DESIGN CODES USED:	
 FLORIDA BUILDING CODE 7TH EDITION [2020] - BUILDING FLORIDA BUILDING CODE 7TH EDITION [2020] - ACCESSIBILITY FLORIDA BUILDING CODE 7TH EDITION [2020] - ENERGY CONSERVATION FLORIDA FIRE PREVENTION CODE, 7TH EDITION [2020] FLORIDA BUILDING CODE 7TH EDITION [2020] - LIFE SAFETY FLORIDA BUILDING CODE 7TH EDITION [2020] - FUEL GAS FLORIDA BUILDING CODE 7TH EDITION [2020] - MECHANICAL FLORIDA BUILDING CODE 7TH EDITION [2020] - PLUMBING 	
• NATIONAL ELECTRIC CODE [N.E.C.] NFPA 70 2014 (FBC ADOPTED DEC. 31, 2017)	

FLG

FMG FS FT

FTG GA

GALV

GB GC

GFC

GT

HORIZ

LNTL

LP

LONG

MAS

MAX

MB

M

MCJ

MECH MEZZ

MFR

MIN

MO

MS

MTL

NTS OA

OC

OD

OF

OH

OPP PAF

PEMB

PERP PC

PL

PLF

PLYWD

PNL

PSF

PSI

PTN

OPNG

MISC

MBM

15

AB	ANCHOR BOLT
ABV	ABOVE
ACI	AMERICAN CONCRETE
	INSTITUTE
ADD'L	
AFF AGGR	ABOVE FINISHED FLR. AGGREGATE
AISC	AMERICAN IRON AND
/ 100	STEEL CONSTRUCTION
AISI	AMERICAN IRON AND
	STEEL INSTITUTE
AL	ALUMINUM
ALT	ALTERNATE
ARCH ASTM	ARCHITECT(URAL) AMERICAN SOCIETY OF
AJTM	TESTING MATERIALS
AWS	AMERICAN WELDING
	SOCIETY
B/	BOTTOM OF
BB	BOND BEAM
BLDG	BUILDING
BLK	BLOCKING
BM BOT	BEAM BOTTOM
BP	BASE PLATE
BRDG	BRIDGING
BRG	BEARING
BRK	BRICK
BS	BOTH SIDES
BT	BOTTOM TRACK
C/C CANT	CENTER TO CENTER CANTILEVER
CANT	CONCRETE BEAM
CFS	COLD FORMED STEEL
CJ	CONSTRUCTION /
	CONTROL JOINT
CL	CENTER LINE
CLR	CLEAR(ANCE)
СМИ	CONCRETE MASONRY
COL	UNIT
CONC	CONCRETE
CONN	CONNECTION
CONT	CONTINUOUS
CONTR	CONTRACTOR
CTR	CENTER
CTR'D	CENTERED
CY DIA	CUBIC YARD DIAMETER
DIAG	DIAGONAL
DL	DEAD LOAD
DL	DOUBLE LOK
DN	DOWN
DTL	DETAIL
DWG	DRAWING
DWL	DOWEL
EA EE	EACH EACH END
EF	EACH FACE
EJ	EXPANSION JOINT
ENG	ENGINEER
EOS	EDGE OF SLAB
EL	ELEVATION
EQ	EQUAL
EQ SP	EQUAL SPACE(S) (ING)
ES EW	EACH SIDE EACH WAY
EXP	EXPANSION
EXT	EXTERIOR
F/	FROM
FDN	

FOUNDATION

FDN

BE	BREVIATIONS
	FINISHED FLOOR
	FLOOR
	FLANGE FRAMING
	FAR SIDE
	FOOT
	FOOTING
	GAUGE
	GALVANIZE(D)
	GRADE BEAM
	GENERAL CONTRACTOR
	GROUT FILLED CELL(S)
	COARSE
	HORIZONTAL
	HIGH POINT
	HEADED STUD
	HOLLOW STRUCTURAL
	SECTION
	INSIDE DIAMETER
	INSIDE FACE
	INTERIOR
	JOIST
	JOINT
	KIP
	LONG
	LINTEL
	LONG POINT
	MASONRY
	MAXIMUM
	MACHINE BOLT
	METAL BUILDING
	MANUFACTURER
	MOMENT CONNECTION
	MASONRY CONTROL
	JOINT
	MECHANICAL
	MEZZANINE
	MANUFACTURE(ER)
	MINIMUM
	MASONRY OPENING
	METAL STUD
	METAL
	NOT TO SCALE
	OVERALL
	ON CENTER
	OUTSIDE DIAMETER
	OUTSIDE FACE
	OVER HEAD
	OPENING
	OPPOSITE
	POWER-ACTUATED
	FASTENER PRE-ENGINEERED
	METAL BUILDING
	PERPENDICULAR
	PRECAST
	PLATE
	POUNDS PER
	LINEAR FOOT
	PLYWOOD
	PANEL
	POUNDS PER
	SQUARE FOOT
	POUNDS PER
	SQUARE INCH
	PARTITION
	RADIUS

	RC	REINFORCED CONC.
	REF	REFERENCE
	REINF	REINFORCE(D) (ING)
	REQ	REQUIRE
	REQ'D	REQUIRED
_	RF	ROOF
_	RL	RAIN LIP
_	RO	ROUGH OPENING
_	RP	RADIUS POINT
_	RTN	RETURN
_	RW	RETAINING WALL
_	SC	SLIP CRITICAL SCHEDULE
_	SCH SECT	SECTION
_	SG	SUBGIRT
_	SHT	SHEET
	SJ	SAWCUT JOINT
	SJI	STEEL JOIST INSTITUTE
	SL	SHEET LEDGE
	SP	SPECIAL
	SPECS	SPECIFICATIONS
	SQ	SQUARE
	SSL	SHORT SLOTTED
	STD	STANDARD
	STIFF'R	STIFFENER
	STL	STEEL
	STR	STRENGTH
	STRL	STRUCTURAL
	SW	SHORT WAY OR
		SHEAR WALL
	STMM	SYMMETRICAL
	T¢B	TOP ∉ BOTTOM
	TB	TIE BEAM
	TC	TIE COLUMN
	TDS	TURNED-DOWN SLAB
	TEMP	TEMPERATURE
	THK	THICK
	THNS	THICKENED SLAB
	TOP'G	TOPPING
	TYP	TYPICAL
	T/	TOP OF
_	TT	TOP TRACK
	UNO	UNLESS NOTED
_		OTHERWISE
	UD	ULTRA DEK
	VERT	VERTICAL
	WD	
	WF WO	WALL FOOTING WINDOW OPENING
	WWF	WINDOW OF LINING
	W/	WITH
	VV/	
		<u></u>

School B06 C B 95 C Delray Beach Golf Club	
	RID NU
TYPICAL GRID E	
	6" R- 29GA 26GA
	6" R- 29GA
	PBR, PANE
	CROS 5' BA
	HORI. 29GA
	HALL

L COLU — — _____ ZEE

TWO @ ST

TWO @ Ell

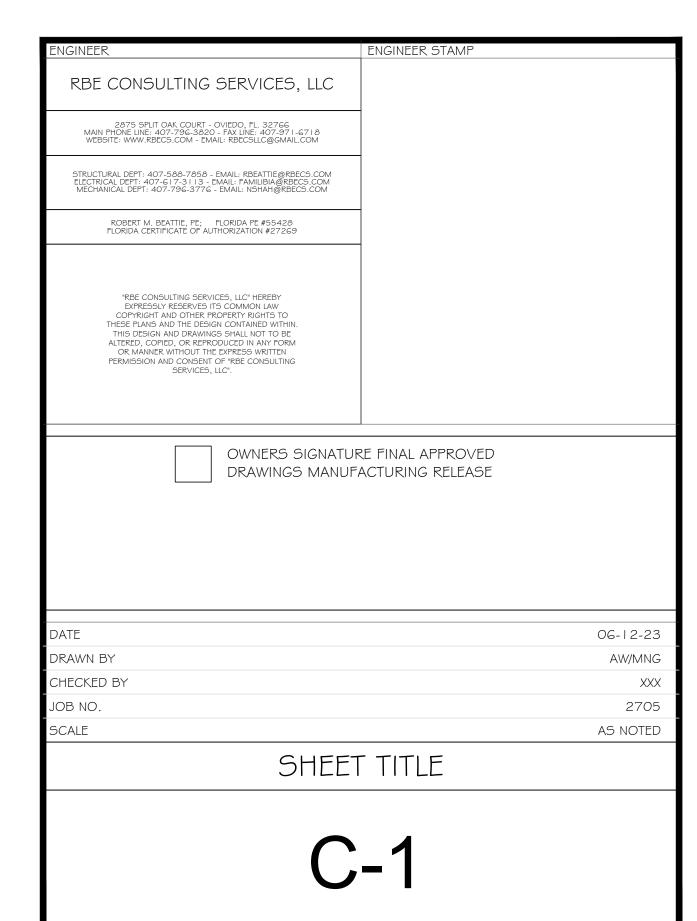
LIBERTY DELRAY DELRAY, FL

	JOB SITE			SHEE
				JIILL
	Puttin Around		C-1	COVER SHEET
	IdaRd		C-2	BUILDING LAYOUT
			C-3	BUILDING LAYOUT
	Hampton Inn Delray B		S-0	GENERAL NOTES
	Hampton Inn Delray B		S-0.0	CODE ANALYSIS
			S-0.1	CODE ANALYSIS
	Arts Garage	ATA Deal Grand Report	5-0.2	ROOF ANALYSIS
		Opal Grand Resort	5-0.3	INSULATION SPECS
	16 Delray Beach		5-0.4	SPECIAL INSPECTION
			S-0.5	SPECIAL INSPECTION
	Courtyard by		5-0.6	TYPICAL SCHEDULE
			S-0.7	TYPICAL SCHEDULE
			5-1.0	BUILDING C PARTITION PLAN
			5-1.1	BUILDING C PARTITION PLAN
			5-1.2	BUILDING C COLUMN FRAMIN
			5-1.3	BUILDING C COLUMN FRAMIN
			5-1.4	BUILDING C ROOF FRAMING
			S-1.5	BUILDING C ROOF FRAMING
			5-1.6	BUILDING C INSULATION & RC
			S-1.7	BUILDING C ELEVATIONS
TYPI	CAL NOMINCLATURE		5-1.8	BUILDING C ELEVATIONS & SE
	А		5-2.0	BUILDING D PARTITION PLAN
	A		5-2.1	BUILDING D PARTITION PLAN
	AAVIEWING DIRECTION	VIEWING DIRECTION	5-2.2	BUILDING D COLUMN FRAMIN
NID NUM. / LETTER	DETAIL NUM.	SECTION CUT LABEL	5-2.3	BUILDING D COLUMN FRAMIN
	PAGE NUM.		5-2.4	BUILDING D ROOF FRAMING I
			S-2.5	BUILDING D ROOF FRAMING I
			5-2.6	BUILDING D INSULATION & RC
			S-2.7	BUILDING D ELEVATIONS
			5-2.8	BUILDING D ELEVATIONS & SE
BUBBLE	TYPICAL DETAIL FLAG	TYPICAL SECTION CUT		
AL LINE KEY	TYP	ICAL PANELS	5-3.0	BUILDING E PARTITION PLAN
			5-3.1	BUILDING E COLUMN FRAMIN
		3' COVERAGE	5-3.2	BUILDING E ROOF FRAMING F
CORNER PIER, HEADER, PIER		3 COVERAGE	5-3.3	BUILDING E INSULATION & RC
			5-3.4	BUILDING E ELEVATIONS
6" R-19 INSULATION IN PERIMETER	WALL	PBR PANEL STEEL LINE	S-3.5	BUILDING E SECTION
29GA LINER PANEL INTERIOR AND				
26GA VERTICAL PANEL EXTERIOR			S-4.0	BUILDING F PARTITION PLAN
			5-4.1	BUILDING F COLUMN FRAMIN
6" R-19 INSULATION IN WALL WITH	ł	3' COVERAGE	5-4.2	BUILDING F ROOF FRAMING F
29GA LINER PANEL BOTH SIDES			5-4.3	BUILDING FINSULATION & RC
		PBU PANEL REV ROLL STEEL LINE	5-4.4	BUILDING F ELEVATIONS
PBR, 26GA VERTICAL PERIMETER W	ALL		S-4.5	BUILDING F SECTION
PANEL			0 1.0	
			SD-1	STRUCTURAL DETAILS
CROSS BEAMS NEED MORE THAN		2'-8" COVERAGE	SD-2	STRUCTURAL DETAILS
5' BAY			SD-3	STRUCTURAL DETAILS
		PBD PANEL STEEL LINE	SD-4	STRUCTURAL DETAILS
HORIZONTAL PARTITION PANEL			50-4	
29GA GALVALUME TYPE "U"			SB-1.0	BUILDING C BLACK IRON LAYO
			SB-2.0	BUILDING D BLACK IRON LAYO
HALL WAY SYSTEM	ł – – – – – – – – – – – – – – – – – – –	3' COVERAGE	SB-3.0	
				BUILDING E BLACK IRON LAYO
		7.2 PANEL STEEL LINE	SBD-1	BLACK IRON DETAILS
COLUMNS LI-FLANGE	EG)			
WEB			SF-1.0	BUILDING C FOUNDATION PLA
TOP FLANGE			SF-2.0	BUILDING D FOUNDATION PLA
ŕ	ł	3' COVERAGE	SF-3.0	BUILDING E FOUNDATION PLA
ZEE BEAMS WEB			SF-4.0	BUILDING F FOUNDATION PLA
BOT FLANGE				
		2.0 VL 18GA GGO STEEL LINE	SFD-1.0	FOUNDATION DETAILS
TWO HOUR RATED CMU WALL @ STAIR UL # U905				
TWO HOUR RATED CMU WALL		3' COVERAGE		
@ ELEVATOR UL # U905				
THREE HOUR RATED SEPERATION		1.5" 20GA B-DECK STEEL LINE		
WALL UL #U419				

	ALL OPENINGS MUST BE FIELD VERIFIED
SHEET INDEX	
	UPDATED 06-26-23
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TION PLAN	06-26-23
6	06-26-23

MARKUPS / REVISIONS									
NUMBER	BY	ISSUE	DATE						
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FOUNDATION NOTES

GENERAL

- THESE PLANS AND THE INFORMATION CONTAINED HEREIN ARE THE PROPERTY OF METAL BUILDING COMPANY, UNAUTHORIZED COPYING, DISCLOSURE OR OTHER UNAUTHORIZED USES ARE PROHIBITED.
- OWNER / CONTRACTOR IS RESPONSIBLE TO PROVIDE METAL BUILDING COMPANY WITH APPROVED PLANS PRIOR TO FABRICATION.
- 3. OWNER / CONTRACTOR IS RESPONSIBLE FOR ALL NECESSARY AND REQUIRED PERMITS, FEES, DEPOSITS, ECT.
- THE OWNER AND/OR CONTRACTOR SHALL REVIEW AND DETERMINE THAT ALL DIMENSIONS ARE COORDINATED AS REQUIRED WITH ALL OTHER DESIGN PROFESSIONALS DRAWINGS AND SHOP DRAWINGS FOR PROJECT PRIOR TO FABRICATION OF MATERIALS OR THE START OF CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO THE METAL BUILDING COMPANY AND ENGINEER OF RECORD.
- 5. SHOP DRAWING ARE CRITICAL TO ENSURE THE DIMENSIONS AND DESIGN OUTLINED IN THESE PLANS MEET THE MINIMUM REQUIREMENTS REQUIRED BY THESE SCOPES OF WORK IF UNDER CONTRACT BY OTHERS. IN THE EVENT THE CONTRACTOR'S OR OWNER'S FAILING TO PROVIDE, HE SHALL BE RESPONSIBLE FOR THE RESULTS OR ANY SUCH ERRORS OR OMISSIONS AND THE COST OF RECTIFYING THE SAME (EXAMPLES: ELEVATOR, STAIRWELL, DOORS, ETC....).
- 6. IF ANY ERRORS OR OMISSIONS APPEAR IN THE DRAWINGS. SPECIFICATIONS OR OTHER DOCUMENT, THE OWNER / CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD AND METAL BUILDING COMPANY IN WRITING OF SUCH OMISSIONS OR ERRORS PRIOR TO PROCEEDING WITH ANY WORK WHICH APPEARS IN QUESTION. EVERY REASONABLE EFFORT HAS BEEN MADE TO ENSURE COORDINATION BETWEEN THESE DRAWINGS AND THE STRUCTURAL PLANS. IN THE EVENT THE CONTRACTOR'S OR OWNER'S FAILING TO GIVE SUCH NOTICE, THEY SHALL BE RESPONSIBLE FOR THE RESULTS OR ANY SUCH ERRORS OR OMISSIONS AND THE COST OF RECTIFYING THE SAME.
- 7. ANY OMISSIONS AND/OR CONFLICTS WITH PLANS SHALL BE REPORTED TO METAL BUILDING COMPANY SO THAT THEY CAN BE RESOLVED PRIOR TO PROCEEDING WITH WORK.
- 8. DO NOT SCALE DRAWINGS IF A REQUIRED DIMENSION IS MISSING PLEASE CONTACT THE METAL BUILDING COMPANY AND / OR ENGINEER OF RECORD.
- 9. NO MODIFICATIONS TO PLANS SHALL BE MADE WITHOUT THE PERMISSION OF METAL BUILDING COMPANY AND ENGINEER OF RECORD. MODIFICATIONS REQUIRED DUE TO FIELD CONDITIONS OR OTHER CONTRACTORS OR ITEMS THAT WHICH MAY ADVERSELY AFFECT THE STRUCTURE REQUIRES WRITTEN PERMISSION (NO MODIFICATIONS TO STRUCTURAL MEMBERS IS ALLOWED).
- IO. ALL SECTIONS AND DETAILS SHALL BE CONSIDERED TO BE TYPICAL OR SIMILAR UNLESS ANOTHER SECTION OR DETAIL IS REFERENCED ON THE PLANS.
- II. SCOPE OF WORK OF METAL BUILDING COMPANY IS INDICATED IN THE CONTRACT. THE DRAWINGS REFLECT SCOPES OF WORK AS REQUIRED FOR PERMITTING OR AT THE DIRECTION OF OWNER / CONTRACTOR.
- 12. SUBMITTALS TO THE ENGINEER OF RECORD FOR REVIEW MUST CONTAIN THE CONTRACTOR'S OR OWNER'S STAMP SIGNIFYING THEIR REVIEW / ACCEPTANCE. SUBMITTALS SENT WITHOUT WILL BE RETURNED AT THEIR EXPENSE WITHOUT REVIEW. A MAX. OF THREE SETS ADDITIONAL SETS WILL BE DISCARDED
- 13. THE CONTRACTOR OR OWNER SHALL TAKE ALL NECESSARY STEPS TO PROTECT THE STRUCTURE, THE WORK PERSONS AND OTHER PEOPLE DURING CONSTRUCTION. HE SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION.

SLAB ON GRADE

UNLESS SPECIFICALLY STATED OTHERWISE IN THE GEOTECHNICAL SOILS REPORT, THE FOLLOWING MINIMUM CRITERIA SHALL BE ADHERED TO.

- a.) INTERIOR FILL SHALL BE COMPACTED TO A MINIMUM OF 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY TEST (ASTM D | 557).
- b.) COMPACTION OF THE SOIL IN THE FIELD SHALL BE MONITORED/CONTROLLED BY A REPRESENTATIVE OF A QUALIFIED LABORATORY.
- c.) EACH LAYER OF FILL SHALL NOT EXCEED 12" THICK AND SHALL BE COMPACTED PRIOR TO PLACEMENT OF THE NEXT LAYER.

MAXIMUM SPACING OF CONTROL JOINTS SHALL BE AS SHOWN IN THE TABLE BELOW. PATTERNS SHALL BE APPROXIMATELY SQUARE W/ RATIO OF LONG SIDE TO SHORT SIDE NOT TO EXCEED 1.5 TO 1.0.

SLAB THICKNESS (IN) -	4	5	6	7	8	9	10
SPACING (FT)	12	13	15	18	20	23	25

MIX DESIGNS CONTAINING AGGREGATE LESS THAN 3/4" ARE NOT ACCEPTABLE CUT SLAB AS SOON AS AGGREGATE DOES NOT DISLODGE (MUST BE WITHIN THE SAME DAY AS THE CONC. WAS PLACED)

CARE SHALL BE TAKEN BY THE GENERAL CONTRACTOR WHEN DETERMINING THE LOCATION OF SJ'S AND CJ'S TO ENSURE SLAB JOINTS DOES NOT READ THROUGH THE ARCHITECTURAL FINISHES.

WAREHOUSE SLABS SHALL BE POWER-TROWELLED TO A HARD, SMOOTH BURNISHED FINISH. THE FINAL TROWEL PASS SHALL BE DONE BY MACHINE - NOT BY HAND. WITHIN 30 MINUTES OF THE FINAL TROWEL PASS, THE FLOOR SHALL BE CURED WITH EUCLID'S SUPER REZ-SEAL OR APPROVED EQUAL, WHICH MAY BE WAIVED AT THE OWNER'S OPTION.

SLAB THICKNESS SHALL BE INCREASED AS REQUIRED TO PROVIDE ADEQUATE SUPPORT FOR CRANE LOADS WITHOUT CRACKING SLAB.

ALL CONCRETE SLABS ON GRADE SHALL BE A MINIMUM OF 4" THICK AND BE REINFORCED WITH 6 X6 W I.4 X W I.4 EXCEPT WERE SPECIFICALLY NOTED ON PLANS. FIBERMESH CONCRETE INSTEAD OF WIRE MESH IS AN ACCEPTABLE ALTERNATE ON SINGLE STORY BUILDING AND MULTISTORY BUILDING WITHOUT LOAD BEARING FLOOR PADS. FIBERMESH SHALL BE IN COMPLIANCE WITH ASTM C-1116 TYPE III AND ASTM CIIG LEVEL I AND SHALL BE PLANT BATCH MIX WITH PROPORTIONS OF I.5 POUNDS OF FIBERMESH PER CUBIC YARD OF CONCRETE.

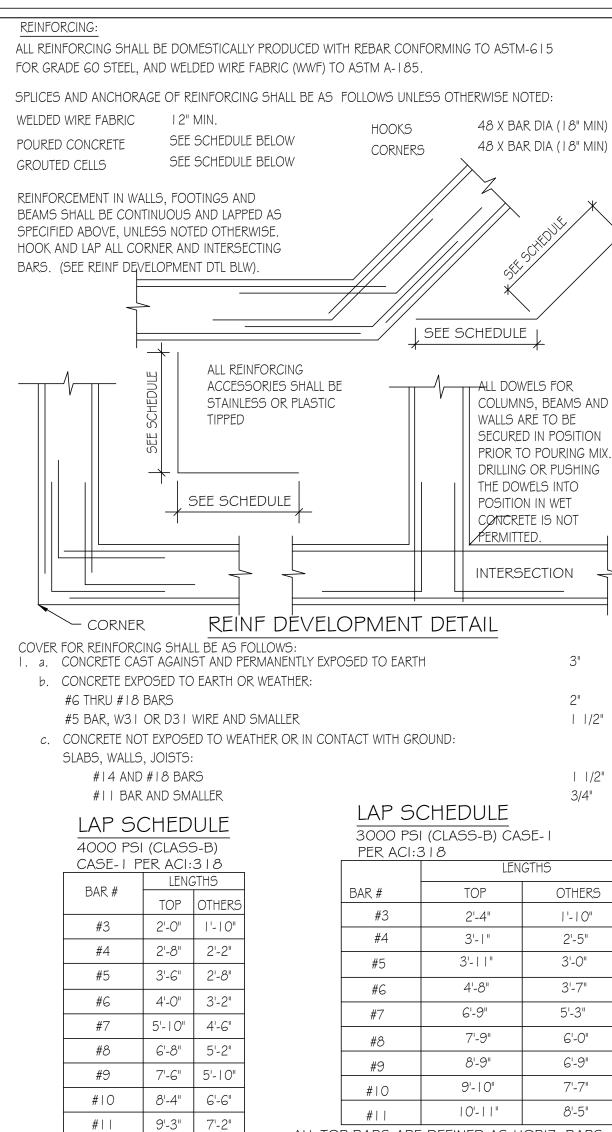
PLACE A MINIMUM 6 MIL POLYETHYLENE VAPOR BARRIER (LAPPED A MIN. OF 6") OVER COMPACTED SOIL BETWEEN FOUNDATION AND SLAB UNLESS NOTED OTHERWISE IN GEOTHECHNICAL ENGINEERS REPORT FOR THE PROJECT.

DESIGN CRITERIA

CONSTRUCTION DOCUMENTS WERE DESIGNED AND MEET THE REQUIREMENTS OF THE OF THE LOCAL BUILDING CODE DESIGNATED UNDER STRUCTURAL DESIGN CRITERIA. (SEE STRUCTURAL PLANS FOR DESIGN LOADS).

FOUNDATIONS

- CONFORMANCE WITH THE REQUIREMENTS OUTLINED IN THE GEOTECHNICAL SOILS REPORT SHALL BE DETERMINED BY PERFORMING INDUSTRY STANDARD SOIL DENSITY TESTS BY A CERTIFIED TESTING AGENCY.
- 2. IF SUBSURFACE INVESTIGATION FOR FOUNDATIONS HAS NOT BEEN PERFORMED AND GEOTECHNICAL ENGINEERING REPORT WAS NOT AVAILABLE FOR FOUNDATION DESIGN. PRIOR TO CONSTRUCTION, THE OWNER SHALL RETAIN THE SERVICES OF A QUALIFIED GEOTECHNICAL ENGINEER TO PERFORM SOILS BORINGS. PROVIDE RECOMMENDATIONS FOR FOUNDATION DESIGN (INCLUDING NET ALLOWABLE SOIL BEARING PRESSURE) PROVIDE EARTHWORK CONSTRUCTION CRITERIA AND PERFORM SOIL TESTING DURING CONSTRUCTION. THE OWNER / CONTRACTOR IS RESPONSIBLE FOR PERFORMING ALL EARTHWORK OPERATIONS IN STRICT ACCORDANCE WITH THIS REPORT. IF THE FOUNDATION RECOMMENDATIONS AND NET ALLOWABLE SOIL BEARING CAPACITY OR ANY OTHER ASSUMPTIONS (SEE BELOW) DIFFER FROM THE ASSUMED VALUE, THEN MODIFICATIONS TO THE STRUCTURAL DRAWINGS SHALL BE REQUIRED. SHOULD THIS OCCUR, THE OWNER / CONTRACTOR SHALL STOP CONSTRUCTION AND NOTIFY METAL BUILDING COMPANY AND THE ENGINEER OF RECORD IMMEDIATELY.
- FOUNDATION PLANS HAVE BEEN DESIGNED WITH THE FOLLOWING ASSUMPTIONS IN THE ABSENCE OF A SUBSURFACE INVESTIGATION BY A GEOTECHNICAL ENGINEER. NET ALLOW. SOIL BEARING PRESSURE OF 2,000 PSF.
- CONDITIONS DISCOVERED BY THE CONTRACTOR AND/OR GEOTECHNICAL FIELD REPRESENTATIVE DURING EXCAVATION WHICH MAY PREVENT THE ATTAINEMENT OF THE ALLOWABLE BEARING PRESSURE STATED IN THE GEOTECHNICAL SOILS REPORT, SHALL BE REPORTED TO THE ENGINEER.
- 5. THE SHEETLEDGES, RAINLIPS AND MASONRY LEDGES ARE VITAL TO THE PROPER FIT OF THE STEEL CONSTRUCTION. OWNER / CONTRACTOR SHALL FIELD VERIFY ALL TO BE AS SHOWN ON PLANS. IF THEY ARE NOT TO THE DRAWINGS CONTACT METAL BUILDING COMPANY PRIOR TO FABRICATION OR BEFORE STEEL ERECTION STARTS.
- 6. ALL ISOLATED PAD FOOTING ARE TO BEAR A MINIMUM OF 18" BELOW THE TOP OF CONCRETE SLAB / PAVEMENT OR A MINIMUM OF 12" BELOW FINISHED GRADE U.N.O. ON PLANS.
- 7. THE MINIMUM BEARING WIDTH OF CONTINUOUS FOOTINGS SHALL NOT BE LESS THAN I 2" UNLESS SPECIFICALLY NOTED IN GEOTECHNICAL ENGINEERING REPORT.
- 8. PLANS AND DETAILS REFLECT A DESIGN TO ACCOMMODATE A MAXIMUM FROST PROTECTION OF 12" UNLESS NOTED OTHERWISE IN THE CONSTRUCTION DOCUMENTS. IF THE REQUIRED FROST DEPTH EXCEEDS 12" THAN ENGINEER OF RECORD SHALL BE NOTIFIED PRIOR TO EXCAVATION OR FOUNDATIONS.



AND REP	RESENTS TH	E RECOMMENDED	OW IS FROM THE VUL WELDED WIRE FABRIC	2.		IANUAL
	ECK TYPE .5VL,VLIorR	TOTAL SLAB DI <= 4 3/4"		NDED WELDED WIR WI.4 X WI.4	E FABRIC	
	.5VL,VLIorR			W2. X W2.		
	2VLI 2VLI	<= 5 /4"		NI.4 X WI.4 V2.1 X W2.1		
	3VLI	<= 6 /4"		WI.4 X WI.4		
	3VLI	> 6 1/4"	6 X 6 - V	W2.1 X W2.1		
 CO MEI DET 25I CO MA DET 4LE 	ETING THE CI ERMINED BY B/CU YD IS A NCRETE SPEC CROSYNTHE ERMINED BY	CIFIED IN ACCORE RITERIA OF ASTM 7 THE FIBER MANL AN ACCEPTABLE A CIFIED IN ACCORE TIC FIBERS MEETI 7 THE FIBER MANL	DANCE WITH ASTM CI A820, TYPE I, TYPE II, JFACTURER FOR THE A ALTERNATIVE TO THE W DANCE WITH ASTM CI NG THE CRITERIA OF A JFACTURER FOR THE A .TERNATIVE TO THE W	OR TYPE V, AT A PPLICATION, BUT WF SPECIFIED AE I I G, TYPE III, CO STM D7508 AT A PPLICATION, BUT	DOSAGE RATE NOT LESS THA 30VE. NTAINING DOSAGE RATE NOT LESS THA	N
NCRETE: CONCRETE	SHALL HAV		MIN. PROPERTIES:	28 DAY STRENGTH	SLUMP	MAX AGGR.
	FOUNDAT		OCATION	3,000 psi	4" <u>+ </u> "	/2"
		GRADE (UP TO 4" GRADE (OVER 4"	•	3,000 psi 4,000 psi	4" <u>+</u> " 4" + "	/2"
	TIE BEAMS			4,000 psi	4" + "	3/4"
	TIE COLUN	MNS PLACE BEAMS		3,000 psi 4,000 psi	4" <u>+</u> " 4" + "	3/4"
		PLACE COLUMNS		4,000 psi	4" + "	"
	EQUIPMEN TILT-UP PA	NT SUPPORTS		4,000 psi 4,000 psi	4" <u>+</u> " 4" + "	"
		NDER TILT-UP PAN	IELS	4,000 psi 5,000 psi	8" + "	3/8"
		SLABS FORMED		4,000 psi	4" + "	"
		SLABS FORMED	W/ MIL DECK	4,000 psi 2,500 psi	4" <u>+</u> " 8" + "	" 3/8"
	NOTES:					_,_
	 SEE M COLD 	IASONRY GENERA JOINTS ARE NOT	D SLOPING SURFACES L NOTES FOR GROUT RECOMMENDED - ALTI MINIMUM OF 2'-0" OFF	TESTING REQUIRE HOUGHT IF REQUI	MENTS. RED THEY	
S R	TRUCTURAL EINFORCED NCRETE MIX PROPOS METHOD ENTRAPP	CONCRETE FOR E CONCRETE AND A DESIGN SHALL N ED MIX DESIGN S 2. PED AIR CONTENT	ORM TO LATEST EDITIO BUILDINGS, ACI 318 B CI 315 DETAILS AND I IEET THE FOLLOWING (HALL BE ACCORDANCE SHALL NOT EXCEED 3 TRAIN AIR ARE NOT AC	UILDING CODE RE DETAILING OF COI CRITERIA: E WITH ACI 301 N %.	QUIREMENTS F NCRETE REINFO 1ETHOD I OR	OR RCEMENT.
4			ESIGN STRENGTH AT 2		CONCILLE TO L	
			PTABLE. ADDING WAT	ER TO THE MIX W	ILL RESULT IN R	REJECTION
		S BY THE ENGINE				
		E IN THEIR USE AN	FOR THE ADEQUACY O ID REMOVAL.	I THE FURING ANI	J SHUKING ANI	JTUR
EX0 TU FEE	CEEDING 7 I BULAR SHUT T. ALTERNA	/2 FEET IN HEIGH ES LOCATED SUC TE PLACEMENT M	EINFORCED COLUMNS T. CONCRETE SHALL E CH THAT THE FREE AIR ETHOD OF CONCRETE D BY ENGINEER OF RE	BE PLACED THROL DROP OF THE MIX WITH OR WITHOL	IGH "ELEPHANT (DOES NOT EX	TRUNK CEED G
<u>۱</u>	MONOLITHIC	SLAB FINISHES				
			TS ARE BASED ON THE			
,		()	ES/METHODS. BIDS FORCEMENT THEREOF			
	. FLOAT F 2. SPECIFI 3. MINIMU 4. APPLY F MUD SE	ET TILE AND OTHE	JE: FF25/FL20 FF20/FL17 MONOLITHIC SLAB SU R THICK FINISHES, AN	D TO SLAB SURF		RE
-	TYPICAL CLAS TROWEL FINIS . SPECIFI 2. MINIML	SSROOM, CORRII SH I (TR-FNI) IED OVERALL VALU IM LOCAL VALUE:	,	ROOMS (100 TO		
-	ARGE ROOM ROWEL FINIS . SPECIFI 2. MINIML	AS (GOI SF AND (SH 2 (TR-FN2) IED OVERALL VALU JM LOCAL VALUE:	FF30/FL22	EAS:		
Ē	4. RESILIE ELEVATED SL	NT FLOORING, CA ABS SHALL HAVE	O MONOLITHIC SLAB S ARPET, PAINT, OR OTHI A SPECIFIED OVERALL	ER THIN FILM FINIS	5H COATING SY	STEM.
			'H NO FL NUMBER DEF MENTS: FLOOR FLATN		F99 TF979 014	
	ELOOR SLABS N ASTM EI I MAY BE WAIN MIN = A/10.	S SHALL BE CONE 55-87, WITH THE /ED AT THE OWNE FLOOR TOLERAN	DUCTED IN ACCORDAI E EXCEPTION OF SUBP R'S OPTION, ALSO ZI (ICE MEASUREMENTS S	NCE WITH THE PRO ARAGRAPHS 7.2.3 CALCULATION TO	DVISIONS SET I 3 AND 7.3.2 W BE USED SHALI Y THE INDEPENE E FINAL TROWEL	HICH L BE N DENT ING

HANDRAILS, POSTS AND SUPPORT CONNECTIONS SHALL BE DESIGNED BY A SPECIALTY ENGINEER, HIRED BY THE STEEL FABRICATOR. SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW AND MUST BE SIGNED & SEALED BY A ENGINEER REGISTERED IN THE SAME STATE AS THE PROJECT LOCATION. DESIGN LOADING(S) SHALL CONFORM TO AT LEAST THE MINIMUM REQUIREMENTS OF THE APPLICABLE BUILDING CODE. (SEE DESIGN CRITERIA).

LOCATION. MINIMUM DESIGN LIVE LOAD SHALL BE 100 PSF.

MASONRY

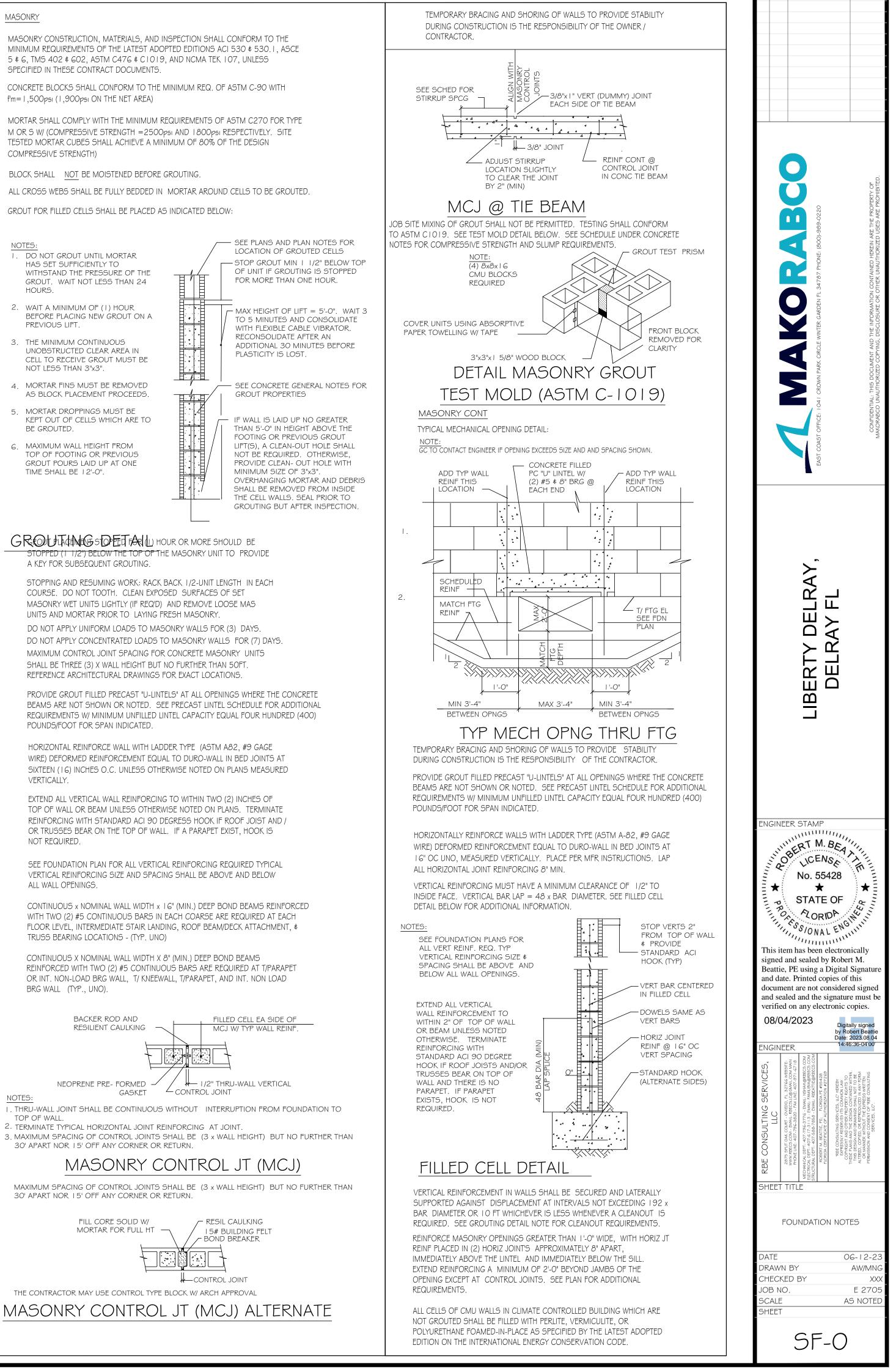
SPECIFIED IN THESE CONTRACT DOCUMENTS.

f'm=1,500psi (1,900psi ON THE NET AREA)

COMPRESSIVE STRENGTH)

BLOCK SHALL NOT BE MOISTENED BEFORE GROUTING.

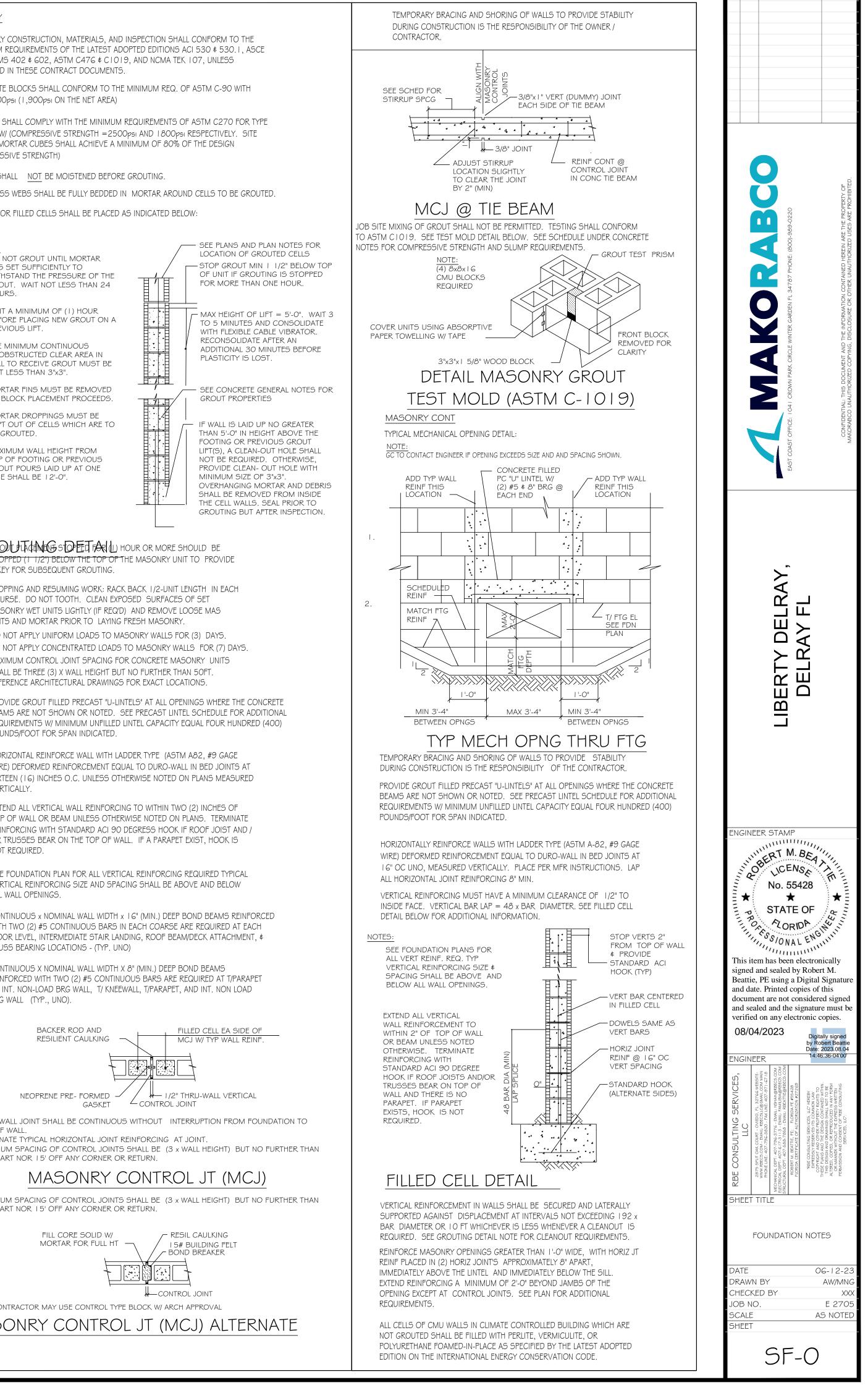
GROUT FOR FILLED CELLS SHALL BE PLACED AS INDICATED BELOW



GROUTATINGS DEFTALL HOUR OR MORE SHOULD BE

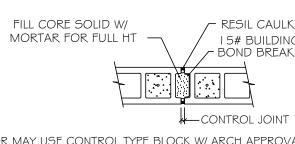
COURSE. DO NOT TOOTH. CLEAN EXPOSED SURFACES OF SET UNITS AND MORTAR PRIOR TO LAYING FRESH MASONRY.

TRUSS BEARING LOCATIONS - (TYP. UNO)



ISSUE

30' APART NOR 15' OFF ANY CORNER OR RETURN.



