS 0x 0x3/8	16	16	3/4	7	3/4							
CI0I0-8	HSS Ox Ox /2	16	16		7	3/4							
C 0 0- 0	HSS 0x 0x5/8	16	16	- /4	7	3/4							
NOTES: I. ANCHOR THREAD TACK W BOTTON CONFOR	OTES: . ANCHOR BOLTS SHALL BE ASTM A3G (UNO) THREADED EACH END WITH NUT AT BOTTOM, TACK WELDED SECURE. PLATE WASHER AT BOTTOM NUT SHALL NOT BE REQUIRED. CONFORM TO CHART AT RIGHT FOR EMBEDMENT,				N	BOLT IATERIAL TYPE	M EME LE	INIMUM BEDDED ENGTH	MAXIMU EMBEDD LENGTH	JM)ED 1	MINIMUM EMBEDDED EDGE DISTANCE		
UNLESS	NOTED OTHERWISE IN 5" ARE NOT ACCEPTAB	SCHED	ULE ABOV RNATES	E.	AB	307, A36		12d E = FTG 5d >4 m.			5d >4 ın.	++	
3. PLATE W EQUAL 1	PLATE WASHERS SHALL HAVE HOLE WITH DIAMETER A325, A445				A325, A449 17d - 5" 7d >4 in.				7d - 5" 7d >4 in.		7d >4 ın.		
	OVERSIZ	ED H F(OLES OR BA	AND Se f	PLA PLATI	TE WA ES	SHEF	25				<u>2</u> d	"A"
BOLT DIAMETE	- HOLE ER DIAMETER	۲	IL WASH	IERS	D	BOLT	R	HOLE DIAME	E TER	₽U	VASHERS	RASE	PI ATF
3/4"	5/16	п	5/16";	<3"x3"		/2"		2 5/	16"		7/16"x5"x5"	NOTF (*).	NOT TC
7/8"	1 9/16	11	5/16";	<3"x3"		3/4"		2 3/	/4"		7/16"x5"x5"	• COL. THI	CKNESS

| |3/|6"

2 1/16"

|"

| |/4"

3/8"x4"x4"

3/8"x4"x4"

2"

2 1/2"

3 1/4"

3 3/4"

1/2"x6"x6"

1/2"x6"x6"

BASE PLATE & ANCHOR BOLT DETAIL

SCALE: N.T.S.

Received 8.9.23

BEAM SIZE (SEE PLAN)	NO. ROWS OF BOLTS	ANGLE LENGTH	MAXIMUM ALLOWABLE END REACTION (KIPS)
W8, WIO	2	12	37.1
WI2, WI4	3	20	55.7
WIG, WI8	4	21	74.2
W21	5	15	92.8
W24	6	18	111.0
W27	7	21	130.0