8'-5" LL TOP BARS ARE DEFINED AS HORIZ. BARS WITH MORE THAN 12" OF CONCRETE BELOW BARS

7'-7"

|'-|0" 2'-5" 3'-0" 3'-7" 5'-3" 6'-0" 6'-9"

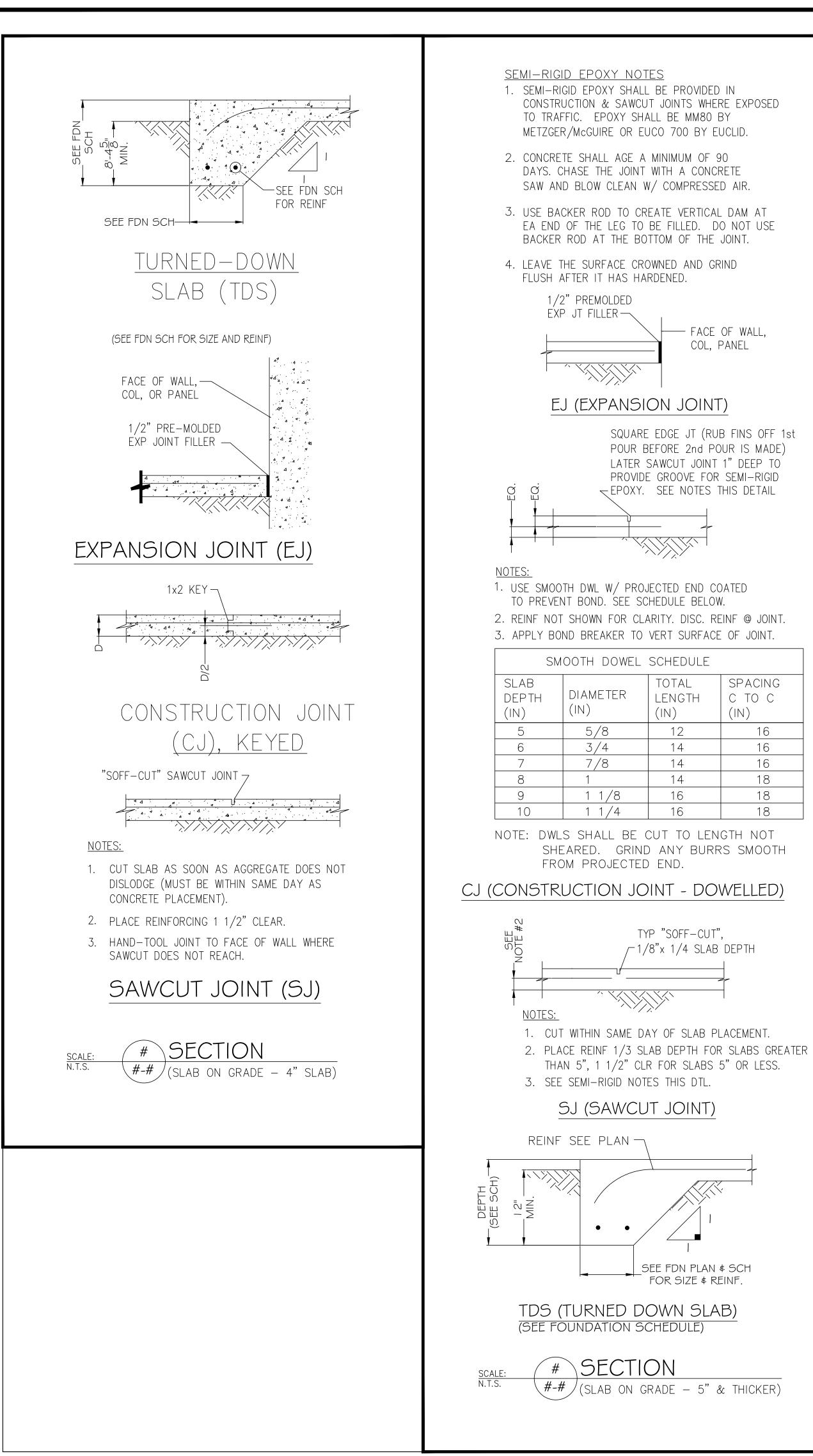
| |/2" 3/4"

| |/2"

INTERSECTION

LENGTHS

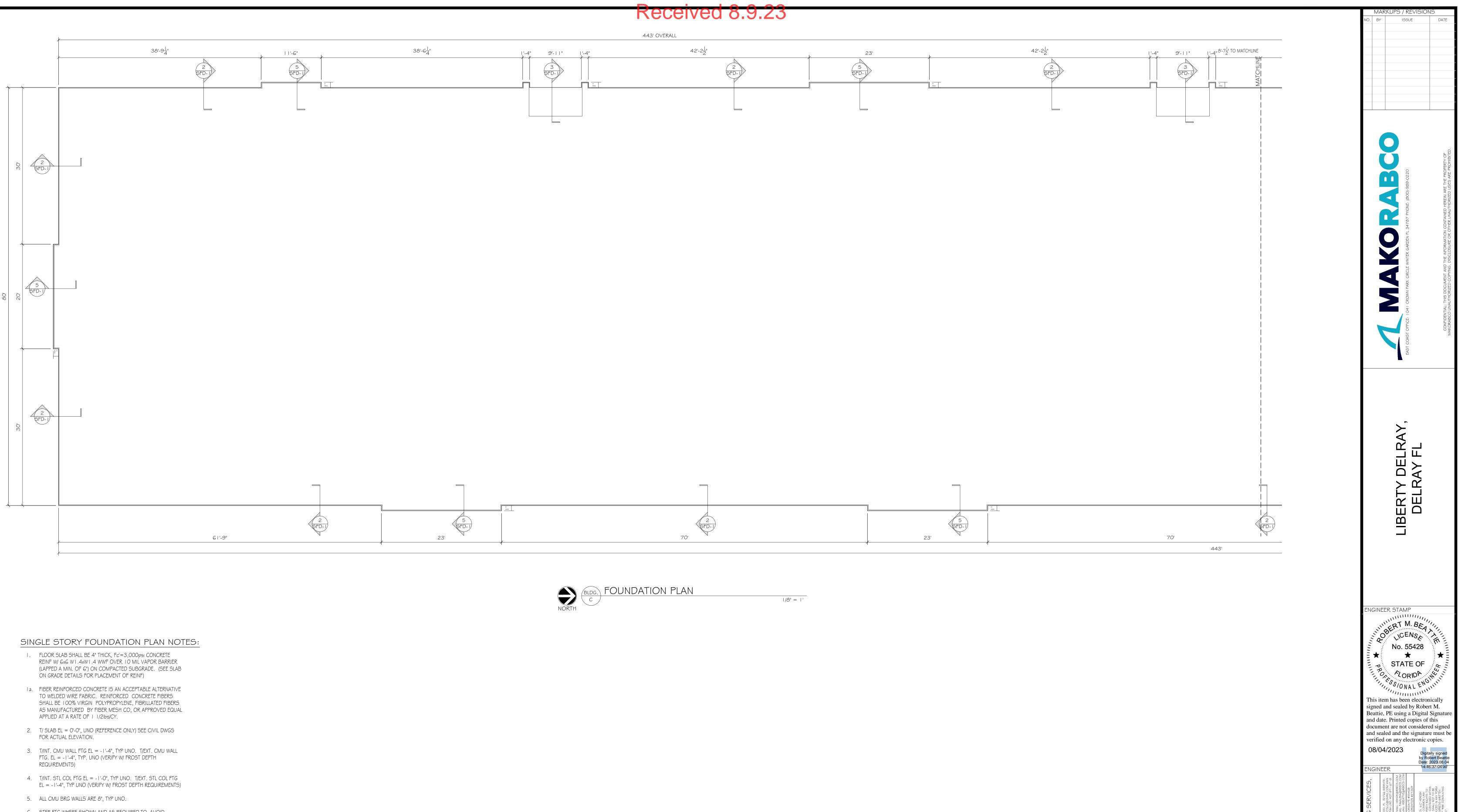
OTHERS



Received 8.9.23

SPACING С ТО С (IN)16 16 16 18 18 18

ISSUE	DATE
EAST COAST OFFICE: 104 I CROWN PARK CIRCLE WINTER GARDEN FL 34767 PHONE: (800)-969-0220	CONFIDENTIAL: THIS DOCUMENT AND THE INFORMATION CONTAINED HEREIN ARE THE PROPERTY OF MAKORABCO UNAUTHORIZED COPYING, DISCLOSURE OR OTHER UNAUTHORIZED USES ARE PROHIBITED.
LIBERTY DELRAY,	
★ STATE C S/ORIO S/ONAL S/ONA	28 → F NG NG NG NG NG NG NG NG NG NG
WWW.KEC2.JON - EMAIL: KELOLGGWMI.LOUN MAIN PHORE INIC: 4077-396-3760 - FAX INIC: 407-971-6716 MECHANICAL DEPT: 4077-36-3750 - FAXILI: NSHH@REEC5.COM ELCTRICAL DEPT: 407-617-3113 - EMAIL: NSHH@REEC5.COM STRUCTURAL DEPT: 407-628-7585 - EMAIL: NSHH@REEC5.COM RELCTRICAL DEPT: 407-617-3113 - EMAIL: RELATIE@REEC5.COM RELCTRICAL DEPT: 407-617-3113 - EMAIL: RELATIE@REEC5.COM FILORIDAL DEPT: 407-617-91 - S24-258 ROBERT M. BEATTIE, PE. FLORIDA FF #55428 FLORIDA CERTIFICATE OF AUTHORIZATION #27269	THE CONSULTING ERVICES LLC HEREBY REPECTIVE DERIVERSING ERVICES LLC HEREBY COPRESING RESERVESTING COMMON LAW COPRESING AND DHER REPORTERY REATES TO THESE ERVISIAND DRAWINGS SHALL NOT TO BE ALTERED. COPIED. OR RETREDUCED IN ANY FORM OR MANNER MITHOUT THE EXPRESS WATTEN FERNISSION AND CONSENT OF REEC CONSULTING FERNISSION AND CONSENT OF REEC CONSULTING SERVICES, LLC.
ITLE DUNDATION BY D BY	NOTES OG-12-23 AW/MNG XXX E 2705 AS NOTED



- 6. STEP FTG WHERE SHOWN AND AS REQUIRED TO AVOID INTERFERENCE W/ OTHER TRADES. SEE TYP STEPPED FTG DETAIL
- 7. ALL FTGS ARE CENTERED BENEATH BEARING WALLS AND COLUMNS, TYP UNO.
- 8. REINF CMU WALLS WITH #5 VERT BAR CENTERED IN GROUT FILLED CELL AT ENDS, CORNERS AND AT MAX SPACING OF 48"o.c. SEE "TYP FILLED CELL PARTIAL PLAN" FOR ADDITIONAL INFORMATION.
- 9. TYPICAL SPACING OF FILLED CELLS SHALL APPLY ABOVE AND BELOW OPENINGS ALSO. SEE GENERAL NOTES SHEETS FOR ADDITIONAL INFORMATION.
- IO. (2) #4 x 4'-0" LONG @ 3" C/C PLACED 2" CLEAR FROM CORNER, CENTERED IN SLAB, TYP.
- II. MAINTAIN STRUCTURAL SLAB THICKNESS AT ALL FLOOR SLOPES AND DEPRESSIONS.





NOTES:

- WITHOUT EXCEPTION, ALL RAINLIP, SHEETLEDGE AND BLOCKLEDGE SHALL BE 1-1/2" DOWN FROM THE T/FINISHED FLOOR ELEVATION. . WITHOUT EXCEPTION, ALL DOOR THRESHOLD SHALL BE $\frac{3}{4}$ " DOWN FROM THE T/FINISHED FLOOR ELEVATION.
- 3. G.C. SHALL VERIFY WIDTH OF RAINLIP, SHEETLEDGE, BLOCKLEDGE AND DOOR THRESHOLD WITH CONTRACT DOCUMENTS PRIOR TO FORMING SLAB EDGE.
- 4. IN THE ABSENCE OF RAINLIP, SHEETLEDGE, BLOCKLEDGE OR DOOR THRESHOLD BEING SPECIFIED ON THE PLANS, THE G.C. SHALL VERIFY IN WRITING WITH THE STEEL CONTRACTOR THE INTENT PRIOR TO FORMING
- SLAB EDGE. RAINLIP, SHEETLEDGE, BLOCKLEDGE AND DOOR THRESHOLD ARE VITAL TO THE PROPER FIT OF THE STEEL
- CONSTRUCTION. G.C. SHALL FIELD VERIFY THE RAINLIP, SHEETLEDGE, AND BLOCKLEDGE HAVE BEEN CONSTRUCTED IN
- ACCORDANCE WITH CONTRACT DOCUMENTS BEFORE STEEL CONSTRUCTION BEGINS.

NOTE:

GC SHALL COORDINATE ALL T/CMU WALL HEIGHTS WITH RABCO ENTERPRISES, LLC PRIOR TO CONSTRUCTION TO ENSURE PROPER FIT OF THE METAL BUILDING COMPONENTS. FAILURE TO GET WRITTEN APPROVAL PRIOR TO ERECTING THE CMU WALLS MAY RESULT IN NEEDED MODIFICATIONS TO EITHER THE CMU WALL AND/OR THE METAL BUILDING COMPONENTS AT THE G.C.'S EXPENSE.

SF-1.0

BLDG. C

FOUNDATION PLANS

06-12-23

AW/MNG

E 2705

AS NOTED

XX

SHEET TITLE

DATE

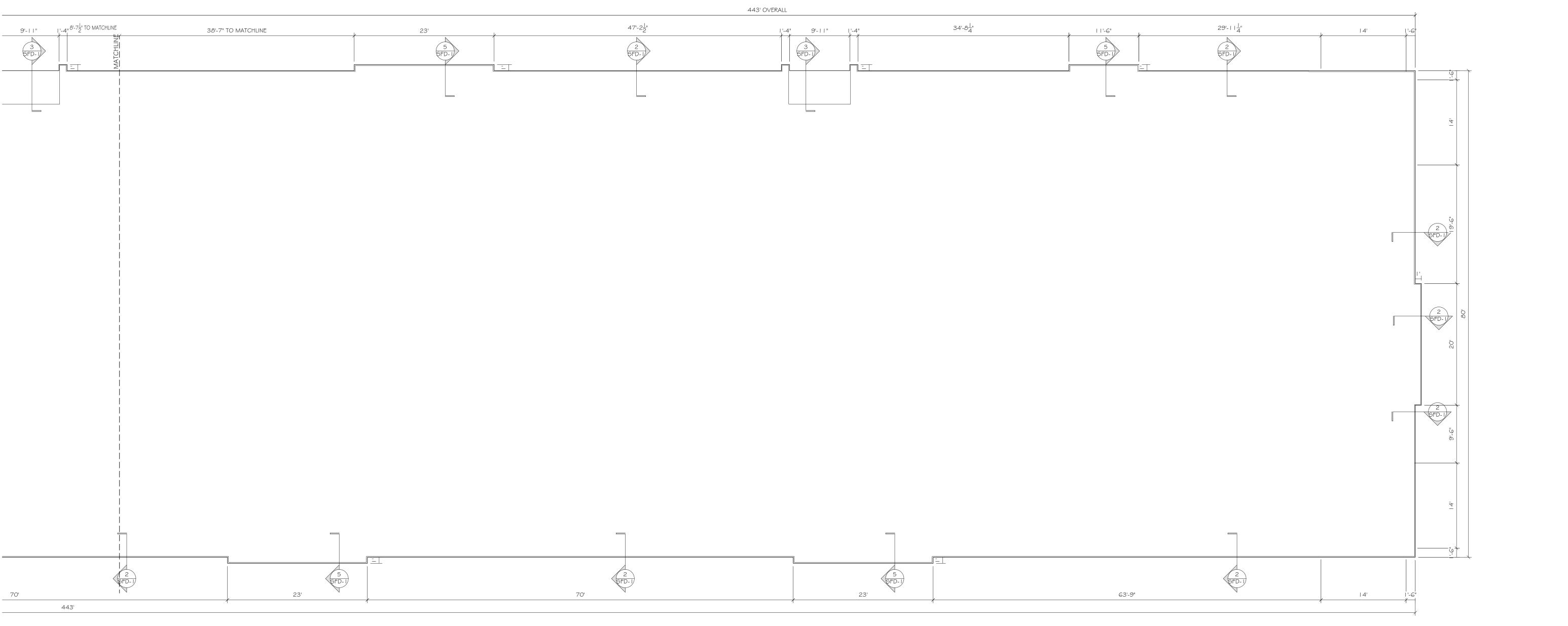
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JOB NO.

SCALE

SHEET

CHECKED BY



SINGLE STORY FOUNDATION PLAN NOTES:

- FLOOR SLAB SHALL BE 4" THICK, Fc'=3,000psi CONCRETE REINF W/ GxG W1.4xW1.4 WWF OVER 10 MIL VAPOR BARRIER (LAPPED A MIN. OF G") ON COMPACTED SUBGRADE. (SEE SLAB ON GRADE DETAILS FOR PLACEMENT OF REINF)
- I.a. FIBER REINFORCED CONCRETE IS AN ACCEPTABLE ALTERNATIVE TO WELDED WIRE FABRIC. REINFORCED CONCRETE FIBERS SHALL BE 100% VIRGIN POLYPROPYLENE, FIBRILLATED FIBERS AS MANUFACTURED BY FIBER MESH CO, OR APPROVED EQUAL APPLIED AT A RATE OF 1 1/21b5/CY.
- T/ SLAB EL = O'-O", UNO (REFERENCE ONLY) SEE CIVIL DWGS FOR ACTUAL ELEVATION.
- T/INT. CMU WALL FTG EL = -1'-4", TYP UNO. T/EXT. CMU WALL FTG. EL = -1'-4", TYP. UNO (VERIFY W/ FROST DEPTH REQUIREMENTS)
- 4. T/INT. STL COL FTG EL = I '-0", TYP UNO. T/EXT. STL COL FTG EL = I '-4", TYP UNO (VERIFY W/ FROST DEPTH REQUIREMENTS)
- 5. ALL CMU BRG WALLS ARE 8", TYP UNO.
- 6. STEP FTG WHERE SHOWN AND AS REQUIRED TO AVOID INTERFERENCE W/ OTHER TRADES. SEE TYP STEPPED FTG DETAIL
- 7. ALL FTGS ARE CENTERED BENEATH BEARING WALLS AND COLUMNS, TYP UNO.
- REINF CMU WALLS WITH #5 VERT BAR CENTERED IN GROUT FILLED CELL AT ENDS, CORNERS AND AT MAX SPACING OF 48"o.c. SEE "TYP FILLED CELL PARTIAL PLAN" FOR ADDITIONAL INFORMATION.
- TYPICAL SPACING OF FILLED CELLS SHALL APPLY ABOVE AND BELOW OPENINGS ALSO. SEE GENERAL NOTES SHEETS FOR ADDITIONAL INFORMATION.
- I O. (2) #4 x 4'-0" LONG @ 3" C/C PLACED 2" CLEAR FROM CORNER, CENTERED IN SLAB, TYP.
- 11. MAINTAIN STRUCTURAL SLAB THICKNESS AT ALL FLOOR SLOPES AND DEPRESSIONS.





NOTES:

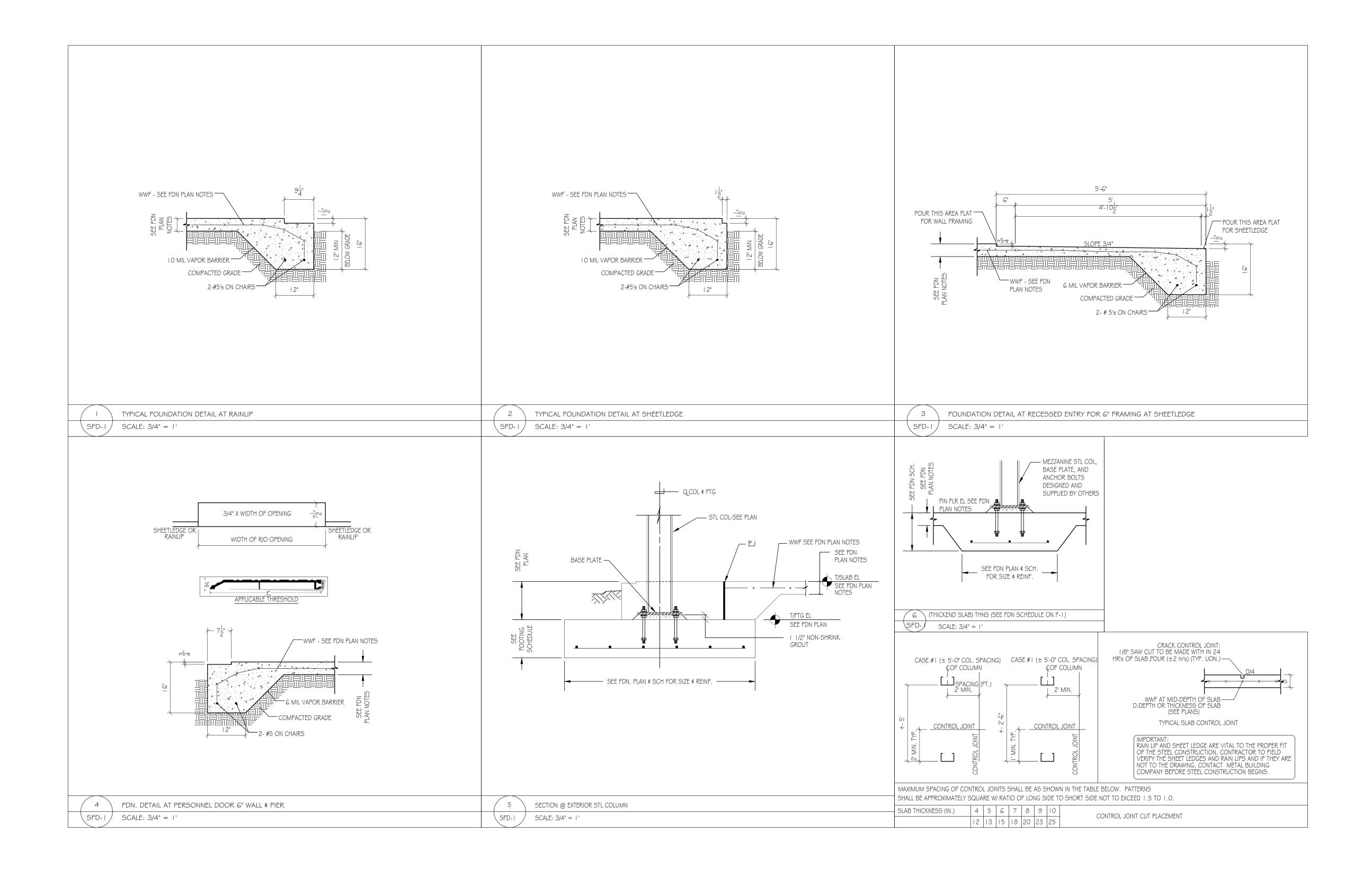
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NOTE:

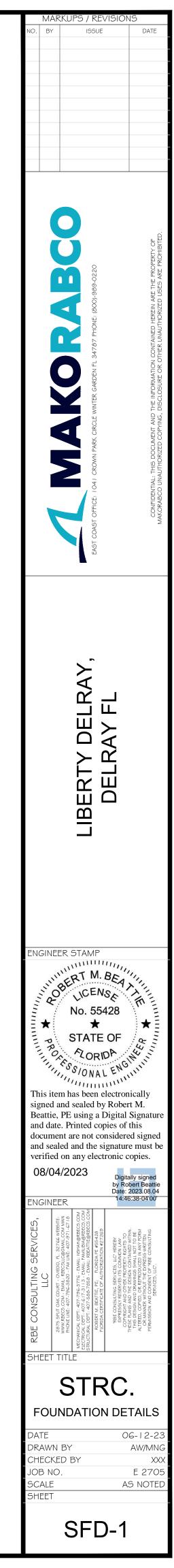
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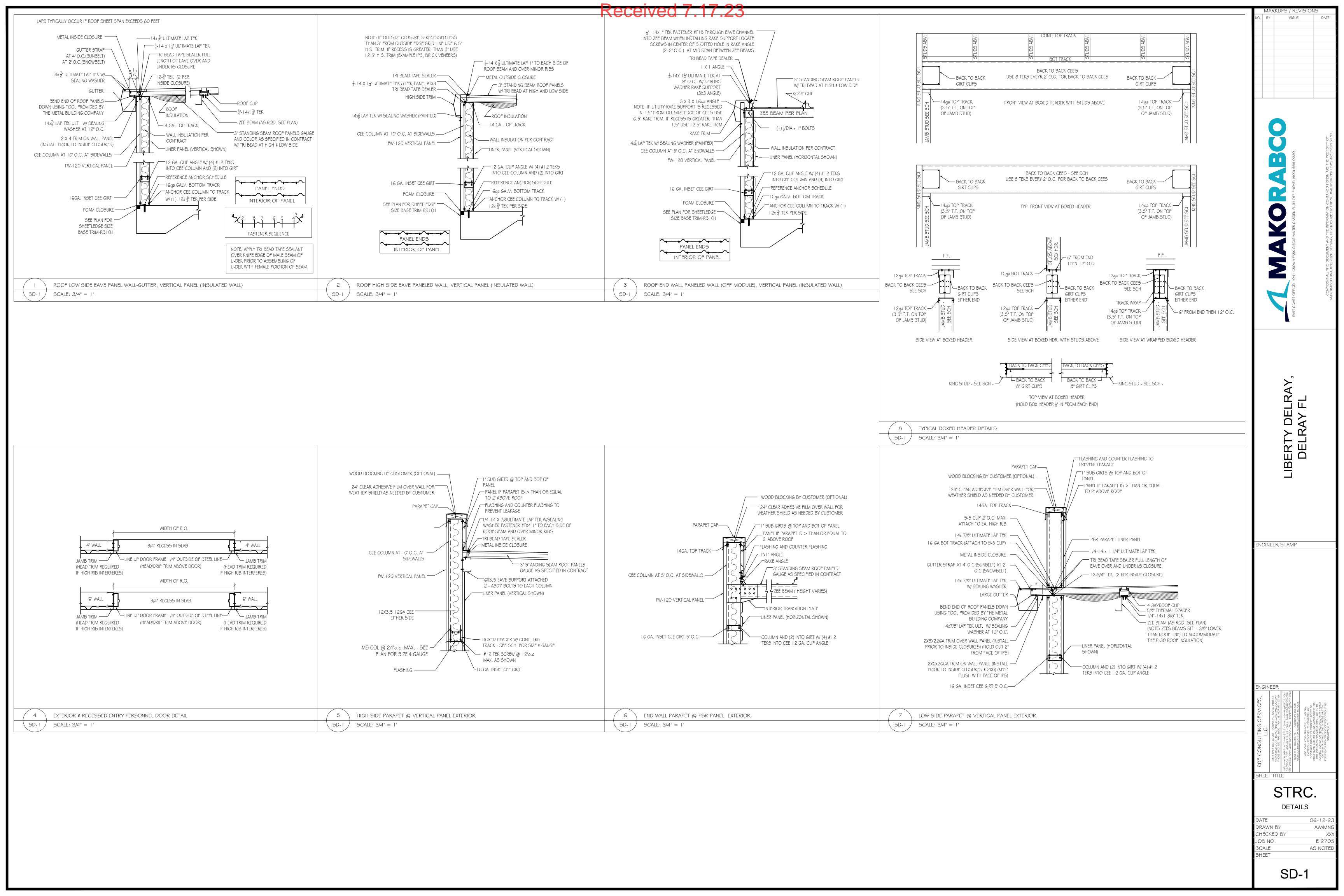
			EAST COAST OFFICE: 1041 CROWN PARK CIRCLE WINTER GARDEN FL 34787 PHONE: (800)-989-0220			CONFIDENTIAL: THIS DOCUMENT AND THE INFORMATION CONTAINED HEREIN ARE THE PROPERTY OF MAKORABCO UNAUTHORIZED COPYING, DISCLOSURE OR OTHER UNAUTHORIZED USES ARE PROHIBITED.
ENGIN	EERS	LIBERTY DELRAY.				
This i signed beatting and beatting and second seco	tem h l and e, PE ate. Pinent a caled a cd on 04/20	No. 4 STA 20 8/0 N as bee sealed using inted re not and th any e	M. L ENS 5554 ★ TE NAL Cop t con t con t con	28 OF ENG ENG ectron Rober igital ies of sidere gnature onic c	t M. Signa this ed sig e mus	ture ned st be
LE RBE CONSULTING SERVICES,	2875 SFUT OAK COURT - OVIEDO, FL. 32766 WEBGITE: WWW.RBECS.COM - EMAIL: REECSLC@GMALL.COM MAIN PHONE LINE: 407-791-6716	M MECHANICAL DEPT: 407-796-3776 - EMAIL: NSHAH@REECS.COM ELECTRICAL DEPT: 407-517-3113 - EMAIL: FAMILBIA@REECS.COM STRUCTURAL DEPT: 407-568-7569 - EMAIL: REFATTE@REECS.COM	ROBERT M. BEATTIE, PE; FLORIDA PE #55428 FLORIDA CERTIFICATE OF AUTHORIZATION #27269	*RBE CONSULTING SERVICES, LLC* HEREBY EXPRESSIV RESERVED ITS COMMON LAW COPRIGHT TAUD OTHER REPORTEX TRADIETS TO THEEE PUNAL AND THE PRECON CONTINUED VITATION	THIS DESIGN AND FILL PLOAD CONTINUED WITTING. THIS DESIGN AND REPRODUCED IN ANY FORM ALTERED, COPIED, OR REPRODUCED IN ANY FORM OR MANNER WITHOUT THE EXPRESS WRITTEN	PERMISSION AND CONSINT OF YEBE CONSULTING SERVICES, LLC'.
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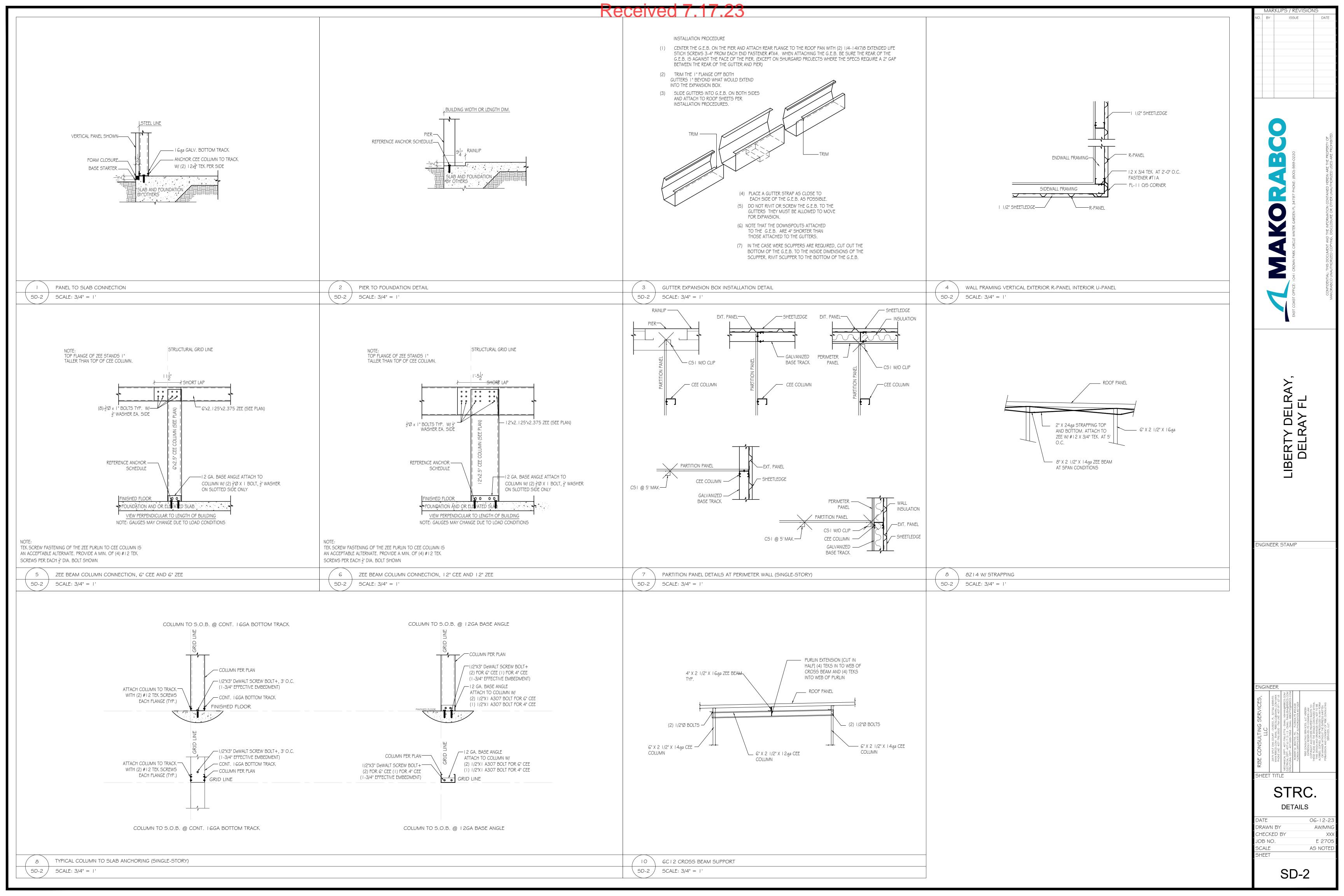
SF-1.1

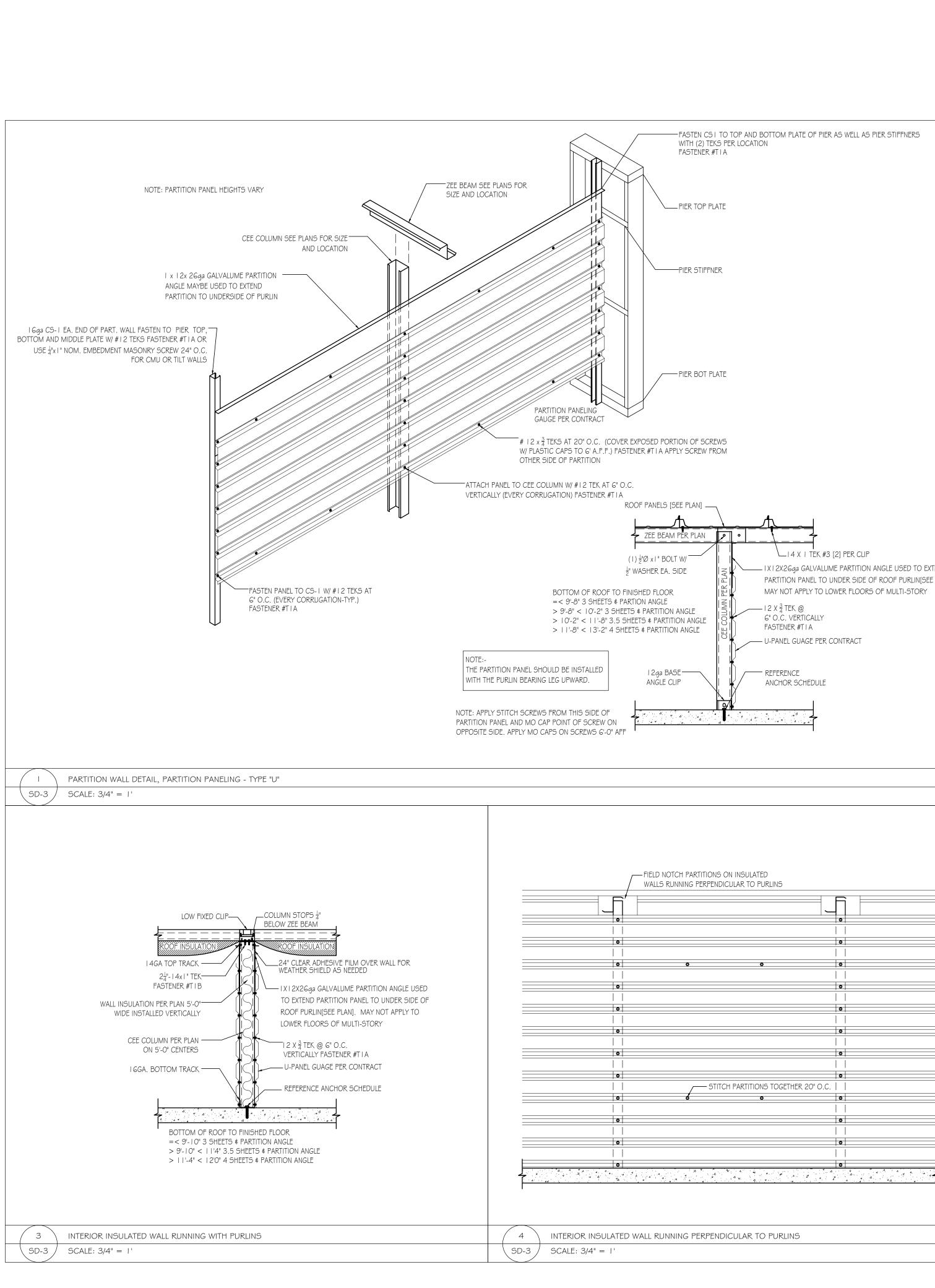


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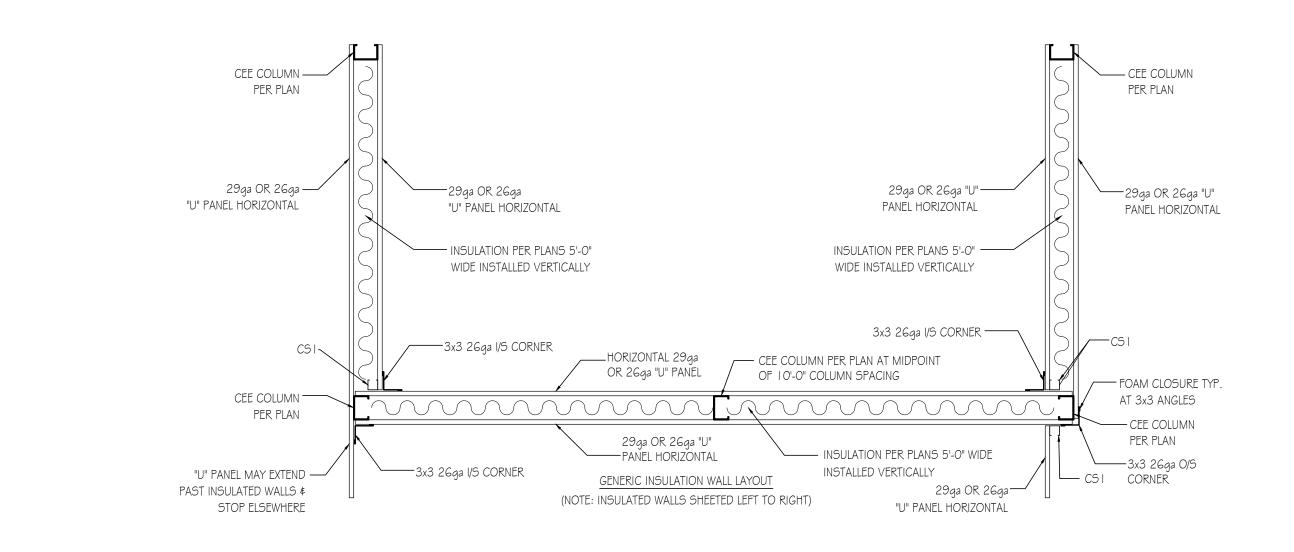
Received 7.17.23

- IXI 2X26ga GALVALUME PARTITION ANGLE USED TO EXTEND PARTITION PANEL TO UNDER SIDE OF ROOF PURLIN[SEE PLAN].

PIFR -MULLION-∽CSI W/O CLIP ∽CSI W/O CLIP CSI W/O CLIP - CSI W/O CLIP CEE CEE COLUMN CEE COLUMN COLUMN CSI W/ CLIP CS-15 THIS SIDE OF COLUMN SAME HEIGHT OF CEE COLUMN CSI W/ CLIP-CSI W/O CLIP-CSI W/ CLIP - CSI W/O CLIP U-PANEL _ CEE CEE COLUMN U-PANEL CEE COLUMN COLUMN CSI W/ CLIP NOTE: CS-1'S TO BE THE SAME HEIGHT AS COLUMNS IN THE SAME ROW - CSI W/ CLIP CEE COLUMN CEE COLUMN See CEE COLUMN _CSIW/O CSI W/O CLIP CSI W/O CLIP CSI W/O CLIP CLIP MULLION -PIER-PIER — PIER — 10' 10'

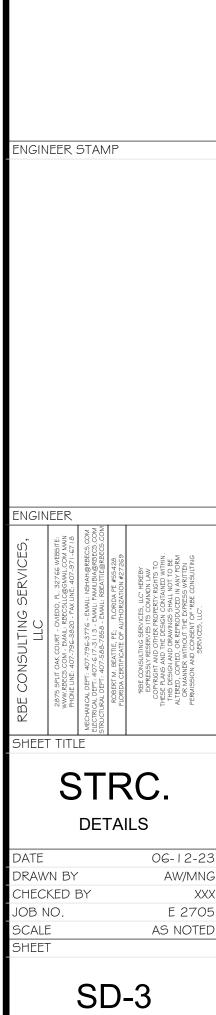
´2 `

GENERIC FRAMING FOR PARTITIONS WITH CROSS PARTITIONS (N.T.S.) $\$ SD-3 / SCALE: 3/4" = 1'



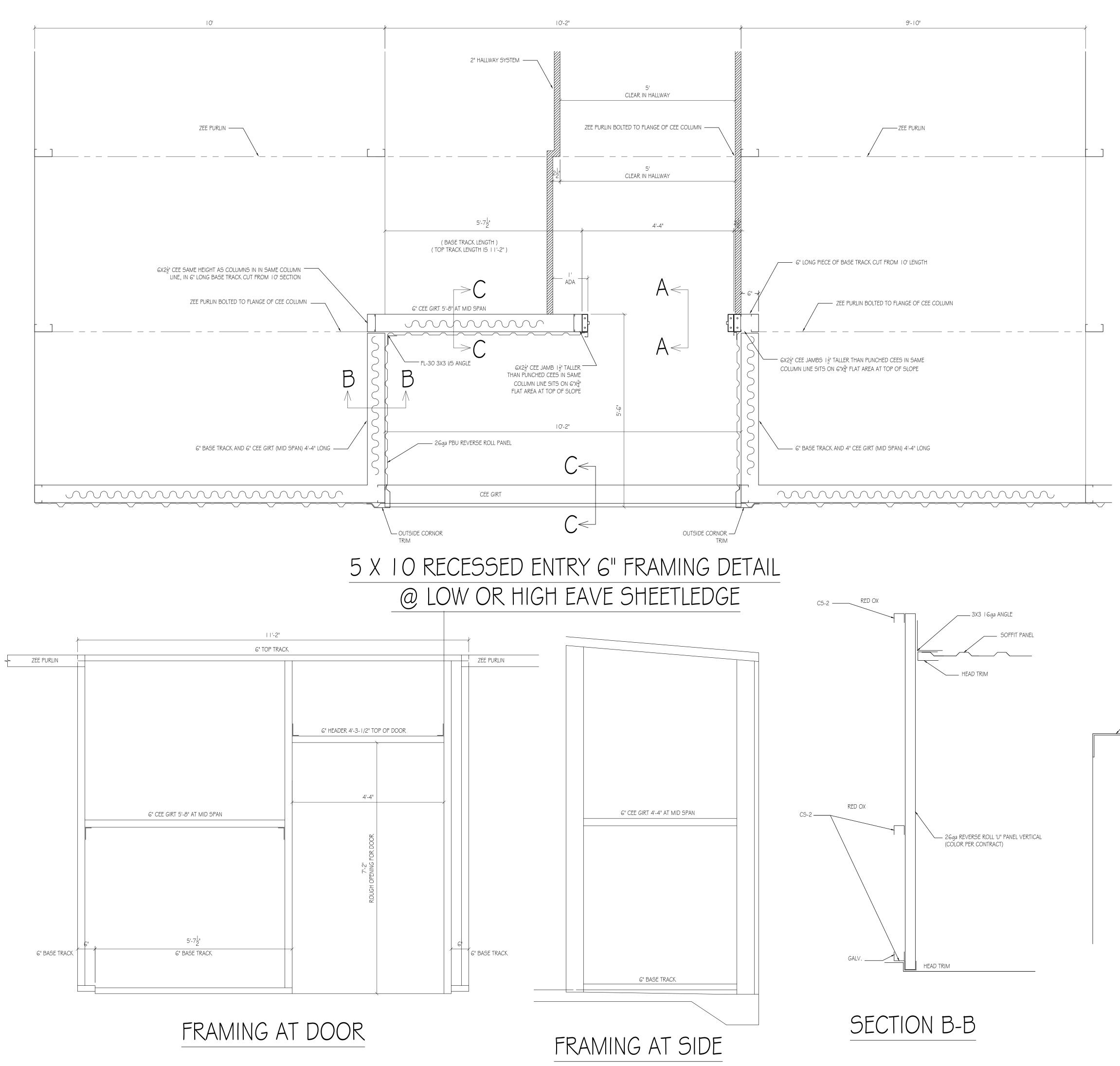
5

INSULATED WALL FRAMING

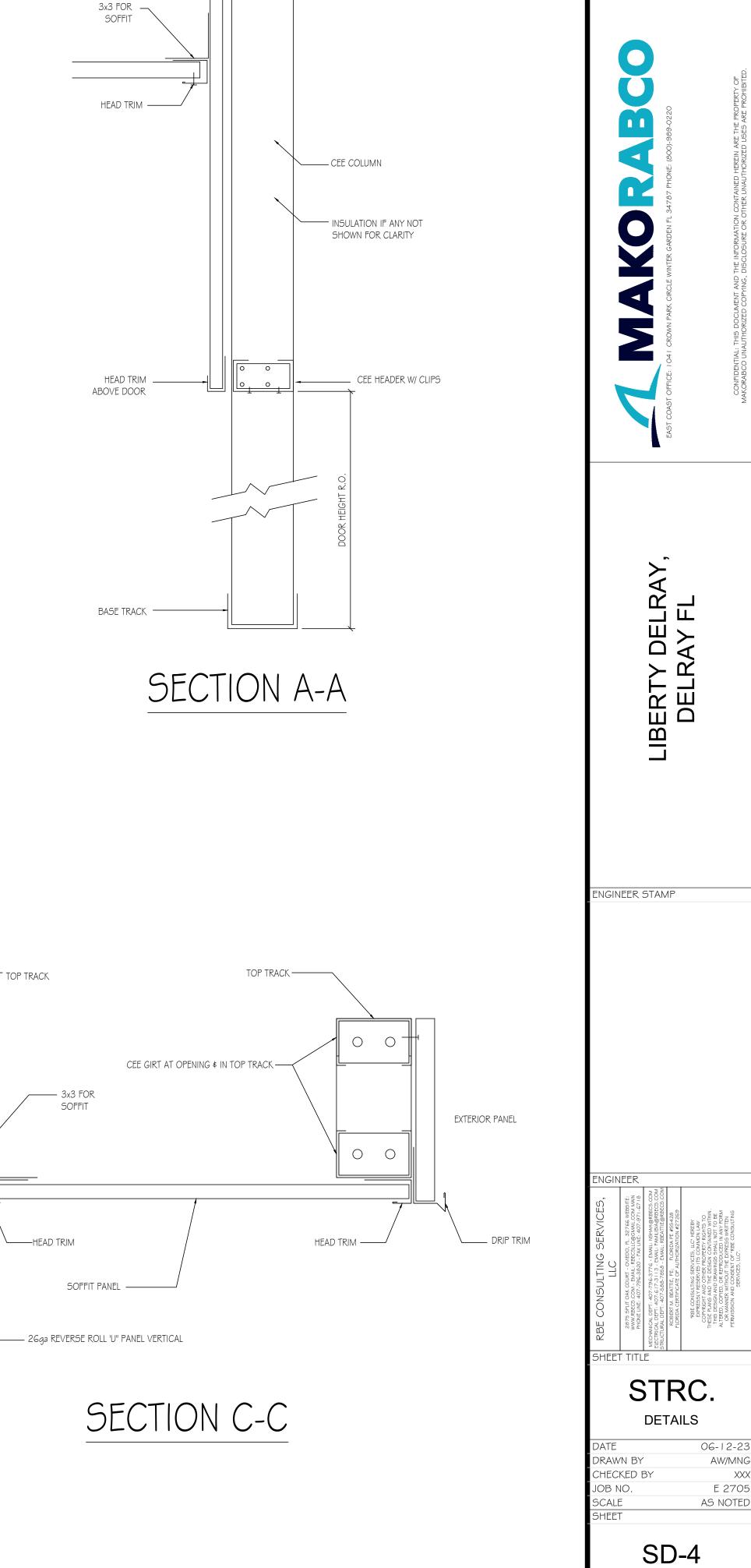




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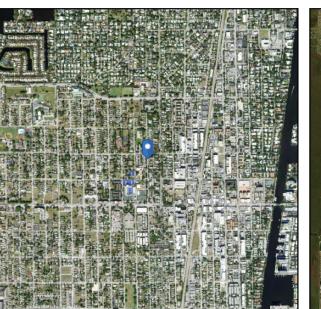
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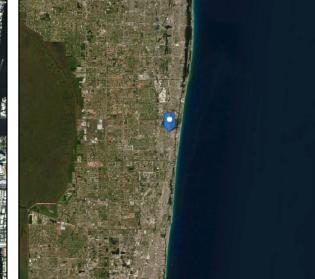


ASCE 7 Hazards Report

Standard: ASCE/SEI 7-16 Latitude: 26.4649 Risk Category: II Section 11.4.3)

Longitude: -80.07419 Soil Class: D - Default (see Elevation: 19.6092138164712 ft (NAVD 88)





Wind

Results: Wind Speed 10-year MRI 25-year MRI

50-year MRI

100-year MRI

Data Source:

Date Accessed:

168 Vmph 90 Vmph 112 Vmph 127 Vmph 138 Vmph

ASCE/SEI 7-16, Fig. 26.5-1B and Figs. CC.2-1–CC.2-4, and Section 26.5.2 Tue Jun 27 2023

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-16 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

Site is in a hurricane-prone region as defined in ASCE/SEI 7-16 Section 26.2. Glazed openings shall be protected against wind-borne debris as specified in Section 26.12.3.

https://asce7hazardtool.online/

Page 1 of 6

Tue Jun 27 2023



Results:

15-minute Precipitation Intensity: 9.58 in./h

60-minute Precipitation Intensity: 5.23 in./h

Data Source:

https://asce7hazardtool.online/

Date Accessed:

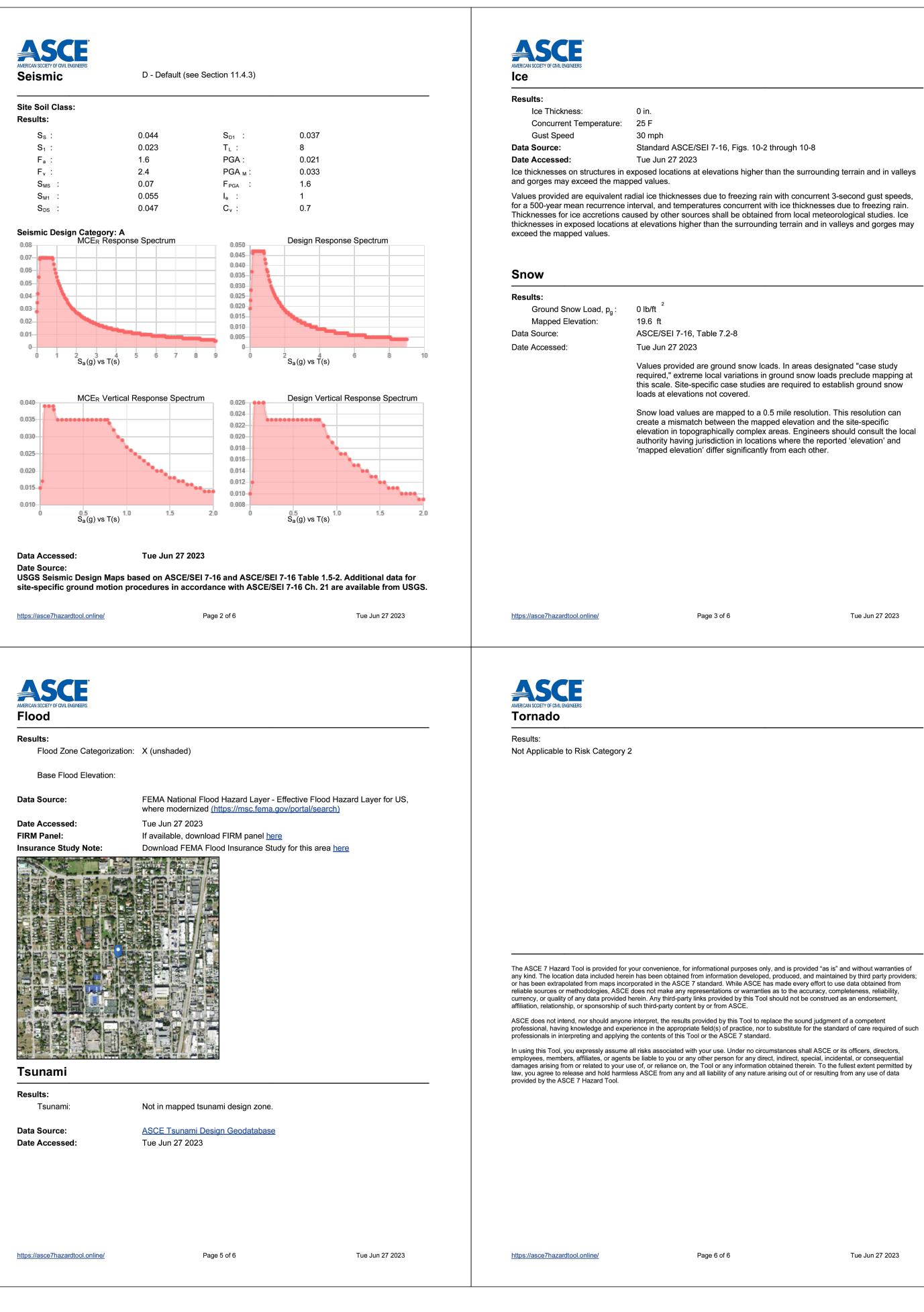
(https://www.nws.noaa.gov/oh/hdsc/) Tue Jun 27 2023

NOAA National Weather Service, Precipitation Frequency Data Server, Atlas 14

Page 4 of 6

Tue Jun 27 2023

Received 8.9.23



Values provided are ground snow loads. In areas designated "case study required," extreme local variations in ground snow loads preclude mapping at

Snow load values are mapped to a 0.5 mile resolution. This resolution can create a mismatch between the mapped elevation and the site-specific elevation in topographically complex areas. Engineers should consult the local authority having jurisdiction in locations where the reported 'elevation' and

Tue Jun 27 2023

Tue Jun 27 2023

0 DELRAY AY FL ERTY | DELR/ m NGINEER STAMP BERT M. BEY No. 55428 * * STATE OF TON YORIDA This item has been electronically signed and sealed by Robert M. Beattie, PE using a Digital Signature and date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies. 08/04/2023 Digitally signed by Robert Beattie Date: 2023.08.04 14:46:27-04'00 NGINEER SHEET TITLE CODE ANALYSIS 06-12-23 DATE DRAWN BY AW/MNG CHECKED BY X JOB NO. E 2705 SCALE AS NOTED **SHEET**

S-0-0

STRUCTURAL DESIGN CRITERIA	WIND LOADS - ENCLOSED BUILDINGS METHOD 2 ANALYTICAL PROCEDURE	PROJECT STRUCTURE(S) TOTAL SF = 89,589.00				PROJECT BUILDIN TOTAL SQUARE FOOTAGE
DESIGN PARAMETERS SOURCE:	DESIGN WIND PRESSURES ON COMPONENT & CLADDING ELEMENTS OF BUILDINGS WITH H \leq 60 FT.					TOTAL SQUAKE FOUTAGE
NOTE: CHANGES IN THE DESIGN PARAMETERS COULD AFFECT STRUCTURE DESIGN AND / OR BUILDING COMPONENTS)	ASSUMES NO TOPOGRAPHIC FACTORS	NO: C	D	E	F	
CODES AND REFERENCES ILL WORK SHALL BE DONE IN ACCORDANCE WITH AT LEAST THE MINIMUM REQUIREMENTS OF THE APPLICABLE	DIRECTION FACTOR K _d IS APPLIED (0.85)	DESCRIPTION				
LL WORK SHALL BE DONE IN ACCORDANCE WITH AT LEAST THE MINIMUM REQUIREMENTS OF THE APPLICABLE UILDING CODE. THE PLANS AND SPECIFICATIONS HAVE AS WELL BEENDEVELOPED IN ACCORDANCE THERE OF.	GUST FACTOR EQUAL TO 0.85	ROOF PITCH RISE - INCHES / FEET: 1/4 ROOF PITCH RISE - DEGREES: 1.19	1/4	1/4	1/4	USE AND OCCUPANCY
STATE JURISDICTION: FLORIDA	STRUCTURES HAVE BEEN DESIGN WITHOUT OVERHANG, CONTRACTOR / OWNER ARE REQUIRED TO ENSURE ALL OVERHANGS ARE DESIGNED / BUILT TO RESISTING WIND PRESSURES.	EAVE HEIGHT - FEET ONLY: 10	10	24	24	
PROJECT ZIP CODE: 33444	ALL OVERHANGS ARE DESIGNED / BUILT TO RESISTING WIND PRESSURES.	EAVE HEIGHT - INCHES ONLY: 0	0	6	6	
BUILDING CODE. 2018 INTERNATIONAL BUILDING CODE	DESCRIPTION: BLDG - WL	DISTANCE TO RIDGE OR HIGHSIDE - FEET: 80.00	80.00	40.00	40.00	SQUARE FOO
LOCAL CODE: 2020 FLORIDA BUILDING CODE 7th EDITION	RISK CATEGORY: II	MEAN ROOF HEIGHT: 10' - 0"	10' - 0"	24' - 6"	24' - 6"	
REFERENCE STANDARD: ASCE 7-16	NATURE OF OCCUPANCY (HAZ. TO HUMAN LIFE): OTHER		00.00	40.00	10.00	
LOAD COMBINATIONS? PER ASCE 7 DESIGN MANUALS (LATEST ADOPTED EDITIONS): AISC - MANUAL OF STEEL CONSTRUCTION	IMPORTANCE FACTOR: 1	LEAST HORIZONTAL DIMENSION - FEET: 80.00 EDGE CONDITION - a - FEET: 4.00	80.00 4.00	40.00 4.00	40.00	
ALLOWABLE STRESS DESIGN (ASD)	HEIGHT ABOVE GROUND LEVEL, h - FEET: 25 MAXIMUM	EDGE CONDITION - 0.2h - FEET: 2.00	2.00	4.90	4.90	SQUARE FOC
AISI - NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF	STRUCTURE ENCLOSURE CLASSIFICATION: ENCLOSED	EDGE CONDITION - 0.6h - FEET: 6.00	6.00	14.70	14.70	
COLD FORMED STEEL STRUCTURAL MEMBERS	WIND ROOF TYPE: GABLE ROOFS <= 7 DEGREES & MONOSLOPE ROOFS <= 3 DEGREES ROOF PITCH: <= 10 DEGREES					TYPES OF CONSTRUCT
ACI - AMERICAN CONCRETE INSTITUTE BUILDING CODE REQ. FOR STRUCTURAL CONCRETE	ROOT FILCH. <= 10 DEGREES	WIDTH - FEET: 80.00	80.00	40.00	40.00	
BUILDING CODE REQ. FOR STRUCTURAL CONCRETE BUILDING CODE REQ. FOR MASONRY STRUCTURES	CATEGORY: II	LENGTH - FEET: 443.00	450.00	215.00	235.50	
/IND LOAD DESIGN PARAMETERS	z_{g} (feet): 900	ADJUSTMENT - SF: 0.00	0.00	43.00	86.00	FIRE RESISTANCE RATING
ULTIMATE DESIGN WIND SPEEDS - Vult - MPH: 170	ALPHA: 9.5	STORIES: 1 SQUARE FOOTAGE - SF: 35,440.00	36,000.00	1 8.643.00	9,506.00	
NOMINAL DESIGN WIND SPEED - Vasd - MPH. 135	VELOCITY PRESSURE EXP. COEFFICIENTS, Kh = $2.01(h/z_g)^{(2/ALPHA)} = 0.95$	SQUARE FOUTAGE - SF. 33,440.00	30,000.00	0,043.00	9,506.00	
RISK CATEGORY: II	INTERNAL PRESSURE COEFFICIENTS, GCpi: 0.18 -0.18	LOADING				
EXPOSURE OF ROOF. C	$q_h = 0.00256K_hK_{zl}K_dV^2I$ (lb/ft ²) = 37.49	ROOF DEAD LOAD - PSF: 3.00	3.00	3.00	3.00	EXTERIOR NON-BEAR
PROCEDURE: ANALYTICAL - METHOD 2		COLLATERAL LOAD - PSF 4.00	4.00	4.00	4.00	INTERIOR NON-BEAR
METHOD: LOW-RISE BUILDING & BUILDINGS - H \leq 60 FT.	COMPONENT AND CLADDING PRESSURES - p = q _b [(GCp)-(GCpi)] (lb/ft ²)	SPECIFIED COLLATERAL LOADING IS PER INFORMATION BY OTHERS AND IN ABSENCE OF PER	4.00	4.00	4.00	
WIND BORNE DEBRIS REGION: YES GLAZING IN LOWER 60 FEET IN BUILDING: UNPROTECTED	EFFECTIVE AREAS (SF)	METAL BUILDING INDUSTRY STANDARDS - CLIENT IS RESPONSIBLE TO VERIFY AND APPROVE.				
SNOW LOAD DESIGN PARAMETERS	<u>ROOF PRESSURES</u> 10 20 50 100 200 500	TYP. COLLATERAL LOADS IS FOR CEILING, LT. DUCTWORK, BRANCH SPRINKLERS AND				FIRE-RESISTANCE RATING
GROUND SNOW LOAD - Pg - PSF. 0	NE 1, 2 & 3 (POSITIVE) WITH & WITHOUT OVERHANG 17.99 16.87 15.37 14.25 14.25 14.25					
EXPOSURE OF ROOF. PARTIALLY EXPOSED	ZONE 1 (NEGATIVE) WITHOUT OVERHANG -44.23 -43.11 -41.61 -40.49 -40.49 -40.49					
TERRAIN CATEGORY. B	ZONE 2 (NEGATIVE) WITHOUT OVERHANG -74.22 -66.32 -55.88 -47.98 -47.98 -47.98					
IMPORTANCE FACTOR - Is: 1	ZONE 3 (NEGATIVE) WITHOUT OVERHANG -111.71 -92.53 -67.17 -47.98 -47.98 -47.98					
SNOW EXPOSURE FACTOR - Ce: 1	ZONE 1 (NEGATIVE) WITH OVERHANG -70.48 -69.35 -67.86 -66.73 -58.66 -47.99		2.00	2.00	2.00	FIRE PROTECTION SYS
GAMMA: 14 <u>HEATED: 0.7 x Ce x Ct x ls x Pg</u>	ZONE 2 (NEGATIVE) WITH OVERHANG -70.48 -69.35 -67.86 -66.73 -58.66 -47.99		6.00	6.00	6.00	SPRIN SPRINKLER PIPES
THERMAL FACTOR - Ct: 1	ZONE 3 (NEGATIVE) WITH OVERHANG -111.71 -89.14 -59.31 -36.74 -36.74 -36.74	4 SOLAR: 10.00	10.00	10.00	10.00	SUPPORTED FROM
FLAT ROOF SNOW LOAD - Pf - PSF: 0		OFFICE: 7.00	7.00	7.00	7.00	OUL OKIED TROW
- HEIGHT - FEET: 0		RESIDENTIAL: 10.00	10.00	10.00	10.00	AN AUTOMATIC SPRINKLE
NON-HEATED: 0.7 x Ce x Ct x ls x Pg	WALL PRESSURES 10 20 50 100 200 500	1				CONTAINING A GROUP S-1
THERMAL FACTOR - Ct: 1.2 FLAT ROOF SNOW LOAD - Pf - PSF: 0	ZONE 4 & 5 (POSITIVE): 40.49 38.69 36.32 34.53 34.53 34.53	MIN. UNIFORMLY DISTRIBUTED LIVE LOADS - PSF	(EXIST:
- HEIGHT - FEET: 0	ZONE 4 (NEGATIVE): -43.86 -42.07 -39.70 -37.90 -37.90 -37.90	LIGHT STORAGE: 125.00	125.00	125.00	125.00	1. FIRE AREA EXCEEDS
FROST LINE DEPTH - FEET: 0	ZONE 5 (NEGATIVE): -53.98 -50.39 -45.65 -42.06 -42.06 -42.00		100.00	100.00	100.00	2. FIRE AREA IS LOCATE
SOURCE: ASSUMED			40.00	40.00	40.00	3. COMBINED FIRE ARE
SEISMIC DESIGN PARAMETERS		STAIRS AND EXIT WAYS: 100.00 1ST FLOOR CORRIDORS: 100.00	100.00	100.00	100.00	
ANALYSIS PROCEDURE: SIMPLIFIED	MAIN WIND FORCE RESISTING SYSTEM - $p = q_h GC_p - q_h (GCpi)$ (lb/ft ²)	*CORRIDORS OTHER FLOORS = OCCUPANCY	OCCUPANCY	OCCUPANCY	OCCUPANCY	POTABLE FIRE EXTINGUIS
SEISMIC USE GROUP / OCCUPANCY CATEGORY: I OR II	ROOF PRESSURES h/L = MEAN ROOF HT. / HOR. DIM.	ROOFS - ORDINARY FLAT OR PITCHED: 20.00	20.00	20.00	20.00	EXTINGUSHER AND SHALL
SITE CLASS. D SOURCE: ASSUMED	HOR. DIST. FROM WINDWARD EDGE - FEET 0.25 0.5 1 0 TO h/2: -35.43 -35.43 -48.17	REDUCED IN ROOF LIVE LOAD			[EXTINGUISHER NOT TO EX
SPECTRA RESPONSE COEFFICIENT - Ss. 0.04	h/2 TO h: -35.43 -35.43 -29.05	ROOF L.L. X [1.2 - 0.05 X PITCH(MIN 4, MAX 12)]: 20.00	20.00	20.00	20.00	
SPECTRA RESPONSE COEFFICIENT - S1: 0.02	h TO 2h: -22.68 -22.68 -29.05					GENERAL BUILDING H
SITE COEFFICIENT - Fa: 1.60	> 2h: -16.31 -16.31 -22.68					
SITE COEFFICIENTS - Fv: 2.40	WALL PRESSURES: ROOF PITCH <= 10 DEGREES	METAL DECK AND CONCRETE:				
SPECTRAL RESPONSE COEFFICIENT - Sds: 0.05	WINDWARD: 32.24	GOVERNING LATERAL FORCE/BASE SHEAR				
SPECTRAL RESPONSE COEFFICIENT - Sd1: 0.04	LEEWARD: -22.68	EFFECTIVE WEIGHT - KIPS: 425.28	432.00	103.72	114.07	
SEISMIC DESIGN CATEGORY: A	SIDE WALL: _29.05	ADJUSTMENTS - KIPS: 0.00	0.00	0.00	0.00	ALLOWABI
RAIN DESIGN PARAMETERS						
RAINFALL RATE - INCHES / HOUR: 5.23		TOTAL FEFECTIVE WEIGHT - KIPS 425 28	432 00	103 72	114 07	
		TOTAL EFFECTIVE WEIGHT - KIPS: 425.28 LONGITUDINAL: WIND	432.00 WIND	103.72 WIND	114.07 WIND	ΔR
SOURCE: PER MAP		TOTAL EFFECTIVE WEIGHT - KIPS: 425.28 LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8				AR
SOURCE: PER MAP		LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2				
SOURCE: PER MAP		LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69	WIND 8 6 1/2 3.75	WIND 8 6 1/2 0.90	WIND 8 6 1/2 0.99	BUILDING P
SOURCE: PER MAP		LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67	WIND 8 6 1/2 3.75 866.67	WIND 8 6 1/2 0.90 996.67	WIND 8 6 1/2 0.99 996.67	BUILDING P PERIMETER
SOURCE: PER MAP		LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67 ADJUSTMENTS - SF: 0.00	WIND 8 6 1/2 3.75 866.67 0.00	WIND 8 6 1/2 0.90 996.67 0.00	WIND 8 6 1/2 0.99 996.67 0.00	BUILDING P PERIMETER
SOURCE: PER MAP		LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 866.67	WIND 8 6 1/2 3.75 866.67 0.00 866.67	WIND 8 6 1/2 0.90 996.67 0.00 996.67	WIND 8 6 1/2 0.99 996.67 0.00 996.67	BUILDING P PERIMETER MIN
SOURCE: PER MAP		LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 866.67 WIND DESIGN BASE SHEAR - KIPS: 33.21	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21	WIND 8 6 1/2 0.90 996.67 0.00 996.67 -38.61	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61	BUILDING P PERIMETER MIN AUTOMATIC SPRIN
SOURGE: PER MAP		LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 866.67 WIND DESIGN BASE SHEAR - KIPS: 33.21 TRANSVERSE: WIND	WIND 8 6 1/2 3.75 866.67 0.00 866.67	WIND 8 6 1/2 0.90 996.67 0.00 996.67	WIND 8 6 1/2 0.99 996.67 0.00 996.67	BUILDING P PERIMETER MIN AUTOMATIC SPRIN
SOURCE: PER MAP		LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 866.67 WIND DESIGN BASE SHEAR - KIPS: 33.21	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21	WIND 8 6 1/2 0.90 996.67 0.00 996.67 -38.61	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61	BUILDING P PERIMETER MIN AUTOMATIC SPRIN
SOURCE: PER MAP		LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 866.67 WIND DESIGN BASE SHEAR - KIPS: -33.21 <u>TRANSVERSE</u> : WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21 WIND 8	WIND 8 6 1/2 0.90 996.67 0.00 996.67 -38.61 WIND 8	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61 WIND 8	BUILDING PE PERIMETER MIN AUTOMATIC SPRIN A FIRE-RESISTANCE-RA
SOURCE: PER MAP		LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 866.67 WIND DESIGN BASE SHEAR - KIPS: 33.21 TRANSVERSE: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 5,168.33	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21 WIND 8 6 1/2	WIND 8 6 1/2 0.90 996.67 0.00 996.67 -38.61 WIND 8 6 1/2	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61 WIND 8 6 1/2	BUILDING P PERIMETER MIN AUTOMATIC SPRIN FIRE-RESISTANCE-RA MAXIMUM AREA OF EXTE
SOURCE: PER MAP		LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 866.67 WIND DESIGN BASE SHEAR - KIPS: 33.21 TRANSVERSE: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 5,168.33 ADJUSTMENTS - SF: 0.00	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21 WIND 8 6 1/2 3.75	WIND 8 6 1/2 0.90 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.90	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.99 5,966.00 0.00	BUILDING P PERIMETER MIN AUTOMATIC SPRIN FIRE-RESISTANCE-RA MAXIMUM AREA OF EXTE
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SOURCE: PER MAP		LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 866.67 WIND DESIGN BASE SHEAR - KIPS: -33.21 <u>TRANSVERSE</u> : WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 5,168.33 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 5,168.33 WIND DESIGN BASE SHEAR - KIPS: -196.28	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21 WIND 8 6 1/2 3.75 5,250.00 0.00	WIND 8 6 1/2 0.90 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.90 5,446.67 0.00	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.99 5,966.00 0.00	BUILDING P PERIMETER MIN AUTOMATIC SPRIN A FIRE-RESISTANCE-RA MAXIMUM AREA OF EXTE SHALL BE PERMITTED TO MIXED (PRIMA
SOURCE: PER MAP		LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 866.67 WIND DESIGN BASE SHEAR - KIPS: 33.21 TRANSVERSE: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 5,168.33 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 5,168.33 WIND DESIGN BASE SHEAR - KIPS: 196.28 BASIC SEISMIC-FORCE RESISTING SYSTEM LEGEND:	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21 WIND 8 6 1/2 3.75 5,250.00 0.00 5,250.00	WIND 8 6 1/2 0.90 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.90 5,446.67 0.00 5,446.67	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.99 5,966.00 5,966.00	BUILDING PE PERIMETER MIN AUTOMATIC SPRIN A FIRE-RESISTANCE-RA MAXIMUM AREA OF EXTEI SHALL BE PERMITTED TO MIXED (PRIMAR REQ. SEPERATION PRI
SOURCE: PER MAP		LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 866.67 WIND DESIGN BASE SHEAR - KIPS: 33.21 TRANSVERSE: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 5,168.33 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 5,168.33 WIND DESIGN BASE SHEAR - KIPS: -196.28 BASIC SEISMIC-FORCE RESISTING SYSTEM LEGEND: 2 ORDINARY REINFORCED CONCRETE	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21 WIND 8 6 1/2 3.75 5,250.00 0.00 5,250.00	WIND 8 6 1/2 0.90 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.90 5,446.67 0.00 5,446.67	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.99 5,966.00 5,966.00	BUILDING PE PERIMETER MIN AUTOMATIC SPRIN A FIRE-RESISTANCE-RA MAXIMUM AREA OF EXTEN SHALL BE PERMITTED TO MIXED (PRIMAF REQ. SEPERATION PRI TYP. RATED BLDG AREAS
SOURCE: PER MAP		LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 866.67 WIND DESIGN BASE SHEAR - KIPS: 33.21 TRANSVERSE: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 5,168.33 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 5,168.33 WIND DESIGN BASE SHEAR - KIPS: -196.28 BASIC SEISMIC-FORCE RESISTING SYSTEM LEGEND: 2 ORDINARY REINFORCED CONCRETE 3 SPECIAL REINFORCED MASONRY	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21 WIND 8 6 1/2 3.75 5,250.00 0.00 5,250.00	WIND 8 6 1/2 0.90 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.90 5,446.67 0.00 5,446.67	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.99 5,966.00 5,966.00	BUILDING F PERIMETEF MIN AUTOMATIC SPRIN FIRE-RESISTANCE-RA MAXIMUM AREA OF EXTE SHALL BE PERMITTED TO MIXED (PRIMA REQ.SEPERATION PR TYP. RATED BLDG AREAS TENANT SEPERATIO
		LONGITUDINAL:WINDBASIC SEISMIC-FORCE RESISTING SYSTEM:8RESPONSE MODIFICATION COEFFICIENT - R:6 1/2SEISMIC DESIGN BASE SHEAR - KIPS:3.69EFFFECTIVE WIND AREA -SF:866.67ADJUSTMENTS - SF:0.00TOTAL EFFFECTIVE WIND AREA -SF:866.67WIND DESIGN BASE SHEAR - KIPS:-33.21TRANSVERSE:WINDBASIC SEISMIC-FORCE RESISTING SYSTEM:8RESPONSE MODIFICATION COEFFICIENT - R:6 1/2SEISMIC DESIGN BASE SHEAR - KIPS:3.69EFFFECTIVE WIND AREA -SF:5,168.33ADJUSTMENTS - SF:0.00TOTAL EFFFECTIVE WIND AREA -SF:5,168.33ADJUSTMENTS - SF:0.00TOTAL EFFFECTIVE WIND AREA -SF:5,168.33WIND DESIGN BASE SHEAR - KIPS:-196.28BASIC SEISMIC-FORCE RESISTING SYSTEM LEGEND:2QORDINARY REINFORCED CONCRETE3SPECIAL REINFORCED MASONRY4INTERMEDIATE REINFORCED MASONRY4	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21 WIND 8 6 1/2 3.75 5,250.00 0.00 5,250.00	WIND 8 6 1/2 0.90 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.90 5,446.67 0.00 5,446.67	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.99 5,966.00 5,966.00	BUILDING P PERIMETER MIN AUTOMATIC SPRIN FIRE-RESISTANCE-RA MAXIMUM AREA OF EXTE SHALL BE PERMITTED TO MIXED (PRIMA REQ. SEPERATION PR TYP. RATED BLDG AREAS TENANT SEPERATIO SHAFT ENCL. <= 3 STOR
	G DOOR WIND LOADS FOR BIJII DINGS IN	LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 866.67 WIND DESIGN BASE SHEAR - KIPS: 33.21 TRANSVERSE: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 5,168.33 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 5,168.33 WIND DESIGN BASE SHEAR - KIPS: -196.28 BASIC SEISMIC-FORCE RESISTING SYSTEM LEGEND: 2 ORDINARY REINFORCED CONCRETE 3 SPECIAL REINFORCED MASONRY	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21 WIND 8 6 1/2 3.75 5,250.00 0.00 5,250.00	WIND 8 6 1/2 0.90 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.90 5,446.67 0.00 5,446.67	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.99 5,966.00 5,966.00	BUILDING P PERIMETER MIN AUTOMATIC SPRIN FIRE-RESISTANCE-RA MAXIMUM AREA OF EXTE SHALL BE PERMITTED TO MIXED (PRIMA REQ.SEPERATION PR TENANT SEPERATION SHAFT ENCL. <= 3 STOR SHAFT ENCL. <= 3 STOR
NOMINAL (ASD) GARAGE DOOR & ROLLIN		LONGITUDINAL:WINDBASIC SEISMIC-FORCE RESISTING SYSTEM:8RESPONSE MODIFICATION COEFFICIENT - R:6 1/2SEISMIC DESIGN BASE SHEAR - KIPS:3.69EFFFECTIVE WIND AREA -SF:866.67ADJUSTMENTS - SF:0.00TOTAL EFFFECTIVE WIND AREA -SF:866.67WIND DESIGN BASE SHEAR - KIPS:-33.21TRANSVERSE:WINDBASIC SEISMIC-FORCE RESISTING SYSTEM:8RESPONSE MODIFICATION COEFFICIENT - R:6 1/2SEISMIC DESIGN BASE SHEAR - KIPS:3.69EFFFECTIVE WIND AREA -SF:5,168.33ADJUSTMENTS - SF:0.00TOTAL EFFFECTIVE WIND AREA -SF:5,168.33ADJUSTMENTS - SF:0.00TOTAL EFFFECTIVE WIND AREA -SF:5,168.33WIND DESIGN BASE SHEAR - KIPS:-196.28BASIC SEISMIC-FORCE RESISTING SYSTEM LEGEND:2QORDINARY REINFORCED CONCRETE3SPECIAL REINFORCED MASONRY4INTERMEDIATE REINFORCED MASONRY5SORDINARY REINFORCED MASONRY	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21 WIND 8 6 1/2 3.75 5,250.00 0.00 5,250.00	WIND 8 6 1/2 0.90 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.90 5,446.67 0.00 5,446.67	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.99 5,966.00 5,966.00	BUILDING P PERIMETER MIN AUTOMATIC SPRIN AUTOMATIC SPRIN FIRE-RESISTANCE-RA MAXIMUM AREA OF EXTE SHALL BE PERMITTED TO MIXED (PRIMA REQ.SEPERATION PR TENANT SEPERATIO SHAFT ENCL. <= 3 STOR SHAFT ENCL. > 3 STOR A SHAFT ENCLOSURE TH
		LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA - SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA - SF: 866.67 WIND DESIGN BASE SHEAR - KIPS: -33.21 TRANSVERSE: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA - SF: 5,168.33 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA - SF: 5,168.33 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA - SF: 5,168.33 WIND DESIGN BASE SHEAR - KIPS: -196.28 BASIC SEISMIC-FORCE RESISTING SYSTEM LEGEND: 2 ORDINARY REINFORCED CONCRETE 3 SPECIAL REINFORCED MASONRY 4 INTERMEDIATE REINFORCED MASONRY 5 ORDINARY REINFORCED MASONRY 6 DETAILED PLAIN MASONRY	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21 WIND 8 6 1/2 3.75 5,250.00 0.00 5,250.00	WIND 8 6 1/2 0.90 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.90 5,446.67 0.00 5,446.67	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.99 5,966.00 5,966.00	BUILDING P PERIMETER MIN AUTOMATIC SPRIN A FIRE-RESISTANCE-RA MAXIMUM AREA OF EXTE SHALL BE PERMITTED TO MIXED (PRIMAN REQ.SEPERATION PRI TYP. RATED BLDG AREAS TENANT SEPERATIO SHAFT ENCL. <= 3 STORI SHAFT ENCL. > 3 STORI A SHAFT ENCLOSURE THA SHEATHING, DECK OR SLA
NOMINAL (ASD) GARAGE DOOR & ROLLIN		LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA - SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA - SF: 866.67 WIND DESIGN BASE SHEAR - KIPS: -33.21 TRANSVERSE: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA - SF: 5,168.33 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA - SF: 5,168.33 WIND DESIGN BASE SHEAR - KIPS: -196.28 BASIC SEISMIC-FORCE RESISTING SYSTEM LEGEND: 2 ORDINARY REINFORCED CONCRETE 3 SPECIAL REINFORCED MASONRY 4 INTERMEDIATE REINFORCED MASONRY 5 ORDINARY REINFORCED MASONRY 6 DETAILED PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL.	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21 WIND 8 6 1/2 3.75 5,250.00 0.00 5,250.00	WIND 8 6 1/2 0.90 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.90 5,446.67 0.00 5,446.67	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.99 5,966.00 5,966.00	BUILDING PE PERIMETER MIN AUTOMATIC SPRIN A FIRE-RESISTANCE-RA MAXIMUM AREA OF EXTEN SHALL BE PERMITTED TO MIXED (PRIMAF REQ.SEPERATION PRI TYP. RATED BLDG AREAS TENANT SEPERATIO SHAFT ENCL. <= 3 STORI SHAFT ENCL. > 3 STORI
NOMINAL (ASD) GARAGE DOOR & ROLLIN EXPOSURE "B" W/ MEAN RC	OF HEIGHT OF 30FT (PSF)	LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 866.67 WIND DESIGN BASE SHEAR - KIPS: 33.21 TRANSVERSE: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 5,168.33 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 5,168.33 WIND DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 5,168.33 MUND DESIGN BASE SHEAR - KIPS: -196.28 BASIC SEISMIC-FORCE RESISTING SYSTEM LEGEND: 2 ORDINARY REINFORCED CONCRETE 3 SPECIAL REINFORCED MASONRY 4 INTERMEDIATE REINFORCED MASONRY 5 ORDINARY REINFORCED MASONRY 6 DETAILED PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21 WIND 8 6 1/2 3.75 5,250.00 0.00 5,250.00	WIND 8 6 1/2 0.90 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.90 5,446.67 0.00 5,446.67	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.99 5,966.00 5,966.00	BUILDING P PERIMETER MIN AUTOMATIC SPRIN AUTOMATIC SPRIN FIRE-RESISTANCE-RA MAXIMUM AREA OF EXTE SHALL BE PERMITTED TO MIXED (PRIMA REQ. SEPERATION PR TYP. RATED BLDG AREAS TENANT SEPERATIO SHAFT ENCL. <= 3 STOR SHAFT ENCL. <= 3 STOR SHAFT ENCL. <= 3 STOR SHAFT ENCL. > 3 STOR A SHAFT ENCL. > 3 STOR A SHAFT ENCL. > 3 STOR SHAFT ENCL. > 3 STOR A SHAFT ENCLOSURE TH/ SHEATHING, DECK OR SL ENCLOSURE. ELEVATOR MECH RO
NOMINAL (ASD) GARAGE DOOR & ROLLIN EXPOSURE "B" W/ MEAN RC	OOF HEIGHT OF 30FT (PSF) 0 MPH 1 50 MPH 1 60 MPH 1 70 MPH 1 80 MPH 1 90 MPH 200 MPH	LONGITUDINAL:WINDBASIC SEISMIC-FORCE RESISTING SYSTEM:8RESPONSE MODIFICATION COEFFICIENT - R:6 1/2SEISMIC DESIGN BASE SHEAR - KIPS:3.69EFFFECTIVE WIND AREA -SF:866.67ADJUSTMENTS - SF:0.00TOTAL EFFFECTIVE WIND AREA -SF:866.67WIND DESIGN BASE SHEAR - KIPS:-33.21TRANSVERSE:WINDBASIC SEISMIC-FORCE RESISTING SYSTEM:8RESPONSE MODIFICATION COEFFICIENT - R:6 1/2SEISMIC DESIGN BASE SHEAR - KIPS:3.69EFFFECTIVE WIND AREA -SF:5,168.33ADJUSTMENTS - SF:0.00TOTAL EFFFECTIVE WIND AREA -SF:5,168.33ADJUSTMENTS - SF:0.00TOTAL EFFFECTIVE WIND AREA -SF:5,168.33WIND DESIGN BASE SHEAR - KIPS:-196.28BASIC SEISMIC-FORCE RESISTING SYSTEM LEGEND:-196.28QORDINARY REINFORCED CONCRETE3SPECIAL REINFORCED MASONRY4INTERMEDIATE REINFORCED MASONRY5ORDINARY REINFORCED MASONRY6DETAILED PLAIN MASONRY7STL. SYS. NOT DET. SEISMIC EXCL. COL.8LIGHT-FRAMED WALLS SHEATHED	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21 WIND 8 6 1/2 3.75 5,250.00 0.00 5,250.00	WIND 8 6 1/2 0.90 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.90 5,446.67 0.00 5,446.67	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.99 5,966.00 5,966.00	BUILDING PE PERIMETER MINI AUTOMATIC SPRINI A FIRE-RESISTANCE-RAT MAXIMUM AREA OF EXTER SHALL BE PERMITTED TO I MIXED (PRIMAR REQ.SEPERATION PRII TYP. RATED BLDG AREAS TENANT SEPERATION SHAFT ENCL. <= 3 STORII SHAFT ENCL
NOMINAL (ASD) GARAGE DOOR & ROLLIN EXPOSURE "B" W/ MEAN RC	OOF HEIGHT OF 30FT (PSF) 0 MPH 150 MPH 160 MPH 170 MPH 180 MPH 190 MPH 200 MPH	LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 866.67 WIND DESIGN BASE SHEAR - KIPS: -33.21 TRANSVERSE: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 5,168.33 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 5,168.33 WIND DESIGN BASE SHEAR - KIPS: -196.28 BASIC SEISMIC-FORCE RESISTING SYSTEM LEGEND: 2 ORDINARY REINFORCED CONCRETE 3 SPECIAL REINFORCED MASONRY 4 INTERMEDIATE REINFORCED MASONRY 5 ORDINARY REINFORCED MASONRY 6 DETAILED PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY 7 ORDINARY PLAIN MASONRY 8 ORDINARY PLAIN MASONRY 7 ORDINARY PLAIN MASONRY 8 ORDINARY PLAIN MASONRY 7 ORDINARY PLAIN MASONRY 8 ORDINARY PLAIN MASONRY	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21 WIND 8 6 1/2 3.75 5,250.00 0.00 5,250.00	WIND 8 6 1/2 0.90 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.90 5,446.67 0.00 5,446.67	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.99 5,966.00 0.00 5,966.00 -226.96	BUILDING PE PERIMETER MIN AUTOMATIC SPRIN A FIRE-RESISTANCE-RA MAXIMUM AREA OF EXTER SHALL BE PERMITTED TO MIXED (PRIMAF REQ.SEPERATION PRI TYP. RATED BLDG AREAS TENANT SEPERATIO SHAFT ENCL. <= 3 STORI SHAFT ENCL. <= 3 STORI SHAF
NOMINAL (ASD) GARAGE DOOR & ROLLIN EXPOSURE "B" W/ MEAN RC WIDTH HEIGHT VUIE 100 MPH 110 MPH 120 MPH 130 MPH 14 (PT) (PT) VIDTH 100 MPH 100 MPH 100 MPH 101 MP	OOF HEIGHT OF 30FT (PSF) 0 MPH 150 MPH 160 MPH 170 MPH 180 MPH 190 MPH 200 MPH 18 MPH 116 MPH 124 MPH 132 MPH 139 MPH 147 MPH 155 MPH	LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 866.67 WIND DESIGN BASE SHEAR - KIPS: -33.21 TRANSVERSE: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 5,168.33 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 5,168.33 WIND DESIGN BASE SHEAR - KIPS: -196.28 BASIC SEISMIC-FORCE RESISTING SYSTEM LEGEND: 2 ORDINARY REINFORCED CONCRETE 3 SPECIAL REINFORCED MASONRY 4 INTERMEDIATE REINFORCED MASONRY 5 ORDINARY REINFORCED MASONRY 6 DETAILED PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21 WIND 8 6 1/2 3.75 5,250.00 0.00 5,250.00	WIND 8 6 1/2 0.90 996.67 -38.61 WIND 8 6 1/2 0.90 5,446.67 0.00 5,446.67 -207.28	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.99 5,966.00 5,966.00	BUILDING PE PERIMETER (MINI AUTOMATIC SPRINE AUTOMATIC SPRINE A FIRE-RESISTANCE-RAT MAXIMUM AREA OF EXTER SHALL BE PERMITTED TO B MIXED (PRIMAR REQ.SEPERATION PRIN TENANT SEPERATION SHAFT ENCL. <= 3 STORIE SHAFT ENCL. <=
NOMINAL (ASD) GARAGE DOOR & ROLLIN EXPOSURE "B" W/ MEAN RC MIDTH HEIGHT VUIL 100 MPH 110 MPH 120 MPH 130 MPH 14 (FT) (FT) VNOM 78 MPH 35 MPH 130 MPH 101 MPH 14 ROOF ANGLE 0 < 1	OOF HEIGHT OF 30FT (PSF) 0 MPH 150 MPH 160 MPH 170 MPH 180 MPH 190 MPH 200 MPH 18 MPH 116 MPH 124 MPH 132 MPH 139 MPH 147 MPH 155 MPH 0 DEGREES 0 0 0 0 0 0	LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 866.67 WIND DESIGN BASE SHEAR - KIPS: -33.21 TRANSVERSE: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 5,168.33 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 5,168.33 WIND DESIGN BASE SHEAR - KIPS: -196.28 BASIC SEISMIC-FORCE RESISTING SYSTEM LEGEND: 2 ORDINARY REINFORCED CONCRETE 3 SPECIAL REINFORCED MASONRY 4 INTERMEDIATE REINFORCED MASONRY 5 ORDINARY REINFORCED MASONRY 6 DETAILED PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY 7 ORDINARY PLAIN MASONRY 8 ORDINARY PLAIN MASONRY 7 ORDINARY PLAIN MASONRY 8 ORDINARY PLAIN MASONRY 7 ORDINARY PLAIN MASONRY 8 ORDINARY PLAIN MASONRY	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21 WIND 8 6 1/2 3.75 5,250.00 0.00 5,250.00	WIND 8 6 1/2 0.90 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.90 5,446.67 0.00 5,446.67	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.99 5,966.00 0.00 5,966.00 -226.96	BUILDING PE PERIMETER MIN AUTOMATIC SPRIN A FIRE-RE SISTANCE-RA MAXIMUM AREA OF EXTER SHALL BE PERMITTED TO MIXED (PRIMAF REQ.SEPERATION PRI TYP. RATED BLDG AREAS TENANT SEPERATION SHAFT ENCL. > 3 STORI SHAFT ENCL. > 3 STORI SHAFT ENCL. > 3 STORI A SHAFT ENCL. > 3 STORI ENCLOSURE. ELEVATOR MECH ROO STORAGE FIRE ARE EXTERIOR WA EXTERIOR WA
NOMINAL (ASD) GARAGE DOOR & ROLLIN EXPOSURE "B" W/ MEAN RC MIDTH HEIGHT VUIL 100 MPH 110 MPH 120 MPH 130 MPH 14 (FT) (FT) VNOM 78 MPH 35 MPH 130 MPH 101 MPH 14 ROOF ANGLE 0 < 1	OOF HEIGHT OF 30FT (PSF) 0 MPH 150 MPH 160 MPH 170 MPH 180 MPH 190 MPH 200 MPH 18 MPH 116 MPH 124 MPH 132 MPH 139 MPH 147 MPH 155 MPH	LONGITUDINAL: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 866.67 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 866.67 WIND DESIGN BASE SHEAR - KIPS: -33.21 TRANSVERSE: WIND BASIC SEISMIC-FORCE RESISTING SYSTEM: 8 RESPONSE MODIFICATION COEFFICIENT - R: 6 1/2 SEISMIC DESIGN BASE SHEAR - KIPS: 3.69 EFFFECTIVE WIND AREA -SF: 5,168.33 ADJUSTMENTS - SF: 0.00 TOTAL EFFFECTIVE WIND AREA -SF: 5,168.33 WIND DESIGN BASE SHEAR - KIPS: -196.28 BASIC SEISMIC-FORCE RESISTING SYSTEM LEGEND: 2 ORDINARY REINFORCED CONCRETE 3 SPECIAL REINFORCED MASONRY 4 INTERMEDIATE REINFORCED MASONRY 5 ORDINARY REINFORCED MASONRY 6 DETAILED PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY 7 STL. SYS. NOT DET. SEISMIC EXCL. COL. 8 LIGHT-FRAMED WALLS SHEATHED 9 ORDINARY PLAIN MASONRY	WIND 8 6 1/2 3.75 866.67 0.00 866.67 -33.21 WIND 8 6 1/2 3.75 5,250.00 0.00 5,250.00	WIND 8 6 1/2 0.90 996.67 -38.61 WIND 8 6 1/2 0.90 5,446.67 0.00 5,446.67 -207.28	WIND 8 6 1/2 0.99 996.67 0.00 996.67 -38.61 WIND 8 6 1/2 0.99 5,966.00 0.00 5,966.00 -226.96	BUILDING PE PERIMETER MINI AUTOMATIC SPRINI A FIRE-RESISTANCE-RAT MAXIMUM AREA OF EXTER SHALL BE PERMITTED TO I MIXED (PRIMAR REQ.SEPERATION PRII TENANT SEPERATION SHAFT ENCL. <= 3 STORIE SHAFT EN

WIDTH	HEIGHT	Vult	100 MPH	I I O MPH	I 20 MPH	130 MPH	140 MPH	I 50 MPH	I GO MPH	170 MPH	180 MPH	190 MPH
(FT)	(FT)	Vnom	78 MPH	85 MPH	93 MPH	IOI MPH	108 MPH	IIG MPH	124 MPH	132 MPH	139 MPH	147 MPH
				ROOF	ANGL	E 0 <u><</u>	I O DE	EGREE	5			
8	8	+ -	8.7 -9.8	-11.9	-14.2	14.7 -16.6	-19.3	19.6 -22.2	22.3 -25.2	25.1 -28.5	28.2 -31.9	31.4 -35.5
10	10	+ -	8.4 -9.4	-11.4	-13.6	-16.0	16.5 -18.5	-21.2	21.5	24.3 -27.3	27.3 -30.6	30.4 -34.1
14	14	+	8.0 -8.9	9.7 -10.8	-12.8	13.5 -15.0	15.7 -17.4	18.0 -20.0	20.5 -22.8	23.1	25.9 -28.8	28.9 -32.1
	•			ROO	F ANG	LE >	IO DEC	GREES				
9	7	+ _	9.6 -10.9	-12.9	13.7 -15.5	16.1 -18.2	18.5 -20.9	21.3	24.3 -27.5	27.6	30.6 -34.6	34.2 -38.6
16	7	+	9.2 -10.3	10.9	3. - 4.6	15.5 -17.2	17.7 -19.7	20.4	23.3 -26.0	26.4 -29.4	29.3 -32.6	32.7 -36.5
-												

NOTES:
 FOR EFFECTIVE AREAS OR WIND SPEEDS BETWEEN THOSE GIVEN ABOVE, THE LOAD MAY BE INTERPOLATED, OTHERWISE USE THE LOAD ASSOCIATED WITH THE LOWER EFFECTIVE AREA.
 VALUES SHOWN ABOVE SHALL BE ADJUSTED FOR HEIGHT AND EXPOSURE BY MULTIPLYING BY THE RELAVENT COEFFICIENT SHOWN IN THE CHART TO THE RIGHT.
 PLUS (+) AND MINUS (-) SIGNS SIGNFY PRESSURES ACTING TOWARDS AND AWAY FROM BUILDING SURFACES.
 VALUES SHOWN IN TABLE ABOVE ALREADY INCLUDE THE LOAD REDUCTION FACTOR OF 0.6.

MEAN ROOF HEIGHT	EXPOSURE CATEGORY						
(FT)	В	С	D				
15	1.00	1.21	1.47				
20	1.00	1.29	I.55				
25	1.00	I.35	1.61				
30	1.00	1.40	1.66				
35	1.05	1.45	1.70				
40	1.09	1.49	1.74				
45	1.12	1.53	1.78				
50	1.16	1.56	1.81				
55	1.19	I.59	1.84				
60	1.22	1.62	1.87				

Received 8.9.23

MEANS OF EGRESS [CHAP OCCUPANCY & EGRESS WIDTH PRIMA

> MINIMUM EGRESS MINIMUM EGRESS SECONDA

MINIMUM EGRESS MINIMUM EGRESS

CORRIDOR WIDTHS / RATINGS

MAX. OCCUPANT CAPA REQUIRED CO

CORRIDO

COMMON

IF OCCUPANT CAPACITY OF A C BOTH DIRECTIONS AND NOT PAI SIGNS TO BE POSTED ON BOTH MAX. EXIT ACCESS TR MAXIMUM OVERALL DIAGON

MIN. DIST. APART DOOR ENCROACHMENT CALCU DOORS OPENING INTO THE PAT WIDTH TO < 50% DURING THE SHALL NOT PROJECT MORE TH MIN. CORR

EXIT SIGNS SIGNS SHALL BE INSTALLED SO TRAVEL, BUT IN NO CASE MORE

EGRESS ILLUMINATION THE MEANS OF EGRESS INCLUE APPROVED FIXTURES MEETING 1-FOOT CANDLE AND SHALL BE I MINIMUM DURATION OF 90 MINI

33.7 -37.8 32.0 38.0 6 -43.0 36.4 ; -40.6 ADJUSTMENT FACTOR FOR BUILDING HEIGHT \$

LDING(S) - SUMMARY	ANALYSIS C	F CODE		
BUILDING(S): PROJECT STRUCTURE(S):		2 D	3 E	4 F
				F
NCY CLASSIFICATION [CHAP PRIMARY OCCUPANCY				
GROUP:	S-1 MODHAZ. STOR.	S-1 MODHAZ. STOR.	S-1 MODHAZ. STOR.	S-1 MODHAZ. STOR.
E FOOTAGE (NEW AND/OR EXIST.): SECOND OCCUPANCY		35,325.00	8,643.00	9,506.00
GROUP:				
FOOTAGE (NEW AND/OR EXIST.):		MERCANTILE 675.00		
RUCTION [CHAPTER 6]				
CONSTRUCTION TYPE:		II-B NON-COMBUSTIBLE	II-B NON-COMBUSTIBLE	II-B NON-COMBUSTIBLE
		NONCOMBOSTIBLE		
TING REQS FOR BUILDING ELEME STRUCTURAL FRAME:	0			
EXTERIOR BEARING WALLS: INTERIOR BEARING WALLS:				
EARING WALLS AND PARTITIONS: EARING WALLS AND PARTITIONS:				
FLOOR CONSTRUCTION: ROOF CONSTRUCTION:	0			
TING REQS FOR EXT. WALLS BASE	·		<u> </u>	<u></u>
X < 5 FEET:	2	2	2	2
5 FEET >= X < 10 FEET: 10 FEET >= X:		0	0	1 0
SYSTEMS [CHAPTER 9]				
PRINKLER SYSTEM(PER NFPA13): PES > 2 1/2" IF APPLICABLE TO BE		YES	YES	YES
OM COLUMNS NOT BEAMS U.N.O.				
KLER SYSTEM SHALL BE PROVID				
P S-1 OCCUPANCY WHERE ONE O	F THE FOLLOWING (CONDITIONS		
EDS 12,000 SQUARE FEET. CATED MORE THAN THREE STOR	IES ABOVE GRADE (OF PLANE.		
AREAS EXCEED 24,000 SQUARE F REQUIRED # FIRE AREAS:	EET	N/A	N/A	N/A
	·			
GUISHERS SHALL BE INSTALLED W HALL BE LOCATED WITH A MAXIMI				
O EXCEED 75 FEET.				
G HEIGHTS AND AREAS [CHA ALLOWABLE HEIGHT - FEET:		NS=55; S=75	NS=55; S=75	NS=55; S=75
ACTUAL HEIGHT - FEET:	10.00	10.00	24.50	24.50
ALLOWABLE STORIES: ACTUAL STORIES:		3 1	3 1	3 1
VABLE AREA PER FLOOR - At - SF: TOTAL SQUARE FOOTAGE:		12,500.00 36,000.00	17,500.00 8,643.00	17,500.00 9,506.00
AREA MODIFICATION REQUIRED:	·	YES	NO	NO
FRONTAGE INCREASE - If =				
100 [F / P - 0.25] W / 30: G PERIMETER => 20 FEET - F - LF:	1,046.00	50.00% 1,060.00		
TER OF ENTIRE BUILDING - P - LF: MINIMUM WIDTH OF PUBLIC WAY		1,060.00		
OR OPEN SPACE - W - LF: PRINKLER SYSTEM INCREASE - Is:		20.00 300.00%		
Aa = At + (At x If) + (At x Is) - SF:		56,250.00		
RATED CONSTRUCTION [CH	APTER 7]			
XTERIOR WALL OPENINGS - MAXII) TO BE UNLIMITED IF EXTERIOR L				
IMARY / SECOND) OCCUPANCIES:		NONSEPERATED		
PRIMARY OCCUPANCY - HOURS:		N/A	N/A	N/A
ATIONS: 1 HR. ASSEM. W/ 3/4 HR. [[ORIES: 1 HR. FIRE ASSEM. W/ 1 H				
ORIES: 2 HR. FIRE ASSEM. W/ 1 1 THAT DOES NOT EXTEND TO UNE		ROOF		
R SLAB OF BUILDING SHALL BE FIF	RE-RESISTANT RATE	D SAME AS		
ROOM: EQUAL TO ELEVATOR				
AREAS: 3 HR. FIRE BARRIER W/ 3 R WALL: 1 HR. ASSEM. W/ 3/4 HR. I	DOOR			
R WALL: 2 HR. ASSEM. W/ 1 1/2 HR	. DOOR			
S [CHAPTER 10]				
SS WIDTH REQUIREMENTS PRIMARY OCCUPANCY GROSS:	500	500	500	500
OCCUPANCY LOAD:	71	71	18	20
EGRESS WIDTH STAIRS - INCHES: EGRESS WIDTH OTHER - INCHES:		N/A 10.65	N/A 2.7	N/A 3
ECONDARY OCCUPANCY GROSS: OCCUPANCY LOAD:		60 12		
EGRESS WIDTH STAIRS - INCHES: EGRESS WIDTH OTHER - INCHES:	N/A	N/A 1.8	N/A	N/A
TOTAL OCCUPANCY LOAD:	71	83	18	20
RATINGS & EGRESS REQUIREMEN ANT CAPACITY FOR A CORRIDOR:		X > 50	X <= 30	X <= 30
JIRED CORRIDOR WIDTH-INCHES: CORRIDOR FIRE RATING - HOURS:		44 0	36 0	36 0
REQUIRED DOOR SWING: OMMON PATH OF TRAVEL - FEET:	DIR. EGRESS	DIR. EGRESS	EITHER DIR.	EITHER DIR. 100
TY OF A COORIDOR IS <= 50 THEM	CAN BE USED FOR	EGRESS IN	100	100
D NOT PART OF COMMON PATH (ON BOTH SIDES OF DOORS THAT	EXIST IN CORRIDO	R.	[
CCESS TRAVEL DISTANCE - FEET: LL DIAGONAL DIMENSION - FEET:	451	250 458	250 219	250 239
T. APART OF EXIT DOORS - FEET: NT CALCULATIONS		152.6666667	73	79.66666667
O THE PATH OF EGRESS TRAVELS				
NG THE COARSE OF THE SWING. MORE THAN 7 INCHES INTO THE F	REQUIRED WIDTH.	, THE DUUK		
IN. CORRIDOR WIDTH REQ FEET 3 FEET DOOR:	4.83	4.83	4.5	4.5
3 FEET 6" DOOR: 4 FEET DOOR:		5.33 5.83	5 5.5	5 5.5
ALLED SO AS TO BE VISIBLE FROM			<u>.</u>	<u>.</u>
ALLED SO AS TO BE VISIBLE FROM ASE MORE THAN 100 FEET APART N				
SS INCLUDING EXIT DISCARGE, SI				
MEETING THE MINIMUM FLOOR I SHALL BE PROVIDED WITH EMER(
OF 90 MINUTES.				

				EAST COAST OFFICE: 1041 CROWN PARK CIRCLE WINTER GARDEN FL 34787 PHONE: (800)-989-0220		CONFIDENTIAL: THIS DOCUMENT AND THE INFORMATION CONTAINED HEREIN ARE THE PROPERTY OF MAKORABCO UNAUTHORIZED COPYING, DISCLOSURE OR OTHER UNAUTHORIZED USES ARE PROHIBITED.
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ve		on	any e		onic c Digita by Rol	e must be opies. ally signed bert Beattie 2023.08.04
RBE CONSULTING SERVICES,	LLC 2875 SPLIT OAK COURT - OVEDO, FL. 32766 WEBDITE: D	WWW.RBECS.COM - EMALI: RBECSLL@GMAIL.COM MAIN PHONE LINE: 407-796-3620 - FAX LINE: 407-971-6718	III MECHANICAL DEPT: 407-796-3776 - EMAIL: NEHAH@REECS.COM ELECTRICAL DEPT: 407-506-3776 - EMAIL: FAMILIBIA@REECS.COM STRUCTURAL DEPT: 407-585-7859 - EMAIL: REEATTIE@REECS.COM	ROBERT M. BEATTIE, PE; FLORIDA PE #55428 FLORIDA CERTIFICATE OF AUTHORIZATION #27269	ES, LLC" HEREBY COMMON LAW PERTY RIGHTS TO	THESE TRANS AND THE DESIGN CONTACT WITHIN THESE TREAD RAWINGS SHALL NOT TO BE ALTREED. COPIED, OR REFEREDUCED OR MANNER WITHOUT THE REFEREDUCED RAWINGSON AND CONSENT OF TREE CONSULTING FERMISSION AND CONSENT OF TREE CONSULTING
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CH JOI SC	TE AWN ECKE 3 NO ALE EET		3Y			6- I 2-23 AW/MNG XXX E 2705
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MARKUPS / REVISIONS

NO. BY

					E	Edge Condit	tion -a- Ft.	8.00	Edg	e Condition	ı - 0.6h-Ft.	14.70	Edge Conditio	on - 0.2h-Ft.	4.90									
	SELF WT.	UPLIFT	LIVE LOAD					5 FT SPAN						SELF WT.	UPLIFT	LIVE LOAD				2.5 FT SPA	N			
ROOFING SYSTEM: MBCI	PSF	CAPACITY -	CAPACITY -				NET DES	gn wind Pf	RESSURE				ROOFING SYSTEM: MBCI	PSF	CAPACITY	CAPACITY				NET DESIGN WIND P	RESSURE			
	101	PSF	PSF	ZONE 1'	ZONE 1	ZONE 2	ZONE 2e	ZONE 2n	ZONE 2r	ZONE 3	ZONE 3e	ZONE 3r		-	PSF	PSF	ZONE 1'	ZONE 1	ZONE 2	ZONE 2e ZONE 2n	ZONE 2r	ZONE 3	ZONE 3e	ZONE 3r
Final Comparison Values			Positive	16.00	16.00	16.89				16.89			Final Comparison Values			Positive	16.00	16.00	16.89		-	16.89		
			Negative	36.87	54.88	86.25				117.95						Negative	36.87	54.88	86.25			117.95		
24" 26GA UD	1.02	17.65	55.40	No	No	No	NA	NA	NA	No	NA	NA	24" 26GA UD	1.02	30.10	146.90	No	No	No	NA NA	NA	No	NA	NA
24" 26GA UD CLIPS	1.02	36.86	55.40	No	No	No	NA	NA	NA	No	NA	NA	24" 26GA UD CLIPS	1.02	<mark>86.67</mark>	146.90	Yes	Yes	Yes	NA NA	NA	No	NA	NA
24" 24GA UD	1.23	23.87	75.90	No	No	No	NA	NA	NA	No	NA	NA	24" 24GA UD	1.23	54.14	204.00	Yes	No	No	NA NA	NA	No	NA	NA
24" 24GA UD CLIPS	1.23	42.83	75.90	Yes	No	No	NA	NA	NA	No	NA	NA	24" 24GA UD CLIPS	1.23	76.04	204.00	Yes	Yes	No	NA NA	NA	No	NA	NA
24" 22GA UD	1.56	28.73	106.30	No	No	No	NA	NA	NA	No	NA	NA	24" 22GA UD	1.56	74.95	296.90	Yes	Yes	No	NA NA	NA	No	NA	NA
24" 22GA UD CLIPS	1.56	48.53	106.30	Yes	No	No	NA	NA	NA	No	NA	NA	24" 22GA UD CLIPS	1.56	86.17	296.90	Yes	Yes	No	NA NA	NA	No	NA	NA
18" 24GA UD	1.32	24.95	101.60	No	No	No	NA	NA	NA	No	NA	NA	18" 24GA UD	1.32	55.19	272.00	Yes	Yes	No	NA NA	NA	No	NA	NA
18" 24GA UD CLIPS	1.32	47.54	101.60	Yes	No	No	NA	NA	NA	No	NA	NA	18" 24GA UD CLIPS	1.32	113.90	272.00	Yes	Yes	Yes	NA NA	NA	No	NA	NA
18" 22GA UD	1.66	40.48	142.10	Yes	No	No	NA	NA	NA	No	NA	NA	18" 22GA UD	1.66	77.67	395.90	Yes	Yes	No	NA NA	NA	No	NA	NA
18" 22GA UD CLIPS	1.66	66.13	142.10	Yes	Yes	No	NA	NA	NA	No	NA	NA	18" 22GA UD CLIPS	1.66	150.29	395.90	Yes	Yes	Yes	NA NA	NA	Yes	NA	NA
12" 24GA UD	1.48	52.00	123.00	Yes	No	No	NA	NA	NA	No	NA	NA	12" 24GA UD	1.48	95.33	408.00	Yes	Yes	Yes	NA NA	NA	No	NA	NA
12" 22GA UD	1 .86	78.00	178.60	Yes	Yes	No	NA	NA	NA	No	NA	NA	12" 22GA UD	1.86	104.00	593.90	Yes	Yes	Yes	NA NA	NA	No	NA	NA
24" 24GA DL	1.23	40.84	79.00	Yes	No	No	NA	NA	NA	No	NA	NA	24" 24GA DL	1.23	63.75	204.00	Yes	Yes	No	NA NA	NA	No	NA	NA
24" 24GA DL CLIPS	1.23	61.12	79.00	Yes	Yes	No	NA	NA	NA	No	NA	NA	24" 24GA DL CLIPS	1.23	126.24	204.00	Yes	Yes	Yes	NA NA	NA	Yes	NA	NA
18" 22GA LOKSEAM	1.60		47.50	No	No	No	NA	NA	NA	No	NA	NA	18" 22GA LOKSEAM	1.60	55.00	200.00	Yes	Yes	No	NA NA	NA	No	NA	NA
16" 24GA LOKSEAM	1.34		36.20	No	No	No	NA	NA	NA	No	NA	NA	16" 24GA LOKSEAM	1.34	44.11	166.10	Yes	No	No	NA NA	NA	No	NA	NA
16" 22GA LOKSEAM	1.71		53.20	No	No	No	NA	NA	NA	No	NA	NA	16" 22GA LOKSEAM	1.71	65.39	200.00	Yes	Yes	No	NA NA	NA	No	NA	NA
12" 24GA LOKSEAM	1.41		47.70	No	No	No	NA	NA	NA	No	NA	NA	12" 24GA LOKSEAM	1.41	59. <mark>4</mark> 8	200.00	Yes	Yes	No	NA NA	NA	No	NA	NA
12" 22GA LOKSEAM	1.81		70.10	No	No	No	NA	NA	NA	No	NA	NA	12" 22GA LOKSEAM	1.81	82.55	200.00	Yes	Yes	No	NA NA	NA	No	NA	NA
26GA PBR SD	0.94	43.33	46.37	Yes	No	No	NA	NA	NA	No	NA	NA	26GA PBR SD	0.94	133.48	119.08	Yes	Yes	Yes	NA NA	NA	Yes	NA	NA
24GA PBR SD	1.14	44.56	44.80	Yes	No	No	NA	NA	NA	No	NA	NA	24GA PBR SD	1.14	126.37	125.69	Yes	Yes	Yes	NA NA	NA	Yes	NA	NA
	ĺ	UPLIFT	LIVE LOAD					4 FT SPAN							UPLIFT	LIVE LOAD				3 FT SPAN	1			
ROOFING SYSTEM: MBCI	SELF WT.	CAPACITY -	CAPACITY -										ROOFING SYSTEM: MBCI	SELF WT.	CAPACITY	CAPACITY								
	PSF	PSF	PSF	ZONE 1'	ZONE 1	ZONE 2	ZONE 2e	GN WIND PF ZONE 2n		ZONE 3	ZONE 3e	ZONE 3r		PSF	PSF	PSF	ZONE 1'	ZONE 1	ZONE 2	NET DESIGN WIND P ZONE 2e ZONE 2n		ZONE 3	ZONE 3e	ZONE 3r
Final Comparison Values		101					ZONE Ze	ZONE ZII	ZONE 2r		ZONE Se	ZONE ST	Final Comparison Values		101					ZONE ZE ZONE ZI	ZUNE Zr		ZONE Se	ZUNE ST
Final Comparison Values			Positive	16.00	16.00	16.89 86.25				16.89 117.95			Final Comparison Values			Positive	16.00 36.87	16.00 54.88	16.89 86.25			16.89 117.95		
	1.02	00.00	Negative	36.87	54.88			NIA	A1.A					1.02	07.04	Negative	1. Construction of the	1		A14 A14				
24" 26GA UD	1.02	22.63	91.80	No	No	No	NA	NA	NA	No	NA	NA	24" 26GA UD	1.02	27.61	122.40	No	No	No	NA NA	NA	No	NA	NA
24" 26GA UD CLIPS	1.02	56.78	91.80	Yes	Yes	No	NA	NA	NA	No	NA	NA	24" 26GA UD CLIPS	1.02	76.70	122.40	Yes	Yes	No	NA NA	NA	No	NA	NA
24" 24GA UD	1.23	35.98	127.50	No	No	No	NA	NA	NA	No	NA	NA	24" 24GA UD	1.23	48.09	170.00	Yes	No	No	NA NA	NA	No	NA	NA
24" 24GA UD CLIPS 24" 22GA UD	1.23	56.12 47.22	127.50	Yes	Yes	No	NA	NA	NA	No	NA	NA	24" 24GA UD CLIPS	1.23	69.40	170.00	Yes	Yes	No	NA NA	NA	No	NA	NA
24" 22GA UD CLIPS	1.56 1.56	63.59	185.60 185.60	Yes	No	No	NA	NA NA	NA	No	NA	NA NA	24" 22GA UD 24" 22GA UD CLIPS	1.56 1.56	65.70 78.64	247.50 247.50	Yes	Yes	No	NA NA NA NA	NA NA	No	NA NA	NA
18" 24GA UD				Yes Yes	Yes No	No No	NA NA	NA	NA NA	No No	NA NA	NA NA	18" 24GA UD	1.30	49.13	226.70	Yes Yes	Yes No	No No	NA NA NA NA	NA NA	No No	NA	NA NA
10 Z4GA UD	1 2 2	27 02	1 /0 00		INO				N/H			11/4			49.15							No		
	1.32	37.02	170.00									NIA			100.62	226 70	V OC	Voo	V oo	R R R R R R R R R R	RI A	INU	NA	NA NA
18" 24GA UD CLIPS	1.32	74.09	170.00	Yes	Yes	No	NA	NA	NA	No	NA	NA	18" 24GA UD CLIPS	1.32	100.63	226.70	Yes	Yes	Yes	NA NA	NA		Δ <i>Ι</i> Δ Ι	
18" 24GA UD CLIPS 18" 22GA UD	1.32 1.66	74.09 55.36	170.00 239.30	Yes Yes	Yes Yes	No No	NA NA	NA NA	NA NA	No No	NA NA	NA	18" 24GA UD CLIPS 18" 22GA UD	1.32 1.66	70.23	329.90	Yes	Yes	No	NA NA	NA	No	NA	
18" 24GA UD CLIPS 18" 22GA UD 18" 22GA UD CLIPS	1.32 1.66 1.66	74.09 55.36 99.79	170.00 239.30 239.30	Yes Yes Yes	Yes Yes Yes	No No Yes	NA NA NA	NA NA NA	NA NA NA	No No No	NA NA NA	NA NA	18" 24GA UD CLIPS 18" 22GA UD 18" 22GA UD CLIPS	1.32 1.66 1.66	70.23 133.46	329.90 329.90	Yes Yes	Yes Yes	No Yes	NA NA NA NA	NA NA	No Yes	NA	NA
18" 24GA UD CLIPS 18" 22GA UD 18" 22GA UD CLIPS 12" 24GA UD	1.32 1.66 1.66 1.48	74.09 55.36 99.79 69.33	170.00 239.30 239.30 255.00	Yes Yes Yes Yes	Yes Yes Yes Yes	No No Yes No	NA NA NA NA	NA NA NA NA	NA NA NA NA	No No No No	NA NA NA NA	NA NA NA	18" 24GA UD CLIPS 18" 22GA UD 18" 22GA UD CLIPS 12" 24GA UD	1.32 1.66 1.66 1.48	70.23 133.46 86.66	329.90 329.90 340.00	Yes Yes Yes	Yes Yes Yes	No Yes Yes	NA NA NA NA NA NA	NA NA NA	No Yes No	NA NA	NA NA
18" 24GA UD CLIPS 18" 22GA UD 18" 22GA UD CLIPS 12" 24GA UD 12" 24GA UD 12" 22GA UD	1.32 1.66 1.66 1.48 1.86	74.09 55.36 99.79 69.33 88.40	170.00 239.30 239.30 255.00 335.10	Yes Yes Yes Yes Yes	Yes Yes Yes Yes Yes	No No Yes No Yes	NA NA NA NA NA	NA NA NA NA NA	NA NA NA NA	No No No No	NA NA NA NA NA	NA NA NA NA	18" 24GA UD CLIPS 18" 22GA UD 18" 22GA UD CLIPS 12" 24GA UD 12" 24GA UD 12" 22GA UD	1.32 1.66 1.66 1.48 1.86	70.23 133.46 86.66 98.80	329.90 329.90 340.00 494.70	Yes Yes Yes Yes	Yes Yes Yes Yes	No Yes Yes Yes	NA NA NA NA NA NA NA NA	NA NA NA NA	No Yes No No	NA NA NA	NA NA NA
18" 24GA UD CLIPS 18" 22GA UD 18" 22GA UD CLIPS 12" 24GA UD 12" 22GA UD 22GA UD 22GA UD 224 22GA UD	1.32 1.66 1.66 1.48 1.86 1.23	74.09 55.36 99.79 69.33 88.40 50.01	170.00 239.30 239.30 255.00 335.10 127.50	Yes Yes Yes Yes Yes Yes	Yes Yes Yes Yes Yes No	No No Yes No Yes No	NA NA NA NA NA	NA NA NA NA NA	NA NA NA NA NA	No No No No No	NA NA NA NA NA	NA NA NA NA NA	18" 24GA UD CLIPS 18" 22GA UD 18" 22GA UD CLIPS 12" 24GA UD 12" 22GA UD 22GA UD 22GA UD 24" 24GA DL	1.32 1.66 1.66 1.48 1.86 1.23	70.23 133.46 86.66 98.80 59.17	329.90 329.90 340.00 494.70 170.00	Yes Yes Yes Yes Yes	Yes Yes Yes Yes Yes	NoYesYesYesNo	NANANANANANANANANANA	NA NA NA NA	No Yes No No No	NA NA NA NA	NA NA NA NA
18" 24GA UD CLIPS 18" 22GA UD 18" 22GA UD CLIPS 12" 24GA UD 12" 22GA UD 24" 24GA DL 24" 24GA DL CLIPS	1.32 1.66 1.66 1.48 1.86 1.23 1.23	74.09 55.36 99.79 69.33 88.40 50.01 87.17	170.00 239.30 239.30 255.00 335.10 127.50 127.50	Yes Yes Yes Yes Yes Yes Yes	Yes Yes Yes Yes No Yes	No No Yes No Yes No Yes	NA NA NA NA NA NA	NA NA NA NA NA NA	NA NA NA NA NA NA	No No No No No No	NA NA NA NA NA NA	NA NA NA NA NA	18" 24GA UD CLIPS 18" 22GA UD 18" 22GA UD CLIPS 12" 24GA UD 12" 22GA UD 24" 24GA DL 24" 24GA DL CLIPS	1.32 1.66 1.66 1.48 1.86 1.23 1.23	70.23 133.46 86.66 98.80 59.17 113.22	329.90 329.90 340.00 494.70 170.00 170.00	Yes Yes Yes Yes Yes Yes	Yes Yes Yes Yes Yes Yes	No Yes Yes Yes No Yes	NANANANANANANANANANANANA	NA NA NA NA NA	No Yes No No No	NA NA NA NA NA	NA NA NA NA
18" 24GA UD CLIPS 18" 22GA UD 18" 22GA UD CLIPS 12" 24GA UD 12" 22GA UD 24" 24GA DL 24" 24GA DL CLIPS 18" 22GA LOKSEAM	1.32 1.66 1.66 1.48 1.86 1.23 1.23 1.60	74.09 55.36 99.79 69.33 88.40 50.01 87.17 41.60	170.00 239.30 239.30 255.00 335.10 127.50 127.50 81.80	Yes Yes Yes Yes Yes Yes Yes	Yes Yes Yes Yes No Yes No	NoNoYesNoYesNoYesNo	NA NA NA NA NA NA NA	NA NA NA NA NA NA NA	NA NA NA NA NA NA NA	No No No No No No No	NA NA NA NA NA NA NA	NA NA NA NA NA NA	18" 24GA UD CLIPS 18" 22GA UD 18" 22GA UD CLIPS 12" 24GA UD 12" 22GA UD 24" 24GA DL 24" 24GA DL CLIPS 18" 22GA LOKSEAM	1.32 1.66 1.66 1.48 1.86 1.23 1.23 1.60	70.23 133.46 86.66 98.80 59.17 113.22 46.80	329.90 329.90 340.00 494.70 170.00 170.00 145.30	Yes Yes Yes Yes Yes Yes	Yes Yes Yes Yes Yes No	NoYesYesYesNoYesNo	NANANANANANANANANANANANANANANANA	NA NA NA NA NA NA	No Yes No No No No	NA NA NA NA NA	NA NA NA NA NA
18" 24GA UD CLIPS 18" 22GA UD 18" 22GA UD CLIPS 12" 24GA UD 12" 22GA UD 24" 24GA UD 24" 24GA DL 24" 24GA DL CLIPS 18" 22GA LOKSEAM 16" 24GA LOKSEAM	1.32 1.66 1.66 1.48 1.86 1.23 1.23 1.60 1.34	74.09 55.36 99.79 69.33 88.40 50.01 87.17 41.60 33.22	170.00 239.30 255.00 335.10 127.50 127.50 81.80 64.90	Yes Yes Yes Yes Yes Yes Yes No	Yes Yes Yes Yes No Yes No No	NoNoYesNoYesNoYesNoNoNoNo	NA NA NA NA NA NA NA NA	NA NA NA NA NA NA NA NA	NA NA NA NA NA NA NA NA	No No No No No No No No	NA NA NA NA NA NA NA NA	NA NA NA NA NA NA NA	18" 24GA UD CLIPS 18" 22GA UD 18" 22GA UD CLIPS 12" 24GA UD 12" 22GA UD 24" 24GA DL 24" 24GA DL CLIPS 18" 22GA LOKSEAM 16" 24GA LOKSEAM	1.32 1.66 1.66 1.48 1.86 1.23 1.23 1.60 1.34	70.23 133.46 86.66 98.80 59.17 113.22 46.80 36.33	329.90 329.90 340.00 494.70 170.00 170.00 145.30 115.30	Yes Yes Yes Yes Yes Yes No	Yes Yes Yes Yes Yes No No	NoYesYesNoYesNoNoNo	NA	NA NA NA NA NA NA NA	No Yes No No No No No No	NA NA NA NA NA NA	NA NA NA NA NA NA
18" 24GA UD CLIPS 18" 22GA UD 18" 22GA UD CLIPS 12" 24GA UD 12" 22GA UD 24" 24GA DL 24" 24GA DL CLIPS 18" 22GA LOKSEAM 16" 22GA LOKSEAM 16" 22GA LOKSEAM	1.32 1.66 1.48 1.86 1.23 1.60 1.34 1.71	74.09 55.36 99.79 69.33 88.40 50.01 87.17 41.60 33.22 56.05	170.00 239.30 255.00 335.10 127.50 127.50 81.80 64.90 91.30	Yes Yes Yes Yes Yes Yes Yes No Yes	Yes Yes Yes Yes No Yes No Yes	No No Yes No Yes No Yes No No No No No	NA NA NA NA NA NA NA NA NA	NA NA NA NA NA NA NA NA NA	NA NA NA NA NA NA NA NA NA	No No No No No No No No No	NA NA NA NA NA NA NA NA NA	NA NA NA NA NA NA NA NA	18" 24GA UD CLIPS 18" 22GA UD 18" 22GA UD CLIPS 12" 24GA UD 12" 22GA UD 24" 24GA DL 24" 24GA DL CLIPS 18" 22GA LOKSEAM 16" 22GA LOKSEAM 16" 22GA LOKSEAM	1.32 1.66 1.48 1.86 1.23 1.60 1.34 1.71	70.23 133.46 86.66 98.80 59.17 113.22 46.80 36.33 57.08	329.90 329.90 340.00 494.70 170.00 170.00 145.30 115.30 162.40	Yes Yes Yes Yes Yes Yes Yes Yes	Yes Yes Yes Yes Yes Yes No No Yes	NoYesYesNoYesNoNoNoNo	NA	NA NA NA NA NA NA NA NA	No Yes No No No No No No No	NA NA NA NA NA NA NA	NA NA NA NA NA NA NA
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Roof Span Comparison

Received 8.9.23

RCCF PITC	HTABL
RISE (in/ft	
0.250	1.19
0.375	1.79
0.500	239
0.625	298
0.750	3.58
0.875	4.17
1.000	4.76
1.125	56.36
1.250	595
1.375	6.54
1.500	7.13
1.625	7.71
1.750	8.30
1.875	8.88
2000	9.46
2125	10.04
2250	10.04
2375	11.20
2500	11.20
2625	
2750	1234
2/30	1291
	13.4/
3.000	14.04
3,250	15.15
3.375	15.71
3.500	16.26
3.625	16.81
3.750	
3.875	17.90
4.000	
4.125	18.97
4.250	19.50
4.375	20.03
4,500	20.56
4.625	21.08
4.750	21.60
4.875	2211
5.000	2262
5.125	23.13
5.250	2363
5.375	24.13
5.500	24.62
5.625	25.11
5.750	25.6
5.875	26.09
6.000	26.57

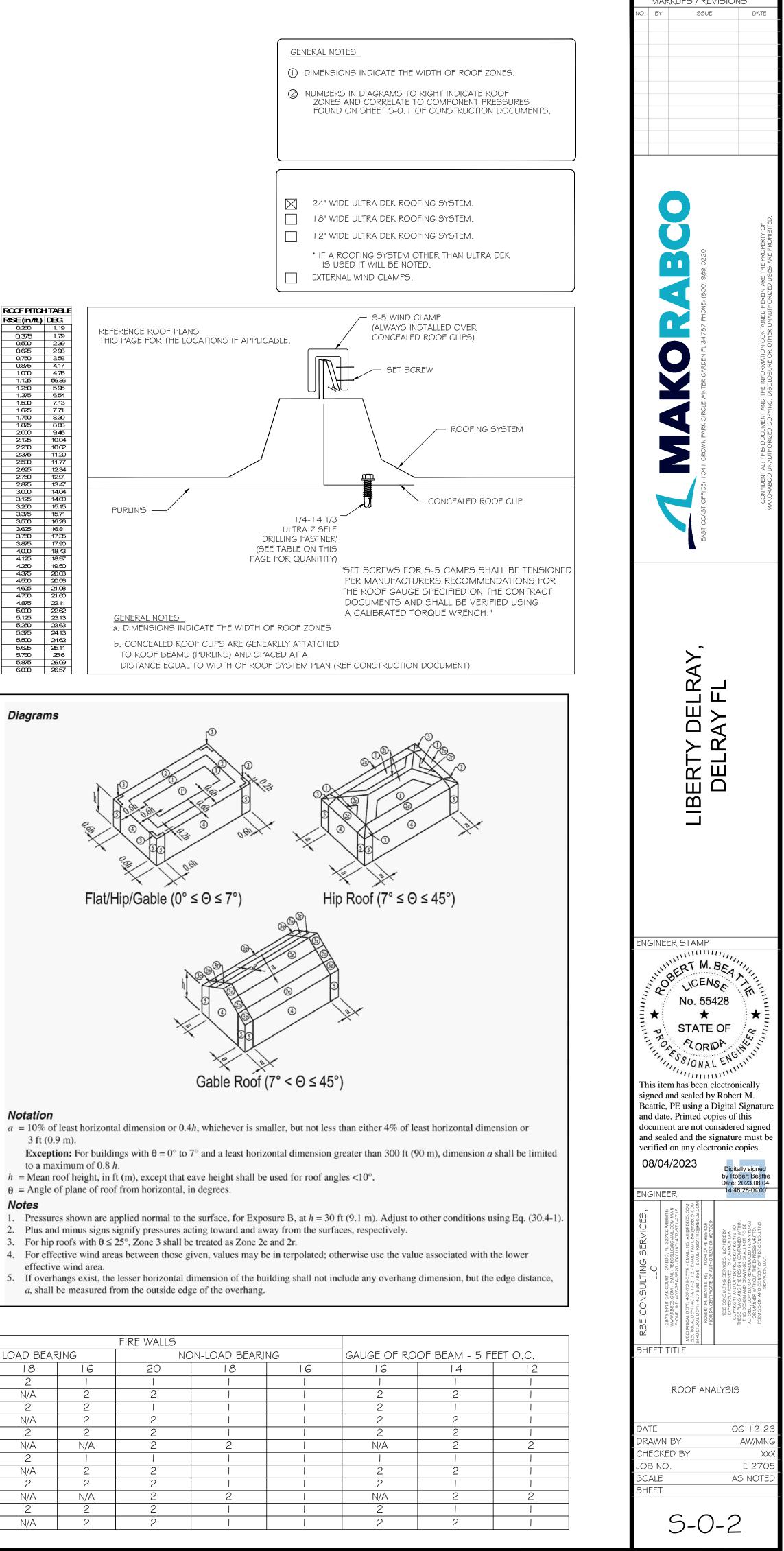
Diagrams

٧	ot	ation	
ı	=	10% of le	
		3 ft (0.9)	1

3 ft (0.9 m).

Notes

TYPICAL ULTRA DEK ROOFING SYSTEM		
(SEE ELEVATIONS FOR ROOFING SYSTEM BY STRUCTURE)	LOAD BEAR	RING
	18	
24" 26GA UD	2	
24" 26GA UD WITH EXTERNAL WIND CLAMPS	N/A	
24" 24GA UD	2	
24" 24GA UD WITH EXTERNAL WIND CLAMPS	N/A	
24" 22GA UD	2	
24" 22GA UD WITH EXTERNAL WIND CLAMPS	N/A	
18" 24GA UD	2	
18" 24GA UD WITH EXTERNAL WIND CLAMPS	N/A	
18" 22GA UD	2	
18" 22GA UD WITH EXTERNAL WIND CLAMPS	N/A	
12" 24GA UD	2	
12" 22GA UD	N/A	



sustainable (2) insulation.

Specification Sheet

Metal Building Insulation 202-96

Composition and Materials: The product is composed of tan, uniformly textured, inorganic fibrous

pre-engineered metal buildings. It should be laminated on a first-in, first-out basis and should be kept.

dry at all times during processing and end use. After lamination, packaging should not exceed a 5.5:1

compression ratio. For additional information, please refer to the appendix of the NAIMA 202-96 (Rev.

Sizes: Standard available sizes as noted in table below. Contact CertainTeed for non-standard sizes.

Limitations: This product is designed for use in interior (weather protected) walls and roots of

PRODUCT DESCRIPTION

2000) standard.

INSTALLATION

21* 6% 152.4

Basic Use CertainTeed Fiber Glass Metal Building Insulation 202-96 is a flexible blanket insulation furnished in rolls and intended to be laminated on one side with a suitable vapor netarder. It is used as a thermal and acoustical insulation in the roofs and sidewalls of pre-engineered metal buildings and post frame construction.

Benefits: Metal Building Insulation 202-96 reduces transmission of exterior sound to the interior of the building and absorbs reverberating sounds within the building.

glass and formed with a formaldehyde-free binding agent.



THERMAL PERFORMANCE

36,48,60,72 914,1219,1524,1829 46 13.7

Nominal Thickness

CertainTeed Metal Building Product Name Insulation 202-96 CertainTeed Corporation Manufacturer P.O. Box 860 Address Valley Forge, PA 19482-0105 Phone 610-341-7000 + 800-233-8990 Fax 610-341-7571

www.certainteed.com/insulation

Taineeo Saint-gobain

TECHNICAL DATA

Website

Applicable Standards

 Model Building Codes: -100

- Material Standards:
- AST M C991, Type I - NAIMA 202-96 (Rev. 2000) Fire Resistance

Fire Hazard Classification:

- UL 723, AST M E84, NFPA 255 Max. Flame Spread Index: 25 Max. Smoke Developed Index: 50 - CAN/ULC-S102-M88
- Non-combustible:

- ASTM E136 / Meets requirements Physical/Chemical Properties

- Thermal Resistance:
- ASTM C518 and/or ASTM C177 at 75°F (24°C) mean temperature: see table at left Acoustical Performance: see tables on other side
- Water Vapor Sorption: - ASTM C1104 / No greater than 5.0% by weight
- Corrosiveness: - ASTM C665 / Meets requirements for steel, copper and aluminum
 - Odor Emission:
 - ASTM C1304 / Pass
 - Fungi Resistance: - AST M C1338 / Pass Test
 - Quality Assurance

CertainTeed's commitment to quality and environmental management has ensured the registration of the Athens, Chowchilla and Kansas City plants to ISO 9001:2000 and ISO 14001: 2004 standards.

	R-Value									
conditioned spaces in the building. The lation is normally applied over or betwee structural members of the building and										
							11 1.9			
eld in place by the covering sheets or isulation support system. When using high I-Value systems, it is recommended that the										
					wity between the exterior metal sheet and le faced fiber glass insulation should be omoletely filled.					
Value	length									
R	ft.									
10	29 100 ;									
1000	<mark>29 100 :</mark>									
11										
11 13	29 75 2									
	99 76 : 99 50 :									
faced fib pletely f Value										

30° 9¼ 203.2 36,48,60,72 914,1219,1624,1829 26 7.6 Non-standard widths are available and subject to an upchange on an individual basis determined by manufacturer's capability, quantity, lead times and packaging availability. "R-21, R-25 and R-30 are made to order.

25 8 203.2 36,48,60,72 914,1219,1524,1829 30 9.1

Received 8.9.23

www.certainteed.com/insulation/hvac-mechanical

AVAILABILITY AND COST

Manufactured and sold throughout the United State and Canada. For availability and cost, contact your local distributor or call CertainTeed Sales Support Group in Valley Forge, PA at 800-233-8990.

WARRANTY

In as much as CertainTeed has no control over installation design, installation workmanship, accessory materials or conditions of application, CertainTeed does not warrant the performance or results of any installation containing its products.

MAINTENANCE

An inspection and preventative maintenance program for the insulation and vapor retarder system is recommended to ensure optimum performance.

TECHNICAL SERVICES

Technical assistance can be obtained either from the local CertainTeed sales representative, or by calling CertainTeed Sales Support Group in Valley Forge, PA at 800-233-8990.

FILING SYSTEMS CertainTeed Pub. No. 30-25-056.

 Additional product information available upon request.

			ACED	FION - UNF	D ABSURP	SUUN								
NIDO	(Hz)	requencies	ctave Band F	ickness	Nom. Th	R-Value								
NRC	4000	2000	1000	500	250	125	mm	in.	R					
0.95	0.98	0.96	0.94	1.02	0.82	0.29	86	3%	10					
0.95	0.98	0.93	0.92	1.01	0.91	0.39	95	3¾	11					
0.95	0.98	0.95	0.90	1.04	0.97	0.53	111	43%	13					
1.00	0.99	0.98	0.92	1.02	1.05	0.67	133	5¼	16					
1.05	1.00	1.01	0.98	1.02	1.22	0.89	162	6%	19					

SOUND TRANSMISSION

	-	ransmissior	Loss in dB	at the Octave	e Frequencie	s	STC	
Construction Type	125	250 500		1000	2000	4000	Rating	
		RO	0FS					
No Insulation	12	13	19	24	30	32	24	
R-10 Faced 202-96 Insulation Over the Purlins	12	16	26	37	45	49	29	
R-19 Faced 202-96 Insulation Over the Purlins	13	20	30	41	49	51	32	
202-96 Insulation Over & Between the Purlins to Fill the Cavity (R-25 Combined)	14	24	34	44	53	56	36	
		WA	LLS					
No Insulation	12	14	19	19	20	27	21	
R-10 Faced 202-96 Insulation Over the Girts	13	16	25	32	37	46	28	
R-13 Faced 202-96 Insulation Over the Girts	13	17	26	33	38	47	29	
R-13 Faced 202-96 Insulation Over the Girts 3-5/8" Steel Studs on 24" Centers with 1/2" Gyp. Board on Interior	26	40	51	60	64	65	50	
R-13 Faced 202-96 Insulation Over the Girts 3-5/8 " Steel Studs on 24" Centers with R-11 Batts & 1/2 " Gyp. Board on Interior	31	43	55	68	73	75	54	

Sound Transmission Class (STC) in accordance with ASTM E90. - Roof construction is 24ga. standing seam roof with 8" Z purlins on 5' centers. Wall construction is 2.9a, wall panels screwed to 8" Z girts placed on 7" centers.
 Interior metal furring wall studs were 3-5/8" by 25ga. on 24' centers.







ASK ABOUT OUR OTHER CERTAINTEED PRODUCTS AND SYSTEMS:

EXTERIOR: ROOFING • SIDING • WINDOWS • FENCE • RAILING • TRIM • DECKING • FOUNDATIONS • PIPE INTERIOR: INSULATION • GYPSUM • CEILINGS

CertainTeed Corporation P.O. Box 860

Professional: 800-233-8990 Consumer: 800-782-8777 Valley Forge, PA 19482 www.certainteed.com/insulation





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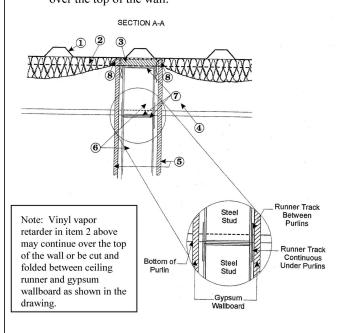
Insurance Bulletin 19

Fire Testing of Interior Separation Wall Joint

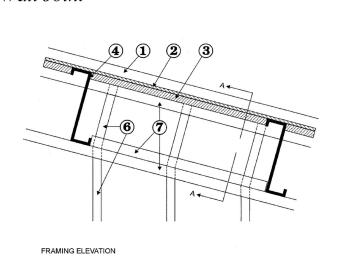
This test program was undertaken by the Metal Building Manufacturers Association and tested at Omega Point Laboratories to evaluate the fire performance of the junction between a one-hour fire rated wall assembly and the non-combustible metal roof panels. (Ref. Omega Point Project No. 16343-108145).

The fire exposure used to evaluate the joint was the timetemperature curve in ASTM E 119 for fire rated walls. There is no test method for evaluating the fire integrity of the joint where a fire rated wall meets the roof insulation under non-combustible metal roof panels. Also, building codes do not require this joint to be fire rated. This bulletin serves to answer questions raised by building and fire code officials concerning fire spread over the top of the fire rated wall via the vinyl vapor retarder.

The test was performed in the worst-case orientation, with purlins framed perpendicular to the wall framing of the one-hour wall. Therefore, the same performance can be expected with the wall framed in any orientation with respect to the purlins. The test also showed there is no need to cut the vapor retarder, so it can be continuous over the top of the wall.



Interior Separation Wall Joint



Note: Gypsum Wallboard not shown. Fire caulking shall fill all openings where gypsum wallboard fits around purlin penetrations.

The fire test was terminated at 1 hour and forty-one minutes, during which time no flaming occurred on the unexposed surface of the wall or roof at the joint. The construction details that successfully prevented flaming over the top of the wall in the test are shown below.

- 1. Metal Roof Panels No. 26 MSG min. galvanized, Galvalume®, or painted steel. 2. Batts or Blankets – Vinyl faced compressible glass fiber
- insulation weighing between 0.6 and 0.7 pcf. Installed at the bottom side of roof deck panels over top of purlins. Note: Fiberglass insulation with alternate facing materials can be used, if flame spread is less than or equal to 25, and has a smoke developed rating of less than or equal to 50 per ASTM E84. 3. Mineral Wool Batts – 2 inches thick, 8-9 pcf density, fire
- stop across top of wall. 4. Steel Roof Purlin - C or Z-shaped, minimum 8 inches deep,
- No. 16 MSG min. galvanized or painted steel. 5. Wallboard, gypsum - 5/8 inch thick, Type X gypsum wallboard.
- 6. Steel Studs Channel shaped min. 2 ¹/₂ inch wide, 1 ¹/₄ inch flanges, and 1/4 inch return steel studs of No. 25 MSG min. steel, spaced 24 inches O.C. max. (Ref. UL Design No. U425).
- 7. Floor and Ceiling Runners Channel shaped of 25 MSG min. steel with min. 1-inch long legs to accommodate main wall studs. Runner at the top of the wall attached to the bottom of the purlins. Inverted runner attached to the top of the wall runner between purlins in order to support the short length of steel studs capped with a runner to compress the mineral wool batts and the glass fiber insulation under the roof panels.
- 8. Fire Caulking Bead of fire caulk along the joint formed by the gypsum wallboard and the glass fiber insulation applied along the top of the wall and on both sides of the wall in accordance with the manufacturer's specifications. Fire caulking of all openings between the gypsum wallboard and the purlins on both sides of the wall.

	high performance fabrics		
ALPHA	ASSOCIATES, INC. • 145 LEHIGH AVE. •		
2002/01/17/2005	T: 732.634.5700 • 800.631.5399 • F: VR-R PLUS	732.634.1430	
DESCRIPTION Rein	nforced Polypropylene Film – Metalized P	olyester Film Laminate	
APPLICATIONS Met	tal Building Insulation Facing		
CERTIFICATIONS Med	ets all criteria for ASTM C1136 Types II, IV	UL 723/ASTM E84, ULC-	S102M fire testing
	COMPOSITION		
COMPONENT	DECODIDUON	VALUES*	
White Film	<u>DESCRIPTION</u> Polypropylene	<u>English</u> 0.0011 inch	<u>Metric</u> 28.0 micron
Adhesive	Flame Resistant	0.0011 men	28.0 1110101
Reinforcement	Tri-directional Fiberglass	4/inch (MD)	20/100 mm (MD)
		4/inch (CD)	8/100 mm (MD)
Film	Polyester	0.0005 inch	12.7 micron
	TYPICAL PHYSICAL PROF	PERTIES	
		VALUES*	
CHARACTERISTIC	METHOD	<u>ENGLISH</u>	METRIC
BASIS WEIGHT	Scale	13.2 lbs/1000 SF	64.1 g/SM
THICKNESS	ASTM-D-1777	0.007"	178 microns
PERMEABILITY (WVTR)	ASTM-E-96	0.02 Perms	1.15 ng/N's
BURST STRENGTH	ASTM-D-774	100 psi	7.0 kg/cm ²
PUNCTURE RESISTANCE	ASTM-C-1136	300 units (79 in-lbs)	9.0 Joules
TENSILE STRENGTH	ASTM-C-1136	30 lbs/inch MD 30 lbs/inch CD	5.25 kN /m 5.25 kN /m
ACCELERATED AGING	30 days @ 95% RH, 120 °F (49°C)	No Corrosion No Delamination	No Corrosion No Delamination
TEMPERATURE RESISTANC	E	-40 °F to 220 °F	-40 °C to 104 °C
WATER IMMERSION	24 hours @ 73°F (23°C)	No Delamination	No Delamination
MOLD RESISTANCE	ASTM-C-665	No Growth	No Growth
LIGHT REFLECTIVITY	ASTM-C-523	85%	85%
	Physical properties are based upon nominal values.	Weight / Thickness +/- 10%	
FIRE TESTING		TM E84/CAN ULC-5102M	
	FLAME SPREAD	<u>Composite</u> 25	
		25	

intense exposure to UV sources such as direct or indirect sunlight and/or high-output UV lighting is not recommended as these will degrade the plastic film facing.

NO.	BY			EAST COAST OFFICE: TO 4 L CROWN PARK CIRCLE WINTER GARDEN FL 34787 PHONE: (800)-989-0220		THE INFORMATION CONTAINED HEREIN ARE THE PROPERTY OF DISCLOSURE OR OTHER UNAUTHORIZED USES ARE PROHIBITED.
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	STATEMENT OF S			-					
	TIAL INSPECTIONS SHALL BE A CTION OR TESTING FOR SEISMI								
	SCHEDULE OF SPEC	AL I	NSPECTION SER	VICES					
PROJECT									
			AS APPLICABLE TO						
MATERIAL / ACTIVITY 1704.2.5 Inspection of	SERVICE	Y/N	EXTENT	AGENT*	DATE COMPLETED				
Fabricators									
Verify fabrication/quality control procedures	In-plant review (3)	N	Periodic						
1705.1.1 Special Cases (work unusual in nature, including but not limited to alternative materials and systems, unusual design applications, materials and systems with special manufacturer's requirements)	Submittal review, shop (3) and/or field inspection	N							
1705.2 Steel Construction									
1. Fabricator and erector documents Verify reports and certificates as listed n AISC 360, chapter N, paragraph 3.2 for compliance with construction documents)	Submittal Review	N	Each submittal						
2. Material verification of structural steel	Shop (3) and field inspection	Y	Periodic						
3. Embedments (Verify diameter, grade, type, length, embedment. See 1705.3 for anchors)	Field inspection	Y	Periodic						
 Verify member locations, braces, stiffeners, and application of joint details at each connection comply with construction documents 	Field inspection	Y	Periodic						
5. Structural steel welding:					м М				
a. Inspection tasks Prior to Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-1)	Shop (3) and field inspection	Y	Observe or Perform as noted (4)						
b. Inspection tasks During Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-2)	Shop (3) and field inspection	Y	Observe (4)						
c. Inspection tasks After Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-3)	Shop (3) and field inspection	Y	Observe or Perform as noted (4)						
d. Nondestructive testing (NDT) of welded joints: see Commentary		Y							
 Complete penetration groove welds 5/16" or greater in risk category III or IV 	Shop (3) or field ultrasonic testing - 100%	Y	Periodic						
 Complete penetration groove welds 5/16" or greater in risk category II 	Shop (3) or field ultrasonic testing - 10% of welds minimum	Y	Periodic						
 Thermally cut surfaces of access holes when material t > 2" 	Shop (3) or field magnetic Partical or Penetrant testing	Y	Periodic						
4) Welded joints subject to fatigue when required by AISC 360, Appendix 3, Table A-3.1	Shop (3) or field radiographic or Ultrasonic testing	Y	Periodic						
5) Fabricator's NDT reports when fabricator performs NDT	Verify reports	Y	Each submittal (5)						
 Structural steel bolting: a. Inspection tasks Prior to Bolting (Observe, or perform tasks for each bolted connection, in accordance with QA tasks listed in AISC 360, Table N5.6-1) 	Shop (3) and field inspection	Y	Observe or Perform as noted (4)						

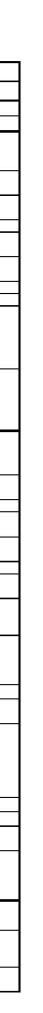
	SCHEDULE OF SPEC	AL II	NSPECTION SER	RVICES	
PROJECT					
MATERIAL / ACTIVITY	SERVICE	Y/N	EXTENT	AGENT*	DATE COMPLETED
3. Inspection of anchors cast in concrete where allowable loads have been increased per section 1908.5 or where strength design is used	Shop (3) and field inspection	Y	Periodic		
4. Inspection of anchors and reinforcing steel post-installed in hardened concrete: Per research reports including verification of anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, concrete minimum thickness, anchor embedment and tightening torque	Field inspection	Y	Periodic or as required by the research report issued by an approved source		
5. Verify use of approved design mix	Shop (3) and field inspection	Y	Periodic		
6. Fresh concrete sampling, perform slump and air content tests and determine temperature of concrete	Shop (3) and field inspection	Y	Continuous		
7. Inspection of concrete and shotcrete placement for proper application techniques	Shop (3) and field inspection	Y	Continuous		
8. Inspection for maintenance of specified curing temperature and techniques	Shop (3) and field inspection	Y	Periodic		
9. Inspection of prestressed concrete:	Shop (3) and field inspection	NA			
a. Application of prestressing force		NA	Continuous		
 Brouting of bonded prestressing tendons in the seismic-force-resisting system 		NA	Continuous		
10. Erection of precast concrete members		NA		-	
a. Inspect in accordance with construction documents	Field inspection	NA	In accordance with construction documents		
 b. Perform inspections of welding and bolting in accordance with Section 1705.2 	Field inspection	NA	In accordance with Section 1705.2		
11. Verification of in-situ concrete strength, prior to stressing of tendons in post tensioned concrete and prior to removal of shores and forms from beams and structural slabs	Review field testing and laboratory reports	NA	Periodic		
12. Inspection of formwork for shape, lines, location and dimensions	Field inspection	Y	Periodic		
13. Concrete strength testing and verification of compliance with construction documents	Field testing and review of laboratory reports	Y	Periodic		
1705.4 Masonry Construction					
(A) Level A, B and C Quality Assurance:					
1. Verify compliance with approved submittals	Field Inspection	Y	Periodic		
(B) Level B Quality Assurance:					
1. Verification of fm and f_{AAC} prior to construction	Testing by unit strength method or prism test method	Y	Periodic		

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1	SCHEDULE OF SPEC	IAL I	NSPECTION SER	VICES	
PROJECT					
			AS APPLICABLE TO		
MATERIAL / ACTIVITY	SERVICE	Y/N	EXTENT	AGENT*	DATE COMPLETED
b.Inspection tasks During Bolting (Observe the QA tasks listed in AISC 360, Table N5.6-2)		Y	Observe (4)		
 Pre-tensioned and slip-critical joints 		Y			
a) Turn-of-nut with matching markings		Y	Periodic		
b) Direct tension indicator		Y	Periodic		
c) Twist-off type tension control bolt		Y	Periodic		
d) Turn-of-nut without matching markings		Y	Continuous		
e) Calibrated wrench		Y	Continuous		
2) Snug-tight joints		Y	Periodic		
c. Inspection tasks After Bolting					
(Perform tasks for each bolted connection in accordance with QA tasks listed in AISC 360, Table N5.6- 3)		Y	Perform (4)		
7. Inspection of steel elements of		1	с		
composite construction prior to concrete placement in accordance with QA tasks listed in AISC 360, Table N6.1	Shop (3) and field inspection and testing	N	Observe or Perform as noted (4)		
1705.2.2 Steel Construction Other Than Structural Steel					
1. Material verification of cold-formed steel deck:					
a. Identification markings	Field inspection	Y	Periodic		
b. Manufacturer's certified test reports	Submittal Review	Y	Each submittal		
2. Connection of cold-formed steel deck to supporting structure:	Shop (3) and field inspection				
a. Welding		NA	Periodic		
b. Other fasteners (in accordance with AISC 360, Section N6)		Y			
 Verify fasteners are in conformance with approved submittal 		Y	Periodic		
 Verify fastener installation is in conformance with approved submittal and manufacturer's recommendations 		Y	Periodic		
3. Reinforcing steel	Shop (3) and field inspection				
a. Verification of weldability of steel other than ASTM A706		N	Periodic		
 b. Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames, boundary elements of special concrete structural walls and shear reinforcement 		N	Continuous		
c. Shear reinforcement		Y	Continuous		
d. Other reinforcing steel		Ν	Periodic		
4. Cold-formed steel trusses spanning 60 feet or greater		N			
a. Verify temporary and permanent restraint/bracing are installed in accordance with the approved truss submittal package	Field inspection	N	Periodic		
1705.3 Concrete Construction					
1. Inspection of reinforcing steel installation (see 1705.2.2 for welding)	Shop (3) and field inspection	Y	Periodic		
2. Inspection of prestressing steel installation	Shop (3) and field inspection	NA	Periodic		

PROJECT					
FROJECT			AS APPLICABLE TO	THIS PROJECT	
MATERIAL / ACTIVITY	SERVICE	Y/N	EXTENT	AGENT*	DATE COMPLETED
C) Level C Quality Assurance:					
1. Verification of fm and f _{AAC} prior to construction and for every 5,000 SF during construction	Testing by unit strength method or prism test method	N	Periodic		
2. Verification of proportions of materials in premixed or preblended mortar, prestressing grout, and grout other than self- consolidating grout, as delivered to the project site	Field inspection	N	Continuous		
3. Verify placement of masonry units	Field Inspection	N	Periodic		
D) Levels B and C Quality Assurance:					
1. Verification of Slump Flow and Visual Stability Index (VSI) of self- consolidating grout as delivered to the project	Field testing	Y	Continuous		
2. Verify compliance with approved submittals	Field inspection	Y	Periodic		
 Verify proportions of site-mixed mortar, grout and prestressing grout for bonded tendons 	Field Inspection	Y	Periodic		
 Verify grade, type, and size of reinforcement and anchor bolts, and prestressing tendons and anchorages 	Field Inspection	Y	Periodic		
5. Verify construction of mortar joints	Field Inspection	Y	Periodic		
 Verify placement of reinforcement, connectors, and prestressing tendons and anchorages 	Field Inspection	Y	Level B - Periodic		
		N	Level C - Continuous		
7. Verify grout space prior to grouting	Field Inspection	Y NA	Level B - Periodic Level C - Continuous		с С
8. Verify placement of grout and prestressing grout for bonded tendons	Field Inspection	N	Continuous		
9. Verify size and location of structural masonry elements	Field Inspection	Y	Periodic		
10. Verify type, size, and location of anchors, including details of anchorage of masonry to structural members, frames, or other construction.	Field inspection	Y	Level B - Periodic		
		N	Level C - Continuous		
11. Verify welding of reinforcement (see 1705.2.2)	Field inspection	N	Continuous		
12. Verify preparation, construction, and protestion of masonry during cold weather (temperature below 40°F) or hot weather (temperature above 90°F)	Field inspection	Y	Periodic		
13. Verify application and measurement of prestressing force	Field Inspection	N	Continuous		

			SPECTION SER				
PROJECT	AS APPLICABLE TO THIS PROJECT						
MATERIAL / ACTIVITY	SERVICE	Y/N	EXTENT	AGENT*	DATE COMPLETED		
	OERVICE		EATERI	AGENT	DATE COMIT LETED		
14. Verify placement of AAC masonry units and construction of thin-bed mortar joints (first 5000 SF of AAC masonry)	Field inspection	N	Continuous				
15. Verify placement of AAC masonry units and construction of thin-bed mortar joints (after the first 5000 SF of AAC masonry)	Field inspection	N	Level B - Periodic				
		N	Level C - Continuous				
16. Verify properties of thin-bed mortar for AAC masonry (first 5000 SF of AAC masonry)	Field inspection	N	Continuous				
17. Verify properties of thin-bed mortar forAAC masonry (after the first 5000 SF of AAC masonry)	Field inspection	N	Level B - Periodic				
		Ν	Level C - Continuous				
18. Prepare grout and mortar specimens	Field testing	Y	Level B - Periodic				
opeaniene		N	Level C - Continuous				
19. Observe preparation of prisms	Field inspection	Y	Level B - Periodic				
		N	Level C - Continuous				
705.5 Wood Construction							
. Inspection of the fabrication process of wood structural elements and issemblies in accordance with Section 704.2.5	In-plant review (3)	N	Periodic				
P. For high-load diaphragms, verify grade and thickness of structural panel heathing agree with approved building plans	Field inspection	N	Periodic				
b. For high-load diaphragms, verify nominal size of framing members at adjoining panel edges, nail or staple liameter and length, number of astener lines, and that spacing between fasteners in each line and at adge margins agree with approved building plans	Field inspection	N	Periodic				
Metal-plate-connected wood trusses panning 60 feet or greater: verify emporary and permanent estraint/bracing are installed in accordance with the approved truss submittal package	Field inspection	N	Periodic				
. Verify materials below shallow							
bundations are adequate to achieve the design bearing capacity.	Field inspection	Y	Periodic				
. Verify excavations are extended to roper depth and have reached proper naterial.	Field inspection	Y	Periodic				
. Perform classification and testing of ontrolled fill materials.	Field inspection	Y	Periodic				
. Verify use of proper materials, ensities, and lift thicknesses during lacement and compaction of ontrolled fill	Field inspection	Y	Continuous				
. Prior to placement of controlled fill, bserve subgrade and verify that site as been prepared properly	Field inspection	Y	Periodic				



NO. BY	ARKUP	'S / RE ISSUE	VISION	IS date
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RBE CONSULTING SERVICES, DI THE LLC		MICHANICIA DET1.407-617-61.216.EMML: INHIBIAGREES.COM ELECTRICAL DET1.407-617-3113 - EMML: FAMILIBIAGREES.COM STRUCTURAL DET1:407-568-7656 - EMAL: FREATTIE@REECS.COM RODERT M. EBATTE, FRE. FLOEDA FF #55428 FLOEDARTM, EPATTE, FREATTE, FREATTIER & FEATURE	THE CONTRACT AND A CO	THIS DESIGN AND PRAVINGS SHALL NOT TO BE THIS DESIGN AND PRAVINGS SHALL NOT TO BE ALTERD, COFFICIO, REFREADED IN ANY POSIN OR MANURE WITHOUT THE EXPRESS WRITTEN PERMISSION AND CONSENT OF PERE CONSULTING PERMISSION AND CONSENT OF PERE CONSULTING
DATE DRAW CHECI JOB N SCALE SHEET	KED B` O.		орести Орести А.	DN G-12-23 AW/MNG XXX E 2705 B NOTED

1	SCHEDULE OF SPECIAL INSPECTION SERVICES							
PROJECT	AS APPLICABLE TO THIS PROJECT							
	0500405	MAL						
MATERIAL / ACTIVITY	SERVICE	Y/N	EXTENT	AGENT*	DATE COMPLETED			
1705.7 Driven Deep Foundations								
I. Verify element materials, sizes and engths comply with requirements	Field inspection	N	Continuous					
2. Determine capacities of test elements and conduct additional load ests, as required	Field inspection	N	Continuous					
 Observe driving operations and naintain complete and accurate ecords for each element 	Field inspection	N	Continuous					
4. Verify placement locations and blumbness, confirm type and size of nammer, record number of blows per toot of penetration, determine required benetrations to achieve design capacity, record tip and butt elevations and document any damage to boundation element	Field inspection	N	Continuous					
5. For steel elements, perform additional inspections per Section 1705.2	See Section 1705.2	N	See Section 1705.2					
6. For concrete elements and concrete- illed elements, perform additional nspections per Section 1705.3	See Section 1705.3	N	See Section 1705.3					
7. For specialty elements, perform additional inspections as determined by the registered design professional in esponsible charge	Field inspection	N	In accordance with construction documents					
 Perform additional inspections and ests in accordance with the construction documents 	Field Inspection and testing	N	In accordance with construction documents					
1705.8 Cast-in-Place Deep Foundations								
. Observe drilling operations and naintain complete and accurate ecords for each element	Field inspection	N	Continuous					
2. Verify placement locations and olumbness, confirm element diameters, bell diameters (if applicable), lengths, embedment into pedrock (if applicable) and adequate end-bearing strata capacity. Record concrete or grout volumes	Field inspection	N	Continuous					
3. For concrete elements, perform additional inspections in accordance vith Section 1705.3	See Section 1705.3	N	See Section 1705.3					
I. Perform additional inspections and ests in accordance with the construction documents	Field Inspection and testing	N	In accordance with construction documents					
1705.9 Helical Pile Foundations								
. Verify installation equipment, pile dimensions, tip elevations, final depth, inal installation torque and other data as required.	Field inspection	N	Continuous					
2. Perform additional inspections and ests in accordance with the construction documents	Field Inspection and testing	N	In accordance with construction documents					

PROJECT	SCHEDULE OF SPEC					
FROJECT			AS APPLICABLE T	O THIS PROJECT		
MATERIAL / ACTIVITY	SERVICE	Y/N EXTENT AGENT* DATE COMPL				
1705.11.5 Architectural Components Special Inspections for Seismic Resistance						
 Inspection during the erection and fastening of exterior cladding and interior and exterior veneer 	Field inspection	N	Periodic			
Inspection during the erection and fastening of interior and exterior nonbearing walls	Field inspection	N	Periodic			
Inspection during anchorage of access floors	Field inspection	N	Periodic			
1705.11.6 Mechanical and Electrical Components Special Inspections for Seismic Resistance						
 Inspection during the anchorage of electrical equipment for emergency or standby power systems 	Field inspection	N	Periodic			
2. Inspection during the anchorage of other electrical equipment	Field inspection	N	Periodic			
 Inspection during installation and anchorage of piping systems designed to carry hazardous materials, and their associated mechanical units 	Field inspection	N	Periodic			
4. Inspection during the installation and anchorage of HVAC ductwork that will contain hazardous materials	Field inspection	N	Periodic			
5. Inspection during the installation and anchorage of vibration isolation systems	Field inspection	N	Periodic			
1705.11.7 Storage Racks Special Inspections for Seismic Resistance						
Inspection during the anchorage of storage racks 8 feet or greater in height	Field inspection	N	Periodic			
1705.11.8 Seismic Isolation Systems						
Inspection during the fabrication and installation of isolator units and energy dissipation devices used as part of the seismic isolation system	Shop and field inspection	N	Periodic			
1705.12.1 Concrete Reinforcement Testing and Qualification for Seismic Resistance						
1. Review certified mill test reports for each shipment of reinforcement used to resist earthquake-induced flexural and axial forces in reinforced concrete special moment frames, special structural walls, and coupling beams connecting special structural walls	Review certified mill test reports	N	Each shipment			

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	SCHEDULE OF SPEC	IAL IN	ISPECTION SEF	RVICES	
PROJECT					
			AS APPLICABLE TO	O THIS PROJECT	
MATERIAL / ACTIVITY	SERVICE	Y/N	EXTENT	AGENT*	DATE COMPLETED
1705.10.1 Structural Wood Special Inspections For Wind Resistance					
1. Inspection of field gluing operations of elements of the main windforce- resisting system	Field inspection	N	Continuous		
 Inspection of nailing, bolting, anchoring and other fastening of components within the main windforce- resisting system 	Shop (3) and field inspection	N	Periodic		
1705.10.2 Cold-formed Steel Special Inspections For Wind Resistance					
1.Inspection during welding operations of elements of the main windforce- resisting system	Shop (3) and field inspection	N	Periodic		
2.Inspections for screw attachment, bolting, anchoring and other fastening of components within the main windforce-resisting system	Shop (3) and field inspection	Y	Periodic		
1705.10.3 Wind-resisting Components					
1. Roof cladding	Shop (3) and field inspection	Ν	Periodic		
2. Wall cladding	Shop (3) and field inspection	N	Periodic		
1705.11.1 Structural Steel Special Inspections for Seismic Resistance					
Inspection of structural steel in accordance with AISC 341	Shop (3) and field inspection	N	In accordance with AISC 341		
1705.11.2 Structural Wood Special Inspections for Seismic Resistance					
1. Inspection of field gluing operations of elements of the seismic-force resisting system	Field inspection	N	Continuous		
 Inspection of nailing, bolting, anchoring and other fastening of components within the seismic-force- resisting system 	Shop (3) and field inspection	N	Periodic		
1705.11.3 Cold-formed Steel Light-Frame Construction Special Inspections for Seismic Resistance					
 Inspection during welding operations of elements of the seismic-force- resisting system 	Shop (3) and field inspection	N	Periodic		
 Inspections for screw attachment, bolting, anchoring and other fastening of components within the seismic-force- resisting system 	Shop (3) and field inspection	Y	Periodic		
1705.11.4 Designated Seismic Systems Verification					
Inspect and verify that that the component label, anchorage or mounting conforms to the certificate of compliance in accordance with Section 1705.12.3	Field inspection	N	Periodic		

	SCHEDULE OF SPEC		SPECTION SEF	RVICES	
PROJECT					
		N/AI	AS APPLICABLE T		
MATERIAL / ACTIVITY	SERVICE	Y/N	EXTENT	AGENT*	DATE COMPLETED
 Verify reinforcement weldability of ASTM A615 reinforcement used to resist earthquake-induced flexural and axial forces in reinforced concrete special moment frames, special structural walls, and coupling beams connecting special structural walls 	Review test reports	N	Each shipment		
1705.12.2 Structural Steel Testing and Qualification for Seismic Resistance					
Test in accordance with the quality assurance requirements of AISC 341	Shop (3) and field testing	N	Per AISC 341		
1705.12.3 Seismic Certification of Nonstructural Components					
Review certificate of compliance for designated seismic system components.	Certificate of compliance review	N	Each submittal		
1705.12.4 Seismic Isolation Systems					
Test seismic isolation system in accordance with ASCE 7 Section 17.8	Prototype testing	N	Per ASCE 7		
1705.13 Sprayed Fire-resistant Materials					
 Verify surface condition preparation of structural members 	Field inspection	N	Periodic		
 Verify application of sprayed fire- resistant materials 	Field inspection	N	Periodic		
 Verify average thickness of sprayed fire-resistant materials applied to structural members 	Field inspection	N	Periodic		
 Verify density of the sprayed fire- resistant material complies with approved fire-resistant design 	Field inspection and testing	N	Per IBC Section 1705.13.5		
5. Verify the cohesive/adhesive bond strength of the cured sprayed fire- resistant material	Field inspection and testing	N	Per IBC Section 1705.13.6		
1705.14 Mastic and Inturnescent Fire-Resistant Coatings					
Inspect mastic and intumescent fire- resistant coatings applied to structural elements and decks	Field inspection	N	Periodic		
1705.15 Exterior Insulation and Finish Systems (EIFS)					
 Verify materials, details and installations are per the approved construction documents 	Field inspection	N	Periodic		
 Inspection of water-resistive barrier over sheathing substrate 	Field inspection	N	Periodic		

PROJECT					
		100	AS APPLICABLE TO) THIS PROJECT	
MATERIAL / ACTIVITY	SERVICE	Y/N	EXTENT	AGENT*	DATE COMPLETED
1705.16 Fire-Resistant					
Penetrations and Joints					
1. Inspect penetration firestop systems	Field testing	Y	Per ASTM E2174		
2. Inspect fire-resistant joint systems	Field testing	Y	Per ASTM E2393		
1705.17 Smoke Control Systems					
1. Leakage testing and recording of device locations prior to concealment	Field testing	N	Periodic		
2. Prior to occupancy and after sufficient completion, pressure difference testing, flow measurements, and detection and control verification	Field testing	N	Periodic		
* INSPECTION AGENTS FIRM 1.			ADDRESS		TELEPHONE NO.
2.					
3.					
 Notes: 1. The inspection and testing agent(s) shall be tested. Any conflict of interest must be disclete testing agencies may be subject to the appro 2. The list of Special Inspectors may be submitt 3. Special Insepctions as required by Section 1 	osed to the Building Official p wal of the Building Official an ed as a separate document, i 704.2.5 are not required when	rior to commenc d/or the Design if noted so above re the fabricator re inspections. P	ing work. The qualifications o Professional. e. is approved in accordance w. erform these tasks for each w	f the Special Inspector(: th IBC Section 1704.2. elded joint, bolted conn	s) and/or 5.2 ection, or steel element.
 Observe on a random basis, operations need NDT of welds completed in an approved fabri 	(S 2) (C	ed by that fabric	ator when approved by the AF	4J. Refer to AISC 360, 1	V7.
 Observe on a random basis, operations need 5. NDT of welds completed in an approved fabric 	cator's shop may be perform			1J. Refer to AISC 360, 1	V7.
4. Observe on a random basis, operations need	cator's shop may be perform ed in the Statement of Sp	ecial Inspectio	กรา	Yes No	V7.



 \mathbf{O} MAKOR LIBERTY DELRAY, DELRAY FL ENGINEER STAMP ATTILL OBELICENSE No. 55428 * * : 🖈 STATE OF TORIDA STATE OF (1)This item has been electronically signed and sealed by Robert M. Beattie, PE using a Digital Signature and date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies. 08/04/2023 Digitally signed by Robert Beattie Date: 2023.08.04 14:46:30-04'00' ENGINEER N BE N WWW.KUL PHONE LI CAL DEPT URAL DEPT ROBERT THESE PL EXPR EXPR EXPR EXPR THESE PL ALTERED OR M SHEET TITLE SPECIAL INSPECTIONS 06-12-23 DATE DRAWN BY AW/MNG CHECKED BY JOB NO. XXX E 2705 AS NOTED SCALE SHEET S-0-5

	POWERS LINTEL SCHEDULE (8" OR 12" CMU)								
MARK	W (")	H (")	REINFORCING	MARK	W (")	H (")	REINFORCING		
PL88	8	8	(I) #5 MID BAR	PLI 28	12	8	(I) #5 MID BAR		
PL812	8	12	(I) #5 BOT	PL1212	12	12	(I) #5 BOT		
PL816	8	16	(I) #5 TOP ∉ BOT BAR	PL1216	12	16	(I) #5 TOP ∉ BOT BAR		
PL816-1	8	16	(2) #5 TOP ∉ BOT BAR	PL1216-1	12	16	(2) #5 TOP ∉ BOT BAR		
PL820	8	20	(I) #5 TOP ¢ BOT BAR	PL1220	12	20	(I) #5 TOP ∉ BOT BAR		
PL824	8	24	(I) #5 TOP ∉ BOT BAR	PL1224	12	24	(I) #5 TOP ∉ BOT BAR		
PL824-1	8	24	(2) #5 TOP ∉ BOT BAR	PL1224-1	12	24	(2) #5 TOP ∉ BOT BAR		
PL828	8	28	(I) #5 TOP ¢ BOT BAR	PL1228	12	28	(I) #5 TOP ∉ BOT BAR		
PL832	8	32	(2) #5 TOP ¢ BOT BAR	PL1232	12	32	(2) #5 TOP ≰ BOT BAR		
PL836	8	36	(2) #5 TOP ¢ BOT BAR	PL1236	12	36	(2) #5 TOP ≰ BOT BAR		
PL840	8	40	(2) #5 TOP, MID, ∉ BOT BAR	PL1240	12	40	(2) #5 TOP, MID, ¢ BOT BAR		
PL840-1	8	40	(2) #G TOP & BOT BAR, (2) #5 MID BARS	PL1240-1	12	40	(2) #G TOP & BOT BAR, (2) #5 MID BARS		
PL844	8	44	(2) #5 TOP, MID, ≰ BOT BAR	PL1244	12	44	(2) #5 TOP, MID, ¢ BOT BAR		
PL848	8	48	(2) #5 TOP, MID, ≰ BOT BAR	PL1248	12	48	(2) #5 TOP, MID, ¢ BOT BAR		
PL848-1	8	48	(2) #6 TOP ¢ BOT BAR, (2) #5 MID BARS	PLI 248-1	12	48	(2) #6 TOP ¢ BOT BAR, (2) #5 MID BARS		

FOOTNOTES: I. ALL HEIGHTS SHOWN ARE MINIMUM DIMENSIONS. INCREASE IN DEPTH IS ACCEPTABLE AS FIELD CONDITIONS DICTATE. WHERE POWERS LINTEL EXTENDS INTO BOND BEAM, CONTINUE THE BOND BEAM REINFORCING THROUGH THE POWERS LINTEL AND ELIMINATE THE TOP BAR SPECIFIED IN THE SCHEDULE ABOVE. HEIGHT OF _____ POWERS STEEL L-2 LINTEL (PATENT NO. 5465538) COMPOSITE LINTEL PREFORMED STEEL LINTEL: GALVANIZED I G GA. THICKNESS (0.0596 INCHES) AND I 8 GAGE THICKNESS (0.0478 INCHES) COIL STEEL AS MANUFACTURED BY POWERS STEEL & WIRE PRODUCTS, INC. STEEL GRADE IS ASTM AS70 GRADE C (PY=40 KS)). GALVANIZTION SHAIL COMPLY WITH ASTM AS25 THE POWERS GALVANIZED STEEL VARIES - SEE PLAN UNTEL COMPLIES WITH ASTM ASZS THE POWERS GALVANIZED STEEL LINTEL COMPLIES WITH ASTM AS70 WHICH IS THE SPECIFICATION FOR STRUCTURAL QUALITY STEEL. 2 SHORE LINTELS AS REQUIRED TO COMPENSATE FOR DEAD LOAD DEFLECTION ON NON-CURED MASONRY GROUT. ALL LINTELS GREATER THAN 18-O" ARE BUILT WITH 1/2" CHAMBER. LINTEL TO BE USED WITH CONCRETE MASONRY UNITS HAVING MINIMUM Pm AS SHOWN. 4. STEEL SURFACES IN CONTACT WITH GROUT AND/OR MORTAR SHALL BE UNPAINTED AND FREE OF MATERIAL THAT MIGHT INHIBIT BOND. BEARING EACH END SHALL BE 4" <u>+</u> 1". BEARING SHALL BE ON A MINIMUM 8" DEEP GROUTED CELL EXCEPT THAT L2 LINTELS MAY BEAR ON 1 1/4" THICK MINIMUM FACE SHELL. MATCH WALL WIDTH (NOM) 6. fm = 1500PSI. MASONRY UNITS SHALL CONFORM TO ASTM C90, GRADE N. TYPICAL POWER /____ 2 1/2" LINTEL SECTION GROUT = 3000PSI. SLUMP RANGE: 8" TO II". ROD OR VIBRATE GROUT **→/** ADEQUATELY TO ENSURE CONSOLIDATION OF GROUT (NO AIR POCKETS), GROUT SHALL COMPLY WITH ASTM C476-83 AND BE EITHER COARSE OR FINE GROUT. ALL LINTELS SHALL BE GROUT FILLED. 8. MORTAR: TYPE "S" OR TYPE "M" 1800PSI. ⊟∙∙∙⊢ —INVERTED KO, TOP REINFORCING OR TOP OF WALL REINFORCING, IS REQUIRED BY CODES TO PROVIDE A CONTINUOUS TIE AROUND A STRUCTURE AND TO PROVIDE FOR UPLIFT RESISTANCE AT LINTELS. TYP 10. ATTACHMENTS TO TOP OF WALL PER ARCHITECTURAL AND/OR ENGINEERING DRAWINGS. II. NOT USED. 12. NOT USED. || • | 13. MASONRY WALL MAY OCCUR ABOVE COMPOSITE LINTEL HEIGHT. IN THE 1.10 4 CASE THAT THE MASONRY WALL IS TWICE THE HEIGHT OF A 24" AND GREATER LINTEL, THE DEAD LOAD WEIGHT OF THE LINTEL MAY BE CLUDED FROM THE LOAD CALCULATIONS DUE TO THE COMPOSITE

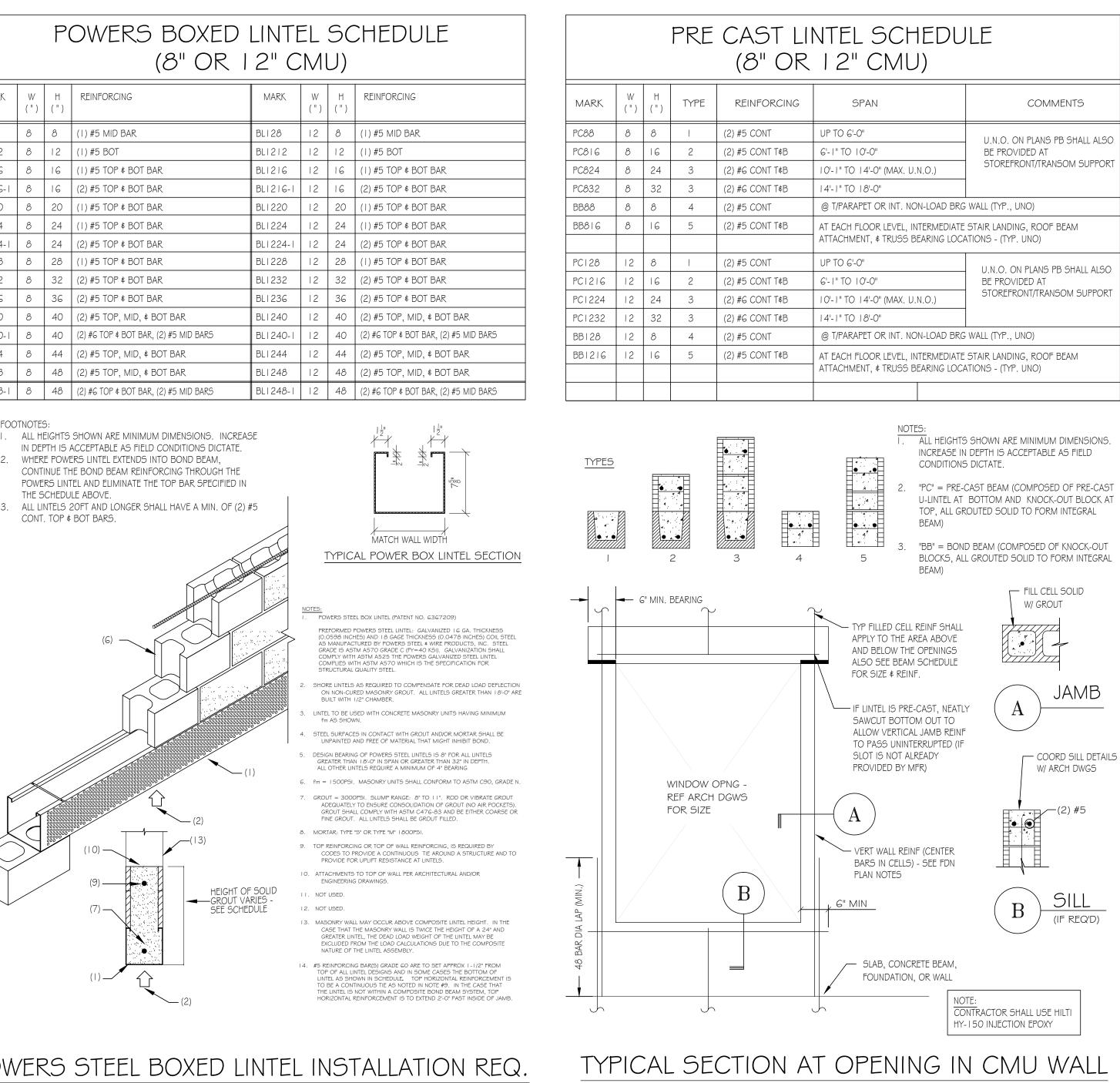
POWERS STEEL LINTEL INSTALLATION REQ.

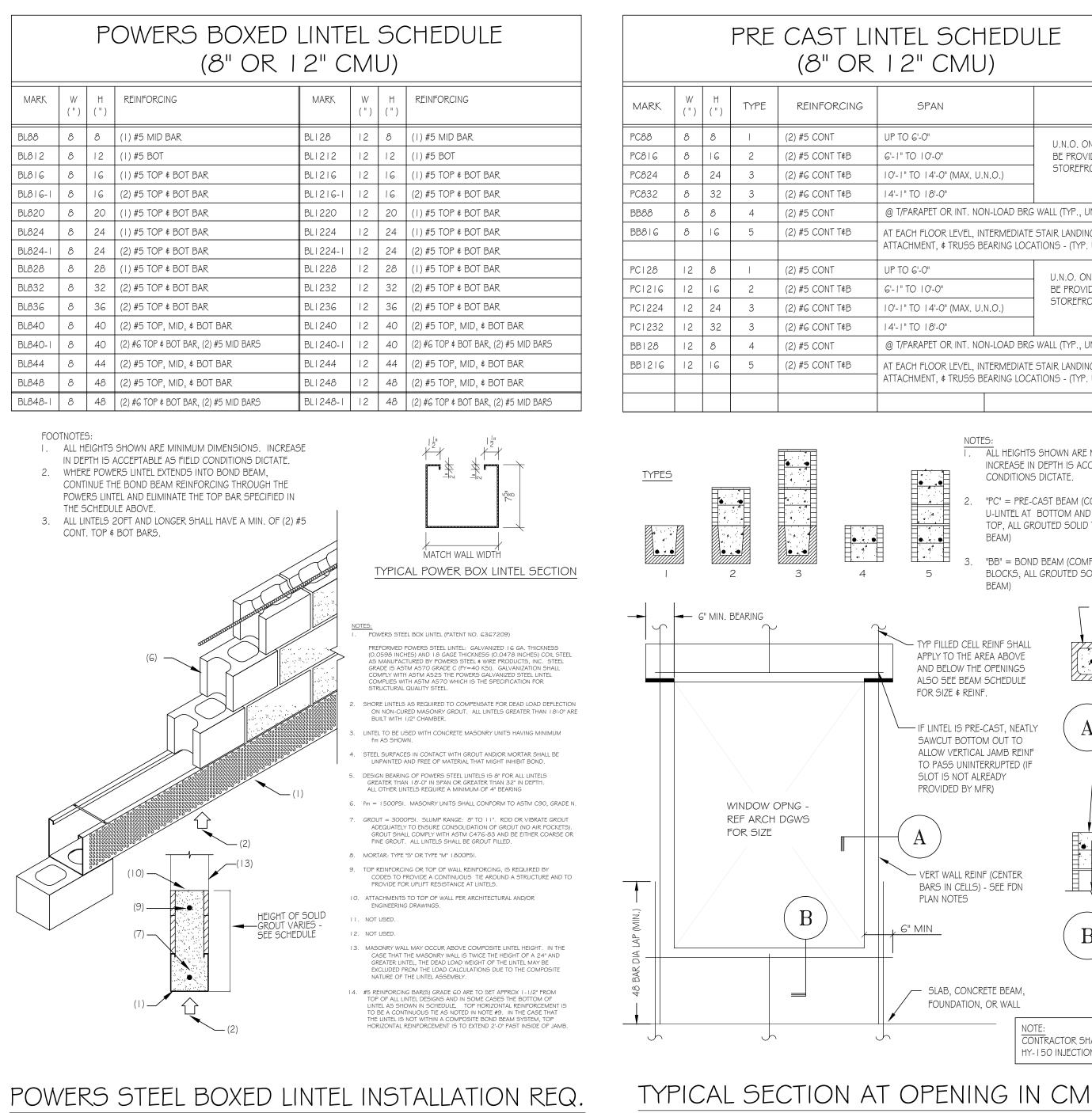
L-2 STEEL LINTEL — (2)

- L-2 STEEL LINTEL NATURE OF THE LINTEL ASSEMBLY.

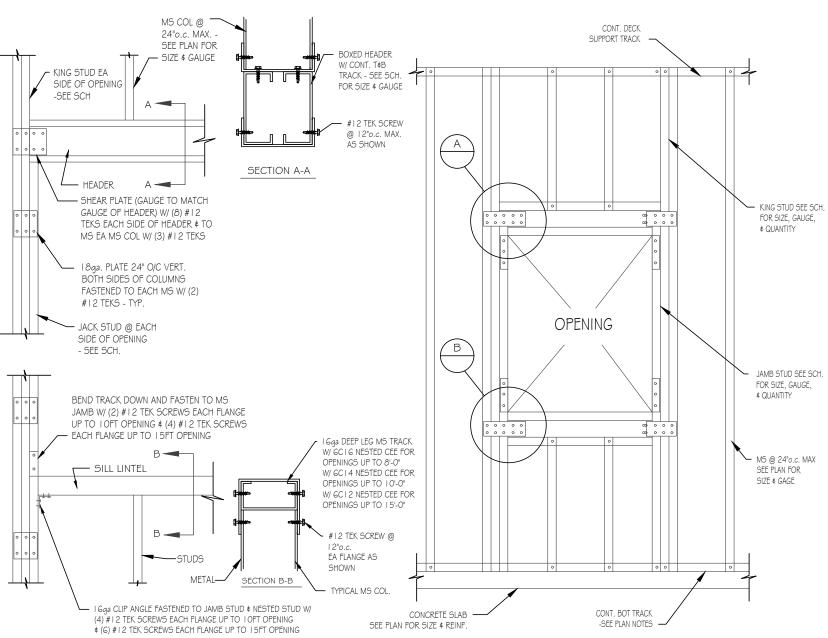
MARK	W (")	H (")	REINFORCING	
BL88	8	8	(I) #5 MID BAR	В
BL8 2	8	12	(I) #5 BOT	В
BL8 6	8	16	(I) #5 TOP ¢ BOT BAR	В
BL8 6-	8	16	(2) #5 TOP ¢ BOT BAR	В
BL820	8	20	(I) #5 TOP ≰ BOT BAR	В
BL824	8	24	(I) #5 TOP ¢ BOT BAR	В
BL824-1	8	24	(2) #5 TOP ¢ BOT BAR	В
BL828	8	28	(I) #5 TOP ∉ BOT BAR	В
BL832	8	32	(2) #5 TOP ¢ BOT BAR	В
BL836	8	36	(2) #5 TOP ¢ BOT BAR	В
BL840	8	40	(2) #5 TOP, MID, ∉ BOT BAR	В
BL840-1	8	40	(2) #6 TOP \$ BOT BAR, (2) #5 MID BARS	В
BL844	8	44	(2) #5 TOP, MID, ≰ BOT BAR	В
BL848	8	48	(2) #5 TOP, MID, ¢ BOT BAR	В
BL848-1	8	48	(2) #6 TOP ¢ BOT BAR, (2) #5 MID BARS	B

- IN DEPTH IS ACCEPTABLE AS FIELD CONDITIONS DICTATE.
- CONTINUE THE BOND BEAM REINFORCING THROUGH THE
- THE SCHEDULE ABOVE.





Received 8.9.23



FLC 3RD

IST FLOO

JAMB

W/ ARCH DWGS

SILL

(IF REQ'D)

(2) #5

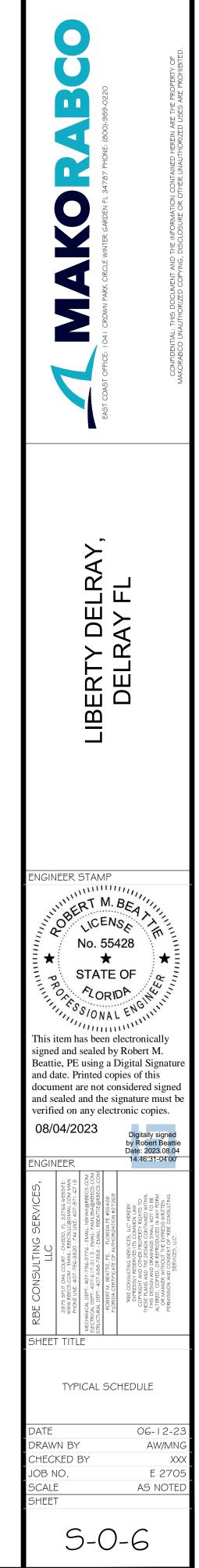
HEADER/JAMB SCHEDULE FOR GROUND SNOW LOAD < 30PSF

FLOOR LEVEL	ROUGH OPENING	HEADER SIZE	HEADER TOP TRACK	HEADER BOT TRACK	JAMB STUD	KING STUD
3RD FLOOR	UP TO 8'-0"	DBL 8C16	CONT.x1 Ggax2"DEEP LEG	CONT.x 6gax2"DEEP LEG	(I) 6CI4	() 6C 4
to t/wall	UP TO 10'-0"	DBL IOCI6	CONT.x 6gax2"DEEP LEG	CONT.x16gax2"DEEP LEG	() 6C 4	()6C 4
	UP TO 12'-0"	DBL 12C16	CONT.x 6gax2"DEEP LEG	CONT.x16gax2"DEEP LEG	(2) 6C 4	(2) 6C 4
2ND TO	UP TO 8'-0"	DBL 8C12	CONT.x 6gax2"DEEP LEG	CONT.x 6gax2"DEEP LEG	(1)6C12	(1)6C12
3RD FLOOR	UP TO 10'-0"	DBL IOCI2	CONT.x 6gax2"DEEP LEG	CONT.x 6gax2"DEEP LEG	(1)6C12	(1)6C12
	UP TO 12'-0"	DBL 12C12	CONT.x 2gax2"DEEP LEG	CONT.x 2gax2"DEEP LEG	(1) 6x4x12ga	(2) 6C 2
IST TO 2ND	UP TO 8'-0"	DBL 8C12	CONT.x 2gax2"DEEP LEG	CONT.x 2gax2"DEEP LEG	(1)6C12	(1)6C12
FLOOR	UP TO 10'-0"	DBL IOCI2	CONT.x 2gax2"DEEP LEG	CONT.x 2gax2"DEEP LEG	(1) 6x4x12ga	(2) 6C 2
	UP TO 12'-0"	DBL 2x3x 2ga	CONT.x 2gax2"DEEP LEG	CONT.x 2gax2"DEEP LEG	(2) 6x4x 2ga	(2) 6x4x 2ga

HEADER/JAMB SCHEDULE FOR GROUND SL > 30PSF < 60PSF

FLOOR LEVEL	ROUGH OPENING	HEADER SIZE	HEADER TOP TRACK	HEADER BOT TRACK	JAMB STUD	KING STUD
3RD FLOOR	UP TO 6'-0"	DBL 8C16	CONT.x 6gax2"DEEP LEG	CONT.x 6gax2"DEEP LEG	(1)6C14	(1)6C14
TO T/WALL	UP TO 8'-0"	DBL IOCI6	CONT.x 6gax2"DEEP LEG	CONT.x 6gax2"DEEP LEG	(1)6C14	(1)6C14
	UP TO 10'-0"	DBL 12C16	CONT.x 6gax2"DEEP LEG	CONT.x 6gax2"DEEP LEG	(2) 6C 4	(2) 6C 4
2ND TO	UP TO 6'-0"	DBL 8C12	CONT.x 6gax2"DEEP LEG	CONT.x 6gax2"DEEP LEG	(1)6C12	(1)6C12
3RD FLOOR	UP TO 8'-0"	DBL IOCI2	CONT.x 6gax2"DEEP LEG	CONT.x 6gax2"DEEP LEG	(1)6C12	(1)6C12
	UP TO 10'-0"	DBL 12C12	CONT.x 2gax2"DEEP LEG	CONT.x 2gax2"DEEP LEG	() 6x4x 2ga	(2) 6C 2
IST TO 2ND	UP TO 6'-0"	DBL 8C12	CONT.x 2gax2"DEEP LEG	CONT.x 2gax2"DEEP LEG	(1)6C12	(1)6C12
FLOOR	UP TO 8'-0"	DBL IOCI2	CONT.x 2gax2"DEEP LEG	CONT.x 2gax2"DEEP LEG	() 6x4x 2ga	(2) 6C 2
	UP TO 10'-0"	DBL 2x3x 2ga	CONT.x 2gax2"DEEP LEG	CONT.x 2gax2"DEEP LEG	(2) 6x4x 2ga	(2) 6x4x 2ga

TYPICAL SECTION EXTERIOR LIGHT-GAGE LOAD BEARING WALL



GENERAL NOTES

- THESE PLANS AND THE INFORMATION CONTAINED HEREIN ARE THE PROPERTY OF SELLER/ SUBCONTRACTOR AND ARE TO BE USED SOLELY IN CONNECTION WITH ERECTION OF BUILDING SYSTEMS AND MATERIALS SOLD TO OWNER/ CONTRACTOR BY SELLER/ SUBCONTRACTOR. UNAUTHORIZED COPYING, DISCLOSURE OR OTHER UNAUTHORIZED USES ARE PROHIBITED.
- OWNER/ CONTRACTOR IS RESPONSIBLE TO PROVIDE SELLER/ SUBCONTRACTOR WITH APPROVED PLANS PRIOR TO FABRICATION. OWNER/ CONTRACTOR IS RESPONSIBLE FOR ALL NECESSARY AND
- REQUIRED PERMITS FEES DEPOSITS ETC. THE OWNER AND/ OR CONTRACTOR SHALL REVIEW AND DETERMINE THAT ALL DIMENSIONS ARE COORDINATED AS REQUIRED WITH ALL OTHER DESIGN PROFESSIONALS' DRAWINGS AND SHOP DRAWINGS FOR PROJECT PRIOR TO FABRICATION OF MATERIALS OR THE START OF CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO THE SELLER/ SUBCONTRACTOR
- AND ENGINEER OF RECORD PRIOR TO FABRICATION. PRODUCTS SHIPPED TO OWNER/ CONTRACTOR SHALL BE INSPECTED
- IMMEDIATELY UPON ARRIVAL. THE SHEET LEDGES, RAINLIPS, AND MASONRY LEDGES ARE VITAL TO THE PROPER FIT OF THE STEEL CONSTRUCTION. OWNER/ CONTRACTOR SHALL FIELD VERIFY ALL TO BE AS SHOWN ON DRAWINGS. IF THEY ARE NOT IN COMPLIANCE WITH THE DRAWINGS, OWNER/ CONTRACTOR SHALL CONTACT SELLER/ SUBCONTRACTOR PRIOR TO START OF STEEL ERECTION.
- 7. ANY OMISSIONS AND/OR CONFLICTS WITH PLANS SHALL BE REPORTED TO METAL BUILDING COMPANY SO THAT THEY CAN BE RESOLVED PRIOR TO PROCEEDING WITH WORK. DO NOT SCALE DRAWINGS - IF A REQUIRED DIMENSION IS MISSING PLEASE
- CONTACT THE METAL BUILDING COMPANY AND / OR ENGINEER OF RECORD. 6. NO MODIFICATIONS TO PLANS SHALL BE MADE WITHOUT THE PERMISSION OF METAL BUILDING COMPANY AND ENGINEER OF RECORD MODIFICATIONS REQUIRED DUE TO FIELD CONDITIONS OR OTHER CONTRACTORS OR ITEMS THAT WHICH MAY ADVERSELY AFFECT THE
- STRUCTURE REQUIRES WRITTEN PERMISSION (NO MODIFICATIONS TO STRUCTURAL MEMBERS IS ALLOWED). 10. ALL SECTIONS AND DETAILS SHALL BE CONSIDERED TO BE TYPICAL OR SIMILAR UNLESS ANOTHER SECTION OR DETAIL IS REFERENCED ON THE
- PLANS II. SCOPE OF WORK OF METAL BUILDING COMPANY IS INDICATED IN THE
- CONTRACT. THE DRAWINGS REFLECT SCOPES OF WORK AS REQUIRED FOR PERMITTING OR AT THE DIRECTION OF OWNER / CONTRACTOR. 12. METAL BUILDING COMPANY DOES NOT PROVIDE A RATED CEILING IN SHAFT
- ENCLOSURES UNLESS SPECIFIC NOTED ON PLANS. 13. CONSTRUCTION DOCUMENTS WERE DESIGNED AND MEET THE REQUIREMENTS OF THE OF THE LOCAL BUILDING CODE DESIGNATED
- UNDER STRUCTURAL DESIGN CRITERIA. 14. ALL COMPONENTS AND CLADDING DESIGNED AND/OR FURNISHED BY OTHERS NOT SHALL MEET THE MINIMUM REQUIREMENTS OUTLINED IN THESE CONSTRUCTION DOCUMENTS AND SHOP DRAWINGS SHALL BE SUBMITTED REVIEW/APPROVAL BY EOR AND GOVERNING AUTHORITIES PRIOR TO FABRICATION AND/OR ERECTION
- 15. PONDING LOADS ARE NOT APPLICABLE ON SELF STORAGE BUILDINGS WHERE THE PRIMARY ROOF DRAINAGE IS ACCOMPLISHED BY GUTTERS AND DOWNSPOUTS. SECONDARY DRAINAGE OCCURS WITH GUTTER OVERFLOW WHICH IS AT OR BELOW EAVE HEIGHT OF BLDG(S).
- 16. THE CONTRACTOR OR OWNER SHALL TAKE ALL NECESSARY STEPS TO PROTECT THE STRUCTURE, THE WORK PERSONS AND OTHER PEOPLE DURING CONSTRUCTION. HE SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION.
- 17. THE CONTRACTOR/ OWNER SHALL TAKE ALL NECESSARY STEPS TO PROTECT THE STRUCTURE. THE WORK. AND OTHER PEOPLE DURING CONSTRUCTION. IF OWNER/ CONTRACTOR PERFORMS THE ERECTION. CONTRACTOR/ OWNER SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION.
- 18. IF ANYTHING IN THESE DRAWINGS CONFLICTS WITH THE CONTRACT, THE TERMS OF THE CONTRACT SHALL GOVERN. 19. TEMPORARY BRACING AND SHORING OF WALLS TO PROVIDE STABILITY
- DURING CONSTRUCTION IS THE RESPONSIBILITY OF THE OWNER AND/ OR CONTRACTOR U.N.O. IN CONSTRUCTION DOCUMENTS.
- THE OWNER AND/ OR CONTRACTOR SHALL TEMPORARILY BRACE AND/ OR SHORE EXISTING AND NEW CONSTRUCTION AS REQUIRED TO ENSURE THE STRUCTURAL STABILITY IS NOT COMPROMISED IF DEMOLITION IS INVOLVED. BRACING AND/ OR SHALL REMAIN IN PLACE UNTIL THE STRUCTURAL WORK IS COMPLETE AND HAS BEEN INSPECTED BY A TESTING AGENCY AND IS CERTIFIED TO BE IN SUBSTANTIAL COMPLIANCE WITH PLANS AND SPECIFICATIONS.
- 21. EVERY REASONABLE EFFORT HAS BEEN MADE TO ENSURE COORDINATION BETWEEN THESE DRAWINGS AND THE BOUND STRUCTURAL SPECIFICATIONS. IF A DISCREPANCY IS DISCOVERED, THE OWNER/ CONTRACTOR SHALL, IN WRITING, REQUEST A CLARIFICATION IN THE ABSENCE OF SAID REQUEST, THE MORE STRINGENT REQUIREMENT SHALL GOVERN
- 22. ALL CELLS OF CMU WALLS IN CLIMATE CONTROLLED BUILDING WHICH ARE NOT GROUTED SHALL BE FILLED WITH PERLITE, VERMICULITE, OR POLYURETHANE FOAMED-IN-PLACE AS SPECIFIED BY THE LATEST ADOPTED EDITION ON THE INTERNATIONAL ENERGY CONSERVATION CODE.
- 23. AREAS WITHIN HURRICANE-PRONE REGIONS LOCATED WITHIN I MILE OF THE COASTAL MEAN HIGH WATER LINE WHERE THE BASIC DESIGN WIND SPEED, V, IS I 30MPH OR GREATER; OR IN ALL AREAS WHERE THE BASIC DESIGN WIND SPEED IS I 40MPH OR GREATER SHALL MEET THE REQUIREMENTS FOR WIND BORNE DEBRIS REGION.
- 24. GLAZING IN BUILDINGS IN AREAS THAT MEET THE CRITERIA FOR WIND BORNE DEBRIS REGION SHALL BE IMPACT RESISTANT OR PROTECTED WITH AN IMPACT RESISTANT COVERING MEETING THE LARGE MISSILE TEST (ASTM E 1996) FOR OPENINGS LOCATED WITHIN 30FT OF GRADE OR SHALL MEET THE REQUIREMENTS FOR SMALL MISSILE TEST (ASTM E1996) FOR OPENINGS LOCATED MORE THAN 30FT ABOVE GRADE.
- THE GENERAL CONTRACTOR SHALL TAKE ALL NECESSARY STEPS TO PROTECT THE STRUCTURE, THE WORK PERSONS, AND OTHER PEOPLE DURING CONSTRUCTION. HE SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION.
- NO STRUCTURAL MEMBER SHALL BE CUT, NOTCHED OR OTHERWISE REDUCED IN STRENGTH
- THE GENERAL CONTRACTOR SHALL COORDINATE ALL DESIGN PROFESSIONALS DRAWINGS AND SPECIFICATIONS, FOR ITEMS WHICH MAY ADVERSELY AFFECT THE STRUCTURE AND NOTIFY THE ARCHITECT/ENGINEER 4. LOAD-BEARING STUD TO TRACK CONNECTIONS: THE ENDS OF THE OF ANY DISCREPANCIES AND/OR OMISSIONS.
- SUBMITTAL SETS SHALL CONSIST OF A MAXIMUM OF 1 SEPIA AND 2 BLUE LINES, ADDITIONAL BLUE LINES WILL BE DISCARDED. • SUBMITTALS TO ENGINEER OF RECORD FOR REVIEW MUST CONTAIN THE
- GENERAL CONTRACTOR'S STAMP SIGNIFYING HIS REVIEW/ACCEPTANCE. SUBMITTALS SENT WITHOUT THE GC'S REVIEW STAMP WILL BE RETURNED AT HIS EXPENSE AND WITHOUT REVIEW.

STRUCTURAL NOTES

- TEMPORARY BRACING SHOULD BE PROVIDED TO RESIST WIND LOADING ON STRUCTURAL COMPONENTS AND STRUCTURAL ASSEMBLIES DURING
- ERECTION AND CONSTRUCTION PHASE. 2. NEVER ALLOW YOUR ROOF TO COME IN CONTACT WITH, OR WATER RUNOFF FROM, ANY DISSIMILAR METAL, INCLUDING, BUT NOT LIMITED TO, COPPER, LEAD, OR GRAPHITE, THIS INCLUDES COPPER AND ARSENIC SALTS USED IN TREATED LUMBER, AND CALCIUM USED IN CONCRETE, MORTAR, AND GROUT.
- 3. SCOPES OF WORK BY OTHERS WHOSE LATERAL LOADS WILL BE TRANSFERRED INTO STEEL MEMBER PROVIDED BY METAL BUILDING COMPANY SHALL BE TEMPORARILY BRACED BY OTHERS IN A METHOD THAT DOES NOT INTERFERE WITH ERECTION OF STEEL, UNTIL STEEL ERECTION COMPLETE.
- THE UNCOATED MINIMUM STEEL THICKNESS OF THE COLD-FORMED PRODUCTS AS DELIVERED SHALL NOT BE LESS THAN 95% PERCENT OF THE DESIGN THICKNESS. THICKNESS MEASUREMENTS MAY BE MADE ANYWHERE ACROSS THE WIDTH OF THE SHEET, BUT NOT CLOSER TO THE EDGES THAN 3. THE MINIMUM DISTANCES SPECIFIED IN THE RELEVANT ASTM SPECIFICATIONS. THICKNESS AT BENDS, SUCH AS CORNERS, MAY BE LESS THAN 95% OF DESIGN THICKNESS, DUE TO COLD-FORMING EFFECTS, AND STILL BE ACCEPTABLE
- RECESSED ENTRIES AND BREEZEWAYS MUST BE RECESSED BELOW FINISHED FLOOR TO AVOID POTENTIAL WATER PROBLEMS, ROLL-UP DOORS LOCATED IN BREEZE WAY NEED TO BE INSTALLED IN RECESSED AREA. IF A CHANGE IS MADE BY OWNER/ CONTRACTOR, SELLER/ SUBCONTRACTOR MUST BE NOTIFIED IMMEDIATELY.
- ALL ERECTION. FABRICATION. WORKMANSHIP AND INSTALLATION SHALL BE IN ACCORDANCE WITH INSTALLATION PROCEDURES MANUAL AND/ OR INDUSTRY STANDARDS APPROVED BY SELLER/ SUBCONTRACTOR AND THE ENGINEER OF RECORD.

ROOF SYSTEMS: MBCI OR EQUAL

- ROOFING SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS AND SPECS, ALONG WITH SELLER/ SUBCONTRACTOR INSTALLATION PROCEDURES MANUAL ROOF SHEETS SHOULD BE INSTALLED FROM THE LOWEST STEP-DOWN TO
- HIGHEST ELEVATION.

INSULATION: BAY INSULATION SUPPLY CO. OR EQUAL

- I. VINYL-BACKED INSULATION- BY BAY INSULATION CO. OR EQUAL. R-VALUES FOR WINTER CONDITIONS (MEAN 40 DEGREES F.): 3.0"- 0.6 LB DENSITY FIBERGLASS 3.0" = 10 3.5"- 0.6 LB DENSITY FIBERGLASS 3.0" = 1 1
- 4.0"- 0.6 LB DENSITY FIBERGLASS 3.0" = 13
- 6.0"- 0.6 LB DENSITY FIBERGLASS 3.0" = 19 POLYFOIL FIRE-RATED R-FOIL BY TVM OR EQUAL
- SINGLE BUBBLE R-VALUES FOR ROOF = 10
- DOUBLE BUBBLE R-VALUES FOR ROOF = 10INSULATION SHALL BE INSTALLED PER THE MANUFACTURER'S PUBLISHED INSTRUCTIONS. INSULATION MAY BE INSTALLED IN ONE OR MULTIPLE LAYERS TO MEET THE REQUIRED R-VALUE.

FASTENERS AND ANCHORS

- I. THE FOLLOWING OUTLINES THE MECHANICAL ANCHORS APPROVED FOR USE ON THIS PROJECT.
- A. EXPANSION ANCHORS- "KWICK BOLT 3" BY HILTI OR EQUAL DRILL HOLE IN CONCRETE OR GROUT-FILLED CMU AND REMOVE DUST. THE MIN. HOLE DEPTH MUST EXCEED THE ANCHOR EMBEDMENT PRIOR TO TORQUING BY ONE HOLE DIAMETER. CRIVE THE ANCHOR INTO THE HOLE USING A HAMMER. A MINIMUM OF SIX THREADS MUST BE BELOW THE SURFACE OF THE FIXTURE. TIGHTEN THE NUT TO THE
- RECOMMENDED INSTALLATION TORQUE $(\frac{1}{2}" = 40 \text{ lbs./ft.})$. ADHESIVE ANCHORS IN CONCRETE- "HIT HY 150" BY HILTI OR EQUAL C. ADHESIVE ANCHORS IN GROUT-FILLED BLOCK- "HIT HY 20" BY HILTI OR
- EQUAL. D. ADHESIVE ANCHORS IN HOLLOW BLOCK- "HIT HY 20" WITH SCREEN TUBES BY HILTI OR EQUAL.
- CONCRETE MASONRY SCREWS- "KWIK CON II" BY HILTI OR EQUAL. POWDER-ACTUATED FASTENERS (PAF)- "DX" BY HILTI OR EQUAL.
- 2. ALL FASTENERS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS AND SPECIFICATIONS.

COLD FORMED STEEL: LGSI SECTIONS OR EQUAL

- COLD FORM STEEL SECTIONS SHALL CONFORM TO APPLICABLE PROVISIONS OF ASTM A572, ASTM AG07 AND/OR ASTM AG11
- MIN. DELIVERED THICKNESS OF COLD FORMED STEEL C'S \notin Z'S

GAGE	DESIGN THICKNESS	FINISH	Fy
12	0.105	RED-OXIDE UNO ON PLANS	55ksı
14	0.07	RED-OXIDE UNO ON PLANS	55ksı
16	0.059	RED-OXIDE UNO ON PLANS	55ksı
18	0.0468	GALVANIZED	33ksı
20	0.0352	GALVANIZED	33ksı

3. MIN. DELIVERIED THICKNESS OF COLD FORMED STEEL PIERS & HEADERS

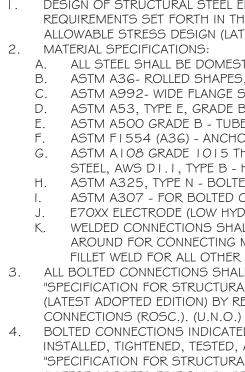
GAGE	DESIGN THICKNESS	FINISH	Fy			
4	0.07	TEX-COTE FINISH APPLIED IN FIELD	55ksı			
16	0.059	TEX-COTE FINISH APPLIED IN FIELD	55ksı			
16	0.055	PRE-FINISHED	55ksı			
I 8 0.0468 TEX-COTE FINISH APPLIED IN FIELD 33ksi						
NOTES: INSIDE RETURN ON PIERS IS 3 1/2" TO 4", VERIFY REQUIRED						
DIMEN	USIONS FOR EVTERIOR	P POLL LIP DOOPS				

DIMENSIONS FOR EXTERIOR ROLL UP DOORS LOAD-BEARING STUDS MUST BE INSTALLED INTO THE TOP AND BOTTOM TRACKS SO THAT THE GAP BETWEEN THE ENDS OF THE STUD AND THE WEB OF THE TRACK IS AS SMALL AS PRACTICABLE AND IN NO CASE GREATER THAN $\frac{3}{16}$ " AT THE TIME OF INSTALLATION. THE GAP MUST BE LESS THAN $\frac{1}{16}$ " AFTER THE DEAD LOAD OF THE STRUCTURE IS IN PLACE. ALL BOLTED CONNECTIONS OF COLD FORMED STEEL BEAMS

SUPPORTING ROOF LOADING ONLY SHALL BE MADE USING 1/2"DIA. A307 BOLTS OR HIGHER GRADE (U.N.O.).

- ALL BOLTED CONNECTIONS OF COLD FORMED STEEL BEAMS SUPPORTING FLOOR LOADING SHALL BE MADE USING A325 BOLTS OR HIGHER GRADE. (U.N.O.)
- 7. IN THE EVENT THAT THE BOLT HOLES IN THE COLD FORMED STEEL BEAMS DO NOT ALIGN WITH THE SUPPORTING CONNECTION MEMBER IT IS ACCEPTABLE TO SUBSTITUTING #12 TEK SCREWS FOR THE FOLLOWING SIZE/GRADE BOLTS. * (4) #12 TEK SCREWS PER EACH MIS-ALIGNED 1/2" DIAMETER A307
- BOLT * (6) #12 TEK SCREWS PER EACH MIS-ALIGNED 1/2" DIAMETER A325

HOT ROLLED STEEL



ENCLOSED.

9.

Received 8.9.23

DESIGN OF STRUCTURAL STEEL ELEMENTS WAS COMPLETED UNDER THE REQUIREMENTS SET FORTH IN THE "MANUAL OF STEEL CONSTRUCTION-ALLOWABLE STRESS DESIGN (LATEST EDITION)"

ALL STEEL SHALL BE DOMESTICALLY PRODUCED. ASTM A36- ROLLED SHAPES, PLATES, AND BARS. Fy=36ksı ASTM A992- WIDE FLANGE SECTIONS. Fy=50ksı ASTM A53, TYPE E, GRADE B - PIPE. Fy=35ksı ASTM A500 GRADE B - TUBES. Fv=46ksi ASTM F1554 (A36) - ANCHOR BOLTS, RODS, NUTS ¢ WASHERS.

ASTM A 108 GRADE 1015 THROUGH 1020, COLD FINISHED CARBON STEEL, AWS DI.I, TYPE B - HEADED STUDS. ASTM A325, TYPE N - BOLTED STRUCTURAL CONNECTIONS.

ASTM A307 - FOR BOLTED CONN. OF LESS THEN 5/8" DIA. E70XX ELECTRODE (LOW HYDROGEN) - WELDED CONNECTIONS (U.N.O.). WELDED CONNECTIONS SHALL BE A MIN. OF 3/16" FILLET WELD ALL AROUND FOR CONNECTING MEMBERS UP TO 1/4" THICK. USE 1/4" FILLET WELD FOR ALL OTHER MEMBER THICKNESSES (U.N.O.). ALL BOLTED CONNECTIONS SHALL BE "SNUG-TIGHT" AS DEFINED IN THE SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS" (LATEST ADOPTED EDITION) BY RESEARCH COUNCIL ON STRUCTURAL

BOLTED CONNECTIONS INDICATED TO BE "SLIP-CRITICAL" (SC) SHALL BE INSTALLED, TIGHTENED, TESTED, AND INSPECTED AS OUTLINED IN THE "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS" (LATEST ADOPTED EDITION) BY RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (ROSC). (U.N.O.)

BRACE AND MAINTAIN ALL STEEL IN ALIGNMENT UNTIL OTHER PARTS OF CONSTRUCTION NECESSARY FOR PERMANENT SUPPORT ARE COMPLETED. CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLING TEMPORARY SHORING AS REQUIRED FOR THE STABILITY OF THE STEEL FRAME UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN COMPLETED AND BUILDING IS

ALL WELDING SHALL CONFORM TO THE REQUIREMENTS OF "THE STANDARD CODE FOR WELDING IN BUILDING CONSTRUCTION" OF THE AMERICAN WELDING SOCIETY

GROUT FOR COLUMN BASE PLATES AND PRESET BEARING PLATES SHALL BE NON-SHRINK, NON-METALLIC GROUT. (5000psi MIN) SUBMIT SHOP DRAWINGS INDICATING ALL SHOP AND ERECTION DETAILS INCLUDING PROFILES, SIZES, SPACING AND LOCATIONS OF STRUCTURAL MEMBERS, CONNECTION ATTACHMENTS, FASTENERS, LOADS AND

TOLERANCES. ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED IN ACCORDANCE WITH THE MINIMUM REQUIREMENTS OF ASTM A 1 23. 10. STRUCTURAL STEEL SHALL RECEIVE SHOP COAT OF PRIMER (COLOR AS DIRECTED BY ARCHITECT) EXCEPT AREAS THAT WILL RECEIVE SPRAY-ON FIRE PROTECTION.

II. BEAM TO BEAM AND BEAM TO COLUMN CONNECTIONS SHALL BE DESIGNED BY THE STEEL FABRICATOR FOR THE REACTIONS SHOWN ON THE FRAMING PLANS. SIGNED AND SEALED SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW, WHICH CLEARLY INDICATES THE ALLOWABLE LOAD CAPACITY OF EACH UNIQUE CONNECTION. WHERE REACTION IS NOT INDICATED ON THE PLANS, THE CONNECTION SHALL BE DESIGNED FOR THE MAXIMUM SHEAR CAPACITY OF THE BEAM, FOR THE GIVEN SPAN.

STRUCTURAL STEEL

I. A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND FNGINFFR

FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL BE IN STRICT CONFORMANCE WITH THE LATEST ADOPTED EDITION OF THE AISC MANUAL. DESIGN OF STRUCTURAL STEEL ELEMENTS WAS COMPLETED UNDER THE REQUIREMENTS SET FORTH IN THE "MANUAL OF STEEL CONSTRUCTION -ALLOWABLE STRESS DESIGN (LATEST EDITION)".

STEEL DECKING

THE REQUIREMENTS OF THE LATEST ADOPTED EDITION OF THE AISI SECTION A3, SHALL GOVERN FABRICATION OF THE SPECIFIED STEEL DECK. THE MINIMUM YIELD STRENGTH OF STEEL USED, SHALL BE, 33KSI (230

ALL FIELD WELDING OF DECK SHALL BE IN STRICT CONFORMANCE WITH ANSI/AWSD I .3 STRUCTURAL WELDING CODE. 4. GALVANIZING SHALL CONFORM TO ASTM-A653, STRUCTURAL QUALITY,

AND FEDERAL SPEC. QQ-S-775. 5. THE VALUES LISTED IN THE TABLE SHOWN BELOW IS FROM THE MBCI ULTRA-DEK TECHNICAL-INSTALLATION INFORMATION MANUAL. AND REPRESENTS THE MINIMUM ROOF DECK SECTION PROPERTIES THAT ARE

REQUIRED BY DESIGN.							
ROOF DECK							
DECK TYPE	DESIGN THICK (1n)	lxe ın^4/ft	Sxe ın^3/ft	lxe ın^4/ft	Sxe ın^3/ft	Fy (ksı)	
24"x26ga ULTRA-DEK	0.0179	0.1025	0.0694	0.2202	0.0901	50	
24"x24ga ULTRA-DEK	0.0239	0.1355	0.0951	0.2803	0.1156	50	
24"x22ga ULTRA-DEK	0.0299	0.1837	0.1332	0.3640	0.1504	50	

COMPOSITE STEEL FLOOR DECK SHALL BE ATTACHED TO THE SUPPORTING STRUCTURE AS RECOMMENDED BY THE MANUFACTURER, WITH MAXIMUM SPACING NOT TO EXCEED 24" oc.

7. THE VALUES LISTED IN THE TABLE SHOWN BELOW IS FROM THE VULCRAFT METAL DECK PRODUCT MANUAL, AND REPRESENTS THE MINIMUM COMPOSITE FLOOR DECK SECTION PROPERTIES THAT ARE REQUIRED BY DESIGN

COMPOSITE FLOOR DECK

DECK TYPEDESIGN THICK (II)Ip 4 (II)In 4 (III)Sp 3 (III)Sn 3 (III)STL. STRENGTH1.5VL220.02950.1500.1820.1780.186Fy=50ksi1.5VL200.03580.1950.2220.2310.240Fy=50ksi1.5VL180.04740.2820.2950.3150.327Fy=50ksi2.0VL220.02950.3220.3290.2740.277Fy=50ksi2.0VL200.03580.4180.4150.3550.360Fy=50ksi2.0VL180.04740.5590.5580.4950.504Fy=50ksi2.0VL160.05980.7040.7040.6530.653Fy=40ksi3.0VL200.03580.9380.9370.5530.572Fy=50ksi3.0VL200.03581.2511.2510.7950.803Fy=50ksi	(LUN		
I.SVL20 0.0358 0.195 0.222 0.231 0.240 Fy=50ksi I.SVL18 0.0474 0.282 0.295 0.315 0.327 Fy=50ksi 2.0VL22 0.0295 0.322 0.329 0.274 0.277 Fy=50ksi 2.0VL20 0.0358 0.418 0.415 0.355 0.360 Fy=50ksi 2.0VL18 0.0474 0.559 0.558 0.495 0.504 Fy=50ksi 2.0VL18 0.0474 0.559 0.558 0.495 0.504 Fy=50ksi 2.0VL16 0.0598 0.704 0.704 0.653 0.653 Fy=40ksi 3.0VL20 0.0358 0.938 0.937 0.553 0.572 Fy=50ksi		THICK	4	4	3	3	
1.5VL18 0.0474 0.282 0.295 0.315 0.327 Fy=50ksi 2.0VL22 0.0295 0.322 0.329 0.274 0.277 Fy=50ksi 2.0VL20 0.0358 0.418 0.415 0.355 0.360 Fy=50ksi 2.0VL18 0.0474 0.559 0.558 0.495 0.504 Fy=50ksi 2.0VL16 0.0598 0.704 0.704 0.653 0.653 Fy=40ksi 3.0VL22 0.0295 0.746 0.745 0.429 0.442 Fy=50ksi 3.0VL20 0.0358 0.938 0.937 0.553 0.572 Fy=50ksi	1.5VL22	0.0295	0.150	0.182	0.178	0.186	Fy=50ksı
2.0VL22 0.0295 0.322 0.329 0.274 0.277 Fy=50ksi 2.0VL20 0.0358 0.418 0.415 0.355 0.360 Fy=50ksi 2.0VL18 0.0474 0.559 0.558 0.495 0.504 Fy=50ksi 2.0VL16 0.0598 0.704 0.704 0.653 0.653 Fy=40ksi 3.0VL22 0.0295 0.746 0.745 0.429 0.442 Fy=50ksi 3.0VL20 0.0358 0.938 0.937 0.553 0.572 Fy=50ksi	1.5VL20	0.0358	0.195	0.222	0.231	0.240	Fy=50ksı
2.0VL20 0.0358 0.418 0.415 0.355 0.360 Fy=50ksi 2.0VL18 0.0474 0.559 0.558 0.495 0.504 Fy=50ksi 2.0VL16 0.0598 0.704 0.704 0.653 0.653 Fy=40ksi 3.0VL22 0.0295 0.746 0.745 0.429 0.442 Fy=50ksi 3.0VL20 0.0358 0.938 0.937 0.553 0.572 Fy=50ksi	1.5VL18	0.0474	0.282	0.295	0.315	0.327	Fy=50ksı
2.0VL18 0.0474 0.559 0.558 0.495 0.504 Fy=50ksi 2.0VL16 0.0598 0.704 0.653 0.653 Fy=40ksi 3.0VL22 0.0295 0.746 0.745 0.429 0.442 Fy=50ksi 3.0VL20 0.0358 0.938 0.937 0.553 0.572 Fy=50ksi	2.0VL22	0.0295	0.322	0.329	0.274	0.277	Fy=50ksı
2.0VL16 0.0598 0.704 0.704 0.653 0.653 Fy=40ksi 3.0VL22 0.0295 0.746 0.745 0.429 0.442 Fy=50ksi 3.0VL20 0.0358 0.938 0.937 0.553 0.572 Fy=50ksi	2.0VL20	0.0358	0.418	0.415	0.355	0.360	Fy=50ksı
3.0VL22 0.0295 0.746 0.745 0.429 0.442 Fy=50ksi 3.0VL20 0.0358 0.938 0.937 0.553 0.572 Fy=50ksi	2.0VL18	0.0474	0.559	0.558	0.495	0.504	Fy=50ksı
3.0VL20 0.0358 0.938 0.937 0.553 0.572 Fy=50ksi	2.0VL16	0.0598	0.704	0.704	0.653	0.653	Fy=40ksı
	3.0VL22	0.0295	0.746	0.745	0.429	0.442	Fy=50ksı
3.0VL18 0.0474 1.251 1.251 0.795 0.803 Fy=50ks	3.0VL20	0.0358	0.938	0.937	0.553	0.572	Fy=50ksı
	3.0VL18	0.0474	1.251	1.251	0.795	0.803	Fy=50ksı

8. NON-COMPOSITE STEEL FLOOR DECK SHALL BE ATTACHED TO THE SUPPORTING STRUCTURE AS RECOMMENDED BY THE MANUFACTURER, WITH MAXIMUM SPACING NOT TO EXCEED 24" O.C.

STEEL USED TO MANUFACTURE THE NON-COMPOSITE METAL FLOOR DECKING SHALL CONFORM TO THE REQUIREMENTS OF ASTM-A 1008 GRADES C, D, OR E, AND/OR A653 STRUCTURAL QUALITY.

10. THE VALUES LISTED IN THE TABLE SHOWN BELOW IS FROM THE VULCRAFT METAL DECK PRODUCT MANUAL, AND REPRESENTS THE MINIMUM NON-COMPOSITE FLOOR DECK SECTION PROPERTIES THAT ARE REQUIRED BY DESIGN

NON COMPOSITE FLOOR DECK

					`
DECK TYPE	DESIGN THICK (1n)	lp 4 ın /ft	In 4 ın /ft	Sp 3 1n /ft	Sn 3 ın /ft
0.6C26	0.0179	0.015	0.015	0.043	0.043
0.6C24	0.0239	0.019	0.019	0.057	0.057
0.6C22	0.0298	0.024	0.024	0070	0.070
0.6C26 0.6C24	(in) 0.0179 0.0239	0.015	0.015	ın /ft 0.043 0.057	ın /ft 0.043 0.057

MINIMUM STANDARD FOR ELEVATED FLOOR SLABS

A	ALL CONCRETE SHALL HAVE THE FOLLOWING MIN. PROPERTIES:					
	LOCATION	28 DAY STRENGTH	SLUMP	MAX AGGR.		
ELEV	ATED SLABS FORMED AND POURED	4,000 psi	4" <u>+</u> "	1"		
ELEV	ATED SLABS FORMED W/ MTL DECK	4,000 psi	4" <u>+</u> "	1"		
А. В.						

- CONCRETE PROPERTIES SHALL BE VERIFIED THROUGH INDUSTRY STANDARD TESTING PROCEDURES BY A CERTIFIED TESTING AGENCY. MIN. TEST REQUIRED SHALL INCLUDE SLUMP AND CYLINDER BEAKS FOR COMPRESSIVE STRENGTH. FINDINGS SHALL BE SUBMITTED TO THE ARCH./ENG. FOR REVIEW.
- 3. CONCRETE WORK SHALL CONFORM TO LATEST EDITIONS OF ACI 301 SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS, ACI 318 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE AND ACI 3 | 5 DETAILS AND DETAILING OF CONCRETE REINFORCEMENT.
- 4. CONCRETE MIX DESIGN SHALL MEET THE FOLLOWING CRITERIA
- A. PROPOSED MIX DESIGN SHALL BE ACCORDANCE WITH ACI 301 METHOD 1 OR METHOD 2 .
- ENTRAPPED AIR CONTENT SHALL NOT EXCEED 3%. ADMIXTURES USED TO ENTRAIN AIR ARE NOT ACCEPTABLE ALL CONCRETE TO BE NORMAL WEIGHT WITH A DESIGN STRENGTH AT 28 D.
- SITE ADDED WATER IS NOT ACCEPTABLE. HOLD BACK WATER CLEARLY IDENTIFIED TO BE ADDED AT THE SITE AS SPECIFIED IN EITHER THE MIX DESIGN AND/OR THE BATCH PLANT LOG SHALL BE CONSIDERED ACCEPTABLE. IN THE ABSENCE SPECIFIED HOLD BACK WATER. THE MIX WILL RESULT IN A REJECTION OF THE RESULTS BY THE ENGINEER OF RECORD.
- 6. THE USE OF MIX DESIGNS WITH EITHER SLAG OR FLY ASH ARE NOT ACCEPTABLE FOR ALL EXPOSED SURFACES. SLABS ON GRADE. OR ELEVATED SLABS, AND WHERE THE SURFACE IS INTENDED TO BE EXPOSED.
- THE USE OF RIDE ON POWER TROWELS IS STRICTLY PROHIBITED ON ELEVATED SLABS. THE USE OF WALK BEHIND POWER TROWELS ARE RECOMMENDED.
- CONTRACTOR IS RESPONSIBLE FOR THE ADEQUACY OF THE FORMS AND SHORING AND FOR SAFE PRACTICE IN THEIR USE AND REMOVAL.
- 9. THE VALUES IN THE TABLE SHOWN BELOW IS FROM THE VULCRAFT METAL DECK PRODUCT MANUAL AND REPRESENTS THE RECOMMENDED WELDED WIRE FABRIC.

DECK TYPE	TOTAL SLAB DEPTH	RECOMMENDED WELDED WIRE FABRIC
1.5 VL, VLI or R	<= 4 3/4"	6 x 6 - W2. x W2.
I.5 VL, VLI or R	> 4 3/4"	6 x 6 - W2.1 x W2.1
2VLI	<= 5 /4"	6 x 6 - W2.1 x W2.1
2VLI	> 5 /4"	6 x 6 - W2.1 x W2.1
3VLI	<= 6 /4"	6 x 6 - W2. x W2.
3VLI	> 6 /4"	6 x 6 - W2.1 x W2.1

THE FOLLOWING METHODS OF REINFORCEMENT SHALL BE ACCEPTABLE ALTERNATES TO THE WELDED WIRE FABRIC SPECIFIED FOR THE ELEVATED SLABS ABOVE

- I. CONCRETE IN ACCORDANCE WITH ASTM CIIIG, TYPE I, CONTAINING STEEL FIBERS MEETING THE CRITERIA OF ASTM A820, TYPE I, TYPE II OR TYPE V, AT A DOSAGE RATE DETERMINED BY THE FIBER MANUFACTURER FOR THIS APPLICATION, BUT NOT LESS THAN 25 LB/CU YD.
- 2. CONCRETE IN ACCORDANCE WITH ASTM CILLG. TYPE III CONTAINING MACROSYNTHETIC FIBERS MEETING THE CRITERIA OF ASTM D7508 AT A DOSAGE RATE DETERMINED BY THE FIBER MANUFACTURER FOR THE APPLICATION, BUT NOT LESS THAN 4 LB/CU YD.

CONCRETE WORK

- MONOLITHIC SLAB FINISHES: THE FOLLOWING REQUIREMENTS ARE BASED ON THE LATEST FLOOR FLATNESS (FF)/ FLOOR LEVELNESS (FL) VALUES/ METHODS. BIDS FOR THIS WORK SHALL REFLECT THESE REQUIREMENTS AND ENFORCEMENT THEREOF CAN BE EXPECTED.
- A. NON-CRITICAL FLOOR TOLERANCE
- a. FLOAT-FINISH (FLT-FN)
- SPECIFIED OVERALL VALUE: FF25/FL20 b MINIMUM LOCAL VALUE: FF25/FL20
- d. APPLY FLOAT FINISH TO MONOLITHIC SLAB SURFACES THAT ARE TO RECEIVE MUD SET TILE AND OTHER THICK FINISHES, AND TO
- SLAB SURFACES WHICH ARE TO BE COVERED WITH WATERPROOFING MEMBRANE. B. TYPICAL CORRIDOR OR NORMAL SIZED ROOMS (100-600 SF)
- a. TROWEL FINISH | (TR-FN |) SPECIFIED OVERALL VALUE: FF30/FL23
- MINIMUM LOCAL VALUE: FF25/FL20
- APPLY TROWEL FINISH TO MONOLITHIC SLAB SURFACES THAT ARE TO RECEIVE RESILIENT FLOORING, CARPET, PAINT, OR OTHER THIN FILM FINISH COATING SYSTEMS
- ELEVATED SLABS SHALL HAVE A SPECIFIED OVERALL VALUE OF FF22 AND FF27 AND A MINIMUM LOCAL OF FF20 WITH NO FL NUMBER DEFINED.
- MINIMUM THICKNESS OF SLAB ON GRADE IS THE GREATER OF 3" OR 0.8 TIMES ANCHOR EMBEDMENT SPECIFIED IN CONSTRUCTION DOCUMENTS (ASSUMES USE OF HILTI KWIK BOLT 3).
- CONTROL JOINTS SHOULD BE PLACED A MINIMUM 2'-O" OFF THE CENTERLINE OF COLUMNS. IF THE DISTANCE BETWEEN COLUMNS IS LESS THAN 4'-O" BUT GREATER THAN 2'-G" THEN PLACE AT MID-POINT OTHERWISE CONTACT ENGINEER OF RECORD.

SITE REQUIREMENT NOTES

- I. OWNER/ CONTRACTOR SHALL ENSURE THAT SITE IS STABILIZED AND MAINTAINED CURING HEAVY PRECIPITATION.
- 2. OWNER/ CONTRACTOR TO PROVIDE MATERIAL STORAGE AREA ON SITE
- OTHER THAN BUILDING BEING ERECTED. 3. OWNER/ CONTRACTOR SHALL PROVIDE A CONSTRUCTION DUMPSTER UNIT ON THE JOBSITE AT NO COST TO SELLER/ CONTRACTOR.
- ERECTED.
- 5. TEMPORARY POWER MUST MEET ALL APPLICABLE CODES AND SAFETY REQUIREMENTS
- BEGINNING INSTALLATION SEQUENCE AS AGREED.
- OBSTRUCTIONS TO DELIVERY OR ERECTION.
- AND STRUCTURAL MATERIALS, IS NOT THE RESPONSIBILITY OF SELLER/ SUBCONTRACTOR.
- PUBLIC SAFETY AND AGAINST ACCIDENTS, WEATHER OR ANY OTHER (INCLUDES FALL PROTECTION ON MULTI-STORY BUILDINGS).

MAINTENANCE NOTES

- I. ROOF MAINTENANCE GUIDELINES
- INSPECT ROOF FOR DAMAGE AFTER HEAVY STORM.
- PENETRATIONS WITH URETHANE SEALANT. F. ALWAYS GET MANUFACTURER APPROVAL BEFORE MAKING ANY
- MODIFICATION TO THE ROOF. WHEN PERFORMING ROOF MAINTENANCE ALWAYS TAKE THE FOLLOWING PRECAUTIONS
- A. USE FALL PROTECTION AND OTHER SAFETY EQUIPMENT AS REQUIRED.
- OR RIDGE FLASHING.
- C. DO NOT WALK ON LIGHT TRANSMITTING PANELS (LTP). THEY DO NOT SUPPORT A PERSON'S WEIGHT. GUARD ALL LTP'S AND ROOF OPENINGS
- STEP ONLY IN THE PANEL FLAT DIRECTLY ON OR IN CLOSE PROXIMITY TO A SUPPORTING ROOF STRUCTURAL.
- **KEY INSPECTION TIMES:**
- ROOF ARFA
- HAIL OR ABNORMALLY HEAVY RAINS OR ICE AND SNOW. AFTER OTHER TRADES HAVE BEEN ON THE ROOF FOR ANY REASON
- MUST BE PROTECTED DESIGNED AND INSTALLED ROOF WALKWAY SYS
- CORROSION POST-ERECTION ROOF CLEANING: AFTER THE ROOF INSTALLATION IS
- ANY RESULTING CORROSION.
- DEBRIS REMOVAL: ANY FOREIGN DEBRIS SUCH AS SAWDUST, DIRT, TIME. THE ROOF SHOULD BE PERIODICALLY INSPECTED FOR SUCH CONDITIONS AND IF FOUND. THEY SHOULD BE RECTIFIED IN A MANNER CONSISTENT WITH THESE ROOF MAINTENANCE GUIDELINES. CONTACT WITH ROOF PANELS, ESPECIALLY GALVALUME, FOR EXTENDED PERIOD OF TIME.

CONTINGENCIES

- THE CONTINGENCIES SPECIFIED IN THIS SECTION. LIMITS TO, THE FOLLOWING: CONSTRUCTION MANAGEMENT FEES.
- MATERIAL, DETAILING, FABRICATION, SHIPPING, INSTALLATION, AND INSPECTION COSTS. CONCRETE: 5CYREINFORCING STEEL: STRUCTURAL STEEL: 3.0 TONS C.F.M.F.:
- ANCHORS: THE SER.

4. OWNER/ CONTRACTOR TO PROVIDE TEMPORARY POWER TO ALLOW FOR A MAXIMUM POWER LEAD RUN OF 200 FEET TO EACH STRUCTURE BEING

6. OWNER/ CONTRACTOR MUST ENSURE THAT BUILDING PADS ARE BROOM CLEAN AND FREE OF DEBRIS PRIOR TO SELLER/ SUBCONTRACTOR CREW 7. OWNER/ CONTRACTOR SHALL ENSURE THAT THE SITE AND ALL SIDES OF

BUILDING ARE ACCESSIBLE WITH EQUIPMENT AND FREE FROM ANY 8. CLEANING OF MATERIALS, INCLUDING BUT NOT LIMITED TO WALL PANELS

9. THE OWNER/ CONTRACTOR SHALL PROPERLY PROTECT THE WORK FOR

HAZARDS WITH LIGHTS, GUARDRAILS OR BARRICADES AS APPLICABLE

A. WALK IN THE FLAT OF THE PANEL NEAR THE STRUCTURAL SUPPORTS. KEEP ROOF, GUTTERS AND DOWNSPOUTS FREE OF DEBRIS.

REMOVE EXCESS ICE AND SNOW ACCUMULATIONS AS NECESSARY. INSPECT AND RESEAL AS NECESSARY ALL ROOF CURBS AND OTHER

DO NOT WALK ON THE ROOF FLASHING SUCH AS GUTTER, RAKE, HIP,

A. AFTER A FIRE, VANDALISM, OR KNOWN DAMAGE TO AN ADJACENT

EXPOSURE TO SEVERE WEATHER CONDITIONS, INCLUDING HIGH WINDS,

INSPECT THE ROOF FOR DAMAGE CAUSED BY WORKERS INCLUDING CHEMICAL OR SOLVENT SPILLS, SCRATCHES IN THE FINISH, EXCESSIVE ROOF TRAFFIC, AND PUNCTURES. MAKE SURE THAT ANY DEBRIS OR SCRAP LEFT BEHIND BY THE WORKERS IS REMOVED FROM THE ROOF IMMEDIATELY. AVOID USING CUTOFF SAWS AND WELDING EQUIPMENT OVER THE ROOF IN CASES WHERE THIS IS NOT POSSIBLE, THE ROOF

FOOT TRAFFIC: KEEP FOOT TRAFFIC TO A MINIMUM. HEAVY FOOT TRAFFIC CAN CAUSE PONDING ON LOW PITCHED ROOFS. THIS IS A PARTICULARLY TRUE JUST ABOVE THE EAVE AND AT ENDLAPS. ALWAYS WALK IN A FLAT OF PANEL NEAR A SUPPORTING ROOF STRUCTURAL. DO NOT WALK ON TRIM OR IN GUTTERS. ON BARE GALVALUME ROOFS, EXCESSIVE FOOT TRAFFIC IS PLANNED FOR A ROOF. PROVISIONS SHOULD BE MADE FOR A PROPERLY

DRAINAGE: KEEP ROOF FREE OF DEBRIS AND KEEP DEBRIS OUT OF GUTTER TO ALLOW WATER TO QUICKLY DRAIN FROM ROOF. DO NOT USE WOOD BLOCKING TO HOLD UP EQUIPMENT OFF PANEL SEAMS. THIS BLOCKS THE FLOW OF WATER AND HOLDS MOISTURE. DO NOT ALLOW ROOFTOP A/C UNITS OR EVAPORATIVE COOLERS TO DRAIN ONTO THE ROOF. ANYTHING THAT TRAPS OF HOLDS MOISTURE ON A ROOF WILL CAUSE PREMATURE

COMPLETE, ALL FITTINGS, SHAVINGS, ETC. FROM FASTENER INSTALLATION, ETC. SHALL BE SWEPT COMPLETELY CLEAR OF THE ROOF PANELS. IF THIS DOES NOT HAPPEN, THE MANUFACTURER WILL NOT BE RESPONSIBLE FOR

LEAVES, ANIMAL DROPPINGS, ETC. WILL CAUSE CORROSION OF THE ROOF, GUTTERS, TRIM, ETC. IF LEFT ON BUILDING SURFACE FOR A LONG ENOUGH

NEVER ALLOW TREATED LUMBER OR CONCRETE/ MORTAR/ GROUT TO COME

THE CONTRACTOR IS TO PROVIDE AN ALLOWANCE IN THE BASE BID FOR

THE CONTINGENCIES ARE TO INCLUDE ALL COSTS ASSOCIATED WITH THE INSTALLATION OF THE STRUCTURAL COMPONENT. THIS INCLUDES BUT NOT

100 FEET, #G BAR

(30) 600-5-168-68 10'-0" LG. STUDS, 100 FT. OF 600-T-125-54. (20) ³/₄ ∅ x 1'-0" LG. ALL CONTINGENCIES TO BE FABRICATED AND INSTALLED AS DIRECTED BY

MAINTENANCE NOTES

TERMITE PROTECTION

- ALL BUILDING PADS ARE TO BE PROTECTED FOR TERMITES AS REQUIRED BY GOVERNING CODE AND LOCAL JURISDICTION.
- FIRE RESISTANCE RATING REQUIREMENTS FOR BUILDING ELEMENTS CONSTRUCTION TYPES II-B ∉ V-B ALL BUILDING ELEMENTS RATINGS ARE ZERO (O).

FIRE RESISTANCE RATING REQUIREMENTS FOR EXTERIOR WALLS

CONSTRUCTION TYPES II-B ∉ V-B (ALL USE GROUPS EXCEPT H) REQUIRE A MINIMUM FIRE SEPARATION DISTANCE OF TEN (10) FEET FOR A ZERO (0) RATING.

ERCENTAGE OF OPENINGS IN EXTERIOR WALLS

BUILDINGS WHOSE EXTERIOR WALL AND STRUCTURAL FRAME ARE NOT REQUIRED TO BE FIRE-RESISTANCE RATED SHALL BE PERMITTED TO HAVE UNLIMITED UNPROTECTED OPENINGS.

FIRE BARRIERS/ FIRE WALLS/ STRUCTURE DESIGN W/O SPRINKLERS

STORAGE OCCUPANCIES DESIGNED WITHOUT SPRINKLER SYSTEM CAN'T BE MORE THAN 3 STORIES CONTAIN FIRE AREA GREATER THAN 12,000 SF, OF HAVE COMBINED FIRE AREAS GREATER THAN 24,000 SF. OUR TYPICAL DESIGN STANDARD IS TO USE 3 HOUR FIRE BARRIERS TO DIVIDE BUILDING INTO FIRE AREAS AND FIRE WALLS TO SEPARATE LARGE STRUCTURES INTO SEPARATE BUILDING TO AVOID EXCEEDING MAXIMUM COMBINED FIRE AREAS.

CORRIDOR FIRE RESISTANCE RATING

IT IS THE INTENT OF DESIGN TO KEEP OCCUPANCY LOAD SERVED BY A CORRIDOR LESS THAN OR EQUAL TO COMPLY WITH ZERO RATING IN A BUILDING WITHOUT SPRINKLERS.

FIRE RATED DOOR AND HARDWARE

FIRE DOORS AND FRAME SHALL BE LABELED SHOWING THE NAME OF MANUFACTURER, THE NAME OF THE THIRD-PARTY INSPECTOR AGENCY, AND THE FIRE PROTECTION RATING. ALL RATED DOORS ARE TO BE INSTALLED WITH A POSITIVE STRIKE DEVICE, AND BE SELF-CLOSING.

PROTE	CTION	LIMITING SIZES OF						
RATII	NGS	WIRED GLASS						
TYPE	MINIMUM RATING (HOURS)	MAX. AREA (SQ. INCHES)	HEIGHT	MAX. WIDTH (INCHES)				
FIRE BARRI	FIRE BARRIERS							
3 HOUR	3	0	0	0				
2 HOUR	/2	100	33	10				
I HOUR	Ι	100	33	10				
RATED EXT	ERIOR WALLS							
3 HOUR	/2	0	0	0				
2 HOUR	/2	0	0	0				
I HOUR	3/4	1296	54	54				
FIRE PARTI	TIONS							
I HOUR	1/3	1296	54	54				

HARDWARE REQUIREMENTS FOR RATED DOORS

DOOR SHALL BE SELF OR AUTOMATIC CLOSING. DOORS SHALL BE PROVIDED WITH AN ACTIVE LATCH BOLT THAT WILL SECURE THE DOOR WHEN IT IS CLOSED

- 3. DOOR AND FRAME SHALL BE LABELED SHOWING NAME OF MANUFACTURER AND THE THIRD-PARTY INSPECTION AGENCY.

COMMON PATH OF TRAVEL

IF A BUILDING'S OCCUPANCY LOAD IS LESS THAN 50 THEN PERSONNEL DOORS ARE ALLOWED TO SWING BOTH IN THE DIRECTION OF EGRESS AND AGAINST THE DIRECTION OF EGRESS THUS INSTALLING EXIT LIGHTS ON BOTH SIDE OF PERSONAL DOORS IN CORRIDORS WOULD MAKE THE CORRIDOR A TWO-WAY PATH VERSUS A SINGLE-DIRECTION PATH.

ACCESSIBLE AREA REQUIREMENTS

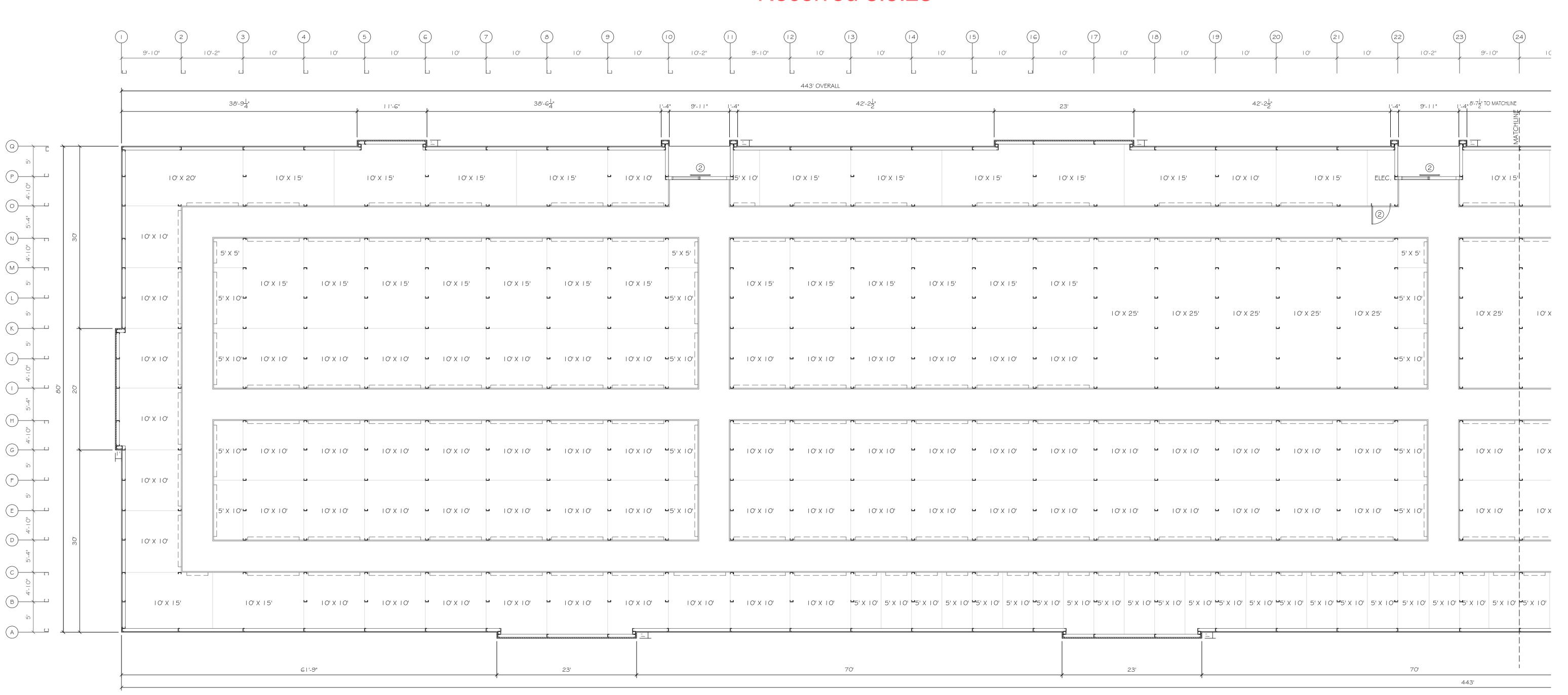
HANDLES, PULLS, LATCHES, LOCKS, AND OTHER OPENING DEVICES SHALL HAVE A LEVER OPERATED MECHANISM. IF THE DOOR HAS A CLOSER, THEN THE SWEEP PERIOD OF THE CLOSER SHALL BE ADJUSTED SO THAT FROM AN OPEN POSITION OF 70 DEGREES, THE DOOR WILL TAKE AT LEAST 3 SECONDS TO MOVE TO A POINT 3 INCHES FROM THE LATCH MEASURED TO THE LEADING EDGE OF THE DOOR. THE MAXIMUM FORCE FOR PUSHING OR PULLING OPEN A DOOR SHALL BE AS FOLLOWS: EXTERIOR DOOR ≤ 8.5 LBF,

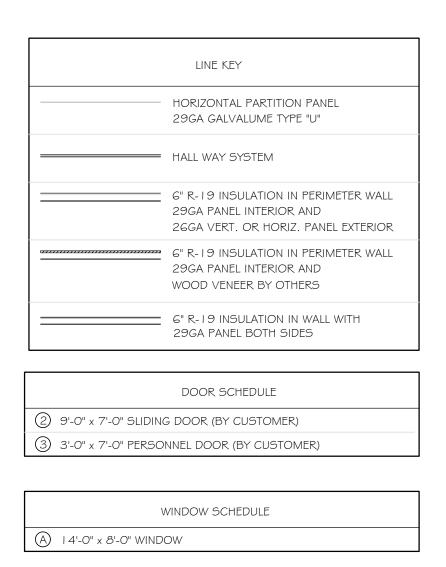
INTERIOR DOOR < 5 LBF.

REFERENCE THE DOOR DETAIL PAGE FOR DETAILS WITH REGARDS TO PROPER INSTALLATION.

			EAST COAST OFFICE: 1041 CROWN PARK CIRCLE WINTER GARDEN FL		CONFIDENTIAL: THIS DOCUMENT AND THE INFORMATION MAKORABCO UNAUTHORIZED COPYING, DISCLOSURE OR C
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and da docum and se verifie 08/0	ate. Prenet a contract of the second	rinted re not and th any e	cop t cop	lectronicall Robert M. Digital Sign bies of this nsidered sig gnature mu ronic copie Digitally sig by Robert B Date: 2023.0 14:46:26-0	ature gned list be s. ned eattie 08.04
RBE CONSULTING SERVICES, A	2875 SPLIT DAK COURT - OVIEDO, FL. 32766 WEBBITE: WWW.RBECS.COM - EMAIL: RBECSLLC@GMAIL.COM MANN PHONE LINE: 407-796-3820 - FAX LINE: 407-971-6718	MECHANICAL DEPT: 407-796.3776 - EMAIL: NSHAH@REECS.COM ELECTRCAL DEPT: 407-56.17.3113 - EMAIL: FAMILBIA@RAECS.COM STRUCTURAL DEPT: 407-568-7658 - EMAIL: REEATTE@REECS.COM	ROBERT M. BEATTIE, PE, FLORIDA PE #55428 FLORIDA CERTIFICATE OF AUTHORIZATION #27269	REE CONSULTING SERVICES, LLC* HEREBY EXPECSES RESERVES ITS COMMON LAW COPRIGATI AND OTHER PROPERTY RIGHT'S TO THESE FLANS AND THE DESIGN CONTAINED WITHIN. THIS DESIGN AND DRAWINGS STALL NOT TO BE ATTERD. COPIED OR ERPRODUCED IN MAY PORM OR MANUEM MITHOLIN THE FUPERES WRITTEN	UN N
SHEET		L ENERA	AL M	NOTES	2-23
DRAW CHECI		ЗY		AW/	MNG XXX 2705

ISSUE





GC/CUSTOMER TO PROVIDE DISTANCE TO HOLD DOWN PARTITIONS FROM BOTTOM OF ROOF DECK BEFORE ANY MATERIALS CAN BE ORDERED.

PIER & HEADER FINISHES:

PREPAINTED PIERS & HEADERS

- THERE IS NO ADDITIONAL APPLICATIONS REQ'D. • REMOVE THE PROTECTIVE FILM FROM AREAS OF BOTH PIERS & HEADERS WHERE THEY CONTACT EACH OTHER.
- LEAVE THE FILM ON TO PROTECT THE PREPAINTED SURFACE UNTIL • CONSTRUCTION IS NEAR COMPLETION.

GALVANEAL PIERS ≰ HEADERS REQUIRING FIELD PAINTING.

- VERIFY PAINT COLOR WITH COLOR REQ'D IN LABOR CONTRACT. • CAULK PIERS TO HEADERS AND HEADERS TO HEADERS IF STACKED.
- PRIME ALL EXPOSED SURFACES OF PIERS & HEADERS.
- PAINT ALL EXPOSED SURFACES OF PIERS & HEADERS WITH PAINT SPECIFIED IN LABOR CONTRACT EVENLY SO PRIMER CAN NOT BE SEEN THROUGH TOP COAT.
- IF TEXTURED, ROLL OUT TO ACHIEVE AND SAND FINISH STUCCO LIKE LOOK.

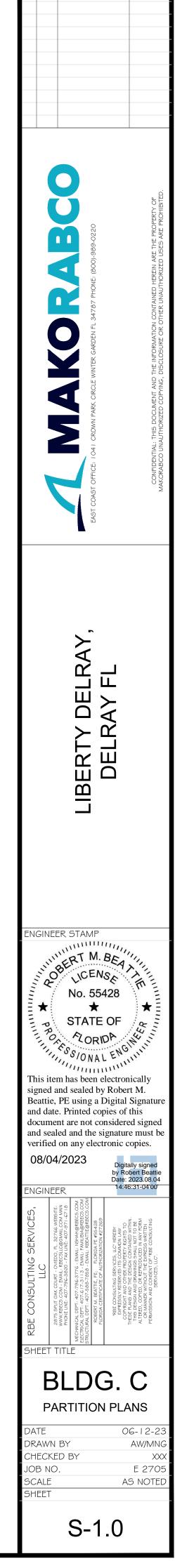


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BLDG. PARTITION PLAN

|/8" = |'



." 9'- "	I,'-4", 8'-7 []] " TO MATCHLINE		38'-7" TO N	NATCHLINE		2	3'			47'-2 <u>1</u> "		443' OVERA	LL '-4" 9'-11"	'-4"	34'-8 <u>4</u>	Ţ"	 	'-6"
2			IO'X I5' ■=======	iO'X 15'	j	IO'X I5' <u>→</u> ——————	╹ IO'X I5' <u></u>	j.	IO'X I5' 	L IO'X I5'	,	IO'X I5'			- IO'X I5'		IO'X 15'	
Г 5' X 5' L		•			•		•	•	• <u> </u>		•	-	5' X 5' L					
5' X 10'	- I O' X 25'	I O' X 25'	IO'X I5'	10' X 15'	IO'X I5'	IO'X I5'	10' X 15'	IO'X I5'	IO'X I5'	IO'X I5'	O' X 5' ,	I O' X I 5'	⊷5' X O' 	IO'X 5'	IO'X I5'	IO'X I5'	IO'X I5'	
5' X O' 	-									• IO'XIO' •					• IO'XIO'			
		•														_		
5' X 10' 	- IO'XIO'	• 10' X 10'	• 10' X 10'	• IO'XIO'	- 10' X 10'	• 10' X 10'	- 10'X10'	• 10' X 10'	➡ 10' X 10'	- IO'XIO' -	• 0'X 0'	- 10' X 10'	•5' X 10'	- IO'XIO'	- 10'X10'	u 10' X 10'	• 10' X 10'	
 5' X 0' 	• 10'X 10'	• 10'X 10' •	• 10'X10'							• 10'X10' •				- IO'XIO'	• 10' X 10'	u 10'X10'	• 10' X 10'	י ו
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	LINE KEY
	HORIZONTAL PARTITION PANEL 29GA GALVALUME TYPE "U"
	HALL WAY SYSTEM
	6" R- I 9 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND 26GA VERT. OR HORIZ. PANEL EXTERIOR
	6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND WOOD VENEER BY OTHERS
	6" R-19 INSULATION IN WALL WITH 29GA PANEL BOTH SIDES
	DOOR SCHEDULE
2 9'-0" x 7'-0" SLIDING	G DOOR (BY CUSTOMER)
3'-0" x 7'-0" PERSON	NNEL DOOR (BY CUSTOMER)
W	VINDOW SCHEDULE
A 14'-0" x 8'-0" WINDO	W

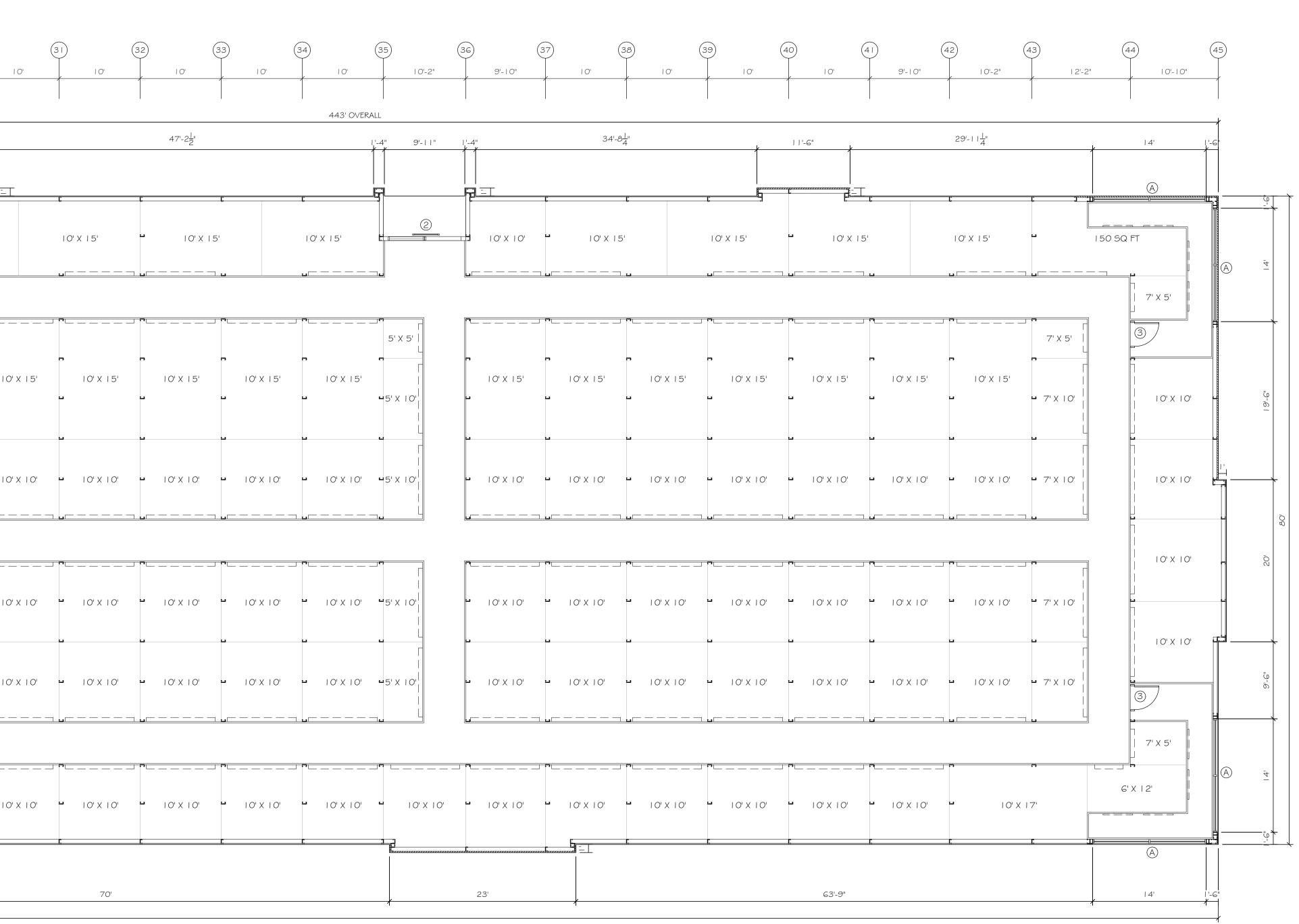
NOTE: GC/CUSTOMER TO PROVIDE DISTANCE TO HOLD DOWN PARTITIONS FROM BOTTOM OF ROOF DECK BEFORE ANY MATERIALS CAN BE ORDERED.

PIER & HEADER FINISHES:

PREPAINTED PIERS & HEADERS • THERE IS NO ADDITIONAL APPLICATIONS REQ'D.

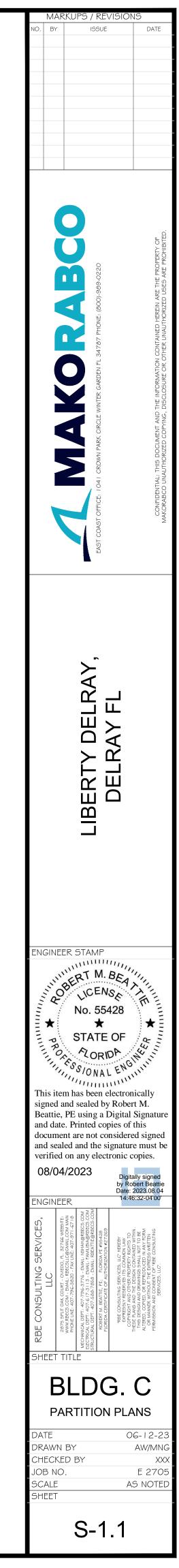
- REMOVE THE PROTECTIVE FILM FROM AREAS OF BOTH PIERS & HEADERS WHERE THEY CONTACT EACH OTHER.
- LEAVE THE FILM ON TO PROTECT THE PREPAINTED SURFACE UNTIL CONSTRUCTION IS NEAR COMPLETION.
- GALVANEAL PIERS & HEADERS REQUIRING FIELD PAINTING.
- VERIFY PAINT COLOR WITH COLOR REQ'D IN LABOR CONTRACT.
- CAULK PIERS TO HEADERS AND HEADERS TO HEADERS IF STACKED. • PRIME ALL EXPOSED SURFACES OF PIERS & HEADERS.
- PAINT ALL EXPOSED SURFACES OF PIERS & HEADERS WITH PAINT SPECIFIED IN
- LABOR CONTRACT EVENLY SO PRIMER CAN NOT BE SEEN THROUGH TOP COAT. • IF TEXTURED, ROLL OUT TO ACHIEVE AND SAND FINISH STUCCO LIKE LOOK.

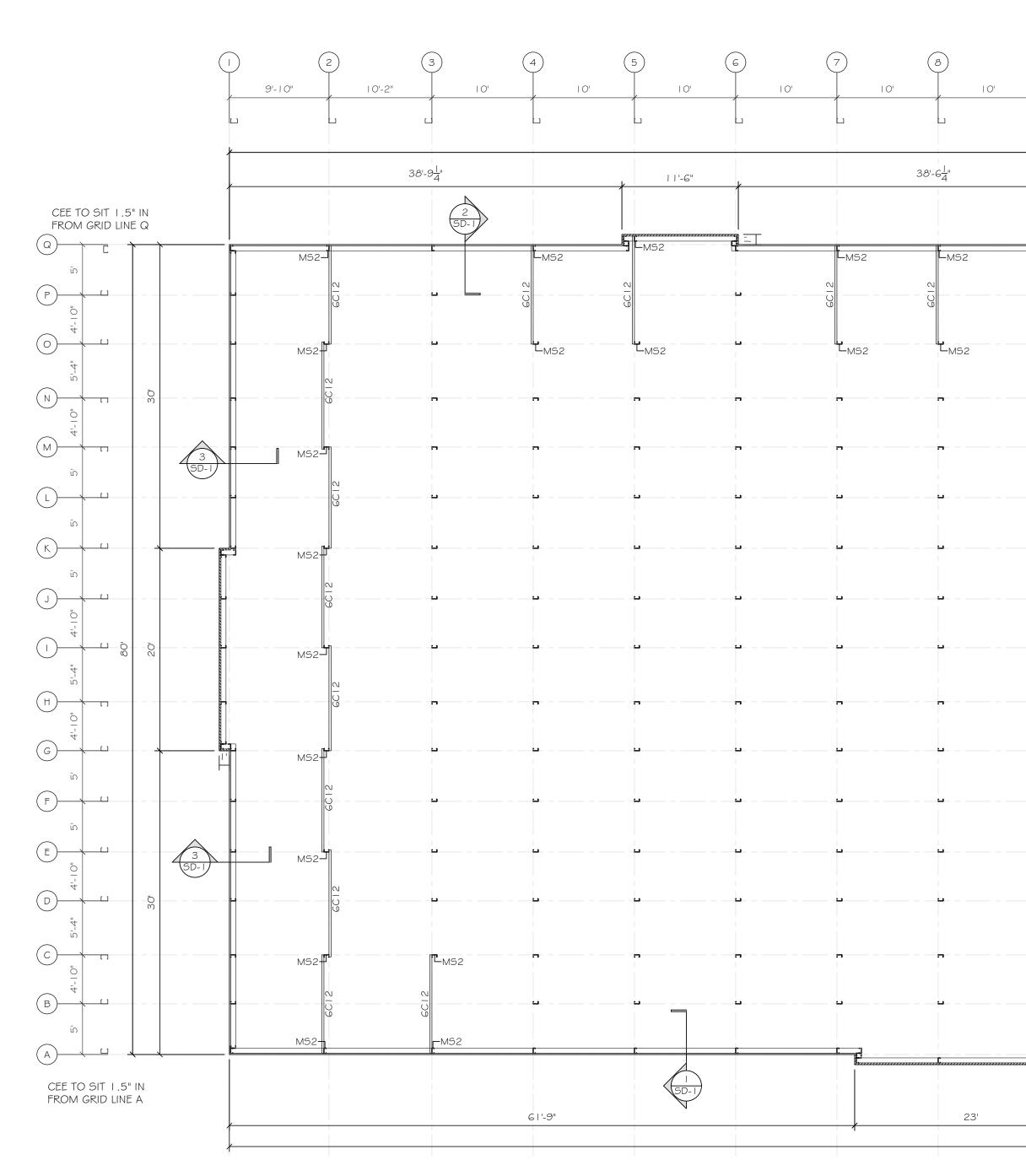






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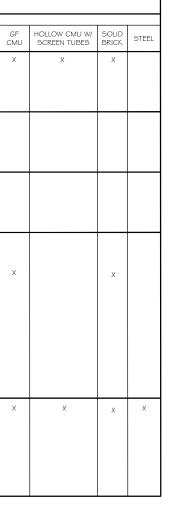




	LINE KEY
BOX HDR	FRAMED COLUMN OPENING W/ BOX HEADER
	6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND 26GA VERT. OR HORIZ. PANEL EXTERIOR
	6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND WOOD VENEER BY OTHERS
	6" R-19 INSULATION IN WALL WITH 29GA PANEL BOTH SIDES
	CROSS BEAMS NEED MORE THAN 5' BAY
L	COLUMNS
	ZEE BEAMS
TYPICAL COLUMNS	¢ BEAMS UNLESS NOTED OTHERWISE
COLUMN : MSI = 6CI	6 - 6"X2 1/2" X 16GA CEE PRIME PAINTED
COLUMN : MS2 = 6C1	4 - 6"X2 1/2" X 14GA CEE PRIME PAINTED

PURLIN : PI = 6ZI6 - 6"X2 I/2" X I6GA ZEE PRIME PAINTED

ANCHOR SCH	IEDULE			S AND ANCHORS	ED PER MANUFACTURE	r's recomme	ENDATIONS AN	D SPEC		IONS
BRAND: DeWA	LT (OR EQUAL)									
ANCHOR TYPE	ANCHOR NAME	ICC-ES #	ANCHOR DIA.(IN)	MIN BASE MATERIAL THICKNESS (IN)	(MIN. OR NOM) / MAX. OR REF.) EMBEDMENT (IN.)	MIN ANCHOR SPACING (IN)	MIN EDGE / END DISTANCE (IN)	ALLOWA	BLE BAS	E MATERIAL CONC. OVER STEEL DECK
EPOXY	AC100+GOLD	ESR-2582	1/2"dıa.	4" [CONC]	2-3/4"(MIN) / 6" MAX	2-1/2"	2-1/2"	X	x	
		[CONC]	3/4"dıa.	5-3/4" [CONC]	3-1/2"(MIN) / 9" MAX	3-3/4"	3-3/4"			
		ESR-3200	1/2"dia.	8" GF CMU	4" (MIN)	(I) PER CELL	4"	1		
		[CMU]	3/4"dıa.	8" GF CMU	G" (MIN)	(I) PER CELL	4"	1		
MECHANICAL	POWER STUD	ESR-2818	1/2"dıa.	4" [CONC]	2-1/2"(NOM) / 2" (EFF)	4-1/2"	6"	х	х	х
	+SD1	[CONC]	3/4"dıa.	6" [CONC]	4"(NOM) / 2" (EFF)	6"	6"	1		
		ESR-2966	3/8"dıa.	8" GF CMU	2-1/2"(MIN)	(I) PER CELL	4"	1		
		[CONC]	1/2"dia.	8" GF CMU	4-3/4"(MIN)	(I) PER CELL	12"	1		
	POWER STUD	ESR-2502	1/2"dia.	4-1/2" [CONC]	2-1/2"(NOM) / 2" (EFF)	6"	8"	x	x	x
	+SD2	[CONC]	3/4"dıa.	7" [CONC]	4-1/2"(NOM) / 3-3/4" (EFF)	6"	12"	1		
			3/8"dıa.	$2\frac{1}{2}$ " [CONC OVER STL DECK]	2-3/8"(NOM) / 2" (EFF)	- /2"	4"	1		
			1/2"dia.	2 ¹ / ₂ " [CONC OVER STL DECK]	2-1/2"(NOM) / 2" (EFF)	8"	4"	1		
	SCREW-	ESR-3889	1/4"dıa.	8" GF [CONC]	2-1/2" (MIN.)	4"	2"			
	BOLT+	[CONC]	1/2"dia.	8" GF [CONC]	3-1/4" (MIN.)	6"	3"	1		
			1/2"dia.	3" [CONC]	-5/8"(NOM) / .2" (EFF)	- /2"	- /2"	l _x	x	x
			3/4"dıa.	3" [CONC]	2"(NOM) / 1-1/3" (EFF)	2"	- /2"	1		
			3/8"dıa.	3" [CONC]	2-1/2"(NOM) / 1-3/4" (EFF)	2-3/4"	I -3/4"]		
			1/2"dıa.	3-3/4" [CONC]	4-1/4"(NOM) / 3.08" (EFF)	3"	I -3/4"]		
			1/4"dıa.	2 [⊥] / ₂ " [CONC OVER STL DECK]	I -5/8"(NOM) / I .2" (EFF)	- /2"	- /2"			
			3/8"dıa.	$2\frac{1}{2}$ " [CONC OVER STL DECK]	2"(NOM) / 1.33" (EFF)	2"	2"]		
			1/2"dıa.	2 ¹ / ₂ " [CONC OVER STL DECK]	2-1/2"(NOM) / 1-3/4" (EFF)	2-1/2"	2-1/2"			
		ESR-4042	1/2"dıa.	8" GF CMU	4-1/4"(MIN)	(I) PER CELL	4"			
		[CMU]	3/4"dıa.	8" GF CMU	6-1/4"(MIN)	(I) PER CELL	4"			
DIRECT FASTEN	CSI SPIRAL DRIVE PINS	ESR-2024	0157.dia	3/16" (STEEL)	FASTENER POINT MUST PENETRATE STEEL MEMBER	- /2"	1/2"	х	х	х
			0157.dia	2-1/4" [CONC]	3/4"(MIN)	4"	3-1/2"			
			0157.dıa	3" [CONC]	l"(MIN)	4"	3-1/2"			
			0157.dia	3-3/4" [CONC]	- /4"(MIN)	4"	3-1/2"			
			0157.dia	8" GF CMU	L"(MIN)	4"	3-3/4"]		
			0157.dia	8" HOLLOW CMU	l "(MIN)	8"	3-3/4"]		



ANCHOR SCHEDULE UNLESS NOTED OTHERWISE
BOTTOM FLOOR (MULTI)
ASE TRACK INSTALLATION /2"X3" DeWALT SCREW BOLT+ (OR EQUAL) SPACING 3' O.C. (1 3/4" EFFECTIVE EMBEDMENT)
IER INSTALLATION /2"X3" DeWALT SCREW BOLT+ (OR EQUAL) @ EA. PIER (1 3/4" EFFECTIVE EMBEDMENT)
EAVY STEEL DECK SUPPORT ANGLE (1/4" THICK OR GREATER) (TO CMU) /4"X6" DeWALT SREW BOLT+ (OR EQUAL) 2' O.C. (5" MIN. EMBEDMENT) (4" MIN. FROM EDGE OF CMU)
/4" MIN. EXPANSION ANGLE INSTALLATION (TO CMU) 3) 3/4"XG" DeWALT SREW BOLT+ (OR EQUAL) (5" MIN. EMBEDMENT) (4" MIN. FROM EDGE OF CMU)
SS STEEL COLUMN BASE PLATE INSTALLATION SE DeWALT AC200+ EPOXY TO INSTALL. DIAMETER TO MATCH ANCHOR IN COLUMN SCHEDULE X I 2" DNG THREADED ROD WITH LEVELING NUTS (7" MIN. EMBEDMENT)

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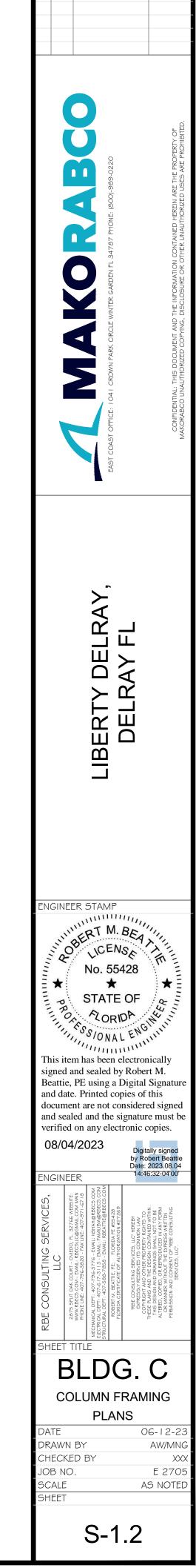


NORTH COLUMN FRAMING PLAN

(12)(18)| 0'-2" 9'- | 0" 1 O' | O' | O' 10' 443' OVERALL 42'-2<u>1</u>" 9'-11" 23' 1'-4" SD-4 C, (2 (SD--MS2 -MSIII-6I/8" -MS2 -MS2 MSI 11-2 7/8" MSI ||'-| 5/8" MSI 11'-0 3/8" MSI 10'-11 1/8" MSI 10'-9 7/8" MSI 10'-8 5/8" -MSI 10'-7 3/8" -MSI 10'-6 1/4" -MSI 10'-4 7/8" MSI 10'-3 5/8" MSI 10'-2 3/8" MSI 10'-1 1/8" MSI 9'-II 7/8" MSI 9'-10 5/8" MSI 9'-9 3/8" -MSI 9'-I0 I/4" SD-I 70' 23'

Received 8.9.23



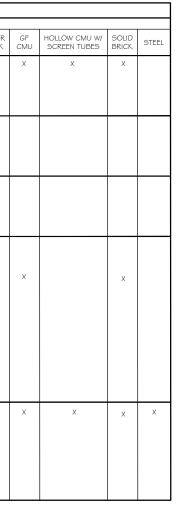


(22) O'-2"	(23) 9'-10''	10'	25 26 10'	5) (27 10') (28) I O'	10'	9) (3 10'	0 (3 0'	I) (3 	2) (33) , IO'	(34) I O'	(35) 10' 10'-2"	(36) (37) 9'- I 0"	(38) 10' 10'	(39) (40) I O'
'-4" 9'- "	и д. 8'-7 ¹ " ТО МАТСНЫЛЕ		38'-7" TO MA			23	21			47'-2 ¹ / ₂ "		443' OVERALL	11 /10	34'-8 <u>1</u> "	. '-6"
			2 5D-1		H			+		2 5D-1				4	
		LMS2			M52	52			-M52		52			MS2	
			O O O					0			U U U			Contraction of the second seco	O O
			L _{MS2}		MS2 LMS	52	• I	LMS2	MS2		52 ^L M52			L _{MS2}	-M52
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70'				23'	*			Y	70'			1	23'		\forall
	443'														

	LINE KEY
BOX HDR	FRAMED COLUMN OPENING W/ BOX HEADER
	6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND 26GA VERT. OR HORIZ. PANEL EXTERIOR
<u></u>	6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND WOOD VENEER BY OTHERS
	6" R-19 INSULATION IN WALL WITH 29GA PANEL BOTH SIDES
	CROSS BEAMS NEED MORE THAN 5' BAY
Ľ	COLUMNS
	ZEE BEAMS
TYPICAL COLUMNS	# BEAMS UNLESS NOTED OTHERWISE
· · ·	I G - G"X2 I/2" X I GGA CEE PRIME PAINTED
COLUMN : MS2 = 6C	4 - 6"X2 1/2" X 14GA CEE PRIME PAINTED

PURLIN : PI = GZIG - G"X2 I/2" X IGGA ZEE PRIME PAINTED

ANCHOR SCH	IEDULE			<u>RS AND ANCHORS</u> ENERS SHALL BE INSTALLE	D PER MANUFACTURE	r's recomme	ENDATIONS AN	D SPEC		IONS
BRAND: DeWA	LT (OR EQUAL)	I								
ANCHOR TYPE	ANCHOR NAME	ICC-ES #	ANCHOR DIA.(IN)	MIN BASE MATERIAL THICKNESS (IN)	(MIN. OR NOM) / MAX. OR REF.) EMBEDMENT (IN.)	MIN ANCHOR SPACING (IN)	MIN EDGE / END DISTANCE (IN)	ALLOWA	BLE BAS	E MATERIAL CONC. OVER STEEL DECK
EPOXY	AC100+GOLD	ESR-2582	1/2"dıa.	4" [CONC]	2-3/4"(MIN) / 6" MAX	2-1/2"	2-1/2"	х	х	
		[CONC]	3/4"dia.	5-3/4" [CONC]	3-1/2"(MIN) / 9" MAX	3-3/4"	3-3/4"			
		ESR-3200	1/2"dia.	8" GF CMU	4" (MIN)	(I) PER CELL	4"			
		[CMU]	3/4"dia.	8" GF CMU	G" (MIN)	(I) PER CELL	4"			
MECHANICAL	POWER STUD	ESR-2818	1/2"dia.	4" [CONC]	2-1/2"(NOM) / 2" (EFF)	4-1/2"	6"	х	х	х
	+SD1	[CONC]	3/4"dia.	6" [CONC]	4"(NOM) / 2" (EFF)	6"	6"			
		ESR-2966	3/8"dia.	8" GF CMU	2-1/2"(MIN)	(I) PER CELL	4"	1		
		[CONC]	CONC] 1/2"dia. 8" GF CMU 4-3/4"(MIN)	4-3/4"(MIN)	(I) PER CELL	12"				
	POWER STUD	ESR-2502	1/2"dia.	4-1/2" [CONC]	2-1/2"(NOM) / 2" (EFF)	6"	8"	х	х	x
	+SD2	[CONC]	3/4"dia.	7" [CONC]	4-1/2"(NOM) / 3-3/4" (EFF)	6"	12"		~	~
			3/8"dia.	2≟" [CONC OVER STL DECK]	2-3/8"(NOM) / 2" (EFF)	- /2"	4"			
			1/2"dia.	2≟" [CONC OVER STL DECK]	2-1/2"(NOM) / 2" (EFF)	8"	4"	İ		
	SCREW-	ESR-3889	1/4"dia.	8" GF [CONC]	2-1/2" (MIN.)	4"	2"			
	BOLT+	[CONC]	1/2"dia.	8" GF [CONC]	3-1/4" (MIN.)	6"	3"			
			1/2"dia.	3" [CONC]	I -5/8"(NOM) / I .2" (EFF)	- /2"	- /2"	x	x	x
			3/4"dia.	3" [CONC]	2"(NOM) / 1-1/3" (EFF)	2"	- /2"		~	~
			3/8"dia.	3" [CONC]	2-1/2"(NOM) / 1-3/4" (EFF)	2-3/4"	I -3/4"	1		
			1/2"dia.	3-3/4" [CONC]	4-1/4"(NOM) / 3.08" (EFF)	3"	I -3/4"	1		
			1/4"dia.	$2\frac{1}{2}$ [CONC OVER STL DECK]	1-5/8"(NOM) / 1.2" (EFF)	- /2"	- /2"			
			3/8"dia.	$2\frac{1}{2}$ " [CONC OVER STL DECK]	2"(NOM) / 1.33" (EFF)	2"	2"	1		
			1/2"dia.	2 []] [CONC OVER STL DECK]	2-1/2"(NOM) / 1-3/4" (EFF)	2-1/2"	2-1/2"	1		
		ESR-4042	1/2"dia.	8" GF CMU	4-1/4"(MIN)	(I) PER CELL	4"	1		
		[CMU]	3/4"dıa.	8" GF CMU	6-1/4"(MIN)	(I) PER CELL	4"	1		
DIRECT FASTEN	CSI SPIRAL DRIVE PINS	ESR-2024	0157.dia	3/16" (STEEL)	FASTENER POINT MUST PENETRATE STEEL MEMBER	- /2"	1/2"	х	х	х
			0157.dia	2-1/4" [CONC]	3/4"(MIN)	4"	3-1/2"			
			0157.dia	3" [CONC]	l "(MIN)	4"	3-1/2"			
			0157.dia	3-3/4" [CONC]	- /4"(MIN)	4"	3-1/2"			
			0157.dia	8" GF CMU	I "(MIN)	4"	3-3/4"			
			0157.dia	8" HOLLOW CMU	I "(MIN)	8"	3-3/4"	1		



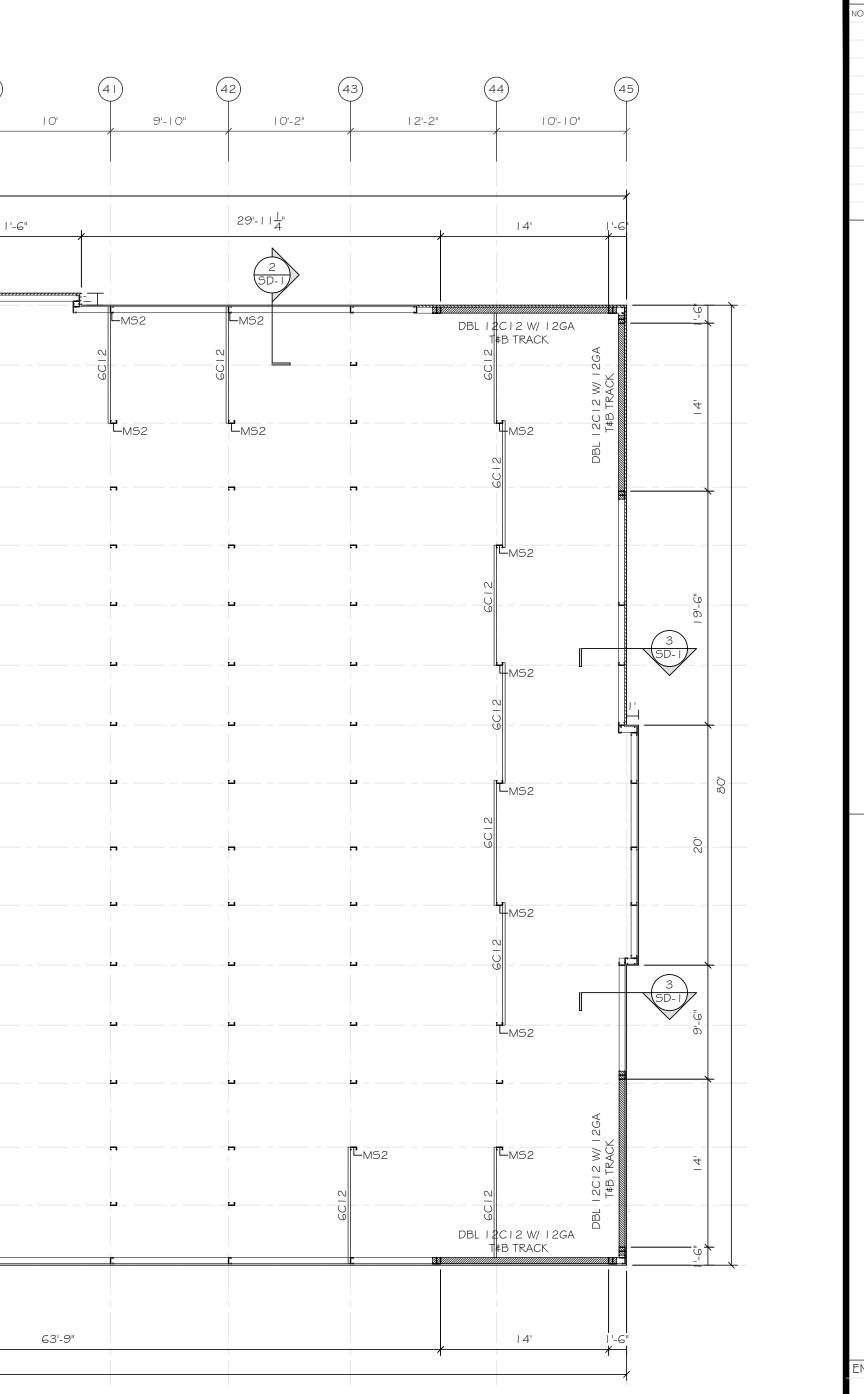
ANCHOR SCHEDULE UNLESS NOTED OTHERWISE
BOTTOM FLOOR (MULTI)
ASE TRACK INSTALLATION '2"X3" DeWALT SCREW BOLT+ (OR EQUAL) SPACING 3' O.C. (1 3/4" EFFECTIVE EMBEDMENT)
ER INSTALLATION '2"X3" DeWALT SCREW BOLT+ (OR EQUAL) @ EA. PIER (1 3/4" EFFECTIVE EMBEDMENT)
EAVY STEEL DECK SUPPORT ANGLE (1/4" THICK OR GREATER) (TO CMU) 4"XG" DeWALT SREW BOLT+ (OR EQUAL) 2' O.C. (5" MIN. EMBEDMENT) (4" MIN. FROM EDGE OF CMU)
'4" MIN. EXPANSION ANGLE INSTALLATION (TO CMU)) 3/4"X6" DeWALT SREW BOLT+ (OR EQUAL) (5" MIN. EMBEDMENT) (4" MIN. FROM EDGE OF CMU)
55 STEEL COLUMN BASE PLATE INSTALLATION BE DeWALT AC200+ EPOXY TO INSTALL. DIAMETER TO MATCH ANCHOR IN COLUMN SCHEDULE X I 2" DNG THREADED ROD WITH LEVELING NUTS (7" MIN. EMBEDMENT)

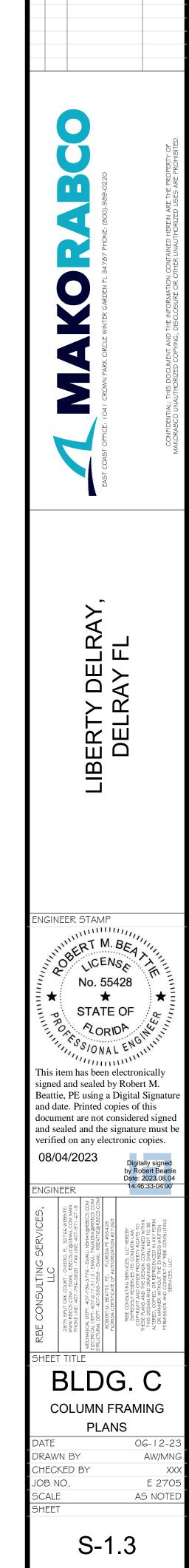


NORTH BLDG. COLUMN FRAMING PLAN

|/8" = |'

Received 8.9.23





ISSUE

	9'-10"	2 10'-2" 10'				10 ¹ 10 ¹ -2"	9'-10" 10'					21) (22) 10' 10'-2"	23 9'- 0" C
		38'-9 <u>1</u> "		'-6" L	38'-6 <u>1</u> "	'-4" 9'- "	443' OVE	ERALL 42'-2 ¹ / ₂ "	23		42'-2 ¹ / ₂ "	<u>'</u> -4" 9'- "	1'-4" 8'-7 ¹ " TO MATCHLINE
TO SIT 1.5" IN M GRID LINE Q			B										
		PI	PI	P I	PI	PI	P1	 PI	PI	PI	PI	PI	
		PI	PI	PI	PI	P1	PI	PI	PI	PI		P1	PI
		P1	P1	P1	PI	P1	PI	P1	P1	P1	PI	PI	Pli
		P1	PI	PI	P1	P1	PI	P1	P1	P1		PI	PI
а 		P1	P1	P1	PI	PI	PI	P1	PI	P1	PI	P1	
		P1	P1	P1	PI	P1	PI	P1	PI	P1	PI	PI	PI
ش 		P1	PI						PI		PI	P1	P
		PI	Pil	Pil	PI	PI	PI	PI	PI	PI	PI	PI	PI
-) , , , , , , , , , , , , , , , , , , ,						PI			P1				
		PI	P1	PI	PI	PI	PI	PI	PI	PI	PI	PI	PI
		PI	PI	PI	PI	PI	PI	PI	PI	PI	PI	P1	P
		PI	P1	PI	PI	PI	PI	PI	PI	PI	PI	PI	P
t 1 1 2 0 2 0 2 0 2 0 2 0 2 0 2 0 2 0 2 0									PI				
		PI	PI	PI	PI	PI	PI	PI	PI	PI	PI	P1	
		PI	P1	PI	PI	PI	PI	PI	PI	PI	PI	PI	Pli
TO SIT 1.5" IN A GRID LINE A			61'-9"		23'			70'		23'		70'	113
	/												443'

	LINE KEY
BOX HDR	FRAMED COLUMN OPENING W/ BOX HEADER
	6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND 26GA VERT. OR HORIZ. PANEL EXTERIOR
	6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND WOOD VENEER BY OTHERS
	6" R-19 INSULATION IN WALL WITH 29GA PANEL BOTH SIDES
	CROSS BEAMS NEED MORE THAN 5' BAY
	COLUMNS
	ZEE BEAMS
	
TYPICAL COLUMNS &	# BEAMS UNLESS NOTED OTHERWISE
COLUMN : MSI = GCI	G - G"X2 1/2" X 16GA CEE PRIME PAINTED

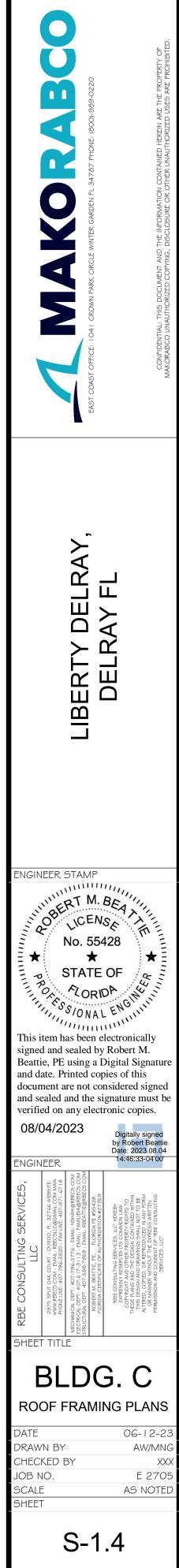
COLUMN : MS2 = 6C14 - 6"X2 1/2" X 14GA CEE PRIME PAINTED PURLIN : PI = 6ZI6 - 6"X2 I/2" X I6GA ZEE PRIME PAINTED





NORTH BLDG. ROOF FRAMING PLAN

|/8" = |'



ISSUE

(22)	23 9'-10" 24 10' 10' 10'	(26) 0' 0' 0'	28) (29) (30) 10' 10' 10'	(31) 10' 10'	33) (34) (10' 10'	35) (36) (37) 0'-2" 9'-10"	(38) (39) 10' 10' 10		(42) (43) - 0" 0'-2"	(44) 2'-2" 0'- 0"	45)
					443' OVERAL						
'-4" 9'-11"	38'-7 1'-4" 8'-7 2" TO MATCHLINE	7" TO MATCHLINE	23'	47'-2 ¹ / ₂ "	- 	4" 9'- " '-4" 7 7	34'-8 ¹ / ₄ "	11'-6"	29'-1 1/4"	14' 1'-6	7 6"
		H						₽			
				PI			PI		PI	PI	
PI		PIF	PI PI	P1	P1	PI	PI	PI	PI	P1	
P1		PIF	PI	PI	PI	P1	PI	PI	PI	P1	
P1	PI	P1 F	PI PI	P1	PI	P1	PI	PI	PI	P1	
P1		PI F	PI PI	P1	PI	P1	PI	P1	PI	PI	
P1	PI	P1F	PI	P1	PI	PI	PI	P1	P1	P1	
PI	PI	PI F	PI	PI	PI	PI	PI	PI	PI	PI	
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											8
PI			PI PI							PI	50
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PI		PI F		PI	PI	PI	PI	PI	PI	PI	
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PI	PI	PI F	PI PI	PI	PI	PI	PI	PI	P1	PI	
											 م
											u
70'	443'	23'		70'		23'		63'-9"		4' '-6	
										,	*

	LINE KEY
BOX HDR	FRAMED COLUMN OPENING W/ BOX HEADER
	6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND 26GA VERT. OR HORIZ. PANEL EXTERIOR
	6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND WOOD VENEER BY OTHERS
	6" R-19 INSULATION IN WALL WITH 29GA PANEL BOTH SIDES
	CROSS BEAMS NEED MORE THAN 5' BAY
	COLUMNS
	ZEE BEAMS
TYPICAL COLUMNS	# BEAMS UNLESS NOTED OTHERWISE
COLUMN : MSI = GCI	G - G"X2 1/2" X 1GGA CEE PRIME PAINTED

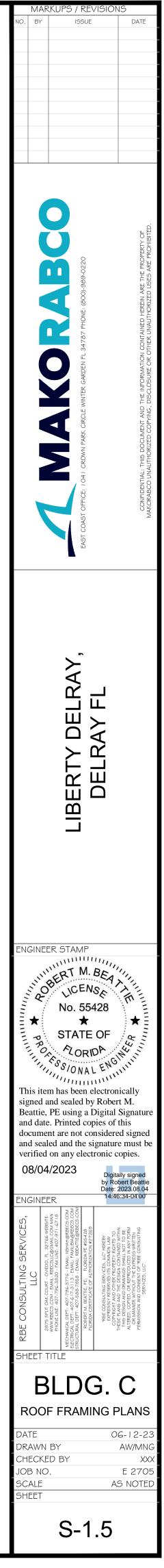
COLUMN : MS2 = 6CI4 - 6"X2 I/2" X I4GA CEE PRIME PAINTED PURLIN : PI = 6ZI6 - 6"X2 I/2" X I6GA ZEE PRIME PAINTED

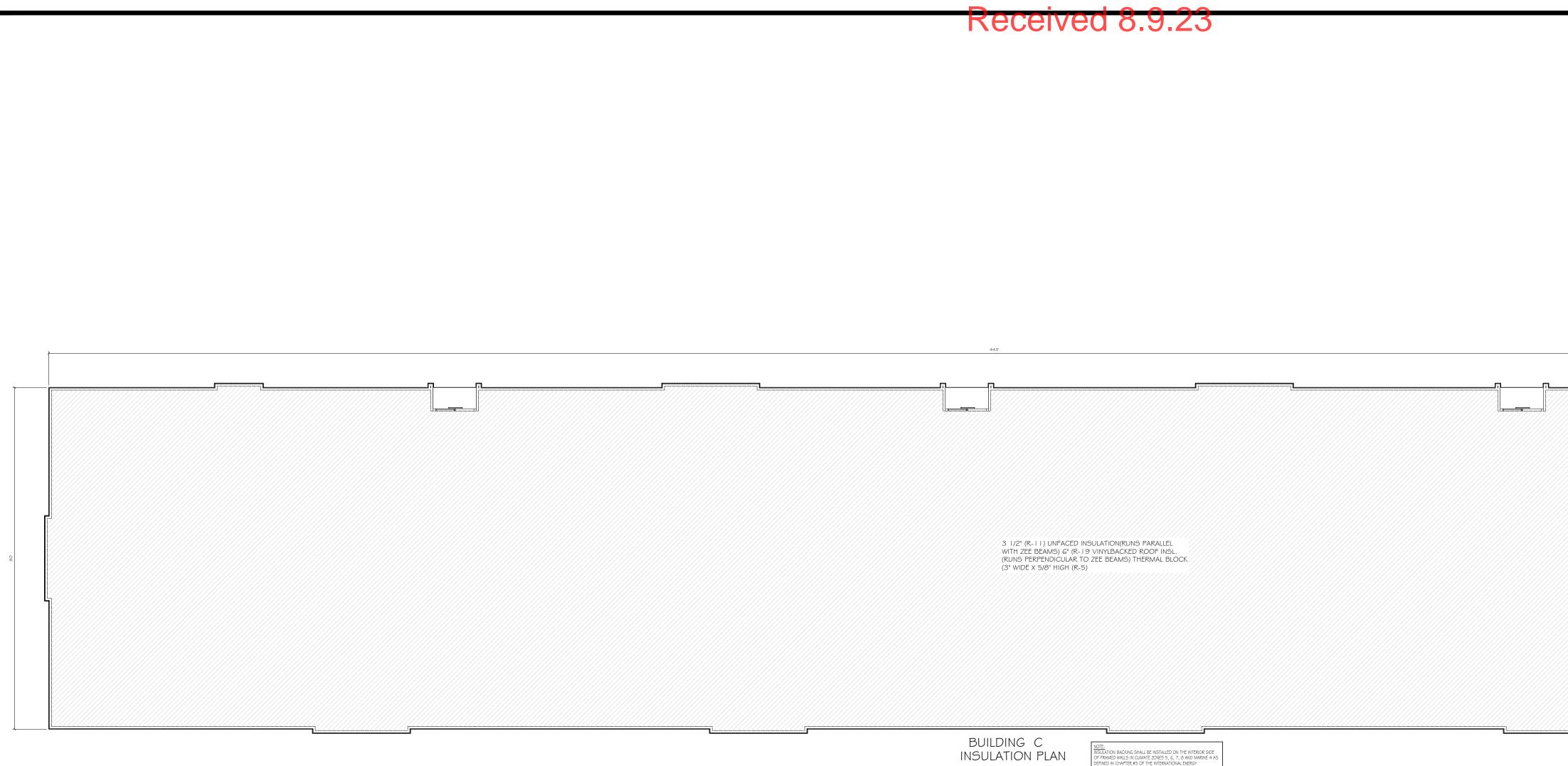


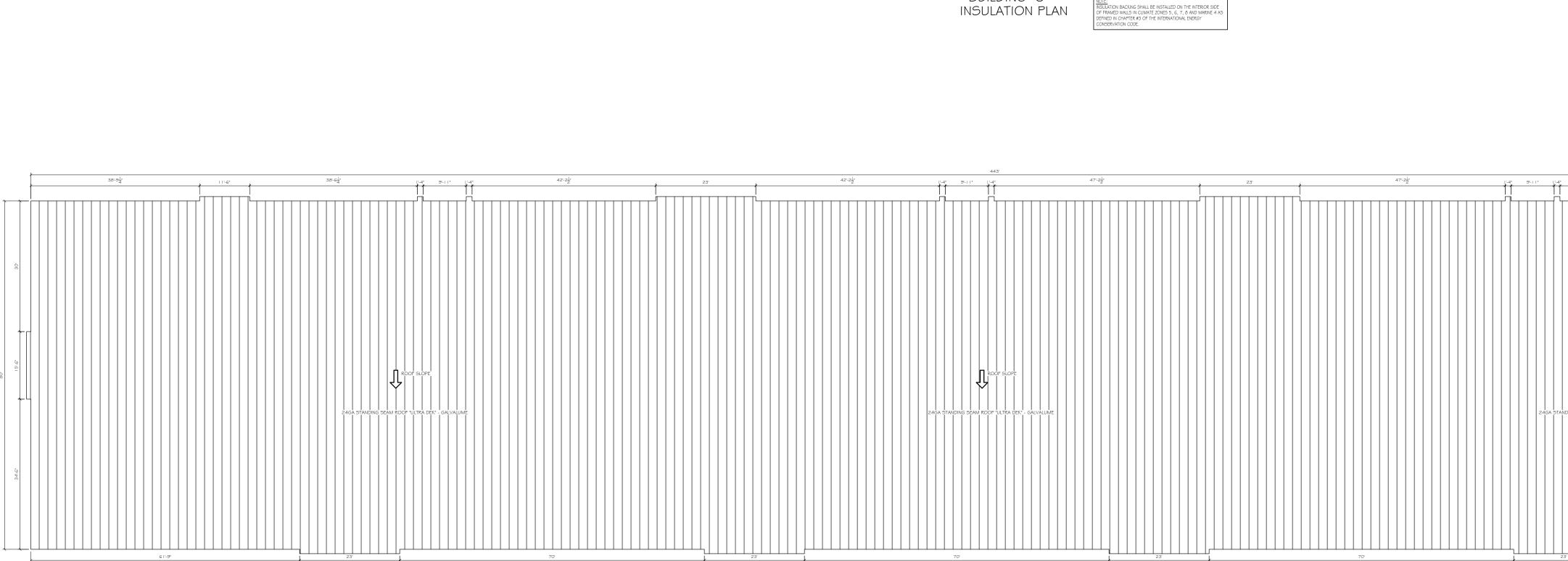


NORTH BLDG. ROOF FRAMING PLAN

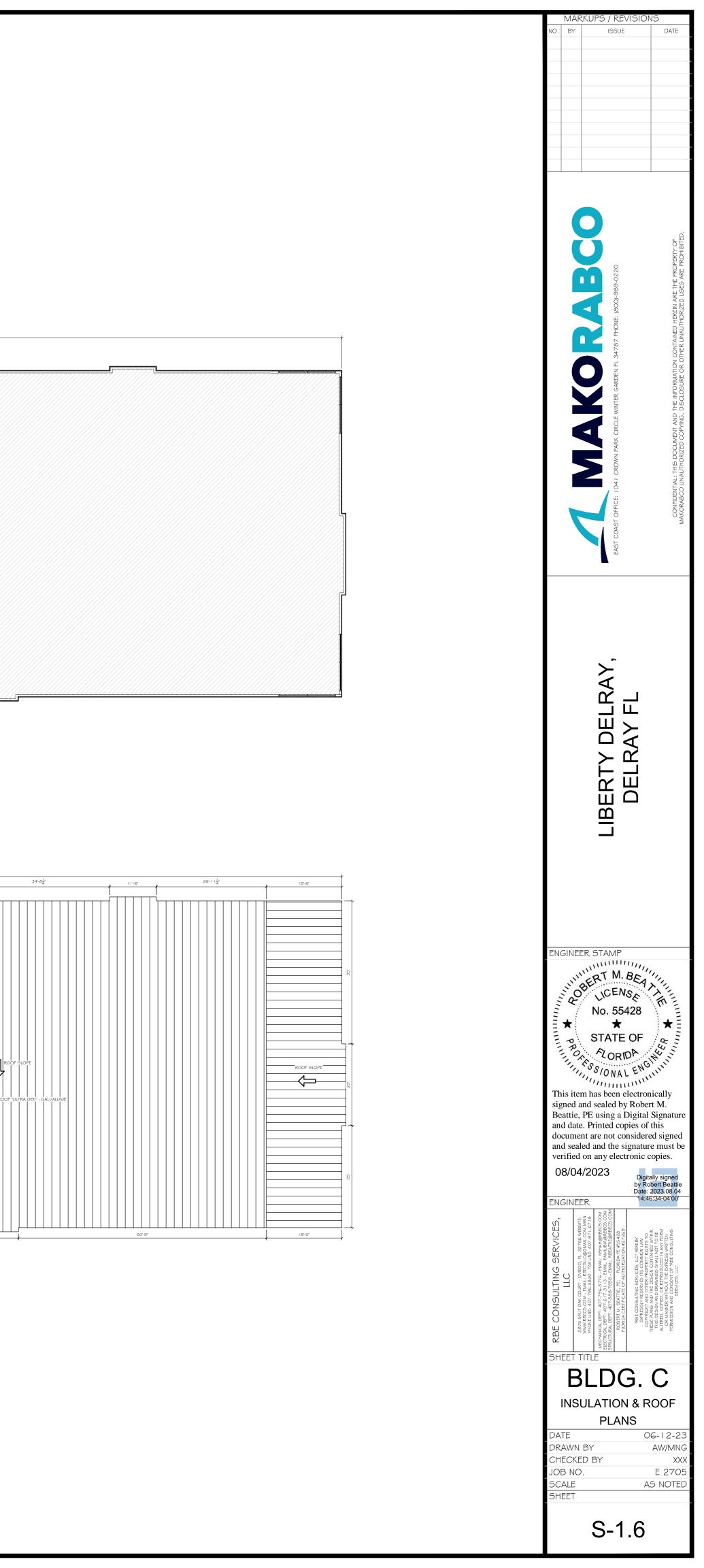
|/8" = |'

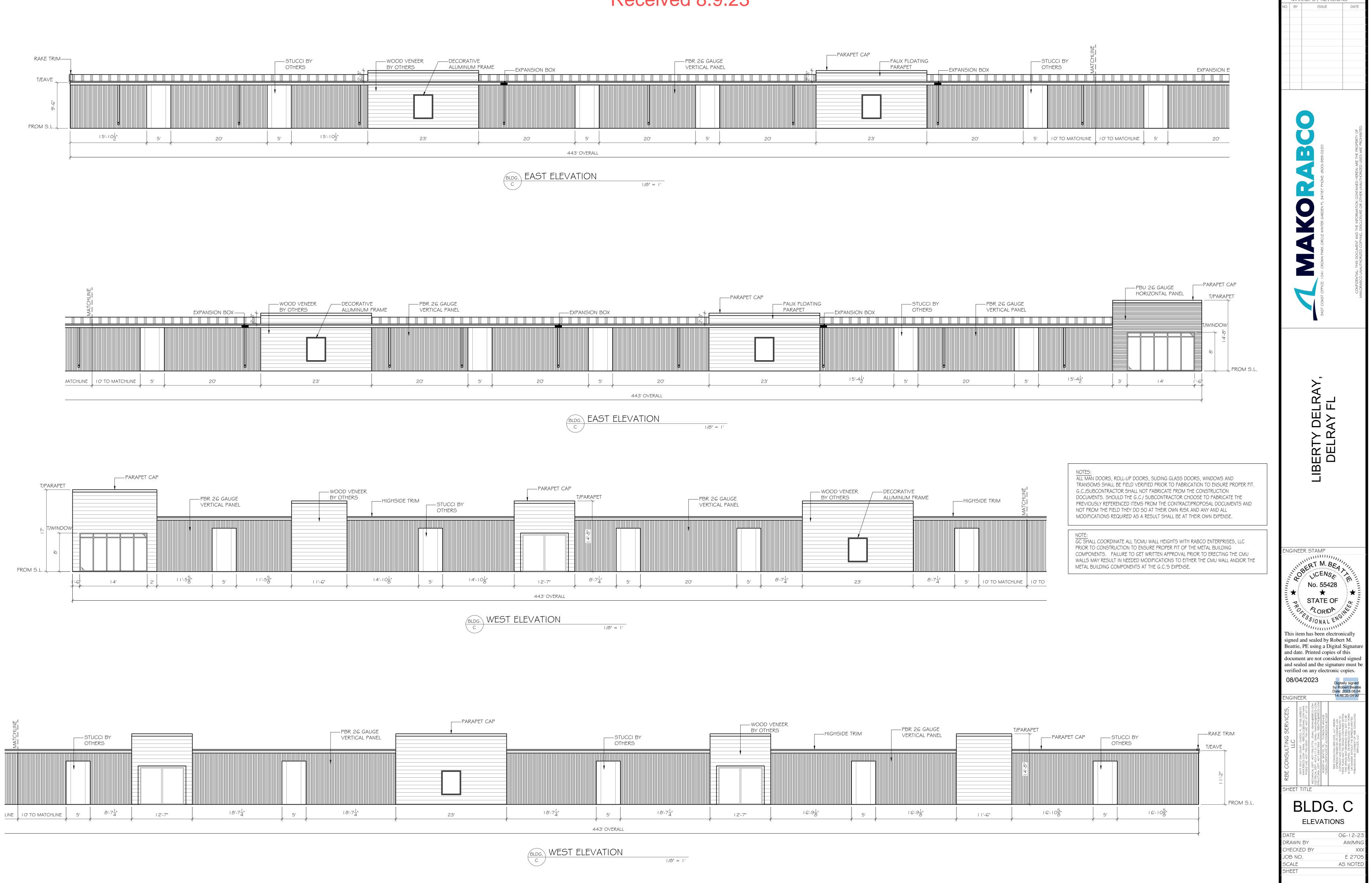






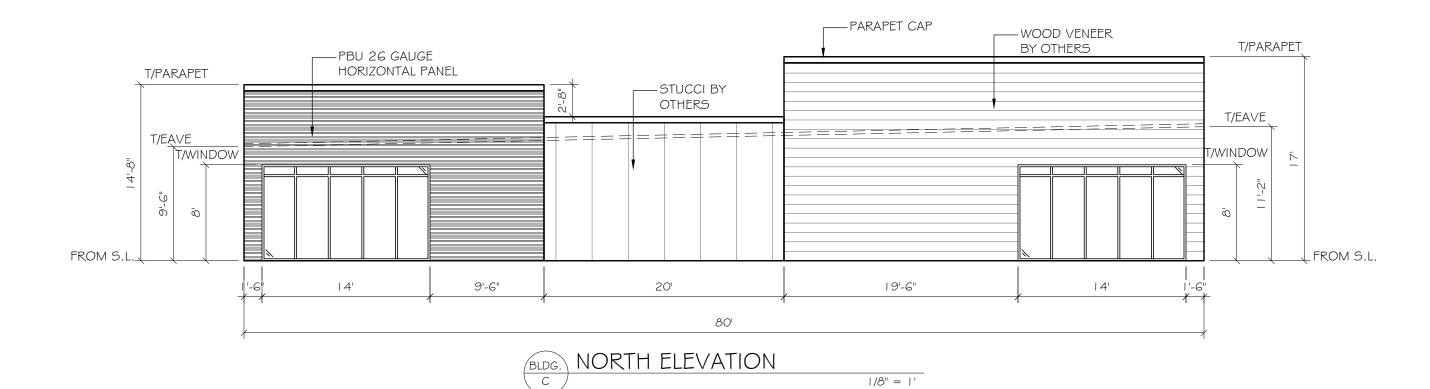
BUILDING D ROOF PLAN

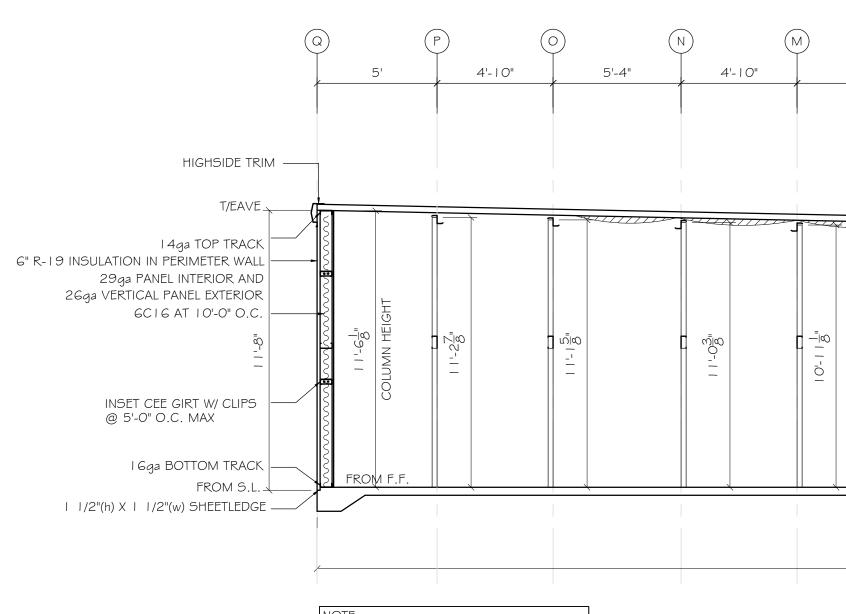




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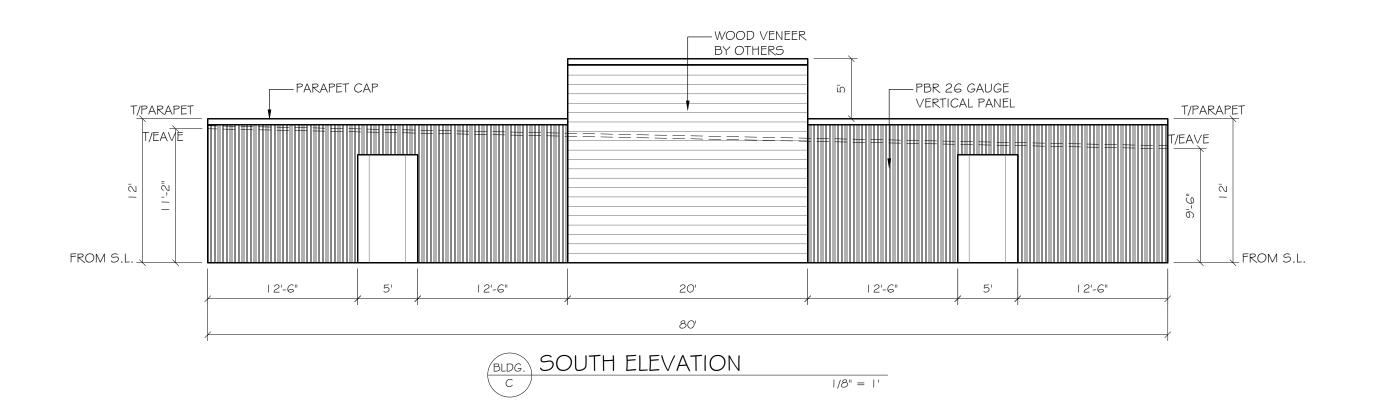
S-1.7





VERTICAL COLUMN DIMENSION IS ACTUAL LENGTH OF CEE FROM F.F. (U.N.O.)





		K (T (G	\bigvee			
5'	5'	5' - 3 1/2" (R-11) UNFA WITH ZEE BEAMS) ((RUNS PERPENDICU (3" WIDE X 5/8" HIG	" (R-19 VINYLBACK LAR TO ZEE BEAMS	ED ROOF INSL.	4'-10"	5'	5' (24ga STANDING SE	4'-10" AM	5'-4"	4'-
	ν Ν Θ Ο -			-0-6 <u>+</u> "			ັ ຫຼືສ ເບິ່ງ -		9 	<u>و</u> آرا ال
			ء Sectic	NC-C		NOT	ES:			

SCALE: |/4" = |'

NOTES:

I. UNLESS NOTED OTHERWISE ON THE PLANS. THE FOLLOWING OUTLINES STANDARD PARTITION HEIGHTS USED FOR DESIGN PURPOSES.

2. NON-CLIMATE CONTROLLED SINGLE STORY BUILDINGS HAVE BEEN DESIGNED BASED ON THE TOP OF ALL INTERIOR PARTITION PANELS BEING HELD TO WITHIN 8" OF THE BOTTOM OF THE ROOF DECK.

3. CLIMATE CONTROLLED SINGLE STORY BUILDING HAVE BEEN DESIGNED BASED ON THE TOP OF ALL INTERIOR PARTITION PANELS BEING HELD TO WITHIN 18" OF THE BOTTOM OF THE ROOF DECK.

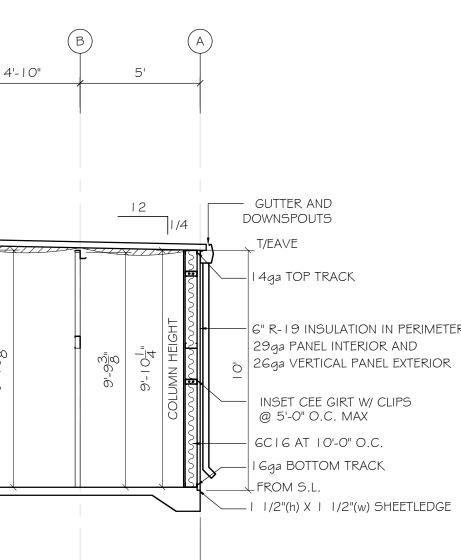
4. CLIMATE CONTROLLED MULTI-STORY BUILDING HAVE BEEN DESIGNED BASED ON THE TOP OF ALL INTERIOR PARTITION PANELS BEING HELD TO WITHIN 18" OF THE BOTTOM OF THE FLOOR/ROOF DECK.

5. SHOULD THE PARTITION PANELS NEED TO BE HELD DOWN FURTHER THAN 18" FROM THE BOTTOM OF THE FLOOR/ROOF DECK, THE REQUEST SHALL BE SUBMITTED IN WRITING AND A FORMAL WRITTEN RESPONSE PROVIDED PRIOR TO FABRICATION AND/OR INSTALLATION.

NOTES

ALL MAN DOORS, ROLL-UP DOORS, SLIDING GLASS DOORS, WINDOWS AND TRANSOMS SHALL BE FIELD VERIFIED PRIOR TO FABRICATION TO ENSURE PROPER FIT. G.C./SUBCONTRACTOR SHALL NOT FABRICATE FROM THE CONSTRUCTION DOCUMENTS. SHOULD THE G.C./ SUBCONTRACTOR CHOOSE TO FABRICATE THE PREVIOUSLY REFERENCED ITEMS FROM THE CONTRACT/PROPOSAL DOCUMENTS AND NOT FROM THE FIELD THEY DO SO AT THEIR OWN RISK AND ANY AND ALL MODIFICATIONS REQUIRED AS A RESULT SHALL BE AT THEIR OWN EXPENSE.

NOTE GC SHALL COORDINATE ALL T/CMU WALL HEIGHTS WITH RABCO ENTERPRISES, LLC PRIOR TO CONSTRUCTION TO ENSURE PROPER FIT OF THE METAL BUILDING COMPONENTS. FAILURE TO GET WRITTEN APPROVAL PRIOR TO ERECTING THE CMU WALLS MAY RESULT IN NEEDED MODIFICATIONS TO EITHER THE CMU WALL AND/OR THE METAL BUILDING COMPONENTS AT THE G.C.'S EXPENSE.



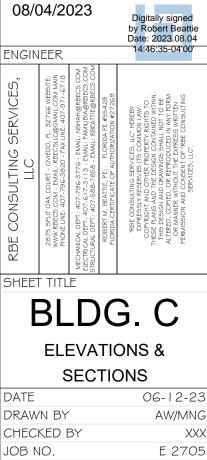
6" R-19 INSULATION IN PERIMETER WALL

 \leq

ISSUE

BERTY DELRAY, DELRAY FL m Ë

ENGINEER STAMP OBERT M. BEA. No. 55428 * * STATE OF TON TORIDA This item has been electronically signed and sealed by Robert M. Beattie, PE using a Digital Signature and date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies. 08/04/2023

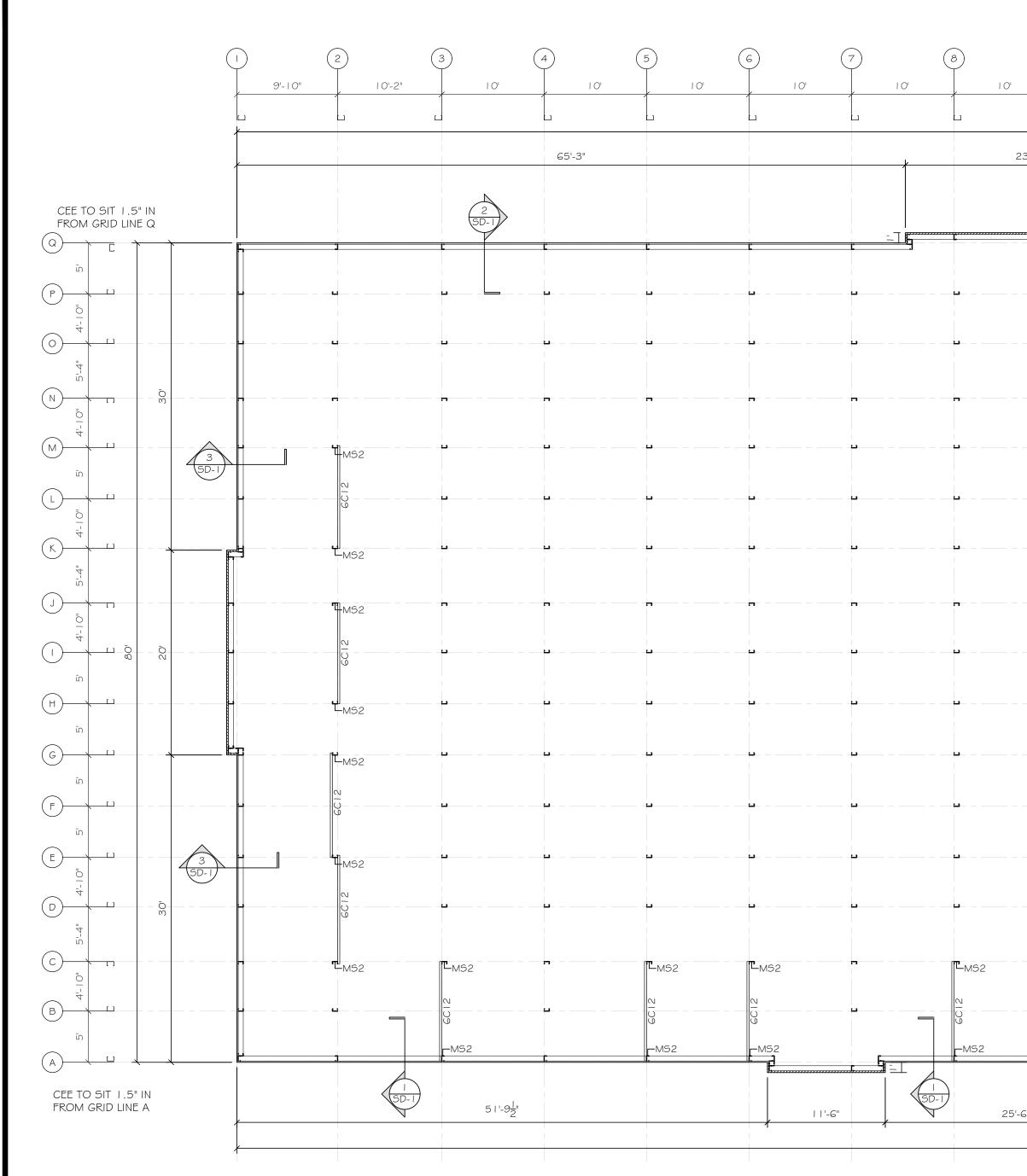


S-1.8

AS NOTED

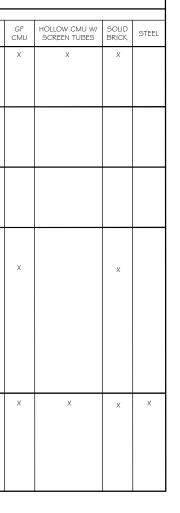
SCALE

SHEET

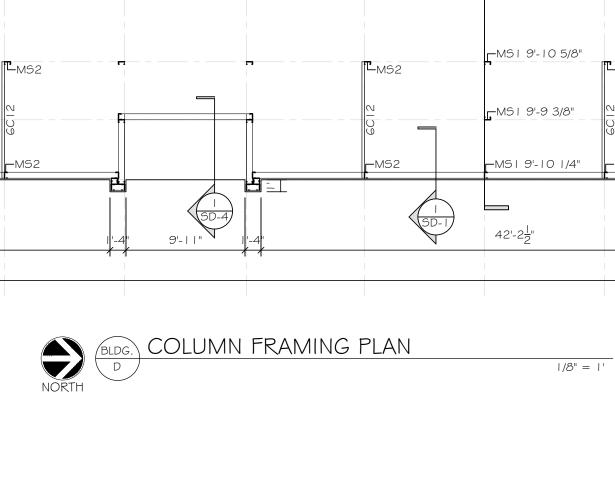


	LINE KEY
BOX HDR	FRAMED COLUMN OPENING W/ BOX HEADER
	6" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND 26GA VERT. OR HORIZ. PANEL EXTERIOR
	G" R-19 INSULATION IN PERIMETER WALL 29GA PANEL INTERIOR AND WOOD VENEER BY OTHERS
	6" R-19 INSULATION IN WALL WITH 29GA PANEL BOTH SIDES
	CROSS BEAMS NEED MORE THAN 5' BAY
	COLUMNS
	ZEE BEAMS
×××××	ROOF STRAPPING
[
TYPICAL COLUMNS \$	BEAMS UNLESS NOTED OTHERWISE
COLUMN : MSI = 6CI	S - 6"X2 1/2" X 16GA CEE PRIME PAINTED
COLUMN : MS2 = 6CI	4 - 6"X2 1/2" X 14GA CEE PRIME PAINTED
PURLIN : PI = 6ZIG -	G"X2 1/2" X 1GGA ZEE PRIME PAINTED

ANCHOR SCH	IEDULE			<u>S AND ANCHORS</u> ENERS SHALL BE INSTALLI	ED PER MANUFACTURE	r's recomme	ENDATIONS AN	D SPEC	CIFICAT	IONS	
BRAND: DeWA	lt (or equal)										
ANCHOR TYPE	ANCHOR NAME	ICC-ES #	ANCHOR DIA.(IN)	MIN BASE MATERIAL THICKNESS (IN)	(MIN. OR NOM) / MAX. OR REF.) EMBEDMENT (IN.)	MIN ANCHOR SPACING (IN)	MIN EDGE / END DISTANCE (IN)	ALLOWA	LW CONC.	E MATERIAL CONC. OVER STEEL DECK	
EPOXY	ACI00+GOLD	ESR-2582	1/2"dia.	4" [CONC]	2-3/4"(MIN) / 6" MAX	2-1/2"	2-1/2"	х	х		
		[CONC]	3/4"dia.	5-3/4" [CONC]	3-1/2"(MIN) / 9" MAX	3-3/4"	3-3/4"				
		ESR-3200	1/2"dia.	8" GF CMU	4" (MIN)	(I) PER CELL	4"				
		[CMU]	3/4"dıa.	8" GF CMU	G" (MIN)	(I) PER CELL	4"				
MECHANICAL		ANICAL POWER STUD	ESR-2818	1/2"dia.	4" [CONC]	2-1/2"(NOM) / 2" (EFF)	4-1/2"	6"	х	х	х
	+5D1	[CONC]	3/4"dia.	6" [CONC]	4"(NOM) / 2" (EFF)	6"	6"				
		ESR-2966	3/8"dia.	8" GF CMU	2-1/2"(MIN)	(I) PER CELL	4"	1			
		[CONC]	1/2"dia.	8" GF CMU	4-3/4"(MIN)	(I) PER CELL	12"				
	POWER STUD	ESR-2502	1/2"dia.	4-1/2" [CONC]	2-1/2"(NOM) / 2" (EFF)	6"	8"	x	x	×	
	+SD2	[CONC]	3/4"dia.	7" [CONC]	4-1/2"(NOM) / 3-3/4" (EFF)		^	^			
			3/8"dia.	2 ¹ / ₂ " [CONC OVER STL DECK]	2-3/8"(NOM) / 2" (EFF)	- /2"	4"	1			
			1/2"dia.	2 ¹ / ₂ " [CONC OVER STL DECK]	2-1/2"(NOM) / 2" (EFF)	8"	4"				
	SCREW-	ESR-3889	1/4"dia.	8" GF [CONC]	2-1/2" (MIN.)	4"	2"				
	BOLT+	[CONC]	1/2"dia.	8" GF [CONC]	3-1/4" (MIN.)	6"	3"				
			1/2"dia.	3" [CONC]	-5/8"(NOM) / .2" (EFF)	- /2"	- /2"	×	x	×	
			3/4"dia.	3" [CONC]	2"(NOM) / 1-1/3" (EFF)	2"	- /2"	Â			
			3/8"dıa.	3" [CONC]	2-1/2"(NOM) / 1-3/4" (EFF)	2-3/4"	I -3/4"	1			
			1/2"dia.	3-3/4" [CONC]	4-1/4"(NOM) / 3.08" (EFF)	3"	I -3/4"	1			
			1/4"dia.	2 ¹ / ₂ " [CONC OVER STL DECK]	-5/8"(NOM) / .2" (EFF)	- /2"	- /2"				
			3/8"dia.	2≟" [CONC OVER STL DECK]	2"(NOM) / 1.33" (EFF)	2"	2"	1			
			1/2"dia.	2 []] [CONC OVER STL DECK]	2-1/2"(NOM) / 1-3/4" (EFF)	2-1/2"	2-1/2"	1			
		ESR-4042	1/2"dia.	8" GF CMU	4-1/4"(MIN)	(I) PER CELL	4"	1			
		[CMU]	3/4"dia.	8" GF CMU	6-1/4"(MIN)	(I) PER CELL	4"	1			
DIRECT FASTEN	CSI SPIRAL DRIVE PINS	ESR-2024	0157.dia	3/1 G" (STEEL)	FASTENER POINT MUST PENETRATE STEEL MEMBER	- /2"	1/2"	х	х	х	
			0157.dia	2-1/4" [CONC]	3/4"(MIN)	4"	3-1/2"				
			0157.dia	3" [CONC]	I "(MIN)	4"	3-1/2"	1			
			0157.dia	3-3/4" [CONC]	I - I /4"(MIN)	4"	3-1/2"	1			
			0157.dia	8" GF CMU	l "(MIN)	4"	3-3/4"	1			
			0157.dia	8" HOLLOW CMU	L"(MIN)	8"	3-3/4"	1			

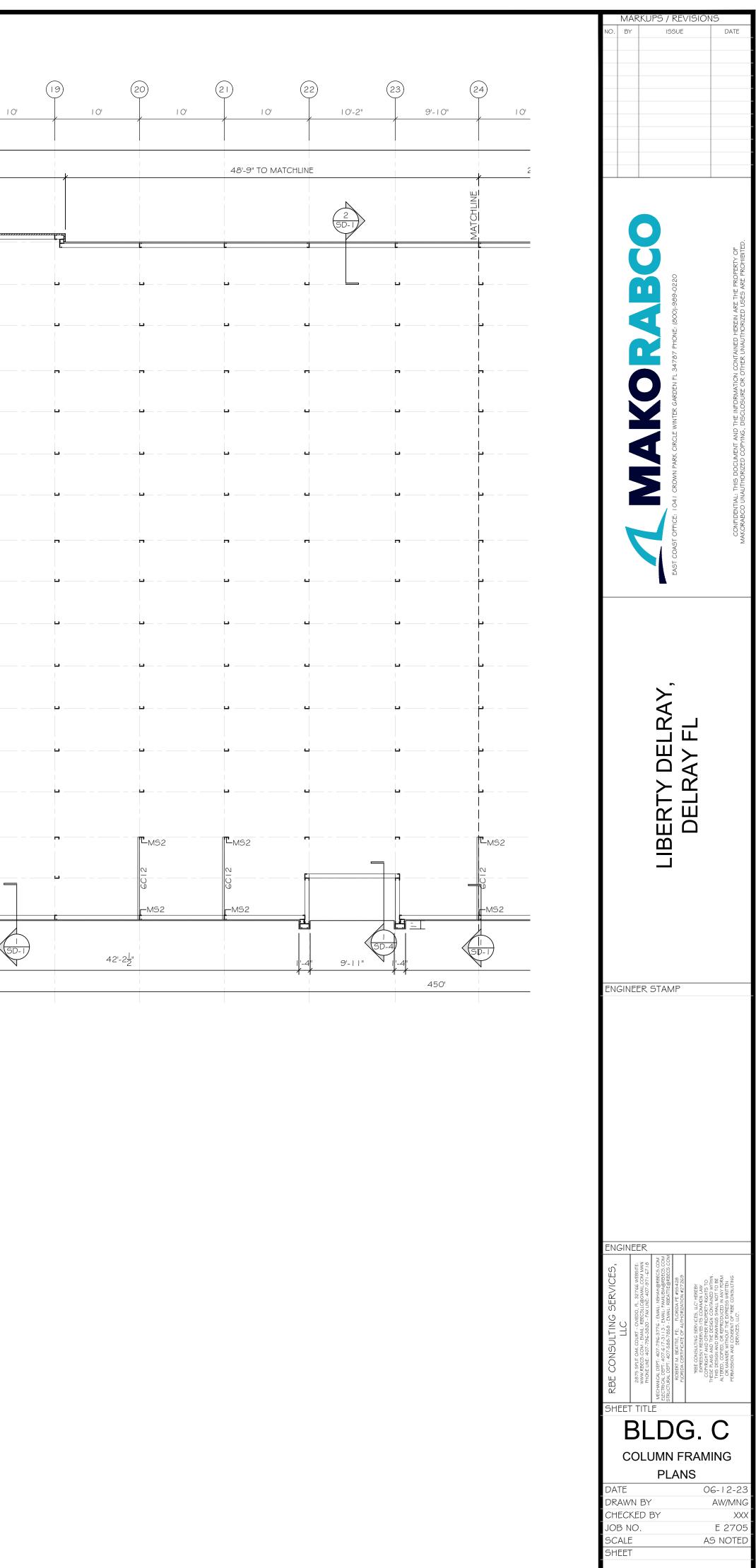


ANCHOR SCHEDULE UNLESS NOTED OTHERWISE
BOTTOM FLOOR (MULTI)
ASE TRACK INSTALLATION /2"X3" DeWALT SCREW BOLT+ (OR EQUAL) SPACING 3' O.C. (1 3/4" EFFECTIVE EMBEDMENT)
ER INSTALLATION /2"X3" DeWALT SCREW BOLT+ (OR EQUAL) @ EA. PIER (1 3/4" EFFECTIVE EMBEDMENT)
EAVY STEEL DECK SUPPORT ANGLE (1/4" THICK OR GREATER) (TO CMU) '4"XG" DeWALT SREW BOLT+ (OR EQUAL) 2' O.C. (5" MIN. EMBEDMENT) (4" MIN. FROM EDGE OF CMU)
/4" MIN. EXPANSION ANGLE INSTALLATION (TO CMU)) 3/4"XG" DeWALT SREW BOLT+ (OR EQUAL) (5" MIN. EMBEDMENT) (4" MIN. FROM EDGE OF CMU)
SS STEEL COLUMN BASE PLATE INSTALLATION SE DeWALT AC200+ EPOXY TO INSTALL. DIAMETER TO MATCH ANCHOR IN COLUMN SCHEDULE X 12" DNG THREADED ROD WITH LEVELING NUTS (7" MIN. EMBEDMENT)



						.20				
			\downarrow					6) (1		8)
μ 	10'	l 0'-2"	9'-10"	450' OVERALL	10'	, 10' 		, I O'	10'	10'
23'	ł				70'			ł	23	3'
				([2 5D-1				
	E E	-	2		MSI 11'-6 1/8"					Ľ
		L		 	MSI II'-2 7/8"	.		• •		
		L		 	, MSI '- 5/8"				L	- -
		-		.	MSI II'-0 3/8"		.	.		
	ے ا	-	<u>ل</u> ے		MSI 10'-11 1/8"	'	.	.	۵	
		-		.	MSI 10'-9 7/8"		· · · · · · · · · · · · · · · · · · ·	•	-	
		_			MSI 10'-8 5/8"	-				
	 	F		n	MSI 10'-7 3/8"			.		
		u			MSI 10'-6 1/8"	 				.
		-	.		MSI 10'-4 7/8"			<u>. </u>	· · · · · · · · · · · · · · · · · · ·	
		L			MSI 10'-3 5/8"			<u></u>	u	
		u	p	u	MSI 10'-2 3/8"	u	u	u	u	ບ
	.	e		.	_MS1_10'-1_1/8"	u	⊔	.	u	<u>.</u>
				• • • • • • • • • • • • • • • • • • • •	MSI 9'-11 7/8"		••	•		· · · · ·
	LMS2		.	L _{MS2}	MSI 9'-10 5/8"	См52	L _{MS2}		L _{MS2}	LM52
	9 9 9 9			ec 5	_MS1_9'-9_3/8"	ec 12 	eC 2 60		I	ec 15
	-M52			ГMS2	MSI 9'-10 1/4"	MS2			_M52 F	MS2
5'-6"		-4" 9'- "	⊫ '-4 '	I SD-1	42'-2 <u>1</u> "		C 1	23'		I SD-1
	×	* · · ·	<i>Ҟ</i>				*		*	





S-2.2

29 (30	0 31 32 10' 10' 10'	33 34 35 10' 10' 10'-2"		38 39 40 10' 10' 10'	(41) (42) (43) 9'-10" 10'-2"	(44) (45 2'-2" 0'- 0"	5)
		443' OVERALL					Ŀ
23'		'-4" 9'- '	34'-8	4 II'-6"		C-	C-
						B- IG-IO"	
						□ <u>-</u> 0 <u>-</u> 0 - 0	
							C-
							C-
						$22'-4\frac{1}{2}"$	
	70'		23'	63'-1	9" 	14'	C-





cer	veu	1.	17.23	

|/8" = |'

	ENING.
COLUMNS (S	SEE COL. SCHEDULE ON
•	FOR ADDITIONAL INFO.)
COLUMN MARK	COLUMN SIZE
C44-4	HSS4x4x1/4
C44-6	HSS4x4x3/8
C44-8	HSS4x4x1/2
C66-4	HSS6x6x1/4
C66-6	HSS6x6x3/8
C66-8	HSS6x6x1/2
C88-4	HSS8x8x1/4
C88-6	HSS8x8x3/8
C88-8	HSS8x8x1/2
C88-10	HSS8x8x5/8
CI0I0-4	HSS Ox Ox /4
C1010-6	HSS Ox Ox3/8
C1010-8	HSS10x10x1/2
C 0 0- 0	HSS Ox Ox5/8
CB-1	8x16 CONC. BM.
TC-I	8x24 CONC. COL.
TC-2	6x 6x8 CONC. COL.
TC-3	

W-BEAM SCHEDULE

(TO BE VERIFIED BY G.C./STEEL FABRICATOR PRIOR TO CONSTRUCTION / FABRICATION.) BOTTOM & MIDDLE FLOORS TOP OF BEAMS = BOTTOM OF METAL DECK (4.5" BELOW ELEVATED SLAB, U.N.O.) INFILL FRAMING BELOW BEAMS MAY BE REQUIRED AT ANY OPENINGS, U.N.O.

R=_K

R=_K

W_X_

W_X_

ΒI B2

TYP. BEAM HEIGHTS

TOP FLOOR

PRELIMINARY

